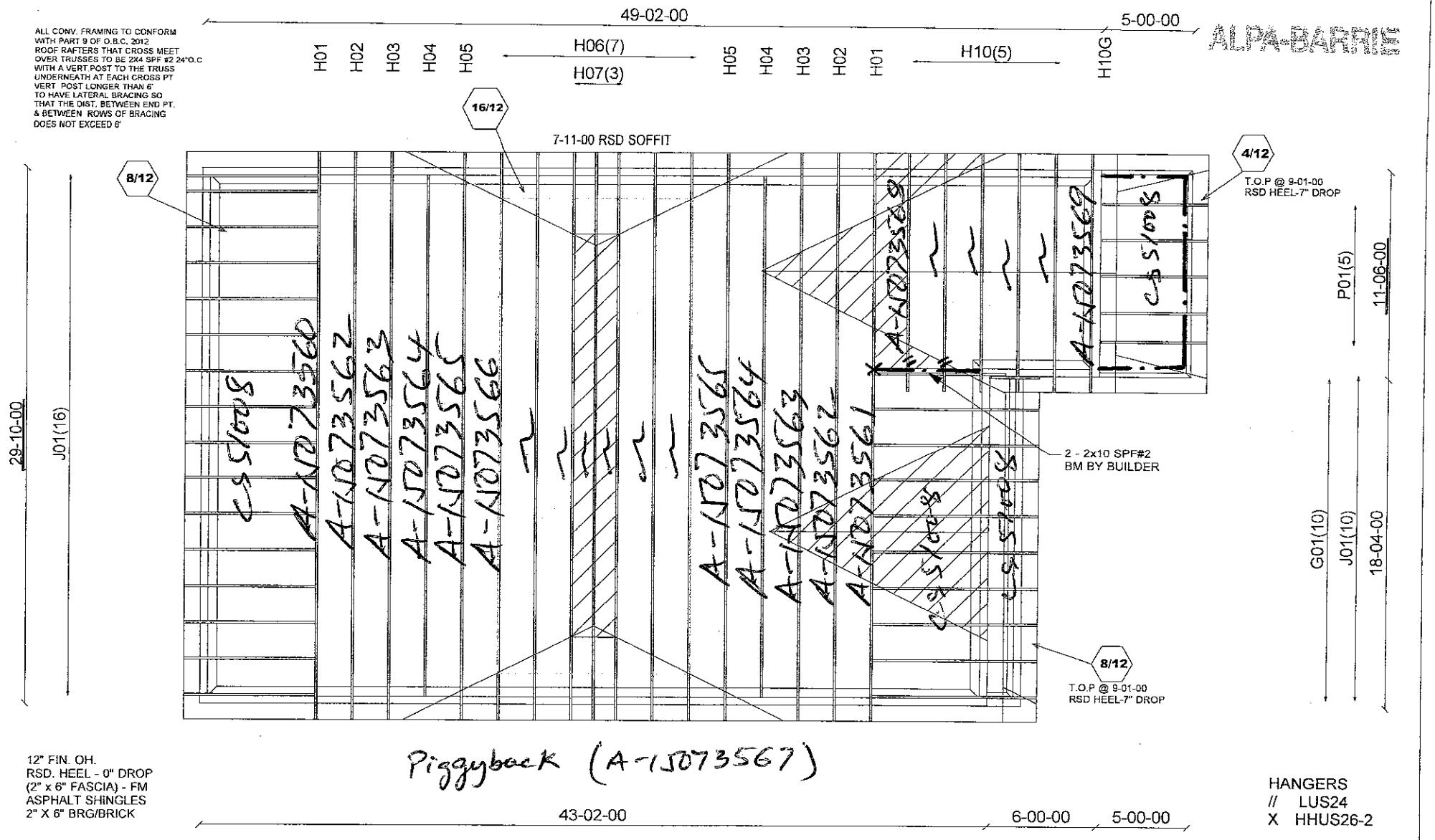
ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C, 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



Piggyback (A-15073567)

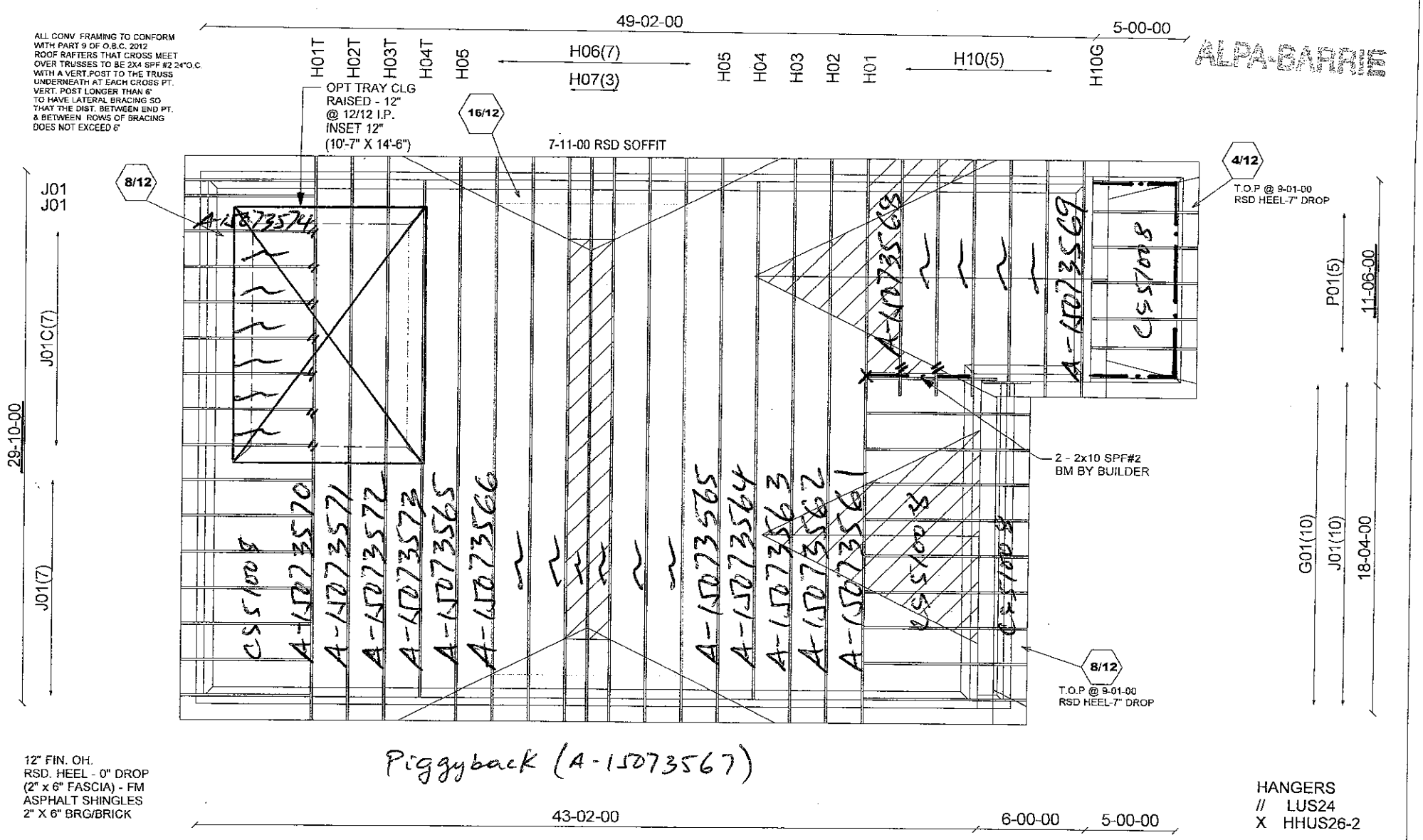
12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK

HANGERS
// LUS24
X HHUS26-2

CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-1 / A	
Layout ID: 253426	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/23/2015	Designer: cheryle	



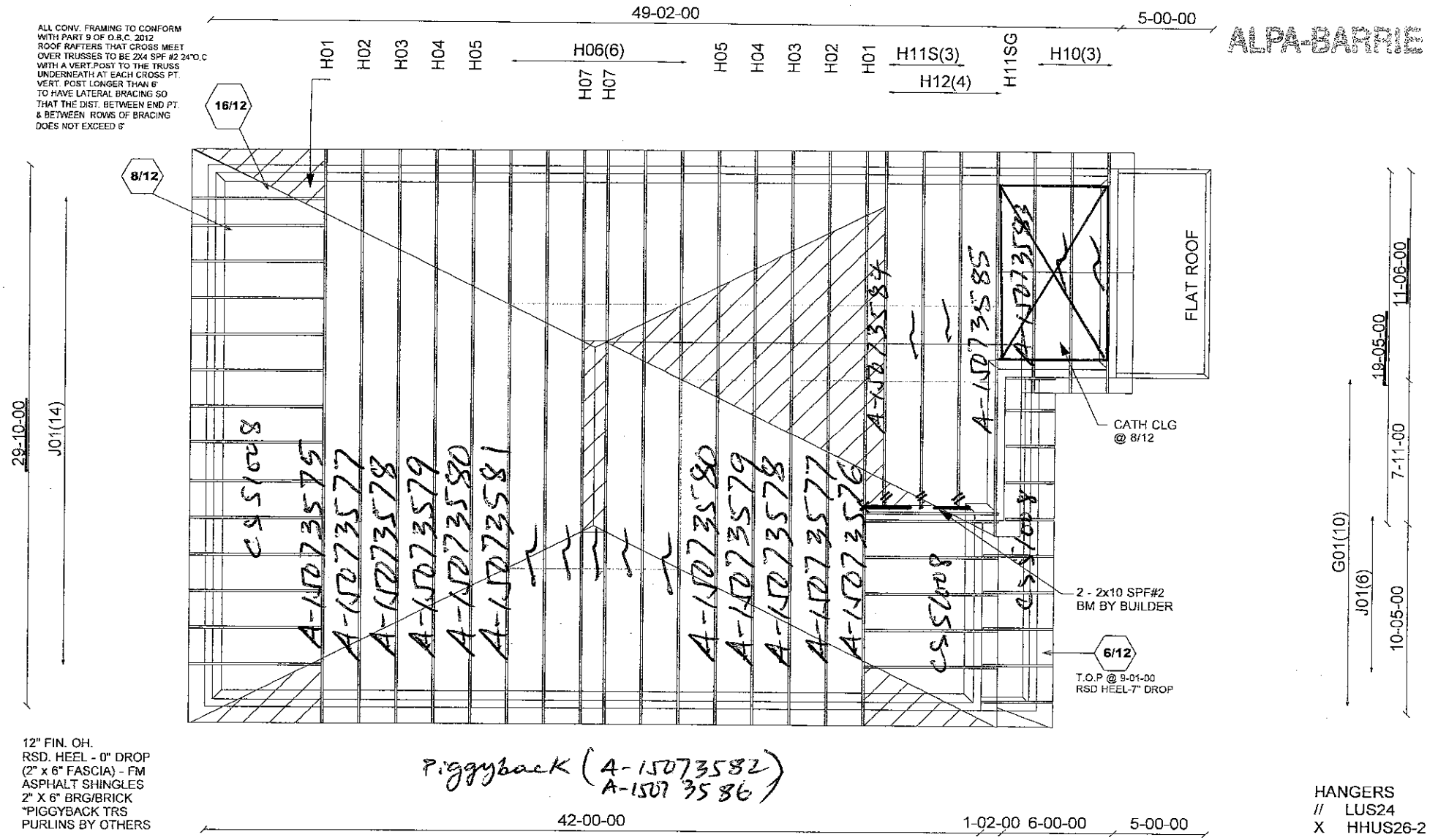
ALPA-BARRIE

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-1 / A OPT TRAY	
Layout ID: 253427	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/23/2015	Designer: cheryle	

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
"PIGGYBACK TRS
PURLINS BY OTHERS

Piggyback (A-15073582 A-15073586)

HANGERS
// LUS24
X HHUS26-2

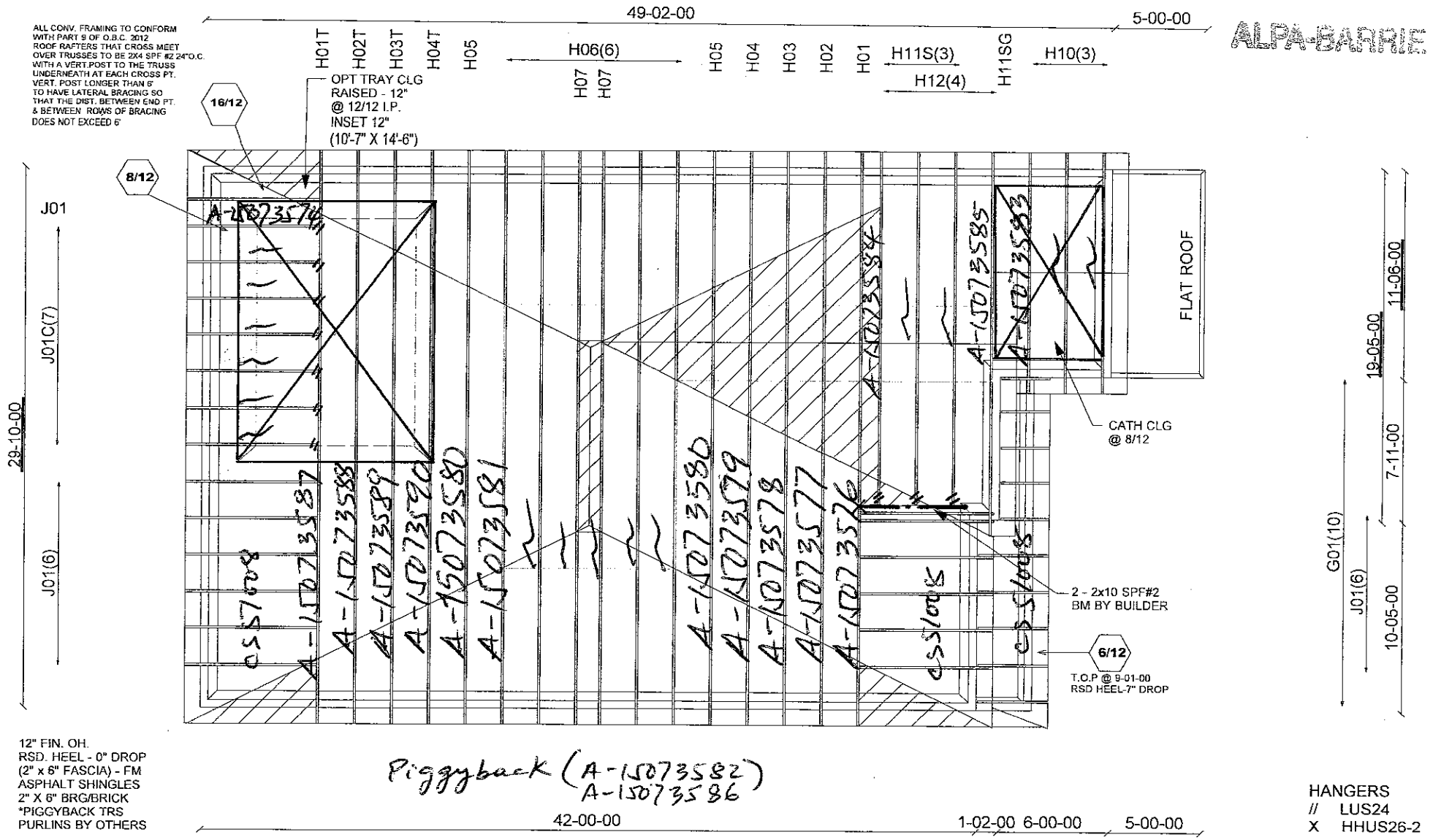
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-1 / B	
Layout ID: 253428	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/22/2015	Designer: cheryle	

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



HANGERS
// LUS24
X HHUS26-2

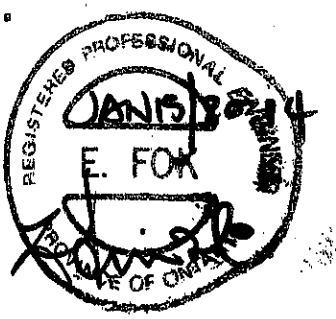
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 38-1 / B OPT TRAY
Layout ID: 253429	Project: HUNTINGTON & NASHVILLE	
Plan Log: 79711	Date: 4/22/2015 Designer: cheryle	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

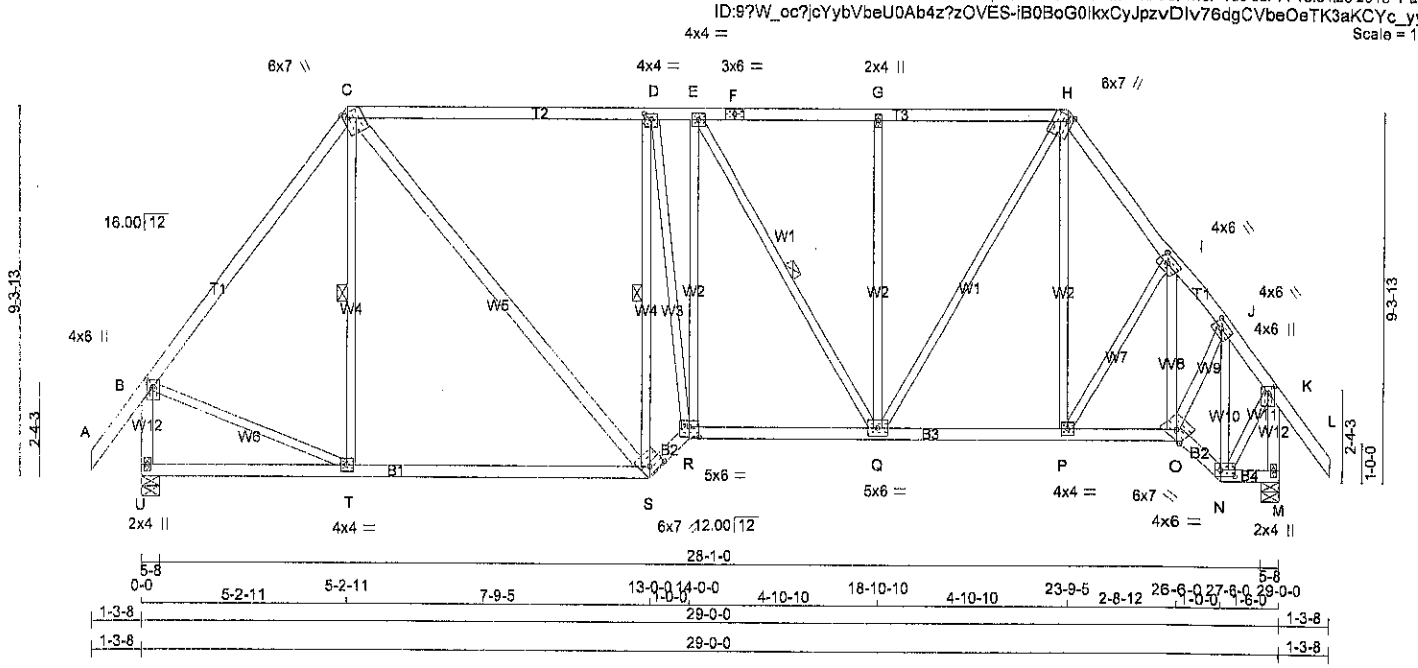
January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253429	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:34:26 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-iB0BoG0IkxCyJpzvDIv76dgCVbeOeTK3aKCYc_yy76x

Scale = 1:58.4



TOTAL WEIGHT = 172 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMWW-t	MT20	4.0	4.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	7.0	Edge	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-t	MT20	4.0	6.0	2.00	4.50
O	BBWW-h	MT20	6.0	7.0	Edge	
P	BMWW-t	MT20	4.0	4.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BBWW-t	MT20	5.0	6.0	3.00	3.00
S	BBWW-h	MT20	6.0	7.0	2.00	4.25
T	BMWW-t	MT20	4.0	4.0		
U	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1372	0	1372	0	0	5-8	1-8
M	1377	0	1377	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	964	866 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	968	869 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-T, D-S, E-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-1038 / 0	-70.5	-70.5	0.43 (1)	5.67
C- D	-1062 / 0	-70.5	-70.5	0.76 (1)	4.88
D- E	-1160 / 0	-70.5	-70.5	0.66 (1)	4.70
E- F	-1069 / 0	-70.5	-70.5	0.28 (1)	5.73
F- G	-1069 / 0	-70.5	-70.5	0.28 (1)	5.73
G- H	-1069 / 0	-70.5	-70.5	0.28 (1)	5.73
H- I	-1156 / 0	-70.5	-70.5	0.08 (1)	5.86
I- J	-1084 / 0	-70.5	-70.5	0.06 (1)	6.01
J- K	-784 / 0	-70.5	-70.5	0.10 (1)	6.25
K- L	0 / 40	-70.5	-70.5	0.10 (1)	10.00
U- B	-1344 / 0	0.0	0.0	0.17 (1)	6.99
M- K	-1364 / 0	0.0	0.0	0.17 (1)	6.96
U- T	0 / 0	-17.5	-17.5	0.23 (4)	10.00
T- S	0 / 619	-17.5	-17.5	0.29 (4)	10.00
S- R	0 / 1524	-17.5	-17.5	0.25 (1)	10.00
R- Q	0 / 1155	-17.5	-17.5	0.24 (1)	10.00
Q- P	0 / 682	-17.5	-17.5	0.16 (1)	10.00
P- O	0 / 680	-17.5	-17.5	0.14 (1)	10.00
O- N	0 / 562	-17.5	-17.5	0.10 (1)	10.00
N- M	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-85 / 86	0.05 (1)
C- S	0 / 676	0.11 (1)
S- D	-1522 / 0	0.87 (1)
D- R	0 / 731	0.16 (1)
R- E	0 / 402	0.09 (1)
E- Q	-168 / 0	0.10 (1)
Q- G	-400 / 0	0.55 (1)
Q- H	0 / 742	0.17 (1)
P- H	0 / 72	0.02 (4)
P- I	0 / 6	0.00 (4)
O- I	-221 / 0	0.07 (1)
O- J	0 / 715	0.16 (1)
N- J	-954 / 0	0.25 (1)
B- T	0 / 661	0.15 (1)
N- K	0 / 714	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.76 (C-D:1) , BC=0.29 (S-T:4) , WB=0.87 (D-S:1) , SSI=0.36 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (R) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

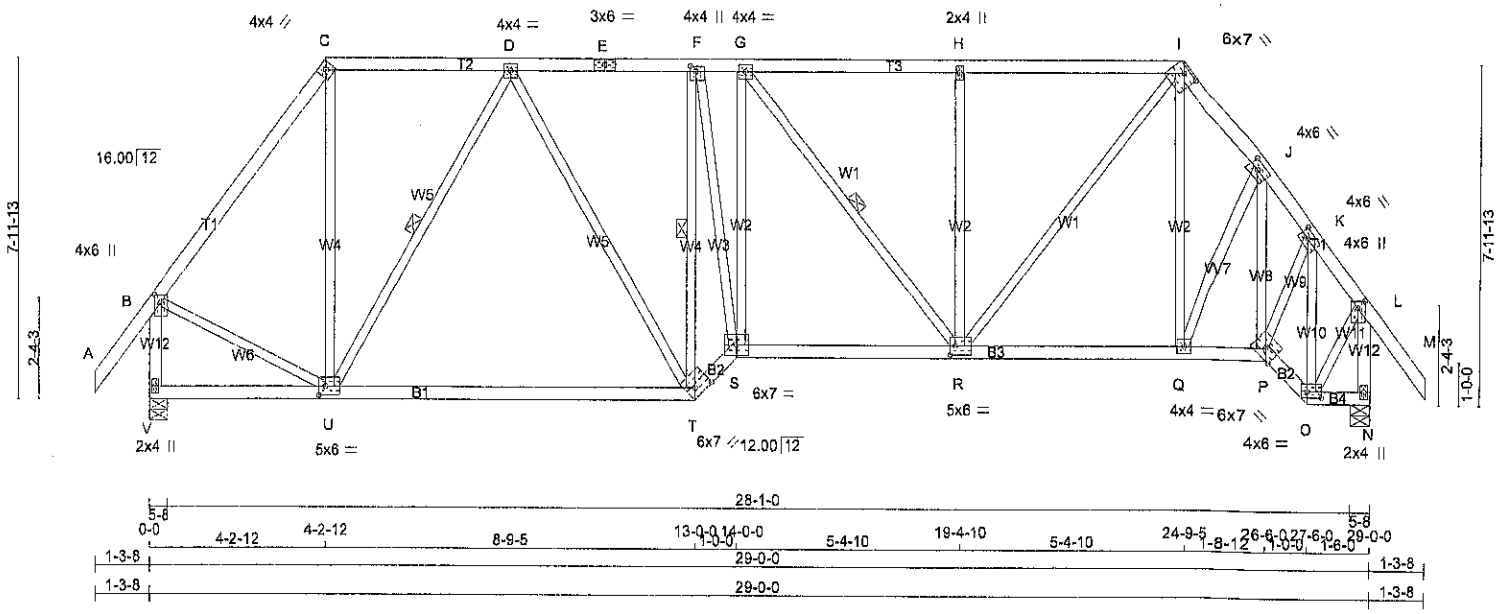
A-150735 90

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253429	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:34:25 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-D?Spaw0gze4SifOjf1OuaP77ABHzv4PwLgS?4Yyy76y

Scale = 1:54.3



TOTAL WEIGHT = 162 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW-h	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	4.0	1.50	1.50
G	TMWW-t	MT20	4.0	4.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	6.0	7.0	1.50	3.75
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMWW-t	MT20	4.0	6.0	2.00	2.50
L	TMVW+p	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	2.0	4.0		
O	BBWW-t	MT20	4.0	6.0	1.75	4.25
P	BBWW-h	MT20	6.0	7.0	Edge	
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	6.0	2.50	1.50
S	BBWW-t	MT20	6.0	7.0		
T	BBWW-h	MT20	6.0	7.0	2.25	4.50
U	BMWWW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1372	0	1372	0	0	5-8	1-8
N	1377	0	1377	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
N	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-U, F-T, G-R.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B-C	-1038 / 0	-70.5	-70.5	0.26 (1)	5.86
C-D	-627 / 0	-70.5	-70.5	0.21 (1)	6.25
D-E	-1199 / 0	-70.5	-70.5	0.23 (1)	5.56
E-F	-1199 / 0	-70.5	-70.5	0.23 (1)	5.56
F-G	-1390 / 0	-70.5	-70.5	0.14 (1)	5.37
G-H	-1258 / 0	-70.5	-70.5	0.35 (1)	5.30
H-I	-1258 / 0	-70.5	-70.5	0.36 (1)	5.30
I-J	-1166 / 0	-70.5	-70.5	0.04 (1)	5.87
J-K	-1097 / 0	-70.5	-70.5	0.04 (1)	6.00
K-L	-784 / 0	-70.5	-70.5	0.10 (1)	6.25
L-M	0 / 40	-70.5	-70.5	0.10 (1)	10.00
V-B	-1362 / 0	0.0	0.0	0.17 (1)	6.96
N-L	-1364 / 0	0.0	0.0	0.17 (1)	6.95
V-U	0 / 0	-17.5	-17.5	0.29 (4)	10.00
U-T	0 / 1009	-17.5	-17.5	0.36 (4)	10.00
T-S	0 / 1720	-17.5	-17.5	0.28 (1)	10.00
S-R	0 / 1390	-17.5	-17.5	0.29 (1)	10.00
R-Q	0 / 689	-17.5	-17.5	0.18 (4)	10.00
Q-P	0 / 673	-17.5	-17.5	0.15 (1)	10.00
P-O	0 / 585	-17.5	-17.5	0.10 (1)	10.00
O-N	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	0 / 557	0.13 (1)
U-D	-785 / 0	0.42 (1)
D-T	0 / 383	0.09 (1)
T-F	-1474 / 0	0.59 (1)
F-S	0 / 1282	0.29 (1)
S-G	0 / 39	0.01 (4)
G-R	-213 / 0	0.10 (1)
R-H	-437 / 0	0.36 (1)
R-I	0 / 903	0.20 (1)
Q-I	0 / 57	0.02 (4)
Q-J	0 / 42	0.01 (1)
P-J	-213 / 0	0.07 (1)
P-K	0 / 694	0.16 (1)
O-K	-957 / 0	0.25 (1)
B-U	0 / 680	0.15 (1)
O-L	0 / 716	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.35 (H-I:1) , BC=0.36 (T-U:4) , WB=0.59 (F-T:1) , SSI=0.18 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

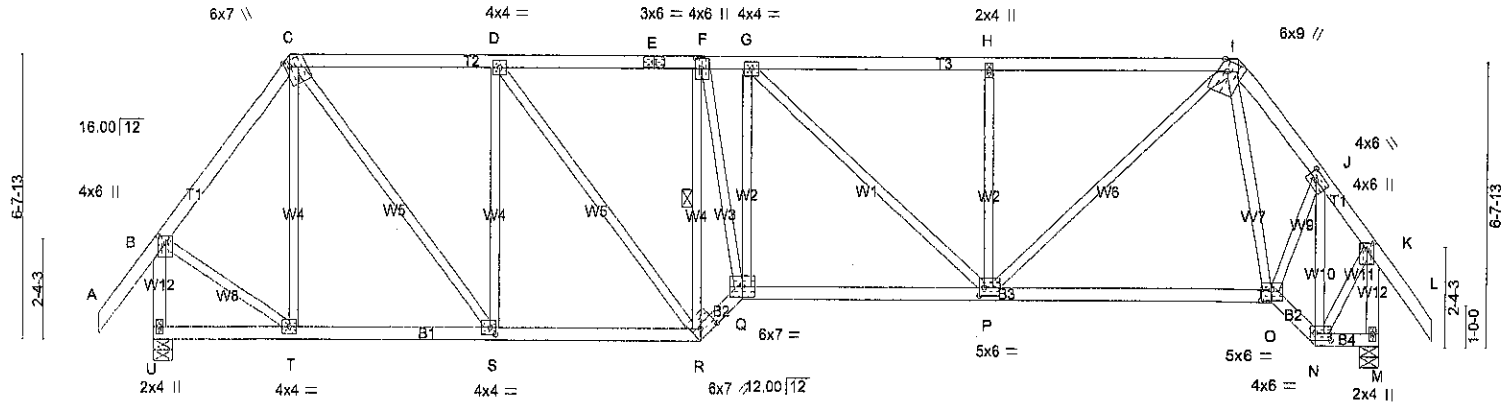
JSI GRIP= 0.79 (L) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

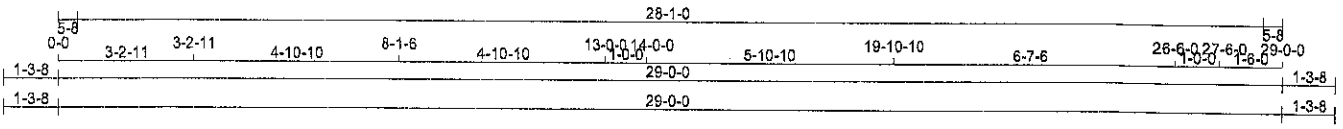
A-N0735589

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253429	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:34:24 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-IpuRNa?1CKxF4VpX6Ksf1Cbx_nwNAeFn70jSX5yy76z
Scale = 1:54.2





TOTAL WEIGHT = 149 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0	3.00	1.75
G	TMWW-t	MT20	4.0	4.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-l	MT20	4.0	6.0	2.00	4.50
O	BBWW-l	MT20	5.0	6.0	3.00	3.00
P	BMWWW-t	MT20	5.0	6.0	2.25	1.50
Q	BBWW-l	MT20	6.0	7.0		
R	BBWW-h	MT20	6.0	7.0	2.25	4.50
S	BMWW-t	MT20	4.0	4.0	2.00	1.50
T	BMWW-t	MT20	4.0	4.0		
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1372	0	1372	0	0	5-8	1-8
M	1377	0	1377	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	964	666 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	968	669 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-R.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	1 (1)	0.10	10.00
B- C	-987 / 0	-70.5	-70.5	1 (1)	0.15	6.11
C- D	-1203 / 0	-70.5	-70.5	1 (1)	0.29	5.48
D- E	-1464 / 0	-70.5	-70.5	1 (1)	0.30	5.08
E- F	-1464 / 0	-70.5	-70.5	1 (1)	0.30	5.08
F- G	-1743 / 0	-70.5	-70.5	1 (1)	0.17	4.88
G- H	-1542 / 0	-70.5	-70.5	1 (1)	0.44	4.79
H- I	-1542 / 0	-70.5	-70.5	1 (1)	0.45	4.79
I- J	-1105 / 0	-70.5	-70.5	1 (1)	0.06	5.97
J- K	-782 / 0	-70.5	-70.5	1 (1)	0.10	6.25
K- L	0 / 40	-70.5	-70.5	1 (1)	0.10	10.00
U- B	-1352 / 0	0.0	0.0	1 (1)	0.17	6.97
M- K	-1364 / 0	0.0	0.0	1 (1)	0.17	6.95
U- T	0 / 0	-17.5	-17.5	4 (4)	0.06	10.00
T- S	0 / 584	-17.5	-17.5	4 (4)	0.15	10.00
S- R	0 / 1204	-17.5	-17.5	4 (4)	0.24	10.00
R- Q	0 / 2089	-17.5	-17.5	4 (4)	0.34	10.00
Q- P	0 / 1743	-17.5	-17.5	4 (4)	0.38	10.00
P- O	0 / 675	-17.5	-17.5	4 (4)	0.25	10.00
O- N	0 / 568	-17.5	-17.5	4 (4)	0.10	10.00
N- M	0 / 0	-17.5	-17.5	4 (4)	0.01	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
T- C	-268 / 0	0.20	1 (1)
C- S	0 / 1012	0.23	1 (1)
S- D	-714 / 0	0.52	1 (1)
D- R	0 / 410	0.09	1 (1)
R- F	-1751 / 0	0.50	1 (1)
F- Q	0 / 1563	0.35	1 (1)
Q- G	-7 / 38	0.01	4 (4)
G- P	-274 / 0	0.35	1 (1)
P- H	-474 / 0	0.23	1 (1)
P- I	0 / 1168	0.26	1 (1)
I- O	-135 / 0	0.07	1 (1)
O- J	0 / 653	0.15	1 (1)
N- J	-964 / 0	0.25	1 (1)
B- T	0 / 682	0.15	1 (1)
N- K	0 / 721	0.16	1 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
6.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.45 (H-I:1) , BC=0.38 (P-Q:1) , WB=0.62 (D-S:1) , SSI=0.20 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.29 (Q) (INPUT = 1.00)

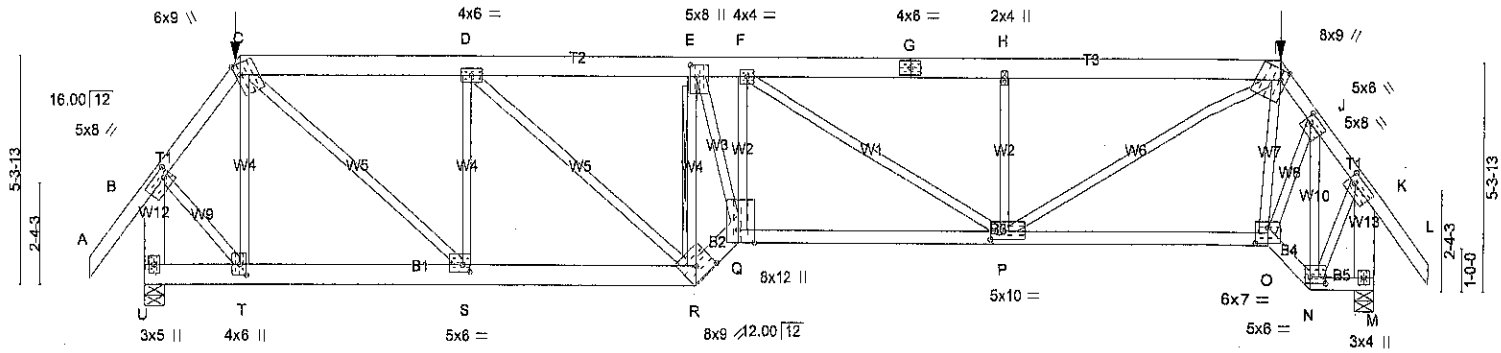
A-10073588

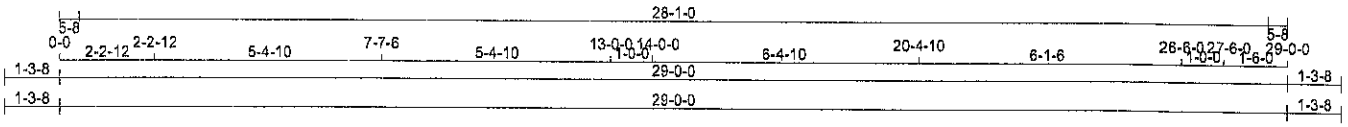
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253429	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:97W_oc?jcYybVbeU0Ab4z?zOVES-HcK39E_PR0pOSLEKYcLQU_2mdNTuR52duMzu0fyy77_

Scale = 1:54.0





TOTAL WEIGHT = 164 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - Q	2x6	DRY	No.2	SPF
Q - O	2x4	DRY	2100F 1.8E	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.00
C	TTWW+m	MT20	6.0	9.0	3.00	1.25
D	TMWW-t	MT20	4.0	6.0		
E	TMWW+t	MT20	5.0	8.0	3.25	1.75
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	4.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	8.0	9.0	2.75	1.75
J	TMWW-t	MT20	5.0	8.0	2.25	1.75
K	TMVW-t	MT20	5.0	8.0	2.25	2.00
M	BMV1+p	MT20	3.0	4.0		
N	BBWW-l	MT20	5.0	6.0	1.75	4.50
O	BBWW-l	MT20	6.0	7.0	4.25	3.50
P	BMWWW-t	MT20	5.0	10.0	2.25	2.75
Q	BBWW+p	MT20	8.0	12.0	Edge	4.75
R	BBWW-h	MT20	8.0	9.0	3.50	4.75
S	BMWW-t	MT20	5.0	6.0	2.00	2.25
T	BMWW+t	MT20	4.0	6.0	3.00	1.75
U	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2572	0	2572	0	0	5-8	2-14
M	2561	0	2561	0	0	5-8	2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0
M	1803	1228 / 0	0 / 0	0 / 0	0 / 0	575 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-R
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00
B-C	-1874 / 0	-70.5	-70.5	0.12 (1)	4.79
C-D	-3010 / 0	-138.7	-138.7	0.38 (1)	4.44
D-E	-3800 / 0	-138.7	-138.7	0.42 (1)	3.98
E-F	-4716 / 0	-138.7	-138.7	0.26 (1)	3.80
F-G	-4051 / 0	-70.5	-70.5	0.35 (1)	3.99
G-H	-4051 / 0	-70.5	-70.5	0.35 (1)	3.99
H-I	-4051 / 0	-70.5	-70.5	0.34 (1)	4.00
I-J	-2231 / 0	-70.5	-70.5	0.11 (1)	4.46
J-K	-1531 / 0	-70.5	-70.5	0.14 (1)	5.15
K-L	0 / 40	-70.5	-70.5	0.11 (1)	10.00
U-B	-2582 / 0	0.0	0.0	0.23 (1)	6.42
M-K	-2535 / 0	0.0	0.0	0.24 (1)	6.47
U-T	0 / 0	-34.5	-34.5	0.09 (1)	10.00
T-S	0 / 1108	-34.5	-34.5	0.23 (1)	10.00
S-R	0 / 3011	-34.5	-34.5	0.47 (1)	10.00
R-Q	0 / 5140	-34.5	-34.5	0.67 (1)	10.00
Q-P	0 / 4705	-102.7	-102.7	0.86 (1)	10.00
P-O	0 / 1336	-102.7	-102.7	0.57 (1)	10.00
O-N	0 / 1174	-34.5	-34.5	0.21 (1)	10.00
N-M	0 / 0	-34.5	-34.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-707 / 0	0.27 (1)
C-S	0 / 2561	0.63 (1)
S-D	-1566 / 0	0.59 (1)
D-R	0 / 1059	0.26 (1)
R-E	-4133 / 0	0.90 (1)
E-Q	0 / 3650	0.90 (1)
Q-F	0 / 385	0.10 (1)
F-P	-773 / 0	0.83 (1)
P-H	-543 / 0	0.15 (1)
P-I	0 / 3189	0.79 (1)
O-I	-231 / 0	0.06 (1)
O-J	0 / 1421	0.35 (1)
N-J	-1923 / 0	0.53 (1)
B-T	0 / 1442	0.36 (1)
N-K	0 / 1438	0.36 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-115	-115	---	FRONT	VERT	TOTAL
I	26-9-5	-115	-115	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 14-0-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CStdGirder
START DISTANCE = 14-0-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 26-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 614 (0.57")

CSI: TC=0.42 (D-E:1) , BC=0.85 (P-Q:1) , WB=0.90 (E-Q:1) , SSI=0.34 (P-Q:1)

A-H07 3587

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.5 lbs FACTORED DOWN AT 2-2-12, AND 114.5 lbs FACTORED DOWN AT 26-9-5 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.70 (Q) (INPUT = 1.00)

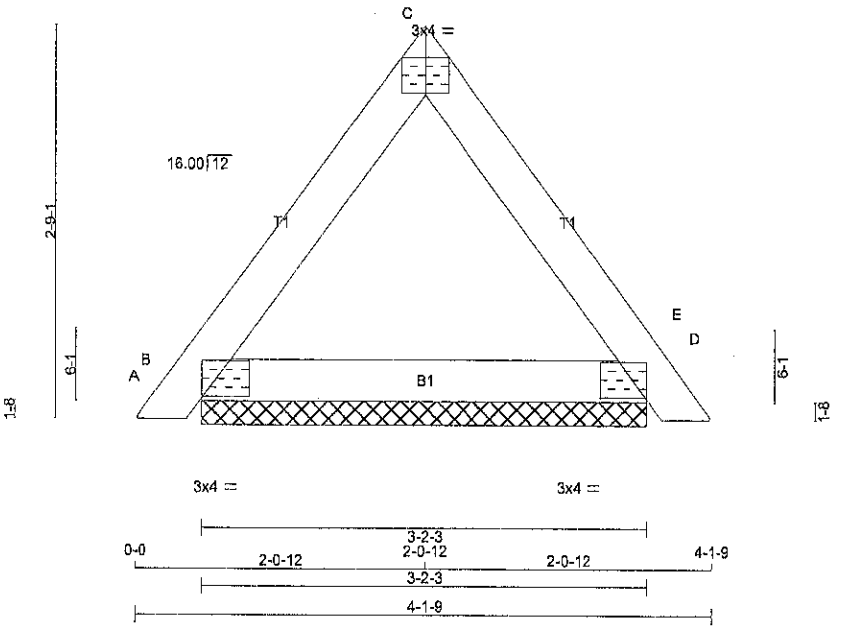
A-10073587 (2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H12	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-xsstKanl9P5dK0CejEd2wpAZlVF8dNFY5tQ1KGyy7CP

Scale = 1:16.2



TOTAL WEIGHT = 4 X 11 = 46 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TT-p	MT20	3.0	4.0	3.25	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	165	0	165	0	0	3-2-3	1-8
D	165	0	165	0	0	3-2-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	115	82 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
D	115	82 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B- C	-69 / 0	-70.5	-70.5	0.03 (1)	6.25			
C- D	-69 / 0	-70.5	-70.5	0.03 (1)	6.25			
D- E	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B- D	0 / 40	-17.5	-17.5	0.05 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03 (C-D:1) , BC=0.05 (B-D:4) , WB=0.00 (n/a:0) , SSI=0.04 (B-D:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)

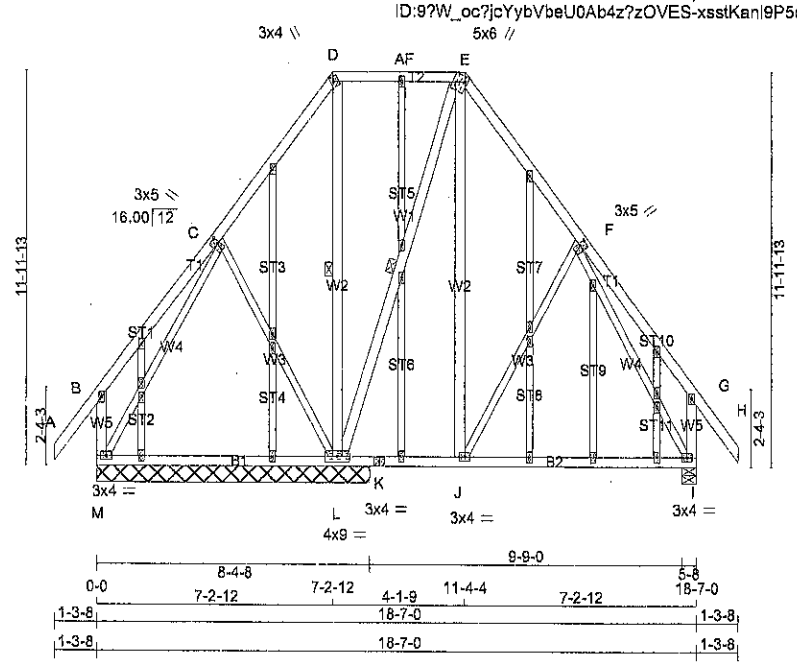
A-150735 86

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H11SG	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:28:36 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-xsstKanl9P5dK0CejEd2wpAWCVChdHYY5tQ1KGyy7CP

Scale = 1:70.4



TOTAL WEIGHT = 160 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - L	2x3	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
F - I	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW+t	MT20	3.0	5.0	2.25	0.75
D	TTW+m	MT20	3.0	4.0	1.75	1.00
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMWW+t	MT20	3.0	5.0	2.25	0.75
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	3.0	4.0		
J	BMWW-t	MT20	3.0	4.0		
K	BS-t	MT20	3.0	4.0		
L	BMWWW1-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	3.0	4.0		
N, O, P, Q, R, R, S, T, U, U, V, W, X, Y, Z, Z, AA, AB, AC, AD, AE, AE						
N	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	894	0	894	0	0	8-4-8	1-8
M	393	0	393	0	0	8-4-8	1-8
I	577	0	577	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	632	417 / 0	0 / 0	0 / 0	0 / 0	215 / 0	0 / 0
M	275	198 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
I	405	284 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-L, E-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)		10.00
B- C	0 / 26	-70.5	-70.5	0.16 (1)		10.00
C- D	-6 / 0	-70.5	-70.5	0.12 (1)		10.00
D-AF	-2 / 15	-78.0	-78.0	0.23 (1)		10.00
AF- E	-2 / 15	-78.0	-78.0	0.23 (1)		10.00
E- F	-197 / 0	-70.5	-70.5	0.12 (1)		6.25
F- G	0 / 25	-70.5	-70.5	0.15 (1)		10.00
G- H	0 / 40	-70.5	-70.5	0.10 (1)		10.00
M- B	-192 / 0	0.0	0.0	0.02 (1)		7.81
I- G	-193 / 0	0.0	0.0	0.02 (1)		7.81
M- L	0 / 78	-17.5	-17.5	0.20 (4)		10.00
L- K	0 / 106	-17.5	-17.5	0.19 (4)		10.00
K- J	0 / 106	-17.5	-17.5	0.19 (4)		10.00
J- I	0 / 174	-17.5	-17.5	0.21 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- L	-180 / 0	0.22 (1)
L- D	-274 / 0	0.22 (1)
L- E	-372 / 0	0.33 (1)
J- E	0 / 230	0.04 (1)
J- F	-144 / 0	0.18 (1)
M- C	-167 / 0	0.19 (1)
F- I	-372 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.23 (D-E;1) , BC=0.21 (I-J;4) , WB=0.43 (F-I;1) , SSI=0.13 (D-E;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.21 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/14/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

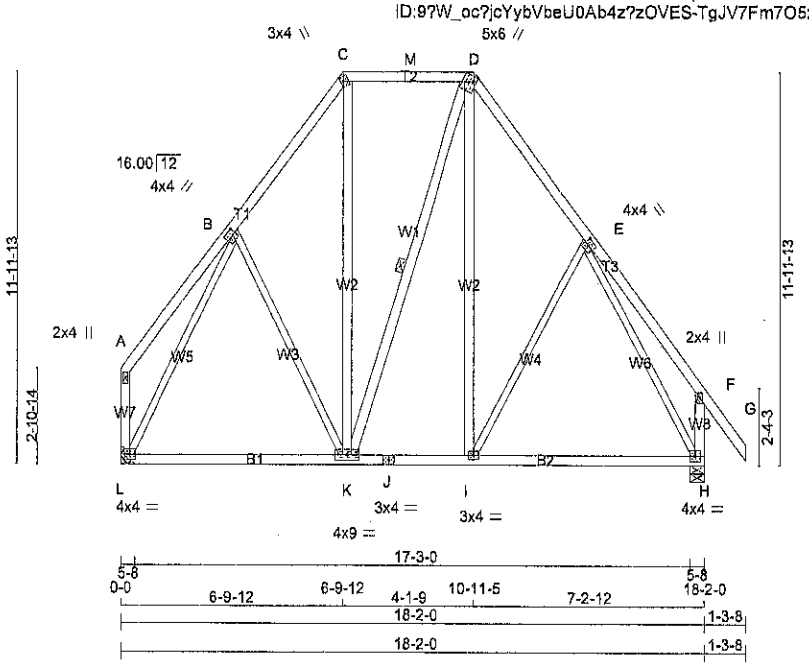
A-10073585

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H11S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:28:35 2015 Page 1
ID:97W_oc?jcYybVbeU0Ab4z?zOVES-TgJV7Fm7O5zmisdS9X6pOceLT5sGuisPsDgUoqyy7CQ

Scale = 1:70.8



TOTAL WEIGHT = 3 X 124 = 373 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - K	2x3	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
L - B	2x3	DRY	No.2	SPF
E - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+m	MT20	3.0	4.0	1.75	1.00
D	TTWW+m	MT20	5.0	6.0	1.75	1.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	4.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	815	0	815	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	913	0	913	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	576	381 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0
H	643	440 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 23	-70.5	-70.5	0.13 (1)	10.00	
B- C	-534 / 0	-70.5	-70.5	0.10 (1)	6.25	
C- M	-307 / 0	-78.0	-78.0	0.23 (1)	6.25	
M- D	-307 / 0	-78.0	-78.0	0.23 (1)	6.25	
D- E	-547 / 0	-70.5	-70.5	0.12 (1)	6.25	
E- F	0 / 24	-70.5	-70.5	0.15 (1)	10.00	
F- G	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
L- A	-89 / 0	0.0	0.0	0.01 (1)	7.81	
H- F	-194 / 0	0.0	0.0	0.02 (1)	7.81	
L- K	0 / 323	-17.5	-17.5	0.20 (4)	10.00	
K- J	0 / 315	-17.5	-17.5	0.21 (4)	10.00	
J- I	0 / 315	-17.5	-17.5	0.21 (4)	10.00	
I- H	0 / 352	-17.5	-17.5	0.22 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	-37 / 25	0.05 (1)
K- C	0 / 155	0.02 (1)
K- D	-22 / 0	0.02 (1)
I- D	0 / 180	0.03 (1)
I- E	-81 / 13	0.10 (1)
L- B	-747 / 0	0.91 (1)
E- H	-752 / 0	0.87 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.23 (C-D:1) , BC=0.22 (H-I:4) , WB=0.91 (B-L:1) , SSI=0.13 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073584

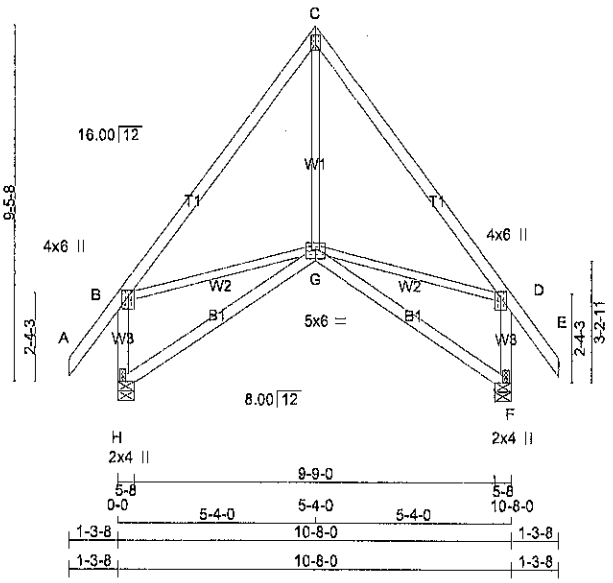
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H10	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-?TI7wvmVdorv5j3GbpbarO59BhXE9T_FeZxwGOyy7CR

3x5 ||

Scale = 1:61.6



TOTAL WEIGHT = 3 X 59 = 176 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	568	0	568	0	0	5-8	1-8
F	568	0	568	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	398	283 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	398	283 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G- C	0 / 197	0.05 (4)
B- C	-358 / 0	-70.5	-70.5	0.26 (1)	6.25	B- G	0 / 221	0.05 (1)
C- D	-358 / 0	-70.5	-70.5	0.26 (1)	6.25	G- D	0 / 221	0.05 (1)
D- E	0 / 40	-70.5	-70.5	0.10 (1)	10.00			
H- B	-522 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-522 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-17.5	-17.5	0.15 (4)	10.00			
G- F	0 / 0	-17.5	-17.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.26 (B-C:1) , BC=0.15 (G-H:4) , WB=0.05 (B-G:1) , SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (H) (INPUT = 0.90)
JSI METAL= 0.12 (H) (INPUT = 1.00)

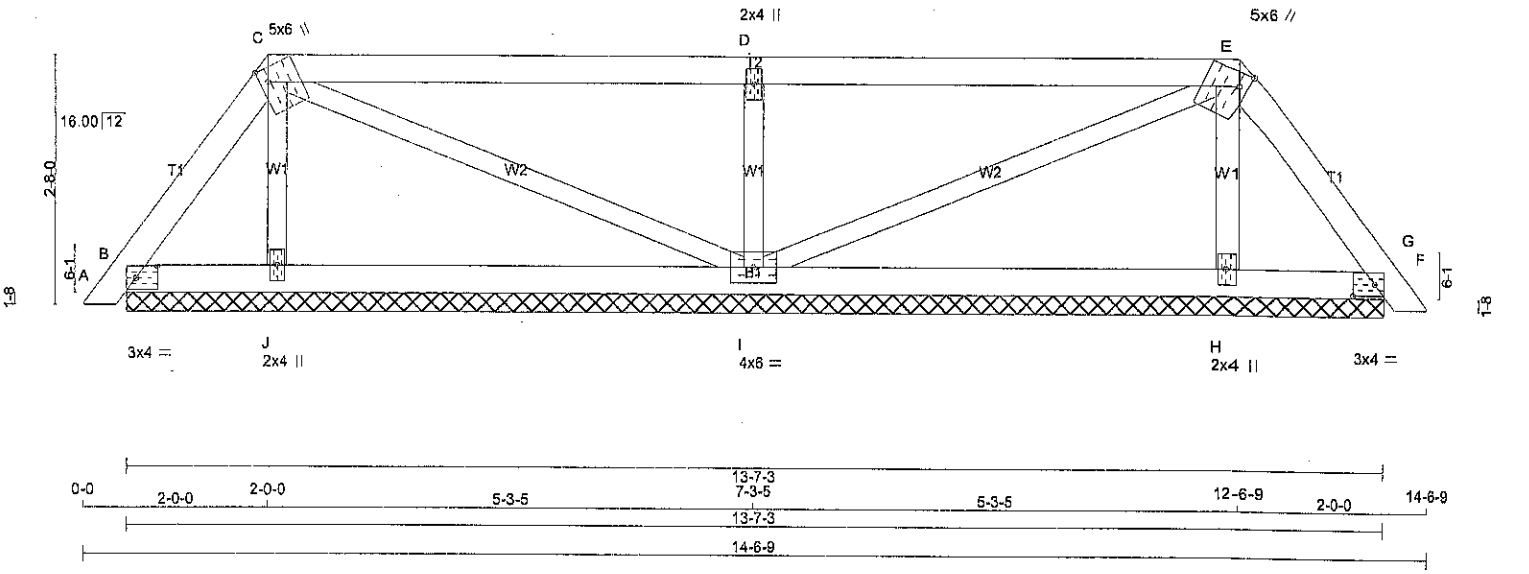
A-1707358 3

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H07	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:28:33 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-XHBlIZlttUj2TZU3263LJBYzLICfQ?L6PvBNkxyy7CS

Scale = 1:24.8



TOTAL WEIGHT = 2 X 49 = 97 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	129	0	129	0	0	13-7-3	1-8
F	129	0	129	0	0	13-7-3	1-8
J	205	0	205	0	0	13-7-3	1-8
I	578	0	578	0	0	13-7-3	1-8
H	205	0	205	0	0	13-7-3	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	87	79 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
F	87	79 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
J	148	78 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
I	405	287 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
H	148	78 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	
B- C	-66 / 0	-70.5	-70.5	0.03 (1)	6.25	
C- D	-9 / 0	-70.5	-70.5	0.33 (1)	10.00	
D- E	-9 / 0	-70.5	-70.5	0.33 (1)	10.00	
E- F	-66 / 0	-70.5	-70.5	0.03 (1)	6.25	
F- G	0 / 9	-70.5	-70.5	0.01 (1)	10.00	
B- J	0 / 37	-17.5	-17.5	0.07 (4)	10.00	
J- I	0 / 31	-17.5	-17.5	0.10 (4)	10.00	
I- H	0 / 31	-17.5	-17.5	0.10 (4)	10.00	
H- F	0 / 37	-17.5	-17.5	0.07 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-131 / 0	0.02 (1)
I- C	-23 / 0	0.01 (1)
I- D	-464 / 0	0.07 (1)
I- E	-23 / 0	0.01 (1)
H- E	-131 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33 (D-E:1) , BC=0.10 (H-I:4) , WB=0.07 (D-I:1) , SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (F) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

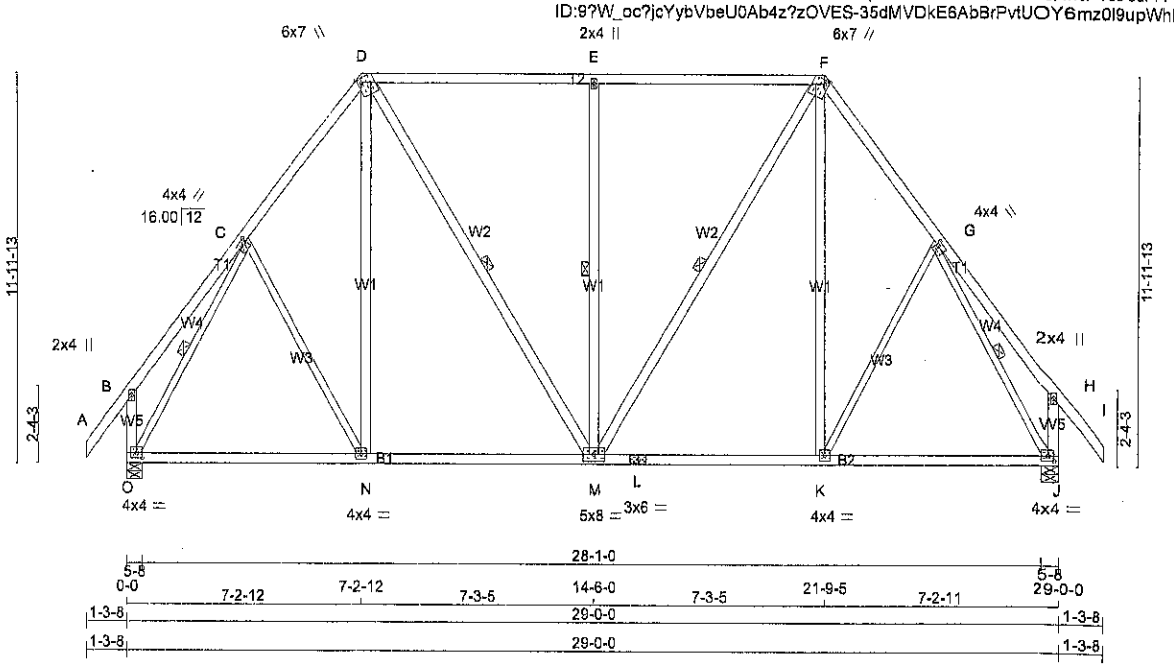
A-150735 82

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H06	6	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-35dMVDkE6AbBrPvtUOY6mz0l9upWhRmzAFRpBVyy7CT

Scale = 1:71.1



TOTAL WEIGHT = 6 X 181 = 1084 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - N	2x3	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF
O - C	2x3	DRY	No.2	SPF
G - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.00
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMWW-t	MT20	4.0	4.0	1.50	1.00
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	4.0	1.75	2.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1429	0	1429	0	0	5-8	1-9
J	1429	0	1429	0	0	5-8	1-9

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1010	668 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0
J	1010	668 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-M, E-M, F-M, C-O, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	C- N	0 / 51	0.02 (4)
B- C	0 / 23	-70.5	-70.5	0.14 (1)	10.00	N- D	0 / 130	0.03 (4)
C- D	-1088 / 0	-70.5	-70.5	0.13 (1)	5.93	D- M	0 / 477	0.08 (1)
D- E	-889 / 0	-78.0	-78.0	0.55 (1)	5.67	M- E	-695 / 0	0.54 (1)
E- F	-889 / 0	-78.0	-78.0	0.55 (1)	5.67	M- F	0 / 477	0.08 (1)
F- G	-1088 / 0	-70.5	-70.5	0.13 (1)	5.93	K- F	0 / 130	0.03 (4)
G- H	0 / 23	-70.5	-70.5	0.14 (1)	10.00	K- G	0 / 51	0.02 (4)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O- C	-1338 / 0	0.52 (1)
O- B	-195 / 0	0.0	0.0	0.02 (1)	7.81	G- J	-1338 / 0	0.52 (1)
J- H	-195 / 0	0.0	0.0	0.02 (1)	7.81			
O- N	0 / 627	-17.5	-17.5	0.28 (4)	10.00			
N- M	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
M- L	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
L- K	0 / 637	-17.5	-17.5	0.29 (4)	10.00			
K- J	0 / 627	-17.5	-17.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.55 (D-E:1) , BC=0.29 (M-N:4) , WB=0.54 (E-M:1) , SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.49 (G) (INPUT = 1.00)

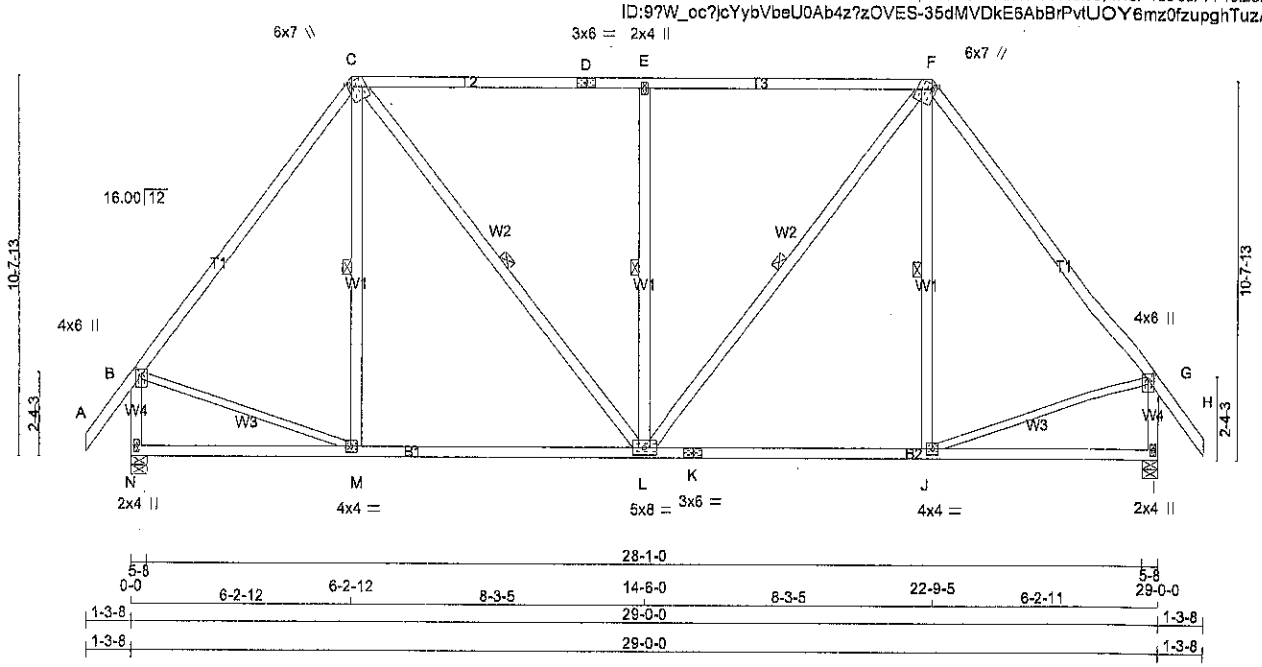
A-1073581

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:97W_oc?jcYybVbeU0Ab4z?zOVES-35dMVDkE6AbBrPvtUOY6mz0fzupghTuzAFRpBVyy7CT

Scale = 1:64.7



TOTAL WEIGHT = 2 X 160 = 319 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - M	2x3	DRY	No.2	SPF
J - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	7.0	Edge	1.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1375	0	1375	0	0	5-8	1-8
I	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
I	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-M, C-L, E-L, F-L, F-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.07 (1)	10.00	M- C	-48 / 95	0.03 (1)
B- C	-1029 / 0	-70.5	-70.5	0.38 (1)	6.25	C- L	0 / 550	0.09 (1)
C- D	-959 / 0	-70.5	-70.5	0.95 (1)	4.39	L- E	-715 / 0	0.41 (1)
D- E	-959 / 0	-70.5	-70.5	0.95 (1)	4.39	L- F	0 / 550	0.09 (1)
E- F	-958 / 0	-70.5	-70.5	0.95 (1)	4.39	J- F	-48 / 95	0.03 (1)
F- G	-1029 / 0	-70.5	-70.5	0.38 (1)	6.25	B- M	0 / 644	0.14 (1)
G- H	0 / 40	-70.5	-70.5	0.07 (1)	10.00	J- G	0 / 644	0.14 (1)
N- B	-1333 / 0	0.0	0.0	0.17 (1)	7.01			
I- G	-1333 / 0	0.0	0.0	0.17 (1)	7.01			
N- M	0 / 0	-17.5	-17.5	0.21 (4)	10.00			
M- L	0 / 615	-17.5	-17.5	0.28 (4)	10.00			
L- K	0 / 615	-17.5	-17.5	0.28 (4)	10.00			
K- J	0 / 615	-17.5	-17.5	0.28 (4)	10.00			
J- I	0 / 0	-17.5	-17.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.95 (C-E:1) , BC=0.28 (L-M:4) , WB=0.41 (E-L:1) , SSI=0.28 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

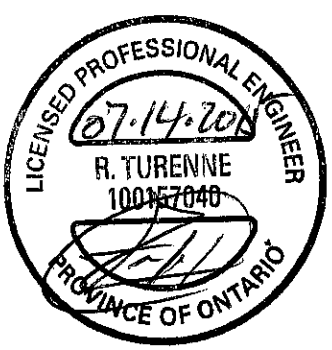
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (M) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)



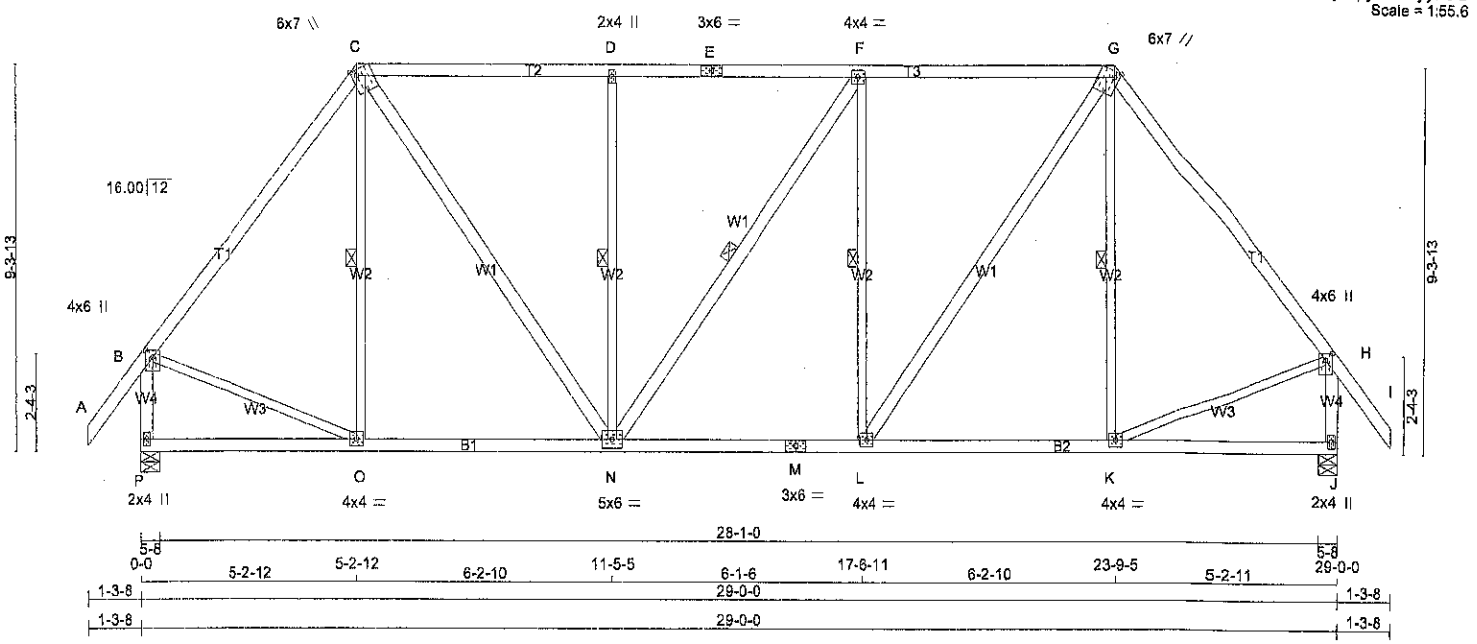
A-1073580

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H04	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-bu3_HtjcLtTKEFKhwh1tDmTcMUU9y2rpybiGf3yy7CU

Scale = 1:55.6



TOTAL WEIGHT = 2 X 158 = 315 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, L, O						
K	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-O, D-N, F-N, F-L, G-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00		O- C	-114 / 51	0.07 (1)
B- C	-1033 / 0	-70.5	-70.5	0.43 (1)	5.68		C- N	0 / 698	0.11 (1)
C- D	-1013 / 0	-70.5	-70.5	0.40 (1)	5.68		N- D	-468 / 0	0.27 (1)
D- E	-1013 / 0	-70.5	-70.5	0.41 (1)	5.66		N- F	-2 / 0	0.00 (1)
E- F	-1013 / 0	-70.5	-70.5	0.41 (1)	5.66		L- F	-468 / 0	0.27 (1)
F- G	-1014 / 0	-70.5	-70.5	0.41 (1)	5.66		L- G	0 / 700	0.11 (1)
G- H	-1033 / 0	-70.5	-70.5	0.43 (1)	5.68		K- G	-115 / 51	0.07 (1)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00		B- O	0 / 657	0.15 (1)
P- B	-1339 / 0	0.0	0.0	0.17 (1)	7.00		K- H	0 / 657	0.15 (1)
J- H	-1338 / 0	0.0	0.0	0.17 (1)	7.00				
P- O	0 / 0	-17.5	-17.5	0.13 (4)	10.00				
O- N	0 / 616	-17.5	-17.5	0.18 (4)	10.00				
N- M	0 / 1014	-17.5	-17.5	0.23 (1)	10.00				
M- L	0 / 1014	-17.5	-17.5	0.23 (1)	10.00				
L- K	0 / 616	-17.5	-17.5	0.18 (4)	10.00				
K- J	0 / 0	-17.5	-17.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43 (B-C:1) , BC=0.23 (L-N:1) , WB=0.27 (F-L:1) , SSI=0.20 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	364	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.28 (M) (INPUT = 1.00)

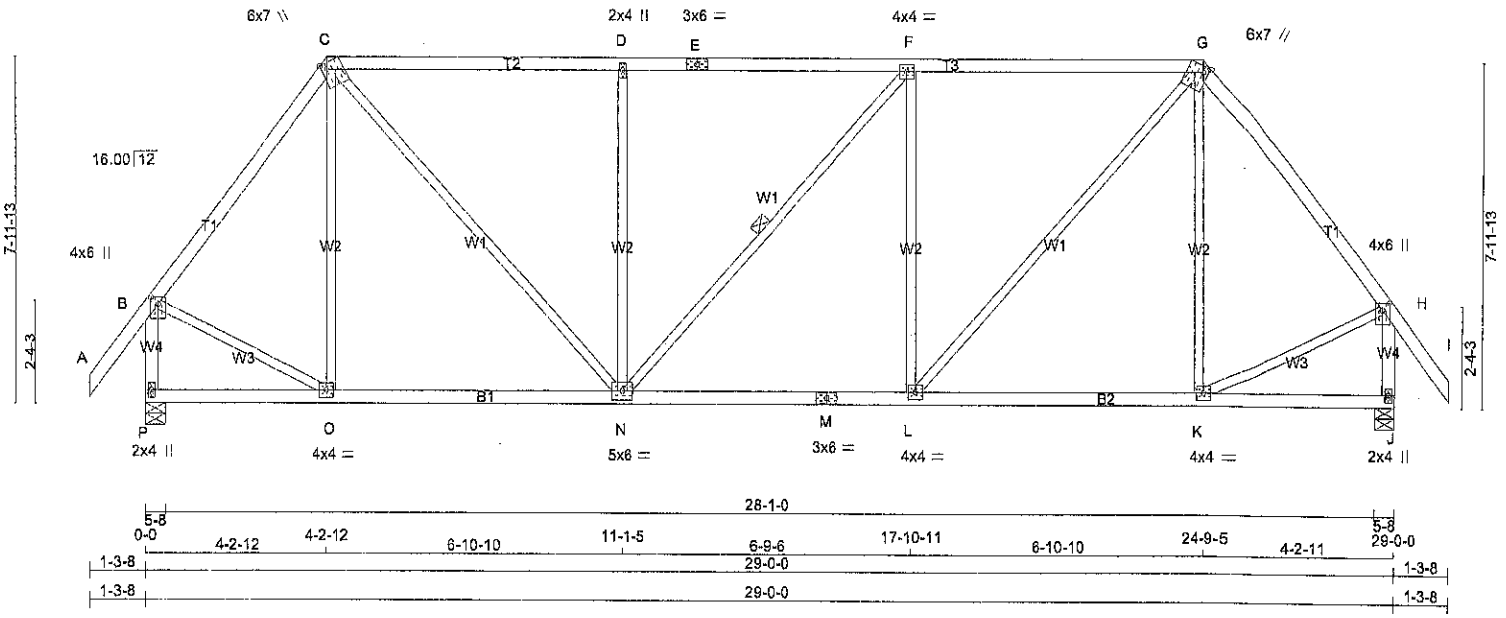
A-150735 79

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:28:30 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-7iVc4X|_aZLTc5IUM_VVehYwP448DOWwgjxyj7dyy7CV

Scale = 1:53.3



TOTAL WEIGHT = 2 X 138 = 277 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, L, O						
K	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O- C	-167 / 31	0.20 (1)
B- C	-1027 / 0	-70.5	-70.5	0.26 (1)	5.88	C- N	0 / 859	0.19 (1)
C- D	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16	N- D	-519 / 0	0.63 (1)
D- E	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16	N- F	-1 / 0	0.00 (1)
E- F	-1186 / 0	-70.5	-70.5	0.52 (1)	5.16	L- F	-519 / 0	0.63 (1)
F- G	-1186 / 0	-70.5	-70.5	0.53 (1)	5.16	L- G	0 / 861	0.19 (1)
G- H	-1027 / 0	-70.5	-70.5	0.26 (1)	5.88	K- G	-168 / 31	0.20 (1)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	B- O	0 / 673	0.15 (1)
P- B	-1350 / 0	0.0	0.0	0.17 (1)	6.98	K- H	0 / 673	0.15 (1)
J- H	-1350 / 0	0.0	0.0	0.17 (1)	6.98			
P- O	0 / 0	-17.5	-17.5	0.13 (4)	10.00			
O- N	0 / 611	-17.5	-17.5	0.21 (4)	10.00			
N- M	0 / 1187	-17.5	-17.5	0.28 (1)	10.00			
M- L	0 / 1187	-17.5	-17.5	0.28 (1)	10.00			
L- K	0 / 611	-17.5	-17.5	0.21 (4)	10.00			
K- J	0 / 0	-17.5	-17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.53 (F-G:1) , BC=0.28 (L-N:1) , WB=0.63 (F-L:1) , SSI=0.23 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

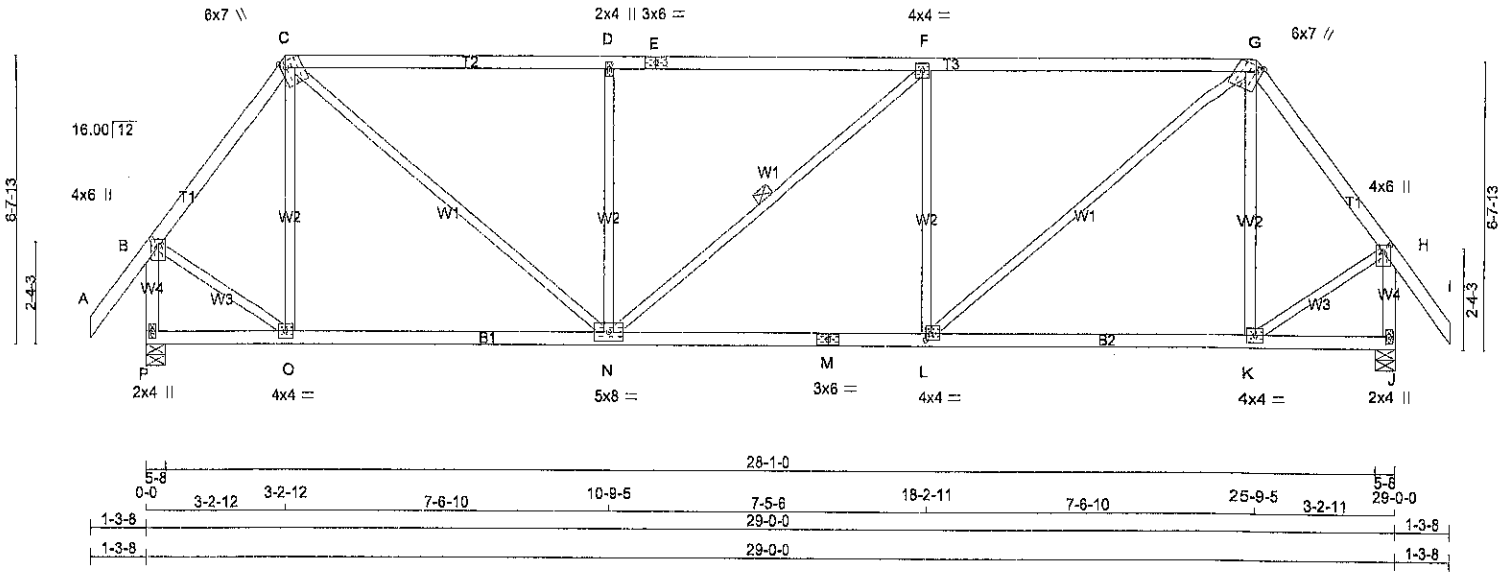
JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.34 (M) (INPUT = 1.00)

A-H073578

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 15:28:30 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-7iVc4X|_aZLTc5lUM_WehYwNb47KDZFg|xyj7dyy7CV
Scale = 1:53.3



TOTAL WEIGHT = 2 X 130 = 260 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1375	0	1375	0	0	5-8	1-8
J	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O- C	-237 / 8	0.17 (1)
B- C	-999 / 0	-70.5	-70.5	0.15 (1)	6.09	C- N	0 / 1094	0.25 (1)
C- D	-1430 / 0	-70.5	-70.5	0.68 (1)	4.60	N- D	-570 / 0	0.42 (1)
D- E	-1431 / 0	-70.5	-70.5	0.68 (1)	4.59	N- F	-1 / 0	0.00 (1)
E- F	-1431 / 0	-70.5	-70.5	0.68 (1)	4.59	L- F	-570 / 0	0.42 (1)
F- G	-1431 / 0	-70.5	-70.5	0.68 (1)	4.58	L- G	0 / 1096	0.25 (1)
G- H	-998 / 0	-70.5	-70.5	0.15 (1)	6.09	K- G	-238 / 8	0.17 (1)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	B- O	0 / 690	0.16 (1)
P- B	-1365 / 0	0.0	0.0	0.17 (1)	6.95	K- H	0 / 690	0.16 (1)
J- H	-1365 / 0	0.0	0.0	0.17 (1)	6.95			
P- O	0 / 0	-17.5	-17.5	0.15 (4)	10.00			
O- N	0 / 591	-17.5	-17.5	0.24 (4)	10.00			
N- M	0 / 1432	-17.5	-17.5	0.34 (1)	10.00			
M- L	0 / 1432	-17.5	-17.5	0.34 (1)	10.00			
L- K	0 / 591	-17.5	-17.5	0.24 (4)	10.00			
K- J	0 / 0	-17.5	-17.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68 (F-G:1) , BC=0.34 (L-N:1) , WB=0.42 (F-L:1) , SSI=0.25 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)

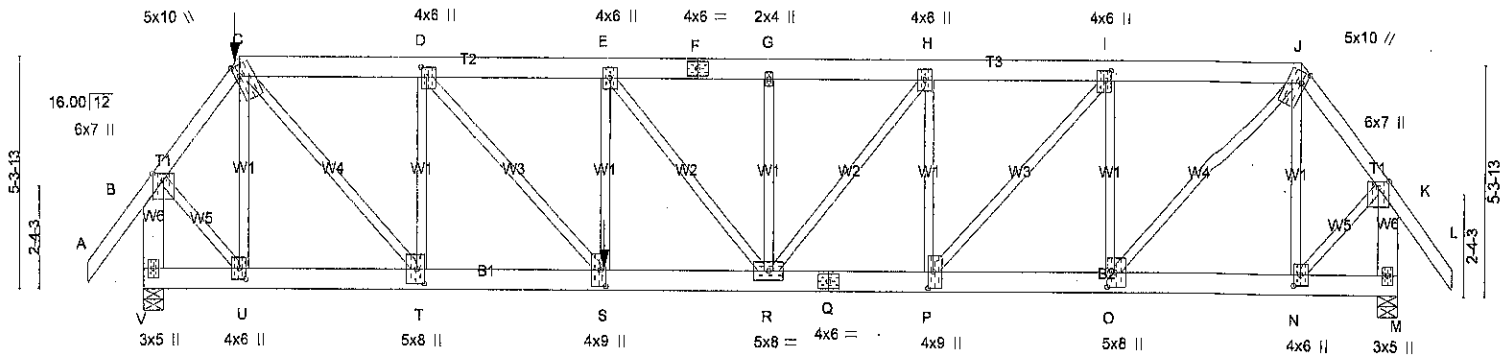
A-10073577

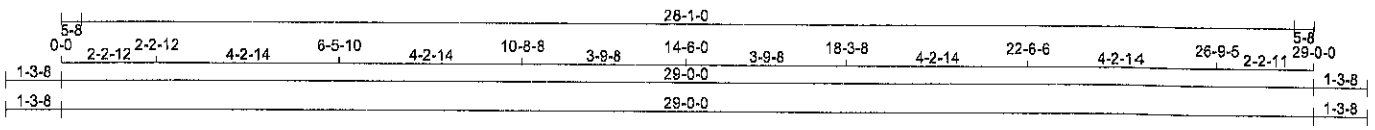
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H01-Cond2	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-fWxEsBiMpFDc_xAipG?P8LOIQgjAU1UWUHD9aAyy7CW

Scale = 1:53.1





TOTAL WEIGHT = 2 X 176 = 351 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	7.0	1.75	3.00
C	TTWW+m	MT20	5.0	10.0	3.00	1.25
D	TMWW+t	MT20	4.0	6.0	2.75	1.50
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	4.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW+t	MT20	4.0	6.0	2.75	1.50
J	TTWW+m	MT20	5.0	10.0	3.00	1.25
K	TMVW+p	MT20	6.0	7.0	1.75	3.00
M	BMV1+p	MT20	3.0	5.0		
N	BMWW+t	MT20	4.0	6.0	3.00	1.75
O	BMWW+t	MT20	5.0	8.0	4.00	2.00
P	BMWW+t	MT20	4.0	9.0	4.50	1.50
Q	BS-t	MT20	4.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	9.0	4.50	1.50
T	BMWW+t	MT20	5.0	8.0	4.00	2.00
U	BMWW+t	MT20	4.0	6.0	3.00	1.75
V	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2803	0	2803	0	0	5-8	3-10
M	1970	0	1970	0	0	5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2042	999 / 0	0 / 0	0 / 0	0 / 0	1044 / 0	0 / 0
M	1426	750 / 0	0 / 0	0 / 0	0 / 0	676 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.11 (1)	10.00
B- C	-2049 / 0	-70.5	-70.5	0.13 (1)	4.61
C- D	-2967 / 0	-138.7	-138.7	0.22 (1)	4.66
D- E	-4010 / 0	-138.7	-138.7	0.26 (1)	4.08
E- F	-3683 / 0	-70.5	-70.5	0.15 (1)	4.35
F- G	-3683 / 0	-70.5	-70.5	0.15 (1)	4.35
G- H	-3683 / 0	-70.5	-70.5	0.15 (1)	4.35
H- I	-3093 / 0	-70.5	-70.5	0.14 (1)	4.66
I- J	-2132 / 0	-70.5	-70.5	0.11 (1)	5.45
J- K	-1405 / 0	-70.5	-70.5	0.09 (1)	5.39
K- L	0 / 40	-70.5	-70.5	0.11 (1)	10.00
V- B	-2806 / 0	0.0	0.0	0.25 (1)	6.20
M- K	-1980 / 0	0.0	0.0	0.18 (1)	7.13
V- U	0 / 0	-34.5	-34.5	0.08 (4)	10.00
U- T	0 / 1210	-34.5	-34.5	0.23 (1)	10.00
T- S	0 / 2967	-34.5	-34.5	0.47 (1)	10.00
S- R	0 / 4010	-17.5	-17.5	0.59 (1)	10.00
R- Q	0 / 3093	-17.5	-17.5	0.44 (1)	10.00
Q- P	0 / 3093	-17.5	-17.5	0.44 (1)	10.00
P- O	0 / 2132	-17.5	-17.5	0.31 (1)	10.00
O- N	0 / 829	-17.5	-17.5	0.18 (1)	10.00
N- M	0 / 0	-17.5	-17.5	0.05 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-814 / 0	0.31 (1)
C- T	0 / 2671	0.66 (1)
T- D	-1861 / 0	0.71 (1)
D- S	0 / 1586	0.48 (4)
S- E	0 / 415	0.16 (4)
E- R	-563 / 0	0.41 (4)
R- G	-257 / 0	0.10 (1)
R- H	0 / 961	0.28 (4)
P- H	-1031 / 0	0.39 (1)
P- I	0 / 1461	0.36 (1)
O- I	-1433 / 0	0.54 (1)
O- J	0 / 1980	0.49 (1)
N- J	-573 / 0	0.22 (1)
B- U	0 / 1576	0.39 (1)
N- K	0 / 1081	0.27 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-115	-115	---	FRONT	VERT	TOTAL
S	10-8-4	-1149	-1149	---	FRONT	VERT	DEAD

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-12
RIGHT SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 10-8-4 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.26 (D-E:1) , BC=0.59 (R-S:1) , WB=0.71 (D-T:1) , SSI=0.23 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

A-150735 76

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H01-Cond2	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.5 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD, AND 1286.6 lbs FACTORED DOWN AT 10-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (!) (INPUT = 0.90)
JSI METAL= 0.83 (Q) (INPUT = 1.00)

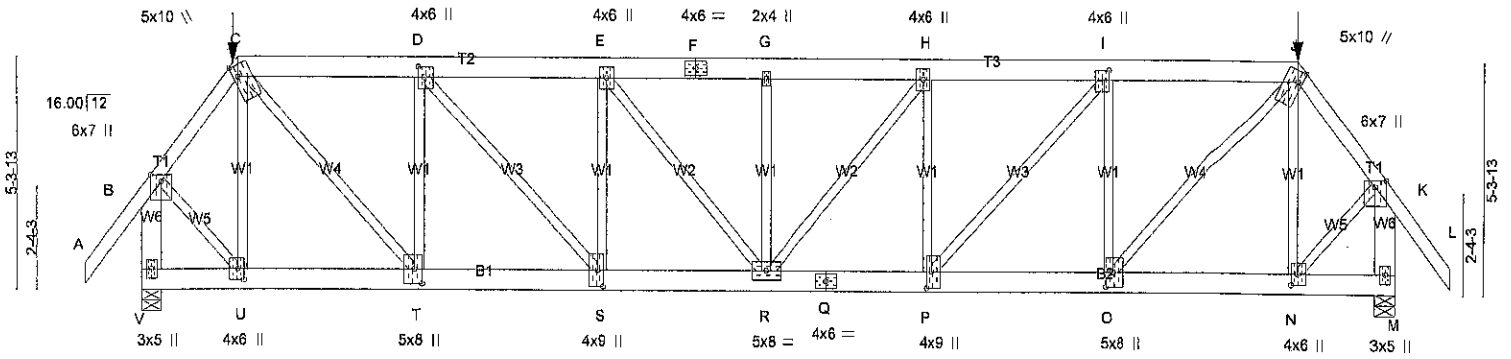
LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

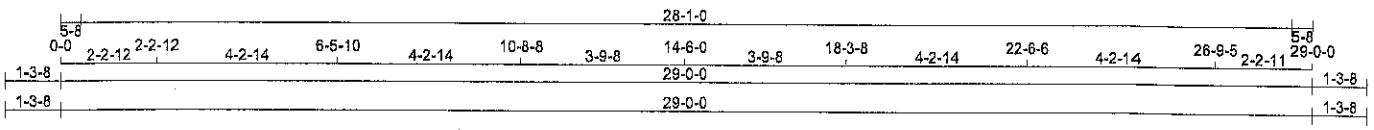
A-1073576(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H01-Cond1	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 MiTek Industries, Inc. Tue Jul 14 15:26:29 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-fWxEsBiMpFDc_xAlpG?P8LOJlgkDU3iWUHD9aAyy7CW
Scale = 1:53.1





TOTAL WEIGHT = 2 X 176 = 351 lb [M]

LUMBER
N. L. G. A. RULES

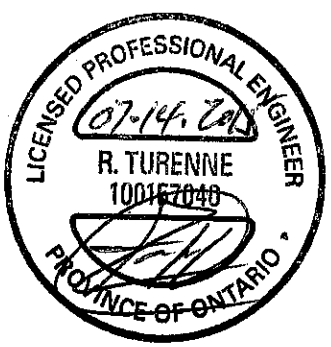
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	7.0	1.75	3.00
C	TTWW+m	MT20	5.0	10.0	3.00	1.25
D	TMWW+t	MT20	4.0	6.0	2.75	1.50
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	4.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW+t	MT20	4.0	6.0	2.75	1.50
J	TTWW+m	MT20	5.0	10.0	3.00	1.25
K	TMVW+p	MT20	6.0	7.0	1.75	3.00
M	BMV1+p	MT20	3.0	5.0		
N	BMWW+t	MT20	4.0	6.0	3.00	1.75
O	BMWW+t	MT20	5.0	8.0	4.00	2.00
P	BMWW+t	MT20	4.0	8.0	4.50	1.50
Q	BS-t	MT20	4.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	8.0	4.50	1.50
T	BMWW+t	MT20	5.0	8.0	4.00	2.00
U	BMWW+t	MT20	4.0	6.0	3.00	1.75
V	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2572	0	2572	0	0	5-8	2-14
M	2572	0	2572	0	0	5-8	2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0
M	1811	1235 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	
B- C	-1868 / 0	-70.5	-70.5	0.12 (1)	4.79	
C- D	-2658 / 0	-138.7	-138.7	0.21 (1)	4.89	
D- E	-3535 / 0	-138.7	-138.7	0.24 (1)	4.32	
E- F	-3776 / 0	-138.7	-138.7	0.20 (1)	4.26	
F- G	-3776 / 0	-138.7	-138.7	0.20 (1)	4.26	
G- H	-3776 / 0	-138.7	-138.7	0.20 (1)	4.26	
H- I	-3535 / 0	-138.7	-138.7	0.24 (1)	4.32	
I- J	-2658 / 0	-138.7	-138.7	0.21 (1)	4.89	
J- K	-1868 / 0	-70.5	-70.5	0.12 (1)	4.79	
K- L	0 / 40	-70.5	-70.5	0.11 (1)	10.00	
V- B	-2575 / 0	0.0	0.0	0.23 (1)	6.42	
M- K	-2574 / 0	0.0	0.0	0.23 (1)	6.42	
V- U	0 / 0	-34.5	-34.5	0.07 (1)	10.00	
U- T	0 / 1104	-34.5	-34.5	0.22 (1)	10.00	
T- S	0 / 2658	-34.5	-34.5	0.40 (1)	10.00	
S- R	0 / 3535	-34.5	-34.5	0.52 (1)	10.00	
R- Q	0 / 3535	-34.5	-34.5	0.52 (1)	10.00	
Q- P	0 / 3535	-34.5	-34.5	0.52 (1)	10.00	
P- O	0 / 2658	-34.5	-34.5	0.40 (1)	10.00	
O- N	0 / 1104	-34.5	-34.5	0.22 (1)	10.00	
N- M	0 / 0	-34.5	-34.5	0.07 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-725 / 0	0.28 (1)
C- T	0 / 2362	0.58 (1)
T- D	-1655 / 0	0.63 (1)
D- S	0 / 1333	0.33 (1)
S- E	-856 / 0	0.33 (1)
E- R	0 / 391	0.10 (1)
R- G	-506 / 0	0.19 (1)
R- H	0 / 391	0.10 (1)
P- H	-858 / 0	0.33 (1)
P- I	0 / 1333	0.33 (1)
O- I	-1655 / 0	0.63 (1)
O- J	0 / 2362	0.58 (1)
N- J	-725 / 0	0.28 (1)
B- U	0 / 1437	0.36 (1)
N- K	0 / 1437	0.36 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-115	-115	---	FRONT	VERT	TOTAL
J	26-9-5	-114	-114	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-12
RIGHT SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.24 (H-I:1) , BC=0.52 (R-S:1) , WB=0.63 (I-O:1) , SSI=0.23 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (U) (INPUT = 0.80)
JSI METAL= 0.76 (Q) (INPUT = 1.00)

A-4073575

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253428	H01-Cond1	2	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 26-9-5, AND 114.5 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LICENSED PROFESSIONAL ENGINEER
07-14-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

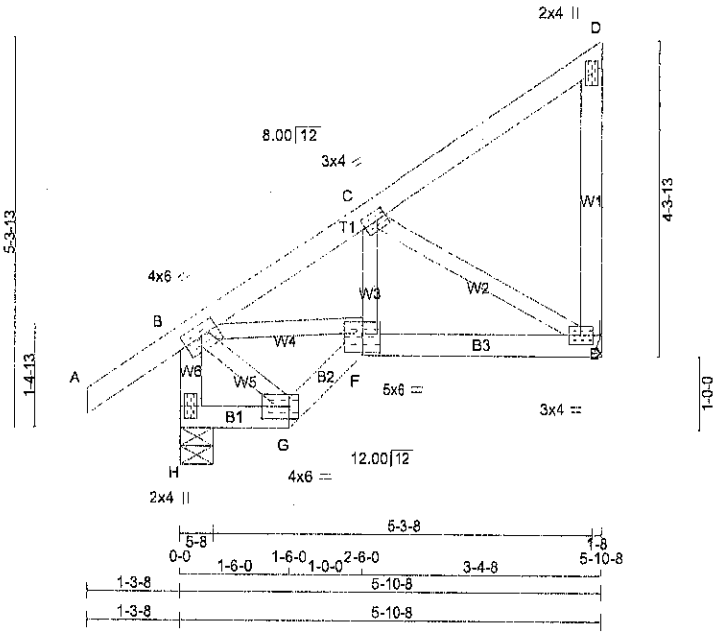
A-1073575(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	J01C	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-W6xnVD3hhVShzxZK?b0oaho1O_JoLC3LHxyJQZyy7mC

Scale: 3/8"=1'



TOTAL WEIGHT = 7 X 30 = 208 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
H- B	-342 / 0	0.0	0.0	0.04 (1)	7.81	B- G	0 / 18	0.01 (4)
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B- F	0 / 238	0.05 (1)
B- C	-287 / 0	-70.5	-70.5	0.10 (1)	6.25	F- C	0 / 57	0.02 (4)
C- D	-12 / 0	-70.5	-70.5	0.10 (1)	6.25	C- E	-284 / 0	0.07 (1)
E- D	-93 / 0	0.0	0.0	0.03 (1)	7.81			
H- G	0 / 0	-17.5	-17.5	0.01 (4)	10.00			
G- F	0 / 20	-17.5	-17.5	0.01 (4)	10.00			
F- E	0 / 248	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.08 (E-F:4) , WB=0.07 (C-E:1) , SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

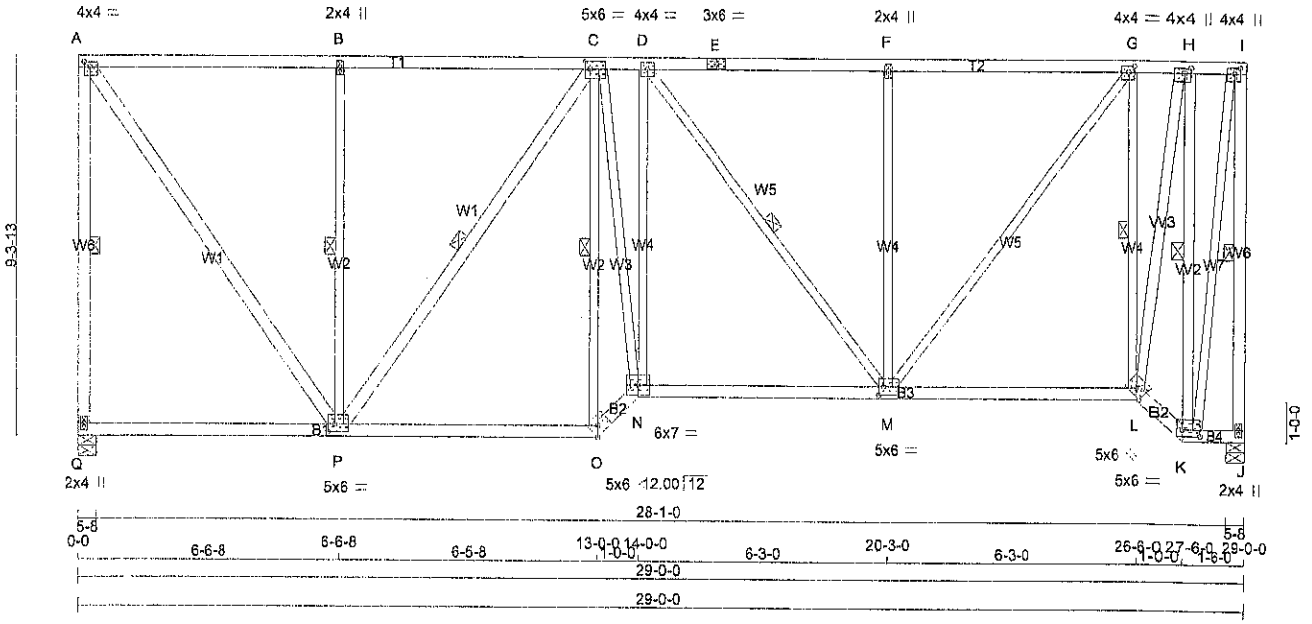
A-1073574

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:50:25 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-W6xnVD3hhVShzxZK?b0oahowv_GRL14LHxyJQZyy7mC

Scale = 1:56.9



TOTAL WEIGHT = 188 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - P	2x4	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	1.50	1.75
B	TMW+w	MT20	2.0	4.0		
C	TMWWW-t	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0	1.75	1.75
H	TMWW+t	MT20	4.0	4.0	1.50	1.50
I	TMWW+p	MT20	4.0	4.0	1.75	1.75
J	BMV1+p	MT20	2.0	4.0		
K	BBWW-I	MT20	5.0	6.0	2.00	4.50
L	BBWW-h	MT20	5.0	6.0	2.00	3.00
M	BMWWW-t	MT20	5.0	6.0	2.50	1.50
N	BBWW-I	MT20	6.0	7.0		
O	BBW-h	MT20	5.0	6.0	Edge	2.50
P	BMWWW-t	MT20	5.0	6.0	2.25	2.00
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1275	0	1275	0	0	5-8	1-8
J	1275	0	1275	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	899	609 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0
J	899	609 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.56FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF A-Q, I-J, C-O, G-L, H-K, B-P, C-P, D-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
Q- A	-1231 / 0	0.0	0.0	0.50 (1)	5.79
A- B	-760 / 0	-70.5	-70.5	0.47 (1)	6.16
B- C	-760 / 0	-70.5	-70.5	0.47 (1)	6.16
C- D	-1170 / 0	-70.5	-70.5	0.26 (1)	5.66
D- E	-1013 / 0	-70.5	-70.5	0.41 (1)	5.66
E- F	-1013 / 0	-70.5	-70.5	0.41 (1)	5.66
F- G	-1013 / 0	-70.5	-70.5	0.41 (1)	5.66
G- H	-376 / 0	-70.5	-70.5	0.16 (1)	6.25
H- I	-198 / 0	-70.5	-70.5	0.05 (1)	6.25
J- I	-1262 / 0	0.0	0.0	0.51 (1)	5.73
Q- P	0 / 0	-17.5	-17.5	0.21 (4)	10.00
P- O	0 / 1034	-17.5	-17.5	0.29 (4)	10.00
O- N	0 / 1447	-17.5	-17.5	0.24 (1)	10.00
N- M	0 / 1170	-17.5	-17.5	0.29 (1)	10.00
M- L	0 / 392	-17.5	-17.5	0.22 (4)	10.00
L- K	0 / 258	-17.5	-17.5	0.04 (1)	10.00
K- J	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-966 / 0	0.55 (1)
C- N	0 / 1077	0.24 (1)
N- D	0 / 44	0.02 (4)
L- G	-1168 / 0	0.51 (1)
L- H	0 / 1417	0.32 (1)
K- H	-1349 / 0	0.77 (1)
K- I	0 / 1205	0.27 (1)
A- P	0 / 1295	0.21 (1)
P- B	-524 / 0	0.30 (1)
P- C	-477 / 0	0.32 (1)
D- M	-258 / 0	0.19 (1)
M- F	-460 / 0	0.66 (1)
M- G	0 / 1022	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.51 (I-J:1) , BC=0.29 (M-N:1) , WB=0.77 (H-K:1) , SSI=0.22 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.36 (L) (INPUT = 1.00)

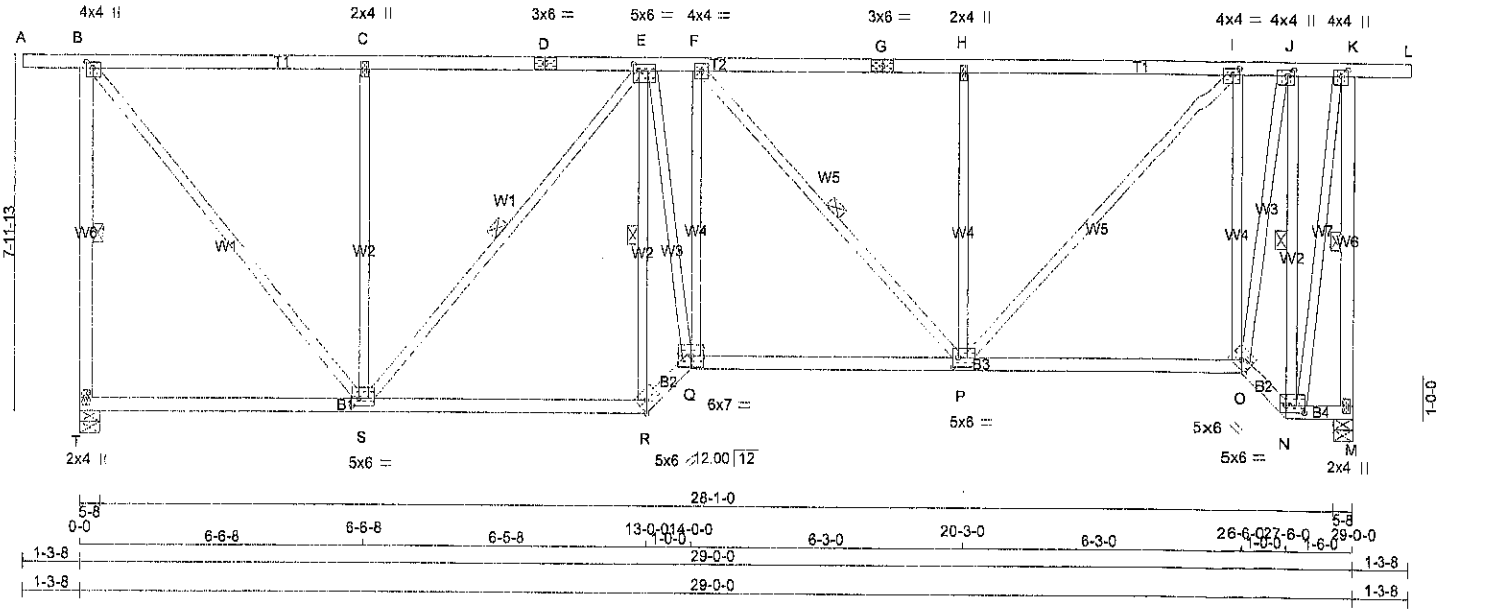
LICENSED PROFESSIONAL ENGINEER
@7.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-150735 73

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:50:24 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-2wNPIt23xBKqMn_8 StVZ1UFmjavecXKB2HClu7yy7mD
Scale = 1:52.2



TOTAL WEIGHT = 168 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	1.75
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	6.0	1.75	1.50
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	4.0	1.75	1.75
J	TMWW+t	MT20	4.0	4.0	1.50	1.50
K	TMVW+p	MT20	4.0	4.0	1.75	1.75
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-t	MT20	5.0	6.0	2.00	4.50
O	BBWW-h	MT20	5.0	6.0	2.00	3.00
P	BMWWW-t	MT20	5.0	6.0	2.50	1.50
Q	BBWW-t	MT20	6.0	7.0		
R	BBW-h	MT20	5.0	6.0	Edge	2.50
S	BMWWW-t	MT20	5.0	6.0	2.00	2.00
T	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1364	0	1364	0	0	5-8	1-8
M	1368	0	1368	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	959	662 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	962	664 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF B-T, K-M, E-R, F-P, J-N, E-S.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-1319 / 0	0.0	0.0	0.38 (1)	5.63		R- E	-1136 / 0	0.46 (1)
A- B	0 / 0	-70.5	-70.5	0.08 (1)	10.00		E- Q	0 / 1258	0.28 (1)
B- C	-889 / 0	-70.5	-70.5	0.48 (1)	5.81		Q- F	0 / 44	0.02 (4)
C- D	-889 / 0	-70.5	-70.5	0.48 (1)	5.81		F- P	-284 / 0	0.16 (1)
D- E	-889 / 0	-70.5	-70.5	0.48 (1)	5.81		P- H	-477 / 0	0.40 (1)
E- F	-1400 / 0	-70.5	-70.5	0.26 (1)	5.19		P- I	0 / 1101	0.25 (1)
F- G	-1208 / 0	-70.5	-70.5	0.42 (1)	5.30		O- I	-1191 / 0	0.99 (1)
G- H	-1208 / 0	-70.5	-70.5	0.42 (1)	5.30		O- J	0 / 1469	0.33 (1)
H- I	-1208 / 0	-70.5	-70.5	0.42 (1)	5.30		N- J	-1321 / 0	0.53 (1)
I- J	-445 / 0	-70.5	-70.5	0.17 (1)	6.25		N- K	0 / 1180	0.26 (1)
J- K	-223 / 0	-70.5	-70.5	0.08 (1)	6.25		B- S	0 / 1373	0.31 (1)
K- L	0 / 0	-70.5	-70.5	0.08 (1)	10.00		S- C	-525 / 0	0.64 (1)
M- K	-1355 / 0	0.0	0.0	0.39 (1)	5.56		S- E	-504 / 0	0.36 (1)
T- S	0 / 0	-17.5	-17.5	0.21 (4)	10.00				
S- R	0 / 1210	-17.5	-17.5	0.30 (1)	10.00				
R- Q	0 / 1690	-17.5	-17.5	0.27 (1)	10.00				
Q- P	0 / 1400	-17.5	-17.5	0.33 (1)	10.00				
P- O	0 / 464	-17.5	-17.5	0.23 (4)	10.00				
O- N	0 / 290	-17.5	-17.5	0.05 (1)	10.00				
N- M	0 / 0	-17.5	-17.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.48 (B-C:1) , BC=0.33 (P-Q:1) ,
WB=0.99 (I-O:1) , SSI=0.22 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL=0.37 (0) (INPUT = 1.00)

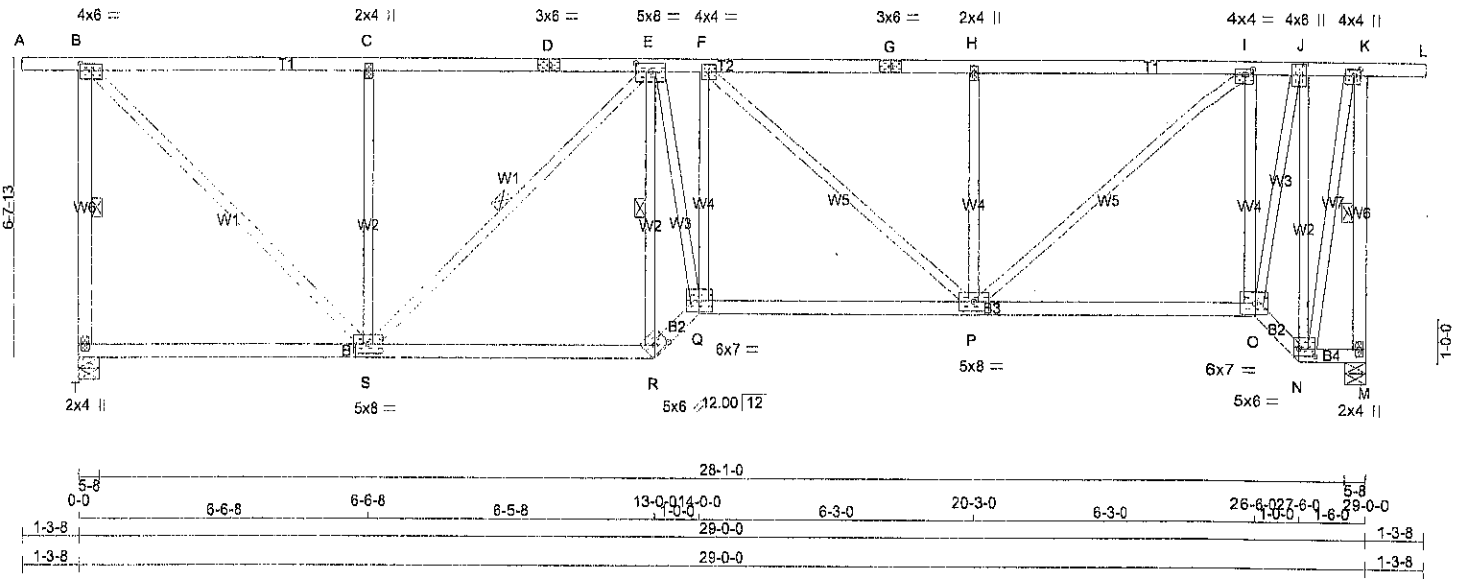
LICENSED PROFESSIONAL ENGINEER
07/14/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-150735 72

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:50:23 2015 Page 1
ID:97W_oc?jcYybVbeU0Ab4z?zOVES-ajp14X1RAtCzkePyuA_KVGibjAZZt452qdTCMgyy7mE
Scale = 1:51.6



TOTAL WEIGHT = 152 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	8.0	2.25	4.00
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	4.0	1.50	1.75
J	TMWW+t	MT20	4.0	6.0	3.00	1.75
K	TMVW+p	MT20	4.0	4.0	1.75	1.75
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-l	MT20	5.0	6.0	2.00	4.50
O	BBWW-l	MT20	6.0	7.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BBWW-l	MT20	6.0	7.0		
R	BBW-h	MT20	5.0	6.0	2.00	3.25
S	BMWWW-t	MT20	5.0	8.0		
T	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1364	0	1364	0	0	5-8	1-8
M	1368	0	1368	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	959	662 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	962	664 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF B-T, K-M, E-R, E-S.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-1319 / 0	0.0	0.0	0.27 (1)	5.63	R- E	-1382 / 0	0.39 (1)
A- B	0 / 0	-70.5	-70.5	0.08 (1)	10.00	E- Q	0 / 1523	0.34 (1)
B- C	-1076 / 0	-70.5	-70.5	0.49 (1)	5.40	Q- F	0 / 43	0.02 (4)
C- D	-1076 / 0	-70.5	-70.5	0.49 (1)	5.40	O- I	-1208 / 0	0.56 (1)
D- E	-1076 / 0	-70.5	-70.5	0.49 (1)	5.40	O- J	0 / 1540	0.35 (1)
E- F	-1751 / 0	-70.5	-70.5	0.27 (1)	4.74	N- J	-1357 / 0	0.99 (1)
F- G	-1507 / 0	-70.5	-70.5	0.44 (1)	4.85	N- K	0 / 1164	0.26 (1)
G- H	-1507 / 0	-70.5	-70.5	0.44 (1)	4.85	B- S	0 / 1500	0.34 (1)
H- I	-1507 / 0	-70.5	-70.5	0.44 (1)	4.85	S- C	-526 / 0	0.38 (1)
I- J	-558 / 0	-70.5	-70.5	0.19 (1)	6.25	S- E	-550 / 0	0.31 (1)
J- K	-268 / 0	-70.5	-70.5	0.08 (1)	6.25	F- P	-323 / 0	0.46 (1)
K- L	0 / 0	-70.5	-70.5	0.08 (1)	10.00	P- H	-473 / 0	0.23 (1)
M- K	-1355 / 0	0.0	0.0	0.28 (1)	5.58	P- I	0 / 1228	0.28 (1)
T- S	0 / 0	-17.5	-17.5	0.21 (4)	10.00			
S- R	0 / 1464	-17.5	-17.5	0.34 (1)	10.00			
R- Q	0 / 2039	-17.5	-17.5	0.33 (1)	10.00			
Q- P	0 / 1750	-17.5	-17.5	0.38 (1)	10.00			
P- O	0 / 582	-17.5	-17.5	0.24 (4)	10.00			
O- N	0 / 348	-17.5	-17.5	0.06 (1)	10.00			
N- M	0 / 0	-17.5	-17.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.49 (B-C:1) , BC=0.38 (P-Q:1) , WB=0.99 (J-N:1) , SSI=0.22 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)

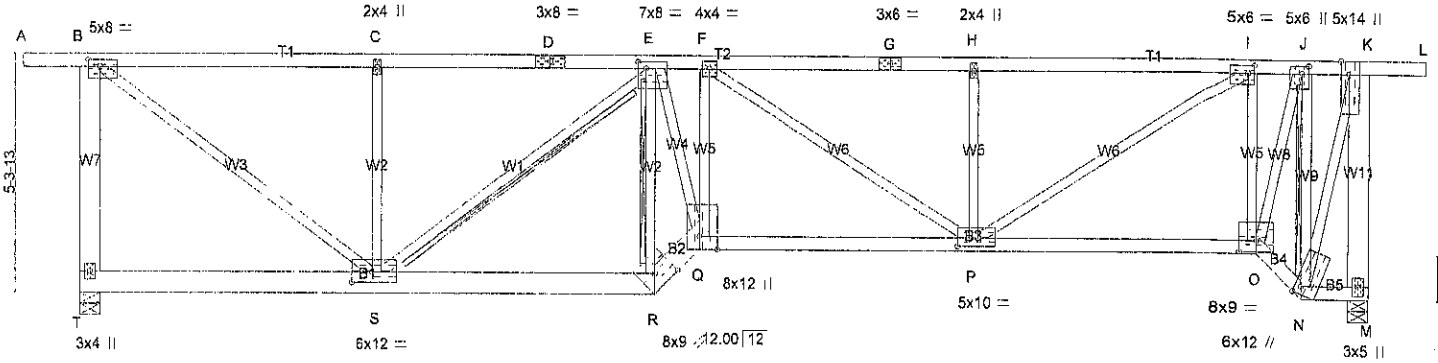
A-N073571

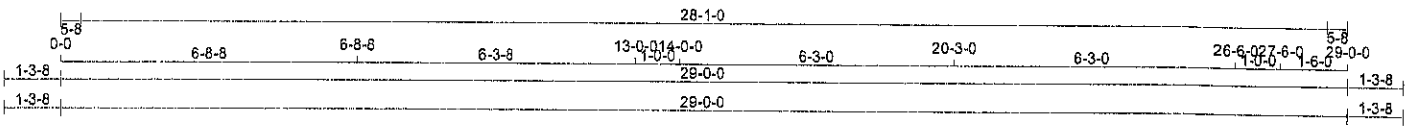
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-6XFftB1oPa466UrlKST5y3ALnn6A8fKubzjfpEyy7mF

Scale = 1:51.6





TOTAL WEIGHT = 151 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	2100F 1.8E	SPF
D - G	2x4	DRY	2100F 1.8E	SPF
G - L	2x4	DRY	2100F 1.8E	SPF
M - K	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - Q	2x6	DRY	No.2	SPF
Q - O	2x4	DRY	2100F 1.8E	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	8.0	1.75	3.25
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	7.0	8.0	1.75	2.25
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	5.0	6.0	2.00	1.75
J	TMWW+t	MT20	5.0	6.0	2.00	1.75
K	TMW+p	MT20	5.0	14.0	Edge	2.25
M	BMV1+p	MT20	3.0	5.0		
N	BBWW+m	MT20	6.0	12.0	Edge	1.75
O	BBWW-l	MT20	8.0	9.0	3.00	4.50
P	BMWWW-t	MT20	5.0	10.0	2.25	3.00
Q	BBWW+p	MT20	8.0	12.0	Edge	4.75
R	BBW-h	MT20	8.0	9.0	3.75	5.00
S	BMWWW-t	MT20	6.0	12.0	2.75	5.50
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2804	0	2804	0	0	5-8	2-15
M	2814	0	2814	0	0	5-8	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1832	1254 / 0	0 / 0	0 / 0	0 / 0	579 / 0	0 / 0
M	1839	1259 / 0	0 / 0	0 / 0	0 / 0	580 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-R, J-N, E-S
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
T-B	-2500 / 0	0.0	0.0	0.73 (1)	6.50	R-E	-3371 / 0	0.76 (1)
A-B	0 / 0	-70.5	-70.5	0.06 (1)	10.00	E-Q	0 / 3626	0.90 (1)
B-C	-2763 / 0	-138.7	-138.7	0.79 (1)	3.97	Q-F	0 / 352	0.09 (1)
C-D	-2763 / 0	-138.7	-138.7	0.77 (1)	3.97	O-I	-2106 / 0	0.61 (1)
D-E	-2763 / 0	-138.7	-138.7	0.77 (1)	3.97	O-J	0 / 3112	0.77 (1)
E-F	-4644 / 0	-138.7	-138.7	0.40 (1)	3.64	N-J	-2851 / 0	0.67 (1)
F-G	-3987 / 0	-70.5	-70.5	0.43 (1)	3.99	N-K	0 / 2371	0.59 (1)
G-H	-3987 / 0	-70.5	-70.5	0.43 (1)	3.99	B-S	0 / 3433	0.85 (1)
H-I	-3987 / 0	-70.5	-70.5	0.43 (1)	3.99	S-C	-1040 / 0	0.42 (1)
I-J	-1451 / 0	-138.7	-138.7	0.13 (1)	6.25	S-E	-1246 / 0	0.69 (1)
J-K	-682 / 0	-138.7	-138.7	0.12 (1)	6.25	F-P	-774 / 0	0.83 (1)
K-L	0 / 0	-70.5	-70.5	0.06 (1)	10.00	P-H	-486 / 0	0.14 (1)
M-K	-2588 / 0	0.0	0.0	0.83 (1)	6.42	P-I	0 / 2966	0.73 (1)
T-S	0 / 0	-34.5	-34.5	0.19 (4)	10.00			
S-R	0 / 3737	-34.5	-34.5	0.58 (1)	10.00			
R-Q	0 / 5077	-34.5	-34.5	0.66 (1)	10.00			
Q-P	0 / 4635	-102.7	-102.7	0.84 (1)	10.00			
P-O	0 / 1506	-102.7	-102.7	0.58 (1)	10.00			
O-N	0 / 880	-34.5	-34.5	0.16 (1)	10.00			
N-M	0 / 0	-34.5	-34.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 14-0-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 14-0-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 26-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 588 (0.59")

CSI: TC=0.83 (K-M:1) , BC=0.84 (P-Q:1) , WB=0.90 (E-Q:1) , SS=0.48 (B-C:1)

A-40135-10

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253427	H01T	1	1	TRUSS DESC.	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.69 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.14.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073570(2)

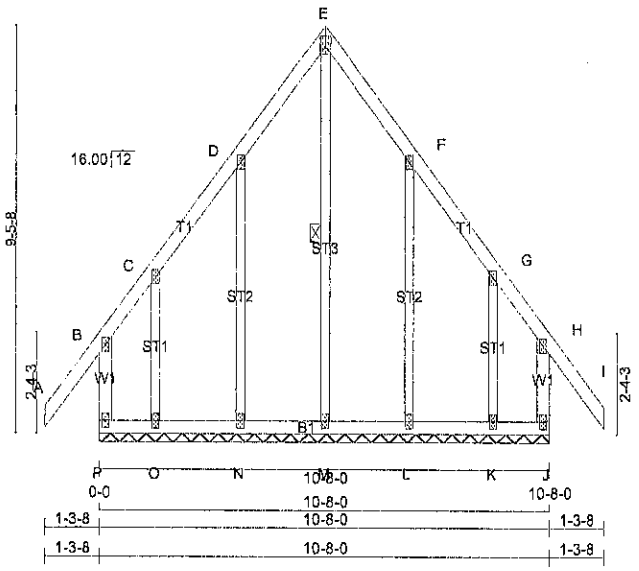
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H10G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:40:52 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-otpAFH7GeRG_kuGCxIEQssZ6y?mib3Q?WleUpGyy7v9

3x5 II

Scale = 1:53.8



TOTAL WEIGHT = 65 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	3.0	5.0	2.00	Edge
H	TMV+p	MT20	2.0	4.0		
J	BMV1+p	MT20	2.0	4.0		
K, L, M, N, O						
K	BMW1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-161 / 0	0.0	0.0	0.03 (1)	7.81	M- E	-181 / 0		0.10 (1)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	N- D	-154 / 0		0.12 (1)
B- C	-9 / 0	-70.5	-70.5	0.07 (1)	10.00	O- C	-70 / 0		0.02 (1)
C- D	0 / 27	-70.5	-70.5	0.04 (1)	10.00	L- F	-154 / 0		0.12 (1)
D- E	0 / 21	-70.5	-70.5	0.04 (1)	10.00	K- G	-70 / 0		0.02 (1)
E- F	0 / 21	-70.5	-70.5	0.04 (1)	10.00				
F- G	0 / 27	-70.5	-70.5	0.04 (1)	10.00				
G- H	-9 / 0	-70.5	-70.5	0.07 (1)	10.00				
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00				
J- H	-161 / 0	0.0	0.0	0.03 (1)	7.81				
P- O	-9 / 0	-17.5	-17.5	0.01 (4)	10.00				
O- N	-12 / 0	-17.5	-17.5	0.01 (4)	6.25				
N- M	-14 / 0	-17.5	-17.5	0.01 (4)	6.25				
M- L	-14 / 0	-17.5	-17.5	0.01 (4)	6.25				
L- K	-12 / 0	-17.5	-17.5	0.01 (4)	6.25				
K- J	-9 / 0	-17.5	-17.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1) , BC=0.01 (L-M:4) , WB=0.12 (F-L:1) , SSI=0.05 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-14-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1007 3569

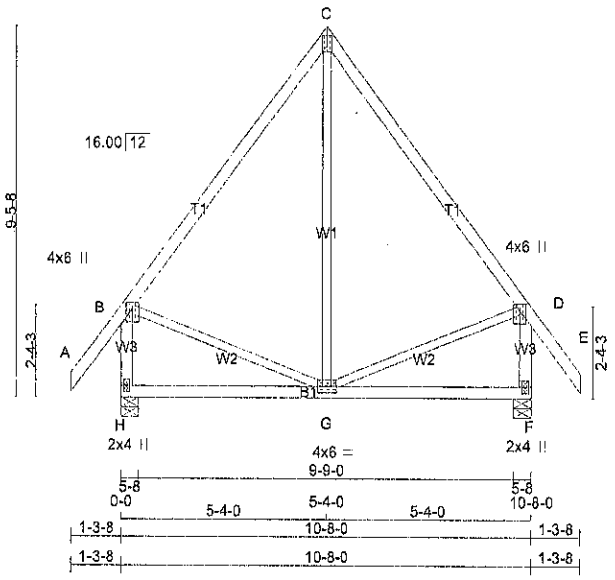
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H10	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:97W_oc?jcYybVbeU0Ab4z?zOVES-KhFo1x6et8876kh0N1jBJe0umcOTsdSrlevxHqyy7vA

3x5 II

Scale = 1:59.0



TOTAL WEIGHT = 5 X 58 = 292 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	568	0	568	0	0	5-8	1-8
F	568	0	568	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	398	283 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	398	283 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)		10.00
B- C	-241 / 0	-70.5	-70.5	0.26 (1)		6.25
C- D	-241 / 0	-70.5	-70.5	0.26 (1)		6.25
D- E	0 / 40	-70.5	-70.5	0.10 (1)		10.00
H- B	-533 / 0	0.0	0.0	0.07 (1)		7.81
F- D	-533 / 0	0.0	0.0	0.07 (1)		7.81
H- G	0 / 0	-17.5	-17.5	0.14 (4)		10.00
G- F	0 / 0	-17.5	-17.5	0.14 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 92	0.03 (4)
B- G	0 / 154	0.03 (1)
G- D	0 / 154	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.26 (B-C:1) , BC=0.14 (G-H:4) , WB=0.03 (B-G:1) , SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (D) (INPUT = 0.80)
JSI METAL= 0.10 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-14-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

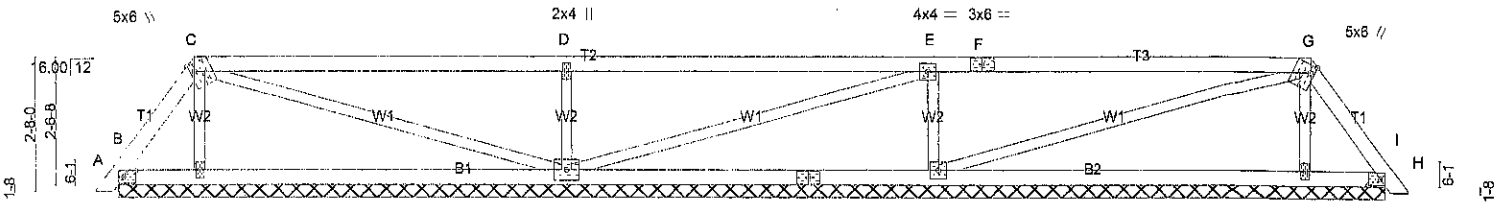
A-1073568

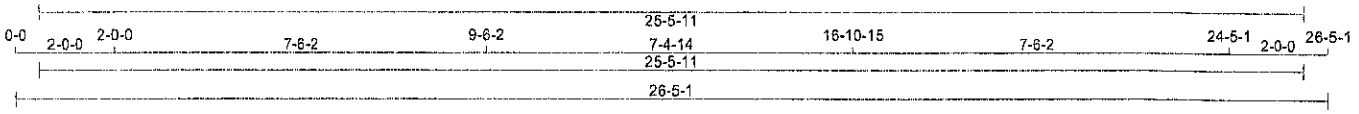
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H07	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:40:50 2015 Page 1
ID:8?W_oc?jcYybVbeU0Ab4z?zOVES-sUhPqb506q0GVa6qqKCymRTfhC1P79JI3_9NIOyy7vB

Scale = 1:46.0





TOTAL WEIGHT = 3 X 87 = 260 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.00
H	TMB1-I	MT20	3.0	4.0	1.50	2.75
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW1-t	MT20	5.0	6.0		
N	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	114	0	114	0	-12	25-5-11 (11-6#8)	
N	313	0	313	0	0	25-5-11 (11-6#8)	
M	728	0	728	0	0	25-5-11 (11-6#8)	
K	702	0	702	0	0	25-5-11 (11-6#8)	
J	303	0	303	0	0	25-5-11 (11-6#8)	
H	130	0	130	0	-10	25-5-11 (11-6#8)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	74	83 / 0	0 / 0	0 / 0	0 / 0	0 / -9	0 / 0
N	227	117 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
M	512	356 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0
K	494	340 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0
J	220	111 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
H	86	93 / 0	0 / 0	0 / 0	0 / 0	0 / -7	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX (LC)	MAX UNBRAC LENGTH
A-B	0 / 9	-70.5	-70.5	1	0.01	(1)	10.00
B-C	-74 / 0	-70.5	-70.5	1	0.03	(1)	6.25
C-D	-3 / 0	-70.5	-70.5	1	0.53	(1)	10.00
D-E	-3 / 0	-70.5	-70.5	1	0.53	(1)	10.00
E-F	-37 / 0	-70.5	-70.5	1	0.54	(1)	6.25
F-G	-37 / 0	-70.5	-70.5	1	0.54	(1)	6.25
G-H	-95 / 0	-70.5	-70.5	1	0.03	(1)	6.25
H-I	0 / 9	-70.5	-70.5	1	0.01	(1)	10.00
B-N	0 / 42	-17.5	-17.5	1	0.15	(4)	10.00
N-M	0 / 33	-17.5	-17.5	1	0.20	(4)	10.00
M-L	0 / 36	-17.5	-17.5	1	0.20	(4)	10.00
L-K	0 / 36	-17.5	-17.5	1	0.20	(4)	10.00
K-J	0 / 45	-17.5	-17.5	1	0.20	(4)	10.00
J-H	0 / 54	-17.5	-17.5	1	0.15	(4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-197 / 0	0.03 (1)
C-M	-31 / 0	0.04 (1)
M-D	-578 / 0	0.09 (1)
M-E	-35 / 0	0.04 (1)
K-E	-567 / 0	0.09 (1)
K-G	-10 / 0	0.01 (1)
J-G	-186 / 0	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.54 (E-G:1) , BC=0.20 (J-K:4) , WB=0.09 (D-M:1) , SSI=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.26 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-14-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

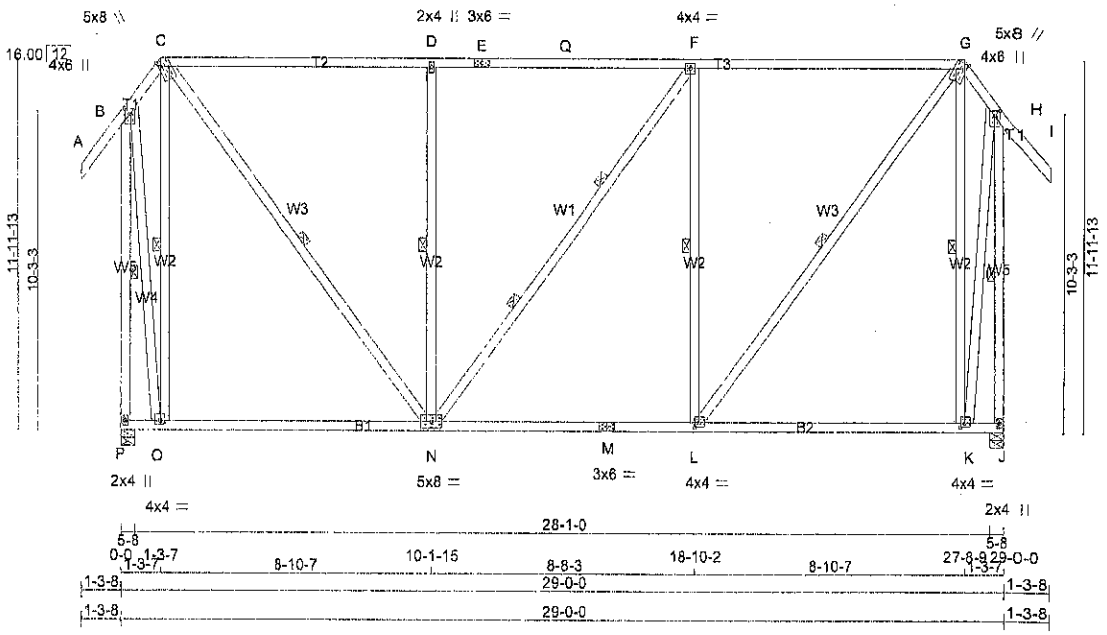
A-1073567

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H06	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-OI71cG5NLWuPtQXdGchjEDxQKofhOXGYqKQqCyyy7vC

Scale = 1:74.6



TOTAL WEIGHT = 7 X 220 = 1542 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - G	2x4	DRY	2100F 1.8E	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	5.0	8.0	Edge	1.00
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	8.0	Edge	1.00
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.50	1.75
L	BMWW-t	MT20	4.0	4.0	1.75	1.75
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	4.0	4.0	1.50	1.75
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1474	0	1474	0	0	5-8	1-10
J	1474	0	1474	0	0	5-8	1-10

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1045	668 / 0	0 / 0	0 / 0	0 / 0	378 / 0	0 / 0
J	1045	668 / 0	0 / 0	0 / 0	0 / 0	378 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-O, G-K, B-P, H-J, C-N, D-N, F-L, G-L.
2 LATERAL BRACE(S) REQUIRED AT 1/ 3 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O- C	-998 / 0	0.78 (1)
B- C	-270 / 0	-70.5	-70.5	0.07 (1)	6.25	K- G	-999 / 0	0.78 (1)
C- D	-835 / 0	-78.0	-78.0	0.49 (1)	6.25	B- O	0 / 1151	0.18 (1)
D- E	-836 / 0	-78.0	-78.0	0.43 (1)	6.25	K- H	0 / 1149	0.18 (1)
E- Q	-836 / 0	-78.0	-78.0	0.43 (1)	6.25	C- N	0 / 1140	0.18 (1)
Q- F	-836 / 0	-78.0	-78.0	0.43 (1)	6.25	N- D	-746 / 0	0.59 (1)
F- G	-836 / 0	-78.0	-78.0	0.49 (1)	6.25	N- F	-1 / 0	0.00 (1)
G- H	-270 / 0	-70.5	-70.5	0.07 (1)	6.25	L- F	-747 / 0	0.59 (1)
H- I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	L- G	0 / 1141	0.18 (1)
P- B	-1525 / 0	0.0	0.0	0.83 (1)	5.32			
J- H	-1523 / 0	0.0	0.0	0.83 (1)	5.33			
P- O	0 / 0	-17.5	-17.5	0.22 (4)	10.00			
O- N	0 / 145	-17.5	-17.5	0.29 (4)	10.00			
N- M	0 / 836	-17.5	-17.5	0.35 (4)	10.00			
M- L	0 / 836	-17.5	-17.5	0.35 (4)	10.00			
L- K	0 / 145	-17.5	-17.5	0.29 (4)	10.00			
K- J	0 / 0	-17.5	-17.5	0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.83 (B-P:1) , BC=0.35 (L-N:4) , WB=0.78 (G-K:1) , SSI=0.32 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.29 (O) (INPUT = 1.00)

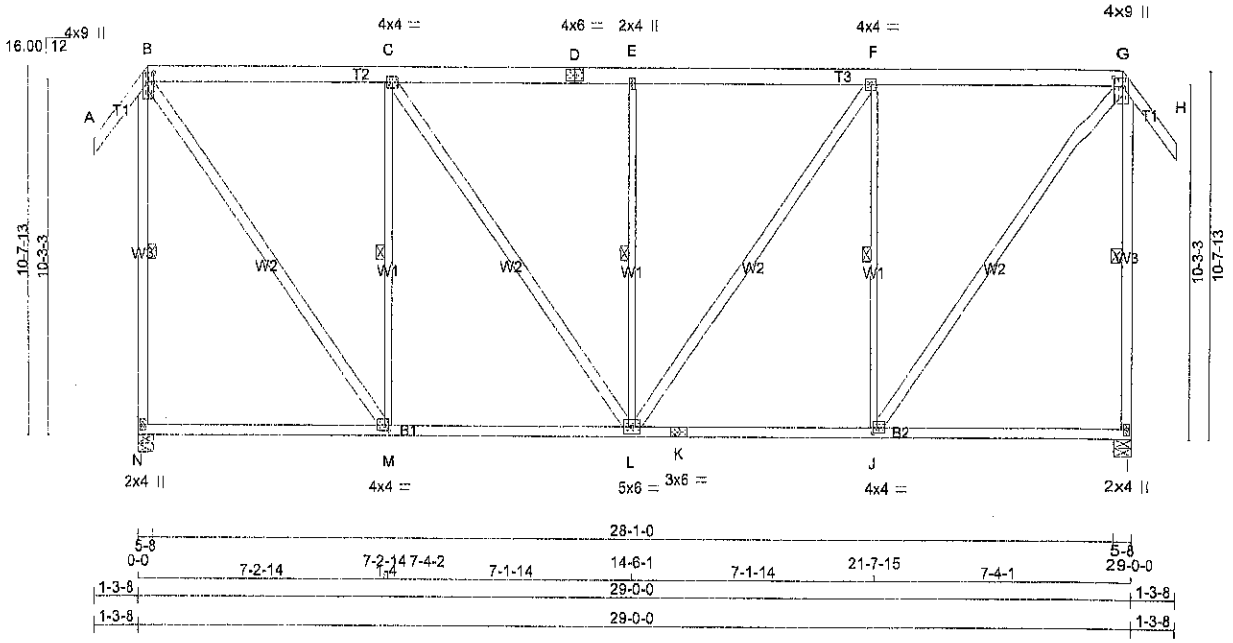
A-1073566

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:40:48 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-w6afPw4laDmZFGyRivAUh0OGHOLUf6xPbggGgVyy7vD

Scale = 1:66.9



TOTAL WEIGHT = 2 X 193 = 386 lb

[M]

LUMBER

N. L. G, A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
L - E	2x3	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TTVW+w	MT20	4.0	9.0	3.00	2.25
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTVW+w	MT20	4.0	9.0	3.00	2.25
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0	1.75	1.50
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0	1.75	1.50
N	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1375	0	1375	0	0	5-8	1-8
I	1375	0	1375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
I	966	668 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF B-N, G-I, C-M, E-L, F-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 48	-70.5	-70.5	0.15 (1)	10.00		B-M	0 / 1221	0.20 (1)
B-C	-729 / 0	-70.5	-70.5	0.25 (1)	6.25		M-C	-860 / 0	0.68 (1)
C-D	-932 / 0	-70.5	-70.5	0.25 (1)	6.25		C-L	0 / 356	0.06 (1)
D-E	-932 / 0	-70.5	-70.5	0.25 (1)	6.25		L-E	-472 / 0	0.36 (1)
E-F	-932 / 0	-70.5	-70.5	0.25 (1)	6.25		L-F	0 / 357	0.06 (1)
F-G	-728 / 0	-70.5	-70.5	0.25 (1)	6.25		J-F	-860 / 0	0.66 (1)
G-H	0 / 48	-70.5	-70.5	0.15 (1)	10.00		J-G	0 / 1221	0.20 (1)
N-B	-1324 / 0	0.0	0.0	0.72 (1)	5.63				
I-G	-1324 / 0	0.0	0.0	0.72 (1)	5.63				
N-M	0 / 38	-17.5	-17.5	0.23 (4)	10.00				
M-L	0 / 729	-17.5	-17.5	0.29 (4)	10.00				
L-K	0 / 728	-17.5	-17.5	0.29 (4)	10.00				
K-J	0 / 728	-17.5	-17.5	0.29 (4)	10.00				
J-I	0 / 38	-17.5	-17.5	0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.72 (G-I:1) , BC=0.29 (L-M:4) , WB=0.66 (F-J:1) , SSI=0.18 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	364	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.26 (M) (INPUT = 1.00)

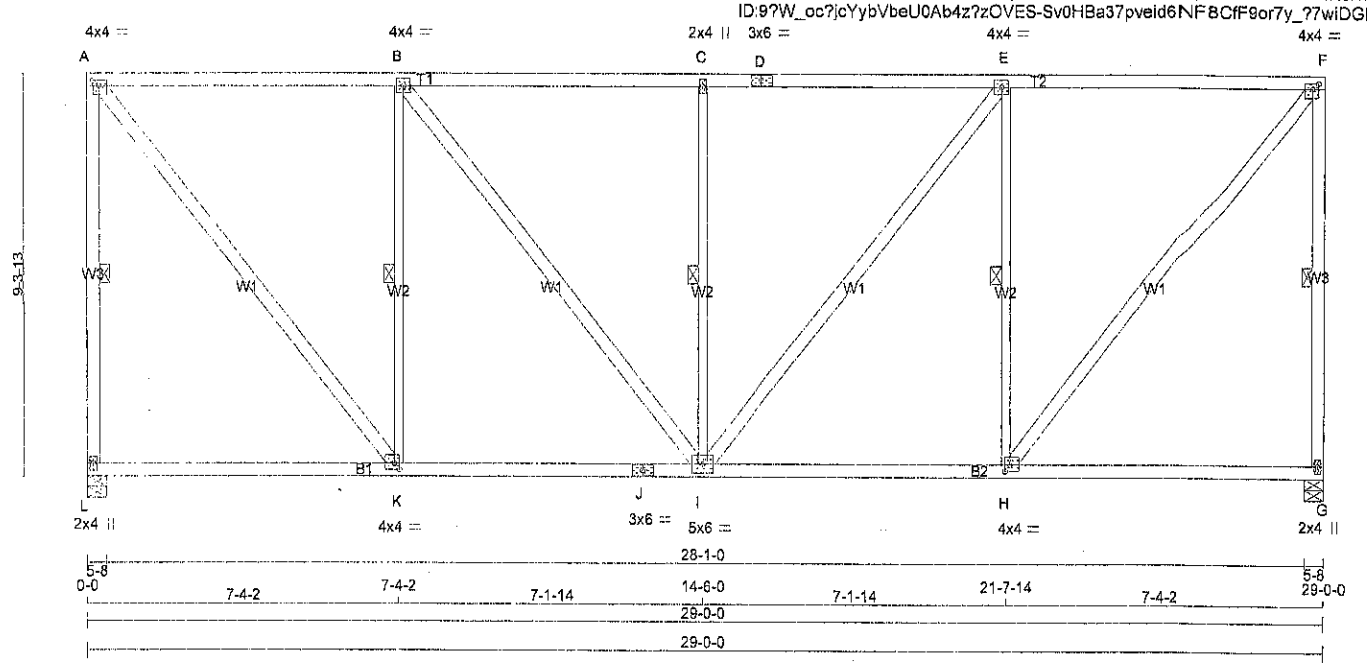
A-5073565

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H04	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:40:47 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-Sv0HBa37pveid6NF8CfF9or7y_?7wiDGN0xj83yy7vE

Scale = 1:53.6



TOTAL WEIGHT = 2 X 158 = 317 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - B	2x3	DRY	No.2	SPF
I - C	2x3	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	1.50	1.75
B	TMWW-t	MT20	4.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	1.50	1.75
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	4.0	4.0	1.75	1.50
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	1.75	1.50
L	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1275	0	1275	0	0	5-8	1-8
G	1275	0	1275	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	899	609 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0
G	899	609 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF A-L, F-G, B-K, C-I, E-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- A	-1224 / 0	0.0	0.0	0.50 (1)	5.80	A- K	0 / 1314	0.21 (1)
A- B	-830 / 0	-70.5	-70.5	0.61 (1)	5.70	K- B	-874 / 0	0.50 (1)
B- C	-1058 / 0	-70.5	-70.5	0.63 (1)	5.18	B- I	0 / 367	0.06 (1)
C- D	-1058 / 0	-70.5	-70.5	0.63 (1)	5.18	I- C	-462 / 0	0.26 (1)
D- E	-1058 / 0	-70.5	-70.5	0.63 (1)	5.18	I- E	0 / 367	0.06 (1)
E- F	-830 / 0	-70.5	-70.5	0.61 (1)	5.70	H- E	-874 / 0	0.50 (1)
G- F	-1224 / 0	0.0	0.0	0.50 (1)	5.80	H- F	0 / 1314	0.21 (1)
L- K	0 / 0	-17.5	-17.5	0.23 (4)	10.00			
K- J	0 / 830	-17.5	-17.5	0.30 (4)	10.00			
J- I	0 / 830	-17.5	-17.5	0.30 (4)	10.00			
I- H	0 / 830	-17.5	-17.5	0.30 (4)	10.00			
H- G	0 / 0	-17.5	-17.5	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.63 (B-C:1) , BC=0.30 (I-K:4) , WB=0.50 (E-H:1) , SSI=0.24 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.31 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-14-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

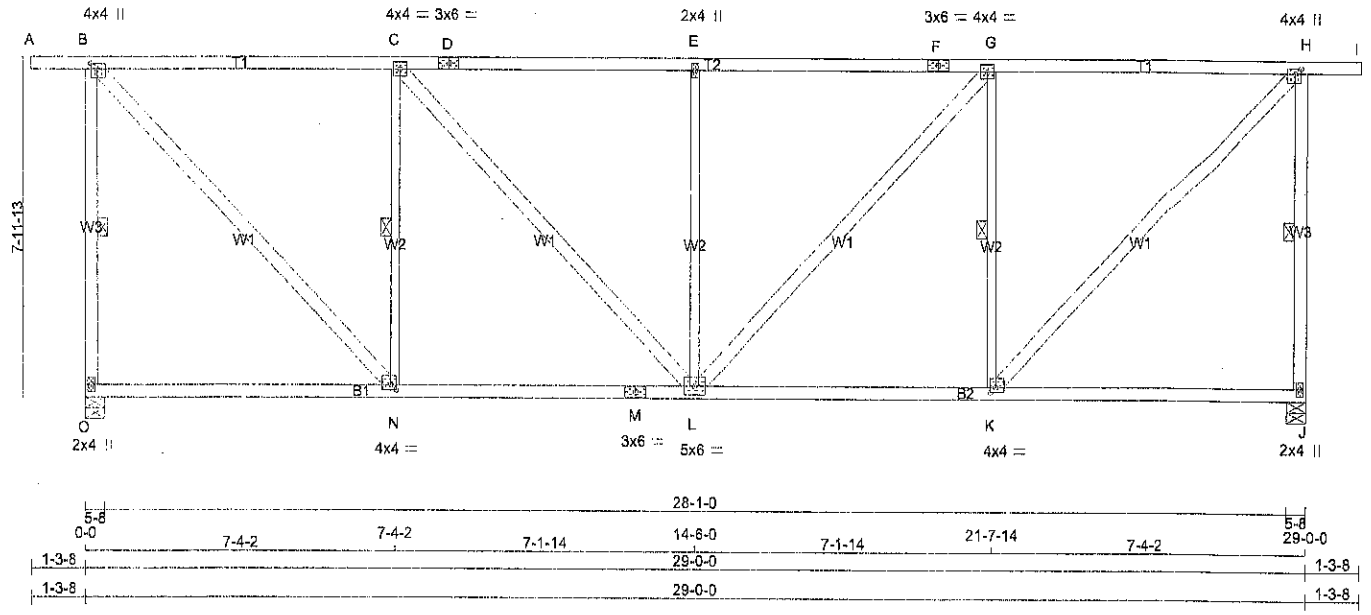
A-1073564

JOB-NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:40:46 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-zjSv_E2V2bWr0yo2bU80cbJxsbfjBE?68MBAcdyy7vF

Scale = 1:54.3



TOTAL WEIGHT = 2 X 150 = 301 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
N - C	2x3	DRY	No.2	SPF
L - E	2x3	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	1.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	4.0	1.50	1.75
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.50	1.50
L	BMWWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1366	0	1366	0	0	5-8	1-8
J	1366	0	1366	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	960	663 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	960	663 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF B-O, H-J, C-N, G-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- B	-1315 / 0	0.0	0.0	0.38 (1)	5.64	B- N	0 / 1409	0.23 (1)
A- B	0 / 0	-70.5	-70.5	0.08 (1)	10.00	N- C	-874 / 0	0.36 (1)
B- C	-973 / 0	-70.5	-70.5	0.62 (1)	5.35	C- L	0 / 393	0.06 (1)
C- D	-1241 / 0	-70.5	-70.5	0.65 (1)	4.85	L- E	-462 / 0	0.56 (1)
D- E	-1241 / 0	-70.5	-70.5	0.65 (1)	4.85	L- G	0 / 393	0.06 (1)
E- F	-1241 / 0	-70.5	-70.5	0.65 (1)	4.85	K- G	-874 / 0	0.35 (1)
F- G	-1241 / 0	-70.5	-70.5	0.65 (1)	4.85	K- H	0 / 1409	0.23 (1)
G- H	-973 / 0	-70.5	-70.5	0.62 (1)	5.35			
H- I	0 / 0	-70.5	-70.5	0.08 (1)	10.00			
J- H	-1315 / 0	0.0	0.0	0.38 (1)	5.64			
O- N	0 / 0	-17.5	-17.5	0.23 (4)	10.00			
N- M	0 / 973	-17.5	-17.5	0.31 (4)	10.00			
M- L	0 / 973	-17.5	-17.5	0.31 (4)	10.00			
L- K	0 / 973	-17.5	-17.5	0.31 (4)	10.00			
K- J	0 / 0	-17.5	-17.5	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.65 (E-G:1) , BC=0.31 (L-N:4) , WB=0.56 (E-L:1) , SSI=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.36 (K) (INPUT = 1.00)

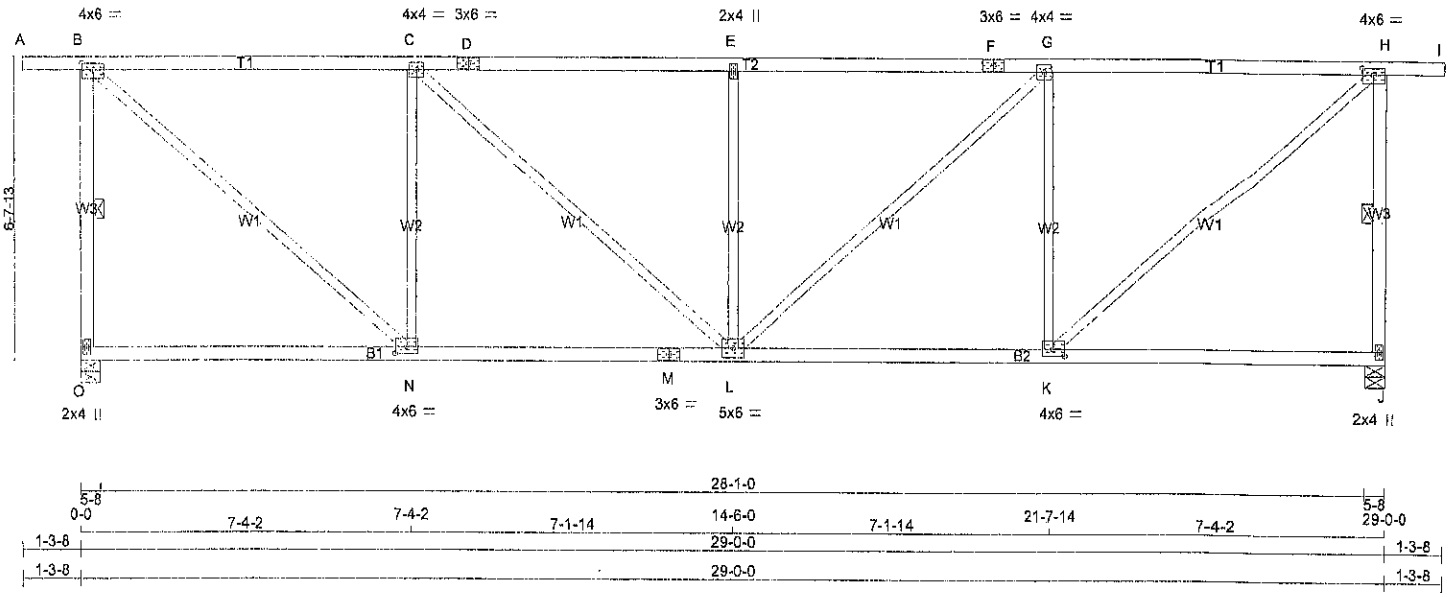
A-4073563

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:40:46 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-zjSv_E2V2bWr0yo2bU80cbJxLbeVBDn68MBAcdyy7vF

Scale = 1:50.8



TOTAL WEIGHT = 2 X 128 = 256 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	6.0	1.75	3.00
L	BMWWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0	1.75	3.00
O	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1366	0	1366	0	0	5-8	1-8
J	1366	0	1366	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	960	663 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	960	663 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF B-O, H-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- B	-1315 / 0	0.0	0.0	0.27 (1)	5.64		B- N	0 / 1556	0.35 (1)
A- B	0 / 0	-70.5	-70.5	0.08 (1)	10.00		N- C	-874 / 0	0.64 (1)
B- C	-1176 / 0	-70.5	-70.5	0.65 (1)	4.97		C- L	0 / 434	0.10 (1)
C- D	-1501 / 0	-70.5	-70.5	0.69 (1)	4.48		L- E	-462 / 0	0.34 (1)
D- E	-1501 / 0	-70.5	-70.5	0.69 (1)	4.48		L- G	0 / 434	0.10 (1)
E- F	-1501 / 0	-70.5	-70.5	0.69 (1)	4.48		K- G	-874 / 0	0.64 (1)
F- G	-1501 / 0	-70.5	-70.5	0.69 (1)	4.48		K- H	0 / 1556	0.35 (1)
G- H	-1176 / 0	-70.5	-70.5	0.65 (1)	4.97				
H- I	0 / 0	-70.5	-70.5	0.08 (1)	10.00				
J- H	-1315 / 0	0.0	0.0	0.27 (1)	5.64				
O- N	0 / 0	-17.5	-17.5	0.23 (4)	10.00				
N- M	0 / 1176	-17.5	-17.5	0.32 (4)	10.00				
M- L	0 / 1176	-17.5	-17.5	0.32 (4)	10.00				
L- K	0 / 1176	-17.5	-17.5	0.32 (4)	10.00				
K- J	0 / 0	-17.5	-17.5	0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.69 (E-G:1) , BC=0.32 (L-N:4) , WB=0.64 (G-K:1) , SSI=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.31 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/14/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

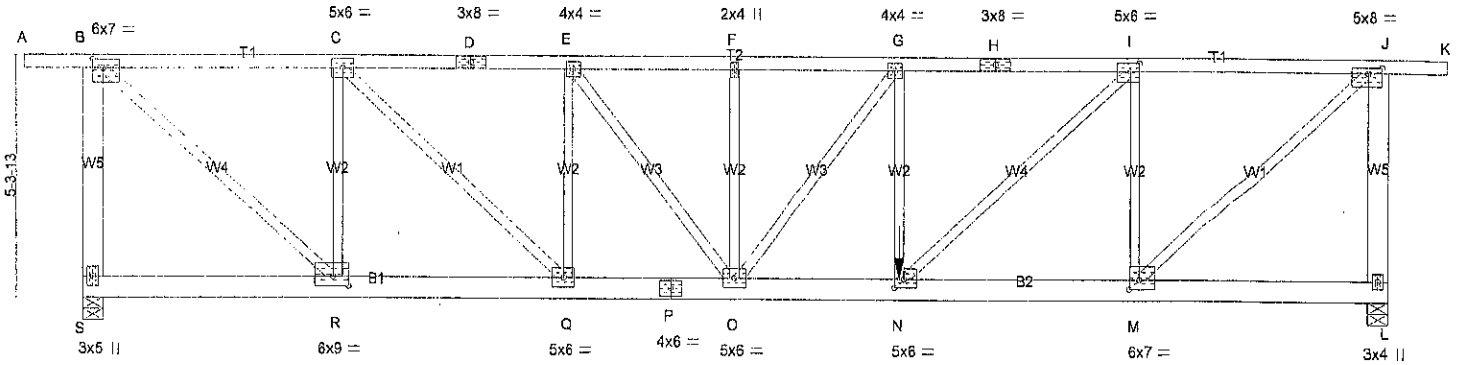
A-4073562

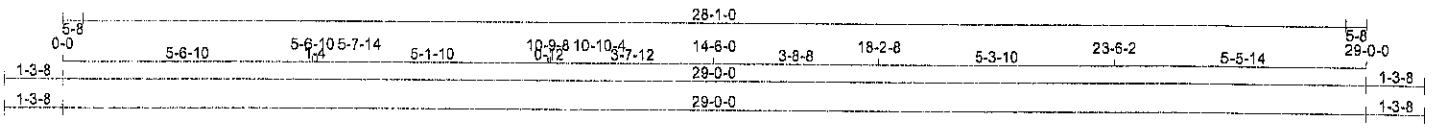
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H01-Cond2	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:40:45 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-VXuWnu2tHIO_OpDs1ncn3Nmk8BE?SlJzviSc3Ayy7vG

Scale = 1:50.8





TOTAL WEIGHT = 2 X 154 = 309 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	1650F 1.5E	SPF
D - H	2x4	DRY	1650F 1.5E	SPF
H - K	2x4	DRY	1650F 1.5E	SPF
L - J	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	7.0	2.25	2.75
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	5.0	6.0	2.25	2.50
J	TMVW-t	MT20	5.0	8.0	1.50	3.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	6.0	7.0	2.50	2.75
N	BMWW-t	MT20	5.0	6.0	2.25	2.50
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	6.0	9.0	2.50	4.25
S	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 817.5 lbs FACTORED DOWN AT 18-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2735	0	2735	0	0	5-8	3-6
L	2367	0	2367	0	0	5-8	2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1925	1316 / 0	0 / 0	0 / 0	0 / 0	609 / 0	0 / 0
L	1665	1140 / 0	0 / 0	0 / 0	0 / 0	525 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
S-B	-2649 / 0	0.0	0.0	0.78 (1)	6.36	
A-B	0 / 0	-70.5	-70.5	0.07 (1)	10.00	
B-C	-2568 / 0	-138.7	-138.7	0.69 (1)	3.96	
C-D	-3789 / 0	-138.7	-138.7	0.77 (1)	3.25	
D-E	-3769 / 0	-138.7	-138.7	0.77 (1)	3.25	
E-F	-4090 / 0	-138.7	-138.7	0.37 (1)	3.67	
F-G	-4090 / 0	-138.7	-138.7	0.29 (1)	3.77	
G-H	-3946 / 0	-70.5	-70.5	0.45 (1)	3.70	
H-I	-3946 / 0	-70.5	-70.5	0.45 (1)	3.70	
I-J	-2309 / 0	-70.5	-70.5	0.34 (1)	4.70	
J-K	0 / 0	-70.5	-70.5	0.07 (1)	10.00	
L-J	-2326 / 0	0.0	0.0	0.68 (1)	6.70	
S-R	0 / 0	-34.5	-34.5	0.13 (4)	10.00	
R-Q	0 / 2568	-34.5	-34.5	0.40 (1)	10.00	
Q-P	0 / 3769	-34.5	-34.5	0.55 (1)	10.00	
P-O	0 / 3769	-34.5	-34.5	0.55 (1)	10.00	
O-N	0 / 3946	-34.5	-34.5	0.59 (1)	10.00	
N-M	0 / 2309	-17.5	-17.5	0.36 (1)	10.00	
M-L	0 / 0	-17.5	-17.5	0.06 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-E	-1007 / 0	0.41 (1)
E-O	0 / 535	0.13 (1)
O-F	-500 / 0	0.20 (1)
O-G	0 / 240	0.06 (1)
N-G	-620 / 0	0.25 (1)
B-R	0 / 3410	0.84 (1)
R-C	-2032 / 0	0.83 (1)
C-Q	0 / 1667	0.41 (1)
N-I	0 / 2236	0.55 (1)
M-I	-1960 / 0	0.80 (1)
M-J	0 / 3107	0.77 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	18-2-8	-818	-818	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 18-2-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.78 (B-S:1) , BC=0.59 (N-O:1) , WB=0.84 (B-R:1) , SSI=0.40 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

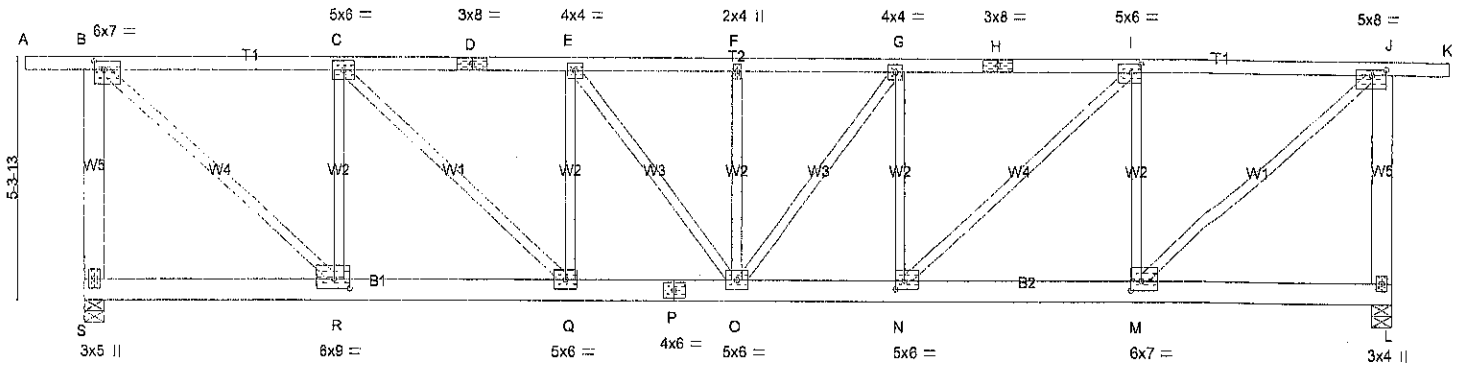
A-1073561

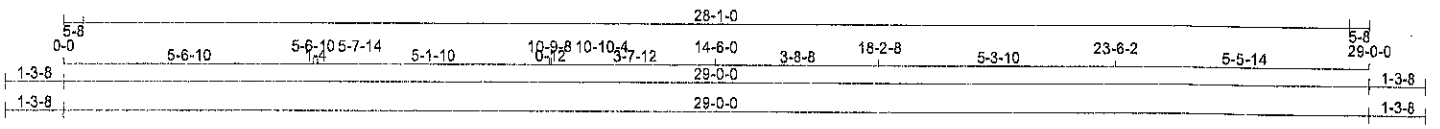
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253426	H01-Cond1	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.820 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 14:40:45 2015 Page 1
ID:9?W_oc?jcYybVbeU0Ab4z?zOVES-VXuWnu2tHIO_OpDs1ncn3NmlkBFASj6zviSc3Ayy7vG

Scale = 1:50.8





TOTAL WEIGHT = 2 X 154 = 309 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	1650F 1.5E	SPF
D - H	2x4	DRY	1650F 1.5E	SPF
H - K	2x4	DRY	1650F 1.5E	SPF
L - J	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	7.0	2.25	2.75
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	5.0	6.0	2.25	2.50
J	TMVW-t	MT20	5.0	8.0	1.50	3.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	6.0	7.0	2.50	2.75
N	BMWW-t	MT20	5.0	6.0	2.25	2.50
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	6.0	9.0	2.50	4.25
S	BMV1+p	MT20	3.0	5.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2602	0	2602	0	0	5-8	2-15
L	2602	0	2602	0	0	5-8	2-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1831	1252 / 0	0 / 0	0 / 0	0 / 0	579 / 0	0 / 0
L	1831	1252 / 0	0 / 0	0 / 0	0 / 0	579 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
S-B	-2516 / 0	0.0	0.0	0.74 (1)	6.48
A-B	0 / 0	-70.5	-70.5	0.07 (1)	10.00
B-C	-2415 / 0	-138.7	-138.7	0.68 (1)	4.07
C-D	-3481 / 0	-138.7	-138.7	0.74 (1)	3.40
D-E	-3481 / 0	-138.7	-138.7	0.74 (1)	3.40
E-F	-3693 / 0	-138.7	-138.7	0.35 (1)	3.85
F-G	-3693 / 0	-138.7	-138.7	0.38 (1)	3.81
G-H	-3486 / 0	-138.7	-138.7	0.74 (1)	3.42
H-I	-3486 / 0	-138.7	-138.7	0.74 (1)	3.42
I-J	-2361 / 0	-138.7	-138.7	0.65 (1)	4.14
J-K	0 / 0	-70.5	-70.5	0.07 (1)	10.00
L-J	-2519 / 0	0.0	0.0	0.74 (1)	6.48
S-R	0 / 0	-34.5	-34.5	0.13 (4)	10.00
R-Q	0 / 2415	-34.5	-34.5	0.38 (1)	10.00
Q-P	0 / 3481	-34.5	-34.5	0.52 (1)	10.00
P-O	0 / 3481	-34.5	-34.5	0.52 (1)	10.00
O-N	0 / 3486	-34.5	-34.5	0.52 (1)	10.00
N-M	0 / 2361	-34.5	-34.5	0.37 (1)	10.00
M-L	0 / 0	-34.5	-34.5	0.12 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-E	-871 / 0	0.35 (1)
E-O	0 / 354	0.09 (1)
O-F	-464 / 0	0.19 (1)
O-G	0 / 346	0.09 (1)
N-G	-888 / 0	0.36 (1)
B-R	0 / 3208	0.79 (1)
R-C	-1900 / 0	0.77 (1)
C-Q	0 / 1479	0.37 (1)
N-I	0 / 1537	0.38 (1)
M-I	-1916 / 0	0.78 (1)
M-J	0 / 3178	0.79 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.74 (J-L:1) , BC=0.52 (O-Q:1) , WB=0.79 (B-R:1) , SSI=0.40 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (M) (INPUT = 0.90)
JSI METAL= 0.76 (P) (INPUT = 1.00)

A-10073560

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

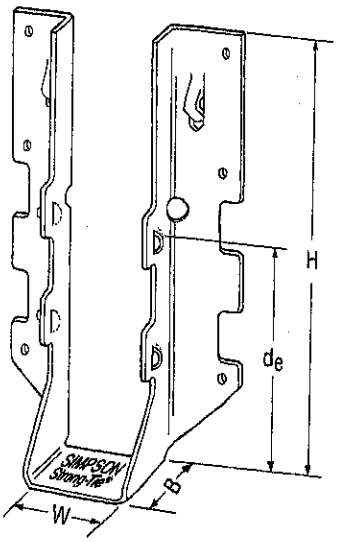
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

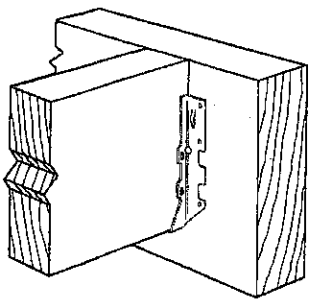
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



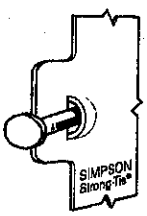
LUS28



Typical LUS Installation

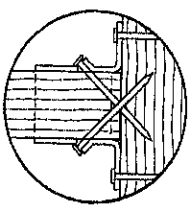
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

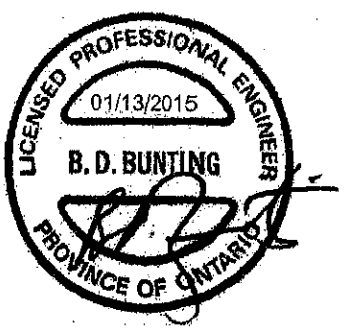


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

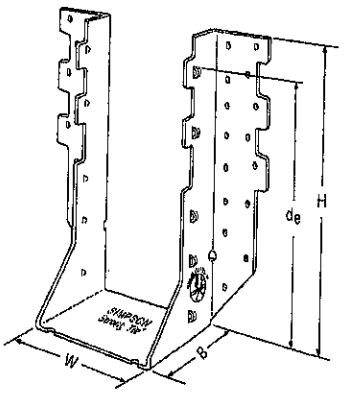
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

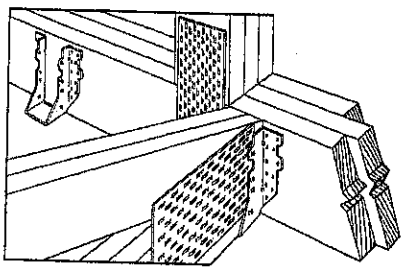
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



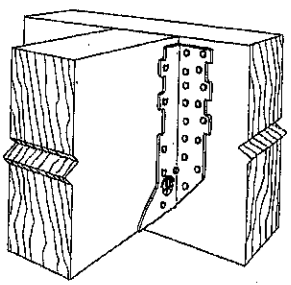
HHUS410



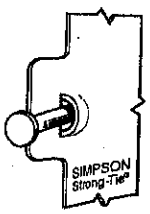
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 9/32	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	30-16d	10-16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 13/32	3	3 15/16	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3 5/8	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	30-16d	10-16d	4745	10770	4310	7650

1. d_e is the distance from the seat of the hanger to the highest joist nail.

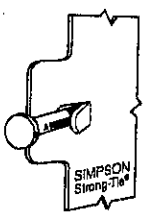


Typical HHUS Installation

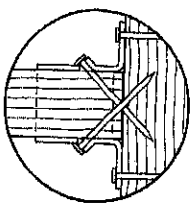


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

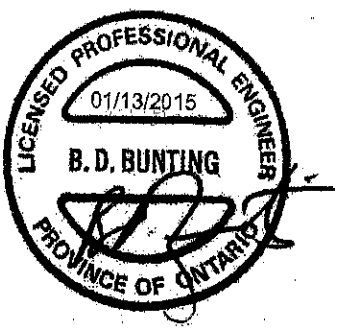
U.S. Patent 5,603,580



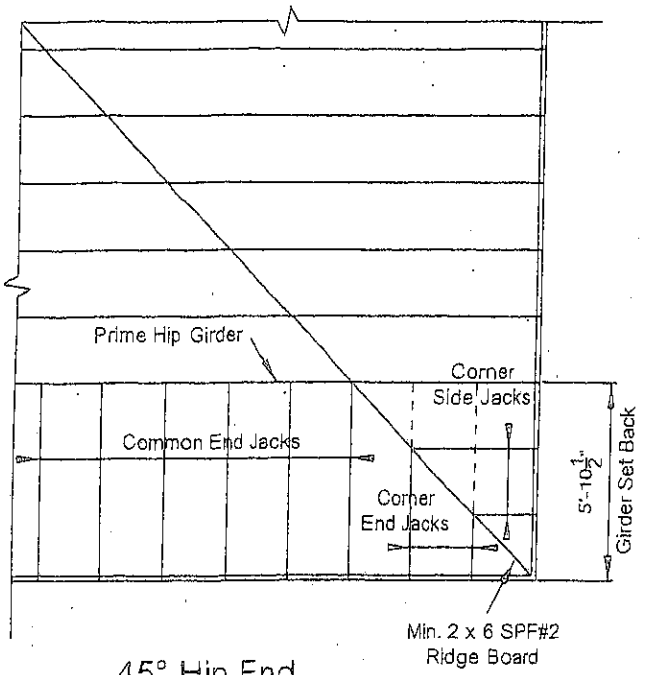
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



STRACON ENGINEERING INC.



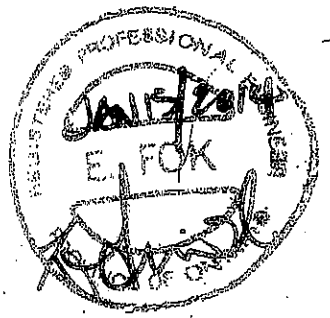
45° Hip End

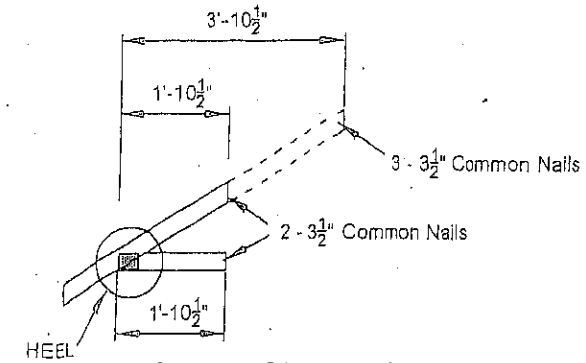
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

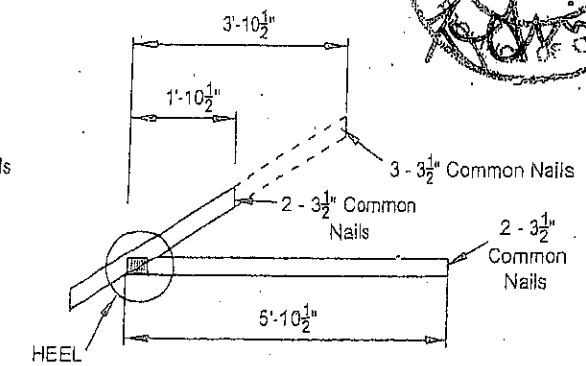
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

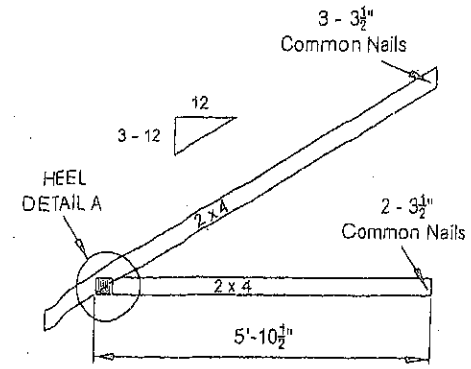




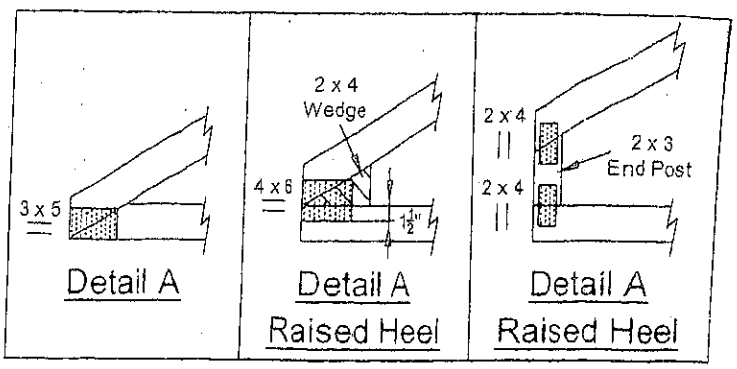
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008