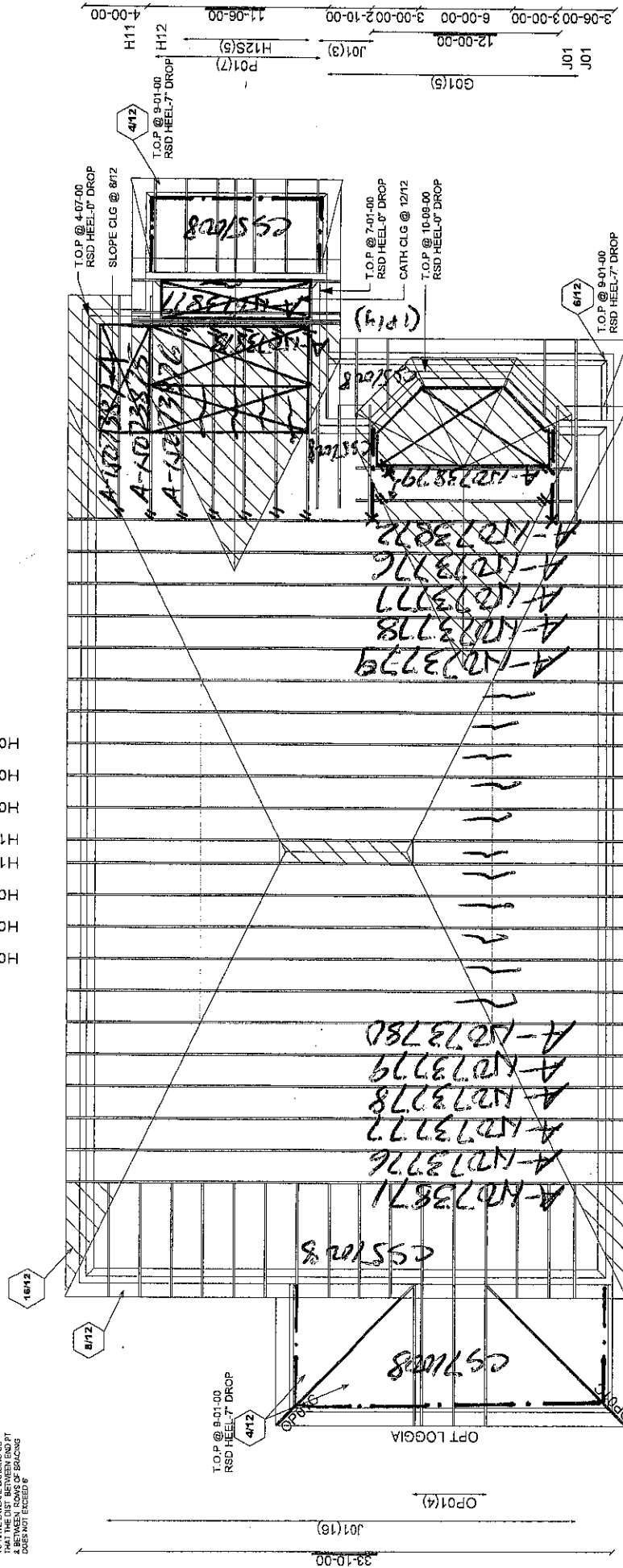


W
O
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O



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
•PIGGYBACK TRS
PURINS BY OTHERS

HANGERS
// LUS24
X HHUS26-2

15-15

54.02-00

3-00-00	6-01-00	5-00-00
---------	---------	---------

Piggyback



CONVENTIONAL
FRAMING BY
OTHERS

Mitek Ver. 7.5.0

Builder / Location:

Job Track: 40297

Job Track: 40291

Plan Log: 79711

Model / Elevation:

42-5

STD OR OPT 2ND FLR.

1A

Project: **HUNTINGTON & NASHVILLE**

Date:	4/29/2015	Designer: cheryle
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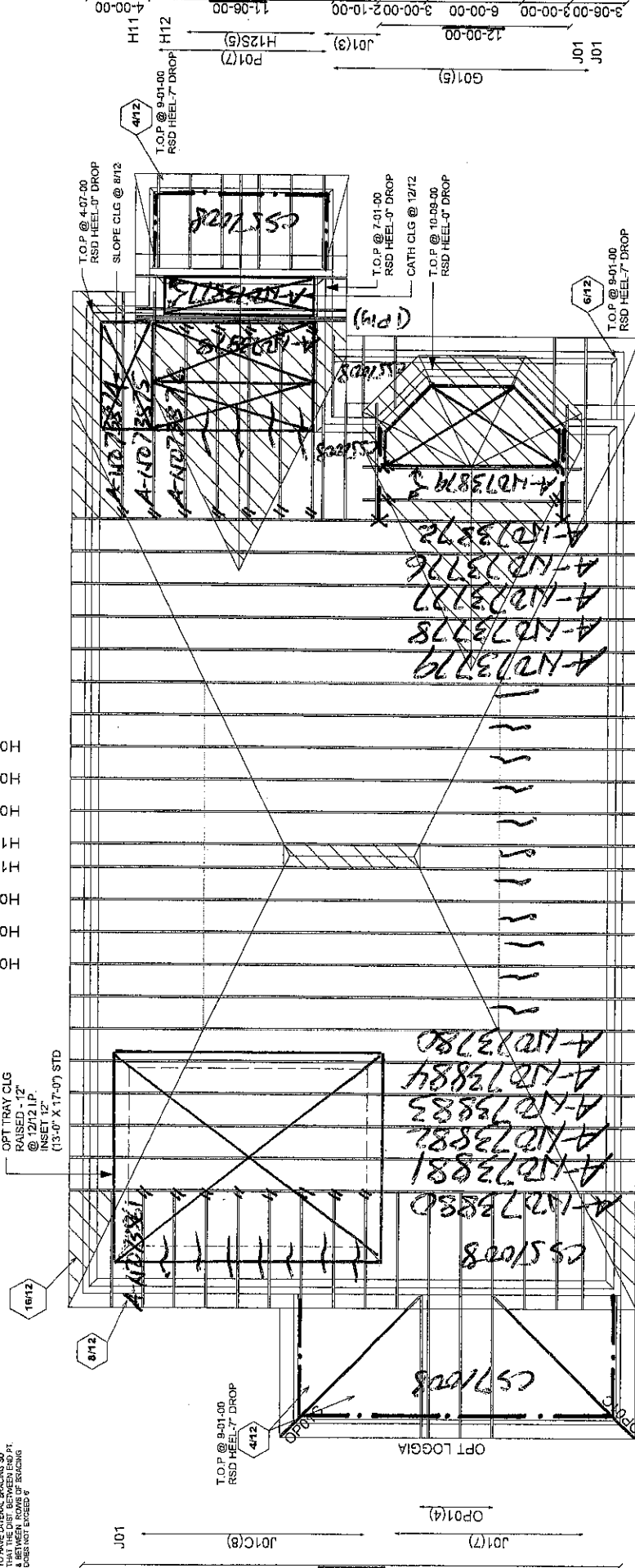


ALL DIM. FRAMING TO CONFORM
WITH ALPA TRUSS MANUFACTURING
PRACTICES. TRUSS MANUFACTURER
RESERVES THE RIGHT TO MAKE
CHANGES TO THIS DRAWING
WITHOUT NOTICE. THE USER
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THE MANUFACTURE OF TRUSSES
BY ALPA ROOF TRUSSES INC.

2-04-00 5-00-00
H01 H02 H03 H04 H05
H07 H08 H09 H10 H11
H12

80-11-00
H06(12)

8-00-00



12' FIN. OH.
RSD. HEEL - 0' DROP
(2' x 6' FASCIA) - FM
ASPHALT SHINGLES
2' x 6' BRICK
PIGGYBACK TRS
PURING BY OTHERS

H01 H02 H03 H04 H05 H06 H07 H08 H09 H10 H11 H12

3-06-00 3-00-00 6-00-00 12-00-00 3-00-00 2-10-00 11-06-00 4-00-00

G01(5) J01(3) J01(7) P01(7) H12 H11

H01 H02 H03 H04 H05 H06 H07 H08 H09 H10 H11 H12

3-00-00 6-01-00 5-00-00

54-02-00

8-00-00

HANGERS
// LUS24
X HHUS26-2

CONVENTIONAL FRAMING BY OTHERS

Model / Elevation: 42-5 / A OPT TRAY

Builder / Location: GOLD PARK / KLEINBURG

Job Track: 40297

Layout ID: 253642

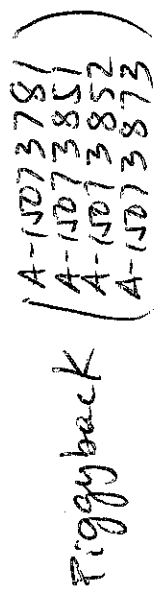
Plan Log: 79711

Project: HUNTINGTON & NASHVILLE

Date: 4/29/2015 Designer: cheryle

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CONVENTIONAL
FRAMING BY
OTHERS

Mitek Ver: 7.5.0

Model / Elevation:

/ A OPT 2ND W/TRAY

Builder / Location:
GOLD PARK / KLEINBURG

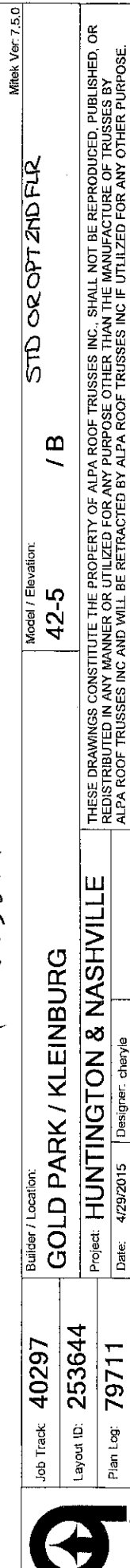
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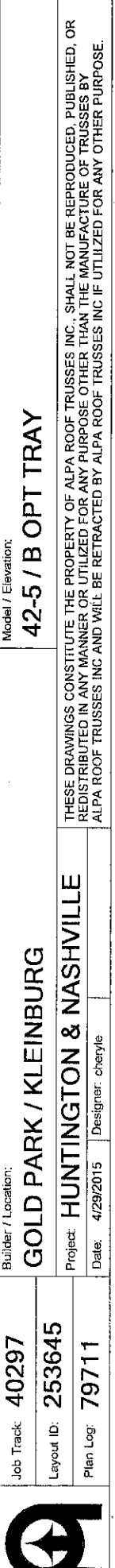
Job Track: **40297**

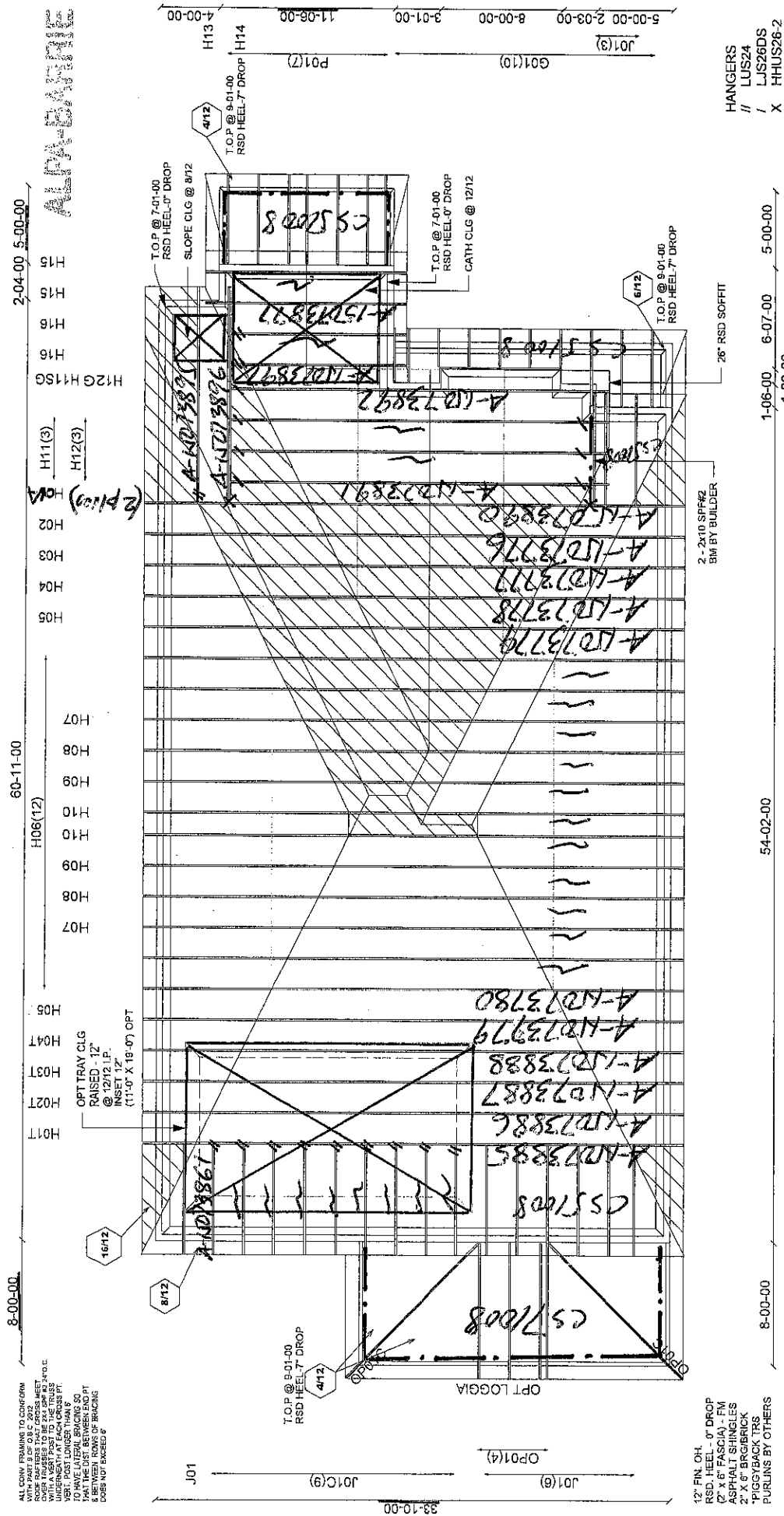
Layout ID: 253643

Plan Log: 79711









CONVENTIONAL
FRAMING BY
OTHERS

Mitek Ver: 7.5.0

Model / Elevation:

42-5

/ B OPT 2ND W/TRAY

Builder / Location:

Job Track: 40297

Job Track: 40297

Layout ID: 253646

Project: **HUNTINGTON & NASHVILLE**

Date:	4/29/2015	Designer: cheryl
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Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

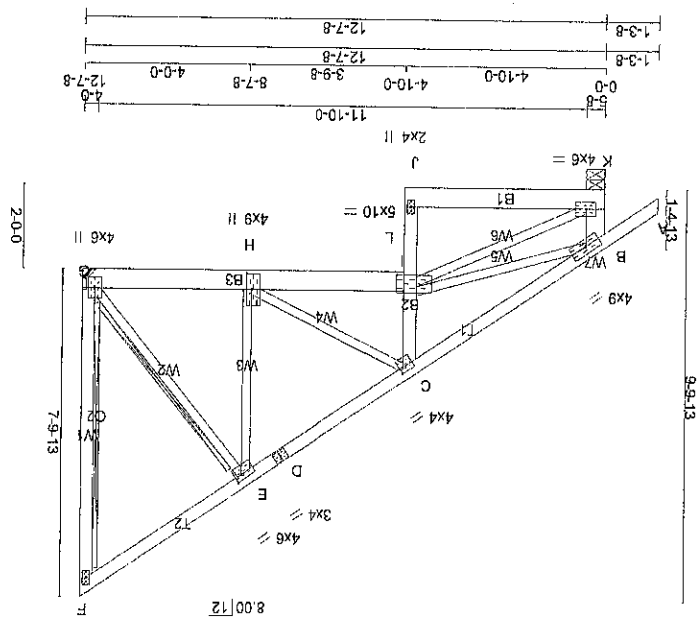
1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Mon Jul 20 10:22:28 2015 Page 1
ID:Ayrlx9Y7a2EXF7DRQXsq4MEz1-fJ-IDBuH2_mHn7_jc2eRgXawfCIVN1Weu475yWD7C
2x4 11
Scale = 1:55.1



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF

DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF
SPACING = 24.0 IN. C/C			

```
GIRDER TYPE: CSlGldr
START DISTANCE = 0-0
START SPAN CARRIED = 10-3-0
END DISTANCE = 6-3-0
END SPAN CARRIED = 10-3-0
END WALL WIDTH = 0-0
```

APPLIED TO FRONT SIDE OF BOTTOM CHORD.
-ADDTL LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)
**** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO

ALL LOAD CASES,
THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
PART 9 OF OBC 2012, BCBC 2012, ABC 2014
CSA 086-09
TPIC 2011
65 % OF 23.0 P.S.F., G.S.L. PLUS

21.0 P.S.F. RAIN LOAD EQUALS

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMB=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50
AUTOSOLVE RIGHT HEEL ONLY
RUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAX MIN MAX MIN MAX MIN MAX MIN
618 354 1667 822 2284 1656
TT20

96
CONTINUED ON PAGE 2

LUMBER	N.L.G.A.	RULES	CHORDS	SIZE	LUMBER	DESCR.
SPP	No.2	Dry	K - B	2x6	Dry	SPP
SPP	No.2	Dry	A - D	2x4	Dry	SPP
SPP	No.2	Dry	D - F	2x4	Dry	SPP
SPP	No.2	Dry	G - J	2x4	Dry	SPP
SPP	No.2	Dry	I - G	2x6	Dry	SPP
SPP	No.2	Dry	J - C	2x4	Dry	SPP
SPP	No.2	Dry	ALL WEBS	2x3	Dry	SPP
SPP	No.2	Dry	DRY SEASONED LUMBER			SPP

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

K-8	2	12	TOP
A-D	1	12	TOP
D-F	1	12	TOP
E-G	1	12	TOP

TOP

J-C	1	12
WEBS : (0.122"x3") SPIRAL NAILS	1	6
2X3	1	
TOP		

MAILS TO BE DRIVEN FROM ONE SIDE ONLY.

FOR THE LOAD TO BE TRANSFERRED TO EACH PLY,
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED

TO ONE SIDE THAT THE CORRESPONDING NAILING
PATTERN SHALL BE CAPABLE OF TRANSFERRING,
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE
SIDE OR ON THE TOP.

PROFESSIONAL

RECEIVED PHOENIX
FEDERAL BUREAU OF INVESTIGATION
U.S. DEPARTMENT OF JUSTICE
MAY 1 1964

100-443614-100
R. TURENNE
JAN 11 1963
FBI - NEW YORK

010752040



10



NOTES: (1)
1) Lateral brace(s) shown shall be 1x4 for Part 5 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

PLATES (tablets in inches)			
PLATE	TYPE	W	LEN
A	PLATE	4.0	9.0
B	TMVW-1	4.0	9.0
C	TMVW-1	4.0	1.50
D	TS-1	4.0	3.0
E	TMVW-1	4.0	6.0
F	TMV+P	4.0	2.0
G	BMVW1+P	4.0	8.0
H	BMVW1+P	4.0	9.0
I	BMVW1+P	4.0	9.0
J	BMV+P	2.0	4.0
K	BMVW1-1	4.0	6.0

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (INPUT = 0.90)

JSI METAL = 0.41 (B) (INPUT = 1.00)

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ID: AyruX9Y7fa2EXF7DRQXSG4zMez1-fJ IDBuHz_mHNT lc2eRgXaWfCIVN1Wau475yWd7C

Alpha Roof Truss, Maple

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

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QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

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QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

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QUANTITY

PLY

JOB DESC.

TRUSS DESC.

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DRWG NO.

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QUANTITY

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JOB DESC.

TRUSS DESC.

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DRWG NO.

253644

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QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

2

1

DRWG NO.

253644

H14

✓A-N073894

TOTAL WEIGHT = 3 X 33 = 98 lb



LUMBER	
N.L.G.A. RULES	
CHORDS	
SIZE	DESCR.
A-C	2x4
C-G	2x4
J-L	2x4
M-K	2x6
W-B	2x6
W-S	2x6
S-O	2x6
D-M	2x6
E-F	2x6
V-C	2x6
2x8	2x8
CHORDS #ROWS	
SURFACE	
SPACING (IN)	
TOP CHORDS: (0.122"x3") SPIRAL NAILS	
SIDE(61.0)	
SIDE(52.1)	
TOP	
CHORDS	
SIDE(174.7)	
SIDE(122.1)	
TOP	
W-B	
W-S	
S-O	
D-M	
E-F	
V-C	
2x8	
FASTENERS	
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.	
GRADER NAILING ASSUMES NAIL HANGERS ARE	
FASTENED WITH MIN. 3-0 INCH NAILS.	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER	
BEARINGS	
FACTORED	
MAXIMUM FACTORED	
INPUT	RECORD
BRG	BRG
IN-SX	IN-SX
DOWN	UP
4021	5882
0	0
5882	0
M	4-1
UNFACTORED REACTIONS	
1ST LCASE	
MAX/MIN. COMPONENT REACTIONS	
COMBINED	SNOW
LIVE	PERM/LIVE
WIND	DEAD
W	SOL
M	W
4149	1926 / 0
2623 / 0	0 / 0
0 / 0	0 / 0
0 / 0	0 / 0
1326 / 0	0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S). W. M.	
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.	
MAX. UNRATED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.	

PLATES (table is in inches)	TYPE	PLATES	LEN	Y	X
B	TW-4	MT20	6.0	9.0	2.00
C	TW-4	MT20	6.0	9.0	2.00
D	TW-4	MT20	4.0	4.0	2.00
E	TW-4	MT20	4.0	4.0	2.25
F	TW-4	MT20	2.0	4.0	2.00
G	TS-1	MT20	3.0	6.0	3.00
H	TW-4	MT20	4.0	4.0	2.00
I	TW-4	MT20	4.0	4.0	2.00
J	TW-4	MT20	8.0	8.0	2.75
K	TW-4	MT20	8.0	8.0	2.00
L	BW-1+p	MT20	3.0	6.0	3.00
M	BW-1+p	MT20	4.0	4.0	3.00
N	BW-1+p	MT20	4.0	4.0	2.25
O	BW-1	MT20	6.0	6.0	4.50
P	BW-1	MT20	4.0	4.0	2.25
Q	RS-1	MT20	4.0	6.0	3.00
R	RS-1	MT20	4.0	6.0	2.25
S	RS-1	MT20	4.0	4.0	2.00
T	BW-1	MT20	4.0	4.0	2.00
U	BW-1	MT20	6.0	6.0	2.25
V	BW-1	MT20	4.0	4.0	1.50
W	BW-1+p	MT20	3.0	5.0	4.50

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.5 lbs FACTORED DOWN AT 2-2-12 DOWN AT 4'-10" AND 3564.0 lbs FACTORED DOWN AT 28'-8" ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



A-1073890(2)

COMPACTION LIVE LOAD FACTOR = 0.50
AUTOSLOVE HELLS OFF
TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GMP(DRY) SHEAR SECTION
(PSI) (PL)
MAX MIN MAX MIN
MT20 618 354 1687 2222 1584 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JIS GRIP = 0.86 (T) (INPUT = 0.90)
JIS METAL = 0.51 (Q) (INPUT = 1.00)

LUMBER DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY TOTAL WEIGHT = 189 lb

A-CHORDS	SIZE	DBY	NO. 3	LUMBER	DESCR.	BEARINGS	EASTORED	MAXIMUM FACTORED	INPUT	STRESS
----------	------	-----	-------	--------	--------	----------	----------	------------------	-------	--------

G - J	2x4	DRY	1650F 15E	SPF	JT	VERT	HORZ	DOWN	HORZ	UP/LIFT	IN-SX	IN-SX
-------	-----	-----	-----------	-----	----	------	------	------	------	---------	-------	-------

W - K	2x6	DRY	No. 2	No. 1	S.P.F.	M	2919	0	2919	0	5-8	4-0	DL = 7.0 PSF
-------	-----	-----	-------	-------	--------	---	------	---	------	---	-----	-----	--------------

S	•	Q	2x6	DRY	No.2	SPF	UNFACTORED REACTIONS	SPACING =	24.0	IN	C/C
---	---	---	-----	-----	------	-----	----------------------	-----------	------	----	-----

[illegible][illegible]

LEFT SETBACK = 2-2-12

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.83FT

APPLIED.

	MT20	TMW-1	LOADING
5.0	8.0	2.00	2.00
-ADDITIONAL LOADS BASED ON 66 % OF GSL.			

10	TMWV-1	MT20	5.0	8.0	2.25	4.00	OR SMALL BUILDING REQUIREMENTS OF
----	--------	------	-----	-----	------	------	-----------------------------------

6	TS-1	M720	3.0	8.0	MEMB.	FORCE	
7	TRAY	M720	2.0	9.0	JACK	LOAD	
8					MEMB.	FORCE	
9					VERT	LOAD	
10					MEMB.	FORCE	
11					WIND	FORCE	
12					SEISMIC	FORCE	
13					TEMPERATURE	FORCE	
14					SHRINKAGE	FORCE	
15					CREEP	FORCE	
16					CONCRETE	FORCE	
17					STEEL	FORCE	
18					JOINT	FORCE	
19					ANCHOR	FORCE	
20					REINFORCING	FORCE	
21					BAR	FORCE	
22					WELD	FORCE	
23					BOLTED	FORCE	
24					BRACKET	FORCE	
25					PLATE	FORCE	
26					SCREW	FORCE	
27					NUT	FORCE	
28					WASHER	FORCE	
29					SPACER	FORCE	
30					CLAMP	FORCE	
31					STRAP	FORCE	
32					ROPE	FORCE	
33					CABLE	FORCE	
34					PIPE	FORCE	
35					DUCT	FORCE	
36					CHANNEL	FORCE	
37					ANGLE	FORCE	
38					FLAT BAR	FORCE	
39					RIB	FORCE	
40					STIFFENER	FORCE	
41					GUSSET	FORCE	
42					END PLATE	FORCE	
43					WEBSIDE	FORCE	
44					TOE	FORCE	
45					FLANGE	FORCE	
46					WEB	FORCE	
47					DIAPHRAGM	FORCE	
48					TRANSVERSE	FORCE	
49					LONGITUDINAL	FORCE	
50					TORSION	FORCE	
51					BENDING	FORCE	
52					SHEAR	FORCE	
53					COMPRESSION	FORCE	
54					TENSION	FORCE	
55					AXIAL	FORCE	
56					ROTATION	FORCE	
57					DISPLACEMENT	FORCE	
58					VELOCITY	FORCE	
59					ACCELERATION	FORCE	
60					STRESS	FORCE	
61					STRAIN	FORCE	
62					MODULUS	FORCE	
63					POISSON'S RATIO	FORCE	
64					YIELD STRESS	FORCE	
65					TENSILE STRENGTH	FORCE	
66					COMPRESSIVE STRENGTH	FORCE	
67					SHEAR STRENGTH	FORCE	
68					BOND STRENGTH	FORCE	
69					WELD TENSILE STRENGTH	FORCE	
70					WELD SHEAR STRENGTH	FORCE	
71					BOLTED JOINT TENSILE STRENGTH	FORCE	
72					BOLTED JOINT SHEAR STRENGTH	FORCE	
73					BRACKET TENSILE STRENGTH	FORCE	
74					BRACKET SHEAR STRENGTH	FORCE	
75					PLATE TENSILE STRENGTH	FORCE	
76					PLATE SHEAR STRENGTH	FORCE	
77					SCREW TENSILE STRENGTH	FORCE	
78							

FR-TO	FROM TO	LENGTH FR-TO	- CSA 086-08
MT20	5.0	8.0	2.25 4.00
TMWW-1	5.0	8.0	2.25 4.00

[illegible]

O	BMW	7.0	2.75	2.25	-138.7	-138.7	0.94 (1)	2.83	T-E	-1349/0	0.55 (1)	21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
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ITEM	QTY	UNIT	PRICE	TOTAL
1.000	1.000	EA	1.000	1.000
2.000	2.000	EA	2.000	2.000
3.000	3.000	EA	3.000	3.000
4.000	4.000	EA	4.000	4.000
5.000	5.000	EA	5.000	5.000
6.000	6.000	EA	6.000	6.000
7.000	7.000	EA	7.000	7.000
8.000	8.000	EA	8.000	8.000
9.000	9.000	EA	9.000	9.000
10.000	10.000	EA	10.000	10.000
11.000	11.000	EA	11.000	11.000
12.000	12.000	EA	12.000	12.000
13.000	13.000	EA	13.000	13.000
14.000	14.000	EA	14.000	14.000
15.000	15.000	EA	15.000	15.000
16.000	16.000	EA	16.000	16.000
17.000	17.000	EA	17.000	17.000
18.000	18.000	EA	18.000	18.000
19.000	19.000	EA	19.000	19.000
20.000	20.000	EA	20.000	20.000
21.000	21.000	EA	21.000	21.000
22.000	22.000	EA	22.000	22.000
23.000	23.000	EA	23.000	23.000
24.000	24.000	EA	24.000	24.000
25.000	25.000	EA	25.000	25.000
26.000	26.000	EA	26.000	26.000
27.000	27.000	EA	27.000	27.000
28.000	28.000	EA	28.000	28.000
29.000	29.000	EA	29.000	29.000
30.000	30.000	EA	30.000	30.000
31.000	31.000	EA	31.000	31.000
32.000	32.000	EA	32.000	32.000
33.000	33.000	EA	33.000	33.000
34.000	34.000	EA	34.000	34.000
35.000	35.000	EA	35.000	35.000
36.000	36.000	EA	36.000	36.000
37.000	37.000	EA	37.000	37.000
38.000	38.000	EA	38.000	38.000
39.000	39.000	EA	39.000	39.000
40.000	40.000	EA	40.000	40.000
41.000	41.000	EA	41.000	41.000
42.000	42.000	EA	42.000	42.000
43.000	43.000	EA	43.000	43.000
44.000	44.000	EA	44.000	44.000
45.000	45.000	EA	45.000	45.000
46.000	46.000	EA	46.000	46.000
47.000	47.000	EA	47.000	47.000
48.000	48.000	EA	48.000	48.000
49.000	49.000	EA	49.000	49.000
50.000	50.000	EA	50.000	50.000
51.000	51.000	EA	51.000	51.000
52.000	52.000	EA	52.000	52.000
53.000	53.000	EA	53.000	53.000
54.000	54.000	EA	54.000	54.000
55.000	55.000	EA	55.000	55.000
56.000	56.000	EA	56.000	56.000
57.000	57.000	EA	57.000	57.000
58.000	58.000	EA	58.000	58.000
59.000	59.000	EA	59.000	59.000
60.000	60.000	EA	60.000	60.000
61.000	61.000	EA	61.000	61.000
62.000	62.000	EA	62.000	62.000
63.000	63.000	EA	63.000	63.000
64.000	64.000	EA	64.000	64.000
65.000	65.000	EA	65.000	65.000
66.000	66.000	EA	66.000</	

T	BMW4	MT20	5.0	8.0	2.25	2.00	J-K	-2156/0	-70.5	-70.5	0.13(1)	4.50	P-1	0/2473	0.61(1)	CALCULATED VERT. DEF. (L) = 1/840 (0.42)
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[illegible]

W-V	0/0	-34.5	-34.5	0.11 (1)	10.00	N-K	0.11643	0.41 (1)	DOL LUMBER=1.00 NAIL=1.00 FS BRND=1.00
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[illegible]

R-Q	0 / 4263	-34.5, -34.5	0.66 (1)	10.00	AUTOSOLVE HEELS OFF
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[illegible]

THE PROPOSED CHANGING PLANS:

LOC.	LCT	MAX-	FACE	DIR.	VEBT	TOTAL	PLATE GRIP(DRY) SHEAR SECTION
2-2-12		-115					
2-2-13		-115					
		MAX+					

MT20	618	354	1667	822	2284	1656
	1000	1000	1000	1000	1000	1000

PLATE PLACEMENT TOL. = 0.250 inches

INCE OF ON

JSI METAL = 0.86 (Q) (INPWT = 1.00)

Continued on page 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
253644	H01	1	1			

Alpha Roof Truss, Maple

ID:AvrJx9y7ia2EXF7DRQxSg4zMEz1-mE3TS780dKUKpIdVWmzhGqMMkIEGNmRb0w1 KYWD75
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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGERS) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 114.2 lbs FACTORED DOWN AT 30-9.5.
AND 114.5 lbs FACTORED DOWN AT 2-2-12 ON
TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

NOTES - (1)

OBC 8.23.13.11, and no less than 2x4 for Part 4 design as per
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per



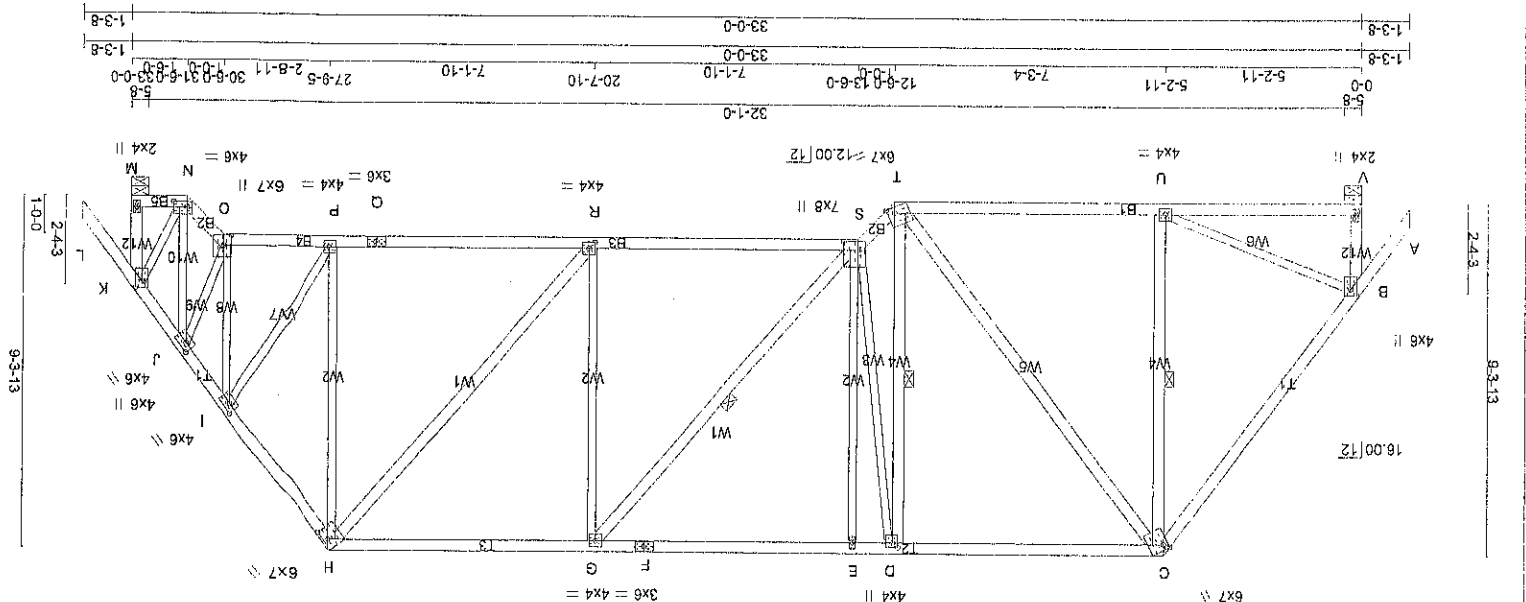
A-11073889(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253643	H04T	1	1	TRUSS DESC.	

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ID:AyrJX9Y7fa2EXF7DQRXSq4MfEZ1-ddsXJJBH8co75X8X/A.D382EQRCVzmq7CQZKgzHjWwEhT

Scale = 1:61



LUMBER	N.L.G.A. RULES	BUILDING DESIGNER	DESIGN CRITERIA
	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY		

[illegible]

DRY, SEASONED LUMBER,					
U	C	T	2x4	DRY	No.2
S	D	2x4	DRY	No.2	
R	H	2x4	DRY	No.2	
BRACING					
S	T	2x4	DRY	No.2	
S	D	2x4	DRY	No.2	
S	H	2x4	DRY	No.2	
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43FT.					
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY					
APPLIED.					
1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-U, D-T, G-S.					
OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 8 OF CBC 2012, BOBC 2012, ABC 2014 - CSA 086-09 - TPC 2011					

END VERTICAL(S) MUST BE SHEARED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

PLATES (tab is in inches)	LOADING	TOTAL LOAD CASES: (4)	CH:	(66 % OF 23.0 P.S.F. G.S.L. PLUS
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ITEM	TYPE	W	L	Y	PLATES	CHORDS	MAX FACTORED	FACTORED	MAX FACTORED	WEBS	MAX FACTORED	FACTORED
B	TMWV+p	MT20	4.0	6.0	2.00	2.00						
C	TMWV+m	MT20	6.0	7.0	Edge	1.50						

8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

MEMB.	FORCE	VERT. LOAD	CS1 (LC)	UNBRAC	CS1 (LC)	FORCE MAX	CS1 (LC)
D TMMW+1	MT20	4.0	4.0	1.50	1.75		
E TMMW+	MT20	2.0	2.0	4.0			
F TMMW	MT20	2.0	2.0	4.0			

ALLOWABLE DEF. (LT) = 0.360 (1.10%)
 CALCULATED VERT. DEF. (LT) = 0.989 (0.10%)

ITEM	QTY	UNIT	PRICE	TOTAL	AMOUNT	DATE	REMARKS
1	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
2	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
3	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
4	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
5	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
6	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
7	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
8	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
9	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
10	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
11	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
12	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
13	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
14	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
15	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
16	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
17	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
18	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
19	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
20	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
21	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
22	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
23	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
24	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
25	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
26	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
27	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
28	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
29	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
30	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
31	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
32	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
33	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
34	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
35	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
36	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
37	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
38	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
39	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
40	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
41	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
42	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
43	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
44	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
45	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
46	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
47	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
48	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
49	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
50	1	EA	1.00	1.00	1.00	1/1/00	INITIAL
51	1	EA	1.00	1.00	1.00	1/1/00	INITIAL</

	CSH	TC	0.71	(C-D)	1), BC=0.36	(R-S)	1), WB=0.81	(G-R)	1), SS=0.26	(O-E)	1)
C-D	-1294 / 0	-70.5	-70.5	0.74 (1)	4.66	T-D -1957 / 0	4.42	D-S	0 / 1338	0.30 (1)	
D-E	-1466 / 0	-70.5	-70.5	0.60 (1)	4.42	D-S	0 / 1338	0.30 (1)			

N	B8W-1	M720	G-H	-1463/0	-70.5	-70.5	0.66(1)	4.50	S-G	-3/16	0.00(1)	0.91(1)
M	BMW1-p	M720	F-C	-1464/0	-70.5	-70.5	0.66(1)	4.50	S-E	0/37	0.01(1)	0.01(1)
M	TBMW-P	M720	E-F	-1464/0	-70.5	-70.5	0.66(1)	4.50	S-E	0/37	0.01(1)	0.01(1)

	O	BWMM+p	M720	6.0	7.0	3.25	2.75	H-1	-1384 / 0	-70.5	-70.5	0.09 (1)	5.51	R-H	0 / 981	0.16 (1)	COMP=1.10 SHEAR=1.10 LENS=1.10	COMPACTION LIVE LOAD FACTOR = 0.60
P		BWMM-1	M720	4.0	4.0			I-J	-1255 / 0	-70.5	-70.5	0.06 (1)	5.88	P-H	0 / 88	0.03 (4)		

[illegible][illegible][illegible]

PLATE	GRID	DIRECTION	SECTION	MAX MIN	MAX MIN	MAX MIN
1	1	1	1	10.00	10.00	10.00
2	2	2	2	10.00	10.00	10.00
3	3	3	3	10.00	10.00	10.00
4	4	4	4	10.00	10.00	10.00
5	5	5	5	10.00	10.00	10.00
6	6	6	6	10.00	10.00	10.00
7	7	7	7	10.00	10.00	10.00
8	8	8	8	10.00	10.00	10.00
9	9	9	9	10.00	10.00	10.00
10	10	10	10	10.00	10.00	10.00
11	11	11	11	10.00	10.00	10.00
12	12	12	12	10.00	10.00	10.00
13	13	13	13	10.00	10.00	10.00
14	14	14	14	10.00	10.00	10.00
15	15	15	15	10.00	10.00	10.00
16	16	16	16	10.00	10.00	10.00
17	17	17	17	10.00	10.00	10.00
18	18	18	18	10.00	10.00	10.00
19	19	19	19	10.00	10.00	10.00
20	20	20	20	10.00	10.00	10.00
21	21	21	21	10.00	10.00	10.00
22	22	22	22	10.00	10.00	10.00
23	23	23	23	10.00	10.00	10.00
24	24	24	24	10.00	10.00	10.00
25	25	25	25	10.00	10.00	10.00
26	26	26	26	10.00	10.00	10.00
27	27	27	27	10.00	10.00	10.00
28	28	28	28	10.00	10.00	10.00
29	29	29	29	10.00	10.00	10.00
30	30	30	30	10.00	10.00	10.00
31	31	31	31	10.00	10.00	10.00
32	32	32	32	10.00	10.00	10.00
33	33	33	33	10.00	10.00	10.00
34	34	34	34	10.00	10.00	10.00
35	35	35	35	10.00	10.00	10.00
36	36	36	36	10.00	10.00	10.00
37	37	37	37	10.00	10.00	10.00
38	38	38	38	10.00	10.00	10.00
39	39	39	39	10.00	10.00	10.00
40	40	40	40	10.00	10.00	10.00
41	41	41	41	10.00	10.00	10.00
42	42	42	42	10.00	10.00	10.00
43	43	43	43	10.00	10.00	10.00
44	44	44	44	10.00	10.00	10.00
45	45	45	45	10.00	10.00	10.00
46	46	46	46	10.00	10.00	10.00
47	47	47	47	10.00	10.00	10.00
48	48	48	48	10.00	10.00	10.00
49	49	49	49	10.00	10.00	10.00
50	50	50	50	10.00	10.00	10.00
51	51	51	51	10.00	10.00	10.00
52	52	52	52	10.00	10.00	10.00
53	53	53	53	10.00	10.00	10.00
54	54	54	54	10.00	10.00	10.00
55	55	55	55	10.00	10.00	10.00
56	56	56	56	10.00	10.00	10.00
57	57	57</				

	C-P	P-O	D-N
-17.5 -17.5 0.29(4)	0.799	0.783	0.671
10.00	10.00	10.00	10.00

MT20 618 354 1687 822 2284 1656

PLATE ROTATION TOL. = 5.0 Deg
PLATE PLACEMENT TOL. = 0.250 inches

JSI GRIP = 0.90 (K) (INP) = 0.90



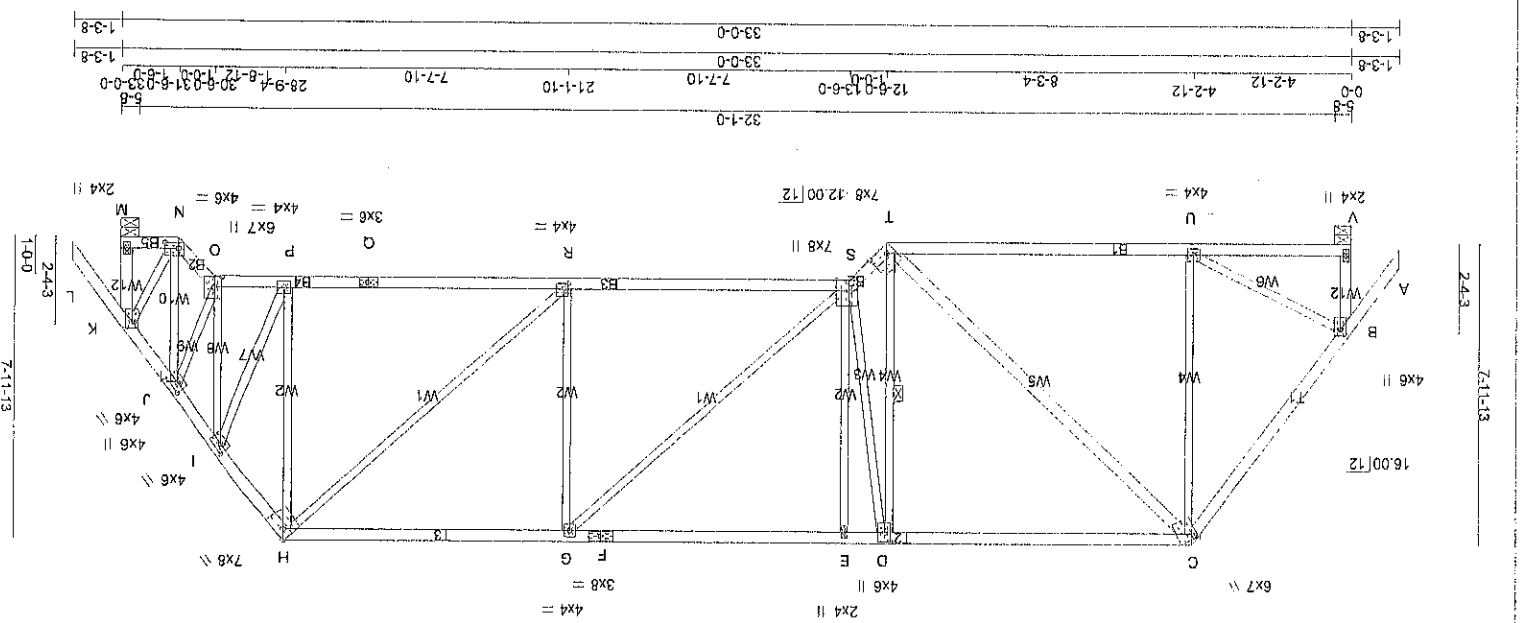
US MEAL=0.29 (B) (NPOT=1.00)

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4-171388

05861201



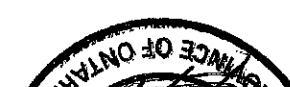


TOTAL WEIGHT = 172 lb

LUMBER		N.L.G.A. RULES		CHORDS		SIZE		DESCR.	
A	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF
B	F	F	F	2x4	2x4	2x4	2x4	SPF	SPF
C	H	H	H	2x4	2x4	2x4	2x4	SPF	SPF
D	L	L	L	2x4	2x4	2x4	2x4	SPF	SPF
E	N	N	N	2x4	2x4	2x4	2x4	SPF	SPF
F	O	O	O	2x4	2x4	2x4	2x4	SPF	SPF
G	S	S	S	2x4	2x4	2x4	2x4	SPF	SPF
H	T	T	T	2x4	2x4	2x4	2x4	SPF	SPF
I	V	V	V	2x4	2x4	2x4	2x4	SPF	SPF
J	X	X	X	2x4	2x4	2x4	2x4	SPF	SPF
K	Z	Z	Z	2x4	2x4	2x4	2x4	SPF	SPF
L	1	1	1	2x4	2x4	2x4	2x4	SPF	SPF
M	2	2	2	2x4	2x4	2x4	2x4	SPF	SPF
N	3	3	3	2x4	2x4	2x4	2x4	SPF	SPF
O	4	4	4	2x4	2x4	2x4	2x4	SPF	SPF
P	5	5	5	2x4	2x4	2x4	2x4	SPF	SPF
Q	6	6	6	2x4	2x4	2x4	2x4	SPF	SPF
R	7	7	7	2x4	2x4	2x4	2x4	SPF	SPF
S	8	8	8	2x4	2x4	2x4	2x4	SPF	SPF
T	9	9	9	2x4	2x4	2x4	2x4	SPF	SPF
U	10	10	10	2x4	2x4	2x4	2x4	SPF	SPF
V	11	11	11	2x4	2x4	2x4	2x4	SPF	SPF
W	12	12	12	2x4	2x4	2x4	2x4	SPF	SPF
X	13	13	13	2x4	2x4	2x4	2x4	SPF	SPF
Y	14	14	14	2x4	2x4	2x4	2x4	SPF	SPF
Z	15	15	15	2x4	2x4	2x4	2x4	SPF	SPF
1	16	16	16	2x4	2x4	2x4	2x4	SPF	SPF
2	17	17	17	2x4	2x4	2x4	2x4	SPF	SPF
3	18	18	18	2x4	2x4	2x4	2x4	SPF	SPF
4	19	19	19	2x4	2x4	2x4	2x4	SPF	SPF
5	20	20	20	2x4	2x4	2x4	2x4	SPF	SPF
6	21	21	21	2x4	2x4	2x4	2x4	SPF	SPF
7	22	22	22	2x4	2x4	2x4	2x4	SPF	SPF
8	23	23	23	2x4	2x4	2x4	2x4	SPF	SPF
9	24	24	24	2x4	2x4	2x4	2x4	SPF	SPF
10	25	25	25	2x4	2x4	2x4	2x4	SPF	SPF
11	26	26	26	2x4	2x4	2x4	2x4	SPF	SPF
12	27	27	27	2x4	2x4	2x4	2x4	SPF	SPF
13	28	28	28	2x4	2x4	2x4	2x4	SPF	SPF
14	29	29	29	2x4	2x4	2x4	2x4	SPF	SPF
15	30	30	30	2x4	2x4	2x4	2x4	SPF	SPF
16	31	31	31	2x4	2x4	2x4	2x4	SPF	SPF
17	32	32	32	2x4	2x4	2x4	2x4	SPF	SPF
18	33	33	33	2x4	2x4	2x4	2x4	SPF	SPF
19	34	34	34	2x4	2x4	2x4	2x4	SPF	SPF
20	35	35	35	2x4	2x4	2x4	2x4	SPF	SPF
21	36	36	36	2x4	2x4	2x4	2x4	SPF	SPF
22	37	37	37	2x4	2x4	2x4	2x4	SPF	SPF
23	38	38	38	2x4	2x4	2x4	2x4	SPF	SPF
24	39	39	39	2x4	2x4	2x4	2x4	SPF	SPF
25	40	40	40	2x4	2x4	2x4	2x4	SPF	SPF
26	41	41	41	2x4	2x4	2x4	2x4	SPF	SPF
27	42	42	42	2x4	2x4	2x4	2x4	SPF	SPF
28	43	43	43	2x4	2x4	2x4	2x4	SPF	SPF
29	44	44	44	2x4	2x4	2x4	2x4	SPF	SPF
30	45	45	45	2x4	2x4	2x4	2x4	SPF	SPF
31	46	46	46	2x4	2x4	2x4	2x4	SPF	SPF
32	47	47	47	2x4	2x4	2x4	2x4	SPF	SPF
33	48	48	48	2x4	2x4	2x4	2x4	SPF	SPF
34	49	49	49	2x4	2x4	2x4	2x4	SPF	SPF
35	50	50	50	2x4	2x4	2x4	2x4	SPF	SPF
36	51	51	51	2x4	2x4	2x4	2x4	SPF	SPF
37	52	52	52	2x4	2x4	2x4	2x4	SPF	SPF
38	53	53	53	2x4	2x4	2x4	2x4	SPF	SPF
39	54	54	54	2x4	2x4	2x4	2x4	SPF	SPF
40	55	55	55	2x4	2x4	2x4	2x4	SPF	SPF
41	56	56	56	2x4	2x4	2x4	2x4	SPF	SPF
42	57	57	57	2x4	2x4	2x4	2x4	SPF	SPF
43	58	58	58	2x4	2x4	2x4	2x4	SPF	SPF
44	59	59	59	2x4	2x4	2x4	2x4	SPF	SPF
45	60	60	60	2x4	2x4	2x4	2x4	SPF	SPF
46	61	61	61	2x4	2x4	2x4	2x4	SPF	SPF
47	62	62	62	2x4	2x4	2x4	2x4	SPF	SPF
48	63	63	63	2x4	2x4	2x4	2x4	SPF	SPF
49	64	64	64	2x4	2x4	2x4	2x4	SPF	SPF
50	65	65	65	2x4	2x4	2x4	2x4	SPF	SPF
51	66	66	66	2x4	2x4	2x4	2x4	SPF	SPF
52	67	67	67	2x4	2x4	2x4	2x4	SPF	SPF
53	68	68	68	2x4	2x4	2x4	2x4	SPF	SPF
54	69	69	69	2x4	2x4	2x4	2x4	SPF	SPF
55	70	70	70	2x4	2x4	2x4	2x4	SPF	SPF
56	71	71	71	2x4	2x4	2x4	2x4	SPF	SPF
57	72	72	72	2x4	2x4	2x4	2x4	SPF	SPF
58	73	73	73	2x4	2x4	2x4	2x4	SPF	SPF
59	74	74	74	2x4	2x4	2x4	2x4	SPF	SPF
60	75	75	75	2x4	2x4	2x4	2x4	SPF	SPF
61	76	76	76	2x4	2x4	2x4	2x4	SPF	SPF
62	77	77	77	2x4	2x4	2x4	2x4	SPF	SPF
63	78	78	78	2x4	2x4	2x4	2x4	SPF	SPF
64	79	79	79	2x4	2x4	2x4	2x4	SPF	SPF
65	80	80	80	2x4	2x4	2x4	2x4	SPF	SPF
66	81	81	81	2x4	2x4	2x4	2x4	SPF	SPF
67	82	82	82	2x4	2x4	2x4	2x4	SPF	SPF
68	83	83	83	2x4	2x4	2x4	2x4	SPF	SPF
69	84	84	84	2x4	2x4	2x4	2x4	SPF	SPF
70	85	85	85	2x4	2x4	2x4	2x4	SPF	SPF
71	86	86	86	2x4	2x4	2x4	2x4	SPF	SPF
72	87	87	87	2x4	2x4	2x4	2x4	SPF	SPF
73	88	88	88	2x4	2x4	2x4	2x4	SPF	SPF
74	89	89	89	2x4	2x4	2x4	2x4	SPF	SPF
75	90	90	90	2x4	2x4	2x4	2x4	SPF	SPF
76	91	91	91	2x4	2x4	2x4	2x4	SPF	SPF
77	92	92	92	2x4	2x4	2x4	2x4	SPF	SPF
78	93	93	93	2x4	2x4	2x4	2x4	SPF	SPF
79	94	94	94	2x4	2x4	2x4	2x4	SPF	SPF
80	95	95	95	2x4	2x4	2x4	2x4	SPF	SPF
81	96	96	96	2x4	2x4	2x4	2x4	SPF	SPF
82	97	97	97	2x4	2x4	2x4	2x4	SPF	SPF
83	98	98	98	2x4	2x4	2x4	2x4	SPF	SPF
84	99	99	99	2x4	2x4	2x4	2x4	SPF	SPF
85	100	100	100	2x4	2x4	2x4	2x4	SPF	SPF
86	101	101	101	2x4	2x4	2x4	2x4	SPF	SPF
87	102	102	102	2x4	2x4	2x4	2x4	SPF	SPF
88	103	103	103	2x4	2x4	2x4	2x4	SPF	SPF
89	104	104	104	2x4	2x4	2x4	2x4	SPF	SPF
90	105	105	105	2x4	2x4	2x4	2x4	SPF	SPF
91	106	106	106	2x4	2x4	2x4	2x4	SPF	SPF
92	107	107	107	2x4	2x4	2x4	2x4	SPF	SPF
93	108	108	108	2x4	2x4	2x4	2x4	SPF	SPF
94	109	109	109	2x4	2x4	2x4	2x4	SPF	SPF
95	110	110	110	2x4	2x4	2x4	2x4	SPF	SPF
96	111	111	111	2x4	2x4	2x4	2x4	SPF	SPF
97	112	112	112	2x4	2x4	2x4	2x4	SPF	SPF
98	113	113	113	2x4	2x4	2x4	2x4	SPF	SPF
99	114	114	114	2x4	2x4	2x4	2x4	SPF	SPF
100	115	115	115	2x4	2x4	2x4	2x4	SPF	SPF

LUMBER		N.L.G.A. RULES		CHORDS		SIZE		DESCR.	
A	C	C	C	2x4	2x4	2x4	2x4	SPF	FACTORED
B	F	F	F	2x4	2x4	2x4	2x4	SPF	GROSS REACTION
C	H	H	H	2x4	2x4	2x4	2x4	SPF	GROSS REACTION
D	L	L	L	2x4	2x4	2x4	2x4	SPF	VERT. UP/LT
E	N	N	N	2x4	2x4	2x4	2x4	SPF	DOWN
F	O	O	O	2x4	2x4	2x4	2x4	SPF	1560F 1.5E
G	S	S	S	2x4	2x4	2x4	2x4	SPF	1560F 1.5E
H	T	T	T	2x4	2x4	2x4	2x4	SPF	RECORD
I	V	V	V	2x4	2x4	2x4	2x4	SPF	BRG
J	X	X	X	2x4	2x4	2x4	2x4	SPF	BRG
K	Z	Z	Z	2x4	2x4	2x4	2x4	SPF	BRG
L	1	1	1	2x4	2x4	2x4	2x4	SPF	BRG
M	2	2	2	2x4	2x4	2x4	2x4	SPF	BRG
N	3	3	3	2x4	2x4	2x4	2x4	SPF	BRG
O	4	4	4	2x4	2x4	2x4	2x4	SPF	BRG
P	5	5	5	2x4	2x4	2x4	2x4	SPF	BRG
Q	6	6	6	2x4	2x4	2x4	2x4	SPF	BRG
R	7	7	7	2x4	2x4	2x4	2x4	SPF	BRG
S	8	8	8	2x4	2x4	2x4	2x4	SPF	BRG
T	9	9	9	2x4	2x4	2x4	2x4	SPF	BRG
U	10	10	10	2x4	2x4	2x4	2x4	SPF	BRG
V	11	11	11	2x4	2x4	2x4	2x4	SPF	BRG
W	12	12	12	2x4	2x4	2x4	2x4	SPF	BRG
X	13	13	13	2x4	2x4	2x4	2x4	SPF	BRG
Y	14	14	14	2x4	2x4	2x4	2x4	SPF	BRG
Z	15	15	15	2x4	2x4	2x4	2x4	SPF	BRG
1	16	16	16	2x4	2x4	2x4	2x4	SPF	BRG
2	17	17	17	2x4	2x4	2x4	2x4	SPF	BRG

[illegible][illegible]

		THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2012, CBC 2012, ABC 2014 - CSA 086-09 - TPC 2011		DESIGN ASSUMPTIONS OVERHANG NOT TO BE ALTERED OR CUT OFF. 84 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFLECTION = L/360 (1.10") CALCULATED VERT. DEFLECTION = L/999 (0.36") ALLOWABLE DEFLECTION = L/360 (1.10")		CSL: TO=0.66 (F-H), BC=0.74 (S-T), (E-V), SM=0.22 (G-L)		CONTINUED ON PAGE 2	
0.50 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)		0.40 (1)	
0.40 (1)		0.40 (1)							

253643	H01T	1	1	TRUSS DESC.	DRWG NO.
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Alpe Roof Truss, Maple

ID:AvrJx9Y7aZEXF7DRQXSg4zMz1-D2AO9FJUIdWghpGVSWQ0bbpzZf6oX7dIU5NjWwEJC
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PLATES (table in inches)

PLATE	TYPE	W	LEN	Y	X
W	BMWW+1	MT20	4.0	6.0	3.00
X	BMV1+p	MT20	3.0	5.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11, AND 114.2 lbs FACTORED DOWN AT 30-9-6 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

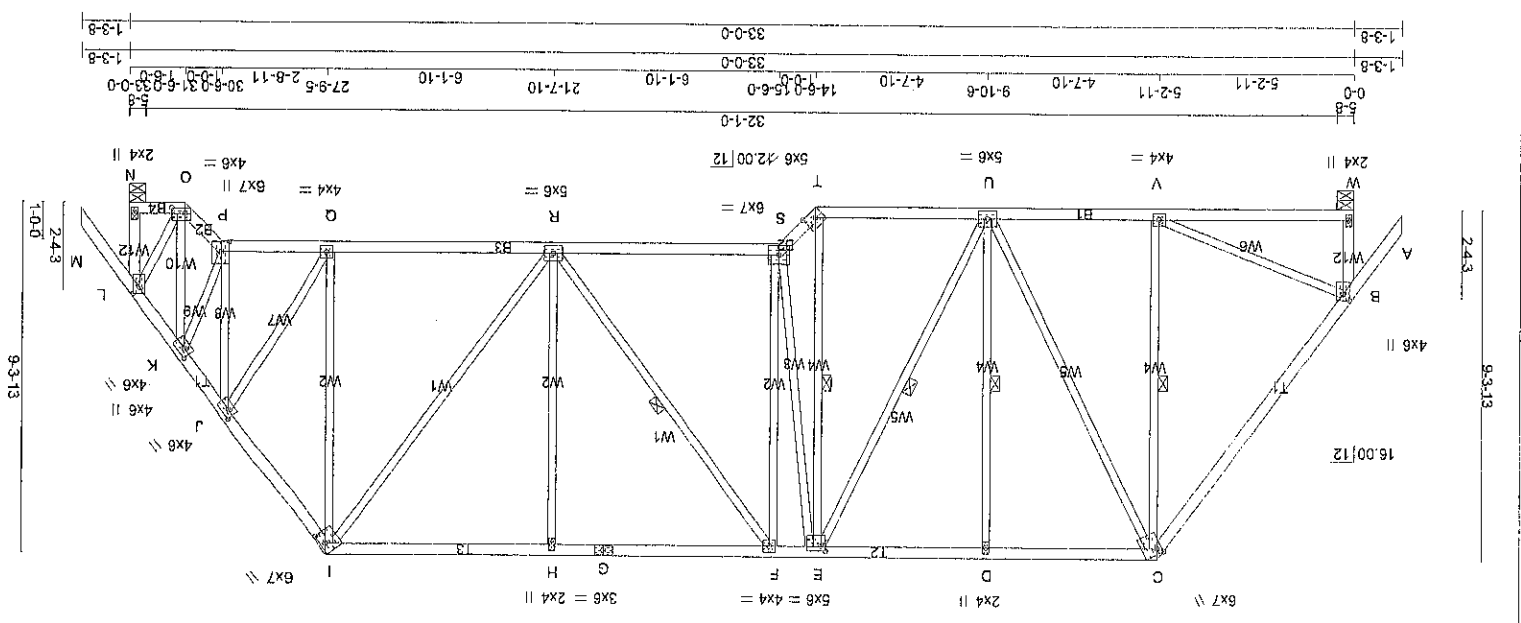
NOTES. (1)

1) Lateral brace(s) shown shall be 1x4 for Part B design as per OBC 9.23.13.11, and no less than 2x4 for Part A design.



DOL LUMBER=1.00 NAIL=1.00 L5 BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 0.50
AUTOSOLVE HEELS OFF
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PSI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.85 (R) (INPUT = 1.00)

A-11073885(2)



TOTAL WEIGHT = 192 lb

LUMBER		N.L.G.A. RULES		CHORDS		SIZE		DESCR.		LUMBER		DRY, SEASONED LUMBER.	
A	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	ALL WEBS	2x3
B	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	EXCEPT	2x3
C	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
D	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
E	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
F	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
G	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
H	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
I	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
J	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
K	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
L	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
M	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
N	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
O	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
P	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
Q	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
R	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
S	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
T	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
U	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
V	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
W	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
X	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
Y	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3
Z	C	C	C	2x4	2x4	2x4	2x4	SPF	SPF	No.2	SPF	DRY	2x3

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W/ N

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-V, D-U, E-U, E-T, F-R. THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

CHORDS MAX. FACTORED MEMB. FORCE VERT. LOAD C1 MAX (PLF) FROM TO

WEBS MAX. FACTORED MEMB. FORCE VERT. LOAD C1 MAX (PLF) FROM TO

ALLOWABLE DEFLECTION (L) = L/360 (1.10) CALCULATED VERT. DEFLECTION (L) = L/360 (0.09)

OSI: TC=0.48 (H-L), BC=0.33 (R-S), WB=0.71 (E-T), SSI=0.21 (H-L)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PL) (PL) (PL)

MAX MIN MAX MIN

MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (L) (INPUT = 0.90)

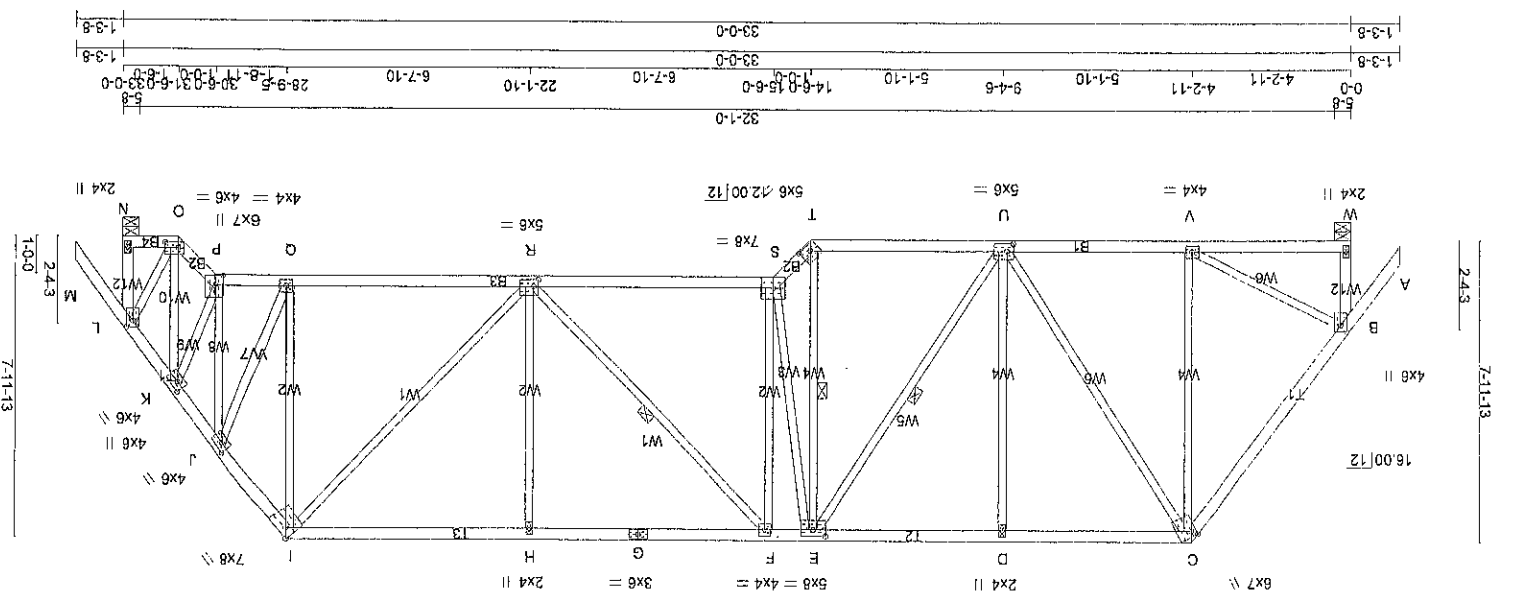
JSI METAL=1.00

4-N73883

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Job Name: 256342 Truss Name: H04T Quantity: 1 Job Desc: TRUSS DESC. Drawn No. 1

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TOTAL WEIGHT = 179 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, NBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(66% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFLECTION (L/360 (1.10"))
CALCULATED DEFLECTION (L/999 (0.11"))
ALLOWABLE DEFLECTION (L/360 (1.10"))

CALCULATED VERT. DEFLECTION (L/999 (0.23"))
ALLOWABLE DEFLECTION (L/360 (1.10"))

COR. TO = 5.9 (H-H), BC = 0.39 (R-S-1), WB = 0.59
(E-T-1), SS = 0.22 (H-H-1)

DOL LUMBER = 1.00 NAIL = 1.00 L. BEND = 1.10
COMP = 1.10 SHEAR = 1.10 TENS = 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
SHEAR SECTION
PLATE GRIP (DRY) (PSI)
MAX MIN MAX MIN
M20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.80 (L) (INPUT = 0.90)
JSI METAL = 0.37 (E) (INPUT = 1.00)

A-15073882

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

MAXIMUM FACTORED INPUT RECORD

GROSS REACTION DOWN HORIZ UPLIFT IN-SX

VERT HORIZ UPLIFT IN-SX

FACTORED REACTION

1ST CASE

UNFACTORED REACTIONS

SNOW COMBINED

LIVE PERM. LIVE

DEAD WIND

SOL

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) W. N

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-U, E-T, F-R

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN

THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

MEMB. FORCE

VERT. LOAD LC: MAX

CS (LC) UNBRAC

LENGTH FR-TO

FROM TO

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

K-L

L-M

M-N

N-O

O-P

P-Q

Q-R

R-S

S-T

T-U

U-V

V-W

W-X

X-Y

Y-Z

Z-A

A-B

B-C

C-D

D-E

E-F

F-G

G-H

LUMBER

N.L.G. A. RULES

CHORDS

A-C

C-G

G-I

I-M

M-W

W-B

B-N

N-L

L-T

T-S

S-P

P-O

O-M

M-2x4

2x4

DRY

No. 2

ALL WEBS

EXCEPT

2x3

DRY

No. 2

SPF

ALL WEBS

EXCEPT

2x3

DRY

No. 2

SPF

ALL WEBS

EXCEPT

2x3

DRY

No. 2

SPF

ALL WEBS

EXCEPT

2x3

DRY

No. 2

SPF

ALL WEBS

EXCEPT

2x3

DRY

No. 2

SPF

ALL WEBS

EXCEPT

2x3

DRY

No. 2

SPF

ALL WEBS

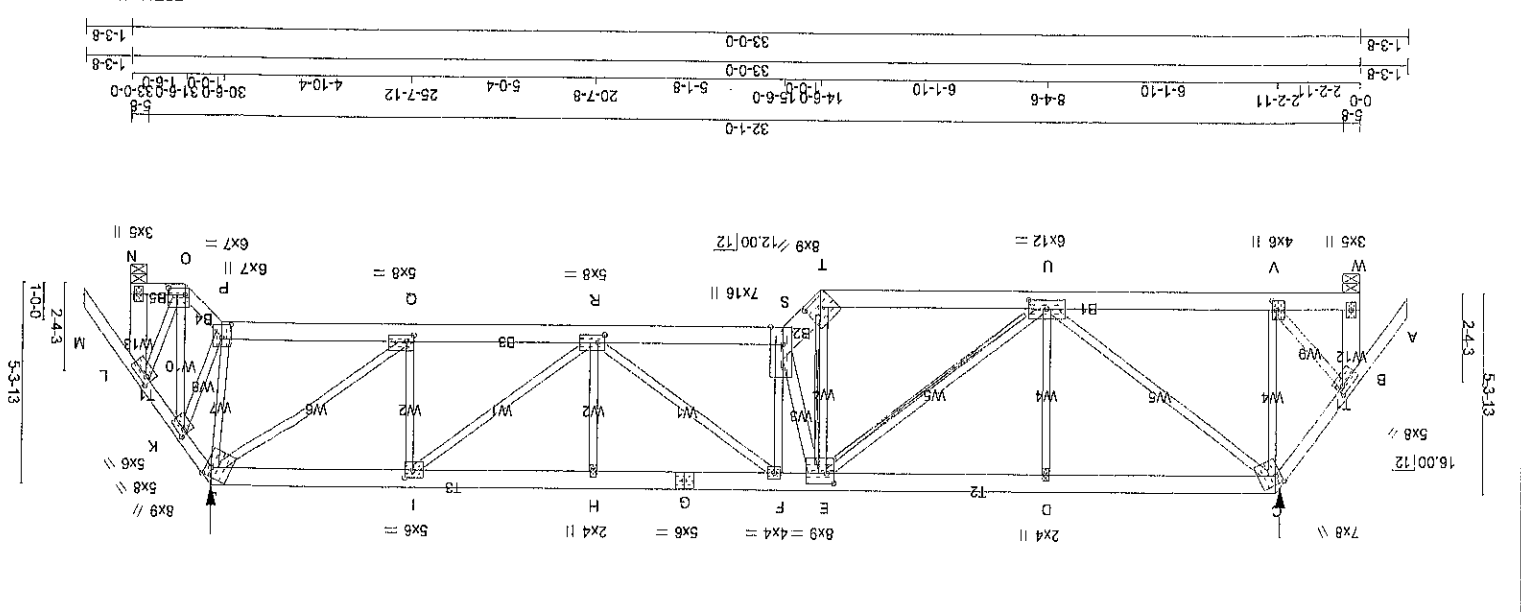
EXCEPT

2x3

DRY

No. 2



[illegible]

PLATES (table is in inches)	W	LEN	Y	X
2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U	5.0	8.0	2.00	1.75
FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"	5.0	8.0	2.00	1.75
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH	2.0	4.0	2.0	4.0
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW	2.0	4.0	2.0	4.0
19-1	2.0	4.0	2.0	4.0
18-1	2.0	4.0	2.0	4.0
17-1	2.0	4.0	2.0	4.0
16-1	2.0	4.0	2.0	4.0
15-1	2.0	4.0	2.0	4.0
14-1	2.0	4.0	2.0	4.0
13-1	2.0	4.0	2.0	4.0
12-1	2.0	4.0	2.0	4.0
11-1	2.0	4.0	2.0	4.0
10-1	2.0	4.0	2.0	4.0
9-1	2.0	4.0	2.0	4.0
8-1	2.0	4.0	2.0	4.0
7-1	2.0	4.0	2.0	4.0
6-1	2.0	4.0	2.0	4.0
5-1	2.0	4.0	2.0	4.0
4-1	2.0	4.0	2.0	4.0
3-1	2.0	4.0	2.0	4.0
2-1	2.0	4.0	2.0	4.0
1-1	2.0	4.0	2.0	4.0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

19-1

18-1

17-1

16-1

15-1

14-1

13-1

12-1

11-1

10-1

9-1

8-1

7-1

6-1

5-1

4-1

3-1

2-1

1-1

PLATES (table is in inches)

W

LEN

Y

X

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

19-1

18-1

17-1

16-1

15-1

14-1

13-1

12-1

11-1

10-1

9-1

8-1

7-1

6-1

5-1

4-1

3-1

2-1

1-1

PLATES (table is in inches)

W

LEN

Y

X

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

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19-1

18-1

17-1

16-1

15-1

14-1

13-1

12-1

11-1

10-1

9-1

8-1

7-1

6-1

5-1

4-1

3-1

2-1

1-1

PLATES (table is in inches)

W

LEN

Y

X

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH

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19-1

18-1

17-1

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15-1

14-1

13-1

12-1

11-1

10-1

9-1

8-1

7-1

6-1

5-1

4-1

3-1

2-1

1-1

PLATES (table is in inches)

W

LEN

Y

X

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

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19-1

18-1

17-1

16-1

15-1

14-1

13-1

12-1

11-1

10-1

9-1

8-1

7-1

6-1

5-1

4-1

3-1

2-1

1-1

PLATES (table is in inches)

W

LEN

Y

X

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

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16-1

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12-1

11-1

10-1

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7-1

6-1

5-1

4-1

3-1

2-1

1-1

PLATES (table is in inches)

W

LEN

Y

X

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH

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9-1

8-1

7-1

6-1

5-1

4-1

3-1

2-1

1-1

PLATES (table is in inches)

W

LEN

Y

X

2x4 DRY SPF No.2 1BRACE REQUIRED AT E-U

FASTEN T AND 1BRACE TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH

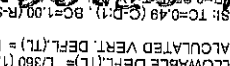
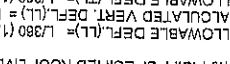
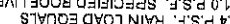
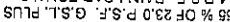
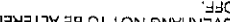
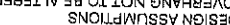
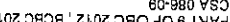
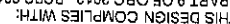
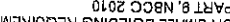
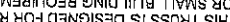
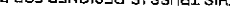
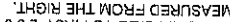
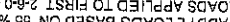
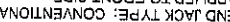
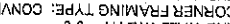
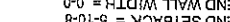
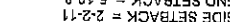
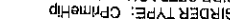
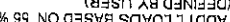
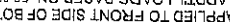
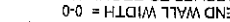
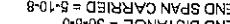
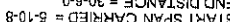
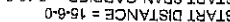
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

19-1

18-1

17-1

16-1

																																																																		
--	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	---	--	--

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRAWG NO.
253642	H01T	1	1			

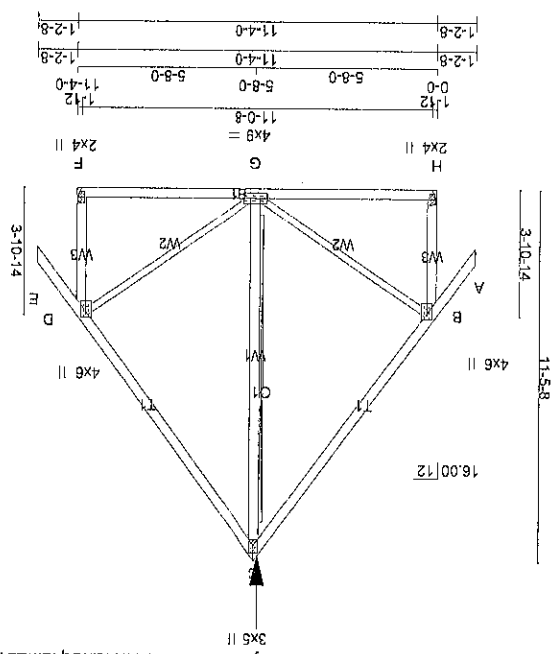
DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00
 COMPANION LIVE LOAD FACTOR = 0.50
 AUTOSOLVE HEELS OFF
 TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL IN
 THE TRUSS MANUFACTURING PLANT.
 NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI)
 MAX MIN MAX MIN
 MT20 818 364 1667 822 2284 1656
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 USI GRIP= 0.90 (Q) (INPUT = 0.90)
 USI METAL= 0.78 (E) (INPUT = 1.00)

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.
HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S)
 REQUIRED TO SUPPORT CONCENTRATED
 LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11.
 AND 114.2 lbs FACTORED DOWN AT 30-9-6 ON
 TOP CHORD. DESIGN FOR UNSPECIFIED
 CONNECTION(S) IS DELEGATED TO THE
 BUILDING DESIGNER.
NOTES - (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per
 OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



A-15073880(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG. NO.
253641	H15	2	1	TRUSS DESC.	



TOTAL WEIGHT = 2 X 70 = 141 lb

SPECIFIED LOADS:	
TOP CH.	LL = 21.0 PSF
DL	= 3.0 PSF
BOT CH.	LL = 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 31.0 PSF
SPACING = 24.0 IN./C	
GIRDER TYPE: CPMHHP	
SILE STRUCK: 2 x 8	

END SETBACK = 6'-0"
CORNER FRAMING TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
-ADPT. LOADS BASED ON 56 % OF GSI.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCO 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BOBC 2012, ABC 2014
- CSA 086-09
- TPI-C 2011

6.64 % OF Z33 P.S.F. R55 P.S.F. G.S.L. PLUS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEF. (LL) = L/850 (0.38")
CALCULATED VERT. DEF. (LL) = L/998 (0.01")
ALLOWABLE DEF. (TL) = L/860 (0.37")
CALCULATED VERT. DEF. (TL) = L/989 (0.04")
CS1: TC=0.48 (C-D-1), BC=0.34 (G-H-4),
WB=0.13 (C-G-1), SS=0.18 (F-G-3)
DOL NUMBER=1.00 NAL=1.00LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

SECTION	GRIP (D)	SHEAR	(PL)	(PL)
MT20	MAX MIN	618 354	1667 622	2264 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 6.0 Deg.

JSI GRIP = 0.52 (B) (INPUT = 0.90)
JSI METAL = 0.16 (F) (INPUT = 1.00)

FACTORED		GROSS REACTION		VERT. HORZ.		DOWN. HORZ. UPLIFT		BRG. IN-SX		MIN. SEAT SIZE: 1-8		HANGER BY OTHERS		MIN. SEAT SIZE: 1-8	
MAXIMUM FACTORED	INPUT	RECORD	BRG	IN-SX	BRG	IN-SX	DOWN	HORZ.	UPLIFT	IN-SX	BRG	IN-SX	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8	HANGER BY OTHERS
960	0	960	0	960	0	0	0	0	0	0	0	0	0	0	0
UNFACTORED REACTIONS															
1ST CASE															
MAX./MIN. COMPONENT REACTIONS															
COMBINED		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOL.		0/0	
676	468/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	218/0	0/0	0/0	0/0	0/0
676	468/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	218/0	0/0	0/0	0/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.
4x4 DRY SPP No.2 T-BRACE REQUIRED AT C-G
EASTERN 1 AND L-BRACES TO NARROW EDGE OF WFB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW
AND VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN

[illegible]

DESCR.	LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
SPF	2x4	A - C	4 - 6	2x4	DRY
SPF	2x4	C - E	6 - 8	2x4	DRY
SPF	2x4	E - B	8 - 10	2x4	DRY
SPF	2x4	B - F	10 - 12	2x4	DRY
SPF	2x4	F - D	12 - 14	2x4	DRY
SPF	2x4	D - G	14 - 16	2x4	DRY
SPF	2x3	EXCEPT		2x3	DRY
SPF	2x4	ALL WEBS		2x4	DRY
SPF	No.2			No.2	DRY
SPF	No.2			No.2	DRY
SPF	No.2			No.2	DRY

PLATES, (table is in inches)			
JT	TYPE	PLATES	W
B	T4WV+p	MT20	4.0
C	TTW+p	MT20	3.0
D	T4WV+p	MT20	4.0
F	B4WV+p	MT20	2.0
G	B4WVW+p	MT20	4.0
H	B4WV+p	MT20	2.0

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

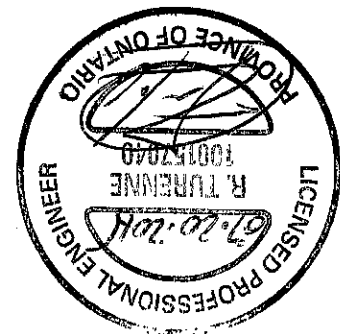
NOTES- (1)



DESCR.	LUMBER	N L G A RULES	CHORDS	SIZE	LUMBER
SPF	DRY	C	A	C	SPF
SPF	DRY	E	C	E	SPF
SPF	DRY	B	F	B	SPF
SPF	DRY	D	F	D	SPF
SPF	DRY	G	F	G	SPF
SPF	DRY	F	F	F	SPF
SPF	2x3	ALL WEBS	EXCEPT		SPF
No.2	DRY				No.2

PLATE TYPE	PLATES (table in inches)	Edge - INDICATES REFERENCE CORNER OF PLATE	TOUCHES EDGE OF CHORD.
A	W	3.0 6.0	Edge 0.25
B	LEN Y	3.0 6.0	Edge 0.25
C	X	3.0 6.0	Edge 0.25
D		3.0 6.0	Edge 0.25
E		3.0 6.0	Edge 0.25
F		3.0 6.0	Edge 0.25
G		3.0 6.0	Edge 0.25
H		3.0 6.0	Edge 0.25
I		3.0 6.0	Edge 0.25
J		3.0 6.0	Edge 0.25
K		3.0 6.0	Edge 0.25
L		3.0 6.0	Edge 0.25
M		3.0 6.0	Edge 0.25
N		3.0 6.0	Edge 0.25
O		3.0 6.0	Edge 0.25
P		3.0 6.0	Edge 0.25
Q		3.0 6.0	Edge 0.25
R		3.0 6.0	Edge 0.25
S		3.0 6.0	Edge 0.25
T		3.0 6.0	Edge 0.25
U		3.0 6.0	Edge 0.25
V		3.0 6.0	Edge 0.25
W		3.0 6.0	Edge 0.25
X		3.0 6.0	Edge 0.25
Y		3.0 6.0	Edge 0.25
Z		3.0 6.0	Edge 0.25
AA		3.0 6.0	Edge 0.25
AB		3.0 6.0	Edge 0.25
AC		3.0 6.0	Edge 0.25
AD		3.0 6.0	Edge 0.25
AE		3.0 6.0	Edge 0.25
AF		3.0 6.0	Edge 0.25
AG		3.0 6.0	Edge 0.25
AH		3.0 6.0	Edge 0.25
AI		3.0 6.0	Edge 0.25
AJ		3.0 6.0	Edge 0.25
AK		3.0 6.0	Edge 0.25
AL		3.0 6.0	Edge 0.25
AM		3.0 6.0	Edge 0.25
AN		3.0 6.0	Edge 0.25
AO		3.0 6.0	Edge 0.25
AP		3.0 6.0	Edge 0.25
AQ		3.0 6.0	Edge 0.25
AR		3.0 6.0	Edge 0.25
AS		3.0 6.0	Edge 0.25
AT		3.0 6.0	Edge 0.25
AU		3.0 6.0	Edge 0.25
AV		3.0 6.0	Edge 0.25
AW		3.0 6.0	Edge 0.25
AX		3.0 6.0	Edge 0.25
AY		3.0 6.0	Edge 0.25
AZ		3.0 6.0	Edge 0.25
BA		3.0 6.0	Edge 0.25
BB		3.0 6.0	Edge 0.25
BC		3.0 6.0	Edge 0.25
BD		3.0 6.0	Edge 0.25
BE		3.0 6.0	Edge 0.25
BF		3.0 6.0	Edge 0.25
BG		3.0 6.0	Edge 0.25
BH		3.0 6.0	Edge 0.25
BI		3.0 6.0	Edge 0.25
BJ		3.0 6.0	Edge 0.25
BK		3.0 6.0	Edge 0.25
BL		3.0 6.0	Edge 0.25
BM		3.0 6.0	Edge 0.25
BN		3.0 6.0	Edge 0.25
BO		3.0 6.0	Edge 0.25
BP		3.0 6.0	Edge 0.25
BQ		3.0 6.0	Edge 0.25
BR		3.0 6.0	Edge 0.25
BS		3.0 6.0	Edge 0.25
BT		3.0 6.0	Edge 0.25
BU		3.0 6.0	Edge 0.25
BV		3.0 6.0	Edge 0.25
BW		3.0 6.0	Edge 0.25
BX		3.0 6.0	Edge 0.25
BY		3.0 6.0	Edge 0.25
BZ		3.0 6.0	Edge 0.25
CA		3.0 6.0	Edge 0.25
CB		3.0 6.0	Edge 0.25
CC		3.0 6.0	Edge 0.25
CD		3.0 6.0	Edge 0.25
CE		3.0 6.0	Edge 0.25
CF		3.0 6.0	Edge 0.25
CG		3.0 6.0	Edge 0.25
CH		3.0 6.0	Edge 0.25
CI		3.0 6.0	Edge 0.25
CJ		3.0 6.0	Edge 0.25
CK		3.0 6.0	Edge 0.25
CL		3.0 6.0	Edge 0.25
CM		3.0 6.0	Edge 0.25
CN		3.0 6.0	Edge 0

NOTES: (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



BUILDING DESIGNER									
DIMENSIONS, SUPPORTS									
AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY									
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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BRACING
TOP CHORD TO BE SHEAR
MAX. UNBRACED BOTTOM
APPLIED.
LOADING
TOTAL LOAD CASES: (4)

BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)
BEARING SIZE FACTOR = 1.16 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.26FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED.

[illegible]

Category	Value	Value	Value	Value	Value
A	0.00	-17.5	-17.5	0.00	0.00
B	0.00	-17.5	-17.5	0.00	0.00
C	0.00	-17.5	-17.5	0.00	0.00
D	0.00	-17.5	-17.5	0.00	0.00
E	0.00	-17.5	-17.5	0.00	0.00
F	0.00	-17.5	-17.5	0.00	0.00
G	0.00	-17.5	-17.5	0.00	0.00
H	0.00	-17.5	-17.5	0.00	0.00
I	0.00	-17.5	-17.5	0.00	0.00
J	0.00	-17.5	-17.5	0.00	0.00
K	0.00	-17.5	-17.5	0.00	0.00
L	0.00	-17.5	-17.5	0.00	0.00
M	0.00	-17.5	-17.5	0.00	0.00
N	0.00	-17.5	-17.5	0.00	0.00
O	0.00	-17.5	-17.5	0.00	0.00
P	0.00	-17.5	-17.5	0.00	0.00
Q	0.00	-17.5	-17.5	0.00	0.00
R	0.00	-17.5	-17.5	0.00	0.00
S	0.00	-17.5	-17.5	0.00	0.00
T	0.00	-17.5	-17.5	0.00	0.00
U	0.00	-17.5	-17.5	0.00	0.00
V	0.00	-17.5	-17.5	0.00	0.00
W	0.00	-17.5	-17.5	0.00	0.00
X	0.00	-17.5	-17.5	0.00	0.00
Y	0.00	-17.5	-17.5	0.00	0.00
Z	0.00	-17.5	-17.5	0.00	0.00

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBCS 2012, ABC 2014

- CSA 086-09

- TPC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD ROOF LOAD)

ALLOWABLE DEFLECTION = $L/360 (0.367)$

CALCULATED VERT. DEFLECTION = $L/998 (0.017)$

CALCULATED DEFLECTION = $L/760 (0.267)$

CALCULATED VERT. DEFLECTION = $L/998 (0.067)$

CSL: $TC = 0.27$ (C-D-1), $BC = 0.16$ (F-G-4),
WB = 0.10 (C-G-1), $SS = 0.08$ (B-C-1),
DOL LUMBER = 1.00, NAIL = 1.00, TENS = 1.10
COMP = 1.10, SHEAR = 1.10, STRESS = 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

SECTION (Psi)
MAX MIN MAX MIN
PLATE GRIP (DRY) SHEAR (Psi)
MT20 618 364 1667 622 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

ISI GRIP = 0.65 (C) (INPUT = 0.90)
ISI METAL = 0.12 (B) (INPUT = 1.00)

PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP = 0.65 (C) (INPUT = 0.90)
 JSI METAL = 0.12 (B) (INPUT = 1.00)

PLATE PLACEMENT TOL. = 0.250 inches

PLATE GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284	1656

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

AUTOSOLVE HEELS OFF

COMPANION LIVE LOAD FACTOR = 0.50

COMP=1.10 SHEAR=1.10 TENS=1.10

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=

CSI: $IC=0.27$ (C-D:1), $BC=0.18$ (F-G:4),
WB=0.10 (C-G:1), $SSI=0.08$ (B-C:1)

ORGANIZED BY: DEPT. OF HEALTH & HUMAN SERVICES

$$\text{ALLOWABLE DEF. (TL)} = 1/360 (0.36")$$
$$\text{ALLOWABLE DEF. (LL)} = L/360 (0.36")$$

21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

(66% OF 23.0 P.S.F., G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS

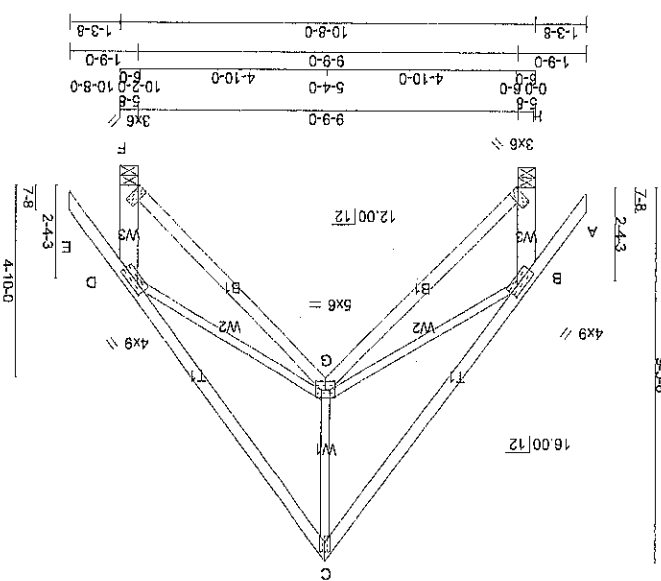
- IPIC 2014

-PART 9 OF CBC 2012, CBC 2012, ABC 2014
-CSA 086-09
-TBS 2014

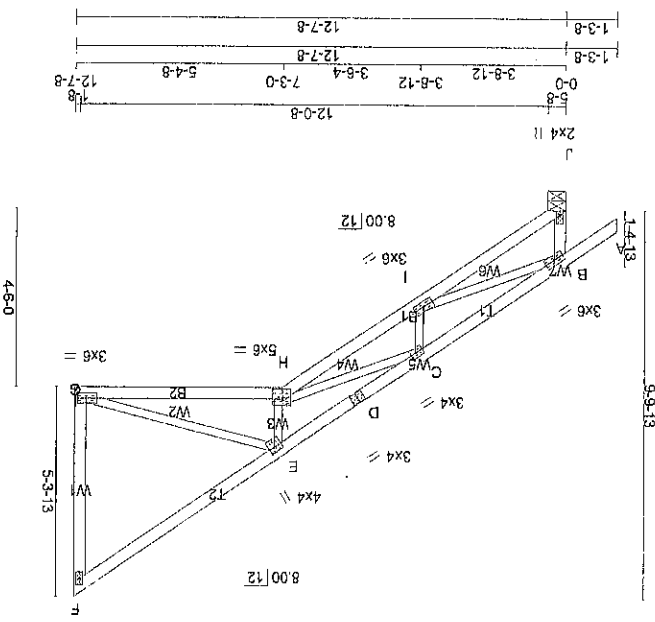
THIS DESIGN COMPLIES WITH:
FEDERAL OF ORC 2013, B000 2013, B000 2014, B000 2015, B000 2016, B000 2017, B000 2018, B000 2019, B000 2020, B000 2021, B000 2022, B000 2023, B000 2024, B000 2025, B000 2026, B000 2027, B000 2028, B000 2029, B000 2030, B000 2031, B000 2032, B000 2033, B000 2034, B000 2035, B000 2036, B000 2037, B000 2038, B000 2039, B000 2040, B000 2041, B000 2042, B000 2043, B000 2044, B000 2045, B000 2046, B000 2047, B000 2048, B000 2049, B000 2050, B000 2051, B000 2052, B000 2053, B000 2054, B000 2055, B000 2056, B000 2057, B000 2058, B000 2059, B000 2060, B000 2061, B000 2062, B000 2063, B000 2064, B000 2065, B000 2066, B000 2067, B000 2068, B000 2069, B000 2070, B000 2071, B000 2072, B000 2073, B000 2074, B000 2075, B000 2076, B000 2077, B000 2078, B000 2079, B000 2080, B000 2081, B000 2082, B000 2083, B000 2084, B000 2085, B000 2086, B000 2087, B000 2088, B000 2089, B000 2090, B000 2091, B000 2092, B000 2093, B000 2094, B000 2095, B000 2096, B000 2097, B000 2098, B000 2099, B000 2100, B000 2101, B000 2102, B000 2103, B000 2104, B000 2105, B000 2106, B000 2107, B000 2108, B000 2109, B000 2110, B000 2111, B000 2112, B000 2113, B000 2114, B000 2115, B000 2116, B000 2117, B000 2118, B000 2119, B000 2120, B000 2121, B000 2122, B000 2123, B000 2124, B000 2125, B000 2126, B000 2127, B000 2128, B000 2129, B000 2130, B000 2131, B000 2132, B000 2133, B000 2134, B000 2135, B000 2136, B000 2137, B000 2138, B000 2139, B000 2140, B000 2141, B000 2142, B000 2143, B000 2144, B000 2145, B000 2146, B000 2147, B000 2148, B000 2149, B000 2150, B000 2151, B000 2152, B000 2153, B000 2154, B000 2155, B000 2156, B000 2157, B000 2158, B000 2159, B000 2160, B000 2161, B000 2162, B000 2163, B000 2164, B000 2165, B000 2166, B000 2167, B000 2168, B000 2169, B000 2170, B000 2171, B000 2172, B000 2173, B000 2174, B000 2175, B000 2176, B000 2177, B000 2178, B000 2179, B000 2180, B000 2181, B000 2182, B000 2183, B000 2184, B000 2185, B000 2186, B000 2187, B000 2188, B000 2189, B000 2190, B000 2191, B000 2192, B000 2193, B000 2194, B000 2195, B000 2196, B000 2197, B000 2198, B000 2199, B000 2200, B000 2201, B000 2202, B000 2203, B000 2204, B000 2205, B000 2206, B000 2207, B000 2208, B000 2209, B000 2210, B000 2211, B000 2212, B000 2213, B000 2214, B000 2215, B000 2216, B000 2217, B000 2218, B000 2219, B000 2220, B000 2221, B000 2222, B000 2223, B000 2224, B000 2225, B000 2226, B000 2227, B000 2228, B000 2229, B000 2230, B000 2231, B000 2232, B000 2233, B000 2234, B000 2235, B000 2236, B000 2237, B000 2238, B000 2239, B000 2240, B000 2241, B000 2242, B000 2243, B000 2244, B000 2245, B000 2246, B000 2247, B000 2248, B000 2249, B000 2250, B000 2251, B000 2252, B000 2253, B000 2254, B000 2255, B000 2256, B000 2257, B000 2258, B000 2259, B000 2260, B000 2261, B000 2262, B000 2263, B000 2264, B000 2265, B000 2266, B000 2267, B000 2268, B000 2269, B000 2270, B000 2271, B000 2272, B000 2273, B000 2274, B000 2275, B000 2276, B000 2277, B000 2278, B000 2279, B000 2280, B000 2281, B000 2282, B000 2283, B000 2284, B000 2285, B000 2286, B000 2287, B000 2288, B000 2289, B000 2290, B000 2291, B000 2292, B000 2293, B000 2294, B000 2295, B000 2296, B000 2297, B000 2298, B000 2299, B000 2300, B000 2301, B000 2302, B000 2303, B000 2304, B000 2305, B000 2306, B000 2307, B000 2308, B000 2309, B000 2310, B000 2311, B000 2312, B000 2313, B000 2314, B000 2315, B000 2316, B000 2317, B000 2318, B000 2319, B000 2320, B000 2321, B000 2322, B000 2323, B000 2324, B000 2325, B000 2326, B000 2327, B000 2328, B000 2329, B000 2330, B000 2331, B000 2332, B000 2333, B000 2334, B000 2335, B000 2336, B000 2337, B000 2338, B000 2339, B000 2340, B000 2341, B000 2342, B000 2343, B000 2344, B000 2345, B000 2346, B000 2347, B000 2348, B000 2349, B000 2350, B000 2351, B000 2352, B000 2353, B000 2354, B000 2355, B000 2356, B000 2357, B000 2358, B000 2359, B000 2360, B000 2361, B000 2362, B000 2363, B000 2364, B000 2365, B000 2366, B000 2367, B000 2368, B000 2369, B000 2370, B000 2371, B000 2372, B000 2373, B000 2374, B000 2375, B000 2376, B000 2377, B000 2378, B000 2379, B000 2380, B000 2381, B000 2382, B000 2383, B000 2384, B000 2385, B000 2386, B000 2387, B000 2388, B000 2389, B000 2390, B000 2391, B000 2392, B000 2393, B000 2394, B000 2395, B000 2396, B000 2397, B000 2398, B000 2399, B000 2400, B000 2401, B000 2402, B000 2403, B000 2404, B000 2405, B000 2406, B000 2407, B000 2408, B000 2409, B000 2410, B000 2411, B000 2412, B000 2413, B000 2414, B000 2415, B000 2416, B000 2417, B000 2418, B000 2419, B000

PART 9, NBCC 2010

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

$$\overline{\text{SPACING}} = \overline{24.0} \quad \overline{\text{IN. C/C}}$$


A-1073877



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
J - B	2X4	DRY	No.2	SPP	
A - D	2X4	DRY	No.2	SPP	
G - F	2X4	DRY	No.2	SPP	
J - H	2X4	DRY	No.2	SPP	
H - G	2X4	DRY	No.2	SPP	
ALL WEBS	2X3	DRY	No.2	SPP	
DRY; SEASONED LUMBER					

PLATES (tablets in inches)	JT	TYPE	W	LEN	Y	X
C	B	TN/W-4	MT20	3.0	6.0	
C	B	TN/W-1	MT20	3.0	4.0	1.50
D	1	T-4	MT20	3.0	4.0	1.50
E	D	TN/W-1	MT20	4.0	4.0	1.50
F	G	TN/W-4	MT20	2.0	4.0	1.50
G	H	BM/W-1	MT20	3.0	6.0	2.50
H	I	BM/W-4	MT20	3.0	6.0	3.00
J	J	BM/W-4	MT20	2.0	4.0	Edge

NOTES: (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

[illegible]

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGN CRITERIA

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBCS 2012

- CSA 086-08

- T-PIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 2.4 P.S.F. RAIN LOAD EQUALS ALLOWABLE DEFLL.(L) = L/360 (0.42")

CALCULATED DEFLL.(L) = L/999 (0.07")

ALLOWABLE DEFLL.(L) = L/999 (0.12")

COSI: C=0.24 (E-F:1), BC=0.29 (G-H:1), WB=0.62 (E-G:1), SSI=0.14 (E-F:1)

DOL NUMBER=1.00 NAIL=1.10 ST BEND=1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GMP(DRY) SHEAR SECTION

(PSI) (PL)

MT20 618 364 1067 822 2284 1656

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (IN/PT = 0.80)

JSI METAL = 0.47 (E) (IN/PT = 1.00)

TOTAL WEIGHT = 54 lb (M/G)

A-1273875

LUMBER	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER	DESIGN CONTRACT
TOTAL WEIGHT = 2 X 70 = 141 lb		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

J	BMW11-MT20	3.0	4.0	BRACING TOP CHORD TO BE SHEATHED OR MAX. BRUIN SPACING - 6.065T (55% OF 23.0 P.S.F. G.S.L. PLUS 4 P.S.F. BRUIN SPACING)
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DO NOT SCALE FROM THIS DRAWING. TOTAL LOAD CASES: (4)

H-F	0.64	-17.5	-17.5	0.20	(4)	10.00
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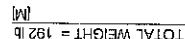
PLATE PLACEMENT TOL. = 0.250 inches

07.10.2014

CONFIDENTIAL

 PROVINCE OF ONTARIO
 MINISTRY OF REVENUE AND FINANCE
 100 KING STREET WEST, 10TH FLOOR, TORONTO, ONTARIO M5X 1C3
 TEL: (416) 325-2600 FAX: (416) 325-2601
 WWW.ONTARIO.GOV.CA

26022017	
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PROVINCE OF ONTARIO
100157040
R. TURENNE
07.20.2011
LICENSED PROFESSIONAL ENGINEER

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

(M)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253641	H01-Cond2	1	1	TRUSS DESC.	

PLATES (table is in inches)

JT TYPE	W	LEN	Y	X
W BMWV+1	MT20	5.0	8.0	4.00 2.00
X BMWV+1	MT20	5.0	9.0	5.50
Y BMWV+1	MT20	6.0	9.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-11 ON TOP CHORD, AND 1293.0 lbs FACTORED DOWN AT 3-3-8, AND 1293.0 lbs FACTORED DOWN AT 14-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

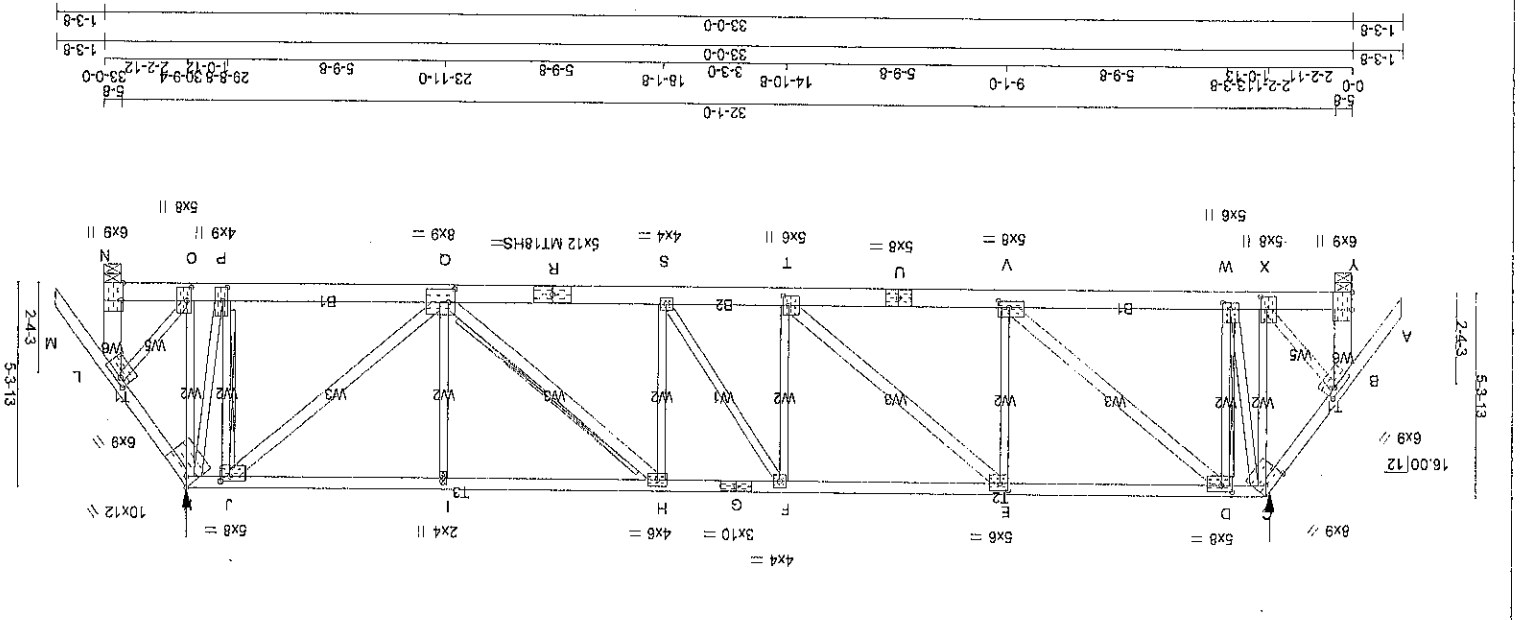
NOTES- (1)

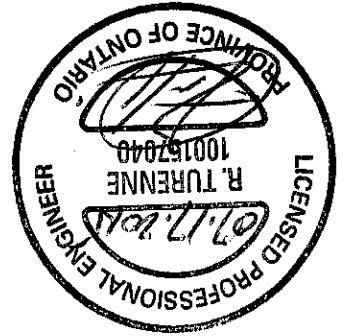
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per CBC 9.23.13.11, and no less than 2x4 for Part 4 design.



COMPANION LIVE LOAD FACTOR = 0.60
 AUTOSOLVE HEELS OFF
 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
 NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 818 364 1667 822 2284 1656
 MT18HS 580 364 2729 1406 4191 2010
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 6.0 Deg.
 JSI GRIP = 0.89 (V) (INPUT = 0.90)
 JSI METAL = 0.92 (U) (INPUT = 1.00)

A-H073872(2)

[illegible][illegible][illegible]



NOTES: (1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.2.3, 13.11, and no less than 2x4 for Part 4 design.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	4.0	6.0	1.75	3.00
B	4.0	6.0	1.50	1.50
C	3.0	4.0	4.0	4.0
D	2.0	4.0	4.0	4.0
E	3.0	4.0	4.0	4.0
F	3.0	4.0	4.0	4.0
G	4.0	6.0	3.00	3.00
H	2.0	4.0	4.0	4.0
BMV1+p	2.0	4.0	4.0	4.0

DRY, SEASONED LUMBER.

ALL WEBS	2x3	DRY	No.2	SPP
F - E	2x4	DRY	No.2	SPP
G - F	2x4	DRY	No.2	SPP
H - G	2x4	DRY	No.2	SPP
E - D	2x4	DRY	No.2	SPP
A - D	2x4	DRY	No.2	SPP
H - B	2x4	DRY	No.2	SPP
H - C	2x4	DRY	No.2	SPP

CHORDS

CHORDS	W	LEN	Y	X
JT TYPE	4.0	6.0	1.75	3.00
B	4.0	6.0	1.50	1.50
C	3.0	4.0	4.0	4.0
D	2.0	4.0	4.0	4.0
E	3.0	4.0	4.0	4.0
F	3.0	4.0	4.0	4.0
G	4.0	6.0	3.00	3.00
H	2.0	4.0	4.0	4.0
BMV1+p	2.0	4.0	4.0	4.0

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO	-342/0	0.0	0.0	0.0
H-B	-287/0	-70.5	-70.5	-70.5
A-B	-287/0	-70.5	-70.5	-70.5
B-C	-12/0	-70.5	-70.5	-70.5
C-D	-93/0	0.0	0.0	0.0
E-D	0/0	-17.5	-17.5	-17.5
G-F	0/20	-17.5	-17.5	-17.5
F-E	0/248	-17.5	-17.5	-17.5

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO	-342/0	0.0	0.0	0.0
H-B	-287/0	-70.5	-70.5	-70.5
A-B	-287/0	-70.5	-70.5	-70.5
B-C	-12/0	-70.5	-70.5	-70.5
C-D	-93/0	0.0	0.0	0.0
E-D	0/0	-17.5	-17.5	-17.5
G-F	0/20	-17.5	-17.5	-17.5
F-E	0/248	-17.5	-17.5	-17.5

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

1ST CASE	MAX. MIN. COMPONENT REACTIONS	FACTORED	MAX. FACTORED	INPUT	RECORD
JT	COMBINED SNOW	181/0	0/0	0/0	0/0
H	248	181/0	0/0	0/0	0/0
E	182	123/0	0/0	0/0	0/0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	FACTORED	MAX. FACTORED	INPUT	RECORD
JT	COMBINED SNOW	181/0	0/0	0/0	0/0
H	248	181/0	0/0	0/0	0/0
E	182	123/0	0/0	0/0	0/0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	21.0	PSF
BOT CH.	LL	0.0	PSF
DL		7.0	PSF
TOTAL LOAD		31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- CSA 086-09
- PART 9 OF OBC 2012, BCB 2012, ABC 2014
- TPC 2011

6.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFLECTION (LL) = L/360 (0.20")

ALLOWABLE DEFLECTION (TL) = L/360 (0.20")

CALCULATED VERT. DEFLECTION (LL) = L/999 (0.00")

CALCULATED VERT. DEFLECTION (TL) = L/999 (0.01")

CSL: TC=0.10 (B-C-1), BC=0.08 (E-F-4), WB=0.07 (C-E-1), SSI=0.09 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (OR) SHEAR SECTION (PSI) (PLI)

MAX MIN MAX MIN

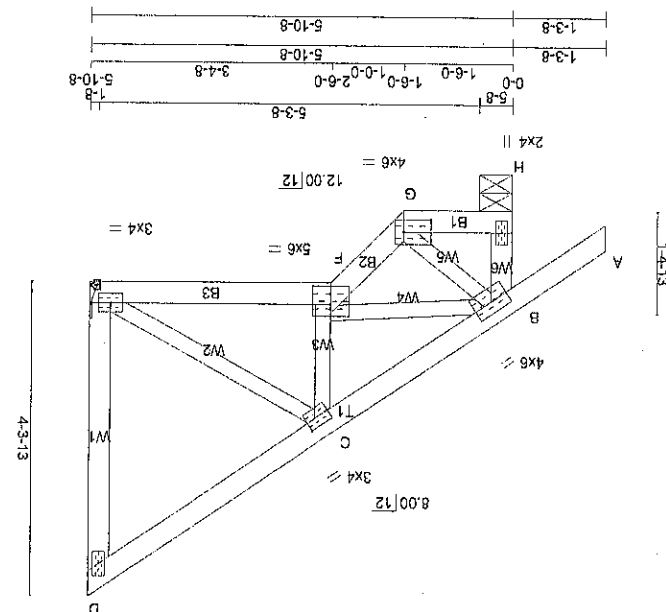
MT20 618 384 1887 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

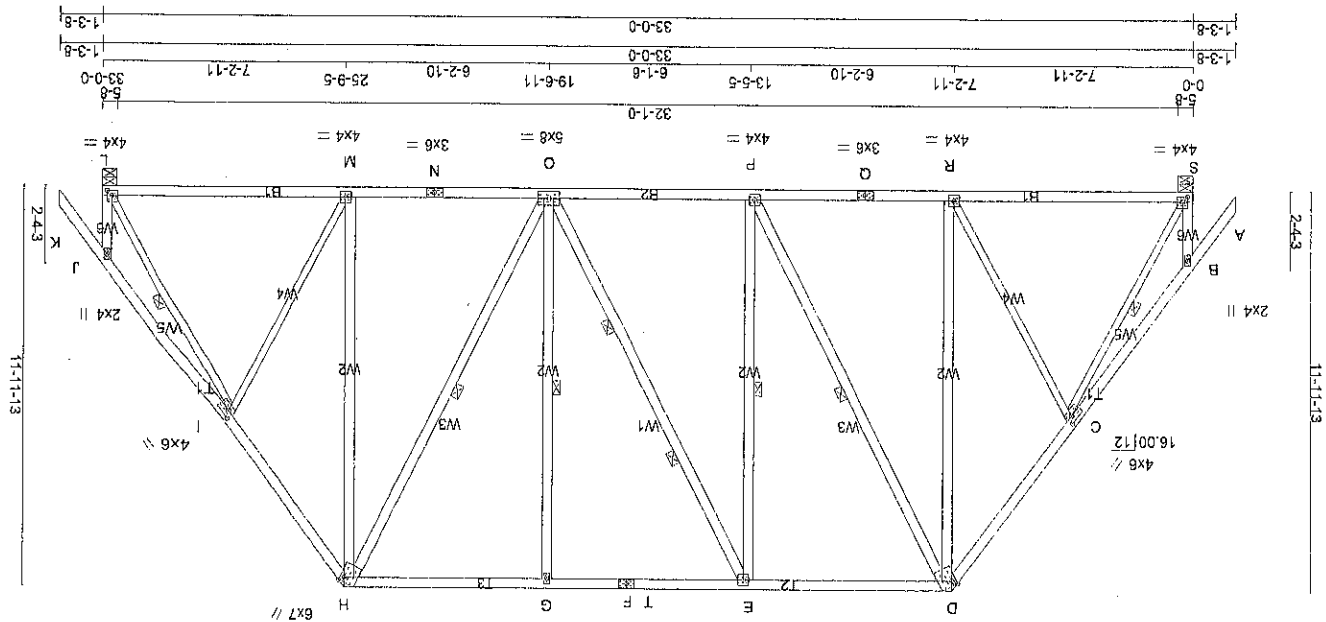
JSI GRIP=0.41 (B) (INPUT = 0.90)

JSI METAL=0.10 (B) (INPUT = 1.00)



ID: Ayrlx9y7a2EXF7DROXSq4zMEZ1-MUDdrcLJLbXk9FowGwG16QFDW_3wd3xJy3bYzUnyx9R1
Version 7.20 S Apr 15 2015 Mitek Industries, Inc. Fri Jul 17 13:44:53 2015 Page 1

JOB NAME	253636
TRUSS NAME	J01C
QUANTITY	8
PLY	1
JOB DESC.	TRUSS DESC.
DRWG NO.	



LUMBER	N.L.G.	A.RULES	CHORDS	SIZE	LUMBER	DESCR.
DRY	2x4	DRY	No.2	SPP		
DRY	2x4	DRY	No.2	SPP		
H-K	2x4	DRY	No.2	SPP		
F-H	2x4	DRY	No.2	SPP		
J-L	2x4	DRY	No.2	SPP		
O-Q	2x4	DRY	No.2	SPP		
N-L	2x4	DRY	No.2	SPP		
ALL WEBS	2x4	DRY	No.2	SPP		
EXCEPT						
C-R	2x3	DRY	No.2	SPP		
M-I	2x3	DRY	No.2	SPP		
S-C	2x3	DRY	No.2	SPP		
I-L	2x3	DRY	No.2	SPP		
DRY: SEASONED LUMBER,						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED									
GROSS REACTION									
VERT	HORIZ	DOWN	MAXIMUM FACTORED	INPUT	BRG	RECORD			
ST	VERT	DOWN	1620	0	IN-SX	1-12			
L	1620	0	1620	0	5-8	1-12			
UNFACTORED REACTIONS									
1ST LOOSE									
MAX/MIN COMPONENT REACTIONS									
LT	COMBINED	SNOW	LIVE	PERMILIVE	WIND	DEAD			
S	1146	762 / 0	0 / 0	0 / 0	0 / 0	394 / 0			
L	1146	762 / 0	0 / 0	0 / 0	0 / 0	394 / 0			

PLATES (tablets in inches)		PLATE TYPE		W		LEN		X	
B	TW+P	MT20	2.0	2.0	4.0	2.00	Edge 1.50		
C	TW+V+M	MT20	6.0	7.0	4.0	2.00			
D	TW+V+M	MT20	6.0	7.0	4.0	2.00			
E	TW+V+M	MT20	3.0	6.0	4.0	2.00			
F	T3-1	MT20	3.0	6.0	4.0	2.00			
G	TW+V+M	MT20	2.0	4.0	4.0	2.00			
H	TW+V+M	MT20	6.0	7.0	4.0	2.00			
I	TW+V+M	MT20	6.0	7.0	4.0	2.00			
J	TW+P	MT20	2.0	4.0	4.0	2.00			
L	TW+P	MT20	2.0	4.0	4.0	2.00			
M	P	MT20	4.0	4.0	4.0	2.00			
N	BS-1	MT20	3.0	6.0	4.0	2.00			
O	BS-1	MT20	3.0	6.0	4.0	2.00			
S	BW+V+1	MT20	4.0	4.0	4.0	2.00			

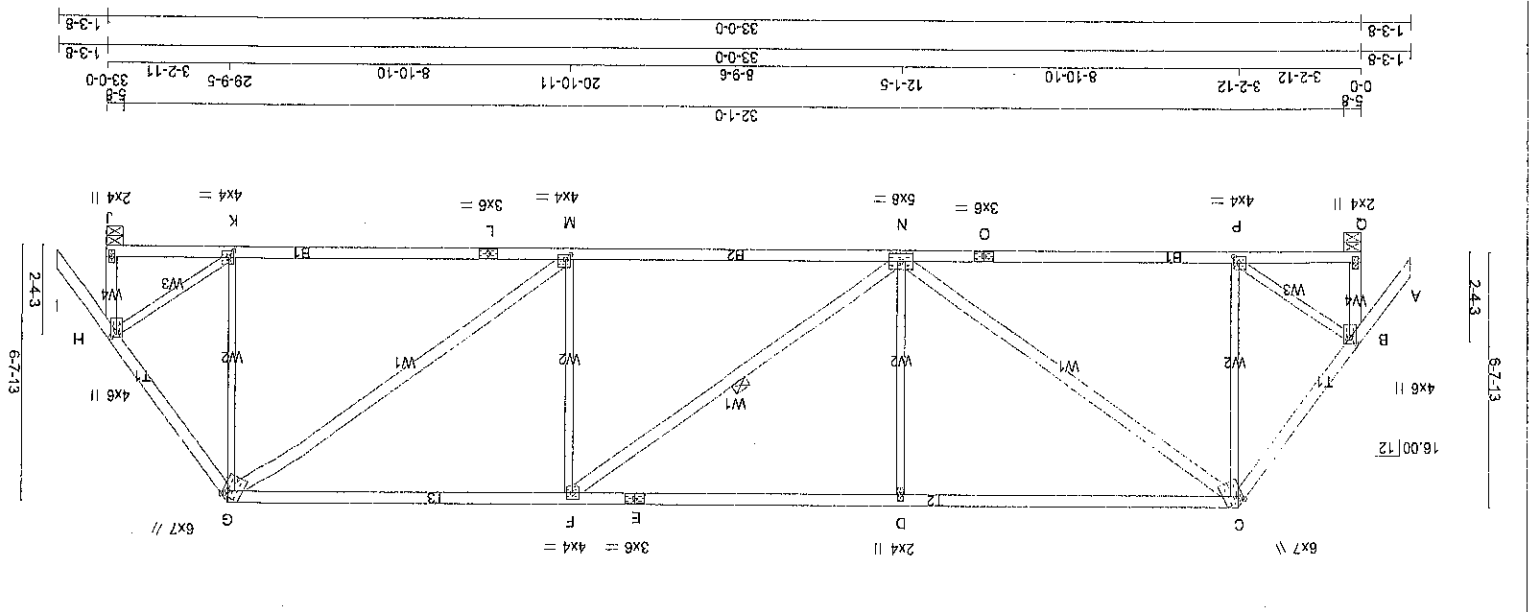
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB.		FORCE		VERT. LOAD		FACTORED		WEBS	
(LBS)		(KIPF)		(KIPF)		(KIPF)		(LBS)	
MEMB.		FORCE		VERT. LOAD		FACTORED		MEMB.	
MAX		MAX		MAX		MAX		MAX	
CS1 (L)		CS1 (L)		CS1 (L)		CS1 (L)		CS1 (L)	
UNBRACED		UNBRACED		UNBRACED		UNBRACED		UNBRACED	
FR-TO		FR-TO		FR-TO		FR-TO		FR-TO	
A-B		A-B		A-B		A-B		A-B	
C-D		C-D		C-D		C-D		C-D	
D-E		D-E		D-E		D-E		D-E	
E-F		E-F		E-F		E-F		E-F	
F-G		F-G		F-G		F-G		F-G	
G-H		G-H		G-H		G-H		G-H	
H-I		H-I		H-I		H-I		H-I	
I-J		I-J		I-J		I-J		I-J	
J-K		J-K		J-K		J-K		J-K	
K-L		K-L		K-L		K-L		K-L	
L-M		L-M		L-M		L-M		L-M	
M-N		M-N		M-N		M-N		M-N	
N-O		N-O		N-O		N-O		N-O	
O-P		O-P		O-P		O-P		O-P	
P-Q		P-Q		P-Q		P-Q		P-Q	
Q-R		Q-R		Q-R		Q-R		Q-R	
R-S		R-S		R-S		R-S		R-S	
S-T		S-T		S-T		S-T		S-T	
T-U		T-U		T-U		T-U		T-U	
U-V		U-V		U-V		U-V		U-V	
V-W		V-W		V-W		V-W		V-W	
W-X		W-X		W-X		W-X		W-X	
X-Y		X-Y		X-Y		X-Y		X-Y	
Y-Z		Y-Z		Y-Z		Y-Z		Y-Z	
Z-AA		Z-AA		Z-AA		Z-AA		Z-AA	
0.40	-70.5	-70.5	0.10	(1)	10.00	C-R	0.64	0.02	(4)
0.23	-70.5	-70.5	0.14	(1)	10.00	R-D	0.97	0.02	(4)
-1268.0	-70.5	-70.5	0.36	(1)	6.62	D-P	0.702	0.11	(1)
-1085.0	-78.0	-78.0	0.33	(1)	6.66	F-E	-519.0	0.41	(1)
-1085.0	-78.0	-78.0	0.33	(1)	6.66	E-O	-2.0	0.00	(1)
-1085.0	-78.0	-78.0	0.33	(1)	6.66	O-G	-518.0	0.41	(1)
-1064.0	-78.0	-78.0	0.36	(1)	5.82	M-H	0.98	0.02	(4)
-1268.0	-70.5	-70.5	0.14	(1)	5.57	M-I	0.64	0.02	(4)
0.23	-70.5	-70.5	0.14	(1)	10.00	S-C	-1552.0	0.60	(1)
0.40	-70.5	-70.5	0.10	(1)	10.00	I-L	-1553.0	0.60	(1)
-196.0	0.0	0.0	0.02	(1)	7.81				
-196.0	0.0	0.0	0.02	(1)	7.81				
0.727	-17.5	-17.5	0.28	(4)	10.00				
0.766	-17.5	-17.5	0.28	(4)	10.00				
0.766	-17.5	-17.5	0.28	(4)	10.00				
0.766	-17.5	-17.5	0.28	(4)	10.00				
0.766	-17.5	-17.5	0.28	(4)	10.00				
0.727	-17.5	-17.5	0.28	(4)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

LOADING IN FLAT SECTION BASED ON	
PIGGYBACK TRUSS WITH BLORES OF 6.00/12	
AND 6.00/12 AND RESPECTIVE WALL	
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL	
DEAD LOAD OF 3.0 P.S.F.	
THIS TRUSS IS DESIGNED FOR RESIDENTIAL	
OR SMALL BUILDING REQUIREMENTS OF	
PART 9, NBCC 2010	
THIS DESIGN COMPLIES WITH:	
-PART 9 OF CBC 2012, BCBC 2012, ABC 2014	
-CSA 098-08	
-TIP 2011	
(66% OF 23.0 P.S.F. G.S.L. PLUS	
8.4 P.S.F. RAIN LOAD EQUALS	
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD	
ALLOWABLE DEFLECTION = $L/360$ (1.10")	
CALCULATED VERT. DEFLECTION = $L/999$ (0.66")	
ALLOWABLE DEFLECTION = $L/360$ (1.10")	
CALCULATED VERT. DEFLECTION = $L/999$ (0.66")	
CS: $TC=0.36$ (D-E-1), $BC=0.28$ (M-C-4),	
$WB=0.60$ (H-L-1), $SS=0.23$ (C-E-1)	
DOL. COMPRESS=1.00 NAL=1.00 LS BEND=1.10	
COMPANION LIVE LOAD FACTOR = 0.50	
TRUSS PLATE MANUFACTURER IS NOT	
RESPONSIBLE FOR QUALITY CONTROL IN	
THE TRUSS MANUFACTURING PLANT.	
NAIL VALUES	
PLATE GRIP(DRY) SHEAR SECTION	
(PSI) (PLI)	
MAX MIN MAX MIN	
MT20 618 354 1687 222 2284 1656	
PLATE PLACEMENT TOL. = 0.250 inches	
PLATE ROTATION TOL. = 5.0 Deg.	
JSI GRIP = 0.89 (I) (INPUT = 0.80)	
JSI METAL = 0.36 (I) (INPUT = 1.00)	

[illegible][illegible][illegible]

ITEM	QTY	UNIT	PRICE	TOTAL	REMARKS
1. TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.	0.16 (1)	B - P	0 / 800	0.16 (1)	
2. TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.	0.18 (1)	K - H	0 / 800	0.18 (1)	
3. NATL VALUES	10.00			10.00	
4. PLATE GRIP(DRY) SHEAR SECTION	10.00			10.00	
5. (PL)	10.00			10.00	
6. MAX MIN	10.00			10.00	
7. MAX MIN	10.00			10.00	
8. MT20	10.00			10.00	
9. 618 354 1687 822 2254 1656	10.00			10.00	
10. PLATE PLACEMENT TOL. = 0.250 inches	10.00			10.00	
11. TOUCHES EDGE OF CHORD.	0.40	H - I	0 / 40	0.40	
12. Q - B	1548 / 0			1548 / 0	
13. J - H	1548 / 0			1548 / 0	
14. Q - P	0 / 0			0 / 0	
15. N - M	0 / 1852			0 / 1852	
16. O - N	0 / 685			0 / 685	
17. L - K	0 / 685			0 / 685	
18. K - J	0 / 0			0 / 0	
19. 17.5	0.20 (4)			0.20 (4)	
20. 17.5	0.33 (4)			0.33 (4)	
21. 17.5	0.45 (1)			0.45 (1)	
22. 17.5	0.33 (4)			0.33 (4)	
23. 17.5	0.20 (4)			0.20 (4)	

PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.81 (C) (INPUT = 0.90)
JSI METAL = 0.44 (W) (INPUT = 1.00)



4-15073776

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factorored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factorored resistances given.
- The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

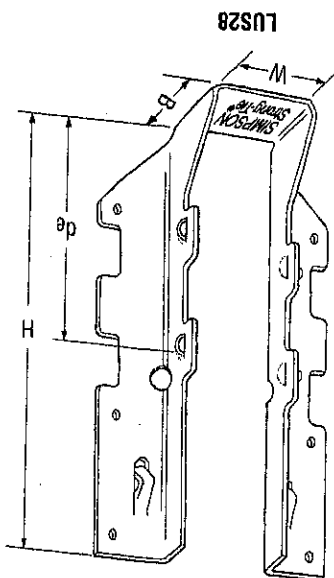
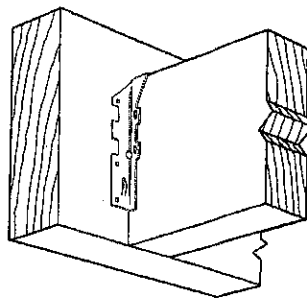
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/4" long common wire.
- 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

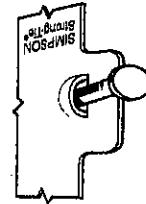
- These hangers cannot be modified.

Typical LUS Installation

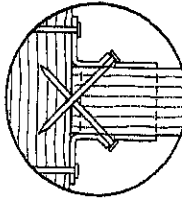


Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)	
		W	H	B	de	Face	Joist	Uplift	D, Fl-L
LUS24	18	1 1/8	3 1/4	3 1/4	1 1/8	4-10d	2-10d	710	1630
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	4-16d	2-16d	835	2020
LUS26	18	1 1/8	4 1/4	1 1/4	3 3/8	4-10d	4-10d	1420	2170
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595
LUS26-3	18	4 1/8	4 3/8	2	3 1/4	4-16d	4-16d	1720	2595
LUS28	18	1 1/8	6 5/8	1 1/4	3 3/4	6-10d	4-10d	1420	2520
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325
LUS28-3	18	4 1/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325
LUS210	18	1 1/8	7 1/8	1 1/4	3 3/8	8-10d	4-10d	1420	2785
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500
LUS210-3	18	4 1/8	8 3/8	2	5 1/4	8-16d	6-16d	2580	4500

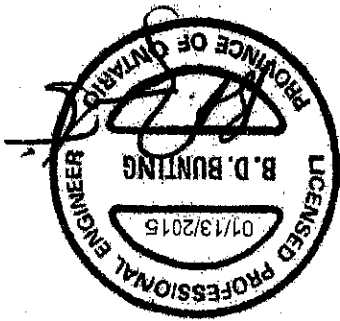
1. de is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing
Top View



800-999-5099
www.strongtie.com

HUS/LJS - Double Shear Joist Hangers

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted

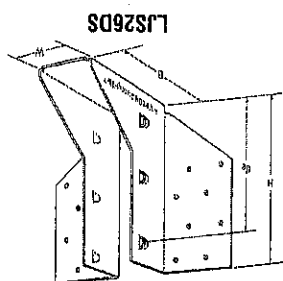
Wood shear is not considered in the factored resistances. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

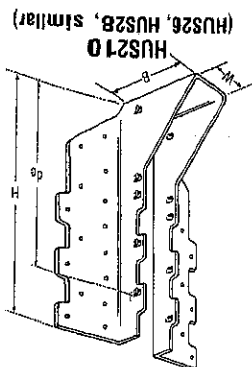
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/4" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

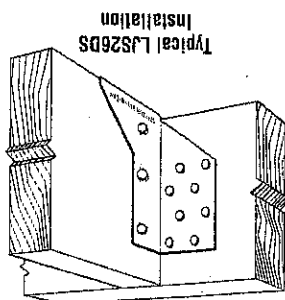
- See current catalogue for options



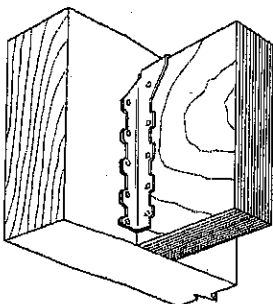
LJS26DS



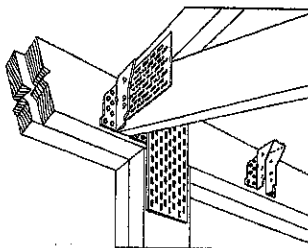
HUS210 (HUS26, HUS28, similar)



Typical LJS26DS Installation



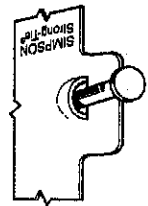
Typical HUS Installation



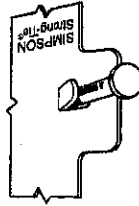
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	W	H	B	dg	Face	Joist	Fasteners		Factored Resistance (lbs)	
								D, Fir-L	S-P-F	Uplift	Normal
LJS26DS	18	1 3/16	5	3 1/4	4 5/8	16-16d	6-16d	2055	4265	1480	4115
HUS26	16	1 1/4	5 1/2	3	3 3/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/4	7 1/2	3	6 3/4	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/4	9 3/4	3	7 3/4	30-16d	10-16d	4505	5795	4010	4740
HUS1.8/1.7/1.0	16	1 3/16				30-16d	10-16d	4505	6450	4010	5200

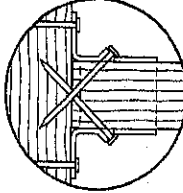
1. dg is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models). U.S. Patent 5,603,580



Double Shear Nailing Side View, Do not bend tab back.



Double Shear Nailing Top View.



HHUS – Double Shear Joist Hangers



All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge
FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

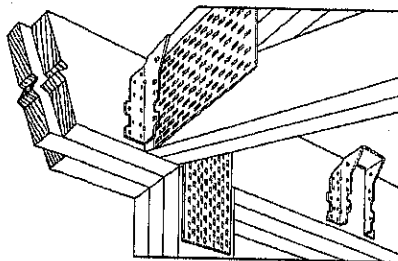
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

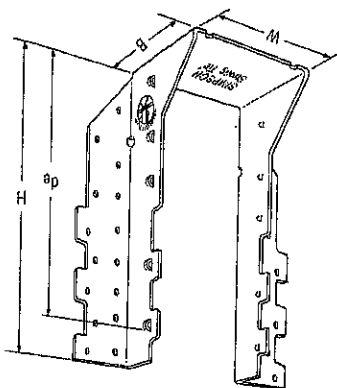
OPTIONS:

- See current catalogue for options

Typical HHUS installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

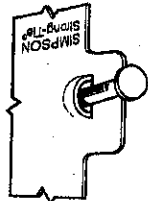


HHUS410

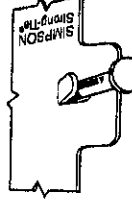


Model	No.	Ga	W	H	B	dg	Face	Joist	Fasteners			Dimensions (in)		Factor Resistance (lbs)	
									Uplift	Normal	S-P-F				
HHUS26-2	14	3/8	5 1/8	3	3 1/2	14-16d	6-16d	2850	7335	2065	5205	14	3 1/8	2065	5205
HHUS28-2	14	3/8	7 1/2	3	6 5/8	22-16d	8-16d	3765	8940	2675	6345	14	3 1/8	2675	6345
HHUS210-2	14	3/8	9 3/8	3	8	30-16d	10-16d	4745	9660	4310	7000	14	3 1/8	4310	7000
HHUS210-3	14	4 1/8	9	3	7 5/8	30-16d	10-16d	4745	10545	4310	7485	14	4 1/8	4310	7485
HHUS210-4	14	6 1/8	8 23/32	3	7 27/32	30-16d	10-16d	4745	10545	4310	7485	14	6 1/8	4310	7485
HHUS46	14	3/8	5 3/8	3	3 15/16	14-16d	6-16d	2540	7335	2065	5205	14	3/8	2065	5205
HHUS48	14	3/8	7 1/8	3	6 1/4	22-16d	8-16d	3765	8945	2675	6345	14	3/8	2675	6345
HHUS410	14	3/8	9	3	8	30-16d	10-16d	4745	9855	4310	7000	14	3/8	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d	4745	10545	4310	7485	14	5 1/2	4310	7485
HHUS7.25/10	14	7 1/4	9	3	8	30-16d	10-16d	4745	10770	4310	7650	14	7 1/4	4310	7650

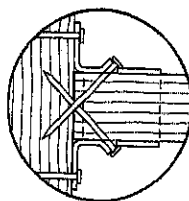
1. dg is the distance from the seat of the hanger to the highest joist nail.



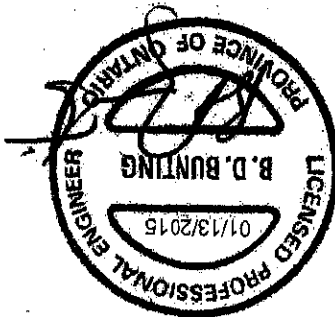
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



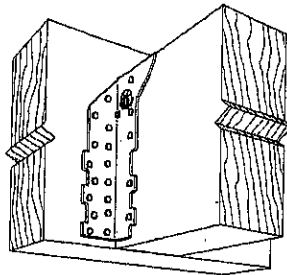
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



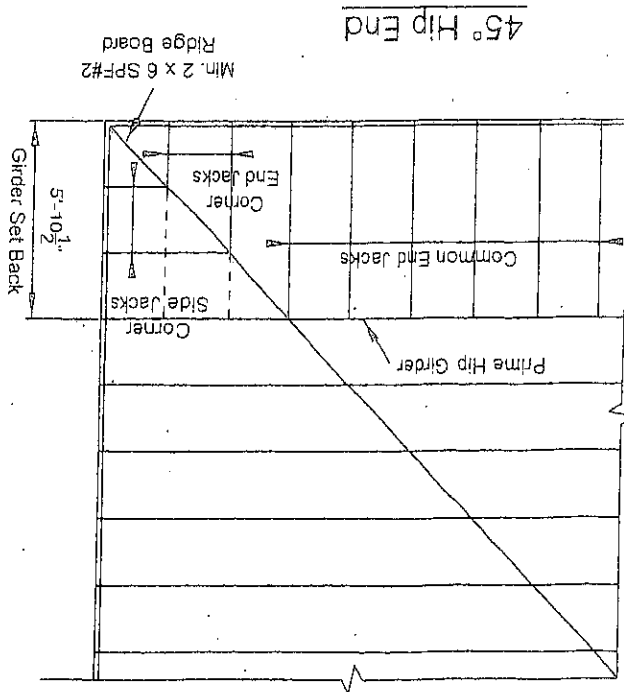
Typical HHUS Installation



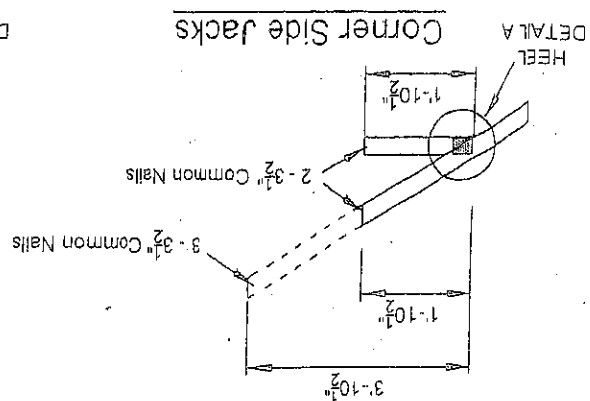
800-999-5099
www.strongtie.com

LUMBER SPECIFICATION
 TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

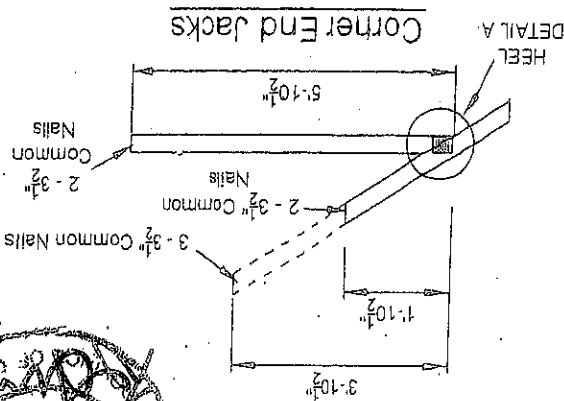
DESIGN LOAD	
TOP CHORD SNOW LOAD	40.5 P.S.F.
TOP CHORD DEAD LOAD	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	7.0 P.S.F.
TOTAL LOAD	50.5 P.S.F.



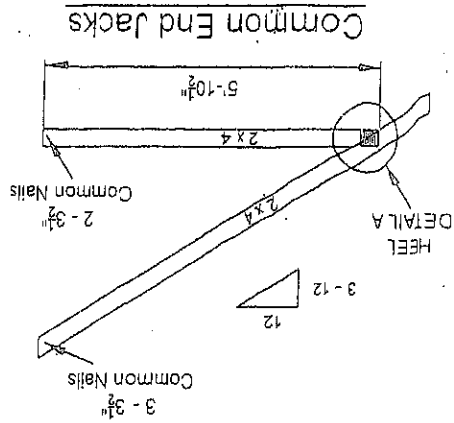
45° Hip End



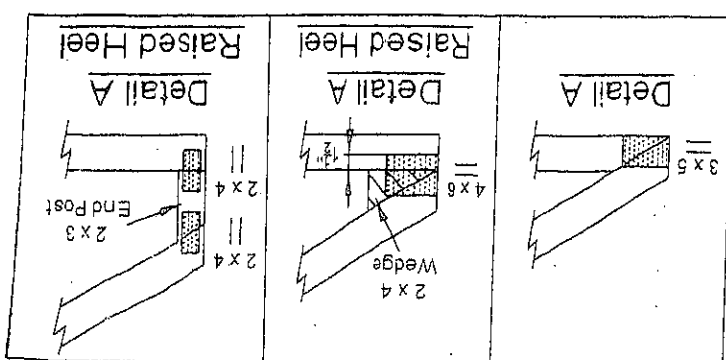
Corner Side Jacks



Corner End Jacks



Common End Jacks



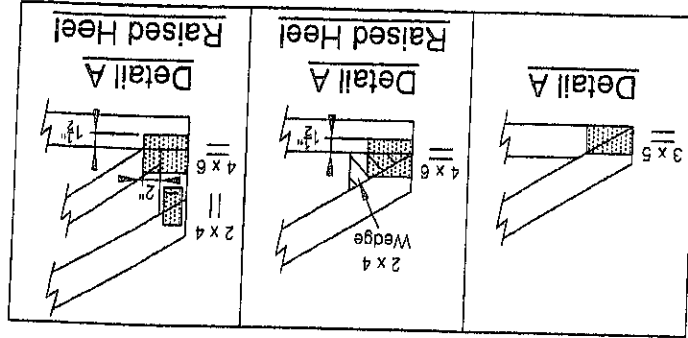
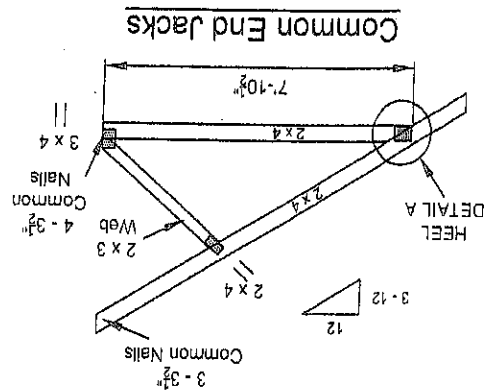
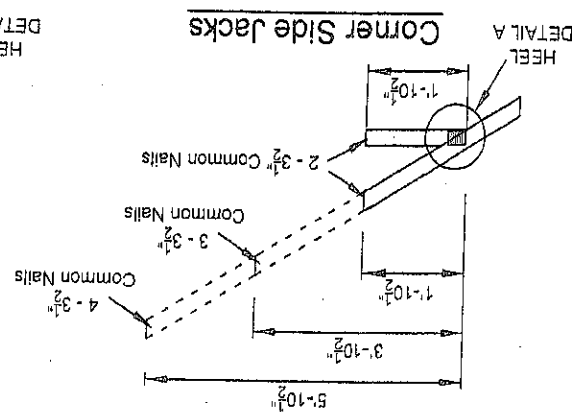
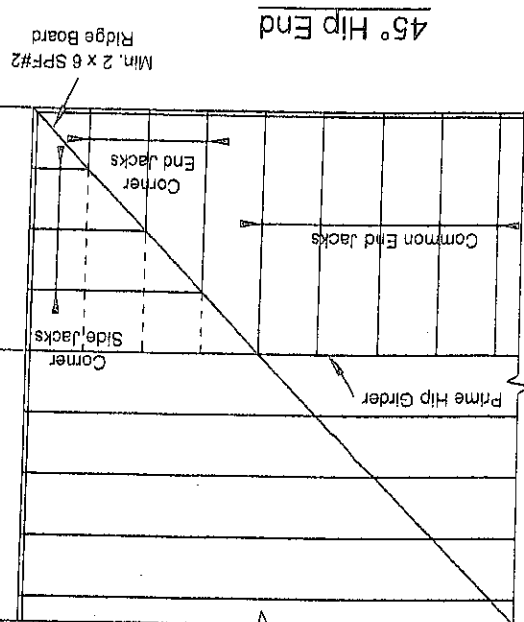
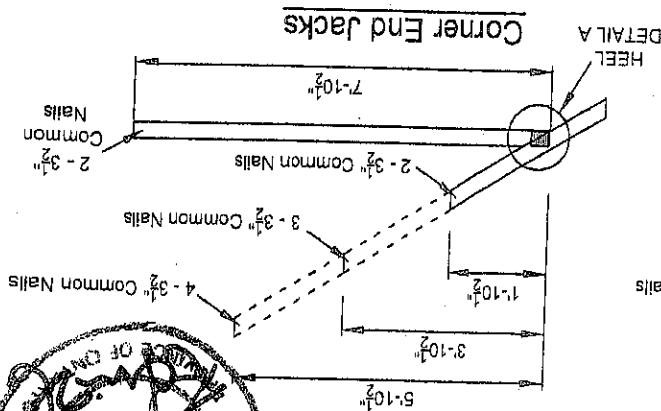
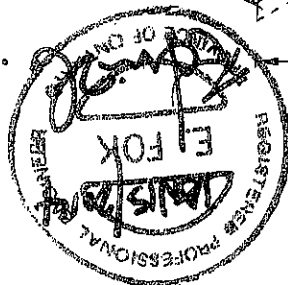
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

STRACON ENGINEERING INC.

LUMBER SPECIFICATION
 TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD	
TOP CHORD SNOW LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-71008N