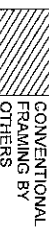


THE UNIVERSITY OF CHICAGO



STD OR OPT 2NDFLR.

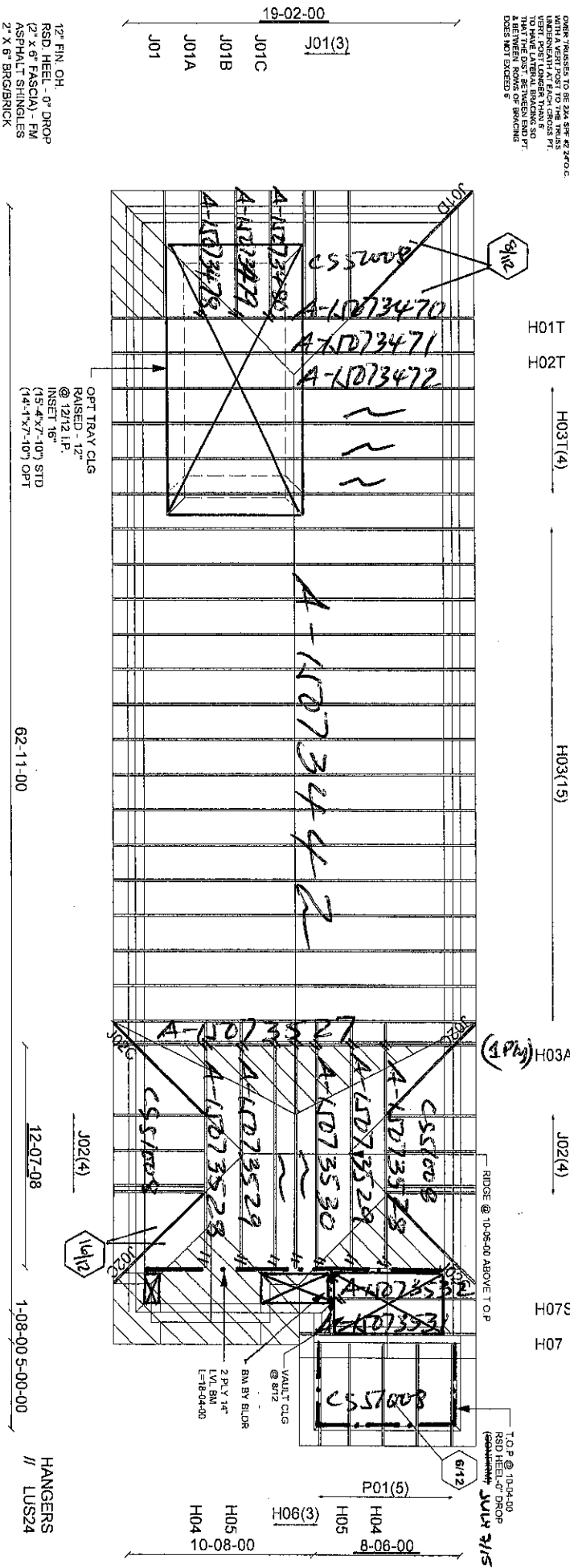
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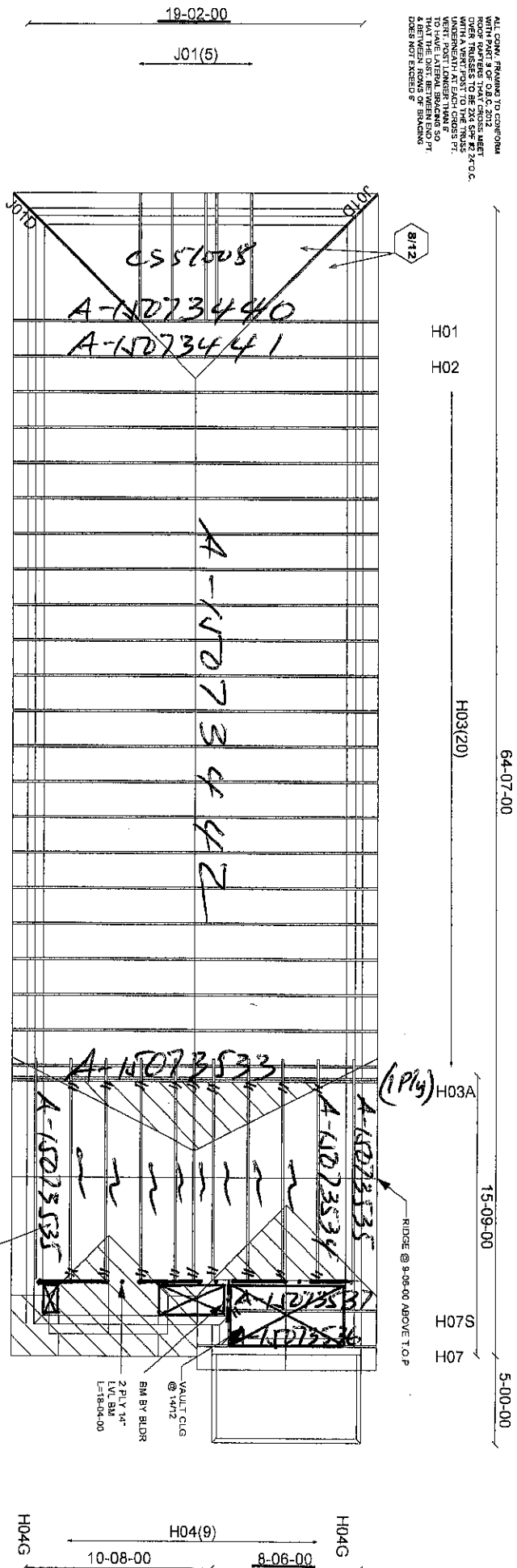
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ALL CORN FRAMING TO CONFORM WITH THE FOLLOWING: ROOF TRUSSES TO BE 2X 8 SP-22 24" O.C. OVER 1" MIN. THICK INSULATION. ROOF TRUSSES TO BE 2X 6 SP-22 24" O.C. UNDERNEATH AT EACH CROSS PT. TRUSSES TO BE 2X 6 SP-22 24" O.C. BETWEEN JOISTS. JOISTS TO BE 2X 8 SP-22 24" O.C. BETWEEN TRUSSES. JOISTS TO BE 2X 6 SP-22 24" O.C. UNDERNEATH AT EACH CROSS PT. THAT THE DIST. BETWEEN END PT. JOISTS NOT EXCEEDS 8'.

12" FIN. OH.
RSD. HEEL - 0" DROP
2" x 6" FASCLA - FM
ASPHALT SHINGLES
2" x 6" BRGRICK



PL8098C



Job Track: 40297
Layout ID: 252547
Plan Log: 79711

Builder / Location: GOLD PARK / KLEINBURG
Project: HUNTINGTON & NASHVILLE
Date: 4/21/2015
Designer: chenyle

Model / Elevation: 25-7 / B
STD OR OPT 2ND FLR
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

CONVENTIONAL
FRAMING BY
OTHERS

Mike Ver 7.5.0

HANGERS
// LUS24

ALL CORN. FRAMING TO CONFORM
TO THE ROOF TRUSS TO BE SET
OVER TRUSSES TO BE SET BY 24" O.C.
UNDER TRUSSES TO BE SET BY 24" O.C.
UNDER TRUSSES TO BE SET BY 24" O.C.
UNDER TRUSSES TO BE SET BY 24" O.C.
UNDER TRUSSES TO BE SET BY 24" O.C.
UNDER TRUSSES TO BE SET BY 24" O.C.
UNDER TRUSSES TO BE SET BY 24" O.C.
UNDER TRUSSES TO BE SET BY 24" O.C.

64-07-00

ALPHA-BARRAGE

5-00-00

15-09-00

H03(16)

H03T(4)

H01T
H02T

H03A (1PLY)

H07S
H07

RIDGE @ 9-06-00 ABOVE T.O.P

H04G

H04(9)

10-08-00

8-06-00

H04G

18-02-00
J01(3)
J01C
J01B
J01A
J01

12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCI) - FM
ASPHALT SHINGLES
2" X 6" BRICK

OPT TRAY CLG
RAISED - 12"
@ 12/12 I.P.
INSET 16"
(15'-4"x7'-10") STD
(14'-1"x7'-10") OPT

62-11-00

1-08-00 5-00-00

HANGERS
// LUS24

CONVENTIONAL
FRAMING BY
OTHERS

Mitek Ver 7.5.0



PL 80980

Job Track 40297

Layout ID: 252548

Plan Log: 79711

Builder / Location:

GOLD PARK / KLEINBURG

Project: HUNTINGTON & NASHVILLE

Date: 4/20/2015 Designer: cheryle

Model / Elevation:

25-7

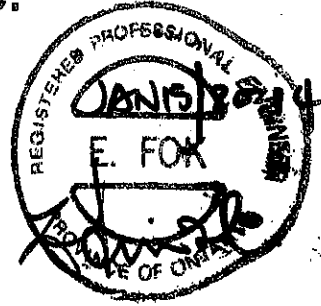
/ B OPT TRAY

STD OR OPT 2ND FLR.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPHA ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPHA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPHA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

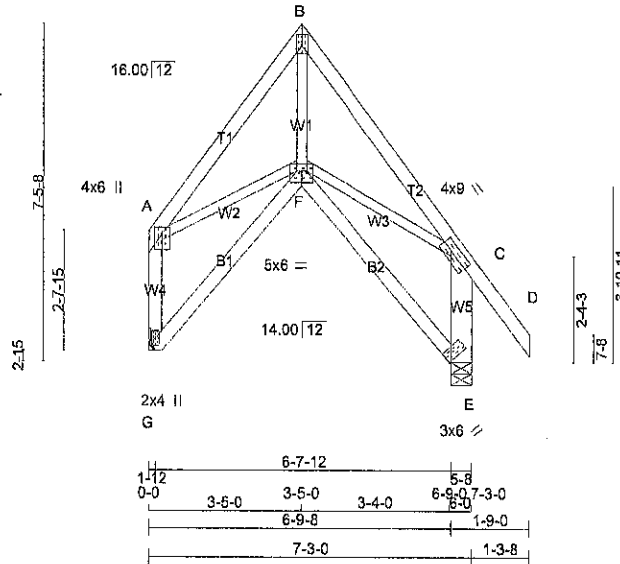
January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252547	H07S	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Tue Jul 14 11:16:44 2015 Page 1

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LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW-t	MT20	4.0	9.0	2.00	3.00
E	BVM1+I	MT20	3.0	6.0	2.00	Edge
F	BBWWW-p	MT20	5.0	6.0	3.00	3.00
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
G	319	0	319	0	0	5-8	1-8		
E	418	0	418	0	0	5-8	1-8		

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	225	162 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
E	282	211 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E
BEARING SIZE FACTOR = 1.15 AT JNT(S) E (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	MAX (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX (LC)	MAX (LC)
FR-TO						FR-TO			
A-B	-309 / 0	-70.5	-70.5	0.11 (1)	6.25	F-B	0 / 239	0.05 (1)	
B-C	-309 / 0	-70.5	-70.5	0.13 (1)	6.25	A-F	0 / 202	0.05 (1)	
C-D	0 / 40	-70.5	-70.5	0.10 (1)	10.00	F-C	0 / 209	0.05 (1)	
G-A	-289 / 0	0.0	0.0	0.04 (1)	7.81				
E-C	-384 / 0	0.0	0.0	0.04 (1)	7.81				
G-F	0 / 0	-17.5	-17.5	0.06 (4)	10.00				
F-E	0 / 0	-17.5	-17.5	0.08 (4)	10.00				

TOTAL WEIGHT = 45 lb
(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.13 (B-C:1), BC=0.08 (E-F:4), WB=0.05 (B-F:1), SSI=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 622 2264 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.37 (B) (INPUT = 0.90)
JSI METAL= 0.09 (G) (INPUT = 1.00)



A-H073537

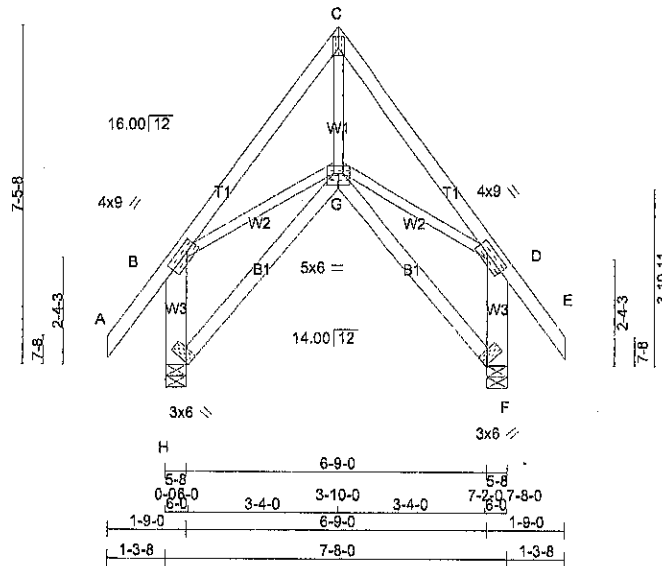
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252547	H07	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2016 MITek Industries, Inc. Tue Jul 14 11:07:41 2015 Page 1

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3x5 || Scale = 1:51.0



TOTAL WEIGHT = 50 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	9.0	2.00	3.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW-I	MT20	4.0	9.0	2.00	3.00
F	BVM1+I	MT20	3.0	6.0	2.00	Edge
G	BBWWW-p	MT20	5.0	6.0	3.00	3.00
H	BVM1+I	MT20	3.0	6.0	2.00	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	436	0	436	0	0	5-8	1-8
F	436	0	436	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	305	220 / 0	0 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0	0 / 0
F	305	220 / 0	0 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F
BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)
BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC1)	MAX. FORCE (LBS)	MAX. HORIZ. LOAD (LC1)	MAX. FORCE (LBS)	MAX. HORIZ. LOAD (LC1)
FR-TO								
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G-C	0 / 285	0.06 (1)
B-C	-347 / 0	-70.5	-70.5	0.13 (1)	6.25	B-G	0 / 235	0.05 (1)
C-D	-347 / 0	-70.5	-70.5	0.13 (1)	6.25	G-D	0 / 235	0.05 (1)
D-E	0 / 40	-70.5	-70.5	0.10 (1)	10.00			
H-B	-403 / 0	0.0	0.0	0.04 (1)	7.81			
F-D	-403 / 0	0.0	0.0	0.04 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.08 (4)	10.00			
G-F	0 / 0	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360 (0.26")$
CALCULATED VERT. DEFL.(LL) = $L/899 (0.01")$
ALLOWABLE DEFL.(TL) = $L/360 (0.26")$
CALCULATED VERT. DEFL.(TL) = $L/899 (0.02")$

CSI: TC=0.13 (B-C:1), BC=0.08 (G-H:4),
WB=0.08 (C-G:1), SSI=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (C) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

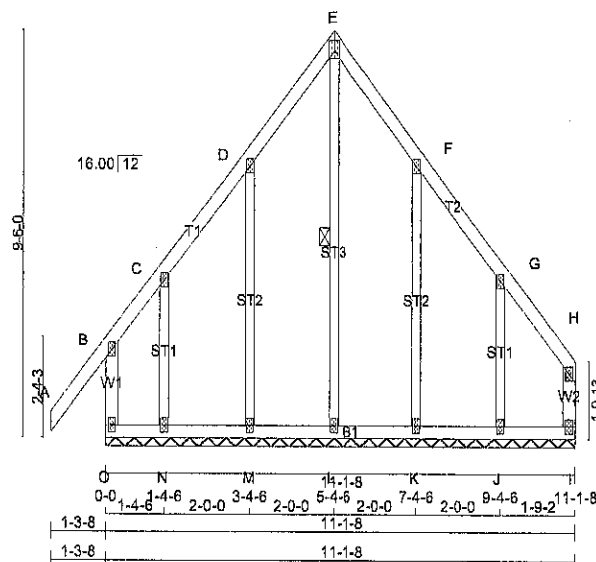


A-H073536

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252547	H04G	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MTEK Industries, Inc. Tue Jul 14 11:07:40 2016 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
O - I	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B - TMV+p	MT20	2.0	4.0		
C, D, F, G					
C - TMW+w	MT20	2.0	4.0		
E - TTW+p	MT20	3.0	5.0	2.00	Edge
H - TMV+p	MT20	2.0	4.0		
I - BMV1+p	MT20	2.0	4.0		
J, K, L, M, N					
J - BMW1+w	MT20	2.0	4.0		
O - BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC) UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC) UNBRAC
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-70.5 -70.5	0.10 (1)	10.00	L-E	-154 / 0	0.09 (1)
B-C	-35 / 0	-70.5 -70.5	0.08 (1)	8.25	M-D	-153 / 0	0.12 (1)
C-D	0 / 8	-70.5 -70.5	0.04 (1)	10.00	N-C	-63 / 0	0.02 (1)
D-E	0 / 3	-70.5 -70.5	0.04 (1)	10.00	K-F	-145 / 0	0.11 (1)
E-F	0 / 4	-70.5 -70.5	0.04 (1)	10.00	J-G	-129 / 0	0.03 (1)
F-G	0 / 2	-70.5 -70.5	0.04 (1)	10.00			
G-H	0 / 0	-70.5 -70.5	0.03 (1)	10.00			
O-B	-185 / 0	0.0 0.0	0.02 (1)	7.81			
I-H	-64 / 0	0.0 0.0	0.01 (1)	7.81			
O-N	0 / 2	-17.5 -17.5	0.02 (1)	10.00			
N-M	0 / 0	-17.5 -17.5	0.01 (4)	10.00			
M-L	-3 / 0	-17.5 -17.5	0.01 (4)	10.00			
L-K	-3 / 0	-17.5 -17.5	0.01 (4)	10.00			
K-J	0 / 0	-17.5 -17.5	0.01 (4)	10.00			
J-I	0 / 4	-17.5 -17.5	0.01 (4)	10.00			

TOTAL WEIGHT = 2 X 64 = 128 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(66 % OF 23.0 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1), BC=0.02 (N-O:1), WB=0.12 (D-M:1), SSI=0.05 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX
MT20	616	354	1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)

JSI METAL= 0.06 (B) (INPUT = 1.00)

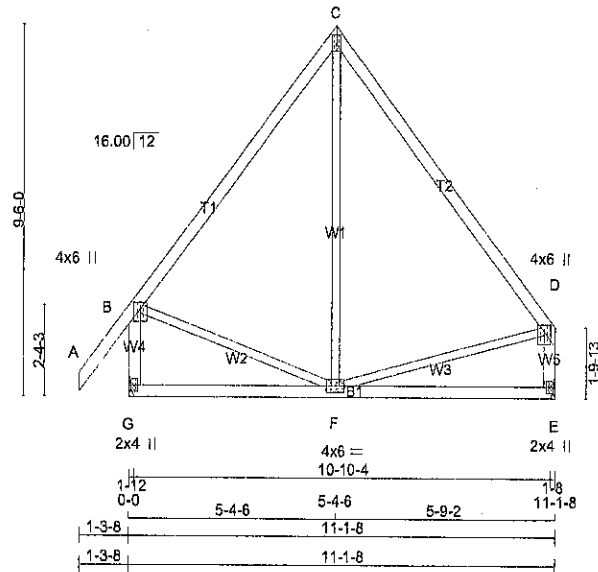


A-1073535

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252547	H04	9	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 8 Apr 15 2015 MITek Industries, Inc. Tue Jul 14 11:07:39 2015 Page 1
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3x5 ||
Scale = 1:59.3



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.00 2.00
C	TTW+p	MT20	3.0	5.0	2.00 Edge
D	TMVW+p	MT20	4.0	6.0	2.00 2.00
E	BMV1+p	MT20	2.0	4.0	
F	BMVWW-t	MT20	4.0	6.0	
G	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	588	0	588	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8
E	489	0	489	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	412	283 / 0	0 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0	0 / 0
E	345	233 / 0	0 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MEMB.		WEBS	
MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO	FROM TO	FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	0 / 102
B-C	-260 / 0	-70.5	-70.5 0.26 (1)	6.25	0 / 166
C-D	-260 / 0	-70.5	-70.5 0.30 (1)	6.25	0 / 161
G-B	-554 / 0	0.0	0.0 0.07 (1)	7.81	
E-D	-450 / 0	0.0	0.0 0.05 (1)	7.81	
G-F	0 / 0	-17.5	-17.5 0.16 (4)	10.00	
F-E	0 / 0	-17.5	-17.5 0.16 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.30 (C-D:1), BC=0.16 (F-G:4),
WB=0.04 (B-F:1), SSI=0.10 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

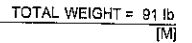
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.10 (G) (INPUT = 1.00)



A-N073534



SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

GIRDER TYPE: CStdGlrdr
START DISTANCE = 0-0
START SPAN CARRIED = 11-1-8
END DISTANCE = 18-4-0
END SPAN CARRIED = 11-1-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.41 (B-C:1), BC=0.71 (I-K:1), WB=0.61 (B-L:1), SSI=0.41 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

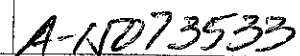
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP (DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

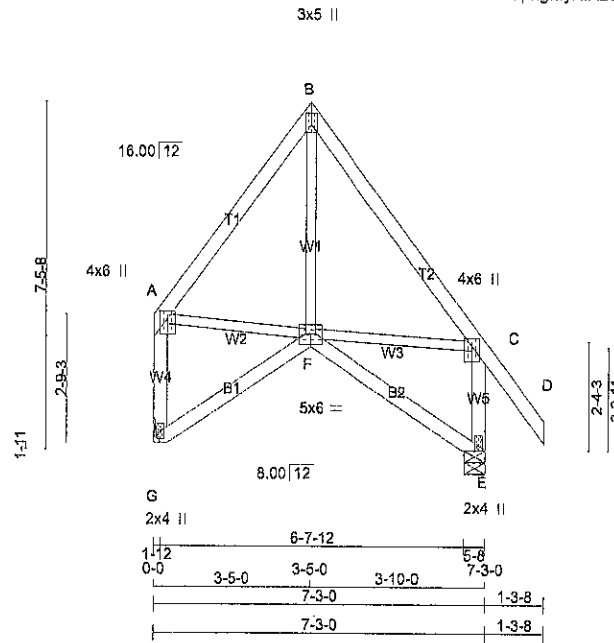
PLATE PLACEMENT TOL. ≈ 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.62 (J) (INPUT = 1.00)



Alpa Roof Truss, Maple



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
G - A	2x4	DRY	No.2
E - C	2x4	DRY	No.2
G - F	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0	Edge	
F	BBVWW+p	MT20	5.0	6.0	2.75	3.00
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	IN-SX	IN-SX
G	319	0	319	0
E	418	0	418	0

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	PERM.LIVE		
G	225	152 / 0	0 / 0	73 / 0	0 / 0
E	292	211 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO	CSI (LC)		FR-TO		
A-B	-199 / 0	-70.5 -70.5	0.11 (1)	6.25	F-B	0 / 71	0.02 (4)
B-C	-199 / 0	-70.5 -70.5	0.13 (1)	6.25	A-F	0 / 120	0.03 (1)
C-D	0 / 40	-70.5 -70.5	0.10 (1)	10.00	F-C	0 / 120	0.03 (1)
G-A	-289 / 0	0.0 0.0	0.05 (1)	7.81			
E-C	-384 / 0	0.0 0.0	0.05 (1)	7.81			
G-F	0 / 0	-17.5 -17.5	0.08 (4)	10.00			
F-E	0 / 0	-17.5 -17.5	0.08 (4)	10.00			

TOTAL WEIGHT = 41 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.24")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13 (B-C:1), BC=0.08 (E-F:4), WB=0.03 (A-F:1), SSI=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (E) (INPUT = 0.90)
 JSI METAL= 0.09 (E) (INPUT = 1.00)



A-15073532

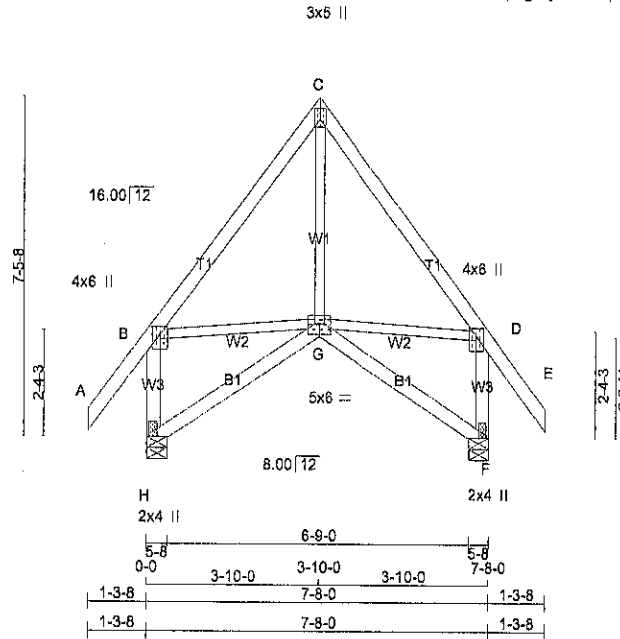
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252544	H07	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 MITek Industries, Inc. Tue Jul 14 10:43:28 2015 Page 1

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Scale = 1:51.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00	
C	TTW+p	MT20	3.0	5.0	2.00	Edge	
D	TMVW+p	MT20	4.0	6.0	2.00	2.00	
F	BMV1+p	MT20	2.0	4.0	Edge		
G	BBWWV+p	MT20	5.0	6.0	2.75	3.00	
H	BMV1+p	MT20	2.0	4.0	Edge		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	436	0	436	0
F	436	0	436	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
H	305	220 / 0	0 / 0	0 / 0	0 / 0	85 / 0
F	305	220 / 0	0 / 0	0 / 0	0 / 0	85 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX
FR-TO			FROM TO	CSI (LC)		FR-TO		
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G-C	0 / 87	0.03 (4)
B-C	-223 / 0	-70.5	-70.5	0.13 (1)	6.25	B-G	0 / 134	0.03 (1)
C-D	-223 / 0	-70.5	-70.5	0.13 (1)	6.25	G-D	0 / 134	0.03 (1)
D-E	0 / 40	-70.5	-70.5	0.10 (1)	10.00			
H-B	-403 / 0	0.0	0.0	0.06 (1)	7.81			
F-D	-403 / 0	0.0	0.0	0.06 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.08 (4)	10.00			
G-F	0 / 0	-17.5	-17.5	0.08 (4)	10.00			

TOTAL WEIGHT = 45 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13 (B-C:1), BC=0.08 (F-G:4),
WB=0.03 (B-G:1), SSI=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

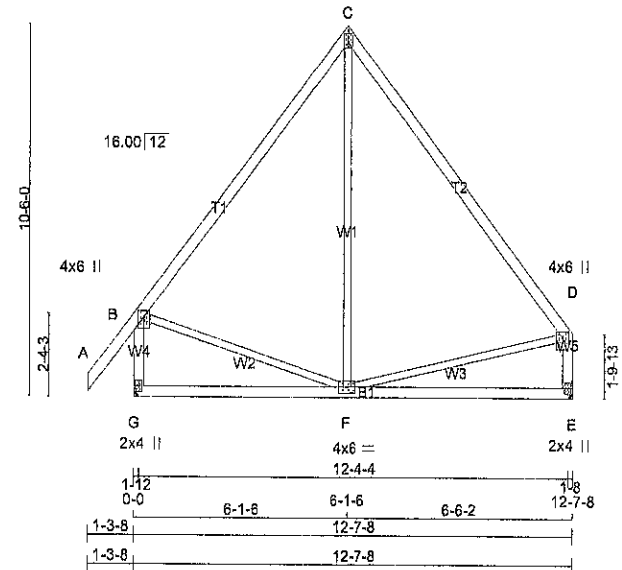
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (F) (INPUT = 0.90)
JSI METAL= 0.10 (H) (INPUT = 1.00)



A-15073531

Alpa Roof Truss, Maple



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - D	2x4	DRY	2100F 1.8E	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	6.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW+I	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	654	0	654	0	0	
E	555	0	555	0	0	

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	459	324 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
E	391	265 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED VERT. LOAD LC1 MAX		MAX UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
			FROM	TO				
A-B	0 / 40	-70.5	-70.5	0.08 (1)	10.00	F-C	0 / 119	0.04 (4)
B-C	-302 / 0	-70.5	-70.5	0.29 (1)	6.25	B-F	0 / 191	0.04 (1)
C-D	-302 / 0	-70.5	-70.5	0.25 (1)	6.25	F-D	0 / 186	0.04 (1)
G-B	-815 / 0	0.0	0.0	0.08 (1)	7.81			
E-D	-511 / 0	0.0	0.0	0.06 (1)	7.81			
G-F	0 / 0	-17.5	-17.5	0.20 (4)	10.00			
F-E	0 / 0	-17.5	-17.5	0.20 (4)	10.00			

TOTAL WEIGHT = 3 X 84 = 191 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010
 THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD)

ALLOWABLE DEFL. (LL) = L/360 (0.42")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
 ALLOWABLE DEFL. (TL) = L/360 (0.42")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.29 (B-C:1), BC=0.20 (F-G:4), WB=0.04 (B-F:1), SS=0.11 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

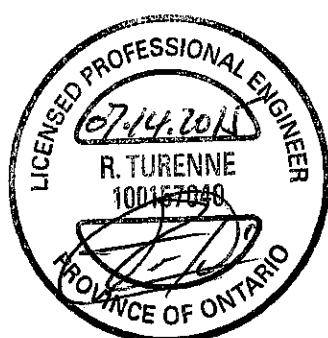
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN
MT20	618	354	1687	822

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP = 0.36 (B) (INPUT = 0.90)
 JSI METAL = 0.11 (G) (INPUT = 1.00)

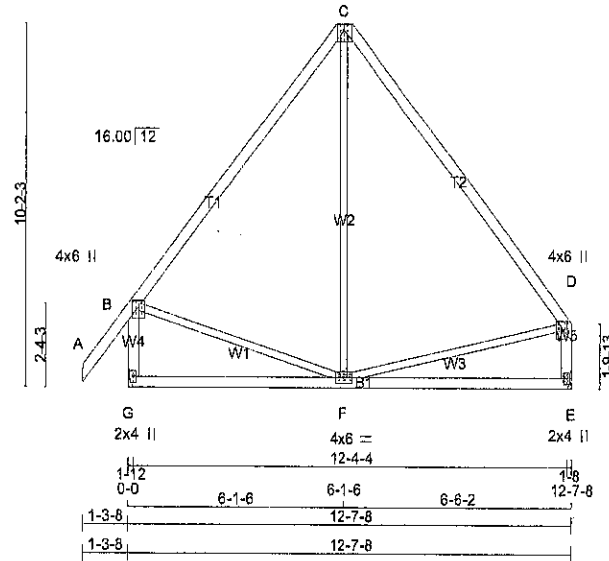


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252544	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MTEK Industries, Inc. Tue Jul 14 10:43:27 2015 Page 1
ID:NCRsbaluoxc9YpxgnlyAfAzOpco-ONazXQktMgYBFkmRyYAw014h?mw6TV4FuYTBKRyyBNl
5x6 II
Scale = 1:64.8



TOTAL WEIGHT = 2 X 63 = 126 lb
(M)(F)

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - C	2x4	DRY	1650F 1.5E	SPF	SPF
C - D	2x4	DRY	2100F 1.8E	SPF	SPF
E - B	2x4	DRY	No.2	SPF	SPF
E - D	2x4	DRY	No.2	SPF	SPF
G - E	2x4	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B - TMVW+p	MT20	4.0	6.0	2.00	2.00
C - TMTMW+p	MT20	5.0	6.0	Edge	
D - TMVW+p	MT20	4.0	6.0	2.00	2.00
E - BMV1+p	MT20	2.0	4.0		
F - BMVWW-l	MT20	4.0	6.0		
G - BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	654	0	654	0	0	HANGER BY OTHERS	HANGER BY OTHERS
E	555	0	555	0	0	MIN. SEAT SIZE: 1-8	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	469	324 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
E	391	265 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO	CS (LC)	UNBRAC	LENGTH	FR-TO
A-B	0 / 40	-70.5	-70.5	0.08 (1)	10.00	F-C	0 / 119
B-C	-302 / 0	-70.5	-70.5	0.29 (1)	6.25	B-F	0 / 181
C-D	-302 / 0	-70.5	-70.5	0.25 (1)	6.25	F-D	0 / 186
G-B	-615 / 0	0.0	0.0	0.08 (1)	7.81		
E-D	-611 / 0	0.0	0.0	0.06 (1)	7.81		
G-F	0 / 0	-17.5	-17.5	0.20 (4)	10.00		
F-E	0 / 0	-17.5	-17.5	0.20 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.29 (B-C:1), BC=0.20 (F-G:4),
WB=0.04 (B-F:1), SS=0.11 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

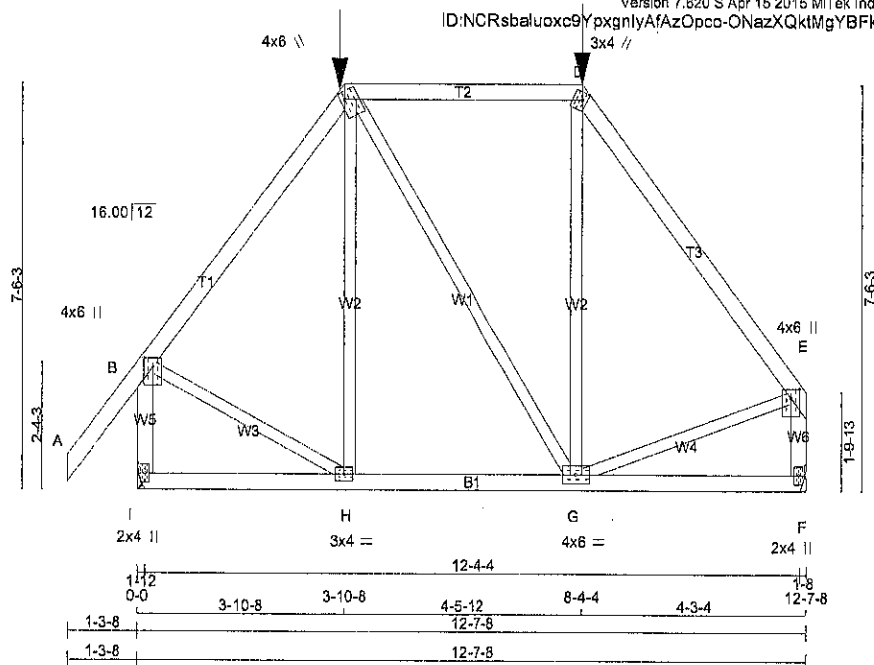
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.36 (B) (INPUT = 0.90)
JSI METAL = 0.11 (G) (INPUT = 1.00)



A-10073529



TOTAL WEIGHT = 2 X 67 = 134 lb

LUMBER		N. L. G. A. RULES		LUMBER		DESIGN	
CHORDS	SIZE						
A - C	2x4	DRY	No.2	SPF			
C - D	2x4	DRY	No.2	SPF			
D - E	2x4	DRY	No.2	SPF			
E - B	2x4	DRY	No.2	SPF			
F - E	2x4	DRY	No.2	SPF			
I - F	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF			

DRY; SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWV+m	MT20	4.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	1.75	1.00
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWWW-t	MT20	4.0	6.0		
H	BMWW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 119.1 lbs FACTORED DOWN AT 8-4-4, AND 108.1 lbs FACTORED DOWN AT 3-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			FACTORED			MAXIMUM FACTORED			INPUT		REQRD	
JT	GROSS REACTION		GROSS REACTION		UPLIFT	BRG	IN-SX	BRG	IN-SX			
	VERT	HORZ	DOWN	HORZ								
I	898	0	898	0	0							
F	791	0	791	0	0	HANGER BY OTHERS						
						MIN. SEAT SIZE: 1-8						
						HANGER BY OTHERS						
						MIN. SEAT SIZE: 1-8						

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	630	438 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
F	558	374 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED

LOADING

LOADING
TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	H-C	-52 / 78	0.05 (1)	
B-C	-668 / 0	-70.5	-70.5	0.21 (1)	6.25	C-G	0 / 35	0.01 (1)	
C-D	-355 / 0	-103.5	-103.5	0.40 (1)	6.25	G-D	-23 / 92	0.03 (4)	
D-E	-595 / 0	-70.5	-70.5	0.26 (1)	6.25	B-H	0 / 378	0.09 (1)	
I-B	-856 / 0	0.0	0.0	0.12 (1)	7.81	G-E	0 / 374	0.09 (1)	
F-E	-745 / 0	0.0	0.0	0.09 (1)	7.81				
I-H	0 / 0	-25.7	-25.7	0.10 (4)	10.00				
H-G	0 / 337	-25.7	-25.7	0.16 (4)	10.00				
G-F	0 / 0	-25.7	-25.7	0.13 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-108	-108	---	FRONT	VERT	TOTAL
D	8-4-4	-119	-119	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-10-8
RIGHT SETBACK = 4-3-4
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.40 (C-D:1), BC=0.16 (G-H:4),
WB=0.09 (B-H:1), SS=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1687	822	2284 1656

PLATE PLACEMENT TOL. = 0,250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (G) (INPUT = 0.90)
JSI METAL= 0.16 (I) (INPUT = 1.00)

A-15073528



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252544	H03A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Tue Jul 14 10:43:26 2015 Page 2
ID:NCRsbaluoxc9YpxgnyAfAzOpco-wA0bK4JFbMQKdaBEQqhtTfXT?MPBkt06fuec?yyBN

COMPANION LIVE LOAD FACTOR = 0.60
AUTOSOLVE HEELS OFF
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.82 (J) (INPUT = 1.00)

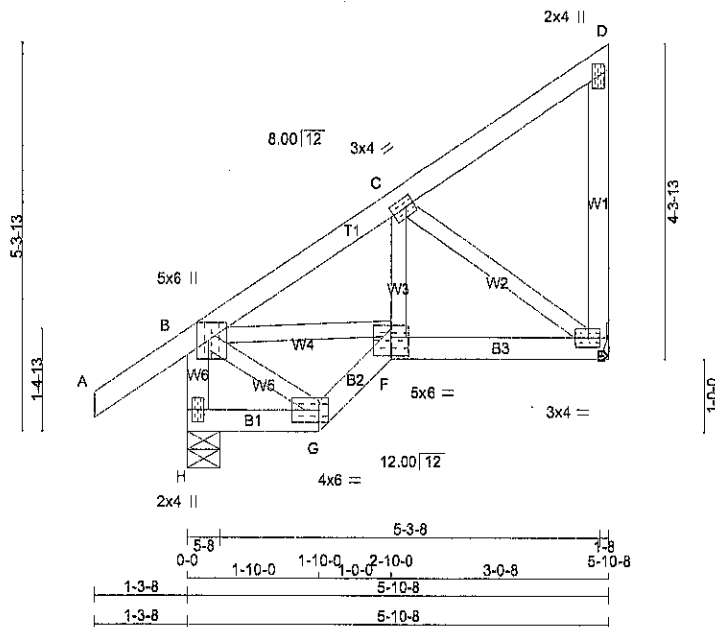


A-1073527(2)

JOB-NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	J01C	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 8 Apr 15 2015 MITek Industries, Inc. Mon Jul 13 13:31:56 2015 Page 1
ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-B2JSWYSaEyZ8QHPjgsS9yRI_H_dV71xkZ0f9cRyyU7c
Scale: 3/8"=1'



TOTAL WEIGHT = 30 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF		
A - D	2x4	DRY	No.2	SPF		
E - D	2x4	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW+p	MT20	5.0	8.0	Edge	
C	TMVWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-t	MT20	5.0	6.0	3.00	3.00
G	BBW-t	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT	REQRD		
GROSS REACTION		GROSS REACTION		BRG	BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS	
MIN. SEAT SIZE: 1-8							

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	248	181/0	0/0	0/0	0/0	87/0	0/0
E	182	123/0	0/0	0/0	0/0	59/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							FR-TO			
H-B	-339 / 0	0.0	0.0	0.03 (1)	7.81	B-G	0 / 20	0.01 (4)		
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B-F	0 / 209	0.05 (1)		
B-C	-267 / 0	-70.5	-70.5	0.10 (1)	6.25	F-C	0 / 57	0.02 (4)		
C-D	-13 / 0	-70.5	-70.5	0.10 (1)	6.25	C-E	-271 / 0	0.06 (1)		
E-D	-79 / 0	0.0	0.0	0.02 (1)	7.81					
H-G	0 / 0	-17.5	-17.5	0.02 (4)	10.00					
G-F	0 / 25	-17.5	-17.5	0.01 (4)	10.00					
F-E	0 / 223	-17.5	-17.5	0.06 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.10 (B-C:1), BC=0.06 (E-F:1), WB=0.06 (C-E:1), SS=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
618	354	1667
822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

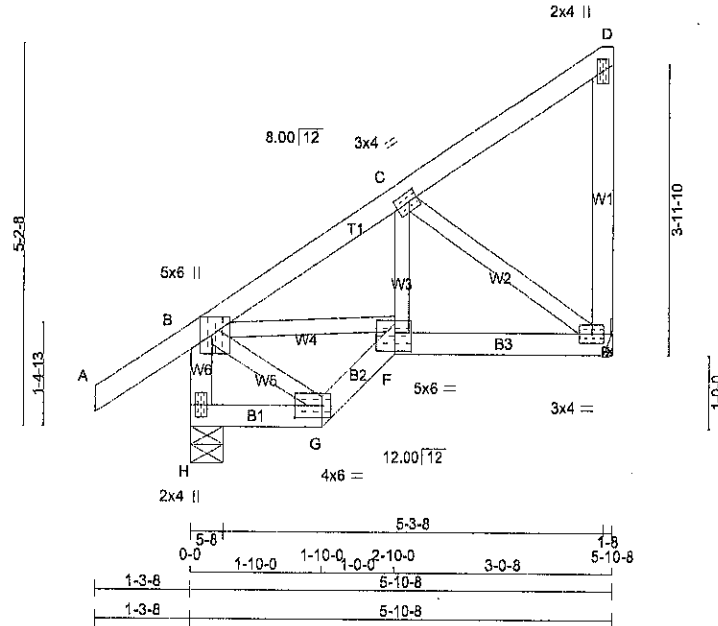


A-15073480

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	J01B	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.820 S Apr 15 2015 MITek Industries, Inc. Mon Jul 13 13:31:54 2015 Page 1
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Scale = 1:31.6



LUMBER					
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW+p	MT20	5.0	6.0	Edge	
C	TMVWW-l	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-l	MT20	5.0	6.0	3.00	3.00
G	BBW-l	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
E	258	0	258	0	0	HANGER BY OTHERS
H	355	0	355	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO				FR-TO			
A-B	0 / 27	-70.5	-70.5 0.08 (1)	B-F	0 / 209	0.05 (1)	
B-C	-257 / 0	-70.5	-70.5 0.10 (1)	F-C	0 / 57	0.02 (4)	
C-D	-13 / 0	-70.5	-70.5 0.10 (1)	C-E	-271 / 0	0.06 (1)	
E-D	-79 / 0	0.0	0.0 0.02 (1)	B-G	0 / 20	0.01 (4)	
H-B	-339 / 0	0.0	0.0 0.03 (1)				
H-G	0 / 0	-17.5	-17.5 0.02 (4)				
G-F	0 / 25	-17.5	-17.5 0.01 (4)				
F-E	0 / 223	-17.5	-17.5 0.06 (1)				

TOTAL WEIGHT = 30 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.10 (B-C:1), BC=0.06 (E-F:1), WB=0.06 (C-E:1), SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

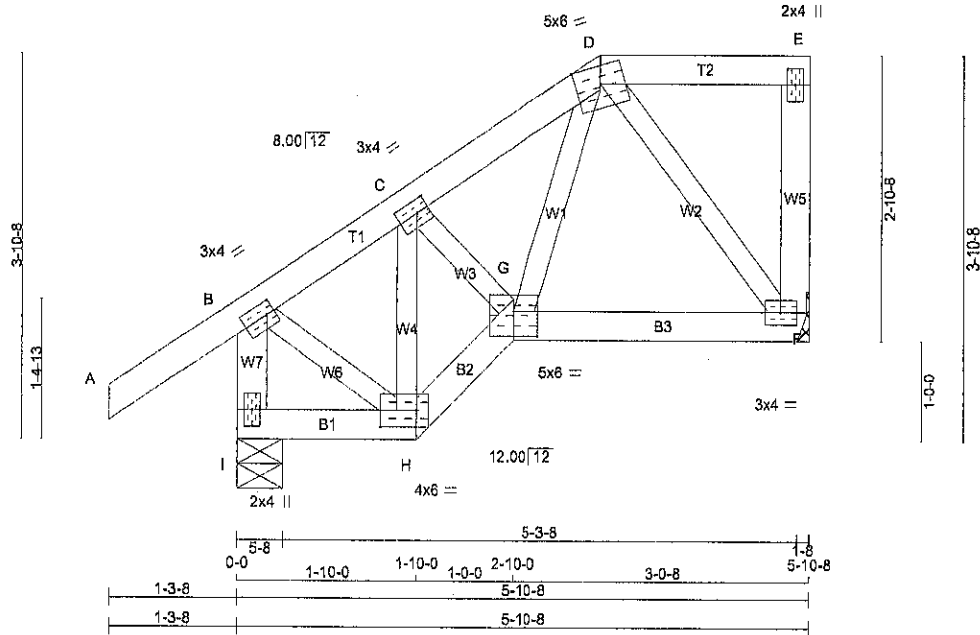


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JOB-NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	J01A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MIT Industries, Inc. Mon Jul 13 13:31:54 2015 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTVW-m	MT20	5.0	6.0	Edge	
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BBVW1-t	MT20	6.0	6.0	3.00	3.00
H	BBVW1-t	MT20	4.0	6.0	2.00	4.50
I	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	
F	247	0	247	0	0	0	
I	366	0	366	0	5-8	1-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	174	117 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
I	256	188 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 27	-70.5	-70.5 0.09 (1)	10.00	H-C	-170 / 0	0.03 (1)
B-C	-173 / 0	-70.5	-70.5 0.09 (1)	6.25	C-G	0 / 61	0.01 (1)
C-D	-197 / 0	-70.5	-70.5 0.04 (1)	6.25	G-D	0 / 119	0.03 (1)
D-E	0 / 0	-70.5	-70.5 0.06 (1)	10.00	D-F	-190 / 0	0.04 (1)
F-E	-76 / 0	0.0	0.0 0.01 (1)	7.81	B-H	0 / 146	0.03 (1)
I-B	-350 / 0	0.0	0.0 0.04 (1)	7.81			
I-H	0 / 0	-17.5	-17.5 0.02 (4)	10.00			
H-G	0 / 171	-17.5	-17.5 0.03 (1)	10.00			
G-F	0 / 123	-17.5	-17.5 0.05 (4)	10.00			

TOTAL WEIGHT = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.09 (A-B:1), BC=0.06 (F-G:4), WB=0.04 (D-F:1), SS=0.06 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

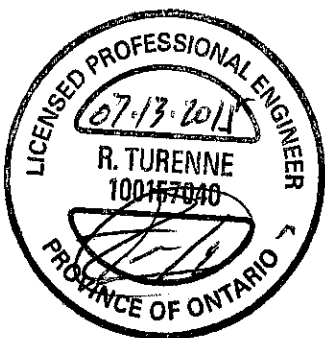
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

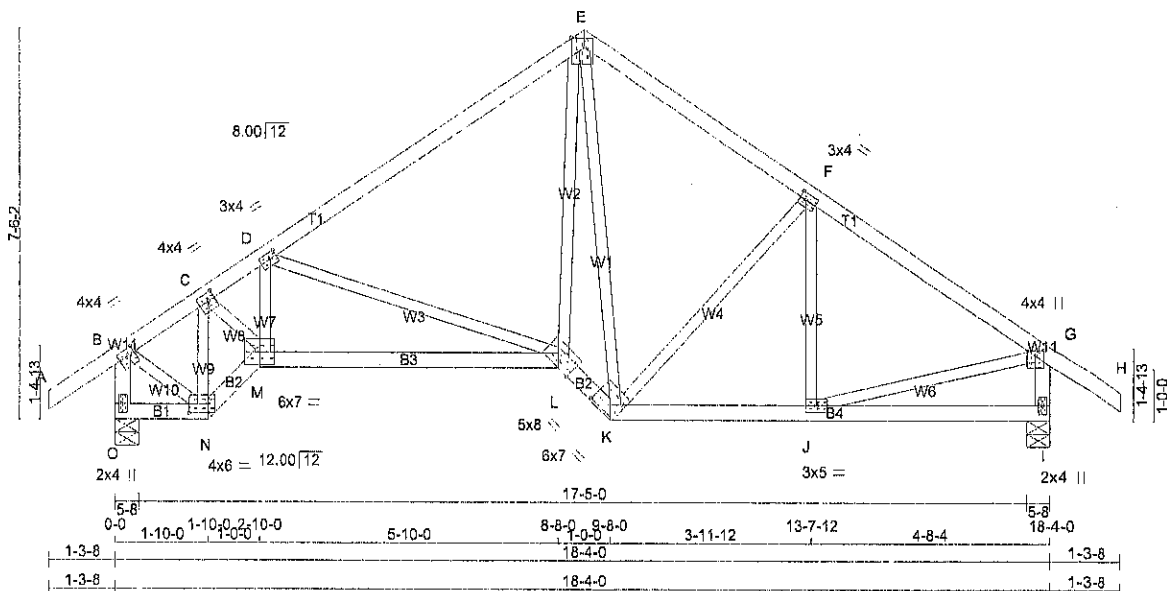
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



A-15073478



TOTAL WEIGHT = $4 \times 86 = 343$ lb

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-i	MT20	4.0	4.0	1.50	1.00
C	TMVW-i	MT20	4.0	4.0	1.75	1.50
D	TMVW-i	MT20	3.0	4.0	1.50	1.50
E	TTVW+p	MT20	5.0	6.0	Edge	3.00
F	TMVW-i	MT20	3.0	4.0	1.50	1.50
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-i	MT20	3.0	5.0		
K	BBVW-h	MT20	6.0	7.0	3.75	6.00
L	BBVW-h	MT20	5.0	8.0	Edge	
M	BBVW-i	MT20	6.0	7.0		
N	BBVW-i	MT20	4.0	6.0	2.00	4.50
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
0	907	0	907	0	0	5-8
1	899	0	899	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	636	444 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
I	631	440 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O. I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY
APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX	MEMB.	FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	N-C	-768 / 0	0.12 (1)	
B-C	-877 / 0	-70.5	-70.5	0.08 (1)	6.25	C-M	0 / 888	0.20 (1)	
C-D	-1152 / 0	-70.5	-70.5	0.30 (1)	5.57	M-D	-104 / 57	0.02 (4)	
D-E	-781 / 0	-70.5	-70.5	0.33 (1)	8.25	D-L	-512 / 0	0.35 (1)	
E-F	-656 / 0	-70.5	-70.5	0.19 (1)	6.25	L-E	0 / 781	0.18 (1)	
F-G	-838 / 0	-70.5	-70.5	0.20 (1)	6.25	E-K	-313 / 0	0.32 (1)	
G-H	0 / 27	-70.5	-70.5	0.09 (1)	10.00	K-F	-272 / 0	0.16 (1)	
O-B	-890 / 0	0.0	0.0	0.09 (1)	7.81	J-F	-79 / 51	0.02 (1)	
I-G	-865 / 0	0.0	0.0	0.09 (1)	7.81	B-N	0 / 607	0.14 (1)	
						J-G	0 / 734	0.17 (1)	
O-N	0 / 0	-17.5	-17.5	0.02 (4)	10.00				
N-M	0 / 693	-17.5	-17.5	0.11 (1)	10.00				
M-L	0 / 1096	-17.5	-17.5	0.28 (1)	10.00				
L-K	0 / 777	-17.5	-17.5	0.13 (1)	10.00				
K-J	0 / 715	-17.5	-17.5	0.16 (1)	10.00				
J-I	0 / 0	-17.5	-17.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD =		31.0	PSF

TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.33 (D-E:1), BC=0.28 (L-M:1), WB=0.35 (D-L:1), SS=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP (DRY)		SHEAR		SECTION	
	(PSI)	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

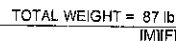
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)



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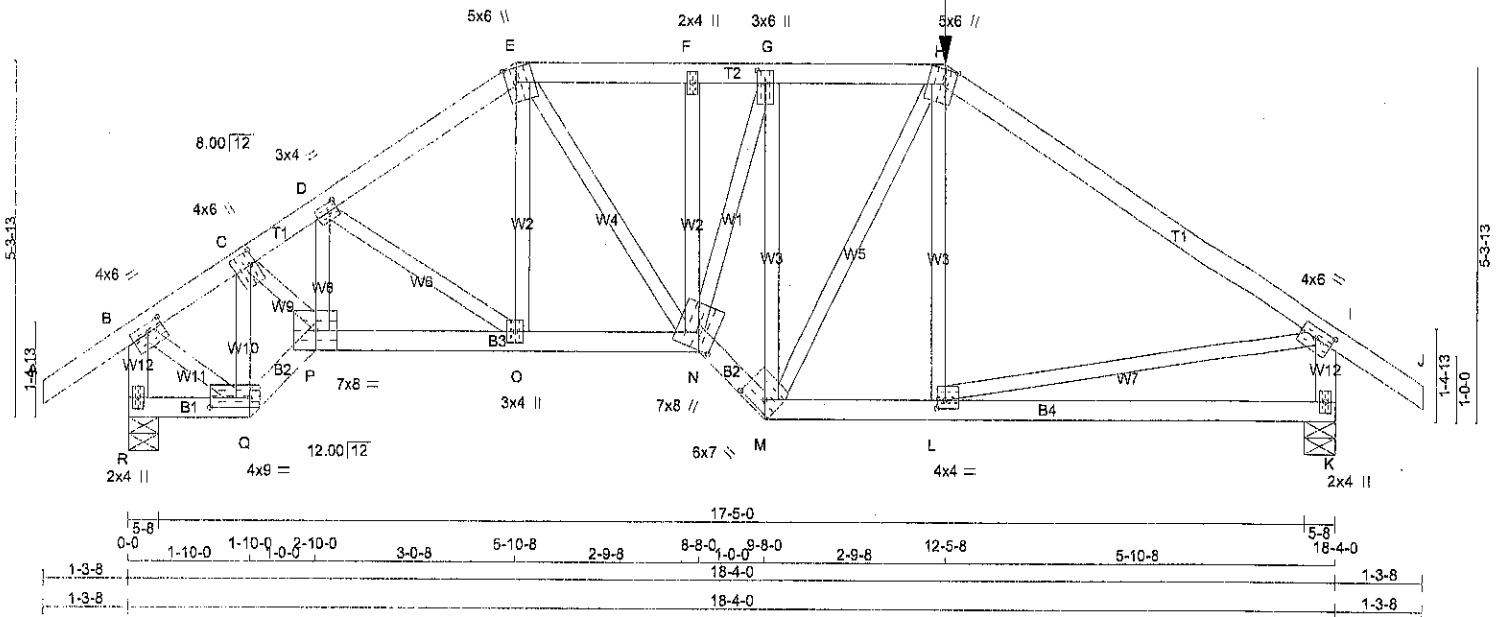


A-15073471

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MTEK Industries, Inc. Mon Jul 13 11:47:53 2015 Page 1
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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
O - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50 3.00
C	TMVW-t	MT20	4.0	6.0	2.50 1.00
D	TMVW-t	MT20	3.0	4.0	1.50 1.50
E	TTVW+m	MT20	5.0	6.0	Edge 1.75
F	TMVW-t	MT20	2.0	4.0	
G	TMVW-t	MT20	3.0	6.0	2.25 1.50
H	TTVW+m	MT20	5.0	6.0	Edge 1.75
I	TMVW-t	MT20	4.0	6.0	1.75 3.00
K	BMV1+p	MT20	2.0	4.0	
L	BMVW-t	MT20	4.0	4.0	1.50 1.50
M	BBVW-h	MT20	6.0	7.0	2.00 4.25
N	BBVW+m	MT20	7.0	8.0	3.00 3.00
O	BMVW-t	MT20	3.0	4.0	
P	BBVW-t	MT20	7.0	8.0	
Q	BBVW-t	MT20	4.0	9.0	1.75 7.25
R	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
R	1730	0	1730	0
K	1553	0	1553	0

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1215	842 / 0	0 / 0	0 / 0	0 / 0	373 / 0	0 / 0
K	1095	737 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM TO	CS (LC)	FR-TO		FROM TO	CS (LC)
A-B	0 / 27	-70.5 -70.5	0.10 (1)	Q-C	-1680 / 0	0.29 (1)	0.34 (1)
B-C	-1413 / 0	-138.7 -138.7	0.13 (1)	C-P	0 / 1378	0.06 (1)	0.11 (1)
C-D	-2377 / 0	-138.7 -138.7	0.13 (1)	P-D	0 / 220	0.06 (1)	0.11 (1)
D-E	-1980 / 0	-70.5 -70.5	0.15 (1)	D-O	-422 / 0	0.14 (1)	0.10 (1)
E-F	-1885 / 0	-70.5 -70.5	0.16 (1)	O-E	0 / 566	0.14 (1)	0.10 (1)
F-G	-1883 / 0	-138.7 -138.7	0.16 (1)	E-N	0 / 402	0.10 (1)	0.02 (1)
G-H	-1536 / 0	-138.7 -138.7	0.17 (1)	N-F	-83 / 3	0.36 (1)	0.78 (1)
H-I	-1648 / 0	-70.5 -70.5	0.61 (1)	F-G	0 / 1441	0.08 (1)	0.06 (4)
I-J	0 / 27	-70.5 -70.5	0.10 (1)	M-G	-1758 / 0	0.33 (1)	0.34 (1)
R-B	-1698 / 0	0.0 0.0	0.19 (1)	L-H	-57 / 151	0.06 (4)	0.06 (4)
K-I	-1468 / 0	0.0 0.0	0.17 (1)	B-Q	0 / 1324	0.33 (1)	0.34 (1)
R-Q	0 / 0	-34.5 -34.5	0.04 (4)	L-I	0 / 1392	0.34 (1)	
Q-P	0 / 1512	-34.5 -34.5	0.27 (1)				
P-O	0 / 1990	-102.7 -102.7	0.50 (1)				
O-N	0 / 1654	-102.7 -102.7	0.44 (1)				
N-M	0 / 2120	-34.5 -34.5	0.38 (1)				
M-L	0 / 1368	-34.5 -34.5	0.41 (4)				
L-K	0 / 0	-34.5 -34.5	0.29 (4)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
H	12-5-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN./G.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-10-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGlder
START DISTANCE = 2-10-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 8-8-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 9-8-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.61 (H-I-1), BC=0.50 (O-P-1), WB=0.76 (G-M-1), SSI=0.13 (H-I-1)

A-1013470

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Mon Jul 13 11:47:53 2015 Page 2

ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-yBYISBvBE58oZ1D30wTOz7m8Yo7lcofzDuctXEyyVXh

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 12-5-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 8.23.13.11, and no less than 2x4 for Part 4 design.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

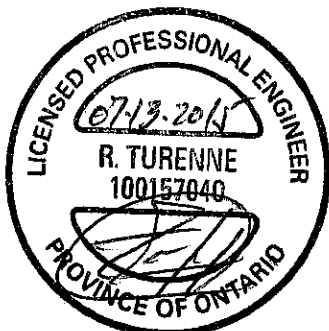
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1607 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.51 (L) (INPUT = 1.00)

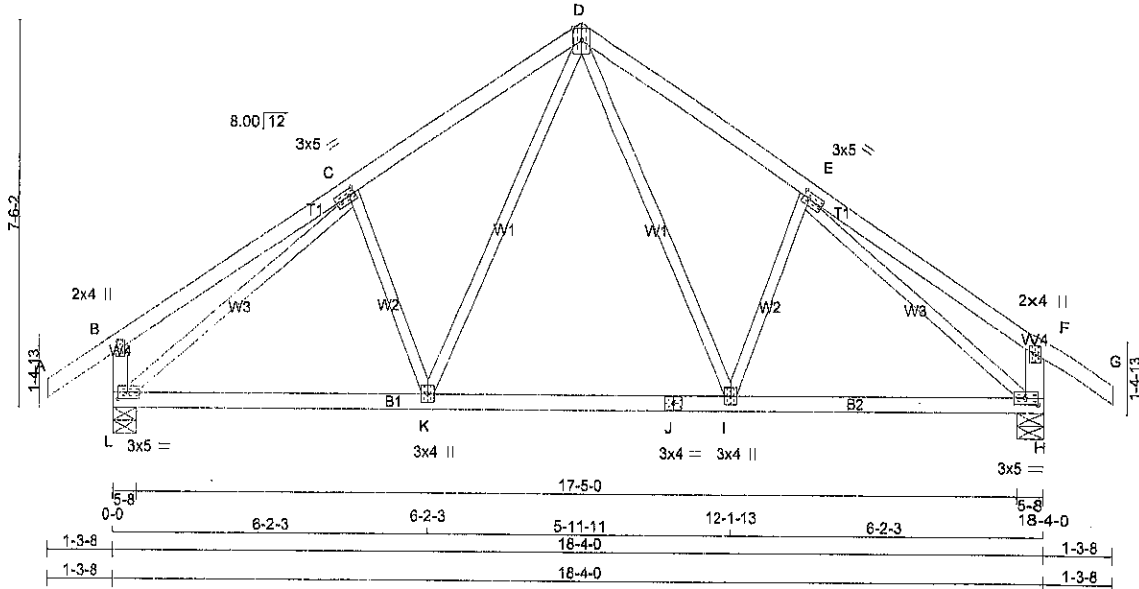


A-15073470(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252512	H03	14	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Mon Jul 13 08:58:51 2015 Page 1
ID: gUzNWMpyWh_3Mf_ncsznAqzVRmF-v1kdTHzLpQurtwV2wNj1pVkJGjvD94_qBZ4yyY7o
4x6 II
Scale = 1:44.9



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	3.0	5.0	1.50	1.50
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	5.0	1.50	2.25
I	BMWW-t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	4.0		
K	BMWW-t	MT20	3.0	4.0		
L	BMVW1-t	MT20	3.0	5.0	1.50	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	903	0	903	0	5-8	1-8
H	903	0	903	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
L	834	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
H	834	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED	FACTORED		MAX UNBRAC	MEMB.	MAX. FACTORED	MAX	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)			FORCE (LBS)		CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	D-I	0 / 323	0.07 (1)
B-C	0 / 21	-70.5	-70.5	0.24 (1)	10.00	I-E	-196 / 3	0.07 (1)
C-D	-784 / 0	-70.5	-70.5	0.19 (1)	6.25	K-D	0 / 323	0.07 (1)
D-E	-784 / 0	-70.5	-70.5	0.19 (1)	6.25	C-K	-196 / 3	0.07 (1)
E-F	0 / 21	-70.5	-70.5	0.24 (1)	10.00	L-C	-963 / 0	0.62 (1)
F-G	0 / 27	-70.5	-70.5	0.09 (1)	10.00	E-H	-963 / 0	0.62 (1)
L-B	-219 / 0	0.0	0.0	0.02 (1)	7.81			
H-F	-219 / 0	0.0	0.0	0.02 (1)	7.81			
L-K	0 / 707	-17.5	-17.5	0.20 (4)	10.00			
K-J	0 / 511	-17.5	-17.5	0.19 (4)	10.00			
J-I	0 / 511	-17.5	-17.5	0.19 (4)	10.00			
I-H	0 / 707	-17.5	-17.5	0.20 (4)	10.00			

TOTAL WEIGHT = 14 X 81 = 1128 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CSI: TC=0.24 (E-F:1), BC=0.20 (H-I:4), WB=0.62 (E-H:1), SSI=0.13 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

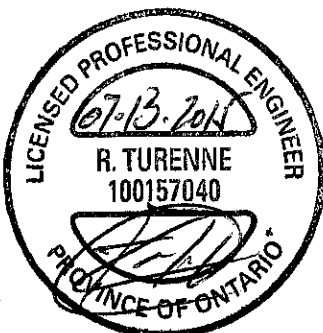
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)

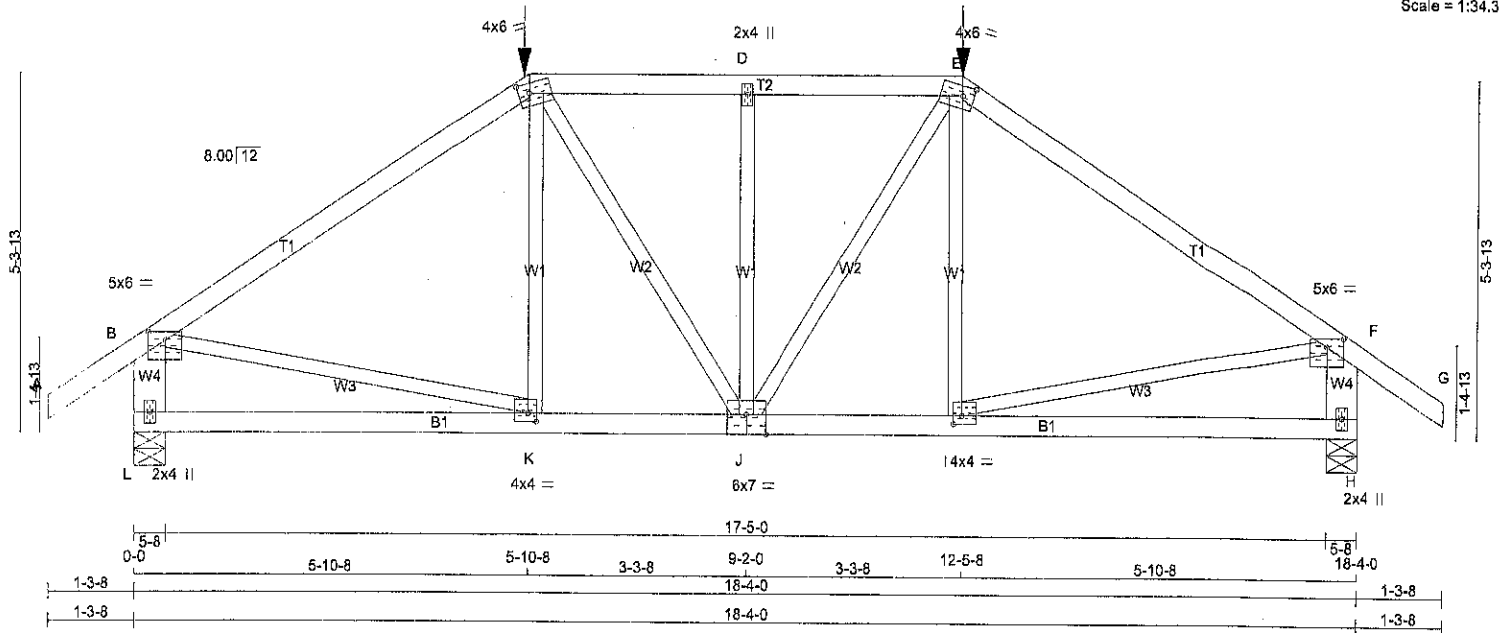
JSI METAL= 0.30 (C) (INPUT = 1.00)



A-15073442



A-15073441



TOTAL WEIGHT = 82 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF
 A - C 2x4 DRY No.2 SPF
 C - E 2x4 DRY No.2 SPF
 E - G 2x4 DRY No.2 SPF
 L - B 2x6 DRY No.2 SPF
 H - F 2x8 DRY No.2 SPF
 L - J 2x4 DRY No.2 SPF
 J - H 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.00
F	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMV1+p	MT20	2.0	4.0		
I	BSWW-t	MT20	4.0	4.0	1.50	1.50
J	BSWW-H	MT20	6.0	7.0	Edge	3.50
K	BSWW-t	MT20	4.0	4.0	1.50	1.50
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 12-5-8, AND 301.8 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	1585	0	1585	0	5-8	1-11
H	1585	0	1585	0	5-8	1-11

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1117	756 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0
H	1117	756 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX UNBRAC	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)			(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 27	-70.5	-70.5	0.10 (1)	10.00	K-C	-37 / 163	0.06 (4)
B-C	-1694 / 0	-70.5	-70.5	0.82 (1)	4.38	C-J	0 / 341	0.08 (1)
C-D	-1594 / 0	-138.7	-138.7	0.29 (1)	4.86	J-D	-544 / 0	0.24 (1)
D-E	-1694 / 0	-138.7	-138.7	0.29 (1)	4.86	J-E	0 / 341	0.08 (1)
E-F	-1694 / 0	-70.5	-70.5	0.82 (1)	4.38	I-E	-37 / 163	0.06 (4)
F-G	0 / 27	-70.5	-70.5	0.10 (1)	10.00	B-K	0 / 1430	0.35 (1)
L-B	-1500 / 0	0.0	0.0	0.11 (1)	7.81	I-F	0 / 1430	0.35 (1)
H-F	-1500 / 0	0.0	0.0	0.11 (1)	7.81			
L-K	0 / 0	-34.5	-34.5	0.29 (4)	10.00			
K-J	0 / 1407	-34.5	-34.5	0.41 (4)	10.00			
J-I	0 / 1407	-34.5	-34.5	0.41 (4)	10.00			
I-H	0 / 0	-34.5	-34.5	0.29 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-302	-302	---	FRONT	VERT	TOTAL
E	12-5-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 3.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPPrimeHip
 SIDE SETBACK = 5-10-8
 END SETBACK = 5-10-8
 END WALL WIDTH = 0-0
 CORNER FRAMING TYPE: CONVENTIONAL
 END JACK TYPE: CONVENTIONAL
 APPLIED TO FRONT SIDE
 - ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL) = L/360 (0.61")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.62 (B-C:1), BC=0.41 (J-K:4), WB=0.35 (F-I:1), SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
 JSI METAL= 0.63 (K) (INPUT = 1.00)

A-15073440



LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

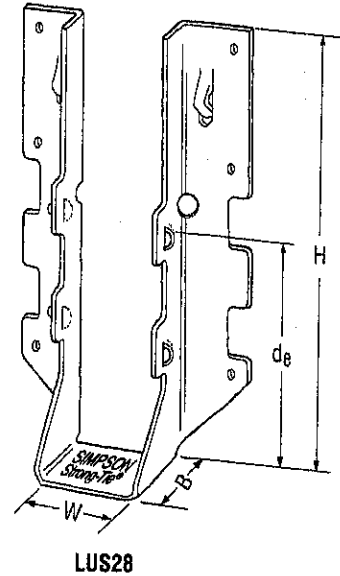
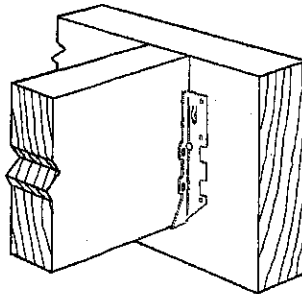
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

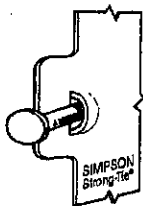
- These hangers cannot be modified.

Typical LUS
Installation



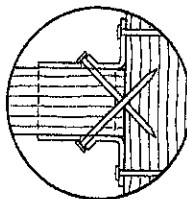
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D. Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 1/8	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/4	1 3/4	3 3/4	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 3/8	4 3/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 3/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 3/4	2	3 3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 3/4	3 3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 3/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

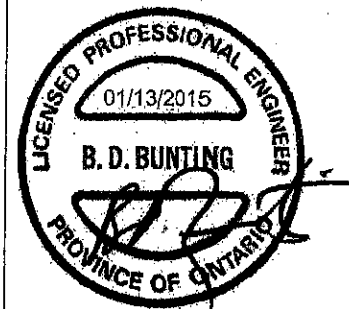


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580

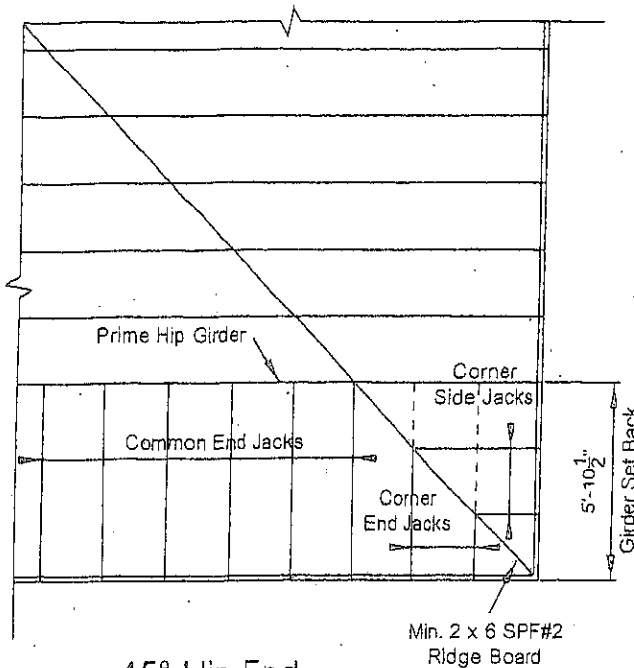


Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

STRACON ENGINEERING INC.



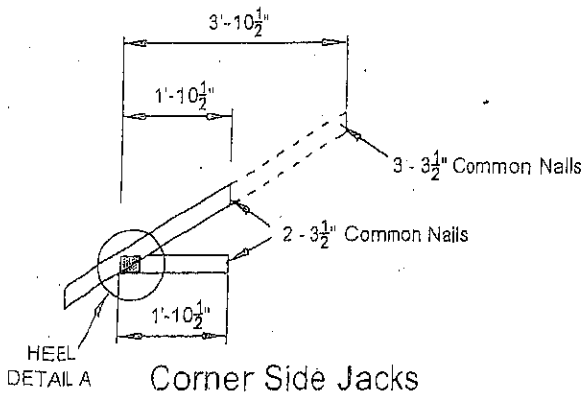
45° Hip End

LUMBER SPECIFICATION

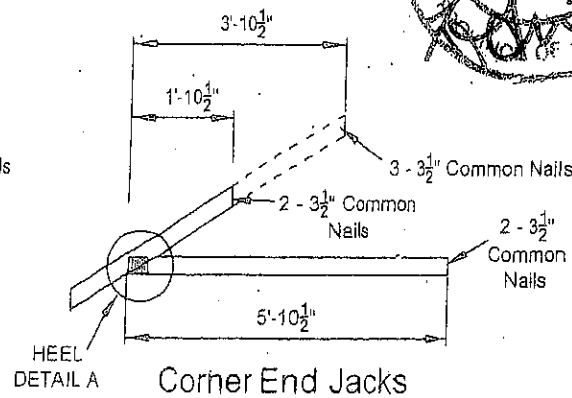
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

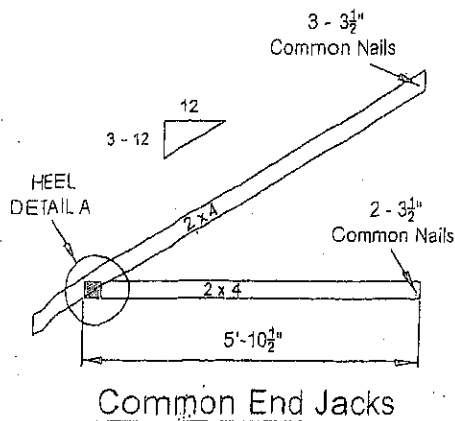
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 50.5 P.S.F.



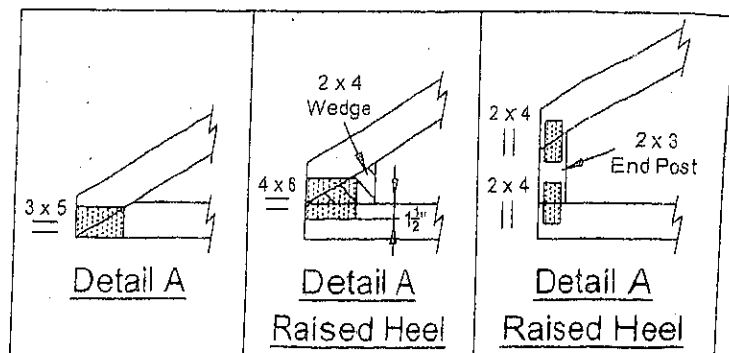
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008