



PL 80980

Job Track:	40297	Builder / Location:	GOLD PARK / KLEINBURG		Model / Elevation:	42-4	/A	STD OR OPT 2ND FLR
Layout ID:	253635	Project:	HUNTINGTON & NASHVILLE		THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
Plan Log:	79711	Date:	5/12/2015	Designer:	cheyle			

CONVENTIONAL
FRAMING BY
OTHERS

Milltek Ver 7.5.0

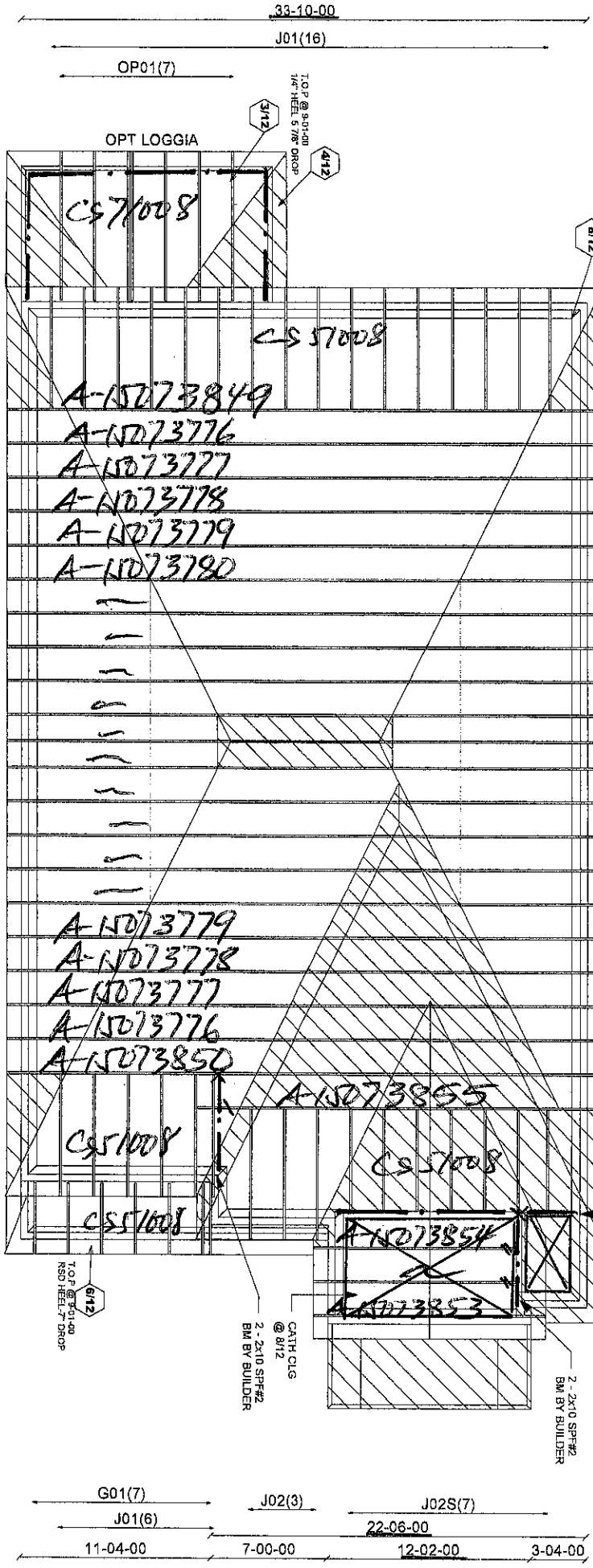
12" FIN. CH.
RSD. HEEL - 0" DROP
12" x 6" FASCIA - FM
ASPHALT SHINGLES
27" x 6" BRG/BRK
PIGGYBACK TRS
PURLINS BY OTHERS

8-00-00

51-10-00

2-06-00 5-10-00 5-00-00

HANGERS
// LUS24
/ LUS26DS
X HHUS26-2



ALL CONC. PAVING TO CONFORM WITH 9\"/>

8-00-00
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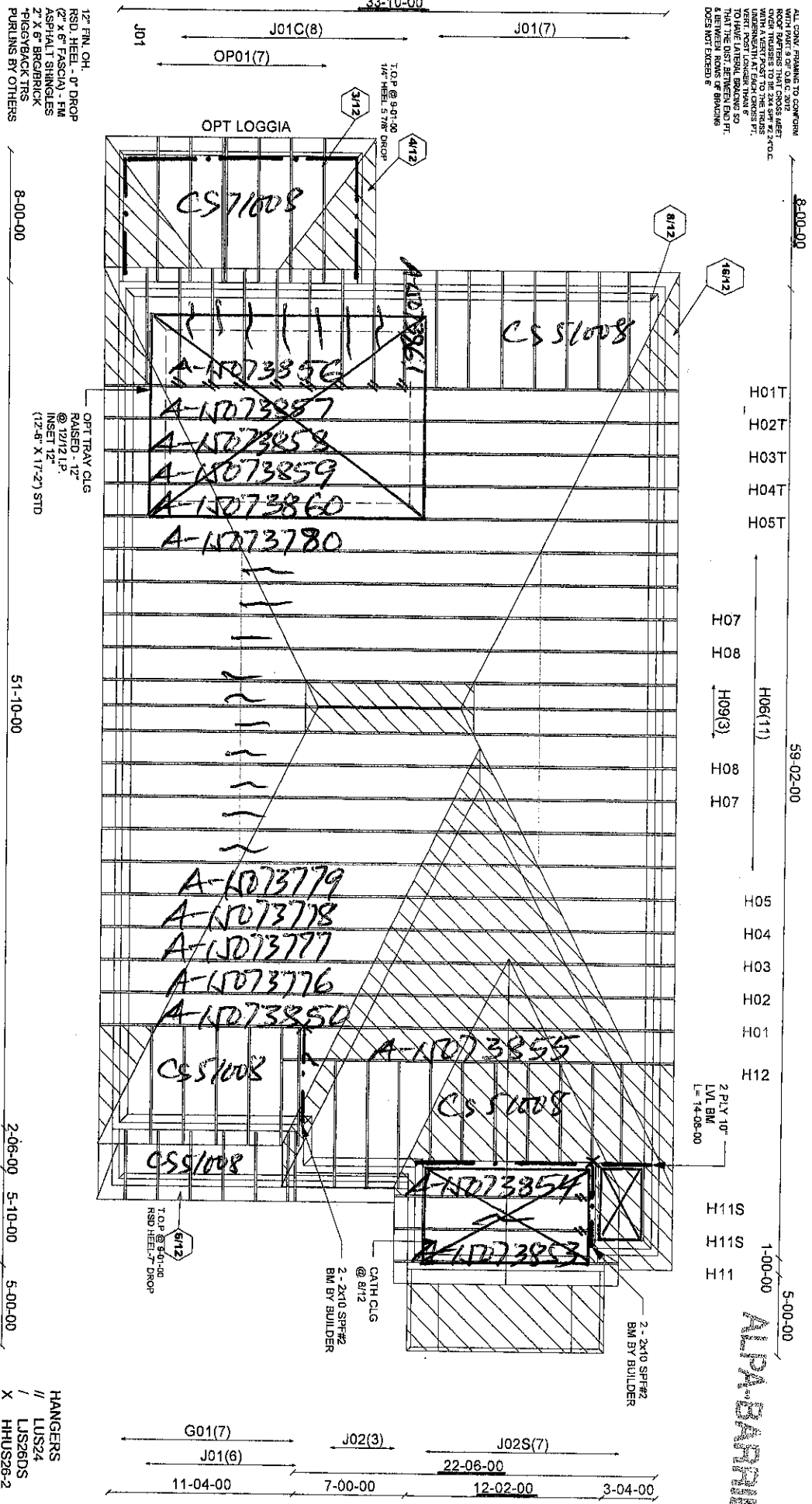
PL 80980



Job Track:	40297	Builder / Location:	GOLD PARK / KLEINBURG	Model / Elevation:	42-4	/ A OPT TRAY
Layout ID:	253636	Project:	HUNTINGTON & NASHVILLE	THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.		
Plan Log:	79711	Date:	5/1/2015	Designer:	cheyle	

CONVENTIONAL FRAMING BY OTHERS

Millk Var 7.5.0



ALL CORNERS TO CONFORM WITH PART 9 OF O.B.C. 2012. ROOF RATHER THAN CROSS JET. UNDERSTAND THAT THE TRUSSES ARE TO BE INSTALLED IN SUCH A MANNER THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BAYNITE

Job Track: 40297
Layout ID: 253638
Plan Log: 79711

Builder / Location:	GOLD PARK / KLEINBURG	
Project:	HUNTINGTON & NASHVILLE	
Date:	5/1/2015	Designer: cheryl

Model / Elevation:
42-4

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALFA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALFA ROOF TRUSSES INC. AND WILL BE RETRACTED BY ALFA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

/ A OPT 2ND W/TRAY

 CONVENTIONAL FRAMING BY OTHERS

PL 80980

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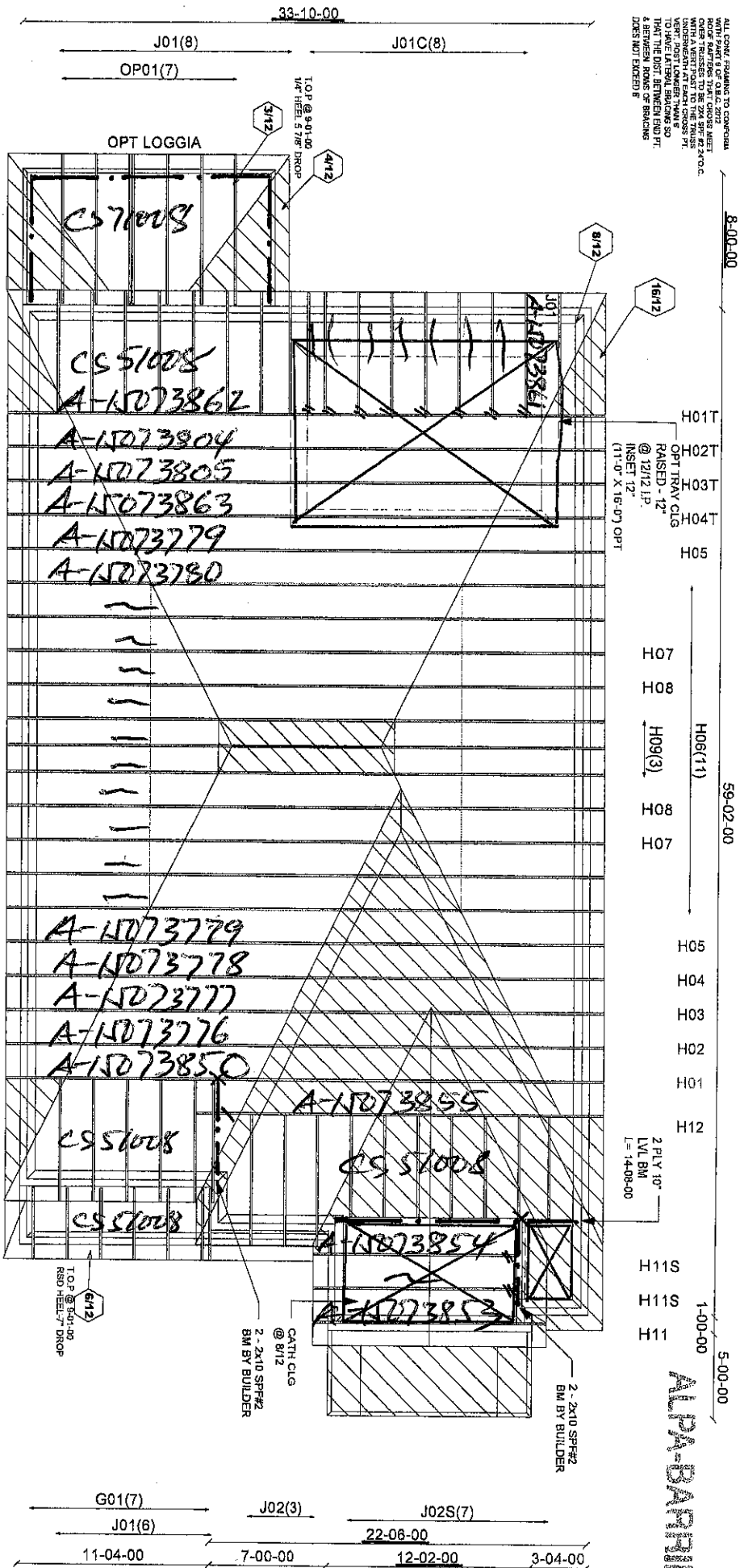
12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

8-00-00

51-10-00

2-06-00 5-10-00 5-00-00

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2

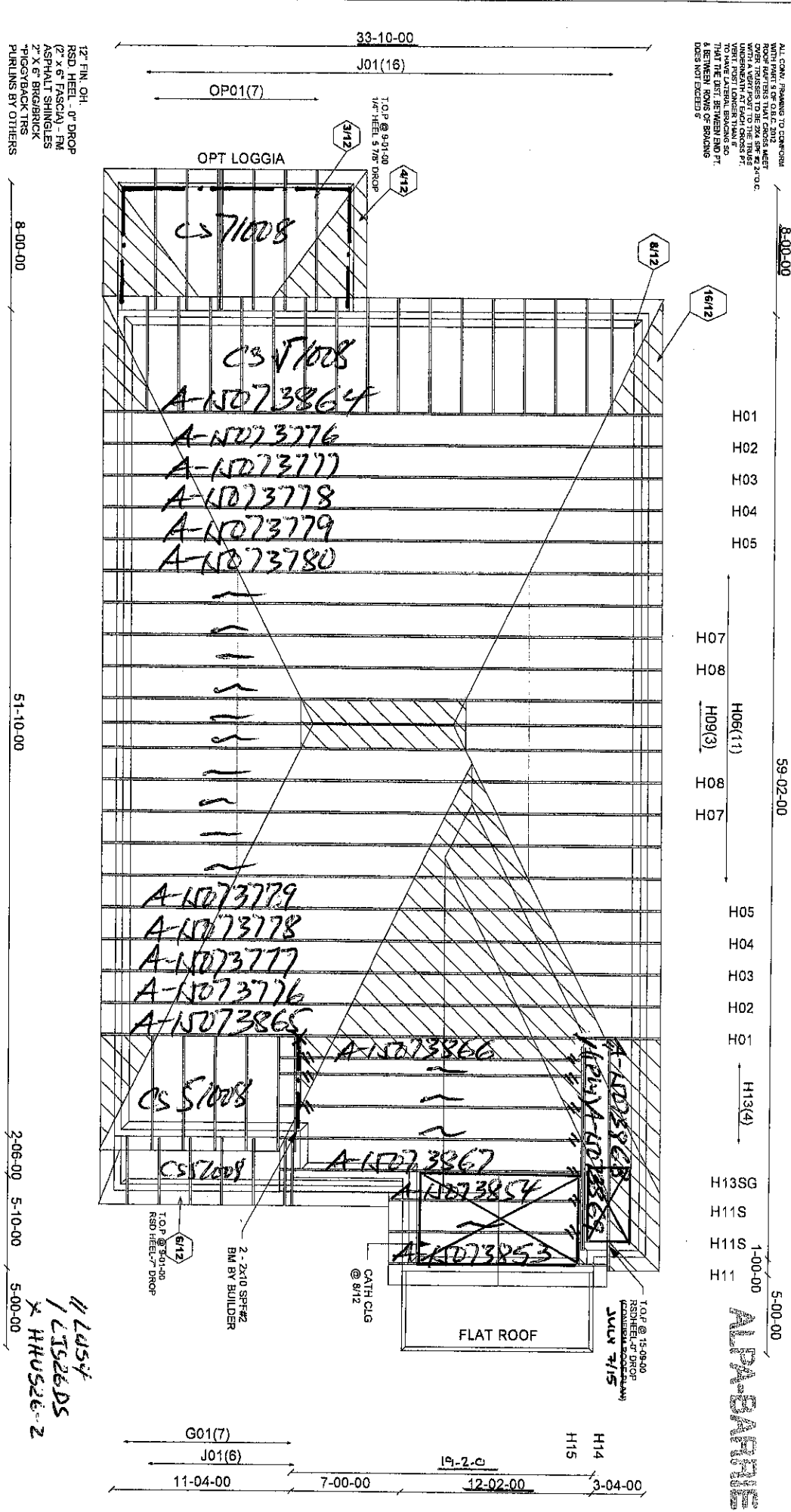


ALL CONN. FRAMING TO CONFORM WITH PART 1 OF I.B.C. 2012. OVER TRUSSES TO BE 2x4 SP@ 24" O.C. WITH A MIN. 2" OF INSULATION. VERT. POST LONGER THAN 8' THAT THE OVERHANGS DO NOT EXCEED 8'.

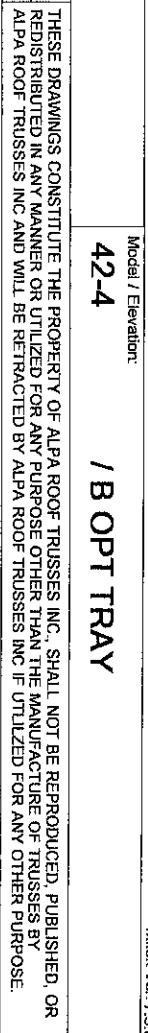
12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRICK
PIGSTICKERS
FURNISH BY OTHERS

		PL 80980	
Job Track	40297	Builder / Location	GOLD PARK / KLEINBURG
Layout ID	253639	Project	HUNTINGTON & NASHVILLE
Plan Log	79711	Date	5/1/2015
		Designer	chyle
Model / Elevation	42-4	/ B	
		STD OR OPT TRAY	
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALFA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALFA ROOF TRUSSES INC. AND WILL BE RETRACTED BY ALFA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			

Piggyback
 (A-15073781)
 A-15073851
 A-15073852
 A-15073870



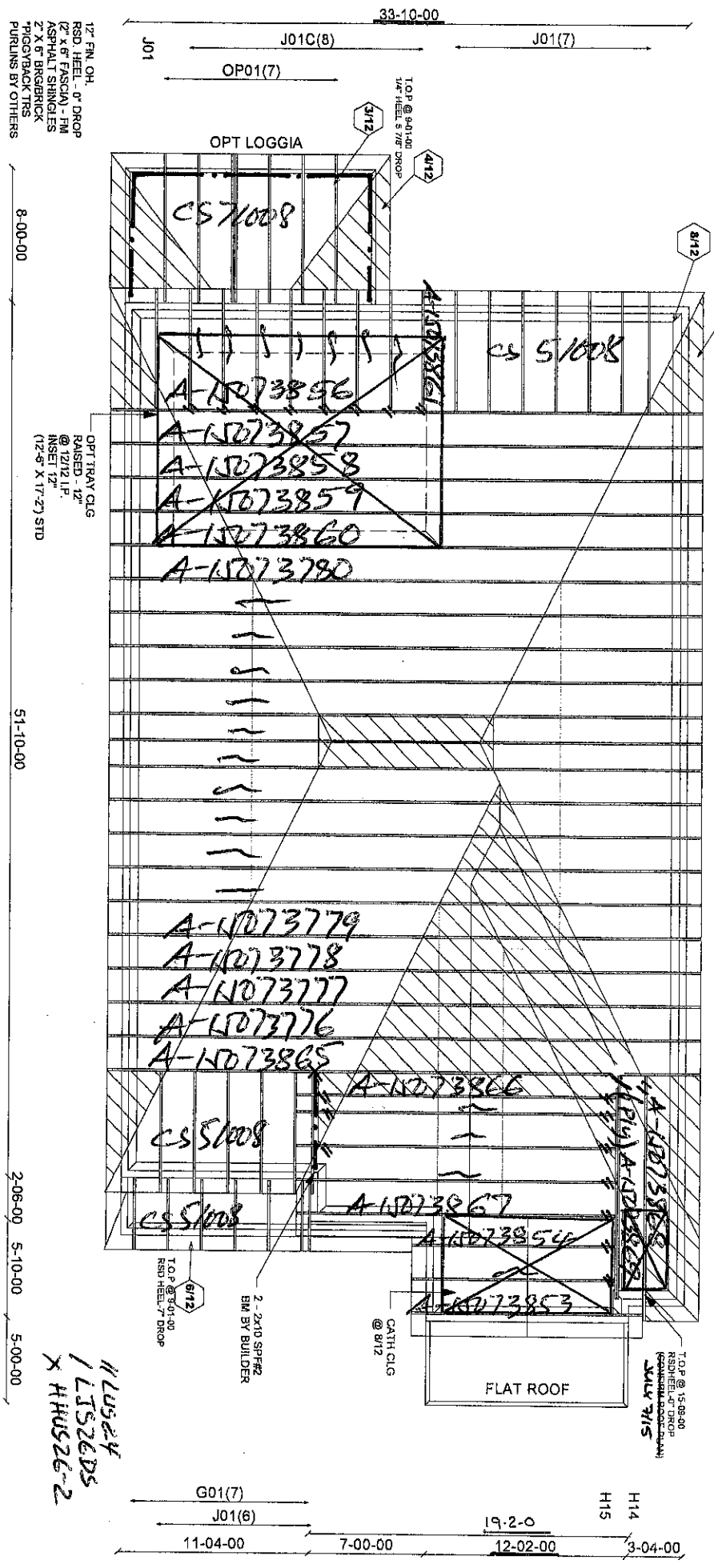
ALFA-BATTUE



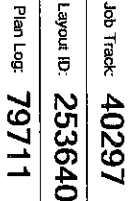
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11 LUS-24
1 LJS26DS
X HHS26-2



WIK / KLEINBURG
NGTON & NASHVILLE
Designer: chryle

Model / Elevation:
42-4

/ B OPT 2ND W/TRAY

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REISSUED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC. AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

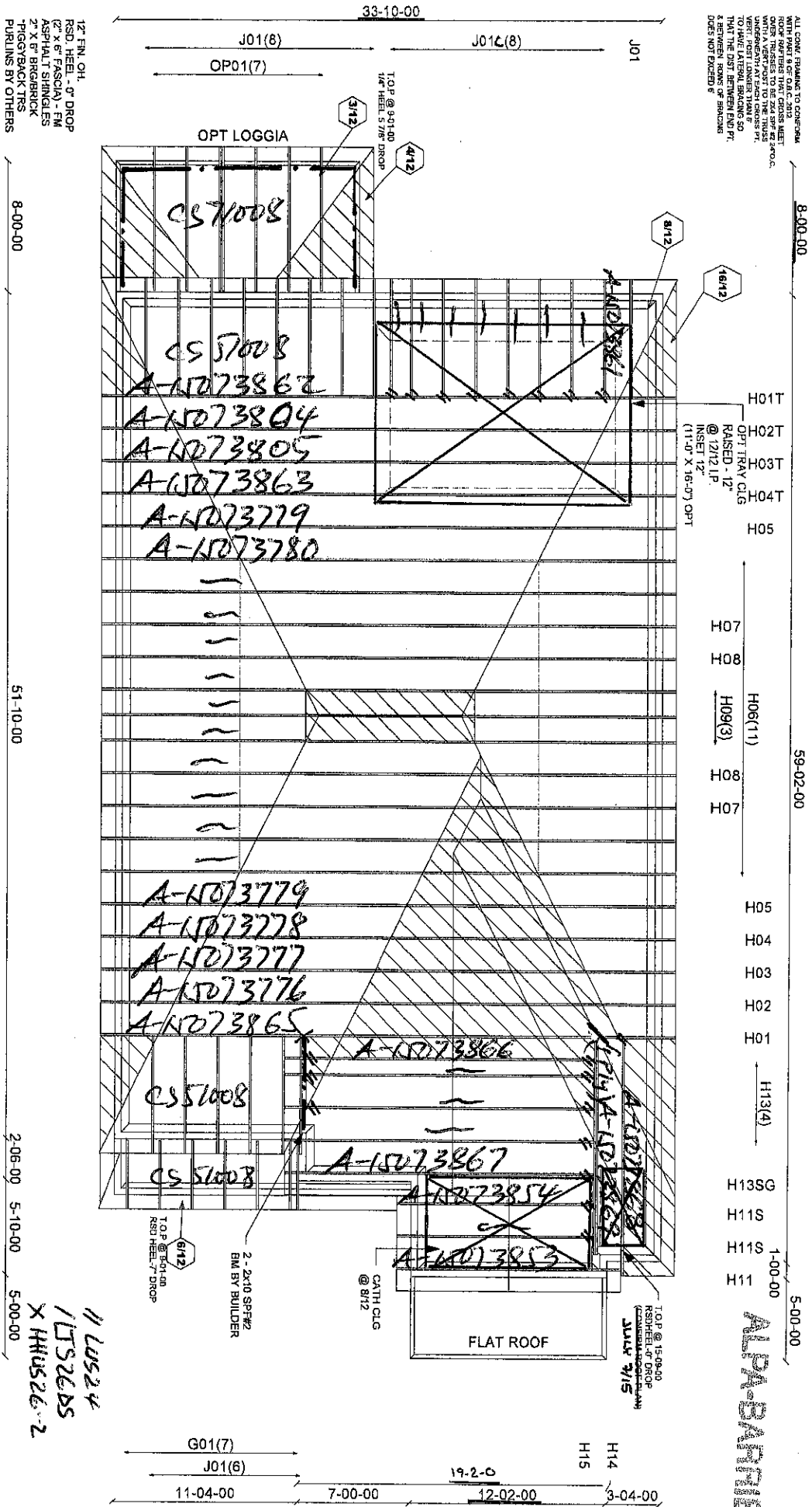
PL 80980

Mitek Ver. 7.5.0

Piggyback (A-15073781
A-15073851
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A-15073870)

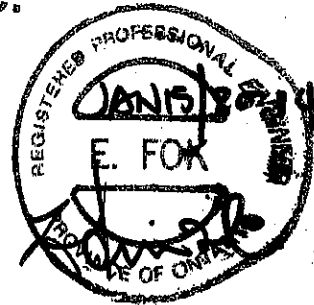


CONVENTIONAL
FRAMING BY
OTHERS



Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

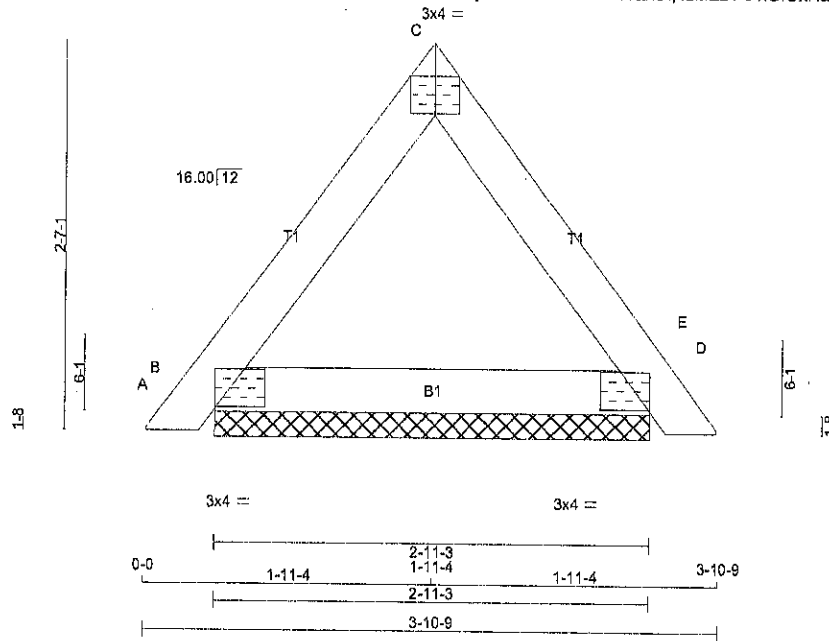
January 15, 2014

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Fri Jul 17 15:22:08 2015 Page 1

ID: AyrJx8Y7fa2EXF7DRQxSq4zMEz1-01UIOxRaPhDGtPEENtQP1OhcBPCjxsPqvVEB8yx80T

Scale = 1:15.4



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TT-p	MT20	3.0	4.0	3.25	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
B	154	0	154	0	2-11-3	1-8
D	154	0	154	0	2-11-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	108	76 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
D	108	76 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 9	-70.5	-70.5 0.01 (1)	10.00			
B-C	-83 / 0	-70.5	-70.5 0.03 (1)	6.25			
C-D	-83 / 0	-70.5	-70.5 0.03 (1)	6.25			
D-E	0 / 9	-70.5	-70.5 0.01 (1)	10.00			
B-D	0 / 37	-17.5	-17.5 0.05 (4)	10.00			

TOTAL WEIGHT = 5 X 11 = 63 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03 (C-D:1), BC=0.05 (B-D:4), WB=0.00 (N/A:0), SSI=0.03 (B-D:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822 2264 1656

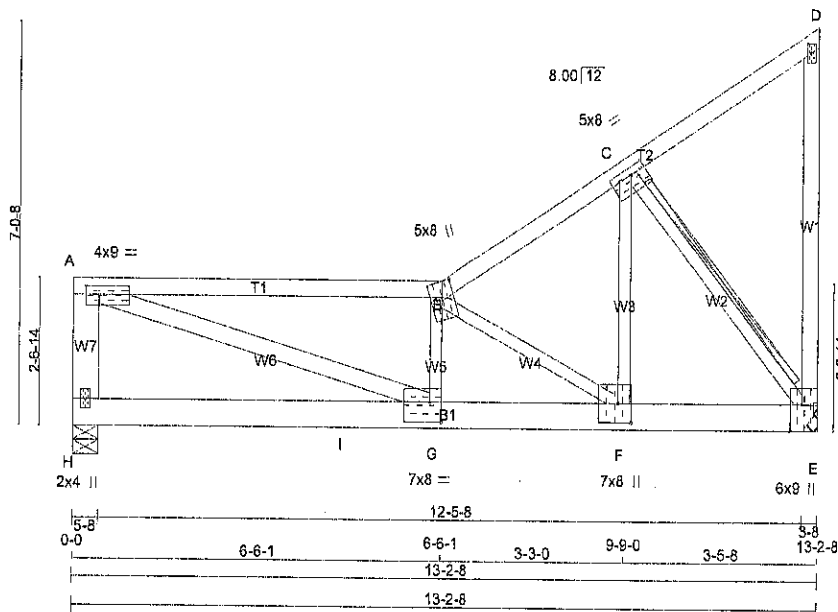
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (D) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



A-10073870



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
H - A	2x6	DRY	No.2 SPF
A - B	2x4	DRY	1650F 1.5E SPF
B - D	2x4	DRY	No.2 SPF
E - D	2x4	DRY	No.2 SPF
H - E	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF
A - G	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	9.0	1.75	2.75
B	TTWW+m	MT20	5.0	8.0	3.25	2.50
C	TMVW-t	MT20	5.0	8.0	1.50	3.25
D	TMV+p	MT20	2.0	4.0		
E	BMVWV-t	MT20	6.0	9.0	Edge	2.50
F	BMVW+t	MT20	7.0	8.0	4.00	2.75
G	BMVW-t	MT20	7.0	8.0	4.25	2.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT	REQRD
GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX
2284	0	2284	0	0	5-8
2756	0	2756	0	0	2-8
HANGER BY OTHERS					
MIN. SEAT SIZE: 3-8					

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	1609	1090 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0
E	1942	1315 / 0	0 / 0	0 / 0	0 / 0	627 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT C-E
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WAVE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER
90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO		
H-A	-1668 / 0	0.0	0.0	0.14 (1)	7.61	A-G	0 / 4459
A-B	-4218 / 0	-70.5	-70.5	0.81 (1)	3.23	G-B	0 / 281
B-C	-1981 / 0	-70.5	-70.5	0.20 (1)	4.58	B-F	-3060 / 0
C-D	-15 / 0	-70.5	-70.5	0.14 (1)	6.25	F-C	0 / 2898
E-D	-84 / 0	0.0	0.0	0.09 (1)	7.81	C-E	-2671 / 0
H-I	0 / 0	-213.6	-213.6	0.66 (1)	10.00		
I-G	0 / 0	-367.5	-367.5	0.65 (1)	10.00		
G-F	0 / 4204	-367.5	-367.5	1.00 (1)	10.00		
F-E	0 / 1685	-367.5	-367.5	0.55 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	21.0 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGlrdr
START DISTANCE = 0-0
START SPAN CARRIED = 10-11-0
END DISTANCE = 4-10-0
END SPAN CARRIED = 10-11-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 4-10-0
START SPAN CARRIED = 17-11-0
END DISTANCE = 13-2-8
END SPAN CARRIED = 17-11-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIQ 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 979 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 522 (0.30")

CSI: TC=0.81 (A-B:1), BC=1.00 (F-G:1), WB=0.85 (B-F:1), SSI=0.66 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

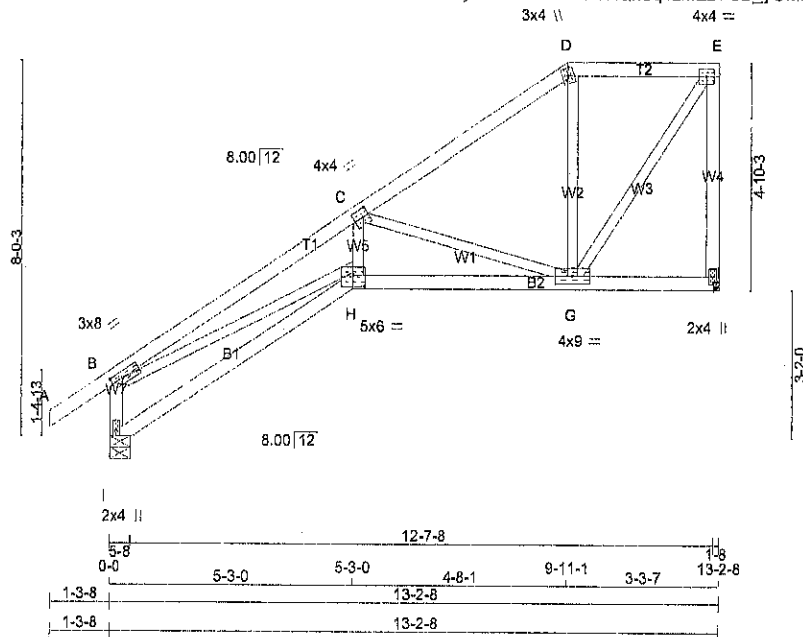
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253639	H14	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:49.3



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	8.0	1.50	3.25
C	TMVW-I	MT20	4.0	4.0	2.00	1.50
D	TTW+M	MT20	3.0	4.0		
E	TMVW-I	MT20	4.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-I	MT20	4.0	9.0		
H	BMVW-I	MT20	5.0	8.0	3.00	3.00
I	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
I	678	0	678	0
F	581	0	581	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
I	COMBINED	SNOW	LIVE	0/0	140/0	0/0
F	409	277/0	0/0	0/0	132/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED MOMENT (FT-K)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED MOMENT (FT-K)
FR-TO							
I-B	-832/0	0.0	0.0	0.07 (1)	7.81	B-H	0/1530
A-B	0/27	-70.5	-70.5	0.09 (1)	10.00	H-C	0/655
B-C	-1689/0	-70.5	-70.5	0.29 (1)	4.84	C-G	-1111/0
C-D	-400/0	-70.5	-70.5	0.18 (1)	6.25	G-D	-30/40
D-E	-314/0	-70.5	-70.5	0.10 (1)	6.25	G-E	0/548
F-E	-564/0	0.0	0.0	0.22 (1)	7.81		
I-H	0/0	-17.5	-17.5	0.14 (4)	10.00		
H-G	0/1374	-17.5	-17.5	0.27 (1)	10.00		
G-F	0/0	-17.5	-17.5	0.09 (4)	10.00		

TOTAL WEIGHT = 59 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.29 (B-C:1), BC=0.27 (G-H:1), WB=0.45 (C-G:1), SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

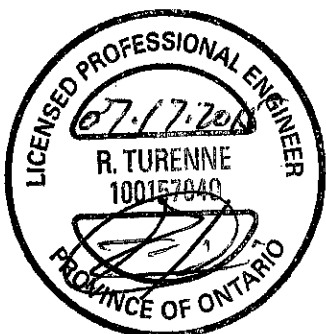
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.49 (H) (INPUT = 1.00)



A-1073868

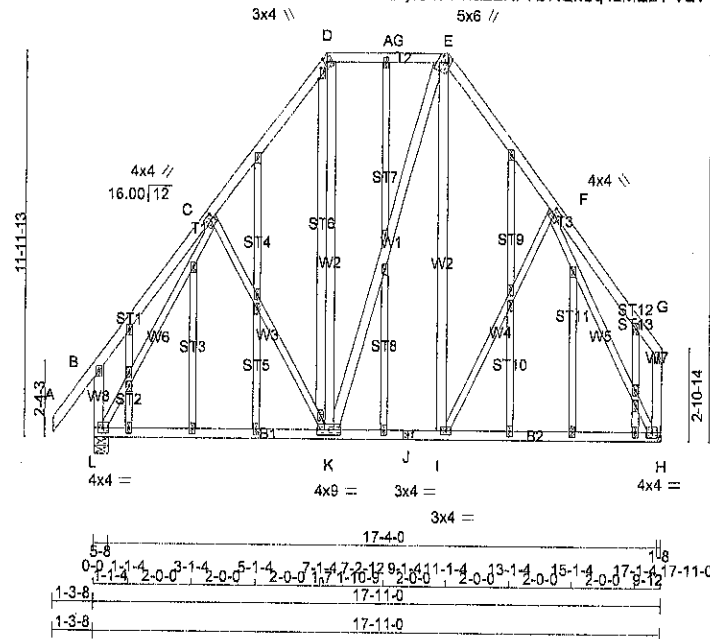
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253639	H13SG	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.820 S Apr 15 2015 MiTek Industries, Inc. Fri Jul 17 15:23:34 2015 Page 1

ID:AyrJx9Y?fa2EXF7DRQxSq4zMez1-VaV?LozNjQW0a5OmHRQauAylAsjtdB4blnC9yx877

Scale = 1:71.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
C - K	2x3	DRY	No.2	SPF	
I - F	2x3	DRY	No.2	SPF	
L - C	2x3	DRY	No.2	SPF	
F - H	2x3	DRY	No.2	SPF	

ALL GABLE WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
ST3	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-I	MT20	4.0	4.0	2.00	1.50
D	TTW+m	MT20	3.0	4.0	1.75	1.00
E	NP+w	MT20	2.0	4.0	4.25	0.75
F	TTWW+m	MT20	5.0	6.0	1.75	1.00
G	TMV+p	MT20	4.0	4.0	2.00	1.25
H	BMVW1-I	MT20	2.0	4.0		
I	BMVW1-I	MT20	3.0	4.0		
J	BS-I	MT20	3.0	4.0		
K	BMVWW-I	MT20	4.0	9.0		
L	NP+w	MT20	2.0	4.0	0.75	0.50
M	NP+w	MT20	2.0	4.0		

M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z, AA, AB, AC, AD, AE, AF, AG

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	901	0	801	0
H	603	0	803	0

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	634	435 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0
H	567	376 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 40	-70.5 -70.5 0.10 (1)	C-K	-83 / 12
B-C	0 / 24	-70.5 -70.5 0.15 (1)	K-E	0 / 159
C-D	-535 / 0	-70.5 -70.5 0.12 (1)	E-F	0 / 24
D-A	-307 / 0	-78.0 -78.0 0.20 (1)	F-H	0 / 133
A-G	-307 / 0	-78.0 -78.0 0.20 (1)	I-F	-40 / 24
E-F	-522 / 0	-70.5 -70.5 0.10 (1)	L-C	-739 / 0
F-G	0 / 23	-70.5 -70.5 0.13 (1)	F-H	-734 / 0
L-B	-194 / 0	0.0 0.0 0.02 (1)		
H-G	-89 / 0	0.0 0.0 0.01 (1)		
L-K	0 / 346	-17.5 -17.5 0.22 (4)		
K-J	0 / 300	-17.5 -17.5 0.21 (4)		
J-I	0 / 300	-17.5 -17.5 0.21 (4)		
I-H	0 / 317	-17.5 -17.5 0.20 (4)		

TOTAL WEIGHT = 173 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.06")

CSI: TC=0.20 (D-E:1), BC=0.22 (K-L:4), WB=0.89
(F-H:1), SS=0.12 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

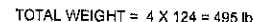
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.80)
JSI METAL= 0.29 (C) (INPUT = 1.00)

A-N073867

CONTINUED ON PAGE 2





DRY; SEASONED LUMBER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

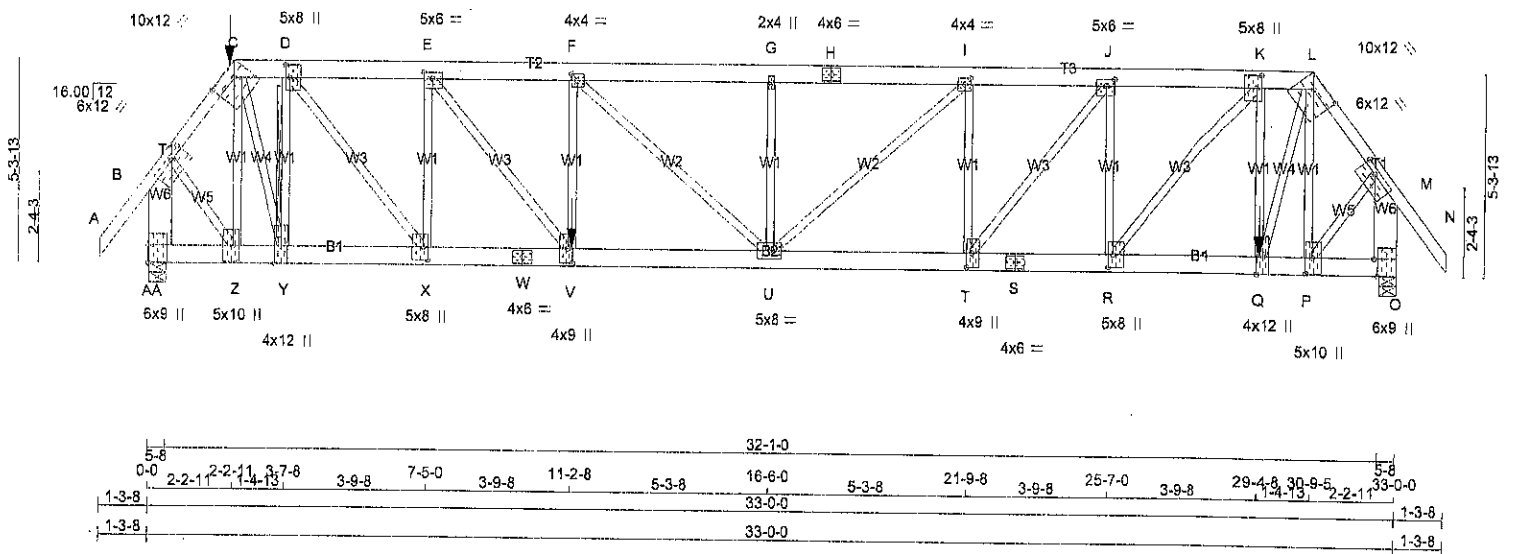
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX MAX UNBRAC	MEMB.	FORCE (LBS)	MAX MAX UNBRAC		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C-K	-83 / 12	0.10 (1)	
B-C	0 / 24	-70.5	-70.5	0.15 (1)	10.00	K-D	0 / 159	0.03 (1)	
C-D	-535 / 0	-70.5	-70.5	0.12 (1)	6.25	K-E	0 / 24	0.00 (1)	
D-M	-307 / 0	-78.0	-78.0	0.20 (1)	6.25	I-E	0 / 133	0.02 (4)	
M-E	-307 / 0	-78.0	-78.0	0.20 (1)	6.25	I-F	-40 / 24	0.05 (1)	
E-F	-522 / 0	-70.5	-70.5	0.10 (1)	6.25	L-C	-739 / 0	0.85 (1)	
F-G	0 / 23	-70.5	-70.5	0.13 (1)	10.00	F-H	-734 / 0	0.89 (1)	
L-B	-194 / 0	0.0	0.0	0.02 (1)	7.81				
H-G	-89 / 0	0.0	0.0	0.01 (1)	7.81				
L-K	0 / 346	-17.5	-17.5	0.22 (4)	10.00				
K-J	0 / 300	-17.5	-17.5	0.21 (4)	10.00				
J-I	0 / 300	-17.5	-17.5	0.21 (4)	10.00				
I-H	0 / 317	-17.5	-17.5	0.20 (4)	10.00				

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)



A-707386



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - H	2x6	DRY	No.2 SPF
H - L	2x6	DRY	No.2 SPF
L - N	2x4	DRY	No.2 SPF
AA - B	2x8	DRY	No.2 SPF
O - M	2x8	DRY	No.2 SPF
AA - W	2x6	DRY	No.2 SPF
W - S	2x6	DRY	No.2 SPF
S - O	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	6.0	12.0 1.50 4.75
C	TTWW-h	MT20	10.0	12.0 3.00 7.50
D	TMVW-t	MT20	6.0	8.0 4.00 1.50
E	TMVW-t	MT20	6.0	6.0 2.00 2.75
F	TMVW-t	MT20	4.0	4.0 2.00 1.75
G	TMVW-t	MT20	2.0	4.0
H	TS-t	MT20	4.0	6.0
I	TMVW-t	MT20	4.0	4.0 2.00 1.75
J	TMVW-t	MT20	6.0	6.0 2.00 2.75
K	TMVW-t	MT20	6.0	8.0 4.00 1.50
L	TTWW-h	MT20	10.0	12.0 3.00 7.50
M	TMVW-t	MT20	6.0	12.0 1.50 4.75
O	BMV1-t	MT20	6.0	9.0 Edge 1.25
P	BMVW-t	MT20	6.0	10.0 5.00 2.00
Q	BMVW-t	MT20	4.0	12.0 Edge
R	BMVW-t	MT20	6.0	8.0 4.00 1.50
S	BS-t	MT20	4.0	6.0
T	BMVW-t	MT20	4.0	9.0 4.50 1.50
U	BMVW-t	MT20	6.0	8.0
V	BMVW-t	MT20	4.0	9.0 4.50 1.50
W	BS-t	MT20	4.0	6.0
X	BMVW-t	MT20	6.0	8.0 4.00 1.50
Y	BMVW-t	MT20	4.0	12.0 Edge
Z	BMVW-t	MT20	6.0	10.0 5.00 2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
AA	3403	0	3403	0	0	5-8	3-13
O	5398	0	5398	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
AA	2467	1274 / 0	0 / 0	0 / 0	1194 / 0
O	3837	2404 / 0	0 / 0	0 / 0	1433 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-Y
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	UNBRAC	FR-TO	
A-B	0 / 40	-70.5 -70.5 0.11 (1)	10.00	Z-C	-729 / 0
B-C	-2589 / 0	-70.5 -70.5 0.18 (1)	4.14	C-Y	0 / 2532
C-D	-2237 / 0	-138.7 -138.7 0.12 (4)	5.36	Y-D	-2698 / 0
D-E	-4171 / 0	-138.7 -138.7 0.30 (1)	3.67	D-X	0 / 3145
E-F	-5481 / 0	-138.7 -138.7 0.32 (1)	3.52	X-E	-2289 / 0
F-G	-5551 / 0	-70.5 -70.5 0.34 (1)	3.49	E-V	0 / 2128
G-H	-5551 / 0	-70.5 -70.5 0.34 (1)	3.49	V-F	-481 / 237
H-I	-5551 / 0	-70.5 -70.5 0.34 (1)	3.49	F-U	-395 / 96
I-J	-5081 / 0	-70.5 -70.5 0.25 (1)	3.72	U-G	-397 / 0
J-K	-4439 / 0	-70.5 -70.5 0.34 (1)	3.82	U-I	0 / 675
K-L	-3377 / 0	-70.5 -70.5 0.27 (1)	4.37	T-I	-751 / 0
L-M	-3998 / 0	-70.5 -70.5 0.29 (1)	3.29	T-J	0 / 1011
M-N	0 / 40	-70.5 -70.5 0.11 (1)	10.00	R-J	-1165 / 0
AA-B	-3500 / 0	0.0 0.0 0.25 (1)	6.35	R-K	0 / 1728
O-M	-5308 / 0	0.0 0.0 0.38 (1)	6.30	Q-K	-1251 / 0
AA-Z	0 / 0	-34.5 -34.5 0.24 (1)	10.00	Q-L	0 / 3586
Z-Y	0 / 1536	-34.5 -34.5 0.43 (1)	10.00	P-L	-573 / 0
Y-X	0 / 2237	-34.5 -34.5 0.37 (1)	10.00	B-Z	0 / 1982
X-W	0 / 4171	-34.5 -34.5 0.62 (1)	10.00	P-M	0 / 3075
W-V	0 / 4171	-34.5 -34.5 0.62 (1)	10.00		
V-U	0 / 5481	-17.5 -17.5 0.79 (1)	10.00		
U-T	0 / 5081	-17.5 -17.5 0.72 (1)	10.00		
T-S	0 / 4439	-17.5 -17.5 0.62 (1)	10.00		
S-R	0 / 4439	-17.5 -17.5 0.62 (1)	10.00		
R-Q	0 / 3376	-17.5 -17.5 0.69 (1)	10.00		
Q-P	0 / 2383	-263.9 -263.9 0.68 (1)	10.00		
P-O	0 / 0	-263.9 -263.9 0.37 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)		FACE		TYPE	
JT	LOC.	LOC1	MAX-	MAX+	DIR.
C	2-2-11	-114	-114	---	FRONT VERT
Q	29-4-8	-2756	-2756	---	FRONT VERT
V	11-2-8	-1133	-1133	---	FRONT VERT

TOTAL WEIGHT = 209 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 11-2-8 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 29-4-8
START SPAN CARRIED = 13-2-8
END DISTANCE = 33-0-0
END SPAN CARRIED = 13-2-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/943 (0.42")

CSI: TC=0.38 (M-O:1), BC=0.79 (U-V:1), WB=0.89 (L-Q:1), SS=0.55 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

A-11073865



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253639	H01-Cond2	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Fri Jul 17 15:02:01 2015 Page 2
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-sqsC gKKqYcTyO2G1OkN VuLceH7QrBIHddsKbyx8Jk

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
AA	BMV1+1	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD, AND 1288.4 lbs FACTORED DOWN AT 11-2-8, AND 2756.3 lbs FACTORED DOWN AT 28-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES: (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.91 (S) (INPUT = 1.00)



A-LTD/3865(2)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253639	H01-Cond1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.820 S Apr 15 2015 MITek Industries, Inc. Fri Jul 17 15:02:00 2015 Page 2
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-OelpmKJi3FUcLET4ThC8SiM9YEz3hObc2zuIn9yx8Jl

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
AA	BMV1+t	MT20	6.0	9.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 114.2 lbs FACTORED DOWN AT 30-8-8,
AND 114.2 lbs FACTORED DOWN AT 2-2-11 ON
TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

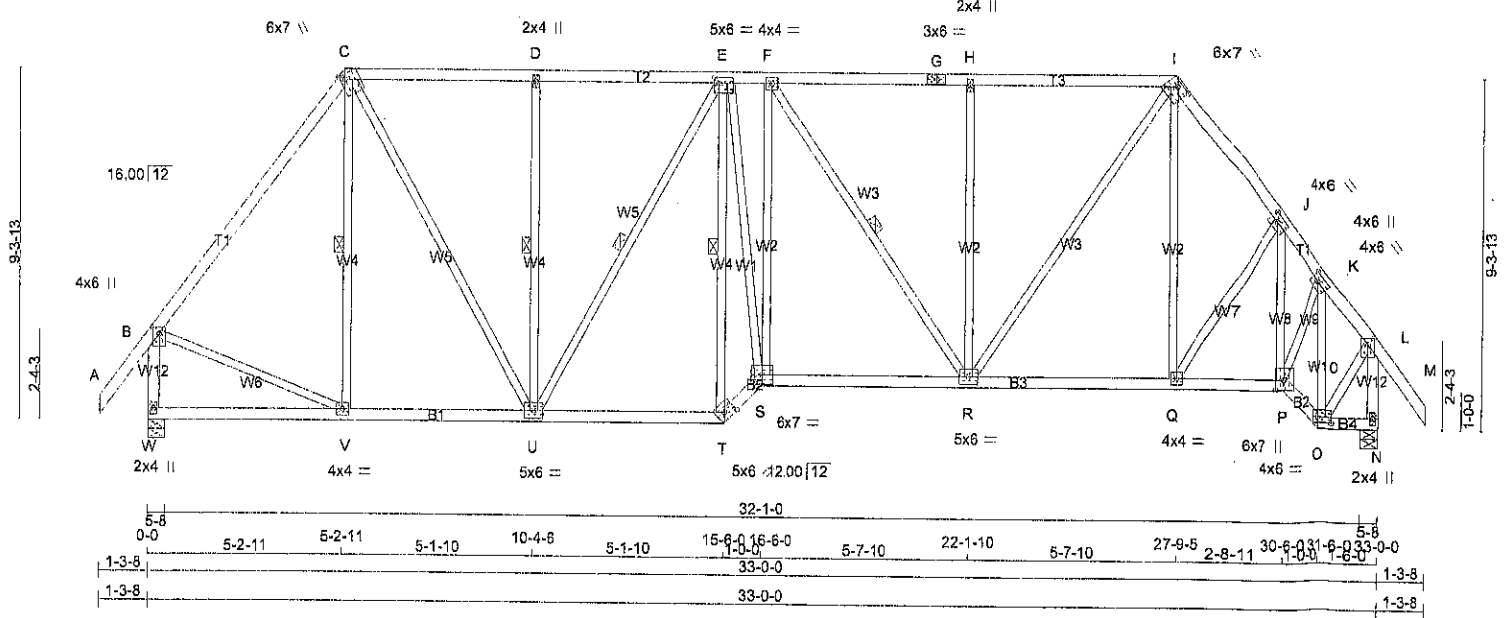
NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



A-15073864(2)

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Fri Jul 17 14:25:24 2015 Page 1
ID: AyrJx9Y7fa2EXF7DRQxSq4zMEz1-hAcetRkDDWpCtibOqUpASCpyOZe9FuN4Jw6iRyx8r
Scale = 1:61.4



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - M	2x4	DRY	No.2
W - B	2x4	DRY	No.2
N - L	2x4	DRY	No.2
W - T	2x4	DRY	No.2
T - S	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - N	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.00	2.00
C TTWW+m	MT20	6.0	7.0	Edge	1.50
D TMW+w	MT20	2.0	4.0		
E TMWWW-t	MT20	5.0	6.0	1.75	1.50
F TMWW-t	MT20	4.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMW+w	MT20	2.0	4.0		
I TTWW-h	MT20	6.0	7.0	1.50	3.75
J TMWW-t	MT20	4.0	6.0	2.00	2.50
K TMWW-t	MT20	4.0	6.0	2.00	2.50
L TMVW+p	MT20	4.0	6.0	2.00	2.00
N BMV1+p	MT20	2.0	4.0		
O BBWW-t	MT20	4.0	6.0	1.75	4.25
P BBWW+p	MT20	6.0	7.0	3.25	2.75
Q BMWW-t	MT20	4.0	4.0		
R BMWWW-t	MT20	5.0	6.0		
S BBWW-t	MT20	6.0	7.0		
T BBW-h	MT20	5.0	6.0	2.25	3.50
U BMWWW-t	MT20	6.0	6.0		
V BMWW-t	MT20	4.0	4.0		
W BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
W	1548	0	1548	0	5-8	1-11		
N	1553	0	1553	0	5-8	1-11		

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
W	1089	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0	0 / 0
N	1091	753 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-V, D-U, E-U, E-T, F-R, END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH (FT)	MAX CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-70.5 -70.5	0.10 (1)	V-C	-159 / 28	10.00	0.09 (1)
B-C	-1202 / 0	-70.5 -70.5	0.45 (1)	C-U	0 / 903	5.35	0.20 (1)
C-D	-1164 / 0	-70.5 -70.5	0.29 (1)	U-D	-396 / 0	5.55	0.23 (1)
D-E	-1164 / 0	-70.5 -70.5	0.29 (1)	E-U	-350 / 0	5.55	0.27 (1)
E-F	-1502 / 0	-70.5 -70.5	0.20 (1)	T-E	-1264 / 0	5.14	0.72 (1)
F-G	-1349 / 0	-70.5 -70.5	0.39 (1)	E-S	0 / 1329	5.10	0.30 (1)
G-H	-1349 / 0	-70.5 -70.5	0.39 (1)	S-F	0 / 61	5.10	0.02 (4)
H-I	-1349 / 0	-70.5 -70.5	0.40 (1)	F-R	-288 / 0	5.10	0.18 (1)
I-J	-1351 / 0	-70.5 -70.5	0.09 (1)	R-H	-458 / 0	5.51	0.62 (1)
J-K	-1255 / 0	-70.5 -70.5	0.06 (1)	H-R	0 / 956	5.68	0.22 (1)
K-L	-898 / 0	-70.5 -70.5	0.10 (1)	Q-I	0 / 73	5.68	0.03 (4)
L-M	0 / 40	-70.5 -70.5	0.10 (1)	Q-J	0 / 26	5.68	0.01 (1)
W-B	-1511 / 0	0.0 0.0	0.19 (1)	P-J	-262 / 0	5.68	0.09 (1)
N-L	-1539 / 0	0.0 0.0	0.20 (1)	P-K	0 / 824	5.68	0.19 (1)
				O-K	-1102 / 0	5.68	0.29 (1)
W-V	0 / 0	-17.5 -17.5	0.11 (4)	B-V	0 / 765	10.00	0.17 (1)
V-U	0 / 716	-17.5 -17.5	0.19 (1)	O-L	0 / 822	10.00	0.19 (1)
U-T	0 / 1335	-17.5 -17.5	0.27 (1)				
T-S	0 / 1867	-17.5 -17.5	0.30 (1)				
S-R	0 / 1501	-17.5 -17.5	0.31 (1)				
R-Q	0 / 798	-17.5 -17.5	0.20 (4)				
Q-P	0 / 763	-17.5 -17.5	0.17 (1)				
P-O	0 / 671	-17.5 -17.5	0.11 (1)				
O-N	0 / 0	-17.5 -17.5	0.01 (4)				

TOTAL WEIGHT = 192 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.45 (B-C:1), BC=0.31 (R-S:1), WB=0.72 (E-T:1), SSI=0.19 (H:1)

DOL LUMBER=1.00 NAIL=1.00LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.35 (E) (INPUT = 1.00)



A-15073863

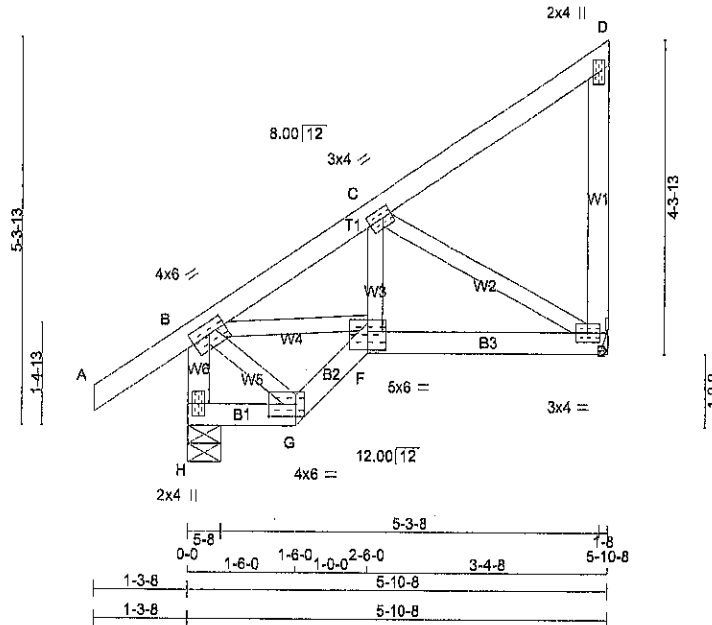


A-15073862 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253636	J01C	8	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Fri Jul 17 13:44:53 2015 Page 1
ID: AyrJx9Y?fa2EXF7DRQxSq4zMEz1-MUDdFCJLbX7k9FowQI5QYDW_3wd3xJY3bYzuryx8Ri
Scale: 3/8"=1'



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4 DRY	No.2	SPF		
A - D	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
	DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVWW-I	MT20	4.0	6.0	1.75	3.00
C	TMVWW-I	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	355	0	355	0
E	258	0	258	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	248	181 / 0	0 / 0
E	182	123 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
H-B	-342 / 0	0.0	0.0	0.04 (1)	7.61	B-G	0 / 18	0.01 (4)	
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B-F	0 / 238	0.06 (1)	
B-C	-287 / 0	-70.5	-70.5	0.10 (1)	6.25	F-C	0 / 57	0.02 (4)	
C-D	-12 / 0	-70.5	-70.5	0.10 (1)	6.25	C-E	-284 / 0	0.07 (1)	
E-D	-93 / 0	0.0	0.0	0.03 (1)	7.81				
H-G	0 / 0	-17.5	-17.5	0.01 (4)	10.00				
G-F	0 / 20	-17.5	-17.5	0.01 (4)	10.00				
F-E	0 / 248	-17.5	-17.5	0.08 (4)	10.00				

TOTAL WEIGHT = 8 X 30 = 238 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.10 (B-C:1), BC=0.08 (E-F:4), WB=0.07 (C-E:1), SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 364 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

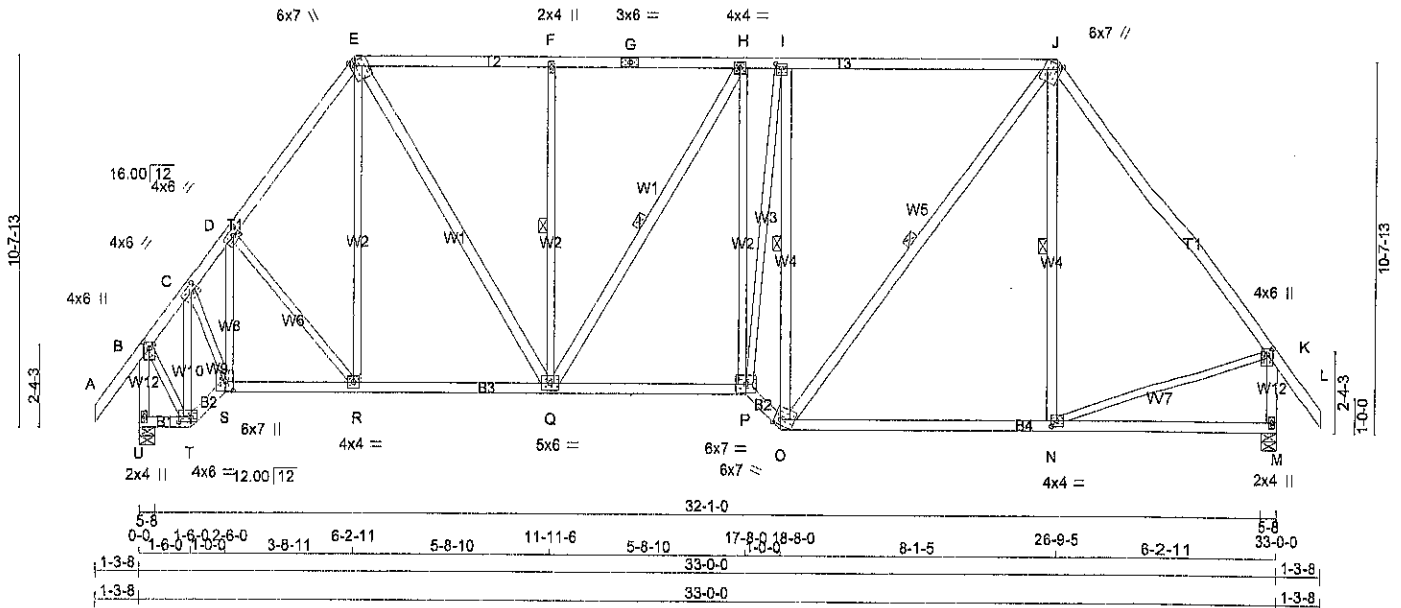


A-12073861

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253636	H05T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 M/Tek Industries, Inc. Fri Jul 17 14:07:44 2015 Page 1
ID: AyrJx9Y7fa2EXF7DRQxSq4zMEz1-jEZwP7vKrrGcjsYiksobUgVe_inPonLKp90Wtyx96D
4x4 =
Scale = 1:66.4



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	2100F 1.8E	SPF	
E - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
J - L	2x4	DRY	2100F 1.8E	SPF	
U - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
U - T	2x4	DRY	No.2	SPF	
T - S	2x4	DRY	No.2	SPF	
S - P	2x4	DRY	No.2	SPF	
P - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
E - Q	2x4	DRY	No.2	SPF	
Q - H	2x4	DRY	No.2	SPF	
O - I	2x4	DRY	No.2	SPF	
O - J	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMVW-t	MT20	4.0	6.0	2.00	2.50
D	TMVW-t	MT20	4.0	6.0	2.00	2.50
E	TTVW+m	MT20	6.0	7.0	Edge 1.50	
F	TMVW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TMVW-t	MT20	4.0	4.0	1.60	2.00
J	TTVW+m	MT20	6.0	7.0	Edge 1.50	
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMVW-t	MT20	4.0	4.0	2.00	1.75
O	BBVW-m	MT20	6.0	7.0	Edge 2.25	
P	BBVW-t	MT20	6.0	7.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BMVW-t	MT20	4.0	4.0		
S	BBVW+p	MT20	6.0	7.0	3.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	UPLIFT	IN-SX
U 1553	0	1553	0
M 1548	0	1548	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERMILIVE	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE			
U 1091	753 / 0	0 / 0	0 / 0	339 / 0	0 / 0
M 1089	750 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-Q, H-Q, I-Q, J-Q, J-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO							FR-TO			
A-B	0 / 40		-70.5	-70.5	0.07 (1)	10.00	T-C	-1085 / 0	0.29 (1)	
B-C	-897 / 0		-70.5	-70.5	0.07 (1)	6.25	C-S	0 / 866	0.19 (1)	
C-D	-1234 / 0		-70.5	-70.5	0.08 (1)	6.25	S-D	-285 / 0	0.10 (1)	
D-E	-1333 / 0		-70.5	-70.5	0.09 (1)	6.25	D-R	-13 / 5	0.01 (1)	
E-F	-1211 / 0		-70.5	-70.5	0.41 (1)	5.29	R-E	0 / 92	0.03 (4)	
F-G	-1211 / 0		-70.5	-70.5	0.41 (1)	5.29	E-Q	0 / 812	0.13 (1)	
G-H	-1211 / 0		-70.5	-70.5	0.41 (1)	5.29	Q-F	-471 / 0	0.29 (1)	
H-I	-1283 / 0		-70.5	-70.5	0.67 (1)	4.47	F-H	-134 / 0	0.09 (1)	
I-J	-1187 / 0		-70.5	-70.5	0.80 (1)	4.61	P-H	0 / 250	0.06 (1)	
J-K	-1209 / 0		-70.5	-70.5	0.40 (1)	6.25	P-I	0 / 977	0.22 (1)	
K-L	0 / 40		-70.5	-70.5	0.07 (1)	10.00	O-I	-1687 / 0	0.96 (1)	
U-B	-1539 / 0		0.0	0.0	0.20 (1)	6.63	O-J	0 / 752	0.12 (1)	
M-K	-1510 / 0		0.0	0.0	0.19 (1)	6.68	N-J	-67 / 103	0.04 (1)	
U-T	0 / 0		-17.5	-17.5	0.01 (4)	10.00	B-T	0 / 817	0.18 (1)	
T-S	0 / 667		-17.5	-17.5	0.11 (1)	10.00	N-K	0 / 756	0.17 (1)	
S-R	0 / 793		-17.5	-17.5	0.18 (1)	10.00				
R-Q	0 / 786		-17.5	-17.5	0.20 (4)	10.00				
Q-P	0 / 1280		-17.5	-17.5	0.28 (1)	10.00				
P-O	0 / 1646		-17.5	-17.5	0.27 (1)	10.00				
O-N	0 / 722		-17.5	-17.5	0.33 (4)	10.00				
N-M	0 / 0		-17.5	-17.5	0.27 (4)	10.00				

TOTAL WEIGHT = 208 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.17")

CSI: TC=0.80 (I-J:1), BC=0.33 (N-O:4), WB=0.86 (I-O:1), SSI=0.30 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.29 (K) (INPUT = 1.00)

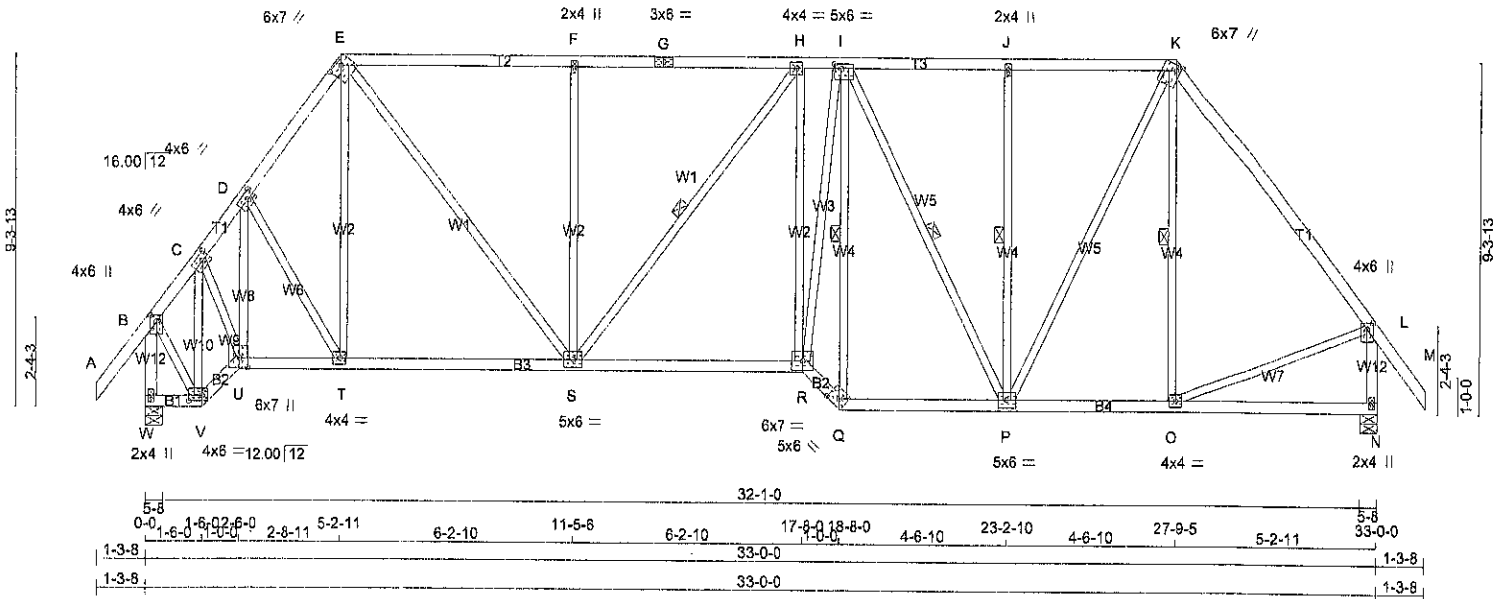


A-10073860

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253636	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-NpptUvGW0lUhJGtxSpO9nCgXQgA5yqd7Ph20hyx95m
Scale = 1:61.2



TOTAL WEIGHT = 192 lb (M)

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.00	2.00
C TMVW-t	MT20	4.0	6.0	2.00	2.50
D TMVW-t	MT20	4.0	6.0	2.00	2.50
E TMVW-h	MT20	6.0	7.0	1.50	3.75
F TMVW-w	MT20	2.0	4.0		
G TS-t	MT20	3.0	8.0		
H TMVW-t	MT20	4.0	4.0		
I TMVW-t	MT20	5.0	6.0	1.50	2.00
J TMVW-w	MT20	2.0	4.0		
K TMVW+m	MT20	6.0	7.0	Edge	1.50
L TMVW+p	MT20	4.0	6.0	2.00	2.00
N BMV1+p	MT20	2.0	4.0		
O BMVW-t	MT20	4.0	4.0		
P BMVW-t	MT20	5.0	6.0		
Q BW-h	MT20	5.0	8.0	2.25	3.50
R BWW-l	MT20	6.0	7.0		
S BMVW-t	MT20	5.0	6.0		
T BMVW-t	MT20	4.0	4.0		
U BWVW+p	MT20	6.0	7.0	3.25	2.75
V BWVW-l	MT20	4.0	6.0	1.75	4.25
W BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
W	1553	0	1553	0	5-8	1-11
N	1548	0	1548	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM LIVE				
W	1091	753 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
N	1089	750 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF H-S, I-Q, I-P, J-P, K-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	V-C -1102 / 0	0.29 (1)
B-C	-896 / 0	-70.5	-70.5	0.10 (1)	6.25	C-U 0 / 824	0.19 (1)
C-D	-1255 / 0	-70.5	-70.5	0.06 (1)	5.88	U-D -264 / 0	0.09 (1)
D-E	-1362 / 0	-70.5	-70.5	0.08 (1)	5.61	D-T 0 / 27	0.01 (1)
E-F	-1391 / 0	-70.5	-70.5	0.49 (1)	4.91	T-E 0 / 79	0.03 (4)
F-G	-1392 / 0	-70.5	-70.5	0.49 (1)	4.90	E-S 0 / 965	0.22 (1)
G-H	-1392 / 0	-70.5	-70.5	0.49 (1)	4.90	S-F -505 / 0	0.69 (1)
H-I	-1498 / 0	-70.5	-70.5	0.17 (1)	5.18	S-H -177 / 0	0.13 (1)
I-J	-1121 / 0	-70.5	-70.5	0.22 (1)	5.73	R-H -76 / 27	0.10 (1)
J-K	-1121 / 0	-70.5	-70.5	0.22 (1)	5.73	R-I 0 / 1449	0.33 (1)
K-L	-1202 / 0	-70.5	-70.5	0.45 (1)	5.36	Q-I -1248 / 0	0.71 (1)
L-M	0 / 40	-70.5	-70.5	0.10 (1)	10.00	I-P -435 / 0	0.32 (1)
W-B	-1539 / 0	0.0	0.0	0.20 (1)	6.63	P-J -343 / 0	0.20 (1)
N-L	-1511 / 0	0.0	0.0	0.19 (1)	6.68	P-K 0 / 898	0.20 (1)
						O-K -161 / 25	0.09 (1)
W-V	0 / 0	-17.5	-17.5	0.01 (4)	10.00	B-V 0 / 822	0.19 (1)
V-U	0 / 671	-17.5	-17.5	0.11 (1)	10.00	O-L 0 / 765	0.17 (1)
U-T	0 / 783	-17.5	-17.5	0.18 (1)	10.00		
T-S	0 / 798	-17.5	-17.5	0.23 (4)	10.00		
S-R	0 / 1499	-17.5	-17.5	0.33 (1)	10.00		
R-Q	0 / 1838	-17.5	-17.5	0.30 (1)	10.00		
Q-P	0 / 1314	-17.5	-17.5	0.26 (1)	10.00		
P-O	0 / 716	-17.5	-17.5	0.18 (1)	10.00		
O-N	0 / 0	-17.5	-17.5	0.11 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.49 (E-F:1), BC=0.33 (R-S:1), WB=0.71 (I-Q:1), SSI=0.21 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	818	354
	1667	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)



A-10073859

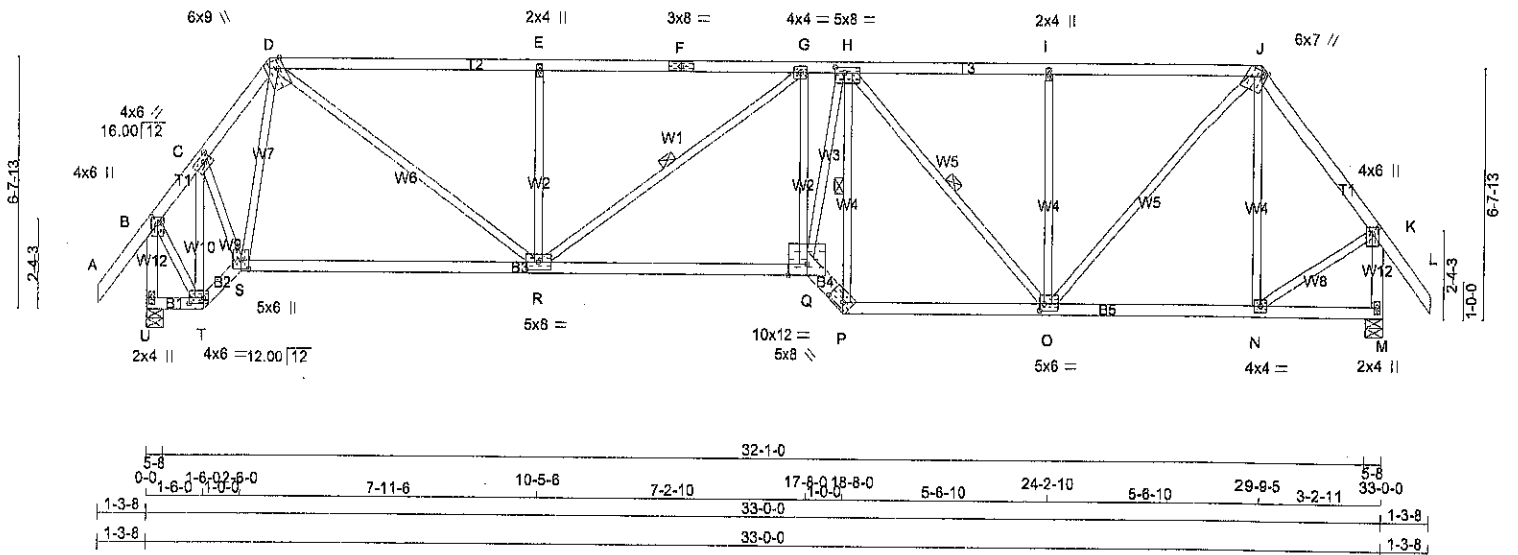


A-N273858

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253636	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-yvYU1AHSQg9ZtWDE18Nowbq3snxsU6NeKJHwYx9Rr
Scale = 1:61.2



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - J	2x4	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
U - T	2x4	DRY	No.2	SPF	
T - S	2x4	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
Q - P	2x6	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-l	MT20	4.0	6.0	2.00	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-l	MT20	3.0	8.0		
G	TMWW-l	MT20	4.0	4.0		
H	TMWWW-l	MT20	5.0	8.0	1.75	3.00
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-l	MT20	4.0	4.0		
O	BMWWW-l	MT20	5.0	6.0	2.25	1.50
P	BBW-h	MT20	5.0	8.0	1.75	5.00
Q	BBWW-l	MT20	10.0	12.0	Edge	
R	BMWWW-l	MT20	5.0	8.0		
S	BBWW+p	MT20	5.0	6.0	2.75	2.25
T	BBWW-l	MT20	4.0	6.0	1.75	4.50
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
U	1553	0	0	5-8
M	1548	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
U	1091	753 / 0	0 / 0	0 / 0	339 / 0	0 / 0
M	1089	750 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-R, H-P, H-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX CS1 (LC)	UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO				FR-TO		
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	T-C	-1112 / 0	0.29 (1)
B-C	-894 / 0	-70.5	-70.5	0.10 (1)	6.25	C-S	0 / 762	0.17 (1)
C-D	-1276 / 0	-70.5	-70.5	0.07 (1)	5.65	S-D	-164 / 0	0.08 (1)
D-E	-2020 / 0	-70.5	-70.5	0.78 (1)	3.89	D-R	0 / 1538	0.35 (1)
E-F	-2020 / 0	-70.5	-70.5	0.78 (1)	3.89	R-E	-580 / 0	0.28 (1)
F-G	-2020 / 0	-70.5	-70.5	0.78 (1)	3.89	G-R	-331 / 0	0.18 (1)
G-H	-2283 / 0	-70.5	-70.5	0.28 (1)	4.24	Q-G	-85 / 31	0.04 (1)
H-I	-1509 / 0	-70.5	-70.5	0.37 (1)	4.93	Q-H	0 / 2071	0.47 (1)
I-J	-1509 / 0	-70.5	-70.5	0.37 (1)	4.93	P-H	-1778 / 0	0.51 (1)
J-K	-1142 / 0	-70.5	-70.5	0.15 (1)	5.79	H-O	-582 / 0	0.28 (1)
K-L	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O-I	-432 / 0	0.32 (1)
U-B	-1539 / 0	0.0	0.0	0.20 (1)	6.83	O-J	0 / 1262	0.28 (1)
M-K	-1531 / 0	0.0	0.0	0.20 (1)	6.86	N-J	-312 / 0	0.23 (1)
						B-T	0 / 829	0.19 (1)
						N-K	0 / 789	0.18 (1)
U-T	0 / 0	-17.5	-17.5	0.01 (4)	10.00			
T-S	0 / 677	-17.5	-17.5	0.11 (1)	10.00			
S-R	0 / 781	-17.5	-17.5	0.35 (4)	10.00			
R-Q	0 / 2285	-17.5	-17.5	0.52 (1)	10.00			
Q-P	0 / 2628	-17.5	-17.5	0.31 (1)	10.00			
P-O	0 / 1888	-17.5	-17.5	0.37 (1)	10.00			
O-N	0 / 675	-17.5	-17.5	0.18 (4)	10.00			
N-M	0 / 0	-17.5	-17.5	0.08 (4)	10.00			

TOTAL WEIGHT = 162 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(66 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.10")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.14")
ALLOWABLE DEFL.(TL) = $L/360$ (1.10")
CALCULATED VERT. DEFL.(TL) = $L/899$ (0.29")

CSI: TC=0.78 (D-E:1), BC=0.52 (Q-R:1),
WB=0.51 (H-P:1), SSI=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
1667	822	2264
1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (D) (INPUT = 0.90)

JSI METAL=0.42 (H) (INPUT = 1.00)



A-1073857



PLATES (table is in inches)						
J	TYPE	PLATES	W	LEN	Y	X
B	TMW-V-I	MT20	5.0	8.0	2.00	2.00
C	BMW-W-I	MT20	5.0	6.0	2.00	1.60
D	TTWW+M	MT20	8.0	9.0	2.75	1.75
E	TMW-V-I	MT20	5.0	6.0	2.50	2.76
F	TMW+W	MT20	2.0	4.0		
G	TS-I	MT20	5.0	4.0		
H	TMW-W-I	MT20	5.0	8.0	2.25	1.75
I	TMW-W-I	MT20	4.0	6.0		
J	TTWW+M	MT20	8.0	9.0	2.75	1.75
K	TMW-V-I	MT20	5.0	8.0	2.00	2.00
L	BMW-V+P	MT20	3.0	5.0		
M	BMW-W-I	MT20	4.0	6.0	2.75	1.75
N	BMW-W-I	MT20	6.0	7.0	2.75	2.60
O	BMW-W-I	MT20	10.0	12.0	Edge	4.00
P	BMW-W-I	MT20	10.0	12.0	4.50	6.00
Q	BMW-W-I	MT20	5.0	8.0	2.50	3.26
R	BMW-W-I	MT20	5.0	6.0	2.00	2.00

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
K	30-9-5	-114	-114	---	FRONT	VERT	TOTAL

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 648 (0.61")

CSI: TC=0.59 (I-J:1), BC=0.99 (R-S:1), WB=0.90 (I-Q:1), SS=0.33 (J-K:1)

A-1073856



PLATES (table is in inches)							DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00			
JT	TYPE	PLATES	W	LEN	Y	X	COMPANION LIVE LOAD FACTOR = 0.50			
U	BBWW-I	MT20	6.0	7.0	4.00	3.00	AUTOSOLVE HEELS OFF			
V	BBWW-I	MT20	6.0	7.0	2.00	5.50	TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
W	BMV1+p	MT20	3.0	5.0			NAIL VALUES			
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.							PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
HANGERS NOTES							MAX MIN MAX MIN MAX MIN			
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11, AND 114.2 lbs FACTORED DOWN AT 30-9-5 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.							MT20 618 354 1667 822 2264 1656			
NOTES- (1)							PLATE PLACEMENT TOL. = 0.250 inches			
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.							PLATE ROTATION TOL. = 5.0 Deg.			
							JSI GRIP= 0.90 (T) (INPUT = 0.90) JSI METAL= 0.71 (I) (INPUT = 1.00)			

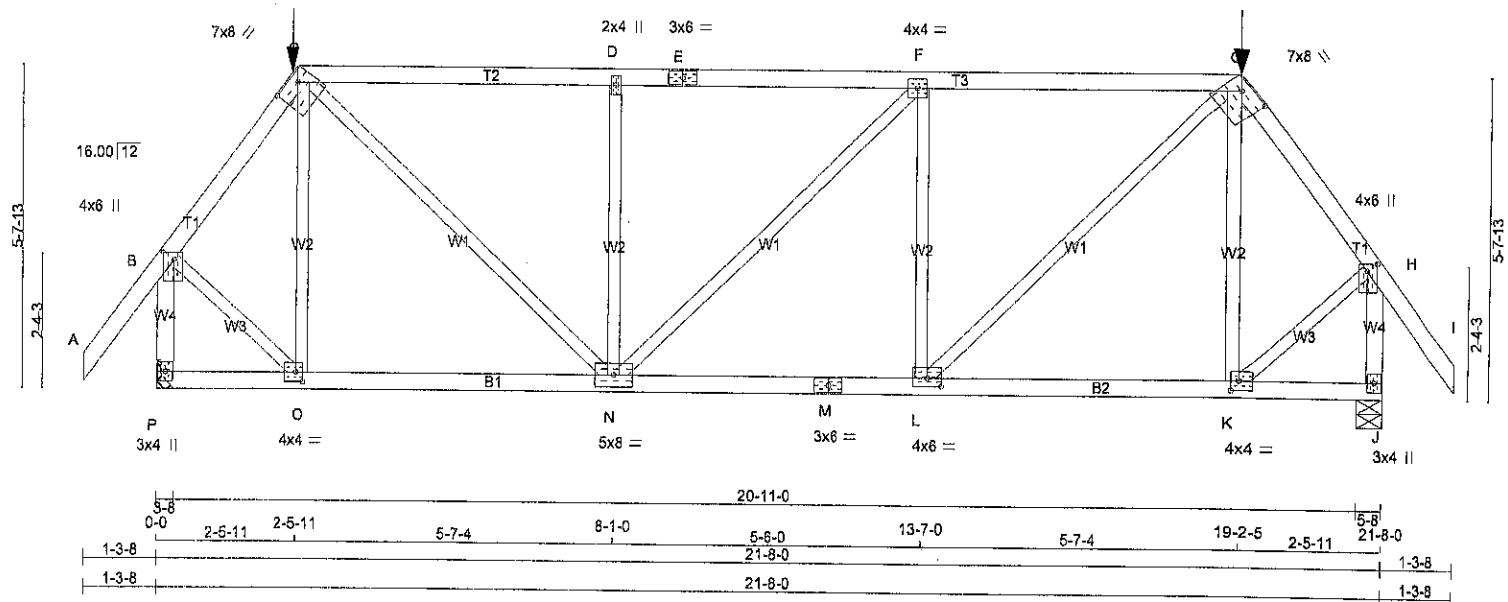


A-N273856(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253635	H12	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:40.5



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	1.50	2.25
C	TTWW-h	MT20	7.0	8.0	1.75	5.00
D	TMW+w	MT20	2.0	4.0		
E	TS-l	MT20	3.0	6.0		
F	TMWW-l	MT20	4.0	4.0		
G	TTWW-h	MT20	7.0	8.0	1.75	5.00
H	TMVW+p	MT20	4.0	6.0	1.50	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-l	MT20	4.0	4.0	2.00	1.50
L	BMWW-l	MT20	4.0	6.0	1.75	3.00
M	BS-l	MT20	3.0	6.0		
N	BMWW-l	MT20	5.0	8.0		
O	BMWW-l	MT20	4.0	4.0	2.00	1.50
P	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 141.6 lbs FACTORED DOWN AT 19-2-5, AND 141.6 lbs FACTORED DOWN AT 2-5-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
P	2045	0	2045	0	0	0
J	2045	0	2045	0	0	0

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
P	1440	982 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
J	1440	982 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT, OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	O-C	-483 / 0 0.24 (1)
B-C	-1467 / 0	-70.5	-70.5	0.12 (1)	5.27	C-N	0 / 1572 0.39 (1)
C-D	-2003 / 0	-147.5	-147.5	0.82 (1)	3.54	N-D	-879 / 0 0.44 (1)
D-E	-2004 / 0	-147.5	-147.5	0.82 (1)	3.50	N-F	-3 / 0 0.00 (1)
E-F	-2004 / 0	-147.5	-147.5	0.82 (1)	3.50	L-F	-880 / 0 0.44 (1)
F-G	-2006 / 0	-147.5	-147.5	0.83 (1)	3.50	L-G	0 / 1576 0.39 (1)
G-H	-1488 / 0	-70.5	-70.5	0.12 (1)	5.27	K-G	-485 / 0 0.24 (1)
H-I	0 / 40	-70.5	-70.5	0.11 (1)	10.00	B-O	0 / 1095 0.27 (1)
P-B	-2028 / 0	0.0	0.0	0.28 (1)	5.88	K-H	0 / 1094 0.27 (1)
J-H	-2027 / 0	0.0	0.0	0.28 (1)	5.88		
P-O	0 / 0	-36.6	-36.6	0.19 (4)	10.00		
O-N	0 / 862	-36.6	-36.6	0.32 (4)	10.00		
N-M	0 / 2006	-36.6	-36.6	0.48 (1)	10.00		
M-L	0 / 2006	-36.6	-36.6	0.48 (1)	10.00		
L-K	0 / 862	-36.6	-36.6	0.32 (4)	10.00		
K-J	0 / 0	-36.6	-36.6	0.18 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-5-11	-142	-142	---	FRONT	VERT	TOTAL
G	19-2-5	-142	-142	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 103 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 2-5-11
END SETBACK = 6-4-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.83 (F-G:1), BC=0.48 (L-N:1), WB=0.44 (F-L:1), SSI=0.42 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (L) (INPUT = 0.90)
JSI METAL=0.67 (M) (INPUT = 1.00)



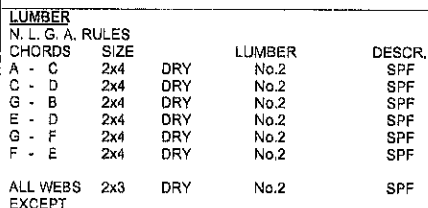
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Alpa Roof Truss, Maple

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Scale: 3/16"=1'



DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	MTWV+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	MTWV+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BBWVV+p	MT20	5.0	6.0	2.75	3.00
G	BMV1+p	MT20	2.0	4.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

FACTORED			MAXIMUM FACTORED			INPUT	REQRD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	679	0	679	0	0	5-8	1-8
E	480	0	480	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	406	288 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
E	338	229 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	F-C	0 / 193	0.05 (4)	
B-C	-381 / 0	-70.5	-70.5	0.30 (1)	6.25	B-F	0 / 224	0.05 (1)	
C-D	-381 / 0	-70.5	-70.5	0.25 (1)	6.25	F-D	0 / 220	0.05 (1)	
G-B	-630 / 0	0.0	0.0	0.07 (1)	7.81				
E-D	-434 / 0	0.0	0.0	0.07 (1)	7.81				
G-F	0 / 0	-17.5	-17.5	0.16 (4)	10.00				
F-E	0 / 0	-17.5	-17.5	0.14 (4)	10.00				

TOTAL WEIGHT = 2 X 58 = 116 lb

[MIF]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	#	21.0	PSF
	DL	#	3.0	PSF
BOT CH.	LL	#	0.0	PSF
	DL	#	7.0	PSF
TOTAL LOAD			#	31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.30 (B-C:1), BC=0.16 (F-G:4),
WB=0.05 (B-F:1), SS=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (G) (INPUT = 0.90)
JSI METAL= 0.13 (G) (INPUT = 1.00)

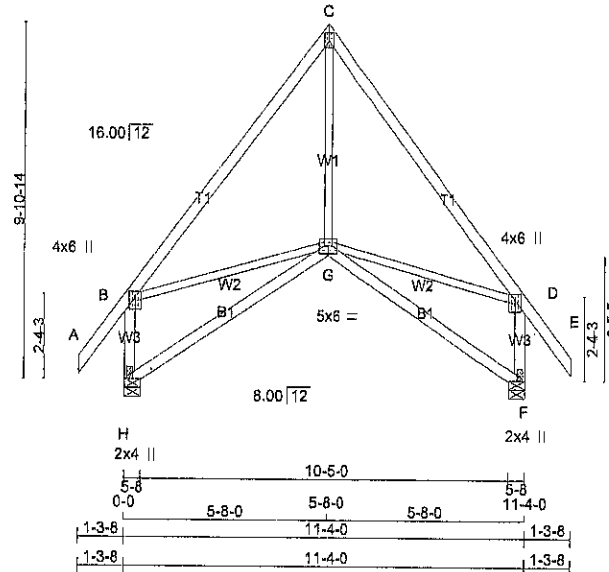


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253635	H11	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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3x5 II
Scale: 3/16"=1"



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW+p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	598	0	598	0	5-8	1-8
F	598	0	598	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX / MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
H	419	297 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0
F	419	297 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO	CS1 (LC)	LENGTH	FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G-C	0 / 224 0.05 (4)	
B-C	-390 / 0	-70.5	-70.5	0.30 (1)	6.25	B-G	0 / 242 0.05 (1)	
C-D	-390 / 0	-70.5	-70.5	0.30 (1)	6.25	G-D	0 / 242 0.05 (1)	
D-E	0 / 40	-70.5	-70.5	0.10 (1)	10.00			
H-B	-548 / 0	0.0	0.0	0.08 (1)	7.81			
F-D	-548 / 0	0.0	0.0	0.08 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.16 (4)	10.00			
G-F	0 / 0	-17.5	-17.5	0.16 (4)	10.00			

TOTAL WEIGHT = 82 lb
(M)(F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/998 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/998 (0.06")

CSI: TC=0.30 (C-D:1), BC=0.16 (F-G:4),
WB=0.05 (D-G:1), SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2264 1656

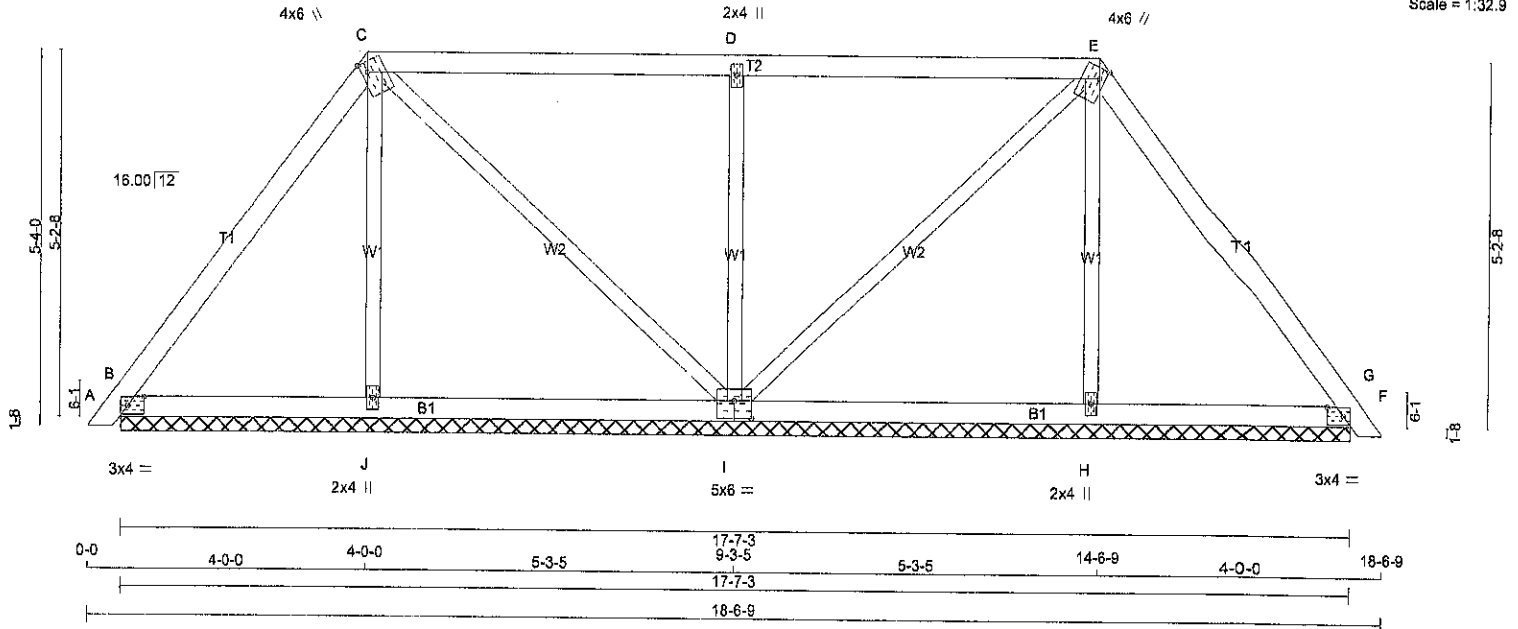
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.13 (H) (INPUT = 1.00)



A-1073853



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TMWW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	4.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BSWWW1-I	MT20	5.0	6.0	3.00	3.00
J	BMW1+w	MT20	2.0	4.0		

NOTES - (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	245	0	245	0	17-7-3 (8-9-10) 3/8	
J	231	0	231	0	17-7-3 (8-9-10) 3/8	
I	646	0	646	0	17-7-3 (8-9-10) 3/8	
H	231	0	231	0	17-7-3 (8-9-10) 3/8	
F	245	0	245	0	17-7-3 (8-9-10) 3/8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
B	169	132 / 0	0 / 0	0 / 0	0 / 0	37 / 0
J	167	87 / 0	0 / 0	0 / 0	0 / 0	80 / 0
I	451	329 / 0	0 / 0	0 / 0	0 / 0	122 / 0
H	167	87 / 0	0 / 0	0 / 0	0 / 0	80 / 0
F	169	132 / 0	0 / 0	0 / 0	0 / 0	37 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, I, H, F

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	J-C	-146 / 0		0.06 (1)
B-C	-81 / 0	-70.5	-70.5	0.15 (1)	6.25	C-I	-86 / 0		0.06 (1)
C-D	-1 / 0	-70.5	-70.5	0.33 (1)	10.00	I-D	-464 / 0		0.19 (1)
D-E	-1 / 0	-70.5	-70.5	0.33 (1)	10.00	I-E	-66 / 0		0.06 (1)
E-F	-91 / 0	-70.5	-70.5	0.15 (1)	6.25	H-E	-146 / 0		0.06 (1)
F-G	0 / 9	-70.5	-70.5	0.01 (1)	10.00				
B-J	0 / 53	-17.5	-17.5	0.08 (4)	10.00				
J-I	0 / 50	-17.5	-17.5	0.10 (4)	10.00				
I-H	0 / 50	-17.5	-17.5	0.10 (4)	10.00				
H-F	0 / 53	-17.5	-17.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(65% OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33 (C-D:1), BC=0.10 (H-I:4), WB=0.19 (D-I:1), SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

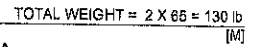
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
 JSI METAL= 0.21 (I) (INPUT = 1.00)



A-15073852



NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

B-J	0/51	-17.5	-17.5	0.10 (4)	10.00
J-I	0/46	-17.5	-17.5	0.15 (4)	10.00
I-H	0/46	-17.5	-17.5	0.15 (4)	10.00
H-F	0/51	-17.5	-17.5	0.10 (4)	10.00

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.23 (I) (INPUT = 1.00)

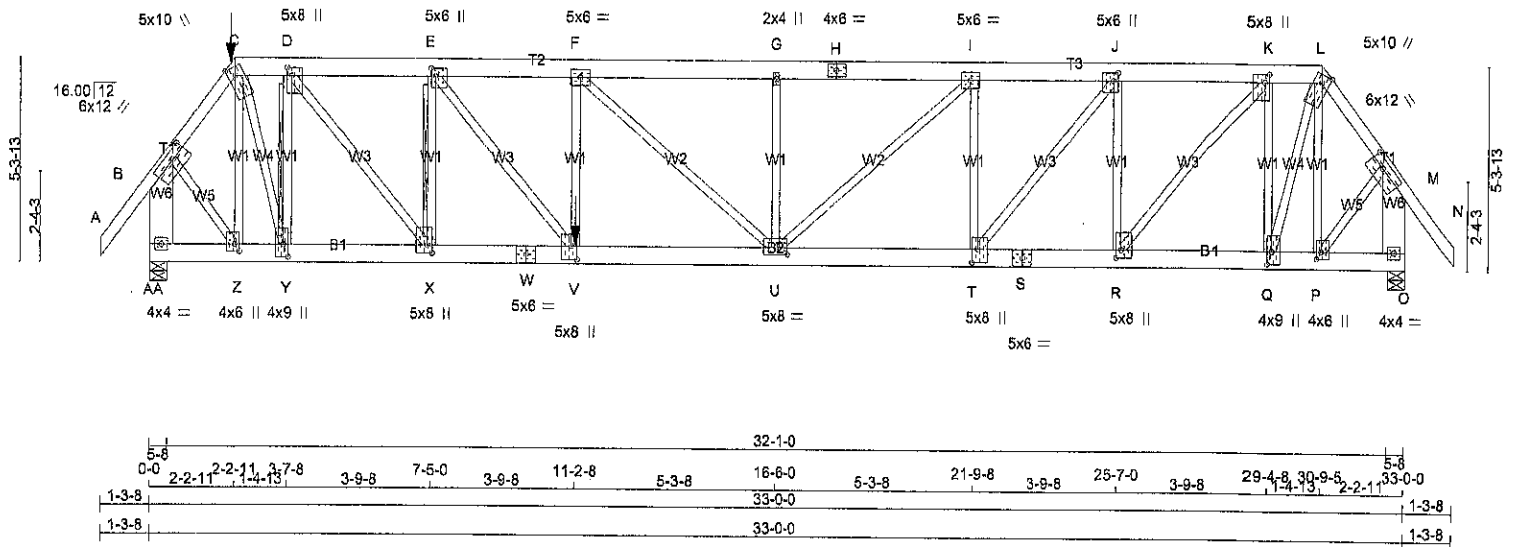


A-15023851

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253635	H01-Cond3	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:60.4



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - H	2x6	DRY	No.2
H - L	2x6	DRY	No.2
L - N	2x4	DRY	No.2
AA - B	2x8	DRY	No.2
O - M	2x8	DRY	No.2
AA - W	2x8	DRY	No.2
W - S	2x6	DRY	No.2
S - O	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMVW-I	MT20	6.0	12.0	2.25 4.75
C	TTWW+m	MT20	5.0	10.0	2.50 2.25
D	TMVW-I	MT20	5.0	8.0	2.50 1.50
E	TMVW-I	MT20	5.0	6.0	2.50 1.50
F	TMVW-I	MT20	5.0	6.0	
G	TMVW-I	MT20	2.0	4.0	
H	TS-I	MT20	4.0	6.0	
I	TMVW-I	MT20	5.0	6.0	
J	TMVW-I	MT20	5.0	6.0	2.50 1.50
K	TMVW-I	MT20	5.0	8.0	2.50 1.50
L	TTWW+m	MT20	5.0	10.0	2.50 2.25
M	TMVW-I	MT20	6.0	12.0	2.25 4.75
O	BMVW-I	MT20	4.0	4.0	
P	BMVW-I	MT20	4.0	6.0	2.25 1.50
Q	BMVW-I	MT20	4.0	9.0	4.00 1.60
R	BMVW-I	MT20	5.0	8.0	2.50 1.50
S	BS-I	MT20	5.0	6.0	
T	BMVW-I	MT20	5.0	8.0	4.25 1.75
U	BMVW-I	MT20	5.0	8.0	2.25 4.00
V	BMVW-I	MT20	5.0	8.0	4.25 1.75
W	BS-I	MT20	5.0	6.0	
X	BMVW-I	MT20	5.0	8.0	2.50 1.50
Y	BMVW-I	MT20	4.0	9.0	4.00 1.50
Z	BMVW-I	MT20	4.0	6.0	2.25 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
AA	3864	0	3864	0	0	5-8	5-5	5-5	5-5
O	2518	0	2518	0	0	5-8	2-12	2-12	2-12

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	DEAD	SOIL
AA	2721	1861 / 0	0 / 0	0 / 0	0 / 0	870 / 0	0 / 0	870 / 0	0 / 0
O	1772	1214 / 0	0 / 0	0 / 0	0 / 0	558 / 0	0 / 0	558 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-Y, E-X
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO
FR-TO			
A-B	0 / 40	-70.5 -70.5 0.11 (1)	10.00
B-C	-2966 / 0	-70.5 -70.5 0.19 (1)	3.88
C-D	-2572 / 0	-138.7 -138.7 0.13 (1)	5.04
D-E	-4876 / 0	-138.7 -138.7 0.36 (1)	3.65
E-F	-6522 / 0	-138.7 -138.7 0.43 (1)	3.14
F-G	-5781 / 0	-70.5 -70.5 0.38 (1)	3.39
G-H	-5781 / 0	-70.5 -70.5 0.36 (1)	3.41
H-I	-5781 / 0	-70.5 -70.5 0.36 (1)	3.41
I-J	-4451 / 0	-70.5 -70.5 0.19 (1)	3.99
J-K	-3210 / 0	-70.5 -70.5 0.18 (1)	4.57
K-L	-1652 / 0	-70.5 -70.5 0.09 (1)	6.02
L-M	-1888 / 0	-70.5 -70.5 0.12 (1)	4.77
M-N	0 / 40	-70.5 -70.5 0.11 (1)	10.00
AA-B	-3983 / 0	0.0 0.0 0.29 (1)	6.01
O-M	-2601 / 0	0.0 0.0 0.19 (1)	7.14
FR-TO			
A-B	0 / 0	-34.5 -34.5 0.27 (1)	10.00
Z-Y	0 / 1759	-34.5 -34.5 0.50 (1)	10.00
Y-X	0 / 2572	-34.5 -34.5 0.43 (1)	10.00
X-W	0 / 4876	-34.5 -34.5 0.76 (1)	10.00
W-V	0 / 4876	-34.5 -34.5 0.76 (1)	10.00
V-U	0 / 6522	-17.5 -17.5 0.97 (1)	10.00
U-T	0 / 4451	-17.5 -17.5 0.83 (1)	10.00
T-S	0 / 3210	-17.5 -17.5 0.45 (1)	10.00
S-R	0 / 3210	-17.5 -17.5 0.45 (1)	10.00
R-Q	0 / 1652	-17.5 -17.5 0.27 (1)	10.00
Q-P	0 / 1120	-17.5 -17.5 0.32 (1)	10.00
P-O	0 / 0	-17.5 -17.5 0.18 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE	
JT	LOC.	LC1	MAX-	MAX+	FRONT	VERT	TOTAL
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
V	11-2-8	-2365	-2365	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 2 X 209 = 417 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	
TOP CH.	LL = 21.0 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPprimeHip
SIDE SETBACK = 2-2-11
END SETBACK = 6-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 11-2-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-08
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/638 (0.47")

CSI: TC=0.43 (E-F:1), BC=0.97 (U-V:1), WB=0.96 (F-U:1), SS=0.26 (Y-Z:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

A-15073857



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253635	H01-Cond3	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-E02?cJNMMkgoKICRfpOx00NXo87IXfkH9i6U9oyx9Z

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
AA	BMV1-H	MT20	4.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD, AND 2364.6 lbs FACTORED DOWN AT 11-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- Lateral brace(s) shown shall be 1x4 for Part 8 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.85 (W) (INPUT = 1.00)

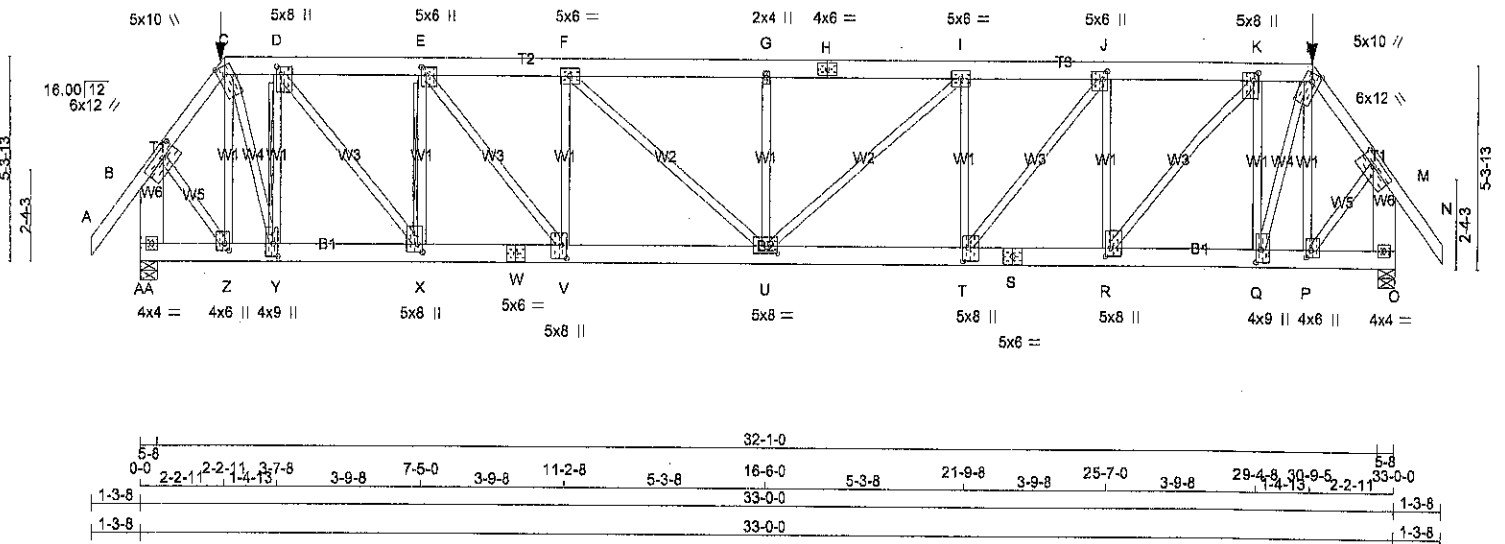


A-N073850(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253635	H01-Cond1	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.820 S Apr 15 2015 MITek Industries, Inc. Fri Jul 17 13:36:42 2015 Page 1
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Scale = 1:60.4



TOTAL WEIGHT = 2 X 209 = 417 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - N	2x4	DRY	No.2	SPF
AA - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
AA - W	2x6	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	12.0	2.25	4.75
C	TTWW+m	MT20	5.0	10.0	2.50	2.25
D	TMVW-t	MT20	5.0	8.0	2.50	1.50
E	TMVW-t	MT20	5.0	6.0	2.50	1.50
F	TMVW-t	MT20	5.0	6.0		
G	TMVW-t	MT20	2.0	4.0		
H	TS-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.50	1.50
K	TMVW-t	MT20	5.0	8.0	2.50	1.50
L	TTWW+m	MT20	5.0	10.0	2.50	2.25
M	TMVW-t	MT20	6.0	12.0	2.25	4.75
O	BMV-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	6.0	2.25	1.50
Q	BMVW-t	MT20	4.0	9.0	4.00	1.50
R	BMVW-t	MT20	5.0	8.0	2.50	1.50
S	BS-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	8.0	4.25	1.75
U	BMVW-t	MT20	5.0	8.0	2.25	4.00
V	BMVW-t	MT20	5.0	8.0	4.25	1.75
W	BS-t	MT20	5.0	6.0		
X	BMVW-t	MT20	5.0	8.0	2.50	1.50
Y	BMVW-t	MT20	4.0	9.0	4.00	1.50
Z	BMVW-t	MT20	4.0	6.0	2.25	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
AA	2919	0	2919	0	5-8
O	2919	0	2919	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	2055	1400	0	0	0	0	655	0
O	2055	1400	0	0	0	0	655	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-Y, E-X
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	UNBRAC	FR-TO	
A-B	0/40	-70.5 -70.5 0.11 (1)	10.00	Z-C	-617/0
B-C	-2195/0	-70.5 -70.5 0.14 (1)	4.47	C-Y	0/2114
C-D	-1887/0	-138.7 -138.7 0.08 (1)	5.75	Y-D	-2229/0
D-E	-3427/0	-138.7 -138.7 0.25 (1)	4.38	D-X	0/2504
E-F	-4410/0	-138.7 -138.7 0.28 (1)	3.91	X-E	-1822/0
F-G	-4930/0	-138.7 -138.7 0.40 (1)	3.60	E-V	0/1597
G-H	-4930/0	-138.7 -138.7 0.40 (1)	3.60	V-F	-1105/0
H-I	-4930/0	-138.7 -138.7 0.40 (1)	3.60	F-U	0/707
I-J	-4410/0	-138.7 -138.7 0.28 (1)	3.91	U-G	-770/0
J-K	-3427/0	-138.7 -138.7 0.25 (1)	4.38	U-I	0/707
K-L	-1887/0	-138.7 -138.7 0.08 (1)	5.75	T-I	-1105/0
L-M	-2195/0	-70.5 -70.5 0.14 (1)	4.47	T-J	0/1597
M-N	0/40	-70.5 -70.5 0.11 (1)	10.00	R-J	-1822/0
AA-B	-2993/0	0.0 0.0 0.22 (1)	6.76	R-K	0/2504
O-M	-2993/0	0.0 0.0 0.22 (1)	6.76	Q-K	-2229/0
AA-Z	0/0	-34.5 -34.5 0.20 (1)	10.00	Q-L	0/2114
Z-Y	0/1302	-34.5 -34.5 0.37 (1)	10.00	P-L	-617/0
Y-X	0/1887	-34.5 -34.5 0.32 (1)	10.00	B-Z	0/1688
X-W	0/3427	-34.5 -34.5 0.49 (1)	10.00	P-M	0/1688
W-V	0/3427	-34.5 -34.5 0.49 (1)	10.00		
V-U	0/4410	-34.5 -34.5 0.66 (1)	10.00		
U-T	0/4410	-34.5 -34.5 0.66 (1)	10.00		
T-S	0/3427	-34.5 -34.5 0.49 (1)	10.00		
S-R	0/3427	-34.5 -34.5 0.49 (1)	10.00		
R-Q	0/1887	-34.5 -34.5 0.32 (1)	10.00		
Q-P	0/1302	-34.5 -34.5 0.37 (1)	10.00		
P-O	0/0	-34.5 -34.5 0.20 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
L	30-9-5	-114	-114	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 65 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(66 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")

CSI: TC=0.40 (G-I:1), BC=0.66 (T-U:1), WB=0.85 (K-Q:1), SSI=0.26 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.70 (H) (INPUT=0.90)
JSI METAL=0.55 (W) (INPUT=1.00)

A-N073849
CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253635	H01-Cond1	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
AA	BMV1-4	MT20	4.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 30-9-5, AND 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

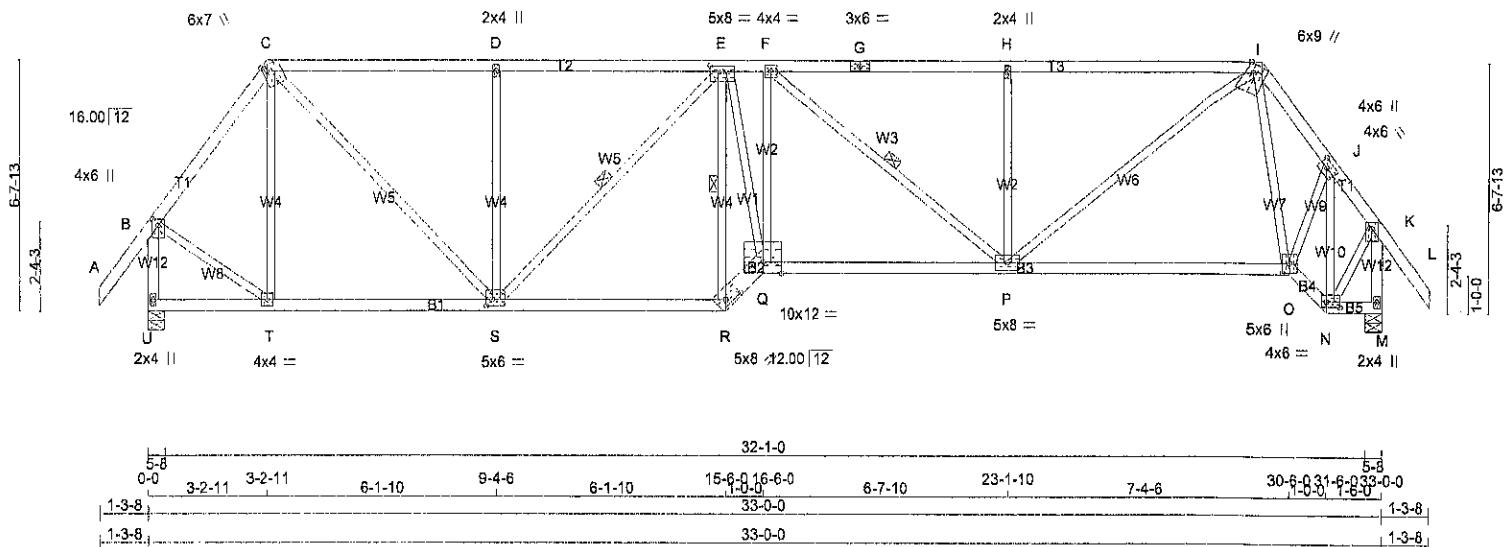


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DRY: SEASONED LUMBER.

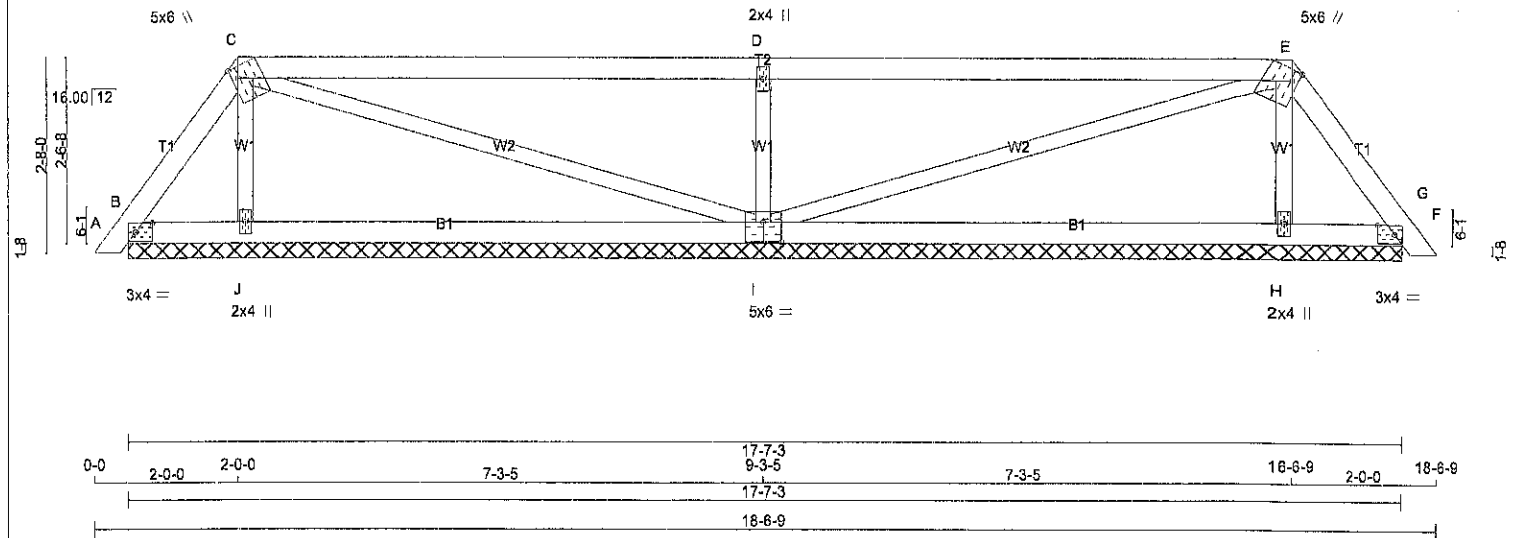
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253622	H07	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 MiTek Industries, Inc. Thu Jul 16 14:32:01 2015 Page 1
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Scale: 3/8"=1'



TOTAL WEIGHT = 2 X 61 = 122 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - I	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TTWW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BSWWW1-I	MT20	5.0	6.0	3.25	3.00
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	119	0	119	0	-8	17-7-3 (8-9-10) 8
J	287	0	287	0	0	17-7-3 (8-9-10) 8
I	785	0	785	0	0	17-7-3 (8-9-10) 8
H	287	0	287	0	0	17-7-3 (8-9-10) 8
F	119	0	119	0	-8	17-7-3 (8-9-10) 8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	79	84 / 0	0 / 0	0 / 0	0 / 0	0 / -5	0 / 0
J	208	105 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
I	550	389 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
H	208	105 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
F	79	84 / 0	0 / 0	0 / 0	0 / 0	0 / -5	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, I, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MAX. FACTORED		FACTORED			MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 8	-70.5	-70.5	0.01 (1)	10.00	J-C	-177 / 0	0.03 (1)
B-C	-78 / 0	-70.5	-70.5	0.03 (1)	6.25	C-I	-22 / 0	0.03 (1)
C-D	-13 / 0	-70.5	-70.5	0.03 (1)	6.25	I-D	-640 / 0	0.10 (1)
D-E	-13 / 0	-70.5	-70.5	0.03 (1)	6.25	I-E	-22 / 0	0.03 (1)
E-F	-78 / 0	-70.5	-70.5	0.03 (1)	6.25	H-E	-177 / 0	0.03 (1)
F-G	0 / 8	-70.5	-70.5	0.01 (1)	10.00			
B-J	0 / 43	-17.5	-17.5	0.14 (4)	10.00			
J-I	0 / 35	-17.5	-17.5	0.20 (4)	10.00			
I-H	0 / 35	-17.5	-17.5	0.20 (4)	10.00			
H-F	0 / 43	-17.5	-17.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63 (D-E:1), BC=0.20 (I-J:4), WB=0.10 (D-I:1), SS=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX MIN
MT20	618	354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.24 (I) (INPUT = 1.00)



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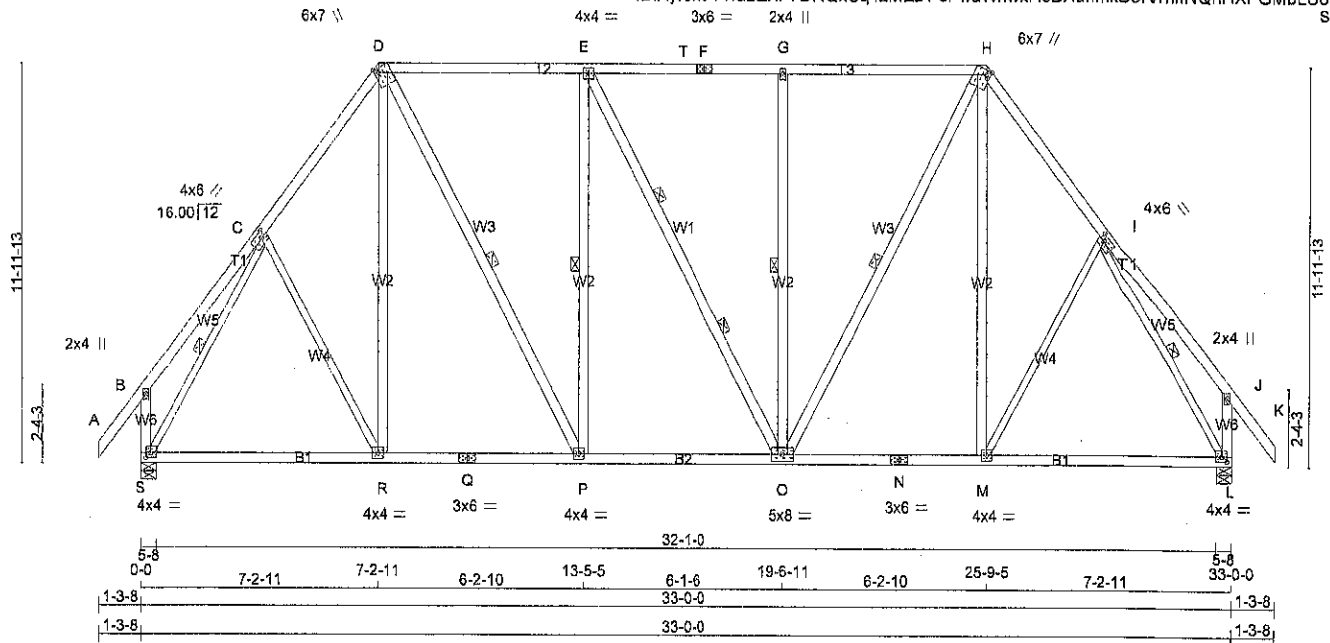
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253622	H06	9	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:69.5



TOTAL WEIGHT = 9 X 216 = 1946 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
S - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
C - R	2x3	DRY	No.2	SPF	
M - I	2x3	DRY	No.2	SPF	
S - C	2x3	DRY	No.2	SPF	
I - L	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW+H	MT20	4.0	6.0	2.00	2.00
D	TMVW+m	MT20	6.0	7.0	Edge	1.50
E	TMVW+H	MT20	4.0	4.0		
F	TS-4	MT20	3.0	6.0		
G	TMVW+w	MT20	2.0	4.0		
H	TMVW+m	MT20	6.0	7.0	Edge	1.50
I	TMVW+H	MT20	4.0	6.0	2.00	2.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW+H	MT20	4.0	4.0	1.75	1.75
M, P, R						
M	BMVW+H	MT20	4.0	4.0		
N	BS-4	MT20	3.0	6.0		
O	BMVW+H	MT20	5.0	8.0		
Q	BS-4	MT20	3.0	6.0		
S	BMVW+H	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
S	1620 0	1620 0	5-8	1-12
L	1620 0	1620 0	5-8	1-12

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
S	COMBINED	SNOW LIVE PERM.LIVE WIND	394 / 0	0 / 0
L	COMBINED	SNOW LIVE PERM.LIVE WIND	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-P, E-P, G-O, H-O, C-S, I-L.
2 LATERAL BRACE(S) REQUIRED AT 1/3 LENGTH OF E-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (UNBRAC)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO			FROM	TO	LENGTH	FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C-R	0 / 64	0.02 (4)	
B-C	0 / 23	-70.5	-70.5	0.14 (1)	10.00	R-D	0 / 97	0.02 (4)	
C-D	-1286 / 0	-70.5	-70.5	0.14 (1)	5.67	D-P	0 / 702	0.11 (1)	
D-E	-1085 / 0	-78.0	-78.0	0.36 (1)	5.62	P-E	-519 / 0	0.41 (1)	
E-T	-1085 / 0	-78.0	-78.0	0.33 (1)	5.66	E-O	-2 / 0	0.00 (1)	
T-F	-1085 / 0	-78.0	-78.0	0.33 (1)	5.66	O-G	-518 / 0	0.41 (1)	
F-G	-1085 / 0	-78.0	-78.0	0.33 (1)	5.66	G-H	0 / 700	0.11 (1)	
G-H	-1064 / 0	-78.0	-78.0	0.36 (1)	5.62	H-I	0 / 96	0.02 (4)	
H-I	-1286 / 0	-70.5	-70.5	0.14 (1)	5.57	I-L	0 / 64	0.02 (4)	
I-J	0 / 23	-70.5	-70.5	0.14 (1)	10.00	S-C	-1552 / 0	0.60 (1)	
J-K	0 / 40	-70.5	-70.5	0.10 (1)	10.00	L-L	-1553 / 0	0.60 (1)	
S-B	-196 / 0	0.0	0.0	0.02 (1)	7.81				
L-J	-196 / 0	0.0	0.0	0.02 (1)	7.81				
S-R	0 / 727	-17.5	-17.5	0.27 (4)	10.00				
R-Q	0 / 755	-17.5	-17.5	0.28 (4)	10.00				
Q-P	0 / 755	-17.5	-17.5	0.28 (4)	10.00				
P-O	0 / 1085	-17.5	-17.5	0.24 (1)	10.00				
O-N	0 / 755	-17.5	-17.5	0.28 (4)	10.00				
N-M	0 / 755	-17.5	-17.5	0.28 (4)	10.00				
M-L	0 / 727	-17.5	-17.5	0.27 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/989 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/989 (0.10")

CSI: TC=0.36 (D-E:1), BC=0.28 (M-O:4), WB=0.60 (I-L:1), SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

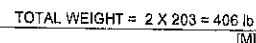
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.80)
JSI METAL= 0.38 (I) (INPUT = 1.00)



A-15073780



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S. L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT, OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-P, E-O, G-O, C-S, H-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

LOADING
TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS:			
TOP CH.	LL	=	21.0 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.52 (D-E:1), BC=0.27 (O-P:1)
WB=0.46 (I-L:1), SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

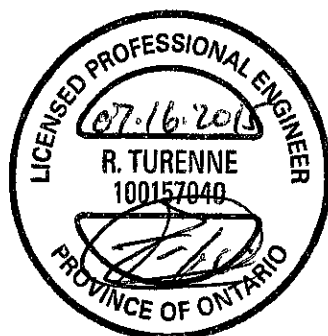
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	822	2284	1656

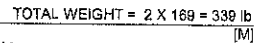
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

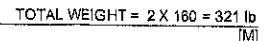
JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)



A-15073779

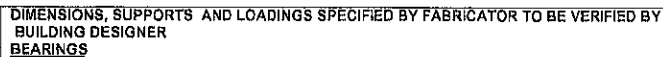


A-15073778



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MAX. FACTORED		FACTORED			MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	P-C	-198 / 32	0.24 (1)
B-C	-1195 / 0	-70.5	-70.5	0.27 (1)	5.55	C-N	0 / 1125	0.18 (1)
C-D	-1534 / 0	-70.5	-70.5	0.86 (1)	4.19	N-D	-622 / 0	0.75 (1)
D-E	-1534 / 0	-70.5	-70.5	0.86 (1)	4.17	N-F	-1 / 0	0.00 (1)
E-F	-1634 / 0	-70.5	-70.5	0.86 (1)	4.17	M-G	-622 / 0	0.76 (1)
F-G	-1535 / 0	-70.5	-70.5	0.87 (1)	4.17	M-G	0 / 1126	0.18 (1)
G-H	-1195 / 0	-70.5	-70.5	0.27 (1)	5.55	K-G	-199 / 32	0.24 (1)
H-I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	B-P	0 / 783	0.18 (1)
Q-B	-1530 / 0	0.0	0.0	0.20 (1)	6.65	K-H	0 / 783	0.18 (1)
J-H	-1530 / 0	0.0	0.0	0.20 (1)	6.65			
Q-P	0 / 0	-17.5	-17.5	0.18 (4)	10.00			
P-O	0 / 711	-17.5	-17.5	0.29 (4)	10.00			
O-N	0 / 711	-17.5	-17.5	0.29 (4)	10.00			
N-M	0 / 1535	-17.5	-17.5	0.38 (1)	10.00			
M-L	0 / 710	-17.5	-17.5	0.29 (4)	10.00			
L-K	0 / 710	-17.5	-17.5	0.29 (4)	10.00			
K-J	0 / 0	-17.5	-17.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.87 (F-G:1), BC=0.38 (M-N:1),
WB=0.76 (F-M:1), SSI=0.27 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1067	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

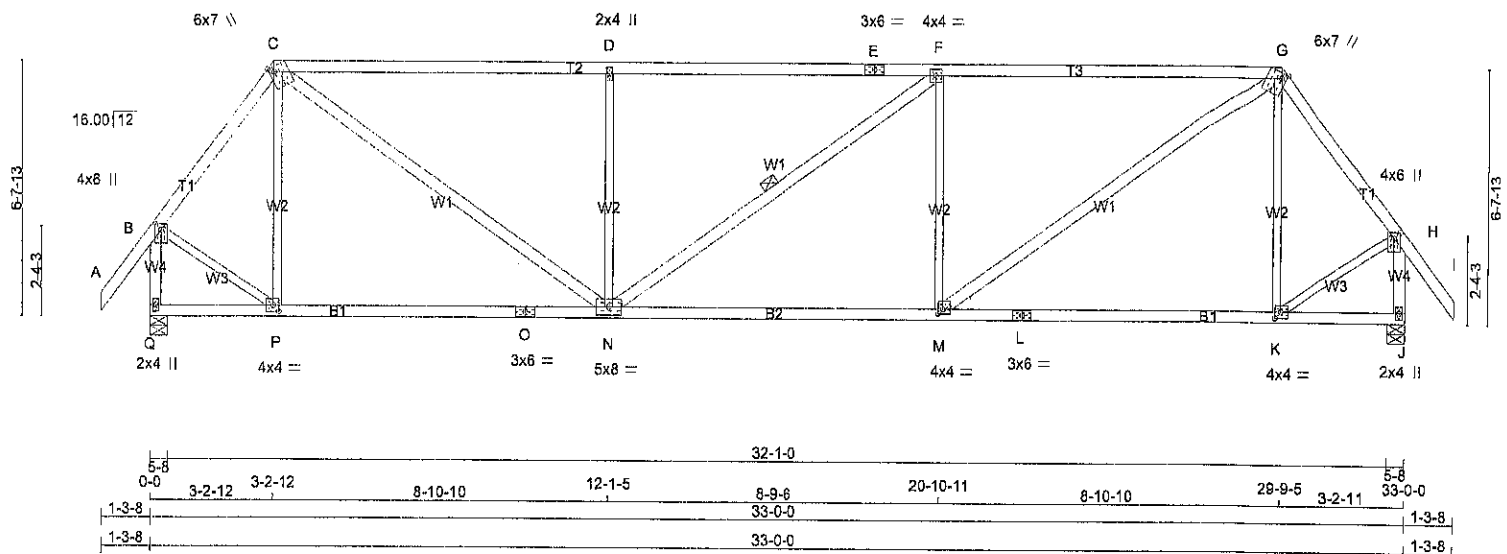
JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.31 (M) (INPUT = 1.00)

A-15073777

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253622	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 14:31:57 2015 Page 1
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-RqERugt2_trb1K29xzIC83lnh4Tw31YuoqHVW1yxTrM
Scale = 1:60.3



TOTAL WEIGHT = 2 X 152 = 304 lb
(M)

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
D - B	2x4	DRY	No.2	SPF
B - A	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	1.75	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	1.75	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BS-I	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0	1.50	1.50
N	BMWW-t	MT20	5.0	8.0		
O	BS-I	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
Q	1550	0	1550	0
J	1550	0	1550	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE	
Q	1090	752 / 0	0 / 0	0 / 0	338 / 0
J	1090	752 / 0	0 / 0	0 / 0	338 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MEMB.		MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	P-C	-275 / 9	0.20 (1)	
B-C	-1157 / 0	-70.5	-70.5	0.15 (1)	5.76	C-N	0 / 1431	0.23 (1)	
C-D	-1851 / 0	-70.5	-70.5	0.86 (1)	4.44	N-D	-673 / 0	0.49 (1)	
D-E	-1851 / 0	-70.5	-70.5	0.86 (1)	4.44	N-F	0 / 0	0.00 (1)	
E-F	-1851 / 0	-70.5	-70.5	0.86 (1)	4.44	M-F	-673 / 0	0.49 (1)	
F-G	-1852 / 0	-70.5	-70.5	0.86 (1)	4.43	M-G	0 / 1432	0.23 (1)	
G-H	-1157 / 0	-70.5	-70.5	0.15 (1)	5.76	K-G	-276 / 9	0.20 (1)	
H-I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	B-P	0 / 800	0.18 (1)	
Q-B	-1548 / 0	0.0	0.0	0.20 (1)	6.61	K-H	0 / 800	0.18 (1)	
J-H	-1548 / 0	0.0	0.0	0.20 (1)	6.62				
Q-P	0 / 0	-17.5	-17.5	0.20 (4)	10.00				
P-O	0 / 685	-17.5	-17.5	0.33 (4)	10.00				
O-N	0 / 685	-17.5	-17.5	0.33 (4)	10.00				
N-M	0 / 1862	-17.5	-17.5	0.45 (1)	10.00				
M-L	0 / 685	-17.5	-17.5	0.33 (4)	10.00				
L-K	0 / 686	-17.5	-17.5	0.33 (4)	10.00				
K-J	0 / 0	-17.5	-17.5	0.20 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
DL =	3.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.0	PSF	
TOTAL LOAD =	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.10")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (1.10")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.20")

CSI: TC=0.86 (F-G:1), BC=0.45 (M-N:1),
WB=0.49 (F-M:1), SI=0.28 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (Q) (INPUT = 0.90)
JSI METAL= 0.44 (M) (INPUT = 1.00)



A-15073776

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

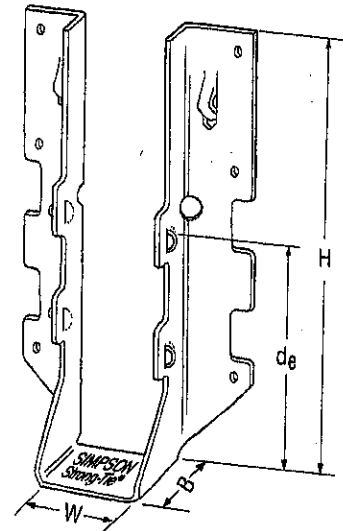
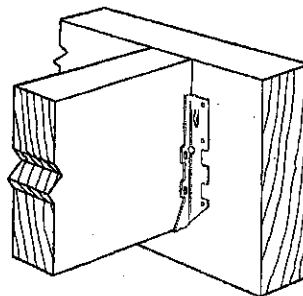
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.

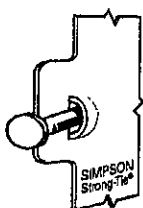
Typical LUS Installation



LUS28

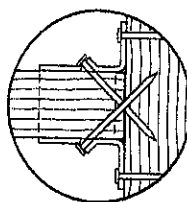
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/16	4 1/4	1 3/4	3 3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3 3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 1/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 15/16	1 3/4	3 3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

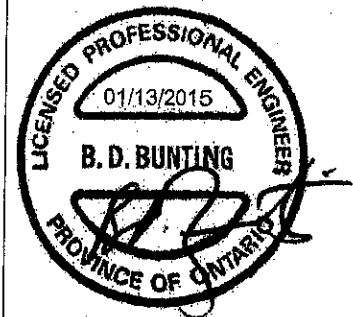


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

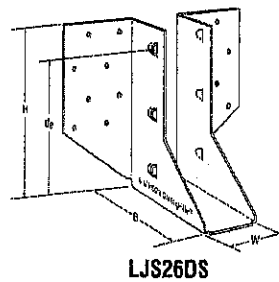
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%
No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

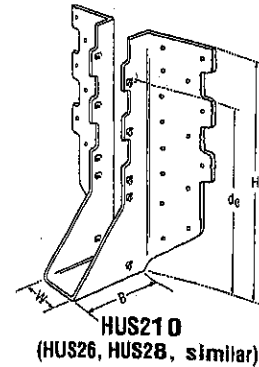
- Use all specified fasteners
- Nails: $16d = 0.162"$ dia. x $3\frac{1}{2}"$ long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

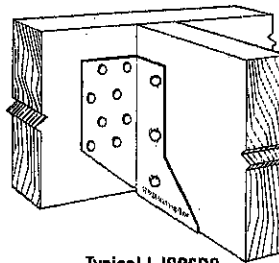
- See current catalogue for options



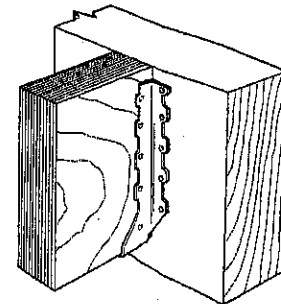
LJS26DS



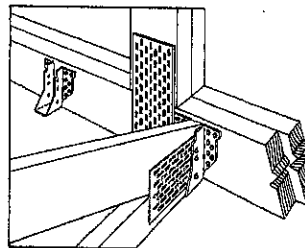
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



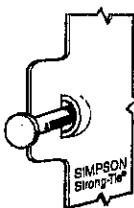
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

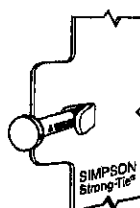
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d_g^1	Face	Joist	D.Fir-L		S-P-F	
								Uplift ($K_o=1.15$)	Normal ($K_o=1.00$)	Uplift ($K_o=1.15$)	Normal ($K_o=1.00$)
LJS26DS	18	$1\frac{9}{16}$	5	$3\frac{1}{2}$	$4\frac{9}{16}$	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	$1\frac{9}{16}$	$5\frac{3}{8}$	3	$3\frac{15}{16}$	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	$1\frac{9}{16}$	$7\frac{3}{32}$	3	$6\frac{3}{32}$	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	$1\frac{9}{16}$	$9\frac{3}{32}$	3	$7\frac{25}{32}$	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	18	$1\frac{13}{16}$	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

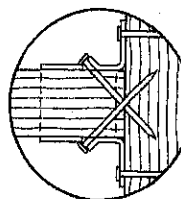


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

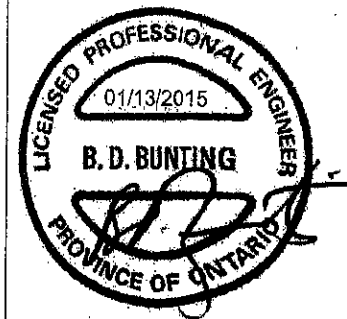
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

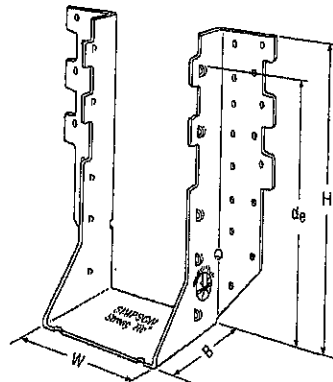
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

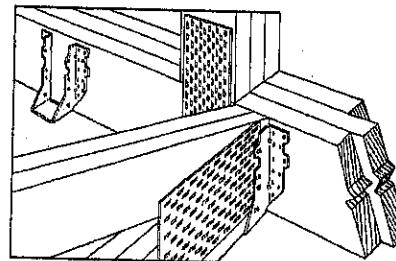
- Use all specified fasteners
- Nails: $16d = 0.162"$ dia. x $3\frac{1}{2}"$ long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



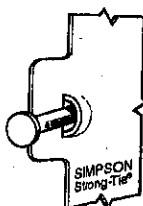
HHUS410



Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)

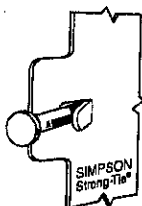
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	dg ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HHUS26-2	14	3 ¹ / ₁₆	5 ¹ / ₁₆	3	3 ¹ / ₁₆	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3 ³ / ₁₆	7 ¹ / ₃₂	3	6 ³ / ₃₂	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3 ³ / ₁₆	9 ³ / ₃₂	3	8	30-16d	10-16d	4745	9660	4310	7000
HHUS210-3	14	4 ¹ / ₁₆	9	3	7 ¹ / ₁₆	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6 ¹ / ₁₆	8 ²⁹ / ₃₂	3	7 ² / ₃₂	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3 ³ / ₁₆	5 ¹ / ₃₂	3	3 ¹ / ₁₆	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3 ³ / ₁₆	7 ¹ / ₁₆	3	6 ¹ / ₁₆	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3 ³ / ₁₆	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5 ¹ / ₂	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7 ¹ / ₄	9	3 ³ / ₁₆	7 ²⁹ / ₃₂	30-16d	10-16d	4745	10770	4310	7650

1. dg is the distance from the seat of the hanger to the highest joist nail.

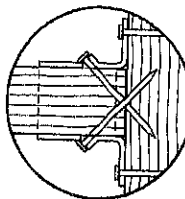


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

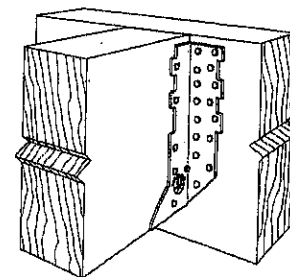
U.S. Patent
5,603,580



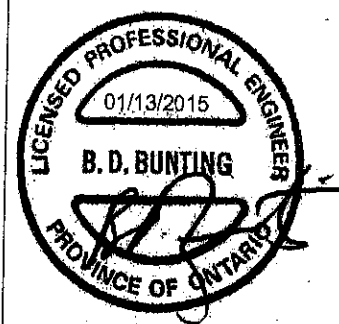
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

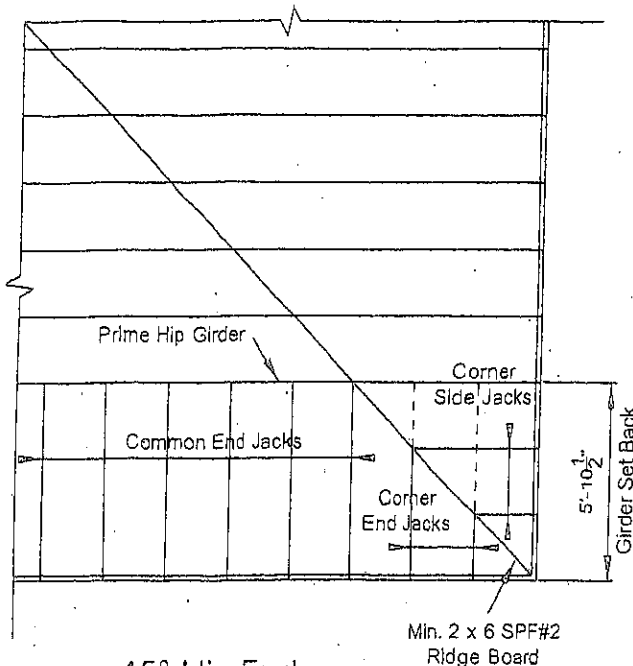


Typical HHUS Installation



800-999-5099
www.strongtie.com

STRACON ENGINEERING INC.



45° Hip End

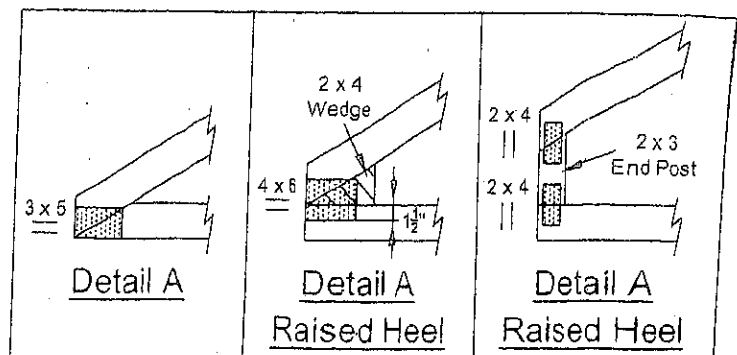
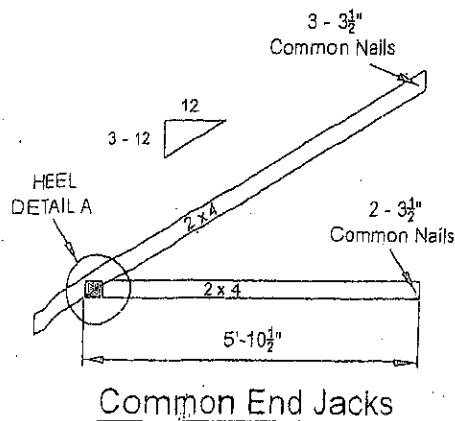
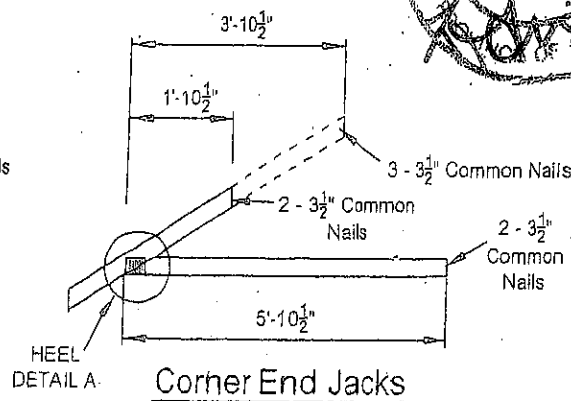
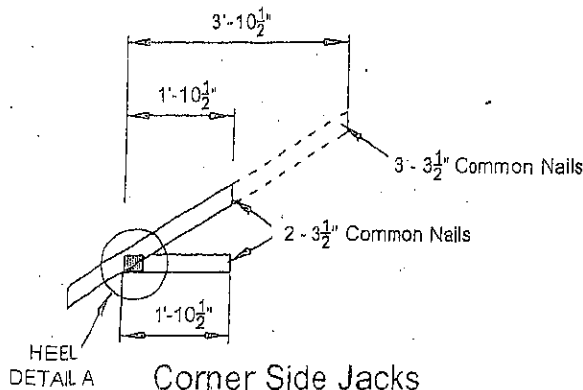
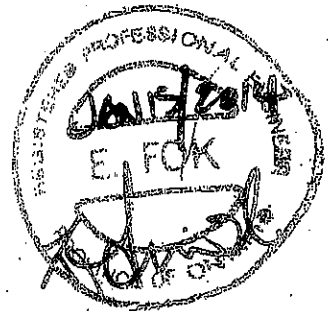
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

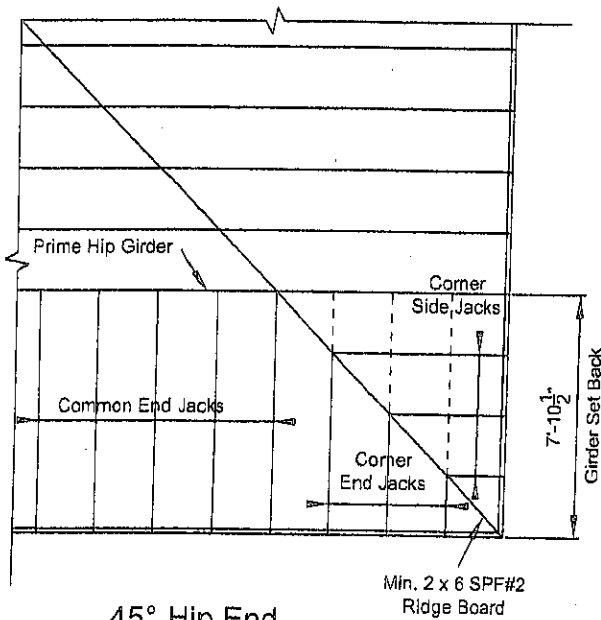
TOTAL LOAD : 50.5 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

STRACON ENGINEERING INC.



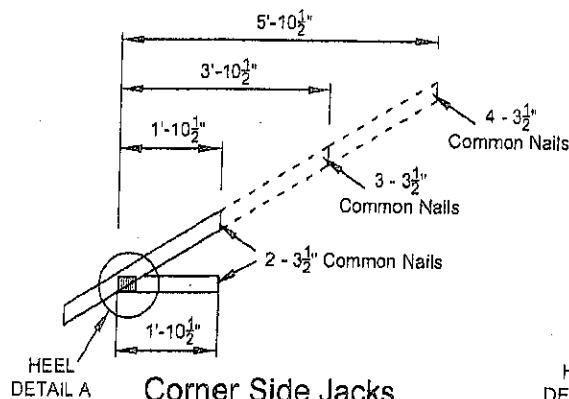
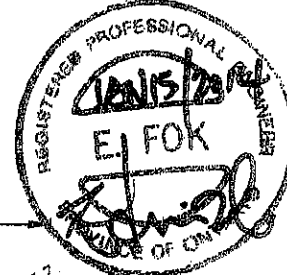
45° Hip End

LUMBER SPECIFICATION

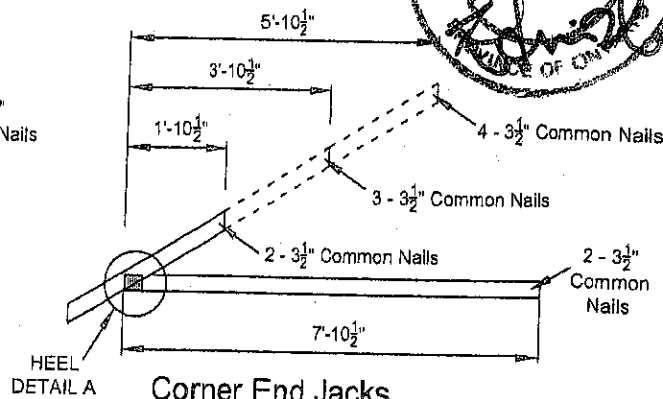
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

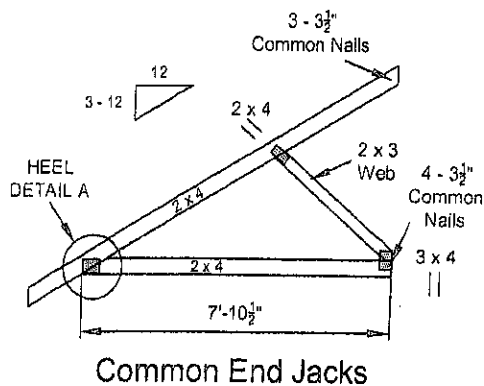
TOP CHORD SNOW LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 44.8 P.S.F.



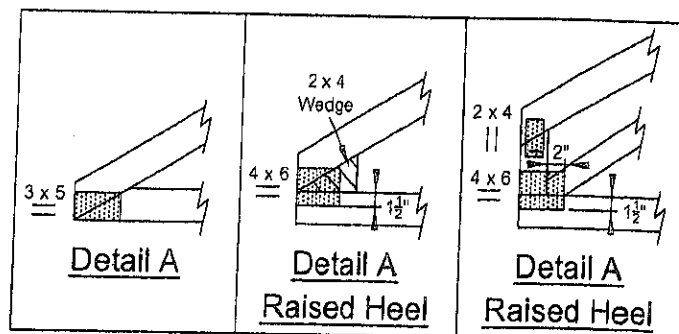
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-71008N