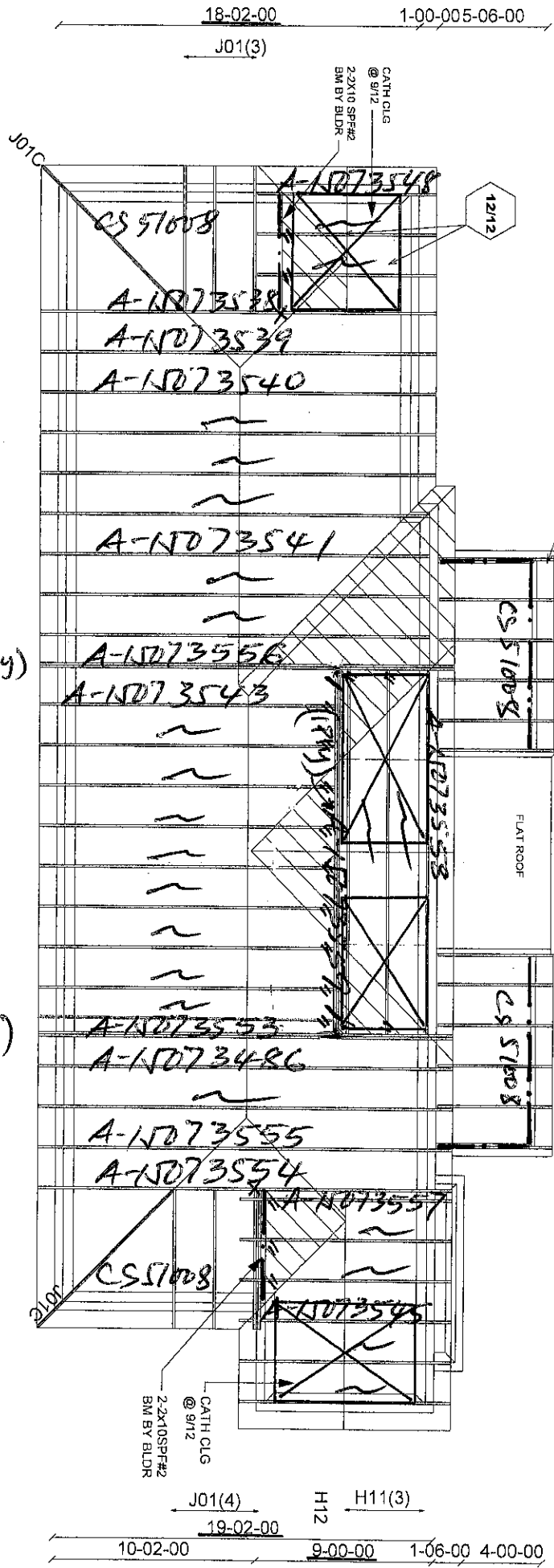


ALL CONG. FRAMING TO CONFORM WITH PART 9 OF O.S.C. 2012. ROOF RAFTERS THAT CROSS WITH A VERT. POST TO THE TRUSS WITHIN THE TRUSS PLY TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 8'

15-09-00 12-03-00 29-03-00 1-04-00 2-01-00

H07(3) H01 H02 H03(4) H03E(3) H09A H04S(4) H04B H04 H09 H08 H10(3) H05(3)

12/12 4/12 T.O.P. @ 8'-0" H.O. 1/4" HEEL - 1/4" DROP P01(6) P01(6)



12" FIN. OH. RSD. HEEL - 0" DROP (2" x 6" FASCIA) - FM ASPHALT SHINGLES 2" X 6" BRG/BRICK

(i Ply)

55-09-00

(1 Ply)

4-11-00

HANGERS // LUS24 / LUS26DS X HHUS26-2

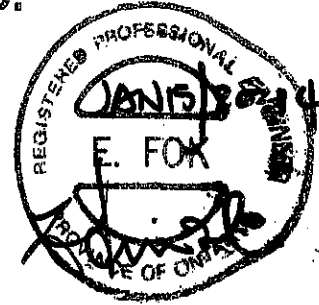


Job Track	40297	Builder / Location	GOLD PARK / KLEINBURG	Model / Elevation	25-8 / B
Layout ID	253403	Project	HUNTINGTON & NASHVILLE	THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
Plan Log	79711	Date	4/21/2015	Designer	cheryl

104 Ver 7.5.0

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



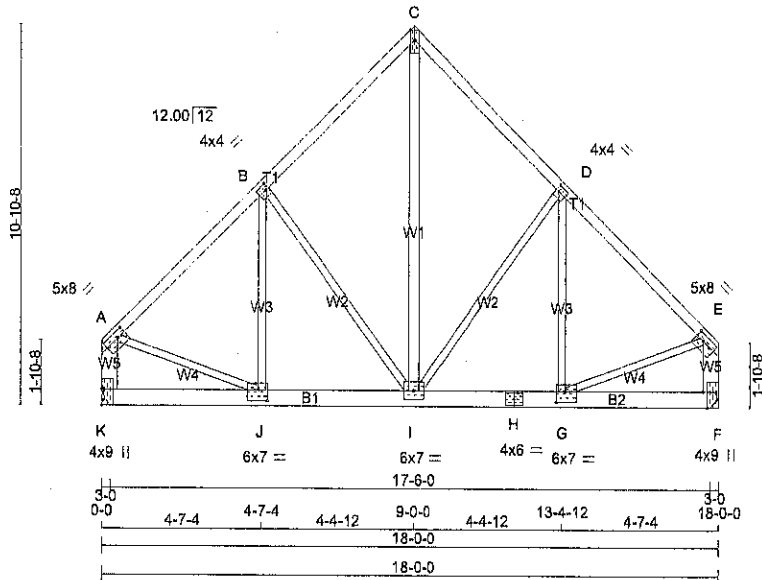
RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014



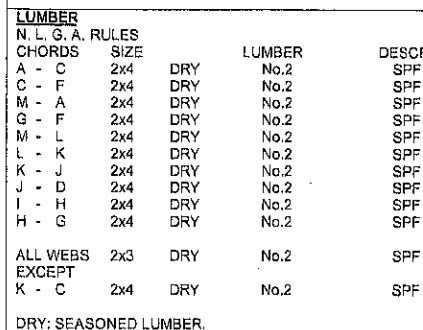
LUMBER N. L. G. A. RULES					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 106 lb [M]	
CHORDS					BEARINGS										DESIGN CRITERIA	
A - C 2x4 DRY No.2 SPF					FACTORED GROSS REACTION					MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		
C - E 2x4 DRY No.2 SPF					JT VERT HORZ					DOWN HORZ UPLIFT		IN-SX IN-SX				
K - A 2x6 DRY No.2 SPF					K 3084 0					3084 0 0		HANGER BY OTHERS				
F - E 2x6 DRY No.2 SPF					F 3084 0					3084 0 0		MIN. SEAT SIZE: 3-0				
K - H 2x6 DRY No.2 SPF												HANGER BY OTHERS				
H - F 2x6 DRY No.2 SPF																
ALL WEBS 2x3 DRY No.2 SPF																
EXCEPT I - C 2x4 DRY No.2 SPF																
DRY: SEASONED LUMBER.																
PLATES (table is in inches)																
JT TYPE PLATES W LEN Y X																
A TMVW-1 MT20 5.0 8.0 2.00 3.00																
B TMVW-1 MT20 4.0 4.0 2.00 1.00																
C TTW+p MT20 3.0 8.0 Edge																
D TMVW-1 MT20 4.0 4.0 2.00 1.00																
E TMVW-1 MT20 5.0 8.0 2.00 3.00																
F BMV1+1 MT20 4.0 9.0 Edge 1.50																
G BMWW-1 MT20 6.0 7.0 3.75 3.25																
H BS-1 MT20 4.0 6.0																
I BMVWW-1 MT20 6.0 7.0																
J BMWW-1 MT20 6.0 7.0 3.75 3.25																
K BMV1+1 MT20 4.0 9.0 5.50																
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.																
NOTES: (1)																
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.																

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.									
LOADING TOTAL LOAD CASES: (4)									
CHORDS									
MEMB.		MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)		MAX UNBRAC LENGTH	
FR-TO		FROM TO						FR-TO	
A-B		-2597 / 0		-70.5		-70.5 0.44 (1)		3.92	
B-C		-1958 / 0		-70.5		-70.5 0.35 (1)		4.47	
C-D		-1958 / 0		-70.5		-70.5 0.35 (1)		4.47	
D-E		-2597 / 0		-70.5		-70.5 0.44 (1)		3.92	
K-A		-2581 / 0		0.0		0.0 0.20 (1)		8.44	
F-E		-2581 / 0		0.0		0.0 0.20 (1)		8.44	
K-J		0 / 0		-272.2		-272.2 0.37 (1)		10.00	
J-I		0 / 1853		-272.2		-272.2 0.59 (1)		10.00	
I-H		0 / 1853		-272.2		-272.2 0.59 (1)		10.00	
H-G		0 / 1853		-272.2		-272.2 0.59 (1)		10.00	
G-F		0 / 0		-272.2		-272.2 0.37 (1)		10.00	
WEBS									
MEMB.		MAX. FACTORED FORCE (LBS)		MAX CSI (LC)					
I-C		0 / 2495		0.44 (1)					
I-D		-826 / 0		0.90 (1)					
G-D		0 / 768		0.19 (1)					
B-I		-826 / 0		0.90 (1)					
J-B		0 / 768		0.19 (1)					
A-J		0 / 1941		0.48 (1)					
G-E		0 / 1941		0.48 (1)					

SPECIFIED LOADS:									
TOP CH.		LL = 21.0 PSF		DL = 3.0 PSF		BOT CH.		LL = 0.0 PSF	
		DL = 7.0 PSF		TOTAL LOAD = 31.0 PSF					
SPACING = 24.0 IN. C/C									
GIRDER TYPE: CStdGirder									
START DISTANCE = 0-0									
START SPAN CARRIED = 13-7-0									
END DISTANCE = 18-0-0									
END SPAN CARRIED = 13-7-0									
END WALL WIDTH = 0-0									
APPLIED TO FRONT SIDE OF BOTTOM CHORD.									
- ADDTL LOADS BASED ON 55 % OF GSL.									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
- CSA 086-09									
- TPIC 2011									
(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEFL.(LL)= L/380 (0.60")									
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06")									
ALLOWABLE DEFL.(TL)= L/360 (0.60")									
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.11")									
CSI: TC=0.44 (D-E:1), BC=0.59 (G-I:1), WB=0.90 (D-I:1), SSI=0.48 (F-G:1)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00									
COMP=1.00 SHEAR=1.00 TENS=1.00									
COMPANION LIVE LOAD FACTOR = 0.50									
AUTOSOLVE HEELS OFF									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									
PLATE		GRIP(DRY)		SHEAR		SECTION			
		(PSI)		(PLI)		(PLI)			
MAX MIN		MAX MIN		MAX MIN		MAX MIN			
MT20		618 354		1867 822		2284 1656			
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.87 (A) (INPUT = 0.90)									
JSI METAL= 0.51 (A) (INPUT = 1.00)									
A-10073559									



A-15073559



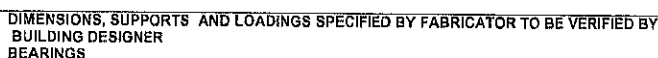
DRY; SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV+p	MT20	5.0	6.0	2.75	2.00
B	TMVVW	MT20	4.0	6.0	2.00	2.25
C	TTVVW-	MT20	4.0	9.0		Edge
D	TMV+p	MT20	2.0	4.0		
E	TMVVW-	MT20	4.0	6.0	2.00	2.50
F	TMVV+p	MT20	5.0	6.0		
G	BMV1+p	MT20	2.0	4.0		
H	BWVVW-p	MT20	5.0	6.0	2.75	2.00
I	BVMVVWW-	MT20	6.0	9.0	3.00	3.00
J	BMV+p	MT20	2.0	4.0		
K	BWVVW-w	MT20	5.0	8.0		Edge 3.50
L	BMVVW-p	MT20	5.0	6.0	2.75	2.00
M	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per



JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	792	0	792	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	792	0	792	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	558	378 / 0	0 / 0	0 / 0	0 / 0	180 / 0	0 / 0
G	558	378 / 0	0 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY
APPLIED

1 LATERAL BRACE(S) REQUIRED FOR D-J. BRACE LOCATED AT EVERY 1/2 LENGTH OF D-J.
1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF B-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 (LC)	MAX. UNBRAC (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	-1395 / 0	-70.5	-70.5	0.20 (1)	5.33	L-B	0 / 1204	0.27 (1)	
B-C	-814 / 0	-70.5	-70.5	0.17 (1)	6.25	K-C	0 / 83	0.09 (1)	
C-D	-680 / 0	-70.5	-70.5	0.05 (1)	6.25	K-I	0 / 383	0.09 (1)	
D-E	-708 / 0	-70.5	-70.5	0.10 (1)	6.25	C-I	0 / 590	0.13 (1)	
E-F	-1376 / 0	-70.5	-70.5	0.21 (1)	5.34	I-E	-1043 / 0	0.46 (1)	
M-A	-753 / 0	0.0	0.0	0.09 (1)	7.81	H-E	0 / 1120	0.25 (1)	
G-F	-753 / 0	0.0	0.0	0.09 (1)	7.81	A-L	0 / 1070	0.24 (1)	
						H-F	0 / 1049	0.24 (1)	
						B-K	-1139 / 0	0.41 (1)	
M-L	0 / 0	-17.5	-17.5	0.10 (4)	10.00				
L-K	0 / 1256	-17.5	-17.5	0.25 (1)	10.00				
K-J	0 / 11	-17.5	-17.5	0.03 (4)	10.00				
J-I	0 / 25	0.0	0.0	0.02 (1)	10.00				
I-D	-126 / 0	0.0	0.0	0.02 (1)	6.25				
I-H	0 / 1204	-17.5	-17.5	0.21 (1)	10.00				
H-G	0 / 0	-17.5	-17.5	0.10 (4)	10.00				

TOTAL WEIGHT = 3 X 110 = 330 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.21 (E-F:1), BC=0.25 (K-L:1), WB=0.46 (E-I:1), SSI=0.11 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

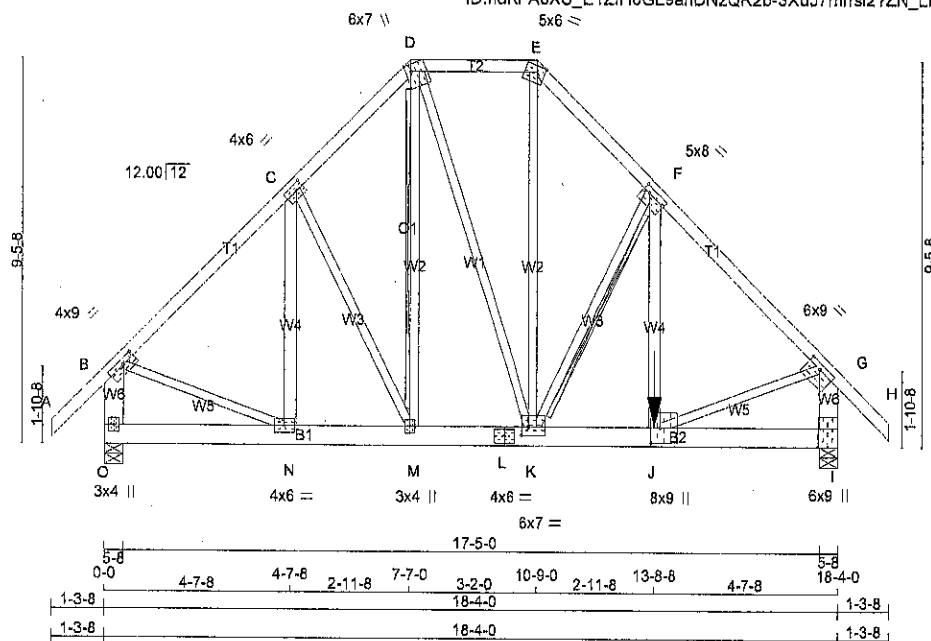
JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.25 (A) (INPUT = 1.00)

A-1107 3558

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253403	H09A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:hdKFA8XS_E1ZiH0GL9ehDNzQR2b-3XdJ7mrrsi27ZN_LNHMISExODRhCMB1Z57lyy8VWF
Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Tue Jul 14 13:59:10 2015 Page 1
Scale = 1:56.9



<u>LUMBER</u>					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR	
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
O - B	2x6	DRY	No.2	SPF	
I - G	2x6	DRY	No.2	SPF	
O - L	2x8	DRY	No.2	SPF	
L - I	2x8	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
EXCEPT					
N - C	2x4	DRY	No.2	SPF	
J - E	2x4	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	1.50	4.00
C	TMVW-t	MT20	4.0	8.0	2.00	2.25
D	TTWW+m	MT20	6.0	7.0	Edge	1.75
E	TTWW-m	MT20	5.0	6.0	Edge	2.25
F	TMVW-t	MT20	5.0	8.0	1.75	2.00
G	TMVW-t	MT20	6.0	9.0	1.60	4.00
I	BMV1-t	MT20	6.0	9.0	Edge	0.50
J	BMVW-t	MT20	8.0	9.0	4.50	2.75
K	BMVW-t	MT20	6.0	7.0	2.50	2.00
L	BS-t	MT20	4.0	6.0		
M	BMVW-t	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	6.0		
O	BMV1+P	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
O	1795	0	5-8	1-15
I	4357	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1328	541 / 0	0 / 0	0 / 0	0 / 0	0 / 0	787 / 0	0 / 0
I	3262	1122 / 0	0 / 0	0 / 0	0 / 0	0 / 0	2140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.18FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-M, F-K
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS: (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS: (LC)
FR-TO			FROM TO			FR-TO			
A-B	0 / 35	-70.5	-70.5	0.12 (1)	10.00	N-C	-318 / 0	0.18 (1)	
B-C	-1618 / 0	-70.5	-70.5	0.34 (1)	4.80	C-M	-27 / 98	0.04 (4)	
C-D	-1663 / 0	-70.5	-70.5	0.29 (1)	4.75	M-D	-22 / 120	0.03 (1)	
D-E	-1442 / 0	-70.5	-70.5	0.19 (1)	5.17	D-K	0 / 916	0.34 (4)	
E-F	-2054 / 0	-70.5	-70.5	0.27 (1)	4.39	K-E	0 / 1261	0.41 (4)	
F-G	-3577 / 0	-70.5	-70.5	0.69 (1)	3.16	K-F	-2470 / 0	0.94 (1)	
G-H	0 / 35	-70.5	-70.5	0.12 (1)	10.00	J-F	0 / 2979	0.74 (4)	
O-B	-1751 / 0	0.0	0.0	0.15 (1)	7.43	B-N	0 / 1214	0.34 (1)	
I-G	-3570 / 0	0.0	0.0	0.31 (1)	5.51	J-G	0 / 2861	0.81 (4)	
O-N	0 / 0	-17.5	-17.5	0.05 (4)	10.00				
N-M	0 / 1159	-17.5	-17.5	0.21 (1)	10.00				
M-L	0 / 1148	-17.5	-17.5	0.23 (4)	10.00				
L-K	0 / 1148	-17.5	-17.5	0.23 (4)	10.00				
K-J	0 / 2541	-17.5	-17.5	0.63 (1)	10.00				
J-I	0 / 0	-369.3	-369.3	0.70 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	13-8-8	-2716	-2716	---	FRONT	VERT	DEAD

TOTAL WEIGHT = 123 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSIdGirder
START DISTANCE = 13-8-8
START SPAN CARRIED = 18-0-0
END DISTANCE = 18-4-0
END SPAN CARRIED = 18-0-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 65 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.69 (F-G:1), BC=0.70 (I-J:1), WB=0.94 (F-K:1), SS=0.69 (J-I:1)

DOL LUMBER=0.88 NAIL=0.88 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	397	1667
MT20	618	397	1667

CONFIRMED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253403	H09A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:hdKFA8XS E1ZIH0GL9ahDNzQR2b-3XdJ7mrsi27ZN_LNHMISExODRhCmHCJB1Z57lyy8WF

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3042.2 lbs FACTORED DOWN AT 13-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.90)

JSI METAL = 0.77 (G) (INPUT = 1.00)



A-15073556(2)

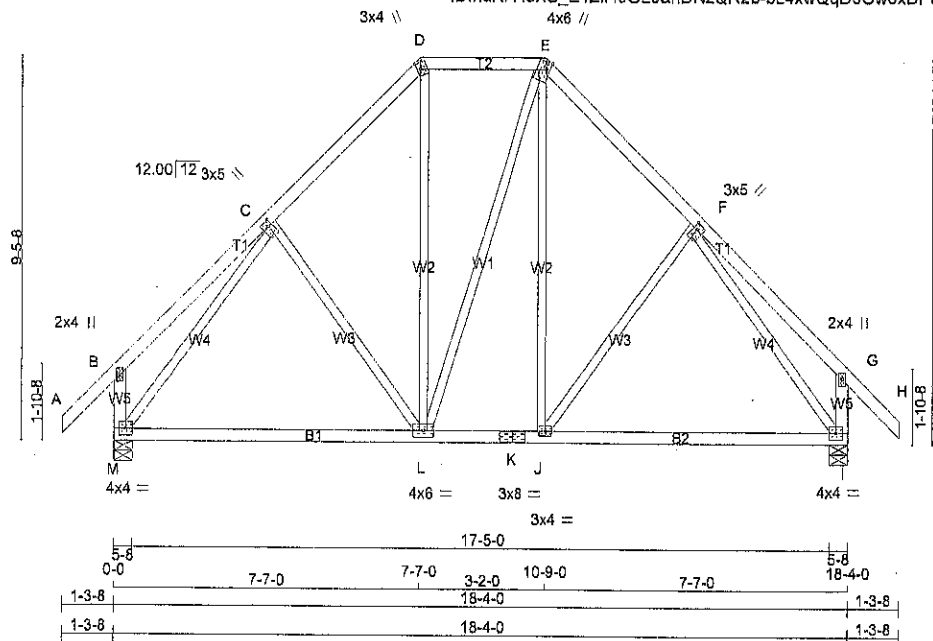
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253403	H09	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-bL4xwQgD5Ow8xDP8paqTv0OLF1SWdvKayNqYTry8WVG

Scale = 1:56.9



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW+H	MT20	3.0	5.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	2.00	1.00
E	TTWW+m	MT20	4.0	6.0	Edge	1.00
F	TMWW+H	MT20	3.0	5.0	1.75	1.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	3.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW+H	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
M	904	0	904	0	5-8	1-8
I	904	0	904	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
I	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED	FACTORED		MAX UNBRAC	MEMB.	MAX. FACTORED	MAX CSI (LC)	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX			FORCE (LBS)		
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 35	-70.5	-70.5	0.10 (1)	10.00	C-L	-103 / 9	0.08 (1)
B-C	0 / 22	-70.5	-70.5	0.16 (1)	10.00	L-D	0 / 187	0.04 (1)
C-D	-589 / 0	-70.5	-70.5	0.13 (1)	6.25	L-E	0 / 1	0.00 (1)
D-E	-402 / 0	-70.5	-70.5	0.09 (1)	6.25	J-E	0 / 185	0.04 (1)
E-F	-589 / 0	-70.5	-70.5	0.13 (1)	6.25	J-F	-104 / 9	0.08 (1)
F-G	0 / 22	-70.5	-70.5	0.16 (1)	10.00	M-C	-797 / 0	0.59 (1)
G-H	0 / 35	-70.5	-70.5	0.10 (1)	10.00	F-I	-796 / 0	0.58 (1)
M-B	-199 / 0	0.0	0.0	0.02 (1)	7.81			
I-G	-199 / 0	0.0	0.0	0.02 (1)	7.81			
M-L	0 / 462	-17.5	-17.5	0.24 (4)	10.00			
L-K	0 / 401	-17.5	-17.5	0.24 (4)	10.00			
K-J	0 / 401	-17.5	-17.5	0.24 (4)	10.00			
J-I	0 / 461	-17.5	-17.5	0.24 (4)	10.00			

TOTAL WEIGHT = 99 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.16 (F-G:1), BC=0.24 (I-J:4), WB=0.59 (C-M:1), SSI=0.11 (L-M:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	364
	1667	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.80)

JSI METAL= 0.37 (C) (INPUT = 1.00)



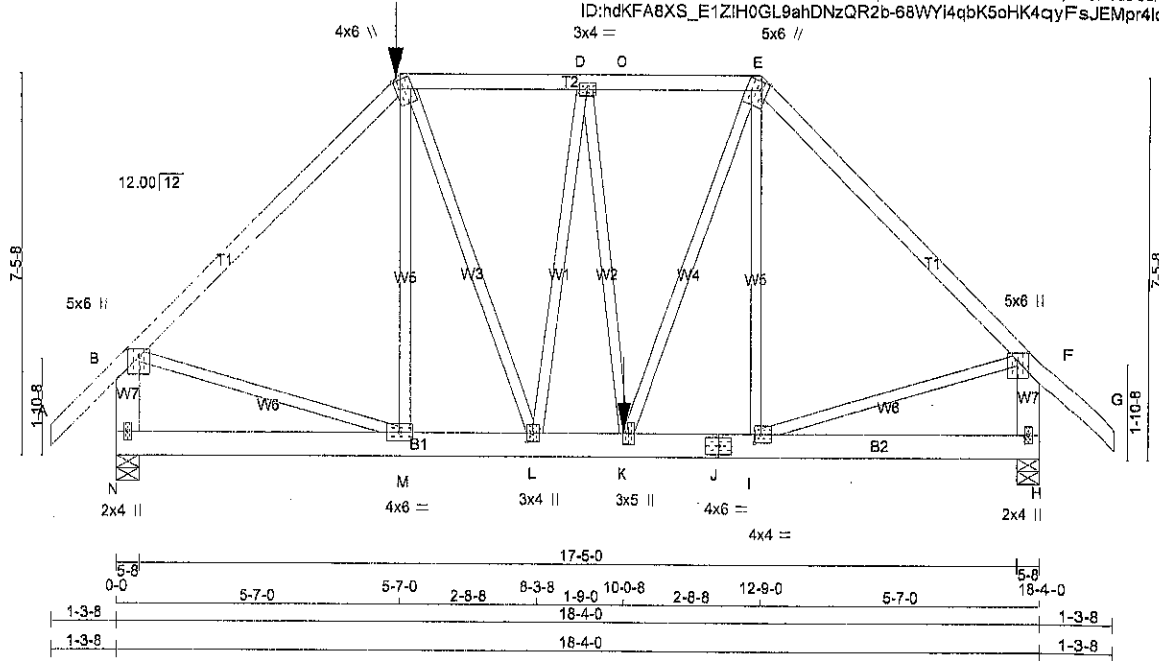
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253403	H08	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-68WVY4qbK5oHK4qyFsJEMpr4Id6fuXeQk4_wPyy8WH
Scale = 1:45.2



TOTAL WEIGHT = 111 lb
(M)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	4.0	6.0	2.00	1.25
D	TMVW-l	MT20	3.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	1.75	2.75
H	BMV1+p	MT20	2.0	4.0		
I	BMVW-l	MT20	4.0	4.0	2.00	1.50
J	BS-l	MT20	4.0	6.0		
K	BMVW+l	MT20	3.0	5.0		
L	BMVW+l	MT20	3.0	4.0	2.00	1.25
M	BMVW-l	MT20	4.0	6.0		
N	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 267.7 lbs FACTORED DOWN AT 5-7-0 ON TOP CHORD, AND 618.0 lbs FACTORED DOWN AT 10-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	1615	0	1615	0	5-8	1-12
H	1451	0	1451	0	5-8	1-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
N	1182	650/0	0/0	0/0	0/0	511/0	0/0
H	1048	563/0	0/0	0/0	0/0	486/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/35	-70.5 -70.5	0.11 (1)	10.00	M-C	-56/83	0.05 (1)
B-C	-1398/0	-70.5 -70.5	0.56 (1)	4.85	C-L	0/402	0.12 (4)
C-D	-1127/0	-133.6 -133.6	0.26 (1)	5.82	L-D	-307/0	0.30 (1)
D-O	-1145/0	-133.6 -133.6	0.26 (1)	5.58	D-K	-179/150	0.17 (1)
O-E	-1146/0	-70.5 -70.5	0.26 (1)	5.58	K-E	0/724	0.18 (1)
E-F	-1260/0	-70.5 -70.5	0.54 (1)	5.07	I-E	-78/34	0.07 (1)
F-G	0/35	-70.5 -70.5	0.11 (1)	10.00	B-M	0/1017	0.25 (1)
N-B	-1536/0	0.0 0.0	0.12 (1)	7.81	I-F	0/917	0.23 (1)
H-F	-1414/0	0.0 0.0	0.11 (1)	7.81			
N-M	0/0	-33.2 -33.2	0.12 (4)	10.00			
M-L	0/985	-33.2 -33.2	0.21 (4)	10.00			
L-K	0/1167	-33.2 -33.2	0.22 (4)	10.00			
K-J	0/888	-17.5 -17.5	0.19 (4)	10.00			
J-I	0/888	-17.5 -17.5	0.19 (4)	10.00			
I-H	0/0	-17.5 -17.5	0.08 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-7-0	-268	-268	---	FRONT	VERT	TOTAL
K	10-0-8	-560	-560	---	FRONT	VERT	DEAD

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 31.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHp
SIDE SETBACK = 5-7-0
END SETBACK = 6-7-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-0-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.56 (B-C:1), BC=0.22 (K-L:4), WB=0.30 (D-L:1), SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

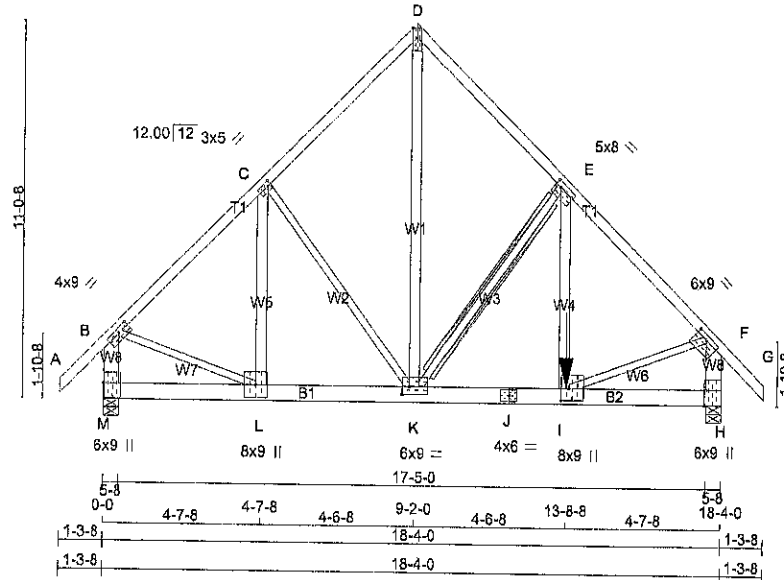
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253403	H04B	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:hdKFA8XS_E1ZiH0GL9ahDNzQR2b-58WYI4qbK5oHK4qyFsJEMpr1_d_uuNWQkj4_wPpy8WH
3x8 II
Scale = 1:67.5



LUMBER				
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	9.0	1.50	4.00
C	TMVW-I	MT20	3.0	5.0	1.50	1.25
D	TTW+p	MT20	3.0	8.0	Edge	
E	TMVW-I	MT20	5.0	8.0	1.75	2.00
F	TMVW-I	MT20	6.0	9.0	1.50	4.00
H	BMV1+I	MT20	6.0	9.0	Edge	0.50
I	BMVW+I	MT20	8.0	9.0	4.25	2.75
J	BS-I	MT20	4.0	6.0		
K	BMVW+I	MT20	6.0	9.0	2.75	2.50
L	BMVW+I	MT20	8.0	9.0		
M	BMV1+p	MT20	6.0	9.0	5.25	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3042.2 lbs FACTORED DOWN AT 13-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
M	1810	0	1810	0	5-8	1-15	5-8
H	4364	0	4364	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
M	1339	544 / 0	0 / 0	0 / 0	795 / 0	0 / 0	0 / 0
H	3266	1130 / 0	0 / 0	0 / 0	2136 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 I-BRACE REQUIRED AT E-K
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS MAX. FACTORED		FACTORED		WEBS MAX. FACTORED		FR-TO
	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	FORCE (LBS)	
A-B		0 / 35	-70.5	-70.5 0.12 (1)	L-C	-275 / 0	0.13 (1)
B-C		-1643 / 0	-70.5	-70.5 0.41 (1)	C-K	-87 / 87	0.10 (1)
C-D		-1622 / 0	-70.5	-70.5 0.40 (1)	K-D	0 / 2010	0.45 (4)
D-E		-1620 / 0	-70.5	-70.5 0.37 (1)	E-K	-2415 / 0	0.95 (1)
E-F		-3588 / 0	-70.5	-70.5 0.73 (1)	I-E	0 / 2985	0.75 (4)
F-G		0 / 35	-70.5	-70.5 0.12 (1)	B-L	0 / 1236	0.35 (1)
M-B		-1772 / 0	0.0	0.0 0.15 (1)	I-F	0 / 2671	0.81 (4)
H-F		-3568 / 0	0.0	0.0 0.31 (1)			
M-L		0 / 0	-17.5	-17.5 0.05 (4)			
L-K		0 / 1180	-17.5	-17.5 0.22 (4)			
K-J		0 / 2554	-17.5	-17.5 0.65 (1)			
J-I		0 / 2554	-17.5	-17.5 0.65 (1)			
I-H		0 / 0	-369.3	-369.3 0.72 (1)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	13-8-8	-2716	-2716	---	FRONT	VERT	DEAD

TOTAL WEIGHT = 116 lb [M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 13-8-8
START SPAN CARRIED = 18-0-0
END DISTANCE = 18-4-0
END SPAN CARRIED = 18-0-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.73 (E-F:1), BC=0.72 (H-I:1), WB=0.95 (E-K:1), SS=0.70 (H-I:1)

DOL LUMBER=0.88 NAIL=0.88 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 3.000

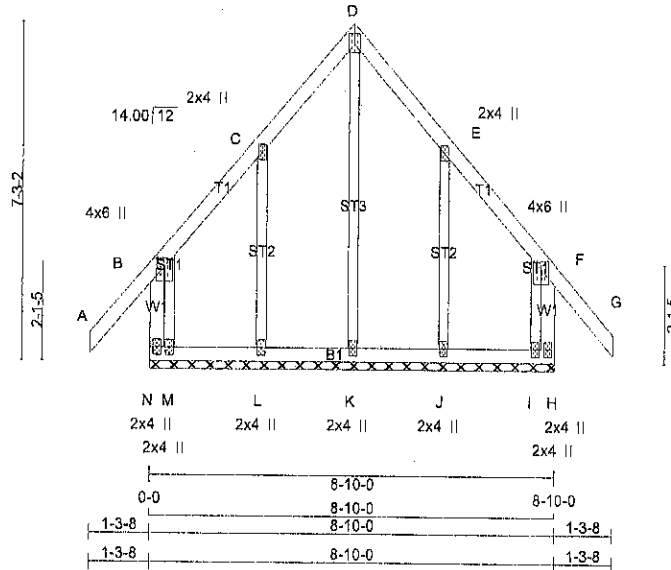


4-2073053 CONTINUED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	P02G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 11:57:43 2015 Page 1
ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-wdqVW5fzmyxrWPr_N7lcYdEM?oVWo28Ypt_k89yyAIG
3x5 II
Scale = 1:49.6



LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				TOTAL WEIGHT = 50 lb			
CHORDS	SIZE	LUMBER	DESCR.	[M]			
N - B	2x4	DRY	No.2	SPECIFIED LOADS:			
A - D	2x4	DRY	No.2	TOP CH. LL = 21.0 PSF			
D - G	2x4	DRY	No.2	DL = 3.0 PSF			
H - F	2x4	DRY	No.2	BOT CH. LL = 0.0 PSF			
N - H	2x4	DRY	No.2	DL = 7.0 PSF			
ALL WEBS 2x3 DRY No.2 SPF				TOTAL LOAD = 31.0 PSF			
ALL GABLE WEBS 2x3 DRY No.2 SPF				SPACING = 24.0 IN. C/C			
DRY: SEASONED LUMBER.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
GABLE STUDS SPACED AT 2-0-0 OC.				THIS DESIGN COMPLIES WITH:			
				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
				- CSA 086-09			
				- TPIC 2011			
				DESIGN ASSUMPTIONS			
				- OVERHANG NOT TO BE ALTERED OR CUT OFF.			
				(55% OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD			
				CSI: TC=0.10 (F-G:1), BC=0.02 (H-I:1), WB=0.12 (D-K:1), SSI=0.05 (A-B:1)			
				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10			
				COMPANION LIVE LOAD FACTOR = 0.50			
				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				NAIL VALUES			
				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
				MAX MIN MAX MIN MAX MIN			
				MT20 618 354 1667 822 2284 1656			
				PLATE PLACEMENT TOL. = 0.250 inches			
				PLATE ROTATION TOL. = 5.0 Deg.			
				JSI GRIP= 0.13 (E) (INPUT = 0.80)			
				JSI METAL= 0.04 (C) (INPUT = 1.00)			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TMVW+w	MT20	2.0	4.0		
D	TTVW+p	MT20	3.0	5.0	2.75	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	2.0	4.0		
I, J, K, L, M						
I	BMV1+w	MT20	2.0	4.0		
N	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
N-B	-172 / 0	0.0 0.0	0.02 (1)	7.81	K-D	-135 / 0	0.12 (1)
A-B	0 / 38	-70.5 -70.5	0.10 (1)	10.00	L-C	-150 / 0	0.05 (1)
B-C	-15 / 0	-70.5 -70.5	0.05 (1)	6.25	M-B	-21 / 0	0.00 (1)
C-D	-8 / 0	-70.5 -70.5	0.04 (1)	10.00	J-E	-150 / 0	0.05 (1)
D-E	-8 / 0	-70.5 -70.5	0.05 (1)	6.25	I-F	-21 / 0	0.00 (1)
E-F	-15 / 0	-70.5 -70.5	0.05 (1)	6.25			
F-G	0 / 38	-70.5 -70.5	0.10 (1)	10.00			
H-F	-172 / 0	0.0 0.0	0.02 (1)	7.81			
N-M	0 / 13	-17.5 -17.5	0.02 (1)	10.00			
M-L	0 / 8	-17.5 -17.5	0.01 (4)	10.00			
L-K	0 / 4	-17.5 -17.5	0.01 (4)	10.00			
K-J	0 / 4	-17.5 -17.5	0.01 (4)	10.00			
J-I	0 / 8	-17.5 -17.5	0.01 (4)	10.00			
I-H	0 / 13	-17.5 -17.5	0.02 (1)	10.00			

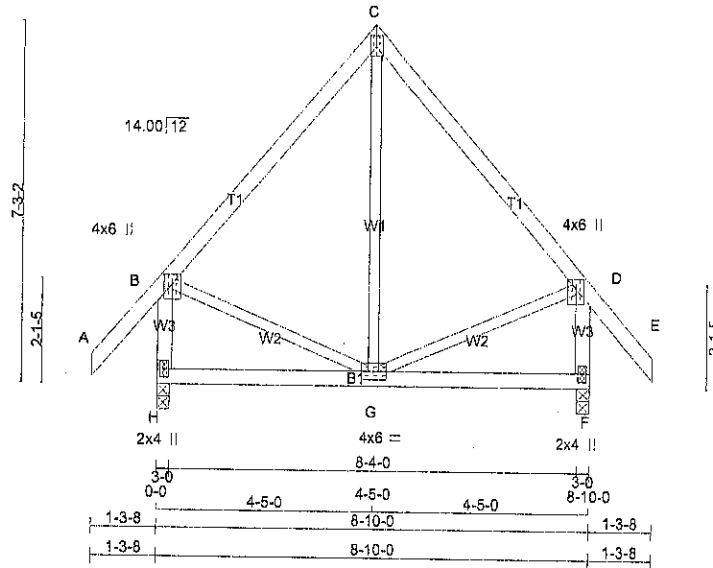


A-15073552

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	P02	3	1	TRUSS DESC.	

Alpine Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 11:57:42 2015 Page 1
ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-SRG8XleLCTq4DNreQgce3L52QbR0nM87J98RCjyA17
3x5 II Scale = 1:46.4



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTW+p	MT20	3.0	5.0	2.75	1.50
D	TMVW+p	MT20	4.0	6.0	2.25	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-I	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
H	487	0	487	0	3-0	1-8
F	487	0	487	0	3-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
H	341	244 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
F	341	244 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH
FR-TO		FROM TO	CSI (LC)		FR-TO		CSI (LC)
A-B	0 / 38	-70.5	-70.5	0.10 (1)	10.00	G-C	-7 / 71
B-C	-200 / 0	-70.5	-70.5	0.18 (1)	6.25	B-G	0 / 140
C-D	-200 / 0	-70.5	-70.5	0.18 (1)	6.25	G-D	0 / 140
D-E	0 / 38	-70.5	-70.5	0.10 (1)	10.00		
H-B	-458 / 0	0.0	0.0	0.05 (1)	7.81		
F-D	-458 / 0	0.0	0.0	0.05 (1)	7.81		
H-G	0 / 0	-17.5	-17.5	0.10 (4)	10.00		
G-F	0 / 0	-17.5	-17.5	0.10 (4)	10.00		

TOTAL WEIGHT = 3 X 47 = 142 lb

[MIF]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.18 (B-C:1), BC=0.10 (G-H:4),
WB=0.03 (D-G:1), SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 1.00)

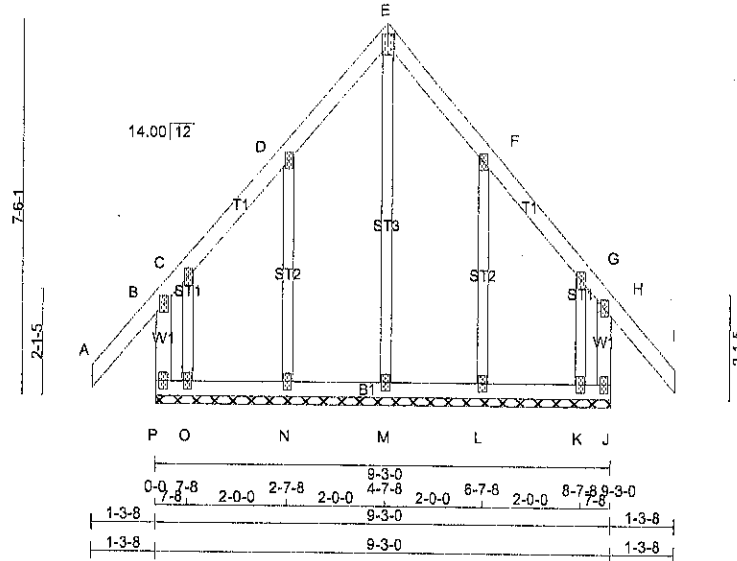


A-15073551

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	P01G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.820 S Apr 15 2015 Mitek Industries, Inc. Tue Jul 14 11:57:42 2015 Page 1
ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-SRG8XleLCTq4DNreQgce3L53cbSlnK17J98RCjyyA17
Scale = 1:46.3



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT	2-0-0	OC.		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
C, D, F, G				
C	TMV+w	MT20	2.0	4.0
E	TTW+p	MT20	3.0	5.0 2.75 1.50
H	TMV+p	MT20	2.0	4.0
J	BMV1+p	MT20	2.0	4.0
K, L, M, N, O				
K	BMV1+w	MT20	2.0	4.0
P	BMV1+p	MT20	2.0	4.0

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO					FR-TO		
A-B	0 / 38	-70.5	-70.5 0.10 (1)	10.00	M-E	-173 / 0	0.17 (1)
B-C	-40 / 0	-70.5	-70.5 0.07 (1)	6.25	N-D	-148 / 0	0.08 (1)
C-D	0 / 17	-70.5	-70.5 0.04 (1)	10.00	O-C	-14 / 0	0.00 (1)
D-E	0 / 17	-70.5	-70.5 0.04 (1)	10.00	L-F	-148 / 0	0.06 (1)
E-F	0 / 17	-70.5	-70.5 0.04 (1)	10.00	K-G	-14 / 0	0.00 (1)
F-G	0 / 17	-70.5	-70.5 0.04 (1)	10.00			
G-H	-40 / 0	-70.5	-70.5 0.07 (1)	6.25			
H-I	0 / 38	-70.5	-70.5 0.10 (1)	10.00			
P-B	-177 / 0	0.0	0.0 0.02 (1)	7.81			
J-H	-177 / 0	0.0	0.0 0.02 (1)	7.81			
P-O	-8 / 0	-17.5	-17.5 0.01 (4)	10.00			
O-N	-9 / 0	-17.5	-17.5 0.01 (4)	10.00			
N-M	-12 / 0	-17.5	-17.5 0.01 (4)	6.25			
M-L	-12 / 0	-17.5	-17.5 0.01 (4)	6.25			
L-K	-9 / 0	-17.5	-17.5 0.01 (4)	10.00			
K-J	-8 / 0	-17.5	-17.5 0.01 (4)	10.00			

TOTAL WEIGHT = 53 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS

8.4 P.S.F. RAIN LOAD EQUALS

21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (H-I:1), BC=0.01 (L-M:4), WB=0.17

(E-M:1), SSI=0.05 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)

JSI METAL= 0.06 (B) (INPUT = 1.00)

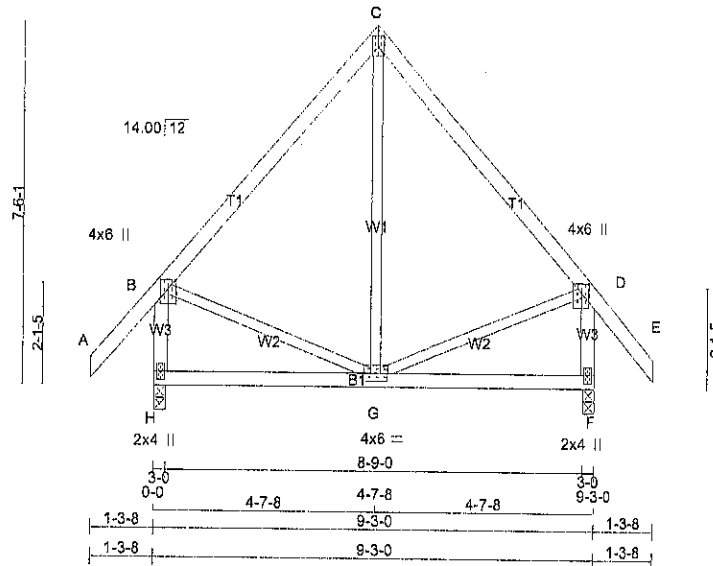


A-1073550

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	P01	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Tue Jul 14 11:57:41 2015 Page 1
ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b_FimKPejR9IDbDGStz4OX7YsQB5d2vsr4VOtgHyyA16
3x5 II
Scale: 1/4"=1'



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.25	2.00
C TTW+p	MT20	3.0	5.0	2.75	1.50
D TMVW+p	MT20	4.0	6.0	2.25	2.00
F BMV1+p	MT20	2.0	4.0		
G BMVWW-t	MT20	4.0	6.0		
H BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
H	506	0	506	0
F	506	0	506	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS	2ND CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM.LIVE
H	354	253 / 0	0 / 0
F	354	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO		
A-B	0 / 38	-70.5	-70.5	0.10 (1)	10.00	G-C	-4 / 76 0.03 (4)
B-C	-212 / 0	-70.5	-70.5	0.19 (1)	8.25	B-G	0 / 147 0.03 (1)
C-D	-212 / 0	-70.5	-70.5	0.19 (1)	6.25	G-D	0 / 147 0.03 (1)
D-E	0 / 38	-70.5	-70.5	0.10 (1)	10.00		
H-B	-475 / 0	0.0	0.0	0.06 (1)	7.81		
F-D	-475 / 0	0.0	0.0	0.06 (1)	7.81		
H-G	0 / 0	-17.5	-17.5	0.11 (4)	10.00		
G-F	0 / 0	-17.5	-17.5	0.11 (4)	10.00		

TOTAL WEIGHT = 3 X 49 = 147 lb

(M)(F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.19 (B-C:1), BC=0.11 (G-H:4),
WB=0.03 (D-G:1), SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.09 (H) (INPUT = 1.00)

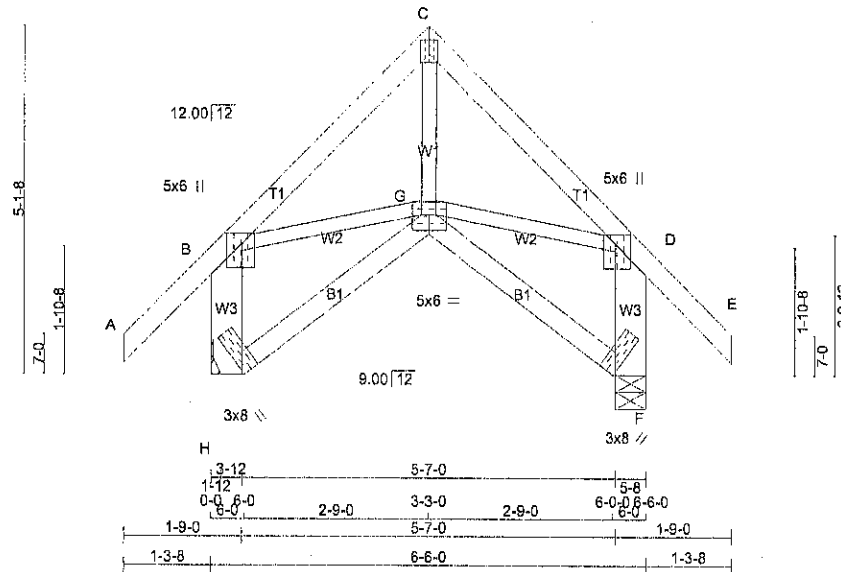


A-15073549

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	H07	3	1	TRUSS DESC.	

Alpha Roof Truss, Maple

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ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b_FimKPeJR9iDbdGStz4OX7YuvB6R2vmr4VQtgHyyA18
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTW+p	MT20	3.0	4.0	Edge	
D	TMVW+p	MT20	5.0	6.0	1.75	2.75
F	BVM1+H	MT20	3.0	8.0	Edge	0.50
G	BBWW+p	MT20	5.0	6.0	2.75	3.00
H	BVM1+H	MT20	3.0	8.0	Edge	0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	384	0	384	0
F	384	0	384	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	268	195 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H		268	195 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F
BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM TO			FR-TO			
A-B	0 / 35	-70.5 -70.5	0.10 (1)	10.00	G-C	0 / 118	0.03 (4)	
B-C	-246 / 0	-70.5 -70.5	0.10 (1)	6.25	B-G	0 / 176	0.04 (1)	
C-D	-246 / 0	-70.5 -70.5	0.10 (1)	6.25	G-D	0 / 176	0.04 (1)	
D-E	0 / 35	-70.5 -70.5	0.10 (1)	10.00				
H-B	-356 / 0	0.0 0.0	0.03 (1)	7.81				
F-D	-356 / 0	0.0 0.0	0.03 (1)	7.81				
H-G	0 / 0	-17.5 -17.5	0.05 (4)	10.00				
G-F	0 / 0	-17.5 -17.5	0.05 (4)	10.00				

TOTAL WEIGHT = 3 X 37 = 111 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.10 (D-E:1), BC=0.05 (G-H:4), WB=0.04 (B-G:1), SSI=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667

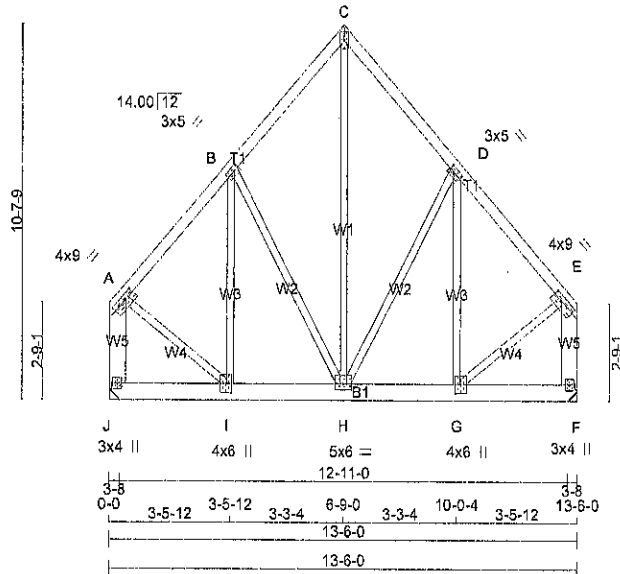
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (C) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)



A-15073548



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW-I	MT20	4.0	8.0	1.50	3.50
B - TMWW-I	MT20	3.0	6.0	1.50	1.75
C - TTW+p	MT20	3.0	6.0		
D - TMWW-I	MT20	3.0	6.0	1.50	1.75
E - TMVW-I	MT20	4.0	9.0	1.50	3.60
F - BMV1+p	MT20	3.0	4.0		
G - BMWW-I	MT20	4.0	6.0	3.00	1.75
H - BMWW-I	MT20	6.0	6.0	2.00	3.00
I - BMWW-I	MT20	4.0	6.0	3.00	1.75
J - BMV1+p	MT20	3.0	4.0		

NOTES: (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
J	2313	0	2313	0	0	HANGER BY OTHERS
F	2313	0	2313	0	0	MIN. SEAT SIZE: 2-8 HANGER BY OTHERS MIN. SEAT SIZE: 2-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	1630	1104 / 0	0 / 0	0 / 0	0 / 0	526 / 0	0 / 0
F	1630	1104 / 0	0 / 0	0 / 0	0 / 0	526 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CS (LC)		UNBRAC LENGTH		MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
	FR-TO	FROM TO	FR-TO	FROM TO	FR-TO	FROM TO	FR-TO	FROM TO		FR-TO	FROM TO	FR-TO	FROM TO
A-B	-1605 / 0		-70.5	-70.5	0.16 (1)	5.18	H-C	0 / 1651	0.41 (1)				
B-C	-1226 / 0		-70.5	-70.5	0.15 (1)	5.62	H-D	-446 / 0	0.44 (1)				
C-D	-1226 / 0		-70.5	-70.5	0.15 (1)	5.62	G-D	0 / 352	0.09 (1)				
D-E	-1505 / 0		-70.5	-70.5	0.16 (1)	5.18	B-H	-446 / 0	0.44 (1)				
J-A	-1911 / 0		0.0	0.0	0.19 (1)	7.23	I-B	0 / 352	0.09 (1)				
F-E	-1911 / 0		0.0	0.0	0.19 (1)	7.23	A-I	0 / 1188	0.29 (1)				
J-I	0 / 0		-272.2	-272.2	0.22 (1)	10.00	G-E	0 / 1188	0.29 (1)				
I-H	0 / 990		-272.2	-272.2	0.31 (1)	10.00							
H-G	0 / 990		-272.2	-272.2	0.31 (1)	10.00							
G-F	0 / 0		-272.2	-272.2	0.22 (1)	10.00							

TOTAL WEIGHT = 92 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CSldGirder
 START DISTANCE = 0-0
 START SPAN CARRIED = 13-7-0
 END DISTANCE = 13-6-0
 END SPAN CARRIED = 13-7-0
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 - ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(56 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.45")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL) = L/360 (0.45")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.19 (A-J:1), BC=0.31 (G-H:1), WB=0.44 (D-H:1), SSI=0.36 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
 JSI METAL= 0.33 (H) (INPUT = 1.00)

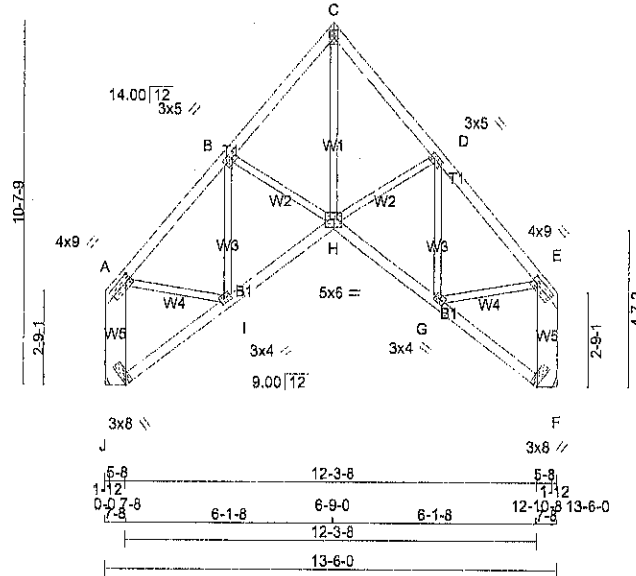


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Alpa Roof Truss, Maple

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ID:hdKFA8XS_E1ZiH0GL9ahDNzQR2b-1sa0VjcSVySVMv63IY2wSITYFOP8azXYdBvnbOyyAlA
3x5 II
Scale = 1:67.7



TOTAL WEIGHT = 3 X 78 = 234 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x8	DRY	No.2	SPF
F - E	2x8	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW-I	MT20	4.0	9.0	2.00	2.25
B - TMWW-I	MT20	3.0	5.0	1.50	1.75
C - TTW+p	MT20	3.0	5.0	2.75	1.50
D - TMWW-I	MT20	3.0	5.0	1.50	1.75
E - TMVW-I	MT20	4.0	9.0	2.00	2.25
F - BVM1-I	MT20	3.0	8.0	Edge	0.25
G - BMWW-I	MT20	3.0	4.0	1.50	1.50
H - BBWWW-p	MT20	5.0	6.0	2.75	3.00
I - BMWW-I	MT20	3.0	4.0	1.50	1.50
J - BVM1-I	MT20	3.0	8.0	Edge	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
J	594	0	594	0	0	HANGER BY OTHERS
F	594	0	594	0	0	MIN. SEAT SIZE: 1-8 HANGER BY OTHERS

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	418	283 / 0	0 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
F	418	283 / 0	0 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FACTORED	FORCE	VERT. LOAD	LC1 MAX	MAX	MEMB.	MAX. FACTORED	FORCE
FR-TO		(LBS)	(PLF)	CSI (LC)	UNBRAC LENGTH	FR-TO		(LBS)
A-B	-580 / 0	-70.5	-70.5	0.11 (1)	6.25	H-C	0 / 870	0.15 (1)
B-C	-570 / 0	-70.5	-70.5	0.10 (1)	6.25	I-B	-231 / 0	0.08 (1)
C-D	-570 / 0	-70.5	-70.5	0.10 (1)	6.25	A-I	0 / 390	0.09 (1)
D-E	-580 / 0	-70.5	-70.5	0.11 (1)	6.25	B-H	-41 / 0	0.01 (1)
J-A	-568 / 0	0.0	0.0	0.06 (1)	7.81	G-D	-231 / 0	0.08 (1)
F-E	-568 / 0	0.0	0.0	0.06 (1)	7.81	H-D	-41 / 0	0.01 (1)
						G-E	0 / 390	0.08 (1)
J-I	0 / 4	-17.5	-17.5	0.06 (4)	10.00			
I-H	0 / 484	-17.5	-17.5	0.11 (1)	10.00			
H-G	0 / 484	-17.5	-17.5	0.11 (1)	10.00			
G-F	0 / 4	-17.5	-17.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.45")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.45")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.11 (D-E:1), BC=0.11 (H-I:1), WB=0.15 (C-H:1), SSI=0.08 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618	364 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (I) (INPUT = 0.80)
JSI METAL= 0.17 (G) (INPUT = 1.00)



A-15073546

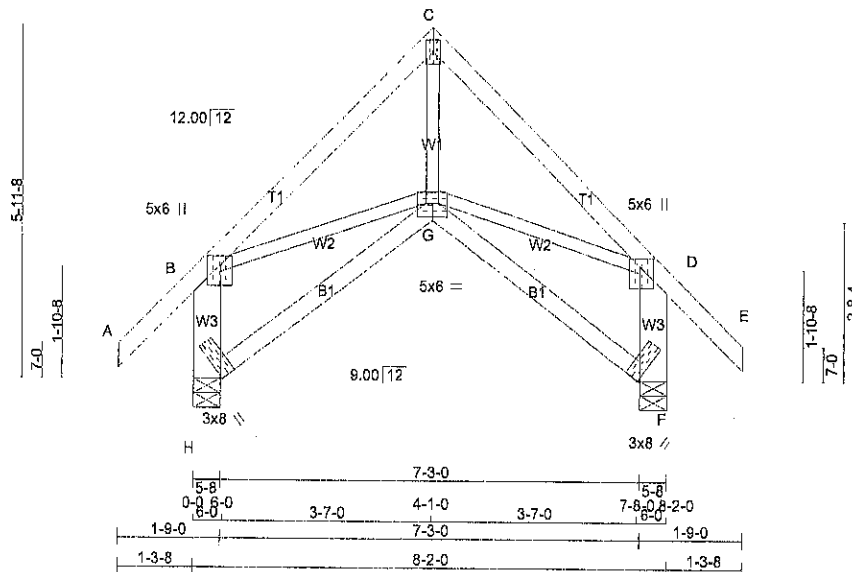
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	H05	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-Zg0diNbq8EKekIX1BqXhvVwMq_4FrZiPOXAD3yyyAIE

Scale = 1:39.2



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTW+p	MT20	3.0	5.0		
D	TMVW+p	MT20	5.0	6.0	1.75	2.75
F	BVM1+H	MT20	3.0	6.0	Edge	0.50
G	BBWW+p	MT20	5.0	6.0	2.75	3.00
H	BVM1+H	MT20	3.0	6.0	Edge	0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	457	0	457	0
F	457	0	457	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	320	230 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
F	320	230 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F
BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)
BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED			MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 35	-70.5	-70.5	0.10 (1)	10.00	G-C	0 / 221 0.05 (1)
B-C	-360 / 0	-70.5	-70.5	0.15 (1)	6.25	B-G	0 / 265 0.06 (1)
C-D	-360 / 0	-70.5	-70.5	0.15 (1)	6.25	G-D	0 / 265 0.06 (1)
D-E	0 / 35	-70.5	-70.5	0.10 (1)	10.00		
H-B	-422 / 0	0.0	0.0	0.03 (1)	7.81		
F-D	-422 / 0	0.0	0.0	0.03 (1)	7.81		
H-G	0 / 0	-17.5	-17.5	0.09 (4)	10.00		
G-F	0 / 0	-17.5	-17.5	0.09 (4)	10.00		

TOTAL WEIGHT = 3 X 44 = 131 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/899 (0.02")

CSI: TC=0.15 (C-D:1), BC=0.09 (F-G:4),
WB=0.06 (B-G:1), SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

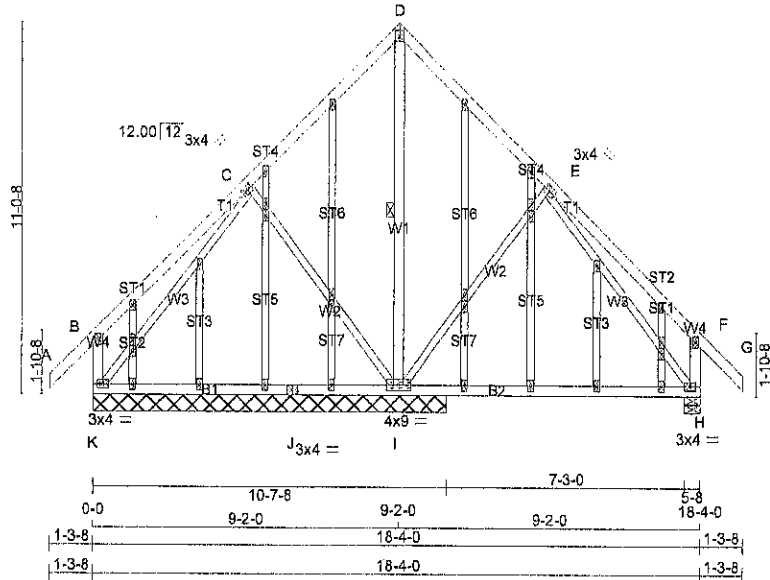
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



A-15073545



LUMBER

CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
K - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
I - D	2x4	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2'-0" O.C.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0	Edge	
E	TMWW-t	MT20	3.0	4.0	1.50	1.75
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0		
I	BMVWW1-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	4.0		
K	BMVW1-t	MT20	3.0	4.0		
L, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z, AA, AB, AB, AC, AD, AE, AF, AG, AG						
L NP+w	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
I	926	0	10'-7-8 (6'-0"-1'-8)	1'-8
K	441	0	10'-7-8 (6'-0"-1'-8)	1'-8
H	441	0	5'-8	1'-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	655	432 / 0	0 / 0	0 / 0	0 / 0	222 / 0	0 / 0
K	308	227 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
H	308	227 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-I.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC LENGTH	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC LENGTH
FR-TO				FR-TO			
A-B	0 / 35	-70.5	-70.5 0.10 (1)	I-D	-315 / 0	0.19 (1)	10.00
B-C	0 / 29	-70.5	-70.5 0.25 (1)	I-E	-258 / 0	0.30 (1)	10.00
C-D	0 / 20	-70.5	-70.5 0.25 (1)	C-I	-258 / 0	0.30 (1)	10.00
D-E	0 / 20	-70.5	-70.5 0.25 (1)	K-C	-204 / 0	0.23 (1)	10.00
E-F	0 / 29	-70.5	-70.5 0.25 (1)	E-H	-204 / 0	0.23 (1)	10.00
F-G	0 / 35	-70.5	-70.5 0.10 (1)				
K-B	-218 / 0	0.0	0.0 0.02 (1)				
H-F	-218 / 0	0.0	0.0 0.02 (1)				
K-J	0 / 122	-17.5	-17.5 0.43 (4)				
J-I	0 / 122	-17.5	-17.5 0.43 (4)				
I-H	0 / 122	-17.5	-17.5 0.43 (4)				

TOTAL WEIGHT = 126 lb (M)(F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/878 (0.13")

CSI: TC=0.25 (E-F:1), BC=0.43 (I-K:4), WB=0.30 (E-I:1), SSI=0.13 (H-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

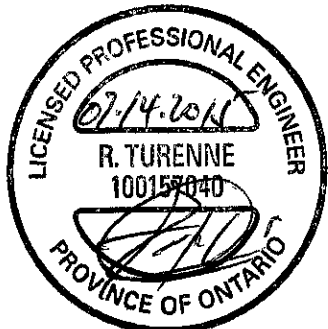
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.19 (J) (INPUT = 1.00)

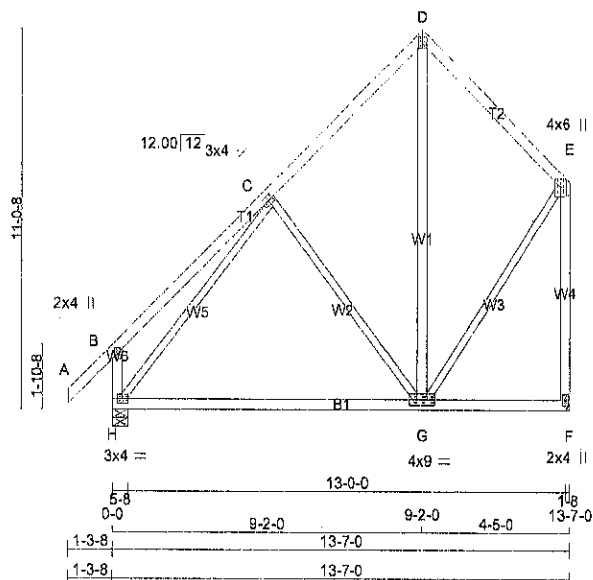


A-15073544

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	H04S	6	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:67.2



TOTAL WEIGHT = 6 X 79 = 471 lb (M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4 DRY	No.2	SPF	
D - E	2x4 DRY	No.2	SPF	
H - B	2x4 DRY	No.2	SPF	
F - E	2x4 DRY	No.2	SPF	
H - F	2x4 DRY	No.2	SPF	
ALL WEBS	2x3 DRY	No.2	SPF	
EXCEPT				
G - D	2x4 DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW+t	MT20	3.0	4.0	1.60	1.75
D	TTW+p	MT20	3.0	4.0	Edge	
E	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWV+t	MT20	4.0	6.0		
H	BMVW1+t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23, 13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	597	0	597	0
H	696	0	696	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
F	421	285 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0	
H	488	344 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 35	-70.5	-70.5 0.10 (1)	10.00	C-G	-213 / 0	0.25 (1)
B-C	0 / 28	-70.5	-70.5 0.25 (1)	10.00	G-D	0 / 101	0.03 (4)
C-D	-281 / 0	-70.5	-70.5 0.19 (1)	6.25	H-C	-514 / 0	0.58 (1)
D-E	-253 / 0	-70.5	-70.5 0.18 (1)	6.25	G-E	0 / 311	0.07 (1)
H-B	-219 / 0	0.0	0.0 0.02 (1)	7.81			
F-E	-589 / 0	0.0	0.0 0.58 (1)	7.81			
H-G	0 / 308	-17.5	-17.5 0.34 (4)	10.00			
G-F	0 / 0	-17.5	-17.5 0.32 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL)= L/915 (0.18")

CSI: TC=0.58 (E-F:1), BC=0.34 (G-H:4),
WB=0.58 (C-H:1), SSI=0.13 (G-H:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

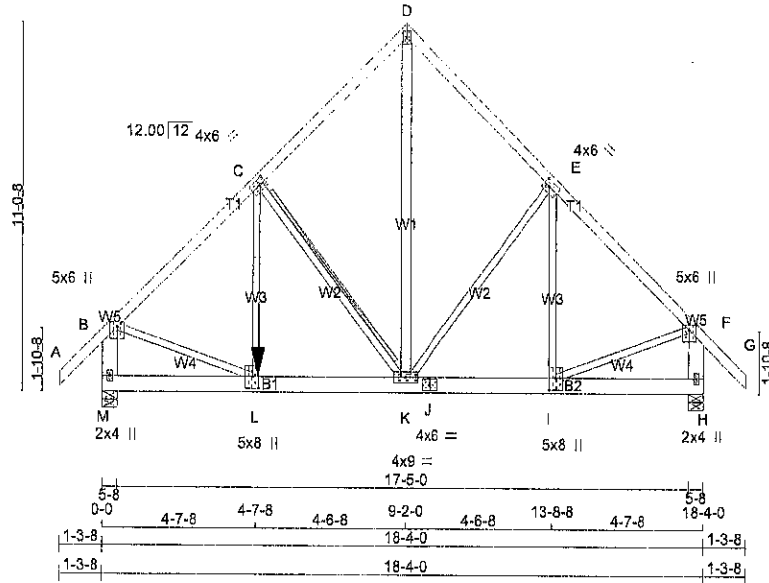


A-1073543

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOBS DESC.	DRWG NO.
253402	H04A	2	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:hdKFA8XS_E1ZiH0GL9ahDNzQR2b-dHvtHhaacd4wVVSNU4PVDq4r1AMTNXZ6xDh7_3yyAIC
3x5 II
Scale = 1:69.2



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	TOP
D-G	12	TOP
M-B	12	TOP
H-F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M-J	12	SIDE(126.4)
J-H	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
C-L	6	SIDE(546.8)
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	3656	0	3656	0	5-8
H	1636	0	1636	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
M	2574	1756 / 0	0 / 0
H	1150	792 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT C-K
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 35	K-D	0 / 1716
B-C	-3043 / 0	K-E	-109 / 0
C-D	-1413 / 0	I-E	-245 / 0
D-E	-1415 / 0	C-K	-1981 / 0
E-F	-1464 / 0	L-C	0 / 2380
F-G	0 / 35	B-L	0 / 2271
M-B	-3070 / 0	I-F	0 / 1096
H-F	-1597 / 0		
M-L	0 / 0		
L-K	0 / 2169		
K-J	0 / 1047		
J-I	0 / 1047		
I-H	0 / 0		

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	4-8-4	-2313	-2313	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 4 X 112 = 449 lb
(M)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. G/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 13-6-0
END DISTANCE = 4-8-4
END SPAN CARRIED = 13-6-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.19 (B-C:1), BC=0.23 (L-M:1), WB=0.55 (C-K:1), SSI=0.22 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 30 Deg.

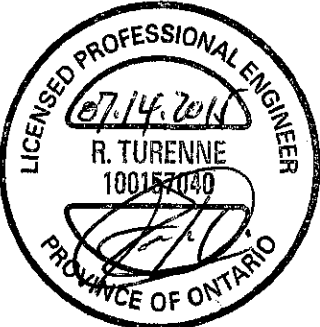


CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	H04A	2	2	TRUSS DESC.	

Alpa Roof Truss, Maple

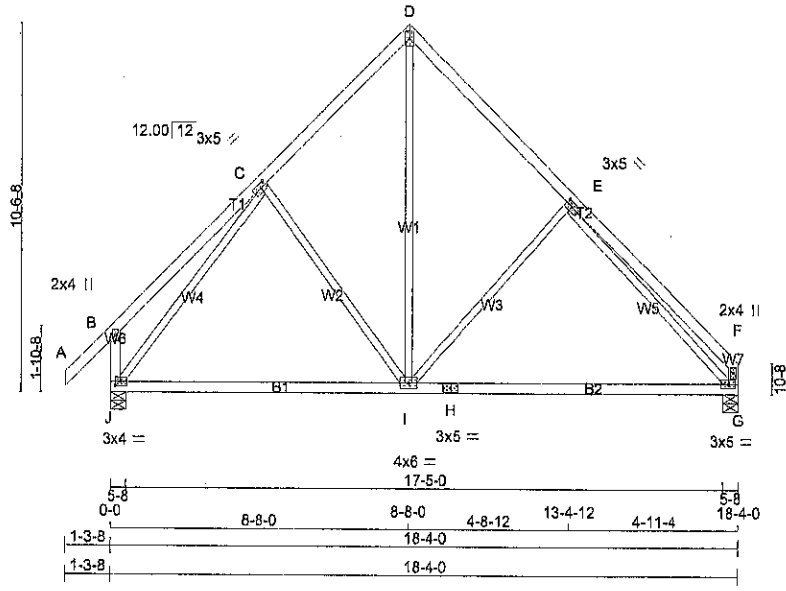
Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Tue Jul 14 11:57:36 2015 Page 2
ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-dHvHhaacd4wVSNU4PVDq4r1IAMTNXZ6xDh7_3yyA/C

PLATES (table is in inches) <table> <tr> <th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr> <tr> <td>B</td><td>TMVW+p</td><td>MT20</td><td>5.0</td><td>6.0</td><td>1.75</td><td>2.75</td></tr> <tr> <td>C</td><td>TMWW-I</td><td>MT20</td><td>4.0</td><td>6.0</td><td>2.00</td><td>2.25</td></tr> <tr> <td>D</td><td>TTW+p</td><td>MT20</td><td>3.0</td><td>5.0</td><td></td><td></td></tr> <tr> <td>E</td><td>TMVW-I</td><td>MT20</td><td>4.0</td><td>6.0</td><td>2.00</td><td>2.25</td></tr> <tr> <td>F</td><td>TMVW+p</td><td>MT20</td><td>5.0</td><td>6.0</td><td>1.75</td><td>2.75</td></tr> <tr> <td>H</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr> <tr> <td>I</td><td>BMWW+I</td><td>MT20</td><td>5.0</td><td>8.0</td><td>4.25</td><td>2.25</td></tr> <tr> <td>J</td><td>BS-I</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr> <tr> <td>K</td><td>BMWWW-I</td><td>MT20</td><td>4.0</td><td>9.0</td><td></td><td></td></tr> <tr> <td>L</td><td>BMWW+I</td><td>MT20</td><td>5.0</td><td>8.0</td><td>4.25</td><td>2.25</td></tr> <tr> <td>M</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr> </table>						JT	TYPE	PLATES	W	LEN	Y	X	B	TMVW+p	MT20	5.0	6.0	1.75	2.75	C	TMWW-I	MT20	4.0	6.0	2.00	2.25	D	TTW+p	MT20	3.0	5.0			E	TMVW-I	MT20	4.0	6.0	2.00	2.25	F	TMVW+p	MT20	5.0	6.0	1.75	2.75	H	BMV1+p	MT20	2.0	4.0			I	BMWW+I	MT20	5.0	8.0	4.25	2.25	J	BS-I	MT20	4.0	6.0			K	BMWWW-I	MT20	4.0	9.0			L	BMWW+I	MT20	5.0	8.0	4.25	2.25	M	BMV1+p	MT20	2.0	4.0			JSI GRIP= 0.88 (L) (INPUT = 0.80) JSI METAL= 0.29 (B) (INPUT = 1.00)
JT	TYPE	PLATES	W	LEN	Y	X																																																																																				
B	TMVW+p	MT20	5.0	6.0	1.75	2.75																																																																																				
C	TMWW-I	MT20	4.0	6.0	2.00	2.25																																																																																				
D	TTW+p	MT20	3.0	5.0																																																																																						
E	TMVW-I	MT20	4.0	6.0	2.00	2.25																																																																																				
F	TMVW+p	MT20	5.0	6.0	1.75	2.75																																																																																				
H	BMV1+p	MT20	2.0	4.0																																																																																						
I	BMWW+I	MT20	5.0	8.0	4.25	2.25																																																																																				
J	BS-I	MT20	4.0	6.0																																																																																						
K	BMWWW-I	MT20	4.0	9.0																																																																																						
L	BMWW+I	MT20	5.0	8.0	4.25	2.25																																																																																				
M	BMV1+p	MT20	2.0	4.0																																																																																						
HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2313.5 lbs FACTORED DOWN AT 4-B-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.																																																																																										
NOTES- (1) 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.																																																																																										
																																																																																										
						A-ND 73542(2)																																																																																				

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	H03E	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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 ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-J8LIJ_in53FeEQWpPXF0HGUISb0D_5RDB05oFyy98P
 3x5 II
 Scale = 1:66.2



TOTAL WEIGHT = 4 X 86 = 343 lb (MIF)

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C	TMWW-t	MT20	3.0	5.0	1.50 2.00
D	TTW+p	MT20	3.0	5.0	
E	TMWW-t	MT20	3.0	5.0	1.50 2.25
F	TMV+p	MT20	2.0	4.0	
G	BMWW-t	MT20	3.0	5.0	
H	BS-t	MT20	3.0	5.0	
I	BMWW-t	MT20	4.0	6.0	
J	BMWW-t	MT20	3.0	4.0	1.50 1.75

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
J	904	0	904	0	0	5-8	1-8	5-8	1-8
G	806	0	806	0	0	5-8	1-8	5-8	1-8

UNFACTORED REACTIONS		1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
J	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0		
G	568	385 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING		TOTAL LOAD CASES: (4)		CHORDS		WEBS	
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED CSI (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 35	-70.5	-70.5 0.10 (1)	C-I	-150 / 2		0.18 (1)
B-C	0 / 26	-70.5	-70.5 0.22 (1)	I-D	0 / 50B		0.11 (1)
C-D	-561 / 0	-70.5	-70.5 0.17 (1)	J-E	-256 / 0		0.26 (1)
D-E	-565 / 0	-70.5	-70.5 0.21 (1)	J-C	-791 / 0		0.79 (1)
E-F	0 / 29	-70.5	-70.5 0.27 (1)	E-G	-824 / 0		0.80 (1)
J-B	-213 / 0	0.0	0.0 0.02 (1)				
G-F	-128 / 0	0.0	0.0 0.01 (1)				
J-I	0 / 470	-17.5	-17.5 0.48 (4)				
I-H	0 / 552	-17.5	-17.5 0.47 (4)				
H-G	0 / 552	-17.5	-17.5 0.47 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.I. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.61")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.27 (E-F:1), BC=0.47 (G-I:4), WB=0.80 (E-G:1), SSI=0.14 (G-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.80

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
 JSI METAL= 0.33 (H) (INPUT = 1.00)

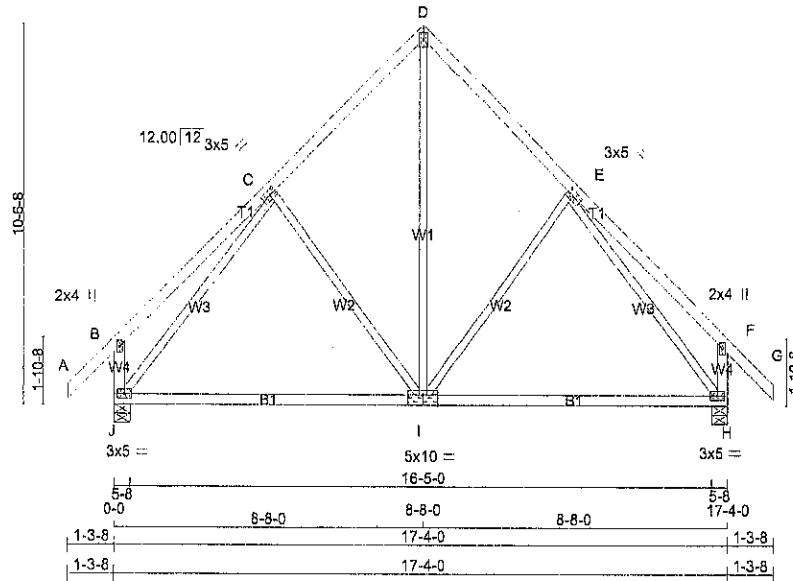


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	H03	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-95LV3LZyrJy3tloIW_HslrYnzTe1TziZxZSdyAIE
Scale: 3/16"=1'



TOTAL WEIGHT = 4 X 86 = 346 lb
(M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C	TMWW-t	MT20	3.0	5.0	1.50 2.00
D	TTW+p	MT20	3.0	5.0	
E	TMWW-t	MT20	3.0	5.0	1.50 2.00
F	TMV+p	MT20	2.0	4.0	
H	BMVW1-t	MT20	3.0	5.0	
I	BSWWW-t	MT20	5.0	10.0	Edge 5.00
J	BMVW1-t	MT20	3.0	5.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
J	861	0	861	0	0	5-8	1-8		
H	861	0	861	0	0	5-8	1-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	604	422 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	604	422 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-70.5 -70.5	0.10 (1)	I-D	0 / 443	0.10 (1)	
B-C	0 / 26	-70.5 -70.5	0.22 (1)	I-E	-159 / 0	0.17 (1)	
C-D	-508 / 0	-70.5 -70.5	0.17 (1)	C-I	-159 / 0	0.17 (1)	
D-E	-508 / 0	-70.5 -70.5	0.22 (1)	J-C	-733 / 0	0.73 (1)	
E-F	0 / 26	-70.5 -70.5	0.22 (1)	E-H	-733 / 0	0.73 (1)	
F-G	0 / 35	-70.5 -70.5	0.10 (1)				
J-B	-213 / 0	0.0 0.0	0.02 (1)				
H-F	-213 / 0	0.0 0.0	0.02 (1)				
J-I	0 / 435	-17.5 -17.5	0.41 (4)				
I-H	0 / 435	-17.5 -17.5	0.41 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL)= L/999 (0.12")

CSI: TC=0.22 (E-F:1), BC=0.41 (I-J:4), WB=0.73 (E-H:1), SSI=0.13 (H-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR= 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.31 (I) (INPUT = 1.00)



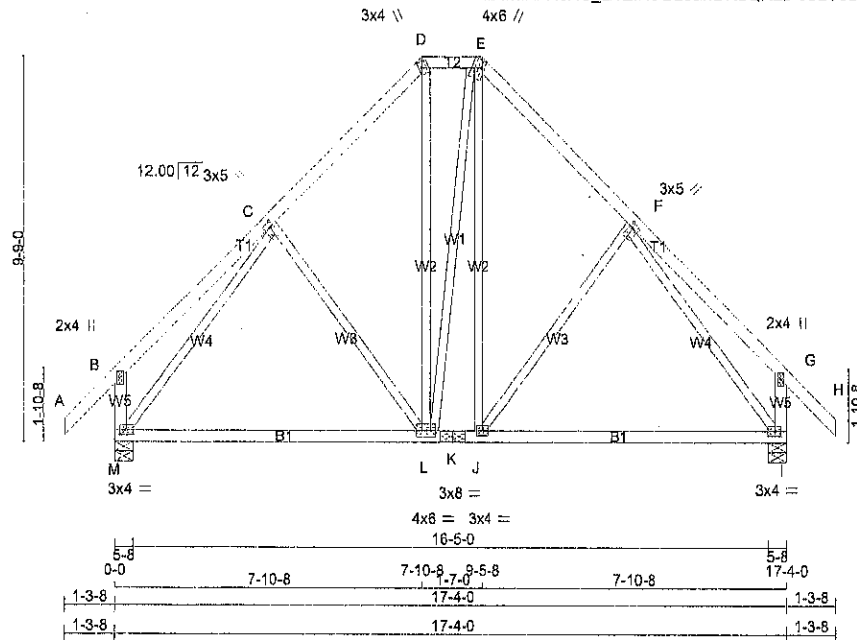
A-15073540

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	H02	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:hdKFA8XS_E1ZiH0GL9ahDNzQR2b-95LV3LZyrJy3tlo1W_Hsls9n7Je3YziZxZSdyAIE
Scale = 1:58.7



TOTAL WEIGHT = 95 lb
[M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
E - H	2x4 DRY	No.2	SPF		
M - B	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		
M - K	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW+H	MT20	3.0	5.0	1.50	1.00
D	TTVW+m	MT20	3.0	4.0	2.00	1.00
E	TTVW+m	MT20	4.0	6.0	2.25	0.75
F	TMVW+H	MT20	3.0	5.0	1.50	1.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	3.0	4.0	1.50	1.75
J	BMVW-t	MT20	3.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	3.0	4.0	1.50	1.75

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	861	0	861	0	0	5-8	1-8		
I	861	0	861	0	0	5-8	1-8		

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	604	422 / 0	0 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
I	604	422 / 0	0 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 35	-70.5	-70.5	0.10 (1)	10.00	C-L	-126 / 4 0.11 (1)
B-C	0 / 23	-70.5	-70.5	0.18 (1)	10.00	L-D	0 / 196 0.04 (1)
C-D	-530 / 0	-70.5	-70.5	0.14 (1)	6.25	L-E	0 / 6 0.00 (1)
D-E	-359 / 0	-70.5	-70.5	0.02 (1)	6.25	J-E	0 / 189 0.04 (1)
E-F	-529 / 0	-70.5	-70.5	0.14 (1)	6.25	J-F	-126 / 4 0.11 (1)
F-G	0 / 23	-70.5	-70.5	0.18 (1)	10.00	M-C	-742 / 0 0.60 (1)
G-H	0 / 35	-70.5	-70.5	0.10 (1)	10.00	F-I	-740 / 0 0.60 (1)
M-B	-202 / 0	0.0	0.0	0.02 (1)	7.81		
I-G	-202 / 0	0.0	0.0	0.02 (1)	7.81		
M-L	0 / 433	-17.5	-17.5	0.28 (4)	10.00		
L-K	0 / 358	-17.5	-17.5	0.29 (4)	10.00		
K-J	0 / 358	-17.5	-17.5	0.29 (4)	10.00		
J-I	0 / 432	-17.5	-17.5	0.28 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.18 (F-G:1), BC=0.29 (J-L:4), WB=0.60 (C-M:1), SSI=0.11 (L-M:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.47 (K) (INPUT = 1.00)



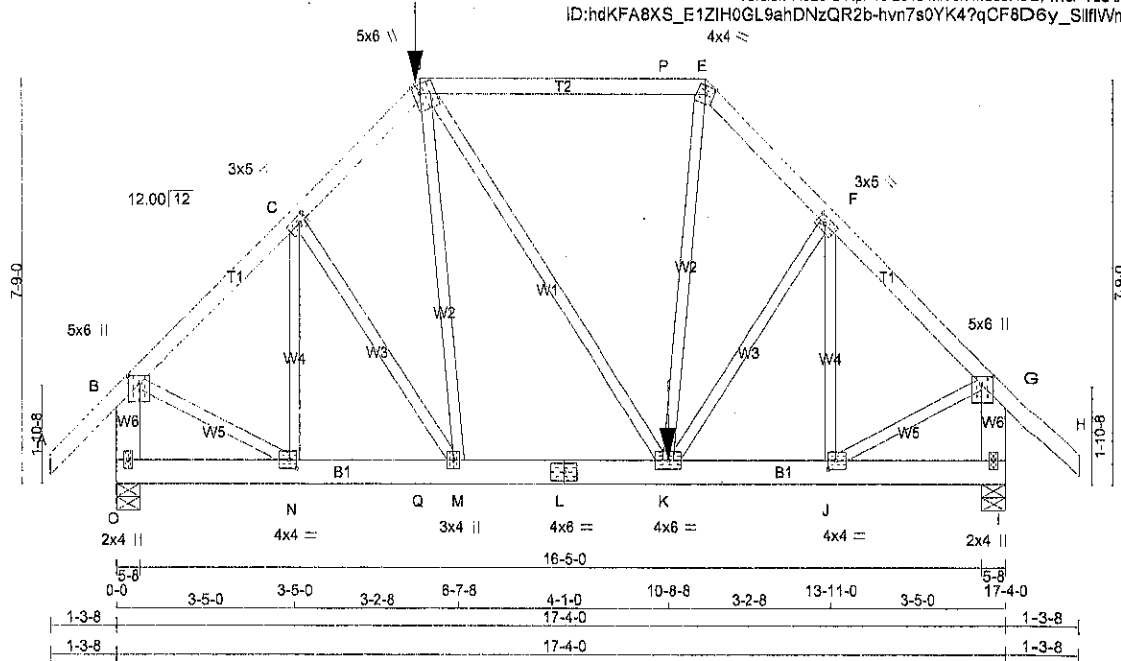
A-15073539

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253402	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 Mitek Industries, Inc. Tue Jul 14 11:57:34 2015 Page 1

ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-hvn7s0YK4?qCf8D6y_SlIfWmNh9vImpTwC0wByyAIf
Scale = 1:44.2



TOTAL WEIGHT = 106 lb
[M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
E - H	2x4 DRY	No.2	SPF		
O - B	2x6 DRY	No.2	SPF		
I - G	2x6 DRY	No.2	SPF		
O - L	2x6 DRY	No.2	SPF		
L - I	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
DRY; SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75	
C	TMWW-t	MT20	3.0	5.0	1.50	2.00	
D	TTWW+m	MT20	5.0	6.0	2.00	1.50	
E	TTW-m	MT20	4.0	4.0	Edge		
F	TMWW-t	MT20	3.0	5.0	1.50	2.00	
G	TMVW+p	MT20	5.0	6.0	1.75	2.75	
I	BMV1+p	MT20	2.0	4.0			
J	BMWW-t	MT20	4.0	4.0	2.00	1.75	
K	BMWW-t	MT20	4.0	6.0			
L	BS-t	MT20	4.0	6.0			
M	BMWW-t	MT20	3.0	4.0			
N	BMWW-t	MT20	4.0	4.0	2.00	1.75	
O	BMV1+p	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD, AND 548.3 lbs FACTORED DOWN AT 10-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
O	1568	0	1568	0
I	1515	0	1515	0

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
O	1106	744 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0	
I	1064	739 / 0	0 / 0	0 / 0	0 / 0	326 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING	TOTAL LOAD CASES: (4)								
CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A-B	0 / 35	-70.5	-70.5	0.11 (1)	10.00	N-C	-281 / 8	0.11 (1)	
B-C	-1292 / 0	-70.5	-70.5	0.14 (1)	5.51	C-M	-28 / 8	0.02 (4)	
C-D	-1343 / 0	-70.5	-70.5	0.14 (1)	5.43	D-M	0 / 170	0.06 (4)	
D-P	-986 / 0	-138.7	-138.7	0.86 (1)	4.35	D-K	0 / 79	0.02 (1)	
P-E	-986 / 0	-70.5	-70.5	0.86 (1)	4.35	K-E	0 / 545	0.13 (1)	
E-F	-1342 / 0	-70.5	-70.5	0.13 (1)	5.44	K-F	0 / 46	0.01 (1)	
F-G	-1264 / 0	-70.5	-70.5	0.14 (1)	5.56	J-F	-330 / 0	0.13 (1)	
G-H	0 / 35	-70.5	-70.5	0.11 (1)	10.00	B-N	0 / 1003	0.25 (1)	
O-B	-1511 / 0	0.0	0.0	0.12 (1)	7.81	J-G	0 / 981	0.24 (1)	
I-G	-1483 / 0	0.0	0.0	0.11 (1)	7.81				
O-N	0 / 0	-34.5	-34.5	0.05 (4)	10.00				
N-Q	0 / 924	-34.5	-34.5	0.14 (1)	10.00				
Q-M	0 / 924	-34.5	-34.5	0.14 (1)	10.00				
M-L	0 / 943	-34.5	-34.5	0.16 (1)	10.00				
L-K	0 / 943	-34.5	-34.5	0.16 (1)	10.00				
K-J	0 / 904	-17.5	-17.5	0.13 (1)	10.00				
J-I	0 / 0	-17.5	-17.5	0.03 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-302	-302	--	--	FRONT	VERT	TOTAL
K	10-8-8	-549	-549	--	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-8-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.88 (D-E:1), BC=0.16 (K-M:1),
WB=0.25 (B-N:1), SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

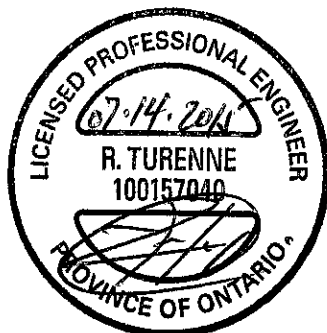
COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

A-15073538

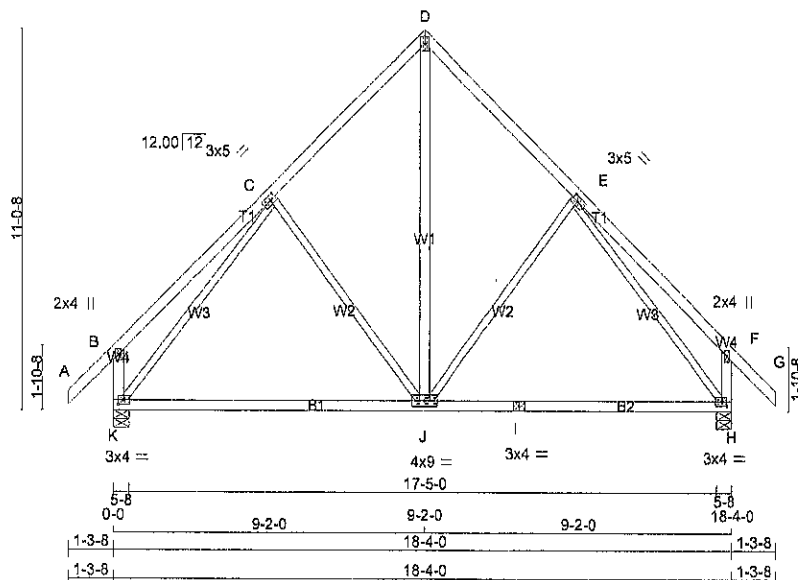
CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H06	19	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Mon Jul 13 14:15:53 2015 Page 1
ID:RvT5uBQd627LEbyHCUAd8syFMxT-yY8jAZNjzTwwqvh4jCunnskYmFOsTCv41ARglCXyyTMz
3x5 II
Scale = 1:67.2



TOTAL WEIGHT = 19 X 94 = 1788 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - G	2x4 DRY	No.2	SPF		
K - B	2x4 DRY	No.2	SPF		
K - F	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
I - H	2x4 DRY	No.2	SPF		

ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
J - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+p	MT20	3.0	5.0		
E	TMVW-t	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0	1.50	1.75
I	BS-t	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	9.0		
K	BMVW1-t	MT20	3.0	4.0	1.50	1.75

NOTES: (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
K	904	0	904	0
H	904	0	904	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
K	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0	
H	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-70.5	-70.5 0.10 (1)	J-D	0 / 475	0.08 (1)	
B-C	0 / 28	-70.5	-70.5 0.24 (1)	J-E	-174 / 0	0.20 (1)	
C-D	-639 / 0	-70.5	-70.5 0.19 (1)	C-J	-174 / 0	0.20 (1)	
D-E	-639 / 0	-70.5	-70.5 0.19 (1)	K-C	-780 / 0	0.89 (1)	
E-F	0 / 28	-70.5	-70.5 0.24 (1)	E-H	-780 / 0	0.89 (1)	
F-G	0 / 35	-70.5	-70.5 0.10 (1)				
K-B	-219 / 0	0.0	0.0 0.02 (1)				
H-F	-219 / 0	0.0	0.0 0.02 (1)				
K-J	0 / 468	-17.5	-17.5 0.46 (4)				
J-I	0 / 468	-17.5	-17.5 0.46 (4)				
I-H	0 / 468	-17.5	-17.5 0.46 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.24 (B-C:1), BC=0.46 (J-K:4), WB=0.89 (E-H:1), SSI=0.13 (J-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

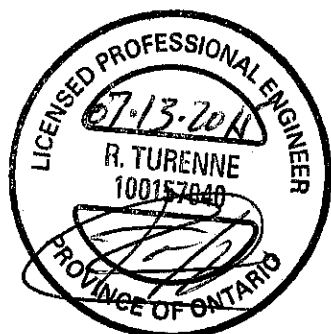
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

JSI METAL= 0.24 (C) (INPUT = 1.00)



A-15073486

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

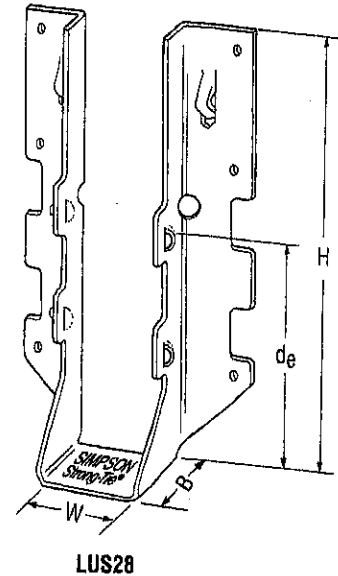
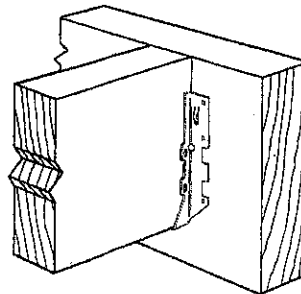
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

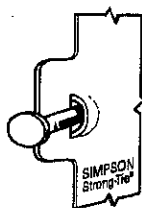
- These hangers cannot be modified.

Typical LUS
Installation



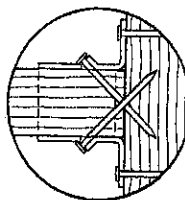
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/16	4 3/4	1 3/4	3 3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 15/16	1 3/4	3 3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

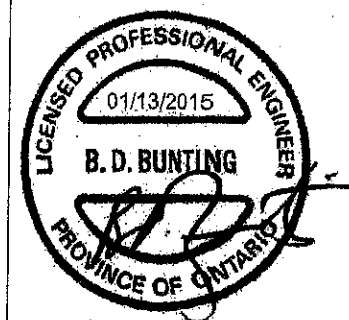


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

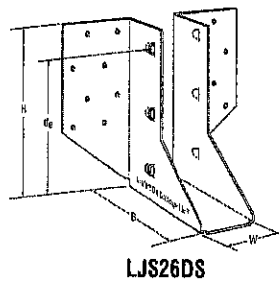
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

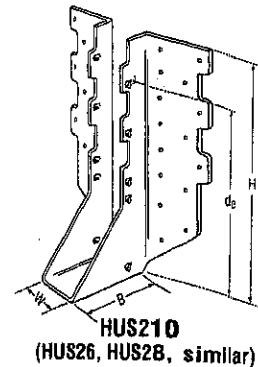
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

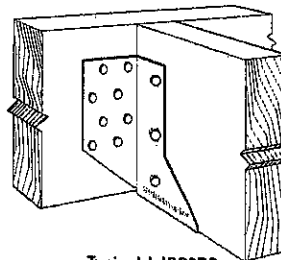
- See current catalogue for options



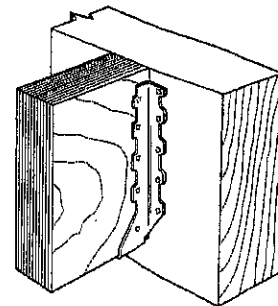
LJS26DS



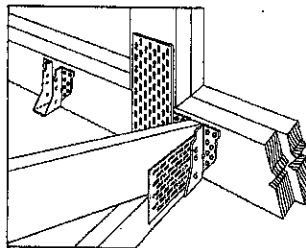
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



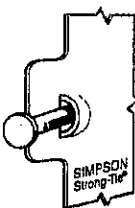
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

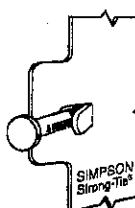
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 9/16	5	3 1/2	4%	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1%	5%	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1%	7 3/32	3	6 3/32	22-16d	8-16d	3605	5365	2875	4345
HUS210	16	1%	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

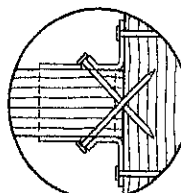


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

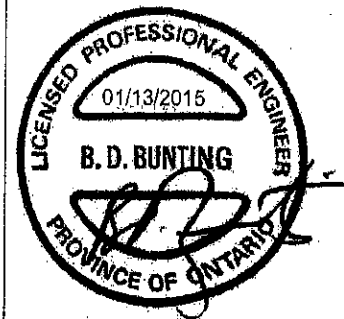
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

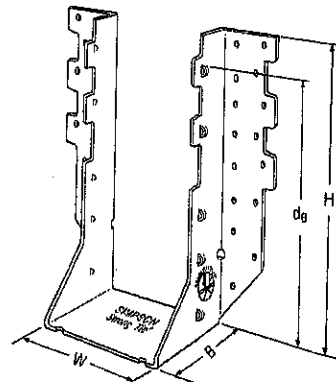
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

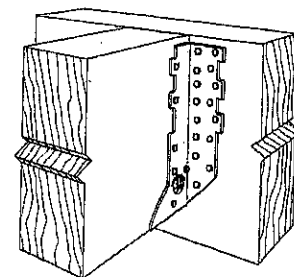
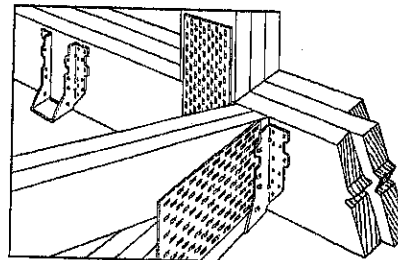
OPTIONS:

- See current catalogue for options

Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)



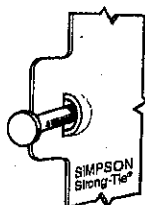
HHUS410



Typical HHUS
Installation

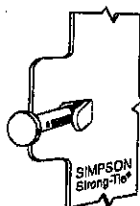
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
HHUS26-2	14	3 1/16	5 1/16	3	3 15/16	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3 3/16	7 1/32	3	6 5/32	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3 3/16	9 9/32	3	8	30-16d	10-16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3 3/8	5 1/32	3	3 15/16	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3 3/8	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 3/16	7 29/32	30-16d	10-16d	4745	10770	4310	7650

1. d_g is the distance from the seat of the hanger to the highest joist nail.

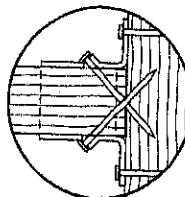


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

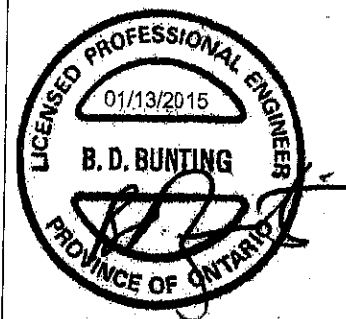
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.

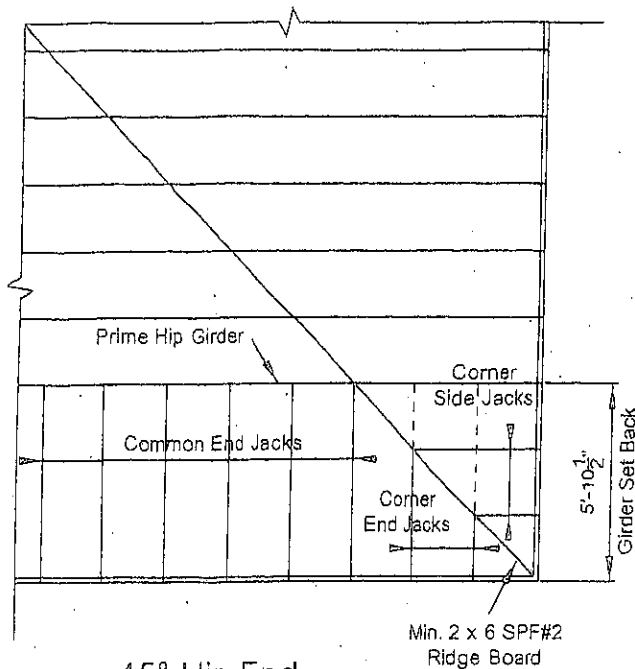


Double
Shear
Nailing
Top View.



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STRACON ENGINEERING INC.



45° Hip End

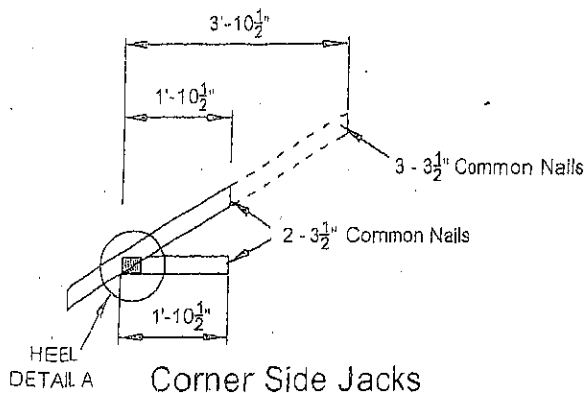
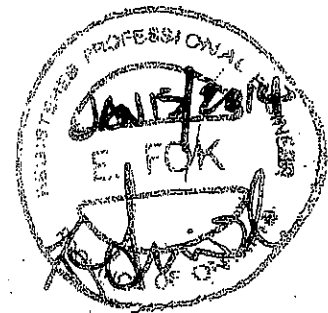
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

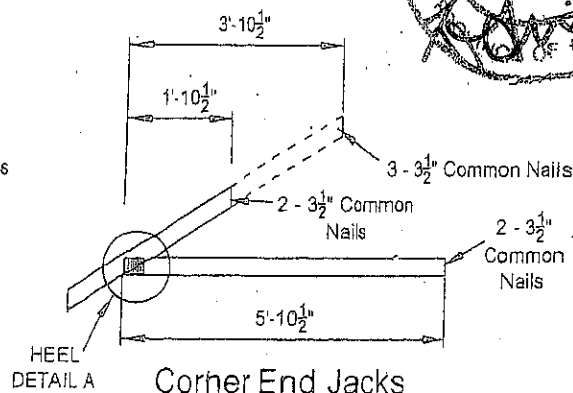
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

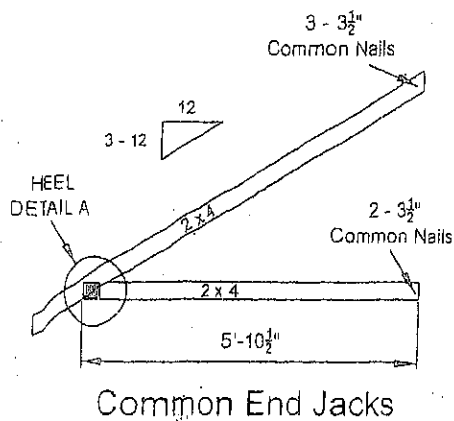
TOTAL LOAD : 50.5 P.S.F.



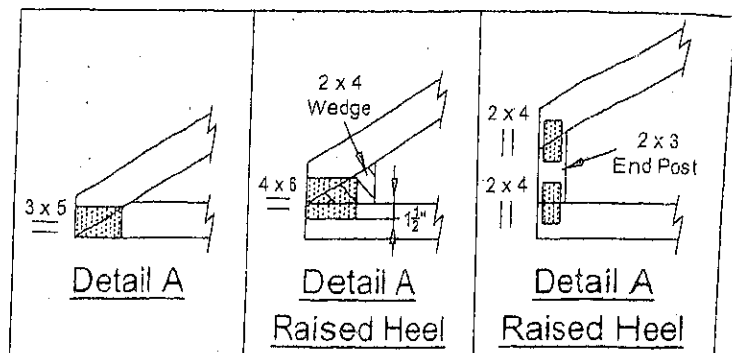
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008