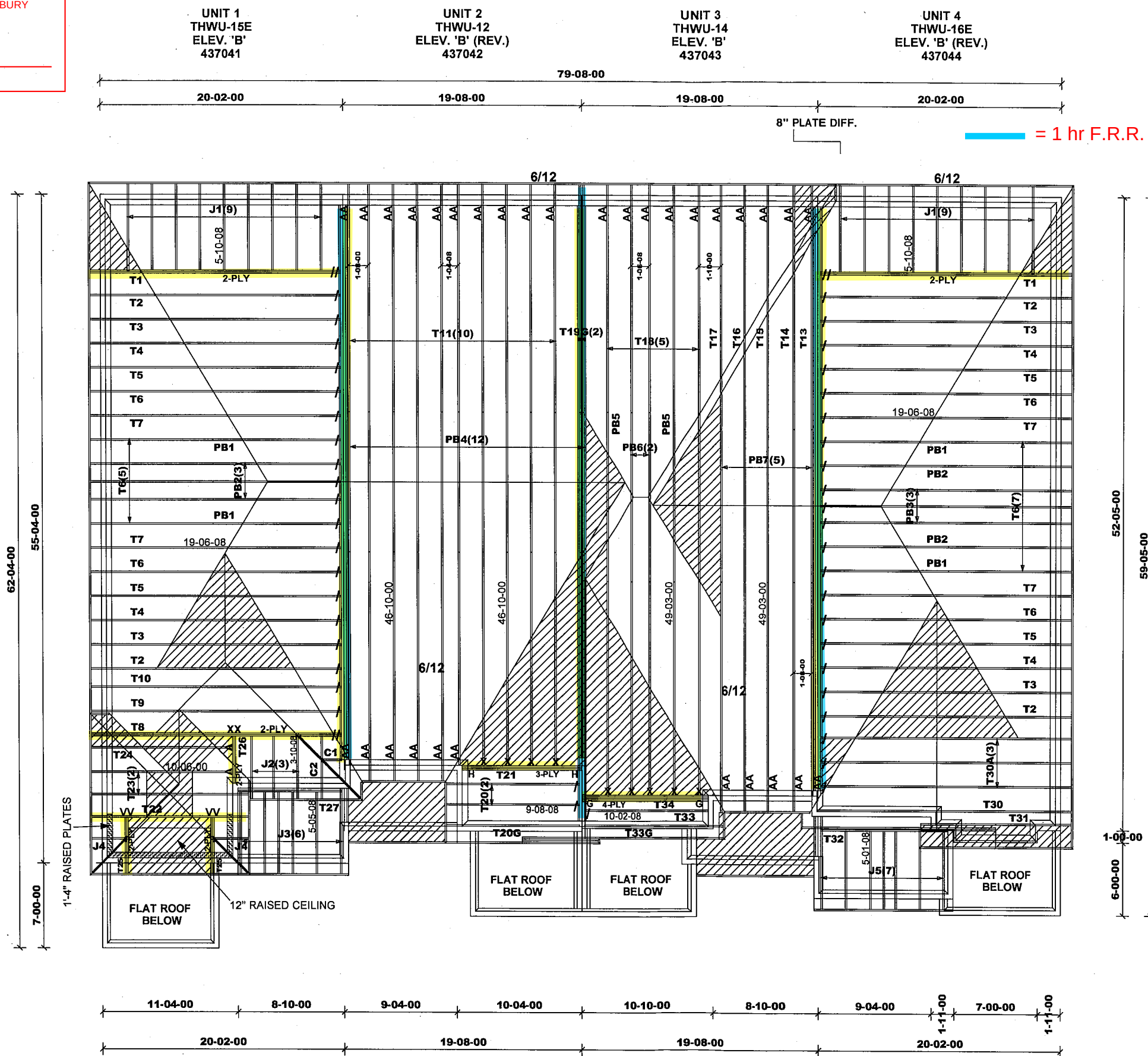


TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2024-05-14
INSPECTOR: SE



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

ROOF PITCHES TO BE 10/12 UNLESS
NOTED OTHERWISE

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.9 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LJS26DS - (V)
HGUS26 - (X)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (I)
H2.5A - (A)
LGT3-SDS2.5 - (H)
LGT4-SDS3 - (G)



Job Track: 53568
Plan Log: 207901
Layout ID: 437040

Builder / Location:
BAYVIEW WELLINGTON / BRADFORD
Project: GREEN VALLEY ESTATE (2024)
Date: 2024-04-03 Sales: Rick DiCiano Designer:

Model / Elevation:
BLOCK 403-3 / UNIT 1-4

Mitek ver 8.6.3.353

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR
REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

REVIEWED

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY ESTATE (2024)
 Location: BRADFORD
 Model: BLOCK 403-3
 Lot #:
 Elevation: UNIT 1

Job Track: 53568
 PlanLog: 207901
 Layout ID: 437041
 Ref #
 Page: 1 of 2
 Date: 04-03-2024
 Designer:
 Sales Rep: Rick DiCiano

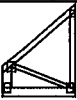
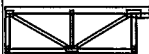
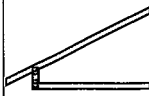
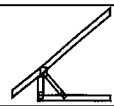
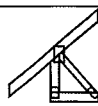
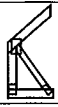
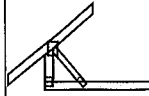
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	10 /12	19-06-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	192.62 123.00		
	2	T2 Half Hip	10 /12	19-06-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	170.45 109.00		
	2	T3 Half Hip	10 /12	19-06-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	182.6 117.33		
	2	T4 Half Hip	10 /12	19-06-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	181.94 112.67		
	2	T5 Half Hip	10 /12	19-06-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	192.07 121.33		
	7	T6 Half Hip	10 /12	19-06-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	725.64 453.83		
	2	T7 Half Hip	10 /12	19-06-08	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	232.34 145.67		
	1 2-ply	T8 Hip Girder	10 /12	19-06-08	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 1-10-10	199.59 127.33		
	1	T9 Hip	10 /12	19-06-08	6-06-07	2 x 4	1-03-08	1-07-11 1-10-10	94.3 61.00		
	1	T10 Hip	10 /12	19-06-08	8-02-07	2 x 4	1-03-08	1-07-11 1-10-10	93.15 59.83		
	1	T22 Hip Girder	10 /12	10-06-00	4-05-00	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	56.84 39.00		
	2	T23 Hip	10 /12	10-06-00	5-09-00	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	112.72 73.67		
	1	T24 Hip	10 /12	10-06-00	4-01-00	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	53.61 35.00		
	2 2-ply	T25 Half Hip Girder	10 /12	3-04-00	2-11-02	2 x 4	1-03-08	1-07-11 2-11-02	79.54 56.00		

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST					
	Lumber Yard: TAMARACK LUMBER				Job Track: 53568	
	Builder: BAYVIEW WELLINGTON				PlanLog: 207901	
	Project: GREEN VALLEY ESTATE (2024)				Layout ID: 437041	
	Location: BRADFORD				Ref #	
	Model: BLOCK 403-3				Page: 2 of 2	
Lot #:				Date: 04-03-2024		
Elevation: UNIT 1				Designer:		
				Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T26 Monopitch Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	44.83 30.67		
	1	T27 Flat Girder	0 /12	8-07-08	3-00-15	2 x 6 2 x 4		3-00-15 3-00-15	42.35 27.17		
	2	PB1 Piggyback	10 /12	10-07-00	2-00-00	2 x 4		2-00-00	68.49 45.33		
	3	PB2 Piggyback	10 /12	10-07-00	3-00-00	2 x 4		3-00-00	106.76 66.50		
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	3	J2 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.42 29.00		
	6	J3 Jack-Open	6 /12	5-05-08	3-08-07	2 x 4	1-03-08	4-03 3-00-15	89.98 56.00		
	2	J4 Jack-Open	10 /12	1-06-08	2-11-02	2 x 4	1-03-08	1-07-11 2-11-02	18.03 12.67		
	1	C1 Jack-Open	10 /12	1-07-00	3-01-09	2 x 4		1-10-10 3-02-07	7.5 5.67		
	1	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.04 8.33		

TOTAL # TRUSS= 61
TOTAL BFT OF ALL TRUSSES= 2012
BFT.
TOTAL WEIGHT OF ALL TRSSES 3153.97 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
2	Hardware	LUS26-2	
1	Hardware	HGUS26-2	
23	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 28

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	53568
	Builder:	BAYVIEW WELLINGTON	PlanLog:	207901
	Project:	GREEN VALLEY ESTATE (2024)	Layout ID:	437042
	Location:	BRADFORD	Ref #	
	Model:	BLOCK 403-3	Page:	1 of 1
Lot #:		Date:	04-03-2024	
Elevation:	UNIT 2	Designer:		
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	10	T11 Piggyback Base	6 / 12	46-10-00	9-11-11	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	3140.66 1853.33		
	1	T19G GABLE	6 / 12	46-02-00	9-11-11	2 x 6	1-03-08	1-02-00 1-06-00	290.29 186.50		
	2	T20 Monopitch	10 / 12	9-08-08	9-08-12	2 x 4	1-03-08	1-07-11 9-08-12	105.65 67.33		
	1	T20G GABLE	10 / 12	9-08-08	9-08-12	2 x 4	1-03-08	1-07-11 9-08-12	57.07 36.67		
	1 3-ply	T21 Monopitch Girder	10 / 12	9-10-08	9-07-08	2 x 4 2 x 8		1-07-11 9-10-07	231.89 145.00		
	11	PB4 Piggyback	6 / 12	11-07-04	2-10-13	2 x 4			303.02 192.50		

TOTAL # TRUSS= 28 TOTAL BFT OF ALL TRUSSES= 2481.33 BFT. TOTAL WEIGHT OF ALL TRSSES 4128.57 LBS

HARDWARE

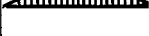
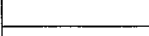
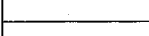
QTY	TYPE	MODEL	LENGTH
4	Hardware	HGUS26	
2	Hardware	H2.5T	
32	Hardware	H2.5A	
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= 40

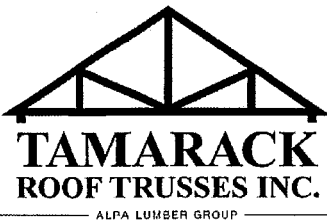
REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST					
	Lumber Yard: TAMARACK LUMBER				Job Track: 53568	
	Builder: BAYVIEW WELLINGTON				PlanLog: 207901	
	Project: GREEN VALLEY ESTATE (2024)				Layout ID: 437043	
	Location: BRADFORD				Ref #	
	Model: BLOCK 403-3				Page: 1 of 2	
Lot #:				Date: 04-03-2024		
Elevation: UNIT 3				Designer:		
				Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T13 Piggyback Base	6 / 12	49-03-00	9-11-11	2 x 6	1-03-08 1-03-08	6-00 1-02-00	310.58 183.67		
	1	T14 Piggyback Base	6 / 12	49-03-00	9-11-11	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	323.8 193.83		
	1	T15 Piggyback Base	6 / 12	49-03-00	9-11-11	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	336.28 203.33		
	1	T16 Piggyback Base	6 / 12	49-03-00	9-11-11	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	330.82 198.00		
	1	T17 Piggyback Base	6 / 12	49-03-00	9-11-11	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	348.08 206.67		
	5	T18 Piggyback Base	6 / 12	49-03-00	9-11-11	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1621.21 963.33		
	1	T19G GABLE	6 / 12	46-02-00	9-11-11	2 x 6	1-03-08	1-02-00 1-06-00	290.29 186.50		
	1	T33 Monopitch	10 / 12	10-02-08	10-01-12	2 x 4	1-03-08	1-07-11 10-01-12	55.24 35.50		
	1	T33G GABLE	10 / 12	10-02-08	10-01-12	2 x 4	1-03-08	1-07-11 10-01-12	60.53 39.00		
	1 4-ply	T34 Monopitch Girder	10 / 12	10-04-08	10-00-08	2 x 4 2 x 8		1-07-11 10-03-07	323.82 209.33		
	1	PB4 Piggyback	6 / 12	11-07-04	2-10-13	2 x 4			27.55 17.50		
	2	PB5 Piggyback	6 / 12	14-00-04	1-08-00	2 x 4			77.33 49.67		
	2	PB6 Piggyback	6 / 12	14-00-04	3-04-00	2 x 4			77.41 49.67		
	5	PB7 Piggyback	6 / 12	12-08-04	3-06-00	2 x 4			180.96 115.83		

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	53568
	Builder:	BAYVIEW WELLINGTON		PlanLog:	207901
	Project:	GREEN VALLEY ESTATE (2024)		Layout ID:	437043
	Location:	BRADFORD		Ref #	
	Model:	BLOCK 403-3		Page:	2 of 2
Lot #:			Date:	04-03-2024	
Elevation:	UNIT 3		Designer:		
			Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
TOTAL # TRUSS= 27 TOTAL BFT OF ALL TRUSSES= 2651.83 BFT. TOTAL WEIGHT OF ALL TRSSES 4363.9 LBS											

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	HGUS26	
1	Hardware	H2.5T	
32	Hardware	H2.5A	
2	Hardware	LGT4	

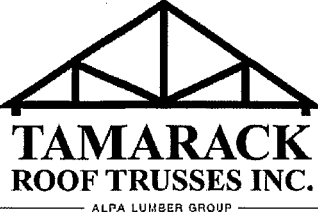
TOTAL NUMBER OF ITEMS= 39

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY ESTATE (2024) Location: BRADFORD Model: BLOCK 403-3 Lot #: Elevation: UNIT 4	Job Track: 53568 PlanLog: 207901 Layout ID: 437044 Ref # Page: 1 of 2 Date: 04-03-2024 Designer: Sales Rep: Rick DiCiano		

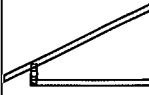
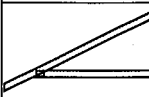
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	10 /12	19-06-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	192.62 123.00		
	2	T2 Half Hip	10 /12	19-06-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	170.45 109.00		
	2	T3 Half Hip	10 /12	19-06-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	182.6 117.33		
	2	T4 Half Hip	10 /12	19-06-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	185.05 114.33		
	2	T5 Half Hip	10 /12	19-06-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	195.68 123.33		
	9	T6 Half Hip	10 /12	19-06-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	932.97 583.50		
	2	T7 Half Hip	10 /12	19-06-08	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	232.34 145.67		
	1	T30 Common	10 /12	19-10-00	9-10-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	94.45 60.00		
	3	T30A Common	10 /12	19-06-08	9-10-14	2 x 4	1-03-08	1-07-11 1-10-10	275.61 176.00		
	1	T31 Common	10 /12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	45.34 29.33		
	1	T32 Flat Girder	0 /12	10-06-08	2-10-15	2 x 6		2-10-15 2-10-15	52.53 33.67		
	2	PB1 Piggyback	10 /12	10-07-00	2-00-00	2 x 4		2-00-00	68.49 45.33		
	2	PB2 Piggyback	10 /12	10-07-00	3-00-00	2 x 4		3-00-00	71.17 44.33		
	3	PB3 Piggyback	10 /12	10-07-00	4-00-00	2 x 4		4-00-00	109.67 70.50		

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY ESTATE (2024) Location: BRADFORD Model: BLOCK 403-3 Lot #: Elevation: UNIT 4	Job Track: 53568 PlanLog: 207901 Layout ID: 437044 Ref # Page: 2 of 2 Date: 04-03-2024 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	7	J5 Jack-Open	6 /12	5-01-08	3-06-07	2 x 4	1-03-08	4-03 2-10-15	99.34 65.33		

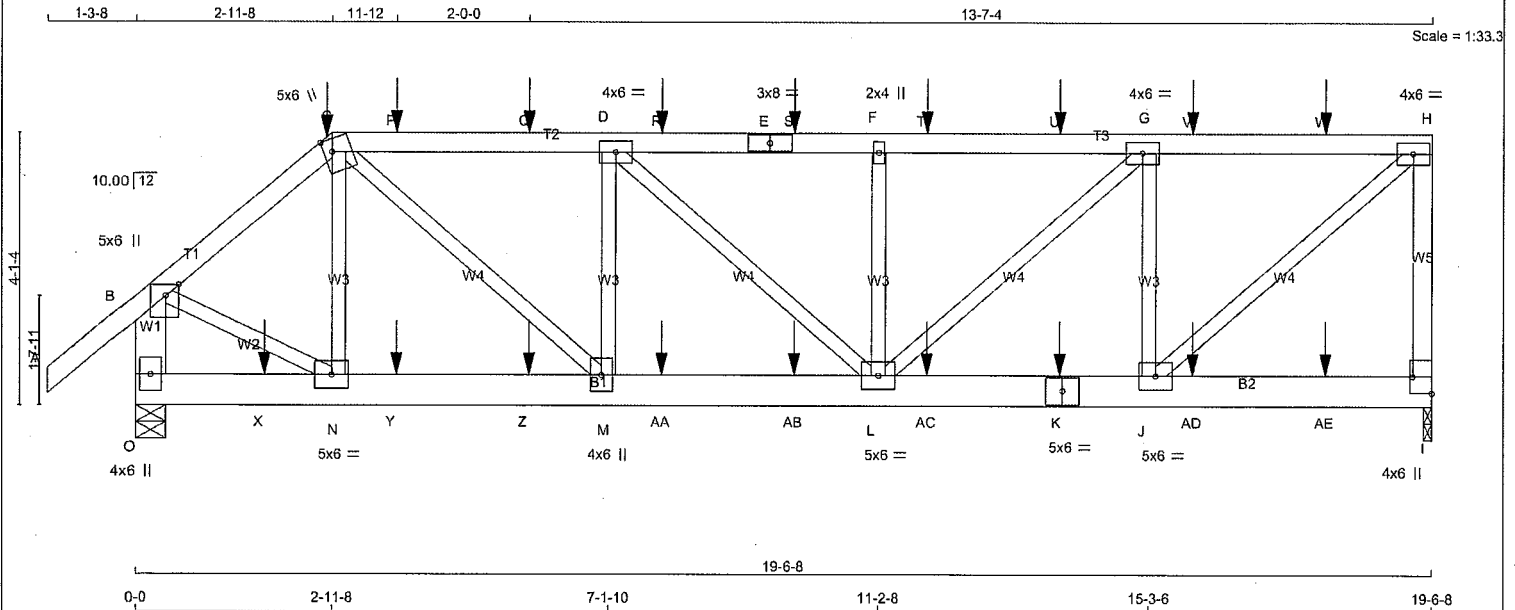
TOTAL # TRUSS= 50
TOTAL BFT OF ALL TRUSSES= 1936.65
BFT.
TOTAL WEIGHT OF ALL TRSSES 3059.47
LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
24	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 24

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T1	2	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					
Version 8.630 S Aug 30 2023 MITek Industries, Inc. Tue Apr 2 11:23:17 2024 Page 1					
ID:Sb 47nW9yvpKeYGkMELfPzxU46-KATlozk IOWOZL8k6SB A Hr2s2 YKROGxex ozUneO					
TRUSS DESC.					



TOTAL WEIGHT = 4 X 96 = 385 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
O - B	2x6	DRY	No.2	SPF	
O - K	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - E	12	SIDE(61.0)
E - H	12	SIDE(0.0)
H - I	12	TOP
O - B	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
O - K	12	SIDE(0.0)
K - I	12	SIDE(0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
I	2047	0	0	1-8
O	2260	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1436	1009 / 0	0 / 0	0 / 0	0 / 0	428 / 0	0 / 0
O	1581	1134 / 0	0 / 0	0 / 0	0 / 0	447 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO							
A-B	0 / 50	-112.4	-112.4	0.09 (1)	10.00	N-C	-474 / 0
B-C	-2019 / 0	-112.4	-112.4	0.11 (1)	6.09	C-M	0 / 1538
C-P	-2682 / 0	-112.4	-112.4	0.28 (1)	5.23	M-D	-898 / 0
P-Q	-2682 / 0	-112.4	-112.4	0.28 (1)	5.23	D-L	0 / 86
Q-D	-2682 / 0	-112.4	-112.4	0.28 (1)	5.23	L-F	-667 / 0
D-R	-2746 / 0	-112.4	-112.4	0.28 (1)	5.18	L-G	0 / 1056
R-E	-2746 / 0	-112.4	-112.4	0.28 (1)	5.18	J-G	-1573 / 0
E-S	-2746 / 0	-112.4	-112.4	0.28 (1)	5.18	J-H	0 / 2614
S-F	-2746 / 0	-112.4	-112.4	0.28 (1)	5.18	B-N	0 / 1664
F-T	-2746 / 0	-112.4	-112.4	0.29 (1)	5.18		
T-U	-2746 / 0	-112.4	-112.4	0.29 (1)	5.18		
U-G	-2746 / 0	-112.4	-112.4	0.29 (1)	5.18		
G-V	-1967 / 0	-112.4	-112.4	0.27 (1)	5.90		
V-W	-1967 / 0	-112.4	-112.4	0.27 (1)	5.90		
W-H	-1967 / 0	-112.4	-112.4	0.27 (1)	5.90		
I-H	-1989 / 0	0.0	0.0	0.25 (1)	7.80		
O-B	-2238 / 0	0.0	0.0	0.08 (1)	7.81		

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO							
O-X	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
X-N	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
N-Y	0 / 1532	-18.5	-18.5	0.12 (1)	10.00		
Y-Z	0 / 1532	-18.5	-18.5	0.12 (1)	10.00		
Z-M	0 / 1532	-18.5	-18.5	0.12 (1)	10.00		
M-AA	0 / 2683	-18.5	-18.5	0.21 (1)	10.00		
AA-AB	0 / 2683	-18.5	-18.5	0.21 (1)	10.00		
AB-L	0 / 2683	-18.5	-18.5	0.21 (1)	10.00		
L-AC	0 / 1967	-18.5	-18.5	0.16 (1)	10.00		
AC-K	0 / 1967	-18.5	-18.5	0.16 (1)	10.00		
K-J	0 / 1967	-18.5	-18.5	0.16 (1)	10.00		
J-AD	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
AD-AE	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
AE-I	0 / 0	-18.5	-18.5	0.04 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-29	-29	---	FRONT	VERT	DEAD	C1
C	2-11-8	-157	-157	---	FRONT	VERT	SNOW	C1
K	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	C1
P	3-11-4	-93	-93	---	FRONT	VERT	TOTAL	C1
Q	5-11-4	-93	-93	---	FRONT	VERT	TOTAL	C1
R	7-11-4	-93	-93	---	FRONT	VERT	TOTAL	C1
S	9-11-4	-93	-93	---	FRONT	VERT	TOTAL	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.29/1.00 (F-G:1), BC=0.21/1.00 (L-M:1), WB=0.32/1.00 (H-J:1), SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (H) (INPUT = 0.90)
JSI METAL= 0.29 (H) (INPUT = 0.95)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR24040190

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T1	2	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	4.0	6.0	3.00	Edge
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	11-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
U	13-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
V	15-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
W	17-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
X	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AB	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AC	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AD	15-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AE	17-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



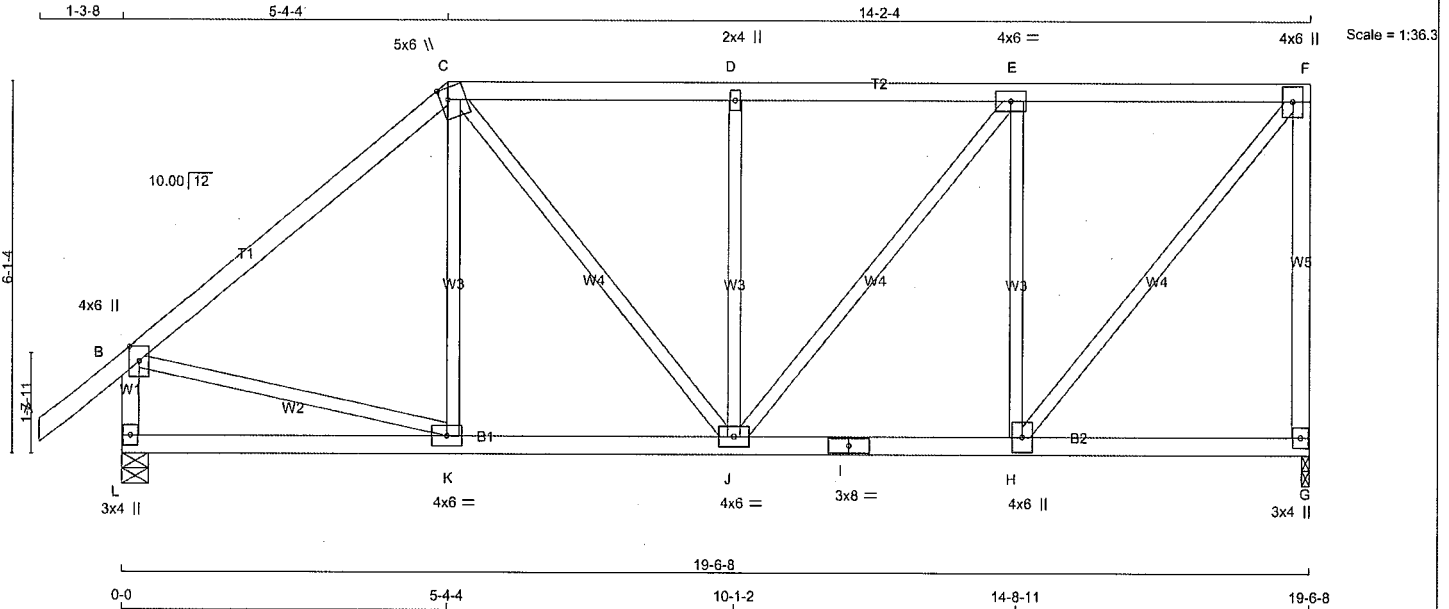
STRUCTURAL COMPONENT ONLY
DWG # TR24040190

REVIEWED

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T3	4	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - F	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
L - B	2x4 DRY	No.2	SPF		
L - I	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	Edge	
C TTWW+m	MT20	5.0	6.0	2.25	1.50
D TMW+w	MT20	2.0	4.0		
E TMW+t	MT20	4.0	6.0		
F TMVW+p	MT20	4.0	6.0		
G BMV1+p	MT20	3.0	4.0		
H BMW+t	MT20	4.0	6.0		
I BS-t	MT20	3.0	8.0		
J BMW+t	MT20	4.0	6.0		
K BMW+t	MT20	4.0	6.0		
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
JT	VERT	HORZ	1279	0	1279	0	0	1-8
G	1279	0	1279	0	0	1-8	1-8	
L	1435	0	1435	0	0	5-8	1-9	

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN.	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0	
L	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	K-C	-97 / 61
B-C	-1138 / 0	-112.4	-112.4	0.84 (1)	5.00	B-K	0 / 895
C-D	-1116 / 0	-112.4	-112.4	0.35 (1)	5.53	H-F	0 / 1329
D-E	-1117 / 0	-112.4	-112.4	0.37 (1)	5.50	C-J	0 / 391
E-F	-848 / 0	-112.4	-112.4	0.36 (1)	6.10	H-E	-922 / 0
G-F	-1243 / 0	0.0	0.0	0.90 (1)	7.20	J-D	-568 / 0
L-B	-1394 / 0	0.0	0.0	0.15 (1)	6.90	J-E	0 / 430
L-K	0 / 0	-18.5	-18.5	0.12 (4)	10.00		
K-J	0 / 889	-18.5	-18.5	0.21 (1)	10.00		
J-I	0 / 848	-18.5	-18.5	0.19 (1)	10.00		
I-H	0 / 848	-18.5	-18.5	0.19 (1)	10.00		
H-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.90/1.00 (F-G:1), BC=0.21/1.00 (J-K:1), WB=0.54/1.00 (E-H:1), SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 0.95)

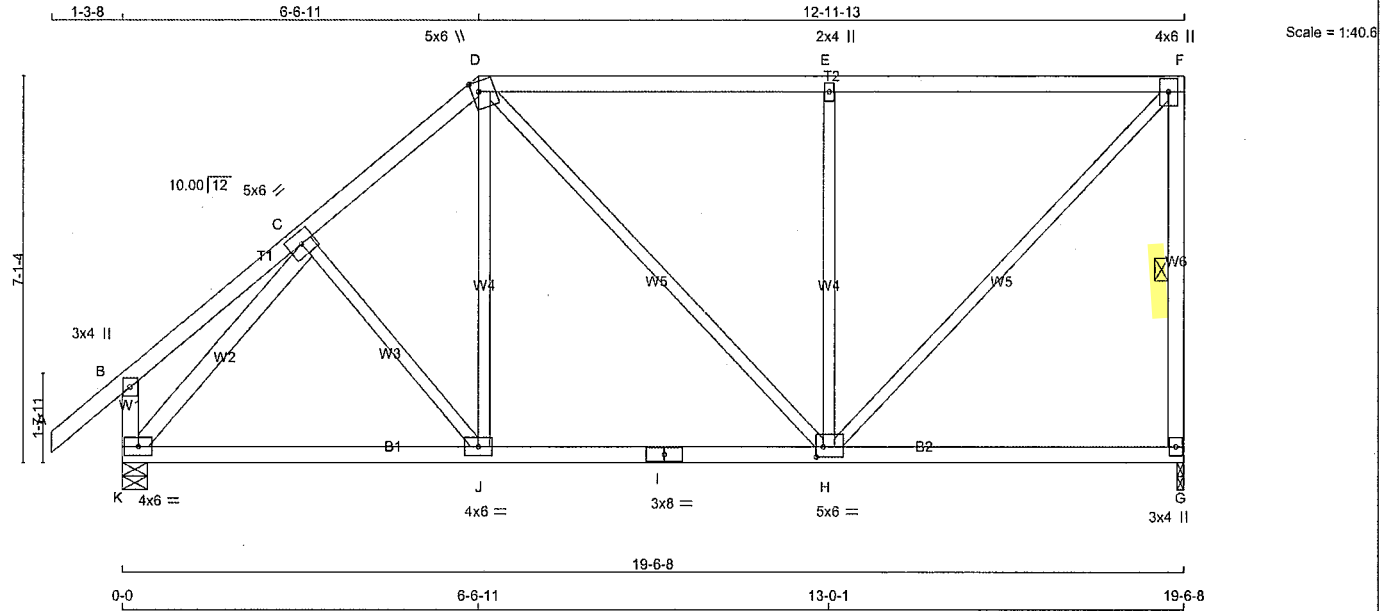


STRUCTURAL COMPONENT ONLY
DWG # TR24040192

REVIEWED

JOB NAME 437040	TRUSS NAME T4	QUANTITY 4	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 4 X 93 = 370 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
K - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - D	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	5.0	6.0			
D	TTWW+m	MT20	5.0	6.0	2.25	1.50	
E	TMW+w	MT20	2.0	4.0			
F	TMW+p	MT20	4.0	6.0			
G	BMV1+p	MT20	3.0	4.0			
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50	
I	BS-t	MT20	3.0	8.0			
J	BMWW-t	MT20	4.0	6.0			
K	BMVW1-t	MT20	4.0	6.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
G	1279	0	1-8	1-8
K	1435	0	5-8	1-9

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
K	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 50	-112.4 -112.4 0.15 (1)	C-J	-79 / 20
B-C	0 / 27	-112.4 -112.4 0.19 (1)	J-D	0 / 193
C-D	-1110 / 0	-112.4 -112.4 0.21 (1)	D-H	0 / 114
D-E	-911 / 0	-112.4 -112.4 0.88 (1)	H-E	-905 / 0
E-F	-911 / 0	-112.4 -112.4 0.87 (1)	H-F	0 / 1317
G-F	-1229 / 0	0.0 0.0 0.28 (1)	K-C	-1401 / 0
K-B	-296 / 0	0.0 0.0 0.03 (1)		
K-J	0 / 881	-18.5 -18.5 0.24 (1)		
J-I	0 / 833	-18.5 -18.5 0.24 (4)		
I-H	0 / 833	-18.5 -18.5 0.24 (4)		
H-G	0 / 0	-18.5 -18.5 0.17 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.87/1.00 (E-F:1), BC=0.24/1.00 (H-J:4),
WB=0.79/1.00 (E-H:1), SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 0.95)

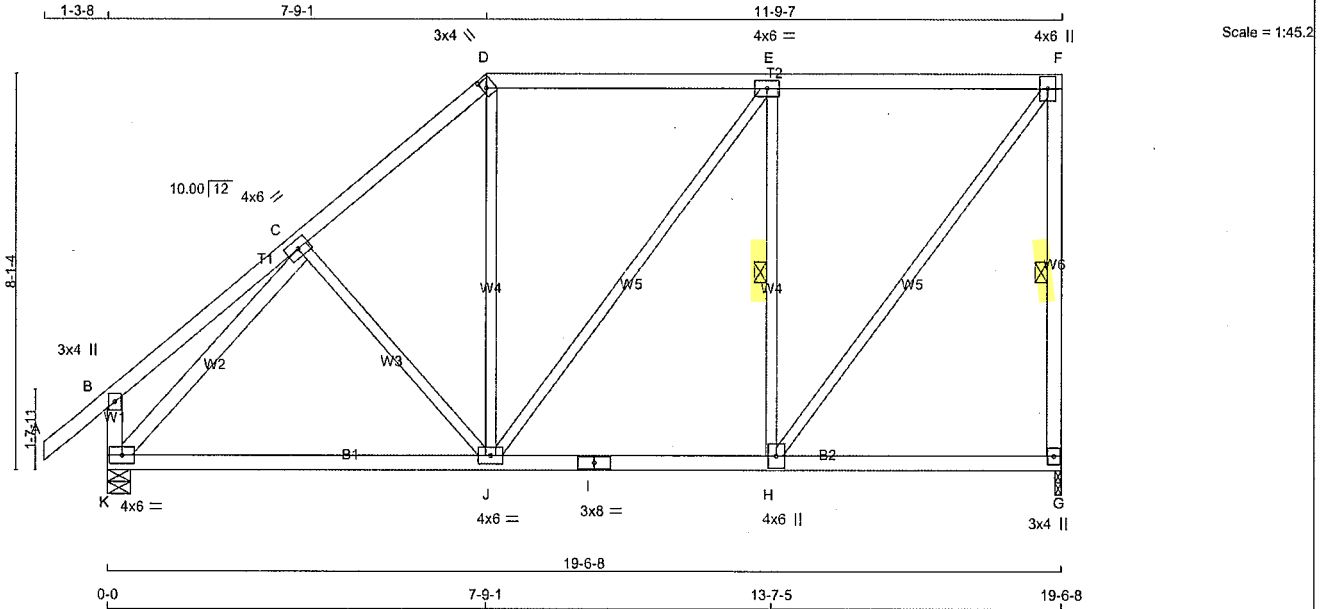


STRUCTURAL COMPONENT ONLY
DWG # TR24040193

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T5	4	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:Sb 47nW9vypKeYGkKmELfPzxU46-h7GBsqn77Z8gg60iu?n9tn bStkbDwZiQDLiz?zUneJ



TOTAL WEIGHT = 4 X 98 = 391 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - C	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	6.0	
D	TTW+h	MT20	3.0	4.0	2.00 1.00
E	TMVW-t	MT20	4.0	6.0	
F	TMVW+p	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMVW-t	MT20	4.0	6.0	
I	BS-t	MT20	3.0	8.0	
J	BMVW-t	MT20	4.0	6.0	
K	BMVW-t	MT20	4.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
G	1279 0	1279 0	0 0	1-8	1-8
K	1435 0	1435 0	0 0	5-8	1-9

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
G	897	635 / 0	0 / 0
K	1003	725 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
A-B	0 / 50	C-J	-183 / 0
B-C	0 / 33	J-D	0 / 233
C-D	-1047 / 0	J-E	0 / 62
D-E	-780 / 0	H-E	-871 / 0
E-F	-744 / 0	H-F	0 / 1229
G-F	-1232 / 0	K-C	-1385 / 0
K-B	-320 / 0		
K-J	0 / 899		
J-I	0 / 744		
I-H	0 / 744		
H-G	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.89/1.00 (D-E:1), BC=0.30/1.00 (J-K:4), WB=0.59/1.00 (C-K:1), SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.34 (F) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040194

REVIEWED

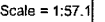
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Scale = 1:50.6

TOTAL WEIGHT = 16 X 104 = 1659 lb

REVIEWED

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 4 X 116 = 465 lb

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 0.95)

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	K-C	-129 / 55	0.07 (1)	
B-C	-1176 / 0	-112.4	-112.4	0.40 (1)	5.38	C-J	-450 / 0	0.52 (1)	
C-D	-875 / 0	-112.4	-112.4	0.39 (1)	6.04	J-D	0 / 124	0.03 (4)	
D-E	-837 / 0	-112.4	-112.4	0.32 (1)	6.25	J-E	0 / 322	0.05 (1)	
E-F	-522 / 0	-112.4	-112.4	0.31 (1)	6.25	H-E	-943 / 0	0.66 (1)	
G-F	-1444 / 0	0.0	0.0	0.62 (1)	5.76	H-F	0 / 1155	0.19 (1)	
L-B	-1396 / 0	0.0	0.0	0.15 (1)	6.89	B-K	0 / 966	0.22 (1)	
L-K	0 / 0	-18.5	-18.5	0.12 (4)	10.00				
K-J	0 / 937	-18.5	-18.5	0.21 (1)	10.00				
J-I	0 / 502	-18.5	-18.5	0.13 (1)	10.00				
I-H	0 / 502	-18.5	-18.5	0.13 (1)	10.00				
H-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

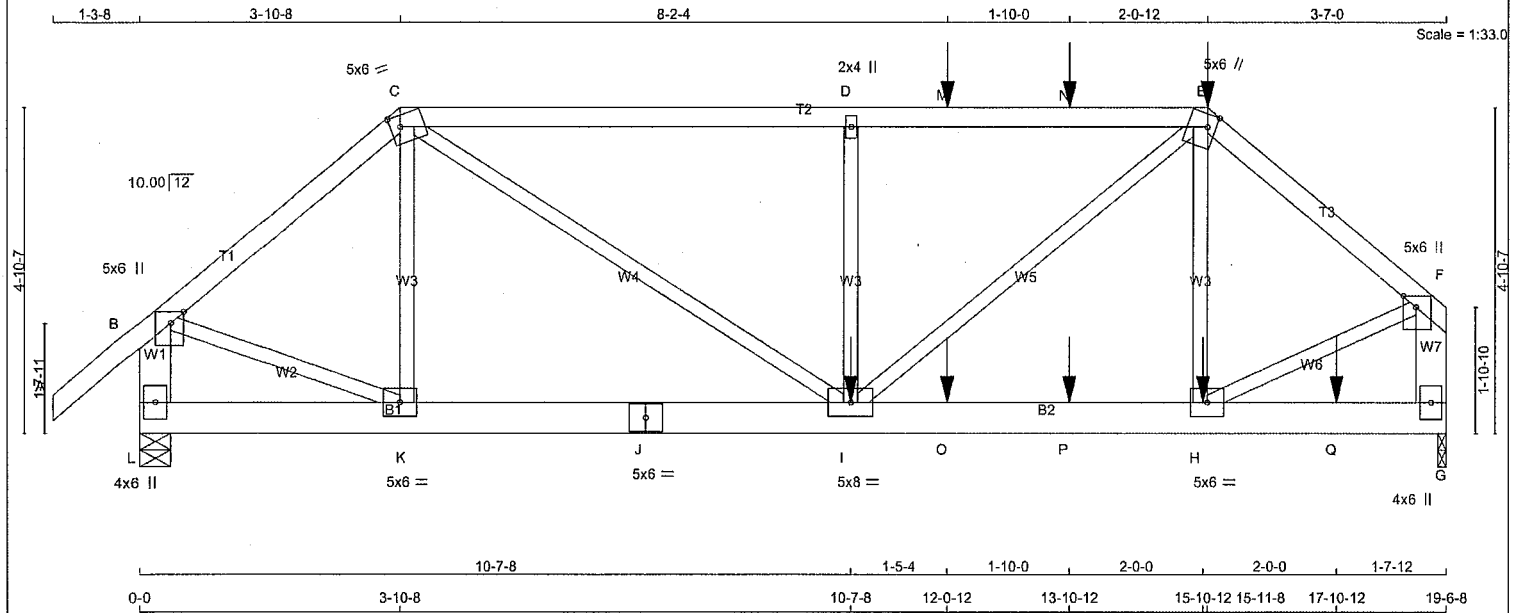


REVIEWED

JOB NAME 437040	TRUSS NAME T8	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:Sb 47nW9yvpKeYgKmeLIPzxU46-1534vOrGx5mzmvtvfhZNKaqhUNuT0uHOVaV3TeDzUneE



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
L - B	2x6	DRY No.2	SPF
G - F	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
J - G	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-E	12	SIDE(61.0)
E-F	12	SIDE(61.0)
L-B	12	TOP
G-F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-J	12	TOP
J-G	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	SIDE(93.5)
D-I	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
L	2071	0	2071	0	0	5-8	1-8		
G	2340	0	2340	0	0	1-8	1-8		

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN.	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
L	1446	1057 / 0	0 / 0	0 / 0	0 / 0	389 / 0	0 / 0	
G	1636	1183 / 0	0 / 0	0 / 0	0 / 0	452 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 50	K-C	-300 / 7
B-C	-1921 / 0	C-I	0 / 1836
C-D	-2995 / 0	I-D	-970 / 0
D-M	-2994 / 0	E-E	0 / 1689
M-N	-2994 / 0	H-E	-502 / 0
N-E	-2994 / 0	B-K	0 / 1539
E-F	-2236 / 0	H-F	0 / 1843
L-B	-2060 / 0		
G-F	-2320 / 0		
L-K	0 / 0		
K-J	0 / 1465		
J-I	0 / 1465		
I-O	0 / 1701		
O-P	0 / 1701		
P-H	0 / 1701		
H-Q	0 / 0		
Q-G	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	15-11-8	-245	-245	---	FRONT	VERT	TOTAL	---	C1
H	15-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
I	10-7-8	-737	-737	---	FRONT	VERT	TOTAL	---	C1
M	12-0-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
N	13-10-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
O	12-0-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
P	13-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Q	17-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.55/1.00 (C-D:1), BC=0.15/1.00 (H-I:1),
WB=0.23/1.00 (F-H:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.23 (C) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040197

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T8	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMVW+p MT20 5.0 6.0 2.00 2.25
C TTWW-m MT20 5.0 6.0 2.00 1.75
D TMW+w MT20 2.0 4.0
E TTWW+m MT20 5.0 6.0 2.25 1.50
F TMVW+p MT20 5.0 6.0 2.00 2.25
G BMV1+p MT20 4.0 6.0
H BMWW-t MT20 5.0 6.0
I BMWWW-t MT20 5.0 8.0
J BS-t MT20 5.0 6.0
K BMWW-t MT20 5.0 6.0
L BMV1+p MT20 4.0 6.0

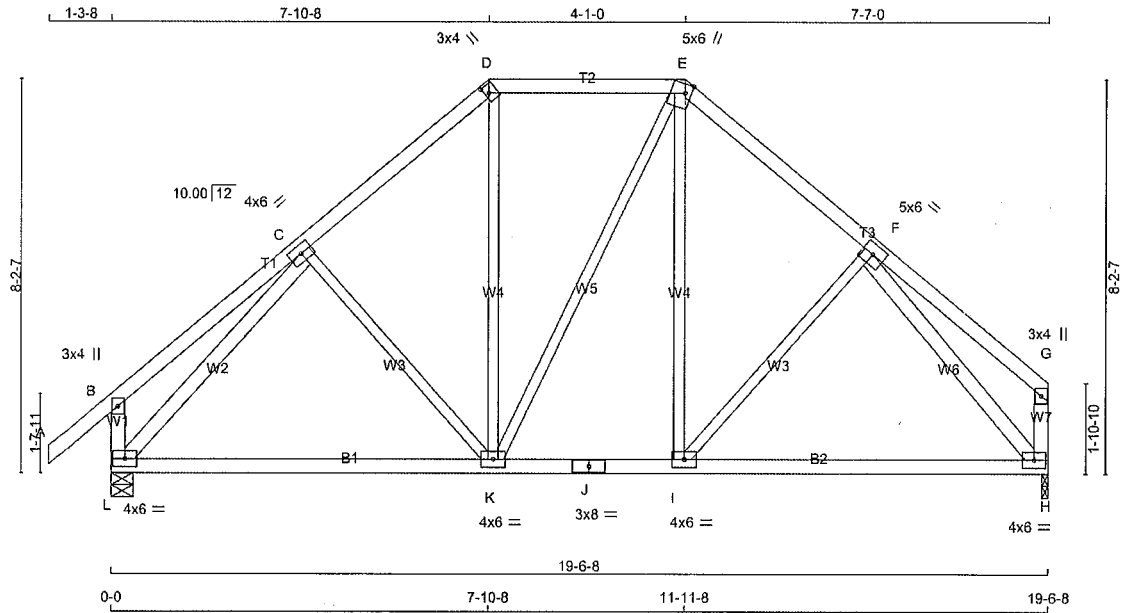
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR24040197

REVIEWED

REVIEWED



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
L - C	2x4	DRY	No.2
F - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW+t	MT20	4.0	6.0	
D	TTW+h	MT20	3.0	4.0	2.00 1.00
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMWW+t	MT20	5.0	6.0	
G	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BMWW-t	MT20	4.0	6.0	
J	BS-t	MT20	3.0	8.0	
K	BMVW1-t	MT20	4.0	6.0	
L	BMVW1-t	MT20	4.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
L	1435	0	0	1435	0	0	5-8	1-9	
H	1279	0	0	1279	0	0	1-8	1-8	

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
L	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0		
H	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	C-K	-192 / 0	0.13 (1)
B-C	0 / 34	-112.4 -112.4	0.28 (1)	10.00	K-D	0 / 246	0.06 (1)
C-D	-1039 / 0	-112.4 -112.4	0.23 (1)	5.90	K-E	0 / 25	0.01 (1)
D-E	-774 / 0	-112.4 -112.4	0.25 (1)	6.25	I-E	0 / 211	0.05 (1)
E-F	-1023 / 0	-112.4 -112.4	0.21 (1)	5.95	I-F	-130 / 11	0.08 (1)
F-G	0 / 34	-112.4 -112.4	0.26 (1)	10.00	L-C	-1382 / 0	0.60 (1)
L-B	-323 / 0	0.0 0.0	0.03 (1)	7.81	F-H	-1363 / 0	0.55 (1)
H-G	-150 / 0	0.0 0.0	0.02 (1)	7.81			
L-K	0 / 899	-18.5 -18.5	0.30 (4)	10.00			
K-J	0 / 763	-18.5 -18.5	0.28 (4)	10.00			
J-I	0 / 763	-18.5 -18.5	0.28 (4)	10.00			
I-H	0 / 847	-18.5 -18.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.13")

CSI: TC=0.28/1.00 (B-C:1), BC=0.30/1.00 (K-L:4), WB=0.60/1.00 (C-L:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.35 (J) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040199

REVIEWED

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T11	10	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:31 2024 Page 2			
		ID:Sb_47nW9yvpKeYGkKmElfPzxU46-wsJblum?KHPFVCQwORGkgs67Wokq_14U71gn_zUneA			

NOTES-
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40'-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



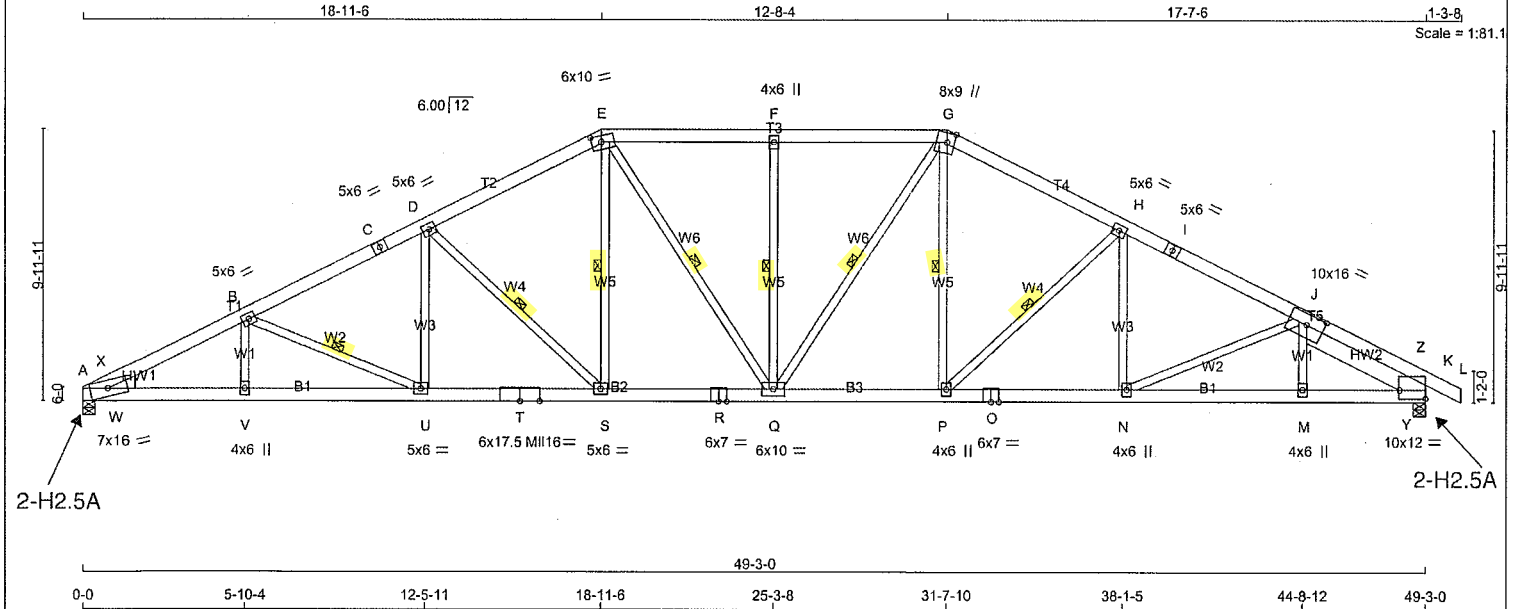
STRUCTURAL COMPONENT ONLY
DWG # TR24040200

REVIEWED

JOB NAME 437040	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MITek Industries, Inc. Tue Apr 2 11:23:32 2024 Page 1
ID:Sb 47nW9yvpKeYGkKmlfPzxU46-Q2tzy5vOmdPGfndU6zVHtONGv5oZPMEjnmEKQzU9e9



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x6	DRY	2100F 1.8E
C - E	2x6	DRY	2100F 1.8E
E - G	2x6	DRY	2100F 1.8E
G - I	2x6	DRY	2100F 1.8E
I - L	2x6	DRY	2100F 1.8E
A - T	2x6	DRY	2100F 1.8E
T - R	2x6	DRY	2100F 1.8E
R - O	2x6	DRY	2100F 1.8E
O - K	2x6	DRY	2100F 1.8E

REINFORCING MEMBERS			
HW2	2x8	DRY	No.2
ALL WEBS	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBMH1-m	MT20	7.0	16.0	
B, D, H					
B	TMWW-t	MT20	5.0	6.0	
C	TS-t	MT20	5.0	6.0	
E	TTWW-m	MT20	6.0	10.0	2.75 4.25
F	TMWW-w	MT20	4.0	6.0	
G	TTWW+m	MT20	8.0	9.0	Edge 3.25
J	TS-t	MT20	5.0	6.0	
K	TMWWW-t	MT20	10.0	16.0	4.75 7.50
M	TBMW-l	MT20	10.0	12.0	3.75 Edge
N	BMWW-w	MT20	4.0	6.0	
O	BMWW-t	MT20	4.0	6.0	
N	BS-t	MT20	6.0	7.0	
P	BMWW-t	MT20	4.0	6.0	
Q	BMWWW-t	MT20	6.0	10.0	
R	BS-t	MT20	6.0	7.0	
S	BMWW-t	MT20	5.0	6.0	
T	BS-t	MT20	6.0	17.5	
U	BMWW-t	MT20	5.0	6.0	
V	BMWW-w	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED				MAXIMUM FACTORED			
GROSS REACTION				BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	HEEL WEDGE
A	4613	0	4613	-209	-767	5-8	2x6 L
K	4819	0	4819	0	-798	5-8	5-4

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 767 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT K FOR 798 LBS. FACTORED UPLIFT

PROVIDE FOR 209 LBS. FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

1ST LCASE MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	3366	2139 / 0	517 / 0	0 / 0	130 / -896	709 / 0	0 / 0
K	3507	2261 / 0	517 / 0	0 / 0	114 / -929	729 / 0	0 / 0

HORIZONTAL REACTIONS

A	---	0 / 0	0 / 0	149 / -149	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K

BRACING

FOR SECTION E-G, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-U, D-S, E-S, E-Q, F-Q, G-Q, G-P, H-P.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (CS) (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (CS) (LC)	
FR-TO		FROM	TO	FR-TO			
A-X	-9215 / 1427	-145.3	-145.3 0.18 (1)	3.59	V-B	0 / 313	0.05 (17)
X-B	-9093 / 1495	-145.3	-145.3 0.30 (1)	3.52	B-U	-1465 / 378	0.36 (2)
B-C	-7824 / 1295	-145.3	-145.3 0.41 (2)	3.68	U-D	-92 / 877	0.14 (2)
C-D	-7824 / 1295	-145.3	-145.3 0.41 (2)	3.68	D-S	-2085 / 537	0.82 (2)
D-E	-6421 / 1097	-145.3	-145.3 0.36 (2)	4.05	S-E	-313 / 1687	0.27 (2)
E-F	-6109 / 1026	-155.3	-155.3 0.34 (1)	2.00	E-Q	-380 / 1361	0.25 (9)
F-G	-6109 / 1026	-155.3	-155.3 0.34 (1)	2.00	Q-F	-1189 / 227	0.52 (1)
G-H	-6241 / 1054	-145.3	-145.3 0.35 (3)	4.10	Q-G	-267 / 1577	0.25 (2)
H-I	-7247 / 1188	-145.3	-145.3 0.39 (3)	3.82	P-G	-260 / 1382	0.22 (3)
I-J	-7247 / 1188	-145.3	-145.3 0.39 (3)	3.82	P-H	-1639 / 458	0.65 (3)
J-Z	-3631 / 669	-145.3	-145.3 0.12 (3)	5.34	N-H	-9 / 435	0.07 (6)
Z-K	-5525 / 876	-145.3	-145.3 0.11 (1)	4.52	N-J	-308 / 287	0.21 (3)
K-L	0 / 1	-145.3	-145.3 0.06 (3)	10.00	M-J	0 / 273	0.05 (17)
					J-Y	-4137 / 590	0.64 (1)
A-W	-1458 / 8162	-39.5	-39.5 0.48 (1)	6.25	W-X	-246 / 124	0.00 (1)
W-V	-1458 / 8162	-39.5	-39.5 0.50 (1)	6.25	Y-Z	-312 / 2517	0.00 (1)
V-U	-1458 / 8162	-39.5	-39.5 0.46 (1)	6.25			
U-T	-1113 / 7022	-39.5	-39.5 0.38 (1)	6.25			
T-S	-1113 / 7022	-39.5	-39.5 0.38 (1)	6.25			
S-R	-727 / 5723	-39.5	-39.5 0.32 (1)	6.25			
R-Q	-727 / 5723	-39.5	-39.5 0.32 (1)	6.25			
Q-P	-544 / 5556	-39.5	-39.5 0.31 (1)	6.25			
P-O	-808 / 6508	-39.5	-39.5 0.36 (1)	6.25			
O-N	-808 / 6508	-39.5	-39.5 0.36 (1)	6.25			
N-M	-962 / 6526	-39.5	-39.5 0.38 (1)	6.25			
M-Y	-961 / 6532	-39.5	-39.5 0.35 (1)	6.25			
Y-K	-493 / 3251	-39.5	-39.5 0.18 (1)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 43.5 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 67.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, NBC-2019AE
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.29")
ALLOWABLE DEFL.(TL)= L/180 (3.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")

CSI: TC=0.41/1.00 (B-D:2), BC=0.50/1.00 (V-W:1), WB=0.82/1.00 (D-S:2), SS=0.39/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
MI16 438 302 2547 1256 4283 1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.94 (O) (INPUT = 0.95)

CONTINUED ON PAGE 2

REVIEWED



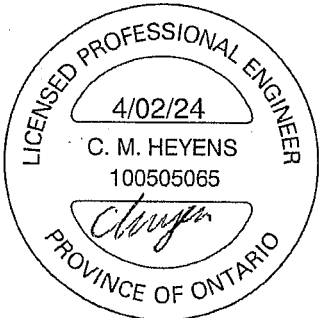
STRUCTURAL COMPONENT ONLY
DWG # TR24040201

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T13	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:33 2024 Page 2
ID:Sb_47nW9yvpKeYGkKmELfPzxU46-sFRM9Rw0XxX6UoMp2pUkp5xY0JR1IsbNyRWnsszUne8

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



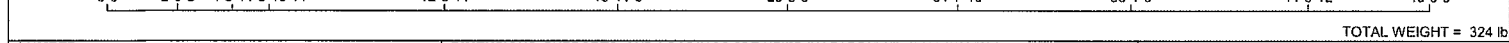
STRUCTURAL COMPONENT ONLY
DWG # TR24040201

REVIEWED

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:34 2024 Page 1

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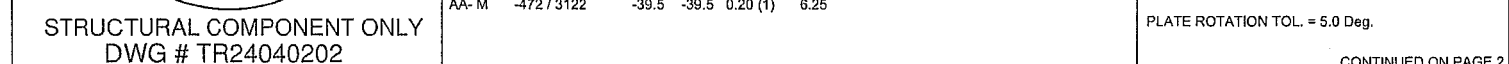


BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M	OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015
BRACING FOR SECTION G-I, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.	THIS DESIGN COMPLIES WITH:

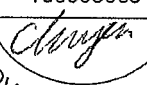
D	TTW-m	MT20	6.0	7.0	2.75	3.50	1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-U, G-U, G-S, H-S, I-S, I-R, J-R.	DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR NOT USED
E	TTWW-m	MT20	6.0	10.0	3.50	5.00		
F	TMWW-t	MT20	5.0	6.0				

[illegible]

STRUCTURAL COMPONENT ONLY
DWG # TR24040202



REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																														
437040	T14	1	1	BAYVIEW WELLINGTON																															
Tamarack Roof Truss, Burlington				Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:34 2024 Page 2																															
				ID:Sb_47nW9yypKeYGkKmlfPzxU46-KR_kNnxflFfz6yx?bX?zMIUcsinh1ljWB4FLOJzUne7																															
PLATES (table is in inches) <table><tr><td>JT</td><td>TYPE</td><td>PLATES</td><td>W</td><td>LEN</td><td>Y</td><td>X</td></tr><tr><td>V</td><td>BS-I</td><td>M116</td><td>6.0</td><td>17.5</td><td></td><td></td></tr><tr><td>W</td><td>BMWW-I</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>X</td><td>BMWW-I</td><td>MT20</td><td>6.0</td><td>10.0</td><td>2.75</td><td>5.00</td></tr></table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES: (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.		JT	TYPE	PLATES	W	LEN	Y	X	V	BS-I	M116	6.0	17.5			W	BMWW-I	MT20	5.0	6.0			X	BMWW-I	MT20	6.0	10.0	2.75	5.00	TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING <u>AS PER NBCC 4.1.6.2.(8)</u> WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.				JSI GRIP= 0.90 (D) (INPUT = 0.90) JSI METAL= 0.95 (Q) (INPUT = 0.95)	
JT	TYPE	PLATES	W	LEN	Y	X																													
V	BS-I	M116	6.0	17.5																															
W	BMWW-I	MT20	5.0	6.0																															
X	BMWW-I	MT20	6.0	10.0	2.75	5.00																													
LICENSED PROFESSIONAL ENGINEER 4/02/24 C. M. HEYENS 100505065  PROVINCE OF ONTARIO STRUCTURAL COMPONENT ONLY DWG # TR24040202																																			

REVIEWED

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T15	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BS-t	MI16	6.0	17.5		
X	BMWWW-t	MT20	6.0	10.0	2.75	5.00
Y	BMW+w	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (8)

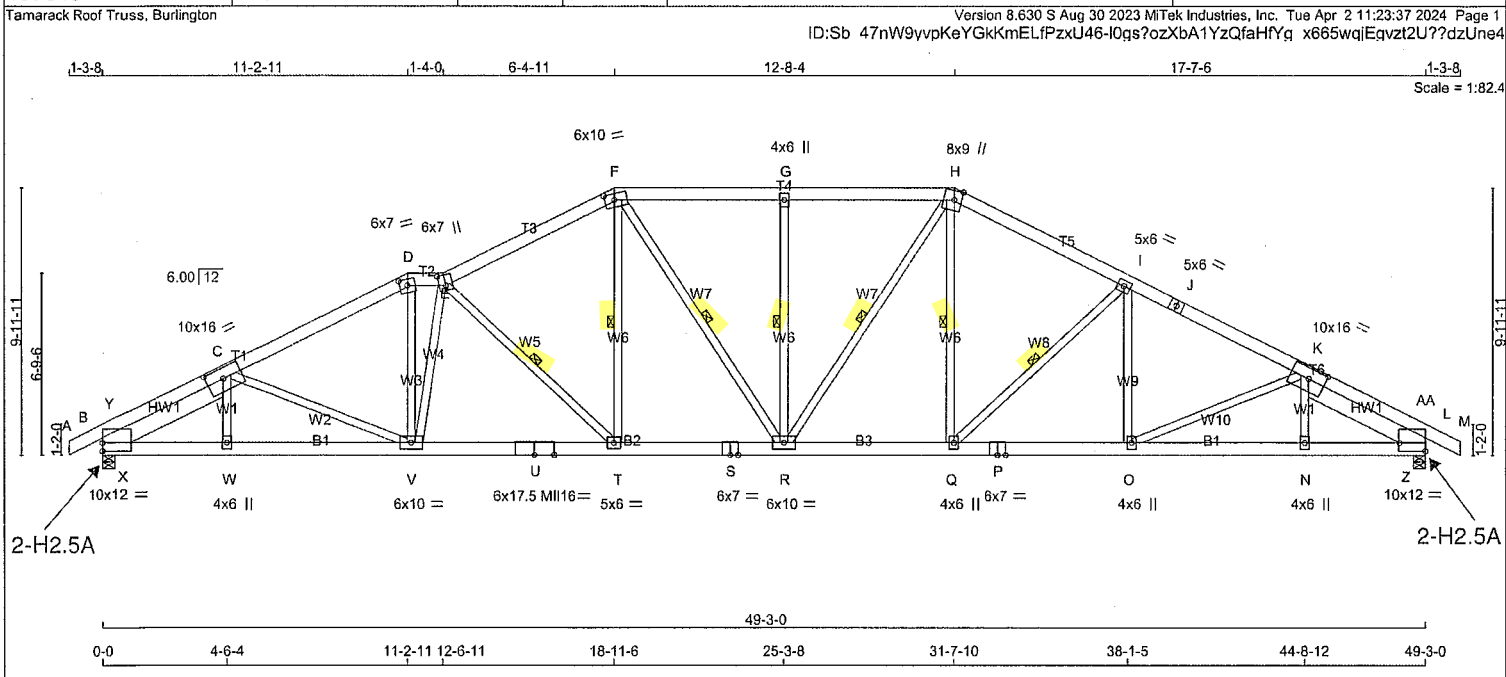
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCq, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.95 (Q) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040203

REVIEWED



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2 SPF
D - E	2x6	DRY	No.2 SPF
E - F	2x6	DRY	No.2 SPF
F - H	2x6	DRY	No.2 SPF
H - J	2x6	DRY	No.2 SPF
J - M	2x6	DRY	No.2 SPF
B - U	2x6	DRY	2100F 1.8E SPF
U - S	2x6	DRY	2100F 1.8E SPF
S - P	2x6	DRY	2100F 1.8E SPF
P - L	2x6	DRY	2100F 1.8E SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2 SPF
HW2	2x8	DRY	No.2 SPF

ALL WEBS EXCEPT

V - E	2x4	DRY	2100F 1.8E SPF
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DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
B	4816	0	4816	209
L	4819	0	4819	0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 815 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 798 LBS. FACTORED UPLIFT

PROVIDE FOR 209 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
B	3504	2261 / 0
L	3507	2261 / 0

HORIZONTAL REACTIONS

B	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 43.5 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 67.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.30")
ALLOWABLE DEFL.(TL) = L/180 (3.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.86/1.00 (I-K:1), BC=0.38/1.00 (T-V:1), WB=0.74/1.00 (E-T:2), SSI=0.39/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
MI16 438 302 2547 1256 4283 1816

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75
C	TMWVW-t	MT20	10.0	16.0	4.50
D	TTW-m	MT20	6.0	7.0	2.75
E	TTWVW+m	MT20	6.0	7.0	4.75
F	TTWVW-m	MT20	6.0	10.0	2.75
G	TMWVW	MT20	4.0	6.0	
H	TTWVW+m	MT20	8.0	9.0	Edge 3.25
I	TMWVW-t	MT20	5.0	6.0	
J	TS-I	MT20	5.0	6.0	
K	TMBMWV-t	MT20	10.0	16.0	4.75
L	TMBMW1-I	MT20	10.0	12.0	3.75
N	BMVW-w	MT20	4.0	6.0	
O	BMVWV-t	MT20	4.0	6.0	
P	BS-I	MT20	6.0	7.0	
Q	BMVWV-t	MT20	4.0	6.0	
R	BMVWVW-t	MT20	6.0	10.0	
S	BS-I	MT20	6.0	7.0	
T	BMVWV-t	MT20	5.0	6.0	
U	BS-I	MI16	6.0	17.5	

STRUCTURAL COMPONENT ONLY
DWG # TR24040204

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T16	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID:Sb 47nW9yypKeYGkKmELfPzxU46-lQgs?ozXbA1YzQfaHfYg x665wqjEgvzt2U??dzUne4

PLATES (table is in inches)							WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.		JSI GRIP= 0.88 (F) (INPUT = 0.90) JSI METAL= 0.95 (P) (INPUT = 0.95)					
JT	TYPE	PLATES	W	LEN	Y	X								
V	BMWWW-t	MT20	6.0	10.0										
W	BMW+w	MT20	4.0	6.0										
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.														
NOTES- (1)														
1) Lateral braces to be a minimum of 2X4 SPF #2.														
LICENSED PROFESSIONAL ENGINEER 4/02/24 C. M. HEYENS 100505065 <i>Chayen</i> PROVINCE OF ONTARIO STRUCTURAL COMPONENT ONLY DWG # TR24040204														



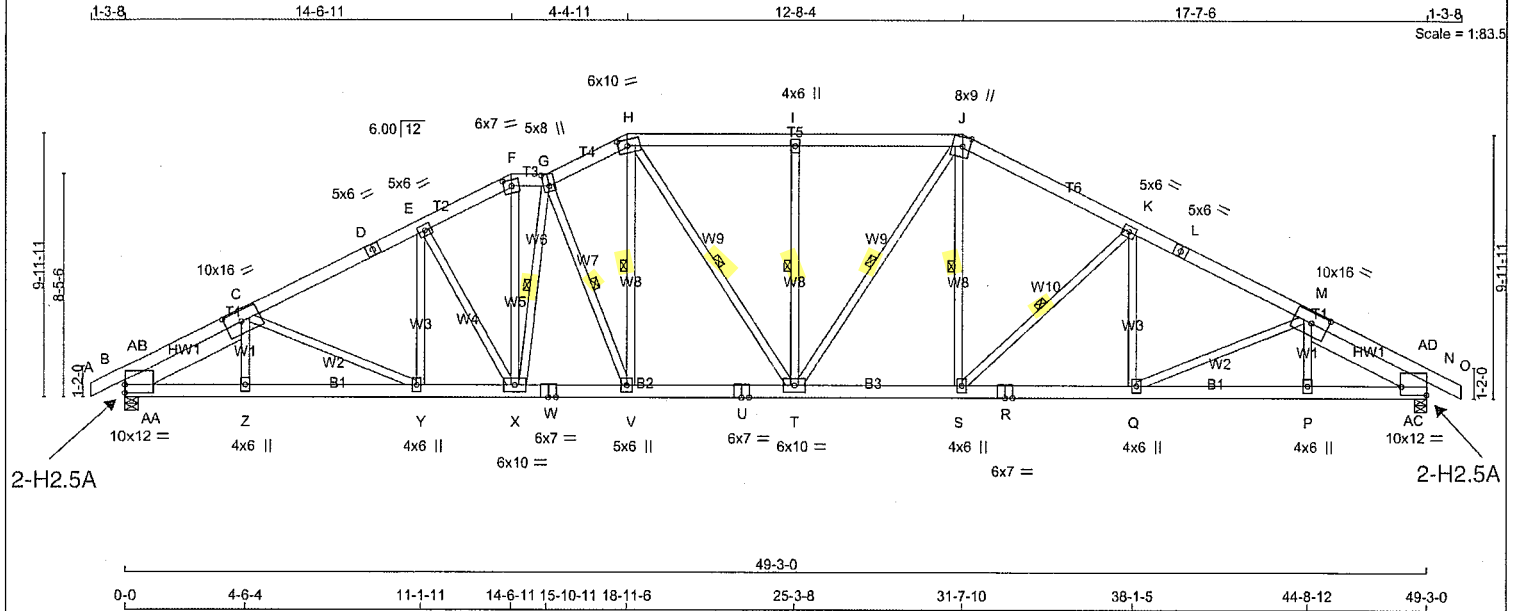
STRUCTURAL COMPONENT ONLY
DWG # TR24040204

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T17	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID:Sb 47nW9vvpKeY GkKmELfPzXU46-hOodQU?n6nHGcJpzO4b83MBSbkWDIYtGkMz64WzUne2



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
F - G	2x6	DRY	No.2
G - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
L - O	2x6	DRY	No.2
B - W	2x6	DRY	2100F 1.8E
W - U	2x6	DRY	2100F 1.8E
U - R	2x6	DRY	2100F 1.8E
R - N	2x6	DRY	2100F 1.8E

REINFORCING MEMBERS			
HW1	2x8	DRY	No.2
HW2	2x8	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
X - G	2x4	DRY	2100F 1.8E

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75
C	TMBWW1-t	MT20	10.0	16.0	4.75 7.50
D	TS-I	MT20	5.0	6.0	
E	TMBWW-t	MT20	5.0	6.0	
F	TTW-m	MT20	6.0	7.0	2.75 3.50
G	TTWW+m	MT20	5.0	8.0	5.50 2.50
H	TTWW-m	MT20	6.0	10.0	2.75 4.25
I	TMW+w	MT20	4.0	6.0	
J	TTWW+m	MT20	8.0	9.0	Edge 3.25
K	TMBWW-I	MT20	5.0	6.0	
L	TS-I	MT20	5.0	6.0	
M	TMBWW-t	MT20	10.0	16.0	4.75 7.50
N	TMBMW1-I	MT20	10.0	12.0	3.75 Edge
P	BMW+w	MT20	4.0	6.0	
Q, S, Y					
Q	BMW+w	MT20	4.0	6.0	
R, U, W					
R	BS-I	MT20	6.0	7.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT DOWN	REQRD IN-SX	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	4816	0	4816	209	-815
N	4819	0	4819	0	-798

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 815 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 798 LBS. FACTORED UPLIFT

PROVIDE FOR 209 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS						
	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
B	3504	2261 / 0	517 / 0	0 / 0	122 / -940	726 / 0
N	3507	2261 / 0	517 / 0	0 / 0	114 / -929	729 / 0
						SOIL
						0 / 0

HORIZONTAL REACTIONS			
B	---	0 / 0	0 / 0
N	---	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N

BRACING

FOR SECTION H-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-V, H-T, I-T, J-T, J-S, K-S, G-X, G-V.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO				FR-TO			
A-B	0 / 1	-145.3	-145.3	0.11 (2)	10.00	Z-C	0 / 299
B-AB	-5351 / 868	-145.3	-145.3	0.19 (1)	3.69	C-Y	-402 / 221
AB-C	-3433 / 644	-145.3	-145.3	0.34 (2)	4.34	Y-E	-30 / 400
C-D	-7210 / 1214	-145.3	-145.3	0.87 (1)	2.83	V-H	-381 / 1864
D-E	-7210 / 1214	-145.3	-145.3	0.87 (1)	2.83	H-T	-369 / 1374
E-F	-6708 / 1202	-145.3	-145.3	0.48 (2)	3.05	T-I	-1193 / 228
F-G	-5998 / 1111	-145.3	-145.3	0.17 (1)	3.51	T-J	-268 / 1582
G-H	-8373 / 1133	-145.3	-145.3	0.28 (1)	3.30	S-J	-280 / 1381
H-I	-6112 / 1026	-155.3	-155.3	0.73 (1)	2.00	S-K	-1637 / 458
I-J	-8112 / 1026	-155.3	-155.3	0.73 (1)	2.00	Q-K	-9 / 438
J-K	-6240 / 1054	-145.3	-145.3	0.75 (1)	2.90	Q-M	-329 / 275
K-L	-7248 / 1189	-145.3	-145.3	0.86 (1)	2.58	P-M	0 / 286
L-M	-7248 / 1189	-145.3	-145.3	0.86 (1)	2.58	X-F	-455 / 2694
M-AD	-3476 / 645	-145.3	-145.3	0.27 (3)	4.39	E-X	-1253 / 353
AD-N	-5363 / 848	-145.3	-145.3	0.22 (1)	3.67	X-G	-1706 / 270
N-O	0 / 1	-145.3	-145.3	0.11 (3)	10.00	G-V	-1777 / 464
B-AA	-684 / 3096	-39.5	-39.5	0.20 (1)	6.25	AA-AB	-338 / 2546
AA-Z	-1207 / 6579	-39.5	-39.5	0.37 (1)	6.25	AA-C	-4391 / 659
Z-Y	-1209 / 6572	-39.5	-39.5	0.38 (1)	6.25	M-AC	-4324 / 621
Y-X	-1031 / 6450	-39.5	-39.5	0.33 (1)	6.25	AC-AD	-307 / 2500
X-W	-892 / 6260	-39.5	-39.5	0.32 (1)	6.25		
W-V	-892 / 6260	-39.5	-39.5	0.32 (1)	6.25		
V-U	-727 / 5716	-39.5	-39.5	0.31 (1)	6.25		
U-T	-727 / 5716	-39.5	-39.5	0.31 (1)	6.25		
T-S	-544 / 5556	-39.5	-39.5	0.31 (1)	6.25		
S-R	-808 / 6507	-39.5	-39.5	0.36 (1)	6.25		
R-Q	-808 / 6507	-39.5	-39.5	0.36 (1)	6.25		
Q-P	-966 / 6546	-39.5	-39.5	0.38 (1)	6.25		
P-AC	-964 / 6552	-39.5	-39.5	0.37 (1)	6.25		
AC-N	-472 / 3122	-39.5	-39.5	0.20 (1)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 43.5 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 67.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.64")
CALCULATED VERT. DEFL.(LL)= L/999 (0.30")
ALLOWABLE DEFL.(TL)= L/180 (3.28")
CALCULATED VERT. DEFL.(TL)= L/999 (0.40")

CSI: TC=0.86/1.00 (K-M:1), BC=0.38/1.00 (Y-Z:1),
WB=0.84/1.00 (E-X:2), SSI=0.39/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

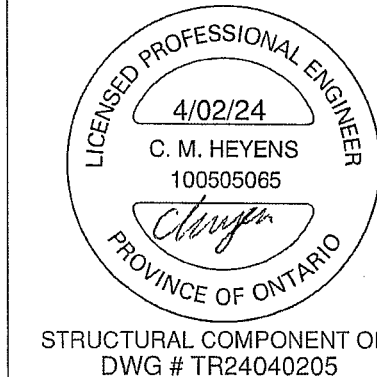
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

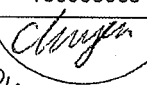
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																			
437040	T17	1	1	BAYVIEW WELLINGTON																																				
Tamarack Roof Truss, Burlington				Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:39 2024 Page 2																																				
				ID:Sb 47nW9yvpKeYGkKmELfPzxU46-hOodQU?n6nHGCjzO4b83MBSbkWDIYtGKMz64WzUne2																																				
PLATES (table is in inches) <table><tr><td>JT</td><td>TYPE</td><td>PLATES</td><td>W</td><td>LEN</td><td>Y</td><td>X</td></tr><tr><td>T</td><td>BMW+V-t</td><td>MT20</td><td>6.0</td><td>10.0</td><td></td><td></td></tr><tr><td>V</td><td>BMW+V-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>X</td><td>BMW+V-t</td><td>MT20</td><td>6.0</td><td>10.0</td><td></td><td></td></tr><tr><td>Z</td><td>BMW+w</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr></table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES: (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.		JT	TYPE	PLATES	W	LEN	Y	X	T	BMW+V-t	MT20	6.0	10.0			V	BMW+V-t	MT20	5.0	6.0			X	BMW+V-t	MT20	6.0	10.0			Z	BMW+w	MT20	4.0	6.0			TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8) WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY. JSI GRIP= 0.89 (H) (INPUT = 0.90) JSI METAL= 0.95 (R) (INPUT = 0.95)			
JT	TYPE	PLATES	W	LEN	Y	X																																		
T	BMW+V-t	MT20	6.0	10.0																																				
V	BMW+V-t	MT20	5.0	6.0																																				
X	BMW+V-t	MT20	6.0	10.0																																				
Z	BMW+w	MT20	4.0	6.0																																				
LICENSED PROFESSIONAL ENGINEER 4/02/24 C. M. HEYENS 100505065  PROVINCE OF ONTARIO STRUCTURAL COMPONENT ONLY DWG # TR24040205																																								

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T18	5	1	BAYVIEW WELLINGTON	

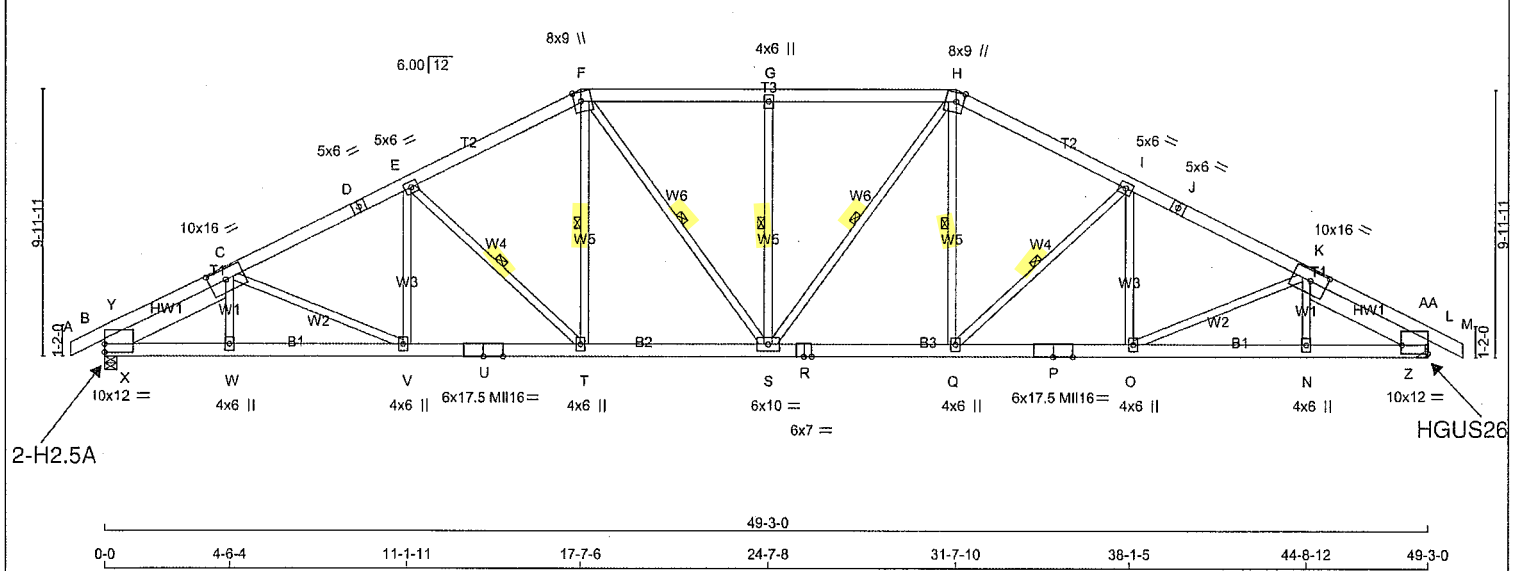
Tamarack Roof Truss, Burlington

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ID:Sb 47nW9yvpKeYGkKmELfPzxU46-9bM?dq?Pi5P7qtO9yn6NcZkce8sWR28PZ0ifcyzUne1

1-3-8 17-7-6 14-0-4 17-7-6 1-3-8

Scale = 1:82.4



TOTAL WEIGHT = 5 X 324 = 1621 lb		[M]	
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS	
A - D 2x6 DRY No.2 SPF D - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF J - M 2x6 DRY No.2 SPF B - U 2x6 DRY 2100F 1.8E SPF U - R 2x6 DRY 2100F 1.8E SPF R - P 2x6 DRY 2100F 1.8E SPF P - L 2x6 DRY 2100F 1.8E SPF		FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX B 4824 0 4824 209 -802 5-8 5-4 L 4824 0 4824 0 -802 MECHANICAL	
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 4-4.			
REINFORCING MEMBERS HW1 2x8 DRY No.2 SPF HW2 2x8 DRY No.2 SPF		PROVIDE ANCHORAGE AT BEARING JOINT B FOR 802 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT L FOR 802 LBS FACTORED UPLIFT	
ALL WEBS 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.		PROVIDE FOR 209 LBS FACTORED HORIZONTAL REACTION AT JOINT B	
PLATES (table is in inches)		UNFACTORED REACTIONS	
JT TYPE PLATES W LEN Y X		1ST LCASE MAX/MIN. COMPONENT REACTIONS	
B TMBMW-1 MT20 10.0 12.0 3.75		JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL	
D TMBWWW-t MT20 10.0 16.0 4.75 7.50		B 3510 2261 / 0 517 / 0 0 / 0 118 / -935 733 / 0 0 / 0	
C TS-1 MT20 5.0 6.0		L 3510 2261 / 0 517 / 0 0 / 0 118 / -935 733 / 0 0 / 0	
D TMBWW-t MT20 5.0 6.0		HORIZONTAL REACTIONS	
F TTBWW-t MT20 8.0 9.0 Edge 3.25		B 0 / 0 0 / 0 0 / 0 149 / -149 0 / 0 0 / 0	
G TTBWW-w MT20 4.0 6.0		BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B	
H TTBWW-m MT20 8.0 9.0 Edge 3.25		BRACING	
I TMBWW-t MT20 5.0 6.0		FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.	
J TS-1 MT20 5.0 6.0		FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.58 FT.	
K TMBWWW-t MT20 10.0 16.0 4.75 7.50		MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.	
L TMBMW-1 MT20 10.0 12.0 3.75 Edge		ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.	
N TMBWW-w MT20 4.0 6.0		1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T, F-T, F-S, G-S, H-S, H-Q, I-Q.	
O, Q, T, V		LOADING	
O BMBWW-t MT20 4.0 6.0		TOTAL LOAD CASES: (18)	
P BS-1 MI16 6.0 17.5		CHORDS	
R BS-1 MT20 6.0 7.0		MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. CSI (LC) UNBRAC	
S BMBWWW-t MT20 6.0 10.0		WEBS	
U BS-1 MI16 6.0 17.5		MEMB. MAX. FACTORED FORCE (LBS) MAX. CSI (LC)	
W BMBWW-w MT20 4.0 6.0		FR-TO FROM TO FR-TO	
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.		A-B 0 / 1 -145.3 -145.3 0.11 (2) 10.00 W-C 0 / 288 0.05 (17)	
		B-Y -5368 / 852 -145.3 -145.3 0.22 (1) 3.66 C-V -314 / 260 0.22 (2)	
		Y-C -3479 / 647 -145.3 -145.3 0.27 (2) 4.39 V-E -9 / 428 0.07 (5)	
		C-D -7257 / 1196 -145.3 -145.3 0.86 (2) 2.58 E-T -1627 / 458 0.64 (2)	
		D-E -7257 / 1196 -145.3 -145.3 0.86 (2) 2.58 T-F -258 / 1392 0.22 (2)	
		E-F -6253 / 1062 -145.3 -145.3 0.75 (1) 2.89 F-S -261 / 1560 0.25 (3)	
		F-G -6148 / 1012 -155.3 -155.3 0.90 (1) 2.00 S-G -1328 / 254 0.58 (1)	
		G-H -6148 / 1012 -155.3 -155.3 0.90 (1) 2.00 S-H -261 / 1560 0.25 (2)	
		H-I -6253 / 1062 -145.3 -145.3 0.75 (1) 2.89 Q-H -258 / 1392 0.22 (3)	
		I-J -7257 / 1196 -145.3 -145.3 0.86 (3) 2.58 Q-I -1627 / 458 0.64 (3)	
		J-K -7257 / 1196 -145.3 -145.3 0.86 (3) 2.58 O-I -9 / 428 0.07 (6)	
		K-AA -3479 / 648 -145.3 -145.3 0.27 (3) 4.39 O-K -314 / 260 0.22 (3)	
		AA-L -5368 / 853 -145.3 -145.3 0.22 (1) 3.66 N-K 0 / 288 0.05 (17)	
		L-M 0 / 1 -145.3 -145.3 0.11 (3) 10.00 X-Y -308 / 2502 0.00 (1)	
		B-X -683 / 3125 -39.5 -39.5 0.20 (1) 6.25 X-C -4331 / 623 0.67 (1)	
		X-W -1177 / 6560 -39.5 -39.5 0.37 (1) 6.25 K-Z -4331 / 625 0.67 (1)	
		W-V -1178 / 6554 -39.5 -39.5 0.38 (1) 6.25 Z-AA -309 / 2502 0.00 (1)	
		V-U -1024 / 6514 -39.5 -39.5 0.37 (1) 6.25	
		U-T -1024 / 6514 -39.5 -39.5 0.37 (1) 6.25	
		T-S -695 / 5567 -39.5 -39.5 0.32 (1) 6.25	
		S-R -523 / 5567 -39.5 -39.5 0.32 (1) 6.25	
		R-Q -523 / 5567 -39.5 -39.5 0.32 (1) 6.25	
		Q-P -815 / 6514 -39.5 -39.5 0.37 (1) 6.25	
		P-O -815 / 6514 -39.5 -39.5 0.37 (1) 6.25	
		O-N -971 / 6554 -39.5 -39.5 0.38 (1) 6.25	
		N-Z -970 / 6560 -39.5 -39.5 0.37 (1) 6.25	
		Z-L -475 / 3125 -39.5 -39.5 0.20 (1) 6.25	
DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH. LL = 43.5 PSF			
DL = 6.0 PSF			
BOT CH. LL = 10.5 PSF			
DL = 7.4 PSF			
TOTAL LOAD = 67.3 PSF			
SPACING = 24.0 IN. C/C			
LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.			
THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015			
THIS DESIGN COMPLIES WITH:			
- PART 4 OF CBC 2018, NBC-2019AE			
- PART 4 OF CBC 2012 (2019 AMENDMENT)			
- CSA 086-14			
- TPIC 2014			
DESIGN ASSUMPTIONS			
- SLOPE REDUCTION FACTOR NOT USED			
(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL.(LL)= L/360 (1.64")			
CALCULATED VERT. DEFL.(LL)= L/999 (0.28")			
ALLOWABLE DEFL.(TL)= L/180 (3.28")			
CALCULATED VERT. DEFL.(TL)= L/999 (0.38")			
CSI: TC=0.90/1.00 (G-H:1), BC=0.38/1.00 (N-O:1), WB=0.67/1.00 (C-X:1), SSI=0.43/1.00 (F-G:2)			
DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
SNOW LOAD IMPORTANCE FACTOR = 1.00			
WIND LOAD IMPORTANCE FACTOR = 1.00			
LIVE LOAD IMPORTANCE FACTOR = 1.00			
COMPANION LIVE LOAD FACTOR = 1.00			
AUTOSOLVE HEELS OFF			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
NAIL VALUES			
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
MAX MIN MAX MIN MAX MIN			
MT20 650 371 1747 788 1987 1873			
MI16 438 302 2547 1256 4283 1816			
PLATE PLACEMENT TOL. = 0.250 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JSI GRIP= 0.84 (Q) (INPUT = 0.90)			
JSI METAL= 0.80 (R) (INPUT = 0.95)			

LICENSED PROFESSIONAL ENGINEER

4/02/24

C. M. HEYENS

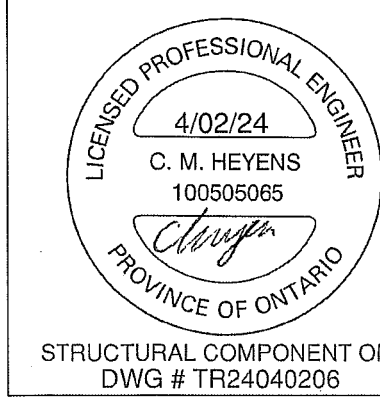
100505065

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY

DWG # TR24040206

CONTINUED ON PAGE 2



REVIEWED

CONTINUED ON PAGE 2

JOB NAME 437040	TRUSS NAME T18	QUANTITY 5	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:40 2024 Page 2 ID:Sb_47nW9yvpKeYGkMELfPzxU46-9bM?dq?Pt5P7qtO9yn6NcZkce8sWR28PZ0ifcyzUne1			

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCd, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



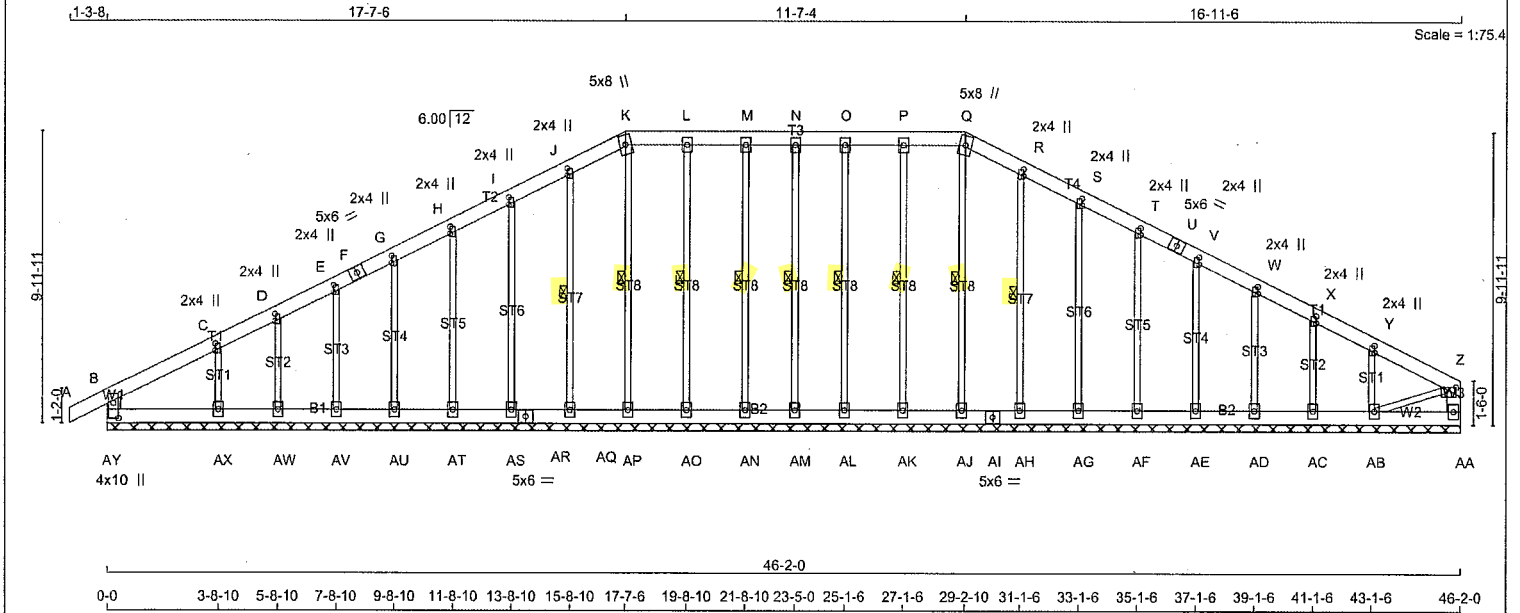
STRUCTURAL COMPONENT ONLY
DWG # TR24040206

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T19G	2	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:42 2024 Page 1
ID:Sb 47nW9yvpKeYGkKMLfPzxU46-5zTl2W1gPifr3BYy3C8rh p9sxdTv3ti0KBmqrzUne?



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS SIZE LUMBER DESCR.				THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				TOP CH. LL = 32.5 PSF			
A - F 2x6 DRY No.2 SPF								DL = 6.0 PSF			
F - K 2x6 DRY No.2 SPF								BOT CH. LL = 0.0 PSF			
K - Q 2x6 DRY No.2 SPF				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				DL = 7.4 PSF			
Q - U 2x6 DRY No.2 SPF								TOTAL LOAD = 45.9 PSF			
U - Z 2x6 DRY No.2 SPF											
AY- B 2x6 DRY No.2 SPF				BRACING				SPACING = 240 IN. C/C			
AA- Z 2x6 DRY No.2 SPF				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
AY- AR 2x6 DRY No.2 SPF				MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
AR- AI 2x6 DRY No.2 SPF				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				THIS DESIGN COMPLIES WITH:			
AI - AA 2x6 DRY No.2 SPF				1 LATERAL BRACE(S) AT 1/2 LENGTH OF Q-AJ, K-AP, L-AO, M-AN, N-AM, O-AL, P-AK, J-AQ, R-AH.				- PART 9 OF CBC 2018, NBC-2019AE			
ALL WEBS 2x3 DRY No.2 SPF				END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
ALL GABLE WEBS 2x3 DRY No.2 SPF								- CSA 086-14			
DRY: SEASONED LUMBER.								- TPIC 2014			
GABLE STUDS SPACED AT 200-0-0 OC.								(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
								CSI: TC=0.09/1.00 (B-C:1), BC=0.03/1.00 (AW-AX:4), WB=0.24/1.00 (I-AS:1), SSI=0.13/1.00 (B-C:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
								COMPANION LIVE LOAD FACTOR = 1.00			
								AUTOSOLVE HEELS OFF			
								TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 650 371 1747 788 1987 1873			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.75 (AB) (INPUT = 0.90)			
								JSI METAL= 0.16 (C) (INPUT = 0.95)			

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T19G	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:42 2024 Page 2

ID:Sb 47nW9yvpKeYGkKmELfPzxU46-5zTI2W1gPifr3BYY3C8rh p9sxdTv3ti0KBmgrzUne?

NOTES-
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
AG-AF	-34 / 0	-18.5	-18.5	0.01 (4)	6.25				
AF-AE	-30 / 0	-18.5	-18.5	0.01 (4)	6.25				
AE-AD	-26 / 0	-18.5	-18.5	0.01 (4)	6.25				
AD-AC	-21 / 0	-18.5	-18.5	0.01 (4)	6.25				
AC-AB	-15 / 0	-18.5	-18.5	0.02 (4)	6.25				
AB-AA	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The outer ring contains the text "LICENSED PROFESSIONAL ENGINEER" at the top and "PROVINCE OF ONTARIO" at the bottom. Inside the ring, the date "4/02/24" is at the top, the name "C. M. HEYENS" is in the center, and the license number "100505065" is at the bottom. A signature is written across the bottom half of the seal.

STRUCTURAL COMPONENT ONLY
DWG # TR24040207

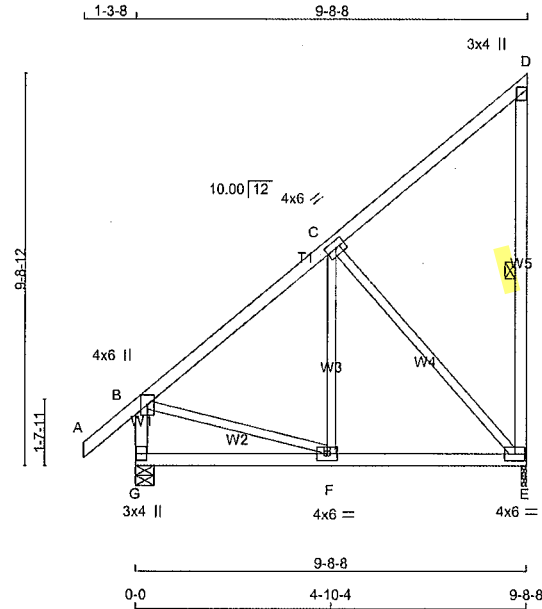


STRUCTURAL COMPONENT ONLY
DWG # TR24040207

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T20	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:43 2024 Page 1
ID:Sb 47nW9yvpKeYGkKmELfPzxU46-ZA18Gs2IA0nihL7kdwf4DCMGZLxqeRvrF xJDHzUne



TOTAL WEIGHT = 2 X 53 = 106 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	636	0	636	0	0	1-8	1-8		
G	791	0	791	0	0	5-8	1-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
E	445	315 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0	
G	552	405 / 0	0 / 0	0 / 0	0 / 0	147 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 50	-112.4	-112.4 0.15 (1)	10.00	F-C	0 / 97	0.03 (4)
B-C	-423 / 0	-112.4	-112.4 0.35 (1)	6.25	C-E	-533 / 0	0.51 (1)
C-D	-43 / 0	-112.4	-112.4 0.34 (1)	6.25	B-F	0 / 371	0.08 (1)
E-D	-205 / 0	0.0	0.0 0.09 (1)	6.25			
G-B	-756 / 0	0.0	0.0 0.08 (1)	7.81			
G-F	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
F-E	0 / 358	-18.5	-18.5 0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.35/1.00 (B-C:1), BC=0.15/1.00 (E-F:4), WB=0.51/1.00 (C-E:1), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040208

REVIEWED

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 57 lb

DESCR.	QTY	UNIT	PRICE	TOTAL
SPF				
SPF				
SPF				
SPF				

SPF
SPF

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPE NO.2 OR BETTER AT JOINT(S)

BRACING

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY ABOVE

ALL RITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1. LATERAL BRACE(S) AT 1/3 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
M-B	-330 / 0	0.0	0.0	0.04 (1)	7.81	I-F	-254 / 0 0.32 (1)
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	J-E	-214 / 0 0.14 (1)
B-C	-58 / 0	-112.4	-112.4	0.15 (1)	6.25	K-D	-239 / 0 0.08 (1)
C-D	-6 / 0	-112.4	-112.4	0.06 (1)	10.00	L-C	-127 / 0 0.02 (1)
D-E	-11 / 0	-112.4	-112.4	0.06 (1)	6.25	B-L	0 / 20 0.00 (1)
E-F	-1 / 0	-112.4	-112.4	0.06 (1)	10.00		
F-G	-14 / 0	-112.4	-112.4	0.06 (1)	6.25		
H-G	-96 / 0	0.0	0.0	0.04 (1)	6.25		
M-L	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
L-K	0 / 12	-18.5	-18.5	0.02 (4)	10.00		
K-J	0 / 6	-18.5	-18.5	0.01 (4)	10.00		
J-I	0 / 3	-18.5	-18.5	0.02 (4)	10.00		
I-H	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9. NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , NBC-2019AE
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (H-I:4),
WB=0.32/1.00 (F-I:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

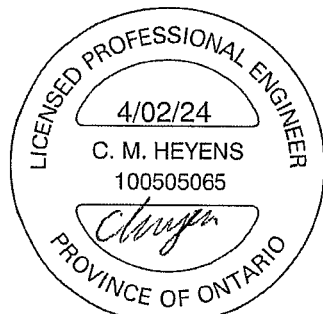
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL: = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

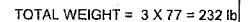
JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040209

REVIEWED

Tamarack Roof Truss, Burlington



10



REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T21	1	3	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID:Sb 47nW9yvpKeYGKkMELfPzxU46- lJGut4ATx9GYorJI2Dnrq pjZwErncly9zoczUndx

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	6.0	7.0	Edge	
B	TMWW-t	MT20	4.0	6.0	2.00	1.50
C	TMWW-t	MT20	5.0	8.0	2.25	2.25
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1+p	MT20	10.0	12.0	Edge	3.00
F	BMWW-t	MT20	6.0	10.0	5.50	3.00
G	BMWW-t	MT20	6.0	10.0	5.50	2.50
H	BMV1+p	MT20	6.0	12.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE,TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



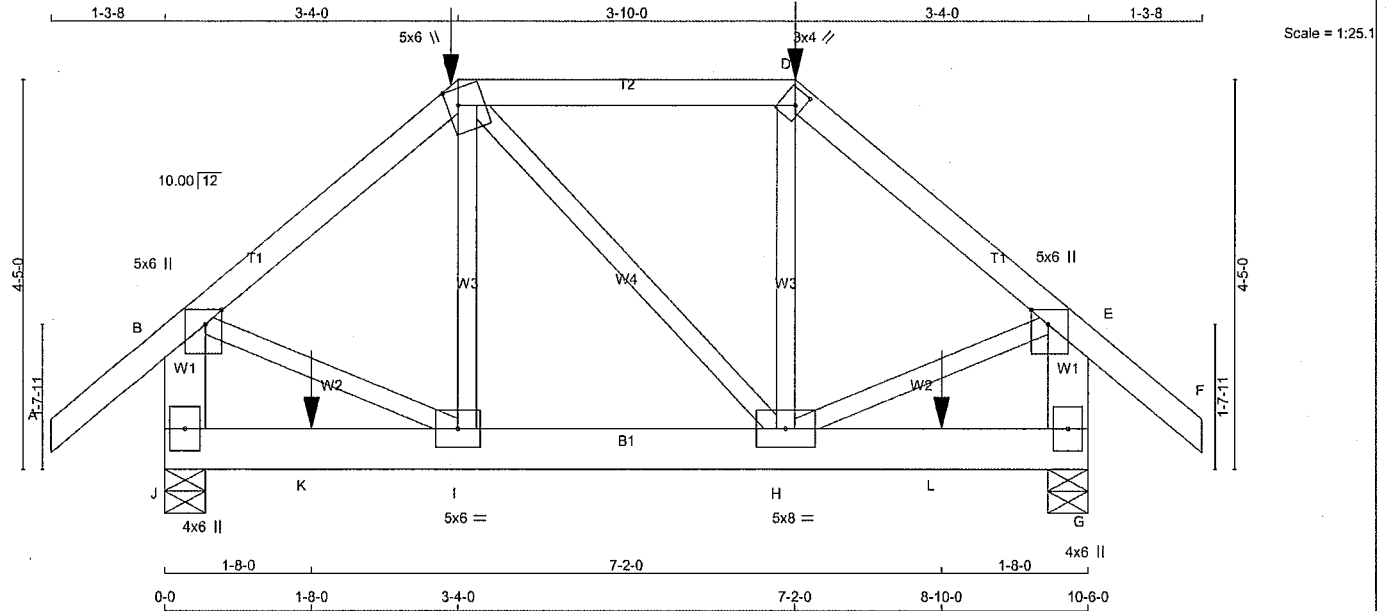
STRUCTURAL COMPONENT ONLY
DWG # TR24040210

REVIEWED

JOB NAME 437040	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:47 2024 Page 1
ID:Sb 47nW9yvpKeYgKkMELfPzxU46-SxHe5D5oEEI7AyQVslk0O2WxEyGRaKeRacvXL2zUndw



TOTAL WEIGHT = 57 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - D 2x4 DRY No.2

D - F 2x4 DRY No.2

J - B 2x6 DRY No.2

G - E 2x6 DRY No.2

J - G 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES

B TMVW+p MT20 5.0 6.0 2.00 2.25

C TTWW+m MT20 5.0 6.0 2.25 1.50

D TTWW+h MT20 3.0 4.0 2.00 1.00

E TMVW+p MT20 5.0 6.0 2.00 2.25

G BMV1+p MT20 4.0 6.0

H BMVW+t MT20 5.0 6.0

I BMVW+t MT20 5.0 6.0

J BMV1+p MT20 4.0 6.0

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
J	1557	0	1557	0
G	1557	0	1557	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1087	793/0	0/0	0/0	0/0	0/0	294/0	0/0
G	1087	793/0	0/0	0/0	0/0	0/0	294/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/50	-112.4 -112.4	0.17 (1)	I-C	0/96	0.04 (4)	
B-C	-1039/0	-112.4 -112.4	0.26 (1)	C-H	0/1	0.00 (4)	
C-D	-798/0	-149.9 -149.9	0.43 (1)	H-D	0/97	0.04 (4)	
D-E	-1039/0	-112.4 -112.4	0.26 (1)	B-I	0/845	0.21 (1)	
E-F	0/50	-112.4 -112.4	0.17 (1)	H-E	0/844	0.21 (1)	
J-B	-1294/0	0.0 0.0	0.10 (1)				
G-E	-1294/0	0.0 0.0	0.10 (1)				
J-K	0/0	-24.7 -24.7	0.30 (1)				
K-I	0/0	-24.7 -24.7	0.30 (1)				
I-H	0/798	-24.7 -24.7	0.19 (1)				
H-L	0/0	-24.7 -24.7	0.30 (1)				
L-G	0/0	-24.7 -24.7	0.30 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-4-0	-78	-78	---	FRONT	VERT	TOTAL	---	C1
D	7-2-0	-78	-78	---	FRONT	VERT	TOTAL	---	C1
K	1-8-0	-345	-345	---	FRONT	VERT	TOTAL	---	C1
L	8-10-0	-345	-345	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-4-0
END SETBACK = 3-4-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
-ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

GSI: TC=0.43/1.00 (C-D:1), BC=0.30/1.00 (G-H:1), WB=0.21/1.00 (B-I:1), SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR24040211

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T22	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:47 2024 Page 2
ID:Sb 47nW9yvpKeYGkKmELfPzxU46-SxHe5D5oEEI7AyQVslk0O2WxEyGRaKeRAcvXL2zUndw

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 0.95)



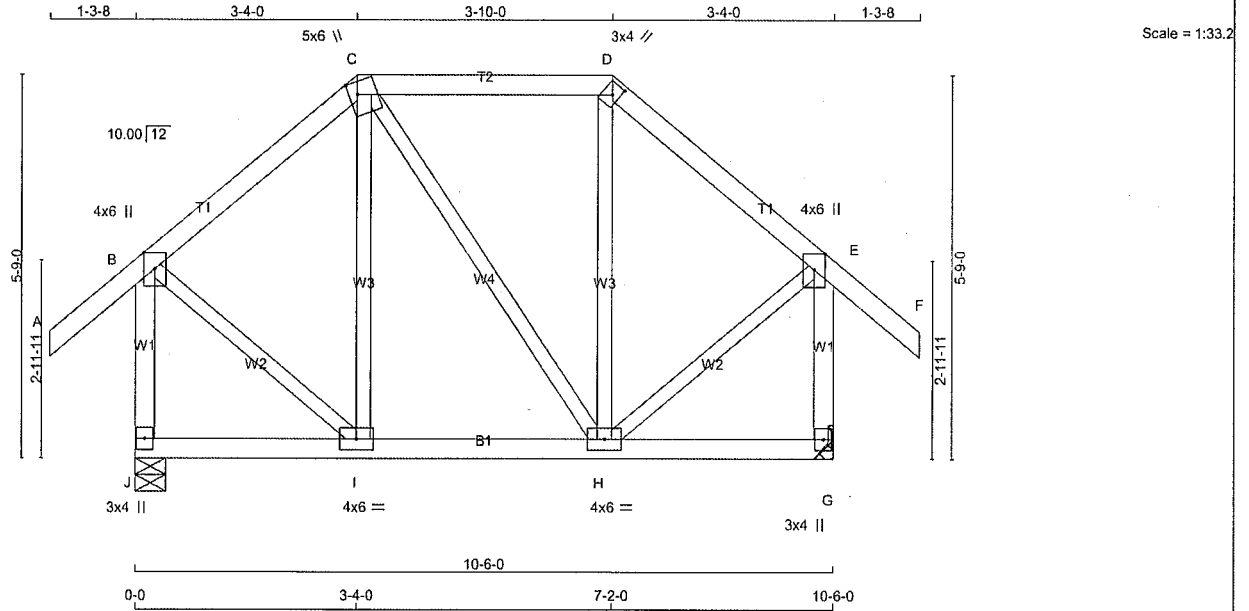
STRUCTURAL COMPONENT ONLY
DWG # TR24040211

REVIEWED

JOB NAME 437040	TRUSS NAME T23	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 Mittek Industries, Inc. Tue Apr 2 11:23:49 2024 Page 1
ID:Sb 47nW9yvpKeYgKkMELfPzU46-OKOPWv63msYrPGauzAmUTTbK8m?N2G6kdwOdPzU46



TOTAL WEIGHT = 2 X 56 = 113 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BMWW+t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	843	0	843	0
G	843	0	843	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
J	588	431/0	0/0	0/0	0/0	0/0
G	588	431/0	0/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD LC1	MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/50	-112.4	-112.4 0.15 (1)	I-C	-154/13	0.08 (1)	
B-C	-385/0	-112.4	-112.4 0.16 (1)	C-H	0/0	0.00 (1)	
C-D	-292/0	-112.4	-112.4 0.21 (1)	H-D	-156/13	0.08 (1)	
D-E	-385/0	-112.4	-112.4 0.16 (1)	B-I	0/372	0.08 (1)	
E-F	0/50	-112.4	-112.4 0.15 (1)	H-E	0/371	0.08 (1)	
J-B	-817/0	0.0	0.0 0.13 (1)				
G-E	-817/0	0.0	0.0 0.13 (1)				
J-I	0/0	-18.5	-18.5 0.05 (4)				
I-H	0/292	-18.5	-18.5 0.07 (1)				
H-G	0/0	-18.5	-18.5 0.05 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (C-D-1), BC=0.07/1.00 (H-I-1),
WB=0.08/1.00 (B-I-1), SSI=0.17/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 0.95)



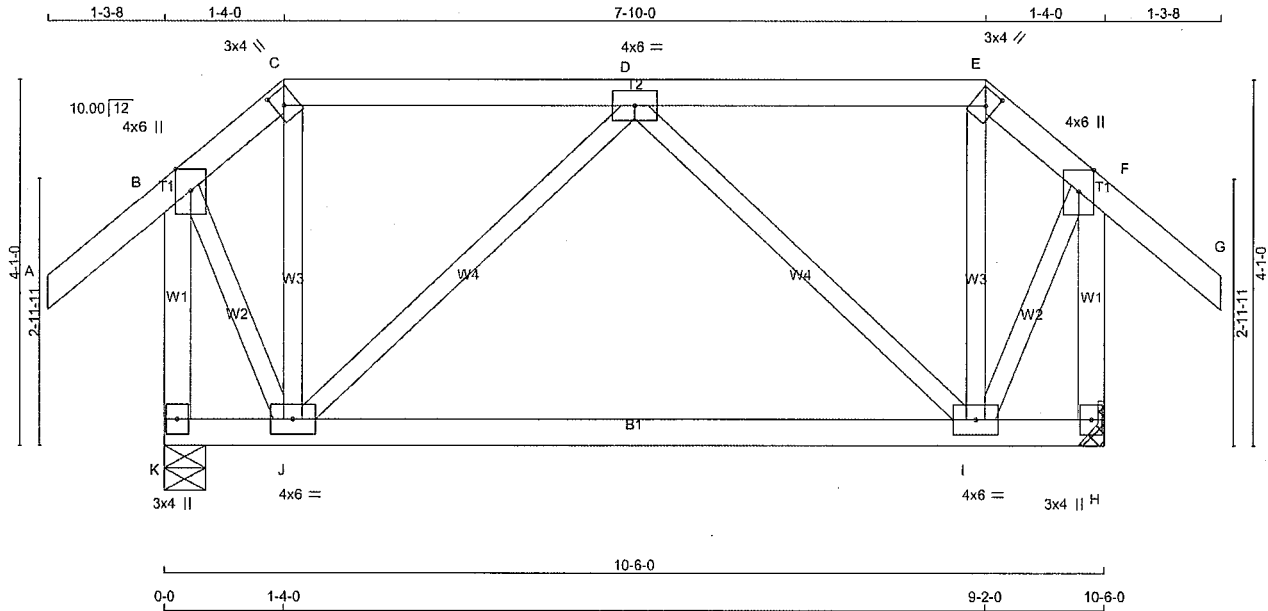
STRUCTURAL COMPONENT ONLY
DWG # TR24040212

REVIEWED

JOB NAME 437040	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:50 2024 Page 1
ID:Sb 47nW9yvpKeYGkKmELfPzxU46-sWynKF7hX9gi1Q94XuhJ0g8UmAJQnitsa7BxNzUndt



TOTAL WEIGHT = 54 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
K - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	Edge	
C TTW+h	MT20	3.0	4.0	2.00	1.25
D TMWW-t	MT20	4.0	6.0		
E TTW+h	MT20	3.0	4.0	2.00	1.25
F TMVW+p	MT20	4.0	6.0	Edge	
H BMV1+p	MT20	3.0	4.0		
I BMWWW-t	MT20	4.0	6.0		
J BMWWW-t	MT20	4.0	6.0		
K BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
K	843	0	843	0
H	843	0	843	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	588	431/0	0/0	0/0	0/0	157/0	0/0
H	588	431/0	0/0	0/0	0/0	157/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0/50	-112.4	-112.4 0.15 (1)	J-C	0/45	0.02 (4)	
B-C	-331/0	-112.4	-112.4 0.15 (1)	J-D	-385/0	0.19 (1)	
C-D	-212/0	-112.4	-112.4 0.28 (1)	D-I	-385/0	0.19 (1)	
D-E	-212/0	-112.4	-112.4 0.28 (1)	I-E	0/45	0.02 (4)	
E-F	-331/0	-112.4	-112.4 0.15 (1)	B-J	0/444	0.10 (1)	
F-G	0/50	-112.4	-112.4 0.15 (1)	I-F	0/444	0.10 (1)	
K-B	-881/0	0.0	0.0 0.14 (1)				
H-F	-881/0	0.0	0.0 0.14 (1)				
K-J	0/0	-18.5	-18.5 0.18 (4)				
J-I	0/486	-18.5	-18.5 0.21 (4)				
I-H	0/0	-18.5	-18.5 0.18 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.28/1.00 (D-E:1), BC=0.21/1.00 (I-J:4), WB=0.19/1.00 (D-J:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (I) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 0.95)



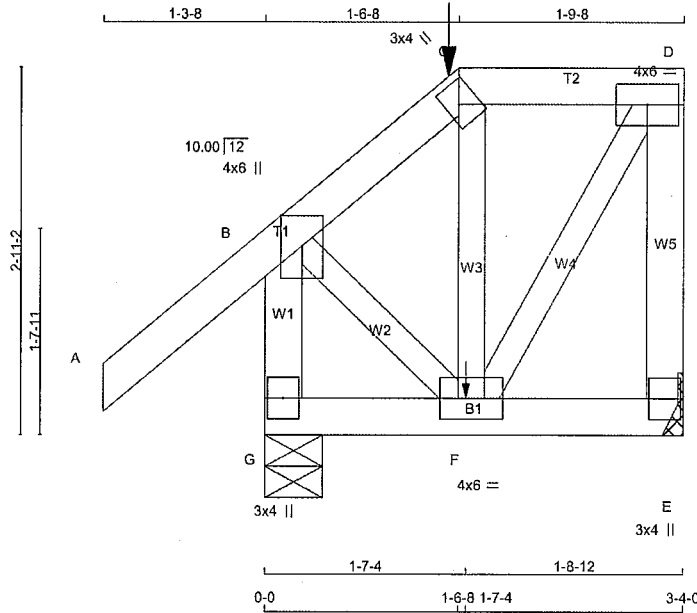
STRUCTURAL COMPONENT ONLY
DWG # TR24040213

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437040	T25	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:Sb 47nW9vypKeYGkKmELfPzxU46-KiW9xb8JlToZeZkG5boyYuhi8ahnWBM15EtUqzUnds



TOTAL WEIGHT = 4 X 20 = 80 lb
(M)

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A - C	1	12	SIDE(61.0)
C - D	1	12	SIDE(77.3)
D - E	1	12	TOP
G - B	1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
G - E	1	12	SIDE(14.2)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	513	0	513	0	0
G	738	0	738	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	360	254 / -19	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
G	514	384 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 50	F-C	-297 / 32
B-C	-261 / 0	F-D	0 / 283
C-D	-153 / 0	B-F	0 / 210
E-D	-478 / 0		
G-B	-702 / 0		
G-F	0 / 0		
F-E	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)		FACE	DIR.	TYPE	HEEL	CONN.
JT	LOC.	LC1	MAX.	MAX+		
C	1-8-8	-41	-41	105	FRONT	VERT
F	1-7-4	1	1	—	FRONT	VERT

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 7-6-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO BACK SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:5) , BC=0.01/1.00 (E-F:4) , WB=0.04/1.00 (D-F:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

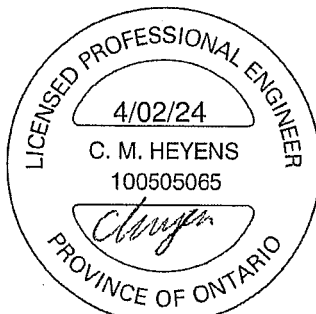
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

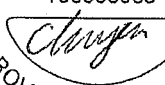
JSI GRIP= 0.22 (C) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 0.95)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR24040214

REVIEWED

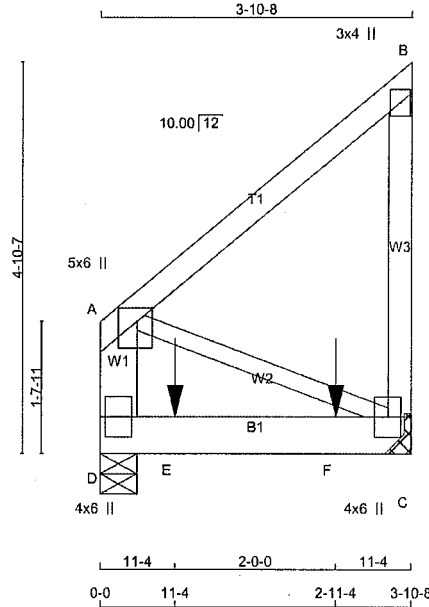
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																																	
437040	T25	2	2	BAYVIEW WELLINGTON																																																		
Tamarack Roof Truss, Burlington				Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:51 2024 Page 2	ID:Sb 47nW9yypKeYGkKmELfPzxU46-KiW9xb8JlToZeZkG5boyYuhI8ahnWBM15EtUgzUnds																																																	
PLATES (table is in inches) <table><tr><td>JT</td><td>TYPE</td><td>PLATES</td><td>W</td><td>LEN</td><td>Y</td><td>X</td></tr><tr><td>B</td><td>TMVW+p</td><td>MT20</td><td>4.0</td><td>6.0</td><td>Edge</td><td></td></tr><tr><td>C</td><td>TTW+h</td><td>MT20</td><td>3.0</td><td>4.0</td><td>2.00</td><td>1.25</td></tr><tr><td>D</td><td>TMVW-l</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>E</td><td>BMV1+p</td><td>MT20</td><td>3.0</td><td>4.0</td><td></td><td></td></tr><tr><td>F</td><td>BMVWW-l</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>G</td><td>BMV1+p</td><td>MT20</td><td>3.0</td><td>4.0</td><td></td><td></td></tr></table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.						JT	TYPE	PLATES	W	LEN	Y	X	B	TMVW+p	MT20	4.0	6.0	Edge		C	TTW+h	MT20	3.0	4.0	2.00	1.25	D	TMVW-l	MT20	4.0	6.0			E	BMV1+p	MT20	3.0	4.0			F	BMVWW-l	MT20	4.0	6.0			G	BMV1+p	MT20	3.0	4.0		
JT	TYPE	PLATES	W	LEN	Y	X																																																
B	TMVW+p	MT20	4.0	6.0	Edge																																																	
C	TTW+h	MT20	3.0	4.0	2.00	1.25																																																
D	TMVW-l	MT20	4.0	6.0																																																		
E	BMV1+p	MT20	3.0	4.0																																																		
F	BMVWW-l	MT20	4.0	6.0																																																		
G	BMV1+p	MT20	3.0	4.0																																																		
LICENSED PROFESSIONAL ENGINEER 4/02/24 C. M. HEYENS 100505065  PROVINCE OF ONTARIO STRUCTURAL COMPONENT ONLY DWG # TR24040214																																																						

REVIEWED

JOB NAME 437040	TRUSS NAME T26	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:53 2024 Page 1
ID:Sb 47nW9yvpKeYGkKrnELfPzxU46-H5evMH9Zp42HutufC0rQdJm0 NJqz5OJYYMrYizUndq



Scale = 1:27.5

TOTAL WEIGHT = 2 X 22 = 45 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - A	2x6	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
		DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	TOP
B-C 1	12	TOP
D-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
C	1079	0	1079	0
D	1079	0	1079	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	752	557 / 0	0 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0
D	752	557 / 0	0 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 0	-112.4	-112.4	0.16 (1)	10.00	A-C	0 / 0	0.00 (1)
C-B	-218 / 0	0.0	0.0	0.04 (1)	7.81			
D-A	-218 / 0	0.0	0.0	0.01 (1)	7.81			
D-E	0 / 0	-18.5	-18.5	0.30 (1)	10.00			
E-F	0 / 0	-18.5	-18.5	0.30 (1)	10.00			
F-C	0 / 0	-18.5	-18.5	0.30 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-574	-574	---	BACK	VERT	TOTAL	---	C1
F	2-11-4	-574	-574	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.30/1.00 (C-D:1) ,
WB=0.00/1.00 (A-C:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040215

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T26	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:53 2024 Page 2
ID:Sb_47nW9yvpKeYGkKmELfPzxU46-H5evMH9Zp42HutufC0rQdJm0 NJqz5OjYYMrYizUnd0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR24040215

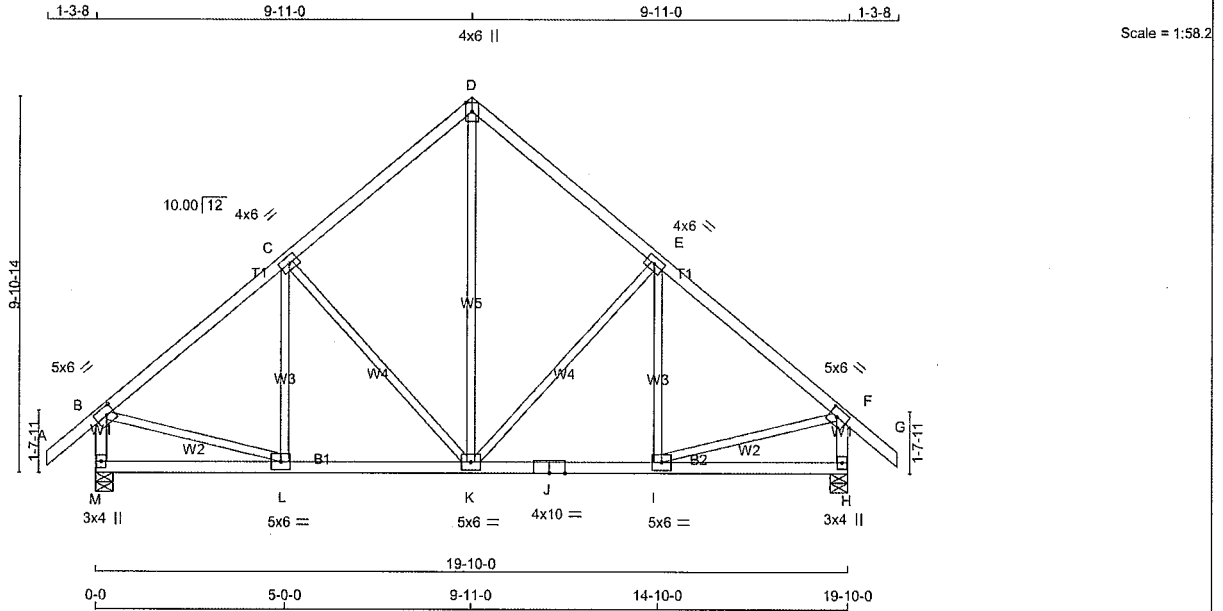
REVIEWED

JOB NAME 437040	TRUSS NAME T30	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:55 2024 Page 1

ID:Sb 47nW9yvpKeYGkKmELfPzxU46-DTmgnyBqLiI?7B22KRtUjkrEIBtTRqOc0srydbzUndo



TOTAL WEIGHT = 94 lb

LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					DESIGN CRITERIA				
N. L. G. A. RULES					BEARINGS					*** SPECIAL LOADS ANALYSIS ***				
CHORDS SIZE LUMBER DESCR.					FACTORED GROSS REACTION					GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.				
A - D 2x4 DRY No.2 SPF					MAXIMUM FACTORED GROSS REACTION					LOADS WERE DERIVED FROM USER INPUT				
D - G 2x4 DRY No.2 SPF					INPUT BRG					NO FURTHER MODIFICATIONS WERE MADE				
M - B 2x4 DRY No.2 SPF					DOWN HORZ UPLIFT IN-SX									
H - F 2x4 DRY No.2 SPF					M 1834 0 1834 0 0 5-8 2-7									
M - J 2x4 DRY No.2 SPF					H 2080 0 2080 0 0 5-8 3-4									
J - H 2x4 DRY No.2 SPF														
ALL WEBS 2x3 DRY No.2 SPF					UNFACTORED REACTIONS					SPECIFIED LOADS:				
EXCEPT					1ST LCASE MAX./MIN. COMPONENT REACTIONS					TOP CH. LL = 32.5 PSF				
DRY: SEASONED LUMBER.					JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL					DL = 6.0 PSF				
					M 1320 734 / 0 0 / 0 0 / 0 0 / 0 586 / 0 0 / 0					BOT CH. LL = 0.0 PSF				
					H 1517 734 / 0 0 / 0 0 / 0 0 / 0 783 / 0 0 / 0					DL = 7.4 PSF				
					BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H					TOTAL LOAD = 45.9 PSF				
					BRACING					SPACING = 24.0 IN. C/C				
					TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.					*** NON STANDARD GIRDER ***				
					MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.					ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.				
					ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015				
PLATES (table is in inches)					LOADING					THIS DESIGN COMPLIES WITH:				
JT TYPE PLATES W LEN Y X					TOTAL LOAD CASES: (4)					- PART 9 OF BCBC 2018, NBC-2019AE				
B TMVW-t MT20 5.0 6.0 2.50 2.50					CHORDS					- PART 9 OF OBC 2012 (2019 AMENDMENT)				
C TMVW-t MT20 4.0 6.0					MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. LC1 (LC) UNBRAC LENGTH FR-TO					- CSA 086-14				
D TTW+p MT20 4.0 6.0 Edge										- TPIC 2014				
E TMVW-t MT20 4.0 6.0														
F TMVW-t MT20 5.0 6.0 2.50 2.50														
H BMV1+p MT20 3.0 4.0														
I BMVW-t MT20 5.0 6.0														
J BS-t MT20 4.0 10.0														
K BMVW-t MT20 5.0 6.0														
L BMVW-t MT20 5.0 6.0														
M BMV1+p MT20 3.0 4.0														
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.														
NOTES- (1)														
1) Lateral braces to be a minimum of 2X4 SPF #2.														



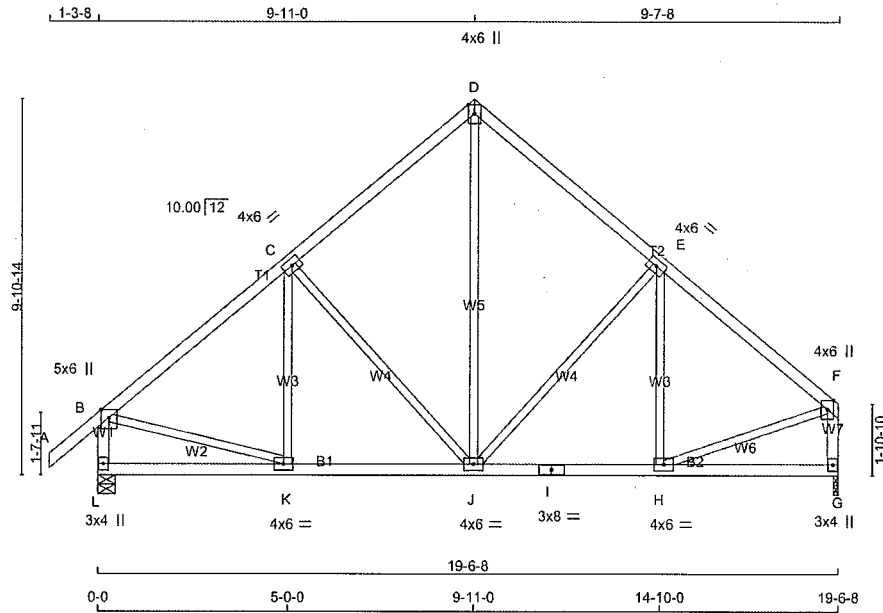
STRUCTURAL COMPONENT ONLY
DWG # TR24040217

REVIEWED

JOB NAME 437040	TRUSS NAME T30A	QUANTITY 3	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:57 2024 Page 1
ID:Sb 47nW9yvpKeYGKkMELfPzxU46-9stQCd4JYjNUBQRsvMo9xfX hFvoQvT9K2hTzUndm



TOTAL WEIGHT = 3 X 92 = 276 lb
[M][F]

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge		
C	TMWW-t	MT20	4.0	6.0			
D	TTW+p	MT20	4.0	6.0	Edge		
E	TMWW-t	MT20	4.0	6.0			
F	TMVW+p	MT20	4.0	6.0	Edge		
G	BMV1+p	MT20	3.0	4.0			
H	BMWW-t	MT20	4.0	6.0			
I	BS-t	MT20	3.0	8.0			
J	BMWW-t	MT20	4.0	6.0			
K	BMWW-t	MT20	4.0	6.0			
L	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
L	1435	0	1435	0	0	5-8	1-9		
G	1279	0	1279	0	0	1-8	1-8		

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
L	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0		
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)		CHORDS		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED
FR-TO		FROM	TO	CSi (LC)	FR-TO		CSi (LC)
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	K-C	-144 / 46
B-C	-1178 / 0	-112.4	-112.4	0.38 (1)	5.42	C-J	-421 / 0
C-D	-898 / 0	-112.4	-112.4	0.37 (1)	6.01	J-D	0 / 673
D-E	-895 / 0	-112.4	-112.4	0.35 (1)	6.05	J-E	-364 / 0
E-F	-1128 / 0	-112.4	-112.4	0.35 (1)	5.54	H-E	-199 / 28
L-B	-1396 / 0	0.0	0.0	0.15 (1)	6.89	B-K	0 / 969
G-F	-1243 / 0	0.0	0.0	0.14 (1)	7.20	H-F	0 / 947
L-K	0 / 0	-18.5	-18.5	0.11 (4)	10.00		
K-J	0 / 938	-18.5	-18.5	0.20 (1)	10.00		
J-I	0 / 900	-18.5	-18.5	0.19 (1)	10.00		
I-H	0 / 900	-18.5	-18.5	0.19 (1)	10.00		
H-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2018AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSi: TC=0.38/1.00 (B-C:1), BC=0.20/1.00 (J-K:1), WB=0.45/1.00 (C-J:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR SECTION	
(PSI)	(PLI)	(PSI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.54 (F) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040218

REVIEWED

Tamarack Roof Truss, Burlington



REVIEWED

JOB NAME

437040

TRUSS NAME

T32

QUANTITY

1

PLY

1

JOB DESC.

BAYVIEW WELLINGTON

DRWG NO.

Tamarack Roof Truss, Burlington

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ID:Sb 47nW9yvpKeYGkKmeLFPzxU46-aRZZqgFyAEwHEyw77 T3QnZaOCk268oL97ZjlozUndj

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x6	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.75
B	TMVW-w	MT20	2.0	4.0	2.50	1.00
C	TMVW-t	MT20	5.0	6.0	2.50	2.75
D	BMV1+p	MT20	4.0	6.0	3.00	Edge
E	BMVWW-t	MT20	6.0	10.0		
F	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 1412 0	HORZ 1412 0	UPLIFT 0 0	1-8 1-8
D	VERT 1429 0	HORZ 1429 0	UPLIFT 0 0	3-8 1-9

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN. COMPONENT REACTIONS
JT COMBINED	SNOW 987 712 / 0
F	LIVE 0 / 0
D	PERM. LIVE 0 / 0
	WIND 0 / 0
	DEAD 275 / 0
	SOIL 0 / 0
	292 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
F-A	-1312 / 0	0.0 0.0	0.18 (1)	7.02	A-E	0 / 1898	0.47 (1)
A-G	-1721 / 0	-112.4 -112.4	0.38 (1)	5.52	E-B	-1260 / 0	0.22 (1)
G-H	-1721 / 0	-112.4 -112.4	0.38 (1)	5.52	E-C	0 / 1898	0.47 (1)
H-B	-1721 / 0	-112.4 -112.4	0.38 (1)	5.52			
B-I	-1721 / 0	-112.4 -112.4	0.38 (1)	5.52			
I-J	-1721 / 0	-112.4 -112.4	0.38 (1)	5.52			
J-K	-1721 / 0	-112.4 -112.4	0.38 (1)	5.52			
K-C	-1721 / 0	-112.4 -112.4	0.38 (1)	5.52			
D-C	-1269 / 0	0.0 0.0	0.18 (1)	7.11			
F-L	0 / 0	-18.5 -18.5	0.12 (1)	10.00			
L-M	0 / 0	-18.5 -18.5	0.12 (1)	10.00			
M-E	0 / 0	-18.5 -18.5	0.12 (1)	10.00			
E-N	0 / 0	-18.5 -18.5	0.13 (1)	10.00			
N-O	0 / 0	-18.5 -18.5	0.13 (1)	10.00			
O-P	0 / 0	-18.5 -18.5	0.13 (1)	10.00			
P-D	0 / 0	-18.5 -18.5	0.13 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-131	-131	---	FRONT	VERT	TOTAL	---	C1
B	5-2.4	-95	-95	---	FRONT	VERT	TOTAL	---	C1
E	5-2.4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
G	2-0-12	-95	-95	---	FRONT	VERT	TOTAL	---	C1
H	4-0-12	-95	-95	---	FRONT	VERT	TOTAL	---	C1
I	6-2.4	-95	-95	---	FRONT	VERT	TOTAL	---	C1
J	8-2.4	-95	-95	---	FRONT	VERT	TOTAL	---	C1
K	10-2.4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
L	2-0-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
M	4-0-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
N	6-2.4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
O	8-2.4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
P	10-2.4	-53	-53	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")

CALCULATED VERT. DEFL.(LL) = L/999 (0.04")

ALLOWABLE DEFL.(TL)= L/360 (0.35")

CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.38/1.00 (A-B:1), BC=0.13/1.00 (D-E:1), WB=0.47/1.00 (A-E:1), SSI=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)

JSI METAL= 0.39 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER

4/02/24

C. M. HEYENS

100505065

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY

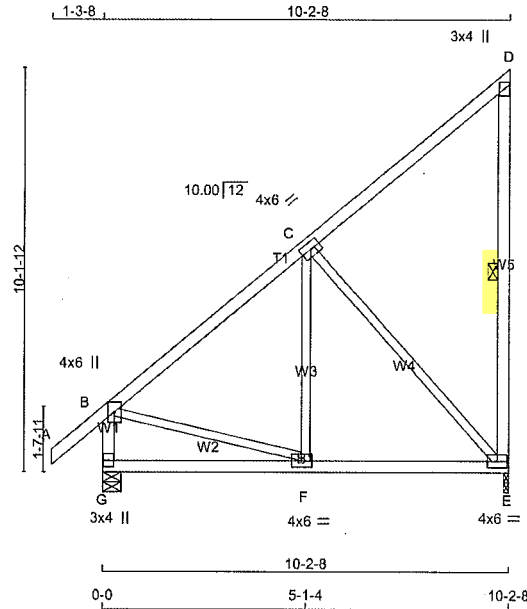
DWG # TR24040220

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T33	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:55.5

TOTAL WEIGHT = 55 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW-t	MT20	4.0	6.0		
F	BMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
E	668	0	668	0
G	824	0	824	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	468	332 / 0	0 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
G	575	421 / 0	0 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO						FR-TO			
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	F-C	0 / 103	0.04 (4)	
B-C	-451 / 0	-112.4	-112.4	0.38 (1)	6.25	C-E	-564 / 0	0.61 (1)	
C-D	-45 / 0	-112.4	-112.4	0.38 (1)	6.25	B-F	0 / 394	0.09 (1)	
E-D	-216 / 0	0.0	0.0	0.11 (1)	6.25				
G-B	-787 / 0	0.0	0.0	0.08 (1)	7.81				
G-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
F-E	0 / 382	-18.5	-18.5	0.16 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.38/1.00 (B-C:1), BC=0.16/1.00 (E-F:4), WB=0.61/1.00 (C-E:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (B) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 0.95)



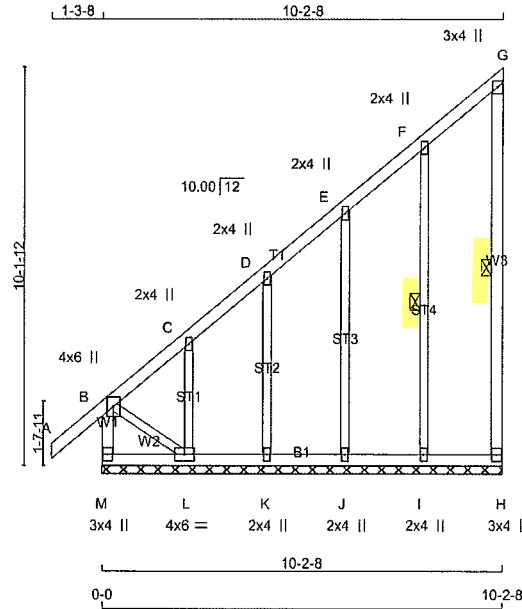
STRUCTURAL COMPONENT ONLY
DWG # TR24040221

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T33G	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 61 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
M - B	2x4	DRY No.2	SPF
A - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - H	2x4	DRY No.2	SPF

ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F						
C	TMV+w	MT20	2.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K						
I	BMV1+w	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRAC LENGTH (LC)	MAX (LC)
FR-TO		FROM TO		FR-TO			
A-B	-277 / 0	0.0	0.0 0.03 (1)	I-F	-253 / 0	0.12 (1)	
A-B	0 / 50	-112.4	-112.4 0.15 (1)	J-E	-220 / 0	0.17 (1)	
B-C	-8 / 0	-112.4	-112.4 0.07 (1)	K-D	-213 / 0	0.08 (1)	
C-D	-18 / 0	-112.4	-112.4 0.07 (1)	L-C	-258 / 0	0.05 (1)	
D-E	-7 / 0	-112.4	-112.4 0.05 (1)	B-L	0 / 21	0.00 (1)	
E-F	-1 / 0	-112.4	-112.4 0.06 (1)				
F-G	-14 / 0	-112.4	-112.4 0.06 (1)				
H-G	-96 / 0	0.0	0.0 0.05 (1)				
M-L	0 / 0	-18.5	-18.5 0.02 (4)				
L-K	0 / 10	-18.5	-18.5 0.02 (4)				
K-J	0 / 6	-18.5	-18.5 0.01 (4)				
J-I	0 / 3	-18.5	-18.5 0.02 (4)				
I-H	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (K-L:4), WB=0.17/1.00 (E-J:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.14 (C) (INPUT = 0.95)



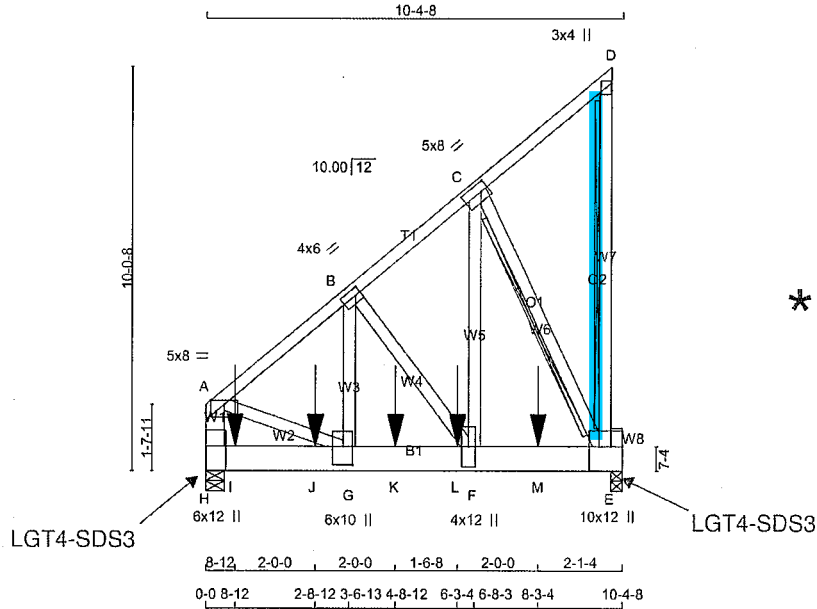
STRUCTURAL COMPONENT ONLY
DWG # TR24040222

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	T34	1	4	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:54.9

* CONNECT TRUSS PLIES
WITH MIFLK006 SCREWS
AS PER BELOW.

TOTAL WEIGHT = 4 X 81 = 324 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - A	2x6	DRY	No.2	SPF	
H - E	2x8	DRY	1950F 1.7E	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
					DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : MIFLK006 SCREWS *		
A-D 1	24	TOP
D-E 1	24	TOP
BOTTOM CHORDS : MIFLK006 SCREWS *		
H-E 2	5	SIDE(3328.3)
WEBS : MIFLK006 SCREWS *		
2x4 1	24	

SCREWS TO BE DRIVEN FROM ONE SIDE ONLY. INSTALL SCREWS PER MITEK'S INSTALLATION INSTRUCTIONS.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV-p	MT20	5.0	8.0	Edge	
B	TMVVW-t	MT20	4.0	6.0	2.00	1.75
C	TMVVW-t	MT20	5.0	8.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
E	11717	0	11850	0
H	14074	0	14207	479

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 2142 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 2315 LBS FACTORED UPLIFT

PROVIDE FOR 479 LBS FACTORED HORIZONTAL REACTION AT JOINT H

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
E	8512	5622 / 0
H	10225	6734 / 0

HORIZONTAL REACTIONS	H	0 / 0	0 / 0	342 / -224	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) E (BASED ON SUPPORT DEPTH = 1-8)

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-E, C-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO	A-B	-10662 / 1767	-145.3	-145.3	0.19 (2)	4.11	A-G	-1428 / 8611	0.35 (2)
	B-C	-6338 / 1139	-145.3	-145.3	0.11 (2)	5.16	G-B	-1164 / 6755	0.27 (3)
	C-D	-149 / 200	-145.3	-145.3	0.07 (2)	6.25	B-F	-5483 / 1092	0.41 (2)
	D-E	-230 / 129	0.0	0.0	0.11 (11)	7.81	F-C	-2278 / 13066	0.53 (2)
	H-A	-9763 / 1587	0.0	0.0	0.16 (2)	6.61	C-E	-10831 / 2048	0.42 (2)
	H-I	-460 / 304	-39.5	-39.5	0.18 (2)	6.25			
	I-J	-460 / 304	-39.5	-39.5	0.18 (2)	6.25			
	J-G	-460 / 304	-39.5	-39.5	0.18 (2)	6.25			
	G-K	-1554 / 8197	-39.5	-39.5	0.23 (2)	6.25			
	K-L	-1554 / 8197	-39.5	-39.5	0.23 (2)	6.25			
	L-F	-1554 / 8197	-39.5	-39.5	0.23 (2)	6.25			
	F-M	-936 / 4895	-39.5	-39.5	0.25 (2)	6.25			
	M-E	-936 / 4895	-39.5	-39.5	0.25 (2)	6.25			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	8-12	-3480	-3480	381	FRONT	VERT	TOTAL	---	C1
J	2-8-12	-3475	-3475	382	FRONT	VERT	TOTAL	---	C1
K	4-8-12	-3475	-3475	382	FRONT	VERT	TOTAL	---	C1
L	6-3-4	-3475	-3475	382	FRONT	VERT	TOTAL	---	C1
M	8-3-4	-3475	-3475	382	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 43.5 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 67.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/180 (0.67")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.19/1.00 (A-B:2), BC=0.25/1.00 (E-F:2), WB=0.53/0.90 (C-F:2), SI=0.77/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.62 (G) (INPUT = 0.95)

CONTINUED ON PAGE 2

REVIEWED



STRUCTURAL COMPONENT ONLY
DWG # TR24040223

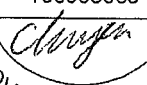
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																										
437040	T34	1	4	BAYVIEW WELLINGTON																																											
Tamarack Roof Truss, Burlington				Version 8.630 S Aug 30 2023 Mitek Industries, Inc. Tue Apr 2 11:24:05 2024 Page 2																																											
				ID:Sb_47nW9yvpKeYGkMELfPzxU46-wPMStNJ5?mZaKjozvX2E6rG4TDQQnO94JPGUz0zUnde																																											
PLATES (table is in inches) <table><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>D</td><td>TMV+p</td><td>MT20</td><td>3.0</td><td>4.0</td><td></td><td></td></tr><tr><td>E</td><td>BMVWW1+p</td><td>MT20</td><td>10.0</td><td>12.0</td><td>Edge</td><td>3.00</td></tr><tr><td>F</td><td>BMWW+H</td><td>MT20</td><td>4.0</td><td>12.0</td><td></td><td></td></tr><tr><td>G</td><td>BMWW+H</td><td>MT20</td><td>6.0</td><td>10.0</td><td>5.50</td><td>2.75</td></tr><tr><td>H</td><td>BMV1+H</td><td>MT20</td><td>6.0</td><td>12.0</td><td>7.25</td><td></td></tr></tbody></table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.						JT	TYPE	PLATES	W	LEN	Y	X	D	TMV+p	MT20	3.0	4.0			E	BMVWW1+p	MT20	10.0	12.0	Edge	3.00	F	BMWW+H	MT20	4.0	12.0			G	BMWW+H	MT20	6.0	10.0	5.50	2.75	H	BMV1+H	MT20	6.0	12.0	7.25	
JT	TYPE	PLATES	W	LEN	Y	X																																									
D	TMV+p	MT20	3.0	4.0																																											
E	BMVWW1+p	MT20	10.0	12.0	Edge	3.00																																									
F	BMWW+H	MT20	4.0	12.0																																											
G	BMWW+H	MT20	6.0	10.0	5.50	2.75																																									
H	BMV1+H	MT20	6.0	12.0	7.25																																										
CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED. <u>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)</u> WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.																																															

LICENSED PROFESSIONAL ENGINEER

4/02/24

C. M. HEYENS

100505065



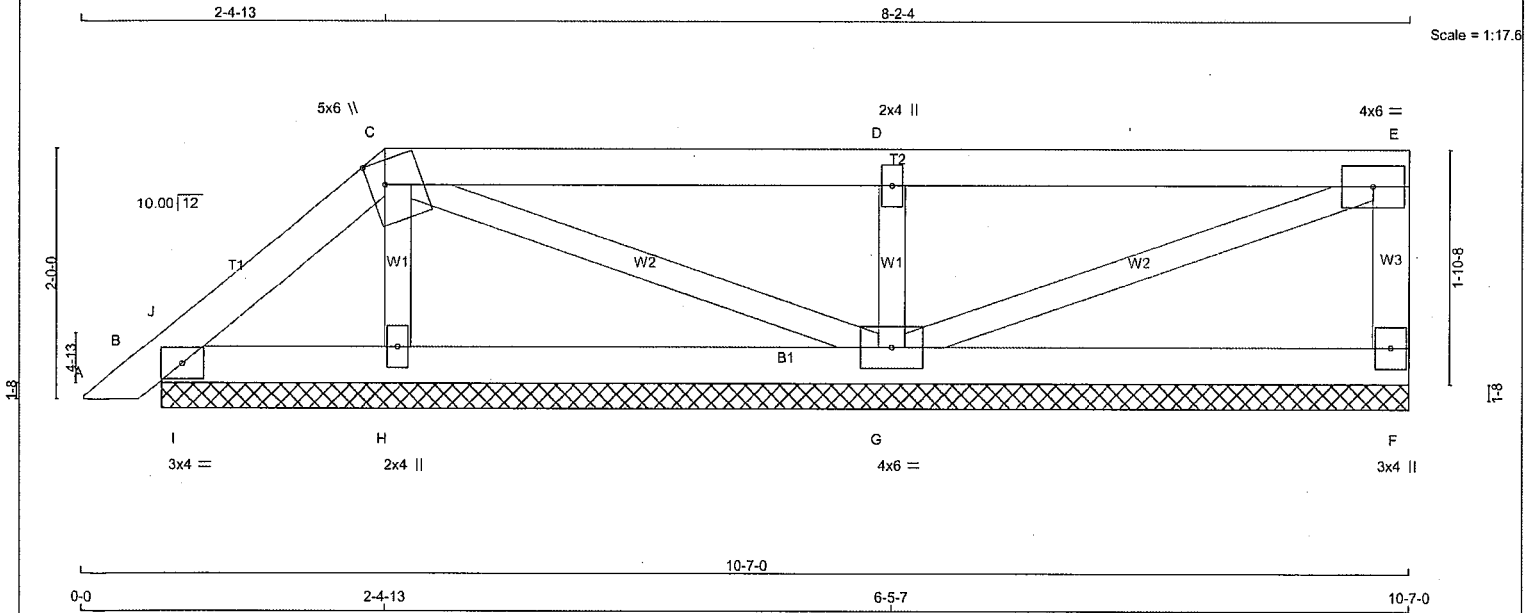
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040223

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	PB1	4	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:07 2024 Page 1
ID:Sb 47nW9yvpKeYGkKmELIPzxU46-dEsXiYcfyFpNpNpWM0emcs73qfSCGjYzODPxNzUneY



TOTAL WEIGHT = 4 X 34 = 137 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-I	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW1-I	MT20	4.0	6.0		
H	BMV1+w	MT20	2.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
JT				
F	205	0	205	0
B	206	0	206	0
H	277	0	277	0
G	674	0	674	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	144	101/0	0/0	0/0	0/0	43/0	0/0
B	142	114/0	0/0	0/0	0/0	28/0	0/0
H	196	127/0	0/0	0/0	0/0	69/0	0/0
G	472	339/0	0/0	0/0	0/0	133/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/17	-112.4	-112.4 0.02 (1)	10.00	H-C	-198/0	0.03 (1)
B-J	-41/0	-112.4	-112.4 0.01 (1)	6.25	C-G	-37/0	0.01 (1)
J-C	-64/0	-112.4	-112.4 0.04 (1)	6.25	G-D	-575/0	0.08 (1)
C-D	0/2	-112.4	-112.4 0.32 (1)	10.00	G-E	-2/0	0.00 (1)
D-E	0/2	-112.4	-112.4 0.32 (1)	10.00	I-J	-130/0	0.00 (1)
F-E	-175/0	0.0	0.0 0.02 (1)	7.81			
B-I	0/46	-18.5	-18.5 0.05 (1)	10.00			
I-H	0/46	-18.5	-18.5 0.05 (1)	10.00			
H-G	0/33	-18.5	-18.5 0.08 (4)	10.00			
G-F	0/0	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-D:1), BC=0.08/1.00 (G-H:4), WB=0.08/1.00 (D-G:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 0.95)



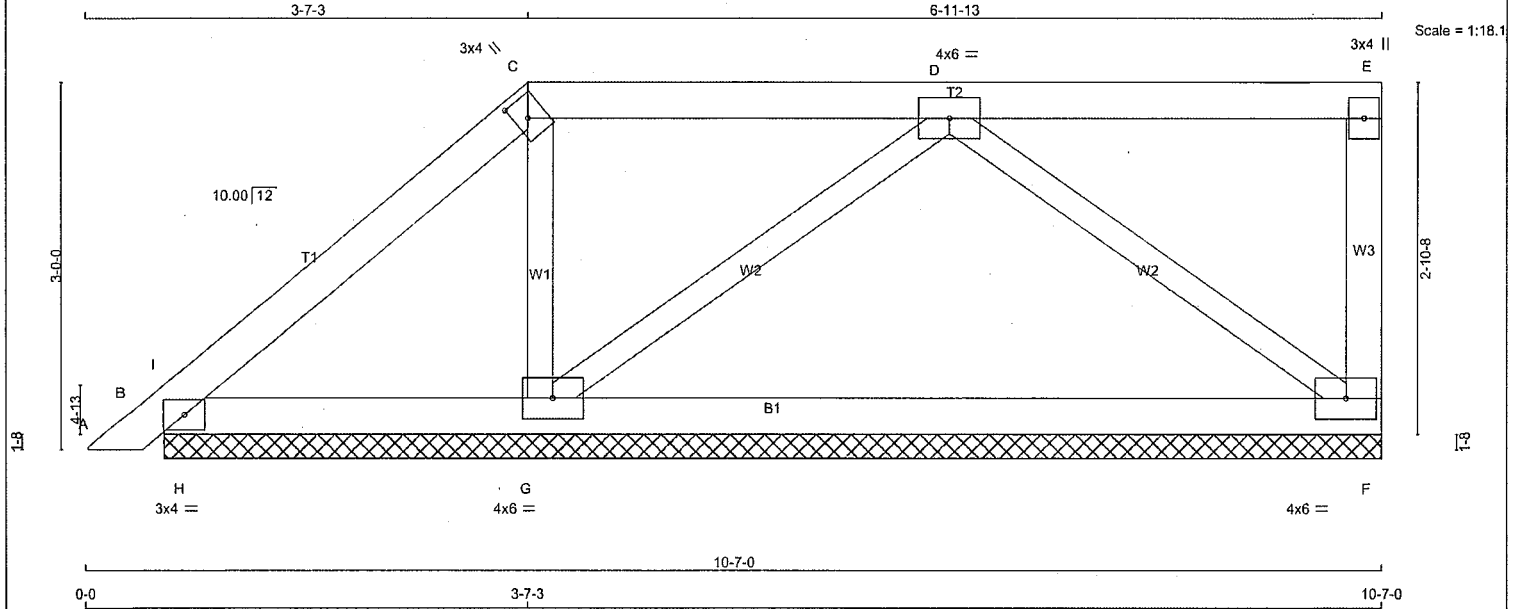
STRUCTURAL COMPONENT ONLY
DWG # TR24040183

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	PB2	5	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 M/Tek Industries, Inc. Tue Apr 2 11:23:08 2024 Page 1
ID:Sb 47nW9yvpKsYgKkMELfPzxU46-5RQvwudLQFNq zy043XlJqPJLEzdxJiC2yzTpzUneX



TOTAL WEIGHT = 5 X 36 = 178 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMB1-I	MT20	3.0	4.0	
C TTW+h	MT20	3.0	4.0	2.00 1.25
D TMWW-t	MT20	4.0	6.0	
E TMV+p	MT20	3.0	4.0	
F BMVW1-t	MT20	4.0	6.0	
G BMVW1-t	MT20	4.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	439	0	439	0	0	9-11-7	1-8	
B	234	0	234	0	0	9-11-7	1-8	
G	690	0	690	0	0	9-11-7	1-8	

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED							
F	306	225 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
B	160	137 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
G	488	319 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO			
A-B	0 / 17	-112.4 -112.4	0.02 (1)	10.00	G-C	-273 / 0	0.05 (1)
B-I	0 / 130	-112.4 -112.4	0.09 (1)	10.00	G-D	-408 / 0	0.12 (1)
I-C	-27 / 0	-112.4 -112.4	0.11 (1)	6.25	D-F	-401 / 0	0.12 (1)
C-D	0 / 4	-112.4 -112.4	0.22 (1)	10.00	H-I	-381 / 0	0.00 (1)
D-E	0 / 0	-112.4 -112.4	0.22 (1)	10.00			
F-E	-153 / 0	0.0 0.0	0.02 (1)	7.81			
B-H	-3 / 12	-18.5 -18.5	0.12 (1)	10.00			
H-G	-3 / 12	-18.5 -18.5	0.20 (4)	10.00			
G-F	0 / 324	-18.5 -18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (D-E:1), BC=0.21/1.00 (F-G:4), WB=0.12/1.00 (D-G:1), SSI=0.27/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
JSI METAL= 0.08 (G) (INPUT = 0.95)



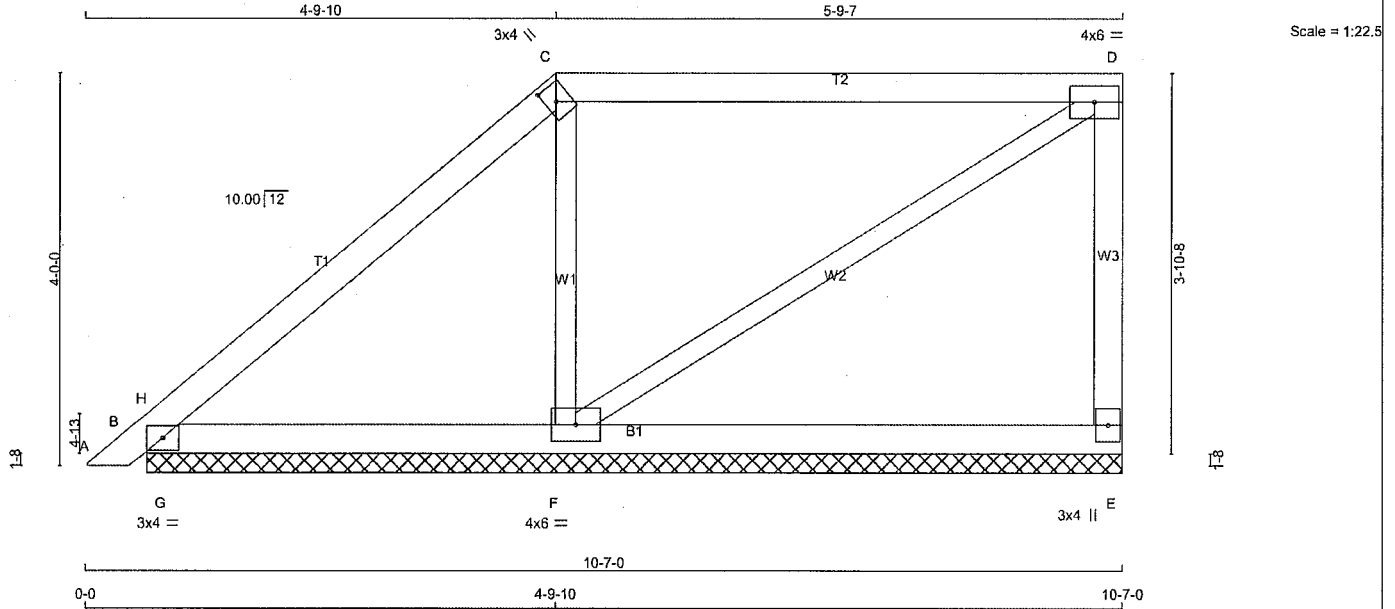
STRUCTURAL COMPONENT ONLY
DWG # TR24040184

REVIEWED

JOB NAME 437040	TRUSS NAME PB3	QUANTITY 3	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:23:10 2024 Page 1
ID:Sb 47nW9yvpKeYGkMELiPzxU46-1pYfKaebtdNEG5OBUZLOFUYE2f5Pd7 fLR3YizUneV



TOTAL WEIGHT = 3 X 37 = 110 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF
B - E	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TMVW-I	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVW1-I	MT20	4.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	COMBINED	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
E	410	0	410	0	0	9-11-7	1-8		
B	389	0	389	0	0	9-11-7	1-8		
F	563	0	563	0	0	9-11-7	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	286	212/0	0/0	0/0	0/0	74/0	0/0
B	269	209/0	0/0	0/0	0/0	61/0	0/0
F	399	260/0	0/0	0/0	0/0	138/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX (LC)	UNBRAC LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0/17	-112.4	-112.4 0.02 (1)	10.00	F-C	-447/0	0.10 (1)
B-H	0/149	-112.4	-112.4 0.15 (1)	10.00	F-D	0/91	0.02 (1)
H-C	-133/0	-112.4	-112.4 0.24 (1)	6.25	G-H	-618/0	0.00 (1)
C-D	-77/0	-112.4	-112.4 0.64 (1)	6.25			
E-D	-374/0	0.0	0.0 0.09 (1)	7.81			
B-G	0/90	-18.5	-18.5 0.21 (1)	10.00			
G-F	0/90	-18.5	-18.5 0.21 (1)	10.00			
F-E	0/0	-18.5	-18.5 0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.64/1.00 (C-D:1), BC=0.21/1.00 (B-G:1), WB=0.10/1.00 (C-F:1), SSI=0.47/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (C) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)

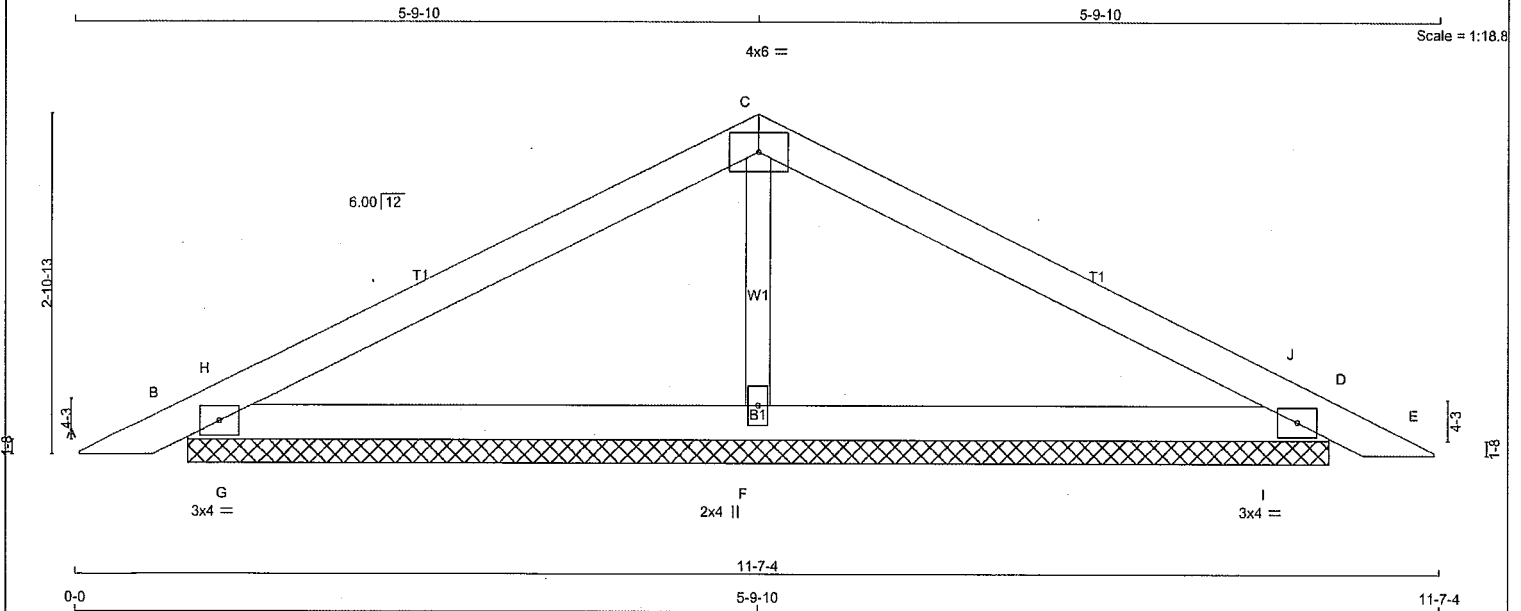


STRUCTURAL COMPONENT ONLY
DWG # TR24040185

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	PB4	12	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:Sb 47nW9yvpKeYGkMELfPzxU46-V051YvfDIAIerQqaC4awS1p6S y85F8u?Bd48zUneU



TOTAL WEIGHT = 12 X 28 = 331 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
B - D	2x4 DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	6.0	
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
B	VERT	HORZ	DOWN	HORZ	UPLIFT
B	461	0	461	0	0
D	461	0	461	0	0
F	540	0	540	0	0

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
B	321	243 / 0	0 / 0	0 / 0	78 / 0	0 / 0
D	321	243 / 0	0 / 0	0 / 0	78 / 0	0 / 0
F	381	256 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 20	-112.4	-112.4	0.06 (1)	10.00	F-C	-294 / 0
B-H	-58 / 0	-112.4	-112.4	0.14 (1)	6.25	G-H	-534 / 0
H-C	-190 / 0	-112.4	-112.4	0.32 (1)	6.25	I-J	-534 / 0
C-J	-190 / 0	-112.4	-112.4	0.32 (1)	6.25		
J-D	-58 / 0	-112.4	-112.4	0.14 (1)	6.25		
D-E	0 / 20	-112.4	-112.4	0.06 (1)	10.00		
B-G	0 / 161	-18.5	-18.5	0.30 (1)	10.00		
G-F	0 / 161	-18.5	-18.5	0.30 (1)	10.00		
F-I	0 / 161	-18.5	-18.5	0.30 (1)	10.00		
I-D	0 / 161	-18.5	-18.5	0.30 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-J:1), BC=0.30/1.00 (D-I:1), WB=0.05/1.00 (C-F:1), SSI=0.39/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 0.95)

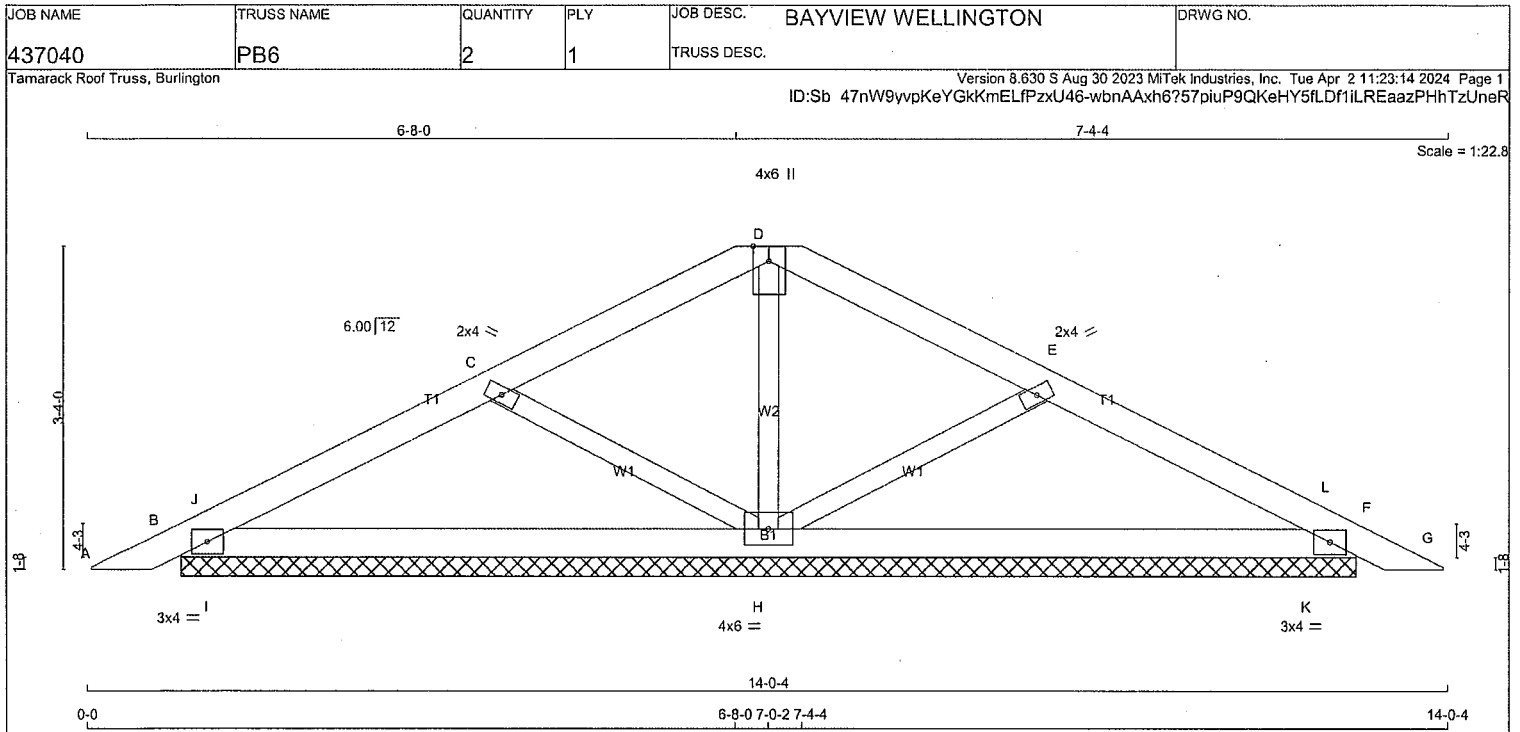


STRUCTURAL COMPONENT ONLY
DWG # TR24040186

REVIEWED

Tamarack Roof Truss, Burlington

REVIEWED



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+u	MT20	2.0	4.0	
D	TMTMW+p	MT20	4.0	6.0	Edge
E	TMW+u	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMWW1-I	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	384	0	384	0	0
F	384	0	384	0	0
H	1011	0	1011	0	0

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	268	196 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
F	268	196 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
H	707	505 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 20	-112.4	-112.4	0.06 (1)	10.00	H-D	-416 / 0
B-J	-277 / 0	-112.4	-112.4	0.08 (4)	6.25	C-H	-445 / 0
J-C	-198 / 0	-112.4	-112.4	0.17 (1)	6.25	H-E	-445 / 0
C-D	0 / 187	-112.4	-112.4	0.20 (1)	10.00	I-J	-19 / 97
D-E	0 / 187	-112.4	-112.4	0.20 (1)	10.00	K-L	-19 / 97
E-L	-198 / 0	-112.4	-112.4	0.17 (1)	6.25		
L-F	-277 / 0	-112.4	-112.4	0.08 (4)	6.25		
F-G	0 / 20	-112.4	-112.4	0.06 (1)	10.00		
B-I	0 / 200	-18.5	-18.5	0.06 (1)	10.00		
I-H	0 / 200	-18.5	-18.5	0.17 (4)	10.00		
H-K	0 / 200	-18.5	-18.5	0.17 (4)	10.00		
K-F	0 / 200	-18.5	-18.5	0.06 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (D-E:1), BC=0.17/1.00 (H-K:4), WB=0.10/1.00 (E-H:1), SSI=0.15/1.00 (E-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (C) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 0.95)



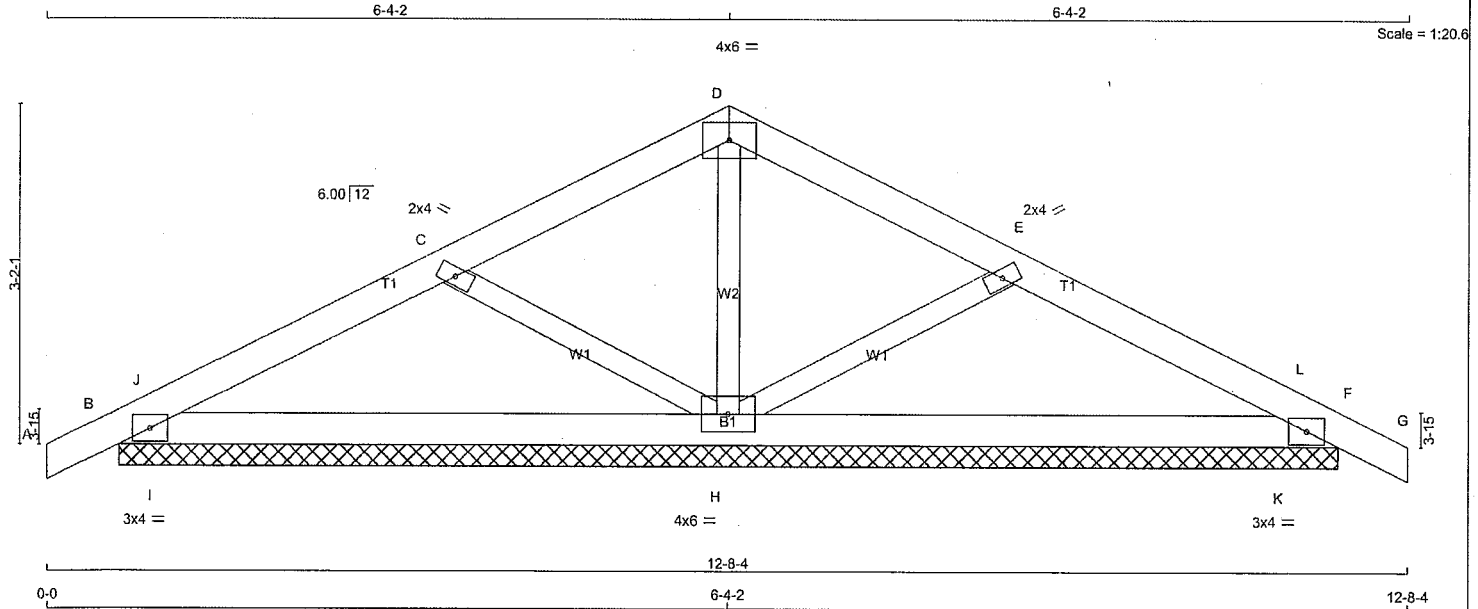
STRUCTURAL COMPONENT ONLY
DWG # TR24040188

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	PB7	5	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 5 X 36 = 181 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TMW+w	MT20	2.0	4.0
D	TTW-p	MT20	4.0	6.0
E	TMW+w	MT20	2.0	4.0
F	TMB1-I	MT20	3.0	4.0
H	BMWW1-I	MT20	4.0	6.0

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
B	344	0	344	0	1-8
F	344	0	344	0	1-8
H	964	0	964	0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
B	240	176 / 0	0 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
F	240	176 / 0	0 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
H	675	480 / 0	0 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
	FORCE		FORCE
	(LBS)		(LBS)
FR-TO		FR-TO	
A-B	0 / 17	H-D	-401 / 0
B-J	-233 / 0	H-E	-416 / 0
J-C	-164 / 0	C-H	-416 / 0
C-D	0 / 193	I-J	-24 / 80
D-E	0 / 193	K-L	-24 / 80
E-L	-164 / 0		
L-F	-233 / 0		
F-G	0 / 17		
B-I	0 / 169		
I-H	0 / 169		
H-K	0 / 169		
K-F	0 / 169		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (D-E:1), BC=0.15/1.00 (H-I:4), WB=0.08/1.00 (E-H:1), SSI=0.14/1.00 (E-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (C) (INPUT = 0.90)
JSI METAL= 0.19 (E) (INPUT = 0.95)



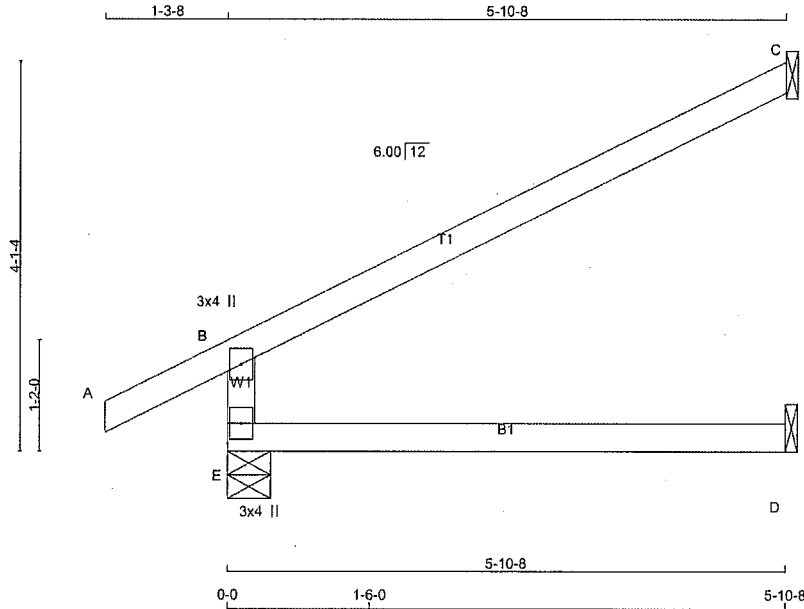
STRUCTURAL COMPONENT ONLY
DWG # TR24040189

REVIEWED

JOB NAME 437040	TRUSS NAME J1	QUANTITY 18	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:23.3

TOTAL WEIGHT = 18 X 17 = 302 lb

LUMBER

N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	629	0	629	0
C	248	0	248	0
D	45	0	45	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	438	327 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	170	143 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
E-B	-565 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00		
B-C	-37 / 0	-112.4	-112.4	0.66 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.66/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (E) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 0.95)

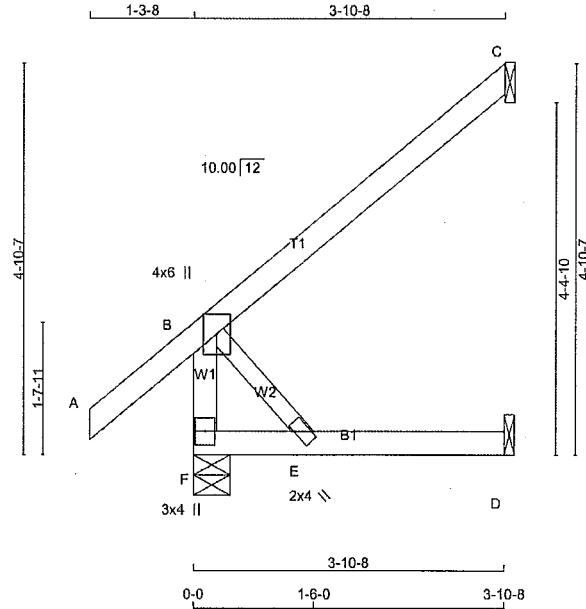


STRUCTURAL COMPONENT ONLY
DWG # TR24040178

REVIEWED

JOB NAME 437040	TRUSS NAME J2	QUANTITY 3	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:27.5

TOTAL WEIGHT = 3 X 15 = 45 lb

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	SIZE
F - B	2x4 DRY No.2
A - C	2x4 DRY No.2
F - D	2x4 DRY No.2
ALL WEBS	2x3 DRY No.2
DRY: SEASONED LUMBER.	

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B TMVW+p	MT20	4.0	6.0	Edge
E BMVW+w	MT20	2.0	4.0	
F BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	409	0	409	0
C	218	0	218	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
JT	COMBINED						
F	284	216/0	0/0	0/0	0/0	69/0	0/0
C	149	126/0	0/0	0/0	0/0	23/0	0/0
D	29	0/0	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED CSI (LC)	UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO		
F-B	-374/0	0.0	0.0	0.04 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/50	-112.4	-112.4	0.16 (5)	10.00			
B-C	0/0	-112.4	-112.4	0.29 (1)	10.00			
F-E	0/0	-18.5	-18.5	0.07 (4)	10.00			
E-D	0/0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.29/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

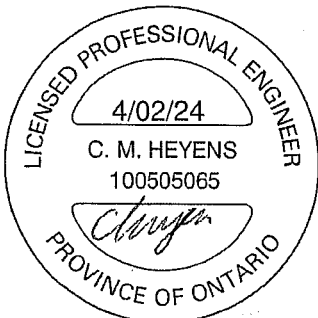
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 0.95)



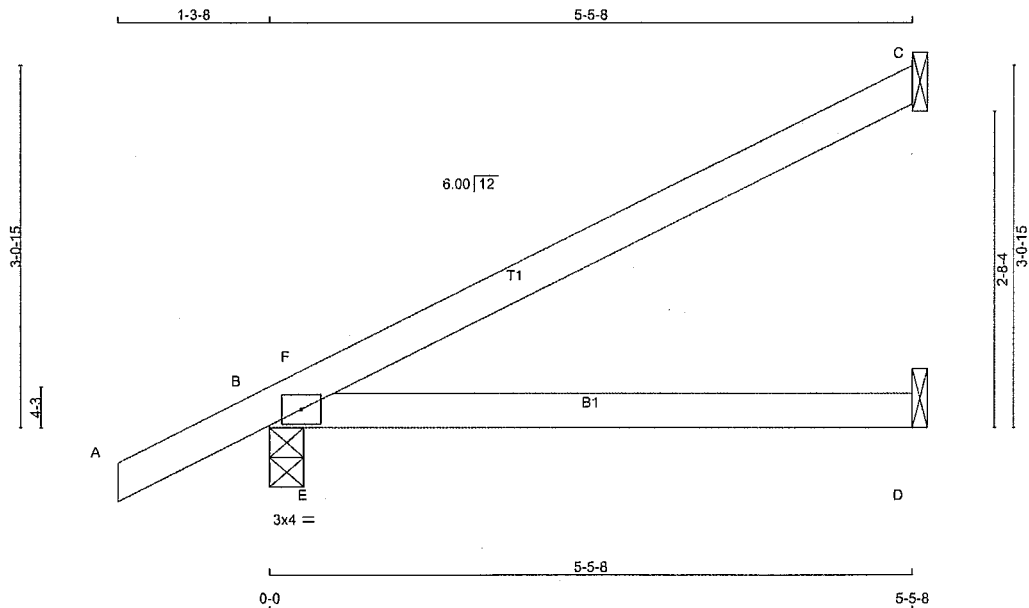
STRUCTURAL COMPONENT ONLY
DWG # TR24040179

REVIEWED

JOB NAME 437040	TRUSS NAME J3	QUANTITY 6	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:Sb 47nW9yvpKeYGkKmELfPzxU46-kTd0sAZCbIINuB32HWximiQJDF5GT0y2mFCoczUnec



TOTAL WEIGHT = 6 X 15 = 90 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
C	267	0	267	0
B	510	0	510	0
D	90	0	90	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED						
C	184	150 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	355	265 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	67	27 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. FACTORED UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 33	-112.4	-112.4	0.15 (1)	10.00	E-F	-533 / 9
B-F	-26 / 107	-112.4	-112.4	0.13 (1)	6.25		
F-C	-8 / 2	-112.4	-112.4	0.43 (1)	10.00		
B-E	0 / 0	-18.5	-18.5	0.30 (1)	10.00		
E-D	0 / 0	-18.5	-18.5	0.30 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/525 (0.12")

CSI: TC=0.43/1.00 (C-F:1), BC=0.30/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.42/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040180

REVIEWED

JOB NAME

437040

TRUSS NAME

J4

QUANTITY

2

PLY

1

JOB DESC.

BAYVIEW WELLINGTON

DRWG NO.

Tamarack Roof Truss, Burlington

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ID: Sb 47nW9yvpKeYGkKmELfPzxU46-DgAO4WaqM1tEWLeErDSx8 EeOdf?wG6HQ IK2zUneb

1-3-8

1-6-8

2-11-2

1-7-11

A

B

C

D

E

W1

W2

B1

3x4 II

2x4 \

10.00 12 4x6 II

0-0

1-6-0

1-6-8

Scale = 1:17.8

LUMBER

N. L. G. A. RULES

CHORDS SIZE

LUMBER

DESCR.

SPF

SPF

SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

B TMVW+p

MT20

4.0

6.0

Edge

D BMV1+w

MT20

2.0

4.0

E BMV1+p

MT20

3.0

4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQD

GROSS REACTION

GROSS REACTION

BRG

BRG

JT VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

E 325 0 325 0 0 5-8 1-8

C 17 0 17 0 -62 1-8 1-8

D 13 0 14 0 0 1-8 1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D.

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE

MAX/MIN. COMPONENT REACTIONS

JT COMBINED

SNOW

LIVE

PERM. LIVE

WIND

DEAD

SOIL

E 224 180 / 0 0 / 0 0 / 0 43 / 0 0 / 0

C 11 10 / -40 0 / 0 0 / 0 0 / 0 2 / 0 0 / 0

D 10 0 / 0 0 / 0 0 / 0 0 / 0 10 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MAX. FACTORED

FACTORED

WEBS

MAX. FACTORED

MEMB. FORCE

VERT. LOAD

LC1 MAX

MAX. MEMB.

MEMB. FORCE

MAX

(LBS)

(PLF)

CSI (LC)

UNBRAC

(LBS)

CSI (LC)

FR-TO

FROM

TO

LENGTH

FR-TO

E-B -312 / 0 0.0 0.0 0.03 (1) 7.81 B-D 0 / 0 0.00 (1)

A-B 0 / 50 -112.4 -112.4 0.15 (1) 10.00

B-C -45 / 0 -112.4 -112.4 0.15 (1) 6.25

E-D 0 / 0 -18.5 -18.5 0.01 (4) 10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2018AE

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 886-14

- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)

JSI METAL= 0.08 (B) (INPUT = 0.95)

4/02/24

C. M. HEYENS

100505065

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

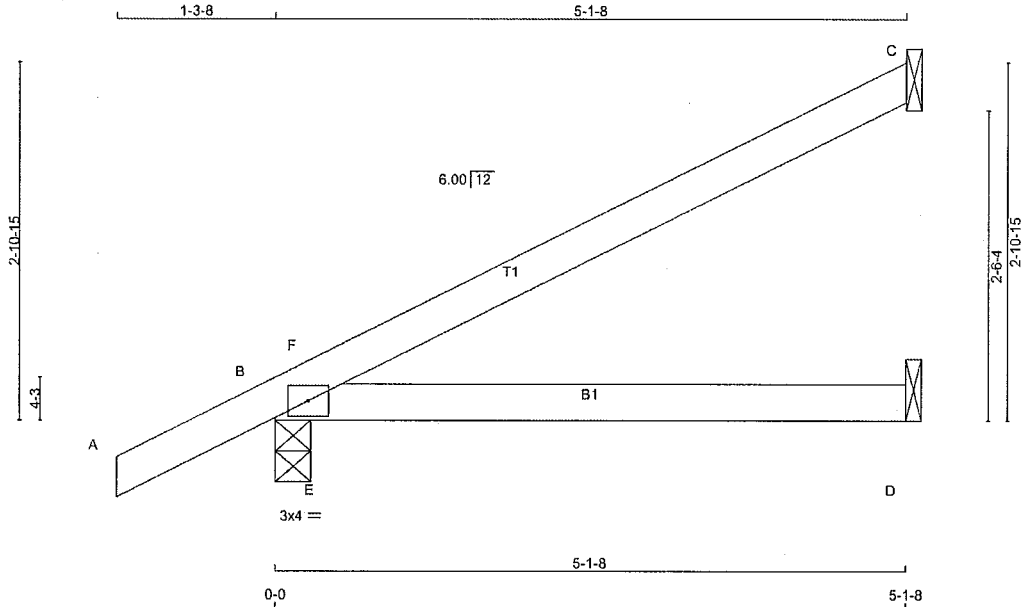
STRUCTURAL COMPONENT ONLY

DWG # TR24040181

REVIEWED

JOB NAME 437040	TRUSS NAME J5	QUANTITY 7	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:Sb 47nW9yvpKeYGkKmELfPzxU46-hskmHsaS7K757VDRPx AhBnmd1x5kNFW4kltUzUnea



TOTAL WEIGHT = 7 X 14 = 98 lb

LUMBER

N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
B - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
C	250	0	250	0	0	1-8	1-8	
B	488	0	488	0	0	3-8	1-8	
D	85	0	85	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	DOWN	HORZ				
C	172	141/0	0/0	0/0	0/0	0/0	31/0	0/0
B	340	255/0	0/0	0/0	0/0	0/0	85/0	0/0
D	63	25/0	0/0	0/0	0/0	0/0	37/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0/33	-112.4	-112.4	0.15 (1)	10.00	E-F	-474/8
B-F	-24/88	-112.4	-112.4	0.11 (1)	6.25		
F-C	-7/2	-112.4	-112.4	0.38 (1)	10.00		
B-E	0/0	-18.5	-18.5	0.27 (1)	10.00		
E-D	0/0	-18.5	-18.5	0.27 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/630 (0.10")

CSI: TC=0.38/1.00 (C-F:1), BC=0.27/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.38/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)

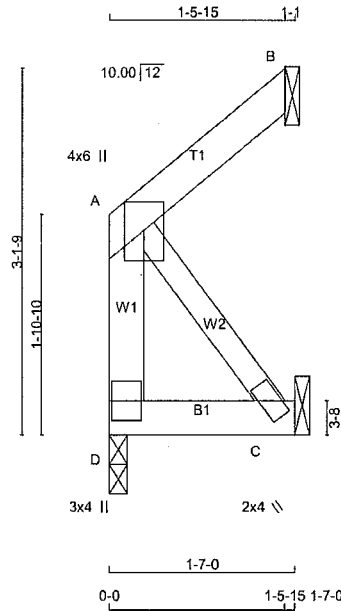


STRUCTURAL COMPONENT ONLY
DWG # TR24040182

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437040	C1	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:18.8

TOTAL WEIGHT = 8 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
D - A	2x4 DRY	No.2	SPF
A - B	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW+p	MT20	4.0	6.0 Edge
C	BMV1+w	MT20	2.0	4.0
D	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
D	97	0	0	97	0	0	1-13	1-8	
B	84	0	0	84	0	0	1-8	1-8	
C	13	0	0	14	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
D	68	49/0	0/0	0/0	0/0	19/0	0/0		
B	58	49/0	0/0	0/0	0/0	9/0	0/0		
C	10	0/0	0/0	0/0	0/0	10/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
D-A	-84/0	D-C	0/0
A-B	0/0		
D-C	0/0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2018AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.01/1.00 (C-D:4), WB=0.00/1.00 (A-C:1), SSI=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR24040176

REVIEWED

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 12 lb

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)



REVIEWED

EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

REVIEWED

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

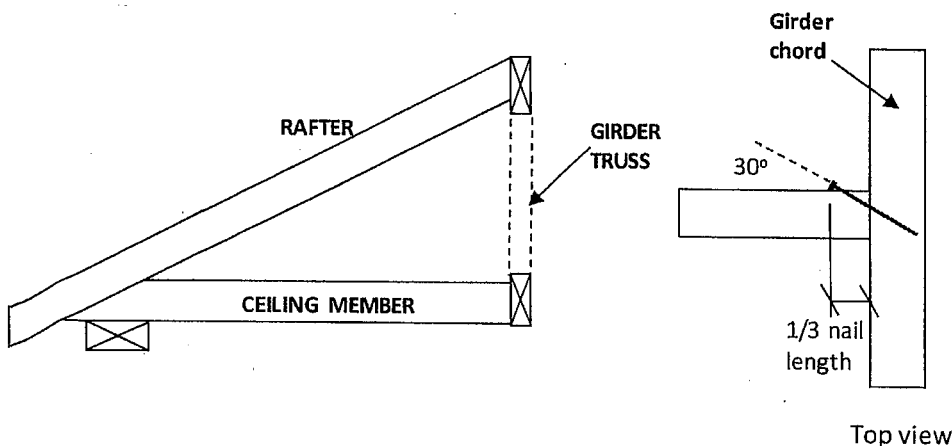


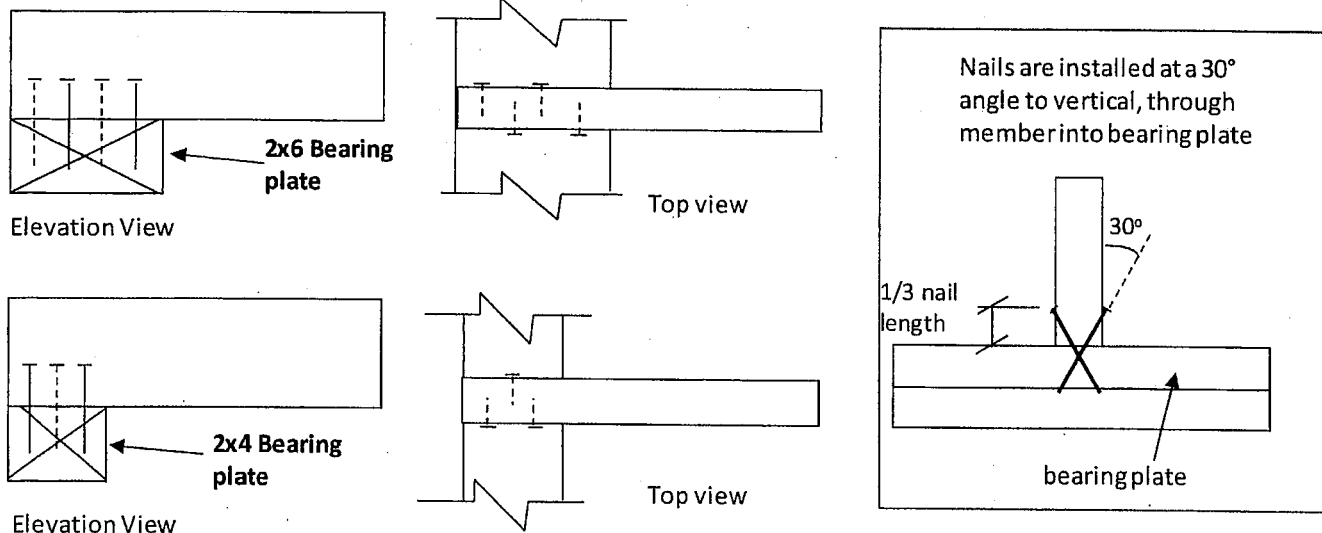
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_0 factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



TECHNICAL BULLETIN

LUS — Double-Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

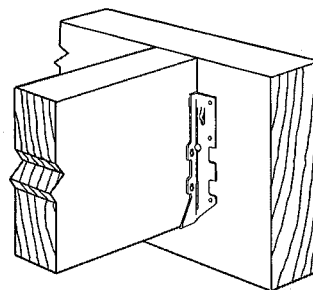
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

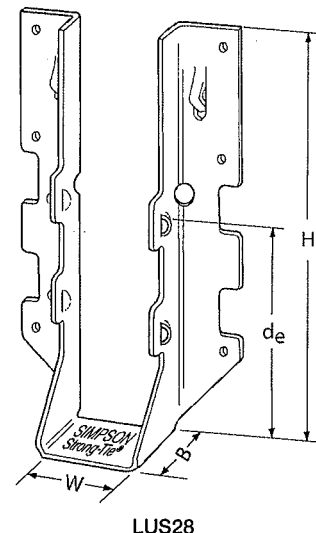
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- These hangers cannot be modified

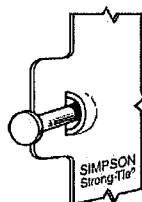


Typical LUS
Installation



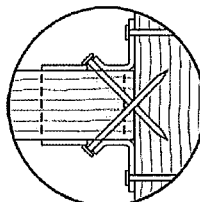
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1¾	3½	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1¾	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1¾	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1¾	7 13/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double double-shear nailing prevents tabs breaking off (available on some models).

US Patent
5,603,580



Double-shear nailing top view.



This technical bulletin is effective until December 31, 2024, and reflects information available as of July 1, 2022. This information is updated periodically and should not be relied upon after December 31, 2024. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS22 7/22 exp. 12/24

(800) 999-5099
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REVIEWED

HUS/LJS — Double-Shear Joist Hangers



All hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

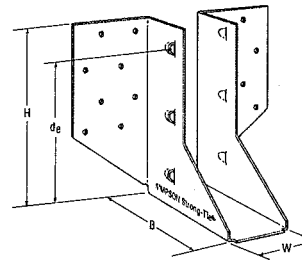
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

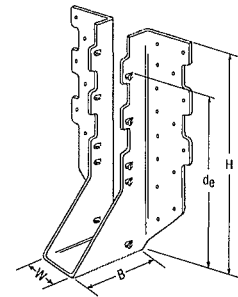
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

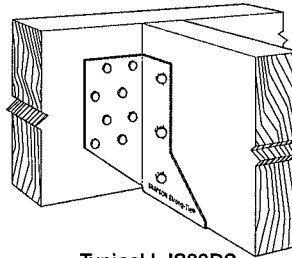
- See current catalogue for options



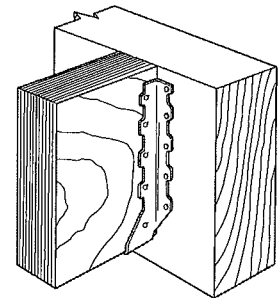
LJS26DS



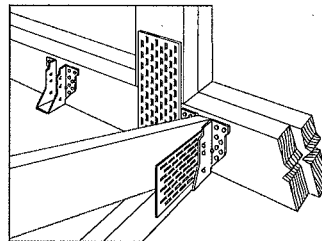
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



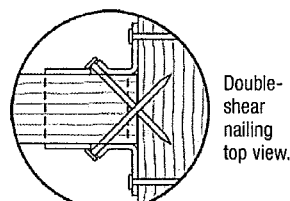
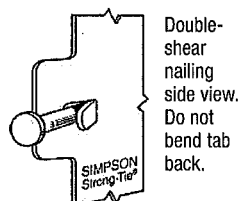
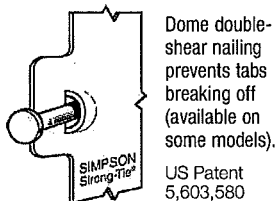
Typical HUS
Installation



Typical HUS Installation
(Truss designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _b ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _b =1.15)	Normal (K _b =1.00)	Uplift (K _b =1.15)	Normal (K _b =1.00)
LJS26DS	18	1⅞	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅞	5⅝	3	3⅞	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅞	9⅞	3	7⅞	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅞	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_b is the distance from the seat of the hanger to the highest joist nail.



This technical bulletin is effective until December 31, 2024, and reflects information available as of July 1, 2022. This information is updated periodically and should not be relied upon after December 31, 2024. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
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REVIEWED

TECHNICAL BULLETIN

HGUS — Double-Shear Joist Hangers



All HGUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

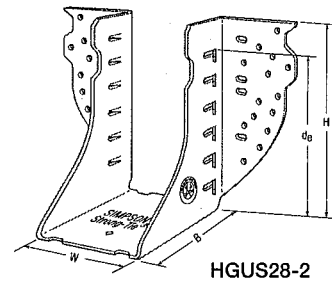
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

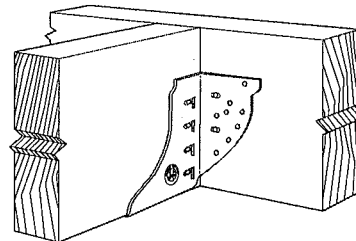
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

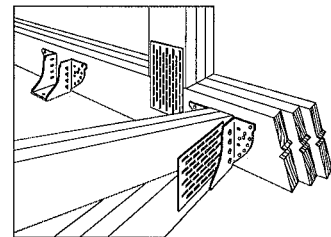
- See current catalogue for options



HGUS28-2



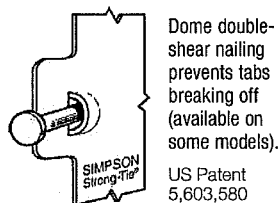
Typical HGUS Installation



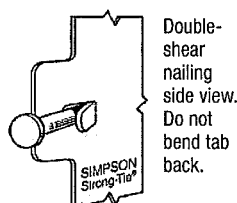
Typical HGUS Installation
(Truss designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HGUS26	12	1⅝	5⅝	5	4⅜	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅝	5⅞	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅝	5½	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅝	5⅞	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅞	5	6⅝	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅝	7⅞	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅝	7¼	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅝	7⅞	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3⅝	9⅞	4	8⅝	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅝	9¼	4	8⅝	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅝	9⅞	4	8⅝	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅝	10⅝	4	10⅝	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅝	12⅝	4	11⅝	(66) 16d	(22) 16d	10130	16400	7195	11645

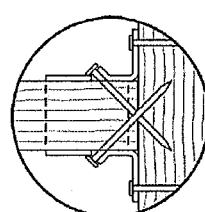
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome double-shear nailing prevents tabs breaking off (available on some models).
US Patent 5,603,580



Double-shear nailing side view. Do not bend tab back.



Double-shear nailing top view.



This technical bulletin is effective until December 31, 2024, and reflects information available as of July 1, 2022. This information is updated periodically and should not be relied upon after December 31, 2024. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS22 7/22 exp. 12/24

(800) 999-5099
strongtie.com

REVIEWED

H — Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge

Finish: G90 galvanized

Design:

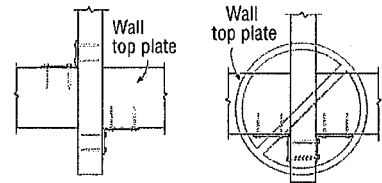
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Factored resistances have been increased 15%. No further increase is permitted.

Installation:

- Use all specified fasteners
- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

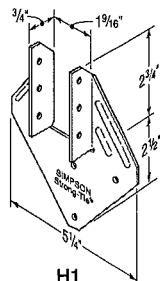
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

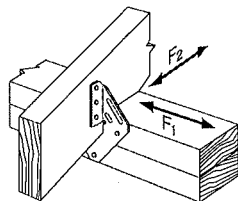


Install diagonally across from each other for minimum 2x truss.

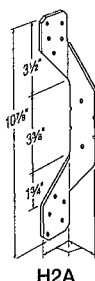
Nailing into both sides of a single ply 2x truss may cause the wood to split.



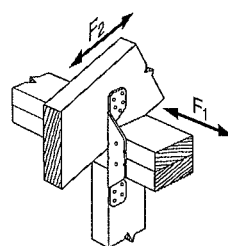
H1



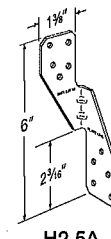
H1 Installation



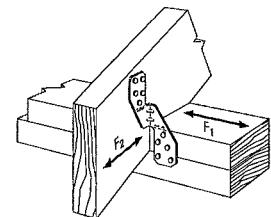
H2A



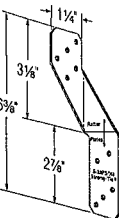
H2A Installation



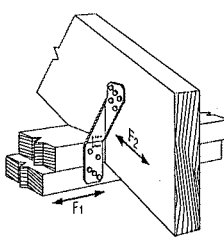
H2.5A



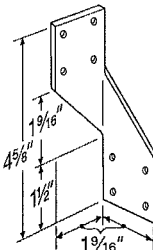
H2.5A Installation



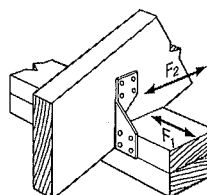
H2.5T



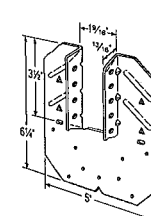
H2.5T Installation
(Nails into both top plates)



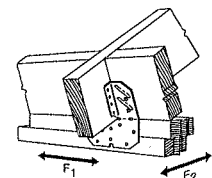
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
										(K ₀ =1.15)
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



This technical bulletin is effective until December 31, 2024, and reflects information available as of July 1, 2022. This information is updated periodically and should not be relied upon after December 31, 2024. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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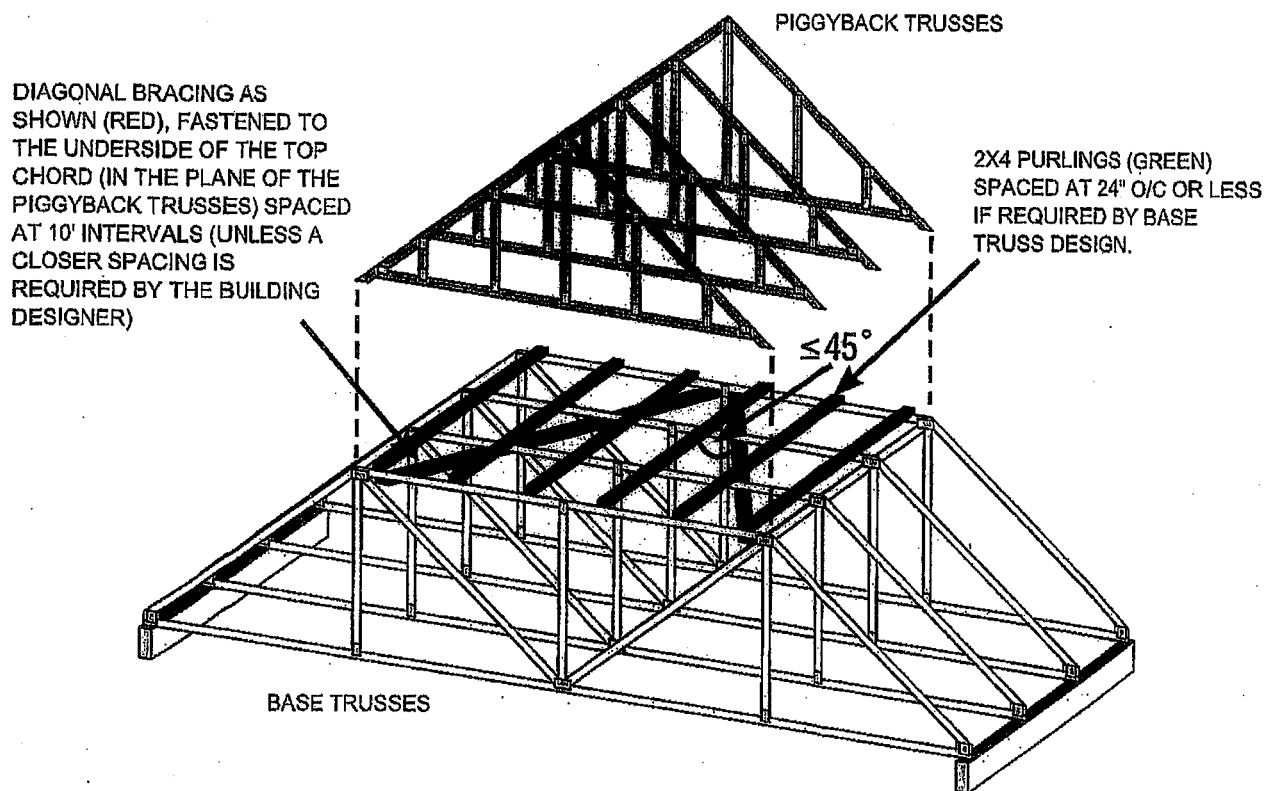
REVIEWED

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

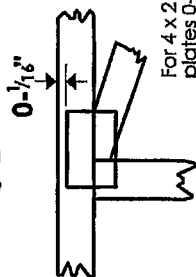
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

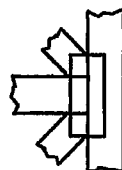
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use L, I or Eliminator bracing if indicated.

BEARING



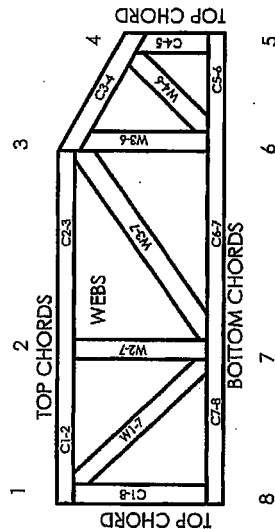
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MI-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

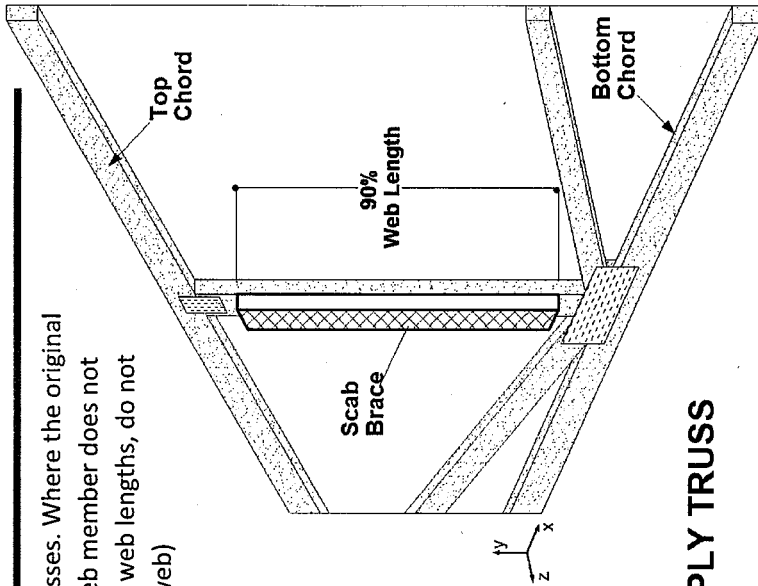
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

ALTERNATIVE WEB BRACING SOLUTIONS

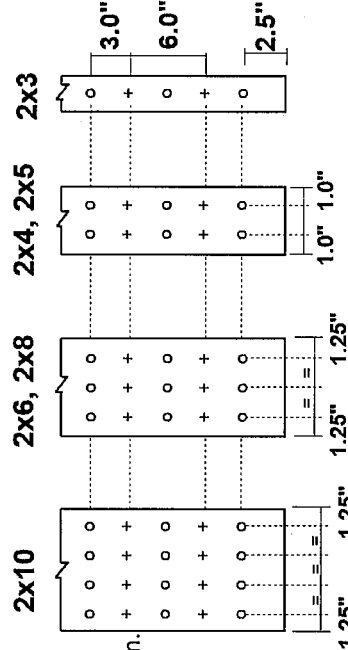
The scab brace detail shown on this page provides an alternative method of bracing compression webs of single ply trusses. Where the original design calls for web bracing, the scab-brace is an acceptable alternative provided that the factored axial force in the web member does not exceed the tabulated values shown below. This detail applies to web lengths of 4.0 ft. to 10.0 ft. only. For intermediate web lengths, do not interpolate, use the tabulated value of the longer length. (ex. For a 6.25 ft. web, use the tabulated values for a 6.5 ft. web)

Maximum factored web force, lbs (1-Ply Truss)						
Web size	2x3	2x4	2x5	2x6	2x8+	
4.0	4331	6064	7796	9529	12561	
4.5	3794	5312	6829	8347	11003	
5.0	3285	4599	5913	7227	9527	
5.5	2823	3952	5081	6210	8186	
6.0	2415	3381	4347	5313	7003	
6.5	2063	2888	3713	4538	5982	
7.0	1763	2468	3174	3879	5113	
7.5	1510	2114	2718	3322	4379	
8.0	1297	1816	2335	2854	3762	
8.5	1117	1564	2011	2458	3240	
9.0	966	1353	1740	2126	2803	
9.5	840	1176	1512	1848	2436	
10.0	733	1027	1320	1614	2127	

SCAB BRACE DETAIL 1-PLY TRUSS



SCAB CONNECTION: 1-PLY TRUSS



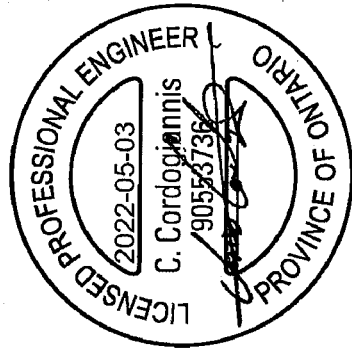
NOTES:

1. This detail **CANNOT** be used to repair damaged webs.
2. Scab and web sizes must be equal (i.e. use a 2x6 scab on a 2x6 web, etc.).
3. Scab & web lumber must be DRY ($\leq 19\%$ moisture content) at time of installation.
4. Scab must cover minimum 90% of the entire length of web.
5. For 2x12 webs use 2x10 nail pattern, but with 5 rows of nails instead of 4 rows.
6. This detail is for webs loaded axially only (not for axial/bending members).
7. Web and scab lumber shall be SPF No. 2 (or better) grade.
8. Tabulated resistances are for standard load duration only ($K_D=1.0$) and DRY service conditions ($K_S=1.0$). Do not use detail for WET service applications.
9. This detail shall be used only in conjunction with sealed MiTek truss drawings.

- + 0.122" dia. x 3.0" nail driven from front face
- o 0.122" dia. x 3.0" nail driven from back face

Note: Connect scabs to truss along their entire length.

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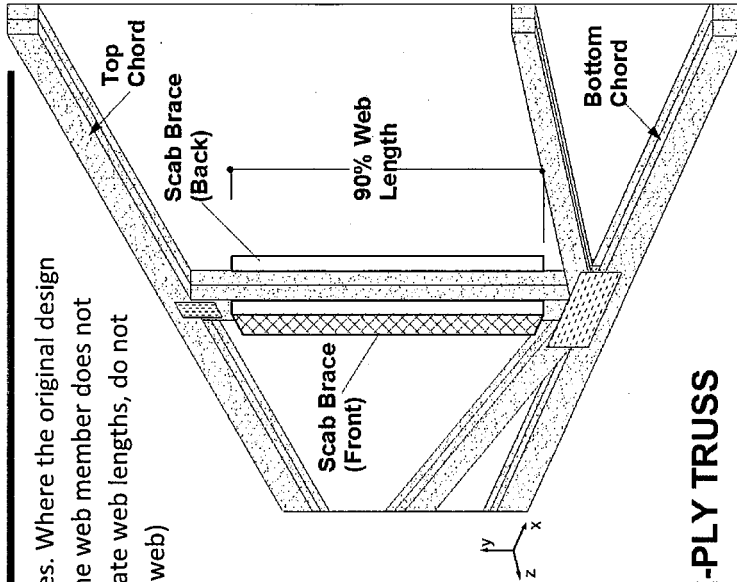
ALTERNATIVE WEB BRACING SOLUTIONS

The scab brace detail shown on this page provides an alternative method of bracing compression webs of 2-PLY trusses. Where the original design calls for web bracing, the scab-brace is an acceptable alternative provided that the maximum factored axial force in the web member does not exceed the tabulated values shown below. This detail applies to web lengths of 4.0 Ft. to 10.0 Ft. only. For intermediate web lengths, do not interpolate, use the tabulated value of the longer length. (ex. For a 6.25 ft. web, use the tabulated values for a 6.5 ft. web)

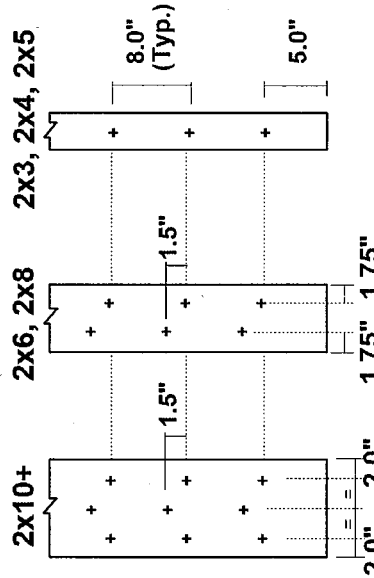
Maximum factored web force, lbs (2-Ply Truss)

Web size	2x3	2x4	2x5	2x6	2x8+
4.0	8663	12128	15593	19058	25122
4.5	7588	10623	13659	16694	22006
5.0	6570	9198	11826	14455	19054
5.5	5645	7903	10162	12420	16371
6.0	4830	6762	8694	10626	14007
6.5	4126	5776	7426	9077	11965
7.0	3526	4937	6347	7758	10226
7.5	3020	4228	5436	6644	8758
8.0	2594	3632	4670	5708	7524
8.5	2235	3128	4022	4916	6480
9.0	1933	2706	3479	4253	5606
9.5	1680	2352	3024	3696	4872
10.0	1467	2054	2640	3227	4254

SCAB BRACE DETAIL 2-PLY TRUSS



SCAB CONNECTION: 2-PLY TRUSS



NOTES:

1. This detail **CANNOT** be used to repair damaged webs.
2. Scab sizes must be equal to web size (i.e. use a 2x6 scab on a 2x6 web, etc.).
3. Scabs & web lumber must be DRY ($\leq 19\%$ moisture content) at time of installation.
4. Scabs must cover 90% of the entire length of web and installed on both faces.
5. This detail shall **NOT** apply to vertical webs used for girder load transfer.
6. Web & scab lumber to be SPF No. 2 (or better) grade.
7. This detail is for webs loaded axially only (not for axial/bending members).
8. Ensure scabs will not interfere with incoming trusses, prior to using this detail.
9. Tabulated resistances are for standard load duration only ($K_D=1.0$) and DRY service conditions ($K_S=1.0$). Do not use detail for WET service applications.
10. This detail shall be used only in conjunction with sealed MiTek truss drawings.

+ MITEK MIFLK006 Screw @ 8 in. cc

Note: Connect scabs to truss along their entire length.

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Certificate No. 10889485

