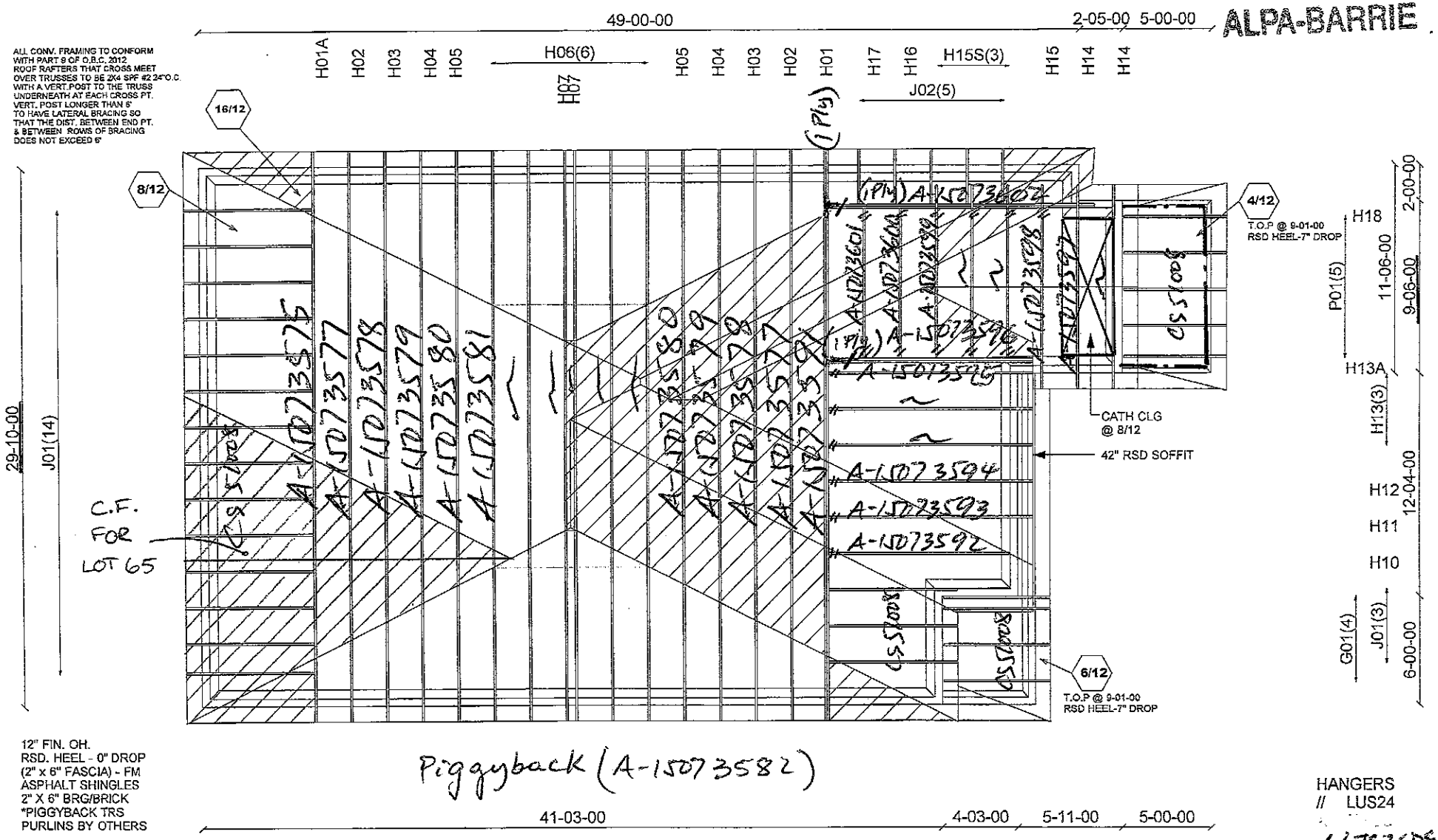
ALPA-BARRIE



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

HANGERS
// LUS24
/ LJS26DS

CONVENTIONAL FRAMING BY OTHERS

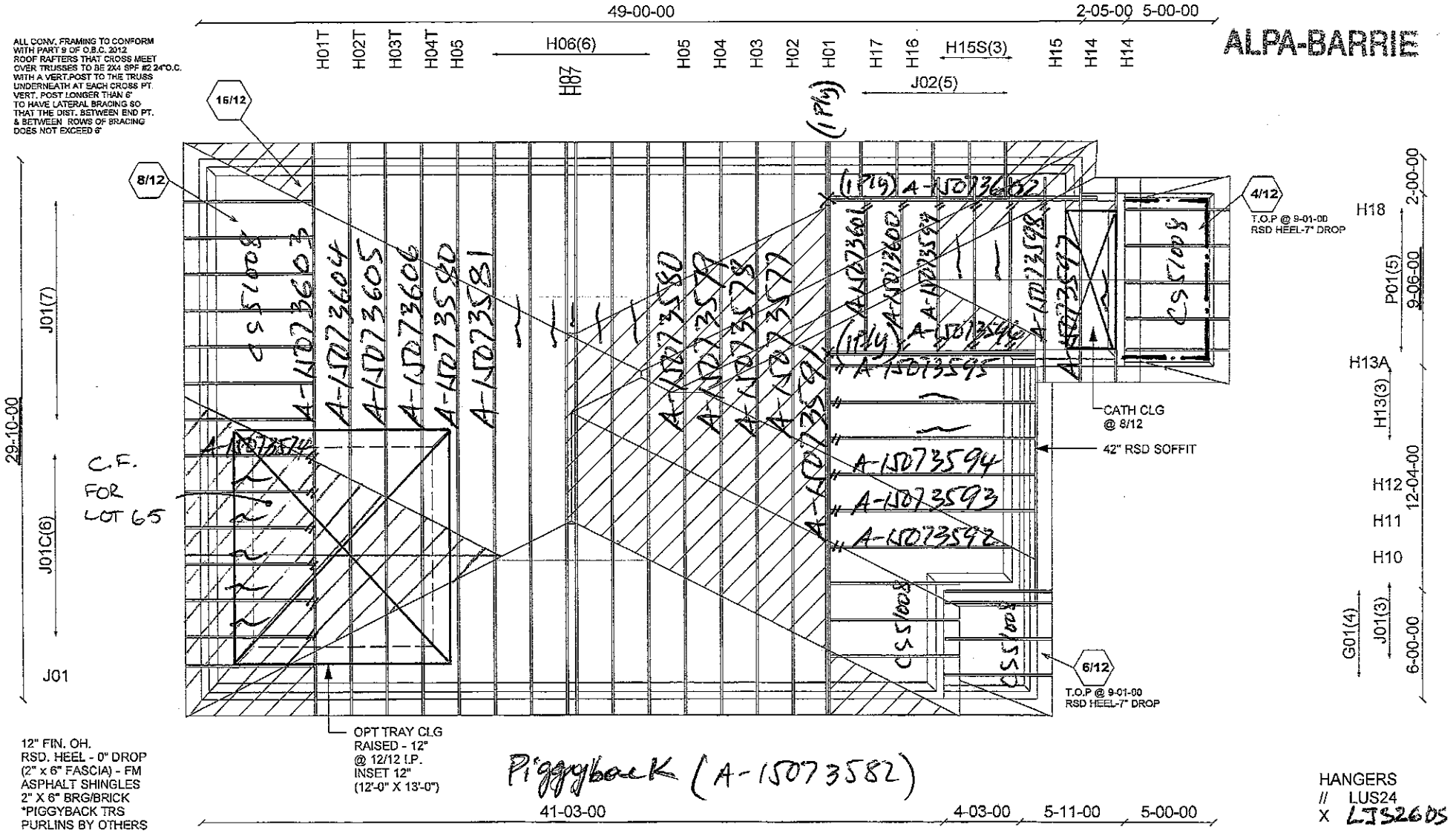
Mitek Ver: 7.5.0

P.L. 82444, 85748, 88463 (LOT 65)

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-2 / A	STD OR LOT 65 (REAR UPGRD)
Layout ID: 253430	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015 Designer: cheryle		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

Piggyback (A-15073582)

HANGERS
// LUS24
X LTS26DS

CONVENTIONAL FRAMING BY OTHERS

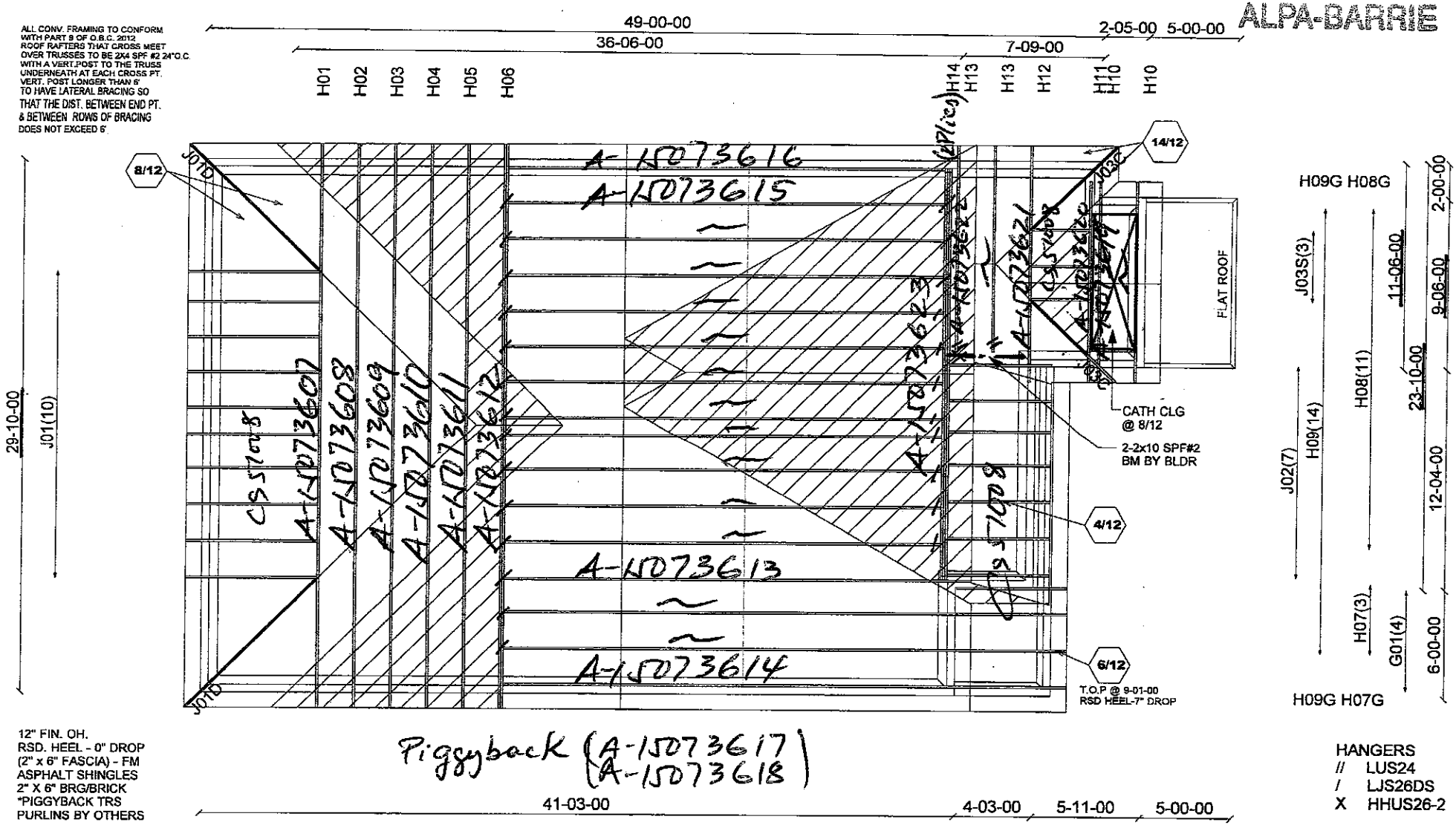
Mitek Ver: 7.5.0

PL. 82444, 85748, 88463 (LOT 65)

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-2 / A OPT TRAY STD OR LOT 65 (REAR UPGRD)
Layout ID: 253431	Project: HUNTINGTON & NASHVILLE	
Plan Log: 79711	Date: 4/22/2015 Designer: cheryle	

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

ALPA-BARRIE



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

Piggyback (A-15073617 A-15073618)

41-03-00 4-03-00 5-11-00 5-00-00

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2

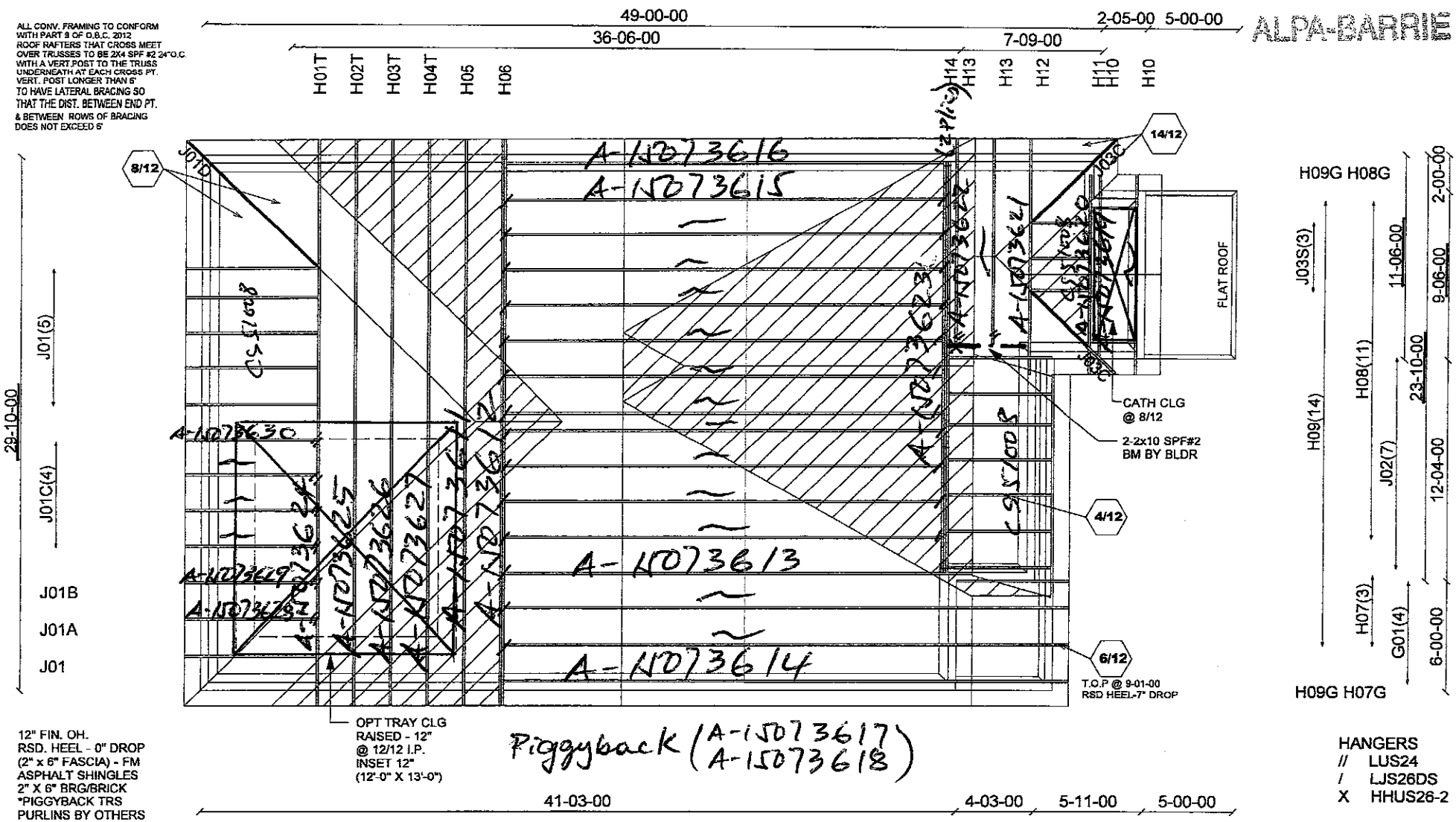
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-2 / B	
Layout ID: 253432	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/23/2015 Designer: cheryle		

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



Piggyback (A-150736 17) (A-150736 18)

12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2

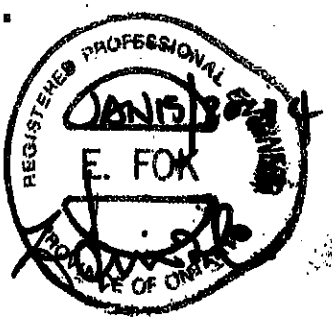
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-2 / B OPT TRAY	
Layout ID: 253433	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/23/2015	Designer: cheryle	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253433	J01C	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-v4DgXX6M6wDUmXAE6wiUUtg?bDXooC_urwoBvXyxuN1

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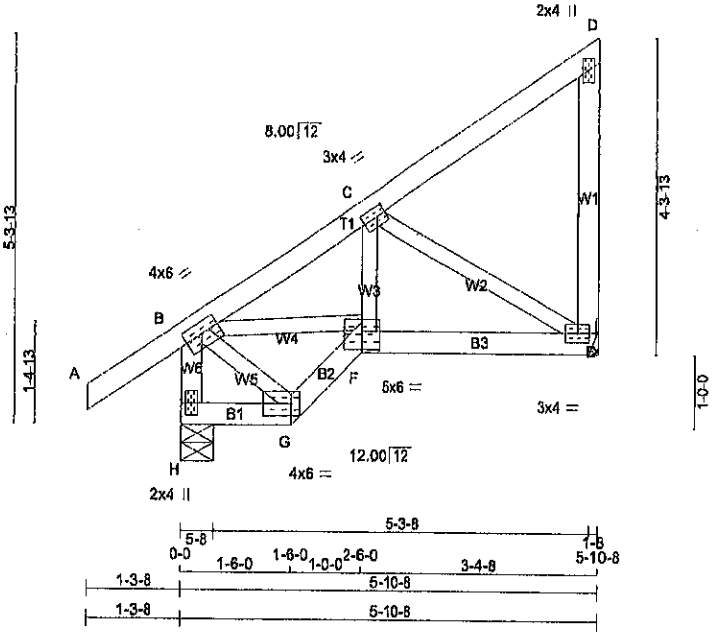

TOTAL WEIGHT = 4 X 30 = 119 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-6	1-8
E	258	0	258	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-B	-342 / 0	0.0	0.0	0.04 (1)	7.81	B-G	0 / 18	0.01 (4)
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B-F	0 / 238	0.06 (1)
B-C	-287 / 0	-70.5	-70.5	0.10 (1)	6.25	F-C	0 / 67	0.02 (4)
C-D	-12 / 0	-70.5	-70.5	0.10 (1)	6.25	C-E	-284 / 0	0.07 (1)
E-D	-93 / 0	0.0	0.0	0.03 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.01 (4)	10.00			
G-F	0 / 20	-17.5	-17.5	0.01 (4)	10.00			
F-E	0 / 248	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.08 (E-F:4) , WB=0.07 (C-E:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073630

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
263433	J01B	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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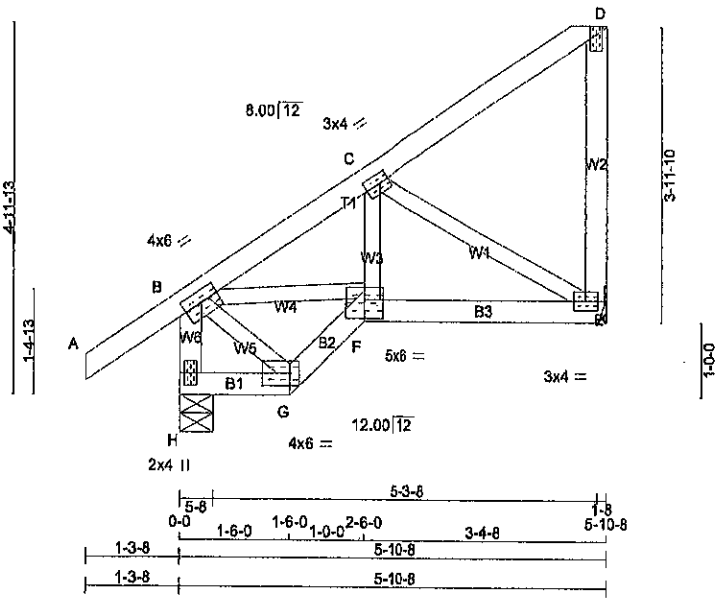

TOTAL WEIGHT = 30 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0	Edge	
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-B	-342 / 0	0.0	0.0	4 (1)	7.81		B-G	0 / 18	0.01 (4)
A-B	0 / 27	-70.5	-70.5	8 (1)	10.00		B-F	0 / 238	0.05 (1)
B-C	-267 / 0	-70.5	-70.5	5 (1)	6.25		F-C	0 / 57	0.02 (4)
C-D	-12 / 0	-70.5	-70.5	5 (1)	6.25		C-E	-284 / 0	0.07 (1)
E-D	-93 / 0	0.0	0.0	3 (1)	7.81				
H-G	0 / 0	-17.5	-17.5	1 (4)	10.00				
G-F	0 / 20	-17.5	-17.5	1 (4)	10.00				
F-E	0 / 248	-17.5	-17.5	8 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.08 (E-F:4) , WB=0.07 (C-E:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1507362 9

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253433	J01A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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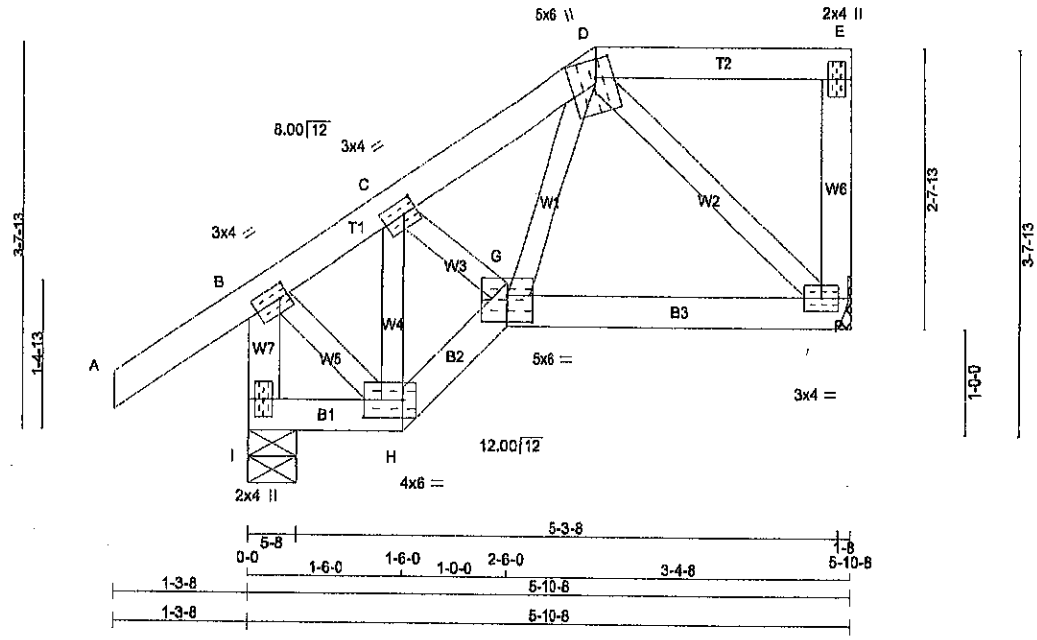

TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	2.00
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BBWW-I	MT20	5.0	6.0	3.00	3.00
H	BBWW-I	MT20	4.0	6.0	2.00	4.50
I	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	247	0	247	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	366	0	366	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	174	117 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
I	256	188 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.6	-70.5	0.09 (1)	10.00	
B- C	-168 / 0	-70.6	-70.5	0.09 (1)	6.25	
C- D	-213 / 0	-70.6	-70.5	0.04 (1)	6.25	
D- E	0 / 0	-70.6	-70.5	0.07 (1)	10.00	
F- E	-88 / 0	0.0	0.0	0.01 (1)	7.81	
I- B	-353 / 0	0.0	0.0	0.04 (1)	7.81	
I- H	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
H- G	0 / 157	-17.5	-17.5	0.03 (1)	10.00	
G- F	0 / 139	-17.5	-17.5	0.07 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-174 / 0	0.03 (1)
C- G	0 / 86	0.02 (1)
G- D	0 / 101	0.02 (1)
D- F	-191 / 0	0.04 (1)
B- H	0 / 147	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.09 (A-B:1) , BC=0.07 (F-G:4) , WB=0.04 (D-F:1) , SSI=0.07 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

A-N073628

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253433	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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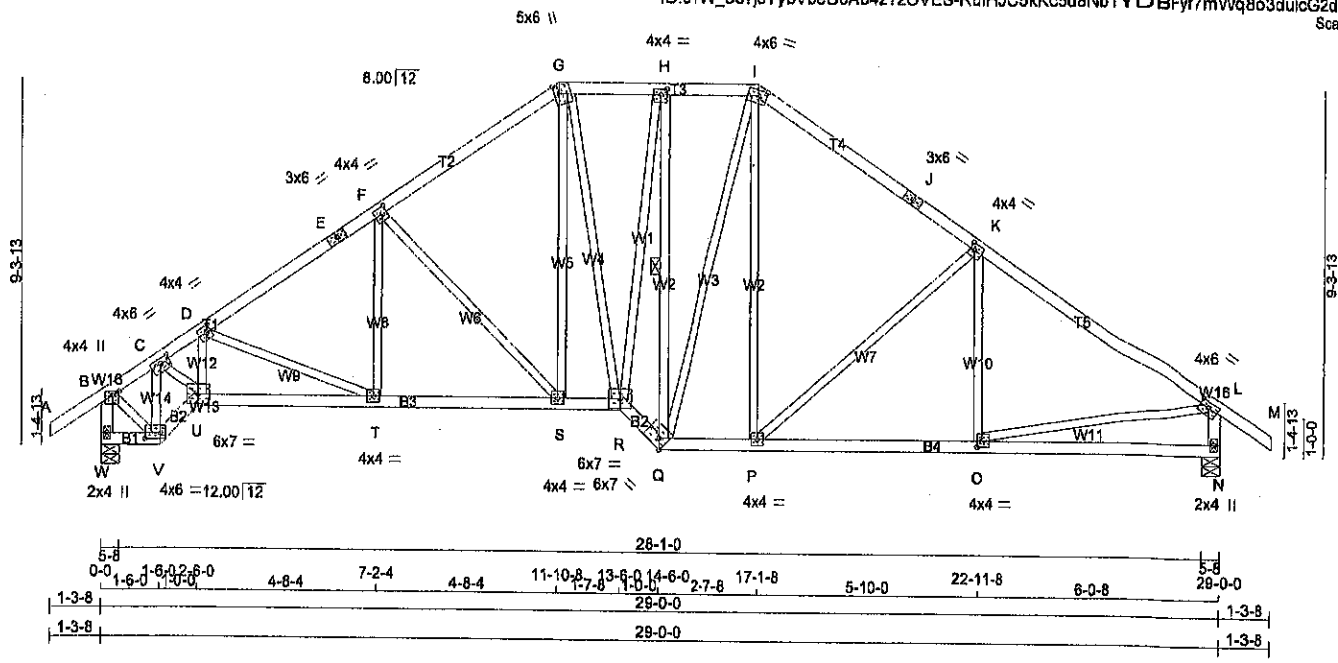

TOTAL WEIGHT = 162 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.75
C	TMWW-t	MT20	4.0	6.0	1.50	3.00
D, F, K						
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TS-t	MT20	3.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.50	1.50
H	TMWW-t	MT20	4.0	4.0	1.75	1.75
I	TTWW-m	MT20	4.0	6.0	1.75	2.50
J	TS-t	MT20	3.0	6.0		
L	TMVW-t	MT20	4.0	6.0	1.75	3.00
N	BMV1+p	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	4.0	1.75	1.50
P, S, T						
P	BMWW-t	MT20	4.0	4.0		
Q	BBWW-h	MT20	6.0	7.0	Edge	2.50
R	BBWW-I	MT20	6.0	7.0		
U	BBWW-I	MT20	6.0	7.0		
V	BBWW-I	MT20	4.0	6.0	1.75	4.50
W	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	1374	0	1374	0	0	5-8	1-8
N	1370	0	1370	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	968	667 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
N	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.73FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF H-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
B- C	-1040 / 0	-70.5	-70.5	0.08 (1)	6.08	
C- D	-1975 / 0	-70.5	-70.5	0.10 (1)	4.73	
D- E	-1725 / 0	-70.5	-70.5	0.22 (1)	4.88	
E- F	-1725 / 0	-70.5	-70.5	0.22 (1)	4.88	
F- G	-1364 / 0	-70.5	-70.5	0.21 (1)	5.38	
G- H	-1142 / 0	-70.5	-70.5	0.07 (1)	5.88	
H- I	-1029 / 0	-70.5	-70.5	0.06 (1)	6.13	
I- J	-1219 / 0	-70.5	-70.5	0.35 (1)	5.40	
J- K	-1219 / 0	-70.5	-70.5	0.35 (1)	5.40	
K- L	-1489 / 0	-70.5	-70.5	0.38 (1)	4.88	
L- M	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
W- B	-1361 / 0	0.0	0.0	0.14 (1)	6.96	
N- L	-1326 / 0	0.0	0.0	0.14 (1)	7.03	
W- V	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
V- U	0 / 1068	-17.5	-17.5	0.17 (1)	10.00	
U- T	0 / 1691	-17.5	-17.5	0.32 (1)	10.00	
T- S	0 / 1445	-17.5	-17.5	0.28 (1)	10.00	
S- R	0 / 1114	-17.5	-17.5	0.20 (1)	10.00	
R- Q	0 / 1437	-17.5	-17.5	0.23 (1)	10.00	
Q- P	0 / 993	-17.5	-17.5	0.20 (1)	10.00	
P- O	0 / 1262	-17.5	-17.5	0.28 (1)	10.00	
O- N	0 / 0	-17.5	-17.5	0.16 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1311 / 0	0.20 (1)
C- U	0 / 1260	0.28 (1)
U- D	-21 / 42	0.01 (4)
D- T	-264 / 0	0.11 (1)
T- F	0 / 179	0.05 (4)
F- S	-480 / 0	0.42 (1)
S- G	0 / 365	0.09 (1)
G- R	0 / 141	0.03 (1)
R- H	0 / 819	0.21 (1)
Q- H	-1109 / 0	0.63 (1)
Q- I	0 / 128	0.03 (1)
P- I	0 / 305	0.07 (1)
P- K	-359 / 0	0.45 (1)
O- K	-107 / 64	0.06 (1)
B- V	0 / 1006	0.23 (1)
O- L	0 / 1282	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.38 (K-L:1) , BC=0.32 (T-U:1) , WB=0.63 (H-Q:1) , SSI=0.17 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (V) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

A-1073627

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253433	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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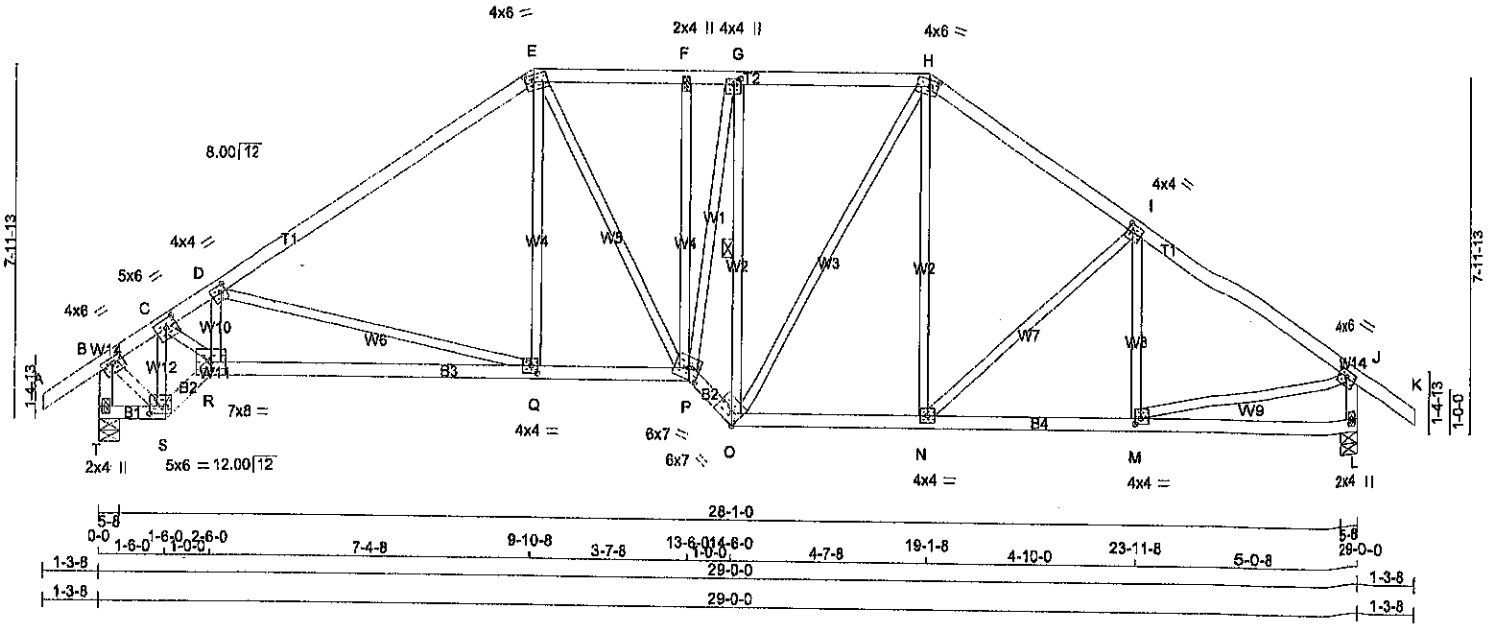

TOTAL WEIGHT = 142 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	3.00
C	TMWW-t	MT20	5.0	6.0	2.25	3.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	4.0	4.0	1.50	1.75
H	TTWW-m	MT20	4.0	6.0	1.75	2.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-t	MT20	4.0	6.0	1.75	3.00
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0	1.50	1.50
N	BMWW-t	MT20	4.0	4.0		
O	BBWW-h	MT20	6.0	7.0	Edge	2.50
P	BBWWW-m	MT20	6.0	7.0	3.00	3.00
Q	BMWW-t	MT20	4.0	4.0	2.00	1.75
R	BBWW-t	MT20	7.0	8.0		
S	BBWW-t	MT20	5.0	6.0	2.00	4.60
T	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1374	0	1374	0	0	5-8	1-8
L	1370	0	1370	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	966	867 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B-C	-1024 / 0	-70.5	-70.5	0.14 (1)		6.04
C-D	-1898 / 0	-70.5	-70.5	0.43 (1)		4.29
D-E	-1529 / 0	-70.5	-70.5	0.64 (1)		4.72
E-F	-1380 / 0	-70.5	-70.5	0.16 (1)		5.40
F-G	-1379 / 0	-70.5	-70.5	0.17 (1)		5.36
G-H	-1224 / 0	-70.5	-70.5	0.17 (1)		5.62
H-I	-1324 / 0	-70.5	-70.5	0.24 (1)		5.37
I-J	-1475 / 0	-70.5	-70.5	0.25 (1)		5.15
J-K	0 / 27	-70.5	-70.5	0.09 (1)		10.00
T-B	-1361 / 0	0.0	0.0	0.14 (1)		6.96
L-J	-1332 / 0	0.0	0.0	0.14 (1)		7.02
T-S	0 / 0	-17.5	-17.5	0.01 (4)		10.00
S-R	0 / 1023	-17.5	-17.5	0.17 (1)		10.00
R-Q	0 / 1866	-17.5	-17.5	0.42 (1)		10.00
Q-P	0 / 1251	-17.5	-17.5	0.32 (1)		10.00
P-O	0 / 1704	-17.5	-17.5	0.28 (1)		10.00
O-N	0 / 1084	-17.5	-17.5	0.23 (1)		10.00
N-M	0 / 1246	-17.5	-17.5	0.25 (1)		10.00
M-L	0 / 0	-17.5	-17.5	0.10 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-C	-1255 / 0	0.19 (1)
C-R	0 / 1612	0.34 (1)
R-D	-211 / 40	0.03 (1)
D-Q	-638 / 0	0.76 (1)
Q-E	0 / 267	0.06 (4)
E-P	0 / 270	0.06 (1)
P-F	-60 / 6	0.05 (1)
P-G	0 / 1067	0.24 (1)
O-G	-1388 / 0	0.66 (1)
O-H	0 / 270	0.06 (1)
N-H	0 / 234	0.05 (1)
N-I	-222 / 0	0.18 (1)
M-I	-175 / 23	0.08 (1)
B-S	0 / 963	0.22 (1)
M-J	0 / 1274	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.64 (D-E:1) , BC=0.42 (Q-R:1), WB=0.76 (D-Q:1) , SSI=0.30 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.47 (M) (INPUT = 1.00)

A-15073626

	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253433	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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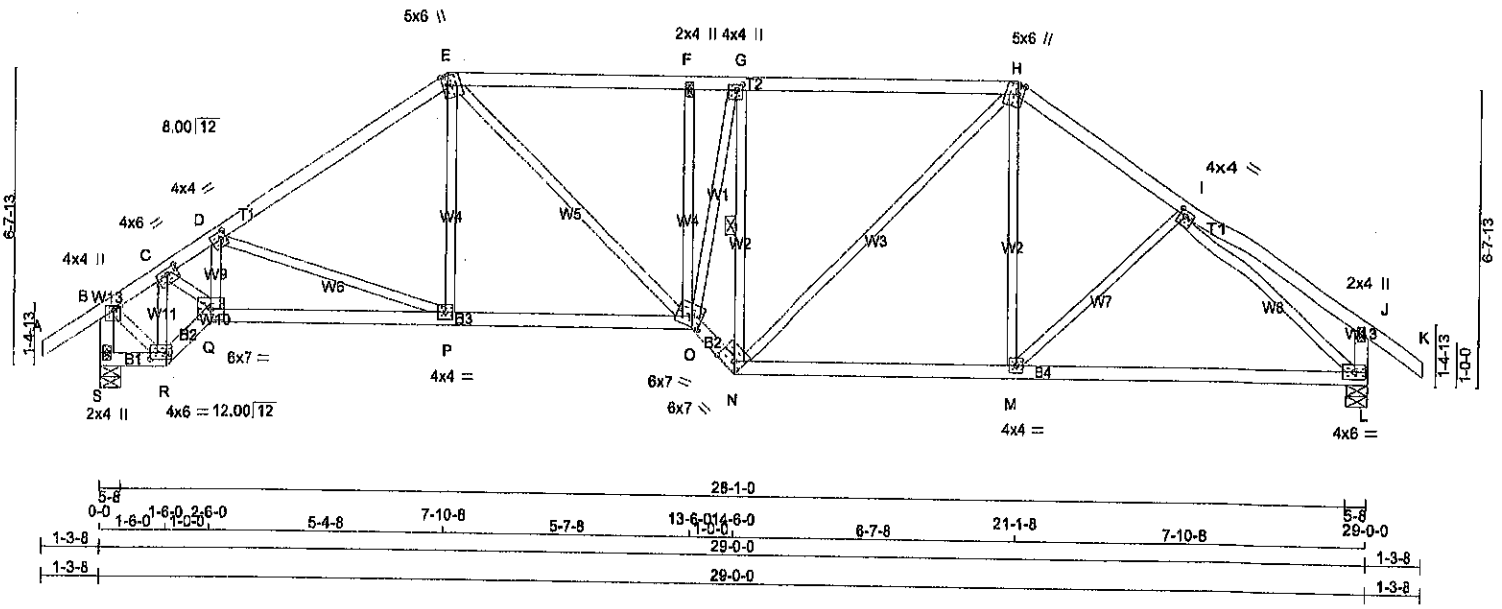

TOTAL WEIGHT = 130 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.75
C	TMWW-t	MT20	4.0	6.0	1.50	3.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	4.0	4.0	1.50	1.50
H	TTWW+m	MT20	5.0	6.0	2.50	1.50
I	TMWW-t	MT20	4.0	4.0	1.50	1.75
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BBWW-h	MT20	6.0	7.0	2.25	4.50
O	BBWWW-m	MT20	6.0	7.0	2.75	3.00
P	BMWW-t	MT20	4.0	4.0		
Q	BBWW-l	MT20	6.0	7.0		
R	BBWW-l	MT20	4.0	6.0	1.75	4.50
S	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1374	0	1374	0	0	5-8	1-8
L	1370	0	1370	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	966	667 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	963	665 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.08 (1)	10.00
B- C	-1034 / 0	-70.5	-70.5	0.10 (1)	6.07
C- D	-1984 / 0	-70.5	-70.5	0.23 (1)	4.59
D- E	-1665 / 0	-70.5	-70.5	0.27 (1)	4.89
E- F	-1753 / 0	-70.5	-70.5	0.31 (1)	4.74
F- G	-1760 / 0	-70.5	-70.5	0.35 (1)	4.66
G- H	-1508 / 0	-70.5	-70.5	0.39 (1)	4.92
H- I	-1421 / 0	-70.5	-70.5	0.15 (1)	5.33
I- J	0 / 17	-70.5	-70.5	0.16 (1)	10.00
J- K	0 / 27	-70.5	-70.5	0.09 (1)	10.00
S- B	-1361 / 0	0.0	0.0	0.14 (1)	6.96
L- J	-205 / 0	0.0	0.0	0.02 (1)	7.81
S- R	0 / 0	-17.5	-17.5	0.01 (4)	10.00
R- Q	0 / 1051	-17.5	-17.5	0.17 (1)	10.00
Q- P	0 / 1756	-17.5	-17.5	0.37 (1)	10.00
P- O	0 / 1372	-17.5	-17.5	0.30 (1)	10.00
O- N	0 / 2092	-17.5	-17.5	0.34 (1)	10.00
N- M	0 / 1169	-17.5	-17.5	0.37 (4)	10.00
M- L	0 / 1178	-17.5	-17.5	0.36 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-1291 / 0	0.20 (1)
C- Q	0 / 1347	0.30 (1)
Q- D	-96 / 37	0.01 (1)
D- P	-410 / 0	0.23 (1)
P- E	0 / 233	0.06 (4)
E- O	0 / 524	0.12 (1)
O- F	-136 / 0	0.07 (1)
O- G	0 / 1336	0.30 (1)
N- G	-1749 / 0	0.50 (1)
N- H	0 / 469	0.11 (1)
M- H	0 / 163	0.05 (4)
M- I	-15 / 48	0.02 (4)
B- R	0 / 990	0.22 (1)
I- L	-1617 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS
6.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.39 (G-H:1) , BC=0.37 (M-N:4) , WB=0.77 (I-L:1) , SSI=0.22 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.60 (I) (INPUT = 1.00)

A-1073625

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253433	H01T	1	1	TRUSS DESC.	

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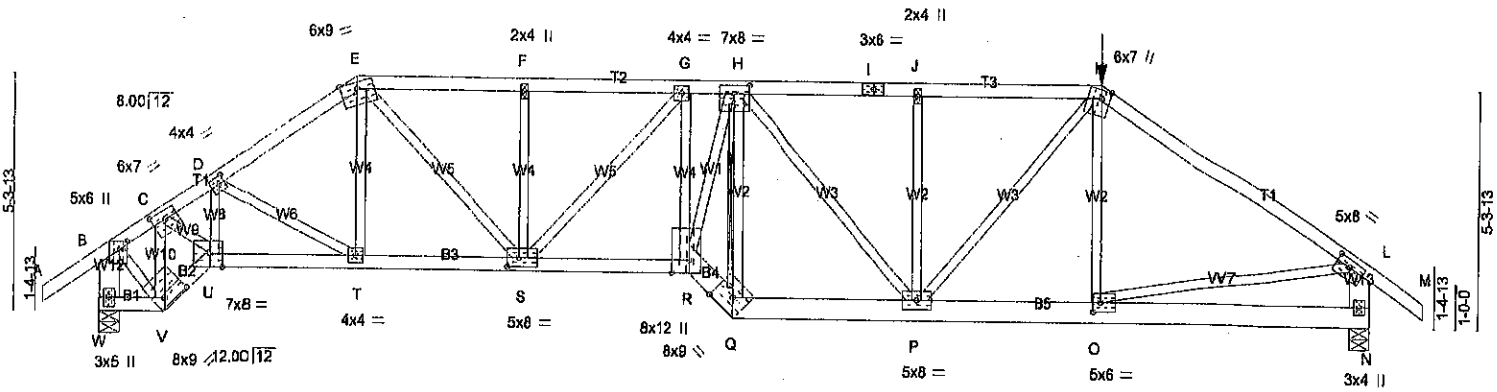
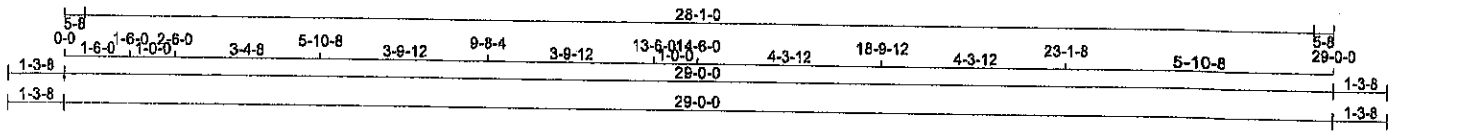




TOTAL WEIGHT = 142 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - R	2x4	DRY	1650F 1.5E	SPF
R - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.25
C	TMWW-t	MT20	6.0	7.0	2.25	3.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	6.0	9.0	Edge	
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWWW-t	MT20	7.0	8.0	2.25	4.00
I	TS-t	MT20	3.0	6.0		
J	TMW+w	MT20	2.0	4.0		
K	TTWW+m	MT20	6.0	7.0	2.25	2.25
L	TMVW-t	MT20	5.0	8.0	2.00	3.75
N	BMV1+p	MT20	3.0	4.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BMWWW-t	MT20	5.0	8.0		
Q	BBW-h	MT20	8.0	9.0	3.75	5.00
R	BBWW+p	MT20	8.0	12.0	Edge	4.75
S	BMWWW-t	MT20	5.0	8.0	2.50	3.00
T	BMWW-t	MT20	4.0	4.0		
U	BBWW-l	MT20	7.0	8.0	Edge	3.50
V	BBWW-h	MT20	8.0	9.0	2.00	6.25
W	BMV1+p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2641	0	2641	0	0	5-8	3-1
N	2489	0	2489	0	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1857	1275 / 0	0 / 0	0 / 0	0 / 0	582 / 0	0 / 0
N	1754	1165 / 0	0 / 0	0 / 0	0 / 0	569 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.72FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT H-Q
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.10 (1)	10.00
B-C	-2095 / 0	-138.7	-138.7	0.18 (1)	4.49
C-D	-3984 / 0	-138.7	-138.7	0.32 (1)	3.22
D-E	-3693 / 0	-70.5	-70.5	0.32 (1)	3.43
E-F	-4131 / 0	-70.5	-70.5	0.43 (1)	3.10
F-G	-4131 / 0	-70.5	-70.5	0.43 (1)	3.09
G-H	-4667 / 0	-138.7	-138.7	0.52 (1)	2.72
H-I	-3472 / 0	-138.7	-138.7	0.60 (1)	3.13
I-J	-3472 / 0	-138.7	-138.7	0.60 (1)	3.13
J-K	-3472 / 0	-138.7	-138.7	0.61 (1)	3.13
K-L	-3019 / 0	-70.5	-70.5	0.87 (1)	3.24
L-M	0 / 27	-70.5	-70.5	0.10 (1)	10.00
W-B	-2615 / 0	0.0	0.0	0.19 (1)	6.39
N-L	-2401 / 0	0.0	0.0	0.17 (1)	6.61
W-V	0 / 0	-34.5	-34.5	0.02 (4)	10.00
V-U	0 / 2194	-34.5	-34.5	0.39 (1)	10.00
U-T	0 / 3349	-102.7	-102.7	0.59 (1)	10.00
T-S	0 / 2995	-102.7	-102.7	0.54 (1)	10.00
S-R	0 / 4652	-102.7	-102.7	0.76 (1)	10.00
R-Q	0 / 5182	-34.5	-34.5	0.68 (1)	10.00
Q-P	0 / 3615	-34.5	-34.5	0.59 (1)	10.00
P-O	0 / 2505	-34.5	-34.5	0.40 (1)	10.00
O-N	0 / 0	-34.5	-34.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-2697 / 0	0.45 (1)
C-U	0 / 2414	0.60 (1)
U-D	0 / 138	0.03 (4)
D-T	-419 / 0	0.11 (1)
T-E	0 / 575	0.14 (1)
E-S	0 / 1650	0.41 (1)
S-F	-321 / 0	0.09 (1)
S-G	-768 / 0	0.39 (1)
R-G	0 / 574	0.14 (1)
R-H	0 / 3428	0.85 (1)
Q-H	-3433 / 0	0.78 (1)
H-P	-529 / 0	0.39 (1)
P-J	-867 / 0	0.27 (1)
P-K	0 / 1468	0.36 (1)
O-K	-188 / 105	0.08 (1)
B-V	0 / 2067	0.51 (1)
O-L	0 / 2544	0.63 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	23-1-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 13-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 15-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 781 (0.46")

CSI: TC=0.87 (K-L:1) , BC=0.76 (R-S:1) , WB=0.65 (H-R:1) , SSI=0.31 (J-K:1)

A-150736

CONTINUED ON PAGE 2

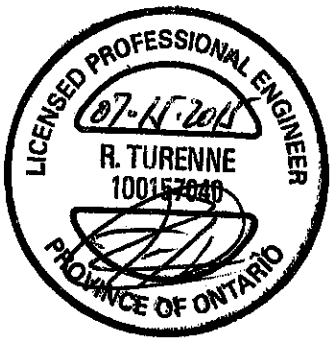
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 23-1-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	364	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.69 (R) (INPUT = 1.00)

A-N073624(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
263432	H14	1	2	TRUSS DESC.	

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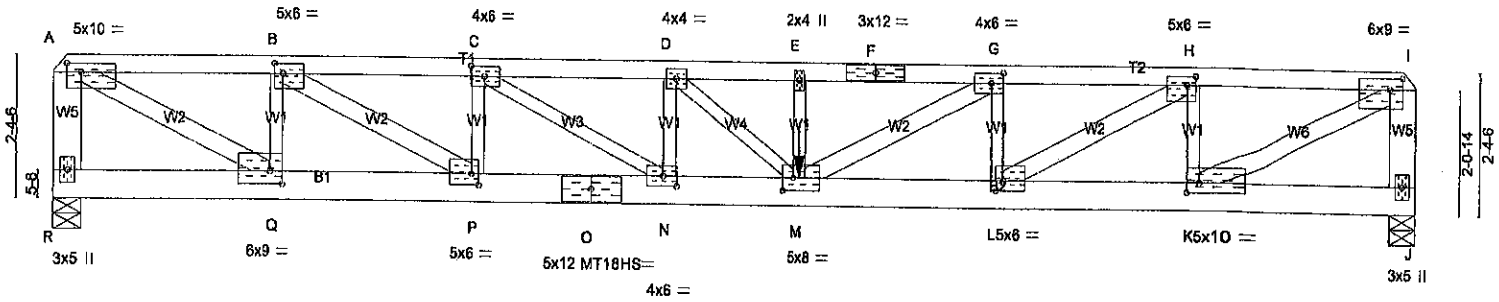

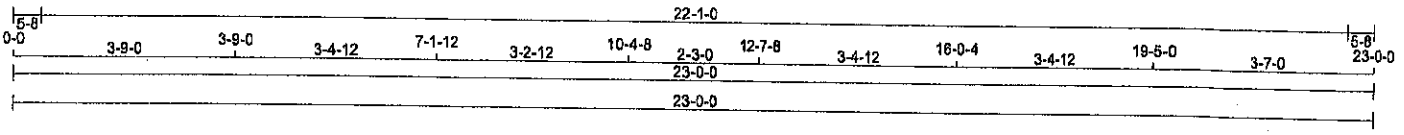

TOTAL WEIGHT = 2 X 106 = 212 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	2100F 1.8E	SPF
F - I	2x4	DRY	2100F 1.8E	SPF
R - A	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
R - O	2x6	DRY	2100F 1.8E	SPF
O - J	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - Q	2x4	DRY	1650F 1.5E	SPF
B - P	2x4	DRY	1650F 1.5E	SPF
M - G	2x4	DRY	1650F 1.5E	SPF
L - H	2x4	DRY	1650F 1.5E	SPF
K - I	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- F	1	12	SIDE(21.3)
F- I	1	12	TOP
R- A	2	12	TOP
J- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	12	SIDE(245.8)
O- J	2	12	SIDE(288.4)
WEBS : (0.122"X3") SPIRAL NAILS			
E- M	1	3	SIDE(123.2)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	7266	0	7266	0	0	5-8	4-2
J	7014	0	7014	0	0	5-8	3-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	5142	3354 / 0	0 / 0	0 / 0	0 / 0	1788 / 0	0 / 0
J	4969	3210 / 0	0 / 0	0 / 0	0 / 0	1760 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J
BEARING SIZE FACTOR = 1.15 AT JNT(S) R (BASED ON SUPPORT DEPTH = 1-8)
BEARING SIZE FACTOR = 1.15 AT JNT(S) J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-11639 / 0	-113.0	-113.0	0.28 (1)	3.48
B- C	-18110 / 0	-113.0	-113.0	0.51 (1)	2.62
C- D	-20863 / 0	-113.0	-113.0	0.66 (1)	2.26
D- E	-20911 / 0	-113.0	-113.0	0.62 (1)	2.26
E- F	-20911 / 0	-70.5	-70.5	0.68 (1)	2.26
F- G	-20911 / 0	-70.5	-70.5	0.66 (1)	2.26
G- H	-17835 / 0	-70.5	-70.5	0.48 (1)	2.69
H- I	-10915 / 0	-70.5	-70.5	0.22 (1)	3.64
R- A	-6375 / 0	0.0	0.0	0.25 (1)	6.88
J- I	-6188 / 0	0.0	0.0	0.25 (1)	6.95
R- Q	0 / 0	-509.1	-509.1	0.16 (1)	10.00
Q- P	0 / 11639	-509.1	-509.1	0.43 (1)	10.00
P- O	0 / 18110	-509.1	-509.1	0.65 (1)	10.00
O- N	0 / 18110	-509.1	-509.1	0.65 (1)	10.00
N- M	0 / 20863	-509.1	-509.1	0.69 (1)	10.00
M- L	0 / 17635	-498.5	-498.5	0.65 (1)	10.00
L- K	0 / 10915	-498.5	-498.5	0.41 (1)	10.00
K- J	0 / 0	-498.5	-498.5	0.14 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- D	-354 / 0	0.03 (1)
D- M	0 / 285	0.05 (4)
M- E	-196 / 0	0.02 (1)
A- Q	0 / 13173	0.84 (1)
Q- B	-4212 / 0	0.35 (1)
B- P	0 / 7499	0.48 (1)
P- C	-2078 / 0	0.17 (1)
C- N	0 / 3232	0.40 (1)
M- G	0 / 3796	0.24 (1)
L- G	-2176 / 0	0.18 (1)
L- H	0 / 7787	0.50 (1)
K- H	-4181 / 0	0.34 (1)
K- I	0 / 12482	0.80 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	12-7-8	-524	-524	—	FRONT	VERT	DEAD

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 4-5-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 12-7-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 23-10-8
END DISTANCE = 23-0-0
END SPAN CARRIED = 23-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 728 (0.38")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 381 (0.73")

CSI: TC=0.66 (E-G:1) , BC=0.69 (M-N:1) , WB=0.84 (A-Q:1) , SSI=0.34 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

A-407362

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	10.0	1.75	3.00
B	TMWW-t	MT20	5.0	6.0	2.00	1.75
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	12.0		
G	TMWW-t	MT20	4.0	6.0	2.00	2.50
H	TMWW-t	MT20	5.0	6.0	1.75	1.75
I	TMVW-t	MT20	6.0	9.0	2.25	2.75
J	BMV1+p	MT20	3.0	5.0		
K	BMWW-t	MT20	5.0	10.0	2.00	2.50
L	BMWW-t	MT20	5.0	6.0	2.00	1.50
M	BMWWW-t	MT20	5.0	8.0	2.50	2.50
N	BMWW-t	MT20	4.0	6.0	2.00	2.75
O	BS-t	MT18HS	5.0	12.0		
P	BMWW-t	MT20	5.0	6.0	2.25	1.50
Q	BMWW-t	MT20	6.0	9.0	2.50	2.50
R	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 586.6 lbs FACTORED DOWN AT 12-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MT18HS	580	354	2729	1405	4191	2010

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.99 (I) (INPUT = 1.00)

A-N073623(2)

JOB NUMBER	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
263432	H13	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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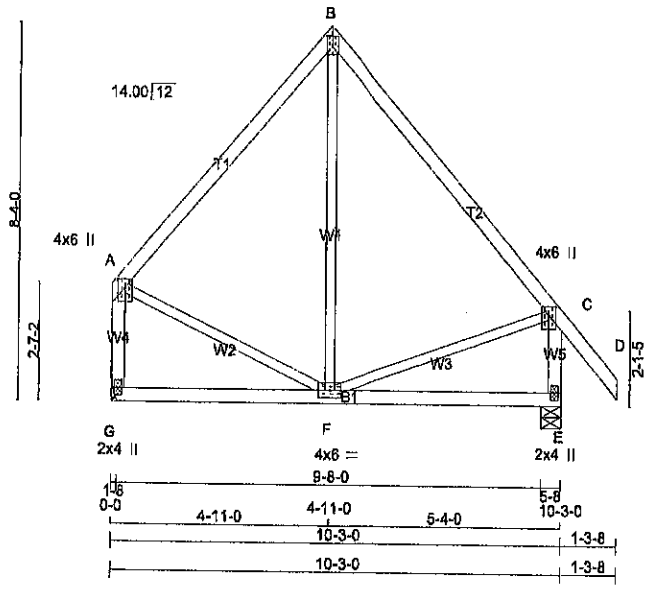

TOTAL WEIGHT = 2 X 52 = 103 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.25	2.00
B	TTW+p	MT20	3.0	5.0	2.75	1.50
C	TMVW+p	MT20	4.0	6.0	2.25	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	451	0	451	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	560	0	560	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	318	215 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
E	385	274 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB. FR-TO	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-233 / 0	-70.5	-70.5	0.22 (1)	6.25	F- B	-7 / 83	0.03 (4)	
B- C	-233 / 0	-70.5	-70.5	0.26 (1)	6.25	A- F	0 / 167	0.04 (1)	
C- D	0 / 38	-70.5	-70.5	0.10 (1)	10.00	F- C	0 / 160	0.04 (1)	
G- A	-419 / 0	0.0	0.0	0.06 (1)	7.81				
E- C	-513 / 0	0.0	0.0	0.06 (1)	7.81				
G- F	0 / 0	-17.5	-17.5	0.13 (4)	10.00				
F- E	0 / 0	-17.5	-17.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.26 (B-C:1) , BC=0.13 (E-F:4) , WB=0.04 (A-F:1) , SSI=0.10 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (C) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-150736 22

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H12	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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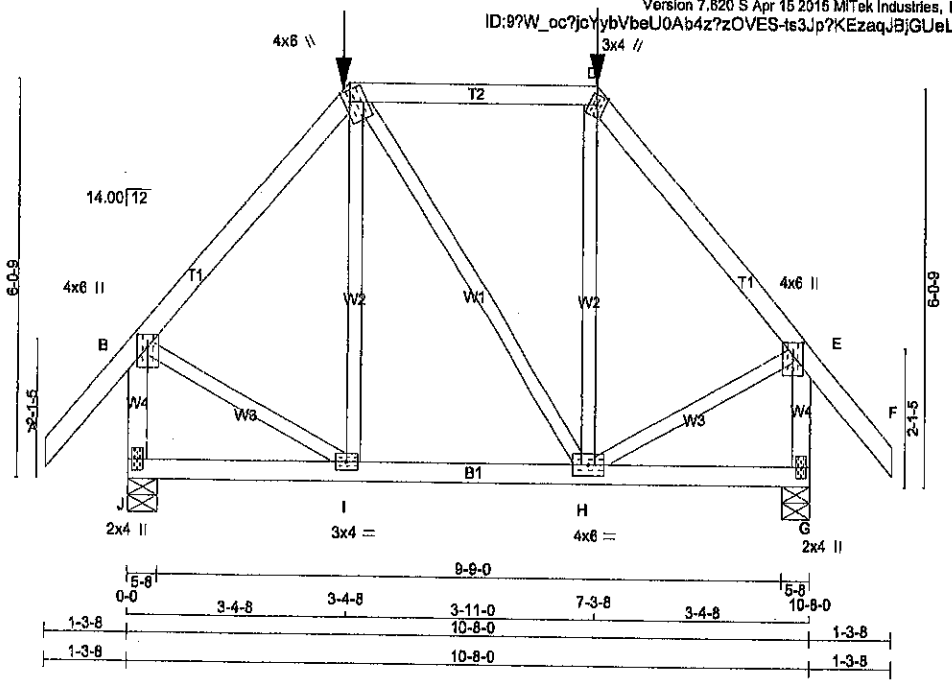

TOTAL WEIGHT = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	4.0	6.0	2.00	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMVW+p	MT20	4.0	6.0	2.25	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	3.0	4.0		
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 64.4 lbs FACTORED DOWN AT 7-3-8, AND 64.4 lbs FACTORED DOWN AT 3-4-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	697	0	697	0	0	5-8	1-8
G	697	0	697	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	489	344 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
G	489	344 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.26FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 38	-70.5	-70.5	0.11 (1)	10.00	I- C	-45 / 67	0.03 (1)
B- C	-413 / 0	-70.5	-70.5	0.16 (1)	6.25	C- H	0 / 0	0.00 (1)
C- D	-266 / 0	-90.3	-90.3	0.26 (1)	6.25	H- D	-46 / 67	0.03 (1)
D- E	-412 / 0	-70.5	-70.5	0.16 (1)	6.25	B- I	0 / 299	0.07 (1)
E- F	0 / 38	-70.5	-70.5	0.11 (1)	10.00	H- E	0 / 298	0.07 (1)
J- B	-666 / 0	0.0	0.0	0.09 (1)	7.81			
G- E	-666 / 0	0.0	0.0	0.09 (1)	7.81			
J- I	0 / 0	-22.4	-22.4	0.07 (4)	10.00			
I- H	0 / 266	-22.4	-22.4	0.10 (4)	10.00			
H- G	0 / 0	-22.4	-22.4	0.07 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-4-8	-64	-64	---	FRONT	VERT	TOTAL
D	7-3-8	-64	-64	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-4-8
END SETBACK = 3-1-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.26 (C-D:1) , BC=0.10 (H-I:4) , WB=0.07 (B-I:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.12 (J) (INPUT = 1.00)

A-H073621

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H11	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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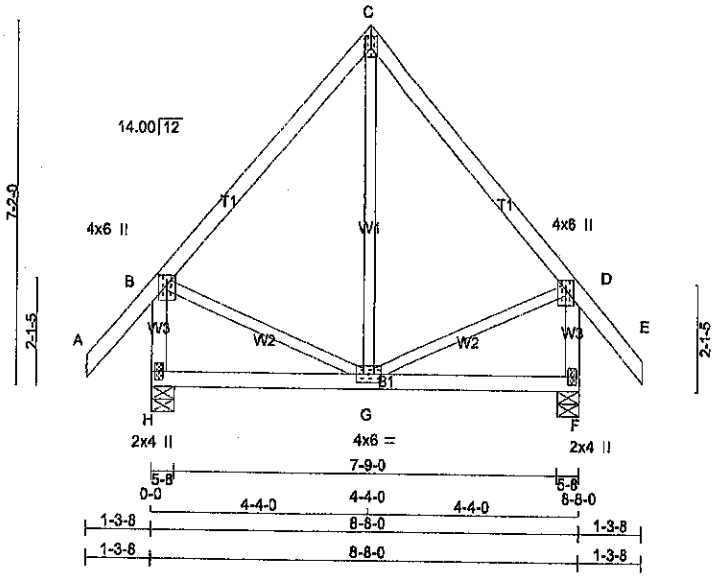

TOTAL WEIGHT = 46 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTW+p	MT20	3.0	5.0	2.75	1.50
D	TMVW+p	MT20	4.0	6.0	2.25	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-I	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	587	0	587	0	0	5-8	1-8
F	587	0	587	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	411	292 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
F	411	292 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 38	-70.5	-70.5	0.11 (1)	10.00
B-C	-261 / 0	-70.5	-70.5	0.25 (1)	6.25
C-D	-261 / 0	-70.5	-70.5	0.25 (1)	6.25
D-E	0 / 38	-70.5	-70.5	0.11 (1)	10.00
H-B	-517 / 0	0.0	0.0	0.07 (1)	7.81
F-D	-517 / 0	0.0	0.0	0.07 (1)	7.81
H-G	0 / 0	-42.2	-42.2	0.14 (4)	10.00
G-F	0 / 0	-42.2	-42.2	0.14 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	0 / 101	0.04 (4)
B-G	0 / 183	0.05 (1)
G-D	0 / 183	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 3-1-8
END DISTANCE = 8-8-0
END SPAN CARRIED = 3-1-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.25 (C-D:1) , BC=0.14 (F-G:4) ,
WB=0.05 (B-G:1) , SSI=0.10 (F-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (D) (INPUT = 0.90)
JSI METAL= 0.10 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.N.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-N073620

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H10	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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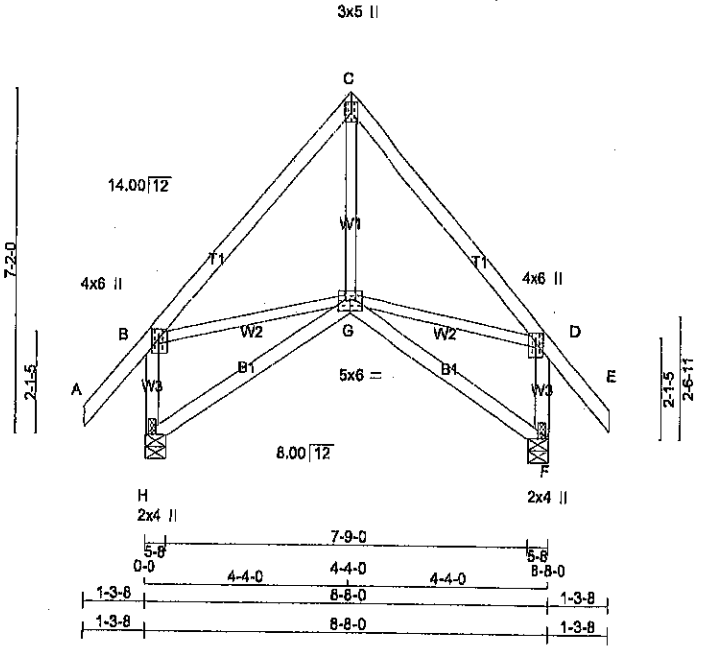

TOTAL WEIGHT = 2 X 47 = 93 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTW+p	MT20	3.0	5.0	2.75	1.50
D	TMVW+p	MT20	4.0	6.0	2.25	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	460	0	460	0	0	5-8	1-8
F	460	0	460	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	336	241 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	336	241 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-70.5	-70.5	0.10 (1)	10.00	G- C	0 / 152	0.04 (4)	
B- C	-301 / 0	-70.5	-70.5	0.17 (1)	6.25	B- G	0 / 200	0.04 (1)	
C- D	-301 / 0	-70.5	-70.5	0.17 (1)	6.25	G- D	0 / 200	0.04 (1)	
D- E	0 / 38	-70.5	-70.5	0.10 (1)	10.00				
H- B	-442 / 0	0.0	0.0	0.06 (1)	7.81				
F- D	-442 / 0	0.0	0.0	0.06 (1)	7.81				
H- G	0 / 0	-17.5	-17.5	0.10 (4)	10.00				
G- F	0 / 0	-17.5	-17.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.17 (C-D:1) , BC=0.10 (G-H:4) , WB=0.04 (D-G:1) , SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (F) (INPUT = 0.90)
JSI METAL= 0.11 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-N073619

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H09G	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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3x4 II

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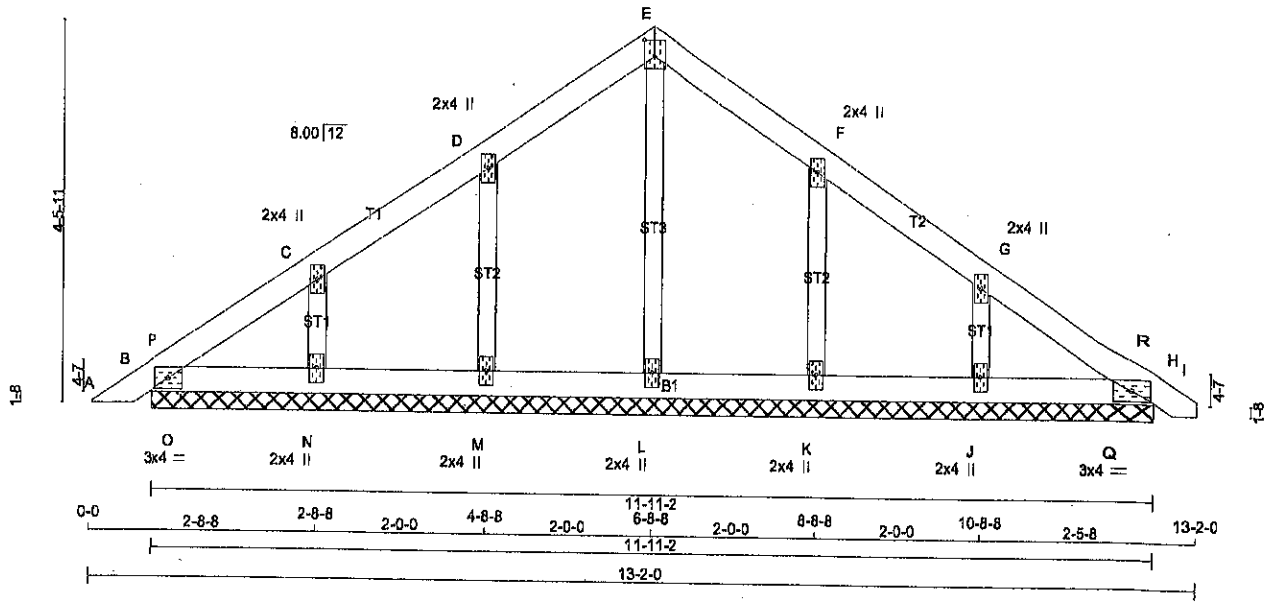

TOTAL WEIGHT = 2 X 41 = 81 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	3.0	4.0	2.25	1.50
H	TMB1-I	MT20	3.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 11	-70.5	-70.5	0.02 (1)	10.00
B-P	-45 / 0	-70.5	-70.5	0.01 (4)	6.25
P-C	-30 / 0	-70.5	-70.5	0.03 (1)	6.25
C-D	-26 / 0	-70.5	-70.5	0.04 (1)	6.25
D-E	-30 / 0	-70.5	-70.5	0.04 (1)	6.25
E-F	-30 / 0	-70.5	-70.5	0.04 (1)	6.25
F-G	-26 / 0	-70.5	-70.5	0.04 (1)	6.25
G-R	-30 / 0	-70.5	-70.5	0.03 (1)	6.25
R-H	-45 / 0	-70.5	-70.5	0.01 (4)	6.25
H-I	0 / 8	-70.5	-70.5	0.01 (1)	10.00
B-O	0 / 32	-17.5	-17.5	0.02 (1)	10.00
O-N	0 / 32	-17.5	-17.5	0.02 (1)	10.00
N-M	0 / 24	-17.5	-17.5	0.02 (1)	10.00
M-L	0 / 19	-17.5	-17.5	0.01 (4)	10.00
L-K	0 / 19	-17.5	-17.5	0.01 (4)	10.00
K-J	0 / 24	-17.5	-17.5	0.02 (1)	10.00
J-Q	0 / 32	-17.5	-17.5	0.02 (1)	10.00
Q-H	0 / 32	-17.5	-17.5	0.02 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-E	-93 / 0	0.03 (1)
M-D	-159 / 0	0.03 (1)
N-C	-132 / 0	0.02 (1)
K-F	-159 / 0	0.03 (1)
J-G	-132 / 0	0.02 (1)
O-P	-40 / 7	0.00 (1)
Q-R	-40 / 7	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (E-F:1) , BC=0.02 (N-O:1) , WB=0.03 (F-K:1) , SSI=0.06 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (H) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)

A-100736 18

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H09	14	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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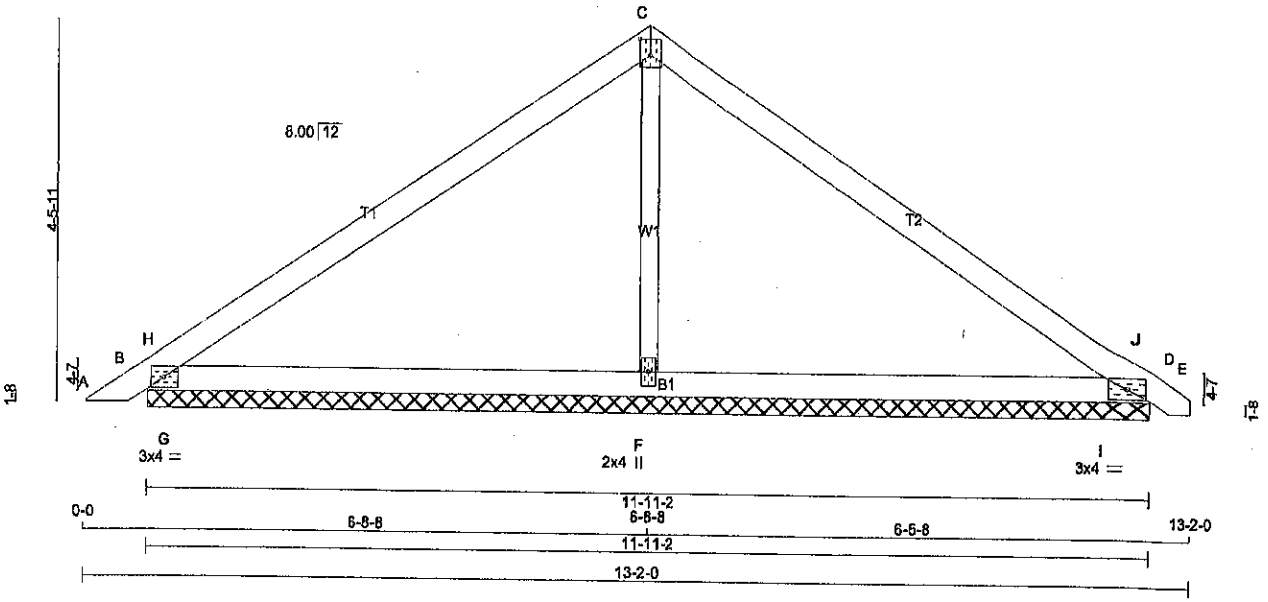

TOTAL WEIGHT = 14 X 35 = 486 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+p	MT20	3.0	4.0	2.26	1.60
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	394	0	394	0	0	11-11-2	1-8
D	383	0	383	0	0	11-11-2	1-8
F	362	0	352	0	0	11-11-2	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	274	203 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
D	267	197 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0
F	252	148 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 11	-70.5	-70.5	0.02 (1)		10.00	F-C	-128 / 0	0.04 (1)
B-H	-60 / 40	-70.5	-70.5	0.18 (1)		6.25	G-H	-613 / 0	0.00 (1)
H-C	-217 / 0	-70.5	-70.5	0.31 (1)		6.25	I-J	-613 / 0	0.00 (1)
C-J	-217 / 0	-70.5	-70.5	0.31 (1)		6.25			
J-D	-60 / 40	-70.5	-70.5	0.18 (1)		6.25			
D-E	0 / 8	-70.5	-70.5	0.01 (1)		10.00			
B-G	0 / 170	-17.5	-17.5	0.28 (1)		10.00			
G-F	0 / 170	-17.5	-17.5	0.28 (1)		10.00			
F-I	0 / 170	-17.5	-17.5	0.28 (1)		10.00			
I-D	0 / 170	-17.5	-17.5	0.28 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31 (C-H:1) , BC=0.28 (B-G:1) , WB=0.04 (C-F:1) , SSI=0.47 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
@7-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-150736 17

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H08G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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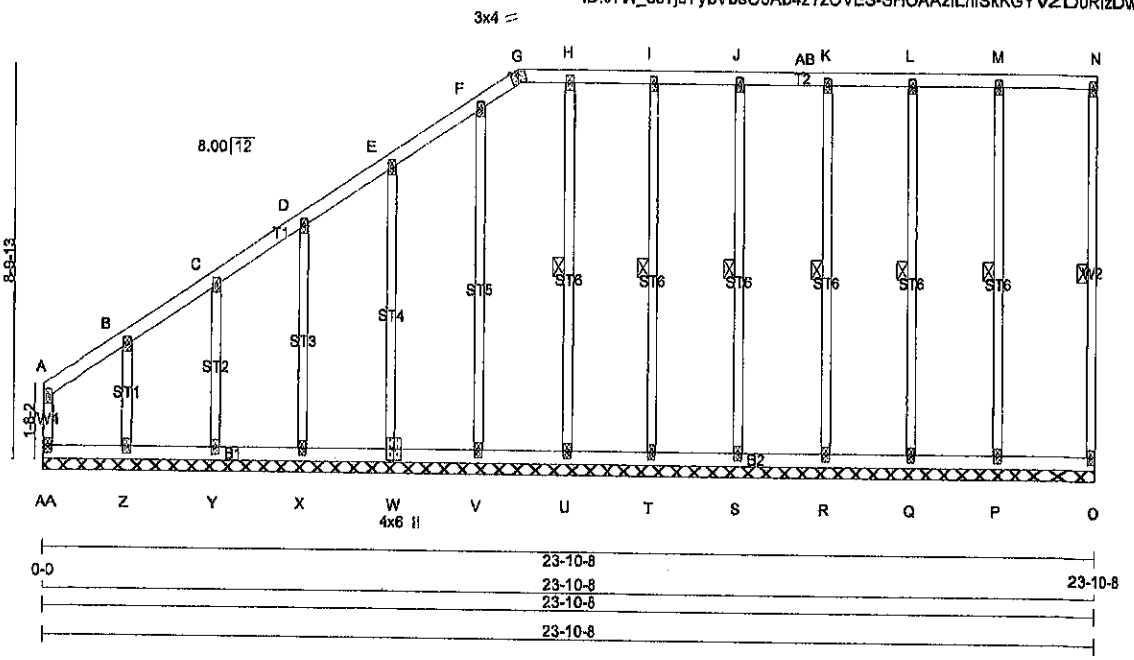

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AA- A	2x3	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - N	2x4	DRY	No.2	SPF
O - N	2x3	DRY	No.2	SPF
AA- W	2x4	DRY	No.2	SPF
W - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, D, E, F, H, I, J, K, L, M						
B	TMW+w	MT20	2.0	4.0		
G	TT-m	MT20	3.0	4.0	Edge	
N	TMV+p	MT20	2.0	4.0		
O	BMV1+p	MT20	2.0	4.0		
P, Q, R, S, T, U, V, X, Y, Z						
P	BMW1+w	MT20	2.0	4.0		
W	BSW1+!	MT20	4.0	6.0		
AA	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF N-O, M-P, L-Q, K-R, J-S, I-T, H-U. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
AA- A	-87 / 0	0.0	0.0	0.03 (1)	7.81
A- B	-15 / 0	-70.5	-70.5	0.03 (1)	6.25
B- C	-12 / 0	-70.5	-70.5	0.03 (1)	6.25
C- D	-9 / 0	-70.5	-70.5	0.03 (1)	10.00
D- E	-7 / 0	-70.5	-70.5	0.03 (1)	10.00
E- F	-2 / 0	-70.5	-70.5	0.03 (1)	10.00
F- G	-25 / 0	-70.5	-70.5	0.03 (1)	6.25
G- H	0 / 0	-78.0	-78.0	0.04 (1)	10.00
H- I	0 / 0	-78.0	-78.0	0.04 (1)	10.00
I- J	0 / 0	-78.0	-78.0	0.04 (1)	10.00
J-AB	0 / 0	-78.0	-78.0	0.04 (1)	10.00
AB- K	0 / 0	-78.0	-78.0	0.04 (1)	10.00
K- L	0 / 0	-78.0	-78.0	0.04 (1)	10.00
L- M	0 / 0	-78.0	-78.0	0.04 (1)	10.00
M- N	0 / 0	-78.0	-78.0	0.04 (1)	10.00
O- N	-66 / 0	0.0	0.0	0.01 (1)	6.25
AA- Z	0 / 15	-17.5	-17.5	0.02 (4)	10.00
Z- Y	0 / 11	-17.5	-17.5	0.02 (4)	10.00
Y- X	0 / 8	-17.5	-17.5	0.01 (4)	10.00
X- W	0 / 6	-17.5	-17.5	0.01 (4)	10.00
W- V	0 / 2	-17.5	-17.5	0.01 (4)	10.00
V- U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U- T	0 / 0	-17.5	-17.5	0.01 (4)	10.00
T- S	0 / 0	-17.5	-17.5	0.01 (4)	10.00
S- R	0 / 0	-17.5	-17.5	0.01 (4)	10.00
R- Q	0 / 0	-17.5	-17.5	0.01 (4)	10.00
Q- P	0 / 0	-17.5	-17.5	0.02 (4)	10.00
P- O	0 / 0	-17.5	-17.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- M	-169 / 0	0.08 (1)
Q- L	-166 / 0	0.08 (1)
R- K	-166 / 0	0.08 (1)
S- J	-166 / 0	0.08 (1)
T- I	-156 / 0	0.08 (1)
U- H	-159 / 0	0.08 (1)
V- F	-145 / 0	0.19 (1)
W- E	-138 / 0	0.11 (1)
X- D	-139 / 0	0.07 (1)
Y- C	-140 / 0	0.04 (1)
Z- B	-136 / 0	0.02 (1)

TOTAL WEIGHT = 126 lb

(M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (L-M:1) , BC=0.02 (O-P:4) , WB=0.19 (F-V:1) , SSI=0.07 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (N) (INPUT = 0.90)
JSI METAL= 0.05 (W) (INPUT = 1.00)

A-N073616

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H08	11	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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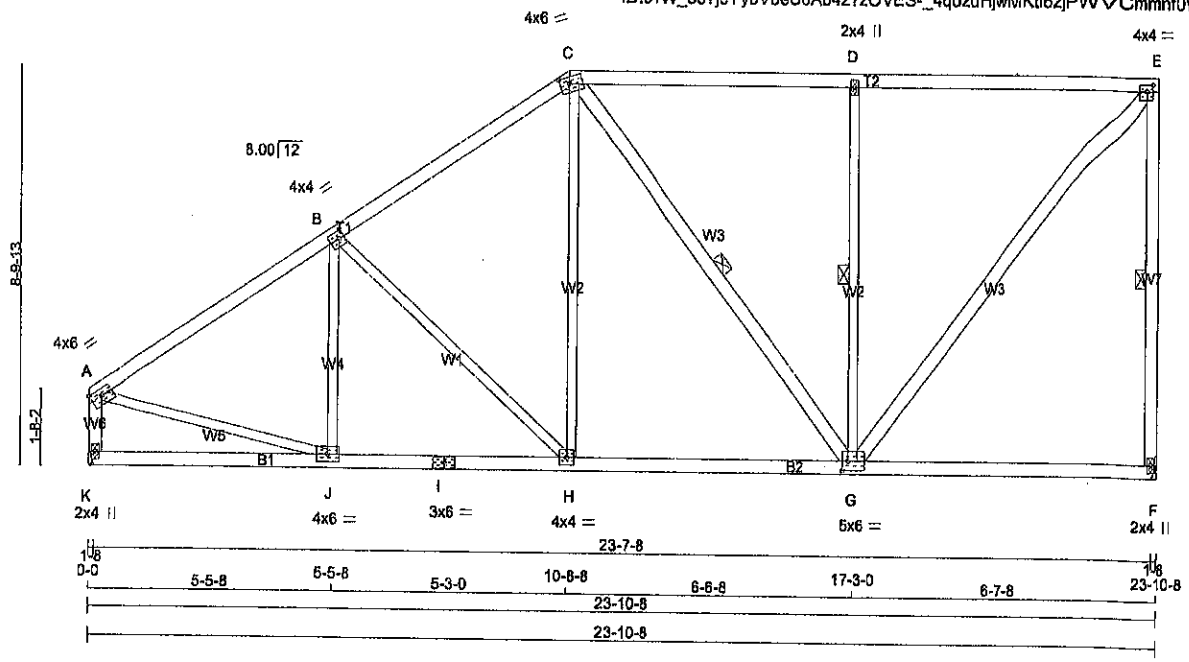

TOTAL WEIGHT = 11 X 119 = 1306 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0	1.75	1.75
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1121	0	1121	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
K	1077	0	1077	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	501 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0
K	762	501 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.83FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-F, C-G, D-G.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1143 / 0	-70.5	-70.5	0.28 (1)	5.83
B- C	-946 / 0	-70.5	-70.5	0.27 (1)	6.05
C- D	-683 / 0	-78.0	-78.0	0.44 (1)	6.25
D- E	-683 / 0	-78.0	-78.0	0.44 (1)	6.26
F- E	-1075 / 0	0.0	0.0	0.38 (1)	6.09
K- A	-1037 / 0	0.0	0.0	0.11 (1)	7.71
K- J	0 / 0	-17.5	-17.5	0.12 (4)	10.00
J- I	0 / 972	-17.5	-17.5	0.22 (1)	10.00
I- H	0 / 972	-17.5	-17.5	0.22 (1)	10.00
H- G	0 / 768	-17.5	-17.5	0.26 (4)	10.00
G- F	0 / 0	-17.5	-17.5	0.19 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-139 / 35	0.06 (1)
B- H	-283 / 0	0.30 (1)
H- C	0 / 291	0.07 (1)
A- J	0 / 1001	0.23 (1)
C- G	-140 / 0	0.09 (1)
G- D	-837 / 0	0.32 (1)
G- E	0 / 1113	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.44 (D-E:1) , BC=0.28 (G-H:4) , WB=0.32 (D-G:1) , SSI=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1666

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.31 (A) (INPUT = 1.00)

A-1507361 5

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H07G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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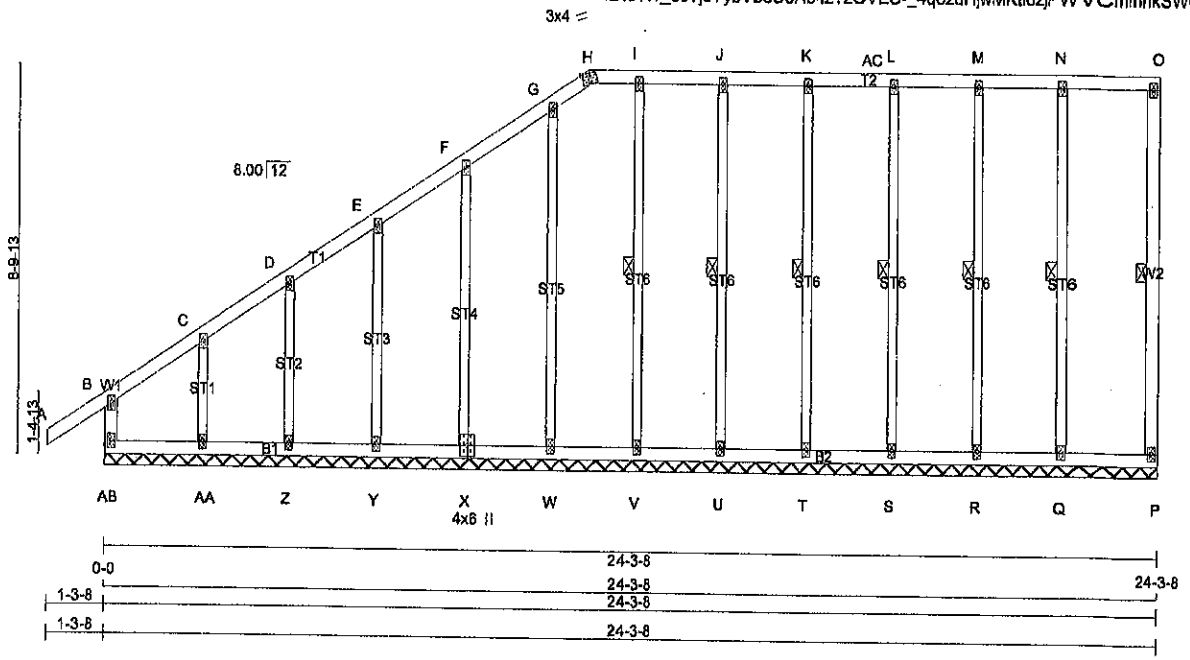

TOTAL WEIGHT = 131 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AB- B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AB- X	2x4	DRY	No.2	SPF
X - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, E, F, G, I, J, K, L, M, N						
C	TMW+w	MT20	2.0	4.0		
H	TT-m	MT20	3.0	4.0	Edge	
O	TMV+p	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		
Q, R, S, T, U, V, W, Y, Z, AA						
Q	BMW1+w	MT20	2.0	4.0		
X	BSW1+I	MT20	4.0	6.0		
AB	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF O-P, N-Q, M-R, L-S, K-T, J-U, I-V.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
AB- B	-188 / 0	0.0	0.0	0.01 (1)	7.81	
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
B- C	-21 / 0	-70.5	-70.5	0.04 (1)	6.25	
C- D	-15 / 0	-70.5	-70.5	0.03 (1)	6.25	
D- E	-10 / 0	-70.5	-70.5	0.03 (1)	6.25	
E- F	-8 / 0	-70.5	-70.5	0.03 (1)	10.00	
F- G	-3 / 0	-70.5	-70.5	0.03 (1)	10.00	
G- H	-26 / 0	-70.5	-70.5	0.03 (1)	6.25	
H- I	-1 / 0	-78.0	-78.0	0.04 (1)	10.00	
I- J	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	
J- K	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	
K-AC	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	
AC- L	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	
L- M	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	
M- N	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	
N- O	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	
P- O	-69 / 0	0.0	0.0	0.01 (1)	6.25	
AB-AA	0 / 16	-17.5	-17.5	0.02 (4)	10.00	
AA- Z	0 / 11	-17.5	-17.5	0.02 (4)	10.00	
Z- Y	0 / 9	-17.5	-17.5	0.02 (4)	10.00	
Y- X	0 / 6	-17.5	-17.5	0.02 (4)	10.00	
X- W	0 / 3	-17.5	-17.5	0.01 (4)	10.00	
W- V	0 / 2	-17.5	-17.5	0.01 (4)	10.00	
V- U	0 / 2	-17.5	-17.5	0.01 (4)	10.00	
U- T	0 / 2	-17.5	-17.5	0.01 (4)	10.00	
T- S	0 / 2	-17.5	-17.5	0.01 (4)	10.00	
S- R	0 / 2	-17.5	-17.5	0.01 (4)	10.00	
R- Q	0 / 2	-17.5	-17.5	0.02 (4)	10.00	
Q- P	0 / 2	-17.5	-17.5	0.02 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- N	-185 / 0	0.08 (1)
R- M	-156 / 0	0.08 (1)
S- L	-156 / 0	0.08 (1)
T- K	-156 / 0	0.08 (1)
U- J	-156 / 0	0.08 (1)
V- I	-158 / 0	0.08 (1)
W- G	-144 / 0	0.19 (1)
X- F	-138 / 0	0.11 (1)
Y- E	-140 / 0	0.07 (1)
Z- D	-136 / 0	0.04 (1)
AA- C	-144 / 0	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09 (A-B:1) , BC=0.02 (AA-AB:4) , WB=0.19 (G-W:1) , SSI=0.07 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (G) (INPUT = 0.90)
JSI METAL= 0.05 (X) (INPUT = 1.00)

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H07	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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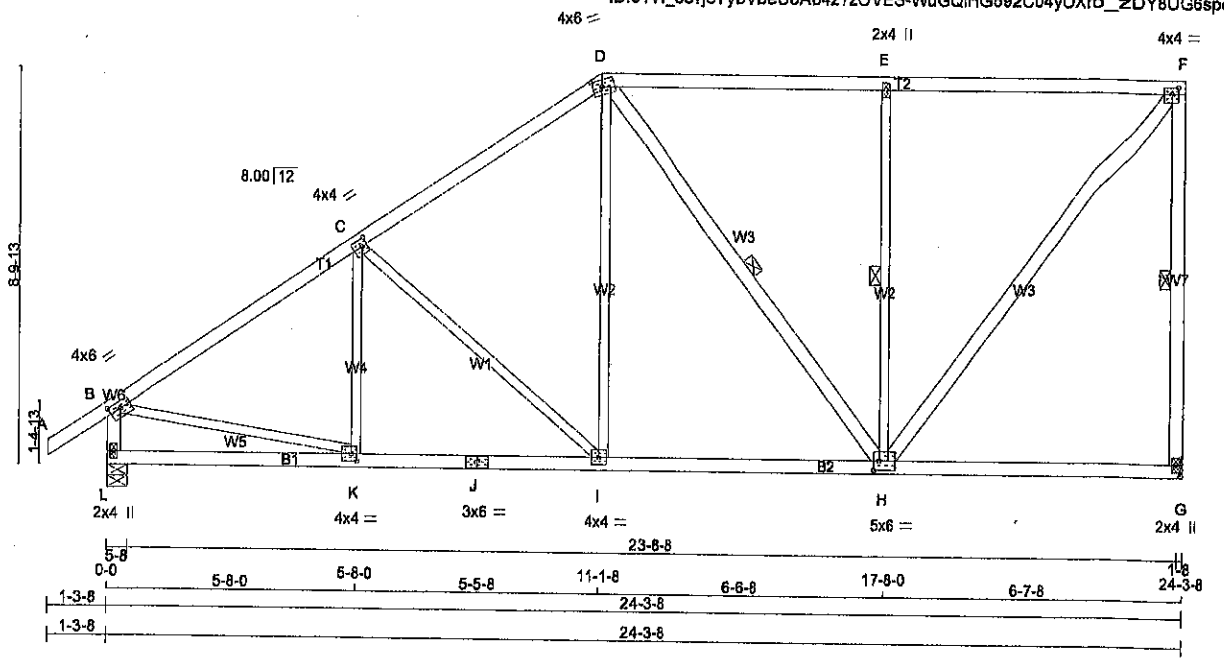

TOTAL WEIGHT = 3 X 121 = 364 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0	1.75	1.75
G	BMV1+p	MT20	2.0	4.0		
H	BMWWW-t	MT20	5.0	8.0	2.50	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1140	0	1140	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
L	1182	0	1182	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	810	510 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0
L	840	567 / 0	0 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-G, D-H, E-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-1236 / 0	-70.5	-70.5	0.31 (1)	5.43
C- D	-982 / 0	-70.5	-70.5	0.28 (1)	5.93
D- E	-698 / 0	-78.0	-78.0	0.44 (1)	6.25
E- F	-698 / 0	-78.0	-78.0	0.44 (1)	6.25
G- F	-1094 / 0	0.0	0.0	0.39 (1)	6.05
L- B	-1150 / 0	0.0	0.0	0.12 (1)	7.42
L- K	0 / 0	-17.5	-17.5	0.13 (4)	10.00
K- J	0 / 1050	-17.5	-17.5	0.23 (1)	10.00
J- I	0 / 1050	-17.5	-17.5	0.23 (1)	10.00
I- H	0 / 798	-17.5	-17.5	0.26 (4)	10.00
H- G	0 / 0	-17.5	-17.5	0.19 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-93 / 55	0.04 (1)
C- I	-340 / 0	0.37 (1)
I- D	0 / 324	0.07 (1)
B- K	0 / 1068	0.24 (1)
D- H	-184 / 0	0.10 (1)
H- E	-637 / 0	0.32 (1)
H- F	0 / 1137	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.44 (E-F:1) , BC=0.26 (H-I:4) , WB=0.37 (C-I:1) , SSI=0.25 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.39 (K) (INPUT = 1.00)

A-150736/3

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H06	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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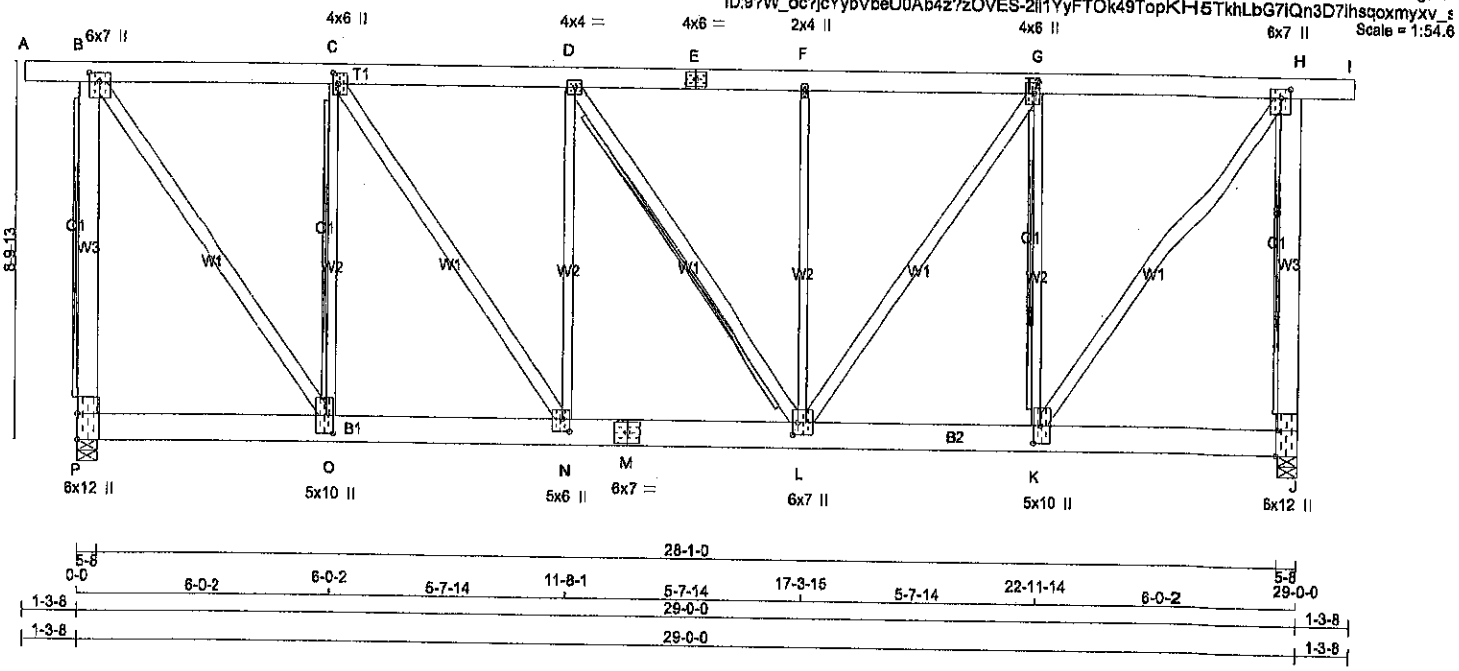

TOTAL WEIGHT = 2 X 232 = 464 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x6	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
E - I	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x8	DRY	No.2	SPF
M - J	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
O - C	2x3	DRY	No.2	SPF
N - D	2x3	DRY	No.2	SPF
L - F	2x3	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
P- B	2	12	TOP
A- E	2	12	TOP
E- I	2	12	TOP
H- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- M	2	9	SIDE(245.1)
M- J	2	9	SIDE(245.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	8473	0	8473	0	0	5-8	5-8
J	8473	0	8473	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	MAX./MIN. COMPONENT REACTIONS PERM.LIVE	WIND	DEAD	SOIL
P	5968	4054 / 0	0 / 0	0 / 0	0 / 0	1914 / 0	0 / 0
J	5968	4054 / 0	0 / 0	0 / 0	0 / 0	1914 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x6 DRY SPF No.2 T-BRACE REQUIRED AT B-P, H-J, C-O, G-K
2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-L
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
P- B	-7232 / 0	0.0	0.0	0.69 (1)		7.81
A- B	0 / 0	-70.5	-70.5	0.02 (1)		10.00
B- C	-5043 / 0	-70.5	-70.5	0.16 (1)		5.08
C- D	-7193 / 0	-70.5	-70.5	0.16 (1)		4.40
D- E	-7184 / 0	-70.5	-70.5	0.16 (1)		4.40
E- F	-7184 / 0	-70.5	-70.5	0.16 (1)		4.40
F- G	-7184 / 0	-70.5	-70.5	0.16 (1)		4.40
G- H	-5045 / 0	-70.5	-70.5	0.16 (1)		5.08
H- I	0 / 0	-70.5	-70.5	0.02 (1)		10.00
J- H	-7234 / 0	0.0	0.0	0.69 (1)		7.81
P- O	0 / 0	-507.6	-507.6	0.42 (1)		10.00
O- N	0 / 5043	-507.6	-507.6	0.69 (1)		10.00
N- M	0 / 7193	-507.6	-507.6	0.64 (1)		10.00
M- L	0 / 7193	-507.6	-507.6	0.64 (1)		10.00
L- K	0 / 5045	-507.6	-507.6	0.69 (1)		10.00
K- J	0 / 0	-507.6	-507.6	0.43 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- O	0 / 8587	0.76 (1)
O- C	-3582 / 0	0.75 (1)
C- N	0 / 3813	0.34 (1)
N- D	-388 / 0	0.27 (1)
D- L	-16 / 0	0.01 (1)
L- F	-389 / 0	0.27 (1)
L- G	0 / 3793	0.34 (1)
K- G	-3572 / 0	0.75 (1)
K- H	0 / 8591	0.76 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 24-3-8
END DISTANCE = 29-0-0
END SPAN CARRIED = 24-3-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.60 (H-J:1) , BC=0.69 (K-L:1) , WB=0.76 (H-K:1) , SSI=0.53 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.86 (K) (INPUT = 1.00)

A-150736/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
263432	H06	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	7.0	2.50	2.75
C	TMWW+t	MT20	4.0	6.0	3.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	4.0	6.0	3.00	1.50
H	TMVW+p	MT20	6.0	7.0	2.50	2.75
J	BMV1+t	MT20	6.0	12.0	Edge	0.50
K	BMWW+t	MT20	5.0	10.0	5.00	2.00
L	BMWWW+t	MT20	6.0	7.0	3.50	1.50
M	BS-t	MT20	6.0	7.0		
N	BMWW+t	MT20	5.0	6.0	3.50	2.00
O	BMWW+t	MT20	5.0	10.0	5.00	2.00
P	BMV1+t	MT20	6.0	12.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

A-1073612-(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
263432	H05	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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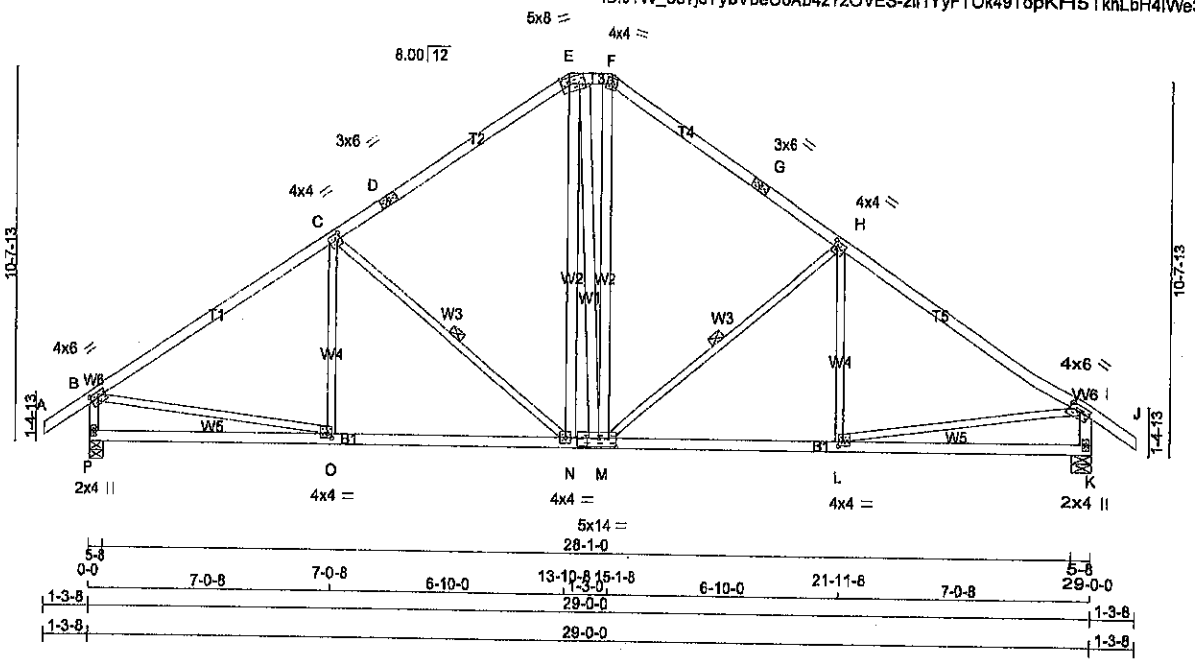

TOTAL WEIGHT = 147 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - E	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.75
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BSWWW-t	MT20	5.0	14.0	3.00	6.50
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1372	0	1372	0	0	5-8	1-8
K	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
K	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-N, H-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-1487 / 0	-70.5	-70.5	0.53 (1)	4.77
C- D	-1105 / 0	-70.5	-70.5	0.48 (1)	5.40
D- E	-1105 / 0	-70.5	-70.5	0.48 (1)	5.40
E- F	-899 / 0	-70.5	-70.5	0.02 (1)	6.25
F- G	-1106 / 0	-70.5	-70.5	0.49 (1)	5.39
G- H	-1106 / 0	-70.5	-70.5	0.49 (1)	5.39
H- I	-1486 / 0	-70.5	-70.5	0.53 (1)	4.77
I- J	0 / 27	-70.5	-70.5	0.09 (1)	10.00
P- B	-1322 / 0	0.0	0.0	0.14 (1)	7.03
K- I	-1322 / 0	0.0	0.0	0.14 (1)	7.03
P- O	0 / 0	-17.5	-17.5	0.22 (4)	10.00
O- N	0 / 1264	-17.5	-17.5	0.32 (1)	10.00
N- M	0 / 896	-17.5	-17.5	0.23 (1)	10.00
M- L	0 / 1263	-17.5	-17.5	0.31 (1)	10.00
L- K	0 / 0	-17.5	-17.5	0.21 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-53 / 98	0.03 (4)
C- N	-484 / 0	0.27 (1)
N- E	0 / 343	0.06 (1)
E- M	0 / 29	0.00 (1)
M- F	0 / 372	0.06 (1)
M- H	-481 / 0	0.27 (1)
L- H	-57 / 96	0.03 (1)
B- O	0 / 1279	0.29 (1)
L- I	0 / 1278	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.53 (H-I:1) , BC=0.32 (N-O:1) , WB=0.29 (B-O:1) , SSI=0.20 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (O) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

A-1073611

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H04	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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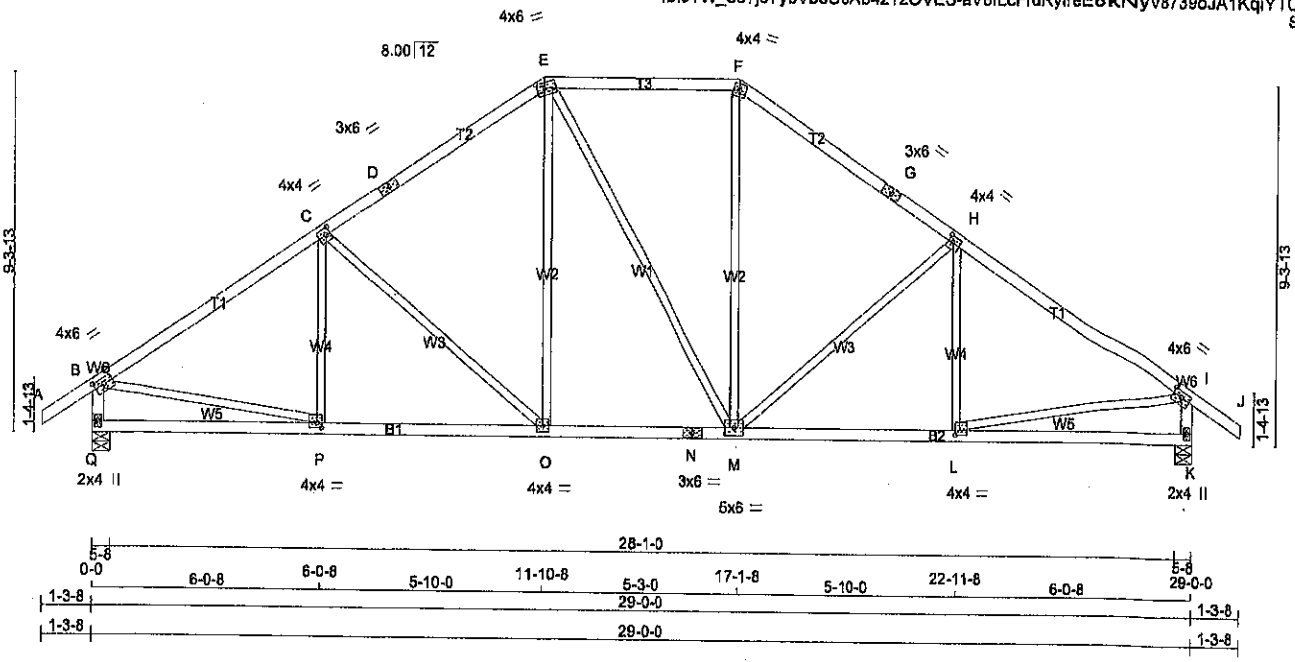

TOTAL WEIGHT = 130 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1372	0	1372	0	0	5-8	1-8
K	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
K	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
B-C	-1492 / 0	-70.5	-70.5	0.36 (1)	4.97	
C-D	-1224 / 0	-70.5	-70.5	0.35 (1)	5.40	
D-E	-1224 / 0	-70.5	-70.5	0.35 (1)	5.40	
E-F	-998 / 0	-70.5	-70.5	0.26 (1)	5.94	
F-G	-1224 / 0	-70.5	-70.5	0.35 (1)	5.40	
G-H	-1224 / 0	-70.5	-70.5	0.35 (1)	5.40	
H-I	-1491 / 0	-70.5	-70.5	0.36 (1)	4.97	
I-J	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
Q-B	-1328 / 0	0.0	0.0	0.14 (1)	7.02	
K-I	-1328 / 0	0.0	0.0	0.14 (1)	7.02	
Q-P	0 / 0	-17.5	-17.5	0.15 (4)	10.00	
P-O	0 / 1264	-17.5	-17.5	0.28 (1)	10.00	
O-N	0 / 997	-17.5	-17.5	0.22 (1)	10.00	
N-M	0 / 997	-17.5	-17.5	0.22 (1)	10.00	
M-L	0 / 1264	-17.5	-17.5	0.28 (1)	10.00	
L-K	0 / 0	-17.5	-17.5	0.15 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-109 / 63	0.05 (1)
C-O	-357 / 0	0.46 (1)
O-E	0 / 325	0.07 (1)
E-M	0 / 1	0.00 (1)
M-F	0 / 326	0.07 (1)
M-H	-358 / 0	0.45 (1)
L-H	-110 / 62	0.05 (1)
B-P	0 / 1264	0.29 (1)
L-I	0 / 1264	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.38 (B-C:1) , BC=0.28 (C-P:1) , WB=0.45 (C-O:1) , SSI=0.17 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	364	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (P) (INPUT = 0.90)
JSI METAL= 0.47 (P) (INPUT = 1.00)

A-15073610

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H03	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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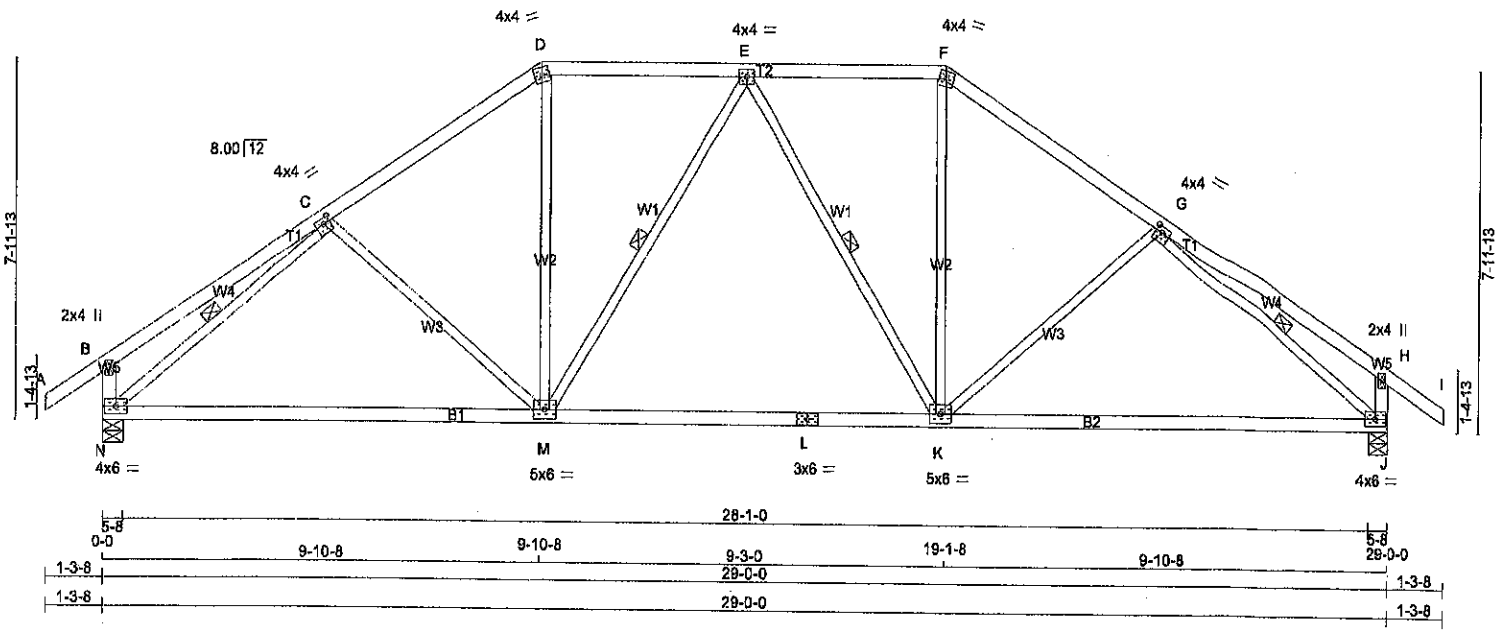

TOTAL WEIGHT = 123 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	1.50	1.75
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1372	0	1372	0	0	5-8	1-8
J	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M, E-K, C-N, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.6	0.09 (1)	10.00
B- C	0 / 22	-70.5	-70.5	0.27 (1)	10.00
C- D	-1345 / 0	-70.5	-70.5	0.24 (1)	5.35
D- E	-1106 / 0	-70.5	-70.5	0.20 (1)	5.80
E- F	-1106 / 0	-70.5	-70.5	0.20 (1)	5.80
F- G	-1345 / 0	-70.5	-70.5	0.24 (1)	5.35
G- H	0 / 22	-70.5	-70.5	0.27 (1)	10.00
H- I	0 / 27	-70.5	-70.5	0.09 (1)	10.00
N- B	-230 / 0	0.0	0.0	0.02 (1)	7.81
J- H	-230 / 0	0.0	0.0	0.02 (1)	7.81
N- M	0 / 1208	-17.5	-17.5	0.46 (4)	10.00
M- L	0 / 1222	-17.5	-17.5	0.47 (4)	10.00
L- K	0 / 1222	-17.5	-17.5	0.47 (4)	10.00
K- J	0 / 1208	-17.5	-17.5	0.46 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-144 / 22	0.12 (1)
M- D	0 / 474	0.11 (1)
M- E	-229 / 0	0.13 (1)
E- K	-229 / 0	0.13 (1)
K- F	0 / 474	0.11 (1)
K- G	-144 / 22	0.12 (1)
N- C	-1614 / 0	0.48 (1)
G- J	-1614 / 0	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.27 (B-C:1) , BC=0.47 (K-M:4) , WB=0.48 (C-N:1) , SSI=0.16 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (N) (INPUT = 0.80)
JSI METAL= 0.58 (G) (INPUT = 1.00)

A-4073609

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H02	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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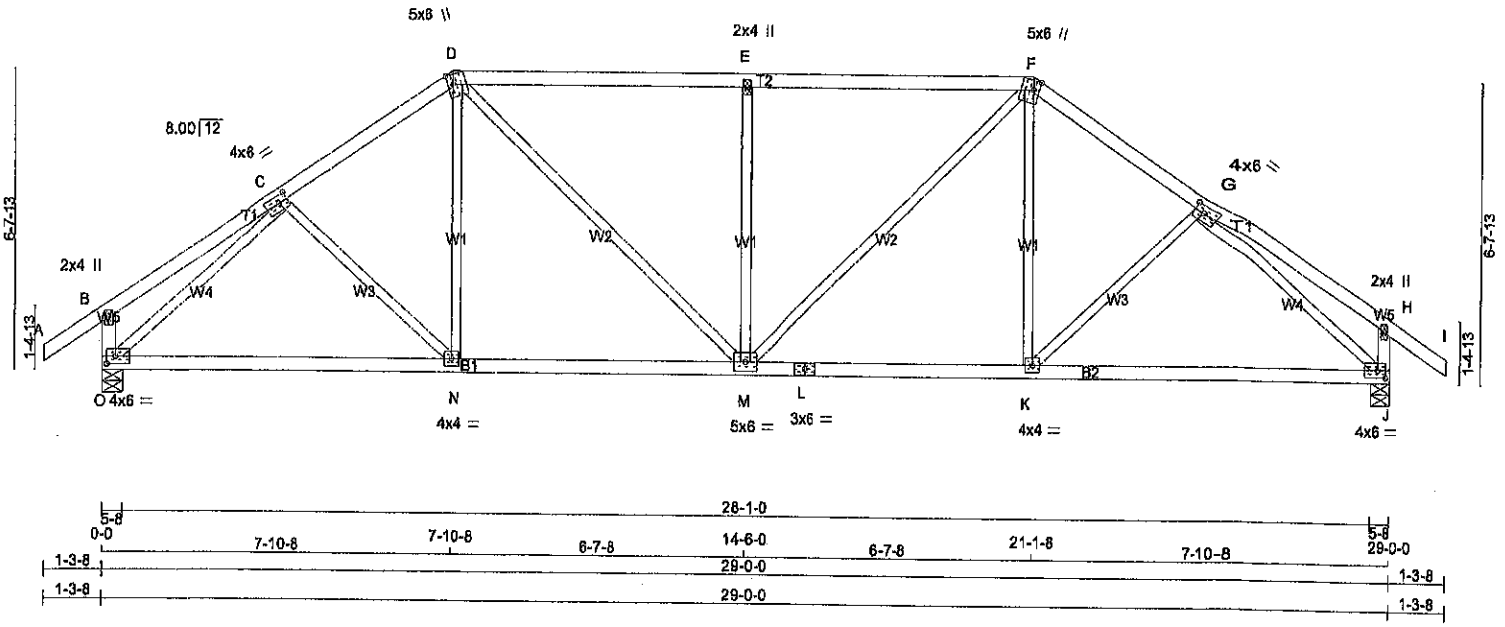

TOTAL WEIGHT = 122 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.50
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	6.0	2.00	2.25
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	6.0	2.00	2.25

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1372	0	1372	0	0	5-8	1-8
J	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	0 / 17	-70.5	-70.5	0.16 (1)	10.00
C- D	-1423 / 0	-70.5	-70.5	0.15 (1)	5.33
D- E	-1518 / 0	-70.5	-70.5	0.44 (1)	4.83
E- F	-1518 / 0	-70.5	-70.5	0.44 (1)	4.83
F- G	-1423 / 0	-70.5	-70.5	0.15 (1)	5.33
G- H	0 / 17	-70.5	-70.5	0.16 (1)	10.00
H- I	0 / 27	-70.5	-70.5	0.09 (1)	10.00
O- B	-205 / 0	0.0	0.0	0.02 (1)	7.81
J- H	-205 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 1179	-17.5	-17.5	0.34 (4)	10.00
N- M	0 / 1171	-17.5	-17.5	0.35 (4)	10.00
M- L	0 / 1171	-17.5	-17.5	0.35 (4)	10.00
L- K	0 / 1171	-17.5	-17.5	0.35 (4)	10.00
K- J	0 / 1179	-17.5	-17.5	0.34 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-15 / 48	0.02 (4)
N- D	0 / 161	0.05 (4)
D- M	0 / 481	0.11 (1)
M- E	-573 / 0	0.42 (1)
M- F	0 / 481	0.11 (1)
K- F	0 / 161	0.05 (4)
K- G	-15 / 48	0.02 (4)
O- C	-1619 / 0	0.77 (1)
G- J	-1619 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.44 (D-E:1) , BC=0.35 (M-N:4) , WB=0.77 (C-O:1) , SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (O) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

A-4073608

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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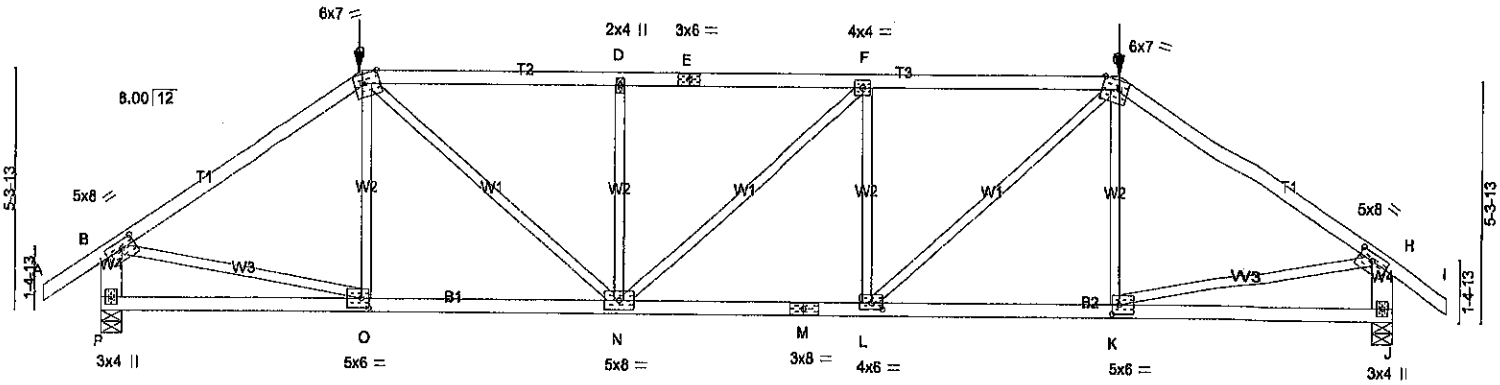

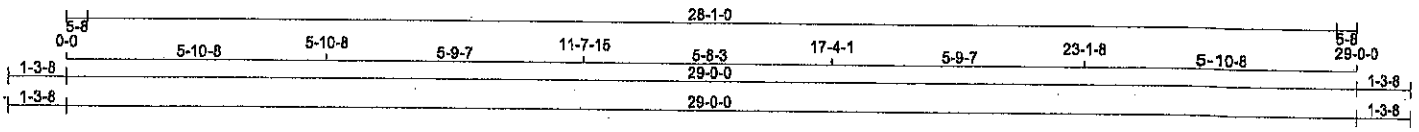

TOTAL WEIGHT = 119 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.75
C	TTWW-m	MT20	6.0	7.0	Edge	4.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	6.0	7.0	Edge	4.75
H	TMVW-t	MT20	5.0	8.0	2.00	3.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	4.0	6.0	1.75	3.00
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2508	0	2508	0	0	5-8	2-11
J	2508	0	2508	0	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1767	1197 / 0	0 / 0	0 / 0	0 / 0	571 / 0	0 / 0
J	1767	1197 / 0	0 / 0	0 / 0	0 / 0	571 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 27	-70.5	-70.5	0.10 (1)	10.00	O-C	-222 / 113	0.10 (1)
B-C	-3003 / 0	-70.5	-70.5	0.87 (1)	3.25	C-N	0 / 1599	0.40 (1)
C-D	-3700 / 0	-138.7	-138.7	0.78 (1)	3.31	N-D	-860 / 0	0.37 (1)
D-E	-3700 / 0	-138.7	-138.7	0.78 (1)	3.30	N-F	-3 / 0	0.00 (1)
E-F	-3700 / 0	-138.7	-138.7	0.78 (1)	3.30	L-F	-860 / 0	0.37 (1)
F-G	-3702 / 0	-138.7	-138.7	0.79 (1)	3.30	L-G	0 / 1602	0.40 (1)
G-H	-3003 / 0	-70.5	-70.5	0.87 (1)	3.25	K-G	-223 / 113	0.10 (1)
H-I	0 / 27	-70.5	-70.5	0.10 (1)	10.00	B-O	0 / 2536	0.63 (1)
P-B	-2425 / 0	0.0	0.0	0.18 (1)	6.58	K-H	0 / 2536	0.63 (1)
J-H	-2425 / 0	0.0	0.0	0.18 (1)	6.58			
P-O	0 / 0	-34.5	-34.5	0.32 (4)	10.00			
O-N	0 / 2492	-34.5	-34.5	0.62 (1)	10.00			
N-M	0 / 3702	-34.5	-34.5	0.76 (1)	10.00			
M-L	0 / 3702	-34.5	-34.5	0.76 (1)	10.00			
L-K	0 / 2491	-34.5	-34.5	0.61 (1)	10.00			
K-J	0 / 0	-34.5	-34.5	0.31 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-302	-302	—	FRONT	VERT	TOTAL
G	23-1-8	-302	-302	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.87 (B-C:1) , BC=0.78 (L-N:1) , WB=0.63 (B-O:1) , SSI=0.41 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.99 (M) (INPUT = 1.00)

A-150736O7

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253432	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 23-1-8, AND 301.8 lbs FACTORED DOWN AT 6-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

A-1507360(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253431	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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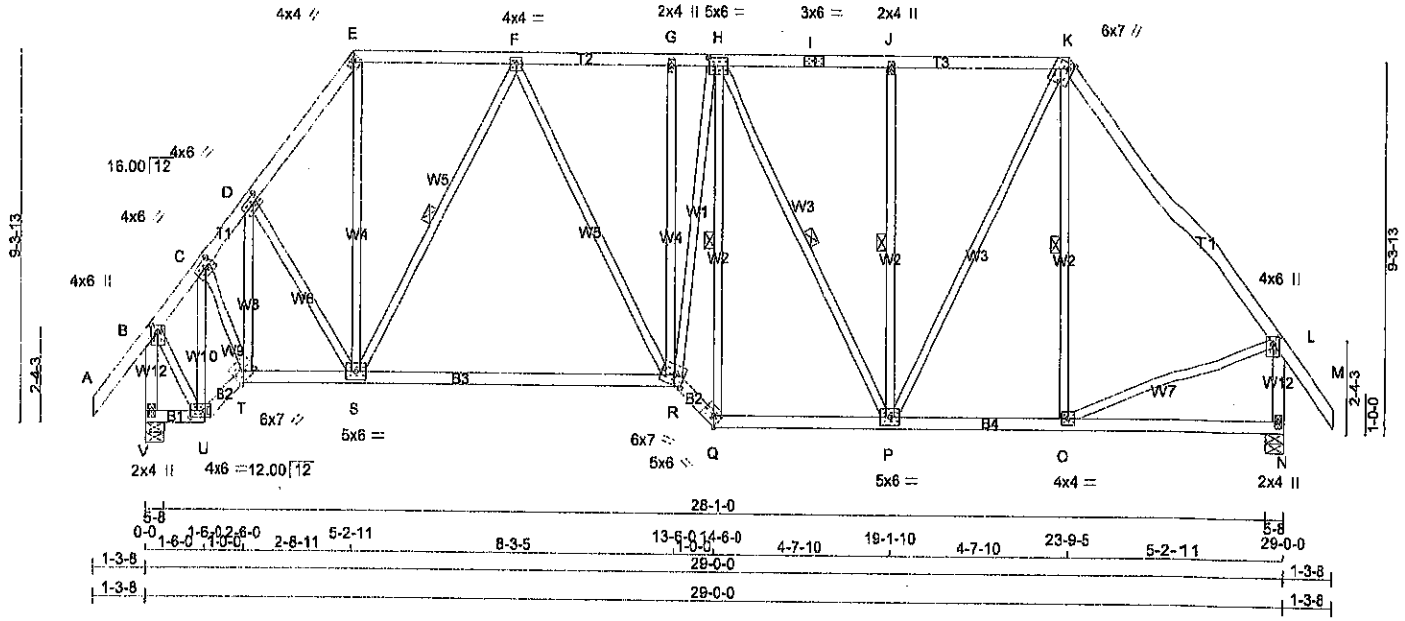

TOTAL WEIGHT = 175 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTW-h	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWWW-t	MT20	5.0	6.0	2.50	2.00
I	TS-t	MT20	3.0	6.0		
J	TMW+w	MT20	2.0	4.0		
K	TTWW+m	MT20	6.0	7.0	Edge	1.50
L	TMVW+p	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BBW-h	MT20	5.0	6.0	Edge	2.50
R	BBWWW-m	MT20	6.0	7.0	3.00	3.00
S	BMWWW-t	MT20	5.0	6.0		
T	BBWW-h	MT20	6.0	7.0	Edge	
U	BBWW-t	MT20	4.0	6.0	2.00	4.50
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1377	0	1377	0	0	5-8	1-8
N	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
N	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-S, H-Q, H-P, J-P, K-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-784 / 0	-70.5	-70.5	0.10 (1)	6.25
C- D	-1089 / 0	-70.5	-70.5	0.10 (1)	6.01
D- E	-1170 / 0	-70.5	-70.5	0.09 (1)	5.81
E- F	-700 / 0	-70.5	-70.5	0.19 (1)	6.25
F- G	-1148 / 0	-70.5	-70.5	0.21 (1)	5.89
G- H	-1147 / 0	-70.5	-70.5	0.16 (1)	5.74
H- I	-936 / 0	-70.5	-70.5	0.22 (1)	6.12
I- J	-936 / 0	-70.5	-70.5	0.22 (1)	6.12
J- K	-936 / 0	-70.5	-70.5	0.22 (1)	6.12
K- L	-1029 / 0	-70.5	-70.5	0.43 (1)	5.69
L- M	0 / 40	-70.5	-70.5	0.10 (1)	10.00
V- B	-1364 / 0	0.0	0.0	0.17 (1)	6.95
N- L	-1335 / 0	0.0	0.0	0.17 (1)	7.01
V- U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U- T	0 / 583	-17.5	-17.5	0.10 (1)	10.00
T- S	0 / 678	-17.5	-17.5	0.31 (4)	10.00
S- R	0 / 1009	-17.5	-17.5	0.34 (4)	10.00
R- Q	0 / 1448	-17.5	-17.5	0.24 (1)	10.00
Q- P	0 / 1036	-17.5	-17.5	0.22 (1)	10.00
P- O	0 / 613	-17.5	-17.5	0.17 (1)	10.00
O- N	0 / 0	-17.5	-17.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-955 / 0	0.26 (1)
C- T	0 / 707	0.16 (1)
T- D	-243 / 0	0.08 (1)
D- S	0 / 29	0.01 (4)
S- E	0 / 731	0.16 (1)
S- F	-687 / 0	0.39 (1)
F- R	0 / 303	0.07 (1)
R- G	-65 / 6	0.09 (1)
R- H	0 / 899	0.20 (1)
Q- H	-973 / 0	0.55 (1)
H- P	-223 / 0	0.16 (1)
P- J	-354 / 0	0.20 (1)
P- K	0 / 702	0.16 (1)
O- K	-124 / 37	0.07 (1)
B- U	0 / 715	0.16 (1)
O- L	0 / 655	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.43 (K-L:1) , BC=0.34 (R-S:4) , WB=0.55 (H-Q:1) , SSI=0.15 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (E) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 1.00)

A-15073606

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253431	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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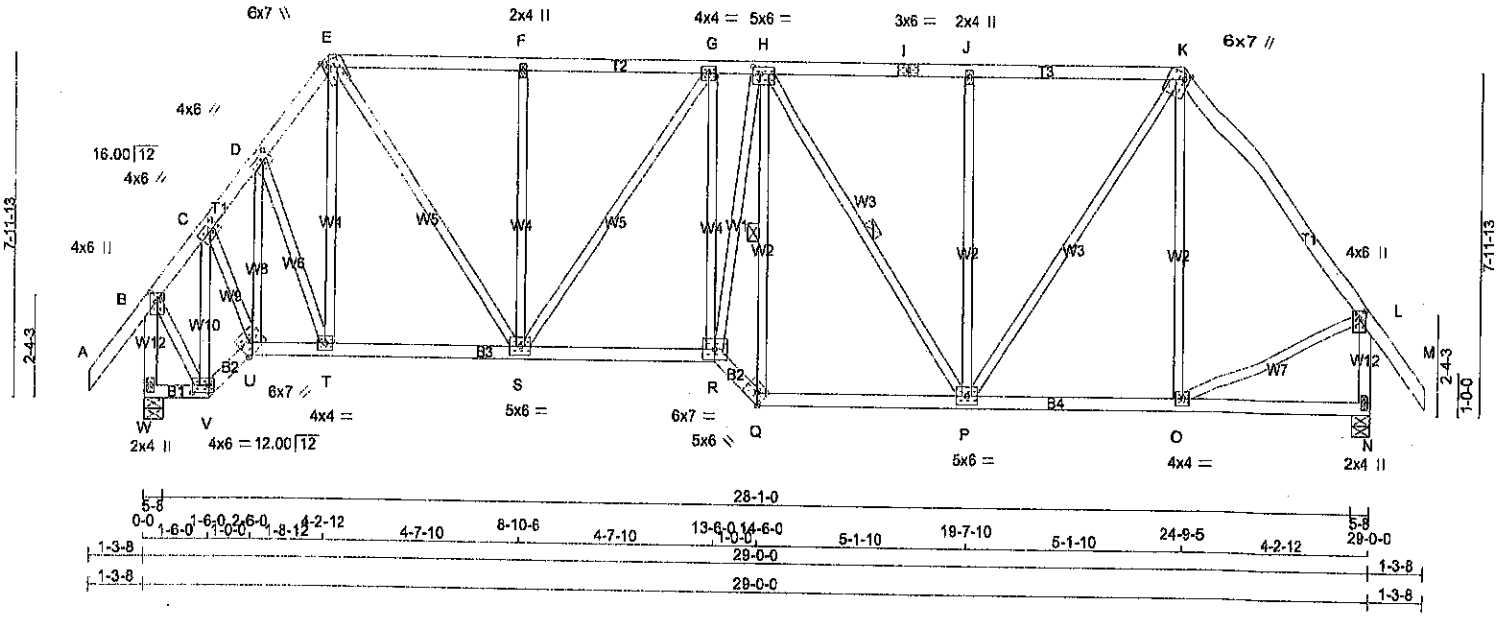

TOTAL WEIGHT = 168 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWWW-t	MT20	5.0	6.0	2.00	2.00
I	TS-t	MT20	3.0	6.0		
J	TMW+w	MT20	2.0	4.0		
K	TTWW+m	MT20	6.0	7.0	Edge	1.50
L	TMWW+p	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BBW-h	MT20	6.0	6.0	Edge	2.50
R	BBWW-t	MT20	6.0	7.0		
S	BMWWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BBWW-h	MT20	6.0	7.0	Edge	
V	BBWW-t	MT20	4.0	6.0	2.00	4.50
W	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	1377	0	1377	0	0	5-8	1-8
N	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
N	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF H-Q, H-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
B-C	-784 / 0	-70.5	-70.5	0.10 (1)	6.25	
C-D	-1097 / 0	-70.5	-70.5	0.04 (1)	6.00	
D-E	-1167 / 0	-70.5	-70.5	0.04 (1)	5.87	
E-F	-1195 / 0	-70.5	-70.5	0.26 (1)	5.54	
F-G	-1195 / 0	-70.5	-70.5	0.26 (1)	5.54	
G-H	-1361 / 0	-70.5	-70.5	0.18 (1)	5.33	
H-I	-1077 / 0	-70.5	-70.5	0.29 (1)	5.71	
I-J	-1077 / 0	-70.5	-70.5	0.29 (1)	5.71	
J-K	-1077 / 0	-70.5	-70.5	0.29 (1)	5.71	
K-L	-1021 / 0	-70.5	-70.5	0.26 (1)	5.90	
L-M	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
W-B	-1364 / 0	0.0	0.0	0.17 (1)	6.85	
N-L	-1343 / 0	0.0	0.0	0.17 (1)	6.99	
W-V	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
V-U	0 / 584	-17.5	-17.5	0.10 (1)	10.00	
U-T	0 / 673	-17.5	-17.5	0.14 (1)	10.00	
T-S	0 / 888	-17.5	-17.5	0.16 (1)	10.00	
S-R	0 / 1379	-17.5	-17.5	0.27 (1)	10.00	
R-Q	0 / 1698	-17.5	-17.5	0.28 (1)	10.00	
Q-P	0 / 1216	-17.5	-17.5	0.25 (1)	10.00	
P-O	0 / 607	-17.5	-17.5	0.15 (4)	10.00	
O-N	0 / 0	-17.5	-17.5	0.09 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-957 / 0	0.25 (1)
C-U	0 / 694	0.16 (1)
U-D	-210 / 0	0.07 (1)
D-T	0 / 40	0.01 (1)
T-E	0 / 49	0.02 (4)
E-S	0 / 886	0.20 (1)
S-F	-375 / 0	0.31 (1)
S-G	-327 / 0	0.45 (1)
R-G	0 / 183	0.04 (1)
R-H	0 / 1104	0.26 (1)
Q-H	-1147 / 0	0.46 (1)
H-P	-254 / 0	0.15 (1)
P-J	-400 / 0	0.49 (1)
P-K	0 / 843	0.19 (1)
O-K	-184 / 10	0.22 (1)
B-V	0 / 716	0.18 (1)
O-L	0 / 669	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.29 (J-K:1) , BC=0.28 (Q-R:1) , WB=0.49 (J-P:1) , SSI=0.17 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (Q) (INPUT = 0.90)
JSI METAL= 0.29 (H) (INPUT = 1.00)

A-15073605

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253431	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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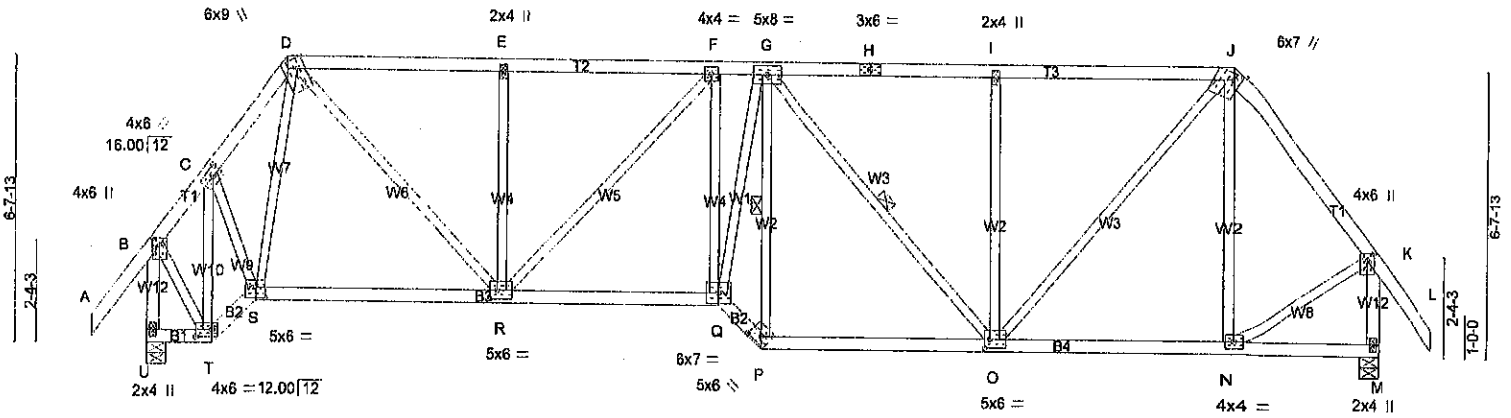

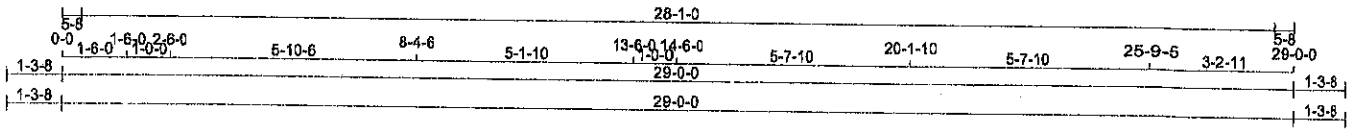

TOTAL WEIGHT = 149 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWWW-t	MT20	5.0	8.0		
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BBW-h	MT20	5.0	6.0	2.00	3.25
Q	BBWW-I	MT20	6.0	7.0		
R	BMWWW-t	MT20	5.0	6.0	2.50	2.00
S	BBWW-I	MT20	5.0	6.0	3.00	3.00
T	BBWW-I	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1377	0	1377	0	0	5-8	1-8
M	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
M	964	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-P, G-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	1	0.10	10.00
B- C	-782 / 0	-70.5	-70.5	1	0.10	6.26
C- D	-1105 / 0	-70.5	-70.5	1	0.06	5.97
D- E	-1455 / 0	-70.5	-70.5	1	0.33	5.05
E- F	-1456 / 0	-70.5	-70.5	1	0.33	5.05
F- G	-1730 / 0	-70.5	-70.5	1	0.21	4.84
G- H	-1274 / 0	-70.5	-70.5	1	0.37	5.26
H- I	-1274 / 0	-70.5	-70.5	1	0.37	5.26
I- J	-1274 / 0	-70.5	-70.5	1	0.37	5.26
J- K	-988 / 0	-70.5	-70.5	1	0.15	6.11
K- L	0 / 40	-70.5	-70.5	1	0.10	10.00
U- B	-1364 / 0	0.0	0.0	1	0.17	6.95
M- K	-1354 / 0	0.0	0.0	1	0.17	6.97
U- T	0 / 0	-17.5	-17.5	4	0.01	10.00
T- S	0 / 588	-17.5	-17.5	1	0.10	10.00
S- R	0 / 875	-17.5	-17.5	4	0.20	10.00
R- Q	0 / 1727	-17.5	-17.5	1	0.36	10.00
Q- P	0 / 2054	-17.5	-17.5	1	0.33	10.00
P- O	0 / 1475	-17.5	-17.5	1	0.30	10.00
O- N	0 / 585	-17.5	-17.5	4	0.18	10.00
N- M	0 / 0	-17.5	-17.5	4	0.08	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-964 / 0	0.25 (1)
C- S	0 / 653	0.15 (1)
S- D	-139 / 0	0.07 (1)
D- R	0 / 1122	0.25 (1)
R- E	-413 / 0	0.20 (1)
R- F	-397 / 0	0.42 (1)
Q- F	0 / 163	0.04 (1)
Q- G	0 / 1370	0.31 (1)
P- G	-1394 / 0	0.40 (1)
G- O	-306 / 0	0.15 (1)
O- I	-442 / 0	0.32 (1)
O- J	0 / 1035	0.23 (1)
N- J	-259 / 0	0.19 (1)
B- T	0 / 721	0.16 (1)
N- K	0 / 664	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.37 (I-J:1) , BC=0.36 (Q-R:1) , WB=0.42 (F-R:1) , SSI=0.19 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (R) (INPUT = 0.90)
JSI METAL= 0.29 (G) (INPUT = 1.00)

A-1073604

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253431	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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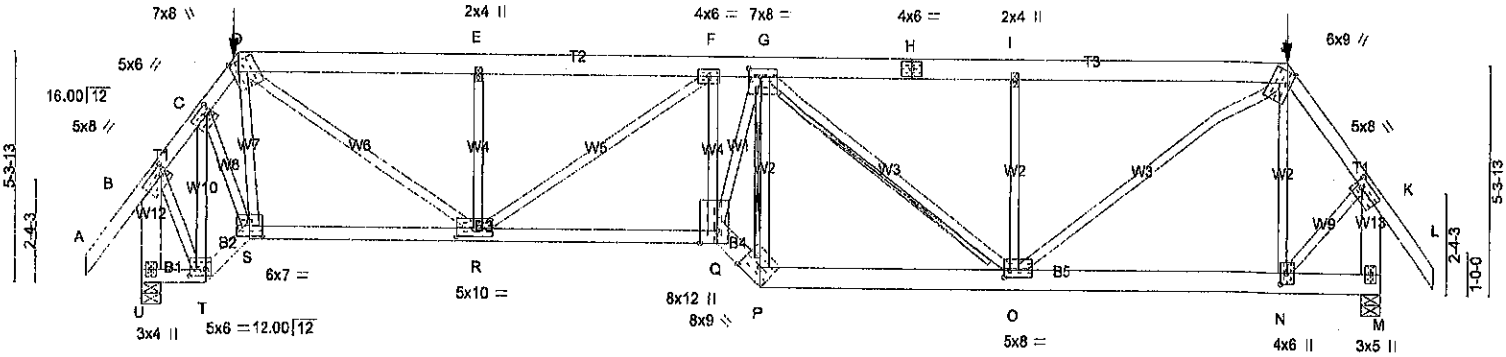

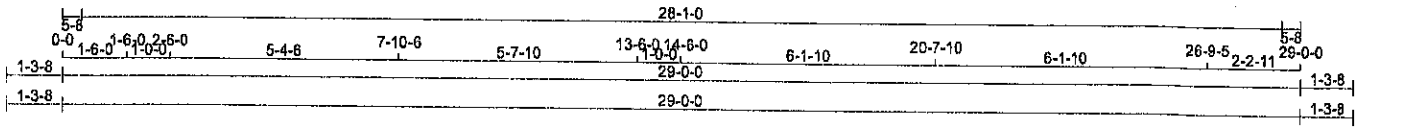

TOTAL WEIGHT = 165 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	2100F 1.8E	SPF
Q - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TMWW-t	MT20	5.0	6.0	2.26	1.75
D	TTWW+m	MT20	7.0	8.0	2.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMWWW-t	MT20	7.0	8.0	2.25	3.25
H	TS-t	MT20	4.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	9.0	3.00	1.25
K	TMVW-t	MT20	5.0	8.0	2.25	2.00
M	BMV1+p	MT20	3.0	5.0		
N	BMWW+t	MT20	4.0	8.0	3.00	1.75
O	BMWWW-t	MT20	5.0	8.0	2.00	1.75
P	BBW-h	MT20	8.0	9.0	3.75	5.00
Q	BBWW+p	MT20	8.0	12.0	Edge	4.75
R	BMWWW-t	MT20	5.0	10.0	2.25	4.75
S	BBWW-l	MT20	6.0	7.0	4.25	3.50
T	BBWW-l	MT20	5.0	6.0	1.75	4.50
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2562	0	2562	0	0	5-8	2-13
M	2572	0	2572	0	0	5-8	2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1804	1228 / 0	0 / 0	0 / 0	0 / 0	575 / 0	0 / 0
M	1810	1234 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT G-P, G-O
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00
B-C	-1531 / 0	-70.5	-70.5	0.14 (1)	5.15
C-D	-2232 / 0	-70.5	-70.5	0.11 (1)	4.46
D-E	-3802 / 0	-70.5	-70.5	0.27 (1)	4.18
E-F	-3802 / 0	-70.5	-70.5	0.31 (1)	4.13
F-G	-4691 / 0	-138.7	-138.7	0.38 (1)	3.67
G-H	-3194 / 0	-138.7	-138.7	0.45 (1)	4.25
H-I	-3194 / 0	-138.7	-138.7	0.45 (1)	4.25
I-J	-3194 / 0	-138.7	-138.7	0.45 (1)	4.25
J-K	-1884 / 0	-70.5	-70.5	0.12 (1)	4.77
K-L	0 / 40	-70.5	-70.5	0.11 (1)	10.00
U-B	-2536 / 0	0.0	0.0	0.24 (1)	6.47
M-K	-2595 / 0	0.0	0.0	0.23 (1)	6.41
U-T	0 / 0	-34.5	-34.5	0.02 (4)	10.00
T-S	0 / 1174	-34.5	-34.5	0.21 (1)	10.00
S-R	0 / 1334	-102.7	-102.7	0.46 (1)	10.00
R-Q	0 / 4669	-102.7	-102.7	0.74 (1)	10.00
Q-P	0 / 5245	-34.5	-34.5	0.68 (1)	10.00
P-O	0 / 3863	-34.5	-34.5	0.61 (1)	10.00
O-N	0 / 1114	-34.5	-34.5	0.25 (1)	10.00
N-M	0 / 0	-34.5	-34.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-1924 / 0	0.63 (1)
D-S	-258 / 0	0.07 (1)
R-E	-491 / 0	0.13 (1)
R-F	-1065 / 0	0.91 (1)
Q-F	0 / 793	0.20 (1)
Q-G	0 / 3280	0.81 (1)
P-G	-3481 / 0	0.76 (1)
G-O	-858 / 0	0.39 (1)
O-I	-944 / 0	0.36 (1)
O-J	0 / 2652	0.66 (1)
N-J	-880 / 0	0.26 (1)
B-T	0 / 1439	0.36 (1)
N-K	0 / 1449	0.36 (1)
C-S	0 / 1420	0.35 (1)
D-R	0 / 3009	0.74 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	2-2-12	-115	-115	---	FRONT	VERT	TOTAL
J	26-9-5	-114	-114	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-12
RIGHT SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 13-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-12
RIGHT SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 15-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 762 (0.46")

A-H073603

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.5 lbs FACTORED DOWN AT 2-2-12, AND 114.2 lbs FACTORED DOWN AT 26-9-5 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

CSI: TC=0.45 (I-J:1), BC=0.74 (Q-R:1), WB=0.91 (F-R:1), SSI=0.33 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.69 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100167040
PROVINCE OF ONTARIO

A-1073603(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H18	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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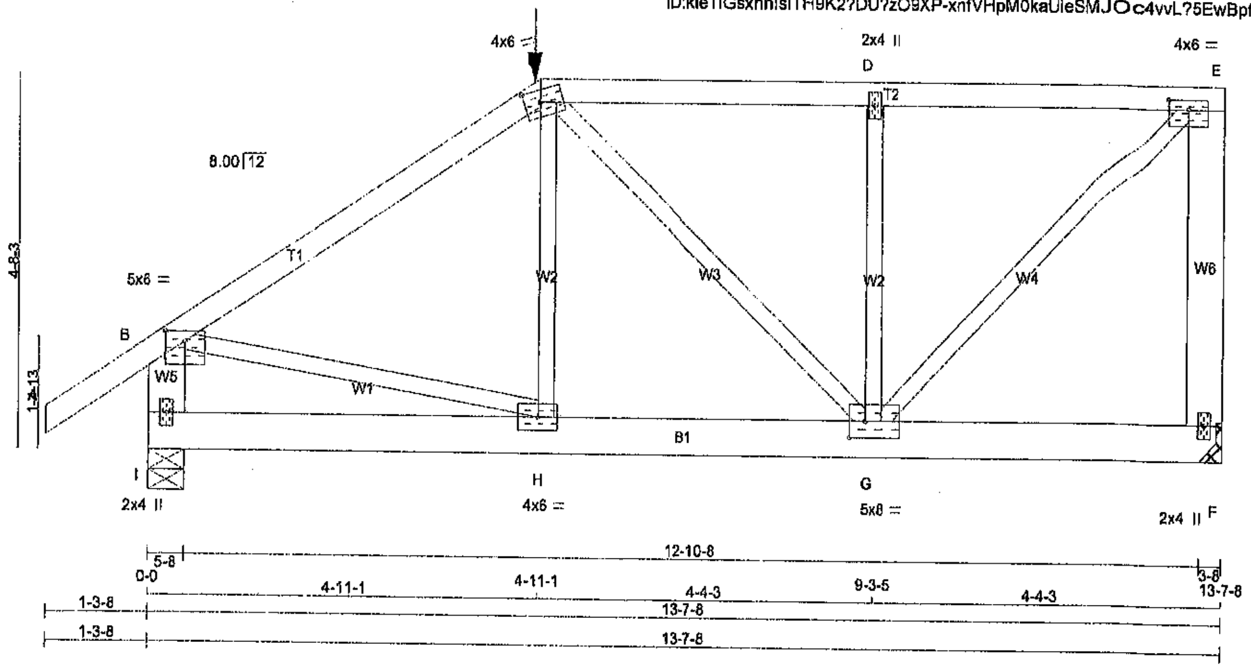

TOTAL WEIGHT = 71 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
I - B	2x6	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0	1.50	3.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	5.0	8.0	2.50	2.50
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 28.9 lbs FACTORED DOWN AT 4-11-1 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1608	0	1608	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-12	
I	1713	0	1713	0	0	5-8	1-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1133	769 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0
I	1204	831 / 0	0 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.10 (1)	10.00
B- C	-1520 / 0	-70.5	-70.5	0.40 (1)	4.85
C- D	-1197 / 0	-70.5	-70.5	0.25 (1)	5.51
D- E	-1197 / 0	-70.5	-70.5	0.25 (1)	5.51
F- E	-1305 / 0	0.0	0.0	0.28 (1)	7.81
I- B	-1367 / 0	0.0	0.0	0.10 (1)	7.81
I- H	0 / 0	-164.1	-164.1	0.27 (1)	10.00
H- G	0 / 1278	-164.1	-164.1	0.41 (1)	10.00
G- F	0 / 0	-164.1	-164.1	0.21 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 600	0.15 (1)
C- G	-113 / 0	0.07 (1)
G- D	-370 / 0	0.12 (1)
G- E	0 / 1685	0.42 (1)
B- H	0 / 1288	0.32 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-11-1	-29	-29	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 8-8-0
END DISTANCE = 13-7-8
END SPAN CARRIED = 8-8-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-11-1
RIGHT SETBACK = 0-0
END SETBACK = 2-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.40 (B-C:1) , BC=0.41 (G-H:1) , WB=0.42 (E-G:1) , SSI=0.32 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	610	[illegible]	1067	822	2204	1666

A-N013602

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H17	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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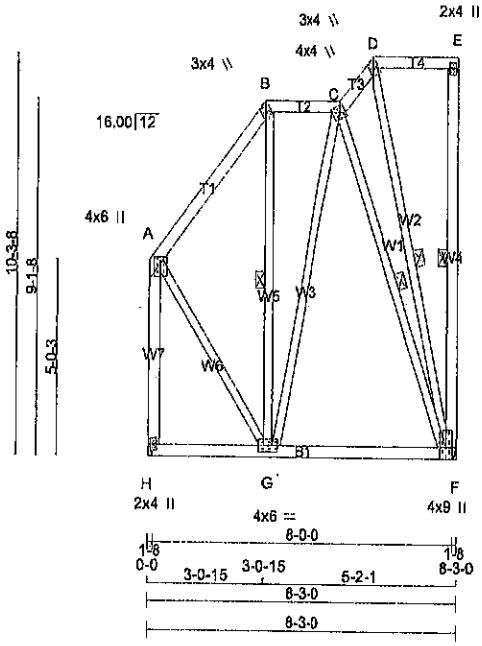

TOTAL WEIGHT = 73 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+m	MT20	3.0	4.0	Edge	
C	TTWW+m	MT20	4.0	4.0	2.00	1.76
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMV+p	MT20	2.0	4.0		
F	BMVWW1+p	MT20	4.0	9.0	Edge	
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	363	0	363	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
H	363	0	363	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	256	173 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
H	256	173 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-F, B-G, C-F, D-F.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-143 / 0	-70.5	-70.5	0.11 (1)	6.25
B- C	-84 / 0	-70.5	-70.5	0.05 (1)	6.25
C- D	-33 / 0	-70.5	-70.5	0.01 (1)	6.25
D- E	0 / 0	-70.5	-70.5	0.06 (1)	10.00
F- E	-81 / 0	0.0	0.0	0.04 (1)	6.25
H- A	-347 / 0	0.0	0.0	0.18 (1)	7.81
H- G	0 / 0	-17.5	-17.5	0.10 (4)	10.00
G- F	0 / 77	-17.5	-17.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-64 / 20	0.03 (1)
G- C	0 / 41	0.01 (4)
C- F	-170 / 0	0.10 (1)
D- F	-86 / 0	0.07 (1)
A- G	0 / 150	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16 (A-H:1) , BC=0.10 (F-G:4) , WB=0.10 (C-F:1) , SSI=0.07 (F-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (H) (INPUT = 1.00)

A-H073601

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H16	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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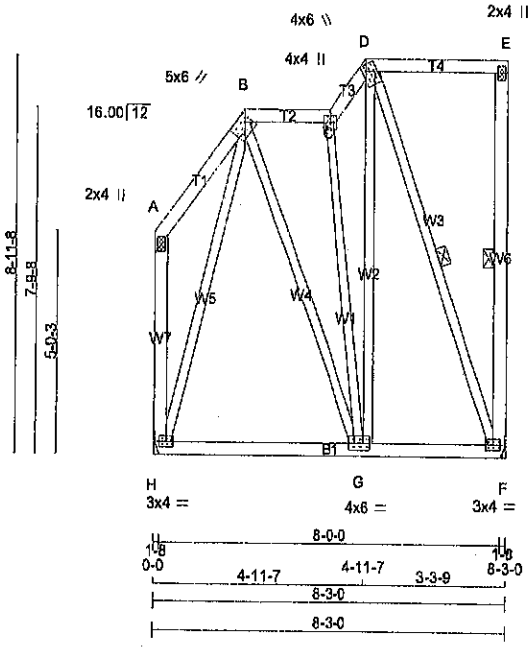

TOTAL WEIGHT = 69 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TTWW-h	MT20	5.0	6.0	2.00	1.50
C	TTW+p	MT20	4.0	4.0		
D	TTWW+m	MT20	4.0	6.0	1.75	1.00
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	383	0	363	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	383	0	363	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	256	173 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
H	256	173 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-F, D-F.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 0	-70.5	-70.5	0.05 (1)	10.00	B-G	0 / 117	0.03 (1)
B-C	-112 / 0	-70.5	-70.5	0.05 (1)	6.25	C-G	-221 / 0	0.26 (1)
C-D	-148 / 0	-70.5	-70.5	0.01 (1)	6.25	G-D	0 / 198	0.04 (1)
D-E	0 / 0	-70.5	-70.5	0.13 (1)	10.00	D-F	-243 / 0	0.14 (1)
F-E	-116 / 0	0.0	0.0	0.04 (1)	6.25	H-B	-263 / 0	0.30 (1)
H-A	-73 / 0	0.0	0.0	0.03 (1)	7.81			
H-G	0 / 71	-17.5	-17.5	0.09 (4)	10.00			
G-F	0 / 87	-17.5	-17.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.13 (D-E:1) , BC=0.10 (F-G:4) , WB=0.30 (B-H:1) , SSI=0.09 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073600

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H15S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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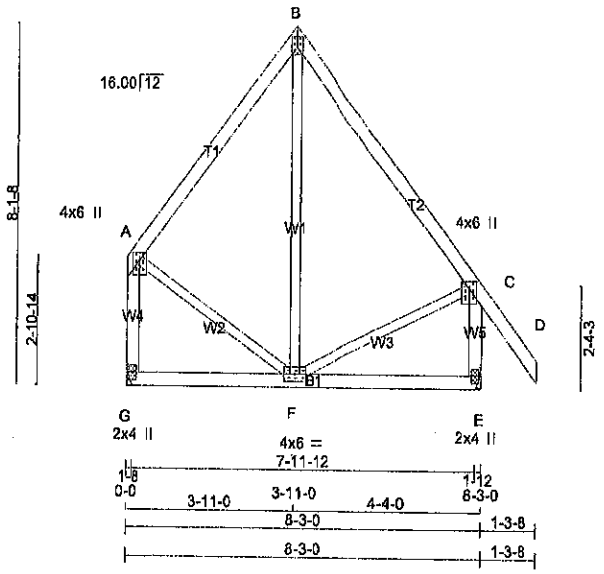

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	363	0	363	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	462	0	462	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	256	173 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
E	323	232 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-168 / 0	-70.5	-70.5	0.14 (1)	6.25	F- B	-21 / 61	0.03 (1)
B- C	-168 / 0	-70.5	-70.5	0.17 (1)	6.25	A- F	0 / 120	0.03 (1)
C- D	0 / 40	-70.5	-70.5	0.10 (1)	10.00	F- C	0 / 111	0.02 (1)
G- A	-337 / 0	0.0	0.0	0.05 (1)	7.81			
E- C	-432 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-17.5	-17.5	0.08 (4)	10.00			
F- E	0 / 0	-17.5	-17.5	0.08 (4)	10.00			

TOTAL WEIGHT = 3 X 47 = 140 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.17 (B-C:1) , BC=0.08 (E-F:4) , WB=0.03 (A-F:1) , SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (C) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

A-1507359

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H15	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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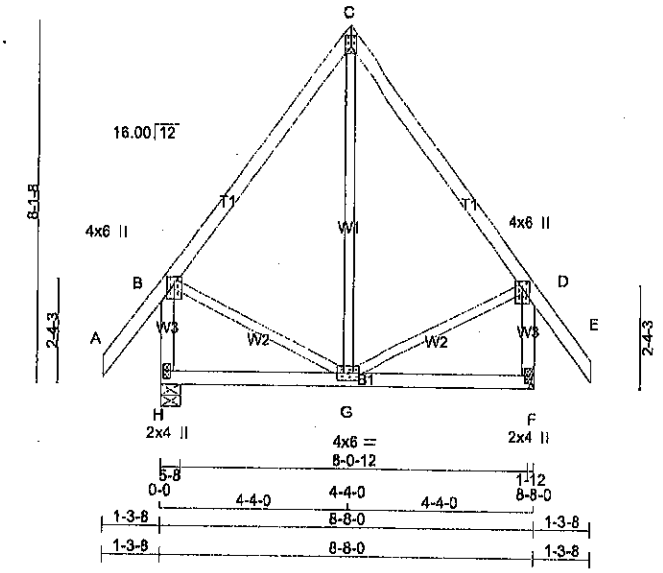

TOTAL WEIGHT = 50 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	480	0	480	0	0	IN-SX	IN-SX
F	480	0	480	0	0	5-8	1-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	336	241 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	336	241 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-186 / 0	-70.5	-70.5	0.17 (1)	6.25
C- D	-186 / 0	-70.5	-70.5	0.17 (1)	6.25
D- E	0 / 40	-70.5	-70.5	0.10 (1)	10.00
H- B	-451 / 0	0.0	0.0	0.08 (1)	7.81
F- D	-451 / 0	0.0	0.0	0.06 (1)	7.81
H- G	0 / 0	-17.5	-17.5	0.09 (4)	10.00
G- F	0 / 0	-17.5	-17.5	0.09 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-8 / 70	0.02 (4)
B- G	0 / 123	0.03 (1)
G- D	0 / 123	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.17 (C-D:1) , BC=0.09 (G-H:4) , WB=0.03 (B-G:1) , SSI=0.07 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-14-2016
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-4073598

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H14	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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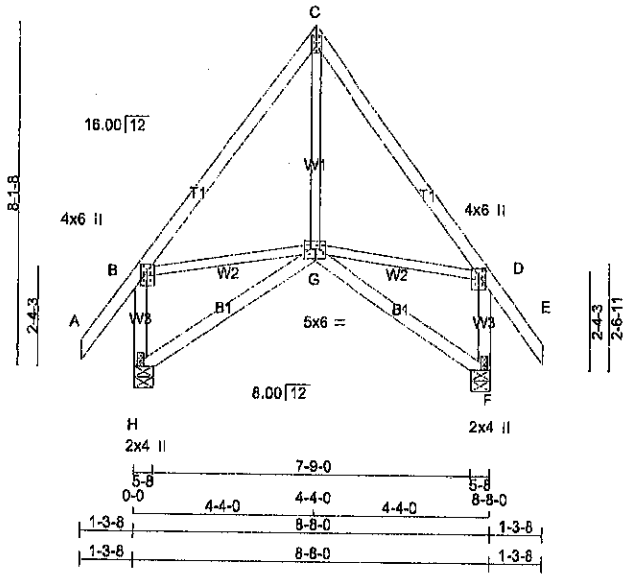

TOTAL WEIGHT = 2 X 50 = 99 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	8.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	8.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	6.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	480	0	480	0	0	5-8	1-8
F	480	0	480	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	336	241 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	336	241 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G-C	0 / 122	0.03 (4)
B-C	-267 / 0	-70.5	-70.5	0.17 (1)	6.25	B-G	0 / 162	0.04 (1)
C-D	-267 / 0	-70.5	-70.5	0.17 (1)	6.25	G-D	0 / 162	0.04 (1)
D-E	0 / 40	-70.5	-70.5	0.10 (1)	10.00			
H-B	-442 / 0	0.0	0.0	0.06 (1)	7.81			
F-D	-442 / 0	0.0	0.0	0.06 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.10 (4)	10.00			
G-F	0 / 0	-17.5	-17.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.17 (C-D:1) , BC=0.10 (G-H:4) , WB=0.04 (D-G:1) , SSI=0.07 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (F) (INPUT = 0.90)
JSI METAL= 0.11 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-4073597

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H13A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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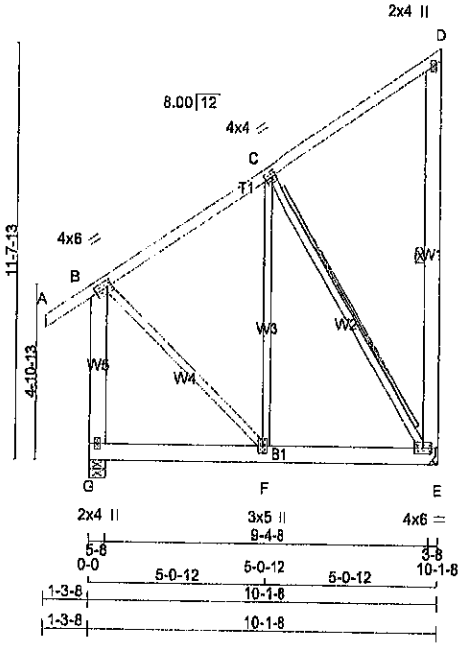

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	1.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMWW+t	MT20	3.0	5.0		
G	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1238	0	1238	0	0	5-8	1-8
E	1141	0	1141	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	870	602 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0
E	804	544 / 0	0 / 0	0 / 0	0 / 0	259 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-E,
2x4 DRY SPF No.2 T-BRACE REQUIRED AT C-E
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-933 / 0		0.0	0.0	0.25 (1)	7.81	B- F	0 / 604	0.15 (1)
A- B	0 / 27		-70.5	-70.5	0.10 (1)	10.00	F- C	0 / 559	0.14 (1)
B- C	-518 / 0		-70.5	-70.5	0.35 (1)	6.25	C- E	-834 / 0	0.56 (1)
C- D	-24 / 0		-70.5	-70.5	0.33 (1)	6.25			
E- D	-135 / 0		0.0	0.0	0.06 (1)	6.25			
G- F	0 / 0		-164.9	-154.9	0.33 (1)	10.00			
F- E	0 / 451		-164.9	-154.9	0.39 (1)	10.00			

TOTAL WEIGHT = 80 lb
(M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 8-3-0
END DISTANCE = 10-1-8
END SPAN CARRIED = 8-3-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.35 (B-C:1) , BC=0.38 (E-F:1) , WB=0.56 (C-E:1) , SSI=0.32 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)

A-1507 3596

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H13	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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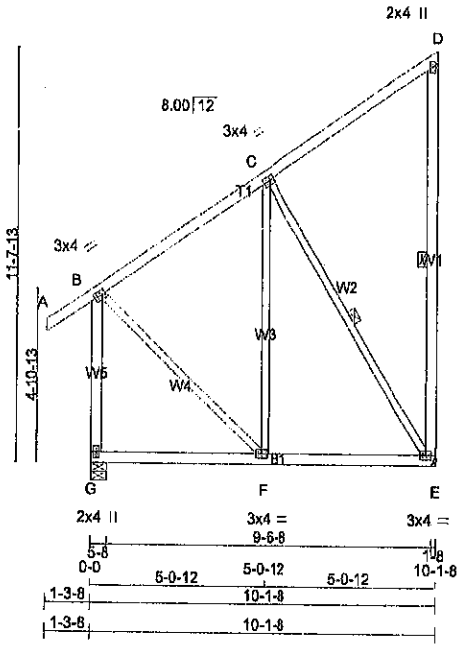

TOTAL WEIGHT = 3 X 64 = 192 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMWW-t	MT20	3.0	4.0		
G	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	542	0	542	0	0	5-8	1-8
E	446	0	445	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	380	270 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
E	314	212 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-E, C-E.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
G- B	-508 / 0	0.0	0.0	0.22 (1)		7.81
A- B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B- C	-187 / 0	-70.5	-70.5	0.23 (1)		6.25
C- D	-24 / 0	-70.5	-70.5	0.23 (1)		6.25
E- D	-134 / 0	0.0	0.0	0.10 (1)		6.25
G- F	0 / 0	-17.5	-17.5	0.13 (4)		10.00
F- E	0 / 176	-17.5	-17.5	0.14 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- F	0 / 236	0.05 (1)
F- C	-50 / 73	0.07 (1)
C- E	-328 / 0	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.23 (B-C:1) , BC=0.14 (E-F:4) , WB=0.20 (C-E:1) , SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

A-15073595

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H12	1	1	TRUSS DESC.	

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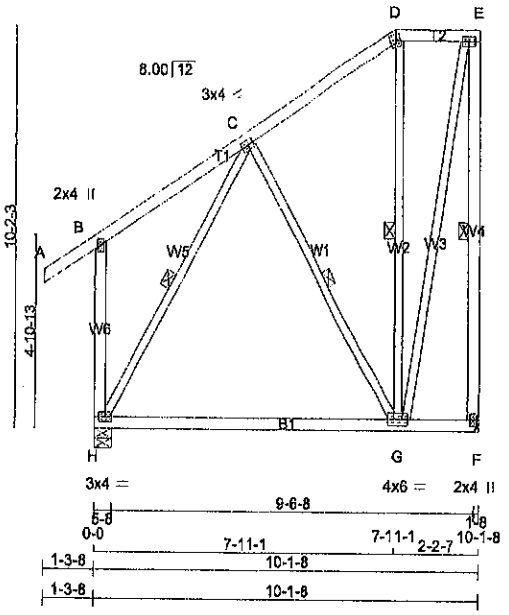


TOTAL WEIGHT = 72 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	4.0		
D	TTW+m	MT20	3.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	446	0	446	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
H	542	0	542	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	314	212 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
H	380	270 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-F, C-G, D-G, C-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	0 / 19	-70.5	-70.5	0.18 (1)	10.00
C- D	-123 / 0	-70.5	-70.5	0.14 (1)	6.25
D- E	-86 / 0	-70.5	-70.5	0.04 (1)	6.25
F- E	-476 / 0	0.0	0.0	0.24 (1)	6.25
H- B	-203 / 0	0.0	0.0	0.09 (1)	7.81
H- G	0 / 155	-17.5	-17.5	0.27 (4)	10.00
G- F	0 / 0	-17.5	-17.5	0.26 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-142 / 0	0.07 (1)
G- D	-124 / 2	0.09 (1)
G- E	0 / 407	0.09 (1)
H- C	-321 / 0	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.24 (E-F:1) , BC=0.27 (G-H:4) , WB=0.14 (C-H:1) , SSI=0.11 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

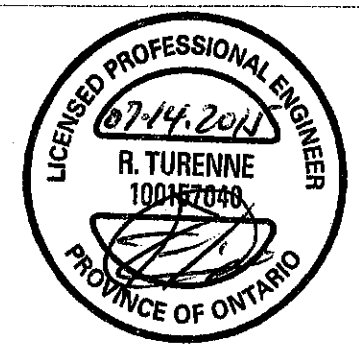
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.11 (G) (INPUT = 1.00)



A-1J073594

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H11	1	1	TRUSS DESC.	

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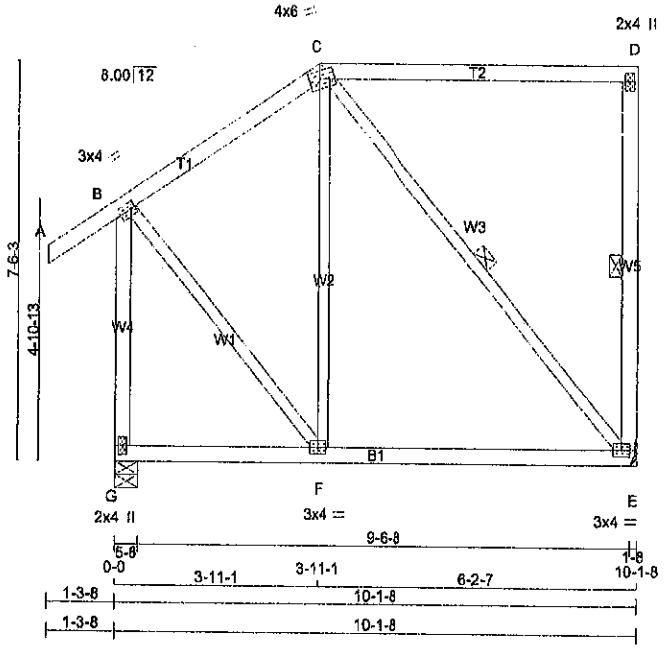

TOTAL WEIGHT = 57 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMWW-t	MT20	3.0	4.0		
G	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	445	0	445	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	642	0	642	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	314	212 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
G	380	270 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-E, C-E.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-191 / 0	-70.5	-70.5	0.19 (1)	6.25
C- D	0 / 0	-70.5	-70.5	0.46 (1)	10.00
E- D	-219 / 0	0.0	0.0	0.06 (1)	6.25
G- B	-521 / 0	0.0	0.0	0.22 (1)	7.81
G- F	0 / 0	-17.5	-17.5	0.14 (4)	10.00
F- E	0 / 158	-17.5	-17.5	0.18 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-67 / 63	0.07 (1)
C- E	-242 / 0	0.15 (1)
B- F	0 / 240	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.46 (C-D:1) , BC=0.16 (E-F:4) , WB=0.15 (C-E:1) , SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

A-150735 93

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H10	1	1	TRUSS DESC.	

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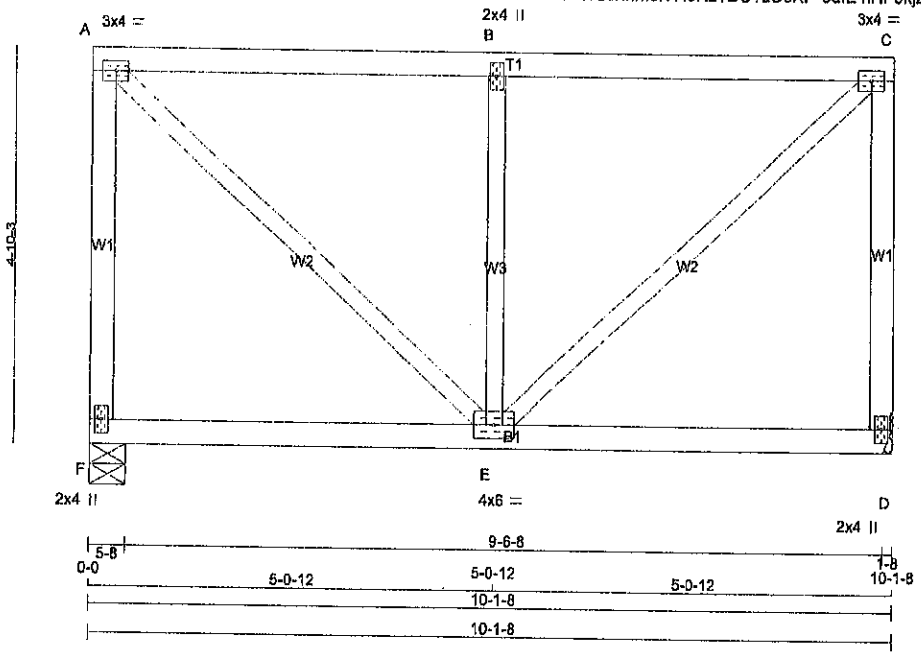

TOTAL WEIGHT = 47 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMW-t	MT20	3.0	4.0	
B	TMW+w	MT20	2.0	4.0	
C	TMW-t	MT20	3.0	4.0	
D	BMV1+p	MT20	2.0	4.0	
E	BMWWW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	2.0	4.0	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	445	0	445	0	0	5-8	1-8
D	445	0	445	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	314	212 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
D	314	212 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-410 / 0	0.0	0.0	0.16 (1)	7.81	A- E	0 / 411	0.09 (1)
A- B	-306 / 0	-70.5	-70.5	0.30 (1)	6.25	E- B	-442 / 0	0.15 (1)
B- C	-306 / 0	-70.5	-70.5	0.30 (1)	6.25	E- C	0 / 411	0.09 (1)
D- C	-410 / 0	0.0	0.0	0.16 (1)	7.81			
F- E	0 / 0	-17.5	-17.5	0.13 (4)	10.00			
E- D	0 / 0	-17.5	-17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.30 (A-B:1) , BC=0.13 (E-F:4) , WB=0.15 (B-E:1) , SSI=0.17 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (C) (INPUT = 0.90)
JSI METAL= 0.11 (A) (INPUT = 1.00)

A-1507392

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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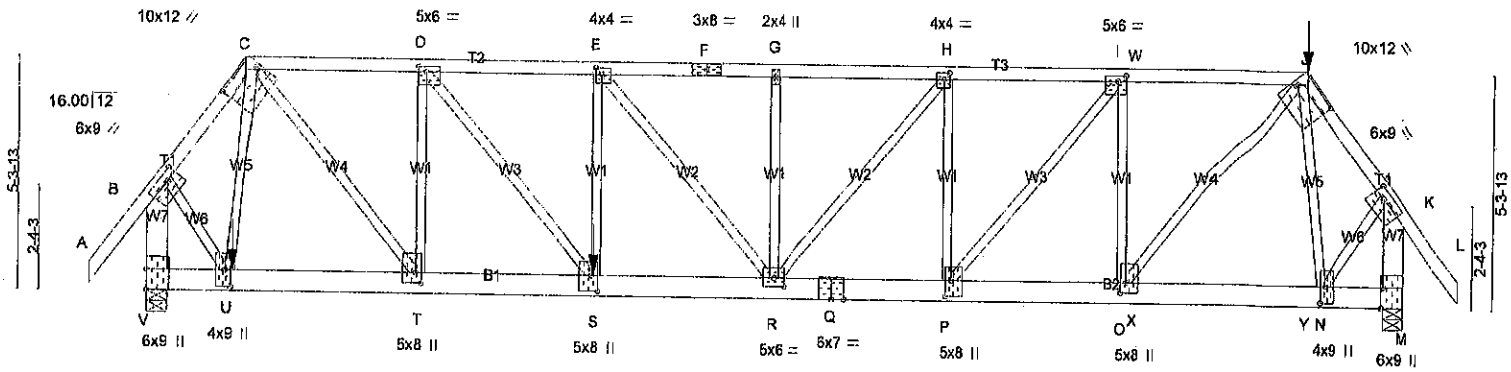

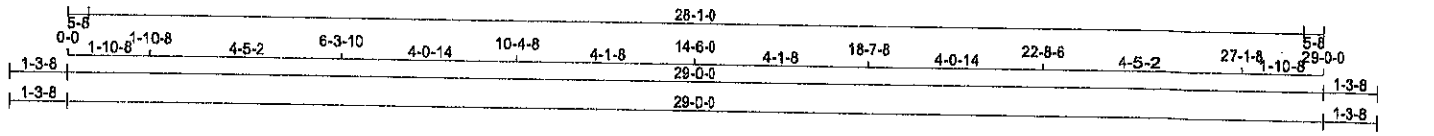

TOTAL WEIGHT = 161 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.00	3.00
C	TTWW-h	MT20	10.0	12.0	3.75	10.50
D	TMWW-t	MT20	6.0	6.0	1.50	2.26
E	TMWW-t	MT20	4.0	4.0	1.75	1.75
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0	1.75	1.75
I	TMWW-t	MT20	6.0	8.0	1.50	2.25
J	TTWW-h	MT20	10.0	12.0	3.75	10.50
K	TMVW-t	MT20	6.0	9.0	2.00	3.00
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWW+t	MT20	4.0	9.0	4.50	1.50
O	BMWW+t	MT20	6.0	8.0	2.75	1.50
P	BMWW+t	MT20	6.0	8.0	4.25	1.75
Q	BS-t	MT20	6.0	7.0		
R	BMWWW-t	MT20	6.0	6.0	2.25	3.00
S	BMWW+t	MT20	6.0	8.0	4.25	1.75
T	BMWW+t	MT20	6.0	8.0	2.75	1.50
U	BMWW+t	MT20	4.0	9.0	4.50	1.50
V	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4626	0	4626	0	0	5-8	5-8
M	3601	0	3601	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3257	2220 / 0	0 / 0	0 / 0	0 / 0	1037 / 0	0 / 0
M	2536	1725 / 0	0 / 0	0 / 0	0 / 0	811 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.27FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.11 (1)	10.00
B- C	-3150 / 0	-70.5	-70.5	0.21 (1)	3.76
C- D	-3996 / 0	-70.5	-70.5	0.44 (1)	3.17
D- E	-5783 / 0	-70.5	-70.5	0.74 (1)	2.36
E- F	-6091 / 0	-70.5	-70.5	0.77 (1)	2.28
F- G	-6091 / 0	-70.5	-70.5	0.77 (1)	2.28
G- H	-6091 / 0	-70.5	-70.5	0.77 (1)	2.27
H- I	-5431 / 0	-70.5	-70.5	0.80 (1)	2.33
I- W	-3891 / 0	-70.5	-70.5	0.60 (1)	3.00
W- J	-3891 / 0	-138.7	-138.7	0.60 (1)	3.00
J- K	-2448 / 0	-70.5	-70.5	0.16 (1)	4.26
K- L	0 / 40	-70.5	-70.5	0.11 (1)	10.00
V- B	-4608 / 0	0.0	0.0	0.41 (1)	4.97
M- K	-3621 / 0	0.0	0.0	0.32 (1)	5.66

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-241 / 0	0.10 (1)
C- T	0 / 3283	0.82 (1)
T- D	-2445 / 0	0.99 (1)
D- S	0 / 2811	0.70 (1)
S- E	-642 / 0	0.26 (1)
E- R	0 / 480	0.12 (1)
R- G	-303 / 0	0.12 (1)
R- H	0 / 1030	0.25 (1)
P- H	-1042 / 0	0.42 (1)
P- I	0 / 2421	0.60 (1)
O- I	-2301 / 0	0.93 (1)
O- J	0 / 3652	0.90 (1)
J- N	-1319 / 0	0.56 (1)
B- U	0 / 2663	0.66 (1)
N- K	0 / 2070	0.51 (1)

V- U	0 / 0	-17.5	-17.5	0.02 (4)	10.00
U- T	0 / 1889	-17.5	-17.5	0.27 (1)	10.00
T- S	0 / 3996	-17.5	-17.5	0.54 (1)	10.00
S- R	0 / 5783	-196.1	-196.1	0.99 (1)	10.00
R- Q	0 / 6431	-196.1	-196.1	0.87 (1)	10.00
Q- P	0 / 6431	-196.1	-196.1	0.87 (1)	10.00
P- O	0 / 3891	-196.1	-196.1	0.69 (1)	10.00
O- X	0 / 1554	-196.1	-196.1	0.28 (1)	10.00
X- Y	0 / 1554	-34.5	-34.5	0.28 (1)	10.00
Y- N	0 / 1554	-34.5	-34.5	0.28 (1)	10.00
N- M	0 / 0	-34.5	-34.5	0.07 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	26-9-5	-114	-114	---	FRONT	VERT	TOTAL
S	10-3-12	-1141	-1141	---	FRONT	VERT	TOTAL
U	1-11-4	-1608	-1608	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-12
RIGHT SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 65 % OF GSL.
LOADS APPLIED TO FIRST 6-0-0 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CStdGirder
START DISTANCE = 10-3-12
START SPAN CARRIED = 10-1-8
END DISTANCE = 23-0-0
END SPAN CARRIED = 10-1-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 772 (0.45")

CSI: TC=0.80 (H-I:1) , BC=0.99 (R-S:1) , WB=0.99 (D-T:1) , SSI=0.30 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

A-1073591

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253430	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:kieTIGsxnnlsITH9K2?DU?zO9XP-eRkspPGdOQbilNJyUeSG7tCok5f4WeklPjRbsayy5bn

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 26-9-5 ON TOP CHORD, AND 1141.1 lbs FACTORED DOWN AT 10-3-12, AND 1608.1 lbs FACTORED DOWN AT 1-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.86 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1507359 (2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H07	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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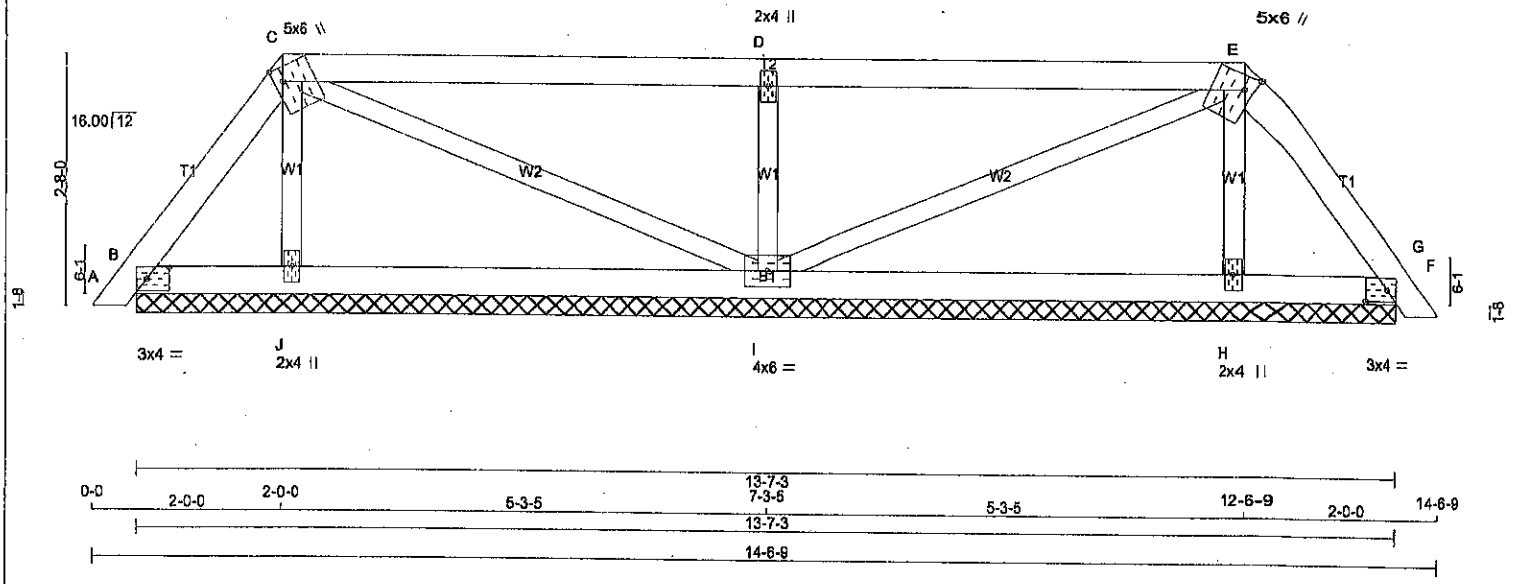

TOTAL WEIGHT = 2 X 49 = 97 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	129	0	129	0	0	13-7-3	1-8
F	129	0	129	0	0	13-7-3	1-8
J	205	0	206	0	0	13-7-3	1-8
I	578	0	578	0	0	13-7-3	1-8
H	205	0	205	0	0	13-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	87	79 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
F	87	79 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
J	148	78 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
I	405	267 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
H	148	78 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	
B- C	-86 / 0	-70.5	-70.5	0.03 (1)	6.25	
C- D	-9 / 0	-70.5	-70.5	0.33 (1)	10.00	
D- E	-9 / 0	-70.5	-70.5	0.33 (1)	10.00	
E- F	-86 / 0	-70.5	-70.5	0.03 (1)	6.25	
F- G	0 / 9	-70.5	-70.5	0.01 (1)	10.00	
B- J	0 / 37	-17.5	-17.5	0.07 (4)	10.00	
J- I	0 / 31	-17.5	-17.5	0.10 (4)	10.00	
I- H	0 / 31	-17.5	-17.5	0.10 (4)	10.00	
H- F	0 / 37	-17.5	-17.5	0.07 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-131 / 0	0.02 (1)
C- I	-23 / 0	0.01 (1)
I- D	-464 / 0	0.07 (1)
I- E	-23 / 0	0.01 (1)
H- E	-131 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33 (D-E;1) , BC=0.10 (H-I;4) , WB=0.07 (D-I;1) , SSI=0.18 (C-D;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (F) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

A-150735 82

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H06	6	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-35dMVDkE6AbBrPvtUOY6mz0l9upWhRmzAFRpBVyy7CT

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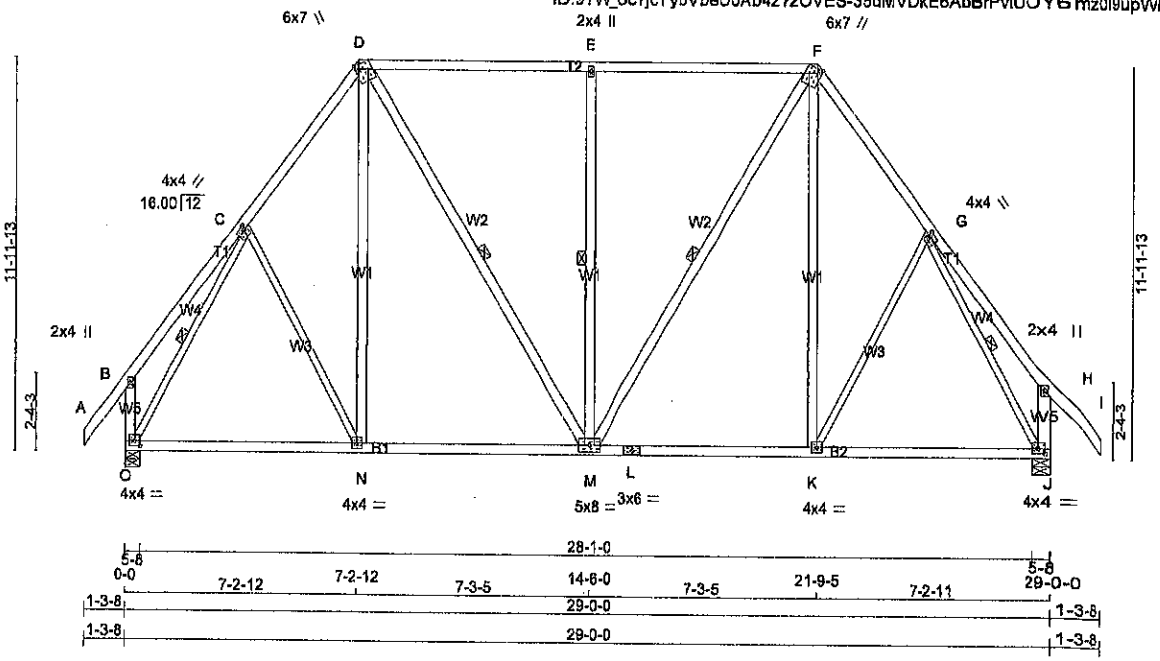

TOTAL WEIGHT = 6 X 181 = 1084 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - N	2x3	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF
O - C	2x3	DRY	No.2	SPF
G - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.00
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMWW-t	MT20	4.0	4.0	1.50	1.00
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	4.0	1.75	2.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1429	0	1429	0	0	5-8	1-9
J	1429	0	1429	0	0	5-8	1-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1010	668 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0
J	1010	668 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-M, E-M, F-M, C-O, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C- N	0 / 51	0.02 (4)
B- C	0 / 23	-70.5	-70.5	0.14 (1)	10.00	N- D	0 / 130	0.03 (4)
C- D	-1088 / 0	-70.5	-70.5	0.13 (1)	5.83	D- M	0 / 477	0.08 (1)
D- E	-889 / 0	-78.0	-78.0	0.55 (1)	5.67	M- E	-695 / 0	0.54 (1)
E- F	-889 / 0	-78.0	-78.0	0.55 (1)	5.67	M- F	0 / 477	0.08 (1)
F- G	-1088 / 0	-70.5	-70.5	0.13 (1)	5.83	K- F	0 / 130	0.03 (4)
G- H	0 / 23	-70.5	-70.5	0.14 (1)	10.00	K- G	0 / 51	0.02 (4)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O- C	-1338 / 0	0.52 (1)
O- B	-195 / 0	0.0	0.0	0.02 (1)	7.81	G- J	-1338 / 0	0.52 (1)
J- H	-195 / 0	0.0	0.0	0.02 (1)	7.81			
C- N	0 / 627	-17.5	-17.5	0.28 (4)	10.00			
N- M	0 / 837	-17.5	-17.5	0.29 (4)	10.00			
M- L	0 / 837	-17.5	-17.5	0.29 (4)	10.00			
L- K	0 / 837	-17.5	-17.5	0.29 (4)	10.00			
K- J	0 / 627	-17.5	-17.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.55 (D-E:1) , BC=0.29 (M-N:4) , WB=0.54 (E-M:1) , SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.49 (G) (INPUT = 1.00)

A-1073581

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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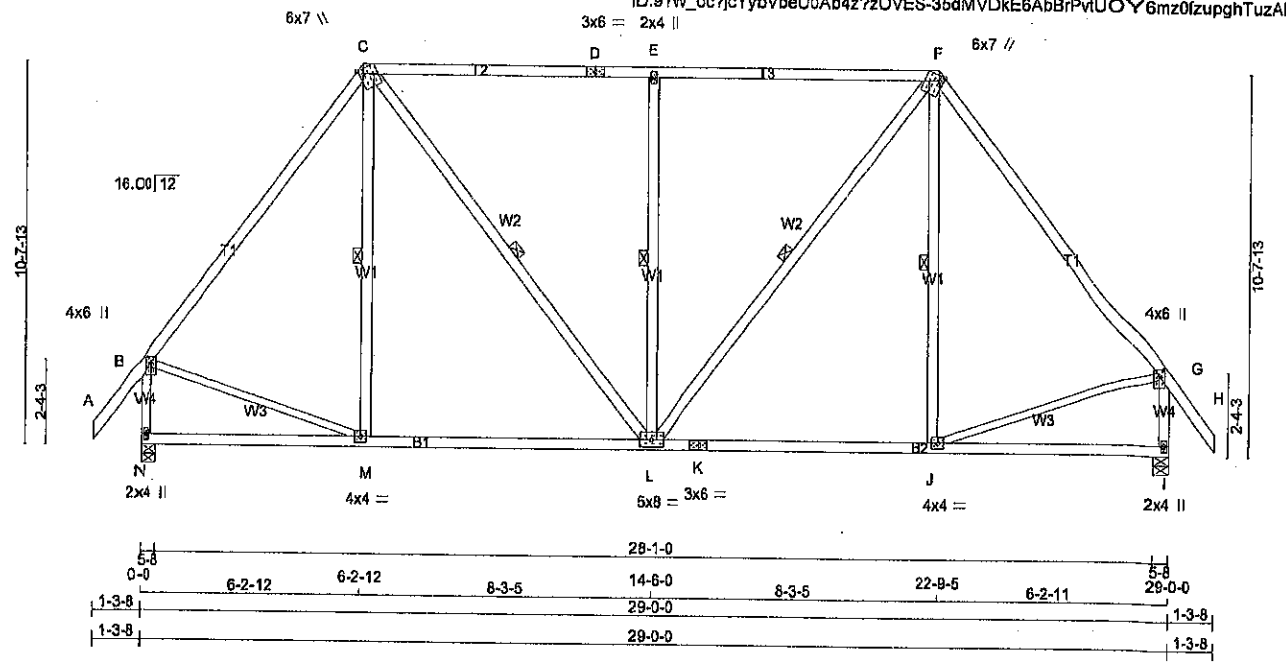

TOTAL WEIGHT = 2 X 160 = 319 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - M	2x3	DRY	No.2	SPF
J - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1375	0	1375	0	0	5-8	1-8
I	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
I	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-M, C-L, E-L, F-L, F-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.07 (1)	10.00	M- C	-48 / 95	0.03 (1)	
B- C	-1029 / 0	-70.5	-70.5	0.38 (1)	6.25	C- L	0 / 550	0.09 (1)	
C- D	-959 / 0	-70.5	-70.5	0.95 (1)	4.39	L- E	-715 / 0	0.41 (1)	
D- E	-959 / 0	-70.5	-70.5	0.95 (1)	4.39	L- F	0 / 550	0.09 (1)	
E- F	-958 / 0	-70.5	-70.5	0.95 (1)	4.39	J- F	-48 / 95	0.03 (1)	
F- G	-1029 / 0	-70.5	-70.5	0.38 (1)	6.25	B- M	0 / 644	0.14 (1)	
G- H	0 / 40	-70.5	-70.5	0.07 (1)	10.00	J- G	0 / 644	0.14 (1)	
N- B	-1333 / 0	0.0	0.0	0.17 (1)	7.01				
I- G	-1333 / 0	0.0	0.0	0.17 (1)	7.01				
N- M	0 / 0	-17.5	-17.5	0.21 (4)	10.00				
M- L	0 / 615	-17.5	-17.5	0.28 (4)	10.00				
L- K	0 / 615	-17.5	-17.5	0.28 (4)	10.00				
K- J	0 / 615	-17.5	-17.5	0.28 (4)	10.00				
J- I	0 / 0	-17.5	-17.5	0.21 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.95 (C-E:1) , BC=0.28 (L-M:4) , WB=0.41 (E-L:1) , SSI=0.28 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (M) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073580

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H04	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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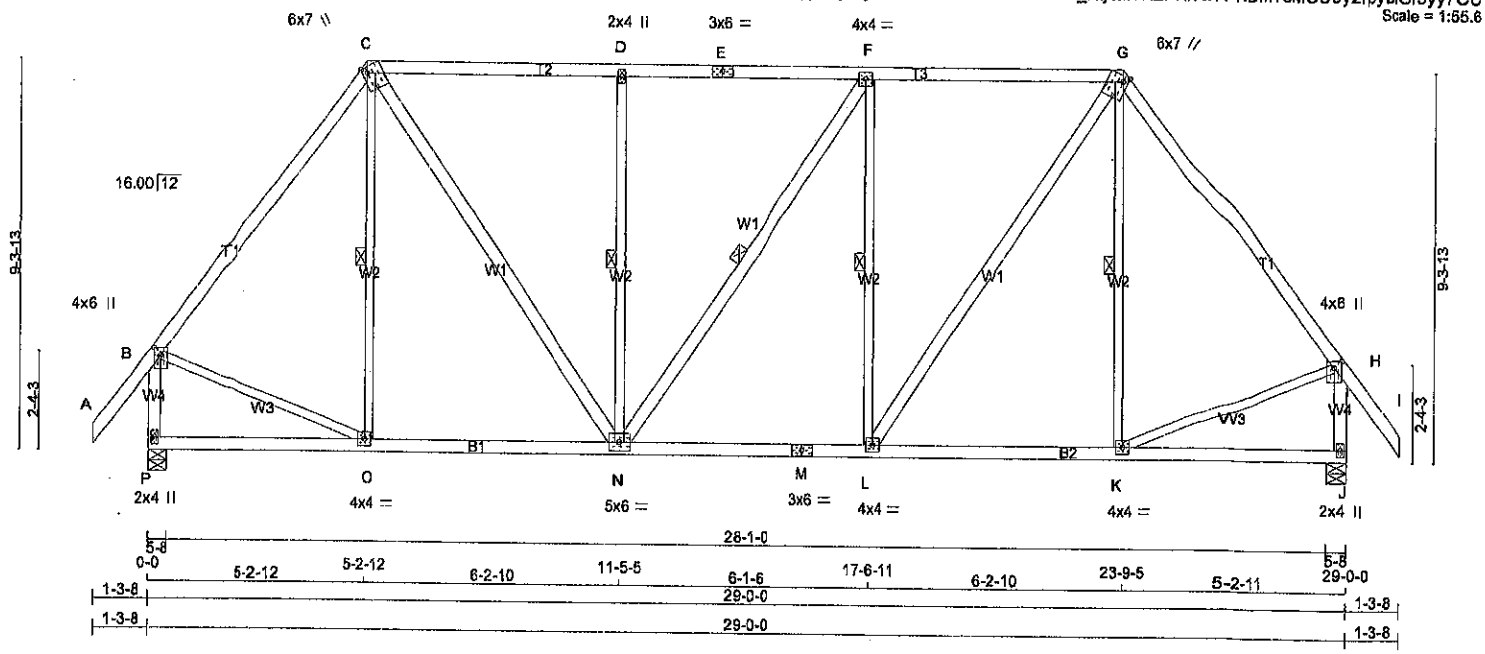

TOTAL WEIGHT = 2 X 158 = 315 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, L, O						
K	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	968	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	968	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-O, D-N, F-N, F-L, G-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-1033 / 0	-70.5	-70.5	0.43 (1)	5.66
C- D	-1013 / 0	-70.5	-70.5	0.40 (1)	5.66
D- E	-1013 / 0	-70.5	-70.5	0.41 (1)	5.66
E- F	-1013 / 0	-70.5	-70.5	0.41 (1)	5.66
F- G	-1014 / 0	-70.5	-70.5	0.41 (1)	5.66
G- H	-1033 / 0	-70.5	-70.5	0.43 (1)	5.66
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00
P- B	-1339 / 0	0.0	0.0	0.17 (1)	7.00
J- H	-1338 / 0	0.0	0.0	0.17 (1)	7.00
P- O	0 / 0	-17.5	-17.5	0.13 (4)	10.00
O- N	0 / 616	-17.5	-17.5	0.18 (4)	10.00
N- M	0 / 1014	-17.5	-17.5	0.23 (1)	10.00
M- L	0 / 1014	-17.5	-17.5	0.23 (1)	10.00
L- K	0 / 616	-17.5	-17.5	0.18 (4)	10.00
K- J	0 / 0	-17.5	-17.5	0.13 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- C	-114 / 51	0.07 (1)
C- N	0 / 698	0.11 (1)
N- D	-468 / 0	0.27 (1)
N- F	-2 / 0	0.00 (1)
L- F	-468 / 0	0.27 (1)
L- G	0 / 700	0.11 (1)
K- G	-115 / 51	0.07 (1)
B- O	0 / 657	0.15 (1)
K- H	0 / 657	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43 (B-C:1) , BC=0.23 (L-N:1) , WB=0.27 (F-L:1) , SSI=0.20 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.28 (M) (INPUT = 1.00)

A-150735 79

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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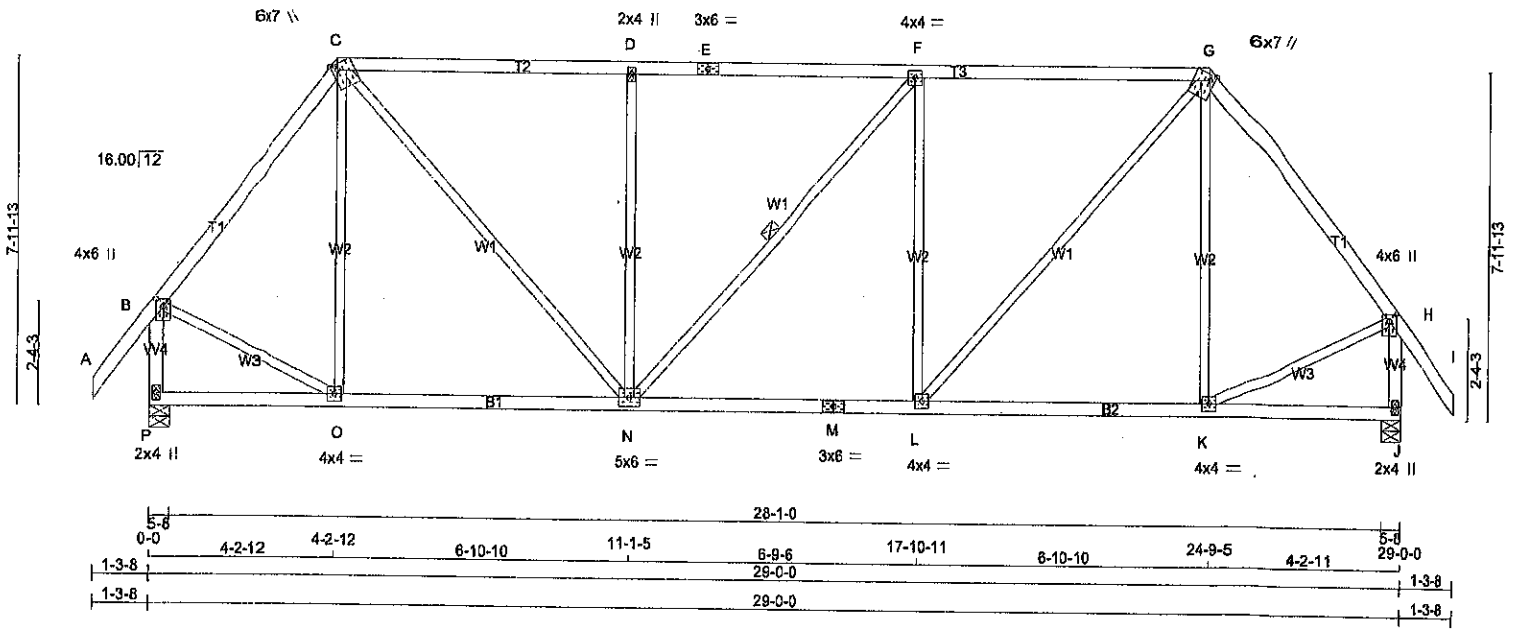


TOTAL WEIGHT = 2 X 138 = 277 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

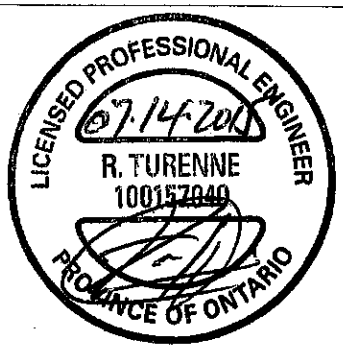
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, L, O						
K	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B-C	-1027 / 0	-70.5	-70.5	0.26 (1)	5.88
C-D	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16
D-E	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16
E-F	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16
F-G	-1186 / 0	-70.5	-70.5	0.53 (1)	5.16
G-H	-1027 / 0	-70.5	-70.5	0.26 (1)	5.88
H-I	0 / 40	-70.5	-70.5	0.10 (1)	10.00
P-B	-1350 / 0	0.0	0.0	0.17 (1)	6.98
J-H	-1350 / 0	0.0	0.0	0.17 (1)	6.98
P-O	0 / 0	-17.5	-17.5	0.13 (4)	10.00
O-N	0 / 611	-17.5	-17.5	0.21 (4)	10.00
N-M	0 / 1187	-17.5	-17.5	0.28 (1)	10.00
M-L	0 / 1187	-17.5	-17.5	0.28 (1)	10.00
L-K	0 / 611	-17.5	-17.5	0.21 (4)	10.00
K-J	0 / 0	-17.5	-17.5	0.13 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-167 / 31	0.20 (1)
C-N	0 / 859	0.19 (1)
N-D	-519 / 0	0.63 (1)
N-F	-1 / 0	0.00 (1)
L-F	-519 / 0	0.63 (1)
L-G	0 / 861	0.19 (1)
K-G	-168 / 31	0.20 (1)
B-O	0 / 873	0.15 (1)
K-H	0 / 873	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.53 (F-G:1) , BC=0.28 (L-N:1) , WB=0.63 (F-L:1) , SSI=0.23 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.34 (M) (INPUT = 1.00)

A-10073578

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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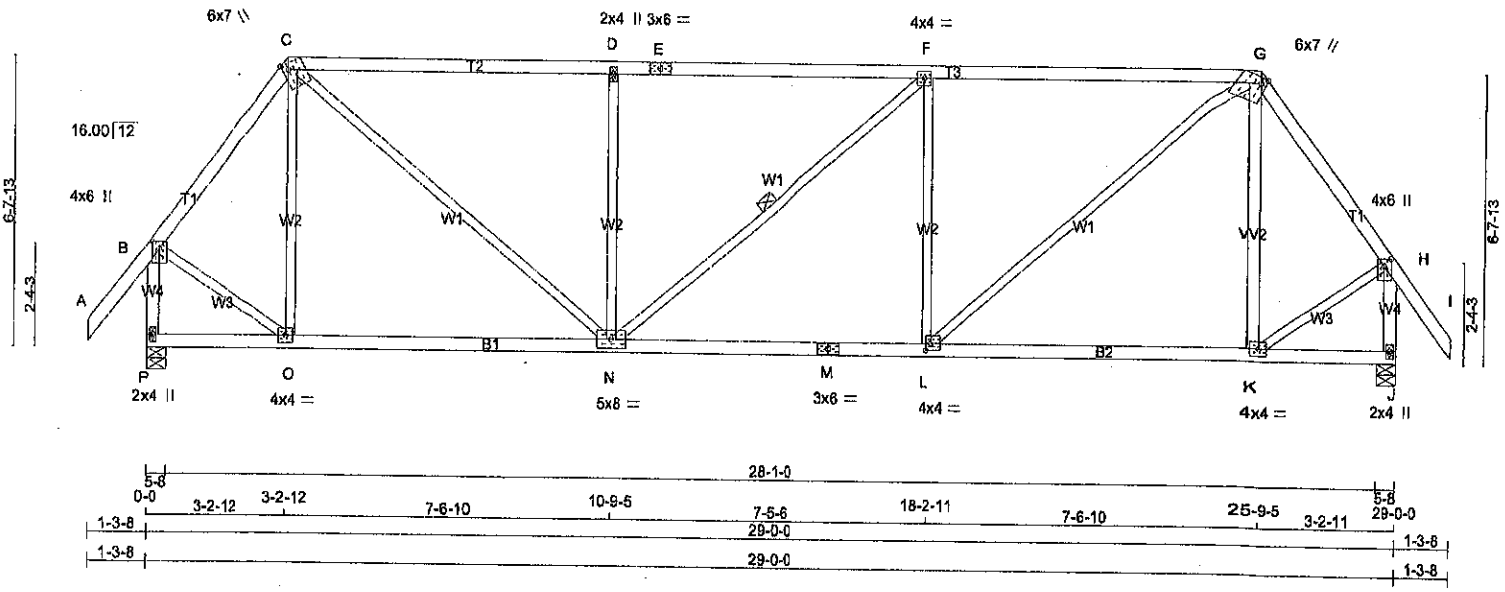

TOTAL WEIGHT = 2 X 130 = 260 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
B- C	-999 / 0	-70.5	-70.5	0.15 (1)	6.09	
C- D	-1430 / 0	-70.5	-70.5	0.68 (1)	4.60	
D- E	-1431 / 0	-70.5	-70.5	0.68 (1)	4.59	
E- F	-1431 / 0	-70.5	-70.5	0.68 (1)	4.59	
F- G	-1431 / 0	-70.5	-70.5	0.68 (1)	4.58	
G- H	-998 / 0	-70.5	-70.5	0.15 (1)	6.09	
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
P- B	-1365 / 0	0.0	0.0	0.17 (1)	6.95	
J- H	-1365 / 0	0.0	0.0	0.17 (1)	6.95	
P- O	0 / 0	-17.5	-17.5	0.15 (4)	10.00	
O- N	0 / 591	-17.5	-17.5	0.24 (4)	10.00	
N- M	0 / 1432	-17.5	-17.5	0.34 (1)	10.00	
M- L	0 / 1432	-17.5	-17.5	0.34 (1)	10.00	
L- K	0 / 591	-17.5	-17.5	0.24 (4)	10.00	
K- J	0 / 0	-17.5	-17.5	0.15 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- C	-237 / 8	0.17 (1)
C- N	0 / 1094	0.25 (1)
N- D	-570 / 0	0.42 (1)
N- F	-1 / 0	0.00 (1)
L- F	-570 / 0	0.42 (1)
L- G	0 / 1096	0.25 (1)
K- G	-238 / 8	0.17 (1)
B- O	0 / 690	0.16 (1)
K- H	0 / 690	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68 (F-G:1) , BC=0.34 (L-N:1) , WB=0.42 (F-L:1) , SSI=0.26 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)

A-1073577

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H01-Cond1	2	1	TRUSS DESC.	

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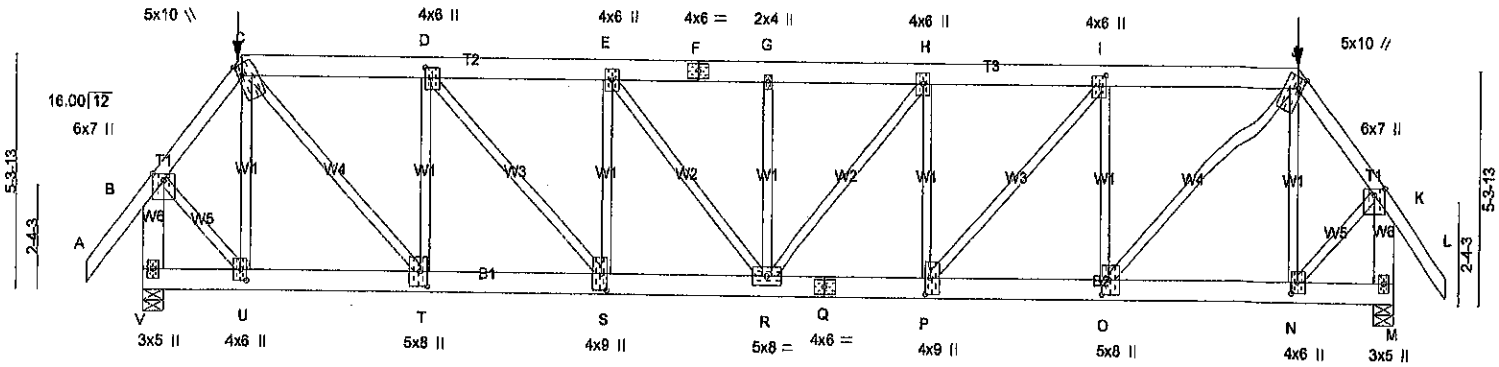

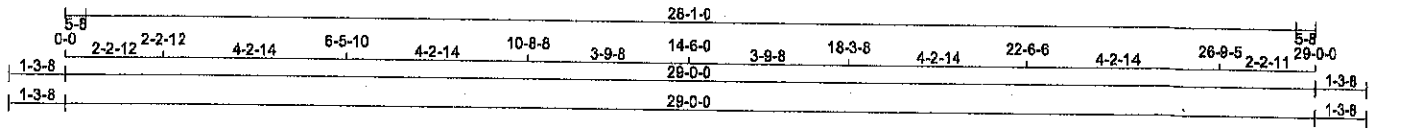

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	7.0	1.75	3.00
C	TTWW+m	MT20	5.0	10.0	3.00	1.25
D	TMWW+t	MT20	4.0	6.0	2.75	1.50
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	4.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW+t	MT20	4.0	6.0	2.75	1.50
J	TTWW+m	MT20	5.0	10.0	3.00	1.25
K	TMVW+p	MT20	6.0	7.0	1.75	3.00
M	BMV1+p	MT20	3.0	5.0		
N	BMWW+t	MT20	4.0	6.0	3.00	1.75
O	BMWW+t	MT20	5.0	8.0	4.00	2.00
P	BMWW+t	MT20	4.0	9.0	4.50	1.50
Q	BS-t	MT20	4.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	9.0	4.50	1.50
T	BMWW+t	MT20	5.0	8.0	4.00	2.00
U	BMWW+t	MT20	4.0	6.0	3.00	1.75
V	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2572	0	2572	0	0	5-8	2-14
M	2572	0	2572	0	0	5-8	2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0
M	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	
B-C	-1868 / 0	-70.5	-70.5	0.12 (1)	4.79	
C-D	-2658 / 0	-138.7	-138.7	0.21 (1)	4.89	
D-E	-3535 / 0	-138.7	-138.7	0.24 (1)	4.32	
E-F	-3776 / 0	-138.7	-138.7	0.20 (1)	4.26	
F-G	-3776 / 0	-138.7	-138.7	0.20 (1)	4.26	
G-H	-3776 / 0	-138.7	-138.7	0.20 (1)	4.26	
H-I	-3535 / 0	-138.7	-138.7	0.24 (1)	4.32	
I-J	-2658 / 0	-138.7	-138.7	0.21 (1)	4.89	
J-K	-1868 / 0	-70.5	-70.5	0.12 (1)	4.79	
K-L	0 / 40	-70.5	-70.5	0.11 (1)	10.00	
V-B	-2575 / 0	0.0	0.0	0.23 (1)	6.42	
M-K	-2574 / 0	0.0	0.0	0.23 (1)	6.42	
V-U	0 / 0	-34.5	-34.5	0.07 (1)	10.00	
U-T	0 / 1104	-34.5	-34.5	0.22 (1)	10.00	
T-S	0 / 2658	-34.5	-34.5	0.40 (1)	10.00	
S-R	0 / 3535	-34.5	-34.5	0.52 (1)	10.00	
R-Q	0 / 3536	-34.5	-34.5	0.52 (1)	10.00	
Q-P	0 / 3535	-34.5	-34.5	0.52 (1)	10.00	
P-O	0 / 2658	-34.5	-34.5	0.40 (1)	10.00	
O-N	0 / 1104	-34.5	-34.5	0.22 (1)	10.00	
N-M	0 / 0	-34.5	-34.5	0.07 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	-725 / 0	0.28 (1)
C-T	0 / 2362	0.58 (1)
T-D	-1655 / 0	0.63 (1)
D-S	0 / 1333	0.33 (1)
S-E	-858 / 0	0.33 (1)
E-R	0 / 391	0.10 (1)
R-G	-506 / 0	0.19 (1)
R-H	0 / 391	0.10 (1)
P-H	-858 / 0	0.33 (1)
P-I	0 / 1333	0.33 (1)
O-I	-1655 / 0	0.63 (1)
O-J	0 / 2362	0.58 (1)
N-J	-725 / 0	0.28 (1)
B-U	0 / 1437	0.36 (1)
N-K	0 / 1437	0.36 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-115	-115	---	FRONT	VERT	TOTAL
J	26-9-5	-114	-114	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 2 X 176 = 351 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-12
RIGHT SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.24 (H-I:1) , BC=0.52 (R-S:1) , WB=0.63 (I-O:1) , SSI=0.23 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (U) (INPUT = 0.90)
JSI METAL= 0.76 (Q) (INPUT = 1.00)

A-H073575

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H01-Cond1	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 28-9-5, AND 114.5 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LICENSED PROFESSIONAL ENGINEER
07-14-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073575(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	J01C	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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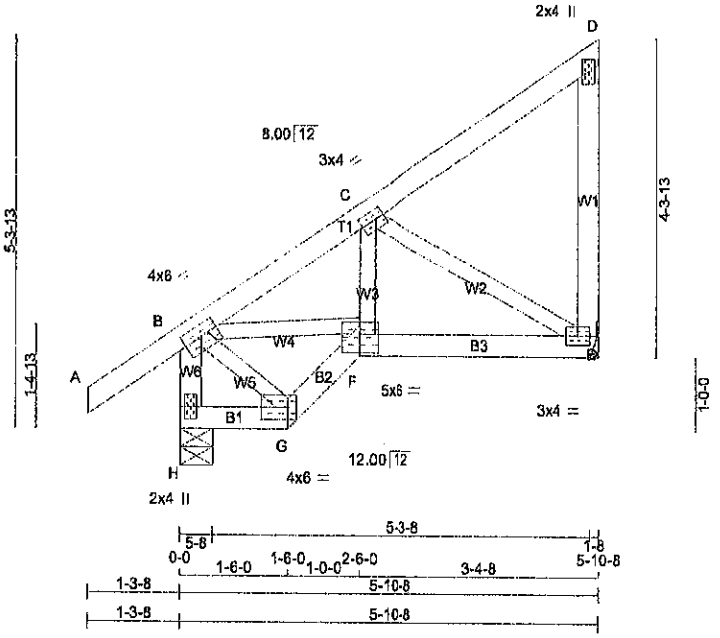


TOTAL WEIGHT = 7 X 30 = 208 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-B	-342 / 0	0.0	0.0	0.04 (1)	7.81	B-G	0 / 18	0.01 (4)
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B-F	0 / 238	0.05 (1)
B-C	-287 / 0	-70.5	-70.5	0.10 (1)	6.25	F-C	0 / 67	0.02 (4)
C-D	-12 / 0	-70.5	-70.5	0.10 (1)	6.25	C-E	-264 / 0	0.07 (1)
E-D	-93 / 0	0.0	0.0	0.03 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.01 (4)	10.00			
G-F	0 / 20	-17.5	-17.5	0.01 (4)	10.00			
F-E	0 / 248	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C;1) , BC=0.08 (E-F;4) , WB=0.07 (C-E;1) , SSI=0.09 (C-D;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

A-1507357Y

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

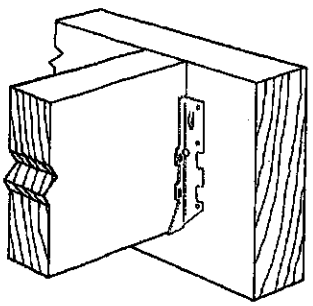
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

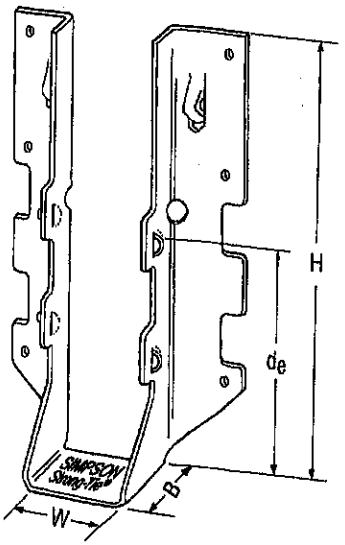
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



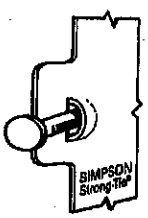
Typical LUS Installation



LUS28

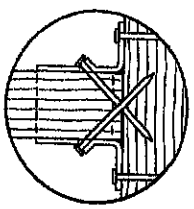
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

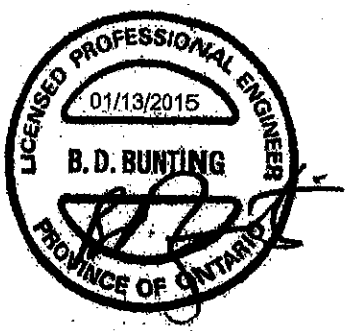


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

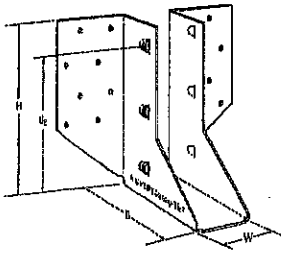
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

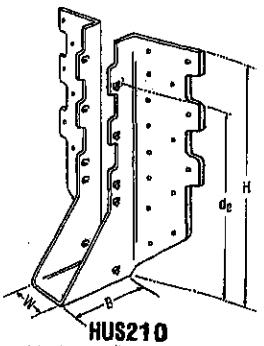
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

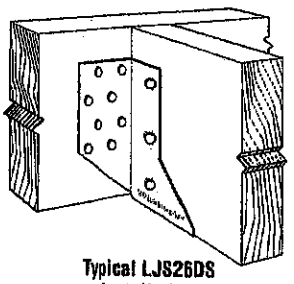
- See current catalogue for options



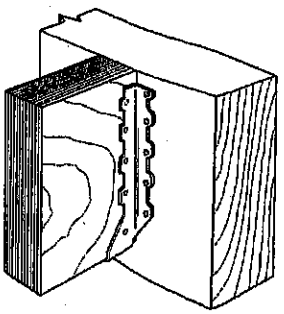
LJS26DS



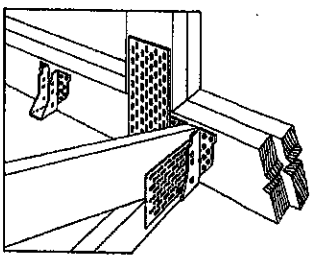
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



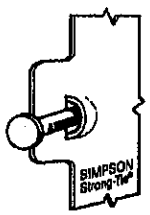
Typical HUS Installation



Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

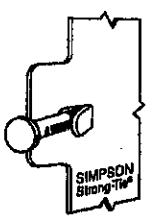
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	16-16d	6-16d	2055	4265	1480	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6³⁄₃₂	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

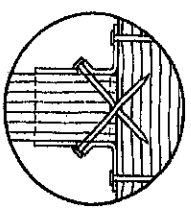


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

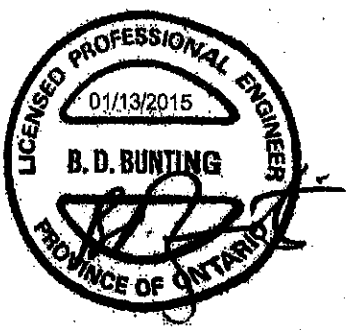
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

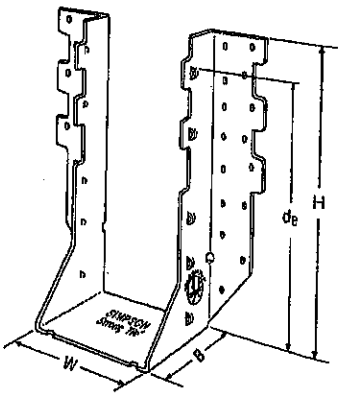
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

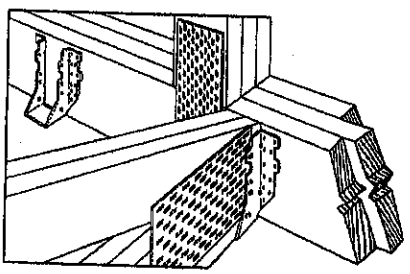
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



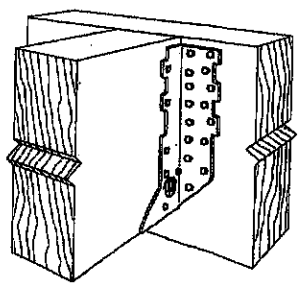
HHUS410



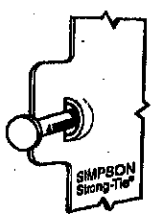
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_g[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 1/32	3	6 5/32	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	30-16d	10-16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 25/32	3	7 21/32	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 13/32	3	3 15/16	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3 5/8	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 23/32	30-16d	10-16d	4745	10770	4310	7650

1. d_g is the distance from the seat of the hanger to the highest joist nail.

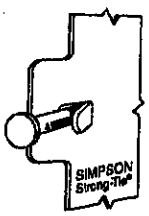


Typical HHUS Installation

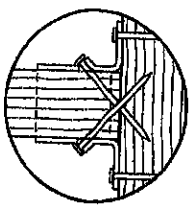


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

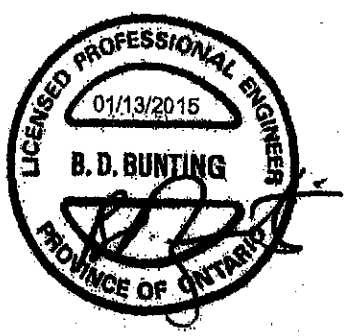
U.S. Patent 5,603,580



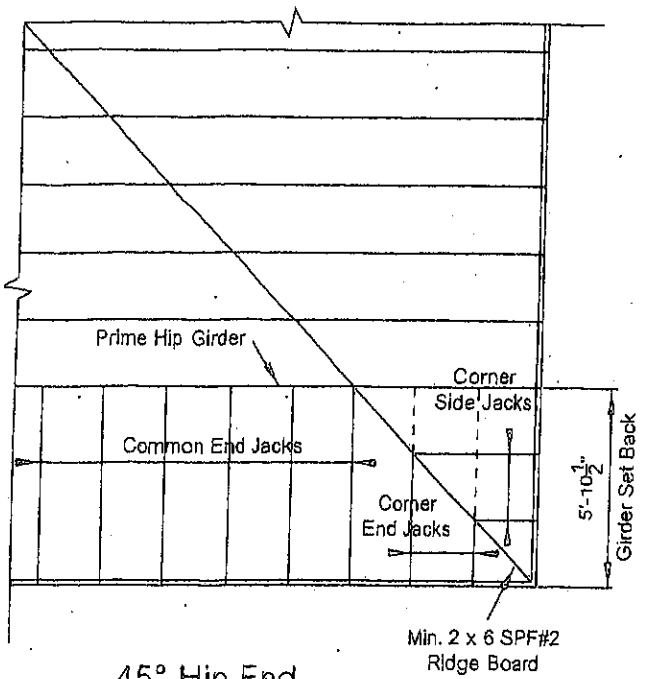
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



STRACON ENGINEERING INC.



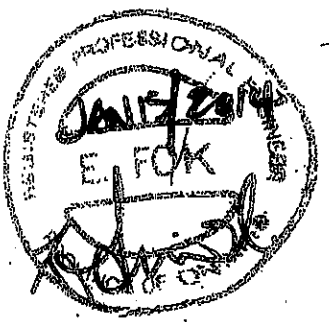
45° Hip End

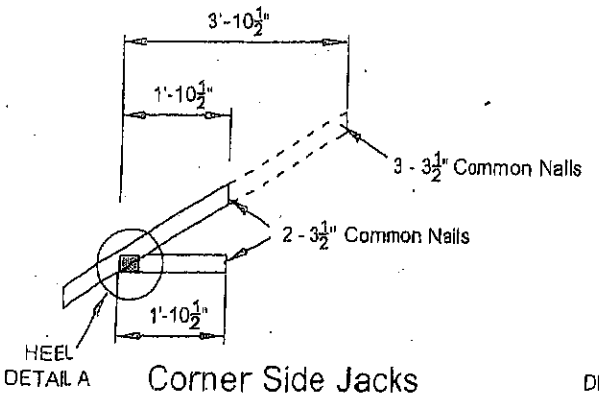
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

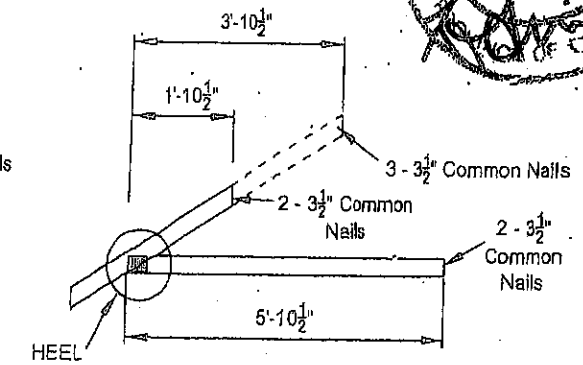
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

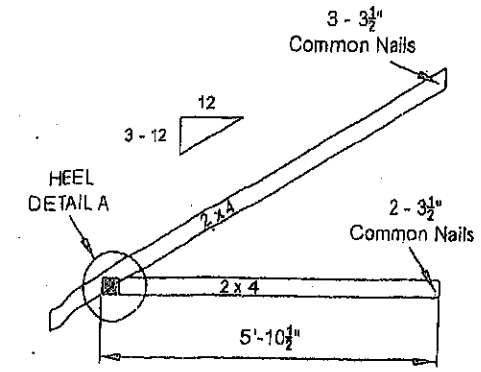




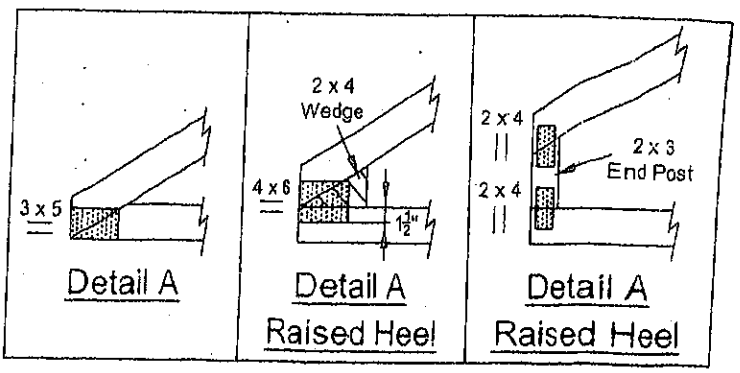
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008