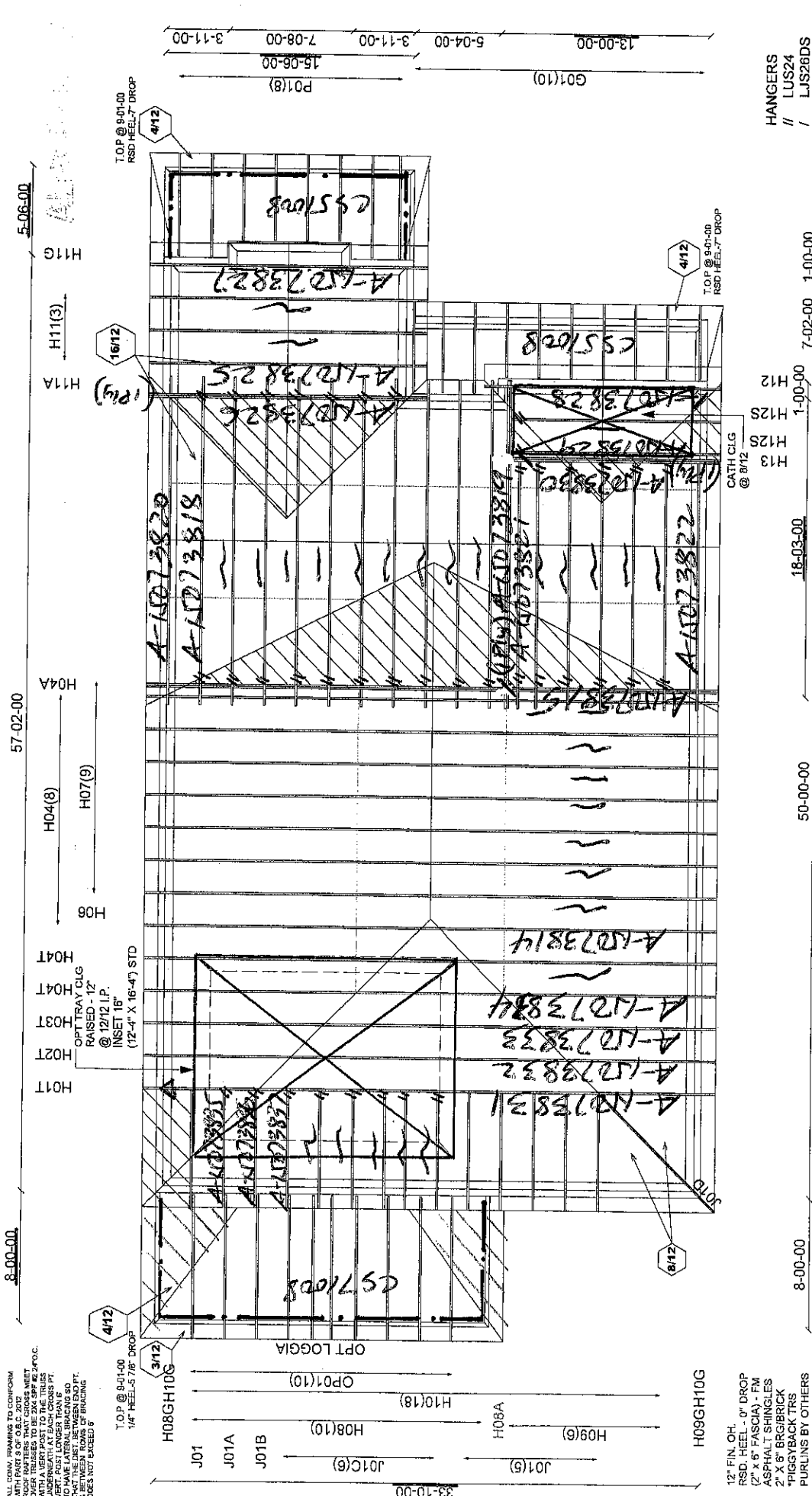


ALL CORN. FRAMING TO CONFORM WITH PART 9 OF S.E.C. 2012. ROOF PARTS THAT CROSS MEET WITH A VERT POST TO THE TRUSS JOINTS AT EACH CROSS PT. TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. DOES NOT EXCEED 8'.



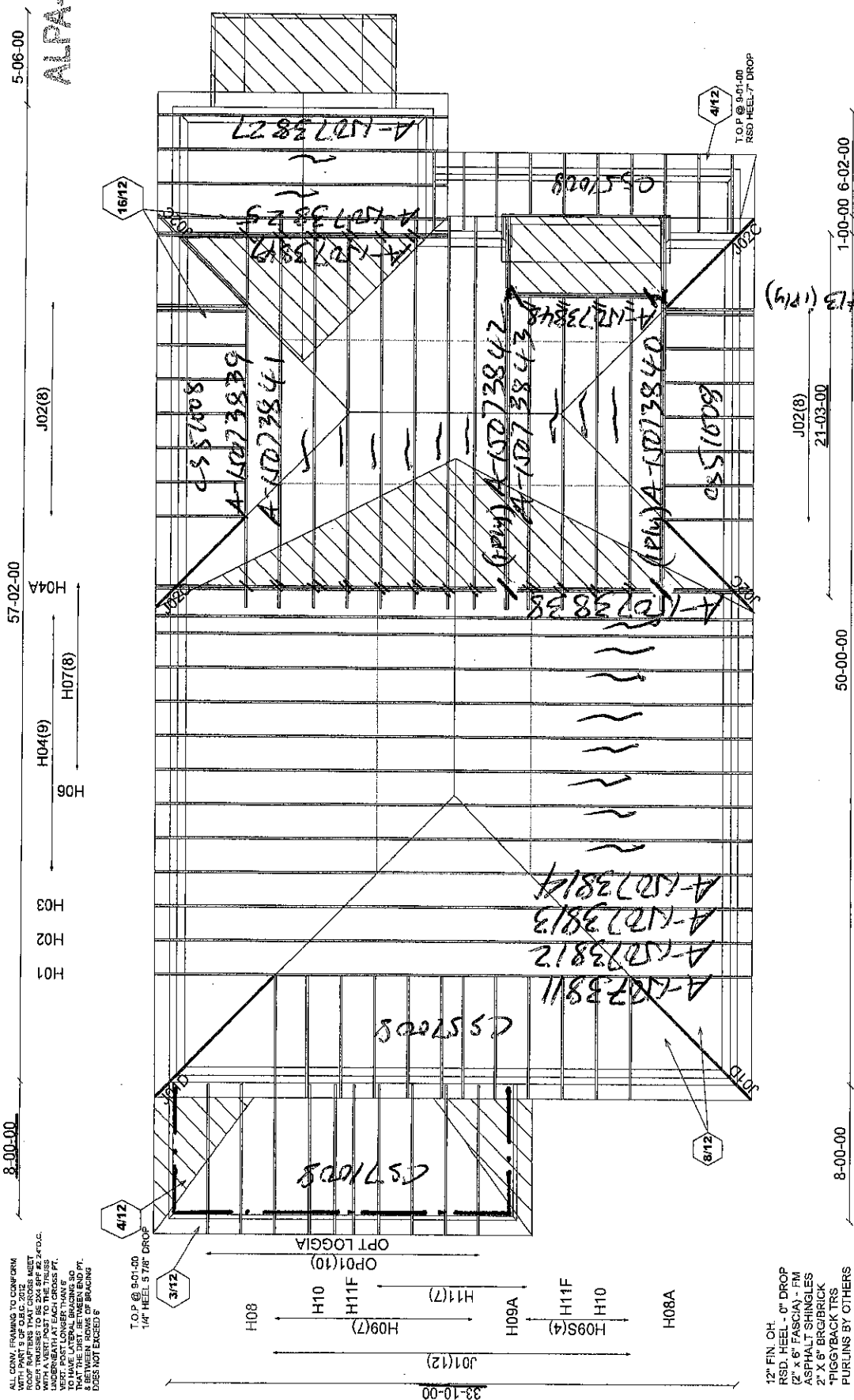
Piggyback
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 A-15073817
 A-15073823
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HANGERS
 // LUS24
 / LJS26DS
 A7C26

CONVENTIONAL FRAMING BY OTHERS
 Mitek Ver. 7.5.0

		Job Track: 40297		Builder / Location:		Model / Elevation:		Mitek Ver. 7.5.0	
Layout ID: 253630				GOLD PARK / KLEINBURG		42-3		/ A OPT TRAY	
Plan Log: 79711				Project: HUNTINGTON & NASHVILLE				THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
				Date: 4/30/2015		Designer: cheryle			

ALL ROOF FRAMING TO CONFORM WITH PART 5 OF D.C. 240.0. ROOF RAFTERS THAT CROSS MEET WITHIN THE TRUSSES SHALL BE WITH A VENT POST TO THE TRUSSES UNDERBATH AT EACH CROSS PT. TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. DOES NOT EXCEED 8'



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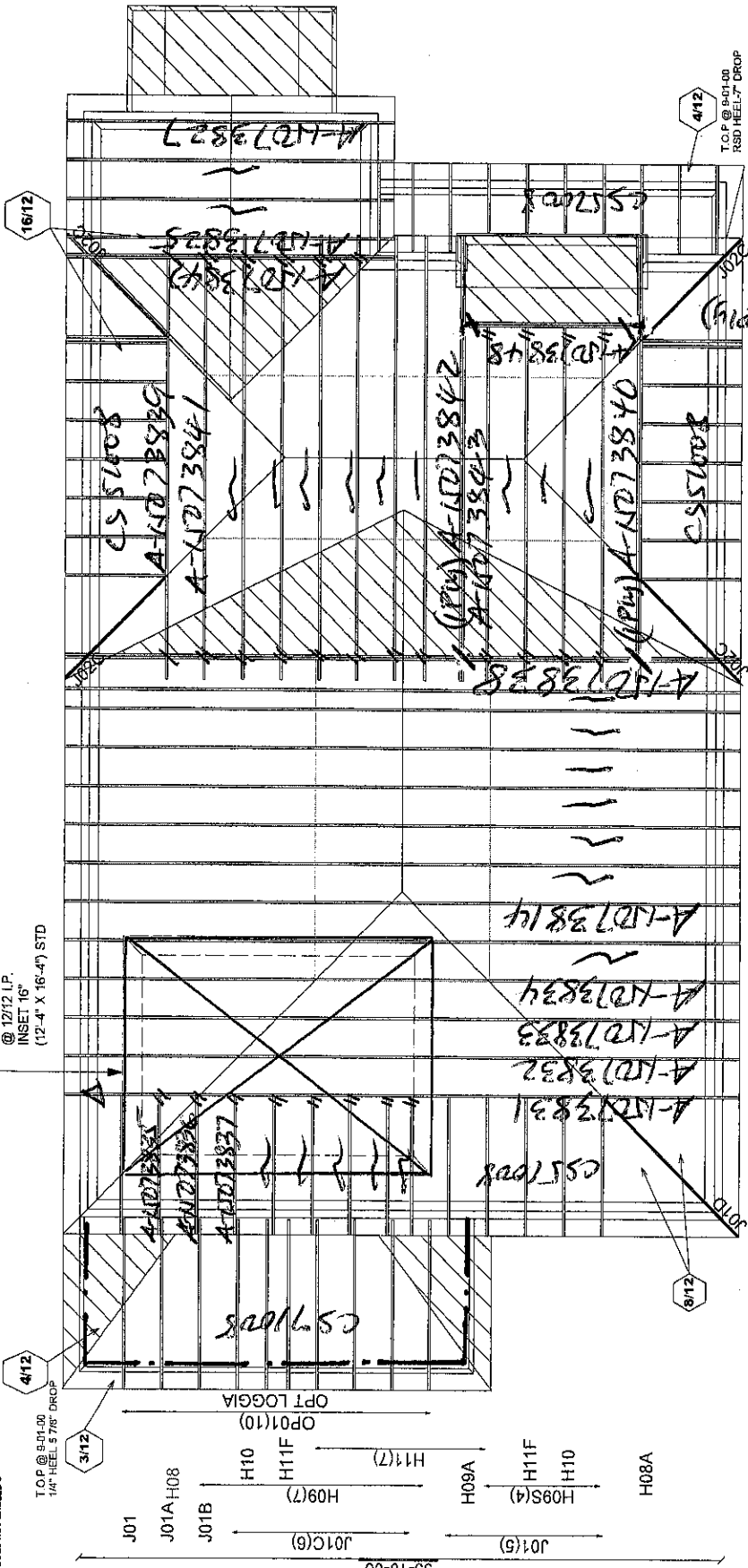
	Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 42-3 / B	STD OR OPT 2ND FLR.
	Layout ID: 253632	Project: HUNTINGTON & NASHVILLE		
	Plan Log: 79711	Date: 4/30/2015	Designer: chenyle	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

ALL CORN. FRAMING TO CONFORM WITH PART 3 OF O.C. 2007. ROOF TRUSSES TO BE ORDERED MEET WITH A VERT. POST TO THE TRUSSES UNDERNATH AT EACH CROSS PT. TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. DOES NOT EXCEED 6'

8-00-00 57-02-00 5-06-00
H01T H02T H03T H04T H06 H04(7) H07(8) H04A J02(8) H12A H12(3) H12G

ALPA-BARRE



12" FIN. OH.
RSD, HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGBACK TRS
PURLINS BY OTHERS

HANGERS
// LUS24
/ LJS26DS
A7C226

CONVENTIONAL
FRAMING BY
OTHERS

Mitek Ver. 7.5.0

Piggyback
(A-15073844
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A-15073847)

	Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Modal / Elevation: 42-3	/ B OPT TRAY
Layout ID: 253633	Project: HUNTINGTON & NASHVILLE	Project: HUNTINGTON & NASHVILLE	THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
Plan Log: 79711	Date: 4/30/2015	Designer: cheryle		

January 15, 2014

1. Trusses designed by Stracon Engineering Inc. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

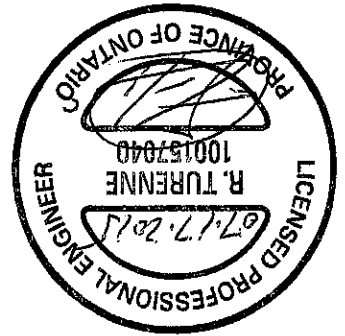
SPECIFICATIONS

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

RESPONSIBILITIES

Stracon Engineering Inc.
69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286





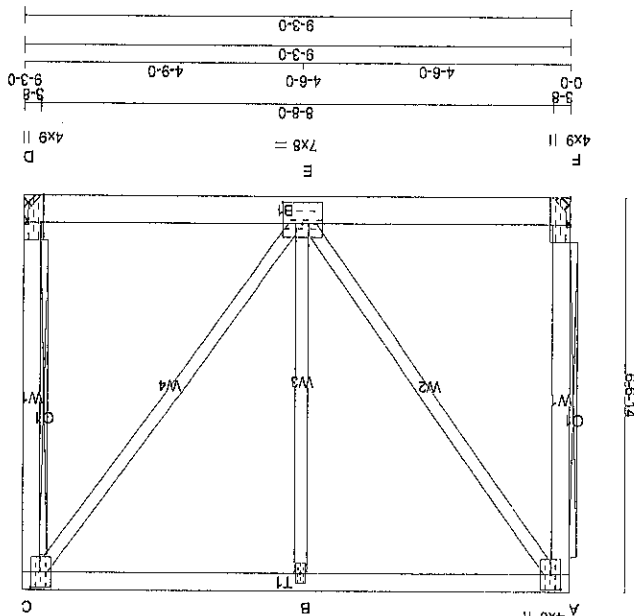
NOTES: (1)
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

TOUCHES EDGE OF CHORD.
Edge - INDICATES REFERENCE CORNER OF PLATE

PLATES (Table is in inches)	JT TYPE	W	LEN	Y	X
A TMW+P	MT20	4.0	6.0	3.00	1.75
B TMW+P	MT20	2.0	4.0		
C TMW+P	MT20	4.0	6.0	3.00	1.75
D BMV1+H	MT20	4.0	8.0	4.25	4.00
E BMW1+H	MT20	7.0	8.0		
F BMV1+H	MT20	4.0	9.0	5.50	

ALL WEBS	2x3	DRY	SEASONED LUMBER.
F - D	2x6	DRY	No.2
D - C	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
CHORDS	SIZE	LUMBER	No.2
N.L.G.A RULES			

BUILDING DESIGNER									
DIMENSIONS, SUPPORTS									
AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY									
BEARINGS									
FACTORED									
GROSS REACTION									
VERT. HORZ									
DOWN HORZ									
GROSS REACTION									
DOWN HORZ									
UP/LIFT									
N-SX									
BRG									
IN-SX									
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TOTAL WEIGHT = 57 lb

DESIGN CRITERIA	SPACING = 24.0 IN. C/S
LOADING IN PLAT SECTION BASED ON A	
SLOPE OF 2.00/12 MINIMUM	
GIRDER TYPE: C/Pinehip	
SIDE SETBACK = 0-0	
END SETBACK = 4-7-0	
CORNER FRAMING TYPE: CONVENTIONAL	
END JACK TYPE: CONVENTIONAL	
APPLIED TO FRONT SIDE	
-ADDTL LOADS BASED ON 55% OF GSL.	
GIRDER TYPE: C/Sidgider	
START SPAN CARRIED = 17-0-0	
END DISTANCE = 9-0-0	
END SPAN CARRIED = 17-0-0	
END WALL WIDTH = 0-0	
CORNER FRAMING TYPE: CONVENTIONAL	
END JACK TYPE: CONVENTIONAL	
APPLIED TO FRONT SIDE	
-ADDTL LOADS BASED ON 55% OF GSL.	

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010
THIS DESIGN COMPLIES WITH:
- CSA 086-08
- TFC 2011
- 21.0 P.S.F. RAIN LOAD EQUALS
- 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEF. (LL) = L/360 (0.31")
ALLOWABLE VERT. DEF. (LL) = L/999 (0.06")
ALLOWABLE DEF. (TL) = L/360 (0.31")
ALLOWABLE VERT. DEF. (TL) = L/999 (0.06")
CS1: TC=0.45 (B-C-1), BC=0.61 (D-E-1), WB=0.44 (B-E-1), SS=0.68 (D-E-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 0.60
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION (PLI)
MAX MIN MAX MIN
MT20 618 364 1667 822 2284 1656
A-11273848

[illegible]

DRY: SEASONED LUMBER.									
PLATES (table in inches)									
PLATES	W	LEN	Y	X	JT TYPE				
BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) L.H									
1	L	COMBINED SNOW	LIVE	PERMANENT WIND	0/0	0/0	0/0	0/0	0/0
2	H	2886	1832/0		0/0	0/0	0/0	0/0	0/0
3	L	3265	2226/0		0/0	0/0	0/0	0/0	0/0
4	DEAD	1040/0			0/0	0/0	0/0	0/0	0/0
5	SOIL								
6	BOI	CH.	DL	=	0.0	PSF			
7	TOTAL	LOAD	=	31.0	PSF				
SPACING = 24.0 IN./C									
GRIDER TYPE: CSDGrider									
START DISTANCE = 10'-7.4									

[illegible]

HANGERS NOTES		CHORDS		W E A S		THIS DESIGN COMPLIES WITH:	
1) SPECIAL HANGERS (S) OR CONNECTIONS (S)	MEMB. FORCE	FACTORED	FROM TO	MEMB. FORCE	MAX	CSA 086-09	-PART 8 OF CBC 2012, B30C 2012, ABC 2014
REQUIRED TO SUPPORT CONCENTRATED LOADS (S) 1419.3 lbs FACTORED DOWN AT 10'-7" ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION (S) IS DELEGATED TO THE BUILDING DESIGNER.	FR-TO	0.40	-70.5	FR-TO	0.88 (1)	-T10C 2011	
	(LBS)		-70.5	LENGTH	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		
			-70.5	MAX	0.3861		
			-70.5	CS (LC)	0.11 (1)		
			-70.5	UNBRAC	10.00		
			-70.5	MEMB. FORCE	0.3861		

									
L-K	0/0	-532.0	-532.0	0.60 (1)	10.00				
K-J	0/0	-532.0	-532.0	0.69 (1)	10.00				
J-I	0/0	-532.0	-532.0	0.66 (1)	10.00				
I-H	0/0	-61.5	-17.5	0.05 (1)	10.00				
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE		
1	10-7-4	-1419	-1419		FRONT	VERT.	TOTAL		
COMPANION LIVE LOAD FACTOR = 0.50									
DO1 LUMBER=1.00 NAIL=1.00 LS BEND=1.00									
COMP=1.00 SHEAR=1.00 TENS=1.00									
CALCULATED VERT. DEFLECTION = L/999 (0.14")									
CST 1C=0.48 (F-1), BC=0.89 (4-K-1), WB=0.88 (D-1), SSI=0.81 (K-L-1)									

AUTOSOLVE HEELS OFF
 TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL IN
 THE TRUSS MANUFACTURING PLANT.
 NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI)
 MAX MIN MAX MIN
 MT20 618 354 1667 822 2264 1656
 PLATE PLACEMENT - 10.250 in.





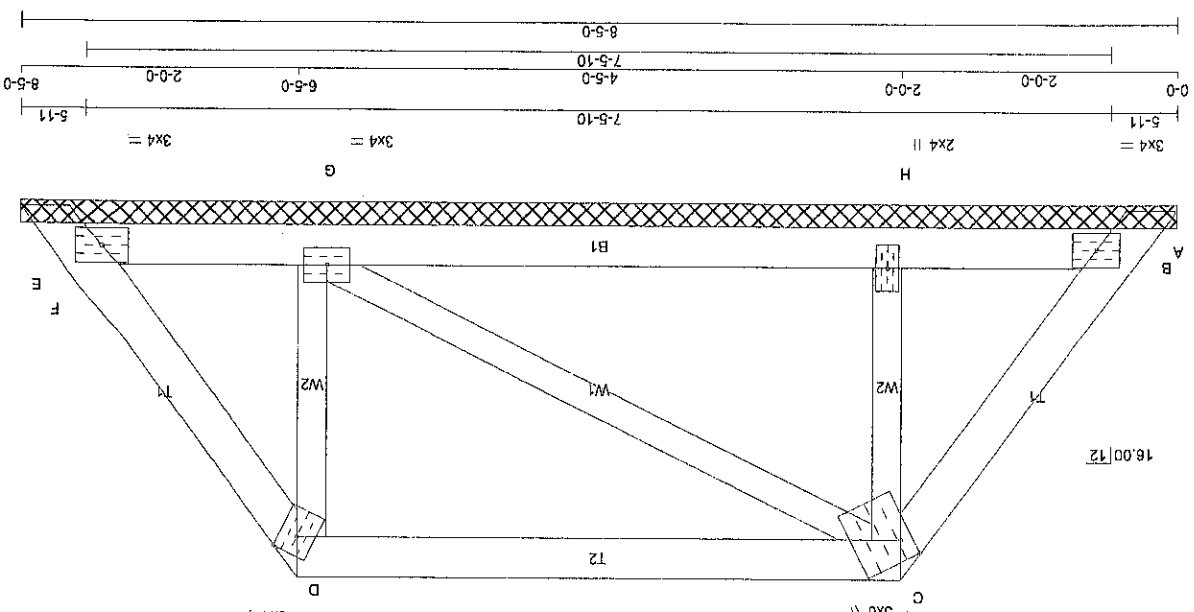
NOTES - (1)
OSC 9.23.13.11, and no less than 2x4 for Part 4 design.
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

PLATE	TYPE	W	LEN	Y	X
B	TM81	MT20	3.0	4.0	
C	TTW+m	MT20	6.0	4.0	
D	TTW+m	MT20	3.0	4.0	
E	TM81	MT20	3.0	4.0	
G	BMW1	MT20	3.0	4.0	
H	BMW1	MT20	2.0	4.0	

DESCR.	LUMBER	SIZE	CHORDS	N.L.G.A. RULES	ALL WEBS	2x3	DRY	SPF
A	C	2x4	DRY	No.2				
C	D	2x4	DRY	No.2				
D	F	2x4	DRY	No.2				
B	E	2x4	DRY	No.2				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED	MAXIMUM FACTORED	INPUT	BRG	RECORD	BRG	VERT	UP/LIFT	IN-SX	FACTORED
UT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	DOWN	HORIZ	UP/LIFT	IN-SX	FACTORED
A	-36	0	0	0	-36	0	0	0	0
F	-37	0	0	0	-37	0	0	0	0
B	199	0	0	0	199	0	0	0	0
E	186	0	0	0	186	0	0	0	0
H	191	0	0	0	191	0	0	0	0
G	216	0	0	0	216	0	0	0	0
VALUE IN PARENTHESES INDICATES EFFECTIVE BEARING LENGTH									
A	-36	0	0	0	-36	0	0	0	0
F	-37	0	0	0	-37	0	0	0	0
B	199	0	0	0	199	0	0	0	0
E	186	0	0	0	186	0	0	0	0
H	191	0	0	0	191	0	0	0	0
G	216	0	0	0	216	0	0	0	0
BEVELLED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S) A, F									
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT									
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT									
UNFACTORED REACTIONS									
1ST CASE	SNOW	LIVE	PERMITIVE	WIND	DEAD	SOIL			
A	-24	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
F	-25	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
B	136	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
E	127	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
H	137	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
G	154	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, E, H, G									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB. FORCE (LBS)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)
FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED
CS (LBS)	CS (LBS)	CS (LBS)	CS (LBS)	CS (LBS)	CS (LBS)	CS (LBS)	CS (LBS)	CS (LBS)	CS (LBS)
FROM TO	FROM TO	FROM TO	FROM TO	FROM TO	FROM TO	FROM TO	FROM TO	FROM TO	FROM TO
A-B	0/36	-98.0	-88.0	0.03 (1)	10.00	H-C	-126/0		0.02 (1)
B-C	-90.0	-70.5	-70.5	0.02 (1)	8.25	C-G	-12/0		0.00 (1)
C-D	-29.0	-70.5	-70.5	0.03 (1)	8.25	G-D	-146/0		0.02 (1)
D-E	-73.0	-70.5	-70.5	0.03 (1)	8.25	G-D	-146/0		0.02 (1)
E-F	0/37	-88.0	-88.0	0.03 (1)	10.00	H-C	-126/0		0.02 (1)
F-G	0/40	-17.5	-17.5	0.06 (4)	10.00	H-C	-126/0		0.02 (1)
G-H	0/40	-17.5	-17.5	0.06 (4)	10.00	H-C	-126/0		0.02 (1)
H-E	0/36	-17.5	-17.5	0.06 (4)	10.00	H-C	-126/0		0.02 (1)

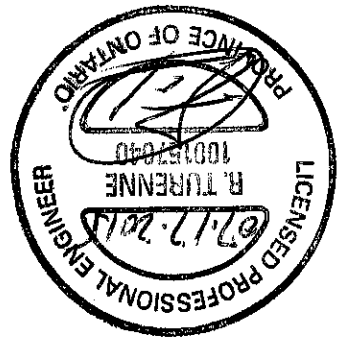


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H09A	1	1	TRUSS DESC.	

Alpha Roof Truss, Maple

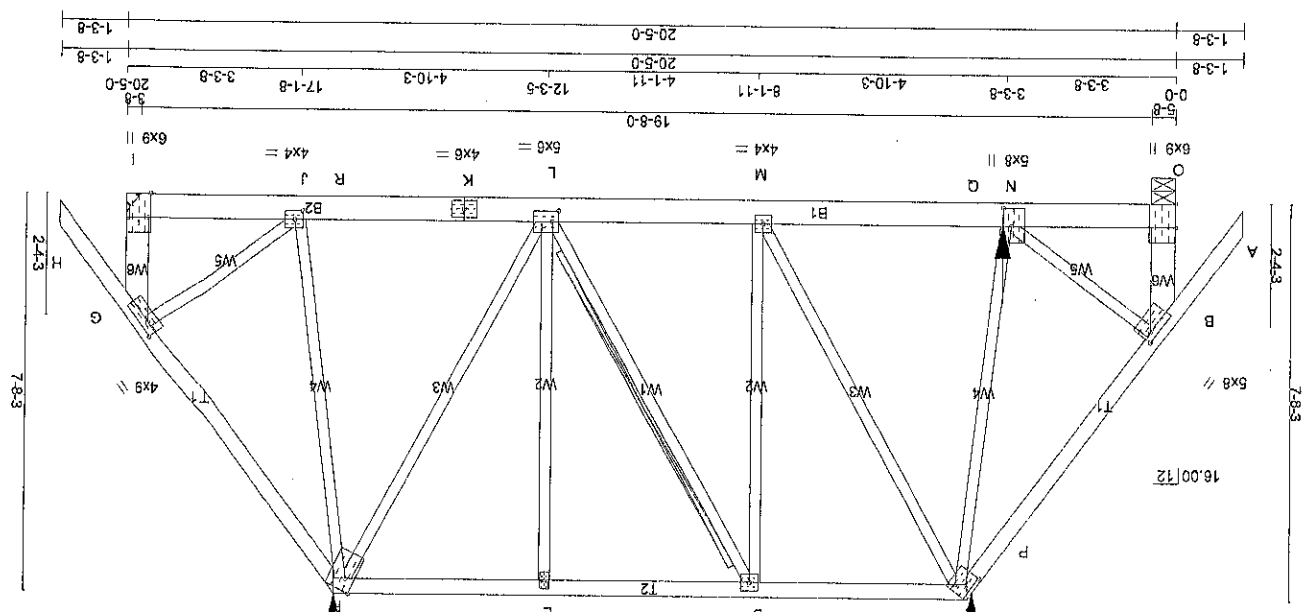
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Version 7.620 S Apr 15 2016 Mittek Industries, Inc. Fri Jul 17 11:12:58 2015 Page 2

NOTES- (1)
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per



NAIL VALUES
SECTION SHEAR (PLI) (PSI)
MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656
PLATE GRIP(DRY) SHEAR
PLATE GRIP=0.89 (J) (INPUT = 0.90)
USI METAL=0.56 (H) (INPUT = 1.00)
PLATE ROTATION TOL. = 5.0 Deg.
PLATE PLACEMENT TOL. = 0.250 inches

A-15073842(2)



DESIGN CRITERIA
TOTAL WEIGHT = 127 lb

... SPECIAL LOADS ANALYSIS ...
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE
SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
BOT CH. LL = 7.0 PSF
TOTAL LOAD = 31.0 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM
GIRDER TYPE: CPMmhp
SIDE SETBACK = 0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
ADDTL LOADS BASED ON 56% OF GSL.
2x4 DRY SPF NO.2 T-BRACE REQUIRED AT D1.
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER
90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW
LOADING
TOTAL LOAD CASES: (4)

CHORDS
MAX. FACTORED
FORCE
MEMB. FACTORED
FORCE
VERT. LOAD L1
MAX
UNBRACED
LENGTH
FROM TO
FR-TO
A-B
-2772/0
B-C
-2772/0
C-D
-1626/0
D-E
-1389/0
E-F
-1389/0
F-G
-1389/0
G-H
-1389/0
H-I
-1389/0
I-J
-1389/0
J-K
-1389/0
K-L
-1389/0
L-M
-1389/0
M-N
-1389/0
N-O
-1389/0
O-P
-1389/0
P-Q
-1389/0
Q-R
-1389/0
R-S
-1389/0
S-T
-1389/0
T-U
-1389/0
U-V
-1389/0
V-W
-1389/0
W-X
-1389/0
X-Y
-1389/0
Y-Z
-1389/0

PLATES (table in inches)
W L E N Y X
B TMWV-1 MT20 5.0 8.0 1.50 2.00
C TMWV-1 MT20 5.0 8.0 3.50 4.50
D TMWV-1 MT20 4.0 4.0 4.0
E TMWV-1 MT20 4.0 4.0 4.0
F TMWV-1 MT20 4.0 4.0 4.0
G TMWV-1 MT20 4.0 4.0 4.0
H TMWV-1 MT20 4.0 4.0 4.0
I TMWV-1 MT20 4.0 4.0 4.0
J TMWV-1 MT20 4.0 4.0 4.0
K TMWV-1 MT20 4.0 4.0 4.0
L TMWV-1 MT20 4.0 4.0 4.0
M TMWV-1 MT20 4.0 4.0 4.0
N TMWV-1 MT20 4.0 4.0 4.0
O TMWV-1 MT20 4.0 4.0 4.0
P TMWV-1 MT20 4.0 4.0 4.0
Q TMWV-1 MT20 4.0 4.0 4.0
R TMWV-1 MT20 4.0 4.0 4.0
S TMWV-1 MT20 4.0 4.0 4.0
T TMWV-1 MT20 4.0 4.0 4.0
U TMWV-1 MT20 4.0 4.0 4.0
V TMWV-1 MT20 4.0 4.0 4.0
W TMWV-1 MT20 4.0 4.0 4.0
X TMWV-1 MT20 4.0 4.0 4.0
Y TMWV-1 MT20 4.0 4.0 4.0
Z TMWV-1 MT20 4.0 4.0 4.0

HANGERS NOTES
TOUCHES EDGE OF CHORD.
EDGE - INDICATES REFERENCE CORNER OF PLATE
O BMV1+1 MT20 6.0 9.0 5.50
N BMWV-1 MT20 6.0 8.0 4.25 2.00
M BMWV-1 MT20 4.0 4.0
L BMWV-1 MT20 5.0 5.0 2.50 1.50
K BS-1 MT20 4.0 6.0
J BMV1+0 MT20 4.0 4.0 2.00 1.75
I TMWV-1 MT20 4.0 9.0 1.75 3.00
H TMWV-1 MT20 6.0 9.0 Edge 2.00
G TMWV-1 MT20 6.0 9.0
F TMWV-1 MT20 2.0 4.0
E TMWV-1 MT20 4.0 4.0
D TMWV-1 MT20 4.0 4.0
C TMWV-1 MT20 5.0 6.0
B TMWV-1 MT20 5.0 8.0 1.50 2.00

FACTORED CONCENTRATED LOADS (LBS)
O-N 0/0
O-M 0/1500
O-L 0/1500
O-K 0/1500
O-J 0/1500
O-I 0/1500
O-H 0/1500
O-G 0/1500
O-F 0/1500
O-E 0/1500
O-D 0/1500
O-C 0/1500
O-B 0/1500
O-A 0/1500

ALLOWABLE DEFLECTION (LBS)
O-N 0.00
O-M 0.00
O-L 0.00
O-K 0.00
O-J 0.00
O-I 0.00
O-H 0.00
O-G 0.00
O-F 0.00
O-E 0.00
O-D 0.00
O-C 0.00
O-B 0.00
O-A 0.00



253632	H08A	1	1	TRUSS DESC.	DRWG NO.
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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1174 lbs FACTORED DOWN AT 16'-6" ON TOP CHORD, AND 2194.3 lbs FACTORED DOWN AT 4'-0" ON AT 3'-4" ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES - (1)

OBC 9.23.13.11, and no less than 2x4 for Part 9 design as per

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per



AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI)

MAX MIN MAX MIN

MT20 618 364 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)

JSI METAL = 0.64 (C) (INPUT = 1.00)

A-1073840(2)



HANGERS NOTES
1) SPECIAL HANGERS OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 117.4 lbs FACTORED DOWN AT 4.0-0 ON AND 117.4 lbs FACTORED DOWN AT 4.0-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

Edge - INDICATES REFERENCE CORNER OF PLATE

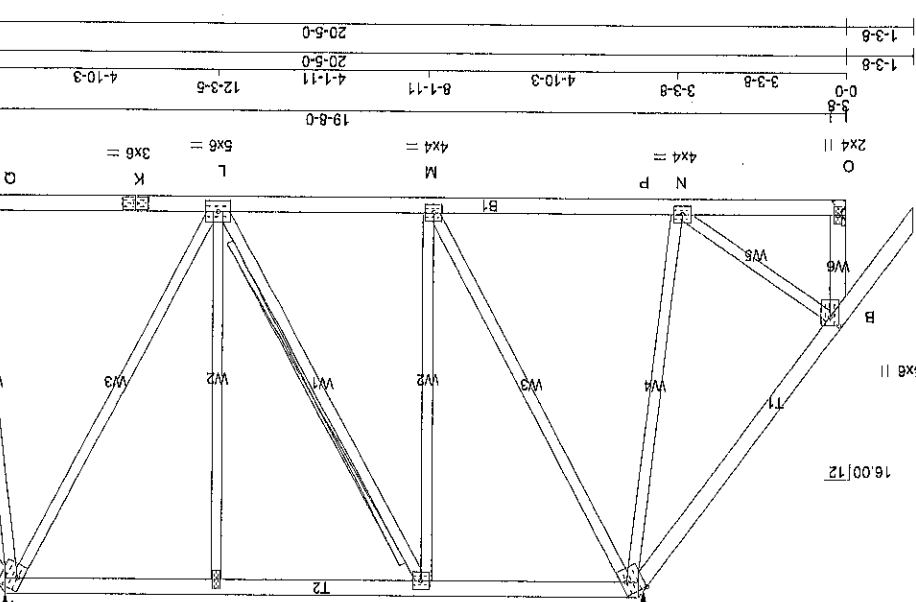
Edge	PLATE (table is in inches)	W	LEN	Y	X
O	BMV1+p	MT20	2.0	4.0	
L	BMVWV-1	MT20	6.0	6.0	
K	BS-1	MT20	3.0	6.0	
J	BMVWV-1	MT20	4.0	4.0	
I	J.M.N	MT20	4.0	4.0	
G	BMV1+p	MT20	2.0	4.0	
F	TMVW+p	MT20	4.0	6.0	2.00
E	TMVW+m	MT20	6.0	6.0	Edge 3.75
D	TMVW-1	MT20	2.0	4.0	
C	TMVW-1	MT20	4.0	4.0	
B	TMVW+p	MT20	6.0	6.0	Edge 3.75
A	TMVW-1	MT20	4.0	2.00	

LUMBER
N.L.G.A. RULES
CHORDS
SIZE

DESCR.	SPF	NO.2	DRY	2x4	ALL WEBS	EXCEPT	2x3	DRY	No.2	SPF
A - C	SPF	No.2	DRY	2x4	A - C	K - I	2x4	DRY	No.2	SPF
C - F	SPF	No.2	DRY	2x4	I - G	O - K	2x4	DRY	No.2	SPF
F - H	SPF	No.2	DRY	2x4	O - B	K - I	2x4	DRY	No.2	SPF
H - J	SPF	No.2	DRY	2x4	J - M	I - G	2x4	DRY	No.2	SPF
J - L	SPF	No.2	DRY	2x4	L - BMVWV-1	K - I	2x4	DRY	No.2	SPF
L - BMVWV-1	SPF	No.2	DRY	2x4	BMV1+p	O - K	2x4	DRY	No.2	SPF
BMV1+p	SPF	No.2	DRY	2x4						

CHORDS
MEMB. FORCE
FACTORED

CHORDS	MEMB. FORCE	FACTORED	CHORDS	MEMB. FORCE	FACTORED
O-N	0.0	0.0	O-N	0.0	0.0
N-P	0.0	0.0	N-P	0.0	0.0
P-M	0.0	0.0	P-M	0.0	0.0
M-L	0.0	0.0	M-L	0.0	0.0
L-K	0.0	0.0	L-K	0.0	0.0
K-O	0.0	0.0	K-O	0.0	0.0
O-J	0.0	0.0	O-J	0.0	0.0
J-I	0.0	0.0	J-I	0.0	0.0
I-G	0.0	0.0	I-G	0.0	0.0
G-H	0.0	0.0	G-H	0.0	0.0
H-F	0.0	0.0	H-F	0.0	0.0
F-E	0.0	0.0	F-E	0.0	0.0
E-D	0.0	0.0	E-D	0.0	0.0
D-C	0.0	0.0	D-C	0.0	0.0
C-B	0.0	0.0	C-B	0.0	0.0
B-A	0.0	0.0	B-A	0.0	0.0
A-B	0.0	0.0	A-B	0.0	0.0
B-C	0.0	0.0	B-C	0.0	0.0
C-D	0.0	0.0	C-D	0.0	0.0
D-E	0.0	0.0	D-E	0.0	0.0
E-F	0.0	0.0	E-F	0.0	0.0
F-G	0.0	0.0	F-G	0.0	0.0
G-H	0.0	0.0	G-H	0.0	0.0
H-I	0.0	0.0	H-I	0.0	0.0
I-J	0.0	0.0	I-J	0.0	0.0
J-K	0.0	0.0	J-K	0.0	0.0
K-L	0.0	0.0	K-L	0.0	0.0
L-M	0.0	0.0	L-M	0.0	0.0
M-N	0.0	0.0	M-N	0.0	0.0
N-O	0.0	0.0	N-O	0.0	0.0



Version 7.620 S Apr 15 2015 Mitek Industries, Inc. File 17 11:22:22 TO00qUkyxgGf
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TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.
253632	1	1		

DRWG NO.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF
SPACING = 24.0 IN. G.C.
LOADING IN PLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM
GIRDER TYPE: CPMhip
SIDE SETBACK = 4.0-0
END SETBACK = 4.0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
-ADPT. LOADS BASED ON 65% OF GSL.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010
THIS DESIGN COMPLETES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-08
- TPC 2011
65% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFLL (LL) = L/360 (0.68")
ALLOWABLE VERT. DEFLL (LL) = L/999 (0.03")
ALLOWABLE DEFLL (LL) = L/360 (0.68")
ALLOWABLE VERT. DEFLL (LL) = L/999 (0.03")
CALCULATED VERT. DEFLL (LL) = L/999 (0.03")
CS: TC=0.28 (C-D-1), BC=0.23 (L-M-1),
WB=0.51 (D-M-1), SS=0.22 (C-D-1),
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 0.60
TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL)
MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1666
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI METAL=0.81 (G) (INPUT = 0.80)
JSI METAL=0.26 (I) (INPUT = 1.00)

Scale = 1:44.7

DRWG NO.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	X
B	TMWV4	MT20	7.0	12.0	2.75
C	TMWV4	MT20	4.0	2.00	1.50
D	TMWV4	MT20	4.0	4.0	1.75
E	TS4	MT20	3.0	8.0	
F	TTWV4	MT18HS	6.0	14.0	1.50
G	TTWV4	MT20	2.0	4.0	
H	TTWV4	MT18HS	6.0	14.0	1.50
I	TS4	MT20	3.0	8.0	
J	TMWV4	MT20	4.0	4.0	1.75
K	TMWV4	MT20	4.0	2.00	1.50
L	TMWV4	MT20	7.0	12.0	2.75
N	BMV14	MT20	6.0	8.0	Edge 0.50
O	BMWV4	MT20	8.0	9.0	
P	BMWV4	MT20	4.0	6.0	
Q	BS4	MT20	5.0	6.0	
R	BMWV4	MT20	4.0	8.0	
S	BMWV4	MT20	6.0	7.0	4.00
T	BS4	MT20	4.0	9.0	
U	BS4	MT20	5.0	8.0	
V	BMWV4	MT20	4.0	6.0	
W	BMWV4	MT20	8.0	9.0	4.25
X	BMV14	MT20	6.0	8.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGERS OR CONNECTIONS REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1813 lbs FACTORED DOWN AT 4-0-12, AND 1433.5 lbs FACTORED DOWN AT 13-3-12, AND 1418.3 lbs FACTORED DOWN AT 28-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES - (1)
OBC 8.2.3, 13.11, and no less than 2x4 for Part 4 design.
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per



AUTOSOLVE HELLS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI)

MAX MIN MAX MIN (PLI)

MT20 618 364 1667 822 2284 1656

MT18HS 580 364 2729 1405 4191 2010

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (D) (INPUT = 0.90)

JSI METAL = 0.98 (B) (INPUT = 1.00)

A-11073838(2)

LUMBER	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY	DESIGN CRITERIA	(Mile)
N. L. G. A. RULES	BUILDING DESIGNER		

TOTAL WEIGHT = 2 X 170 = 339 lb

[illegible]

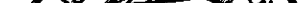
DRY; SEASONED LUMBER.

PLATES (table in inches)										
PLATE	TYPE	W	LEN	Y	X					
B	TIMBER-1	MT20	4.0	6.0	1.75	3.00				
C, I, K										
D	TS-1	MT20	3.0	6.0		1.50				
E	TIMBER-1	MT20	5.0	6.0	1.75	1.50				
F	TIMBER-1	MT20	4.0	4.0		1.50				
G	TIMBER-1	MT20	2.0	4.0		2.50				
H	TIMBER-1	MT20	4.0	4.0		2.50				
J	TS-1	MT20	3.0	6.0		1.50				
TOTAL LOAD CASES: (4)										
LOADING										
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN										
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW										
1 LATERAL BRACES) REQUIRED AT 1/2 LENGTH OF FU.										
APPLIED.										
DO NOT OVER STRENGTHEN OR MAXIMUM SPACING 4'0" MAX.										
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY										
THIS DESIGN COMPLIES WITH										
-PART 9 OF CBC 2012, CBC 2012, ABC 2014										
-CSA 086-09										
-TPIC 2011										
DESIGN ASSUMPTIONS										
OVERHANG NOT TO BE ALTERED OR CUT										
OFF.										
(65% OF 23.0 P.S.F. G.S.L. PLUS										
8.4 P.S.F. RAIN LOAD EQUALS										
21.0 P.S.F. SPECIFIED MAX LIVE LOAD										
ALLOWABLE DEFLECTION = L/360 (1.10")										
CALCULATED VERT. DEFLECTION = L/989 (0.12")										

[illegible]

Q	P	R	S	T	U	V	W	X	Y	Z	AA	AB	AC	AD	AE	AF	AG	AH	AI	AJ	AK	AL	AM	AN	AO	AP	AQ	AR	AS	AT	AU	AV	AW	AX	AY	AZ	BA	BB	BC	BD	BE	BF	BG	BH	BI	BJ	BK	BL	BM	BN	BO	BP	BQ	BR	BS	BT	BU	BV	BW	BX	BY	BZ	CA	CB	CC	CD	CE	CF	CG	CH	CI	CJ	CK	CL	CM	CN	CO	CP	CQ	CR	CS	CT	CU	CV	CW	CX	CY	CZ	DA	DB	DC	DD	DE	DF	DG	DH	DI	DJ	DK	DL	DM	DN	DO	DP	DQ	DR	DS	DT	DU	DV	DW	DX	DY	DZ	EA	EB	EC	ED	EE	EF	EG	EH	EI	EJ	EK	EL	EM	EN	EO	EP	EQ	ER	ES	ET	EU	EV	EW	EX	EY	EZ	FA	FB	FC	FD	FE	FF	FG	FH	FI	FJ	FK	FL	FM	FN	FO	FP	FQ	FR	FS	FT	FU	FV	FW	FX	FY	FZ	GA	GB	GC	GD	GE	GF	GG	GH	GI	GJ	GK	GL	GM	GN	GO	GP	GQ	GR	GS	GT	GU	GV	GW	GX	GY	GZ	HA	HB	HC	HD	HE	HF	HG	HH	HI	HJ	HK	HL	HM	HN	HO	HP	HQ	HR	HS	HT	HU	HV	HW	HX	HY	HZ	IA	IB	IC	ID	IE	IF	IG	IH	II	IJ	IK	IL	IM	IN	IO	IP	IQ	IR	IS	IT	IU	IV	IW	IX	IY	IZ	JA	JB	JC	JD	JE	JF	JG
---	---	---	---	---	---	---	---	---	---	---	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----

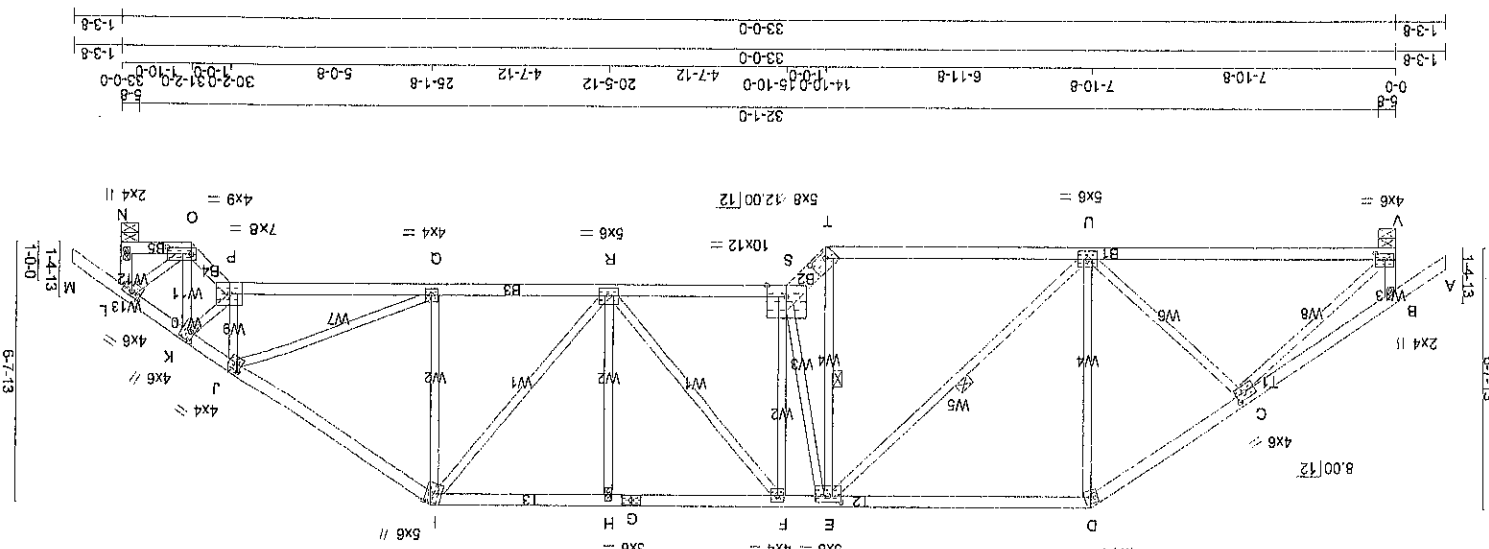
USJMETAL=0.55 (W) (INPUT = 1.00)
USJGMP=0.87 (W) (INPUT = 0.90)



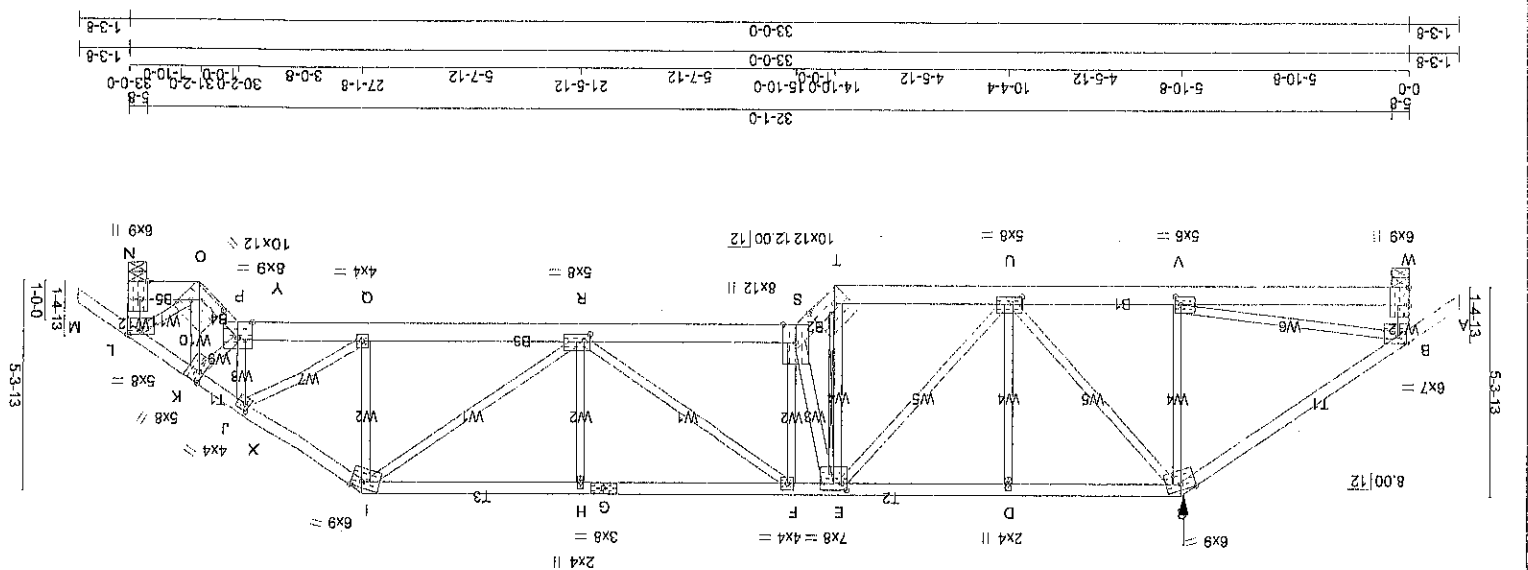
Page 10 of 10

7-1073834		
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[illegible][illegible][illegible]

ISI GRIP = 0.88 (C) (INPUT = 0.90)
ISI METAL = 0.46 (C) (INPUT = 1.00)



LUMBER										A. C. RULES										N. L. G. A. RULES													
NUMBER										CHORDS										SIZE													
A	C	2x4	DRY	No.2	SPP	BEARINGS	MAXIMUM FACTORED	INPUT	REGRD	A	C	2x4	DRY	1650F 1.5E	SPP	GROSS REACTION	DOWN	BRG	IN-SX	A	C	2x4	DRY	No.2	SPP	BEARINGS	MAXIMUM FACTORED	INPUT	REGRD				
I	M	2x4	DRY	No.2	SPP	VERT	HORZ	BRG	IN-SX	I	M	2x4	DRY	No.2	SPP	VERT	HORZ	BRG	IN-SX	I	M	2x4	DRY	No.2	SPP	VERT	HORZ	BRG	IN-SX				
W	B	2x4	DRY	No.2	SPP	2836	0	2836	0	W	B	2x4	DRY	No.2	SPP	2836	0	2836	0	W	B	2x4	DRY	No.2	SPP	2836	0	2836	0				
N	L	2x4	DRY	No.2	SPP	2977	0	2977	0	N	L	2x4	DRY	No.2	SPP	2977	0	2977	0	N	L	2x4	DRY	No.2	SPP	2977	0	2977	0				
ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD										ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD										ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD													
S	P	2x6	DRY	1650F 1.5E	SPP					S	P	2x6	DRY	No.2	SPP					S	P	2x6	DRY	No.2	SPP								
T	S	2x6	DRY	No.2	SPP					T	S	2x6	DRY	No.2	SPP					T	S	2x6	DRY	No.2	SPP								
O	N	2x6	DRY	No.2	SPP					O	N	2x6	DRY	No.2	SPP					O	N	2x6	DRY	No.2	SPP								
UNFACTORED REACTIONS										UNFACTORED REACTIONS										UNFACTORED REACTIONS													
SPP	SPP	SPP	SPP	SPP	SPP	1ST CASE	MAX/MIN COMPONENT REACTIONS																										
W	W	W	W	W	W	COMBINED	SNOW	LIVE	PERM	WIND	WIND	DEAD	SOIL																				
N	N	N	N	N	N	2096	1436 / 0	0 / 0	0 / 0	0 / 0	660 / 0	0 / 0	0 / 0																				
BEARING MATERIAL TO BE SPP NO.2 OR BETTER AT JOINT(S) W. N.										BEARING MATERIAL TO BE SPP NO.2 OR BETTER AT JOINT(S) W. N.										BEARING MATERIAL TO BE SPP NO.2 OR BETTER AT JOINT(S) W. N.													
DRY: SEASONED LUMBER.										DRY: SEASONED LUMBER.										DRY: SEASONED LUMBER.													

DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH.	DL	=	21.0	PSF	TOP CH.	DL	=	3.0	PSF
BOT CH.	DL	=	7.0	PSF	BOT CH.	DL	=	0.0	PSF
TOTAL LOAD				=	31.0	PSF			
SPACING = 24.0 IN./C/C									
LOADING IN FLAT SECTION BASED ON A									
SLOPE OF 2.00/12 MINIMUM									
GRADE TYPE: Cpmhip									
SIDE SETBACK = 5-10-8									
END SETBACK = 5-10-8									
END WALL WIDTH = 0-0									

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
LUMBER									

[M]									
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PLATES (table in inches)		BRACING	
BT	BM2W-2	6.0	2.26
D	TM2W-m	6.0	2.26
C	TM2W-w	6.0	2.26
F	TM2W-w	7.0	2.00
G	TS-4	3.0	8.0
H	TM2W-w	2.0	4.0
I	TM2W-m	6.0	3.50
J	TM2W-w	4.0	2.00
K	TM2W-w	5.0	3.26
L	BM2W-2	6.0	2.26
N	BM2W-1	6.0	2.26
P	BM2W-1	6.0	2.26
R	BM2W-w	6.0	2.26
S	BR2W-w	6.0	2.26
T	BR2W-w	6.0	2.26
U	BM2W-w	6.0	2.26
V	BM2W-1	6.0	2.26
W	BM2W-1	6.0	2.26

[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253630	H01T	1	1	TRUSS DESC.	

Alpha Roof Truss, Maple

ID:AYrJx9y7fa2EXF7DRQXSq4zMEz1-RZIEgIA3INQyDmnhzubbhVYxk08b5sm6lIPxftXxDOL
Version 7.620 S Apr 15 2015 MITek Industries, Inc. Fri Jul 17 09:40:56 2016 Page 2

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 301.8 lbs FACTORED DOWN AT 5-10-8
ON TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

NOTES- (1)

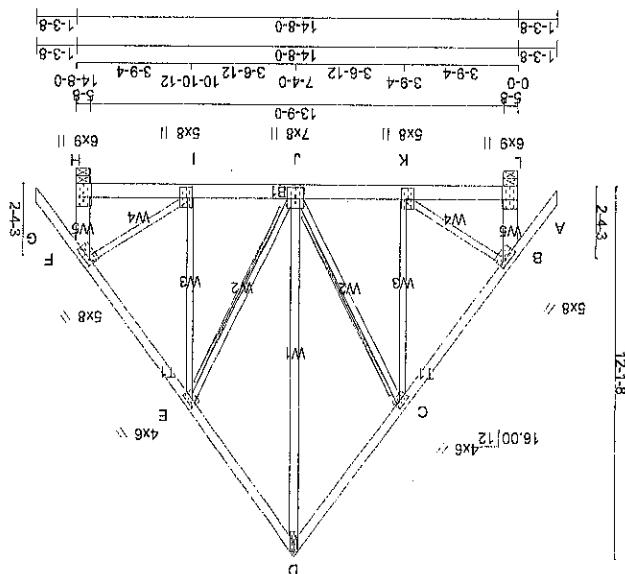
OBC 9.23, 13.11, and no less than 2x4 for Part 9 design as per



DOL LUMBER=1.00 NAIL=1.00 L8 BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 0.50
TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP=0.90 (U) (NPUT = 0.90)
JSI METAL=0.84 (E) (NPUT = 1.00)

A-17073831(2)

JOB NAME	TRUSS NAME	1	QUANTITY	1	JOB DESC.	TRUSS DESC.	DRWG NO.
253629	H11A						



LUMBER N I G A RULES

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPACING = 24.0 IN. C/C

TOTAL LOAD	=	31.0 PSF
DL	=	7.0 PSF
BOT CH, LL	=	0.0 PSF
DL	=	3.0 PSF
TOP CH, LL	=	21.0 PSF

SPECIFIED LOADS:

*** NON STANDARD GIRDER ***
 ADD USER-DEFINED LOADS APPLIED TO
 ALL LOAD CASES.
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL
 OR SMALL BUILDING REQUIREMENTS OF
 PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 8 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-08
- TPIC 2011
75% OF 23.0 P.S.F. , G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
10.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.49")
CALCULATED VERT. DEF. (LL) = L/999 (0.06")
ALLOWABLE DEF. (TL) = L/360 (0.49")
CALCULATED VERT. DEF. (TL) = L/999 (0.11")
CS: TC = .33 (E-F1), BC = .58 (J-K:1), WB = .56 (D-J:1), SS = 0.67 (H-I:1)

COMPANION LIVE LOAD FACTOR = 0.50

RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
(S) GRIP = 0.89 (F) (INPUT = 0.90)
(S) METAL = 0.59 (D) (INPUT = 1.00)

A-1073826

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	BOX	LUMBER	NO.	DESCR.

SPF	ALL WEBS	EXCEPT	J - D	2x4	DRY	No.2	SPF
SPF	2x3	DRY	No.2				
SPF	2x6	DRY	No.2				
SPF	2x6	DRY	No.2				
SPF	2x6	DRY	No.2				
SPF	2x4	DRY	No.2				
SPF	2x4	DRY	No.2				

PLATES (table is in inches)		J.T. TYPE		PLATES		W		LEN Y		X	
C	BMWW-4	MT20	6.0	8.0	1.75	2.00					
B	TMWW-4	MT20	6.0	8.0	2.00	2.25					
E	TMWW-4	MT20	4.0	3.0	3.75	Edge					
D	TMWW-4	MT20	4.0	3.0	2.00	2.25					
F	TMWW-4	MT20	5.0	6.0	1.75	2.00					
H	BMW-4	MT20	6.0	8.0	Edge	0.50					
I	BMWW-4	MT20	7.0	8.0	4.25	2.00					
J	BMWW-4	MT20	7.0	8.0	4.25	2.00					
K	BMWW-4	MT20	6.0	8.0	4.25	2.00					
L	BMW-4	MT20	6.0	8.0	5.0						

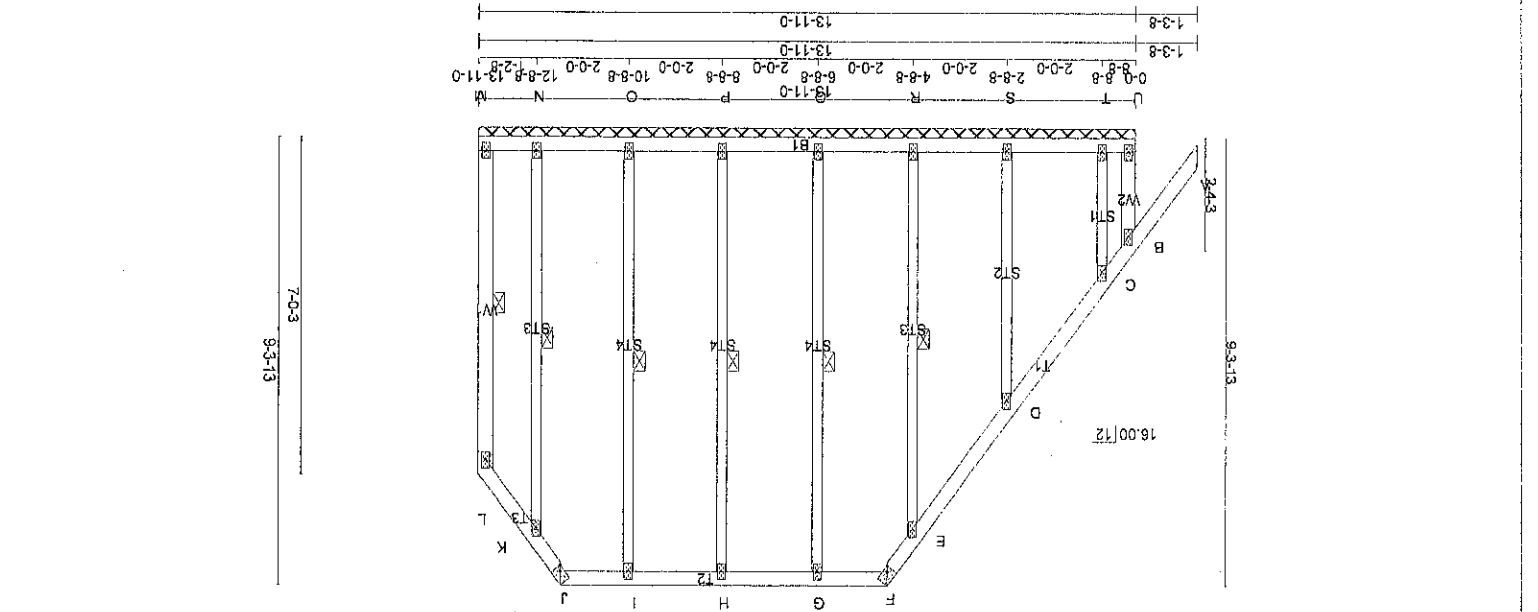
NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 8.9, 23.13.11, and no less than 2x4 for Part 4 design.

[illegible]

A circular professional engineer seal. The outer ring contains the text "REGISTERED PROFESSIONAL ENGINEER" at the top and "STATE OF TEXAS" at the bottom. The inner circle contains the number "17,720" and the year "2017".


 R. TURENNE
 100157040
 LICENCE
 MINISTÈRE DE L'INTÉRIEUR





TOTAL WEIGHT = 91 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 210 PSF

DL = 3.0 PSF

DL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 310 PSF

SPACING = 24.0 IN. C/G

LOADING IN FLAT SECTION BASED ON

PIGBACK TRUSS WITH SLOPES OF 6.00/12

AND 6.00/12 AND RESPECTIVE WALL

HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL

DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

OR SMALL BUILDING REQUIREMENTS OF

PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT

OFF.

66% OF 23.0 P.S.F. G.S.L. PLUS

8.4 P.S.F. RAIN LOAD EQUALS

21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.10 (A-B:1), BC=0.02 (T-U:1), WB=0.10

(I-O:1), SSI=0.07 (I-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPLAN LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN

THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

SECTION SHEAR (PLI)

MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.19 (B) (INPUT = 0.80)

JSI METAL=0.06 (B) (INPUT = 1.00)

A-1073822

PROVINCE OF ONTARIO

100177040

R. TURKIN

07-17-2013

LICENSED PROFESSIONAL ENGINEER

NOTES: (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

OSBC 9.23.13.11, and no less than 2x4 for Part 4 design.

TOUCHES EDGE OF CHORD.

EDGE - INDICATES REFERENCE CORNER OF PLATE

U BMV1+p MT20 2.0 4.0

N BMV1+w MT20 2.0 4.0

M BMV1+p MT20 2.0 4.0

L TMV+p MT20 2.0 4.0

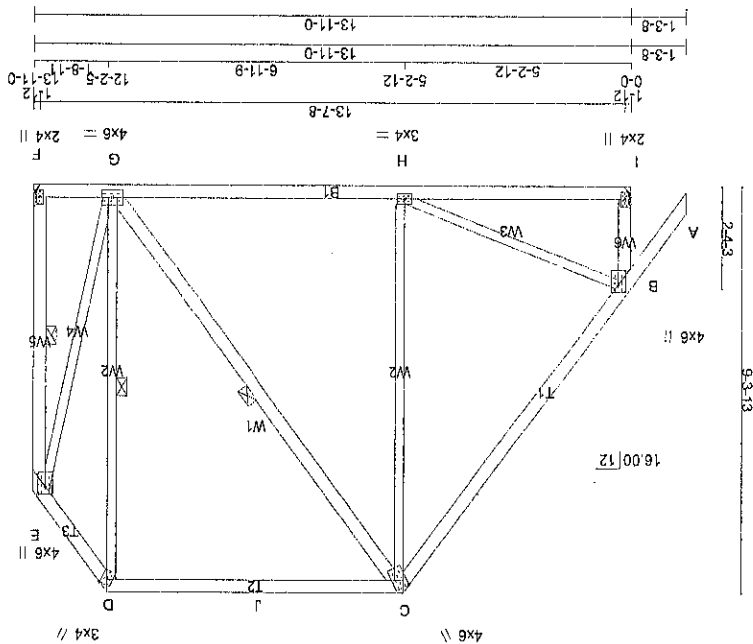
J TT-h MT20 3.0 4.0

F TT-h MT20 3.0 4.0

C TMV+w MT20 2.0 4.0

B TMV+p MT20 2.0 4.0

JT TYPE PLATES W LEN Y X



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

DESIGN CRITERIA

TOTAL WEIGHT = 6 X 87 = 521 lb

$$\overline{\text{SPACING}} = \frac{24.0}{11.5/\text{C/C}}$$

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BNBC 2012 , ABC 2014
- CSA 088-09

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF.L (LL) = L/360 (0.46")
CALCULATED VERT. DEF.L (LL) = L/999 (0.01")
ALLOWABLE DEF.L (LL) = L/360 (0.46")
CALCULATED VERT. DEF.L (LL) = L/999 (0.04")

WB=0.14 (C-G:1) , SSI=0.21 (C-D:1)

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

TABLE 1. Nail Values

MAX MIN	MAX MIN	MAX MIN	MAX MIN
618 354	1667 822	2284 1656	MT20

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP = 0.76 (G) (INPUT = 0.90)
USI METAL = 0.13 (G) (INPUT = 1.00)

15772871 ✓

1200/10000

<u>LUMBER</u>	N.L.G.A.RULES	CHORDS	SIZE	LUMBER	DESCR

A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	1650F 1.5E	SPF
D - E	2x4	DRY	No.2	SPF

1 - B	2x4	DRY	No.2	SPF
1 - E	2x4	DRY	No.2	SPF
1 - F	2x4	DRY	No.2	SPF

ALL WEBS	2X3	DRY	No.2	SPF	EXCEPT
----------	-----	-----	------	-----	--------

C - G	2x4	DRY	No.2	SPE	DRY: SEASONED LUMBER.
-------	-----	-----	------	-----	-----------------------

PLATES (table is in inches)	
JT TYPE	PLATES
W	LEN Y
X	

[illegible]

Model	Year	Price
BMW 1	2000	2.00
BMW 1	2001	2.00
BMW 1	2002	2.00
BMW 1	2003	2.00
BMW 1	2004	2.00
BMW 1	2005	2.00
BMW 1	2006	2.00
BMW 1	2007	2.00
BMW 1	2008	2.00
BMW 1	2009	2.00
BMW 1	2010	2.00
BMW 1	2011	2.00
BMW 1	2012	2.00
BMW 1	2013	2.00
BMW 1	2014	2.00
BMW 1	2015	2.00
BMW 1	2016	2.00
BMW 1	2017	2.00
BMW 1	2018	2.00
BMW 1	2019	2.00
BMW 1	2020	2.00
BMW 1	2021	2.00
BMW 1	2022	2.00
BMW 1	2023	2.00
BMW 1	2024	2.00
BMW 1	2025	2.00
BMW 1	2026	2.00
BMW 1	2027	2.00
BMW 1	2028	2.00
BMW 1	2029	2.00
BMW 1	2030	2.00
BMW 1	2031	2.00
BMW 1	2032	2.00
BMW 1	2033	2.00
BMW 1	2034	2.00
BMW 1	2035	2.00
BMW 1	2036	2.00
BMW 1	2037	2.00
BMW 1	2038	2.00
BMW 1	2039	2.00
BMW 1	2040	2.00
BMW 1	2041	2.00
BMW 1	2042	2.00
BMW 1	2043	2.00
BMW 1	2044	2.00
BMW 1	2045	2.00
BMW 1	2046	2.00
BMW 1	2047	2.00
BMW 1	2048	2.00
BMW 1	2049	2.00
BMW 1	2050	2.00
BMW 1	2051	2.00
BMW 1	2052	2.00
BMW 1	2053	2.00
BMW 1	2054	2.00
BMW 1	2055	2.00
BMW 1	2056	2.00
BMW 1	2057	2.00
BMW 1	2058	2.00
BMW 1	2059	2.00
BMW 1	2060	2.00
BMW 1	2061	2.00
BMW 1	2062	2.00
BMW 1	2063	2.00
BMW 1	2064	2.00
BMW 1	2065	2.00
BMW 1	2066	2.00
BMW 1	2067	2.00
BMW 1	2068	2.00
BMW 1	2069	2.00
BMW 1	2070	2.00
BMW 1	2071	2.00
BMW 1	2072	2.00
BMW 1	2073	2.00
BMW 1	2074	2.00
BMW 1	2075	2.00
BMW 1	2076	2.00
BMW 1	2077	2.00
BMW 1	2078	2.00
BMW 1	2079	2.00
BMW 1	2080	2.00
BMW 1	2081	2.00
BMW 1	2082	2.00
BMW 1	2083	2.00
BMW 1	2084	2.00
BMW 1	2085	2.00
BMW 1	2086	2.00
BMW 1	2087	2.00
BMW 1	2088	2.00
BMW 1	2089	2.00
BMW 1	2090	2.00
BMW 1	2091	2.00
BMW 1	2092	2.00
BMW 1	2093	2.00
BMW 1	2094	2.00
BMW 1	2095	2.00
BMW 1	2096	2.00
BMW 1	2097	2.00
BMW 1	2098	2.00
BMW 1	2099	2.00
BMW 1	2100	2.00
BMW 1	2101	2.00
BMW 1	2102	2.00
BMW 1	2103	2.00
BMW 1	2104	2.00
BMW 1	2105	2.00
BMW 1	2106	2.00
BMW 1	2107	2.00
BMW 1	2108	2.00
BMW 1	2109	2.00
BMW 1	2110	2.00
BMW 1	2111	2.00
BMW 1	2112	

1	BMV1+p	MT20	2.0	4.0
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TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

100% 99% 98% 97% 96% 95% 94% 93% 92% 91% 90% 89% 88% 87% 86% 85% 84% 83% 82% 81% 80% 79% 78% 77% 76% 75% 74% 73% 72% 71% 70% 69% 68% 67% 66% 65% 64% 63% 62% 61% 60% 59% 58% 57% 56% 55% 54% 53% 52% 51% 50% 49% 48% 47% 46% 45% 44% 43% 42% 41% 40% 39% 38% 37% 36% 35% 34% 33% 32% 31% 30% 29% 28% 27% 26% 25% 24% 23% 22% 21% 20% 19% 18% 17% 16% 15% 14% 13% 12% 11% 10% 9% 8% 7% 6% 5% 4% 3% 2% 1% 0%

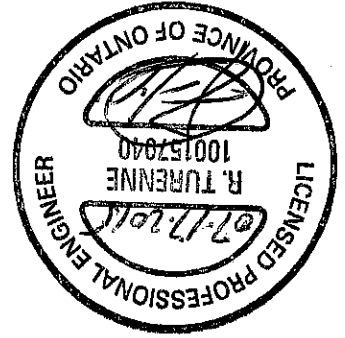


07/17/2015

RECEIVED
R. TURENNE
JAN 10 1964

100-87001

MINCE OF ONT.



NOTES - (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

PLATE (table is in inches)	W	LEN	Y	X
AA BMV1+p	2.0	4.0		
U BS4	2.0	4.0		
O BMV1+w	2.0	4.0		
Q, R, S, T, V, W, X, Y, Z	2.0	4.0		
P BMV1+p	2.0	4.0		
N TMV+p	2.0	4.0		
J TH-h	2.0	4.0		
F TMV+p	2.0	4.0		
C TMV+w	2.0	4.0		
G, D, E, G, H, I, K, L, M	2.0	4.0		
B TMV+p	2.0	4.0		
BT TYPE PLATES	2.0	4.0		

GABLE STUDS SPACED AT 24.0 OC.

DESCR.	LUMBER	SIZE	CHORDS	PLATES	W	LEN	Y	X
A - F	2x4	DRY	No.2					
F - J	2x4	DRY	No.2					
J - O	2x4	DRY	No.2					
AA - B	2x4	DRY	No.2					
P - N	2x4	DRY	No.2					
U - P	2x4	DRY	No.2					
ALL GABLE WEBS	2x3	DRY	No.2					
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.								
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.								
1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF H-V, G-W, E-X, I-T, K-S.								
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW								
LOADING TOTAL LOAD CASES: (4)								

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF H-V, G-W, E-X, I-T, K-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT

OFF.

(6% OF 23.0 P.S.F. G.S.L. PLUS

8.4 P.S.F. RAIN LOAD EQUALS

21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B=1), BC=0.01 (X-Y=4), WB=0.10

(G-W=1), SSI=0.07 (F-G=1)

COMP=1.10 SHEAR=1.10 TENS=1.10

COMP=1.10 LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN

THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

MAX MIN MAX MIN

M20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP=0.46 (F) (INPUT = 0.90)

JSI METAL=0.05 (N) (INPUT = 1.00)

A-N73826

Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Fri Jul 17 08:27:04 2015 Page 1

ID:AYrJx9V7re2EXF7DRQXsg4ZMEz1-Cw/ViVr1Ve2oVgyuK55K3KxHmHEIjgd7m9LPQ5gyxE5f

Scale = 1:50.0

DRWG NO.

TRUSS DESC.

QUANTITY

PLY

TRUSS DESC.

253629

H08G

Alpha Roof Truss, Maple

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
253629	H08A	1	1			

Alpha Roof Truss, Maple

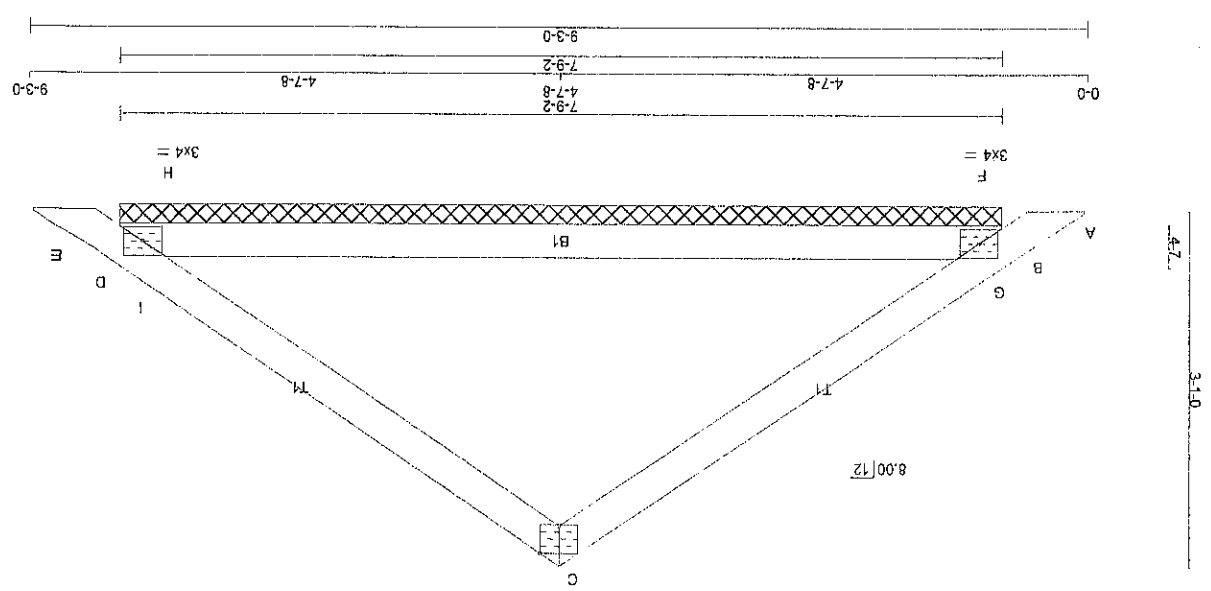
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Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Fri Jul 17 08:27:04 2016 Page 2

NOTES: (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI)
 MAX MIN MAX MIN
 MT20 618 354 1667 922 2284 1666
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.87 (J) (INPUT = 0.90)
 JSI METAL= 0.50 (G) (INPUT = 1.00)

A-11273819(2)



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

OR SMALL BUILDING REQUIREMENTS OF

PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 08-08

- TPI 2011

66% OF 23.0 P.S.F. G.S.L. PLUS

8.4 P.S.F. RAIN LOAD EQUALS

21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL: TC=0.20 (C-G:1), BC=0.17 (F-H:1),

WB=0.00 (F-G:1), SS=0.21 (B-G:4),

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN

THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI)

MAX MIN MAX MIN

MT20 618 364 1687 822 2284 1656

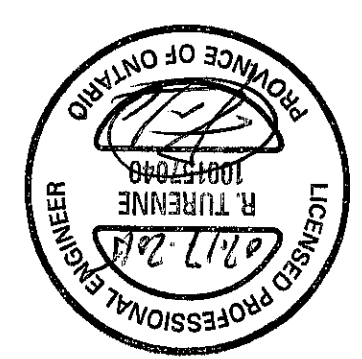
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.36 (D) (INPUT = 0.80)

JSI METAL=0.08 (D) (INPUT = 1.00)

4-NB73817



NOTES: (1) OBC 9.23.13.11, and no less than 1x4 for Part 9 design as per

TOUCHES EDGE OF CHORD.

PLATES (table in inches)

DRY, SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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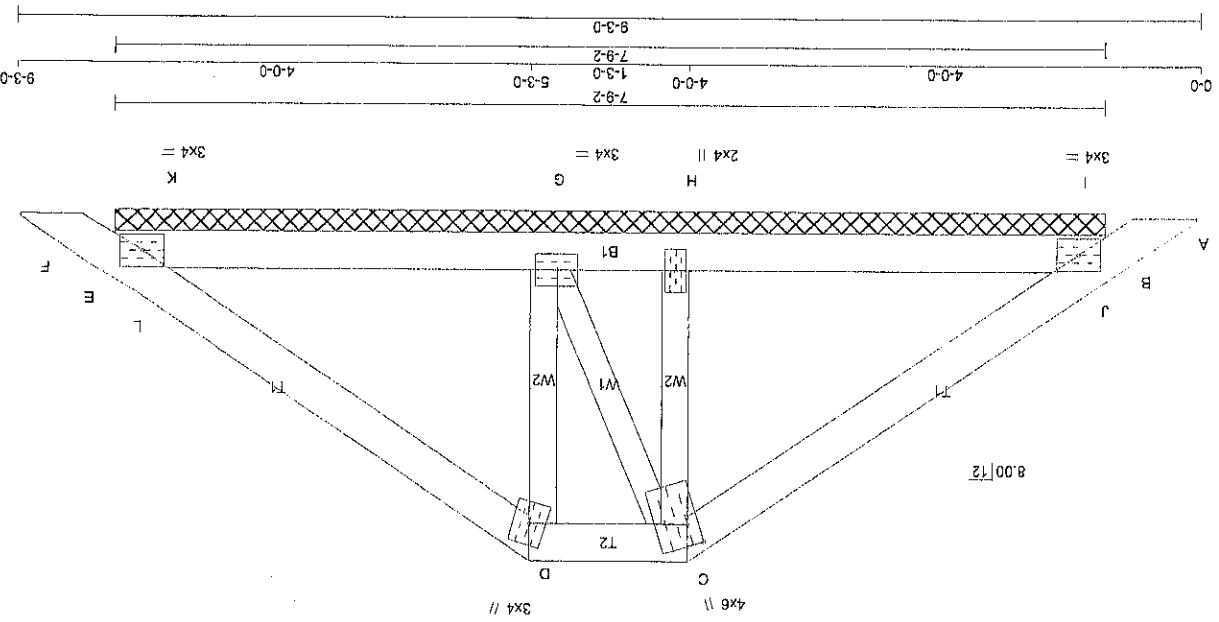
SPF

SPF

SPF

SPF

SPF



TOTAL WEIGHT = 26 lb

[illegible][illegible]

NOTES: (1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per CBC 8.23.13.11, and no less than 2x4 for Part 4 design.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

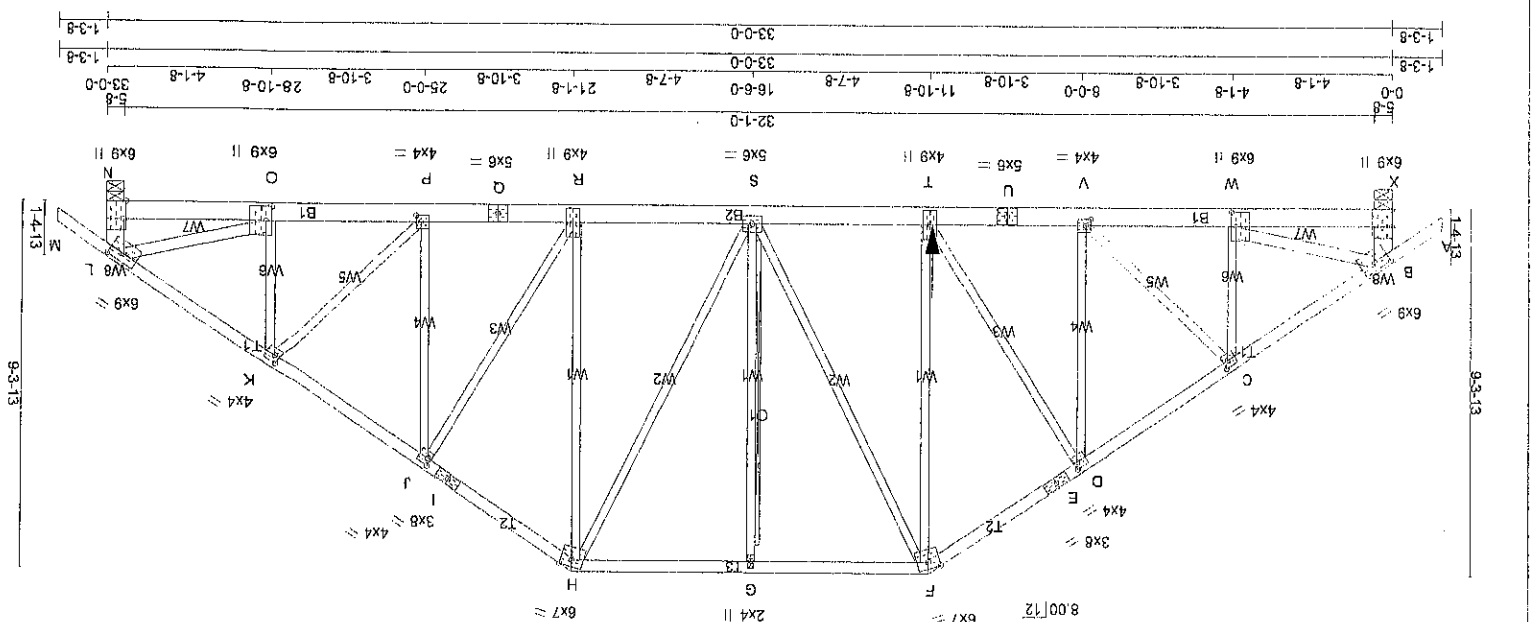
LOADING
8'4" P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
(66% OF 23.0 P.S.F. G.S.L. PLUS
-TPIC 2011
-CSA 088-08

TOTAL LOAD CASES: (4)									
CHORDS		MEMB.		FROM TO		FR-TO		A-B	
MAX. FACTORED	FACTORED	FORCE	VERT. LOAD	MAX	MEMB.	MAX	MEMB.	MAX	MEMB.
(PLF)	(PLF)	(LBS)	(LBS)	CS1 (LC)	UNBRAC	CS1 (LC)	UNBRAC	CS1 (LC)	UNBRAC
				FROM TO		LENGTH FR-TO			
				(LBS)		(LBS)		CS1 (LC)	
B-1	0.66	-17.5	-17.5	0.08 (1)	10.00	H-C	-65.0	0.01 (1)	
H-1	0.66	-17.5	-17.5	0.08 (1)	10.00	G-C	-37.0	0.01 (1)	
H-2	0.62	-17.5	-17.5	0.08 (1)	10.00	G-D	-108.0	0.02 (1)	
G-K	0.50	-17.5	-17.5	0.08 (1)	10.00	I-J	-178.11	0.00 (1)	
K-E	0.50	-17.5	-17.5	0.08 (1)	10.00	K-L	-180.10	0.00 (1)	
PLATE PLACEMENT TOL. = 0.250 inches COMPANION LIVE LOAD FACTOR = 0.50 DOL NUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 CS1: TC=0.09 (D-L:1), BC=0.08 (B-L:1), WB=0.02 (D-G:1), SS1=0.14 (E-K:1) TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PLI) MAX MIN MAX MIN (PSI) MT20 618 364 1687 822 2284 1656									

PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP = 0.23 (B) (INPUT = 0.90)
 JSI METAL = 0.04 (B) (INPUT = 1.00)

JSI GRIP=0.23 (B) (INPUT = 0.90)
JSI METAL=0.04 (B) (INPUT = 1.00)





DESIGN CRITERIA TOTAL WEIGHT = 2 X 189 = 379 lb

DESIGN CRITERIA	DESIGN CRITERIA
LOADING IN FLAT SECTION BASED ON	LOADING IN FLAT SECTION BASED ON
PIGBACK TRUSS WITH SLOPES OF 6.00/12	PIGBACK TRUSS WITH SLOPES OF 6.00/12
AND 6.00/12 AND RESPECTIVE WALL	AND 6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL	HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.	DEAD LOAD OF 3.0 P.S.F.
*** NON STANDARD GIRDER ***	*** NON STANDARD GIRDER ***
ADDITIONAL USER-DEFINED LOADS APPLIED TO	ADDITIONAL USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.	ALL LOAD CASES.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL	THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF	OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010	PART 9, NBC 2010
THIS DESIGN COMPLIES WITH:	THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, ABC 2014	- PART 9 OF CBC 2012, ABC 2014
- CSA 086-09	- CSA 086-09
- TPIC 2011	- TPIC 2011

DESIGN CRITERIA	DESIGN CRITERIA
LOADING IN FLAT SECTION BASED ON	LOADING IN FLAT SECTION BASED ON
PIGBACK TRUSS WITH SLOPES OF 6.00/12	PIGBACK TRUSS WITH SLOPES OF 6.00/12
AND 6.00/12 AND RESPECTIVE WALL	AND 6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL	HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.	DEAD LOAD OF 3.0 P.S.F.
*** NON STANDARD GIRDER ***	*** NON STANDARD GIRDER ***
ADDITIONAL USER-DEFINED LOADS APPLIED TO	ADDITIONAL USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.	ALL LOAD CASES.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL	THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF	OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010	PART 9, NBC 2010
THIS DESIGN COMPLIES WITH:	THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, ABC 2014	- PART 9 OF CBC 2012, ABC 2014
- CSA 086-09	- CSA 086-09
- TPIC 2011	- TPIC 2011

DESIGN CRITERIA	DESIGN CRITERIA
LOADING IN FLAT SECTION BASED ON	LOADING IN FLAT SECTION BASED ON
PIGBACK TRUSS WITH SLOPES OF 6.00/12	PIGBACK TRUSS WITH SLOPES OF 6.00/12
AND 6.00/12 AND RESPECTIVE WALL	AND 6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL	HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.	DEAD LOAD OF 3.0 P.S.F.
*** NON STANDARD GIRDER ***	*** NON STANDARD GIRDER ***
ADDITIONAL USER-DEFINED LOADS APPLIED TO	ADDITIONAL USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.	ALL LOAD CASES.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL	THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF	OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010	PART 9, NBC 2010
THIS DESIGN COMPLIES WITH:	THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, ABC 2014	- PART 9 OF CBC 2012, ABC 2014
- CSA 086-09	- CSA 086-09
- TPIC 2011	- TPIC 2011

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.97 (W) (INPUT = 1.00)

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (tablets in inches)

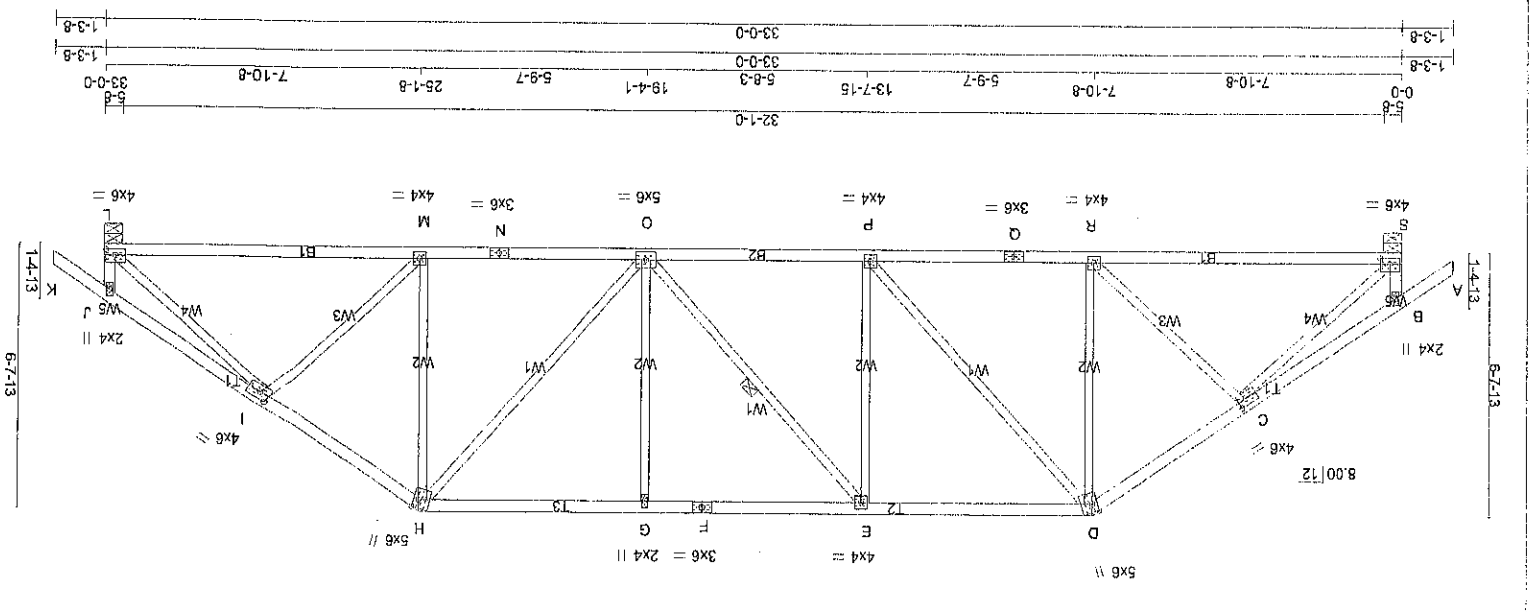
PLATE	TYPE	W	LEN	Y	X
A	TMW-4	MT20	6.0	8.0	2.50
B	TMW-4	MT20	4.0	4.0	2.00
C	TMW-4	MT20	4.0	4.0	1.80
D	TMW-4	MT20	4.0	4.0	1.00
E	TS-4	MT20	3.0	8.0	
F	TTW-4-m	MT20	6.0	7.0	Edge
G	TTW-4-m	MT20	2.0	4.0	
H	TTW-4-m	MT20	6.0	7.0	Edge
I	TS-4	MT20	3.0	8.0	
J	TMW-4	MT20	4.0	4.0	2.00
K	TMW-4	MT20	4.0	4.0	1.50
L	TMW-4	MT20	6.0	9.0	Edge 0.50
N	BMV-4	MT20	6.0	9.0	4.50
O	BMW-4	MT20	6.0	9.0	1.50
P	BMW-4	MT20	4.0	4.0	2.00
Q	BS-4	MT20	6.0	8.0	
R	BMW-4	MT20	4.0	9.0	
S	BMW-4	MT20	6.0	6.0	
T	BMW-4	MT20	4.0	9.0	
U	BS-4	MT20	6.0	5.0	
V	BMW-4	MT20	4.0	4.0	1.50
X	BMV-4	MT20	6.0	8.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGERS OR CONNECTIONS REQUIRED TO SUPPORT CONCENTRATED LOADS 1442.0 lbs FACTORED DOWN AT 11-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTIONS IS DELEGATED TO THE BUILDING DESIGNER.
NOTES - (1)
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per



4-15073815(2)



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	LUMBER	SIZE	CHORDS	N.L.G.A. RULES
A-D	DRY	2x4	DRY	No.2
E-H	DRY	2x4	DRY	No.2
I-K	DRY	2x4	DRY	No.2
L-N	DRY	2x4	DRY	No.2
O-Q	DRY	2x4	DRY	No.2
R-S	DRY	2x4	DRY	No.2
T-U	DRY	2x4	DRY	No.2
V-W	DRY	2x4	DRY	No.2
X-Y	DRY	2x4	DRY	No.2
Z	DRY	2x4	DRY	No.2

DESCR.	LUMBER	SIZE	CHORDS	N.L.G.A. RULES
A-D	DRY	2x4	DRY	No.2
E-H	DRY	2x4	DRY	No.2
I-K	DRY	2x4	DRY	No.2
L-N	DRY	2x4	DRY	No.2
O-Q	DRY	2x4	DRY	No.2
R-S	DRY	2x4	DRY	No.2
T-U	DRY	2x4	DRY	No.2
V-W	DRY	2x4	DRY	No.2
X-Y	DRY	2x4	DRY	No.2
Z	DRY	2x4	DRY	No.2

DESCR.	LUMBER	SIZE	CHORDS	N.L.G.A. RULES
A-D	DRY	2x4	DRY	No.2
E-H	DRY	2x4	DRY	No.2
I-K	DRY	2x4	DRY	No.2
L-N	DRY	2x4	DRY	No.2
O-Q	DRY	2x4	DRY	No.2
R-S	DRY	2x4	DRY	No.2
T-U	DRY	2x4	DRY	No.2
V-W	DRY	2x4	DRY	No.2
X-Y	DRY	2x4	DRY	No.2
Z	DRY	2x4	DRY	No.2

DESCR.	LUMBER	SIZE	CHORDS	N.L.G.A. RULES
A-D	DRY	2x4	DRY	No.2
E-H	DRY	2x4	DRY	No.2
I-K	DRY	2x4	DRY	No.2
L-N	DRY	2x4	DRY	No.2
O-Q	DRY	2x4	DRY	No.2
R-S	DRY	2x4	DRY	No.2
T-U	DRY	2x4	DRY	No.2
V-W	DRY	2x4	DRY	No.2
X-Y	DRY	2x4	DRY	No.2
Z	DRY	2x4	DRY	No.2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
253629	H01	1	1			

Alpha Roof Truss, Maple

Version 7.620 S Apr 15 2016 Mitek Industries, Inc. File Path: C:\Users\mike\Documents\253629.dwg Page 2

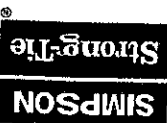
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HANGERS NOTES
 (1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 27'-1-8" AND 301.8 lbs FACTORED DOWN AT 5'-10-8" ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.
 NOTES- (1)
 OBC 9.23.13.11, and no less than 2x4 for Part 4 design as per



A-1073811 (2)

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given.
- The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

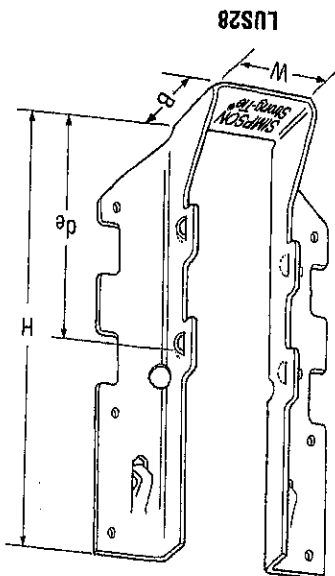
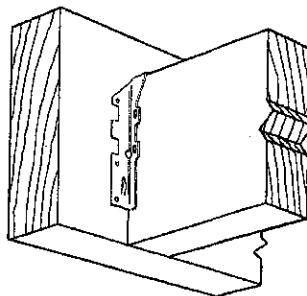
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire.
- 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

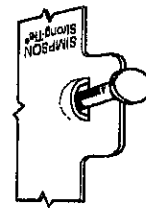
- These hangers cannot be modified.

Typical LUS installation

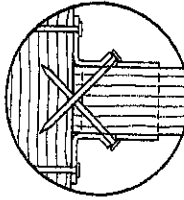


Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)	
		W	H	B	de	Face	Joist	Uplift	D.Fir-L
LUS24	18	1 1/8	3 1/8	3 1/8	1 1/8	4-10d	2-10d	(Kp=1.15)	(Kp=1.00)
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	4-16d	2-16d	(Kp=1.15)	(Kp=1.00)
LUS26	18	1 1/8	4 1/8	1 1/8	3%	4-10d	4-10d	Uplift	Normal
LUS26-2	18	3 1/8	4 1/8	2	4	4-16d	4-16d	Uplift	Normal
LUS26-3	18	4 1/8	4 1/8	2	3%	4-16d	4-16d	Uplift	Normal
LUS28	18	1 1/8	6 1/8	1 1/8	3%	6-10d	4-10d	Uplift	Normal
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	Uplift	Normal
LUS28-3	18	4 1/8	6 1/8	2	3%	6-16d	4-16d	Uplift	Normal
LUS210	18	1 1/8	7 1/8	1 1/8	3%	8-10d	4-10d	Uplift	Normal
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	Uplift	Normal
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	8-16d	6-16d	Uplift	Normal

1. de is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models). U.S. Patent 5,603,589



Double Shear Nailing Top View



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HUS/LJS - Double Shear Joist Hangers

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table
FINISH: G90 galvanized

DESIGN:

- Factorized resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factorized resistances. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

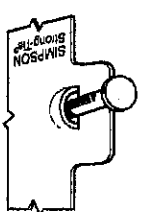
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

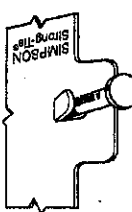
OPTIONS:

- See current catalogue for options

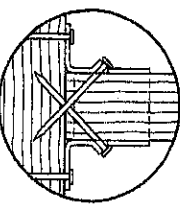
Model No.	Ga	Dimensions (in)					Fasteners	Factorized Resistance (lbs)			
		W	H	B	dg	Face		Joist	D-Fit-L		
									Uplift	Normal	
LJS26DS	18	1 1/4	5	3 1/2	4 5/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/4	5 3/8	3	3 1/8	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/4	7 3/8	3	6 3/8	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/4	9 3/8	3	7 3/8	30-16d	10-16d	4505	5795	4010	4740
HUS1.8/1.10	16	1 1/4	9	3	8	30-16d	10-16d	4505	6450	4010	5200
1. dg is the distance from the seat of the hanger to the highest joist nail.											



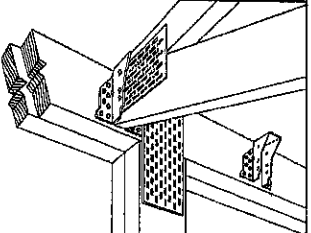
Dome Double Shear Nailing prevents tabs breaking off (available on some models). U.S. Patent 5,803,580



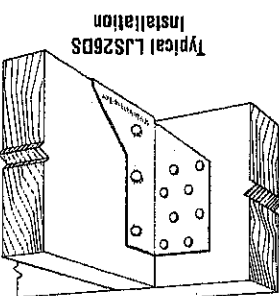
Double Shear Nailing Side View. Do not bend tab back.



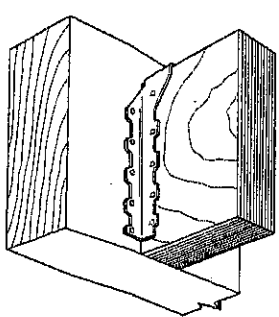
Double Shear Nailing Top View.



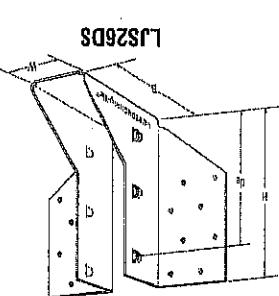
Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)



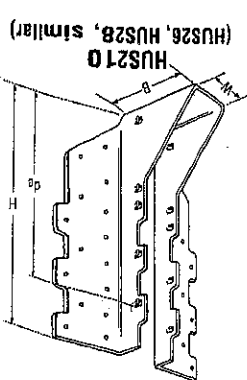
Typical LJS26DS Installation



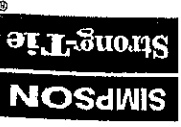
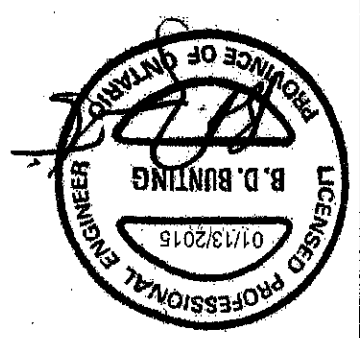
Typical HUS Installation



LJS26DS



HUS210 (HUS26, HUS28, similar)



TC - Truss Connectors



The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plates to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

MATERIAL: 16 gauge

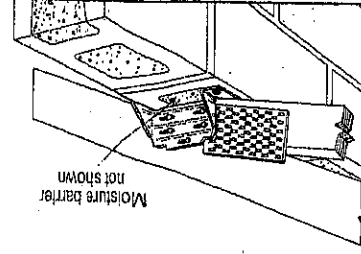
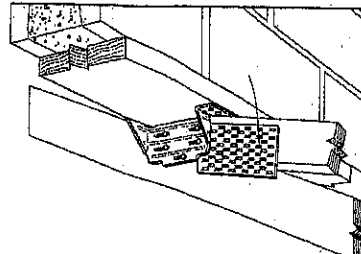
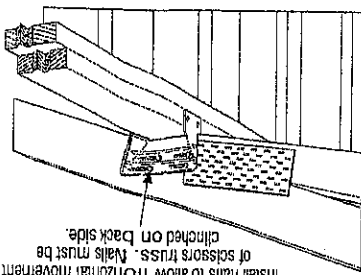
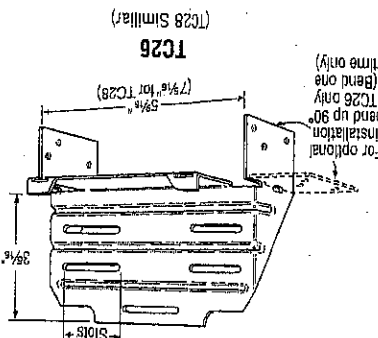
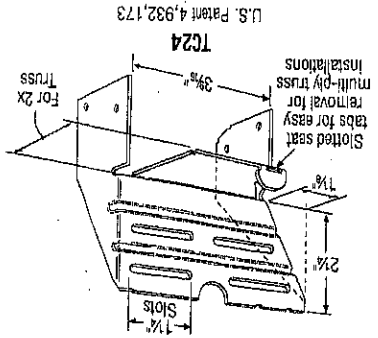
FINISH: G90 galvanized

DESIGN: Factored resistances are in accordance with CSA D86-14

INSTALLATION:

- Use all specified fasteners
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.

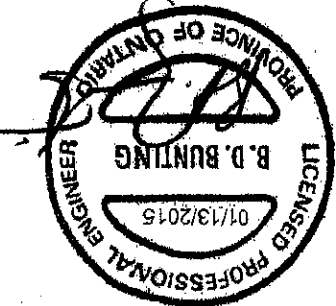


Model No.	Fasteners	Factored Resistance			
		Uplift (K _p = 1.15)	Plates (K _p = 1.15)	lbs	lbs
TC24	Truss	4-10d	605	430	720
TC26	Truss	5-10d	810	720	720
TC28	Truss	5-10d	1015	720	720

Model No.	Fasteners	Factored Resistance			
		Uplift (K _p = 1.15)	Plates (K _p = 1.15)	lbs	lbs
TC26	Wall	D, Fir-L	810	660	660
TC28	Wall	D, Fir-L	930	660	660

OPTIONAL TC INSTALLATION TABLE

1. Factored resistances have been increased 15% for earthquake or wind loading; no other loads govern. allowed; reduce where further increase
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/16" x 2 1/4" Titen screws has a factored uplift resistance of 275 lbs.

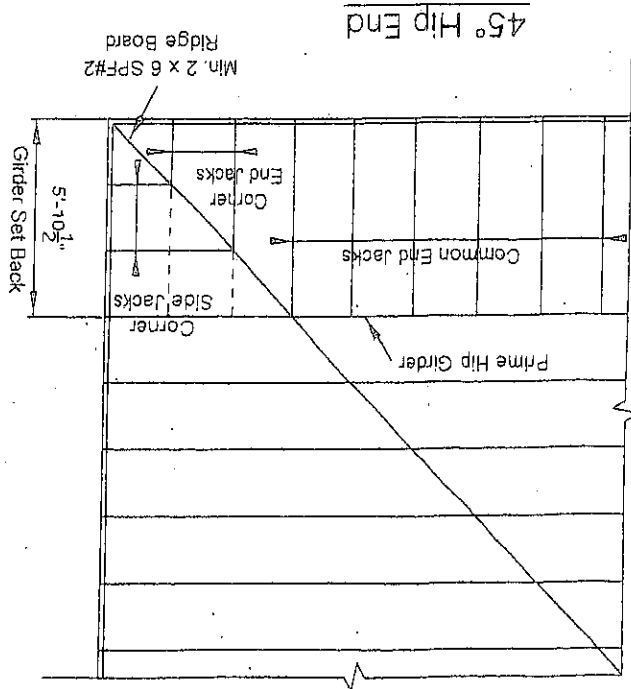


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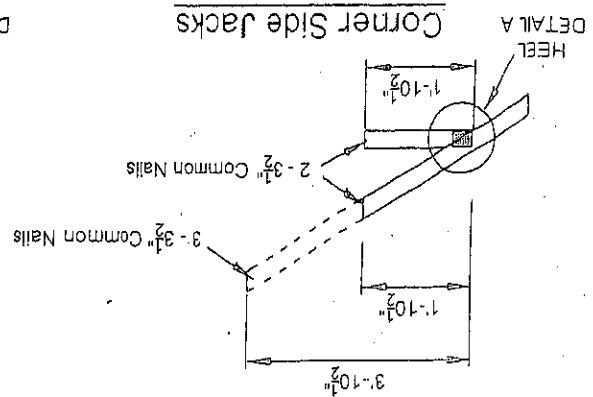
STRACON ENGINEERING INC.

LUMBER SPECIFICATION
 TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

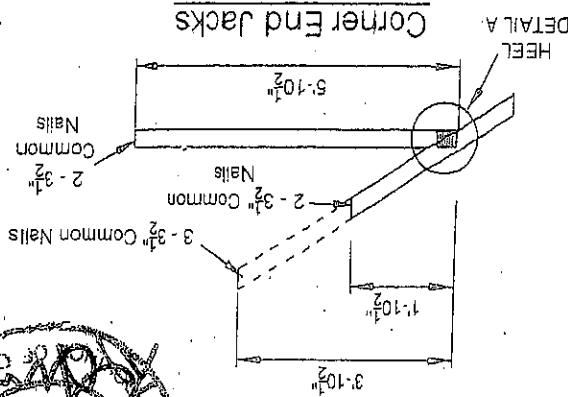
DESIGN LOAD	
TOP CHORD SNOW LOAD	40.5 P.S.F.
TOP CHORD DEAD LOAD	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	7.0 P.S.F.
TOTAL LOAD	50.5 P.S.F.



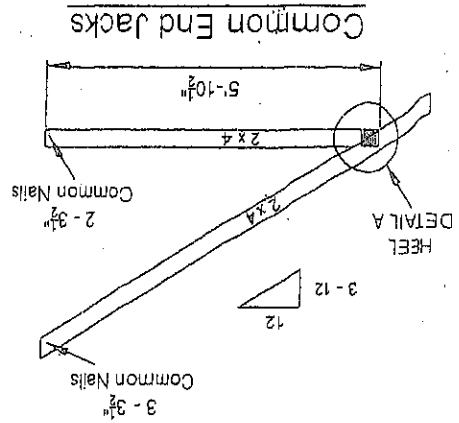
45° Hip End



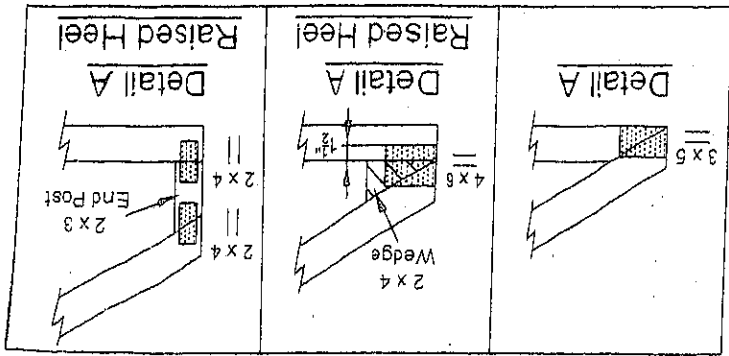
Corner Side Jacks



Corner End Jacks



Common End Jacks



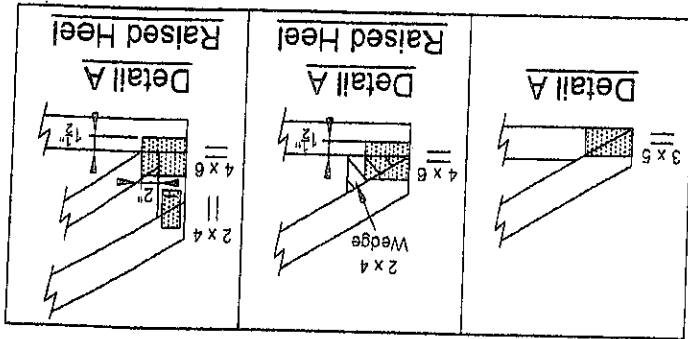
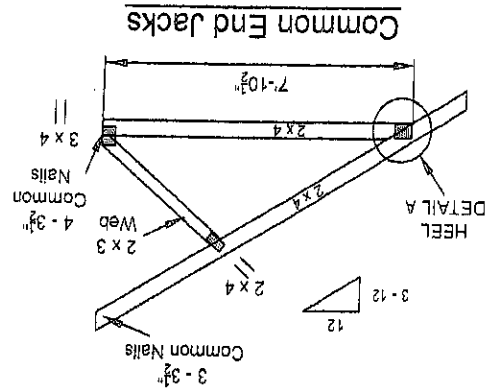
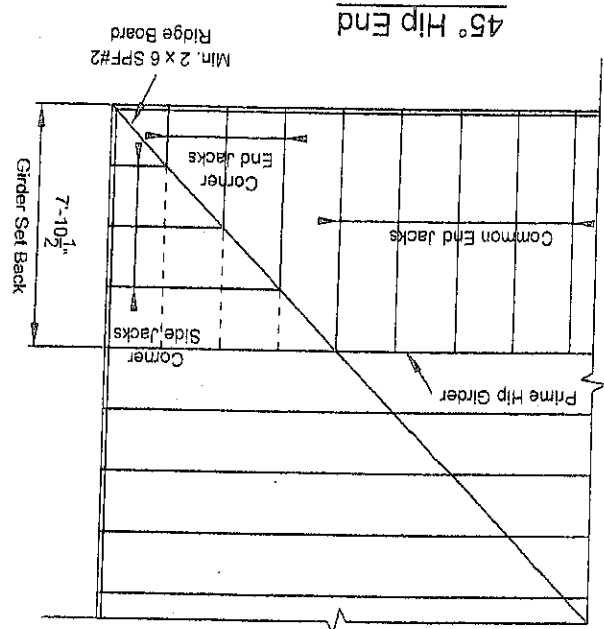
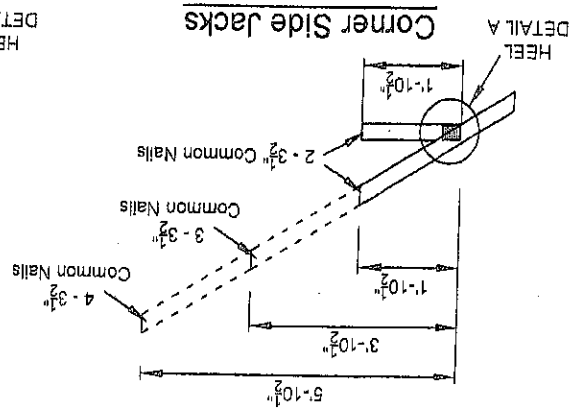
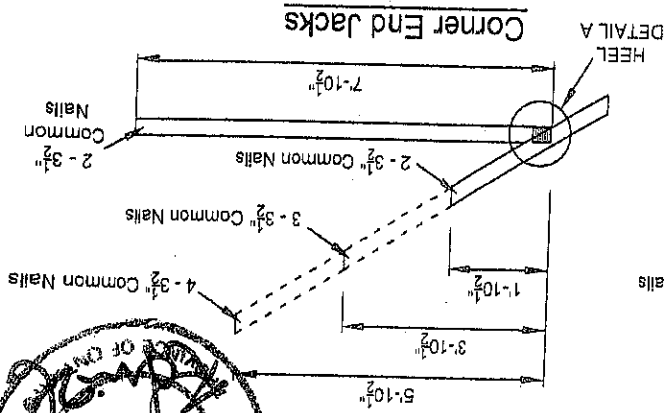
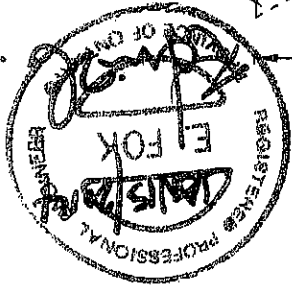
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (U.S.D. DESIGN)

CS-51008

STRACON ENGINEERING INC.

LUMBER SPECIFICATION
 TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD	
TOP CHORD SNOW LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-71008N