

MODEL : 38-3
ELEVATION A

SECOND FLOOR FRAMING

Ceramic Tile
Ceramic Tile

JT: 40297/79712
File: 253719

Builder: GOLD PARK
Project: HUNTINGTON & NASHVILLE

Location: KLEINBURG
Date: May 12/15

Designer: MO
Sheet: 1 of 4

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek Frankfort
Home Lumber

S.129491-S.129507

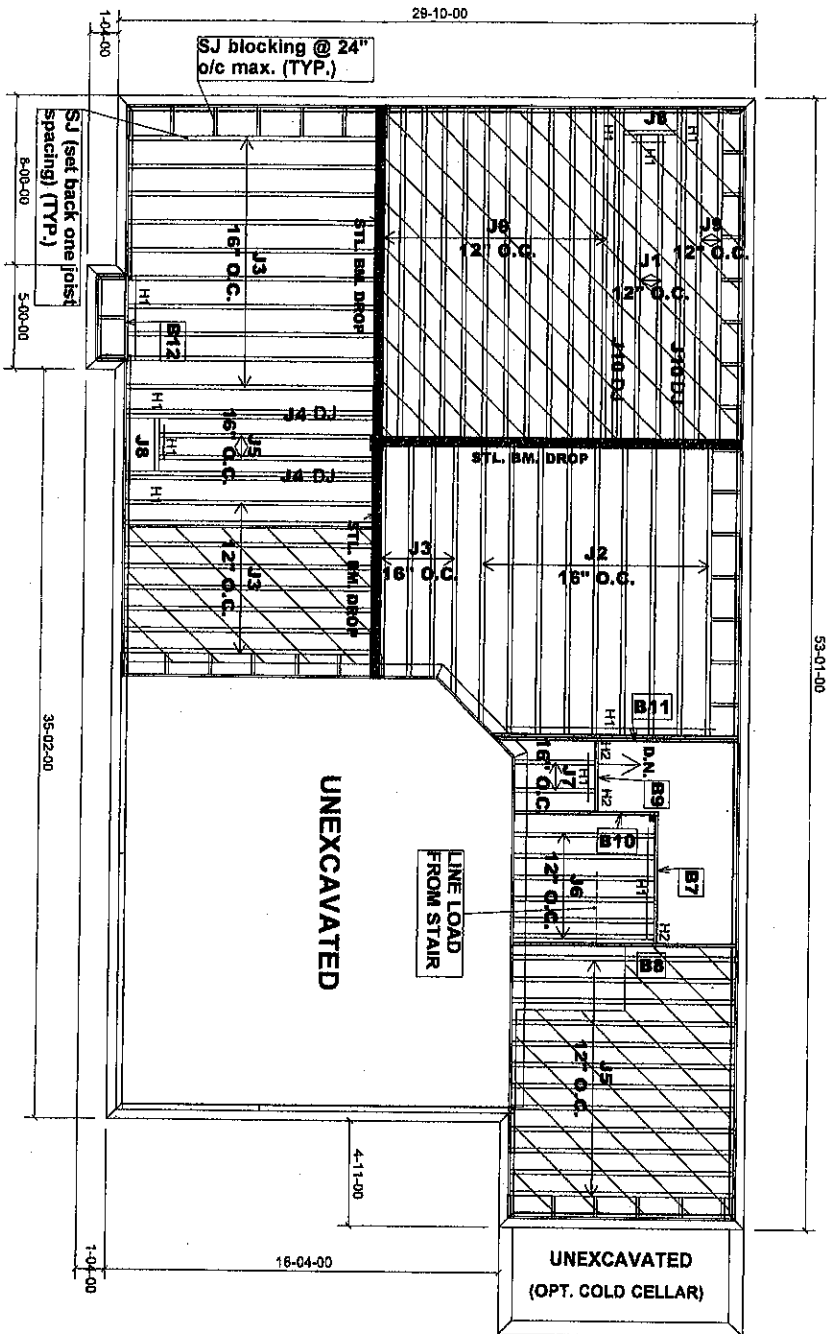
Product	Length	Product	Piles	Net Qty
J1	16'-00"-00	9 1/2" NI-20	1	16
J2	14'-00"-00	9 1/2" NI-20	1	10
J3	12'-00"-00	9 1/2" NI-20	1	23
J4	11'-00"-00	9 1/2" NI-20	1	12
J5	10'-00"-00	9 1/2" NI-20	1	27
J6	9'-00"-00	9 1/2" NI-20	1	9
J7	7'-00"-00	9 1/2" NI-20	1	1
B2	12'-00"-00	VERSALAM-10 2.0E	2	2
B3	11'-00"-00	VERSALAM-10 2.0E	1	1
B1	10'-00"-00	VERSALAM-10 2.0E	1	1
B6	10'-00"-00	VERSALAM-10 2.0E	3	3
B5	8'-00"-00	VERSALAM-10 2.0E	2	2
B4	5'-00"-00	VERSALAM-10 2.0E	1	1

RIMBOARD
1-1/8" X 9 1/2" O.S.B
SUBFLOOR - 5/8" NAILED & GLUED
APP - AS PER PLAN
BBO - BEAM BY OTHERS

HANGERS SCHEDULE

H1	1.7259
H2	HUS1.81/10
H3	HGUS410

1-2x6 SPF #2 Squash Block req'd on one side of joists under interior load bearing wall
Multiple squash blocks are required under concentrated loads
Joists spacing under ceramic tile is 12" o/c
Ceramic tiles application is as per O.B.C 9.30.6
Do not scale - refer to architectural plans for dimensions



PileID	Length	Products		Piles	Net Qty
		Product			
J1	15-00-00	9 1/2" NI-20		1	2
J2	14-00-00	9 1/2" NI-20		1	9
J3	12-00-00	9 1/2" NI-20		1	22
J4	12-00-00	9 1/2" NI-20		2	4
J5	11-00-00	9 1/2" NI-20		1	14
J6	7-00-00	9 1/2" NI-20		1	6
J7	4-00-00	9 1/2" NI-20		1	2
J8	3-00-00	9 1/2" NI-20		1	2
J9	16-00-00	9 1/2" NI-40x		1	14
J10	16-00-00	9 1/2" NI-40x		2	4
B11	12-00-00	VERSALAM-10 2.0E		2	2
B8	11-00-00	VERSALAM-10 2.0E		1	1
B10	7-00-00	VERSALAM-10 2.0E		1	1
B7	7-00-00	VERSALAM-10 2.0E		1	1
B12	5-00-00	VERSALAM-10 2.0E		1	1
B9	4-00-00	VERSALAM-10 2.0E		1	1

BLOCKING: 9 1/2" NI-20 - 45 FEET

HANGERS SCHEDULE

H1 - LT259
H2 - HUS1.8/1/10

RIMBOARD

1-1/8" X 9 1/2" O.S.B

SUBFLOOR - 5/8" NAILED & GLUED

APP - AS PER PLAN

BBO - BEAM BY OTHERS

1-2x6 SPF#2 Squash Block req'd on one side of joists under interior load bearing wall

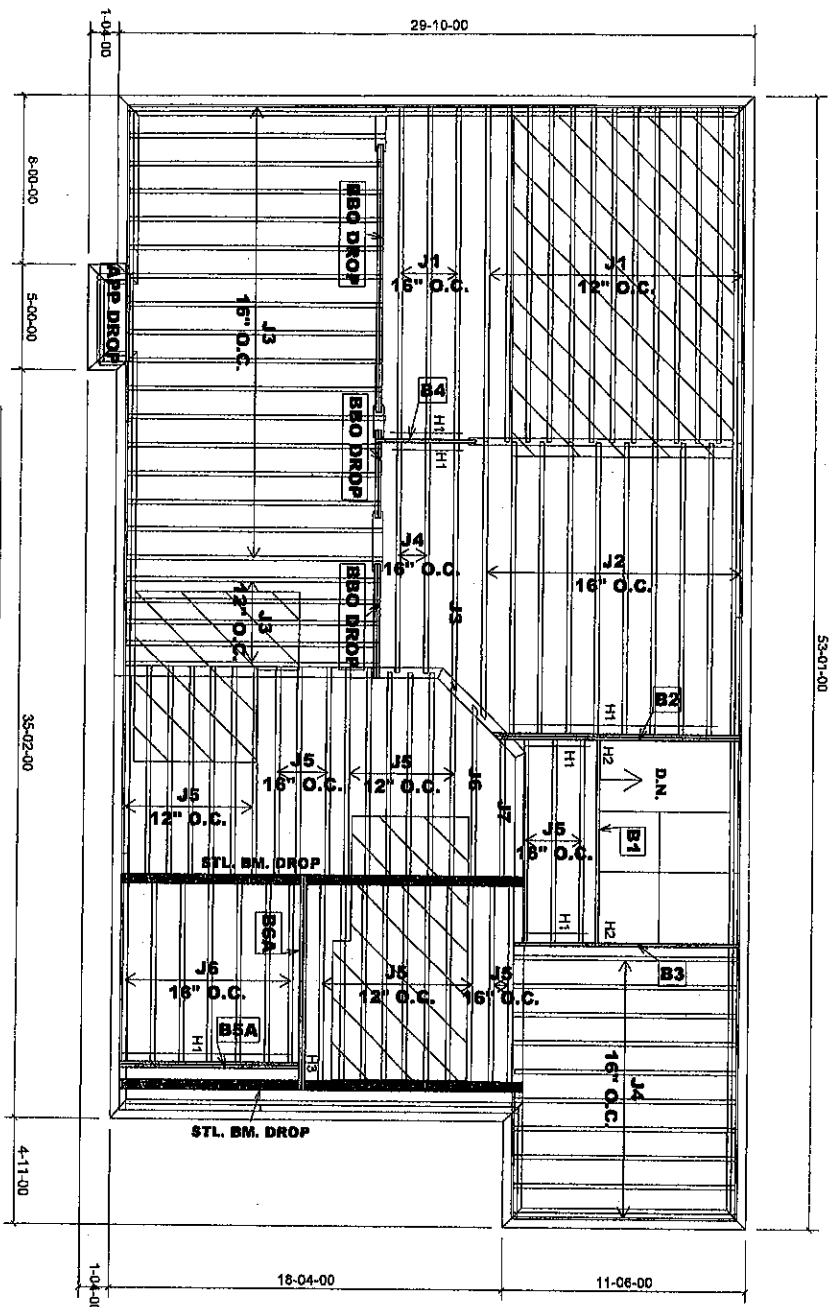
Multiple squash blocks are required under concentrated loads

Joists spacing under ceramic tile is 12" o/c

Ceramic tiles application is as per O.B.C. 9.30.6

Do not scale - refer to architectural plans for dimensions

JT: 40297/79712 Builder: GOLD PARK Location: KLEINBURG Designer: MQ
File: 253719 Project: HUNTINGTON & NASHVILLE Date: May 12/15 Sheet: 2 of 4
Alpa Roof Trusses Inc. Salesperson: Derek Frankfort
Maple, Ontario Home Lumber



Products				
ProdID	Length	Product	Piles	Net Qty
J1	16-00-00	9 1/2" NI-20	1	16
J2	14-00-00	9 1/2" NI-20	1	10
J3	12-00-00	9 1/2" NI-20	1	23
J4	11-00-00	9 1/2" NI-20	1	12
J5	10-00-00	9 1/2" NI-20	1	29
J6	9-00-00	9 1/2" NI-20	1	8
J7	7-00-00	9 1/2" NI-20	1	1
B2	12-00-00	VERSALAM-10 2.0E	2	2
B3	11-00-00	VERSALAM-10 2.0E	1	1
B6A	11-00-00	VERSALAM-10 2.0E	2	2
B1	10-00-00	VERSALAM-10 2.0E	1	1
B5A	9-00-00	VERSALAM-10 2.0E	2	2
B4	5-00-00	VERSALAM-10 2.0E	1	1

RIMBOARD

1-1/8" X 9 1/2" O.S.B

SUBFLOOR - 5/8" NAILLED & GLUED

APP - AS PER PLAN

BBO - BEAM BY OTHERS

FLANGERS SCHEDULE

H1 - L1759

H2 - HUS1.81/10

H3 - HGUS410

1-2x6 SPF#2 Squash Block req'd on one side of joists under interior load bearing wall

Multiple squash blocks are required under concentrated loads

Joists spacing under ceramic tile is 12" o/c

Ceramic tiles application is as per O.B.C 9.30.6

Do not scale - refer to architectural plans for dimensions

JT: 40297/79712
File: 253719

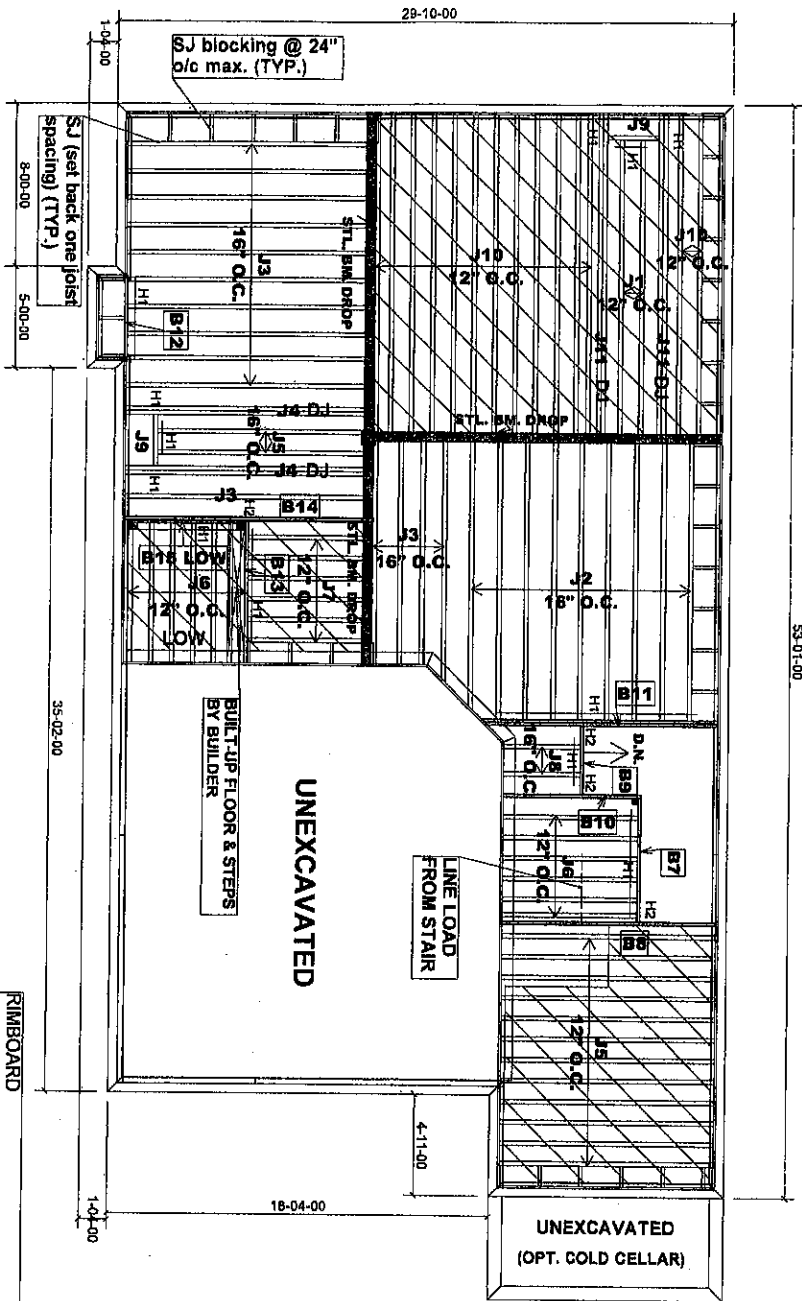
Builder: GOLD PARK
Project: HUNTINGTON & NASHVILLE

Location: KLEINBURG
Date: May 12/15

Designer: MQ
Sheet: 3 of 4

Alpha Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek Frankfort
Home Lumber



MODEL: 383
ELEVATION A & B
W/OPT. SUNKEN MUD ROOM

GROUND FLOOR FRAMING

Ceramic Tile
Ceramic Tile

RIMBOARD
1-1/8" X 9 1/2" O.S.B
SUBFLOOR - 5/8" NAILED & GLUED
APP - AS PER PLAN
BBO - BEAM BY OTHERS

BUILT-UP FLOOR & STEPS
BY BUILDER

LINE LOAD
FROM STAIR

UNEXCAVATED
(OPT. COLD CELLAR)

Product	Length	Product	Piles	Net Qty
J1	15-00-00	9 1/2" NI-20	1	2
J2	14-00-00	9 1/2" NI-20	1	9
J3	12-00-00	9 1/2" NI-20	1	15
J4	12-00-00	9 1/2" NI-20	2	4
J5	11-00-00	9 1/2" NI-20	1	14
J6	7-00-00	9 1/2" NI-20	1	13
J7	6-00-00	9 1/2" NI-20	1	6
J8	4-00-00	9 1/2" NI-20	1	2
J9	3-00-00	9 1/2" NI-20	1	2
J10	16-00-00	9 1/2" NI-40x	1	14
J11	16-00-00	9 1/2" NI-40x	2	4
B14	12-00-00	VERSALAM-10.2.0E	1	1
B11	12-00-00	VERSALAM-10.2.0E	2	2
B8	11-00-00	VERSALAM-10.2.0E	1	1
B10	7-00-00	VERSALAM-10.2.0E	1	1
B13	7-00-00	VERSALAM-10.2.0E	1	1
B7	7-00-00	VERSALAM-10.2.0E	1	1
B15 LOW	6-00-00	VERSALAM-10.2.0E	1	1
B12	5-00-00	VERSALAM-10.2.0E	1	1
B9	4-00-00	VERSALAM-10.2.0E	1	1

BLOCKING: 9 1/2" NI-20 - 45 FEET

HANGERS SCHEDULE

H1 - LT259
H2 - HUST.81/10

1-2x6 SPF#2 Squash Block req'd on one side of joists under interior load bearing wall

Multiple squash blocks are required under concentrated loads

Joists spacing under ceramic tile is 12" o/c

Ceramic tiles application is as per O.B.C 9.30.6

Do not scale - refer to architectural plans for dimensions

JT: 40297/79712 Builder: GOLD PARK Location: KLEINBURG Designer: MQ Date: May 12/15 Sheet: 4 of 4
File: 253719 Project: HUNTINGTON & NASHVILLE Date: May 12/15
Alpa Roof Trusses Inc. Salesperson: Derek Frankfort
Maple, Ontario Home Lumber

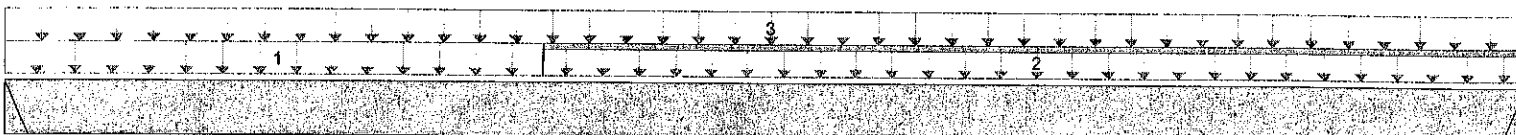
BC CALC® Design Report


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\01
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



B0

09-06-00

B1

Total Horizontal Product Length = 09-06-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0	958 / 0	501 / 0		
B1	295 / 0	385 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Unf. Area (lb/ft^2)	L	00-00-00	03-04-00	40	15			07-06-00
2		Unf. Lin. (lb/ft)	L	03-04-00	09-06-00	0	60			n/a
3		Unf. Area (lb/ft^2)	L	00-00-00	09-06-00	40	15			00-08-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,068 ft-lbs	12,704 ft-lbs	0.24	1	03-02-03
End Shear	1,441 lbs	5,785 lbs	0.25	1	00-11-08
Total Load Defl.	L/848 (0.132")	0.465"	0.28	4	04-05-11
Live Load Defl.	L/999 (0.072")	n/a	n/a	5	04-03-12
Span / Depth	11.7	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Hanger	2" x 1-3/4"	2,063 lbs	n/a	0.48	Hanger
B1 Hanger	2" x 1-3/4"	923 lbs	n/a	0.22	Hanger

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

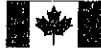
User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS
 @ / O.C., STAGGERED IN TWO ROWS

Disclosure

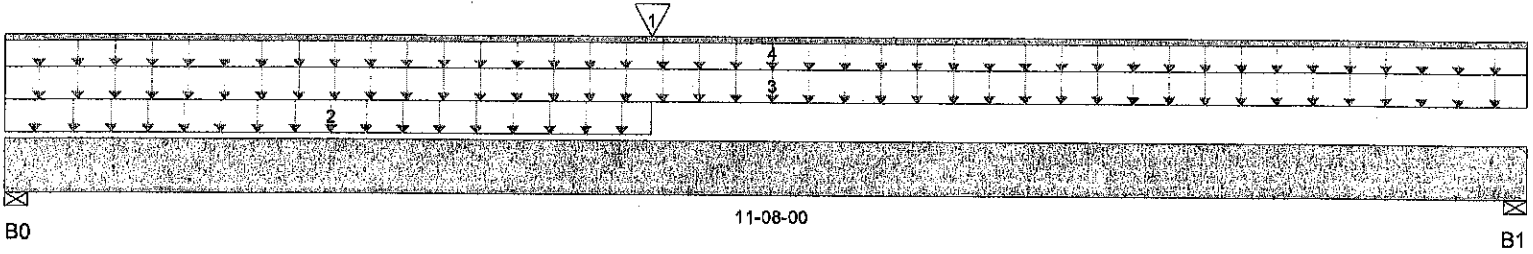
Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation. \n\nBC CALC®, BC FRAMER®, AJST™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.





Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\02
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 11-08-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	2,997 / 0	1,613 / 0		
B1, 3-1/2"	2,250 / 0	1,309 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Conc. Pt. (lbs)	L	04-11-00	04-11-00	958	501			n/a
2		Unf. Area (lb/ft^2)	L	00-00-00	04-11-00	40	15			05-00-00
3		Unf. Area (lb/ft^2)	L	00-00-00	11-08-00	40	15			07-01-00
4		Unf. Lin. (lb/ft)	L	00-00-00	11-08-00	0	60			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	18,001 ft-lbs	25,408 ft-lbs	0.71	1	04-11-00
End Shear	5,387 lbs	11,571 lbs	0.47	1	01-01-00
Total Load Defl.	L/249 (0.541")	0.56"	0.97	4	05-08-09
Live Load Defl.	L/387 (0.348")	0.374"	0.93	5	05-07-08
Span / Depth	14.2	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0	Wall/Plate 3-1/2" x 3-1/2"	6,512 lbs	0.86	0.44	Spruce Pine Fir
B1	Wall/Plate 3-1/2" x 3-1/2"	5,011 lbs	0.66	0.34	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS
 @ 12" O.C., STAGGERED IN TWO ROWS



S.129492



Build 3272

Job Name:

40297

Address:

HUNTINGTON & NASHVILLE

City, Province, Postal Code: KLEINBURG, ON

Customer:

GOLD PARK

Code reports:

CCMC 12472-R

File Name: 253719.bcc

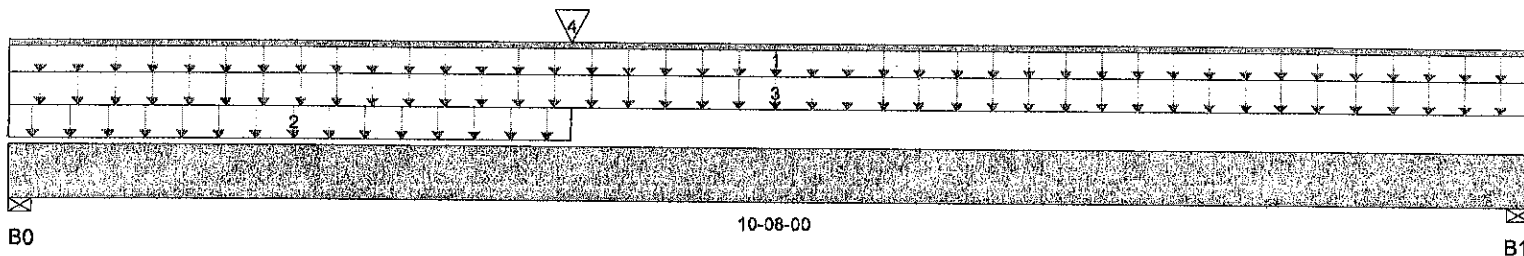
Description: Designs\03

Specifier: 38-3

Designer: MQ

Company: Alpa Roof Trusses Inc

Misc:



Total Horizontal Product Length = 10-08-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	981 / 0	889 / 0		
B1, 3-1/2"	382 / 0	588 / 0		

Load Summary

Tag Description	Load Type	Ref.	Start	End	Live	Dead	Snow	Wind	Trib.
1	Unf. Lin. (lb/ft)	L	00-00-00	10-08-00	0	60			n/a
2	Unf. Area (lb/ft^2)	L	00-00-00	03-11-00	40	15			05-00-00
3	Unf. Area (lb/ft^2)	L	00-00-00	10-08-00	40	15			00-08-00
4	Conc. Pt. (lbs)	L	03-11-00	03-11-00	295	385			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5,491 ft-lbs	12,704 ft-lbs	0.43	1	03-11-00
End Shear	2,011 lbs	5,785 lbs	0.35	1	01-01-00
Total Load Defl.	L/453 (0.27")	0.51"	0.53	4	04-11-12
Live Load Defl.	L/999 (0.123")	n/a	n/a	5	04-11-12
Max Defl.	0.27"	1"	0.27	4	04-11-12
Span / Depth	12.9	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	2,582 lbs	0.69	0.35	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	1,308 lbs	0.35	0.18	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.


User Notes

S.129493

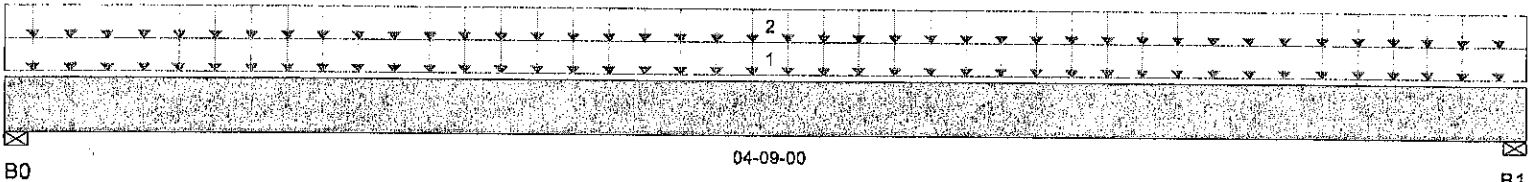
BC CALC® Design Report


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\04
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 04-09-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	1,354 / 0	519 / 0		
B1, 3-1/2"	1,354 / 0	519 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Unf. Area (lb/ft^2)	L	00-00-00	04-09-00	40	15			08-00-00
2		Unf. Area (lb/ft^2)	L	00-00-00	04-09-00	40	15			06-03-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,597 ft-lbs	12,704 ft-lbs	0.2	1	02-04-08
End Shear	1,457 lbs	5,785 lbs	0.25	1	01-01-00
Total Load Defl.	L/999 (0.024")	n/a	n/a	4	02-04-08
Live Load Defl.	L/999 (0.017")	n/a	n/a	5	02-04-08
Span / Depth	5.4	n/a	n/a		00-00-00

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation. \n\nBC CALC®, BC FRAMER®, AJST™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

Bearing Supports

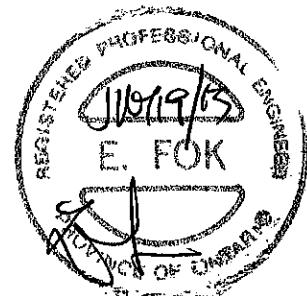
	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	2,679 lbs	0.71	0.36	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	2,679 lbs	0.71	0.36	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

User Notes

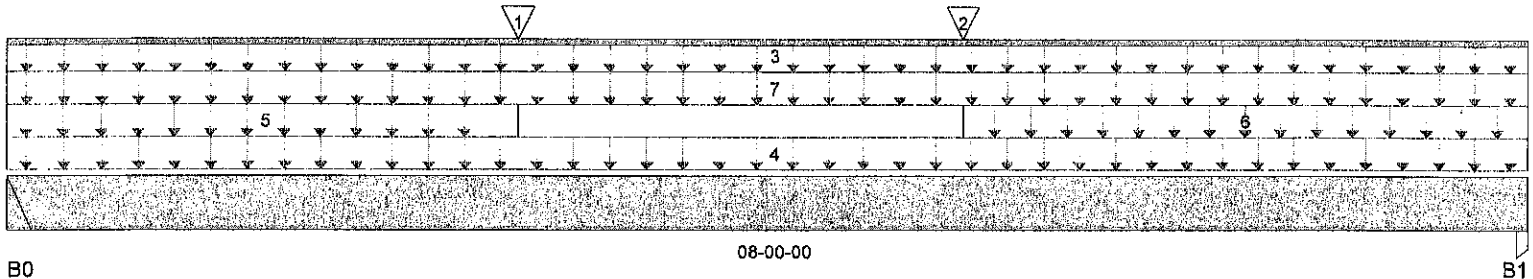
NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS
 @ O.C., STAGGERED IN TWO ROWS





Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\05
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 08-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0	1,855 / 0	2,033 / 0	2,754 / 0	
B1, 3-1/2"	1,910 / 0	2,093 / 0	2,832 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Conc. Pt. (lbs)	L	02-08-00	02-08-00	354	393	860		n/a
2		Conc. Pt. (lbs)	L	05-00-00	05-00-00	354	393	860		n/a
3		Unf. Lin. (lb/ft)	L	00-00-00	08-00-00	0	100			n/a
4		Unf. Area (lb/ft^2)	L	00-00-00	08-00-00	40	20			04-06-00
5		Unf. Area (lb/ft^2)	L	00-00-00	02-08-00	11	12	27		24-02-00
6		Unf. Area (lb/ft^2)	L	05-00-00	08-00-00	11	12	27		24-02-00
7		Unf. Area (lb/ft^2)	L	00-00-00	08-00-00	11	10	21		01-03-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	13,416 ft-lbs	25,408 ft-lbs	0.53	5	04-04-02
End Shear	5,814 lbs	11,571 lbs	0.5	5	00-11-08
Total Load Defl.	L/435 (0.211")	0.383"	0.55	13	03-11-12
Live Load Defl.	L/697 (0.132")	0.256"	0.52	17	03-11-12
Span / Depth	9.7	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Hanger	2" x 3-1/2"	7,601 lbs	n/a	0.89	Hanger
B1 Post	3-1/2" x 3-1/2"	7,820 lbs	n/a	0.52	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

User Notes NAIL ONE FOOT TO ANOTHER WITH 3/2" SPIRAL NAILS @ 12" O.C.,



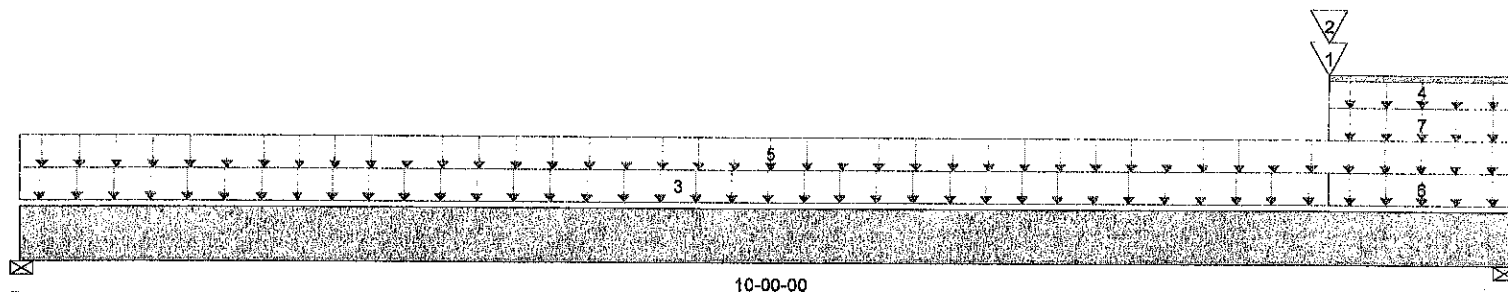
BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272
Job Name: 40297
Address: HUNTINGTON & NASHVILLE
City, Province, Postal Code: KLEINBURG, ON
Customer: GOLD PARK
Code reports: CCMC 12472-R

File Name: 253719.bcc
Description: Designs\06
Specifier: 38-3
Designer: MQ
Company: Alpa Roof Trusses Inc
Misc:



BO

B1

Total Horizontal Product Length = 10-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 2"	570 / 0	535 / 0	605 / 0	
B1, 4-3/4"	3,238 / 0	3,629 / 0	5,835 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	1.00	0.65	1.00	1.15
1		Conc. Pt. (lbs)	L	08-09-00	08-09-00	1,855	2,033	2,754	n/a
2		Conc. Pt. (lbs)	L	08-09-00	08-09-00	1,341	1,543	3,471	n/a
3		Unf. Area (lb/ft^2)	L	00-00-00	08-09-00	40	20		00-08-00
4		Unf. Lin. (lb/ft)	L	08-09-00	10-00-00	0	100		n/a
5		Unf. Area (lb/ft^2)	L	00-00-00	10-00-00	40	15		00-08-00
6		Unf. Area (lb/ft^2)	L	08-09-00	10-00-00	11	10	21	07-02-00
7		Unf. Area (lb/ft^2)	L	08-09-00	10-00-00	11	10	21	01-00-00

Controls Summary

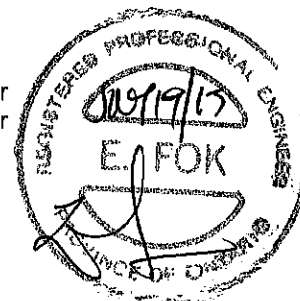
Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	13,261 ft-lbs	39,636 ft-lbs	0.33	5	08-09-00
End Shear	14,227 lbs	17,356 lbs	0.82	5	08-09-12
Total Load Defl.	L/768 (0.15")	0.478"	0.31	13	05-05-09
Live Load Defl.	L/999 (0.096")	n/a	n/a	17	05-07-00
Span / Depth	12.1	n/a	n/a		00-00-00

Bearing Supports

Bearing Supports			Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	2" x 5-1/4"	1,861 lbs	0.29	0.15	Spruce Pine Fir	
B1	Wall/Plate	4-3/4" x 5-1/4"	14,908 lbs	0.97	0.49	Spruce Pine Fir	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Calculations assume Member is Fully Braced.
Resistance Factor phi has been applied to all presented results per CSA 086.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
Design based on Dry Service Condition.
Importance Factor : Normal Part code : Part 4
Deflections less than 1/8" were ignored in the results.



User Notes: NAIL ONE PUT TO ANOTHER WITH $3\frac{1}{2}$ " SPIRAL NAILS @ 12" O.C. STRUTTED IN 2 ROWS S-129496
 Page 1 of 2

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272

Job Name:

40297

Address:

HUNTINGTON & NASHVILLE

City, Province, Postal Code: KLEINBURG, ON

Customer:

GOLD PARK

Code reports:

CCMC 12472-R

File Name: 253719.bcc

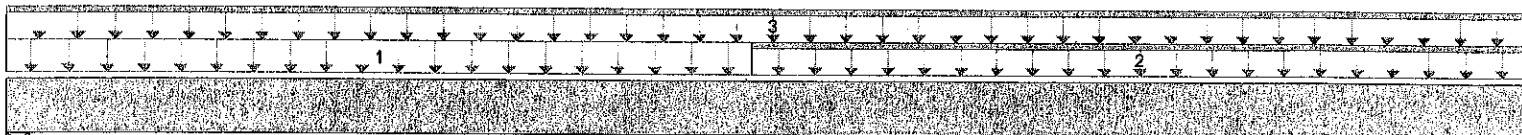
Description: Designs\07

Specifier: 38-3

Designer: MQ

Company: Alpha Roof Trusses Inc

Misc:



B0

06-02-00

B1

Total Horizontal Product Length = 06-02-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	608 / 0	434 / 0		
B1, 3-1/2"	993 / 0	592 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	Unf. Area (lb/ft ²)		L	00-00-00	03-00-00	40	15			03-06-00
2	Unf. Lin. (lb/ft)		L	03-00-00	06-02-00	373	148			n/a
3	Unf. Lin. (lb/ft)		L	00-00-00	06-02-00	0	60			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,522 ft-lbs	12,704 ft-lbs	0.2	1	03-05-08
End Shear	1,335 lbs	5,785 lbs	0.23	1	05-01-00
Total Load Defl.	L/999 (0.041")	n/a	n/a	4	03-02-06
Live Load Defl.	L/999 (0.025")	n/a	n/a	5	03-02-06
Max Defl.	0.041"	n/a	n/a	4	03-02-06
Span / Depth	7.2	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	1,454 lbs	0.39	0.19	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	2,230 lbs	0.59	0.3	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS

@ O.C., STAGGERED IN TWO ROWS

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation. \n\nBC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



BC CALC® Design Report


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272

Job Name: 40297

Address: HUNTINGTON & NASHVILLE

City, Province, Postal Code: KLEINBURG, ON

Customer: GOLD PARK

Code reports: CCMC 12472-R

File Name: 253719.bcc

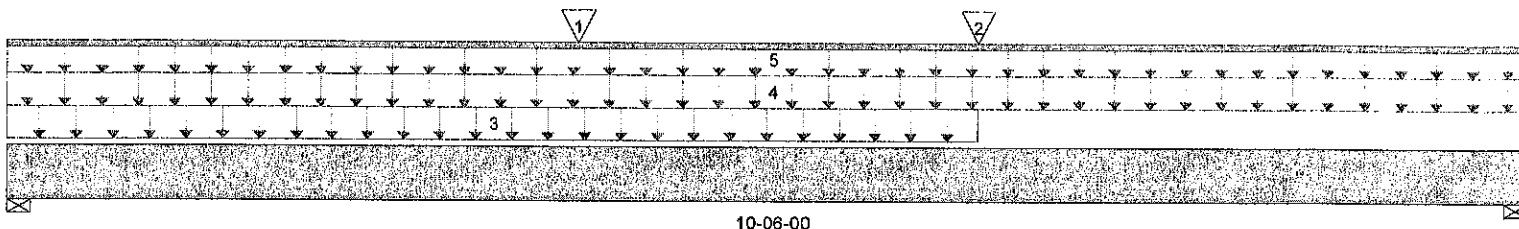
Description: Designs\08

Specifier: 38-3

Designer: MQ

Company: Alpa Roof Trusses Inc

Misc:



Total Horizontal Product Length = 10-06-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	872 / 0	764 / 0		
B1, 3-1/2"	978 / 0	865 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live	Dead	Snow	Wind	Trib.
1		Conc. Pt. (lbs)	L	03-11-00	03-11-00	399	150			n/a
2		Conc. Pt. (lbs)	L	06-08-00	06-08-00	993	592			n/a
3		Unf. Area (lb/ft^2)	L	00-00-00	06-08-00	40	15			00-08-00
4		Unf. Area (lb/ft^2)	L	00-00-00	10-06-00	40	20			00-08-00
5		Unf. Lin. (lb/ft)	L	00-00-00	10-06-00	0	60			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	8,180 ft-lbs	12,704 ft-lbs	0.64	1	06-08-00
End Shear	2,400 lbs	5,785 lbs	0.41	1	09-05-00
Total Load Defl.	L/302 (0.398")	0.502"	0.79	4	05-03-08
Live Load Defl.	L/539 (0.224")	0.335"	0.67	5	05-03-08
Max Defl.	0.398"	1"	0.4	4	05-03-08
Span / Depth	12.7	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	2,262 lbs	0.6	0.3	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	2,549 lbs	0.68	0.34	Spruce Pine Fir

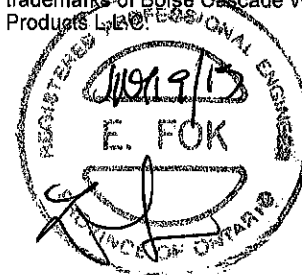
Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products Ltd.



BC CALC® Design Report

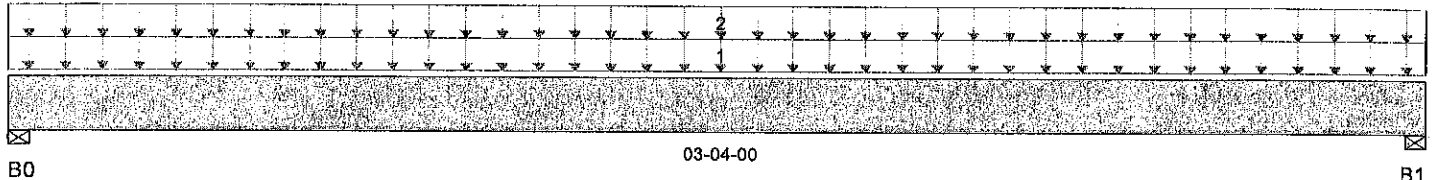


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\09
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 03-04-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	400 / 0	158 / 0		
B1, 3-1/2"	400 / 0	158 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Unf. Area (lb/ft^2)	L	00-00-00	03-04-00	40	15			02-06-00
2		Unf. Area (lb/ft^2)	L	00-00-00	03-04-00	40	15			03-06-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	494 ft-lbs	12,704 ft-lbs	0.04	1	01-08-00
End Shear	279 lbs	5,785 lbs	0.05	1	01-01-00
Total Load Defl.	L/999 (0.002")	n/a	n/a	4	01-08-00
Live Load Defl.	L/999 (0.001")	n/a	n/a	5	01-08-00
Max Defl.	0.002"	n/a	n/a	4	01-08-00
Span / Depth	3.6	n/a	n/a		00-00-00

Disclosure

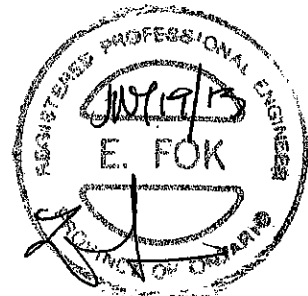
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Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	798 lbs	0.21	0.11	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	798 lbs	0.21	0.11	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.



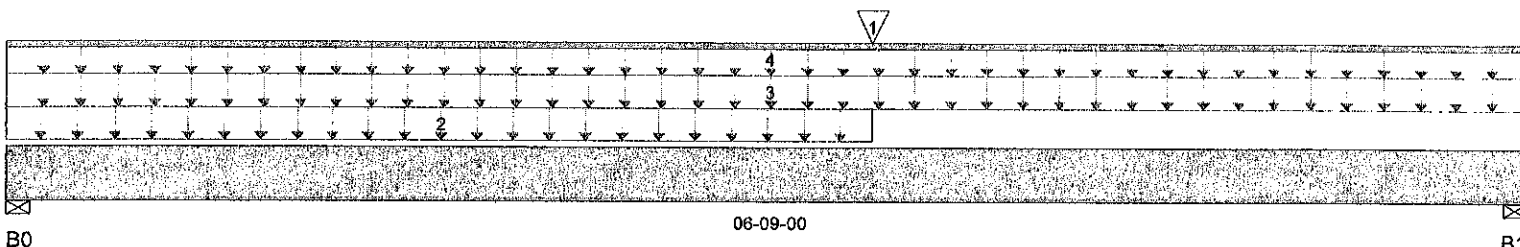
BC CALC® Design Report


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\10
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 06-09-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	335 / 0	348 / 0		
B1, 3-1/2"	347 / 0	353 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Conc. Pt. (lbs)	L	03-10-00	03-10-00	400	158			n/a
2		Unf. Area (lb/ft^2)	L	00-00-00	03-10-00	40	15			00-08-00
3		Unf. Area (lb/ft^2)	L	00-00-00	06-09-00	40	15			00-08-00
4		Unf. Lin. (lb/ft)	L	00-00-00	06-09-00	0	60			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,020 ft-lbs	12,704 ft-lbs	0.16	1	03-10-00
End Shear	817 lbs	5,785 lbs	0.14	1	05-08-00
Total Load Defl.	L/999 (0.037")	n/a	n/a	4	03-04-13
Live Load Defl.	L/999 (0.02")	n/a	n/a	5	03-05-14
Max Defl.	0.037"	n/a	n/a	4	03-04-13
Span / Depth	7.9	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	938 lbs	0.25	0.13	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	962 lbs	0.26	0.13	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

User Notes


BC CALC® Design Report

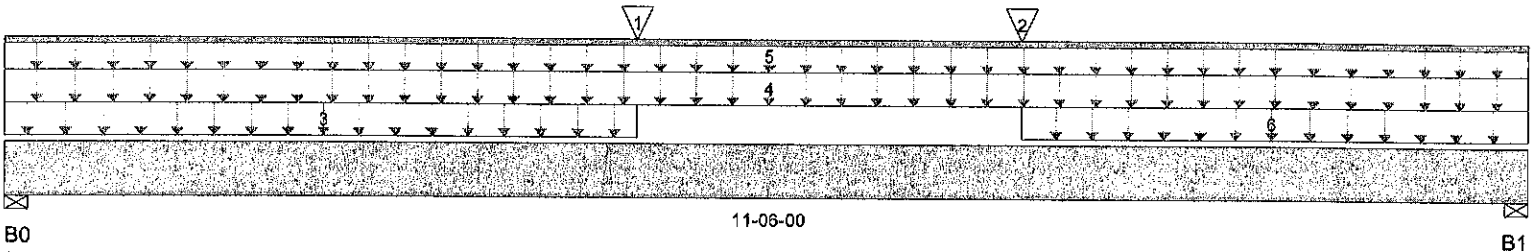


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\11
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 11-06-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	2,341 / 0	1,356 / 0		
B1, 3-1/2"	2,750 / 0	1,587 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Conc. Pt. (lbs)	L	04-09-00	04-09-00	400	158			n/a
2		Conc. Pt. (lbs)	L	07-08-00	07-08-00	999	600			n/a
3		Unf. Area (lb/ft^2)	L	00-00-00	04-09-00	40	15			00-08-00
4		Unf. Area (lb/ft^2)	L	00-00-00	11-06-00	40	15			07-01-00
5		Unf. Lin. (lb/ft)	L	00-00-00	11-06-00	0	60			n/a
6		Unf. Area (lb/ft^2)	L	07-08-00	11-06-00	40	15			02-00-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	16,618 ft-lbs	25,408 ft-lbs	0.65	1	06-04-11
End Shear	5,239 lbs	11,571 lbs	0.45	1	10-05-00
Total Load Defl.	L/257 (0.515")	0.552"	0.93	4	05-10-02
Live Load Defl.	L/405 (0.327")	0.368"	0.89	5	05-10-02
Max Defl.	0.515"	1"	0.52	4	05-10-02
Span / Depth	13.9	n/a	n/a		00-00-00

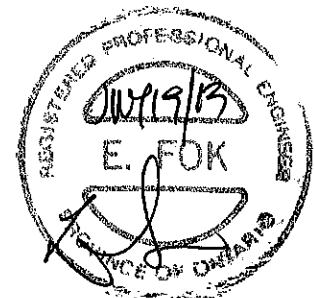
Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material	
B0	Wall/Plate	3-1/2" x 3-1/2"	5,206 lbs	0.69	0.35	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	6,109 lbs	0.81	0.41	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

User Notes NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPACER NAILS @ 12" O.C., STAGGERED IN 2 ROWS



S.129501

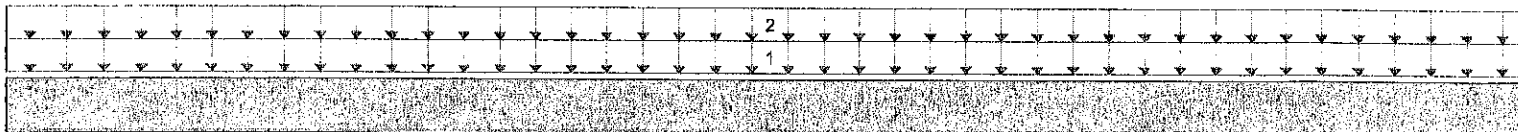
BC CALC® Design Report


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\12
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



B0 04-02-00 B1

Total Horizontal Product Length = 04-02-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	556 / 0	218 / 0		
B1, 3-1/2"	556 / 0	218 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Unf. Area (lb/ft^2)	L	00-00-00	04-02-00	40	15			06-00-00
2		Unf. Area (lb/ft^2)	L	00-00-00	04-02-00	40	15			00-08-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	913 ft-lbs	12,704 ft-lbs	0.07	1	02-01-00
End Shear	531 lbs	5,785 lbs	0.09	1	01-01-00
Total Load Defl.	L/999 (0.006")	n/a	n/a	4	02-01-00
Live Load Defl.	L/999 (0.005")	n/a	n/a	5	02-01-00
Max Defl.	0.006"	n/a	n/a	4	02-01-00
Span / Depth	4.7	n/a	n/a		00-00-00

Disclosure

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Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	1,106 lbs	0.29	0.15	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	1,106 lbs	0.29	0.15	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

User Notes

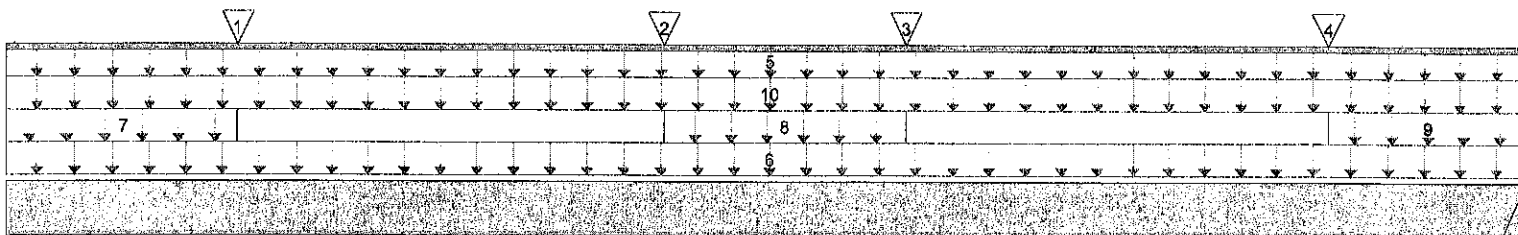
NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS
 @ O.C., STAGGERED IN TWO ROWS





Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\05A
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



08-04-00

B0

B1

Total Horizontal Product Length = 08-04-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	1,063 / 0	1,030 / 0	551 / 0	
B1	1,032 / 0	1,001 / 0	536 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Conc. Pt. (lbs)	L	01-03-00	01-03-00	68	69	131		n/a
2		Conc. Pt. (lbs)	L	03-07-00	03-07-00	68	69	131		n/a
3		Conc. Pt. (lbs)	L	04-11-00	04-11-00	68	69	131		n/a
4		Conc. Pt. (lbs)	L	07-03-00	07-03-00	68	69	131		n/a
5		Unf. Lin. (lb/ft)	L	00-00-00	08-04-00	0	100			n/a
6		Unf. Area (lb/ft^2)	L	00-00-00	08-04-00	40	15			04-07-00
7		Unf. Area (lb/ft^2)	L	00-00-00	01-03-00	11	10	21		04-08-00
8		Unf. Area (lb/ft^2)	L	03-07-00	04-11-00	11	10	21		04-08-00
9		Unf. Area (lb/ft^2)	L	07-03-00	08-04-00	11	10	21		04-08-00
10		Unf. Area (lb/ft^2)	L	00-00-00	08-04-00	11	10	21		01-02-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5,995 ft-lbs	25,408 ft-lbs	0.24	1	04-03-00
End Shear	2,369 lbs	11,571 lbs	0.2	1	01-01-00
Total Load Defl.	L/999 (0.101")	n/a	n/a	11	04-03-00
Live Load Defl.	L/999 (0.057")	n/a	n/a	15	04-03-00
Span / Depth	10.1	n/a	n/a		00-00-00

Bearing Supports

				Support	Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	3,158 lbs	0.42	0.21	Spruce Pine Fir
B1	Hanger	2" x 3-1/2"	3,067 lbs	n/a	0.36	Hanger

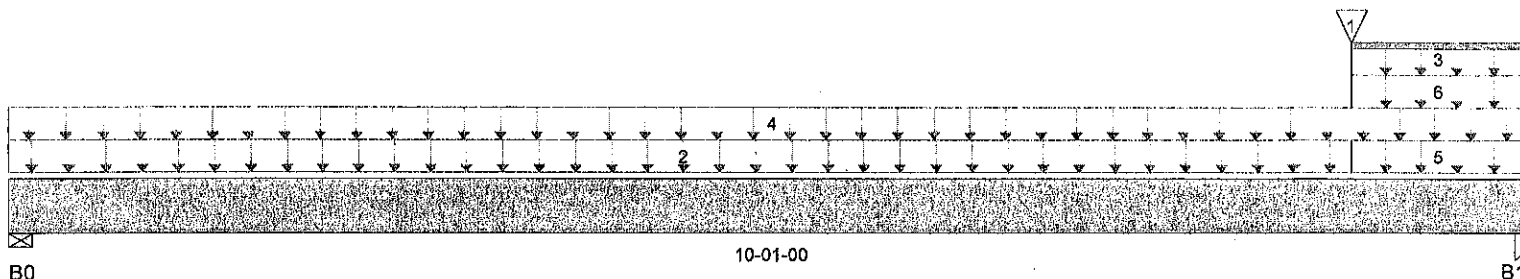
Notes NAIL ONE END TO ANOTHER WITH 3/4" SPIRAL NAILS
User Notes @ 12" O.C., STRUTTED IN 2 ROWS





Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\06A
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 10-01-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	370 / 0	270 / 0	56 / 0	
B1, 3-1/2"	1,221 / 0	1,216 / 0	580 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Conc. Pt. (lbs)	L	08-11-00	08-11-00	1,032	1,001	536		n/a
2		Unf. Area (lb/ft^2)	L	00-00-00	08-11-00	40	15			00-08-00
3		Unf. Lin. (lb/ft)	L	08-11-00	10-01-00	0	100			n/a
4		Unf. Area (lb/ft^2)	L	00-00-00	10-01-00	40	20			00-08-00
5		Unf. Area (lb/ft^2)	L	08-11-00	10-01-00	11	10	21		03-01-00
6		Unf. Area (lb/ft^2)	L	08-11-00	10-01-00	11	10	21		01-00-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,287 ft-lbs	25,408 ft-lbs	0.13	1	07-06-13
End Shear	3,257 lbs	11,571 lbs	0.28	1	09-00-00
Total Load Defl.	L/999 (0.078")	n/a	n/a	11	05-05-14
Live Load Defl.	L/999 (0.045")	n/a	n/a	15	05-04-06
Span / Depth	12.2	n/a	n/a		00-00-00

Bearing Supports

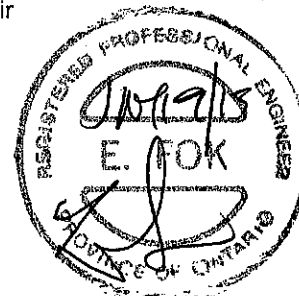
B0	Wall/Plate	3-1/2" x 3-1/2"	920 lbs	0.12	0.06	Spruce Pine Fir
B1	Post	3-1/2" x 3-1/2"	3,642 lbs	n/a	0.24	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PM TO ANOTHER WITH 3/2" SPIRAL NAILS
 @ 12" O.C., STRUTTED IN 2 ROWS



BC CALC® Design Report


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272

Job Name:

40297

Address:

HUNTINGTON & NASHVILLE

City, Province, Postal Code: KLEINBURG, ON

Customer:

GOLD PARK

Code reports:

CCMC 12472-R

File Name: 253719.bcc

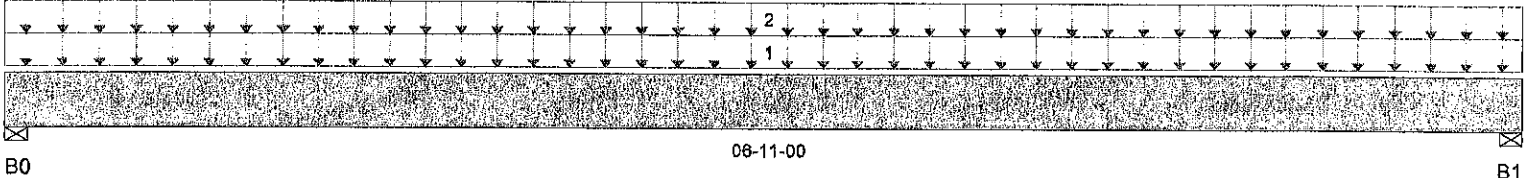
Description: Designs\13

Specifier: 38-3

Designer: MQ

Company: Alpa Roof Trusses Inc

Misc:



Total Horizontal Product Length = 06-11-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	565 / 0	282 / 0		
B1, 3-1/2"	565 / 0	282 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Unf. Area (lb/ft ²)	L	00-00-00	06-11-00	40	20			03-01-00
2		Unf. Area (lb/ft ²)	L	00-00-00	06-11-00	40	15			01-00-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,808 ft-lbs	12,704 ft-lbs	0.14	1	03-05-08
End Shear	824 lbs	5,785 lbs	0.14	1	01-01-00
Total Load Defl.	L/999 (0.038")	n/a	n/a	4	03-05-08
Live Load Defl.	L/999 (0.026")	n/a	n/a	5	03-05-08
Max Defl.	0.038"	n/a	n/a	4	03-05-08
Span / Depth	8.2	n/a	n/a		00-00-00

Bearing Supports

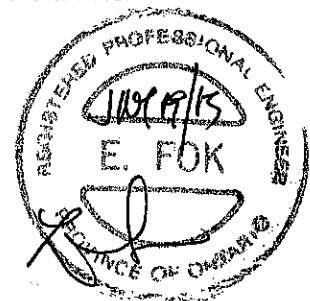
	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	1,200 lbs	0.32	0.16	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	1,200 lbs	0.32	0.16	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation. \n\nBC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



BC CALC® Design Report

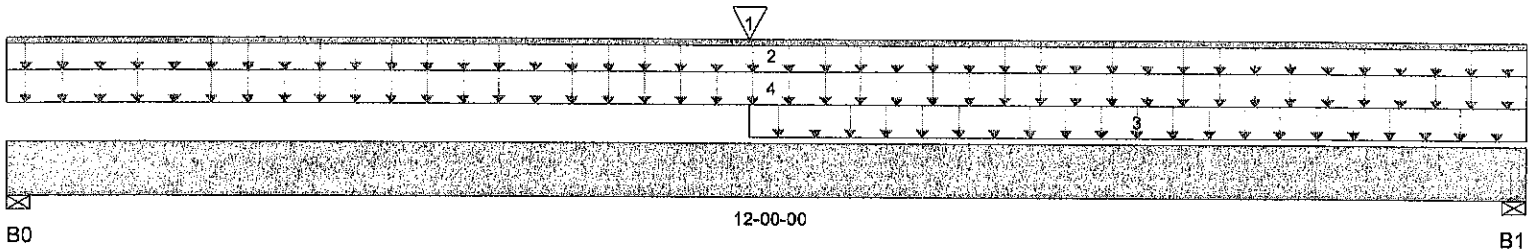


Dry | 1 span | No cantilevers | 0/12 slope (deg)

May-14-15

Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\14
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 12-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	491 / 0	614 / 0		
B1, 3-1/2"	558 / 0	648 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live	Dead	Snow	Wind	Trib.
1		Conc. Pt. (lbs)	L	05-10-00	05-10-00	565	282			n/a
2		Unf. Lin. (lb/ft)	L	00-00-00	12-00-00	0	60			n/a
3		Unf. Area (lb/ft²)	L	05-10-00	12-00-00	40	20			00-08-00
4		Unf. Area (lb/ft²)	L	00-00-00	12-00-00	40	15			00-08-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,166 ft-lbs	12,704 ft-lbs	0.49	1	05-10-00
End Shear	1,441 lbs	5,785 lbs	0.25	1	10-11-00
Total Load Defl.	L/362 (0.383")	0.577"	0.66	4	05-11-15
Live Load Defl.	L/730 (0.19")	0.385"	0.49	5	05-11-15
Max Defl.	0.383"	1"	0.38	4	05-11-15
Span / Depth	14.6	n/a	n/a		00-00-00

Disclosure

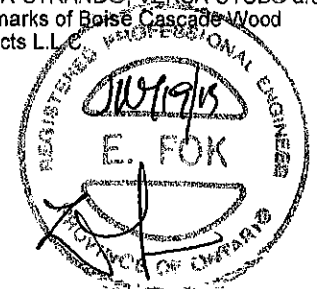
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Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0	Wall/Plate 3-1/2" x 1-3/4"	1,505 lbs	0.4	0.2	Spruce Pine Fir
B1	Wall/Plate 3-1/2" x 1-3/4"	1,647 lbs	0.44	0.22	Spruce Pine Fir

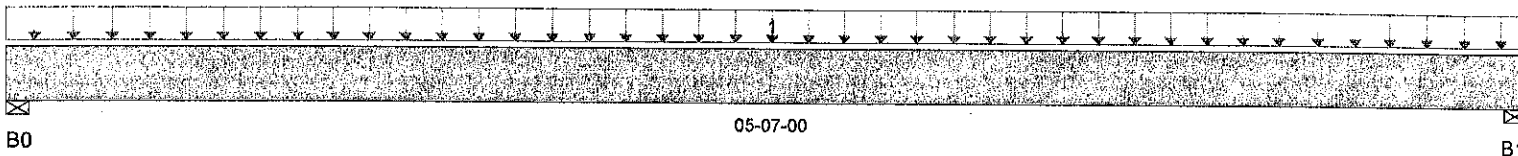
Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.



Build 3272
 Job Name: 40297
 Address: HUNTINGTON & NASHVILLE
 City, Province, Postal Code: KLEINBURG, ON
 Customer: GOLD PARK
 Code reports: CCMC 12472-R

File Name: 253719.bcc
 Description: Designs\15
 Specifier: 38-3
 Designer: MQ
 Company: Alpa Roof Trusses Inc
 Misc:



Total Horizontal Product Length = 05-07-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	391 / 0	209 / 0		
B1, 3-1/2"	391 / 0	209 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live	Dead	Snow	Wind	Trib.
1	Unf. Area (lb/ft^2)	L	00-00-00	05-07-00	40	20		1.00	1.15	03-06-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	997 ft-lbs	12,704 ft-lbs	0.08	1	02-09-08
End Shear	519 lbs	5,785 lbs	0.09	1	01-01-00
Total Load Defl.	L/999 (0.013")	n/a	n/a	4	02-09-08
Live Load Defl.	L/999 (0.009")	n/a	n/a	5	02-09-08
Max Defl.	0.013"	n/a	n/a	4	02-09-08
Span / Depth	6.5	n/a	n/a		00-00-00

Disclosure

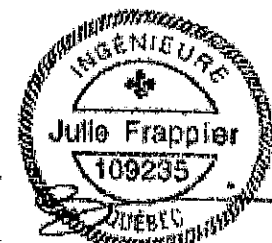
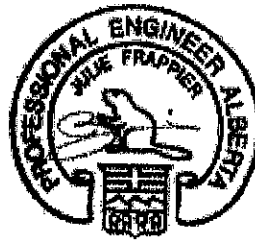
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Bearing Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0 Wall/Plate	3-1/2" x 1-3/4"	847 lbs	0.22	0.11	Spruce Pine Fir
B1 Wall/Plate	3-1/2" x 1-3/4"	847 lbs	0.22	0.11	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.





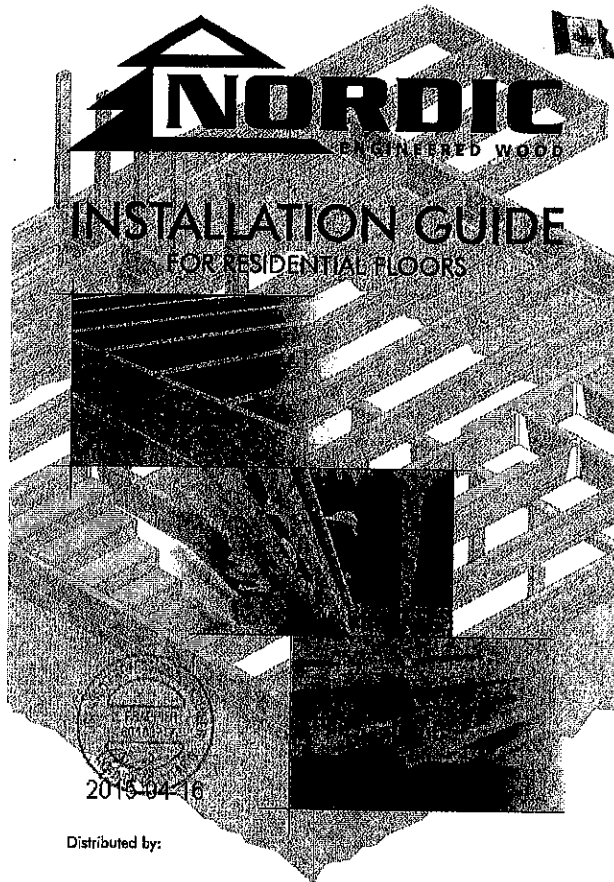
Maximum Floor Spans

Live Load = 40 psf, Dead Load = 15 psf
Simple Spans, L/360 Deflection Limit
5/8" OSB G&N Sheathing

Depth	Series	Bare				1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
11-7/8"	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
14"	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-5"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
16"	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

Depth	Series	Mid-Span Blocking				Mid-Span Blocking and 1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-10"	15'-5"	14'-6"	N/A	17'-1"	15'-5"	14'-6"	N/A
	NI-40x	17'-11"	16'-11"	16'-4"	N/A	18'-5"	17'-4"	16'-7"	N/A
	NI-60	18'-2"	17'-1"	16'-6"	N/A	18'-7"	17'-6"	16'-10"	N/A
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A
11-7/8"	NI-20	19'-6"	18'-1"	17'-5"	N/A	20'-2"	18'-8"	17'-6"	N/A
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-3"	N/A
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
14"	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
16"	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/360 and a total load deflection limit of L/240.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- Minimum bearing length shall be 1-3/4 inches for the end bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, and NBC 2010.
- Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Distributed by:



SAFETY AND CONSTRUCTION PRECAUTIONS

WARNING

I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

Avoid Accidents by Following these Important Guidelines:

1. Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-briding at joist ends. When I-joists are applied continuously over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
 - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on center, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Top ends of adjoining bracing over at least two I-joists.
 - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-briding.
4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
5. Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use wall stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

STORAGE AND HANDLING GUIDELINES

1. Bundle wrap can be slippery when wet. Avoid walking on wrapped bundles.
2. Store, stack, and handle I-joists vertically and level only.
3. Always stack and handle I-joists in the upright position only.
4. Do not store I-joists in direct contact with the ground and/or flatwise.
5. Protect I-joists from weather, and use spacers to separate bundles.
6. Bundled units should be kept intact until time of installation.
7. When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury to your work crew.
 - Pick I-joists in bundles as shipped by the supplier.
 - Orient the bundles so that the webs of the I-joists are vertical.
 - Pick the bundles at the 5th points, using a spreader bar if necessary.
8. Do not handle I-joists in a horizontal orientation.
9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.

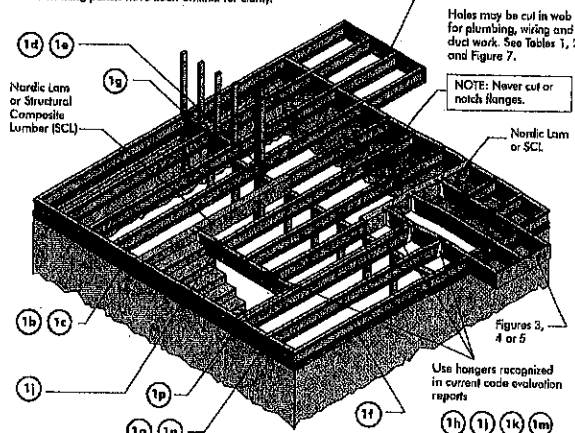


INSTALLING NORDIC I-JOISTS

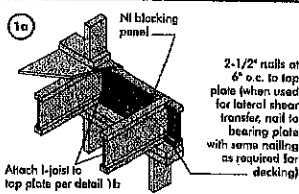
1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, contact supplier.
2. Except for cutting to length, I-joist flanges should **never** be cut, drilled, or notched.
3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple span joists must be level.
5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings.
6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
7. Leave a 1/16-inch gap between the I-joist end and a header.
8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the I-joist webs.
9. Never install I-joists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
12. Due to shrinkage, common framing lumber set on edge may **never** be used as blocking or rim boards. I-joist blocking panels or other engineered wood products – such as rim board – must be cut to fit between the I-joists, and an I-joist-compatible depth selected.
13. Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed.
15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

FIGURE 1
TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS

Some framing requirements such as erection bracing and blocking panels have been omitted for clarity.

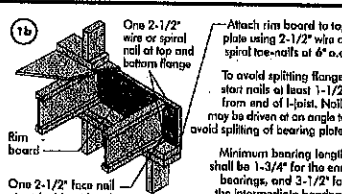


All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.



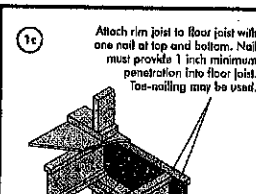
Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
Ni Joist	3,300

*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a banding member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



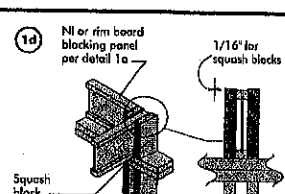
Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
1-1/8" Rim Board Plus	8,090

*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a banding member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



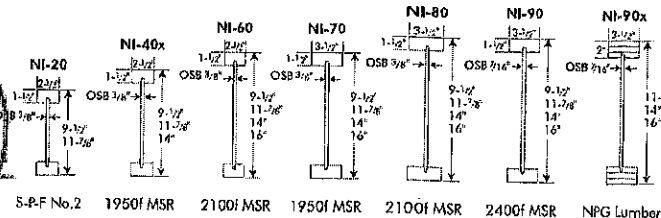
Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
1-1/8" Rim Board Plus	8,090

*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a banding member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



Pair of Squash Blocks	Maximum Factored Vertical per Pair of Squash Blocks (plf)
2x Lumber	3-1/2" wide 5,500 5-1/2" wide 6,500
1-1/8" Rim Board Plus	4,500 6,600

Provide lateral bracing per detail 1a, 1b, or 1c



Refer to the Installation Guide for Residential Floors for additional information.
CMC EVALUATION REPORT 13032-R

WEB HOLE SPECIFICATIONS

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- l-joint top and bottom flanges must NEVER be cut, notched, or otherwise modified. Whenever possible, field-cut holes should be centred on the middle of the web. The maximum size hole or the maximum depth of a duct chase opening that can be cut into an l-joint web shall equal the clear distance between the flanges of the l-joint minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent l-joint flange.

5. The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the longest side of the longest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
7. A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
8. Holes measuring 1-1/2 inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.

9. A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
11. Limit three maximum size holes per span, of which one may be a duct chase opening.
12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1
LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf																
Joist Depth	Joist Series	Minimum Distance from Inside Face of Any Support to Centre of Hole (ft - in.)														
		Round Hole Diameter (in.)														
		2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4	11	12	12-3/4
1-1/2"	NI-20	0'-7"	1'-6"	2'-10"	4'-3"	5'-8"	6'-0"	---	---	---	---	---	---	---	---	---
	NI-40x	0'-7"	1'-6"	3'-0"	4'-4"	6'-0"	6'-4"	---	---	---	---	---	---	---	---	---
	NI-60	1'-3"	2'-6"	4'-0"	5'-4"	7'-0"	7'-5"	---	---	---	---	---	---	---	---	---
	NI-70	2'-0"	3'-4"	4'-9"	6'-3"	8'-0"	8'-4"	---	---	---	---	---	---	---	---	---
1-7/8"	NI-80	2'-3"	3'-6"	5'-0"	6'-6"	8'-2"	8'-8"	---	---	---	---	---	---	---	---	---
	NI-20	0'-7"	0'-8"	1'-0"	2'-4"	3'-8"	4'-0"	5'-0"	6'-6"	7'-9"	---	---	---	---	---	---
	NI-40x	0'-7"	0'-8"	1'-3"	2'-8"	4'-0"	4'-4"	5'-5"	7'-0"	8'-4"	---	---	---	---	---	---
	NI-60	0'-7"	1'-8"	3'-0"	4'-3"	5'-9"	6'-0"	7'-3"	8'-10"	10'-0"	---	---	---	---	---	---
4"	NI-70	1'-3"	2'-6"	4'-0"	5'-4"	6'-9"	7'-2"	8'-4"	10'-0"	11'-2"	---	---	---	---	---	---
	NI-80	1'-6"	2'-10"	4'-2"	5'-6"	7'-0"	7'-5"	8'-6"	10'-3"	11'-4"	---	---	---	---	---	---
	NI-90	0'-7"	0'-8"	1'-5"	3'-2"	4'-10"	5'-4"	6'-9"	8'-9"	10'-2"	---	---	---	---	---	---
	NI-90x	0'-7"	0'-8"	0'-9"	2'-5"	4'-4"	4'-9"	6'-3"	---	---	---	---	---	---	---	---
6"	NI-40x	0'-7"	0'-8"	0'-8"	1'-0"	2'-4"	2'-9"	3'-9"	5'-2"	6'-0"	6'-6"	8'-3"	10'-2"	---	---	---
	NI-60	0'-7"	0'-8"	1'-8"	3'-0"	4'-3"	4'-8"	5'-8"	7'-2"	8'-0"	8'-8"	10'-4"	11'-9"	---	---	---
	NI-70	0'-8"	1'-10"	3'-0"	4'-5"	5'-10"	6'-2"	7'-3"	8'-9"	9'-9"	10'-4"	12'-0"	13'-5"	---	---	---
	NI-80	0'-10"	2'-0"	3'-4"	4'-9"	6'-2"	6'-5"	7'-6"	9'-0"	10'-0"	10'-8"	12'-4"	13'-9"	---	---	---
6"	NI-90	0'-7"	0'-8"	0'-10"	2'-5"	4'-0"	4'-5"	5'-9"	7'-5"	8'-8"	9'-4"	11'-4"	12'-11"	---	---	---
	NI-90x	0'-7"	0'-8"	0'-8"	2'-0"	3'-9"	4'-2"	5'-5"	7'-3"	8'-5"	9'-0"	---	---	---	---	---
	NI-60	0'-7"	0'-8"	0'-8"	1'-6"	2'-10"	3'-2"	4'-4"	5'-6"	6'-4"	7'-2"	8'-5"	9'-8"	10'-2"	12'-2"	13'-9"
	NI-70	0'-7"	1'-0"	2'-3"	3'-6"	4'-10"	5'-3"	6'-3"	7'-8"	8'-6"	9'-2"	10'-8"	12'-0"	12'-4"	14'-0"	15'-6"
6"	NI-80	0'-7"	1'-3"	2'-6"	3'-10"	5'-3"	6'-6"	6'-6"	8'-0"	9'-0"	11'-6"	12'-3"	12'-9"	14'-5"	16'-0"	---
	NI-90	0'-7"	0'-8"	0'-8"	1'-9"	3'-3"	3'-8"	4'-9"	6'-5"	7'-5"	8'-0"	9'-10"	11'-3"	11'-9"	13'-9"	15'-4"
	NI-90x	0'-7"	0'-8"	0'-9"	2'-0"	3'-6"	4'-0"	5'-0"	6'-9"	7'-9"	8'-4"	10'-2"	11'-6"	12'-0"	---	---

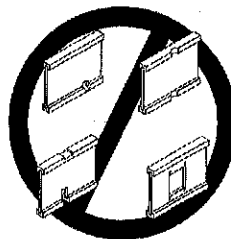
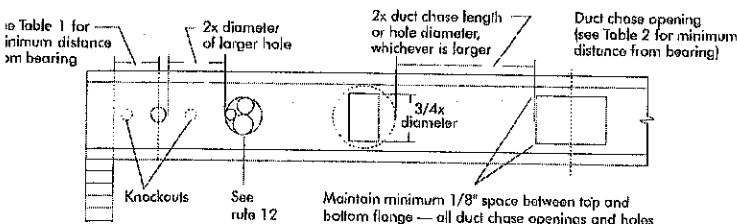
Above table may be used for 1-joint spacing of 24 inches on centre or less.
Hole location distance is measured from inside face of supports to centre of hole.
Distances in this chart are based on uniformly loaded joists.
The above table is based on the 1-joists being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

TABLE 2
DUCT CHASE OPENING SIZES AND LOCATIONS
Simple Span Only

Joist Depth	Joist Series	Minimum distance from inside face of supports to centre of opening (ft. - in.)									
		Duct Chase Length (in.)									
		4	10	12	14	16	18	20	22	24	
9-1/2"	NI-20	4'-1"	4'-5"	4'-10"	5'-4"	5'-8"	6'-1"	6'-6"	7'-1"	7'-5"	
	NI-40x	5'-3"	5'-8"	6'-0"	6'-5"	6'-10"	7'-3"	7'-8"	8'-2"	8'-6"	
	NI-60	5'-4"	5'-9"	6'-2"	6'-7"	7'-1"	7'-6"	8'-0"	8'-3"	8'-9"	
	NI-70	5'-1"	5'-5"	5'-10"	6'-3"	6'-7"	7'-1"	7'-6"	8'-1"	8'-4"	
	NI-80	5'-3"	5'-8"	6'-0"	6'-5"	6'-10"	7'-3"	7'-8"	8'-2"	8'-6"	
11-7/8"	NI-20	5'-9"	6'-2"	6'-6"	7'-1"	7'-5"	7'-9"	8'-3"	8'-9"	9'-4"	
	NI-40x	6'-8"	7'-2"	7'-6"	8'-1"	8'-6"	9'-1"	9'-6"	10'-1"	10'-9"	
	NI-60	7'-3"	7'-8"	8'-0"	8'-6"	9'-0"	9'-3"	9'-9"	10'-3"	10'-9"	
	NI-70	7'-1"	7'-4"	7'-9"	8'-3"	8'-7"	9'-1"	9'-6"	10'-1"	10'-4"	
	NI-80	7'-2"	7'-7"	8'-0"	8'-5"	8'-10"	9'-3"	9'-8"	10'-2"	10'-8"	
14"	NI-90	7'-6"	7'-11"	8'-4"	8'-9"	9'-2"	9'-7"	10'-1"	10'-7"	10'-11"	
	NI-90x	7'-7"	8'-1"	8'-5"	8'-10"	9'-4"	9'-8"	10'-2"	10'-8"	11'-2"	
	NI-40x	8'-1"	8'-7"	9'-0"	9'-6"	10'-1"	10'-7"	11'-2"	12'-0"	12'-8"	
	NI-60	8'-9"	9'-3"	9'-8"	10'-1"	10'-6"	11'-1"	11'-6"	12'-3"	13'-0"	
	NI-70	8'-7"	9'-1"	9'-5"	9'-10"	10'-4"	10'-8"	11'-3"	12'-1"	12'-3"	
16"	NI-80	9'-0"	9'-3"	9'-9"	10'-1"	10'-7"	11'-1"	11'-6"	12'-1"	12'-6"	
	NI-90	9'-2"	9'-8"	10'-0"	10'-6"	10'-11"	11'-5"	11'-9"	12'-4"	12'-11"	
	NI-90x	9'-4"	9'-9"	10'-3"	10'-7"	11'-1"	11'-7"	12'-1"	12'-7"	13'-2"	
	NI-60	10'-3"	10'-8"	11'-1"	11'-6"	12'-1"	12'-6"	13'-2"	14'-1"	14'-10"	
	NI-70	10'-1"	10'-5"	11'-0"	11'-4"	11'-10"	12'-3"	12'-8"	13'-3"	14'-0"	
16"	NI-80	10'-4"	10'-9"	11'-3"	11'-9"	12'-1"	12'-7"	13'-1"	13'-8"	14'-4"	
	NI-90	10'-9"	11'-2"	11'-8"	12'-0"	12'-6"	13'-0"	13'-6"	14'-2"	14'-10"	
	NI-90x	11'-1"	11'-5"	11'-10"	12'-4"	12'-10"	13'-2"	13'-9"	14'-4"	15'-2"	

1. Above table may be used for I-joist spacing of 24 inches on centre or less.
2. Duct chase opening location distance is measured from inside face of supports to centre of opening.
3. The above table is based on simple-span joists only. For other applications, contact your local distributor.
4. Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of $L/480$.
5. The above table is based on the I-joists being used of their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

FIGURE 7
FIELD-CUT HOLE LOCATOR



Knockouts are prescored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the web.

Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

SAFETY AND CONSTRUCTION PRECAUTIONS



WARNING: I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

1. Brace and nail each I-joint as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging of joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required of the interior support.
2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
 - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-joists.
 - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
5. Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic Joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.



PRODUCT WARRANTY

Chantiers Clibaugeman guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chantiers Chiboungman warrants that our products, when utilized in accordance with our handling and installation instructions, will meet or exceed our specifications for the lifetime of the structure.

1a

Blocking Panel or Rim Joist

NI blocking panel

Joist

2-1/2" nails at 6" o.c. to top plate (when used for lateral shear transfer, nail to bearing plate with same nailing as required for decking)

Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
NI Joists	3,300

*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

1b

Rim board

Joist

One 2-1/2" face nail at each side at bearing

One 2-1/2" wire or spiral nail at top and bottom flange

Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
1-1/8" Rim Board Plus	8,090

*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

Attach rim board to top plate using 2-1/2" wire or spiral toe-nails at 6" o.c.

To avoid splitting flange, start nails at least 1-1/2" from end of I-joist. Nails may be driven at an angle to avoid splitting of bearing plate.

Minimum bearing length shall be 1-3/4" for the end bearings, and 3-1/2" for the intermediate bearings when applicable.

1d

Pair of Squash Blocks

2x Lumber

1-1/8" Rim Board Plus

Pair of Squash Blocks	Maximum Factored Vertical Load per Pair of Squash Blocks (lbs)
3-1/2" wide	5,500
5-1/2" wide	8,500
1-1/8" Rim Board Plus	4,300
	6,600

Provide lateral bracing per detail 1a or 1b

1e

Transfer load from above to bearing below. Install squash blocks per detail 1d. Match bearing area of blocks below to post above.

1g

Joist attachment per detail 1b

Load bearing wall above shall align vertically with the bearing below. Other conditions, such as offset bearing walls, are not covered by this detail.

Blocking required over all interior supports under load-bearing walls or when floor joists are not continuous over support

NI blocking panel per detail 1a

2-1/2" nails at 6" o.c. to top plate

1h

Backer block (use if hanger load exceeds 360 lbs). Before installing a backer block to a double I-joist, drive three additional 3" nails through the webs and filler block where the backer block will fit. Clinch. Install backer tight to top flange. Use twelve 3" nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs.

ACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	1"	5-1/2"
3-1/2"	1-1/2"	7-1/4"

Minimum grade for backer block material shall be S-P-F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard.

*For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".

1i

Top- or face-mount hanger

Double I-joist header

Backer block required (both sides for face-mount hangers)

NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

Filler block per detail 1p

For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

1j

Nordic Lam or Structural Composite Lumber (SCL)

Top- or face-mount hanger installed per manufacturer's recommendations

NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

1k

2x plate flush with inside face of wall or beam. 1/8" overhang allowed past inside face of wall or beam.

NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

Top-mount hanger installed per manufacturer's recommendations

1m

Multiple I-joist header with full depth filler block shown. Nordic Lam or SCL headers may also be used. Verify double I-joist capacity to support concentrated loads.

Backer block attached per detail 1h. Nail with twelve 3" nails, clinch when possible.

Install hanger per manufacturer's recommendations

Filler block per detail 1p

Maximum support capacity = 1,620 lbs.

1n

Do not bevel-cut joist beyond inside face of wall

Attach I-joist per detail 1b

NOTE: Blocking required at bearing for lateral support, not shown for clarity.

1r

Lumber 2x4 min., extend block to face of adjacent web. Two 2-1/2" spiral nails from each web to lumber piece, alternate on opposite side.

NI blocking panel

OPTIONAL: Minimum 1x4 inch strap applied to underside of joist at blocking line or 1/2 inch minimum gypsum ceiling attached to underside of joists.

1p

Filler block

Offset nails from opposite face by 6"

1/8" to 1/4" gap between top flange and filler block

FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION

NOTES:

- Support back of I-joist web during nailing to prevent damage to web/flange connection.
- Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist flange.
- Filler block is required between joists for full length of span.
- Nail joists together with two rows of 3" nails at 12 inches o.c. (clinched when possible) on each side of the double I-joist. Total of four nails per foot required. If nails can be clinched, only two nails per foot are required.
- The maximum factored load that may be applied to one side of the double joist using this detail is 860 lb/ft. Verify double I-joist capacity.

Flange Size	Net Depth	Filler Block Size
2-1/2" x 1-1/2"	9-1/2"	2-1/8" x 6"
	11-7/8"	2-1/8" x 8"
	14"	2-1/8" x 10"
	16"	2-1/8" x 12"
3-1/2" x 1-1/2"	9-1/2"	3" x 6"
	11-7/8"	3" x 8"
	14"	3" x 10"
	16"	3" x 12"
3-1/2" x 2"	11-7/8"	3" x 7"
	14"	3" x 9"
	16"	3" x 11"

1s

One 2-1/2" nail at top and bottom flange

2x4 min. (1/8" gap minimum)

Two 2-1/2" nails from each web to lumber piece

1-1/2" gap minimum

One 2-1/2" nail one side only

NOTE:

- In some local codes, blocking is prescriptively required in the first joist space (or first and second joist space) next to the starter joist. Where required, see local code requirements for spacing of the blocking.
- All nails are common spiral in this detail.

WEB STIFFENERS

RECOMMENDATIONS:

A **bearing stiffener** is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found in the I-joist Construction Guide (C101). The gap between the stiffener and the flange is at the top.

A **bearing stiffener** is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.

A **load stiffener** is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

FIGURE 2

WEB STIFFENER INSTALLATION DETAILS

Flange width 2-1/2" or 3-1/2"

Approx. 2"

1/8"-1/4" Gap

(4) 2-1/2" nails, 3" nails required for I-joists with 3-1/2" flange width

No Gap

See the adjacent table for web stiffener size requirements

CONCENTRATED LOAD (Load stiffener)

Tight Joint No Gap

Gap

END BEARING (Bearing stiffener)

Gap

Tight Joint No Gap

STIFFENER SIZE REQUIREMENTS

Flange Width	Web Stiffener Size Each Side of Web
2-1/2"	1" x 2-5/16" minimum width
3-1/2"	1-1/2" x 2-5/16" minimum width

ANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET

Method 1 — SHEATHING REINFORCEMENT ONE SIDE

NI blocking panel or rim board blocking, attach per detail 1g

Attach I-joist to plate per detail 1b

Use same installation as Method 1 but reinforce both sides of I-joist with sheathing.

Use nailing pattern shown for Method 1 with opposite face nailing offset by 3".

Method 2 — SHEATHING REINFORCEMENT TWO SIDES

OTE: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall attach the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

RIM BOARD INSTALLATION DETAILS

8a ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

Rim Board Joint Between Floor Joists

(1) 2-1/2" nail top and bottom (typical)

2-1/2" nails at 6" o.c. (typical)

2-1/2" toe-nails at 6" o.c. (typical)

Rim Board Joint at Corner

2-1/2" nails

h

1-1/2"

Rim board

Top or sole plate

30°

2015

MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to simple-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 10 psf. The ultimate limit states are based on the factored loads of 1.5DL + 1.2SD. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CGS-71.26 Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the use of gypsum and/or a row of blocking at mid-span.
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Limit States Design per CAN/CSA C86-09 Standard, and NBC 2010.
- SI units conversion: 1 inch = 25.4 mm
1 foot = 0.305 m

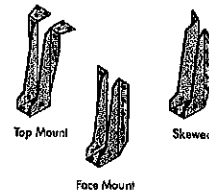
MAXIMUM FLOOR SPANS FOR NORDIC I-JOIST SERIES

Joist Depth	Joist Series	Simple spans				Multiple spans			
		On center spacing				On center spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
12"	N1-20	12'	10'	8'	6'	12'	10'	8'	6'
16"	N1-40	16'	14'	11'	9'	16'	14'	11'	9'
20"	N1-60	20'	18'	15'	12'	20'	18'	15'	12'
24"	N1-80	24'	21'	18'	14'	24'	21'	18'	14'
30"	N1-100	30'	26'	22'	17'	30'	26'	22'	17'
36"	N1-120	36'	31'	26'	20'	36'	31'	26'	20'
42"	N1-140	42'	36'	30'	23'	42'	36'	30'	23'
48"	N1-160	48'	41'	34'	26'	48'	41'	34'	26'
54"	N1-180	54'	46'	38'	29'	54'	46'	38'	29'
60"	N1-200	60'	51'	42'	32'	60'	51'	42'	32'

CCMC EVALUATION REPORT 11012-R

I-JOIST HANGERS

- Hangers shown illustrate the three most commonly used metal hangers to support I-joists.
- All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- Web stiffeners are required when the sides of the hangers do not laterally brace the top flange of the I-joist.



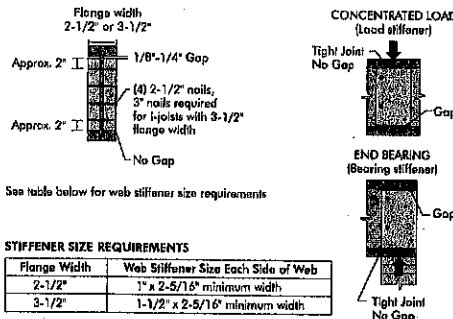
WEB STIFFENERS

RECOMMENDATIONS:

- A bearing stiffener is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found in the I-joist Construction Guide (C101). The gap between the stiffener and the flange is at the top.
- A bearing stiffener is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
- A load stiffener is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

SI units conversion: 1 inch = 25.4 mm

FIGURE 2
WEB STIFFENER INSTALLATION DETAILS



STIFFENER SIZE REQUIREMENTS

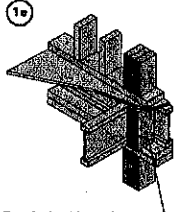
Flange Width	Web Stiffener Size Each Side of Web
2-1/2"	1" x 2-5/16" minimum width
3-1/2"	1-1/2" x 2-5/16" minimum width

NORDIC I-JOIST SERIES

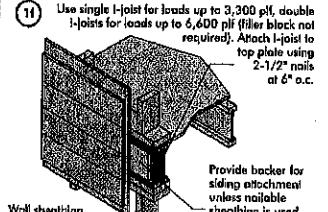
Series	Depth	Flange Width	Material Thickness	Minimum Depth
N1-20	20"	2-1/2"	1"	5-1/2"
N1-40	40"	3-1/2"	1-1/2"	7-1/4"
N1-60	60"	4-1/2"	1-3/4"	8-1/2"
N1-80	80"	5-1/2"	1-7/8"	9-1/2"
N1-100	100"	6-1/2"	2"	10-1/2"
N1-120	120"	7-1/2"	2-1/8"	11-1/2"
N1-140	140"	8-1/2"	2-1/4"	12-1/2"
N1-160	160"	9-1/2"	2-3/8"	13-1/2"
N1-180	180"	10-1/2"	2-7/8"	14-1/2"
N1-200	200"	11-1/2"	3"	15-1/2"

Chantiers Chibougamau Ltd. harvests its own trees, which enables Nordic products to adhere to strict quality control procedures throughout the manufacturing process. Every phase of the operation, from the raw material to the finished product, reflects our commitment to quality.

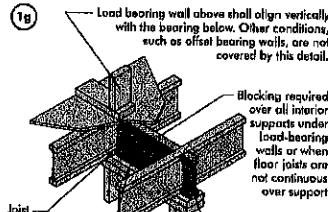
Nordic Engineered Wood I-joists use only finger-jointed black spruce lumber in their flanges, ensuring consistent quality, superior strength and longer span carrying capacity.



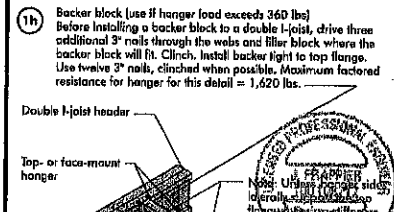
Transfer load from above to bearing below. Install squash blocks per detail 1d. Match bearing area of blocks below to post above.



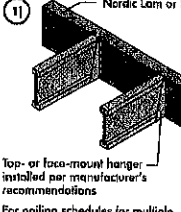
Use single I-joist for loads up to 3,300 plf, double I-joists for loads up to 6,600 plf (filler block not required). Attach I-joist to top plate using 2-1/2" nails at 6" o.c.



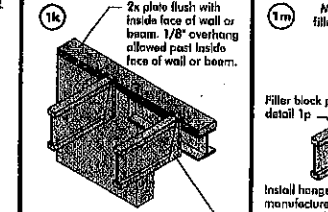
Load bearing wall above shall align vertically with the bearing below. Other conditions, such as offset bearing walls, are not covered by this detail.



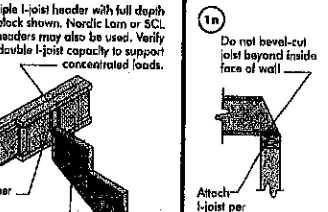
Backer block (use if hanger load exceeds 360 lbs). Before installing a backer block to a double I-joist, drive three additional 3" nails through the webs and filler block where the backer block will fit. Clinch. Install backer block to top flange. Use twelve 3" nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs.



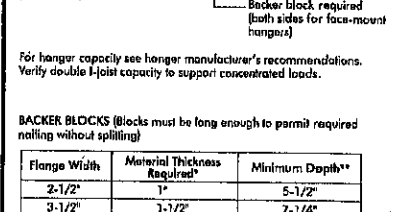
Nordic Lamin or SCL. Top- or face-mount hanger installed per manufacturer's recommendations.



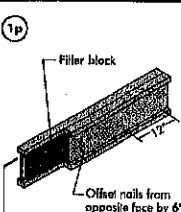
2x plate flush with inside face of wall or beam. 1/8" overhang allowed past inside face of wall or beam.



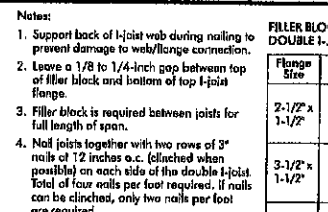
Multiple I-joist header with full depth filler block shown. Nordic Lamin or SCL headers may also be used. Verify double I-joist capacity to support concentrated loads.



Do not bevel-cut joist beyond inside face of wall. Attach I-joist per detail 1b.



Filler block. 1/8" to 1/4" gap between top flange and filler block.

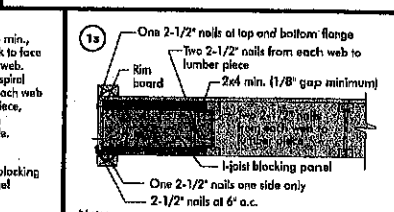


Notes:
1. Support back of I-joist web during nailing to prevent damage to web/flange connection.
2. Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist flange.
3. Filler block is required between joists for full length of span.
4. Nail joists together with two rows of 3" nails of 12 inches o.c. (clinched when possible) on each side of the double I-joist. Total of four nails per foot required. If nails can be clinched, only two nails per foot are required.
5. The maximum factored load that may be applied to one side of the double joist using this detail is 860 lb/ft. Verify double I-joist capacity.

FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION

Flange Size	Joist Depth	Filler Block Size
2-1/2" x 1-1/2"	9-1/2"	2-1/8" x 6"
	11-7/8"	2-1/8" x 8"
	14"	2-1/8" x 10"
	16"	2-1/8" x 12"
3-1/2" x 1-1/2"	9-1/2"	3" x 6"
	11-7/8"	3" x 8"
	14"	3" x 10"
	16"	3" x 12"
3-1/2" x 2"	11-7/8"	3" x 7"
	14"	3" x 9"
	16"	3" x 11"

Optional: Minimum 1x4 inch strip applied to underside of joist at blocking line or 1/2 inch minimum gypsum ceiling attached to underside of joist.



Notes:
- In some local codes, blocking is prescriptively required in the first joist space (for first and second joist space) next to the starter joist. Where required, see local code requirements for spacing of the blocking.
- All nails are common spiral in this detail.