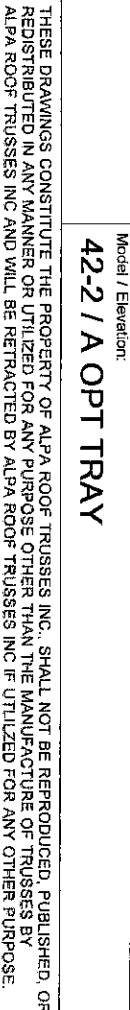
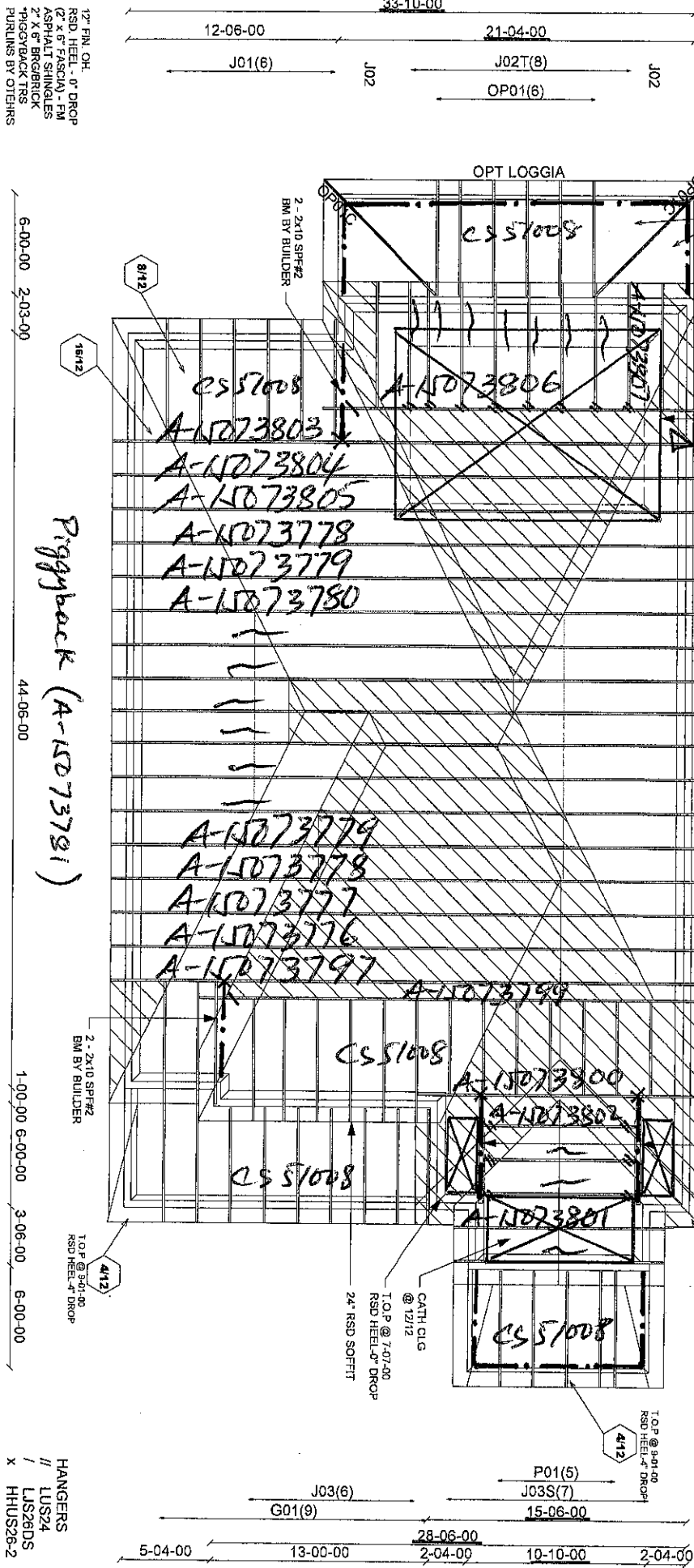
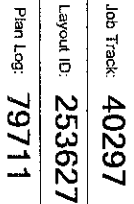


Mitrek Ver. 7.5.0



SECRET





Builder / Location:	GOLD PARK / KLEINBURG		
Project:	HUNTINGTON & NASHVILLE		
Date:	4/30/2015	Designer:	cheryl

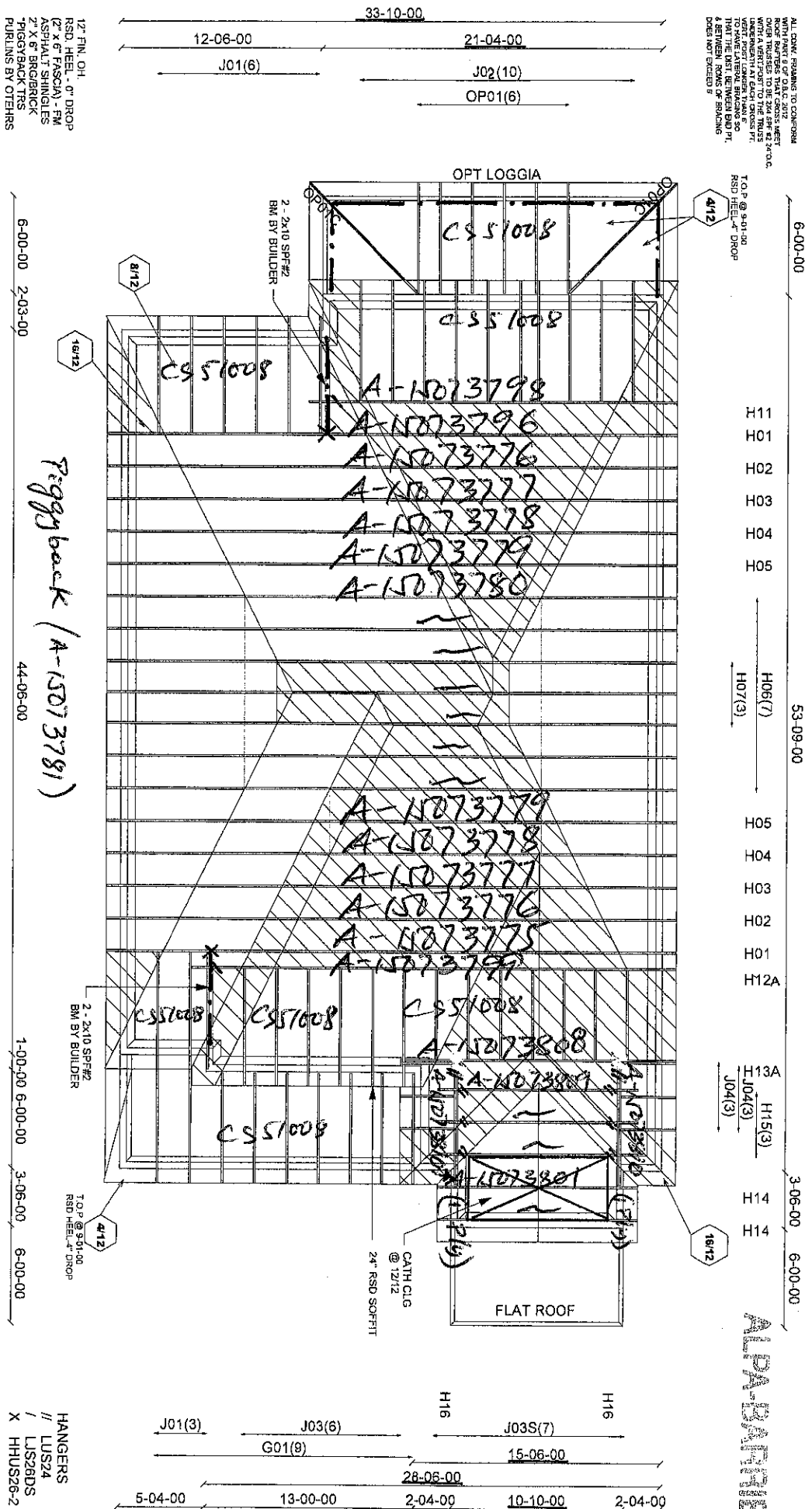
Model / Elevation: 42-2 / B

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPHA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPHA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPHA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



CONVENTIONAL
FRAMING BY
OTHERS

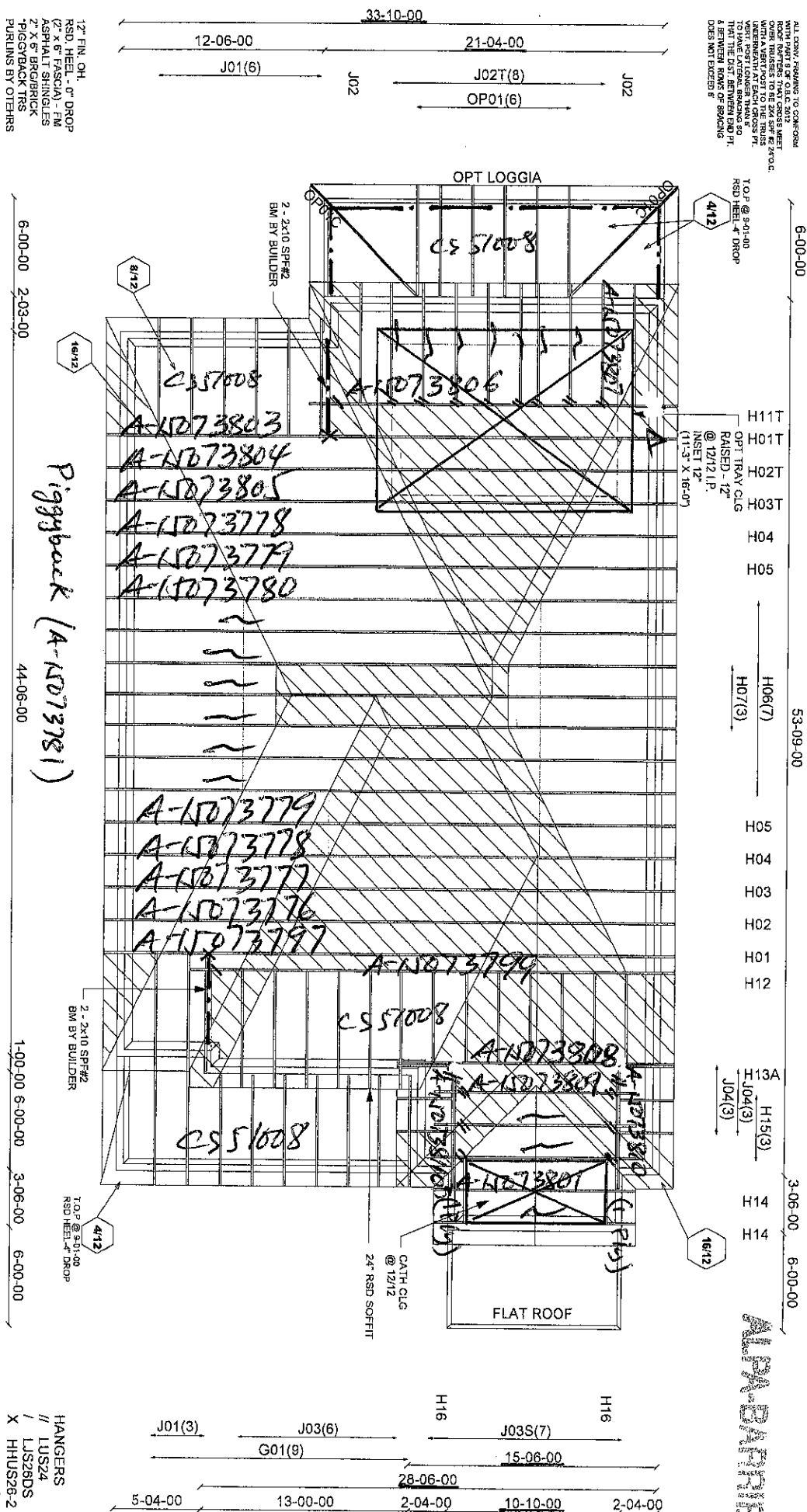
Mitek Ver. 7.5.0



CONVENTIONAL
FRAMING BY
OTHERS

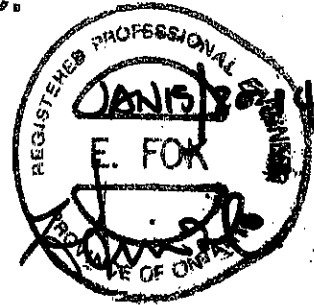
12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*P/GYBACK TRS
PURLINS BY OTHERS

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2
ATC26



Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

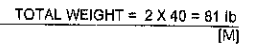
SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MTEK Industries, Inc. Fri Jul 17 07:31:55 2015 Page 1
ID:2niiWe84H8Pd7cRawCqj4UzLbaC-aj3p0OVPmy8J08Vw_eDMSUQF6MIKs0ciF1lqSvyxEv
2x4 || Scale = 1:31.4



DRY: SEASONED LUMBER.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284
	1656				



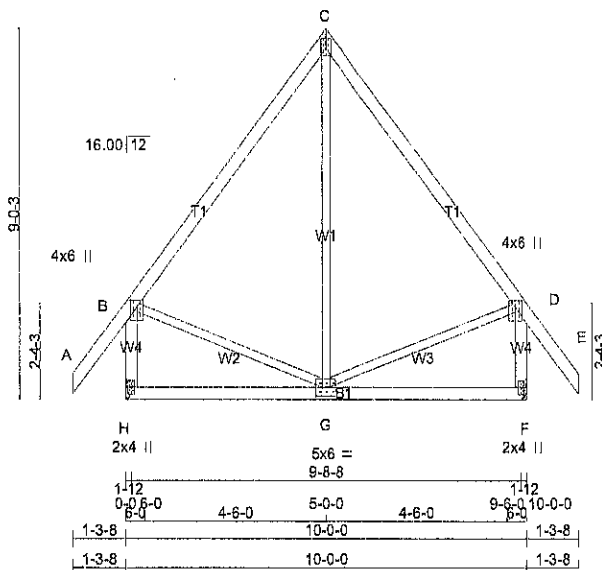
MT20 618 354 1667 822 2284 1656
A-40738-10

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253627	H15	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Fri Jul 17 08:43:13 2015 Page 1

ID:AyrJx9Y?fa2EXF7DRQxSq4zMeZ1-KLY4tBGrlN7JGaX7b6zZrdaKnRPlCu7O_b2BcxYxDsC
3x5 ||
Scale = 1:56.3



TOTAL WEIGHT = 3 X 56 = 167 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00 2.00
C	TTW+p	MT20	3.0	5.0	2.00 Edge
D	TMVW+p	MT20	4.0	6.0	2.00 2.00
F	BMV1+p	MT20	2.0	4.0	
G	BMVWVW+I	MT20	5.0	6.0	2.50 2.00
H	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	539	0	539	0
F	539	0	539	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
H	377 269 / 0 0 / 0 0 / 0 108 / 0 0 / 0
F	377 269 / 0 0 / 0 0 / 0 108 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC)	MEMB.	FORCE (LBS) MAX CSI (LC)
FR-TO		FROM TO	FR-TO	
A-B	0 / 40	-70.5 -70.5 0.10 (1)	G-C	0 / 85 0.03 (4)
B-C	-222 / 0	-70.5 -70.5 0.23 (1)	B-G	0 / 143 0.03 (1)
C-D	-222 / 0	-70.5 -70.5 0.23 (1)	G-D	0 / 143 0.03 (1)
D-E	0 / 40	-70.5 -70.5 0.10 (1)		
H-B	-506 / 0	0.0 0.0 0.06 (1)		
F-D	-506 / 0	0.0 0.0 0.06 (1)		
H-G	0 / 0	-17.5 -17.5 0.12 (4)		
G-F	0 / 0	-17.5 -17.5 0.12 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23 (C-D:1), BC=0.12 (F-G:4), WB=0.03 (B-G:1), SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION	(PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN			
MT20	618	354	1667 622 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (D) (INPUT = 0.80)
JSI METAL= 0.09 (H) (INPUT = 1.00)

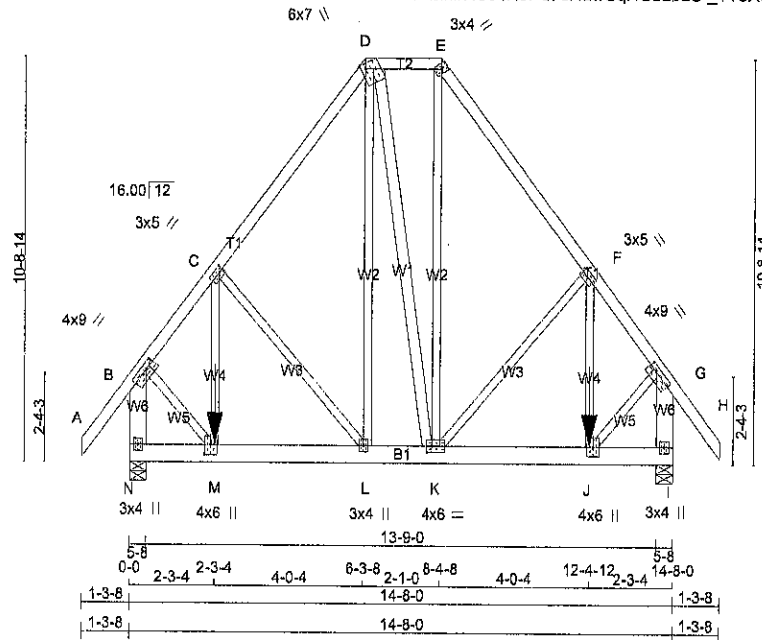


A-15073809

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253627	H13A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Fri Jul 17 07:29:25 2015 Page 1
ID:2nllW684H8Pd7cRawCq4UzLbaC_T76X5hJ5iFb1515C8XBnAdr6Tj9AdmhPu8ByxExe
Scale = 1:61.4



TOTAL WEIGHT = 115 lb
(M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
N - B	2x6	DRY	No.2	SPF	
I - G	2x6	DRY	No.2	SPF	
N - I	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - K	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	1.50	3.00
C	TMVW-t	MT20	3.0	5.0	1.50	1.50
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TTWW-h	MT20	3.0	4.0	1.75	1.25
F	TMVW-t	MT20	3.0	5.0	1.50	1.50
G	TMVW-t	MT20	4.0	9.0	1.50	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	8.0		
K	BMVW-t	MT20	4.0	8.0	2.00	2.00
L	BMVW-t	MT20	3.0	4.0		
M	BMVW-t	MT20	4.0	8.0	3.25	2.00
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 848.7 lbs FACTORED DOWN AT 12-4-12, AND 848.7 lbs FACTORED DOWN AT 2-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	2151	0	2151	0
I	2151	0	2151	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	
	COMBINED	SNOW	LIVE	PERM.LIVE
N	1513	1038 / 0	0 / 0	0 / 0
I	1513	1038 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 40	-70.5	-70.5 0.11 (1)	10.00	M-C	0 / 428	0.11 (1)
B-C	-1452 / 0	-70.5	-70.5 0.18 (1)	5.21	C-L	-445 / 0	0.33 (1)
C-D	-1033 / 0	-70.5	-70.5 0.19 (1)	5.96	L-D	0 / 560	0.14 (1)
D-E	-624 / 0	-70.5	-70.5 0.06 (1)	6.25	D-K	0 / 73	0.01 (1)
E-F	-1054 / 0	-70.5	-70.5 0.19 (1)	5.90	K-E	0 / 639	0.16 (1)
F-G	-1438 / 0	-70.5	-70.5 0.18 (1)	5.23	K-F	-413 / 0	0.30 (1)
G-H	0 / 40	-70.5	-70.5 0.11 (1)	10.00	J-F	0 / 375	0.09 (1)
N-B	-2082 / 0	0.0	0.0 0.18 (1)	7.02	B-M	0 / 1166	0.29 (1)
I-G	-2044 / 0	0.0	0.0 0.18 (1)	7.04	J-G	0 / 1146	0.28 (1)
N-M	0 / 0	-93.5	-93.5 0.03 (4)	10.00			
M-L	0 / 891	-93.5	-93.5 0.23 (1)	10.00			
L-K	0 / 610	-93.5	-93.5 0.21 (1)	10.00			
K-J	0 / 883	-93.5	-93.5 0.25 (1)	10.00			
J-I	0 / 0	-93.5	-93.5 0.04 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	12-4-12	-849	-849	---	FRONT	VERT	TOTAL
M	2-3-4	-849	-849	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSdGirder

START DISTANCE = 0-0
START SPAN CARRIED = 5-5-8
END DISTANCE = 14-8-0
END SPAN CARRIED = 5-5-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.19 (E-F:1), BC=0.25 (J-K:1), WB=0.33 (C-L:1), SSI=0.16 (J-K:1)

DOE LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

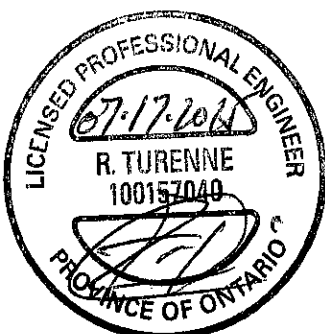
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)

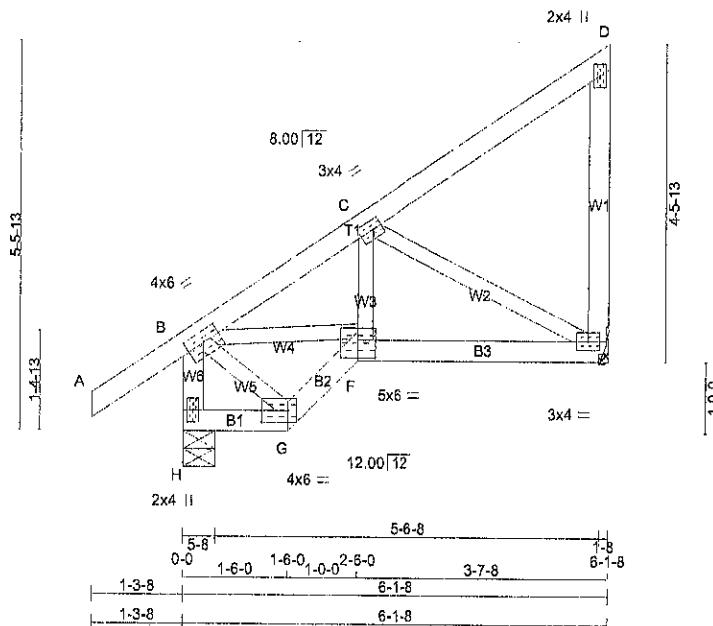
MAX MIN MAX MIN MAX MIN
M10 214 2094 1836 228 1556



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253626	J02T	8	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Thu Jul 16 16:55:07 2015 Page 1
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-wPHpluqF185mbQk5B1PbHwXpIEj6aG3G5208yxRj
Scale = 1:32.7



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4 DRY	No.2	SPF		
A - D	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVWW-I	MT20	4.0	6.0	1.75	3.00
C	TMVWW-I	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-I	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	366	0	366	0
E	269	0	269	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	266	186 / 0	0 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
E	190	129 / 0	0 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO							FR-TO			
H-B	-353 / 0	0.0	0.0	0.04 (1)	7.81		B-G	0 / 18	0.01 (4)	
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00		B-F	0 / 259	0.08 (1)	
B-C	-310 / 0	-70.5	-70.5	0.09 (1)	6.25		F-C	0 / 69	0.02 (4)	
C-D	-13 / 0	-70.5	-70.5	0.09 (1)	8.25		C-E	-303 / 0	0.08 (1)	
E-D	-101 / 0	0.0	0.0	0.03 (1)	7.81					
H-G	0 / 0	-17.5	-17.5	0.01 (4)	10.00					
G-F	0 / 20	-17.5	-17.5	0.01 (4)	10.00					
F-E	0 / 269	-17.5	-17.5	0.09 (4)	10.00					

TOTAL WEIGHT = 8 X 31 = 246 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.09 (A-B:1), BC=0.09 (E-F:4), WB=0.08 (C-E:1), SSI=0.10 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 522 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

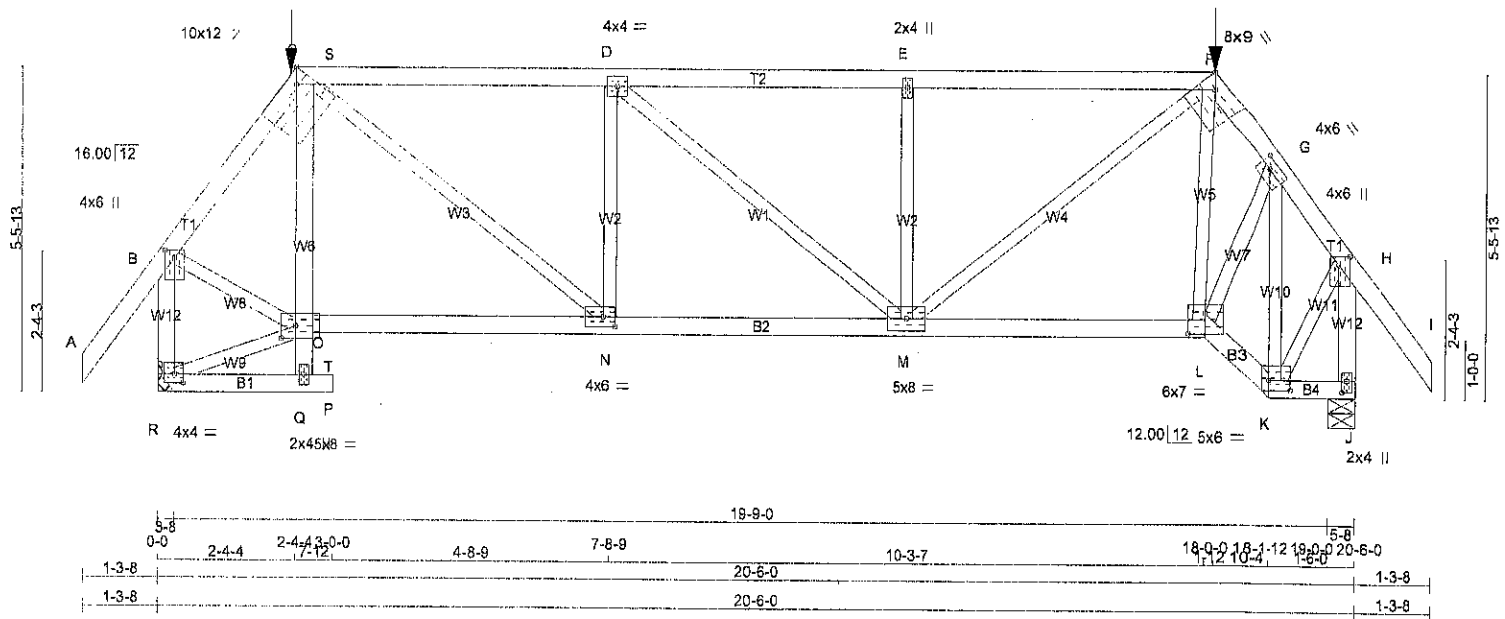


A-10073807

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253626	H11T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y7fa2EXF7DRQXsq4zMEz1-SDjRXYqdGrzvzG9ubJtMJ_hGLnp_ac7qcLUtiyxRl
Scale = 1:39.1



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
Q - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	1.50 2.00
C	TTWW-h	MT20	10.0	12.0	Edge 2.75
D	TMVW-h	MT20	4.0	4.0	
E	TMVW-w	MT20	2.0	4.0	
F	TTWW-h	MT20	8.0	9.0	Edge 2.75
G	TMVW-l	MT20	4.0	6.0	1.75 2.25
H	TMVW-p	MT20	4.0	6.0	1.75 2.25
J	BMV1+p	MT20	2.0	4.0	2.25 1.00
K	BBWW-l	MT20	5.0	6.0	2.00 4.50
L	BBWW-l	MT20	6.0	7.0	4.25 3.50
M	BMVWW-l	MT20	5.0	8.0	
N	BMVWW-l	MT20	4.0	6.0	2.00 2.25
O	BMVWW-l	MT20	5.0	8.0	2.50 3.00
Q	BMVW-w	MT20	2.0	4.0	
R	BMVWW-l	MT20	4.0	4.0	1.75 2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	REORD
	GROSS	REACTION	GROSS	REACTION		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
J	1883	0	1883	0	0	2-10
R	1888	0	1888	0	0	HANGER BY OTHERS
						MIN. SEAT SIZE: 2-1

UNFACTORED REACTIONS

	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
JT	1326	904 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0
R	1338	904 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81FT.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 40	-70.5	-70.5 0.11 (1)	10.00	K-G	-1347 / 0	0.37 (1)
B-C	-1577 / 0	-70.5	-70.5 0.11 (1)	5.13	K-H	0 / 1017	0.25 (1)
C-S	-2232 / 0	-143.1	-143.1 0.40 (1)	4.15	Q-O	0 / 70	0.04 (1)
S-D	-2232 / 0	-70.5	-70.5 0.40 (1)	4.15	O-Q	-163 / 33	0.03 (1)
D-E	-2214 / 0	-70.5	-70.5 0.38 (1)	4.17	N-D	-374 / 0	0.12 (1)
E-F	-2214 / 0	-70.5	-70.5 0.38 (1)	4.19	M-E	-378 / 0	0.12 (1)
F-G	-1583 / 0	-70.5	-70.5 0.08 (1)	5.16	C-N	0 / 1638	0.41 (1)
G-H	-1099 / 0	-70.5	-70.5 0.12 (1)	5.87	D-M	-23 / 0	0.02 (1)
H-I	0 / 40	-70.5	-70.5 0.11 (1)	10.00	M-F	0 / 1616	0.40 (1)
R-B	-1847 / 0	0.0	0.0 0.25 (1)	6.12	L-F	-59 / 42	0.02 (1)
J-H	-1867 / 0	0.0	0.0 0.26 (1)	6.10	L-G	0 / 965	0.24 (1)
					R-O	-21 / 0	0.00 (1)
					B-O	0 / 1017	0.25 (1)
R-Q	0 / 20	-35.5	-35.5 0.07 (4)	10.00			
Q-P	0 / 0	-35.5	-35.5 0.01 (4)	10.00			
O-T	0 / 937	-35.5	-35.5 0.57 (1)	10.00			
T-N	0 / 937	-108.2	-108.2 0.57 (1)	10.00			
N-M	0 / 2232	-108.2	-108.2 0.80 (1)	10.00			
M-L	0 / 940	-108.2	-108.2 0.57 (1)	10.00			
L-K	0 / 831	-35.5	-35.5 0.15 (1)	10.00			
K-J	0 / 0	-35.5	-35.5 0.03 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-4-4	-128	-128	---	FRONT	VERT	TOTAL
F	18-1-12	-128	-128	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 103 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSIdGirder
START DISTANCE = 3-0-0
START SPAN CARRIED = 8-1-8
END DISTANCE = 18-0-0
END SPAN CARRIED = 6-1-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 2-4-4
END SETBACK = 6-1-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-0-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 2-4-4
END SETBACK = 6-1-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-08
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.40 (C-D:1), BC=0.80 (M-N:1),
WB=0.41 (C-N:1), SSI=0.29 (N-O:1)

A-12013806



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253626	H11T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 M/Tek Industries, Inc. Thu Jul 16 16:55:06 2015 Page 2
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-SDjRXyqGGrzvzG9ubJtMlJ_hGLnp_aC7qcLUTiyxRI.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 127.8 lbs FACTORED DOWN AT 2-4-4, AND 127.8 lbs FACTORED DOWN AT 16-1-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)



A-15073806 (2)



DRY: SEASONED LUMBER.

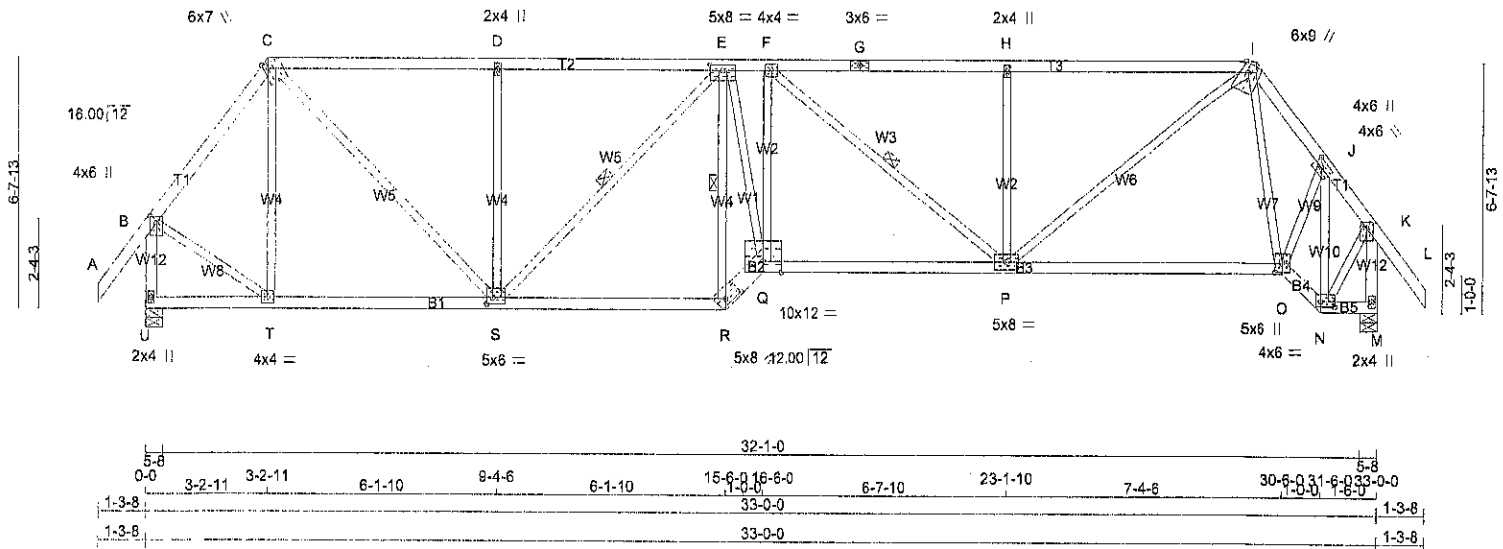
JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

4-15073805

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253626	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 16:55:05 2015 Page 1
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1_093KCp7VXr2L6a11cM7C5RTvyXJF5GzcyxGyxRI
Scale = 1:81.4



TOTAL WEIGHT = 162 lb
[M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
U - R	2x4	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	
O - N	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWVW+t	MT20	5.0	8.0	1.75	4.00
F	TMWVW-l	MT20	4.0	4.0		
G	TS-l	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWVW-l	MT20	4.0	6.0	2.00	2.50
K	TMWVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWVW-l	MT20	4.0	6.0	1.75	4.50
O	BBWVW+p	MT20	5.0	6.0	2.75	2.25
P	BMWVW+t	MT20	5.0	8.0		
Q	BBWVW-l	MT20	10.0	12.0	Edge	
R	BBW-h	MT20	5.0	8.0	1.75	5.00
S	BMWVW+t	MT20	5.0	6.0	2.00	2.00
T	BMWVW-l	MT20	4.0	4.0		
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
U	1548	0	1548	0	0	5-8	1-11		
M	1553	0	1553	0	0	5-8	1-11		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
U	1089	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0	
M	1091	753 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-S, E-R, F-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD	PLF	LC1 MAX	LC2 MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO						FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	T-C	-308 / 0	0.23 (1)	
B-C	-1144 / 0	-70.5	-70.5	0.15 (1)	5.78	C-S	0 / 1296	0.29 (1)	
C-D	-1579 / 0	-70.5	-70.5	0.47 (1)	4.71	S-D	-481 / 0	0.35 (1)	
D-E	-1579 / 0	-70.5	-70.5	0.47 (1)	4.71	S-E	-494 / 0	0.26 (1)	
E-F	-2288 / 0	-70.5	-70.5	0.28 (1)	4.25	R-E	-1808 / 0	0.51 (1)	
F-G	-1944 / 0	-70.5	-70.5	0.63 (1)	4.14	E-Q	0 / 1943	0.44 (1)	
G-H	-1944 / 0	-70.5	-70.5	0.63 (1)	4.14	Q-F	0 / 70	0.02 (4)	
H-I	-1944 / 0	-70.5	-70.5	0.63 (1)	4.14	F-P	-442 / 0	0.21 (1)	
I-J	-1276 / 0	-70.5	-70.5	0.07 (1)	5.65	P-H	-534 / 0	0.26 (1)	
J-K	-894 / 0	-70.5	-70.5	0.10 (1)	6.25	P-I	0 / 1489	0.34 (1)	
K-L	0 / 40	-70.5	-70.5	0.10 (1)	10.00	I-O	-168 / 0	0.08 (1)	
U-B	-1532 / 0	0.0	0.0	0.20 (1)	6.65	O-J	0 / 762	0.17 (1)	
M-K	-1539 / 0	0.0	0.0	0.20 (1)	6.63	N-J	-1112 / 0	0.29 (1)	
						B-T	0 / 790	0.18 (1)	
						N-K	0 / 829	0.19 (1)	
U-T	0 / 0	-17.5	-17.5	0.09 (4)	10.00				
T-S	0 / 676	-17.5	-17.5	0.21 (4)	10.00				
S-R	0 / 1919	-17.5	-17.5	0.39 (1)	10.00				
R-Q	0 / 2672	-17.5	-17.5	0.32 (1)	10.00				
Q-P	0 / 2286	-17.5	-17.5	0.50 (1)	10.00				
P-O	0 / 781	-17.5	-17.5	0.31 (4)	10.00				
O-N	0 / 677	-17.5	-17.5	0.11 (1)	10.00				
N-M	0 / 0	-17.5	-17.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/998 (0.27")

CS1: TC=0.63 (H-I:1), BC=0.50 (P-Q:1), WB=0.51 (E-R:1), SSI=0.22 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (I) (INPUT = 0.90)
JSI METAL=0.42 (E) (INPUT = 1.00)



A-15073804



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BMWW-t	MT20	5.0	6.0		
T	BBWW+p	MT20	10.0	12.0	Edge	
U	BBW-h	MT20	8.0	9.0	3.00	4.25
V	BMWW-t	MT20	4.0	9.0		
W	BMWWW-h	MT20	6.0	12.0	2.50	4.50
X	BMWW-t	MT20	5.0	6.0	2.00	2.75
Y	BMV1-t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD, AND 2180.1 lbs FACTORED DOWN AT 12-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

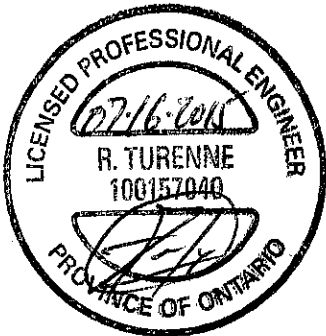
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354 1667	822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.85 (T) (INPUT = 1.00)

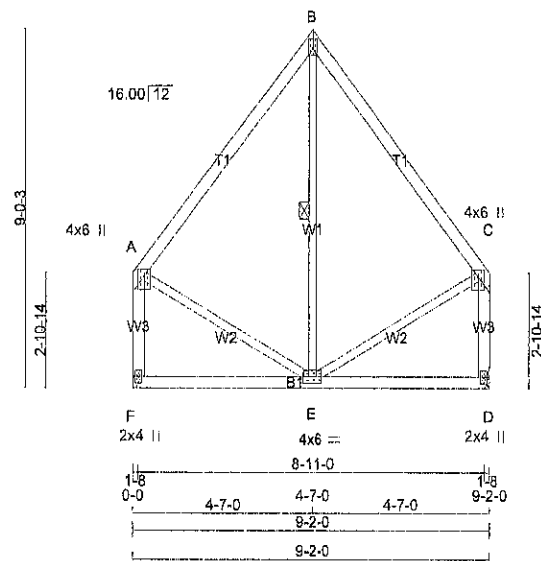


A-11073803(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253625	H15	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID: AyrJx9Y7fa2EXF7DRQxSq4zMEz1-cs8L8IV4ISHXQXZgHqX0sDUBD08InVmsdZh4KayxRw:
3x5 II Scale = 1:58.3



TOTAL WEIGHT = 3 X 49 = 147 lb
(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
F - A	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - D	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	BMV1+p	MT20	2.0	4.0		
E	BMVWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
			DOWN	HORZ	DOWN	HORZ		
F	403	0	403	0	403	0	0	0
D	403	0	403	0	403	0	0	0

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS							
JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	284	192 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
D	284	192 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF B-E.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED				WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO					
A-B	-187 / 0	-70.5	-70.5	0.19 (1)	8.25	E-B	-24 / 68	0.02 (4)
B-C	-187 / 0	-70.5	-70.5	0.19 (1)	6.25	A-E	0 / 128	0.03 (1)
F-A	-372 / 0	0.0	0.0	0.06 (1)	7.81	E-C	0 / 128	0.03 (1)
D-C	-372 / 0	0.0	0.0	0.06 (1)	7.81			
F-E	0 / 0	-17.5	-17.5	0.10 (4)	10.00			
E-D	0 / 0	-17.5	-17.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 21.0 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.01")

CSI: TC=0.19 (A-B:1), BC=0.10 (D-E:4), WB=0.03 (A-E:1), SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

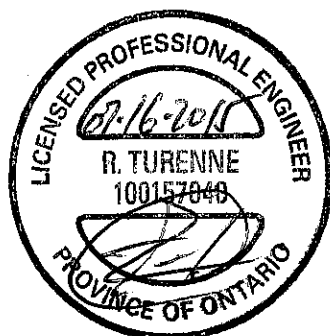
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (A) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)

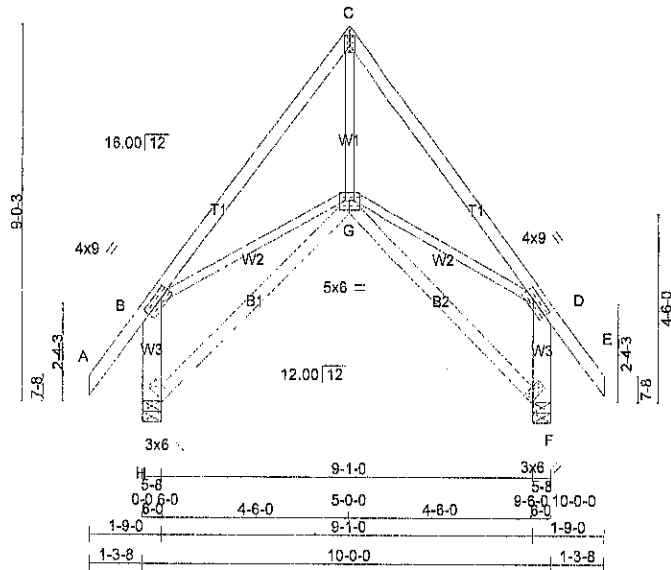


A-15073802

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253625	H14	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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 ID: AyrJx9Y?fa2EXF7DRQxSq4zMEz1-cs8L8IV4ISHXQXZgHqX0sDUAZ07NnVwsdZh4KayxRw:
 3x5 II Scale = 1:55.6



TOTAL WEIGHT = 2 X 60 = 120 lb
 (M/F)

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2		SPF
C - E	2x4	DRY	No.2		SPF
H - B	2x6	DRY	No.2		SPF
F - D	2x6	DRY	No.2		SPF
H - G	2x4	DRY	No.2		SPF
G - F	2x4	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	4.0	9.0	2.00 3.00
C	TTW+p	MT20	3.0	5.0	2.00 Edge
D	TMVW-I	MT20	4.0	9.0	2.00 3.00
F	BVM1-I	MT20	3.0	6.0	Edge 0.25
G	BBWWW-p	MT20	5.0	6.0	2.75 3.00
H	BVM1-I	MT20	3.0	6.0	Edge 0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
H	539	0	539	0	0	5-8	1-8		
F	539	0	539	0	0	5-8	1-8		

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
H	377	269 / 0	0 / 0	0 / 0	0 / 0
F	377	269 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F
 BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)
 BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE	VERT.	LOAD LC1	MAX	MAX	MEMB.	FORCE	MAX	MAX
	(LBS)	FROM	TO	CS1 (LC)	UNBRAC		(LBS)	CS1 (LC)	CS1 (LC)
FR-TO					LENGTH	FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G-C	0 / 369	0.08 (1)	
B-C	-451 / 0	-70.5	-70.5	0.23 (1)	6.25	B-G	0 / 305	0.07 (1)	
C-D	-451 / 0	-70.5	-70.5	0.23 (1)	6.25	G-D	0 / 305	0.07 (1)	
D-E	0 / 40	-70.5	-70.5	0.10 (1)	10.00				
H-B	-495 / 0	0.0	0.0	0.05 (1)	7.81				
F-D	-495 / 0	0.0	0.0	0.05 (1)	7.81				
H-G	0 / 0	-17.5	-17.5	0.13 (4)	10.00				
G-F	0 / 0	-17.5	-17.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.33")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.23 (C-D:1), BC=0.13 (F-G:4),
 WB=0.08 (C-G:1), SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)		
MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822 2284 1656

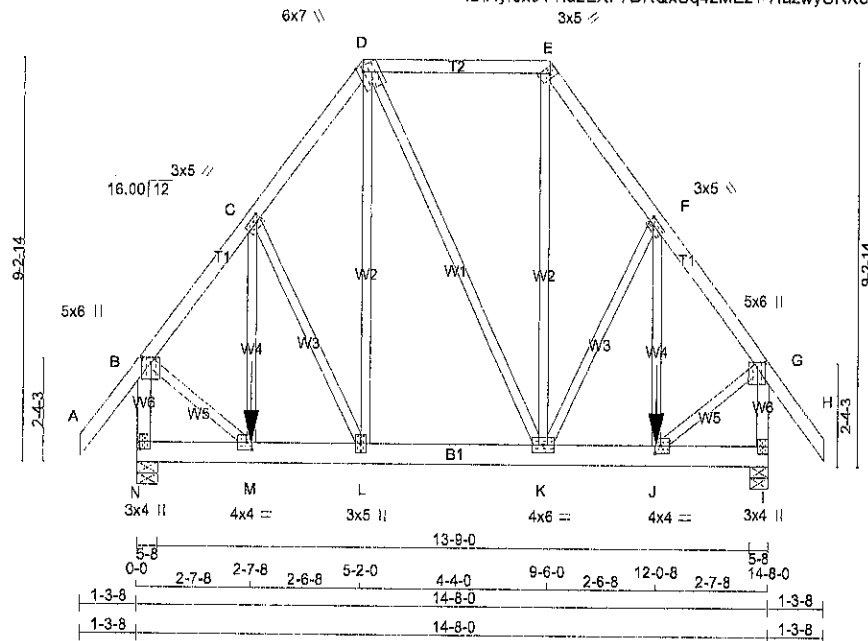
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (C) (INPUT = 0.80)
 JSI METAL= 0.11 (B) (INPUT = 1.00)



A-11073801



LUMBER					
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
N - I	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.50	2.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.50
D	TTW+h	MT20	6.0	7.0	Edge	1.50
E	TTW+h	MT20	3.0	5.0	2.00	1.25
F	TMVW-t	MT20	3.0	5.0	1.50	1.50
G	TMVW+p	MT20	5.0	6.0	1.50	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	4.0	2.00	1.50
K	BMVW-t	MT20	4.0	4.0	2.00	1.50
L	BMVW-t	MT20	3.0	5.0		
M	BMVW-t	MT20	4.0	4.0	2.00	1.50
N	BMV1+p	MT20	3.0	4.0	2.00	0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 772.1 lbs FACTORED DOWN AT 2-7-8, AND 772.1 lbs FACTORED DOWN AT 12-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
N	2322	0	2322	0
I	2322	0	2322	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1633	1120 / 0	0 / 0	0 / 0	0 / 0	514 / 0	0 / 0
I	1633	1120 / 0	0 / 0	0 / 0	0 / 0	514 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-70.5	-70.5 0.11 (1)	10.00	M-C	0 / 310	0.08 (1)
B-C	-1615 / 0	-146.0	-146.0 0.18 (1)	5.13	C-L	-444 / 0	0.28 (1)
C-D	-1244 / 0	-70.5	-70.5 0.14 (1)	5.69	L-D	0 / 750	0.19 (1)
D-E	-743 / 0	-70.5	-70.5 0.26 (1)	6.25	D-K	0 / 20	0.00 (1)
E-F	-1258 / 0	-70.5	-70.5 0.14 (1)	5.67	K-E	0 / 779	0.19 (1)
F-G	-1511 / 0	-146.0	-146.0 0.18 (1)	5.13	K-F	-420 / 0	0.27 (1)
G-H	0 / 40	-70.5	-70.5 0.11 (1)	10.00	J-F	0 / 273	0.07 (1)
N-B	-2152 / 0	0.0	0.0 0.28 (1)	5.73	B-M	0 / 1134	0.28 (1)
I-G	-2147 / 0	0.0	0.0 0.28 (1)	5.74	J-G	0 / 1130	0.28 (1)
N-M	0 / 0	-112.3	-112.3 0.09 (1)	10.00			
M-L	0 / 922	-93.5	-93.5 0.18 (1)	10.00			
L-K	0 / 735	-93.5	-93.5 0.21 (1)	10.00			
K-J	0 / 919	-93.5	-93.5 0.23 (1)	10.00			
J-I	0 / 0	-112.3	-112.3 0.10 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	12-0-8	-772	-772	---	FRONT	VERT	TOTAL
M	2-7-8	-772	-772	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 104 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 31.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-3-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-7-8 OF SPAN
MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-3-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-7-8 OF SPAN
MEASURED FROM THE RIGHT.

GIRDER TYPE: CSIdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-5-8
END DISTANCE = 14-8-0
END SPAN CARRIED = 5-5-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L., PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.49")
CALCULATED DEF. (RT) DEF. (0.99") (0.02")
A-107 3800



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253625	H13	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.020 S Apr 15 2015 MiTek Industries, Inc. Thu Jul 16 16:43:37 2015 Page 2
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-7fazwyURX88gpN UJ60nJoy_0cnV2?ZiPvxXo8yxRw

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.28 (B-N:1) , BC=0.23 (J-K:1) , WB=0.28 (C-L:1) , SSI=0.14 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)

JSI METAL= 0.35 (M) (INPUT = 1.00)



A-15073800(2)



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

1



BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

TOTAL LOAD CASES: (4)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-115	-115	---	FRONT	VERT	TOTAL
H	23-11-5	-192	-192	---	FRONT	VERT	TOTAL

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.66 (N) (INPUT = 1.00)

A-15073799

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253625	H12	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-IT1bjcTpmr1pBEPHAPVYnoPgNCN1JSQZAFczGhyxRwf

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 181.6 lbs FACTORED DOWN AT 23-11-6, AND 114.6 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

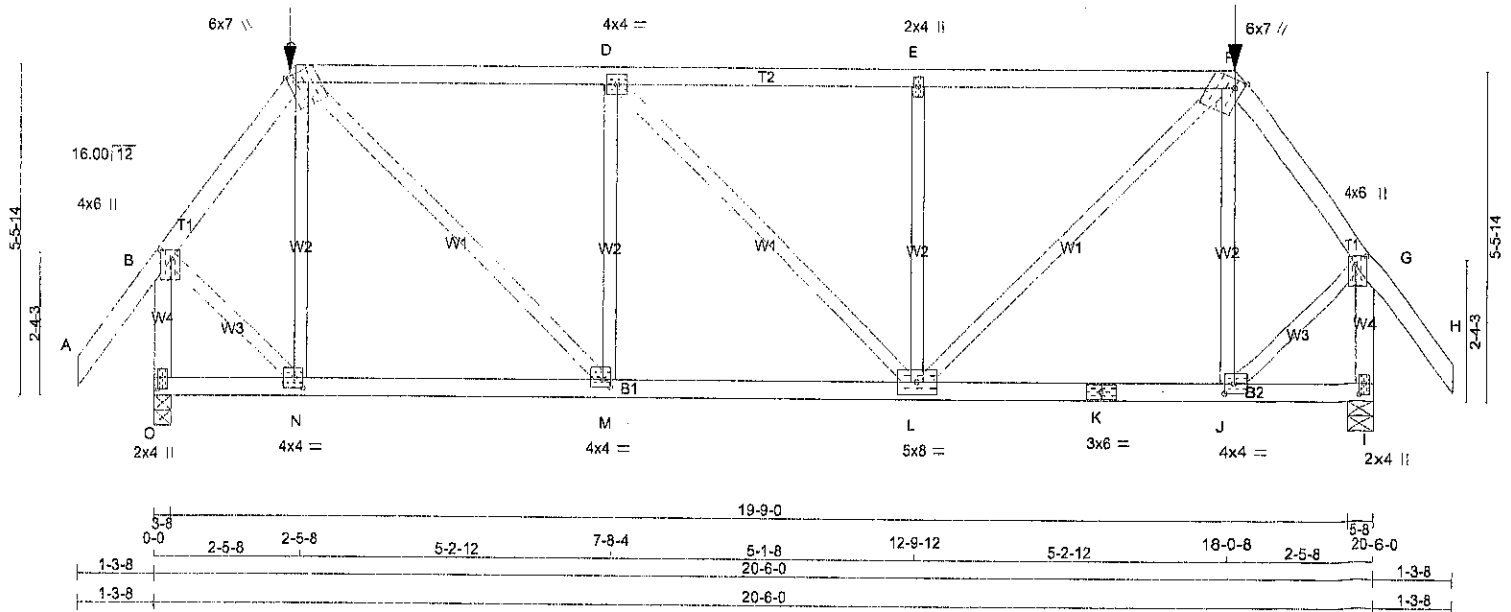


A-15073799(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253625	H11	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Thu Jul 16 16:43:35 2015 Page 1
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Scale = 1:38.5



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW+p	MT20	4.0	6.0	1.75 2.25
C TTWW+m	MT20	6.0	7.0	Edge 1.50
D TMWW+t	MT20	4.0	4.0	
E TMW+w	MT20	2.0	4.0	
F TTWW+m	MT20	6.0	7.0	Edge 1.50
G TMVW+p	MT20	4.0	6.0	1.75 2.25
I BMV1+p	MT20	2.0	4.0	2.25 1.00
J BMWW-t	MT20	4.0	4.0	2.00 1.75
K BS-t	MT20	3.0	6.0	
L BMWWW-t	MT20	5.0	8.0	
M BMWW-t	MT20	4.0	4.0	1.50 1.50
N BMWW-t	MT20	4.0	4.0	2.00 1.75
O BMV1+p	MT20	2.0	4.0	2.25 1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 127.8 lbs FACTORED DOWN AT 18-1-12, AND 127.8 lbs FACTORED DOWN AT 2-4-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
O	1887	0	1887	0	3-8
I	1887	0	1887	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
O	1328	907 / 0	0 / 0	0 / 0	0 / 0	421 / 0	0 / 0	
I	1328	907 / 0	0 / 0	0 / 0	0 / 0	421 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)		CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO		FROM	TO	FROM	TO	FR-TO		FROM	TO
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	N-C	-464 / 0	0.22 (1)	0.22 (1)
B-C	-1321 / 0	-70.5	-70.5	0.10 (1)	5.51	J-F	-461 / 0	0.22 (1)	0.22 (1)
C-D	-1796 / 0	-143.1	-143.1	0.69 (1)	3.99	M-D	-806 / 0	0.38 (1)	0.38 (1)
D-E	-1794 / 0	-143.1	-143.1	0.69 (1)	3.99	L-E	-805 / 0	0.38 (1)	0.38 (1)
E-F	-1794 / 0	-143.1	-143.1	0.68 (1)	4.01	C-M	0 / 1424	0.35 (1)	0.35 (1)
F-G	-1322 / 0	-70.5	-70.5	0.10 (1)	5.51	D-L	-3 / 0	0.00 (1)	0.00 (1)
G-H	0 / 40	-70.5	-70.5	0.11 (1)	10.00	L-F	0 / 1420	0.36 (1)	0.36 (1)
O-B	-1871 / 0	0.0	0.0	0.28 (1)	6.09	B-N	0 / 1004	0.25 (1)	0.25 (1)
I-G	-1872 / 0	0.0	0.0	0.28 (1)	6.09	J-G	0 / 1004	0.25 (1)	0.25 (1)
O-N	0 / 0	-35.5	-35.5	0.16 (4)	10.00				
N-M	0 / 776	-35.5	-35.5	0.28 (4)	10.00				
M-L	0 / 1797	-35.5	-35.5	0.43 (1)	10.00				
L-K	0 / 776	-35.5	-35.5	0.27 (4)	10.00				
K-J	0 / 776	-35.5	-35.5	0.27 (4)	10.00				
J-I	0 / 0	-35.5	-35.5	0.16 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
JT		2-4-4	-128	-128	---	FRONT	VERT	TOTAL
C		18-1-12	-128	-128	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-4-4
END SETBACK = 6-1-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.69 (C-D:1), BC=0.43 (L-M:1),
WB=0.38 (D-M:1), SSI=0.39 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

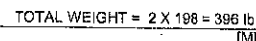
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.38 (M) (INPUT = 1.00)



A-10073798



A-15073797

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253625	H01-Cond2	2	1	TRUSS DESC.	
Alpa Roof Truss, Maple					
Version 7.820 S Apr 15 2015 MITek Industries, Inc. Thu Jul 16 16:43:34 2015 Page 2					
ID:AyrJx9Y7fa2EXF7DRQxSg4zMEz1-J4vrlwSZEOn5ywFv2_T4JNKSuPgdrVZGixsBoyxRw7					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD, AND 2180.1 lbs FACTORED DOWN AT 5-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1666
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.60 (T) (INPUT = 1.00)

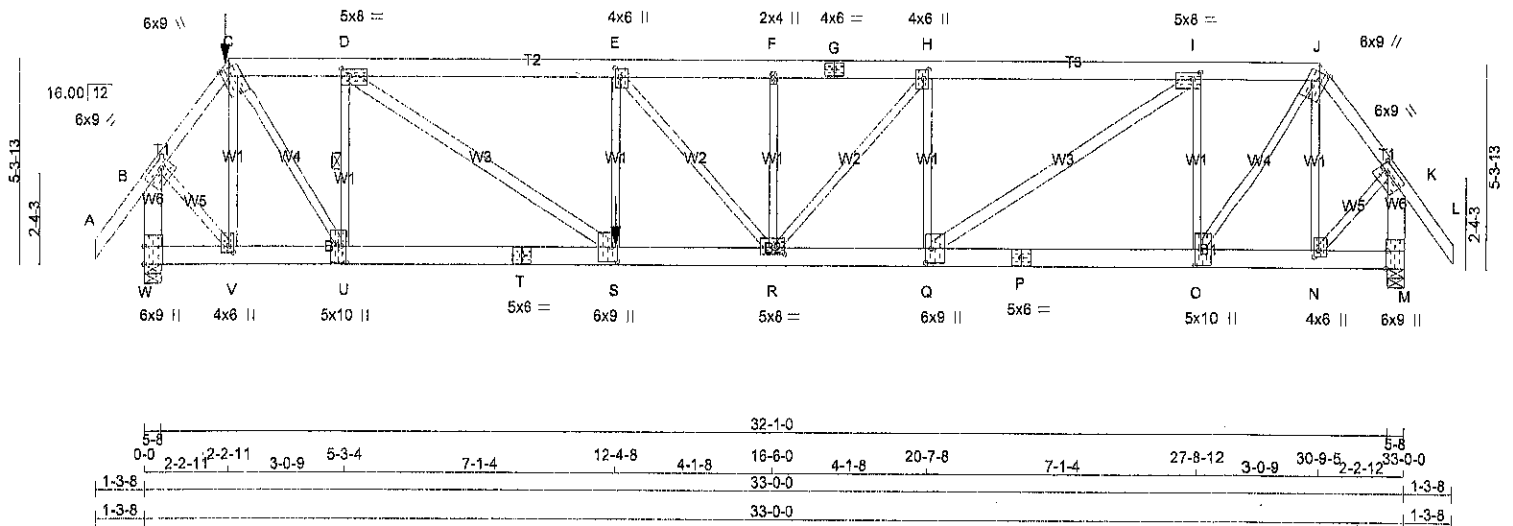


A-15073797(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253625	H01-Cond1	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Thu Jul 16 18:43:34 2015 Page 1
ID:AyrJx9Y7fa2EXF7DRQXsq4zMEz1-j4vrlwSZEDn5yWfV2_T4INKLfPajrUxGixsBpyxRw/
Scale = 1:60.1



TOTAL WEIGHT = 2 X 198 = 396 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - G	2x6	DRY No.2	SPF
G - J	2x8	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
W - B	2x6	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF
W - T	2x6	DRY No.2	SPF
T - P	2x6	DRY No.2	SPF
P - M	2x8	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
D - S	2x4	DRY No.2	SPF
Q - I	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+	MT20	6.0	9.0	2.25	3.00
C	TTWW+m	MT20	6.0	9.0	2.50	2.25
D	TMVW+	MT20	5.0	8.0	2.25	2.50
E	TMVW+	MT20	4.0	6.0	2.75	1.50
F	TMVW+	MT20	2.0	4.0		
G	TS+	MT20	4.0	6.0		
H	TMVW+	MT20	4.0	6.0	2.75	1.50
I	TMVW+	MT20	5.0	8.0	2.25	2.50
J	TTWW+m	MT20	6.0	9.0	2.50	2.25
K	TMVW+	MT20	6.0	9.0	2.25	3.00
M	BMV1+	MT20	6.0	9.0	Edge	0.50
N	BMVW+	MT20	4.0	6.0	2.00	1.50
O	BMVW+	MT20	5.0	10.0	5.00	1.75
P	BS+	MT20	5.0	6.0		
Q	BMVW+	MT20	6.0	9.0	4.50	1.75
R	BMVW+	MT20	5.0	8.0	2.00	4.00
S	BMVW+	MT20	6.0	9.0	4.50	1.75
T	BS+	MT20	5.0	6.0		
U	BMVW+	MT20	6.0	10.0	5.00	1.75
V	BMVW+	MT20	4.0	6.0	2.00	1.50
W	BMV1+	MT20	6.0	9.0	5.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	3567	0	3567	0	0	5-8	5-8
M	2471	0	2471	0	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
W	2626	1136 / 0	0 / 0	1490 / 0	0 / 0
M	1807	847 / 0	0 / 0	960 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.79FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 40	-70.5	-70.5 0.12 (1)	10.00	V-C	-1119 / 0	0.43 (1)	
B-C	-2664 / 0	-70.5	-70.5 0.18 (1)	4.06	C-U	0 / 3508	0.91 (1)	
C-D	-3430 / 0	-138.7	-138.7 0.42 (1)	4.11	U-D	-2795 / 0	0.61 (1)	
D-E	-6279 / 0	-138.7	-138.7 0.80 (1)	2.79	D-S	0 / 3452	0.75 (4)	
E-F	-5566 / 0	-70.5	-70.5 0.51 (1)	3.24	S-E	0 / 747	0.28 (4)	
F-G	-5566 / 0	-70.5	-70.5 0.41 (1)	3.38	E-R	-1101 / 0	0.82 (4)	
G-H	-5566 / 0	-70.5	-70.5 0.41 (1)	3.38	R-F	-129 / 0	0.05 (1)	
H-I	-4700 / 0	-70.5	-70.5 0.41 (1)	3.89	R-H	0 / 1338	0.45 (4)	
I-J	-2367 / 0	-70.5	-70.5 0.20 (1)	5.11	Q-H	-1481 / 0	0.57 (1)	
J-K	-1804 / 0	-70.5	-70.5 0.12 (1)	4.84	Q-I	0 / 2828	0.53 (1)	
K-L	0 / 40	-70.5	-70.5 0.12 (1)	10.00	O-I	-2013 / 0	0.78 (1)	
W-B	-3583 / 0	0.0	0.0 0.33 (1)	5.55	O-J	0 / 2451	0.64 (1)	
M-K	-2492 / 0	0.0	0.0 0.23 (1)	6.48	N-J	-757 / 0	0.29 (1)	
					B-V	0 / 2041	0.53 (1)	
					N-K	0 / 1387	0.36 (1)	
W-V	0 / 0	-34.5	-34.5 0.10 (1)	10.00				
V-U	0 / 1566	-34.5	-34.5 0.35 (4)	10.00				
U-T	0 / 3430	-34.5	-34.5 0.63 (4)	10.00				
T-S	0 / 3430	-34.5	-34.5 0.63 (4)	10.00				
S-R	0 / 6279	-17.5	-17.5 0.94 (4)	10.00				
R-Q	0 / 4700	-17.5	-17.5 0.69 (1)	10.00				
Q-P	0 / 2367	-17.5	-17.5 0.39 (1)	10.00				
P-O	0 / 2367	-17.5	-17.5 0.39 (1)	10.00				
O-N	0 / 1064	-17.5	-17.5 0.22 (1)	10.00				
N-M	0 / 0	-17.5	-17.5 0.08 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114		FRONT	VERT	TOTAL
S	12-4-8	-1920	-1920		FRONT	VERT	DEAD

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL
LOADS APPLIED TO FIRST 12-4-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/971 (0.41")

CSI: TC=0.80 (D-E:1), BC=0.94 (R-S:4),
WB=0.91 (C-U:1), SS=0.35 (D-E:1)

DOL LUMBER=0.95 NAIL=0.95 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

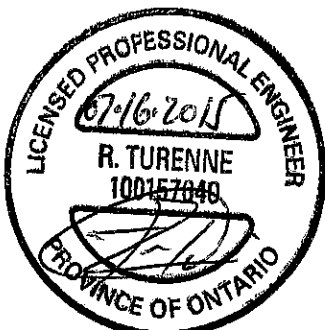
COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

A-1507396

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253625	H01-Cond1	2	1	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD, AND 2160.4 lbs FACTORED DOWN AT 12-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR	SECTION	
	(PSI)	(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284 1656

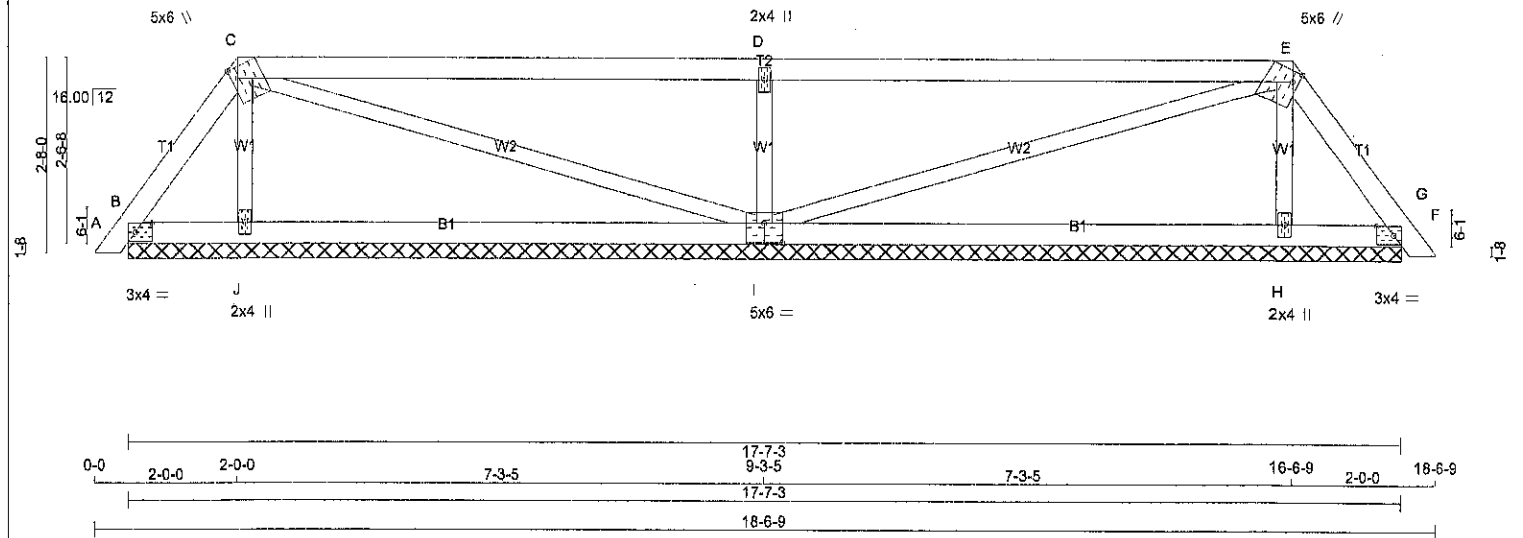
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.76 (S) (INPUT = 1.00)

A circular professional engineer seal for the Province of Ontario. The outer ring contains the text "LICENSED PROFESSIONAL ENGINEER" at the top and "PROVINCE OF ONTARIO" at the bottom. Inside the ring, the date "07.16.2014" is stamped at the top, the name "R. TURENNE" is in the center, and the license number "100157040" is at the bottom. A signature is written across the bottom half of the seal.

A-15073796(2)



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 C - E 2x4 DRY No.2 SPF
 E - G 2x4 DRY No.2 SPF
 B - I 2x4 DRY No.2 SPF
 I - F 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BSWWW1-I	MT20	5.0	6.0	3.25	3.00
J	BMW1+w	MT20	2.0	4.0		

NOTES: (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	119	0	119	0	-8	17-7-3 (6-9-10)8
J	287	0	287	0	0	17-7-3 (6-9-10)8
I	785	0	785	0	0	17-7-3 (6-9-10)8
H	287	0	287	0	0	17-7-3 (6-9-10)8
F	119	0	119	0	-8	17-7-3 (6-9-10)8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	79	84 / 0	0 / 0	0 / 0	0 / 0	0 / -5	0 / 0
J	208	105 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
I	550	389 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
H	208	105 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
F	79	84 / 0	0 / 0	0 / 0	0 / 0	0 / -5	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, I, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 9	-70.5	-70.5 0.01 (1)	10.00	J-C	-177 / 0	0.03 (1)
B-C	-76 / 0	-70.5	-70.5 0.03 (1)	6.25	C-I	-22 / 0	0.03 (1)
C-D	-13 / 0	-70.5	-70.5 0.63 (1)	8.25	I-D	-640 / 0	0.10 (1)
D-E	-13 / 0	-70.5	-70.5 0.63 (1)	8.25	I-E	-22 / 0	0.03 (1)
E-F	-76 / 0	-70.5	-70.5 0.03 (1)	8.25	H-E	-177 / 0	0.03 (1)
F-G	0 / 9	-70.5	-70.5 0.01 (1)	10.00			
B-J	0 / 43	-17.5	-17.5 0.14 (4)	10.00			
J-I	0 / 35	-17.5	-17.5 0.20 (4)	10.00			
I-H	0 / 35	-17.5	-17.5 0.20 (4)	10.00			
H-F	0 / 43	-17.5	-17.5 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63 (D-E:1), BC=0.20 (I-J:4), WB=0.10 (D-I:1), SSI=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 364 1667 822 2284 1656

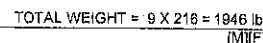
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

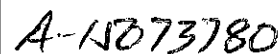
JSI GRIP = 0.88 (I) (INPUT = 0.90)
 JSI METAL = 0.24 (I) (INPUT = 1.00)



A-15073781



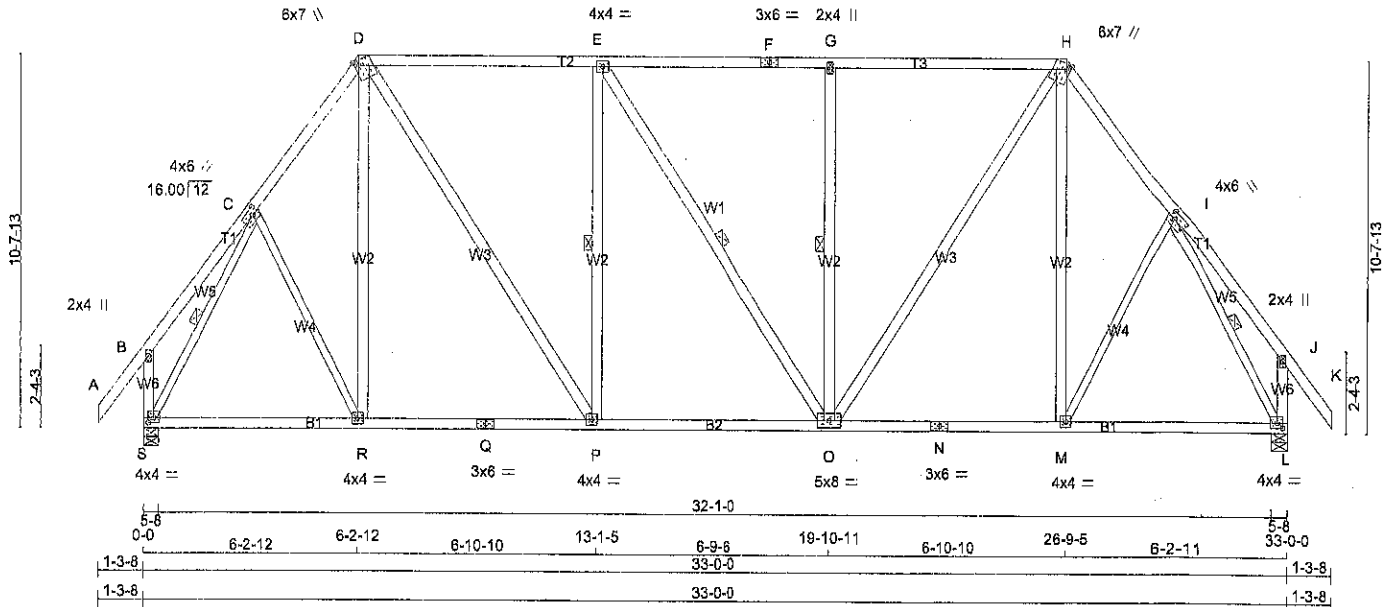
JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253622	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-NDMCJlvJWU5JHdBY2OKgDURCotBAXxVBF8mbawyxTrL
Scale = 1:66.1



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
I - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
C	TMVW-t	MT20	4.0	6.0 2.00 2.00
D	TTWV+m	MT20	6.0	7.0 Edge 1.50
E	TMVW-t	MT20	4.0	4.0
F	TS-t	MT20	3.0	8.0
G	TMVW-w	MT20	2.0	4.0
H	TTWV+m	MT20	6.0	7.0 Edge 1.50
I	TMVW-t	MT20	4.0	6.0 2.00 2.00
J	TMV+p	MT20	2.0	4.0
L	BMVW-t	MT20	4.0	4.0 1.75 1.75
M, P, R	BMVW-t	MT20	4.0	4.0
N	BS-t	MT20	3.0	8.0
O	BMVWV-t	MT20	6.0	8.0
Q	BS-t	MT20	3.0	8.0
S	BMVW-t	MT20	4.0	4.0 1.75 1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
S	1550	0	1550	0	5-8	1-11
L	1550	0	1550	0	5-8	1-11

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
S	1090	752 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
L	1090	752 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-P, E-O, G-O, C-S, H-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO		
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	C-R	0 / 102	0.02 (1)
B-C	0 / 19	-70.5	-70.5 0.10 (1)	10.00	R-D	0 / 88	0.02 (4)
C-D	-1224 / 0	-70.5	-70.5 0.13 (1)	5.67	D-P	0 / 766	0.12 (1)
D-E	-1144 / 0	-70.5	-70.5 0.52 (1)	5.24	P-E	-520 / 0	0.30 (1)
E-F	-1144 / 0	-70.5	-70.5 0.52 (1)	5.24	E-O	-1 / 0	0.00 (1)
F-G	-1144 / 0	-70.5	-70.5 0.52 (1)	5.24	O-G	-520 / 0	0.30 (1)
G-H	-1144 / 0	-70.5	-70.5 0.52 (1)	5.24	H-O	0 / 765	0.12 (1)
H-I	-1224 / 0	-70.5	-70.5 0.13 (1)	5.67	M-H	0 / 88	0.02 (4)
I-J	0 / 19	-70.5	-70.5 0.10 (1)	10.00	M-I	0 / 102	0.02 (1)
J-K	0 / 40	-70.5	-70.5 0.10 (1)	10.00	S-C	-1483 / 0	0.46 (1)
S-B	-183 / 0	0.0	0.0 0.02 (1)	7.81	I-L	-1483 / 0	0.46 (1)
L-J	-183 / 0	0.0	0.0 0.02 (1)	7.81			
S-R	0 / 889	-17.5	-17.5 0.23 (4)	10.00			
R-Q	0 / 719	-17.5	-17.5 0.24 (4)	10.00			
Q-P	0 / 719	-17.5	-17.5 0.24 (4)	10.00			
P-O	0 / 1144	-17.5	-17.5 0.27 (1)	10.00			
O-N	0 / 719	-17.5	-17.5 0.24 (4)	10.00			
N-M	0 / 719	-17.5	-17.5 0.24 (4)	10.00			
M-L	0 / 669	-17.5	-17.5 0.23 (4)	10.00			

TOTAL WEIGHT = 2 X 203 = 406 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.52 (D-E:1), BC=0.27 (O-P:1),
WB=0.46 (H-L:1), SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

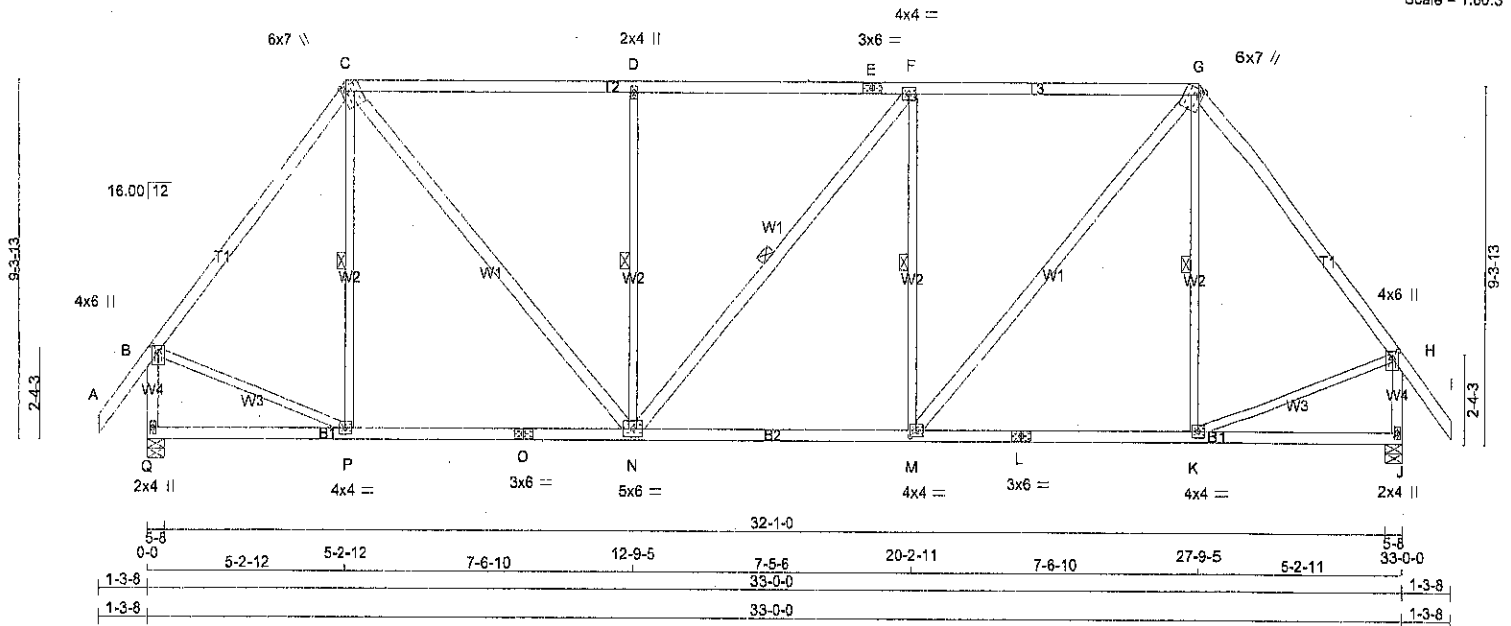
JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)



A-15073779

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253622	H04	2	1	TRUSS DESC.	
Alpa Roof Truss, Maple					

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ID:AyrJx9Y7fa2EXF7DRQxSq4zMeZ1-v0oq5?uglAzSfTdMUhpRgGf?NTr9oWP21U022TyxTrV
Scale = 1:60.3



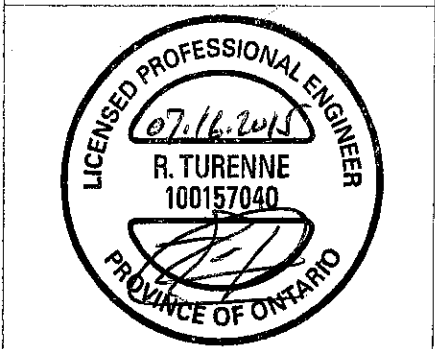
TOTAL WEIGHT = 2 X 169 = 338 lb [M]

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-I	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	4.0	2.00	1.75
N	BMVW-t	MT20	6.0	6.0		
O	BS-I	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
Q	1550	0	1550	0	5-8	1-11
J	1550	0	1550	0	5-8	1-11

JT	UNFACTORED REACTIONS						
	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
Q	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1090	752 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
J	1090	752 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-P, D-N, F-N, F-M, G-K, END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO				FR-TO		
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	P-C	-140 / 52	0.08 (1)	A-B	0 / 40	0.10 (1)
B-C	-1208 / 0	-70.5	-70.5	0.45 (1)	5.35	C-N	0 / 917	0.15 (1)	B-C	-1208 / 0	-70.5
C-D	-1310 / 0	-70.5	-70.5	0.66 (1)	4.76	N-D	-571 / 0	0.33 (1)	C-D	-1310 / 0	-70.5
D-E	-1310 / 0	-70.5	-70.5	0.67 (1)	4.75	F-G	-1 / 0	0.00 (1)	D-E	-1310 / 0	-70.5
E-F	-1310 / 0	-70.5	-70.5	0.67 (1)	4.75	M-F	-571 / 0	0.33 (1)	E-F	-1310 / 0	-70.5
F-G	-1311 / 0	-70.5	-70.5	0.67 (1)	4.75	M-G	0 / 919	0.15 (1)	F-G	-1311 / 0	-70.5
G-H	-1208 / 0	-70.5	-70.5	0.45 (1)	5.35	K-G	-141 / 52	0.08 (1)	G-H	-1208 / 0	-70.5
H-I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	B-P	0 / 769	0.17 (1)	H-I	0 / 40	-70.5
Q-B	-1517 / 0	0.0	0.0	0.19 (1)	6.67	K-H	0 / 769	0.17 (1)	Q-B	-1517 / 0	0.0
J-H	-1517 / 0	0.0	0.0	0.19 (1)	6.67				J-H	-1517 / 0	0.0
Q-P	0 / 0	-17.5	-17.5	0.17 (4)	10.00				Q-P	0 / 0	-17.5
P-O	0 / 720	-17.5	-17.5	0.25 (4)	10.00				P-O	0 / 720	-17.5
O-N	0 / 720	-17.5	-17.5	0.25 (4)	10.00				O-N	0 / 720	-17.5
N-M	0 / 1311	-17.5	-17.5	0.32 (1)	10.00				N-M	0 / 1311	-17.5
M-L	0 / 720	-17.5	-17.5	0.25 (4)	10.00				M-L	0 / 720	-17.5
L-K	0 / 720	-17.5	-17.5	0.25 (4)	10.00				L-K	0 / 720	-17.5
K-J	0 / 0	-17.5	-17.5	0.17 (4)	10.00				K-J	0 / 0	-17.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.67 (F-G:1), SC=0.32 (M-N:1), WB=0.33 (F-M:1), SSI=0.25 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

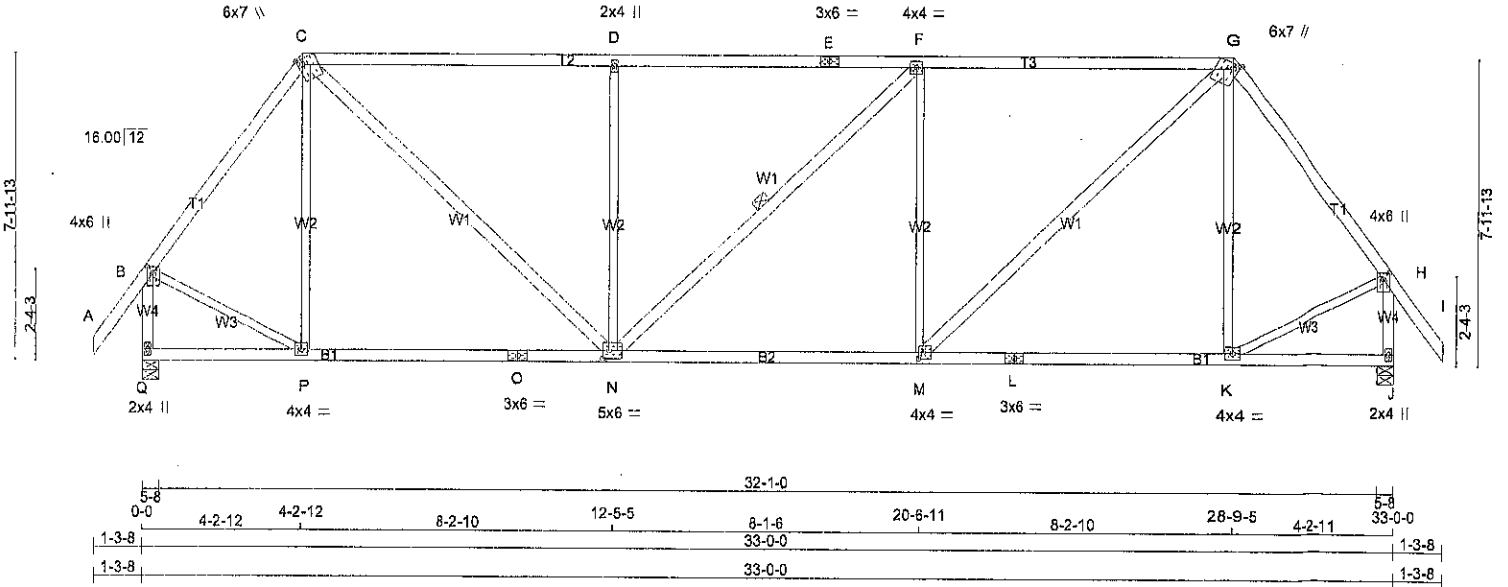
JSI GRIP = 0.89 (P) (INPUT = 0.90)
JSI METAL = 0.29 (B) (INPUT = 1.00)

A-15073778

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253622	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-v0oq5?ugIAzSfTdMUhpRgGlyHtQGoQh21U022TyxTr
Scale = 1:80.3



TOTAL WEIGHT = 2 X 160 = 321 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
C - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
C - N	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	
M - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTVW+m	MT20	6.0	7.0	Edge	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-i	MT20	4.0	4.0		
G	TTVW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-i	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-i	MT20	4.0	4.0	2.00	1.50
N	BMVW-i	MT20	5.0	6.0	2.50	2.00
O	BS-t	MT20	3.0	6.0		
P	BMVW-i	MT20	4.0	4.0		
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	
Q	1550	0	1550	0	0	5-8	1-11		
J	1550	0	1550	0	0	5-8	1-11		

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1090	752 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
J	1090	752 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX UNBRAC	MEMB.	FORCE	MAX
	(LBS)	(PLF)	(LC)	LENGTH		(LBS)	(LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	P-C	-198 / 32	0.24 (1)
B-C	-1195 / 0	-70.5	-70.5 0.27 (1)	5.55	C-N	0 / 1125	0.18 (1)
C-D	-1534 / 0	-70.5	-70.5 0.66 (1)	4.19	N-D	-622 / 0	0.75 (1)
D-E	-1534 / 0	-70.5	-70.5 0.66 (1)	4.17	N-F	-1 / 0	0.00 (1)
E-F	-1534 / 0	-70.5	-70.5 0.66 (1)	4.17	M-F	-822 / 0	0.76 (1)
F-G	-1535 / 0	-70.5	-70.5 0.67 (1)	4.17	M-G	0 / 1126	0.18 (1)
G-H	-1195 / 0	-70.5	-70.5 0.27 (1)	5.55	K-G	-199 / 32	0.24 (1)
H-I	0 / 40	-70.5	-70.5 0.10 (1)	10.00	B-P	0 / 783	0.18 (1)
Q-B	-1530 / 0	0.0	0.0 0.20 (1)	8.65	K-H	0 / 783	0.18 (1)
J-H	-1530 / 0	0.0	0.0 0.20 (1)	8.65			
Q-P	0 / 0	-17.5	-17.5 0.18 (4)	10.00			
P-O	0 / 711	-17.5	-17.5 0.29 (4)	10.00			
O-N	0 / 711	-17.5	-17.5 0.29 (4)	10.00			
N-M	0 / 1535	-17.5	-17.5 0.38 (1)	10.00			
M-L	0 / 710	-17.5	-17.5 0.29 (4)	10.00			
L-K	0 / 710	-17.5	-17.5 0.29 (4)	10.00			
K-J	0 / 0	-17.5	-17.5 0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. G/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.87 (F-G:1), BC=0.38 (M-N:1),
WB=0.76 (F-M:1), SSI=0.27 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

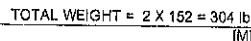
JSI GRIP= 0.89 (B) (INPUT = 0.80)
JSI METAL= 0.31 (M) (INPUT = 1.00)



A-15073777

Alpa Roof Truss, Maple

Version 7.820 S Apr 15 2016 MiTek Industries, Inc. Thu Jul 16 14:31:57 2015 Page 1
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-RqERugt2_trb1K29xzlC83lnh4Tw31YuoqHVW1yxTrV
Scale = 1:60.3



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

ESTIMATING BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q. J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.10")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (1.10")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.20")

CSI: TC=0.86 (F-G:1), BC=0.45 (M-N:1),
WB=0.49 (F-M:1), SSI=0.29 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (Q) (INPUT = 0.90)
JSI METAL= 0.44 (M) (INPUT = 1.00)



A-15073776

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

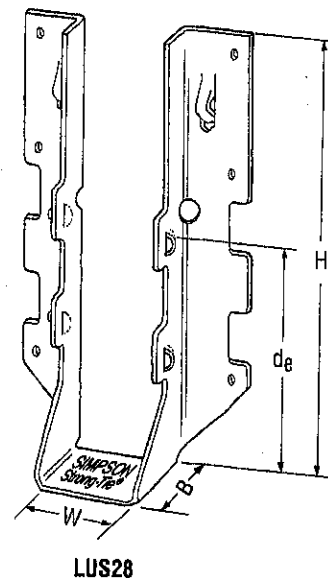
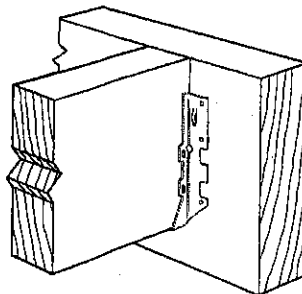
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

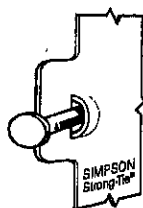
- These hangers cannot be modified.

Typical LUS
Installation



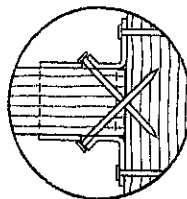
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1½	3½	1½	1½	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	4-16d	2-16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3¾	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

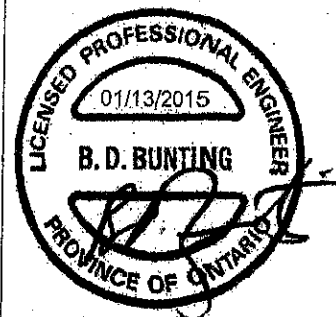


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

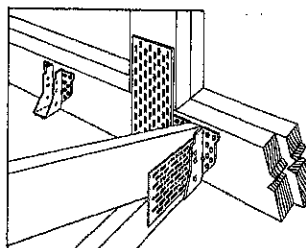
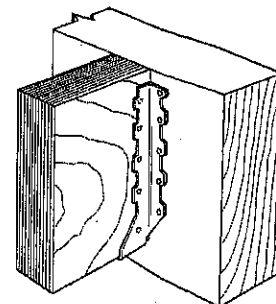
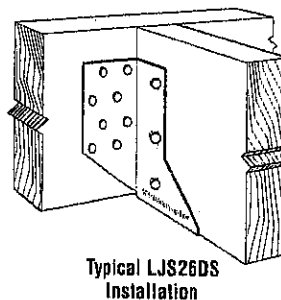
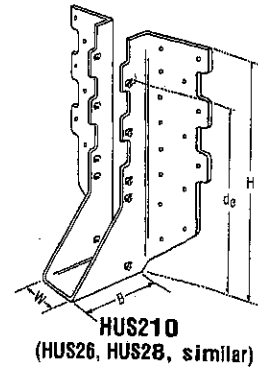
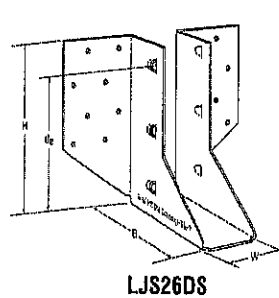
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

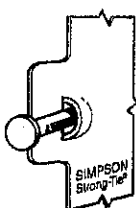
OPTIONS:

- See current catalogue for options



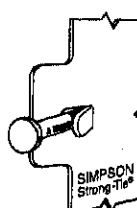
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 3/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 1/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/8	22-16d	8-16d	3605	5365	2875	4345
HUS210	16	1 1/8	9 3/8	3	7 3/8	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

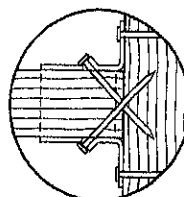


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

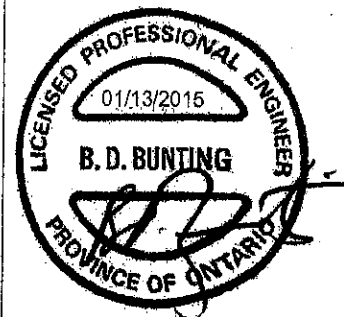
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



800-999-5099
www.strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

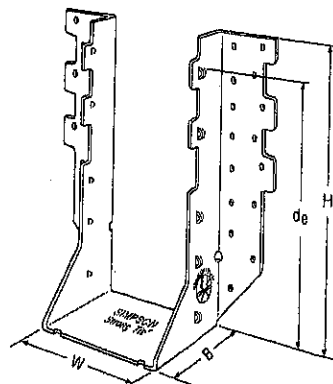
- Factored resistances are in accordance with CSA Q86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

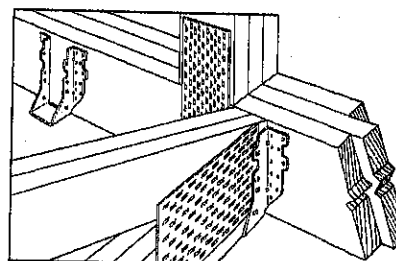
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

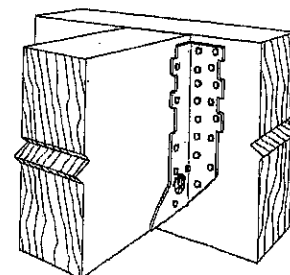
- See current catalogue for options



HHUS410



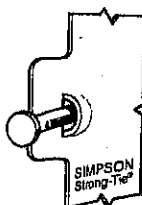
Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)



Typical HHUS
Installation

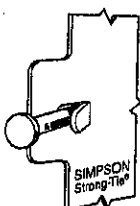
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
HHUS26-2	14	3 1/8	5 1/8	3	3 15/16	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 1/2	3	6 5/32	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3 1/8	9 3/32	3	8	30-16d	10-16d	4745	9660	4310	7000
HHUS210-3	14	4 1/8	9	3	7 15/16	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 21/32	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3 1/8	5 1/2	3	3 15/16	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3 1/8	7 1/8	3	6 1/8	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3 3/8	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 3/8	7 29/32	30-16d	10-16d	4745	10770	4310	7650

1. d_g is the distance from the seat of the hanger to the highest joist nail.

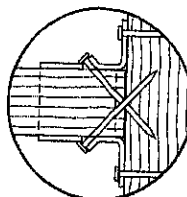


Deme Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

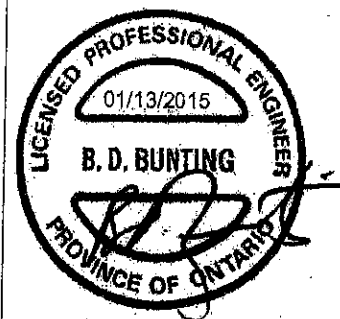
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



800-999-5099
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TC – Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

MATERIAL: 16 gauge

FINISH: G90 galvanized

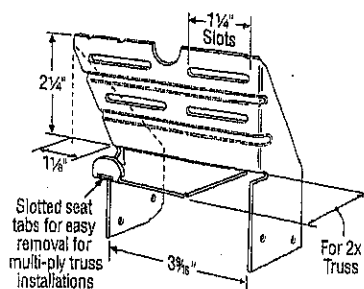
DESIGN: Factored resistances are in accordance with CSA 086-14

INSTALLATION:

- Use all specified fasteners
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

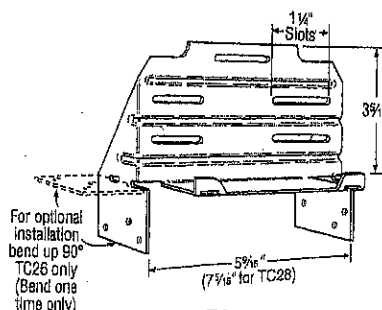
Optional TC Installation

- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



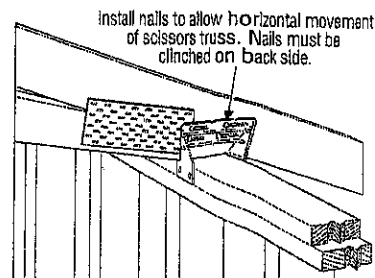
TC24

U.S. Patent 4,932,173

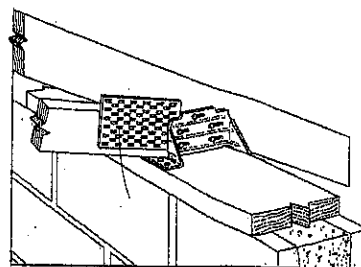


TC26

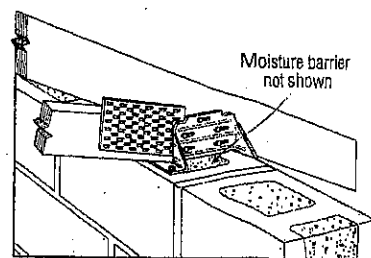
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



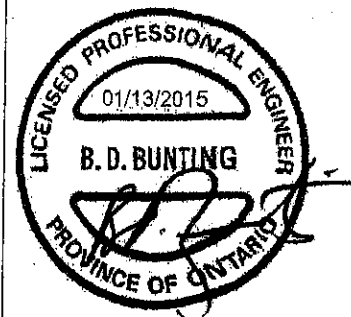
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _D = 1.15)	Uplift (K _D = 1.15)
			lbs	lbs
TC24	4-10d	4-10d	605	430
TC26	5-10d	6-10d	1015	720
TC28	5-10d	6-10d	1015	720

OPTIONAL TC INSTALLATION TABLE

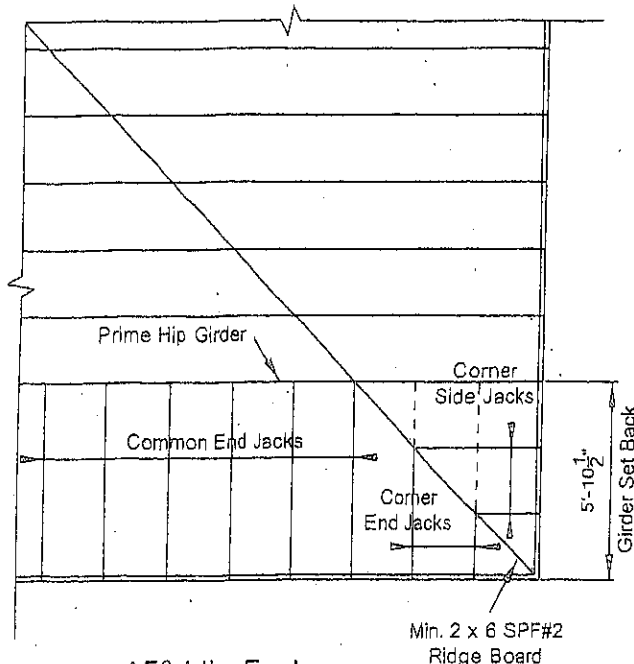
Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _D = 1.15)	Uplift (K _D = 1.15)
			lbs	lbs
TC26	5-10d	6-10dx1½"	810	660
	5-10d	6-10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (8) - ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lbs.



800-999-5099
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45° Hip End

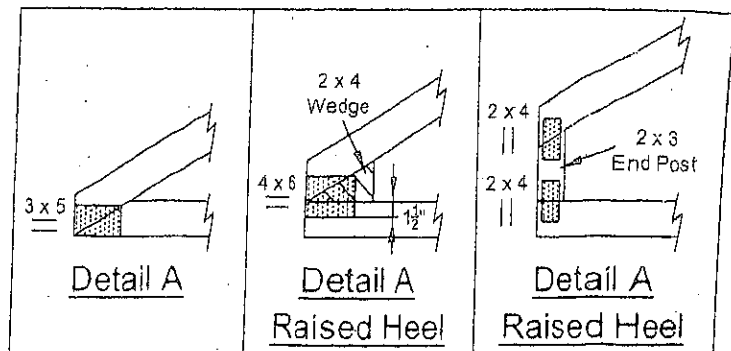
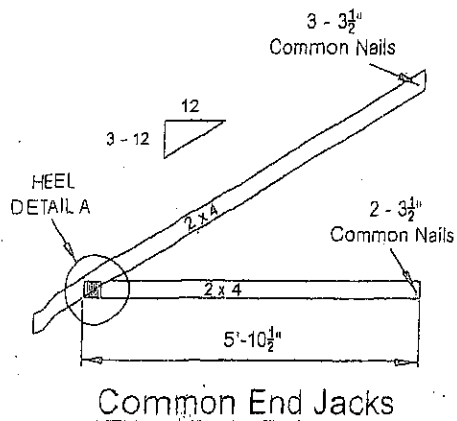
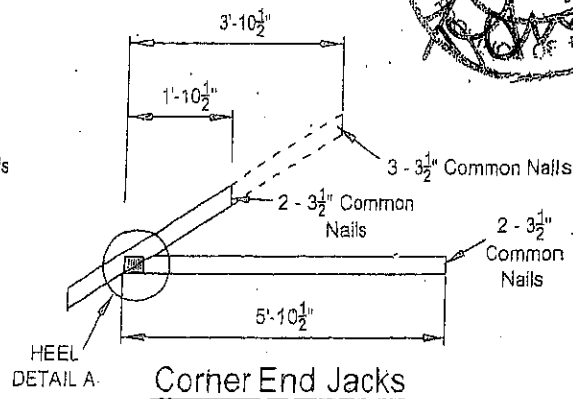
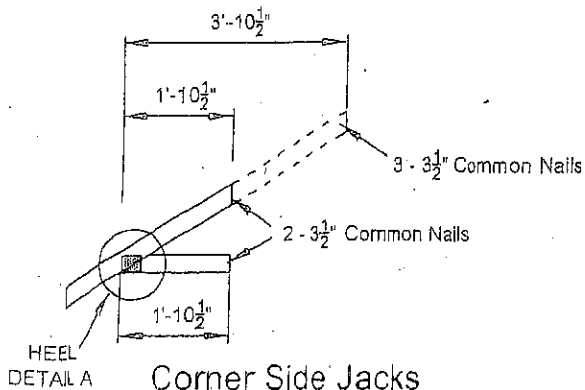
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008