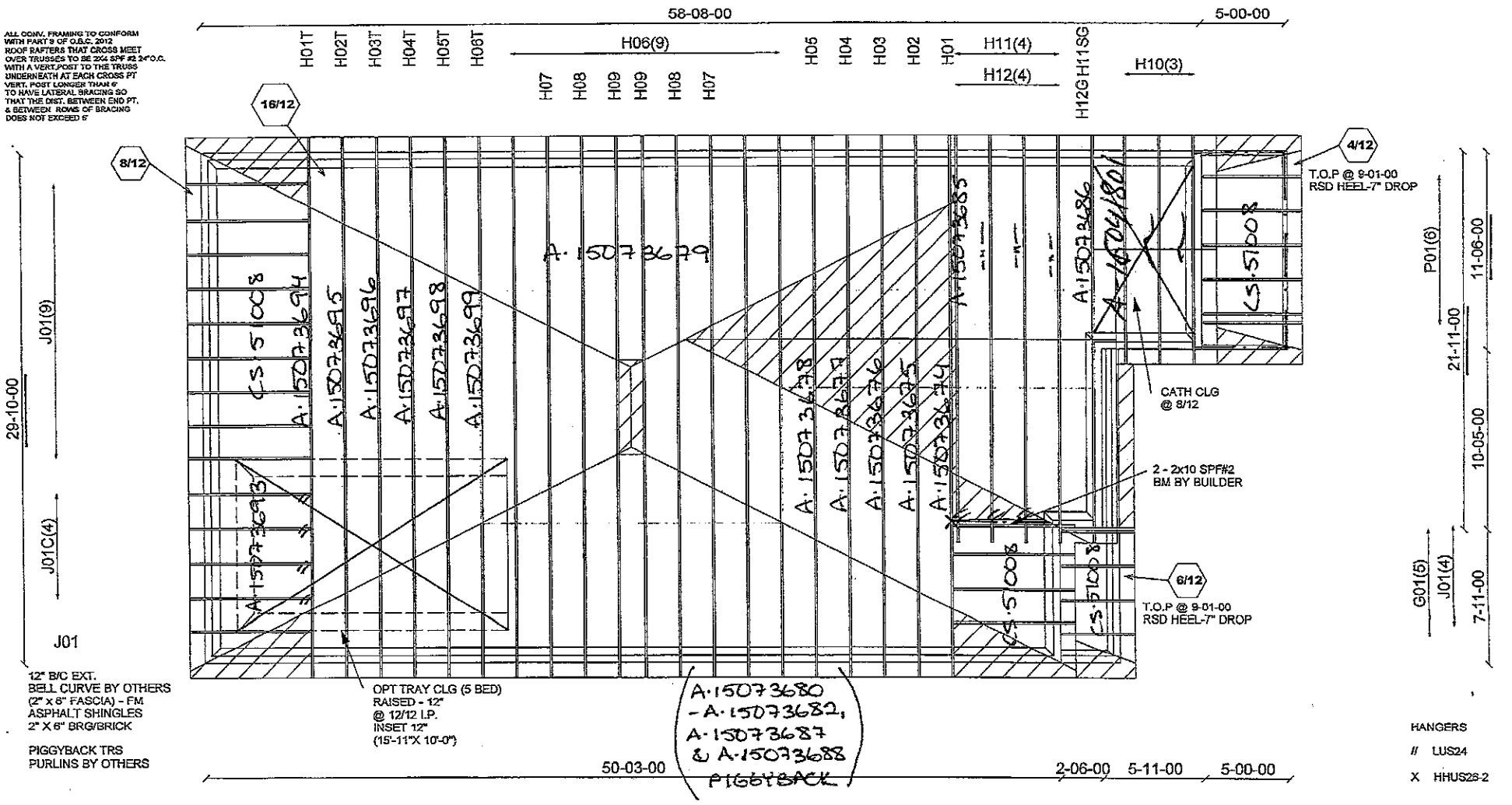
ALPA-BARRIE



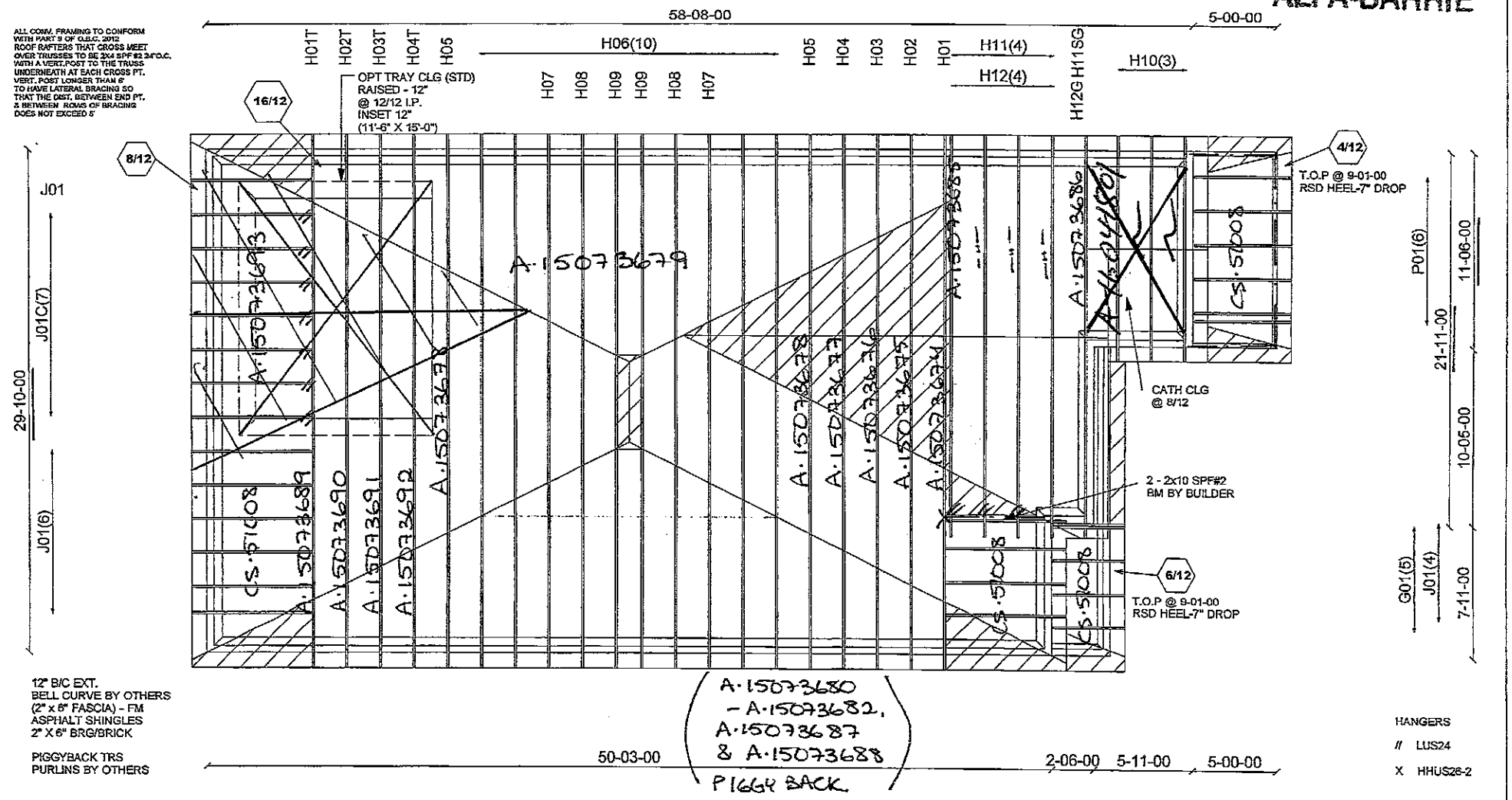
LOT 69
88062

CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 REV 1	/ A OPT 2ND W/TRAY LOT 69 REAR UPGRAD
Layout ID: 266204	Project: HUNTINGTON & NASHVILLE		
Plan Log: 85748	Date: 4/1/2016 Designer: marika		

ALPA-BARRIE



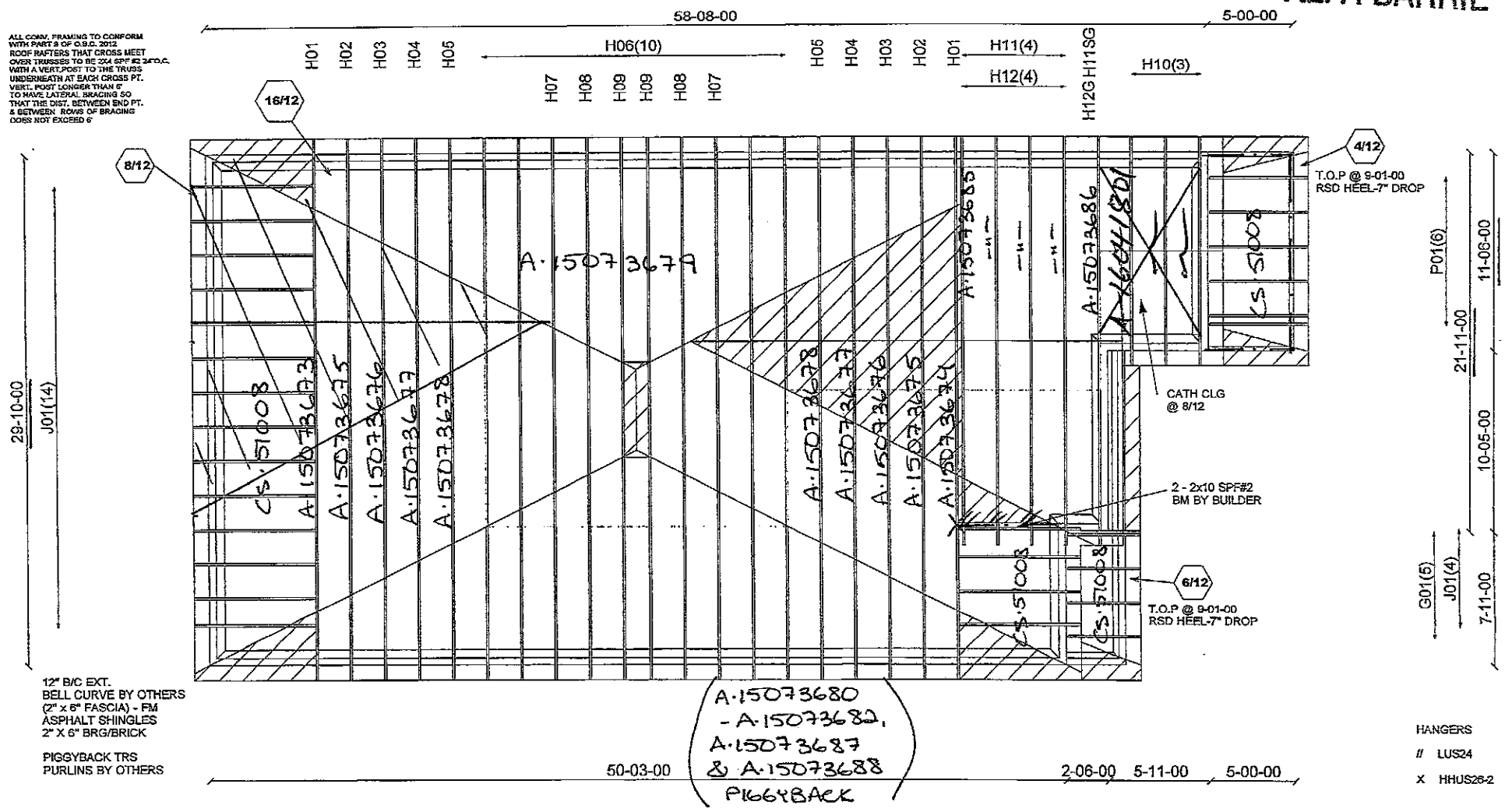
LOT 69
08062

CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 REV 1	/ A OPT TRAY LOT 69 REAR UPGRADE.
Layout ID: 266203	Project: HUNTINGTON & NASHVILLE	THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.	
Plan Log: 85748	Date: 4/1/2016 Designer: marika		

ALPA-BARRIE



ALL CONN. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 @PT #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

LOT 69
88062

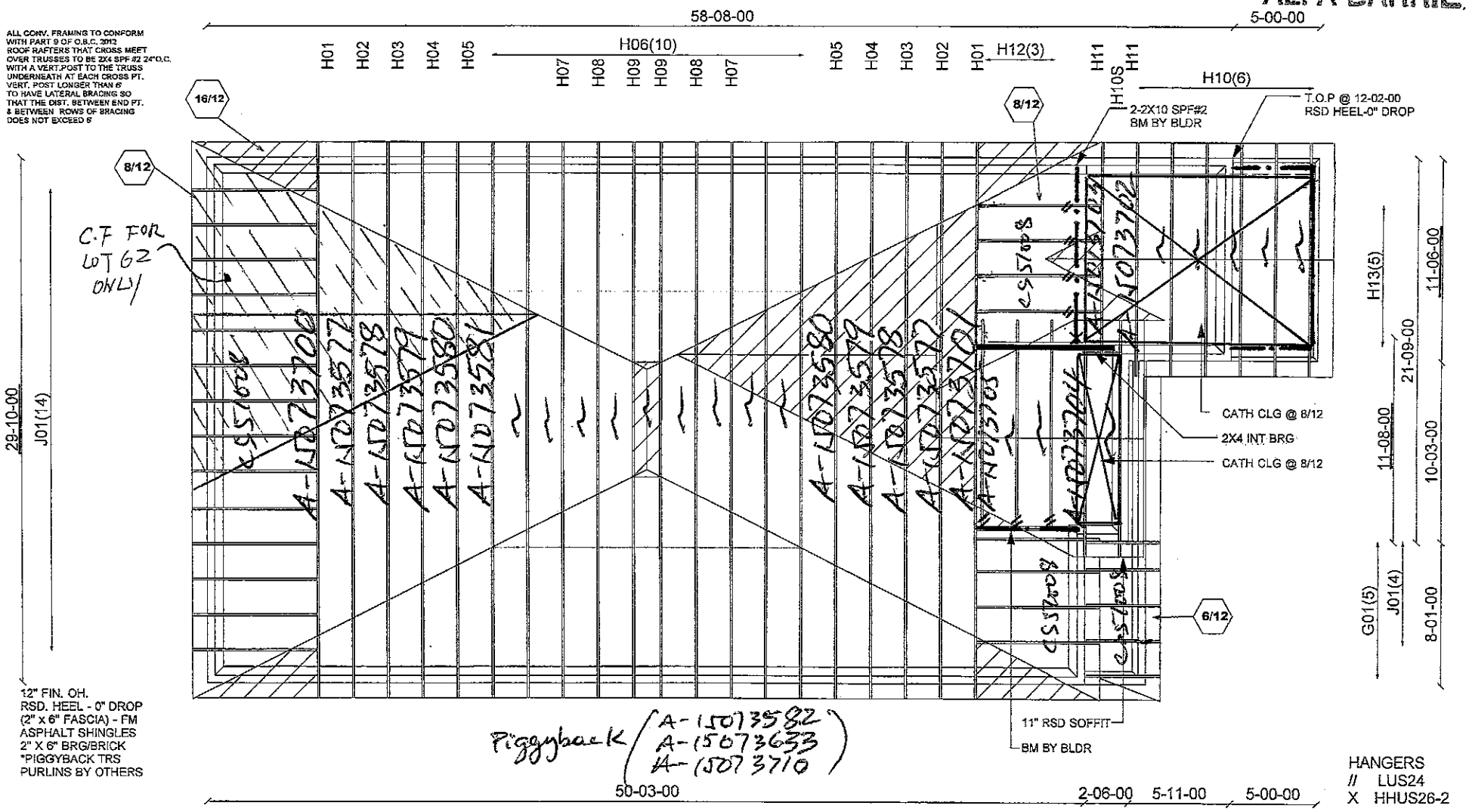
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 REV 1 / A	STD OR OPT. 2ND FLR LOT 69 REAR UPGRADE.
Layout ID: 266202	Project: HUNTINGTON & NASHVILLE		
Plan Log: 85748	Date: 4/1/2016 Designer: marika		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



HANGERS
// LUS24
X HHUS26-2

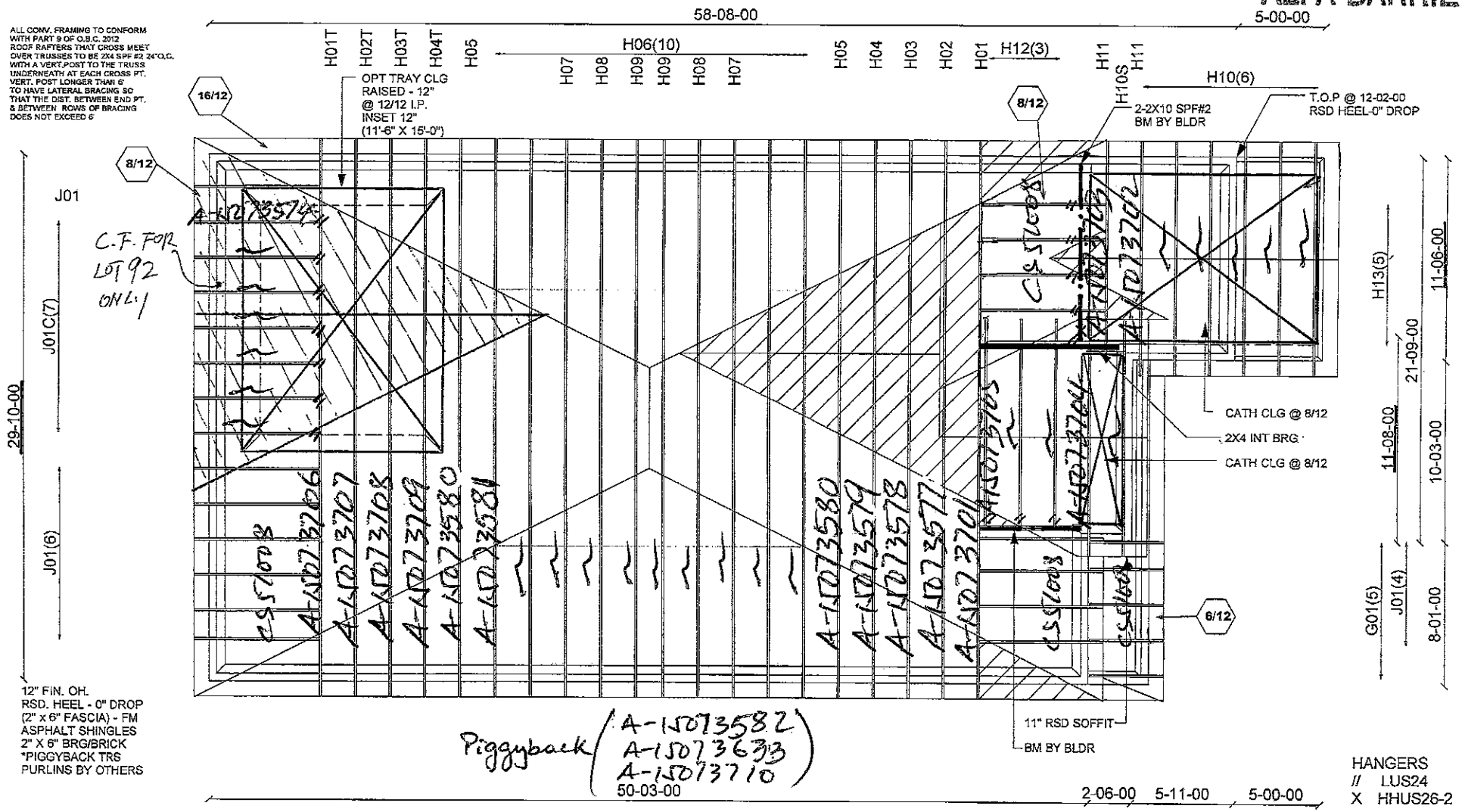
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

PL 80980, 82444, 85748, 88768 (LOT 62)

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 / B	STD OR OPT 2ND FLR. OR LOT 62
Layout ID: 253444	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015 Designer: cheryle		

ALPA-BARRIE



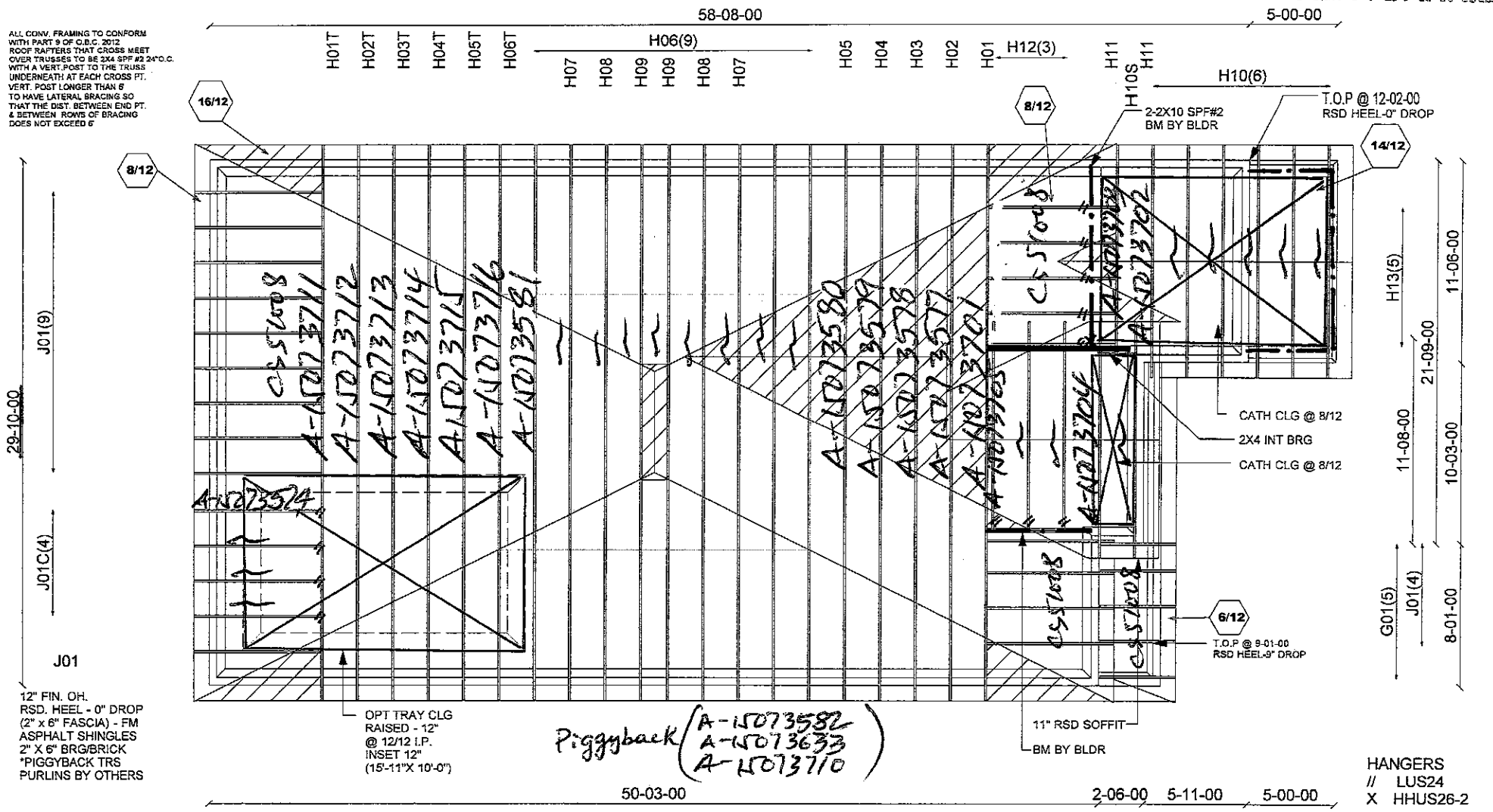
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

PL80980, 82444, 85748, 88768 (LOT 92)

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 / B OPT TRAY OR LOT 92	
Layout ID: 253445	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015	Designer: cheryle	

ALPA-BARRIE



CONVENTIONAL FRAMING BY OTHERS

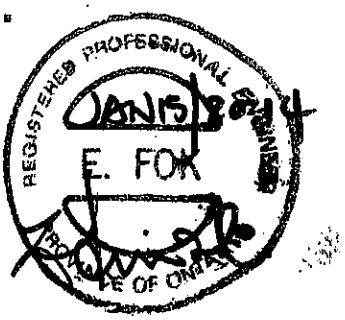
PL80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 / B OPT 2ND W/TRAY	
Layout ID: 253460	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015	Designer: cheryle	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

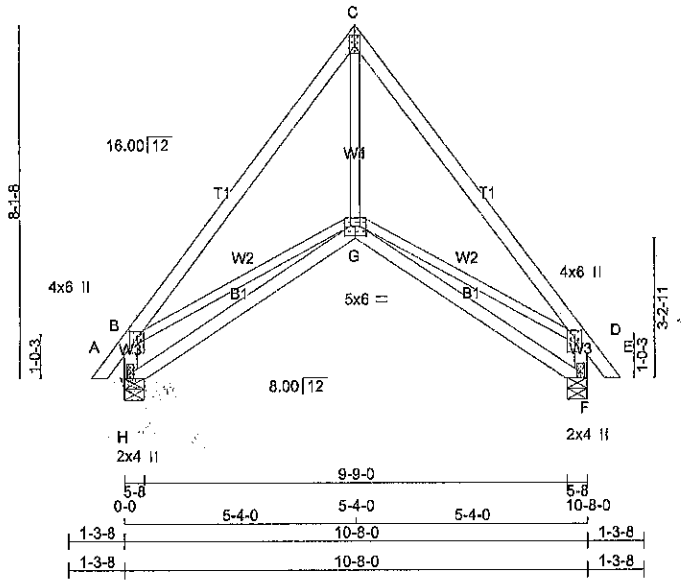
January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
266204	H10	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.000 S Feb 26 2016 MiTek Industries, Inc. Fri Apr 08 15:30:21 2016 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-pustFsJcGmsvUApK5ylnxdo1yieMqJLwxJntTJzSlSm
Scale = 1:53.5

3x5 II



TOTAL WEIGHT = 3 X 53 = 158 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	515	0	515	0	0	5-8	1-8
F	515	0	515	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	362	251 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
F	362	251 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 18	-70.5	-70.5	0.02 (1)	10.00	G- C	0 / 368	0.08 (1)
B- C	-465 / 0	-70.5	-70.5	0.26 (1)	6.25	B- G	0 / 311	0.07 (1)
C- D	-465 / 0	-70.5	-70.5	0.27 (1)	6.25	G- D	0 / 311	0.07 (1)
D- E	0 / 18	-70.5	-70.5	0.02 (1)	10.00			
H- B	-468 / 0	0.0	0.0	0.05 (1)	7.81			
F- D	-468 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-17.5	-17.5	0.15 (4)	10.00			
G- F	0 / 0	-17.5	-17.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.27 (C-D:1) , BC=0.15 (G-H:4) , WB=0.08 (C-G:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.59 (F) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

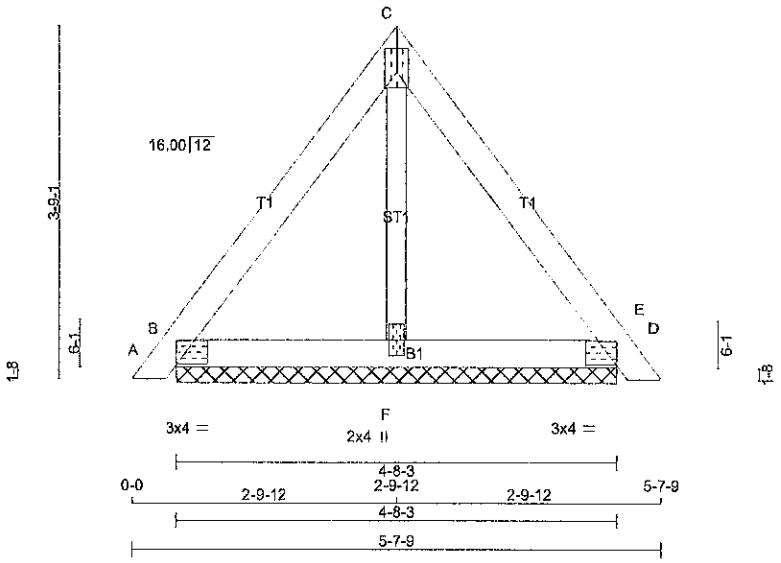
A-1604/801

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H12G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:52 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncszn AqzVRmF-trSJxqADBHBW_0RLj9EyXdLurRxfMjHlSqbrxyxoSf
Scale = 1:24.7

3x5 II



TOTAL WEIGHT = 18 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

LE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+p	MT20	3.0	6.0	2.00	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	F-C	-74 / 0	0.01 (1)
B-C	-56 / 0	-70.5	-70.5	0.07 (1)	6.25			
C-D	-56 / 0	-70.5	-70.5	0.07 (1)	6.25			
D-E	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B-F	0 / 33	-17.5	-17.5	0.03 (4)	10.00			
F-D	0 / 33	-17.5	-17.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07 (C-D:1) , BC=0.03 (B-F:4) , WB=0.01 (C-F:1) , SSI=0.04 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

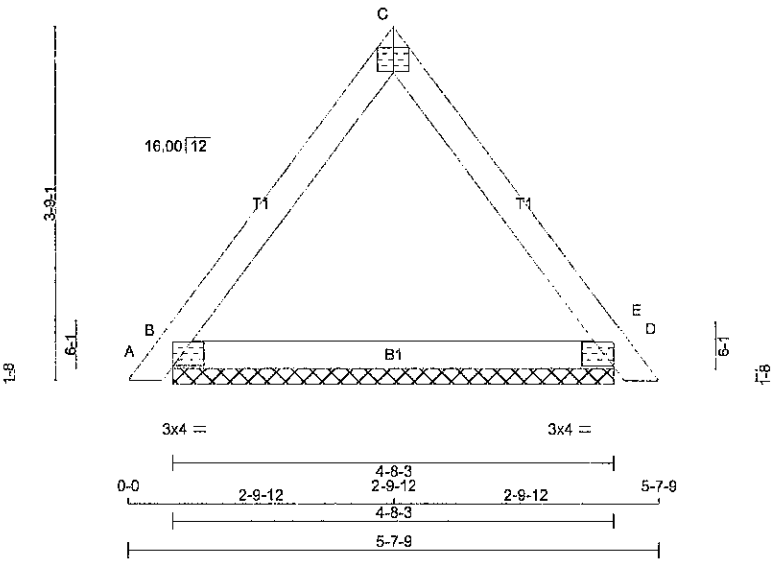
A-15073688

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H12	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:52 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-trSJxqADBHBW_0RLjj9EyXdLurQcfMxHISqbrxyxoSf
Scale = 1:24.7

3x4 =



TOTAL WEIGHT = 4 X 16 = 64 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

	TYPE	PLATES	W	LEN	Y	X
	TMB1-H	MT20	3.0	4.0	1.50	2.75
C	TT-p	MT20	3.0	4.0	3.25	Edge
D	TMB1-H	MT20	3.0	4.0	1.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	231	0	231	0	0	4-8-3	1-8
D	231	0	231	0	0	4-8-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	162	113 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
D	162	113 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 9	-70.5	-70.5	0.01 (1)		10.00			
B- C	-102 / 0	-70.5	-70.5	0.07 (1)		6.25			
C- D	-102 / 0	-70.5	-70.5	0.07 (1)		6.25			
D- E	0 / 9	-70.5	-70.5	0.01 (1)		10.00			
B- D	0 / 60	-17.5	-17.5	0.11 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07 (C-D:1) , BC=0.11 (B-D:4) , WB=0.00 (n/a:0) , SSI=0.06 (B-D:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

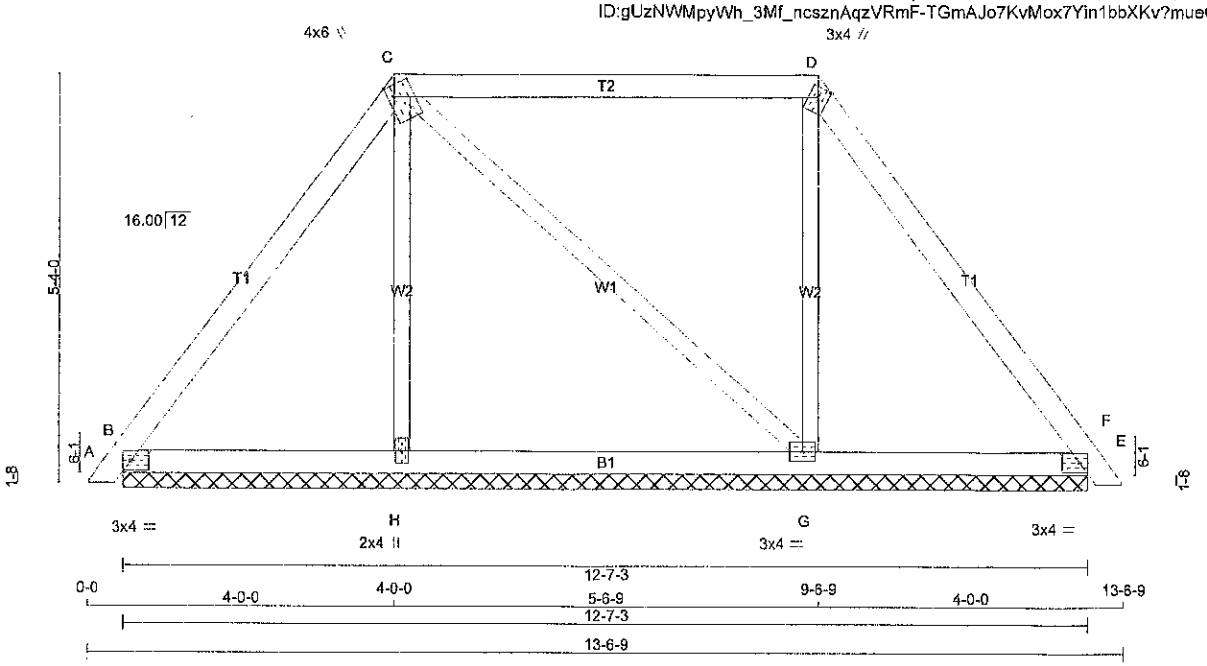
LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-10073687

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H09	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:49 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-TGmAJo7KvMox7Yin1bbXKv?mueOHS_kqcVcxEcyxoSt
Scale = 1:30.4



TOTAL WEIGHT = 2 X 49 = 98 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.50	2.75
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	290	0	290	0	0	12-7-3	1-8
E	255	0	255	0	0	12-7-3	1-8
H	266	0	266	0	0	12-7-3	1-8
G	348	0	348	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	200	160 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
E	176	139 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
H	191	105 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
G	247	154 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- C	-147 / 0	-70.5	-70.5	0.15 (1)	6.25
C- D	-56 / 0	-70.5	-70.5	0.37 (1)	6.25
D- E	-104 / 0	-70.5	-70.5	0.15 (1)	6.25
E- F	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- H	0 / 86	-17.5	-17.5	0.09 (4)	10.00
H- G	0 / 83	-17.5	-17.5	0.09 (4)	10.00
G- E	0 / 61	-17.5	-17.5	0.09 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-177 / 0	0.07 (1)
C- G	-36 / 0	0.04 (1)
G- D	-235 / 0	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37 (C-D:1) , BC=0.09 (B-H:4) , WB=0.09 (D-G:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

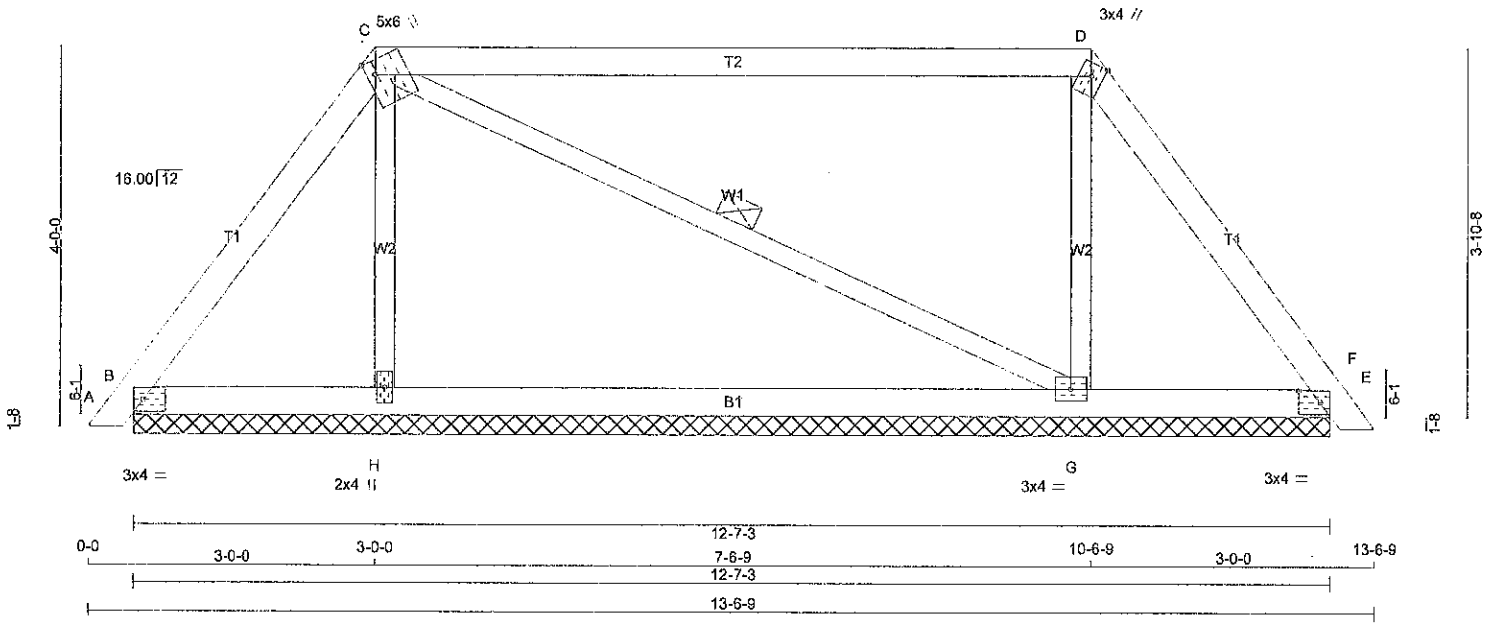
LICENSED PROFESSIONAL ENGINEER
07/15/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073682

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H08	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 MiTek Industries, Inc. Wed Jul 15 15:04:48 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-?4Co5T7i83g4VO7aUt4InhSZrE17jY0hNrsOiAyxoSj
Scale = 1:24.5



TOTAL WEIGHT = 2 X 46 = 92 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	2100F 1.8E	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

...ATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.50	2.75
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	230	0	230	0	0	12-7-3	1-8
E	209	0	209	0	0	12-7-3	1-8
H	341	0	341	0	0	12-7-3	1-8
G	377	0	377	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	157	138 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
E	142	126 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
H	246	136 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
G	270	158 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-G.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	H- C	-229 / 0	0.05 (1)
B- C	-146 / 0	-70.5	-70.5	0.08 (1)	6.25	C- G	-17 / 0	0.01 (1)
C- D	-62 / 0	-70.5	-70.5	0.45 (1)	6.25	G- D	-257 / 0	0.06 (1)
D- E	-120 / 0	-70.5	-70.5	0.08 (1)	6.25			
E- F	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B- H	0 / 85	-17.5	-17.5	0.16 (4)	10.00			
H- G	0 / 78	-17.5	-17.5	0.16 (4)	10.00			
G- E	0 / 70	-17.5	-17.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.45 (C-D:1) , BC=0.16 (B-H:4) , WB=0.06 (D-G:1) , SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (D) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

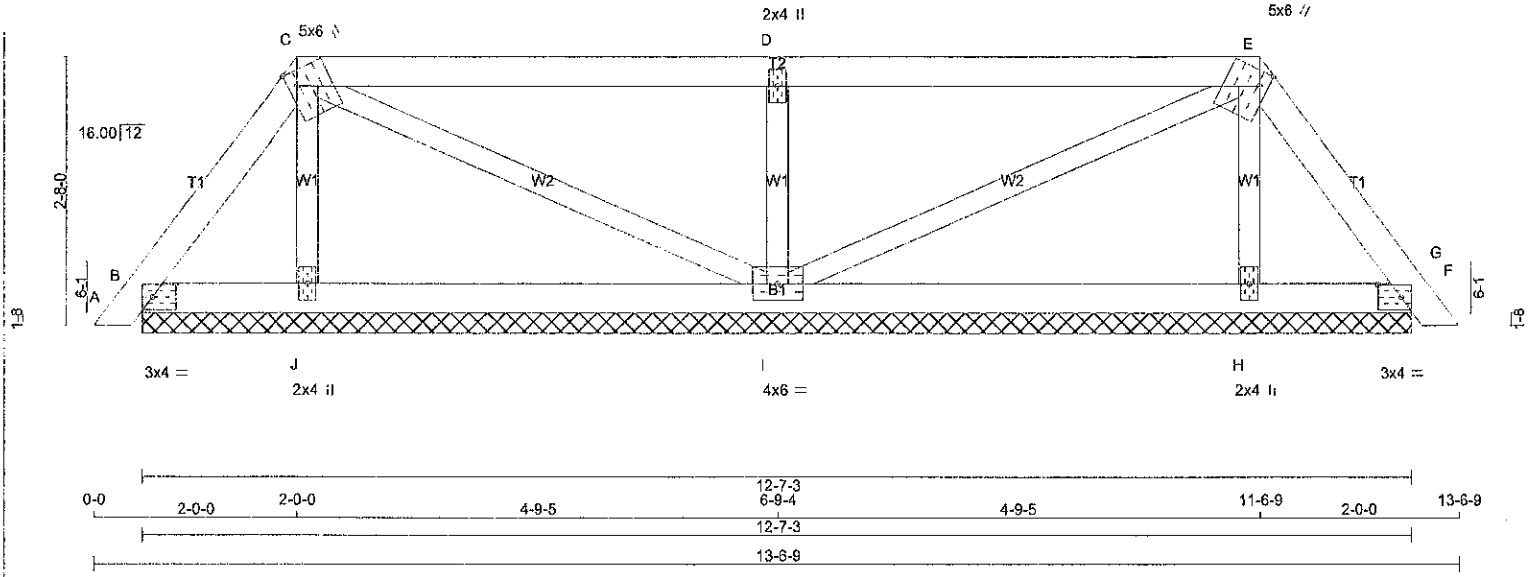
LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073681

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H07	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:47 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-WueQu764NlYDuEYOwAZ3FUwRxqiu_4fY8B7q9jyxoSk
Scale = 1:23.1



TOTAL WEIGHT = 2 X 46 = 91 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	130	0	130	0	0	12-7-3	1-8
F	130	0	130	0	0	12-7-3	1-8
J	186	0	186	0	0	12-7-3	1-8
I	527	0	527	0	0	12-7-3	1-8
H	186	0	186	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	88	77 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
F	88	77 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
J	134	71 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
I	369	262 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	134	71 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	J- C	-119 / 0	0.02 (1)
B- C	-62 / 0	-70.5	-70.5	0.03 (1)	6.25	C- I	-24 / 0	0.01 (1)
C- D	-8 / 0	-70.5	-70.5	0.27 (1)	10.00	I- D	-420 / 0	0.07 (1)
D- E	-8 / 0	-70.5	-70.5	0.27 (1)	10.00	I- E	-24 / 0	0.01 (1)
E- F	-62 / 0	-70.5	-70.5	0.03 (1)	6.25	H- E	-119 / 0	0.02 (1)
F- G	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B- J	0 / 36	-17.5	-17.5	0.06 (4)	10.00			
J- I	0 / 30	-17.5	-17.5	0.09 (4)	10.00			
I- H	0 / 30	-17.5	-17.5	0.09 (4)	10.00			
H- F	0 / 36	-17.5	-17.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27 (C-D:1) , BC=0.09 (I-J:4) , WB=0.07 (D-I:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

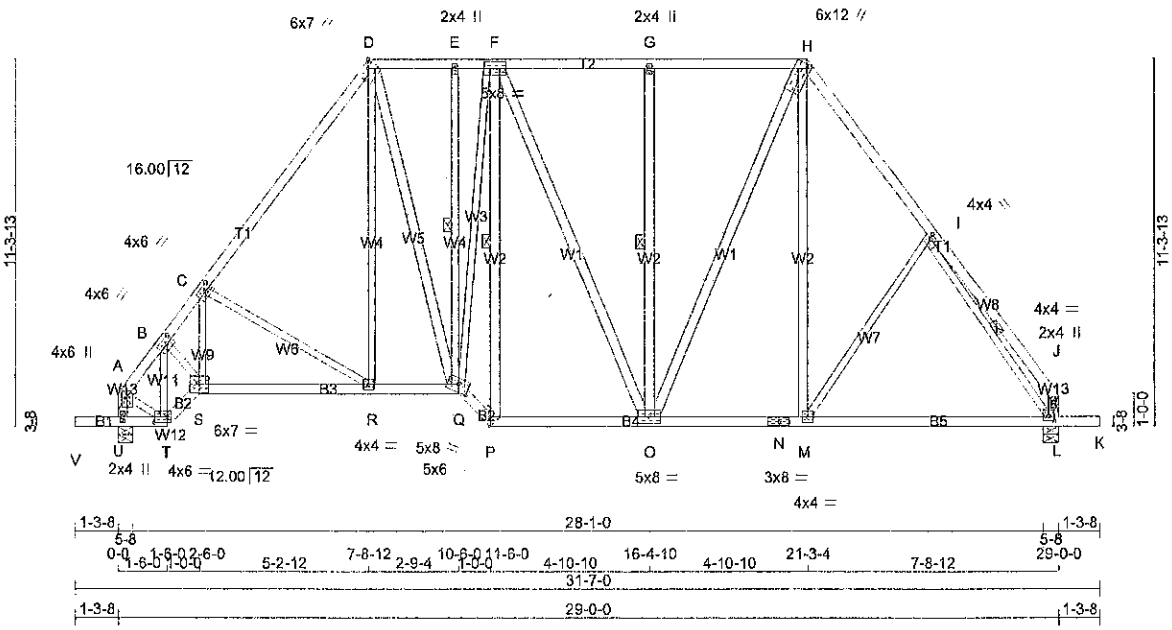
A-107 3680

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H06T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 16:13:14 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-m?J1sRpFAd_U6O1j0Ck9CZWILONIlSZ2YfVfpAyxnSZ

Scale = 1:71.5



TOTAL WEIGHT = 205 lb

[M][F]

LUMBER

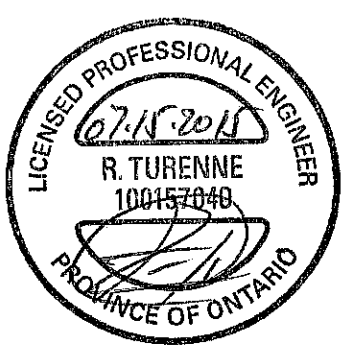
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
N	2x4	DRY	No.2	SPF
K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	Edge	2.75
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	12.0	Edge	1.50
I	TMWW-t	MT20	4.0	4.0	1.50	1.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.50	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BBW-h	MT20	5.0	6.0	Edge	2.50
Q	BBWWW-m	MT20	5.0	8.0	2.75	4.00



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-Q, F-P, G-O, H-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	-1117 / 0	-70.5	-70.5	0.08 (1)	5.92
B-C	-1554 / 0	-70.5	-70.5	0.22 (1)	5.07
C-D	-1236 / 0	-70.5	-70.5	0.26 (1)	5.51
D-E	-850 / 0	-70.5	-70.5	0.09 (1)	6.25
E-F	-850 / 0	-70.5	-70.5	0.12 (1)	6.25
F-G	-839 / 0	-70.5	-70.5	0.19 (1)	6.25
G-H	-838 / 0	-70.5	-70.5	0.19 (1)	6.25
H-I	-1135 / 0	-70.5	-70.5	0.15 (1)	5.81
I-J	0 / 25	-70.5	-70.5	0.17 (1)	10.00
U-A	-1216 / 0	0.0	0.0	0.12 (1)	7.26
L-J	-103 / 0	0.0	0.0	0.01 (1)	7.81
V-U	0 / 0	-88.0	-88.0	0.10 (1)	10.00
U-T	0 / 0	-17.5	-17.5	0.08 (1)	10.00
T-S	0 / 869	-17.5	-17.5	0.14 (1)	10.00
S-R	0 / 1022	-17.5	-17.5	0.22 (1)	10.00
R-Q	0 / 724	-17.5	-17.5	0.17 (1)	10.00
Q-P	0 / 1136	-17.5	-17.5	0.19 (1)	10.00
P-O	0 / 814	-17.5	-17.5	0.19 (1)	10.00
O-N	0 / 666	-17.5	-17.5	0.28 (4)	10.00
N-M	0 / 666	-17.5	-17.5	0.28 (4)	10.00
M-L	0 / 743	-17.5	-17.5	0.28 (4)	10.00
L-K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-B	-901 / 0	0.15 (1)
B-S	0 / 682	0.15 (1)
S-C	0 / 124	0.03 (4)
C-R	-350 / 0	0.23 (1)
R-D	0 / 258	0.06 (1)
D-Q	0 / 470	0.08 (1)
Q-E	-25 / 11	0.02 (1)
Q-F	0 / 404	0.09 (1)
P-F	-751 / 0	0.50 (1)
F-O	0 / 64	0.01 (1)
O-G	-378 / 0	0.25 (1)
O-H	0 / 425	0.07 (1)
M-H	0 / 250	0.04 (1)
M-I	-138 / 0	0.13 (1)
A-T	0 / 702	0.16 (1)
I-L	-1338 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28 (C-D:1) , BC=0.28 (M-O:4) , WB=0.50 (F-P:1) , SSI=0.16 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

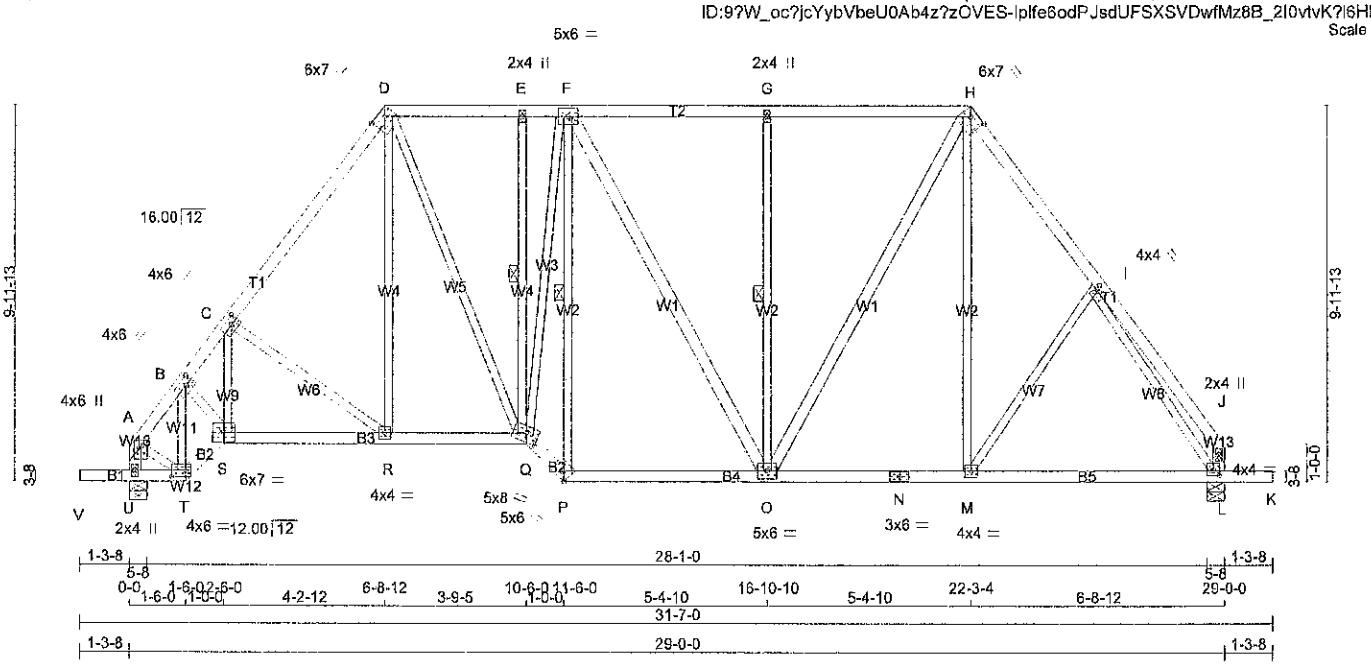
JSI GRIP= 0.74 (H) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

A-4073699

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H05T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 16:13:13 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-lplfe6odPJsdUFSXSVDwfMz8B_2l0vtvK?l6HkyxnSs
Scale = 1:61.4



TOTAL WEIGHT = 178 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
- N	2x4	DRY	No.2	SPF
- K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	2.00	4.25
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-h	MT20	6.0	7.0	2.00	4.25
I	TMWW-t	MT20	4.0	4.0	1.50	1.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.50	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BBW-h	MT20	5.0	6.0	Edge	2.50
Q	BBWWW-m	MT20	5.0	8.0	2.75	4.00
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-l	MT20	6.0	7.0		
T	BBWW-l	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-Q, F-P, G-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1119 / 0	-70.5	-70.5	0.07 (1)	5.93	T- B	-910 / 0	0.16 (1)
B- C	-1575 / 0	-70.5	-70.5	0.14 (1)	5.13	B- S	0 / 635	0.14 (1)
C- D	-1291 / 0	-70.5	-70.5	0.17 (1)	5.52	S- C	0 / 160	0.04 (1)
D- E	-986 / 0	-70.5	-70.5	0.14 (1)	6.13	C- R	-299 / 0	0.14 (1)
E- F	-985 / 0	-70.5	-70.5	0.15 (1)	6.12	R- D	0 / 251	0.06 (1)
F- G	-950 / 0	-70.5	-70.5	0.23 (1)	6.09	D- Q	0 / 564	0.13 (1)
G- H	-950 / 0	-70.5	-70.5	0.24 (1)	6.08	Q- E	-90 / 2	0.05 (1)
H- I	-1173 / 0	-70.5	-70.5	0.12 (1)	5.79	Q- F	0 / 534	0.12 (1)
I- J	0 / 21	-70.5	-70.5	0.12 (1)	10.00	P- F	-864 / 0	0.58 (1)
U- A	-1216 / 0	0.0	0.0	0.12 (1)	7.26	F- O	0 / 48	0.01 (1)
L- J	-91 / 0	0.0	0.0	0.01 (1)	7.81	O- G	-421 / 0	0.29 (1)
						O- H	0 / 535	0.09 (1)
V- U	0 / 0	-88.0	-88.0	0.10 (1)	10.00	M- H	0 / 203	0.05 (4)
U- T	0 / 0	-17.5	-17.5	0.08 (1)	10.00	M- I	-99 / 4	0.07 (1)
T- S	0 / 877	-17.5	-17.5	0.14 (1)	10.00	A- T	0 / 709	0.16 (1)
S- R	0 / 998	-17.5	-17.5	0.21 (1)	10.00	I- L	-1355 / 0	0.85 (1)
R- Q	0 / 760	-17.5	-17.5	0.17 (1)	10.00			
Q- P	0 / 1298	-17.5	-17.5	0.21 (1)	10.00			
P- O	0 / 927	-17.5	-17.5	0.21 (1)	10.00			
O- N	0 / 689	-17.5	-17.5	0.23 (4)	10.00			
N- M	0 / 689	-17.5	-17.5	0.23 (4)	10.00			
M- L	0 / 744	-17.5	-17.5	0.23 (4)	10.00			
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24 (G-H:1) , BC=0.23 (M-O:4) , WB=0.85 (I-L:1) , SSI=0.18 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

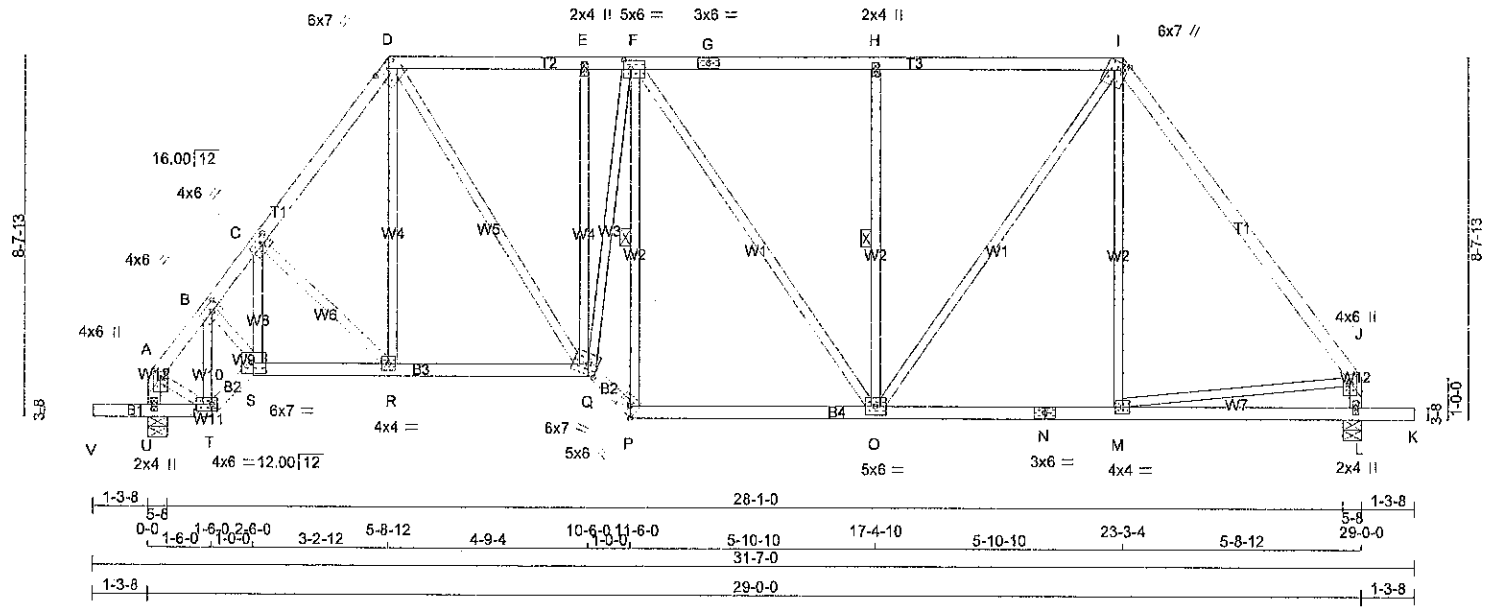
JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

A-15073698

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 16:13:12 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-qdBHRmn?e0kms5tLuoih78RuJaimHYOm5L0YilyxnSb
Scale = 1:55.4



TOTAL WEIGHT = 156 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
P	2x4	DRY	No.2	SPF
N	2x4	DRY	No.2	SPF
K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	1.75	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	6.0	2.50	2.00
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	1.50
J	TMVW+p	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BBW-h	MT20	5.0	6.0	Edge	2.50
Q	BBWWW-m	MT20	6.0	7.0	3.00	2.75
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-l	MT20	6.0	7.0		
T	BBWW-l	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-P, H-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	-1120 / 0	-70.5	-70.5	0.06 (1)		5.94
B- C	-1591 / 0	-70.5	-70.5	0.10 (1)		5.15
C- D	-1350 / 0	-70.5	-70.5	0.11 (1)		5.48
D- E	-1175 / 0	-70.5	-70.5	0.23 (1)		5.62
E- F	-1173 / 0	-70.5	-70.5	0.22 (1)		5.62
F- G	-1096 / 0	-70.5	-70.5	0.39 (1)		5.52
G- H	-1095 / 0	-70.5	-70.5	0.39 (1)		5.52
H- I	-1095 / 0	-70.5	-70.5	0.39 (1)		5.52
I- J	-1189 / 0	-70.5	-70.5	0.57 (1)		5.26
U- A	-1216 / 0	0.0	0.0	0.12 (1)		7.26
L- J	-1232 / 0	0.0	0.0	0.12 (1)		7.23
V- U	0 / 0	-88.0	-88.0	0.10 (1)		10.00
U- T	0 / 0	-17.5	-17.5	0.10 (1)		10.00
T- S	0 / 884	-17.5	-17.5	0.15 (1)		10.00
S- R	0 / 981	-17.5	-17.5	0.21 (1)		10.00
R- Q	0 / 799	-17.5	-17.5	0.18 (1)		10.00
Q- P	0 / 1506	-17.5	-17.5	0.25 (1)		10.00
P- O	0 / 1078	-17.5	-17.5	0.25 (1)		10.00
O- N	0 / 711	-17.5	-17.5	0.19 (1)		10.00
N- M	0 / 711	-17.5	-17.5	0.19 (1)		10.00
M- L	0 / 0	-17.5	-17.5	0.14 (4)		10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-917 / 0	0.16 (1)
B- S	0 / 600	0.13 (1)
S- C	0 / 183	0.04 (1)
C- R	-257 / 0	0.08 (1)
R- D	0 / 252	0.06 (1)
D- Q	0 / 686	0.15 (1)
Q- E	-175 / 0	0.19 (1)
Q- F	0 / 722	0.16 (1)
P- F	-1008 / 0	0.48 (1)
F- O	0 / 30	0.01 (1)
O- H	-463 / 0	0.22 (1)
O- I	0 / 664	0.16 (1)
M- I	0 / 100	0.03 (4)
A- T	0 / 714	0.16 (1)
M- J	0 / 715	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.57 (I-J:1) , BC=0.25 (O-P:1) , WB=0.48 (F-P:1) , SSI=0.20 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

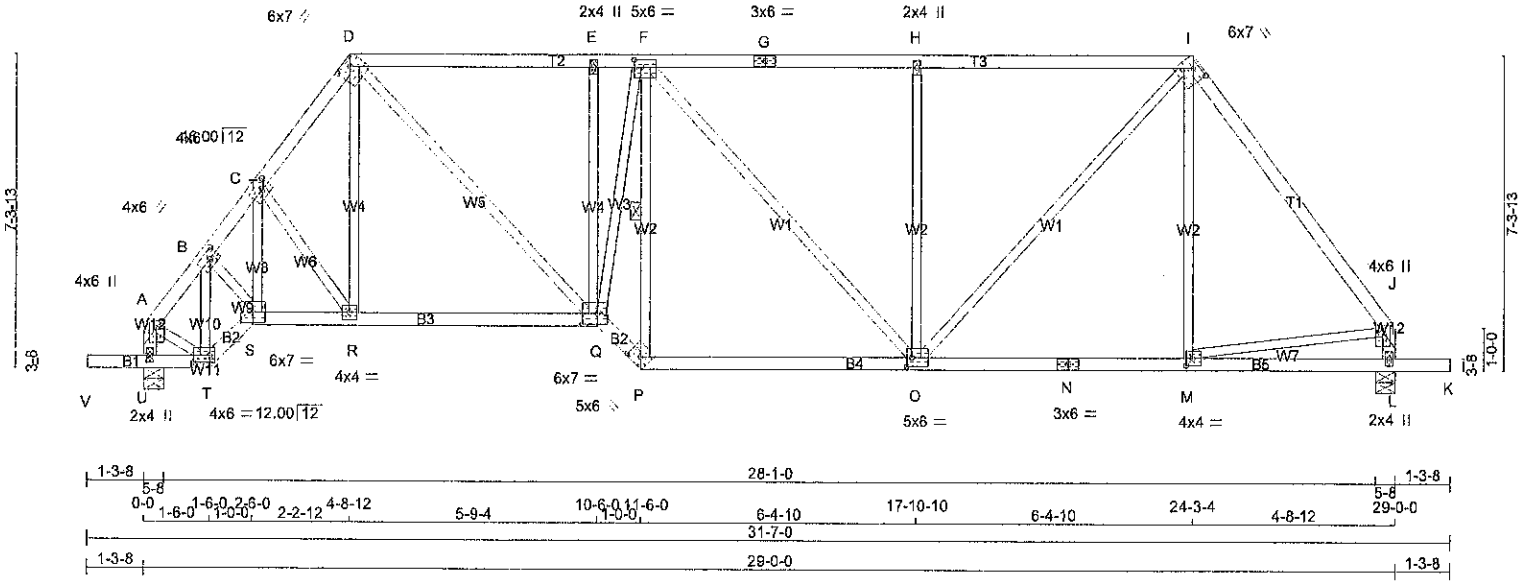
JSI GRIP= 0.75 (M) (INPUT = 0.90)
JSI METAL= 0.27 (J) (INPUT = 1.00)

A-H073697

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 16:13:11 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-MQdvDQmNticwFxI8L4BSaxukrAMmY5DcshG?CryxnSc
Scale = 1:53.9



TOTAL WEIGHT = 145 lb

(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
P	2x4	DRY	No.2	SPF
N	2x4	DRY	No.2	SPF
K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	1.50	3.75
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	6.0	2.00	2.00
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	6.0	7.0	1.50	3.75
J	TMVW+p	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BBW-h	MT20	5.0	6.0	2.00	3.25
Q	BBWWW-p	MT20	6.0	7.0	3.00	4.25
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-t	MT20	6.0	7.0		
T	BBWW-t	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	-1121 / 0	-70.5	-70.5	0.05 (1)	5.94
B-C	-1603 / 0	-70.5	-70.5	0.05 (1)	5.19
C-D	-1420 / 0	-70.5	-70.5	0.07 (1)	5.42
D-E	-1450 / 0	-70.5	-70.5	0.33 (1)	5.05
E-F	-1446 / 0	-70.5	-70.5	0.27 (1)	5.11
F-G	-1292 / 0	-70.5	-70.5	0.49 (1)	5.05
G-H	-1292 / 0	-70.5	-70.5	0.49 (1)	5.05
H-I	-1292 / 0	-70.5	-70.5	0.49 (1)	5.05
I-J	-1221 / 0	-70.5	-70.5	0.36 (1)	5.43
U-A	-1218 / 0	0.0	0.0	0.12 (1)	7.26
L-J	-1241 / 0	0.0	0.0	0.12 (1)	7.21
V-U	0 / 0	-88.0	-88.0	0.10 (1)	10.00
U-T	0 / 0	-17.5	-17.5	0.10 (1)	10.00
T-S	0 / 888	-17.5	-17.5	0.15 (1)	10.00
S-R	0 / 968	-17.5	-17.5	0.23 (1)	10.00
R-Q	0 / 844	-17.5	-17.5	0.21 (1)	10.00
Q-P	0 / 1793	-17.5	-17.5	0.29 (1)	10.00
P-O	0 / 1286	-17.5	-17.5	0.29 (1)	10.00
O-N	0 / 729	-17.5	-17.5	0.23 (4)	10.00
N-M	0 / 729	-17.5	-17.5	0.23 (4)	10.00
M-L	0 / 0	-17.5	-17.5	0.12 (4)	10.00
L-K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-B	-921 / 0	0.16 (1)
B-S	0 / 574	0.13 (1)
S-C	0 / 190	0.04 (1)
C-R	-222 / 0	0.06 (1)
R-D	0 / 255	0.06 (1)
D-Q	0 / 871	0.20 (1)
Q-E	-271 / 0	0.17 (1)
Q-F	0 / 983	0.22 (1)
P-F	-1207 / 0	0.41 (1)
F-O	0 / 10	0.00 (4)
O-H	-505 / 0	0.48 (1)
O-I	0 / 834	0.19 (1)
M-I	0 / 85	0.03 (4)
A-T	0 / 718	0.16 (1)
M-J	0 / 735	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.49 (H-I:1) , BC=0.29 (O-P:1) , WB=0.48 (H-O:1) , SSI=0.21 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (F) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

A-1073696

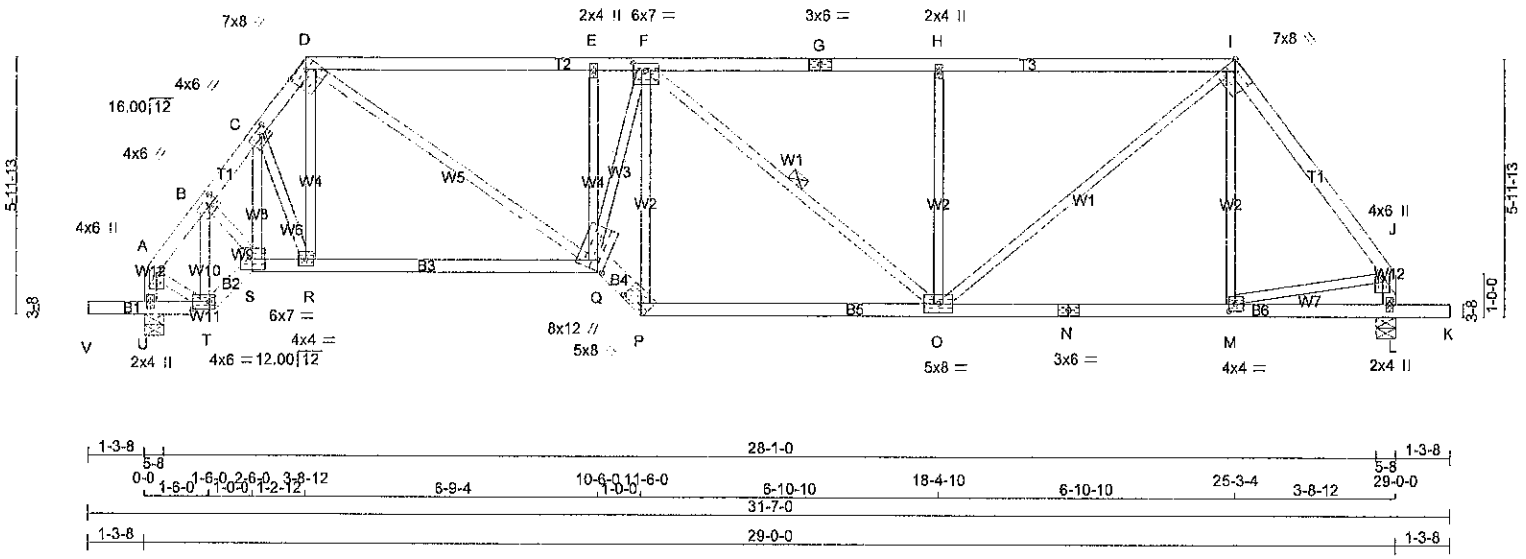
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 16:13:10 2015 Page 1

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-uE3X04mI6OU3dnjynNgD1jMX5m?TpYNTd1XSgPyxnSc

Scale = 1:53.9



TOTAL WEIGHT = 136 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
- P	2x6	DRY	No.2	SPF
- N	2x4	DRY	No.2	SPF
- K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	7.0	8.0	1.75	5.00
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	6.0	7.0	2.00	3.50
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	7.0	8.0	Edge	2.75
J	TMVW+p	MT20	4.0	6.0	1.75	1.75
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BBW-h	MT20	5.0	8.0	1.75	5.00
Q	BBWWW+m	MT20	8.0	12.0	3.00	2.50
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-l	MT20	6.0	7.0		
T	BBWW-l	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1122 / 0	-70.5	-70.5	0.04 (1)	5.95
B- C	-1612 / 0	-70.5	-70.5	0.06 (1)	5.17
C- D	-1508 / 0	-70.5	-70.5	0.05 (1)	5.32
D- E	-1910 / 0	-70.5	-70.5	0.52 (1)	4.35
E- F	-1902 / 0	-70.5	-70.5	0.41 (1)	4.35
F- G	-1578 / 0	-70.5	-70.5	0.61 (1)	4.50
G- H	-1578 / 0	-70.5	-70.5	0.61 (1)	4.50
H- I	-1578 / 0	-70.5	-70.5	0.62 (1)	4.50
I- J	-1245 / 0	-70.5	-70.5	0.21 (1)	5.54
U- A	-1216 / 0	0.0	0.0	0.12 (1)	7.26
L- J	-1252 / 0	0.0	0.0	0.13 (1)	7.19
V- U	0 / 0	-88.0	-88.0	0.10 (1)	10.00
U- T	0 / 0	-17.5	-17.5	0.10 (1)	10.00
T- S	0 / 892	-17.5	-17.5	0.15 (1)	10.00
S- R	0 / 959	-17.5	-17.5	0.27 (1)	10.00
R- Q	0 / 898	-17.5	-17.5	0.27 (4)	10.00
Q- P	0 / 2235	-17.5	-17.5	0.27 (1)	10.00
P- O	0 / 1608	-17.5	-17.5	0.36 (1)	10.00
O- N	0 / 742	-17.5	-17.5	0.26 (4)	10.00
N- M	0 / 742	-17.5	-17.5	0.26 (4)	10.00
M- L	0 / 0	-17.5	-17.5	0.11 (4)	10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-925 / 0	0.16 (1)
B- S	0 / 555	0.12 (1)
S- C	0 / 166	0.04 (1)
C- R	-188 / 0	0.04 (1)
R- D	0 / 288	0.06 (1)
D- Q	0 / 1221	0.27 (1)
Q- E	-376 / 0	0.14 (1)
Q- F	0 / 1380	0.31 (1)
P- F	-1502 / 0	0.84 (1)
F- O	-39 / 0	0.02 (1)
O- H	-546 / 0	0.30 (1)
O- I	0 / 1081	0.24 (1)
M- I	-20 / 72	0.02 (4)
A- T	0 / 721	0.16 (1)
M- J	0 / 752	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.62 (H-I:1) , BC=0.36 (O-P:1) , WB=0.84 (F-P:1) , SSI=0.23 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

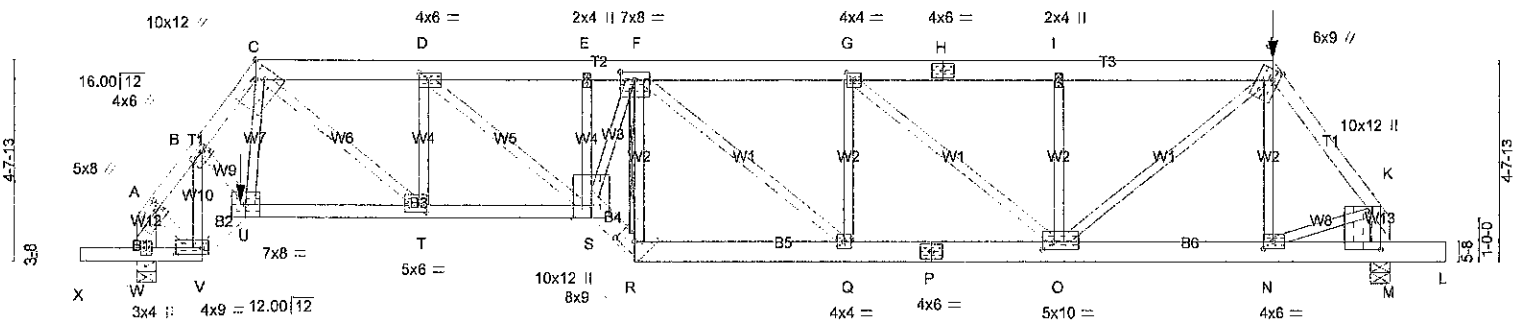
JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.38 (Q) (INPUT = 1.00)

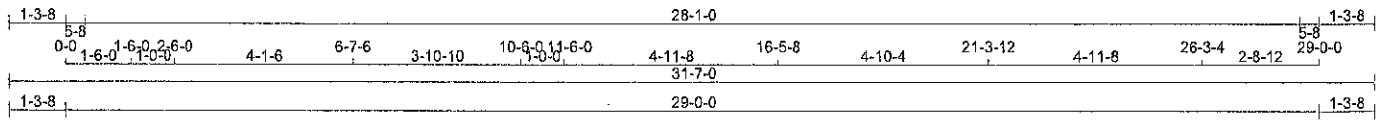
A-150736 95

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 MiTek Industries, Inc. Wed Jul 15 16:16:27 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-d8T?MR8JjwJ6m3HppFiQWG0FFB4mTefTxAlN5UyxnPY
Scale = 1:53.7





TOTAL WEIGHT = 157 lb

[M]

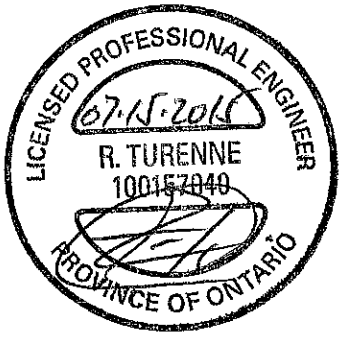
LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
W - A	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
- R	2x6	DRY	No.2	SPF
- P	2x6	DRY	No.2	SPF
- L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.25	2.00
B	TMWW-t	MT20	4.0	6.0	1.50	1.75
C	TTWW-h	MT20	10.0	12.0	Edge	4.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	7.0	8.0	2.25	4.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TS-t	MT20	4.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	9.0	2.75	1.75
K	TMBMWW+p	MT20	10.0	12.0	10.25	3.00
N	BMWW-t	MT20	4.0	6.0	2.00	2.75
O	BMWWW-t	MT20	5.0	10.0	2.25	3.25
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BBW-h	MT20	8.0	9.0	3.00	4.50
S	BBWWW+p	MT20	10.0	12.0	Edge	
T	BMWW-t	MT20	5.0	6.0	2.00	2.00
U	BBWW-I	MT20	7.0	8.0		
V	BBWW-I	MT20	4.0	9.0	1.75	7.25
W	BMV1+p	MT20	3.0	4.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2676	0	2676	0	0	5-8	3-0
M	2582	0	2582	0	0	5-8	2-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1882	1298 / 0	0 / 0	0 / 0	0 / 0	584 / 0	0 / 0
M	1820	1228 / 0	0 / 0	0 / 0	0 / 0	592 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT F-R
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-2329 / 0	-70.5	-70.5	1	0.14 (1)	4.35
B- C	-3324 / 0	-70.5	-70.5	1	0.21 (1)	3.65
C- D	-4045 / 0	-70.5	-70.5	1	0.18 (1)	4.15
D- E	-5237 / 0	-70.5	-70.5	1	0.43 (1)	3.44
E- F	-5249 / 0	-138.7	-138.7	1	0.42 (1)	3.44
F- G	-4346 / 0	-138.7	-138.7	1	0.32 (1)	3.88
G- H	-3488 / 0	-138.7	-138.7	1	0.29 (1)	4.29
H- I	-3488 / 0	-138.7	-138.7	1	0.29 (1)	4.29
I- J	-3488 / 0	-138.7	-138.7	1	0.31 (1)	4.27
J- K	-2568 / 0	-70.5	-70.5	1	0.21 (1)	4.12
W- A	-2464 / 0	0.0	0.0	1	0.18 (1)	6.55
M- K	-2447 / 0	0.0	0.0	1	0.17 (1)	6.56
X- W	0 / 0	-88.0	-88.0	1	0.11 (1)	10.00
W- V	0 / 0	-66.7	-66.7	1	0.11 (1)	10.00
V- U	0 / 1837	-66.7	-66.7	1	0.34 (1)	10.00
U- T	0 / 1955	-102.7	-102.7	1	0.62 (1)	10.00
T- S	0 / 4045	-102.7	-102.7	1	0.99 (1)	10.00
S- R	0 / 6014	-34.5	-34.5	1	0.78 (1)	10.00
R- Q	0 / 4271	-34.5	-34.5	1	0.64 (1)	10.00
Q- P	0 / 4346	-34.5	-34.5	1	0.65 (1)	10.00
P- O	0 / 4346	-34.5	-34.5	1	0.65 (1)	10.00
O- N	0 / 1536	-34.5	-34.5	1	0.26 (1)	10.00
N- M	0 / 0	-34.5	-34.5	1	0.07 (4)	10.00
M- L	0 / 0	-88.0	-88.0	1	0.05 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- B	-1814 / 0	0.33 (1)
B- U	0 / 1170	0.29 (1)
U- C	0 / 721	0.18 (1)
C- T	0 / 2731	0.68 (1)
T- D	-1362 / 0	0.30 (1)
D- S	0 / 1542	0.38 (1)
S- E	0 / 367	0.09 (1)
S- F	0 / 3510	0.87 (1)
R- F	-4295 / 0	0.83 (1)
F- Q	0 / 99	0.02 (1)
Q- G	0 / 167	0.06 (4)
G- O	-1134 / 0	0.81 (1)
O- I	-747 / 0	0.22 (1)
O- J	0 / 2554	0.63 (1)
N- J	-121 / 77	0.04 (1)
A- V	0 / 1481	0.37 (1)
N- K	0 / 1567	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
J	26-3-4	-140	-140	---	FRONT	VERT	TOTAL
U	2-6-0	-139	-139	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 18-6-0 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 10-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 814 (0.43")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 505 (0.03")
ALLOWABLE DEFL.(TL)= L/120 (0.19")

A-N0136607

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H01T	1	1	TRUSS DESC.	

1 - INDICATES REFERENCE CORNER OF PLATE
JCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.2 lbs FACTORED DOWN AT 26-3-4 ON TOP CHORD, AND 138.7 lbs FACTORED DOWN AT 2-6-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

CALCULATED VERT. DEFL.(TL) = L/ 280 (0.06")

CSI: TC=0.43 (D-E:1) , BC=0.99 (S-T:1) , WB=0.87 (F-S:1) , SSI=0.37 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.94 (P) (INPUT = 1.00)

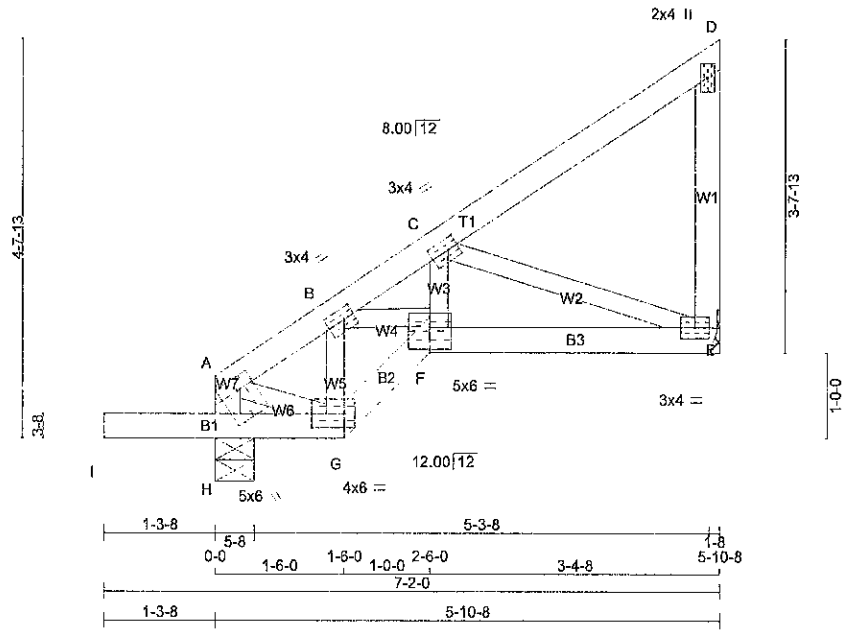
A-150736 94(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	J01C	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-1FC_75bW?IzsIlcwUWl5W32eM5wKeZ9vRRCqtryxnOz

Scale = 1:27.0



TOTAL WEIGHT = 4 X 27 = 107 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

"Y: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBWW-I	MT20	4.0	6.0	2.00	4.50
H	TMBMVW1*+h	MT20	5.0	6.0	1.75	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	384	0	384	0	0	5-8	1-8
E	246	0	246	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	271	163 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
E	173	117 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-209 / 0	0.0	0.0	0.02 (1)	7.81	A- G	0 / 172	0.04 (1)
A- B	-203 / 0	-70.5	-70.5	0.02 (1)	6.25	G- B	-219 / 0	0.03 (1)
B- C	-403 / 0	-70.5	-70.5	0.08 (1)	6.25	B- F	0 / 221	0.05 (1)
C- D	-10 / 0	-70.5	-70.5	0.08 (1)	6.25	F- C	0 / 131	0.03 (1)
E- D	-97 / 0	0.0	0.0	0.02 (1)	7.81	C- E	-370 / 0	0.08 (1)
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00			
H- G	0 / 0	-17.5	-17.5	0.08 (1)	10.00			
G- F	0 / 211	-17.5	-17.5	0.04 (1)	10.00			
F- E	0 / 349	-17.5	-17.5	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.08 (C-D:1) , BC=0.10 (H-I:1) , WB=0.08 (C-E:1) , SSI=0.09 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.53 (E) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

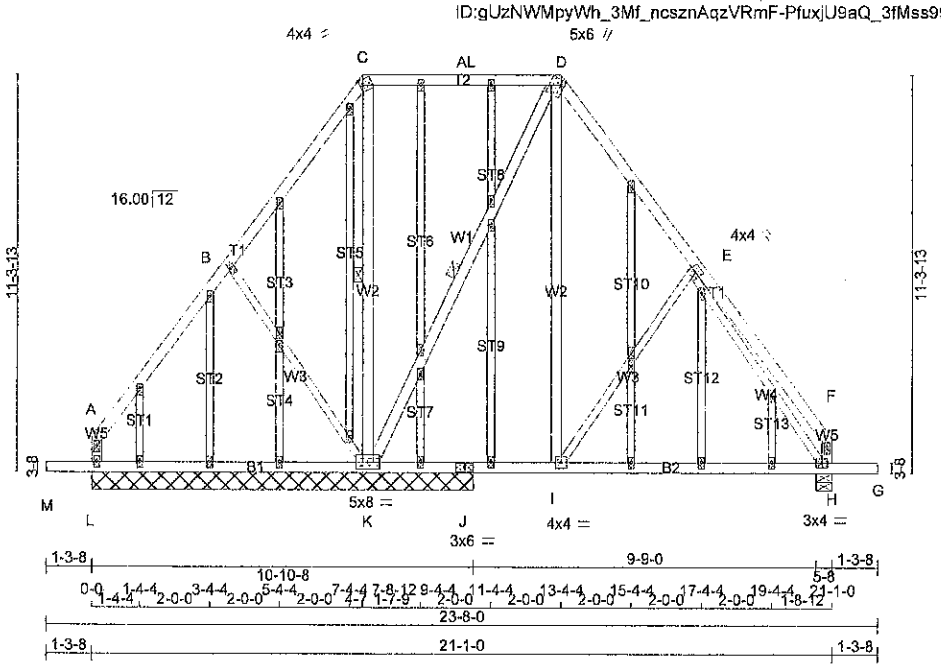
LICENSED PROFESSIONAL ENGINEER
@7-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-15073693

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H11SG	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:51 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-PfuxjU9aQ_3fMss99?d?PK45HR2Owoa73o52lUyxoSg
Scale = 1:66.2



TOTAL WEIGHT = 168 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K	2x3	DRY	No.2	SPF
E - E	2x3	DRY	No.2	SPF
E - H	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-m	MT20	4.0	4.0	1.75	1.00
D	TTWW+m	MT20	5.0	8.0	1.75	1.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW1-t	MT20	5.0	8.0		
L	BMV1+p	MT20	2.0	4.0		
N, O, P, Q, R, S, T, U, V, W, X, X, Y, Z, AA, AB, AC, AD, AE, AF, AG, AG, AH, AI, AJ, AK						
N	NP+w	MT20	2.0	4.0		

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1260	0	1260	0	0	10-10-8 (10-7-8)	
H	592	0	592	0	0	5-8	1-8
L	273	0	273	0	0	10-10-8 (10-7-8)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	886	610 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0
H	419	269 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
L	195	115 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-K, D-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 27	-70.5	-70.5	0.18 (1)	10.00	B-K	-213 / 0	0.20 (1)
B-C	0 / 178	-70.5	-70.5	0.21 (1)	10.00	K-C	-487 / 0	0.33 (1)
C-AL	0 / 129	-78.0	-78.0	0.44 (1)	10.00	K-D	-524 / 0	0.46 (1)
AL-D	0 / 129	-78.0	-78.0	0.44 (1)	10.00	I-D	0 / 260	0.04 (1)
D-E	-196 / 0	-70.5	-70.5	0.18 (1)	6.25	I-E	-191 / 0	0.18 (1)
E-F	0 / 27	-70.5	-70.5	0.18 (1)	10.00	E-H	-363 / 0	0.34 (1)
H-F	-101 / 0	0.0	0.0	0.01 (1)	7.81			
L-A	-102 / 0	0.0	0.0	0.01 (1)	7.81			
M-L	0 / 0	-88.0	-88.0	0.10 (1)	10.00			
L-K	0 / 0	-17.5	-17.5	0.22 (4)	10.00			
K-J	0 / 105	-17.5	-17.5	0.22 (4)	10.00			
J-I	0 / 105	-17.5	-17.5	0.22 (4)	10.00			
I-H	0 / 213	-17.5	-17.5	0.24 (4)	10.00			
H-G	0 / 0	-88.0	-88.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.44 (C-D:1) , BC=0.24 (H-I:4) , WB=0.46 (D-K:1) , SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (H) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)

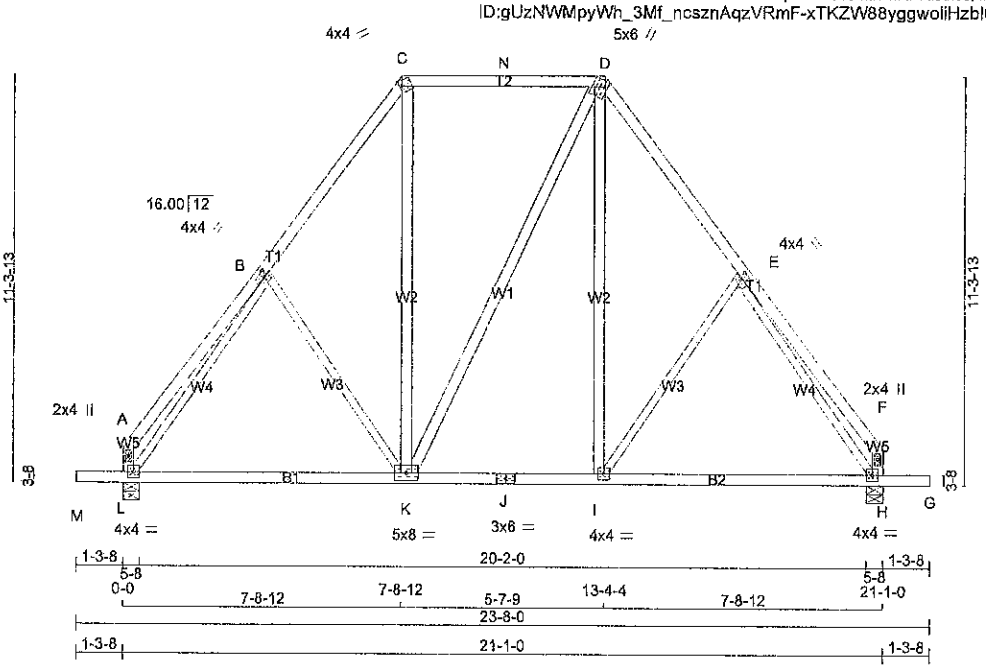
A-1073686

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H11	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:50 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-xTKZW88yggwoliHzbl6mt6Ywo1hoBGU_r9LVm2yxoSh

Scale: 3/16"=1'



TOTAL WEIGHT = 4 X 125 = 499 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K	2x3	DRY	No.2	SPF
- E	2x3	DRY	No.2	SPF
L - B	2x3	DRY	No.2	SPF
E - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0	1.75	1.00
D	TTWW+m	MT20	5.0	6.0	1.75	1.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1062	0	1062	0	0	5-8	1-8
H	1062	0	1062	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	750	497 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0
H	750	497 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-70.5	-70.5	0.17 (1)	10.00	B- K	-160 / 0		0.15 (1)
B- C	-751 / 0	-70.5	-70.5	0.14 (1)	6.25	K- C	0 / 257		0.04 (1)
C- N	-437 / 0	-78.0	-78.0	0.42 (1)	6.25	K- D	0 / 0		0.00 (1)
N- D	-437 / 0	-78.0	-78.0	0.42 (1)	6.25	I- D	0 / 256		0.04 (1)
D- E	-751 / 0	-70.5	-70.5	0.14 (1)	6.25	I- E	-160 / 0		0.15 (1)
E- F	0 / 26	-70.5	-70.5	0.17 (1)	10.00	L- B	-948 / 0		0.83 (1)
L- A	-102 / 0	0.0	0.0	0.01 (1)	7.81	E- H	-947 / 0		0.83 (1)
H- F	-102 / 0	0.0	0.0	0.01 (1)	7.81				
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00				
L- K	0 / 526	-17.5	-17.5	0.26 (4)	10.00				
K- J	0 / 436	-17.5	-17.5	0.24 (4)	10.00				
J- I	0 / 436	-17.5	-17.5	0.24 (4)	10.00				
I- H	0 / 526	-17.5	-17.5	0.26 (4)	10.00				
H- G	0 / 0	-88.0	-88.0	0.10 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.42 (C-D:1) , BC=0.26 (H-I:4) , WB=0.83 (B-L:1) , SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)

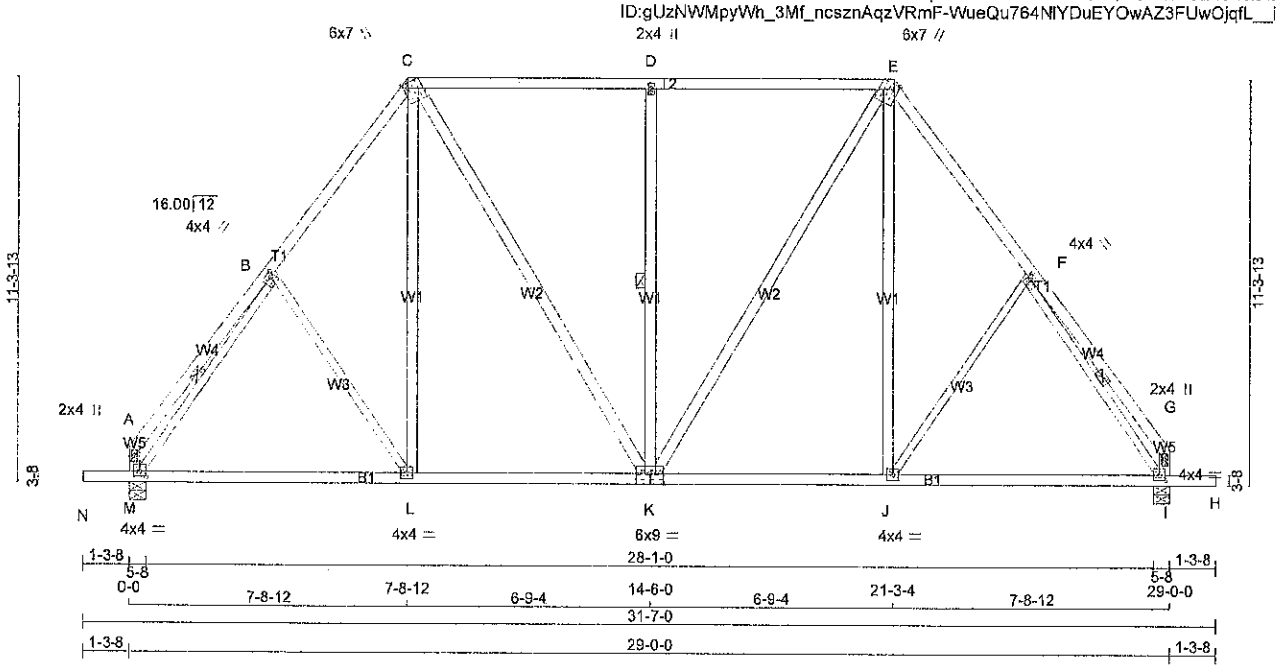
A-4073685

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H06	10	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:47 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-WueQu764NIYDuEYOwAZ3FUwOjqfL__iY8B7q9jyxoSk

Scale = 1:64.8



TOTAL WEIGHT = 10 X 170 = 1699 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L	2x3	DRY	No.2	SPF
F	2x3	DRY	No.2	SPF
M - B	2x3	DRY	No.2	SPF
F - I	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.50	1.25
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMWW-t	MT20	4.0	4.0	1.50	1.25
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	1.75	2.00
J	BMWW-t	MT20	4.0	4.0		
K	BSWWW-t	MT20	6.0	9.0	Edge	4.50
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1440	0	1440	0	0	5-8	1-8
I	1440	0	1440	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1019	663 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0
I	1019	663 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-K, B-M, F-I,
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 25	-70.5	-70.5	0.16 (1)	10.00	B- L	-135 / 0	0.13 (1)
B- C	-1200 / 0	-70.5	-70.5	0.16 (1)	5.69	L- C	0 / 259	0.04 (1)
C- D	-933 / 0	-78.0	-78.0	0.48 (1)	5.73	C- K	0 / 436	0.07 (1)
D- E	-933 / 0	-78.0	-78.0	0.48 (1)	5.73	K- D	-644 / 0	0.43 (1)
E- F	-1200 / 0	-70.5	-70.5	0.16 (1)	5.69	K- E	0 / 436	0.07 (1)
F- G	0 / 25	-70.5	-70.5	0.16 (1)	10.00	J- E	0 / 259	0.04 (1)
M- A	-103 / 0	0.0	0.0	0.01 (1)	7.81	J- F	-135 / 0	0.13 (1)
I- G	-103 / 0	0.0	0.0	0.01 (1)	7.81	M- B	-1403 / 0	0.45 (1)
						F- I	-1403 / 0	0.45 (1)
N- M	0 / 0	-88.0	-88.0	0.10 (1)	10.00			
M- L	0 / 779	-17.5	-17.5	0.31 (4)	10.00			
L- K	0 / 704	-17.5	-17.5	0.31 (4)	10.00			
K- J	0 / 704	-17.5	-17.5	0.31 (4)	10.00			
J- I	0 / 779	-17.5	-17.5	0.31 (4)	10.00			
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND 6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.48 (C-D:1) , BC=0.31 (J-K:4) , WB=0.45 (F-I:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

A-15073679

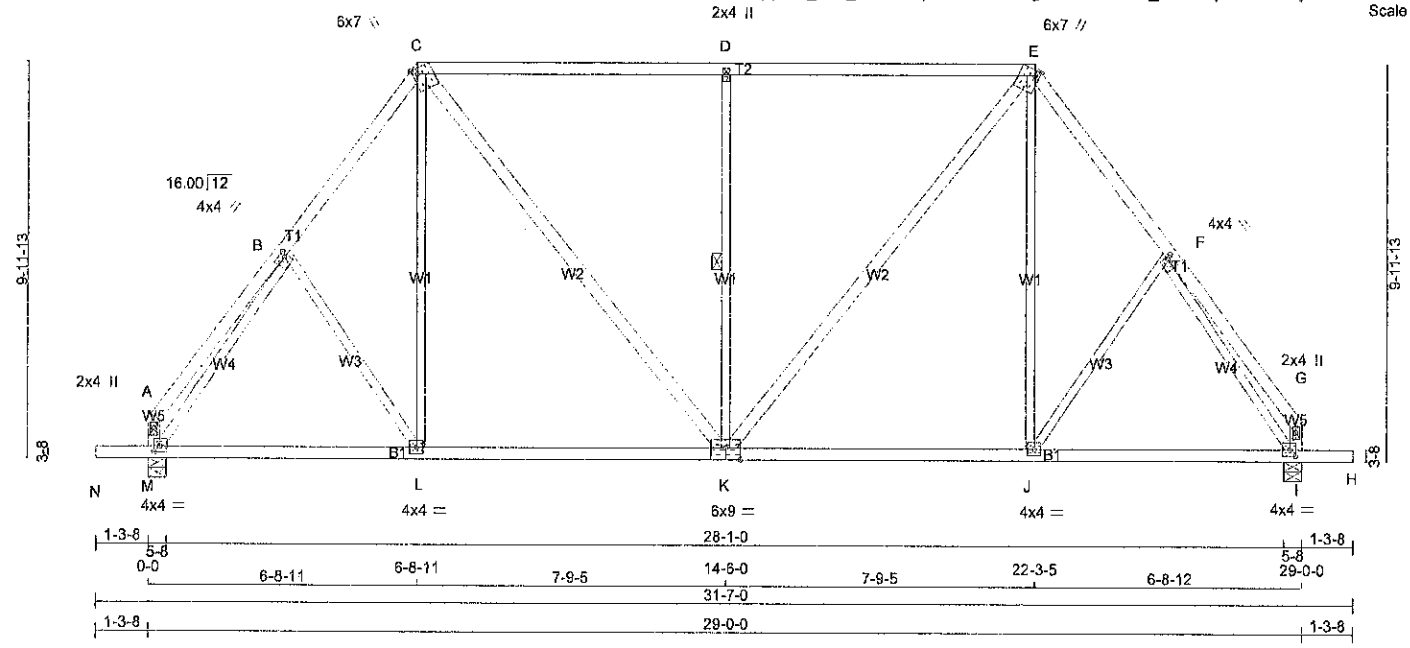
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:48 2015 Page 1

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-2h42gn5ScRQMG5_CMS2qiGNBGQKjFR4OwXNHdHyxoSI

Scale = 1:58.5



TOTAL WEIGHT = 2 X 151 = 302 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

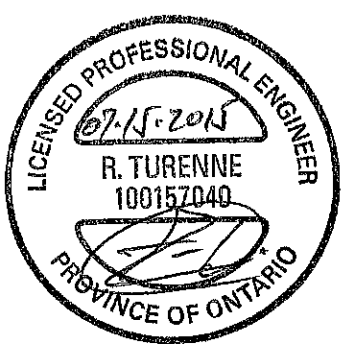
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.50	1.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMWW-t	MT20	4.0	4.0	1.50	1.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	1.75	1.75
J	BMWW-t	MT20	4.0	4.0		
K	BSWWW-I	MT20	6.0	9.0	Edge	4.50
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 21	-70.5	-70.5	0.12 (1)	10.00	B- L	-98 / 4	0.07 (1)
B- C	-1179 / 0	-70.5	-70.5	0.12 (1)	5.77	L- C	0 / 222	0.05 (4)
C- D	-1016 / 0	-70.5	-70.5	0.59 (1)	5.33	C- K	0 / 515	0.08 (1)
D- E	-1016 / 0	-70.5	-70.5	0.59 (1)	5.33	K- D	-670 / 0	0.45 (1)
E- F	-1179 / 0	-70.5	-70.5	0.12 (1)	5.77	K- E	0 / 515	0.08 (1)
F- G	0 / 21	-70.5	-70.5	0.12 (1)	10.00	J- E	0 / 222	0.05 (4)
M- A	-91 / 0	0.0	0.0	0.01 (1)	7.81	J- F	-98 / 4	0.07 (1)
I- G	-91 / 0	0.0	0.0	0.01 (1)	7.81	M- B	-1361 / 0	0.86 (1)
						F- I	-1361 / 0	0.86 (1)
N- M	0 / 0	-88.0	-88.0	0.10 (1)	10.00			
M- L	0 / 747	-17.5	-17.5	0.27 (4)	10.00			
L- K	0 / 693	-17.5	-17.5	0.27 (4)	10.00			
K- J	0 / 693	-17.5	-17.5	0.27 (4)	10.00			
J- I	0 / 747	-17.5	-17.5	0.27 (4)	10.00			
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.59 (C-D:1) , BC=0.27 (J-K:4) , WB=0.86 (B-M:1) , SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.52 (F) (INPUT = 1.00)

A-1073678

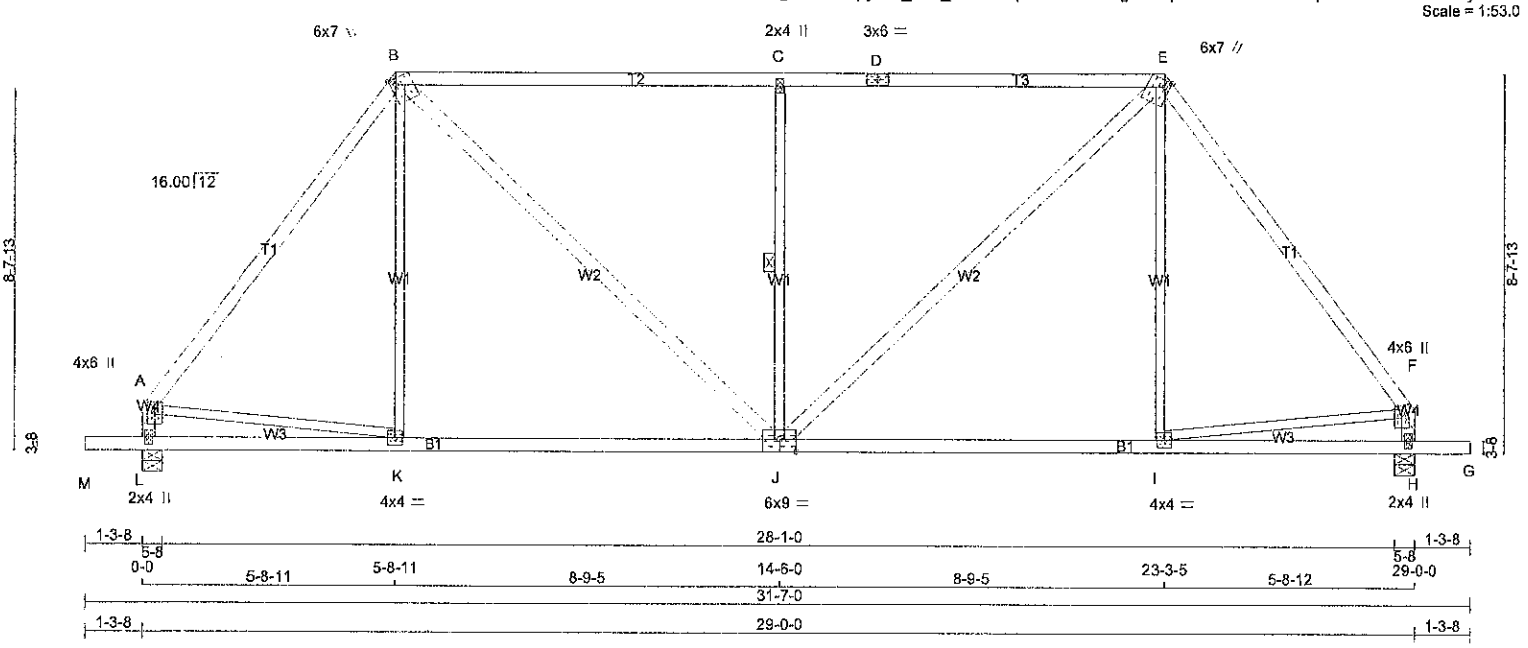
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H04	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:45 2015 Page 1

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-aVXgTR4qr8IWexP?olXbA3qxc0zXW6YFhtek5ryxoSrr

Scale = 1:53.0



TOTAL WEIGHT = 2 X 135 = 271 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	1650F 1.5E	SPF
D - E	2x4	DRY	1650F 1.5E	SPF
E - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
WEBS	2x3	DRY	No.2	SPF
EPT				
- J	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BSWWW-i	MT20	6.0	9.0	Edge	4.50
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1389	0	1389	0	0	5-8	1-8
H	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
H	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1199 / 0	-70.5	-70.5	0.57 (1)	5.24	K- B	0 / 131	0.05 (4)
B- C	-1198 / 0	-70.5	-70.5	0.90 (1)	4.79	B- J	0 / 664	0.11 (1)
C- D	-1200 / 0	-70.5	-70.5	0.90 (1)	4.79	J- C	-756 / 0	0.36 (1)
D- E	-1200 / 0	-70.5	-70.5	0.90 (1)	4.79	J- E	0 / 664	0.11 (1)
E- F	-1199 / 0	-70.5	-70.5	0.57 (1)	5.24	I- E	0 / 131	0.05 (4)
L- A	-1240 / 0	0.0	0.0	0.12 (1)	7.21	A- K	0 / 720	0.16 (1)
H- F	-1240 / 0	0.0	0.0	0.12 (1)	7.21	I- F	0 / 720	0.16 (1)
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00			
L- K	0 / 0	-17.5	-17.5	0.21 (4)	10.00			
K- J	0 / 717	-17.5	-17.5	0.33 (4)	10.00			
J- I	0 / 717	-17.5	-17.5	0.33 (4)	10.00			
I- H	0 / 0	-17.5	-17.5	0.21 (4)	10.00			
H- G	0 / 0	-88.0	-88.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.0 PSF
TOTAL LOAD			=	31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.90 (C-E:1) , BC=0.33 (I-J:4) , WB=0.36 (C-J:1) , SSI=0.30 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

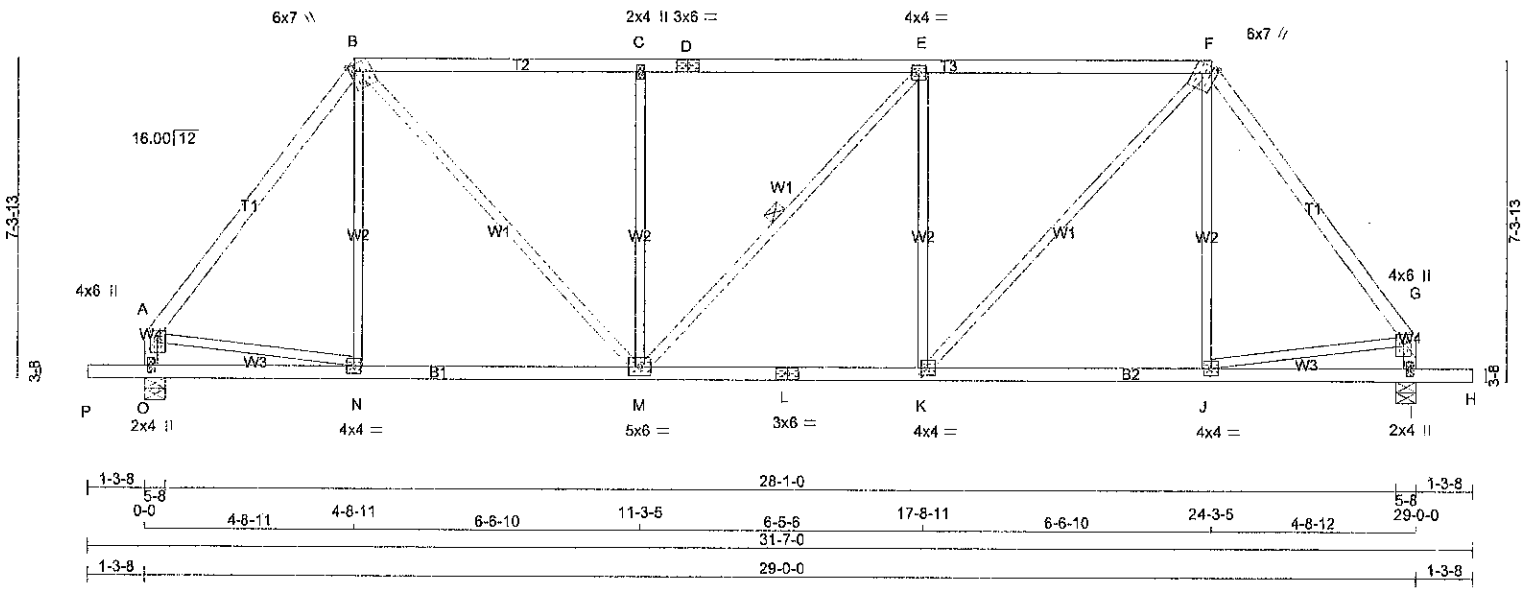
JSI GRIP= 0.75 (I) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)

A-1507 3677

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:45 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-aVXgTR4qr8lWexP?olXbA3q2B0_GW4wFhtek5ryxoSm
Scale = 1:53.0



TOTAL WEIGHT = 2 X 130 = 261 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
WEBS EPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1225 / 0	-70.5	-70.5	0.36 (1)	5.41	N- B	0 / 92	0.03 (4)
B- C	-1299 / 0	-70.5	-70.5	0.47 (1)	5.07	B- M	0 / 830	0.19 (1)
C- D	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06	M- C	-492 / 0	0.47 (1)
D- E	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06	M- E	-1 / 0	0.00 (1)
E- F	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06	K- E	-492 / 0	0.47 (1)
F- G	-1225 / 0	-70.5	-70.5	0.36 (1)	5.41	K- F	0 / 831	0.19 (1)
O- A	-1245 / 0	0.0	0.0	0.13 (1)	7.20	J- F	0 / 91	0.03 (4)
I- G	-1244 / 0	0.0	0.0	0.13 (1)	7.20	A- N	0 / 738	0.17 (1)
						J- G	0 / 738	0.17 (1)
P- O	0 / 0	-88.0	-88.0	0.10 (1)	10.00			
O- N	0 / 0	-17.5	-17.5	0.13 (4)	10.00			
N- M	0 / 732	-17.5	-17.5	0.20 (4)	10.00			
M- L	0 / 1301	-17.5	-17.5	0.29 (1)	10.00			
L- K	0 / 1301	-17.5	-17.5	0.29 (1)	10.00			
K- J	0 / 732	-17.5	-17.5	0.20 (4)	10.00			
J- I	0 / 0	-17.5	-17.5	0.13 (4)	10.00			
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.48 (E-F:1) , BC=0.29 (K-M:1) , WB=0.47 (E-K:1) , SSI=0.21 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

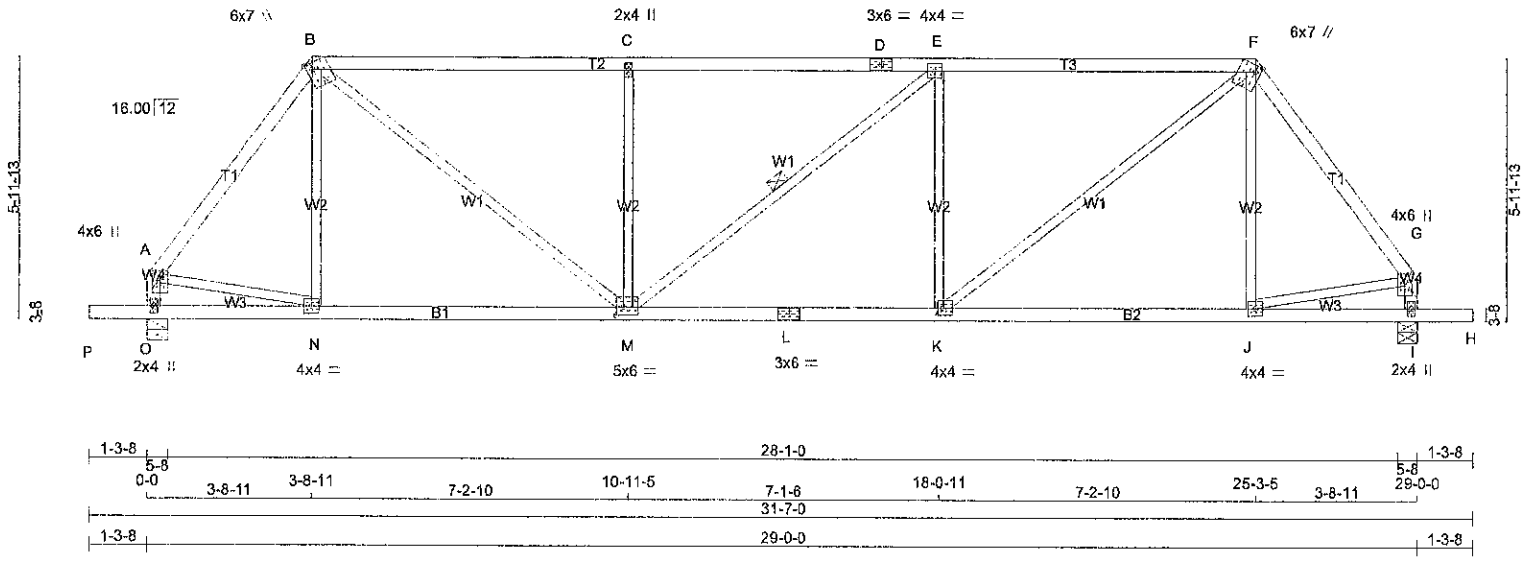
JSI GRIP= 0.79 (M) (INPUT = 0.90)
JSI METAL= 0.40 (L) (INPUT = 1.00)

A-1073676

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 15:04:44 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-6JzHF53B4qAf1nqpF1?MdrIr_dd1ngF5SDuAZOyxoSn
Scale = 1:53.0



TOTAL WEIGHT = 2 X 122 = 244 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
WEBS EPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0	2.25	2.00
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1251 / 0	-70.5	-70.5	0.21 (1)	5.53
B- C	-1600 / 0	-70.5	-70.5	0.63 (1)	4.48
C- D	-1600 / 0	-70.5	-70.5	0.63 (1)	4.47
D- E	-1600 / 0	-70.5	-70.5	0.63 (1)	4.47
E- F	-1601 / 0	-70.5	-70.5	0.64 (1)	4.47
F- G	-1250 / 0	-70.5	-70.5	0.21 (1)	5.53
O- A	-1257 / 0	0.0	0.0	0.13 (1)	7.17
I- G	-1257 / 0	0.0	0.0	0.13 (1)	7.17
P- O	0 / 0	-88.0	-88.0	0.10 (1)	10.00
O- N	0 / 0	-17.5	-17.5	0.14 (4)	10.00
N- M	0 / 746	-17.5	-17.5	0.24 (4)	10.00
M- L	0 / 1601	-17.5	-17.5	0.35 (1)	10.00
L- K	0 / 1601	-17.5	-17.5	0.35 (1)	10.00
K- J	0 / 745	-17.5	-17.5	0.24 (4)	10.00
J- I	0 / 0	-17.5	-17.5	0.14 (4)	10.00
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-12 / 81	0.03 (4)
B- M	0 / 1085	0.24 (1)
M- C	-542 / 0	0.30 (1)
M- E	-1 / 0	0.00 (1)
K- E	-543 / 0	0.30 (1)
K- F	0 / 1086	0.24 (1)
J- F	-12 / 81	0.03 (4)
A- N	0 / 756	0.17 (1)
J- G	0 / 756	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.64 (E-F:1) , BC=0.35 (K-M:1) , WB=0.30 (E-K:1) , SSI=0.24 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.49 (L) (INPUT = 1.00)

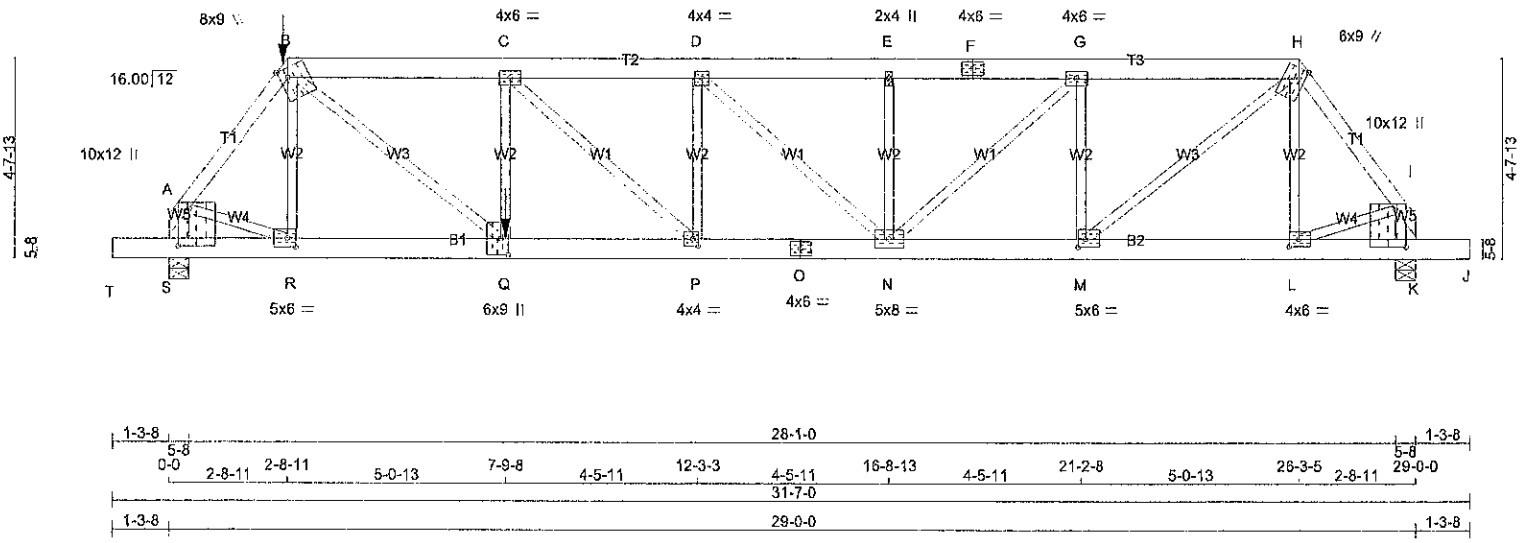
A-H073675

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H01-Cond2	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-e6Pv2I3ZJW2oPdFdhKU74eljlDDQ25OyEZ9d0yyxoSo

Scale = 1:54.1



TOTAL WEIGHT = 2 X 158 = 317 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
S - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - O	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
WEBS EPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMVW*+p	MT20	10.0	12.0	10.25	3.00
B	TTWW+m	MT20	8.0	9.0	2.50	2.25
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	2.75	1.75
I	TMBMVW*+p	MT20	10.0	12.0	10.25	3.00
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BMWW-t	MT20	5.0	6.0	2.25	2.00
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BMWW+t	MT20	6.0	9.0	4.50	2.00
R	BMWW-t	MT20	5.0	6.0	2.50	2.25

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3026	0	3026	0	0	5-8	4-2
K	1892	0	1892	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2117	1527 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0
K	1327	936 / 0	0 / 0	0 / 0	0 / 0	391 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

MEMB. FR-TO	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-3070 / 0	-70.5	-70.5	0.26 (1)	3.77	R- B	-136 / 92	0.04 (1)
B- C	-4296 / 0	-138.7	-138.7	0.41 (1)	3.79	B- Q	0 / 3191	0.79 (1)
C- D	-4175 / 0	-70.5	-70.5	0.24 (1)	4.04	Q- C	-444 / 0	0.13 (1)
D- E	-3633 / 0	-70.5	-70.5	0.18 (1)	4.35	C- P	-166 / 25	0.11 (1)
E- F	-3633 / 0	-70.5	-70.5	0.18 (1)	4.35	P- D	0 / 217	0.05 (1)
F- G	-3633 / 0	-70.5	-70.5	0.18 (1)	4.35	D- N	-743 / 0	0.47 (1)
G- H	-2698 / 0	-70.5	-70.5	0.18 (1)	4.90	N- E	-302 / 0	0.09 (1)
H- I	-1831 / 0	-70.5	-70.5	0.16 (1)	4.79	N- G	0 / 1282	0.32 (1)
S- A	-2907 / 0	0.0	0.0	0.21 (1)	6.12	M- G	-1247 / 0	0.36 (1)
K- I	-1772 / 0	0.0	0.0	0.13 (1)	7.45	M- H	0 / 2081	0.52 (1)
						L- H	-114 / 30	0.03 (1)
T- S	0 / 0	-88.0	-88.0	0.05 (1)	10.00	A- R	0 / 1873	0.46 (1)
S- R	0 / 0	-34.5	-34.5	0.09 (1)	10.00	L- I	0 / 1117	0.28 (1)
R- Q	0 / 1836	-34.5	-34.5	0.32 (1)	10.00			
Q- P	0 / 4296	-17.5	-17.5	0.63 (1)	10.00			
P- O	0 / 4175	-17.5	-17.5	0.59 (1)	10.00			
O- N	0 / 4175	-17.5	-17.5	0.59 (1)	10.00			
N- M	0 / 2698	-17.5	-17.5	0.39 (1)	10.00			
M- L	0 / 1094	-17.5	-17.5	0.18 (1)	10.00			
L- K	0 / 0	-17.5	-17.5	0.04 (4)	10.00			
K- J	0 / 0	-88.0	-88.0	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-8-11	-140	-140	---	FRONT	VERT.	TOTAL
Q	7-9-8	-1524	-1524	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 7-9-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.41 (B-C:1) , BC=0.63 (P-Q:1) , WB=0.79 (B-Q:1) , SSI=0.25 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

A-H0736766

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H01-Cond2	2	1	TRUSS DESC.	

HANGERS NOTES
SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 139.9 lbs FACTORED DOWN AT 2-8-11 ON TOP CHORD, AND 1524.6 lbs FACTORED DOWN AT 7-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.85 (O) (INPUT = 1.00)

A-4073674(2)

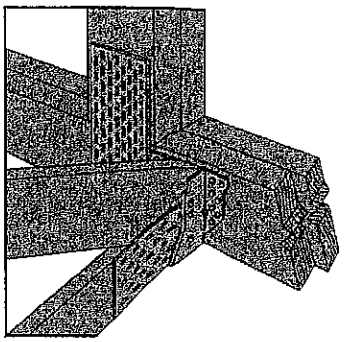
FACE MOUNT HANGERS

SIMPSON Strong-Tie

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Model No.	Ga.	Dimensions (in) W	H	B	d_e	Fasteners Header	Joist	Factored Resistance D Fir-L Uplift (K_D = 1.15) lbs kN	D Fir-L Normal (K_D = 1.00) lbs kN	S-P-F Uplift (K_D = 1.15) lbs kN	S-P-F Normal (K_D = 1.00) lbs kN
DOUBLE 2x SIZES											
LUS24-2	18	3⅛	3⅞	2	1½	4-16d	2-16d	835 3.71	2020 8.99	590 2.62	1435 6.38
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS26-2	14	3⁹⁄₁₆	5⅜	3	3¹⁵⁄₁₆	14-16d	6-16d	2850 12.68	7335 32.63	2065 9.20	5205 23.15
HGUS26-2	12	3⁹⁄₁₆	5⅞	4	4⅛	20-16d	8-16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS28-2	14	3⁹⁄₁₆	7⅜	3	6⅛	22-16d	8-16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS28-2	12	3⁹⁄₁₆	7⅞	4	6⅛	36-16d	12-16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HHUS210-2	14	3⁹⁄₁₆	9⅜	3	8	30-16d	10-16d	4745 21.11	9680 42.97	4310 19.17	7000 31.14
HGUS210-2	12	3⁹⁄₁₆	9⅞	4	8⅛	46-16d	16-16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
TRIPLE 2x SIZES											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	20-16d	8-16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	36-16d	12-16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-3	14	4⅝	9	3	7⁹⁄₁₆	30-16d	10-16d	4745 21.11	10545 46.91	4310 19.17	7485 33.29
HGUS210-3	12	4⅝	9¼	4	8⅛	46-16d	16-16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
QUADRUPLE 2x SIZES											
HGUS26-4	12	6⁹⁄₁₆	5⅞	4	4⅛	20-16d	8-16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-4	12	6⁹⁄₁₆	7⅜	4	6⅛	36-16d	12-16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-4	14	6⅛	8⅞	3	7⁹⁄₁₆	30-16d	10-16d	4745 21.11	10545 46.91	4310 19.17	7485 33.29
HGUS210-4	12	6⅛	9¾	4	8⅛	46-16d	16-16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅜	4	10⅛	56-16d	20-16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS214-4	12	6⅛	12⅜	4	11⅞	66-16d	22-16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80
4x SIZES											
LUS46	18	3⁹⁄₁₆	4⅞	2	3⅛	4-16d	4-16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS46	14	3⁹⁄₁₆	5⅜	3	3¹⁵⁄₁₆	14-16d	6-16d	2540 11.30	7335 32.63	2065 9.20	5205 23.15
HGUS46	12	3⅝	5⅞	4	4⅛	20-16d	8-16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS48	18	3⁹⁄₁₆	6¾	2	3⅛	6-16d	4-16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	22-16d	8-16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	36-16d	12-16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS410	18	3⁹⁄₁₆	8⅞	2	5⅛	8-16d	6-16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HGUS410	12	3⅝	9	4	8⅛	46-16d	16-16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS412	12	3⅝	10⅛	4	10⅛	56-16d	20-16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS414	12	3⅝	12⅜	4	11⅞	66-16d	22-16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80

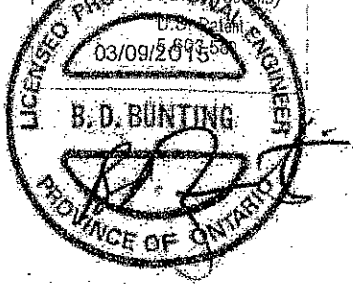


Typical HUS26 Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Double Shear Nailing Top View

Double Shear Nailing Side View Do not bend tab back

Dome Double Shear Nailing prevents tabs breaking off *(Available on select models)*



1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-N10FORTRUSS and/or see installation notes.
5. **NAILS:** 16d = 0.162" dia. x 3½" long. See pages 22-23 for other nail sizes and information.

LUL/LUS/LJS/HUS/HHUS/HGUS *Standard & Double Shear Joist Hangers*

SIMPSON Strong-Tie

VALUE ENGINEERED

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double shear nailing – an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. *(Do not bend or remove tabs)*

Double shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

MATERIAL: See tables below and on page 155

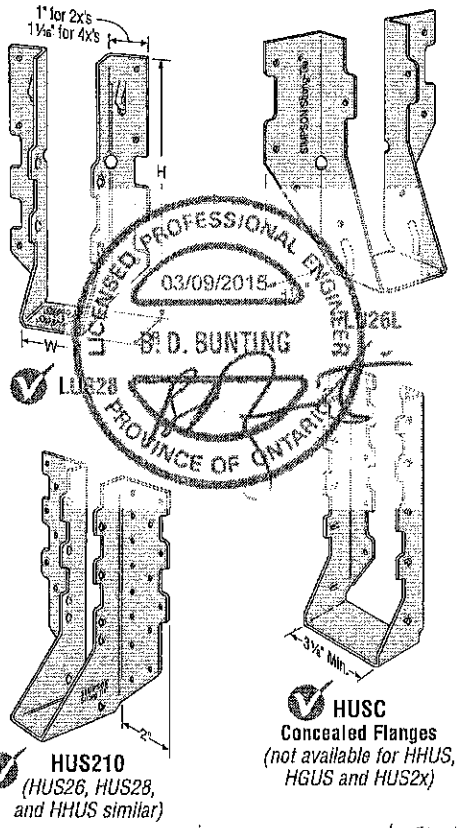
FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

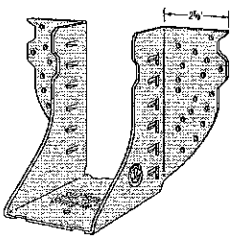
INSTALLATION: • Use all specified fasteners. See General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances *(except LUL)*.
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With 3x carrying members, use 16dx2½" nails into the header and 16d commons into the joist with no reduction in resistance. With 2x carrying members, use 10dx1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value.

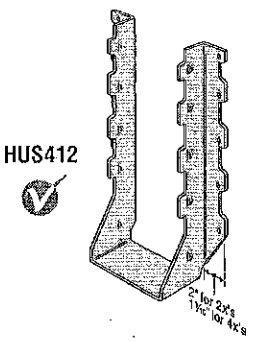
OPTIONS: • LUS and LUL hangers cannot be modified.
- HUS hangers available with the header flanges turned in for 2-2x (3⅛") and 4x only, with no load reduction. See HUSC Concealed Flange illustration.
- Concealed flanges are not available for HGUS.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See hanger options on page 230.

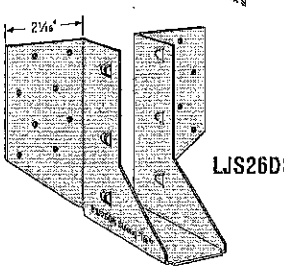
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.





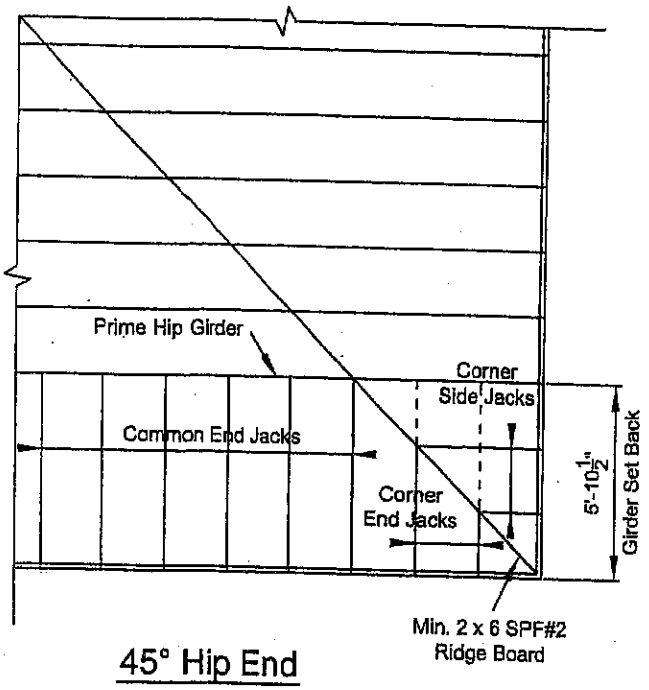




Model No.	Ga	Dimensions (in) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lbs / kN	D.Fir-L Normal (K_D = 1.00) lbs / kN	S-P-F Uplift (K_D = 1.15) lbs / kN	S-P-F Normal (K_D = 1.00) lbs / kN
SINGLE 2x SIZES											
LUS24	18	1⁹⁄₁₆	3⅛	1¾	2¼	4-10d	2-10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⁹⁄₁₆	3	1⅝	2¹¹⁄₁₆	4-10d	2-10dx1½	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⁹⁄₁₆	5	1⅝	4⅝	6-10d	4-10dx1½	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⁹⁄₁₆	4¾	1¾	3¾	4-10d	4-10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	14-16d	6-16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	16-16d	6-16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅛	20-16d	8-16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1⁹⁄₁₆	6¾	1⅝	5⅞	8-10d	6-10dx1½	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	6-10d	4-10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	18	1⅝	7¹⁄₁₆	3	6¹⁄₁₆	22-16d	8-16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⁹⁄₁₆	8	1⅝	7⅝	10-10d	6-10dx1½	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⁹⁄₁₆	7¹⁵⁄₁₆	1¾	3⅞	8-10d	4-10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

See footnotes on page 155.

STRACON ENGINEERING INC.



45° Hip End

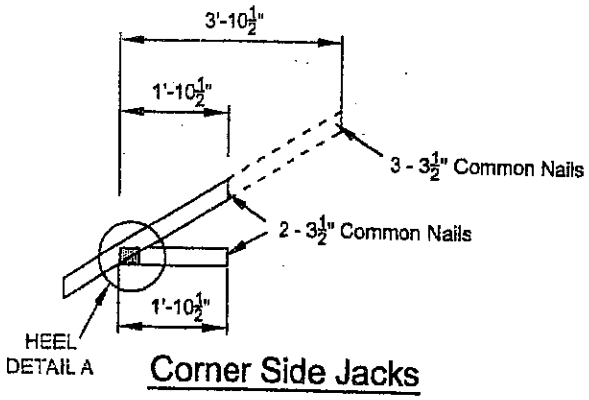
LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

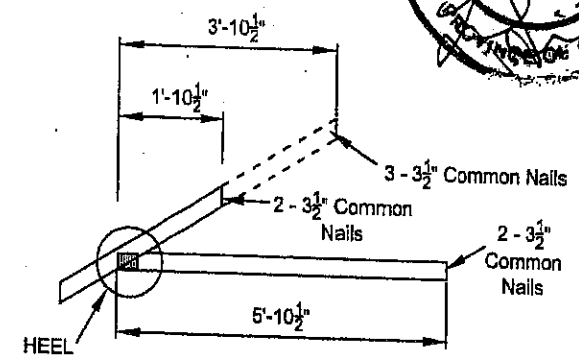
UNLESS OTHERWISE SHOWN

DESIGN LOAD

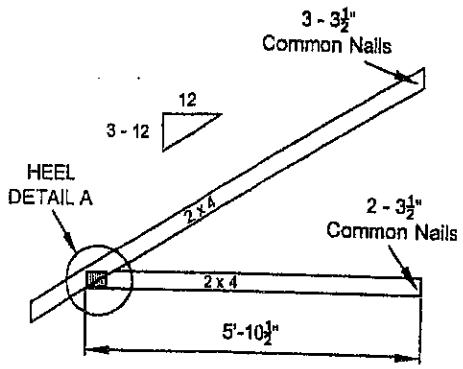
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



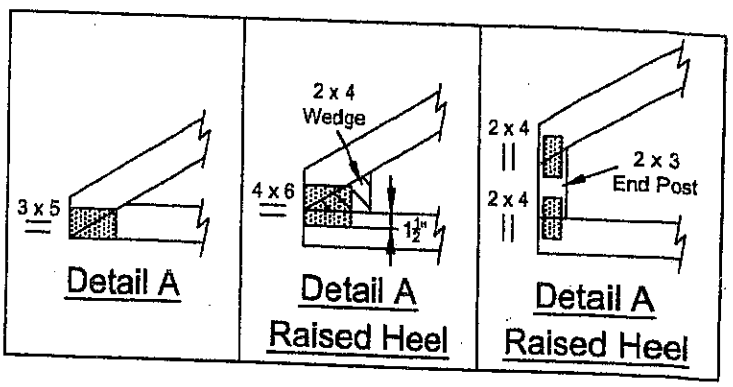
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008