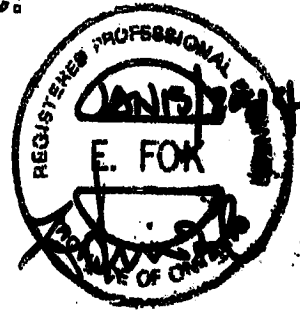


Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



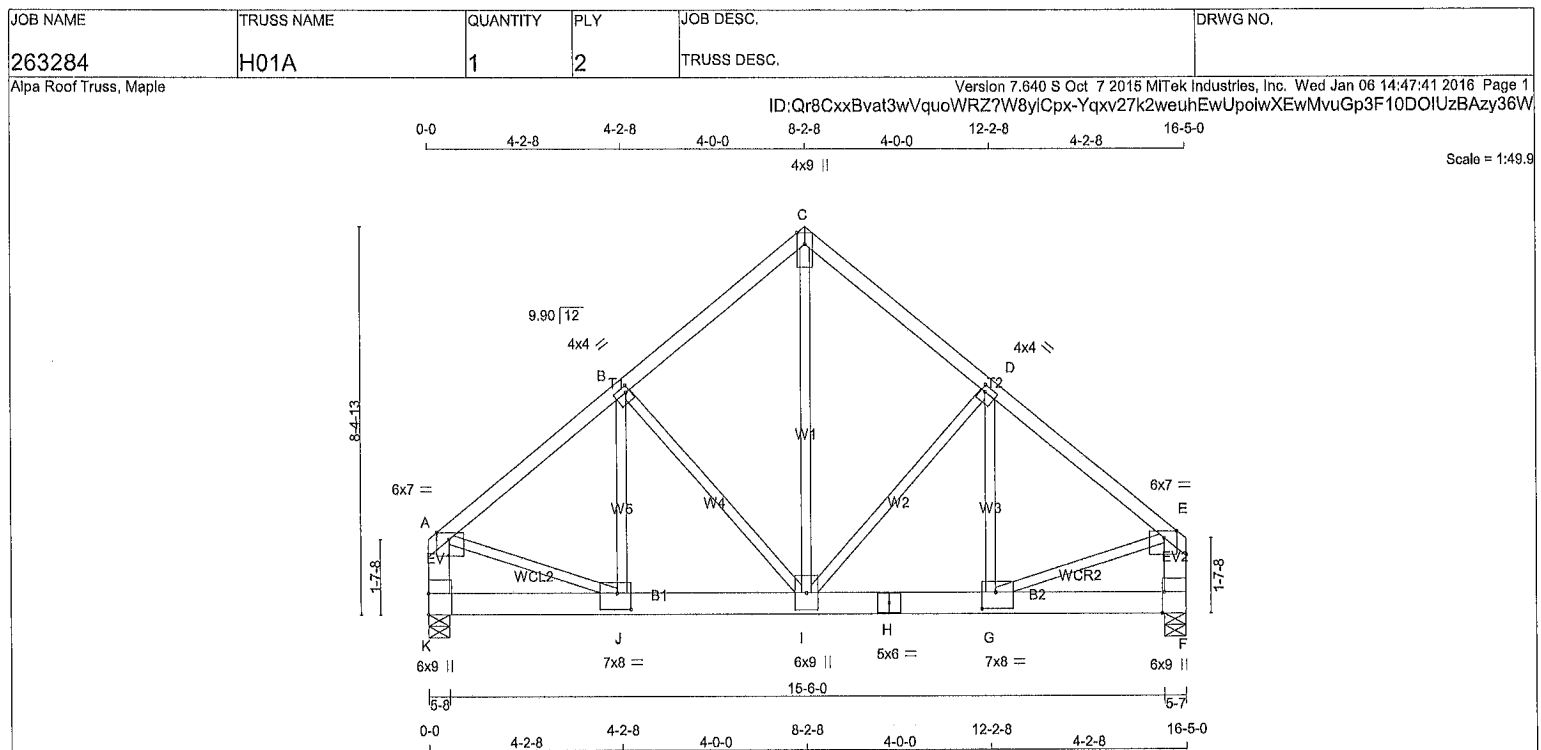
RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - E 1	12	TOP
K - A 2	12	TOP
F - E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - H 2	6	SIDE(404.8)
H - F 2	6	SIDE(404.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	7.0	1.75	3.25
B	TMVW-t	MT20	4.0	4.0	1.50	1.00
C	TTW+p	MT20	4.0	9.0	Edge	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
	VERT	HORZ	DOWN	IN-SX	IN-SX
K	7652	0	7652	0	5-8
F	7652	0	7652	0	5-7

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	6183	3624 / 0	1311 / 0	0 / 0	0 / 0	1248 / 0	0 / 0
F	6183	3624 / 0	1311 / 0	0 / 0	0 / 0	1248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F
BEARING SIZE FACTOR = 1.15 AT JNT(S) K, F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-6974 / 0	I-C	0 / 6376
B-C	-5280 / 0	I-D	-2054 / 0
C-D	-5280 / 0	G-D	0 / 2219
D-E	-6974 / 0	B-I	-2054 / 0
K-A	-6170 / 0	J-B	0 / 2219
F-E	-6170 / 0	A-J	0 / 5611
		G-E	0 / 5611
K-J	0 / 0		
J-I	0 / 5393		
I-H	0 / 5393		
H-G	0 / 5393		
G-F	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN./C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 28-5-0
END DISTANCE = 16-5-0
END SPAN CARRIED = 28-5-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.36 (D-E:1), BC=0.79 (G-I:1), WB=0.79 (C-I:1), SSI=0.68 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

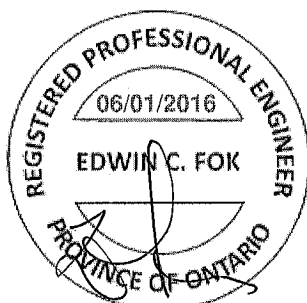
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.04 (H) (INPUT = 1.00)



A-1501160

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
263284	H01A	1	2	TRUSS DESC.	
Alpa Roof Truss, Maple					
Version 7.640 S Oct 7 2015 MITek Industries, Inc. Wed Jan 06 14:47:42 2016 Page 2					
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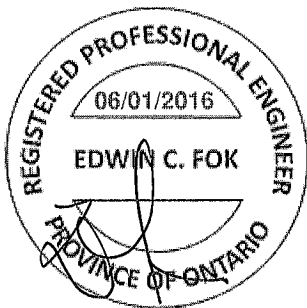
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TMWW-t	MT20	4.0	4.0	1.50	1.00
E	TMWW-p	MT20	6.0	7.0	1.75	3.25
F	BMV1+t	MT20	6.0	9.0	Edge	0.50
G	BMWW-t	MT20	7.0	8.0	4.25	3.50
H	BS-t	MT20	5.0	6.0		
I	BMWWWW-t	MT20	6.0	9.0		
J	BMWW-t	MT20	7.0	8.0	4.25	3.50
K	BMV1+t	MT20	6.0	9.0	5.50	

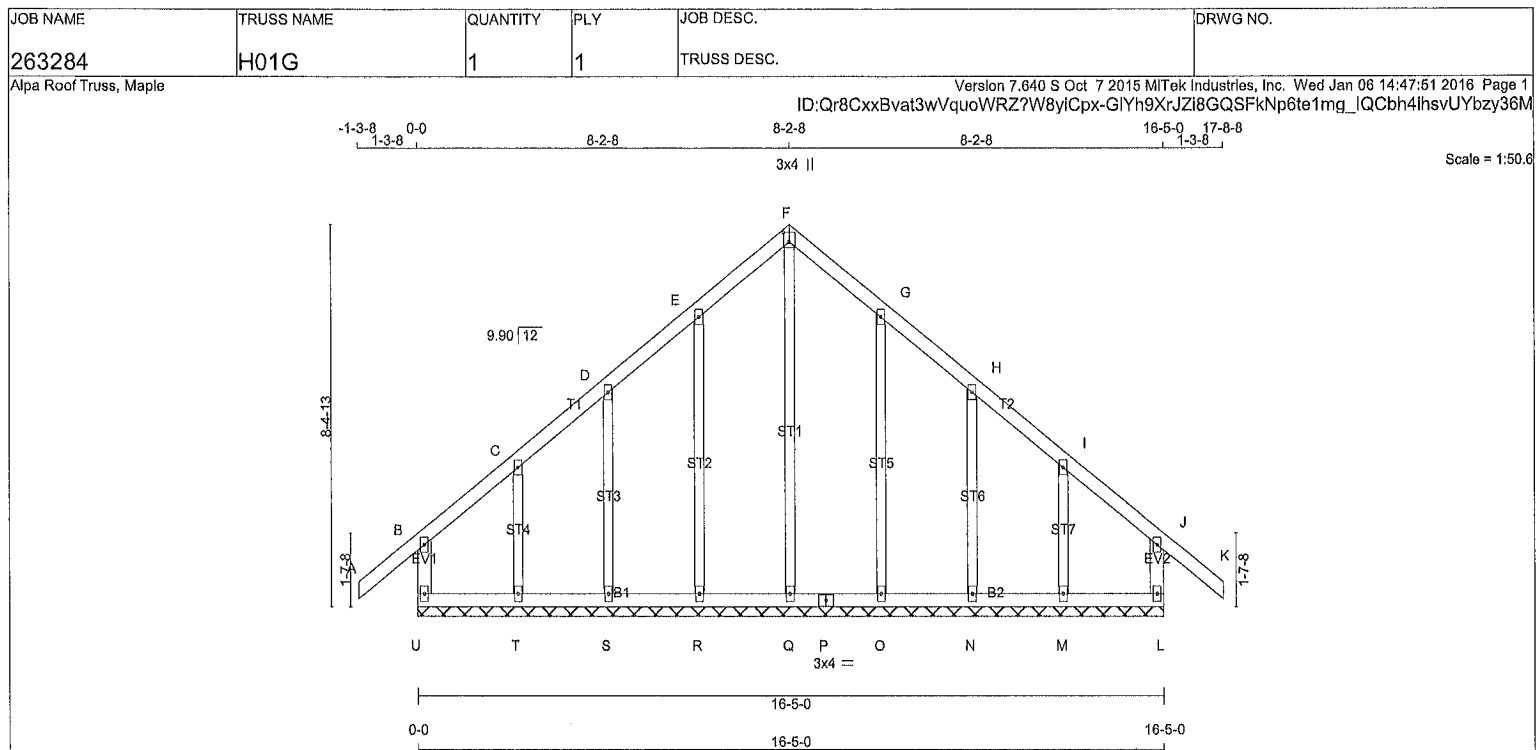
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



A-1601160(2)



TOTAL WEIGHT = 78 lb
(M)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
U - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
C, D, E, G, H, I	TMW+w	MT20	2.0	4.0
F	TTW+p	MT20	3.0	4.0 2.50 1.50
J	TMV+p	MT20	2.0	4.0
L	BMV1+p	MT20	2.0	4.0
M, N, O, Q, R, S, T	BMW1+w	MT20	2.0	4.0
P	BS-t	MT20	3.0	4.0
U	BMV1+p	MT20	2.0	4.0

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM TO		FR-TO			
U-B	-255 / 0	0.0 0.0 0.03 (1)	7.81	Q-F	-176 / 0	0.24 (1)	
A-B	0 / 42	-94.6 -94.6 0.13 (1)	10.00	R-E	-191 / 0	0.15 (1)	
B-C	-36 / 0	-94.6 -94.6 0.05 (1)	6.25	S-D	-185 / 0	0.07 (1)	
C-D	-27 / 0	-94.6 -94.6 0.04 (1)	6.25	T-C	-188 / 0	0.04 (1)	
D-E	-21 / 0	-94.6 -94.6 0.04 (1)	6.25	O-G	-191 / 0	0.15 (1)	
E-F	-18 / 0	-94.6 -94.6 0.05 (1)	6.25	N-H	-185 / 0	0.07 (1)	
F-G	-18 / 0	-94.6 -94.6 0.05 (1)	6.25	M-I	-188 / 0	0.04 (1)	
G-H	-21 / 0	-94.6 -94.6 0.04 (1)	6.25				
H-I	-27 / 0	-94.6 -94.6 0.04 (1)	6.25				
I-J	-36 / 0	-94.6 -94.6 0.05 (1)	6.25				
J-K	0 / 42	-94.6 -94.6 0.13 (1)	10.00				
L-J	-255 / 0	0.0 0.0 0.03 (1)	7.81				
U-T	0 / 26	-28.0 -28.0 0.03 (3)	10.00				
T-S	0 / 20	-28.0 -28.0 0.03 (3)	10.00				
S-R	0 / 17	-28.0 -28.0 0.02 (2)	10.00				
R-Q	0 / 14	-28.0 -28.0 0.02 (2)	10.00				
Q-P	0 / 14	-28.0 -28.0 0.02 (2)	10.00				
P-O	0 / 14	-28.0 -28.0 0.02 (2)	10.00				
O-N	0 / 17	-28.0 -28.0 0.02 (2)	10.00				
N-M	0 / 20	-28.0 -28.0 0.03 (3)	10.00				
M-L	0 / 26	-28.0 -28.0 0.03 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI=TC=0.13 (A-B:1), BC=0.03 (L-M:3), WB=0.24 (F-Q:1), SSI=0.08 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

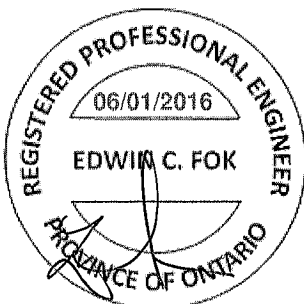
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

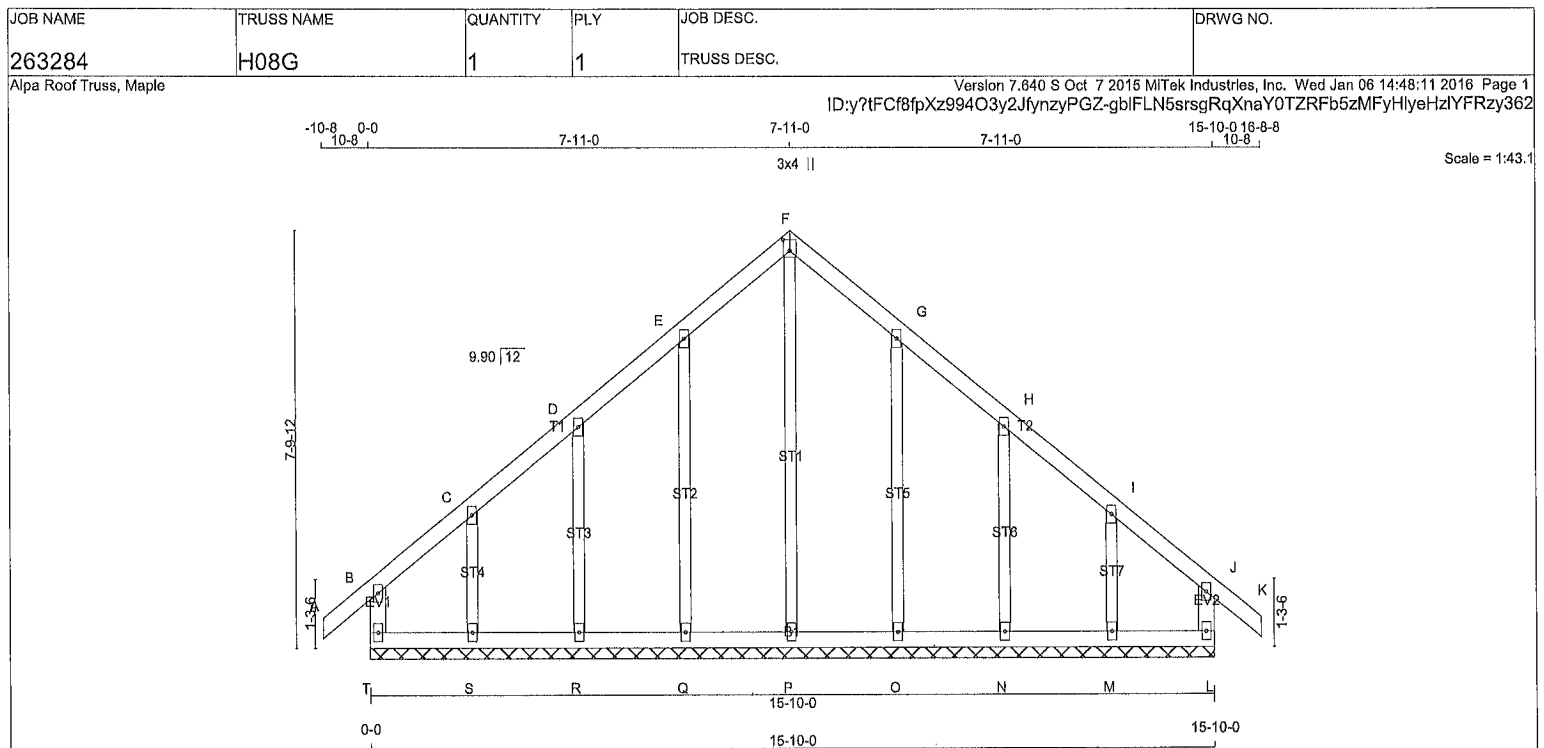
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (F) (INPUT = 0.90)
JSI METAL= 0.08 (J) (INPUT = 1.00)



A:1601161



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C, D, E, G, H, I					
C	TMW+w	MT20	2.0	4.0	
F	TTW+p	MT20	3.0	4.0	2.50 1.50
J	TMV+p	MT20	2.0	4.0	
L	BMV1+p	MT20	2.0	4.0	
M, N, O, P, Q, R, S					
M	BMW1+w	MT20	2.0	4.0	
T	BMV1+p	MT20	2.0	4.0	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
T-B	-206 / 0	0.0 0.0 0.03 (1)	7.81	P-F	-164 / 0	0.18 (1)	
A-B	0 / 29	-94.6 -94.6 0.06 (1)	10.00	Q-E	-190 / 0	0.11 (1)	
B-C	-46 / 0	-94.6 -94.6 0.04 (1)	6.25	R-D	-187 / 0	0.06 (1)	
C-D	-37 / 0	-94.6 -94.6 0.04 (1)	6.25	S-C	-175 / 0	0.03 (1)	
D-E	-33 / 0	-94.6 -94.6 0.04 (1)	6.25	O-G	-190 / 0	0.11 (1)	
E-F	-28 / 0	-94.6 -94.6 0.05 (1)	6.25	N-H	-187 / 0	0.06 (1)	
F-G	-28 / 0	-94.6 -94.6 0.05 (1)	6.25	M-I	-175 / 0	0.03 (1)	
G-H	-33 / 0	-94.6 -94.6 0.04 (1)	6.25				
H-I	-37 / 0	-94.6 -94.6 0.04 (1)	6.25				
I-J	-46 / 0	-94.6 -94.6 0.04 (1)	6.25				
J-K	0 / 29	-94.6 -94.6 0.06 (1)	10.00				
L-J	-206 / 0	0.0 0.0 0.03 (1)	7.81				
T-S	0 / 36	-28.0 -28.0 0.03 (2)	10.00				
S-R	0 / 29	-28.0 -28.0 0.03 (2)	10.00				
R-Q	0 / 25	-28.0 -28.0 0.02 (2)	10.00				
Q-P	0 / 22	-28.0 -28.0 0.02 (2)	10.00				
P-O	0 / 22	-28.0 -28.0 0.02 (2)	10.00				
O-N	0 / 25	-28.0 -28.0 0.02 (2)	10.00				
N-M	0 / 29	-28.0 -28.0 0.03 (2)	10.00				
M-L	0 / 36	-28.0 -28.0 0.03 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06 (J-K:1), BC=0.03 (L-M:2), WB=0.18 (F-P:1), SSI=0.06 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

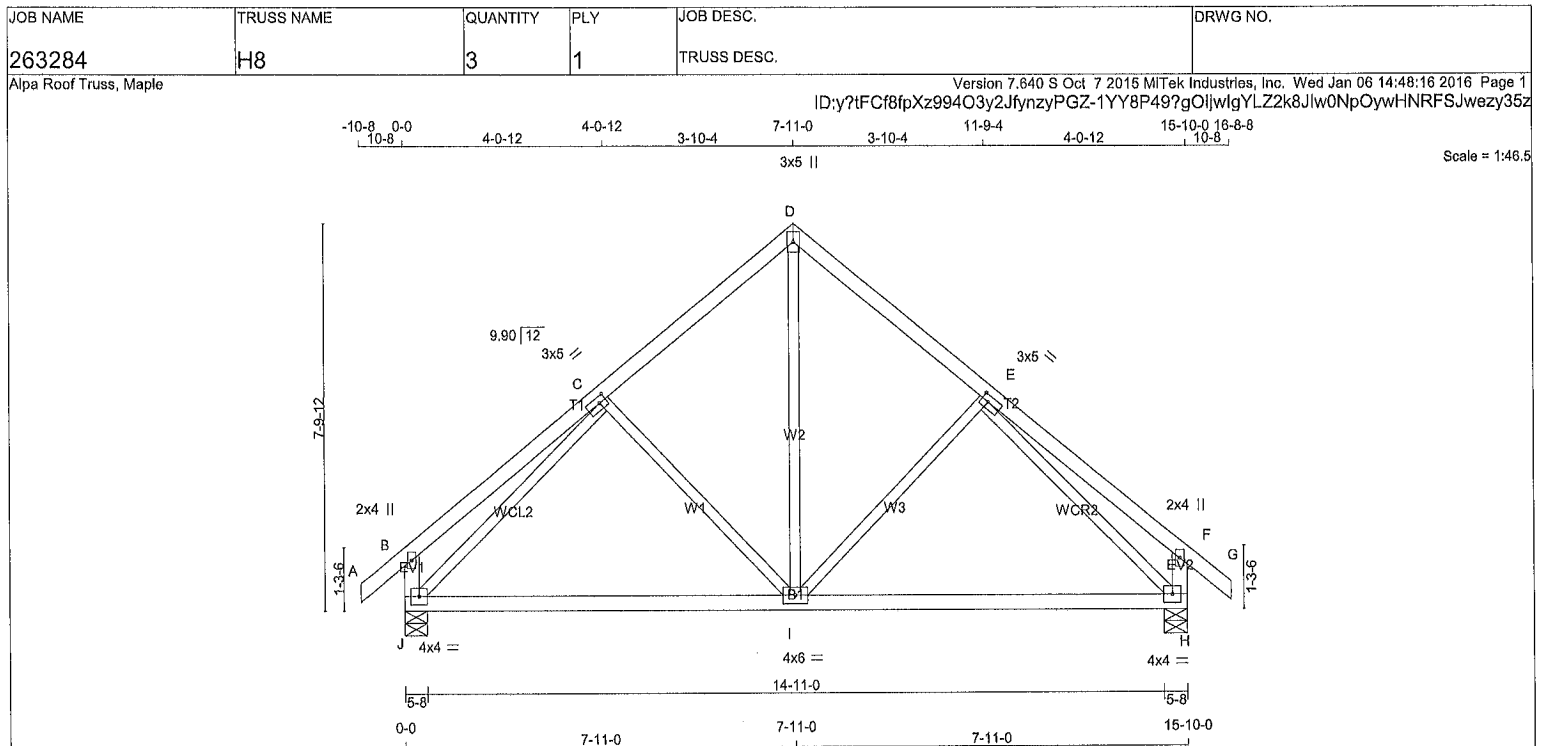
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (F) (INPUT = 0.90)
JSI METAL = 0.07 (F) (INPUT = 1.00)



A-1601162



TOTAL WEIGHT = 3 X 70 = 210 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	1.75
D	TTW+p	MT20	3.0	5.0		
E	TMWW-t	MT20	3.0	5.0	1.50	1.75
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
J	1062	0	1062	0	5-8	1-8
H	1062	0	1062	0	5-8	1-8

UNFACTORED REACTIONS

JT	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
J	846	516 / 0	166 / 0	0 / 0	0 / 0	164 / 0
H	846	516 / 0	166 / 0	0 / 0	0 / 0	164 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 29	-94.6	-94.6 0.06 (1)	10.00	C-I	-226 / 24	0.13 (1)
B-C	0 / 28	-94.6	-94.6 0.24 (1)	10.00	I-D	0 / 603	0.14 (1)
C-D	-730 / 0	-94.6	-94.6 0.19 (1)	6.25	I-E	-227 / 24	0.13 (1)
D-E	-730 / 0	-94.6	-94.6 0.19 (1)	6.25	J-C	-1020 / 0	0.56 (1)
E-F	0 / 28	-94.6	-94.6 0.24 (1)	10.00	E-H	-1020 / 0	0.56 (1)
F-G	0 / 29	-94.6	-94.6 0.06 (1)	10.00			
J-B	-234 / 0	0.0	0.0 0.02 (1)	7.81			
H-F	-234 / 0	0.0	0.0 0.02 (1)	7.81			
J-I	0 / 698	-28.0	-28.0 0.58 (2)	10.00			
I-H	0 / 698	-28.0	-28.0 0.58 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/988 (0.19")

CSI: TC=0.24 (E-F:1), BC=0.58 (H-I:2), WB=0.56 (E-H:1), SSI=0.19 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

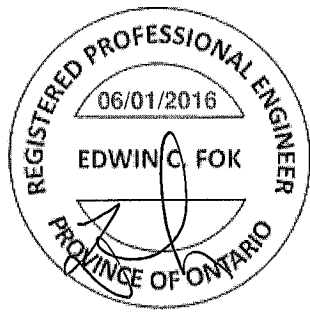
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

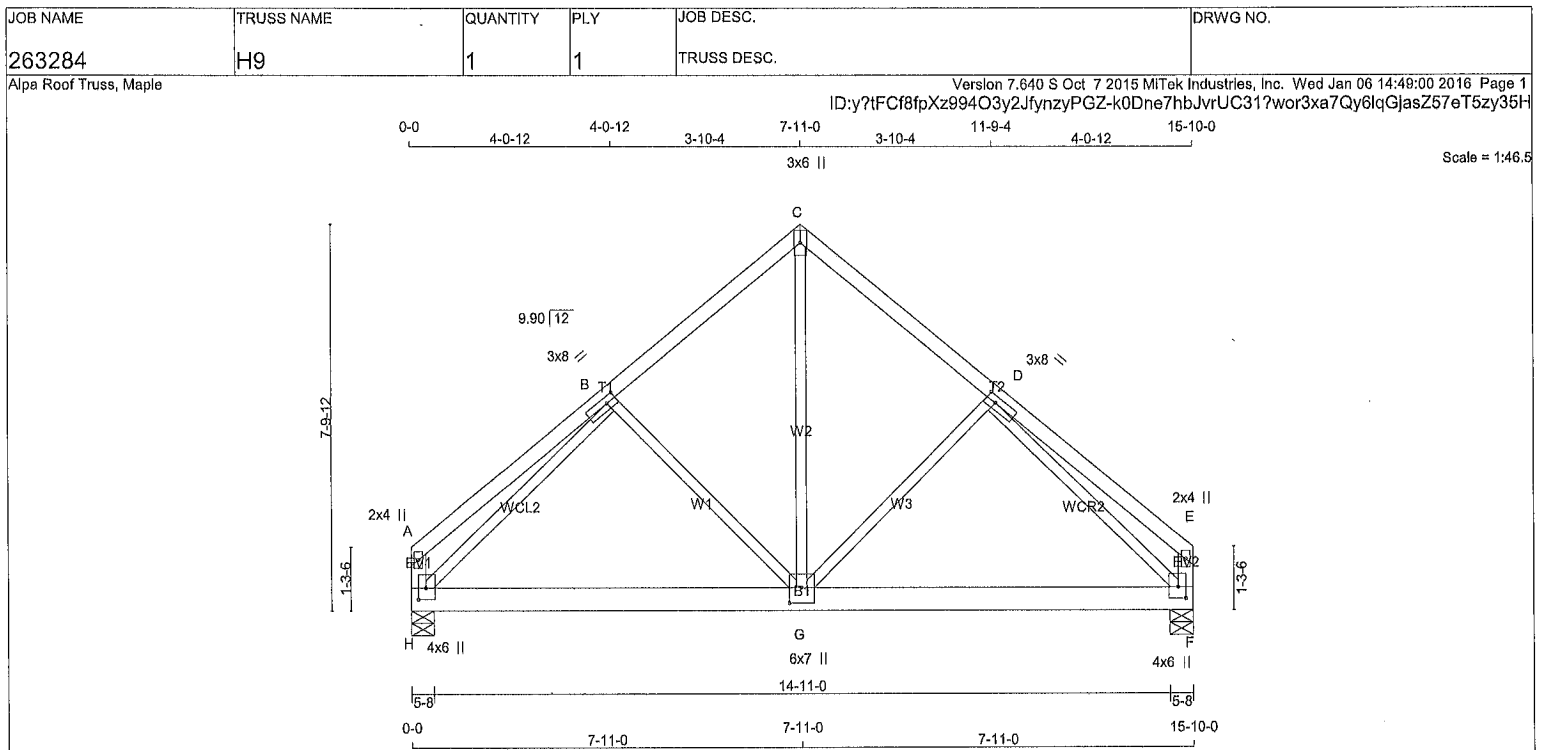
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL = 0.30 (E) (INPUT = 1.00)



A-1601163



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	3.0	8.0	1.50	2.50
C	TTW+p	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	8.0	1.50	2.50
E	TMV+p	MT20	2.0	4.0		
F	BMVW1+p	MT20	4.0	6.0	2.75	1.75
G	BMVWW+t	MT20	6.0	7.0	3.75	3.00
H	BMVW1+p	MT20	4.0	6.0	2.75	1.75

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	1911	0	1911	0	5-8	2-1
F	1911	0	1911	0	5-8	2-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
H	1544	905 / 0	327 / 0	0 / 0	0 / 0	312 / 0	0 / 0
F	1544	905 / 0	327 / 0	0 / 0	0 / 0	312 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-94.6 -94.6	0.24 (1)	10.00	B-G	-137 / 66	0.08 (1)
B-C	-1541 / 0	-94.6 -94.6	0.30 (1)	4.95	G-C	0 / 1632	0.40 (1)
C-D	-1541 / 0	-94.6 -94.6	0.30 (1)	4.95	G-D	-137 / 66	0.08 (1)
D-E	0 / 26	-94.6 -94.6	0.24 (1)	10.00	H-B	-1828 / 0	0.98 (1)
H-A	-146 / 0	0.0 0.0	0.02 (1)	7.81	D-F	-1828 / 0	0.98 (1)
F-E	-146 / 0	0.0 0.0	0.02 (1)	7.81			
H-G	0 / 1264	-146.8 -146.8	0.96 (1)	10.00			
G-F	0 / 1265	-146.8 -146.8	0.96 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN./C

GIRDER TYPE: CstdGlrder
START DISTANCE = 0-0
START SPAN CARRIED = 6-10-8
END DISTANCE = 15-10-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(56 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/892 (0.21")

CSI: TC=0.30 (C-D:1), BC=0.96 (F-G:1),
WB=0.98 (D-F:1), SSI=0.48 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

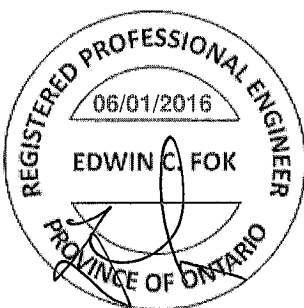
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

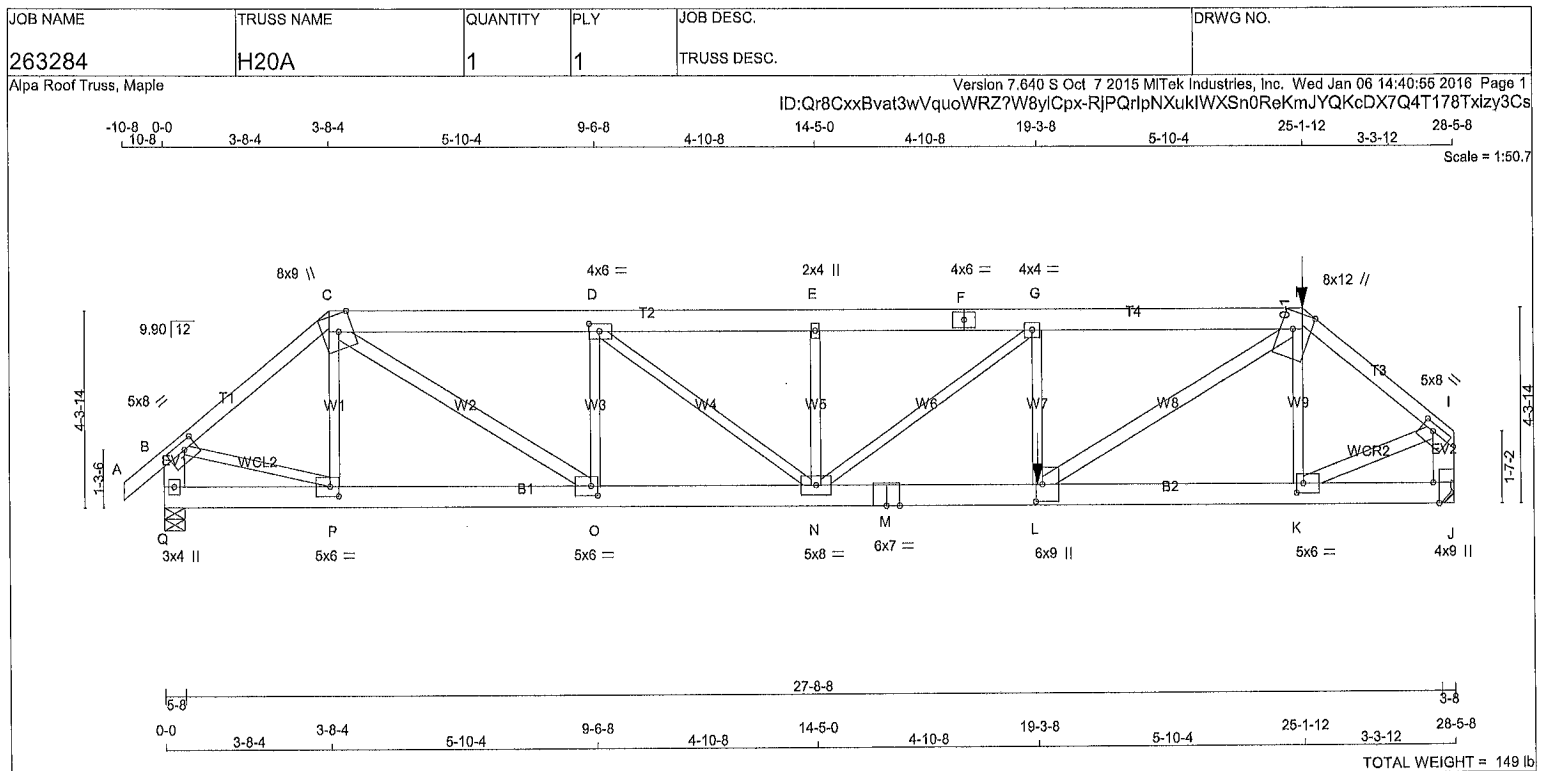
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)



A-1601164



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - O	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.25
C	TTWW+m	MT20	8.0	9.0	Edge	3.75
D	TMVW-t	MT20	4.0	6.0	2.00	2.75
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	8.0	12.0	Edge	4.75
I	TMVW-t	MT20	5.0	8.0	1.75	3.25
J	BMV1+t	MT20	4.0	9.0	Edge	1.50
K	BMVW-t	MT20	5.0	6.0	2.50	1.75
L	BMVW-t	MT20	6.0	9.0	4.50	1.75
M	BS-t	MT20	6.0	7.0		
N	BMVWVW-t	MT20	5.0	8.0		
O	BMVW-t	MT20	5.0	6.0	2.50	1.75
P	BMVW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
Q	2576	0	2576	0
J	3743	0	3743	0

MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	2068	1234 / 0	424 / 0	0 / 0	0 / 0	410 / 0	0 / 0	0 / 0
J	3036	1761 / 0	656 / 0	0 / 0	0 / 0	619 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO					FR-TO		
A-B	0 / 29	-94.6	-94.6	0.07 (1)	10.00	P-C	-335 / 33
B-C	-2808 / 0	-94.6	-94.6	0.40 (1)	3.75	C-O	0 / 3116
C-D	-4759 / 0	-94.6	-94.6	0.39 (1)	3.67	O-D	-1575 / 0
D-E	-5968 / 0	-94.6	-94.6	0.44 (1)	3.25	D-N	0 / 1544
E-F	-5968 / 0	-94.6	-94.6	0.54 (1)	3.13	N-E	-338 / 0
F-G	-5968 / 0	-94.6	-94.6	0.54 (1)	3.13	N-G	-780 / 0
G-H	-6580 / 0	-186.2	-186.2	0.81 (1)	2.60	L-G	-455 / 140
H-I	-3940 / 0	-94.6	-94.6	0.45 (1)	3.15	L-H	0 / 4260
Q-B	-2540 / 0	0.0	0.0	0.18 (1)	6.46	K-H	-702 / 47
J-I	-3702 / 0	0.0	0.0	0.27 (1)	5.51	B-P	0 / 2227
						K-I	0 / 3225
Q-P	0 / 0	-28.0	-28.0	0.08 (2)	10.00		
P-O	0 / 2162	-28.0	-28.0	0.34 (1)	10.00		
O-N	0 / 4759	-28.0	-28.0	0.68 (1)	10.00		
N-M	0 / 6580	-28.0	-28.0	0.95 (1)	10.00		
M-L	0 / 6580	-28.0	-28.0	0.95 (1)	10.00		
L-K	0 / 3031	-55.1	-55.1	0.53 (1)	10.00		
K-J	0 / 0	-55.1	-55.1	0.19 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	25-1-12	-228	-228	---	FRONT	VERT	TOTAL
L	19-3-8	-1725	-1725	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 29.0	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 49.5	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 3-8-4
RIGHT SETBACK = 3-3-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 9-3-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/880 (0.39")

CSI: TC=0.81 (G-H:1), BC=0.95 (L-N:1), WB=0.75 (H-L:1), SSI=0.40 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

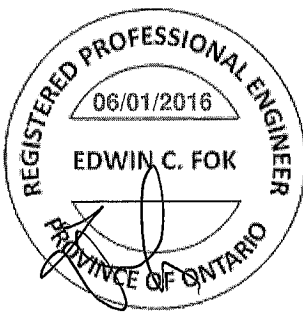
COMPANION LIVE LOAD FACTOR = 0.50

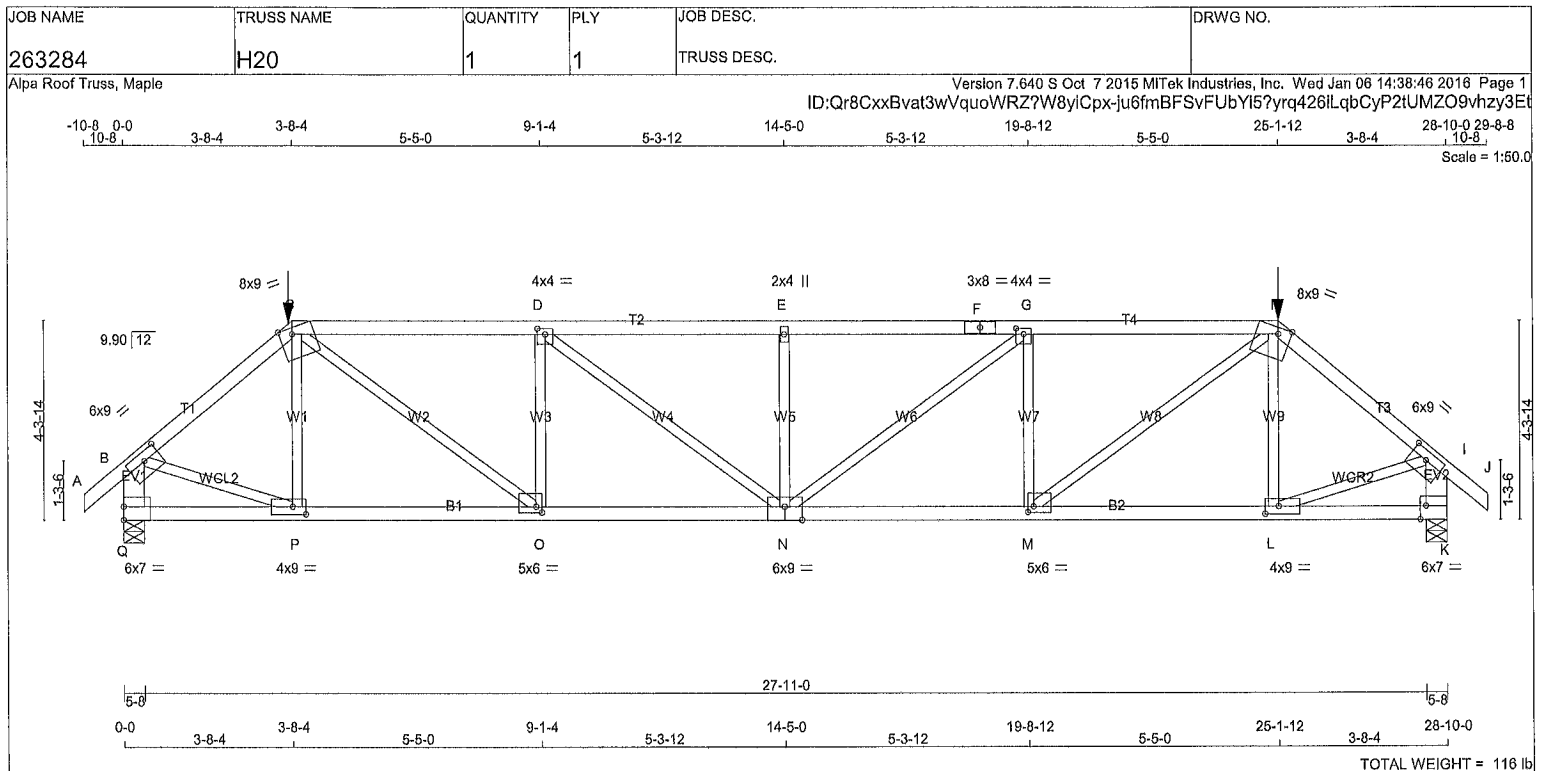
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

A-1601165

CONTINUED ON PAGE 2





LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	DRY	No.2	SPF	DESCR.	SPF	DESCR.
A - C	2x4	DRY	No.2	SPF	2100F 1.8E	SPF	2100F 1.8E
C - F	2x4	DRY	No.2	SPF	2100F 1.8E	SPF	2100F 1.8E
F - H	2x4	DRY	No.2	SPF	2100F 1.8E	SPF	2100F 1.8E
H - J	2x4	DRY	No.2	SPF	2100F 1.8E	SPF	2100F 1.8E
J - B	2x6	DRY	No.2	SPF	2100F 1.8E	SPF	2100F 1.8E
K - I	2x6	DRY	No.2	SPF	2100F 1.8E	SPF	2100F 1.8E
Q - N	2x4	DRY	No.2	SPF	2100F 1.8E	SPF	2100F 1.8E
N - K	2x4	DRY	No.2	SPF	2100F 1.8E	SPF	2100F 1.8E

ALL WEBS	2x3	DRY	No.2	SPF	DESCR.	SPF	DESCR.
EXCEPT							
DRY: SEASONED LUMBER.							

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.25	4.25
C	TTWW-m	MT20	8.0	9.0	Edge	3.25
D	TMVW-t	MT20	4.0	4.0	1.50	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW-t	MT20	4.0	4.0	1.50	2.00
H	TTWW-m	MT20	8.0	9.0	Edge	3.25
I	TMVW-t	MT20	6.0	9.0	2.25	4.25
K	BMV1-t	MT20	6.0	7.0	Edge	1.50
L	BMVW-t	MT20	4.0	9.0	2.00	3.50
M	BMVW-t	MT20	5.0	6.0	1.50	1.50
N	BSWWW-t	MT20	6.0	9.0	Edge	4.50
O	BMVW-t	MT20	5.0	6.0	1.50	1.50
P	BMVW-t	MT20	4.0	9.0	2.00	3.50
Q	BMV1-t	MT20	6.0	7.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 254.3 lbs FACTORED DOWN AT 25-1-12, AND 254.3 lbs FACTORED DOWN AT 3-8-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
Q	3487	0	3487	0	5-8
K	3487	0	3487	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2817	1653 / 0	596 / 0	0 / 0	0 / 0	568 / 0	0 / 0
K	2817	1653 / 0	596 / 0	0 / 0	0 / 0	568 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) Q, K (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 29	-94.6	-94.6	0.07 (1)	10.00	P-C	-444 / 152	0.13 (1)	
B-C	-3786 / 0	-94.6	-94.6	0.52 (1)	3.16	C-O	0 / 3371	0.83 (1)	
C-D	-5609 / 0	-186.2	-186.2	0.87 (1)	2.83	O-D	-1715 / 0	0.50 (1)	
D-E	-6403 / 0	-186.2	-186.2	0.94 (1)	2.56	D-N	0 / 996	0.25 (1)	
E-F	-6403 / 0	-186.2	-186.2	0.94 (1)	2.56	N-E	-915 / 0	0.27 (1)	
F-G	-6403 / 0	-186.2	-186.2	0.94 (1)	2.56	N-G	0 / 996	0.25 (1)	
G-H	-5609 / 0	-186.2	-186.2	0.87 (1)	2.83	M-G	-1715 / 0	0.50 (1)	
H-I	-3786 / 0	-94.6	-94.6	0.52 (1)	3.16	M-H	0 / 3371	0.83 (1)	
I-J	0 / 29	-94.6	-94.6	0.07 (1)	10.00	L-H	-444 / 152	0.13 (1)	
Q-B	-3414 / 0	0.0	0.0	0.25 (1)	5.71	B-P	0 / 3002	0.74 (1)	
K-I	-3414 / 0	0.0	0.0	0.25 (1)	5.71	L-I	0 / 3002	0.74 (1)	
Q-P	0 / 0	-55.1	-55.1	0.25 (2)	10.00				
P-O	0 / 2901	-55.1	-55.1	0.53 (2)	10.00				
O-N	0 / 5609	-55.1	-55.1	0.85 (1)	10.00				
N-M	0 / 5609	-55.1	-55.1	0.85 (1)	10.00				
M-L	0 / 2901	-55.1	-55.1	0.53 (2)	10.00				
L-K	0 / 0	-55.1	-55.1	0.25 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-8-4	-254	-254	---	FRONT	VERT	TOTAL
H	25-1-12	-254	-254	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	49.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-8-4
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/989 (0.32")
ALLOWABLE DEFL.(TL) = L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/689 (0.50")

CSI: TC=0.94 (D-E:1), BC=0.85 (N-O:1), WB=0.83 (H-M:1), SSI=0.52 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

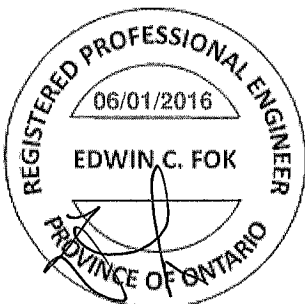
NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

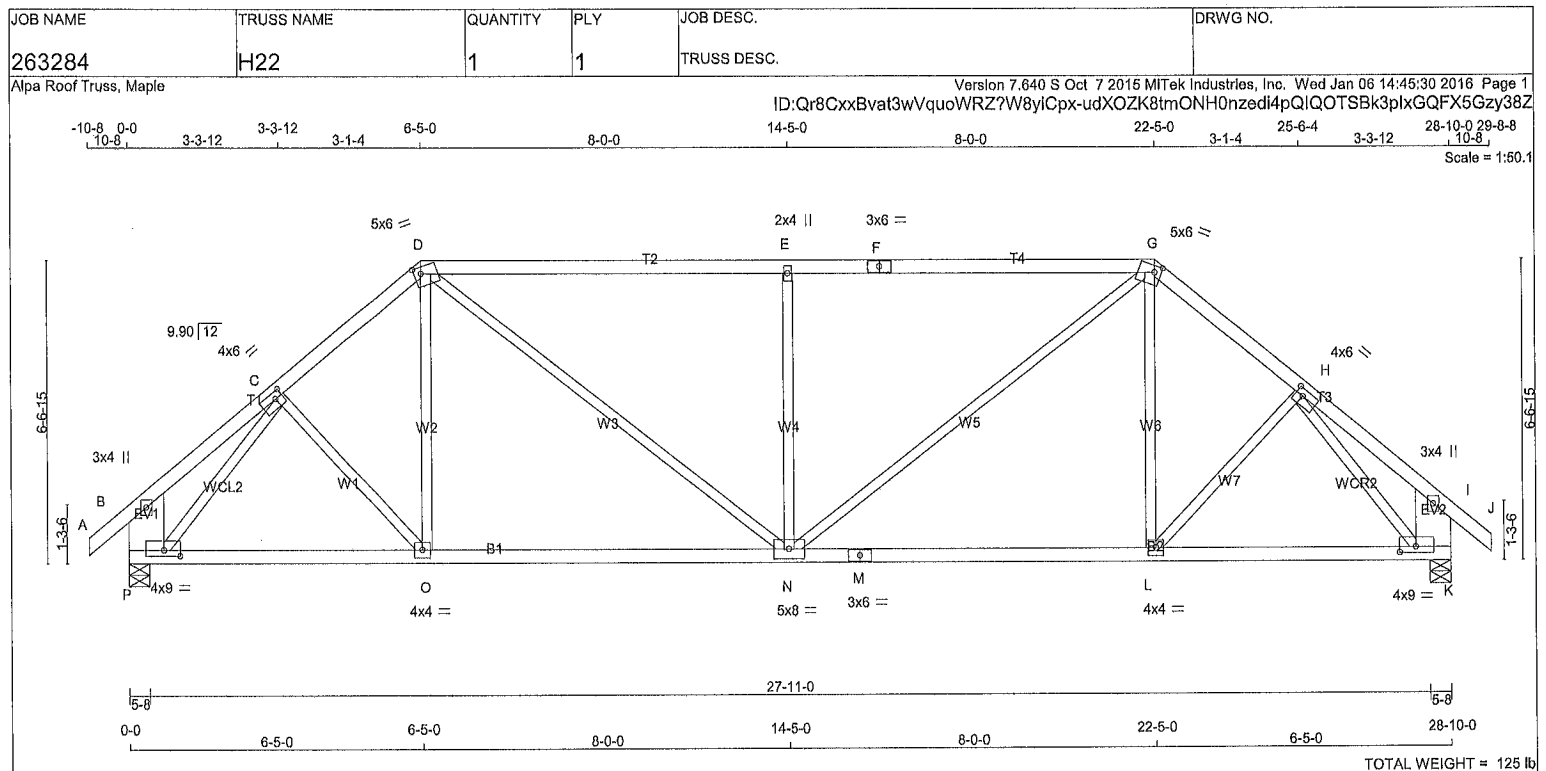
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.98 (N) (INPUT = 1.00)

A-1601166



A.1601167



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	2100F 1.8E	SPF
F - G	2x4	DRY	2100F 1.8E	SPF
G - J	2x4	DRY	No.2	SPF
P - B	2x10	DRY	No.2	SPF
K - I	2x10	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	1.75 2.00
D	TTWW-m	MT20	5.0	6.0	1.75 1.75
E	TMW+w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TTWW-m	MT20	5.0	6.0	1.75 1.75
H	TMWW-t	MT20	4.0	6.0	1.75 2.00
I	TMV+p	MT20	3.0	4.0	
K	BMVW1-t	MT20	4.0	9.0	1.50 4.25
L	BMWW-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	6.0	
N	BMWWW-t	MT20	5.0	8.0	
O	BMWW-t	MT20	4.0	4.0	
P	BMVW1-t	MT20	4.0	9.0	1.50 4.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT
P	1859	0	1859	0	5-8
K	1859	0	1859	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1490	893 / 0	303 / 0	0 / 0	0 / 0	294 / 0	0 / 0
K	1490	893 / 0	303 / 0	0 / 0	0 / 0	294 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	FACTORED	VERT. LOAD LC1	MEMB.	FORCE	FACTORED	MAX
(LBS)	(PLF)	CS1 (LC)	UNBRAC	(LBS)	(LBS)	CS1 (LC)	
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	
A-B	0 / 29	-94.6	-94.6 0.06 (1)	10.00	C-O	0 / 119	0.03 (3)
B-C	0 / 19	-94.6	-94.6 0.13 (1)	10.00	O-D	0 / 313	0.07 (2)
C-D	-1877 / 0	-94.6	-94.6 0.18 (1)	4.74	D-N	0 / 931	0.21 (1)
D-E	-2166 / 0	-94.6	-94.6 0.84 (1)	4.35	N-E	-928 / 0	0.66 (1)
E-F	-2166 / 0	-94.6	-94.6 0.84 (1)	4.35	N-G	0 / 931	0.21 (1)
F-G	-2166 / 0	-94.6	-94.6 0.84 (1)	4.35	L-G	0 / 313	0.07 (2)
G-H	-1877 / 0	-94.6	-94.6 0.18 (1)	4.74	L-H	0 / 119	0.03 (3)
H-I	0 / 19	-94.6	-94.6 0.13 (1)	10.00	P-C	-2112 / 0	0.68 (1)
I-J	0 / 29	-94.6	-94.6 0.06 (1)	10.00	H-K	-2112 / 0	0.68 (1)
P-B	-212 / 0	0.0	0.0 0.01 (1)	7.81			
K-I	-212 / 0	0.0	0.0 0.01 (1)	7.81			
P-O	0 / 1404	-28.0	-28.0 0.50 (2)	10.00			
O-N	0 / 1432	-28.0	-28.0 0.51 (2)	10.00			
N-M	0 / 1432	-28.0	-28.0 0.51 (2)	10.00			
M-L	0 / 1432	-28.0	-28.0 0.51 (2)	10.00			
L-K	0 / 1404	-28.0	-28.0 0.50 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CS1: TC=0.84 (E-G:1), BC=0.51 (N-O:2), WB=0.68 (C-P:1), SSI=0.37 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

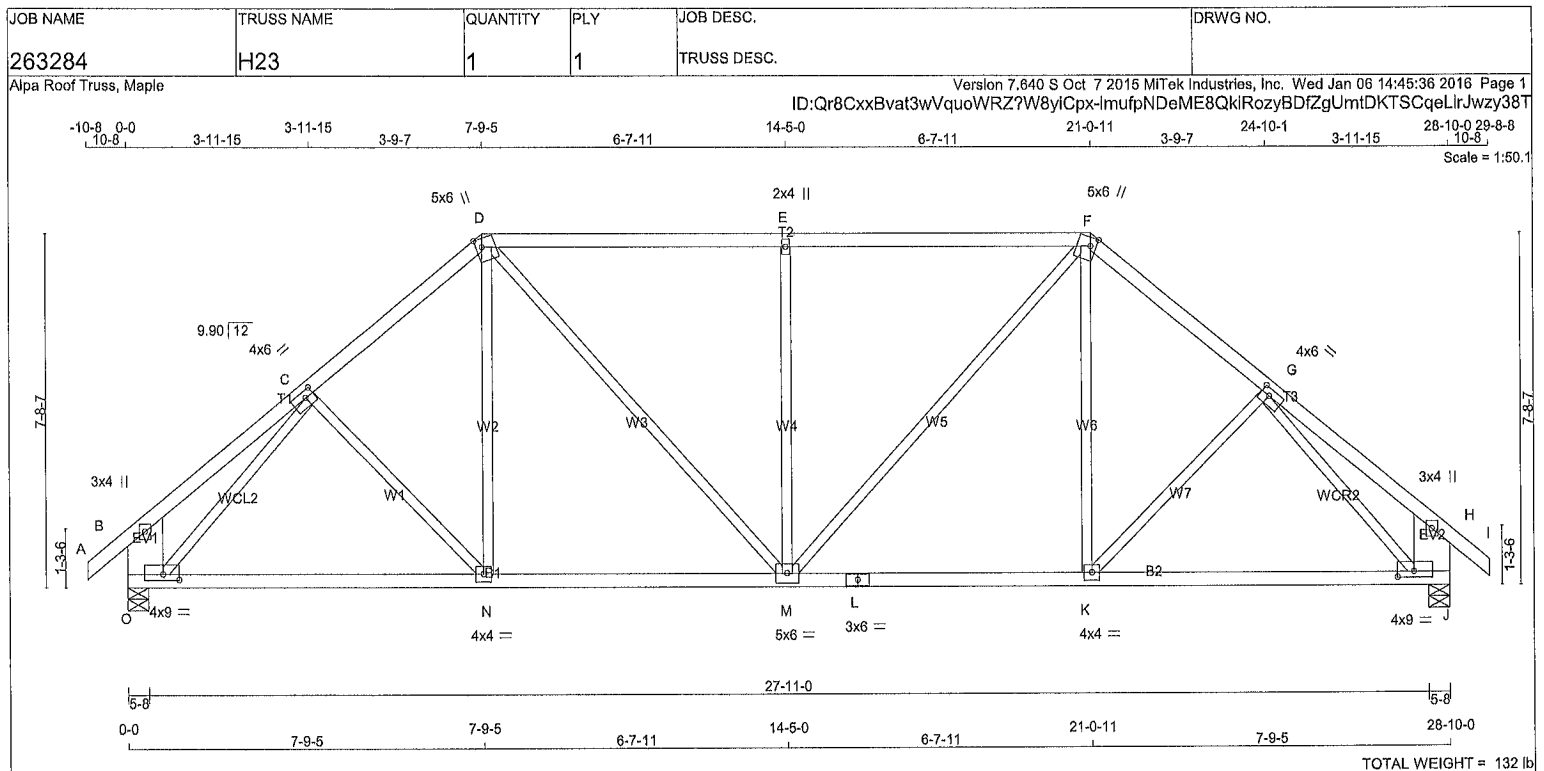
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL = 0.52 (H) (INPUT = 1.00)



A-1601168



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
O - B	2x10	DRY No.2	SPF
J - H	2x10	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	1.75	2.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	1.75	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	9.0	1.50	4.25
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	9.0	1.50	4.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	1859	0	1859	0	5-8	2-0
J	1859	0	1859	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	UNBRAC			
O	1490	893 / 0	303 / 0	0 / 0	0 / 0	294 / 0	0 / 0
J	1490	893 / 0	303 / 0	0 / 0	0 / 0	294 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 29	-94.6 -94.6 0.06 (1)	C-N	-80 / 88 0.05 (1)
B-C	0 / 26	-94.6 -94.6 0.22 (1)	N-D	0 / 386 0.09 (2)
C-D	-1817 / 0	-94.6 -94.6 0.21 (1)	D-M	0 / 618 0.14 (1)
D-E	-1795 / 0	-94.6 -94.6 0.60 (1)	M-E	-769 / 0 0.84 (1)
E-F	-1795 / 0	-94.6 -94.6 0.60 (1)	M-F	0 / 618 0.14 (1)
F-G	-1817 / 0	-94.6 -94.6 0.21 (1)	K-F	0 / 386 0.09 (2)
G-H	0 / 28	-94.6 -94.6 0.22 (1)	K-G	-80 / 88 0.05 (1)
H-I	0 / 29	-94.6 -94.6 0.06 (1)	O-C	-2101 / 0 0.97 (1)
O-B	-234 / 0	0.0 0.0 0.01 (1)	G-J	-2101 / 0 0.97 (1)
J-H	-234 / 0	0.0 0.0 0.01 (1)		
O-N	0 / 1434	-28.0 -28.0 0.55 (2)		
N-M	0 / 1383	-28.0 -28.0 0.56 (2)		
M-L	0 / 1383	-28.0 -28.0 0.56 (2)		
L-K	0 / 1383	-28.0 -28.0 0.56 (2)		
K-J	0 / 1434	-28.0 -28.0 0.55 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	29.0	PSF
BOT CH.	LL	PSF
DL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	49.5	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 37.6 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.60 (D-E:1), BC=0.56 (M-N:2), WB=0.97 (G-J:1), SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

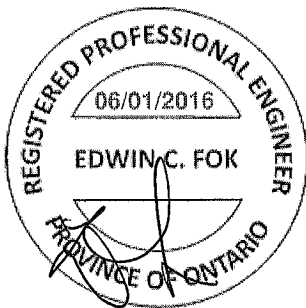
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

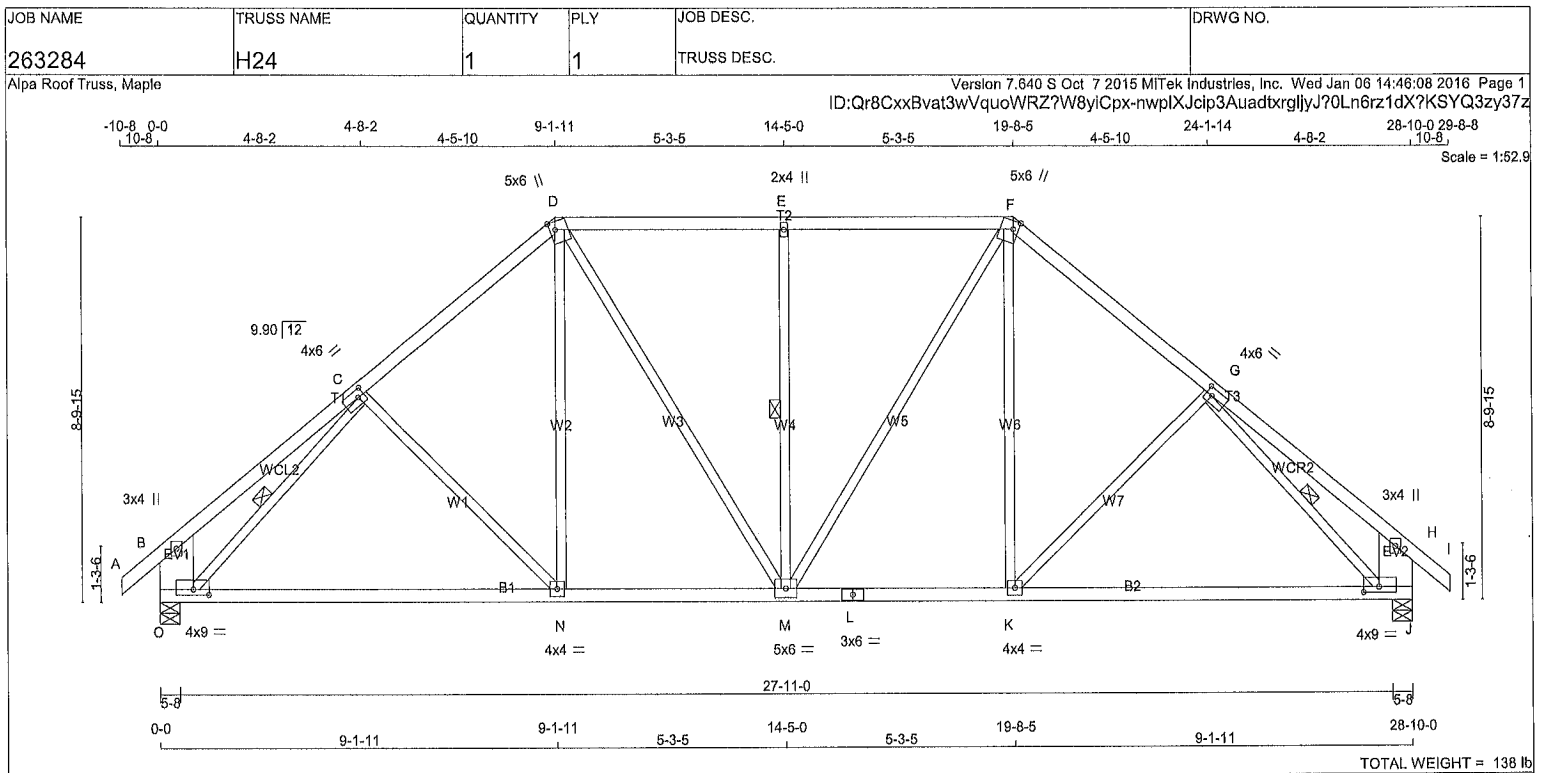
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.52 (G) (INPUT = 1.00)



A.1601169



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
F - I	2x4 DRY	No.2	SPF
O - B	2x10 DRY	No.2	SPF
J - H	2x10 DRY	No.2	SPF
O - L	2x4 DRY	No.2	SPF
L - J	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	1.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	9.0	1.50	4.25
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	9.0	1.50	4.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	1859	0	1859	0	5-8	2-0
J	1859	0	1859	0	5-8	2-0

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1490	893 / 0	303 / 0	0 / 0	0 / 0	294 / 0	0 / 0
J	1490	893 / 0	303 / 0	0 / 0	0 / 0	294 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC (LC)
FR-TO					FR-TO		
A-B	0 / 29	-94.6	-94.6 0.06 (1)	10.00	C-N	-183 / 64	0.15 (1)
B-C	0 / 32	-94.6	-94.6 0.31 (1)	10.00	N-D	0 / 482	0.11 (2)
C-D	-1745 / 0	-94.6	-94.6 0.29 (1)	4.78	D-M	0 / 387	0.09 (1)
D-E	-1527 / 0	-94.6	-94.6 0.36 (1)	4.92	M-E	-606 / 0	0.30 (1)
E-F	-1527 / 0	-94.6	-94.6 0.36 (1)	4.92	M-F	0 / 387	0.09 (1)
F-G	-1745 / 0	-94.6	-94.6 0.29 (1)	4.78	K-F	0 / 482	0.11 (2)
G-H	0 / 32	-94.6	-94.6 0.31 (1)	10.00	K-G	-183 / 64	0.15 (1)
H-I	0 / 29	-94.6	-94.6 0.06 (1)	10.00	O-C	-2082 / 0	0.56 (1)
O-B	-257 / 0	0.0	0.0 0.01 (1)	7.81	G-J	-2082 / 0	0.56 (1)
J-H	-257 / 0	0.0	0.0 0.01 (1)	7.81			
O-N	0 / 1448	-28.0	-28.0 0.69 (2)	10.00			
N-M	0 / 1324	-28.0	-28.0 0.69 (2)	10.00			
M-L	0 / 1324	-28.0	-28.0 0.69 (2)	10.00			
L-K	0 / 1324	-28.0	-28.0 0.69 (2)	10.00			
K-J	0 / 1448	-28.0	-28.0 0.69 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 49.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.96")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.29")
ALLOWABLE DEFL.(TL) = $L/360$ (0.96")
CALCULATED VERT. DEFL.(TL) = $L/731$ (0.47")

CSI: TC=0.36 (D-E:1), BC=0.69 (M-N:2), WB=0.56 (C-O:1), SI=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1665

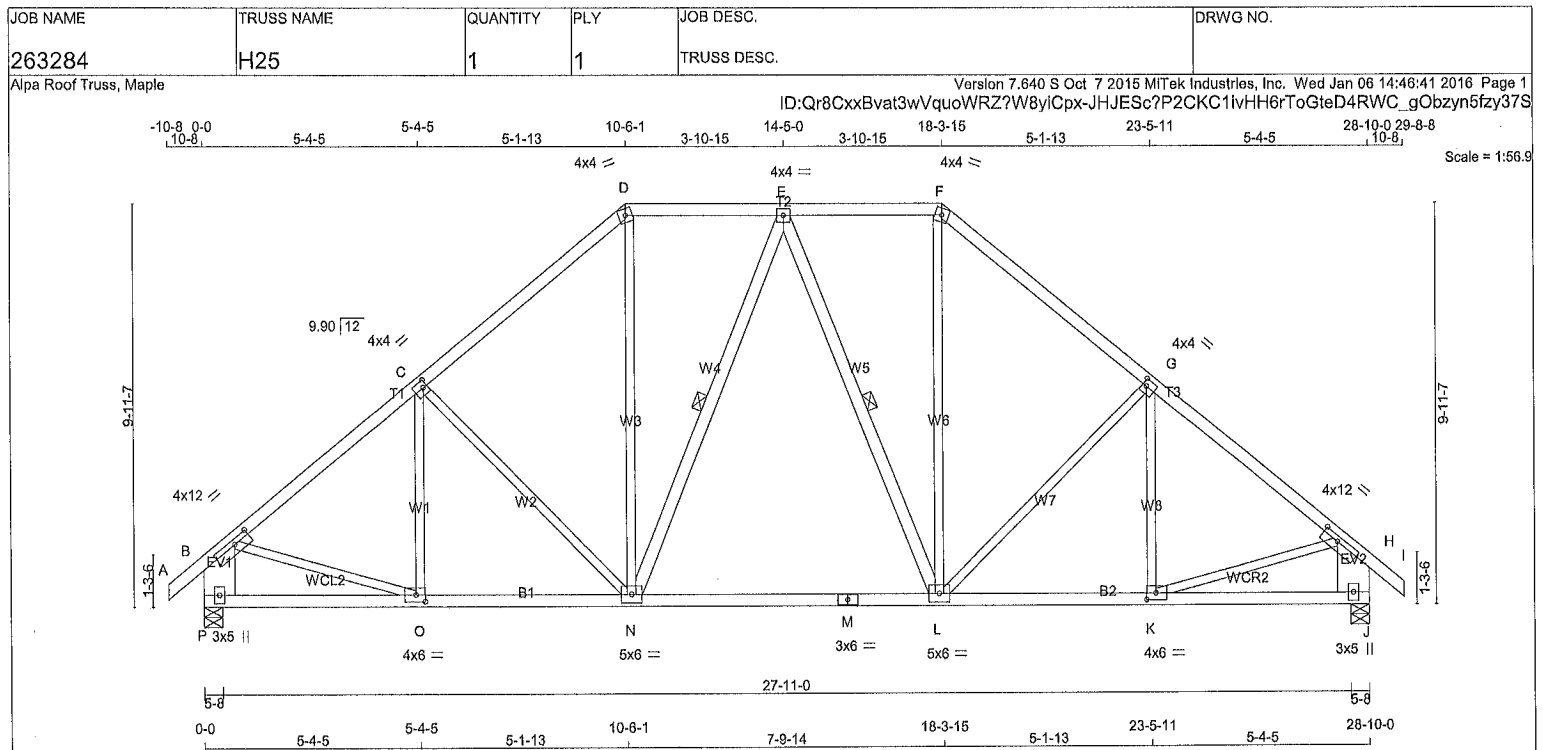
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 1.00)



A.1601170



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x10	DRY	No.2	SPF
J - H	2x10	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - E	2x4	DRY	No.2	SPF
E - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	12.0	1.50	5.00
C TMVW-t	MT20	4.0	4.0	2.00	1.25
D TTV-m	MT20	4.0	4.0		
E TMVW-t	MT20	4.0	4.0		
F TTV-m	MT20	4.0	4.0		
G TMVW-t	MT20	4.0	4.0	2.00	1.25
H TMVW-t	MT20	4.0	12.0	1.50	5.00
J BMV1+p	MT20	3.0	5.0		
K BMVW-t	MT20	4.0	6.0	2.00	2.75
L BMVWW-t	MT20	5.0	6.0		
M BS-t	MT20	3.0	6.0		
N BMVWW-t	MT20	5.0	6.0		
O BMVW-t	MT20	4.0	6.0	2.00	2.75
P BMV1+p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
P	1859	0	1859	0	0	5-8	2-0	
J	1859	0	1859	0	0	5-8	2-0	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1490	893 / 0	303 / 0	0 / 0	0 / 0	294 / 0	0 / 0
J	1490	893 / 0	303 / 0	0 / 0	0 / 0	294 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)
FR-TO					FR-TO		
A-B	0 / 29	-94.6	-94.6 0.06 (1)	10.00	O-C	-118 / 145	0.06 (1)
B-C	-1913 / 0	-94.6	-94.6 0.41 (1)	4.47	C-N	-389 / 0	0.44 (1)
C-D	-1639 / 0	-94.6	-94.6 0.38 (1)	4.78	N-D	0 / 688	0.15 (1)
D-E	-1239 / 0	-94.6	-94.6 0.19 (1)	5.57	N-E	-236 / 0	0.13 (1)
E-F	-1239 / 0	-94.6	-94.6 0.19 (1)	5.57	E-L	-236 / 0	0.13 (1)
F-G	-1639 / 0	-94.6	-94.6 0.38 (1)	4.78	L-F	0 / 688	0.15 (1)
G-H	-1913 / 0	-94.6	-94.6 0.41 (1)	4.47	L-G	-389 / 0	0.44 (1)
H-I	0 / 29	-94.6	-94.6 0.06 (1)	10.00	K-G	-118 / 145	0.06 (1)
P-B	-1794 / 0	0.0	0.0 0.07 (1)	7.81	B-O	0 / 1528	0.34 (1)
J-H	-1794 / 0	0.0	0.0 0.07 (1)	7.81	K-H	0 / 1528	0.34 (1)
P-O	0 / 0	-28.0	-28.0 0.17 (2)	10.00			
O-N	0 / 1504	-28.0	-28.0 0.43 (2)	10.00			
N-M	0 / 1326	-28.0	-28.0 0.41 (2)	10.00			
M-L	0 / 1326	-28.0	-28.0 0.41 (2)	10.00			
L-K	0 / 1504	-28.0	-28.0 0.43 (2)	10.00			
K-J	0 / 0	-28.0	-28.0 0.17 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.41 (B-C:1), BC=0.43 (N-O:2), WB=0.44 (C-N:1), SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

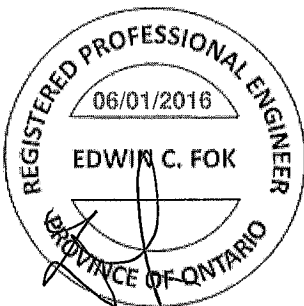
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

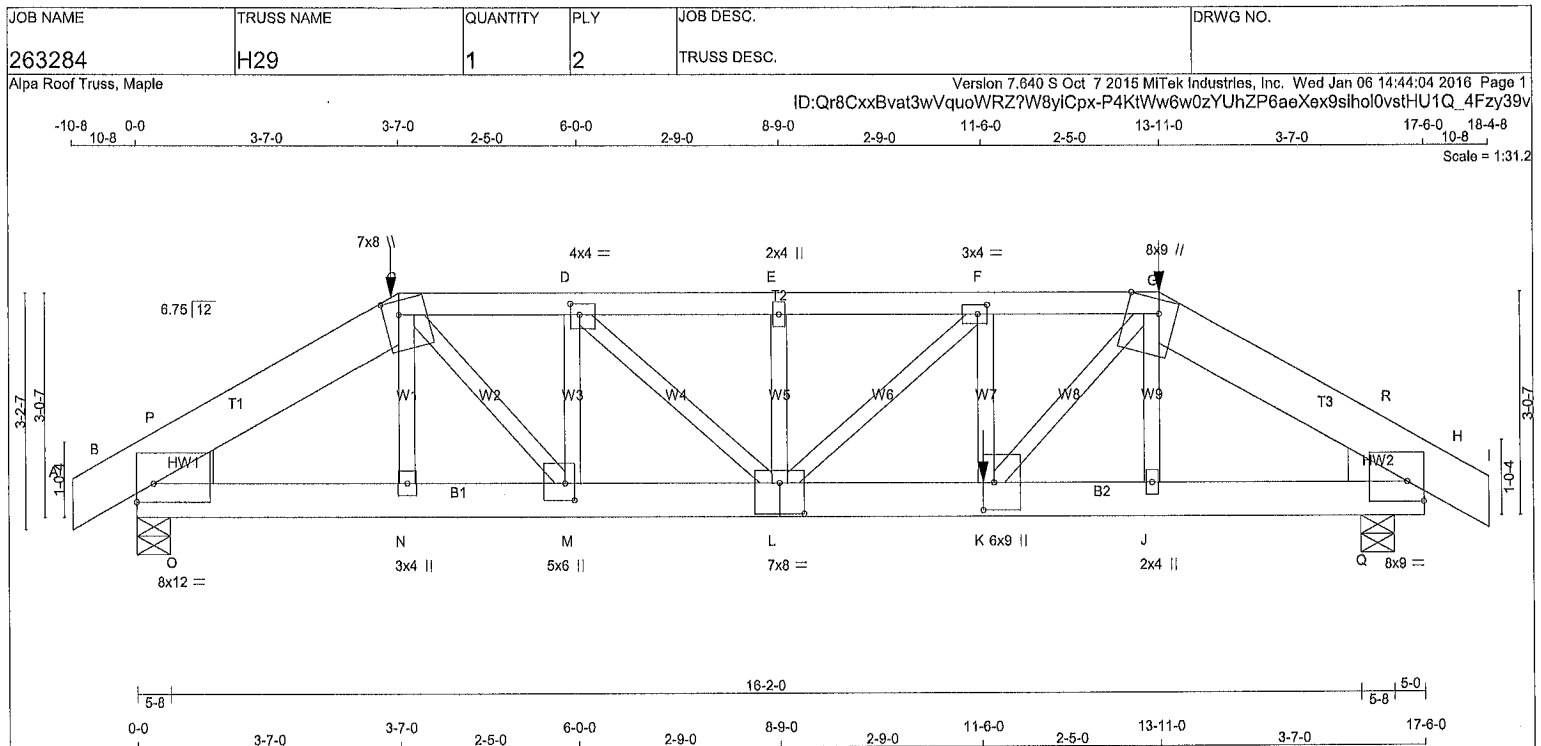
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)



A-1601171



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x8	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x8	DRY	No.2	SPF
B - L	2x6	DRY	1650F 1.5E	SPF
L - H	2x6	DRY	1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	SIDE(122.0)
G - I 2	12	SIDE(122.1)
C - G 1	12	SIDE(60.9)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - L 2	6	SIDE(411.6)
L - H 2	3	SIDE(715.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x3 1	2	SIDE(496.5)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	8.0	12.0	3.00	2.75
C	TTWW+m	MT20	7.0	8.0	2.25	2.50
D	TMWW-t	MT20	4.0	4.0	1.75	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TTWW+m	MT20	8.0	9.0	Edge	5.25
H	TMBH1-I	MT20	8.0	9.0	3.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	8897	0	8897	0	5-8	4-13	2x6 L
H	6943	0	6943	0	5-8	3-12	2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	7173	4231 / 0	1503 / 0	0 / 0	0 / 0	1439 / 0	0 / 0
H	5595	3305 / 0	1169 / 0	0 / 0	0 / 0	1121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 1.55 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 6	-94.6	-94.6	0.01 (1)	10.00	N-C	0 / 1797	0.22 (1)
B-P	-14531 / 0	-94.6	-94.6	0.52 (1)	3.15	C-M	0 / 5081	0.63 (1)
P-C	-11802 / 0	-94.6	-94.6	0.49 (1)	3.56	M-D	-1408 / 0	0.13 (1)
C-D	-13443 / 0	-97.5	-97.5	0.63 (1)	2.11	D-L	0 / 2511	0.31 (1)
D-E	-15255 / 0	-97.5	-97.5	0.85 (1)	1.59	L-E	-267 / 0	0.02 (1)
E-F	-15255 / 0	-97.5	-97.5	0.87 (1)	1.55	F-F	0 / 1371	0.17 (1)
F-G	-14273 / 0	-97.5	-97.5	0.72 (1)	1.89	K-F	-557 / 0	0.05 (1)
G-R	-10951 / 0	-94.6	-94.6	0.54 (1)	3.59	K-G	0 / 7630	0.94 (1)
R-H	-12024 / 0	-94.6	-94.6	0.55 (1)	3.41	J-G	-436 / 0	0.04 (1)
H-I	0 / 6	-94.6	-94.6	0.01 (1)	10.00	O-P	0 / 4169	0.00 (1)
						Q-R	0 / 1723	0.00 (1)
B-O	0 / 9919	-851.2	-851.2	0.72 (1)	10.00			
O-N	0 / 9919	-851.2	-851.2	0.72 (1)	10.00			
N-M	0 / 9992	-851.2	-851.2	0.51 (1)	10.00			
M-L	0 / 13431	-851.2	-851.2	0.72 (1)	10.00			
L-K	0 / 14259	-851.2	-851.2	0.80 (1)	10.00			
K-J	0 / 8095	-88.8	-53.7	0.47 (1)	10.00			
J-Q	0 / 9113	-53.7	-34.2	0.61 (1)	10.00			
Q-H	0 / 9113	-34.2	-28.9	0.61 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-7-0	-35	-35	---	FRONT	VERT	TOTAL
G	13-11-0	-35	-35	---	FRONT	VERT	TOTAL
K	11-6-4	-3767	-3767	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 49.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGlrder
START DISTANCE = 11-6-4
START SPAN CARRIED = 28-10-0
END DISTANCE = 11-6-4
END SPAN CARRIED = 28-10-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGlrder
START DISTANCE = 11-6-4
START SPAN CARRIED = 3-3-12
END DISTANCE = 17-6-0
END SPAN CARRIED = 2-0-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 3-7-0
END SETBACK = 2-1-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

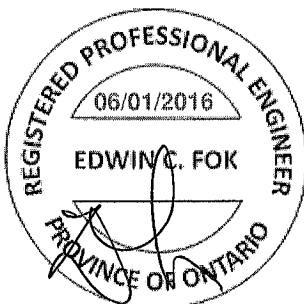
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL)= L/999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL)= L/667 (0.31")

A.1601172 CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
263284	H29	1	2	TRUSS DESC.	
Alpa Roof Truss, Maple					
Version 7.640 S Oct 7 2015 MiTek Industries, Inc. Wed Jan 08 14:44:04 2016 Page 2 ID:Qr8CxxBvat3wVquoWRZ?W8ylCpx-P4KtVWw6w0zYUhZP6aeXex9sihol0vstHU1Q_4Fzy39v					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
J	BMW+w	MT20	2.0	4.0		
K	BMW+w	MT20	6.0	9.0	4.50	1.75
L	BSWWW-l	MT20	7.0	8.0	5.00	4.00
M	BMW+w	MT20	5.0	6.0	2.75	1.50
N	BMW+w	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 35.3 lbs FACTORED DOWN AT 13-11-0, AND 35.3 lbs FACTORED DOWN AT 3-7-0 ON TOP CHORD, AND 3757.1 lbs FACTORED DOWN AT 11-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

CSI: TC=0.87 (E-F:1) , BC=0.80 (K-L:1) , WB=0.94 (G-K:1) , SSI=0.81 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

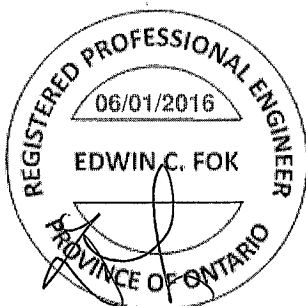
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)

JSI METAL= 0.93 (L) (INPUT = 1.00)



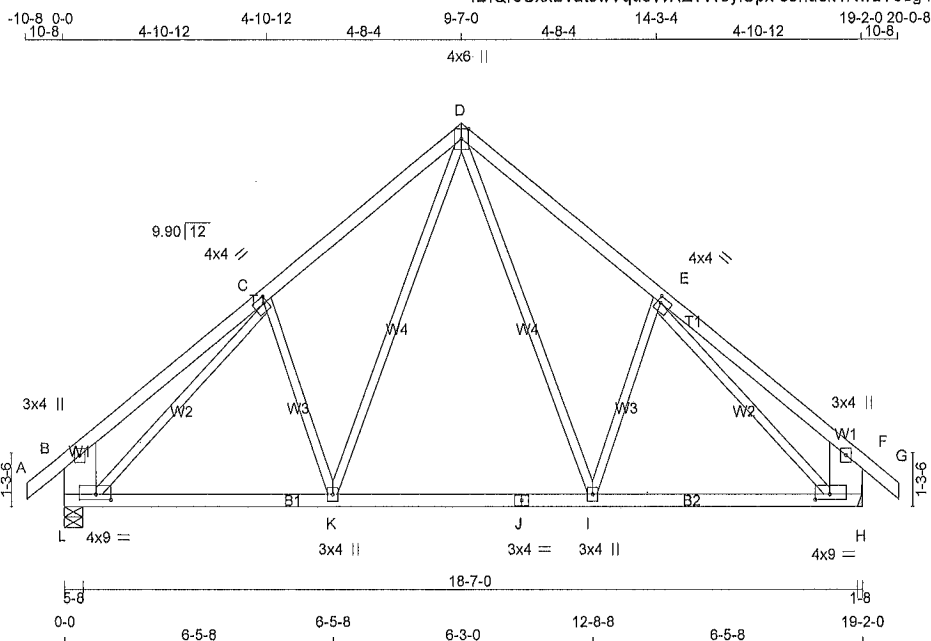
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
245428	H27	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.500 S Oct 23 2013 MiTek Industries, Inc. Fri Sep 12 11:54:14 2014 Page 1

ID:Qr8CxxBvat3wVquoVVRZ?W8yiCpx-c8nuck4RwLV8sg4YfbkKhX3BLE8j__9pSwFEYGyeOjN



Scale = 1:55.1

TOTAL WEIGHT = 3 X 94 = 281 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x10	DRY	No.2	SPF	
H - F	2x10	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	4.0	4.0	1.75	1.00
D	TTVW+p	MT20	4.0	6.0	Edge	
E	TMVW+t	MT20	4.0	4.0	1.75	1.00
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	9.0	1.50	4.25
I	BMVW+t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	4.0		
K	BMVW+t	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	9.0	1.50	4.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1266	0	5-8	1-8
H	1266	0	HANGER BY OTHERS	
			MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	WIND	DEAD
L	1011	613 / 0	201 / 0	0 / 0	197 / 0	
H	1011	613 / 0	201 / 0	0 / 0	197 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.74FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	UNBRAC	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. (PLF)	CSI (LC)	MAX	MAX	MAX	LENGTH	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO					FR-TO		
A-B	0 / 29	-94.6	-94.6	0.06 (1)	10.00	D-I	0 / 517	0.12 (1)		
B-C	0 / 34	-94.6	-94.6	0.35 (1)	10.00	I-E	-311 / 0	0.14 (1)		
C-D	-1076 / 0	-94.6	-94.6	0.29 (1)	5.74	K-D	0 / 517	0.12 (1)		
D-E	-1076 / 0	-94.6	-94.6	0.29 (1)	5.74	C-K	-311 / 0	0.14 (1)		
E-F	0 / 34	-94.6	-94.6	0.35 (1)	10.00	L-C	-1299 / 0	0.97 (1)		
F-G	0 / 29	-94.6	-94.6	0.06 (1)	10.00	E-H	-1299 / 0	0.97 (1)		
L-B	-262 / 0	0.0	0.0	0.01 (1)	7.81					
H-F	-262 / 0	0.0	0.0	0.01 (1)	7.81					
L-K	0 / 907	-28.0	-28.0	0.36 (2)	10.00					
K-J	0 / 632	-28.0	-28.0	0.33 (2)	10.00					
J-I	0 / 632	-28.0	-28.0	0.33 (2)	10.00					
I-H	0 / 907	-28.0	-28.0	0.36 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

(65 % OF 37.6 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.35 (E-F:1), BC=0.36 (K-L:2), WB=0.97 (C-L:1), SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.52 (E) (INPUT = 1.00)

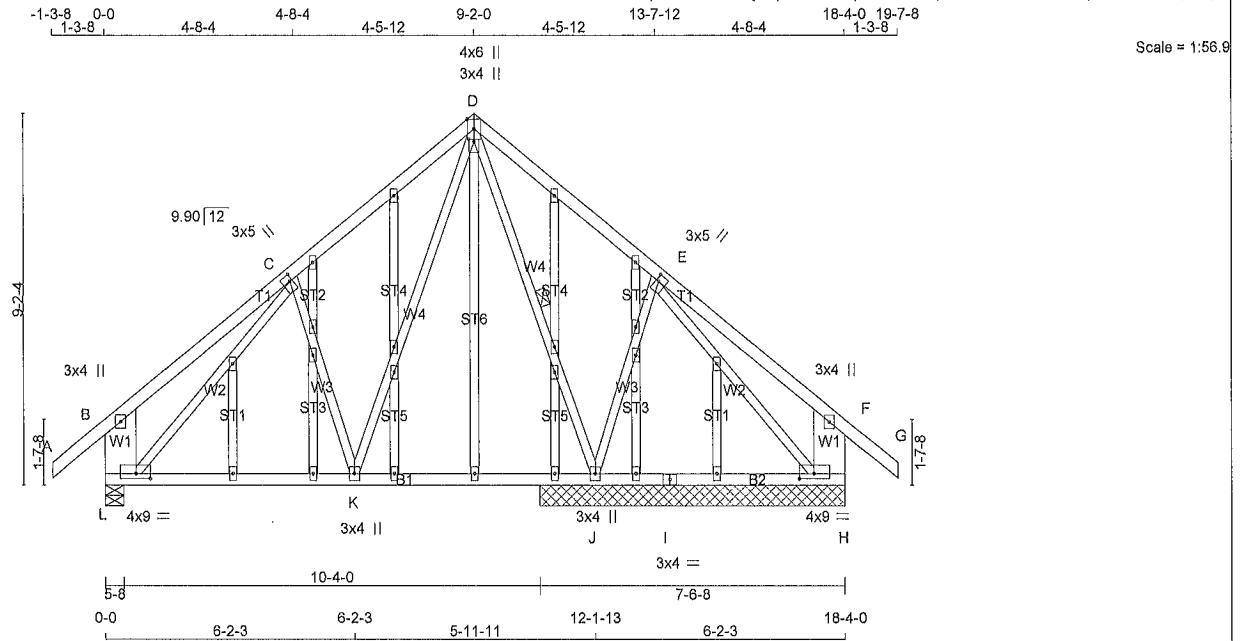


A-1609950

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
245428	H27G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 126 lb
(M)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x10	DRY	No.2
H - F	2x10	DRY	No.2
L - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW+t	MT20	3.0	5.0	2.00 0.75
D	TTWW+p	MT20	4.0	6.0	Edge
D	NP+p	MT20	3.0	4.0	0.75 1.50
E	TMWW+t	MT20	3.0	5.0	2.00 0.75
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	9.0	1.50 4.25
I	BS-t	MT20	3.0	4.0	
J	BMVW1+t	MT20	3.0	4.0	
K	BMVW+t	MT20	3.0	4.0	
L	BMVW1-t	MT20	4.0	9.0	1.50 4.25
M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z, AA, AB, AC					
M	NP+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQRD BRG
JT	VERT	HORZ	DOWN
J	1125	0	1125
L	875	0	875
H	509	0	509

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS		
	COMBINED	SNOW	LIVE	WIND DEAD
J	918	524 / 0	204 / 0	0 / 0 190 / 0
L	687	436 / 0	124 / 0	0 / 0 128 / 0
H	388	266 / 0	57 / 0	0 / 0 66 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (3)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-94.6 -94.6	0.13 (1) 10.00	D-J	-566 / 0	0.35 (1) 10.00	
B-C	0 / 33	-94.6 -94.6	0.33 (1) 10.00	J-E	-418 / 0	0.20 (1) 10.00	
C-D	-494 / 0	-94.6 -94.6	0.34 (1) 6.25	K-D	0 / 529	0.12 (1) 10.00	
D-E	-42 / 0	-94.6 -94.6	0.34 (1) 6.25	C-K	-336 / 0	0.16 (1) 10.00	
E-F	0 / 35	-94.6 -94.6	0.35 (1) 10.00	L-C	-687 / 0	0.51 (1) 10.00	
F-G	0 / 42	-94.6 -94.6	0.13 (1) 10.00	E-H	-202 / 0	0.15 (1) 10.00	
L-B	-293 / 0	0.0 0.0	0.01 (1) 7.81				
H-F	-291 / 0	0.0 0.0	0.01 (1) 7.81				
L-K	0 / 460	-28.0 -28.0	0.28 (2) 10.00				
K-J	0 / 186	-28.0 -28.0	0.27 (2) 10.00				
J-I	0 / 135	-28.0 -28.0	0.27 (2) 10.00				
I-H	0 / 135	-28.0 -28.0	0.27 (2) 10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 49.5 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.35 (E-F:1), BC=0.28 (K-L:2), WB=0.51 (C-L:1), SSI=0.16 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

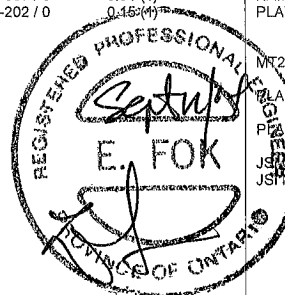
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

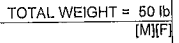
JST P=0.84 (I) (INPUT = 0.90)
JST M TOL=0.37 (E) (INPUT = 1.00)



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CONTINUED ON PAGE 2

Alpa Roof Truss, Maple



JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

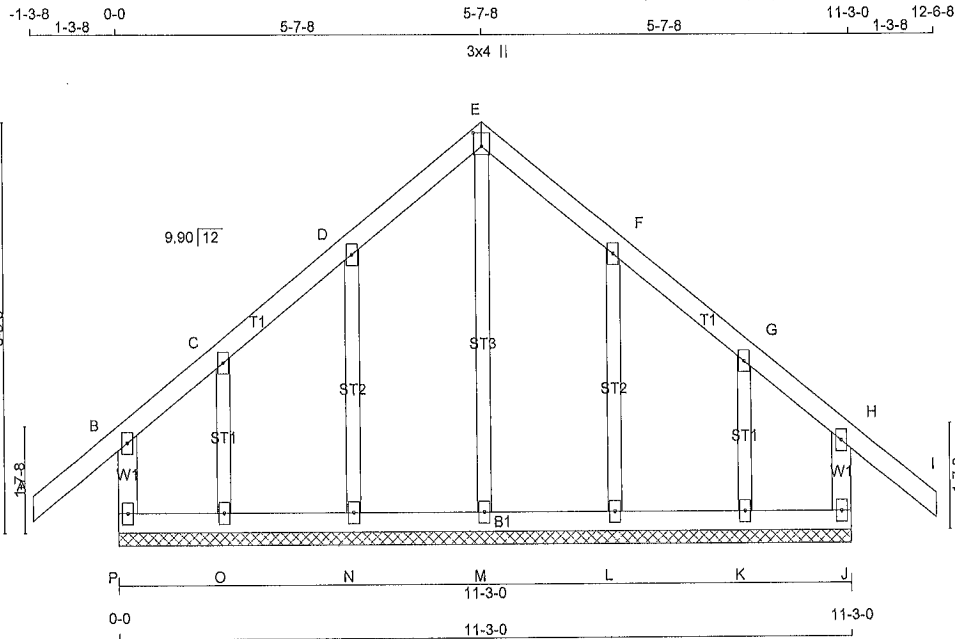
A-14995

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
245428	H28G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:Qr8CxxBvat3wVquoWRZ7W8yiCpx-YWue1Q5hSyls5zExm0momy8alSvTS5U6vEkLc9yeOjL



Scale = 1:35.2

TOTAL WEIGHT = 52 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
E - I	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - J	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
SEASONED LUMBER.					

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	3.0	4.0	2.50	1.50
H	TMV+p	MT20	2.0	4.0		
J	BMV1+p	MT20	2.0	4.0		
K, L, M, N, O						
K	BMW1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
P-B	-215 / 0	0.0	0.0 0.04 (1)	M-E	-238 / 0	0.14 (1)	
A-B	0 / 42	-94.6	-94.6 0.13 (1)	N-D	-205 / 0	0.06 (1)	
B-C	0 / 8	-94.6	-94.6 0.08 (1)	O-C	-125 / 0	0.02 (1)	
C-D	0 / 38	-94.6	-94.6 0.06 (1)	L-F	-205 / 0	0.06 (1)	
D-E	0 / 35	-94.6	-94.6 0.06 (1)	K-G	-125 / 0	0.02 (1)	
E-F	0 / 35	-94.6	-94.6 0.06 (1)				
F-G	0 / 38	-94.6	-94.6 0.06 (1)				
G-H	0 / 8	-94.6	-94.6 0.08 (1)				
H-I	0 / 42	-94.6	-94.6 0.13 (1)				
J-H	-215 / 0	0.0	0.0 0.04 (1)				
P-O	-20 / 0	-28.0	-28.0 0.02 (2)				
O-N	-24 / 0	-28.0	-28.0 0.02 (2)				
N-M	-29 / 0	-28.0	-28.0 0.02 (2)				
M-L	-29 / 0	-28.0	-28.0 0.02 (2)				
L-K	-24 / 0	-28.0	-28.0 0.02 (2)				
K-J	-20 / 0	-28.0	-28.0 0.02 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 49.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13 (H-I:1), BC=0.02 (L-M:2), WB=0.14 (E-M:1), SSI=0.08 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (E) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)



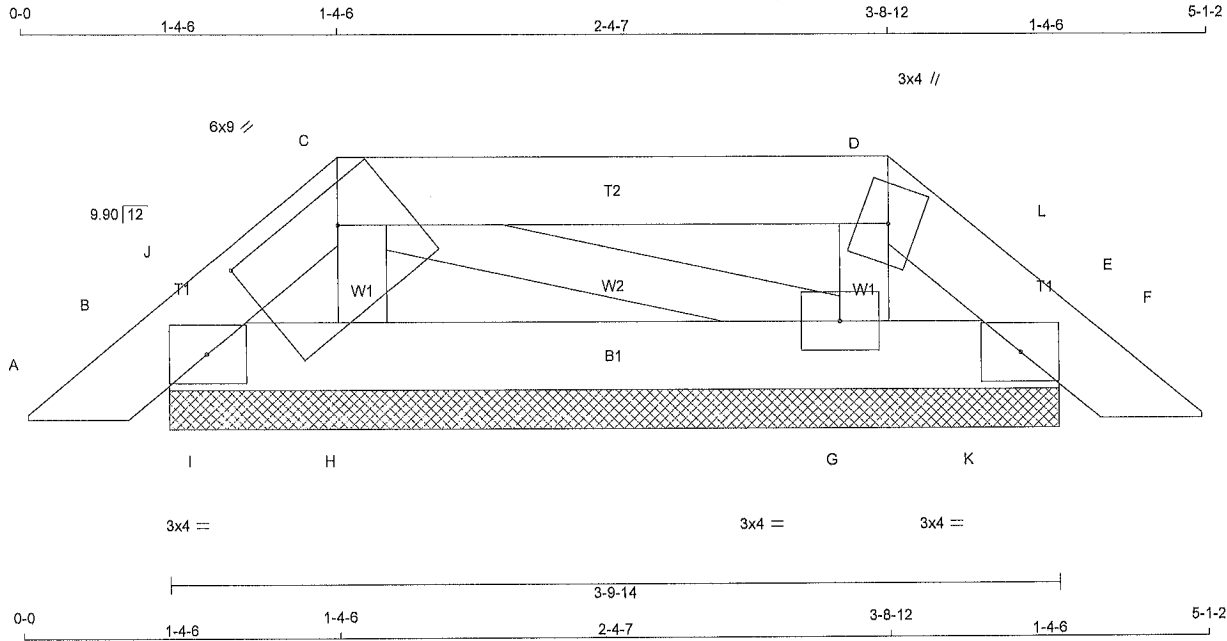
A-149953

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
245428	P26	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:Qr8CxxBvat3wVquoWRZ?W8y/Cpx-Vv0OS57x_a?alHOJuRpGrNExQGbw1_PNXDSh1yeOJJ



TOTAL WEIGHT = 3 X 13 = 40 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

S (table is in inches)					
J.	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTBWW*-t	MT20	6.0	9.0	1.75 5.75
D	TTW+m	MT20	3.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMWW1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	125	0	125	0	3-9-14	1-8
E	121	0	121	0	3-9-14	1-8
H	158	0	158	0	3-9-14	1-8
G	165	0	165	0	3-9-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND DEAD
	SNOW	LIVE	SNOW	LIVE	
B	85	77 / 0	0 / 0	0 / 0	8 / 0
E	82	74 / 0	0 / 0	0 / 0	8 / 0
H	138	64 / 0	40 / 0	0 / 0	33 / 0
G	142	68 / 0	40 / 0	0 / 0	34 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 14	-94.6 -94.6	0.02 (1)	10.00	H-C	-103 / 0	0.01 (1)
B-J	-57 / 0	-94.6 -94.6	0.02 (1)	6.25	C-G	-5 / 0	0.00 (1)
J-C	-38 / 0	-94.6 -94.6	0.01 (1)	6.25	G-D	-108 / 0	0.02 (1)
C-D	-3 / 0	-94.6 -94.6	0.09 (1)	10.00	I-J	-14 / 0	0.00 (1)
D-L	-32 / 0	-94.6 -94.6	0.01 (1)	6.25	K-L	-14 / 0	0.00 (1)
L-E	-51 / 0	-94.6 -94.6	0.02 (1)	6.25			
E-F	0 / 14	-94.6 -94.6	0.02 (1)	10.00			
B-I	0 / 23	-28.0 -28.0	0.01 (1)	10.00			
I-H	0 / 23	-28.0 -28.0	0.02 (2)	10.00			
H-G	0 / 8	-28.0 -28.0	0.02 (2)	10.00			
G-K	0 / 19	-28.0 -28.0	0.02 (2)	10.00			
K-E	0 / 19	-28.0 -28.0	0.00 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 49.5 PSF

SPACING = 24.0 IN. O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09 (C-D:1), BC=0.02 (H-I:2), WB=0.02 (D-G:1), SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

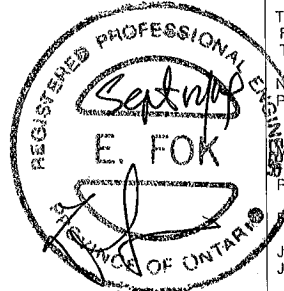
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

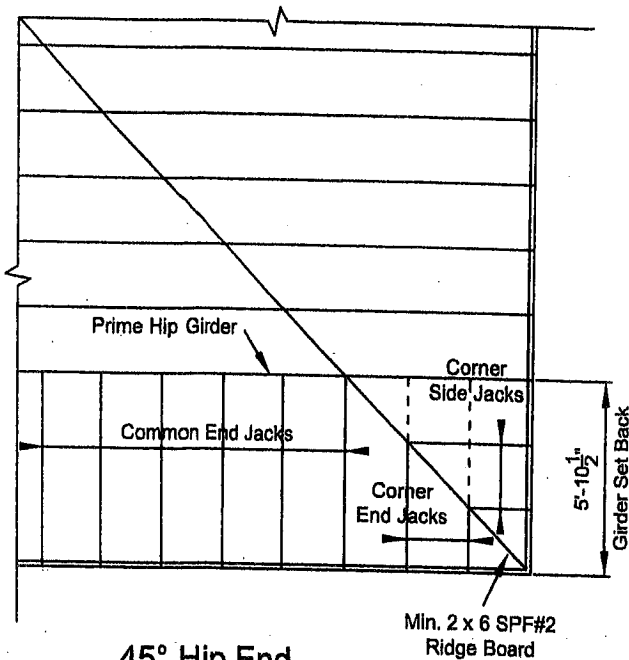
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.04 (C) (INPUT = 1.00)



A-1469955

STRACON ENGINEERING INC.



45° Hip End

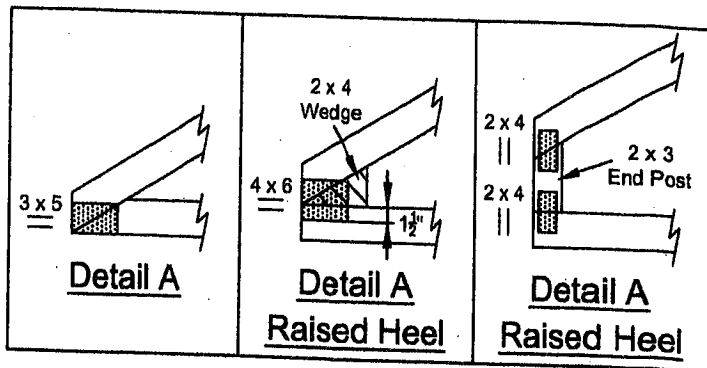
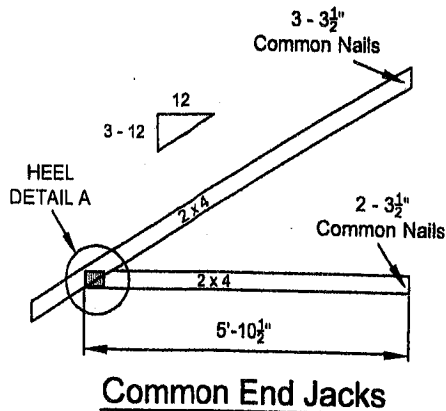
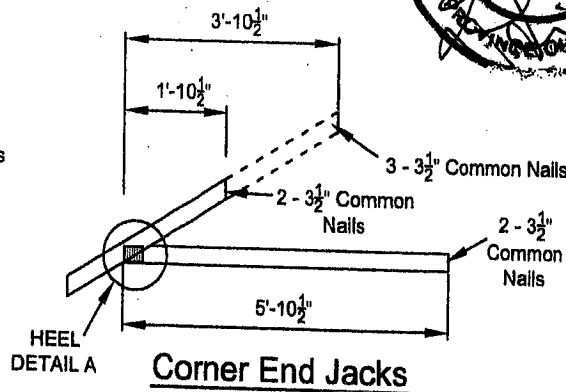
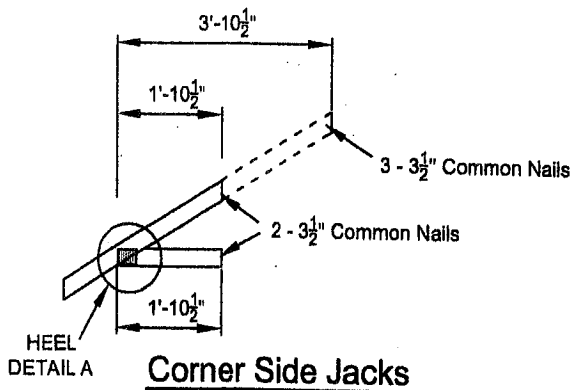
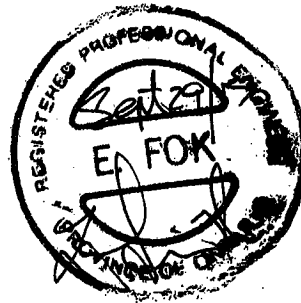
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

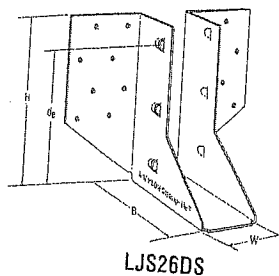
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

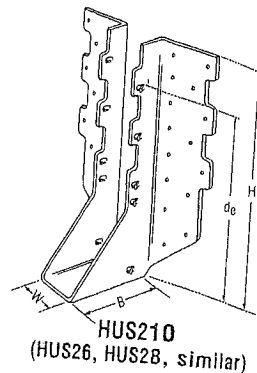
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

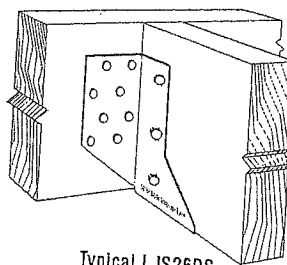
- See current catalogue for options



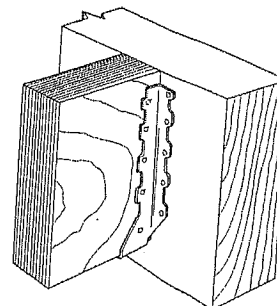
LJS26DS



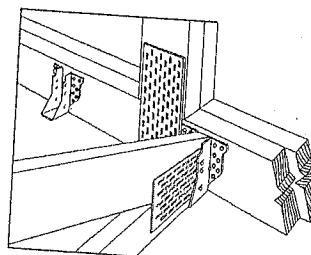
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



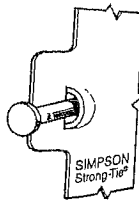
Typical HUS
Installation



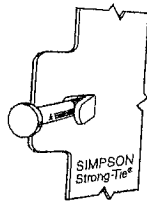
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 5/8	5 3/8	3	3 1/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 3/8	3	6 3/8	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 5/8	9 3/8	3	7 3/8	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

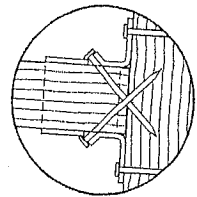
1. d_e is the distance from the seat of the hanger to the highest joist nail.



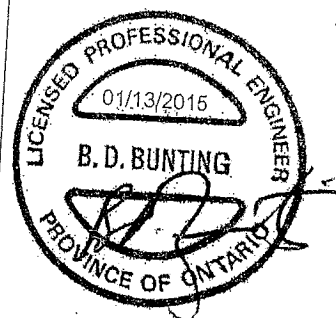
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



UNITED STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and related information available until January 1, 2017. This information is updated periodically and should be checked for updates after December 31, 2016. Contact Simpson Strong-Tie for current information and product literature at www.strongtie.com.
SIMPSON STRONG-TIE
15000 15th Ave. S.E. Everett, WA 98203
1-800-999-5099

800-999-5099
www.strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

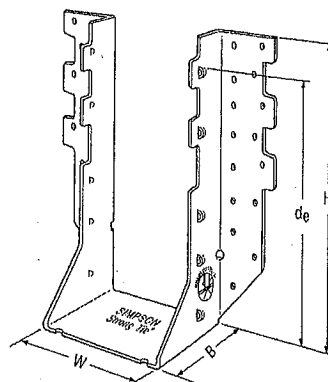
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

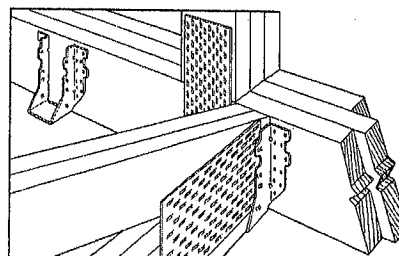
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



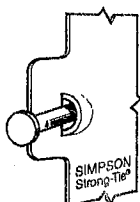
HHUS410



Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)

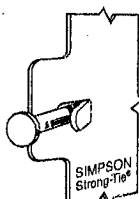
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
HHUS26-2	14	3⅞	5⅞	3	3⅞	14-16d	6-16d	2850	7335	2065	5205
HHUS28-2	14	3⅞	7⅞	3	6⅞	22-16d	8-16d	3765	8940	2675	6345
HHUS210-2	14	3⅞	9⅞	3	8	30-16d	10-16d	4745	9660	4310	7000
HHUS210-3	14	4⅞	9	3	7⅞	30-16d	10-16d	4745	10545	4310	7485
HHUS210-4	14	6⅞	8⅞	3	7⅞	30-16d	10-16d	4745	10545	4310	7485
HHUS46	14	3⅞	5⅞	3	3⅞	14-16d	6-16d	2540	7335	2065	5205
HHUS48	14	3⅞	7⅞	3	6⅞	22-16d	8-16d	3765	8945	2675	6345
HHUS410	14	3⅞	9	3	8	30-16d	10-16d	4745	9855	4310	7000
HHUS5.50/10	14	5⅞	9	3	8	30-16d	10-16d	4745	10545	4310	7485
HHUS7.25/10	14	7⅞	9	3⅞	7⅞	30-16d	10-16d	4745	10770	4310	7650

1. d_g is the distance from the seat of the hanger to the highest joist nail.

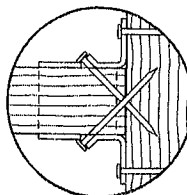


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

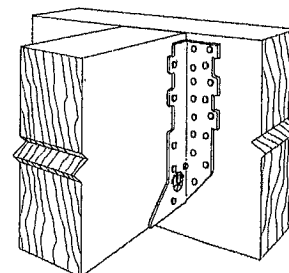
U.S. Patent
5,603,580



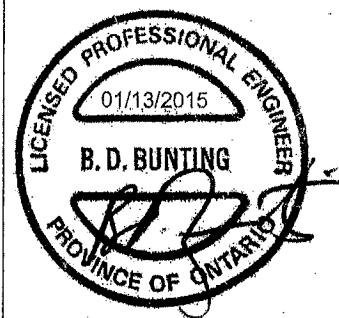
Double
Shear
Nailing
Side View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



Typical HHUS
Installation



800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given.
The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

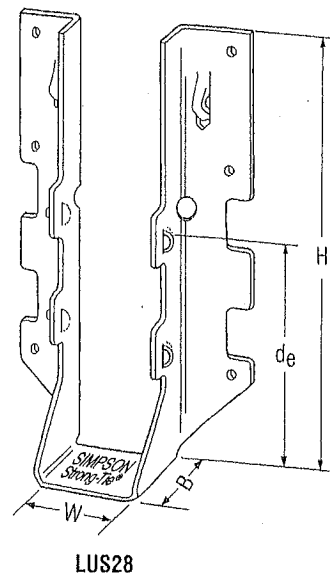
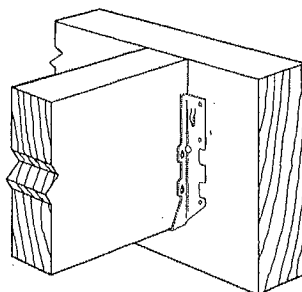
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

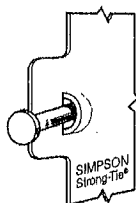
- These hangers cannot be modified.

Typical LUS
Installation

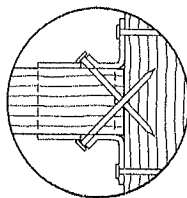


Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1⅞	3⅞	1¼	1⅝	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅝	4-16d	2-16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¼	3⅞	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¼	3¼	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅝	1¼	3⅞	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8⅞	2	5¼	8-16d	6-16d	2580	3345	2320	2375

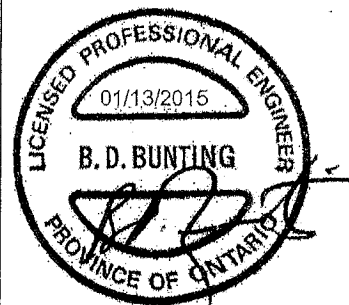
1. d_g is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
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(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



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