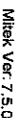


3000



ALL CONN. FRAMING TO CONFORM WITH PART 8 OF D.E.C. 2012. TRUSSES THAT CROSS OVER OTHER TRUSSES SHALL BE LOCATED AT LEAST 1" ABOVE THE TRUSS THEY CROSS WITH A VERTICAL TO THE TRUSS UNDERNEATH AT EACH CROSS PT. TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 8'

12" B/C EXT. BELL CURVE BY OTHERS RSD HEEL-D DROP (2" x 6" FASCIA) - FM ASPHALT SHINGLES 2" x 6" BRICKWORK PIGGYBACK TRS PURLINS BY OTHERS

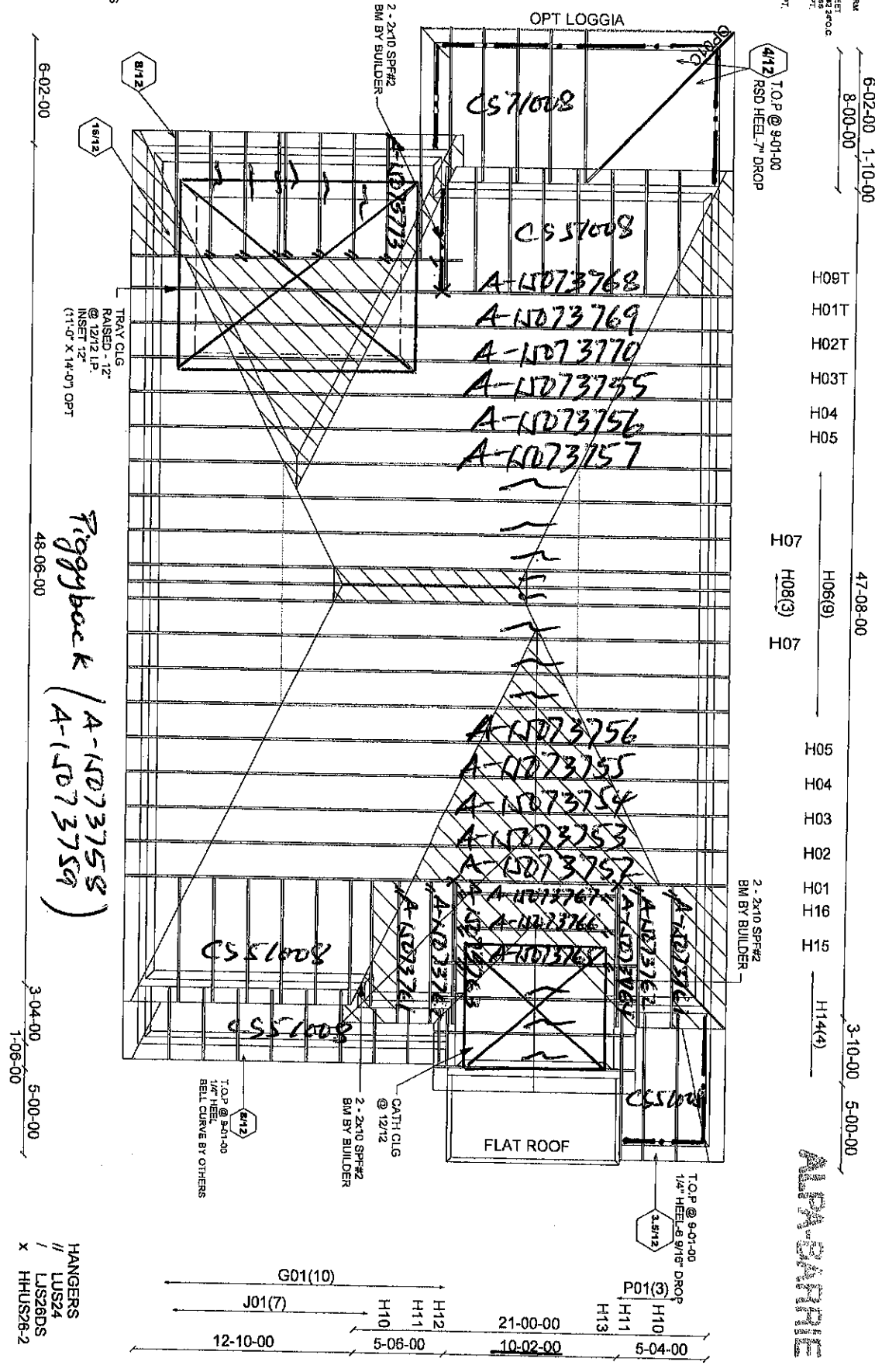


Job Track: 40297
Layout ID: 253621
Plan Log: 79711

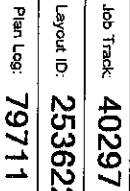
Builder / Location: GOLD PARK / KLEINBURG
Project: HUNTINGTON & NASHVILLE
Date: 4/28/2015 Designer: chenye

Model / Elevation: 42-1 / A OPT 2ND W/TRAY
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALFA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALFA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALFA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

CONVENTIONAL FRAMING BY OTHERS
Mitek Ver. 7.5.0



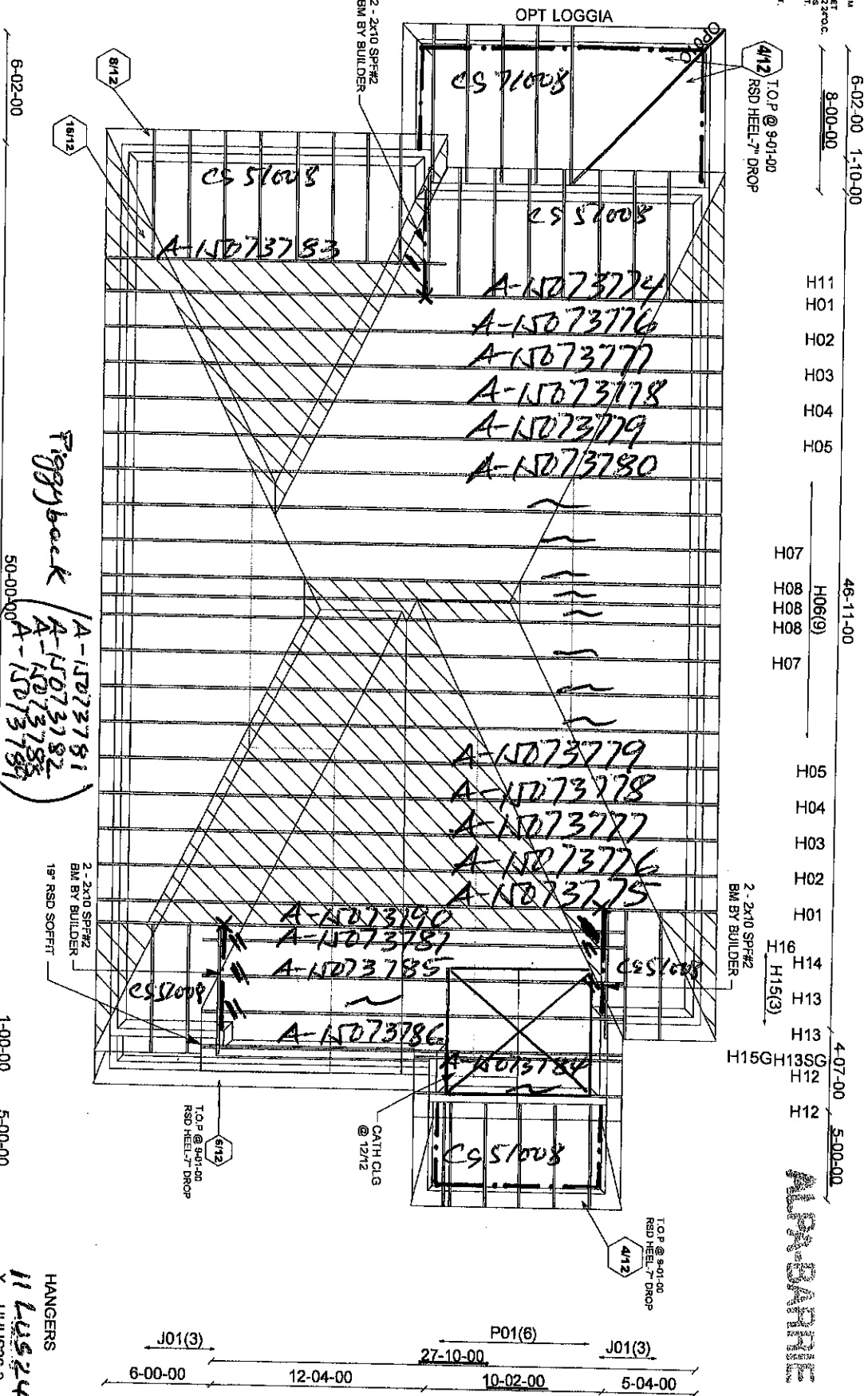
ALFA ROOF TRUSSES



Builder / Location:	GOLD PARK / KLEINBURG		
Project:	HUNTINGTON & NASHVILLE		
Date:	4/29/2015	Designer:	chancie

Model / Elevation:	42-1	/ B	STD OR OPT 2ND FLR	Meek Ver. 7.5.0
--------------------	------	-----	--------------------	-----------------

12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 8" FASCIA) - FM
ASPHALT SHINGLES
2" x 6" BRG/BRICK
PIGGYBACK TRS
PURLINS BY OTHERS



ALL CONV. FRAMING TO CONFORM WITH PART 5 OF O.B.C. 2012. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPS @ 24" O.C. WITH A VERT POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

4/12 T.O.P @ 9-01-05
BSN HEEI 7" DBOB

H11
H01
H02
H03
H04
H05

H07		
H08	H06(9)	46-11-00
H08		
H08		
H07		

H05
H04
H03
H02
H01

H15(3)

H13SG
H12
H12

THE
FEDERAL
BUREAU OF
INVESTIGATION
OF THE
DEPARTMENT OF JUSTICE
WASHINGTON, D. C. 20535

 CONVENTIONAL FRAMING BY OTHERS

HANGERS
116524
X HHUS26-2
/LJS26DS

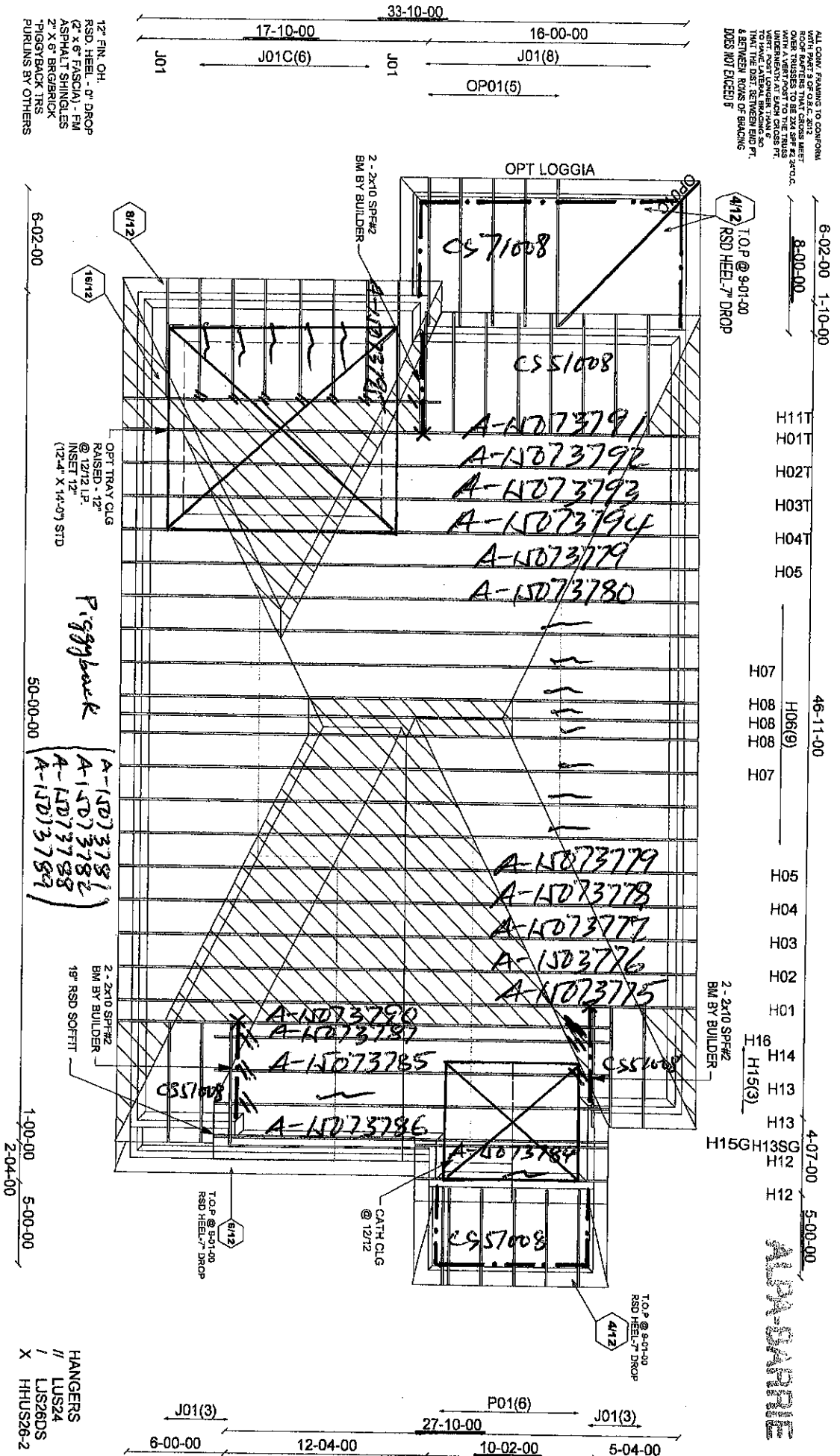


Job Track: 40297
Layout ID: 253623
Plan Log: 79711

Builder / Location: GOLD PARK / KLEINBURG
Project: HUNTINGTON & NASHVILLE
Date: 4/29/2015
Designer: cheryle

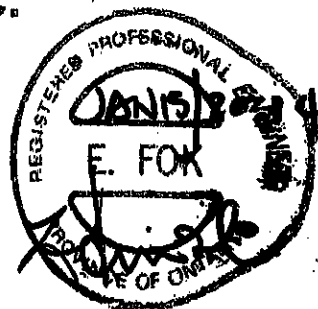
Model / Elevation: 42-1 / B OPT TRAY
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Miner Ver 7.5.0



Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

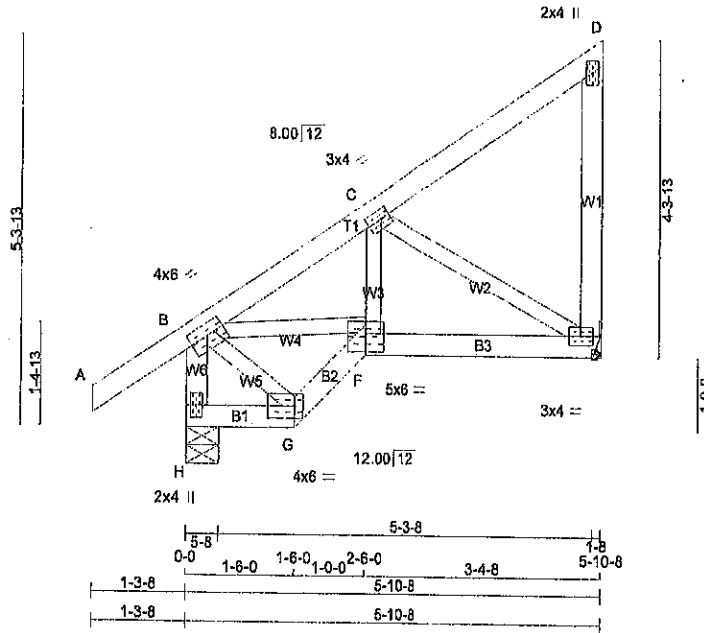
1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

Alpa Roof Truss, Maple



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
E - G	2x4	DRY	No.2
F - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TMV-t	MT20	2.0	4.0		
E	BMVW-t	MT20	3.0	4.0		
F	BBW-t	MT20	5.0	6.0	3.00	3.00
G	BBW-t	MT20	4.0	6.0	2.00	4.50
H	BMV-t	MT20	2.0	4.0		

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per CBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	365	0	355	0	6-8	1-8
E	258	0	258	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181/0	0/0	0/0	0/0	67/0	0/0
E	182	123/0	0/0	0/0	0/0	59/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
H-B	-342/0	0.0	0.0	0.04 (1)	7.81	B-G	0/18
A-B	0/27	-70.5	-70.5	0.09 (1)	10.00	F-C	0/57
B-C	-287/0	-70.5	-70.5	0.10 (1)	6.25	C-E	-284/0
C-D	-12/0	-70.5	-70.5	0.10 (1)	6.25	B-F	0/238
E-D	-93/0	0.0	0.0	0.03 (1)	7.81		
H-G	0/0	-17.5	-17.5	0.01 (4)	10.00		
G-F	0/20	-17.5	-17.5	0.01 (4)	10.00		
F-E	0/248	-17.5	-17.5	0.06 (4)	10.00		

TOTAL WEIGHT = 6 X 30 = 179 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(56 % OF 23.0 P.S.F. G.S.I. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.10 (B-C:1), BC=0.08 (E-F:4), WB=0.07 (C-E:1), SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.260 inches

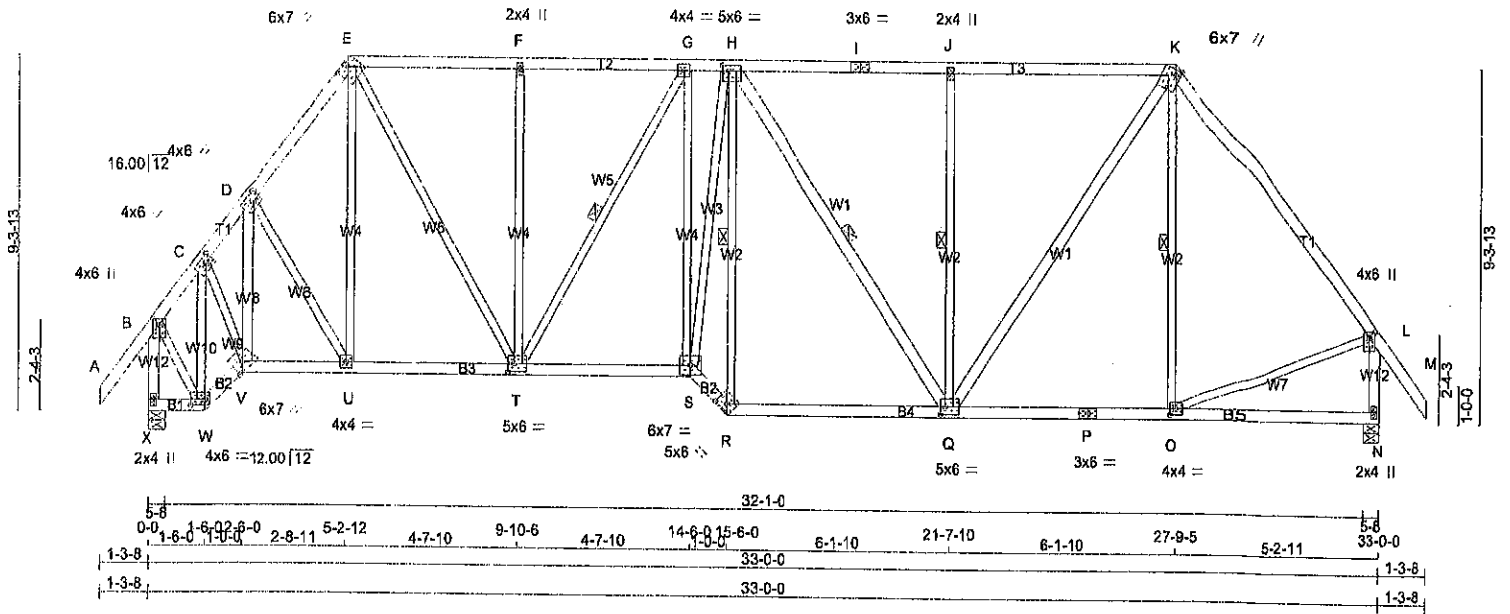
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
 JSI METAL= 0.10 (B) (INPUT = 1.00)



A-14073795

Alpa Roof Truss, Maple



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - E	2x4	DRY No.2	SPF
E - I	2x4	DRY No.2	SPF
I - K	2x4	DRY No.2	SPF
K - M	2x4	DRY No.2	SPF
X - B	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
X - W	2x4	DRY No.2	SPF
W - V	2x4	DRY No.2	SPF
V - S	2x4	DRY No.2	SPF
S - R	2x4	DRY No.2	SPF
R - P	2x4	DRY No.2	SPF
P - N	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
H - Q	2x4	DRY No.2	SPF
Q - K	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	1.75	2.00
C	TMVW-i	MT20	4.0	6.0	1.75	2.25
D	TMVW-i	MT20	4.0	6.0	2.00	2.50
E	TTWV-h	MT20	6.0	7.0	1.60	3.75
F	TMVW-w	MT20	2.0	4.0		
G	TMVW-i	MT20	4.0	4.0		
H	TMVWV-i	MT20	5.0	6.0	1.75	2.00
I	TS-i	MT20	3.0	6.0		
J	TMVW-w	MT20	2.0	4.0		
K	TTWV+m	MT20	6.0	7.0	Edge	1.50
L	TMVW+p	MT20	4.0	6.0	1.75	2.00
N	BMV1+p	MT20	2.0	4.0		
O	BMVW-i	MT20	4.0	4.0	2.00	1.75
P	BS-i	MT20	3.0	6.0		
Q	BMVWV-i	MT20	5.0	6.0	2.50	1.50
R	BBW-h	MT20	5.0	6.0	2.00	3.25
S	BBWV-i	MT20	6.0	7.0		
T	BMVWV-i	MT20	5.0	6.0	2.50	2.00
U	BMVW-i	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
X	1553	0	1553	0
N	1548	0	1548	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
X	COMBINED		753 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0
N	COMBINED		750 / 0	0 / 0	0 / 0	0 / 0	336 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-T, H-R, H-Q, J-Q, K-O.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-70.5 -70.5 0.10 (1)	10.00	W-C	-1102 / 0	0.29 (1)	
B-C	-896 / 0	-70.5 -70.5 0.10 (1)	8.25	C-V	0 / 824	0.19 (1)	
C-D	-1255 / 0	-70.5 -70.5 0.06 (1)	5.68	V-D	-258 / 0	0.09 (1)	
D-E	-1349 / 0	-70.5 -70.5 0.09 (1)	5.51	D-U	0 / 24	0.01 (1)	
E-F	-1269 / 0	-70.5 -70.5 0.26 (1)	5.41	U-E	0 / 63	0.02 (4)	
F-G	-1289 / 0	-70.5 -70.5 0.26 (1)	5.41	E-T	0 / 941	0.21 (1)	
G-H	-1475 / 0	-70.5 -70.5 0.25 (1)	5.10	T-F	-376 / 0	0.51 (1)	
H-I	-1232 / 0	-70.5 -70.5 0.44 (1)	6.21	T-G	-411 / 0	0.24 (1)	
I-J	-1232 / 0	-70.5 -70.5 0.44 (1)	5.21	S-G	0 / 296	0.07 (1)	
J-K	-1232 / 0	-70.5 -70.5 0.44 (1)	5.21	S-H	0 / 1092	0.25 (1)	
K-L	-1202 / 0	-70.5 -70.5 0.45 (1)	5.35	R-H	-1266 / 0	0.72 (1)	
L-M	0 / 40	-70.5 -70.6 0.10 (1)	10.00	H-Q	-195 / 0	0.12 (1)	
X-B	-1539 / 0	0.0 0.0 0.20 (1)	6.83	Q-J	-463 / 0	0.28 (1)	
N-L	-1512 / 0	0.0 0.0 0.19 (1)	6.88	Q-K	0 / 914	0.15 (1)	
X-W	0 / 0	-17.5 -17.5 0.01 (4)	10.00	O-K	-155 / 34	0.09 (1)	
W-V	0 / 871	-17.5 -17.5 0.11 (1)	10.00	B-W	0 / 822	0.18 (1)	
V-U	0 / 783	-17.5 -17.5 0.16 (1)	10.00	O-L	0 / 785	0.17 (1)	
U-T	0 / 797	-17.5 -17.5 0.17 (1)	10.00				
T-S	0 / 1471	-17.5 -17.5 0.28 (1)	10.00				
S-R	0 / 1875	-17.5 -17.5 0.30 (1)	10.00				
R-Q	0 / 1340	-17.5 -17.5 0.29 (1)	10.00				
Q-P	0 / 718	-17.5 -17.5 0.21 (4)	10.00				
P-O	0 / 716	-17.5 -17.5 0.21 (4)	10.00				
O-N	0 / 0	-17.5 -17.5 0.12 (4)	10.00				

TOTAL WEIGHT = 199 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
 CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
 ALLOWABLE DEFL.(TL)= L/360 (1.10")
 CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.45 (K-L:1), BC=0.30 (R-S:1), WB=0.72 (H-R:1), SS=0.20 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1656

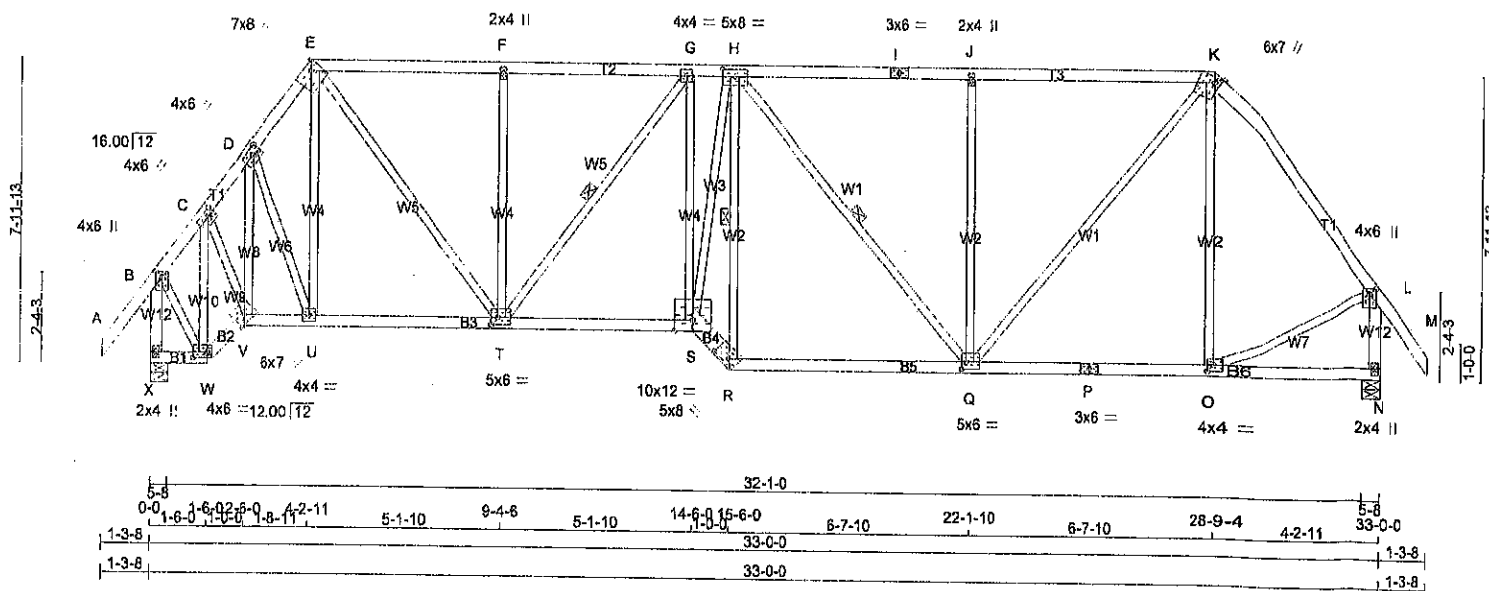
PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (X) (INPUT = 0.80)
 JSI METAL= 0.30 (L) (INPUT = 1.00)



A-15073794



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - E	2x4	No.2	SPF
E - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
K - M	2x4	DRY	No.2
X - B	2x4	DRY	No.2
N - L	2x4	DRY	No.2
X - W	2x4	DRY	No.2
W - V	2x4	DRY	No.2
V - S	2x4	DRY	No.2
S - R	2x6	DRY	No.2
R - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2

ALL WEB
EXCEPT

DRY; SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV+p	MT20	4.0	6.0	1.75 2.00
C	TMWV-i	MT20	4.0	6.0	1.75 2.25
D	TMWV-i	MT20	4.0	6.0	2.00 2.50
E	TTWV-h	MT20	7.0	8.0	Edge 2.75
F	TMWV+w	MT20	2.0	4.0	
G	TMWV-i	MT20	4.0	4.0	
H	TMWVW-i	MT20	5.0	8.0	2.25 4.00
I	TS-i	MT20	3.0	6.0	
J	TMWV+w	MT20	2.0	4.0	
K	TTWV+m	MT20	6.0	7.0	Edge 1.50
L	TMWV+p	MT20	4.0	6.0	1.75 2.00
N	BMV-i+p	MT20	2.0	4.0	
O	BMWV-i	MT20	4.0	4.0	2.00 1.75
P	BS-i	MT20	3.0	6.0	
Q	BMWVW-i	MT20	5.0	6.0	2.25 1.50
R	BBW-h	MT20	5.0	8.0	1.75 5.00
S	BBWV-i	MT20	10.0	12.0	Edge
T	BMWVW-i	MT20	5.0	6.0	2.25 2.00
U	BMWV-i	MT20	4.0	4.0	
V	BBWV-h	MT20	6.0	7.0	Edge
W	BBW-i	MT20	4.0	6.0	1.75 4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
X	1553	0	1553	0	0	5-8	1-11
N	1548	0	1548	0	0	5-8	1-11

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
X	1091	753 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0
N	1089	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X. N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-T, H-R, H-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	W- C	-1108 / 0	0.28 (1)	
B-C	-896 / 0	-70.5	-70.5	0.10 (1)	6.25	C- V	0 / 803	0.18 (1)	
C-D	-1268 / 0	-70.5	-70.5	0.06 (1)	5.00	V- D	-250 / 0	0.08 (1)	
D-E	-1357 / 0	-70.5	-70.5	0.06 (1)	5.54	D- U	0 / 65	0.01 (1)	
E-F	-1483 / 0	-70.5	-70.5	0.34 (1)	5.00	E- U	0 / 47	0.02 (4)	
F-G	-1484 / 0	-70.5	-70.5	0.33 (1)	5.00	U- T	0 / 1116	0.25 (1)	
G-H	-1789 / 0	-70.5	-70.5	0.29 (1)	4.68	T- F	-416 / 0	0.35 (1)	
H-I	-1423 / 0	-70.5	-70.5	0.64 (1)	4.79	T- G	-499 / 0	0.24 (1)	
I-J	-1423 / 0	-70.5	-70.5	0.84 (1)	4.79	S- G	0 / 326	0.07 (1)	
J-K	-1423 / 0	-70.5	-70.5	0.55 (1)	4.79	S- H	0 / 1337	0.30 (1)	
K-L	-1168 / 0	-70.5	-70.5	0.27 (1)	5.57	R- H	-1481 / 0	0.80 (1)	
L-M	0 / 40	-70.5	-70.5	0.10 (1)	10.00	H- Q	-256 / 0	0.19 (1)	
X-B	-1539 / 0	0.0	0.0	0.20 (1)	6.83	Q- J	-524 / 0	0.64 (1)	
N-L	-1522 / 0	0.0	0.0	0.18 (1)	6.86	Q- K	0 / 1094	0.25 (1)	
						O- K	-217 / 9	0.26 (1)	
X-W	0 / 0	-17.5	-17.5	0.01 (4)	10.00	B- W	0 / 825	0.19 (1)	
W-V	0 / 873	-17.5	-17.5	0.11 (1)	10.00	O- L	0 / 778	0.16 (1)	
V-U	0 / 777	-17.5	-17.5	0.17 (1)	10.00				
U-T	0 / 801	-17.5	-17.5	0.19 (1)	10.00				
T-S	0 / 1784	-17.5	-17.5	0.34 (1)	10.00				
S-R	0 / 2218	-17.5	-17.5	0.28 (1)	10.00				
R-Q	0 / 1589	-17.5	-17.5	0.35 (1)	10.00				
Q-P	0 / 706	-17.5	-17.5	0.24 (4)	10.00				
P-O	0 / 706	-17.5	-17.5	0.24 (4)	10.00				
O-N	0 / 0	-17.5	-17.5	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.0	PSF
TOTAL LOAD				=	31.0 PSF

TOTAL LOAD	=	31.0	PSE
------------	---	------	-----

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.55 (J-K:1), BC=0.35 (Q-R:1), WB=0.64 (J-Q:1), SSI=0.22 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LSBEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1856

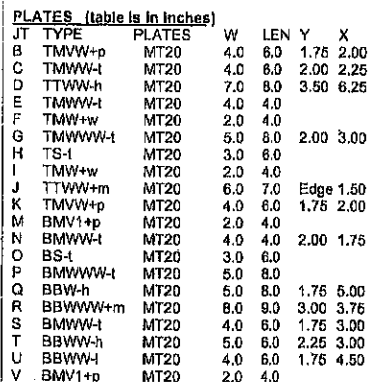
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (X) (INPUT = 0.90)
JSI METAL= 0.30 (S) (INPUT = 1.00)



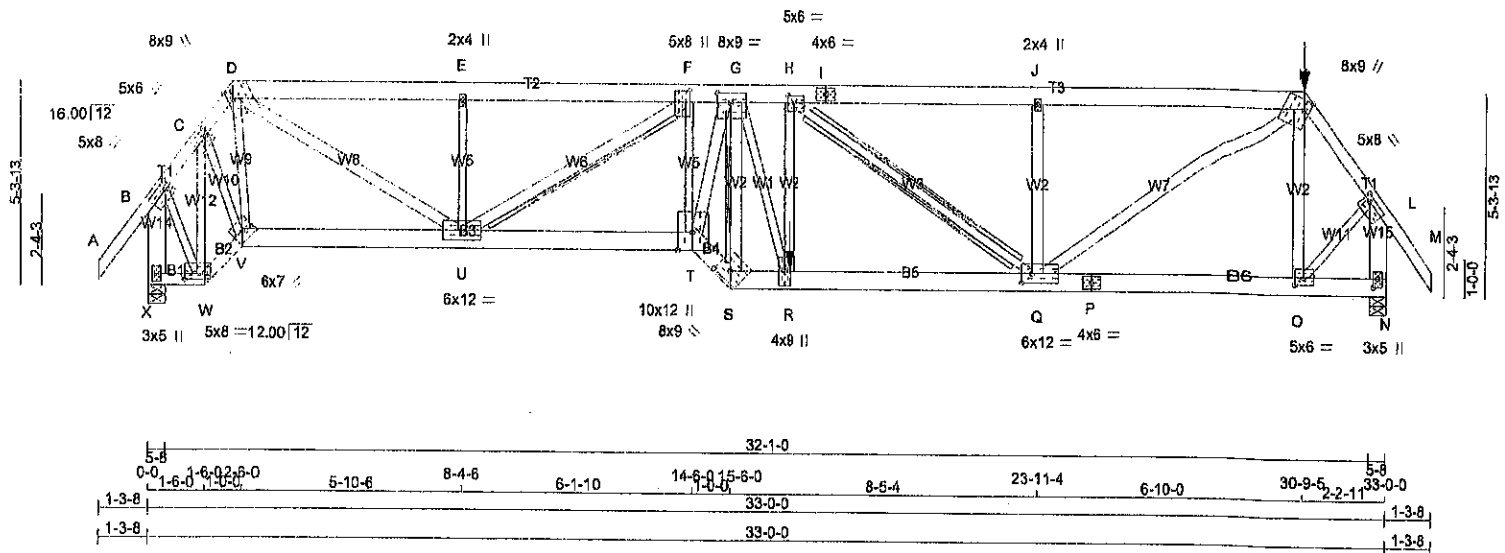
A-NO73793



CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	U-C	-1112 / 0	0.29 (1)
B-C	-694 / 0	-70.5	-70.5 0.10 (1)	6.25	C-T	0 / 761	0.17 (1)
C-D	-1278 / 0	-70.5	-70.5 0.07 (1)	5.85	T-D	-174 / 0	0.09 (1)
D-E	-1806 / 0	-70.5	-70.5 0.44 (1)	4.51	D-S	0 / 1408	0.32 (1)
E-F	-2244 / 0	-70.5	-70.5 0.48 (1)	4.10	S-E	-867 / 0	0.42 (1)
F-G	-2243 / 0	-70.5	-70.5 0.36 (1)	4.18	E-R	0 / 800	0.14 (1)
G-H	-1691 / 0	-70.5	-70.5 0.68 (1)	4.30	R-F	-65 / 5	0.03 (5)
H-I	-1691 / 0	-70.5	-70.5 0.88 (1)	4.30	R-G	0 / 1665	0.37 (1)
I-J	-1690 / 0	-70.5	-70.5 0.68 (1)	4.30	Q-G	-1918 / 0	0.52 (1)
J-K	-1148 / 0	-70.5	-70.5 0.15 (1)	5.77	G-P	-326 / 0	0.21 (1)
K-L	0 / 40	-70.5	-70.5 0.10 (1)	10.00	P-I	-563 / 0	0.41 (1)
V-B	-1639 / 0	0.0	0.0 0.20 (1)	6.63	N-J	0 / 1352	0.30 (1)
M-K	-1536 / 0	0.0	0.0 0.29 (1)	6.64	N-J	-289 / 0	0.22 (1)
					B-U	0 / 829	0.19 (1)
V-U	0 / 0	-17.5	-17.5 0.01 (4)	10.00	N-K	0 / 792	0.18 (1)
U-T	0 / 877	-17.5	-17.5 0.11 (1)	10.00			
T-S	0 / 782	-17.5	-17.5 0.24 (4)	10.00			
S-R	0 / 1805	-17.5	-17.5 0.39 (1)	10.00			
R-Q	0 / 2689	-17.5	-17.5 0.32 (1)	10.00			
Q-P	0 / 1931	-17.5	-17.5 0.42 (1)	10.00			
P-O	0 / 678	-17.5	-17.5 0.27 (4)	10.00			
O-N	0 / 678	-17.5	-17.5 0.27 (4)	10.00			
N-M	0 / 0	-17.5	-17.5 0.12 (4)	10.00			

JSI GRIP= 0.81 (V) (INPUT = 0.90)
JSI METAL= 0.34 (G) (INPUT = 1.00)

A-1073792



TOTAL WEIGHT = 208 lb									
[M]									
DESIGN CRITERIA									
*** SPECIAL LOADS ANALYSIS ***									
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.									
LOADS WERE DERIVED FROM USER INPUT									
NO FURTHER MODIFICATIONS WERE MADE									
SPECIFIED LOADS:									
TOP CH. LL = 21.0 PSF									
DL = 3.0 PSF									
BOT CH. LL = 0.0 PSF									
DL = 7.0 PSF									
TOTAL LOAD = 31.0 PSF									
SPACING = 24.0 IN. C/C									
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
GIRDER TYPE: CPrimeHip									
SIDE SETBACK = 2-2-11									
END SETBACK = 5-10-8									
END WALL WIDTH = 0-0									
CORNER FRAMING TYPE: CONVENTIONAL									
END JACK TYPE: CONVENTIONAL									
APPLIED TO FRONT SIDE									
- ADDTL. LOADS BASED ON 55 % OF GSL.									
LOADS APPLIED TO FIRST 15-10-8 OF SPAN									
MEASURED FROM THE RIGHT.									
*** NON STANDARD GIRDER ***									
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
- CSA 086-09									
- TPIC 2011									
DESIGN ASSUMPTIONS									
- OVERHANG NOT TO BE ALTERED OR CUT OFF.									
(55 % OF 23.0 P.S.F. G.S.L. PLUS									
8.4 P.S.F. RAIN LOAD EQUALS									
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEFL.(LL)= L/360 (1.10")									
CALCULATED VERT. DEFL.(LL)= L/989 (0.20")									
ALLOWABLE DEFL.(TL)= L/360 (1.10")									
CALCULATED VERT. DEFL.(TL)= L/680 (0.58")									
CSI: TC=0.76 (J-K:1), BC=0.71 (Q-R:4), WB=0.93 (G-S:1), SST=0.39 (J-K:1)									
DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.00									
COMP=1.00 SHEAR=1.00 TENS=1.00									
COMPANION LIVE LOAD FACTOR = 0.50									
AUTOSOLVE HEELS OFF									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT									
CONTINUED ON PAGE 2									



253623

H01T

QUANTITY

PLY

JOB DESC.

DRWG NO.

Alpa Roof Truss, Maple

TRUSS DESC.

Version 7.620 S Apr 15 2016 MITek Industries, Inc. Thu Jul 18 15:17:12 2016 Page 2
ID: VysbEZneA3WozYuArtGwQlZLtyt-S9ngl0k4b2vMGZ5zUgVtgbln6V0_lqvW6JRFJAyxTB5**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
P	BS-I	MT20	4.0	8.0		
Q	BMWWW-I	MT20	8.0	12.0	3.00	3.50
R	BMWWW-H	MT20	4.0	8.0		
S	BBWW-h	MT20	8.0	9.0	3.00	4.25
T	BBWW+p	MT20	10.0	12.0	Edge	
U	BMWWWW-I	MT20	6.0	12.0		
V	BBWWW-h	MT20	6.0	7.0		
W	BBWW-I	MT20	5.0	8.0	1.75	6.25
X	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.**HANGERS NOTES**

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 114.2 lbs FACTORED DOWN AT 30-9-5
ON TOP CHORD, AND 1821.4 lbs FACTORED
DOWN AT 17-1-8 ON BOTTOM CHORD. DESIGN
FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES

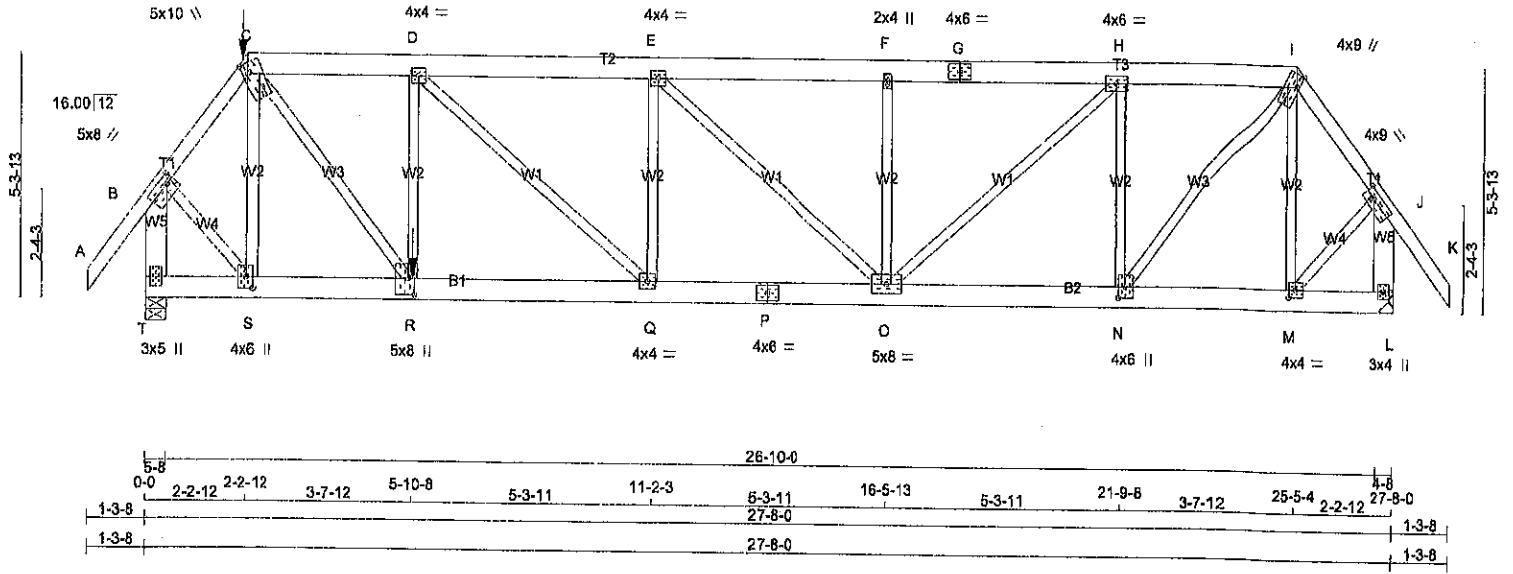
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.85 (T) (INPUT = 1.00)

A-15073791(2)



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 C - G 2x6 DRY No.2 SPF
 G - I 2x6 DRY No.2 SPF
 I - K 2x4 DRY No.2 SPF
 T - B 2x6 DRY No.2 SPF
 L - J 2x6 DRY No.2 SPF
 T - P 2x8 DRY No.2 SPF
 P - L 2x8 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWW-1	MT20	5.0	8.0	2.00 2.00
C	TTWW+m	MT20	5.0	10.0	2.75 1.50
D	TMWW-1	MT20	4.0	4.0	
E	TMWW-1	MT20	4.0	4.0	
F	TMWW-1	MT20	2.0	4.0	
G	TS-1	MT20	4.0	6.0	
H	TMWW-1	MT20	4.0	6.0	
I	TTWW+m	MT20	4.0	9.0	3.25 0.75
J	TMWW-1	MT20	4.0	9.0	1.75 3.00
L	BMV1+p	MT20	3.0	4.0	
M	BMWW-1	MT20	4.0	4.0	2.00 1.75
N	BMWW-1	MT20	4.0	6.0	3.00 1.75
O	BMWW-1	MT20	5.0	8.0	
P	BS-1	MT20	4.0	6.0	
Q	BMWW-1	MT20	4.0	4.0	
R	BMWW-1	MT20	5.0	6.0	4.25 1.75
S	BMWW-1	MT20	4.0	6.0	3.00 1.50
T	BMV1+p	MT20	3.0	5.0	

HANGERS NOTES
 1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
T	2783	0	2783	0	5-8	3-9
L	1858	0	1858	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-13

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
T	2081	829 / 0	0 / 0	0 / 0	0 / 0	1232 / 0	0 / 0
L	1193	867 / 0	0 / 0	0 / 0	0 / 0	526 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH	MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)
			LC1	LC2				
A-B	0 / 40	-70.5	-70.5	0.12 (1)	10.00	S-C	-778 / 0	0.30 (1)
B-C	-2040 / 0	-70.5	-70.5	0.13 (1)	4.59	C-R	0 / 2483	0.68 (4)
C-D	-2698 / 0	-138.7	-138.7	0.18 (1)	4.88	R-D	-805 / 134	0.23 (1)
D-E	-2885 / 0	-70.5	-70.5	0.18 (1)	4.78	D-Q	-230 / 228	0.24 (4)
E-F	-2482 / 0	-70.5	-70.5	0.16 (1)	5.07	Q-E	-35 / 283	0.11 (4)
F-G	-2482 / 0	-70.5	-70.5	0.15 (1)	5.08	E-O	-518 / 0	0.53 (4)
G-H	-2482 / 0	-70.5	-70.5	0.15 (1)	5.08	O-F	-376 / 0	0.14 (1)
H-I	-1698 / 0	-70.5	-70.5	0.11 (1)	6.07	C-H	0 / 1199	0.31 (1)
I-J	-1166 / 0	-70.5	-70.5	0.09 (1)	5.90	N-H	-1149 / 0	0.44 (1)
J-K	0 / 40	-70.5	-70.5	0.12 (1)	10.00	N-I	0 / 1524	0.39 (1)
T-B	-2795 / 0	0.0	0.0	0.26 (1)	6.20	M-I	-471 / 0	0.18 (1)
L-J	-1861 / 0	0.0	0.0	0.15 (1)	7.81	B-S	0 / 1669	0.40 (1)
T-S	0 / 0	-34.5	-34.5	0.12 (4)	10.00	M-J	0 / 880	0.23 (1)
S-R	0 / 1206	-34.5	-34.5	0.28 (4)	10.00			
R-Q	0 / 2698	-17.5	-17.5	0.46 (4)	10.00			
Q-P	0 / 2865	-17.5	-17.5	0.43 (1)	10.00			
P-O	0 / 2865	-17.5	-17.5	0.43 (1)	10.00			
O-N	0 / 1598	-17.5	-17.5	0.26 (1)	10.00			
N-M	0 / 883	-17.5	-17.5	0.14 (1)	10.00			
M-L	0 / 0	-17.5	-17.5	0.04 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-115	-115	-	FRONT	VERT	TOTAL
R	5-10-6	-1346	-1346	-	FRONT	VERT	DEAD

TOTAL WEIGHT = 184 lb [M]

DESIGN CRITERIA
 *** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
 APPLIED TO FRONT SIDE
 - ADDTL. LOADS BASED ON 55 % OF GSL.
 LOADS APPLIED TO FIRST 5-10-8 OF SPAN
 MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
 ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.92")
 CALCULATED VERT. DEFL.(LL) = L/899 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.92")
 CALCULATED VERT. DEFL.(TL) = L/899 (0.18")

CSI: TC=0.26 (B-T:1), BC=0.48 (Q-R:4),
 WB=0.68 (C-R:4), SSI=0.19 (C-D:1)

DOL LUMBER=0.96 NAIL=0.96 L.S BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



A-15073790

253622

H16

QUANTITY

PLY

JOB DESC.

DRWG NO.

Alpha Roof Truss, Maple

TRUSS DESC.

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ID:VysbEZneA3WozYuArtGwQzLtyt-dyPcCQOyOFE1s0NH4n n4NioIVCE8yKWK1RaPuyxTrl

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.6 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD, AND 1507.8 lbs FACTORED DOWN AT 5-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

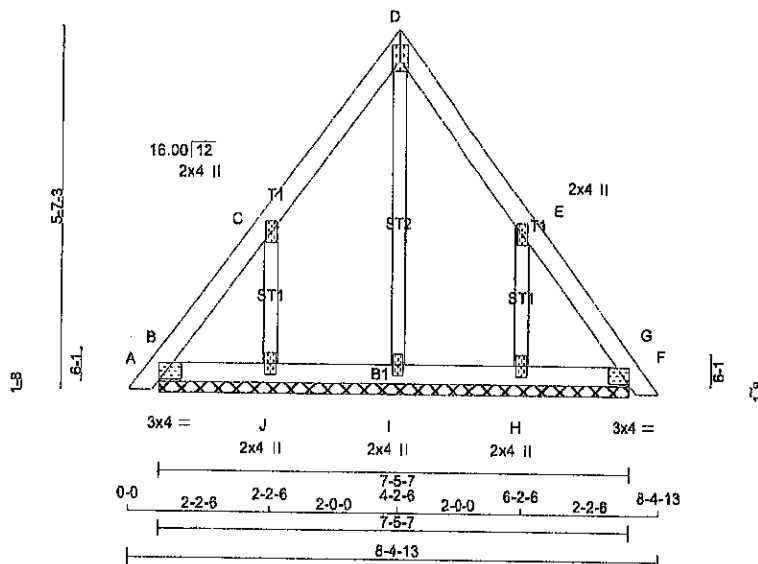
JSI GRIP = 0.87 (R) (INPUT = 0.90)
JSI METAL = 0.62 (P) (INPUT = 1.00)

A-15073790(2)

Alpa Roof Truss, Maple

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ID:VysbEZneA3WozYuArtGwQizLtyt-8lrD_47Kdx6AEsp4VV4 TYYAAiv6z0PgJN5N0sSyxTrv Scale = 1:36.0



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.76
C	TTW+w	MT20	2.0	4.0	
D	TTW+p	MT20	3.0	5.0	2.00 Edge
E	TTW+w	MT20	2.0	4.0	
F	TMB1-J	MT20	3.0	4.0	1.50 2.76
H, I, J	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX (LC)	UNBRAC LENGTH	FR-TO
A-B	0 / 9	-70.5	-70.5 0.01 (1)	10.00	I-D	-58 / 0	0.02 (1)		
B-C	-38 / 0	-70.5	-70.5 0.04 (1)	6.25	J-C	-153 / 0	0.03 (1)		
C-D	-51 / 0	-70.5	-70.5 0.04 (1)	6.25	H-E	-153 / 0	0.03 (1)		
D-E	-51 / 0	-70.5	-70.5 0.04 (1)	6.25					
E-F	-38 / 0	-70.5	-70.5 0.04 (1)	6.25					
F-G	0 / 9	-70.5	-70.5 0.01 (1)	10.00					
B-J	0 / 31	-17.5	-17.5 0.02 (4)	10.00					
J-I	0 / 23	-17.5	-17.5 0.02 (4)	10.00					
I-H	0 / 23	-17.5	-17.5 0.02 (4)	10.00					
H-F	0 / 31	-17.5	-17.5 0.02 (4)	10.00					

TOTAL WEIGHT = 32 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (D-E:1), BC=0.02 (B-J:4), WB=0.03 (C-J:1), SSI=0.04 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1867	822	2284

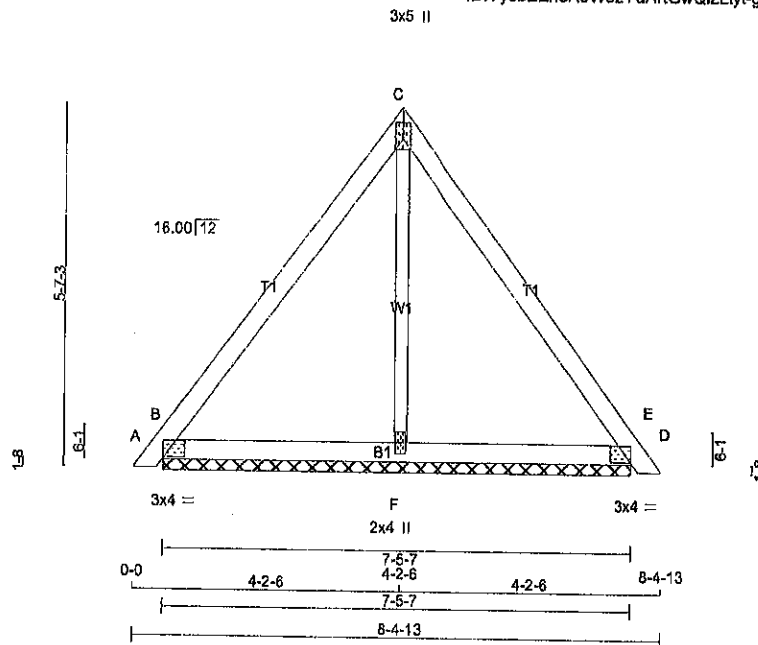
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.80)
JSI METAL= 0.05 (C) (INPUT = 1.00)



A-15073789

**LUMBER****N. L. G. A. RULES**

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			SPF

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-i	MT20	3.0	4.0	1.50 2.75
C	TTW+p	MT20	3.0	5.0	2.00 Edge
D	TMB1-i	MT20	3.0	4.0	1.50 2.75
F	BMW1+u	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
B	263	0	253	0
D	263	0	253	0
F	200	0	200	0

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	175	136 / 0	0 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
D	175	136 / 0	0 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
F	146	70 / 0	0 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MEMB. FORCE (LBS)
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO
A-B	0 / 0	-70.5 -70.5	0.01 (1)	10.00
B-C	-89 / 0	-70.5 -70.5	0.17 (1)	6.25
C-D	-89 / 0	-70.5 -70.5	0.17 (1)	6.25
D-E	0 / 0	-70.5 -70.5	0.01 (1)	10.00
B-F	0 / 53	-17.5 -17.5	0.07 (4)	10.00
F-D	0 / 53	-17.5 -17.5	0.07 (4)	10.00

TOTAL WEIGHT = 3 X 28 = 85 lb

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (B-C:1), BC=0.07 (D-F:4), WB=0.05 (C-F:1), SSI=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

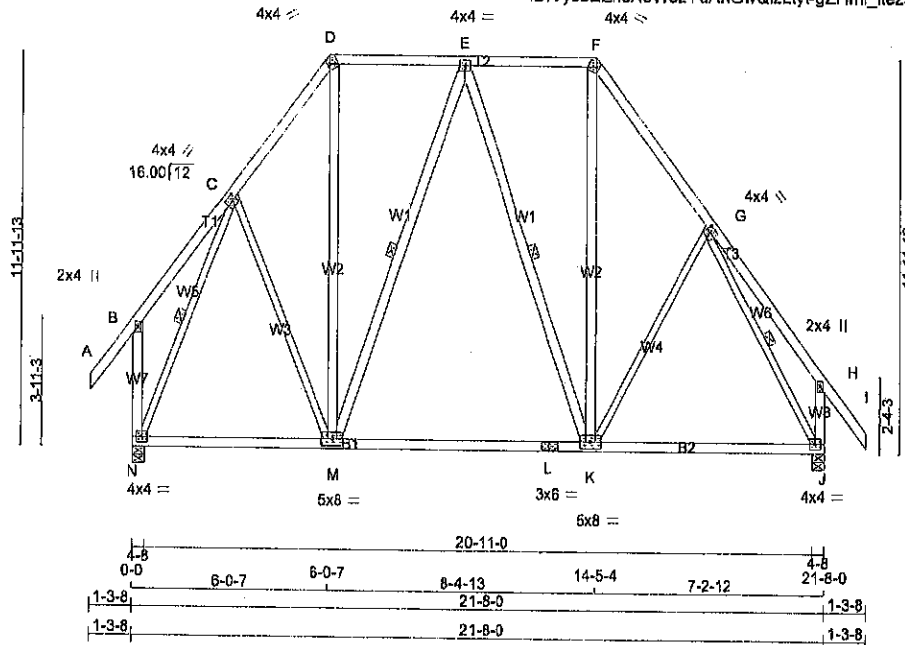
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.80)
JSI METAL= 0.06 (D) (INPUT = 1.00)



A-15073788



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
C - M	2x3	DRY	No.2
K - G	2x3	DRY	No.2
N - C	2x3	DRY	No.2
G - J	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-I	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0	1.75	1.00
E	TMWW-I	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	1.75	1.00
G	TMWW-I	MT20	4.0	4.0	1.75	1.25
H	TMV+p	MT20	2.0	4.0		
J	BMVW-I	MT20	4.0	4.0		
K	BMWW-I	MT20	5.0	8.0		
L	BS-I	MT20	3.0	6.0		
M	BMWW-I	MT20	5.0	8.0		
N	BMVW-I	MT20	4.0	4.0		

NOTES: (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	1085	0	1085	0	4-8	1-8
J	1082	0	1082	0	4-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
N	785	514 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
J	783	514 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-M, E-K, C-N, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO		
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C-M	0 / 94
B-C	0 / 18	-70.5	-70.5	0.10 (1)	10.00	M-D	0 / 303
C-D	-658 / 0	-70.5	-70.5	0.08 (1)	6.25	M-E	-270 / 0
D-E	-384 / 0	-78.0	-78.0	0.17 (1)	6.25	E-K	-149 / 0
E-F	-424 / 0	-78.0	-78.0	0.17 (1)	6.25	K-F	0 / 335
F-G	-728 / 0	-70.5	-70.5	0.12 (1)	6.25	K-G	-48 / 27
G-H	0 / 24	-70.5	-70.5	0.15 (1)	10.00	N-C	-827 / 0
H-I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G-J	-947 / 0
N-B	-185 / 0	0.0	0.0	0.05 (1)	7.81		
J-H	-195 / 0	0.0	0.0	0.02 (1)	7.81		
N-M	0 / 345	-17.5	-17.5	0.23 (4)	10.00		
M-L	0 / 473	-17.5	-17.5	0.29 (4)	10.00		
L-K	0 / 473	-17.5	-17.5	0.29 (4)	10.00		
K-J	0 / 444	-17.5	-17.5	0.29 (4)	10.00		

TOTAL WEIGHT = 150 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 21.0	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -8.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-08
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.17 (E-F:1), BC=0.29 (K-M:4), WB=0.41 (C-N:1), SS=0.16 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.36 (G) (INPUT = 1.00)



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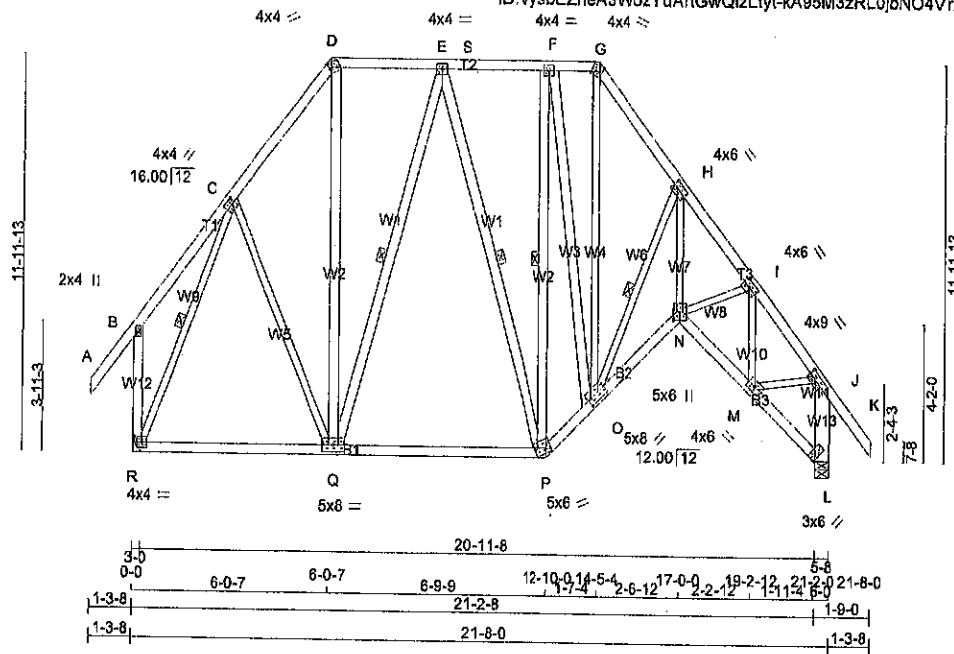


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253622	H13	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:70.4



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x6	DRY	No.2
R - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
Q - D	2x4	DRY	No.2
Q - E	2x4	DRY	No.2
E - P	2x4	DRY	No.2
P - F	2x4	DRY	No.2
F - O	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0	1.75	1.00
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0	1.75	1.00
H	TMWW-t	MT20	4.0	6.0	2.00	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMVV-t	MT20	4.0	9.0	2.00	3.00
L	BVM1-t	MT20	3.0	6.0	Edge	0.25
M	BMWW-t	MT20	4.0	6.0		
N	BBWW-p	MT20	5.0	6.0	3.00	2.25
O	BMWW-t	MT20	5.0	8.0	2.50	1.25
P	BBWW-m	MT20	5.0	6.0	1.75	1.75
Q	BMWW-t	MT20	5.0	8.0		
R	BMVV-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
L	1082	0	1082	0	0
R	1086	0	1085	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
L	763	614 / 0	0 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0	0 / 0
R	765	614 / 0	0 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L
BEARING SIZE FACTOR = 1.15 AT JNT(S) L (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.27FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-Q, E-P, F-P, H-O, C-R.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	C-Q	0 / 85	0.02 (4)
B-C	0 / 18	-70.5	-70.5 0.10 (1)	10.00	Q-D	0 / 326	0.05 (1)
C-D	-659 / 0	-70.5	-70.5 0.08 (1)	6.25	Q-E	-287 / 0	0.25 (1)
D-E	-385 / 0	-78.0	-78.0 0.11 (1)	6.25	E-P	-41 / 0	0.04 (1)
E-S	-451 / 0	-78.0	-78.0 0.11 (1)	6.25	P-F	-374 / 0	0.29 (1)
S-F	-451 / 0	-78.0	-78.0 0.11 (1)	6.25	F-O	0 / 196	0.03 (1)
F-G	-481 / 0	-78.0	-78.0 0.04 (1)	6.25	O-G	0 / 504	0.11 (1)
G-H	-802 / 0	-70.5	-70.5 0.08 (1)	6.25	G-H	-1186 / 0	0.40 (1)
H-I	-1637 / 0	-70.5	-70.5 0.07 (1)	5.27	H-I	0 / 1517	0.34 (1)
I-J	-1114 / 0	-70.5	-70.5 0.07 (1)	5.94	I-J	0 / 269	0.06 (1)
J-K	0 / 40	-70.5	-70.5 0.10 (1)	10.00	M-I	-617 / 0	0.13 (1)
R-B	-185 / 0	0.0	0.0 0.05 (1)	7.81	R-C	-827 / 0	0.41 (1)
L-J	-1082 / 0	0.0	0.0 0.11 (1)	7.81	M-J	0 / 673	0.15 (1)
R-Q	0 / 346	-17.5	-17.5 0.24 (4)	10.00			
Q-P	0 / 482	-17.5	-17.5 0.25 (4)	10.00			
P-O	0 / 657	-17.5	-17.5 0.13 (1)	10.00			
O-N	0 / 1295	-17.5	-17.5 0.23 (1)	10.00			
N-M	0 / 980	-17.5	-17.5 0.16 (1)	10.00			
M-L	0 / 3	-17.5	-17.5 0.02 (4)	10.00			

TOTAL WEIGHT = 2 X 177 = 355 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -8.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/989 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/989 (0.11")

CSI: TC=0.11 (E-F:1), BC=0.25 (P-Q:4), WB=0.41 (C-R:1), SSI=0.13 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1686

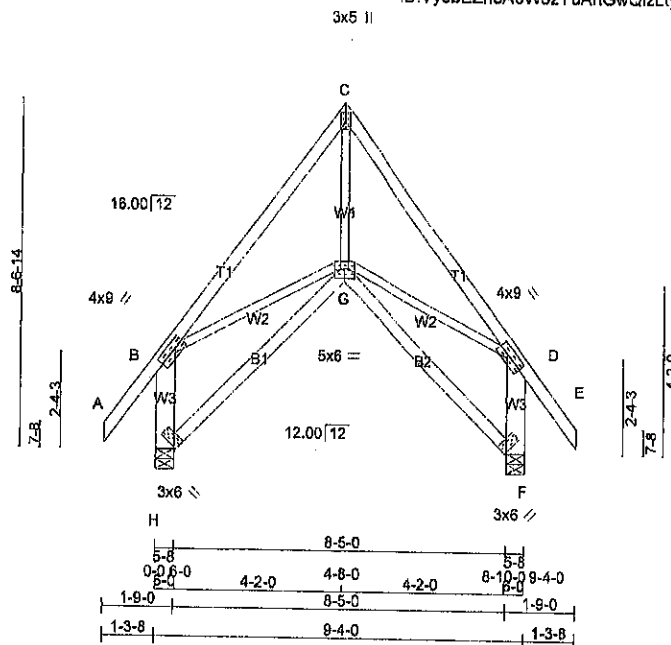
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

A-N073785





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	9.0	2.00 3.00
C	TTW+p	MT20	3.0	5.0	2.00 Edge
D	TMVW4	MT20	4.0	9.0	2.00 3.00
F	BVM1+H	MT20	3.0	6.0	Edge 0.25
G	BBWWV-p	MT20	5.0	6.0	2.75 3.00
H	BVM1+H	MT20	3.0	6.0	Edge 0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	510	0	610	0	5-8	1-8
F	510	0	510	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
H	357	255 / 0	0 / 0	0 / 0	0 / 0	102 / 0
F	357	255 / 0	0 / 0	0 / 0	0 / 0	102 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F
 BEARING SIZE FACTOR = 1.16 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)
 BEARING SIZE FACTOR = 1.16 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	MAX UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G-C	0 / 317	0.07 (1)	
B-C	-404 / 0	-70.5	-70.5	0.20 (1)	6.25	B-G	0 / 269	0.06 (1)	
C-D	-404 / 0	-70.5	-70.5	0.20 (1)	6.25	G-D	0 / 269	0.06 (1)	
D-E	0 / 40	-70.5	-70.5	0.10 (1)	10.00				
H-B	-469 / 0	0.0	0.0	0.06 (1)	7.81				
F-D	-469 / 0	0.0	0.0	0.05 (1)	7.81				
H-G	0 / 0	-17.5	-17.5	0.11 (4)	10.00				
G-F	0 / 0	-17.5	-17.5	0.11 (4)	10.00				

TOTAL WEIGHT = 2 X 57 = 114 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.31")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.20 (C-D:1), BC=0.11 (G-H:4),
 WB=0.07 (C-G:1), SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE HEELS OFF

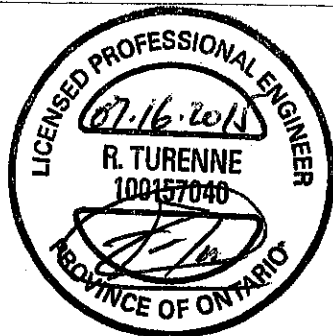
TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL IN
 THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 619 354 1687 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.49 (C) (INPUT = 0.80)
 JSI METAL=0.10 (D) (INPUT = 1.00)



A-15073784



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.5 lbs FACTORED DOWN AT 14-9.4, AND 114.5 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

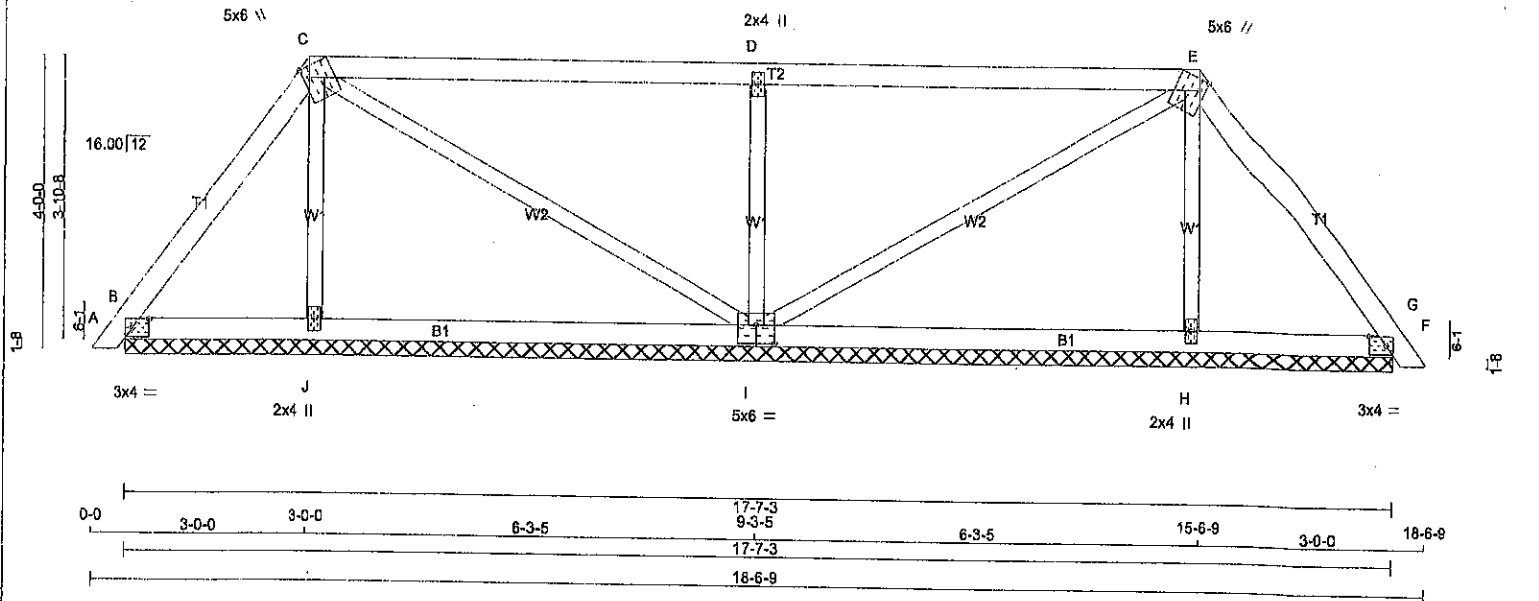
NOTES- (1)



JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-115	-115	--	FRONT	VERT	TOTAL
E	14-8-4	-115	-115	--	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.28 (H) (INPUT = 1.00)

A-15073783



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
B - I	2x4	DRY No.2	SPF
I - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BSVWW1-I	MT20	5.0	6.0	3.00	3.00
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	193	0	193	0	17-7.3 (8-9-10)8	
J	252	0	252	0	17-7.3 (8-9-10)8	
I	709	0	709	0	17-7.3 (8-9-10)8	
H	252	0	252	0	17-7.3 (8-9-10)8	
F	193	0	193	0	17-7.3 (8-9-10)8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	132	110 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
J	182	97 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
I	496	354 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0
H	182	97 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
F	132	110 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, I, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 9	-70.5 -70.5	0.01 (1)	J-C	-163 / 0	0.04 (1)	
B-C	-88 / 0	-70.5 -70.5	0.08 (1)	C-I	-42 / 0	0.04 (1)	
C-D	-9 / 0	-70.5 -70.5	0.47 (1)	I-D	-552 / 0	0.13 (1)	
D-E	-9 / 0	-70.5 -70.5	0.47 (1)	E-H	-42 / 0	0.04 (1)	
E-F	-88 / 0	-70.5 -70.5	0.08 (1)	H-E	-163 / 0	0.04 (1)	
F-G	0 / 9	-70.5 -70.5	0.01 (1)				
B-J	0 / 61	-17.5 -17.5	0.10 (4)				
J-I	0 / 46	-17.5 -17.5	0.15 (4)				
I-H	0 / 46	-17.5 -17.5	0.15 (4)				
H-F	0 / 61	-17.5 -17.5	0.10 (4)				

TOTAL WEIGHT = 3 X 66 = 195 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47 (C-D:1), BC=0.15 (I-J:4), WB=0.13 (D-I:1), SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1666

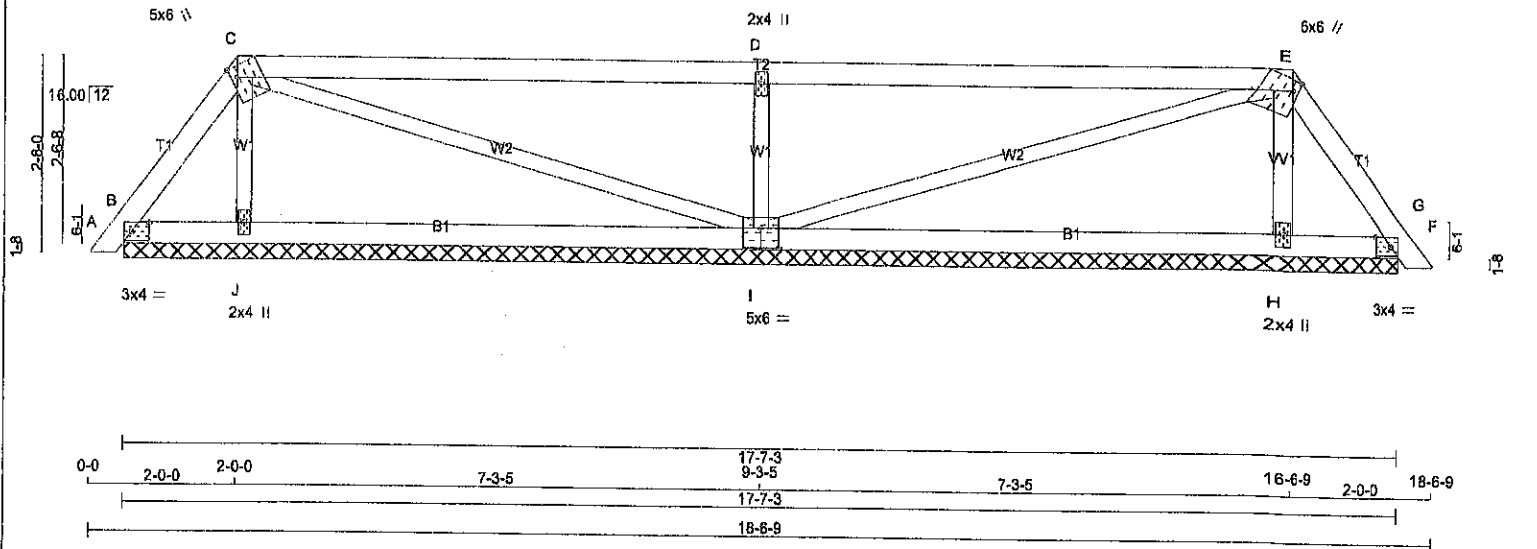
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.80)
JSI METAL = 0.23 (I) (INPUT = 1.00)



A-1073782



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
B - I	2x4	DRY No.2	SPF
I - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	8.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	8.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BSWWW1-I	MT20	5.0	6.0	3.25	3.00
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	119	0	119	0	-6	17-7-3 (8-8-10) 3/8
J	287	0	287	0	0	17-7-3 (8-8-10) 3/8
I	786	0	786	0	0	17-7-3 (8-8-10) 3/8
H	287	0	287	0	0	17-7-3 (8-8-10) 3/8
F	119	0	119	0	-6	17-7-3 (8-8-10) 3/8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 160 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT F FOR 160 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	79	84 / 0	0 / 0	0 / 0	0 / 0	0 / 0
J	208	105 / 0	0 / 0	0 / 0	0 / 0	103 / 0
I	560	388 / 0	0 / 0	0 / 0	0 / 0	161 / 0
H	208	105 / 0	0 / 0	0 / 0	0 / 0	103 / 0
F	79	84 / 0	0 / 0	0 / 0	0 / 0	0 / -5

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, I, H, F

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	J-C	-177 / 0
B-C	-76 / 0	-70.5	-70.5	0.03 (1)	6.25	C-I	-22 / 0
C-D	-13 / 0	-70.5	-70.5	0.63 (1)	6.25	I-D	-640 / 0
D-E	-13 / 0	-70.5	-70.5	0.63 (1)	6.25	I-E	-22 / 0
E-F	-76 / 0	-70.5	-70.5	0.03 (1)	6.25	H-E	-177 / 0
F-G	0 / 9	-70.5	-70.5	0.01 (1)	10.00		
B-J	0 / 43	-17.5	-17.5	0.14 (4)	10.00		
J-I	0 / 35	-17.5	-17.5	0.20 (4)	10.00		
I-H	0 / 35	-17.5	-17.5	0.20 (4)	10.00		
H-F	0 / 43	-17.5	-17.5	0.14 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63 (D-E:1), BC=0.20 (I-J:4), WB=0.10
 (D-I:1), SSI=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1666

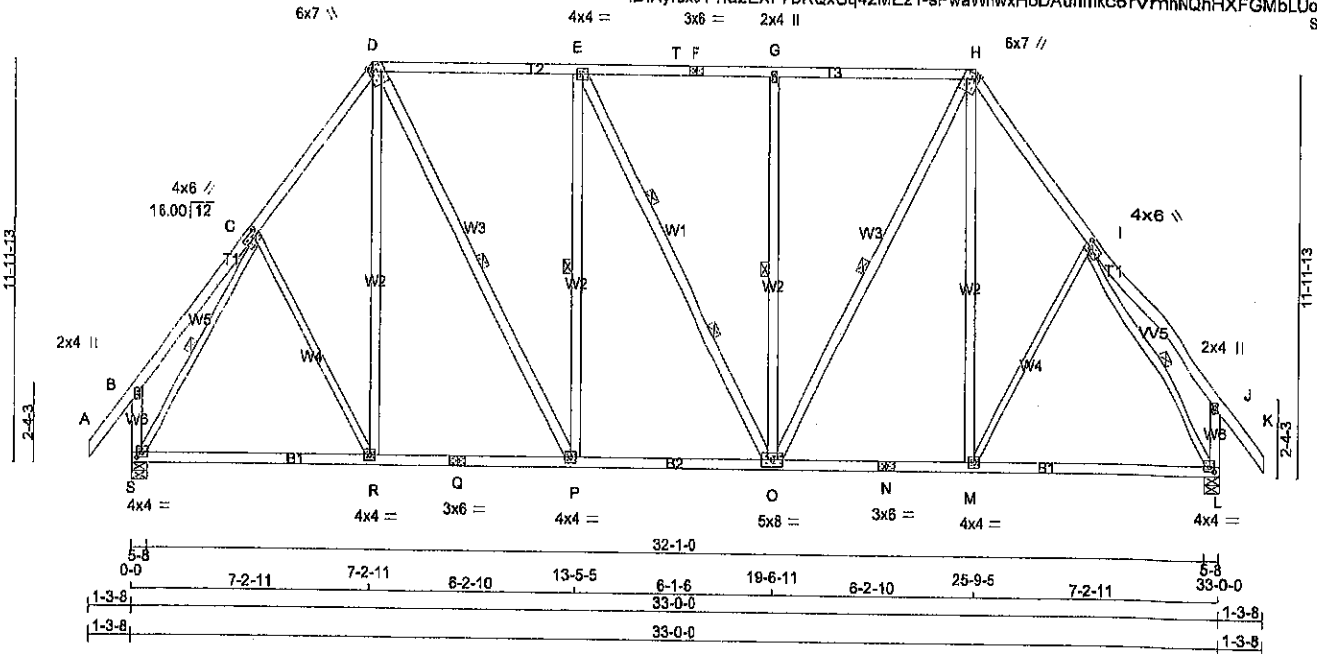
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (I) (INPUT = 0.90)
 JSI METAL = 0.24 (I) (INPUT = 1.00)



A-15073781



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
C - R	2x3	DRY	No.2
M - I	2x3	DRY	No.2
S - C	2x3	DRY	No.2
I - L	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C	TMVW+1	MT20	4.0	6.0	2.00 2.00
D	TTWW+m	MT20	6.0	7.0	Edge 1.50
E	TMVW+1	MT20	4.0	4.0	
F	TS-1	MT20	3.0	6.0	
G	TMVW+w	MT20	2.0	4.0	
H	TTWW+m	MT20	6.0	7.0	Edge 1.50
I	TMVW+1	MT20	4.0	6.0	2.00 2.00
J	TMV+p	MT20	2.0	4.0	
L	BMVW+1	MT20	4.0	4.0	1.75 1.75
M, P, R					
M	BMVW+1	MT20	4.0	4.0	
N	BS-1	MT20	3.0	6.0	
O	BMVW+1	MT20	6.0	6.0	
Q	BS-1	MT20	3.0	6.0	
S	BMVW+1	MT20	4.0	4.0	1.75 1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	1620	0	1620	0	0	6-8	1-12
L	1620	0	1620	0	0	6-8	1-12

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
S	1146	762 / 0	0 / 0	0 / 0	0 / 0
L	1146	762 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.0 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-P, E-P, G-O, H-O, C-S, I-L.
2 LATERAL BRACE(S) REQUIRED AT 1/3 LENGTH OF E-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 40	-70.5	-70.5 0.10 (1)	C-R	0 / 64
B-C	0 / 23	-70.5	-70.5 0.14 (1)	R-D	0 / 97
C-D	-1286 / 0	-70.5	-70.5 0.14 (1)	D-P	0 / 702
D-E	-1085 / 0	-78.0	-78.0 0.36 (1)	P-E	-519 / 0
E-T	-1085 / 0	-78.0	-78.0 0.33 (1)	E-O	-2 / 0
T-F	-1085 / 0	-78.0	-78.0 0.33 (1)	O-G	-518 / 0
F-G	-1085 / 0	-78.0	-78.0 0.36 (1)	G-H	0 / 700
G-H	-1084 / 0	-78.0	-78.0 0.36 (1)	H-I	0 / 98
H-I	-1286 / 0	-70.5	-70.5 0.14 (1)	I-J	0 / 64
I-J	0 / 23	-70.5	-70.5 0.14 (1)	J-K	0 / 64
J-K	0 / 40	-70.5	-70.5 0.10 (1)	K-L	-1552 / 0
S-B	-196 / 0	0.0	0.0 0.02 (1)	L-J	-1553 / 0
L-J	-196 / 0	0.0	0.0 0.02 (1)		
S-R	0 / 727	-17.5	-17.5 0.27 (4)		
R-Q	0 / 756	-17.5	-17.5 0.28 (4)		
Q-P	0 / 755	-17.5	-17.5 0.28 (4)		
P-O	0 / 1085	-17.5	-17.5 0.24 (1)		
O-N	0 / 755	-17.5	-17.5 0.28 (4)		
N-M	0 / 755	-17.5	-17.5 0.28 (4)		
M-L	0 / 727	-17.5	-17.5 0.27 (4)		

TOTAL WEIGHT = 9 X 216 = 1946 lb

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 21.0 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOADEQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.36 (D-E:1), BC=0.28 (M-O:4), WB=0.60 (I-L:1), SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
618	354	1667	822 2284 1656

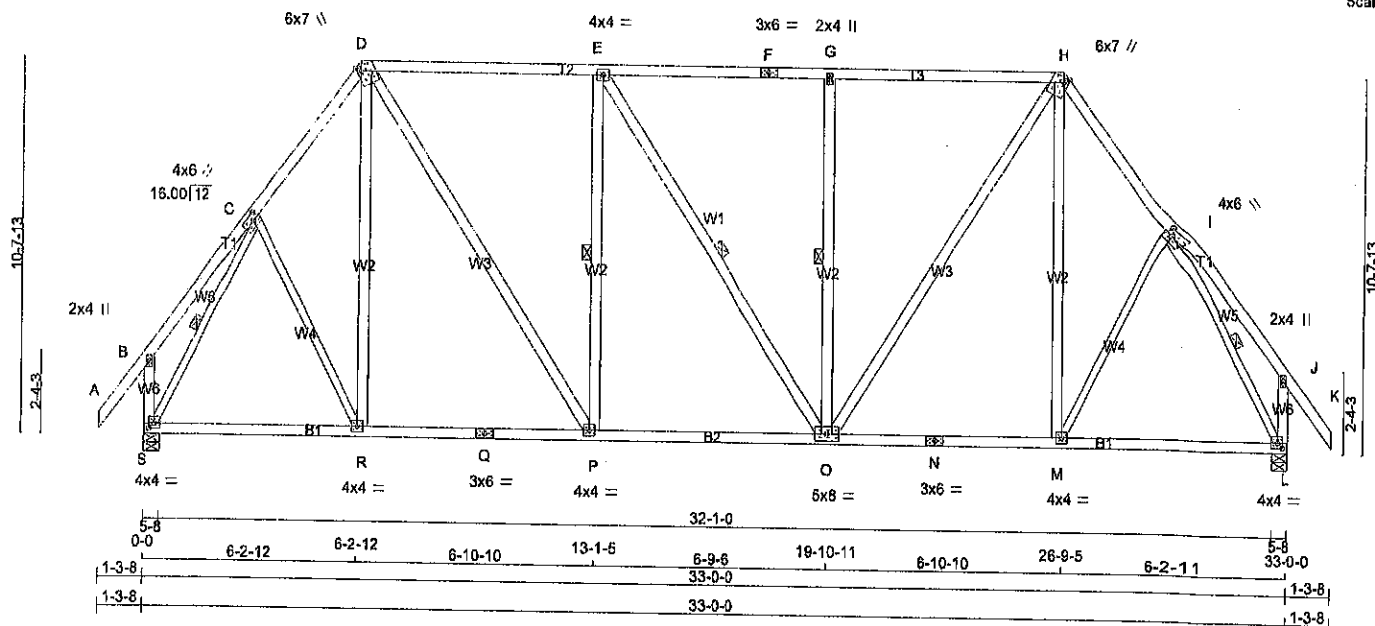
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)

A-1073780



**LUMBER****N. L. G. A. RULES**

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
I - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.00
D	TTWW+m	MT20	6.0	7.0	Edge	1.60
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	6.0	7.0	Edge	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW-t	MT20	4.0	4.0	1.75	1.75
M, P, R						
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	3.0	6.0		
S	BMVW-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
S	1650	0	1550	0
L	1650	0	1550	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX / MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM LVE	WIND	DEAD	SOIL
S	1090	762 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
L	1090	762 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-P, E-O, G-O, C-S, I-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW**LOADING**

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	MAX UNBRACED LENGTH (FT)	MEMB.
FR-TO					FR-TO				
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C-R	0 / 102	0.02 (1)	
B-C	0 / 18	-70.5	-70.5	0.10 (1)	10.00	R-D	0 / 88	0.02 (4)	
C-D	-1224 / 0	-70.5	-70.5	0.13 (1)	5.67	D-P	0 / 766	0.12 (1)	
D-E	-1144 / 0	-70.5	-70.5	0.52 (1)	5.24	P-E	-520 / 0	0.30 (1)	
E-F	-1144 / 0	-70.5	-70.5	0.52 (1)	5.24	E-O	-1 / 0	0.00 (1)	
F-G	-1144 / 0	-70.5	-70.5	0.52 (1)	5.24	O-G	-520 / 0	0.30 (1)	
G-H	-1144 / 0	-70.5	-70.5	0.52 (1)	5.24	O-H	0 / 765	0.12 (1)	
H-I	-1224 / 0	-70.5	-70.5	0.13 (1)	5.67	M-H	0 / 88	0.02 (4)	
I-J	0 / 19	-70.5	-70.5	0.10 (1)	10.00	M-I	0 / 102	0.02 (1)	
J-K	0 / 40	-70.5	-70.5	0.10 (1)	10.00	S-C	-1483 / 0	0.46 (1)	
S-B	-183 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-1483 / 0	0.46 (1)	
L-J	-183 / 0	0.0	0.0	0.02 (1)	7.81				
S-R	0 / 689	-17.5	-17.5	0.23 (4)	10.00				
R-Q	0 / 719	-17.5	-17.5	0.24 (4)	10.00				
Q-P	0 / 719	-17.5	-17.5	0.24 (4)	10.00				
P-O	0 / 1144	-17.5	-17.5	0.27 (1)	10.00				
O-N	0 / 719	-17.5	-17.5	0.24 (4)	10.00				
N-M	0 / 719	-17.5	-17.5	0.24 (4)	10.00				
M-L	0 / 689	-17.5	-17.5	0.23 (4)	10.00				

TOTAL WEIGHT = 2 X 203 = 406 lb [M]

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = $L/360$ (1.10")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (1.10")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")CSI: TC=0.52 (D-E:1), BC=0.27 (O-P:1),
WB=0.46 (I-L:1), SSI=0.23 (D-E:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (I) (INPUT = 0.80)
JSI METAL=0.35 (I) (INPUT = 1.00)

A-15073779

253622

H04

QUANTITY

2

PLY

1

JOB DESC.

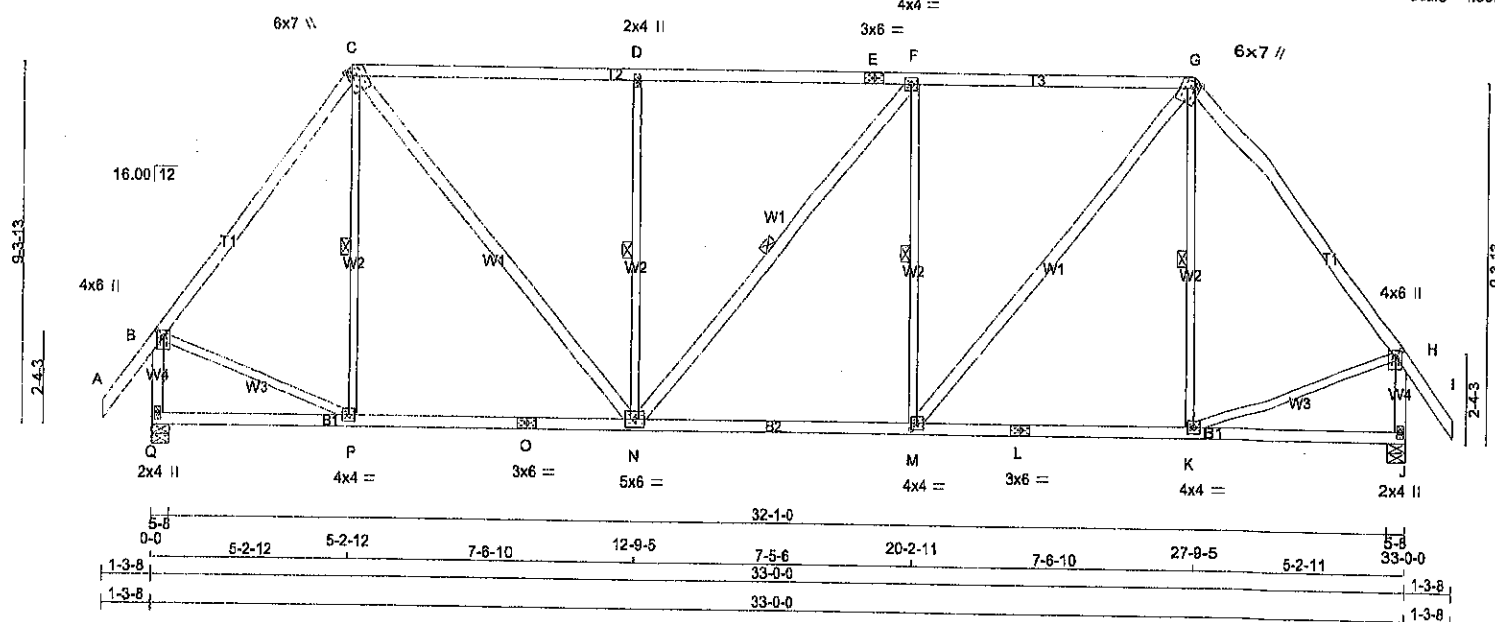
TRUSS DESC.

DRWG NO.

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 Mitek Industries, Inc. Thu Jul 16 14:31:58 2015 Page 1
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Scale = 1:60.3



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
K - M	2x4	DRY	No.2
M - O	2x4	DRY	No.2
O - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
C - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2
M - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TS-i	MT20	3.0	6.0		
F	TMVW-i	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-i	MT20	4.0	4.0		
L	BS-i	MT20	3.0	6.0		
M	BMVW-i	MT20	4.0	4.0	2.00	1.75
N	BMVW-i	MT20	6.0	6.0		
O	BS-i	MT20	3.0	6.0		
P	BMVW-i	MT20	4.0	4.0		
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
Q	VERT	HORZ	DOWN	HORZ
J	1650	0	1650	0
J	1650	0	1650	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
Q	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1090	762 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
J	1090	762 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-P, D-N, F-N, F-M, G-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	P-C	-140 / 62	0.08 (1)	
B-C	-1208 / 0	-70.5	-70.5	0.46 (1)	5.36	C-N	0 / 917	0.15 (1)	
C-D	-1310 / 0	-70.5	-70.5	0.66 (1)	4.76	N-D	-571 / 0	0.33 (1)	
D-E	-1310 / 0	-70.5	-70.5	0.87 (1)	4.76	N-F	-1 / 0	0.00 (1)	
E-F	-1310 / 0	-70.5	-70.5	0.87 (1)	4.76	M-F	-571 / 0	0.33 (1)	
F-G	-1311 / 0	-70.5	-70.5	0.87 (1)	4.76	M-G	0 / 919	0.15 (1)	
G-H	-1208 / 0	-70.5	-70.5	0.45 (1)	5.36	K-G	-141 / 62	0.08 (1)	
H-I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	B-P	0 / 769	0.17 (1)	
Q-B	-1517 / 0	0.0	0.0	0.19 (1)	6.87	K-H	0 / 769	0.17 (1)	
J-H	-1517 / 0	0.0	0.0	0.19 (1)	6.87				
Q-P	0 / 0	-17.5	-17.5	0.17 (4)	10.00				
P-O	0 / 720	-17.5	-17.5	0.25 (4)	10.00				
O-N	0 / 720	-17.5	-17.5	0.25 (4)	10.00				
N-M	0 / 1311	-17.5	-17.5	0.32 (1)	10.00				
M-L	0 / 720	-17.5	-17.5	0.25 (4)	10.00				
L-K	0 / 720	-17.5	-17.5	0.25 (4)	10.00				
K-J	0 / 0	-17.5	-17.5	0.17 (4)	10.00				

TOTAL WEIGHT = 2 X 169 = 339 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011(55 % OF 23.0 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")CSI: TC=0.67 (F-G:1), BC=0.32 (M-N:1),
WB=0.33 (F-M:1), SSI=0.25 (F-G:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

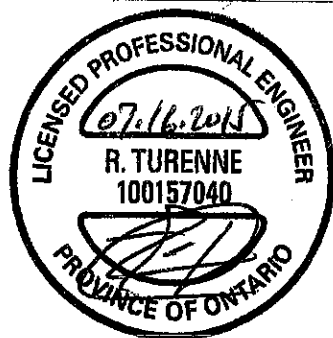
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

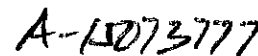
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1666

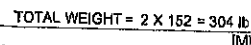
PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.80)
JSI METAL= 0.29 (B) (INPUT = 1.00)

A-15073778





DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q. J

Q-P	0/0	-17.5	-17.5	0.20 (4)	10.00
P-O	0/685	-17.5	-17.5	0.33 (4)	10.00
O-N	0/685	-17.5	-17.5	0.33 (4)	10.00
N-M	0/1852	-17.5	-17.5	0.45 (1)	10.00
M-L	0/685	-17.5	-17.5	0.33 (4)	10.00
L-K	0/685	-17.5	-17.5	0.33 (4)	10.00
K-J	0/0	-17.5	-17.5	0.20 (4)	10.00

WEBS	MAX. FACTORED	
MEMB.	FORCE	MAX
	(LBS)	CSI (LC)
R-TO		
P-C	-275 / 9	0.20 (1)
C-N	0 / 1431	0.23 (1)
N-D	-673 / 0	0.49 (1)
N-F	0 / 0	0.00 (1)
M-F	-673 / 0	0.49 (1)
A-G	0 / 1432	0.23 (1)
C-G	-278 / 9	0.20 (1)
C-P	0 / 800	0.18 (1)
K-H	0 / 800	0.18 (1)

SPECIFIED LOADS:				
TOP	CH.	LL	=	21.0 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.0 PSF
TOTAL LOAD			=	31.0 PSF

JSI GRIP= 0.81 (Q) (INPUT = 0.90)
JSI METAL= 0.44 (M) (INPUT = 1.00)



A-15073776

253622

H01-Cond2

QUANTITY

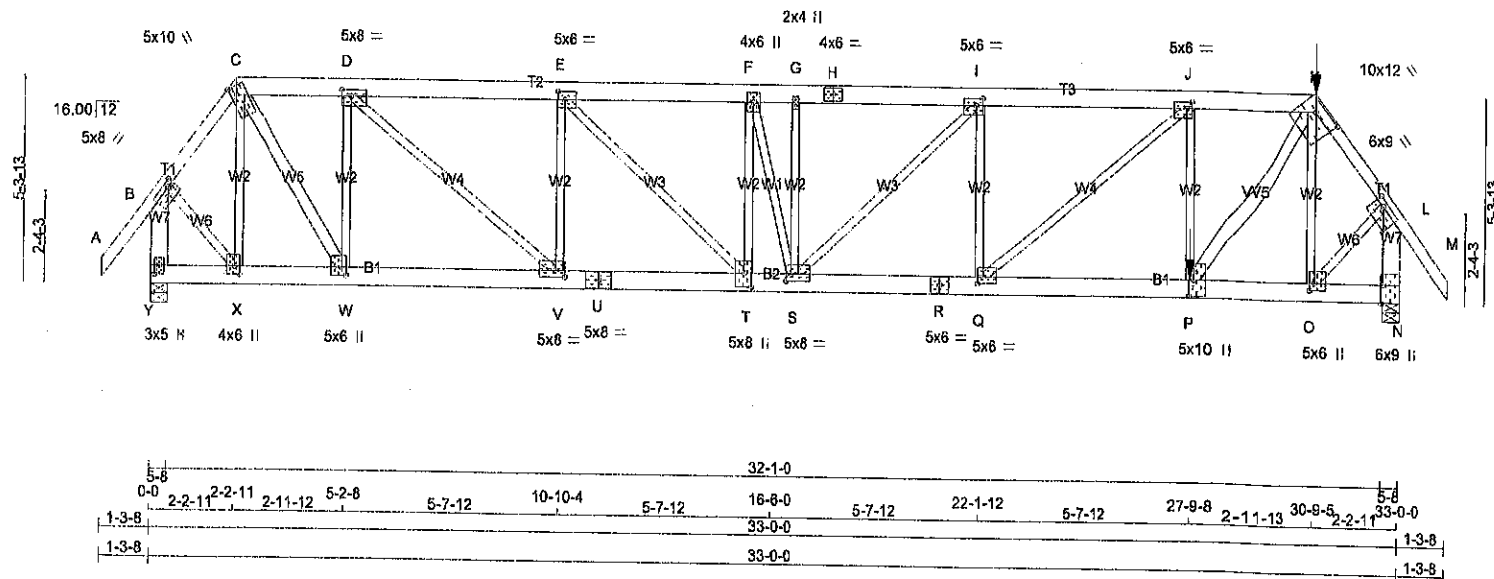
PLY

JOB DESC.

DRWG NO.

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 Mitek Industries, Inc. Thu Jul 16 14:36:30 2015 Page 1
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 Scale = 1:80.3



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - H	2x6	DRY	No.2 SPF
H - K	2x6	DRY	No.2 SPF
K - M	2x4	DRY	No.2 SPF
Y - B	2x6	DRY	No.2 SPF
N - L	2x6	DRY	No.2 SPF
Y - U	2x6	DRY	1650F 1.5E SPF
U - R	2x6	DRY	1650F 1.5E SPF
R - N	2x6	DRY	1650F 1.5E SPF
ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			
C - W	2x4	DRY	No.2 SPF
P - K	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	8.0	1.50	2.00
C TTWW+m	MT20	5.0	10.0	2.75	1.75
D TMVW-t	MT20	5.0	8.0	2.00	3.00
E TMVW-t	MT20	5.0	6.0	2.50	2.25
F TMVW-t	MT20	4.0	6.0		
G TMVW-t	MT20	2.0	4.0		
H TS-t	MT20	4.0	6.0		
I TMVW-t	MT20	5.0	8.0	2.25	2.00
J TMVW-t	MT20	5.0	8.0	2.00	1.75
K TMVW-t	MT20	10.0	12.0	2.75	7.25
L TMVW-t	MT20	6.0	9.0	1.50	3.00
N BMV1-t	MT20	8.0	9.0	Edge	0.50
O BMVW-t	MT20	5.0	6.0	2.25	1.50
P BMVW-t	MT20	5.0	10.0	5.00	1.50
Q BMVW-t	MT20	5.0	6.0	2.00	1.75
R BS-t	MT20	5.0	6.0		
S BMVW-t	MT20	5.0	8.0	2.25	1.50
T BMVW-t	MT20	5.0	8.0	4.25	2.25
U BS-t	MT20	5.0	8.0		
V BMVW-t	MT20	5.0	8.0	2.00	3.00
W BMVW-t	MT20	5.0	6.0	2.50	1.50
X BMVW-t	MT20	4.0	6.0	2.50	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2008	0	2008	0
Y	2008	0	2008	0
N	4159	0	4159	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT COMBINED		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
Y	1462	770 / 0	0 / 0	0 / 0	0 / 0	682 / 0	0 / 0	
N	3143	923 / 0	0 / 0	0 / 0	0 / 0	2220 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO		
A-B	0 / 40	-70.5 -70.5	0.13 (1)	10.00	X-C	-617 / 0	0.25 (1)
B-C	-1427 / 0	-70.5 -70.5	0.11 (1)	5.29	C-W	0 / 1898	0.39 (1)
C-D	-1835 / 0	-70.5 -70.5	0.14 (1)	5.69	W-D	-1637 / 0	0.62 (1)
D-E	-3225 / 0	-70.5 -70.5	0.23 (1)	4.48	D-V	0 / 1833	0.62 (1)
E-F	-3973 / 0	-70.5 -70.5	0.27 (1)	4.05	V-E	-1118 / 0	0.45 (1)
F-G	-4061 / 0	-70.5 -70.5	0.20 (1)	4.08	E-T	0 / 1041	0.31 (4)
G-H	-4061 / 0	-70.5 -70.5	0.30 (1)	3.99	S-I	-815 / 0	0.80 (4)
H-I	-4061 / 0	-70.5 -70.5	0.30 (1)	3.99	Q-I	-187 / 353	0.13 (4)
I-J	-4303 / 0	-70.5 -70.5	0.33 (1)	3.86	Q-J	-288 / 517	0.33 (4)
J-K	-3911 / 0	-138.7 -138.7	0.26 (4)	4.10	P-J	-734 / 197	0.29 (1)
K-L	-3123 / 0	-70.5 -70.5	0.25 (1)	3.67	P-K	0 / 3949	0.88 (4)
L-M	0 / 40	-70.5 -70.5	0.13 (1)	10.00	O-K	-1263 / 0	0.50 (1)
Y-B	-2009 / 0	0.0 0.0	0.20 (1)	7.03	B-X	0 / 1098	0.31 (1)
N-L	-4185 / 0	0.0 0.0	0.41 (1)	5.10	O-L	0 / 2402	0.71 (4)
					T-F	-675 / 0	0.23 (1)
Y-X	0 / 0	-17.5 -17.5	0.03 (1)	10.00	S-G	-188 / 0	0.07 (1)
X-W	0 / 842	-17.5 -17.5	0.11 (1)	10.00	F-S	0 / 410	0.16 (4)
W-V	0 / 1836	-17.5 -17.5	0.20 (1)	10.00			
V-U	0 / 3225	-17.5 -17.5	0.33 (1)	10.00			
U-T	0 / 3225	-17.5 -17.5	0.33 (1)	10.00			
T-S	0 / 3973	-17.5 -17.5	0.42 (1)	10.00			
S-R	0 / 4303	-17.5 -17.5	0.45 (1)	10.00			
R-Q	0 / 4303	-17.5 -17.5	0.45 (1)	10.00			
Q-P	0 / 3912	-17.5 -17.5	0.50 (4)	10.00			
P-O	0 / 1844	-34.5 -34.5	0.29 (4)	10.00			
O-N	0 / 0	-34.5 -34.5	0.11 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	30-9-5	-114	-114	-	FRONT	VERT	TOTAL
P	27-9-8	-2660	-2660	-	FRONT	VERT	DEAD

TOTAL WEIGHT = 2 X 202 = 405 lb

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN./CG

LOADING IN FLAT SECTION BASED ON A
 SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
 SIDE SETBACK = 2-2-11
 END SETBACK = 5-10-8
 END WALL WIDTH = 0-0
 CORNER FRAMING TYPE: CONVENTIONAL
 END JACK TYPE: CONVENTIONAL
 APPLIED TO FRONT SIDE
 - ADDTL LOADS BASED ON 55 % OF GSL.
 LOADS APPLIED TO FIRST 5-2-8 OF SPAN
 MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO
 ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
 OR SMALL BUILDING REQUIREMENTS OF
 PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (1.10")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.41 (L-N:1), BC=0.60 (P-Q:4), WB=0.88
 (K-P:4), SS=0.17 (J-K:1)

DOL LUMBER=0.88 NAIL=0.88 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL IN
 THE TRUSS MANUFACTURING PLANT.

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CONTINUED ON PAGE 2



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Y	BMV1+p	MT20	3.0	6.0	2.75	1.60

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 30-9-6 ON TOP CHORD, AND 2979.2 lbs FACTORED DOWN AT 27-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1607
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

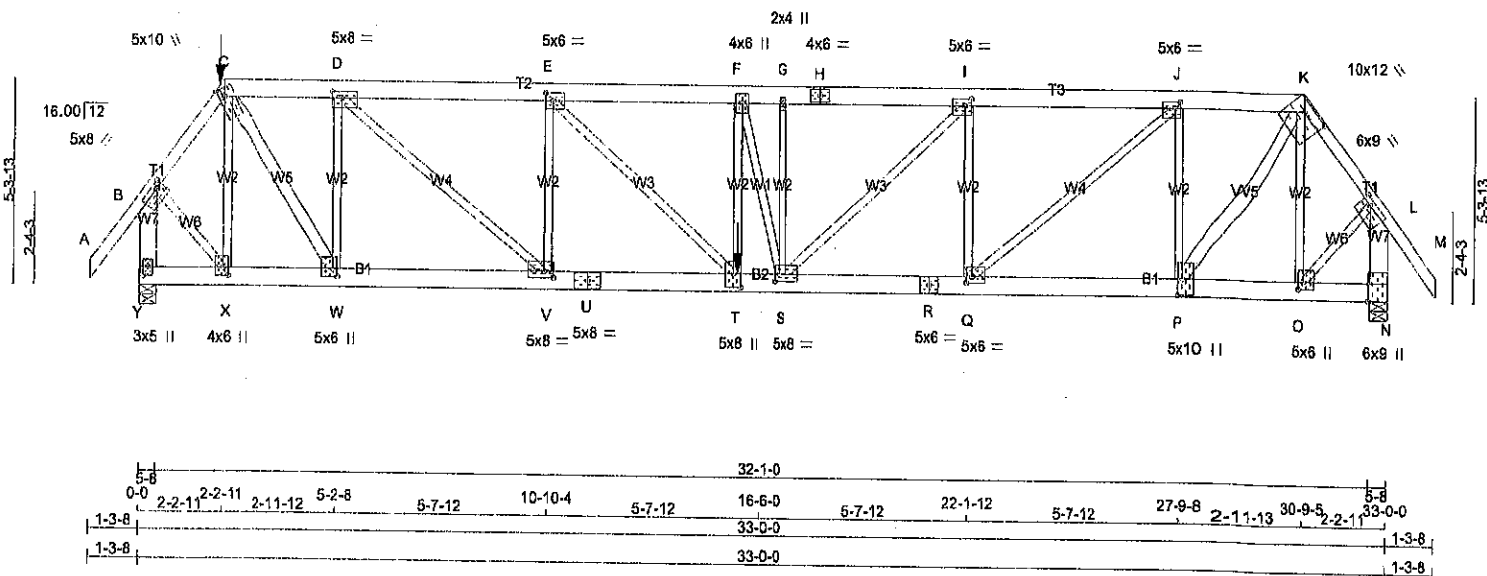
JSI GRIP= 0.89 (L) (INPUT = 0.90)

JSI METAL= 0.80 (K) (INPUT = 1.00)



A-15073775(2)

Alpa Roof Truss, Maple

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ID:VysbEZneA3WozYuArGwQlZLyt-lpczbVSqJgN52q5vCc9G_4Gtb6CTENfNtNGVByxToE
Scale = 1:60.3**LUMBER****N. L. G. A. RULES**

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - H	2x6	DRY	No.2
H - K	2x6	DRY	No.2
K - M	2x4	DRY	No.2
Y - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
Y - U	2x6	DRY	1650F 1.5E
U - R	2x6	DRY	1650F 1.5E
R - N	2x6	DRY	1650F 1.5E

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - W	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-I	MT20	5.0	6.0	1.50	2.00
C TTWW+m	MT20	6.0	10.0	2.75	1.75
D TMWW-I	MT20	5.0	8.0	2.00	3.00
E TMWW-I	MT20	5.0	6.0	2.50	2.25
F TMWW-I	MT20	4.0	6.0		
G TMWW-I	MT20	2.0	4.0		
H TS-I	MT20	4.0	6.0		
I TMWW-I	MT20	5.0	6.0	2.25	2.00
J TMWW-I	MT20	5.0	6.0	2.00	1.75
K TTWW-h	MT20	10.0	12.0	2.75	7.25
L TMWW-I	MT20	6.0	9.0	1.50	3.00
N BMV-I	MT20	6.0	9.0	Edge	0.50
O BMWW-I	MT20	6.0	6.0	2.25	1.50
P BMWW-I	MT20	5.0	10.0	5.00	1.50
Q BMWW-I	MT20	5.0	6.0	2.00	1.75
R BS-I	MT20	5.0	6.0		
S BMWW-I	MT20	5.0	8.0	2.25	1.50
T BMWW-I	MT20	5.0	8.0	4.25	2.25
U BS-I	MT20	5.0	8.0		
V BMWW-I	MT20	6.0	8.0	2.00	3.00
W BMWW-I	MT20	6.0	6.0	2.50	1.50
X BMWW-I	MT20	4.0	6.0	2.50	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
Y	3381	0	3361	0
N	2661	0	2661	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
Y	2461	1218 / 0	0 / 0	0 / 0	0 / 0	1244 / 0	0 / 0
N	1947	908 / 0	0 / 0	0 / 0	0 / 0	1039 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.**LOADING**

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO			FR-TO		
A-B	0 / 40	-70.5 -70.5 0.11 (1)	10.00	X-C	-1060 / 0	0.41 (1)	
B-C	-2497 / 0	-70.5 -70.5 0.18 (1)	4.20	C-W	0 / 3215	0.58 (1)	
C-D	-3156 / 0	-138.7 -138.7 0.27 (1)	4.49	W-D	-2607 / 0	1.00 (1)	
D-E	-5407 / 0	-138.7 -138.7 0.47 (1)	3.39	D-V	0 / 2969	0.76 (4)	
E-F	-6341 / 0	-138.7 -138.7 0.54 (1)	3.06	V-E	-1898 / 0	0.65 (1)	
F-G	-6100 / 0	-70.5 -70.5 0.35 (4)	3.36	E-T	0 / 1319	0.50 (4)	
G-H	-6100 / 0	-70.5 -70.5 0.39 (1)	3.29	S-I	0 / 1983	0.59 (4)	
H-I	-6100 / 0	-70.5 -70.5 0.39 (1)	3.29	Q-I	-1769 / 0	0.68 (1)	
I-J	-4875 / 0	-70.5 -70.5 0.29 (1)	3.81	Q-J	0 / 2620	0.71 (1)	
J-K	-2538 / 0	-70.5 -70.5 0.15 (1)	5.05	P-J	-2185 / 0	0.84 (1)	
K-L	-1943 / 0	-70.5 -70.5 0.12 (1)	4.71	P-K	0 / 2659	0.48 (1)	
L-M	0 / 40	-70.5 -70.5 0.11 (1)	10.00	O-K	-854 / 0	0.33 (1)	
Y-B	-3381 / 0	0.0 0.0 0.31 (1)	5.72	B-X	0 / 1820	0.49 (1)	
N-L	-2670 / 0	0.0 0.0 0.24 (1)	6.32	O-L	0 / 1494	0.38 (1)	
Y-X	0 / 0	-34.5 -34.5 0.05 (1)	10.00	T-F	0 / 752	0.29 (4)	
X-W	0 / 1473	-34.5 -34.5 0.17 (1)	10.00	S-G	-188 / 0	0.07 (1)	
W-V	0 / 3156	-34.5 -34.5 0.31 (1)	10.00	F-S	-968 / 0	0.39 (1)	
V-U	0 / 5407	-34.5 -34.5 0.57 (1)	10.00				
U-T	0 / 5407	-34.5 -34.5 0.57 (1)	10.00				
T-S	0 / 6341	-17.5 -17.5 0.67 (4)	10.00				
S-R	0 / 4875	-17.5 -17.5 0.42 (1)	10.00				
R-Q	0 / 4875	-17.5 -17.5 0.42 (1)	10.00				
Q-P	0 / 2538	-17.5 -17.5 0.25 (1)	10.00				
P-O	0 / 1146	-17.5 -17.5 0.13 (1)	10.00				
O-N	0 / 0	-17.5 -17.5 0.04 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
C	2-2-11	-114	-114	—	FRONT	VERT
T	15-10-8	-1626	-1626	—	FRONT	VERT

TOTAL WEIGHT = 2 X 202 = 405 lb

DESIGN CRITERIA*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE**SPECIFIED LOADS:**

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55% OF GSL.
LOADS APPLIED TO FIRST 15-10-8 OF SPAN MEASURED FROM THE LEFT.*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 066-09
- TPIC 2011(65% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/911 (0.43")

CSI: TC=0.54 (E-F:1), BC=0.67 (S-T:4), WB=1.00 (D-W:1), SS=0.27 (D-E:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



A-15073774

CONTINUED ON PAGE 2

253622

TRUSS NAME

H01-Cond1

QUANTITY

2

PLY

1

JOB DESC.

TRUSS DESC.

DRWG NO.

Alpa Roof Truss, Maple

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ID:VysbEZneA3WozYuArIGwQIzLtyl-ipczbVStqgN52q5vCc9G 4Gtx8CTENFnTNGVByxToE

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Y	BMV1+p	MT20	3.0	6.0	2.75	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11
ON TOP CHORD, AND 1821.4 lbs FACTORED
DOWN AT 15-10-8 ON BOTTOM CHORD.
DESIGN FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618 354	1887 822	2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (V) (INPUT = 0.90)
JSI METAL = 0.88 (U) (INPUT = 1.00)



A-15073774(2)

253620

Alpa Roof Truss, Maple

TRUSS NAME

J01T

QUANTITY

7

PLY

1

JOB DESC.

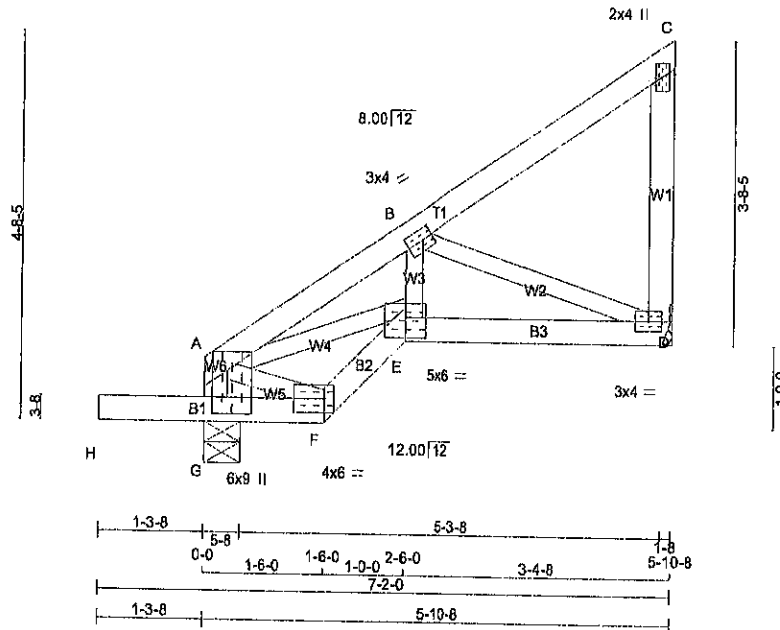
TRUSS DESC.

DRWG NO.

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ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-dIHdFOHSRAIoDIJoGlnNbfQbl_r_KCcXSQ6pyxVWY

Scale = 1:28.2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
G - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-I	MT20	3.0	4.0	1.50	1.50
C	TMV+p	MT20	2.0	4.0		
D	BMVW-I	MT20	3.0	4.0		
E	BBW-I	MT20	5.0	6.0	3.00	3.00
F	BBW-I	MT20	4.0	6.0	2.00	4.50
G	TMBMWW-I	MT20	6.0	9.0	6.75	3.00

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
G	384	0	384	0
D	246	0	246	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	271	183 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	173	117 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX
FR-TO		FROM TO			FR-TO		
G-A	-208 / 0	0.0	0.0	0.02 (1)	7.81	A-F	-22 / 7
A-B	-408 / 0	-70.5	-70.5	0.07 (1)	6.25	A-E	0 / 380
B-C	-11 / 0	-70.5	-70.5	0.08 (1)	6.25	E-B	0 / 108
D-C	-98 / 0	0.0	0.0	0.02 (1)	7.81	B-D	-382 / 0
H-G	0 / 0	-88.0	-88.0	0.10 (1)	10.00		
G-F	0 / 0	-17.5	-17.5	0.08 (1)	10.00		
F-E	-30 / 10	-17.5	-17.5	0.01 (4)	8.25		
E-D	0 / 340	-17.5	-17.5	0.08 (1)	10.00		

TOTAL WEIGHT = 7 X 27 = 190 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = $L/120$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/120$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.08 (B-C:1), BC=0.10 (G-H:1),
WB=0.09 (A-E:1), SSI=0.09 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

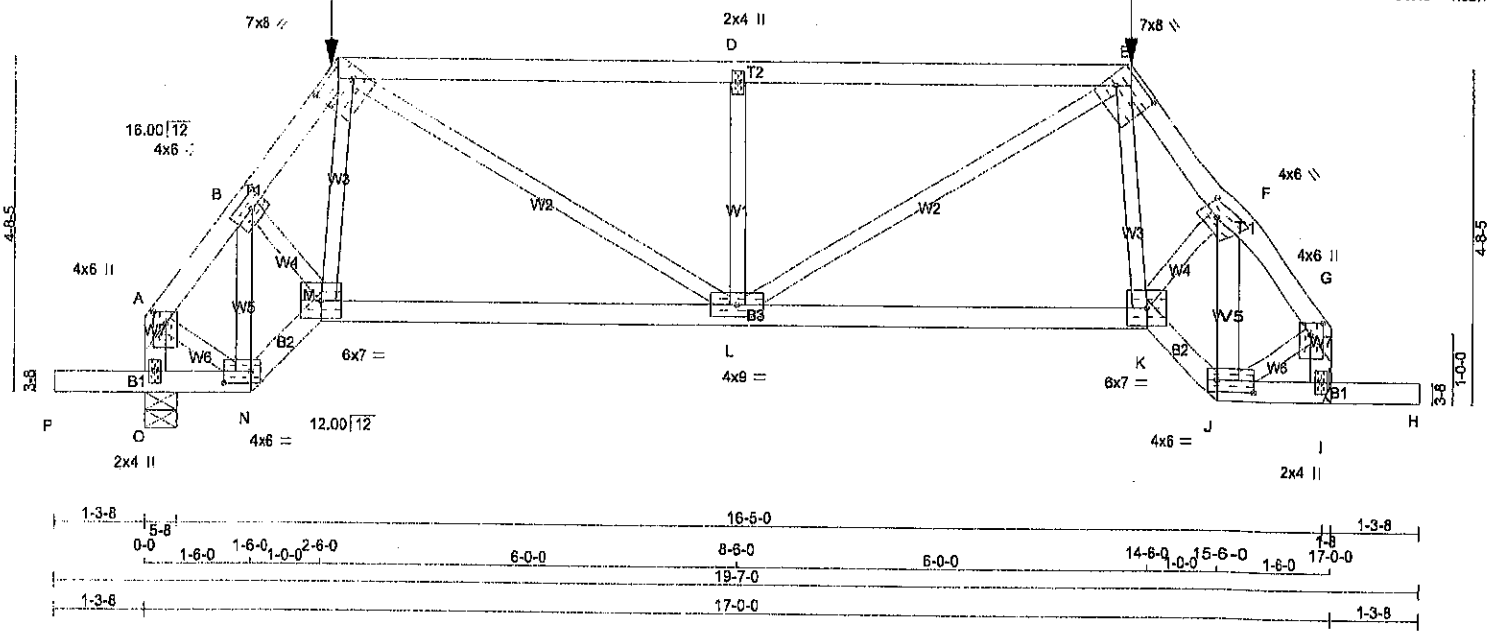
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)



A-15073773



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW+p	MT20	4.0	6.0	2.00	2.00
B - TMVW-l	MT20	4.0	6.0	2.00	2.50
C - TTWW-h	MT20	7.0	8.0	3.50	8.25
D - TMVW-w	MT20	2.0	4.0		
E - TTWW-h	MT20	7.0	8.0	3.50	6.25
F - TMVW-l	MT20	4.0	6.0	2.00	2.50
G - TMVW+p	MT20	4.0	6.0	2.00	2.00
I - BMV1+p	MT20	2.0	4.0		
J - BBWW-l	MT20	4.0	6.0	2.00	4.50
K - BBWW-l	MT20	6.0	7.0		
L - BMVWW-l	MT20	4.0	9.0		
M - BBWW-l	MT20	6.0	7.0		
N - BBWW-l	MT20	4.0	6.0	2.00	4.50
O - BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
O	1555	0	1555	0	5-8	1-10
J	1555	0	1555	0	0	HANGER BY OTHERS

MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1096	738 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
J	1096	738 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	-1246 / 0	-70.5	-70.5	0.06 (1)	5.68	N-B	-1021 / 0	0.19 (1)	
B-C	-1785 / 0	-70.5	-70.5	0.08 (1)	4.92	B-M	0 / 806	0.15 (1)	
C-D	-2074 / 0	-70.5	-70.5	0.54 (1)	4.09	M-C	0 / 488	0.12 (1)	
D-E	-2074 / 0	-70.5	-70.5	0.54 (1)	4.09	C-L	0 / 1210	0.30 (1)	
E-F	-1785 / 0	-70.5	-70.5	0.08 (1)	4.92	L-D	-464 / 0	0.11 (1)	
F-G	-1246 / 0	-70.5	-70.5	0.06 (1)	5.68	L-E	0 / 1210	0.30 (1)	
O-A	-1366 / 0	0.0	0.0	0.15 (1)	6.91	E-K	0 / 468	0.12 (1)	
I-G	-1367 / 0	0.0	0.0	0.15 (1)	6.91	K-F	0 / 608	0.15 (1)	
						J-F	-1021 / 0	0.19 (1)	
P-O	0 / 0	-88.0	-88.0	0.11 (1)	10.00	A-N	0 / 805	0.20 (1)	
O-N	0 / 0	-34.5	-34.5	0.11 (1)	10.00	J-G	0 / 806	0.20 (1)	
N-M	0 / 988	-34.5	-34.5	0.18 (1)	10.00				
M-L	0 / 1027	-102.7	-102.7	0.84 (1)	10.00				
L-K	0 / 1027	-102.7	-102.7	0.84 (1)	10.00				
K-J	0 / 988	-34.5	-34.5	0.18 (1)	10.00				
J-I	0 / 0	-34.5	-34.5	0.11 (1)	10.00				
I-H	0 / 0	-88.0	-88.0	0.11 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	2-8-12	-140	-140	---	FRONT	VERT	TOTAL
E	14-3-4	-140	-140	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 2-8-11
RIGHT SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL. LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-8-12 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 2-8-11
RIGHT SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL. LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-8-12 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CSldGlder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 14-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/908 (0.22")

CANTILEVER DEFLECTION
A-1073272
CONTINUED ON PAGE 2



HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 139.9 lbs FACTORED DOWN AT 2-8-12, AND 140.2 lbs FACTORED DOWN AT 14-3-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.
- NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 898 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.54 (D-E:1), BC=0.64 (K-L:1), WB=0.30 (C-L:1), SSI=0.33 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (J) (INPUT = 0.90)
JSI METAL= 0.35 (E) (INPUT = 1.00)



A-15073772(2)

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H04T

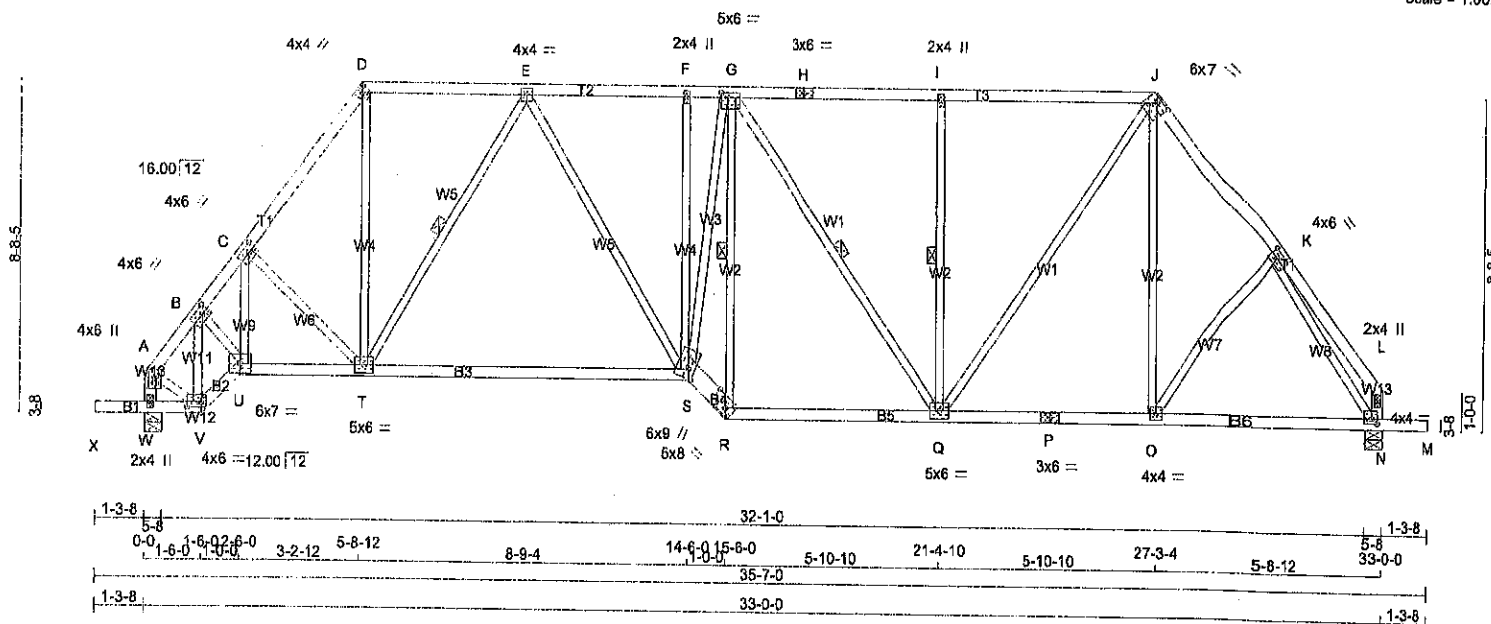
QUANTITY

PLY

JOB DESC.

ORWG NO.

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 MITEK Industries, Inc. Thu Jul 16 11:27:57 2015 Page 1
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Scale = 1:60.9

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

LUMBER

DESCR.

SPF

FACTORED

GROSS REACTION

MAXIMUM FACTORED

INPUT

REQD

BRG

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

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ALL WEBS

EXCEPT

DRY: SEASONED LUMBER.

DRY: SEASONED LUMBER.

DRY: SEASONED LUMBER.

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DRY: SEASONED LUMBER.

DRY: SEASONED LUMBER.

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

MAXIMUM FACTORED

INPUT

REQD

BRG

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

IN-SX

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UNFACTORED REACTIONS

1ST CASE

MAX. MIN. COMPONENT REACTIONS

JT

W

N

W

N

W

N

W

N

W

N

W

N

W

N

W

N

W

N

W

N

W

N

W

N

W

N

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N

W

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W

N

W

N

W

N

W

N

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 4.88FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-T, G-R, G-Q, I-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

MEMB.

FORCE

(LBS)

FR-TO

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

K-L

L-M

N-L

X-W

W-V

V-U

U-T

T-S

S-R

R-Q

Q-P

P-O

O-N

N-M

X-W

W-V

V-U

U-T

T-S

S-R

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 4.88FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-T, G-R, G-Q, I-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

MEMB.

FORCE

(LBS)

FR-TO

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

K-L

L-M

N-L

X-W

W-V

V-U

U-T

T-S

S-R

R-Q

Q-P

P-O

O-N

N-M

X-W

W-V

V-U

U-T

T-S

S-R

WEBS

MAX. FACTORED

MEMB.

FORCE

(LBS)

FR-TO

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

K-L

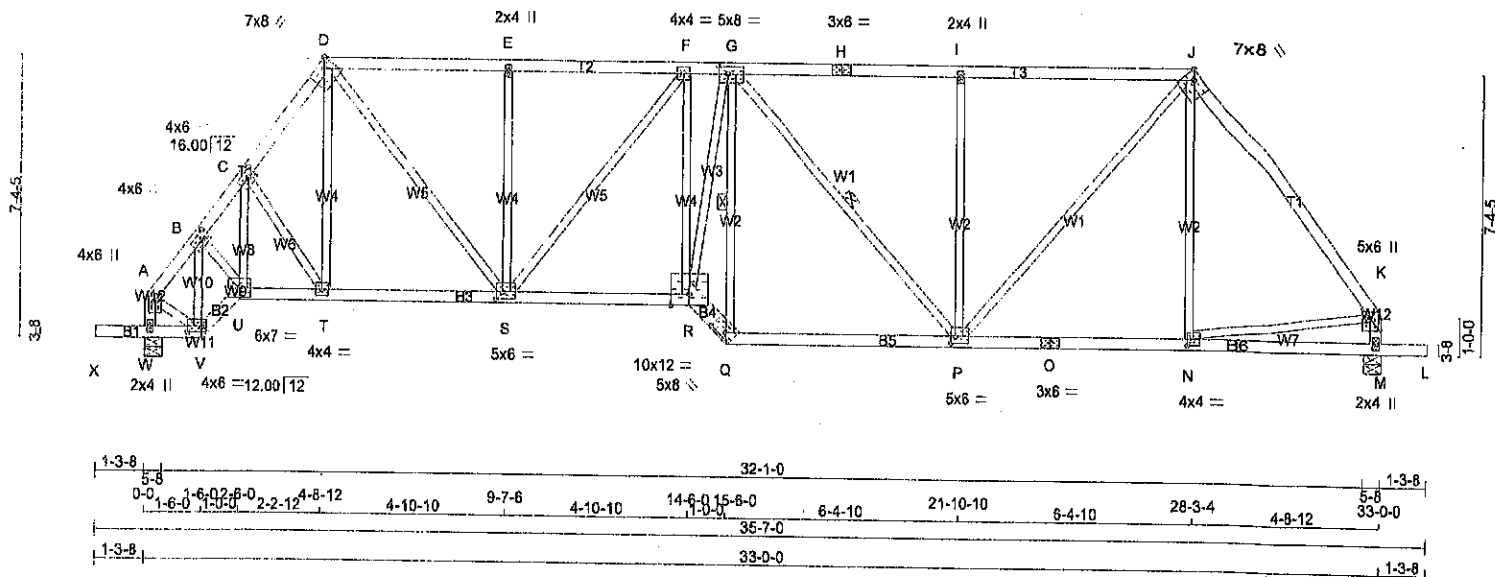
L-M

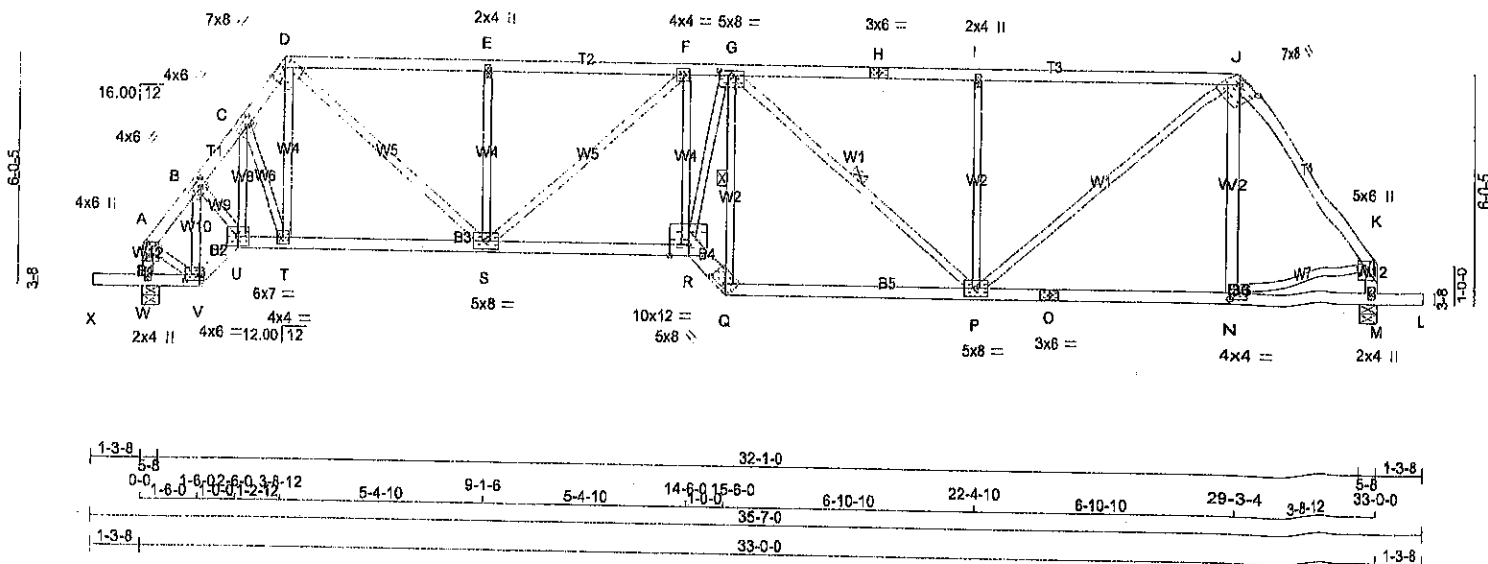
N-L

X-W

W-V

V-U





LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
W - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x6	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	2.00 2.25
B	TMVW-i	MT20	4.0	6.0	2.00 2.25
C	TMVW-i	MT20	4.0	6.0	2.00 2.50
D	TTWV-h	MT20	7.0	8.0	1.75 5.00
E	TMVW-h	MT20	2.0	4.0	
F	TMVW-i	MT20	4.0	4.0	
G	TMVWV-i	MT20	5.0	8.0	1.50 4.00
H	TS-i	MT20	3.0	6.0	
I	TMVW-h	MT20	2.0	4.0	
J	TTWV-h	MT20	7.0	8.0	1.75 5.00
K	TMVW+p	MT20	5.0	6.0	2.00 2.50
M	BMVW-i	MT20	2.0	4.0	
N	BMVW-i	MT20	4.0	4.0	2.00 1.50
O	BS-i	MT20	3.0	6.0	
P	BMVWV-i	MT20	5.0	8.0	
Q	BBWV-h	MT20	5.0	8.0	1.50 4.75
R	BBWV-i	MT20	10.0	12.0	Edge
S	BMVWV-i	MT20	5.0	8.0	
T	BMVWV-i	MT20	4.0	4.0	
U	BBWV-i	MT20	6.0	7.0	
V	BBWV-i	MT20	4.0	6.0	1.75 4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT RECD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
JT	1567	0	1567	0	5-8	1-11
M	1563	0	1583	0	5-8	1-10

UNFACTORED REACTIONS

	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
JT	1104	748 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0
M	1101	748 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-Q, G-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MIN. (LBS)	MAX. FORCE (LBS)	MIN. (LBS)
FR-TO						
A-B	-1271 / 0	-70.5	-70.5	0.05 (1)	5.68	V-B -1084 / 0
B-C	-1833 / 0	-70.5	-70.5	0.07 (1)	4.91	B-U 0 / 852
C-D	-1715 / 0	-70.5	-70.5	0.06 (1)	5.05	U-C 0 / 204
D-E	-2088 / 0	-70.5	-70.5	0.42 (1)	4.31	C-T -219 / 0
E-F	-2068 / 0	-70.5	-70.5	0.41 (1)	4.30	T-D 0 / 267
F-G	-2543 / 0	-70.5	-70.5	0.34 (1)	3.97	D-S 0 / 1388
G-H	-1892 / 0	-70.5	-70.5	0.65 (1)	4.16	S-E -432 / 0
H-I	-1892 / 0	-70.5	-70.5	0.65 (1)	4.16	E-F -829 / 0
I-J	-1892 / 0	-70.5	-70.5	0.66 (1)	4.16	F-R 0 / 346
J-K	-1431 / 0	-70.5	-70.5	0.22 (1)	5.24	R-G 0 / 1900
W-A	-1391 / 0	0.0	0.0	0.14 (1)	6.90	Q-G -2018 / 0
M-K	-1429 / 0	0.0	0.0	0.14 (1)	6.83	G-P -328 / 0
X-W	0 / 0	-88.0	-88.0	0.10 (1)	10.00	P-I -542 / 0
V-U	0 / 0	-17.5	-17.5	0.10 (1)	10.00	I-J 0 / 1348
U-T	0 / 1009	-17.5	-17.5	0.17 (1)	10.00	N-J -46 / 83
T-S	0 / 1019	-17.5	-17.5	0.23 (1)	10.00	A-V 0 / 824
S-R	0 / 2535	-17.5	-17.5	0.22 (1)	10.00	N-K 0 / 886
R-Q	0 / 2878	-17.5	-17.5	0.47 (1)	10.00	
Q-P	0 / 2141	-17.5	-17.5	0.35 (1)	10.00	
P-O	0 / 852	-17.5	-17.5	0.27 (4)	10.00	
O-N	0 / 852	-17.5	-17.5	0.27 (4)	10.00	
N-M	0 / 0	-17.5	-17.5	0.11 (4)	10.00	
M-L	0 / 0	-88.0	-88.0	0.10 (1)	10.00	

TOTAL WEIGHT = 154 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.10")
CALCULATED VERT. DEFL.(LL) = L/988 (0.16")
ALLOWABLE DEFL.(TL) = L/380 (1.10")
CALCULATED VERT. DEFL.(TL) = L/988 (0.29")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/989 (0.01")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/660 (0.02")

CSI: TC=0.66 (I-J:1), BC=0.47 (R-S:1), WB=0.60 (F-S:1), SSI=0.23 (I-J:1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

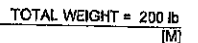
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.80 (G) (INPUT = 0.80)
JSI METAL=0.41 (R) (INPUT = 1.00)



A-15073769



FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	30-3-4	-140	-140	---	FRONT	VERT	TOTAL
S	17-1-8	-1889	-1889	---	FRONT	VERT	TOTAL

COMPANIONATIVE LOAD FACTOR = 1.00
A-1073768
CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253620	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y7fa2EXF7DRQxSg4zMEz1-7HugS3EqNd4myyVhVVDNpVwxNsg8lfor3uP2sJryxV.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMWW-h	MT20	10.0	16.0	6.75	9.50
V	BMWW-h	MT20	5.0	14.0	2.25	6.50
W	BMWW-h	MT20	6.0	7.0	2.50	2.00
X	BSWW-h	MT20	6.0	9.0		
Y	BSWW-h	MT20	7.0	8.0	2.00	5.25
Z	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 140.2 lbs FACTORED DOWN AT 30-3-4
ON TOP CHORD, AND 1888.9 lbs FACTORED
DOWN AT 17-1-8 ON BOTTOM CHORD. DESIGN
FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

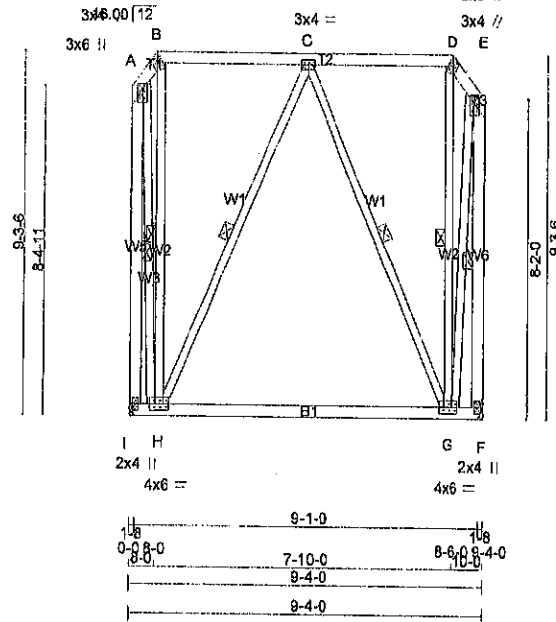
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (C) (INPUT = 0.90)
JSI METAL = 0.99 (S) (INPUT = 1.00)



A-15073768(2)



LUMBER			
N. L. G. A.	RULES		
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2 SPF
B - D	2x4	DRY	No.2 SPF
D - E	2x4	DRY	No.2 SPF
I - A	2x4	DRY	No.2 SPF
F - E	2x4	DRY	No.2 SPF
I - F	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES		(table is in inches)			
JT	TYPE	PLATES	W	LEN	Y X
A	TMWV+p	MT20	3.0	6.0	2.00 1.25
B	TTW+m	MT20	3.0	4.0	Edge
C	MTWV-l	MT20	3.0	4.0	
D	TTW+m	MT20	3.0	4.0	Edge
E	TMWV+p	MT20	3.0	6.0	2.00 1.25
F	BMV/1+p	MT20	2.0	4.0	
G	BMWVWV-l	MT20	4.0	6.0	
H	BMWVWV-l	MT20	4.0	6.0	
I	BMV/1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-XX	IN-XX
I	410	0	410	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	410	0	410	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	289	196 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
F	289	196 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF B-H, C-H, C-G, D-G, A-I, E-F.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-87 / 0	-70.5	-70.5	0.01 (1)	6.25	H-B	-74 / 11
B-C	-36 / 0	-70.5	-70.5	0.18 (1)	6.25	H-C	-196 / 0
C-D	-45 / 0	-70.5	-70.5	0.18 (1)	6.25	C-G	-177 / 0
D-E	-79 / 0	-70.5	-70.5	0.01 (1)	6.25	G-D	-70 / 13
I-A	-481 / 0	0.0	0.0	0.16 (1)	8.25	A-H	0 / 404
F-E	-474 / 0	0.0	0.0	0.15 (1)	6.25	G-E	0 / 384
I-H	0 / 0	-17.5	-17.5	0.17 (4)	10.00		
H-G	0 / 114	-17.5	-17.5	0.18 (4)	10.00		
G-F	0 / 0	-17.5	-17.5	0.17 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

{55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.31")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.31")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI: TC=0.18 (C-D:1), BC=0.18 (G-H:4),
WB=0.13 (C-H:1), SSI=0.13 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (H) (INPUT = 0.90)
JSI METAL= 0.12 (H) (INPUT = 1.00)



A-15073767

253619

H15

QUANTITY

PLY

JOB DESC.

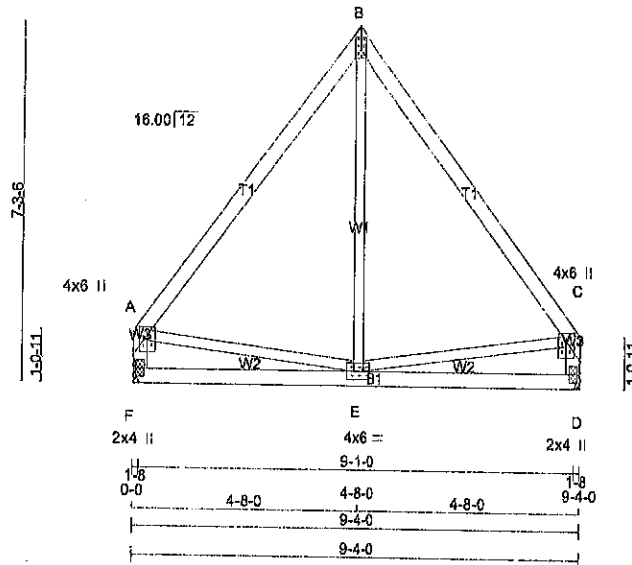
DRWG NO.

Alpa Roof Truss, Maple

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3x5 II

Scale = 1:47.1



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	BMV1+p	MT20	2.0	4.0		
E	BMVWW-L	MT20	4.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT 8RG	REQD 8RG
JT VERT	DOWN	UP	IN-SX
F	410	0	0
D	410	0	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8
HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F	289 196 / 0 0 / 0 0 / 0 93 / 0 0 / 0
D	289 196 / 0 0 / 0 0 / 0 93 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX MAX MEMB. FORCE (LBS)	MAX (LBS)
FR-TO		FROM TO	CSi (LC) UNBRAC	FR-TO
A-B	-242 / 0	-70.5 -70.5 0.20 (1)	6.25	E-B 0 / 99 0.03 (4)
B-C	-242 / 0	-70.5 -70.5 0.20 (1)	6.25	A-E 0 / 147 0.03 (1)
F-A	-379 / 0	0.0 0.0 0.04 (1)	7.81	E-C 0 / 147 0.03 (1)
D-C	-379 / 0	0.0 0.0 0.04 (1)	7.81	
F-E	0 / 0	-17.5 -17.5 0.11 (4)	10.00	
E-D	0 / 0	-17.5 -17.5 0.11 (4)	10.00	

TOTAL WEIGHT = 43 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 21.0 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 31.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")CSi: TC=0.20 (B-C:1), BC=0.11 (D-E:4),
WB=0.03 (B-E:4), SS=0.08 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 518 354	1667 822	2284 1655

PLATE PLACEMENT TOL. = 0.260 inches

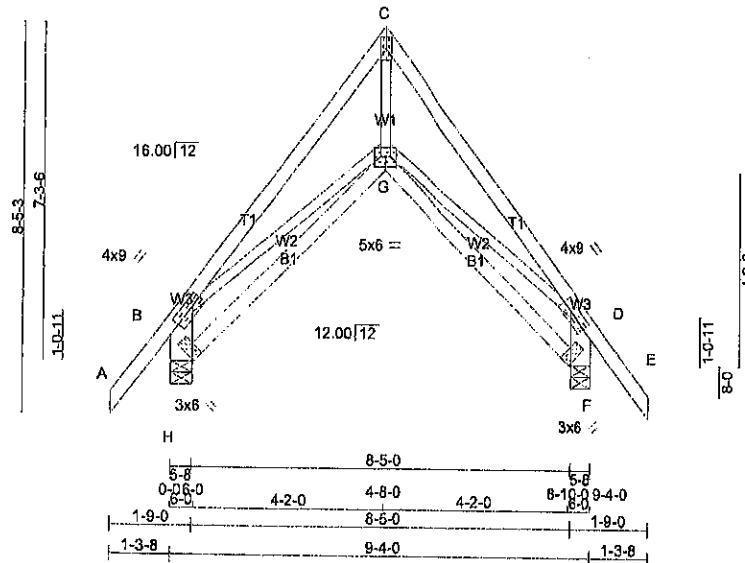
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)

JSI METAL= 0.07 (C) (INPUT = 1.00)



A-1073766



LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	9.0	2.00	3.00
C	TTW+p	MT20	3.0	6.0		
D	TMVW-I	MT20	4.0	9.0	2.00	3.00
F	BVM1+H	MT20	3.0	6.0	Edge 0.25	
G	BBWWW+p	MT20	6.0	6.0	2.75	3.00
H	BVM1+H	MT20	3.0	6.0	Edge 0.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	510	0	510	0	5-8
F	510	0	510	0	6-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	357	266 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
F	357	265 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F
 BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)
 BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (L/C)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 40	-70.5 -70.5	0.10 (1)	10.00	G-C	0 / 630	0.14 (1)
B-C	-599 / 0	-70.5 -70.5	0.21 (1)	6.25	B-G	0 / 452	0.10 (1)
C-D	-599 / 0	-70.5 -70.5	0.21 (1)	6.25	G-D	0 / 452	0.10 (1)
D-E	0 / 40	-70.5 -70.5	0.10 (1)	10.00			
H-B	-469 / 0	0.0 0.0	0.03 (1)	7.81			
F-D	-469 / 0	0.0 0.0	0.03 (1)	7.81			
H-G	0 / 0	-17.5 -17.5	0.11 (4)	10.00			
G-F	0 / 0	-17.5 -17.5	0.11 (4)	10.00			

TOTAL WEIGHT = 4 X 53 = 211 lb [M](F)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(66 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.31")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.21 (B-C:1), BC=0.11 (G-H:4),
 WB=0.14 (C-G:1), SS=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1656

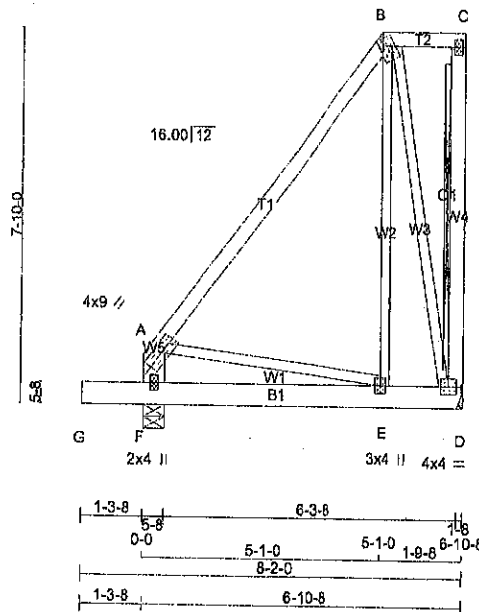
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.47 (G) (INPUT = 0.90)
 JSI METAL= 0.13 (B) (INPUT = 1.00)



A-15073765

**LUMBER**

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2 SPF
B - C	2x4	DRY	No.2 SPF
D - C	2x4	DRY	No.2 SPF
F - A	2x6	DRY	No.2 SPF
G - D	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	12	TOP
B - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - D	12	SIDE(80.6)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	9.0	2.00 3.00
B	TTWW+m	MT20	4.0	6.0	1.75 1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	857	857	0	0
F	970	970	0	0

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	804	409 / 0	0 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0
F	684	463 / 0	0 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT C-D FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX	LC2 MAX
FR-TO											
A-B	-316 / 0	-70.5	-70.5	0.17 (1)	8.25	10.00	8.25	E-B	0 / 838	0.10 (1)	10.00
B-C	0 / 0	-70.5	-70.5	0.02 (1)	10.00	10.00	10.00	B-D	-852 / 0	0.49 (1)	10.00
D-C	-63 / 0	0.0	0.0	0.01 (1)	7.81	7.81	7.81	A-E	0 / 189	0.02 (1)	10.00
F-A	-453 / 0	0.0	0.0	0.02 (1)	7.81	7.81	7.81				
G-F	0 / 0	-88.0	-88.0	0.03 (1)	10.00	10.00	10.00				
F-E	0 / 0	-178.7	-178.7	0.17 (1)	10.00	10.00	10.00				
E-D	0 / 200	-178.7	-178.7	0.13 (1)	10.00	10.00	10.00				

TOTAL WEIGHT = 2 X 52 = 103 lb [M]

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	21.0	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 9-4-0
END DISTANCE = 6-10-8
END SPAN CARRIED = 9-4-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF G.S.L.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/899 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.17 (A-B:1), BC=0.17 (E-F:1), WB=0.49 (B-D:1), SSI=0.17 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 DGS



A 10073764

253619

H12

QUANTITY

PLY

JOB DESC.

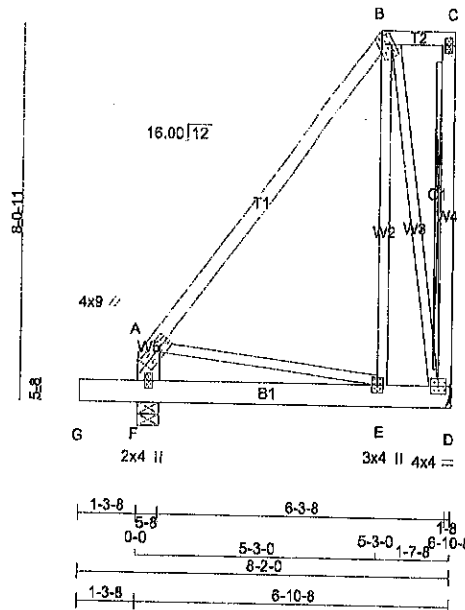
DRWG NO.

Alpa Roof Truss, Maple

TRUSS DESC.

Version 7.620 S Apr 15 2015 Mittek Industries, Inc. Thu Jul 16 11:18:18 2015 Page 1
ID: AyrJx9Y7fa2EXF7DRQxSq4zMEz1-8Xg_JJGshxzBbk4nHLVkr1LPO_oD6Fh0eDAD_yxWn3
4x6 \\\\ 2x4 \\\\

Scale = 1:49.9

**LUMBER**

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - A	2x6	DRY No.2	SPF
G - D	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	TOP
B-C 1	12	TOP
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G-D 2	12	SIDE(80.6)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMW-t	MT20	4.0	9.0	2.00	3.00
B TTWW+m	MT20	4.0	8.0	1.75	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
D	VERT 857	HORZ 0	DOWN 857	HORZ 0
F	VERT 970	HORZ 0	DOWN 970	HORZ 0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	604	409 / 0	0 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0
F	684	463 / 0	0 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT C-D FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX (CSI (LC))	MAX UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (CSI (LC))
FR-TO						FR-TO			
A-B	-293 / 0	-70.5	-70.5	0.19 (1)	6.25	E-B	0 / 873	0.11 (1)	
B-C	0 / 0	-70.5	-70.5	0.02 (1)	10.00	B-D	-899 / 0	0.55 (1)	
D-C	-57 / 0	0.0	0.0	0.01 (1)	7.81	A-E	0 / 176	0.02 (1)	
F-A	-439 / 0	0.0	0.0	0.02 (1)	7.81				
G-F	0 / 0	-88.0	-88.0	0.03 (1)	10.00				
F-E	0 / 0	-178.7	-178.7	0.18 (1)	10.00				
E-D	0 / 187	-178.7	-178.7	0.13 (1)	10.00				

TOTAL WEIGHT = 2 X 52 = 105 lb [M]

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH	LL	=	21.0	PSF
DL	=	3.0	PSF	
BOT CH	LL	=	0.0	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 9-4-0
END DISTANCE = 6-10-8
END SPAN CARRIED = 9-4-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.19 (A-B:1), BC=0.18 (E-F:1), WB=0.55 (B-D:1), SSI=0.18 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

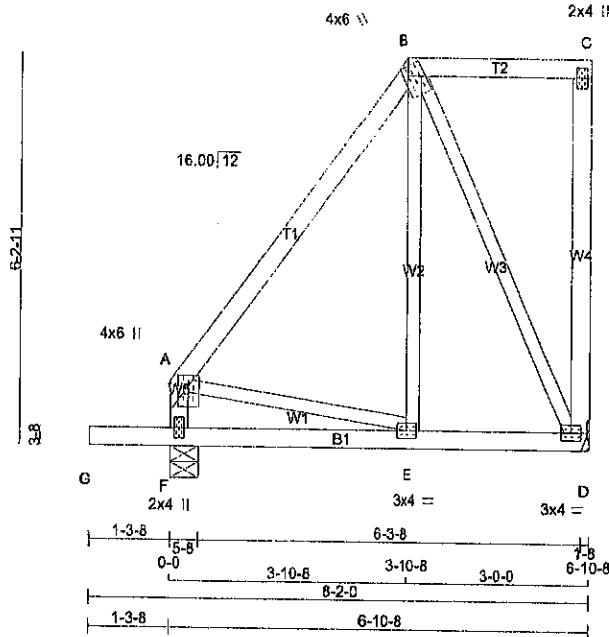
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	622	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

A-NOT 3763





LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	4.0	6.0	1.75	1.00
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	3.0	4.0		
E	BMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
D	302	0	302	0
F	418	0	418	0

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	213	144/0	0/0	0/0	0/0	69/0	0/0
F	293	198/0	0/0	0/0	0/0	95/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-152/0	-70.6 -70.6	0.14 (1)	E-B	0/79	0.03 (4)	
B-C	0/0	-70.6 -70.6	0.11 (1)	B-D	-201/0	0.15 (1)	
D-C	-108/0	0.0 0.0	0.06 (1)	A-E	0/92	0.02 (1)	
F-A	-273/0	0.0 0.0	0.03 (1)				
G-F	0/0	-88.0 -88.0	0.10 (1)				
F-E	0/0	-17.5 -17.5	0.06 (4)				
E-D	0/91	-17.5 -17.5	0.07 (4)				

TOTAL WEIGHT = 2 X 40 = 81 lb

(M/JF)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14 (A-B:1), BC=0.10 (F-G:1), WB=0.15 (B-D:1), SSI=0.09 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	618	354
	1667	822
	2284	1658

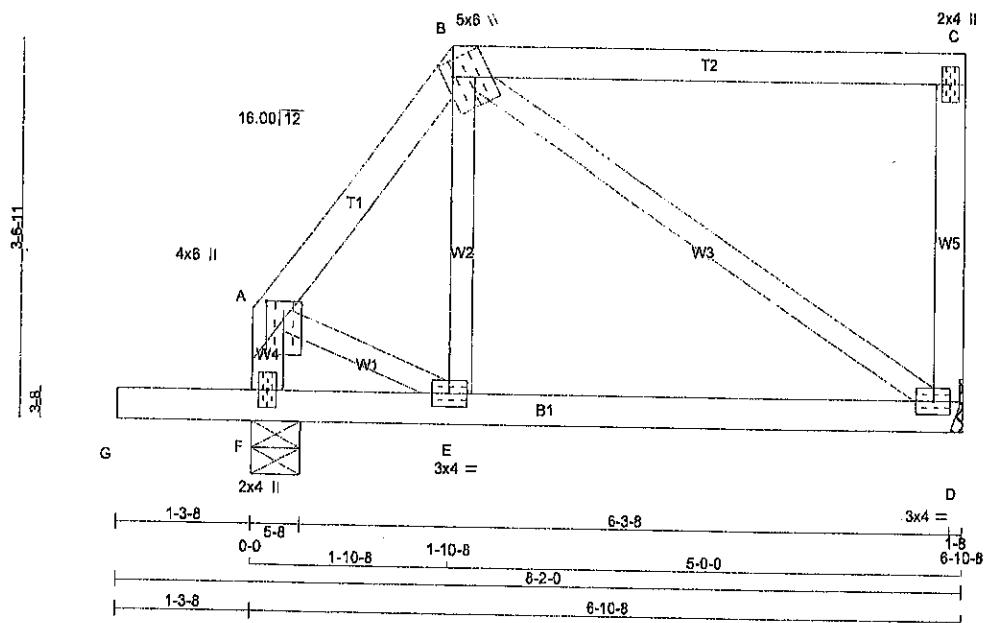
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)



A-1073762



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - A	2x4	DRY No.2	SPF
G - D	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	4.0	6.0	2.00 2.00
B	TTWW+m	MT20	6.0	6.0	1.75 1.00
C	TMV+p	MT20	2.0	4.0	
D	BMV/W1-t	MT20	3.0	4.0	
E	BMV/W1-t	MT20	3.0	4.0	
F	BMV1+p	MT20	2.0	4.0	

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT D	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	302	0	302	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	416	0	416	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	213	144 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
F	293	198 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT,
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT, OR RIGID CEILING DIRECTLY
APPLIED

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-235 / 0	-70.5	-70.5 0.04 (1)	6.25	E-B	0 / 79	0.03 (4)
B-C	0 / 0	-70.5	-70.5 0.30 (1)	10.00	B-D	-167 / 0	0.09 (1)
D-C	-178 / 0	0.0	0.0 0.03 (1)	7.81	A-E	0 / 147	0.03 (1)
F-A	-303 / 0	0.0	0.0 0.03 (1)	7.81			
G-F	0 / 0	-88.0	-88.0 0.10 (1)	10.00			
F-E	0 / 0	-17.5	-17.5 0.09 (4)	10.00			
E-D	0 / 141	-17.5	-17.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL	=	21.0 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.0 PSF
TOTAL LOAD			=	31.0 PSF

SPACING = 24.0 IN, C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= $L/120$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL)= $L/120$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.30 (B-C:1), BC=0.10 (D-E:4),
WB=0.09 (B-D:1), SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	818	354	1087	822	2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

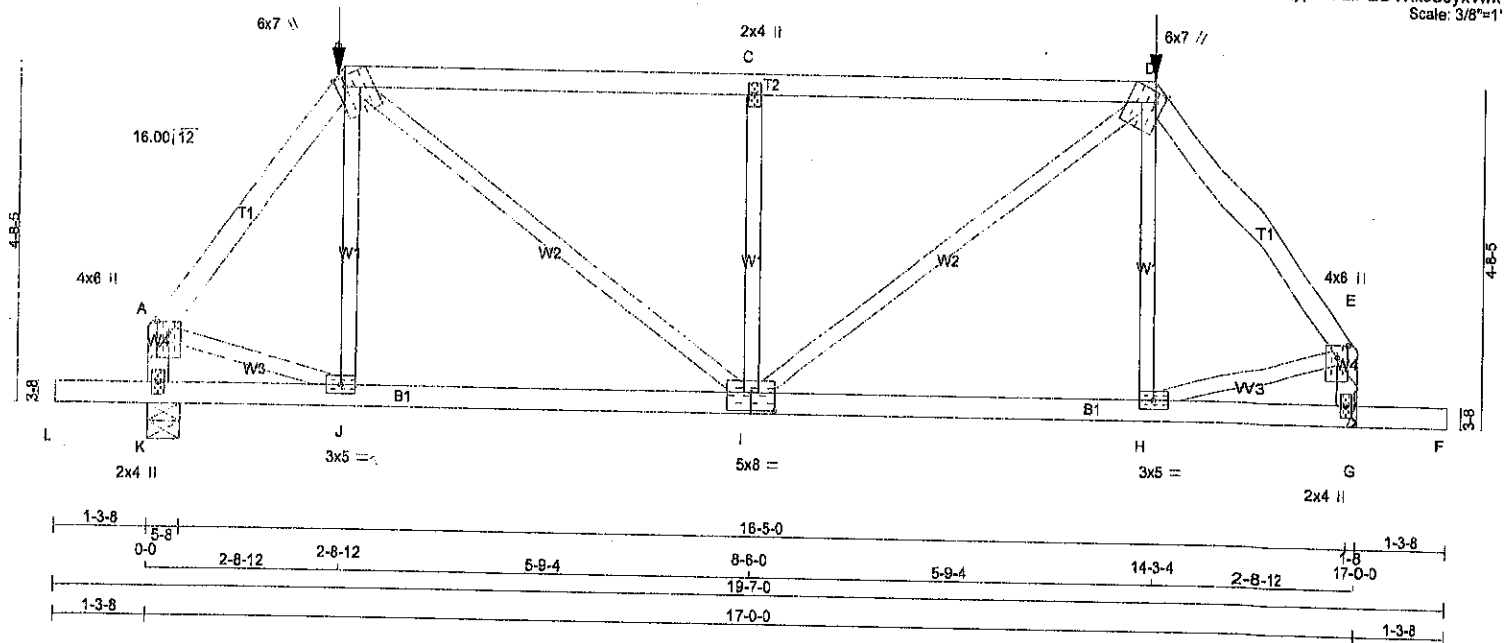
JSI GRIP= 0.24 (E) (INPUT = 0.90)
JSI METAL= 0.06 (A) (INPUT = 1.00)



A-15073761

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Thu Jul 16 11:18:16 2015 Page 1
 ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-C8YEu2EdJTMQB8ifs1eQypeaFzIFEOYKk386yxVh!
 Scale: 3/8"=1'



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-I	MT20	3.0	5.0		
I	BSWWH-I	MT20	5.0	8.0	3.00	4.00
J	BMWW-I	MT20	3.0	5.0		
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
 REQUIRED TO SUPPORT CONCENTRATED
 LOAD(S) 140.2 lbs FACTORED DOWN AT 14-3-4,
 AND 138.9 lbs FACTORED DOWN AT 2-8-12 ON
 TOP CHORD. DESIGN FOR UNSPECIFIED
 CONNECTION(S) IS DELEGATED TO THE
 BUILDING DESIGNER.

NOTES- (1)

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	1639	0	1639	0
G	1639	0	1639	0

HANGER BY OTHERS

MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1086	729/0	0/0	0/0	0/0	357/0	0/0
G	1086	729/0	0/0	0/0	0/0	357/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
 APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1383/0	-70.5 -70.5	0.13 (1)	5.38	J-B	-25/122	0.05 (4)
B-C	-1595/0	-138.7 -138.7	0.97 (1)	3.17	B-I	0/999	0.24 (1)
C-D	-1595/0	-138.7 -138.7	0.97 (1)	3.17	I-C	-987/0	0.33 (1)
D-E	-1383/0	-70.5 -70.5	0.13 (1)	5.38	I-D	0/999	0.24 (1)
K-A	-1402/0	0.0 0.0	0.16 (1)	6.84	H-D	-25/122	0.05 (4)
G-E	-1402/0	0.0 0.0	0.16 (1)	6.84	A-J	0/846	0.21 (1)
L-K	0/0	-88.0 -88.0	0.11 (1)	10.00	H-E	0/846	0.21 (1)
K-J	0/0	-34.5 -34.5	0.18 (4)	10.00			
J-I	0/822	-34.5 -34.5	0.33 (4)	10.00			
I-H	0/822	-34.5 -34.5	0.33 (4)	10.00			
H-G	0/0	-34.5 -34.5	0.19 (4)	10.00			
G-F	0/0	-88.0 -88.0	0.11 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-8-12	-140	-140	---	FRONT	VERT	TOTAL
D	14-3-4	-140	-140	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 74 lb
 (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
 SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPPrimeHip

LEFT SETBACK = 2-8-11

RIGHT SETBACK = 2-8-12

END SETBACK = 5-10-8

END WALL WIDTH = 0-0

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
 OR SMALL BUILDING REQUIREMENTS OF
 PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.57")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL) = L/360 (0.57")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = L/120 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/120 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.97 (B-C:1), BC=0.33 (H-I:4), WB=0.33
 (C-I:1), SSI=0.43 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL IN
 THE TRUSS MANUFACTURING PLANT.

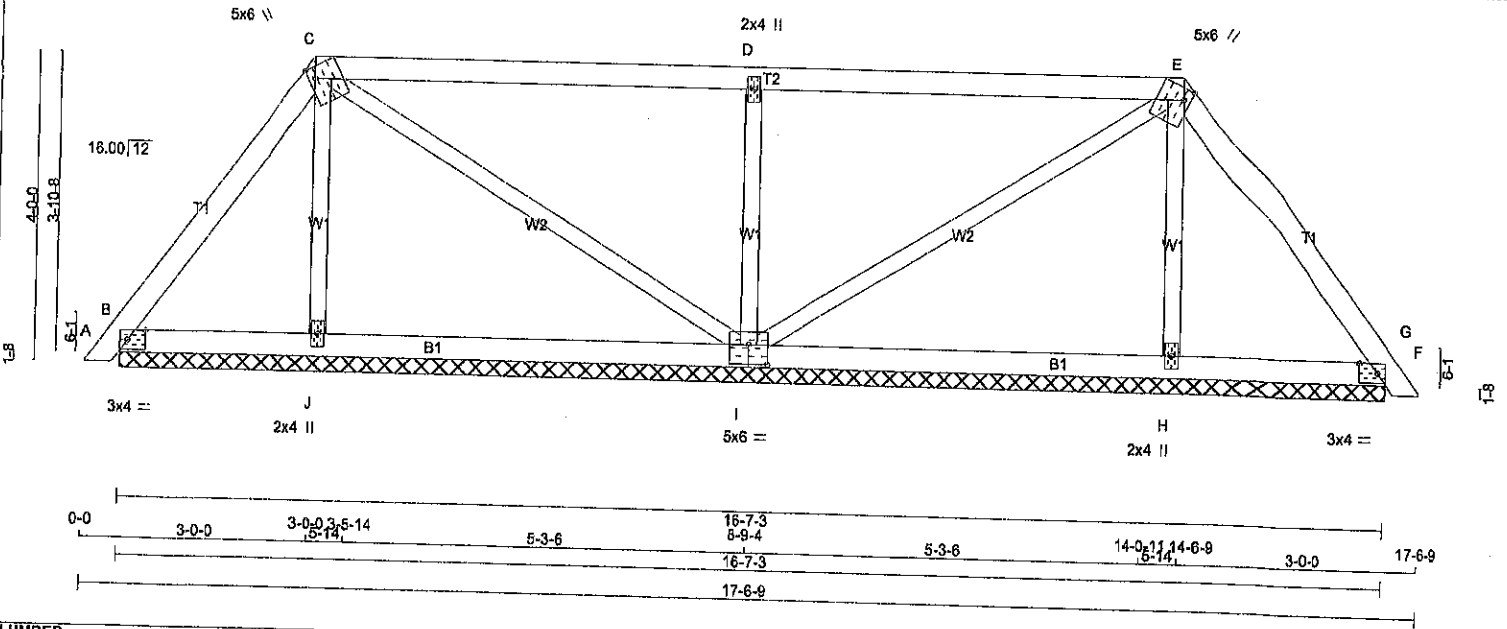
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
WT20	618 354	1867 822	2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.00 deg





LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.75
C	TTWW+M	MT20	5.0	6.0	1.75 1.00
D	TWW+M	MT20	2.0	4.0	
E	TTWW+M	MT20	5.0	6.0	1.75 1.00
F	TMB1-I	MT20	3.0	4.0	1.50 2.75
H	BMW1+M	MT20	2.0	4.0	
I	BSWW+M	MT20	5.0	6.0	3.00 3.00
J	BMW1+M	MT20	2.0	4.0	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	192	0	192	0	16-7-3 (8-3-10)8	
I	658	0	658	0	16-7-3 (8-3-10)8	
F	192	0	192	0	16-7-3 (8-3-10)8	
J	234	0	234	0	16-7-3 (8-3-10)8	
H	234	0	234	0	16-7-3 (8-3-10)8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	132	108/0	0/0	0/0	0/0	23/0
I	481	328/0	0/0	0/0	0/0	132/0
F	132	108/0	0/0	0/0	0/0	23/0
J	169	90/0	0/0	0/0	0/0	79/0
H	169	90/0	0/0	0/0	0/0	79/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I, F, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		WEBS MAX. FACTORED FORCE (LBS)	
		FROM	TO	CSI (LC)	UNBRAC LENGTH	FR-TO	MAX CSI (LC)
A-B		0/9	-70.5	-70.5	0.01 (1)	10.00	
B-C		-84/0	-70.5	-70.5	0.08 (1)	6.25	C-I -43/0 0.04 (1)
C-D		-7/0	-70.5	-70.5	0.40 (1)	10.00	I-D -508/0 0.12 (1)
D-E		-7/0	-70.5	-70.5	0.40 (1)	10.00	E-H -43/0 0.04 (1)
E-F		-84/0	-70.5	-70.5	0.08 (1)	6.25	F-G -151/0 0.04 (1)
F-G		0/9	-70.5	-70.5	0.01 (1)	10.00	G-H -151/0 0.04 (1)
B-J		0/49	-17.5	-17.5	0.08 (4)	10.00	
J-I		0/44	-17.5	-17.5	0.13 (4)	10.00	
I-H		0/44	-17.5	-17.5	0.13 (4)	10.00	
H-F		0/49	-17.5	-17.5	0.08 (4)	10.00	

TOTAL WEIGHT = 3 X 62 = 186 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(56 % OF 23.0 P.S.F. G.S.L. PLUS
6.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40 (C-D:1), BC=0.13 (H-I:4), WB=0.12 (D-I:1), SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1668

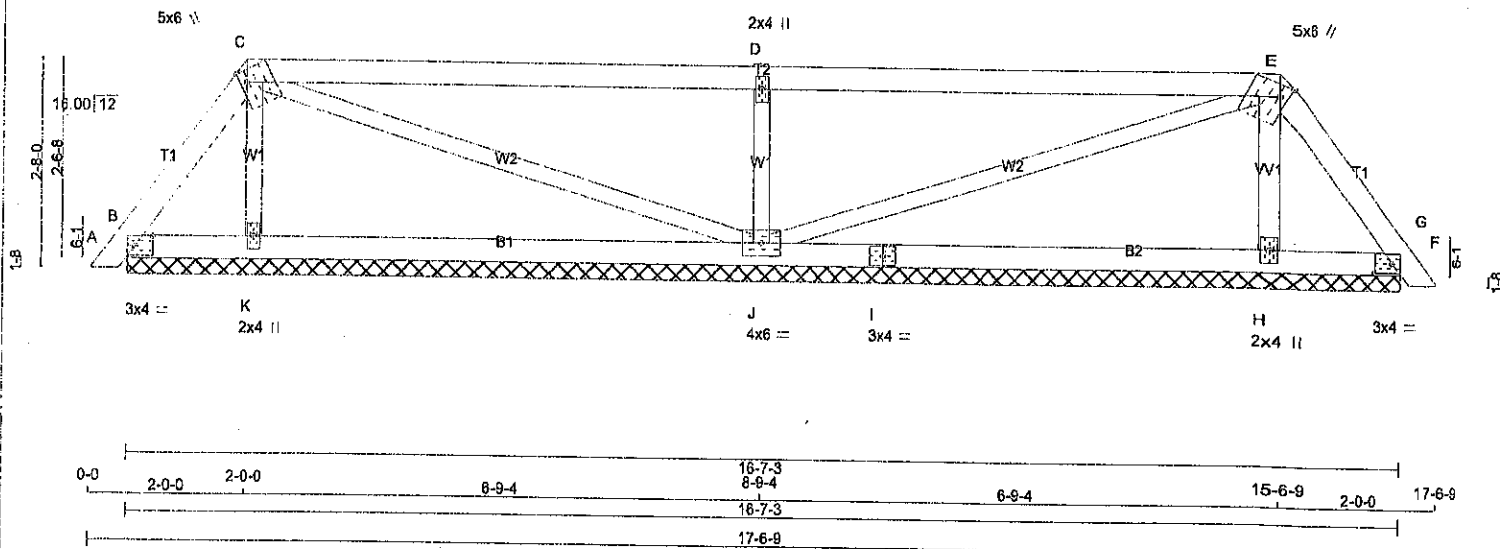
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.21 (I) (INPUT = 1.00)



A-11073759



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
B - I	2x4	DRY No.2	SPF
I - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TM81-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BS-I	MT20	3.0	4.0		
J	BMWWW1-I	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ DOWN	HORZ UPLIFT	IN-SX
JT				
B	122	0	122	0
K	266	0	266	0
J	733	0	733	0
H	266	0	266	0
F	123	0	123	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 160 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 160 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	MAX / MIN COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	81	83 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
K	193	99 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
J	614	363 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
H	193	99 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
F	81	83 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO			FROM	TO		FR-TO			
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	K-C	-166 / 0	0.03 (1)	
B-C	-74 / 0	-70.5	-70.5	0.03 (1)	6.25	C-J	-22 / 0	0.02 (1)	
C-D	-12 / 0	-70.5	-70.5	0.55 (1)	8.25	J-D	-596 / 0	0.10 (1)	
D-E	-12 / 0	-70.5	-70.5	0.55 (1)	8.25	J-E	-22 / 0	0.02 (1)	
E-F	-74 / 0	-70.5	-70.5	0.03 (1)	6.25	H-E	-166 / 0	0.03 (1)	
F-G	0 / 9	-70.5	-70.5	0.01 (1)	10.00				
B-K	0 / 42	-17.5	-17.5	0.12 (4)	10.00				
K-J	0 / 34	-17.5	-17.5	0.17 (4)	10.00				
J-I	0 / 34	-17.5	-17.5	0.17 (4)	10.00				
I-H	0 / 34	-17.5	-17.5	0.17 (4)	10.00				
H-F	0 / 42	-17.5	-17.5	0.12 (4)	10.00				

TOTAL WEIGHT = 2 X 58 = 115 lb

(M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.55 (C-D:1), BC=0.17 (J-K:4), WB=0.10 (D-J:1), SSI=0.23 (D-E:1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

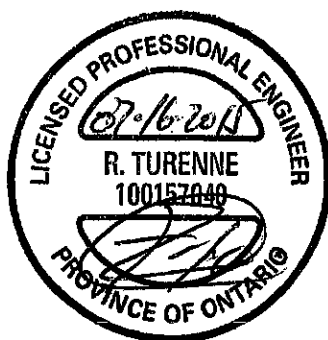
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 364 1667 822 2284 1666

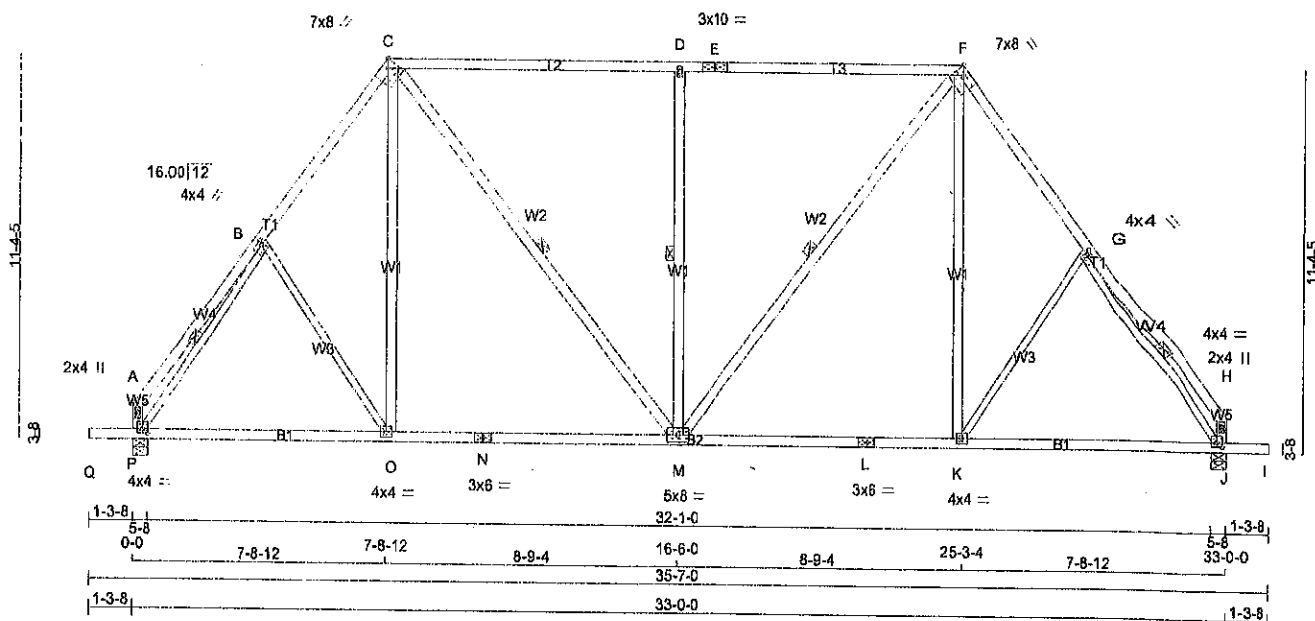
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)



A-1073758



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
B - O	2x3	DRY	No.2
K - G	2x3	DRY	No.2
P - B	2x3	DRY	No.2
G - J	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMVW-t	MT20	4.0	4.0	1.50	1.00
C	TTWW-h	MT20	7.0	8.0	Edge	2.75
D	TMVW-w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	10.0		
F	TTWW-h	MT20	7.0	8.0	Edge	2.75
G	TMVW-t	MT20	4.0	4.0	1.50	1.00
H	TMV+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.50	2.00
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT		REQD	
	GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
P	1631	0	1631	0	0	5-8	1-12	
J	1631	0	1631	0	0	5-8	1-12	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
	COMBINED		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1155	747 / 0	0 / 0	0 / 0	0 / 0	0 / 0	408 / 0	0 / 0
P	1155	747 / 0	0 / 0	0 / 0	0 / 0	0 / 0	408 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-M, D-M, F-M, B-P, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED	FACTORED	MAX. FACTORED	FACTORED
	FORCE	VERT. LOAD LC1	FORCE	FORCE
	(LBS)	(PLF)	(LBS)	(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 24	-70.5 -70.5 0.16 (1)	B-O	-116 / 8
B-C	-1422 / 0	-70.5 -70.5 0.17 (1)	O-C	0 / 257
C-D	-1229 / 0	-78.0 -78.0 0.66 (1)	C-M	0 / 630
D-E	-1230 / 0	-78.0 -78.0 0.66 (1)	M-D	-839 / 0
E-F	-1230 / 0	-78.0 -78.0 0.66 (1)	M-F	0 / 630
F-G	-1422 / 0	-70.5 -70.5 0.17 (1)	K-F	0 / 257
G-H	0 / 24	-70.5 -70.5 0.16 (1)	K-G	-116 / 8
P-A	-103 / 0	0.0 0.0 0.01 (1)	P-B	-1631 / 0
J-H	-103 / 0	0.0 0.0 0.01 (1)	G-J	-1631 / 0
Q-P	0 / 0	-88.0 -88.0 0.10 (1)		
P-O	0 / 901	-17.5 -17.5 0.36 (4)		
O-N	0 / 837	-17.5 -17.5 0.36 (4)		
N-M	0 / 837	-17.5 -17.5 0.36 (4)		
M-L	0 / 837	-17.5 -17.5 0.36 (4)		
L-K	0 / 837	-17.5 -17.5 0.36 (4)		
K-J	0 / 901	-17.5 -17.5 0.36 (4)		
J-I	0 / 0	-88.0 -88.0 0.10 (1)		

TOTAL WEIGHT = 9 X 162 = 1638 lb

(M)F

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 8.00/12
AND 6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.66 (C-D:1), BC=0.36 (K-M:4),
WB=0.67 (D-M:1), SS=0.33 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

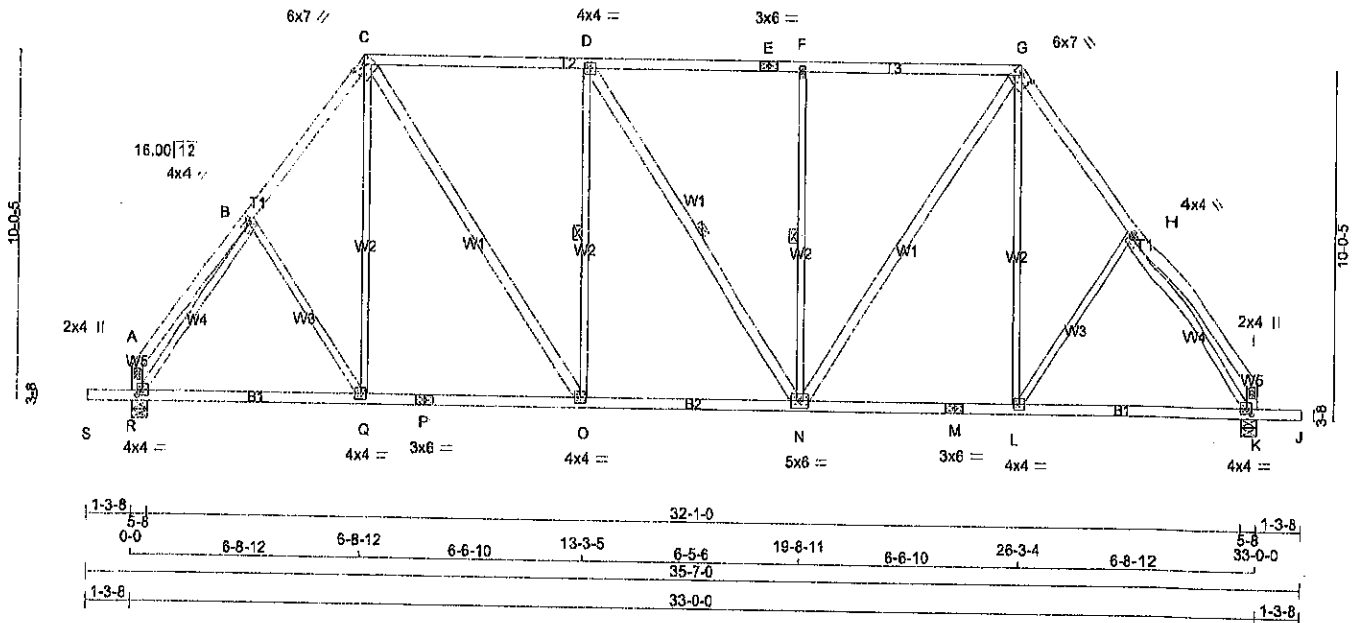
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.62 (G) (INPUT = 1.00)

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - A	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
S - P	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
C - O	2x4	DRY No.2	SPF
D - N	2x4	DRY No.2	SPF
N - G	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMV+p	MT20	2.0	4.0		
B - TMVW-t	MT20	4.0	4.0	1.50	1.00
C - TTWW-h	MT20	6.0	7.0	1.75	4.00
D - TMVW-t	MT20	4.0	4.0		
E - TS-t	MT20	3.0	6.0		
F - TMVW-h	MT20	2.0	4.0		
G - TTWW-h	MT20	6.0	7.0	1.75	4.00
H - TMVW-t	MT20	4.0	4.0	1.50	1.00
I - TMV+p	MT20	2.0	4.0		
K - BMVW-t	MT20	4.0	4.0	1.75	1.75
L - O, Q					
M - BMVW-t	MT20	4.0	4.0		
N - BS-t	MT20	3.0	6.0		
P - BMVW-t	MT20	5.0	6.0		
R - BS-t	MT20	3.0	6.0		
R - BMVW-t	MT20	4.0	4.0	1.75	1.75

NOTES- (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	1565	0	1565	0	5-8	1-10
K	1565	0	1565	0	5-8	1-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
R	1102	747 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0
K	1102	747 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.19FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-O, D-N, F-N.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	0 / 20	-70.5	-70.5	0.12 (1)	10.00	B-Q	-79 / 10	0.05 (1)	
B-C	-1377 / 0	-70.5	-70.5	0.15 (1)	5.40	Q-C	0 / 196	0.05 (4)	
C-D	-1219 / 0	-70.5	-70.5	0.48 (1)	5.19	C-O	0 / 728	0.12 (1)	
D-E	-1218 / 0	-70.5	-70.5	0.47 (1)	5.18	O-D	-493 / 0	0.34 (1)	
E-F	-1218 / 0	-70.5	-70.5	0.47 (1)	5.18	D-N	-2 / 0	0.00 (1)	
F-G	-1218 / 0	-70.5	-70.5	0.47 (1)	5.20	N-F	-492 / 0	0.34 (1)	
G-H	-1377 / 0	-70.5	-70.5	0.15 (1)	5.40	N-G	0 / 728	0.12 (1)	
H-I	0 / 20	-70.5	-70.5	0.15 (1)	10.00	L-G	0 / 197	0.05 (4)	
R-A	-91 / 0	0.0	0.0	0.01 (1)	7.81	L-H	-79 / 10	0.05 (1)	
K-I	-91 / 0	0.0	0.0	0.01 (1)	7.81	R-B	-1565 / 0	1.00 (1)	
						H-K	-1565 / 0	1.00 (1)	
S-R	0 / 0	-88.0	-88.0	0.10 (1)	10.00				
R-Q	0 / 864	-17.5	-17.5	0.25 (4)	10.00				
Q-P	0 / 811	-17.5	-17.5	0.26 (4)	10.00				
P-O	0 / 811	-17.5	-17.5	0.26 (4)	10.00				
O-N	0 / 1218	-17.5	-17.5	0.27 (1)	10.00				
N-M	0 / 811	-17.5	-17.5	0.26 (4)	10.00				
M-L	0 / 811	-17.5	-17.5	0.26 (4)	10.00				
L-K	0 / 864	-17.5	-17.5	0.25 (4)	10.00				
K-J	0 / 0	-88.0	-88.0	0.10 (1)	10.00				

TOTAL WEIGHT = 2 X 179 = 358 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 3.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (1.10")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CANTILEVER DEFLECTION:
 ALLOWABLE DEFL.(LL) = L/120 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/120 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.48 (C-D:1), BC=0.27 (N-O:1), WB=1.00 (H-K:1), SS=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1856

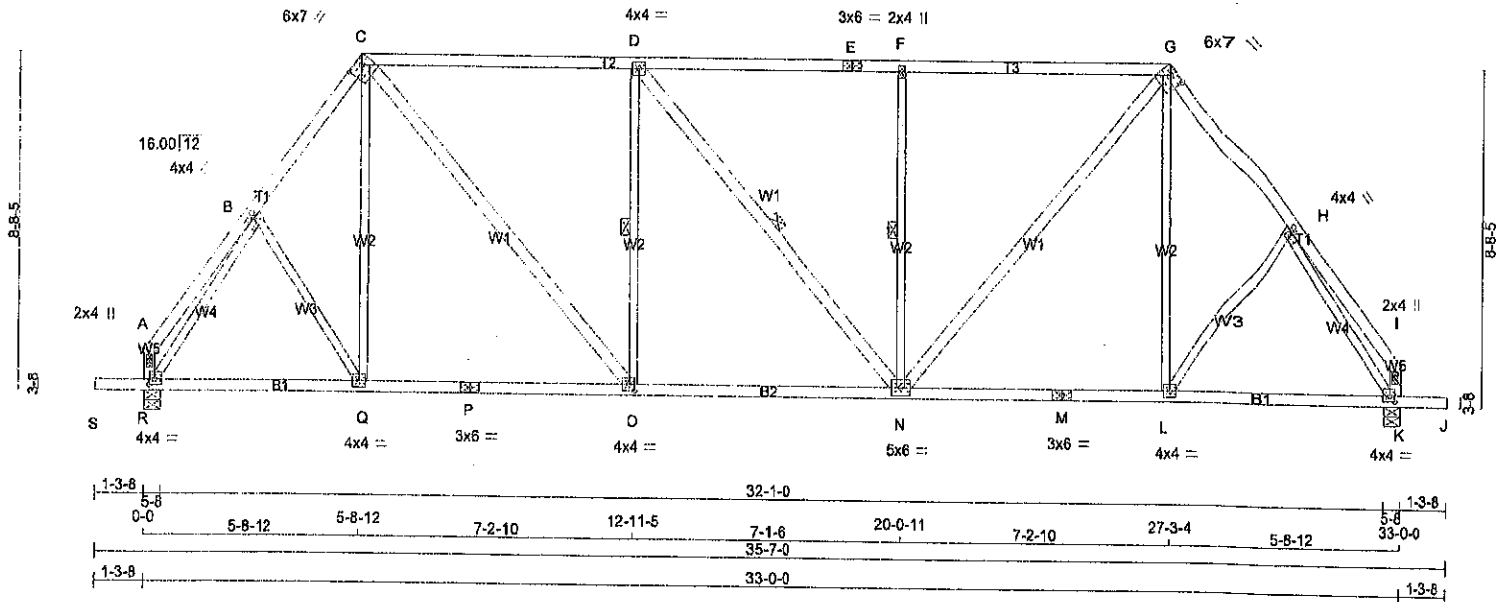
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (K) (INPUT=0.80)
 JSI METAL=0.69 (H) (INPUT=1.00)



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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
R - A	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
S - P	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
C - O	2x4	DRY	No.2	SPF	
D - N	2x4	DRY	No.2	SPF	
N - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-I	MT20	4.0	4.0	1.50	1.00
C	TTWW-h	MT20	6.0	7.0	1.75	4.00
D	TMWW-I	MT20	4.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-h	MT20	6.0	7.0	1.75	4.00
H	TMWW-I	MT20	4.0	4.0	1.50	1.00
I	TMV+p	MT20	2.0	4.0		
K	BMVW-I	MT20	4.0	4.0	1.75	1.75
L	BMWW-I	MT20	4.0	4.0		
M	BS-I	MT20	3.0	6.0		
N	BMWW-I	MT20	5.0	6.0		
O	BMWW-I	MT20	4.0	4.0	2.00	1.75
P	BS-I	MT20	3.0	6.0		
Q	BMWW-I	MT20	4.0	4.0		
R	BMVW-I	MT20	4.0	4.0	1.75	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	1665	0	1665	0	5-8	1-10
K	1665	0	1665	0	5-8	1-10

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
R	1102	747 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0
K	1102	747 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-O, D-N, F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 15	-70.5	-70.5	B-Q	-32 / 20	0.02 (1)	10.00
B-C	-1408 / 0	-70.5	-70.5	Q-C	0 / 151	0.04 (4)	10.00
C-D	-1410 / 0	-70.5	-70.5	C-O	0 / 885	0.14 (1)	10.00
D-E	-1409 / 0	-70.5	-70.5	O-D	-544 / 0	0.26 (1)	10.00
E-F	-1409 / 0	-70.5	-70.5	D-N	-1 / 0	0.00 (1)	10.00
F-G	-1409 / 0	-70.5	-70.5	N-G	-544 / 0	0.26 (1)	10.00
G-H	-1408 / 0	-70.5	-70.5	F-H	0 / 894	0.14 (1)	10.00
H-I	0 / 15	-70.5	-70.5	I-G	0 / 152	0.04 (4)	10.00
R-A	-80 / 0	0.0	0.0	L-H	-32 / 20	0.02 (1)	10.00
K-I	-80 / 0	0.0	0.0	R-B	-1679 / 0	0.71 (1)	10.00
S-R	0 / 0	-88.0	-88.0	H-K	-1679 / 0	0.71 (1)	10.00
R-Q	0 / 848	-17.5	-17.5				
Q-P	0 / 831	-17.5	-17.5				
P-O	0 / 831	-17.5	-17.5				
O-N	0 / 1410	-17.5	-17.5				
N-M	0 / 831	-17.5	-17.5				
M-L	0 / 831	-17.5	-17.5				
L-K	0 / 848	-17.5	-17.5				
K-J	0 / 0	-88.0	-88.0				

TOTAL WEIGHT = 2 X 168 = 337 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.62 (C-D:1), BC=0.32 (N-Q:1),
WB=0.71 (H-K:1), SS=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
MAX	MIN	MAX
MT20	618	354
	1667	822
	2284	1658

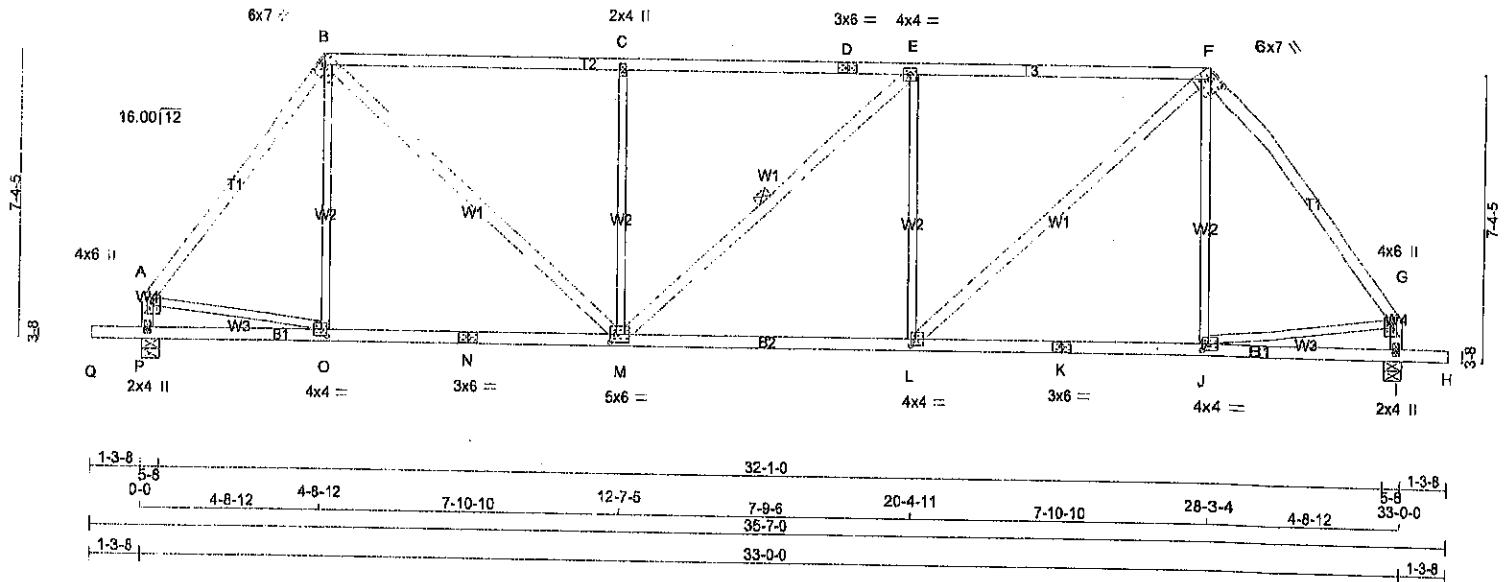
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.69 (H) (INPUT = 1.00)



A-15073755



LUMBER				DESCR.	
N.L.G.A. RULES	CHORDS	SIZE	LUMBER		
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
P - A	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
B - M	2x4	DRY	No.2	SPF	
M - E	2x4	DRY	No.2	SPF	
L - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW-h	MT20	6.0	7.0	1.50	3.75
C	TMVW+h	MT20	2.0	4.0		
D	TS-i	MT20	3.0	6.0		
E	TMVW-h	MT20	4.0	4.0		
F	TTWW-h	MT20	6.0	7.0	1.50	3.75
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	4.0	2.00	1.75
L	BS-i	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	4.0	2.00	1.60
N	BS-i	MT20	5.0	6.0	2.50	2.00
O	BMVW-t	MT20	3.0	6.0		
P	BMV1+p	MT20	4.0	4.0	2.00	1.75

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1565	0	1565	0	5-8	1-10
I	1565	0	1565	0	5-8	1-10

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
P	1102	747 / 0	0 / 0	0 / 0	356 / 0	0 / 0	0 / 0
I	1102	747 / 0	0 / 0	0 / 0	356 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM	TO	FR-TO			
A-B	-1421 / 0	-70.5	-70.5 0.38 (1)	6.11	O-B	0 / 99	0.03 (4)
B-C	-1671 / 0	-70.5	-70.5 0.80 (1)	4.17	B-M	0 / 1102	0.18 (1)
C-D	-1672 / 0	-70.5	-70.5 0.80 (1)	4.16	M-C	-595 / 0	0.57 (1)
D-E	-1672 / 0	-70.5	-70.5 0.80 (1)	4.16	M-E	-1 / 0	0.00 (1)
E-F	-1672 / 0	-70.5	-70.5 0.81 (1)	4.16	L-E	-595 / 0	0.57 (1)
F-G	-1421 / 0	-70.5	-70.5 0.38 (1)	5.11	L-F	0 / 1104	0.18 (1)
P-A	-1424 / 0	0.0	0.0 0.14 (1)	6.84	J-F	0 / 99	0.03 (4)
I-G	-1423 / 0	0.0	0.0 0.14 (1)	6.84	A-O	0 / 856	0.19 (1)
Q-P	0 / 0	-88.0	-88.0 0.10 (1)	10.00	J-G	0 / 856	0.19 (1)
P-O	0 / 0	-17.5	-17.5 0.17 (4)	10.00			
O-N	0 / 848	-17.5	-17.5 0.28 (4)	10.00			
N-M	0 / 848	-17.5	-17.5 0.28 (4)	10.00			
M-L	0 / 1672	-17.5	-17.5 0.39 (1)	10.00			
L-K	0 / 848	-17.5	-17.5 0.28 (4)	10.00			
K-J	0 / 848	-17.5	-17.5 0.28 (4)	10.00			
J-I	0 / 0	-17.5	-17.5 0.17 (4)	10.00			
I-H	0 / 0	-88.0	-88.0 0.10 (1)	10.00			

TOTAL WEIGHT = 2 X 162 = 304 lb

DESIGN CRITERIA:

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360 (1.10")$
CALCULATED VERT. DEFL.(LL) = $L/999 (0.07")$
ALLOWABLE VERT. DEFL.(TL) = $L/360 (1.10")$
CALCULATED VERT. DEFL.(TL) = $L/999 (0.16")$

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = $L/120 (0.19")$
CALCULATED VERT. DEFL.(LL) = $L/999 (0.00")$
ALLOWABLE DEFL.(TL) = $L/120 (0.19")$
CALCULATED VERT. DEFL.(TL) = $L/999 (0.01")$

CSI: TC=0.81 (E-F:1), BC=0.39 (L-M:1), WB=0.57 (E-L:1), SSI=0.26 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
MT20	1667	822
MT20	2284	1656

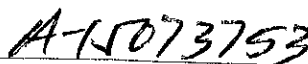
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.34 (N) (INPUT = 1.00)



A-15073754

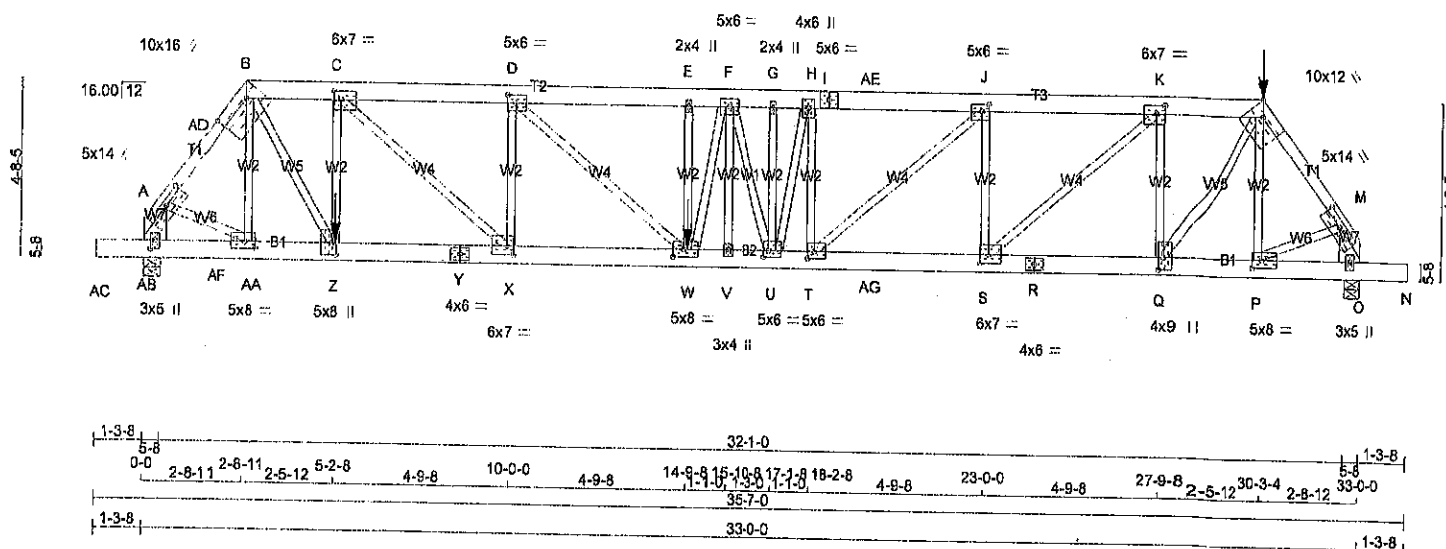


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ID:AyrJx9Y?fa2EXF7DRQXSc4mEz1-N_Bye_Ar0TzKvYhYJbBdP9iva98ILQCVAOHlxSyxWhE

Scale = 1:62.0



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
AB - A	2x8	DRY	No.2	SPF
C - M	2x8	DRY	No.2	SPF
AC - Y	2x6	DRY	1650F 1.6E	SPF
Y - R	2x6	DRY	1650F 1.6E	SPF
R - N	2x6	DRY	1650F 1.6E	SPF

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	14.0	1.50	6.75
B	TTVW-h	MT20	10.0	16.0	3.00	11.50
C	TMVW-I	MT20	6.0	7.0	2.75	2.25
D	TMVW-I	MT20	5.0	6.0	2.50	2.75
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-w-I	MT20	5.0	6.0		
G	TMVW-w	MT20	2.0	4.0		
H	TMVW-h	MT20	4.0	6.0		
I	TS-I	MT20	5.0	6.0		
J	TMVW-I	MT20	5.0	6.0	2.25	2.25
K	TMVW-I	MT20	6.0	7.0	2.75	2.75
L	TTVW-h	MT20	10.0	12.0	Edge	4.50
M	TMVW-I	MT20	5.0	14.0	1.75	6.75
O	BMV1+p	MT20	3.0	5.0		
P	BMVW-I	MT20	5.0	8.0	2.50	3.00
Q	BMVW-I	MT20	4.0	9.0	4.00	1.50
R	BS-I	MT20	4.0	6.0		
S	BMVW-I	MT20	6.0	7.0	2.75	2.75
T	BMVW-I	MT20	5.0	6.0	2.25	2.25
U	BMVW-w-I	MT20	5.0	6.0	2.00	1.50
V	BMVW-w	MT20	3.0	4.0		
W	BMVW-I	MT20	5.0	8.0	2.50	3.25
X	BMVW-I	MT20	6.0	7.0	2.75	2.25
Y	BS-I	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
AB	3785	0	3785	0
O	3271	0	3271	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	2648	1903 / 0	0 / 0	0 / 0	0 / 0	744 / 0	0 / 0
O	2297	1597 / 0	0 / 0	0 / 0	0 / 0	700 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX	WEBS	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-AD	-3741 / 0	-156.3	-156.3	0.44 (1)	3.25	AA-B	-92 / 32
AD-B	-3741 / 0	-70.5	-70.5	0.44 (1)	3.25	B-Z	0 / 3272
B-C	-3884 / 0	-70.5	-70.5	0.14 (1)	4.27	Z-C	-1816 / 0
C-D	-5690 / 0	-70.5	-70.5	0.34 (1)	3.44	C-X	0 / 2410
D-E	-8922 / 0	-70.5	-70.5	0.48 (1)	3.02	X-D	-1484 / 0
E-F	-8922 / 0	-70.5	-70.5	0.36 (1)	3.12	D-W	0 / 1845
F-G	-8824 / 0	-70.5	-70.5	0.36 (1)	3.14	W-E	-181 / 0
G-H	-8824 / 0	-70.5	-70.5	0.36 (1)	3.16	E-F	-38 / 49
H-I	-6784 / 0	-70.5	-70.5	0.50 (1)	3.03	F-V	0 / 169
I-AE	-6784 / 0	-138.7	-138.7	0.50 (1)	3.03	F-U	-304 / 0
J-K	-5601 / 0	-138.7	-138.7	0.40 (1)	3.41	U-G	0 / 24
K-L	-3453 / 0	-138.7	-138.7	0.17 (1)	4.44	G-H	0 / 240
L-M	-3284 / 0	-70.5	-70.5	0.28 (1)	3.64	H-I	-586 / 0
AB-A	-3842 / 0	0.0	0.0	0.20 (1)	6.24	S-J	-1729 / 0
O-M	-3132 / 0	0.0	0.0	0.17 (1)	6.84	J-K	0 / 2898
AC-AB	0 / 0	-88.0	-88.0	0.04 (1)	10.00	K-L	-2425 / 0
AB-AF	0 / 0	-38.8	-38.8	0.06 (1)	10.00	L-Q	0 / 2947
AF-AA	0 / 0	-124.7	-124.7	0.06 (1)	10.00	P-L	-278 / 0
AA-Z	0 / 2228	-124.7	-124.7	0.24 (1)	10.00	A-AA	0 / 2277
Z-Y	0 / 3884	-17.5	-17.5	0.35 (1)	10.00	P-M	0 / 2010
Y-X	0 / 3884	-17.5	-17.5	0.35 (1)	10.00		
X-W	0 / 5690	-17.5	-17.5	0.53 (1)	10.00		
W-V	0 / 8910	-124.7	-124.7	0.83 (1)	10.00		
V-U	0 / 8910	-124.7	-124.7	0.64 (1)	10.00		
U-T	0 / 6784	-124.7	-124.7	0.62 (1)	10.00		
T-AG	0 / 5601	-124.7	-124.7	0.53 (1)	10.00		
AG-S	0 / 5601	-34.5	-34.5	0.53 (1)	10.00		
S-R	0 / 3453	-34.5	-34.5	0.33 (1)	10.00		
R-Q	0 / 3453	-34.5	-34.5	0.33 (1)	10.00		
Q-P	0 / 1981	-34.5	-34.5	0.19 (1)	10.00		
P-O	0 / 0	-34.5	-34.5	0.03 (1)	10.00		
O-N	0 / 0	-88.0	-88.0	0.04 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE
L	30-3-4	-140	-140	---	FRONT	VERT	TOTAL
W	14-9-8	-877	-877	---	FRONT	VERT	TOTAL
Z	5-2-8	-877	-877	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 2 X 199 = 398 lb

(M)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-8-11
RIGHT SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 13'-0" OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 13'-0" OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 1-10-8
START SPAN CARRIED = 6-10-8
END DISTANCE = 6-2-8
END SPAN CARRIED = 8-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 14-9-8
START SPAN CARRIED = 8-10-8
END DISTANCE = 18-9-0
END SPAN CARRIED = 8-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLETES 3752
CONTINUED ON PAGE 2



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Z	BMWV+t	MT20	5.0	8.0	3.75	1.50
AA	BMWV-t	MT20	5.0	8.0	2.25	3.75
AB	BMV1+p	MT20	3.0	5.0	2.75	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.2 lbs FACTORED DOWN AT 30-3-4 ON TOP CHORD, AND 876.9 lbs FACTORED DOWN AT 14-8-8, AND 876.9 lbs FACTORED DOWN AT 5-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES - (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/770 (0.51")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.50 (H-J:1), BC=0.64 (U-V:1), WB=0.81 (B-Z:1), SSI=0.23 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)

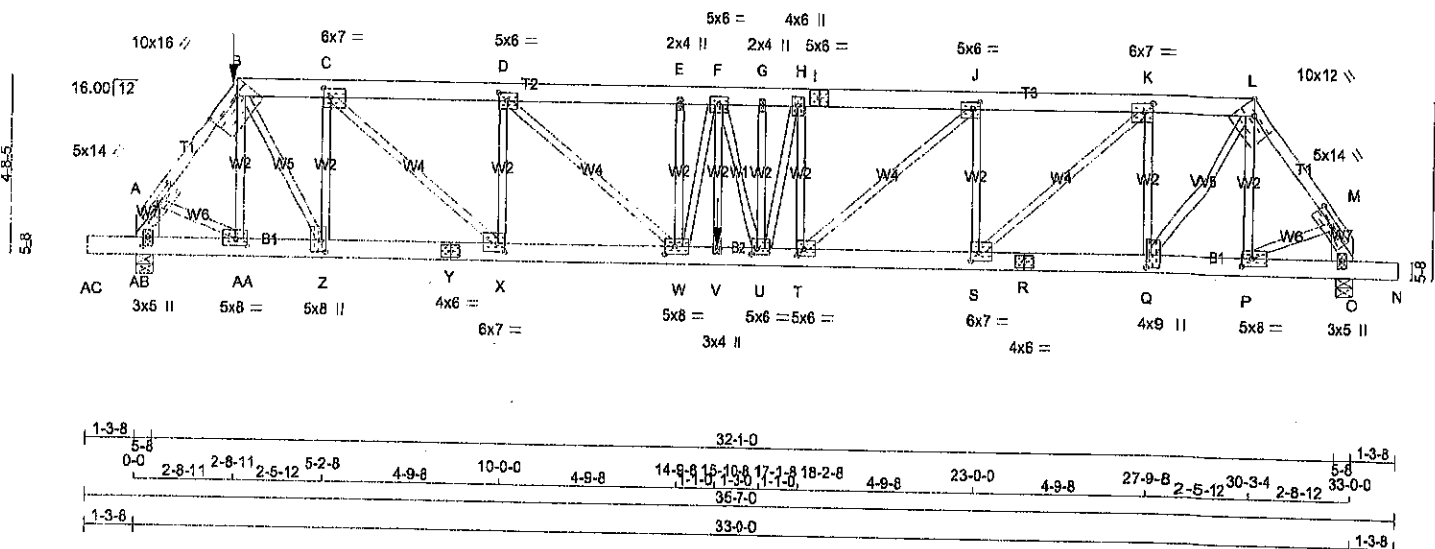
JSI METAL= 0.79 (Y) (INPUT = 1.00)



A-H073752(2)

Alpa Roof Truss, Maple

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 Scale = 1:62.0



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - B 2x4 DRY No.2 SPF

B - I 2x6 DRY No.2 SPF

I - L 2x6 DRY No.2 SPF

L - M 2x4 DRY No.2 SPF

AB - A 2x8 DRY No.2 SPF

O - M 2x8 DRY No.2 SPF

AC - Y 2x6 DRY 1650F 1.5E SPF

Y - R 2x8 DRY 1650F 1.5E SPF

R - N 2x6 DRY 1650F 1.5E SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-I	MT20	5.0	14.0	1.50	6.75
B	TTWW-h	MT20	10.0	18.0	3.00	11.50
C	TMWW-I	MT20	6.0	7.0	2.75	2.25
D	TMWW-I	MT20	5.0	8.0	2.50	2.75
E	TMWW-w	MT20	2.0	4.0		
F	TMWWWW-I	MT20	5.0	8.0		
G	TMWW-w	MT20	2.0	4.0		
H	TMWW-w	MT20	4.0	6.0		
I	TS-I	MT20	5.0	6.0		
J	TMWW-I	MT20	5.0	6.0	2.25	2.25
K	TMWW-w	MT20	6.0	7.0	2.75	2.75
L	TTWW-h	MT20	10.0	12.0	Edge	4.50
M	TMWW-I	MT20	5.0	14.0	1.75	6.75
O	BMV1+p	MT20	3.0	5.0		
P	BMWW-I	MT20	5.0	8.0	2.50	3.00
Q	BMWW-I	MT20	4.0	9.0	4.00	1.50
R	BS-I	MT20	4.0	6.0		
S	BMWW-I	MT20	6.0	7.0	2.75	2.75
T	BMWW-I	MT20	5.0	6.0	2.25	2.25
U	BMWWWW-I	MT20	5.0	6.0	2.00	1.50
V	BMWW-w	MT20	3.0	4.0		
W	BMWWWW-I	MT20	5.0	8.0	2.50	3.25
X	BMWW-I	MT20	6.0	7.0	2.75	2.25
Y	BS-I	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
AB	3522	0	0	3-15
O	2803	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	2457	1762 / 0	0 / 0	0 / 0	0 / 0	0 / 0	715 / 0	0 / 0
O	1980	1410 / 0	0 / 0	0 / 0	0 / 0	0 / 0	551 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.72 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							
A-B	-3558 / 0	-70.5	-70.5	0.31 (1)	3.49	AA-B	-312 / 0
B-C	-3762 / 0	-138.7	-138.7	0.18 (1)	4.28	B-Z	0 / 3234
C-D	-6192 / 0	-138.7	-138.7	0.46 (1)	3.19	Z-C	-2084 / 0
D-E	-7636 / 0	-138.7	-138.7	0.53 (1)	2.72	C-X	0 / 3244
E-F	-7636 / 0	-138.7	-138.7	0.48 (1)	2.86	X-D	-2009 / 0
F-G	-7408 / 0	-70.5	-70.5	0.46 (1)	2.02	D-W	0 / 1926
G-H	-7408 / 0	-70.5	-70.5	0.44 (1)	2.04	W-E	-508 / 0
H-I	-7137 / 0	-70.5	-70.5	0.47 (1)	2.88	E-F	-73 / 37
I-J	-7137 / 0	-70.5	-70.5	0.47 (1)	2.98	F-F	0 / 944
J-K	-6322 / 0	-70.5	-70.5	0.30 (1)	3.59	F-U	-868 / 0
K-L	-3044 / 0	-70.5	-70.5	0.10 (1)	4.76	U-G	-2 / 19
L-M	-2801 / 0	-70.5	-70.5	0.23 (1)	3.85	U-H	0 / 1091
AB-A	-3385 / 0	0.0	0.0	0.18 (1)	6.44	T-H	-1362 / 0
O-M	-2685 / 0	0.0	0.0	0.15 (1)	7.05	T-J	0 / 2423

AC-AB	0 / 0	-88.0	-88.0	0.04 (1)	10.00	S-J	-1971 / 0
AB-AA	0 / 0	-34.5	-34.5	0.04 (1)	10.00	S-K	0 / 3040
AA-Z	0 / 2124	-34.5	-34.5	0.21 (1)	10.00	Q-K	-2269 / 0
Z-Y	0 / 3762	-34.5	-34.5	0.38 (1)	10.00	Q-L	0 / 2711
Y-X	0 / 3762	-34.5	-34.5	0.38 (1)	10.00	P-L	-269 / 0
X-W	0 / 6192	-34.5	-34.5	0.38 (1)	10.00	A-AA	0 / 2177
W-V	0 / 7684	-34.5	-34.5	0.58 (1)	10.00	P-M	0 / 1714
V-U	0 / 7684	-17.5	-17.5	0.94 (1)	10.00		
U-T	0 / 7137	-17.5	-17.5	0.65 (1)	10.00		
T-S	0 / 6322	-17.5	-17.5	0.47 (1)	10.00		
S-R	0 / 3044	-17.5	-17.5	0.29 (1)	10.00		
R-Q	0 / 3044	-17.5	-17.5	0.29 (1)	10.00		
Q-P	0 / 1672	-17.5	-17.5	0.17 (1)	10.00		
P-O	0 / 0	-17.5	-17.5	0.03 (1)	10.00		
O-N	0 / 0	-88.0	-88.0	0.04 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
B	2-8-11	-140	-140	---	FRONT	VERT
V	15-10-8	-1889	-1889	---	FRONT	VERT

TOTAL WEIGHT = 2 X 198 = 396 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A
 SLOPE OF 2.00/12 MIN/MUM

GIRDER TYPE: CPrimeHlp
 LEFT SETBACK = 2-8-11
 RIGHT SETBACK = 2-8-12
 END SETBACK = 5-10-8
 END WALL WIDTH = 0-0
 CORNER FRAMING TYPE: CONVENTIONAL
 END JACK TYPE: CONVENTIONAL
 APPLIED TO FRONT SIDE
 - ADDTL LOADS BASED ON 55 % OF GSL
 LOADS APPLIED TO FIRST 15-10-8 OF SPAN
 MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO
 ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
 OR SMALL BUILDING REQUIREMENTS OF
 PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.34")
 ALLOWABLE DEFL.(TL) = L/360 (1.10")
 CALCULATED VERT. DEFL.(TL) = L/694 (0.67")

CANTILEVER DEFLECTION:
 ALLOWABLE DEFL.(LL) = L/120 (0.18")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/120 (0.18")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.63 (D-E:1), BC=0.94 (V-W:1),
 WB=0.80 (C-X:1), SSI=0.40 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

A-11013751
 CONTINUED ON PAGE 2



Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 MiTek Industries, Inc. Thu Jul 16 11:18:10 2016 Page 2

ID: AyrJx9Y?fa2EXF7DRQxSq4zMEz1-N Bye Ar9TzKeVhYJbBdP9WY935LQJVAOHlxSyxVWhf

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Z	BMWW+1	MT20	5.0	8.0	3.75	1.50
AA	BMWW-1	MT20	5.0	8.0	2.25	3.75
AB	BMV1+p	MT20	3.0	5.0	2.75	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 139.9 lbs FACTORED DOWN AT 2-8-11
ON TOP CHORD, AND 1888.9 lbs FACTORED
DOWN AT 15-10-8 ON BOTTOM CHORD.
DESIGN FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.90)

JSI METAL = 0.82 (Y) (INPUT = 1.00)



A-15073751(2)

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

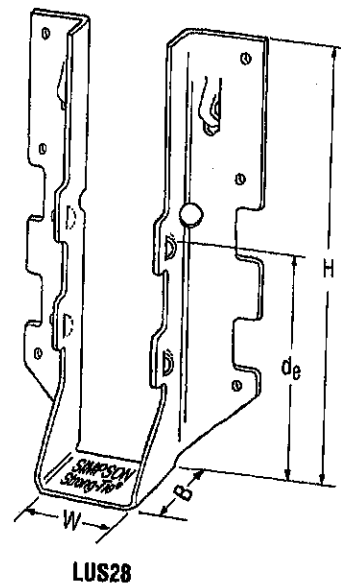
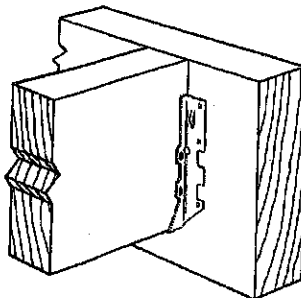
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

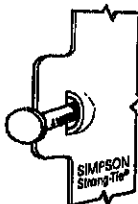
- These hangers cannot be modified.

Typical LUS
Installation



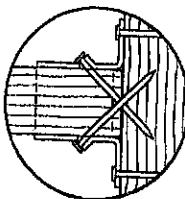
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 1/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 1/8	2	3 3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 3/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 1/16	1 3/4	3 5/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/8	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

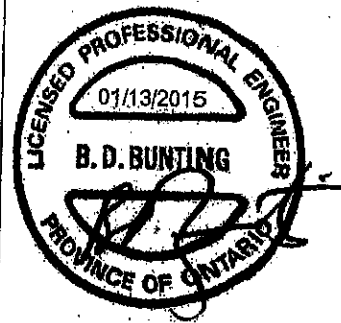


Double
Shear
Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,693,580



Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

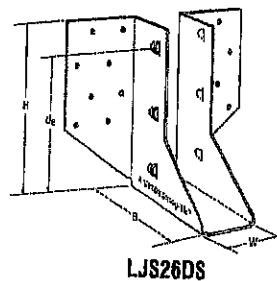
- Factored resistances are in accordance with CSA D86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

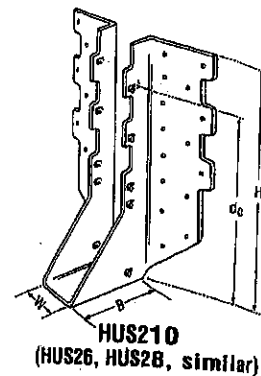
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

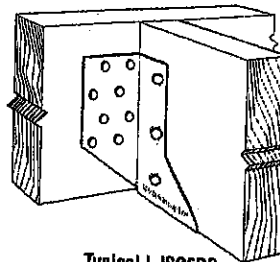
- See current catalogue for options



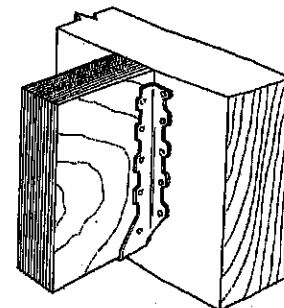
LJS26DS



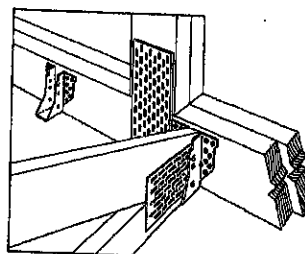
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



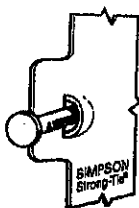
Typical HUS
Installation



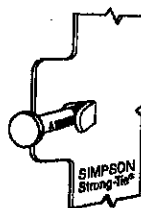
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _p =1.00)	Uplift (K _u =1.15)	Normal (K _p =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 3/4	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 1/4	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/8	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 3/8	30-16d	10-16d	4505	5785	4010	4740
HUS1.81/10	16	1 3/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

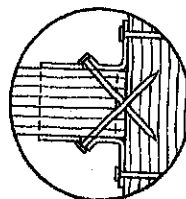
1. d_g is the distance from the seat of the hanger to the highest joist nail.



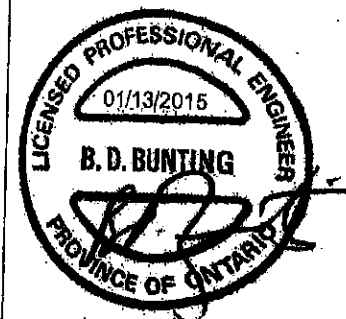
Double Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.

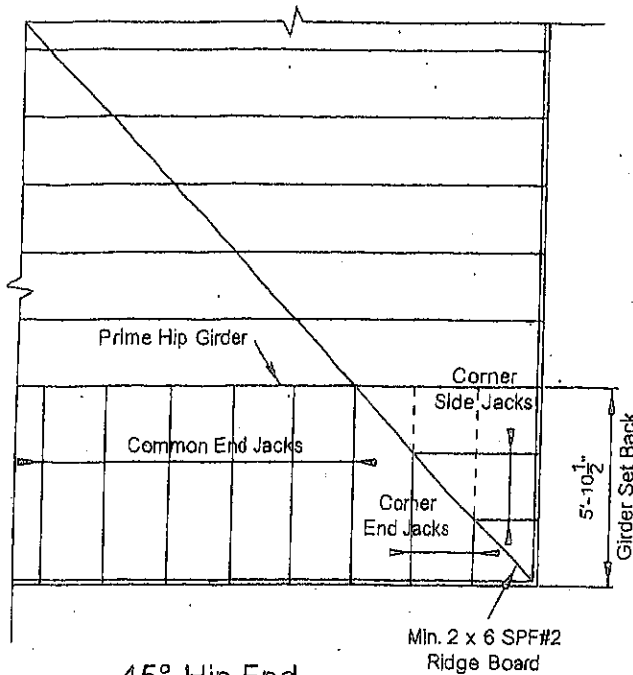


Double
Shear
Nailing
Top View.



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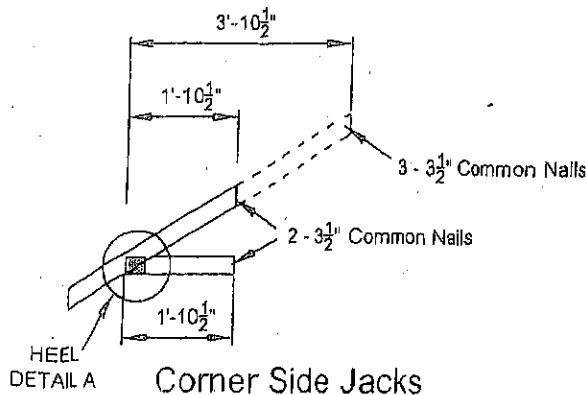
45° Hip End

LUMBER SPECIFICATION

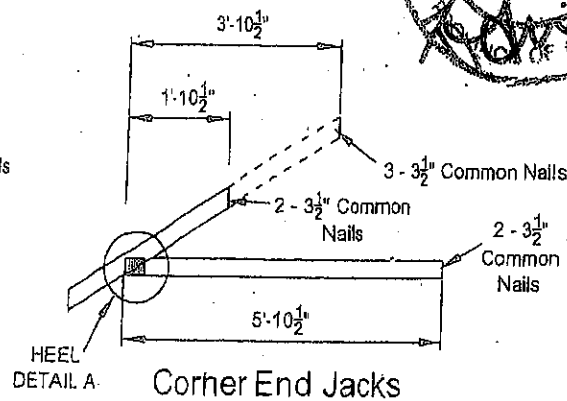
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

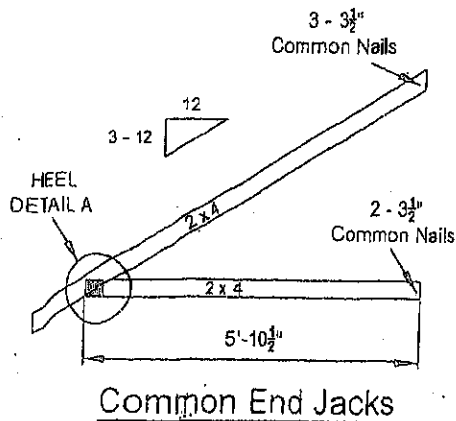
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 50.5 P.S.F.



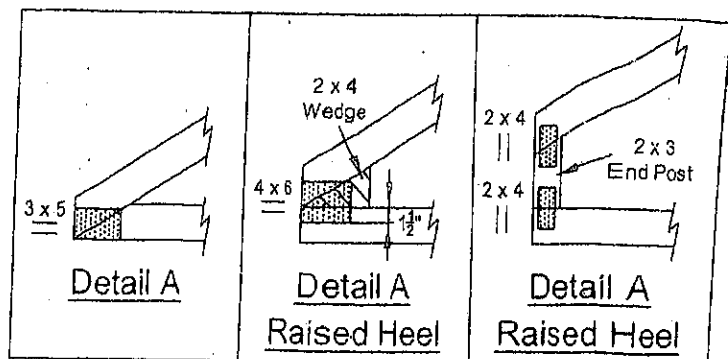
Corner Side Jacks



Corner End Jacks



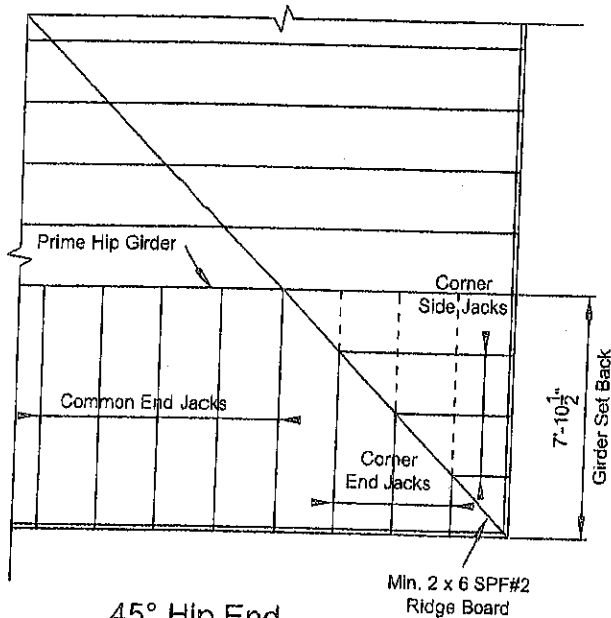
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

STRACON ENGINEERING INC.



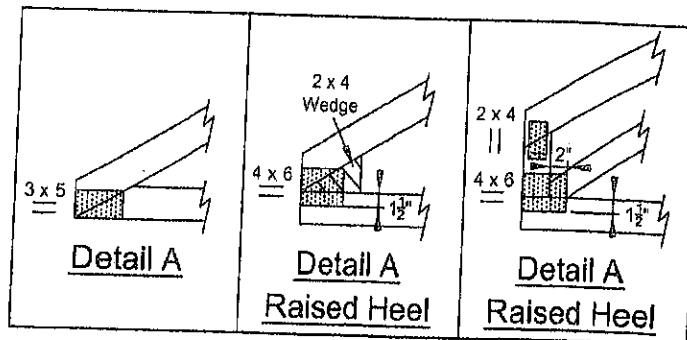
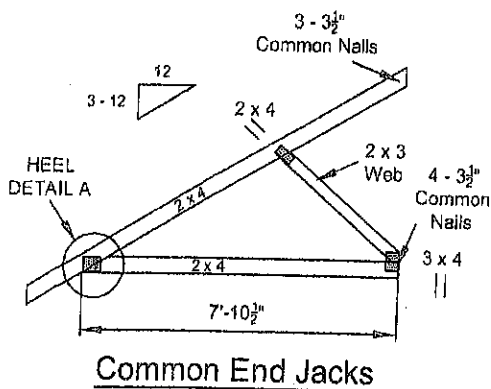
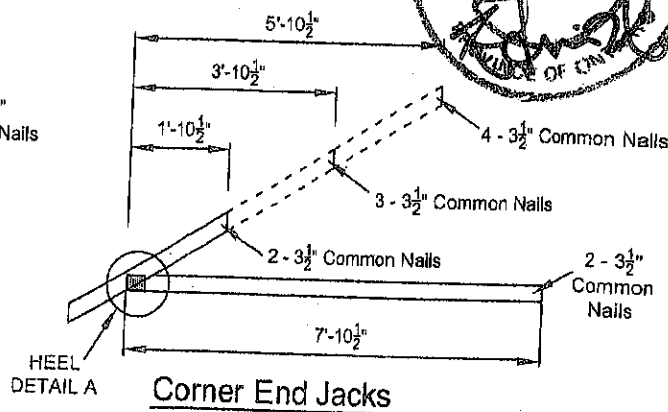
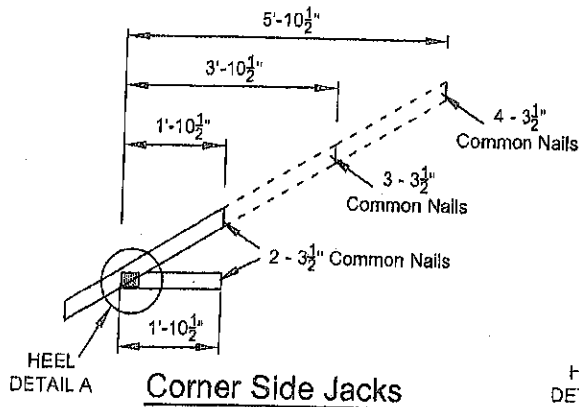
45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 44.8 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-71008N