

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	10-00-00	9 1/2" NI-40x	1	6
J2	18-00-00	11 7/8" NI-40x	1	15
J3DJ	18-00-00	11 7/8" NI-40x	2	2
J3	14-00-00	11 7/8" NI-40x	1	3
J4	12-00-00	11 7/8" NI-40x	1	4
J5	10-00-00	11 7/8" NI-40x	1	5
J6	6-00-00	11 7/8" NI-40x	1	3
J7	2-00-00	11 7/8" NI-40x	1	4
B1	18-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B2	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B5	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

	Connector Summary				
Qty	Manuf	Product			
10	H1	IUS2.56/11.88			
4	H1	IUS2.56/11.88			
2	H2	HUS1.81/10			
1	H2	HUS1.81/10			

TOWN OF BRADFORD WEST GWILLIMBURY BUILDING DEPARTMENT PLANS EXAMINED ONTARIO BUILDING CODE APPLIES DATE: 04/19/2024

INSPECTOR: BG

REVIEWED

DATE: 2021-11-09

1st FLOOR



FROM PLAN DATED: 2021/10

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: TH-1 NAPA 1

ELEVATION: A

LOT:

CITY: BRADFORD

SALESMAN: RICK DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND INSTALLATION.

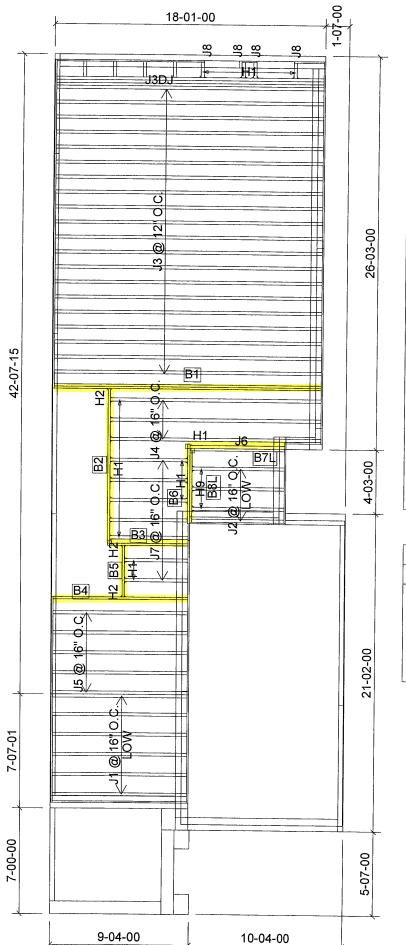
SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE

FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE

APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft² TILE LOAD: 20.0 lb/ft²



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	10-00-00	9 1/2" NI-40x	1	6
J2	8-00-00	9 1/2" NI-40x	1	4
J3	18-00-00	11 7/8" NI-40x	1	20
J3DJ	18-00-00	11 7/8" NI-40x	2	2
J4	16-00-00	11 7/8" NI-40x	1	3
J5	10-00-00	11 7/8" NI-40x	1	5
J6	8-00-00	11 7/8" NI-40x	1	1
J7	6-00-00	11 7/8" NI-40x	1	7
J8	2-00-00	11 7/8" NI-40x	1	4
B7L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B8L	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1	18-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B2	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	_ 1
B4	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B6	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B5	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary				
Qty	Manuf	Product		
14	H1	IUS2.56/11.88		
4	H1	IUS2.56/11.88		
2	H2	HUS1.81/10		
1	H2	HUS1.81/10		
3	H9	IUS2.56/9.5		



DATE: 2021-11-17

1st FLOOR SUNKEN



FROM PLAN DATED: 2021/10

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: TH-1 NAPA 1

ELEVATION: A

LOT:

CITY: BRADFORD

SALESMAN: RICK DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND

INSTALLATION.

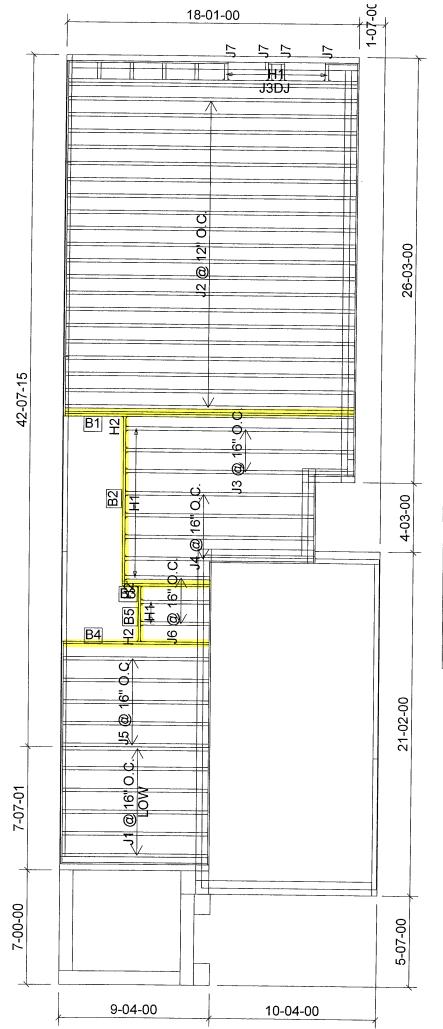
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DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE

APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft2 DEAD LOAD: 15.0 lb/ft2 TILE LOAD: 20.0 lb/ft²



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	10-00-00	9 1/2" NI-40x	1	6
J2	18-00-00	11 7/8" NI-40x	1	20
J3DJ	18-00-00	11 7/8" NI-40x	2	2
J3	16-00-00	11 7/8" NI-40x	1	3
J4	12-00-00	11 7/8" NI-40x	1	4
J5	10-00-00	11 7/8" NI-40x	1	5
J6	6-00-00	11 7/8" NI-40x	1	3
J7	2-00-00	11 7/8" NI-40x	1	4
B1	18-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B2	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B5	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
		1 5/1 X 11 1/0 VENOA-LAIN® 2.0 3 100 SP		1

	Connector Summary				
Qty	Manuf	Product			
10	H1	IUS2.56/11.88			
4	H1	IUS2.56/11.88			
2	H2	HUS1.81/10			
1	H2	HUS1.81/10			



FROM PLAN DATED: 2021/10

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: TH-1 NAPA 1
ELEVATION: B

LLL VAII

LOT:

CITY: BRADFORD

SALESMAN: RICK DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND
INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK DEC

JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

LOADING:

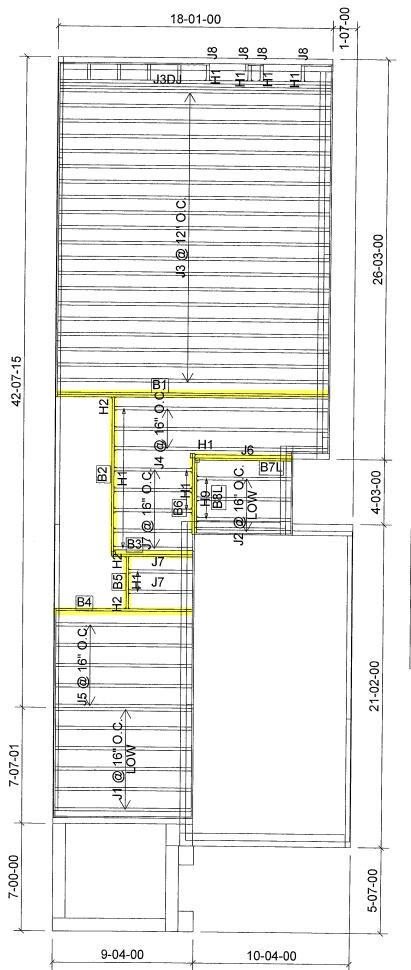
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft² TILE LOAD: 20.0 lb/ft²

SUBFLOOR: 3/4" GLUED AND NAILED

REVIEWED

DATE: 2021-11-17

1st FLOOR



	·	Products		
PlotID	Length	Product	Plies	Net Qty
J1	10-00-00	9 1/2" NI-40x	1	6
J2	8-00-00	9 1/2" NI-40x	1	4
J3	18-00-00	11 7/8" NI-40x	1	20
J3DJ	18-00-00	11 7/8" NI-40x	2	2
J4	16-00-00	11 7/8" NI-40x	1	3
J5	10-00-00	11 7/8" NI-40x	1	5
J6	8-00-00	11 7/8" NI-40x	1	1
J7	6-00-00	11 7/8" NI-40x	1	7
J8	2-00-00	11 7/8" NI-40x	1	4
B7L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B8L	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1	18-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B2	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B6	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B5	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

	Connector Summary				
Qty	Manuf	Product			
14	H1	IUS2.56/11.88			
4	H1	IUS2.56/11.88			
2	H2	HUS1.81/10			
1	H2	HUS1.81/10			
3	H9	IUS2.56/9.5			



DATE: 2021-11-17

1st FLOOR SUNKEN



FROM PLAN DATED: 2021/10

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: TH-1 NAPA 1

ELEVATION: B

LOT:

CITY: BRADFORD

SALESMAN: RICK DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC INSTALLATION
GUIDE FOR PROPER STORAGE AND

INSTALLATION.

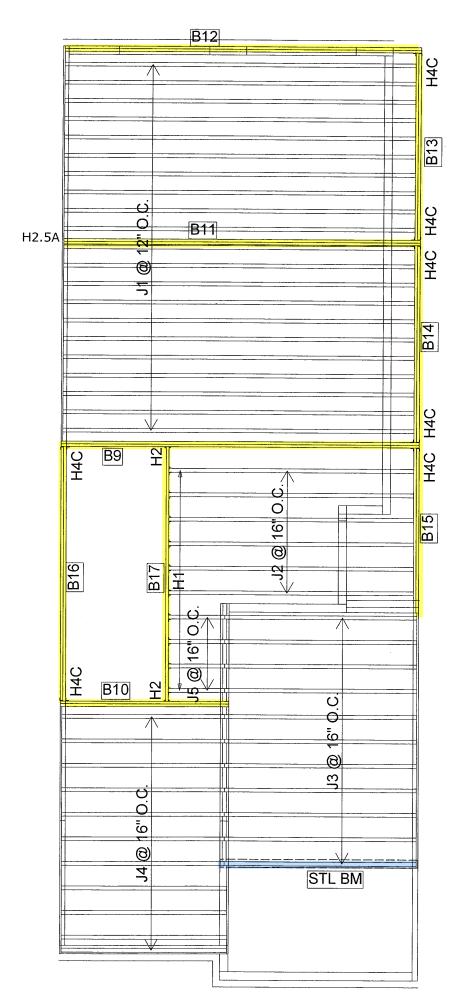
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SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE

APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft² TILE LOAD: 20.0 lb/ft²



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	20-00-00	11 7/8" NI-40x	1	21
J2	14-00-00	11 7/8" NI-40x	1	6
J3	12-00-00	11 7/8" NI-40x	1	11
J4	10-00-00	11 7/8" NI-40x	1	11
J5	4-00-00	11 7/8" NI-40x	1	4
B11	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B12	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B9	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B17	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B16	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B13	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B14	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B10	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B15	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2

	Connector Summary				
Qty	Manuf	Product			
10	H1	IUS2.56/11.88			
2	H2	HUS1.81/10			
7	H4C	HUC412			
1	H2.5A	H2.5A*			



DATE: 2021-12-02

2ND FLOOR



FROM PLAN DATED: 2021/10

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: TH-1 NAPA 1

ELEVATION: A

LOT:

CITY: BRADFORD

SALESMAN: RICK DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND INSTALLATION.

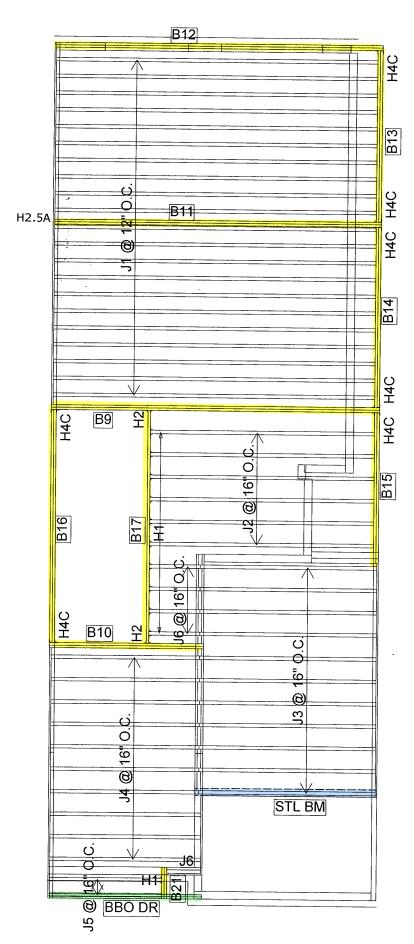
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FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE

APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft² TILE LOAD: 20.0 lb/ft²



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	20-00-00	11 7/8" NI-40x	1	21
J2	14-00-00	11 7/8" NI-40x	1	6
J3	12-00-00	11 7/8" NI-40x	1	11
J4	10-00-00	11 7/8" NI-40x	1	10
J5	8-00-00	11 7/8" NI-40x	1	2
J6	4-00-00	11 7/8" NI-40x	1	5
B11	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B12	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B9	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B17	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	_ 1
B16	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B13	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B14	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B10	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B15	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B21	2-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2

	Connector Summary				
Qty	Manuf	Product			
10	H1	IUS2.56/11.88			
1	H1	IUS2.56/11.88			
2	H2	HUS1.81/10			
7	H4C	HUC412			
1	H2.5A	H2.5A*			



DATE: 2021-12-02

2ND FLOOR



FROM PLAN DATED: 2021/10

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: TH-1 NAPA 1

ELEVATION: B

LOT:

CITY: BRADFORD

SALESMAN: RICK DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND

INSTALLATION.

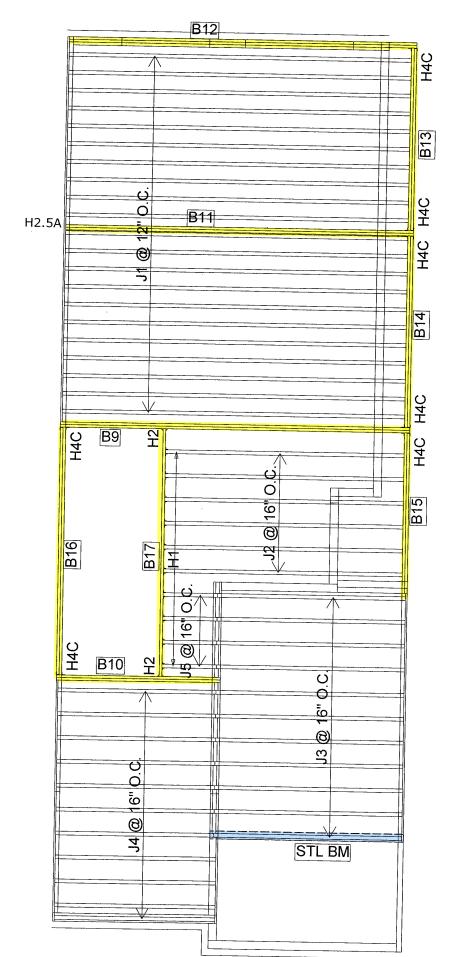
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APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft² TILE LOAD: 20.0 lb/ft²



DI (IB		Products		
PlotID	Length	Product	Plies	Net Qty
J1	20-00-00	11 7/8" NI-40x	1	21
J2	14-00-00	11 7/8" NI-40x	1	6
J3	12-00-00	11 7/8" NI-40x	1	-
J4	10-00-00	11 7/8" NI-40x	1	11
J5	4-00-00	11 7/8" NI-40x	1	11
B11	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	4
B12	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B9	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B17	14-00-00		2	2
B16	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B13	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B14		1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B10	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B15	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2

Connector Summary								
Qty	Manuf	Product						
10	H1	IUS2.56/11.88						
2	H2	HUS1.81/10						
7	H4C	HUC412						
1	H2.5A	H2.5A*						



DATE: 2021-12-02

2ND FLOOR



FROM PLAN DATED: 2021/10

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: TH-1 NAPA 1 **ELEVATION: B** MOD

LOT:

CITY: BRADFORD

SALESMAN: RICK DICIANO

DESIGNER: AJ **REVISION:**

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH **BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft² TILE LOAD: 20.0 lb/ft2

SUBFLOOR: 3/4" GITTED AND MAILED

MORDIC

NORDIC JOIST NS-Gl33 **■**◆■

Engineered Wood Products BASIC INSTALLATION **GUIDE FOR** RESIDENTIAL

NORDIC **U**JOIST

NORDIC STRUCTURES

NAIL SPACING

FLOORS

nordic.ca

1 x 2-5/16 Minimum width

NSTALLING NORDIC I-JOISTS

- Installation of Nordic Ligists shall be as shown in details
- Except for cutting to length, I-joist flanges should never be cut, drilled or notched
- Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment
- bottom flange with the exception of light loads, such as ceiling fans or light fixtures.
- I-joists must not be used in applications where they will be permanently exposed to weather, or will reach a moisture content of 15 percent or greater such as in swimming pool or hot tub areas. They must not be installed where they will remain in direct contact with
- End bearing length must be at least 1-3/4 inch. For multiple-span joists, intermediate bearing length must be at least 3-1/2 inches
- Ends of floor joists shall be restrained to prevent rollover. Use nm board or I-joist blocking panels.
- I-joists installed beneath bearing walls perpendicular to the joists shall have full-depth blocking panels, rim board, or squash blocks
- 0. For I-joists installed directly beneath bearing walls parallel to the joists or used as rim board or blocking panels, the maximum vertical loa using a single I-joist is 3,300 pff, and 6,600 pff if double I-joists are used.
- Continuous lateral support of the I-joist's compression flance is required to prevent rotation and buckling. In simple span uses, lateral support of the top flange is normally supplied by the floor sheathing. In multiple-span or cantilever applications, bracing of the L-joist's bottom flange is also required at interior supports of multiple-span joists, and at the end support next to the cantilever extension. The
- ends of all cantilever extensions must be laterally braced as shown in details 3, 4, or 5, with the applicable building code requirements or approved building plans, but should not be closer than those specified on page 3,3 of
- the Nordic Joist Technical Guide (NS-GT3).
- other fastener requirements, see the applicable building code, 4. For proper temporary bracing of wood I-joists and placement of temporary construction loads, see APA Technical Note. Temporary Construction Loads over I-Joist Roofs and Floors. Form J735.

NORDIC I-JOIST SERIES





1p

and sheathed.



SAFETY AND CONSTRUCTION PRECAUTIONS

Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and r cross-bridging at joist ends. When I-joists are applied continuous over interior supports

and a load-bearing wall is planned at that location, blocking will be required at the interior

When the building is completed, the floor sheathing will provide lateral support for the top

or temporary sheathing must be applied to prevent I-joist rollover or buckling.

end of each bay. Lap ends of adjoining bracing over at least two I-joists.

system. Then, stack building materials over beams or walls only.

flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts

Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced.

no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2-inch

Or sheathing (temporary or permanent) can be nailed to the top flance of the first 4 feet

For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels

Install and fully nail permanent sheathing to each I-joist before placing loads on the floor

mproper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure

use web stiffeners when required can result in serious accidents. Follow these installation

nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the

Avoid Accidents by Following these Important Guidelines



RIM BOARDS Width Length

V3

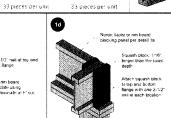
Never stack building

RESIDENTIAL SERIES

N1-40x

3/8 in, web

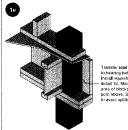


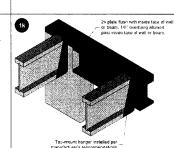


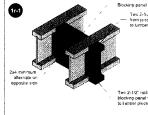
of I-joists at the end of the bay.

rim board, or cross-bridging.

. Never install a damaged I-joist.







8a

Rim board



Flange width (in.) Net depth (in.) Filler block size (in.)

	Zio i sie c. sii sinadang	
-1/8 to 2-1/4 x 8	2x6 + 5/F" or 3/4" shealling	
1/8 to 2-1/4 x 10	2x10 + 5/6" or 3'4" sheathing	
1/8 to 2-1/4 x 12	2x12 + 5/6" or 3:4" sheathing	
3 x 6	2 x 2x6	-
3 x 8	2 > 2×8	
3 x 10	2 < 2<10	
3 x 12	2 x 2x12	
be different from	that specified in the table.	

In some local codes, blocking parels are presentablely required in the first own stepres or the ran second just spaces, not to the states of books and code just space or the results of books and the recommendate of use delicating parels spaced as the code of the recommendate of use delicating parels spaced as the code of the recommendate of use delicating parels spaced as the code of the recommendate of use delicating parels spaced as the code of the recommendate of use delicating parels spaced as recommendated parels and recommendated parels are recommendated on the parel spaced of the recommendated parels and excluding designer. Where blocking parels are required between adjacent joint, or the obtaining the recommendated as the recommendate as the recommendated as the recommendated as the recommendate as the



WEB HOLES AND OPENINGS

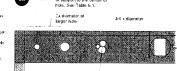
WEB HOLES IN I-JOISTS

- A 1-1/2 inchibble or smaller can be

TABLE 6.1 - LOCATION OF WEB HOLES

Simple or multiple span

- concentrated loads from building material



DUCT CHASE OPENINGS

Rules for Culting Duct Chase Openings in Hoists

- The distance between the inside edge of the support and the controlling of a duct chase opening shall be in compliance with the requirements of Table 5.2.
- . Igoist top and bottom flanges must never be cut, notched or otherwise modifie te maximum death of a duct chase opening that can be out into an ligaist ab shall equal the clear distance between the finance of the Union more web shall equal the clear distance between the flanges of it 1/4 inch. A minimum of 1/8 inch should always be maintains top or bottom of the opening and the adjacent Hoist flange.
- Limit one maximum-size duct chase opening per span.



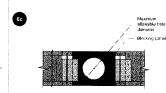
Minimum 1/6" space between top or bottom flange and open

10-3/4

HOLES IN BLOCKING PANELS Maximum Allowable Hole Size in Lateral-restraint-only Blocking Panel

- The maunium slowable hole size for a lateral-restrant-only, blocking panel is 27.5 if the lessed dimension of the blocking's dipth or length, Assuming the blocking panel is longer than its height for depth; the lable aside applies. For other applications, contact Nordic Structures.
- Holes cut into the blocking panels are subject to the following limitations

- While round holes are preferred i rectangle holes may be used provided the comers are not over out, Slightly rounding corners or pre-drilling come with a 1-inch-diameter bit is recommended.



l-joist or rim poard blocking depth (in.) 9-1/2 11-7/8 14 16 Maximum alloxable tick-diameter i	Maximum allowable hole diameter (in.) ⁱⁿ			
9-1/2	5-1-4			
11-7/8	7-3:4			
14	9-1/4			
16	10-162			

TABLE 6.2 - LOCATION OF DUCT CHASE OPENINGS

Simple span

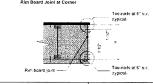


	NI-80	5-3	5-8	6'-0"	6.2	6-10	7-3	7'-8"	8-2
	NI-20	5'-9"	6'-2"	6'-6"	14.5	-		~	100
	NI-40x	68	7.2	7'-6"	8'-1"	86.	9'-1"	9'-6"	
11-7/8	NI-80	7'-3"	7'-8"	8 -0"	8-61	80	9-3*	9:-9:	
	NI-80	7'-2"	7-7	8 -0"	8-5	8'-10"	9.3	9'-8"	10-
	NE-90	7'-6"	7:31	8'-4"	8'-9"	9'-2"	9.7	10:-11	10%
	NL40x	8 -1"	8-7	9 -0"	9'-6"	10"-1"	10-7*	11'-2"	1534
14"	NI-60	8'-9"	9'-3"	9'-8"	10'-11"	10'-6"	11'-1"	11'-6"	
14	NI-80	90	97-3"	99	10-1	10'-7"	11-17	11'-6"	124
	NI-80	9"-2"	9'-8"	10'-0"	10'-6'	10"-11"	111-5	11'-9"	12
	NI-60	10'-3"	10'-8"	11'-2"	11'-6"	12'-1"	12'-8"	13'-2"	
16"	NI-80	10'-4"	10'-9"	11'-3"	11'-0"	12'-1"	12'-7"	13-1"	134
	NI-90	10'-9"	11'-2"	11'-8"	12'-0"	12'-6"	13-0"	13'-6"	141-

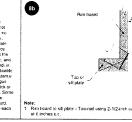
RIM BOARDS

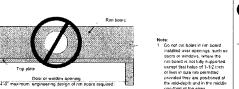
One 2-1/2" mail top

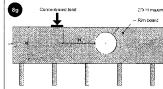


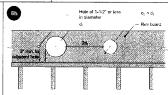




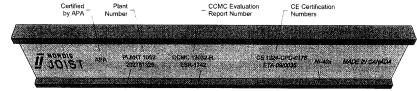






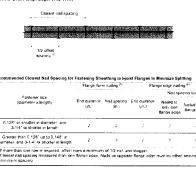


81

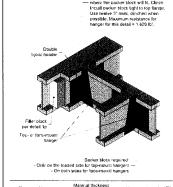


REVIEWED

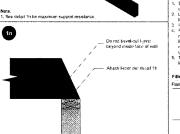
I-ioist Series







Flange width (in.)	Material thickness required (et.) (c)	Minimum depth (in.) 16	
2-1/2	1	5-1/2	
3-1/2	1-1/2	7-1/4	



connection. Leave a 1/8-inch to 1/4-inch gap between top of filler block and bottom of to blaint flange.

Filter block is inquired between joids for full fength of spain.

For flange width of 2-1/2 incines, nail justs bigether with two trons of 3-inch
nails at 12 inches, or, (Linches when possible) on each side of the double
light (total of four natis per foot). For flange width of 3-1/2 inches use two
rows of 3-inch halls at 8 inches or, on each side of the double-light
light (total of four natis per foot).

or the latest version, consult nordic.ca

construction details



COMPANY Nov. 9, 2021 08:40 PROJECT
J3 1ST FLOOR.wwb

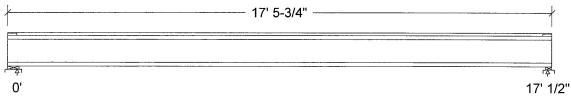
Design Check Calculation Sheet

Nordic Sizer - Canada 8.0

Loads:

Load	Туре	Distribution	Pat-	Location [ft]		Magnituc	de	Unit
			tern	Start	End	Start	End	
Load1	Dead	Full Area				15.00		psf
Load2	Live	Full Area				40.00		psf

Maximum Reactions (lbs) and Support Bearing (in):



<u> </u>		
Unfactored:		
Dead	170	170
Live	454	454
Factored:		
Total	895	895
Bearing:		
Capacity		
Joist	2336	2102
Support	7735	3981
Des ratio	i 1	i
Joist	0.38	0.43
Support	0.12	0.22
Load case		#2
Length	4-3/8	2-3/8
Min req'd	1-1/2	1-1/2
Stiffener	No	No
KD	1.00	1.00
KB support	1.00	1.00
fcp sup	769	769
Kzcp sup	1.15	 1.09

*Minimum bearing length for joists is 1-1/2" for exterior supports

Nordic Joist 11-7/8" NI-40x Floor joist @ 16" o.c.

Supports: All - Lumber Sill plate, No.1/No.2

Total length: 17' 5-3/4"; Clear span: 16' 11"; 3/4" nailed and glued OSB sheathing

This section PASSES the design code check.





J3 1ST FLOOR.wwb

Nordic Sizer - Canada 8.0

Page 2

Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 895	Vr = 2336	lbs	Vf/Vr = 0.38
Moment(+)	Mf = 3811	Mr = 6255	lbs-ft	Mf/Mr = 0.61
Perm. Defl'n	0.09 = < L/999	0.57 = L/360	in	0.17
Live Defl'n	0.25 = L/817	0.43 = L/480	in	0.59
Total Defl'n	0.34 = L/594	0.85 = L/240	in	0.40
Bare Defl'n	0.31 = L/659	0.57 = L/360	in	0.55
Vibration	Lmax = 17'-0.5	Lv = 18'-1.3	ft	0.94
Defl'n	= 0.031	= 0.037	in	0.83

Additional Data:

FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#
Vr	2336	1.00	1.00	_	_	_	_	_	#2
Mr+	6255	1.00	1.00	-	1.000	_	_	_	#2
ΕI	371.1 n	million	_	_	_	_	-	_	#2

CRITICAL LOAD COMBINATIONS:

Support 2 - LC #2 = 1.25D + 1.5L Load Types: D=dead L=live(use,occupancy)

Load Patterns: s=S/2 L=L+Ls _=no pattern load in this span All Load Combinations (LCs) are listed in the Analysis output

 $EIeff = 459.76 lb-in^2 K = 6.18e06 lbs GA = 0.77e06 lb$

CALCULATIONS:

"Live" deflection is due to all non-dead loads (live, wind, snow...) CONFORMS TO ONE 2012

Design Notes:

AMENDED 2020

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. Allowable vibration-controlled span as per the Concluding Report, Development of Design Procedures for Vibration Controlled Spans using Engineered Wood Members, CWC et al for CCMC, 1997.
- 7. Floor vibration design from the CCMC Concluding Report (1997) on vibration controlled spans for engineered wood products.
- 8. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.

OWG NO. TAM 25900-21
STRUCTURAL
COMPONENT ONLY





1ST FLR FRAMING\Flush Beams\B1(i452) (Flush Beam)

Dry | 1 span | No cant.

BC CALC® Member Report Build 7773

November 9, 2021 11:10:31

PASSED

Job name:

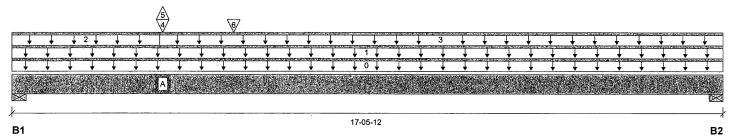
Address: City, Province, Postal Code: File name:

TH-1 EL A.mmdi Description: 1ST FLR FRAMING\Flush Beams\B1(i452)

Wind

Specifier:

Customer: Code reports: CCMC 12472-R Designer: Company:



Total Horizontal Product Length = 17-05-12

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 2-3/8"	1160 / 0	709 / 0
B2, 4-3/8"	492 / 0	359 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-05-12	Тор		12			00-00-00
1	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	17-05-12	Тор	9	5			n\a
2	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	03-07-08	Тор	6	3			n\a
3	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	03-07-08	17-05-12	Тор	18	9			n\a
4	B2(i449)	Conc. Pt. (lbs)	L	03-08-06	03-08-06	Top	1192	627			n\a
5	B2(i449)	Conc. Pt. (lbs)	L	03-08-06	03-08-06	Top	0				n\a
6	STAIR	Conc. Pt. (lbs)	L	05-05-04	05-05-04	Top	37	18			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	9047 ft-lbs	35392 ft-lbs	25.6%	1	03-08-07
End Shear	2571 lbs	14464 lbs	17.8%	1	01-02-04
Total Load Deflection	L/672 (0.304")	n\a	35.7%	6	07-11-05
Live Load Deflection	L/1104 (0.185")	n\a	32.6%	8	07-11-05
Max Defl.	0.304"	n\a	n\a	6	07-11-05
Span / Depth	17.2				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	2-3/8" x 3-1/2"	2627 lbs	51.4%	25.9%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 3-1/2"	1186 lbs	12.6%	6.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

CONFORMS TO OBC 2012

Design meets Code minimum (L/360) Live load deflection criteria.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 13-04-02.

Resistance Factor phi has been applied to all presented results per CSA O86.



198 NO. TAM25901 -21 STRUCTURAL COMPONENT ONLY







1ST FLR FRAMING\Flush Beams\B1(i452) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

> File name: TH-1 EL A.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B1(i452)

Specifier: Designer:

Company:

City, Province, Postal Code: Customer:

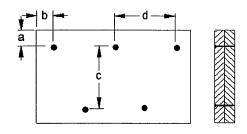
Code reports:

Job name:

Address:

CCMC 12472-R

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3" c = 7-7/8" d = 🕶 8"

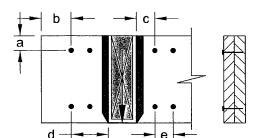
Connectors are:

ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Connection Tag: A

Applies to load tag(s): 4+5



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

e minimum = 4"

Connectors are: 16d

3½" ARDQX SPIRAL



STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,







1ST FLR FRAMING\Flush Beams\B2(i449) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name:

Address:

Customer: Code reports:

City, Province, Postal Code:

TH-1 EL A.mmdl

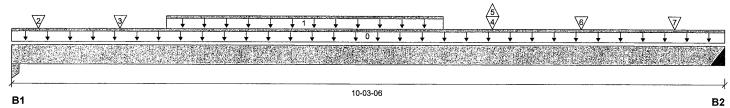
Wind

File name: Description: 1ST FLR FRAMING\Flush Beams\B2(i449)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 10-03-06

Snow

Reaction Summary (Down / Uplift) (lbs)

CCMC 12472-R

Bearing	Live	Dead
B1, 1-3/4"	675 / 0	369 / 0
B2, 2"	1198 / 0	630 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-03-06	Тор		6			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	02-02-10	06-02-10	Top	110	55			n∖a
2	J7(i447)	Conc. Pt. (lbs)	L	00-04-10	00-04-10	Top	86	43			n∖a
3	J7(i460)	Conc. Pt. (lbs)	L	01-06-10	01-06-10	Top	131	66			n\a
4	J5(i307)	Conc. Pt. (lbs)	L	06-11-03	06-11-03	Top	300	150			n\a
5	J5(i307)	Conc. Pt. (lbs)	L	06-11-03	06-11-03	Top	0				n∖a
6	J5(i317)	Conc. Pt. (lbs)	L	08-02-10	08-02-10	Top	497	248			n∖a
7	J5(i329)	Conc. Pt. (lbs)	L	09-06-10	09-06-10	Top	418	209			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4669 ft-lbs	17696 ft-lbs	26.4%	1	06-11-03
End Shear	2195 lbs	7232 lbs	30.4%	1	09-01-08
Total Load Deflection	L/999 (0.124")	n\a	n\a	6	05-04-09
Live Load Deflection	L/999 (0.081")	n\a	n\a	8	05-04-09
Max Defl.	0.124"	n\a	n\a	6	05-04-09
Span / Depth	10.2				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	1-3/4" x 1-3/4"	1473 lbs	59.2%	39.4%	Unspecified
B2	Hanger	2" x 1-3/4"	2585 lbs	n\a	60.5%	HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

OWG NO. TAM 25902-2 STRUCTURAL







1ST FLR FRAMING\Flush Beams\B2(i449) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report

Build 7773 Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

File name: TH-1 EL A.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B2(i449)

Specifier:

Designer: Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-02-01.

CONFORMS TO OBC 2012 AMENDED 2020



Disclosure

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BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®. BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





BC CALC® Member Report

City, Province, Postal Code:



Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

1ST FLR FRAMING\Flush Beams\B3(i461) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

Build 7773 Job name:

Address:

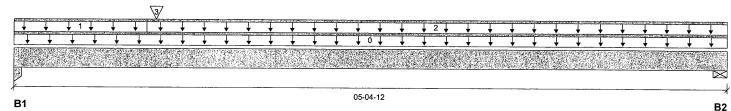
TH-1 EL A.mmdl

Wind

File name: Description: 1ST FLR FRAMING\Flush Beams\B3(i461)

Specifier:

Customer: Code reports: CCMC 12472-R Designer: Company:



Total Horizontal Product Length = 05-04-12

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead
B1, 3-1/2"	321 / 0	185 / 0
B2, 4-3/8"	135 / 0	86 / 0

Lo	ad Summary						Live	Dead
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65
0	Self-Weight	Unf. Lin. (lb/ft)	Ĺ	00-00-00	05-04-12	Тор		6
1	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	01-00-00	Тор	11	5
2	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	01-00-00	05-04-12	Тор	30	15
3	B5(i458)	Conc. Pt. (lbs)	L	01-00-14	01-00-14	Top	314	167

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	583 ft-lbs	17696 ft-lbs	3.3%	1	01-00-13
End Shear	553 lbs	7232 lbs	7.6%	1	01-03-06
Total Load Deflection	L/999 (0.003")	n\a	n\a	4	02-05-14
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	02-05-14
Max Defl.	0.003"	n\a	n\a	4	02-05-14
Span / Depth	4.9				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	3-1/2" x 1-3/4"	713 lbs	14.3%	9.5%	Unspecified
B2	Wall/Plate	4-3/8" x 1-3/4"	310 lbs	6.6%	3.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 03-10-10.



PONCE OF

Wind

1.15

Tributary

00-00-00 n∖a

ROTE n\a

Snow

1.00

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a CONFORMS TO OBC 2012 qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

> BC CALC®, BC FRAMER® , AJS $^{\text{\tiny{M}}}$ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,







1ST FLR FRAMING\Flush Beams\B4(i456) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report **Build 7773**

Job name:

Customer:

Address:

City, Province, Postal Code:

Code reports:

TH-1 EL A.mmdl

Wind

1ST FLR FRAMING\Flush Beams\B4(i456)

Dead

0.65

6 10 Snow

1.00

Wind

1.15

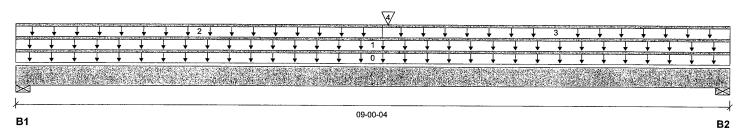
Tributary

00-00-00

Description: Specifier:

File name:

Designer: Company:



Total Horizontal Product Length = 09-00-04

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 2-3/8"	264 / 0	163 / 0
B2, 4-3/8"	338 / 0	202 / 0

CCMC 12472-R

Loa	d Summary						Live
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-00-04	Тор	
1	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	09-00-04	Тор	20
2	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	04-07-08	Тор	3
3	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	04-07-08	09-00-04	Тор	24
4	B5(i458)	Conc. Pt. (lbs)	L	04-08-06	04-08-06	Тор	307

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2128 ft-lbs	17696 ft-lbs	12.0%	1	04-08-06
End Shear	625 lbs	7232 lbs	8.6%	1	07-08-00
Total Load Deflection	L/999 (0.035")	n\a	n\a	4	04-06-03
Live Load Deflection	L/999 (0.022")	n\a	n\a	5	04-06-03
Max Defl.	0.035"	n\a	n\a	4	04-06-03
Span / Depth	8.7				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	2-3/8" x 1-3/4"	600 lbs	23.5%	11.8%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 1-3/4"	760 lbs	16.2%	8.2%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 04-05-02.



106 NO. FAM 25904-21 STRUCTURAL COMPONENT ONLY **Disclosure**

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1ST FLR FRAMING\Flush Beams\B5(i458) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

Build 7773

Job name:

File name:

TH-1 EL A.mmdl

Wind

1ST FLR FRAMING\Flush Beams\B5(i458)

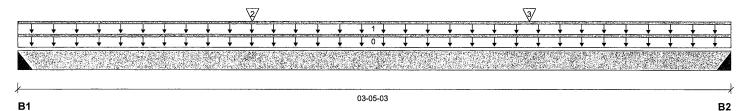
Address: City, Province, Postal Code:

BC CALC® Member Report

Description:

Specifier:

Customer: Code reports: Designer: Company:



Total Horizontal Product Length = 03-05-03

Snow

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead
B1, 2"	307 / 0	164 / 0
B2, 2"	314 / 0	167 / 0

CCMC 12472-R

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-05-03	Тор		6			00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-00-00	03-05-03	Top	120	60			n\a
2	J7(i457)	Conc. Pt. (lbs)	L	01-01-09	01-01-09	Top	108	54	1111	30.000 - 10.00 田花戲歌詞	.; ∈ n\a
3	J7(i455)	Conc. Pt. (lbs)	L	02-05-09	02-05-09	Top	101	50	080	The second second	∕ ∖ n\a
			F4	_	11	•		å	101	^ / ·	

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	553 ft-lbs	17696 ft-lbs	3.1%	1	01-07-12
End Shear	355 lbs	7232 I bs	4.9%	1	01-01-14
Total Load Deflection	L/999 (0.002")	n\a	n\a	4	01-08-10
Live Load Deflection	L/999 (0.001")	n\a	n\a	5	01-08-10
Max Defl.	0.002"	n\a	n\a	4	01-08-10
Span / Depth	3.3				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 1-3/4"	665 lbs	n\a	15.6%	HUS1.81/10
B2	Hanger	2" x 1-3/4"	680 lbs	n\a	15.9%	HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for, adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO DEC 2012

Hanger Manufacturer: Unassigned

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-01-08.

OWO NO. TAM 25905-21 STRUCTURAL COMPONENT ONLY

Not of Shift

Disclosure

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1ST FLR FRAMING\Flush Beams\B6(i451) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

Build 7773

BC CALC® Member Report

Job name:

Address:

Customer: Code reports:

City, Province, Postal Code:

CCMC 12472-R

File name:

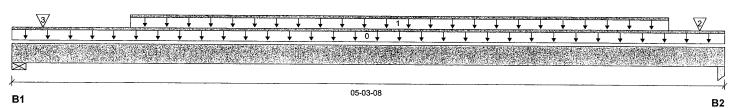
TH-1 EL A.mmdl

1ST FLR FRAMING\Flush Beams\B6(i451)

Wind

Description: Specifier:

Designer: Company:



Total Horizontal Product Length = 05-03-08

Snow

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead			
B1, 10"	350 / 0	230 / 0			
B2, 3-1/2"	250 / 0	140 / 0			

Lo: Tag	ad Summary Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	05-03-08	Тор		6			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-10-08	04-10-08	Top	110	55			n\a
2	J7(i429)	Conc. Pt. (lbs)	L	05-01-04	05-01-04	Top	33	17			n\a
3	2(i128)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	Тор	126	100	عشفاء د		

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	605 ft-lbs	17696 ft-lbs	3.4%	1	02-10-08
End Shear	407 lbs	7232 lbs	5.6%	1	04-00-02
Total Load Deflection	L/999 (0.003")	n\a	n\a	4	02-10-15
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	02-10-15
Max Defl.	0.003"	n\a	n\a	4	02-10-15
Span / Depth	4.3				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	10" x 1-3/4"	812 lbs	7.5%	3.8%	Spruce-Pine-Fir
B2	Column	3-1/2" x 1-3/4"	551 l bs	11.1%	7.4%	Unspecified

UWB NO.TAM 25900-21 STRUCTURAL COMPONENT ONLY

SOURCE OF

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-01-08.

Disclosure

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1ST FLR FRAMING\Flush Beams\B7L(i426) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report

Build 7773

Job name:

Customer:

Code reports:

Address: City, Province, Postal Code:

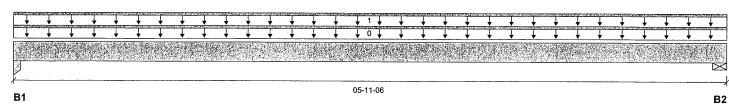
TH-1 EL A.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B7L(i426)

Specifier: Designer:

File name:

CCMC 12472-R Company:



Total Horizontal Product Length = 05-11-06

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	
B1, 3-1/2"	78 / 0	53 / 0	
B2, 4-3/8"	80 / 0	55 / 0	

Loa	Load Summary							Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	05-11-06	Top		5			00-00-00
1	FC3 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	05-11-06	Тор	27	13			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	230 ft-lbs	11610 ft-lbs	2.0%	1	02-11-04
End Shear	116 lbs	5785 lbs	2.0%	1	01-01-00
Total Load Deflection	L/999 (0.003")	n\a	n\a	4	02-11-04
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	02-11-04
Max Defl.	0.003"	n\a	n\a	4	02-11-04
Span / Depth	6.8				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	3-1/2" x 1-3/4"	184 lbs	3.7%	2.5%	Unspecified
B2	Wall/Plate	4-3/8" x 1-3/4"	189 lbs	4.0%	2.0%	Spruce-Pine-Fir

OWE NO. TAM 25907 -21 STRUCTURAL COMPONENT ONLY

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 05-05-04.



CONFORMS TO OBC 2012

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1ST FLR FRAMING\Flush Beams\B8L(i436) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report **Build 7773**

Job name:

Address:

City, Province, Postal Code: Customer:

Code reports:

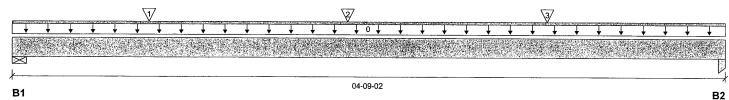
TH-1 EL A.mmdl

File name: Description: 1ST FLR FRAMING\Flush Beams\B8L(i436)

Wind

Specifier:

Designer: Company:



Total Horizontal Product Length = 04-09-02

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	
B1, 4-3/8"	232 / 0	128 / 0	
B2, 1-3/4"	202 / 0	111 / 0	

CCMC 12472-R

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-09-02	Тор		5			00-00-00
1	J2(i425)	Conc. Pt. (lbs)	L	00-10-14	00-10-14	Top	124	62			n\a
2	J2(i423)	Conc. Pt. (lbs)	L	02-02-14	02-02-14	Top	155	77			n\a
3	J2(i418)	Conc. Pt. (lbs)	L	03-06-14	03-06-14	Тор	155	77	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	30.11.	n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	618 ft-lbs	11610 ft-lbs	5.3%	1	02-02-14
End Shear	436 lbs	5785 lbs	7.5%	1	03-09-14
Total Load Deflection	L/999 (0.006")	n\a	n\a	4	02-05-14
Live Load Deflection	L/999 (0.004")	n\a	n∖a	5	02-05-14
Max Defl.	0.006"	n\a	n∖a	4	02-05-14
Span / Depth	5.5				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4-3/8" x 1-3/4"	508 lbs	10.8%	5.4%	Spruce-Pine-Fir
B2	Column	1-3/4" x 1-3/4"	442 lbs	17.8%	11.8%	Unspecified

ONG NO. TAM 25908-21 STRUCTURAL COMPONENT ONLY

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-01-08.

Disclosure

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2ND FLR FRAMING\Flush Beams\B10(i342) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name:

Address:

City, Province, Postal Code: Customer:

TH-1 EL A.mmdl

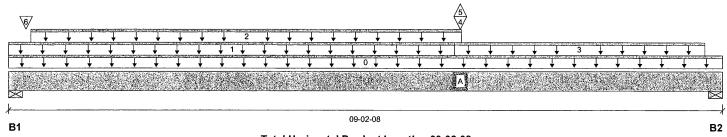
Wind

File name: Description: 2ND FLR FRAMING\Flush Beams\B10(i342)

Specifier:

Designer:

Code reports: CCMC 12472-R Company:



Total Horizontal Product Length = 09-02-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 3-1/2"	478 / 37	536 / 0
B2, 5-1/2"	848 / 70	583 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-02-08	Top		12			00-00-00
1	FC4 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	05-09-00	Тор	17	8			n\a
2	WALL	Unf. Lin. (lb/ft)	L	00-03-08	05-10-01	Top		60			n∖a
3	FC4 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	05-09-00	08-11-12	Тор	29	14			n\a
4	B17(i395)	Conc. Pt. (lbs)	L	05-09-14	05-09-14	Top	1138	557			n\a
5	B17(i395)	Conc. Pt. (lbs)	L	05-09-14	05-09-14	Top	-107				n\a
6	E24(i294)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	Top		24			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	5593 ft-lbs	35392 ft-lbs	15.8%	1	05-09-14
End Shear	1904 lbs	14464 lbs	13.2%	1	07-09-02
Total Load Deflection	L/999 (0.046")	n\a	n\a	6	04-09-04
Live Load Deflection	L/999 (0.026")	n\a	n\a	8	04-10-15
Max Defl.	0.046"	n\a	n\a	6	04-09-04
Span / Depth	8.7				

Bea	ring Supports	Dim. (LxW)	Demand	Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	3-1/2" x 3-1/2"	1388 lbs	18.4%	9.3%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	2000 lbs	16.9%	8.5%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

CONFORMS TO OBC 2012

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 05-05-08.









2ND FLR FRAMING\Flush Beams\B10(i342) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

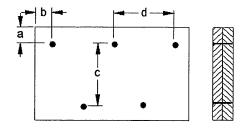
CCMC 12472-R

File name: TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B10(i342)

Specifier: Designer: Company:

Connection Diagram: Full Length of Member



a minimum = 2"

b minimum = 3"

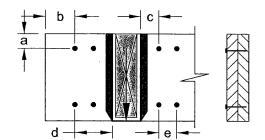
c = 7-7/8" d= 🍅 🛭 "

Connectors are: 3

Nails ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 5+6



a minimum = 2"

b minimum = 4"

c minimum = 4" d maximum = 12"

e minimum = 4"

Connectors are: 16d 🔏 .Nails

3½" ARDOX SPIRAL



148 NO. TAM 25909-2 STRUCTURAL COMPONENT ONLY

Disclosure

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2ND FLR FRAMING\Flush Beams\B11(i373) (Flush Beam)

Dry | 2 spans | R cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

File name:

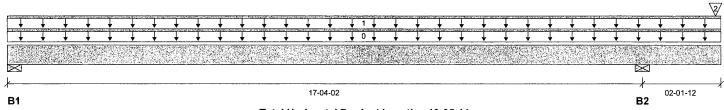
TH-1 EL A.mmdl

Wind

Description: 2ND FLR FRAMING\Flush Beams\B11(i373)

Specifier:

Designer: Company:



Total Horizontal Product Length = 19-05-14

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow
B1, 2-3/8"	175 / 3	39 / 0	0/82
B2, 5-1/2"	218 / 0	1684 / 0	789 / 0

Lo	ad Summary							Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-05-14	Тор	-	12			00-00-00
1	FC4 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	19-05-14	Тор	20	10			n\a
2	-	Conc. Pt. (lbs)	L	19-04-02	19-04-02	Тор		1294	707		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	848 ft-lbs	35392 ft-lbs	2.4%	35	05-10-04
Neg. Moment	-5463 ft-lbs	-11766 ft-lbs	46.4%	37	17-04-02
End Shear	243 lbs	14464 lbs	1.7%	32	01-02-04
Cont. Shear	1840 lbs	9401 lbs	19.6%	0	18-06-12
Total Load Deflection	2xL/1998 (0.082")	n\a	n\a	83	19-05-14
Live Load Deflection	L/999 (-0.049")	n\a	n\a	121	10-00-11
Total Neg. Defl.	L/999 (-0.095")	n\a	n\a	83	10-09-01
Max Defl.	-0.095"	n\a	n\a	83	10-09-01
Span / Depth	17.4				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	2-3/8" x 3-1/2"	311 lbs	6.1%	3.1%	Spruce-Pine-Fir
B1	Uplift		91 lbs			
B2	Wall/Plate	5-1/2" x 3-1/2"	2358 lbs	30.6%	15.4%	Spruce-Pine-Fir

Cautions

Uplift of 90 lbs found at bearing B1.

Concentrated side load(s) 7,12 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.

DWG NO. TAM 25910-21 STRUCTURAL COMPONENT ONLY





2ND FLR FRAMING\Flush Beams\B11(i373) (Flush Beam)

Dry | 2 spans | R cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report

Design meets User specified (2xL/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Build 7773 Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

Notes

CCMC 12472-R

File name: TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B11(i373)

Specifier: Designer:

Company:

AMENDED 2020

CONFORMS TO OBC 2012

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's

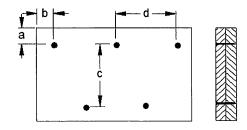
verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 16-11-00.

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 🕶 🗗 🥙

Connectors are:

Nails ARDOX SPIRAL

BWG NO. TAM28910 -21 STRUCTURAL

COMPONENT ONLY

Disclosure

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2ND FLR FRAMING\Flush Beams\B12(i377) (Flush Beam)

Dry | 3 spans | R cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name: Address:

TH-1 EL A.mmdl

Wind

Description: 2ND FLR FRAMING\Flush Beams\B12(i377)

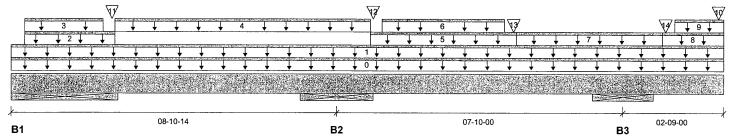
City, Province, Postal Code:

Specifier: Designer:

File name:

Customer: Code reports:

CCMC 12472-R Company:



Total Horizontal Product Length = 19-05-14

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow
B1, 34-15/16"	101 / 5	2355 / 0	4401 / 0
B2, 24"	174 / 0	3001 / 0	5609 / 0
B3. 20"	124 / 0	3246 / 0	5223 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-05-14	Тор		12			00-00-00
1	FC4 Floor Decking (Plan	Unf. Lin. (lb/ft)	L	00-00-00	19-05-14	Top	20	10			n\a
	View Fill)										
2	E25(i293)	Unf. Lin. (lb/ft)	L	00-04-06	02-09-14	Top		81			n\a
3	E25(i293)	Unf. Lin. (lb/ft)	L	00-04-06	02-05-14	Top		322	759		n\a
4	E33(i302)	Unf. Lin. (lb/ft)	L	02-09-14	09-09-14	Top		61			n\a
5	E34(i303)	Unf. Lin. (lb/ft)	L	09-09-14	13-09-14	Top		81			n\a
6	E34(i303)	Unf. Lin. (lb/ft)	L	10-01-14	13-05-14	Top		322	759		n\a
7	E31(i299)	Unf. Lin. (lb/ft)	L	13-09-14	17-09-14	Top		61			n\a
8	E32(i300)	Unf. Lin. (lb/ft)	L	17-09-14	19-05-14	Тор		81			n\a
9	E32(i300)	Unf. Lin. (lb/ft)	L	18-01-14	19-05-14	Тор		322	759		n\a
10	B13(i430)	Conc. Pt. (lbs)	L	19-04-02	19-04-02	Тор		612	333		n\a
11	E25(i293)	Conc. Pt. (lbs)	L	02-08-14	02-08-14	Тор		1261	2918		n\a
12	E34(i303)	Conc. Pt. (lbs)	L	09-10-14	09-10-14	Top		1254	2901		n\a
13	E34(i303)	Conc. Pt. (lbs)	L	13-08-14	13-08-14	Тор		772	1786		n\a
14	E32(i300)	Conc. Pt. (lbs)	L	17-10-14	17-10-14	Тор		759	1756		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	5156 ft-lbs	35392 ft-lbs	14.6%	105	13-00-14
Neg. Moment	-6279 ft-lbs	-10475 ft-lbs	59.9%	101	17-06-14
End Shear	173 lbs	14464 lbs	1.2%	92	03-10-13
Cont. Shear	4885 lbs	14464 lbs	33.8%	101	10-10-12
Total Load Deflection	L/999 (0.019")	n\a	n∖a	249	12-10-06
Live Load Deflection	L/999 (0.013")	n\a	n\a	353	12-10-06
Total Neg. Defl.	L/999 (-0.009")	n\a	n∖a	249	07-02-10
Max Defl.	0.019"	n\a	n\a	249	12-10-06
Span / Depth	6.1				
Dist. Load (B1)	1674.71 lb/ft	57645.1 lb/ft	2.9%		
Dist. Load (B2)	127.28 lb/ft	37469.32 lb/ft	0.3%		
Dist. Load (B3)	99.26 lb/ft	37469.32 lb/ft	0.3%		
Conc. Load (B1)	5953 lbs	16813 lbs	35.4%		



BWG NO . TAM 25911 -21 STRUCTURAL COMPONENT ONLY







2ND FLR FRAMING\Flush Beams\B12(i377) (Flush Beam)

Dry | 3 spans | R cant.

November 9, 2021 11:10:31

PASSED

Build 7773

BC CALC® Member Report

City, Province, Postal Code:

Job name: Address:

Customer:

•

File name: TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B12(i377)

Specifier:

Code reports: CCMC 12472-R

Company:	

Bearii	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	34-15/16" x 3-1/2"	9716 lbs	12.9%	6.5%	Spruce-Pine-Fir
B2	Wall/Plate	24" x 3-1/2"	11832 lbs	22.9%	11.5%	Spruce-Pine-Fir
B3	Wall/Plate	20" x 3-1/2"	12028 lbs	27.9%	14.1%	Spruce-Pine-Fir

Cautions

Concentrated side load(s) 42 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.

) Olys

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

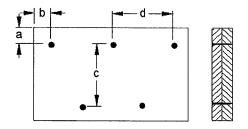
Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 19-00-00.

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 8

Connectors are: 3-4

4

connectors are.

ARDOX SPIRAL



Disclosure

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2ND FLR FRAMING\Flush Beams\B13(i430) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report

Build 7773 Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

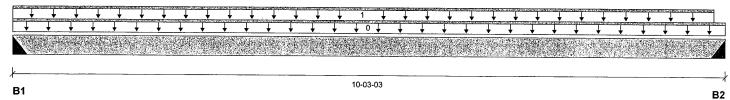
File name:

TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B13(i430)

Specifier: Designer:

Company:



Total Horizontal Product Length = 10-03-03

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 2"		621 / 0	339 / 0	
B2, 2"		603 / 0	328 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-03-03	Тор		12			00-00-00
1	E30(i298)	Unf. Lin. (lb/ft)	L	00-00-00	10-01-03	Тор		109	66		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2140 ft-lbs	23005 ft-lbs	9.3%	0	05-01-09
End Shear	673 lbs	9401 lbs	7.2%	0	01-01-14
Total Load Deflection	L/999 (0.044")	n\a	n\a	12	05-01-09
Live Load Deflection	L/999 (0.016")	n\a	n\a	17	05-01-09
Max Defl.	0.044"	n\a	n\a	12	05-01-09
Span / Depth	10.2				

Bearing	յ Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 3-1/2"	869 lbs	n\a	15.7%	HUC412
B2	Hanger	2" x 3-1/2"	844 lbs	n\a	15.2%	HUC412

Cautions

Header for the hanger HUC412 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUC412 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBE 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-09-08.

VACE OF ON

OWG NO. TAM 25912-21 STRUCTURAL COMPONENT ONLY

REVIEW

>-



Build 7773 Job name:

Address:

BC CALC® Member Report

City, Province, Postal Code:



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2ND FLR FRAMING\Flush Beams\B13(i430) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

File name: TH-1 EL A.mmdl

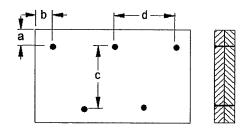
Description: 2ND FLR FRAMING\Flush Beams\B13(i430)

Specifier: Designer:

Company:

Customer: Code reports: CCMC 12472-R

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3" c = 7-7/8" d = 🕶 🔞 "

Connectors are:

Nails

ARDOX SPIRAL

OVINOE OF ON 046 NO. TAM 2591221 STRUCTURAL COMPONENT ONLY

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2ND FLR FRAMING\Flush Beams\B14(i424) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

B2

Build 7773

Job name:

Address:

City, Province, Postal Code: Customer:

BC CALC® Member Report

Code reports:

B1

CCMC 12472-R

File name:

TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B14(i424)

Wind

Specifier:

Designer: Company:

Total Horizontal Product Length = 10-09-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	
B1, 2"		650 / 0	355 / 0	
B2. 2"		650 / 0	355 / 0	

	ad Summary						Live	Dead	Snow	Wind	Tributary
lag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-09-00	Тор		12			00-00-00
1	E30(i298)	Unf. Lin. (lb/ft)	L	00-00-00	10-09-00	Top		109	66		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2351 ft-lbs	23005 ft-lbs	10.2%	0	05-04-08
End Shear	714 lbs	9401 l bs	7.6%	0	01-01-14
Total Load Deflection	L/999 (0.053")	n\a	n\a	12	05-04-08
Live Load Deflection	L/999 (0.019")	n\a	n\a	17	05-04-08
Max Defl.	0.053"	n\a	n\a	12	05-04-08
Span / Depth	10.7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 3-1/2"	910 lbs	n\a	16.4%	HUC412
B2	Hanger	2" x 3-1/2"	910 lbs	n\a	16.4%	HUC412

Cautions

Header for the hanger HUC412 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUC412 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBE 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-09-08.

Octor Old

DWG NO. TAM25913 -21 STRUCTURAL







2ND FLR FRAMING\Flush Beams\B14(i424) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

Build 7773

BC CALC® Member Report

Job name:

Address:

Code reports:

City, Province, Postal Code: Customer:

CCMC 12472-R

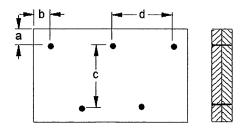
File name: TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B14(i424)

Specifier:

Designer: Company:

Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8" d = 208 11 b minimum = 3"

Connectors are:

Nails 1 -

ARDOX SPIRAL

5WG NO. TAM 25913=21 STRUCTURAL COMPONENT ONLY

Disclosure

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2ND FLR FRAMING\Flush Beams\B15(i414) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773 Job name:

Dry | 1 span | No cant.

November 9, 2021 11:10:31

File name:

Description: 2ND FLR FRAMING\Flush Beams\B15(i414)

TH-1 EL A.mmdl

City, Province, Postal Code:

Specifier:

Customer:

Address:

Designer:

Code reports:

Company:

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T T	Ţ	Ţ	Taxable Control	Ţ.	ļa.	Ţ	**************************************	Zmarijako	Ţ	-710-00-7	CONTRACTOR	Ţ	¥		Ţ	↓ o	*	Ţ	70. S 10. S	\	Ţ	3-0-1	(Paradi)	Ţ	Ţ	Ţ	Ţ	¥	<u> </u>	<u></u>	PER PER	Ţ	1
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⊴																					241 200												7.01
08-11-13										-																							
1																																	

Total Horizontal Product Length = 08-11-13

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wi
B1, 3-1/2"		551 / 0	301 / 0	
B2, 2"		536 / 0	292 / 0	

CCMC 12472-R

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-11-13	Top		12			00-00-00
1	E30(i298)	Unf. Lin. (lb/ft)	L	00-00-00	08-11-13	Top		109	66		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1585 ft-lbs	23005 ft-lbs	6.9%	0	04-06-11
End Shear	554 lbs	9401 lbs	5.9%	0	01-03-06
Total Load Deflection	L/999 (0.024")	n\a	n\a	12	04-06-11
Live Load Deflection	L/999 (0.009")	n∖a	n\a	17	04-06-11
Max Defl.	0.024"	n\a	n\a	12	04-06-11
Span / Depth	8.7				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Resistance Member	Material
B1	Wall/Plate	3-1/2" x 3-1/2"	771 l bs	15.7%	7.9%	Spruce-Pine-Fir
B2	Hanger	2" x 3-1/2"	750 l bs	n\a	13.5%	HUC412

Cautions

Header for the hanger HUC412 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUC412 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-02-01.

DWB NO. TAM 25914-21 STRUCTURAL

COMPONENT ONLY





2ND FLR FRAMING\Flush Beams\B15(i414) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

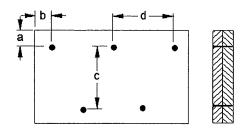
File name: TH-1 EL A.mmdl

ane. HereEAming

Description: 2ND FLR FRAMING\Flush Beams\B15(i414)

Specifier: Designer: Company:

Connection Diagram: Full Length of Member



a minimum = 2"

b minimum = 3"

c = 7-7/8" d = 886

Connectors are:

ļ

Nails

3½" ARDOX SPIRAL

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Disclosure

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BC CALC® Member Report



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2ND FLR FRAMING\Flush Beams\B16(i401) (Flush Beam) Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

Build 0

Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

File name:

TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B16(i401)

Specifier:

Designer:

Company:

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		10.0				1				1 4 9 9 74				24.7					100									建設		

Total Horizontal Product Length = 13-10-08 FULLY SUPPORTED BOTTOM ENGE

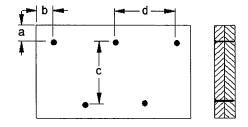
Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-10-08	Тор		12			00-00-00
1	E24(i294)	Unf. Lin. (lb/ft)	L	00-00-00	13-10-08	Top		81			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location		
Dist. Load	113.26 lb/ft	37469.32 lb/ft	0.3%			_	
Conc. Load	0 lbs	16813 lbs	n\a	ŧ	CONFORMS	ŤÖ	0BC 2012

Notes

Calculations assume member is fully braced.

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" $d = \mathcal{B}^{\ell\ell}$

Connectors are:

Nails

ARDOX SPIRAL

Wite of

SWB NO. TAM 289/5-21 STRUCTURAL COMPONENT ONLY

Disclosure

AMENDED 2020

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2ND FLR FRAMING\Flush Beams\B17(i395) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

File name:

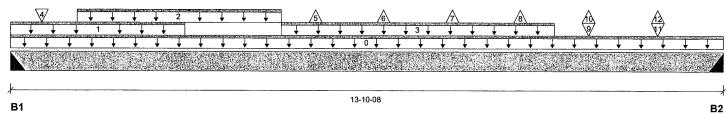
TH-1 EL A.mmdl

Wind

Description: 2ND FLR FRAMING\Flush Beams\B17(i395)

Specifier:

Designer: Company:



Total Horizontal Product Length = 13-10-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 2"	1137 / 107	556 / 0
B2, 2"	1240 / 158	583 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-10-08	Тор		6			00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-00-00	03-04-09	Top	120	60			n∖a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-03-03	05-03-03	Top	65	32			n\a
3	Smoothed Load	Unf. Lin. (lb/ft)	L	05-03-03	10-07-03	Top	188	71			n∖a
4	J5(i383)	Conc. Pt. (lbs)	L	00-07-03	00-07-03	Top	67	33			n\a
5	J2(i427)	Conc. Pt. (lbs)	L	05-11-03	05-11-03	Top	-61				n\a
6	J2(i422)	Conc. Pt. (lbs)	L	07-03-03	07-03-03	Тор	-61				n\a
7	J2(i420)	Conc. Pt. (lbs)	L	08-07-03	08-07-03	Top	-61				n\a
8	J2(i434)	Conc. Pt. (lbs)	L	09-11-03	09-11-03	Тор	-61				n\a
9	J2(i428)	Conc. Pt. (lbs)	L	11-03-03	11-03-03	Тор	316	153			n\a
10	J2(i428)	Conc. Pt. (lbs)	L	11-03-03	11-03-03	Тор	-10				n\a
11	J2(i419)	Conc. Pt. (lbs)	L	12-07-03	12-07-03	Top	326	158			n\a
12	J2(i419)	Conc. Pt. (lbs)	L	12-07-03	12-07-03	Тор	-11				n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	8384 ft-lbs	17696 ft-lbs	47.4%	1	07-03-03
End Shear	2581 lbs	7232 lbs	35.7%	1	12-08-10
Total Load Deflection	L/407 (0.403")	n\a	58.9%	6	07-01-03
Live Load Deflection	L/594 (0.276")	n\a	60.7%	8	07-01-03
Max Defl.	0.403"	n\a	n\a	6	07-01-03
Span / Depth	13.8				

Bearing	յ Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 1-3/4"	2400 lbs	n\a	56.2%	HUS1.81/10
B2	Hanger	2" x 1-3/4"	2589 lbs	n\a	60.6%	HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

NO. TAM 259/6-21 STRUCTURAL COMPONENT ONLY







2ND FLR FRAMING\Flush Beams\B17(i395) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name: Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

File name: T

TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B17(i395)

Specifier:

Designer:

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

CONFORMS TO OBC 2012 AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-02-01.



DWG NO.TAM 25916-2 STRUCTURAL COMPONENT ONLY

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2ND FLR FRAMING\Flush Beams\B9(i327) (Flush Beam)

Dry | 2 spans | R cant.

November 9, 2021 11:10:31

PASSED

Build 7773

Job name: Address:

City, Province, Postal Code:

BC CALC® Member Report

Customer:

Code reports: CCMC 12472-R

File name:

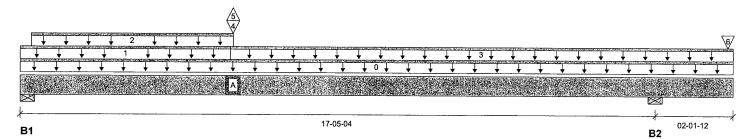
TH-1 EL A.mmdi

Description: 2ND FLR FRAMING\Flush Beams\B9(i327)

Wind

Specifier:

Designer: Company:



Total Horizontal Product Length = 19-07-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow
B1, 3-1/2"	1095 / 112	785 / 0	0 / 77
B2, 5-1/2"	852 / 51	1954 / 0	741 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-07-00	Тор		12			00-00-00
1	FC4 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	05-09-00	Тор	18	9			n\a
2	WALL	Unf. Lin. (lb/ft)	L	00-03-08	05-10-00	Top		60			n\a
3	FC4 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	05-09-00	19-07-00	Тор	43	22			n\a
4	B17(i395)	Conc. Pt. (lbs)	L	05-09-14	05-09-14	Top	1239	583			n\a
5	B17(i395)	Conc. Pt. (lbs)	L	05-09-14	05-09-14	Top	-158				n\a
6	-	Conc. Pt. (lbs)	L	19-05-04	19-05-04	Тор		1213	664		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	12465 ft-lbs	35392 ft-lbs	35.2%	43	05-09-14
Neg. Moment	-5219 ft-lbs	-17596 ft-lbs	29.7%	49	17-05-04
End Shear	2452 lbs	14464 lbs	16.9%	43	01-03-06
Cont. Shear	1742 lbs	9401 lbs	18.5%	0	18-07-14
Total Load Deflection	L/549 (0.376")	n\a	43.7%	102	07-11-04
Live Load Deflection	L/760 (0.272")	n\a	47.4%	151	08-03-02
Total Neg. Defl.	2xL/1998 (-0.104")	n\a	n\a	102	19-07-00
Max Defl.	0.376"	n\a	n\a	102	07-11-04
Span / Depth	17.4				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	3-1/2" x 3-1/2"	2623 lbs	34.8%	17.6%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	4461 lbs	37.7%	19.0%	Spruce-Pine-Fir

Cautions

Concentrated side load(s) 12,17 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.

S. KATSOUBAKOS PE 4

STRUCTURAL COMPONENT ONLY







Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2ND FLR FRAMING\Flush Beams\B9(i327) (Flush Beam)

Dry | 2 spans | R cant.

November 9, 2021 11:10:31

PASSED

BC CALC® Member Report Build 7773

Job name:

Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

File name: TH-1 EL A.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B9(i327)

Specifier:

Designer: Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

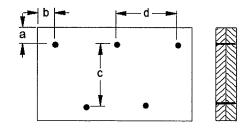
Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 11-03-12.

CONFORMS TO OBC 2012

AMENDED 2020

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = **3** € 4

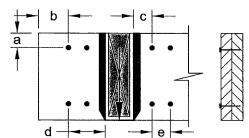
Connectors are:

Nails

ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 5+6



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

e minimum = 4"

Connectors are: 16d / Nails

31/2" ARDOX SPIRAL



STRUCTURAL COMPONENT ONLY

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BC CALC® Member Report



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2ND FLR FRAMING\Flush Beams\B21(i565) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 12:36:39

PASSED

B2

Build 7773 Job name:

Address:
City, Province, Postal Code: BRADFORD

File name:

TH-1 EL B SUNKEN.mmdI

Description: 2ND FLR FRAMING\Flush Beams\B21(i565)

Specifier:

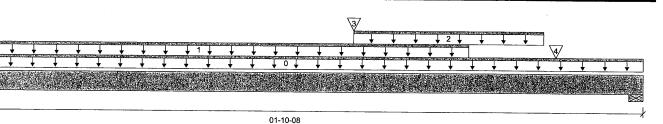
Company:

Designer: A.

Customer: Code reports:

B1

CCMC 12472-R



Total Horizontal Product Length = 01-10-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 3"	42 / 0	115 / 0	55 / 0	
B2, 5-1/2"	109 / 0	177 / 0	69 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	•
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	01-10-08	Top		12			00-00-00
1	E36(i720)	Unf. Lin. (lb/ft)	L	00-00-00	01-05-00	Top		100	66		n\a
2	FC4 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	01-01-06	01-07-06	Тор	6				n\a
3 4	J5(i546) E29(i296)	Conc. Pt. (lbs)	L		01-01-06	Тор	148	74			n\a
4	L23(1230)	Conc. Pt. (lbs)	L	01-07-12	01-07-12	Тор		48	30		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	118 ft-lbs	35392 ft-lbs	0.3%	1	01-01-06
End Shear	63 lbs	9401 lbs	0.7%	0	00-05-02
Span / Depth	1.3				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	3" x 3-1/2"	268 lbs	4.1%	2.1%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	453 lbs	3.8%	1.9%	Spruce-Pine-Fir

Notes

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. CONFORMS TO 05C 2012 Unbalanced snow loads determined from building geometry were used in selected product's verification.

AMENDED 2020

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-08-08.

S. KATSONDAKOS S

STRUCTURAL COMPONENT ONLY





BC CALC® Member Report



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2ND FLR FRAMING\Flush Beams\B21(i565) (Flush Beam)

Dry | 1 span | No cant.

November 9, 2021 12:36:39

PASSED

Build 7773

Job name: Address:

City, Province, Postal Code: BRADFORD

File name: TH-1 EL B SUNKEN.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B21(i565)

Specifier:

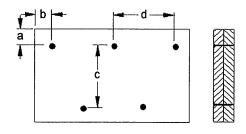
ΑJ

Customer: Code reports:

CCMC 12472-R

Designer: Company:

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 6008(1

Calculated Side Load = 157.3 lb/ft

Connectors are:

Nails

1 ARDOX SPIRAL

> POCE OF CHAP ## NO. TAN 2591821

STRUCTURAL COMPONENT ONLY

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Maximum Floor Spans - S2.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 15 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			В	are			1/2 in. gyp	sum ceiling		
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	-	
9-1/2"	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-	
9-1/2	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-	
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-	
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-	
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-	
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-	
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-	
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-	
	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-	
14"	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-	
14	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-	
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-	
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-	
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	_	
	NI-90	24'-5"	22'-6"	21'-6"	-	25'-1"	23'-2"	22'-2"	_	

		Mi	d-span blocking	with 1x4 inch st	rap	Mid-sp	an blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	e spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-
9-1/2"	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-
9-1/2	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-
	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-2"	-
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	21'-8"	-
14"	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-
14	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
16"	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	_
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	-

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.





Maximum Floor Spans - S4.1

Design Criteria

Spans: Simple span

Loads: Live load = 40 psf and dead load = 15 psf

Deflection limits: L/480 under live load and L/240 under total load

Sheathing: 3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			В	are			1/2 in. gyr	osum ceiling		
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	15'-2"	
9-1/2	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"	
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"	
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-2"	
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"	
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"	
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"	
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"	
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"	
14"	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10'	
14	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"	
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"	
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"	
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10'	
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"	

		Mi	d-span blocking	with 1x4 inch	strap	Mid-sp	an blocking an	d 1/2 in. gypsui	m ceiling	
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"	
	N1-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10'	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"	
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"	
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"	
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"	
	NI-40x	24'-5"	22'-9"	21'-9"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"	
14"	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10'	
14	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"	
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"	
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"	
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"	
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"	

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



Maximum Floor Spans - S6.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 15 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			В	are			1/2 in. gyp	osum ceiling	
Joist depth	Joist series		On cent	re spacing		On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	-
9-1/2"	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-
9-1/2	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	_
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	-
	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17'-4"	16'-9"	-
11-7/8"	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	~
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	_
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	-
	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	-
14"	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	_
14	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	-
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-
	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-
16"	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	-
	NI-90	24'-1"	22'-2"	21'-2"	-	24'-9"	22'-11"	21'-10"	-

		Mi	d-span blocking	with 1x4 inch st	trap	Mid-sp	an blocking and	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On centr	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-
9-1/2"	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	-
9-1/2	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-
	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	-
11-7/8"	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	-
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-
	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	21'-5"	-
14"	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-
14	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	-
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-
	NI-60	26'-2"	24'-3"	23'-2"	-	26'-11"	25'-0"	23'-11"	-
16"	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-
	NI-90	28'-5"	26'-3"	25'-0"	=	29'-0"	26'-11"	25'-8"	-

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C





Maximum Floor Spans - S7.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 15 psf L/480 under live load and L/240 under total load

Deflection limits: Sheathing:

3/4 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			В	are			1/2 in. gyr	osum ceiling		
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'- 4 "	16'-4"	15'-9"	15'-1"	
9-1/2	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"	
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"	
	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"	
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11'	
11-7/8"	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"	
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11'	
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19'-3"	18'-3"	
	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"	
14"	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"	
14	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11'	
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"	
	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"	
16"	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"	
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"	

		Mi	d-span blocking	with 1x4 inch	strap	Mid-sp	an blocking an	d 1/2 in. gypsu	m ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
9-1/2"	NI-40x	18'-7"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-9"	20'-3"	19'-4"	17'-8"	22'-4"	20'-5"	19'-4"	17'-8"
11-7/8"	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7" .	20'-10"	19'-8"	18'-4"
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"
	NI-40x	24'-4"	22'-8"	21'-8"	19'-5"	25'-0"	23'-2"	21'-9"	19'-5"
14"	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10
14	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"
	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"
16"	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.





Maximum Floor Spans - M2.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 20 psf

Deflection limits: L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			В	are			1/2 in. gyp	osum ceiling		
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	-	
9-1/2"	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-	
9-1/2	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-	
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-	
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-	
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-	
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-	
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-	
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-	
	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	_	
14"	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-	
14	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-	
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-	
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-	
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	_	
	NI-90	24'-5"	22'-6"	21'-6"	-	25'-1"	23'-2"	22'-2"	_	

		Mi	d-span blocking	with 1x4 inch st	trap	Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing				
Joist depth	Joist series		On cent	re spacing						
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-	
9-1/2"	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-	
9-1/2	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-	
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-	
on the second se	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-	
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-0"	-	
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-	
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-	
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-	
	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	20'-11"	-	
14"	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-	
14	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-	
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-	
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-	
16"	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-	
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	-	

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



Maximum Floor Spans - M4.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 20 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			В	are			1/2 in. gyr	sum ceiling		
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11'	
9-1/2	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"	
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"	
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"	
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"	
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"	
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"	
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"	
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"	
14"	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10'	
14	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"	
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"	
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"	
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10'	
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"	

		Mi	d-span blocking	with 1x4 inch	strap	Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing				
Joist depth	Joist series		On cent	re spacing						
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	18'-8"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11"	
9-1/2	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"	
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10'	
	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"	
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-6"	19'-0"	17'-0"	
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"	
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"	
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"	
	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"	
14"	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10'	
14	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"	
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"	
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"	
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"	
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"	

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



Maximum Floor Spans - M6.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 20 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			В	are		1/2 in. gypsum ceiling On centre spacing				
Joist depth	Joist series		On cent	re spacing						
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	-	
9-1/2"	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-	
9-1/2	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	-	
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	-	
	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-	
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17'-4"	16'-9"	-	
11-7/8"	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	-	
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	-	
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	-	
	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	-	
14"	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	-	
14	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	-	
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-	
	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-	
16"	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	-	
	NI-90	24'-1"	22'-2"	21'-2"	-	24'-9"	22'-11"	21'-10"	-	

		Mi	d-span blocking	with 1x4 inch st	trap	Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing				
Joist depth	Joist series		On centi	re spacing						
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-	
9-1/2"	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	-	
9-1/2	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-	
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-	
	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-	
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	-	
11-7/8"	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	-	
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-	
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-	
	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	20'-11"	-	
14"	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-	
14	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	-	
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-	
	NI-60	26'-2"	24'-3"	23'-2"	-	26'-11"	25'-0"	23'-11"	-	
16"	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-	
	NI-90	28'-5"	26'-3"	25'-0"	~	29'-0"	26'-11"	25'-8"	_	

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



Maximum Floor Spans - M7.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 20 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

3/4 in. nailed-glued Canadian softwood plywood

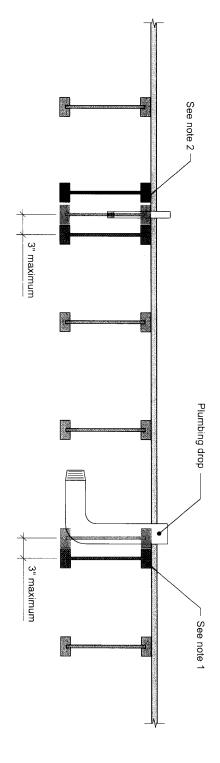
Maximum Floor Spans

			В	are		1/2 in. gypsum ceiling				
Joist depth	Joist series	On centre spacing				On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16'-4"	15'-9"	14'-11'	
9-1/2	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"	
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"	
	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"	
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11	
11-7/8"	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"	
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11	
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19'-3"	18'-3"	
	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"	
14"	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"	
14	N!-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11	
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"	
	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"	
16"	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"	
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"	

		Mi	d-span blocking	with 1x4 inch	strap	Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing				
Joist depth	Joist series		On cent	re spacing						
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	18'-7"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11"	
9-1/2	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"	
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10'	
	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"	
	NI-40x	21'-9"	20'-3"	19'-0"	17'-0"	22'-4"	20'-5"	19'-0"	17'-0"	
11-7/8"	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"	
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"	
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"	
	NI-40x	24'-4"	22'-8"	20'-11"	18'-8"	25'-0"	22'-11"	20'-11"	18'-8"	
14"	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10'	
14	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"	
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"	
	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"	
16"	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11'	
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"	

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C





- Notes:

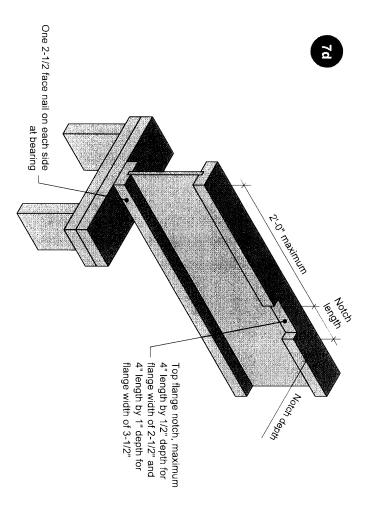
 1. To prevent interference with plumbing, a joist may be shifted up to 3 inches if the edge of the floor panel is supported.
- and the span rating is not exceeded.In all other cases, an additional joist is required.

NORDIC STRUCTURES NS-DC3 || DETAILS NORDIC JOIST CATEGORY Allowance for Piping 7с

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All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails, Individual components not shown to scale for clarify, Openings for Vertical Elements SCALE 2020-10-01 DRAWING 3.10

REVIEWED



Notes:

- Blocking required at bearing for lateral support, not shown for clarity.
 The maximum dimensions for a notch on the side of the top flange are 4-inch length by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch length by 1-inch depth for flange width of 3-1/2 inches.
- This detail applies to simple-span joists and multiple-span joists where the notch is located at the end half-span.
- For other applications, contact Nordic Structures.

NS-DC3 DETAILS NORDIC JOIST CATEGORY

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity

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Openings for Vertical Elements Notch in I-joist for Heat Register SCALE 7d 2020-10-01 DRAWING 3.11

2-1/2" and 1" depth for flange width of 3-1/2" Maximum 1/2" depth for flange width of Heat register

REVIEWE