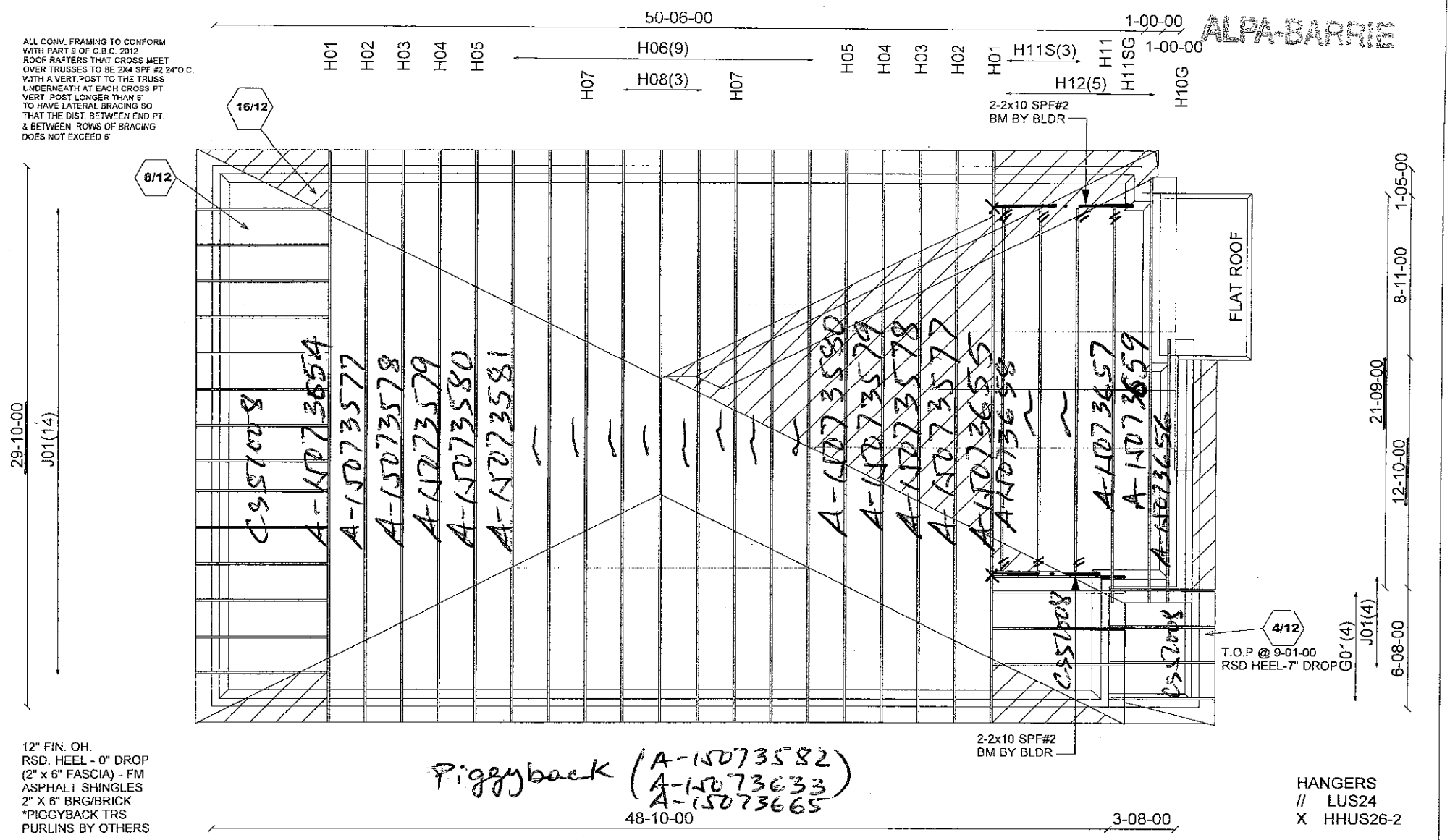
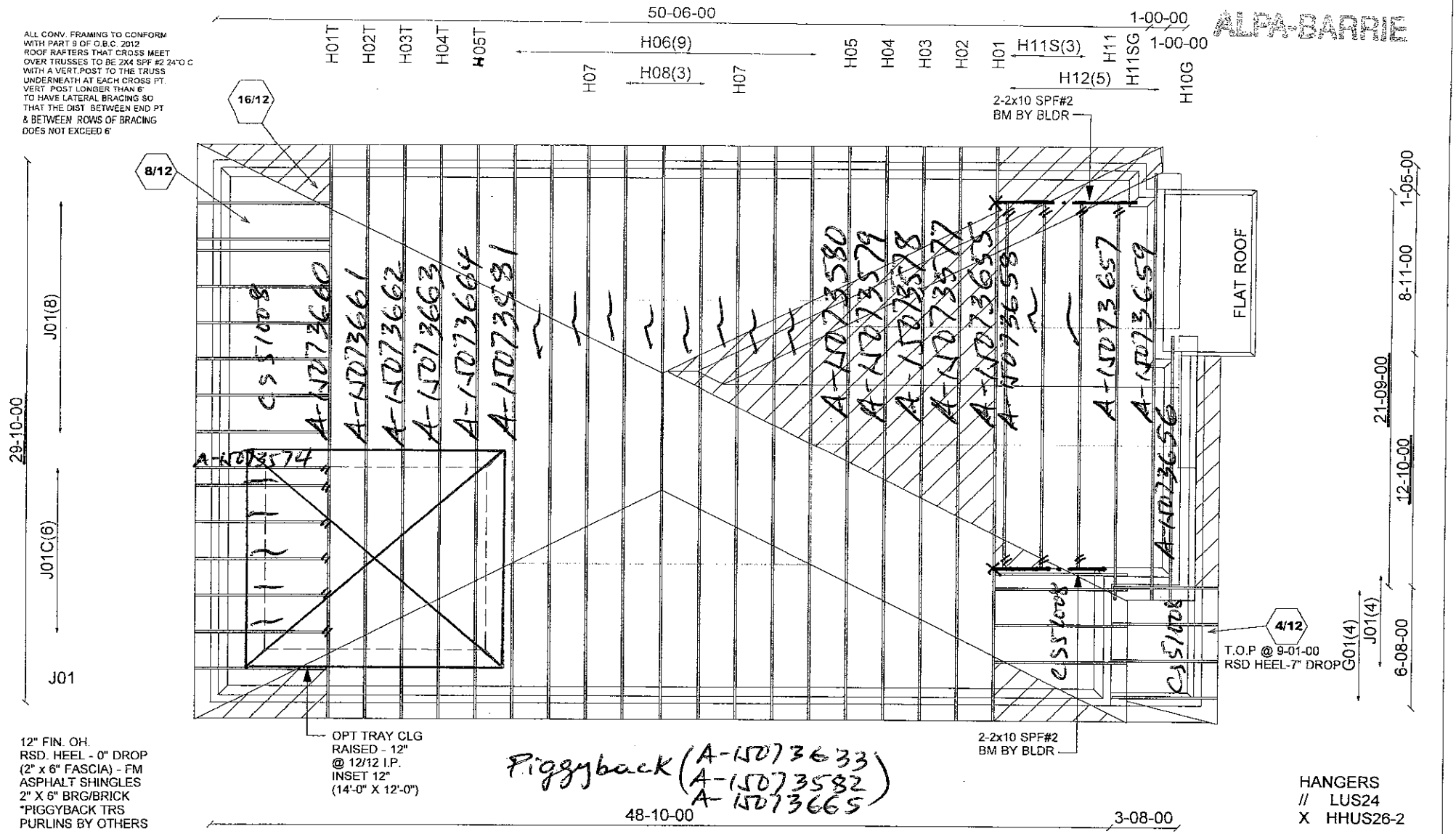


PL 80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-4 / A	
Layout ID: 253438	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015	Designer: cheryle	

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

Piggyback (A-150736 33, A-150735 82, A-150736 65)

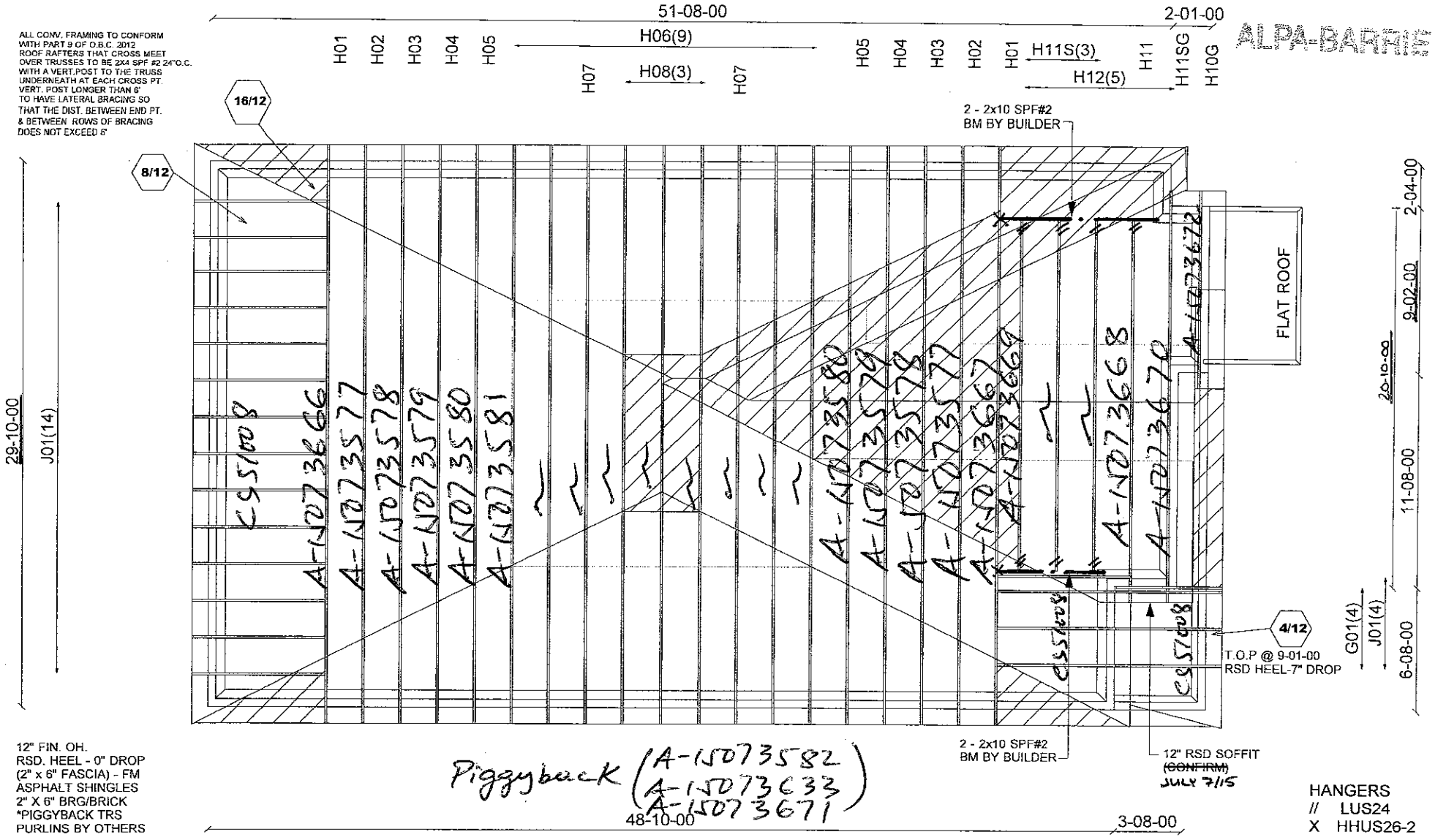
HANGERS
// LUS24
X HHUS26-2

CONVENTIONAL FRAMING BY OTHERS

PL80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-4 / A OPT TRAY
Layout ID: 253439	Project: HUNTINGTON & NASHVILLE	
Plan Log: 79711	Date: 4/22/2015 Designer: cheryle	

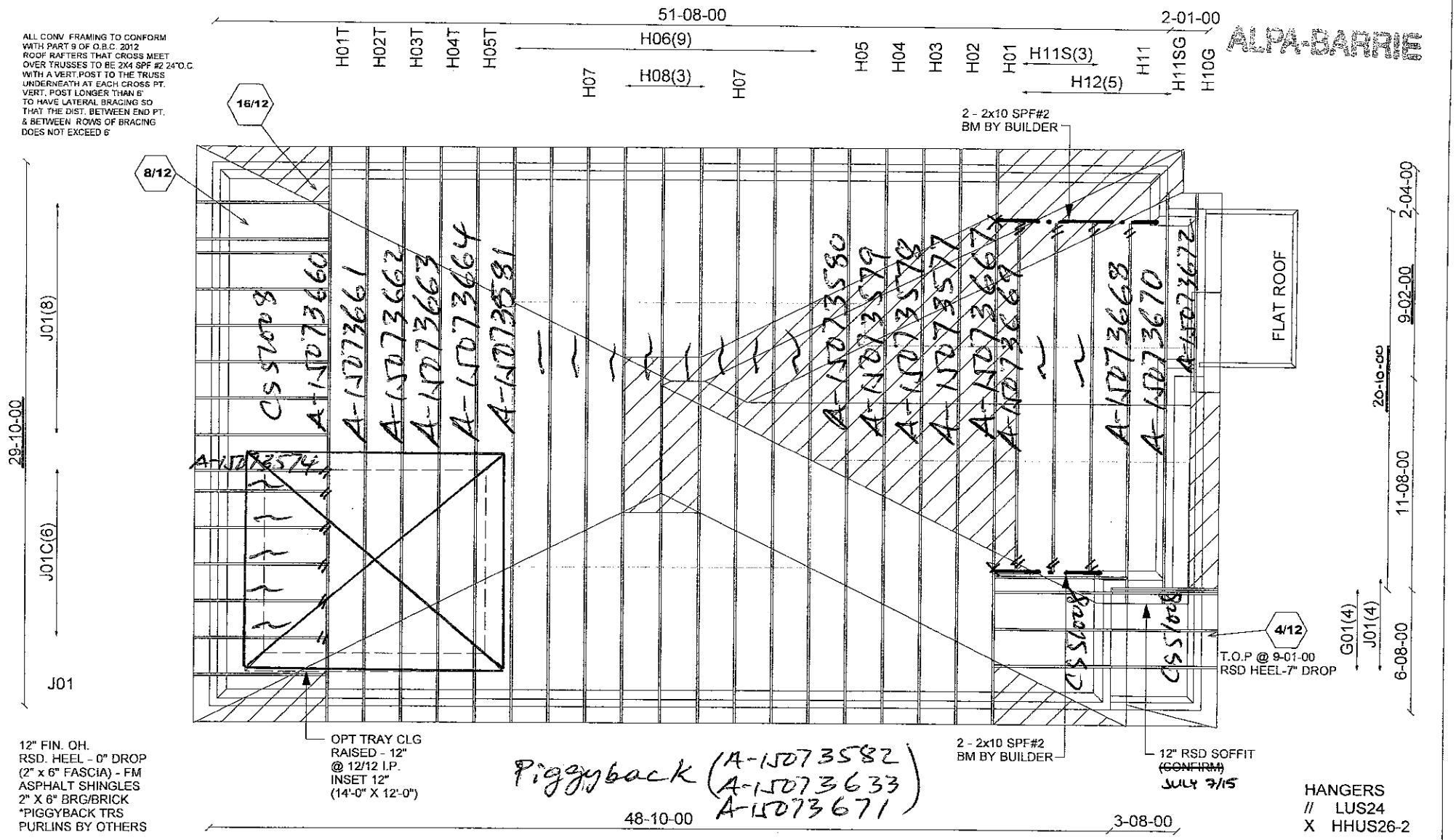


PL80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-4 / B	
Layout ID: 253440	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015	Designer: cheryle	

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH GROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

Piggyback (A-15073582 A-15073633 A-15073671)

HANGERS
// LUS24
X HHUS26-2

CONVENTIONAL FRAMING BY OTHERS

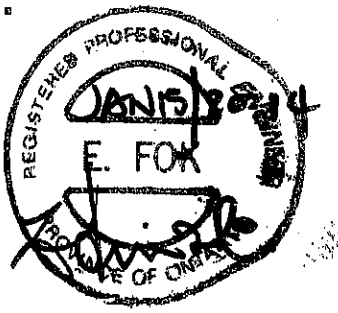
PL80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-4 / B OPT TRAY
Layout ID: 253441	Project: HUNTINGTON & NASHVILLE	
Plan Log: 79711	Date: 4/22/2015 Designer: cheryle	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253440	H10G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 14:07:19 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-UYlxD7NGNL8SDMzjx1iUVN6FkwwH6ef1HkuN8ayxpIc

3x5 II

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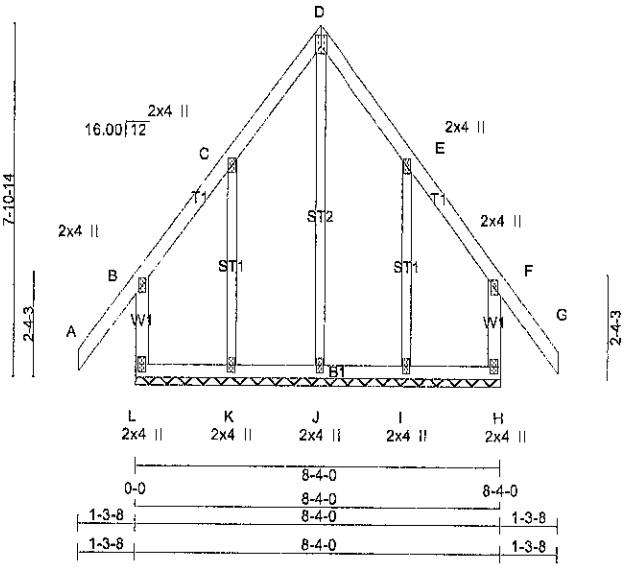

TOTAL WEIGHT = 49 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	5.0	2.00	Edge
E	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	2.0	4.0		
H	BMV1+p	MT20	2.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
L- B	-186 / 0	0.0	0.0	0.02 (1)		7.81
A- B	0 / 40	-70.5	-70.5	0.10 (1)		10.00
B- C	-18 / 0	-70.5	-70.5	0.04 (1)		6.25
C- D	-11 / 0	-70.5	-70.5	0.04 (1)		6.25
D- E	-11 / 0	-70.5	-70.5	0.04 (1)		6.25
E- F	-18 / 0	-70.5	-70.5	0.04 (1)		6.25
F- G	0 / 40	-70.5	-70.5	0.10 (1)		10.00
H- F	-186 / 0	0.0	0.0	0.02 (1)		7.81
L- K	0 / 11	-17.5	-17.5	0.02 (4)		10.00
K- J	0 / 7	-17.5	-17.5	0.02 (4)		10.00
J- I	0 / 7	-17.5	-17.5	0.02 (4)		10.00
I- H	0 / 11	-17.5	-17.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-131 / 0	0.14 (1)
K- C	-141 / 0	0.06 (1)
I- E	-141 / 0	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1) , BC=0.02 (H-I:4) , WB=0.14 (D-J:1) , SSI=0.05 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (F) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)

A-1507367Z

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253440	H12	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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3x5 ||

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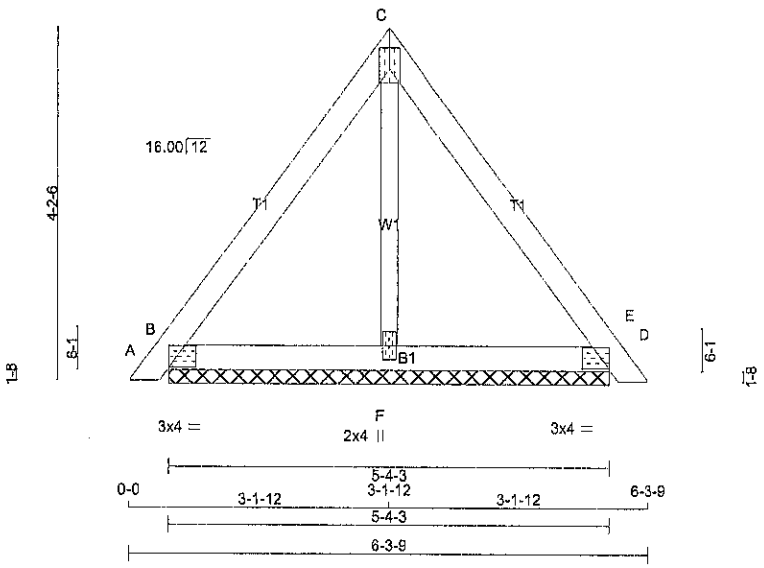

TOTAL WEIGHT = 5 X 21 = 104 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	189	0	189	0	0	5-4-3	1-8
D	189	0	189	0	0	5-4-3	1-8
F	143	0	143	0	0	5-4-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	130	102 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	130	102 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
F	104	50 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	F- C	-84 / 0	0.02 (1)
B- C	-84 / 0	-70.5	-70.5	0.09 (1)	6.25			
C- D	-84 / 0	-70.5	-70.5	0.09 (1)	6.25			
D- E	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B- F	0 / 38	-17.5	-17.5	0.04 (4)	10.00			
F- D	0 / 38	-17.5	-17.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09 (B-C:1) , BC=0.04 (B-F:4) , WB=0.02 (C-F:1) , SSI=0.04 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/15/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-4073671

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253440	H11SG	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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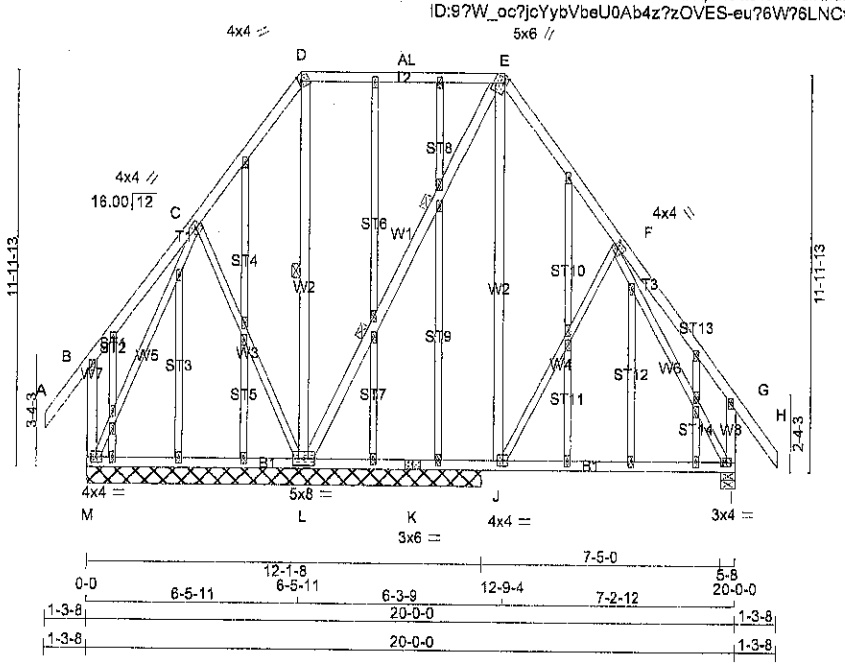

TOTAL WEIGHT = 180 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - L	2x3	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
F - I	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0	1.75	1.00
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW1-t	MT20	5.0	8.0		
M	BMVW1-t	MT20	4.0	4.0		
N, O, P, Q, R, S, T, U, V, V, W, X, Y, Z, AA, AA, AB, AC, AD, AE, AF, AF, AG, AH, AI, AJ, AK, AK						
N	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	952	0	952	0	0	12-1-8 (10-0-0)	8
M	366	0	366	0	0	12-1-8 (10-0-0)	8
I	686	0	686	0	0	5-8	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	675	436 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0
M	256	187 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
I	482	335 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-L.
2 LATERAL BRACE(S) REQUIRED AT 1/ 3 LENGTH OF E-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C- L	-158 / 0	0.22 (1)
B- C	0 / 23	-70.5	-70.5	0.13 (1)	10.00	L- D	-338 / 0	0.26 (1)
C- D	-13 / 0	-70.5	-70.5	0.10 (1)	6.25	L- E	-396 / 0	0.17 (1)
D-AL	0 / 11	-78.0	-78.0	0.53 (1)	10.00	J- E	0 / 236	0.04 (1)
AL- E	0 / 11	-78.0	-78.0	0.53 (1)	10.00	J- F	-123 / 0	0.15 (1)
E- F	-312 / 0	-70.5	-70.5	0.12 (1)	6.25	M- C	-146 / 0	0.19 (1)
F- G	0 / 25	-70.5	-70.5	0.15 (1)	10.00	F- I	-497 / 0	0.57 (1)
G- H	0 / 40	-70.5	-70.5	0.10 (1)	10.00			
M- B	-186 / 0	0.0	0.0	0.04 (1)	7.81			
I- G	-193 / 0	0.0	0.0	0.02 (1)	7.81			
M- L	0 / 59	-17.5	-17.5	0.16 (4)	10.00			
L- K	0 / 175	-17.5	-17.5	0.21 (4)	10.00			
K- J	0 / 175	-17.5	-17.5	0.21 (4)	10.00			
J- I	0 / 233	-17.5	-17.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.53 (D-E:1) , BC=0.21 (J-L:4) , WB=0.57 (F-I:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (I) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/15/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-N073670

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253440	H11S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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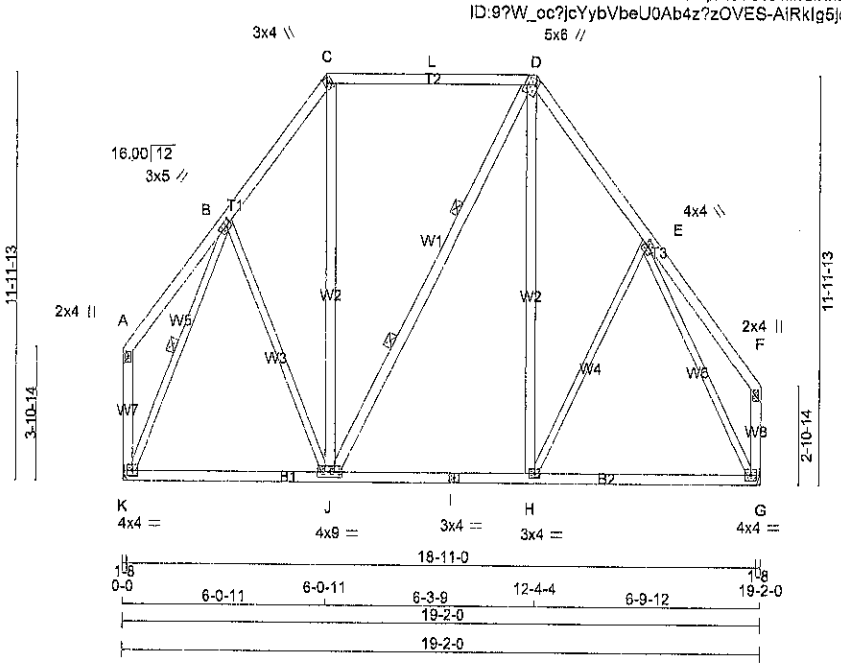

TOTAL WEIGHT = 3 X 127 = 380 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - J	2x3	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF
K - B	2x3	DRY	No.2	SPF
E - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+m	MT20	3.0	4.0	1.75	1.00
D	TTWW+m	MT20	5.0	6.0	1.75	1.00
E	TMWW-t	MT20	4.0	4.0	1.75	1.25
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWW-t	MT20	3.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	867	0	867	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	866	0	866	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	613	402 / 0	0 / 0	0 / 0	0 / 0	211 / 0	0 / 0
G	612	402 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2 LATERAL BRACE(S) REQUIRED AT 1/ 3 LENGTH OF D-J.
1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF B-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 19	-70.5	-70.5	0.10 (1)	10.00	B- J	0 / 54	0.02 (4)
B- C	-553 / 0	-70.5	-70.5	0.08 (1)	6.25	J- C	0 / 102	0.02 (4)
C- L	-319 / 0	-78.0	-78.0	0.53 (1)	6.25	J- D	-41 / 0	0.02 (1)
L- D	-319 / 0	-78.0	-78.0	0.53 (1)	6.25	H- D	0 / 149	0.03 (4)
D- E	-586 / 0	-70.5	-70.5	0.10 (1)	6.25	H- E	-25 / 31	0.03 (1)
E- F	0 / 22	-70.5	-70.5	0.13 (1)	10.00	K- B	-796 / 0	0.35 (1)
K- A	-85 / 0	0.0	0.0	0.02 (1)	7.81	E- G	-805 / 0	0.98 (1)
G- F	-89 / 0	0.0	0.0	0.01 (1)	7.81			
K- J	0 / 298	-17.5	-17.5	0.17 (4)	10.00			
J- I	0 / 338	-17.5	-17.5	0.21 (4)	10.00			
I- H	0 / 338	-17.5	-17.5	0.21 (4)	10.00			
H- G	0 / 348	-17.5	-17.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.53 (C-D:1) , BC=0.21 (H-J:4) , WB=0.98 (E-G:1) , SSI=0.19 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

A-1073669

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253440	H11	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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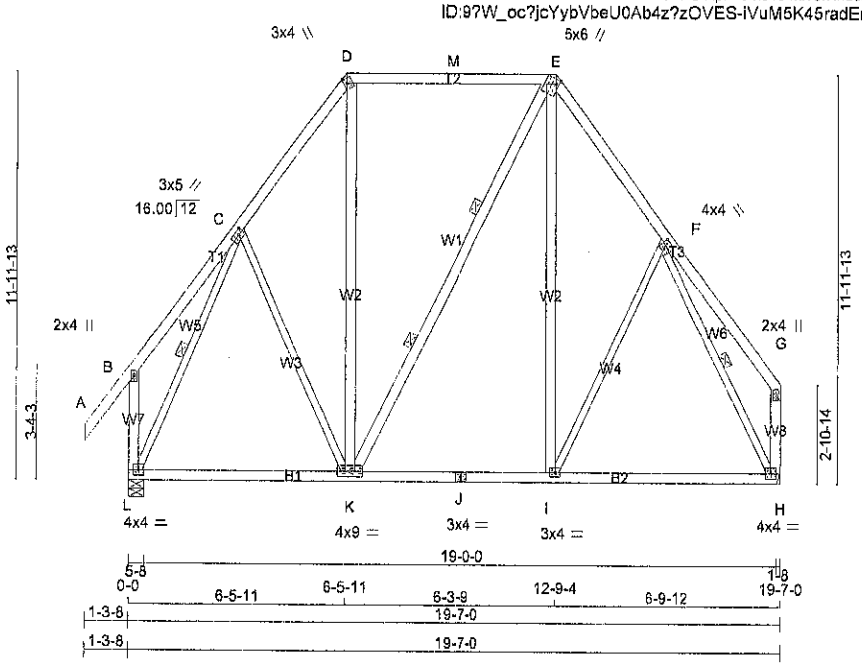

TOTAL WEIGHT = 130 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
I - F	2x3	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
F - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+m	MT20	3.0	4.0	1.75	1.00
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMWW-t	MT20	4.0	4.0	1.75	1.25
G	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	4.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	984	0	984	0	0	5-8	1-8
H	884	0	884	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	694	470 / 0	0 / 0	0 / 0	0 / 0	223 / 0	0 / 0
H	625	411 / 0	0 / 0	0 / 0	0 / 0	214 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2 LATERAL BRACE(S) REQUIRED AT 1/ 3 LENGTH OF E-K.
1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-L, F-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00		C-K	0 / 42	0.01 (4)
B-C	0 / 20	-70.5	-70.5	0.12 (1)	10.00		K-D	0 / 125	0.03 (4)
C-D	-590 / 0	-70.5	-70.5	0.09 (1)	6.25		K-E	-18 / 0	0.01 (1)
D-M	-341 / 0	-78.0	-78.0	0.53 (1)	6.25		I-E	0 / 143	0.03 (4)
M-E	-341 / 0	-78.0	-78.0	0.53 (1)	6.25		I-F	-20 / 32	0.03 (1)
E-F	-604 / 0	-70.5	-70.5	0.10 (1)	6.25		L-C	-819 / 0	0.34 (1)
F-G	0 / 22	-70.5	-70.5	0.13 (1)	10.00		F-H	-826 / 0	0.33 (1)
L-B	-189 / 0	0.0	0.0	0.04 (1)	7.81				
H-G	-89 / 0	0.0	0.0	0.01 (1)	7.81				
L-K	0 / 334	-17.5	-17.5	0.19 (4)	10.00				
K-J	0 / 349	-17.5	-17.5	0.21 (4)	10.00				
J-I	0 / 349	-17.5	-17.5	0.21 (4)	10.00				
I-H	0 / 357	-17.5	-17.5	0.21 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.53 (D-E:1) , BC=0.21 (I-K:4) , WB=0.34 (C-L:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.31 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073668

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253440	H01-Cond2	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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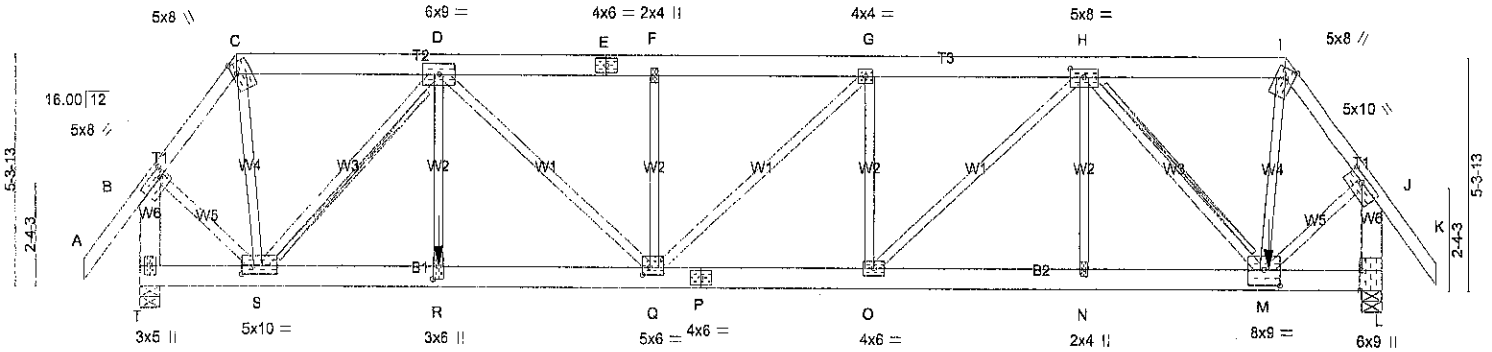

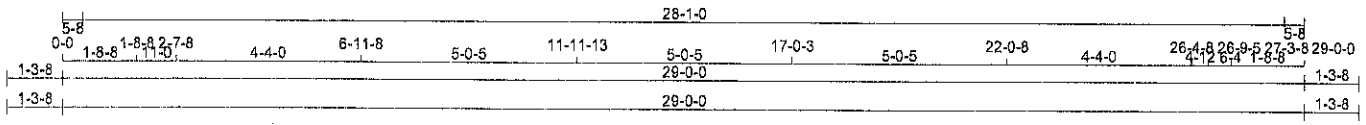

TOTAL WEIGHT = 169 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TTW+m	MT20	5.0	8.0	3.00	1.25
D	TMWWW-t	MT20	6.0	9.0		
E	TS-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWWW-t	MT20	5.0	8.0	2.25	4.00
I	TTW+m	MT20	5.0	8.0	2.50	2.25
J	TMVW-t	MT20	5.0	10.0	1.50	4.25
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWWW-t	MT20	8.0	9.0	4.25	4.50
N	BMW+w	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0	2.00	2.00
R	BMW+w	MT20	3.0	6.0	3.50	1.50
S	BMWWW-t	MT20	5.0	10.0	2.25	3.25
T	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2614	0	2614	0	0	5-8	3-0
L	3617	0	3617	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1839	1259 / 0	0 / 0	0 / 0	0 / 0	580 / 0	0 / 0
L	2546	1738 / 0	0 / 0	0 / 0	0 / 0	808 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-S, H-M
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	
B- C	-2020 / 0	-70.5	-70.5	0.13 (1)	4.63	
C- D	-1360 / 0	-70.5	-70.5	0.13 (1)	6.25	
D- E	-3519 / 0	-70.5	-70.5	0.18 (1)	4.41	
E- F	-3519 / 0	-70.5	-70.5	0.18 (1)	4.41	
F- G	-3519 / 0	-70.5	-70.5	0.17 (1)	4.42	
G- H	-3379 / 0	-70.5	-70.5	0.17 (1)	4.49	
H- I	-1699 / 0	-70.5	-70.5	0.13 (1)	5.67	
I- J	-2805 / 0	-70.5	-70.5	0.18 (1)	3.99	
J- K	0 / 40	-70.5	-70.5	0.11 (1)	10.00	
T- B	-2621 / 0	0.0	0.0	0.23 (1)	6.38	
L- J	-3571 / 0	0.0	0.0	0.32 (1)	5.60	
T- S	0 / 0	-17.5	-17.5	0.06 (1)	10.00	
S- R	0 / 3158	-17.5	-17.5	0.49 (1)	10.00	
R- Q	0 / 3158	-17.5	-17.5	0.50 (1)	10.00	
Q- P	0 / 3379	-17.5	-17.5	0.47 (1)	10.00	
P- O	0 / 3379	-17.5	-17.5	0.47 (1)	10.00	
O- N	0 / 2796	-17.5	-17.5	0.40 (1)	10.00	
N- M	0 / 2796	-17.5	-17.5	0.42 (1)	10.00	
M- L	0 / 0	-17.5	-17.5	0.04 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- D	0 / 1371	0.34 (1)
D- Q	0 / 502	0.12 (1)
Q- F	-358 / 0	0.14 (1)
Q- G	0 / 195	0.05 (1)
O- G	-488 / 0	0.19 (1)
O- H	0 / 812	0.20 (1)
N- H	0 / 102	0.04 (4)
B- S	0 / 1465	0.36 (1)
M- J	0 / 2035	0.50 (1)
C- S	0 / 1397	0.35 (1)
M- I	0 / 2036	0.50 (1)
S- D	-2747 / 0	0.86 (1)
H- M	-1369 / 0	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
M	26-4-8	-2115	-2115	---	FRONT	VERT	TOTAL
R	6-11-8	-1367	-1367	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.32 (J-L:1) , BC=0.50 (Q-R:1) , WB=0.86 (D-S:1) , SSI=0.13 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.63 (P) (INPUT = 1.00)

A-4073667

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253440	H01-Cond1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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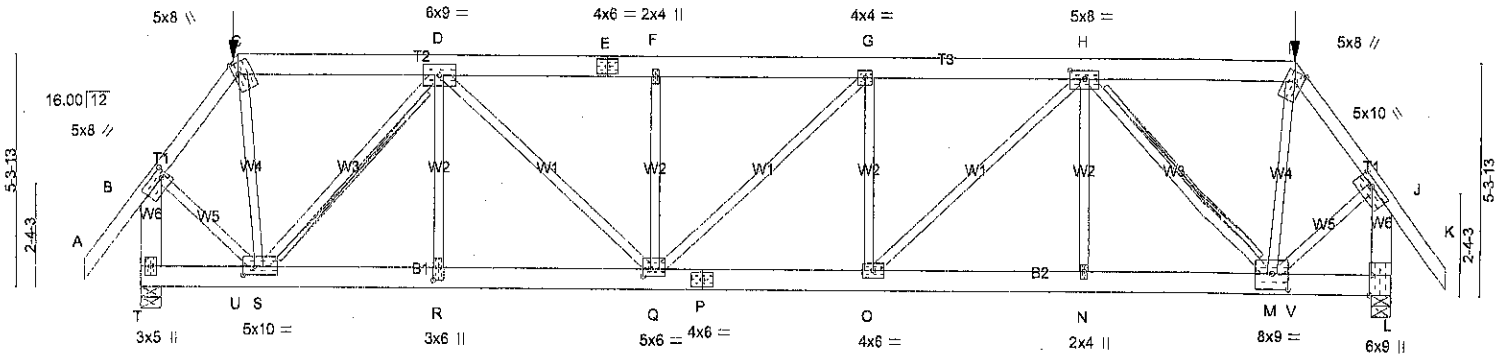

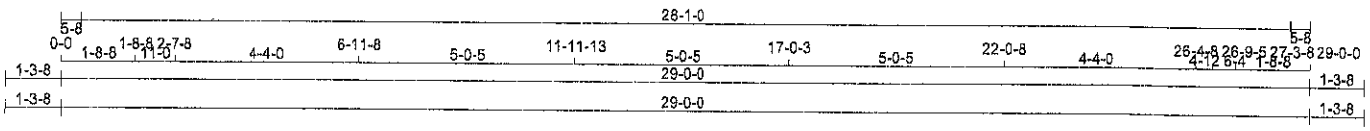

TOTAL WEIGHT = 169 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TTW+m	MT20	5.0	8.0	3.00	1.25
D	TMWWW-t	MT20	6.0	9.0		
E	TS-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWWW-t	MT20	5.0	8.0	2.25	4.00
I	TTW+m	MT20	5.0	8.0	2.50	2.25
J	TMVW-t	MT20	5.0	10.0	1.50	4.25
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWWW-t	MT20	8.0	9.0	4.25	4.50
N	BMW+w	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0	2.00	2.00
R	BMW+w	MT20	3.0	6.0	3.50	1.50
S	BMWWW-t	MT20	5.0	10.0	2.25	3.25
T	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2572	0	2572	0	0	5-8	2-14
L	2572	0	2572	0	0	5-8	2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0
L	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-S, H-M
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
FR-TO					
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00
B-C	-1956 / 0	-70.5	-70.5	0.12 (1)	4.70
C-D	-1291 / 0	-138.7	-138.7	0.24 (1)	6.25
D-E	-3700 / 0	-138.7	-138.7	0.33 (1)	4.15
E-F	-3700 / 0	-138.7	-138.7	0.33 (1)	4.15
F-G	-3700 / 0	-138.7	-138.7	0.28 (1)	4.21
G-H	-3709 / 0	-138.7	-138.7	0.32 (1)	4.15
H-I	-1291 / 0	-138.7	-138.7	0.23 (1)	6.25
I-J	-1956 / 0	-70.5	-70.5	0.12 (1)	4.70
J-K	0 / 40	-70.5	-70.5	0.11 (1)	10.00
T-B	-2542 / 0	0.0	0.0	0.23 (1)	6.46
L-J	-2543 / 0	0.0	0.0	0.23 (1)	6.46
T-U	0 / 0	-34.5	-34.5	0.05 (4)	10.00
U-S	0 / 0	-34.5	-34.5	0.05 (4)	10.00
S-R	0 / 2815	-34.5	-34.5	0.40 (1)	10.00
R-Q	0 / 2815	-34.5	-34.5	0.43 (1)	10.00
Q-P	0 / 3709	-34.5	-34.5	0.54 (1)	10.00
P-O	0 / 3709	-34.5	-34.5	0.54 (1)	10.00
O-N	0 / 2813	-34.5	-34.5	0.43 (1)	10.00
N-M	0 / 2813	-34.5	-34.5	0.40 (1)	10.00
M-V	0 / 0	-34.5	-34.5	0.05 (4)	10.00
V-L	0 / 0	-34.5	-34.5	0.05 (4)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
R-D	0 / 176	0.07 (4)
D-Q	0 / 1231	0.30 (1)
Q-F	-677 / 0	0.26 (1)
Q-G	-12 / 0	0.01 (1)
O-G	-682 / 0	0.26 (1)
O-H	0 / 1246	0.31 (1)
N-H	0 / 175	0.07 (4)
B-S	0 / 1418	0.35 (1)
M-J	0 / 1418	0.35 (1)
C-S	0 / 1113	0.28 (1)
M-I	0 / 1112	0.28 (1)
S-D	-2329 / 0	0.73 (1)
H-M	-2325 / 0	0.73 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
I	26-9-5	-115	-115	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.33 (D-F:1) , BC=0.54 (O-Q:1) , WB=0.73 (D-S:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.75 (P) (INPUT = 1.00)

4-150736666

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157048
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253440	H01-Cond1	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.5 lbs FACTORED DOWN AT 26-9-5, AND 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-4073666(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253439	H12	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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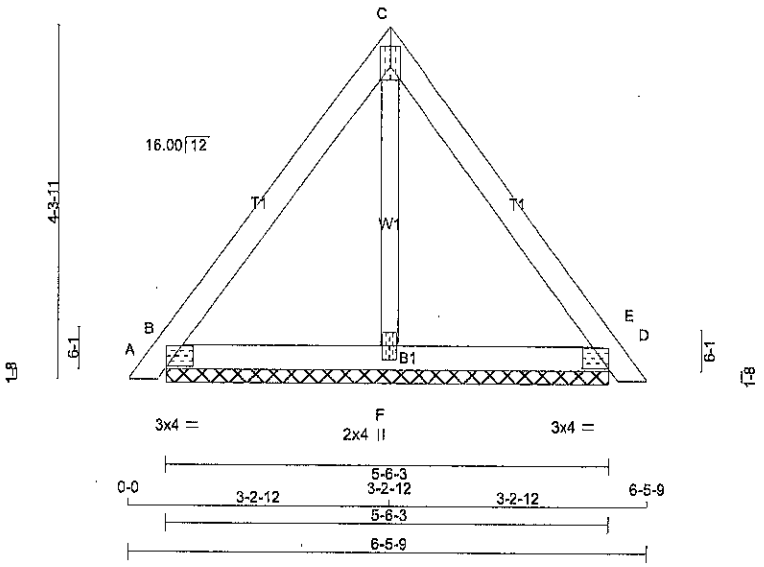

TOTAL WEIGHT = 5 X 21 = 107 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	194	0	194	0	0	5-6-3	1-8
D	194	0	194	0	0	5-6-3	1-8
F	147	0	147	0	0	5-6-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	134	105 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	134	105 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
F	108	52 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B-C	-66 / 0	-70.5	-70.5	0.09 (1)	6.25
C-D	-66 / 0	-70.5	-70.5	0.09 (1)	6.25
D-E	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B-F	0 / 39	-17.5	-17.5	0.04 (4)	10.00
F-D	0 / 39	-17.5	-17.5	0.04 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-87 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09 (B-C:1) , BC=0.04 (B-F:4) , WB=0.02 (C-F:1) , SSI=0.04 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/15/2015
R. TURENNE
100157049
PROVINCE OF ONTARIO

A-N073665

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253439	H05T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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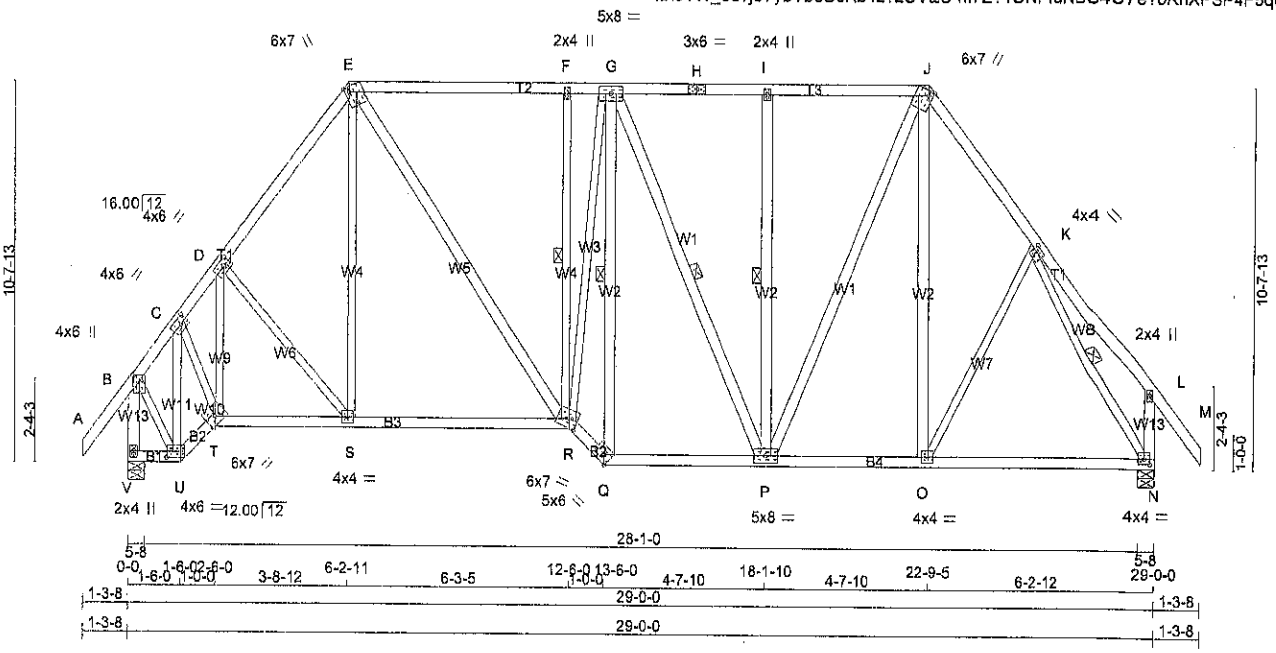

TOTAL WEIGHT = 207 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - R	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
G - P	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	8.0		
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMWW-t	MT20	4.0	4.0	1.50	1.00
L	TMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	4.0	4.0	1.75	2.00
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BBW-h	MT20	5.0	6.0	Edge	2.50
R	BBWWW-m	MT20	6.0	7.0	3.00	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1377	0	1377	0	0	5-8	1-8
N	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
N	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-R, G-Q, G-P, I-P, K-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-785 / 0	-70.5	-70.5	0.10 (1)	6.25
C- D	-1064 / 0	-70.5	-70.5	0.12 (1)	5.96
D- E	-1141 / 0	-70.5	-70.5	0.14 (1)	5.80
E- F	-995 / 0	-70.5	-70.5	0.36 (1)	5.79
F- G	-991 / 0	-70.5	-70.5	0.32 (1)	5.81
G- H	-850 / 0	-70.5	-70.5	0.22 (1)	6.25
H- I	-850 / 0	-70.5	-70.5	0.22 (1)	6.25
I- J	-850 / 0	-70.5	-70.5	0.22 (1)	6.25
J- K	-1041 / 0	-70.5	-70.5	0.12 (1)	6.03
K- L	0 / 19	-70.5	-70.5	0.10 (1)	10.00
L- M	0 / 40	-70.5	-70.5	0.10 (1)	10.00
V- B	-1364 / 0	0.0	0.0	0.17 (1)	6.95
N- L	-183 / 0	0.0	0.0	0.02 (1)	7.81
V- U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U- T	0 / 579	-17.5	-17.5	0.10 (1)	10.00
T- S	0 / 690	-17.5	-17.5	0.20 (4)	10.00
S- R	0 / 671	-17.5	-17.5	0.20 (4)	10.00
R- Q	0 / 1253	-17.5	-17.5	0.20 (1)	10.00
Q- P	0 / 897	-17.5	-17.5	0.19 (1)	10.00
P- O	0 / 610	-17.5	-17.5	0.20 (4)	10.00
O- N	0 / 578	-17.5	-17.5	0.19 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-948 / 0	0.25 (1)
C- T	0 / 747	0.17 (1)
T- D	-254 / 0	0.08 (1)
D- S	-32 / 2	0.02 (1)
S- E	0 / 129	0.04 (4)
E- R	0 / 578	0.09 (1)
R- F	-392 / 0	0.24 (1)
R- G	0 / 861	0.19 (1)
Q- G	-835 / 0	0.46 (1)
G- P	-118 / 0	0.08 (1)
P- I	-356 / 0	0.20 (1)
P- J	0 / 586	0.09 (1)
O- J	0 / 81	0.02 (4)
O- K	0 / 64	0.02 (4)
B- U	0 / 710	0.16 (1)
K- N	-1281 / 0	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.36 (E-F:1) , BC=0.20 (Q-R:1) , WB=0.48 (G-Q:1) , SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.46 (K) (INPUT = 1.00)

A-1073664

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253439	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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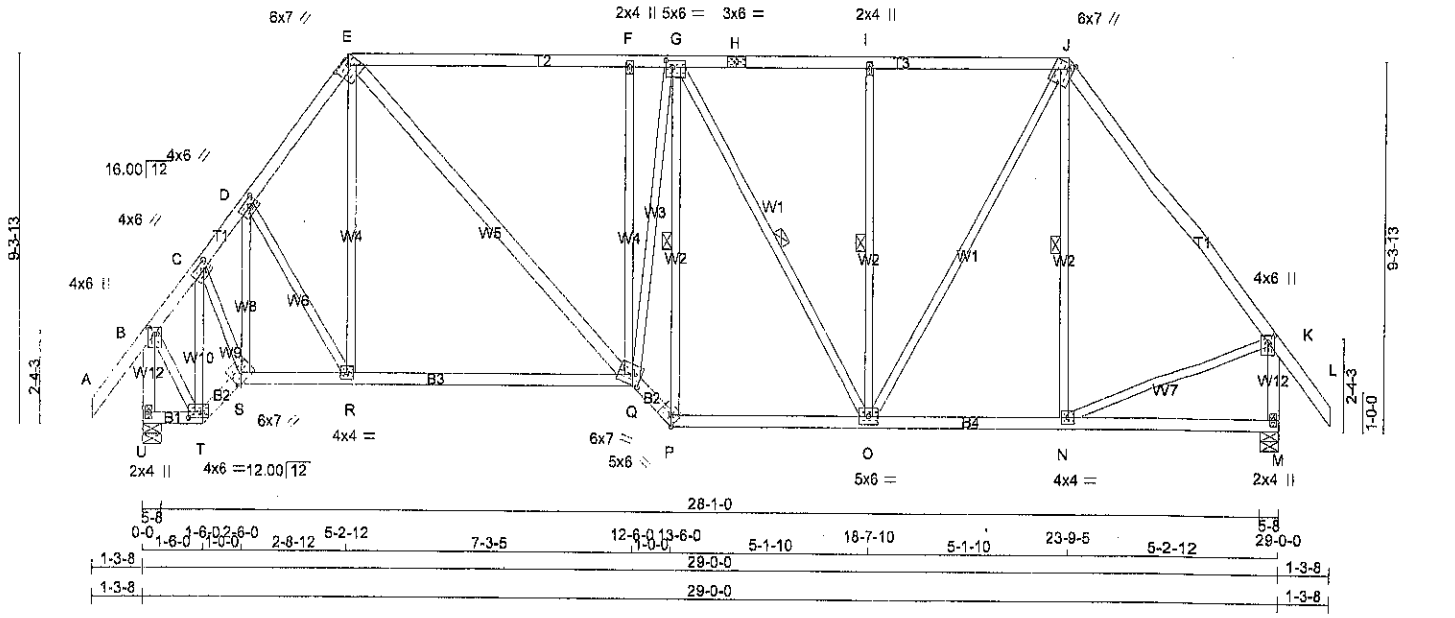

TOTAL WEIGHT = 173 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTWW-h	MT20	6.0	7.0	1.75	4.00
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	6.0	2.00	2.00
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BBW-h	MT20	5.0	6.0	Edge	2.50
Q	BBWWW-m	MT20	6.0	7.0	3.00	2.75
R	BMWW-t	MT20	4.0	4.0		
S	BBWW-h	MT20	6.0	7.0	Edge	
T	BBWW-l	MT20	4.0	6.0	1.75	4.25
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	1377	0	1377	0	0	5-8	1-8
M	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	968	869 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
M	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-P, G-O, I-O, J-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.10 (1)		10.00
B-C	-784 / 0	-70.5	-70.5	0.10 (1)		6.25
C-D	-1085 / 0	-70.5	-70.5	0.06 (1)		6.01
D-E	-1167 / 0	-70.5	-70.5	0.08 (1)		5.82
E-F	-1175 / 0	-70.5	-70.5	0.55 (1)		5.16
F-G	-1168 / 0	-70.5	-70.5	0.47 (1)		5.17
G-H	-963 / 0	-70.5	-70.5	0.28 (1)		5.96
H-I	-963 / 0	-70.5	-70.5	0.28 (1)		5.96
I-J	-962 / 0	-70.5	-70.5	0.28 (1)		5.96
J-K	-1029 / 0	-70.5	-70.5	0.43 (1)		5.69
K-L	0 / 40	-70.5	-70.5	0.10 (1)		10.00
U-B	-1364 / 0	0.0	0.0	0.17 (1)		6.95
M-K	-1335 / 0	0.0	0.0	0.17 (1)		7.01
U-T	0 / 0	-17.5	-17.5	0.01 (4)		10.00
T-S	0 / 583	-17.5	-17.5	0.10 (1)		10.00
S-R	0 / 680	-17.5	-17.5	0.25 (4)		10.00
R-Q	0 / 689	-17.5	-17.5	0.25 (4)		10.00
Q-P	0 / 1444	-17.5	-17.5	0.24 (1)		10.00
P-O	0 / 1032	-17.5	-17.5	0.22 (1)		10.00
O-N	0 / 613	-17.5	-17.5	0.17 (1)		10.00
N-M	0 / 0	-17.5	-17.5	0.11 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-854 / 0	0.25 (1)
C-S	0 / 714	0.16 (1)
S-D	-241 / 0	0.08 (1)
D-R	0 / 22	0.01 (4)
R-E	0 / 123	0.04 (4)
E-Q	0 / 720	0.12 (1)
Q-F	-516 / 0	0.70 (1)
Q-G	0 / 1079	0.24 (1)
P-G	-987 / 0	0.55 (1)
G-O	-142 / 0	0.11 (1)
O-I	-401 / 0	0.23 (1)
O-J	0 / 703	0.16 (1)
N-J	-122 / 40	0.07 (1)
B-T	0 / 714	0.16 (1)
N-K	0 / 656	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.55 (E-F:1) , BC=0.25 (Q-R:4) , WB=0.70 (F-Q:1) , SSI=0.23 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.28 (G) (INPUT = 1.00)

A-15073663

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253439	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Wed Jul 15 11:48:13 2015 Page 1
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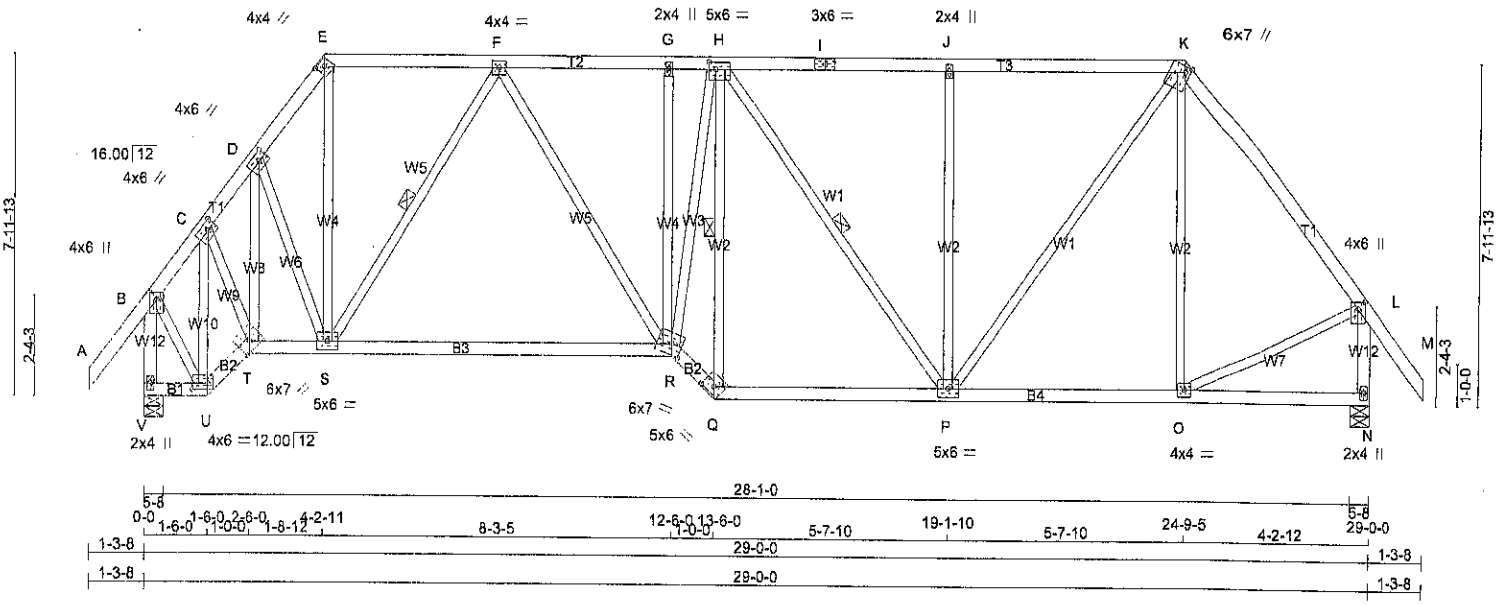

TOTAL WEIGHT = 163 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TTW-h	MT20	4.0	4.0	1.75	2.00
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWWW-t	MT20	5.0	6.0	2.00	2.00
I	TS-t	MT20	3.0	6.0		
J	TMW+w	MT20	2.0	4.0		
K	TTWW+m	MT20	6.0	7.0	Edge	1.50
L	TMVW+p	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	2.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BBW-h	MT20	5.0	6.0	2.00	3.25
R	BBWWW-m	MT20	6.0	7.0	3.00	3.00
S	BMWWW-t	MT20	5.0	6.0		
T	BBWW-h	MT20	6.0	7.0	Edge	
U	BBWW-l	MT20	4.0	6.0	1.75	4.25
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	1377	0	1377	0	0	5-8	1-8
N	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	966	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
N	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-S, H-Q, H-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B-C	-783 / 0	-70.5	-70.5	0.10 (1)	6.25
C-D	-1108 / 0	-70.5	-70.5	0.05 (1)	5.98
D-E	-1180 / 0	-70.5	-70.5	0.06 (1)	5.83
E-F	-713 / 0	-70.5	-70.5	0.20 (1)	6.25
F-G	-1357 / 0	-70.5	-70.5	0.22 (1)	5.33
G-H	-1357 / 0	-70.5	-70.5	0.21 (1)	5.32
H-I	-1112 / 0	-70.5	-70.5	0.35 (1)	5.55
I-J	-1112 / 0	-70.5	-70.5	0.35 (1)	5.55
J-K	-1111 / 0	-70.5	-70.5	0.36 (1)	5.55
K-L	-1022 / 0	-70.5	-70.5	0.26 (1)	5.90
L-M	0 / 40	-70.5	-70.5	0.10 (1)	10.00
V-B	-1364 / 0	0.0	0.0	0.17 (1)	6.95
N-L	-1344 / 0	0.0	0.0	0.17 (1)	6.99
V-U	0 / 0	-17.5	-17.5	0.01 (4)	10.00
U-T	0 / 586	-17.5	-17.5	0.10 (1)	10.00
T-S	0 / 869	-17.5	-17.5	0.33 (4)	10.00
S-R	0 / 1136	-17.5	-17.5	0.36 (4)	10.00
R-Q	0 / 1896	-17.5	-17.5	0.28 (1)	10.00
Q-P	0 / 1215	-17.5	-17.5	0.26 (1)	10.00
P-O	0 / 607	-17.5	-17.5	0.18 (4)	10.00
O-N	0 / 0	-17.5	-17.5	0.09 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	-960 / 0	0.25 (1)
C-T	0 / 660	0.15 (1)
T-D	-245 / 0	0.08 (1)
D-S	0 / 86	0.02 (4)
S-E	0 / 771	0.17 (1)
S-F	-819 / 0	0.34 (1)
F-R	0 / 419	0.09 (1)
R-G	-36 / 8	0.03 (1)
R-H	0 / 963	0.22 (1)
Q-H	-1143 / 0	0.46 (1)
H-P	-177 / 0	0.11 (1)
P-J	-440 / 0	0.53 (1)
P-K	0 / 850	0.19 (1)
O-K	-180 / 14	0.22 (1)
B-U	0 / 719	0.16 (1)
O-L	0 / 669	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.36 (J-K:1) , BC=0.36 (R-S:4) , WB=0.53 (J-P:1) , SSI=0.19 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 1.00)

A-1507366Z

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253439	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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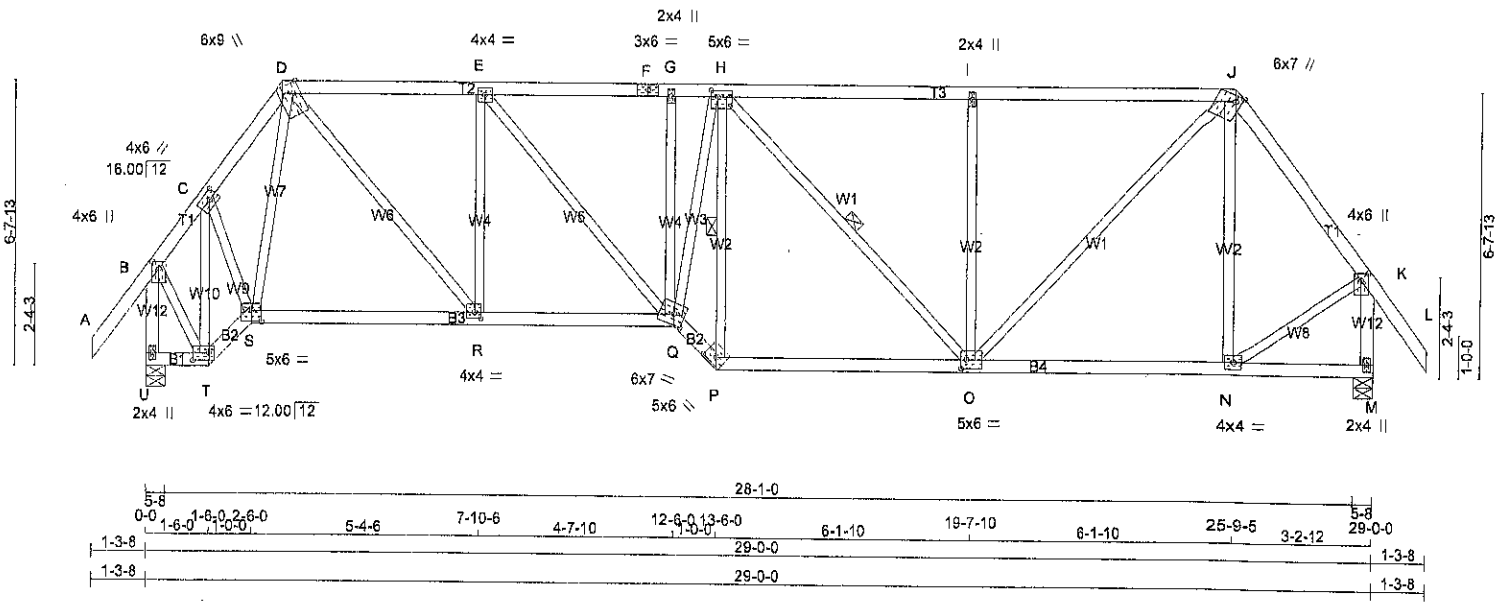

TOTAL WEIGHT = 149 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWWW-t	MT20	5.0	6.0	1.75	2.00
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BBW-h	MT20	5.0	6.0	2.00	3.25
Q	BBWWW-m	MT20	6.0	7.0	3.00	3.00
R	BMWW-t	MT20	4.0	4.0	1.75	1.75
S	BBWW-I	MT20	5.0	6.0	3.00	3.00
T	BBWW-I	MT20	4.0	6.0	2.00	4.50
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1377	0	1377	0	0	5-8	1-8
M	1372	0	1372	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
M	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF H-P, H-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-762 / 0	-70.5	-70.5	0.10 (1)	6.25
C- D	-1105 / 0	-70.5	-70.5	0.06 (1)	5.97
D- E	-1395 / 0	-70.5	-70.5	0.27 (1)	6.21
E- F	-1701 / 0	-70.5	-70.5	0.29 (1)	4.80
F- G	-1701 / 0	-70.5	-70.5	0.29 (1)	4.80
G- H	-1700 / 0	-70.5	-70.5	0.25 (1)	4.82
H- I	-1320 / 0	-70.5	-70.5	0.44 (1)	5.07
I- J	-1320 / 0	-70.5	-70.5	0.45 (1)	5.08
J- K	-990 / 0	-70.5	-70.5	0.16 (1)	6.10
K- L	0 / 40	-70.5	-70.5	0.10 (1)	10.00
U- B	-1364 / 0	0.0	0.0	0.17 (1)	6.95
M- K	-1356 / 0	0.0	0.0	0.17 (1)	6.97
U- T	0 / 0	-17.5	-17.5	0.01 (4)	10.00
T- S	0 / 588	-17.5	-17.5	0.10 (1)	10.00
S- R	0 / 676	-17.5	-17.5	0.18 (4)	10.00
R- Q	0 / 1395	-17.5	-17.5	0.29 (1)	10.00
Q- P	0 / 2052	-17.5	-17.5	0.33 (1)	10.00
P- O	0 / 1474	-17.5	-17.5	0.32 (1)	10.00
O- N	0 / 586	-17.5	-17.5	0.21 (4)	10.00
N- M	0 / 0	-17.5	-17.5	0.09 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-964 / 0	0.25 (1)
C- S	0 / 653	0.15 (1)
S- D	-143 / 0	0.07 (1)
D- R	0 / 1083	0.25 (1)
R- E	-729 / 0	0.35 (1)
E- Q	0 / 466	0.10 (1)
Q- G	-52 / 5	0.03 (1)
Q- H	0 / 1225	0.28 (1)
P- H	-1391 / 0	0.40 (1)
H- O	-223 / 0	0.12 (1)
O- I	-482 / 0	0.35 (1)
O- J	0 / 1054	0.24 (1)
N- J	-255 / 0	0.19 (1)
B- T	0 / 721	0.16 (1)
N- K	0 / 685	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.45 (I-J:1) , BC=0.33 (P-Q:1) , WB=0.40 (H-P:1) , SSI=0.20 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.31 (H) (INPUT = 1.00)

A-15073661

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253439	H01T	1	1	TRUSS DESC.	

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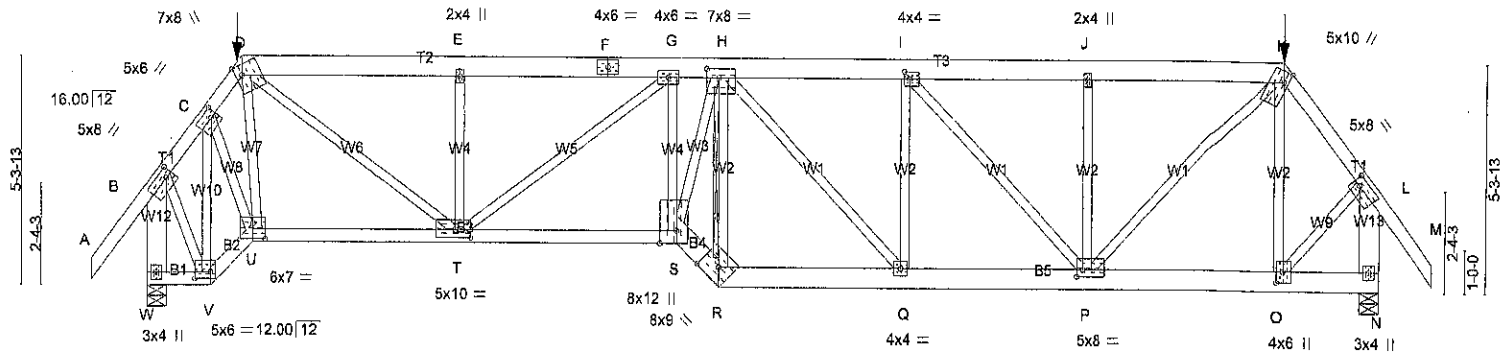

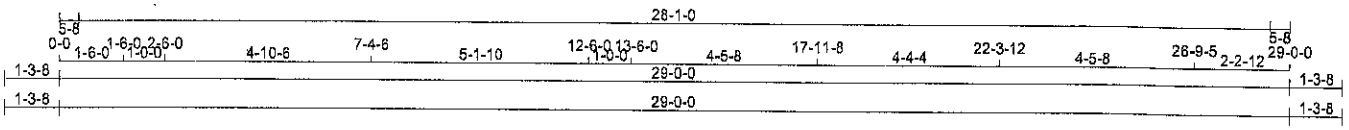

TOTAL WEIGHT = 172 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - K	2x6	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	1650F 1.5E	SPF
S - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	7.0	8.0	2.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMWWW-t	MT20	7.0	8.0	2.50	3.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMW+w	MT20	2.0	4.0		
K	TTWW+m	MT20	5.0	10.0	3.00	1.25
L	TMVW-t	MT20	5.0	8.0	2.25	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW+t	MT20	4.0	6.0	3.00	1.75
P	BMWWW-t	MT20	5.0	8.0	2.00	1.75
Q	BMWW-t	MT20	4.0	4.0		
R	BBW-h	MT20	8.0	9.0	3.75	5.00
S	BBWW+p	MT20	8.0	12.0	Edge	4.75
T	BMWWW-t	MT20	5.0	10.0	2.25	4.50
U	BBWW-l	MT20	6.0	7.0	4.25	3.50
V	BBWW-l	MT20	5.0	6.0	1.75	4.50
W	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2562	0	2562	0	0	5-8	2-13
N	2572	0	2572	0	0	5-8	2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1804	1228 / 0	0 / 0	0 / 0	0 / 0	575 / 0	0 / 0
N	1810	1234 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT H-R
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00
B-C	-1531 / 0	-70.5	-70.5	0.14 (1)	5.15
C-D	-2232 / 0	-70.5	-70.5	0.11 (1)	4.46
D-E	-3626 / 0	-70.5	-70.5	0.23 (1)	4.30
E-F	-3626 / 0	-70.5	-70.5	0.29 (1)	4.22
F-G	-3626 / 0	-70.5	-70.5	0.29 (1)	4.22
G-H	-4611 / 0	-138.7	-138.7	0.27 (1)	3.83
H-I	-3579 / 0	-138.7	-138.7	0.23 (1)	4.32
I-J	-2718 / 0	-138.7	-138.7	0.21 (1)	4.85
J-K	-2718 / 0	-138.7	-138.7	0.24 (1)	4.82
K-L	-1869 / 0	-70.5	-70.5	0.12 (1)	4.79
L-M	0 / 40	-70.5	-70.5	0.11 (1)	10.00
W-B	-2536 / 0	0.0	0.0	0.24 (1)	6.47
N-L	-2576 / 0	0.0	0.0	0.23 (1)	6.42
W-V	0 / 0	-34.5	-34.5	0.02 (4)	10.00
V-U	0 / 1174	-34.5	-34.5	0.21 (1)	10.00
U-T	0 / 1333	-102.7	-102.7	0.54 (1)	10.00
T-S	0 / 4587	-102.7	-102.7	0.96 (1)	10.00
S-R	0 / 5186	-34.5	-34.5	0.68 (1)	10.00
R-Q	0 / 3819	-34.5	-34.5	0.57 (1)	10.00
Q-P	0 / 3579	-34.5	-34.5	0.54 (1)	10.00
P-O	0 / 1105	-34.5	-34.5	0.22 (1)	10.00
O-N	0 / 0	-34.5	-34.5	0.08 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-1924 / 0	0.53 (1)
D-U	-276 / 0	0.08 (1)
T-E	-459 / 0	0.13 (1)
T-G	-1221 / 0	0.89 (1)
S-G	0 / 865	0.21 (1)
S-H	0 / 3148	0.78 (1)
R-H	-3447 / 0	0.75 (1)
H-Q	-360 / 0	0.27 (1)
Q-I	0 / 364	0.09 (4)
I-P	-1289 / 0	0.95 (1)
P-J	-663 / 0	0.25 (1)
P-K	0 / 2385	0.59 (1)
O-K	-718 / 0	0.27 (1)
B-V	0 / 1439	0.36 (1)
O-L	0 / 1438	0.36 (1)
C-U	0 / 1420	0.35 (1)
D-T	0 / 2888	0.71 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
K	26-9-5	-115	-115	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 12-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 16-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 818 (0.43")

A-U073660

CONTINUED ON PAGE 2

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11, AND 114.5 lbs FACTORED DOWN AT 26-9-5 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

CSI: TC=0.29 (E-G:1), BC=0.96 (S-T:1), WB=0.95 (I-P:1), SSI=0.28 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.68 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07. N. 2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1507366O (2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253438	H11SG	1	1	TRUSS DESC.	

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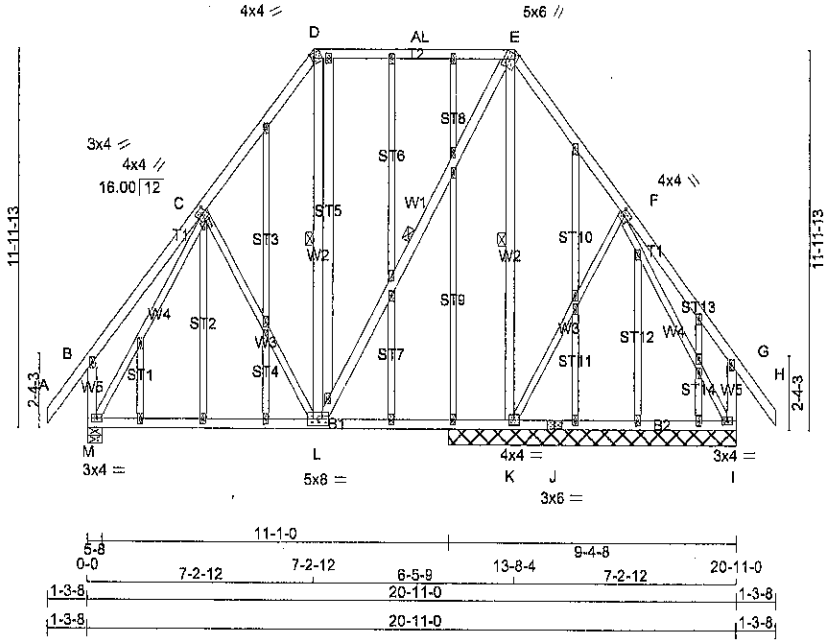

TOTAL WEIGHT = 193 lb [M]

LUMBER
N, L, G, A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - L	2x3	DRY	No.2	SPF
K - F	2x3	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
F - I	2x3	DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2	SPF
ST6	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	NP+t	MT20	3.0	4.0	1.50	0.25
D	TTW-m	MT20	4.0	4.0	1.75	1.00
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMVW1-t	MT20	3.0	4.0		
N, O, P, Q, R, S, T, U, V, W, X, X, Y, Z, AA, AB, AC, AD, AE, AF, AF, AG, AH, AI, AJ, AK, AK						
N	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	797	0	797	0	0	9-4-8 (5-11-0)	3-8
M	762	0	762	0	0	5-8	1-8
I	527	0	527	0	0	9-4-8 (5-11-0)	3-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	567	353 / 0	0 / 0	0 / 0	0 / 0	214 / 0	0 / 0
M	535	374 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
I	368	269 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-L, E-L, E-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B-C	0 / 25	-70.5	-70.5	0.15 (1)	10.00
C-D	-391 / 0	-70.5	-70.5	0.16 (1)	6.25
D-AL	-218 / 0	-78.0	-78.0	0.56 (1)	6.25
AL-E	-218 / 0	-78.0	-78.0	0.56 (1)	6.25
E-F	-148 / 0	-70.5	-70.5	0.16 (1)	6.25
F-G	0 / 26	-70.5	-70.5	0.16 (1)	10.00
G-H	0 / 40	-70.5	-70.5	0.10 (1)	10.00
M-B	-193 / 0	0.0	0.0	0.02 (1)	7.81
I-G	-192 / 0	0.0	0.0	0.02 (1)	7.81
M-L	0 / 273	-17.5	-17.5	0.21 (4)	10.00
L-K	0 / 67	-17.5	-17.5	0.20 (4)	10.00
K-J	0 / 150	-17.5	-17.5	0.21 (4)	10.00
J-I	0 / 150	-17.5	-17.5	0.21 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-109 / 3	0.14 (1)
L-D	-57 / 37	0.04 (1)
L-E	0 / 318	0.05 (1)
K-E	-531 / 0	0.42 (1)
K-F	-154 / 0	0.19 (1)
M-C	-583 / 0	0.67 (1)
F-I	-320 / 0	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND 6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.56 (D-E:1) , BC=0.21 (L-M:4) , WB=0.67 (C-M:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (I) (INPUT = 0.90)
JSI METAL= 0.23 (C) (INPUT = 1.00)

A-15073659

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253438	H11S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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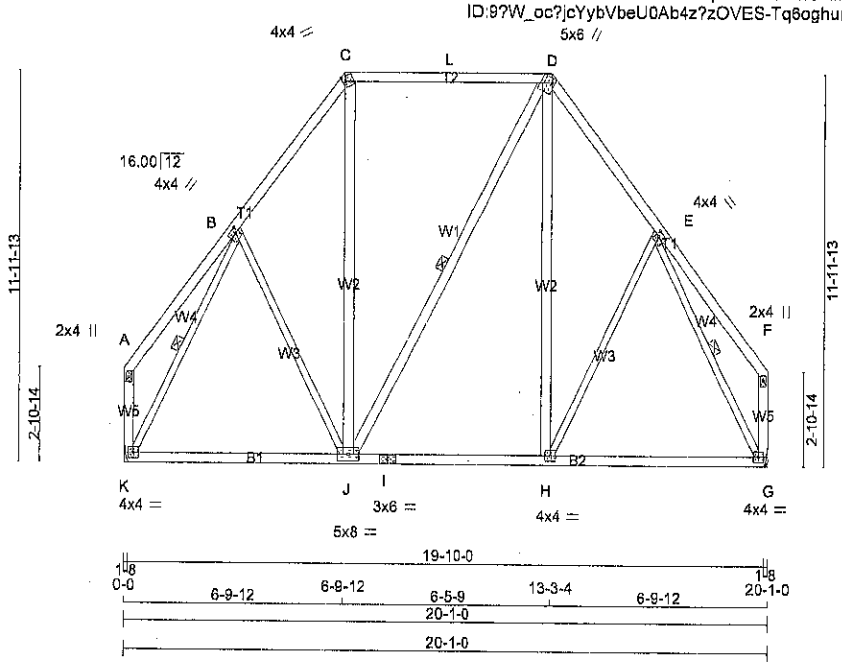

TOTAL WEIGHT = 3 X 128 = 384 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - J	2x3	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF
K - B	2x3	DRY	No.2	SPF
E - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.75	1.25
C	TTW-m	MT20	4.0	4.0	1.75	1.00
D	TTWW+m	MT20	5.0	6.0	1.75	1.00
E	TMWW-t	MT20	4.0	4.0	1.75	1.25
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	907	0	907	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	907	0	907	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	642	421 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0
G	642	421 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-J, B-K, E-G.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 22	-70.5	-70.5	0.13 (1)	10.00
B-C	-627 / 0	-70.5	-70.5	0.11 (1)	6.25
C-L	-363 / 0	-78.0	-78.0	0.56 (1)	6.25
L-D	-363 / 0	-78.0	-78.0	0.56 (1)	6.25
D-E	-627 / 0	-70.5	-70.5	0.11 (1)	6.25
E-F	0 / 22	-70.5	-70.5	0.13 (1)	10.00
K-A	-89 / 0	0.0	0.0	0.01 (1)	7.81
G-F	-89 / 0	0.0	0.0	0.01 (1)	7.81
K-J	0 / 368	-17.5	-17.5	0.21 (4)	10.00
J-I	0 / 363	-17.5	-17.5	0.21 (4)	10.00
I-H	0 / 363	-17.5	-17.5	0.21 (4)	10.00
H-G	0 / 368	-17.5	-17.5	0.21 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-J	-15 / 34	0.02 (1)
J-C	0 / 139	0.03 (4)
J-D	0 / 0	0.00 (1)
H-D	0 / 139	0.03 (4)
H-E	-15 / 34	0.02 (1)
K-B	-851 / 0	0.34 (1)
E-G	-851 / 0	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.56 (C-D:1) , BC=0.21 (H-J:4) , WB=0.34 (B-K:1) , SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.80)
JSI METAL= 0.32 (B) (INPUT = 1.00)

A-11073658

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253438	H11	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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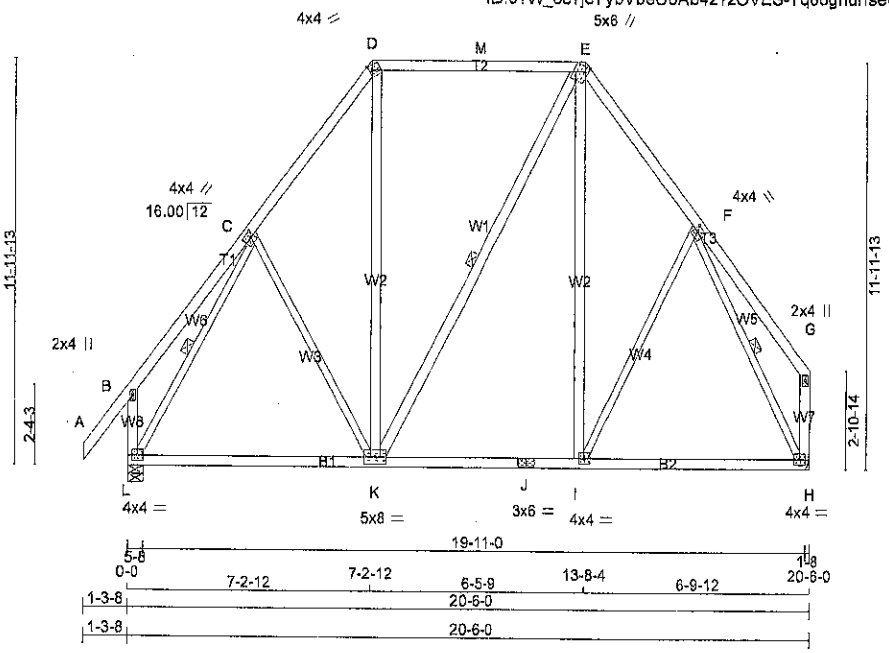

TOTAL WEIGHT = 131 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
I - F	2x3	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
F - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0	1.75	1.00
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMWW-t	MT20	4.0	4.0	1.75	1.25
G	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
L	1024	0	1024	0	0	5-8	1-8
H	926	0	926	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	722	489 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0
H	655	430 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-K, C-L, F-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
B- C	0 / 24	-70.5	-70.5	0.15 (1)	10.00	
C- D	-665 / 0	-70.5	-70.5	0.12 (1)	6.25	
D- M	-385 / 0	-78.0	-78.0	0.56 (1)	6.25	
M- E	-385 / 0	-78.0	-78.0	0.56 (1)	6.25	
E- F	-645 / 0	-70.5	-70.5	0.11 (1)	6.25	
F- G	0 / 22	-70.5	-70.5	0.13 (1)	10.00	
L- B	-194 / 0	0.0	0.0	0.02 (1)	7.81	
H- G	-90 / 0	0.0	0.0	0.01 (1)	7.81	
L- K	0 / 412	-17.5	-17.5	0.23 (4)	10.00	
K- J	0 / 373	-17.5	-17.5	0.23 (4)	10.00	
J- I	0 / 373	-17.5	-17.5	0.23 (4)	10.00	
I- H	0 / 377	-17.5	-17.5	0.20 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-59 / 21	0.07 (1)
K- D	0 / 163	0.03 (4)
K- E	0 / 23	0.00 (1)
I- E	0 / 133	0.03 (4)
I- F	-11 / 36	0.01 (1)
L- C	-879 / 0	0.34 (1)
F- H	-872 / 0	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.56 (D-E:1) , BC=0.23 (I-K:4) , WB=0.35 (F-H:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.15.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-100736 57

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253438	H10G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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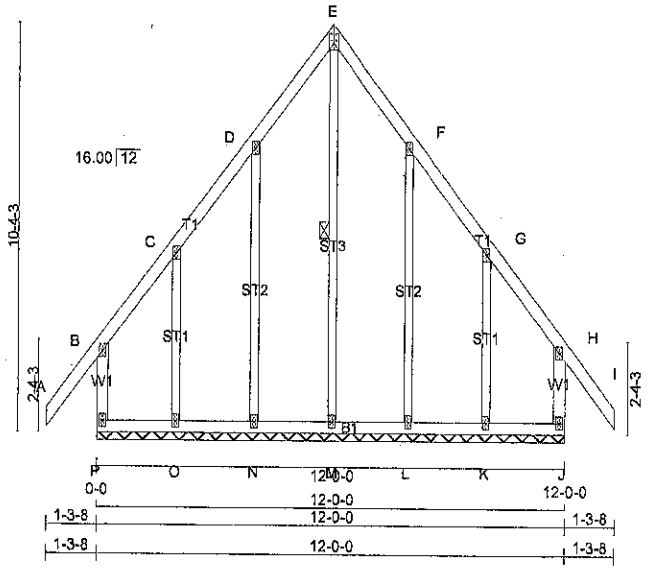

TOTAL WEIGHT = 73 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	3.0	5.0	2.00	Edge
H	TMV+p	MT20	2.0	4.0		
J	BMV1+p	MT20	2.0	4.0		
K, L, M, N, O						
K	BMW1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
P- B	-176 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
B- C	-4 / 0	-70.5	-70.5	0.07 (1)	10.00	
C- D	0 / 21	-70.5	-70.5	0.04 (1)	10.00	
D- E	0 / 18	-70.5	-70.5	0.04 (1)	10.00	
E- F	0 / 18	-70.5	-70.5	0.04 (1)	10.00	
F- G	0 / 21	-70.5	-70.5	0.04 (1)	10.00	
G- H	-4 / 0	-70.5	-70.5	0.07 (1)	10.00	
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
J- H	-176 / 0	0.0	0.0	0.03 (1)	7.81	
P- O	-7 / 0	-17.5	-17.5	0.02 (4)	10.00	
O- N	-9 / 0	-17.5	-17.5	0.02 (4)	10.00	
N- M	-12 / 0	-17.5	-17.5	0.01 (4)	6.25	
M- L	-12 / 0	-17.5	-17.5	0.01 (4)	6.25	
L- K	-9 / 0	-17.5	-17.5	0.02 (4)	10.00	
K- J	-7 / 0	-17.5	-17.5	0.02 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-176 / 0	0.12 (1)
N- D	-151 / 0	0.16 (1)
O- C	-107 / 0	0.04 (1)
L- F	-151 / 0	0.16 (1)
K- G	-107 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1) , BC=0.02 (J-K:4) , WB=0.16 (F-L:1) , SSI=0.05 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.05 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H0736 56

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253438	H01-Cond2	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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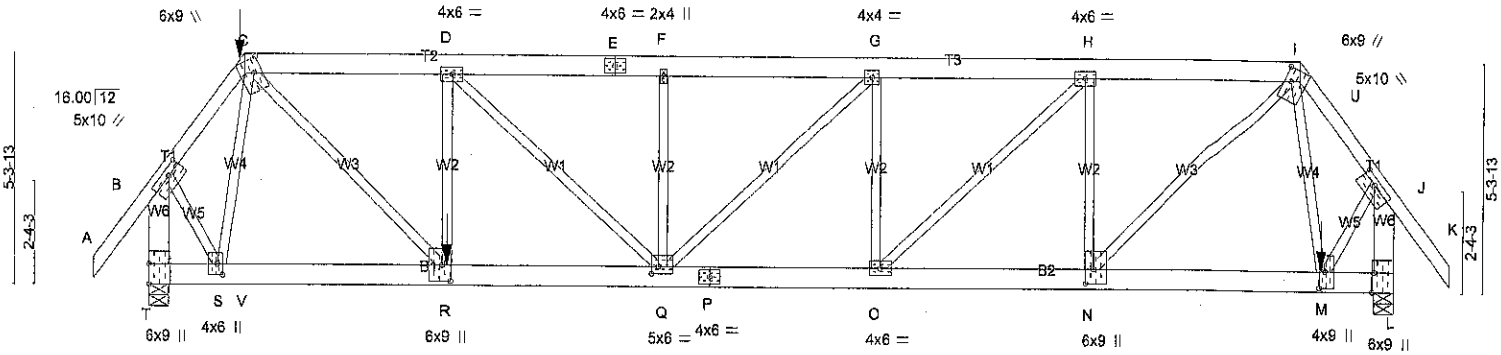

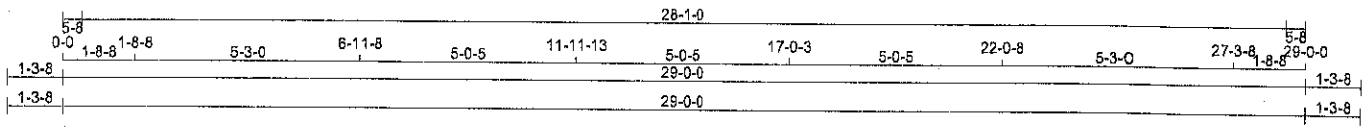

TOTAL WEIGHT = 2 X 169 = 338 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	10.0	1.75	4.25
C	TTWW+m	MT20	6.0	9.0	4.00	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	3.75	1.75
J	TMVW-t	MT20	5.0	10.0	1.50	4.25
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWW+t	MT20	4.0	9.0	4.50	1.75
N	BMWW+t	MT20	6.0	9.0	4.00	2.25
O	BMWW-t	MT20	4.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	6.0	6.0	2.00	2.00
R	BMWW+t	MT20	6.0	9.0	4.50	2.25
S	BMWW+t	MT20	4.0	6.0	3.00	1.50
T	BMV1+p	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2911	0	2911	0	0	5-8	3-15
L	3487	0	3487	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2150	895 / 0	0 / 0	0 / 0	0 / 0	1255 / 0	0 / 0
L	2631	793 / 0	0 / 0	0 / 0	0 / 0	1838 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.13 (1)	10.00	S- C	-1039 / 0	0.43 (1)
B- C	-1896 / 0	-70.5	-70.5	0.14 (1)	4.69	C- R	0 / 2879	0.82 (1)
C- D	-3263 / 0	-138.7	-138.7	0.36 (1)	4.27	R- D	-713 / 53	0.29 (1)
D- E	-3427 / 0	-70.5	-70.5	0.26 (1)	4.32	D- Q	-133 / 227	0.13 (4)
E- F	-3427 / 0	-70.5	-70.5	0.26 (1)	4.32	Q- F	-332 / 0	0.13 (1)
F- G	-3427 / 0	-70.5	-70.5	0.21 (1)	4.37	Q- G	0 / 392	0.14 (4)
G- H	-3144 / 0	-70.5	-70.5	0.21 (1)	4.53	O- G	-628 / 0	0.25 (1)
H- I	-2418 / 0	-70.5	-70.5	0.17 (1)	5.08	O- H	0 / 1010	0.29 (1)
I- U	-2161 / 0	-70.5	-70.5	0.26 (1)	4.30	N- H	-1074 / 0	0.43 (1)
U- J	-2161 / 0	-168.1	-168.1	0.26 (1)	4.30	N- I	0 / 1665	0.47 (1)
J- K	0 / 40	-70.5	-70.5	0.13 (1)	10.00	I- M	0 / 750	0.29 (4)
T- B	-2968 / 0	0.0	0.0	0.30 (1)	5.98	B- S	0 / 1705	0.49 (1)
L- J	-3431 / 0	0.0	0.0	0.34 (1)	5.60	M- J	0 / 1928	0.57 (4)
T- S	0 / 0	-34.5	-34.5	0.16 (4)	10.00			
S- V	0 / 1253	-34.5	-34.5	0.31 (4)	10.00			
V- R	0 / 1253	-34.5	-34.5	0.31 (4)	10.00			
R- Q	0 / 3263	-17.5	-17.5	0.55 (1)	10.00			
Q- P	0 / 3144	-17.5	-17.5	0.52 (1)	10.00			
P- O	0 / 3144	-17.5	-17.5	0.52 (1)	10.00			
O- N	0 / 2418	-17.5	-17.5	0.40 (1)	10.00			
N- M	0 / 1256	-17.5	-17.5	0.27 (4)	10.00			
M- L	0 / 0	-41.7	-41.7	0.07 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
M	27-3-8	-1643	-1643	---	FRONT	VERT	DEAD
R	6-11-8	-1256	-1256	---	FRONT	VERT	DEAD

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 6-11-8 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 7-6-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 1-7-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.36 (C-D:1) , BC=0.55 (Q-R:1) , WB=0.82 (C-R:1) , SSI=0.28 (C-D:1)

DOL LUMBER=0.87 NAIL=0.87 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

A-A07356

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD, AND 1407.0 lbs FACTORED DOWN AT 6-11-8, AND 1839.6 lbs FACTORED DOWN AT 27-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.73 (P) (INPUT = 1.00)

A-1507365 5(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253438	H01-Cond1	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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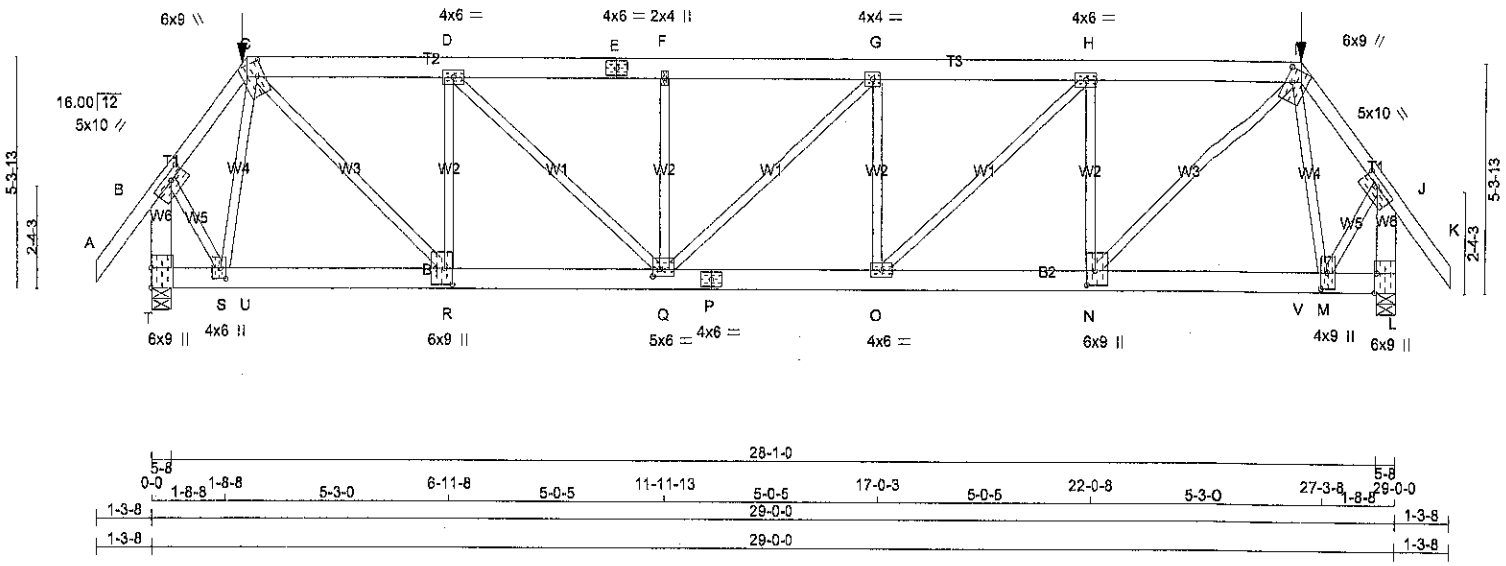

TOTAL WEIGHT = 2 X 169 = 338 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	10.0	1.75	4.25
C	TTWW+m	MT20	6.0	9.0	4.00	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	3.75	1.75
J	TMVW-t	MT20	5.0	10.0	1.50	4.25
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWW+t	MT20	4.0	9.0	4.50	1.75
N	BMWW+t	MT20	6.0	9.0	4.00	2.25
O	BMWW-t	MT20	4.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0	2.00	2.00
R	BMWW+t	MT20	6.0	9.0	4.50	2.25
S	BMWW+t	MT20	4.0	6.0	3.00	1.50
T	BMV1+p	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2572	0	2572	0	0	5-8	2-14
L	2572	0	2572	0	0	5-8	2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0
L	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	1	0.11 (1)	10.00
B-C	-1651 / 0	-70.5	-70.5	1	0.11 (1)	5.05
C-D	-2818 / 0	-138.7	-138.7	1	0.27 (1)	4.71
D-E	-3697 / 0	-138.7	-138.7	1	0.31 (1)	4.17
E-F	-3697 / 0	-138.7	-138.7	1	0.31 (1)	4.17
F-G	-3697 / 0	-138.7	-138.7	1	0.28 (1)	4.21
G-H	-3705 / 0	-138.7	-138.7	1	0.31 (1)	4.17
H-I	-2816 / 0	-138.7	-138.7	1	0.27 (1)	4.71
I-J	-1652 / 0	-70.5	-70.5	1	0.11 (1)	5.05
J-K	0 / 40	-70.5	-70.5	1	0.11 (1)	10.00
T-B	-2608 / 0	0.0	0.0	1	0.23 (1)	6.39
L-J	-2609 / 0	0.0	0.0	1	0.23 (1)	6.39
T-S	0 / 0	-34.5	-34.5	4	0.09 (4)	10.00
S-U	0 / 1093	-34.5	-34.5	1	0.22 (1)	10.00
U-R	0 / 1093	-34.5	-34.5	1	0.22 (1)	10.00
R-Q	0 / 2818	-34.5	-34.5	1	0.43 (1)	10.00
Q-P	0 / 3705	-34.5	-34.5	1	0.56 (1)	10.00
P-O	0 / 3705	-34.5	-34.5	1	0.56 (1)	10.00
O-N	0 / 2816	-34.5	-34.5	1	0.42 (1)	10.00
N-V	0 / 1093	-34.5	-34.5	1	0.22 (1)	10.00
V-M	0 / 1093	-34.5	-34.5	1	0.22 (1)	10.00
M-L	0 / 0	-34.5	-34.5	4	0.09 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-C	-918 / 0	0.36 (1)
C-R	0 / 2471	0.61 (1)
R-D	-1595 / 0	0.61 (1)
D-Q	0 / 1223	0.30 (1)
Q-F	-681 / 0	0.26 (1)
Q-G	-12 / 0	0.01 (1)
O-G	-666 / 0	0.26 (1)
O-H	0 / 1237	0.31 (1)
N-H	-1599 / 0	0.61 (1)
N-I	0 / 2468	0.61 (1)
I-M	-916 / 0	0.36 (1)
B-S	0 / 1485	0.37 (1)
M-J	0 / 1486	0.37 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
I	26-9-5	-115	-115	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.31 (D-F:1) , BC=0.55 (O-Q:1) , WB=0.61 (C-R:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (Q) (INPUT = 0.90)
JSI METAL= 0.78 (P) (INPUT = 1.00)

A-1073654

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253438	H01-Cond1	2	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.5 lbs FACTORED DOWN AT 26-9-5, AND 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

A-H073654(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253436	H08	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:9?W_oc?jcYybVbsU0Ab4z?zOVES-ZbGawB35KmSNcdV8NFU77P539m9cIyIdBUChWCyxtc0

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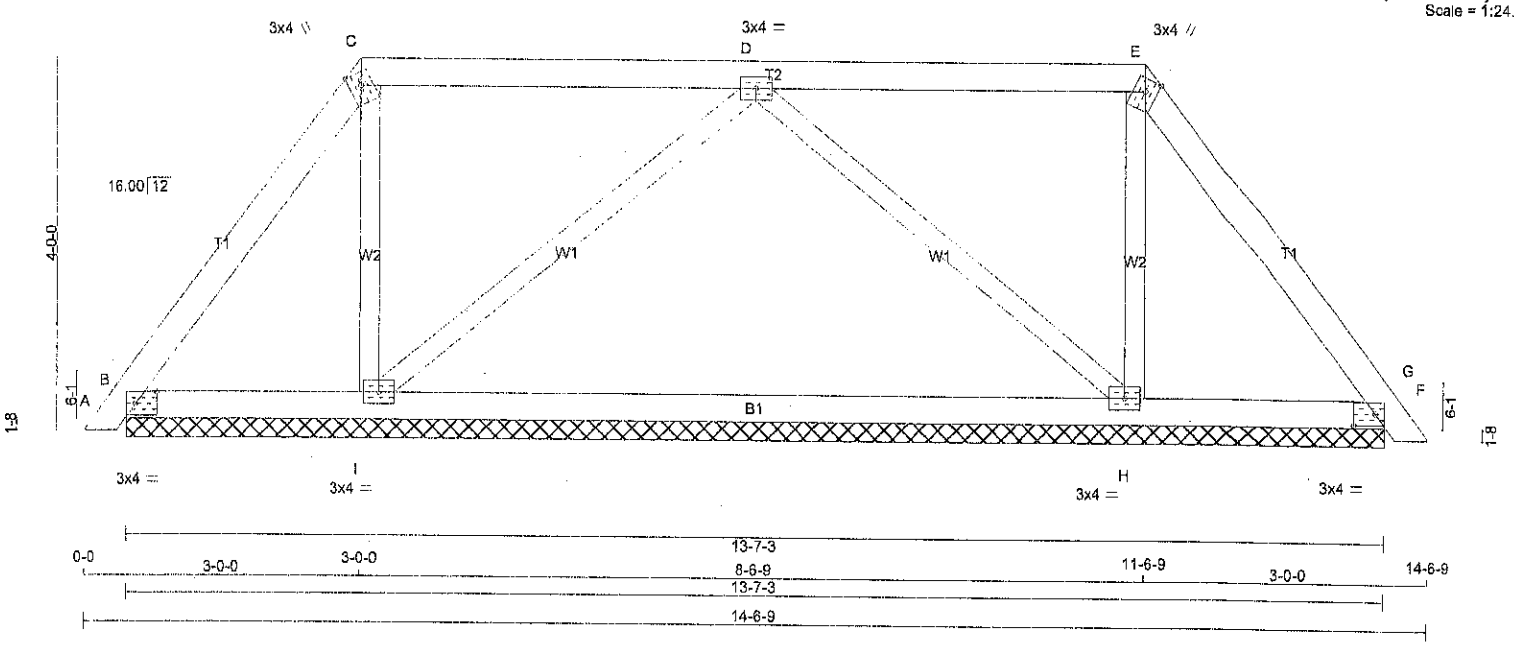

TOTAL WEIGHT = 2 X 51 = 101 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+m	MT20	3.0	4.0	Edge	
D	TMWW-t	MT20	3.0	4.0		
E	TTW+m	MT20	3.0	4.0	Edge	
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMWW1-t	MT20	3.0	4.0		
I	BMWW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	127	0	127	0	0	13-7-3	1-8
F	127	0	127	0	0	13-7-3	1-8
I	496	0	496	0	0	13-7-3	1-8
H	496	0	496	0	0	13-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	85	81 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
F	85	81 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
I	353	219 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
H	353	219 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	
B- C	-28 / 0	-70.5	-70.5	0.06 (1)	6.25	
C- D	-11 / 0	-70.5	-70.5	0.21 (1)	6.25	
D- E	-11 / 0	-70.5	-70.5	0.21 (1)	6.25	
E- F	-28 / 0	-70.5	-70.5	0.08 (1)	6.25	
F- G	0 / 9	-70.5	-70.5	0.01 (1)	10.00	
B- I	0 / 16	-17.5	-17.5	0.20 (4)	10.00	
I- H	0 / 229	-17.5	-17.5	0.21 (4)	10.00	
H- F	0 / 16	-17.5	-17.5	0.20 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-180 / 0	0.04 (1)
I- D	-287 / 0	0.15 (1)
D- H	-287 / 0	0.15 (1)
H- E	-180 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21 (D-E:1) , BC=0.21 (H-I:4) , WB=0.15 (D-I:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (I) (INPUT = 0.90)
JSI METAL= 0.08 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-15-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073633

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H07	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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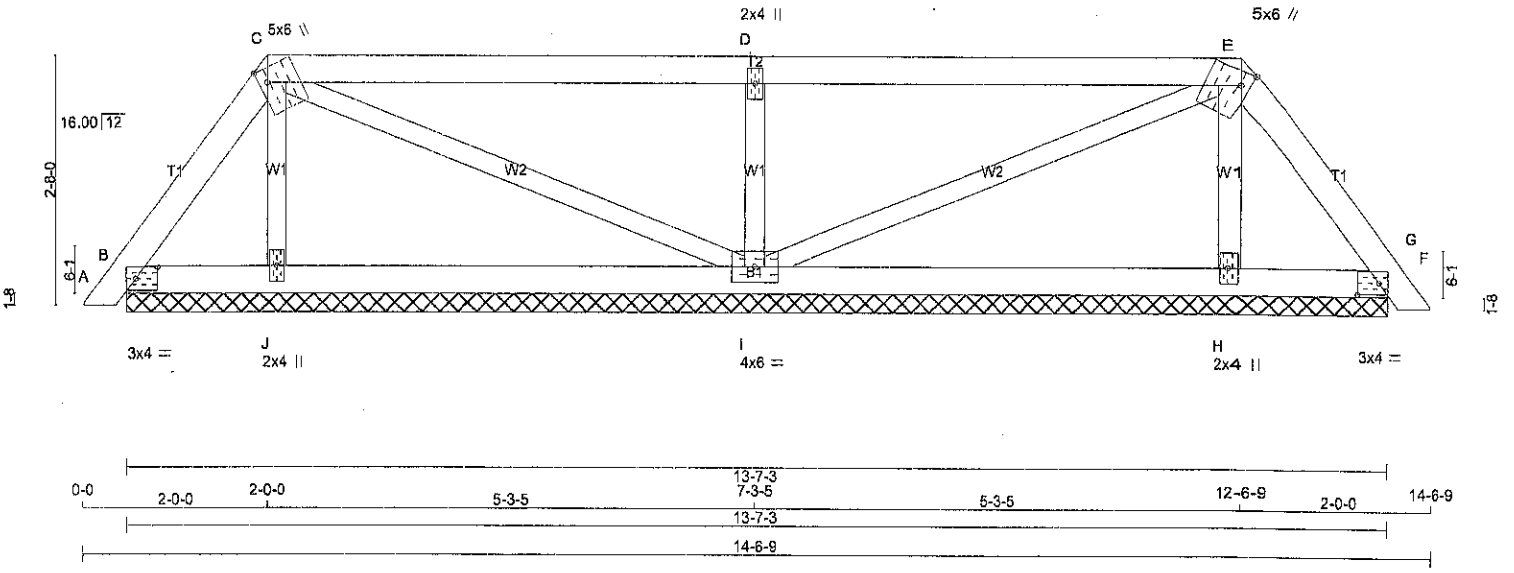

TOTAL WEIGHT = 2 X 49 = 97 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	129	0	129	0	0	13-7-3	1-8
F	129	0	129	0	0	13-7-3	1-8
J	205	0	205	0	0	13-7-3	1-8
I	578	0	578	0	0	13-7-3	1-8
H	205	0	205	0	0	13-7-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	87	79 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
F	87	79 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
J	148	78 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
I	405	267 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
H	148	78 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 9	-70.5	-70.5	0.01 (1)		10.00	J-C	-131 / 0	0.02 (1)
B-C	-66 / 0	-70.5	-70.5	0.03 (1)		6.25	C-I	-23 / 0	0.01 (1)
C-D	-9 / 0	-70.5	-70.5	0.33 (1)		10.00	I-D	-464 / 0	0.07 (1)
D-E	-9 / 0	-70.5	-70.5	0.33 (1)		10.00	I-E	-23 / 0	0.01 (1)
E-F	-66 / 0	-70.5	-70.5	0.03 (1)		6.25	H-E	-131 / 0	0.02 (1)
F-G	0 / 9	-70.5	-70.5	0.01 (1)		10.00			
B-J	0 / 37	-17.5	-17.5	0.07 (4)		10.00			
J-I	0 / 31	-17.5	-17.5	0.10 (4)		10.00			
I-H	0 / 31	-17.5	-17.5	0.10 (4)		10.00			
H-F	0 / 37	-17.5	-17.5	0.07 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33 (D-E:1) , BC=0.10 (H-I:4) , WB=0.07 (D-I:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (F) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

A-150735 82

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H06	6	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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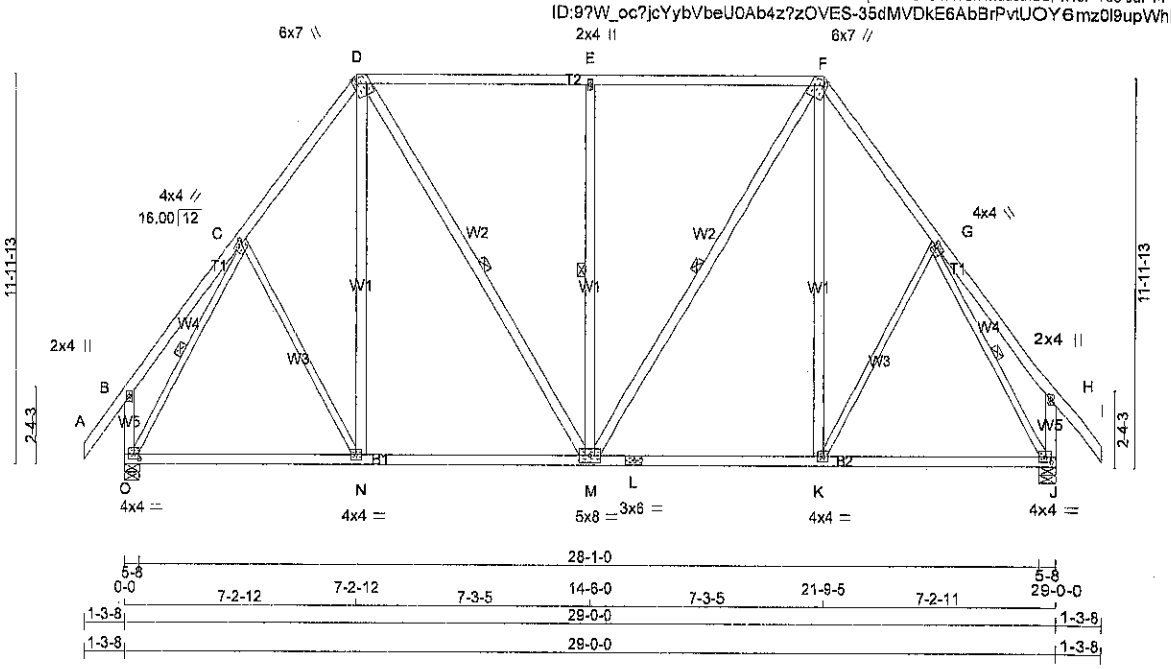

TOTAL WEIGHT = 6 X 181 = 1084 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - N	2x3	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF
O - C	2x3	DRY	No.2	SPF
G - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.00
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMWW-t	MT20	4.0	4.0	1.50	1.00
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	4.0	1.75	2.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1429	0	1429	0	0	5-8	1-9
J	1429	0	1429	0	0	5-8	1-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1010	668 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0
J	1010	668 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-M, E-M, F-M, C-O, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C- N	0 / 51	0.02 (4)
B- C	0 / 23	-70.5	-70.5	0.14 (1)	10.00	N- D	0 / 130	0.03 (4)
C- D	-1088 / 0	-70.5	-70.5	0.13 (1)	5.93	D- M	0 / 477	0.08 (1)
D- E	-889 / 0	-78.0	-78.0	0.55 (1)	5.67	M- E	-695 / 0	0.54 (1)
E- F	-889 / 0	-78.0	-78.0	0.55 (1)	5.67	M- F	0 / 477	0.08 (1)
F- G	-1088 / 0	-70.5	-70.5	0.13 (1)	5.93	K- F	0 / 130	0.03 (4)
G- H	0 / 23	-70.5	-70.5	0.14 (1)	10.00	K- G	0 / 51	0.02 (4)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O- C	-1338 / 0	0.52 (1)
O- B	-195 / 0	0.0	0.0	0.02 (1)	7.81	G- J	-1338 / 0	0.52 (1)
J- H	-195 / 0	0.0	0.0	0.02 (1)	7.81			
O- N	0 / 627	-17.5	-17.5	0.28 (4)	10.00			
N- M	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
M- L	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
L- K	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
K- J	0 / 627	-17.5	-17.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.55 (D-E:1) , BC=0.29 (M-N:4) , WB=0.54 (E-M:1) , SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.49 (G) (INPUT = 1.00)

A-10073581

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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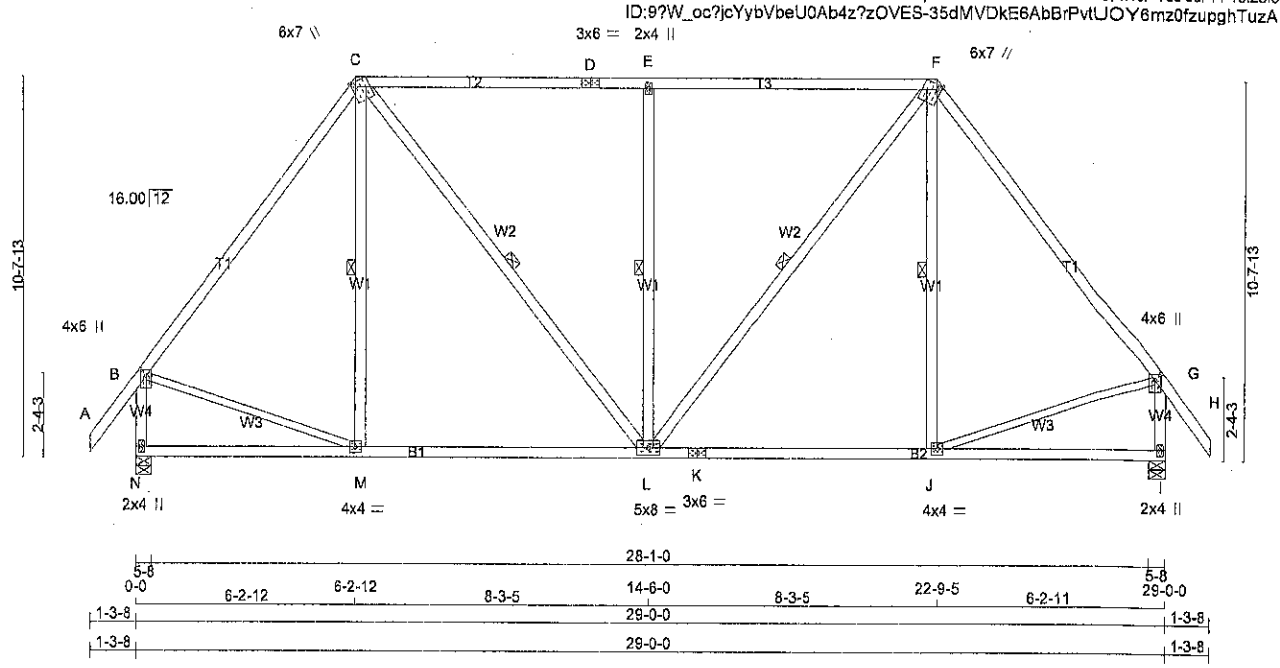

TOTAL WEIGHT = 2 X 160 = 319 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - M	2x3	DRY	No.2	SPF
J - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1375	0	1375	0	0	5-8	1-8
I	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
I	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-M, C-L, E-L, F-L, F-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.07 (1)	10.00
B- C	-1029 / 0	-70.5	-70.5	0.38 (1)	6.25
C- D	-959 / 0	-70.5	-70.5	0.95 (1)	4.39
D- E	-959 / 0	-70.5	-70.5	0.95 (1)	4.39
E- F	-958 / 0	-70.5	-70.5	0.95 (1)	4.39
F- G	-1029 / 0	-70.5	-70.5	0.38 (1)	6.25
G- H	0 / 40	-70.5	-70.5	0.07 (1)	10.00
N- B	-1333 / 0	0.0	0.0	0.17 (1)	7.01
I- G	-1333 / 0	0.0	0.0	0.17 (1)	7.01
N- M	0 / 0	-17.5	-17.5	0.21 (4)	10.00
M- L	0 / 615	-17.5	-17.5	0.28 (4)	10.00
L- K	0 / 615	-17.5	-17.5	0.28 (4)	10.00
K- J	0 / 615	-17.5	-17.5	0.28 (4)	10.00
J- I	0 / 0	-17.5	-17.5	0.21 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-48 / 95	0.03 (1)
C- L	0 / 550	0.09 (1)
L- E	-715 / 0	0.41 (1)
L- F	0 / 550	0.09 (1)
J- F	-48 / 95	0.03 (1)
B- M	0 / 644	0.14 (1)
J- G	0 / 644	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.95 (C-E:1) , BC=0.28 (L-M:4) , WB=0.41 (E-L:1) , SSI=0.28 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (M) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-10073580

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H04	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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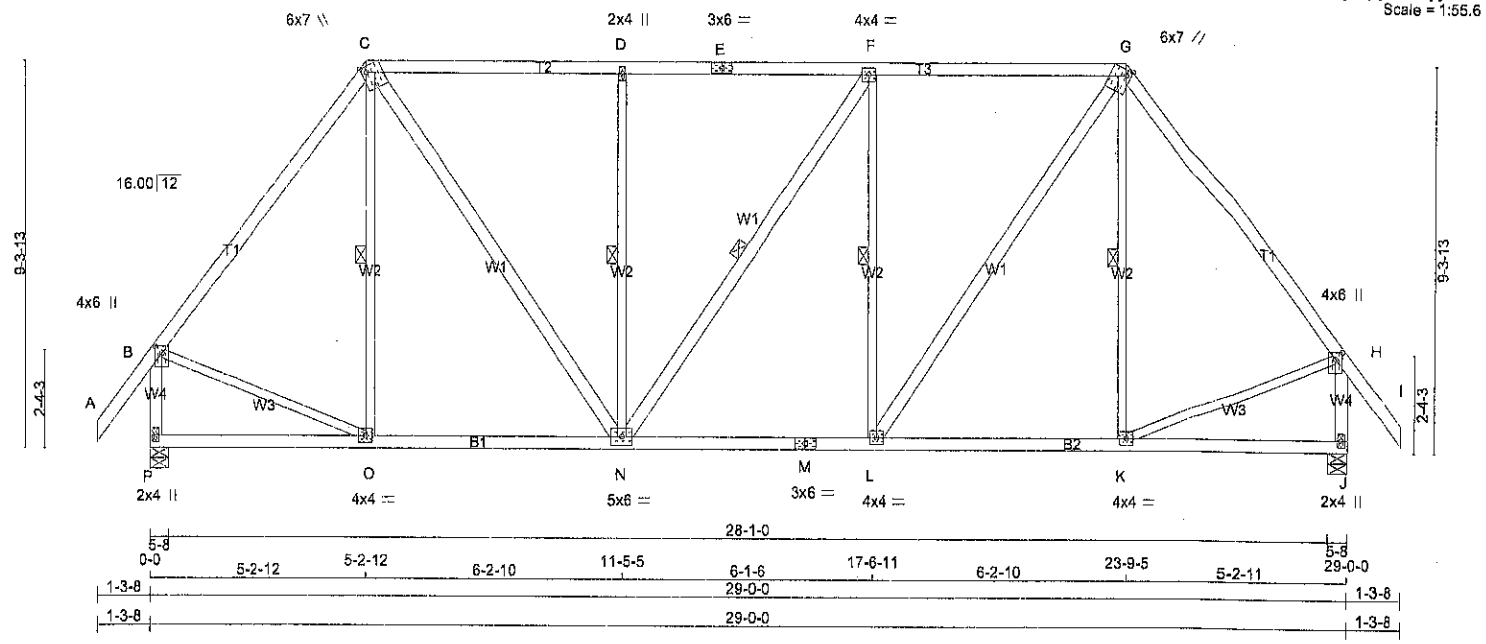

TOTAL WEIGHT = 2 X 158 = 316 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, L, O						
K	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-O, D-N, F-N, F-L, G-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	1	0.10 (1)	10.00	O- C	-114 / 51	0.07 (1)
B- C	-1033 / 0	-70.5	-70.5	1	0.43 (1)	5.66	C- N	0 / 698	0.11 (1)
C- D	-1013 / 0	-70.5	-70.5	1	0.40 (1)	5.66	N- D	-468 / 0	0.27 (1)
D- E	-1013 / 0	-70.5	-70.5	1	0.41 (1)	5.66	N- F	-2 / 0	0.00 (1)
E- F	-1013 / 0	-70.5	-70.5	1	0.41 (1)	5.66	L- F	-468 / 0	0.27 (1)
F- G	-1014 / 0	-70.5	-70.5	1	0.41 (1)	5.66	L- G	0 / 700	0.11 (1)
G- H	-1033 / 0	-70.5	-70.5	1	0.43 (1)	5.66	K- G	-116 / 51	0.07 (1)
H- I	0 / 40	-70.5	-70.5	1	0.10 (1)	10.00	B- O	0 / 657	0.15 (1)
P- B	-1339 / 0	0.0	0.0	1	0.17 (1)	7.00	K- H	0 / 657	0.15 (1)
J- H	-1338 / 0	0.0	0.0	1	0.17 (1)	7.00			
P- O	0 / 0	-17.5	-17.5	1	0.13 (4)	10.00			
O- N	0 / 616	-17.5	-17.5	1	0.18 (4)	10.00			
N- M	0 / 1014	-17.5	-17.5	1	0.23 (1)	10.00			
M- L	0 / 1014	-17.5	-17.5	1	0.23 (1)	10.00			
L- K	0 / 616	-17.5	-17.5	1	0.18 (4)	10.00			
K- J	0 / 0	-17.5	-17.5	1	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43 (B-C:1) , BC=0.23 (L-N:1) , WB=0.27 (F-L:1) , SSI=0.20 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.28 (M) (INPUT = 1.00)

A-1507357 9

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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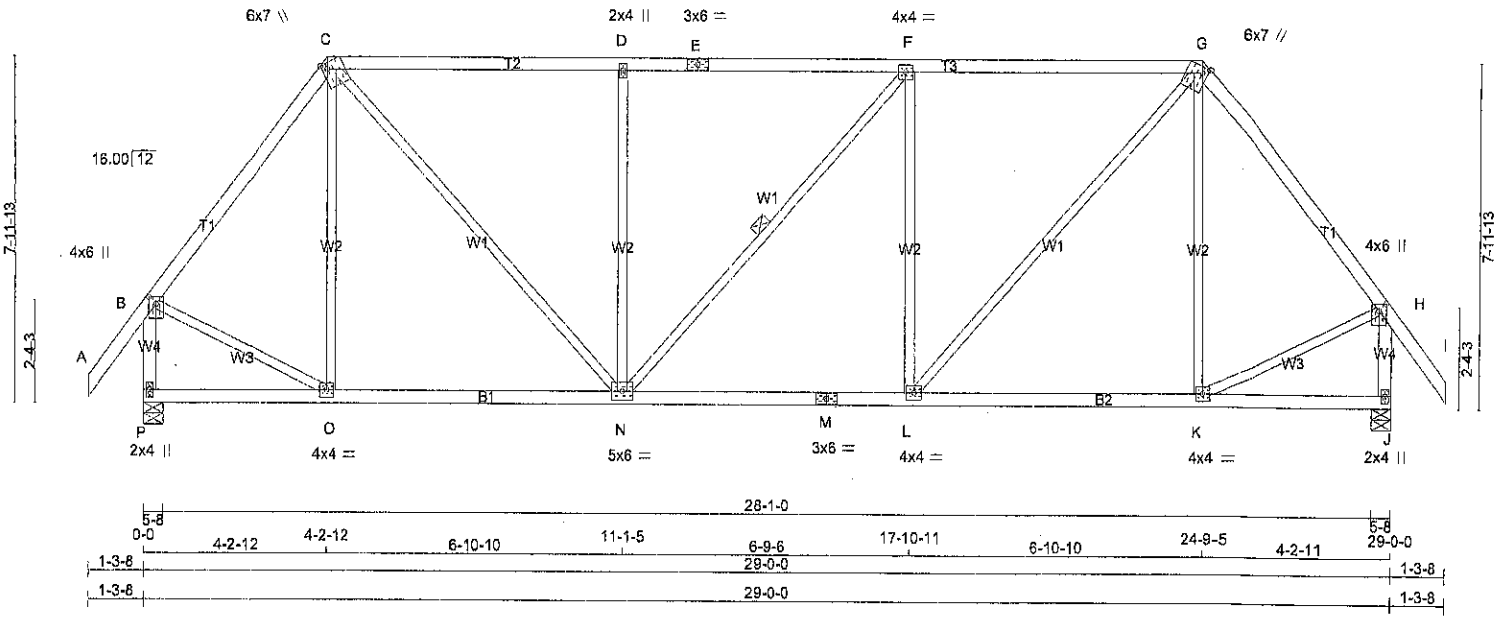


TOTAL WEIGHT = 2 X 138 = 277 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

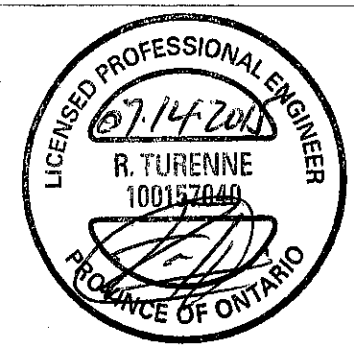
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, L, O						
K	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-1027 / 0	-70.5	-70.5	0.26 (1)	5.88
C- D	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16
D- E	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16
E- F	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16
F- G	-1186 / 0	-70.5	-70.5	0.53 (1)	5.16
G- H	-1027 / 0	-70.5	-70.5	0.26 (1)	5.88
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00
P- B	-1350 / 0	0.0	0.0	0.17 (1)	6.98
J- H	-1350 / 0	0.0	0.0	0.17 (1)	6.98
P- O	0 / 0	-17.5	-17.5	0.13 (4)	10.00
O- N	0 / 611	-17.5	-17.5	0.21 (4)	10.00
N- M	0 / 1187	-17.5	-17.5	0.28 (1)	10.00
M- L	0 / 1187	-17.5	-17.5	0.28 (1)	10.00
L- K	0 / 611	-17.5	-17.5	0.21 (4)	10.00
K- J	0 / 0	-17.5	-17.5	0.13 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-167 / 31	0.20 (1)
C- N	0 / 859	0.19 (1)
N- D	-519 / 0	0.63 (1)
N- F	-1 / 0	0.00 (1)
L- F	-519 / 0	0.63 (1)
L- G	0 / 861	0.19 (1)
K- G	-168 / 31	0.20 (1)
B- O	0 / 673	0.15 (1)
K- H	0 / 673	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.53 (F-G:1) , BC=0.28 (L-N:1) , WB=0.63 (F-L:1) , SSI=0.23 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.34 (M) (INPUT = 1.00)

A-10073578

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:28:30 2015 Page 1
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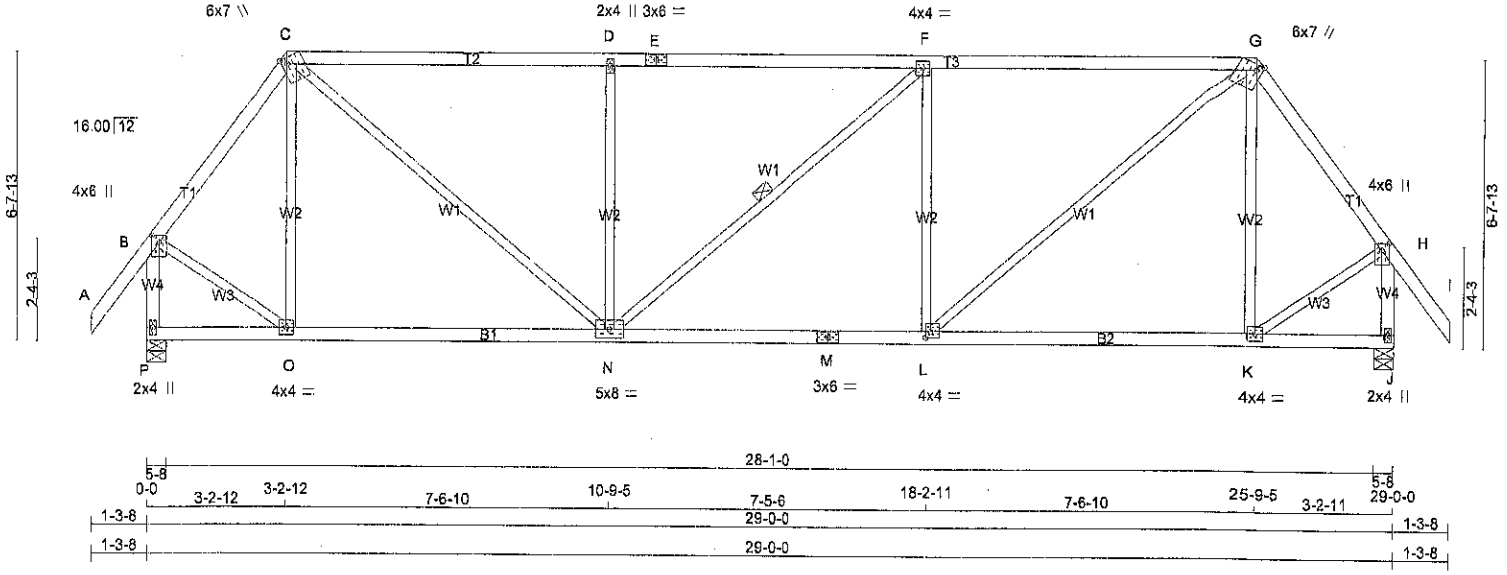


TOTAL WEIGHT = 2 X 130 = 260 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	8.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00		O- C	-237 / 8	0.17 (1)
B- C	-999 / 0	-70.5	-70.5	0.15 (1)	6.09		C- N	0 / 1094	0.25 (1)
C- D	-1430 / 0	-70.5	-70.5	0.68 (1)	4.60		N- D	-570 / 0	0.42 (1)
D- E	-1431 / 0	-70.5	-70.5	0.68 (1)	4.59		N- F	-1 / 0	0.00 (1)
E- F	-1431 / 0	-70.5	-70.5	0.68 (1)	4.59		L- F	-570 / 0	0.42 (1)
F- G	-1431 / 0	-70.5	-70.5	0.68 (1)	4.58		L- G	0 / 1096	0.25 (1)
G- H	-998 / 0	-70.5	-70.5	0.15 (1)	6.09		K- G	-238 / 8	0.17 (1)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00		B- O	0 / 690	0.16 (1)
P- B	-1365 / 0	0.0	0.0	0.17 (1)	6.95		K- H	0 / 690	0.16 (1)
J- H	-1365 / 0	0.0	0.0	0.17 (1)	6.95				
P- O	0 / 0	-17.5	-17.5	0.15 (4)	10.00				
O- N	0 / 591	-17.5	-17.5	0.24 (4)	10.00				
N- M	0 / 1432	-17.5	-17.5	0.34 (1)	10.00				
M- L	0 / 1432	-17.5	-17.5	0.34 (1)	10.00				
L- K	0 / 591	-17.5	-17.5	0.24 (4)	10.00				
K- J	0 / 0	-17.5	-17.5	0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68 (F-G:1) , BC=0.34 (L-N:1) , WB=0.42 (F-L:1) , SSI=0.25 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)

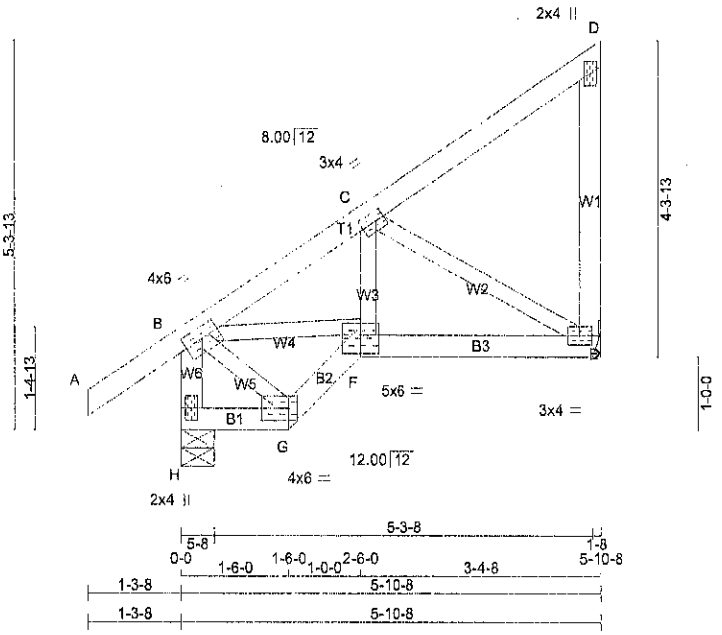
A-10073577

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	J01C	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-W6xnVD3hhVShzxZK?bOoaho1O_JoLC3LHxyJQZyy7mC

Scale: 3/8"=1'



TOTAL WEIGHT = 7 X 30 = 208 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-t	MT20	5.0	6.0	3.00	3.00
G	BBW-t	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-B	-342 / 0	0.0	0.0	0.04	(1)	7.81	B-G	0 / 18	0.01 (4)
A-B	0 / 27	-70.5	-70.5	0.09	(1)	10.00	B-F	0 / 238	0.05 (1)
B-C	-287 / 0	-70.5	-70.5	0.10	(1)	6.25	F-C	0 / 57	0.02 (4)
C-D	-12 / 0	-70.5	-70.5	0.10	(1)	6.25	C-E	-284 / 0	0.07 (1)
E-D	-93 / 0	0.0	0.0	0.03	(1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.01	(4)	10.00			
G-F	0 / 20	-17.5	-17.5	0.01	(4)	10.00			
F-E	0 / 248	-17.5	-17.5	0.08	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.08 (E-F:4) , WB=0.07 (C-E:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
Jul. 14. 2015
R. TURENNE
100157041
PROVINCE OF ONTARIO

A-1507357Y

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

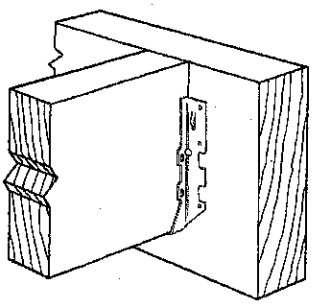
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

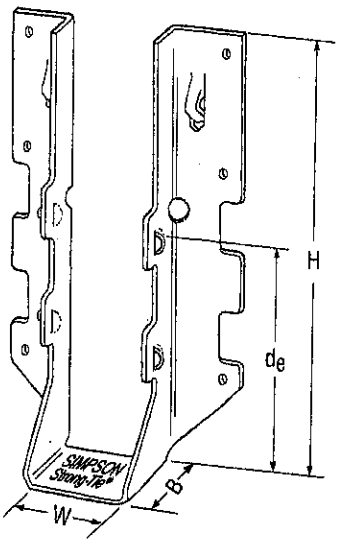
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



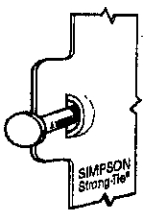
Typical LUS Installation



LUS28

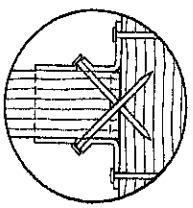
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	11 5/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	11 3/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

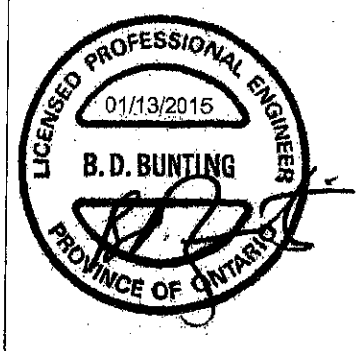


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

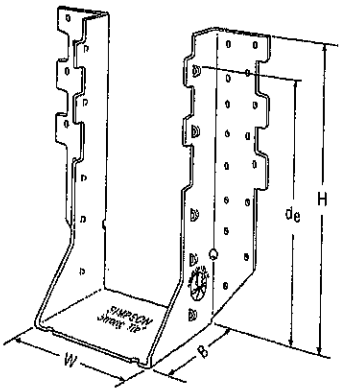
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

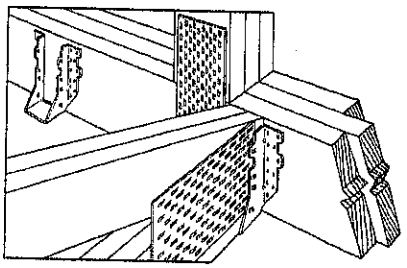
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



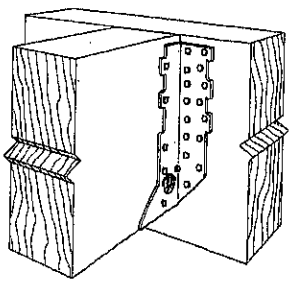
HHUS410



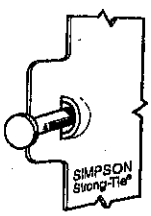
Typical HHUS installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 19/16	3	3 15/16	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 1/32	3	6 5/32	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	30-16d	10-16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 13/32	3	3 15/16	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3 5/8	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	30-16d	10-16d	4745	10770	4310	7650

1. d_e is the distance from the seat of the hanger to the highest joist nail.

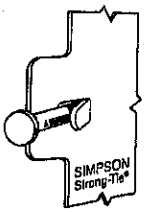


Typical HHUS installation

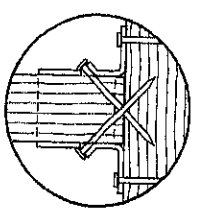


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

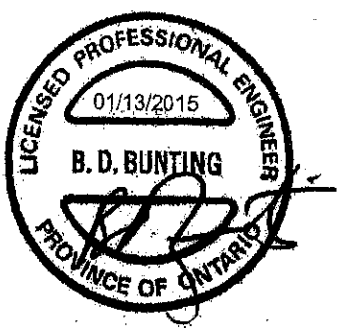
U.S. Patent 5,603,580



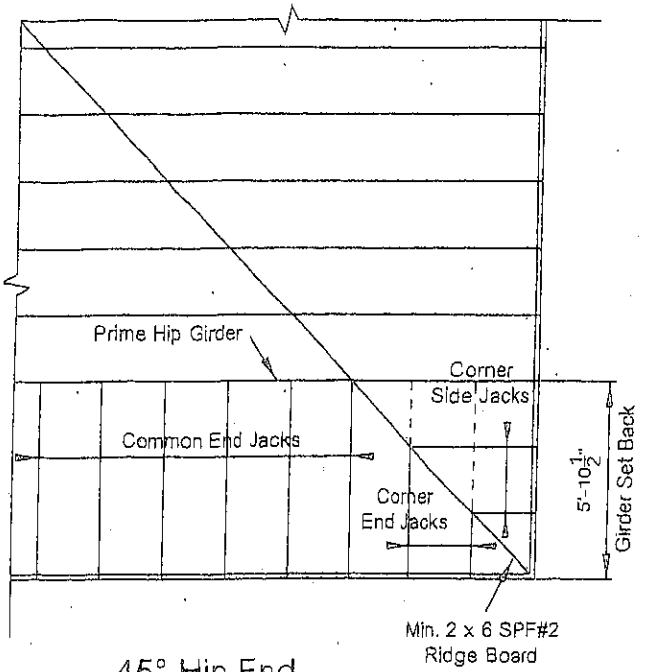
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



STRACON ENGINEERING INC.



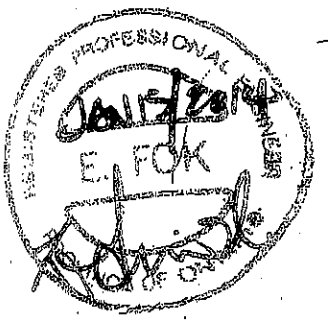
45° Hip End

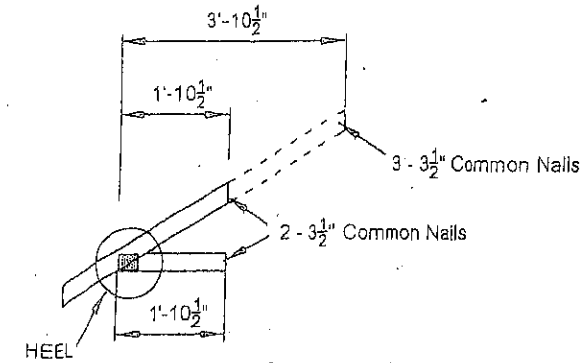
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

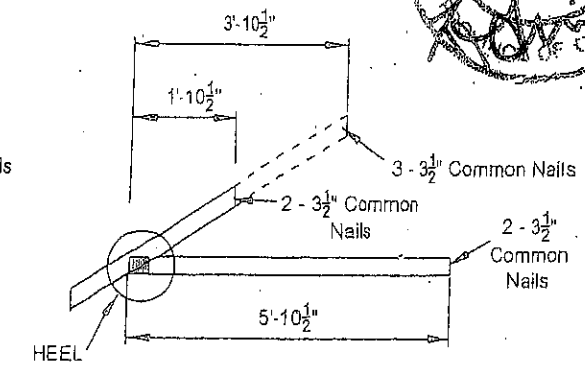
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

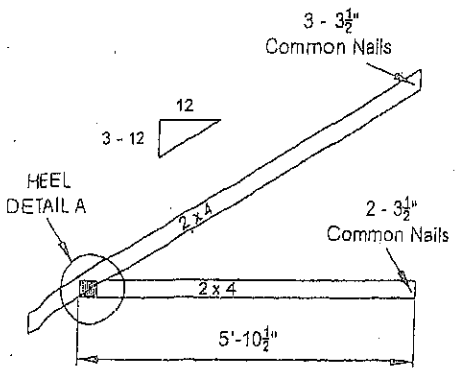




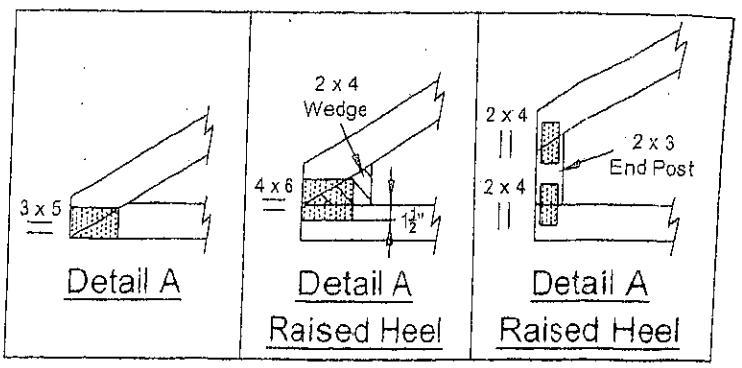
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008