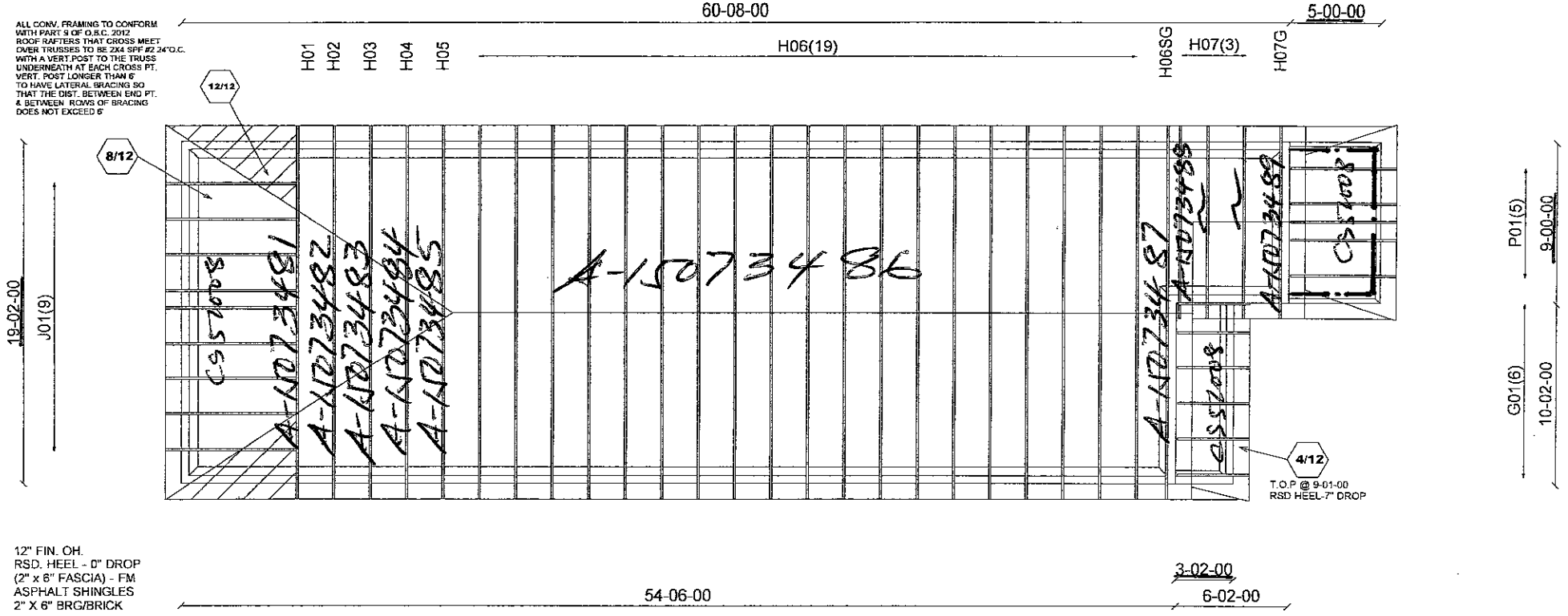
ALPA-BARRIE



CONVENTIONAL FRAMING BY OTHERS

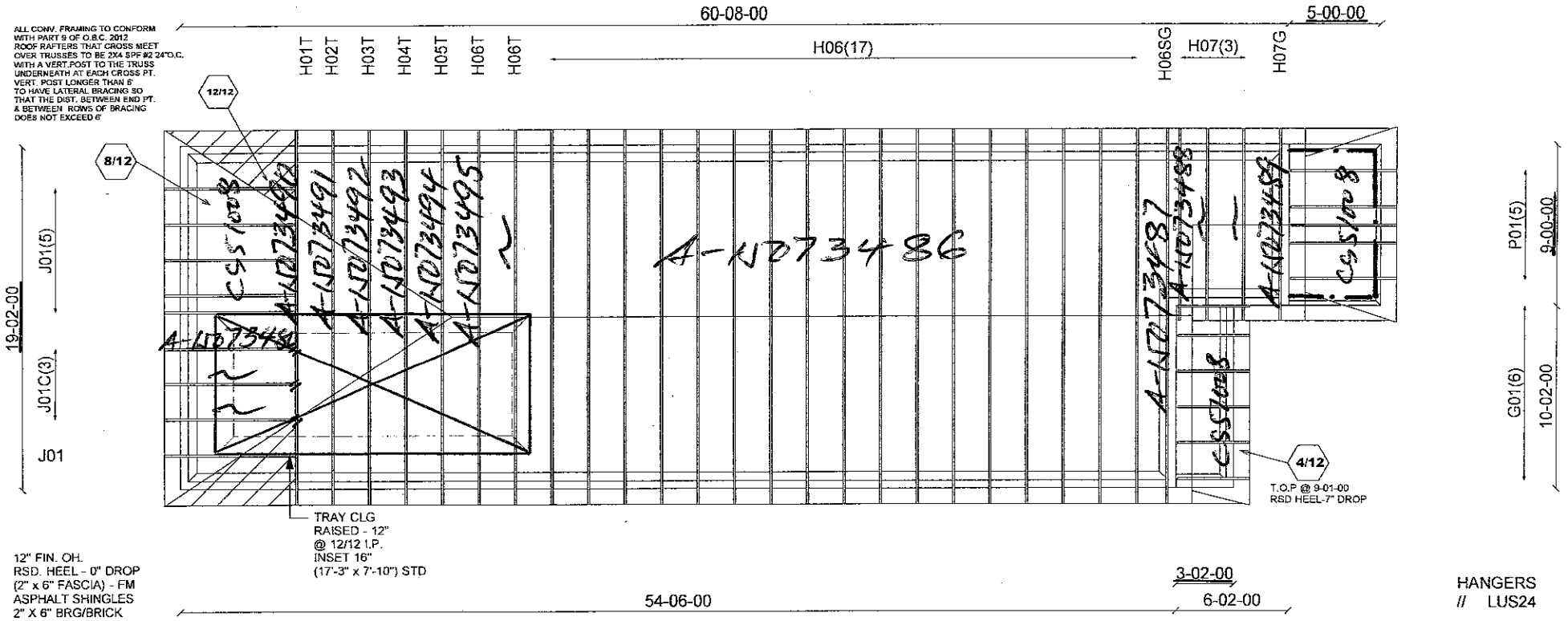
Mitek Ver: 7.5.0

PL 80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 25-3 / A	STD OR OPT 2ND FLR
Layout ID: 252520			
Plan Log: 79711	Project: HUNTINGTON & NASHVILLE		
	Date: 4/16/2015 Designer: cheryle		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



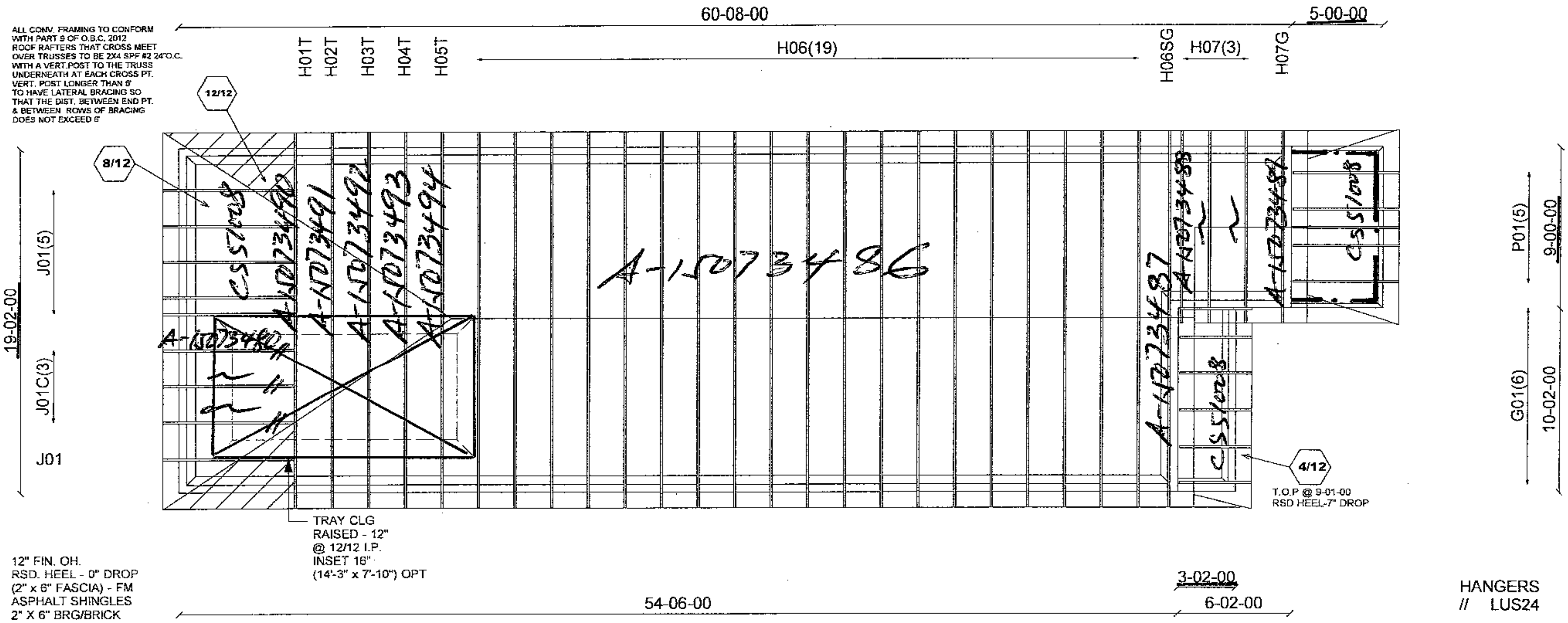
CONVENTIONAL FRAMING BY OTHERS

PL 80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 25-3	/ A OPT TRAY
Layout ID: 252521	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/16/2015 Designer: cheryle		

ALPA-BARRIE



HANGERS
// LUS24

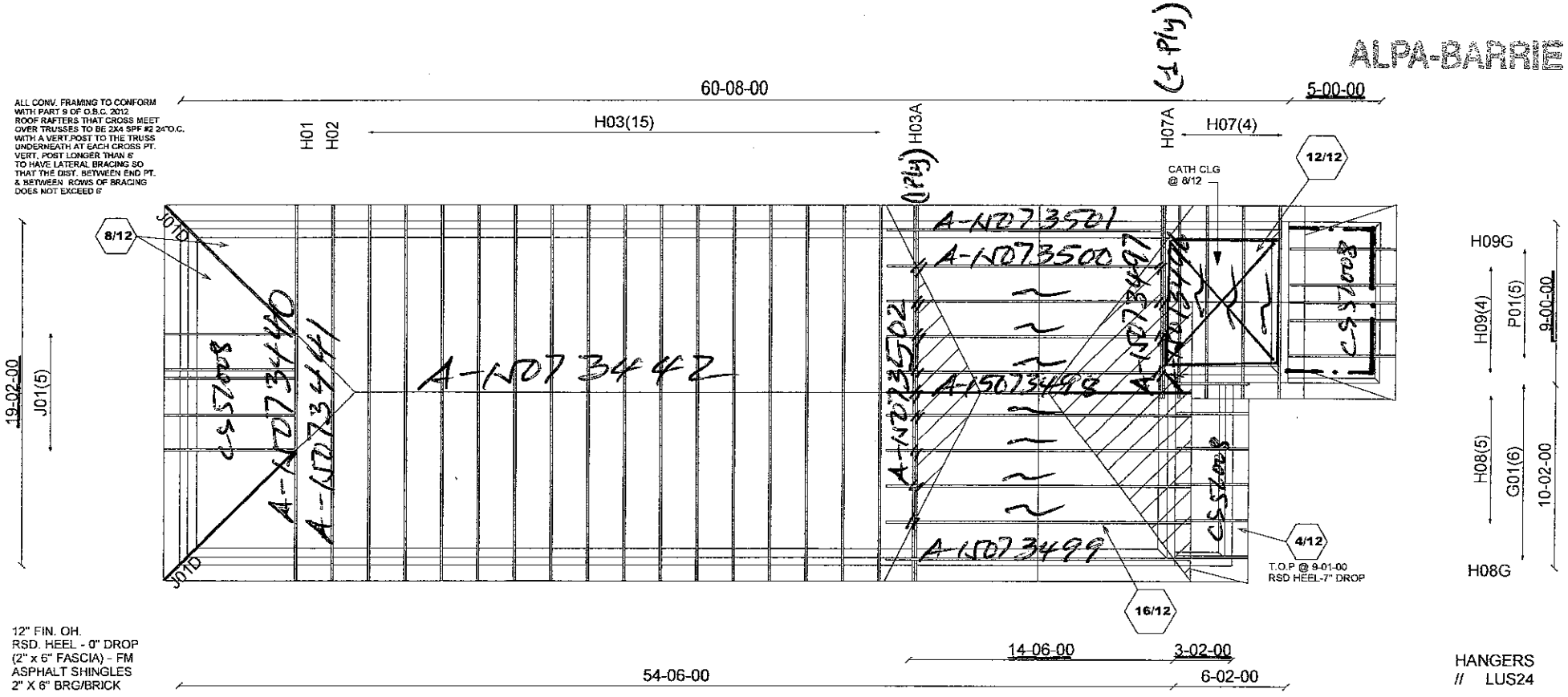
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

PL 80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG		Model / Elevation: 25-3 / A OPT 2ND W/TRAY
Layout ID: 252522	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/16/2015	Designer: cheryle	

ALPA-BARRIE



CONVENTIONAL FRAMING BY OTHERS

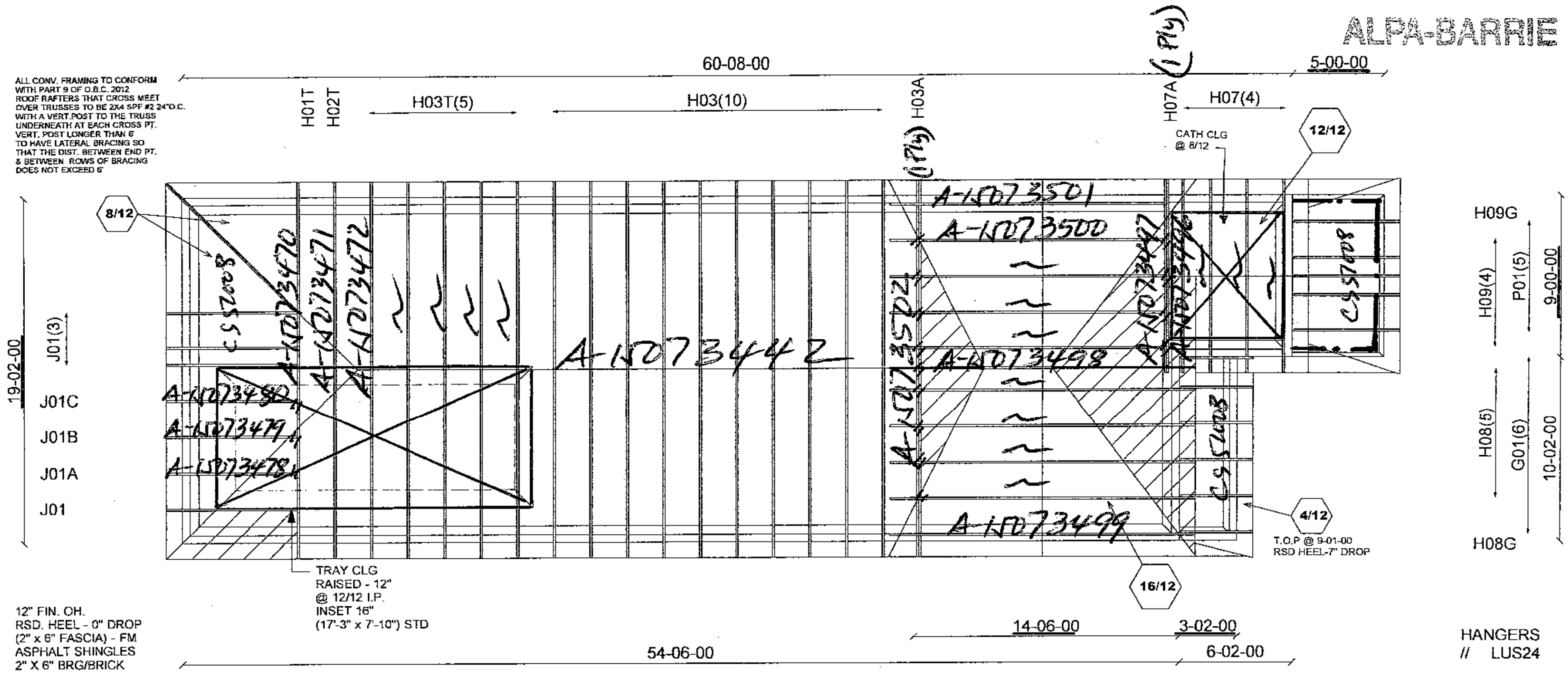
PL 80980

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 25-3 / B	STD OR OPT 2ND FLR
Layout ID: 252523	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/16/2015 Designer: cheryle		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK

HANGERS
// LUS24

CONVENTIONAL FRAMING BY OTHERS

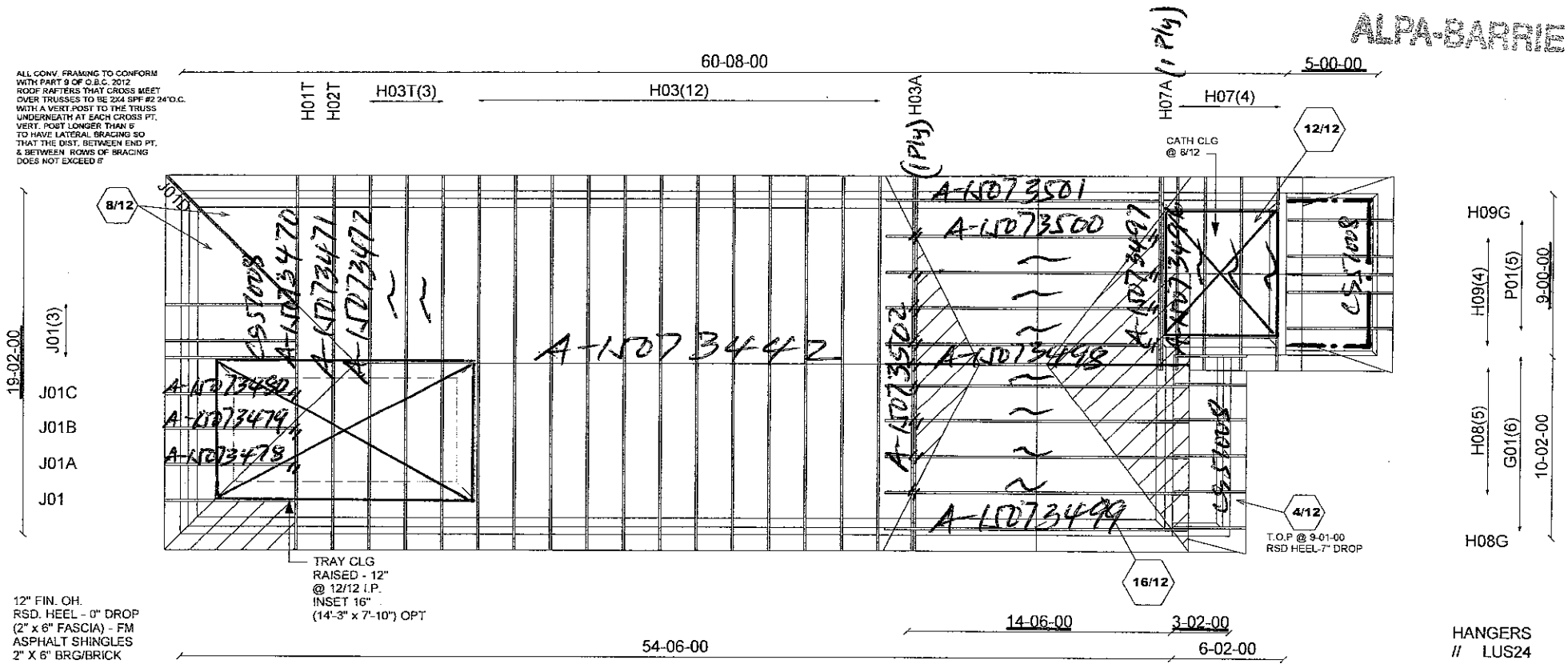
Mitek Ver: 7.5.0

PL 80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 25-3 / B OPT TRAY
Layout ID: 252524	Project: HUNTINGTON & NASHVILLE	
Plan Log: 79711	Date: 4/16/2015 · Designer: cheryle	

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK

HANGERS
// LUS24

CONVENTIONAL FRAMING BY OTHERS

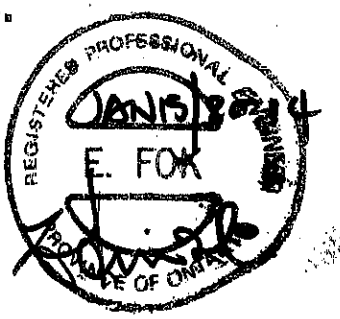
Mitek Ver: 7.5.0

PL 80980

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 25-3	/ B OPT 2ND W/TRAY
Layout ID: 252525	Project: HUNTINGTON & NASHVILLE	THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.	
Plan Log: 79711	Date: 4/16/2015 Designer: cherylie		

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

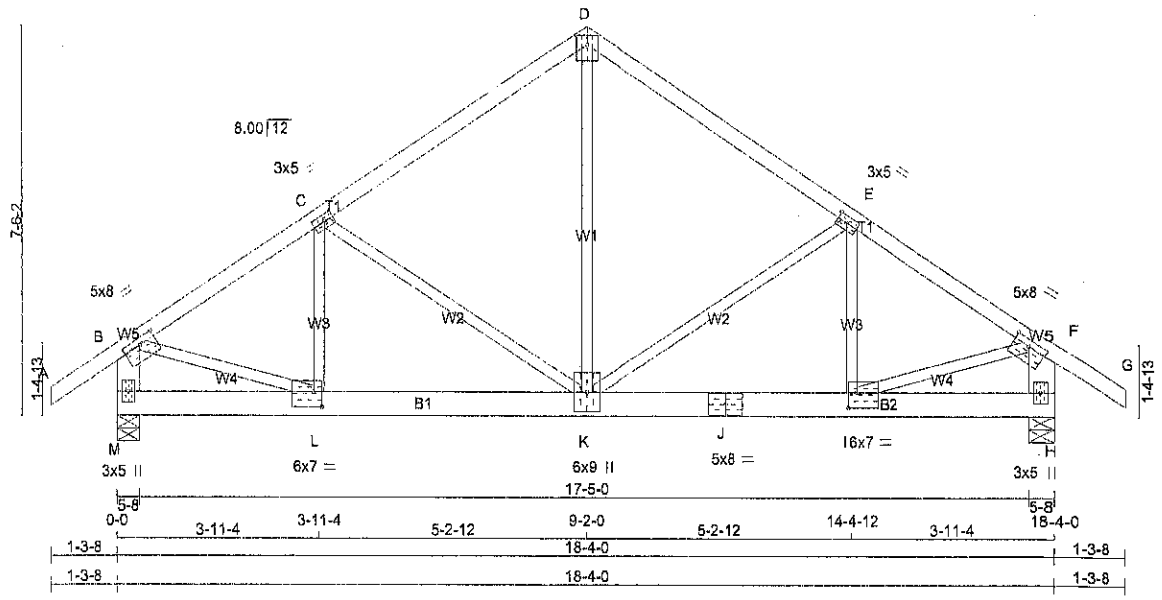
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252523	H03A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-__jLrWaKJL0v2N5GGC5p43uKvPIYIGDb3ydUDMyySE0

5x6 II

Scale = 1:44.7



TOTAL WEIGHT = 91 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.50	3.75
C	TMWW-t	MT20	3.0	5.0	1.50	2.25
D	TTW+p	MT20	5.0	6.0	2.25	2.60
E	TMWW-t	MT20	3.0	5.0	1.50	2.25
F	TMVW-t	MT20	5.0	8.0	1.50	3.75
H	BMV1+p	MT20	3.0	5.0		
I	BMWW-t	MT20	6.0	7.0	3.50	2.00
J	BS-t	MT20	5.0	8.0		
K	BMWWW+t	MT20	6.0	9.0		
L	BMWW-t	MT20	6.0	7.0	3.50	2.00
M	BMV1+p	MT20	3.0	5.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	3254	0	3254	0	0	5-8	5-1
H	3254	0	3254	0	0	5-8	5-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2291	1564 / 0	0 / 0	0 / 0	0 / 0	726 / 0	0 / 0
H	2291	1564 / 0	0 / 0	0 / 0	0 / 0	726 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB. FR-TO	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	0.10 (1)	10.00	K- D	0 / 2534	0.63 (1)
B- C	-3408 / 0	-70.5	-70.5	0.46 (1)	3.38	K- E	-879 / 0	0.64 (1)
C- D	-2579 / 0	-70.5	-70.5	0.45 (1)	3.91	I- E	0 / 589	0.15 (1)
D- E	-2579 / 0	-70.5	-70.5	0.45 (1)	3.91	C- K	-879 / 0	0.64 (1)
E- F	-3408 / 0	-70.5	-70.5	0.46 (1)	3.38	L- C	0 / 589	0.15 (1)
F- G	0 / 27	-70.5	-70.5	0.10 (1)	10.00	B- L	0 / 2945	0.73 (1)
M- B	-2821 / 0	0.0	0.0	0.20 (1)	6.20	I- F	0 / 2945	0.73 (1)
H- F	-2821 / 0	0.0	0.0	0.20 (1)	6.20			
M- L	0 / 0	-274.0	-274.0	0.31 (1)	10.00			
L- K	0 / 2855	-274.0	-274.0	0.87 (1)	10.00			
K- J	0 / 2855	-274.0	-274.0	0.87 (1)	10.00			
J- I	0 / 2855	-274.0	-274.0	0.87 (1)	10.00			
I- H	0 / 0	-274.0	-274.0	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 13-8-0
END DISTANCE = 18-4-0
END SPAN CARRIED = 13-8-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.46 (B-C:1) , BC=0.87 (K-L:1) , WB=0.73 (B-L:1) , SSI=0.51 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.75 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/13/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-H073502

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252523	H09G	1	1	TRUSS DESC.	

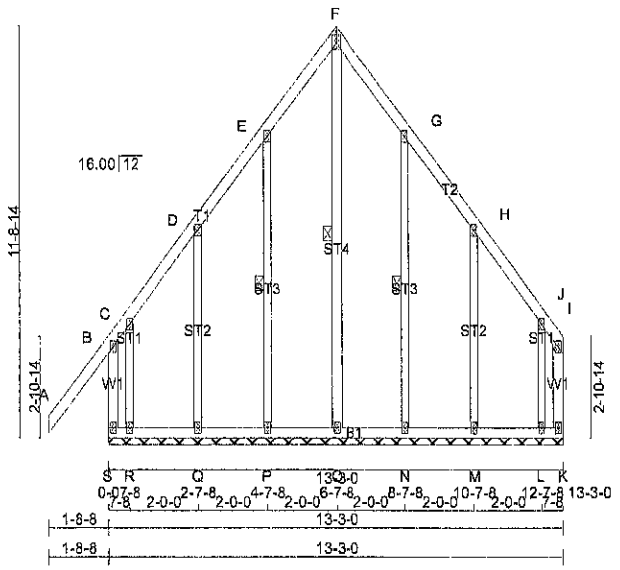
Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Mon Jul 13 15:18:03 2015 Page 1

ID:RvT5uBQd627LEbyHCUAd8syFMxT-k8Vp0_W?ZWsbOvDbjFhU1J1C8z?493O3ulD87KyySSI

3x5 II

Scale = 1:66.1



TOTAL WEIGHT = 91 lb [M]

LUMBER

N, L, G, A, RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - F	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

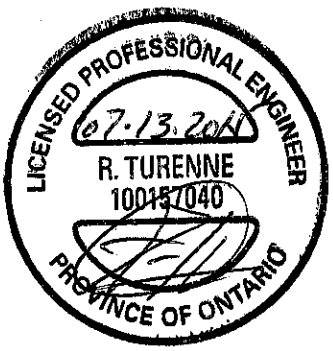
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X	
B	TMV+p	MT20	2.0	4.0			
C, D, E, G, H, I							
C	TMW+w	MT20	2.0	4.0			
F	TTW+p	MT20	3.0	5.0	2.00	Edge	
J	TMV+p	MT20	2.0	4.0			
K	BMV1+p	MT20	2.0	4.0			
L, M, N, O, P, Q, R							
L	BMW1+w	MT20	2.0	4.0			
S	BMV1+p	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT K FOR 160 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT R FOR 150 LBS FACTORED UPLIFT
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-O, E-P, G-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 51	-70.5	-70.5	0.17 (1)	10.00	O- F	-176 / 0	0.12 (1)
B- C	-103 / 0	-70.5	-70.5	0.13 (1)	6.25	P- E	-143 / 0	0.08 (1)
C- D	0 / 9	-70.5	-70.5	0.03 (1)	10.00	Q- D	-137 / 0	0.09 (1)
D- E	0 / 14	-70.5	-70.5	0.04 (1)	10.00	R- C	0 / 54	0.01 (1)
E- F	0 / 17	-70.5	-70.5	0.04 (1)	10.00	N- G	-141 / 0	0.07 (1)
F- G	0 / 17	-70.5	-70.5	0.04 (1)	10.00	M- H	-142 / 0	0.09 (1)
G- H	0 / 13	-70.5	-70.5	0.04 (1)	10.00	L- I	-108 / 0	0.02 (1)
H- I	0 / 12	-70.5	-70.5	0.04 (1)	10.00			
I- J	0 / 25	-70.5	-70.5	0.03 (1)	10.00			
S- B	-281 / 0	0.0	0.0	0.03 (1)	7.81			
K- J	0 / 13	0.0	0.0	0.01 (1)	10.00			
S- R	-7 / 0	-17.5	-17.5	0.01 (4)	10.00			
R- Q	-5 / 0	-17.5	-17.5	0.01 (4)	10.00			
Q- P	-8 / 0	-17.5	-17.5	0.01 (4)	10.00			
P- O	-10 / 0	-17.5	-17.5	0.01 (4)	10.00			
O- N	-10 / 0	-17.5	-17.5	0.01 (4)	10.00			
N- M	-8 / 0	-17.5	-17.5	0.01 (4)	10.00			
M- L	-5 / 0	-17.5	-17.5	0.01 (4)	10.00			
L- K	0 / 0	-17.5	-17.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.01 (P-Q:4) , WB=0.12 (F-O:1) , SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (J) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

A-N073501

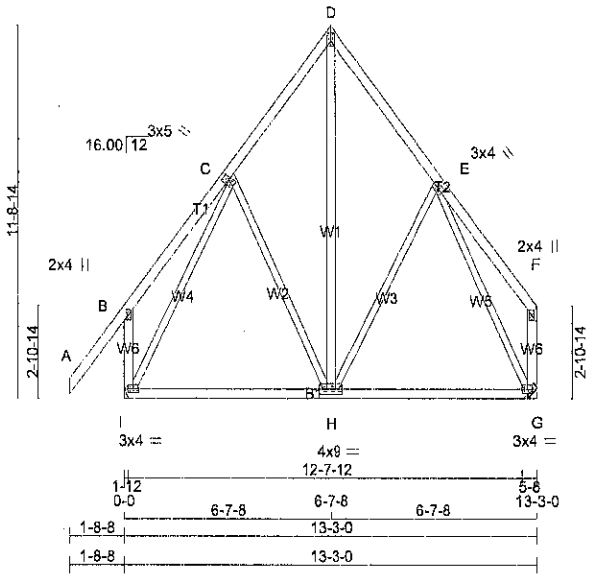
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252523	H09	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-GyxRpeVNoCkknmfP9YAFV6U1OZdNQU4vf5TabuyySSJ

3x5 ||

Scale = 1:73.0



TOTAL WEIGHT = 4 X 86 = 345 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW+t	MT20	3.0	5.0	2.25	0.75
D	TTW+p	MT20	3.0	5.0	2.00	Edge
E	TMWW-t	MT20	3.0	4.0	1.50	1.75
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	711	0	711	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	583	0	583	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	498	355 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0
G	411	278 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 51	-70.5	-70.5	0.17 (1)	10.00	C- H	-82 / 9	0.10 (1)
B- C	0 / 22	-70.5	-70.5	0.13 (1)	10.00	H- D	0 / 285	0.05 (1)
C- D	-318 / 0	-70.5	-70.5	0.10 (1)	6.25	H- E	-77 / 9	0.09 (1)
D- E	-318 / 0	-70.5	-70.5	0.10 (1)	6.25	I- C	-500 / 0	0.58 (1)
E- F	0 / 23	-70.5	-70.5	0.13 (1)	10.00	E- G	-504 / 0	0.51 (1)
I- B	-215 / 0	0.0	0.0	0.03 (1)	7.81			
G- F	-82 / 0	0.0	0.0	0.01 (1)	7.81			
I- H	0 / 214	-17.5	-17.5	0.24 (4)	10.00			
H- G	0 / 214	-17.5	-17.5	0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.17 (A-B:1) , BC=0.24 (H-I:4) , WB=0.58 (C-I:1) , SSI=0.10 (G-H:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

A-1073500

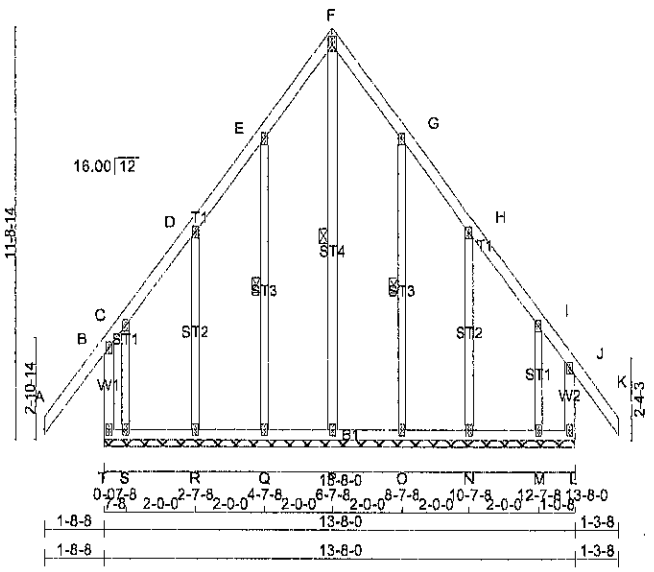
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252523	H08G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-oIN3biUl1vct9c4Dbqf0yuyse9Kch8fmQRk13SyySSK

3x5 =

Scale = 1:66.1



TOTAL WEIGHT = 94 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
P - F	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	3.0	5.0	2.00	Edge
J	TMV+p	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		
M, N, O, P, Q, R, S						
M	BMW1+w	MT20	2.0	4.0		
T	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-P, E-Q, G-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 51	-70.5	-70.5	0.17 (1)	10.00
B- C	-75 / 0	-70.5	-70.5	0.11 (1)	6.25
C- D	0 / 23	-70.5	-70.5	0.03 (1)	10.00
D- E	0 / 28	-70.5	-70.5	0.04 (1)	10.00
E- F	0 / 30	-70.5	-70.5	0.04 (1)	10.00
F- G	0 / 30	-70.5	-70.5	0.04 (1)	10.00
G- H	0 / 27	-70.5	-70.5	0.04 (1)	10.00
H- I	0 / 27	-70.5	-70.5	0.04 (1)	10.00
I- J	-11 / 0	-70.5	-70.5	0.06 (1)	6.25
J- K	0 / 40	-70.5	-70.5	0.10 (1)	10.00
T- B	-252 / 0	0.0	0.0	0.05 (1)	7.81
L- J	-154 / 0	0.0	0.0	0.03 (1)	7.81
T- S	-15 / 0	-17.5	-17.5	0.01 (4)	6.25
S- R	-13 / 0	-17.5	-17.5	0.01 (4)	6.25
R- Q	-16 / 0	-17.5	-17.5	0.01 (4)	6.25
Q- P	-18 / 0	-17.5	-17.5	0.01 (4)	6.25
P- O	-18 / 0	-17.5	-17.5	0.01 (4)	6.25
O- N	-16 / 0	-17.5	-17.5	0.01 (4)	6.25
N- M	-13 / 0	-17.5	-17.5	0.01 (4)	6.25
M- L	-11 / 0	-17.5	-17.5	0.01 (4)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-198 / 0	0.14 (1)
Q- E	-144 / 0	0.08 (1)
R- D	-137 / 0	0.09 (1)
S- C	0 / 36	0.01 (1)
O- G	-142 / 0	0.07 (1)
N- H	-144 / 0	0.09 (1)
M- I	-56 / 0	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.01 (N-O:4) , WB=0.14 (F-P:1) , SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

A-1073499

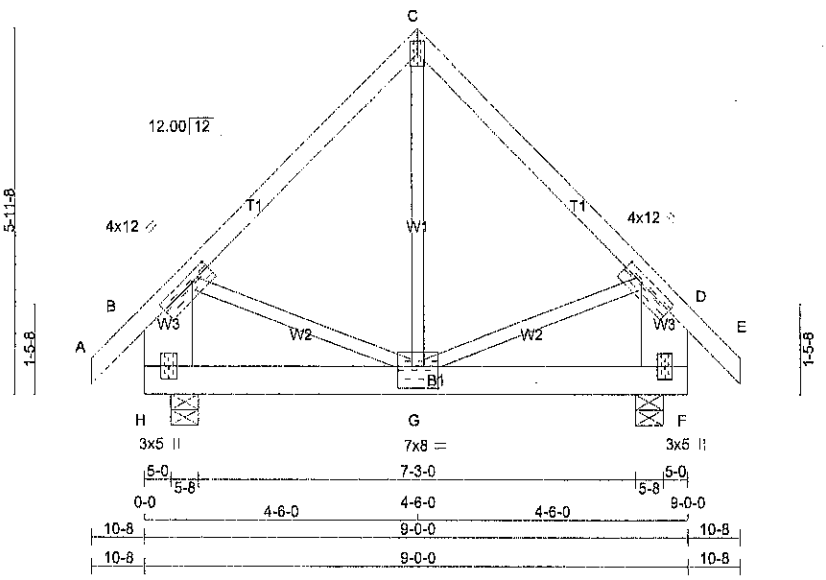
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252523	H07A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-KZpgOyT7GbU0XSV027BnQhPgemunyfEcCn_UW0yySSL

3x5 II

Scale = 1:37.7



TOTAL WEIGHT = 53 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x10	DRY	No.2	SPF
F - D	2x10	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

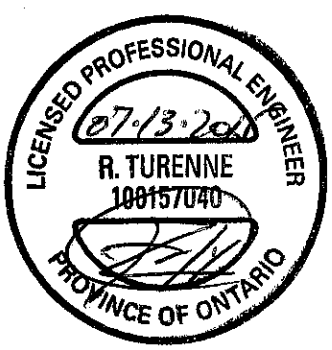
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	12.0	1.50	4.25
C	TTW+p	MT20	3.0	5.0		
D	TMVW-t	MT20	4.0	12.0	1.50	4.25
F	BMV1+p	MT20	3.0	5.0		
G	BMWWW-t	MT20	7.0	8.0	4.25	4.00
H	BMV1+p	MT20	3.0	5.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1578	0	1578	0	0	5-8	1-11
F	1578	0	1578	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1110	761 / 0	0 / 0	0 / 0	0 / 0	349 / 0	0 / 0
F	1110	761 / 0	0 / 0	0 / 0	0 / 0	349 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB. FR-TO	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI
A- B	0 / 24	-70.5	-70.5	0.05 (1)		10.00	G- C	0 / 1127	0.28 (1)
B- C	-1021 / 0	-70.5	-70.5	0.31 (1)		5.79	B- G	0 / 740	0.18 (1)
C- D	-1021 / 0	-70.5	-70.5	0.31 (1)		5.79	G- D	0 / 740	0.18 (1)
D- E	0 / 24	-70.5	-70.5	0.05 (1)		10.00			
H- B	-1114 / 0	0.0	0.0	0.05 (1)		7.81			
F- D	-1114 / 0	0.0	0.0	0.05 (1)		7.81			
H- G	0 / 0	-264.9	-264.9	0.44 (1)		10.00			
G- F	0 / 0	-264.9	-264.9	0.44 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 13-3-0
END DISTANCE = 9-0-0
END SPAN CARRIED = 13-3-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.31 (B-C:1) , BC=0.44 (F-G:1) , WB=0.28 (C-G:1) , SSI=0.48 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

A-1073497

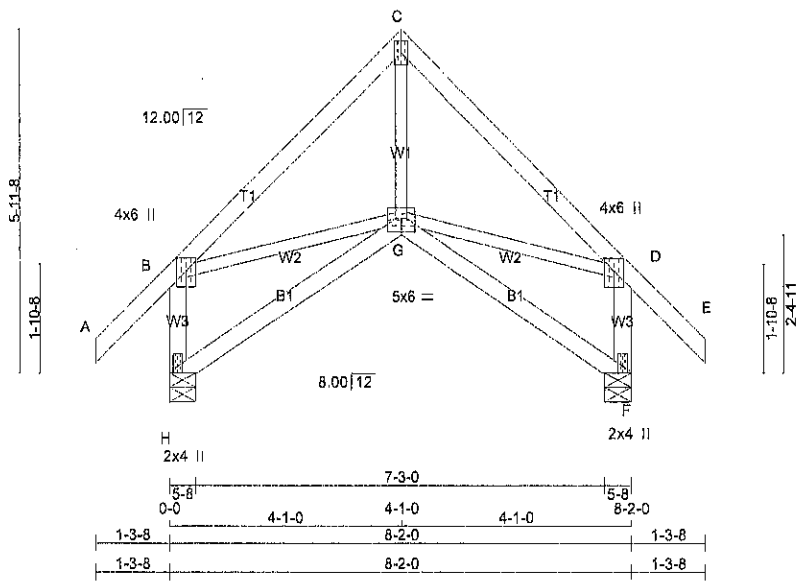
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252523	H07	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-KZpgOyT7GbU0XSV0278nQhPi8mzGyjmcCn_UW0yySSL

3x5 II

Scale = 1:40.2



TOTAL WEIGHT = 4 X 42 = 167 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	3.0	5.0		
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	6.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	457	0	457	0	0	5-8	1-8
F	457	0	457	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	320	230 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
F	320	230 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 36	-70.5	-70.5	0.10 (1)	10.00	G- C	0 / 172	0.04 (4)
B- C	-325 / 0	-70.5	-70.5	0.15 (1)	6.25	B- G	0 / 236	0.05 (1)
C- D	-325 / 0	-70.5	-70.5	0.15 (1)	6.25	G- D	0 / 236	0.05 (1)
D- E	0 / 36	-70.5	-70.5	0.10 (1)	10.00			
H- B	-422 / 0	0.0	0.0	0.05 (1)	7.81			
F- D	-422 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-17.5	-17.5	0.09 (4)	10.00			
G- F	0 / 0	-17.5	-17.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15 (C-D:1) , BC=0.09 (F-G:4) , WB=0.05 (B-G:1) , SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (H) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.13.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

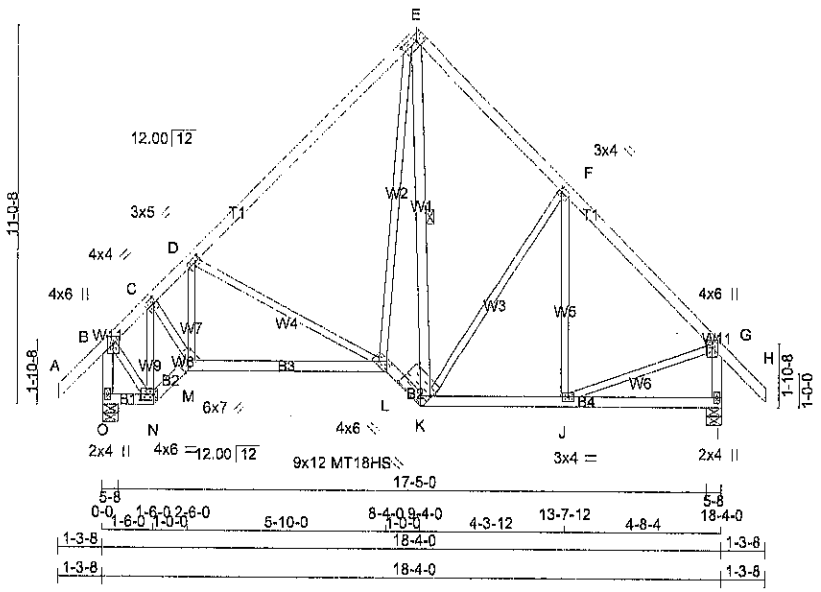
A-N073496

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252521	H06T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-ncCW39wbk4olaceL5b7IfvMrLF2uCsRBmCkle2yySp?

Scale = 1:67.3



TOTAL WEIGHT = 2 X 106 = 213 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMWW-t	MT20	3.0	5.0	1.50	2.00
E	TTWW-I	MT20	4.0	9.0	2.00	5.50
F	TMWW-t	MT20	3.0	4.0	1.50	1.00
G	TMVW+p	MT20	4.0	6.0	2.75	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	3.0	4.0		
K	BBWW-h	MT18HS	9.0	12.0	4.50	11.50
L	BBWW-h	MT20	4.0	6.0	2.75	3.75
M	BBWW-h	MT20	6.0	7.0	Edge	
N	BBWW-I	MT20	4.0	6.0	2.00	4.50
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	908	0	908	0	0	5-8	1-8
I	901	0	901	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	637	445 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
I	632	441 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	0 / 35	-70.5	-70.5	0.10 (1)	10.00	
B-C	-530 / 0	-70.5	-70.5	0.09 (1)	6.25	
C-D	-692 / 0	-70.5	-70.5	0.34 (1)	6.25	
D-E	-595 / 0	-70.5	-70.5	0.39 (1)	6.25	
E-F	-519 / 0	-70.5	-70.5	0.20 (1)	6.25	
F-G	-659 / 0	-70.5	-70.5	0.20 (1)	6.25	
G-H	0 / 35	-70.5	-70.5	0.10 (1)	10.00	
O-B	-895 / 0	0.0	0.0	0.10 (1)	7.81	
I-G	-868 / 0	0.0	0.0	0.10 (1)	7.81	
O-N	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
N-M	0 / 422	-17.5	-17.5	0.07 (1)	10.00	
M-L	0 / 663	-17.5	-17.5	0.22 (4)	10.00	
L-K	0 / 493	-17.5	-17.5	0.08 (1)	10.00	
K-J	0 / 465	-17.5	-17.5	0.14 (4)	10.00	
J-I	0 / 0	-17.5	-17.5	0.10 (4)	10.00	

WEBS MEMB.	FORCE (LBS)	MAX CSI
N-C	-596 / 0	0.11 (1)
C-M	0 / 706	0.16 (1)
M-D	-251 / 18	0.05 (1)
D-L	-303 / 0	0.23 (1)
L-E	0 / 555	0.12 (1)
E-K	-109 / 0	0.07 (1)
K-F	-242 / 0	0.27 (1)
J-F	-65 / 58	0.05 (1)
B-N	0 / 452	0.10 (1)
J-G	0 / 510	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.39 (D-E:1) , BC=0.22 (L-M:4) , WB=0.27 (F-K:1) , SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MT18HS	580	354	2729	1405	4191	2010

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

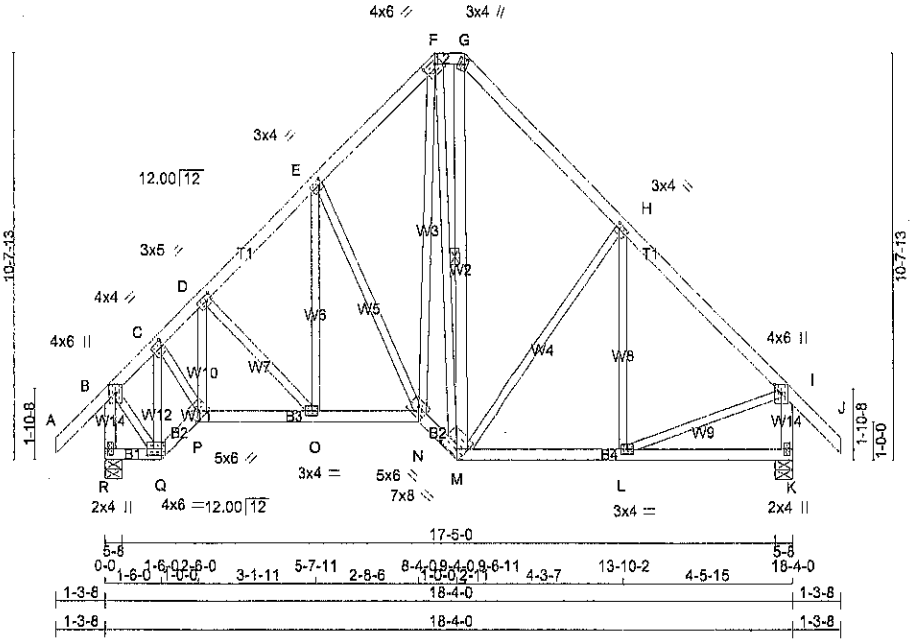
A-N073495

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252521	H05T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-?Sbp_uhkKFGx6dCT7q3aOPqNEqjrqNPR5?2r69yyT9G

Scale = 1:60.6



TOTAL WEIGHT = 122 lb

[M][F]

LUMBER

N. L. G. A. RULES

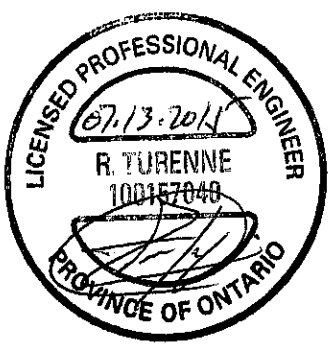
CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMWW-t	MT20	3.0	5.0	1.50	2.00
E	TMWW-t	MT20	3.0	4.0	1.50	1.00
F	TTWW-h	MT20	4.0	6.0	1.25	4.00
G	TTW+m	MT20	3.0	4.0	1.75	1.00
H	TMWW-t	MT20	3.0	4.0	1.50	1.00
I	TMVW+p	MT20	4.0	6.0	2.75	2.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	3.0	4.0		
M	BBWWW-h	MT20	7.0	8.0	2.00	5.25
N	BBWW-h	MT20	5.0	6.0	3.00	3.25
O	BMWW-t	MT20	3.0	4.0		
P	BBWW-h	MT20	5.0	6.0	2.25	3.00
Q	BBWW-l	MT20	4.0	6.0	2.00	4.50
R	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	908	0	908	0	0	5-8	1-8
K	901	0	901	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	637	445 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
K	632	441 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-M.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 35	-70.5	-70.5	0.10 (1)	10.00
B- C	-530 / 0	-70.5	-70.5	0.07 (1)	6.25
C- D	-772 / 0	-70.5	-70.5	0.04 (1)	6.25
D- E	-721 / 0	-70.5	-70.5	0.07 (1)	6.25
E- F	-592 / 0	-70.5	-70.5	0.08 (1)	6.25
F- G	-355 / 0	-70.5	-70.5	0.01 (1)	6.25
G- H	-524 / 0	-70.5	-70.5	0.18 (1)	6.25
H- I	-661 / 0	-70.5	-70.5	0.18 (1)	6.25
I- J	0 / 35	-70.5	-70.5	0.10 (1)	10.00
R- B	-895 / 0	0.0	0.0	0.10 (1)	7.81
K- I	-870 / 0	0.0	0.0	0.10 (1)	7.81
R- Q	0 / 0	-17.5	-17.5	0.01 (4)	10.00
Q- P	0 / 450	-17.5	-17.5	0.08 (1)	10.00
P- O	0 / 572	-17.5	-17.5	0.11 (1)	10.00
O- N	0 / 515	-17.5	-17.5	0.10 (1)	10.00
N- M	0 / 530	-17.5	-17.5	0.09 (1)	10.00
M- L	0 / 485	-17.5	-17.5	0.14 (4)	10.00
L- K	0 / 0	-17.5	-17.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 498	0.11 (1)
P- D	-73 / 11	0.01 (1)
D- O	-81 / 0	0.03 (1)
O- E	0 / 105	0.03 (4)
E- N	-272 / 0	0.24 (1)
N- F	0 / 659	0.15 (1)
F- M	-366 / 0	0.26 (1)
L- H	-72 / 55	0.05 (1)
B- Q	0 / 481	0.11 (1)
L- I	0 / 513	0.12 (1)
G- M	0 / 213	0.03 (1)
M- H	-225 / 0	0.24 (1)
Q- C	-636 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18 (H-I:1) , BC=0.14 (L-M:4) , WB=0.28 (F-M:1) , SSI=0.11 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)

A-N073494

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252521	H04T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:RvT5uBQd627LEbyHCUAd8syFMxT-XG1RnYg6Zx84VTeHZ6YLsBIBDQLc5rVisLJHaIyyT9r

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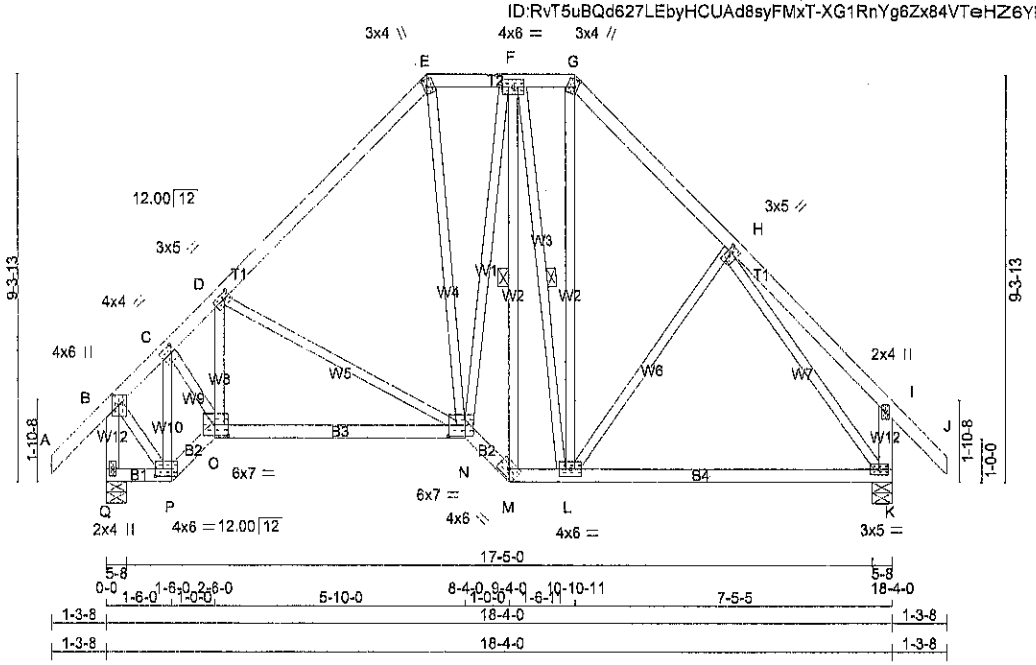

TOTAL WEIGHT = 115 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMWW-t	MT20	3.0	5.0	1.50	2.00
E	TTW+m	MT20	3.0	4.0	2.00	1.00
F	TMWWW-t	MT20	4.0	6.0		
G	TTW+m	MT20	3.0	4.0	2.00	1.00
H	TMWW+t	MT20	3.0	6.0	1.75	1.00
I	TMV+p	MT20	2.0	4.0		
K	BMVW1-t	MT20	3.0	5.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BBW-h	MT20	4.0	6.0		
N	BBWWW-p	MT20	6.0	7.0	3.00	4.75
O	BBWW-t	MT20	6.0	7.0	3.00	3.75
P	BBWW-t	MT20	4.0	6.0	2.00	4.50
Q	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	908	0	908	0	0	5-8	1-8
K	901	0	901	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	637	445 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
K	632	441 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-M, F-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 35	-70.5	-70.5	0.10 (1)	10.00
B- C	-530 / 0	-70.5	-70.5	0.07 (1)	6.25
C- D	-735 / 0	-70.5	-70.5	0.18 (1)	6.25
D- E	-628 / 0	-70.5	-70.5	0.20 (1)	6.25
E- F	-451 / 0	-70.5	-70.5	0.04 (1)	6.25
F- G	-404 / 0	-70.5	-70.5	0.02 (1)	6.25
G- H	-592 / 0	-70.5	-70.5	0.12 (1)	6.25
H- I	0 / 21	-70.5	-70.5	0.16 (1)	10.00
I- J	0 / 35	-70.5	-70.5	0.10 (1)	10.00
Q- B	-895 / 0	0.0	0.0	0.10 (1)	7.81
K- I	-199 / 0	0.0	0.0	0.02 (1)	7.81
Q- P	0 / 0	-17.5	-17.5	0.01 (4)	10.00
P- O	0 / 437	-17.5	-17.5	0.07 (1)	10.00
O- N	0 / 613	-17.5	-17.5	0.22 (4)	10.00
N- M	0 / 570	-17.5	-17.5	0.09 (1)	10.00
M- L	0 / 407	-17.5	-17.5	0.26 (4)	10.00
L- K	0 / 460	-17.5	-17.5	0.27 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-618 / 0	0.12 (1)
C- O	0 / 593	0.13 (1)
O- D	-143 / 30	0.03 (1)
D- N	-213 / 0	0.16 (1)
E- N	0 / 221	0.05 (1)
N- F	0 / 348	0.08 (1)
M- F	-436 / 0	0.25 (1)
F- L	-27 / 53	0.02 (4)
L- G	0 / 251	0.06 (1)
B- P	0 / 468	0.11 (1)
L- H	-99 / 11	0.08 (1)
H- K	-794 / 0	0.58 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.20 (D-E:1) , BC=0.27 (K-L:4) , WB=0.58 (H-K:1) , SSI=0.12 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.38 (H) (INPUT = 1.00)

A-N073493

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252521	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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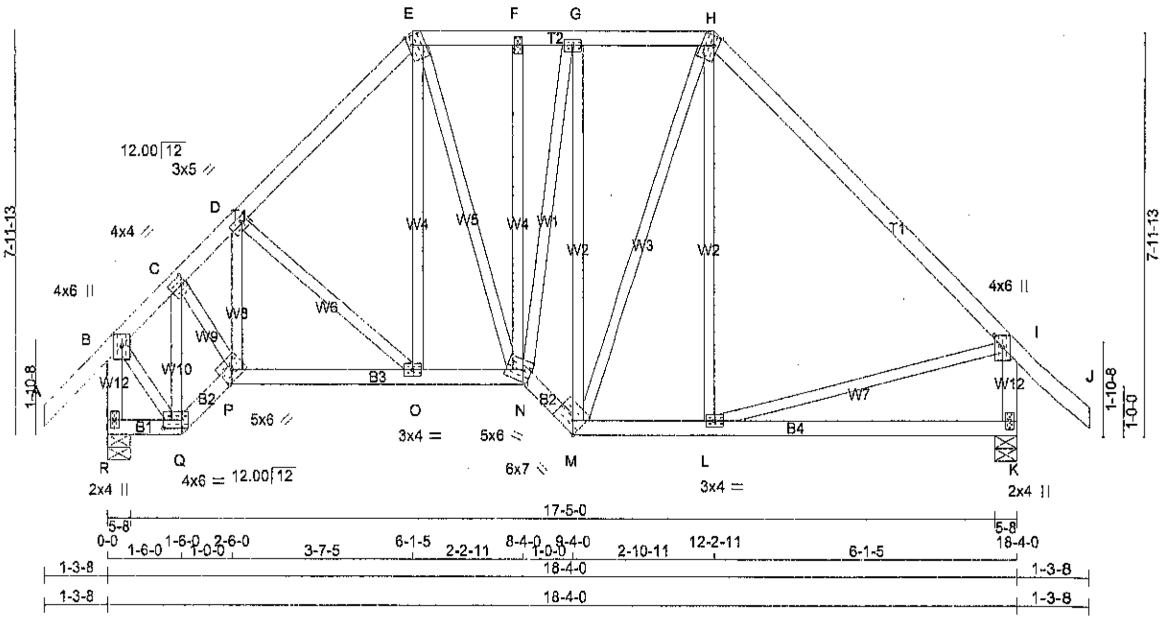

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMWW-t	MT20	3.0	5.0	1.50	2.00
E	TTWW+m	MT20	4.0	6.0	Edge	1.00
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	3.0	4.0		
H	TTWW+m	MT20	4.0	6.0	Edge	1.00
I	TMVW+p	MT20	4.0	6.0	2.75	2.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	3.0	4.0		
M	BBWW-h	MT20	6.0	7.0	Edge	2.50
N	BBWWW-m	MT20	5.0	6.0	2.75	2.00
O	BMWW-t	MT20	3.0	4.0		
P	BBWW-h	MT20	5.0	6.0	2.25	3.00
Q	BBWW-l	MT20	4.0	6.0	2.00	4.50
R	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	908	0	908	0	0	5-8	1-8
K	901	0	901	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
R	637	445 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
K	632	441 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 35	-70.5	-70.5	0.10 (1)	10.00
B- C	-530 / 0	-70.5	-70.5	0.07 (1)	6.25
C- D	-759 / 0	-70.5	-70.5	0.09 (1)	6.25
D- E	-708 / 0	-70.5	-70.5	0.10 (1)	6.25
E- F	-544 / 0	-70.5	-70.5	0.05 (1)	6.25
F- G	-543 / 0	-70.5	-70.5	0.05 (1)	6.25
G- H	-485 / 0	-70.5	-70.5	0.06 (1)	6.25
H- I	-615 / 0	-70.5	-70.5	0.35 (1)	6.25
I- J	0 / 35	-70.5	-70.5	0.10 (1)	10.00
R- B	-895 / 0	0.0	0.0	0.10 (1)	7.81
K- I	-856 / 0	0.0	0.0	0.10 (1)	7.81
R- Q	0 / 0	-17.5	-17.5	0.01 (4)	10.00
Q- P	0 / 446	-17.5	-17.5	0.07 (1)	10.00
P- O	0 / 587	-17.5	-17.5	0.12 (1)	10.00
O- N	0 / 489	-17.5	-17.5	0.11 (1)	10.00
N- M	0 / 675	-17.5	-17.5	0.11 (1)	10.00
M- L	0 / 434	-17.5	-17.5	0.18 (4)	10.00
L- K	0 / 0	-17.5	-17.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-630 / 0	0.12 (1)
C- P	0 / 531	0.12 (1)
P- D	-103 / 12	0.02 (1)
D- O	-132 / 0	0.05 (1)
O- E	0 / 140	0.03 (4)
E- N	0 / 171	0.04 (1)
N- F	-56 / 2	0.05 (1)
N- G	0 / 401	0.09 (1)
M- G	-597 / 0	0.72 (1)
M- H	0 / 145	0.03 (1)
L- H	0 / 87	0.03 (4)
B- Q	0 / 477	0.11 (1)
L- I	0 / 447	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.35 (H-I:1) , BC=0.18 (L-M:4) , WB=0.72 (G-M:1) , SSI=0.12 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

A-15073492

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252521	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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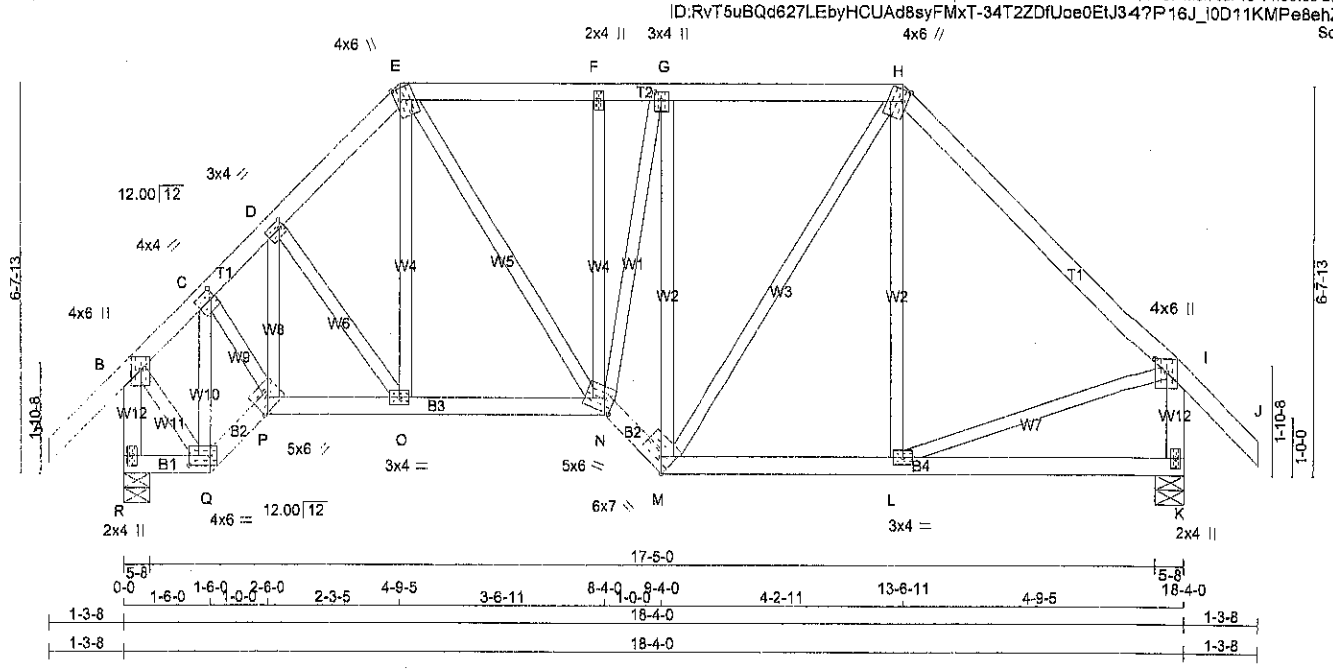

TOTAL WEIGHT = 103 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMWW-t	MT20	3.0	4.0	1.50	1.00
E	TTWW+m	MT20	4.0	6.0	Edge	1.00
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	3.0	4.0	1.75	1.50
H	TTWW+m	MT20	4.0	6.0	Edge	1.00
I	TMVW+p	MT20	4.0	6.0	2.75	2.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	3.0	4.0		
M	BBWW-h	MT20	6.0	7.0	Edge	2.50
N	BBWWW-m	MT20	5.0	6.0	2.75	2.00
O	BMWW-t	MT20	3.0	4.0		
P	BBWW-h	MT20	5.0	6.0	2.25	3.00
Q	BBWW-l	MT20	4.0	6.0	2.00	4.50
R	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	908	0	908	0	0	5-8	1-8
K	901	0	901	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	637	445 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
K	632	441 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 35	-70.5	-70.5	0.10 (1)		10.00
B- C	-530 / 0	-70.5	-70.5	0.07 (1)		6.25
C- D	-775 / 0	-70.5	-70.5	0.03 (1)		6.25
D- E	-758 / 0	-70.5	-70.5	0.04 (1)		6.25
E- F	-691 / 0	-70.5	-70.5	0.11 (1)		6.25
F- G	-689 / 0	-70.5	-70.5	0.12 (1)		6.25
G- H	-598 / 0	-70.5	-70.5	0.13 (1)		6.25
H- I	-646 / 0	-70.5	-70.5	0.21 (1)		6.25
I- J	0 / 35	-70.5	-70.5	0.10 (1)		10.00
R- B	-895 / 0	0.0	0.0	0.10 (1)		7.81
K- I	-867 / 0	0.0	0.0	0.10 (1)		7.81
R- Q	0 / 0	-17.5	-17.5	0.01 (4)		10.00
Q- P	0 / 451	-17.5	-17.5	0.08 (1)		10.00
P- O	0 / 570	-17.5	-17.5	0.12 (1)		10.00
O- N	0 / 529	-17.5	-17.5	0.11 (1)		10.00
N- M	0 / 829	-17.5	-17.5	0.14 (1)		10.00
M- L	0 / 454	-17.5	-17.5	0.14 (4)		10.00
L- K	0 / 0	-17.5	-17.5	0.10 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-638 / 0	0.12 (1)
C- P	0 / 492	0.11 (1)
P- D	-77 / 3	0.02 (1)
D- O	-73 / 0	0.02 (1)
O- E	0 / 111	0.03 (4)
E- N	0 / 291	0.07 (1)
N- F	-112 / 0	0.05 (1)
N- G	0 / 506	0.11 (1)
M- G	-763 / 0	0.56 (1)
M- H	0 / 259	0.06 (1)
L- H	-48 / 62	0.03 (1)
B- Q	0 / 482	0.11 (1)
L- I	0 / 477	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.21 (H-I:1) , BC=0.14 (L-M:4) , WB=0.56 (G-M:1) , SSI=0.13 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)

A-2073491

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252521	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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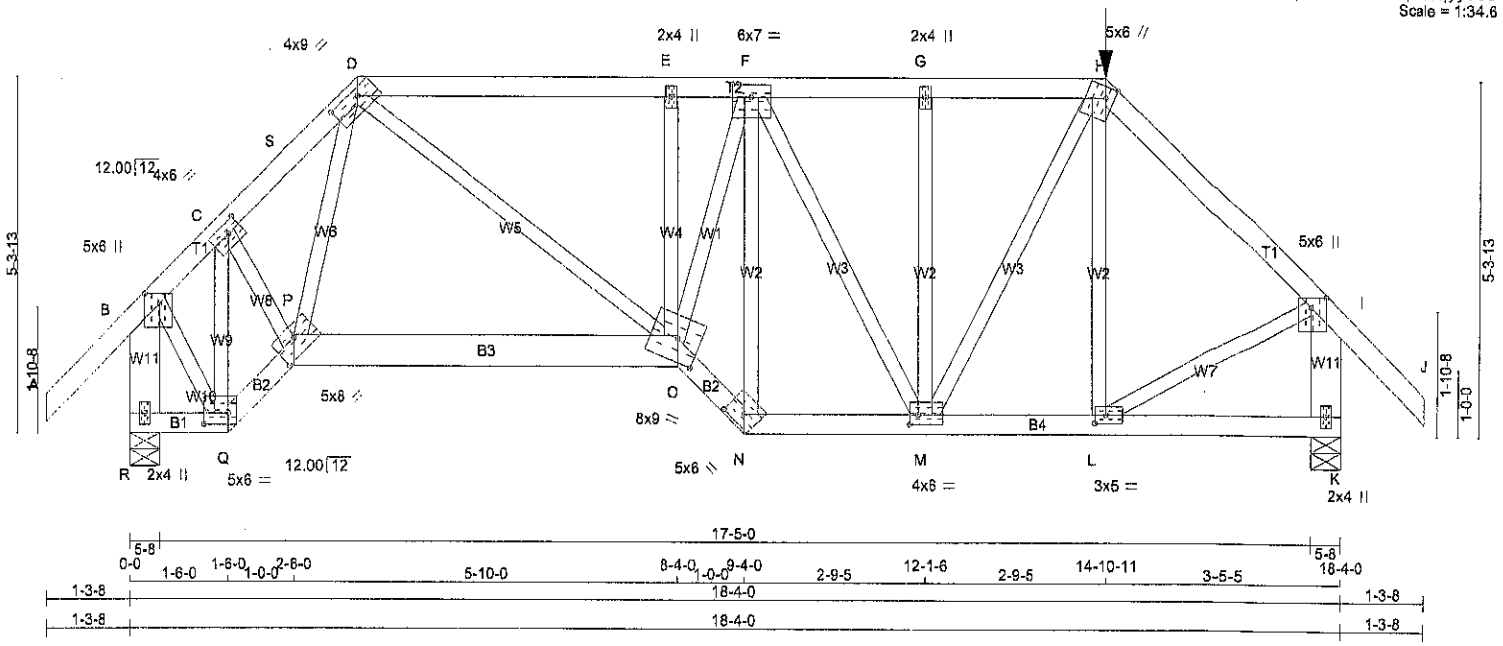

TOTAL WEIGHT = 103 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	6.0	1.75	2.50
D	TTWW-h	MT20	4.0	9.0	1.25	5.50
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	6.0	7.0	2.25	3.50
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.00	1.50
I	TMVW+p	MT20	5.0	6.0	1.75	2.75
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	3.0	5.0	1.50	2.00
M	BMWWW-t	MT20	4.0	6.0	1.75	1.50
N	BBW-h	MT20	5.0	6.0	2.00	3.25
O	BBWWW-m	MT20	8.0	9.0	4.00	4.00
P	BBWW-h	MT20	5.0	8.0	3.00	4.00
Q	BBWW-I	MT20	5.0	6.0	2.00	4.50
R	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1704	0	1704	0	0	5-8	1-14
K	1616	0	1616	0	0	5-8	1-12

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1198	827 / 0	0 / 0	0 / 0	0 / 0	371 / 0	0 / 0
K	1138	774 / 0	0 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 35	-70.5	-70.5	0.11 (1)		10.00
B-C	-1051 / 0	-138.7	-138.7	0.12 (1)		5.98
C-S	-1619 / 0	-138.7	-138.7	0.13 (1)		5.05
S-D	-1619 / 0	-70.5	-70.5	0.13 (1)		5.05
D-E	-1878 / 0	-70.5	-70.5	0.30 (1)		4.57
E-F	-1871 / 0	-138.7	-138.7	0.20 (1)		4.57
F-G	-1341 / 0	-138.7	-138.7	0.16 (1)		5.39
G-H	-1341 / 0	-138.7	-138.7	0.16 (1)		5.39
H-I	-1331 / 0	-70.5	-70.5	0.19 (1)		5.38
I-J	0 / 35	-70.5	-70.5	0.11 (1)		10.00
R-B	-1676 / 0	0.0	0.0	0.13 (1)		7.60
K-I	-1568 / 0	0.0	0.0	0.13 (1)		7.80
R-Q	0 / 0	-34.5	-34.5	0.02 (4)		10.00
Q-P	0 / 965	-34.5	-34.5	0.18 (1)		10.00
P-O	0 / 1070	-102.7	-102.7	0.46 (1)		10.00
O-N	0 / 2103	-34.5	-34.5	0.38 (1)		10.00
N-M	0 / 1486	-34.5	-34.5	0.32 (1)		10.00
M-L	0 / 932	-34.5	-34.5	0.23 (1)		10.00
L-K	0 / 0	-34.5	-34.5	0.10 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-1368 / 0	0.26 (1)
C-P	0 / 896	0.22 (1)
P-D	0 / 251	0.06 (1)
D-O	0 / 1037	0.26 (1)
O-E	-298 / 0	0.08 (1)
O-F	0 / 1540	0.38 (1)
N-F	-1433 / 0	0.62 (1)
F-M	-309 / 0	0.17 (1)
M-G	-408 / 0	0.18 (1)
L-H	-262 / 15	0.11 (1)
B-Q	0 / 1013	0.25 (1)
L-I	0 / 1020	0.25 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
H	14-10-11	-177	-177	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 8-4-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-5-5
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-0-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.30 (D-E:1) , BC=0.46 (O-P:1) , WB=0.62 (F-N:1) , SSI=0.20 (O-P:1)

A-N073490

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 176.9 lbs FACTORED DOWN AT 14-10-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (R) (INPUT = 0.80)
JSI METAL= 0.31 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/13/20
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073490(2)

JOB-NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H07G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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3x4 ||

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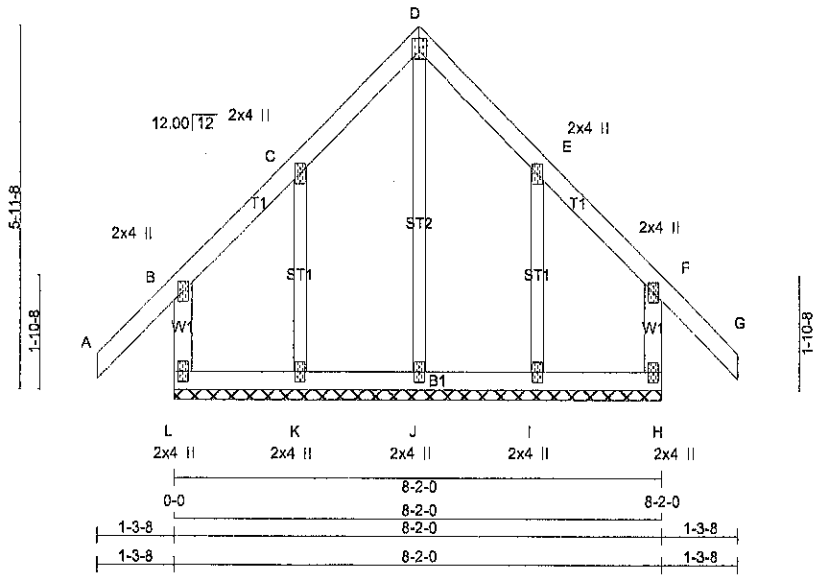

TOTAL WEIGHT = 40 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	4.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	2.0	4.0		
H	BMV1+p	MT20	2.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
L- B	-180 / 0	0.0	0.0	0.01 (1)		7.81
A- B	0 / 35	-70.5	-70.5	0.10 (1)		10.00
B- C	-17 / 0	-70.5	-70.5	0.03 (1)		6.25
C- D	-11 / 0	-70.5	-70.5	0.03 (1)		6.25
D- E	-11 / 0	-70.5	-70.5	0.03 (1)		6.25
E- F	-17 / 0	-70.5	-70.5	0.03 (1)		6.25
F- G	0 / 35	-70.5	-70.5	0.10 (1)		10.00
H- F	-180 / 0	0.0	0.0	0.01 (1)		7.81
L- K	0 / 12	-17.5	-17.5	0.02 (4)		10.00
K- J	0 / 8	-17.5	-17.5	0.02 (4)		10.00
J- I	0 / 8	-17.5	-17.5	0.02 (4)		10.00
I- H	0 / 12	-17.5	-17.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-134 / 0	0.07 (1)
K- C	-139 / 0	0.03 (1)
I- E	-139 / 0	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1) , BC=0.02 (H-I:4) , WB=0.07 (D-J:1) , SSI=0.05 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)

A-1073489

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H07	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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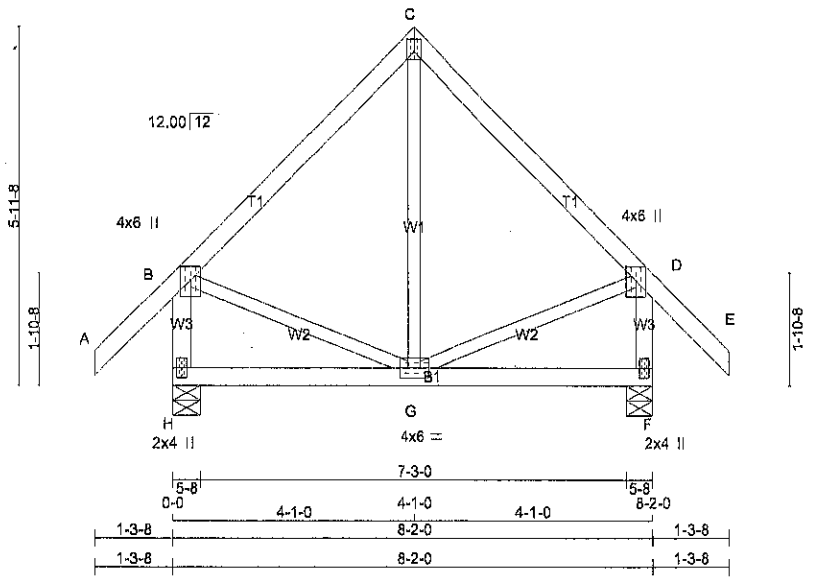

TOTAL WEIGHT = 3 X 41 = 124 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	3.0	4.0	Edge	
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	457	0	457	0	0	5-8	1-8
F	457	0	457	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	320	230 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
F	320	230 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-70.5	-70.5	0.10 (1)	10.00		G- C	-14 / 63	0.02 (4)
B- C	-193 / 0	-70.5	-70.5	0.15 (1)	6.25		B- G	0 / 146	0.03 (1)
C- D	-193 / 0	-70.5	-70.5	0.15 (1)	6.25		G- D	0 / 146	0.03 (1)
D- E	0 / 35	-70.5	-70.5	0.10 (1)	10.00				
H- B	-430 / 0	0.0	0.0	0.05 (1)	7.81				
F- D	-430 / 0	0.0	0.0	0.05 (1)	7.81				
H- G	0 / 0	-17.5	-17.5	0.08 (4)	10.00				
G- F	0 / 0	-17.5	-17.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
6.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15 (C-D:1) , BC=0.08 (F-G:4) , WB=0.03 (B-G:1) , SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.90)
JSI METAL= 0.08 (H) (INPUT = 1.00)

A-H073488

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H06SG	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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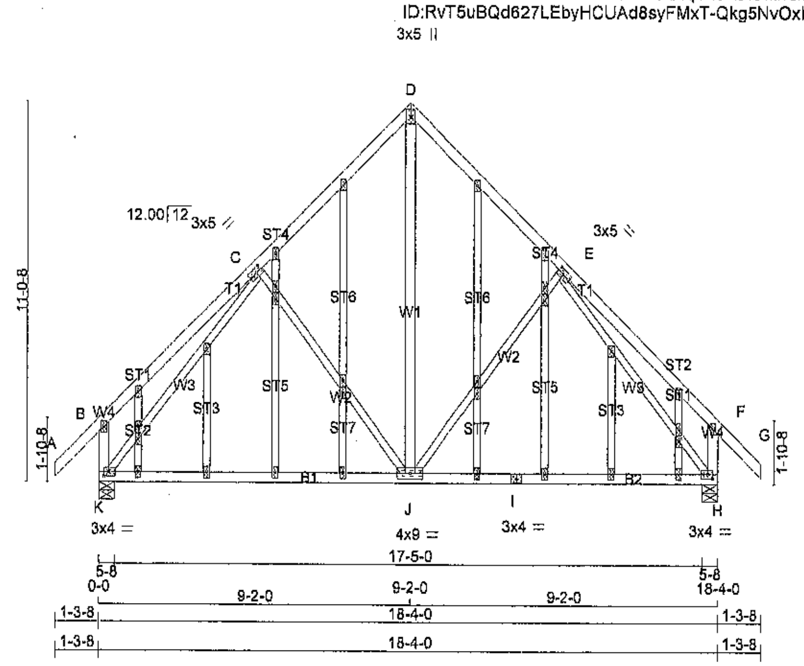


TOTAL WEIGHT = 128 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - D	2x4	DRY	No.2	SPF
ALL GABLE WEBS				
	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+p	MT20	3.0	5.0		
E	TMWW-t	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0	1.50	1.75
I	BS-t	MT20	3.0	4.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMVW1-t	MT20	3.0	4.0	1.50	1.75
L, L, M, N, O, O, P, Q, R, S, T, T, U, V, W, X, Y, Y, Z, AA, AB, AB, AC, AD, AE, AF, AG, AG						
L	NP+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	904	0	904	0	0	5-8	1-8
H	904	0	904	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
H	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 35	-70.5	-70.5	0.10 (1)	10.00
B- C	0 / 28	-70.5	-70.5	0.24 (1)	10.00
C- D	-539 / 0	-70.5	-70.5	0.19 (1)	6.25
D- E	-539 / 0	-70.5	-70.5	0.19 (1)	6.25
E- F	0 / 28	-70.5	-70.5	0.24 (1)	10.00
F- G	0 / 35	-70.5	-70.5	0.10 (1)	10.00
K- B	-219 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-219 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 468	-17.5	-17.5	0.46 (4)	10.00
J- I	0 / 468	-17.5	-17.5	0.46 (4)	10.00
I- H	0 / 468	-17.5	-17.5	0.46 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 475	0.08 (1)
J- E	-174 / 0	0.20 (1)
C- J	-174 / 0	0.20 (1)
K- C	-780 / 0	0.89 (1)
E- H	-780 / 0	0.89 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.24 (B-C:1) , BC=0.46 (J-K:4) , WB=0.89 (E-H:1) , SSI=0.13 (J-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.24 (I) (INPUT = 1.00)

A-N073487

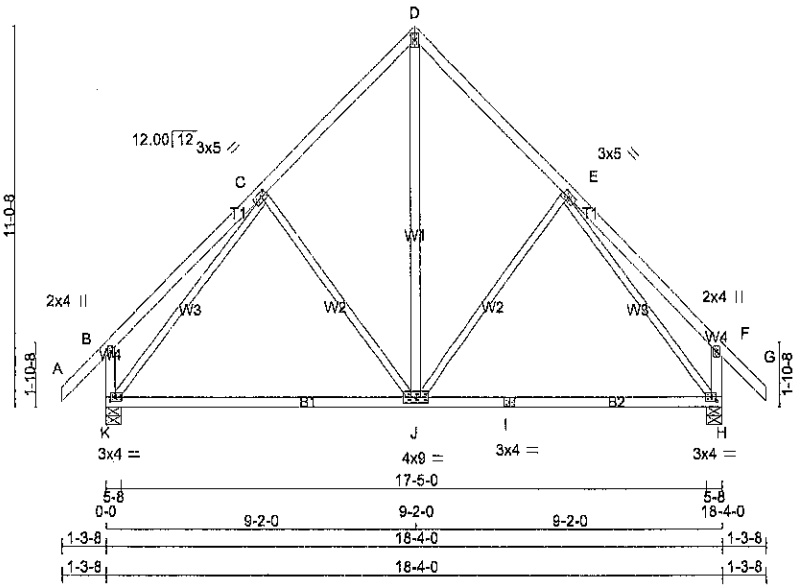
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H06	19	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-yY6jAZNJzTwqvh4jCunnskYmFOsTCv41ARgICXyyTMa

3x5 ||

Scale = 1:67.2



TOTAL WEIGHT = 19 X 94 = 1786 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	6.0	1.50	2.00
D	TTW+p	MT20	3.0	6.0		
E	TMWW-t	MT20	3.0	6.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0	1.50	1.75
I	BS-t	MT20	3.0	4.0		
J	BMWWW-t	MT20	4.0	8.0		
K	BMVW1-t	MT20	3.0	4.0	1.50	1.75

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	904	0	904	0	0	5-8	1-8
H	904	0	904	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
H	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-70.5	-70.5	0.10 (1)	10.00	J- D	0 / 475	0.08 (1)
B- C	0 / 28	-70.5	-70.5	0.24 (1)	10.00	J- E	-174 / 0	0.20 (1)
C- D	-539 / 0	-70.5	-70.5	0.19 (1)	6.25	C- J	-174 / 0	0.20 (1)
D- E	-539 / 0	-70.5	-70.5	0.19 (1)	6.25	K- C	-780 / 0	0.89 (1)
E- F	0 / 28	-70.5	-70.5	0.24 (1)	10.00	E- H	-780 / 0	0.89 (1)
F- G	0 / 35	-70.5	-70.5	0.10 (1)	10.00			
K- B	-219 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-219 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 468	-17.5	-17.5	0.46 (4)	10.00			
J- I	0 / 468	-17.5	-17.5	0.46 (4)	10.00			
I- H	0 / 468	-17.5	-17.5	0.46 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.24 (B-C:1) , BC=0.46 (J-K:4) , WB=0.89 (E-H:1) , SSI=0.13 (J-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-13-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1507 3486

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H05	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Mon Jul 13 14:15:52 2015 Page 1
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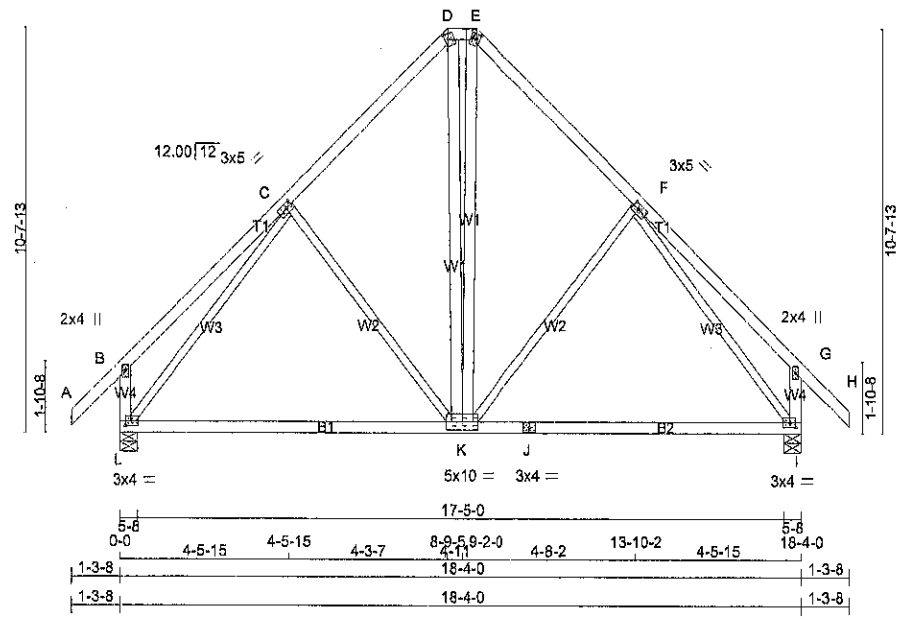


TOTAL WEIGHT = 104 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+m	MT20	3.0	4.0	1.75	1.00
E	TTW+m	MT20	3.0	4.0	1.75	1.00
F	TMWW-t	MT20	3.0	5.0	1.50	2.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	3.0	4.0	1.50	1.75
J	BS-t	MT20	3.0	4.0		
K	BMWWWW"-t	MT20	5.0	10.0		
L	BMVW1-t	MT20	3.0	4.0	1.50	1.75

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	904	0	904	0	0	5-8	1-8
L	904	0	904	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
L	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 35	-70.5	-70.5	0.10 (1)		10.00
B- C	0 / 26	-70.5	-70.5	0.22 (1)		10.00
C- D	-546 / 0	-70.5	-70.5	0.18 (1)		6.25
D- E	-375 / 0	-70.5	-70.5	0.01 (1)		6.25
E- F	-546 / 0	-70.5	-70.5	0.18 (1)		6.25
F- G	0 / 26	-70.5	-70.5	0.22 (1)		10.00
G- H	0 / 35	-70.5	-70.5	0.10 (1)		10.00
L- B	-214 / 0	0.0	0.0	0.02 (1)		7.81
I- G	-214 / 0	0.0	0.0	0.02 (1)		7.81
L- K	0 / 466	-17.5	-17.5	0.46 (4)		10.00
K- J	0 / 466	-17.5	-17.5	0.46 (4)		10.00
J- I	0 / 466	-17.5	-17.5	0.46 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-158 / 0	0.18 (1)
D- K	0 / 222	0.04 (1)
K- E	0 / 222	0.04 (1)
K- F	-158 / 0	0.18 (1)
L- C	-784 / 0	0.80 (1)
F- I	-784 / 0	0.80 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.22 (B-C;1) , BC=0.46 (I-K;4) , WB=0.80 (C-L;1) , SSI=0.13 (K-L;4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
13-07-2015
R. TURENNE
100157048
PROVINCE OF ONTARIO

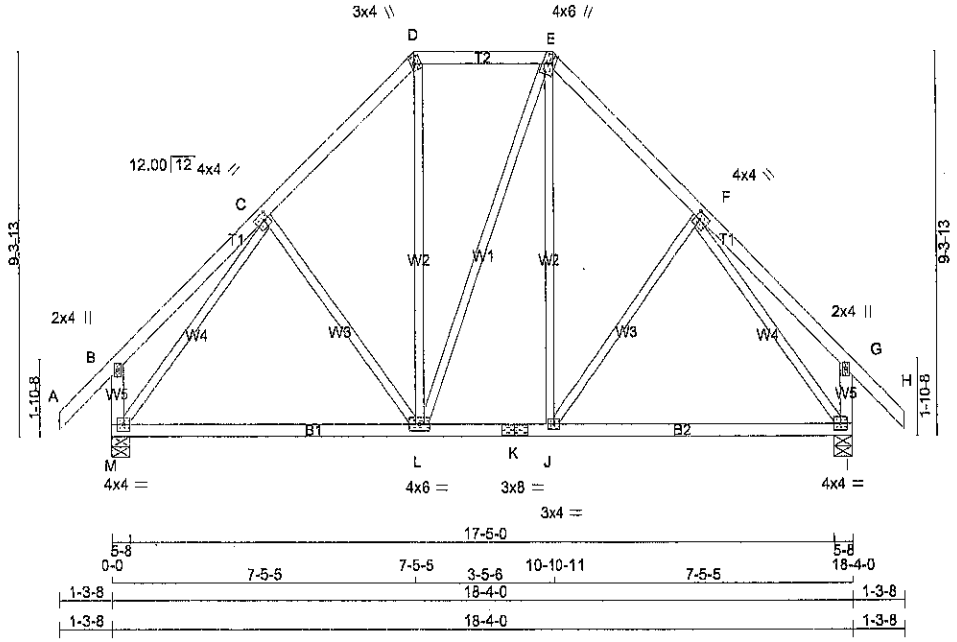
A-N073485

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H04	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-UMYLzDMgDAozHXVXfAFYKW?cs_aiTXiuynwCg5yyTM!

Scale = 1:56.1



TOTAL WEIGHT = 99 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+m	MT20	3.0	4.0	2.00	1.00
E	TTWW+m	MT20	4.0	6.0	Edge	1.00
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	3.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	904	0	904	0	0	5-8	1-8
I	904	0	904	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
I	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-70.5	-70.5	0.10 (1)	10.00	C- L	-97 / 11	0.07 (1)
B- C	0 / 22	-70.5	-70.5	0.16 (1)	10.00	L- D	0 / 182	0.04 (1)
C- D	-594 / 0	-70.5	-70.5	0.12 (1)	6.25	L- E	0 / 1	0.00 (1)
D- E	-405 / 0	-70.5	-70.5	0.11 (1)	6.25	J- E	0 / 181	0.04 (1)
E- F	-593 / 0	-70.5	-70.5	0.12 (1)	6.25	J- F	-97 / 11	0.07 (1)
F- G	0 / 22	-70.5	-70.5	0.16 (1)	10.00	M- C	-798 / 0	0.57 (1)
G- H	0 / 35	-70.5	-70.5	0.10 (1)	10.00	F- I	-798 / 0	0.57 (1)
M- B	-197 / 0	0.0	0.0	0.02 (1)	7.81			
I- G	-197 / 0	0.0	0.0	0.02 (1)	7.81			
M- L	0 / 461	-17.5	-17.5	0.24 (4)	10.00			
L- K	0 / 405	-17.5	-17.5	0.22 (4)	10.00			
K- J	0 / 405	-17.5	-17.5	0.22 (4)	10.00			
J- I	0 / 461	-17.5	-17.5	0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.16 (F-G:1) , BC=0.24 (I-J:4) , WB=0.57 (C-M:1) , SSI=0.10 (L-M:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (J) (INPUT = 0.90)
JSI METAL= 0.30 (K) (INPUT = 1.00)

A-1073484

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H02	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:RvT5uBQd627LEbyHCUAd8syFMxT-0A_zluL2Ssg6fOwK5TkJnJTQDaDBk8Bkj7Be8eyyTMc

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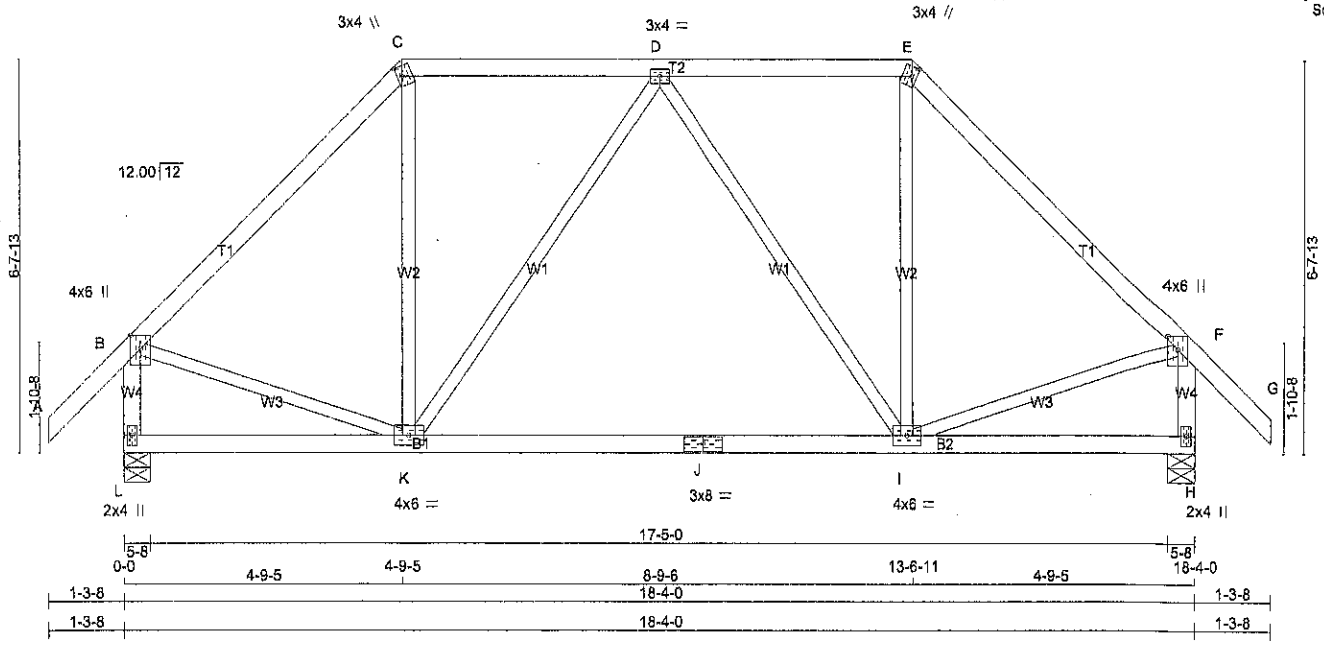

TOTAL WEIGHT = 84 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+m	MT20	3.0	4.0	2.00	1.00
D	TMWW-t	MT20	3.0	4.0		
E	TTW+m	MT20	3.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	904	0	904	0	0	5-8	1-8
H	904	0	904	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
H	635	443 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-70.5	-70.5	0.10 (1)	10.00	K- C	0 / 179	0.04 (4)
B- C	-660 / 0	-70.5	-70.5	0.21 (1)	6.25	K- D	-227 / 0	0.27 (1)
C- D	-468 / 0	-70.5	-70.5	0.17 (1)	6.25	D- I	-227 / 0	0.27 (1)
D- E	-468 / 0	-70.5	-70.5	0.17 (1)	6.25	I- E	0 / 179	0.04 (4)
E- F	-660 / 0	-70.5	-70.5	0.21 (1)	6.25	B- K	0 / 487	0.11 (1)
F- G	0 / 35	-70.5	-70.5	0.10 (1)	10.00	I- F	0 / 487	0.11 (1)
L- B	-880 / 0	0.0	0.0	0.10 (1)	7.81			
H- F	-880 / 0	0.0	0.0	0.10 (1)	7.81			
L- K	0 / 0	-17.5	-17.5	0.21 (4)	10.00			
K- J	0 / 595	-17.5	-17.5	0.26 (4)	10.00			
J- I	0 / 595	-17.5	-17.5	0.26 (4)	10.00			
I- H	0 / 0	-17.5	-17.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN, C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.21 (E-F:1) , BC=0.26 (I-K:4) , WB=0.27 (D-K:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.33 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.13.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-4073482

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252520	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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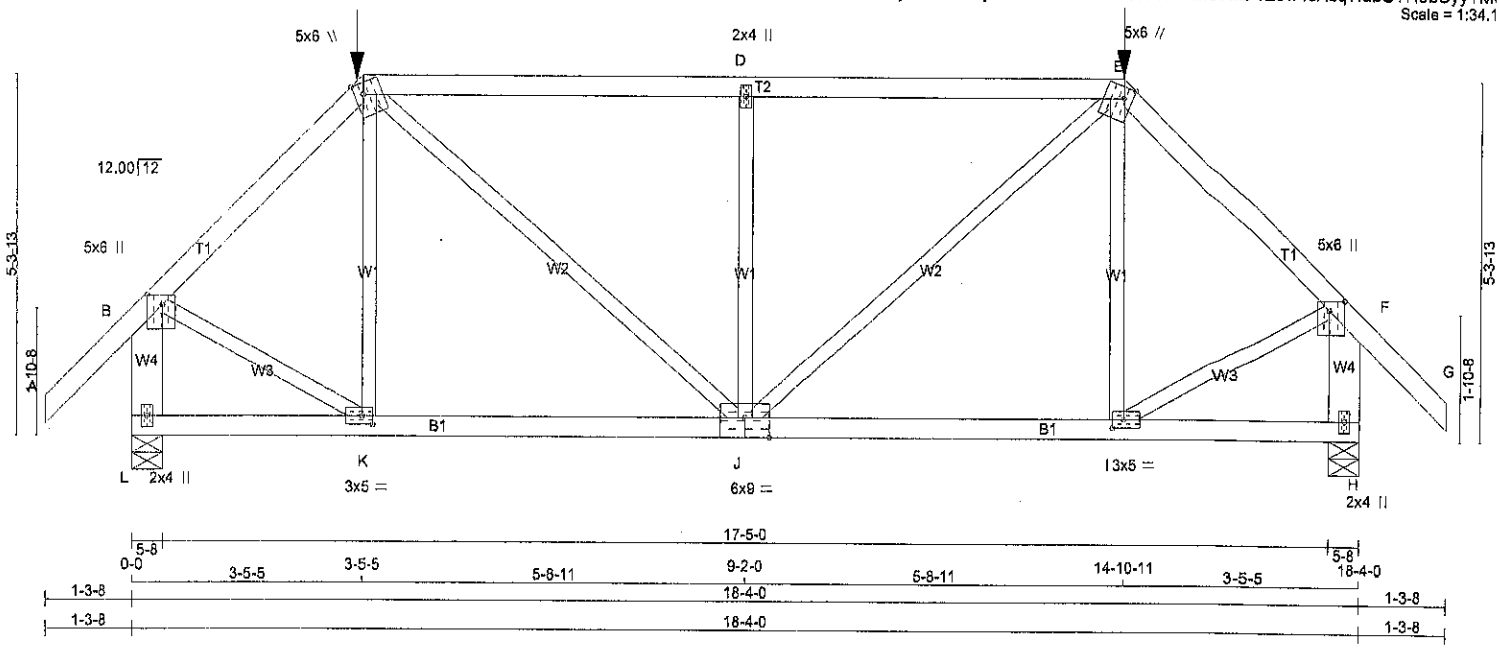

TOTAL WEIGHT = 85 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	1.75	2.75
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	3.0	5.0	1.50	2.00
J	BSWWW-I	MT20	6.0	9.0	Edge	4.50
K	BMWW-t	MT20	3.0	5.0	1.50	2.00
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 176.9 lbs FACTORED DOWN AT 14-10-11, AND 176.9 lbs FACTORED DOWN AT 3-5-5 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1627	0	1627	0	0	5-8	1-12
H	1627	0	1627	0	0	5-8	1-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1146	781 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0
H	1146	781 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 36	-70.5	-70.5	0.11 (1)	10.00	K- C	-230 / 65	0.10 (1)
B- C	-1351 / 0	-70.5	-70.5	0.20 (1)	5.35	C- J	0 / 884	0.22 (1)
C- D	-1613 / 0	-138.7	-138.7	0.96 (1)	3.25	J- D	-968 / 0	0.42 (1)
D- E	-1613 / 0	-138.7	-138.7	0.96 (1)	3.25	J- E	0 / 884	0.22 (1)
E- F	-1351 / 0	-70.5	-70.5	0.20 (1)	5.35	I- E	-230 / 65	0.10 (1)
F- G	0 / 35	-70.5	-70.5	0.11 (1)	10.00	B- K	0 / 1035	0.26 (1)
L- B	-1588 / 0	0.0	0.0	0.13 (1)	7.75	I- F	0 / 1035	0.26 (1)
H- F	-1588 / 0	0.0	0.0	0.13 (1)	7.75			
L- K	0 / 0	-34.5	-34.5	0.19 (4)	10.00			
K- J	0 / 947	-34.5	-34.5	0.33 (4)	10.00			
J- I	0 / 947	-34.5	-34.5	0.33 (4)	10.00			
I- H	0 / 0	-34.5	-34.5	0.19 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-5-5	-177	-177	---	FRONT	VERT	TOTAL
E	14-10-11	-177	-177	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-5-5
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.96 (D-E:1) , BC=0.33 (J-K:4) , WB=0.42 (D-J:1) , SSI=0.42 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.29 (H) (INPUT = 1.00)

A-150734 81

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	J01C	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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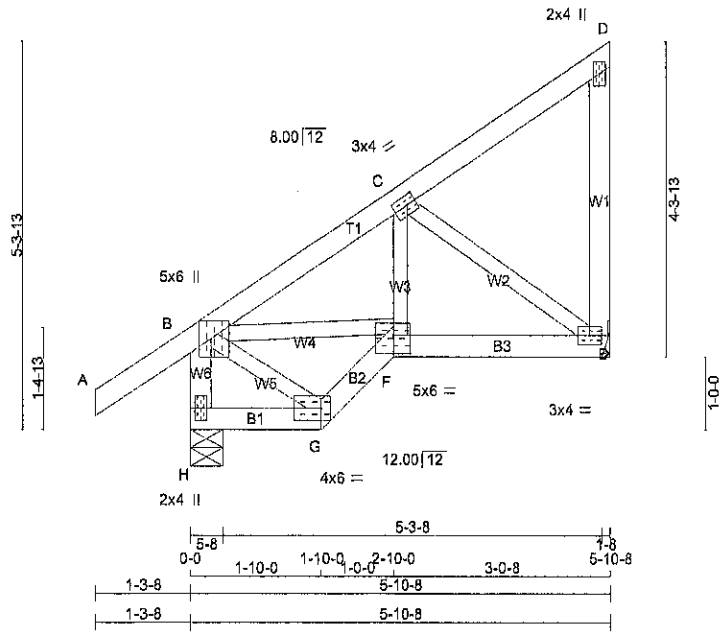

TOTAL WEIGHT = 30 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
H-B	-339 / 0	0.0	0.0	0.03 (1)	7.81	B-G	0 / 20	0.01 (4)
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B-F	0 / 209	0.05 (1)
B-C	-257 / 0	-70.5	-70.5	0.10 (1)	6.25	F-C	0 / 57	0.02 (4)
C-D	-13 / 0	-70.5	-70.5	0.10 (1)	6.25	C-E	-271 / 0	0.06 (1)
E-D	-79 / 0	0.0	0.0	0.02 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.02 (4)	10.00			
G-F	0 / 25	-17.5	-17.5	0.01 (4)	10.00			
F-E	0 / 223	-17.5	-17.5	0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.06 (E-F:1) , WB=0.06 (C-E:1) , SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

A-1073480

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	J01B	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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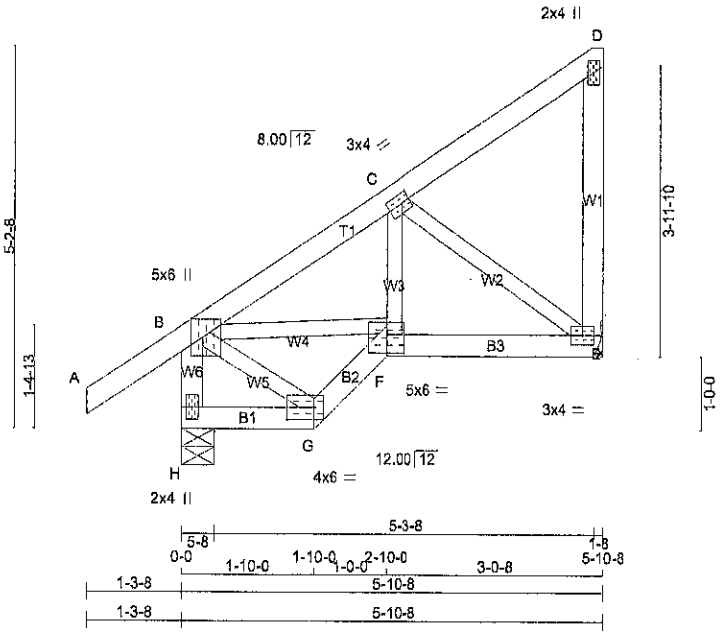

TOTAL WEIGHT = 30 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	258	0	258	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	355	0	355	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.26FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-267 / 0	-70.5	-70.5	0.10 (1)	6.25
C- D	-13 / 0	-70.5	-70.5	0.10 (1)	6.25
E- D	-79 / 0	0.0	0.0	0.02 (1)	7.61
H- B	-339 / 0	0.0	0.0	0.03 (1)	7.61
H- G	0 / 0	-17.5	-17.5	0.02 (4)	10.00
G- F	0 / 25	-17.5	-17.5	0.01 (4)	10.00
F- E	0 / 223	-17.5	-17.5	0.06 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- F	0 / 209	0.05 (1)
F- C	0 / 57	0.02 (4)
C- E	-271 / 0	0.06 (1)
B- G	0 / 20	0.01 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.06 (E-F:1) , WB=0.06 (C-E:1) , SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

A-100734-79

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	J01A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-jr93ICRyTeRHo8qX69wwPDIpabHNOa?bLMvc4?yyU?p

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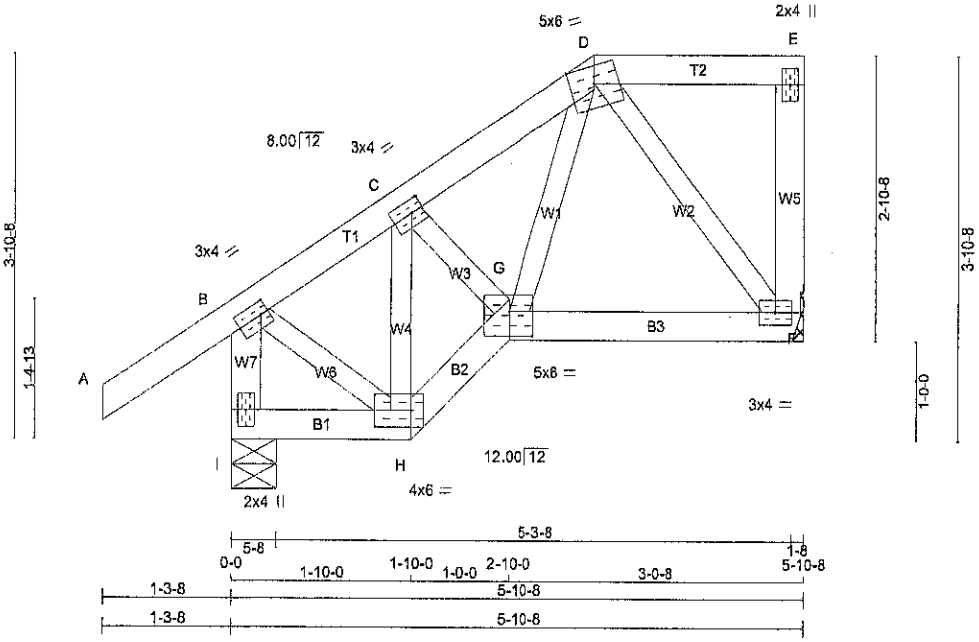

TOTAL WEIGHT = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	5.0	6.0	Edge	
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BBWW-I	MT20	5.0	6.0	3.00	3.00
H	BBWW-I	MT20	4.0	6.0	2.00	4.50
I	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	247	0	247	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	366	0	366	0	0	6-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	174	117 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
I	256	188 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B		0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C		-173 / 0	-70.5	-70.5	0.09 (1)	6.25
C- D		-197 / 0	-70.5	-70.5	0.04 (1)	6.25
D- E		0 / 0	-70.5	-70.5	0.06 (1)	10.00
F- E		-76 / 0	0.0	0.0	0.01 (1)	7.81
I- B		-350 / 0	0.0	0.0	0.04 (1)	7.81
I- H		0 / 0	-17.5	-17.5	0.02 (4)	10.00
H- G		0 / 171	-17.5	-17.5	0.03 (1)	10.00
G- F		0 / 123	-17.5	-17.5	0.08 (4)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C		-170 / 0	0.03 (1)
C- G		0 / 61	0.01 (1)
G- D		0 / 119	0.03 (1)
D- F		-190 / 0	0.04 (1)
B- H		0 / 148	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.09 (A-B:1) , BC=0.06 (F-G:4) , WB=0.04 (D-F:1) , SSI=0.06 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (B) (INPUT = 0.80)
JSI METAL= 0.10 (B) (INPUT = 1.00)

A-1073478

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	H03T	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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5x6 II

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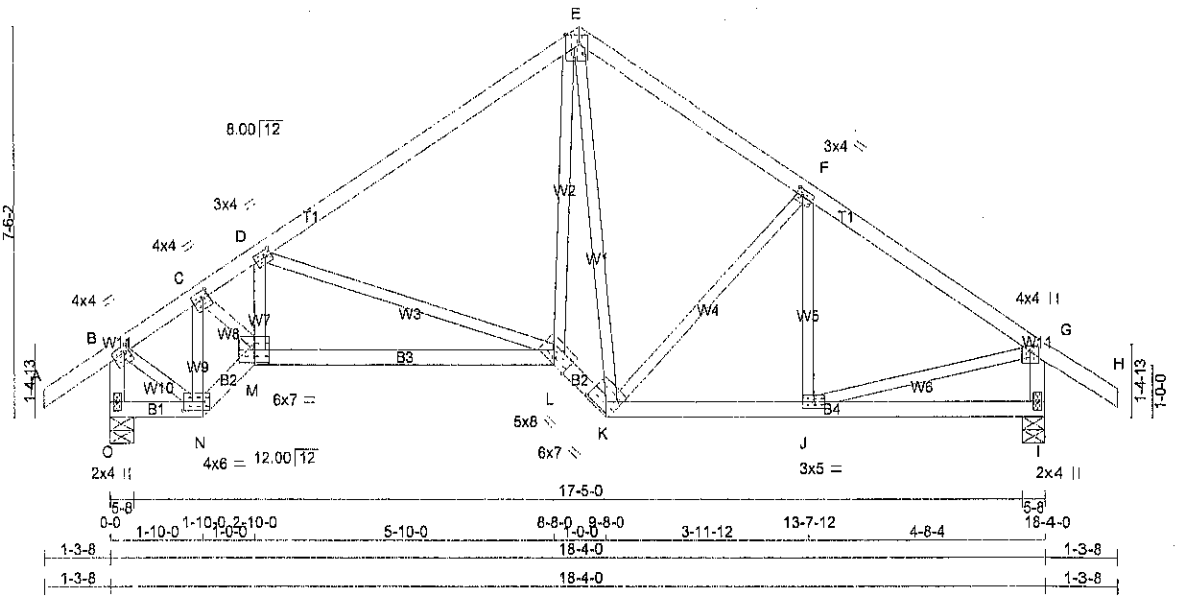

TOTAL WEIGHT = 4 X 86 = 343 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	1.50	1.00
C	TMWW-t	MT20	4.0	4.0	1.75	1.50
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW+p	MT20	5.0	6.0	Edge	3.00
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	3.0	5.0		
K	BBWW-h	MT20	6.0	7.0	3.75	6.00
L	BBWW-h	MT20	5.0	8.0	Edge	
M	BBWW-l	MT20	6.0	7.0		
N	BBWW-l	MT20	4.0	6.0	2.00	4.50
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	907	0	907	0	0	5-8	1-8
I	899	0	899	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	636	444 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
I	631	440 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
B- C	-677 / 0	-70.5	-70.5	0.08 (1)	6.25	
C- D	-1152 / 0	-70.5	-70.5	0.30 (1)	5.57	
D- E	-761 / 0	-70.5	-70.5	0.33 (1)	6.25	
E- F	-656 / 0	-70.5	-70.5	0.19 (1)	6.25	
F- G	-838 / 0	-70.5	-70.5	0.20 (1)	6.25	
G- H	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
O- B	-890 / 0	0.0	0.0	0.09 (1)	7.81	
I- G	-865 / 0	0.0	0.0	0.09 (1)	7.81	
O- N	0 / 0	-17.5	-17.5	0.02 (4)	10.00	
N- M	0 / 693	-17.5	-17.5	0.11 (1)	10.00	
M- L	0 / 1096	-17.5	-17.5	0.28 (1)	10.00	
L- K	0 / 777	-17.5	-17.5	0.13 (1)	10.00	
K- J	0 / 715	-17.5	-17.5	0.16 (1)	10.00	
J- I	0 / 0	-17.5	-17.5	0.09 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-768 / 0	0.12 (1)
M- D	0 / 888	0.20 (1)
M- D	-104 / 57	0.02 (4)
D- L	-512 / 0	0.35 (1)
L- E	0 / 781	0.18 (1)
E- K	-313 / 0	0.32 (1)
K- F	-272 / 0	0.15 (1)
J- F	-79 / 51	0.02 (1)
B- N	0 / 607	0.14 (1)
J- G	0 / 734	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.33 (D-E:1) , BC=0.28 (L-M:1) , WB=0.35 (D-L:1) , SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/13/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073472

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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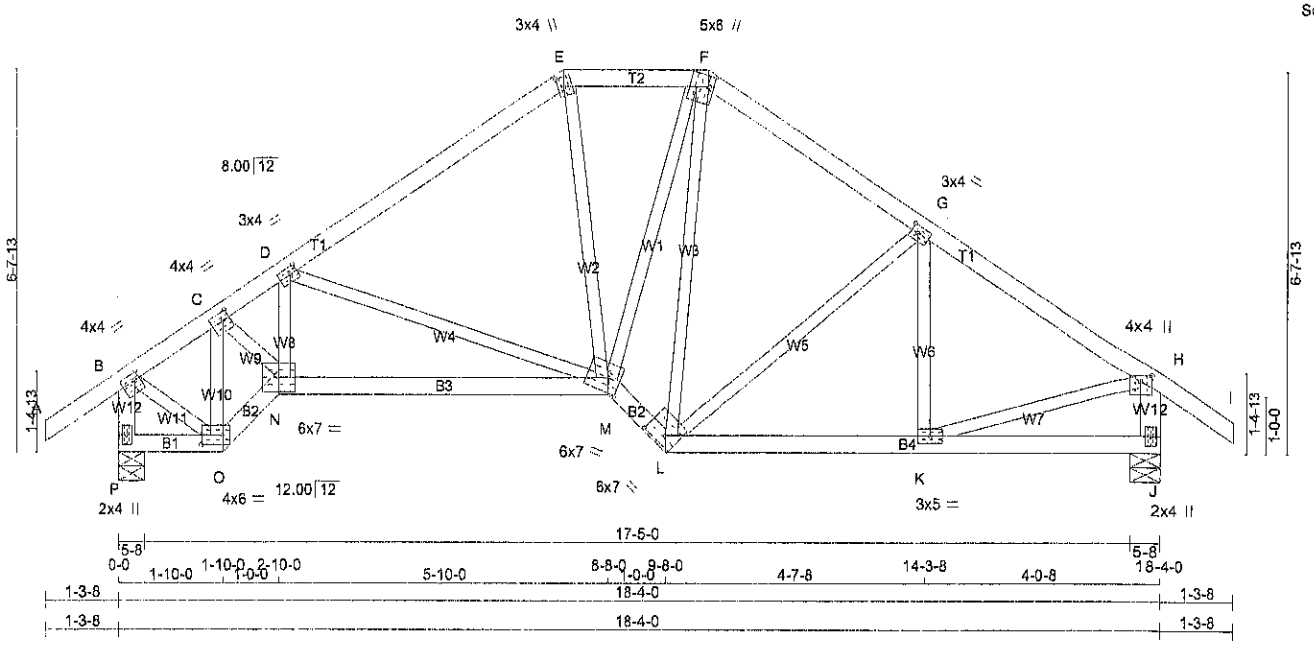

TOTAL WEIGHT = 87 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	1.50	1.00
C	TMWW-t	MT20	4.0	4.0	1.75	1.50
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+m	MT20	3.0	4.0	2.00	1.25
F	TTWW+m	MT20	5.0	6.0	2.25	1.00
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	3.0	5.0		
L	BBWW-h	MT20	6.0	7.0	2.00	4.25
M	BBWWW-m	MT20	6.0	7.0	3.00	2.50
N	BBWW-I	MT20	6.0	7.0		
O	BBWW-I	MT20	4.0	6.0	2.00	4.50
P	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	907	0	907	0	0	5-8	1-8
J	899	0	899	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	636	444 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
J	631	440 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B- C	-682 / 0	-70.5	-70.5	0.07 (1)		6.25
C- D	-1151 / 0	-70.5	-70.5	0.18 (1)		5.73
D- E	-797 / 0	-70.5	-70.5	0.20 (1)		6.25
E- F	-677 / 0	-70.5	-70.5	0.06 (1)		6.25
F- G	-679 / 0	-70.5	-70.5	0.14 (1)		6.25
G- H	-838 / 0	-70.5	-70.5	0.15 (1)		6.25
H- I	0 / 27	-70.5	-70.5	0.09 (1)		10.00
P- B	-890 / 0	0.0	0.0	0.09 (1)		7.81
J- H	-872 / 0	0.0	0.0	0.09 (1)		7.81
P- O	0 / 0	-17.5	-17.5	0.02 (4)		10.00
O- N	0 / 706	-17.5	-17.5	0.12 (1)		10.00
N- M	0 / 1040	-17.5	-17.5	0.27 (1)		10.00
M- L	0 / 718	-17.5	-17.5	0.12 (1)		10.00
L- K	0 / 712	-17.5	-17.5	0.16 (1)		10.00
K- J	0 / 0	-17.5	-17.5	0.09 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-782 / 0	0.13 (1)
C- N	0 / 797	0.18 (1)
N- D	-28 / 67	0.02 (4)
D- M	-416 / 0	0.26 (1)
E- M	0 / 203	0.05 (1)
M- F	0 / 532	0.12 (1)
L- F	-331 / 0	0.25 (1)
L- G	-210 / 0	0.13 (1)
K- G	-104 / 42	0.03 (1)
B- O	0 / 618	0.14 (1)
K- H	0 / 737	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.20 (D-E:1) , BC=0.27 (M-N:1) , WB=0.26 (D-M:1) , SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

A-H07 3471

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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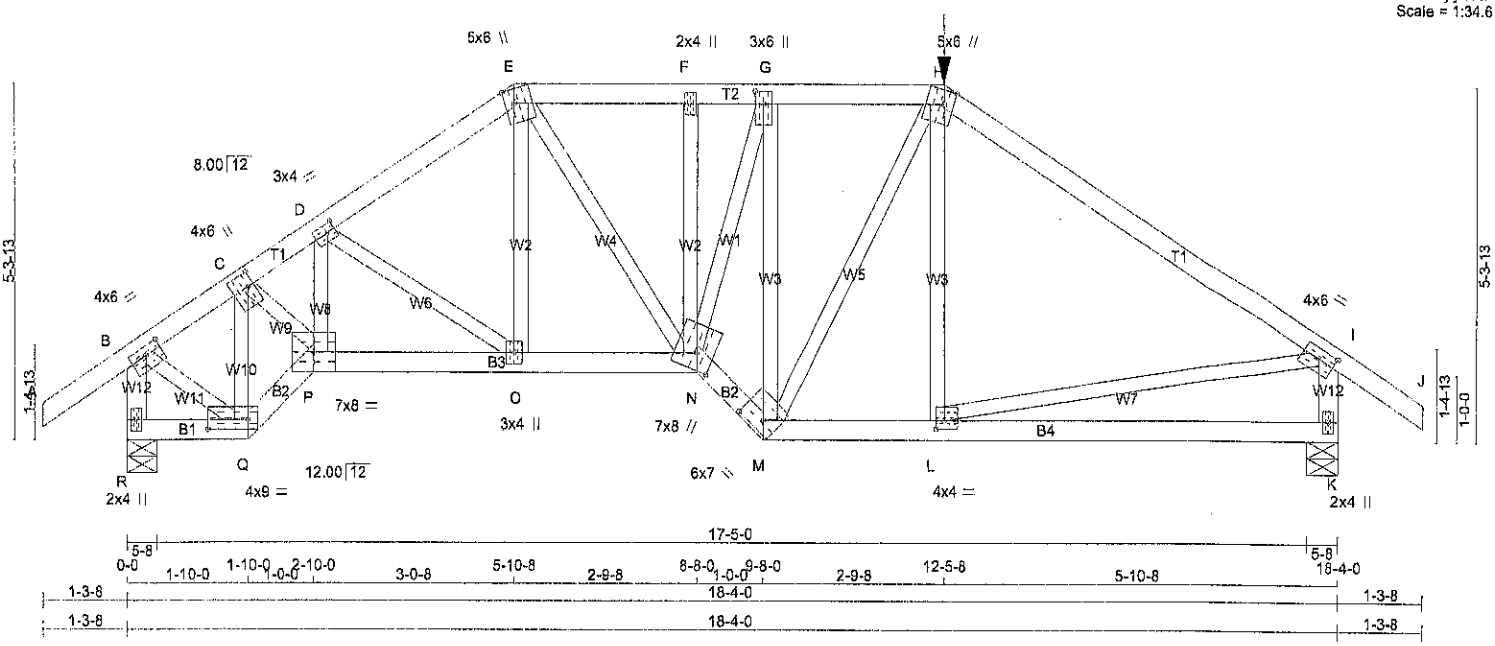

TOTAL WEIGHT = 90 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	1.50	3.00
C	TMWW+t	MT20	4.0	6.0	2.50	1.00
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	3.0	6.0	2.25	1.50
H	TTWW+m	MT20	5.0	6.0	Edge	1.75
I	TMW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BBWW-h	MT20	6.0	7.0	2.00	4.25
N	BBWWW+m	MT20	7.0	8.0	3.00	3.00
O	BMWW+t	MT20	3.0	4.0		
P	BBWW-l	MT20	7.0	8.0		
Q	BBWW-l	MT20	4.0	9.0	1.75	7.25
R	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
R	1730	0	1730	0	0	5-8	2-2
K	1553	0	1553	0	0	5-8	1-11

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1215	842 / 0	0 / 0	0 / 0	0 / 0	373 / 0	0 / 0
K	1095	737 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.10 (1)	10.00	
B- C	-1413 / 0	-138.7	-138.7	0.13 (1)	5.33	
C- D	-2377 / 0	-138.7	-138.7	0.13 (1)	4.32	
D- E	-1980 / 0	-70.5	-70.5	0.15 (1)	4.64	
E- F	-1885 / 0	-70.5	-70.5	0.16 (1)	4.72	
F- G	-1883 / 0	-138.7	-138.7	0.16 (1)	4.72	
G- H	-1536 / 0	-138.7	-138.7	0.17 (1)	5.11	
H- I	-1648 / 0	-70.5	-70.5	0.61 (1)	4.43	
I- J	0 / 27	-70.5	-70.5	0.10 (1)	10.00	
R- B	-1698 / 0	0.0	0.0	0.19 (1)	6.33	
K- I	-1468 / 0	0.0	0.0	0.17 (1)	6.71	
R- Q	0 / 0	-34.5	-34.5	0.04 (4)	10.00	
Q- P	0 / 1512	-34.5	-34.5	0.27 (1)	10.00	
P- O	0 / 1990	-102.7	-102.7	0.50 (1)	10.00	
O- N	0 / 1654	-102.7	-102.7	0.44 (1)	10.00	
N- M	0 / 2120	-34.5	-34.5	0.38 (1)	10.00	
M- L	0 / 1368	-34.5	-34.5	0.41 (4)	10.00	
L- K	0 / 0	-34.5	-34.5	0.29 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-1660 / 0	0.29 (1)
C- P	0 / 1378	0.34 (1)
P- D	0 / 220	0.05 (1)
D- O	-422 / 0	0.11 (1)
O- E	0 / 566	0.14 (1)
E- N	0 / 402	0.10 (1)
N- F	-83 / 3	0.02 (1)
N- G	0 / 1441	0.36 (1)
M- G	-1758 / 0	0.76 (1)
M- H	0 / 342	0.08 (1)
L- H	-57 / 151	0.06 (4)
B- Q	0 / 1324	0.33 (1)
L- I	0 / 1392	0.34 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
H	12-5-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-10-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-10-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 8-8-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 9-8-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.61 (H-I:1) , BC=0.50 (O-P:1) , WB=0.76 (G-M:1) , SSI=0.18 (G-F:1)

A-R073470

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	H01T	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 12-5-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.61 (L) (INPUT = 1.00)

A-1073470(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252512	H03	14	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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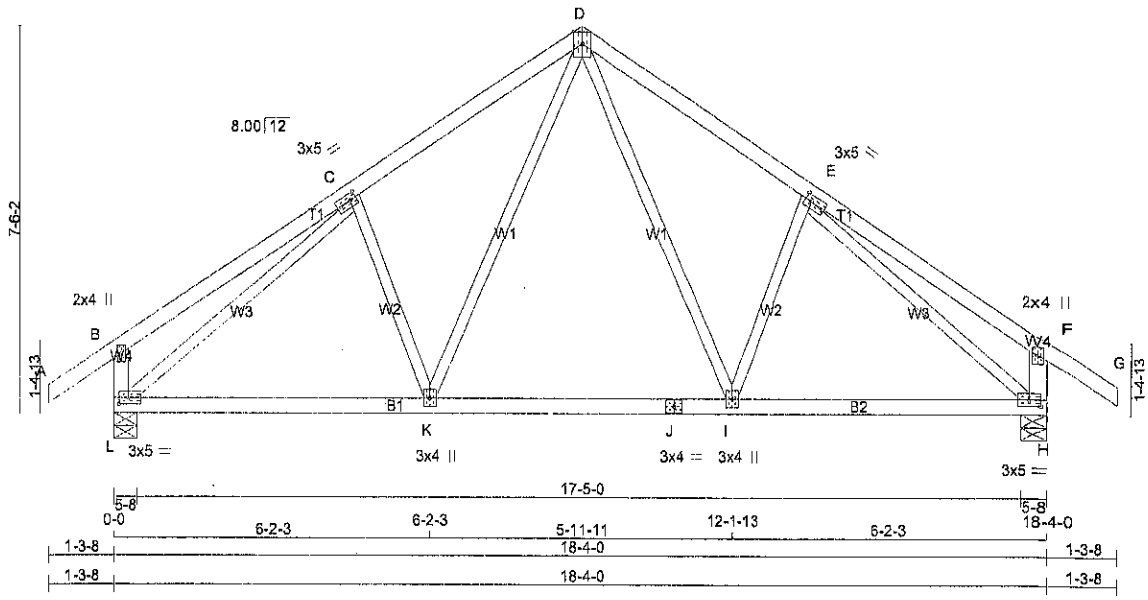


TOTAL WEIGHT = 14 X 81 = 1128 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	3.0	5.0	1.50	1.50
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	5.0	1.50	2.25
I	BMWW+t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	4.0		
K	BMWW+t	MT20	3.0	4.0		
L	BMVW1-t	MT20	3.0	5.0	1.50	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	903	0	903	0	0	5-8	1-8
H	903	0	903	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	634	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
H	634	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B- C	0 / 21	-70.5	-70.5	0.24 (1)		10.00
C- D	-784 / 0	-70.5	-70.5	0.19 (1)		6.25
D- E	-784 / 0	-70.5	-70.5	0.19 (1)		6.25
E- F	0 / 21	-70.5	-70.5	0.24 (1)		10.00
F- G	0 / 27	-70.5	-70.5	0.09 (1)		10.00
L- B	-219 / 0	0.0	0.0	0.02 (1)		7.81
H- F	-219 / 0	0.0	0.0	0.02 (1)		7.81
L- K	0 / 707	-17.5	-17.5	0.20 (4)		10.00
K- J	0 / 511	-17.5	-17.5	0.19 (4)		10.00
J- I	0 / 511	-17.5	-17.5	0.19 (4)		10.00
I- H	0 / 707	-17.5	-17.5	0.20 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 323	0.07 (1)
I- E	-196 / 3	0.07 (1)
K- D	0 / 323	0.07 (1)
C- K	-196 / 3	0.07 (1)
L- C	-953 / 0	0.62 (1)
E- H	-953 / 0	0.62 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.24 (E-F:1) , BC=0.20 (H-I:4) , WB=0.62 (E-H:1) , SSI=0.13 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

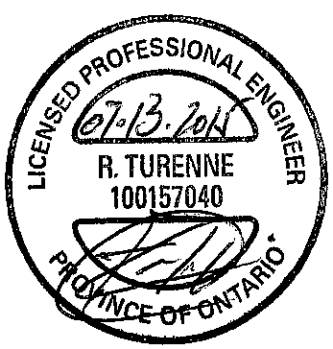
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)



A-4073442

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252512	H02	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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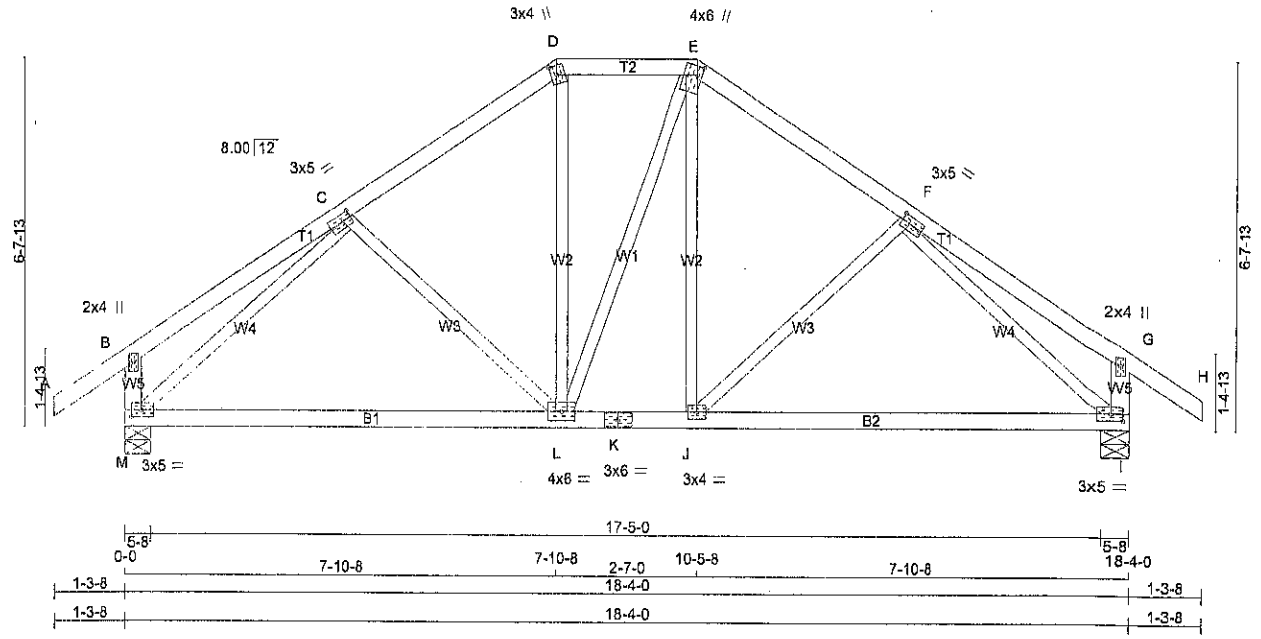

TOTAL WEIGHT = 83 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+m	MT20	3.0	4.0	2.00	1.25
E	TTWW+m	MT20	4.0	6.0	2.00	1.00
F	TMWW-t	MT20	3.0	5.0	1.50	2.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	3.0	5.0	1.50	2.25
J	BMWW-t	MT20	3.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	3.0	5.0	1.50	2.25

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	903	0	903	0	0	5-8	1-8
I	903	0	903	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	634	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
I	634	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B- C	0 / 18	-70.5	-70.5	0.17 (1)		10.00
C- D	-719 / 0	-70.5	-70.5	0.14 (1)		6.25
D- E	-586 / 0	-70.5	-70.5	0.06 (1)		6.25
E- F	-719 / 0	-70.5	-70.5	0.14 (1)		6.25
F- G	0 / 18	-70.5	-70.5	0.17 (1)		10.00
G- H	0 / 27	-70.5	-70.5	0.09 (1)		10.00
M- B	-203 / 0	0.0	0.0	0.02 (1)		7.81
I- G	-203 / 0	0.0	0.0	0.02 (1)		7.81
M- L	0 / 681	-17.5	-17.5	0.28 (4)		10.00
L- K	0 / 586	-17.5	-17.5	0.28 (4)		10.00
K- J	0 / 586	-17.5	-17.5	0.28 (4)		10.00
J- I	0 / 681	-17.5	-17.5	0.27 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-133 / 10	0.07 (1)
L- D	0 / 192	0.04 (1)
L- E	0 / 1	0.00 (1)
J- E	0 / 191	0.04 (1)
J- F	-133 / 10	0.07 (1)
M- C	-936 / 0	0.44 (1)
F- I	-935 / 0	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.17 (F-G:1) , BC=0.28 (J-L:4) , WB=0.44 (C-M:1) , SSI=0.11 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.42 (K) (INPUT = 1.00)

A-4073441

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252512	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Mon Jul 13 08:58:50 2015 Page 1
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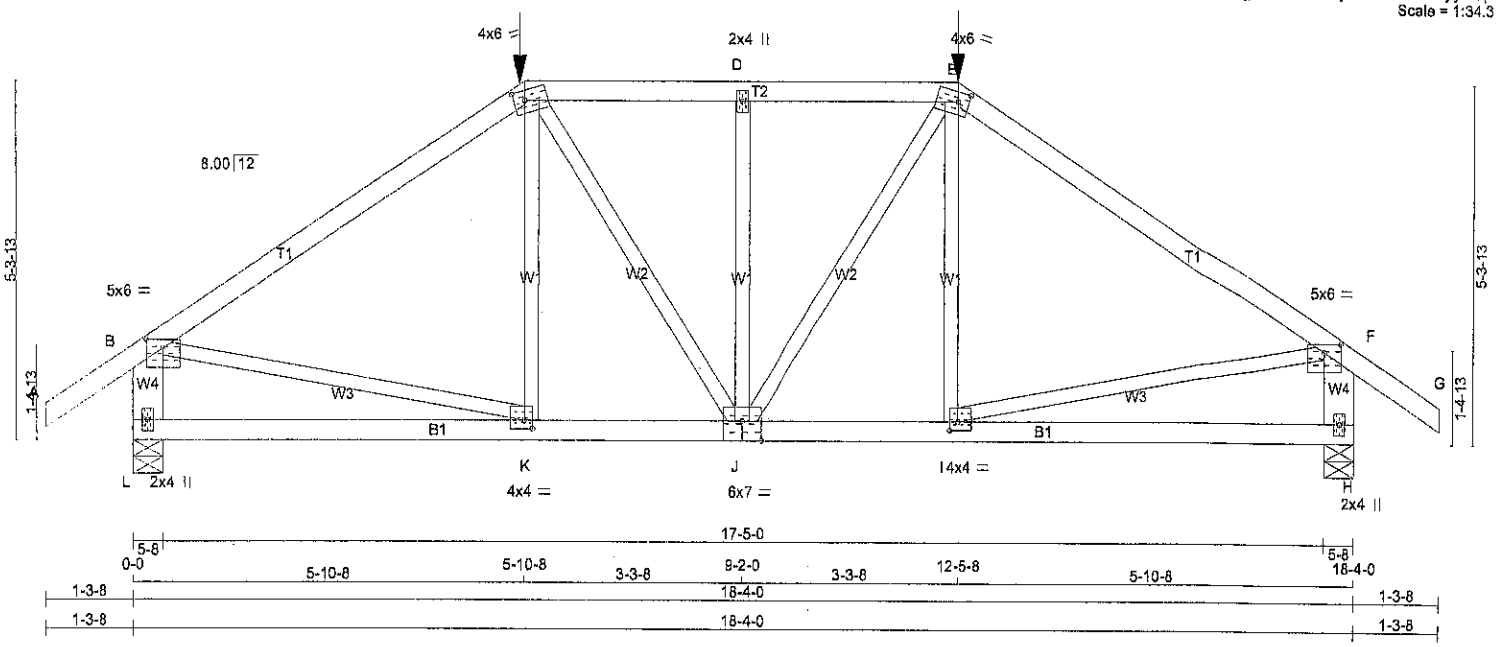


TOTAL WEIGHT = 82 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.00
F	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0	1.50	1.50
J	BSWWW-I	MT20	6.0	7.0	Edge	3.50
K	BMWW-t	MT20	4.0	4.0	1.50	1.50
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 12-5-8, AND 301.8 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1585	0	1585	0	0	5-8	1-11
H	1585	0	1585	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1117	756 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0
H	1117	756 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	1	0.10 (1)	10.00	K- C	-37 / 163	0.08 (4)
B- C	-1694 / 0	-70.5	-70.5	1	0.62 (1)	4.38	C- J	0 / 341	0.08 (1)
C- D	-1594 / 0	-138.7	-138.7	1	0.29 (1)	4.86	J- D	-844 / 0	0.24 (1)
D- E	-1594 / 0	-138.7	-138.7	1	0.29 (1)	4.86	J- E	0 / 341	0.08 (1)
E- F	-1694 / 0	-70.5	-70.5	1	0.62 (1)	4.38	I- E	-37 / 163	0.08 (4)
F- G	0 / 27	-70.5	-70.5	1	0.10 (1)	10.00	B- K	0 / 1430	0.35 (1)
L- B	-1500 / 0	0.0	0.0	1	0.11 (1)	7.81	I- F	0 / 1430	0.35 (1)
H- F	-1500 / 0	0.0	0.0	1	0.11 (1)	7.81			
L- K	0 / 0	-34.5	-34.5	1	0.29 (4)	10.00			
K- J	0 / 1407	-34.5	-34.5	1	0.41 (4)	10.00			
J- I	0 / 1407	-34.5	-34.5	1	0.41 (4)	10.00			
I- H	0 / 0	-34.5	-34.5	1	0.29 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-302	-302	---	FRONT	VERT	TOTAL
E	12-5-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.62 (B-C:1) , BC=0.41 (J-K:4) , WB=0.35 (F-I:1) , SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.53 (K) (INPUT = 1.00)

A-150734440

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

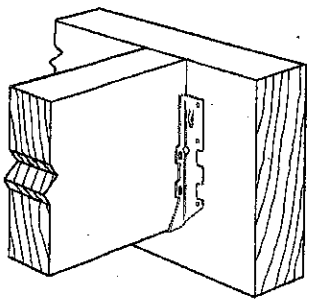
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

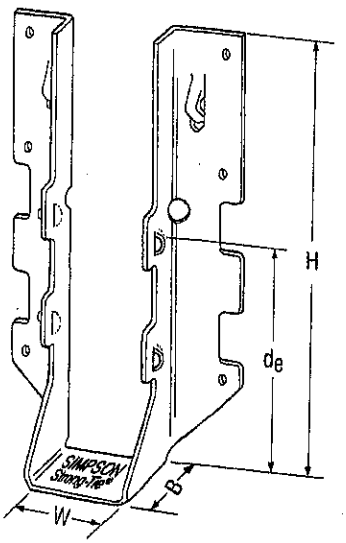
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



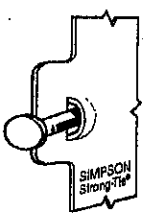
Typical LUS Installation



LUS28

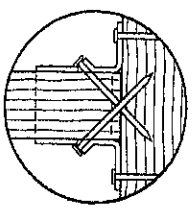
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

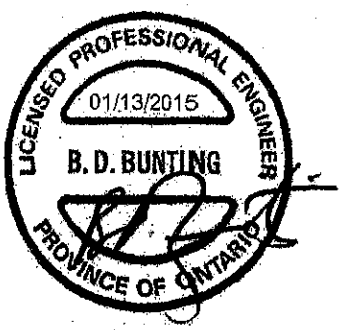


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

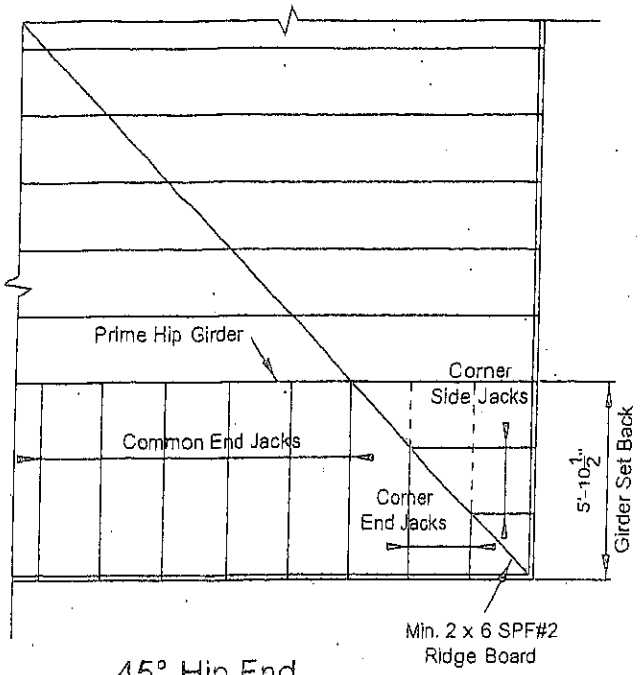
U.S. Patent 5,603,580



Double Shear Nailing Top View.



STRACON ENGINEERING INC.



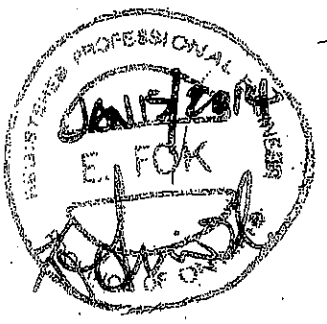
45° Hip End

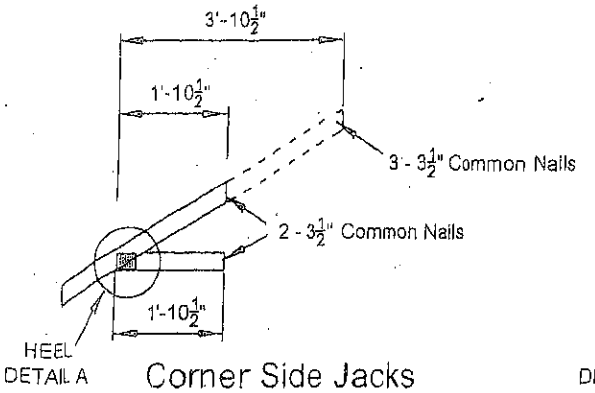
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

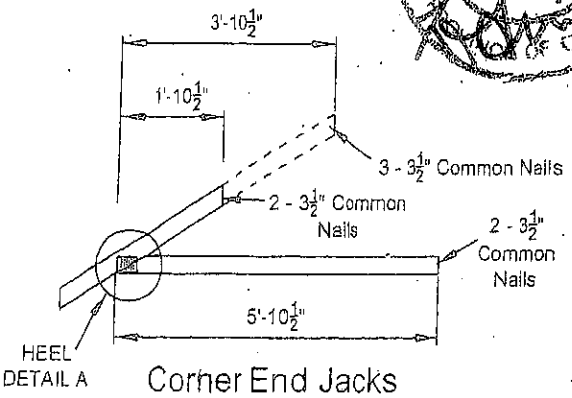
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

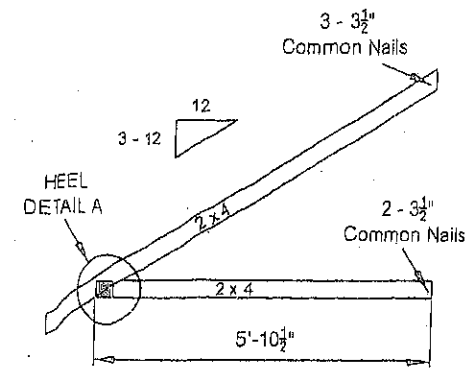




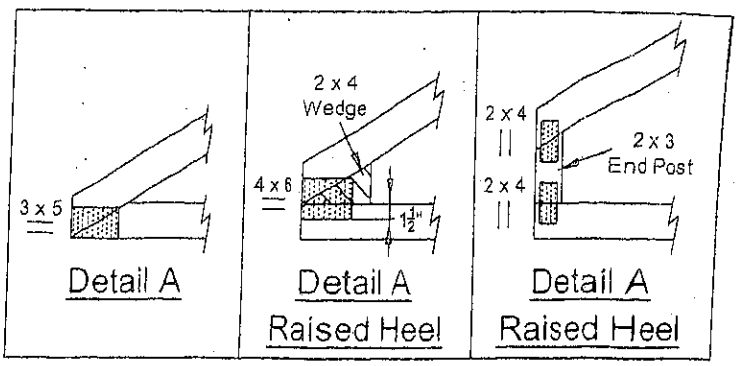
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008