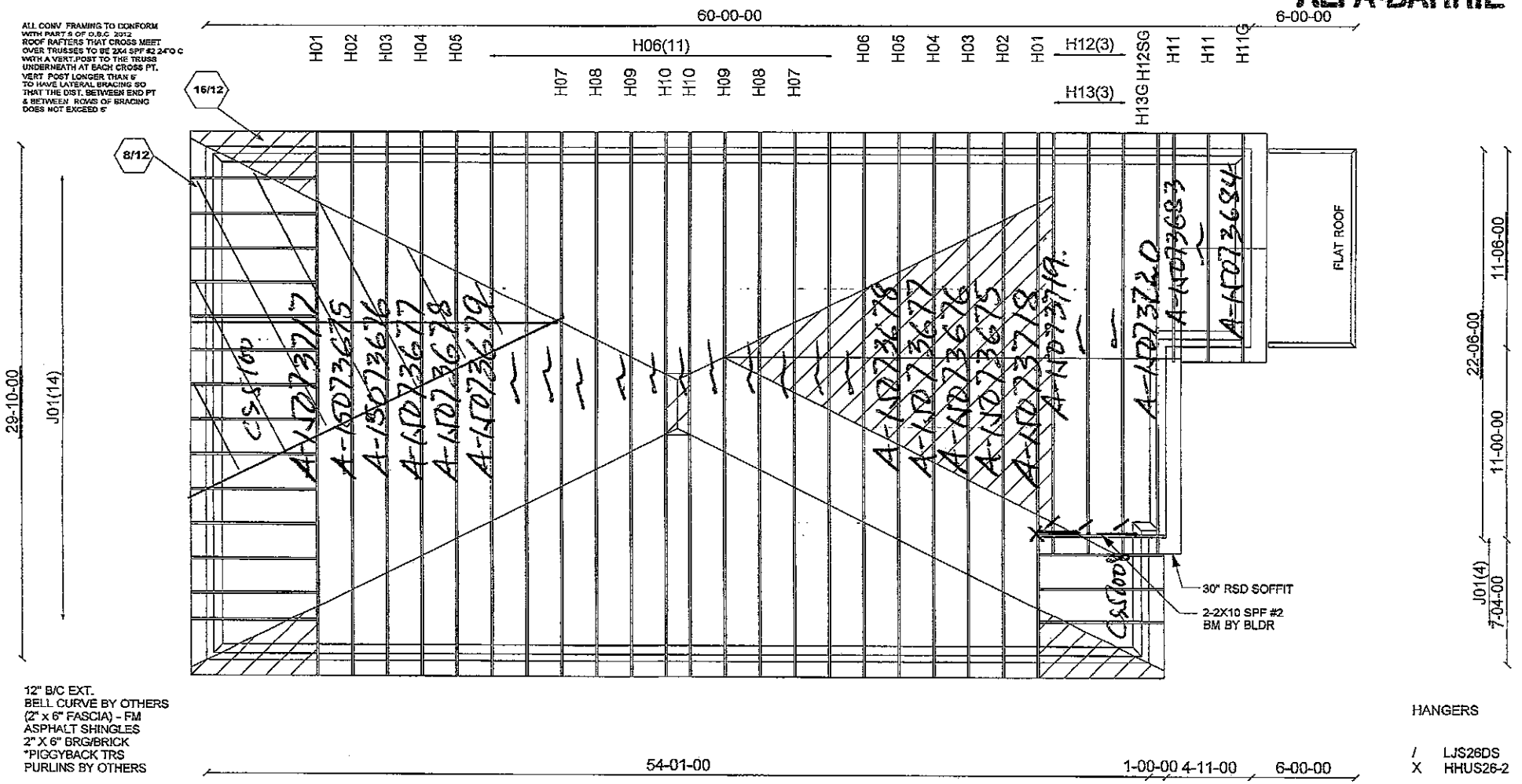
ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



12" B/C EXT.
BELL CURVE BY OTHERS
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

54-01-00 1-00-00 4-11-00 6-00-00

HANGERS
/ LJS26DS
X HHUS26-2

Piggyback / A-1507368 A-1507369 A-1507370 A-1507371 A-1507372 A-1507373 A-1507374 A-1507375 A-1507376 A-1507377 A-1507378 A-1507379 A-1507380 A-1507381 A-1507382 A-1507383 A-1507384 (LOT 63+67) 88062

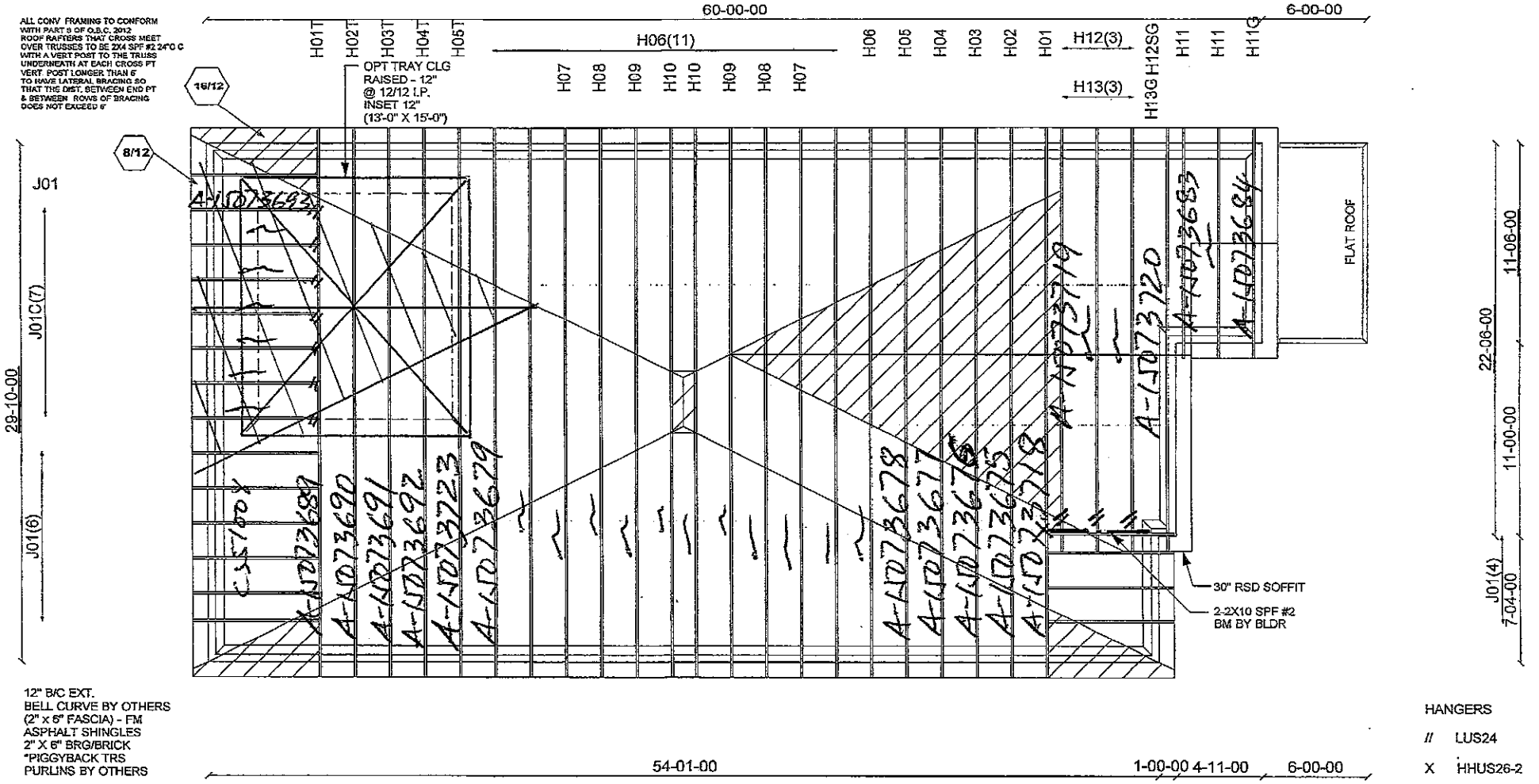
PL 80980, 82444, 85748

CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Tracic: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-6 / A	STD OR OPT 2ND FLR LOT 63 AND 67 REAR UPGRADE
Layout ID: 253446	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 5/28/2015 Designer: cheryle		

ALPA-BARRIE



Piggyback (A-1507 3680, A-1507 3681, A-1507 3682, A-1507 3721, A-1507 3722)

PL 80980, 82444, 85748 (LOT 63+67) 88062

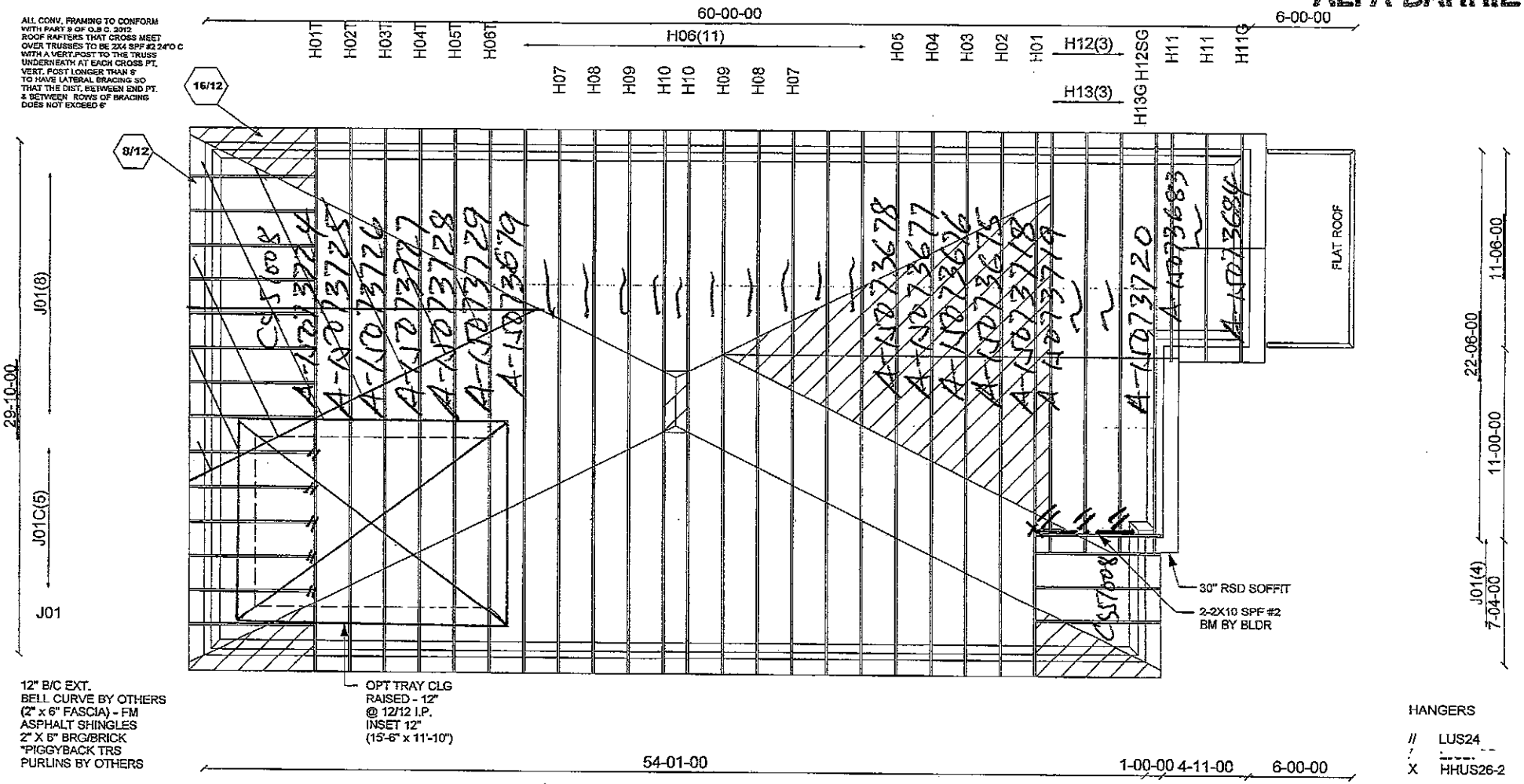
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-6 / A OPT TRAY LOT 63 AND 67 REAR UPGRADE
Layout ID: 253447	Project: HUNTINGTON & NASHVILLE	
Plan Log: 79711	Date: 5/28/2015 Designer: cheryle	

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 8' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 8'



12" B/C EXT.
BELL CURVE BY OTHERS
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

HANGERS

// LUS24

X HHUS26-2

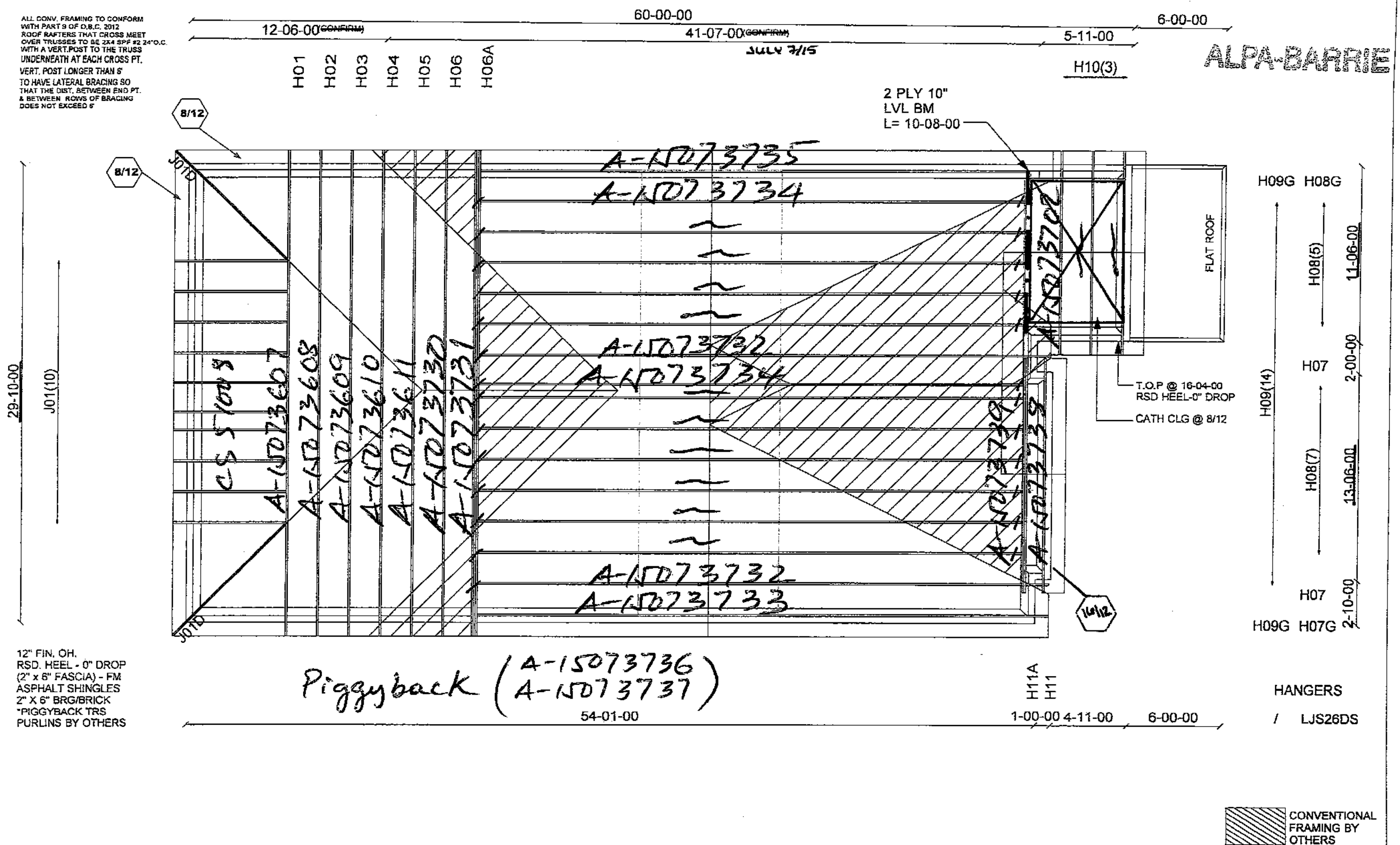
Piggyback (A-150723680, A-150723681, A-150723682, A-150723721, A-150723722)

PL 80980, 82444, 85748 (Lot 63+67 88062)

CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-6	/ A OPT 2ND W/TRAY LOT 63 and 67 REAR UPGRADE
Layout ID: 253448	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 5/28/2015 Designer: cheryle		

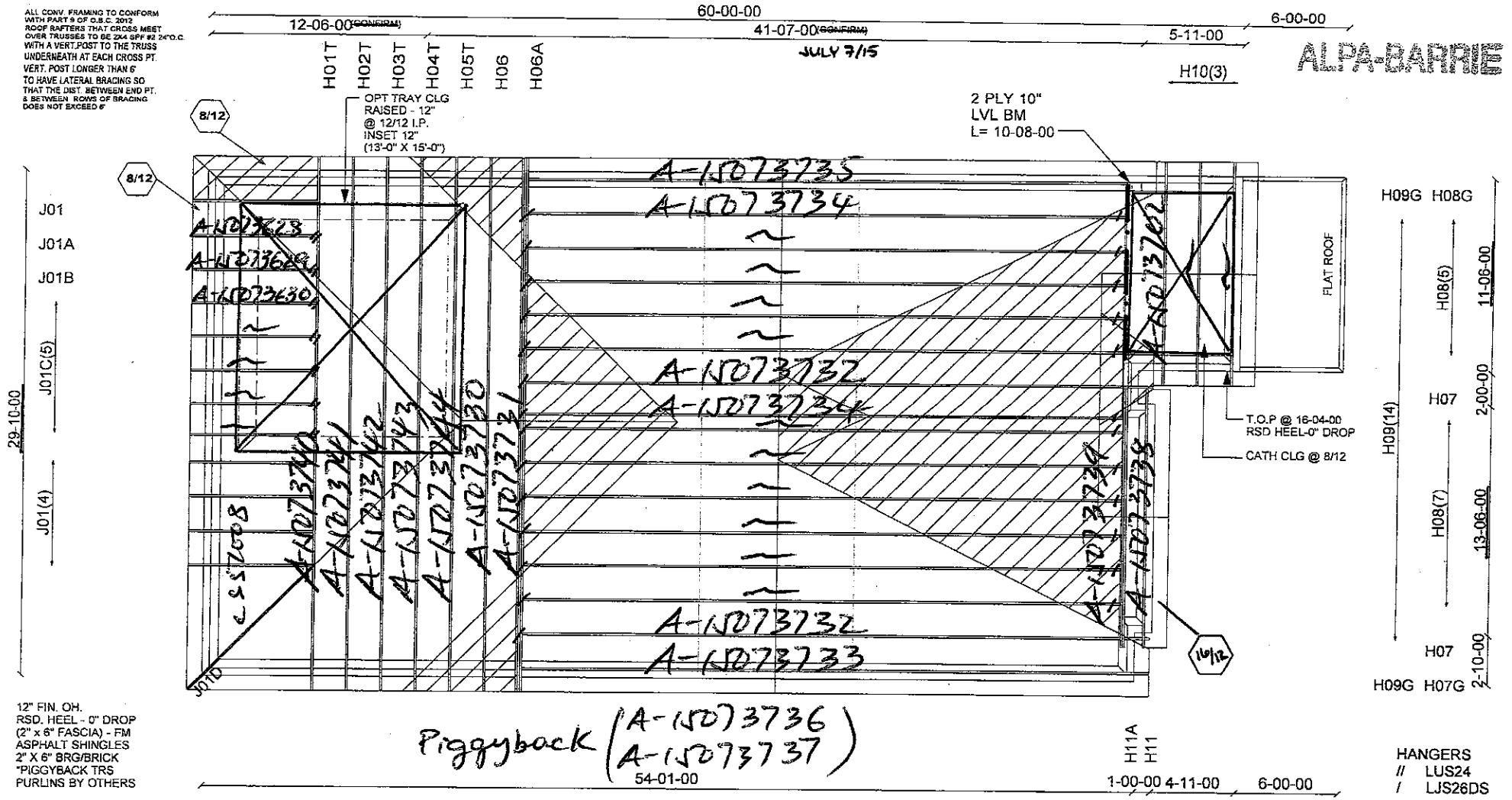


PL 80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-6 / B	STD OR OPT 2ND FLR.
Layout ID: 253449	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/24/2015 Designer: cheryle		

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" SRG/BRICK
"PIGGYBACK TRS
PURLINS BY OTHERS

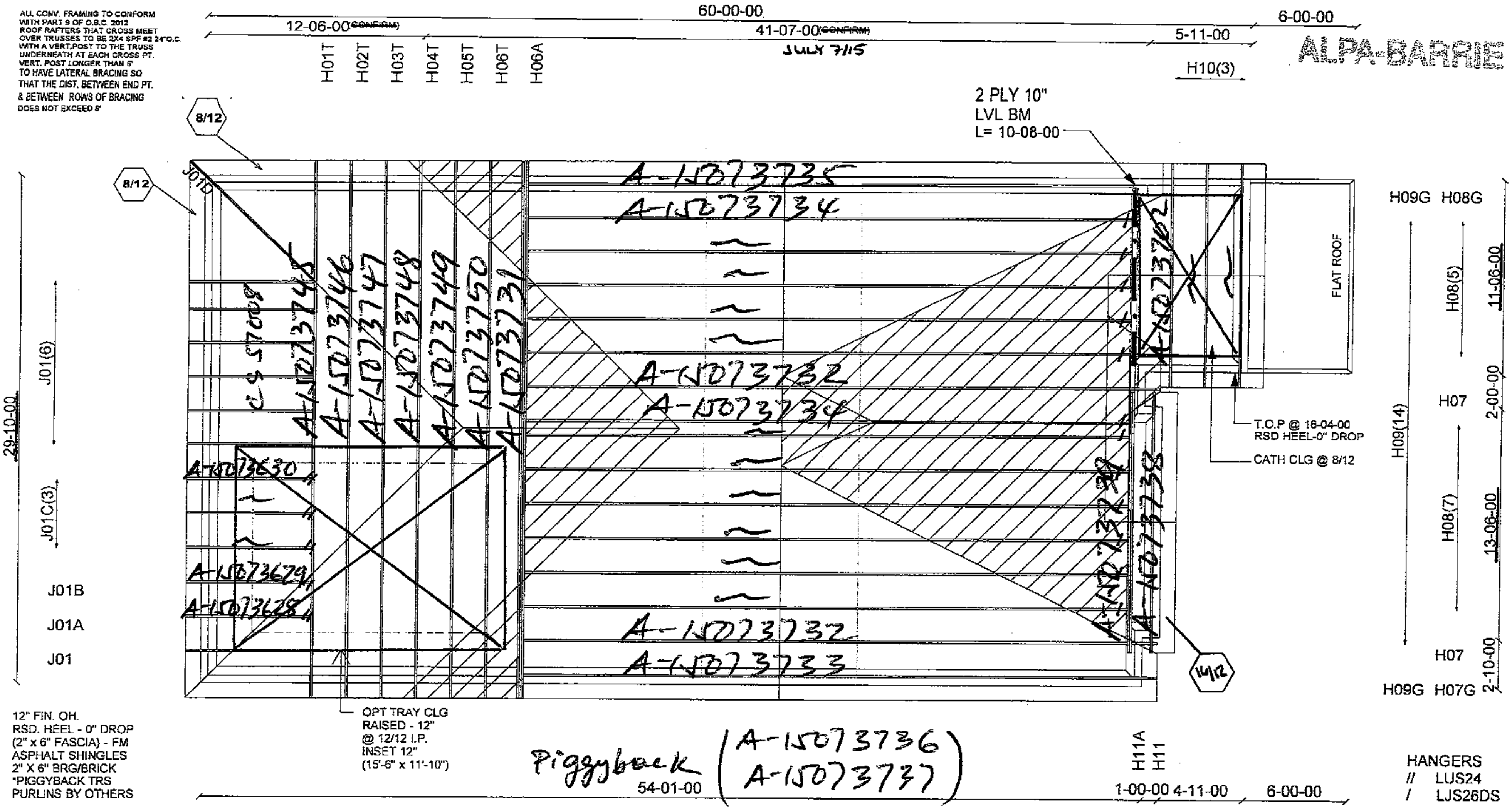
HANGERS
// LUS24
/ LJS26DS

CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

>L80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-6 / B OPT TRAY	
Layout ID: 253450	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/24/2015	Designer: cheryle	



CONVENTIONAL FRAMING BY OTHERS

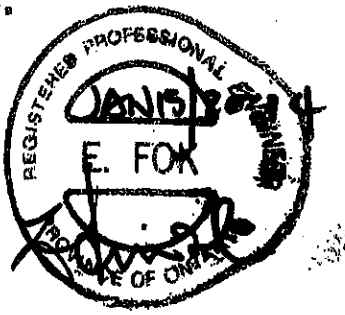
Mitek Ver: 7.5.0

PL 80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG		Model / Elevation: 38-6 / B OPT 2ND W/TRAY
Layout ID: 253451			
Plan Log: 79711	Project: HUNTINGTON & NASHVILLE		THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Date: 4/24/2015	Designer: cheryle	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

253451	H06T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 10:02:44 2015 Page 1

ID:y40yHYHwfsopl7ACUSKwAKzNoB2-uGlxkOOr2skWeyhhA3?GzjFPHlUiuQiJQEvF3nyxXn\

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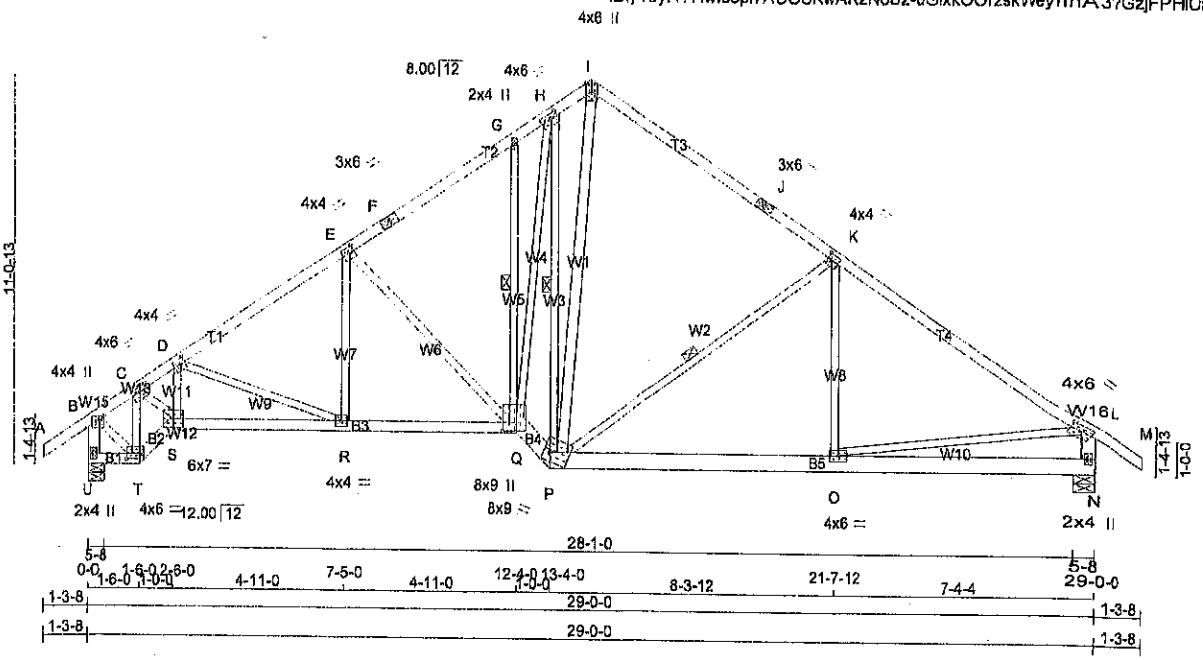

TOTAL WEIGHT = 161 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.75
C	TMWW-t	MT20	4.0	8.0	1.50	3.00
D, E, K						
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0	1.75	2.00
I	TTW+p	MT20	4.0	6.0	Edge	
J	TS-t	MT20	3.0	6.0		
L	TMVW-t	MT20	4.0	6.0	1.75	3.00
N	BMV1+p	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	6.0		
P	BBWWW-m	MT20	8.0	9.0	3.00	2.25
Q	BBWWW+p	MT20	8.0	9.0	2.75	4.75
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-l	MT20	6.0	7.0		
T	BBWW-l	MT20	4.0	6.0	1.75	4.50
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1374	0	1374	0	0	5-8	1-8
N	1370	0	1370	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	966	667 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
N	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF B-Q, H-P, K-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	T- C	-1309 / 0	0.20 (1)
B- C	-1039 / 0	-70.5	-70.5	0.08 (1)	6.09	C- S	0 / 1263	0.28 (1)
C- D	-1977 / 0	-70.5	-70.5	0.12 (1)	4.71	S- D	-32 / 42	0.01 (4)
D- E	-1711 / 0	-70.5	-70.5	0.22 (1)	4.90	D- R	-286 / 0	0.13 (1)
E- F	-1342 / 0	-70.5	-70.5	0.20 (1)	5.40	R- E	0 / 166	0.05 (4)
F- G	-1342 / 0	-70.5	-70.5	0.20 (1)	5.40	E- Q	-464 / 0	0.42 (1)
G- H	-1382 / 0	-70.5	-70.5	0.13 (1)	5.41	Q- G	-296 / 0	0.14 (1)
H- I	-1051 / 0	-70.5	-70.5	0.12 (1)	6.01	Q- H	0 / 1573	0.35 (1)
I- J	-1029 / 0	-70.5	-70.5	0.53 (1)	5.48	P- H	-1315 / 0	0.94 (1)
J- K	-1029 / 0	-70.5	-70.5	0.53 (1)	5.48	P- I	0 / 846	0.14 (1)
K- L	-1500 / 0	-70.5	-70.5	0.59 (1)	4.87	P- K	-550 / 0	0.36 (1)
L- M	0 / 27	-70.5	-70.5	0.09 (1)	10.00	O- K	-19 / 131	0.05 (4)
U- B	-1361 / 0	0.0	0.0	0.14 (1)	6.96	B- T	0 / 1003	0.23 (1)
N- L	-1318 / 0	0.0	0.0	0.13 (1)	7.04	O- L	0 / 1288	0.29 (1)
U- T	0 / 0	-17.5	-17.5	0.01 (4)	10.00			
T- S	0 / 1085	-17.5	-17.5	0.17 (1)	10.00			
S- R	0 / 1700	-17.5	-17.5	0.33 (1)	10.00			
R- Q	0 / 1432	-17.5	-17.5	0.29 (1)	10.00			
Q- P	0 / 1264	-17.5	-17.5	0.15 (1)	10.00			
P- O	0 / 1276	-17.5	-17.5	0.22 (1)	10.00			
O- N	0 / 0	-17.5	-17.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.59 (K-L:1) , BC=0.33 (R-S:1) , WB=0.94 (H-P:1) , SSI=0.21 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (T) (INPUT = 0.90)
JSI METAL= 0.42 (L) (INPUT = 1.00)

A-1507375D

253451	H05T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

ID:y40yHYHwfsopi7ACUSKwAKzNoB2-Q38ZW2NDHYcf1o6UcLT1QWiFSI8U9ycABaAhXLyxXnw

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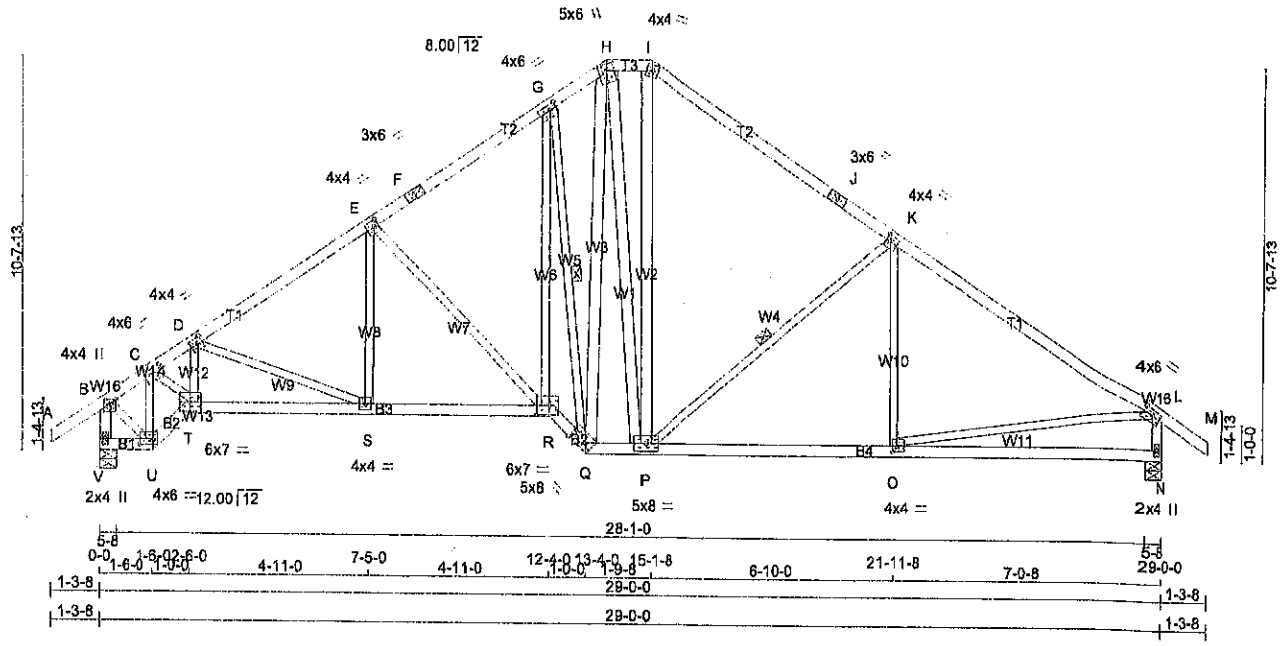

TOTAL WEIGHT = 164 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
H - P	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.75
C	TMWW-t	MT20	4.0	6.0	1.50	3.00
D, E, K						
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0	1.75	2.25
H	TTWW+m	MT20	5.0	6.0	2.00	2.26
I	TTW-m	MT20	4.0	4.0		
J	TS-t	MT20	3.0	6.0		
L	TMVW-t	MT20	4.0	6.0	1.75	3.00
N	BMV1+p	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	4.0	1.75	1.50
P	BMWWW-t	MT20	5.0	8.0		
Q	BBWW-h	MT20	5.0	8.0	2.25	5.50
R	BBWW-t	MT20	6.0	7.0	3.00	4.00
S	BMWW-t	MT20	4.0	4.0		
T	BBWW-t	MT20	6.0	7.0		
U	BBWW-t	MT20	4.0	6.0	1.75	4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	1374	0	1374	0	0	5-8	1-8
N	1370	0	1370	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	966	867 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
N	963	865 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-Q, K-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-1039 / 0	-70.5	-70.5	0.08 (1)	6.09
C- D	-1976 / 0	-70.5	-70.5	0.12 (1)	4.71
D- E	-1711 / 0	-70.5	-70.5	0.23 (1)	4.90
E- F	-1326 / 0	-70.5	-70.5	0.21 (1)	5.41
F- G	-1326 / 0	-70.5	-70.5	0.21 (1)	5.41
G- H	-1130 / 0	-70.5	-70.5	0.07 (1)	5.91
H- I	-895 / 0	-70.5	-70.5	0.02 (1)	6.25
I- J	-1104 / 0	-70.5	-70.5	0.48 (1)	5.40
J- K	-1104 / 0	-70.5	-70.5	0.48 (1)	5.40
K- L	-1482 / 0	-70.5	-70.5	0.53 (1)	4.78
L- M	0 / 27	-70.5	-70.5	0.09 (1)	10.00
V- B	-1361 / 0	0.0	0.0	0.14 (1)	6.96
N- L	-1320 / 0	0.0	0.0	0.14 (1)	7.04
V- U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U- T	0 / 1066	-17.5	-17.5	0.17 (1)	10.00
T- S	0 / 1699	-17.5	-17.5	0.33 (1)	10.00
S- R	0 / 1433	-17.5	-17.5	0.26 (1)	10.00
R- Q	0 / 1524	-17.5	-17.5	0.25 (1)	10.00
Q- P	0 / 891	-17.5	-17.5	0.23 (1)	10.00
P- O	0 / 1261	-17.5	-17.5	0.31 (4)	10.00
O- N	0 / 0	-17.5	-17.5	0.21 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-1309 / 0	0.20 (1)
C- T	0 / 1262	0.28 (1)
T- D	-31 / 42	0.01 (4)
D- S	-266 / 0	0.13 (1)
S- E	0 / 184	0.05 (4)
E- R	-486 / 0	0.44 (1)
R- G	0 / 1475	0.33 (1)
G- Q	-1615 / 0	0.99 (1)
Q- H	0 / 533	0.09 (1)
H- P	0 / 37	0.01 (4)
P- I	0 / 371	0.06 (1)
P- K	-481 / 0	0.27 (1)
O- K	-56 / 97	0.03 (4)
B- U	0 / 1004	0.23 (1)
O- L	0 / 1275	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.53 (K-L:1) , BC=0.33 (S-T:1) , WB=0.99 (G-Q:1) , SSI=0.20 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (U) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

A-15073749

253451	H04T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

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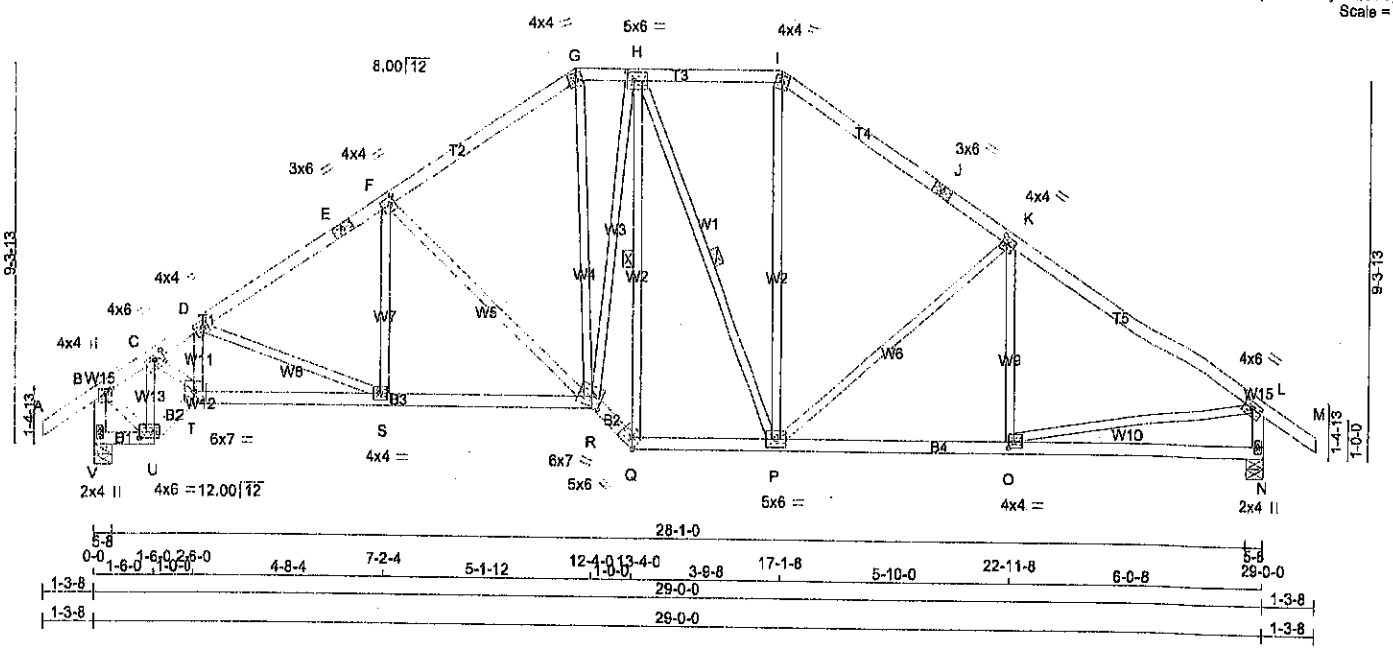

TOTAL WEIGHT = 146 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.75
C	TMWW-t	MT20	4.0	6.0	1.50	3.00
D, F, K						
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TS-t	MT20	3.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWWW-t	MT20	5.0	6.0	2.50	2.00
I	TTW-m	MT20	4.0	4.0		
J	TS-t	MT20	3.0	6.0		
L	TMVW-t	MT20	4.0	6.0	1.75	3.00
N	BMV1+p	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	4.0	1.75	1.50
P	BMWWW-t	MT20	5.0	6.0		
Q	BBW-h	MT20	6.0	6.0	Edge	2.50
R	BBWWW-m	MT20	6.0	7.0	3.00	3.00
S	BMWW-t	MT20	4.0	4.0		
T	BBWW-t	MT20	6.0	7.0		
U	BBWW-t	MT20	4.0	6.0	1.75	4.50
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1374	0	1374	0	0	5-8	1-8
N	1370	0	1370	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	986	687 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
N	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.73FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF H-Q, H-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B-C	-1040 / 0	-70.5	-70.5	0.08 (1)	6.08
C-D	-1975 / 0	-70.5	-70.5	0.10 (1)	4.73
D-E	-1727 / 0	-70.5	-70.5	0.22 (1)	4.88
E-F	-1727 / 0	-70.5	-70.5	0.22 (1)	4.88
F-G	-1336 / 0	-70.5	-70.5	0.20 (1)	5.40
G-H	-1127 / 0	-70.5	-70.5	0.09 (1)	5.88
H-I	-994 / 0	-70.5	-70.5	0.12 (1)	6.14
I-J	-1219 / 0	-70.5	-70.5	0.36 (1)	5.40
J-K	-1219 / 0	-70.5	-70.5	0.35 (1)	5.40
K-L	-1489 / 0	-70.5	-70.5	0.38 (1)	4.98
L-M	0 / 27	-70.5	-70.5	0.09 (1)	10.00
V-B	-1361 / 0	0.0	0.0	0.14 (1)	6.96
N-L	-1326 / 0	0.0	0.0	0.14 (1)	7.03
V-U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U-T	0 / 1068	-17.5	-17.5	0.17 (1)	10.00
T-S	0 / 1691	-17.5	-17.5	0.33 (1)	10.00
S-R	0 / 1447	-17.5	-17.5	0.29 (1)	10.00
R-Q	0 / 1436	-17.5	-17.5	0.23 (1)	10.00
Q-P	0 / 1027	-17.5	-17.5	0.21 (1)	10.00
P-O	0 / 1262	-17.5	-17.5	0.28 (1)	10.00
O-N	0 / 0	-17.5	-17.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	-1311 / 0	0.20 (1)
C-T	0 / 1260	0.26 (1)
T-D	-23 / 40	0.01 (4)
D-S	-263 / 0	0.11 (1)
S-F	0 / 180	0.05 (4)
F-R	-484 / 0	0.45 (1)
G-R	0 / 588	0.13 (1)
R-H	0 / 822	0.19 (1)
Q-H	-974 / 0	0.56 (1)
H-P	-88 / 0	0.06 (1)
P-I	0 / 396	0.09 (1)
P-K	-359 / 0	0.45 (1)
O-K	-107 / 63	0.05 (1)
B-U	0 / 1005	0.23 (1)
O-L	0 / 1282	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.38 (K-L:1) , BC=0.33 (S-T:1) , WB=0.56 (H-Q:1) , SSI=0.17 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

A-150737 48

253451	H03T	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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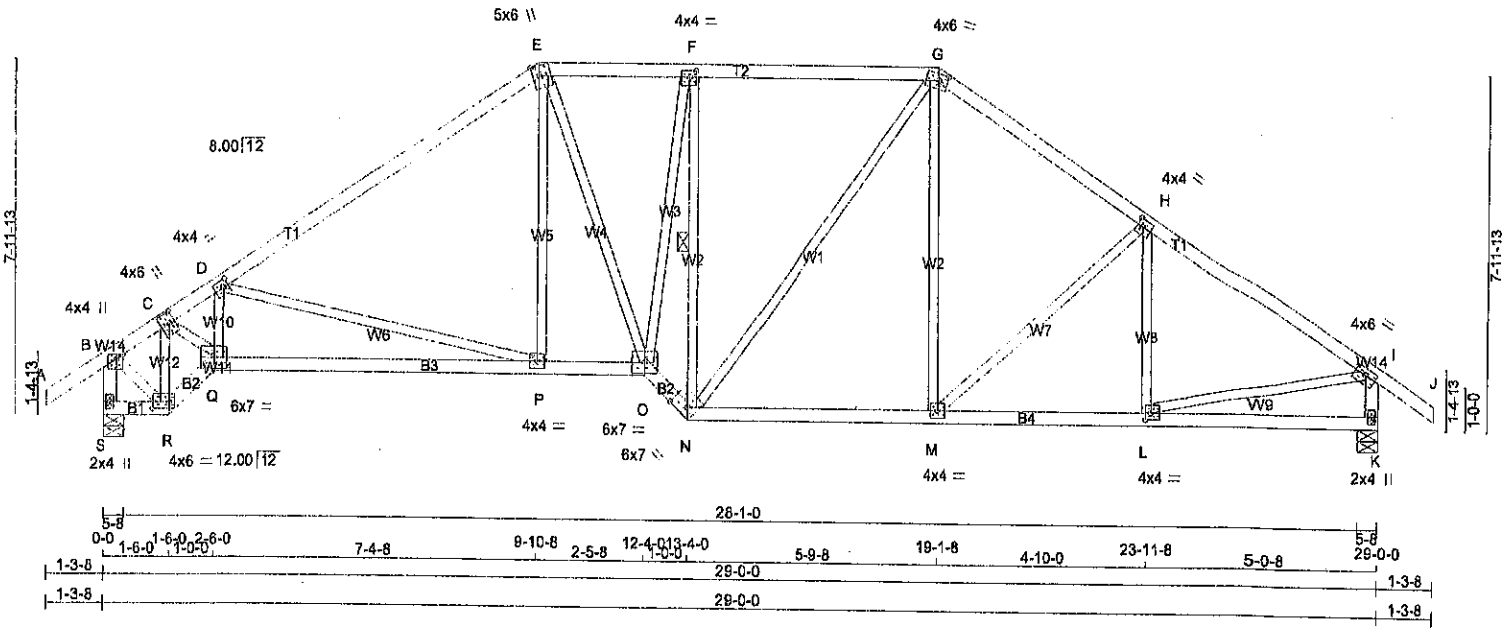

TOTAL WEIGHT = 137 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.75
C	TMWW+t	MT20	4.0	6.0	3.00	1.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
F	TMWW-t	MT20	4.0	4.0	1.75	1.75
G	TTWW-m	MT20	4.0	6.0	1.75	2.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BMWW-t	MT20	4.0	4.0		
N	BBWW-h	MT20	6.0	7.0	2.25	4.50
O	BBWW-I	MT20	6.0	7.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BBWW-I	MT20	6.0	7.0		
R	BBWW-I	MT20	4.0	6.0	1.75	4.50
S	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1374	0	1374	0	0	6-8	1-8
K	1370	0	1370	0	0	6-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	966	667 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
K	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B-C	-1024 / 0	-70.5	-70.5	0.14 (1)		6.04
C-D	-1998 / 0	-70.5	-70.5	0.43 (1)		4.29
D-E	-1529 / 0	-70.5	-70.5	0.54 (1)		4.72
E-F	-1358 / 0	-70.5	-70.5	0.23 (1)		5.32
F-G	-1224 / 0	-70.5	-70.5	0.28 (1)		5.48
G-H	-1327 / 0	-70.5	-70.5	0.24 (1)		5.37
H-I	-1474 / 0	-70.5	-70.5	0.25 (1)		5.15
I-J	0 / 27	-70.5	-70.5	0.09 (1)		10.00
S-B	-1381 / 0	0.0	0.0	0.14 (1)		6.96
K-I	-1331 / 0	0.0	0.0	0.14 (1)		7.02
S-R	0 / 0	-17.5	-17.5	0.01 (4)		10.00
R-Q	0 / 1023	-17.5	-17.5	0.17 (1)		10.00
Q-P	0 / 1866	-17.5	-17.5	0.42 (1)		10.00
P-O	0 / 1251	-17.5	-17.5	0.33 (1)		10.00
O-N	0 / 1706	-17.5	-17.5	0.28 (1)		10.00
N-M	0 / 1086	-17.5	-17.5	0.25 (1)		10.00
M-L	0 / 1245	-17.5	-17.5	0.27 (1)		10.00
L-K	0 / 0	-17.5	-17.5	0.09 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-1256 / 0	0.19 (1)
C-Q	0 / 1512	0.34 (1)
Q-D	-212 / 39	0.03 (1)
D-P	-637 / 0	0.76 (1)
P-E	0 / 266	0.06 (4)
E-O	0 / 309	0.07 (1)
O-F	0 / 935	0.21 (1)
N-F	-1333 / 0	0.54 (1)
N-G	0 / 228	0.05 (1)
M-G	0 / 247	0.06 (1)
M-H	-218 / 0	0.18 (1)
L-H	-178 / 18	0.06 (1)
B-R	0 / 963	0.22 (1)
L-I	0 / 1274	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.54 (D-E:1) , BC=0.42 (P-Q:1) , WB=0.76 (D-P:1) , SSI=0.30 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.47 (L) (INPUT = 1.00)

A-1507374?

253451	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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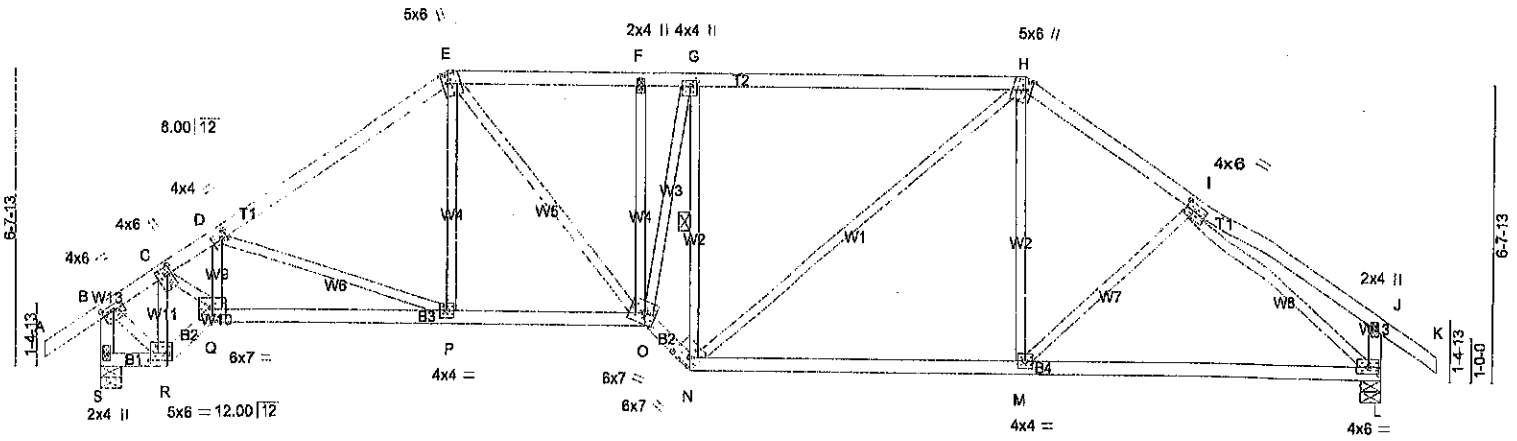
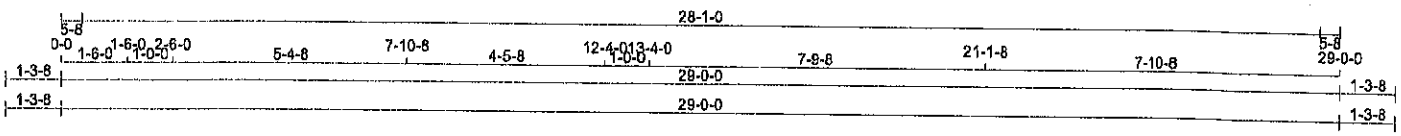




TOTAL WEIGHT = 130 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	3.00
C	TMWW+t	MT20	4.0	6.0	2.25	1.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	4.0	4.0	1.50	1.75
H	TTWW+m	MT20	6.0	6.0	2.50	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	6.0	1.75	3.00
M	BMWW-t	MT20	4.0	4.0		
N	BBWW-h	MT20	6.0	7.0	2.00	4.25
O	BBWWW-m	MT20	6.0	7.0	3.00	3.00
P	BMWW-t	MT20	4.0	4.0		
Q	BBWW-l	MT20	6.0	7.0		
R	BBWW-l	MT20	5.0	6.0	2.00	4.50
S	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1374	0	1374	0	0	5-8	1-8
L	1370	0	1370	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	966	667 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	963	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-1034 / 0	-70.5	-70.5	0.10 (1)	6.07
C- D	-1984 / 0	-70.5	-70.6	0.23 (1)	4.89
D- E	-1662 / 0	-70.5	-70.5	0.27 (1)	4.90
E- F	-1702 / 0	-70.5	-70.5	0.22 (1)	4.90
F- G	-1704 / 0	-70.5	-70.5	0.49 (1)	4.49
G- H	-1520 / 0	-70.5	-70.5	0.56 (1)	4.84
H- I	-1424 / 0	-70.5	-70.5	0.16 (1)	5.33
I- J	0 / 17	-70.5	-70.5	0.16 (1)	10.00
J- K	0 / 27	-70.5	-70.5	0.09 (1)	10.00
S- B	-1361 / 0	0.0	0.0	0.14 (1)	6.96
L- J	-205 / 0	0.0	0.0	0.02 (1)	7.81
S- R	0 / 0	-17.5	-17.5	0.01 (4)	10.00
R- Q	0 / 1051	-17.5	-17.5	0.17 (1)	10.00
Q- P	0 / 1756	-17.5	-17.5	0.35 (1)	10.00
P- O	0 / 1370	-17.5	-17.5	0.29 (1)	10.00
O- N	0 / 2111	-17.5	-17.5	0.34 (1)	10.00
N- M	0 / 1172	-17.5	-17.5	0.41 (4)	10.00
M- L	0 / 1180	-17.5	-17.5	0.40 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-1291 / 0	0.20 (1)
C- Q	0 / 1347	0.30 (1)
Q- D	-94 / 39	0.01 (1)
D- P	-412 / 0	0.23 (1)
P- E	0 / 220	0.05 (4)
E- O	0 / 516	0.12 (1)
O- F	0 / 120	0.03 (1)
O- G	0 / 1044	0.23 (1)
N- G	-1711 / 0	0.49 (1)
N- H	0 / 449	0.10 (1)
M- H	0 / 176	0.05 (4)
M- I	-15 / 49	0.02 (4)
B- R	0 / 990	0.22 (1)
I- L	-1620 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.56 (G-H:1) , BC=0.41 (M-N:4) , WB=0.77 (I-L:1) , SSI=0.28 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

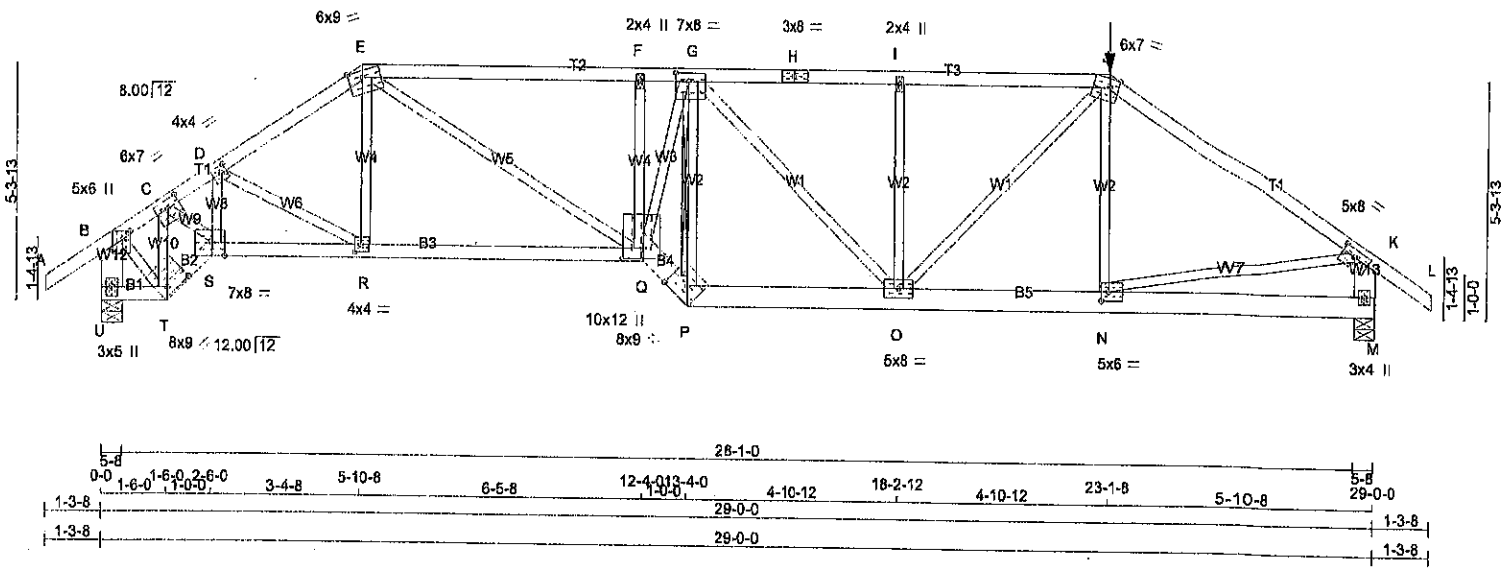
JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.40 (I) (INPUT = 1.00)

A-4073746

253451	H01T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

Scale = 1:52.3



TOTAL WEIGHT = 138 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	1650F 1.5E	SPF
H - J	2x4	DRY	1650F 1.5E	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	1650F 1.5E	SPF
Q - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.25
C	TMWW-t	MT20	6.0	7.0	2.25	3.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	6.0	9.0	1.75	4.00
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	7.0	8.0	2.25	3.50
H	TS-t	MT20	3.0	8.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW-m	MT20	6.0	7.0	Edge	2.50
K	TMVW-t	MT20	5.0	8.0	2.00	3.75
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWWW-t	MT20	5.0	8.0		
P	BBW-h	MT20	8.0	9.0	3.75	5.00
Q	BBWWW+p	MT20	10.0	12.0	2.75	5.50
R	BMWW-t	MT20	4.0	4.0	2.00	1.75
S	BBWW-t	MT20	7.0	8.0	Edge	3.50
T	BBWW-h	MT20	8.0	9.0	2.00	6.25
U	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2641	0	2641	0	0	5-8	3-1
M	2489	0	2489	0	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1858	1276 / 0	0 / 0	0 / 0	0 / 0	582 / 0	0 / 0
M	1754	1185 / 0	0 / 0	0 / 0	0 / 0	569 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT G-P
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.10 (1)	10.00
B-C	-2095 / 0	-138.7	-138.7	0.18 (1)	4.49
C-D	-3985 / 0	-138.7	-138.7	0.32 (1)	3.22
D-E	-3669 / 0	-70.5	-70.5	0.33 (1)	3.39
E-F	-4645 / 0	-70.5	-70.5	0.68 (1)	3.28
F-G	-4636 / 0	-138.7	-138.7	0.36 (1)	3.42
G-H	-3575 / 0	-138.7	-138.7	0.56 (1)	3.64
H-I	-3575 / 0	-138.7	-138.7	0.56 (1)	3.64
I-J	-3574 / 0	-138.7	-138.7	0.56 (1)	3.64
J-K	-3019 / 0	-70.5	-70.5	0.87 (1)	3.24
K-L	0 / 27	-70.5	-70.5	0.10 (1)	10.00
U-B	-2615 / 0	0.0	0.0	0.19 (1)	6.39
M-K	-2401 / 0	0.0	0.0	0.17 (1)	6.61
U-T	0 / 0	-34.5	-34.5	0.02 (4)	10.00
T-S	0 / 2196	-34.5	-34.5	0.39 (1)	10.00
S-R	0 / 3352	-102.7	-102.7	0.91 (1)	10.00
R-Q	0 / 3062	-102.7	-102.7	0.87 (1)	10.00
Q-P	0 / 5157	-34.5	-34.5	0.67 (1)	10.00
P-O	0 / 3797	-34.5	-34.5	0.59 (1)	10.00
O-N	0 / 2506	-34.5	-34.5	0.39 (1)	10.00
N-M	0 / 0	-34.5	-34.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
T-C	-2699 / 0	0.45 (1)
C-S	0 / 2413	0.60 (1)
S-D	0 / 66	0.02 (4)
D-R	-352 / 0	0.10 (1)
R-E	0 / 769	0.19 (1)
E-Q	0 / 1851	0.46 (1)
Q-F	-320 / 0	0.09 (1)
Q-G	0 / 3380	0.84 (1)
P-G	-3421 / 0	0.77 (1)
G-O	-319 / 0	0.28 (1)
O-I	-756 / 0	0.31 (1)
O-J	0 / 1517	0.38 (1)
N-J	-189 / 109	0.08 (1)
B-T	0 / 2068	0.51 (1)
N-K	0 / 2544	0.83 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	23-1-8	-302	-302	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 12-4-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 16-8-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 595 (0.59")

CSI: TC=0.87 (J-K:1) , BC=0.91 (R-S:1) , WB=0.84 (G-O:1) , SSI=0.35 (I-J:1)

A-1507345

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 23-1-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.66 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.16.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

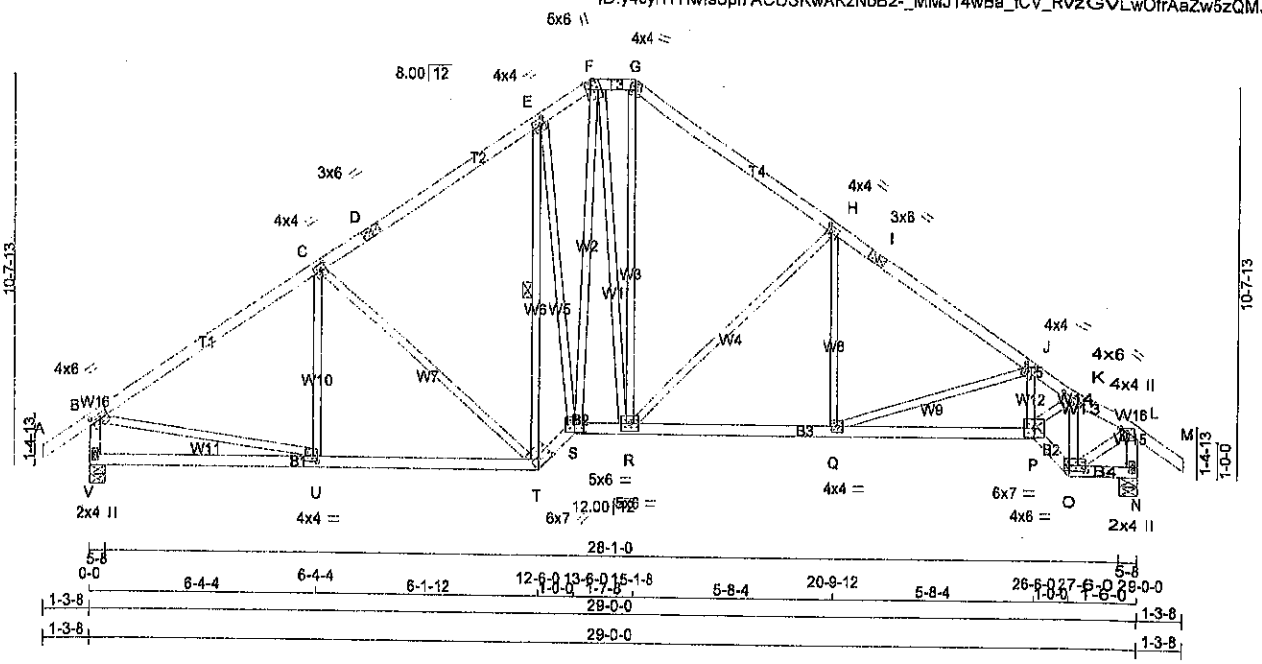
A-N073745(2)

253450	H05T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

ID:y40yHYHwfsopi7ACUSKwAKzNoB2-_MMJ14wBa_tCV_RvzGvLwOfrAaZw5zQMJ5CYNcyxXx

Scale: 3/16"=1'



TOTAL WEIGHT = 162 lb [M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C, E, H, J						
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
F	TTWW+m	MT20	6.0	6.0	2.50	1.50
G	TTW-m	MT20	4.0	4.0		
I	TS-t	MT20	3.0	6.0		
K	TMWW-t	MT20	4.0	6.0	1.50	3.00
L	TMVW+p	MT20	4.0	4.0	1.25	1.75
N	BMV1+p	MT20	2.0	4.0		
O	BBWW-t	MT20	4.0	6.0	1.75	4.50
P	BBWW-t	MT20	6.0	7.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BBWW-t	MT20	5.0	6.0	3.00	3.00
T	BBWW-h	MT20	6.0	7.0	2.25	4.50
U	BMWW-t	MT20	4.0	4.0	1.75	1.50
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1370	0	1370	0	0	5-8	1-8
N	1374	0	1374	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	963	865 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
N	966	867 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-T.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B-C	-1486 / 0	-70.5	-70.5	0.40 (1)	4.96
C-D	-1189 / 0	-70.5	-70.5	0.36 (1)	5.45
D-E	-1189 / 0	-70.5	-70.5	0.36 (1)	5.45
E-F	-1288 / 0	-70.5	-70.5	0.17 (1)	5.52
F-G	-990 / 0	-70.5	-70.5	0.02 (1)	6.25
G-H	-1209 / 0	-70.5	-70.5	0.30 (1)	5.49
H-I	-1865 / 0	-70.5	-70.5	0.32 (1)	4.85
I-J	-1865 / 0	-70.5	-70.5	0.32 (1)	4.85
J-K	-1978 / 0	-70.5	-70.5	0.16 (1)	4.68
K-L	-1038 / 0	-70.5	-70.5	0.08 (1)	6.09
L-M	0 / 27	-70.5	-70.5	0.09 (1)	10.00
V-B	-1326 / 0	0.0	0.0	0.14 (1)	7.03
N-L	-1361 / 0	0.0	0.0	0.14 (1)	6.96
V-U	0 / 0	-17.5	-17.5	0.19 (4)	10.00
U-T	0 / 1259	-17.5	-17.5	0.30 (1)	10.00
T-S	0 / 1390	-17.5	-17.5	0.23 (1)	10.00
S-R	0 / 987	-17.5	-17.5	0.23 (1)	10.00
R-Q	0 / 1397	-17.5	-17.5	0.30 (1)	10.00
Q-P	0 / 1715	-17.5	-17.5	0.34 (1)	10.00
P-O	0 / 1061	-17.5	-17.5	0.17 (1)	10.00
O-N	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	-87 / 81	0.04 (1)
C-T	-372 / 0	0.61 (1)
T-E	-684 / 0	0.43 (1)
E-S	0 / 301	0.07 (1)
S-F	0 / 690	0.16 (1)
F-R	0 / 27	0.01 (4)
R-G	0 / 458	0.10 (1)
R-H	-571 / 0	0.77 (1)
Q-H	0 / 202	0.05 (4)
Q-J	-334 / 0	0.20 (1)
P-J	-44 / 46	0.02 (4)
P-K	0 / 1286	0.29 (1)
O-K	-1304 / 0	0.20 (1)
B-U	0 / 1277	0.29 (1)
O-L	0 / 1000	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.40 (B-C:1) , BC=0.34 (P-Q:1) , WB=0.77 (H-R:1) , SSI=0.18 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.47 (U) (INPUT = 1.00)

A-1073744

253450	H04T	1	1	TRUSS DESC.

Alpa Roof Truss,*Maple

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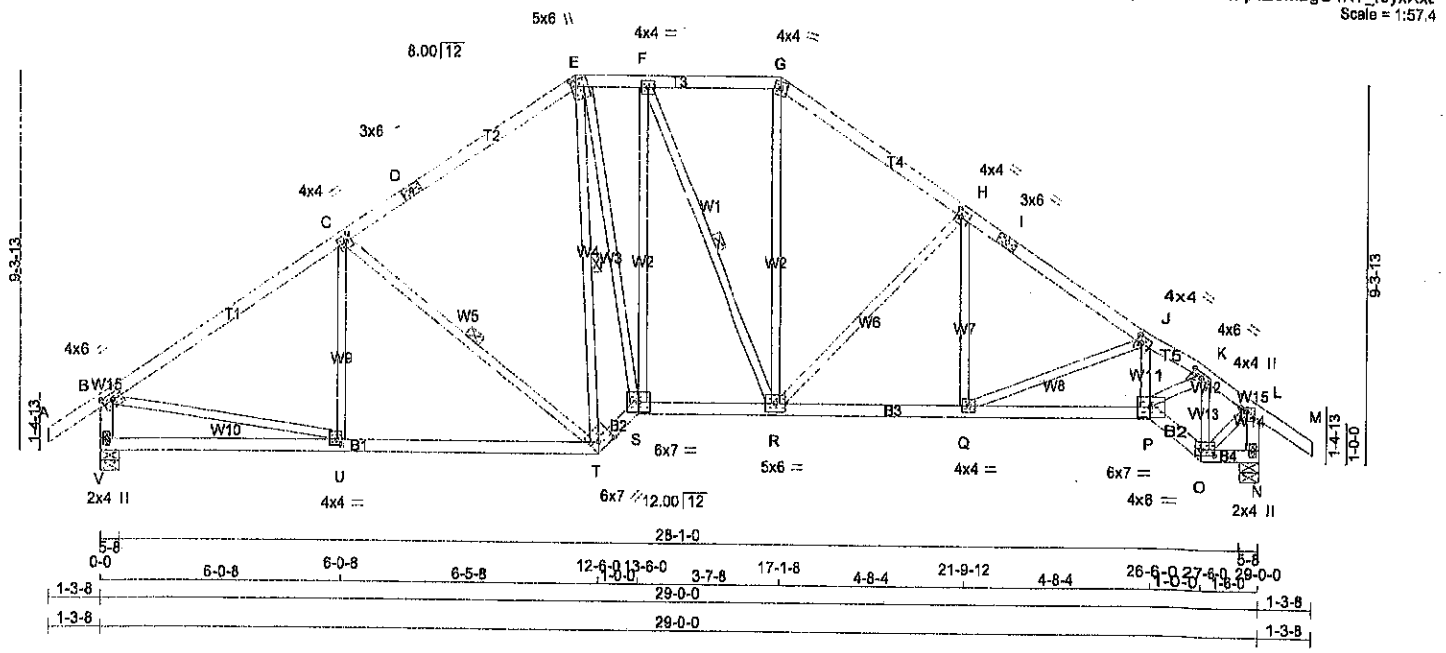


TOTAL WEIGHT = 145 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C, H, J						
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	6.0	6.0	2.00	1.25
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
I	TS-t	MT20	3.0	6.0		
K	TMWW-t	MT20	4.0	6.0	1.50	3.00
L	TMVW+p	MT20	4.0	4.0	1.25	1.75
N	BMV1+p	MT20	2.0	4.0		
O	BBWW-t	MT20	4.0	6.0	1.75	4.50
P	BBWW-t	MT20	6.0	7.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	6.0	6.0		
S	BBWW-t	MT20	6.0	7.0		
T	BBWW-h	MT20	6.0	7.0	2.25	4.50
U	BMWW-t	MT20	4.0	4.0	1.75	1.50
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1370	0	1370	0	0	5-8	1-8
N	1374	0	1374	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
N	966	667 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.73FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-T, E-T, F-R.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-1493 / 0	-70.5	-70.5	0.38 (1)	4.97
C- D	-1200 / 0	-70.5	-70.5	0.35 (1)	5.43
D- E	-1200 / 0	-70.5	-70.5	0.36 (1)	5.43
E- F	-1161 / 0	-70.5	-70.5	0.05 (1)	5.89
F- G	-1116 / 0	-70.5	-70.5	0.13 (1)	5.87
G- H	-1355 / 0	-70.5	-70.5	0.20 (1)	5.37
H- I	-1724 / 0	-70.5	-70.5	0.22 (1)	4.88
I- J	-1724 / 0	-70.5	-70.5	0.22 (1)	4.88
J- K	-1975 / 0	-70.5	-70.5	0.10 (1)	4.73
K- L	-1040 / 0	-70.5	-70.5	0.08 (1)	6.08
L- M	0 / 27	-70.5	-70.5	0.09 (1)	10.00
V- B	-1329 / 0	0.0	0.0	0.14 (1)	7.02
N- L	-1361 / 0	0.0	0.0	0.14 (1)	6.96
V- U	0 / 0	-17.5	-17.5	0.19 (4)	10.00
U- T	0 / 1265	-17.5	-17.5	0.30 (1)	10.00
T- S	0 / 1314	-17.5	-17.5	0.21 (1)	10.00
S- R	0 / 1152	-17.5	-17.5	0.22 (1)	10.00
R- Q	0 / 1445	-17.5	-17.5	0.27 (1)	10.00
Q- P	0 / 1691	-17.5	-17.5	0.32 (1)	10.00
P- O	0 / 1069	-17.5	-17.5	0.17 (1)	10.00
O- N	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-99 / 76	0.04 (1)
C- T	-371 / 0	0.17 (1)
E- T	-641 / 0	0.37 (1)
E- S	0 / 1111	0.25 (1)
S- F	-119 / 0	0.16 (1)
F- R	-92 / 0	0.06 (1)
R- G	0 / 495	0.11 (1)
R- H	-477 / 0	0.41 (1)
Q- H	0 / 176	0.04 (4)
Q- J	-265 / 0	0.11 (1)
P- J	-21 / 43	0.01 (4)
P- K	0 / 1250	0.28 (1)
O- K	-1311 / 0	0.20 (1)
B- U	0 / 1285	0.29 (1)
O- L	0 / 1005	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.38 (B-C:1) , BC=0.32 (P-Q:1) , WB=0.41 (H-R:1) , SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

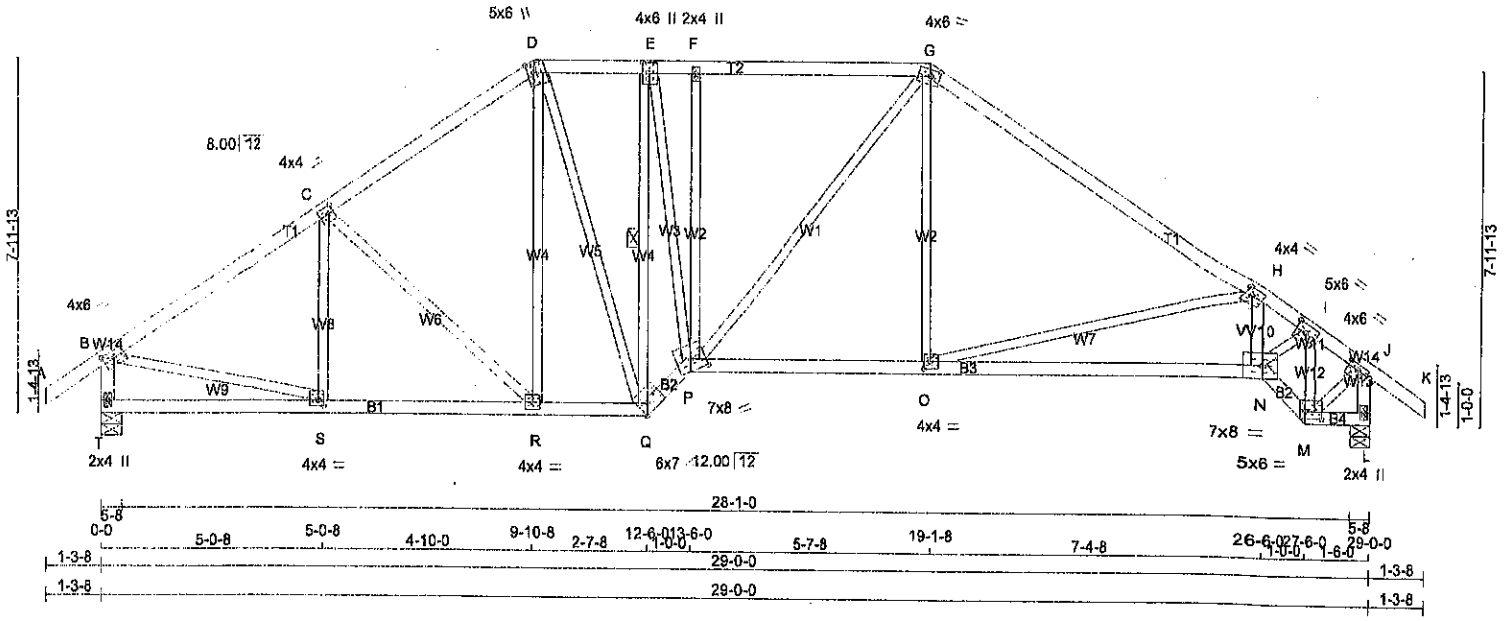
JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.47 (U) (INPUT = 1.00)

A-150137443

253450	H03T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

Scale = 1:52.5



TOTAL WEIGHT = 142 lb
[M][F]

LUMBER
N. L. G. A. RULES

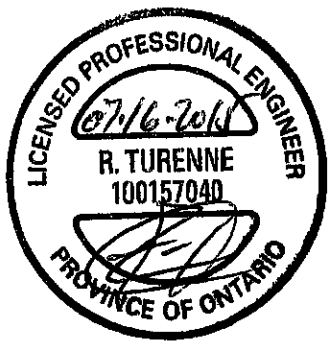
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMWW+t	MT20	4.0	6.0	3.00	1.75
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	4.0	6.0	1.75	2.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-t	MT20	5.0	6.0	2.25	3.00
J	TMW-t	MT20	4.0	6.0	1.50	3.00
L	BMV1+p	MT20	2.0	4.0		
M	BBWW-l	MT20	5.0	6.0	2.00	4.50
N	BBWW-l	MT20	7.0	8.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BBWWW-m	MT20	7.0	8.0	3.25	4.00
Q	BBWW-h	MT20	6.0	7.0	Edge	2.50
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	4.0	4.0	1.50	1.50
T	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1370	0	1370	0	0	5-8	1-8
L	1374	0	1374	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	966	667 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-1476 / 0	-70.5	-70.5	0.25 (1)	5.16
C- D	-1323 / 0	-70.5	-70.5	0.24 (1)	5.38
D- E	-1186 / 0	-70.5	-70.5	0.07 (1)	5.80
E- F	-1398 / 0	-70.5	-70.5	0.19 (1)	5.32
F- G	-1404 / 0	-70.5	-70.5	0.29 (1)	5.19
G- H	-1630 / 0	-70.5	-70.5	0.54 (1)	4.72
H- I	-1998 / 0	-70.5	-70.5	0.43 (1)	4.29
I- J	-1024 / 0	-70.5	-70.5	0.14 (1)	6.04
J- K	0 / 27	-70.5	-70.5	0.09 (1)	10.00
T- B	-1332 / 0	0.0	0.0	0.14 (1)	7.02
L- J	-1361 / 0	0.0	0.0	0.14 (1)	6.96
T- S	0 / 0	-17.5	-17.5	0.10 (4)	10.00
S- R	0 / 1247	-17.5	-17.5	0.25 (1)	10.00
R- Q	0 / 1082	-17.5	-17.5	0.21 (1)	10.00
Q- P	0 / 1701	-17.5	-17.5	0.26 (1)	10.00
P- O	0 / 1252	-17.5	-17.5	0.33 (1)	10.00
O- N	0 / 1866	-17.5	-17.5	0.43 (1)	10.00
N- M	0 / 1023	-17.5	-17.5	0.17 (1)	10.00
M- L	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-171 / 27	0.06 (1)
C- R	-225 / 0	0.18 (1)
R- D	0 / 211	0.05 (1)
D- Q	0 / 319	0.07 (1)
Q- E	-1471 / 0	0.59 (1)
E- P	0 / 1443	0.32 (1)
P- F	-359 / 0	0.30 (1)
P- G	0 / 235	0.05 (1)
O- G	0 / 281	0.07 (4)
O- H	-637 / 0	0.75 (1)
N- H	-212 / 39	0.03 (1)
N- I	0 / 1612	0.34 (1)
M- I	-1255 / 0	0.19 (1)
B- S	0 / 1275	0.29 (1)
M- J	0 / 963	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012., BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.54 (G-H:1) , BC=0.43 (N-O:1) , WB=0.75 (H-O:1) , SSI=0.30 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.47 (S) (INPUT = 1.00)

A-150737 42

253450	H02T	1	1	TRUSS DESC.

Alpa Roof Trusst Maple

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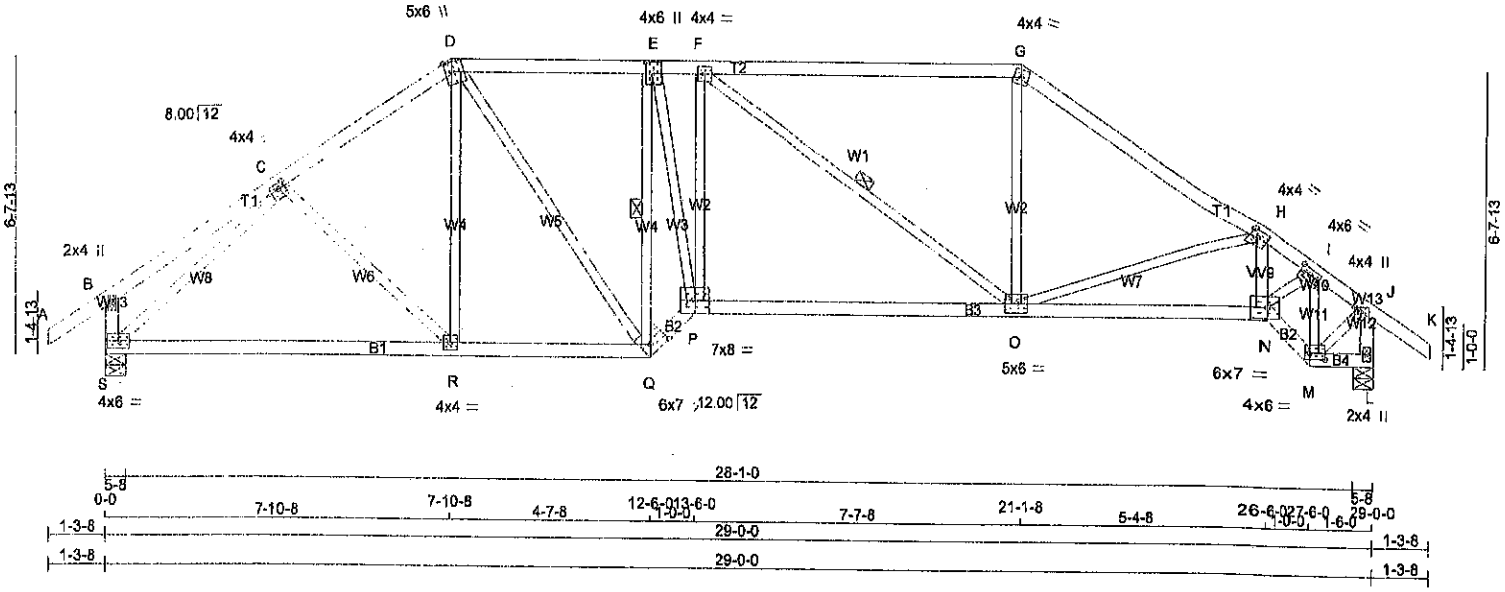

TOTAL WEIGHT = 131 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.75
D	TTWW+m	MT20	5.0	8.0	Edge	1.75
E	TMWW+t	MT20	4.0	6.0	3.00	1.50
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-t	MT20	4.0	6.0	1.50	3.00
J	TMVW+p	MT20	4.0	4.0	1.25	1.75
L	BMV1+p	MT20	2.0	4.0		
M	BBWW-t	MT20	4.0	6.0	1.75	4.50
N	BBWW-t	MT20	6.0	7.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BBWW-t	MT20	7.0	6.0		
Q	BBWW-h	MT20	6.0	7.0	2.25	4.50
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1374	0	1374	0	0	5-8	1-8
S	1370	0	1370	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	968	667 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
S	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-Q, F-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	0 / 17	-70.5	-70.5	0.16 (1)	10.00
C- D	-1418 / 0	-70.5	-70.5	0.15 (1)	5.34
D- E	-1449 / 0	-70.5	-70.5	0.20 (1)	5.24
E- F	-1787 / 0	-70.5	-70.5	0.43 (1)	4.50
F- G	-1384 / 0	-70.5	-70.5	0.51 (1)	4.90
G- H	-1672 / 0	-70.5	-70.5	0.27 (1)	4.88
H- I	-1984 / 0	-70.5	-70.5	0.22 (1)	4.69
I- J	-1034 / 0	-70.5	-70.5	0.10 (1)	6.07
J- K	0 / 27	-70.5	-70.5	0.09 (1)	10.00
S- B	-205 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-1361 / 0	0.0	0.0	0.14 (1)	6.96
S- R	0 / 1176	-17.5	-17.5	0.33 (4)	10.00
R- Q	0 / 1168	-17.5	-17.5	0.34 (4)	10.00
Q- P	0 / 2083	-17.5	-17.5	0.34 (1)	10.00
P- O	0 / 1792	-17.5	-17.5	0.42 (1)	10.00
O- N	0 / 1756	-17.5	-17.5	0.41 (1)	10.00
N- M	0 / 1051	-17.5	-17.5	0.17 (1)	10.00
M- L	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	-18 / 48	0.02 (4)
R- D	0 / 146	0.04 (4)
D- Q	0 / 478	0.11 (1)
Q- E	-1824 / 0	0.52 (1)
E- P	0 / 1851	0.42 (1)
P- F	-283 / 11	0.14 (1)
F- O	-503 / 0	0.29 (1)
O- G	0 / 550	0.12 (1)
O- H	-403 / 0	0.22 (1)
N- H	-101 / 31	0.02 (1)
N- I	0 / 1346	0.30 (1)
M- I	-1291 / 0	0.20 (1)
S- C	-1615 / 0	0.76 (1)
M- J	0 / 990	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.61 (F-G:1) , BC=0.42 (O-P:1) , WB=0.76 (C-S:1) , SSI=0.26 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (S) (INPUT = 0.90)
JSI METAL= 0.60 (C) (INPUT = 1.00)

A-4073741

253450	H01T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

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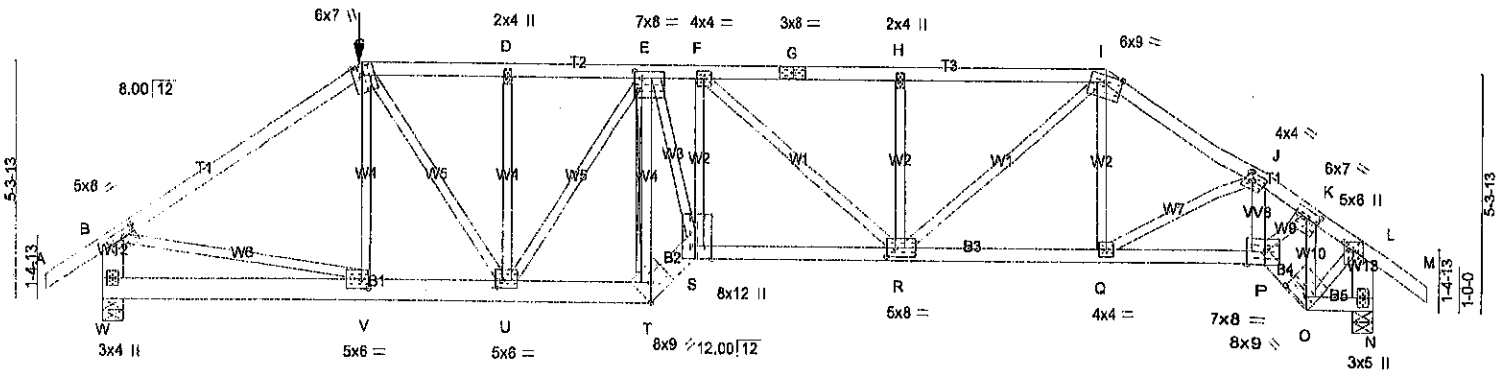

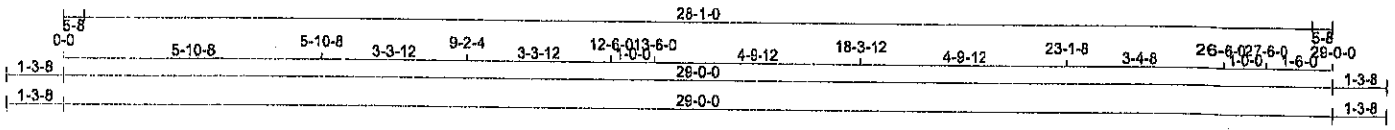

TOTAL WEIGHT = 141 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
S - P	2x4	DRY	1650F 1.5E	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.75
C	TTWW+m	MT20	6.0	7.0	Edge	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	7.0	8.0	1.75	2.00
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	6.0	9.0	Edge	
J	TMWW-t	MT20	4.0	4.0	2.00	1.50
K	TMWW-t	MT20	6.0	7.0	2.25	3.50
L	TMVW+p	MT20	5.0	6.0	2.26	2.25
N	BMV1+p	MT20	3.0	5.0		
O	BBWW-h	MT20	6.0	9.0	2.00	6.25
P	BBWW-t	MT20	7.0	8.0	Edge	3.50
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	2.50
S	BBWW+p	MT20	8.0	12.0	Edge	4.75
T	BBW-h	MT20	6.0	9.0	Edge	4.00
U	BMWWW-t	MT20	5.0	6.0	2.00	2.00
V	BMWW-t	MT20	5.0	6.0	2.50	2.00
W	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
W	2490	0	2490	0	0	5-8	2-11
N	2640	0	2640	0	0	5-8	3-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1755	1186 / 0	0 / 0	0 / 0	0 / 0	569 / 0	0 / 0
N	1857	1275 / 0	0 / 0	0 / 0	0 / 0	582 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.82FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-T
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.10 (1)	10.00
B-C	-3020 / 0	-70.5	-70.5	0.87 (1)	3.24
C-D	-3272 / 0	-138.7	-138.7	0.36 (1)	3.52
D-E	-3272 / 0	-138.7	-138.7	0.36 (1)	3.52
E-F	-4688 / 0	-138.7	-138.7	0.45 (1)	2.82
F-G	-4361 / 0	-70.5	-70.5	0.62 (1)	2.85
G-H	-4361 / 0	-70.5	-70.5	0.62 (1)	2.85
H-I	-4361 / 0	-70.5	-70.5	0.63 (1)	2.85
I-J	-3602 / 0	-70.5	-70.5	0.32 (1)	3.43
J-K	-3983 / 0	-138.7	-138.7	0.32 (1)	3.22
K-L	-2094 / 0	-138.7	-138.7	0.18 (1)	4.48
L-M	0 / 27	-70.5	-70.5	0.10 (1)	10.00
W-B	-2401 / 0	0.0	0.0	0.17 (1)	6.61
N-L	-2614 / 0	0.0	0.0	0.19 (1)	6.39
W-V	0 / 0	-34.5	-34.5	0.14 (4)	10.00
V-U	0 / 2506	-34.5	-34.5	0.43 (1)	10.00
U-T	0 / 3736	-34.5	-34.5	0.69 (1)	10.00
T-S	0 / 5075	-34.5	-34.5	0.66 (1)	10.00
S-R	0 / 4684	-102.7	-102.7	0.88 (1)	10.00
R-Q	0 / 3003	-102.7	-102.7	0.66 (1)	10.00
Q-P	0 / 3348	-102.7	-102.7	0.63 (1)	10.00
P-O	0 / 2194	-34.5	-34.5	0.39 (1)	10.00
O-N	0 / 0	-34.5	-34.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-174 / 105	0.07 (1)
C-U	0 / 1373	0.34 (1)
U-D	-483 / 0	0.20 (1)
U-E	-860 / 0	0.60 (1)
T-E	-3341 / 0	0.76 (1)
E-S	0 / 3793	0.94 (1)
S-F	0 / 165	0.04 (1)
F-R	-425 / 0	0.29 (1)
R-H	-395 / 0	0.12 (1)
R-I	0 / 1768	0.44 (1)
Q-I	0 / 618	0.16 (1)
Q-J	-411 / 0	0.11 (1)
P-J	0 / 129	0.03 (4)
P-K	0 / 2413	0.60 (1)
O-K	-2697 / 0	0.45 (1)
B-V	0 / 2544	0.63 (1)
O-L	0 / 2067	0.51 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 13-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 13-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 26-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 686 (0.51")

CSI: TC=0.87 (B-C:1) , BC=0.88 (R-S:1) , WB=0.94 (E-S:1) , SSI=0.25 (R-S:1)

A-4073760

CONTINUED ON PAGE 2

253449	H11A	1	2	TRUSS DESC.

Alpa Roof Truss, Maple

ID:y40yHYHwfsopi7ACUSKwAKzNoB2-HEOH2luZBnaR9HI7L1FY9bGutwPAgvuToxKFW5yxYO1

3x8 II

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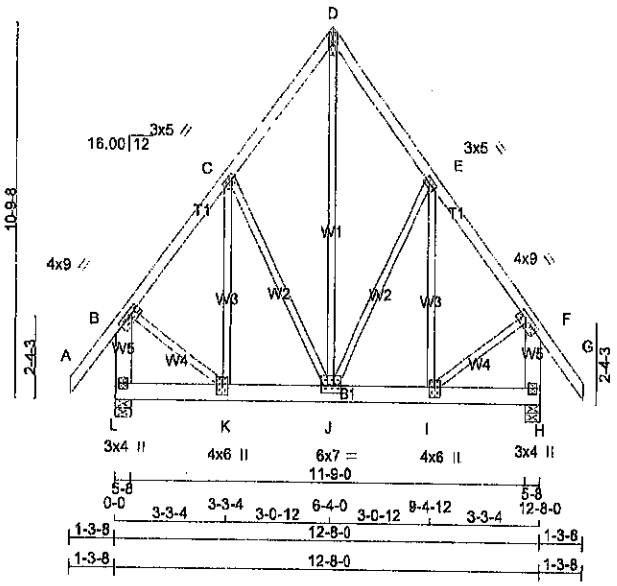

TOTAL WEIGHT = 2 X 94 = 187 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
L- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- H	2	6	SIDE(359.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	9.0	1.50	3.00
C	TMWW-1	MT20	3.0	5.0	1.50	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	5211	0	5211	0	0	5-8	3-0
H	5211	0	5211	0	0	5-8	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	MAX./MIN. COMPONENT REACTIONS PERM.LIVE	WIND	DEAD	SOIL
L	3669	2498 / 0	0 / 0	0 / 0	0 / 0	1171 / 0	0 / 0
H	3669	2498 / 0	0 / 0	0 / 0	0 / 0	1171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.06 (1)		10.00
B- C	-3463 / 0	-70.5	-70.5	0.12 (1)		4.96
C- D	-2700 / 0	-70.5	-70.5	0.09 (1)		5.49
D- E	-2700 / 0	-70.5	-70.5	0.09 (1)		5.49
E- F	-3463 / 0	-70.5	-70.5	0.12 (1)		4.96
F- G	0 / 40	-70.5	-70.5	0.06 (1)		10.00
L- B	-4176 / 0	0.0	0.0	0.19 (1)		6.99
H- F	-4176 / 0	0.0	0.0	0.19 (1)		6.99
L- K	0 / 0	-736.7	-736.7	0.27 (1)		10.00
K- J	0 / 2082	-736.7	-736.7	0.34 (1)		10.00
J- I	0 / 2082	-736.7	-736.7	0.34 (1)		10.00
I- H	0 / 0	-736.7	-736.7	0.27 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 4111	0.61 (1)
J- E	-1059 / 0	0.49 (1)
I- E	0 / 1331	0.16 (1)
C- J	-1059 / 0	0.49 (1)
K- C	0 / 1331	0.16 (1)
B- K	0 / 2400	0.30 (1)
I- F	0 / 2400	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 34-8-8
END DISTANCE = 12-8-0
END SPAN CARRIED = 34-8-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.19 (F-H:1) , BC=0.34 (J-K:1) , WB=0.61 (D-J:1) , SSI=0.46 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.38 (D) (INPUT = 1.00)

A-N073739

253449	H11	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:y40yHYHwfsopi7ACUSKwAKzNoB2-HEOH2luZBnaR9HI7L1FY9bGvCwUAg_7ToxKFW5yxYOf

3x5 II

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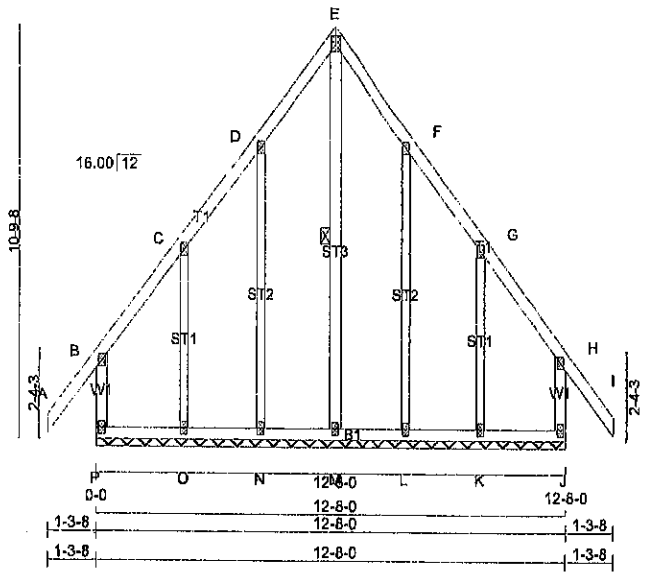

TOTAL WEIGHT = 80 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2	SPF
ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X	
B	TMV+p	MT20	2.0	4.0			
C, D, F, G							
C	TMW+w	MT20	2.0	4.0			
E	TTW+p	MT20	3.0	6.0	2.00		Edge
H	TMV+p	MT20	2.0	4.0			
J	BMV1+p	MT20	2.0	4.0			
K, L, M, N, O							
K	BMW1+w	MT20	2.0	4.0			
P	BMV1+p	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-B	-195 / 0	0.0	0.0	0.02 (1)	7.81	M- E	-139 / 0	0.08 (1)
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	N- D	-139 / 0	0.17 (1)
B-C	-21 / 0	-70.5	-70.5	0.04 (1)	6.25	O- C	-142 / 0	0.06 (1)
C-D	-13 / 0	-70.5	-70.5	0.03 (1)	6.25	L- F	-139 / 0	0.17 (1)
D-E	-7 / 0	-70.5	-70.5	0.04 (1)	10.00	K- G	-142 / 0	0.06 (1)
E-F	-7 / 0	-70.5	-70.5	0.04 (1)	10.00			
F-G	-13 / 0	-70.5	-70.5	0.03 (1)	6.25			
G-H	-21 / 0	-70.5	-70.5	0.04 (1)	6.25			
H-I	0 / 40	-70.5	-70.5	0.10 (1)	10.00			
J-H	-195 / 0	0.0	0.0	0.02 (1)	7.81			
P-O	0 / 11	-17.5	-17.5	0.02 (4)	10.00			
O-N	0 / 8	-17.5	-17.5	0.02 (4)	10.00			
N-M	0 / 5	-17.5	-17.5	0.02 (4)	10.00			
M-L	0 / 5	-17.5	-17.5	0.02 (4)	10.00			
L-K	0 / 8	-17.5	-17.5	0.02 (4)	10.00			
K-J	0 / 11	-17.5	-17.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1) , BC=0.02 (J-K:4) , WB=0.17 (D-N:1) , SSI=0.05 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (H) (INPUT = 0.90)
JSI METAL= 0.06 (H) (INPUT = 1.00)

A-N073738

253449	H09G	2	1	TRUSS DESC.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 09:22:05 2015 Page 1
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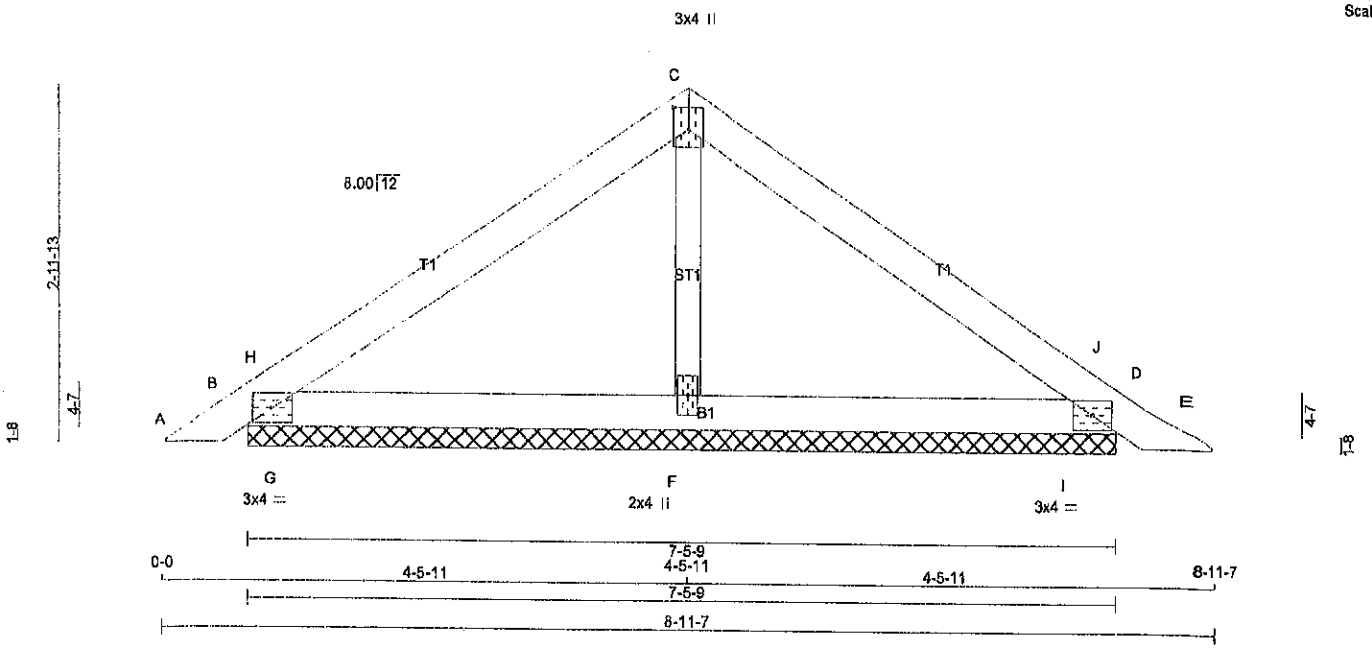

TOTAL WEIGHT = 2 X 23 = 45 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 11	-70.5	-70.5	0.02 (1)	10.00	F-C	-112 / 0	0.02 (1)
B-H	-29 / 0	-70.5	-70.5	0.05 (1)	6.25	G-H	-256 / 0	0.00 (1)
H-C	-102 / 0	-70.5	-70.5	0.12 (1)	6.25	I-J	-256 / 0	0.00 (1)
C-J	-102 / 0	-70.5	-70.5	0.12 (1)	6.25			
J-D	-29 / 0	-70.5	-70.5	0.05 (1)	6.25			
D-E	0 / 11	-70.5	-70.5	0.02 (1)	10.00			
B-G	0 / 80	-17.5	-17.5	0.11 (1)	10.00			
G-F	0 / 80	-17.5	-17.5	0.11 (1)	10.00			
F-I	0 / 80	-17.5	-17.5	0.11 (1)	10.00			
I-D	0 / 80	-17.5	-17.5	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12 (C-H:1) , BC=0.11 (B-G:1) , WB=0.02 (C-F:1) , SSI=0.19 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

A-1073737

253449	H09	14	1	TRUSS DESC.

Alpa Roof Truss, Maple

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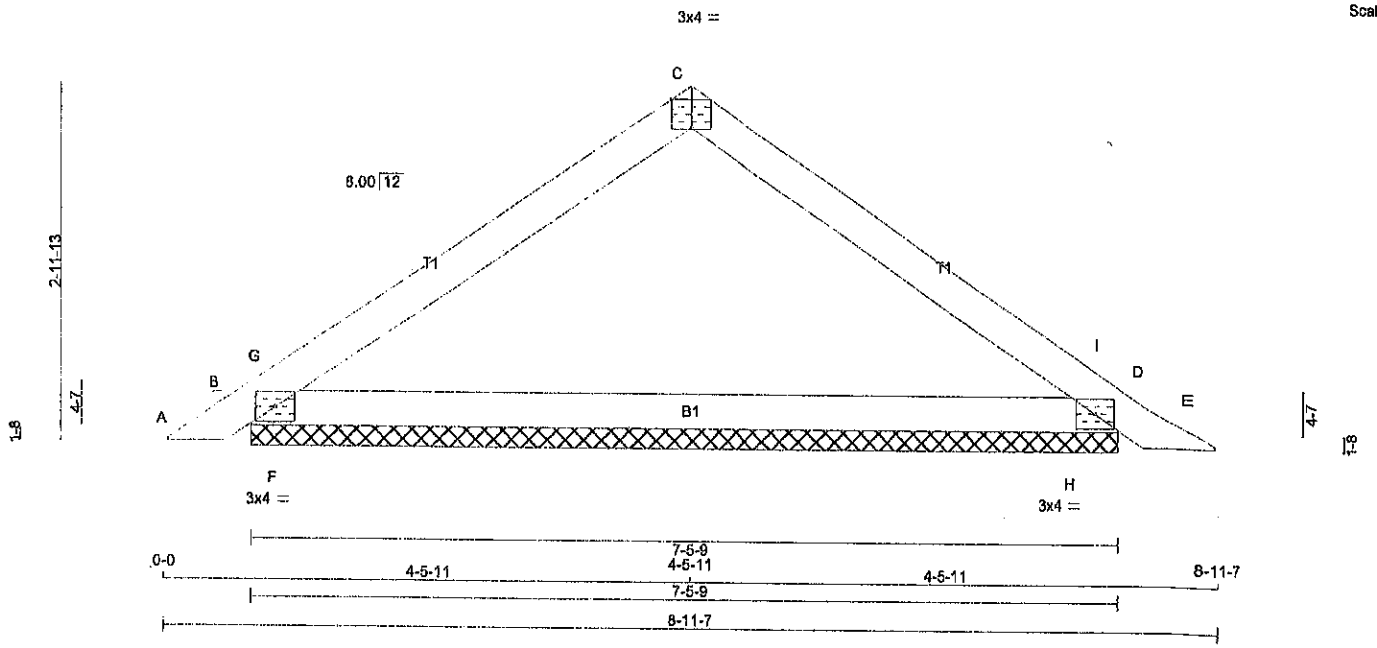

TOTAL WEIGHT = 14 X 21 = 280 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	373	0	373	0	0	7-5-9	1-8
D	373	0	373	0	0	7-5-9	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	262	184 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
D	262	184 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 11	-70.5	-70.5	0.02 (1)	10.00	F- G	0 / 218	0.00 (1)
B- G	-375 / 0	-70.5	-70.5	0.13 (4)	6.25	H- I	0 / 218	0.00 (1)
G- C	-237 / 0	-70.5	-70.5	0.18 (1)	6.25			
C- I	-237 / 0	-70.5	-70.5	0.18 (1)	6.25			
I- D	-375 / 0	-70.5	-70.5	0.13 (4)	6.25			
D- E	0 / 11	-70.5	-70.5	0.02 (1)	10.00			
B- F	0 / 204	-17.5	-17.5	0.11 (4)	10.00			
F- H	0 / 204	-17.5	-17.5	0.16 (1)	10.00			
H- D	0 / 204	-17.5	-17.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18 (C-G:1) , BC=0.16 (F-H:1) , WB=0.00 (F-G:1) , SSI=0.19 (D-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.16.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073736

253449	H08G	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

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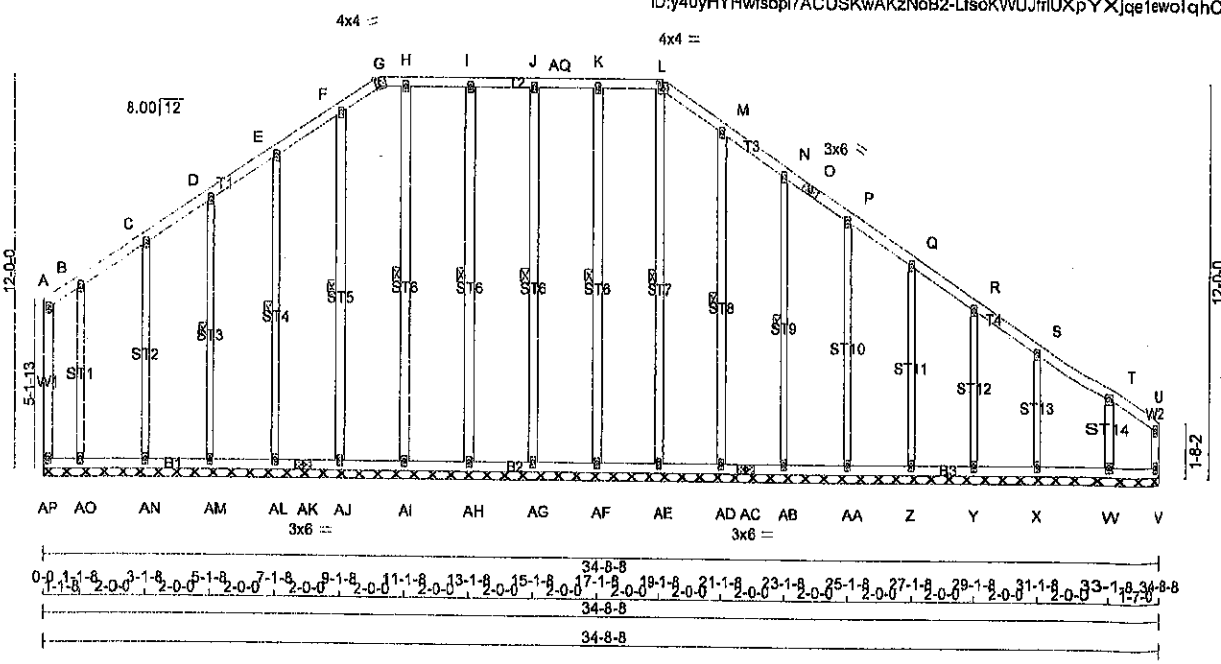

TOTAL WEIGHT = 232 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF
O - U	2x4	DRY	No.2	SPF
AP- A	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
AP- AK	2x4	DRY	No.2	SPF
AK- AC	2x4	DRY	No.2	SPF
AC- V	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
AD- M	2x4	DRY	No.2	SPF
AE- L	2x4	DRY	No.2	SPF
AF- K	2x4	DRY	No.2	SPF
AG- J	2x4	DRY	No.2	SPF
AH- I	2x4	DRY	No.2	SPF
AI - H	2x4	DRY	No.2	SPF
AJ- F	2x4	DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2	SPF
ST7	2x4	DRY	No.2	SPF
ST8	2x4	DRY	No.2	SPF
ST9	2x4	DRY	No.2	SPF
ST10	2x4	DRY	No.2	SPF
ST11	2x4	DRY	No.2	SPF
ST12	2x4	DRY	No.2	SPF
ST13	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, D, E, F, H, I, J, K, M, N, P, Q, R, S, T						
B	TMW+w	MT20	2.0	4.0		
G	TT-m	MT20	4.0	4.0	Edge	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF N-AB, M-AD, L-AE, K-AF, J-AG, I-AH, H-AI, F-AJ, E-AL, D-AM.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 2	-70.5	-70.5	0.02 (1)	10.00	
B- C	0 / 2	-70.5	-70.5	0.03 (1)	10.00	
C- D	0 / 0	-70.5	-70.5	0.03 (1)	10.00	
D- E	0 / 2	-70.5	-70.5	0.03 (1)	10.00	
E- F	0 / 6	-70.5	-70.5	0.04 (1)	10.00	
F- G	-13 / 0	-70.5	-70.5	0.04 (1)	6.25	
G- H	0 / 6	-78.0	-78.0	0.04 (1)	10.00	
H- I	0 / 5	-78.0	-78.0	0.04 (1)	10.00	
I-AQ	0 / 5	-78.0	-78.0	0.04 (1)	10.00	
AQ- J	0 / 5	-78.0	-78.0	0.04 (1)	10.00	
J- K	0 / 5	-78.0	-78.0	0.04 (1)	10.00	
K- L	0 / 5	-78.0	-78.0	0.04 (1)	10.00	
L- M	0 / 4	-70.5	-70.5	0.03 (1)	10.00	
M- N	0 / 3	-70.5	-70.5	0.03 (1)	10.00	
N- O	0 / 0	-70.5	-70.5	0.03 (1)	10.00	
O- P	0 / 0	-70.5	-70.5	0.03 (1)	10.00	
P- Q	0 / 0	-70.5	-70.5	0.03 (1)	10.00	
Q- R	-3 / 0	-70.5	-70.5	0.03 (1)	10.00	
R- S	-6 / 0	-70.5	-70.5	0.03 (1)	10.00	
S- T	-8 / 0	-70.5	-70.5	0.03 (1)	10.00	
T- U	-11 / 0	-70.5	-70.5	0.03 (1)	6.25	
AP- A	-30 / 0	0.0	0.0	0.01 (1)	7.81	
V- U	-55 / 0	0.0	0.0	0.01 (1)	7.81	
AP-AO	0 / 2	-17.5	-17.5	0.01 (4)	10.00	
AO-AN	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
AN-AM	-1 / 0	-17.5	-17.5	0.01 (4)	10.00	
AM-AL	-2 / 0	-17.5	-17.5	0.01 (4)	10.00	
AL-AK	-4 / 0	-17.5	-17.5	0.01 (4)	10.00	
AK-AJ	-4 / 0	-17.5	-17.5	0.01 (4)	10.00	
AJ-AI	-5 / 0	-17.5	-17.5	0.01 (4)	10.00	
AI-AH	-5 / 0	-17.5	-17.5	0.01 (4)	10.00	
AH-AG	-5 / 0	-17.5	-17.5	0.01 (4)	10.00	
AG-AF	-5 / 0	-17.5	-17.5	0.01 (4)	10.00	
AF-AE	-5 / 0	-17.5	-17.5	0.01 (4)	10.00	
AE-AD	-3 / 0	-17.5	-17.5	0.01 (4)	10.00	
AD-AC	-2 / 0	-17.5	-17.5	0.01 (4)	10.00	
AC-AB	-2 / 0	-17.5	-17.5	0.01 (4)	10.00	
AB-AA	-1 / 0	-17.5	-17.5	0.01 (4)	10.00	
AA- Z	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
Z- Y	0 / 2	-17.5	-17.5	0.01 (4)	10.00	
Y- X	0 / 4	-17.5	-17.5	0.01 (4)	10.00	
X- W	0 / 8	-17.5	-17.5	0.01 (4)	10.00	
W- V	0 / 12	-17.5	-17.5	0.01 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- T	-124 / 0	0.02 (1)
X- S	-142 / 0	0.04 (1)
Y- R	-139 / 0	0.06 (1)
Z- Q	-140 / 0	0.11 (1)
AA- P	-141 / 0	0.18 (1)
AB- N	-138 / 0	0.08 (1)
AD- M	-140 / 0	0.06 (1)
AE- L	-153 / 0	0.12 (1)
AF- K	-162 / 0	0.13 (1)
AG- J	-155 / 0	0.12 (1)
AH- I	-156 / 0	0.12 (1)
AI- H	-169 / 0	0.12 (1)
AJ- F	-148 / 0	0.10 (1)
AL- E	-137 / 0	0.09 (1)
AM- D	-140 / 0	0.07 (1)
AN- C	-146 / 0	0.13 (1)
AO- B	-109 / 0	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (K-L:1) , BC=0.01 (AN-AO:4) , WB=0.18 (P-AA:1) , SSI=0.06 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (G) (INPUT = 0.90)
JSI METAL= 0.04 (L) (INPUT = 1.00)

A-5073735

253449	H08	12	1	TRUSS DESC.	DWG NO.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 09:22:02 2015 Page 1
ID:y40yHYHwfsopi7ACUSKwAKzNoB2-OS9mCOr38Z3?gg?L6BBc?i63GJ0Sk6uttJM1NKyxYO3

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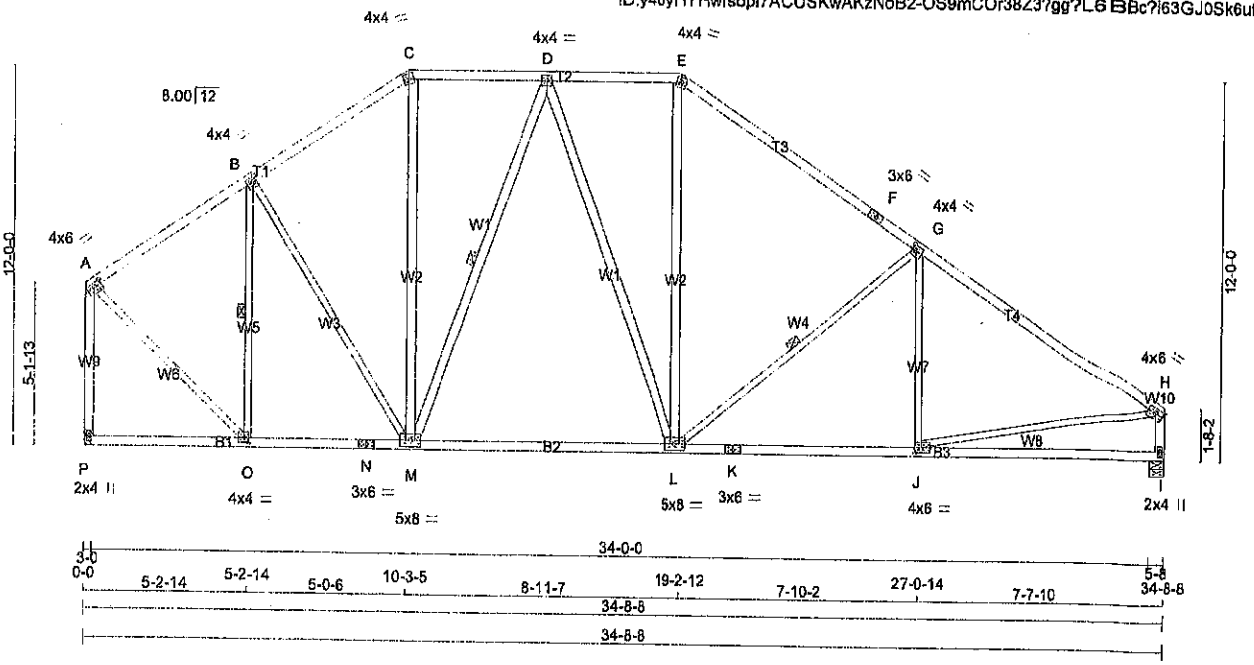

TOTAL WEIGHT = 12 X 185 = 2219 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - A	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
M - D	2x4	DRY	No.2	SPF
D - L	2x4	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-t	MT20	4.0	6.0	1.75	Edge
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.50
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	1.75	1.75
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1665	0	1665	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-11	
I	1555	0	1555	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1106	728 / 0	0 / 0	0 / 0	0 / 0	378 / 0	0 / 0
I	1098	728 / 0	0 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF B-O, D-M, G-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	-1024 / 0	-70.5	-70.5	0.25 (1)	5.90
B-C	-1212 / 0	-70.5	-70.5	0.26 (1)	5.53
C-D	-991 / 0	-78.0	-78.0	0.21 (1)	6.03
D-E	-1167 / 0	-78.0	-78.0	0.21 (1)	5.67
E-F	-1434 / 0	-70.5	-70.5	0.68 (1)	4.60
F-G	-1434 / 0	-70.5	-70.5	0.68 (1)	4.60
G-H	-1812 / 0	-70.5	-70.5	0.72 (1)	4.17
P-A	-1526 / 0	0.0	0.0	0.74 (1)	6.65
I-H	-1500 / 0	0.0	0.0	0.16 (1)	6.70
P-O	0 / 0	-17.5	-17.5	0.10 (4)	10.00
O-N	0 / 871	-17.5	-17.5	0.27 (4)	10.00
N-M	0 / 871	-17.5	-17.5	0.27 (4)	10.00
M-L	0 / 1158	-17.5	-17.5	0.34 (4)	10.00
L-K	0 / 1539	-17.5	-17.5	0.39 (1)	10.00
K-J	0 / 1539	-17.5	-17.5	0.39 (1)	10.00
J-I	0 / 0	-17.5	-17.5	0.24 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-B	-714 / 0	0.34 (1)
B-M	0 / 218	0.05 (1)
M-C	0 / 391	0.06 (1)
M-D	-479 / 0	0.45 (1)
D-L	0 / 39	0.01 (4)
L-E	0 / 429	0.07 (1)
L-G	-485 / 0	0.37 (1)
J-G	-126 / 77	0.10 (1)
A-O	0 / 1185	0.27 (1)
J-H	0 / 1563	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.74 (A-P:1) , BC=0.39 (J-L:1) , WB=0.45 (D-M:1) , SSI=0.22 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.49 (H) (INPUT = 1.00)

A-N0733734

253449	H07G	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

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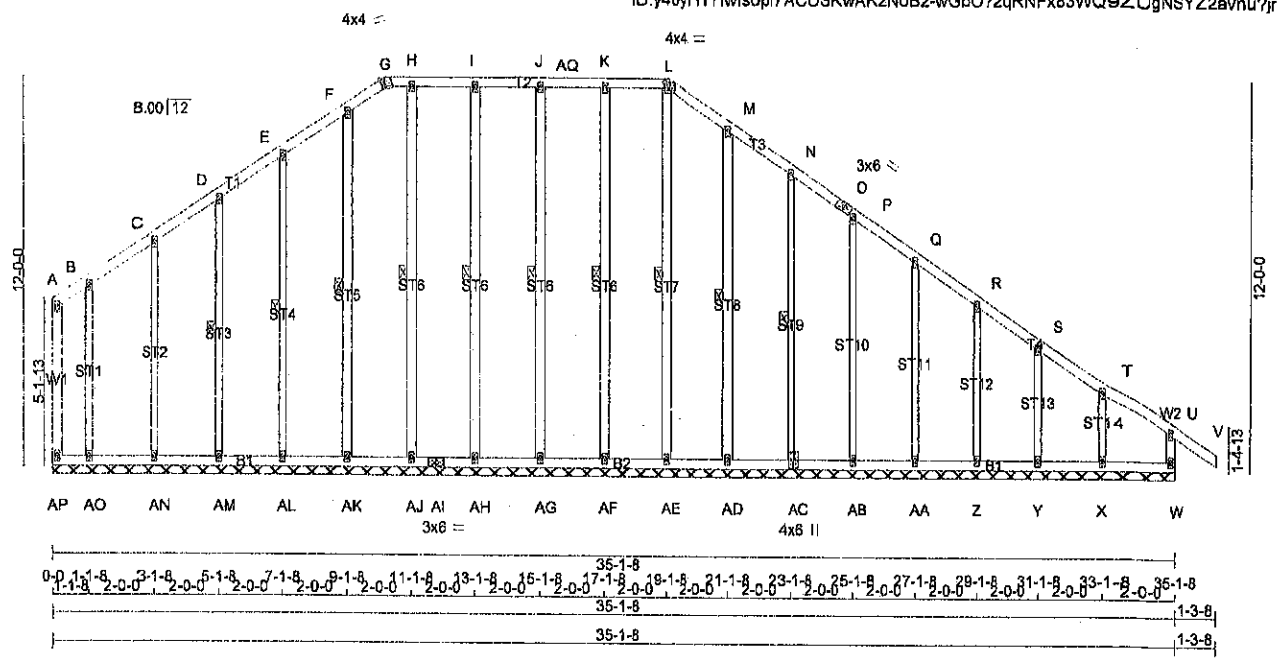

TOTAL WEIGHT = 234 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF
O - V	2x4	DRY	No.2	SPF
AP- A	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
AP- AI	2x4	DRY	No.2	SPF
AI - AC	2x4	DRY	No.2	SPF
AC- W	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
AD- M	2x4	DRY	No.2	SPF
AE- L	2x4	DRY	No.2	SPF
AF- K	2x4	DRY	No.2	SPF
AG- J	2x4	DRY	No.2	SPF
AH- I	2x4	DRY	No.2	SPF
AJ- H	2x4	DRY	No.2	SPF
AK- F	2x4	DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2	SPF
ST7	2x4	DRY	No.2	SPF
ST8	2x4	DRY	No.2	SPF
ST9	2x4	DRY	No.2	SPF
ST10	2x4	DRY	No.2	SPF
ST11	2x4	DRY	No.2	SPF
ST12	2x4	DRY	No.2	SPF
ST13	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, D, E, F, H, I, J, K, M, N, P, Q, R, S, T						
B	TMW+w	MT20	2.0	4.0		
G	TT-m	MT20	4.0	4.0	Edge	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF N-AC, M-AD, L-AE, K-AF, J-AG, I-AH, H-AJ, F-AK, E-AL, D-AM.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 5	-70.5	-70.5	0.02 (1)	10.00	
B- C	0 / 3	-70.5	-70.5	0.03 (1)	10.00	
C- D	0 / 2	-70.5	-70.5	0.03 (1)	10.00	
D- E	0 / 4	-70.5	-70.5	0.03 (1)	10.00	
E- F	0 / 7	-70.5	-70.5	0.04 (1)	10.00	
F- G	-11 / 0	-70.5	-70.5	0.04 (1)	6.25	
G- H	0 / 7	-78.0	-78.0	0.04 (1)	10.00	
H- I	0 / 6	-78.0	-78.0	0.04 (1)	10.00	
I-AQ	0 / 6	-78.0	-78.0	0.04 (1)	10.00	
AQ- J	0 / 6	-78.0	-78.0	0.04 (1)	10.00	
J- K	0 / 6	-78.0	-78.0	0.04 (1)	10.00	
K- L	0 / 6	-78.0	-78.0	0.04 (1)	10.00	
L- M	0 / 6	-70.5	-70.5	0.03 (1)	10.00	
M- N	0 / 5	-70.5	-70.5	0.03 (1)	10.00	
N- O	0 / 4	-70.5	-70.5	0.03 (1)	10.00	
O- P	0 / 4	-70.5	-70.5	0.03 (1)	10.00	
P- Q	0 / 2	-70.5	-70.5	0.03 (1)	10.00	
Q- R	0 / 0	-70.5	-70.5	0.03 (1)	10.00	
R- S	-3 / 0	-70.5	-70.5	0.03 (1)	10.00	
S- T	-3 / 0	-70.5	-70.5	0.03 (1)	10.00	
T- U	-23 / 0	-70.5	-70.5	0.08 (1)	6.25	
U- V	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
AP- A	-27 / 0	0.0	0.0	0.01 (1)	7.81	
W- U	-193 / 0	0.0	0.0	0.03 (1)	7.81	
AP-AO	0 / 1	-17.5	-17.5	0.01 (4)	10.00	
AO-AN	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
AN-AM	-2 / 0	-17.5	-17.5	0.01 (4)	10.00	
AM-AL	-4 / 0	-17.5	-17.5	0.01 (4)	10.00	
AL-AK	-5 / 0	-17.5	-17.5	0.01 (4)	10.00	
AK-AJ	-6 / 0	-17.5	-17.5	0.01 (4)	10.00	
AJ-AI	-6 / 0	-17.5	-17.5	0.01 (4)	10.00	
AI-AH	-6 / 0	-17.5	-17.5	0.01 (4)	10.00	
AH-AG	-6 / 0	-17.5	-17.5	0.01 (4)	10.00	
AG-AF	-6 / 0	-17.5	-17.5	0.01 (4)	10.00	
AF-AE	-6 / 0	-17.5	-17.5	0.01 (4)	10.00	
AE-AD	-5 / 0	-17.5	-17.5	0.01 (4)	10.00	
AD-AC	-3 / 0	-17.5	-17.5	0.01 (4)	10.00	
AC-AB	-3 / 0	-17.5	-17.5	0.01 (4)	10.00	
AB-AA	-2 / 0	-17.5	-17.5	0.01 (4)	10.00	
AA- Z	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
Z- Y	0 / 2	-17.5	-17.5	0.01 (4)	10.00	
Y- X	0 / 5	-17.5	-17.5	0.02 (4)	10.00	
X- W	0 / 9	-17.5	-17.5	0.03 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- T	-107 / 0	0.02 (1)
Y- S	-146 / 0	0.04 (1)
Z- R	-138 / 0	0.06 (1)
AA- Q	-140 / 0	0.11 (1)
AB- P	-141 / 0	0.18 (1)
AC- N	-139 / 0	0.06 (1)
AD- M	-141 / 0	0.08 (1)
AE- L	-153 / 0	0.12 (1)
AF- K	-162 / 0	0.13 (1)
AG- J	-155 / 0	0.12 (1)
AH- I	-156 / 0	0.12 (1)
AJ- H	-159 / 0	0.13 (1)
AK- F	-148 / 0	0.10 (1)
AL- E	-137 / 0	0.09 (1)
AM- D	-140 / 0	0.07 (1)
AN- C	-145 / 0	0.13 (1)
AO- B	-112 / 0	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09 (U-V;1) , BC=0.03 (W-X;1) , WB=0.18 (P-AB:1) , SSI=0.07 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (G) (INPUT = 0.90)
JSI METAL= 0.05 (AC) (INPUT = 1.00)

A-L0073733

253449	H07	2	1	TRUSS DESC.

Alpa Roof Truss, Maple

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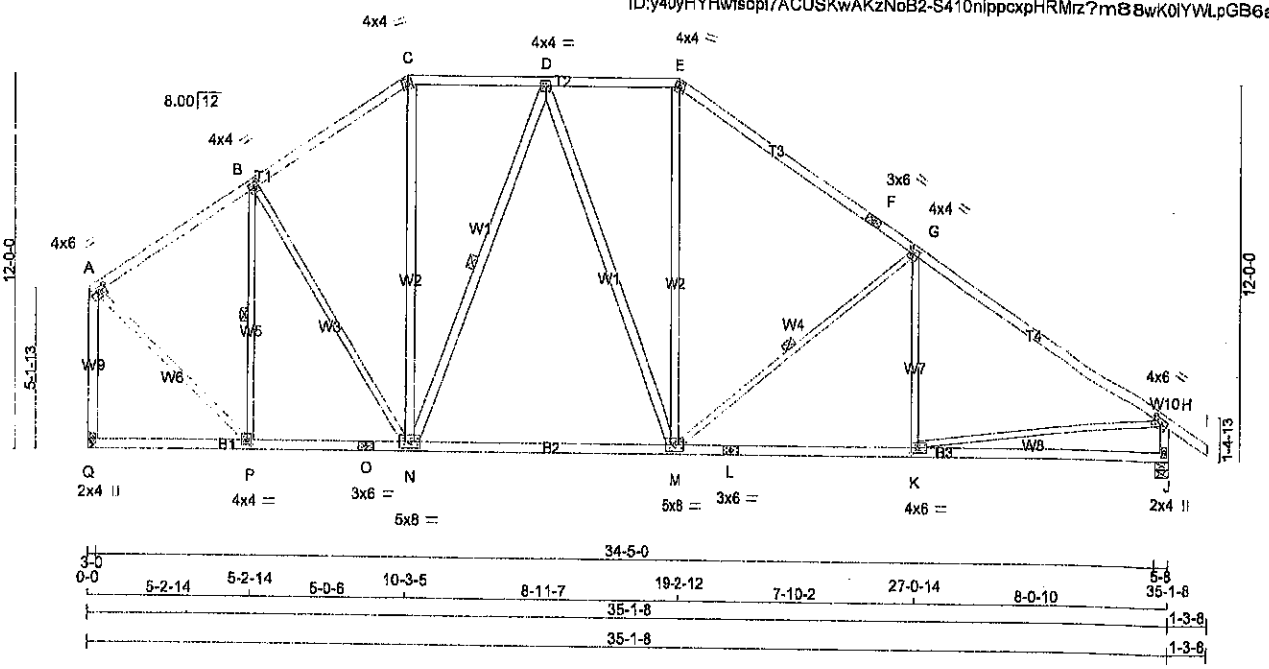
Scale = 1:74.6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
N - D	2x4	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	6.0	8.0		
N	BMWWW-t	MT20	6.0	8.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0	1.75	1.75
Q	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1584	0	1584	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-11	
J	1670	0	1670	0	0	5-8	1-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1119	737 / 0	0 / 0	0 / 0	0 / 0	382 / 0	0 / 0
J	1177	795 / 0	0 / 0	0 / 0	0 / 0	382 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF B-P, D-N, G-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	-1038 / 0	-70.5	-70.5	0.26 (1)		5.87
B-C	-1232 / 0	-70.5	-70.5	0.26 (1)		5.49
C-D	-1008 / 0	-78.0	-78.0	0.21 (1)		5.99
D-E	-1197 / 0	-78.0	-78.0	0.21 (1)		5.52
E-F	-1472 / 0	-70.5	-70.5	0.71 (1)		4.49
F-G	-1472 / 0	-70.5	-70.5	0.71 (1)		4.49
G-H	-1912 / 0	-70.5	-70.5	0.82 (1)		3.97
H-I	0 / 27	-70.5	-70.5	0.09 (1)		10.00
Q-A	-1544 / 0	0.0	0.0	0.75 (1)		6.62
J-H	-1612 / 0	0.0	0.0	0.17 (1)		6.52
Q-P	0 / 0	-17.5	-17.5	0.10 (4)		10.00
P-O	0 / 863	-17.5	-17.5	0.27 (4)		10.00
O-N	0 / 863	-17.5	-17.5	0.27 (4)		10.00
N-M	0 / 1181	-17.5	-17.5	0.34 (4)		10.00
M-L	0 / 1622	-17.5	-17.5	0.40 (1)		10.00
L-K	0 / 1622	-17.5	-17.5	0.40 (1)		10.00
K-J	0 / 0	-17.5	-17.5	0.28 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-B	-725 / 0	0.35 (1)
B-N	0 / 227	0.05 (1)
N-C	0 / 402	0.06 (1)
N-D	-499 / 0	0.46 (1)
D-M	0 / 46	0.01 (4)
M-E	0 / 453	0.07 (1)
M-G	-554 / 0	0.43 (1)
K-G	-65 / 101	0.05 (1)
A-P	0 / 1201	0.27 (1)
K-H	0 / 1637	0.37 (1)

TOTAL WEIGHT = 2 X 188 = 376 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.82 (G-H:1) , BC=0.40 (K-M:1) , WB=0.46 (D-N:1) , SSI=0.23 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.53 (H) (INPUT = 1.00)

A-5073732

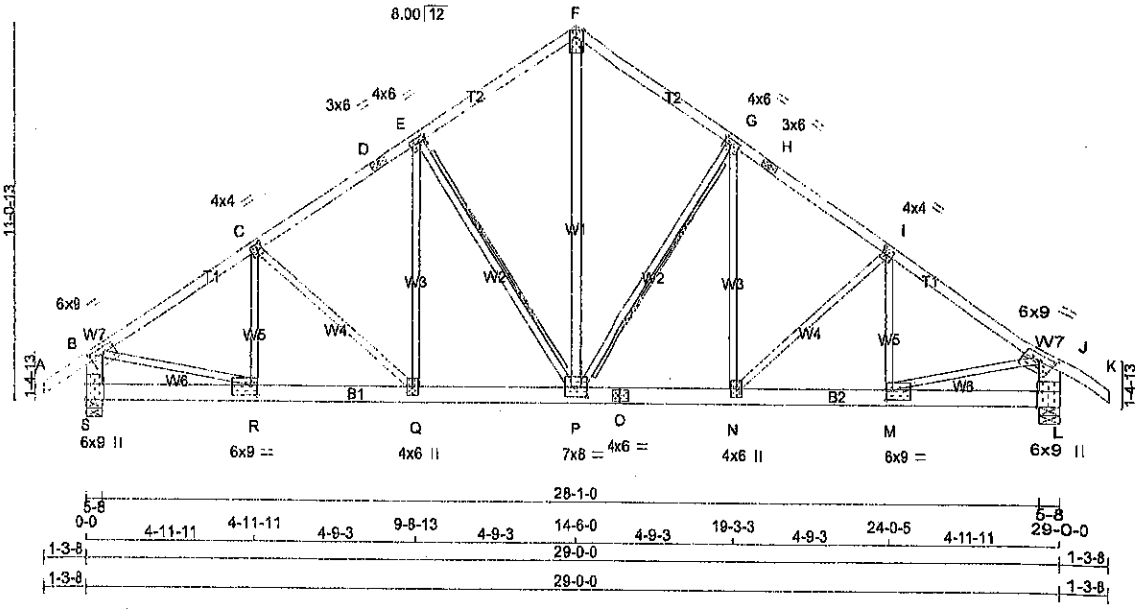
253449	H06A	1	3	TRUSS DESC.

Alpa Roof Truss, Maple

ID:y40yHYHwfsopi7ACUSKwAKzNoB2-_(TeaMpArehRpCGmR3dvN7UbT6vYXe9RBL8Nm?yxYOE

5x8 II

Scale = 1:68.4



TOTAL WEIGHT = 3 X 160 = 479 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- H	1	12	TOP
H- K	1	12	TOP
S- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- O	2	5	SIDE(485.6)
O- L	2	5	SIDE(485.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	11933	0	11933	0	0	5-8	5-8
L	11933	0	11933	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	8405	5705 / 0	0 / 0	0 / 0	0 / 0	2700 / 0	0 / 0
L	8405	5705 / 0	0 / 0	0 / 0	0 / 0	2700 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x6 DRY SPF No.2 T-BRACE REQUIRED AT G-P, E-P
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.03 (1)	10.00
B- C	-14007 / 0	-70.5	-70.5	0.52 (1)	3.05
C- D	-12461 / 0	-70.5	-70.5	0.36 (1)	3.31
D- E	-12461 / 0	-70.5	-70.5	0.36 (1)	3.31
E- F	-9876 / 0	-70.5	-70.5	0.27 (1)	3.74
F- G	-9876 / 0	-70.5	-70.5	0.27 (1)	3.74
G- H	-12461 / 0	-70.5	-70.5	0.36 (1)	3.31
H- I	-12461 / 0	-70.5	-70.5	0.36 (1)	3.31
I- J	-14007 / 0	-70.5	-70.5	0.52 (1)	3.05
J- K	0 / 27	-70.5	-70.5	0.03 (1)	10.00
S- B	-10369 / 0	0.0	0.0	0.23 (1)	5.73
L- J	-10369 / 0	0.0	0.0	0.23 (1)	5.73
S- R	0 / 0	-745.8	-745.8	0.37 (1)	10.00
R- Q	0 / 11660	-745.8	-745.8	0.78 (1)	10.00
Q- P	0 / 10372	-745.8	-745.8	0.79 (1)	10.00
P- O	0 / 10372	-745.8	-745.8	0.79 (1)	10.00
O- N	0 / 10372	-745.8	-745.8	0.79 (1)	10.00
N- M	0 / 11660	-745.8	-745.8	0.78 (1)	10.00
M- L	0 / 0	-745.8	-745.8	0.37 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	0 / 10652	0.67 (1)
P- G	-4034 / 0	0.65 (1)
N- G	0 / 4481	0.34 (1)
N- I	-1737 / 0	0.42 (1)
M- I	0 / 1677	0.13 (1)
E- P	-4034 / 0	0.65 (1)
Q- E	0 / 4481	0.34 (1)
C- Q	-1737 / 0	0.42 (1)
R- C	0 / 1677	0.13 (1)
B- R	0 / 11892	0.89 (1)
M- J	0 / 11892	0.89 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 35-1-8
END DISTANCE = 29-0-0
END SPAN CARRIED = 35-1-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL. (DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.62 (I-J:1) , BC=0.79 (N-P:1) , WB=0.89 (B-R:1) , SSI=0.43 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.80 (B) (INPUT = 1.00)

A-N073731

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.00	4.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0	1.75	2.25
F	TTW+p	MT20	5.0	8.0	Edge	
G	TMWW-t	MT20	4.0	6.0	1.75	2.25
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-t	MT20	6.0	9.0	2.00	4.25
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWW-t	MT20	6.0	9.0	3.50	2.25
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	4.0	6.0		
P	BMWWW-t	MT20	7.0	8.0	3.25	4.00
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	6.0	9.0	3.50	2.25
S	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11. and no less than 2x4 for Part 4 design.

LICENSED PROFESSIONAL ENGINEER
07.16.2015
R. TURENNE
100153040
PROVINCE OF ONTARIO

A+1507373(2)

253449	H06	1	1	TRUSS DESC.

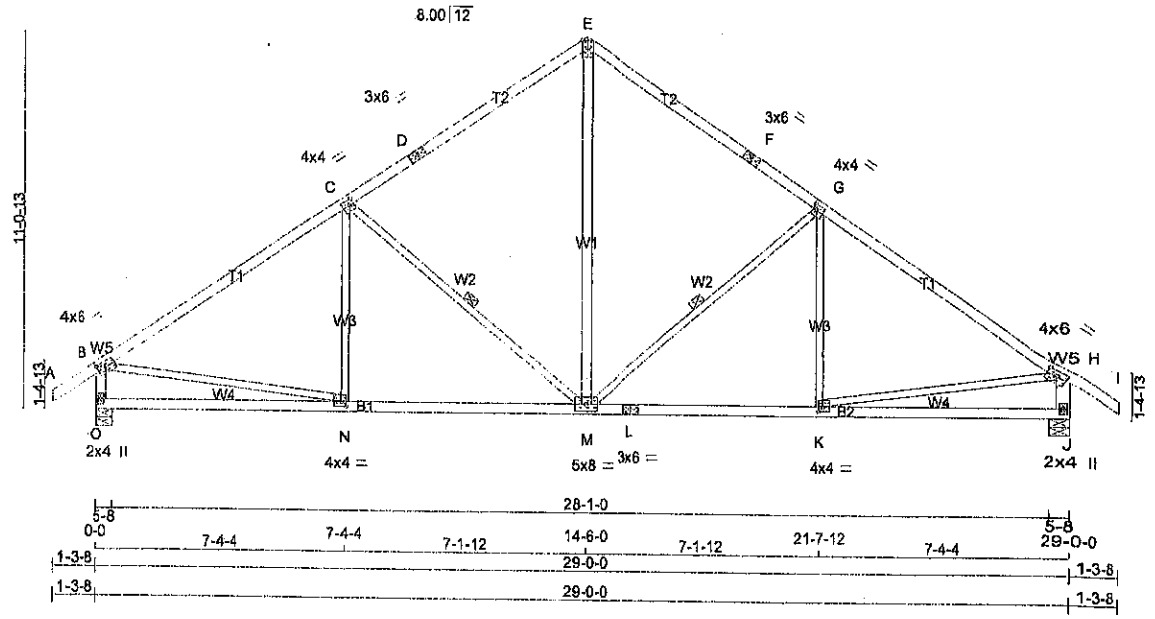
Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 09:21:58 2015 Page 1

ID:y40yHYHwfsopi7ACUSKwAKzNoB2-WhvFN0oY4KZaC2hatM6gqvxPbigZoK?HzhOqDZyxYO7

4x6 II

Scale = 1:68.4



TOTAL WEIGHT = 126 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.75	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	4.0	4.0	1.75	1.50
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1372	0	1372	0	0	5-8	1-8
J	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-M, C-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	M- E	0 / 779	0.13 (1)
B- C	-1481 / 0	-70.5	-70.5	0.59 (1)	4.70	M- G	-514 / 0	0.31 (1)
C- D	-1076 / 0	-70.5	-70.5	0.53 (1)	5.36	K- G	-43 / 103	0.04 (4)
D- E	-1076 / 0	-70.5	-70.5	0.53 (1)	5.36	C- M	-514 / 0	0.31 (1)
E- F	-1076 / 0	-70.5	-70.5	0.53 (1)	5.36	N- C	-43 / 103	0.04 (4)
F- G	-1076 / 0	-70.5	-70.5	0.53 (1)	5.36	B- N	0 / 1274	0.29 (1)
G- H	-1481 / 0	-70.5	-70.5	0.59 (1)	4.70	K- H	0 / 1274	0.29 (1)
H- I	0 / 27	-70.5	-70.5	0.09 (1)	10.00			
O- B	-1319 / 0	0.0	0.0	0.14 (1)	7.04			
J- H	-1319 / 0	0.0	0.0	0.14 (1)	7.04			
O- N	0 / 0	-17.5	-17.5	0.22 (4)	10.00			
N- M	0 / 1260	-17.5	-17.5	0.32 (4)	10.00			
M- L	0 / 1260	-17.5	-17.5	0.32 (4)	10.00			
L- K	0 / 1260	-17.5	-17.5	0.32 (4)	10.00			
K- J	0 / 0	-17.5	-17.5	0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.59 (G-H:1) , BC=0.32 (M-N:4) , WB=0.31 (G-M:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.47 (N) (INPUT = 1.00)

A-1073730

253448	H06T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

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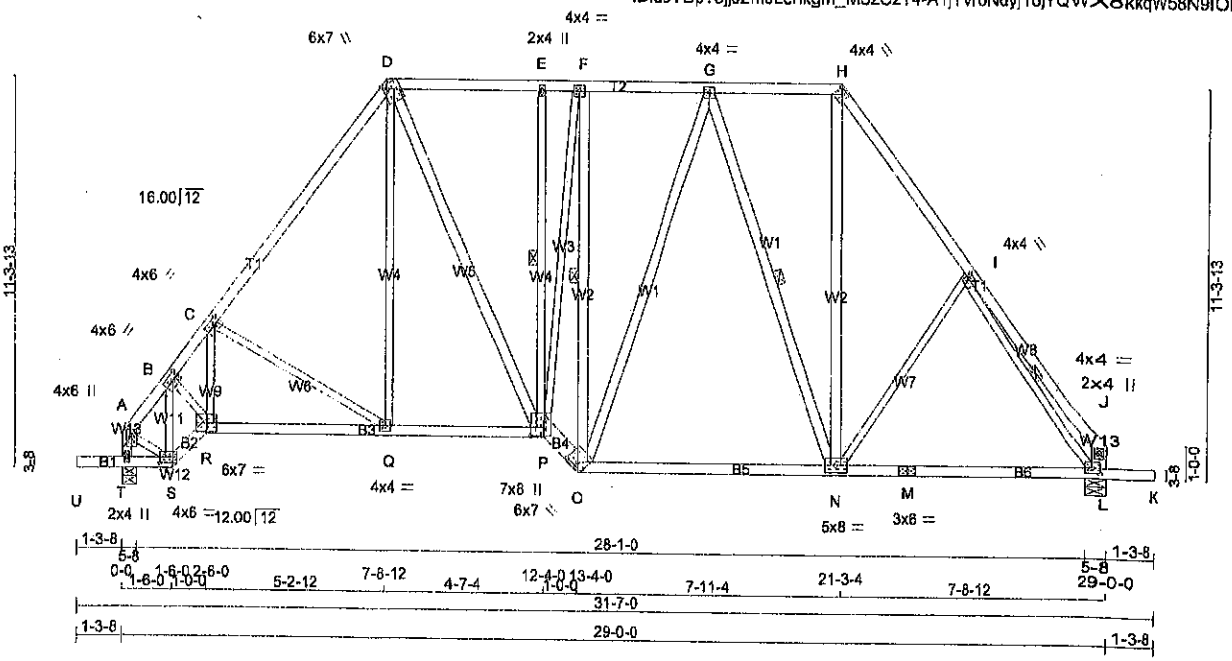


TOTAL WEIGHT = 194 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
G - N	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTW-h	MT20	4.0	4.0	Edge	
I	TMWW-t	MT20	4.0	4.0	1.75	1.26
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.75	2.00
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	6.0	8.0		
O	BBWW-h	MT20	6.0	7.0	1.50	3.75
P	BBWWW+p	MT20	7.0	8.0	3.00	4.50
Q	BMWW-t	MT20	4.0	4.0		
R	BBWW-t	MT20	6.0	7.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-P, F-O, G-N, H-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1117 / 0	-70.5	-70.5	0.08 (1)	5.92
B- C	-1554 / 0	-70.5	-70.5	0.22 (1)	5.07
C- D	-1237 / 0	-70.5	-70.5	0.28 (1)	5.51
D- E	-916 / 0	-70.5	-70.5	0.19 (1)	6.23
E- F	-914 / 0	-70.5	-70.5	0.10 (1)	6.25
F- G	-844 / 0	-70.5	-70.5	0.13 (1)	6.25
G- H	-674 / 0	-70.5	-70.5	0.12 (1)	6.25
H- I	-1140 / 0	-70.5	-70.5	0.15 (1)	5.66
I- J	0 / 25	-70.5	-70.5	0.16 (1)	10.00
T- A	-1216 / 0	0.0	0.0	0.12 (1)	7.26
L- J	-103 / 0	0.0	0.0	0.01 (1)	7.81
U- T	0 / 0	-88.0	-88.0	0.10 (1)	10.00
T- S	0 / 0	-17.5	-17.5	0.06 (1)	10.00
S- R	0 / 869	-17.5	-17.5	0.14 (1)	10.00
R- Q	0 / 1022	-17.5	-17.5	0.23 (1)	10.00
Q- P	0 / 724	-17.5	-17.5	0.18 (1)	10.00
P- O	0 / 1178	-17.5	-17.5	0.14 (1)	10.00
O- N	0 / 812	-17.5	-17.5	0.37 (4)	10.00
N- M	0 / 748	-17.5	-17.5	0.37 (4)	10.00
M- L	0 / 748	-17.5	-17.5	0.37 (4)	10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- B	-901 / 0	0.15 (1)
B- R	0 / 662	0.15 (1)
R- C	0 / 123	0.03 (4)
C- Q	-349 / 0	0.23 (1)
Q- D	0 / 264	0.06 (1)
D- P	0 / 454	0.07 (1)
P- E	-226 / 0	0.17 (1)
P- F	0 / 704	0.16 (1)
O- F	-863 / 0	0.57 (1)
O- G	0 / 95	0.02 (1)
G- N	-419 / 0	0.32 (1)
N- H	0 / 668	0.11 (1)
N- I	-137 / 0	0.13 (1)
A- S	0 / 702	0.16 (1)
I- L	-1343 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28 (C-D:1) , BC=0.37 (N-O:4) , WB=0.67 (F-O:1) , SSI=0.14 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

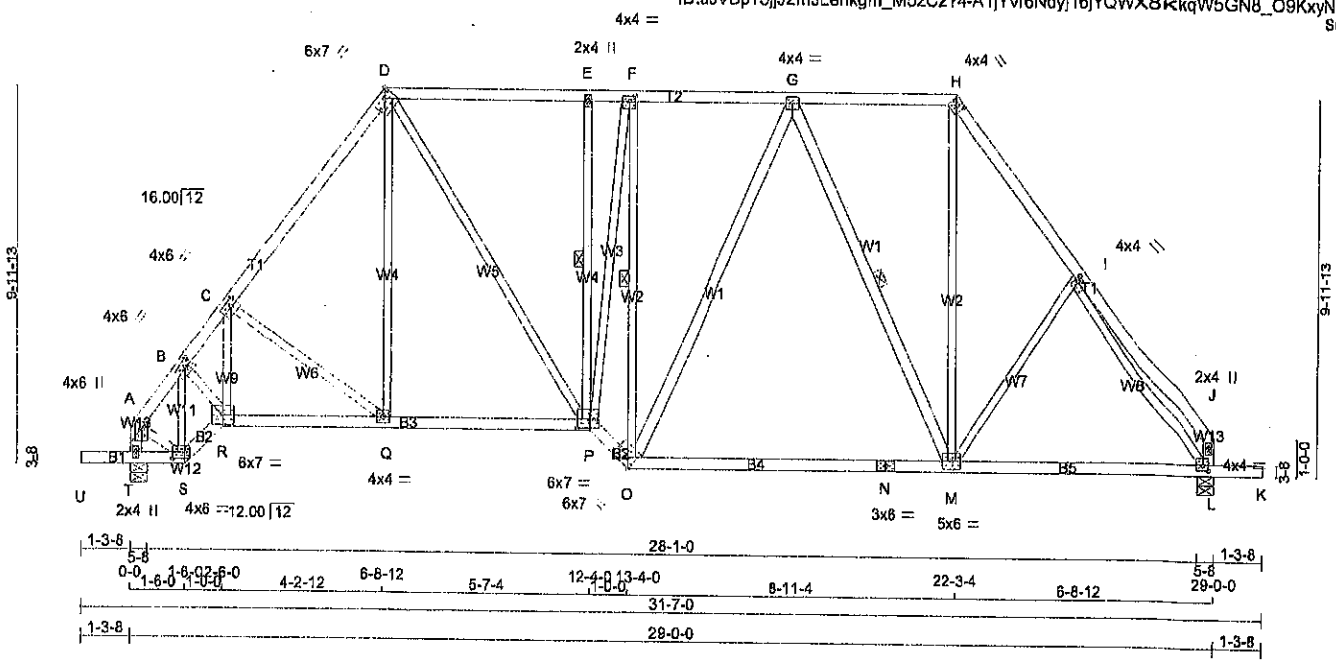
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

A-H073729

253448	H05T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple



TOTAL WEIGHT = 171 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	Edge	2.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0	1.75	1.75
G	TMWW-t	MT20	4.0	4.0		
H	TTW-h	MT20	4.0	4.0	1.75	2.00
I	TMWW-t	MT20	4.0	4.0	1.50	1.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.75	1.75
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BBWW-h	MT20	6.0	7.0	Edge	2.50
P	BBWWW-p	MT20	6.0	7.0	2.75	4.25
Q	BMWW-t	MT20	4.0	4.0		
R	BBWW-t	MT20	6.0	7.0		
S	BBWW-t	MT20	4.0	6.0	2.00	4.50
T	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	980	684 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	652 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-P, F-O, G-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1119 / 0	-70.5	-70.5	0.07 (1)	5.93
B- C	-1575 / 0	-70.5	-70.5	0.14 (1)	5.13
C- D	-1294 / 0	-70.5	-70.5	0.17 (1)	5.52
D- E	-1060 / 0	-70.5	-70.5	0.27 (1)	5.79
E- F	-1056 / 0	-70.5	-70.5	0.18 (1)	5.91
F- G	-956 / 0	-70.5	-70.5	0.17 (1)	6.16
G- H	-701 / 0	-70.5	-70.5	0.16 (1)	6.25
H- I	-1183 / 0	-70.5	-70.5	0.11 (1)	5.77
I- J	0 / 20	-70.5	-70.5	0.11 (1)	10.00
T- A	-1216 / 0	0.0	0.0	0.12 (1)	7.26
L- J	-92 / 0	0.0	0.0	0.01 (1)	7.81
U- T	0 / 0	-88.0	-88.0	0.10 (1)	10.00
T- S	0 / 0	-17.5	-17.5	0.08 (1)	10.00
S- R	0 / 877	-17.5	-17.5	0.14 (1)	10.00
R- Q	0 / 998	-17.5	-17.5	0.23 (1)	10.00
Q- P	0 / 762	-17.5	-17.5	0.19 (1)	10.00
P- O	0 / 1338	-17.5	-17.5	0.22 (1)	10.00
O- N	0 / 908	-17.5	-17.5	0.39 (4)	10.00
N- M	0 / 908	-17.5	-17.5	0.39 (4)	10.00
M- L	0 / 749	-17.5	-17.5	0.36 (4)	10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- B	-910 / 0	0.15 (1)
B- R	0 / 635	0.14 (1)
R- C	0 / 156	0.04 (1)
C- Q	-297 / 0	0.14 (1)
Q- D	0 / 272	0.06 (1)
D- P	0 / 546	0.12 (1)
P- E	-328 / 0	0.17 (1)
P- F	0 / 875	0.20 (1)
O- F	-977 / 0	0.66 (1)
O- G	0 / 116	0.02 (1)
G- M	-503 / 0	0.31 (1)
M- H	0 / 706	0.16 (1)
M- I	-97 / 5	0.07 (1)
A- S	0 / 709	0.16 (1)
I- L	-1364 / 0	0.86 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.27 (D-E:1) , BC=0.39 (M-O:4) , WB=0.86 (I-L:1) , SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (O) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

A-1507372 8

253448	H04T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

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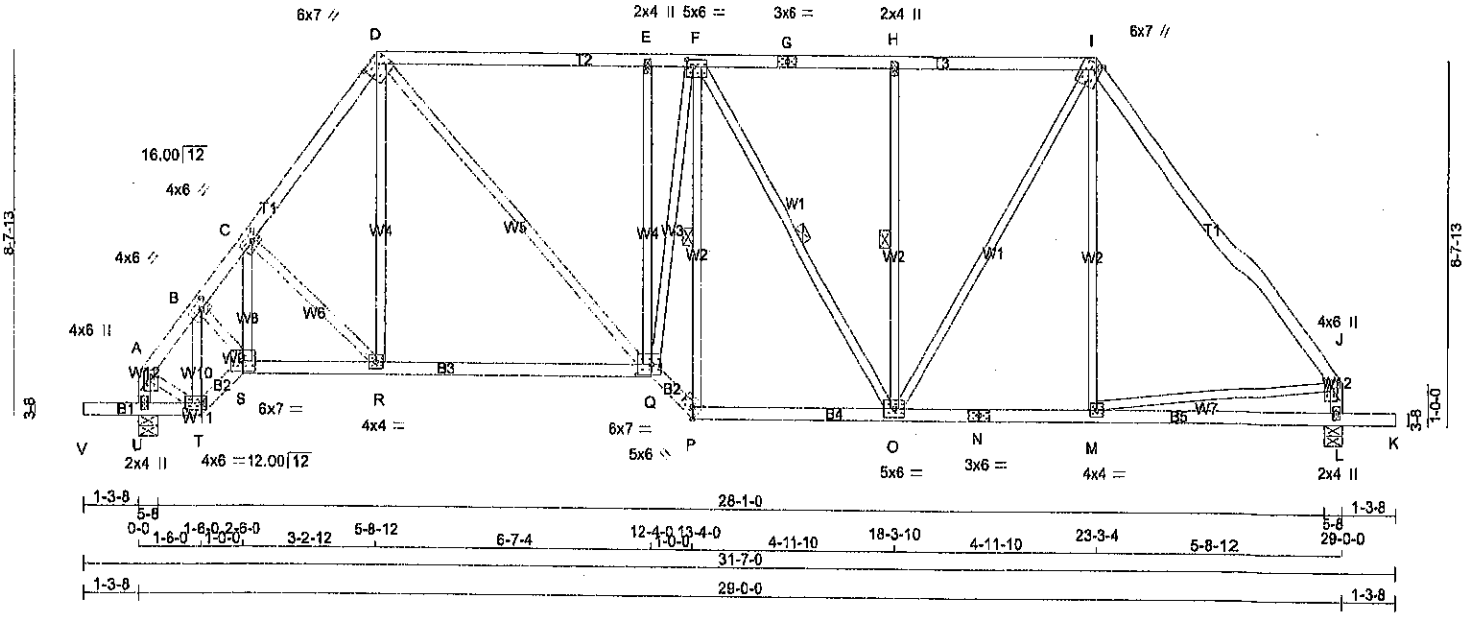
Scale = 1:55.4



TOTAL WEIGHT = 156 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	1.75	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	6.0	2.00	2.00
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	1.50
J	TMVW+p	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BBW-h	MT20	5.0	6.0	Edge	2.50
Q	BBWWW-p	MT20	6.0	7.0	3.00	4.25
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-t	MT20	6.0	7.0		
T	BBWW-t	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-P, F-O, H-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1120 / 0	-70.5	-70.5	0.08 (1)	5.94
B- C	-1591 / 0	-70.5	-70.5	0.10 (1)	5.15
C- D	-1357 / 0	-70.5	-70.5	0.12 (1)	5.47
D- E	-1267 / 0	-70.5	-70.5	0.43 (1)	5.19
E- F	-1261 / 0	-70.5	-70.5	0.37 (1)	5.21
F- G	-1051 / 0	-70.5	-70.5	0.27 (1)	5.79
G- H	-1051 / 0	-70.5	-70.5	0.27 (1)	5.79
H- I	-1051 / 0	-70.5	-70.5	0.27 (1)	5.79
I- J	-1189 / 0	-70.5	-70.5	0.57 (1)	5.26
U- A	-1218 / 0	0.0	0.0	0.12 (1)	7.26
L- J	-1231 / 0	0.0	0.0	0.12 (1)	7.23
V- U	0 / 0	-88.0	-88.0	0.10 (1)	10.00
U- T	0 / 0	-17.5	-17.5	0.10 (1)	10.00
T- S	0 / 884	-17.5	-17.5	0.15 (1)	10.00
S- R	0 / 981	-17.5	-17.5	0.25 (1)	10.00
R- Q	0 / 804	-17.5	-17.5	0.23 (4)	10.00
Q- P	0 / 1554	-17.5	-17.5	0.26 (1)	10.00
P- O	0 / 1112	-17.5	-17.5	0.23 (1)	10.00
O- N	0 / 711	-17.5	-17.5	0.19 (1)	10.00
N- M	0 / 711	-17.5	-17.5	0.19 (1)	10.00
M- L	0 / 0	-17.5	-17.5	0.13 (4)	10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-917 / 0	0.16 (1)
B- S	0 / 599	0.13 (1)
S- C	0 / 174	0.04 (1)
C- R	-251 / 0	0.08 (1)
R- D	0 / 276	0.06 (1)
D- Q	0 / 689	0.16 (1)
Q- E	-429 / 0	0.46 (1)
Q- F	0 / 1088	0.24 (1)
P- F	-1045 / 0	0.50 (1)
F- O	-120 / 0	0.08 (1)
O- H	-386 / 0	0.18 (1)
O- I	0 / 663	0.15 (1)
M- I	0 / 94	0.03 (4)
A- T	0 / 714	0.16 (1)
M- J	0 / 714	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.57 (I-J:1) , BC=0.25 (P-Q:1) , WB=0.50 (F-P:1) , SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.28 (F) (INPUT = 1.00)

A-15073727

253448	H02T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 08:37:50 2015 Page 1
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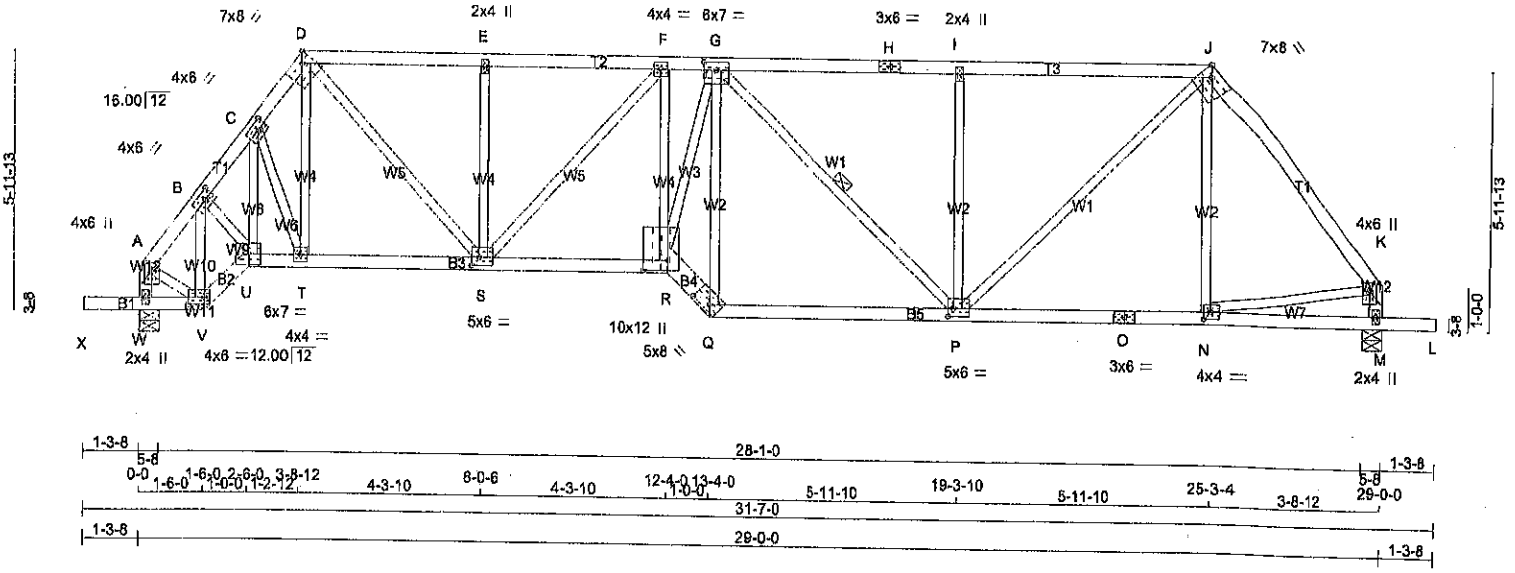

TOTAL WEIGHT = 142 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
W - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x6	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	7.0	8.0	Edge	2.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWWW-t	MT20	6.0	7.0	2.25	3.50
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW-h	MT20	7.0	8.0	Edge	2.75
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	4.0	2.00	1.75
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	6.0	6.0	2.25	1.50
Q	BBW-h	MT20	6.0	8.0	1.75	6.00
R	BBWW+p	MT20	10.0	12.0	2.75	7.00
S	BMWWW-t	MT20	6.0	6.0	2.25	2.00
T	BMWW-t	MT20	4.0	4.0		
U	BBWW-l	MT20	6.0	7.0		
V	BBWW-l	MT20	4.0	6.0	2.00	4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	1391	0	1391	0	0	5-8	1-8
M	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
M	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1122 / 0	-70.5	-70.5	0.04 (1)	5.95
B- C	-1611 / 0	-70.5	-70.5	0.06 (1)	5.18
C- D	-1466 / 0	-70.5	-70.5	0.05 (1)	5.35
D- E	-1602 / 0	-70.5	-70.5	0.24 (1)	4.97
E- F	-1602 / 0	-70.5	-70.5	0.24 (1)	4.97
F- G	-1959 / 0	-70.5	-70.5	0.24 (1)	4.57
G- H	-1497 / 0	-70.5	-70.5	0.43 (1)	4.85
H- I	-1497 / 0	-70.5	-70.5	0.43 (1)	4.85
I- J	-1498 / 0	-70.5	-70.5	0.44 (1)	4.85
J- K	-1242 / 0	-70.5	-70.5	0.21 (1)	5.54
W- A	-1218 / 0	0.0	0.0	0.12 (1)	7.26
M- K	-1250 / 0	0.0	0.0	0.13 (1)	7.19
X- W	0 / 0	-88.0	-88.0	0.10 (1)	10.00
W- V	0 / 0	-17.5	-17.5	0.10 (1)	10.00
V- U	0 / 892	-17.5	-17.5	0.16 (1)	10.00
U- T	0 / 969	-17.5	-17.5	0.19 (1)	10.00
T- S	0 / 885	-17.5	-17.5	0.18 (1)	10.00
S- R	0 / 1952	-17.5	-17.5	0.36 (1)	10.00
R- Q	0 / 2305	-17.5	-17.5	0.27 (1)	10.00
Q- P	0 / 1659	-17.5	-17.5	0.34 (1)	10.00
P- O	0 / 740	-17.5	-17.5	0.21 (4)	10.00
O- N	0 / 740	-17.5	-17.5	0.21 (4)	10.00
N- M	0 / 0	-17.5	-17.5	0.09 (4)	10.00
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- B	-925 / 0	0.16 (1)
B- U	0 / 557	0.13 (1)
U- C	0 / 209	0.05 (1)
C- T	-218 / 0	0.05 (1)
T- D	0 / 251	0.06 (1)
R- F	0 / 337	0.08 (1)
R- G	0 / 1413	0.32 (1)
Q- G	-1553 / 0	0.87 (1)
G- P	-225 / 0	0.10 (1)
P- I	-468 / 0	0.26 (1)
P- J	0 / 1041	0.23 (1)
N- J	-28 / 62	0.02 (4)
A- V	0 / 721	0.16 (1)
N- K	0 / 751	0.17 (1)
D- S	0 / 1055	0.24 (1)
S- E	-345 / 0	0.13 (1)
S- F	-524 / 0	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.44 (I-J:1) , BC=0.35 (R-S:1) , WB=0.87 (G-Q:1) , SSI=0.20 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (G) (INPUT = 0.90)
JSI METAL= 0.32 (R) (INPUT = 1.00)

A-15073725

					DRWG NO.
253448	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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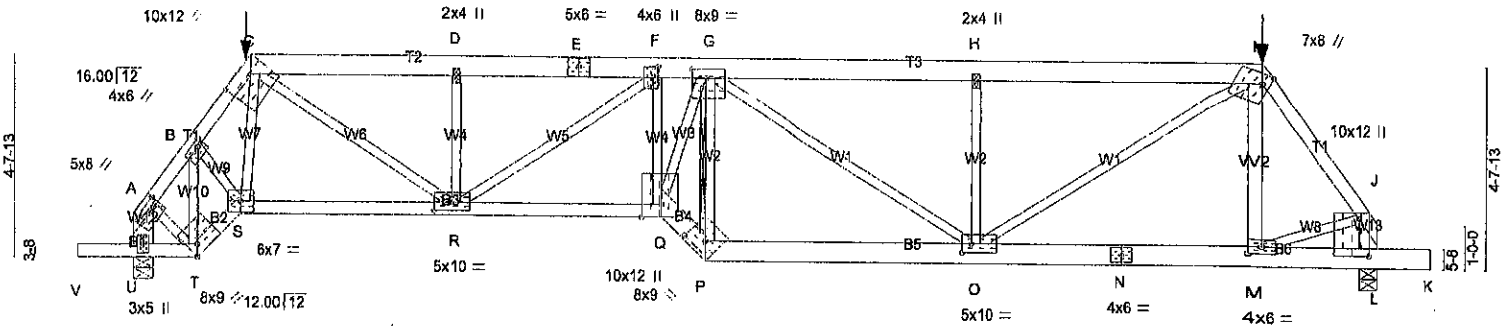

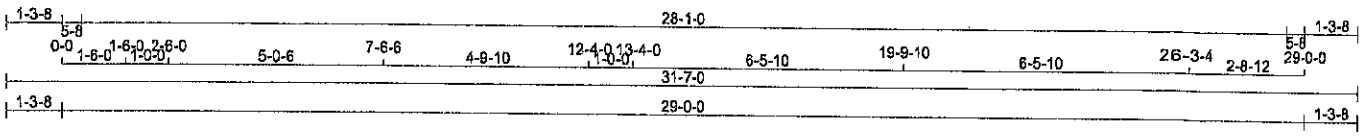

TOTAL WEIGHT = 151 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - I	2x6	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
U - A	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	2100F 1.8E	SPF
Q - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	1.50	1.75
C	TTWW-h	MT20	10.0	12.0	3.00	7.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0	3.00	1.50
G	TMWWW-t	MT20	8.0	9.0	2.50	3.75
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	7.0	8.0	2.75	1.75
J	TMBMVW*+p	MT20	10.0	12.0	10.25	3.00
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BS-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	5.0	10.0	2.50	2.75
P	BBW-h	MT20	8.0	9.0	3.00	5.00
Q	BBWW+p	MT20	10.0	12.0	Edge	5.60
R	BMWWW-t	MT20	5.0	10.0	2.25	4.75
S	BBWW-l	MT20	6.0	7.0		
T	BBWW-h	MT20	8.0	9.0	Edge	2.50
U	BMV1+p	MT20	3.0	5.0		

LICENSED PROFESSIONAL ENGINEER
07/16/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2775	0	2775	0	0	5-8	3-5
L	2589	0	2589	0	0	5-8	2-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1953	1337 / 0	0 / 0	0 / 0	0 / 0	616 / 0	0 / 0
L	1825	1231 / 0	0 / 0	0 / 0	0 / 0	594 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT G-P
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	-2394 / 0	-138.7	-138.7	0.15 (1)	4.28
B-C	-3406 / 0	-138.7	-138.7	0.22 (1)	3.60
C-D	-4498 / 0	-70.5	-70.5	0.25 (1)	3.91
D-E	-4498 / 0	-70.5	-70.5	0.38 (1)	3.76
E-F	-4498 / 0	-70.5	-70.5	0.38 (1)	3.76
F-G	-5610 / 0	-138.7	-138.7	0.44 (1)	3.30
G-H	-3946 / 0	-138.7	-138.7	0.53 (1)	3.78
H-I	-3946 / 0	-138.7	-138.7	0.54 (1)	3.78
I-J	-2697 / 0	-70.5	-70.5	0.21 (1)	4.10
U-A	-2587 / 0	0.0	0.0	0.18 (1)	6.42
L-J	-2474 / 0	0.0	0.0	0.17 (1)	6.63
V-U	0 / 0	-88.0	-88.0	0.11 (1)	10.00
U-T	0 / 0	-34.5	-34.5	0.11 (1)	10.00
T-S	0 / 1901	-34.5	-34.5	0.34 (1)	10.00
S-R	0 / 2004	-102.7	-102.7	0.44 (1)	10.00
R-Q	0 / 5574	-102.7	-102.7	0.73 (1)	10.00
Q-P	0 / 6283	-34.5	-34.5	0.82 (1)	10.00
P-O	0 / 4462	-34.5	-34.5	0.71 (1)	10.00
O-N	0 / 1554	-34.5	-34.5	0.30 (1)	10.00
N-M	0 / 1554	-34.5	-34.5	0.30 (1)	10.00
M-L	0 / 0	-34.5	-34.5	0.11 (4)	10.00
L-K	0 / 0	-88.0	-88.0	0.05 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-B	-1919 / 0	0.35 (1)
B-S	0 / 1168	0.29 (1)
S-C	0 / 648	0.18 (1)
C-R	0 / 3016	0.75 (1)
R-D	-444 / 0	0.10 (1)
R-F	-1312 / 0	0.73 (1)
Q-F	0 / 1080	0.27 (1)
Q-G	0 / 4049	1.00 (1)
P-G	-4475 / 0	0.86 (1)
G-O	-617 / 0	0.71 (1)
O-H	-999 / 0	0.29 (1)
O-I	0 / 2849	0.71 (1)
M-I	-76 / 110	0.04 (4)
A-T	0 / 1534	0.38 (1)
M-J	0 / 1585	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-8-12	-139	-139	---	FRONT	VERT	TOTAL
I	26-3-4	-140	-140	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-6-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 16-8-0 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-8-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 12-4-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. MAIN LOAD TOOLS)

A150722

CONTINUED ON PAGE 2

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.2 lbs FACTORED DOWN AT 26-3-4, AND 138.6 lbs FACTORED DOWN AT 2-8-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Contractor and/or fabricator to verify loadings and dimensions prior to fabrication.

21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 696 (0.50")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 527 (0.03")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 287 (0.06")

CSI: TC=0.54 (H-I:1), BC=0.82 (P-Q:1), WB=1.00 (G-Q:1), SSI=0.38 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.83 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.16.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073724(2)

253447	H05T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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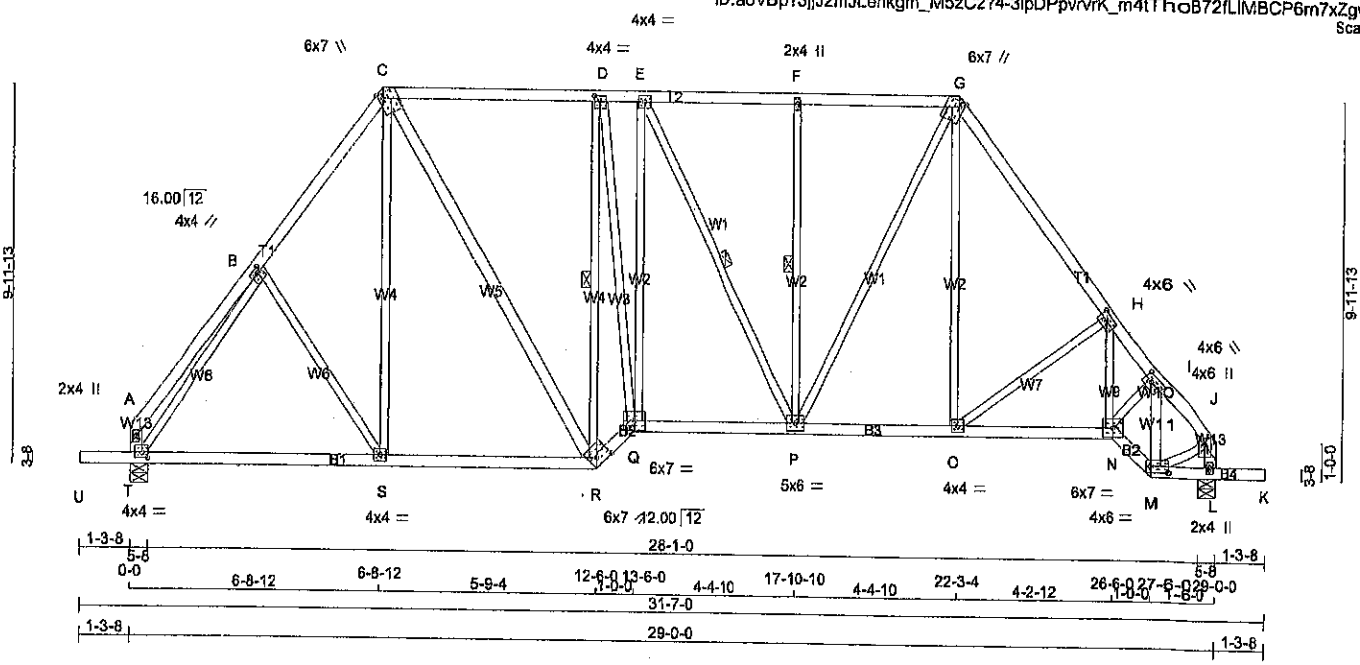


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.50	1.25
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMWW-t	MT20	4.0	4.0	1.75	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMWW-t	MT20	4.0	6.0	2.00	2.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMVW+p	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BBWW-l	MT20	4.0	6.0	2.00	4.50
N	BBWW-l	MT20	6.0	7.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	6.0	6.0		
Q	BBWW-l	MT20	6.0	7.0		
R	BBWW-h	MT20	6.0	7.0	2.25	4.50
S	BMWW-t	MT20	4.0	4.0		
T	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1386	0	1386	0	0	5-8	1-8
L	1391	0	1391	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
L	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-R, E-P, F-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 21	-70.5	-70.5	0.12 (1)	10.00
B- C	-1175 / 0	-70.5	-70.5	0.12 (1)	5.76
C- D	-960 / 0	-70.5	-70.5	0.29 (1)	5.98
D- E	-1062 / 0	-70.5	-70.5	0.28 (1)	5.76
E- F	-1021 / 0	-70.5	-70.5	0.17 (1)	6.01
F- G	-1021 / 0	-70.5	-70.5	0.17 (1)	6.01
G- H	-1290 / 0	-70.5	-70.5	0.17 (1)	5.62
H- I	-1675 / 0	-70.5	-70.5	0.14 (1)	5.13
I- J	-1119 / 0	-70.5	-70.5	0.07 (1)	5.93
T- A	-91 / 0	0.0	0.0	0.01 (1)	7.81
L- J	-1216 / 0	0.0	0.0	0.12 (1)	7.26
U- T	0 / 0	-88.0	-88.0	0.10 (1)	10.00
T- S	0 / 745	-17.5	-17.5	0.26 (4)	10.00
S- R	0 / 690	-17.5	-17.5	0.26 (4)	10.00
R- Q	0 / 1376	-17.5	-17.5	0.22 (1)	10.00
Q- P	0 / 1060	-17.5	-17.5	0.21 (1)	10.00
P- O	0 / 760	-17.5	-17.5	0.16 (1)	10.00
O- N	0 / 998	-17.5	-17.5	0.20 (1)	10.00
N- M	0 / 877	-17.5	-17.5	0.14 (1)	10.00
M- L	0 / 0	-17.5	-17.5	0.08 (1)	10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- S	-99 / 4	0.07 (1)
S- C	0 / 213	0.05 (4)
C- R	0 / 523	0.08 (1)
R- D	-1374 / 0	0.93 (1)
D- Q	0 / 858	0.19 (1)
Q- E	0 / 168	0.04 (1)
E- P	-89 / 0	0.08 (1)
P- F	-361 / 0	0.19 (1)
P- G	0 / 578	0.13 (1)
O- G	0 / 253	0.06 (1)
O- H	-300 / 0	0.14 (1)
N- H	0 / 160	0.04 (1)
N- I	0 / 636	0.14 (1)
M- I	-910 / 0	0.15 (1)
T- B	-1357 / 0	0.85 (1)
M- J	0 / 709	0.16 (1)

TOTAL WEIGHT = 173 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
9.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.29 (C-D:1) , BC=0.26 (R-S:4) , WB=0.93 (D-R:1) , SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (T) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

A-1073723

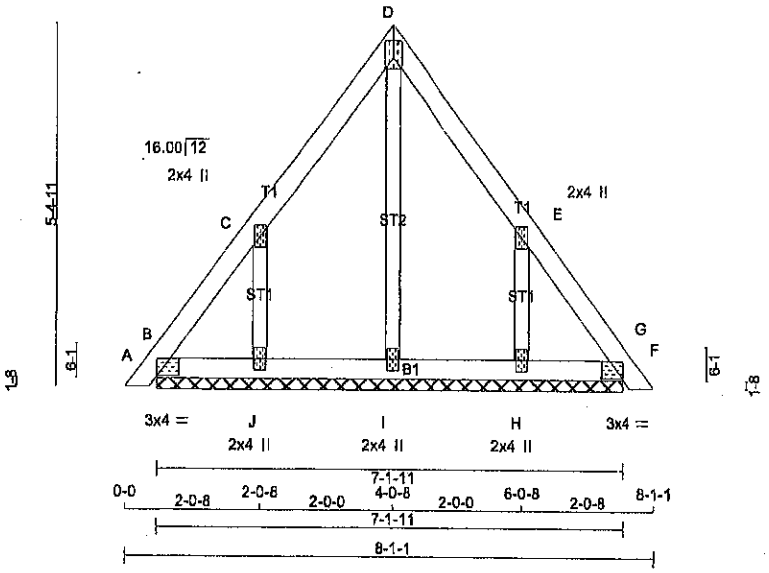
253446	H13G	1	1	TRUSS DESC.

Alpa Roof Trusst Maple

ID:aoVBp?3jjJ2mJLehkgm_M5zC2?4-F10Fkw7u?B10JhBrIxnKBgKwOXC7aJ8B4hovygyxZIV

3x5 II

Scale = 1:34.8



TOTAL WEIGHT = 30 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	5.0	2.00	Edge
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	I-D	-58 / 0	0.02 (1)
B-C	-39 / 0	-70.5	-70.5	0.04 (1)	6.25	J-C	-147 / 0	0.02 (1)
C-D	-51 / 0	-70.5	-70.5	0.04 (1)	6.25	H-E	-147 / 0	0.02 (1)
D-E	-51 / 0	-70.5	-70.5	0.04 (1)	6.25			
E-F	-39 / 0	-70.5	-70.5	0.04 (1)	6.25			
F-G	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B-J	0 / 32	-17.5	-17.5	0.01 (4)	10.00			
J-I	0 / 24	-17.5	-17.5	0.01 (4)	10.00			
I-H	0 / 24	-17.5	-17.5	0.01 (4)	10.00			
H-F	0 / 32	-17.5	-17.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (D-E:1) , BC=0.01 (I-J:4) , WB=0.02 (E-H:1) , SSI=0.04 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (F) (INPUT = 0.90)
JSI METAL= 0.04 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-16-2015
R. TURENNE
100157048
PROVINCE OF ONTARIO

A-1073722

253446	H13	3	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 07:48:48 2015 Page 1
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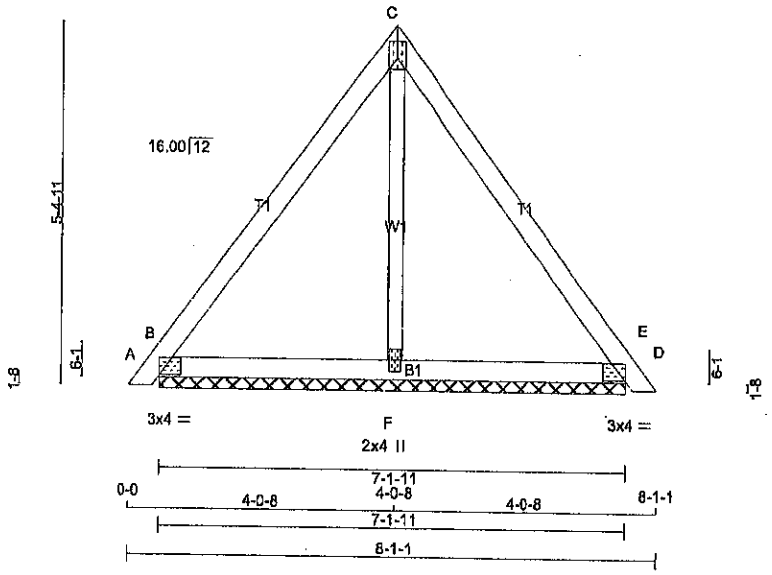
3x5 II



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	243	0	243	0	0	7-1-11	1-8
D	243	0	243	0	0	7-1-11	1-8
F	191	0	191	0	0	7-1-11	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	168	131 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
D	168	131 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
F	140	67 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	0 / 9	-70.5	-70.5	0.01 (1)		10.00
B-C	-86 / 0	-70.5	-70.5	0.15 (1)		6.25
C-D	-86 / 0	-70.5	-70.5	0.15 (1)		6.25
D-E	0 / 9	-70.5	-70.5	0.01 (1)		10.00
B-F	0 / 51	-17.5	-17.5	0.07 (4)		10.00
F-D	0 / 51	-17.5	-17.5	0.07 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-113 / 0	0.04 (1)

TOTAL WEIGHT = 3 X 27 = 81 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15 (C-D:1) , BC=0.07 (D-F:4) , WB=0.04 (C-F:1) , SSI=0.06 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1666

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-16-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-5073721

253446	H12SG	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

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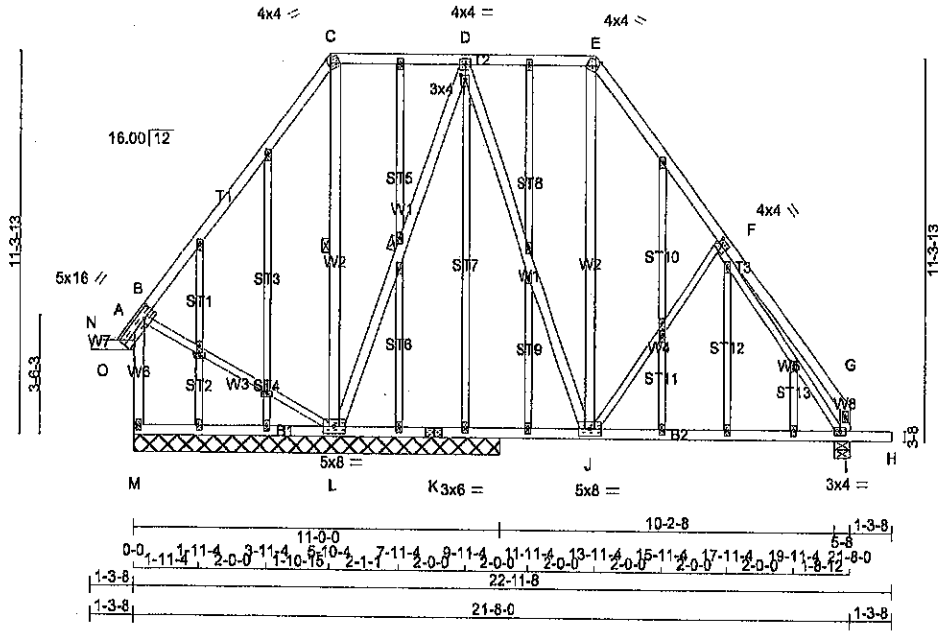

TOTAL WEIGHT = 187 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF
F - I	2x3	DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2	SPF
ST3	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVWW-I	MT20	5.0	16.0	2.50	4.25
C	TTW-m	MT20	4.0	4.0	1.75	1.00
D	TMWW-t	MT20	4.0	4.0		
D	NP+p	MT20	3.0	4.0	1.50	1.50
E	TTW-m	MT20	4.0	4.0	1.75	1.00
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW1-t	MT20	5.0	8.0		
M	BMV1+p	MT20	2.0	4.0		
P, Q, R, S, T, U, V, W, X, Y, Z, AA, AB, AC, AD, AE, AF, AG, AG, AH, AI, AJ, AK						
P	NP+w	MT20	2.0	4.0		
Y	NP-p	MT20	2.0	4.0	0.75	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	349	0	349	0	0	11-0-0 (9-0-0)	1-8
L	926	0	926	0	0	11-0-0 (9-0-0)	1-8
I	837	0	837	0	0	5-8	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	241	191 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
L	663	392 / 0	0 / 0	0 / 0	0 / 0	271 / 0	0 / 0
I	590	400 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, L, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00FT.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-L, D-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-213 / 0	-70.5	-70.5	0.26 (1)	6.25
B- C	-95 / 0	-70.5	-70.5	0.30 (1)	6.25
C- D	-33 / 2	-78.0	-78.0	0.16 (1)	6.25
D- E	-279 / 0	-78.0	-78.0	0.16 (1)	6.25
E- F	-490 / 0	-70.5	-70.5	0.14 (1)	6.25
F- G	0 / 26	-70.5	-70.5	0.17 (1)	10.00
M- O	-314 / 0	0.0	0.0	0.16 (1)	7.81
O- B	-371 / 0	0.0	0.0	0.15 (1)	7.81
I- G	-101 / 0	0.0	0.0	0.01 (1)	7.81
M- L	0 / 76	-17.5	-17.5	0.19 (4)	10.00
L- K	0 / 225	-17.5	-17.5	0.28 (4)	10.00
K- J	0 / 225	-17.5	-17.5	0.28 (4)	10.00
J- I	0 / 379	-17.5	-17.5	0.29 (4)	10.00
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-234 / 0	0.16 (1)
L- D	-574 / 0	0.45 (1)
D- J	0 / 162	0.03 (1)
J- E	0 / 145	0.02 (1)
J- F	-175 / 0	0.17 (1)
B- L	-44 / 0	0.04 (1)
F- I	-682 / 0	0.60 (1)
N- A	0 / 0	0.00 (1)
A- O	0 / 377	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND 6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.30 (B-C:1) , BC=0.29 (I-J:4) , WB=0.60 (F-I:1) , SSI=0.25 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.80)
JSI METAL= 0.28 (F) (INPUT = 1.00)

A-15073720

253446	H01-Cond2	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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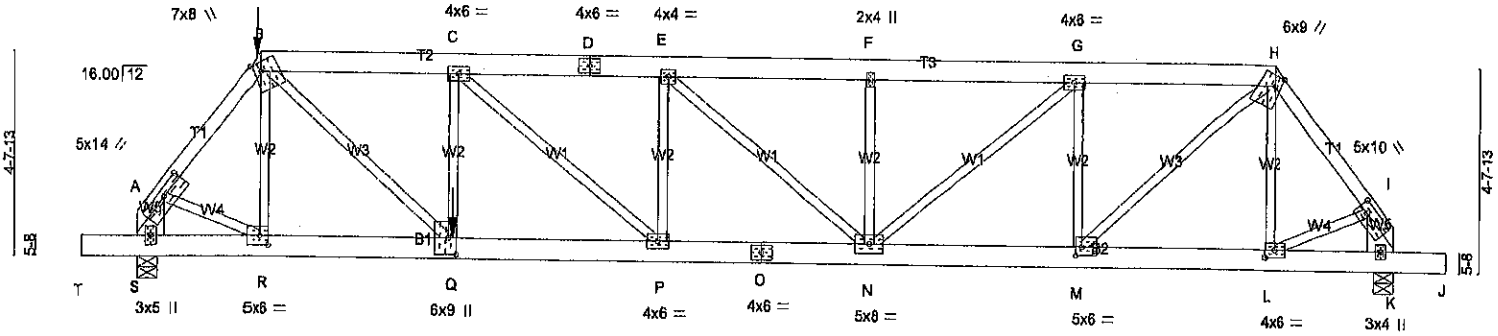

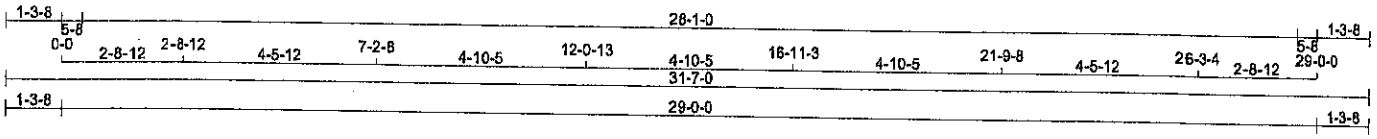

TOTAL WEIGHT = 2 X 160 = 320 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
S - A	2x8	DRY	No.2	SPF
K - I	2x8	DRY	No.2	SPF
T - O	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-t	MT20	5.0	14.0	1.75	6.76
B	TTWW+m	MT20	7.0	8.0	2.50	2.26
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	2.75	1.75
I	TMVW-t	MT20	5.0	10.0	2.25	2.76
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.76
M	BMWW-t	MT20	5.0	6.0	2.25	1.75
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW+t	MT20	6.0	9.0	4.50	2.25
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3051	0	3051	0	0	5-8	3-4
K	1856	0	1856	0	0	5-8	1-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2133	1543 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0
K	1303	918 / 0	0 / 0	0 / 0	0 / 0	385 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	-3088 / 0	-70.5	-70.5	0.26 (1)		3.76
B-C	-4094 / 0	-138.7	-138.7	0.34 (1)		3.96
C-D	-4063 / 0	-70.5	-70.5	0.20 (1)		4.13
D-E	-4063 / 0	-70.5	-70.5	0.20 (1)		4.13
E-F	-3511 / 0	-70.5	-70.5	0.17 (1)		4.42
F-G	-3511 / 0	-70.5	-70.5	0.17 (1)		4.42
G-H	-2484 / 0	-70.5	-70.5	0.13 (1)		5.11
H-I	-1769 / 0	-70.5	-70.5	0.15 (1)		4.84
S-A	-2923 / 0	0.0	0.0	0.16 (1)		6.82
K-I	-1734 / 0	0.0	0.0	0.09 (1)		7.81
T-S	0 / 0	-88.0	-88.0	0.05 (1)		10.00
S-R	0 / 0	-34.5	-34.5	0.07 (1)		10.00
R-Q	0 / 1647	-34.5	-34.5	0.32 (1)		10.00
Q-P	0 / 4094	-17.5	-17.5	0.61 (1)		10.00
P-O	0 / 4063	-17.5	-17.5	0.57 (1)		10.00
O-N	0 / 4063	-17.5	-17.5	0.57 (1)		10.00
N-M	0 / 2484	-17.5	-17.5	0.37 (1)		10.00
M-L	0 / 1069	-17.5	-17.5	0.17 (1)		10.00
L-K	0 / 0	-17.5	-17.5	0.03 (4)		10.00
K-J	0 / 0	-88.0	-88.0	0.05 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-R	-155 / 74	0.05 (1)
B-Q	0 / 3076	0.76 (1)
Q-C	-478 / 0	0.14 (1)
C-P	-42 / 62	0.03 (1)
P-E	0 / 137	0.03 (1)
E-N	-729 / 0	0.52 (1)
N-F	-337 / 0	0.10 (1)
N-G	0 / 1356	0.34 (1)
M-G	-1241 / 0	0.36 (1)
M-H	0 / 1936	0.48 (1)
L-H	-122 / 20	0.04 (1)
A-R	0 / 1884	0.47 (1)
L-I	0 / 1092	0.27 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-8-12	-140	-140	---	FRONT	VERT	TOTAL
Q	7-2-8	-1564	-1564	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 7-2-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.34 (B-C:1) , BC=0.61 (P-Q:1) , WB=0.76 (B-Q:1) , SSI=0.22 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE HEELS OFF

A-150737/8

CONTINUED ON PAGE 2

253446	H01-Cond1	2	1	TRUSS DESC.	JOB NO.

Alpa Roof Truss, Maple

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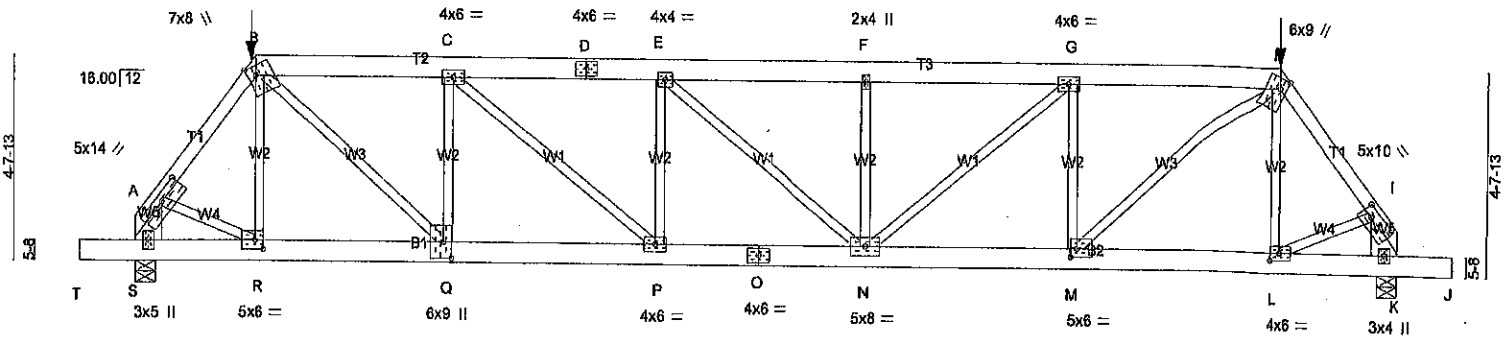
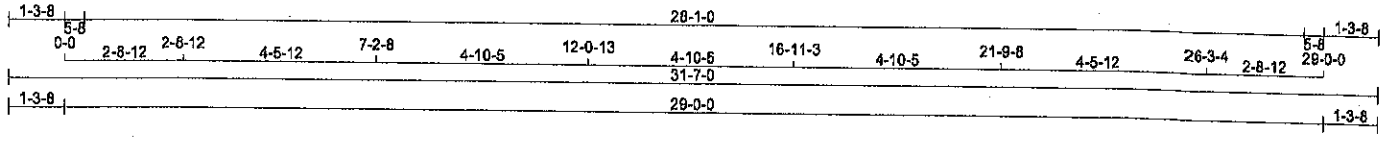




TOTAL WEIGHT = 2 X 160 = 320 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
S - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - O	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	14.0	1.75	6.75
B	TTWW+m	MT20	7.0	8.0	2.50	2.25
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	2.75	1.75
I	TMVW-t	MT20	5.0	10.0	2.25	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BMWW-t	MT20	5.0	6.0	2.25	1.75
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW+t	MT20	6.0	9.0	4.50	2.25
R	BMWW-t	MT20	5.0	8.0	2.50	2.60
S	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2578	0	2578	0	0	5-8	2-12
K	2578	0	2578	0	0	5-8	2-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1818	1226 / 0	0 / 0	0 / 0	0 / 0	593 / 0	0 / 0
K	1818	1225 / 0	0 / 0	0 / 0	0 / 0	593 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	-2558 / 0	-70.5	-70.5	0.21 (1)		4.13
B-C	-3336 / 0	-138.7	-138.7	0.26 (1)		4.41
C-D	-4303 / 0	-138.7	-138.7	0.31 (1)		3.91
D-E	-4303 / 0	-138.7	-138.7	0.31 (1)		3.91
E-F	-4294 / 0	-138.7	-138.7	0.29 (1)		3.94
F-G	-4294 / 0	-138.7	-138.7	0.32 (1)		3.91
G-H	-3336 / 0	-138.7	-138.7	0.26 (1)		4.40
H-I	-2557 / 0	-70.5	-70.5	0.21 (1)		4.13
S-A	-2438 / 0	0.0	0.0	0.13 (1)		7.32
K-I	-2437 / 0	0.0	0.0	0.13 (1)		7.32
T-S	0 / 0	-88.0	-88.0	0.05 (1)		10.00
S-R	0 / 0	-34.5	-34.5	0.08 (4)		10.00
R-Q	0 / 1529	-34.5	-34.5	0.24 (1)		10.00
Q-P	0 / 3336	-34.5	-34.5	0.50 (1)		10.00
P-O	0 / 4303	-34.5	-34.5	0.64 (1)		10.00
O-N	0 / 4303	-34.5	-34.5	0.64 (1)		10.00
N-M	0 / 3336	-34.5	-34.5	0.51 (1)		10.00
M-L	0 / 1529	-34.5	-34.5	0.24 (1)		10.00
L-K	0 / 0	-34.5	-34.5	0.06 (4)		10.00
K-J	0 / 0	-88.0	-88.0	0.05 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-B	-138 / 61	0.04 (1)
B-Q	0 / 2473	0.61 (1)
Q-C	-1636 / 0	0.45 (1)
C-P	0 / 1276	0.32 (1)
P-E	-665 / 0	0.19 (1)
E-N	-11 / 0	0.01 (1)
N-F	-660 / 0	0.19 (1)
N-G	0 / 1282	0.31 (1)
M-G	-1632 / 0	0.45 (1)
M-H	0 / 2478	0.61 (1)
L-H	-140 / 61	0.04 (1)
A-R	0 / 1561	0.39 (1)
L-I	0 / 1560	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-8-12	-140	-140	---	FRONT	VERT	TOTAL
H	26-3-4	-140	-140	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.32 (F-G:1) , BC=0.64 (N-P:1) , WB=0.61 (H-M:1) , SSI=0.24 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.00 Deg

A-R07317

CONTINUED ON PAGE 2

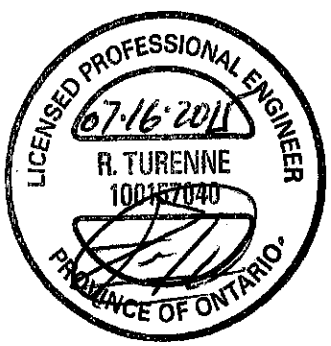
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.2 lbs FACTORED DOWN AT 26-3-4, AND 140.2 lbs FACTORED DOWN AT 2-8-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.93 (O) (INPUT = 1.00)



A-N073717(2)

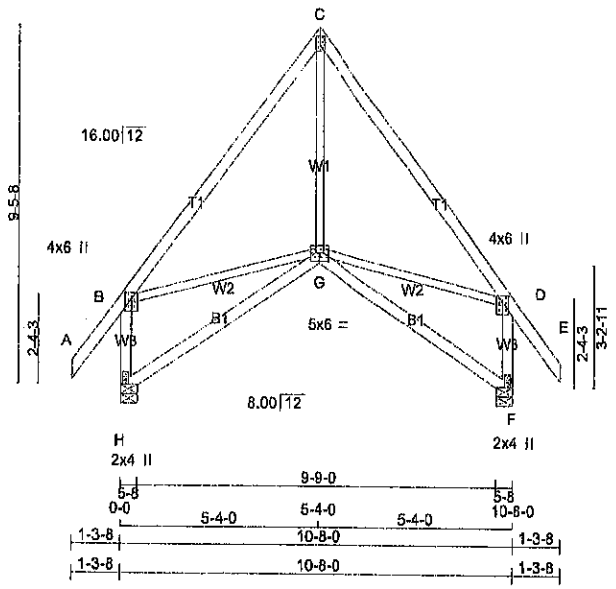
253444	H1D	6	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 17:00:13 2015 Page 1
ID:kieTlGsxnnIsITH9K2?DU?zO9XP-kK8kVMwWHm9Vod1WfIpEy1m2DkXJlWY_nksKFHyxmmW

3x5 II

Scale = 1:61.6



TOTAL WEIGHT = 6 X 59 = 351 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	568	0	568	0	0	5-8	1-8
F	568	0	568	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	398	283 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	398	283 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	1	0.10 (1)	10.00	G- C	0 / 197	0.05 (4)
B- C	-358 / 0	-70.5	-70.5	1	0.26 (1)	6.25	B- G	0 / 221	0.05 (1)
C- D	-358 / 0	-70.5	-70.5	1	0.26 (1)	6.25	G- D	0 / 221	0.05 (1)
D- E	0 / 40	-70.5	-70.5	1	0.10 (1)	10.00			
H- B	-522 / 0	0.0	0.0	1	0.07 (1)	7.81			
F- D	-522 / 0	0.0	0.0	1	0.07 (1)	7.81			
H- G	0 / 0	-17.5	-17.5	1	0.15 (4)	10.00			
G- F	0 / 0	-17.5	-17.5	1	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.26 (C-D:1) , BC=0.15 (G-H:4) , WB=0.05 (D-G:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (H) (INPUT = 0.90)
JSI METAL= 0.12 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-150737 02

253459	J01C	4	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-1FC_75bW?lzsllcwUVVt5W32eM5wKeZ9vRRCqtryxnOz

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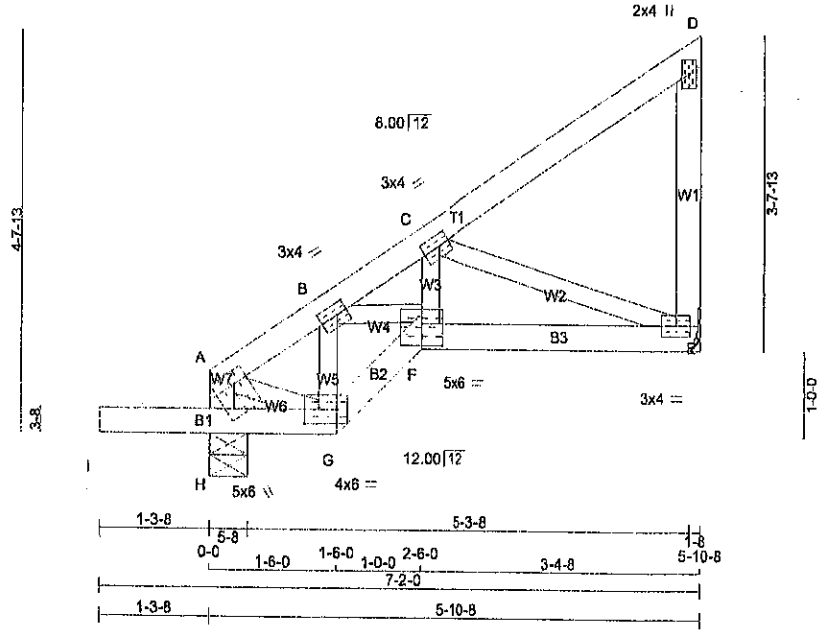


TOTAL WEIGHT = 4 X 27 = 107 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-t	MT20	5.0	6.0	3.00	3.00
G	BBWW-t	MT20	4.0	6.0	2.00	4.50
H	TMBMVW1*+h	MT20	5.0	6.0	1.75	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	384	0	384	0	0	5-8	1-8
E	246	0	246	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	271	183 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
E	173	117 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-A	-209 / 0	0.0	0.0	0.02 (1)	7.81		A-G	0 / 172	0.04 (1)
A-B	-203 / 0	-70.5	-70.5	0.02 (1)	6.25		G-B	-219 / 0	0.03 (1)
B-C	-403 / 0	-70.5	-70.5	0.06 (1)	6.25		B-F	0 / 221	0.05 (1)
C-D	-10 / 0	-70.5	-70.5	0.08 (1)	6.25		F-C	0 / 131	0.03 (1)
E-D	-97 / 0	0.0	0.0	0.02 (1)	7.81		C-E	-370 / 0	0.08 (1)
I-H	0 / 0	-88.0	-88.0	0.10 (1)	10.00				
H-G	0 / 0	-17.5	-17.5	0.08 (1)	10.00				
G-F	0 / 211	-17.5	-17.5	0.04 (1)	10.00				
F-E	0 / 349	-17.5	-17.5	0.09 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.08 (C-D:1) , BC=0.10 (H-I:1) , WB=0.08 (C-E:1) , SSI=0.09 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (E) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

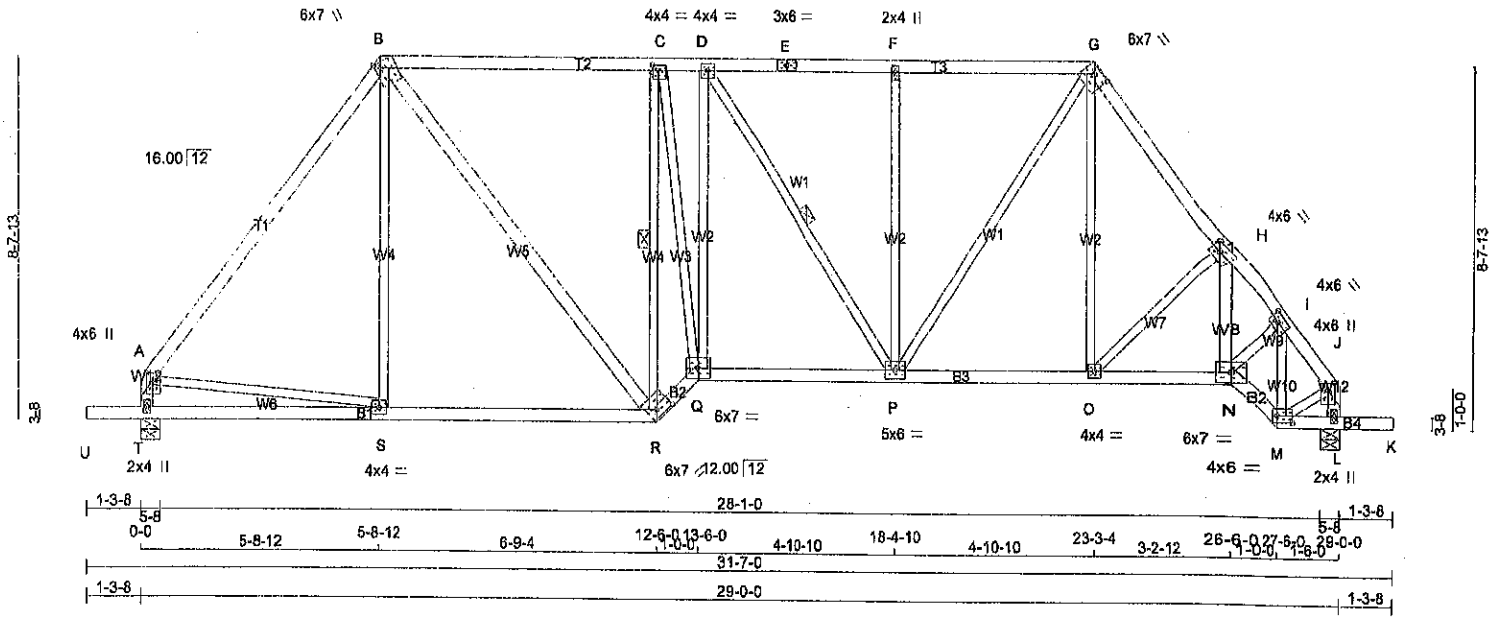
A-15073693

253443	H04T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-8Rz3h1xXXJgujDZDG??7E4GYYO96ngLyzs_jajyxnXz

Scale = 1:55.4



TOTAL WEIGHT = 158 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMWW-t	MT20	4.0	4.0	1.50	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-h	MT20	6.0	7.0	1.75	4.00
H	TMWW-t	MT20	4.0	6.0	2.00	2.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMVW+p	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BBWW-I	MT20	4.0	6.0	2.00	4.50
N	BBWW-I	MT20	6.0	7.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BBWW-I	MT20	6.0	7.0		
R	BBWW-h	MT20	6.0	7.0	2.00	4.25
S	BMWW-t	MT20	4.0	4.0		
T	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1386	0	1386	0	0	5-8	1-8
L	1391	0	1391	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
L	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-R, D-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	-1194 / 0	-70.5	-70.5	0.57 (1)		5.25
B-C	-1125 / 0	-70.5	-70.5	0.57 (1)		5.18
C-D	-1259 / 0	-70.5	-70.5	0.50 (1)		4.94
D-E	-1191 / 0	-70.5	-70.5	0.29 (1)		5.50
E-F	-1191 / 0	-70.5	-70.5	0.29 (1)		5.50
F-G	-1190 / 0	-70.5	-70.5	0.29 (1)		5.50
G-H	-1348 / 0	-70.5	-70.5	0.11 (1)		5.48
H-I	-1591 / 0	-70.5	-70.5	0.10 (1)		5.15
I-J	-1120 / 0	-70.5	-70.5	0.06 (1)		5.94
T-A	-1236 / 0	0.0	0.0	0.12 (1)		7.22
L-J	-1216 / 0	0.0	0.0	0.12 (1)		7.26
U-T	0 / 0	-88.0	-88.0	0.10 (1)		10.00
T-S	0 / 0	-17.5	-17.5	0.20 (4)		10.00
S-R	0 / 714	-17.5	-17.5	0.26 (4)		10.00
R-Q	0 / 1615	-17.5	-17.5	0.26 (1)		10.00
Q-P	0 / 1256	-17.5	-17.5	0.26 (1)		10.00
P-O	0 / 798	-17.5	-17.5	0.18 (1)		10.00
O-N	0 / 961	-17.5	-17.5	0.19 (1)		10.00
N-M	0 / 884	-17.5	-17.5	0.15 (1)		10.00
M-L	0 / 0	-17.5	-17.5	0.10 (1)		10.00
L-K	0 / 0	-88.0	-88.0	0.10 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-B	0 / 127	0.04 (4)
B-R	0 / 649	0.10 (1)
R-C	-1568 / 0	0.76 (1)
C-Q	0 / 951	0.21 (1)
Q-D	0 / 249	0.06 (1)
D-P	-119 / 0	0.06 (1)
P-F	-400 / 0	0.43 (1)
P-G	0 / 707	0.16 (1)
O-G	0 / 246	0.06 (1)
O-H	-258 / 0	0.09 (1)
N-H	0 / 186	0.04 (1)
N-I	0 / 800	0.13 (1)
M-I	-917 / 0	0.16 (1)
A-S	0 / 718	0.16 (1)
M-J	0 / 714	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.57 (A-B:1) , BC=0.26 (Q-R:1) , WB=0.76 (C-R:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

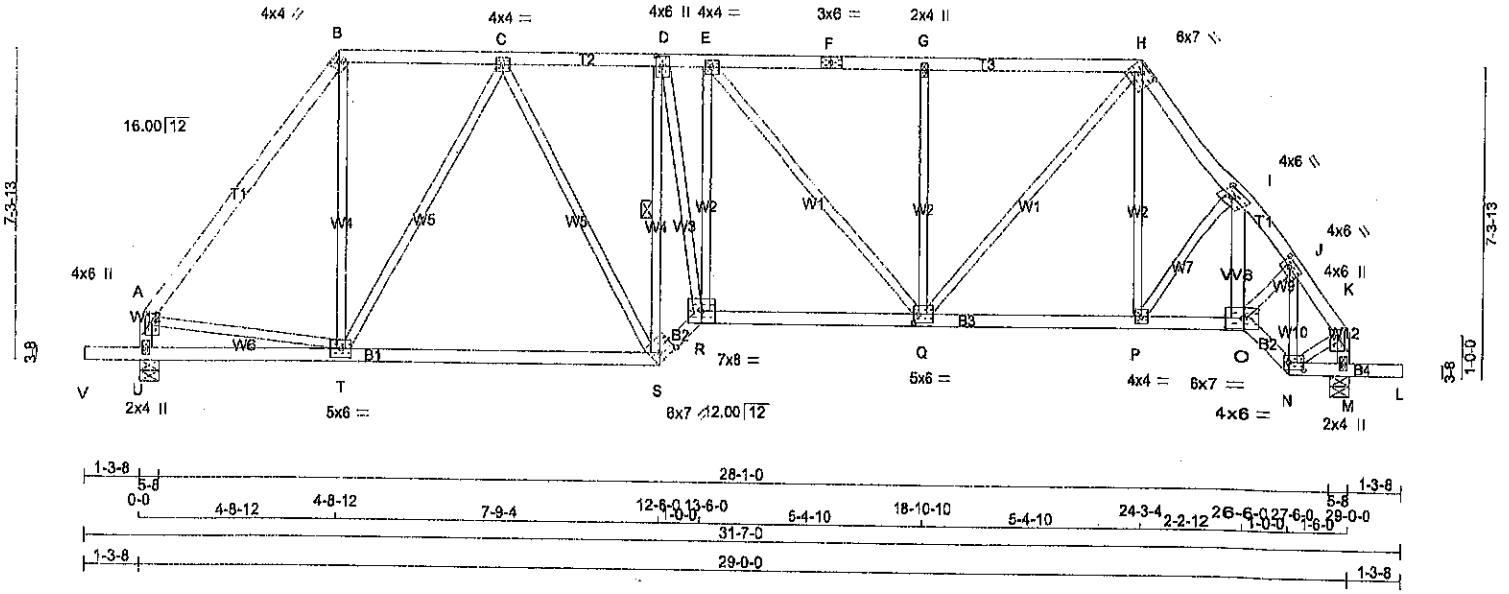
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.27 (A) (INPUT = 1.00)

A-H073692

253443	H03T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple



TOTAL WEIGHT = 148 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+p	MT20	4.0	6.0	2.00	2.00
B	TTW-h	MT20	4.0	4.0	1.75	2.00
C	TMWW-t	MT20	4.0	4.0		
D	TMWW+t	MT20	4.0	6.0	3.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-h	MT20	6.0	7.0	1.50	3.75
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-I	MT20	4.0	6.0	2.00	4.50
O	BBWW-I	MT20	6.0	7.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWWW-t	MT20	5.0	6.0	2.50	1.50
R	BBWW-I	MT20	7.0	8.0		
S	BBWW-h	MT20	6.0	7.0	2.25	4.50
T	BMWWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1386	0	1386	0	0	5-8	1-8
M	1391	0	1391	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
M	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-S.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	-1232 / 0	-70.5	-70.5	0.36 (1)		5.41
B- C	-752 / 0	-70.5	-70.5	0.17 (1)		8.25
C- D	-1300 / 0	-70.5	-70.5	0.19 (1)		5.45
D- E	-1539 / 0	-70.5	-70.5	0.12 (1)		5.19
E- F	-1431 / 0	-70.5	-70.5	0.36 (1)		5.03
F- G	-1431 / 0	-70.5	-70.5	0.36 (1)		5.03
G- H	-1430 / 0	-70.5	-70.5	0.36 (1)		5.04
H- I	-1414 / 0	-70.5	-70.5	0.07 (1)		5.43
I- J	-1603 / 0	-70.5	-70.5	0.05 (1)		5.19
J- K	-1121 / 0	-70.5	-70.5	0.05 (1)		5.84
U- A	-1249 / 0	0.0	0.0	0.13 (1)		7.19
M- K	-1216 / 0	0.0	0.0	0.12 (1)		7.26
V- U	0 / 0	-88.0	-88.0	0.10 (1)		10.00
U- T	0 / 0	-17.5	-17.5	0.22 (4)		10.00
T- S	0 / 1104	-17.5	-17.5	0.31 (4)		10.00
S- R	0 / 1866	-17.5	-17.5	0.30 (1)		10.00
R- Q	0 / 1540	-17.5	-17.5	0.31 (1)		10.00
Q- P	0 / 840	-17.5	-17.5	0.20 (1)		10.00
P- O	0 / 868	-17.5	-17.5	0.20 (1)		10.00
O- N	0 / 868	-17.5	-17.5	0.15 (1)		10.00
N- M	0 / 0	-17.5	-17.5	0.10 (1)		10.00
M- L	0 / 0	-88.0	-88.0	0.10 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	0 / 706	0.16 (1)
T- C	-752 / 0	1.00 (1)
C- S	0 / 404	0.09 (1)
S- D	-1606 / 0	0.54 (1)
D- R	0 / 1462	0.33 (1)
R- E	-59 / 24	0.04 (1)
E- Q	-165 / 0	0.22 (1)
Q- G	-435 / 0	0.28 (1)
Q- H	0 / 883	0.20 (1)
P- H	0 / 249	0.06 (1)
P- I	-228 / 0	0.06 (1)
O- I	0 / 199	0.04 (1)
O- J	0 / 574	0.13 (1)
N- J	-921 / 0	0.16 (1)
A- T	0 / 741	0.17 (1)
N- K	0 / 718	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.36 (G-H:1) , BC=0.31 (Q-R:1) , WB=1.00 (C-T:1) , SSI=0.16 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (D) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)

A-ND73691

253443	H02T	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 16:07:52 2015 Page 1
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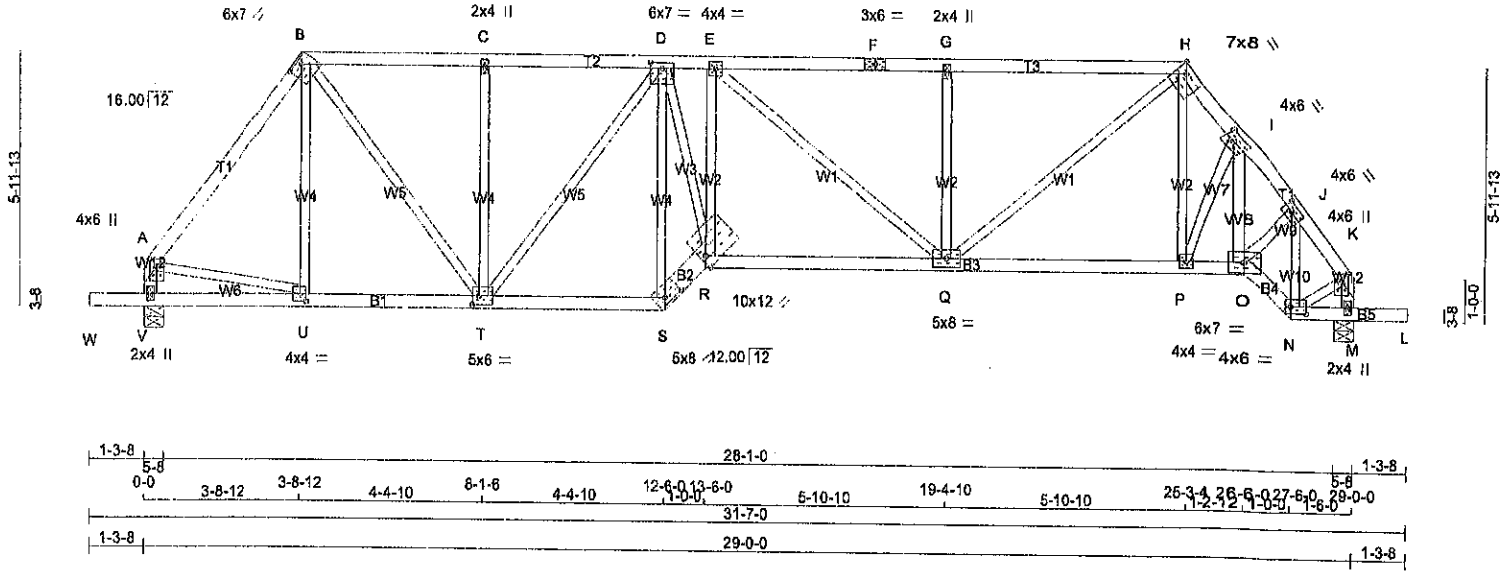

TOTAL WEIGHT = 142 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - R	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW-h	MT20	6.0	7.0	1.50	3.75
C	TMW+w	MT20	2.0	4.0		
D	TMWWW-t	MT20	6.0	7.0	1.50	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-h	MT20	7.0	8.0	Edge	2.75
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-t	MT20	4.0	6.0	2.00	4.50
O	BBWW-t	MT20	6.0	7.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BBWW-h	MT20	10.0	12.0	3.25	1.50
S	BBW-h	MT20	5.0	8.0	1.75	5.00
T	BMWWW-t	MT20	5.0	8.0	2.50	2.00
U	BMWW-t	MT20	4.0	4.0	2.00	1.75
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1386	0	1386	0	0	5-8	1-8
M	1391	0	1391	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
M	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1239 / 0	-70.5	-70.5	0.21 (1)	5.55
B- C	-1332 / 0	-70.5	-70.5	0.22 (1)	5.37
C- D	-1332 / 0	-70.5	-70.5	0.22 (1)	5.37
D- E	-2010 / 0	-70.5	-70.5	0.17 (1)	4.62
E- F	-1803 / 0	-70.5	-70.5	0.46 (1)	4.48
F- G	-1803 / 0	-70.5	-70.5	0.46 (1)	4.48
G- H	-1803 / 0	-70.5	-70.5	0.47 (1)	4.48
H- I	-1494 / 0	-70.5	-70.5	0.05 (1)	5.34
I- J	-1611 / 0	-70.5	-70.5	0.06 (1)	5.17
J- K	-1122 / 0	-70.5	-70.5	0.04 (1)	5.95
V- A	-1247 / 0	0.0	0.0	0.13 (1)	7.19
M- K	-1216 / 0	0.0	0.0	0.12 (1)	7.26
W- V	0 / 0	-88.0	-88.0	0.10 (1)	10.00
V- U	0 / 0	-17.5	-17.5	0.06 (4)	10.00
U- T	0 / 738	-17.5	-17.5	0.16 (1)	10.00
T- S	0 / 1623	-17.5	-17.5	0.31 (1)	10.00
S- R	0 / 2265	-17.5	-17.5	0.27 (1)	10.00
R- Q	0 / 2011	-17.5	-17.5	0.39 (1)	10.00
Q- P	0 / 889	-17.5	-17.5	0.22 (4)	10.00
P- O	0 / 959	-17.5	-17.5	0.21 (1)	10.00
O- N	0 / 892	-17.5	-17.5	0.15 (1)	10.00
N- M	0 / 0	-17.5	-17.5	0.10 (1)	10.00
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- B	-40 / 48	0.02 (1)
B- T	0 / 988	0.22 (1)
T- C	-334 / 0	0.19 (1)
T- D	-483 / 0	0.46 (1)
S- D	-1519 / 0	0.85 (1)
D- R	0 / 1780	0.40 (1)
R- E	-54 / 32	0.02 (1)
E- Q	-266 / 0	0.29 (1)
Q- G	-471 / 0	0.17 (1)
Q- H	0 / 1165	0.26 (1)
P- H	0 / 267	0.06 (1)
P- I	-209 / 0	0.05 (1)
O- I	0 / 196	0.04 (1)
O- J	0 / 556	0.13 (1)
N- J	-925 / 0	0.16 (1)
A- U	0 / 749	0.17 (1)
N- K	0 / 721	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.47 (G-H:1) , BC=0.39 (Q-R:1) , WB=0.85 (D-S:1) , SSI=0.20 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.47 (R) (INPUT = 1.00)

A-N073690

253443	H01T	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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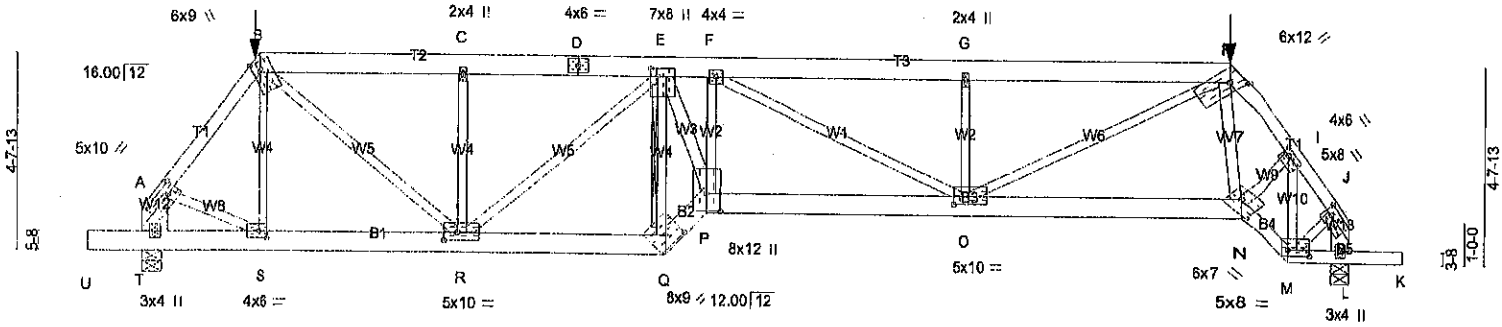

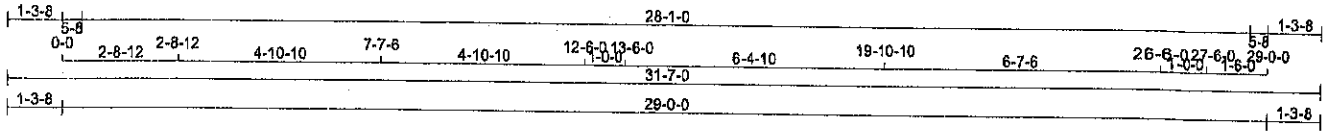

TOTAL WEIGHT = 159 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - A	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	1650F 1.5E	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	10.0	2.25	2.75
B	TTWW+m	MT20	6.0	9.0	2.75	1.75
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	4.0	6.0		
E	TMWWW+t	MT20	7.0	8.0	2.25	2.00
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+h	MT20	6.0	12.0	3.25	2.50
I	TMWW-t	MT20	4.0	6.0	1.50	1.75
J	TMVW-t	MT20	5.0	8.0	2.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BBWW-l	MT20	5.0	8.0	1.75	6.25
N	BBWW-h	MT20	6.0	7.0	3.25	4.25
O	BMWWW-t	MT20	5.0	10.0	2.25	2.25
P	BBWW+p	MT20	6.0	12.0	5.00	4.00
Q	BBW-h	MT20	6.0	9.0	3.25	4.50
R	BMWWW-t	MT20	5.0	10.0	2.25	4.00
S	BMWW-t	MT20	4.0	6.0	2.00	2.75
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2583	0	2583	0	0	5-8	IN-SX 2-12
L	2739	0	2739	0	0	5-8	IN-SX 3-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1821	1228 / 0	0 / 0	0 / 0	0 / 0	592 / 0	0 / 0
L	1926	1332 / 0	0 / 0	0 / 0	0 / 0	593 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-Q
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-2574 / 0	-70.5	-70.5	0.21 (1)		4.11
B- C	-3457 / 0	-138.7	-138.7	0.30 (1)		4.30
C- D	-3457 / 0	-138.7	-138.7	0.29 (1)		4.30
D- E	-3457 / 0	-138.7	-138.7	0.29 (1)		4.30
E- F	-5805 / 0	-138.7	-138.7	0.33 (1)		3.40
F- G	-5168 / 0	-70.5	-70.5	0.43 (1)		3.52
G- H	-5168 / 0	-70.5	-70.5	0.41 (1)		3.54
H- I	-3347 / 0	-70.5	-70.5	0.21 (1)		3.84
I- J	-2282 / 0	-70.5	-70.5	0.14 (1)		4.38
T- A	-2453 / 0	0.0	0.0	0.13 (1)		7.31
L- J	-2436 / 0	0.0	0.0	0.17 (1)		6.58
U- T	0 / 0	-88.0	-88.0	0.05 (1)		10.00
T- S	0 / 0	-34.5	-34.5	0.07 (4)		10.00
S- R	0 / 1540	-34.5	-34.5	0.27 (1)		10.00
R- Q	0 / 4378	-34.5	-34.5	0.66 (1)		10.00
Q- P	0 / 6044	-34.5	-34.5	0.79 (1)		10.00
P- O	0 / 5796	-102.7	-102.7	0.67 (1)		10.00
O- N	0 / 1971	-102.7	-102.7	0.36 (1)		10.00
N- M	0 / 1849	-66.7	-66.7	0.34 (1)		10.00
M- L	-2 / 0	-66.7	-66.7	0.17 (1)		10.00
L- K	0 / 0	-137.2	-137.2	0.17 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- B	-105 / 74	0.03 (1)
B- R	0 / 2525	0.62 (1)
R- C	-748 / 0	0.22 (1)
R- E	-1223 / 0	0.86 (1)
Q- E	-4123 / 0	0.80 (1)
E- P	0 / 4622	0.82 (1)
P- F	0 / 240	0.06 (1)
F- O	-716 / 0	0.67 (1)
O- G	-518 / 0	0.11 (1)
O- H	0 / 3562	0.88 (1)
H- N	0 / 698	0.17 (1)
N- I	0 / 1203	0.30 (1)
M- I	-1869 / 0	0.34 (1)
A- S	0 / 1571	0.39 (1)
M- J	0 / 1463	0.36 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
B	2-8-12	-140	-140	---	FRONT	VERT	TOTAL
H	26-3-4	-139	-139	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 13-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 13-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 26-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 673 (0.52")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 742 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")

4-10136

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.2 lbs FACTORED DOWN AT 2-8-12, AND 138.7 lbs FACTORED DOWN AT 26-3-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

CALCULATED VERT. DEFL.(TL) = L/ 397 (0.04")

CSI: TC=0.43 (F-G:1), BC=0.79 (P-Q:1), WB=0.88 (H-O:1), SSI=0.26 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.72 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100137040
PROVINCE OF ONTARIO

A-H073689(2)

253442	H10G	1	1	TRUSS DESC.	DRW NO.

Alpa Roof Truss, Maple

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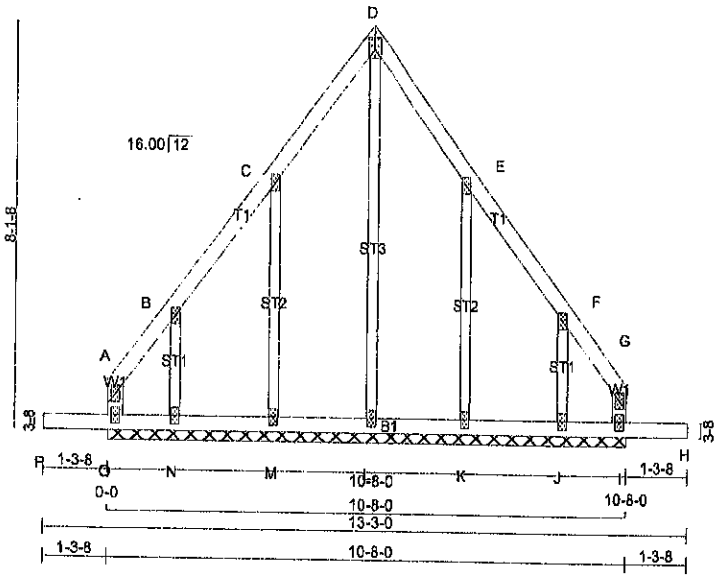
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-xTKZW88yggwollHzb16mt6Y0r1kNBQ2_r9LVm2yxoSh

3x5 II

Scale = 1:46.6



TOTAL WEIGHT = 56 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, E, F						
B	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	6.0	2.00	Edge
G	TMV+p	MT20	2.0	4.0		
I	BMV1+p	MT20	2.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
O-A	-71 / 0	0.0	0.0	0.03 (1)	7.81
A-B	-31 / 0	-70.5	-70.5	0.03 (1)	6.25
B-C	-12 / 4	-70.5	-70.5	0.04 (1)	6.25
C-D	-12 / 4	-70.5	-70.5	0.04 (1)	6.25
D-E	-12 / 4	-70.5	-70.5	0.04 (1)	6.25
E-F	-12 / 4	-70.5	-70.5	0.03 (1)	6.25
F-G	-31 / 0	-70.5	-70.5	0.03 (1)	6.25
I-G	-71 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 0	-88.0	-88.0	0.10 (1)	10.00
O-N	-1 / 15	-17.5	-17.5	0.08 (1)	10.00
N-M	-2 / 10	-17.5	-17.5	0.02 (4)	10.00
M-L	-2 / 7	-17.5	-17.5	0.02 (4)	10.00
L-K	-2 / 7	-17.5	-17.5	0.02 (4)	10.00
K-J	-2 / 10	-17.5	-17.5	0.02 (4)	10.00
J-I	-1 / 15	-17.5	-17.5	0.08 (1)	10.00
I-H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-D	-129 / 0	0.15 (1)
M-C	-148 / 0	0.06 (1)
N-B	-93 / 0	0.02 (1)
K-E	-148 / 0	0.06 (1)
J-F	-93 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (C-D:1) , BC=0.10 (O-P:1) , WB=0.15 (D-L:1) , SSI=0.09 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (C) (INPUT = 0.90)
JSI METAL= 0.04 (C) (INPUT = 1.00)

A-1007368 4

253442	H10	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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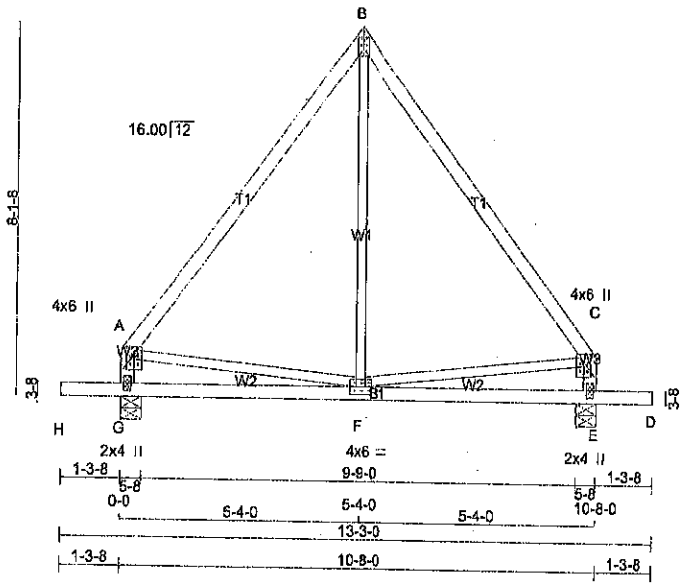
3x5 II

Scale = 1:50.8



TOTAL WEIGHT = 2 X 52 = 104 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW+t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	583	0	583	0	0	5-8	1-8
E	583	0	583	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	411	278 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
E	411	278 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-282 / 0	-70.5	-70.5	0.26 (1)	6.25		F- B	0 / 116	0.04 (4)
B- C	-282 / 0	-70.5	-70.5	0.26 (1)	6.25		A- F	0 / 170	0.04 (1)
G- A	-433 / 0	0.0	0.0	0.04 (1)	7.81		F- C	0 / 170	0.04 (1)
E- C	-433 / 0	0.0	0.0	0.04 (1)	7.81				
H- G	0 / 0	-88.0	-88.0	0.10 (1)	10.00				
G- F	0 / 0	-17.5	-17.5	0.14 (4)	10.00				
F- E	0 / 0	-17.5	-17.5	0.14 (4)	10.00				
E- D	0 / 0	-88.0	-88.0	0.10 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.26 (B-C:1) , BC=0.14 (F-G:4) , WB=0.04 (B-F:4) , SSI=0.09 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1007 3683

253442	H09	2	1	TRUSS DESC.

Alpa Roof Truss-Maple

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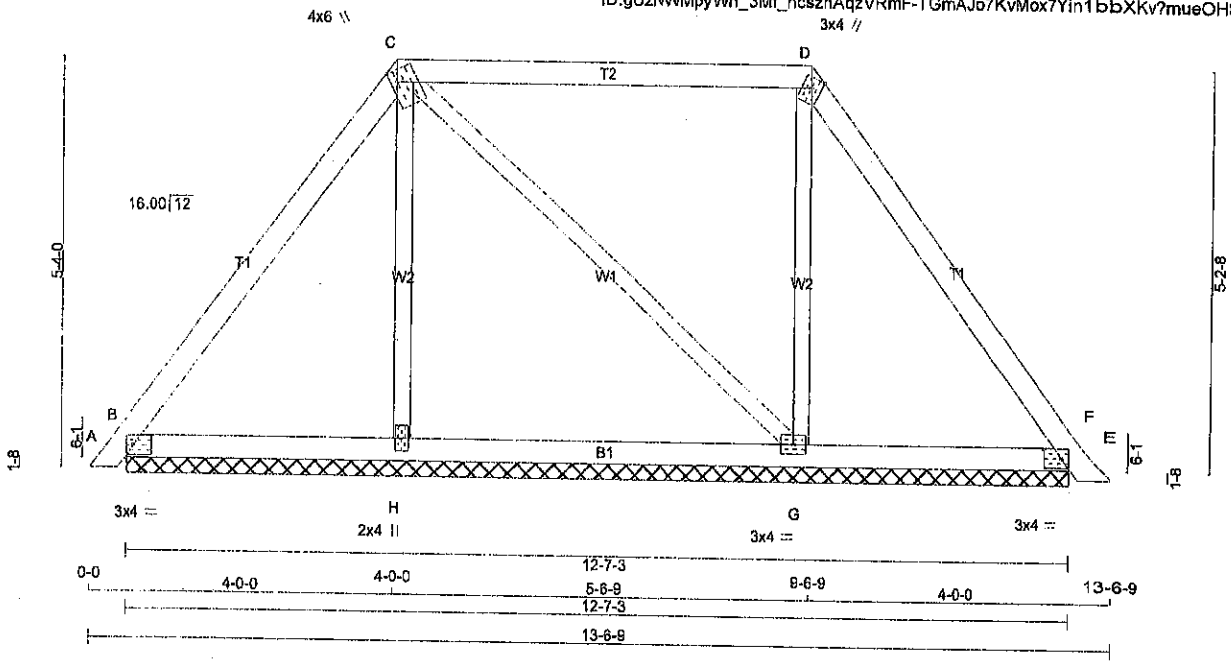

TOTAL WEIGHT = 2 X 49 = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.60	2.75
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.60	2.75
G	BMWW1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	290	0	290	0	0	12-7-3	1-8
E	255	0	255	0	0	12-7-3	1-8
H	266	0	266	0	0	12-7-3	1-8
G	348	0	348	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	200	160 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
E	176	139 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
H	191	105 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
G	247	154 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B-C	-147 / 0	-70.5	-70.5	0.16 (1)	6.25
C-D	-66 / 0	-70.5	-70.5	0.37 (1)	6.25
D-E	-104 / 0	-70.5	-70.5	0.15 (1)	6.25
E-F	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B-H	0 / 86	-17.5	-17.5	0.09 (4)	10.00
H-G	0 / 83	-17.5	-17.5	0.09 (4)	10.00
G-E	0 / 61	-17.5	-17.5	0.09 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-177 / 0	0.07 (1)
C-G	-38 / 0	0.04 (1)
G-D	-235 / 0	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37 (C-D:1) , BC=0.09 (B-H:4) , WB=0.09 (D-G:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-4073682

253442	H08	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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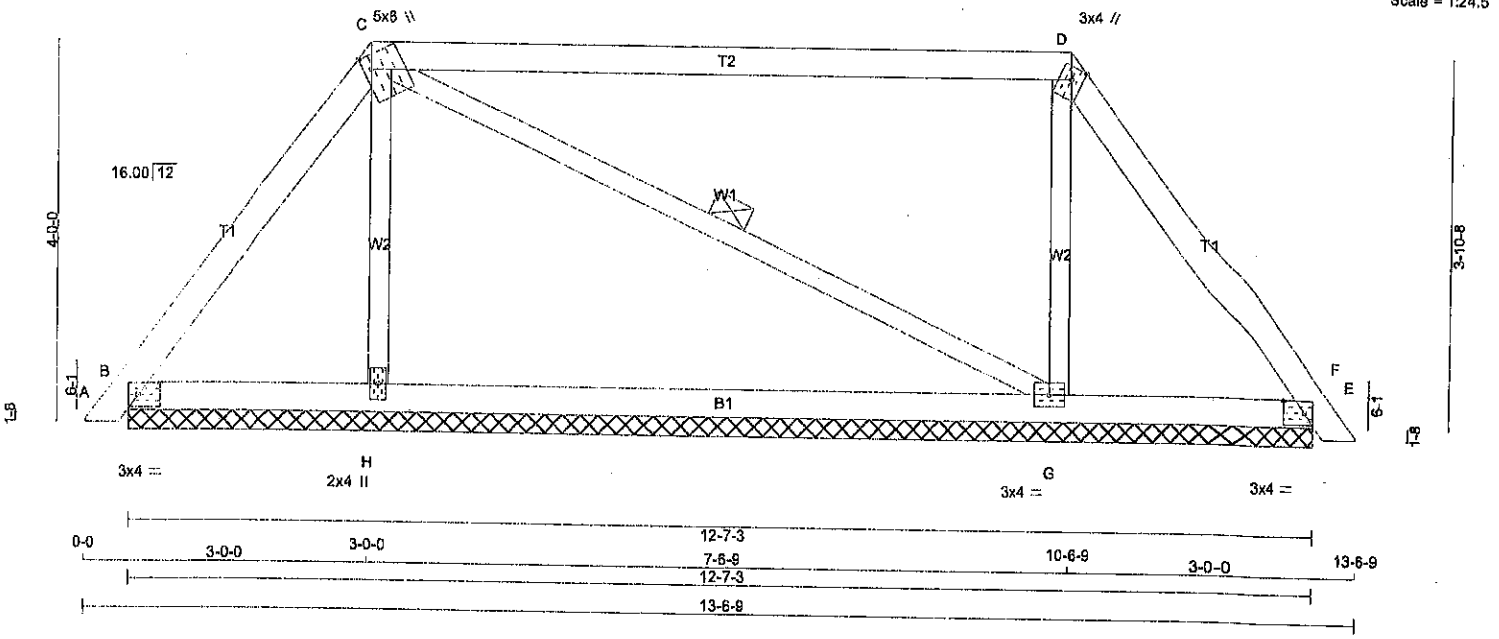

TOTAL WEIGHT = 2 X 46 = 92 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	2100F 1.8E	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	6.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.50	2.75
G	BMWW1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	230	0	230	0	0	12-7-3	1-8
E	209	0	209	0	0	12-7-3	1-8
H	341	0	341	0	0	12-7-3	1-8
G	377	0	377	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	157	138 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
E	142	126 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
H	246	136 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
G	270	158 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-G.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- C	-146 / 0	-70.5	-70.5	0.08 (1)	6.25
C- D	-62 / 0	-70.5	-70.5	0.45 (1)	6.25
D- E	-120 / 0	-70.5	-70.5	0.08 (1)	6.25
E- F	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- H	0 / 86	-17.5	-17.5	0.16 (4)	10.00
H- G	0 / 78	-17.5	-17.5	0.16 (4)	10.00
G- E	0 / 70	-17.5	-17.5	0.16 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-229 / 0	0.05 (1)
C- G	-17 / 0	0.01 (1)
G- D	-257 / 0	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.45 (C-D:1) , BC=0.16 (B-H:4) , WB=0.06 (D-G:1) , SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (D) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073681

253442	H07	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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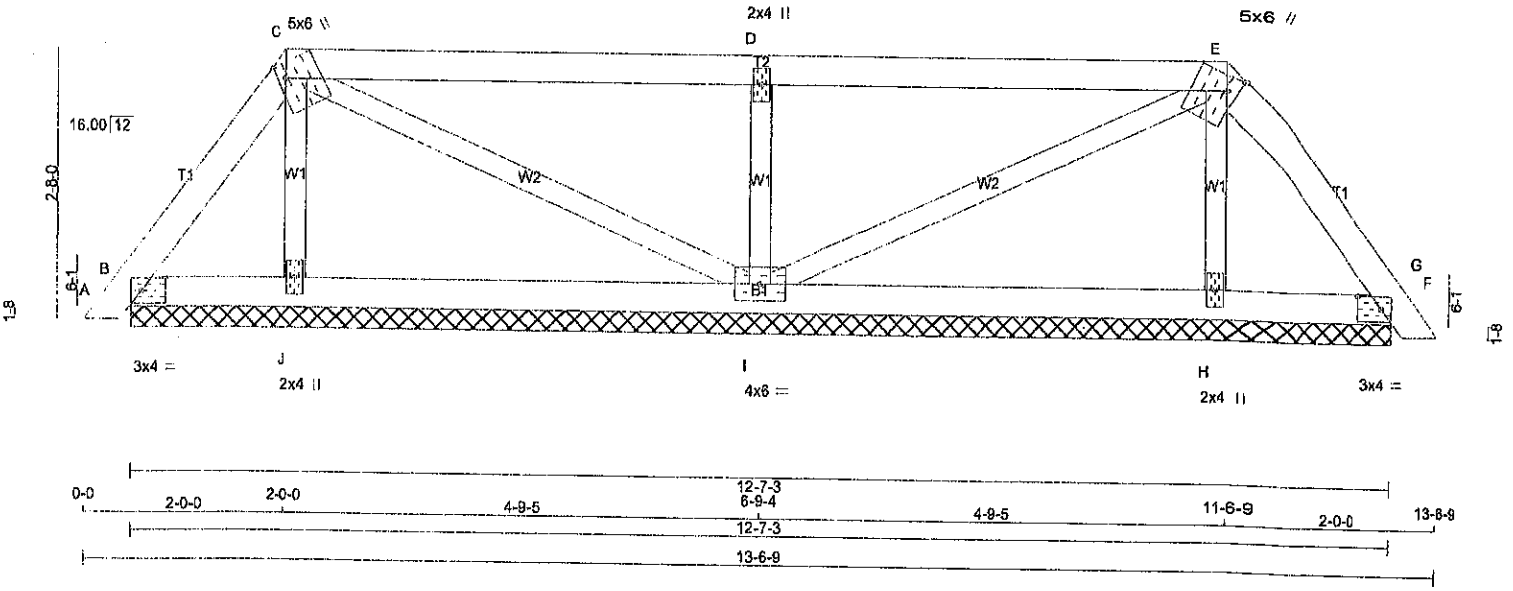

TOTAL WEIGHT = 2 X 48 = 91 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	130	0	130	0	0	12-7-3	1-8
F	130	0	130	0	0	12-7-3	1-8
J	186	0	186	0	0	12-7-3	1-8
I	527	0	527	0	0	12-7-3	1-8
H	186	0	186	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	88	77 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
F	88	77 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
J	134	71 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
I	389	262 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	134	71 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	J- C	-119 / 0		0.02 (1)
B- C	-62 / 0	-70.5	-70.5	0.03 (1)	6.25	C- I	-24 / 0		0.01 (1)
C- D	-8 / 0	-70.5	-70.5	0.27 (1)	10.00	I- D	-420 / 0		0.07 (1)
D- E	-8 / 0	-70.5	-70.5	0.27 (1)	10.00	I- E	-24 / 0		0.01 (1)
E- F	-62 / 0	-70.5	-70.5	0.03 (1)	6.25	H- E	-119 / 0		0.02 (1)
F- G	0 / 9	-70.5	-70.5	0.01 (1)	10.00				
B- J	0 / 36	-17.5	-17.5	0.06 (4)	10.00				
J- I	0 / 30	-17.5	-17.5	0.09 (4)	10.00				
I- H	0 / 30	-17.5	-17.5	0.09 (4)	10.00				
H- F	0 / 36	-17.5	-17.5	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27 (C-D:1) , BC=0.09 (I-J:4) , WB=0.07 (D-I:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-N073680

253442	H06	10	1	TRUSS DESC.

Alpa Roof Truss, Maple

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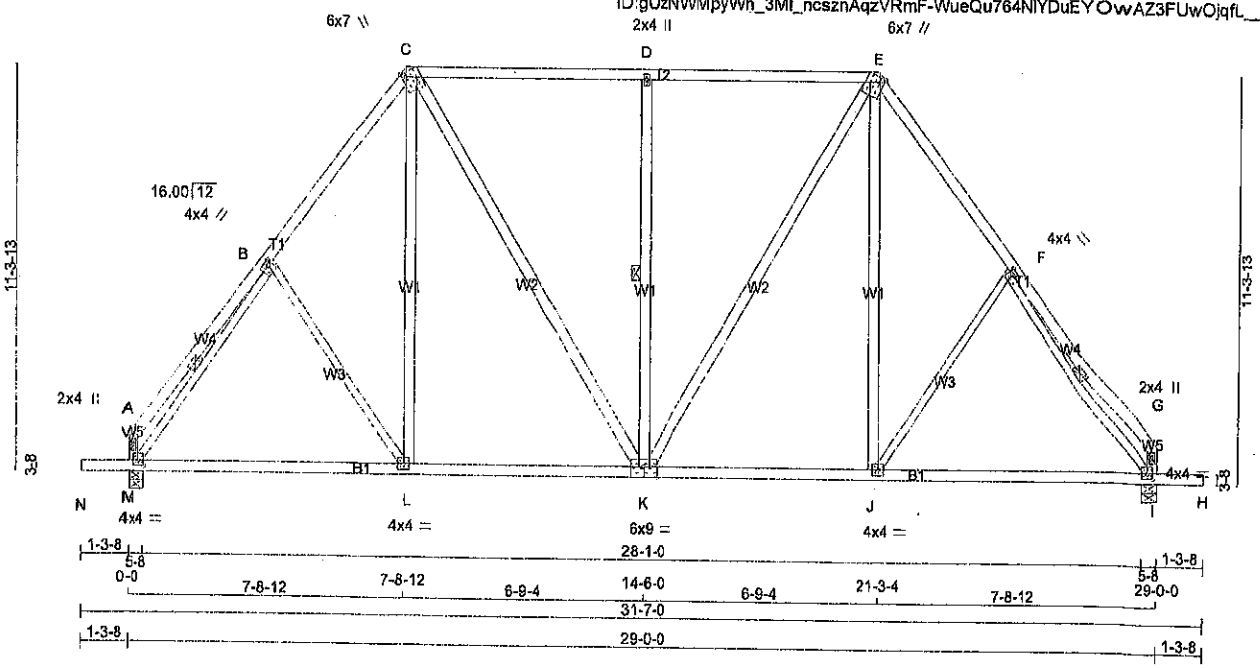


TOTAL WEIGHT = 10 X 170 = 1699 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF
M - B	2x3	DRY	No.2	SPF
F - I	2x3	DRY	No.2	SPF

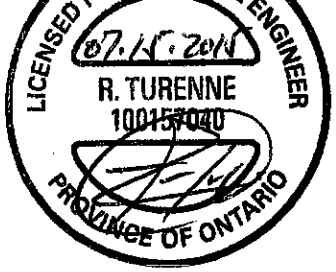
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.50	1.25
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMWW-t	MT20	4.0	4.0	1.50	1.25
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	1.75	2.00
J	BMWW-t	MT20	4.0	4.0		
K	BSWWW-t	MT20	8.0	9.0	Edge	4.50
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1440	0	1440	0	0	5-8	1-8
I	1440	0	1440	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1019	663 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0
I	1019	663 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-K, B-M, F-I.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 25	-70.5	-70.5	0.16 (1)	10.00
B-C	-1200 / 0	-70.5	-70.5	0.16 (1)	5.69
C-D	-933 / 0	-78.0	-78.0	0.48 (1)	5.73
D-E	-933 / 0	-78.0	-78.0	0.48 (1)	5.73
E-F	-1200 / 0	-70.5	-70.5	0.16 (1)	5.69
F-G	0 / 25	-70.5	-70.5	0.16 (1)	10.00
M-A	-103 / 0	0.0	0.0	0.01 (1)	7.81
I-G	-103 / 0	0.0	0.0	0.01 (1)	7.81
N-M	0 / 0	-88.0	-88.0	0.10 (1)	10.00
M-L	0 / 779	-17.5	-17.5	0.31 (4)	10.00
L-K	0 / 704	-17.5	-17.5	0.31 (4)	10.00
K-J	0 / 704	-17.5	-17.5	0.31 (4)	10.00
J-I	0 / 779	-17.5	-17.5	0.31 (4)	10.00
I-H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-L	-135 / 0	0.13 (1)
L-C	0 / 259	0.04 (1)
C-K	0 / 436	0.07 (1)
K-D	-644 / 0	0.43 (1)
K-E	0 / 436	0.07 (1)
J-E	0 / 259	0.04 (1)
J-F	-135 / 0	0.13 (1)
M-B	-1403 / 0	0.45 (1)
F-I	-1403 / 0	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.48 (C-D:1) , BC=0.31 (J-K:4) , WB=0.45 (F-I:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

A-H073679

253442	H05	2	1	TRUSS DESC.

Alpa Roof Truss, Maple

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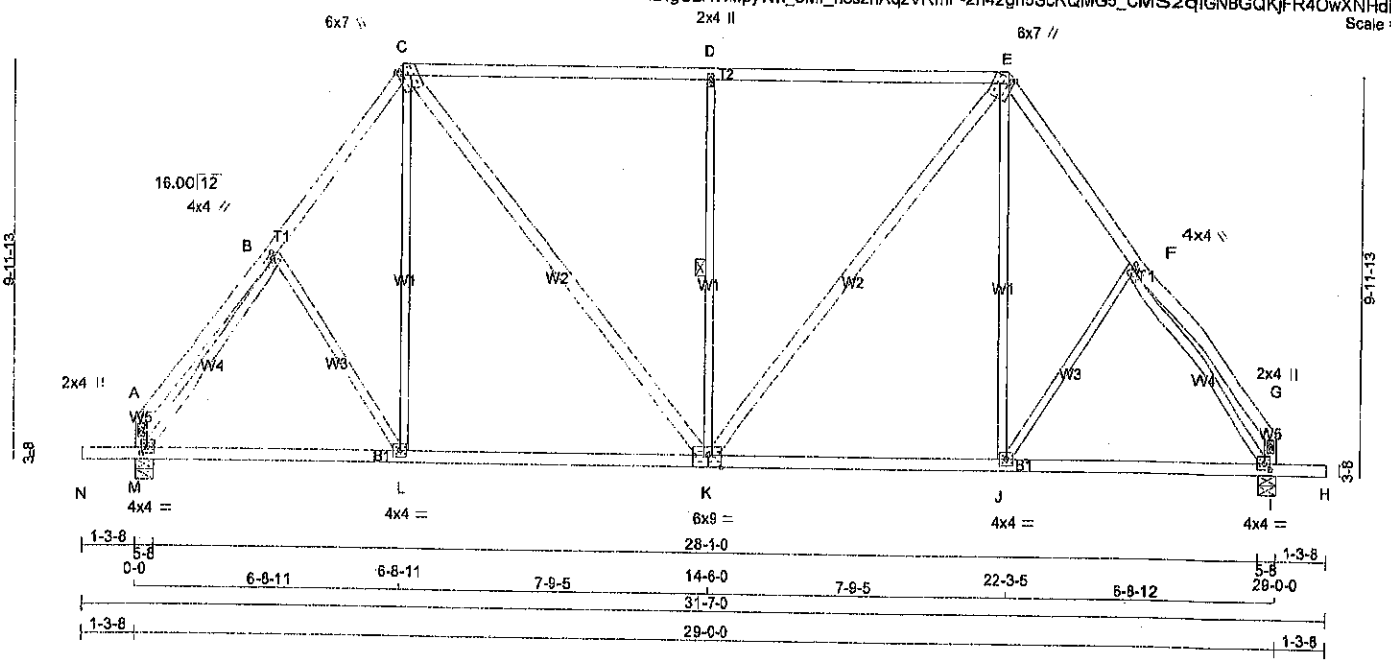


TOTAL WEIGHT = 2 X 151 = 302 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

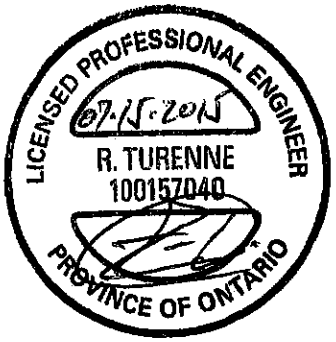
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.66	1.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMWW-t	MT20	4.0	4.0	1.50	1.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	1.75	1.75
J	BMWW-t	MT20	4.0	4.0		
K	BSWWW-t	MT20	6.0	9.0	Edge	4.60
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.33FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 21	-70.5	-70.5	0.12 (1)	10.00
B- C	-1179 / 0	-70.5	-70.5	0.12 (1)	5.77
C- D	-1016 / 0	-70.5	-70.5	0.59 (1)	5.33
D- E	-1016 / 0	-70.5	-70.5	0.59 (1)	5.33
E- F	-1179 / 0	-70.5	-70.5	0.12 (1)	5.77
F- G	0 / 21	-70.5	-70.5	0.12 (1)	10.00
M- A	-91 / 0	0.0	0.0	0.01 (1)	7.81
I- G	-91 / 0	0.0	0.0	0.01 (1)	7.81
N- M	0 / 0	-88.0	-88.0	0.10 (1)	10.00
M- L	0 / 747	-17.5	-17.5	0.27 (4)	10.00
L- K	0 / 893	-17.5	-17.5	0.27 (4)	10.00
K- J	0 / 893	-17.5	-17.5	0.27 (4)	10.00
J- I	0 / 747	-17.5	-17.5	0.27 (4)	10.00
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- L	-98 / 4	0.07 (1)
L- C	0 / 222	0.05 (4)
C- K	0 / 515	0.08 (1)
K- D	-670 / 0	0.45 (1)
K- E	0 / 515	0.08 (1)
J- E	0 / 222	0.05 (4)
J- F	-98 / 4	0.07 (1)
M- B	-1361 / 0	0.86 (1)
F- I	-1361 / 0	0.86 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(56 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.59 (C-D:1) , BC=0.27 (J-K:4) , WB=0.86 (B-M:1) , SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.52 (F) (INPUT = 1.00)

A-N073678

253442	H04	2	1	TRUSS DESC.

Alga Roof Truss, Maple

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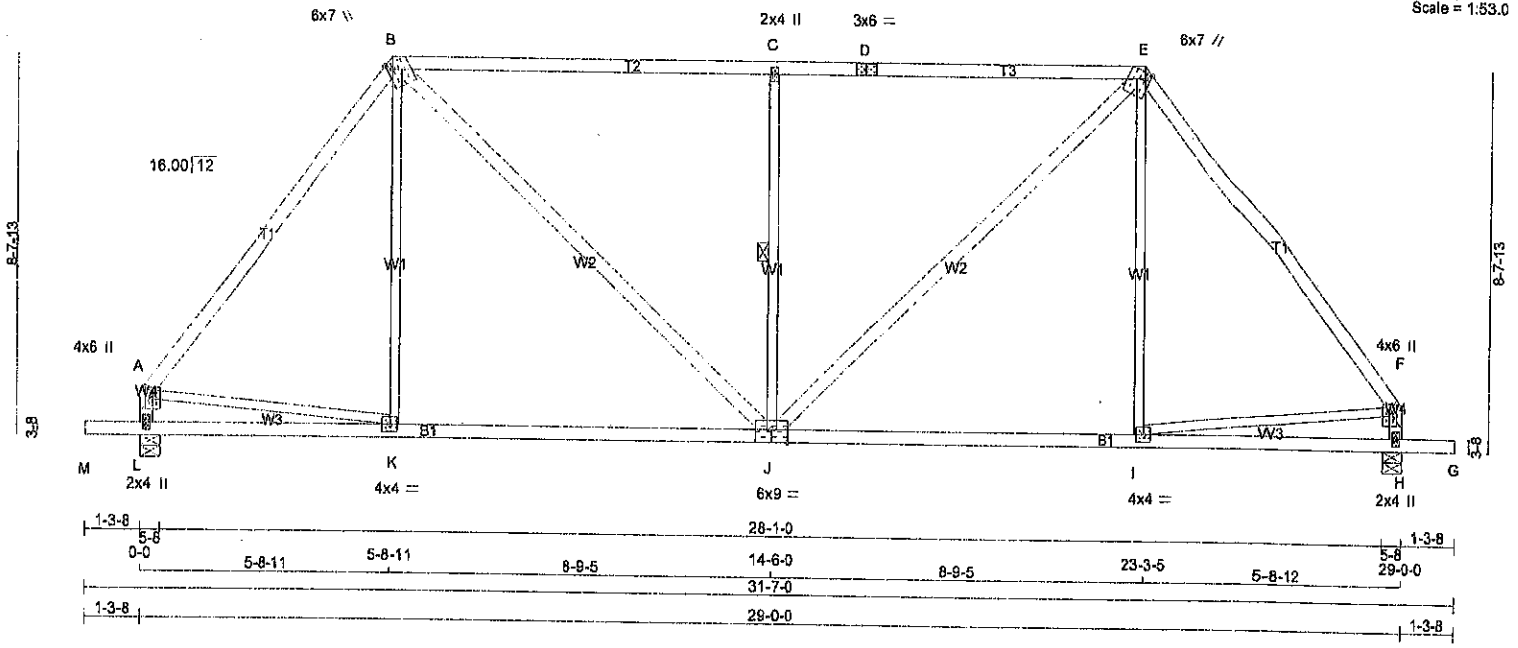

TOTAL WEIGHT = 2 X 135 = 271 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	1650F 1.5E	SPF
D - E	2x4	DRY	1650F 1.5E	SPF
E - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BSWWW-I	MT20	6.0	9.0	Edge	4.50
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1389	0	1389	0	0	5-8	1-8
H	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
H	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1199 / 0	-70.5	-70.5	0.57 (1)	5.24	
B- C	-1198 / 0	-70.5	-70.5	0.90 (1)	4.79	
C- D	-1200 / 0	-70.5	-70.5	0.90 (1)	4.79	
D- E	-1200 / 0	-70.5	-70.5	0.90 (1)	4.79	
E- F	-1199 / 0	-70.5	-70.5	0.57 (1)	5.24	
L- A	-1240 / 0	0.0	0.0	0.12 (1)	7.21	
H- F	-1240 / 0	0.0	0.0	0.12 (1)	7.21	
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00	
L- K	0 / 0	-17.5	-17.5	0.21 (4)	10.00	
K- J	0 / 717	-17.5	-17.5	0.33 (4)	10.00	
J- I	0 / 717	-17.5	-17.5	0.33 (4)	10.00	
I- H	0 / 0	-17.5	-17.5	0.21 (4)	10.00	
H- G	0 / 0	-88.0	-88.0	0.10 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	0 / 131	0.05 (4)
B- J	0 / 664	0.11 (1)
J- C	-756 / 0	0.36 (1)
J- E	0 / 664	0.11 (1)
I- E	0 / 131	0.05 (4)
A- K	0 / 720	0.16 (1)
I- F	0 / 720	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.90 (C-E:1) , BC=0.33 (I-J:4) , WB=0.36 (C-J:1) , SSI=0.30 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (I) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)

A-1507 3677

253442	H03	2	1	TRUSS DESC.

Alpa Roof Trusses Maple

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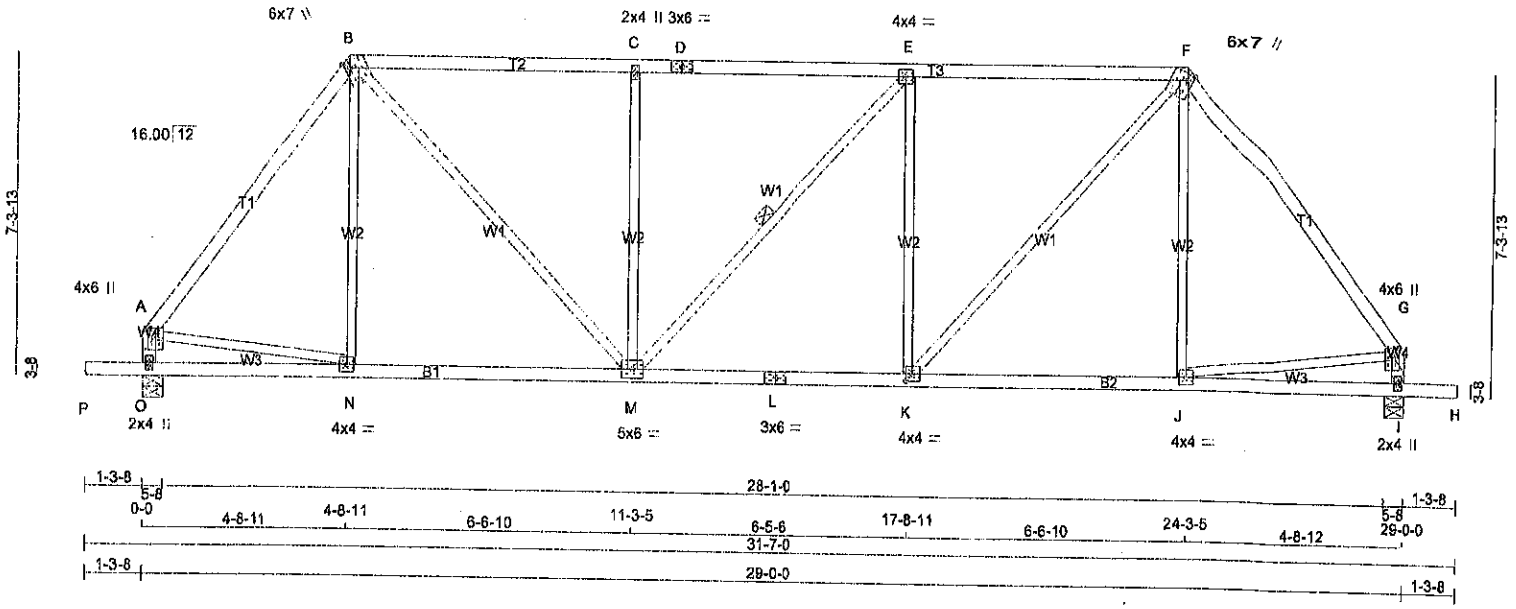


TOTAL WEIGHT = 2 X 130 = 261 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1225 / 0	-70.5	-70.5	0.36 (1)	5.41
B- C	-1299 / 0	-70.5	-70.5	0.47 (1)	5.07
C- D	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06
D- E	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06
E- F	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06
F- G	-1225 / 0	-70.5	-70.5	0.36 (1)	5.41
O- A	-1245 / 0	0.0	0.0	0.13 (1)	7.20
I- G	-1244 / 0	0.0	0.0	0.13 (1)	7.20
P- O	0 / 0	-88.0	-88.0	0.10 (1)	10.00
O- N	0 / 0	-17.5	-17.5	0.13 (4)	10.00
N- M	0 / 732	-17.5	-17.5	0.20 (4)	10.00
M- L	0 / 1301	-17.5	-17.5	0.29 (1)	10.00
L- K	0 / 1301	-17.5	-17.5	0.29 (1)	10.00
K- J	0 / 732	-17.5	-17.5	0.20 (4)	10.00
J- I	0 / 0	-17.5	-17.5	0.13 (4)	10.00
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	0 / 92	0.03 (4)
B- M	0 / 830	0.19 (1)
M- C	-492 / 0	0.47 (1)
M- E	-1 / 0	0.00 (1)
K- E	-492 / 0	0.47 (1)
K- F	0 / 831	0.19 (1)
J- F	0 / 91	0.03 (4)
A- N	0 / 736	0.17 (1)
J- G	0 / 738	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
6.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.48 (E-F:1) , BC=0.29 (K-M:1) , WB=0.47 (E-K:1) , SSI=0.21 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (M) (INPUT = 0.90)
JSI METAL= 0.40 (L) (INPUT = 1.00)

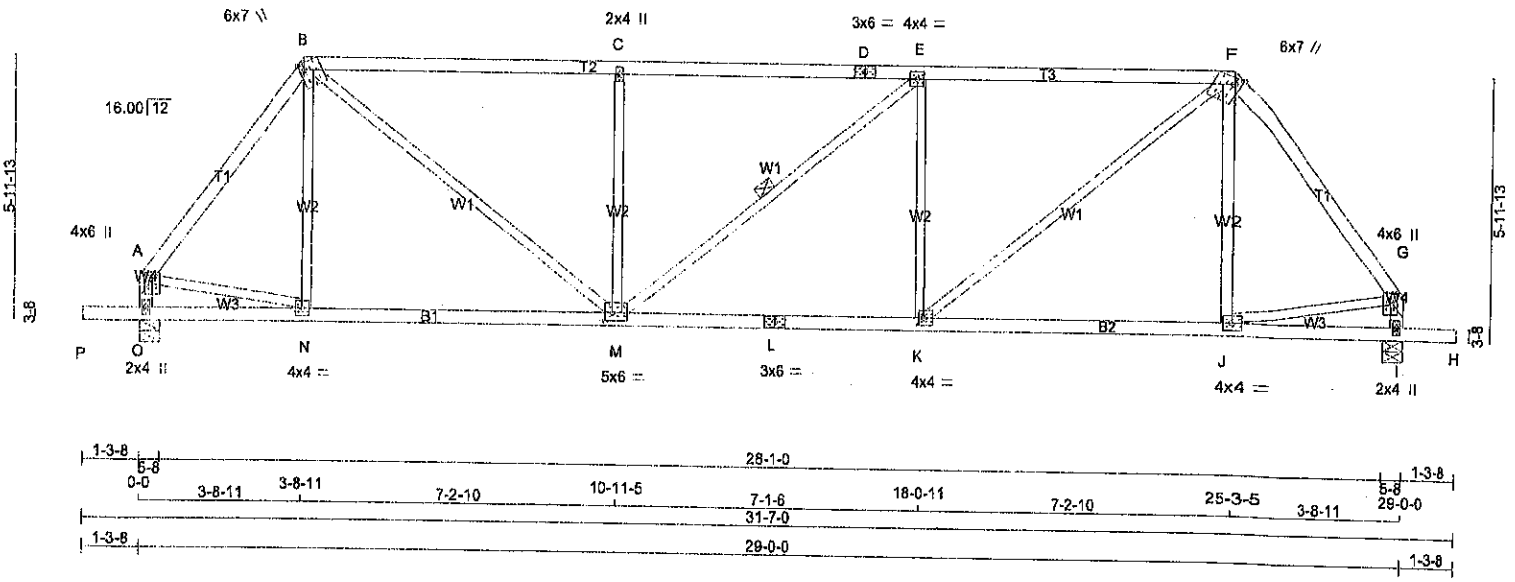
A-H073676

253442	H02	2	1	TRUSS DESC.

Alpa Roof Truss, Maple

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-6JzHF53B4qAf1nqpF1?Mdrlr_dd1ngF5SDuAZOyxoSn

Scale = 1:53.0



TOTAL WEIGHT = 2 X 122 = 244 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0	2.25	2.00
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1251 / 0	-70.5	-70.5	0.21 (1)	5.53	
B- C	-1600 / 0	-70.5	-70.5	0.63 (1)	4.48	
C- D	-1600 / 0	-70.5	-70.5	0.63 (1)	4.47	
D- E	-1600 / 0	-70.5	-70.5	0.63 (1)	4.47	
E- F	-1601 / 0	-70.5	-70.5	0.64 (1)	4.47	
F- G	-1250 / 0	-70.5	-70.5	0.21 (1)	5.53	
O- A	-1257 / 0	0.0	0.0	0.13 (1)	7.17	
I- G	-1257 / 0	0.0	0.0	0.13 (1)	7.17	
P- O	0 / 0	-88.0	-88.0	0.10 (1)	10.00	
O- N	0 / 0	-17.5	-17.5	0.14 (4)	10.00	
N- M	0 / 746	-17.5	-17.5	0.24 (4)	10.00	
M- L	0 / 1601	-17.5	-17.5	0.35 (1)	10.00	
L- K	0 / 1601	-17.5	-17.5	0.35 (1)	10.00	
K- J	0 / 745	-17.5	-17.5	0.24 (4)	10.00	
J- I	0 / 0	-17.5	-17.5	0.14 (4)	10.00	
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-12 / 81	0.03 (4)
B- M	0 / 1085	0.24 (1)
M- C	-542 / 0	0.30 (1)
M- E	-1 / 0	0.00 (1)
K- E	-543 / 0	0.30 (1)
K- F	0 / 1086	0.24 (1)
J- F	-12 / 81	0.03 (4)
A- N	0 / 756	0.17 (1)
J- G	0 / 756	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.64 (E-F:1) , BC=0.35 (K-M:1) , WB=0.30 (E-K:1) , SSI=0.24 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.49 (L) (INPUT = 1.00)

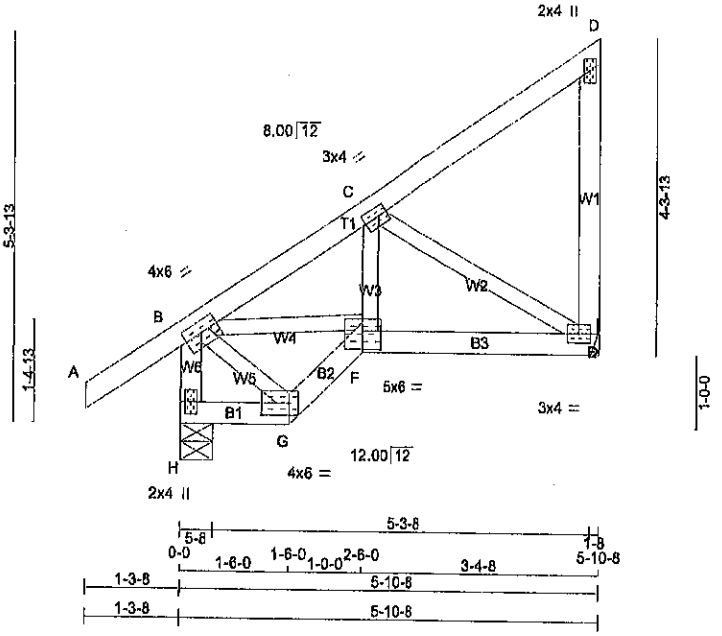
A-4073675

253433	J01C	4	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-v4DgXX6M5wDUmXAE6wiUUtg?bDXooC_unwoBvXyxuN1

Scale: 3/8"=1'



TOTAL WEIGHT = 4 X 30 = 119 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
H-B	-342 / 0	0.0	0.0	0.04 (1)	7.81	B-G	0 / 18	0.01 (4)
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B-F	0 / 238	0.05 (1)
B-C	-267 / 0	-70.5	-70.5	0.10 (1)	6.25	F-C	0 / 57	0.02 (4)
C-D	-12 / 0	-70.5	-70.5	0.10 (1)	6.25	C-E	-284 / 0	0.07 (1)
E-D	-93 / 0	0.0	0.0	0.03 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.01 (4)	10.00			
G-F	0 / 20	-17.5	-17.5	0.01 (4)	10.00			
F-E	0 / 248	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.08 (E-F:4) , WB=0.07 (C-E:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

A-H073630

253433	J01B	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-v4DgXX6M5wDUmXAE6wiUUtg?bDXooC_urwoBvXyxuN1

2x4 ||

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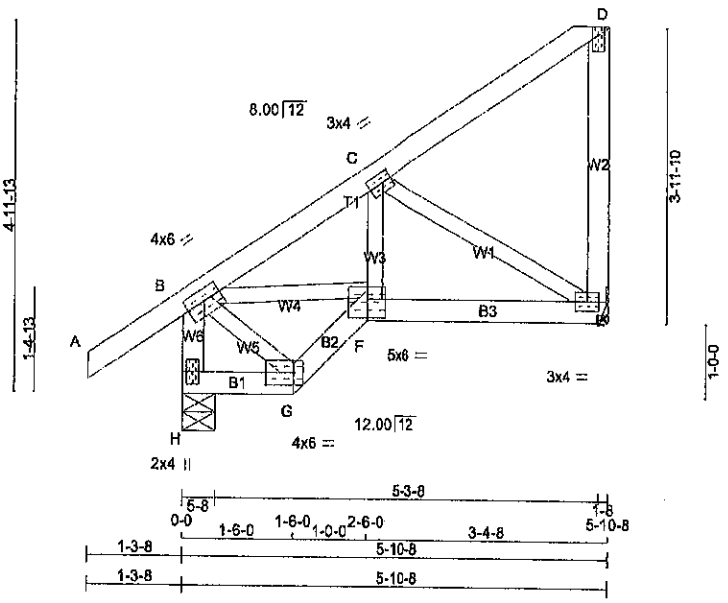

TOTAL WEIGHT = 30 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0	Edge	
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-6	1-8
E	268	0	268	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
H-B	-342 / 0	0.0	0.0	4 (1)	0.04 (1)	7.81
A-B	0 / 27	-70.5	-70.5	1 (1)	0.09 (1)	10.00
B-C	-287 / 0	-70.5	-70.5	1 (1)	0.10 (1)	6.25
C-D	-12 / 0	-70.5	-70.5	1 (1)	0.10 (1)	6.25
E-D	-93 / 0	0.0	0.0	4 (1)	0.03 (1)	7.81
H-G	0 / 0	-17.5	-17.5	4 (1)	0.01 (4)	10.00
G-F	0 / 20	-17.5	-17.5	4 (1)	0.01 (4)	10.00
F-E	0 / 248	-17.5	-17.5	4 (1)	0.08 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-G	0 / 18	0.01 (4)
B-F	0 / 238	0.05 (1)
F-C	0 / 57	0.02 (4)
C-E	-284 / 0	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.08 (E-F:4) , WB=0.07 (C-E:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

A-1073629

253433	J01A	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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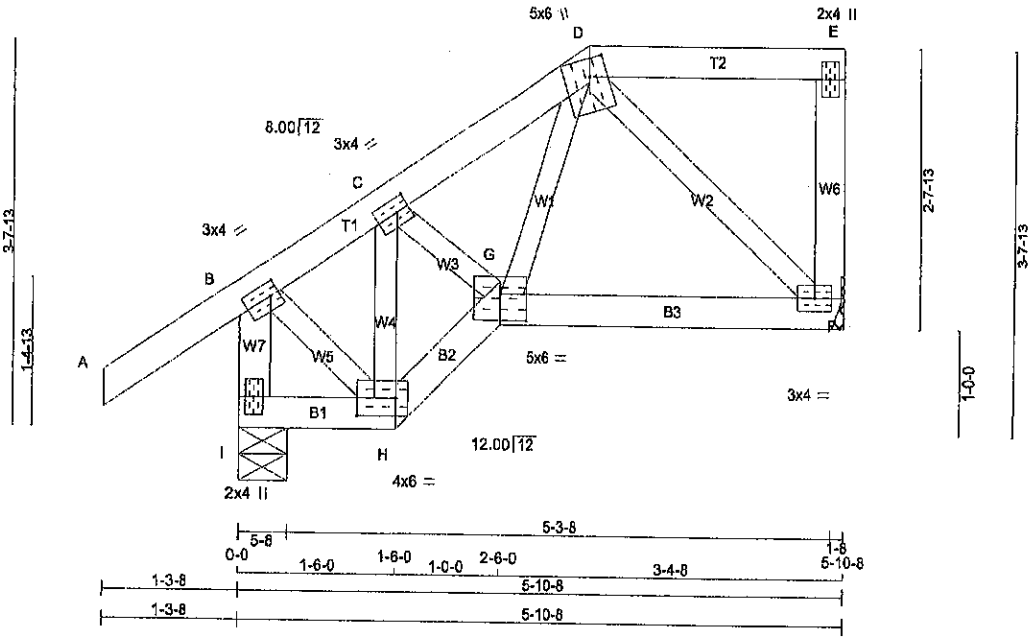

TOTAL WEIGHT = 28 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	6.0	6.0	2.00	2.00
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BBWW-t	MT20	5.0	6.0	3.00	3.00
H	BBWW-t	MT20	4.0	6.0	2.00	4.50
I	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	247	0	247	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	366	0	366	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	174	117 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
I	256	186 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
B-C	-168 / 0	-70.5	-70.5	0.09 (1)	6.25	
C-D	-213 / 0	-70.5	-70.5	0.04 (1)	6.25	
D-E	0 / 0	-70.5	-70.5	0.07 (1)	10.00	
F-E	-68 / 0	0.0	0.0	0.01 (1)	7.81	
I-B	-353 / 0	0.0	0.0	0.04 (1)	7.81	
I-H	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
H-G	0 / 157	-17.5	-17.5	0.03 (1)	10.00	
G-F	0 / 139	-17.5	-17.5	0.07 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-174 / 0	0.03 (1)
C-G	0 / 85	0.02 (1)
G-D	0 / 101	0.02 (1)
D-F	-191 / 0	0.04 (1)
B-H	0 / 147	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.09 (A-B:1) , BC=0.07 (F-G:4) , WB=0.04 (D-F:1) , SSI=0.07 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073628

253432	H05	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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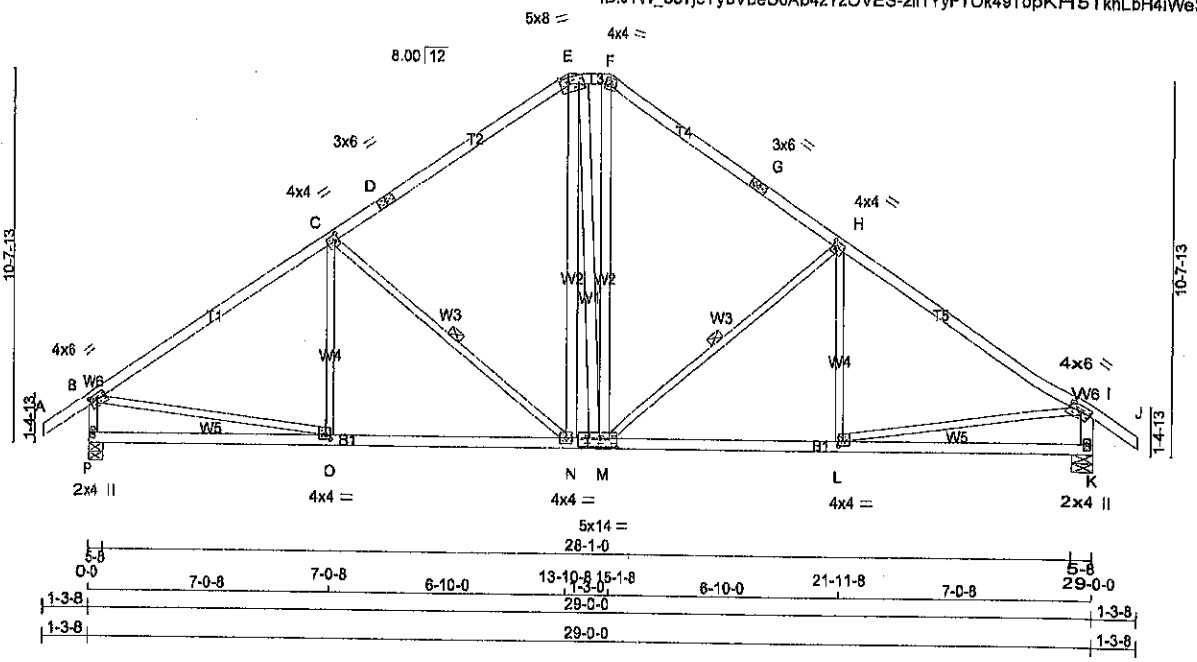

TOTAL WEIGHT = 147 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - E	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.75
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BSWWW-I	MT20	5.0	14.0	3.00	6.50
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1372	0	1372	0	0	5-8	1-8
K	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
K	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-N, H-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS FR-TO	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B- C	-1487 / 0	-70.5	-70.5	0.53 (1)		4.77
C- D	-1105 / 0	-70.5	-70.5	0.48 (1)		5.40
D- E	-1105 / 0	-70.5	-70.5	0.48 (1)		5.40
E- F	-899 / 0	-70.5	-70.5	0.02 (1)		6.25
F- G	-1106 / 0	-70.5	-70.5	0.49 (1)		5.39
G- H	-1106 / 0	-70.5	-70.5	0.49 (1)		5.39
H- I	-1486 / 0	-70.5	-70.5	0.53 (1)		4.77
I- J	0 / 27	-70.5	-70.5	0.09 (1)		10.00
P- B	-1322 / 0	0.0	0.0	0.14 (1)		7.03
K- I	-1322 / 0	0.0	0.0	0.14 (1)		7.03
P- O	0 / 0	-17.5	-17.5	0.22 (4)		10.00
O- N	0 / 1264	-17.5	-17.5	0.32 (1)		10.00
N- M	0 / 896	-17.5	-17.5	0.23 (1)		10.00
M- L	0 / 1263	-17.5	-17.5	0.31 (1)		10.00
L- K	0 / 0	-17.5	-17.5	0.21 (4)		10.00

WEBS FR-TO	MAX. FACTORED MEMB. FORCE (LBS)	MAX CSI
O- C	-53 / 98	0.03 (4)
C- N	-464 / 0	0.27 (1)
N- E	0 / 343	0.08 (1)
E- M	0 / 29	0.00 (1)
M- F	0 / 372	0.06 (1)
M- H	-481 / 0	0.27 (1)
L- H	-57 / 96	0.03 (1)
B- O	0 / 1279	0.29 (1)
L- I	0 / 1278	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.53 (H-I:1) , BC=0.32 (N-O:1) , WB=0.29 (B-O:1) , SSI=0.20 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (O) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

A-H073611

253432	H04	1	1	TRUSS DESC.	DRAWING:

Alpa Roof Truss, Maple

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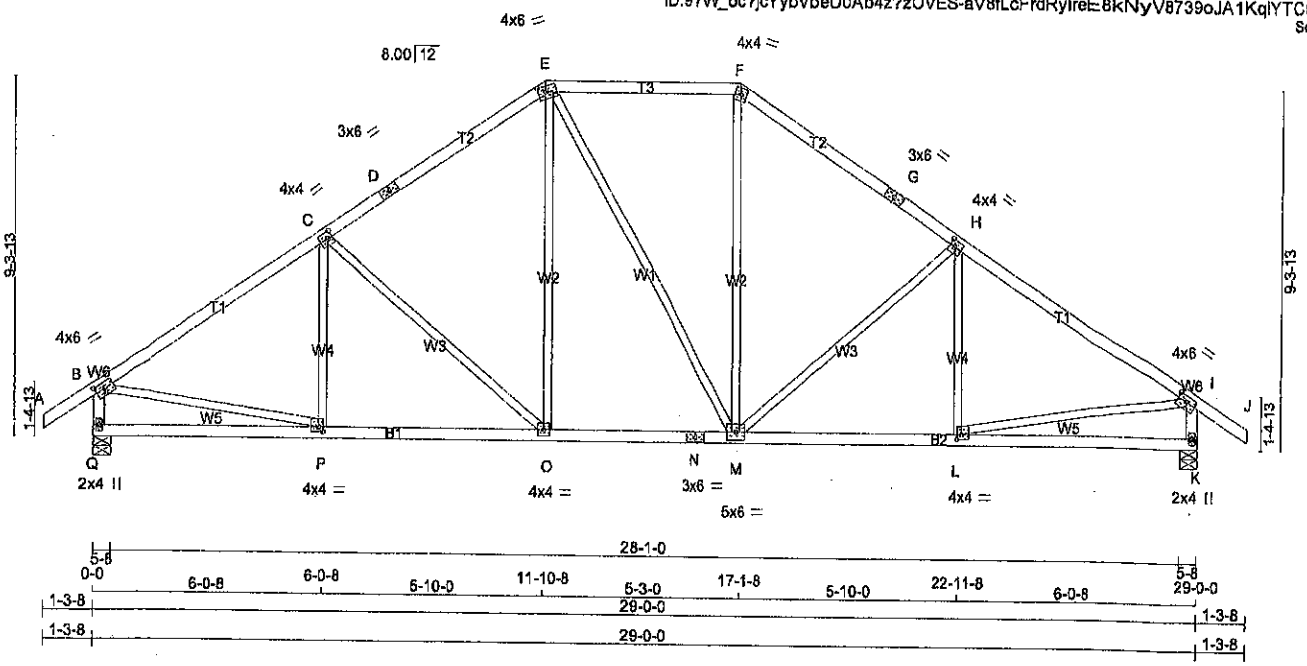

TOTAL WEIGHT = 130 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BMWWW-t	MT20,	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1372	0	1372	0	0	5-8	1-8
K	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
K	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-1492 / 0	-70.5	-70.5	0.38 (1)	4.97
C- D	-1224 / 0	-70.5	-70.5	0.35 (1)	5.40
D- E	-1224 / 0	-70.5	-70.5	0.35 (1)	5.40
E- F	-998 / 0	-70.5	-70.5	0.26 (1)	5.94
F- G	-1224 / 0	-70.5	-70.5	0.35 (1)	5.40
G- H	-1224 / 0	-70.5	-70.5	0.35 (1)	5.40
H- I	-1491 / 0	-70.5	-70.5	0.38 (1)	4.97
I- J	0 / 27	-70.5	-70.5	0.09 (1)	10.00
Q- B	-1328 / 0	0.0	0.0	0.14 (1)	7.02
K- I	-1328 / 0	0.0	0.0	0.14 (1)	7.02
Q- P	0 / 0	-17.5	-17.5	0.15 (4)	10.00
P- O	0 / 1264	-17.5	-17.5	0.28 (1)	10.00
O- N	0 / 997	-17.5	-17.5	0.22 (1)	10.00
N- M	0 / 997	-17.5	-17.5	0.22 (1)	10.00
M- L	0 / 1264	-17.5	-17.5	0.28 (1)	10.00
L- K	0 / 0	-17.5	-17.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-109 / 63	0.05 (1)
C- O	-357 / 0	0.45 (1)
O- E	0 / 325	0.07 (1)
E- M	0 / 1	0.00 (1)
M- F	0 / 326	0.07 (1)
M- H	-356 / 0	0.45 (1)
L- H	-110 / 62	0.05 (1)
B- P	0 / 1284	0.29 (1)
L- I	0 / 1284	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.38 (B-C:1) , BC=0.28 (O-P:1) , WB=0.45 (C-O:1) , SSI=0.17 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (P) (INPUT = 0.90)
JSI METAL= 0.47 (P) (INPUT = 1.00)

A-15073610

253432	H03	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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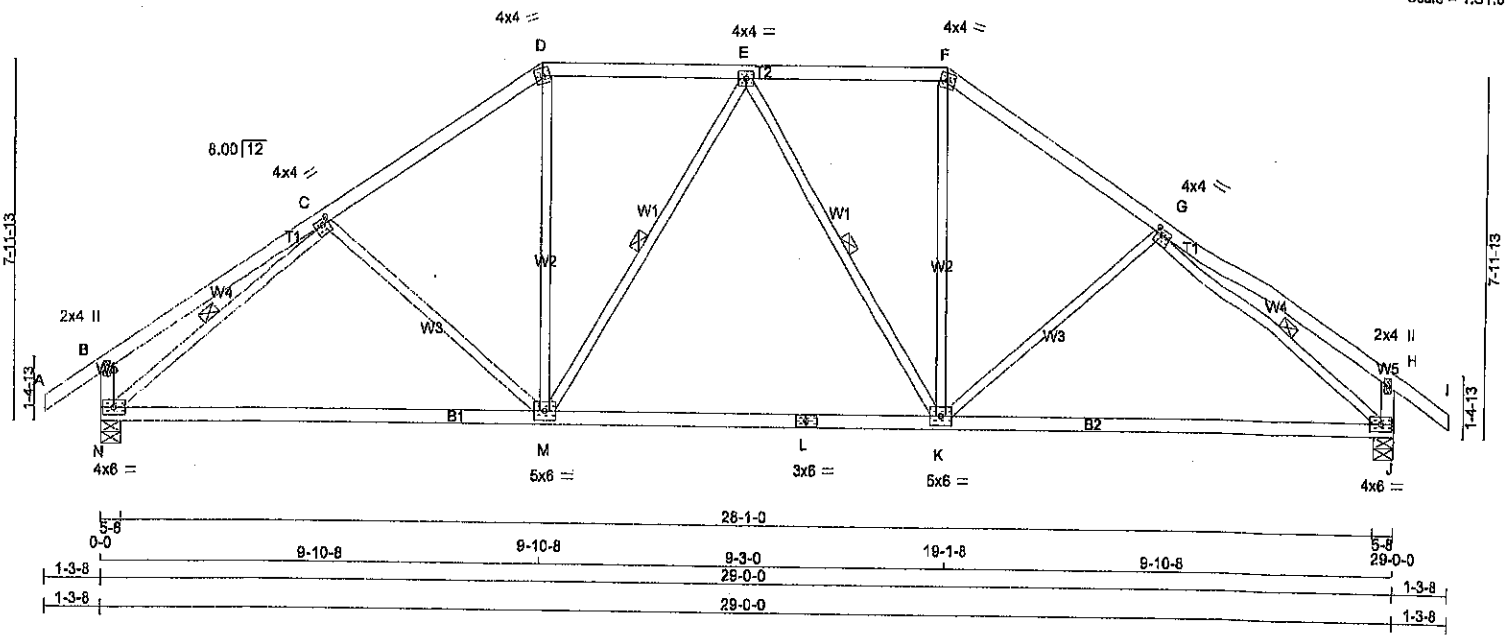

TOTAL WEIGHT = 123 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	1.50	1.75
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1372	0	1372	0	0	5-8	1-8
J	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M, E-K, C-N, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	0 / 22	-70.5	-70.5	0.27 (1)	10.00
C- D	-1345 / 0	-70.5	-70.5	0.24 (1)	5.36
D- E	-1106 / 0	-70.5	-70.5	0.20 (1)	6.80
E- F	-1106 / 0	-70.5	-70.5	0.20 (1)	6.80
F- G	-1345 / 0	-70.5	-70.5	0.24 (1)	5.36
G- H	0 / 22	-70.5	-70.5	0.27 (1)	10.00
H- I	0 / 27	-70.5	-70.5	0.09 (1)	10.00
N- B	-230 / 0	0.0	0.0	0.02 (1)	7.81
J- H	-230 / 0	0.0	0.0	0.02 (1)	7.81
N- M	0 / 1206	-17.5	-17.5	0.46 (4)	10.00
M- L	0 / 1222	-17.5	-17.5	0.47 (4)	10.00
L- K	0 / 1222	-17.5	-17.5	0.47 (4)	10.00
K- J	0 / 1208	-17.5	-17.5	0.46 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-144 / 22	0.12 (1)
M- D	0 / 474	0.11 (1)
M- E	-229 / 0	0.13 (1)
E- K	-229 / 0	0.13 (1)
K- F	0 / 474	0.11 (1)
K- G	-144 / 22	0.12 (1)
N- C	-1614 / 0	0.48 (1)
G- J	-1614 / 0	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.27 (B-C:1) , BC=0.47 (K-M:4) , WB=0.46 (C-N:1) , SSI=0.16 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (N) (INPUT = 0.80)
JSI METAL= 0.58 (G) (INPUT = 1.00)

A-4073609

253432	H02	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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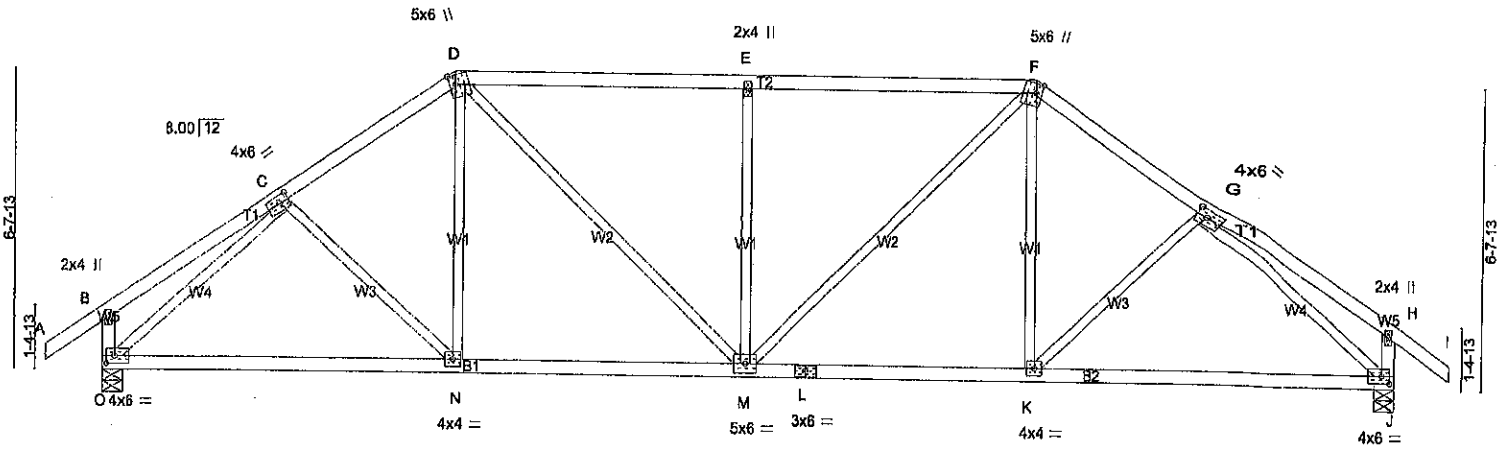

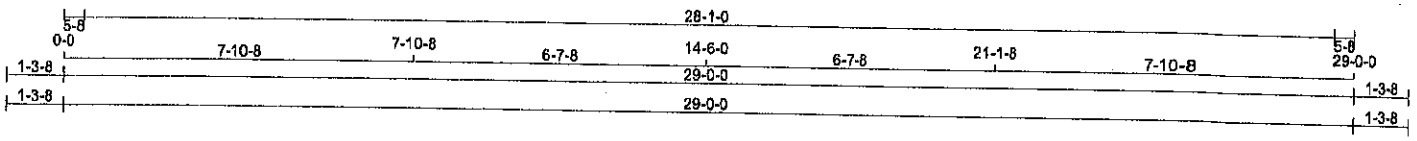

TOTAL WEIGHT = 122 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.60
G	TMWW-t	MT20	4.0	6.0	2.00	2.50
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	6.0	2.00	2.25
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	6.0	2.00	2.25

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1372	0	1372	0	0	5-8	1-8
J	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	964	686 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	964	686 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B-C	0 / 17	-70.5	-70.5	0.16 (1)		10.00
C-D	-1423 / 0	-70.5	-70.5	0.15 (1)		5.33
D-E	-1518 / 0	-70.5	-70.5	0.44 (1)		4.83
E-F	-1518 / 0	-70.5	-70.5	0.44 (1)		4.83
F-G	-1423 / 0	-70.5	-70.5	0.15 (1)		5.33
G-H	0 / 17	-70.5	-70.5	0.16 (1)		10.00
H-I	0 / 27	-70.5	-70.5	0.09 (1)		10.00
O-B	-205 / 0	0.0	0.0	0.02 (1)		7.81
J-H	-205 / 0	0.0	0.0	0.02 (1)		7.81
O-N	0 / 1179	-17.5	-17.5	0.34 (4)		10.00
N-M	0 / 1171	-17.5	-17.5	0.35 (4)		10.00
M-L	0 / 1171	-17.5	-17.5	0.35 (4)		10.00
L-K	0 / 1171	-17.5	-17.5	0.35 (4)		10.00
K-J	0 / 1179	-17.5	-17.5	0.34 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-N	-15 / 48	0.02 (4)
N-D	0 / 161	0.05 (4)
D-M	0 / 481	0.11 (1)
M-E	-573 / 0	0.42 (1)
M-F	0 / 481	0.11 (1)
K-F	0 / 161	0.05 (4)
K-G	-15 / 48	0.02 (4)
O-C	-1619 / 0	0.77 (1)
G-J	-1619 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.44 (D-E:1) , BC=0.35 (M-N:4) , WB=0.77 (C-O:1) , SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (O) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

A-2073608

253432	H01	1	1	TRUSS DESC.

Alpa Roof Truss, Maple

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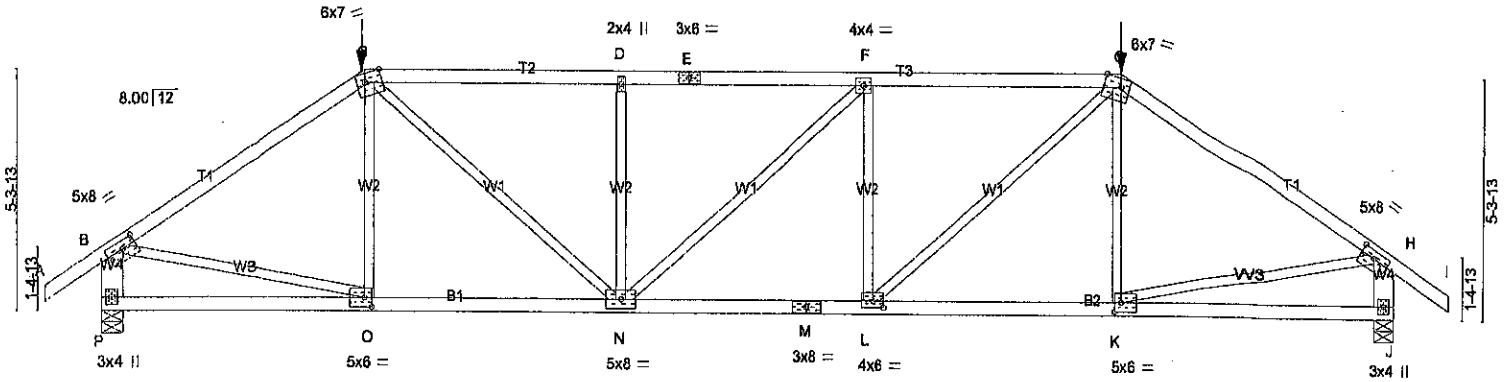
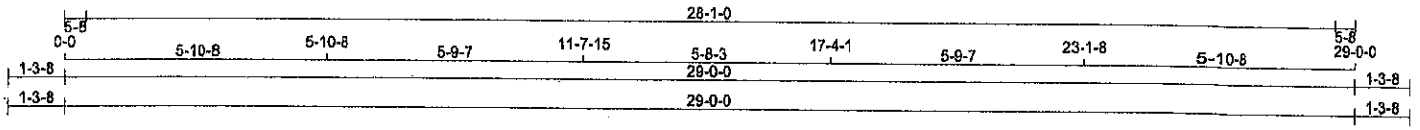
Scale = 1:51.6





TOTAL WEIGHT = 119 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	1650F 1.6E	SPF
E - G	2x4	DRY	1650F 1.6E	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.00	3.75
C	TTWW-m	MT20	6.0	7.0	Edge	4.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	6.0	7.0	Edge	4.75
H	TMWW-t	MT20	5.0	8.0	2.00	3.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	8.0	2.50	2.00
L	BMWW-t	MT20	4.0	8.0	1.75	3.00
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	6.0	8.0	2.50	2.00
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2508	0	2508	0	0	5-8	2-11
J	2508	0	2508	0	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1767	1197 / 0	0 / 0	0 / 0	0 / 0	571 / 0	0 / 0
J	1767	1197 / 0	0 / 0	0 / 0	0 / 0	571 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.10 (1)	10.00
B-C	-3003 / 0	-70.5	-70.5	0.87 (1)	3.25
C-D	-3700 / 0	-138.7	-138.7	0.78 (1)	3.31
D-E	-3700 / 0	-138.7	-138.7	0.78 (1)	3.30
E-F	-3700 / 0	-138.7	-138.7	0.78 (1)	3.30
F-G	-3702 / 0	-138.7	-138.7	0.79 (1)	3.30
G-H	-3003 / 0	-70.5	-70.5	0.87 (1)	3.25
H-I	0 / 27	-70.5	-70.5	0.10 (1)	10.00
P-B	-2426 / 0	0.0	0.0	0.18 (1)	6.58
J-H	-2425 / 0	0.0	0.0	0.18 (1)	6.58
P-O	0 / 0	-34.5	-34.5	0.32 (4)	10.00
O-N	0 / 2492	-34.5	-34.5	0.62 (1)	10.00
N-M	0 / 3702	-34.5	-34.5	0.76 (1)	10.00
M-L	0 / 3702	-34.5	-34.5	0.76 (1)	10.00
L-K	0 / 2491	-34.5	-34.5	0.61 (1)	10.00
K-J	0 / 0	-34.5	-34.5	0.31 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-222 / 113	0.10 (1)
C-N	0 / 1589	0.40 (1)
N-D	-860 / 0	0.37 (1)
N-F	-3 / 0	0.00 (1)
L-F	-860 / 0	0.37 (1)
L-G	0 / 1602	0.40 (1)
K-G	-223 / 113	0.10 (1)
B-O	0 / 2536	0.63 (1)
K-H	0 / 2536	0.63 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-302	-302	---	FRONT	VERT	TOTAL
G	23-1-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.87 (B-C:1) , BC=0.76 (L-N:1) , WB=0.63 (B-O:1) , SSI=0.41 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (O) (INPUT = 0.90)
JSI METAL= 0.99 (M) (INPUT = 1.00)

A-15073607

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 23-1-8, AND 301.8 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

A-150736072

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

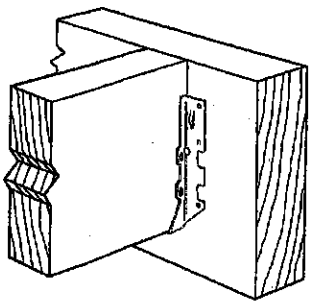
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

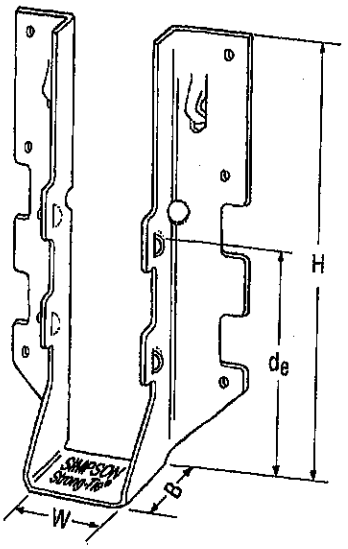
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



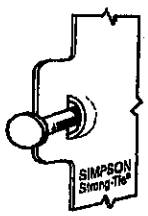
Typical LUS Installation



LUS28

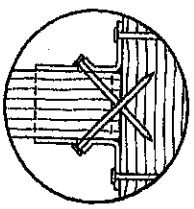
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

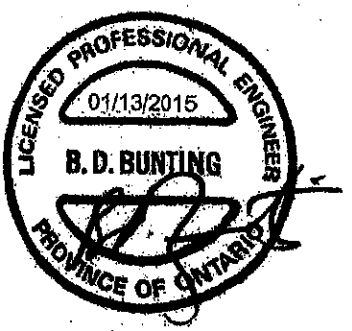


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

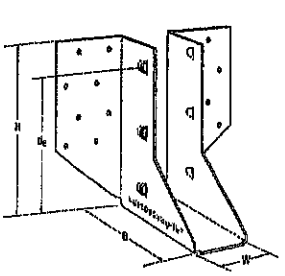
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

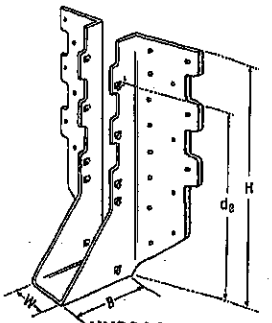
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

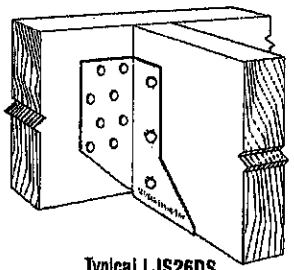
- See current catalogue for options



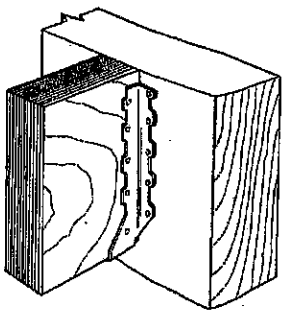
LJS26DS



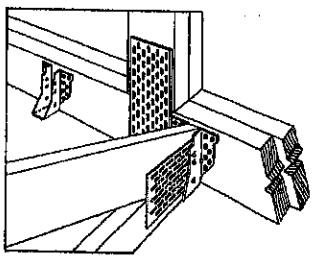
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



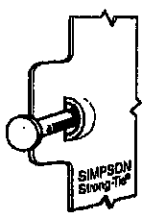
Typical HUS Installation



Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

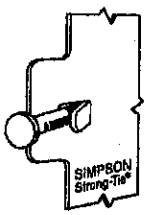
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅝	3	3¹⁵⁄₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7⁹⁄₃₂	3	6⁹⁄₃₂	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

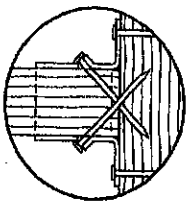


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

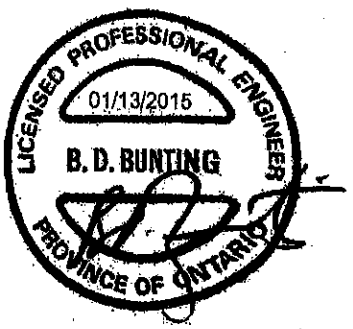
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

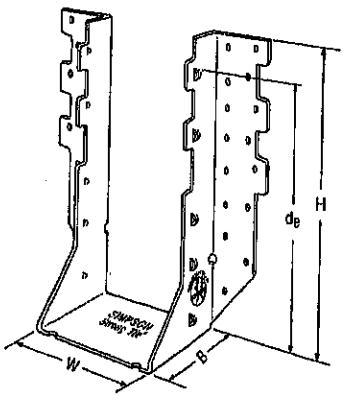
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

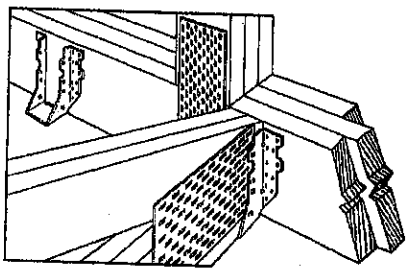
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



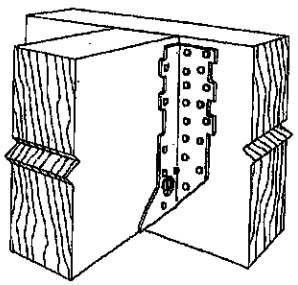
HHUS410



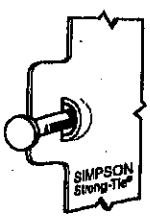
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 1/32	3	6 5/32	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	30-16d	10-16d	4745	9860	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 13/32	3	3 15/16	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3 5/8	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	30-16d	10-16d	4745	10770	4310	7650

1. d_e is the distance from the seat of the hanger to the highest joist nail.

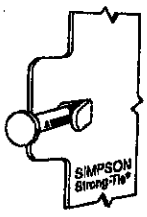


Typical HHUS Installation

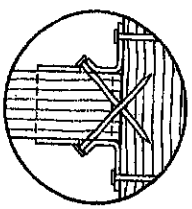


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

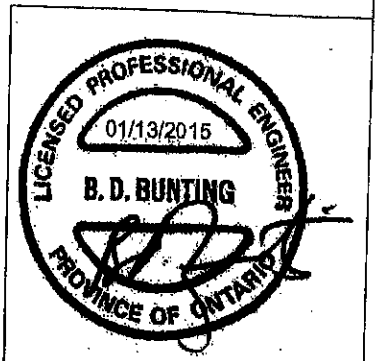
U.S. Patent 5,603,580

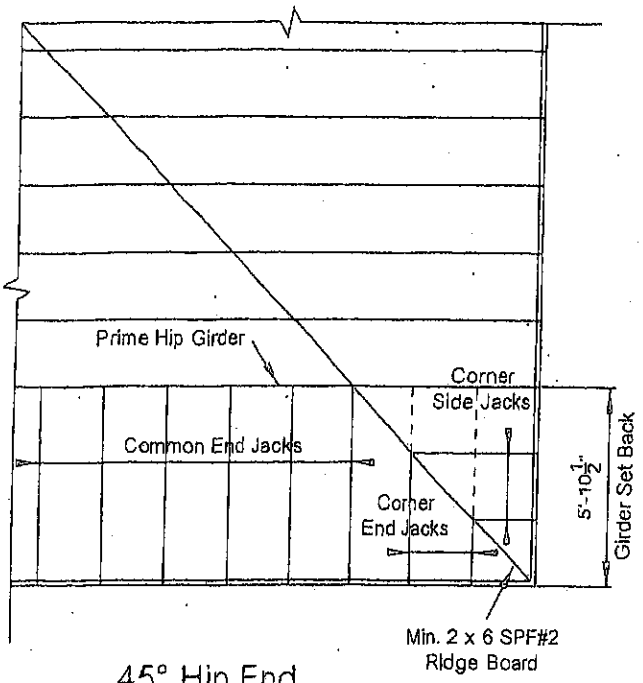


Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.





45° Hip End

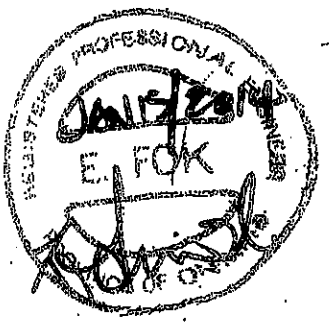
LUMBER SPECIFICATION

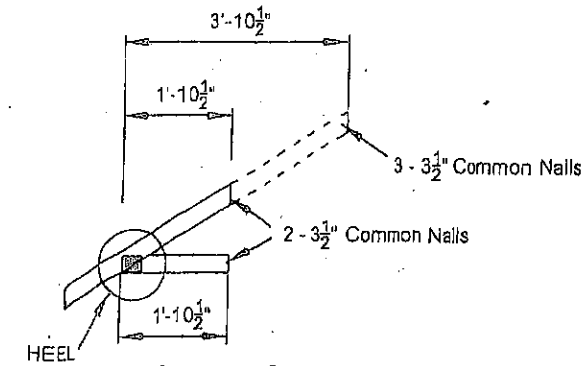
TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

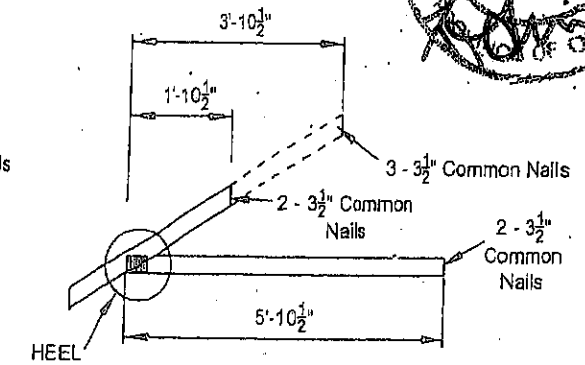
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

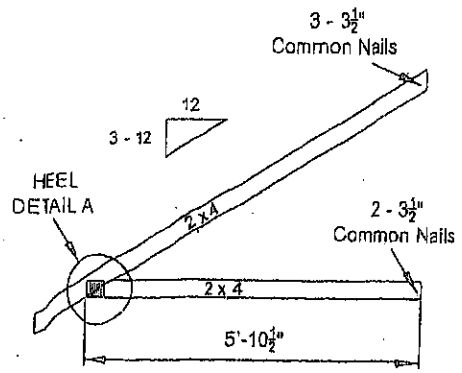




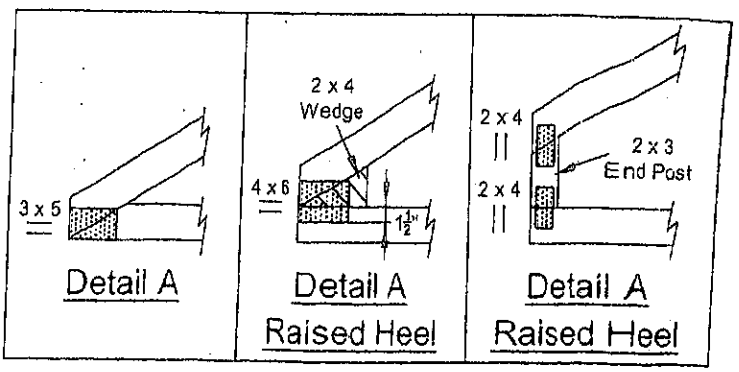
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008