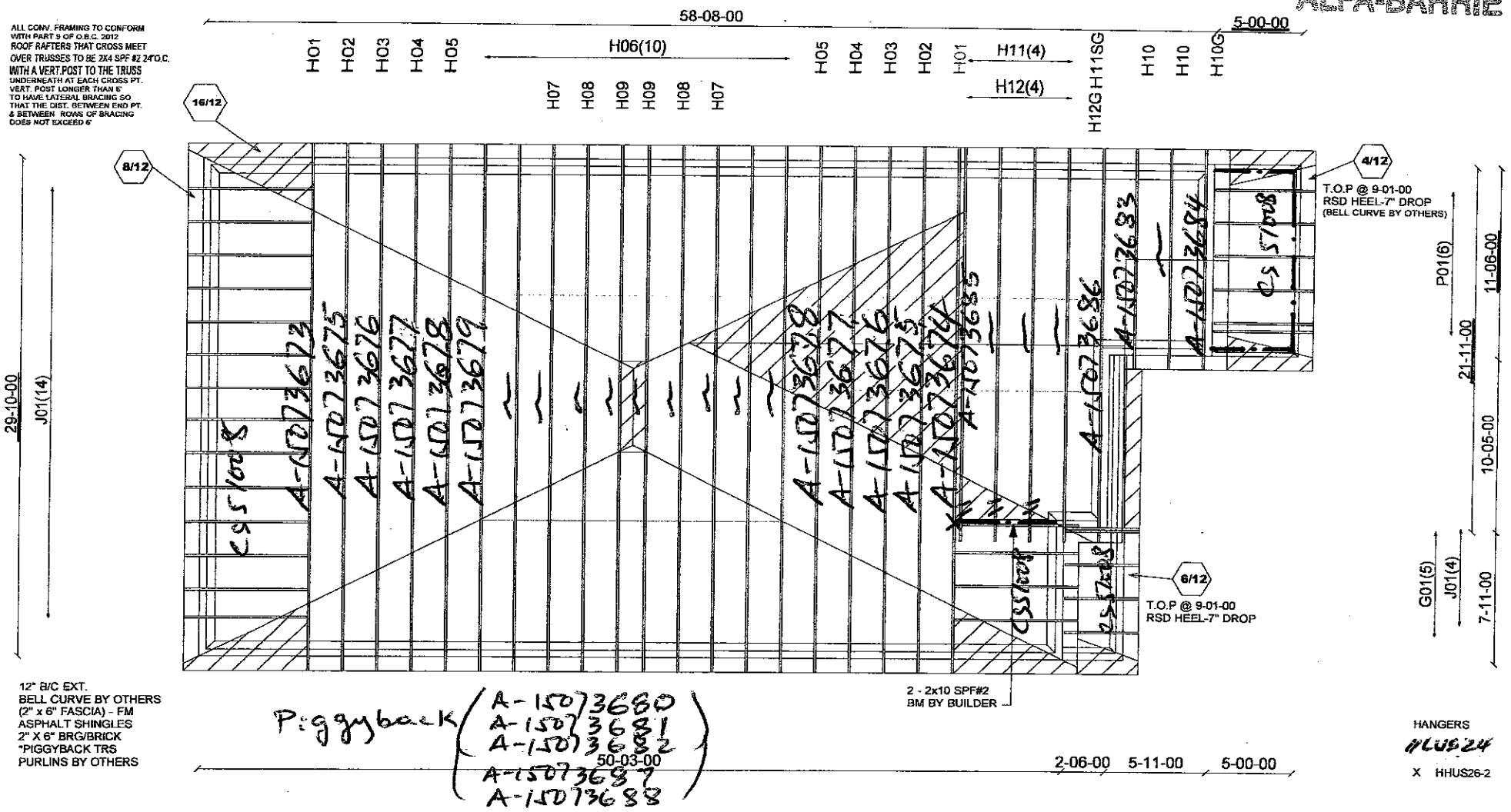
ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



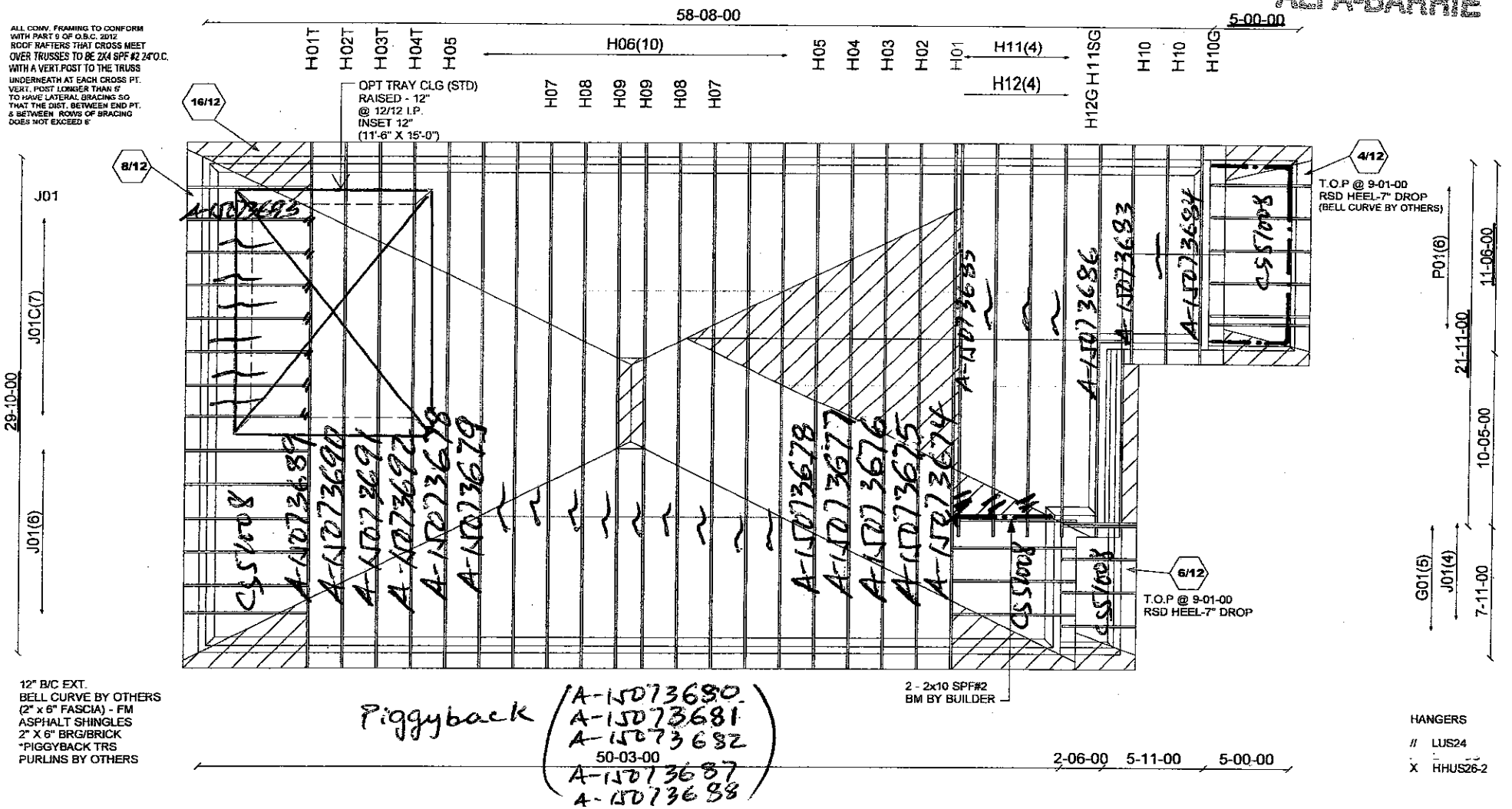
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

DL 80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 / A	STD OR OPT 2ND FLR
Layout ID: 253442	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 5/27/2015 Designer: cheryle		

ALPA-BARRIE



PL80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 / A OPT TRAY	
Layout ID: 253443	Project: HUNTINGTON & NASHVILLE		THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
Plan Log: 79711	Date: 5/27/2015	Designer: cheryle	

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE

58-08-00

5-00-00

H01T H02T H03T H04T H05T H06T H07 H08 H09 H09 H08 H07 H05 H04 H03 H02 H01 H12G H11SG H10 H10 H10G

H06(9)

H11(4)

H12(4)

16/12

8/12

4/12

T.O.P @ 9-01-00
RSD HEEL-7" DROP
(BELL CURVE BY OTHERS)

6/12

T.O.P @ 9-01-00
RSD HEEL-7" DROP

CSS10T8

A-150736 94 A-150736 95 A-150736 96 A-150736 97 A-150736 98 A-150736 99 A-150736 79 A-150736 78 A-150736 77 A-150736 76 A-150736 75 A-150736 74 A-150736 85 A-150736 86 A-150736 83 A-150736 84

A-150736 93

Piggyback

29-10-00

J01(9)

J01C(4)

J01

P01(6)

11-06-00

21-11-00

10-05-00

G01(5)

J01(4)

7-11-00

12" B/C EXT.
BELL CURVE BY OTHERS
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
"PIGGYBACK TRS
PURLINS BY OTHERS

OPT TRAY CLG (5 BED)
RAISED - 12"
@ 12/12 I.P.
INSET 12"
(15'-11"X 10'-0")

2 - 2x10 SPF#2
BM BY BUILDER

A-150736 80
A-150736 81
A-150736 82
A-150736 87
A-150736 88

50-03-00

2-06-00 5-11-00 5-00-00

HANGERS
// LUS24
X HHUS26-2

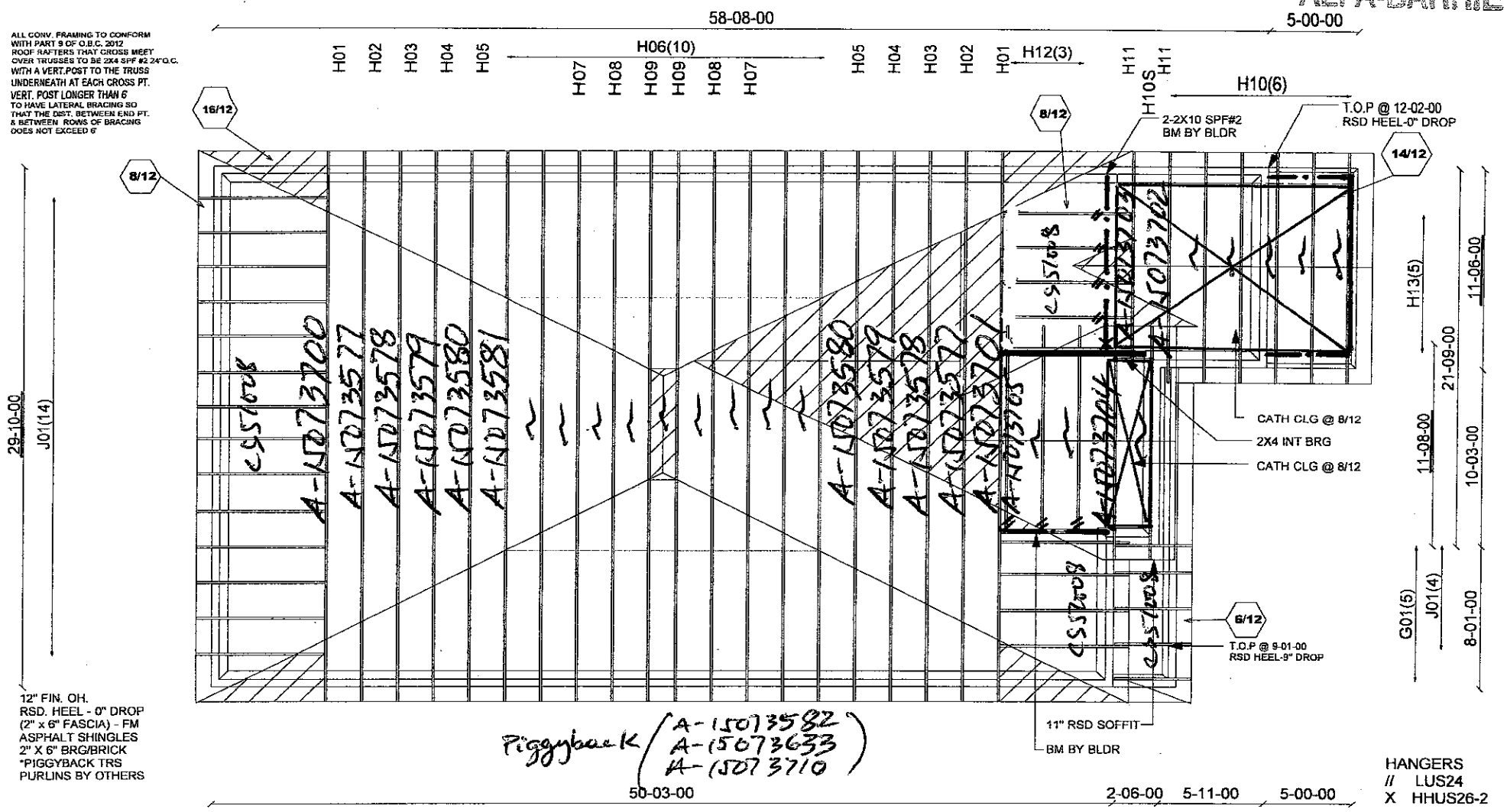
CONVENTIONAL FRAMING BY OTHERS

PL80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5	/ A OPT 2ND W/TRAY
Layout ID: 253459	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 5/27/2015 Designer: cheryle		

ALPA-BARRIE



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

HANGERS
// LUS24
X HHUS26-2

CONVENTIONAL FRAMING BY OTHERS

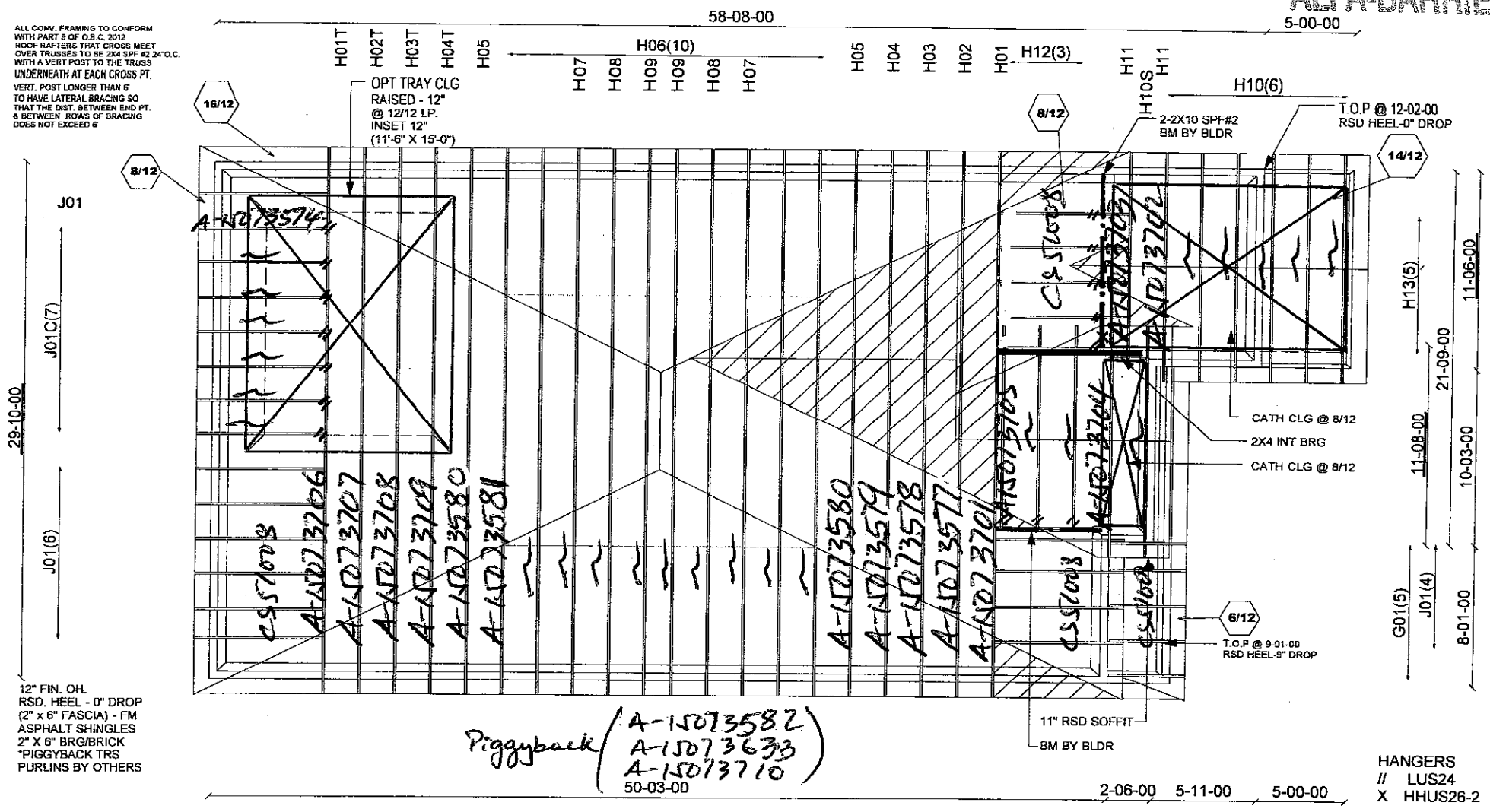
PL 80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 / B	STD OR OPT 2ND FLR.
Layout ID: 253444	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015 Designer: cheryle		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



HANGERS
// LUS24
X HHUS26-2

CONVENTIONAL FRAMING BY OTHERS

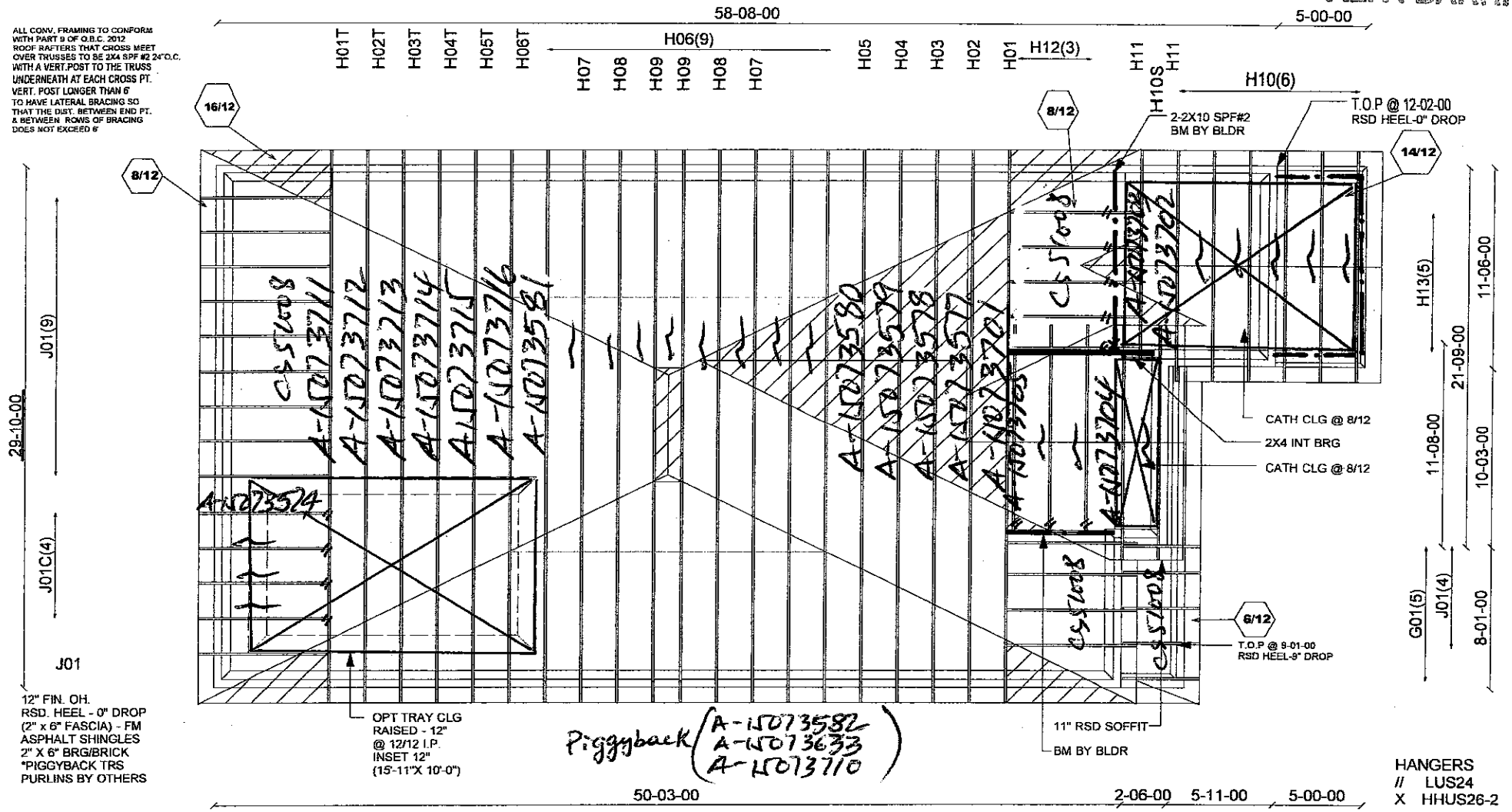
Mitek Ver: 7.5.0

PL80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 / B OPT TRAY	
Layout ID: 253445	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015 Designer: cheryle		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



HANGERS
// LUS24
X HHUS26-2

CONVENTIONAL FRAMING BY OTHERS

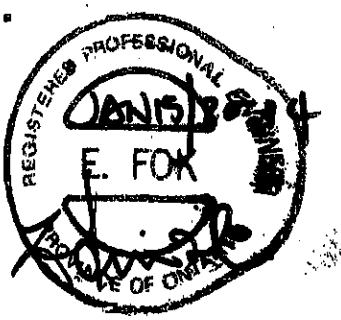
Mitek Ver: 7.5.0

PL80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-5 / B OPT 2ND W/TRAY	
Layout ID: 253460	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015	Designer: cheryle	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253460	H06T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 17:46:53 2015 Page 1
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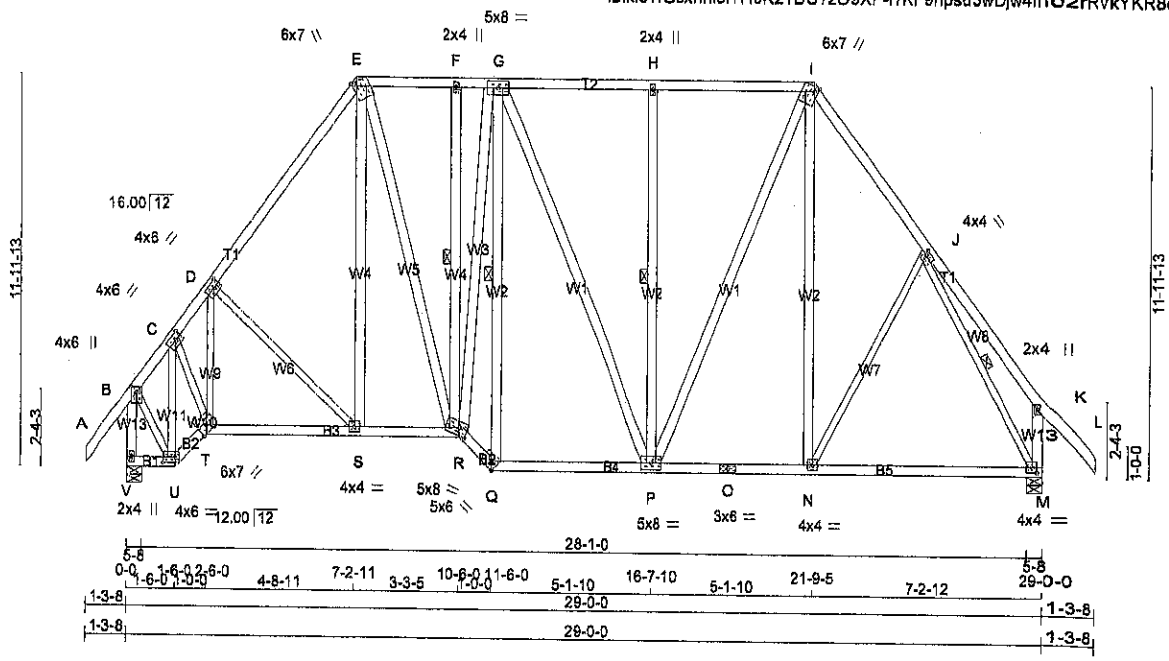

TOTAL WEIGHT = 232 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
U - C	2x3	DRY	No.2	SPF
C - T	2x3	DRY	No.2	SPF
T - D	2x3	DRY	No.2	SPF
D - S	2x3	DRY	No.2	SPF
N - J	2x3	DRY	No.2	SPF
B - U	2x3	DRY	No.2	SPF
J - M	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	8.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	1.50
J	TMWW-t	MT20	4.0	4.0	1.50	1.00
K	TMV+p	MT20	2.0	4.0		
M	BMVW1-t	MT20	4.0	4.0	2.00	1.75
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BBW-h	MT20	5.0	6.0	Edge	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1377	0	1377	0	0	5-8	1-8
M	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
M	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.74FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-R, G-Q, H-P, J-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-767 / 0	-70.5	-70.5	0.10 (1)	6.25
C- D	-1035 / 0	-70.5	-70.5	0.21 (1)	5.90
D- E	-1110 / 0	-70.5	-70.5	0.26 (1)	5.74
E- F	-800 / 0	-70.5	-70.5	0.11 (1)	6.25
F- G	-799 / 0	-70.5	-70.5	0.16 (1)	6.25
G- H	-790 / 0	-70.5	-70.5	0.28 (1)	6.25
H- I	-789 / 0	-70.5	-70.5	0.28 (1)	6.25
I- J	-1027 / 0	-70.5	-70.5	0.17 (1)	6.00
J- K	0 / 23	-70.5	-70.5	0.14 (1)	10.00
K- L	0 / 40	-70.5	-70.5	0.10 (1)	10.00
V- B	-1364 / 0	0.0	0.0	0.17 (1)	6.95
M- K	-195 / 0	0.0	0.0	0.02 (1)	7.81
V- U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U- T	0 / 574	-17.5	-17.5	0.10 (1)	10.00
T- S	0 / 704	-17.5	-17.5	0.17 (1)	10.00
S- R	0 / 649	-17.5	-17.5	0.16 (1)	10.00
R- Q	0 / 1073	-17.5	-17.5	0.18 (1)	10.00
Q- P	0 / 767	-17.5	-17.5	0.18 (1)	10.00
P- O	0 / 601	-17.5	-17.5	0.25 (4)	10.00
O- N	0 / 601	-17.5	-17.5	0.25 (4)	10.00
N- M	0 / 596	-17.5	-17.5	0.24 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-940 / 0	0.24 (1)
C- T	0 / 793	0.18 (1)
T- D	-264 / 0	0.09 (1)
D- S	-78 / 0	0.06 (1)
S- E	0 / 133	0.03 (4)
E- R	0 / 512	0.08 (1)
R- F	-78 / 0	0.05 (1)
R- G	0 / 379	0.06 (1)
Q- G	-706 / 0	0.55 (1)
G- P	0 / 57	0.01 (1)
P- H	-402 / 0	0.32 (1)
P- I	0 / 469	0.08 (1)
N- I	0 / 125	0.03 (4)
N- J	0 / 39	0.01 (4)
B- U	0 / 704	0.16 (1)
J- M	-1272 / 0	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.26 (H-I:1) , BC=0.25 (N-P:4) , WB=0.55 (G-Q:1) , SSI=0.17 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.47 (J) (INPUT = 1.00)

A-1507 3716

253460	H05T	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 17:46:52 2015 Page 1
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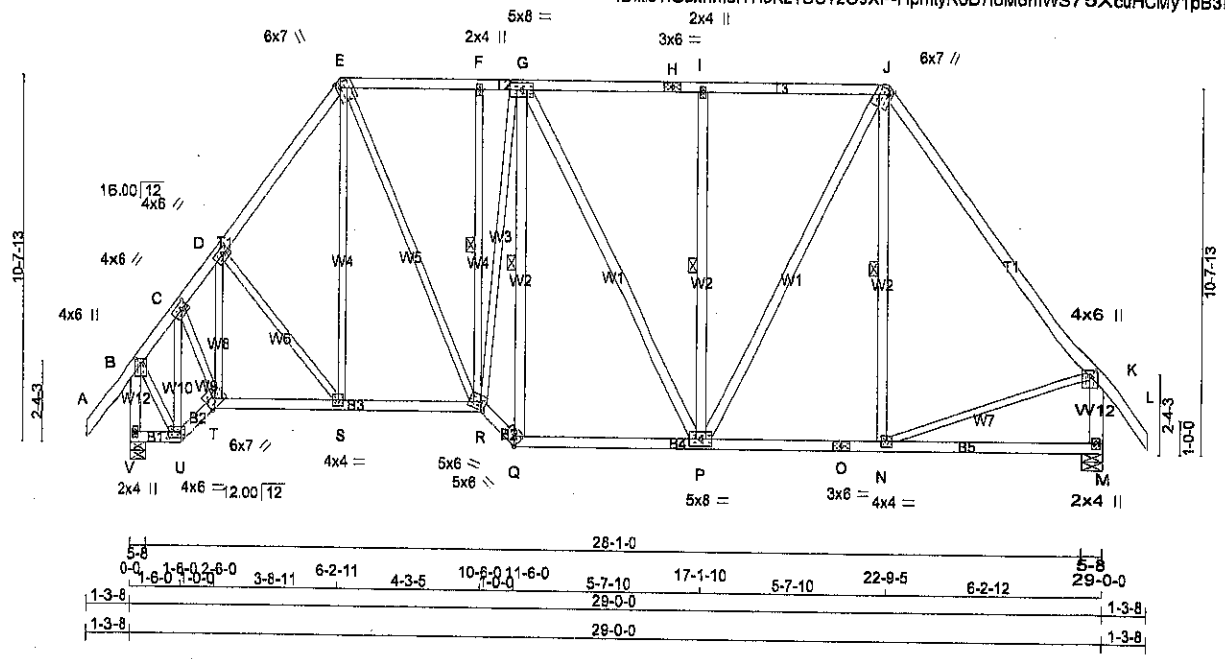

TOTAL WEIGHT = 198 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	2100F 1.8E	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	2100F 1.8E	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
G - P	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	8.0		
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BBW-h	MT20	5.0	6.0	Edge	2.50
R	BBWWW-m	MT20	5.0	6.0	2.75	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1377	0	1377	0	0	5-8	1-8
M	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	968	669 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-R, G-Q, I-P, J-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.07 (1)	10.00
B- C	-786 / 0	-70.5	-70.5	0.07 (1)	6.25
C- D	-1064 / 0	-70.5	-70.5	0.08 (1)	6.25
D- E	-1136 / 0	-70.5	-70.5	0.08 (1)	6.25
E- F	-920 / 0	-70.5	-70.5	0.18 (1)	6.21
F- G	-918 / 0	-70.5	-70.5	0.19 (1)	6.19
G- H	-887 / 0	-70.5	-70.5	0.34 (1)	6.04
H- I	-887 / 0	-70.5	-70.5	0.34 (1)	6.04
I- J	-886 / 0	-70.5	-70.5	0.35 (1)	6.04
J- K	-1023 / 0	-70.5	-70.5	0.38 (1)	6.25
K- L	0 / 40	-70.5	-70.5	0.07 (1)	10.00
V- B	-1364 / 0	0.0	0.0	0.17 (1)	6.95
M- K	-1327 / 0	0.0	0.0	0.17 (1)	7.03
V- U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U- T	0 / 579	-17.5	-17.5	0.10 (1)	10.00
T- S	0 / 690	-17.5	-17.5	0.18 (1)	10.00
S- R	0 / 868	-17.5	-17.5	0.16 (1)	10.00
R- Q	0 / 1213	-17.5	-17.5	0.20 (1)	10.00
Q- P	0 / 869	-17.5	-17.5	0.21 (1)	10.00
P- O	0 / 611	-17.5	-17.5	0.20 (4)	10.00
O- N	0 / 611	-17.5	-17.5	0.20 (4)	10.00
N- M	0 / 0	-17.5	-17.5	0.15 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-847 / 0	0.25 (1)
C- T	0 / 747	0.17 (1)
T- D	-247 / 0	0.08 (1)
D- S	-35 / 0	0.02 (1)
S- E	0 / 104	0.03 (4)
E- R	0 / 602	0.14 (1)
R- F	-140 / 0	0.09 (1)
R- G	0 / 497	0.11 (1)
Q- G	-803 / 0	0.46 (1)
G- P	0 / 38	0.01 (1)
P- I	-443 / 0	0.25 (1)
P- J	0 / 576	0.09 (1)
N- J	-70 / 70	0.04 (1)
B- U	0 / 709	0.16 (1)
N- K	0 / 640	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.38 (J-K:1) , BC=0.21 (P-Q:1) , WB=0.46 (G-Q:1) , SSI=0.19 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (R) (INPUT = 0.90)
JSI METAL= 0.25 (K) (INPUT = 1.00)

A-150737/5

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253460	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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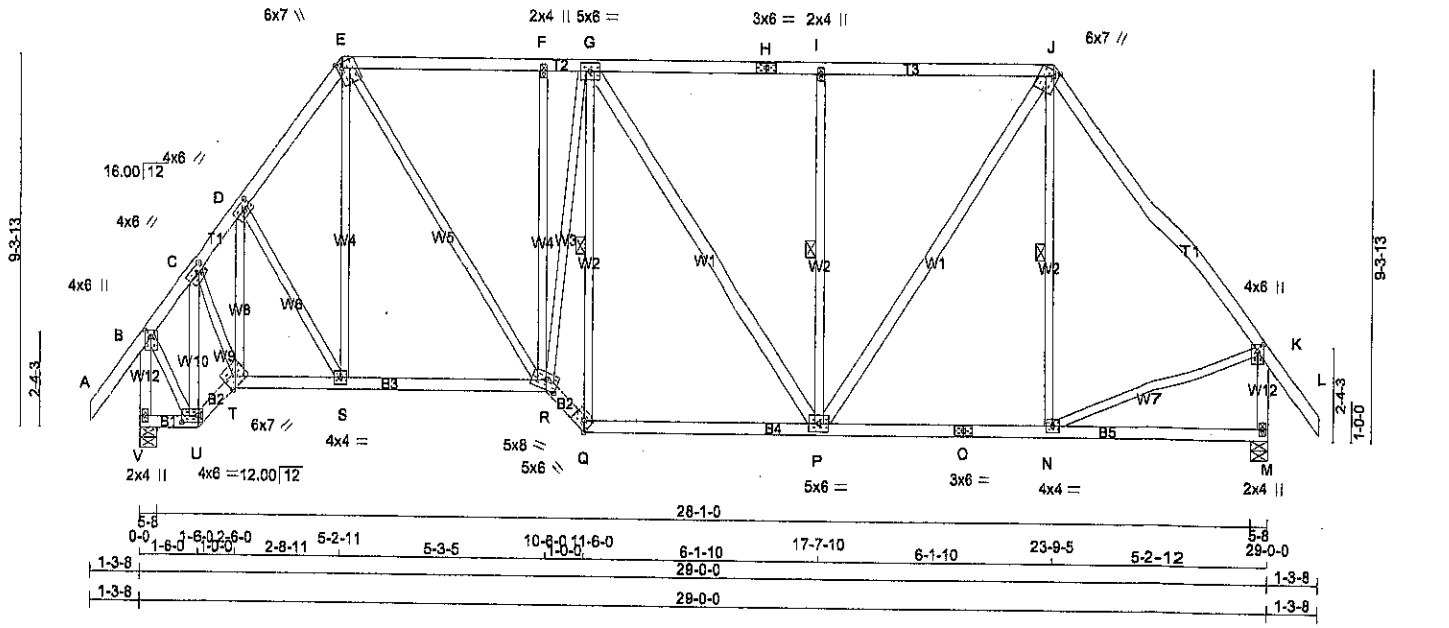

TOTAL WEIGHT = 176 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - P	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BBW-h	MT20	5.0	6.0	Edge	2.50
R	BBWWW-m	MT20	5.0	8.0	2.75	3.75
S	BMWW-t	MT20	4.0	4.0		
T	BBWW-h	MT20	6.0	7.0	Edge	
U	BBWW-t	MT20	4.0	6.0	2.00	4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1377	0	1377	0	0	5-8	1-8
M	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
M	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-Q, I-P, J-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-784 / 0	-70.5	-70.5	0.10 (1)	6.25
C- D	-1084 / 0	-70.5	-70.5	0.06 (1)	6.01
D- E	-1159 / 0	-70.5	-70.5	0.08 (1)	5.84
E- F	-1082 / 0	-70.5	-70.5	0.26 (1)	5.74
F- G	-1080 / 0	-70.5	-70.5	0.23 (1)	5.77
G- H	-1011 / 0	-70.5	-70.5	0.42 (1)	5.63
H- I	-1011 / 0	-70.5	-70.5	0.42 (1)	5.63
I- J	-1010 / 0	-70.5	-70.5	0.43 (1)	5.63
J- K	-1030 / 0	-70.5	-70.5	0.43 (1)	5.69
K- L	0 / 40	-70.5	-70.5	0.10 (1)	10.00
V- B	-1364 / 0	0.0	0.0	0.17 (1)	6.95
M- K	-1336 / 0	0.0	0.0	0.17 (1)	7.00
V- U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U- T	0 / 582	-17.5	-17.5	0.10 (1)	10.00
T- S	0 / 880	-17.5	-17.5	0.17 (1)	10.00
S- R	0 / 684	-17.5	-17.5	0.17 (1)	10.00
R- Q	0 / 1398	-17.5	-17.5	0.23 (1)	10.00
Q- P	0 / 999	-17.5	-17.5	0.24 (1)	10.00
P- O	0 / 614	-17.5	-17.5	0.20 (4)	10.00
O- N	0 / 614	-17.5	-17.5	0.20 (4)	10.00
N- M	0 / 0	-17.5	-17.5	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-954 / 0	0.25 (1)
C- T	0 / 714	0.16 (1)
T- D	-227 / 0	0.08 (1)
D- S	0 / 11	0.00 (4)
S- E	0 / 92	0.03 (4)
E- R	0 / 721	0.16 (1)
R- F	-223 / 0	0.30 (1)
R- G	0 / 866	0.15 (1)
Q- G	-931 / 0	0.53 (1)
G- P	0 / 20	0.00 (1)
P- I	-485 / 0	0.26 (1)
P- J	0 / 703	0.11 (1)
N- J	-118 / 46	0.07 (1)
B- U	0 / 714	0.16 (1)
N- K	0 / 865	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA O86-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.43 (J-K:1) , BC=0.24 (P-Q:1) , WB=0.53 (G-Q:1) , SSI=0.20 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (R) (INPUT = 0.90)
JSI METAL= 0.25 (V) (INPUT = 1.00)

A-1507314

					DRWG NO.
253460	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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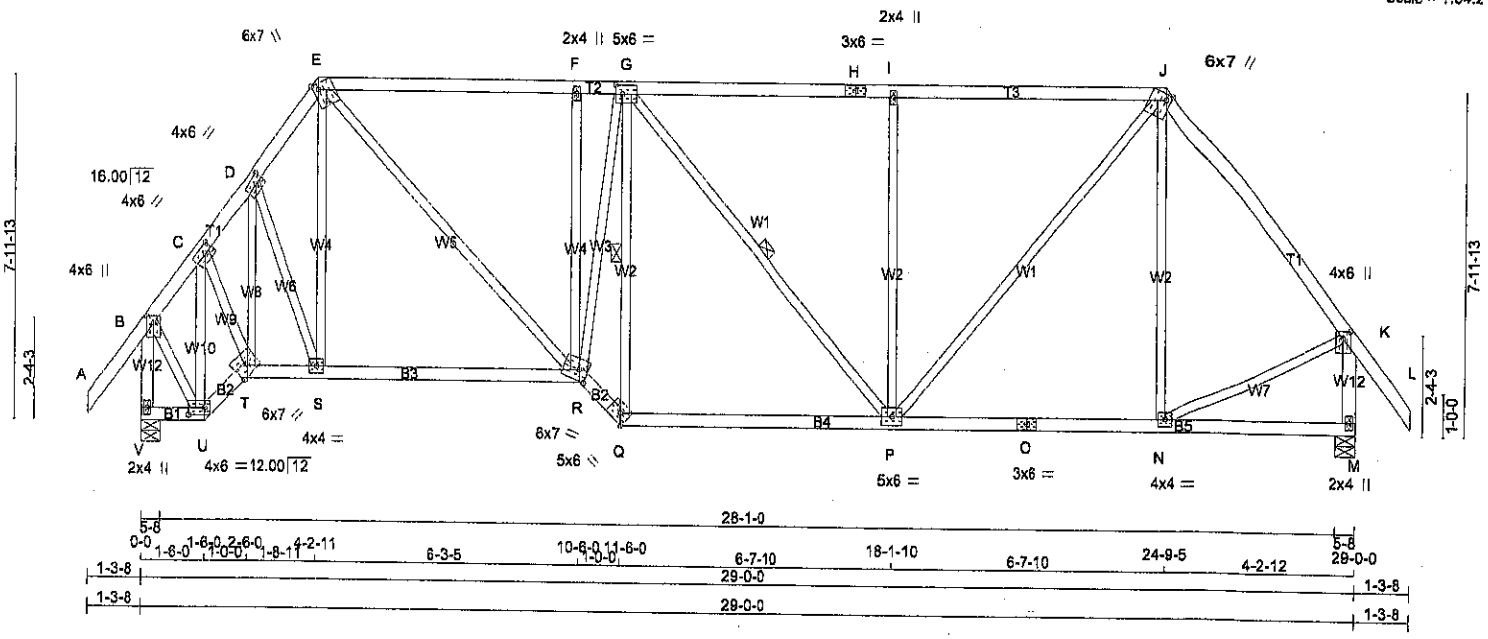

TOTAL WEIGHT = 159 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	6.0	2.50	2.00
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BBW-h	MT20	5.0	6.0	Edge	2.50
R	BBWWW-m	MT20	6.0	7.0	3.00	2.50
S	BMWW-t	MT20	4.0	4.0		
T	BBWW-h	MT20	6.0	7.0	Edge	
U	BBVW-l	MT20	4.0	6.0	2.00	4.50
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1377	0	1377	0	0	5-8	1-8
M	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
M	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF G-Q, G-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)		10.00
B- C	-784 / 0	-70.5	-70.5	0.10 (1)		6.25
C- D	-1098 / 0	-70.5	-70.5	0.04 (1)		6.00
D- E	-1177 / 0	-70.5	-70.5	0.04 (1)		5.85
E- F	-1313 / 0	-70.5	-70.5	0.38 (1)		5.19
F- G	-1308 / 0	-70.5	-70.5	0.33 (1)		5.21
G- H	-1175 / 0	-70.5	-70.5	0.52 (1)		5.18
H- I	-1175 / 0	-70.5	-70.5	0.52 (1)		5.18
I- J	-1174 / 0	-70.5	-70.5	0.52 (1)		5.18
J- K	-1024 / 0	-70.5	-70.5	0.26 (1)		5.90
K- L	0 / 40	-70.5	-70.5	0.10 (1)		10.00
V- B	-1364 / 0	0.0	0.0	0.17 (1)		6.95
M- K	-1346 / 0	0.0	0.0	0.17 (1)		6.99
V- U	0 / 0	-17.5	-17.5	0.01 (4)		10.00
U- T	0 / 585	-17.5	-17.5	0.10 (1)		10.00
T- S	0 / 673	-17.5	-17.5	0.21 (4)		10.00
S- R	0 / 695	-17.5	-17.5	0.21 (4)		10.00
R- Q	0 / 1642	-17.5	-17.5	0.27 (1)		10.00
Q- P	0 / 1176	-17.5	-17.5	0.29 (1)		10.00
P- O	0 / 608	-17.5	-17.5	0.24 (4)		10.00
O- N	0 / 608	-17.5	-17.5	0.24 (4)		10.00
N- M	0 / 0	-17.5	-17.5	0.11 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-958 / 0	0.25 (1)
C- T	0 / 693	0.16 (1)
T- D	-231 / 0	0.08 (1)
D- S	0 / 56	0.01 (4)
S- E	0 / 90	0.03 (4)
E- R	0 / 899	0.20 (1)
R- F	-324 / 0	0.27 (1)
R- G	0 / 901	0.20 (1)
Q- G	-1100 / 0	0.44 (1)
G- P	-2 / 6	0.00 (4)
P- I	-527 / 0	0.64 (1)
P- J	0 / 864	0.19 (1)
N- J	-173 / 23	0.21 (1)
B- U	0 / 717	0.16 (1)
N- K	0 / 670	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.52 (I-J:1) , BC=0.29 (P-Q:1) , WB=0.64 (I-P:1) , SSI=0.22 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.25 (V) (INPUT = 1.00)

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		QUANTITY	PLY	JOB DESC.	DRWG NO.
253460	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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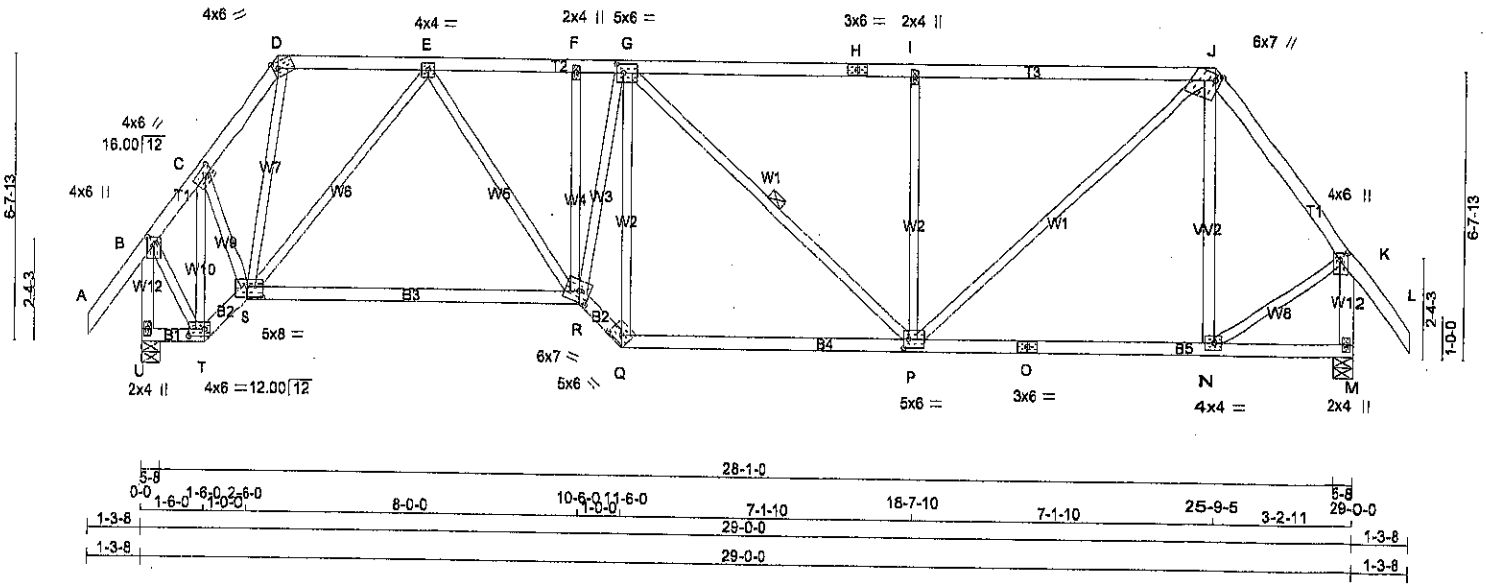

TOTAL WEIGHT = 146 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTW-m	MT20	4.0	6.0	Edge	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	6.0	2.50	2.00
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	6.0	2.50	1.50
Q	BBW-h	MT20	5.0	6.0	2.00	3.25
R	BBWWW-m	MT20	6.0	7.0	3.00	3.00
S	BBWWW-p	MT20	5.0	8.0	3.00	4.75
T	BBWW-l	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1377	0	1377	0	0	5-8	1-8
M	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
M	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
B-C	-784 / 0	-70.5	-70.5	0.10 (1)	6.25	
C-D	-1095 / 0	-70.5	-70.5	0.07 (1)	5.97	
D-E	-558 / 0	-70.5	-70.5	0.15 (1)	6.25	
E-F	-1593 / 0	-70.5	-70.5	0.18 (1)	5.06	
F-G	-1594 / 0	-70.5	-70.5	0.32 (1)	4.83	
G-H	-1404 / 0	-70.5	-70.5	0.64 (1)	4.66	
H-I	-1404 / 0	-70.5	-70.5	0.64 (1)	4.66	
I-J	-1404 / 0	-70.5	-70.5	0.64 (1)	4.66	
J-K	-993 / 0	-70.5	-70.5	0.15 (1)	6.10	
K-L	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
U-B	-1364 / 0	0.0	0.0	0.17 (1)	6.96	
M-K	-1359 / 0	0.0	0.0	0.17 (1)	6.96	
U-T	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
T-S	0 / 582	-17.5	-17.5	0.10 (1)	10.00	
S-R	0 / 1237	-17.5	-17.5	0.42 (4)	10.00	
R-Q	0 / 1990	-17.5	-17.5	0.32 (1)	10.00	
Q-P	0 / 1429	-17.5	-17.5	0.34 (1)	10.00	
P-O	0 / 588	-17.5	-17.5	0.27 (4)	10.00	
O-N	0 / 588	-17.5	-17.5	0.27 (4)	10.00	
N-M	0 / 0	-17.5	-17.5	0.11 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-954 / 0	0.25 (1)
C-S	0 / 661	0.15 (1)
S-D	0 / 712	0.16 (1)
S-E	-1072 / 0	0.95 (1)
E-R	0 / 631	0.14 (1)
R-F	0 / 64	0.01 (1)
R-G	0 / 920	0.21 (1)
Q-G	-1342 / 0	0.98 (1)
G-P	-34 / 0	0.02 (1)
P-I	-566 / 0	0.41 (1)
P-J	0 / 1080	0.25 (1)
N-J	-246 / 0	0.18 (1)
B-T	0 / 714	0.16 (1)
N-K	0 / 687	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.64 (I-J:1) , BC=0.42 (R-S:4) , WB=0.98 (G-Q:1) , SSI=0.24 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL = 0.39 (E) (INPUT = 1.00)

A-1073712

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253460	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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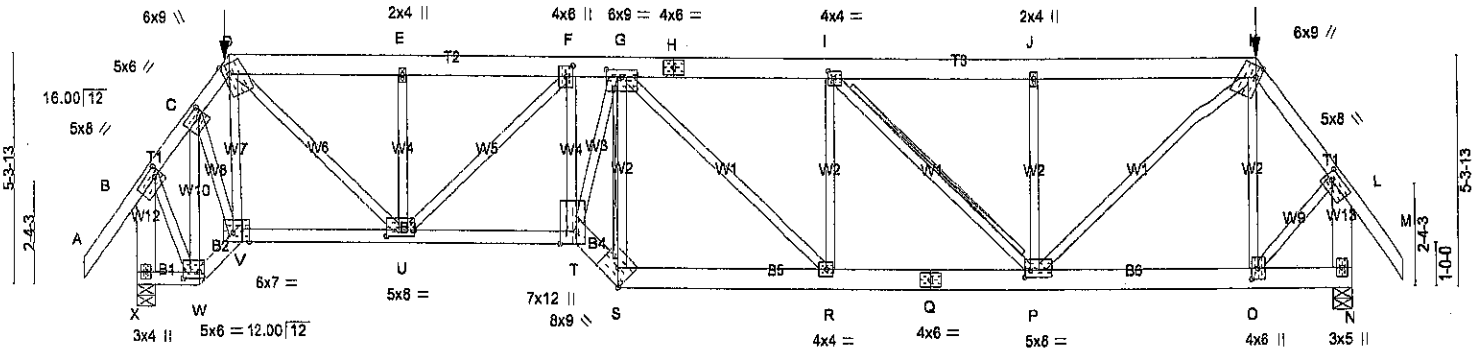
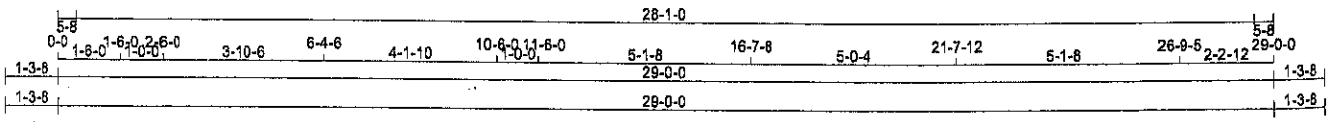




TOTAL WEIGHT = 173 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - W	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	1650F 1.5E	SPF
T - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	6.0	9.0	2.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW+t	MT20	4.0	6.0	3.00	1.75
G	TMWWW-t	MT20	6.0	9.0	2.00	4.50
H	TS-t	MT20	4.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMW+w	MT20	2.0	4.0		
K	TTWW+m	MT20	6.0	9.0	3.00	1.25
L	TMVW-t	MT20	5.0	8.0	2.25	2.00
N	BMV1+p	MT20	3.0	5.0		
O	BMWW+t	MT20	4.0	6.0	3.00	1.75
P	BMWWW-t	MT20	5.0	8.0	2.00	1.75
Q	BS-t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BBW-h	MT20	8.0	9.0	Edge	4.00
T	BBWW+p	MT20	7.0	12.0	Edge	4.50
U	BMWWW-t	MT20	5.0	8.0	1.75	3.50
V	BBWW-t	MT20	6.0	7.0	Edge	4.25
W	BBWW-t	MT20	5.0	6.0	1.75	4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2562	0	2562	0	0	5-8	2-13
N	2571	0	2571	0	0	5-8	2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1804	1229 / 0	0 / 0	0 / 0	0 / 0	575 / 0	0 / 0
N	1810	1234 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT G-S, I-P
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.11 (1)	10.00
B- C	-1531 / 0	-70.5	-70.5	0.14 (1)	5.15
C- D	-2234 / 0	-70.5	-70.5	0.11 (1)	4.46
D- E	-3245 / 0	-70.5	-70.5	0.15 (1)	4.58
E- F	-3245 / 0	-70.5	-70.5	0.24 (1)	4.47
F- G	-4335 / 0	-138.7	-138.7	0.29 (1)	3.91
G- H	-3731 / 0	-138.7	-138.7	0.29 (1)	4.18
H- I	-3731 / 0	-138.7	-138.7	0.29 (1)	4.18
I- J	-2916 / 0	-138.7	-138.7	0.29 (1)	4.61
J- K	-2916 / 0	-138.7	-138.7	0.30 (1)	4.61
K- L	-1873 / 0	-70.5	-70.5	0.12 (1)	4.79
L- M	0 / 40	-70.5	-70.5	0.11 (1)	10.00
X- B	-2536 / 0	0.0	0.0	0.24 (1)	6.47
N- L	-2581 / 0	0.0	0.0	0.23 (1)	6.42
X- W	0 / 0	-34.5	-34.5	0.02 (4)	10.00
W- V	0 / 1175	-34.5	-34.5	0.21 (1)	10.00
V- U	0 / 1331	-102.7	-102.7	0.40 (1)	10.00
U- T	0 / 4301	-102.7	-102.7	0.78 (1)	10.00
T- S	0 / 5009	-34.5	-34.5	0.65 (1)	10.00
S- R	0 / 3689	-34.5	-34.5	0.56 (1)	10.00
R- Q	0 / 3731	-34.5	-34.5	0.56 (1)	10.00
Q- P	0 / 3731	-34.5	-34.5	0.56 (1)	10.00
P- O	0 / 1107	-34.5	-34.5	0.23 (1)	10.00
O- N	0 / 0	-34.5	-34.5	0.09 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-1926 / 0	0.53 (1)
C- V	0 / 1419	0.35 (1)
D- V	-312 / 0	0.09 (1)
U- E	-366 / 0	0.11 (1)
U- F	-1478 / 0	0.79 (1)
T- F	0 / 1238	0.31 (1)
T- G	0 / 2607	0.65 (1)
S- G	-3328 / 0	0.73 (1)
G- R	0 / 59	0.01 (1)
R- I	0 / 175	0.07 (4)
I- P	-1135 / 0	0.41 (1)
P- J	-771 / 0	0.29 (1)
P- K	0 / 2490	0.62 (1)
O- K	-705 / 0	0.27 (1)
B- W	0 / 1440	0.36 (1)
O- L	0 / 1441	0.36 (1)
D- U	0 / 2641	0.65 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	2-2-11	-114	-114	--	FRONT	VERT	TOTAL
K	26-9-5	-115	-115	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 10-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 18-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

A-150374

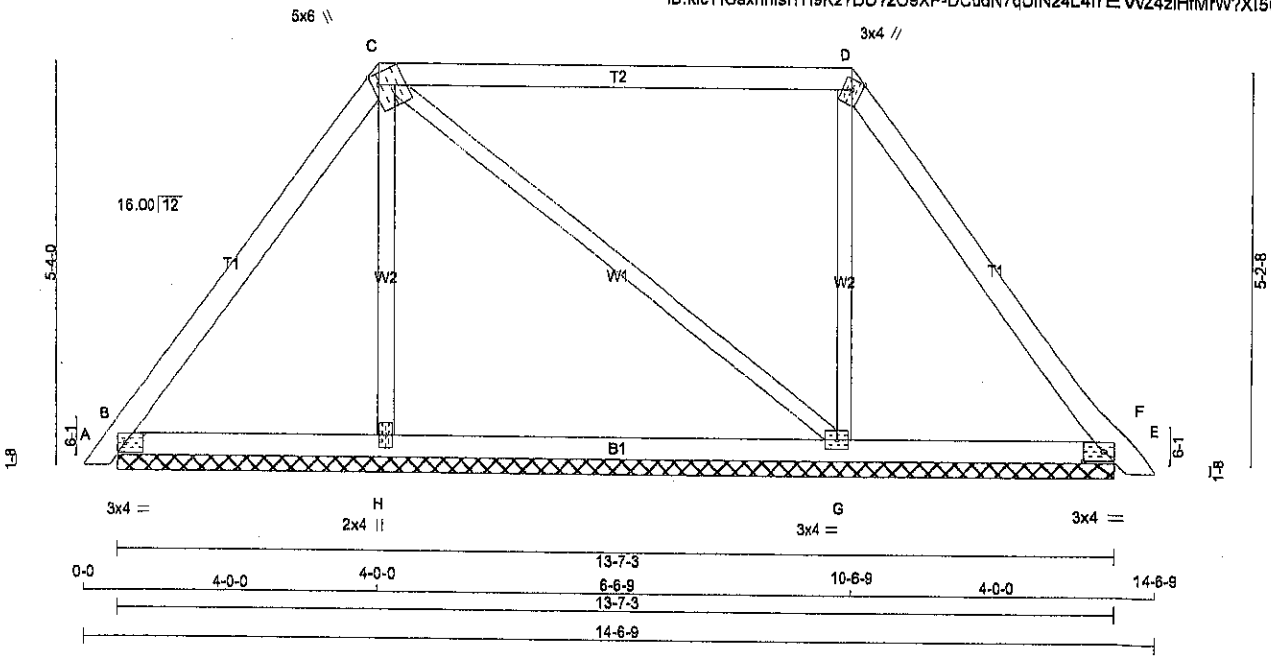
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253460	H09	2	1	TRUSS DESC.	

Alpe Roof Truss, Maple

ID:kleTIGsxnnlsITH9K2?DU?zO9XP-DCudN7qUfN24L4frEVWZ4zlHfMrW?XI5Gt0t13Ryxm4

Scale = 1:30.8



TOTAL WEIGHT = 2 X 52 = 104 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	6.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.50	2.75
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	296	0	296	0	0	13-7-3	1-8
E	261	0	261	0	0	13-7-3	1-8
H	307	0	307	0	0	13-7-3	1-8
G	382	0	382	0	0	13-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	203	165 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
E	180	145 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
H	221	123 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
G	272	168 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	
B- C	-159 / 0	-70.5	-70.5	0.15 (1)	6.25	
C- D	-62 / 0	-70.5	-70.5	0.52 (1)	6.25	
D- E	-116 / 0	-70.5	-70.5	0.15 (1)	6.25	
E- F	0 / 9	-70.5	-70.5	0.01 (1)	10.00	
B- H	0 / 93	-17.5	-17.5	0.12 (4)	10.00	
H- G	0 / 89	-17.5	-17.5	0.12 (4)	10.00	
G- E	0 / 88	-17.5	-17.5	0.12 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-206 / 0	0.08 (1)
C- G	-33 / 0	0.04 (1)
G- D	-261 / 0	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.52 (C-D:1) , BC=0.12 (B-H:4) , WB=0.10 (D-G:1) , SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (D) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-N073710

253445	H04T	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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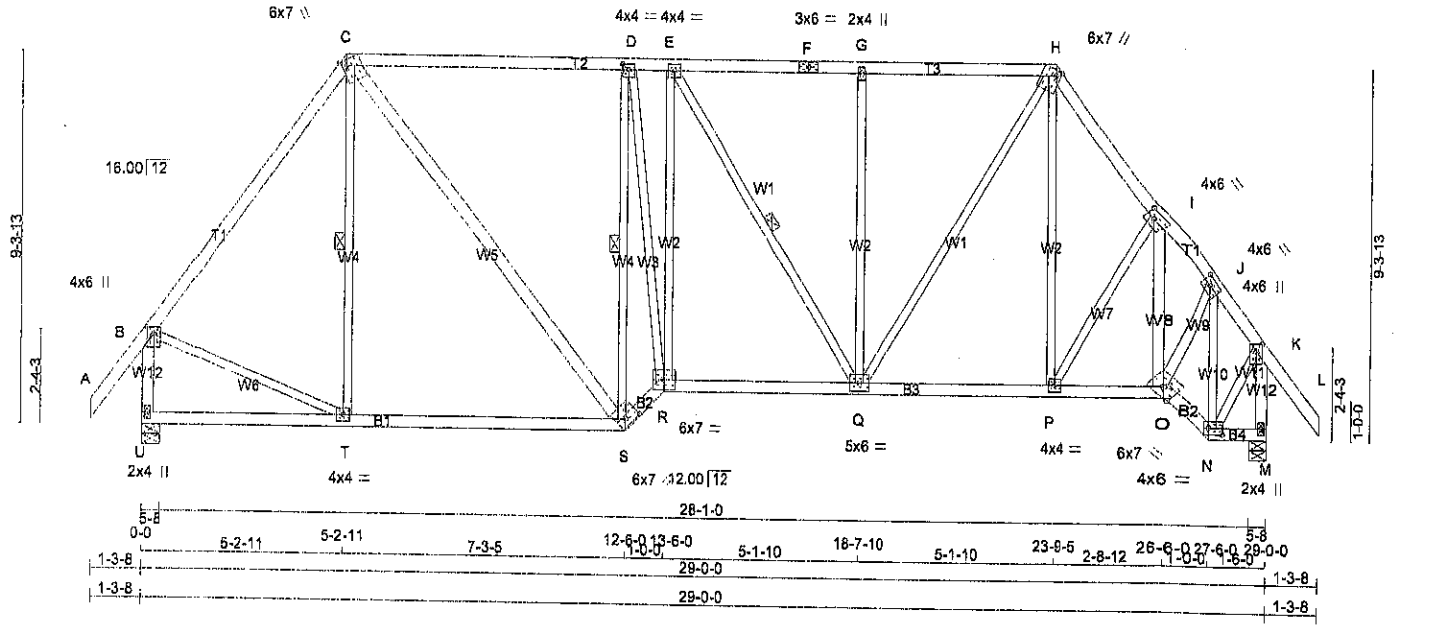

TOTAL WEIGHT = 172 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMWW-t	MT20	4.0	4.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	7.0	Edge	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-l	MT20	4.0	6.0	2.00	4.50
O	BBWW-h	MT20	6.0	7.0	Edge	
P	BMWW-t	MT20	4.0	4.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BBWW-l	MT20	6.0	7.0		
S	BBWW-h	MT20	6.0	7.0	2.00	4.25
T	BMWW-t	MT20	4.0	4.0		
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	1372	0	1372	0	0	5-8	1-8
M	1377	0	1377	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-T, D-S, E-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)		10.00
B- C	-1036 / 0	-70.5	-70.5	0.43 (1)		6.88
C- D	-1047 / 0	-70.5	-70.5	0.66 (1)		5.14
D- E	-1157 / 0	-70.5	-70.5	0.58 (1)		4.94
E- F	-1084 / 0	-70.5	-70.5	0.31 (1)		5.66
F- G	-1084 / 0	-70.5	-70.5	0.31 (1)		5.66
G- H	-1084 / 0	-70.5	-70.5	0.31 (1)		5.66
H- I	-1156 / 0	-70.5	-70.5	0.06 (1)		5.85
I- J	-1084 / 0	-70.5	-70.5	0.06 (1)		6.01
J- K	-784 / 0	-70.5	-70.5	0.10 (1)		6.25
K- L	0 / 40	-70.5	-70.5	0.10 (1)		10.00
U- B	-1342 / 0	0.0	0.0	0.17 (1)		7.00
M- K	-1364 / 0	0.0	0.0	0.17 (1)		6.95
U- T	0 / 0	-17.5	-17.5	0.21 (4)		10.00
T- S	0 / 618	-17.5	-17.5	0.26 (4)		10.00
S- R	0 / 1503	-17.5	-17.5	0.24 (1)		10.00
R- Q	0 / 1153	-17.5	-17.5	0.25 (1)		10.00
Q- P	0 / 682	-17.5	-17.5	0.17 (1)		10.00
P- O	0 / 680	-17.5	-17.5	0.15 (1)		10.00
O- N	0 / 582	-17.5	-17.5	0.10 (1)		10.00
N- M	0 / 0	-17.5	-17.5	0.01 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-92 / 79	0.05 (1)
C- S	0 / 681	0.11 (1)
S- D	-1530 / 0	0.87 (1)
D- R	0 / 828	0.19 (1)
R- E	0 / 292	0.07 (1)
E- Q	-130 / 0	0.08 (1)
Q- G	-420 / 0	0.57 (1)
Q- H	0 / 743	0.17 (1)
P- H	0 / 74	0.03 (4)
P- I	0 / 7	0.00 (4)
O- I	-222 / 0	0.07 (1)
O- J	0 / 715	0.16 (1)
N- J	-954 / 0	0.25 (1)
B- T	0 / 660	0.15 (1)
N- K	0 / 714	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.66 (C-D:1) , BC=0.26 (S-T:4) , WB=0.87 (D-S:1) , SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.25 (M) (INPUT = 1.00)

A-N073709

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253445	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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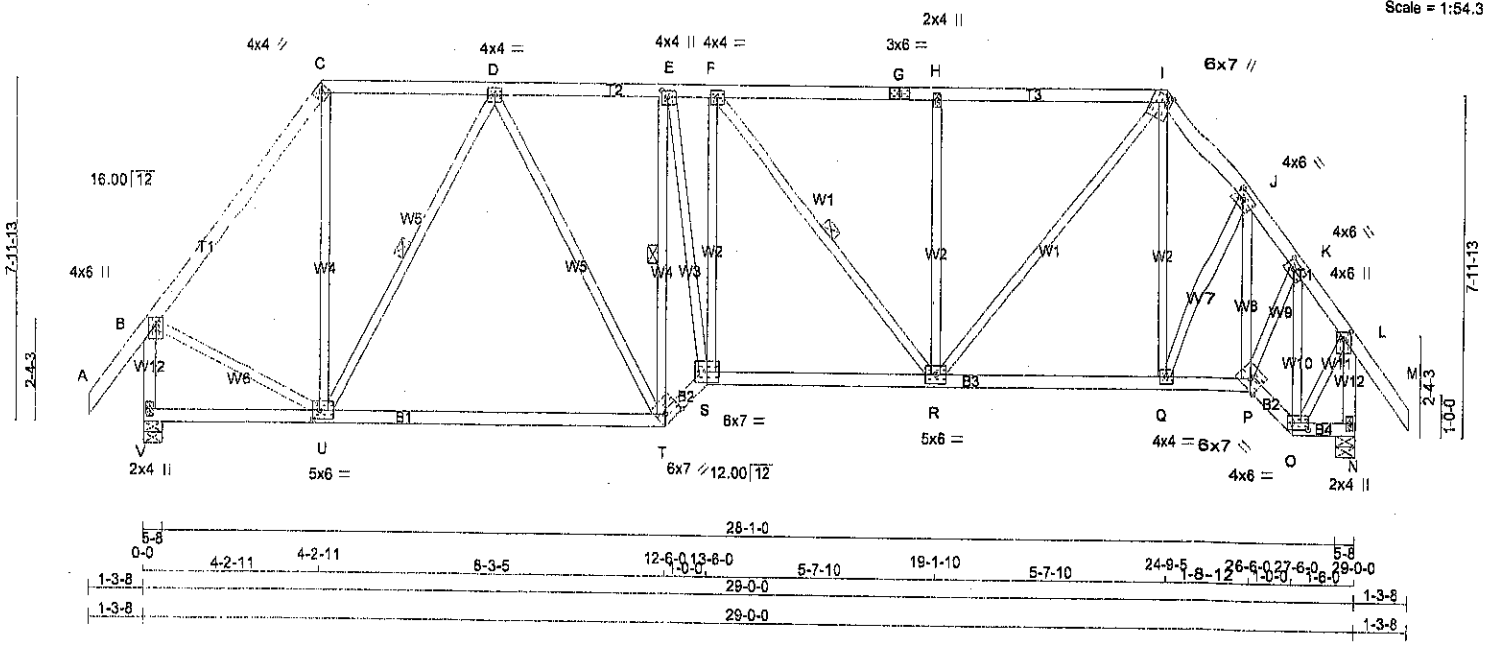

TOTAL WEIGHT = 162 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW-h	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMWW+t	MT20	4.0	4.0	1.75	1.60
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	1.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMWW-t	MT20	4.0	6.0	2.00	2.50
L	TMVW+p	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	2.0	4.0		
O	BBWW-t	MT20	4.0	6.0	2.00	4.50
P	BBWW-h	MT20	6.0	7.0	Edge	
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BBWW-t	MT20	6.0	7.0		
T	BBWW-h	MT20	6.0	7.0	2.25	4.50
U	BMWWW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1372	0	1372	0	0	5-8	1-8
N	1377	0	1377	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
N	966	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-U, E-T, F-R.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	10 (1)	0.10	10.00
B- C	-1035 / 0	-70.5	-70.5	6 (1)	0.26	5.87
C- D	-625 / 0	-70.5	-70.5	6 (1)	0.19	6.25
D- E	-1189 / 0	-70.5	-70.5	6 (1)	0.21	5.62
E- F	-1387 / 0	-70.5	-70.5	6 (1)	0.13	5.39
F- G	-1278 / 0	-70.5	-70.5	6 (1)	0.39	5.21
G- H	-1278 / 0	-70.5	-70.5	6 (1)	0.39	5.21
H- I	-1277 / 0	-70.5	-70.5	6 (1)	0.39	5.22
I- J	-1169 / 0	-70.5	-70.5	6 (1)	0.04	5.87
J- K	-1097 / 0	-70.5	-70.5	6 (1)	0.04	6.00
K- L	-784 / 0	-70.5	-70.5	6 (1)	0.10	6.25
L- M	0 / 40	-70.5	-70.5	10 (1)	0.10	10.00
V- B	-1358 / 0	0.0	0.0	6 (1)	0.17	6.96
N- L	-1364 / 0	0.0	0.0	6 (1)	0.17	6.95
V- U	0 / 0	-17.5	-17.5	4 (1)	0.25	10.00
U- T	0 / 991	-17.5	-17.5	4 (1)	0.33	10.00
T- S	0 / 1705	-17.5	-17.5	6 (1)	0.26	10.00
S- R	0 / 1388	-17.5	-17.5	6 (1)	0.29	10.00
R- Q	0 / 690	-17.5	-17.5	6 (1)	0.19	10.00
Q- P	0 / 673	-17.5	-17.5	6 (1)	0.15	10.00
P- O	0 / 585	-17.5	-17.5	6 (1)	0.10	10.00
O- N	0 / 0	-17.5	-17.5	6 (1)	0.01	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
U- C	0 / 561	0.13 (1)
U- D	-788 / 0	0.41 (1)
D- T	0 / 415	0.09 (1)
T- E	-1500 / 0	0.60 (1)
E- S	0 / 1335	0.30 (1)
S- F	-61 / 25	0.05 (1)
F- R	-173 / 0	0.09 (1)
R- H	-457 / 0	0.38 (1)
R- I	0 / 909	0.20 (1)
Q- I	0 / 60	0.02 (4)
Q- J	0 / 43	0.01 (1)
P- J	-214 / 0	0.07 (1)
P- K	0 / 694	0.16 (1)
O- K	-957 / 0	0.25 (1)
B- U	0 / 678	0.15 (1)
O- L	0 / 716	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.39 (H-I:1) , BC=0.33 (T-U:4) , WB=0.60 (E-T:1) , SSI=0.19 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

A-N073708

JOB NO.	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253445	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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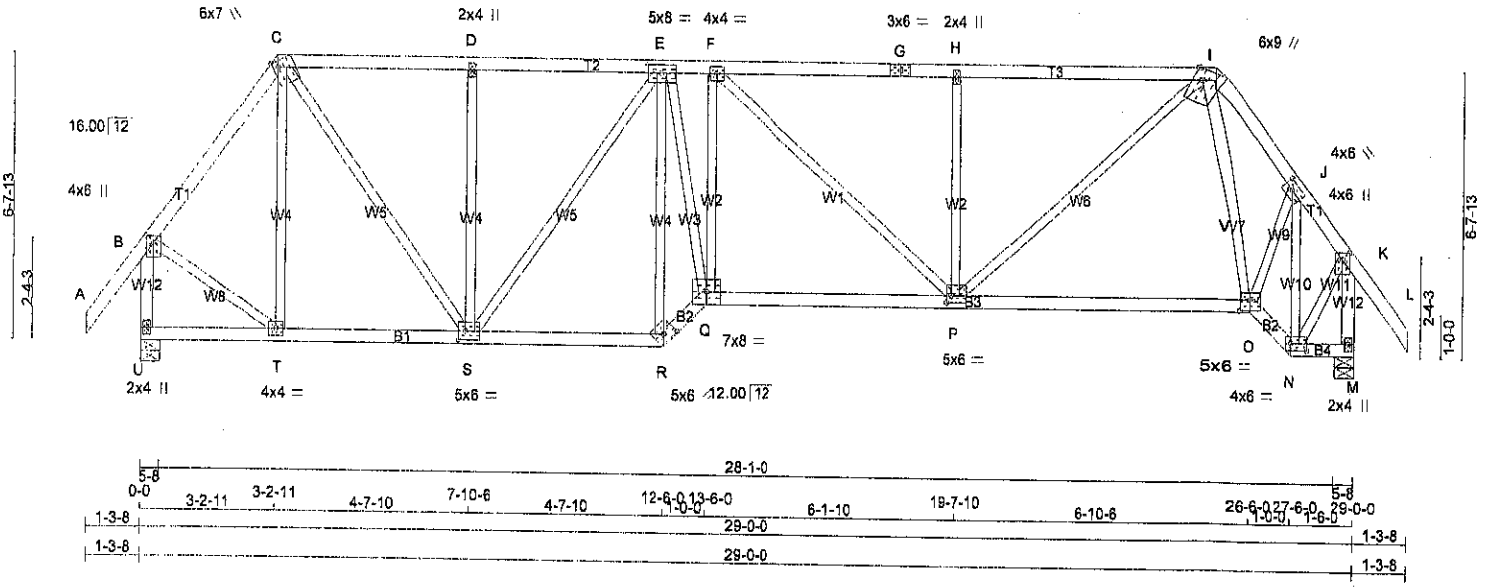


TOTAL WEIGHT = 149 lb [M]

LUMBER
N. L. G. A. RULES

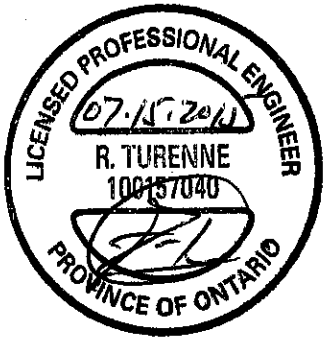
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
8	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	5.0	8.0	2.25	4.00
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-t	MT20	4.0	6.0	2.00	4.50
O	BBWW-t	MT20	5.0	6.0	3.00	3.00
P	BMWWW-t	MT20	5.0	6.0	2.00	1.50
Q	BBWW-t	MT20	7.0	8.0		
R	BBW-h	MT20	5.0	6.0	2.00	3.25
S	BMWWW-t	MT20	5.0	8.0	2.50	2.00
T	BMWW-t	MT20	4.0	4.0		
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1372	0	1372	0	0	5-8	1-8
M	1377	0	1377	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	964	666 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0
M	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-988 / 0	-70.5	-70.5	0.16 (1)	6.11
C- D	-1174 / 0	-70.5	-70.5	0.24 (1)	5.60
D- E	-1174 / 0	-70.5	-70.5	0.24 (1)	5.60
E- F	-1739 / 0	-70.5	-70.5	0.18 (1)	4.88
F- G	-1569 / 0	-70.5	-70.5	0.49 (1)	4.69
G- H	-1569 / 0	-70.5	-70.5	0.49 (1)	4.69
H- I	-1569 / 0	-70.5	-70.5	0.49 (1)	4.69
I- J	-1105 / 0	-70.5	-70.5	0.06 (1)	5.97
J- K	-782 / 0	-70.5	-70.5	0.10 (1)	6.25
K- L	0 / 40	-70.5	-70.5	0.10 (1)	10.00
U- B	-1352 / 0	0.0	0.0	0.17 (1)	6.97
M- K	-1364 / 0	0.0	0.0	0.17 (1)	6.95
U- T	0 / 0	-17.5	-17.5	0.06 (4)	10.00
T- S	0 / 564	-17.5	-17.5	0.14 (1)	10.00
S- R	0 / 1439	-17.5	-17.5	0.28 (1)	10.00
R- Q	0 / 2003	-17.5	-17.5	0.33 (1)	10.00
Q- P	0 / 1741	-17.5	-17.5	0.39 (1)	10.00
P- O	0 / 675	-17.5	-17.5	0.26 (4)	10.00
O- N	0 / 588	-17.5	-17.5	0.10 (1)	10.00
N- M	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-266 / 0	0.19 (1)
C- S	0 / 996	0.22 (1)
S- D	-357 / 0	0.26 (1)
S- E	-457 / 0	0.56 (1)
R- E	-1363 / 0	1.00 (1)
E- Q	0 / 1569	0.36 (1)
Q- F	-82 / 22	0.04 (1)
F- P	-230 / 0	0.31 (1)
P- H	-494 / 0	0.24 (1)
P- I	0 / 1183	0.27 (1)
I- O	-133 / 2	0.07 (1)
O- J	0 / 654	0.15 (1)
N- J	-964 / 0	0.25 (1)
B- T	0 / 663	0.15 (1)
N- K	0 / 721	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.49 (H-I:1) , BC=0.39 (P-Q:1) , WB=1.00 (E-R:1) , SSI=0.21 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

A-4073707

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253445	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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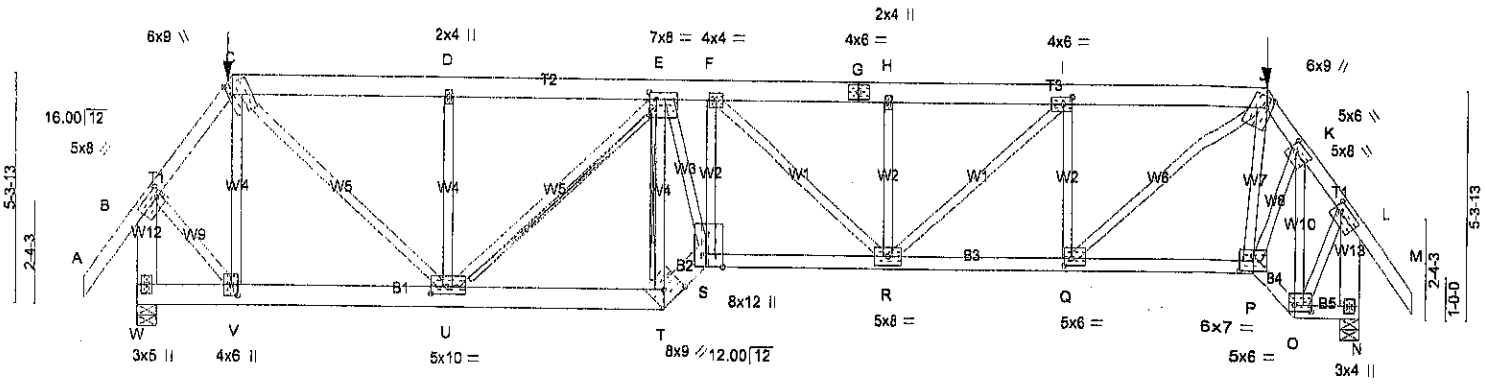

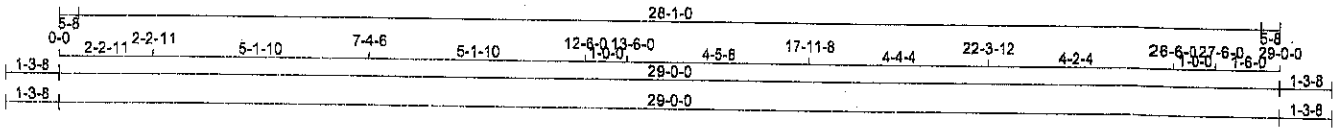

TOTAL WEIGHT = 169 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x8	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
S - P	2x4	DRY	1650F 1.5E	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TTWW+m	MT20	6.0	9.0	3.00	1.25
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	7.0	8.0	2.00	2.00
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	4.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TTWW+m	MT20	6.0	9.0	2.75	1.75
K	TMWW-t	MT20	5.0	6.0	2.25	1.75
L	TMVW-t	MT20	5.0	8.0	2.25	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BBVW-l	MT20	5.0	6.0	1.75	4.50
P	BBWW-l	MT20	6.0	7.0	4.25	3.50
Q	BMWW-t	MT20	5.0	6.0	2.00	2.00
R	BMWWW-t	MT20	5.0	8.0		
S	BBWW+p	MT20	6.0	12.0	Edge	4.75
T	BBW-h	MT20	8.0	9.0	3.75	5.00
U	BMWWW-t	MT20	5.0	10.0	2.00	5.00
V	BMWW+t	MT20	4.0	6.0	3.00	1.75
W	BMV1+p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2572	0	2572	0	0	5-8	2-14
N	2561	0	2561	0	0	5-8	2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0
N	1803	1228 / 0	0 / 0	0 / 0	0 / 0	575 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-U, E-T
FASTEN'T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.11 (1)	10.00
B- C	-1878 / 0	-70.5	-70.5	0.12 (1)	4.79
C- D	-2913 / 0	-138.7	-138.7	0.30 (1)	4.61
D- E	-2913 / 0	-138.7	-138.7	0.29 (1)	4.61
E- F	-4667 / 0	-138.7	-138.7	0.33 (1)	3.73
F- G	-4442 / 0	-70.5	-70.5	0.35 (1)	3.81
G- H	-4442 / 0	-70.5	-70.5	0.35 (1)	3.81
H- I	-4442 / 0	-70.5	-70.5	0.22 (1)	3.96
I- J	-3335 / 0	-70.5	-70.5	0.19 (1)	4.49
J- K	-2232 / 0	-70.5	-70.5	0.11 (1)	4.46
K- L	-1530 / 0	-70.5	-70.5	0.14 (1)	5.15
L- M	0 / 40	-70.5	-70.5	0.11 (1)	10.00
W- B	-2587 / 0	0.0	0.0	0.23 (1)	6.42
N- L	-2535 / 0	0.0	0.0	0.24 (1)	6.47
W- V	0 / 0	-34.5	-34.5	0.09 (1)	10.00
V- U	0 / 1110	-34.5	-34.5	0.24 (1)	10.00
U- T	0 / 3773	-34.5	-34.5	0.68 (1)	10.00
T- S	0 / 5123	-34.5	-34.5	0.67 (1)	10.00
S- R	0 / 4665	-102.7	-102.7	0.82 (1)	10.00
R- Q	0 / 3336	-102.7	-102.7	0.65 (1)	10.00
Q- P	0 / 1332	-102.7	-102.7	0.38 (1)	10.00
P- O	0 / 1174	-34.5	-34.5	0.21 (1)	10.00
O- N	0 / 0	-34.5	-34.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-693 / 0	0.26 (1)
C- U	0 / 2480	0.61 (1)
U- D	-769 / 0	0.29 (1)
U- E	-1194 / 0	0.44 (1)
T- E	-3400 / 0	0.74 (1)
E- S	0 / 3516	0.87 (1)
S- F	0 / 464	0.11 (1)
F- R	-286 / 0	0.17 (1)
R- H	-407 / 0	0.11 (1)
R- I	0 / 1494	0.37 (1)
Q- I	-1302 / 0	0.36 (1)
Q- J	0 / 2671	0.66 (1)
P- J	-288 / 0	0.08 (1)
O- K	-1924 / 0	0.53 (1)
B- V	0 / 1444	0.36 (1)
O- L	0 / 1439	0.36 (1)
P- K	0 / 1419	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	—	FRONT	VERT	TOTAL
J	26-9-5	-115	-115	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 13-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 13-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 26-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , 8CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 789 (0.44")

A-4273706

CONTINUED ON PAGE 2

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253445	H01T	1	1	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11, AND 114.5 lbs FACTORED DOWN AT 26-9-5 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

CSI: TC=0.35 (F-H:1), BC=0.82 (R-S:1), WB=0.87 (E-S:1), SSI=0.34 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.69 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/15/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-NO7 3706(2)

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253444	H12	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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3x5 II

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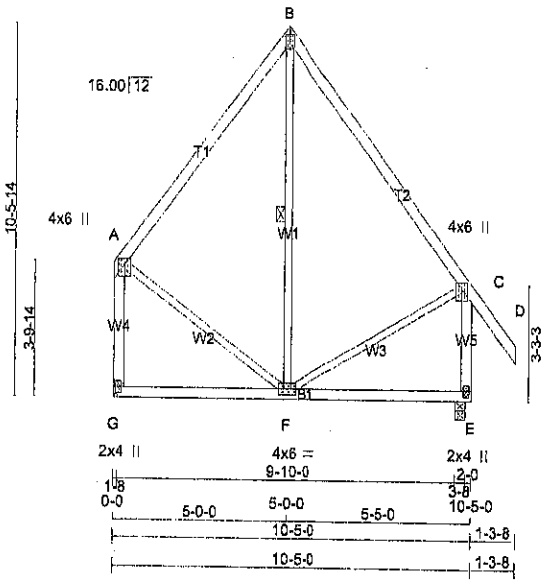

TOTAL WEIGHT = 3 X 60 = 179 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	458	0	458	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	557	0	557	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	323	219 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0
E	390	278 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF B-F.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-206 / 0	-70.5	-70.5	0.23 (1)	6.25		F- B	-37 / 74	0.03 (1)
B- C	-206 / 0	-70.5	-70.5	0.27 (1)	6.25		A- F	0 / 150	0.03 (1)
C- D	0 / 40	-70.5	-70.5	0.10 (1)	10.00		F- C	0 / 140	0.03 (1)
G- A	-426 / 0	0.0	0.0	0.11 (1)	7.81				
E- C	-520 / 0	0.0	0.0	0.10 (1)	7.81				
G- F	0 / 0	-17.5	-17.5	0.14 (4)	10.00				
F- E	0 / 0	-17.5	-17.5	0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.27 (B-C:1) , BC=0.14 (F-G:4) , WB=0.03 (A-F:1) , SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (C) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

A-N073705

253444	H11	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 17:00:15 2015 Page 1

ID:kieTlGsxnnlsITH9K2?DU?zO9XP-gjGUw2ympNPD2wBumAsi1SsObYCiDP8HE2LQKAyxmmL

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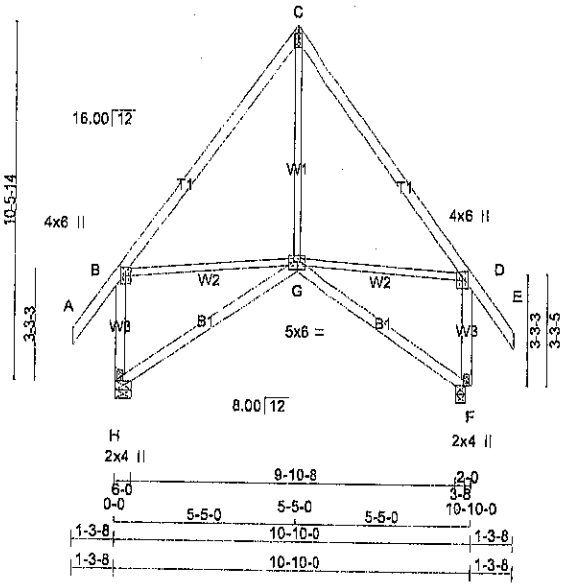

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	576	0	576	0	0	6-0	1-8
F	576	0	576	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	403	286 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0
F	403	286 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00		G- C	0 / 125	0.04 (4)
B- C	-317 / 0	-70.5	-70.5	0.27 (1)	6.25		B- G	0 / 191	0.04 (1)
C- D	-317 / 0	-70.5	-70.5	0.27 (1)	6.25		G- D	0 / 191	0.04 (1)
D- E	0 / 40	-70.5	-70.5	0.10 (1)	10.00				
H- B	-528 / 0	0.0	0.0	0.11 (1)	7.81				
F- D	-528 / 0	0.0	0.0	0.11 (1)	7.81				
H- G	0 / 0	-17.5	-17.5	0.15 (4)	10.00				
G- F	0 / 0	-17.5	-17.5	0.15 (4)	10.00				

TOTAL WEIGHT = 2 X 62 = 124 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.27 (C-D:1) , BC=0.15 (F-G:4) , WB=0.04 (B-G:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (H) (INPUT = 0.90)
JSI METAL= 0.13 (H) (INPUT = 1.00)

A-150737O4

253444	H10S	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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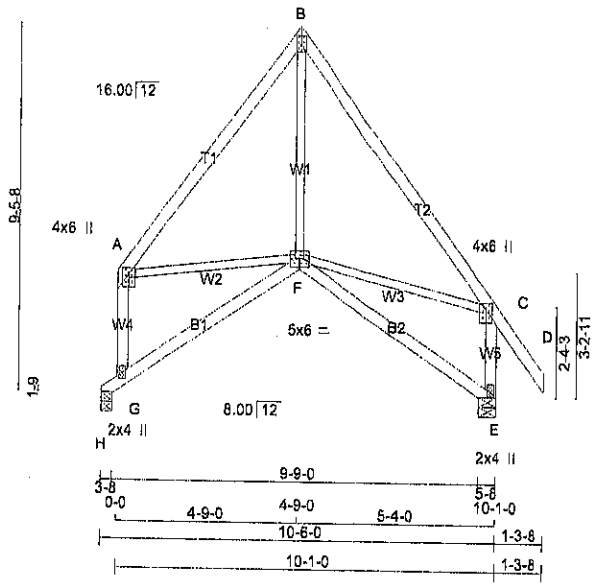

TOTAL WEIGHT = 55 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0	Edge	
F	BBWWW-p	MT20	5.0	6.0	2.75	3.00
G	BMV+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	433	0	433	0	0	3-8	1-8
E	559	0	559	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	306	204 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
E	392	279 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-345 / 0	-70.5	-70.5	0.29 (1)	6.25	
B- C	-345 / 0	-70.5	-70.5	0.36 (1)	6.25	
C- D	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
G- A	-421 / 0	0.0	0.0	0.07 (1)	7.81	
E- C	-513 / 0	0.0	0.0	0.07 (1)	7.81	
H- G	-239 / 0	-17.5	-17.5	0.23 (1)	6.25	
G- F	0 / 20	-17.5	-17.5	0.24 (1)	10.00	
F- E	0 / 0	-17.5	-17.5	0.15 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	0 / 186	0.05 (4)
A- F	0 / 208	0.05 (1)
F- C	0 / 213	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.36 (B-C:1) , BC=0.24 (F-G:1) , WB=0.05 (C-F:1) , SSI=0.28 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (E) (INPUT = 0.90)
JSI METAL= 0.12 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157848
PROVINCE OF ONTARIO

A-1073703

253444	H10	6	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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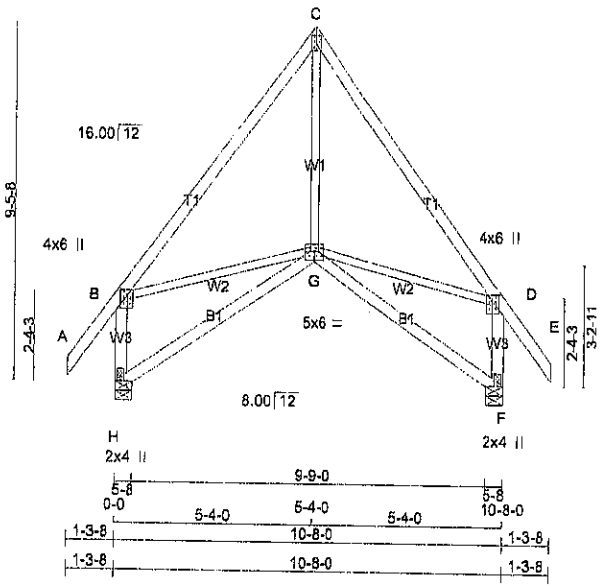

TOTAL WEIGHT = 6 X 59 = 351 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	568	0	568	0	0	5-8	1-8
F	568	0	568	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	398	283 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	398	283 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)		10.00	G- C	0 / 197	0.05 (4)
B- C	-358 / 0	-70.5	-70.5	0.26 (1)		6.25	B- G	0 / 221	0.05 (1)
C- D	-358 / 0	-70.5	-70.5	0.26 (1)		6.25	G- D	0 / 221	0.05 (1)
D- E	0 / 40	-70.5	-70.5	0.10 (1)		10.00			
H- B	-522 / 0	0.0	0.0	0.07 (1)		7.81			
F- D	-522 / 0	0.0	0.0	0.07 (1)		7.81			
H- G	0 / 0	-17.5	-17.5	0.15 (4)		10.00			
G- F	0 / 0	-17.5	-17.5	0.15 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.26 (C-D:1) , BC=0.15 (G-H:4) , WB=0.05 (D-G:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	522	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (H) (INPUT = 0.90)
JSI METAL= 0.12 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-150737O2

253444	H01-Cond2		2	1	
Alpa Roof Truss, Maple				TRUSS DESC.	DRWG NO.

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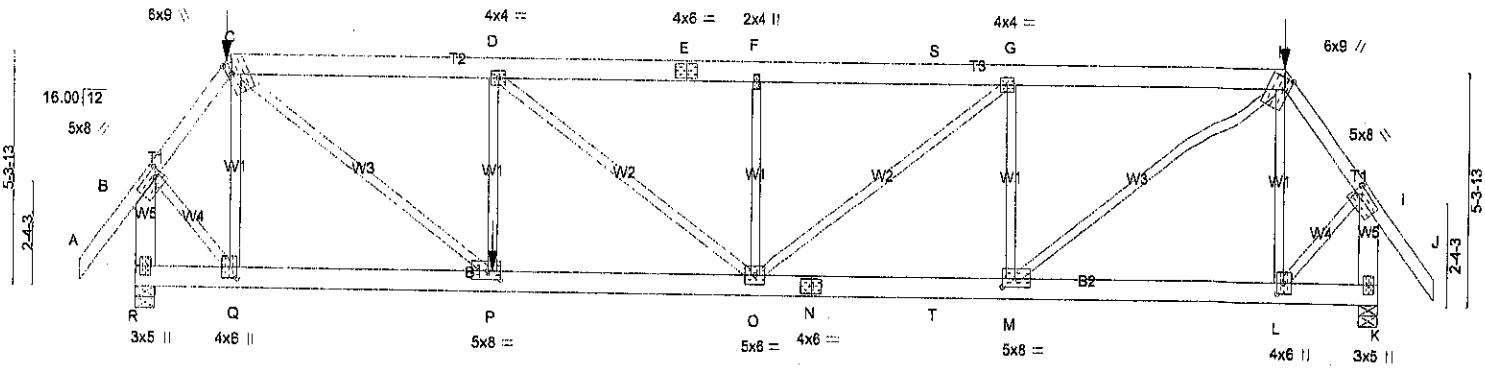

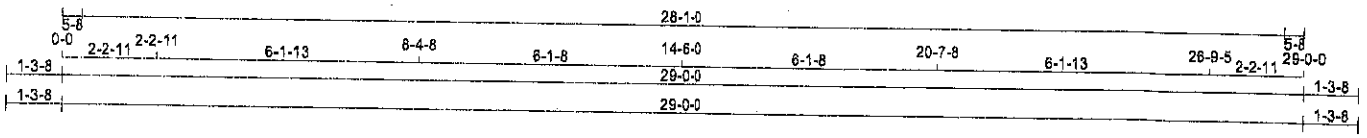

TOTAL WEIGHT = 2 X 164 = 327 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TTWW+m	MT20	6.0	9.0	3.00	1.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	3.00	1.25
I	TMVW-t	MT20	5.0	8.0	2.25	2.00
K	BMV1+p	MT20	3.0	5.0		
L	BMWW+t	MT20	4.0	6.0	3.00	1.75
M	BMWW-t	MT20	5.0	8.0	2.25	3.50
N	BS-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.25	3.50
Q	BMWW+t	MT20	4.0	6.0	3.00	1.75
R	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2670	0	2670	0	0	5-8	3-3
K	2386	0	2386	0	0	5-8	2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1879	1281 / 0	0 / 0	0 / 0	0 / 0	598 / 0	0 / 0
K	1680	1146 / 0	0 / 0	0 / 0	0 / 0	534 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 40	-70.5	-70.5	1	0.11 (1)	10.00	Q- C	-705 / 0	0.27 (1)
B- C	-1965 / 0	-70.5	-70.5	1	0.12 (1)	4.69	C- P	0 / 2771	0.69 (1)
C- D	-3337 / 0	-138.7	-138.7	1	0.46 (1)	4.16	P- D	-792 / 0	0.30 (1)
D- E	-3395 / 0	-70.5	-70.5	1	0.38 (1)	4.24	D- O	0 / 74	0.02 (1)
E- F	-3395 / 0	-70.5	-70.5	1	0.38 (1)	4.24	O- F	-348 / 0	0.13 (1)
F- S	-3395 / 0	-70.5	-70.5	1	0.43 (1)	4.17	O- G	0 / 670	0.17 (1)
S- G	-3395 / 0	-138.7	-138.7	1	0.43 (1)	4.17	M- G	-1287 / 0	0.49 (1)
G- H	-2870 / 0	-138.7	-138.7	1	0.41 (1)	4.49	M- H	0 / 2348	0.58 (1)
H- I	-1736 / 0	-70.5	-70.5	1	0.11 (1)	4.94	L- H	-616 / 0	0.23 (1)
I- J	0 / 40	-70.5	-70.5	1	0.11 (1)	10.00	B- Q	0 / 1512	0.37 (1)
R- B	-2699 / 0	0.0	0.0	1	0.24 (1)	6.30	L- I	0 / 1335	0.33 (1)
K- I	-2405 / 0	0.0	0.0	1	0.21 (1)	6.61			
R- Q	0 / 0	-34.5	-34.5	4	0.13 (4)	10.00			
Q- P	0 / 1162	-34.5	-34.5	1	0.27 (1)	10.00			
P- O	0 / 3337	-17.5	-17.5	1	0.49 (1)	10.00			
O- N	0 / 2870	-17.5	-17.5	1	0.43 (1)	10.00			
N- T	0 / 2870	-17.5	-17.5	1	0.43 (1)	10.00			
T- M	0 / 2870	-34.5	-34.5	1	0.43 (1)	10.00			
M- L	0 / 1027	-34.5	-34.5	1	0.24 (1)	10.00			
L- K	0 / 0	-34.5	-34.5	1	0.12 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
H	26-9-5	-114	-114	---	FRONT	VERT	TOTAL
P	8-4-8	-802	-802	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-2-0 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 6-4-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.46 (C-D:1) , BC=0.49 (O-P:1) , WB=0.69 (C-P:1) , SSI=0.33 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

A-1073206

CONTINUED ON PAGE 2

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253444	H01-Cond2	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 26-9-5, AND 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD, AND 801.9 lbs FACTORED DOWN AT 8-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.68 (N) (INPUT = 1.00)

A-150737012)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253444	H01-Cond1	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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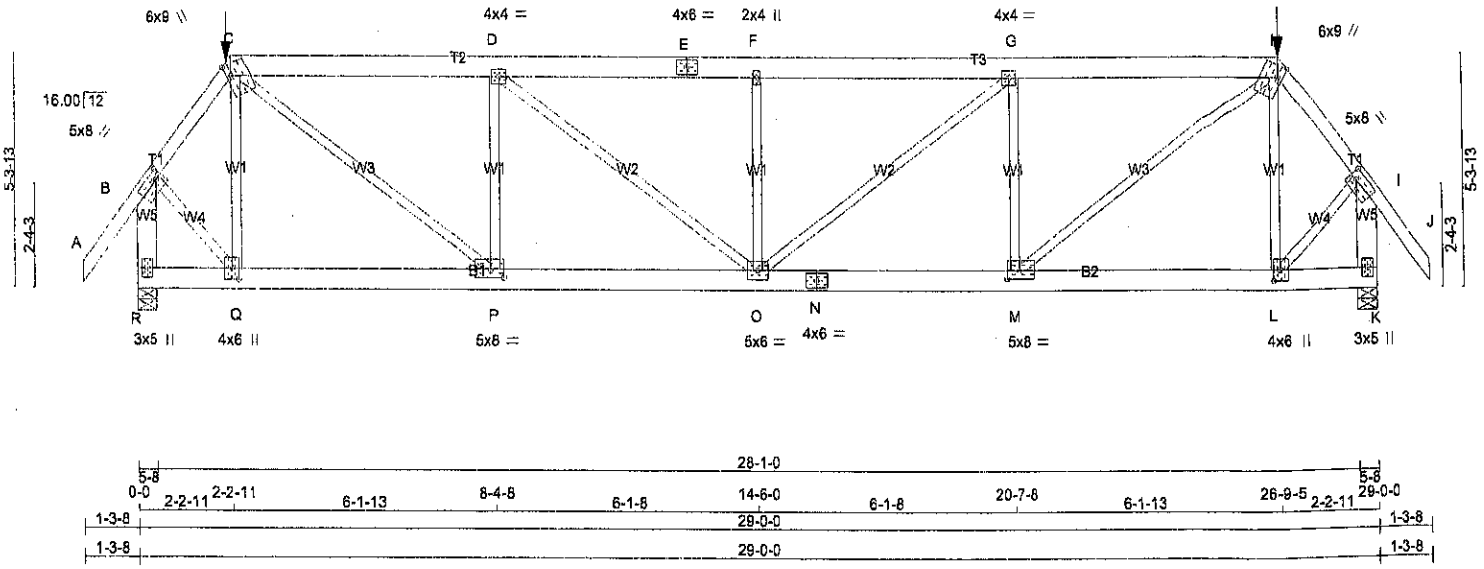

TOTAL WEIGHT = 2 X 164 = 327 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TTWW+m	MT20	6.0	9.0	3.00	1.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	3.00	1.25
I	TMVW-t	MT20	5.0	8.0	2.25	2.00
K	BMV1+p	MT20	3.0	5.0		
L	BMWW+t	MT20	4.0	6.0	3.00	1.75
M	BMWW-t	MT20	5.0	8.0	2.25	3.50
N	BS-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.25	3.50
Q	BMWW+t	MT20	4.0	6.0	3.00	1.75
R	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2572	0	2572	0	0	5-8	2-14
K	2572	0	2572	0	0	5-8	2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0
K	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	Q-C	-689 / 0	0.26 (1)
B-C	-1882 / 0	-70.5	-70.5	0.12 (1)	4.78	C-P	0 / 2669	0.66 (1)
C-D	-3207 / 0	-138.7	-138.7	0.46 (1)	4.22	P-D	-1458 / 0	0.55 (1)
D-E	-3838 / 0	-138.7	-138.7	0.50 (1)	3.87	D-O	0 / 804	0.20 (1)
E-F	-3838 / 0	-138.7	-138.7	0.50 (1)	3.87	O-F	-790 / 0	0.30 (1)
F-G	-3838 / 0	-138.7	-138.7	0.50 (1)	3.87	O-G	0 / 804	0.20 (1)
G-H	-3207 / 0	-138.7	-138.7	0.46 (1)	4.22	M-G	-1458 / 0	0.55 (1)
H-I	-1882 / 0	-70.5	-70.5	0.12 (1)	4.78	M-H	0 / 2669	0.66 (1)
I-J	0 / 40	-70.5	-70.5	0.11 (1)	10.00	L-H	-689 / 0	0.26 (1)
R-B	-2592 / 0	0.0	0.0	0.23 (1)	6.41	B-Q	0 / 1447	0.36 (1)
K-I	-2592 / 0	0.0	0.0	0.23 (1)	6.41	L-I	0 / 1447	0.36 (1)
R-Q	0 / 0	-34.5	-34.5	0.12 (4)	10.00			
Q-P	0 / 1113	-34.5	-34.5	0.25 (1)	10.00			
P-O	0 / 3207	-34.5	-34.5	0.50 (1)	10.00			
O-N	0 / 3207	-34.5	-34.5	0.50 (1)	10.00			
N-M	0 / 3207	-34.5	-34.5	0.50 (1)	10.00			
M-L	0 / 1113	-34.5	-34.5	0.25 (1)	10.00			
L-K	0 / 0	-34.5	-34.5	0.12 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
H	26-9-5	-114	-114	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.50 (D-F:1) , BC=0.50 (M-O:1) , WB=0.66 (H-M:1) , SSI=0.34 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.68 (N) (INPUT = 1.00)

A-N073700

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H06T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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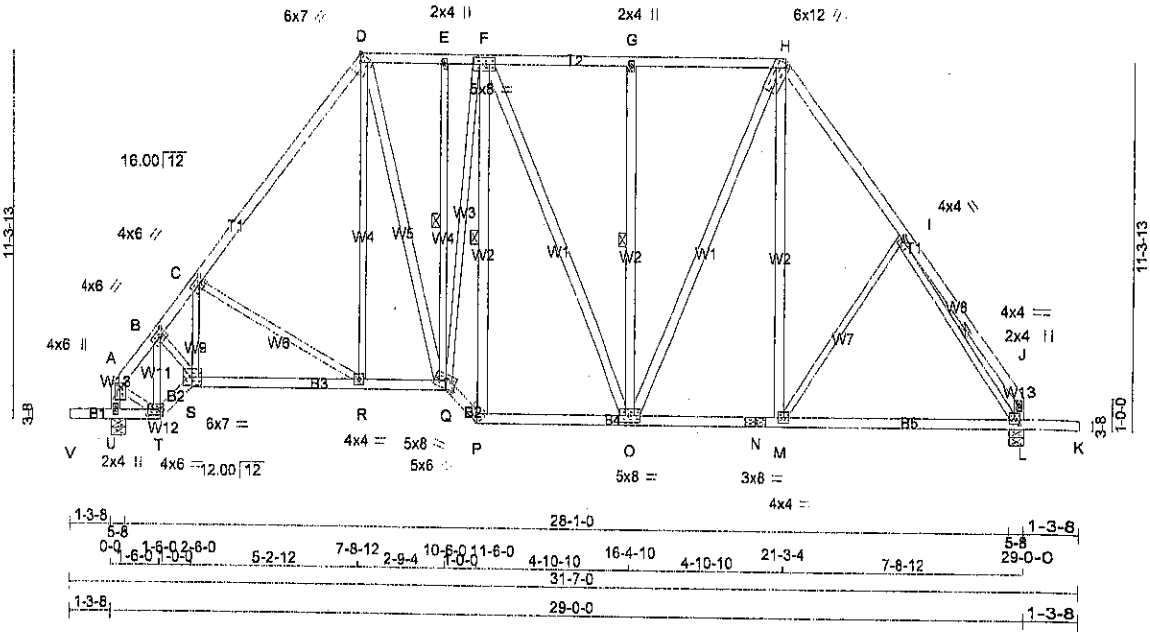

TOTAL WEIGHT = 205 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	Edge	2.75
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	12.0	Edge	1.50
I	TMWW-t	MT20	4.0	4.0	1.50	1.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.50	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BBW-h	MT20	5.0	6.0	Edge	2.50
Q	BBWWW-m	MT20	5.0	8.0	2.75	4.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-Q, F-P, G-O, I-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	-1117 / 0	-70.5	-70.5	0.08 (1)		5.92
B- C	-1554 / 0	-70.5	-70.5	0.22 (1)		5.07
C- D	-1236 / 0	-70.5	-70.5	0.28 (1)		5.51
D- E	-860 / 0	-70.5	-70.5	0.09 (1)		6.25
E- F	-860 / 0	-70.5	-70.5	0.12 (1)		6.25
F- G	-839 / 0	-70.5	-70.5	0.19 (1)		6.25
G- H	-838 / 0	-70.5	-70.5	0.19 (1)		6.25
H- I	-1135 / 0	-70.5	-70.5	0.15 (1)		5.81
I- J	0 / 25	-70.5	-70.5	0.17 (1)		10.00
U- A	-1216 / 0	0.0	0.0	0.12 (1)		7.26
L- J	-103 / 0	0.0	0.0	0.01 (1)		7.81
V- U	0 / 0	-88.0	-88.0	0.10 (1)		10.00
U- T	0 / 0	-17.5	-17.5	0.08 (1)		10.00
T- S	0 / 869	-17.5	-17.5	0.14 (1)		10.00
S- R	0 / 1022	-17.5	-17.5	0.22 (1)		10.00
R- Q	0 / 724	-17.5	-17.5	0.17 (1)		10.00
Q- P	0 / 1136	-17.5	-17.5	0.19 (1)		10.00
P- O	0 / 814	-17.5	-17.5	0.19 (1)		10.00
O- N	0 / 666	-17.5	-17.5	0.28 (4)		10.00
N- M	0 / 666	-17.5	-17.5	0.28 (4)		10.00
M- L	0 / 743	-17.5	-17.5	0.28 (4)		10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-801 / 0	0.15 (1)
B- S	0 / 682	0.15 (1)
S- C	0 / 124	0.03 (4)
C- R	-350 / 0	0.23 (1)
R- D	0 / 258	0.08 (1)
D- Q	0 / 470	0.08 (1)
Q- E	-25 / 11	0.02 (1)
Q- F	0 / 404	0.09 (1)
P- F	-751 / 0	0.50 (1)
F- O	0 / 64	0.01 (1)
O- G	-378 / 0	0.25 (1)
O- H	0 / 425	0.07 (1)
M- H	0 / 250	0.04 (1)
M- I	-138 / 0	0.13 (1)
A- T	0 / 702	0.16 (1)
I- L	-1338 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28 (C-D:1) , BC=0.28 (M-O:4) , WB=0.50 (F-P:1) , SSI=0.16 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (H) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

A-50736 99

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H05T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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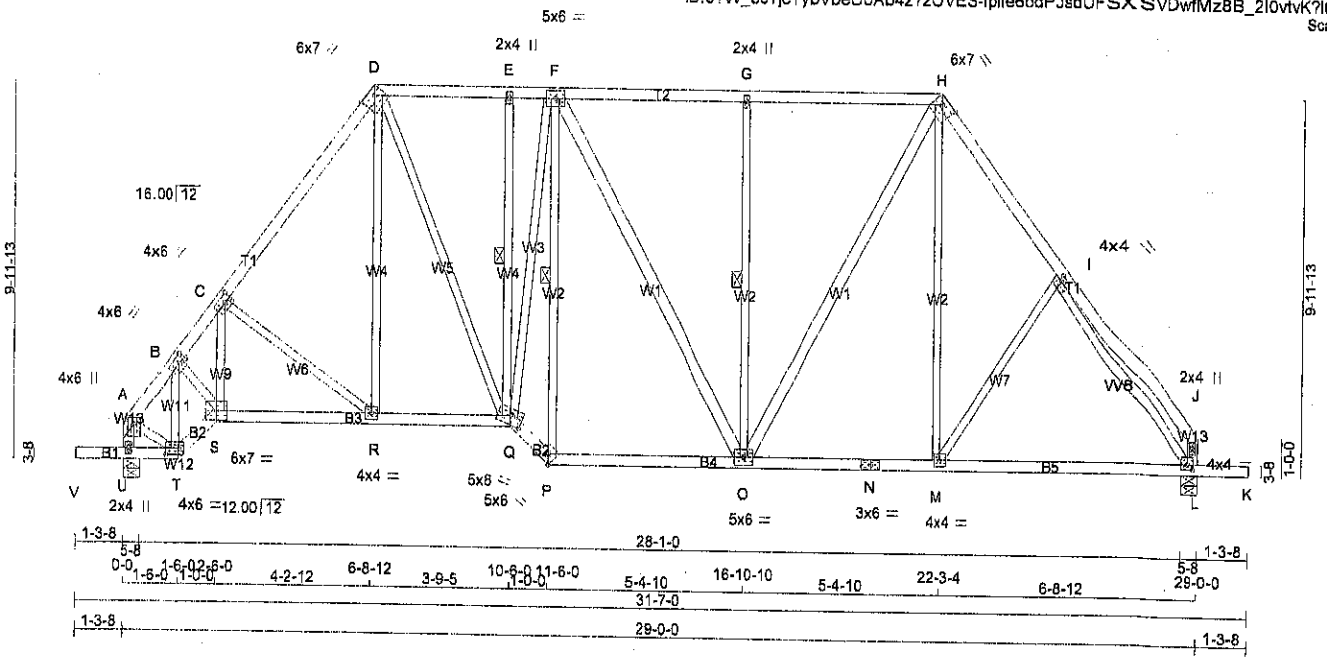


TOTAL WEIGHT = 178 lb

[M][F]

LUMBER

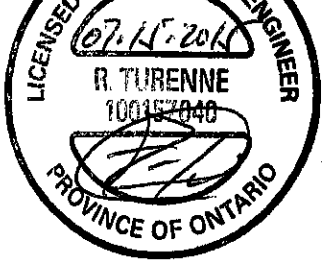
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	2.00	4.25
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-h	MT20	6.0	7.0	2.00	4.25
I	TMWW-t	MT20	4.0	4.0	1.50	1.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.50	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BBW-h	MT20	5.0	6.0	Edge	2.50
Q	BBWWW-m	MT20	5.0	8.0	2.75	4.00
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-I	MT20	6.0	7.0		
T	BBWW-I	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-Q, F-P, G-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1119 / 0	-70.5	-70.5	0.07 (1)	5.93	
B- C	-1575 / 0	-70.5	-70.5	0.14 (1)	5.13	
C- D	-1291 / 0	-70.5	-70.5	0.17 (1)	5.52	
D- E	-986 / 0	-70.5	-70.5	0.14 (1)	6.13	
E- F	-985 / 0	-70.5	-70.5	0.15 (1)	6.12	
F- G	-950 / 0	-70.5	-70.5	0.23 (1)	6.09	
G- H	-950 / 0	-70.5	-70.5	0.24 (1)	6.08	
H- I	-1173 / 0	-70.5	-70.5	0.12 (1)	5.79	
I- J	0 / 21	-70.5	-70.5	0.12 (1)	10.00	
U- A	-1216 / 0	0.0	0.0	0.12 (1)	7.26	
L- J	-91 / 0	0.0	0.0	0.01 (1)	7.81	
V- U	0 / 0	-88.0	-88.0	0.10 (1)	10.00	
U- T	0 / 0	-17.5	-17.5	0.08 (1)	10.00	
T- S	0 / 677	-17.5	-17.5	0.14 (1)	10.00	
S- R	0 / 998	-17.5	-17.5	0.21 (1)	10.00	
R- Q	0 / 760	-17.5	-17.5	0.17 (1)	10.00	
Q- P	0 / 1298	-17.5	-17.5	0.21 (1)	10.00	
P- O	0 / 927	-17.5	-17.5	0.21 (1)	10.00	
O- N	0 / 689	-17.5	-17.5	0.23 (4)	10.00	
N- M	0 / 689	-17.5	-17.5	0.23 (4)	10.00	
M- L	0 / 744	-17.5	-17.5	0.23 (4)	10.00	
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-910 / 0	0.15 (1)
B- S	0 / 635	0.14 (1)
S- C	0 / 160	0.04 (1)
C- R	-299 / 0	0.14 (1)
R- D	0 / 251	0.06 (1)
D- Q	0 / 564	0.13 (1)
Q- E	-90 / 2	0.05 (1)
Q- F	0 / 534	0.12 (1)
P- F	-884 / 0	0.58 (1)
F- O	0 / 48	0.01 (1)
O- G	-421 / 0	0.29 (1)
O- H	0 / 635	0.09 (1)
M- H	0 / 203	0.05 (4)
M- I	-99 / 4	0.07 (1)
A- T	0 / 709	0.16 (1)
I- L	-1355 / 0	0.85 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24 (G-H:1) , BC=0.23 (M-O:4) , WB=0.85 (I-L:1) , SSI=0.18 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

A-15073698

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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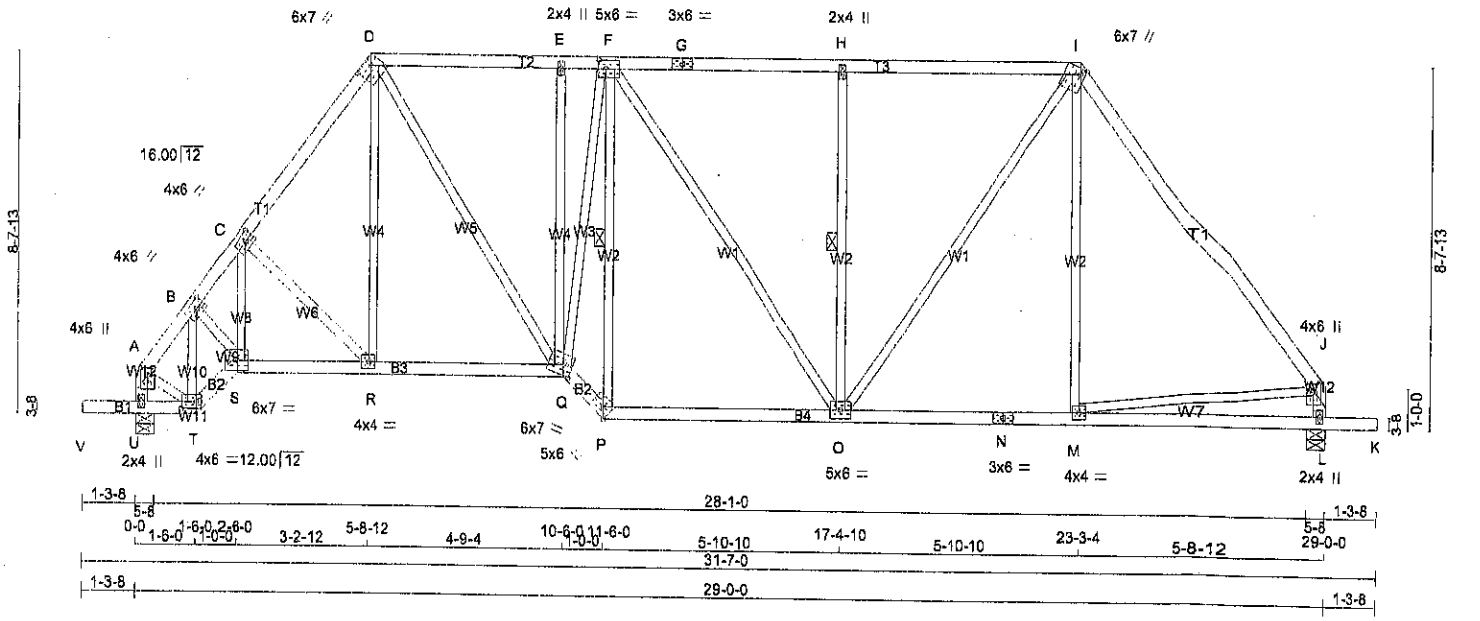


TOTAL WEIGHT = 156 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	1.75	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	6.0	6.0	2.50	2.00
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	1.50
J	TMVW+p	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	6.0	6.0		
P	BBW-h	MT20	5.0	6.0	Edge	2.50
Q	BBWWW-m	MT20	6.0	7.0	3.00	2.75
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-l	MT20	6.0	7.0		
T	BBWW-l	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-P, H-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1120 / 0	-70.5	-70.5	0.06 (1)	6.94
B- C	-1591 / 0	-70.5	-70.5	0.10 (1)	5.15
C- D	-1350 / 0	-70.5	-70.5	0.11 (1)	5.48
D- E	-1175 / 0	-70.5	-70.5	0.23 (1)	5.62
E- F	-1173 / 0	-70.5	-70.5	0.22 (1)	5.62
F- G	-1095 / 0	-70.5	-70.5	0.39 (1)	5.52
G- H	-1095 / 0	-70.5	-70.5	0.39 (1)	5.52
H- I	-1095 / 0	-70.5	-70.5	0.39 (1)	5.52
I- J	-1189 / 0	-70.5	-70.5	0.57 (1)	5.26
U- A	-1216 / 0	0.0	0.0	0.12 (1)	7.26
L- J	-1232 / 0	0.0	0.0	0.12 (1)	7.23
V- U	0 / 0	-88.0	-88.0	0.10 (1)	10.00
U- T	0 / 0	-17.5	-17.5	0.10 (1)	10.00
T- S	0 / 884	-17.5	-17.5	0.15 (1)	10.00
S- R	0 / 981	-17.5	-17.5	0.21 (1)	10.00
R- Q	0 / 799	-17.5	-17.5	0.16 (1)	10.00
Q- P	0 / 1506	-17.5	-17.5	0.25 (1)	10.00
P- O	0 / 1078	-17.5	-17.5	0.25 (1)	10.00
O- N	0 / 711	-17.5	-17.5	0.19 (1)	10.00
N- M	0 / 711	-17.5	-17.5	0.19 (1)	10.00
M- L	0 / 0	-17.5	-17.5	0.14 (4)	10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-917 / 0	0.16 (1)
B- S	0 / 600	0.13 (1)
S- C	0 / 183	0.04 (1)
C- R	-257 / 0	0.08 (1)
R- D	0 / 252	0.06 (1)
D- Q	0 / 686	0.15 (1)
Q- E	-175 / 0	0.19 (1)
Q- F	0 / 722	0.16 (1)
P- F	-1008 / 0	0.48 (1)
F- O	0 / 30	0.01 (1)
O- H	-463 / 0	0.22 (1)
O- I	0 / 664	0.15 (1)
M- I	0 / 100	0.03 (4)
A- T	0 / 714	0.16 (1)
M- J	0 / 715	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.57 (I-J:1) , BC=0.25 (O-P:1) , WB=0.48 (F-P:1) , SSI=0.20 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (M) (INPUT = 0.90)
JSI METAL= 0.27 (J) (INPUT = 1.00)

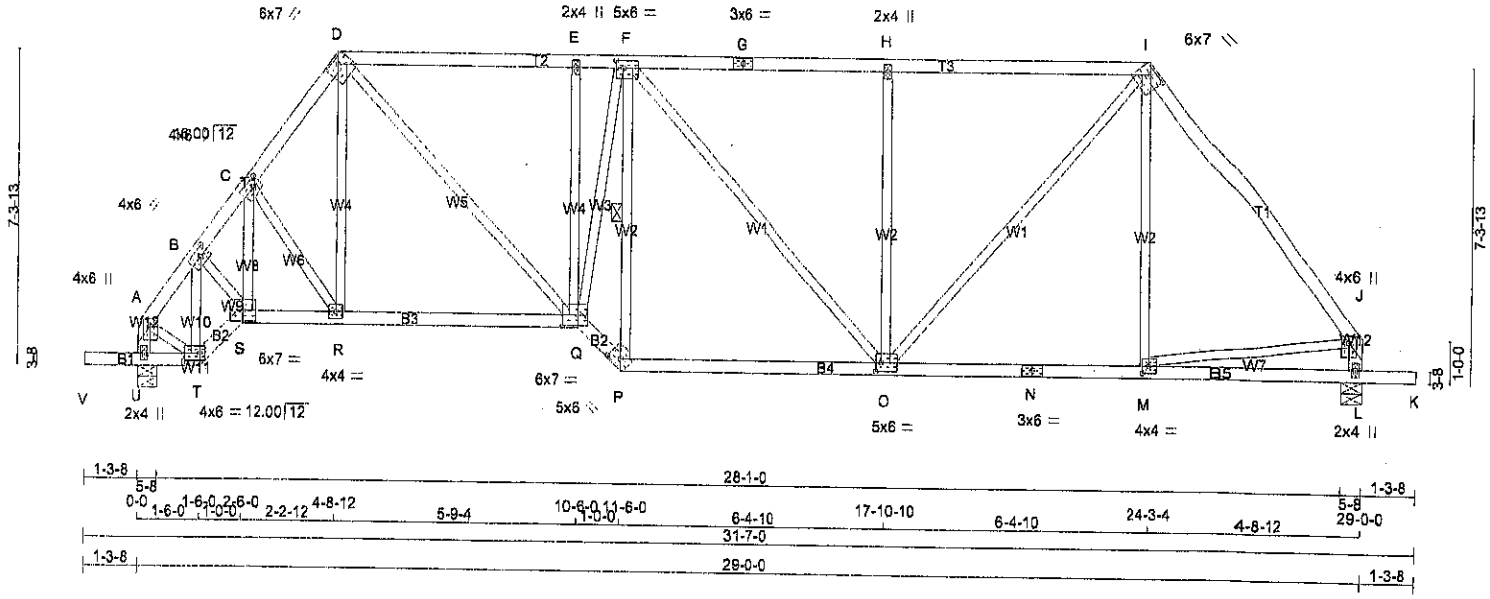
A-H073697

	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-MQdvDQmNticwFxI8L4BSaxukrAMmY5DcshG?CryxnSc

Scale = 1:53.9



TOTAL WEIGHT = 145 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	6.0	7.0	1.50	3.75
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	6.0	2.00	2.00
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	6.0	7.0	1.50	3.75
J	TMVW+p	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BBW-h	MT20	5.0	6.0	2.00	3.25
Q	BBWWW-p	MT20	6.0	7.0	3.00	4.25
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-t	MT20	6.0	7.0		
T	BBWW-t	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	-1121 / 0	-70.5	-70.5	0.05 (1)	5.94
B-C	-1603 / 0	-70.5	-70.5	0.05 (1)	5.19
C-D	-1420 / 0	-70.5	-70.5	0.07 (1)	5.42
D-E	-1450 / 0	-70.5	-70.5	0.33 (1)	5.05
E-F	-1446 / 0	-70.5	-70.5	0.27 (1)	5.11
F-G	-1292 / 0	-70.5	-70.5	0.49 (1)	5.05
G-H	-1292 / 0	-70.5	-70.5	0.49 (1)	5.05
H-I	-1292 / 0	-70.5	-70.5	0.49 (1)	5.05
I-J	-1221 / 0	-70.5	-70.5	0.36 (1)	5.43
U-A	-1216 / 0	0.0	0.0	0.12 (1)	7.26
L-J	-1241 / 0	0.0	0.0	0.12 (1)	7.21
V-U	0 / 0	-88.0	-88.0	0.10 (1)	10.00
U-T	0 / 0	-17.5	-17.5	0.10 (1)	10.00
T-S	0 / 888	-17.5	-17.5	0.15 (1)	10.00
S-R	0 / 968	-17.5	-17.5	0.23 (1)	10.00
R-Q	0 / 844	-17.5	-17.5	0.21 (1)	10.00
Q-P	0 / 1793	-17.5	-17.5	0.29 (1)	10.00
P-O	0 / 1286	-17.5	-17.5	0.29 (1)	10.00
O-N	0 / 728	-17.5	-17.5	0.23 (4)	10.00
N-M	0 / 728	-17.5	-17.5	0.23 (4)	10.00
M-L	0 / 0	-17.5	-17.5	0.12 (4)	10.00
L-K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-B	-921 / 0	0.16 (1)
B-S	0 / 574	0.13 (1)
S-C	0 / 190	0.04 (1)
C-R	-222 / 0	0.06 (1)
R-D	0 / 265	0.06 (1)
D-Q	0 / 871	0.20 (1)
Q-E	-271 / 0	0.17 (1)
Q-F	0 / 983	0.22 (1)
P-F	-1207 / 0	0.41 (1)
F-O	0 / 10	0.00 (4)
O-H	-505 / 0	0.48 (1)
O-I	0 / 834	0.19 (1)
M-I	0 / 85	0.03 (4)
A-T	0 / 718	0.16 (1)
M-J	0 / 735	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.49 (H-I:1) , BC=0.29 (O-P:1) , WB=0.48 (H-O:1) , SSI=0.21 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (F) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

A-1073696

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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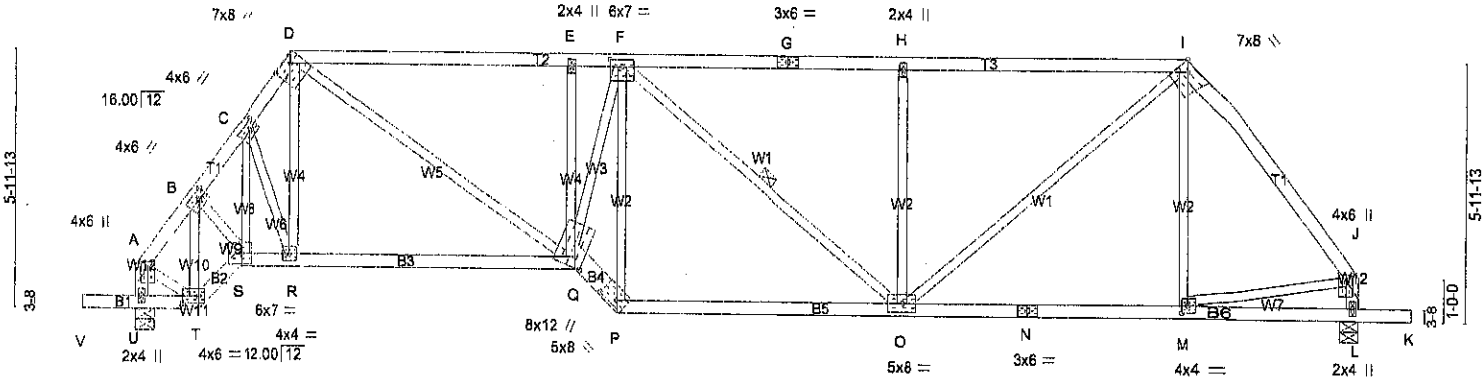

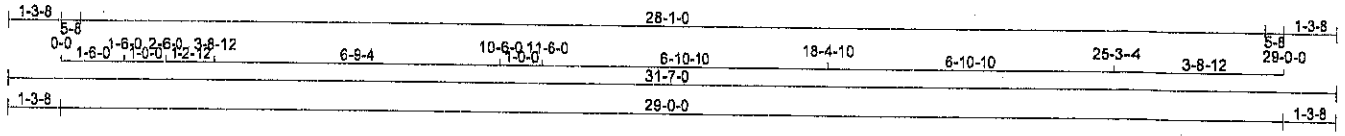

TOTAL WEIGHT = 136 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-h	MT20	7.0	8.0	1.75	5.00
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	6.0	7.0	2.00	3.50
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	7.0	8.0	Edge	2.75
J	TMVW+p	MT20	4.0	6.0	1.75	1.75
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	6.0	8.0		
P	BBW-h	MT20	6.0	8.0	1.75	5.00
Q	BBWWW+m	MT20	8.0	12.0	3.00	2.50
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-l	MT20	6.0	7.0		
T	BBWW-l	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1391	0	1391	0	0	5-8	1-8
L	1386	0	1386	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1122 / 0	-70.5	-70.5	0.04 (1)		5.95
B- C	-1612 / 0	-70.5	-70.5	0.06 (1)		5.17
C- D	-1508 / 0	-70.5	-70.5	0.05 (1)		5.32
D- E	-1910 / 0	-70.5	-70.5	0.52 (1)		4.35
E- F	-1902 / 0	-70.5	-70.5	0.41 (1)		4.35
F- G	-1578 / 0	-70.5	-70.5	0.61 (1)		4.50
G- H	-1578 / 0	-70.5	-70.5	0.61 (1)		4.50
H- I	-1578 / 0	-70.5	-70.5	0.62 (1)		4.50
I- J	-1245 / 0	-70.5	-70.5	0.21 (1)		5.54
U- A	-1216 / 0	0.0	0.0	0.12 (1)		7.26
L- J	-1252 / 0	0.0	0.0	0.13 (1)		7.18
V- U	0 / 0	-88.0	-88.0	0.10 (1)		10.00
U- T	0 / 0	-17.5	-17.5	0.10 (1)		10.00
T- S	0 / 892	-17.5	-17.5	0.15 (1)		10.00
S- R	0 / 959	-17.5	-17.5	0.27 (1)		10.00
R- Q	0 / 898	-17.5	-17.5	0.27 (4)		10.00
Q- P	0 / 2236	-17.5	-17.5	0.27 (1)		10.00
P- O	0 / 1608	-17.5	-17.5	0.36 (1)		10.00
O- N	0 / 742	-17.5	-17.5	0.26 (4)		10.00
N- M	0 / 742	-17.5	-17.5	0.26 (4)		10.00
M- L	0 / 0	-17.5	-17.5	0.11 (4)		10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-925 / 0	0.16 (1)
B- S	0 / 555	0.12 (1)
S- C	0 / 166	0.04 (1)
C- R	-188 / 0	0.04 (1)
R- D	0 / 288	0.06 (1)
D- Q	0 / 1221	0.27 (1)
Q- E	-376 / 0	0.14 (1)
Q- F	0 / 1380	0.31 (1)
P- F	-1502 / 0	0.84 (1)
F- O	-39 / 0	0.02 (1)
O- H	-546 / 0	0.30 (1)
O- I	0 / 1081	0.24 (1)
M- I	-20 / 72	0.02 (4)
A- T	0 / 721	0.16 (1)
M- J	0 / 752	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.62 (H-I:1) , BC=0.36 (O-P:1) , WB=0.84 (F-P:1) , SSI=0.23 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.38 (Q) (INPUT = 1.00)

A-1507695

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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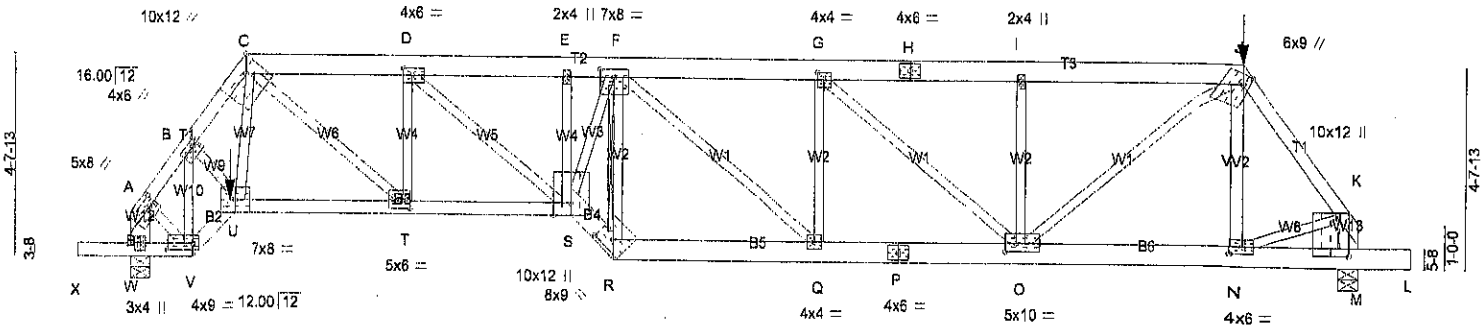

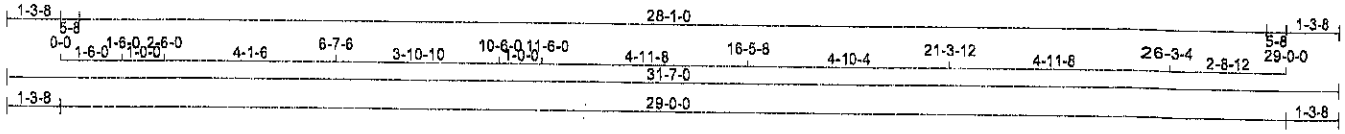

TOTAL WEIGHT = 157 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
W - A	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.25	2.00
B	TMWW-t	MT20	4.0	6.0	1.60	1.75
C	TTWW-h	MT20	10.0	12.0	Edge	4.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	7.0	8.0	2.25	4.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TS-t	MT20	4.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	9.0	2.75	1.75
K	TMBMVW*+p	MT20	10.0	12.0	10.25	3.00
N	BMWW-t	MT20	4.0	6.0	2.00	2.75
O	BMWWW-t	MT20	5.0	10.0	2.25	3.25
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BBW-h	MT20	6.0	9.0	3.00	4.50
S	BBWWW+p	MT20	10.0	12.0	Edge	
T	BMWW-t	MT20	5.0	6.0	2.00	2.00
U	BBWW-I	MT20	7.0	8.0		
V	BBWW-I	MT20	4.0	9.0	1.75	7.25
W	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2676	0	2676	0	0	5-8	3-0
M	2582	0	2582	0	0	5-8	2-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1862	1298 / 0	0 / 0	0 / 0	0 / 0	584 / 0	0 / 0
M	1820	1228 / 0	0 / 0	0 / 0	0 / 0	592 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT F-R
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-2320 / 0	-70.5	-70.5	0.14 (1)	4.35
B- C	-3324 / 0	-70.5	-70.5	0.21 (1)	3.86
C- D	-4045 / 0	-70.5	-70.5	0.18 (1)	4.15
D- E	-5237 / 0	-70.5	-70.5	0.43 (1)	3.44
E- F	-5249 / 0	-138.7	-138.7	0.42 (1)	3.44
F- G	-4346 / 0	-138.7	-138.7	0.32 (1)	3.86
G- H	-3468 / 0	-138.7	-138.7	0.29 (1)	4.29
H- I	-3488 / 0	-138.7	-138.7	0.29 (1)	4.29
I- J	-3488 / 0	-138.7	-138.7	0.31 (1)	4.27
J- K	-2568 / 0	-70.5	-70.5	0.21 (1)	4.12
W- A	-2464 / 0	0.0	0.0	0.18 (1)	6.56
M- K	-2447 / 0	0.0	0.0	0.17 (1)	6.56
X- W	0 / 0	-88.0	-88.0	0.11 (1)	10.00
W- V	0 / 0	-66.7	-66.7	0.11 (1)	10.00
V- U	0 / 1837	-66.7	-66.7	0.34 (1)	10.00
U- T	0 / 1955	-102.7	-102.7	0.62 (1)	10.00
T- S	0 / 4045	-102.7	-102.7	0.99 (1)	10.00
S- R	0 / 6014	-34.5	-34.5	0.78 (1)	10.00
R- Q	0 / 4271	-34.5	-34.5	0.64 (1)	10.00
Q- P	0 / 4346	-34.5	-34.5	0.65 (1)	10.00
P- O	0 / 4346	-34.5	-34.5	0.65 (1)	10.00
O- N	0 / 1538	-34.5	-34.5	0.26 (1)	10.00
N- M	0 / 0	-34.5	-34.5	0.07 (4)	10.00
M- L	0 / 0	-88.0	-88.0	0.05 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- B	-1814 / 0	0.33 (1)
B- U	0 / 1170	0.29 (1)
U- C	0 / 721	0.18 (1)
C- T	0 / 2731	0.68 (1)
T- D	-1362 / 0	0.30 (1)
D- S	0 / 1542	0.38 (1)
S- E	0 / 367	0.09 (1)
S- F	0 / 3510	0.87 (1)
R- F	-4295 / 0	0.83 (1)
F- Q	0 / 99	0.02 (1)
Q- G	0 / 167	0.06 (4)
G- O	-1134 / 0	0.81 (1)
O- I	-747 / 0	0.22 (1)
O- J	0 / 2554	0.63 (1)
N- J	-121 / 77	0.04 (1)
A- V	0 / 1481	0.37 (1)
N- K	0 / 1567	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	26-3-4	-140	-140	---	FRONT	VERT	TOTAL
U	2-8-0	-139	-139	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 18-6-0 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 10-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 814 (0.43")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 505 (0.03")
ALLOWABLE DEFL.(TL)= L/120 (0.19")

A ND 73694

CONTINUED ON PAGE 2

	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.2 lbs FACTORED DOWN AT 26-3-4 ON TOP CHORD, AND 138.7 lbs FACTORED DOWN AT 2-6-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

CALCULATED VERT. DEFL.(TL) = L/ 280 (0.06")

CSI: TC=0.43 (D-E:1), BC=0.99 (S-T:1), WB=0.87 (F-S:1), SSI=0.37 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.94 (P) (INPUT = 1.00)

A-1507369 4(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253459	J01C	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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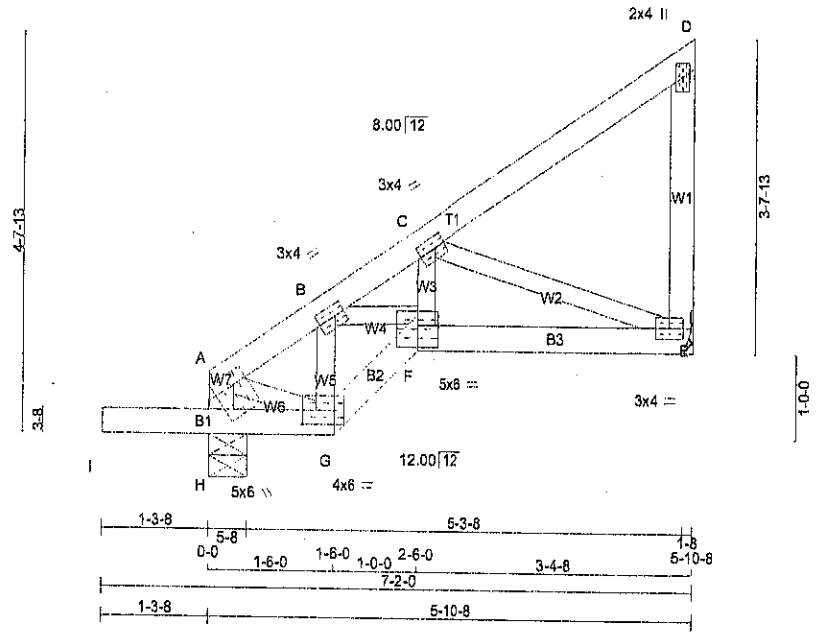

TOTAL WEIGHT = 4 X 27 = 107 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBWW-I	MT20	4.0	6.0	2.00	4.50
H	TMBMVW1*+h	MT20	5.0	6.0	1.75	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	384	0	384	0	0	5-8	1-8
E	246	0	246	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	271	183 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
E	173	117 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-209 / 0	0.0	0.0	0.02 (1)		7.81	A- G	0 / 172	0.04 (1)
A- B	-203 / 0	-70.5	-70.5	0.02 (1)		6.25	G- B	-219 / 0	0.03 (1)
B- C	-403 / 0	-70.5	-70.5	0.06 (1)		6.25	B- F	0 / 221	0.05 (1)
C- D	-10 / 0	-70.5	-70.5	0.06 (1)		6.25	F- C	0 / 131	0.03 (1)
E- D	-97 / 0	0.0	0.0	0.02 (1)		7.81	C- E	-370 / 0	0.08 (1)
I- H	0 / 0	-88.0	-88.0	0.10 (1)		10.00			
H- G	0 / 0	-17.5	-17.5	0.08 (1)		10.00			
G- F	0 / 211	-17.5	-17.5	0.04 (1)		10.00			
F- E	0 / 349	-17.5	-17.5	0.09 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.08 (C-D:1) , BC=0.10 (H-I:1) , WB=0.08 (C-E:1) , SSI=0.09 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (E) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1507369 3

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253443	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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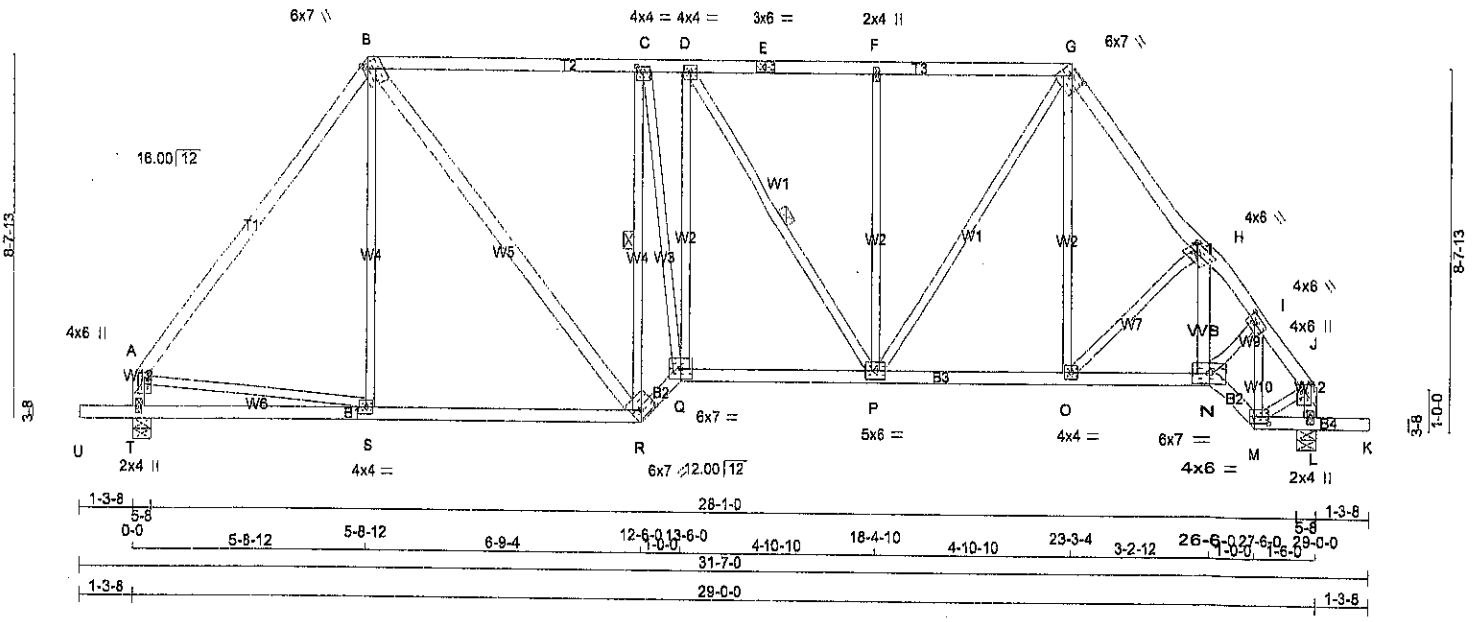

TOTAL WEIGHT = 158 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMWW-t	MT20	4.0	4.0	1.50	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-h	MT20	6.0	7.0	1.75	4.00
H	TMWW-t	MT20	4.0	6.0	2.00	2.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMW+p	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BBWW-I	MT20	4.0	6.0	2.00	4.50
N	BBWW-I	MT20	6.0	7.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BBWW-I	MT20	6.0	7.0		
R	BBWW-h	MT20	6.0	7.0	2.00	4.25
S	BMWW-t	MT20	4.0	4.0		
T	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1386	0	1386	0	0	5-8	1-8
L	1391	0	1391	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
L	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-R, D-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1194 / 0	-70.5	-70.5	0.57 (1)	5.25
B- C	-1125 / 0	-70.5	-70.5	0.57 (1)	5.18
C- D	-1259 / 0	-70.5	-70.5	0.50 (1)	4.94
D- E	-1191 / 0	-70.5	-70.5	0.29 (1)	5.50
E- F	-1191 / 0	-70.5	-70.5	0.29 (1)	5.50
F- G	-1190 / 0	-70.5	-70.5	0.29 (1)	5.50
G- H	-1348 / 0	-70.5	-70.5	0.11 (1)	5.48
H- I	-1591 / 0	-70.5	-70.5	0.10 (1)	5.15
I- J	-1120 / 0	-70.5	-70.5	0.06 (1)	5.94
T- A	-1236 / 0	0.0	0.0	0.12 (1)	7.22
L- J	-1216 / 0	0.0	0.0	0.12 (1)	7.26
U- T	0 / 0	-88.0	-88.0	0.10 (1)	10.00
T- S	0 / 0	-17.5	-17.5	0.20 (4)	10.00
S- R	0 / 714	-17.5	-17.5	0.26 (4)	10.00
R- Q	0 / 1615	-17.5	-17.5	0.26 (1)	10.00
Q- P	0 / 1256	-17.5	-17.5	0.26 (1)	10.00
P- O	0 / 798	-17.5	-17.5	0.18 (1)	10.00
O- N	0 / 981	-17.5	-17.5	0.19 (1)	10.00
N- M	0 / 884	-17.5	-17.5	0.15 (1)	10.00
M- L	0 / 0	-17.5	-17.5	0.10 (1)	10.00
L- K	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- B	0 / 127	0.04 (4)
B- R	0 / 649	0.10 (1)
R- C	-1586 / 0	0.76 (1)
C- Q	0 / 951	0.21 (1)
Q- D	0 / 249	0.06 (1)
D- P	-119 / 0	0.06 (1)
P- F	-400 / 0	0.43 (1)
P- G	0 / 707	0.16 (1)
O- G	0 / 246	0.06 (1)
O- H	-258 / 0	0.09 (1)
N- H	0 / 186	0.04 (1)
N- I	0 / 600	0.13 (1)
M- I	-917 / 0	0.16 (1)
A- S	0 / 718	0.16 (1)
M- J	0 / 714	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.57 (A-B:1) , BC=0.26 (Q-R:1) , WB=0.76 (C-R:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.27 (A) (INPUT = 1.00)

A-1073692

253443	H03T	1	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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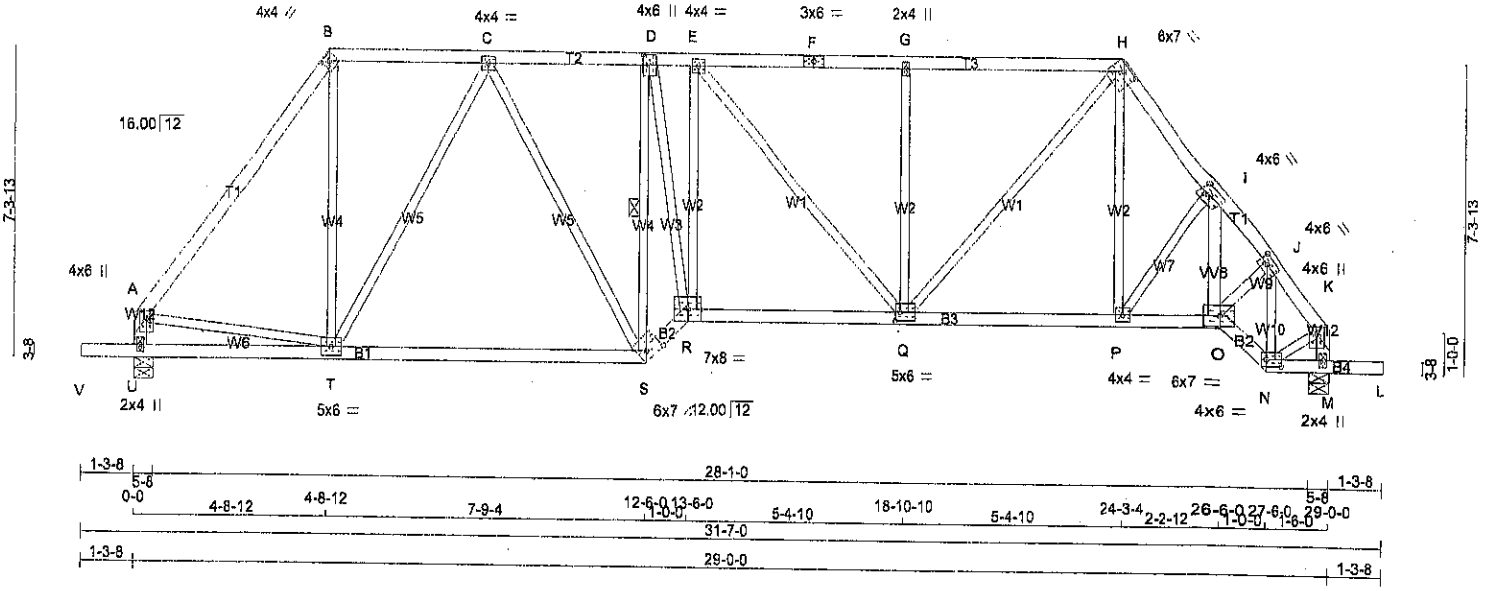

TOTAL WEIGHT = 148 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW-h	MT20	4.0	4.0	1.75	2.00
C	TMWW-t	MT20	4.0	4.0		
D	TMWW+t	MT20	4.0	6.0	3.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-h	MT20	6.0	7.0	1.50	3.75
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-I	MT20	4.0	6.0	2.00	4.50
O	BBWW-I	MT20	6.0	7.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWWW-t	MT20	6.0	6.0	2.50	1.50
R	BBWW-I	MT20	7.0	8.0		
S	BBWW-h	MT20	6.0	7.0	2.25	4.50
T	BMWWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	1386	0	1386	0	0	5-8	IN-SX
M	1391	0	1391	0	0	5-8	IN-SX

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
M	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-S.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH
A- B	-1232 / 0	-70.5	-70.5	0.36 (1)	5.41
B- C	-752 / 0	-70.5	-70.5	0.17 (1)	6.25
C- D	-1300 / 0	-70.5	-70.5	0.19 (1)	5.45
D- E	-1539 / 0	-70.5	-70.5	0.12 (1)	5.19
E- F	-1431 / 0	-70.5	-70.5	0.36 (1)	5.03
F- G	-1431 / 0	-70.5	-70.5	0.36 (1)	5.03
G- H	-1430 / 0	-70.5	-70.5	0.36 (1)	5.04
H- I	-1414 / 0	-70.5	-70.5	0.07 (1)	5.43
I- J	-1603 / 0	-70.5	-70.5	0.05 (1)	5.19
J- K	-1121 / 0	-70.5	-70.5	0.05 (1)	5.94
U- A	-1249 / 0	0.0	0.0	0.13 (1)	7.19
M- K	-1216 / 0	0.0	0.0	0.12 (1)	7.26
V- U	0 / 0	-88.0	-88.0	0.10 (1)	10.00
U- T	0 / 0	-17.5	-17.5	0.22 (4)	10.00
T- S	0 / 1104	-17.5	-17.5	0.31 (4)	10.00
S- R	0 / 1868	-17.5	-17.5	0.30 (1)	10.00
R- Q	0 / 1540	-17.5	-17.5	0.31 (1)	10.00
Q- P	0 / 840	-17.5	-17.5	0.20 (1)	10.00
P- O	0 / 968	-17.5	-17.5	0.20 (1)	10.00
O- N	0 / 888	-17.5	-17.5	0.15 (1)	10.00
N- M	0 / 0	-17.5	-17.5	0.10 (1)	10.00
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	0 / 706	0.16 (1)
T- C	-752 / 0	1.00 (1)
C- S	0 / 404	0.09 (1)
S- D	-1606 / 0	0.54 (1)
D- R	0 / 1452	0.33 (1)
R- E	-59 / 24	0.04 (1)
E- Q	-165 / 0	0.22 (1)
Q- G	-435 / 0	0.26 (1)
Q- H	0 / 883	0.20 (1)
P- H	0 / 249	0.06 (1)
P- I	-228 / 0	0.06 (1)
O- I	0 / 199	0.04 (1)
O- J	0 / 574	0.13 (1)
N- J	-921 / 0	0.16 (1)
A- T	0 / 741	0.17 (1)
N- K	0 / 718	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.36 (G-H:1) , BC=0.31 (Q-R:1) , WB=1.00 (C-T:1) , SSI=0.18 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (D) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)

A-N073691

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253443	H02T	1	1	TRUSS DESC.	

Alpe Roof Truss, Maple

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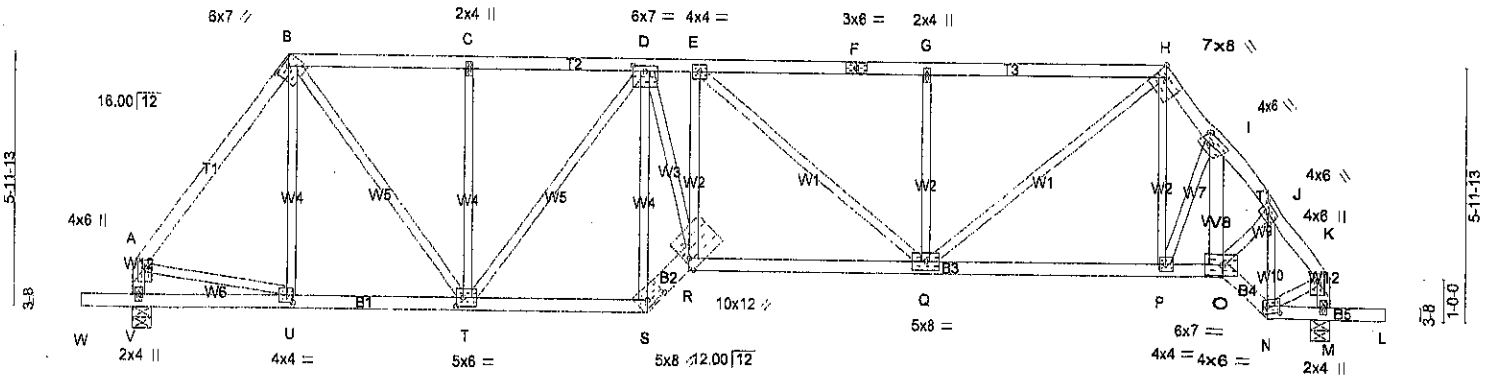

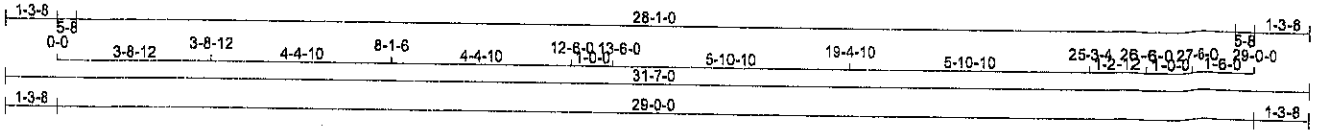

TOTAL WEIGHT = 142 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - R	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW-h	MT20	6.0	7.0	1.50	3.75
C	TMW+w	MT20	2.0	4.0		
D	TMWWW-t	MT20	6.0	7.0	1.50	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-h	MT20	7.0	8.0	Edge	2.75
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-l	MT20	4.0	6.0	2.00	4.50
O	BBWW-l	MT20	6.0	7.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BBWW-h	MT20	10.0	12.0	3.25	1.50
S	BBW-h	MT20	6.0	8.0	1.75	5.00
T	BMWWW-t	MT20	5.0	6.0	2.50	2.00
U	BMWW-t	MT20	4.0	4.0	2.00	1.75
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1386	0	1386	0	0	5-8	1-8
M	1391	0	1391	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	977	662 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
M	980	664 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	-1239 / 0	-70.5	-70.5	0.21 (1)	5.55	
B- C	-1332 / 0	-70.5	-70.5	0.22 (1)	5.37	
C- D	-1332 / 0	-70.5	-70.5	0.22 (1)	5.37	
D- E	-2010 / 0	-70.5	-70.5	0.17 (1)	4.62	
E- F	-1803 / 0	-70.5	-70.5	0.46 (1)	4.46	
F- G	-1803 / 0	-70.5	-70.5	0.46 (1)	4.46	
G- H	-1803 / 0	-70.5	-70.5	0.47 (1)	4.46	
H- I	-1494 / 0	-70.5	-70.5	0.05 (1)	5.34	
I- J	-1611 / 0	-70.5	-70.5	0.06 (1)	5.17	
J- K	-1122 / 0	-70.5	-70.5	0.04 (1)	5.85	
V- A	-1247 / 0	0.0	0.0	0.13 (1)	7.19	
M- K	-1216 / 0	0.0	0.0	0.12 (1)	7.26	
W- V	0 / 0	-88.0	-88.0	0.10 (1)	10.00	
V- U	0 / 0	-17.5	-17.5	0.06 (4)	10.00	
U- T	0 / 738	-17.5	-17.5	0.16 (1)	10.00	
T- S	0 / 1623	-17.5	-17.5	0.31 (1)	10.00	
S- R	0 / 2255	-17.5	-17.5	0.27 (1)	10.00	
R- Q	0 / 2011	-17.5	-17.5	0.39 (1)	10.00	
Q- P	0 / 889	-17.5	-17.5	0.22 (4)	10.00	
P- O	0 / 959	-17.5	-17.5	0.21 (1)	10.00	
O- N	0 / 892	-17.5	-17.5	0.15 (1)	10.00	
N- M	0 / 0	-17.5	-17.5	0.10 (1)	10.00	
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- B	-40 / 48	0.02 (1)
B- T	0 / 968	0.22 (1)
T- C	-334 / 0	0.19 (1)
T- D	-483 / 0	0.46 (1)
S- D	-1519 / 0	0.85 (1)
D- R	0 / 1780	0.40 (1)
R- E	-54 / 32	0.02 (1)
E- Q	-268 / 0	0.29 (1)
Q- G	-471 / 0	0.17 (1)
Q- H	0 / 1165	0.26 (1)
P- H	0 / 267	0.06 (1)
P- I	-209 / 0	0.05 (1)
O- I	0 / 196	0.04 (1)
O- J	0 / 556	0.13 (1)
N- J	-925 / 0	0.16 (1)
A- U	0 / 749	0.17 (1)
N- K	0 / 721	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.47 (G-H:1) , BC=0.39 (Q-R:1) , WB=0.85 (D-S:1) , SSI=0.20 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.47 (R) (INPUT = 1.00)

A-1073690

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253443	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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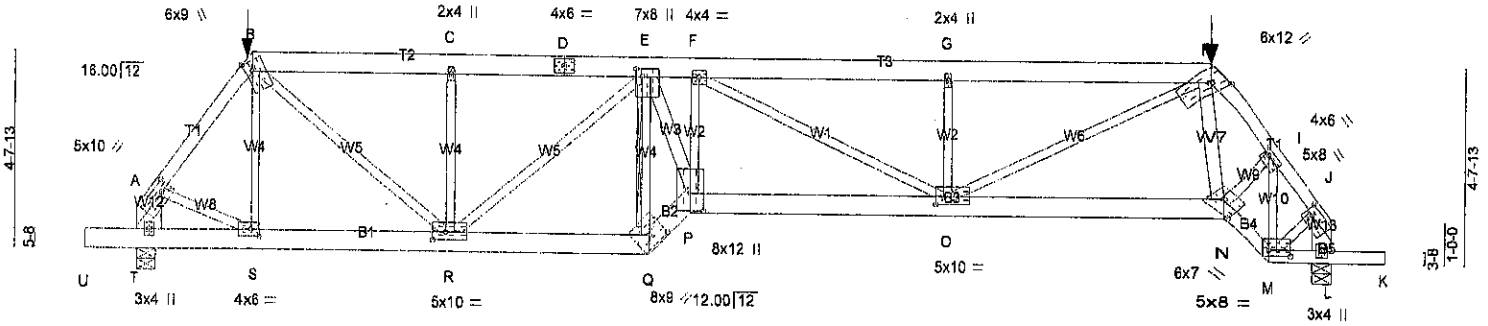

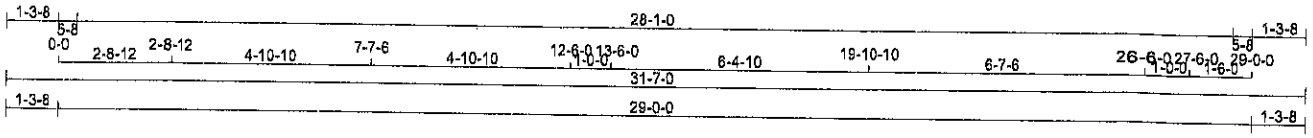

TOTAL WEIGHT = 159 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - A	2x8	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	1650F 1.5E	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	10.0	2.25	2.75
B	TTWW+m	MT20	6.0	9.0	2.75	1.75
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	4.0	6.0		
E	TMWWW+t	MT20	7.0	8.0	2.25	2.00
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+h	MT20	6.0	12.0	3.25	2.50
I	TMWW-t	MT20	4.0	6.0	1.50	1.75
J	TMVW-t	MT20	5.0	8.0	2.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BBWW-t	MT20	5.0	8.0	1.75	6.25
N	BBWW-h	MT20	6.0	7.0	3.25	4.25
O	BMWWW-t	MT20	5.0	10.0	2.25	2.25
P	BBWW+p	MT20	8.0	12.0	5.00	4.00
Q	BBW-h	MT20	6.0	9.0	3.25	4.50
R	BMWWW-t	MT20	5.0	10.0	2.25	4.00
S	BMWW-t	MT20	4.0	6.0	2.00	2.75
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2563	0	2563	0	0	5-8	2-12
L	2739	0	2739	0	0	5-8	3-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1821	1228 / 0	0 / 0	0 / 0	0 / 0	592 / 0	0 / 0
L	1925	1332 / 0	0 / 0	0 / 0	0 / 0	593 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-Q
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	-2574 / 0	-70.5	-70.5	0.21 (1)	4.11
B- C	-3457 / 0	-138.7	-138.7	0.30 (1)	4.30
C- D	-3457 / 0	-138.7	-138.7	0.29 (1)	4.30
D- E	-3457 / 0	-138.7	-138.7	0.29 (1)	4.30
E- F	-5605 / 0	-138.7	-138.7	0.33 (1)	3.40
F- G	-5158 / 0	-70.5	-70.5	0.43 (1)	3.52
G- H	-5158 / 0	-70.5	-70.5	0.41 (1)	3.54
H- I	-3347 / 0	-70.5	-70.5	0.21 (1)	3.64
I- J	-2292 / 0	-70.5	-70.5	0.14 (1)	4.38
T- A	-2453 / 0	0.0	0.0	0.13 (1)	7.31
L- J	-2436 / 0	0.0	0.0	0.17 (1)	6.58
U- T	0 / 0	-88.0	-88.0	0.05 (1)	10.00
T- S	0 / 0	-34.5	-34.5	0.07 (4)	10.00
S- R	0 / 1540	-34.5	-34.5	0.27 (1)	10.00
R- Q	0 / 4378	-34.5	-34.5	0.66 (1)	10.00
Q- P	0 / 6044	-34.5	-34.5	0.79 (1)	10.00
P- O	0 / 5796	-102.7	-102.7	0.67 (1)	10.00
O- N	0 / 1971	-102.7	-102.7	0.36 (1)	10.00
N- M	0 / 1849	-66.7	-66.7	0.34 (1)	10.00
M- L	-2 / 0	-66.7	-66.7	0.17 (1)	10.00
L- K	0 / 0	-137.2	-137.2	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- B	-106 / 74	0.03 (1)
B- R	0 / 2525	0.62 (1)
R- C	-748 / 0	0.22 (1)
R- E	-1223 / 0	0.86 (1)
Q- E	-4123 / 0	0.80 (1)
E- P	0 / 4622	0.82 (1)
P- F	0 / 240	0.06 (1)
F- O	-716 / 0	0.67 (1)
O- G	-518 / 0	0.11 (1)
O- H	0 / 3562	0.88 (1)
H- N	0 / 698	0.17 (1)
N- I	0 / 1203	0.30 (1)
M- I	-1869 / 0	0.34 (1)
A- S	0 / 1571	0.39 (1)
M- J	0 / 1463	0.36 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-8-12	-140	-140	---	FRONT	VERT	TOTAL
H	26-3-4	-139	-139	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 13-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 13-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 26-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 673 (0.52")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 742 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")

A-A0136037

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253443	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.2 lbs FACTORED DOWN AT 2-8-12, AND 138.7 lbs FACTORED DOWN AT 26-3-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

CALCULATED VERT. DEFL.(TL) = L/ 397 (0.04")

CSI: TC=0.43 (F-G:1) , BC=0.79 (P-Q:1) , WB=0.86 (H-O:1) , SSI=0.26 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.72 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073689(2)

	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H12G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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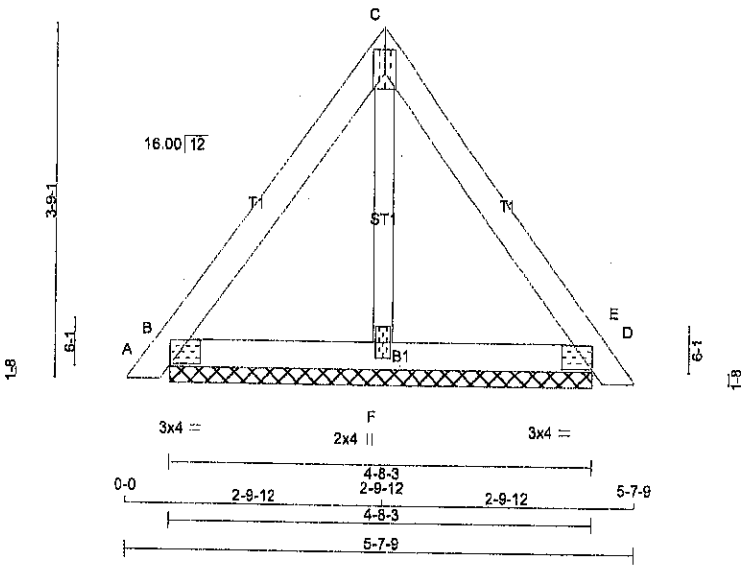

TOTAL WEIGHT = 16 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 9	-70.5	-70.5	0.01 (1)		10.00	F- C	-74 / 0	0.01 (1)
B- C	-56 / 0	-70.5	-70.5	0.07 (1)		6.25			
C- D	-56 / 0	-70.5	-70.5	0.07 (1)		6.25			
D- E	0 / 9	-70.5	-70.5	0.01 (1)		10.00			
B- F	0 / 33	-17.5	-17.5	0.03 (4)		10.00			
F- D	0 / 33	-17.5	-17.5	0.03 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07 (C-D:1) , BC=0.03 (B-F:4) , WB=0.01 (C-F:1) , SSI=0.04 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A150073688

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H12	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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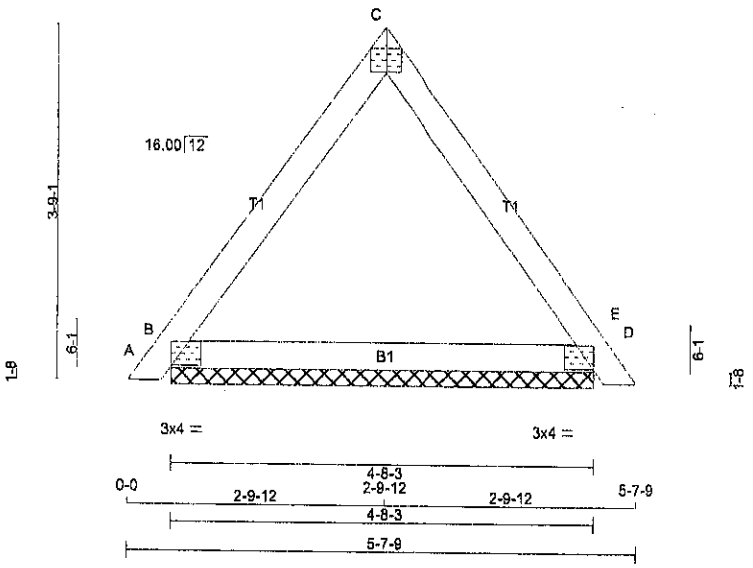

TOTAL WEIGHT = 4 X 16 = 64 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TT-p	MT20	3.0	4.0	3.25	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	231	0	231	0	0	4-8-3	1-8
D	231	0	231	0	0	4-8-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	162	113 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
D	162	113 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 9	-70.5	-70.5	0.01	(1)	10.00			
B-C	-102 / 0	-70.5	-70.5	0.07	(1)	6.25			
C-D	-102 / 0	-70.5	-70.5	0.07	(1)	6.25			
D-E	0 / 9	-70.5	-70.5	0.01	(1)	10.00			
B-D	0 / 60	-17.5	-17.5	0.11	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07 (C-D:1) , BC=0.11 (B-D:4) , WB=0.00 (n/a:0) , SSI=0.06 (B-D:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157840
PROVINCE OF ONTARIO

A-1073687

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H11SG	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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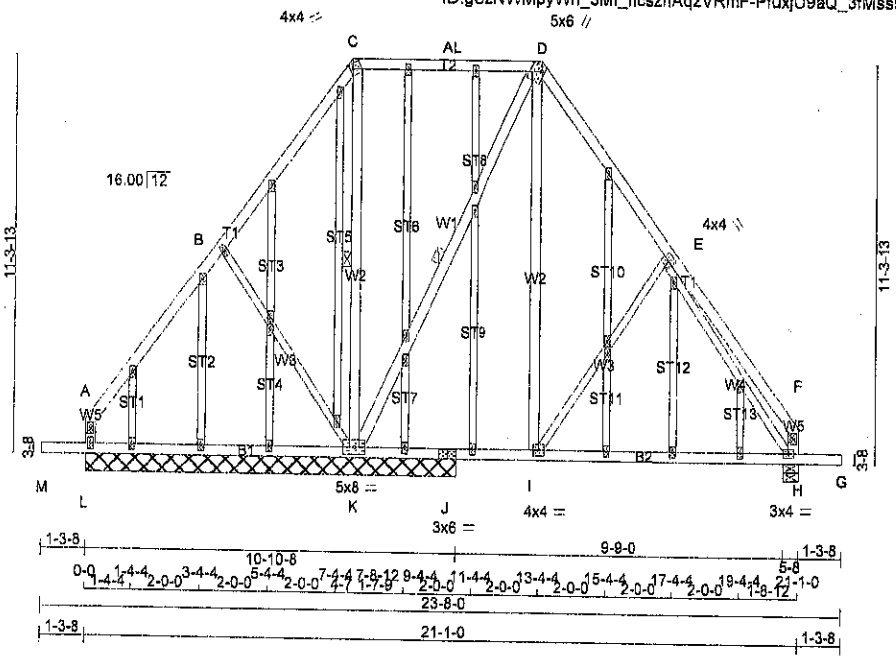

TOTAL WEIGHT = 168 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - K	2x3	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
E - H	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-m	MT20	4.0	4.0	1.75	1.00
D	TTWW+m	MT20	5.0	6.0	1.75	1.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW1-t	MT20	5.0	8.0		
L	BMV1+p	MT20	2.0	4.0		

N, O, P, Q, R, S, T, U, V, W, X, X, Y, Z, AA, AB, AC, AD, AE, AF, AG, AG, AH, AI, AJ, AK

N	NP+w	MT20	2.0	4.0

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1260	0	1260	0	0	10-10-8 (10-7-8)	
H	592	0	592	0	0	5-8	1-8
L	273	0	273	0	0	10-10-8 (10-7-8)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	886	610 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0
H	419	269 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
L	195	115 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-K, D-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	0 / 27	-70.5	-70.5	0.18 (1)	10.00	B- K	-213 / 0	0.20 (1)
B- C	0 / 176	-70.5	-70.5	0.21 (1)	10.00	K- C	-487 / 0	0.33 (1)
C-AL	0 / 129	-78.0	-78.0	0.44 (1)	10.00	K- D	-524 / 0	0.46 (1)
AL- D	0 / 129	-78.0	-78.0	0.44 (1)	10.00	I- D	0 / 280	0.04 (1)
D- E	-196 / 0	-70.5	-70.5	0.18 (1)	6.25	I- E	-191 / 0	0.18 (1)
E- F	0 / 27	-70.5	-70.5	0.18 (1)	10.00	E- H	-383 / 0	0.34 (1)
H- F	-101 / 0	0.0	0.0	0.01 (1)	7.81			
L- A	-102 / 0	0.0	0.0	0.01 (1)	7.81			
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00			
L- K	0 / 0	-17.5	-17.5	0.22 (4)	10.00			
K- J	0 / 105	-17.5	-17.5	0.22 (4)	10.00			
J- I	0 / 105	-17.5	-17.5	0.22 (4)	10.00			
I- H	0 / 213	-17.5	-17.5	0.24 (4)	10.00			
H- G	0 / 0	-88.0	-88.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.44 (C-D:1) , BC=0.24 (H-I:4) , WB=0.46 (D-K:1) , SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (H) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)

A-1507368 6

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H11	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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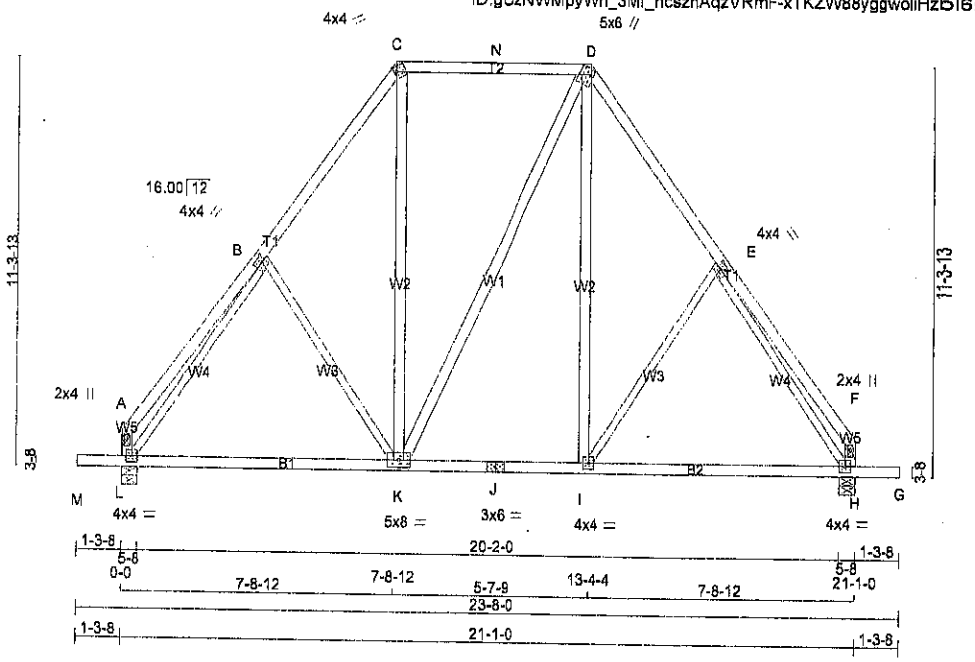

TOTAL WEIGHT = 4 X 125 = 499 lb

[M](F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - K	2x3	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
L - B	2x3	DRY	No.2	SPF
E - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0	1.75	1.00
D	TTWW+m	MT20	5.0	6.0	1.75	1.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1062	0	1062	0	0	5-8	1-8
H	1062	0	1062	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	750	497 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0
H	750	497 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 26	-70.5	-70.5	0.17 (1)	10.00
B- C	-751 / 0	-70.5	-70.5	0.14 (1)	6.25
C- N	-437 / 0	-78.0	-78.0	0.42 (1)	6.25
N- D	-437 / 0	-78.0	-78.0	0.42 (1)	6.25
D- E	-751 / 0	-70.5	-70.5	0.14 (1)	6.25
E- F	0 / 26	-70.5	-70.5	0.17 (1)	10.00
L- A	-102 / 0	0.0	0.0	0.01 (1)	7.81
H- F	-102 / 0	0.0	0.0	0.01 (1)	7.81
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00
L- K	0 / 526	-17.5	-17.5	0.26 (4)	10.00
K- J	0 / 436	-17.5	-17.5	0.24 (4)	10.00
J- I	0 / 436	-17.5	-17.5	0.24 (4)	10.00
I- H	0 / 526	-17.5	-17.5	0.26 (4)	10.00
H- G	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	-160 / 0	0.15 (1)
K- C	0 / 257	0.04 (1)
K- D	0 / 0	0.00 (1)
I- D	0 / 256	0.04 (1)
I- E	-160 / 0	0.15 (1)
L- B	-948 / 0	0.83 (1)
E- H	-947 / 0	0.83 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.42 (C-D:1) , BC=0.26 (H-I:4) , WB=0.83 (B-L:1) , SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)

A-4073685

	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H10G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-xTKZW88yggwolIHzb16mt6YOr1kNBQ2_r9LVm2yxoSh

3x5 II

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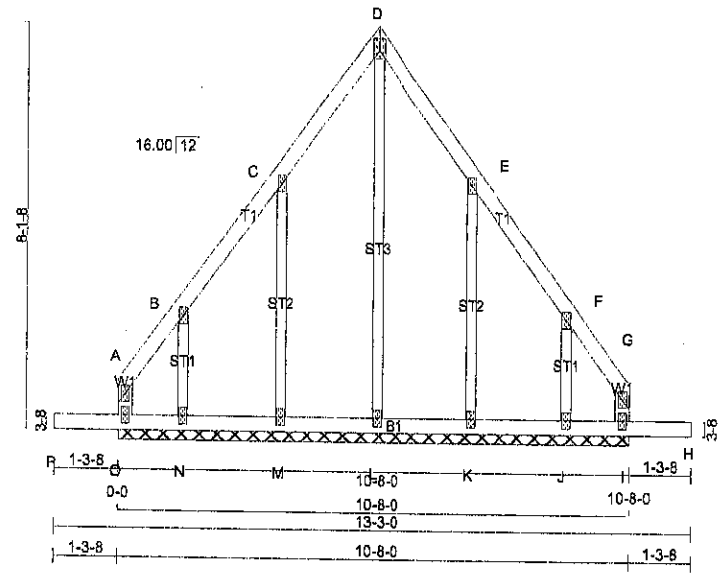

TOTAL WEIGHT = 55 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, E, F						
B	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	5.0	2.00	Edge
G	TMV+p	MT20	2.0	4.0		
I	BMV1+p	MT20	2.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
O- A	-71 / 0	0.0	0.0	0.03 (1)	7.81
A- B	-31 / 0	-70.5	-70.5	0.03 (1)	6.25
B- C	-12 / 4	-70.5	-70.5	0.04 (1)	6.25
C- D	-12 / 4	-70.5	-70.5	0.04 (1)	6.25
D- E	-12 / 4	-70.5	-70.5	0.04 (1)	6.25
E- F	-12 / 4	-70.5	-70.5	0.04 (1)	6.25
F- G	-31 / 0	-70.5	-70.5	0.03 (1)	6.25
I- G	-71 / 0	0.0	0.0	0.03 (1)	7.81
P- O	0 / 0	-88.0	-88.0	0.10 (1)	10.00
O- N	-1 / 15	-17.5	-17.5	0.08 (1)	10.00
N- M	-2 / 10	-17.5	-17.5	0.02 (4)	10.00
M- L	-2 / 7	-17.5	-17.5	0.02 (4)	10.00
L- K	-2 / 7	-17.5	-17.5	0.02 (4)	10.00
K- J	-2 / 10	-17.5	-17.5	0.02 (4)	10.00
J- I	-1 / 15	-17.5	-17.5	0.08 (1)	10.00
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- D	-129 / 0	0.15 (1)
M- C	-148 / 0	0.06 (1)
N- B	-93 / 0	0.02 (1)
K- E	-148 / 0	0.06 (1)
J- F	-93 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (C-D:1) , BC=0.10 (O-P:1) , WB=0.15 (D-L:1) , SSI=0.09 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (C) (INPUT = 0.90)
JSI METAL= 0.04 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1507368 4

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H10	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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3x5 II

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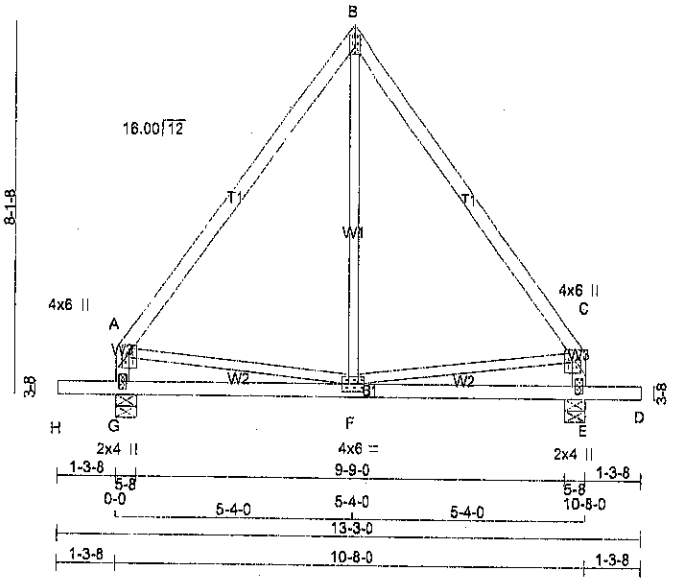

TOTAL WEIGHT = 2 X 52 = 104 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	583	0	583	0	0	5-8	1-8
E	583	0	583	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	411	278 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
E	411	278 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-282 / 0	-70.5	-70.5	0.26 (1)	6.25
B- C	-282 / 0	-70.5	-70.5	0.26 (1)	6.25
G- A	-433 / 0	0.0	0.0	0.04 (1)	7.81
E- C	-433 / 0	0.0	0.0	0.04 (1)	7.81
H- G	0 / 0	-88.0	-88.0	0.10 (1)	10.00
G- F	0 / 0	-17.5	-17.5	0.14 (4)	10.00
F- E	0 / 0	-17.5	-17.5	0.14 (4)	10.00
E- D	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	0 / 116	0.04 (4)
A- F	0 / 170	0.04 (1)
F- C	0 / 170	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.26 (B-C:1) , BC=0.14 (F-G:4) , WB=0.04 (B-F:4) , SSI=0.09 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-10073683

	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H09	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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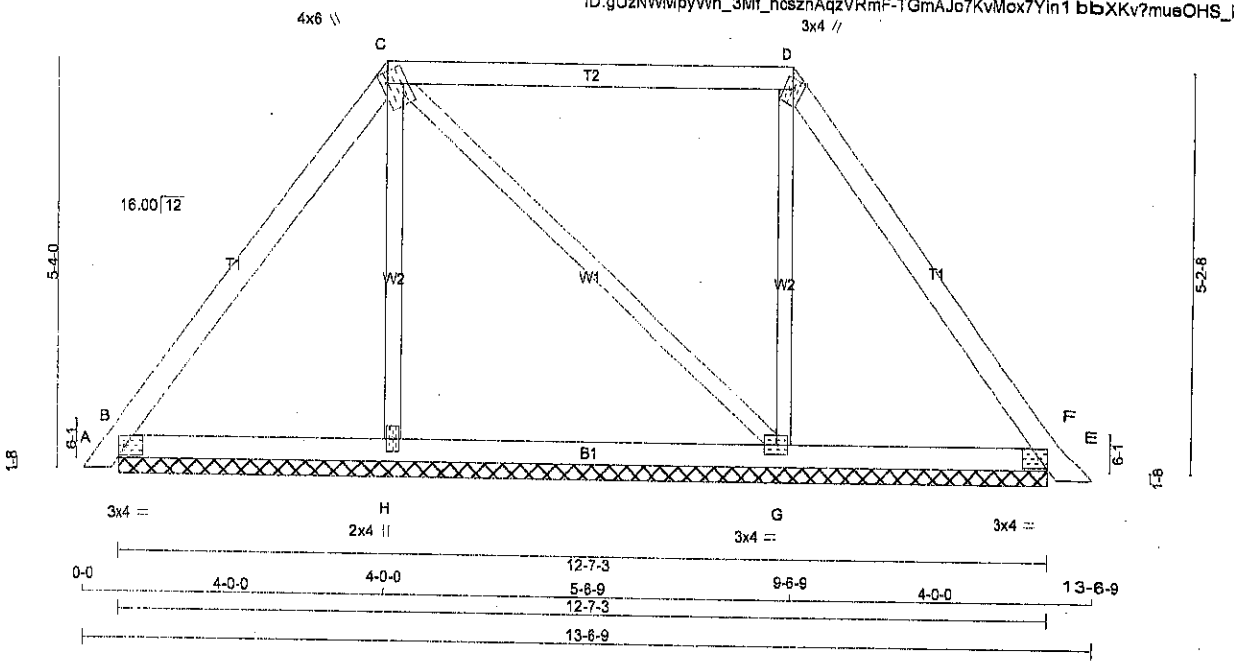

TOTAL WEIGHT = 2 X 49 = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.50	2.75
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	290	0	290	0	0	12-7-3	1-8
E	255	0	255	0	0	12-7-3	1-8
H	266	0	266	0	0	12-7-3	1-8
G	348	0	348	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	200	160 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
E	176	139 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
H	191	105 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
G	247	154 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- C	-147 / 0	-70.5	-70.5	0.15 (1)	6.25
C- D	-56 / 0	-70.5	-70.5	0.37 (1)	6.25
D- E	-104 / 0	-70.5	-70.5	0.15 (1)	6.25
E- F	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- H	0 / 86	-17.5	-17.5	0.09 (4)	10.00
H- G	0 / 83	-17.5	-17.5	0.09 (4)	10.00
G- E	0 / 61	-17.5	-17.5	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-177 / 0	0.07 (1)
C- G	-36 / 0	0.04 (1)
G- D	-235 / 0	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37 (C-D:1) , BC=0.09 (B-H:4) , WB=0.09 (D-G:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073682

	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H08	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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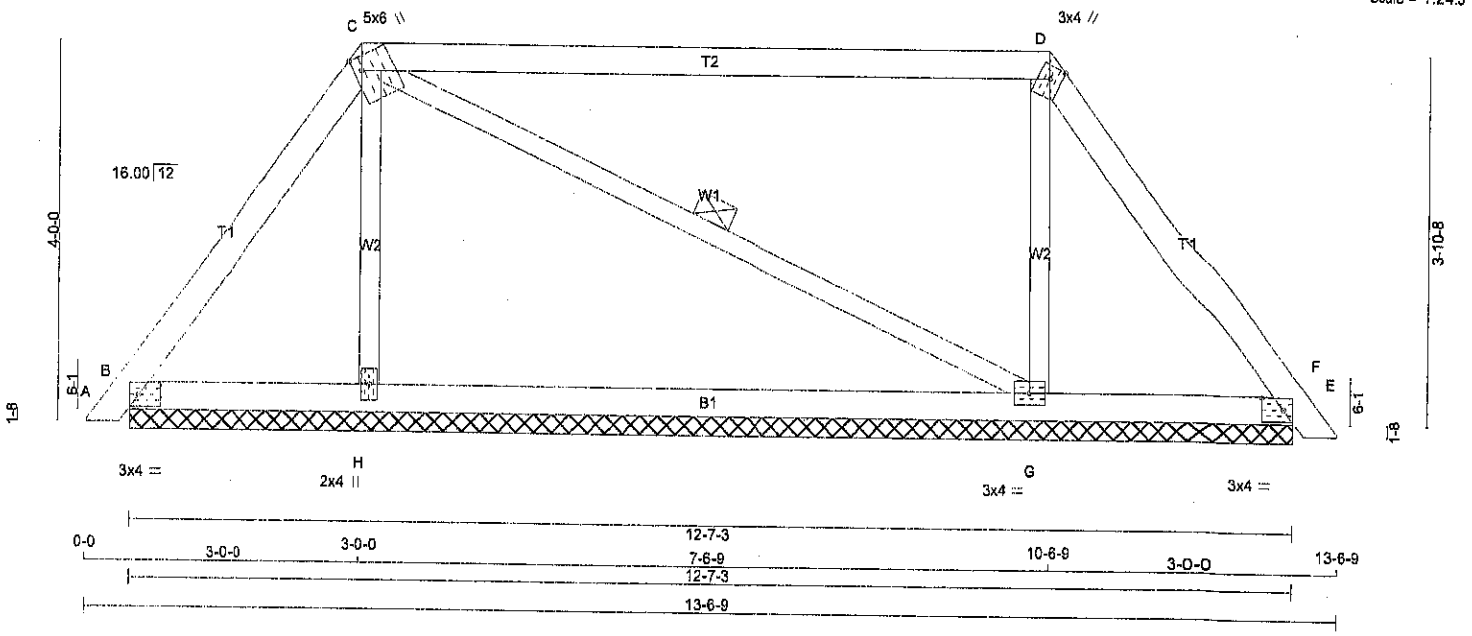

TOTAL WEIGHT = 2 X 46 = 92 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	2100F 1.8E	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.50	2.75
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	230	0	230	0	0	12-7-3	1-8
E	209	0	209	0	0	12-7-3	1-8
H	341	0	341	0	0	12-7-3	1-8
G	377	0	377	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	157	138 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
E	142	126 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
H	246	136 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
G	270	168 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-G.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- C	-146 / 0	-70.5	-70.5	0.08 (1)	6.25
C- D	-82 / 0	-70.5	-70.5	0.46 (1)	6.25
D- E	-120 / 0	-70.5	-70.5	0.08 (1)	6.25
E- F	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- H	0 / 86	-17.5	-17.5	0.16 (4)	10.00
H- G	0 / 78	-17.5	-17.5	0.16 (4)	10.00
G- E	0 / 70	-17.5	-17.5	0.16 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-229 / 0	0.05 (1)
C- G	-17 / 0	0.01 (1)
G- D	-257 / 0	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.45 (C-D:1) , BC=0.16 (B-H:4) , WB=0.06 (D-G:1) , SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (D) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073681

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H07	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 16:04:47 2015 Page 1
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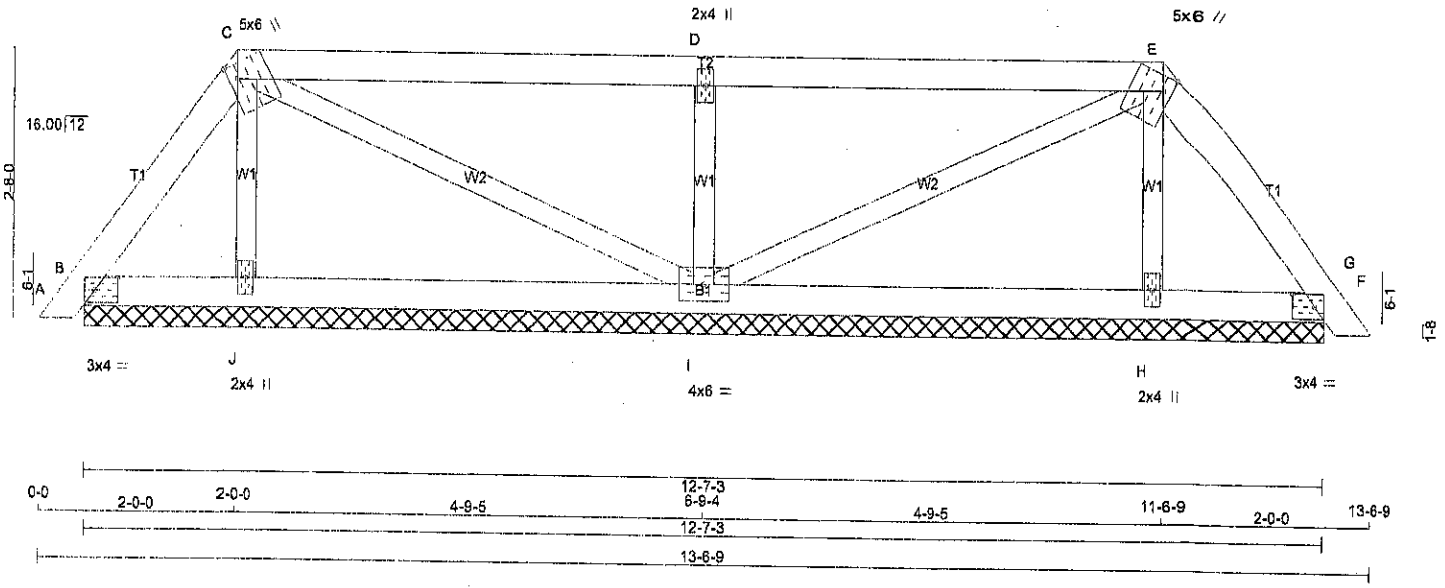

TOTAL WEIGHT = 2 X 46 = 91 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	130	0	130	0	0	12-7-3	1-8
F	130	0	130	0	0	12-7-3	1-8
J	186	0	186	0	0	12-7-3	1-8
I	527	0	527	0	0	12-7-3	1-8
H	186	0	186	0	0	12-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	88	77 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
F	88	77 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
J	134	71 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
I	369	282 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	134	71 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- C	-62 / 0	-70.5	-70.5	0.03 (1)	6.25
C- D	-8 / 0	-70.5	-70.5	0.27 (1)	10.00
D- E	-8 / 0	-70.5	-70.5	0.27 (1)	10.00
E- F	-62 / 0	-70.5	-70.5	0.03 (1)	6.25
F- G	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B- J	0 / 36	-17.5	-17.5	0.08 (4)	10.00
J- I	0 / 30	-17.5	-17.5	0.09 (4)	10.00
I- H	0 / 30	-17.5	-17.5	0.09 (4)	10.00
H- F	0 / 36	-17.5	-17.5	0.06 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-119 / 0	0.02 (1)
C- I	-24 / 0	0.01 (1)
I- D	-420 / 0	0.07 (1)
I- E	-24 / 0	0.01 (1)
H- E	-119 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27 (C-D:1) , BC=0.09 (I-J:4) , WB=0.07 (D-I:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1666

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073680

253442	H06	10	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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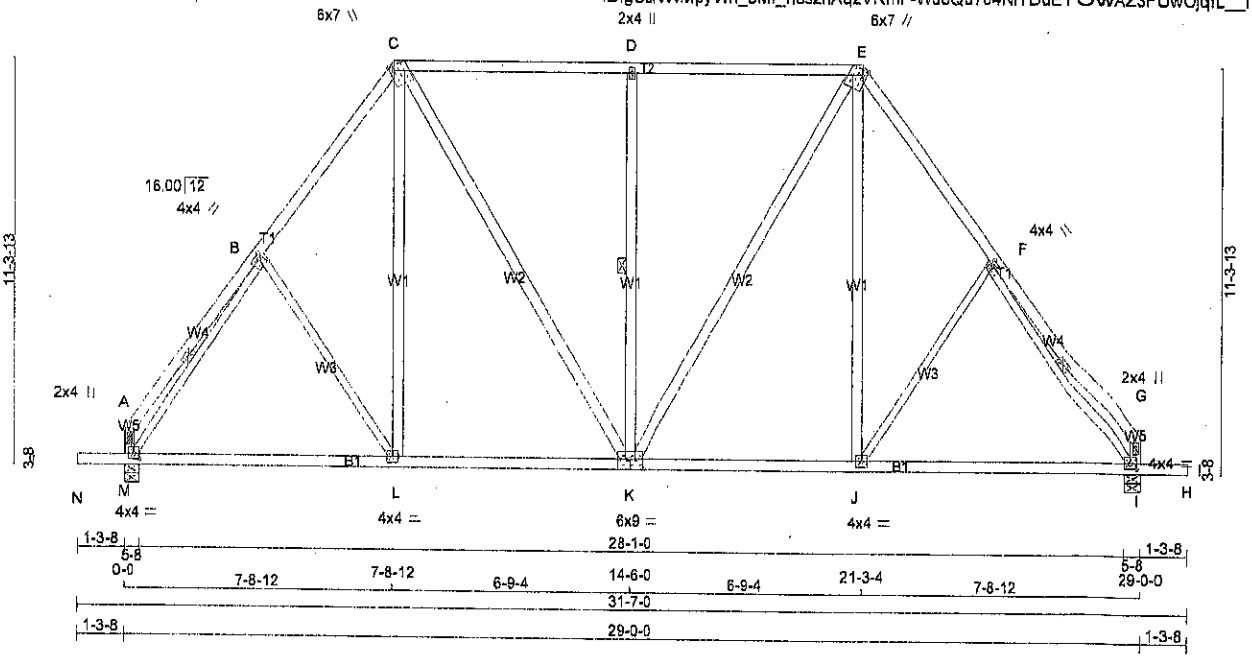

TOTAL WEIGHT = 10 X 170 = 1698 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF
M - B	2x3	DRY	No.2	SPF
F - I	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.50	1.25
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMWW-t	MT20	4.0	4.0	1.50	1.25
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	1.75	2.00
J	BMWW-t	MT20	4.0	4.0		
K	BSWWW-t	MT20	6.0	9.0	Edge	4.50
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1440	0	1440	0	0	5-8	1-8
I	1440	0	1440	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1019	663 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0
I	1019	663 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-K, B-M, F-I.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 25	-70.5	-70.5	0.16 (1)	10.00
B- C	-1200 / 0	-70.5	-70.5	0.16 (1)	5.69
C- D	-933 / 0	-78.0	-78.0	0.48 (1)	5.73
D- E	-933 / 0	-78.0	-78.0	0.48 (1)	5.73
E- F	-1200 / 0	-70.5	-70.5	0.16 (1)	5.69
F- G	0 / 25	-70.5	-70.5	0.16 (1)	10.00
M- A	-103 / 0	0.0	0.0	0.01 (1)	7.81
I- G	-103 / 0	0.0	0.0	0.01 (1)	7.81
N- M	0 / 0	-88.0	-88.0	0.10 (1)	10.00
M- L	0 / 779	-17.5	-17.5	0.31 (4)	10.00
L- K	0 / 704	-17.5	-17.5	0.31 (4)	10.00
K- J	0 / 704	-17.5	-17.5	0.31 (4)	10.00
J- I	0 / 779	-17.5	-17.5	0.31 (4)	10.00
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- L	-135 / 0	0.13 (1)
L- C	0 / 259	0.04 (1)
C- K	0 / 436	0.07 (1)
K- D	-644 / 0	0.43 (1)
K- E	0 / 436	0.07 (1)
J- E	0 / 259	0.04 (1)
J- F	-135 / 0	0.13 (1)
M- B	-1403 / 0	0.45 (1)
F- I	-1403 / 0	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.48 (C-D:1) , BC=0.31 (J-K:4) , WB=0.45 (F-I:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

A-5073679

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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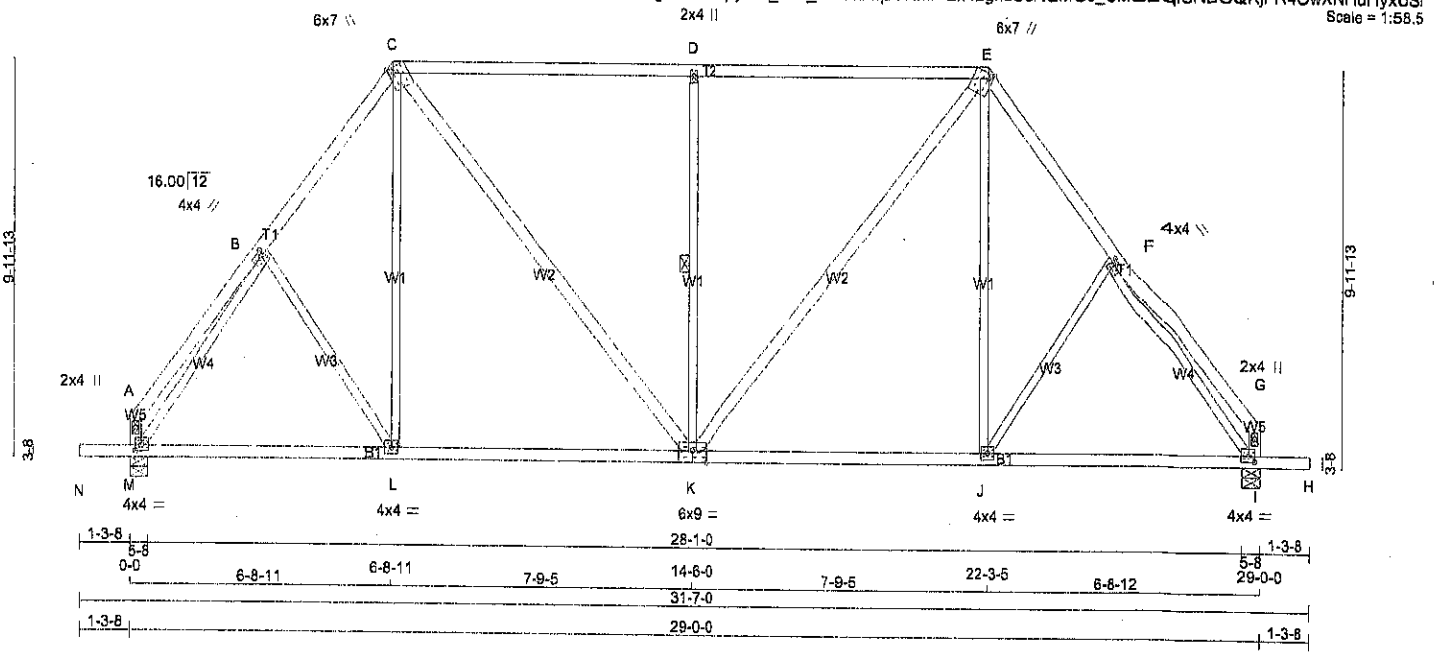

TOTAL WEIGHT = 2 X 151 = 302 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.50	1.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMWW-t	MT20	4.0	4.0	1.50	1.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	1.75	1.75
J	BMWW-t	MT20	4.0	4.0		
K	BSWWW-t	MT20	6.0	9.0	Edge	4.50
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 21	-70.5	-70.5	0.12 (1)	10.00	
B- C	-1179 / 0	-70.5	-70.5	0.12 (1)	5.77	
C- D	-1016 / 0	-70.5	-70.5	0.59 (1)	5.33	
D- E	-1016 / 0	-70.5	-70.5	0.59 (1)	5.33	
E- F	-1179 / 0	-70.5	-70.5	0.12 (1)	5.77	
F- G	0 / 21	-70.5	-70.5	0.12 (1)	10.00	
M- A	-91 / 0	0.0	0.0	0.01 (1)	7.81	
I- G	-91 / 0	0.0	0.0	0.01 (1)	7.81	
N- M	0 / 0	-88.0	-88.0	0.10 (1)	10.00	
M- L	0 / 747	-17.5	-17.5	0.27 (4)	10.00	
L- K	0 / 693	-17.5	-17.5	0.27 (4)	10.00	
K- J	0 / 693	-17.5	-17.5	0.27 (4)	10.00	
J- I	0 / 747	-17.5	-17.5	0.27 (4)	10.00	
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- L	-98 / 4	0.07 (1)
L- C	0 / 222	0.05 (4)
C- K	0 / 516	0.08 (1)
K- D	-670 / 0	0.45 (1)
K- E	0 / 515	0.08 (1)
J- E	0 / 222	0.05 (4)
J- F	-98 / 4	0.07 (1)
M- B	-1361 / 0	0.86 (1)
F- I	-1361 / 0	0.86 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.59 (C-D:1) , BC=0.27 (J-K:4) , WB=0.86 (B-M:1) , SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.52 (F) (INPUT = 1.00)

A-5073678

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H04	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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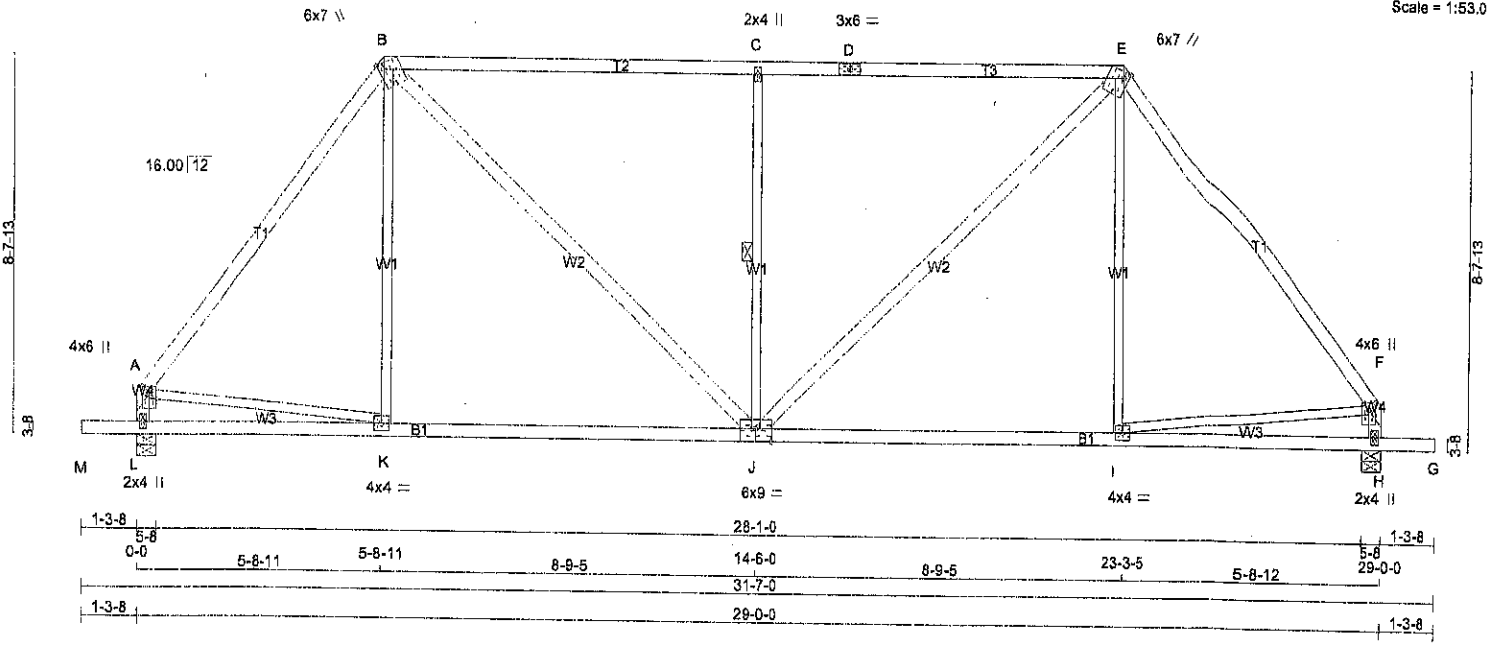

TOTAL WEIGHT = 2 X 135 = 271 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	1650F 1.5E	SPF
D - E	2x4	DRY	1650F 1.5E	SPF
E - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BSWWW-t	MT20	6.0	9.0	Edge	4.50
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1389	0	1389	0	0	5-8	1-8
H	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
H	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1199 / 0	-70.5	-70.5	0.57 (1)	5.24
B- C	-1198 / 0	-70.5	-70.5	0.90 (1)	4.79
C- D	-1200 / 0	-70.5	-70.5	0.90 (1)	4.79
D- E	-1200 / 0	-70.5	-70.5	0.90 (1)	4.79
E- F	-1199 / 0	-70.5	-70.5	0.57 (1)	5.24
L- A	-1240 / 0	0.0	0.0	0.12 (1)	7.21
H- F	-1240 / 0	0.0	0.0	0.12 (1)	7.21
M- L	0 / 0	-88.0	-88.0	0.10 (1)	10.00
L- K	0 / 0	-17.5	-17.5	0.21 (4)	10.00
K- J	0 / 717	-17.5	-17.5	0.33 (4)	10.00
J- I	0 / 717	-17.5	-17.5	0.33 (4)	10.00
I- H	0 / 0	-17.5	-17.5	0.21 (4)	10.00
H- G	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	0 / 131	0.05 (4)
B- J	0 / 664	0.11 (1)
J- C	-756 / 0	0.36 (1)
J- E	0 / 664	0.11 (1)
I- E	0 / 131	0.05 (4)
A- K	0 / 720	0.16 (1)
I- F	0 / 720	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.90 (C-E:1) , BC=0.33 (I-J:4) , WB=0.36 (C-J:1) , SSI=0.30 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (I) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)

A-1507 3677

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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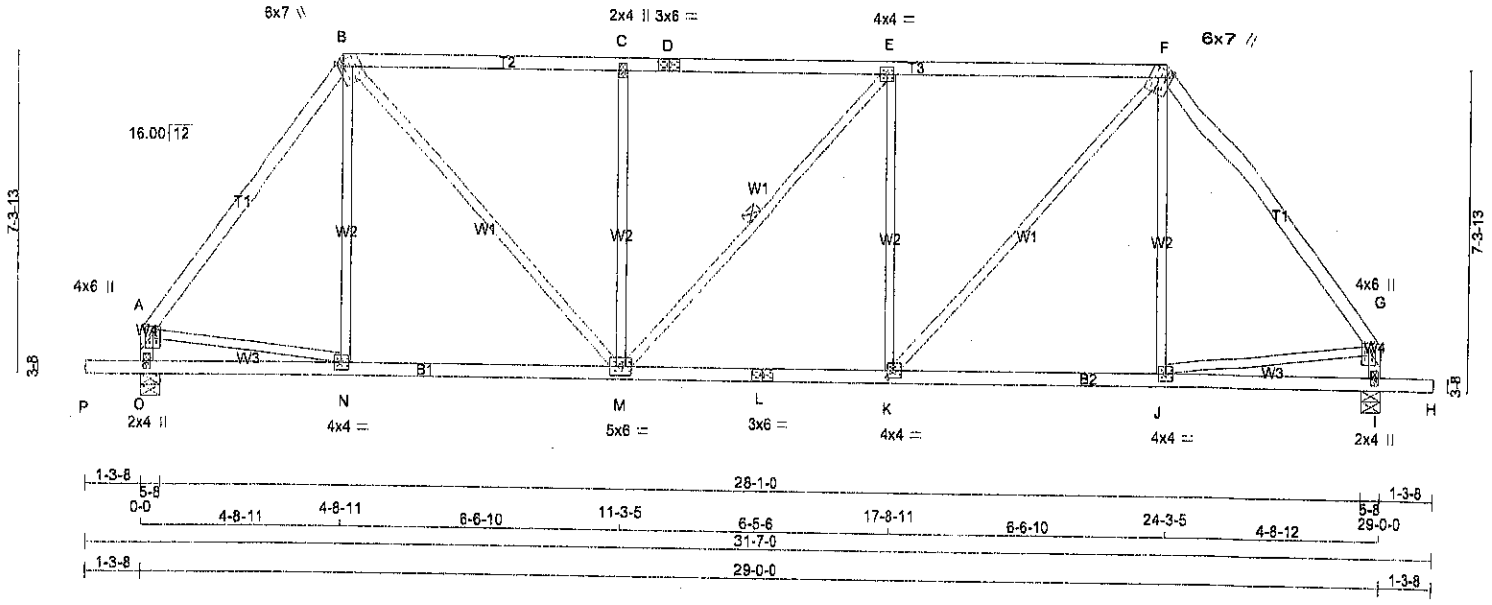

TOTAL WEIGHT = 2 X 130 = 261 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1225 / 0	-70.5	-70.5	0.36 (1)	5.41
B- C	-1299 / 0	-70.5	-70.5	0.47 (1)	5.07
C- D	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06
D- E	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06
E- F	-1300 / 0	-70.5	-70.5	0.48 (1)	5.06
F- G	-1225 / 0	-70.5	-70.5	0.36 (1)	5.41
O- A	-1245 / 0	0.0	0.0	0.13 (1)	7.20
I- G	-1244 / 0	0.0	0.0	0.13 (1)	7.20
P- O	0 / 0	-88.0	-88.0	0.10 (1)	10.00
O- N	0 / 0	-17.5	-17.5	0.13 (4)	10.00
N- M	0 / 732	-17.5	-17.5	0.20 (4)	10.00
M- L	0 / 1301	-17.5	-17.5	0.29 (1)	10.00
L- K	0 / 1301	-17.5	-17.5	0.29 (1)	10.00
K- J	0 / 732	-17.5	-17.5	0.20 (4)	10.00
J- I	0 / 0	-17.5	-17.5	0.13 (4)	10.00
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	0 / 92	0.03 (4)
B- M	0 / 830	0.19 (1)
M- C	-492 / 0	0.47 (1)
M- E	-1 / 0	0.00 (1)
K- E	-492 / 0	0.47 (1)
K- F	0 / 831	0.19 (1)
J- F	0 / 91	0.03 (4)
A- N	0 / 738	0.17 (1)
J- G	0 / 738	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.48 (E-F:1) , BC=0.29 (K-M:1) , WB=0.47 (E-K:1) , SSI=0.21 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1666

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (M) (INPUT = 0.90)
JSI METAL= 0.40 (L) (INPUT = 1.00)

A-140736 76

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253442	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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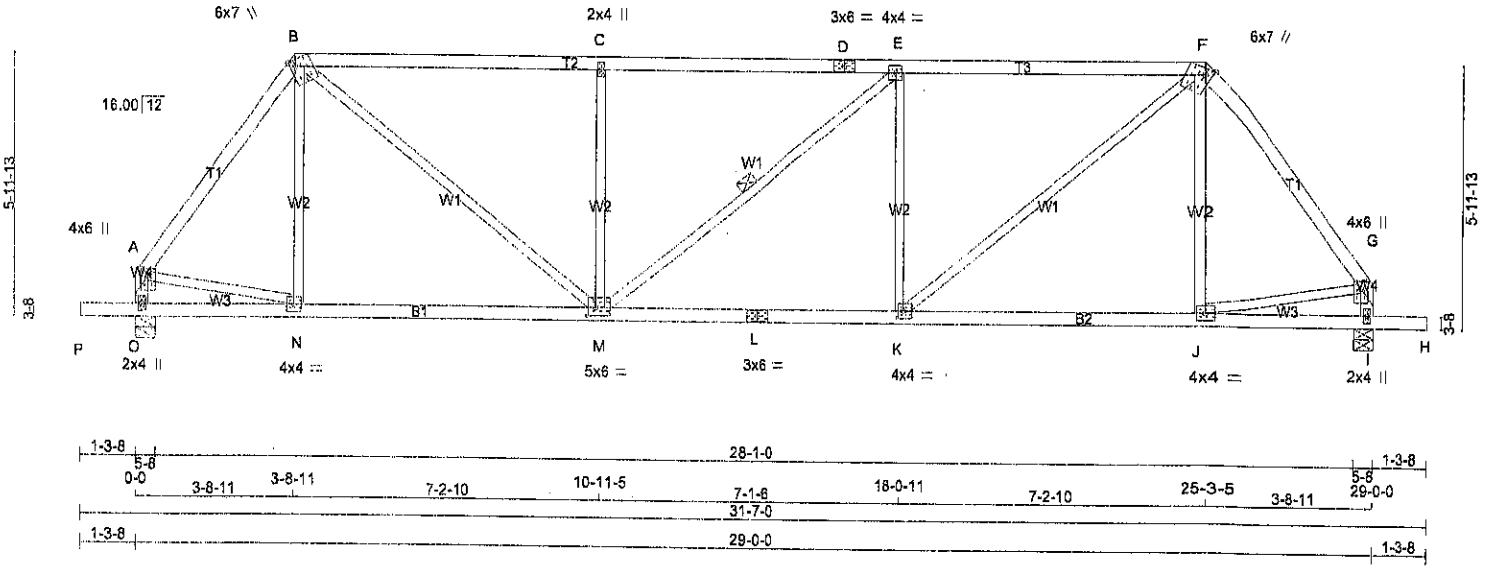

TOTAL WEIGHT = 2 X 122 = 244 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0	2.25	2.00
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1389	0	1389	0	0	5-8	1-8
I	1389	0	1389	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
I	979	663 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-1251 / 0	-70.5	-70.5	0.21 (1)	5.53
B- C	-1600 / 0	-70.5	-70.5	0.63 (1)	4.48
C- D	-1600 / 0	-70.5	-70.5	0.63 (1)	4.47
D- E	-1600 / 0	-70.5	-70.5	0.63 (1)	4.47
E- F	-1601 / 0	-70.5	-70.5	0.64 (1)	4.47
F- G	-1250 / 0	-70.5	-70.5	0.21 (1)	5.53
O- A	-1257 / 0	0.0	0.0	0.13 (1)	7.17
I- G	-1257 / 0	0.0	0.0	0.13 (1)	7.17
P- O	0 / 0	-88.0	-88.0	0.10 (1)	10.00
O- N	0 / 0	-17.5	-17.5	0.14 (4)	10.00
N- M	0 / 746	-17.5	-17.5	0.24 (4)	10.00
M- L	0 / 1601	-17.5	-17.5	0.35 (1)	10.00
L- K	0 / 1601	-17.5	-17.5	0.35 (1)	10.00
K- J	0 / 745	-17.5	-17.5	0.24 (4)	10.00
J- I	0 / 0	-17.5	-17.5	0.14 (4)	10.00
I- H	0 / 0	-88.0	-88.0	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-12 / 81	0.03 (4)
B- M	0 / 1085	0.24 (1)
M- C	-642 / 0	0.30 (1)
M- E	-1 / 0	0.00 (1)
K- E	-643 / 0	0.30 (1)
K- F	0 / 1086	0.24 (1)
J- F	-12 / 81	0.03 (4)
A- N	0 / 756	0.17 (1)
J- G	0 / 756	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.64 (E-F:1) , BC=0.35 (K-M:1) , WB=0.30 (E-K:1) , SSI=0.24 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.80)
JSI METAL= 0.49 (L) (INPUT = 1.00)

A-1073675

253442	H01-Cond2	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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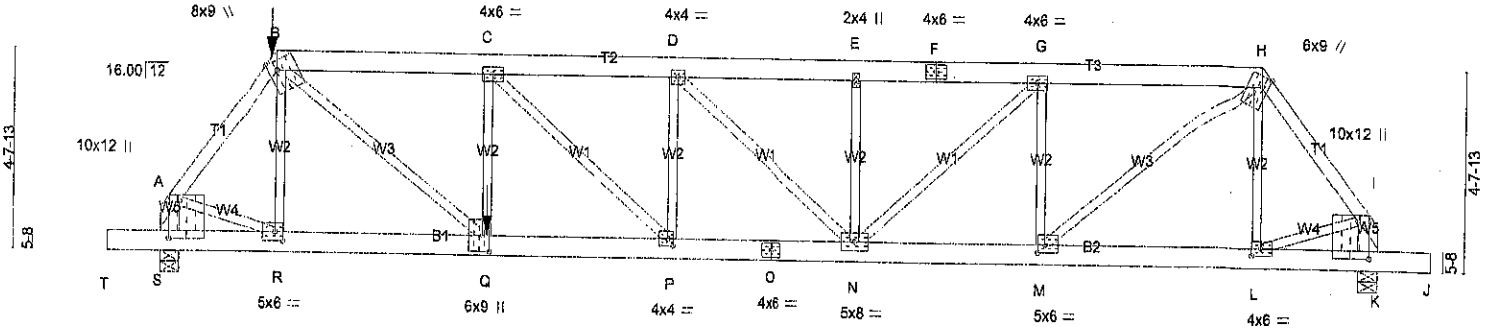

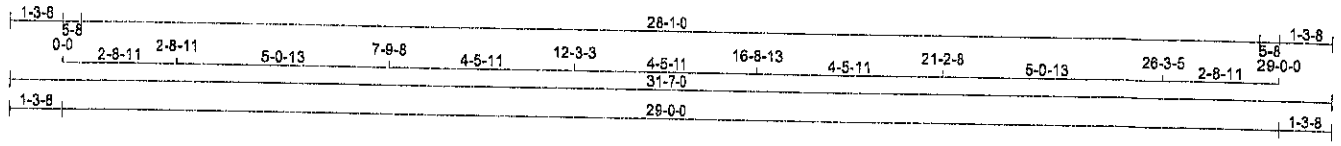

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
S - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - O	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMVW*+p	MT20	10.0	12.0	10.25	3.00
B	TTWW+m	MT20	8.0	9.0	2.50	2.25
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	2.75	1.75
I	TMBMVW*+p	MT20	10.0	12.0	10.25	3.00
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BMWW-t	MT20	5.0	6.0	2.25	2.00
N	BMWWW-t	MT20	6.0	8.0		
O	BS-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BMWW+t	MT20	6.0	9.0	4.50	2.00
R	BMWW-t	MT20	5.0	6.0	2.50	2.25

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3028	0	3028	0	0	5-8	4-2
K	1892	0	1892	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2117	1527 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0
K	1327	936 / 0	0 / 0	0 / 0	0 / 0	391 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-3070 / 0	-70.5	-70.5	0.26 (1)	3.77
B- C	-4296 / 0	-138.7	-138.7	0.41 (1)	3.79
C- D	-4175 / 0	-70.5	-70.5	0.24 (1)	4.04
D- E	-3633 / 0	-70.5	-70.5	0.18 (1)	4.35
E- F	-3633 / 0	-70.5	-70.5	0.18 (1)	4.35
F- G	-3633 / 0	-70.5	-70.5	0.18 (1)	4.35
G- H	-2698 / 0	-70.5	-70.5	0.16 (1)	4.80
H- I	-1831 / 0	-70.5	-70.5	0.16 (1)	4.79
S- A	-2907 / 0	0.0	0.0	0.21 (1)	6.12
K- I	-1772 / 0	0.0	0.0	0.13 (1)	7.45
T- S	0 / 0	-88.0	-88.0	0.05 (1)	10.00
S- R	0 / 0	-34.5	-34.5	0.09 (1)	10.00
R- Q	0 / 1836	-34.5	-34.5	0.32 (1)	10.00
Q- P	0 / 4296	-17.5	-17.5	0.63 (1)	10.00
P- O	0 / 4175	-17.5	-17.5	0.59 (1)	10.00
O- N	0 / 4175	-17.5	-17.5	0.59 (1)	10.00
N- M	0 / 2698	-17.5	-17.5	0.39 (1)	10.00
M- L	0 / 1094	-17.5	-17.5	0.18 (1)	10.00
L- K	0 / 0	-17.5	-17.5	0.04 (4)	10.00
K- J	0 / 0	-88.0	-88.0	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- B	-136 / 92	0.04 (1)
B- Q	0 / 3191	0.79 (1)
Q- C	-444 / 0	0.13 (1)
C- P	-166 / 25	0.11 (1)
P- D	0 / 217	0.05 (1)
D- N	-743 / 0	0.47 (1)
N- E	-302 / 0	0.09 (1)
N- G	0 / 1282	0.32 (1)
M- G	-1247 / 0	0.36 (1)
M- H	0 / 2061	0.52 (1)
L- H	-114 / 30	0.03 (1)
A- R	0 / 1873	0.46 (1)
L- I	0 / 1117	0.28 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-8-11	-140	-140	---	FRONT	VERT	TOTAL
Q	7-9-8	-1524	-1524	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 2 X 158 = 317 lb [M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 7-9-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.41 (B-C:1) , BC=0.63 (P-Q:1) , WB=0.78 (B-Q:1) , SSI=0.25 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE HEELS OFF

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157940
PROVINCE OF ONTARIO

A-HD73676

CONTINUED ON PAGE 2

253442	H01-Cond2	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 139.9 lbs FACTORED DOWN AT 2-8-11 ON TOP CHORD, AND 1524.6 lbs FACTORED DOWN AT 7-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.85 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-4073674(2)

253442	H01-Cond1	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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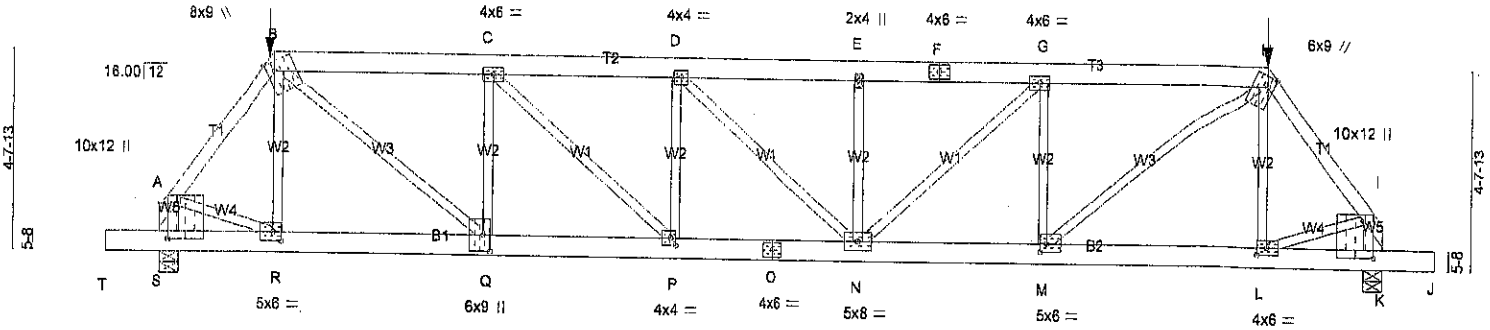

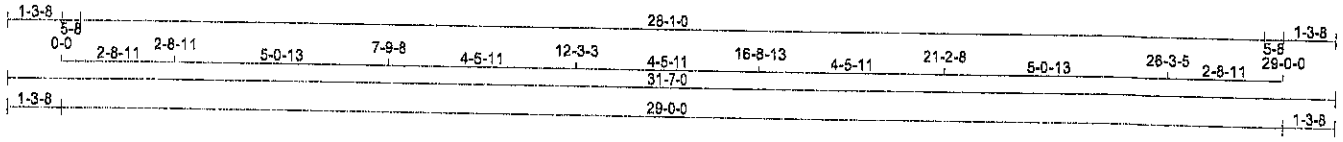

TOTAL WEIGHT = 2 X 158 = 317 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
S - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - O	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMVW*+p	MT20	10.0	12.0	10.25	3.00
B	TTWW+m	MT20	6.0	9.0	2.50	2.25
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	2.75	1.75
I	TMBMVW*+p	MT20	10.0	12.0	10.25	3.00
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BMWW-t	MT20	5.0	6.0	2.25	2.00
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BMWW+t	MT20	6.0	9.0	4.50	2.00
R	BMWW-t	MT20	5.0	6.0	2.50	2.25

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ.	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2578	0	2578	0	0	5-8	2-12
K	2578	0	2578	0	0	5-8	2-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1818	1225 / 0	0 / 0	0 / 0	0 / 0	593 / 0	0 / 0
K	1818	1225 / 0	0 / 0	0 / 0	0 / 0	593 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-2565 / 0	-70.5	-70.5	0.21 (1)	4.12
B- C	-3510 / 0	-138.7	-138.7	0.33 (1)	4.24
C- D	-4306 / 0	-138.7	-138.7	0.33 (1)	3.88
D- E	-4297 / 0	-138.7	-138.7	0.27 (1)	3.96
E- F	-4297 / 0	-138.7	-138.7	0.34 (1)	3.88
F- G	-4297 / 0	-138.7	-138.7	0.34 (1)	3.88
G- H	-3512 / 0	-138.7	-138.7	0.33 (1)	4.24
H- I	-2564 / 0	-70.5	-70.5	0.21 (1)	4.12
S- A	-2444 / 0	0.0	0.0	0.17 (1)	6.56
K- I	-2444 / 0	0.0	0.0	0.17 (1)	6.56
T- S	0 / 0	-88.0	-88.0	0.05 (1)	10.00
S- R	0 / 0	-34.5	-34.5	0.08 (4)	10.00
R- Q	0 / 1534	-34.5	-34.5	0.26 (1)	10.00
Q- P	0 / 3510	-34.5	-34.5	0.52 (1)	10.00
P- O	0 / 4306	-34.5	-34.5	0.63 (1)	10.00
O- N	0 / 4306	-34.5	-34.5	0.63 (1)	10.00
N- M	0 / 3512	-34.5	-34.5	0.52 (1)	10.00
M- L	0 / 1534	-34.5	-34.5	0.26 (1)	10.00
L- K	0 / 0	-34.5	-34.5	0.08 (4)	10.00
K- J	0 / 0	-88.0	-88.0	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- B	-118 / 81	0.03 (1)
B- Q	0 / 2563	0.63 (1)
Q- C	-1482 / 0	0.43 (1)
C- P	0 / 1091	0.27 (1)
P- D	-591 / 0	0.17 (1)
D- N	-12 / 0	0.01 (1)
N- E	-586 / 0	0.17 (1)
N- G	0 / 1076	0.27 (1)
M- G	-1477 / 0	0.43 (1)
M- H	0 / 2566	0.64 (1)
L- H	-119 / 81	0.03 (1)
A- R	0 / 1565	0.39 (1)
L- I	0 / 1565	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-8-11	-140	-140	---	FRONT	VERT	TOTAL
H	26-3-5	-140	-140	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-8-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.34 (E-G:1) , BC=0.63 (N-P:1) , WB=0.64 (H-M:1) , SSI=0.27 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	619	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 60 deg.

A-1507273

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
07/JUL/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

253442	H01-Cond1	2	1	TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-s6Pv2l3ZJW2oPdFdhKU74elkxDDP28oyEZ9d0yyxoSo

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 139.9 lbs FACTORED DOWN AT 26-3-5, AND 139.9 lbs FACTORED DOWN AT 2-8-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.93 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157048
PROVINCE OF ONTARIO

A-N073673(2)

	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253436	H08	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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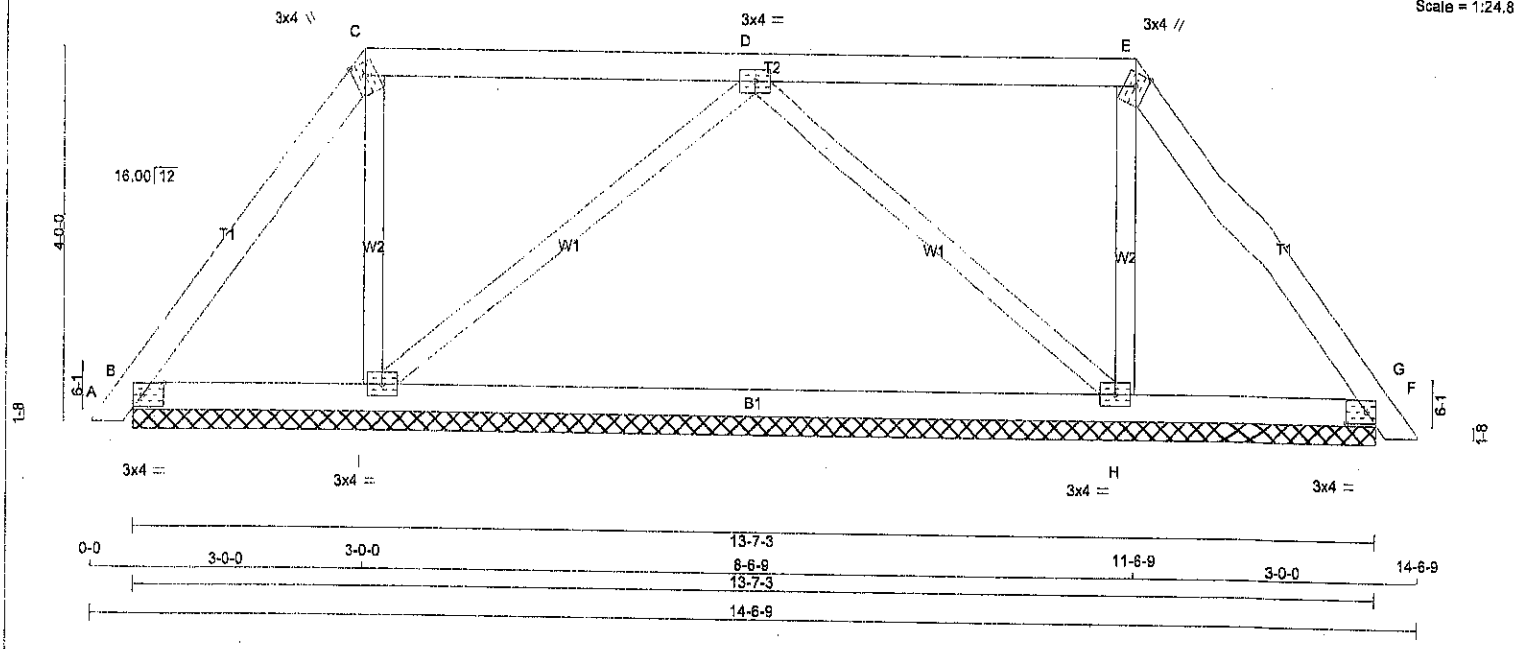

TOTAL WEIGHT = 2 X 51 = 101 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+m	MT20	3.0	4.0	Edge	
D	TMWW-I	MT20	3.0	4.0		
E	TTW+m	MT20	3.0	4.0	Edge	
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMWW1-I	MT20	3.0	4.0		
I	BMWW1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	127	0	127	0	0	13-7-3	1-8
F	127	0	127	0	0	13-7-3	1-8
I	496	0	496	0	0	13-7-3	1-8
H	496	0	496	0	0	13-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	85	81 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
F	85	81 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
I	353	219 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
H	353	219 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX (LC)	MAX UNBRAC LENGTH
A-B	0 / 9	-70.5	-70.5	1	0.01	(1)	10.00
B-C	-26 / 0	-70.5	-70.5	1	0.08	(1)	6.25
C-D	-11 / 0	-70.5	-70.5	1	0.21	(1)	6.25
D-E	-11 / 0	-70.5	-70.5	1	0.21	(1)	6.25
E-F	-28 / 0	-70.5	-70.5	1	0.08	(1)	6.25
F-G	0 / 9	-70.5	-70.5	1	0.01	(1)	10.00
B-I	0 / 16	-17.5	-17.5	4	0.20	(4)	10.00
I-H	0 / 229	-17.5	-17.5	4	0.21	(4)	10.00
H-F	0 / 16	-17.5	-17.5	4	0.20	(4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	-180 / 0	0.04 (1)
I-D	-287 / 0	0.15 (1)
D-H	-287 / 0	0.15 (1)
H-E	-180 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21 (D-E:1) , BC=0.21 (H-I:4) , WB=0.15 (D-I:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (I) (INPUT = 0.90)
JSI METAL= 0.08 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2014
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073633

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H07	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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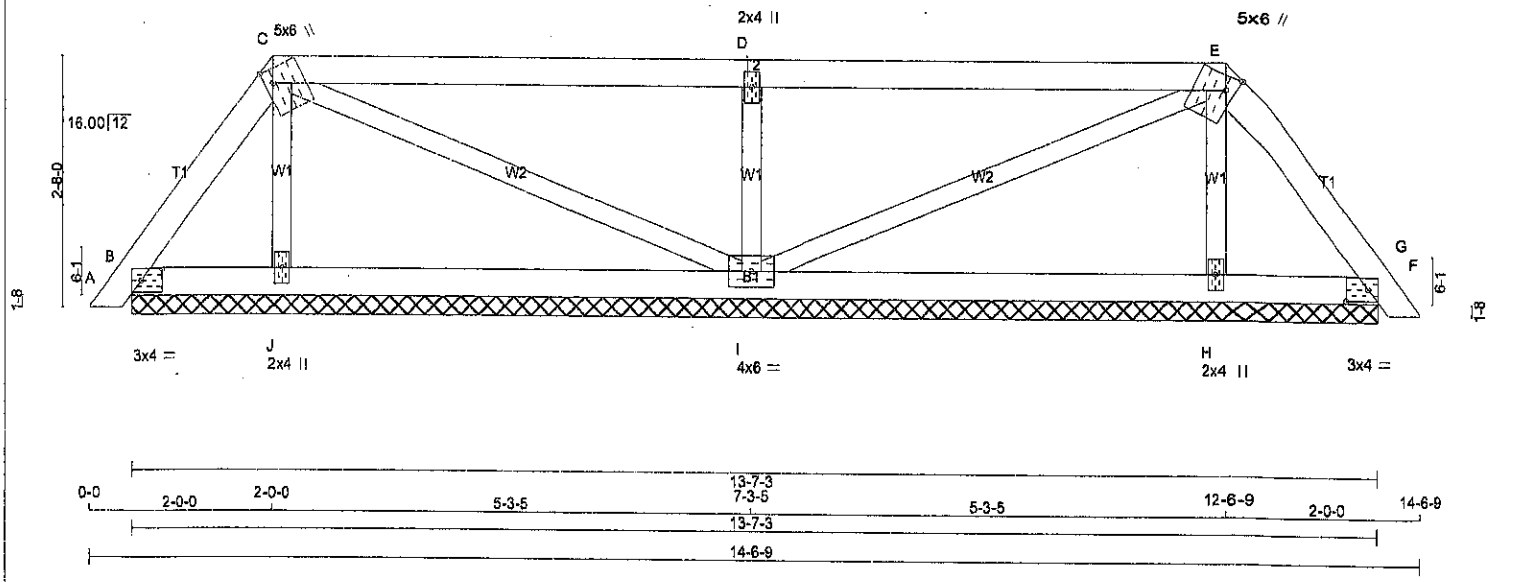

TOTAL WEIGHT = 2 X 49 = 97 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	129	0	129	0	0	13-7-3	1-8
F	129	0	129	0	0	13-7-3	1-8
J	205	0	205	0	0	13-7-3	1-8
I	578	0	578	0	0	13-7-3	1-8
H	205	0	205	0	0	13-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	87	79 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
F	87	79 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
J	148	78 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
I	405	287 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
H	148	78 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 8	-70.5	-70.5	0.01 (1)	10.00		J- C	-131 / 0	0.02 (1)
B- C	-66 / 0	-70.5	-70.5	0.03 (1)	6.25		C- I	-23 / 0	0.01 (1)
C- D	-9 / 0	-70.5	-70.5	0.33 (1)	10.00		I- D	-464 / 0	0.07 (1)
D- E	-9 / 0	-70.5	-70.5	0.33 (1)	10.00		I- E	-23 / 0	0.01 (1)
E- F	-66 / 0	-70.5	-70.5	0.03 (1)	6.25		H- E	-131 / 0	0.02 (1)
F- G	0 / 8	-70.5	-70.5	0.01 (1)	10.00				
B- J	0 / 37	-17.5	-17.5	0.07 (4)	10.00				
J- I	0 / 31	-17.5	-17.5	0.10 (4)	10.00				
I- H	0 / 31	-17.5	-17.5	0.10 (4)	10.00				
H- F	0 / 37	-17.5	-17.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33 (D-E:1) , BC=0.10 (H-I:4) , WB=0.07 (D-I:1) , SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (F) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-14-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-150735 82

253428	H06	6	1	JOB DESC. TRUSS DESC.	DRWG NO.

Alpa Roof Truss, Maple

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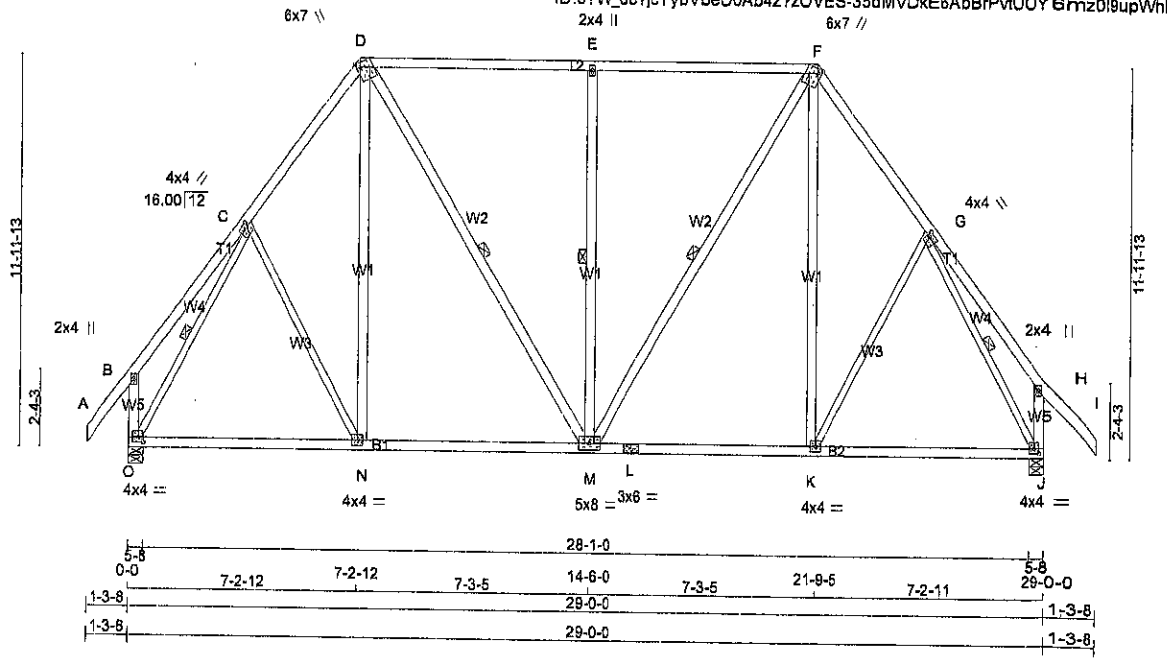

TOTAL WEIGHT = 6 X 181 = 1084 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - N	2x3	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF
O - C	2x3	DRY	No.2	SPF
G - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.00
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMWW-t	MT20	4.0	4.0	1.50	1.00
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	4.0	1.75	2.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	6.0	8.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1429	0	1429	0	0	5-8	1-9
J	1429	0	1429	0	0	5-8	1-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1010	668 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0
J	1010	668 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-M, E-M, F-M, C-O, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C- N	0 / 51	0.02 (4)
B- C	0 / 23	-70.5	-70.5	0.14 (1)	10.00	N- D	0 / 130	0.03 (4)
C- D	-1088 / 0	-70.5	-70.5	0.13 (1)	5.93	D- M	0 / 477	0.08 (1)
D- E	-889 / 0	-78.0	-78.0	0.55 (1)	5.67	M- E	-695 / 0	0.54 (1)
E- F	-889 / 0	-78.0	-78.0	0.55 (1)	5.67	M- F	0 / 477	0.08 (1)
F- G	-1088 / 0	-70.5	-70.5	0.13 (1)	5.93	K- F	0 / 130	0.03 (4)
G- H	0 / 23	-70.5	-70.5	0.14 (1)	10.00	K- G	0 / 51	0.02 (4)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O- C	-1338 / 0	0.52 (1)
O- B	-195 / 0	0.0	0.0	0.02 (1)	7.81	G- J	-1338 / 0	0.52 (1)
J- H	-195 / 0	0.0	0.0	0.02 (1)	7.81			
O- N	0 / 627	-17.5	-17.5	0.28 (4)	10.00			
N- M	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
M- L	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
L- K	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
K- J	0 / 627	-17.5	-17.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.55 (D-E:1) , BC=0.29 (M-N:4) , WB=0.54 (E-M:1) , SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.49 (G) (INPUT = 1.00)

A-1073581

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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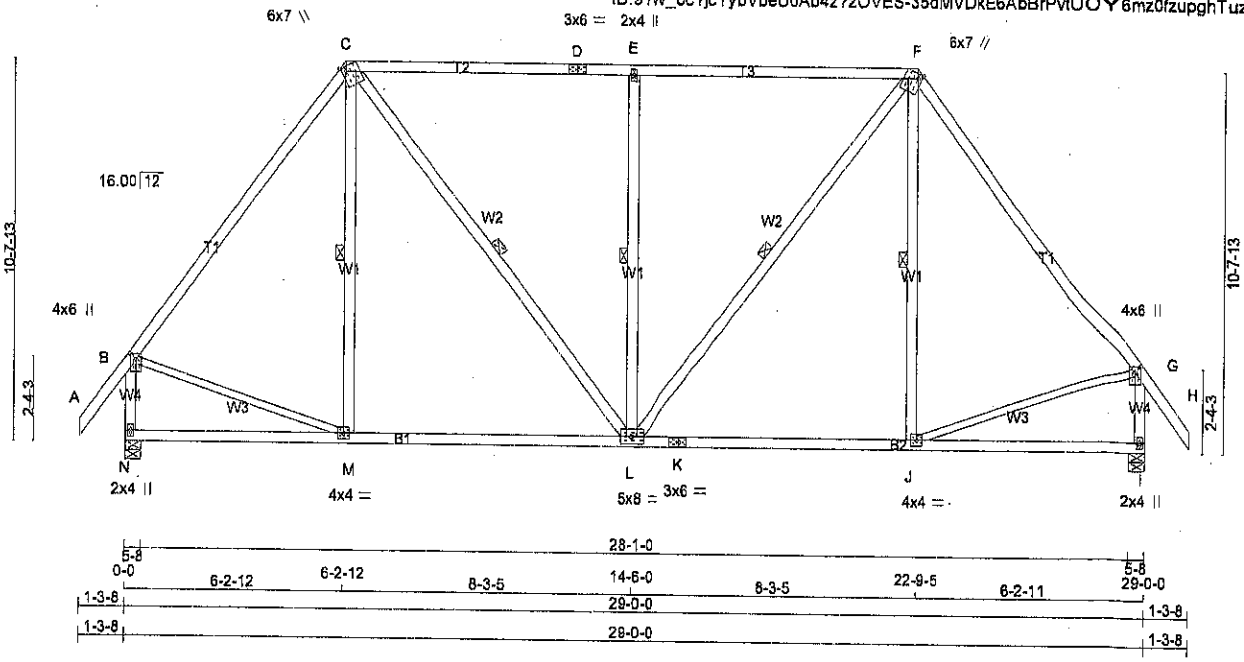


TOTAL WEIGHT = 2 X 160 = 319 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - M	2x3	DRY	No.2	SPF
J - G	2x3	DRY	No.2	SPF

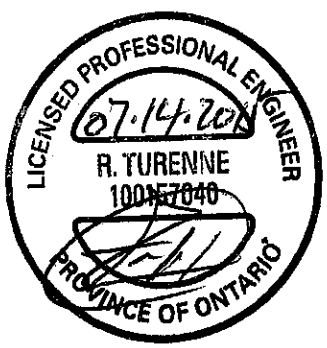
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	6.0	8.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1375	0	1375	0	0	5-8	1-8
I	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
I	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-M, C-L, E-L, F-L, F-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.07 (1)	10.00	M- C	-48 / 95	0.03 (1)
B- C	-1029 / 0	-70.5	-70.5	0.38 (1)	6.25	C- L	0 / 550	0.09 (1)
C- D	-959 / 0	-70.5	-70.5	0.95 (1)	4.39	L- E	-715 / 0	0.41 (1)
D- E	-959 / 0	-70.5	-70.5	0.96 (1)	4.39	L- F	0 / 550	0.09 (1)
E- F	-958 / 0	-70.5	-70.5	0.95 (1)	4.39	J- F	-48 / 95	0.03 (1)
F- G	-1029 / 0	-70.5	-70.5	0.38 (1)	6.25	B- M	0 / 644	0.14 (1)
G- H	0 / 40	-70.5	-70.5	0.07 (1)	10.00	J- G	0 / 644	0.14 (1)
N- B	-1333 / 0	0.0	0.0	0.17 (1)	7.01			
I- G	-1333 / 0	0.0	0.0	0.17 (1)	7.01			
N- M	0 / 0	-17.5	-17.5	0.21 (4)	10.00			
M- L	0 / 615	-17.5	-17.5	0.28 (4)	10.00			
L- K	0 / 615	-17.5	-17.5	0.28 (4)	10.00			
K- J	0 / 615	-17.5	-17.5	0.28 (4)	10.00			
J- I	0 / 0	-17.5	-17.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.95 (C-E:1) , BC=0.28 (L-M:4) , WB=0.41 (E-L:1) , SSI=0.28 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (M) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

A-1507 3580

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H04	2	1	TRUSS DESC.	

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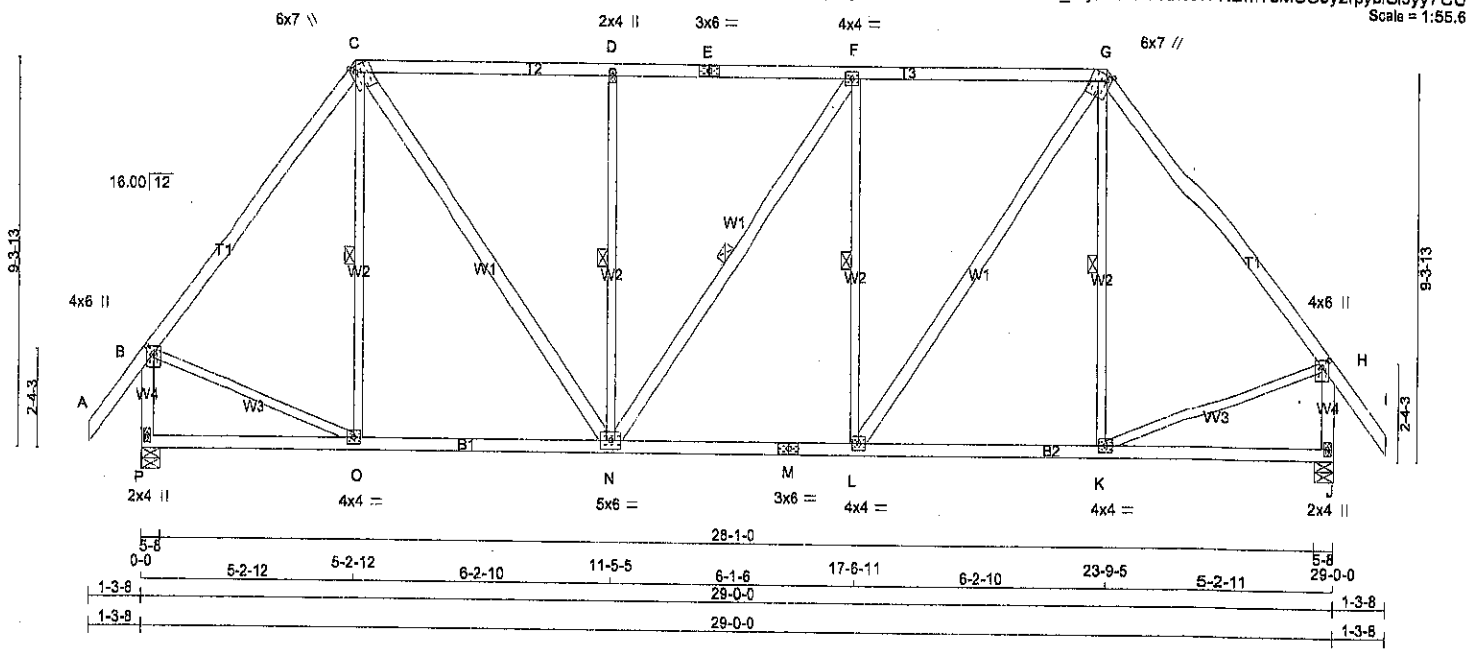

TOTAL WEIGHT = 2 X 158 = 315 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, L, O						
K	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-O, D-N, F-N, F-L, G-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	1	0.10	10.00
B-C	-1033 / 0	-70.5	-70.5	1	0.43	5.66
C-D	-1013 / 0	-70.5	-70.5	1	0.40	5.66
D-E	-1013 / 0	-70.5	-70.5	1	0.41	5.66
E-F	-1013 / 0	-70.5	-70.5	1	0.41	5.66
F-G	-1014 / 0	-70.5	-70.5	1	0.41	5.66
G-H	-1033 / 0	-70.5	-70.5	1	0.43	5.66
H-I	0 / 40	-70.5	-70.5	1	0.10	10.00
P-B	-1339 / 0	0.0	0.0	1	0.17	7.00
J-H	-1338 / 0	0.0	0.0	1	0.17	7.00
P-O	0 / 0	-17.5	-17.5	4	0.13	10.00
O-N	0 / 616	-17.5	-17.5	4	0.18	10.00
N-M	0 / 1014	-17.5	-17.5	1	0.23	10.00
M-L	0 / 1014	-17.5	-17.5	1	0.23	10.00
L-K	0 / 616	-17.5	-17.5	4	0.18	10.00
K-J	0 / 0	-17.5	-17.5	4	0.13	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
O-C	-114 / 51	0.07 (1)
C-N	0 / 696	0.11 (1)
N-D	-468 / 0	0.27 (1)
N-F	-2 / 0	0.00 (1)
L-F	-468 / 0	0.27 (1)
L-G	0 / 700	0.11 (1)
K-G	-115 / 51	0.07 (1)
B-O	0 / 657	0.15 (1)
K-H	0 / 657	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43 (B-C:1) , BC=0.23 (L-N:1) , WB=0.27 (F-L:1) , SSI=0.20 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.28 (M) (INPUT = 1.00)

A-150735 79

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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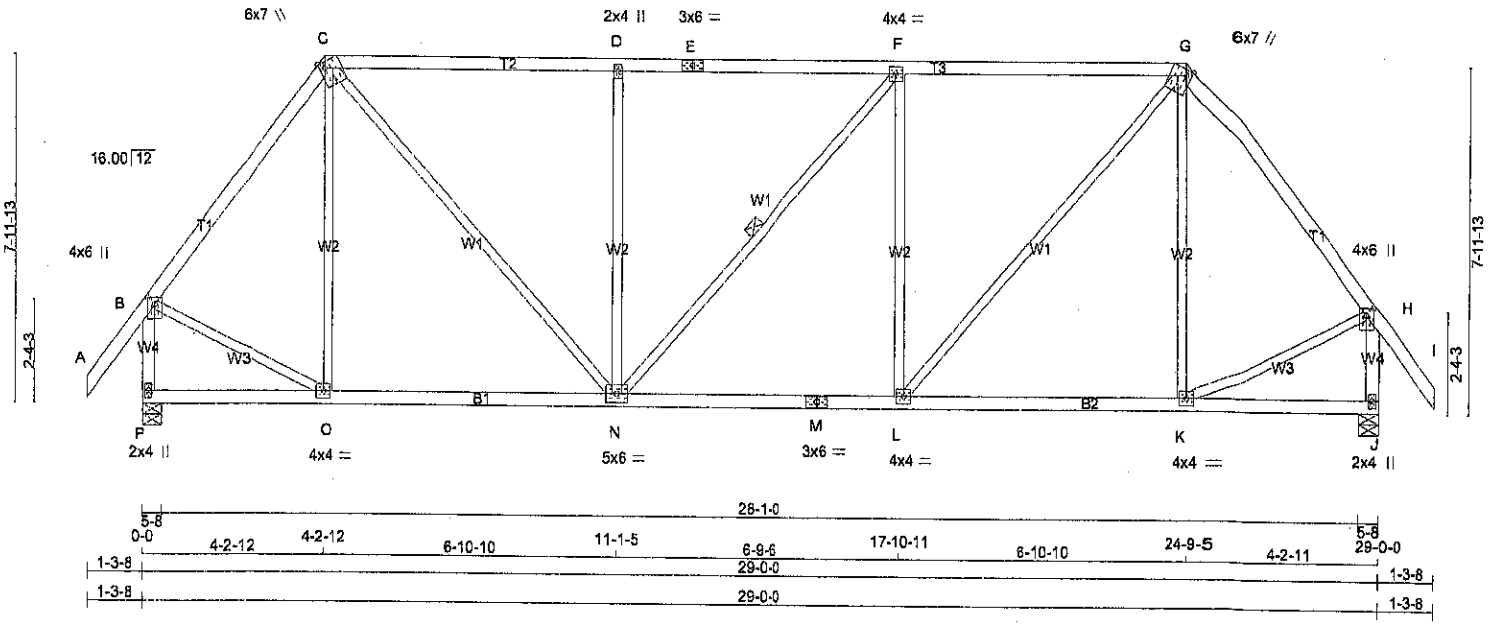

TOTAL WEIGHT = 2 X 138 = 277 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, L, O						
K	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-1027 / 0	-70.5	-70.5	0.26 (1)	5.88
C- D	-1186 / 0	-70.5	-70.5	0.52 (1)	5.18
D- E	-1186 / 0	-70.5	-70.5	0.52 (1)	5.18
E- F	-1186 / 0	-70.5	-70.5	0.52 (1)	5.18
F- G	-1186 / 0	-70.5	-70.5	0.53 (1)	5.18
G- H	-1027 / 0	-70.5	-70.5	0.26 (1)	5.88
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00
P- B	-1350 / 0	0.0	0.0	0.17 (1)	6.98
J- H	-1350 / 0	0.0	0.0	0.17 (1)	6.98
P- O	0 / 0	-17.5	-17.5	0.13 (4)	10.00
O- N	0 / 611	-17.5	-17.5	0.21 (4)	10.00
N- M	0 / 1187	-17.5	-17.5	0.28 (1)	10.00
M- L	0 / 1187	-17.5	-17.5	0.28 (1)	10.00
L- K	0 / 611	-17.5	-17.5	0.21 (4)	10.00
K- J	0 / 0	-17.5	-17.5	0.13 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-167 / 31	0.20 (1)
C- N	0 / 859	0.19 (1)
N- D	-519 / 0	0.63 (1)
N- F	-1 / 0	0.00 (1)
L- F	-519 / 0	0.63 (1)
L- G	0 / 861	0.19 (1)
K- G	-168 / 31	0.20 (1)
B- O	0 / 673	0.15 (1)
K- H	0 / 673	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.53 (F-G:1) , BC=0.28 (L-N:1) , WB=0.63 (F-L:1) , SSI=0.23 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.34 (M) (INPUT = 1.00)

A-K073578

		QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:28:30 2015 Page 1
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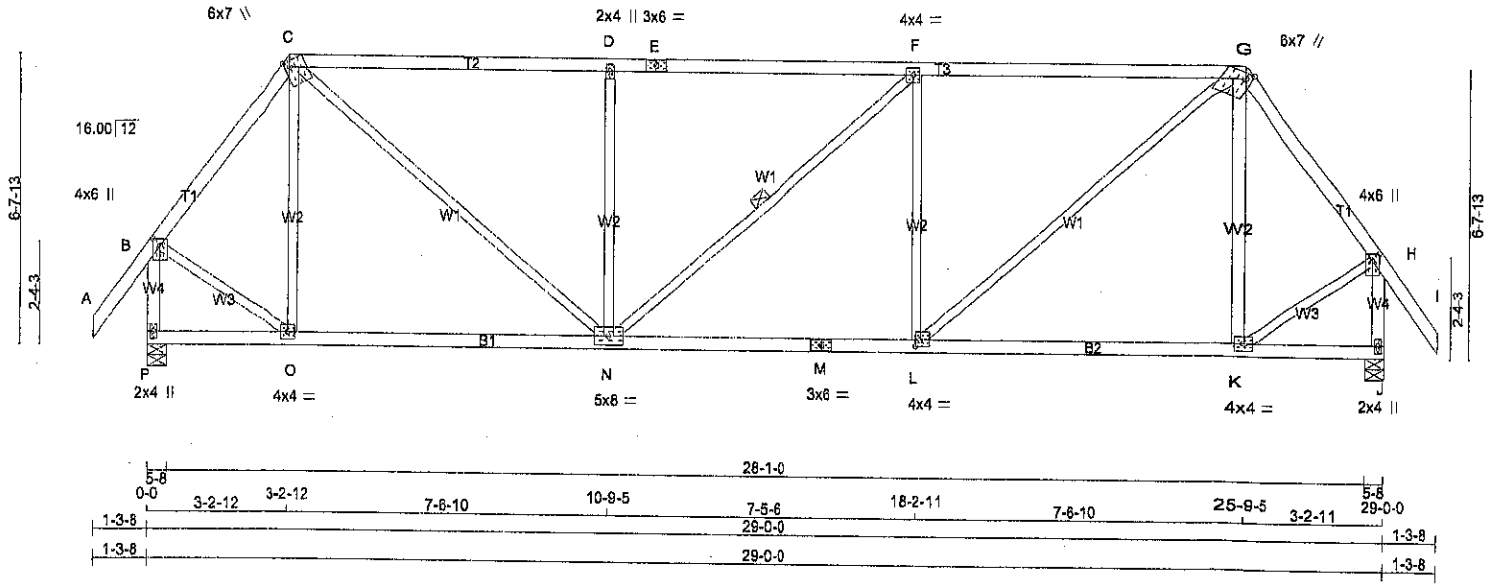

TOTAL WEIGHT = 2 X 130 = 260 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
B- C	-999 / 0	-70.5	-70.5	0.15 (1)	6.09	
C- D	-1430 / 0	-70.5	-70.5	0.68 (1)	4.60	
D- E	-1431 / 0	-70.5	-70.5	0.68 (1)	4.59	
E- F	-1431 / 0	-70.5	-70.5	0.68 (1)	4.59	
F- G	-1431 / 0	-70.5	-70.5	0.68 (1)	4.60	
G- H	-998 / 0	-70.5	-70.5	0.15 (1)	6.09	
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
P- B	-1365 / 0	0.0	0.0	0.17 (1)	6.95	
J- H	-1365 / 0	0.0	0.0	0.17 (1)	6.95	
P- O	0 / 0	-17.5	-17.5	0.15 (4)	10.00	
O- N	0 / 591	-17.5	-17.5	0.24 (4)	10.00	
N- M	0 / 1432	-17.5	-17.5	0.34 (1)	10.00	
M- L	0 / 1432	-17.5	-17.5	0.34 (1)	10.00	
L- K	0 / 591	-17.5	-17.5	0.24 (4)	10.00	
K- J	0 / 0	-17.5	-17.5	0.15 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-237 / 8	0.17 (1)
C- N	0 / 1094	0.25 (1)
N- D	-570 / 0	0.42 (1)
N- F	-1 / 0	0.00 (1)
L- F	-570 / 0	0.42 (1)
L- G	0 / 1096	0.25 (1)
K- G	-238 / 8	0.17 (1)
B- O	0 / 690	0.16 (1)
K- H	0 / 690	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68 (F-G:1) , BC=0.34 (L-N:1) , WB=0.42 (F-L:1) , SSI=0.25 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)

A-A073577

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	J01C	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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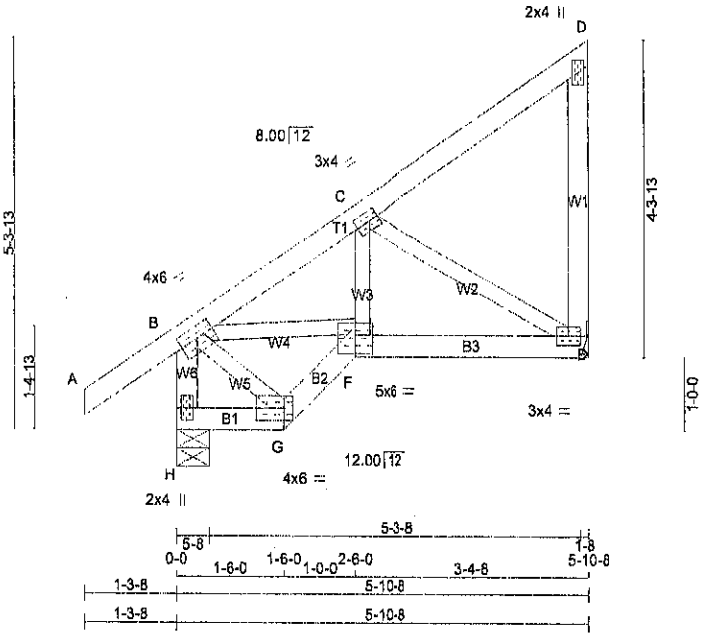
Scale: 3/8"=1'



TOTAL WEIGHT = 7 X 30 = 208 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-t	MT20	5.0	6.0	3.00	3.00
G	BBW-t	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
H- B	-342 / 0	0.0	0.0	0.04 (1)		7.81
A- B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B- C	-287 / 0	-70.5	-70.5	0.10 (1)		6.25
C- D	-12 / 0	-70.5	-70.5	0.10 (1)		6.25
E- D	-93 / 0	0.0	0.0	0.03 (1)		7.81
H- G	0 / 0	-17.5	-17.5	0.01 (4)		10.00
G- F	0 / 20	-17.5	-17.5	0.01 (4)		10.00
F- E	0 / 248	-17.5	-17.5	0.08 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- G	0 / 18	0.01 (4)
B- F	0 / 238	0.05 (1)
F- C	0 / 67	0.02 (4)
C- E	-284 / 0	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.08 (E-F:4) , WB=0.07 (C-E:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-14-2015
R. TURENNE
100187040
PROVINCE OF ONTARIO

A-1507357Y

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

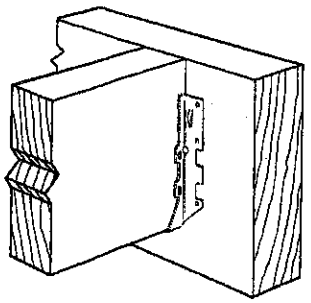
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

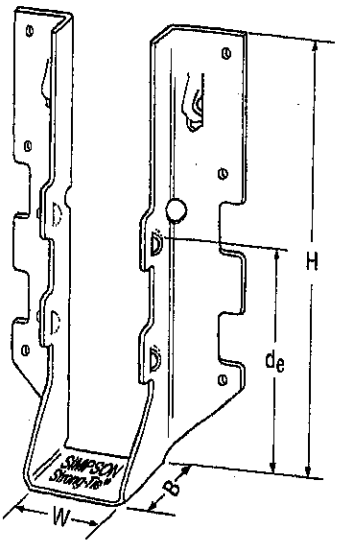
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



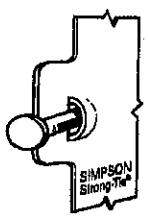
Typical LUS Installation



LUS28

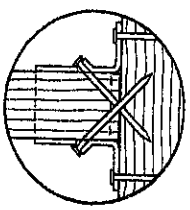
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

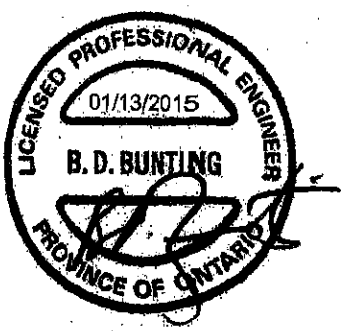


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

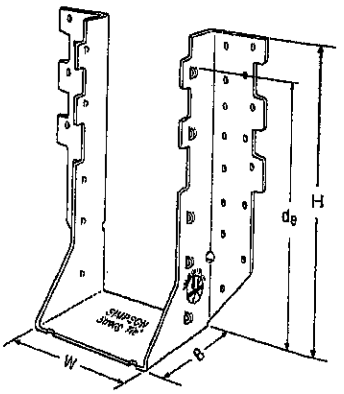
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

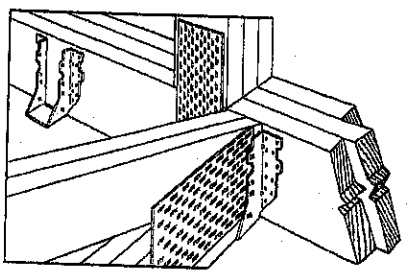
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



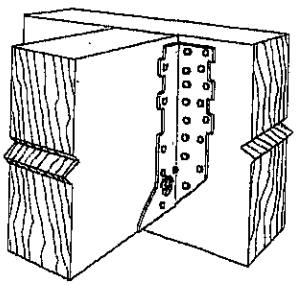
HHUS410



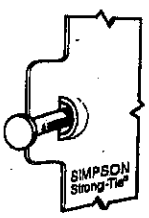
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 1/32	3	6 5/32	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	30-16d	10-16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 11/32	3	3 15/16	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3 5/8	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	30-16d	10-16d	4745	10770	4310	7650

1. d_e is the distance from the seat of the hanger to the highest joist nail.

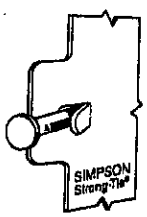


Typical HHUS Installation

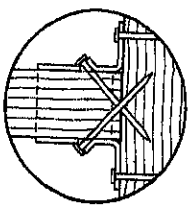


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

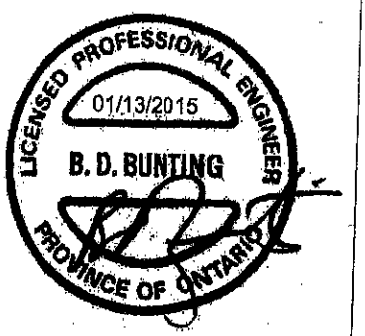
U.S. Patent 5,603,580



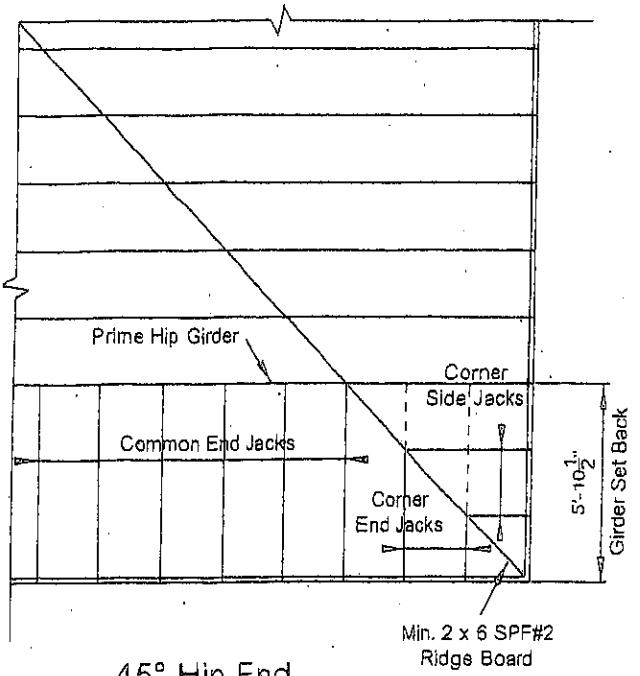
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



STRACON ENGINEERING INC.



45° Hip End

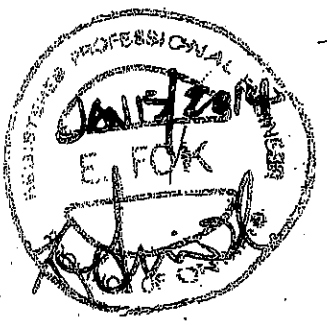
LUMBER SPECIFICATION

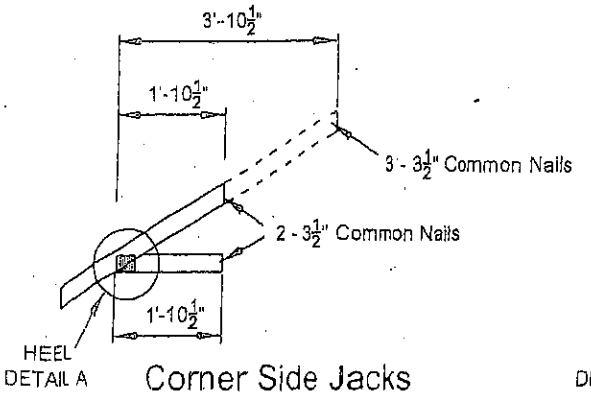
TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

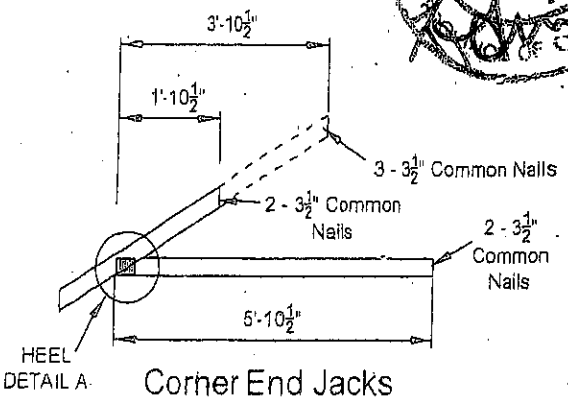
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

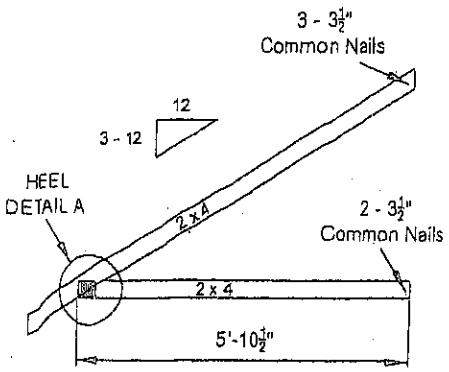




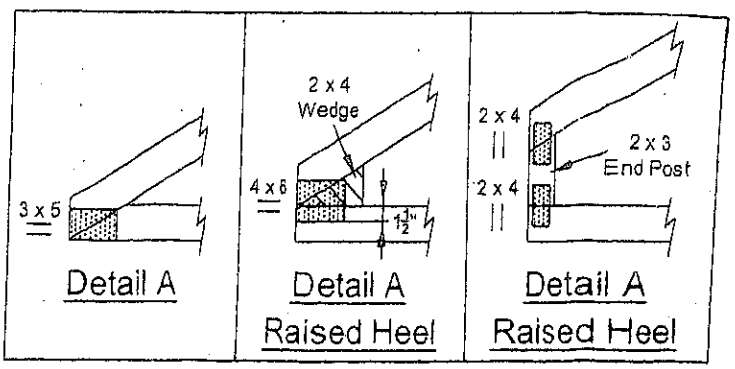
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008