

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012. ROOF PARTS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF #2 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

10-00
CONFIRM

2-06-00
1-00-00
1-00-00

21-06-00

H01G
H10
H12A

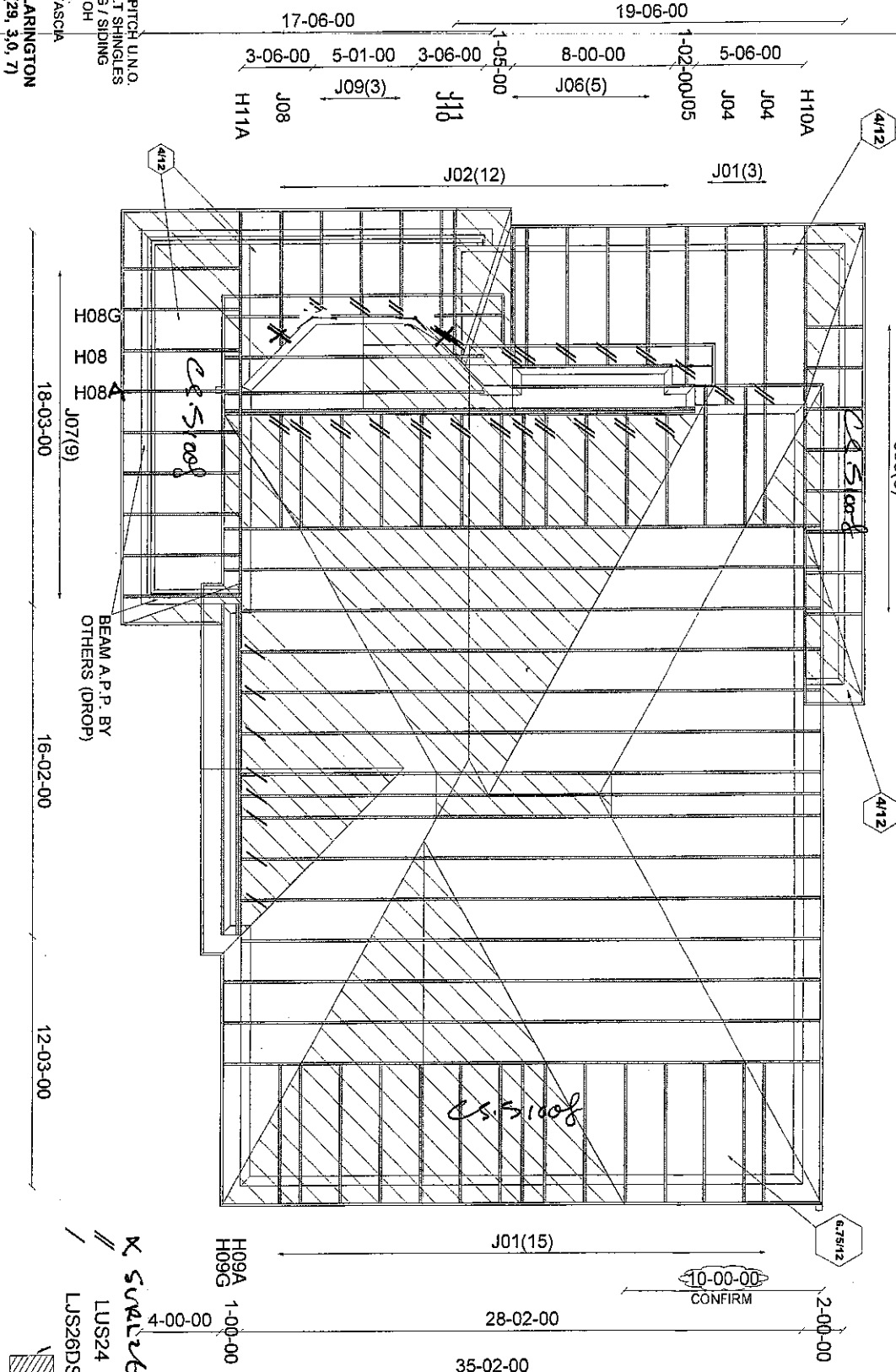
J03(8)

H01
H02
H03
H04
H05
H06
H07(3)
H06
H05
H04
H03
H02
H01

38-00-00

24-04-00

OFFICE USE ONLY
ENGINEERED FROM
A-1602796 TO
A-1602219



11.9/12 PITCH U.N.O.
ASPHALT SHINGLES
2x6 BRG / SIDING
12" FIN OH
R.T.M.C.
W/2x6 FASCIA

CLARINGTON
(29, 3.0, 7)



Job Track: 31379
Layout ID: 264021
Plan Log: 84653

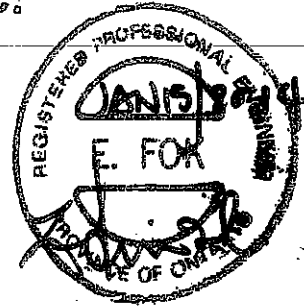
Builder / Location:
ESQUIRE HOMES / CLARINGTON
Project: **NORTHGLEN**
Date: 1/29/2016 Designer: JESSIE

Model / Elevation:
37-4 A / CORNER UPGRADE
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mike Ver 7.5.0

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



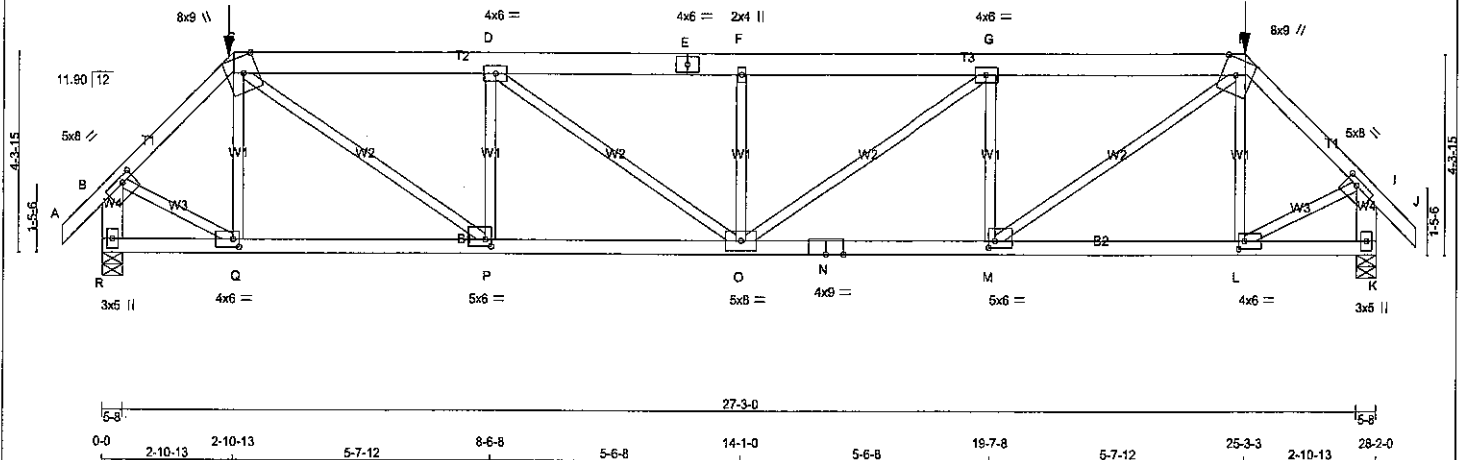
RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014



TOTAL WEIGHT = 2 X 129 = 258 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x6	DRY	No.2 SPF
E - H	2x6	DRY	No.2 SPF
H - J	2x4	DRY	No.2 SPF
R - B	2x6	DRY	No.2 SPF
K - I	2x6	DRY	No.2 SPF
R - N	2x4	DRY	No.2 SPF
N - K	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.50	3.00
C	TTVW+m	MT20	8.0	9.0	4.25	3.75
D	TMVW-t	MT20	4.0	6.0		
E	TS-t	MT20	4.0	6.0		
F	TMW-w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	TTVW+m	MT20	8.0	9.0	4.25	3.75
I	TMVW-t	MT20	5.0	8.0	1.50	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	6.0	2.00	1.50
M	BMVW-t	MT20	5.0	6.0	1.75	1.50
N	BS-t	MT20	4.0	9.0		
O	BMVWVW-t	MT20	5.0	8.0		
P	BMVW-t	MT20	5.0	6.0	1.75	1.50
Q	BMVW-t	MT20	4.0	6.0	2.00	1.50
R	BMV1+p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
R	3135	0	3135	0	5-8	4-11
K	3135	0	3135	0	5-8	4-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED SNOW		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
R	2183	1626 / 0	0 / 0	0 / 0	0 / 0	556 / 0	0 / 0
K	2183	1626 / 0	0 / 0	0 / 0	0 / 0	556 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS		MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)		MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO			FROM	TO	FR-TO		
A-B	0 / 33	-94.6	-94.6	0.07 (1)	10.00	Q-C	-601 / 0
B-C	-2991 / 0	-94.6	-94.6	0.29 (1)	3.76	C-P	0 / 3376
C-D	-4876 / 0	-186.2	-186.2	0.57 (1)	3.38	P-D	-1753 / 0
D-E	-5688 / 0	-186.2	-186.2	0.63 (1)	3.08	D-O	0 / 998
E-F	-5688 / 0	-186.2	-186.2	0.63 (1)	3.08	O-F	-967 / 0
F-G	-5688 / 0	-186.2	-186.2	0.63 (1)	3.08	G-O	0 / 998
G-H	-4876 / 0	-186.2	-186.2	0.57 (1)	3.38	M-G	-1753 / 0
H-I	-2991 / 0	-94.6	-94.6	0.29 (1)	3.76	M-H	0 / 3376
I-J	0 / 33	-94.6	-94.6	0.07 (1)	10.00	L-H	-601 / 0
R-B	-3110 / 0	0.0	0.0	0.23 (1)	5.94	B-Q	0 / 2266
K-I	-3110 / 0	0.0	0.0	0.23 (1)	5.94	L-I	0 / 2266
R-Q	0 / 0	-34.5	-34.5	0.18 (4)	10.00		
Q-P	0 / 2113	-34.5	-34.5	0.50 (1)	10.00		
P-O	0 / 4876	-34.5	-34.5	0.97 (1)	10.00		
O-N	0 / 4876	-34.5	-34.5	0.97 (1)	10.00		
N-M	0 / 4876	-34.5	-34.5	0.97 (1)	10.00		
M-L	0 / 2113	-34.5	-34.5	0.50 (1)	10.00		
L-K	0 / 0	-34.5	-34.5	0.18 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-10-13	-200	-200	---	FRONT	VERT	TOTAL
H	25-3-3	-200	-200	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp

SIDE SETBACK = 2-10-13

END SETBACK = 5-10-8

END WALL WIDTH = 0-0

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL. LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")

CALCULATED VERT. DEFL.(LL)= L/ 889 (0.25")

ALLOWABLE DEFL.(TL)= L/360 (0.94")

CALCULATED VERT. DEFL.(TL)= L/ 819 (0.41")

CSI: TC=0.63 (F-G:1), BC=0.97 (M-O:1), WB=0.84 (C-P:1), SSI=0.41 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

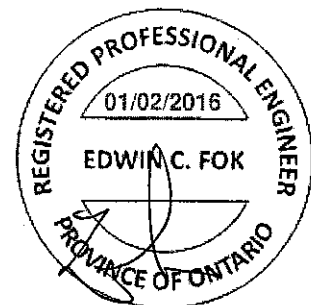
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)

JSI METAL= 0.97 (N) (INPUT = 1.00)

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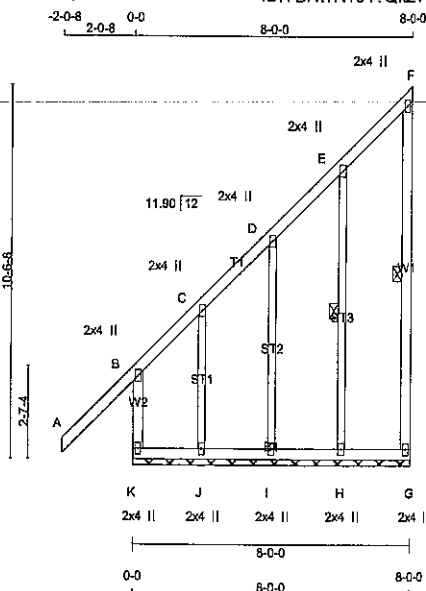
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H01G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.640 S Oct 7 2015 MiTek Industries, Inc. Mon Feb 01 13:51:45 2016 Page 1
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Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 54 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
K - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, E						
C	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	2.0	4.0		
G	BMV1+p	MT20	2.0	4.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		
K	BMV1+p	MT20	2.0	4.0		

NOTES-

- (1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE	VERT. LOAD	LC1	MAX	MAX. UNBRACED	MEMB.	MAX. FORCE	MAX
FR-TO	(LBS)	(PLF)	CS1 (LC)	CS1 (LC)	LENGTH	FR-TO	(LBS)	CS1 (LC)
K-B	-401 / 0	0.0	0.0	0.07 (1)	7.81	H-E	-205 / 0	0.10 (1)
A-B	0 / 71	-94.6	-94.6	0.31 (1)	10.00	I-D	-210 / 0	0.15 (1)
B-C	-82 / 0	-94.6	-94.6	0.25 (1)	6.25	J-C	-60 / 0	0.02 (1)
C-D	0 / 7	-94.6	-94.6	0.05 (1)	10.00			
D-E	-5 / 0	-94.6	-94.6	0.05 (1)	10.00			
E-F	-11 / 0	-94.6	-94.6	0.05 (1)	6.25			
G-F	-84 / 0	0.0	0.0	0.01 (1)	6.25			
K-J	0 / 8	-17.5	-17.5	0.07 (1)	10.00			
J-I	0 / 6	-17.5	-17.5	0.01 (4)	10.00			
I-H	0 / 3	-17.5	-17.5	0.02 (4)	10.00			
H-G	0 / 0	-17.5	-17.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31 (A-B:1), BC=0.07 (J-K:1), WB=0.15 (D-I:1), SSI=0.11 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

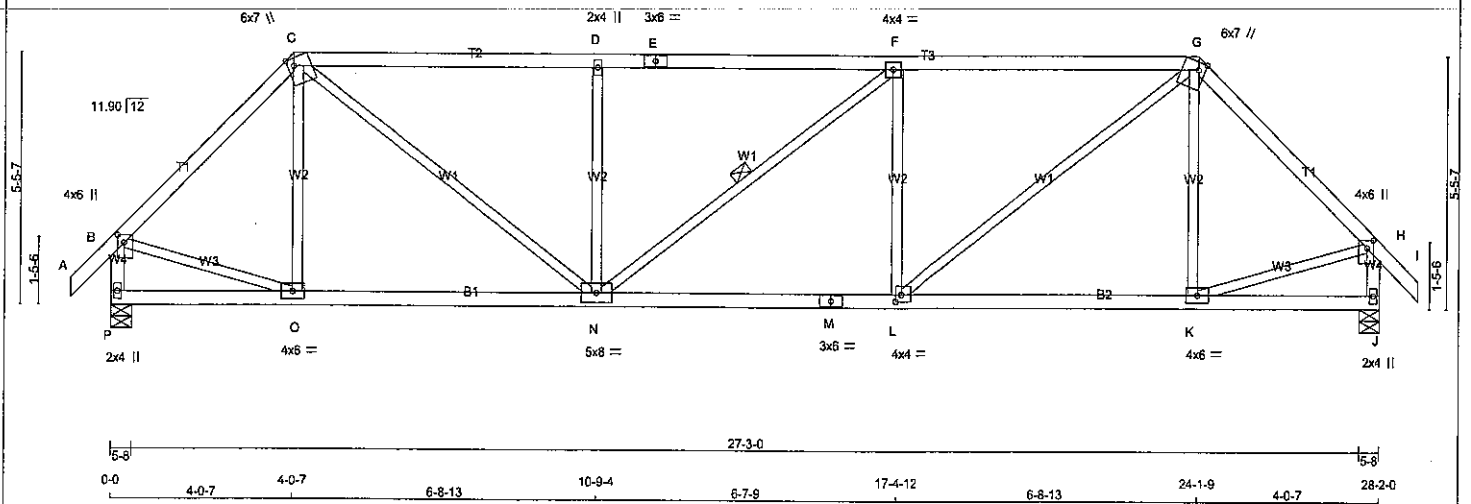
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



A.1602197



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - K	2x4	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
L - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.00 1.75
C	TTWW+m	MT20	6.0	7.0	Edge 1.75
D	TMVW-w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMVW-t	MT20	4.0	4.0	
G	TTWW+m	MT20	6.0	7.0	Edge 1.75
H	TMVW+p	MT20	4.0	6.0	2.00 1.75
J	BMV1+p	MT20	2.0	4.0	
K	BMVW-t	MT20	4.0	6.0	
L	BMVW-t	MT20	4.0	4.0	1.75 1.50
M	BS-t	MT20	3.0	6.0	
N	BMVW-t	MT20	5.0	8.0	
O	BMVW-t	MT20	4.0	6.0	
P	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	REQD	
P	1671	0	1671	0	0	5-8	1-15		
J	1671	0	1671	0	0	5-8	1-15		

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1162	874 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0
J	1162	874 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 33	-94.6 -94.6	0.07 (1)	10.00	O-C	-179 / 44	0.08 (1)
B-C	-1526 / 0	-94.6 -94.6	0.33 (1)	4.97	C-N	0 / 1319	0.30 (1)
C-D	-2124 / 0	-94.6 -94.6	0.78 (1)	3.77	N-D	-683 / 0	0.30 (1)
D-E	-2125 / 0	-94.6 -94.6	0.78 (1)	3.75	N-F	-2 / 0	0.00 (1)
E-F	-2125 / 0	-94.6 -94.6	0.78 (1)	3.75	L-F	-683 / 0	0.30 (1)
F-G	-2126 / 0	-94.6 -94.6	0.78 (1)	3.75	L-G	0 / 1320	0.30 (1)
G-H	-1525 / 0	-94.6 -94.6	0.33 (1)	4.97	K-G	-179 / 44	0.08 (1)
H-I	0 / 33	-94.6 -94.6	0.07 (1)	10.00	B-O	0 / 1117	0.25 (1)
P-B	-1648 / 0	0.0 0.0	0.17 (1)	6.45	K-H	0 / 1117	0.25 (1)
J-H	-1648 / 0	0.0 0.0	0.17 (1)	6.45			
O-O	0 / 0	-17.5 -17.5	0.12 (4)	10.00			
O-N	0 / 1076	-17.5 -17.5	0.25 (1)	10.00			
N-M	0 / 2126	-17.5 -17.5	0.42 (1)	10.00			
M-L	0 / 2126	-17.5 -17.5	0.42 (1)	10.00			
L-K	0 / 1076	-17.5 -17.5	0.25 (1)	10.00			
K-J	0 / 0	-17.5 -17.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.94")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.10")
ALLOWABLE DEFL. (TL) = $L/360$ (0.94")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.18")

CSI: TC=0.76 (F-G:1), BC=0.42 (L-N:1), WB=0.30 (F-L:1), SSI=0.30 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

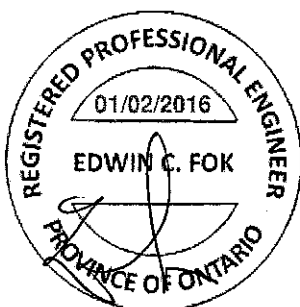
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.55 (M) (INPUT = 1.00)

A-1602128

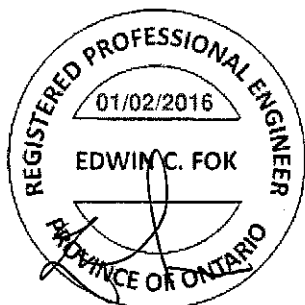


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H02	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

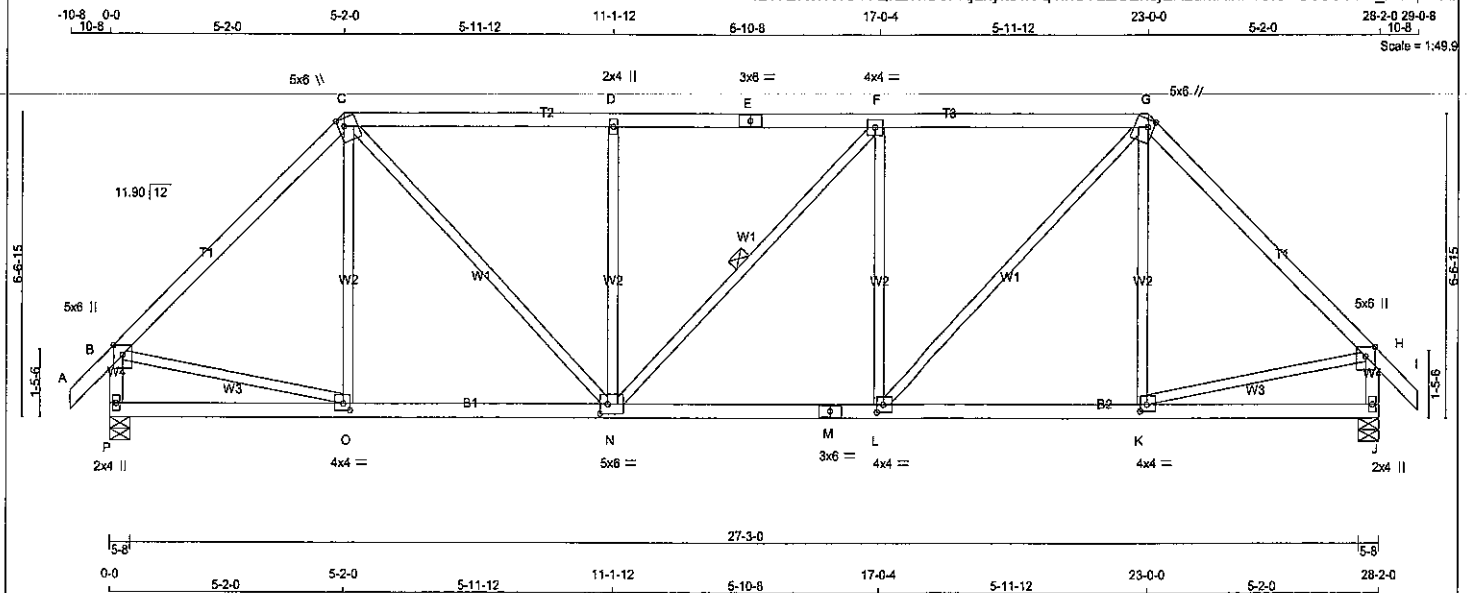


A-162198(1)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.640 S Oct 7 2015 MITek Industries, Inc. Mon Feb 01 13:52:08 2016 Page 1
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TOTAL WEIGHT = 2 X 123 = 245 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMWW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.75	1.75
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	4.0	4.0	1.75	1.75
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1671	0	1671	0	5-8	1-15
J	1671	0	1671	0	5-8	1-15

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
P	1162	874 / 0	0 / 0	0 / 0	0 / 0	288 / 0
J	1162	874 / 0	0 / 0	0 / 0	0 / 0	288 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 33	-94.6	-94.6	0.07 (1)	10.00	O-C	-114 / 65
B-C	-1521 / 0	-94.6	-94.6	0.56 (1)	4.68	C-N	0 / 976
C-D	-1749 / 0	-94.6	-94.6	0.55 (1)	4.39	N-D	-605 / 0
D-E	-1749 / 0	-94.6	-94.6	0.55 (1)	4.38	N-F	-2 / 0
E-F	-1749 / 0	-94.6	-94.6	0.55 (1)	4.38	L-F	-605 / 0
F-G	-1750 / 0	-94.6	-94.6	0.56 (1)	4.38	L-G	0 / 978
G-H	-1521 / 0	-94.6	-94.6	0.56 (1)	4.68	K-G	-115 / 65
H-I	0 / 33	-94.6	-94.6	0.07 (1)	10.00	B-O	0 / 1100
P-B	-1635 / 0	0.0	0.0	0.17 (1)	6.47	K-H	0 / 1100
J-H	-1635 / 0	0.0	0.0	0.17 (1)	6.48		
P-O	0 / 0	-17.5	-17.5	0.13 (4)	10.00		
O-N	0 / 1075	-17.5	-17.5	0.25 (1)	10.00		
N-M	0 / 1751	-17.5	-17.5	0.34 (1)	10.00		
M-L	0 / 1751	-17.5	-17.5	0.34 (1)	10.00		
L-K	0 / 1075	-17.5	-17.5	0.25 (1)	10.00		
K-J	0 / 0	-17.5	-17.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL)= L/989 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL)= L/989 (0.13")

CSI: TC=0.56 (B-C:1), BC=0.34 (L-N:1),
WB=0.43 (F-L:1), SSI=0.26 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1656

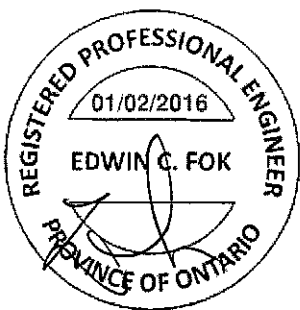
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.44 (M) (INPUT = 1.00)

A-1602199

CONTINUED ON PAGE 2



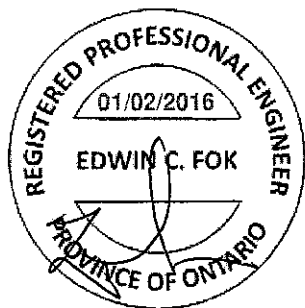
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

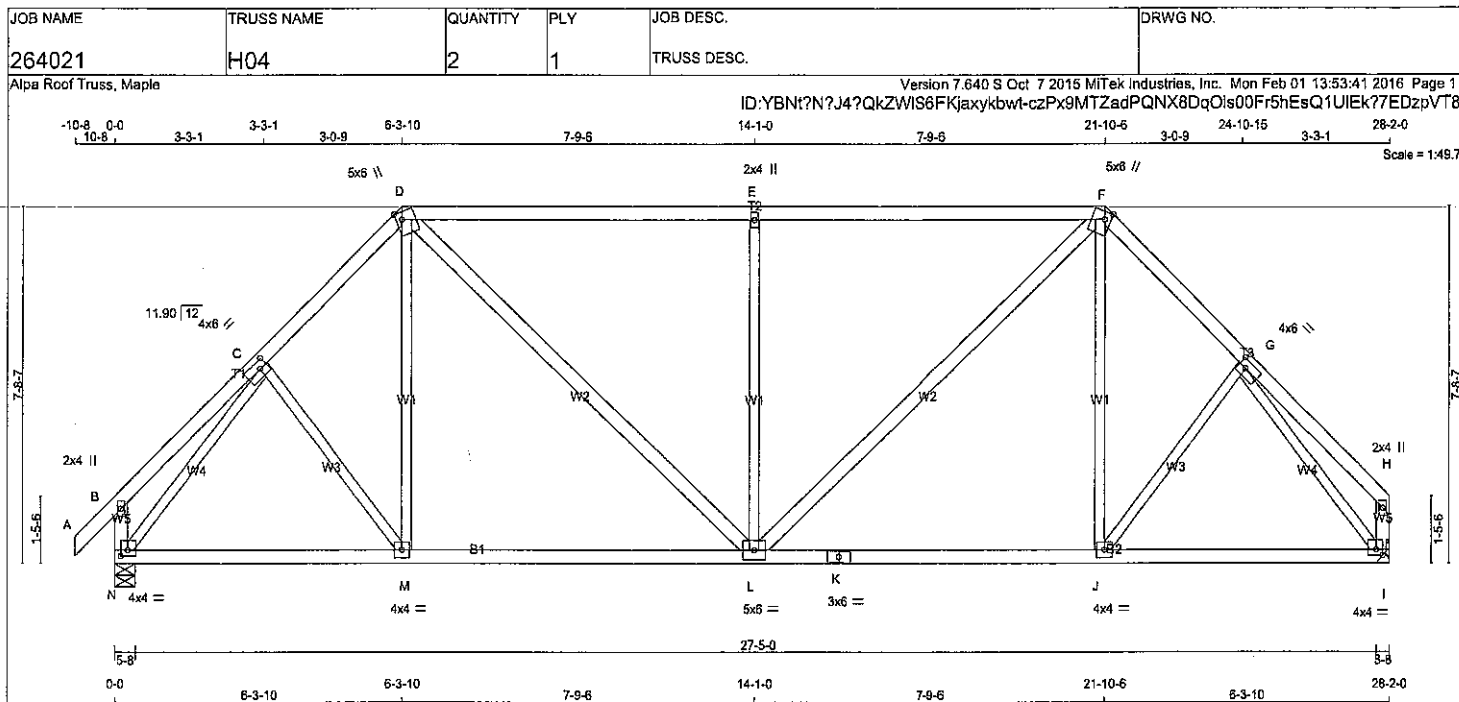
Version 7.640 S Oct 7 2015 MiTek Industries, Inc. Mon Feb 01 13:52:08 2016 Page 2
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NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



A-1602199(2)



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - L	2x4	DRY	No.2	SPF
L - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	2.00
D	TTWV+m	MT20	5.0	6.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTWV+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	6.0	2.00	2.00
H	TMV+p	MT20	2.0	4.0		
I	BMVW-t	MT20	4.0	4.0	1.50	1.75
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVWV-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0	1.50	1.75

NOTES-

(1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
N	1671	0	1671	0
I	1579	0	1579	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1162	874 / 0	0 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0
I	1099	818 / 0	0 / 0	0 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 33	-94.6	-94.6	0.07 (1)	C-M	0 / 40	0.01 (4)
B-C	0 / 22	-94.6	-94.6	0.14 (1)	M-D	0 / 135	0.04 (4)
C-D	-1517 / 0	-94.6	-94.6	0.17 (1)	D-L	0 / 754	0.12 (1)
D-E	-1606 / 0	-94.6	-94.6	0.97 (1)	L-E	-904 / 0	0.99 (1)
E-F	-1606 / 0	-94.6	-94.6	0.97 (1)	L-F	0 / 754	0.12 (1)
F-G	-1517 / 0	-94.6	-94.6	0.17 (1)	J-F	0 / 135	0.04 (4)
G-H	0 / 22	-94.6	-94.6	0.14 (1)	J-G	0 / 40	0.01 (4)
H-B	-208 / 0	0.0	0.0	0.02 (1)	N-C	-1769 / 0	0.78 (1)
I-H	-116 / 0	0.0	0.0	0.01 (1)	G-I	-1769 / 0	0.78 (1)
N-M	0 / 1055	-17.5	-17.5	0.28 (1)			
M-L	0 / 1059	-17.5	-17.5	0.28 (1)			
L-K	0 / 1059	-17.5	-17.5	0.28 (1)			
K-J	0 / 1059	-17.5	-17.5	0.28 (1)			
J-I	0 / 1055	-17.5	-17.5	0.28 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.94")
CALCULATED VERT. DEFL.(LL) = $L/998$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.94")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.97 (E-F:1), BC=0.28 (J-L:1),
WB=0.99 (E-L:1), SS=0.36 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

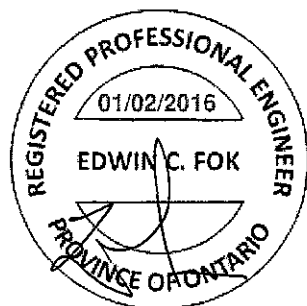
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.80)
JSI METAL = 0.44 (G) (INPUT = 1.00)



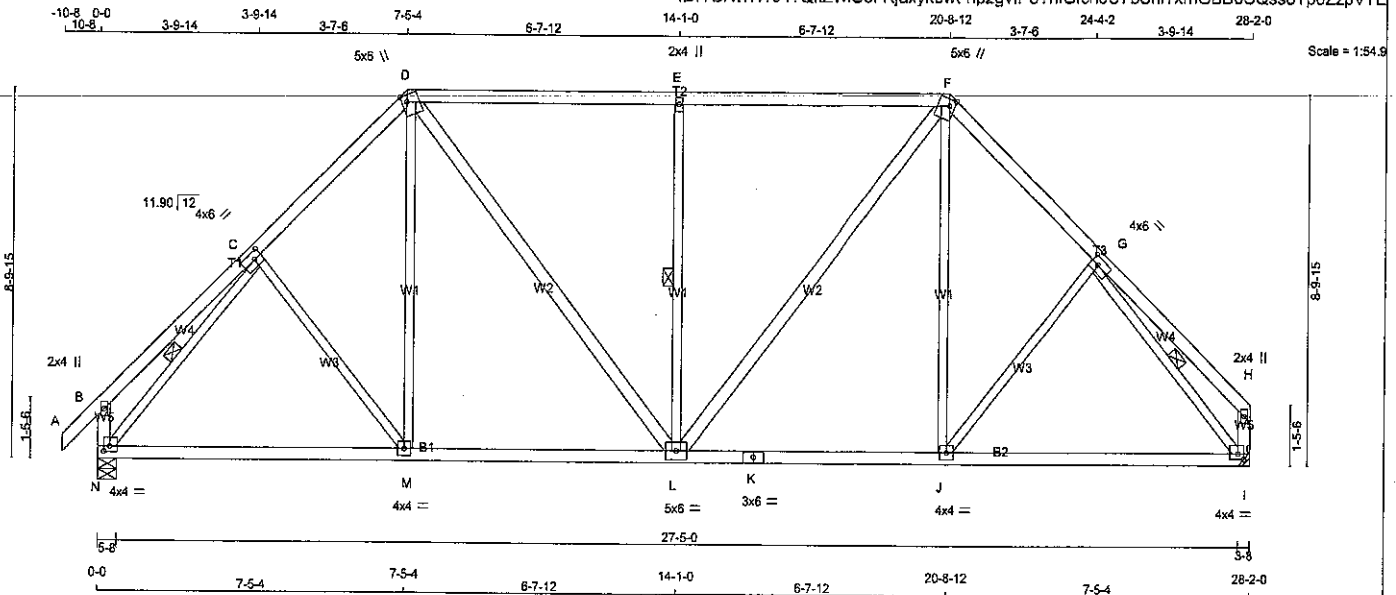
A-1602200

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H05	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 2 X 138 = 276 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - H	2x4 DRY	No.2	SPF		
N - B	2x4 DRY	No.2	SPF		
I - H	2x4 DRY	No.2	SPF		
N - K	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR
D - L	2x4 DRY	No.2	SPF	
L - F	2x4 DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
N	1671	0	0	5-8
I	1579	0	0	1-13

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
N	1162	874 / 0	0 / 0
I	1099	818 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, C-N, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
A-B	0 / 33	-94.6	-94.6	0.07 (1)	10.00	C-M	-83 / 26	0.05 (1)
B-C	0 / 28	-94.6	-94.6	0.20 (1)	10.00	M-D	0 / 208	0.05 (4)
C-D	-1477 / 0	-94.6	-94.6	0.18 (1)	5.22	D-L	0 / 551	0.09 (1)
D-E	-1367 / 0	-94.6	-94.6	0.57 (1)	4.78	L-E	-770 / 0	0.39 (1)
E-F	-1367 / 0	-94.6	-94.6	0.57 (1)	4.78	F-F	0 / 551	0.09 (1)
F-G	-1477 / 0	-94.6	-94.6	0.18 (1)	5.22	J-F	0 / 208	0.05 (4)
G-H	0 / 28	-94.6	-94.6	0.20 (1)	10.00	J-G	-83 / 26	0.05 (1)
N-B	-227 / 0	0.0	0.0	0.02 (1)	7.81	N-C	-1759 / 0	0.45 (1)
I-H	-135 / 0	0.0	0.0	0.01 (1)	7.81	G-I	-1759 / 0	0.45 (1)
N-M	0 / 1078	-17.5	-17.5	0.30 (1)	10.00			
M-L	0 / 1028	-17.5	-17.5	0.30 (4)	10.00			
L-K	0 / 1028	-17.5	-17.5	0.30 (4)	10.00			
K-J	0 / 1028	-17.5	-17.5	0.30 (4)	10.00			
J-I	0 / 1078	-17.5	-17.5	0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.57 (D-E:1), BC=0.30 (M-N:1),
WB=0.45 (C-N:1), SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.44 (G) (INPUT = 1.00)

PLATES (table is in inches)

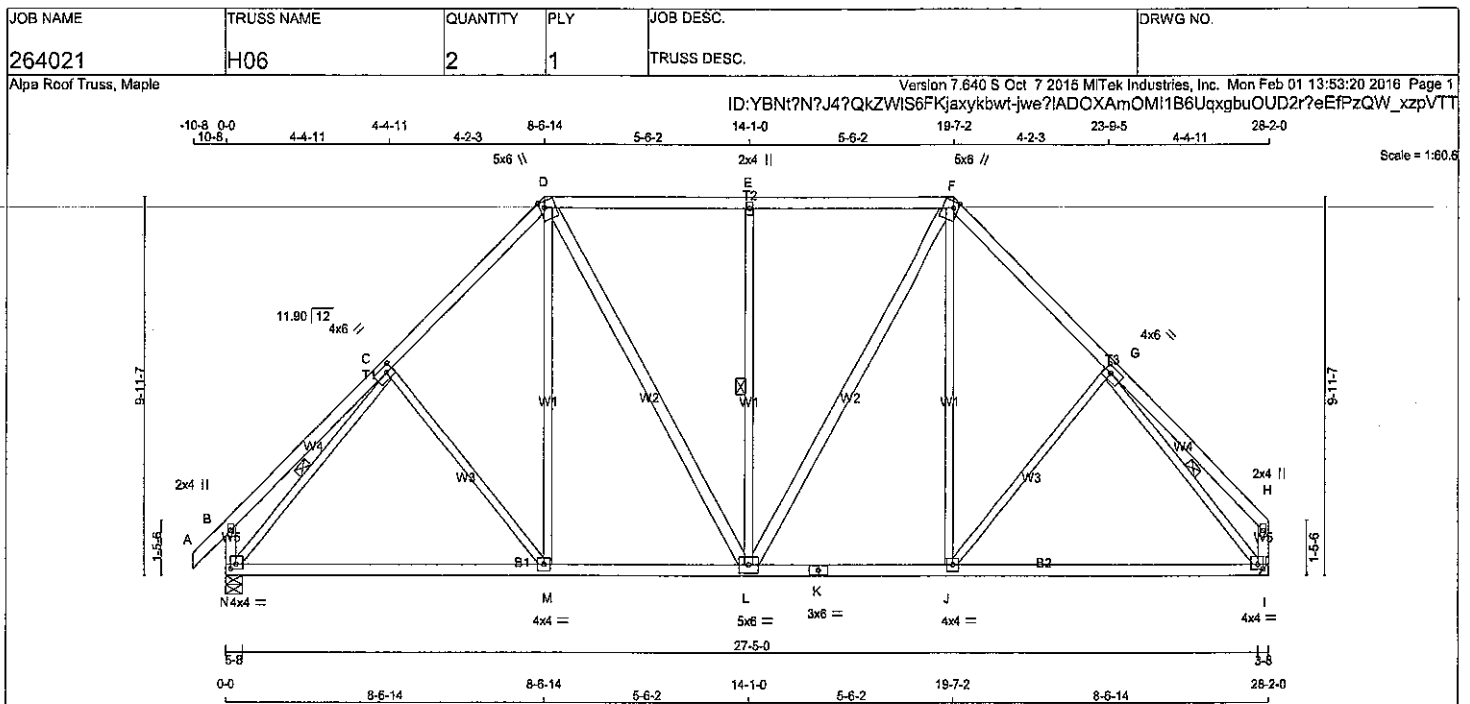
JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	2.25
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	6.0	2.00	2.25
H	TMV+p	MT20	2.0	4.0		
I	BMVW-t	MT20	4.0	4.0	1.50	1.75
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0	1.50	1.75

NOTES-

(1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per



A.160220 |



TOTAL WEIGHT = 2 X 145 = 290 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS				
EXCEPT	SIZE	LUMBER	DESCR.	
D - L	2x4	DRY	No.2	SPF
L - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	2.25
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	6.0	2.00	2.25
H	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	1.50	1.75
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	4.0	1.50	1.75

NOTES:

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
N	1671	0	1671	0	5-8	1-13
I	1579	0	1579	0	5-8	1-13

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
N	1162	874 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0
I	1099	818 / 0	0 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, C-N, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX LC1 MAX CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	MAX CS1 (LC)
FR-TO				FR-TO			
A-B	0 / 33	-94.6	-94.6 0.07 (1)	10.00	C-M	-160 / 14	0.14 (1)
B-C	0 / 33	-94.6	-94.6 0.28 (1)	10.00	M-D	0 / 281	0.06 (1)
C-D	-1428 / 0	-94.6	-94.6 0.25 (1)	5.22	D-L	0 / 395	0.06 (1)
D-E	-1187 / 0	-94.6	-94.6 0.39 (1)	5.38	L-E	-634 / 0	0.43 (1)
E-F	-1187 / 0	-94.6	-94.6 0.39 (1)	5.38	L-F	0 / 395	0.06 (1)
F-G	-1428 / 0	-94.6	-94.6 0.25 (1)	5.22	J-F	0 / 281	0.06 (1)
G-H	0 / 33	-94.6	-94.6 0.28 (1)	10.00	J-G	-160 / 14	0.14 (1)
N-B	-246 / 0	0.0	0.0 0.03 (1)	7.81	N-C	-1745 / 0	0.54 (1)
I-H	-154 / 0	0.0	0.0 0.02 (1)	7.81	G-I	-1745 / 0	0.54 (1)
N-M	0 / 1090	-17.5	-17.5 0.35 (4)	10.00			
M-L	0 / 991	-17.5	-17.5 0.35 (4)	10.00			
L-K	0 / 991	-17.5	-17.5 0.35 (4)	10.00			
K-J	0 / 991	-17.5	-17.5 0.35 (4)	10.00			
J-I	0 / 1090	-17.5	-17.5 0.35 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.94")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.94")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.39 (D-E:1), BC=0.35 (J-L:4),
WB=0.54 (C-N:1), SSI=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

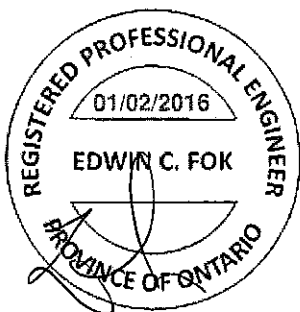
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.44 (G) (INPUT = 1.00)

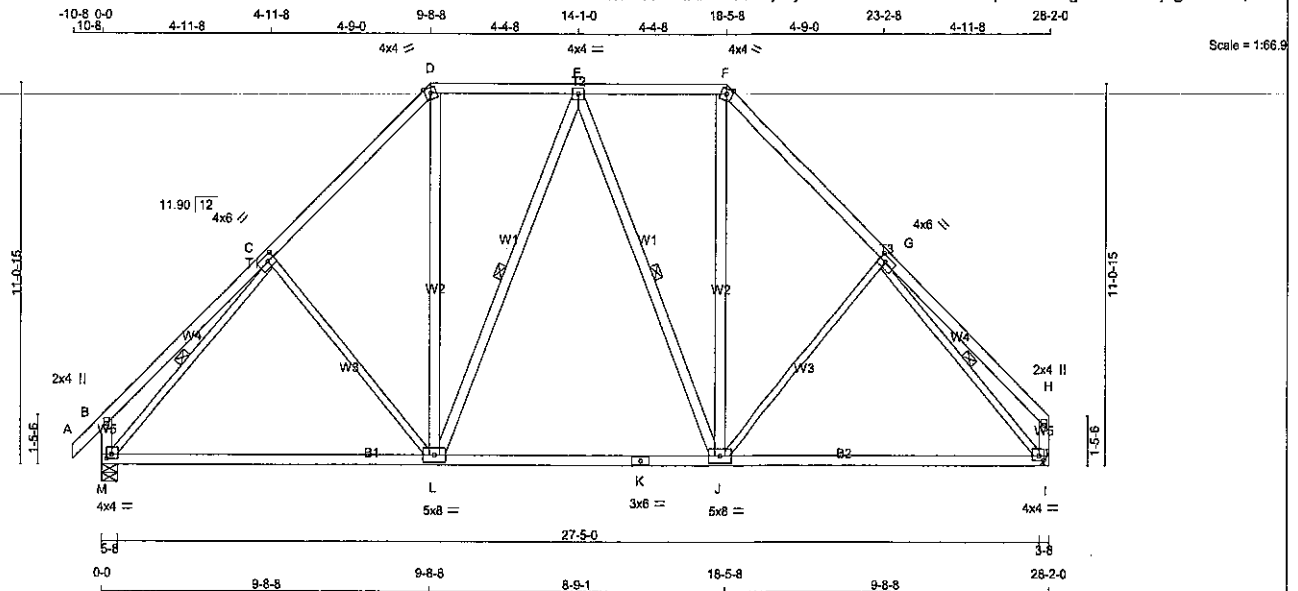
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H07	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 3 X 151 = 453 lb

LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF			
D - F	2x4	DRY	No.2	SPF			
F - H	2x4	DRY	No.2	SPF			
M - B	2x4	DRY	No.2	SPF			
I - H	2x4	DRY	No.2	SPF			
M - K	2x4	DRY	No.2	SPF			
K - I	2x4	DRY	No.2	SPF			
ALL WEBS	2x4	DRY	No.2	SPF			
EXCEPT							
C - L	2x3	DRY	No.2	SPF			
J - G	2x3	DRY	No.2	SPF			
M - C	2x3	DRY	No.2	SPF			
G - I	2x3	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	2.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	Edge	
G	TMVW-t	MT20	4.0	6.0	2.00	2.50
H	TMV+p	MT20	2.0	4.0		
I	BMVW-t	MT20	4.0	4.0	1.50	1.75
J	BMVW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	8.0		
M	BMVW-t	MT20	4.0	4.0	1.50	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS				FACTORED				MAXIMUM FACTORED				INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	JT	VERT	HORZ	GROSS REACTION	JT	VERT	HORZ	GROSS REACTION	BRG	IN-SX	BRG	IN-SX
M	1671	0	1671	0	0	0	0	5-8	1-13						
I	1579	0	1579	0	0	0	0								

UNFACTORED REACTIONS				1ST LCASE				MAX/MIN. COMPONENT REACTIONS				SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE
M	1162	874 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0														
I	1099	818 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	282 / 0	0 / 0														

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, E-J, C-M, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO				FR-TO			
A-B	0 / 33	-94.6	-94.6 0.07 (1)	C-L	-230 / 4	0.28 (1)	0.28 (1)
B-C	0 / 38	-94.6	-94.6 0.36 (1)	L-D	0 / 599	0.10 (1)	0.10 (1)
C-D	-1371 / 0	-94.6	-94.6 0.32 (1)	L-E	-268 / 0	0.20 (1)	0.20 (1)
D-E	-951 / 0	-94.6	-94.6 0.24 (1)	E-J	-268 / 0	0.20 (1)	0.20 (1)
E-F	-951 / 0	-94.6	-94.6 0.24 (1)	J-F	0 / 599	0.10 (1)	0.10 (1)
F-G	-1371 / 0	-94.6	-94.6 0.32 (1)	J-G	-230 / 4	0.28 (1)	0.28 (1)
G-H	0 / 38	-94.6	-94.6 0.36 (1)	M-C	-1725 / 0	0.66 (1)	0.66 (1)
M-B	-266 / 0	0.0	0.0 0.03 (1)	G-I	-1725 / 0	0.66 (1)	0.66 (1)
I-H	-173 / 0	0.0	0.0 0.02 (1)				
M-L	0 / 1094	-17.5	-17.5 0.41 (4)				
L-K	0 / 1050	-17.5	-17.5 0.42 (4)				
K-J	0 / 1050	-17.5	-17.5 0.42 (4)				
J-I	0 / 1094	-17.5	-17.5 0.41 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 29.0 PSF
	DL	= 3.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/998 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.36 (B-C:1), BC=0.42 (J-L:4),
WB=0.66 (C-M:1), SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.43 (G) (INPUT = 1.00)

A.160223

CONTINUED ON PAGE 2



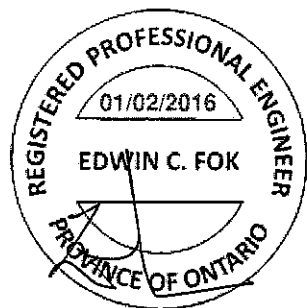
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H07	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

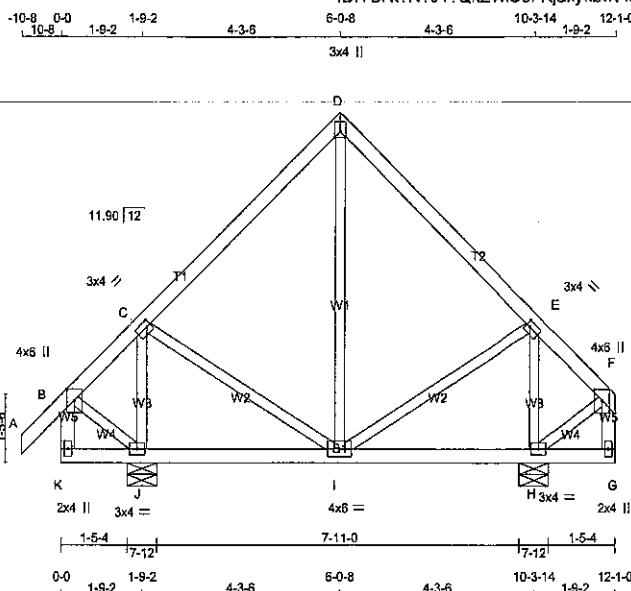


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H08	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1/4\"/>

TOTAL WEIGHT = 2 X 58 = 116 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.00
D	TTW+p	MT20	3.0	4.0	Edge	
E	TMVW-t	MT20	3.0	4.0	1.50	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
G	BMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	3.0	4.0		
K	BMV+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	658	0	658	0
J	789	0	789	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	PERM.LIVE		
H	459	339 / 0	0 / 0	120 / 0	0 / 0
J	547	419 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 33	-94.6 -94.6	0.07 (1)	I-D	-107 / 37	10.00	0.10 (1)
B-C	0 / 135	-94.6 -94.6	0.25 (1)	I-E	0 / 126	10.00	0.03 (1)
C-D	-184 / 0	-94.6 -94.6	0.23 (1)	H-E	-591 / 0	6.25	0.11 (1)
D-E	-184 / 0	-94.6 -94.6	0.23 (1)	C-I	0 / 193	6.25	0.04 (1)
E-F	0 / 56	-94.6 -94.6	0.23 (1)	J-C	-684 / 0	10.00	0.13 (1)
K-B	-3 / 3	0.0 0.0	0.00 (4)	B-J	-58 / 0	7.81	0.01 (1)
G-F	-1 / 4	0.0 0.0	0.00 (4)	H-F	-2 / 8	7.81	0.00 (1)
K-J	0 / 0	-17.5 -17.5	0.05 (4)			10.00	
J-I	-49 / 0	-17.5 -17.5	0.07 (4)			6.25	
I-H	-2 / 7	-17.5 -17.5	0.07 (4)			10.00	
H-G	0 / 0	-17.5 -17.5	0.05 (4)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL)= L/120 (0.18")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.25 (B-C:1), BC=0.07 (I-J:4), WB=0.13 (C-J:1), SSI=0.13 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (C) (INPUT = 0.90)
JSI METAL = 0.20 (E) (INPUT = 1.00)

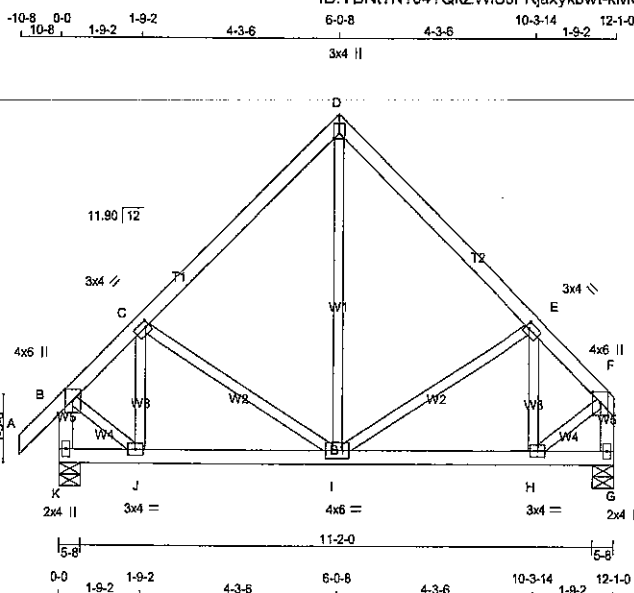


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Alpa Roof Truss, Maple

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Scale = 1:49.0

TOTAL WEIGHT = $2 \times 58 = 116$ lb

LUMBER

COMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
--------------------	-----	-----	------	-----

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW-l	MT20	3.0	4.0	1.50	1.00
D	TTW+p	MT20	3.0	4.0	Edge	
E	TMVW-t	MT20	3.0	4.0	1.50	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMVW-l	MT20	3.0	4.0		
I	BMVWWW-t	MT20	4.0	6.0		
J	BMVW-l	MT20	3.0	4.0		
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BOILING BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	770	0	770	0	0	5-8	1-8
G	677	0	677	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	534	408 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
G	472	351 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K. G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 33	-94.6	-94.6	0.07 (1)	10.00	I-D	0 / 249	0.06 (1)
B-C	-525 / 0	-94.6	-94.6	0.17 (1)	6.25	I-E	-150 / 0	0.07 (1)
C-D	-438 / 0	-94.6	-94.6	0.20 (1)	6.25	H-E	-205 / 4	0.04 (1)
D-E	-438 / 0	-94.6	-94.6	0.20 (1)	6.25	C-I	-150 / 0	0.07 (1)
E-F	-525 / 0	-94.6	-94.6	0.17 (1)	6.25	J-C	-205 / 4	0.04 (1)
K-B	-761 / 0	0.0	0.0	0.08 (1)	7.81	B-J	0 / 492	0.11 (1)
G-F	-669 / 0	0.0	0.0	0.07 (1)	7.81	H-F	0 / 492	0.11 (1)
K-J	0 / 0	-17.5	-17.5	0.04 (4)	10.00			
J-I	0 / 417	-17.5	-17.5	0.11 (1)	10.00			
I-H	0 / 417	-17.5	-17.5	0.11 (1)	10.00			
H-G	0 / 0	-17.5	-17.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC
2014

- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20 (C-D:1), BC=0.11 (H-I:1),
WB=0.11 (F-H:1), SSI=0.13 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

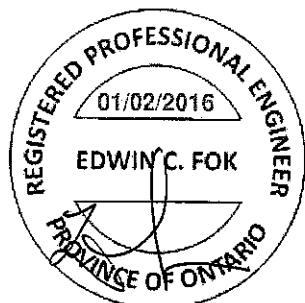
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.16 (J) (INPUT = 1.00)



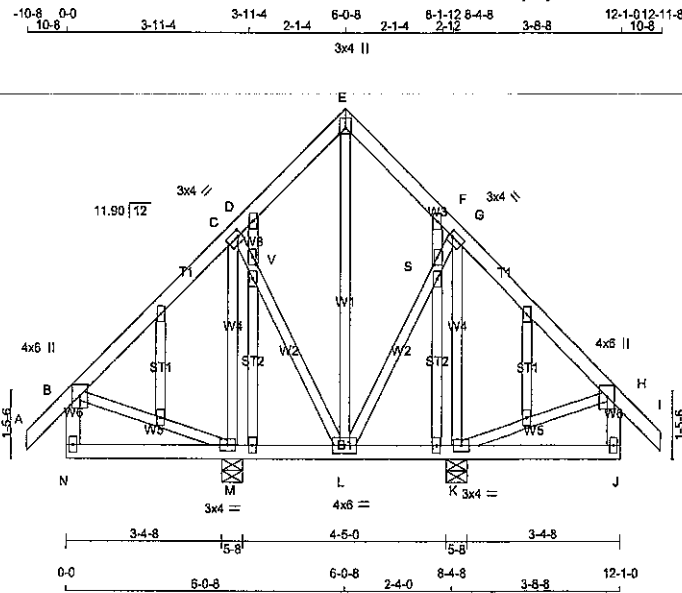
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H08G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:49.0

TOTAL WEIGHT = 77 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT	2-0-0	OC.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	6.0	2.75	2.00
C	TMW-w	MT20	3.0	4.0	1.50	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	3.0	4.0	Edge	
F	TMW+w	MT20	2.0	4.0		
G	TMW-w	MT20	3.0	4.0	1.50	1.00
H	TMW+p	MT20	4.0	6.0	2.75	2.00
J	BMV+p	MT20	2.0	4.0		
K	BMW-w	MT20	3.0	4.0		
L	BMW-w	MT20	4.0	6.0		
M	BMW-w	MT20	3.0	4.0		
N	BMV+p	MT20	2.0	4.0		
O, P, Q, R, S, T, U, V						
O	NP+w	MT20	2.0	4.0		
S	WMW+w	MT20	2.0	4.0	2.00	0.75
V	WMW+w	MT20	2.0	4.0	2.00	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
M	770	0	770	0	-76	5-8	1-8	1-8	
K	770	0	770	0	-76	5-8	1-8	1-8	

PROVIDE ANCHORAGE AT BEARING JOINT M FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT K FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	534	408/-119	0/0	0/0	0/0	127/0	0/0
K	534	408/-119	0/0	0/0	0/0	127/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0/33	-94.6	-94.6 0.07 (1)	10.00	L-E	-344/0	0.33 (1)
B-C	0/299	-94.6	-94.6 0.22 (1)	10.00	B-M	-207/0	0.05 (7)
C-D	0/134	-94.6	-94.6 0.18 (1)	10.00	K-H	-207/0	0.05 (7)
D-E	0/180	-94.6	-94.6 0.12 (7)	10.00	L-S	-117/267	0.10 (6)
E-F	0/180	-94.6	-94.6 0.12 (7)	10.00	S-G	-106/193	0.08 (6)
F-G	0/134	-94.6	-94.6 0.18 (1)	10.00	M-C	-623/134	0.23 (1)
G-H	0/299	-94.6	-94.6 0.22 (1)	10.00	C-V	-108/193	0.08 (5)
H-I	0/33	-94.6	-94.6 0.07 (1)	10.00	V-L	-117/267	0.10 (5)
N-B	0/28	0.0	0.0 0.01 (4)	10.00	K-G	-623/134	0.23 (1)
J-H	0/28	0.0	0.0 0.01 (4)	10.00	V-D	-12/81	0.02 (5)
					S-F	-12/81	0.02 (6)
N-M	0/0	-17.5	-17.5 0.06 (4)	10.00			
M-L	-198/0	-17.5	-17.5 0.06 (4)	6.25			
L-K	-198/0	-17.5	-17.5 0.06 (4)	6.25			
K-J	0/0	-17.5	-17.5 0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/120 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22 (G-H:1), BC=0.08 (K-L:4), WB=0.33 (E-L:1), SSI=0.11 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

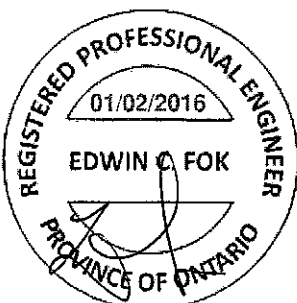
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (C) (INPUT = 0.90)
JSI METAL= 0.15 (C) (INPUT = 1.00)

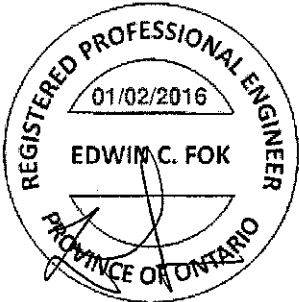
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H08G	1	1	TRUSS DESC.	

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



A: 602206(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H09A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

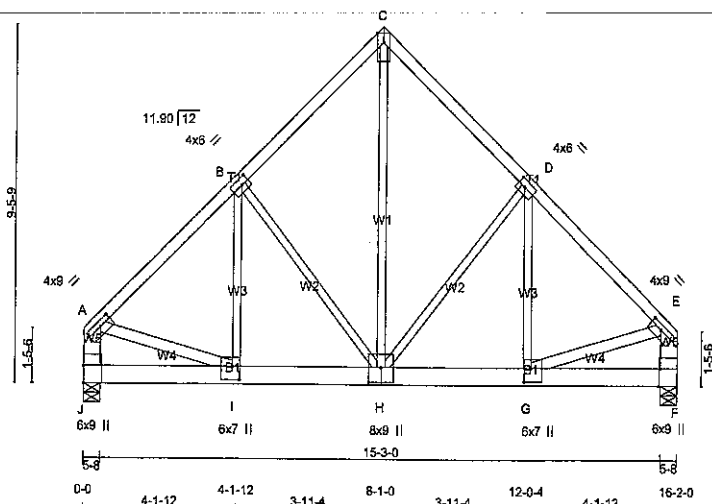
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0-0 4-1-12 4-1-12 3-1-14 8-1-0 3-1-14 12-0-4 4-1-12 16-2-0

4x8 II

Scale = 1:61.0



TOTAL WEIGHT = 2 X 93 = 185 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
A - I	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
C - E	1	12
J - A	2	12
F - E	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - H	2	6
H - F	2	6
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	6833	0	6833	0	5-8
F	6833	0	6833	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
J	4759	3539 / 0
F	4759	3539 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F
BEARING SIZE FACTOR = 1.15 AT JNT(S) J, F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
MAX. FACTORED	FACTORED	MAX. FACTORED	FACTORED
VERT. LOAD	VERT. LOAD	VERT. LOAD	VERT. LOAD
LC1	LC1	LC1	LC1
MAX	MAX	MAX	MAX
UNBRAC	UNBRAC	UNBRAC	UNBRAC
LENGTH	LENGTH	LENGTH	LENGTH
FR-TO	FR-TO	FR-TO	FR-TO
A - B	-6082 / 0	-94.6	-94.6
B - C	-4459 / 0	-94.6	-94.6
C - D	-4459 / 0	-94.6	-94.6
D - E	-6082 / 0	-94.6	-94.6
J - A	-5526 / 0	0.0	0.0
F - E	-5526 / 0	0.0	0.0
J - I	0 / 0	-750.8	-750.8
I - H	0 / 4334	-750.8	-750.8
H - G	0 / 4334	-750.8	-750.8
G - F	0 / 0	-750.8	-750.8

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 28-2-0
END DISTANCE = 16-2-0
END SPAN CARRIED = 28-2-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.32 (D-E:1), BC=0.66 (G-H:1),
WB=0.74 (C-H:1), SS=0.60 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.61 (H) (INPUT = 1.00)

A-1602207

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H09A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

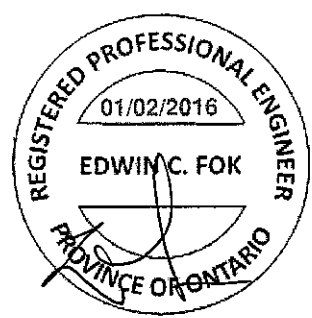
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	9.0	1.50	4.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TTW+p	MT20	4.0	9.0	Edge	
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TMVW-t	MT20	4.0	9.0	1.50	4.00
F	BMV1-t	MT20	6.0	9.0	Edge	0.50
G	BMWW-t	MT20	6.0	7.0	4.25	2.25
H	BSWWW-t	MT20	8.0	9.0	4.75	4.00
I	BMWW-t	MT20	6.0	7.0	4.25	2.25
J	BMV1-t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per
ORC 9.23.13.11, and no less than 2x4 for Part 4 design.



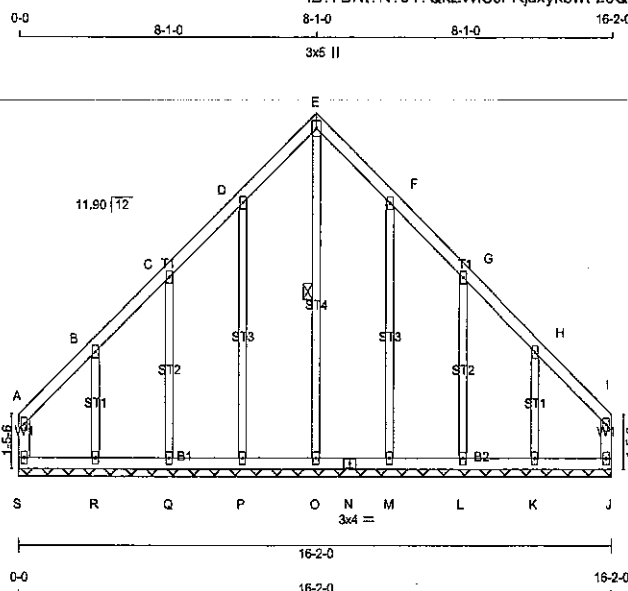
A-160227(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H09G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:61.3

TOTAL WEIGHT = 79 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
S - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMV+p	MT20	2.0	4.0		
B, C, D, F, G, H					
B - TMW+w	MT20	2.0	4.0		
E - TTW+p	MT20	3.0	5.0		
I - TMV+p	MT20	2.0	4.0		
J - BMV1+p	MT20	2.0	4.0		
K, L, M, O, P, Q, R					
K - BMW1+w	MT20	2.0	4.0		
N - BS-1	MT20	3.0	4.0		
S - BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED				WEBS			
	MEMB.	FORCE	VERT. LOAD	LC1	MEMB.	FORCE	VERT. LOAD	LC1
		(LBS)	(PLF)	CSI (LC)		(LBS)	(PLF)	CSI (LC)
FR-TO			FROM TO		FR-TO			
S-A	-132 / 0	0.0	0.0	0.04 (1)	O-E	-149 / 0	0.09 (1)	
A-B	-56 / 0	-94.6	-94.6	0.04 (1)	P-D	-189 / 0	0.19 (1)	
B-C	-48 / 0	-94.6	-94.6	0.04 (1)	Q-C	-185 / 0	0.08 (1)	
C-D	-42 / 0	-94.6	-94.6	0.04 (1)	R-B	-183 / 0	0.04 (1)	
D-E	-37 / 0	-94.6	-94.6	0.05 (1)	M-F	-189 / 0	0.19 (1)	
E-F	-37 / 0	-94.6	-94.6	0.05 (1)	L-G	-185 / 0	0.08 (1)	
F-G	-42 / 0	-94.6	-94.6	0.04 (1)	K-H	-183 / 0	0.04 (1)	
G-H	-48 / 0	-94.6	-94.6	0.04 (1)				
H-I	-56 / 0	-94.6	-94.6	0.04 (1)				
J-I	-132 / 0	0.0	0.0	0.04 (1)				
S-R	0 / 40	-17.5	-17.5	0.03 (1)				
R-Q	0 / 34	-17.5	-17.5	0.02 (4)				
Q-P	0 / 30	-17.5	-17.5	0.02 (4)				
P-O	0 / 27	-17.5	-17.5	0.02 (4)				
O-N	0 / 27	-17.5	-17.5	0.02 (4)				
N-M	0 / 27	-17.5	-17.5	0.02 (4)				
M-L	0 / 30	-17.5	-17.5	0.02 (4)				
L-K	0 / 34	-17.5	-17.5	0.02 (4)				
K-J	0 / 40	-17.5	-17.5	0.03 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(56 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05 (E-F:1), BC=0.03 (J-K:1), WB=0.19 (D-P:1), SSI=0.06 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

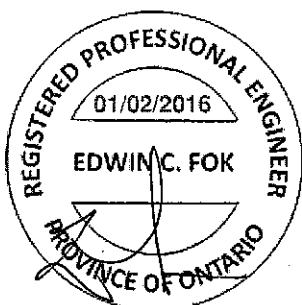
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (I) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)



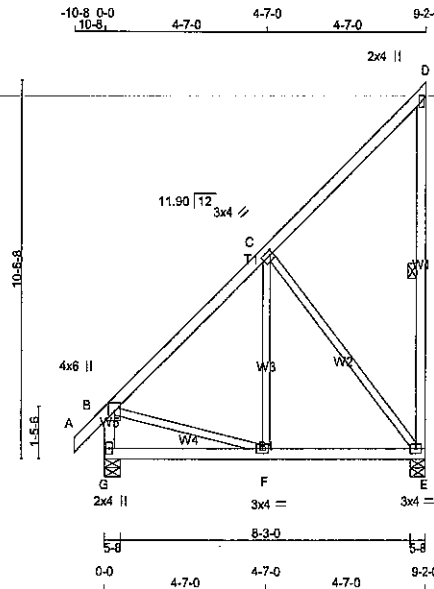
A-1602208

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H10	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 53 lb

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.00
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMVW-t	MT20	3.0	4.0		
G	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	514	0	514	0	5-8	1-8
G	606	0	606	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	358	266 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
G	420	323 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	UNBRACED LENGTH	FR-TO		
A-B	0 / 33	-94.6	-94.6	0.07 (1)	10.00	F-C	0 / 96
B-C	-327 / 0	-94.6	-94.6	0.26 (1)	6.25	C-E	-411 / 0
C-D	-38 / 0	-94.6	-94.6	0.26 (1)	6.25	B-F	0 / 267
E-D	-163 / 0	0.0	0.0	0.09 (1)	6.25		
G-B	-575 / 0	0.0	0.0	0.06 (1)	7.81		
G-F	0 / 0	-17.5	-17.5	0.10 (4)	10.00		
F-E	0 / 259	-17.5	-17.5	0.12 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.26 (B-C:1), BC=0.12 (E-F:4), WB=0.41 (C-E:1), SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (C) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

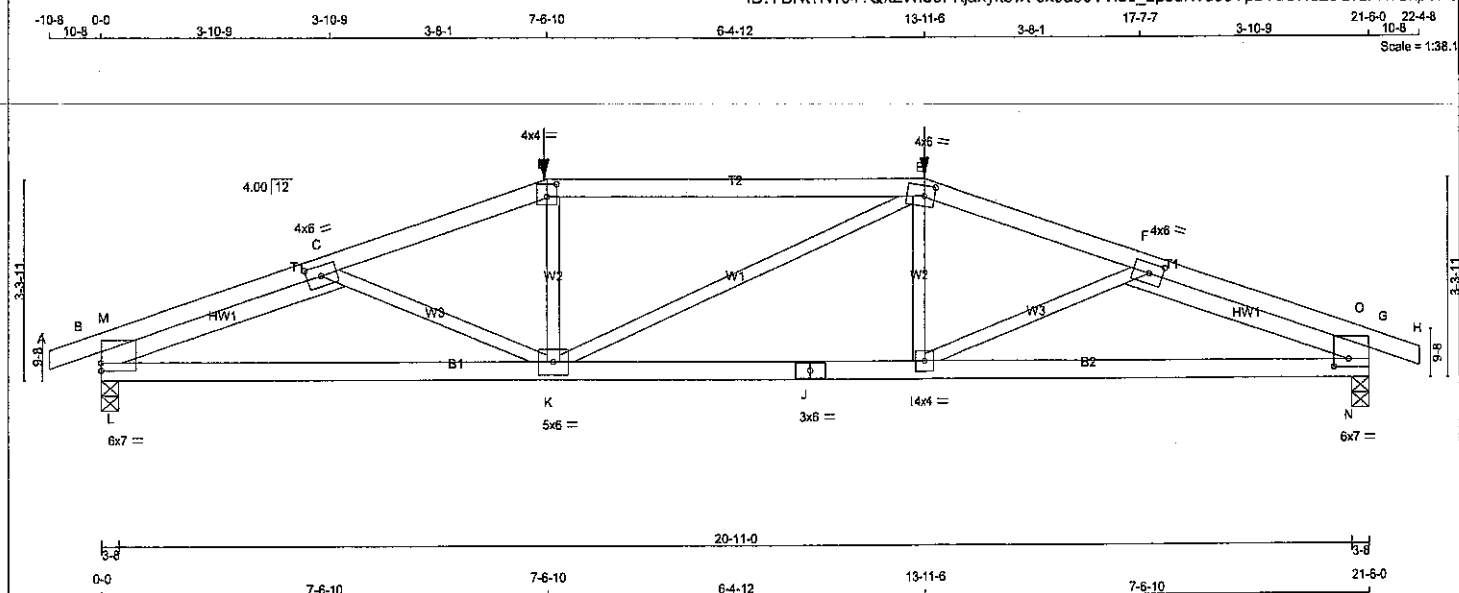


A-1602209

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H10A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
B - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW1	2x4	DRY	No.2
HW2	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW-1	MT20	6.0	7.0	1.50		
C	TMBMW-1	MT20	4.0	6.0	2.00	2.75	
D	TTW-1	MT20	4.0	4.0	2.50	2.00	
E	TTW-1	MT20	4.0	6.0	2.00	2.25	
F	TMBMW-1	MT20	4.0	6.0	2.00	2.75	
G	TMBMW-1	MT20	6.0	7.0	1.50	3.00	
I	BMWW-1	MT20	4.0	4.0			
J	BS-1	MT20	3.0	6.0			
K	BMWW-1	MT20	5.0	6.0			

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 59.5 lbs FACTORED DOWN AT 13-11-6, AND 59.5 lbs FACTORED DOWN AT 7-6-10 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
B	1352	0	1352	0
G	1352	0	1352	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
B	938	714 / 0	0 / 0	0 / 0	224 / 0
G	938	714 / 0	0 / 0	0 / 0	224 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING	TOTAL LOAD CASES: (4)
CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED	MAX. FACTORED
FORCE	FORCE
(LBS)	(LBS)
VERT. LOAD LC1	MAX. UNBRACED
(PLF)	LENGTH
CSI (LC)	FR-TO

FR-TO	FROM	TO	FR-TO	FROM	TO
A-B	-9 / 0	-94.6	C-K	-188 / 7	0.06 (1)
B-M	-1738 / 0	-94.6	K-D	0 / 197	0.05 (4)
M-C	-931 / 0	-94.6	D-E	0 / 0	0.00 (1)
C-D	-2243 / 0	-94.6	E-F	0 / 197	0.05 (4)
D-E	-2127 / 0	-94.6	F-G	-188 / 7	0.06 (1)
E-F	-2243 / 0	-94.6	G-H	0 / 1024	0.00 (1)
F-G	-931 / 0	-94.6	H-I	-1672 / 0	0.37 (1)
G-H	-1738 / 0	-94.6	I-J	-1571 / 0	0.37 (1)
H-I	-9 / 0	-94.6	J-K	0 / 1025	0.00 (1)

MEMB.	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	7-6-10	-80	-60	-	FRONT	VERT	TOTAL
E	13-11-6	-80	-60	-	FRONT	VERT	TOTAL

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	7-6-10	-80	-60	-	FRONT	VERT	TOTAL	
E	13-11-6	-80	-60	-	FRONT	VERT	TOTAL	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 7-6-10
END SETBACK = 2-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL)= L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.98 (D-E:1), BC=0.51 (I-N:1), WB=0.37 (C-L:1), SSI=0.42 (G-N:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

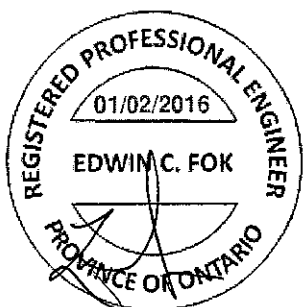
NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.70 (E) (INPUT = 1.00)

A.1602210
CONTINUED ON PAGE 2



11

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-6-12	-220	-220	---	FRONT	VERT	TOTAL
F	16-11-12	-31	-31	---	FRONT	VERT	TOTAL

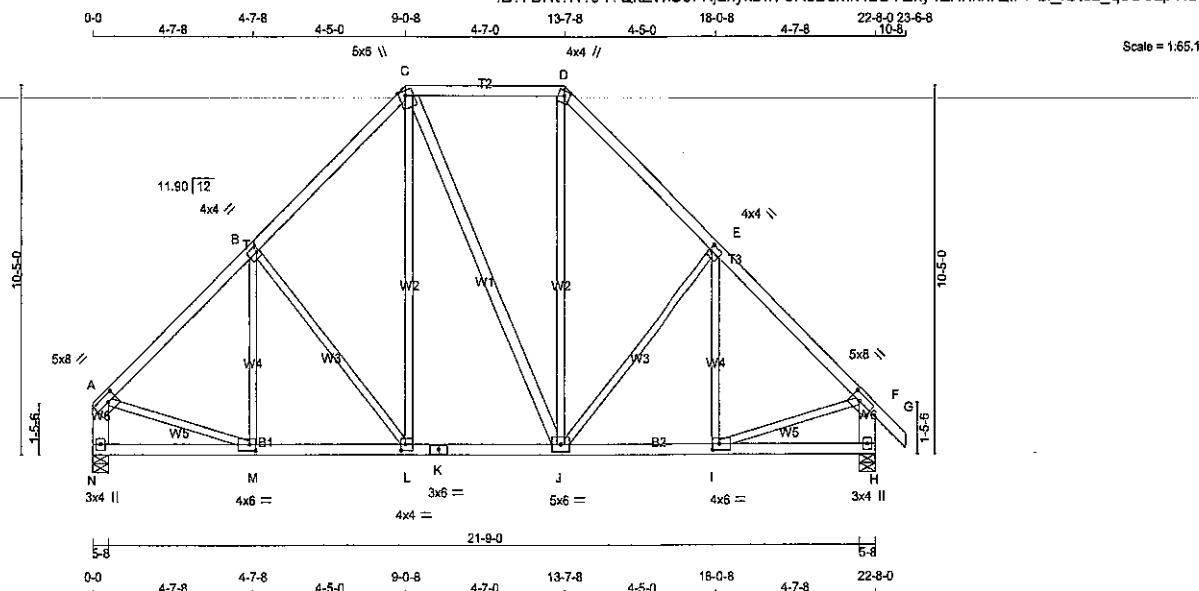
JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.82 (C) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	H12A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	8.0	2.25	3.00
B	TMVWV-1	MT20	4.0	4.0	2.00	1.00
C	TTWV+m	MT20	5.0	6.0	Edge	2.25
D	TTW+m	MT20	4.0	4.0	1.75	1.75
E	TMVWV-1	MT20	4.0	4.0	2.00	1.00
F	TMVWV-1	MT20	5.0	8.0	2.25	3.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVWV-1	MT20	4.0	6.0	2.00	2.25
J	BMVWV-1	MT20	5.0	6.0		
K	BS-1	MT20	3.0	6.0		
L	BMVWV-1	MT20	4.0	4.0	2.00	1.50
M	BMVWV-1	MT20	4.0	6.0	2.00	2.25
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT
N	2501	0	2501	0	0
H	2593	0	2593	0	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
N	1742	1295 / 0	0 / 0	0 / 0	448 / 0
H	1804	1352 / 0	0 / 0	0 / 0	452 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	MAX. UNBRAC (LBS)	MEMB. FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO
A-B	-2368 / 0	-94.6	-94.6	0.54 (1)	3.94
B-C	-1954 / 0	-94.6	-94.6	0.47 (1)	4.32
C-D	-1370 / 0	-94.6	-94.6	0.42 (1)	4.97
D-E	-1957 / 0	-94.6	-94.6	0.47 (1)	4.32
E-F	-2368 / 0	-94.6	-94.6	0.54 (1)	3.94
F-G	0 / 33	-94.6	-94.6	0.07 (1)	10.00
N-A	-2266 / 0	0.0	0.0	0.17 (1)	6.77
H-F	-2357 / 0	0.0	0.0	0.17 (1)	6.66
N-M	0 / 0	-126.1	-126.1	0.39 (1)	10.00
M-L	0 / 1705	-126.1	-126.1	0.69 (1)	10.00
L-K	0 / 1368	-126.1	-126.1	0.55 (1)	10.00
K-J	0 / 1368	-126.1	-126.1	0.55 (1)	10.00
J-I	0 / 1704	-126.1	-126.1	0.68 (1)	10.00
I-H	0 / 0	-126.1	-126.1	0.38 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStd Girder

START DISTANCE = 0-0

START SPAN CARRIED = 5-10-8

END DISTANCE = 22-8-0

END SPAN CARRIED = 5-10-8

END WALL WIDTH = 0-0

APPLIED TO FRONT SIDE OF BOTTOM CHORD.

- ADDTL LOADS BASED ON 55% OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")

CALCULATED VERT. DEFL.(LL) = L/999 (0.07")

ALLOWABLE DEFL.(TL) = L/360 (0.76")

CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.54 (A-B:1), BC=0.69 (L-M:1), WB=0.58 (B-L:1), SSI=0.30 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

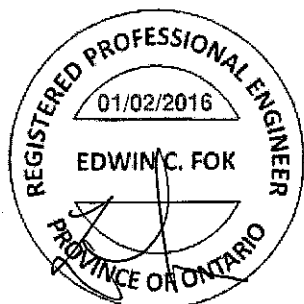
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)

JSI METAL= 0.48 (F) (INPUT = 1.00)

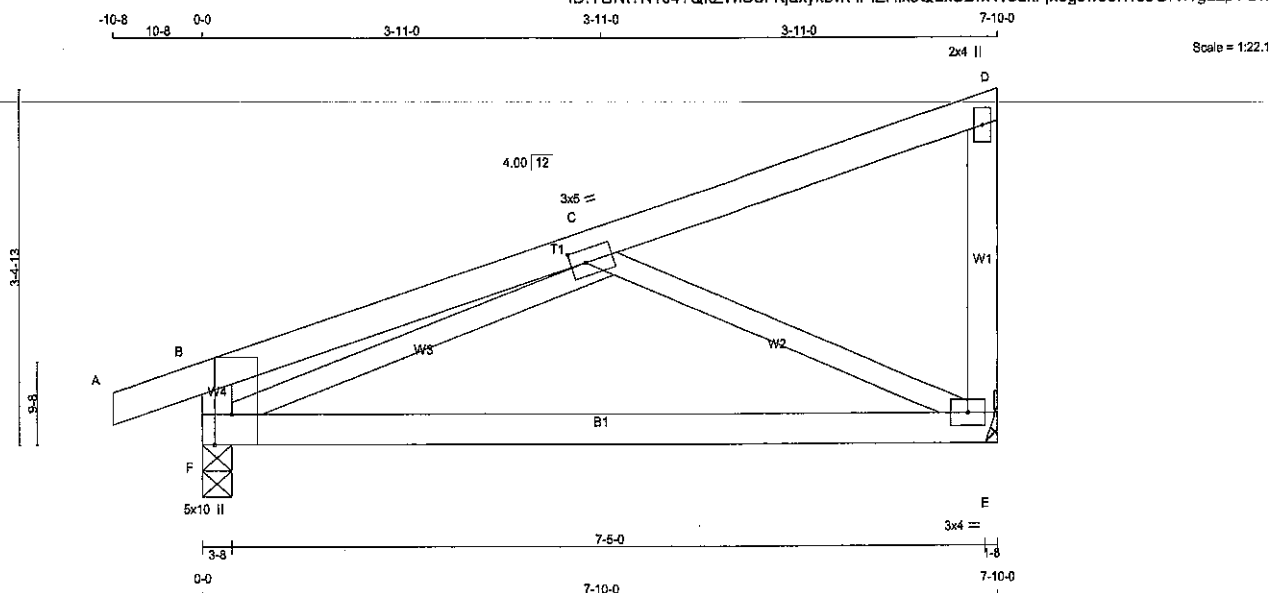
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	J04	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 2 X 30 = 59 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMWW-t	MT20	3.0	5.0	1.50	1.75
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	TMBVW1+p	MT20	5.0	10.0	Edge	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
F	526	0	526	0
E	439	0	439	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	365	281 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
E	306	227 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
F-B		-226 / 0	0.0	0.0	0.02 (1)	7.81	F-C	-552 / 0	0.17 (1)
A-B		0 / 14	-94.6	-94.6	0.06 (1)	10.00	C-E	-549 / 0	0.17 (1)
B-C		0 / 14	-94.6	-94.6	0.23 (1)	10.00			
C-D		-13 / 0	-94.6	-94.6	0.17 (1)	6.25			
E-D		-147 / 0	0.0	0.0	0.03 (1)	7.81			
F-E		0 / 502	-17.5	-17.5	0.33 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/585 (0.16")

CSI: TC=0.23 (B-C:1), BC=0.33 (E-F:4),
WB=0.17 (C-E:1), SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

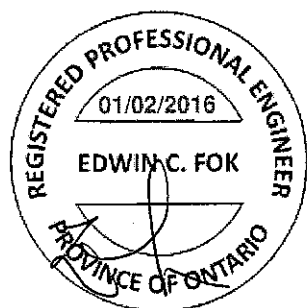
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.19 (E) (INPUT = 1.00)

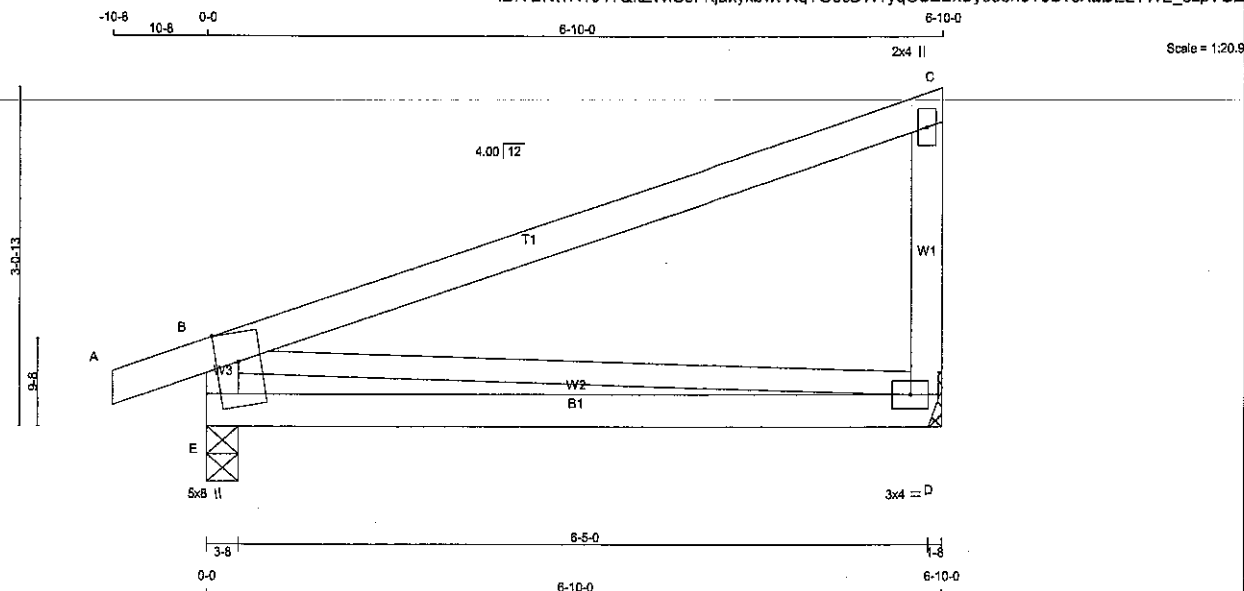


A.1602213

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	J05	1	1	TRUSS DESC.	

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TOTAL WEIGHT = 25 lb
(M/F)

LUMBER

N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	2100F 1.8E	SPF	
D - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	3.0	4.0		
E	TMBMVW1+m	MT20	5.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	470	0	470	0
D	383	0	383	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	326	252 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0
D	267	198 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-410 / 0	0.0	0.0	0.04 (1)	7.81	B-D	0 / 0
A-B	0 / 14	-94.6	-94.6	0.04 (1)	10.00		
B-C	0 / 0	-94.6	-94.6	0.05 (1)	10.00		
D-C	-323 / 0	0.0	0.0	0.05 (1)	7.81		
E-D	0 / 0	-17.5	-17.5	0.24 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.23")

CALCULATED VERT. DEFL.(TL) = L/882 (0.09")

CSI: TC=0.50 (B-C:1), BC=0.24 (D-E:4), WB=0.00 (B-D:1), SSI=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

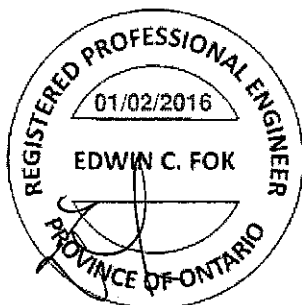
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN
MT20	618 354	1667 822

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (D) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

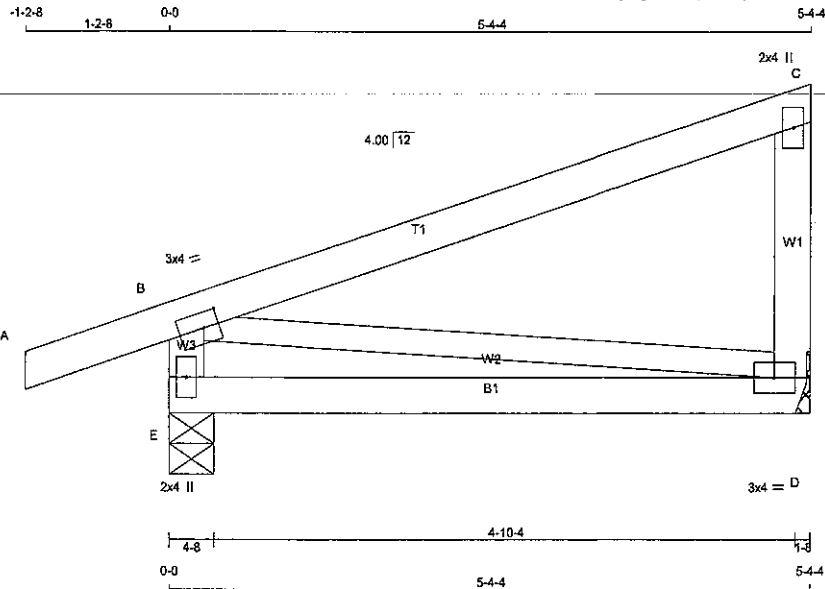


A-160121 4

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	J08	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:18.8

TOTAL WEIGHT = 21 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.50
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	3.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
E	419	0	0	419	0	0	4-8	1-8	
D	300	0	0	300	0	0	HANGER BY OTHERS		
MIN. SEAT SIZE: 1-8									

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS					
E	289	228 / 0	0 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0	0 / 0
D	209	155 / 0	0 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO	CS1 (LC)	UNBRAC LENGTH	FR-TO	CS1 (LC)
E-B	-372 / 0	0.0	0.0	0.04 (1)	7.81	B-D	0 / 0
A-B	0 / 19	-94.6	-94.6	0.10 (1)	10.00		0.00 (1)
B-C	0 / 0	-94.6	-94.6	0.46 (1)	10.00		
D-C	-253 / 0	0.0	0.0	0.03 (1)	7.81		
E-D	0 / 0	-17.5	-17.5	0.16 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.46 (B-C:1), BC=0.15 (D-E:4),
WB=0.00 (B-D:1), SS=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

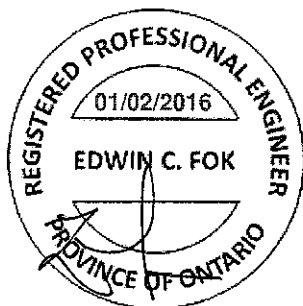
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.80)
JSI METAL= 0.07 (E) (INPUT = 1.00)

A-160216

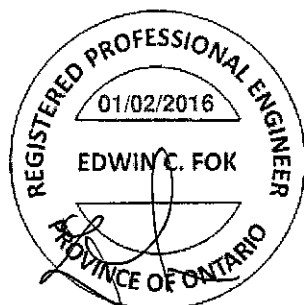


Alpa Roof Truss, Maple



TOTAL WEIGHT = 20 lb

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)

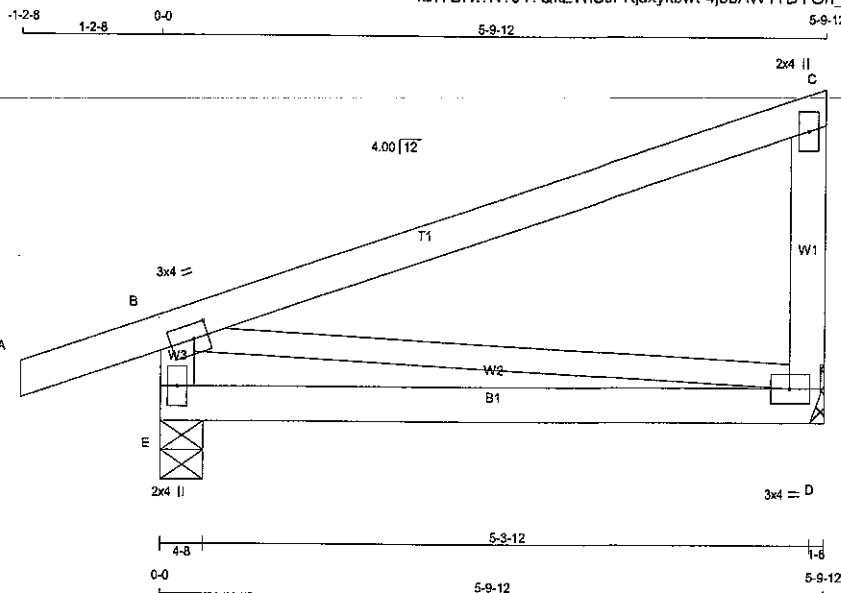


A-1602218

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264021	J11	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 22 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (tab/e is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	3.0	4.0	1.50	1.50
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-1	MT20	3.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

NOTES-

- (1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
E	444	0	444	0
D	326	0	326	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
E	307	242 / 0	0 / 0
D	227	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	LC1		(LBS)
FR-TO		MAX	FR-TO	
E-B	-394 / 0	0.0	0.0	0.04 (1)
A-B	0 / 19	-94.6	-94.6	0.10 (1)
B-C	0 / 0	-94.6	-94.6	0.54 (1)
D-C	-275 / 0	0.0	0.0	0.04 (1)
E-D	0 / 0	-17.5	-17.5	0.17 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 29.0	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09
- TPIC 2011

(55 % OF 37.6 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/899 (0.05")

CSI: TC=0.54 (B-C:1), BC=0.17 (D-E:4),
WB=0.00 (B-D:1), SSI=0.20 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

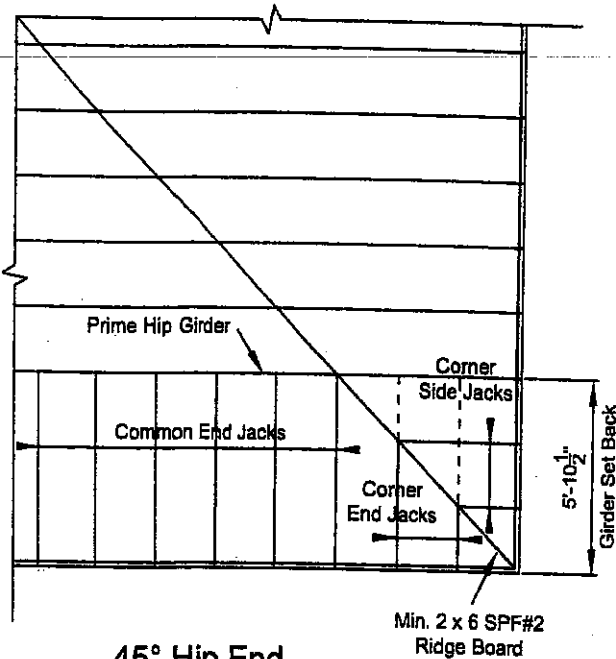
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)



A-1602219

STRACON ENGINEERING INC.



45° Hip End

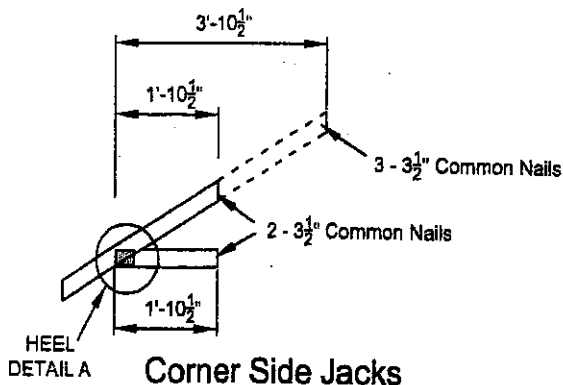
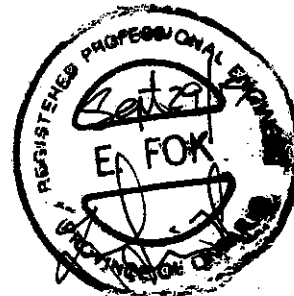
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

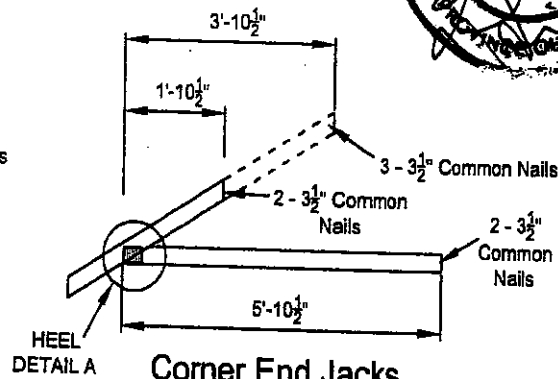
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

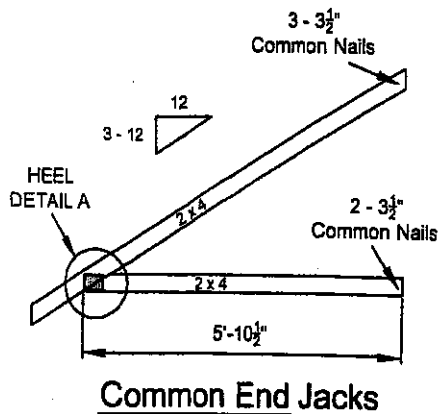
TOTAL LOAD : 50.5 P.S.F



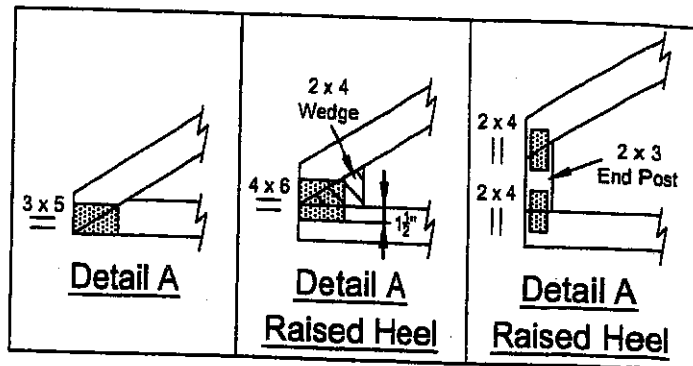
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

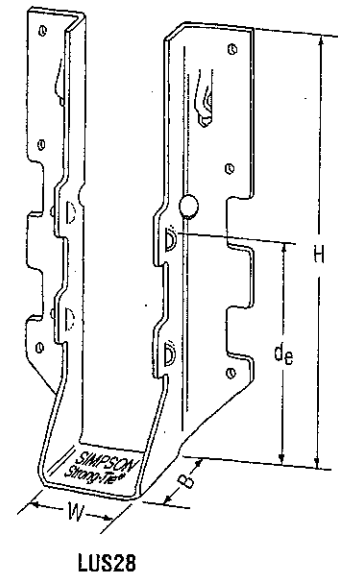
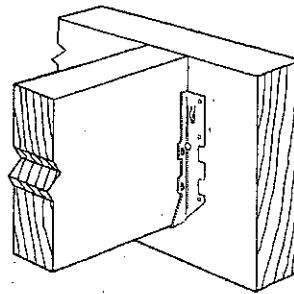
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

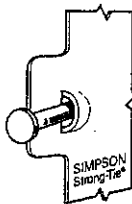
- These hangers cannot be modified.

Typical LUS Installation



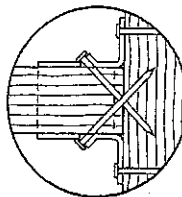
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1½	3½	1¾	1½	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	4-16d	2-16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1½	7¾	1¾	3¾	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

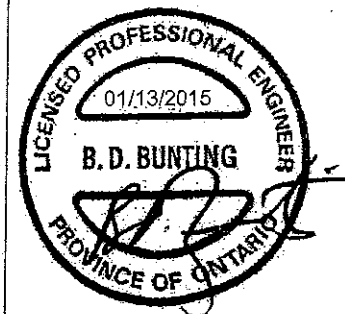


Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



800-999-5099
www.strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

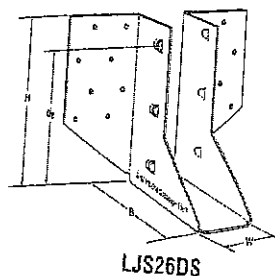
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

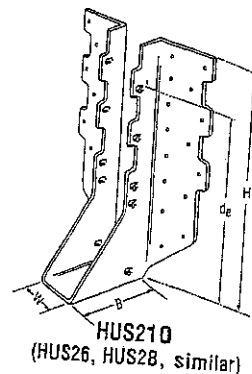
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

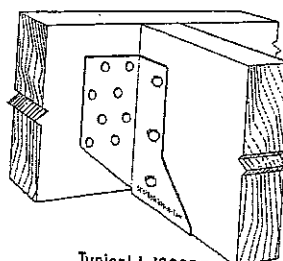
- See current catalogue for options



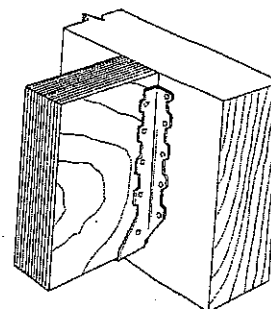
LJS26DS



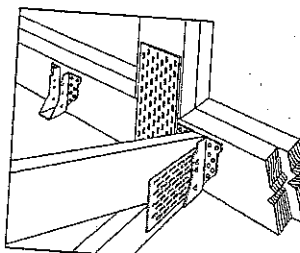
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



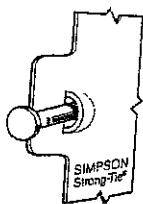
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

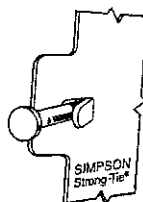
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/4	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 1/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/8	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 3/8	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

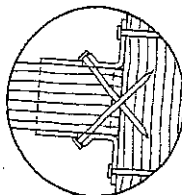


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

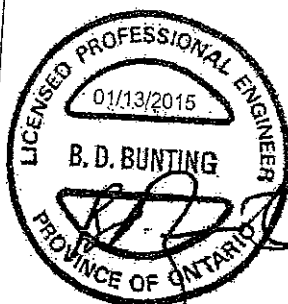
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

SUR/SUL/HSUR/HSUL Skewed 45° Hangers**SIMPSON**
Strong-Tie

This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed cost,
or a combination of these features.

The SUR/L and HSUR/L series of hangers are skewed 45° or right. Angled nail slots direct nails for proper installation.

MATERIAL: SUR and SUL—16 gauge; HSUR and HSUL—14 gauge

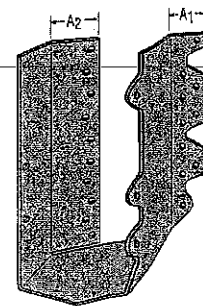
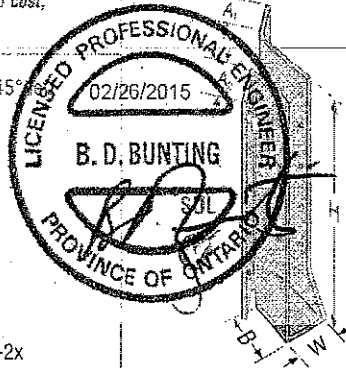
FINISH: Galvanized. Some products available in ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

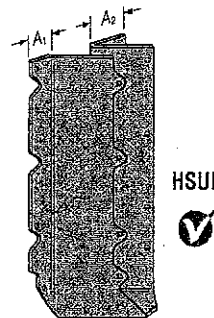
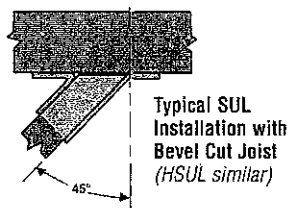
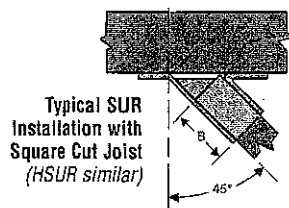
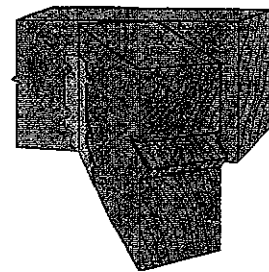
- These hangers will normally accommodate a 40° to 50° skew.
- Illustration shows left and right skews SUR/L (SUR = skewed right; SUL = skewed left).
- The joist end may be square cut or bevel cut.

OPTIONS: • Available with the A₂ flange (acute side) turned in on the 2-2x and 4x models only (see illustration).

- To order, add "C" (for concealed) to the product name.
- For example, specify HSURC46, HSULC46, SURC46, or SULC46.

**HSULC**

Available for 2-2x and 4x models only

**HSUR**

Typical SUR410 Installation

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

Joist Size	Model No.	Dimensions (in)					Fasteners		Factored Resistance			
		W	H	B	A ₁	A ₂	Face	Joist	D.Fir-L		S.P.F.	
									Uplift	Normal	Uplift	Normal
									(K _n = 1.15)	(K _n = 1.00)	(K _n = 1.15)	(K _n = 1.00)
									lbs.	lbs.	lbs.	lbs.
kN	kN	kN	kN									
2x4	SUR/L24	1⅝	3½	2	1⅝	1¼	4-16d	4-10dx1½	850	1210	600	860
2x6, 8	SUR/L26	1⅝	5	2	1⅝	1⅜	6-16d	6-10dx1½	3.78	5.38	2.67	3.83
									1255	2130	890	1530
2x10, 12	SUR/L210	1⅝	8⅝	2	1⅝	1⅝	10-16d	10-10dx1½	5.58	9.47	3.96	6.81
									2085	3820	1480	2710
2x12, 14	SUR/L214	1⅝	10	2	1⅝	1⅝	12-16d	12-10dx1½	9.27	16.99	6.58	12.05
									2690	4585	2175	3255
3x10, 12	SUR/L2.56/9	2⅞	8⅞	3⅞	1⅝	2⅝	14-16d	2-10dx1½	11.97	20.40	9.67	14.48
									385	3950	275	2805
(2) 2x6, 8	SUR/L26-2	3¼	4⅞	2⅞	1⅝	2⅞	8-16d	4-16dx2¼	1.71	17.57	1.22	12.48
									1130	2035	1045	1380
	HSUR/L26-2	3¼	4⅞	2⅞	1⅝	2⅞	12-16d	4-16dx2¼	5.03	9.05	4.85	8.70
									1230	2750	1090	1955
(2) 2x10, 12	SUR/L210-2	3⅝	8⅞	2⅞	1⅝	2⅞	14-16d	6-16dx2½	5.47	12.23	4.85	8.70
									1695	4065	1540	2875
	HSUR/L210-2	3⅝	8⅞	2⅞	1⅝	2⅞	20-16d	6-16dx2½	7.54	18.08	6.85	16.66
									1840	5270	1540	3745
4x6, 8	SUR/L46	3⅝	4⅞	2⅞	1	2⅞	8-16d	4-16d	8.18	23.44	6.85	16.66
									1130	2035	1045	1380
	HSUR/L46	3⅝	4⅞	2⅞	1	2⅞	12-16d	4-16d	5.03	9.05	4.85	8.70
									1230	2750	1090	1955
4x10, 12	SUR/L410	3⅝	8⅞	2⅞	1	2⅞	14-16d	6-16d	5.47	12.23	4.85	8.70
									1695	4065	1540	2875
	HSUR/L410	3⅝	8⅞	2⅞	1	2⅞	20-16d	6-16d	7.54	18.08	6.85	16.66
									1840	5270	1540	3745
									8.18	23.44	6.85	16.66

1. Factored uplift resistances have been increased by 15% for earthquake or wind loading with no further increase allowed; reduce for other load durations as required by code.

2. NAILS: 16d = 0.162" dia. x 3 1/2" long, 16dx2 1/2 = 0.162" dia. x 2 1/2" long, 10dx1 1/2 = 0.148" dia. x 1 1/2" long. See pages 22-23 for other nail sizes and information.