UNIT 1
THWU-15E
ELEV. 'A'
437027

UNIT 2
THWU-12
ELEV. 'A' (REV.)
437028

UNIT 3
THWU-12
ELEV. 'A'
437029

UNIT 4
THWU-21C
ELEV. 'A' (REV.)
437030

82-08-00

20-02-00   19-08-00   19-08-00   22-02-00   5-00-00   1-00-00

10" PLATE DIFF.   8" PLATE DIFF.

= 1 hr F.R.R.

6/12   6/12   6/12

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

ROOF PITCHES TO BE 10/12 UNLESS NOTED OTHERWISE

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.9 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES;
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26 - (X)
HGUS26-2 - (XX)
HGUS26-3 - (XXX)
H2.5T - (I)
H2.5A - (A)
LGT3-SDS2.5 - (H)
LSTA9 - (9)

DB - DROPPED BEAMS BY OTHERS

5/12 INT. PITCH VAULTED CEILING

11-04-00   8-10-00   9-04-00   10-04-00   10-04-00   9-04-00   9-10-00   12-04-00   5-00-00

20-02-00   19-08-00   19-08-00   23-02-00

55-04-00   7-00-00   56-04-00   15-00-00   13-01-00   6-00-00   28-03-00

M16104

TAMARACK ROOF TRUSSES INC.

Job Track: 53568
Plan Log: 207900
Layout ID: 437026

Builder / Location:
BAYVIEW WELLINGTON / BRADFORD

Model / Elevation:
BLOCK 403-2 / UNIT 1-4

Project: GREEN VALLEY ESTATE (2024)
Date: 2024-04-03   Sales: Rick DiCiano   Designer:

Mitek ver 8.6.3.353
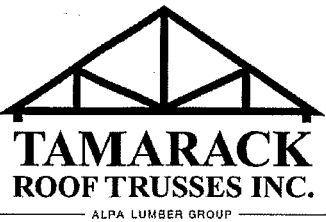
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	53568
Builder:	BAYVIEW WELLINGTON	PlanLog:	207900
Project:	GREEN VALLEY ESTATE (2024)	Layout ID:	437027
Location:	BRADFORD	Ref #	
Model:	BLOCK 403-2	Page:	1 of 2
Lot #:		Date:	04-03-2024
Elevation:	UNIT 1	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Half Hip Girder	10 /12	19-06-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	192.62 123.00		
	2	T2 Half Hip	10 /12	19-06-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	170.45 109.00		
	2	T3 Half Hip	10 /12	19-06-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	182.6 117.33		
	2	T4 Half Hip	10 /12	19-06-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	181.94 112.67		
	2	T5 Half Hip	10 /12	19-06-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	192.07 121.33		
	8	T6 Half Hip	10 /12	19-06-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	829.31 518.67		
	2	T7 Half Hip	10 /12	19-06-08	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	232.34 145.67		
	1 2-ply	T8 Hip Girder	10 /12	19-06-08	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 1-10-10	199.59 127.33		
	1	T9 Hip	10 /12	19-06-08	6-04-12	2 x 4	1-03-08	1-07-11 1-10-10	93.46 60.00		
	3	T10 Common	10 /12	10-06-00	6-00-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	141.64 90.00		
	3	T10S Scissor	10 /12 5 /12	10-06-00	6-00-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	141 89.00		
	1 2-ply	T11 Monopitch Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1	T12 Half Hip Girder	0 /12	8-07-08	3-00-15	2 x 6 2 x 4		2-08-11 3-00-15	42.35 27.17		
	2	PB1 Piggyback	10 /12	10-07-00	2-00-00	2 x 4		2-00-00	68.49 45.33		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC. — ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	GREEN VALLEY ESTATE (2024)
Location:	BRADFORD
Model:	BLOCK 403-2
Lot #:	
Elevation:	UNIT 1

Job Track:	53568
PlanLog:	207900
Layout ID:	437027
Ref # Page:	2 of 2
Date:	04-03-2024
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	PB2 Piggyback	10/12	10-07-00	3-00-00	2 x 4		3-00-00	71.17 44.33		
	2	PB3 Piggyback	10/12	10-07-00	3-10-08	2 x 4		3-10-08	72.63 47.00		
	9	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	3	J2 Jack-Open	10/12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.42 29.00		
	5	J3W Jack-Open	6/12	6-07-00	4-08-03	2 x 4	1-03-08	4-03 3-07-11	118.37 72.50		
	5	J4 Jack-Open	6/12	5-05-08	3-08-07	2 x 4	1-03-08	4-03 3-00-15	74.98 46.67		
	1	C1 Jack-Open	10/12	1-07-00	3-01-09	2 x 4		1-10-10 3-02-07	7.5 5.67		
	1	C2 Jack-Open	10/12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.04 8.33		

TOTAL # TRUSS= **62**   TOTAL BFT OF ALL TRUSSES= **2065.33** BFT.   TOTAL WEIGHT OF ALL TRSSES **3264.28** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
1	Hardware	HGUS26-2	
28	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= **31**

REVIEWED

DELIVERY SHIPLIST
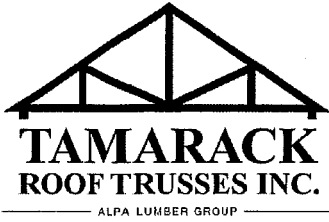
Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	GREEN VALLEY ESTATE (2024)
Location:	BRADFORD
Model:	BLOCK 403-2
Lot #:	
Elevation:	UNIT 2

Job Track:	53568
PlanLog:	207900
Layout ID:	437028
Ref #	
Page:	1 of 2
Date:	04-03-2024
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T14 Piggyback Base	6 /12	46-10-00	9-07-08	2 x 6	1-03-08	4-00 1-02-00	297.27 179.17		
	1	T15 Piggyback Base	6 /12	46-10-00	9-07-08	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	316.76 191.67		
	1	T16 Piggyback Base	6 /12	46-10-00	9-07-08	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	327.6 199.33		
	1	T17 Piggyback Base	6 /12	46-10-00	9-07-08	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	324.35 195.00		
	1	T18 Piggyback Base	6 /12	46-10-00	9-07-08	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	343.27 204.33		
	1 2-ply	T19 Hip Girder	6 /12	46-10-00	9-07-08	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	660.59 394.67		
	4	T22 Piggyback Base	6 /12	38-05-00	9-07-08	2 x 4 2 x 6	1-03-08	1-02-00 5-04-08	842.8 517.33		
	1	T23G GABLE	6 /12	37-09-00	9-07-08	2 x 4 2 x 6	1-03-08	1-02-00 5-08-08	235.77 144.17		
	2	T24 Monopitch	10 /12	9-08-08	9-08-12	2 x 4	1-03-08	1-07-11 9-08-12	105.65 67.33		
	4	T24A Monopitch	10 /12	9-08-08	9-08-12	2 x 4		1-07-11 9-08-12	203.63 129.33		
	1	T24G GABLE	10 /12	9-08-08	9-08-12	2 x 4	1-03-08	1-07-11 9-08-12	57.07 36.67		
	1 3-ply	T25 Monopitch Girder	10 /12	9-08-08	9-07-08	2 x 4 2 x 6		1-07-11 9-08-12	226.42 140.00		
	5	PB4 Piggyback	6 /12	11-04-00	2-10-00	2 x 4			134.19 87.50		
	1	PB5 Piggyback	6 /12	13-00-00	1-07-10	2 x 4			34.95 23.00		

REVIEWED

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	GREEN VALLEY ESTATE (2024)
Location:	BRADFORD
Model:	BLOCK 403-2
Lot #:	
Elevation:	UNIT 2

Job Track:	53568
PlanLog:	207900
Layout ID:	437028
Ref #	
Page:	2 of 2
Date:	04-03-2024
Designer:	
Sales Rep:	Rick DiCiano
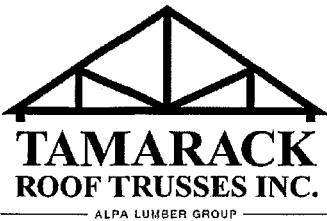
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	PB6 Piggyback	6 /12	13-00-00	3-03-00	2 x 4			143.55 92.67		
	6	J3W Jack-Open	6 /12	6-07-00	4-08-03	2 x 4	1-03-08	4-03 3-07-11	142.06 90.00		

TOTAL # TRUSS= **38**   TOTAL BFT OF ALL TRUSSES= **2692.17** BFT.   TOTAL WEIGHT OF ALL TRSSES **4395.93** LBS
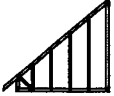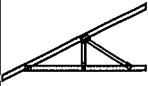
HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	
5	Hardware	LJS26DS	
1	Hardware	HGUS26-3	
14	Hardware	H2.5T	
24	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= **48**

REVIEWED

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	53568
Builder:	BAYVIEW WELLINGTON	PlanLog:	207900
Project:	GREEN VALLEY ESTATE (2024)	Layout ID:	437029
Location:	BRADFORD	Ref #	
Model:	BLOCK 403-2	Page:	1 of 1
Lot #:		Date:	04-03-2024
Elevation:	UNIT 3	Designer:	
		Sales Rep:	Rick DiCiano
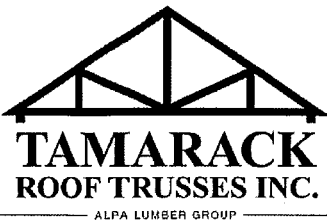
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	10	T20 Piggyback Base	6 /12	46-10-00	9-07-08	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	3107.82 1853.33		
	1	T23G GABLE	6 /12	37-09-00	9-07-08	2 x 4 2 x 6	1-03-08	1-02-00 5-08-08	235.77 144.17		
	2	T24 Monopitch	10 /12	9-08-08	9-08-12	2 x 4	1-03-08	1-07-11 9-08-12	105.65 67.33		
	1	T24G GABLE	10 /12	9-08-08	9-08-12	2 x 4	1-03-08	1-07-11 9-08-12	57.07 36.67		
	1 3-ply	T26 Monopitch Girder	10 /12	9-10-08	9-07-08	2 x 4 2 x 8		1-07-11 9-10-07	231.89 145.00		
	11	PB6 Piggyback	6 /12	13-00-00	3-03-00	2 x 4			394.77 254.83		
	6	J3W Jack-Open	6 /12	6-07-00	4-08-03	2 x 4	1-03-08	4-03 3-07-11	142.06 90.00		
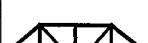
TOTAL # TRUSS= **34**   TOTAL BFT OF ALL TRUSSES= **2591.33** BFT.   TOTAL WEIGHT OF ALL TRSSES **4275.02** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	HGUS26	
8	Hardware	H2.5T	
32	Hardware	H2.5A	
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= **46**

REVIEWED

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	GREEN VALLEY ESTATE (2024)
Location:	BRADFORD
Model:	BLOCK 403-2
Lot #:	
Elevation:	UNIT 4

Job Track:	53568
PlanLog:	207900
Layout ID:	437030
Ref #	
Page:	1 of 3
Date:	04-03-2024
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T30 Hip Girder	10 /12	21-06-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 1-10-10	204.68 129.67		
	1	T31 Hip	10 /12	21-06-08	5-01-04	2 x 4	1-03-08	1-07-11 1-10-10	93.7 61.00		
	1	T32 Hip	10 /12	21-06-08	6-01-04	2 x 4	1-03-08	1-07-11 1-10-10	92.43 58.50		
	1	T33 Half Hip	10 /12	21-06-08	6-05-04	2 x 4	1-03-08	1-07-11 6-05-04	101.5 63.33		
	1	T33A Half Hip	10 /12	21-01-08	6-05-04	2 x 4		1-11-14 6-05-04	98.69 62.00		
	1	T34 Half Hip	10 /12	21-06-08	7-05-04	2 x 4	1-03-08	1-07-11 7-05-04	108.11 68.00		
	1	T34A Half Hip	10 /12	21-01-08	7-05-04	2 x 4		1-11-14 7-05-04	105.29 66.17		
	1	T35 Half Hip	10 /12	21-06-08	8-05-04	2 x 4	1-03-08	1-07-11 8-05-04	117.17 72.33		
	1	T35A Half Hip	10 /12	21-01-08	8-05-04	2 x 4		1-11-14 8-05-04	114.34 71.00		
	4	T36 Half Hip	10 /12	21-06-08	9-05-04	2 x 4	1-03-08	1-07-11 9-05-04	471.22 293.33		
	1	T36A Half Hip	10 /12	21-01-08	9-05-04	2 x 4		1-11-14 9-05-04	114.96 71.33		
	2	T37 Half Hip	10 /12	21-06-08	10-05-04	2 x 4	1-03-08	1-07-11 10-05-04	247.56 153.33		
	1 2-ply	T38 Hip Girder	10 /12	21-06-08	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 1-10-10	213.62 137.00		
	1	T39 Hip	10 /12	21-06-08	6-01-07	2 x 4		1-07-11 1-10-10	90.59 58.33		

REVIEWED

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	GREEN VALLEY ESTATE (2024)
Location:	BRADFORD
Model:	BLOCK 403-2
Lot #:	
Elevation:	UNIT 4

Job Track:	53568
PlanLog:	207900
Layout ID:	437030
Ref #	
Page:	2 of 3
Date:	04-03-2024
Designer:	
Sales Rep:	Rick DiCiano

TAMARACK ROOF TRUSSES INC.
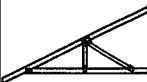ALPA LUMBER GROUP

Roof Trusses
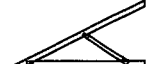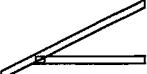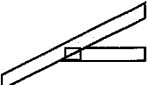
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T40 Hip	10/12	21-01-08	7-09-07	2 x 4		1-11-14 1-10-10	99.3 64.00		
	1	T41 Hip	10/12	21-01-08	9-05-07	2 x 4		1-11-14 1-10-10	107.22 68.67		
	1	T42 Common	10/12	21-01-08	10-08-14	2 x 4		1-11-14 1-10-10	101.46 64.00		
	1 2-ply	T43 Common Girder	10/12	14-02-00	7-06-08	2 x 4 2 x 6		1-07-11 1-07-11	153.47 101.67		
	1	T43G GABLE	10/12	14-02-00	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	69.95 46.17		
	2	T44 Common	10/12	12-04-00	6-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.86 76.33		
	1	T44G GABLE	10/12	12-04-00	6-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	60.03 39.17		
	5	T45 Common	10/12	11-06-00	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	280.5 185.00		
	1	T45G GABLE	10/12	11-06-00	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	55.9 37.00		
	2	T46 Common	6/12	14-11-00	4-00-15	2 x 4		4-03 4-03	90.89 56.00		
	1	T46A Common	6/12	12-08-00	4-00-15	2 x 4		4-03 1-05-11	43.92 27.17		
	1	T46G GABLE	6/12	14-11-00	4-00-15	2 x 4		4-03 4-03	45.73 29.00		
	1	T47 Flat Girder	0/12	9-07-08	2-06-15	2 x 6 2 x 4		2-06-15 2-06-15	44.1 28.00		
	1 2-ply	T48 Monopitch Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		

REVIEWED

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	GREEN VALLEY ESTATE (2024)
Location:	BRADFORD
Model:	BLOCK 403-2
Lot #:	
Elevation:	UNIT 4

Job Track:	53568
PlanLog:	207900
Layout ID:	437030
Ref #	
Page:	3 of 3
Date:	04-03-2024
Designer:	
Sales Rep:	Rick DiCiano

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T49 Half Hip Girder	6 /12	6-03-00	1-10-11	2 x 4	1-03-08	4-03 / 1-03-03	20.88 / 13.83		
	1 2-ply	T11Z Monopitch Girder	10 /12	3-10-08	4-10-07	2 x 4 / 2 x 6		1-07-11 / 4-10-07	43.15 / 29.33		
	3	PB7 Piggyback	10 /12	12-02-04	2-00-00	2 x 4		2-00-00	117.39 / 77.00		
	4	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 / 4-01-04	67.18 / 42.67		
	4	J2 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 / 4-10-07	60.56 / 38.67		
	8	J5W Jack-Open	6 /12	5-07-00	4-02-03	2 x 4	1-03-08	4-03 / 3-01-11	156.18 / 97.33		
	7	J6W Jack-Open	6 /12	4-07-00	3-08-03	2 x 4	1-03-08	4-03 / 2-07-11	104.27 / 65.33		
	6	J7 Jack-Open	6 /12	4-05-08	3-02-07	2 x 4	1-03-08	4-03 / 2-06-15	75.48 / 48.00		
	3	J8 Jack-Open	6 /12	1-10-00	1-10-11	2 x 4	1-03-08	4-03 / 1-03-03	18.72 / 12.00		
	1	C1 Jack-Open	10 /12	1-07-00	3-01-09	2 x 4		1-10-10 / 3-02-07	7.5 / 5.67		
	1	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 / 2-01-01	1-07-11 / 3-01-09	12.04 / 8.33		

TOTAL # TRUSS= **82**   TOTAL BFT OF ALL TRUSSES= **2663.33** BFT.   TOTAL WEIGHT OF ALL TRSSES **4186.95** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
11	Hardware	LJS26DS	
2	Hardware	HGUS26-2	
48	Hardware	H2.5T	
5	Hardware	LSTA9	

TOTAL NUMBER OF ITEMS= **66**

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:56 2024 Page 1
ID:AqdClOSevh3uN4Xyl113N2zyjH4-aGBjYTy5rtgeail44XnfBECABgJ3yVAIVUMrx3zUnxT

1-3-8   2-11-8   11-12   15-7-4

Scale = 1:33.3

10.00 [12]
5x6 ||
5x6 \\
4x6 =
3x8 =
2x4 ||
4x6 =
4x6 =
4-1-4
13-11
T1 T2 T3
B W1 W2 W3 W4 W3 W4 W3 W4 W3 W4 W3 W4 W5
O X N Y Z M AA AB L AC K J AD AE
4x6 || 5x6 = 4x6 || 5x6 = 5x6 = 5x6 = 4x6 ||

19-6-8

0-0   2-11-8   7-1-10   11-2-8   15-3-6   19-6-8

TOTAL WEIGHT = 2 X 96 = 193 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)

TOP CHORDS : (0.122"X3") SPIRAL NAILS
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
E-H	1	12	SIDE(0.0)
H-I	1	12	TOP
O-B	2	12	TOP

BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS
| O-K | 2 | 12 | SIDE(0.0) |
| K-I | 2 | 12 | SIDE(0.0) |

WEBS : (0.122"X3") SPIRAL NAILS
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED
TO ONE SIDE THAT THE CORRESPONDING NAILING
PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE
SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	2047	0	2047	0	0	1-8	1-8
O	2260	0	2260	0	0	5-8	1-8

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1436	1009 / 0	0 / 0	0 / 0	0 / 0	428 / 0	0 / 0
O	1581	1134 / 0	0 / 0	0 / 0	0 / 0	447 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC1 CSI (LC)	MAX. MAX.
A-B	0 / 50	-112.4 -112.4	0.09 (1)	10.00
B-C	-2019 / 0	-112.4 -112.4	0.11 (1)	6.09
C-P	-2682 / 0	-112.4 -112.4	0.28 (1)	5.23
P-Q	-2682 / 0	-112.4 -112.4	0.28 (1)	5.23
Q-D	-2682 / 0	-112.4 -112.4	0.28 (1)	5.23
D-R	-2746 / 0	-112.4 -112.4	0.28 (1)	5.18
R-E	-2746 / 0	-112.4 -112.4	0.28 (1)	5.18
E-S	-2746 / 0	-112.4 -112.4	0.28 (1)	5.18
S-F	-2746 / 0	-112.4 -112.4	0.28 (1)	5.18
F-T	-2746 / 0	-112.4 -112.4	0.29 (1)	5.18
T-U	-2746 / 0	-112.4 -112.4	0.29 (1)	5.18
U-G	-2746 / 0	-112.4 -112.4	0.29 (1)	5.18
G-V	-1967 / 0	-112.4 -112.4	0.27 (1)	5.90
V-W	-1967 / 0	-112.4 -112.4	0.27 (1)	5.90
W-H	-1967 / 0	-112.4 -112.4	0.27 (1)	5.90
I-H	-1989 / 0	0.0 0.0	0.25 (1)	7.80
O-B	-2238 / 0	0.0 0.0	0.08 (1)	7.81
O-X	0 / 0	-18.5 -18.5	0.03 (4)	10.00
X-N	0 / 0	-18.5 -18.5	0.03 (4)	10.00
N-Y	0 / 1532	-18.5 -18.5	0.12 (1)	10.00
Y-Z	0 / 1532	-18.5 -18.5	0.12 (1)	10.00
Z-M	0 / 1532	-18.5 -18.5	0.12 (1)	10.00
M-AA	0 / 2683	-18.5 -18.5	0.21 (1)	10.00
AA-AB	0 / 2683	-18.5 -18.5	0.21 (1)	10.00
AB-L	0 / 2683	-18.5 -18.5	0.21 (1)	10.00
L-AC	0 / 1967	-18.5 -18.5	0.16 (1)	10.00
AC-K	0 / 1967	-18.5 -18.5	0.16 (1)	10.00
K-J	0 / 1967	-18.5 -18.5	0.16 (1)	10.00
J-AD	0 / 0	-18.5 -18.5	0.04 (4)	10.00
AD-AE	0 / 0	-18.5 -18.5	0.04 (4)	10.00
AE-I	0 / 0	-18.5 -18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-474 / 0	0.06 (1)
C-M	0 / 1539	0.19 (1)
M-D	-898 / 0	0.11 (1)
D-L	0 / 86	0.01 (1)
L-F	-667 / 0	0.08 (1)
L-G	0 / 1056	0.13 (1)
J-G	-1573 / 0	0.20 (1)
J-H	0 / 2614	0.32 (1)
B-N	0 / 1664	0.21 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-29 -29	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-157 -157	---	FRONT	VERT	SNOW	---	C1
K	13-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1
P	3-11-4	-93 -93	---	BACK	VERT	TOTAL	---	C1
Q	5-11-4	-93 -93	---	BACK	VERT	TOTAL	---	C1
R	7-11-4	-93 -93	---	BACK	VERT	TOTAL	---	C1
S	9-11-4	-93 -93	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.08")

CSI: TC=0.29/1.00 (F-G:1) , BC=0.21/1.00 (L-M:1) ,
WB=0.32/1.00 (H-J:1) , SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (H) (INPUT = 0.90)
JSI METAL= 0.29 (H) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-aGBjYTy5rtgeail44XnfBECABgJ3yVAIVUMrx3zUnxT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	4.0	6.0	3.00	Edge
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	11-11-4	-93 -93	---	BACK	VERT	TOTAL	---	C1
U	13-11-4	-93 -93	---	BACK	VERT	TOTAL	---	C1
V	15-11-4	-93 -93	---	BACK	VERT	TOTAL	---	C1
W	17-11-4	-93 -93	---	BACK	VERT	TOTAL	---	C1
X	1-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1
Y	3-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1
Z	5-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1
AA	7-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1
AB	9-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1
AC	11-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1
AD	15-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1
AE	17-11-4	-21 -21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040119

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T2	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:57 2024 Page 1
ID:AqdCIOSevh3uN4XyI113N2zy|H4-2SI5lozjcAoUCstGdEIu|SIHt4fzhy5Sk76OTVzUnxS

1-3-8   4-1-14   15-4-10

Scale = 1:33.3

10.00 / 12

5-1-14

1-7-11

TOTAL WEIGHT = 2 X 85 = 170 lb [M]

19-6-8

0-0   4-1-14   9-3-8   14-3-14   19-6-8

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-I	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1279	0	1279	0	0	1-8	1-8
L	1435	0	1435	0	0	5-8	1-9

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L.

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
L	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT.LOAD (PLF) FROM TO	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4 -112.4	0.15 (1)		10.00	K- C	-173 / 29	0.07 (1)
B- C	-1158 / 0	-112.4 -112.4	0.37 (1)		5.43	B- K	0 / 928	0.21 (1)
C- D	-1357 / 0	-112.4 -112.4	0.43 (1)		5.02	H- F	0 / 1478	0.33 (1)
D- E	-1358 / 0	-112.4 -112.4	0.45 (1)		4.98	C- J	0 / 648	0.15 (1)
E- F	-1086 / 0	-112.4 -112.4	0.43 (1)		5.44	H- E	-894 / 0	0.34 (1)
G- F	-1239 / 0	0.0 0.0	0.55 (1)		7.21	J- D	-617 / 0	0.24 (1)
L- B	-1405 / 0	0.0 0.0	0.15 (1)		6.87	J- E	0 / 375	0.08 (1)
L- K	0 / 0	-18.5 -18.5	0.09 (4)		10.00			
K- J	0 / 883	-18.5 -18.5	0.20 (1)		10.00			
J- I	0 / 1086	-18.5 -18.5	0.23 (1)		10.00			
I- H	0 / 1086	-18.5 -18.5	0.23 (1)		10.00			
H- G	0 / 0	-18.5 -18.5	0.12 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.23/1.00 (H-J:1) ,
WB=0.34/1.00 (E-H:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T3	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:58 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-WfJTz8zLNUwLq0RSBxq7GfHM9U?bQMHbznrx?yzUnxR

Scale = 1:36.3

TOTAL WEIGHT = 2 X 91 = 183 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1279	0	1279	0	0	1-8	1-8
L	1435	0	1435	0	0	5-8	1-9

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
L	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MAX. FACTORED		FACTORED				MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO				
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	K- C	-97 / 61	0.06 (1)	
B- C	-1136 / 0	-112.4 -112.4	0.64 (1)	5.00	B- K	0 / 895	0.20 (1)	
C- D	-1116 / 0	-112.4 -112.4	0.35 (1)	5.53	H- F	0 / 1329	0.30 (1)	
D- E	-1117 / 0	-112.4 -112.4	0.37 (1)	5.50	C- J	0 / 391	0.09 (1)	
E- F	-848 / 0	-112.4 -112.4	0.36 (1)	6.10	H- E	-922 / 0	0.54 (1)	
G- F	-1243 / 0	0.0 0.0	0.90 (1)	7.20	J- D	-568 / 0	0.33 (1)	
L- B	-1394 / 0	0.0 0.0	0.15 (1)	6.90	J- E	0 / 430	0.10 (1)	
L- K	0 / 0	-18.5 -18.5	0.12 (4)	10.00				
K- J	0 / 869	-18.5 -18.5	0.21 (1)	10.00				
J- I	0 / 848	-18.5 -18.5	0.19 (1)	10.00				
I- H	0 / 848	-18.5 -18.5	0.19 (1)	10.00				
H- G	0 / 0	-18.5 -18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.90/1.00 (F-G:1) , BC=0.21/1.00 (J-K:1) ,
WB=0.54/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS: 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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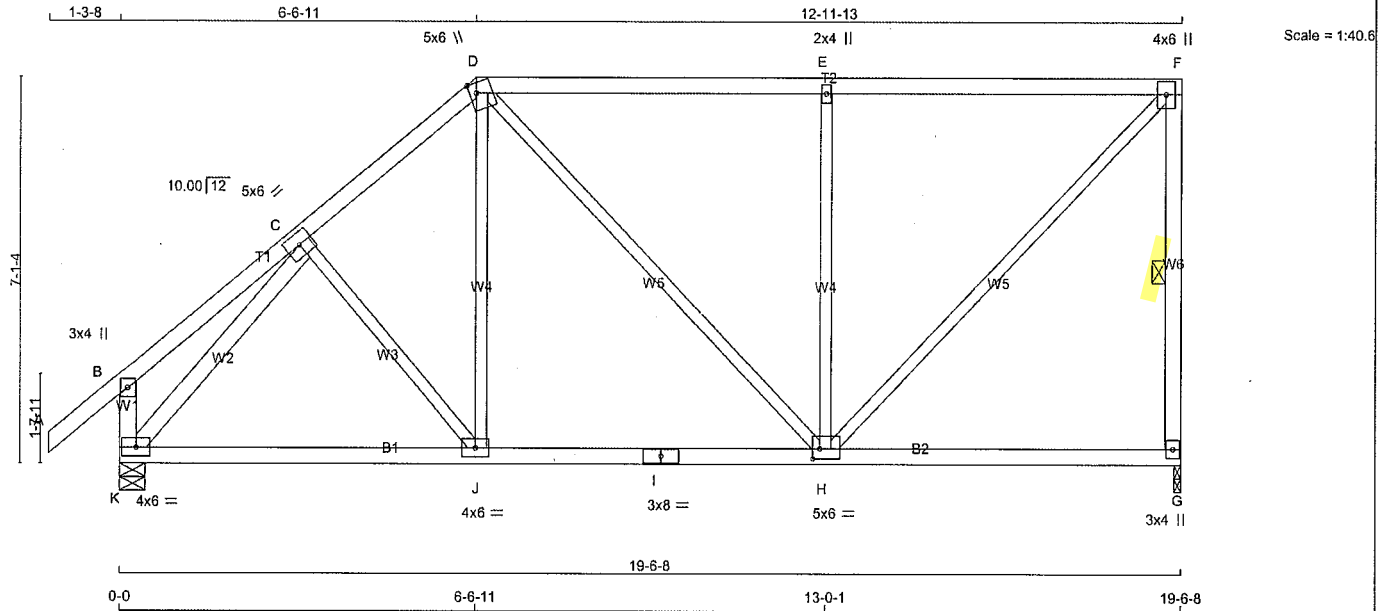
TOTAL WEIGHT = 2 X 93 = 185 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+w	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1279	0	1279	0	0	1-8	1-8
K	1435	0	1435	0	0	5-8	1-9

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
K	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MAX. FACTORED		FACTORED				MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	C- J	-79 / 20	0.04 (1)
B- C	0 / 27	-112.4 -112.4	0.19 (1)	10.00	J- D	0 / 193	0.05 (4)
C- D	-1110 / 0	-112.4 -112.4	0.21 (1)	5.76	D- H	0 / 114	0.03 (1)
D- E	-911 / 0	-112.4 -112.4	0.86 (1)	4.59	H- E	-905 / 0	0.79 (1)
E- F	-911 / 0	-112.4 -112.4	0.87 (1)	4.59	H- F	0 / 1317	0.30 (1)
G- F	-1229 / 0	0.0 0.0	0.28 (1)	5.79	K- C	-1401 / 0	0.43 (1)
K- B	-296 / 0	0.0 0.0	0.03 (1)	7.81			
K- J	0 / 881	-18.5 -18.5	0.24 (1)	10.00			
J- I	0 / 833	-18.5 -18.5	0.24 (4)	10.00			
I- H	0 / 833	-18.5 -18.5	0.24 (4)	10.00			
H- G	0 / 0	-18.5 -18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.87/1.00 (E-F:1) , BC=0.24/1.00 (H-J:4) ,
WB=0.79/1.00 (E-H:1) , SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T5	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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3x4 \\ D   4x6 = E   4x6 || F

10.00 [12 4x6 //

C

T1

3x4 || W2   W3   W4   W5   W4   W5   W6

B
W

K 4x6 =   B1   J   3x8 =   H   B2   G
4x6 =   4x6 || 3x4 ||

1-3-8   7-9-1   11-9-7   Scale = 1:45.2

8-1-4

1-5-11

19-6-8

0-0   7-9-1   13-7-5   19-6-8

TOTAL WEIGHT = 2 X 98 = 196 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMV1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1279	0	1279	0	0	1-8	1-8
K	1435	0	1435	0	0	5-8	1-9

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
K	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 FROM	MAX. TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FR-TO							FR-TO		
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	C- J	-183 / 0	0.12 (1)
B- C	0 / 33	-112.4	-112.4	0.27 (1)	10.00	J- D	0 / 233	0.05 (1)
C- D	-1047 / 0	-112.4	-112.4	0.30 (1)	5.75	J- E	0 / 62	0.01 (4)
D- E	-780 / 0	-112.4	-112.4	0.69 (1)	5.51	H- E	-871 / 0	0.36 (1)
E- F	-744 / 0	-112.4	-112.4	0.69 (1)	5.60	H- F	0 / 1229	0.28 (1)
G- F	-1232 / 0	0.0	0.0	0.37 (1)	5.78	K- C	-1385 / 0	0.59 (1)
K- B	-320 / 0	0.0	0.0	0.03 (1)	7.81			
K- J	0 / 899	-18.5	-18.5	0.30 (4)	10.00			
J- I	0 / 744	-18.5	-18.5	0.29 (4)	10.00			
I- H	0 / 744	-18.5	-18.5	0.29 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.69/1.00 (D-E:1) , BC=0.30/1.00 (J-K:4) ,
WB=0.59/1.00 (C-K:1) , SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.34 (H) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T6	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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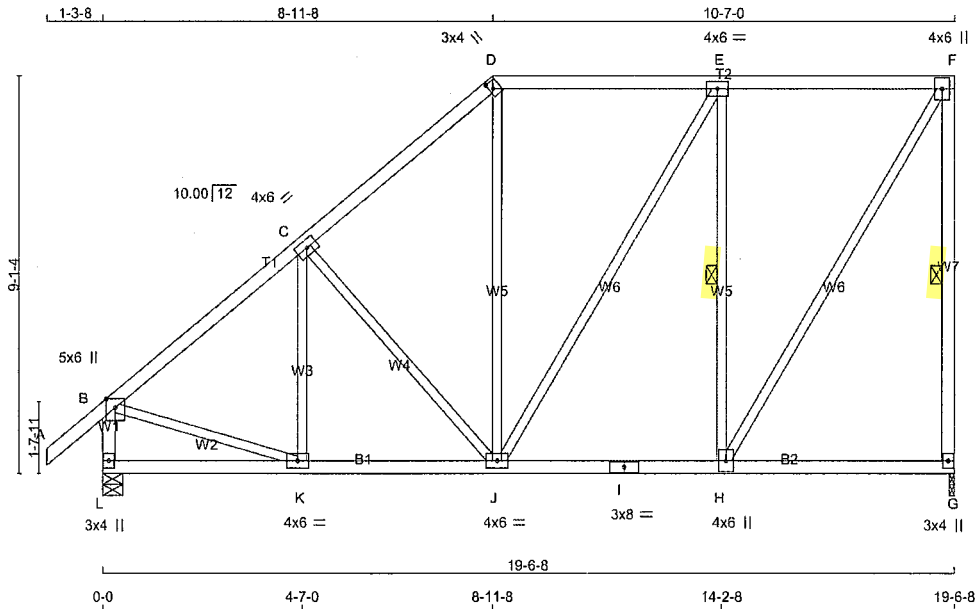
TOTAL WEIGHT = 8 X 104 = 829 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-l	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1279	0	1279	0	0	1-8	1-8
L	1435	0	1435	0	0	5-8	1-9

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L.

UNFACTORED REACTIONS

1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
L	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT.LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	K- C	-175 / 29	0.08 (1)
B- C	-1179 / 0	-112.4 -112.4	0.43 (1)	5.32	C- J	-349 / 0	0.30 (1)
C- D	-960 / 0	-112.4 -112.4	0.41 (1)	5.76	J- D	0 / 182	0.04 (1)
D- E	-707 / 0	-112.4 -112.4	0.55 (1)	6.12	J- E	0 / 190	0.04 (1)
E- F	-612 / 0	-112.4 -112.4	0.54 (1)	6.25	H- E	-900 / 0	0.49 (1)
G- F	-1239 / 0	0.0 0.0	0.48 (1)	5.77	H- F	0 / 1182	0.27 (1)
L- B	-1399 / 0	0.0 0.0	0.15 (1)	6.88	B- K	0 / 972	0.22 (1)
L- K	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
K- J	0 / 935	-18.5 -18.5	0.19 (1)	10.00			
J- I	0 / 612	-18.5 -18.5	0.17 (4)	10.00			
I- H	0 / 612	-18.5 -18.5	0.17 (4)	10.00			
H- G	0 / 0	-18.5 -18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.06")

CSI: TC=0.55/1.00 (D-E:1) , BC=0.19/1.00 (J-K:1) ,
WB=0.49/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T7	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:03 2024 Page 1
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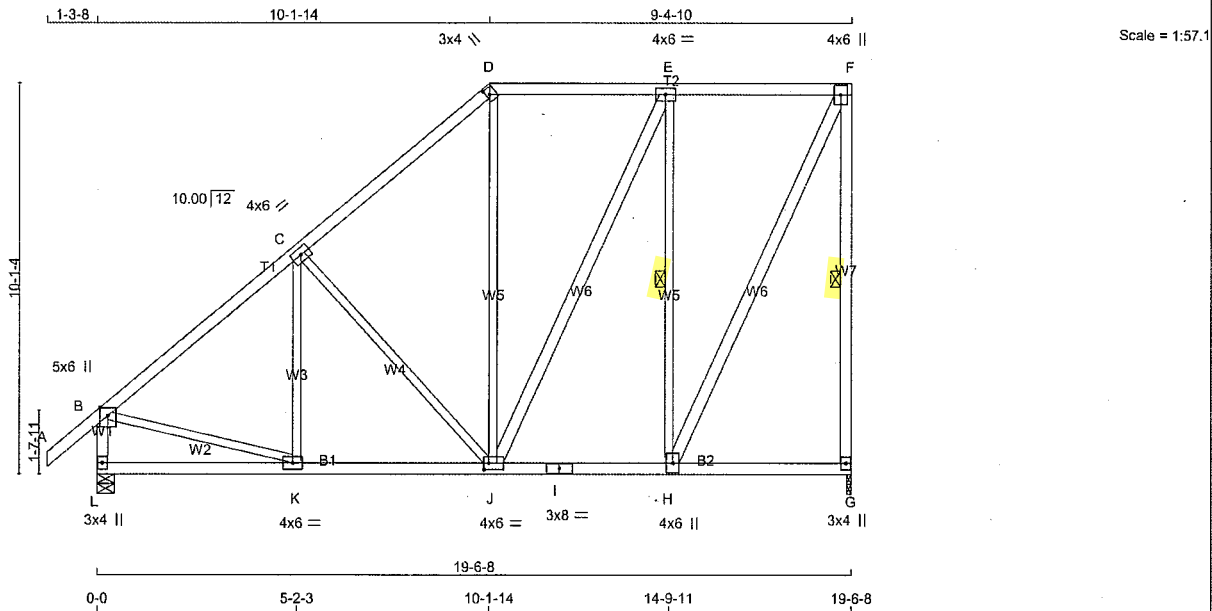
TOTAL WEIGHT = 2 X 116 = 232 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMVW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1279	0	1279	0	0	1-8	1-8
L	1435	0	1435	0	0	5-8	1-9

UNFACTORED REACTIONS

1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
L	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MAX. FACTORED		FACTORED					MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.		FORCE	MAX
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.15 (1)		10.00	K- C	-129 / 55	0.07 (1)
B- C	-1176 / 0	-112.4 -112.4	0.40 (1)		5.38	C- J	-450 / 0	0.52 (1)
C- D	-875 / 0	-112.4 -112.4	0.39 (1)		6.04	J- D	0 / 124	0.03 (4)
D- E	-637 / 0	-112.4 -112.4	0.32 (1)		6.25	J- E	0 / 322	0.05 (1)
E- F	-502 / 0	-112.4 -112.4	0.31 (1)		6.25	H- E	-943 / 0	0.66 (1)
G- F	-1244 / 0	0.0 0.0	0.62 (1)		5.76	H- F	0 / 1155	0.19 (1)
L- B	-1396 / 0	0.0 0.0	0.15 (1)		6.89	B- K	0 / 966	0.22 (1)
L- K	0 / 0	-18.5 -18.5	0.12 (4)		10.00			
K- J	0 / 937	-18.5 -18.5	0.21 (1)		10.00			
J- I	0 / 502	-18.5 -18.5	0.13 (1)		10.00			
I- H	0 / 502	-18.5 -18.5	0.13 (1)		10.00			
H- G	0 / 0	-18.5 -18.5	0.10 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.62/1.00 (F-G:1) , BC=0.21/1.00 (J-K:1) ,
WB=0.66/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
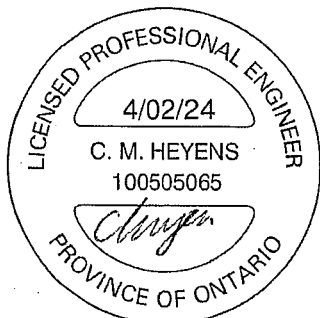COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX MIN		MAX MIN		MAX MIN	
MT20	650 371		1747 788		1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T8	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:05 2024 Page 1
ID:AgdCIOSevh3uN4Xyl113N2zyjH4-p?E7RY3kjepMA4Uo5vSm284fclPyZcYdaN2pl2zUnxK

TOTAL WEIGHT = 2 X 94 = 189 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)

TOP CHORDS : (0.122"X3") SPIRAL NAILS
A- C	1	12	TOP
C- E	1	12	SIDE(61.0)
E- F	1	12	SIDE(61.0)
L- B	2	12	TOP
G- F	2	12	TOP

BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS
| L- J | 2 | 12 | TOP |
| J- G | 2 | 12 | SIDE(183.1) |

WEBS : (0.122"X3") SPIRAL NAILS
| 2x3 | 1 | 6 | |
| D- I | 1 | 6 | SIDE(153.9) |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	2178	0	2178	0	0	5-8	1-8
G	2465	0	2465	0	0	1-8	1-8

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1520	1112 / 0	0 / 0	0 / 0	0 / 0	408 / 0	0 / 0
G	1723	1248 / 0	0 / 0	0 / 0	0 / 0	474 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MAX. FACTORED		FACTORED				MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX. MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH FR-TO			
A- B	0 / 50	-112.4 -112.4	0.09 (1)	10.00	K- C	-324 / 1	0.05 (1)
B- C	-2044 / 0	-112.4 -112.4	0.18 (1)	5.96	C- I	0 / 2020	0.25 (1)
C- D	-3242 / 0	-112.4 -112.4	0.56 (1)	4.52	I- D	-970 / 0	0.16 (1)
D- M	-3242 / 0	-112.4 -112.4	0.52 (1)	4.52	I- E	0 / 1879	0.23 (1)
M- N	-3242 / 0	-112.4 -112.4	0.52 (1)	4.52	H- E	-537 / 0	0.09 (1)
N- E	-3242 / 0	-112.4 -112.4	0.52 (1)	4.52	B- K	0 / 1638	0.20 (1)
E- F	-2370 / 0	-112.4 -112.4	0.16 (1)	5.66	H- F	0 / 1953	0.24 (1)
L- B	-2168 / 0	0.0 0.0	0.08 (1)	7.81			
G- F	-2446 / 0	0.0 0.0	0.09 (1)	7.81			
L- K	0 / 0	-18.5 -18.5	0.04 (4)	10.00			
K- J	0 / 1559	-18.5 -18.5	0.15 (1)	10.00			
J- I	0 / 1559	-18.5 -18.5	0.15 (1)	10.00			
I- O	0 / 1803	-18.5 -18.5	0.16 (1)	10.00			
O- P	0 / 1803	-18.5 -18.5	0.16 (1)	10.00			
P- H	0 / 1803	-18.5 -18.5	0.16 (1)	10.00			
H- Q	0 / 0	-18.5 -18.5	0.04 (4)	10.00			
Q- G	0 / 0	-18.5 -18.5	0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	15-11-8	-245 -245	---	FRONT	VERT	TOTAL	---	C1
H	15-10-12	-14 -14	---	FRONT	VERT	TOTAL	---	C1
I	10-7-8	-898 -898	---	FRONT	VERT	TOTAL	---	C1
M	11-10-12	-72 -72	---	FRONT	VERT	TOTAL	---	C1
N	13-10-12	-72 -72	---	FRONT	VERT	TOTAL	---	C1
O	11-10-12	-14 -14	---	FRONT	VERT	TOTAL	---	C1
P	13-10-12	-14 -14	---	FRONT	VERT	TOTAL	---	C1
Q	17-10-12	-14 -14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL = 32.5	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 45.9	PSF

SPACING = **24.0** IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.56/1.00 (C-D:1) , BC=0.16/1.00 (H-I:1) ,
WB=0.25/1.00 (C-I:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

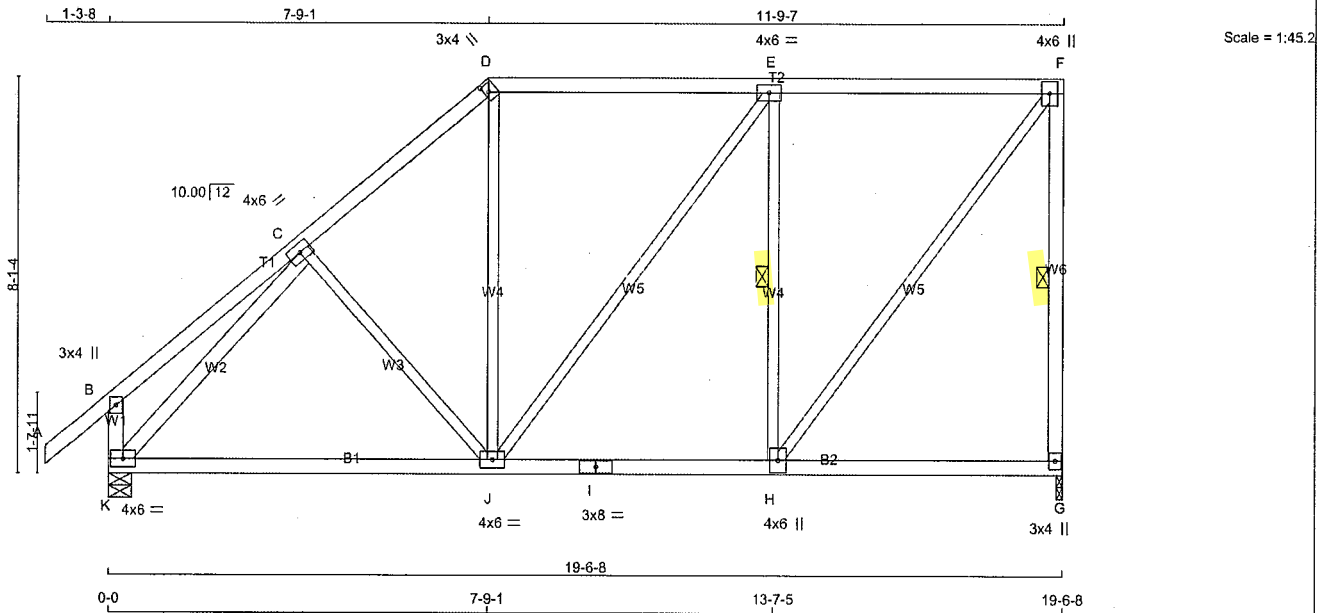JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T8	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:05 2024 Page 2
ID:AgdClOSevh3uN4Xyl113N2zyjH4-p?E7RY3kjepMA4Uo5vSm284fclPyZcYdaN2pl2zUnxK

PLATES   (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.50
F	TMVW+p	MT20	5.0	6.0	2.00	2.25
G	BMV1+p	MT20	4.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	4.0	6.0		

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER

4/02/24

C. M. HEYENS
100505065

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040126

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:06 2024 Page 1
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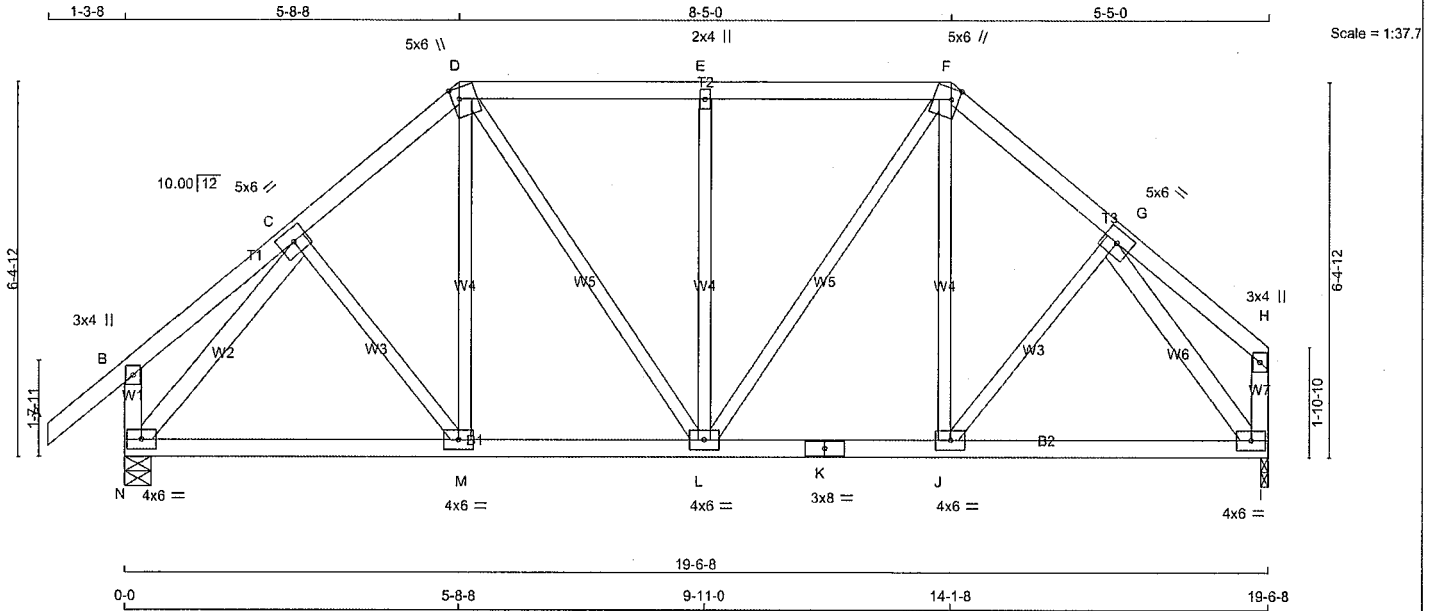
TOTAL WEIGHT = 96 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - C	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1435	0	1435	0	0	5-8	1-9
I	1279	0	1279	0	0	1-8	1-8

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1003	725 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0
I	897	635 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 FROM TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A-B	0 / 50	-112.4 -112.4		0.15 (1)	10.00	C- M	0 / 37	0.01 (4)
B-C	0 / 23	-112.4 -112.4		0.14 (1)	10.00	M-D	0 / 113	0.03 (4)
C-D	-1143 / 0	-112.4 -112.4		0.12 (1)	5.83	D-L	0 / 356	0.08 (1)
D-E	-1062 / 0	-112.4 -112.4		0.26 (1)	5.80	L- E	-573 / 0	0.38 (1)
E-F	-1062 / 0	-112.4 -112.4		0.26 (1)	5.80	L- F	0 / 404	0.09 (1)
F-G	-1107 / 0	-112.4 -112.4		0.11 (1)	5.91	J-F	0 / 79	0.03 (4)
G-H	0 / 23	-112.4 -112.4		0.13 (1)	10.00	J-G	0 / 78	0.02 (4)
N-B	-279 / 0	0.0 0.0		0.03 (1)	7.81	N- C	-1404 / 0	0.34 (1)
I-H	-106 / 0	0.0 0.0		0.01 (1)	7.81	G- I	-1375 / 0	0.31 (1)
N-M	0 / 857	-18.5 -18.5		0.21 (1)	10.00			
M-L	0 / 860	-18.5 -18.5		0.21 (1)	10.00			
L-K	0 / 833	-18.5 -18.5		0.20 (1)	10.00			
K-J	0 / 833	-18.5 -18.5		0.20 (1)	10.00			
J-I	0 / 782	-18.5 -18.5		0.20 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL = 32.5	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD = 45.9		PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.26/1.00 (D-E:1) , BC=0.21/1.00 (L-M:1) , WB=0.38/1.00 (E-L:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040127

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T10	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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1-3-8   5-3-0   5-3-0   1-3-8

4x6 ||

Scale = 1:36.4

10.00 [12]
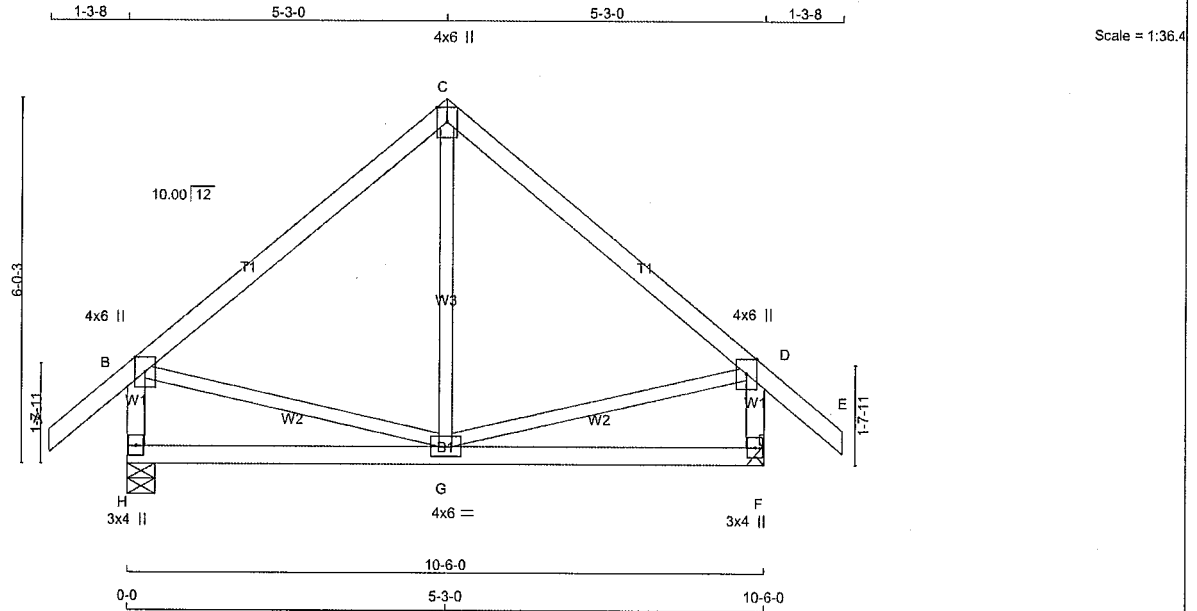
TOTAL WEIGHT = 3 X 47 = 142 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		Edge
C	TTW+p	MT20	4.0	6.0		Edge
D	TMVW+p	MT20	4.0	6.0		Edge
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	843	0	843	0	0	5-8	1-8
F	843	0	843	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	588	431 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
F	588	431 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CSI (LC)	MAX.	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.15 (1)		10.00	G- C	-44 / 84	0.03 (4)
B- C	-427 / 0	-112.4 -112.4	0.40 (1)		6.25	B- G	0 / 338	0.08 (1)
C- D	-427 / 0	-112.4 -112.4	0.40 (1)		6.25	G- D	0 / 338	0.08 (1)
D- E	0 / 50	-112.4 -112.4	0.15 (1)		10.00			
H- B	-806 / 0	0.0 0.0	0.09 (1)		7.81			
F- D	-806 / 0	0.0 0.0	0.09 (1)		7.81			
H- G	0 / 0	-18.5 -18.5	0.15 (4)		10.00			
G- F	0 / 0	-18.5 -18.5	0.15 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")

CSI: TC=0.40/1.00 (C-D:1) , BC=0.15/1.00 (F-G:4) ,
WB=0.08/1.00 (D-G:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (D) (INPUT = 0.90)
JSI METAL = 0.30 (D) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040128

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T10S	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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1-3-8   5-3-0   5-3-0   1-3-8

4x6 ||

Scale = 1:36.9

10.00 |12

6-0-3

5x6 || 5x6 ||

C

W3

T1   T1

B   W2   W2   D

B1   B1

G   5x6 =

1-7-11   1-7-11
1-11-15

W1   W1

5.00 |12

E

H   F
3x4 || 3x4 ||

5-3-0   5-3-0

0-0   5-3-0   10-6-0

TOTAL WEIGHT = 3 X 49 = 147 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER		DESCR.
A - C	2x4	DRY	No.2		SPF
C - E	2x4	DRY	No.2		SPF
H - B	2x6	DRY	No.2		SPF
F - D	2x6	DRY	No.2		SPF
H - G	2x4	DRY	No.2		SPF
G - F	2x4	DRY	No.2		SPF

| ALL WEBS | 2x3 | DRY | No.2 | | SPF |

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BMV1+p	MT20	3.0	4.0	2.00	1.25
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	3.0	4.0	2.00	1.25

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	843	0	843	0	0	5-8	1-8
F	843	0	843	0	0	5-8	1-8

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	588	431 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
F	588	431 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 FROM TO	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 50	-112.4 -112.4		0.15 (1)	10.00	G- C	0 / 229	0.05 (1)
B- C	-640 / 0	-112.4 -112.4		0.40 (1)	6.25	B- G	0 / 496	0.11 (1)
C- D	-640 / 0	-112.4 -112.4		0.40 (1)	6.25	G- D	0 / 496	0.11 (1)
D- E	0 / 50	-112.4 -112.4		0.15 (1)	10.00			
H- B	-795 / 0	0.0 0.0		0.06 (1)	7.81			
F- D	-795 / 0	0.0 0.0		0.06 (1)	7.81			
H- G	0 / 0	-18.5 -18.5		0.15 (4)	10.00			
G- F	0 / 0	-18.5 -18.5		0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.15/1.00 (G-H:4) ,
WB=0.11/1.00 (B-G:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

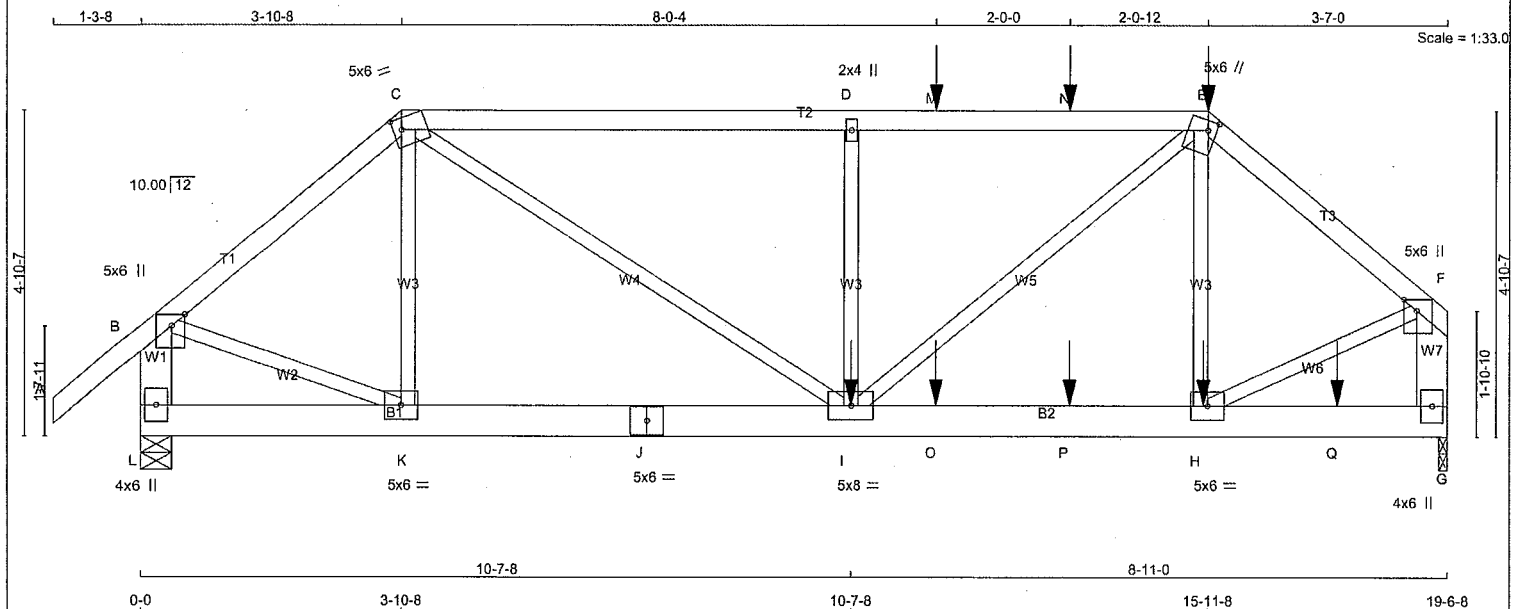JSI GRIP= 0.45 (D) (INPUT = 0.90)
JSI METAL= 0.19 (F) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T11	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington   Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:10 2024 Page 1
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3-10-8

3x4 || B

10.00 [12

T1

W3

4-10-7

5x6 ||

A

1-7-11

W1

W2

B1

D   E   F   C

4x6 || 4x6 ||

Scale = 1:27.5

1-10-12   1-2-0   9-12

0-0   1-10-12   3-0-12   3-10-8

TOTAL WEIGHT = 2 X 22 = 45 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF

ALL WEBS 2x3   DRY   No.2   SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)

TOP CHORDS : (0.122"X3") SPIRAL NAILS
A-B	1	12	TOP
B-C	1	12	TOP
D-A	2	12	TOP

BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS
D-C	2	12	SIDE(0.0)

WEBS : (0.122"X3") SPIRAL NAILS
2x3   1   6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED
TO ONE SIDE THAT THE CORRESPONDING NAILING
PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE
SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	1310	0	1310	0	0	MECHANICAL	
D	848	0	848	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM
BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	913	677 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0
D	591	436 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 0	-112.4 -112.4	0.16 (1)	10.00	A-C	0 / 0	0.00 (1)
C-B	-218 / 0	0.0	0.0 0.04 (1)	7.81			
D-A	-218 / 0	0.0	0.0 0.01 (1)	7.81			
D-E	0 / 0	-18.5 -18.5	0.43 (1)	10.00			
E-F	0 / 0	-18.5 -18.5	0.43 (1)	10.00			
F-C	0 / 0	-18.5 -18.5	0.43 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-10-12	-573	-573 ---	BACK	VERT	TOTAL	---	C1
F	3-0-12	-575	-575 ---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   **24.0**   **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.43/1.00 (C-D:1) ,
WB=0.00/1.00 (A-C:1) , SSI=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.		DRWG NO.
437026	T11	1	2	BAYVIEW WELLINGTON		
Tamarack Roof Truss, Burlington				TRUSS DESC.		

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-Ay10UF7tYAReGsMmuT1xlBncgJ2_EwgMjflaRFzUnxF

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040130

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T11Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-e9bOhb8VJUZVu0xySAZAlOKmQjPrzNwWyJV7zhzUnxE

3-10-8

3x4 ||

Scale = 1:27.5

10.00 √12

TOTAL WEIGHT = 2 X 22 = 45 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)

TOP CHORDS : (0.122"X3") SPIRAL NAILS

A- B	1	12	TOP
B- C	1	12	TOP
D- A	2	12	TOP

BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS

| D- C | 2 | 12 | SIDE(0.0) |

WEBS : (0.122"X3") SPIRAL NAILS

| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED
TO ONE SIDE THAT THE CORRESPONDING NAILING
PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE
SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	6.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	1125	0	1125	0	0	MECHANICAL	
D	1164	0	1164	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM
BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	784	579 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
D	811	599 / 0	0 / 0	0 / 0	0 / 0	212 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A-B	0 / 0	-112.4 -112.4	0.16 (1)	10.00	A- C	0 / 0	0.00 (1)
C-B	-218 / 0	0.0	0.0	0.04 (1)	7.81			
D-A	-218 / 0	0.0	0.0	0.01 (1)	7.81			
D-E	0 / 0	-18.5 -18.5	0.32 (1)	10.00				
E-F	0 / 0	-18.5 -18.5	0.32 (1)	10.00				
F-C	0 / 0	-18.5 -18.5	0.32 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	10-12	-620	-620	---	FRONT	VERT	TOTAL	---	C1
F	2-10-12	-620	-620	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.32/1.00 (C-D:1) ,
WB=0.00/1.00 (A-C:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS: 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040131

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T11Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-e9bOhb8VJUZVu0xySAZAIOKmQjPrzNwWyJV7zhzUnxE

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040131

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T12	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-6L9mvx874nhMV9W80u4Pqctpr7ocilkfBzEhV8zUnxD

TOTAL WEIGHT = 42 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER		DESCR.
A - D	2x6	DRY	No.2		SPF
C - D	2x4	DRY	No.2		SPF
E - C	2x4	DRY	No.2		SPF
G - E	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
EXCEPT					
G - A	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES   (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0		Edge
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMVWW-t	MT20	8.0	16.0	4.75	5.25
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	10.0		
G	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	1103	0	1103	0	0	5-8	1-8
G	1294	0	1294	0	0	1-8	1-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS
CHORD AT JT(S): D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	772	552 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0
G	907	642 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MAX. FACTORED	FACTORED					MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A-H	-1182 / 0	-112.4 -112.4	0.37 (1)	6.25		A- F	0 / 1409	0.35 (1)
H- B	-1182 / 0	-112.4 -112.4	0.37 (1)	6.25		F- B	-1239 / 0	0.23 (1)
B- I	-1182 / 0	-112.4 -112.4	0.69 (1)	5.55		F- C	0 / 1395	0.35 (1)
I- J	-1182 / 0	-112.4 -112.4	0.69 (1)	5.55		G- A	-1160 / 0	0.15 (1)
J- C	-1182 / 0	-112.4 -112.4	0.69 (1)	5.55				
C- D	0 / 0	-112.4 -112.4	0.40 (1)	10.00				
E- C	0 / 128	0.0 0.0	0.02 (4)	10.00				
G- K	0 / 0	-18.5 -18.5	0.16 (4)	10.00				
K- F	0 / 0	-18.5 -18.5	0.16 (4)	10.00				
F- L	0 / 0	-18.5 -18.5	0.16 (4)	10.00				
L- M	0 / 0	-18.5 -18.5	0.16 (4)	10.00				
M- E	0 / 0	-18.5 -18.5	0.16 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	1-12	-139 -139	---	BACK	VERT	TOTAL	---	C1
B	4-3-12	-107 -107	---	BACK	VERT	TOTAL	---	C1
F	4-3-12	-52 -52	---	BACK	VERT	TOTAL	---	C1
G	1-12	-58 -58	---	BACK	VERT	TOTAL	---	C1
H	2-3-12	-107 -107	---	BACK	VERT	TOTAL	---	C1
I	6-3-12	-107 -107	---	BACK	VERT	TOTAL	---	C1
J	8-3-12	-107 -107	---	BACK	VERT	TOTAL	---	C1
K	2-3-12	-52 -52	---	BACK	VERT	TOTAL	---	C1
L	6-3-12	-52 -52	---	BACK	VERT	TOTAL	---	C1
M	8-3-12	-57 -57	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.16/1.00 (F-G:4) ,
WB=0.35/1.00 (A-F:1) , SSI=0.45/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040132

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T14	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-2kHWKdAOcPx4lTgX7l6tw1y7swOKAYQyeHkoa0zUnxB

18-7-0 11-4-0 16-11-0 1-3-8

Scale = 1:80.1

TOTAL WEIGHT = 297 lb [M]

2-H2.5A   2-H2.5A

10x20 MT18HS=   10x12 =

0-0 6-2-4 12-5-8 18-7-0 24-3-0 29-11-0 36-0-8 42-3-12 46-10-0

46-10-0

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
A - S	2x6	DRY	2100F 1.8E	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
O - K	2x6	DRY	2100F 1.8E	SPF

REINFORCING MEMBERS
HW2   2x8   DRY   No.2   SPF

BEARING BLOCKS
BL1   2x6   DRY   No.2   SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBMK1*m	MT18HS	10.0	20.0	5.25	3.00
B, D, H						
B	TMWW+w	MT20	5.0	6.0		
C	TS-l	MT20	5.0	6.0		
E	TTWW-m	MT20	6.0	10.0	2.75	4.50
F	TMW+w	MT20	4.0	6.0		
G	TTWW+m	MT20	8.0	9.0	Edge	3.25
I	TS-t	MT20	5.0	6.0		
J	TMWWW-t	MT20	10.0	16.0	5.00	7.50
K	TMBMW1-l	MT20	10.0	12.0	3.75	Edge
M	BMW+w	MT20	4.0	6.0		
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	6.0	7.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW-t	MT20	5.0	6.0		
S	BS-l	MII16	6.0	17.5		
T	BMWW-t	MT20	5.0	6.0		
U	BMW+w	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	HEEL WEDGE
A	4360	0	4360	-201	-728	5-8	4-12	2x3 L
K	4566	0	4566	0	-759	5-8	4-15	

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 728 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT K FOR 759 LBS FACTORED UPLIFT

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	3181	2024 / 0	489 / 0	0 / 0	123 / -848	668 / 0	0 /0
K	3322	2145 / 0	489 / 0	0 / 0	107 / -880	688 / 0	0 /0

HORIZONTAL REACTIONS

A	---	0 / 0	0 / 0	0 / 0	143 / -144	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K

BRACING
FOR SECTION E-G, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-T, D-R, E-R, E-Q, F-Q, G-Q, G-P, H-P.

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- W	-8628 / 1313	-145.3 -145.3		0.44 (1)	2.63	U- B	0 / 347	0.06 (5)
W- B	-8664 / 1425	-145.3 -145.3		0.74 (1)	2.45	B- T	-1512 / 383	0.34 (2)
B- C	-7354 / 1225	-145.3 -145.3		0.78 (1)	2.66	T- D	-103 / 900	0.14 (2)
C- D	-7354 / 1225	-145.3 -145.3		0.78 (1)	2.66	D- R	-2014 / 516	0.72 (2)
D- E	-6004 / 1034	-145.3 -145.3		0.65 (1)	3.07	R- E	-309 / 1641	0.26 (2)
E- F	-5656 / 964	-155.3 -155.3		0.55 (1)	2.00	E- Q	-409 / 1225	0.23 (9)
F- G	-5656 / 964	-155.3 -155.3		0.55 (1)	2.00	Q- F	-1057 / 202	0.42 (1)
G- H	-5827 / 991	-145.3 -145.3		0.64 (1)	3.11	Q- G	-247 / 1449	0.23 (2)
H- I	-6774 / 1116	-145.3 -145.3		0.73 (3)	2.81	P- G	-253 / 1325	0.21 (3)
I- J	-6774 / 1116	-145.3 -145.3		0.73 (3)	2.81	P- H	-1560 / 436	0.56 (3)
J- Y	-3299 / 519	-145.3 -145.3		0.24 (3)	4.52	N- H	-14 / 431	0.07 (6)
Y- K	-5072 / 800	-145.3 -145.3		0.21 (1)	3.66	N- J	-343 / 249	0.21 (3)
K- L	0 / 1	-145.3 -145.3		0.11 (3)	10.00	M- J	0 / 274	0.05 (17)
A- V	-1391 / 7779	-39.5 -39.5		0.53 (1)	6.25	J- X	-4022 / 571	0.63 (1)
V- U	-1391 / 7779	-39.5 -39.5		0.53 (1)	6.25	X- Y	-274 / 2329	0.00 (1)
U- T	-1391 / 7779	-39.5 -39.5		0.44 (1)	6.25			
T- S	-1045 / 6594	-39.5 -39.5		0.35 (1)	6.25			
S- R	-1045 / 6594	-39.5 -39.5		0.35 (1)	6.25			
R- Q	-679 / 5353	-39.5 -39.5		0.29 (1)	6.25			
Q- P	-513 / 5189	-39.5 -39.5		0.29 (1)	6.25			
P- O	-748 / 6081	-39.5 -39.5		0.33 (1)	6.25			
O- N	-748 / 6081	-39.5 -39.5		0.33 (1)	6.25			
N- M	-901 / 6145	-39.5 -39.5		0.35 (1)	6.25			
M- X	-900 / 6151	-39.5 -39.5		0.35 (1)	6.25			
X- K	-447 / 2961	-39.5 -39.5		0.19 (1)	6.25			

(V-W row: V- W | -598 / 228 | | | 0.00 (1) |)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	43.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	67.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/180 (3.11")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.78/1.00 (B-D:1) , BC=0.53/1.00 (U-V:1) ,
WB=0.72/0.90 (D-R:2) , SSI=0.37/1.00 (A-V:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873
MT18HS	586 403	2455 1382	3163 3004
MII16	438 302	2547 1256	4283 1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg..

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T14	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington   Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:14 2024 Page 2
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NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.85 (O) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040133

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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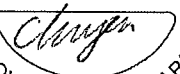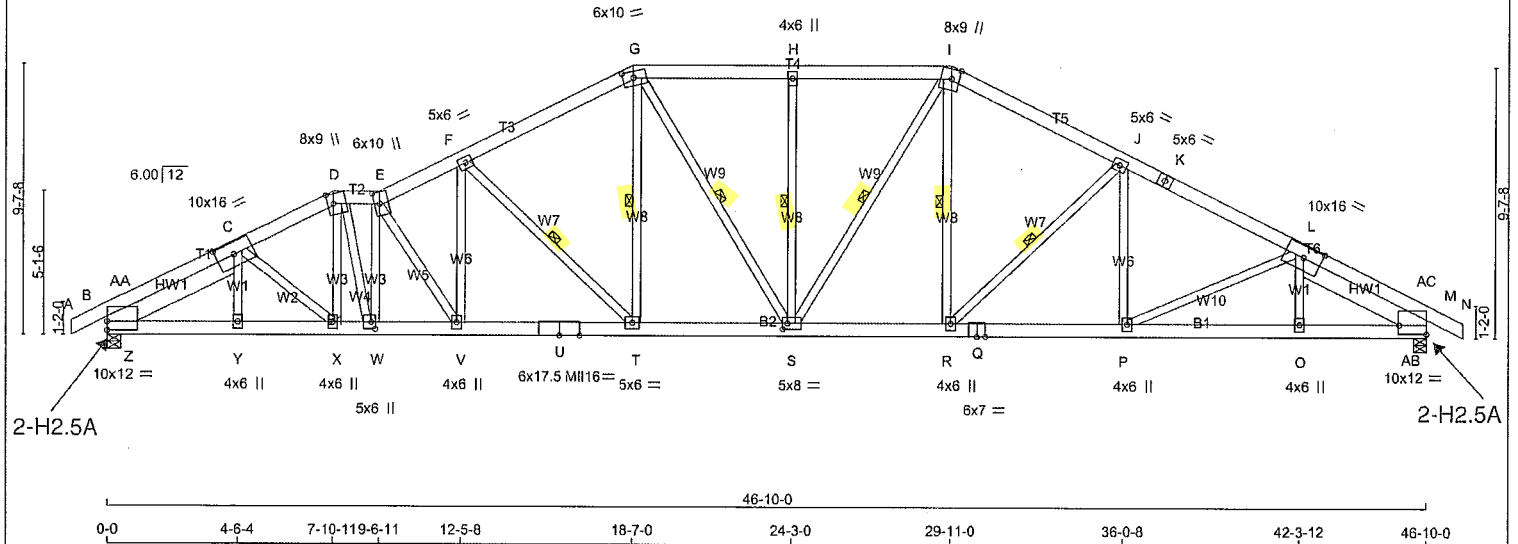ID:AgdClOSevh3uN4Xyl113N2zyjH4-XwqvXyB0Ni4xMdFjh0d6SEVHoKkRvya5txTL6TzUnxA

Dimensions along top: 1-3-8 | 4-6-11 | 1-8-0 | 12-4-5 | 11-4-0 | 16-11-0 | 1-3-8

Scale = 1:78.5

6.00 [12

9-7-8
3-5-6
1-2-0

46-10-0

Bottom dimensions: 0-0 | 2-5-2 | 4-6-11 | 6-2-11 | 12-5-8 | 18-7-0 | 24-3-0 | 29-11-0 | 36-0-8 | 42-3-12 | 46-10-0

2-H2.5A

TOTAL WEIGHT = 317 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
C	TTWWW-m	MT20	10.0	12.0	3.50	5.50
D	TTWW-m	MT20	6.0	10.0	3.25	4.75
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	10.0	2.75	4.50
G	TMW+w	MT20	4.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	3.25
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWWW-t	MT20	10.0	16.0	5.00	7.50
L	TMBMW1-I	MT20	10.0	12.0	3.75	Edge
N	BMW+w	MT20	4.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	6.0	7.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	2.00
S	BMWW-t	MT20	5.0	6.0		
T	BS-t	MII16	6.0	17.5		
U	BMWW-t	MT20	5.0	6.0		
V	BMWW+t	MT20	4.0	10.0	4.25	2.00
W	BMW+w	MT20	4.0	6.0		

(seal)
LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
(signature)
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040134

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	4585	0	4585	201	-782	5-8	5-0
L	4589	0	4589	0	-761	5-8	5-0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 782 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 761 LBS FACTORED UPLIFT

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3335	2156 / 0	492 / 0	0 / 0	116 / -897	688 / 0	0 / 0
L	3339	2156 / 0	492 / 0	0 / 0	107 / -884	691 / 0	0 / 0

HORIZONTAL REACTIONS

B	---	0 / 0	0 / 0	0 / 0	144 / -144	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q.

LOADING
TOTAL LOAD CASES: (18)

CHORDS							WEBS		
	MAX. FACTORED	FACTORED						MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC		MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		LENGTH		FR-TO		
A - B	0 / 1	-145.3	-145.3	0.11 (2)	10.00		W - C	-60 / 418	0.07 (1)
B - Y	-5035 / 805	-145.3	-145.3	0.24 (2)	3.73		C - V	-516 / 3545	0.57 (1)
Y - C	-3365 / 671	-145.3	-145.3	0.20 (2)	4.50		V - D	-3279 / 564	0.43 (1)
C - D	-7745 / 1316	-145.3	-145.3	0.28 (2)	3.00		D - U	-1443 / 334	0.92 (2)
D - E	-7486 / 1243	-145.3	-145.3	0.84 (2)	2.53		U - E	-90 / 915	0.15 (2)
E - F	-6087 / 1049	-145.3	-145.3	0.70 (2)	2.97		E - S	-2122 / 540	0.76 (2)
F - G	-5714 / 974	-155.3	-155.3	0.56 (1)	2.00		S - F	-324 / 1709	0.27 (2)
G - H	-5714 / 974	-155.3	-155.3	0.56 (1)	2.00		F - R	-430 / 1211	0.25 (9)
H - I	-5876 / 996	-145.3	-145.3	0.64 (1)	3.10		R - G	-1056 / 202	0.42 (1)
I - J	-6820 / 1121	-145.3	-145.3	0.73 (1)	2.80		R - H	-252 / 1475	0.24 (2)
J - K	-6820 / 1121	-145.3	-145.3	0.73 (1)	2.80		Q - H	-253 / 1324	0.21 (3)
K - AA	-3317 / 621	-145.3	-145.3	0.24 (3)	4.50		Q - I	-1559 / 436	0.56 (3)
AA - L	-5099 / 804	-145.3	-145.3	0.23 (3)	3.76		O - I	-14 / 429	0.07 (6)
L - M	0 / 1	-145.3	-145.3	0.11 (3)	10.00		O - K	-340 / 255	0.21 (3)
							N - K	0 / 274	0.05 (17)
B - X	-678 / 2986	-39.5	-39.5	0.21 (1)	6.25		C - X	-3953 / 546	0.60 (1)
X - W	-1109 / 6111	-39.5	-39.5	0.36 (1)	6.25		X - Y	-202 / 2172	0.00 (1)
W - V	-1112 / 6131	-39.5	-39.5	0.36 (1)	6.25		K - Z	-4048 / 573	0.63 (1)
V - U	-1375 / 7906	-39.5	-39.5	0.47 (1)	6.25		Z - AA	-275 / 2344	0.00 (1)
U - T	-1074 / 6741	-39.5	-39.5	0.36 (1)	6.25				
T - S	-1074 / 6741	-39.5	-39.5	0.36 (1)	6.25				
S - R	-691 / 5424	-39.5	-39.5	0.30 (1)	6.25				
R - Q	-520 / 5232	-39.5	-39.5	0.29 (1)	6.25				
Q - P	-752 / 6122	-39.5	-39.5	0.33 (1)	6.25				
P - O	-752 / 6122	-39.5	-39.5	0.33 (1)	6.25				
O - N	-905 / 6181	-39.5	-39.5	0.35 (1)	6.25				
N - Z	-904 / 6187	-39.5	-39.5	0.35 (1)	6.25				
Z - L	-449 / 2977	-39.5	-39.5	0.19 (1)	6.25				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL	=	43.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=		67.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/180 (3.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.84/1.00 (D-E:2) , BC=0.47/1.00 (U-V:1) ,
WB=0.92/1.00 (D-U:2) , SSI=0.35/1.00 (D-E:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873
MII16	438	302	2547	1256	4283	1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT
{40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0
PSF AND 5.0 PSF RESPECTIVELY.

JSI GRIP= 0.87 (V) (INPUT = 0.90)
JSI METAL= 0.94 (D) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040134

REVIEWED

JOB NAME		TRUSS NAME		QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026		T16		1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-TJyfyeCGuKKfcxO6oRfaXfafz8RgNv OKFySALzUnx8

Scale = 1:78.5

TOTAL WEIGHT = 328 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER		DESCR.
A - D	2x6	DRY	No.2		SPF
D - E	2x6	DRY	No.2		SPF
E - G	2x6	DRY	No.2		SPF
G - I	2x6	DRY	No.2		SPF
I - K	2x6	DRY	No.2		SPF
K - N	2x6	DRY	No.2		SPF
B - U	2x6	DRY	2100F 1.8E		SPF
U - Q	2x6	DRY	2100F 1.8E		SPF
Q - M	2x6	DRY	2100F 1.8E		SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-l	MT20	10.0	12.0	3.75	
C	TMWWW-t	MT20	10.0	16.0	5.00	7.50
D	TTWW+m	MT20	8.0	9.0	4.25	2.50
E	TWWM+m	MT20	6.0	10.0	5.00	2.25
F	TMWW-t	MT20	5.0	6.0		
G	TTWW-m	MT20	6.0	10.0	2.75	4.50
H	TMW+w	MT20	4.0	6.0		
I	TTWW+m	MT20	8.0	9.0	Edge	3.25
J	TMWW-t	MT20	5.0	6.0		
K	TS-t	MT20	5.0	6.0		
L	TMWW-t	MT20	10.0	16.0	5.00	7.50
M	TMBMW1-l	MT20	10.0	12.0	3.75	Edge
O	BMW+w	MT20	4.0	6.0		
P, R, V, X						
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	6.0	7.0		
S	BMWWW-t	MT20	5.0	8.0	2.50	2.00
T	BMW+t	MT20	5.0	6.0		
U	BS-t	MII16	6.0	17.5		
W	BMWW+t	MT20	5.0	6.0	3.00	1.75
Y	BMW+w	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	4585	0	4585	201	-782	5-8	5-0
M	4589	0	4589	0	-761	5-8	5-0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 782 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 761 LBS FACTORED UPLIFT

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3335	2156 / 0	492 / 0	0 / 0	116 / -897	688 / 0	0 / 0
M	3339	2156 / 0	492 / 0	0 / 0	107 / -884	691 / 0	0 / 0

HORIZONTAL REACTIONS

B	---	0 / 0	0 / 0	0 / 0	144 / -144	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M

BRACING
FOR SECTION G-I, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-T, G-T, G-S, H-S, I-S, I-R, J-R.

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC1 CSI (LC)	MAX. MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A - B	0 / 1	-145.3 -145.3	0.11 (2)	10.00	Y- C	-13 / 161	0.03 (17)
B-AA	-5152 / 828	-145.3 -145.3	0.22 (1)	3.70	C- X	-5 / 310	0.05 (3)
AA- C	-3405 / 659	-145.3 -145.3	0.17 (2)	4.50	X- D	-82 / 410	0.07 (2)
C-D	-7011 / 1210	-145.3 -145.3	0.33 (1)	3.11	D- W	-378 / 2597	0.42 (1)
D-E	-7072 / 1240	-145.3 -145.3	0.24 (1)	3.18	W- E	-2750 / 410	0.66 (1)
E- F	-7401 / 1269	-145.3 -145.3	0.55 (2)	2.76	E- V	-999 / 244	0.33 (2)
F- G	-6079 / 1044	-145.3 -145.3	0.58 (1)	3.12	V- F	-157 / 1071	0.17 (2)
G- H	-5713 / 974	-155.3 -155.3	0.56 (1)	2.00	F- T	-2043 / 520	0.73 (2)
H- I	-5713 / 974	-155.3 -155.3	0.56 (1)	2.00	T- G	-312 / 1685	0.27 (2)
I- J	-5876 / 996	-145.3 -145.3	0.64 (1)	3.10	G- S	-436 / 1206	0.25 (2)
J- K	-6820 / 1121	-145.3 -145.3	0.73 (1)	2.80	S- H	-1057 / 202	0.42 (1)
K- L	-6820 / 1121	-145.3 -145.3	0.73 (1)	2.80	S- I	-252 / 1473	0.24 (2)
L-AC	-3317 / 621	-145.3 -145.3	0.24 (3)	4.50	R- I	-253 / 1325	0.21 (3)
AC- M	-5099 / 804	-145.3 -145.3	0.21 (1)	3.76	R- J	-1558 / 436	0.56 (3)
M- N	0 / 1	-145.3 -145.3	0.11 (3)	10.00	P- J	-14 / 429	0.07 (6)
					P- L	-340 / 254	0.21 (3)
B- Z	-677 / 3035	-39.5 -39.5	0.18 (1)	6.25	O- L	0 / 274	0.05 (17)
Z- Y	-1118 / 6091	-39.5 -39.5	0.36 (1)	6.25	Z-AA	-253 / 2287	0.00 (1)
Y- X	-1119 / 6089	-39.5 -39.5	0.36 (1)	6.25	Z- C	-3854 / 557	0.60 (1)
X- W	-1059 / 6264	-39.5 -39.5	0.38 (1)	6.25	L-AB	-4048 / 573	0.63 (1)
W- V	-1189 / 7159	-39.5 -39.5	0.42 (1)	6.25	AB-AC	-275 / 2344	0.00 (1)
V- U	-1060 / 6693	-39.5 -39.5	0.37 (1)	6.25			
U- T	-1060 / 6693	-39.5 -39.5	0.37 (1)	6.25			
T- S	-691 / 5428	-39.5 -39.5	0.31 (1)	6.25			
S- R	-520 / 5232	-39.5 -39.5	0.29 (1)	6.25			
R- Q	-752 / 6122	-39.5 -39.5	0.33 (1)	6.25			
Q- P	-752 / 6122	-39.5 -39.5	0.33 (1)	6.25			
P- O	-905 / 6181	-39.5 -39.5	0.35 (1)	6.25			
O-AB	-904 / 6187	-39.5 -39.5	0.35 (1)	6.25			
AB- M	-449 / 2977	-39.5 -39.5	0.19 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL	=	43.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	=	7.4	PSF
TOTAL	LOAD	=		67.3	PSF

SPACING =   24.0   IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/180 (3.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.73/1.00 (J-L:1) , BC=0.42/1.00 (V-W:1) ,
WB=0.73/1.00 (F-T:2) , SSI=0.35/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND:1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
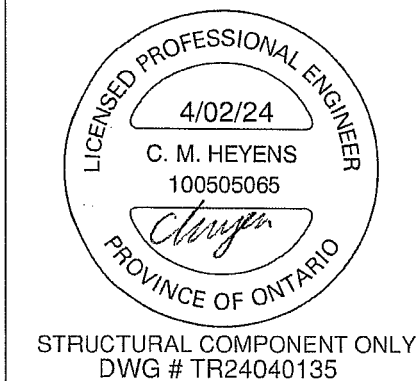COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873
MII16	438	302	2547	1256	4283	1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

STRUCTURAL COMPONENT ONLY
DWG # TR24040135

2-H2.5A   2-H2.5A

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T16	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:17 2024 Page 2
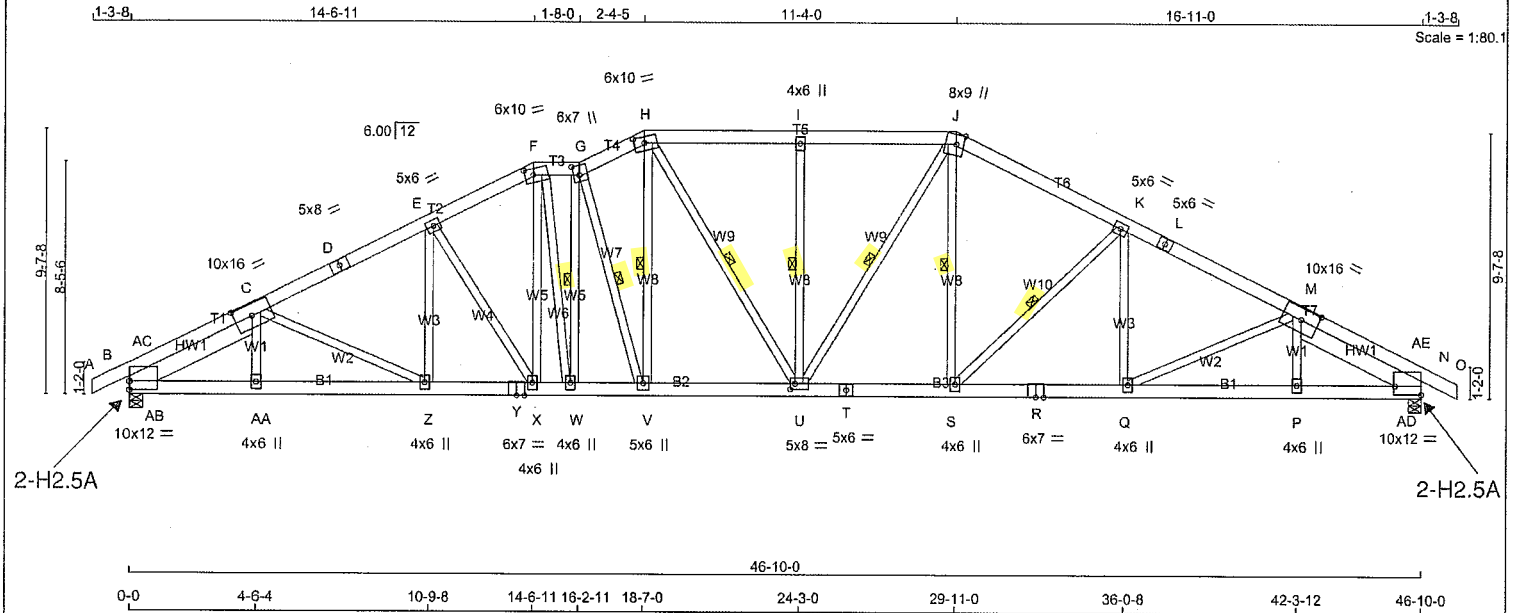ID:AgdCIOSevh3uN4Xyl113N2zyjH4-TJyfyeCGuKKfcxO6oRfaXfafz8RgNv_OKFySALzUnx8

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT
{40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0
PSF AND 5.0 PSF RESPECTIVELY.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.86 (Q) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040135

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T17	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-xVW19 DufdSWD4zlM8Bp4t6qSYoZ6NZXZvi?jnzUnx7

Dimensions (top): 1-3-8 | 11-2-11 | 1-8-0 | 5-8-5 | 11-4-0 | 16-11-0 | 1-3-8

Scale = 1:78.5

TOTAL WEIGHT = 324 lb   [M]

2-H2.5A (left)
2-H2.5A (right)

Bottom dimensions: 46-10-0

0-0 | 4-6-4 | 11-2-11 12-10-11 | 18-7-0 | 24-3-0 | 29-11-0 | 36-0-8 | 42-3-12 | 46-10-0

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF

REINFORCING MEMBERS
| HW1 | 2x8 | DRY | No.2 | SPF |
| HW2 | 2x8 | DRY | No.2 | SPF |

ALL WEBS 2x4   DRY   No.2   SPF
DRY: SEASONED LUMBER.

PLATES   (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-l	MT20	10.0	12.0	4.00	
C	TMWWW-t	MT20	10.0	16.0	5.00	7.50
D	TTWW+m	MT20	8.0	9.0	4.25	3.00
E	TTWW-m	MT20	6.0	7.0	3.50	3.00
F	TTWW-m	MT20	6.0	10.0	2.75	4.50
G	TMW+w	MT20	4.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	3.25
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWWW-t	MT20	10.0	16.0	5.00	7.50
L	TMBMW1-l	MT20	10.0	12.0	3.75	Edge
N	BMW+w	MT20	4.0	6.0		
O, Q, V						
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	6.0	7.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	2.00
S	BMWW-t	MT20	5.0	6.0		
T	BS-t	MII16	6.0	17.5		
U	BMWW+t	MT20	5.0	6.0		
W	BMW+w	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	4585	0	4585	201	-782	5-8	5-0
L	4589	0	4589	0	-761	5-8	5-0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 782 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 761 LBS FACTORED UPLIFT

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3335	2156 / 0	492 / 0	0 / 0	116 / -897	688 / 0	0 / 0
L	3339	2156 / 0	492 / 0	0 / 0	107 / -884	691 / 0	0 / 0

HORIZONTAL REACTIONS
| B | --- | 0 / 0 | 0 / 0 | 0 / 0 | 144 / -144 | 0 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U, E-S, F-S, F-R, G-R, H-R, H-Q, I-Q.

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A - B	0 / 1	-145.3 -145.3	0.11 (2)	10.00	W- C	0 / 289	0.05 (17)
B - Y	-5099 / 832	-145.3 -145.3	0.14 (1)	3.72	C- V	-587 / 255	0.44 (2)
Y - C	-3217 / 596	-145.3 -145.3	0.44 (2)	4.34	V- D	-98 / 619	0.10 (2)
C - D	-6727 / 1132	-145.3 -145.3	0.75 (2)	2.81	D- U	-277 / 1969	0.32 (1)
D - E	-6458 / 1152	-145.3 -145.3	0.20 (1)	3.36	U- E	-2044 / 331	0.39 (1)
E - F	-6040 / 1036	-145.3 -145.3	0.64 (2)	3.02	E- S	-1857 / 462	0.65 (2)
F - G	-5714 / 974	-155.3 -155.3	0.56 (1)	2.00	S- F	-290 / 1618	0.26 (2)
G - H	-5714 / 974	-155.3 -155.3	0.56 (1)	2.00	F- R	-431 / 1208	0.25 (9)
H - I	-5876 / 999	-145.3 -145.3	0.64 (1)	3.10	R- G	-1056 / 202	0.42 (1)
I - J	-6820 / 1121	-145.3 -145.3	0.73 (1)	2.80	R- H	-252 / 1474	0.24 (2)
J - K	-6820 / 1121	-145.3 -145.3	0.73 (1)	2.80	Q- H	-253 / 1325	0.21 (3)
K - AA	-3317 / 621	-145.3 -145.3	0.24 (3)	4.50	Q- I	-1559 / 436	0.56 (3)
AA - L	-5099 / 804	-145.3 -145.3	0.21 (1)	3.65	O- I	-14 / 429	0.07 (6)
L - M	0 / 1	-145.3 -145.3	0.11 (2)	10.00	O- K	-340 / 254	0.21 (3)
					N- K	0 / 274	0.05 (17)
B - X	-643 / 2922	-39.5 -39.5	0.18 (1)	6.25	X- Y	-357 / 2495	0.00 (1)
X - W	-1165 / 6252	-39.5 -39.5	0.35 (1)	6.25	X- C	-4198 / 659	0.65 (1)
W - V	-1167 / 6247	-39.5 -39.5	0.38 (1)	6.25	K- Z	-4048 / 573	0.63 (1)
V - U	-936 / 5997	-39.5 -39.5	0.37 (1)	6.25	Z- AA	-275 / 2344	0.00 (1)
U - T	-1001 / 6505	-39.5 -39.5	0.38 (1)	6.25			
T - S	-1001 / 6505	-39.5 -39.5	0.38 (1)	6.25			
S - R	-691 / 5426	-39.5 -39.5	0.30 (1)	6.25			
R - Q	-520 / 5232	-39.5 -39.5	0.29 (1)	6.25			
Q - P	-752 / 6122	-39.5 -39.5	0.33 (1)	6.25			
P - O	-752 / 6122	-39.5 -39.5	0.33 (1)	6.25			
O - N	-905 / 6181	-39.5 -39.5	0.35 (1)	6.25			
N - Z	-904 / 6187	-39.5 -39.5	0.35 (1)	6.25			
Z - L	-449 / 2977	-39.5 -39.5	0.19 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	43.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	67.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL) = L/180 (3.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.75/1.00 (C-D:2) , BC=0.38/1.00 (V-W:1) , WB=0.65/0.90 (C-X:1) , SSI=0.35/1.00 (C-D:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
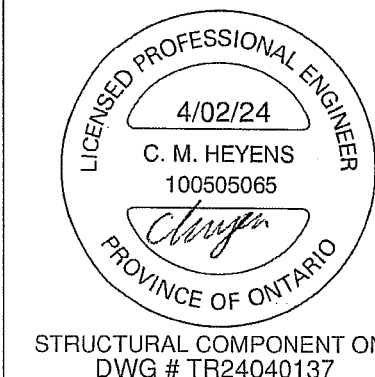COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873
MII16	438	302	2547	1256	4283	1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

STRUCTURAL COMPONENT ONLY
DWG # TR24040136

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
437026	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-xVW19_DufdSWD4zIM8Bp4t6qSYoZ6NZXZvi?jnzUnx7

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.86 (P) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040136

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T18	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AqdClOSevh3uN4Xyl113N2zyjH4-tuenaqE9BFiETO7hUZDH9lCACLUNaE1q0DB6nqzUnx5

Scale = 1:80.1

TOTAL WEIGHT = 343 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER		DESCR.
A - D	2x6	DRY	No.2		SPF
D - F	2x6	DRY	No.2		SPF
F - G	2x6	DRY	No.2		SPF
G - H	2x6	DRY	No.2		SPF
H - J	2x6	DRY	No.2		SPF
J - L	2x6	DRY	No.2		SPF
L - O	2x6	DRY	No.2		SPF
B - Y	2x6	DRY	2100F 1.8E		SPF
Y - T	2x6	DRY	2100F 1.8E		SPF
T - R	2x6	DRY	2100F 1.8E		SPF
R - N	2x6	DRY	2100F 1.8E		SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2		SPF
HW2	2x8	DRY	No.2		SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
C	TMWWW-t	MT20	10.0	16.0	5.00	7.50
D	TS-t	MT20	5.0	8.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	10.0	2.75	3.75
G	TTWW+m	MT20	6.0	7.0	4.25	2.50
H	TTWW+m	MT20	6.0	10.0	2.75	4.50
I	TMW+w	MT20	4.0	6.0		
J	TTWW+m	MT20	8.0	9.0	Edge	3.25
K	TMWW-t	MT20	5.0	6.0		
L	TS-t	MT20	5.0	6.0		
M	TMWWW-t	MT20	10.0	16.0	5.00	7.50
N	TMBMW1-I	MT20	10.0	12.0	3.75	Edge
P	BMW+w	MT20	4.0	6.0		
Q, S, W, X, Z	BMWW+t	MT20	4.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BS-t	MT20	6.0	7.0		
T	BS-t	MT20	5.0	6.0		
U	BMWWW-t	MT20	5.0	8.0	2.50	2.00
V	BMWW+t	MT20	5.0	6.0		
Y	BS-t	MT20	6.0	7.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	4585	0	4585	201	-782	5-8	5-0
N	4589	0	4589	0	-761	5-8	5-0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 782 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 761 LBS FACTORED UPLIFT

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3335	2156 / 0	492 / 0	0 / 0	116 / -897	688 / 0	0 / 0
N	3339	2156 / 0	492 / 0	0 / 0	107 / -884	691 / 0	0 / 0

HORIZONTAL REACTIONS

| B | --- | 0 / 0 | 0 / 0 | 0 / 0 | 144 / -144 | 0 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N

BRACING

FOR SECTION H-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-W, G-V, H-V, H-U, I-U, J-U, J-S, K-S.

LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC1 CSI (LC)	MAX. MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 1	-145.3 -145.3	0.11 (2)	10.00	AA- C	0 / 288	0.05 (17)
B-AC	-5086 / 828	-145.3 -145.3	0.19 (1)	3.78	C- Z	-386 / 198	0.24 (2)
AC- C	-3283 / 626	-145.3 -145.3	0.29 (2)	4.47	Z- E	-30 / 394	0.06 (5)
C- D	-6787 / 1157	-145.3 -145.3	0.57 (1)	3.00	E- X	-1245 / 350	0.84 (2)
D- E	-6787 / 1157	-145.3 -145.3	0.57 (1)	3.00	X- F	-275 / 1192	0.25 (13)
E- F	-6244 / 1126	-145.3 -145.3	0.45 (1)	3.18	F- W	-220 / 1421	0.23 (1)
F- G	-5824 / 1073	-145.3 -145.3	0.17 (1)	3.55	W- G	-1514 / 255	0.45 (1)
G- H	-6026 / 1088	-145.3 -145.3	0.22 (1)	3.45	G- V	-1781 / 456	0.58 (2)
H- I	-5716 / 974	-155.3 -155.3	0.56 (1)	2.00	H- V	-394 / 1883	0.30 (2)
I- J	-5716 / 974	-155.3 -155.3	0.56 (1)	2.00	H- U	-420 / 1229	0.24 (9)
J- K	-5875 / 996	-145.3 -145.3	0.64 (1)	3.10	U- I	-1056 / 202	0.42 (1)
K- L	-6821 / 1121	-145.3 -145.3	0.73 (1)	2.80	U- J	-252 / 1480	0.24 (2)
L- M	-6821 / 1121	-145.3 -145.3	0.73 (1)	2.80	S- J	-253 / 1320	0.21 (3)
M-AE	-3317 / 621	-145.3 -145.3	0.24 (3)	4.50	S- K	-1560 / 436	0.56 (3)
AE- N	-5099 / 804	-145.3 -145.3	0.21 (1)	3.76	Q- K	-14 / 430	0.07 (6)
N- O	0 / 1	-145.3 -145.3	0.11 (3)	10.00	Q- M	-340 / 255	0.21 (3)
					P- M	0 / 274	0.05 (17)
B-AB	-659 / 2956	-39.5 -39.5	0.19 (1)	6.25	AC-AC	-305 / 2374	0.00 (1)
AB-AA	-1143 / 6206	-39.5 -39.5	0.35 (1)	6.25	AB- C	-4098 / 611	0.64 (1)
AA- Z	-1145 / 6199	-39.5 -39.5	0.36 (1)	6.25	M-AD	-4047 / 573	0.63 (1)
Z- Y	-978 / 6075	-39.5 -39.5	0.32 (1)	6.25	AD-AE	-275 / 2344	0.00 (1)
Y- X	-978 / 6075	-39.5 -39.5	0.32 (1)	6.25			
X- W	-794 / 5558	-39.5 -39.5	0.31 (1)	6.25			
W- V	-822 / 5852	-39.5 -39.5	0.32 (1)	6.25			
V- U	-692 / 5416	-39.5 -39.5	0.29 (1)	6.25			
U- T	-520 / 5232	-39.5 -39.5	0.29 (1)	6.25			
T- S	-520 / 5232	-39.5 -39.5	0.29 (1)	6.25			
S- R	-752 / 6123	-39.5 -39.5	0.33 (1)	6.25			
R- Q	-752 / 6123	-39.5 -39.5	0.33 (1)	6.25			
Q- P	-905 / 6181	-39.5 -39.5	0.35 (1)	6.25			
P-AD	-904 / 6187	-39.5 -39.5	0.35 (1)	6.25			
AD- N	-449 / 2977	-39.5 -39.5	0.19 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	43.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	67.3	PSF

SPACING =   24.0   IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL) = L/180 (3.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.73/1.00 (K-M:1), BC=0.36/1.00 (Z-AA:1), WB=0.84/1.00 (E-X:2), SSI=0.35/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
	PLATE GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T18	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-tuenagE9BFiETO7hUZDH9ICACLUNaE1q0DB6ngzUnx5

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
AA	BMW+w	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT
{40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0
PSF AND 5.0 PSF RESPECTIVELY.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.93 (R) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040137

REVIEWED

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | BAYVIEW WELLINGTON | DRWG NO. |
| 437026 | T19 | 1 | 2 | TRUSS DESC. | | |

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-qGlY?MGPjsyxiiH3b FIEjHYB96q2B67UWqDsZzUnx3

1-3-8 | 16-11-1 | 12-11-15 | 16-11-1 | 1-3-8

Scale = 1:78.5

38-7-4 | 7-6-0 | 8-12

0-0 | 4-6-4 | 8-2-12 | 12-6-14 | 16-11-1 | 23-5-0 | 29-10-15 | 34-3-2 | 38-7-4 | 42-3-12 | 46-1-4 46-10-0

TOTAL WEIGHT = 2 X 330 = 661 lb [M]

2-H2.5A

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
K - O	2x6	DRY	No.2	SPF
B - W	2x6	DRY	2100F 1.8E	SPF
W - S	2x6	DRY	2100F 1.8E	SPF
S - N	2x6	DRY	2100F 1.8E	SPF

REINFORCING MEMBERS
| HW1 | 2x8 | DRY | No.2 | SPF |
| HW2 | 2x8 | DRY | No.2 | SPF |

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT
D - X	2x4	DRY	2100F 1.8E	SPF
F - V	2x4	DRY	2100F 1.8E	SPF
T - J	2x4	DRY	2100F 1.8E	SPF
R - L	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS				
A- E	2	12		TOP
E- G	2	12		TOP
G- I	2	12		TOP
I- K	2	12		TOP
K- O	2	12		TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS				
B- W	2	12		TOP
W- S	2	12		TOP
S- N	2	12		SIDE(227.4)
WEBS : (0.122"X3") SPIRAL NAILS				
L- Q	1	2		SIDE(1306.8)
2x4	1	6		
D- Y	1	2		
2x8	2	6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	5793	0	5793	201	-766	5-8	3-2
N	11637	0	11637	0	-766	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 766 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 766 LBS FACTORED UPLIFT

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	4184	2743 / 0	492 / 0	0 / 0	111 / -891	949 / 0	0 / 0
N	8280	5639 / 0	492 / 0	0 / 0	111 / -891	2149 / 0	0 / 0

HORIZONTAL REACTIONS
| B | --- | 0 / 0 | 0 / 0 | 0 / 0 | 144 / -144 | 0 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N
BEARING SIZE FACTOR = 1.15 AT JNT(S) N (BASED ON SUPPORT DEPTH = 1-8)

BRACING
FOR SECTION G-I, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-V, G-U, H-U, I-U, I-T.
2x6 DRY SPF No.2 T-BRACE AT J-T
2x4 DRY SPF No.2 T-BRACE AT L-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90%
OF WEB LENGTH.

LOADING
TOTAL LOAD CASES: (18)

	CHORDS						WEBS		
	MAX. FACTORED	FACTORED			MAX.			MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.		FORCE	MAX
	(LBS)	(PLF)	FROM TO	CSI (LC)	UNBRAC			(LBS)	CSI (LC)
FR-TO					LENGTH	FR-TO			
A- B	0 / 1	-145.3	-145.3	0.06 (2)	10.00	Z- C		0 / 210	0.02 (17)
B-AB	-6465 / 798	-145.3	-145.3	0.15 (1)	4.47	C- Y		0 / 583	0.05 (3)
AB- C	-4303 / 646	-145.3	-145.3	0.12 (2)	5.43	Y- D		-265 / 114	0.03 (10)
C- D	-9273 / 1172	-145.3	-145.3	0.20 (1)	3.89	D- X		-641 / 241	0.10 (2)
D- E	-8953 / 1115	-145.3	-145.3	0.22 (1)	3.94	X- F		-137 / 615	0.05 (2)
E- F	-8953 / 1115	-145.3	-145.3	0.22 (1)	3.94	F- V		-1325 / 403	0.42 (2)
F- G	-8351 / 1028	-145.3	-145.3	0.20 (1)	4.07	V- G		-294 / 1357	0.12 (2)
G- H	-8818 / 956	-155.3	-155.3	0.42 (1)	2.00	G- U		-242 / 2914	0.26 (3)
H- I	-8818 / 956	-155.3	-155.3	0.42 (1)	2.00	U- H		-1208 / 234	0.25 (1)
I- J	-10192 / 1028	-145.3	-145.3	0.26 (1)	3.68	U- I		-1686 / 77	0.55 (10)
J- K	-13105 / 1115	-145.3	-145.3	0.37 (1)	3.19	T- I		-294 / 4572	0.40 (3)
K- L	-13105 / 1115	-145.3	-145.3	0.37 (1)	3.19	T- J		-5155 / 403	0.38 (3)
L- M	-17663 / 1172	-145.3	-145.3	0.63 (1)	2.47	R- J		-137 / 4854	0.43 (3)
M-AD	-8836 / 646	-145.3	-145.3	0.25 (1)	3.93	R- L		-6301 / 241	0.68 (3)
AD- N	-13426 / 799	-145.3	-145.3	0.39 (1)	2.97	Q- L		-19 / 6067	0.54 (3)
N- O	0 / 1	-145.3	-145.3	0.06 (3)	10.00	Q- M		0 / 329	0.03 (2)
						P- M		0 / 1590	0.14 (3)
B-AA	-663 / 3825	-39.5	-39.5	0.14 (1)	6.25	AA-AB		-229 / 2910	0.00 (1)
AA- Z	-1094 / 7946	-39.5	-39.5	0.26 (1)	6.25	AA- C		-5198 / 543	0.41 (1)
Z- Y	-1095 / 7941	-39.5	-39.5	0.23 (1)	6.25	M-AC		-9907 / 545	0.79 (1)
Y- X	-1029 / 8309	-39.5	-39.5	0.24 (1)	6.25	AC-AD		-230 / 6495	0.00 (1)
X- W	-867 / 8017	-39.5	-39.5	0.24 (1)	6.25				
W- V	-867 / 8017	-39.5	-39.5	0.24 (1)	6.25				
V- U	-654 / 7458	-39.5	-39.5	0.22 (1)	6.25				
U- T	-495 / 9155	-39.5	-39.5	0.26 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	43.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	67.3	PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS
OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD
LOAD OF 4.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL
OR INDUSTRIAL BUILDING REQUIREMENTS OF
PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) TIMES IMPORTANCE FACTOR
EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE
LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/180 (3.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.63/1.00 (L-M:1) , BC=0.56/1.00 (P-AC:1)
, WB=0.79/0.90 (M-AC:1) , SSI=0.22/1.00 (G-H:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
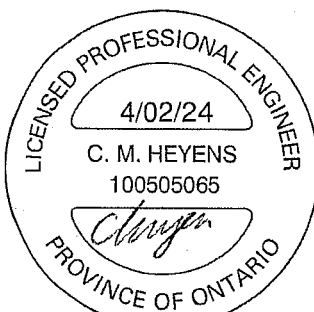| PLATE | GRIP(DRY) | SHEAR | SECTION |
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90)
JSI METAL= 0.91 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T19	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-qGIY?MGPjsyxiiH3b_FIEjHYB96g2B67UWgDsZzUnx3

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.25	
C	TMWWW-t	MT20	10.0	16.0	4.50	7.75
D	TMWW+t	MT20	5.0	6.0	2.50	1.25
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0	2.50	1.75
G	TTWW-m	MT20	6.0	10.0	2.75	3.75
H	TMW+w	MT20	4.0	6.0		
I	TTWW-m	MT20	6.0	10.0	2.75	3.75
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TS-t	MT20	5.0	6.0		
L	TMWW+t	MT20	5.0	6.0	2.50	1.25
M	TMWWW-t	MT20	10.0	16.0	4.50	7.75
N	TMBMW1-I	MT20	10.0	12.0	3.25	Edge
P	BMW+w	MT20	4.0	6.0		
Q	BMWW+t	MT20	6.0	10.0		
R	BMWW+t	MT20	5.0	6.0	2.50	2.25
S	BS-t	MT20	6.0	7.0		
T	BMWW+t	MT20	5.0	6.0		
U	BMWWW-t	MT20	5.0	8.0		
V	BMWW+t	MT20	5.0	6.0		
W	BS-t	MT20	6.0	7.0		
X	BMWW+t	MT20	5.0	6.0	2.50	2.25
Y	BMWW+t	MT20	6.0	10.0		
Z	BMW+w	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (18)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 FROM	MAX TO	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						UNBRAC LENGTH FR-TO		
T- S	-666 / 11737	-39.5	-39.5	0.33 (1)		6.25		
S- R	-666 / 11737	-39.5	-39.5	0.33 (1)		6.25		
R- Q	-828 / 15830	-39.5	-39.5	0.47 (1)		6.25		
Q-AE	-895 / 15659	-39.5	-39.5	0.54 (1)		6.25		
AE- P	-895 / 15659	-39.5	-39.5	0.54 (1)		6.25		
P-AF	-895 / 15698	-39.5	-39.5	0.56 (1)		6.25		
AF-AC	-895 / 15698	-39.5	-39.5	0.56 (1)		6.25		
AC- N	-463 / 7843	-39.5	-39.5	0.27 (1)		6.25		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	42-5-12	-431	-431	---	BACK	VERT	TOTAL	---	C1
Q	38-7-4	-4052	-4052	---	BACK	VERT	TOTAL	---	C1
AC	46-1-4	-433	-433	---	BACK	VERT	TOTAL	---	C1
AE	40-5-12	-431	-431	---	BACK	VERT	TOTAL	---	C1
AF	44-5-12	-431	-431	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT
{40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0
PSF AND 5.0 PSF RESPECTIVELY.

STRUCTURAL COMPONENT ONLY
DWG # TR24040138

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T20	10	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:24 2024 Page 1
ID:AgdCIOSevh3uN4Xyl113N2zyjH4-mftIQ1IfFTCfx?RSjPHDJ8Ms1yrOW7VQxq9KwRzUnx1

Scale = 1:78.5

TOTAL WEIGHT = 10 X 311 = 3108 lb

2-H2.5A

HGUS26 OR 2-H2.5A

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
C	TMWWW-t	MT20	10.0	16.0	5.00	7.50
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	10.0	2.75	3.75
G	TMW+w	MT20	4.0	6.0		
H	TTWW-m	MT20	6.0	10.0	2.75	3.75
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWWW-t	MT20	10.0	16.0	5.00	7.50
L	TMBMW1-I	MT20	10.0	12.0	3.75	Edge
N	BMW+w	MT20	4.0	6.0		
O, Q, S, U						
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	6.0	7.0		
R	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	6.0	7.0		
V	BMW+w	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040139

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	4596	0	4596	201	-766	5-8	5-0
L	4596	0	4596	0	-766	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 4-1.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 766 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 766 LBS FACTORED UPLIFT

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3344	2156 / 0	492 / 0	0 / 0	111 / -891	696 / 0	0 / 0
L	3344	2156 / 0	492 / 0	0 / 0	111 / -891	696 / 0	0 / 0

HORIZONTAL REACTIONS

B	--	0 / 0	0 / 0	0 / 0	144 / -144	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q.

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	LC1 TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-145.3	-145.3	0.11 (2)	10.00
B- X	-5106 / 809	-145.3	-145.3	0.21 (1)	3.75
X- C	-3321 / 624	-145.3	-145.3	0.25 (2)	4.50
C- D	-6832 / 1131	-145.3	-145.3	0.74 (2)	2.80
D- E	-6832 / 1131	-145.3	-145.3	0.74 (2)	2.80
E- F	-5891 / 1006	-145.3	-145.3	0.65 (2)	3.09
F- G	-5761 / 957	-155.3	-155.3	0.74 (1)	2.00
G- H	-5761 / 957	-155.3	-155.3	0.74 (1)	2.00
H- I	-5891 / 1006	-145.3	-145.3	0.65 (2)	3.09
I- J	-6832 / 1131	-145.3	-145.3	0.74 (3)	2.80
J- K	-6832 / 1131	-145.3	-145.3	0.74 (3)	2.80
K- Z	-3321 / 625	-145.3	-145.3	0.25 (3)	4.50
Z- L	-5106 / 809	-145.3	-145.3	0.21 (1)	3.75
L- M	0 / 1	-145.3	-145.3	0.11 (3)	10.00
B- W	-653 / 2980	-39.5	-39.5	0.19 (1)	6.25
W- V	-1110 / 6197	-39.5	-39.5	0.35 (1)	6.25
V- U	-1112 / 6191	-39.5	-39.5	0.36 (1)	6.25
U- T	-962 / 6133	-39.5	-39.5	0.34 (1)	6.25
T- S	-962 / 6133	-39.5	-39.5	0.34 (1)	6.25
S- R	-653 / 5246	-39.5	-39.5	0.30 (1)	6.25
R- Q	-494 / 5246	-39.5	-39.5	0.30 (1)	6.25
Q- P	-761 / 6133	-39.5	-39.5	0.34 (1)	6.25
P- O	-761 / 6133	-39.5	-39.5	0.34 (1)	6.25
O- N	-913 / 6191	-39.5	-39.5	0.36 (1)	6.25
N- Y	-911 / 6197	-39.5	-39.5	0.35 (1)	6.25
Y- L	-453 / 2981	-39.5	-39.5	0.19 (1)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	0 / 276	0.05 (17)
C- U	-322 / 237	0.20 (2)
U- E	-13 / 418	0.07 (5)
E- S	-1547 / 436	0.55 (2)
S- F	-250 / 1334	0.21 (2)
F- R	-244 / 1452	0.23 (3)
R- G	-1227 / 234	0.49 (1)
R- H	-244 / 1452	0.23 (2)
Q- H	-250 / 1334	0.21 (3)
Q- I	-1547 / 436	0.55 (3)
O- I	-13 / 418	0.07 (6)
K- Z		
N- K	0 / 276	0.05 (17)
W- X	-278 / 2347	0.00 (1)
W- C	-4056 / 576	0.63 (1)
K- Y	-4056 / 578	0.63 (1)
Y- Z	-278 / 2347	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 43.5 PSF
         DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
         DL = 7.4 PSF
TOTAL LOAD = 67.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/180 (3.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.74/1.00 (F-G:1) , BC=0.36/1.00 (N-O:1) ,
WB=0.63/1.00 (C-W:1) , SSI=0.40/1.00 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
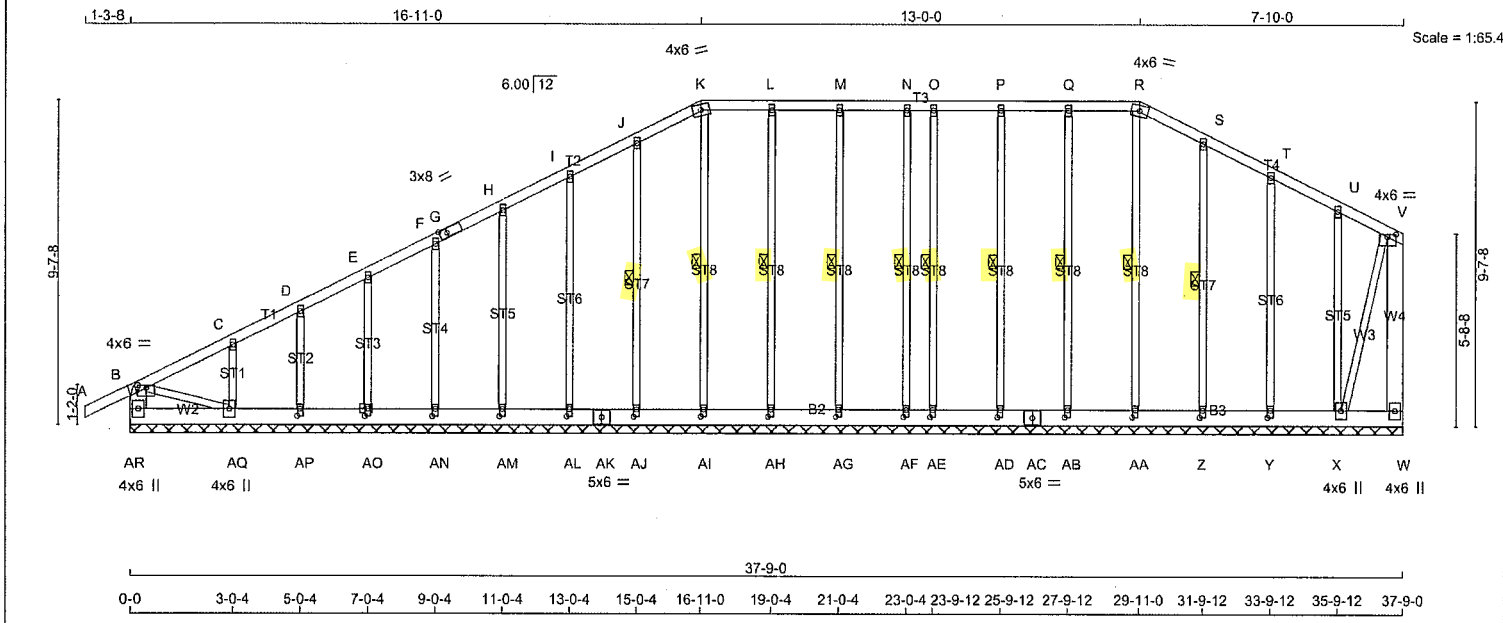JSI METAL= 0.86 (T) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T20	10	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-mftIQ1IfFTCfx?RSjPHDJ8Ms1yrOW7VQxq9KwRzUnx1

NOTES-
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT
{40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0
PSF AND 5.0 PSF RESPECTIVELY.

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040139

REVIEWED

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:25 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-ErRqdNIH0nKWZ90eG6pSsLv0sM9ZFWvZAUutTtzUnx0

1-3-8 16-11-0 13-0-0 8-6-0

Scale = 1:65.0
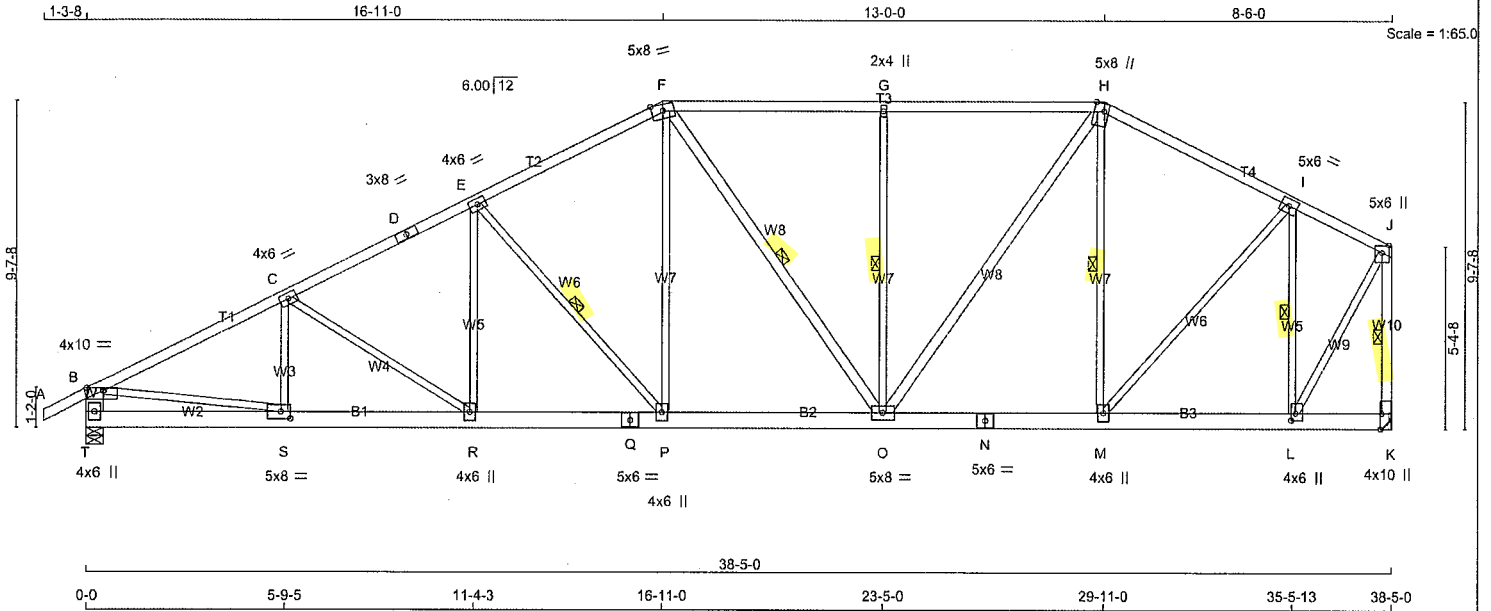
TOTAL WEIGHT = 4 X 211 = 843 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
F - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	10.0	1.00	5.25
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	8.0	Edge	3.50
I	TMWW-t	MT20	5.0	6.0		
J	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+t	MT20	4.0	10.0	Edge	0.50
L	BMWW+t	MT20	4.0	6.0	2.50	1.50
M, P, R						
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	8.0	2.50	3.25
T	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

STRUCTURAL COMPONENT ONLY
DWG # TR24040140

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2718	0	2718	0	0	5-8	3-5
K	2594	0	2594	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM
BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1907	1336 / 0	0 / 0	0 / 0	0 / 0	572 / 0	0 / 0
K	1826	1248 / 0	0 / 0	0 / 0	0 / 0	578 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING
FOR SECTION F-H, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, F-O, G-O, H-M, I-L, J-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT.LOAD (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	S- C	-395 / 0	0.09 (1)
B- C	-3912 / 0	-112.4	-112.4	0.75 (1)	2.95	C- R	-311 / 0	0.25 (1)
C- D	-3647 / 0	-112.4	-112.4	0.61 (1)	3.21	R- E	0 / 284	0.06 (1)
D- E	-3647 / 0	-112.4	-112.4	0.61 (1)	3.21	E- P	-901 / 0	0.45 (1)
E- F	-3010 / 0	-112.4	-112.4	0.57 (1)	3.52	P- F	0 / 799	0.18 (1)
F- G	-2662 / 0	-122.4	-122.4	0.80 (1)	2.00	F- O	-23 / 5	0.02 (1)
G- H	-2662 / 0	-122.4	-122.4	0.80 (1)	2.00	O- G	-982 / 0	0.58 (1)
H- I	-2139 / 0	-112.4	-112.4	0.48 (1)	4.16	O- H	0 / 1357	0.22 (1)
I- J	-1228 / 0	-112.4	-112.4	0.34 (1)	5.37	M- H	-724 / 0	0.43 (1)
T- B	-2652 / 0	0.0	0.0	0.17 (1)	6.39	M- I	0 / 1134	0.26 (1)
K- J	-2567 / 0	0.0	0.0	0.38 (1)	4.28	L- I	-1869 / 0	0.54 (1)
						B- S	0 / 3554	0.80 (1)
T- S	0 / 0	-18.5	-18.5	0.08 (1)	10.00	L- J	0 / 2249	0.51 (1)
S- R	0 / 3522	-18.5	-18.5	0.49 (1)	10.00			
R- Q	0 / 3262	-18.5	-18.5	0.42 (1)	10.00			
Q- P	0 / 3262	-18.5	-18.5	0.34 (1)	10.00			
P- O	0 / 2676	-18.5	-18.5	0.35 (1)	10.00			
O- N	0 / 1881	-18.5	-18.5	0.26 (1)	10.00			
N- M	0 / 1881	-18.5	-18.5	0.26 (1)	10.00			
M- L	0 / 1141	-18.5	-18.5	0.17 (1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

NOTES-
1) Lateral braces to be a minimum of 2X4 SPF #2.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS
OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD
LOAD OF 4.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.80/1.00 (F-G:1) , BC=0.49/1.00 (R-S:1) ,
WB=0.80/1.00 (B-S:1) , SSI=0.39/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (S) (INPUT = 0.90)
JSI METAL = 0.65 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T23G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AqdClOSevh3uN4Xyl113N2zyjH4-AEZR23KYYOaEoT91OXrwxm VYAxMjZMsdoN XmzUnx

1-3-8 16-11-0 13-0-0 7-10-0 Scale = 1:65.4

TOTAL WEIGHT = 2 X 236 = 472 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - R	2x4	DRY	No.2	SPF
R - V	2x4	DRY	No.2	SPF
AR- B	2x6	DRY	No.2	SPF
W - V	2x6	DRY	No.2	SPF
AR- AK	2x6	DRY	No.2	SPF
AK- AC	2x6	DRY	No.2	SPF
AC- W	2x6	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS				
	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 200-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C,D,E,F,H,I,J,L,M,N,O,P,Q,S,T,U						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0	1.50	2.75
K	TTW-m	MT20	4.0	6.0		
R	TTW-m	MT20	4.0	6.0		
V	TMVW-p	MT20	4.0	6.0	1.00	3.00
W	BMV1+p	MT20	4.0	6.0		
X	BMWW1+t	MT20	4.0	6.0		
Y,Z,AA,AB,AD,AE,AF,AG,AH,AI,AJ,AL,AM,AN,AO,AP						
Y	BMW1+w	MT20	2.0	4.0	2.50	1.00
AC	BS-t	MT20	5.0	6.0		
AK	BS-t	MT20	5.0	6.0		
AQ	BMWW1+t	MT20	4.0	6.0		
AR	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF R-AA, K-AI, L-AH, M-AG, Q-AB, P-AD, N-AF, O-AE, J-AJ, S-Z.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	AA- R	-183 / 0	0.11 (1)
B- C	-17 / 0	-112.4	-112.4	0.12 (1)	6.25	AI- K	-184 / 0	0.11 (1)
C- D	-34 / 0	-112.4	-112.4	0.12 (1)	6.25	AH- L	-258 / 0	0.15 (1)
D- E	-12 / 0	-112.4	-112.4	0.05 (1)	6.25	AG- M	-223 / 0	0.13 (1)
E- F	-12 / 0	-112.4	-112.4	0.05 (1)	6.25	AB- Q	-258 / 0	0.15 (1)
F- G	-8 / 0	-112.4	-112.4	0.05 (1)	10.00	AD- P	-223 / 0	0.13 (1)
G- H	-8 / 0	-112.4	-112.4	0.05 (1)	10.00	AF- N	-153 / 0	0.09 (1)
H- I	-6 / 0	-112.4	-112.4	0.05 (1)	10.00	AE- O	-153 / 0	0.09 (1)
I- J	-2 / 0	-112.4	-112.4	0.06 (1)	10.00	AL- I	-220 / 0	0.23 (1)
J- K	-11 / 0	-112.4	-112.4	0.06 (1)	6.25	AJ- J	-247 / 0	0.11 (1)
K- L	0 / 0	-112.4	-112.4	0.06 (1)	10.00	AM- H	-224 / 0	0.16 (1)
L- M	0 / 0	-112.4	-112.4	0.06 (1)	10.00	AQ- C	-336 / 0	0.05 (1)
M- N	0 / 0	-112.4	-112.4	0.05 (1)	10.00	AP- D	-184 / 0	0.04 (1)
N- O	0 / 0	-112.4	-112.4	0.04 (1)	10.00	AO- E	-232 / 0	0.07 (1)
O- P	0 / 0	-112.4	-112.4	0.04 (1)	10.00	AN- F	-222 / 0	0.10 (1)
P- Q	0 / 0	-112.4	-112.4	0.06 (1)	10.00	Z- S	-247 / 0	0.11 (1)
Q- R	0 / 0	-112.4	-112.4	0.06 (1)	10.00	Y- T	-219 / 0	0.23 (1)
R- S	-11 / 0	-112.4	-112.4	0.06 (1)	6.25	X- U	-239 / 0	0.17 (1)
S- T	-2 / 0	-112.4	-112.4	0.06 (1)	10.00	B-AQ	0 / 28	0.01 (1)
T- U	-6 / 0	-112.4	-112.4	0.05 (1)	10.00	X- V	0 / 23	0.01 (1)
U- V	0 / 0	-112.4	-112.4	0.05 (1)	10.00			
AR- B	-309 / 0	0.0	0.0	0.02 (1)	7.81			
W- V	-110 / 0	0.0	0.0	0.04 (1)	7.81			
AR-AQ	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
AQ-AP	0 / 18	-18.5	-18.5	0.02 (4)	10.00			
AP-AO	0 / 14	-18.5	-18.5	0.01 (4)	10.00			
AO-AN	0 / 10	-18.5	-18.5	0.01 (4)	10.00			
AN-AM	0 / 8	-18.5	-18.5	0.01 (4)	10.00			
AM-AL	0 / 5	-18.5	-18.5	0.01 (4)	10.00			
AL-AK	0 / 3	-18.5	-18.5	0.01 (4)	10.00			
AK-AJ	0 / 3	-18.5	-18.5	0.01 (4)	10.00			
AJ-AI	0 / 1	-18.5	-18.5	0.01 (4)	10.00			
AI-AH	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
AH-AG	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
AG-AF	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
AF-AE	0 / 0	-18.5	-18.5	0.00 (4)	10.00			
AE-AD	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
AD-AC	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
AC-AB	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
AB-AA	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
AA- Z	0 / 1	-18.5	-18.5	0.01 (4)	10.00			
Z- Y	0 / 3	-18.5	-18.5	0.01 (4)	10.00			
Y- X	0 / 5	-18.5	-18.5	0.01 (4)	10.00			
X- W	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD	=		45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) BC=0.02/1.00 (AP-AQ:4) , WB=0.23/1.00 (I-AL:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (AQ) (INPUT = 0.90)
JSI METAL= 0.14 (C) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437026	T23G	2	1	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-AEZR23KYYOaEoT91OXrwxm_VYAxMjZMsdoN_XmzUnx

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.		DRWG NO.
437026	T24	4	1	BAYVIEW WELLINGTON		
Tamarack Roof Truss, Burlington				TRUSS DESC.		

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:28 2024 Page 1
ID:AqdClOSevh3uN4Xyl113N2zyjH4-eQ7pGPLAJii5QdkDyFM9U XdBaFaSx70sS7X2CzUnwz

1-3-8   9-8-8

Scale = 1:54.8

10.00 ⌐12   4x6 //
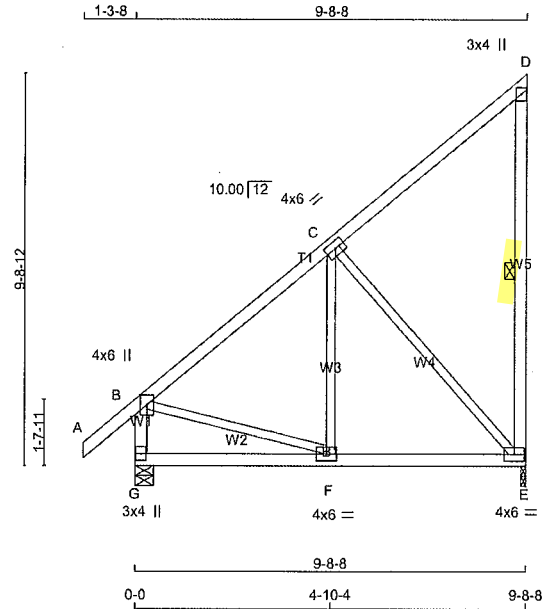
TOTAL WEIGHT = 4 X 53 = 211 lb   [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		Edge
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	636	0	636	0	0	1-8	1-8
G	791	0	791	0	0	5-8	1-8

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	445	315/0	0/0	0/0	0/0	130/0	0/0
G	552	405/0	0/0	0/0	0/0	147/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MAX. FACTORED		FACTORED					MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	F- C	0 / 97	0.03 (4)
B- C	-423 / 0	-112.4 -112.4	0.35 (1)	6.25	C- E	-533 / 0	0.51 (1)
C- D	-43 / 0	-112.4 -112.4	0.34 (1)	6.25	B- F	0 / 371	0.08 (1)
E- D	-205 / 0	0.0 0.0	0.09 (1)	6.25			
G- B	-756 / 0	0.0 0.0	0.08 (1)	7.81			
G- F	0 / 0	-18.5 -18.5	0.12 (4)	10.00			
F- E	0 / 358	-18.5 -18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD			=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.15/1.00 (E-F:4) ,
WB=0.51/1.00 (C-E:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL = 0.29 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T24A	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-6chBTlLo40qy2nJPVytO0B4oxzbpBOM956s4bfzUnwy

9-8-8

3x4 ||

C

Scale = 1:54.8

10.00 |12

4x6 //

B

9-8-12

W5

4x6 ||

A

W3   W4

1-7-11

W1

W2

F   E   D

3x4 || 4x6 =   4x6 =

9-8-8

0-0   4-10-4   9-8-8

TOTAL WEIGHT = 4 X 51 = 204 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	6.0		
E	BMWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	636	0	636	0	0	1-8	1-8
F	636	0	636	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM
BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	445	315 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
F	445	315 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MAX. FACTORED		FACTORED				MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)		CSI (LC)		(LBS)	CSI (LC)
FR-TO		FROM TO		UNBRAC LENGTH	FR-TO		
A- B	-423 / 0	-112.4	-112.4	0.35 (1) 6.25	E- B	0 / 97	0.03 (4)
B- C	-43 / 0	-112.4	-112.4	0.34 (1) 6.25	B- D	-533 / 0	0.51 (1)
D- C	-205 / 0	0.0	0.0	0.09 (1) 6.25	A- E	0 / 371	0.08 (1)
F- A	-600 / 0	0.0	0.0	0.06 (1) 7.81			
F- E	0 / 0	-18.5	-18.5	0.12 (4) 10.00			
E- D	0 / 358	-18.5	-18.5	0.15 (4) 10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   24.0   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.35/1.00 (A-B:1) , BC=0.15/1.00 (D-E:4) ,
WB=0.51/1.00 (B-D:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.37 (A) (INPUT = 0.90)
JSI METAL= 0.25 (A) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T24G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AqdClOSevh3uN4Xyl113N2zyjH4-3?oyuRN2bd5qH4TodNws6c9BQnJlfMxSYQLBfXzUnww

Scale = 1:58.8
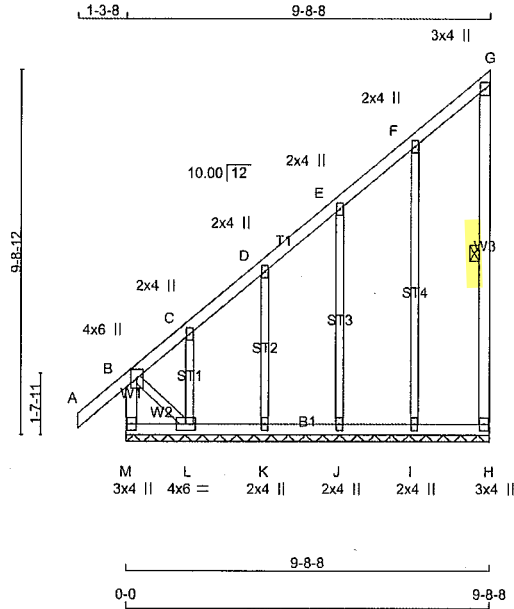
TOTAL WEIGHT = 2 X 57 = 114 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESCR.
M - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS				
	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW+p	MT20	4.0	6.0	Edge
C, D, E, F					
C	TMW+w	MT20	2.0	4.0	
G	TMV+p	MT20	3.0	4.0	
H	BMV1+p	MT20	3.0	4.0	
I, J, K					
I	BMV1+w	MT20	2.0	4.0	
L	BMWW1-t	MT20	4.0	6.0	
M	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

	CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
M- B	-330 / 0	0.0	0.0	0.04 (1)	7.81	I- F	-254 / 0	0.32 (1)		
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	J- E	-214 / 0	0.14 (1)		
B- C	-58 / 0	-112.4	-112.4	0.15 (1)	6.25	K- D	-239 / 0	0.08 (1)		
C- D	-6 / 0	-112.4	-112.4	0.06 (1)	10.00	L- C	-127 / 0	0.02 (1)		
D- E	-11 / 0	-112.4	-112.4	0.06 (1)	6.25	B- L	0 / 20	0.00 (1)		
E- F	-1 / 0	-112.4	-112.4	0.06 (1)	10.00					
F- G	-14 / 0	-112.4	-112.4	0.06 (1)	6.25					
H- G	-96 / 0	0.0	0.0	0.04 (1)	6.25					
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00					
L- K	0 / 12	-18.5	-18.5	0.02 (4)	10.00					
K- J	0 / 6	-18.5	-18.5	0.01 (4)	10.00					
J- I	0 / 3	-18.5	-18.5	0.02 (4)	10.00					
I- H	0 / 0	-18.5	-18.5	0.02 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (H-I:4) ,
WB=0.32/1.00 (F-I:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
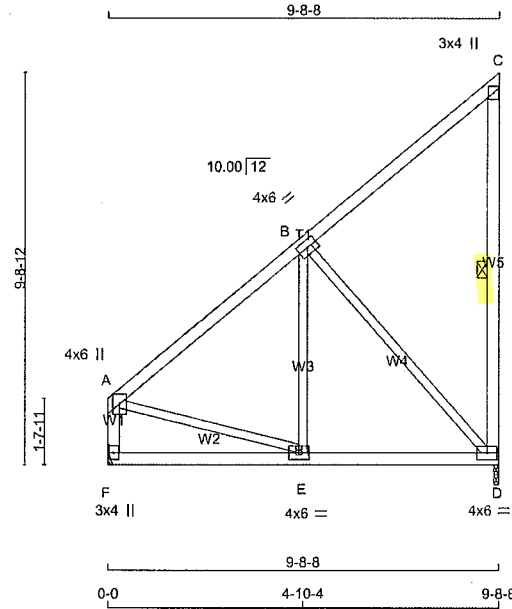JSI METAL= 0.14 (F) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T25	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-XBMK5mOgMxDXvE2 B4R5eqiNwBcGOIQbn45IBzzUnwv

9-8-8

Scale = 1:53.3
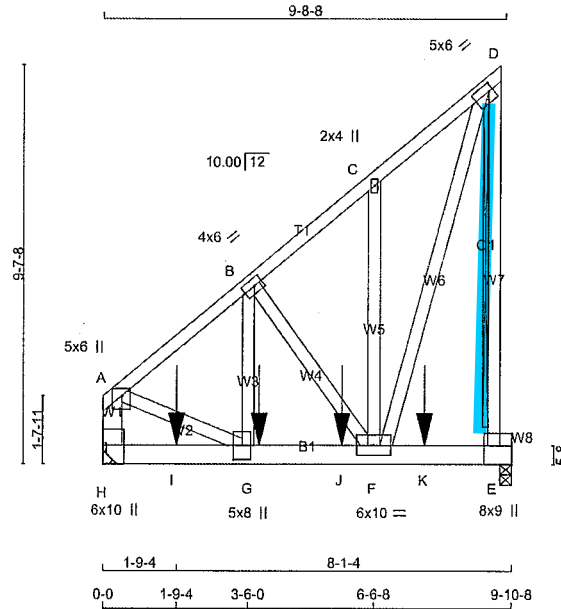
TOTAL WEIGHT = 3 X 75 = 226 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
H - E	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF __3__ TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	TOP
H-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H-E	2	6	SIDE(884.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMW+p	MT20	2.0	4.0		
D	TMWW-t	MT20	5.0	6.0	2.25	1.25
E	BMWW1+t	MT20	8.0	9.0	5.50	Edge

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	5786	0	5786	0	0	MECHANICAL	
E	5752	0	5752	0	0	3-8	2-1

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-11.

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	4067	2810 / 0	0 / 0	0 / 0	0 / 0	1257 / 0	0 / 0
E	4043	2794 / 0	0 / 0	0 / 0	0 / 0	1250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEM. FACTORED FORCE		FACTORED VERT. LOAD LC1 MAX			MAX. UNBRAC	MEM.	MAX. FACTORED FORCE	MAX
MEMB.	FORCE (LBS)	(PLF)		CSI (LC)		MEMB.	(LBS)	CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	-4985 / 0	-112.4 -112.4	0.09 (1)	5.08		E- D	-4781 / 0	0.49 (1)
B- C	-2635 / 0	-112.4 -112.4	0.06 (1)	6.25		A- G	0 / 4072	0.22 (1)
C- D	-2610 / 0	-112.4 -112.4	0.06 (1)	6.25		G- B	0 / 3603	0.19 (1)
H- A	-4712 / 0	0.0 0.0	0.11 (1)	7.81		B- F	-3102 / 0	0.31 (1)
						F- C	-301 / 0	0.06 (1)
H- I	0 / 0	-18.5 -18.5	0.23 (1)	10.00		F- D	0 / 6598	0.35 (1)
I- G	0 / 0	-18.5 -18.5	0.23 (1)	10.00				
G- J	0 / 3845	-18.5 -18.5	0.18 (1)	10.00				
J- F	0 / 3845	-18.5 -18.5	0.18 (1)	10.00				
F- K	0 / 0	-18.5 -18.5	0.19 (1)	10.00				
K- E	0 / 0	-18.5 -18.5	0.19 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	3-9-4	-1811	-1811	---	BACK	VERT	TOTAL	---	C1
I	1-9-4	-1811	-1811	---	BACK	VERT	TOTAL	---	C1
J	5-9-4	-1811	-1811	---	BACK	VERT	TOTAL	---	C1
K	7-9-4	-1811	-1811	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD			=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.11/1.00 (A-H:1) , BC=0.23/1.00 (G-H:1) , WB=0.49/1.00 (D-E:1) , SSI=0.37/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.46 (G) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040145

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T25	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-XBMK5mOgMxDXvE2_B4R5eqlNwBcGOIQbn45IBzzUnwv

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMWWW-t	MT20	6.0	10.0		
G	BMWW+t	MT20	5.0	8.0	4.25	2.50
H	BMV1+p	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040145

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T26	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-?OwiJ6Pl7ELOWOdBkoyKB1EWqbxS7AKI0kqIkQzUnwu

9-10-8

Scale = 1:54.2

TOTAL WEIGHT = 3 X 77 = 232 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
H - E	2x8	DRY	1950F 1.7E	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- D 1	12	TOP
D- E 1	12	TOP
H- A 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
H- E 3	5	SIDE(1368.8)
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 2	4	
A- G 1	6	
B- F 1	6	
C- F 1	6	
C- E 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	9958	0	10085	0	-1844	3-8	3-8
H	10037	0	10163	458	-1640	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 1844 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 1640 LBS FACTORED UPLIFT

PROVIDE FOR 458 LBS FACTORED HORIZONTAL REACTION AT JOINT H

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	7233	4793 / 0	1038 / 0	0 / 0	423 / -2050	1486 / 0	0 / 0
H	7290	4830 / 0	1046 / 0	0 / 0	373 / -1910	1497 / 0	0 / 0

HORIZONTAL REACTIONS

JT							
H	---	0 / 0	0 / 0	0 / 0	327 / -214	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) E (BASED ON SUPPORT DEPTH = 1-8)

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-E, C-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS						WEBS		
	MAX. FACTORED	FACTORED					MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO						
A- B	-8804 / 1462	-145.3 -145.3	0.22 (2)	3.91		A- G	-1188 / 7146	0.38 (2)
B- C	-5254 / 957	-145.3 -145.3	0.13 (2)	4.94		G- B	-969 / 5572	0.30 (3)
C- D	-142 / 191	-145.3 -145.3	0.09 (2)	6.25		B- F	-4560 / 933	0.41 (2)
E- D	-219 / 123	0.0 0.0	0.14 (11)	7.81		F- C	-1902 / 10824	0.58 (2)
H- A	-8206 / 1329	0.0 0.0	0.18 (2)	6.32		C- E	-9041 / 1740	0.58 (2)
H- I	-439 / 291	-39.5 -39.5	0.26 (2)	6.25				
I- G	-439 / 291	-39.5 -39.5	0.26 (2)	6.25				
G- J	-1310 / 6770	-39.5 -39.5	0.21 (2)	6.25				
J- K	-1310 / 6770	-39.5 -39.5	0.21 (2)	6.25				
K- F	-1310 / 6770	-39.5 -39.5	0.21 (2)	6.25				
F- L	-793 / 4060	-39.5 -39.5	0.29 (2)	6.25				
L- E	-793 / 4060	-39.5 -39.5	0.29 (2)	6.25				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	1-9-4	-3308	-3308	366	FRONT	VERT	TOTAL	---	C1
J	3-9-4	-3308	-3308	366	FRONT	VERT	TOTAL	---	C1
K	5-9-4	-3308	-3308	366	FRONT	VERT	TOTAL	---	C1
L	7-9-4	-3308	-3308	366	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	43.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD =		67.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/180 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.22/1.00 (A-B:2) , BC=0.29/1.00 (E-F:2) , WB=0.58/1.00 (C-F:2) , SSI=0.88/1.00 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.68 (G) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040146

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T26	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:33 2024 Page 2
ID:AgdCIOSevh3uN4Xyl113N2zyjH4-?OwiJ6PI7ELOWOdBkoyKB1EWqbxS7AKI0kqIkQzUnwu

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	6.0	7.0	Edge	
B	TMWW-t	MT20	4.0	6.0	2.00	1.50
C	TMWW-t	MT20	5.0	8.0	2.25	2.25
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1+p	MT20	10.0	12.0	Edge	3.00
F	BMWW+t	MT20	6.0	10.0	5.50	3.00
G	BMWW+t	MT20	6.0	10.0	5.50	2.50
H	BMV1+p	MT20	6.0	12.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES· (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT
{40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0
PSF AND 5.0 PSF RESPECTIVELY.

STRUCTURAL COMPONENT ONLY
DWG # TR24040146

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T30	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:35 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-xm2SkoQZfsb5mimZsD oGSKkNOc3b822T2JPolzUnws

Scale = 1:36.6

TOTAL WEIGHT = 2 X 102 = 205 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A - C	1	12	TOP
C - F	1	12	SIDE(61.0)
F - G	1	12	SIDE(61.0)
N - B	2	12	TOP
H - G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
N - K	2	12	TOP
K - H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3	1	6	
E - J	1	6	SIDE(169.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	2302	0	2302	0	0	5-8	1-8
H	2809	0	2809	0	0	1-8	1-8

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1608	1168 / 0	0 / 0	0 / 0	0 / 0	440 / 0	0 / 0
H	1964	1415 / 0	0 / 0	0 / 0	0 / 0	549 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

	CHORDS MAX. FACTORED		FACTORED				WEBS MAX. FACTORED		
MEMB.	FORCE		VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
FR-TO	(LBS)		FROM TO (PLF)		CSI (LC)	UNBRAC	FR-TO	(LBS)	CSI (LC)
A - B	0 / 50		-112.4 -112.4	0.09 (1)		10.00	M - C	-533 / 0	0.07 (1)
B - C	-2075 / 0		-112.4 -112.4	0.11 (1)		6.02	C - L	0 / 2368	0.29 (1)
C - D	-3447 / 0		-112.4 -112.4	0.25 (1)		4.79	L - D	-1337 / 0	0.17 (1)
D - E	-4558 / 0		-112.4 -112.4	0.57 (1)		3.82	D - J	0 / 1418	0.18 (1)
E - O	-4558 / 0		-112.4 -112.4	0.73 (1)		3.69	J - E	-992 / 0	0.13 (1)
O - P	-4558 / 0		-112.4 -112.4	0.73 (1)		3.69	J - F	0 / 3017	0.37 (1)
P - Q	-4558 / 0		-112.4 -112.4	0.73 (1)		3.69	I - F	-768 / 0	0.10 (1)
Q - F	-4558 / 0		-112.4 -112.4	0.73 (1)		3.69	B - M	0 / 1710	0.21 (1)
F - G	-2572 / 0		-112.4 -112.4	0.10 (1)		5.57	I - G	0 / 2226	0.28 (1)
N - B	-2291 / 0		0.0 0.0	0.08 (1)		7.81			
H - G	-2847 / 0		0.0 0.0	0.11 (1)		7.81			
N - M	0 / 0		-18.5 -18.5	0.02 (4)		10.00			
M - L	0 / 1573		-18.5 -18.5	0.12 (1)		10.00			
L - K	0 / 3447		-18.5 -18.5	0.29 (1)		10.00			
K - J	0 / 3447		-18.5 -18.5	0.29 (1)		10.00			
J - R	0 / 1946		-18.5 -18.5	0.20 (1)		10.00			
R - S	0 / 1946		-18.5 -18.5	0.20 (1)		10.00			
S - T	0 / 1946		-18.5 -18.5	0.20 (1)		10.00			
T - I	0 / 1946		-18.5 -18.5	0.20 (1)		10.00			
I - U	0 / 0		-18.5 -18.5	0.07 (1)		10.00			
U - H	0 / 0		-18.5 -18.5	0.07 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	18-10-8	-29	-29	---	FRONT	VERT	DEAD	---	C1
F	18-10-8	-157	-157	---	FRONT	VERT	SNOW	---	C1
J	12-5-8	-940	-940	---	FRONT	VERT	TOTAL	---	C1
O	13-10-0	-93	-93	---	FRONT	VERT	TOTAL	---	C1
P	15-8-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Q	17-8-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
R	13-10-0	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	15-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	17-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
U	19-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		45.9	PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.73/1.00 (E-F:1) , BC=0.29/1.00 (J-L:1) ,
WB=0.37/1.00 (F-J:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS: 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437026	T30	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-xm2SkoQZfsb5mimZsD_oGSKkNOc3b822T2JPolzUnws

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	1.75	2.75
G	TMVW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	4.0	6.0		
I, L, M						
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
N	BMV1+p	MT20	4.0	6.0		

NOTES· (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040147

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T31	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-Pzcrx8RBQ9jyOrLmQwV1pqs?coyHKdRBii3yKlzUnwn

1-3-8   4-1-14   13-6-4   3-10-6

Scale = 1:36.5

TOTAL WEIGHT = 94 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0		Edge
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	6.0		Edge
H	BMV1+p	MT20	3.0	4.0		
I, J, M						
I	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1566	0	1566	0	0	5-8	1-11
H	1410	0	1410	0	0	1-8	1-8

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1095	790 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	988	700 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	M- C	-213 / 12	0.08 (1)
B- C	-1306 / 0	-112.4 -112.4	0.38 (1)	5.18	C- L	0 / 830	0.19 (1)
C- D	-1567 / 0	-112.4 -112.4	0.34 (1)	4.87	L- D	-543 / 0	0.21 (1)
D- E	-1567 / 0	-112.4 -112.4	0.35 (1)	4.86	L- E	0 / 23	0.01 (1)
E- F	-1552 / 0	-112.4 -112.4	0.35 (1)	4.88	J- E	-563 / 0	0.22 (1)
F- G	-1237 / 0	-112.4 -112.4	0.32 (1)	5.36	J- F	0 / 887	0.20 (1)
N- B	-1535 / 0	0.0 0.0	0.16 (1)	6.64	I- F	-281 / 0	0.11 (1)
H- G	-1382 / 0	0.0 0.0	0.15 (1)	6.91	B- M	0 / 1046	0.24 (1)
					I- G	0 / 1017	0.23 (1)
N- M	0 / 0	-18.5 -18.5	0.08 (4)	10.00			
M- L	0 / 996	-18.5 -18.5	0.21 (1)	10.00			
L- K	0 / 1552	-18.5 -18.5	0.29 (1)	10.00			
K- J	0 / 1552	-18.5 -18.5	0.29 (1)	10.00			
J- I	0 / 941	-18.5 -18.5	0.20 (1)	10.00			
I- H	0 / 0	-18.5 -18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.29/1.00 (J-L:1) , WB=0.24/1.00 (B-M:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
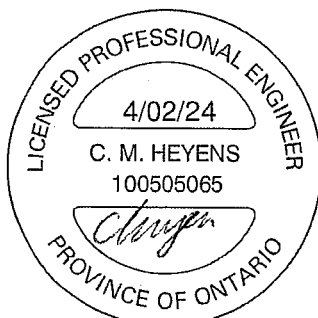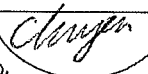| PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.57 (G) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040148

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T32	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-t99D8USpBTrp??wyze0GLtP50CJJ31NKxMoWtBzUnwq

TOTAL WEIGHT = 92 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1566	0	1566	0	0	5-8	1-11
G	1410	0	1410	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1095	790 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
G	988	700 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	K- C	-121 / 59	0.07 (1)
B- C	-1295 / 0	-112.4 -112.4	0.66 (1)	4.74	C- J	0 / 567	0.13 (1)
C- D	-1383 / 0	-112.4 -112.4	0.65 (1)	4.55	J- D	-765 / 0	0.45 (1)
D- E	-1383 / 0	-112.4 -112.4	0.65 (1)	4.55	J- E	0 / 620	0.14 (1)
E- F	-1248 / 0	-112.4 -112.4	0.58 (1)	4.96	H- E	-173 / 41	0.10 (1)
L- B	-1526 / 0	0.0 0.0	0.16 (1)	6.65	B- K	0 / 1021	0.23 (1)
G- F	-1373 / 0	0.0 0.0	0.15 (1)	6.94	H- F	0 / 999	0.22 (1)
L- K	0 / 0	-18.5 -18.5	0.13 (4)	10.00			
K- J	0 / 991	-18.5 -18.5	0.24 (1)	10.00			
J- I	0 / 954	-18.5 -18.5	0.23 (1)	10.00			
I- H	0 / 954	-18.5 -18.5	0.23 (1)	10.00			
H- G	0 / 0	-18.5 -18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.24/1.00 (J-K:1) ,
WB=0.45/1.00 (D-J:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040149

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.		DRWG NO.
437026	T33	1	1	TRUSS DESC.	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington						

Scale = 1:39.8

TOTAL WEIGHT = 103 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER		DESCR.
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
H - G	2x4	DRY	No.2		SPF
M - B	2x4	DRY	No.2		SPF
M - J	2x4	DRY	No.2		SPF
J - H	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
EXCEPT					
M - C	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1410	0	1410	0	0	1-8	1-8
M	1566	0	1566	0	0	5-8	1-11

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	988	700 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
M	1095	790 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT.LOAD (PLF) FROM TO	LC1	MAX CSI (LC)	MAX. UNBRAC	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4 -112.4		0.15 (1)	10.00	C- L	0 / 46	0.02 (4)
B- C	0 / 22	-112.4 -112.4		0.14 (1)	10.00	L- D	0 / 97	0.03 (4)
C- D	-1303 / 0	-112.4 -112.4		0.16 (1)	5.48	M- C	-1570 / 0	0.39 (1)
D- E	-1286 / 0	-112.4 -112.4		0.45 (1)	5.08	I- G	0 / 1494	0.34 (1)
E- F	-1286 / 0	-112.4 -112.4		0.47 (1)	5.05	D- K	0 / 466	0.10 (1)
F- G	-981 / 0	-112.4 -112.4		0.46 (1)	5.62	I- F	-1014 / 0	0.68 (1)
H- G	-1370 / 0	0.0 0.0		0.26 (1)	5.55	K- E	-634 / 0	0.43 (1)
M- B	-280 / 0	0.0 0.0		0.03 (1)	7.81	K- F	0 / 474	0.11 (1)
M- L	0 / 960	-18.5 -18.5		0.24 (1)	10.00			
L- K	0 / 982	-18.5 -18.5		0.24 (1)	10.00			
K- J	0 / 981	-18.5 -18.5		0.23 (1)	10.00			
J- I	0 / 981	-18.5 -18.5		0.23 (1)	10.00			
I- H	0 / 0	-18.5 -18.5		0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.   LL =   32.5   PSF
          DL =   6.0   PSF
BOT CH.   LL =   0.0   PSF
          DL =   7.4   PSF
TOTAL LOAD =   45.9   PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.47/1.00 (E-F:1), BC=0.24/1.00 (K-L:1) ,
WB=0.68/1.00 (F-I:1), SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. **BAYVIEW WELLINGTON**	DRWG NO.
437026	T33A	1	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

TOTAL WEIGHT = 100 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
L - B 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1383	0	1383	0	0	1-8	1-8
L	1383	0	1383	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A- B	0 / 23	-112.4	-112.4	0.12 (1)	10.00	B- K	0 / 143	0.03 (1)	
B- C	-1209 / 0	-112.4	-112.4	0.14 (1)	5.66	K- C	0 / 70	0.02 (4)	
C- D	-1239 / 0	-112.4	-112.4	0.45 (1)	5.16	L- B	-1489 / 0	0.33 (1)	
D- E	-1240 / 0	-112.4	-112.4	0.47 (1)	5.12	H- F	0 / 1458	0.33 (1)	
E- F	-957 / 0	-112.4	-112.4	0.45 (1)	5.67	C- J	0 / 504	0.11 (1)	
G- F	-1343 / 0	0.0	0.0	0.26 (1)	5.59	H- E	-987 / 0	0.66 (1)	
L- A	-100 / 0	0.0	0.0	0.01 (1)	7.81	J- D	-634 / 0	0.43 (1)	
						J- E	0 / 439	0.10 (1)	
L- K	0 / 820	-18.5	-18.5	0.21 (1)	10.00				
K- J	0 / 911	-18.5	-18.5	0.22 (1)	10.00				
J- I	0 / 957	-18.5	-18.5	0.22 (1)	10.00				
I- H	0 / 957	-18.5	-18.5	0.22 (1)	10.00				
H- G	0 / 0	-18.5	-18.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
         DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
         DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.47/1.00 (D-E:1) , BC=0.22/1.00 (H-J:1) , WB=0.66/1.00 (E-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
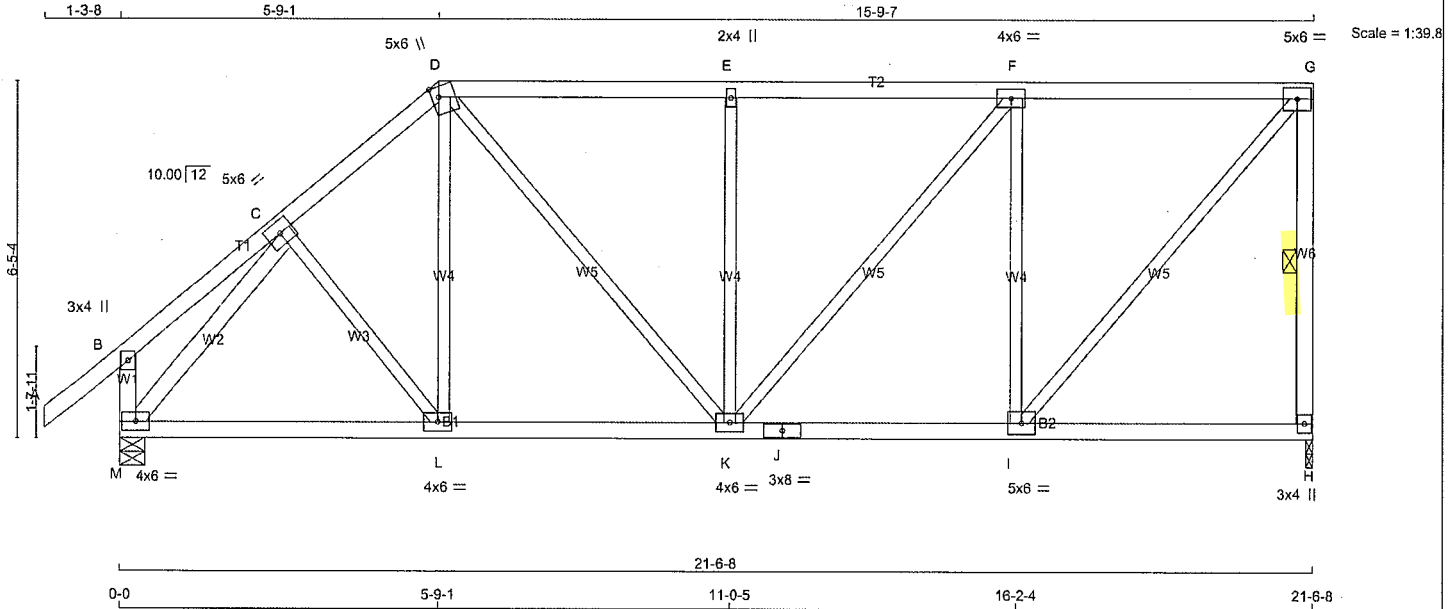JSI METAL= 0.43 (H) (INPUT = 0.95)

Tarnarack Roof Truss, Burlington

Scale = 1:42.3

TOTAL WEIGHT = 110 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER		DESCR.
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
H - G	2x4	DRY	No.2		SPF
M - B	2x4	DRY	No.2		SPF
M - J	2x4	DRY	No.2		SPF
J - H	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
EXCEPT					
M - C	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1410	0	1410	0	0	1-8	1-8
M	1566	0	1566	0	0	5-8	1-11

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	988	700 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
M	1095	790 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS		
MAX. FACTORED							MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO			LENGTH FR-TO			
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	C- L	-83 / 22	0.04 (1)	
B- C	0 / 29	-112.4 -112.4	0.21 (1)	10.00	L- D	0 / 199	0.05 (4)	
C- D	-1258 / 0	-112.4 -112.4	0.24 (1)	5.45	M- C	-1566 / 0	0.54 (1)	
D- E	-1088 / 0	-112.4 -112.4	0.37 (1)	5.55	I- G	0 / 1397	0.31 (1)	
E- F	-1088 / 0	-112.4 -112.4	0.39 (1)	5.52	D- K	0 / 255	0.06 (1)	
F- G	-796 / 0	-112.4 -112.4	0.38 (1)	6.21	I- F	-1041 / 0	0.36 (1)	
H- G	-1374 / 0	0.0 0.0	0.34 (1)	5.55	K- E	-585 / 0	0.58 (1)	
M- B	-304 / 0	0.0 0.0	0.03 (1)	7.81	K- F	0 / 528	0.12 (1)	
M- L	0 / 996	-18.5 -18.5	0.27 (1)	10.00				
L- K	0 / 945	-18.5 -18.5	0.27 (4)	10.00				
K- J	0 / 796	-18.5 -18.5	0.19 (1)	10.00				
J- I	0 / 796	-18.5 -18.5	0.19 (1)	10.00				
I- H	0 / 0	-18.5 -18.5	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.27/1.00 (L-M:1) , WB=0.58/1.00 (E-K:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (I) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040152

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T34A	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-E6z6CBWy0?T66mpvmBcR2x60?D0NkFX44eWGYOzUnwl

Scale = 1:41.7

TOTAL WEIGHT = 107 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
L - B 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0	2.25	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1383	0	1383	0	0	1-8	1-8
L	1383	0	1383	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM
BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS			FACTORED				WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)		VERT. LOAD LC1 (PLF)		MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO			FROM	TO		LENGTH	FR-TO		
A- B	0 / 29		-112.4	-112.4	0.19 (1)	10.00	B- K	0 / 45	0.02 (4)
B- C	-1182 / 0		-112.4	-112.4	0.22 (1)	5.62	K- C	0 / 118	0.04 (4)
C- D	-1051 / 0		-112.4	-112.4	0.37 (1)	5.63	L- B	-1492 / 0	0.46 (1)
D- E	-1052 / 0		-112.4	-112.4	0.39 (1)	5.59	H- F	0 / 1363	0.31 (1)
E- F	-777 / 0		-112.4	-112.4	0.38 (1)	6.25	C- J	0 / 291	0.07 (1)
G- F	-1346 / 0		0.0	0.0	0.33 (1)	5.59	H- E	-1015 / 0	0.35 (1)
L- A	-124 / 0		0.0	0.0	0.01 (1)	7.81	J- D	-585 / 0	0.58 (1)
							J- E	0 / 496	0.11 (1)
L- K	0 / 876		-18.5	-18.5	0.24 (1)	10.00			
K- J	0 / 888		-18.5	-18.5	0.24 (1)	10.00			
J- I	0 / 777		-18.5	-18.5	0.19 (1)	10.00			
I- H	0 / 777		-18.5	-18.5	0.19 (1)	10.00			
H- G	0 / 0		-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL = 32.5	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.39/1.00 (D-E:1), BC=0.24/1.00 (J-K:1),
WB=0.58/1.00 (D-J:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
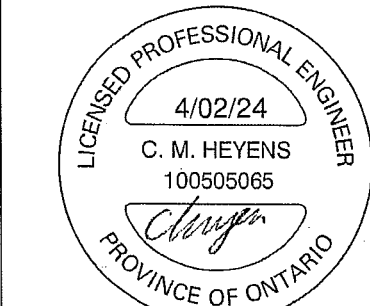
AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

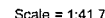JSI GRIP= 0.76 (H) (INPUT = 0.90)
JSI METAL= 0.35 (H) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T35	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-iJXUPXWanJbzjwO6Ku7gb8fArdMCTkFDJlFq4rzUnwk

TOTAL WEIGHT = 117 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

| ALL WEBS | 2x3 | DRY | No.2 | SPF |
| EXCEPT | | | | |

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1410	0	1410	0	0	1-8	1-8
N	1566	0	1566	0	0	5-8	1-11

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	988	700 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
N	1095	790 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, F-I, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS
MAX. FACTORED

MEMB.	FORCE (LBS)	VERT. LOAD LC1 FROM TO (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO				
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00
B- C	-1324 / 0	-112.4 -112.4	0.35 (1)	5.19
C- D	-1185 / 0	-112.4 -112.4	0.34 (1)	5.43
D- E	-932 / 0	-112.4 -112.4	0.30 (1)	5.99
E- F	-932 / 0	-112.4 -112.4	0.32 (1)	5.96
F- G	-653 / 0	-112.4 -112.4	0.31 (1)	6.25
H- G	-1376 / 0	0.0 0.0	0.45 (1)	5.54
N- B	-1532 / 0	0.0 0.0	0.16 (1)	6.65
N- M	0 / 0	-18.5 -18.5	0.07 (4)	10.00
M- L	0 / 1043	-18.5 -18.5	0.20 (1)	10.00
L- K	0 / 884	-18.5 -18.5	0.18 (1)	10.00
K- J	0 / 653	-18.5 -18.5	0.15 (1)	10.00
J- I	0 / 653	-18.5 -18.5	0.15 (1)	10.00
I- H	0 / 0	-18.5 -18.5	0.09 (4)	10.00

WEBS
MAX. FACTORED

MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		
M- C	-246 / 2	0.10 (1)
C- L	-251 / 0	0.18 (1)
L- D	0 / 274	0.06 (1)
B- M	0 / 1093	0.25 (1)
I- G	0 / 1338	0.30 (1)
D- K	0 / 100	0.02 (1)
I- F	-1073 / 0	0.49 (1)
K- E	-535 / 0	0.24 (1)
K- F	0 / 591	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.45/1.00 (G-H:1) , BC=0.20/1.00 (L-M:1) ,
WB=0.49/1.00 (F-I:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR   SECTION
        (PSI)   (PLI)   (PLI)
      MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (I) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T35A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Scale = 1:47.1

TOTAL WEIGHT = 114 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1383	0	1383	0	0	1-8	1-8
M	1383	0	1383	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM
BEARING LENGTH AT JOINT M = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
M	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H, D-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO	FR-TO		
A-B	-1188 / 0	-112.4	-112.4	0.31 (1)	5.47	L- B	-337 / 0	0.13 (1)
B-C	-1124 / 0	-112.4	-112.4	0.31 (1)	5.59	B- K	-159 / 0	0.11 (1)
C-D	-902 / 0	-112.4	-112.4	0.30 (1)	6.07	K- C	0 / 204	0.05 (1)
D-E	-902 / 0	-112.4	-112.4	0.32 (1)	6.04	A- L	0 / 1025	0.23 (1)
E-F	-638 / 0	-112.4	-112.4	0.31 (1)	6.25	H- F	0 / 1307	0.29 (1)
G-F	-1349 / 0	0.0	0.0	0.44 (1)	5.58	C- J	0 / 131	0.03 (1)
M-A	-1353 / 0	0.0	0.0	0.15 (1)	6.97	H- E	-1046 / 0	0.47 (1)
						J- D	-535 / 0	0.24 (1)
M- L	0 / 0	-18.5	-18.5	0.06 (4)	10.00	J- E	0 / 560	0.13 (1)
L- K	0 / 939	-18.5	-18.5	0.19 (1)	10.00			
K- J	0 / 839	-18.5	-18.5	0.17 (1)	10.00			
J- I	0 / 638	-18.5	-18.5	0.15 (1)	10.00			
I- H	0 / 638	-18.5	-18.5	0.15 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 BCBC 2018 , NBC-2019AE
- PART 9 of OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.44/1.00 (F-G:1) , BC=0.19/1.00 (K-L:1) ,
WB=0.47/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.55 (A) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
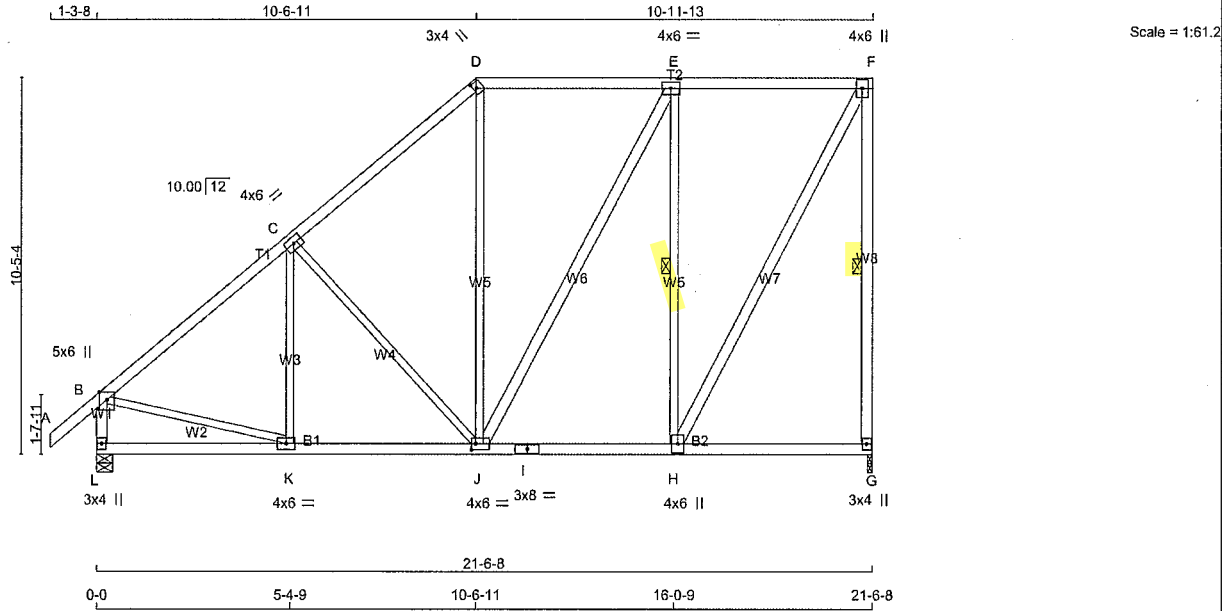DWG # TR24040155

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T36	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-7uCd1ZZS3EzXaO6h?1hNDnHcLqNaq4ef?FUUhAzUnwh

1-3-8 9-4-4 12-2-4

Scale = 1:52.4
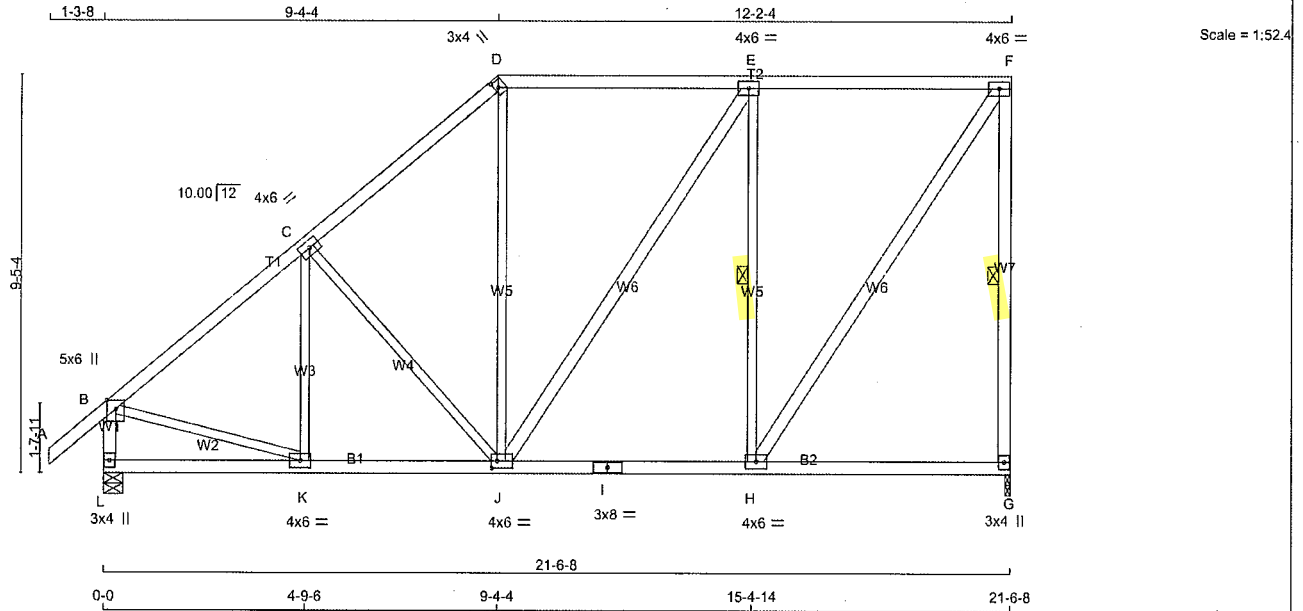
TOTAL WEIGHT = 4 X 118 = 471 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1410	0	1410	0	0	1-8	1-8
L	1566	0	1566	0	0	5-8	1-11

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	988	700 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
L	1095	790 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT.LOAD (PLF) FROM TO	LC1	MAX CSI	MAX (LC)	MAX. UNBRAC LENGTH		WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4 -112.4		0.15	(1)	10.00		K- C	-194 / 26	0.09 (1)
B- C	-1332 / 0	-112.4 -112.4		0.48	(1)	5.00		C- J	-356 / 0	0.34 (1)
C- D	-1109 / 0	-112.4 -112.4		0.46	(1)	5.38		J- D	0 / 235	0.05 (1)
D- E	-821 / 0	-112.4 -112.4		0.75	(1)	5.22		J- E	0 / 148	0.02 (1)
E- F	-740 / 0	-112.4 -112.4		0.74	(1)	5.44		H- E	-974 / 0	0.57 (1)
G- F	-1365 / 0	0.0	0.0	0.57	(1)	5.56		H- F	0 / 1329	0.21 (1)
L- B	-1528 / 0	0.0	0.0	0.16	(1)	6.65		B- K	0 / 1093	0.25 (1)
L- K	0 / 0	-18.5	-18.5	0.09	(4)	10.00				
K- J	0 / 1054	-18.5	-18.5	0.22	(1)	10.00				
J- I	0 / 740	-18.5	-18.5	0.22	(4)	10.00				
I- H	0 / 740	-18.5	-18.5	0.22	(4)	10.00				
H- G	0 / 0	-18.5	-18.5	0.17	(4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.75/1.00 (D-E:1) , BC=0.22/1.00 (H-J:4) ,
WB=0.57/1.00 (E-H:1) , SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T36A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-b4rn?FvZ4qY6OCYhtZkCcl pn8EjrPX8pEvD1DczUnwg

8-11-4   12-2-4

3x4 \\   4x6 =   4x6 =   Scale = 1:52.4

10.00 [12
4x6 //

9-5-4
5x6 ||
1-11-14

21-1-8

0-0   4-4-6   8-11-4   14-11-14   21-1-8

TOTAL WEIGHT = 115 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.00
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWWW-t	MT20	4.0	6.0	2.00	1.50
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1383	0	1383	0	0	1-8	1-8
K	1383	0	1383	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM
BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS		
MAX. FACTORED MEMB. FORCE (LBS)		VERT. LOAD (PLF)	LC1 MAX	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A- B	-1212 / 0	-112.4	-112.4	0.42 (1)	5.26	J- B	-275 / 0	0.13 (1)
B- C	-1059 / 0	-112.4	-112.4	0.42 (1)	5.55	B- I	-271 / 0	0.26 (1)
C- D	-785 / 0	-112.4	-112.4	0.75 (1)	5.31	I- C	0 / 200	0.04 (1)
D- E	-722 / 0	-112.4	-112.4	0.74 (1)	5.49	I- D	0 / 116	0.02 (1)
F- E	-1338 / 0	0.0	0.0	0.56 (1)	5.60	G- D	-947 / 0	0.56 (1)
K- A	-1349 / 0	0.0	0.0	0.15 (1)	6.98	G- E	0 / 1296	0.21 (1)
						A- J	0 / 1029	0.23 (1)
K- J	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
J- I	0 / 962	-18.5	-18.5	0.20 (1)	10.00			
I- H	0 / 722	-18.5	-18.5	0.22 (4)	10.00			
H- G	0 / 722	-18.5	-18.5	0.22 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
         DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
         DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.75/1.00 (C-D:1) , BC=0.22/1.00 (G-I:4) ,
WB=0.56/1.00 (D-G:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.46 (A) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
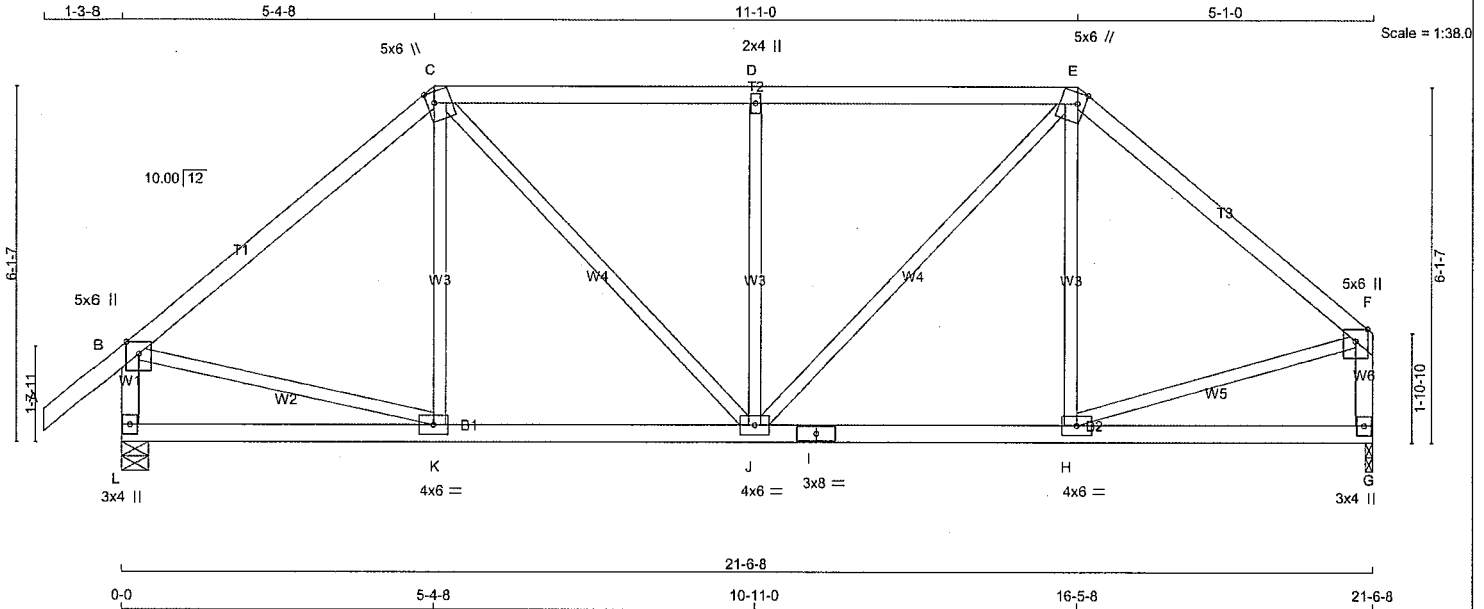DWG # TR24040157

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T37	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zy|H4-XTulgabLM9M6RrrFg9E4qPv7i1P9tNE5hDi8lUzUnwe

TOTAL WEIGHT = 2 X 124 = 248 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
   TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1410	0	1410	0	0	1-8	1-8
L	1566	0	1566	0	0	5-8	1-11

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	988	700 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
L	1095	790 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

	CHORDS						WEBS		
	MAX. FACTORED	FACTORED					MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO			LENGTH	FR-TO			
A- B	0 / 50	-112.4 -112.4	0.15 (1)		10.00	K- C	-146 / 52	0.09 (1)	
B- C	-1331 / 0	-112.4 -112.4	0.63 (1)		4.75	C- J	-458 / 0	0.58 (1)	
C- D	-1025 / 0	-112.4 -112.4	0.59 (1)		5.29	J- D	0 / 174	0.04 (4)	
D- E	-751 / 0	-112.4 -112.4	0.59 (1)		5.86	J- E	0 / 290	0.05 (1)	
E- F	-615 / 0	-112.4 -112.4	0.59 (1)		6.25	H- E	-1021 / 0	0.77 (1)	
G- F	-1369 / 0	0.0 0.0	0.74 (1)		5.55	H- F	0 / 1291	0.21 (1)	
L- B	-1525 / 0	0.0 0.0	0.16 (1)		6.65	B- K	0 / 1088	0.24 (1)	
L- K	0 / 0	-18.5 -18.5	0.13 (4)		10.00				
K- J	0 / 1057	-18.5 -18.5	0.23 (1)		10.00				
J- I	0 / 615	-18.5 -18.5	0.18 (4)		10.00				
I- H	0 / 615	-18.5 -18.5	0.18 (4)		10.00				
H- G	0 / 0	-18.5 -18.5	0.14 (4)		10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD		=		45.9	PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.74/1.00 (F-G:1) , BC=0.23/1.00 (J-K:1) ,
WB=0.77/1.00 (E-H:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (K) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040158

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T38	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

TOTAL WEIGHT = 2 X 107 = 214 lb

Scale = 1:36.3

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF __2__ TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS	#ROWS	SURFACE	LOAD(PLF)
		SPACING (IN)	

TOP CHORDS : (0.122"X3") SPIRAL NAILS
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
N- B	2	12	TOP
H- G	2	12	TOP

BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS
| N- K | 2 | 12 | TOP |
| K- H | 2 | 12 | SIDE(183.1) |

WEBS : (0.122"X3") SPIRAL NAILS
| 2x3 | 1 | 6 | |
| E- J | 1 | 6 | SIDE(105.6) |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	2264	0	2264	0	0	5-8	1-8
H	2579	0	2579	0	0	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1581	1152 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0
H	1803	1302 / 0	0 / 0	0 / 0	0 / 0	501 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.09 (1)	10.00	M- C	-414 / 0	0.07 (1)
B- C	-2120 / 0	-112.4 -112.4	0.18 (1)	5.88	C- L	0 / 1777	0.22 (1)
C- D	-2787 / 0	-112.4 -112.4	0.18 (1)	5.30	L- D	-1240 / 0	0.21 (1)
D- E	-3589 / 0	-112.4 -112.4	0.50 (1)	4.31	D- J	0 / 1237	0.15 (1)
E- O	-3588 / 0	-112.4 -112.4	0.58 (1)	4.29	J- E	-972 / 0	0.16 (1)
O- P	-3588 / 0	-112.4 -112.4	0.58 (1)	4.29	J- F	0 / 2064	0.26 (1)
P- Q	-3588 / 0	-112.4 -112.4	0.58 (1)	4.29	I- F	-550 / 0	0.09 (1)
Q- F	-3588 / 0	-112.4 -112.4	0.58 (1)	4.29	B- M	0 / 1699	0.21 (1)
F- G	-112.4 / 0	-112.4 -112.4	0.17 (1)	5.54	I- G	0 / 2061	0.26 (1)
N- B	-2235 / 0	0.0 0.0	0.08 (1)	7.81			
H- G	-2570 / 0	0.0 0.0	0.10 (1)	7.81			
N- M	0 / 0	-18.5 -18.5	0.02 (4)	10.00			
M- L	0 / 1614	-18.5 -18.5	0.12 (1)	10.00			
L- K	0 / 2787	-18.5 -18.5	0.22 (1)	10.00			
K- J	0 / 2787	-18.5 -18.5	0.22 (1)	10.00			
J- R	0 / 1902	-18.5 -18.5	0.18 (1)	10.00			
R- S	0 / 1902	-18.5 -18.5	0.18 (1)	10.00			
S- T	0 / 1902	-18.5 -18.5	0.18 (1)	10.00			
T- I	0 / 1902	-18.5 -18.5	0.18 (1)	10.00			
I- U	0 / 0	-18.5 -18.5	0.05 (4)	10.00			
U- H	0 / 0	-18.5 -18.5	0.05 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC. LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	17-11-8	-245	-245	---	BACK	VERT	TOTAL	---	C1
I	17-10-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
J	11-7-8	-769	-769	---	BACK	VERT	TOTAL	---	C1
O	12-10-12	-72	-72	---	BACK	VERT	TOTAL	---	C1
P	13-10-12	-72	-72	---	BACK	VERT	TOTAL	---	C1
Q	15-10-12	-72	-72	---	BACK	VERT	TOTAL	---	C1
R	12-10-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
S	13-10-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
T	15-10-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
U	19-10-12	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = __24.0__ **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.58/1.00 (E-F:1) , BC=0.22/1.00 (J-L:1) ,
WB=0.26/1.00 (F-J:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
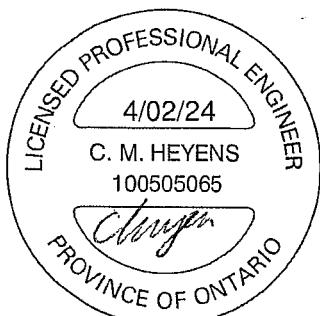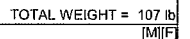RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.27 (K) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040159

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
437026	T38	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:50 2024 Page 2
ID:AgdClOSevh3uN4Xyl113N2zyjH4-?fS7twcz7TUz3?QSEsIJNdRKzRlZcybFwtSiqxzUnwd

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	4.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0	2.50	1.50
K	BS-t	MT20	5.0	6.0		
L	BMWW+t	MT20	4.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040159

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T39	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:51 2024 Page 1
ID:AqdClOSevh3uN4Xyl113N2zyjH4-Tr0W5Gcbumcqh9?eoaGYwq UPr5aLLpO9XBFMNzUnwc

TOTAL WEIGHT = 93 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

·DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW+p	MT20	5.0	6.0			Edge
C	TTWW+m	MT20	5.0	6.0	2.25	1.50	
D	TMW+w	MT20	2.0	4.0			
E	TTWW+m	MT20	5.0	6.0	2.25	1.50	
F	TMVW+p	MT20	5.0	6.0			Edge
G	BMV1+p	MT20	3.0	4.0			
H	BMWW-t	MT20	4.0	6.0			
I	BS-t	MT20	3.0	8.0			
J	BMWWW-t	MT20	4.0	6.0			
K	BMWW-t	MT20	4.0	6.0			
L	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1566	0	1566	0	0	5-8	1-11
G	1410	0	1410	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1095	790 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
G	988	700 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00		K- C	-120 / 59	0.07 (1)
B- C	-1295 / 0	-112.4 -112.4	0.66 (1)	4.73		C- J	0 / 563	0.13 (1)
C- D	-1379 / 0	-112.4 -112.4	0.64 (1)	4.57		J- D	-762 / 0	0.45 (1)
D- E	-1379 / 0	-112.4 -112.4	0.64 (1)	4.57		J- E	0 / 616	0.14 (1)
E- F	-1248 / 0	-112.4 -112.4	0.58 (1)	4.94		H- E	-172 / 41	0.10 (1)
L- B	-1526 / 0	0.0 0.0	0.16 (1)	6.65		B- K	0 / 1020	0.23 (1)
G- F	-1373 / 0	0.0 0.0	0.15 (1)	6.94		H- F	0 / 998	0.22 (1)
L- K	0 / 0	-18.5 -18.5	0.13 (4)	10.00				
K- J	0 / 990	-18.5 -18.5	0.24 (1)	10.00				
J- I	0 / 954	-18.5 -18.5	0.23 (1)	10.00				
I- H	0 / 954	-18.5 -18.5	0.23 (1)	10.00				
H- G	0 / 0	-18.5 -18.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.24/1.00 (J-K:1) , WB=0.45/1.00 (D-J:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (H) (INPUT = 0.90)
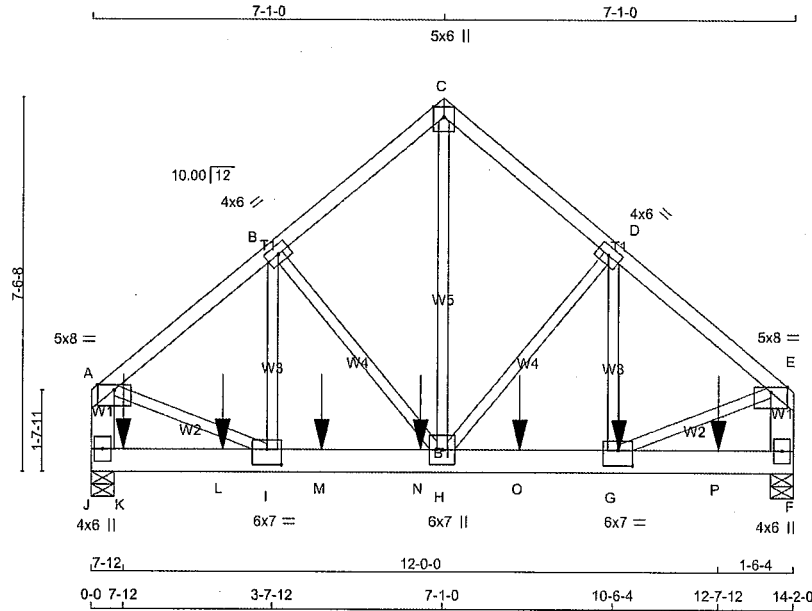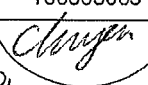JSI METAL= 0.53 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T40	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:52 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-x2ZuIcdDf4khIJaqMHonS2Xm0FQB4mrYNBxovpzUnwb

Scale = 1:43.7

TOTAL WEIGHT = 109 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1383	0	1383	0	0	MECHANICAL	
H	1383	0	1383	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
H	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- B	0 / 29	-112.4 -112.4	0.22 (1)	10.00	B- L	-42 / 33	0.02 (1)
B- C	-1169 / 0	-112.4 -112.4	0.18 (1)	5.71	L- C	0 / 172	0.05 (4)
C- D	-993 / 0	-112.4 -112.4	0.18 (1)	6.06	I- E	0 / 197	0.05 (4)
D- E	-993 / 0	-112.4 -112.4	0.18 (1)	6.06	I- F	-72 / 26	0.04 (1)
E- F	-1180 / 0	-112.4 -112.4	0.19 (1)	5.68	M- B	-1485 / 0	0.57 (1)
F- G	0 / 29	-112.4 -112.4	0.22 (1)	10.00	F- H	-1495 / 0	0.59 (1)
M- A	-148 / 0	0.0 0.0	0.02 (1)	7.81	K- D	-476 / 0	0.54 (1)
H- G	-155 / 0	0.0 0.0	0.02 (1)	7.81	C- K	0 / 274	0.06 (1)
					K- E	0 / 255	0.06 (1)
M- L	0 / 899	-18.5 -18.5	0.26 (4)	10.00			
L- K	0 / 876	-18.5 -18.5	0.27 (4)	10.00			
K- J	0 / 884	-18.5 -18.5	0.28 (4)	10.00			
J- I	0 / 884	-18.5 -18.5	0.28 (4)	10.00			
I- H	0 / 926	-18.5 -18.5	0.27 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.22/1.00 (F-G:1) , BC=0.28/1.00 (I-K:4) , WB=0.59/1.00 (F-H:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T41	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-uQhejlfUBh PYcjDTiqFXTc4z26oYlvrrVQvzizUnwZ

8-11-8   3-1-0   9-1-0

5x6 \\   3x4 //

Scale = 1:55.6

10.00 [12]
4x6 //

9-5-7

5x6 ||
1-11-14

4x6 //
4x6 \\

9-5-7
1-10-10

M   L   K   J   I   H   G
3x4 || 4x6 =   4x6 =   3x8 =   4x6 =   3x4 ||
4x6 =

21-1-8

0-0   4-7-0   8-11-8   12-0-8   16-5-0   21-1-8

TOTAL WEIGHT = 107 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H, K, L						
H	BMWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1383	0	1383	0	0	MECHANICAL	
G	1383	0	1383	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
G	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CSI(LC)	MAX.	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	-1216 / 0	-112.4 -112.4	0.31 (1)		5.45	L- B	-251 / 10	0.13 (1)
B- C	-1057 / 0	-112.4 -112.4	0.30 (1)		5.76	B- K	-290 / 0	0.28 (1)
C- D	-786 / 0	-112.4 -112.4	0.14 (1)		6.25	K- C	0 / 285	0.06 (1)
D- E	-1063 / 0	-112.4 -112.4	0.31 (1)		5.73	C- I	0 / 12	0.00 (1)
E- F	-1240 / 0	-112.4 -112.4	0.32 (1)		5.40	I- D	0 / 301	0.07 (1)
M- A	-1348 / 0	0.0 0.0	0.15 (1)		6.99	I- E	-315 / 0	0.30 (1)
G- F	-1346 / 0	0.0 0.0	0.15 (1)		6.99	H- E	-229 / 18	0.12 (1)
						A- L	0 / 1024	0.23 (1)
M- L	0 / 0	-18.5 -18.5	0.09 (4)		10.00	H- F	0 / 1034	0.23 (1)
L- K	0 / 963	-18.5 -18.5	0.20 (1)		10.00			
K- J	0 / 782	-18.5 -18.5	0.16 (1)		10.00			
J- I	0 / 782	-18.5 -18.5	0.16 (1)		10.00			
I- H	0 / 982	-18.5 -18.5	0.20 (1)		10.00			
H- G	0 / 0	-18.5 -18.5	0.10 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.32/1.00 (E-F:1) , BC=0.20/1.00 (H-I:1) , WB=0.30/1.00 (E-I:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.47 (F) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040162

REVIEWED

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ID:AgdClOSevh3uN4Xyl113N2zvjH4-McF0weq6y?6G9mIP1QLU4g9DjSSjH7k 499TV8zUnwY

10-6-0   10-7-8

4x6 ||

Scale = 1:65.3
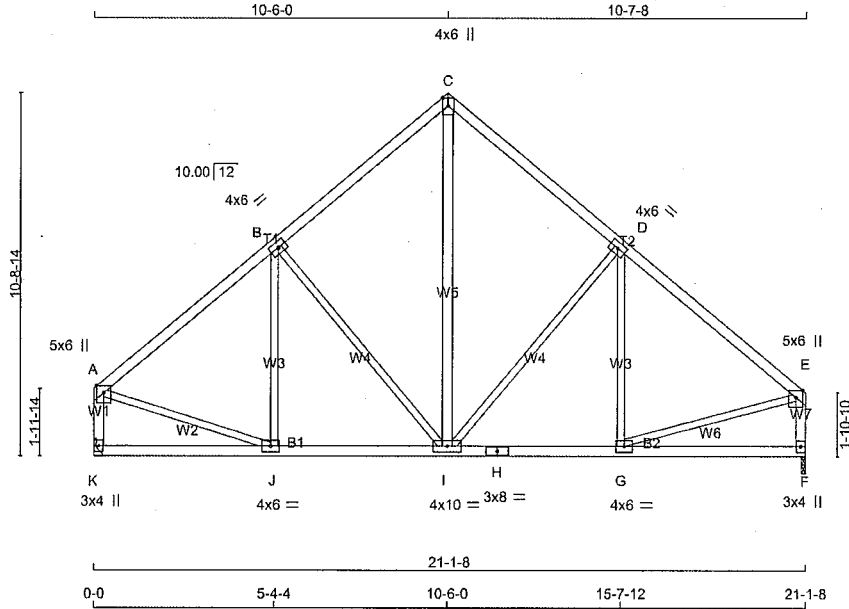
TOTAL WEIGHT = 101 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES   (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWWW-t	MT20	4.0	10.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1383	0	1383	0	0	MECHANICAL	
F	1383	0	1383	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
F	969	686 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS							WEBS		
	MAX. FACTORED	FACTORED						MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD LC1	MAX.	MAX.		MEMB.		FORCE	
	(LBS)	(PLF)	CSI (LC)	UNBRAC				(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO				
A- B	-1222 / 0	-112.4 -112.4	0.43 (1)	5.26	J- B			-193 / 38	0.13 (1)
B- C	-965 / 0	-112.4 -112.4	0.41 (1)	5.77	B- I			-416 / 0	0.55 (1)
C- D	-966 / 0	-112.4 -112.4	0.43 (1)	5.75	I- C			0 / 746	0.12 (1)
D- E	-1242 / 0	-112.4 -112.4	0.45 (1)	5.20	I- D			-440 / 0	0.58 (1)
K- A	-1342 / 0	0.0 0.0	0.15 (1)	7.00	G- D			-170 / 46	0.11 (1)
F- E	-1341 / 0	0.0 0.0	0.15 (1)	7.00	A- J			0 / 1019	0.23 (1)
					G- E			0 / 1028	0.23 (1)
K- J	0 / 0	-18.5 -18.5	0.12 (4)	10.00					
J- I	0 / 973	-18.5 -18.5	0.22 (1)	10.00					
I- H	0 / 989	-18.5 -18.5	0.22 (1)	10.00					
H- G	0 / 989	-18.5 -18.5	0.22 (1)	10.00					
G- F	0 / 0	-18.5 -18.5	0.13 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD		=		45.9	PSF

SPACING =   24.0   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.22/1.00 (G-I:1) ,
WB=0.58/1.00 (D-I:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

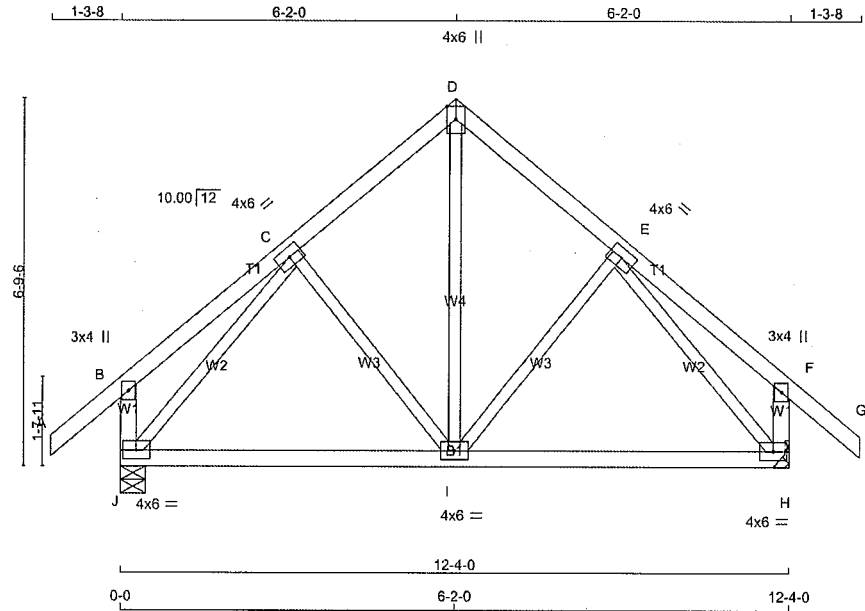JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.48 (E) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T43	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AqdClOSevh3uN4Xyl113N2zyjH4-qppO8 gkjJE7nwtcb7sjcuhSAsju0b77lpv02azUnwX

7-1-0   7-1-0

5x6 ||

Scale = 1:44.5

10.00 [12

4x6 //

4x6 //

7-6-8

5x8 =

5x8 =

1-7-11   1-7-11

4x6 || 6x7 =   6x7 || 6x7 =   4x6 ||

7-12 | 12-0-0 | 1-6-4

0-0 7-12   3-7-12   7-1-0   10-6-4   12-7-12   14-2-0

TOTAL WEIGHT = 2 X 77 = 153 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS
A- C	1	12	TOP
C- E	1	12	TOP
J- A	2	2	SIDE(173.7)
F- E	2	2	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS
J- F	2	2	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS
D- G	1	6	SIDE(172.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED
TO ONE SIDE THAT THE CORRESPONDING NAILING
PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE
SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	6013	0	6013	0	0	5-8	4-4
F	5396	0	5396	0	0	5-8	3-4

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	4207	3016 / 0	0 / 0	0 / 0	0 / 0	1192 / 0	0 / 0
F	3776	2707 / 0	0 / 0	0 / 0	0 / 0	1069 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC		MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH		FR-TO		
A- B	-5021 / 0	-112.4 -112.4	0.21 (1)	4.16		H- C	0 / 4625	0.57 (1)
B- C	-3885 / 0	-112.4 -112.4	0.15 (1)	4.69		H- D	-1424 / 0	0.36 (1)
C- D	-3886 / 0	-112.4 -112.4	0.15 (1)	4.69		G- D	0 / 1395	0.17 (1)
D- E	-5011 / 0	-112.4 -112.4	0.21 (1)	4.16		B- H	-1436 / 0	0.36 (1)
J- A	-4699 / 0	0.0 0.0	0.17 (1)	6.67		I- B	0 / 1411	0.17 (1)
F- E	-4691 / 0	0.0 0.0	0.17 (1)	6.68		A- I	0 / 4083	0.51 (1)
						G- E	0 / 4075	0.50 (1)
J- K	0 / 0	-18.5 -18.5	0.31 (1)	10.00				
K- L	0 / 0	-18.5 -18.5	0.31 (1)	10.00				
L- I	0 / 0	-18.5 -18.5	0.31 (1)	10.00				
I- M	0 / 3872	-18.5 -18.5	0.55 (1)	10.00				
M- N	0 / 3872	-18.5 -18.5	0.55 (1)	10.00				
N- H	0 / 3872	-18.5 -18.5	0.55 (1)	10.00				
H- O	0 / 3864	-18.5 -18.5	0.49 (1)	10.00				
O- G	0 / 3864	-18.5 -18.5	0.49 (1)	10.00				
G- P	0 / 0	-18.5 -18.5	0.39 (1)	10.00				
P- F	0 / 0	-18.5 -18.5	0.39 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	10-7-12	-954	-954	---	FRONT	VERT	TOTAL	---	C1
K	7-12	-957	-957	---	FRONT	VERT	TOTAL	---	C1
L	2-7-12	-954	-954	---	FRONT	VERT	TOTAL	---	C1
M	4-7-12	-954	-954	---	FRONT	VERT	TOTAL	---	C1
N	6-7-12	-954	-954	---	FRONT	VERT	TOTAL	---	C1
O	8-7-12	-954	-954	---	FRONT	VERT	TOTAL	---	C1
P	12-7-12	-954	-954	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   **24.0**   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.55/1.00 (H-I:1) ,
WB=0.57/1.00 (C-H:1) , SSI=0.52/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS: 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.44 (C) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T43	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:57 2024 Page 2
ID:AgdClOSevh3uN4Xyl113N2zyjH4-I?NnLKhMUcM_P4So8qNz95EdwG37l2NHXTeZa1zUnwW

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1+p	MT20	4.0	6.0		
G	BMWW-t	MT20	6.0	7.0	3.75	3.50
H	BMWWW+t	MT20	6.0	7.0		
I	BMWW-t	MT20	6.0	7.0	3.75	3.50
J	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
   TOUCHES EDGE OF CHORD.

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER

4/02/24

C. M. HEYENS
100505065

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040164

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T43G	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:03:58 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-mBx9Zfi FwUq0E1 iYvCiJmpcqXnUblQm7O76TzUnwV

TOTAL WEIGHT = 80 lb

[M]
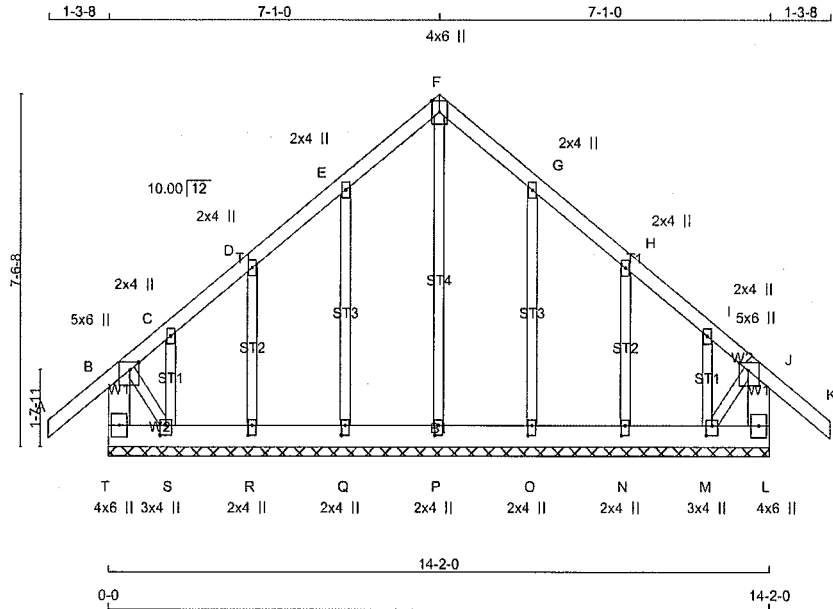
LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x6	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS				
	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
J	TMVW+p	MT20	5.0	6.0	2.00	2.25
L	BMV1+p	MT20	4.0	6.0		
M	BMWW1+t	MT20	3.0	4.0	2.50	1.50
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0	2.50	1.00
S	BMWW1+t	MT20	3.0	4.0	2.50	1.50
T	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MAX. FACTORED	FACTORED				MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
T- B	-367 / 0	0.0 0.0	0.02 (1)	7.81	P- F	-155 / 0	0.15 (1)
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	Q- E	-255 / 0	0.13 (1)
B- C	-97 / 0	-112.4 -112.4	0.15 (1)	6.25	R- D	-218 / 0	0.05 (1)
C- D	-29 / 0	-112.4 -112.4	0.05 (1)	6.25	S- C	-65 / 0	0.01 (1)
D- E	-29 / 0	-112.4 -112.4	0.07 (1)	6.25	O- G	-255 / 0	0.13 (1)
E- F	-42 / 0	-112.4 -112.4	0.07 (1)	6.25	N- H	-218 / 0	0.05 (1)
F- G	-42 / 0	-112.4 -112.4	0.07 (1)	6.25	M- I	-65 / 0	0.01 (1)
G- H	-29 / 0	-112.4 -112.4	0.07 (1)	6.25	B- S	0 / 45	0.01 (1)
H- I	-29 / 0	-112.4 -112.4	0.05 (1)	6.25	M- J	0 / 45	0.01 (1)
I- J	-97 / 0	-112.4 -112.4	0.15 (1)	6.25			
J- K	0 / 50	-112.4 -112.4	0.15 (1)	10.00			
L- J	-367 / 0	0.0 0.0	0.02 (1)	7.81			
T- S	0 / 0	-18.5 -18.5	0.00 (4)	10.00			
S- R	0 / 31	-18.5 -18.5	0.01 (4)	10.00			
R- Q	0 / 25	-18.5 -18.5	0.01 (4)	10.00			
Q- P	0 / 21	-18.5 -18.5	0.01 (1)	10.00			
P- O	0 / 21	-18.5 -18.5	0.01 (1)	10.00			
O- N	0 / 25	-18.5 -18.5	0.01 (1)	10.00			
N- M	0 / 31	-18.5 -18.5	0.01 (1)	10.00			
M- L	0 / 0	-18.5 -18.5	0.00 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

CSI: TC=0.15/1.00 (J-K:1) , BC=0.01/1.00 (Q-R:4) ,
WB=0.15/1.00 (F-P:1) , SSI=0.09/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

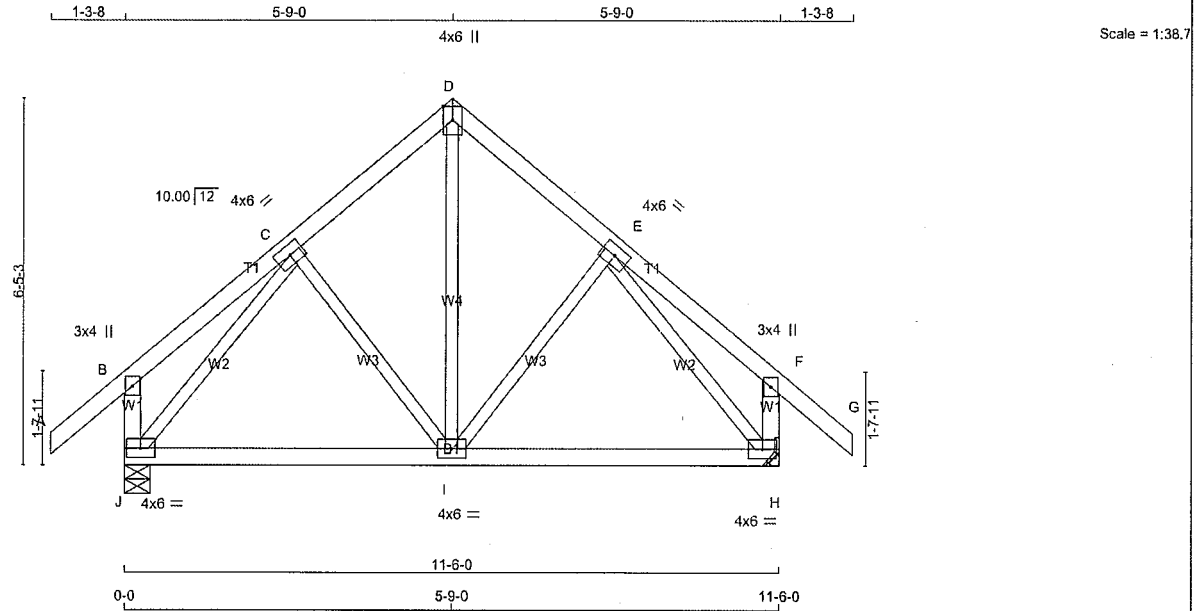JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.14 (G) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.		DRWG NO.
437026	T44	2	1	BAYVIEW WELLINGTON		
Tamarack Roof Truss, Burlington				TRUSS DESC.		

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-EOVXm?jc?EcheOcAGFQREWJz83pVD0ya n7gevzUnwU

1-3-8   6-2-0   6-2-0   1-3-8

4x6 ||

Scale = 1:40.7

10.00 ⌐12   4x6 //   4x6 \\

6-9-6

3x4 || 3x4 ||

1-7-11   1-7-11

TOTAL WEIGHT = 2 X 59 = 119 lb
[M][F]

12-4-0

0-0   6-2-0   12-4-0

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER		DESCR.
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
J - B	2x4	DRY	No.2		SPF
H - F	2x4	DRY	No.2		SPF
J - H	2x4	DRY	No.2		SPF

| ALL WEBS | 2x3 | DRY | No.2 | | SPF |
| EXCEPT | | | | | |

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	963	0	963	0	0	5-8	1-8
H	963	0	963	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM
BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	672	491 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	672	491 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

	CHORDS					WEBS		
	MAX. FACTORED	FACTORED					MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.		FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO			
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	I- D		0 / 392	0.09 (1)
B- C	0 / 25	-112.4 -112.4	0.17 (1)	10.00	I- E		-162 / 0	0.07 (1)
C- D	-544 / 0	-112.4 -112.4	0.13 (1)	6.25	C- I		-162 / 0	0.07 (1)
D- E	-544 / 0	-112.4 -112.4	0.13 (1)	6.25	J- C		-805 / 0	0.31 (1)
E- F	0 / 25	-112.4 -112.4	0.17 (1)	10.00	E- H		-805 / 0	0.31 (1)
F- G	0 / 50	-112.4 -112.4	0.15 (1)	10.00				
J- B	-288 / 0	0.0 0.0	0.03 (1)	7.81				
H- F	-288 / 0	0.0 0.0	0.03 (1)	7.81				
J- I	0 / 501	-18.5 -18.5	0.23 (4)	10.00				
I- H	0 / 501	-18.5 -18.5	0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   24.0   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.23/1.00 (H-I:4) ,
WB=0.31/1.00 (E-H:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

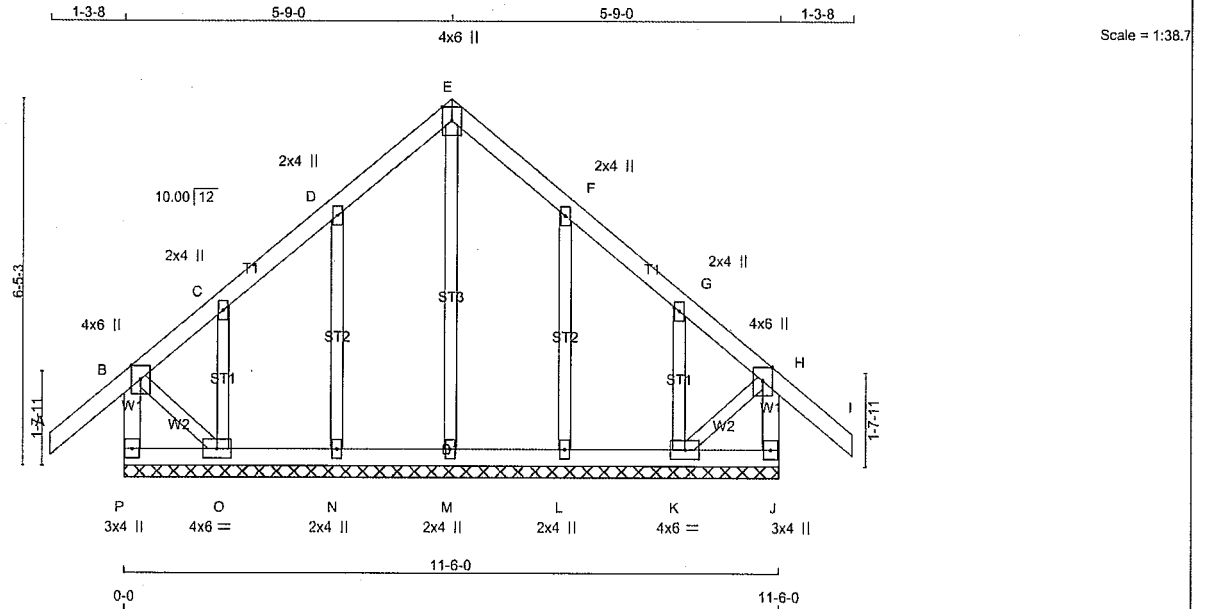PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (C) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040166

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T44G	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-la2vzLjEmXkYGXBNqzxgnks85TC1yWFjDRtDBMzUnwT

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4x6 ||
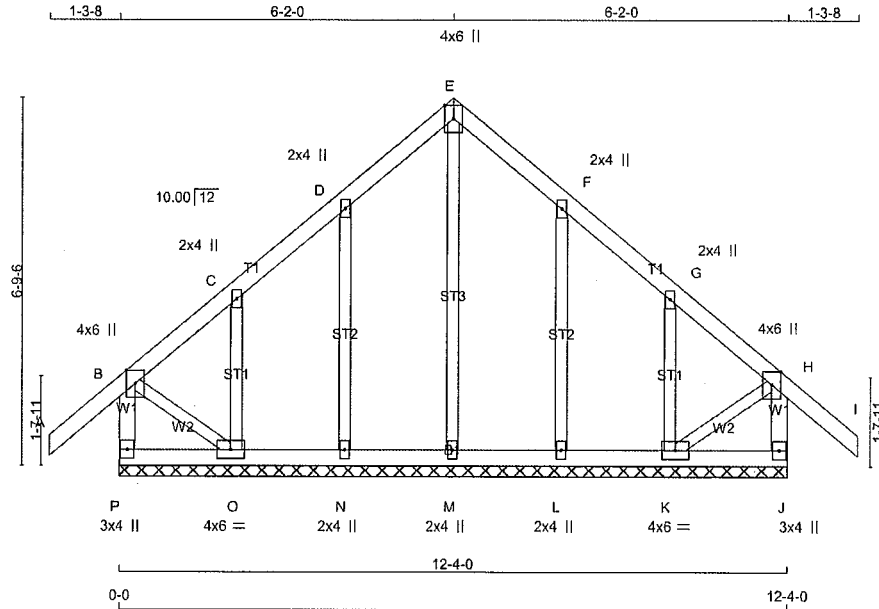
Scale = 1:40.7

TOTAL WEIGHT = 60 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS
2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
P- B	-298 / 0	0.0 0.0	0.03 (1)		7.81	M- E	-157 / 0	0.12 (1)
A- B	0 / 50	-112.4 -112.4	0.15 (1)		10.00	N- D	-247 / 0	0.10 (1)
B- C	-29 / 0	-112.4 -112.4	0.06 (1)		6.25	O- C	-246 / 0	0.05 (1)
C- D	-34 / 0	-112.4 -112.4	0.06 (1)		6.25	L- F	-247 / 0	0.10 (1)
D- E	-41 / 0	-112.4 -112.4	0.06 (1)		6.25	K- G	-246 / 0	0.05 (1)
E- F	-41 / 0	-112.4 -112.4	0.06 (1)		6.25	B- O	0 / 39	0.01 (1)
F- G	-34 / 0	-112.4 -112.4	0.06 (1)		6.25	K- H	0 / 39	0.01 (1)
G- H	-29 / 0	-112.4 -112.4	0.06 (1)		6.25			
H- I	0 / 50	-112.4 -112.4	0.15 (1)		10.00			
J- H	-298 / 0	0.0 0.0	0.03 (1)		7.81			
P- O	0 / 0	-18.5 -18.5	0.02 (4)		10.00			
O- N	0 / 26	-18.5 -18.5	0.02 (4)		10.00			
N- M	0 / 21	-18.5 -18.5	0.02 (4)		10.00			
M- L	0 / 21	-18.5 -18.5	0.02 (4)		10.00			
L- K	0 / 26	-18.5 -18.5	0.02 (4)		10.00			
K- J	0 / 0	-18.5 -18.5	0.02 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (K-L:4) ,
WB=0.12/1.00 (E-M:1) , SSI=0.09/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
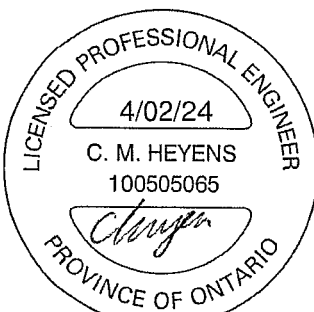RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

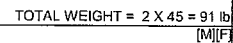JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T45	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-AmcHBhktXrsPthmZNgSvJxOJrtVQhwHtS5cnjozUnwS

1-3-8   5-9-0   5-9-0   1-3-8

4x6 ||

Scale = 1:38.7

10.00 [12 4x6 //

4x6 \\

6-5-3

C   T1   E   T1

3x4 || W4   3x4 ||

B   W2   W3   W3   W2   F

1-7-11   W1   W1   G   1-7-11

I

J 4x6 =   4x6 =   H
             4x6 =

11-6-0

0-0   5-9-0   11-6-0

TOTAL WEIGHT = 5 X 56 = 280 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	909	0	909	0	0	5-8	1-8
H	909	0	909	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM
BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	634	463 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
H	634	463 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MAX. FACTORED	FACTORED					MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD LC1	MAX.	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	I- D	0 / 355	0.08 (1)
B- C	0 / 23	-112.4 -112.4	0.14 (1)	10.00	I- E	-143 / 0	0.05 (1)
C- D	-499 / 0	-112.4 -112.4	0.11 (1)	6.25	C- I	-143 / 0	0.05 (1)
D- E	-499 / 0	-112.4 -112.4	0.11 (1)	6.25	J- C	-743 / 0	0.26 (1)
E- F	0 / 23	-112.4 -112.4	0.14 (1)	10.00	E- H	-743 / 0	0.26 (1)
F- G	0 / 50	-112.4 -112.4	0.15 (1)	10.00			
J- B	-279 / 0	0.0 0.0	0.03 (1)	7.81			
H- F	-279 / 0	0.0 0.0	0.03 (1)	7.81			
J- I	0 / 455	-18.5 -18.5	0.20 (4)	10.00			
I- H	0 / 455	-18.5 -18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (F-G:1) , BC=0.20/1.00 (H-I:4) ,
WB=0.26/1.00 (C-J:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (E) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T45G	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-79k2cNm73S777?vyV5UNPMUfLhEp9t79vP5ungzUnwQ

Scale = 1:38.7

TOTAL WEIGHT = 56 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS
    2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		Edge
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0		Edge
H	TMVW+p	MT20	4.0	6.0		Edge
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
  TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	LC1 TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO			
P- B	-340 / 0	0.0	0.0	0.04 (1)	7.81	M- E	-166 / 0	0.11 (1)
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	N- D	-271 / 0	0.09 (1)
B- C	-65 / 0	-112.4	-112.4	0.15 (1)	6.25	O- C	-124 / 0	0.02 (1)
C- D	-9 / 0	-112.4	-112.4	0.07 (1)	10.00	L- F	-271 / 0	0.09 (1)
D- E	-31 / 0	-112.4	-112.4	0.07 (1)	6.25	K- G	-124 / 0	0.02 (1)
E- F	-31 / 0	-112.4	-112.4	0.07 (1)	6.25	B- O	0 / 27	0.01 (1)
F- G	-9 / 0	-112.4	-112.4	0.07 (1)	10.00	K- H	0 / 27	0.01 (1)
G- H	-65 / 0	-112.4	-112.4	0.15 (1)	6.25			
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00			
J- H	-340 / 0	0.0	0.0	0.04 (1)	7.81			
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 17	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 11	-18.5	-18.5	0.02 (4)	10.00			
M- L	0 / 11	-18.5	-18.5	0.02 (4)	10.00			
L- K	0 / 17	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD		=		45.9	PSF

SPACING =   24.0   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

CSI: TC=0.15/1.00 (H-I:1) , BC=0.02/1.00 (K-L:4) ,
WB=0.11/1.00 (E-M:1) , SSI=0.09/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (H) (INPUT = 0.90)
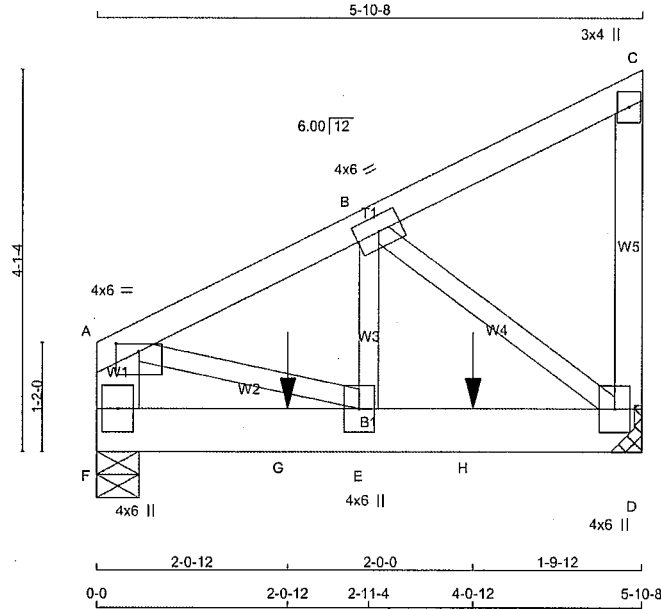JSI METAL= 0.14 (F) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040169

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T46	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-bLIQpjnlqmF l9U83p?cxa0pT4VMuJeJ83rRK7zUnwP

7-5-8   7-5-8

4x6 =

Scale = 1:25.8

6.00 [12

2x4 =   2x4 =

TOTAL WEIGHT = 2 X 45 = 91 lb
[M][F]

14-11-0

0-0   7-5-8   14-11-0

H2.5T   H2.5T

4x6 =   4x6 =

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	4.0	6.0		Edge
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	6.0		
D	TMW+w	MT20	2.0	4.0		
E	TMB1-I	MT20	4.0	6.0		Edge
F	BMWWW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	977	0	977	72	-430	3-0	1-8
E	977	0	977	0	-377	3-0	1-8

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 430 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 377 LBS FACTORED UPLIFT

PROVIDE FOR 72 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	684	484 / 0	0 / 0	0 / 0	0 / -435	200 / 0	0 / 0
E	684	484 / 0	0 / 0	0 / 0	0 / -398	200 / 0	0 / 0

HORIZONTAL REACTIONS

| A | --- | 0 / 0 | 0 / 0 | 0 / 0 | 51 / -51 | 0 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS					WEBS		
MEMB. MAX. FACTORED	FORCE (LBS)	FACTORED VERT.LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- H	-1712 / 724	-112.4 -112.4	0.17 (7)	5.07	F- C	-410 / 687	0.15 (1)
H- B	-1602 / 663	-112.4 -112.4	0.19 (7)	5.04	F- D	-537 / 243	0.15 (1)
B- C	-1132 / 532	-112.4 -112.4	0.18 (1)	5.77	B- F	-531 / 264	0.15 (1)
C- D	-1132 / 547	-112.4 -112.4	0.18 (1)	5.77	G- H	-188 / 149	0.00 (1)
D- J	-1602 / 614	-112.4 -112.4	0.19 (1)	5.04	I- J	-166 / 149	0.00 (1)
J- E	-1712 / 688	-112.4 -112.4	0.16 (7)	5.07			
A- G	-625 / 1453	-18.5 -18.5	0.26 (1)	6.25			
G- F	-625 / 1453	-18.5 -18.5	0.38 (1)	6.25			
F- I	-527 / 1453	-18.5 -18.5	0.38 (1)	6.25			
I- E	-527 / 1453	-18.5 -18.5	0.26 (1)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT
(20-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0
PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD			=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.19/1.00 (B-H:7) , BC=0.38/1.00 (F-I:1) ,
WB=0.15/1.00 (C-F:1) , SSI=0.17/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040170

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437026	T46A	1	1	TRUSS DESC.	

BAYVIEW WELLINGTON

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-3Yso13nNb4NrMJ3KcWXrUnZvPUiddlHSNja sZzUnwO

7-5-8   5-2-8

4x6 =

Scale = 1:25.4

6.00 [12]

2x4 ≈

4-0-15

T1

T2

B

C

W2

W1

W3

4x6 ≈
D

W4

A I

3x4 = H

H2.5T

4-3

B1

G

5x6 =

W5

3-8

F

E

6x16 =

H2.5T

13-6-0

0-0   7-5-8   13-6-0
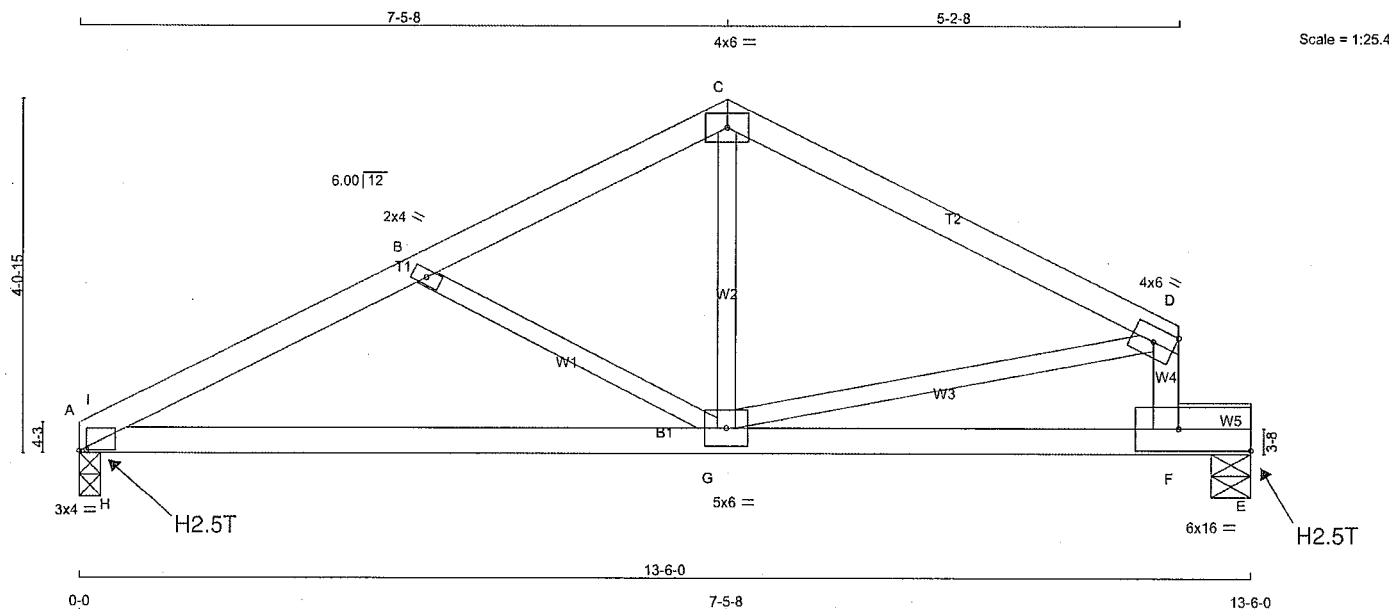
TOTAL WEIGHT = 44 lb [M]

LUMBER
N. L. G. A. RULES
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	3.0	4.0	0.25	1.00
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		Edge
F	BMVW-t	MT20	6.0	16.0	3.00	Edge
G	BMWWW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
  TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	881	0	881	0	0	3-0	1-8
E	793	0	793	0	0	5-8	1-8

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	617	437 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
E	557	386 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM TO		MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-I	-1467 / 0	-112.4	-112.4	0.09 (4)	5.37	B-G	-504 / 0	0.15 (1)
I-B	-1422 / 0	-112.4	-112.4	0.23 (1)	5.21	G-C	0 / 396	0.09 (1)
B-C	-970 / 0	-112.4	-112.4	0.22 (1)	6.04	G-D	0 / 859	0.19 (1)
C-D	-938 / 0	-112.4	-112.4	0.57 (1)	5.48	H-I	-120 / 117	0.00 (1)
F-D	-896 / 0	0.0	0.0	0.09 (1)	7.81			
A-H	0 / 1286	-18.5	-18.5	0.29 (1)	10.00			
H-G	0 / 1286	-18.5	-18.5	0.53 (1)	10.00			
G-F	0 / 0	-18.5	-18.5	0.89 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.37 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL)= L/ 810 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL)= L/ 439 (0.37")

CSI: TC=0.57/1.00 (C-D:1) , BC=0.89/1.00 (F-G:1) ,
WB=0.19/1.00 (D-G:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
  COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI) MAX MIN	SHEAR (PLI) MAX MIN	SECTION (PLI) MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.42 (A) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR24040171

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T46G	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

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7-5-8   7-5-8

4x6 =

6.00 [12

4-0-15

LSTA9 @ 48" O/C ALONG BOTTOM CHORD

Scale = 1:25.5

M 3x4 =   L 2x4 || K 2x4 || J 2x4 || I 2x4 || H 2x4 || O 3x4 =

14-11-0

0-0   14-11-0

TOTAL WEIGHT = 46 lb   [M]
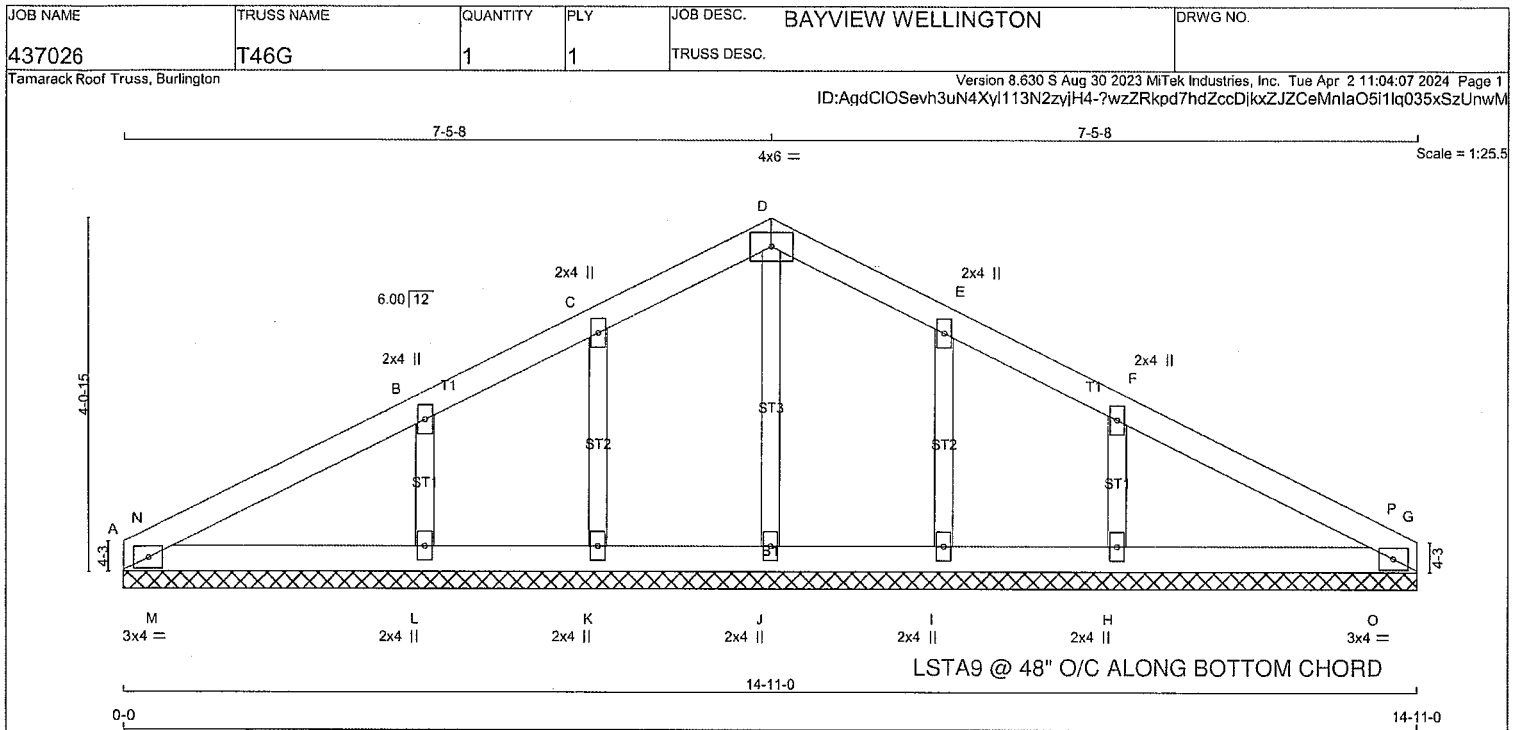
LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	3.0	4.0		
B, C, E, F						
B	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	6.0		
G	TMB1-I	MT20	3.0	4.0		
H, I, J, K, L						
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 72 LBS FACTORED HORIZONTAL REACTION AT JOINT A

HORIZONTAL REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	---	0 / 0	0 / 0	0 / 0	51 / -51	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	LC1 MAX. TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- N	-115 / 0	-112.4	-112.4	0.02 (12)	6.25	J- D	-165 / 0	0.04 (1)
N- B	-56 / 38	-112.4	-112.4	0.13 (1)	6.25	K- C	-222 / 99	0.04 (1)
B- C	-66 / 81	-112.4	-112.4	0.13 (1)	6.25	L- B	-342 / 144	0.05 (1)
C- D	-54 / 114	-112.4	-112.4	0.06 (1)	6.25	I- E	-222 / 99	0.04 (1)
D- E	-52 / 104	-112.4	-112.4	0.06 (1)	6.25	H- F	-342 / 144	0.05 (1)
E- F	-60 / 52	-112.4	-112.4	0.13 (1)	6.25	M- N	-184 / 122	0.00 (1)
F- P	-50 / 9	-112.4	-112.4	0.13 (1)	6.25	O- P	-183 / 121	0.00 (1)
P- G	-85 / 0	-112.4	-112.4	0.02 (12)	6.25			
A- M	0 / 67	-18.5	-18.5	0.10 (1)	10.00			
M- L	0 / 67	-18.5	-18.5	0.10 (1)	10.00			
L- K	0 / 70	-18.5	-18.5	0.07 (1)	10.00			
K- J	0 / 71	-18.5	-18.5	0.02 (1)	10.00			
J- I	0 / 71	-18.5	-18.5	0.02 (1)	10.00			
I- H	0 / 68	-18.5	-18.5	0.07 (1)	10.00			
H- O	0 / 63	-18.5	-18.5	0.10 (1)	10.00			
O- G	0 / 63	-18.5	-18.5	0.10 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {20-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (B-N:1) , BC=0.10/1.00 (A-M:1) , WB=0.05/1.00 (F-H:1) , SSI=0.14/1.00 (F-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
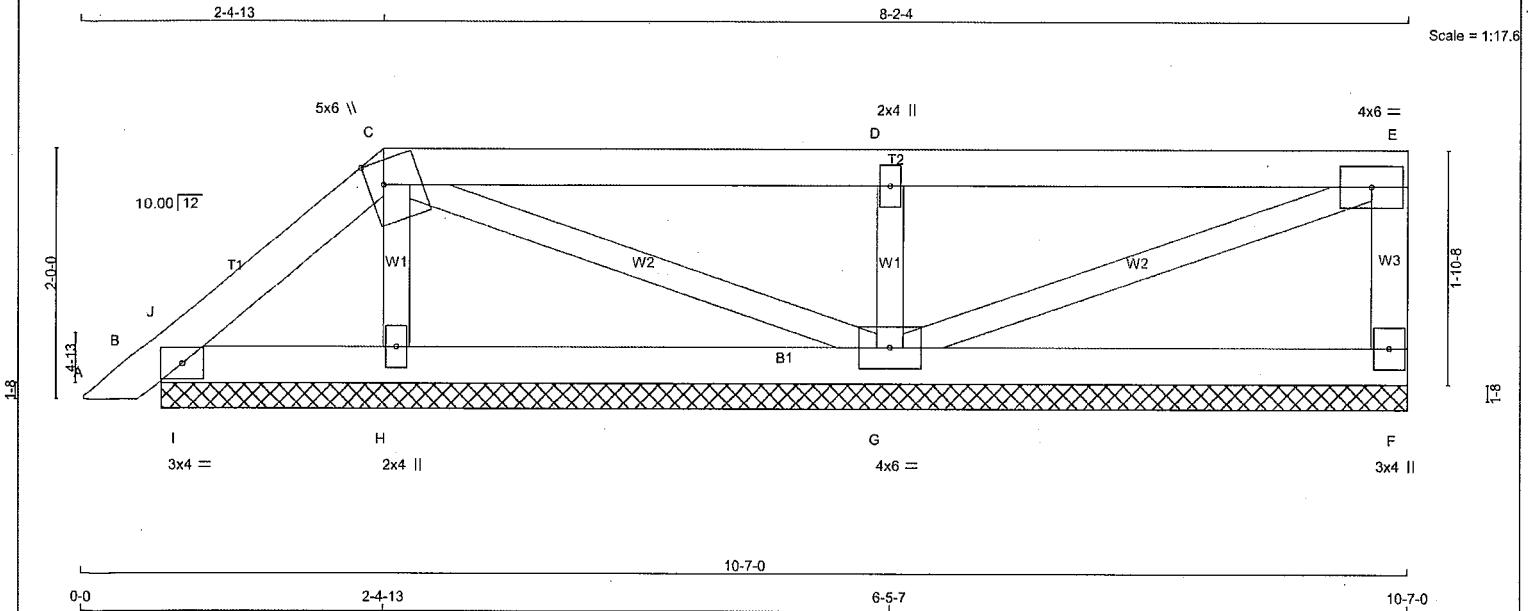JSI METAL= 0.14 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
437026	T47	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-T7Xxf4qpGu IQDmovIe4Y6QBNQiuAq3Fu3gpfTuzUnwL

TOTAL WEIGHT = 44 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - A	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.75
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMVWW-t	MT20	8.0	16.0	5.00	5.50
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	10.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1187	0	1187	0	0	1-8	1-8
D	1136	0	1136	0	0	5-8	1-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	830	598 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0
D	795	568 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- A	-1122 / 0	0.0	0.0 0.15 (1)	7.46	A- F	0 / 1756	0.43 (1)
A- H	-1597 / 0	-112.4 -112.4 0.42 (1)		5.60	F- B	-1200 / 0	0.20 (1)
H- I	-1597 / 0	-112.4 -112.4 0.42 (1)		5.60	F- C	0 / 1756	0.43 (1)
I- B	-1597 / 0	-112.4 -112.4 0.42 (1)		5.60			
B- J	-1597 / 0	-112.4 -112.4 0.71 (1)		4.90			
J- K	-1597 / 0	-112.4 -112.4 0.71 (1)		4.90			
K- C	-1597 / 0	-112.4 -112.4 0.71 (1)		4.90			
C- D	0 / 0	-112.4 -112.4 0.42 (1)		10.00			
E- C	0 / 140	0.0 0.0 0.02 (1)		10.00			
G- L	0 / 0	-18.5 -18.5 0.20 (4)		10.00			
L- M	0 / 0	-18.5 -18.5 0.20 (4)		10.00			
M- F	0 / 0	-18.5 -18.5 0.20 (4)		10.00			
F- N	0 / 0	-18.5 -18.5 0.20 (4)		10.00			
N- O	0 / 0	-18.5 -18.5 0.20 (4)		10.00			
O- E	0 / 0	-18.5 -18.5 0.20 (4)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-108	-108	---	FRONT	VERT	TOTAL	---	C1
C	9-4-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
E	9-7-8	-46	-46	---	FRONT	VERT	TOTAL	---	C1
H	2-0-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
I	4-0-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
J	6-0-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
K	8-0-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
L	2-0-12	-40	-40	---	FRONT	VERT	TOTAL	---	C1
M	4-0-12	-40	-40	---	FRONT	VERT	TOTAL	---	C1
N	6-0-12	-40	-40	---	FRONT	VERT	TOTAL	---	C1
O	8-0-12	-40	-40	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 863 (0.13")

CSI: TC=0.71/1.00 (B-C:1) , BC=0.20/1.00 (E-F:4) , WB=0.43/1.00 (A-F:1), SSI=0.46/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (A) (INPUT = 0.90)
JSI METAL= 0.36 (A) (INPUT = 0.95)

4/02/24
C. M. HEYENS
100505065

LICENSED PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040173

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437026	T48	1	2	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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5-10-8

3x4 ||

Scale = 1:23.7

6.00 [12

4x6 ≈

4x6 =

4x6 =

4x6 ||

4x6 ||

4x6 ||

2-0-12   2-0-0   1-9-12

0-0   2-0-12   2-11-4   4-0-12   5-10-8

TOTAL WEIGHT = 2 X 29 = 58 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS	#ROWS	SURFACE	LOAD(PLF)
		SPACING (IN)	

TOP CHORDS : (0.122"X3") SPIRAL NAILS
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP

BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS
| F- D | 2 | 12 | SIDE(0.0) |

WEBS : (0.122"X3") SPIRAL NAILS
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED
TO ONE SIDE THAT THE CORRESPONDING NAILING
PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE
SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	1370	0	1370	0	0	MECHANICAL	
F	1289	0	1289	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM
BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	955	702 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
F	899	660 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	-1401 / 0	-112.4 -112.4	0.08 (1)	6.25		E- B	0 / 1206	0.15 (1)
B- C	-14 / 0	-112.4 -112.4	0.07 (1)	6.25		B- D	-1592 / 0	0.19 (1)
D- C	-134 / 0	0.0 0.0	0.02 (1)	7.81		A- E	0 / 1308	0.16 (1)
F- A	-1100 / 0	0.0 0.0	0.04 (1)	7.81				
F- G	0 / 0	-18.5 -18.5	0.13 (1)	10.00				
G- E	0 / 0	-18.5 -18.5	0.13 (1)	10.00				
E- H	0 / 1265	-18.5 -18.5	0.25 (1)	10.00				
H- D	0 / 1265	-18.5 -18.5	0.25 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-658	-658	—	BACK	VERT	TOTAL	---	C1
H	4-0-12	-658	-658	—	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		45.9	PSF

SPACING =   **24.0**   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.25/1.00 (D-E:1) ,
WB=0.19/1.00 (B-D:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
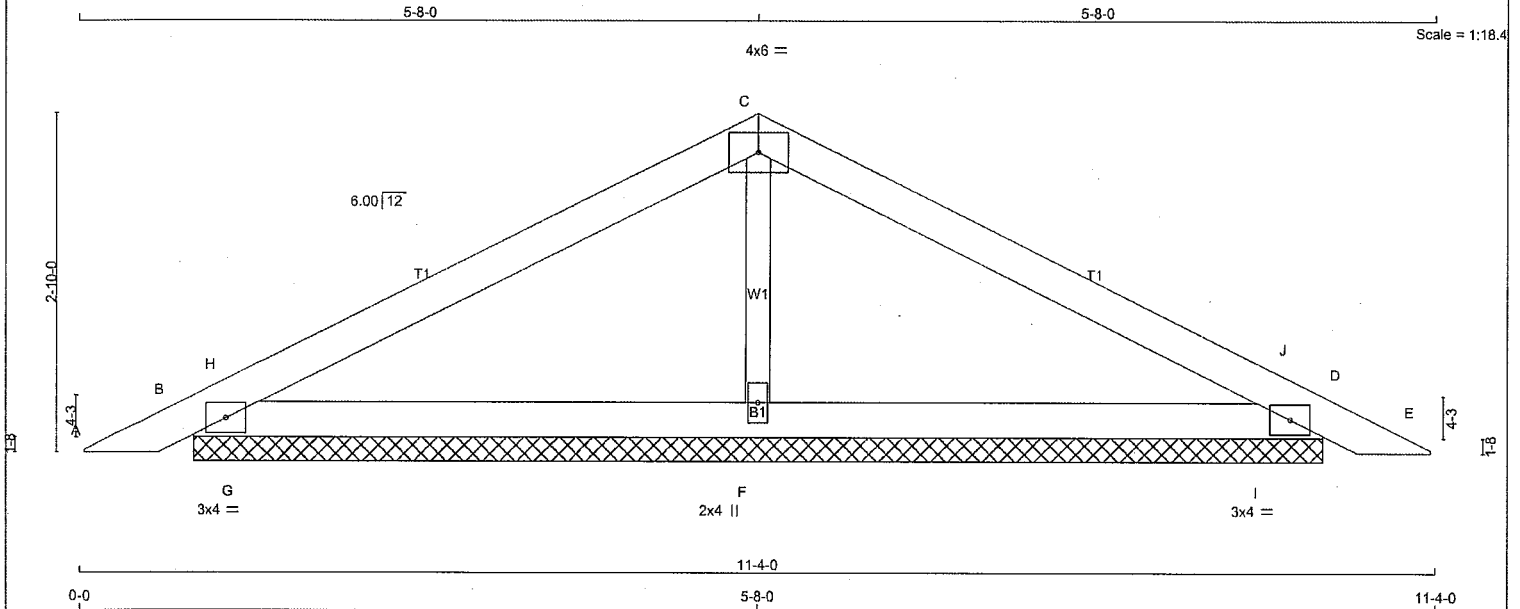JSI METAL= 0.29 (D) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T48	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:04:09 2024 Page 2
ID:AgdCIOSevh3uN4Xyl113N2zyjH4-xJ5JsQqufltHrwN5rMbnedki35EZZZK2IKYC?KzUnwK

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040174

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	T49	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:04:11 2024 Page 1
ID:AgdCIOSevh3uN4Xyl113N2zyjH4-uhD3H6s8Bv7 4EXUzndFj2pzBvyA1U3Lle1J4DzUnwl

1-3-8 | 1-10-0 | 2-0-12 | 1-9-0 | 7-4

Scale = 1:13.8

6.00 ⌐12

TOTAL WEIGHT = 21 lb

1-10-12 | 2-0-0 | 1-9-0 | 7-4

0-0 | 1-10-0 1-10-12 | 3-10-12 | 5-7-12 | 6-3-0

H2.5T

4x6 = , 3x4 =, 4x6 =

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	6.0		
D	TMVW-l	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	372	0	372	0	-202	3-0	1-8
B	567	0	567	55	-241	3-0	1-8

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 202 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 241 LBS FACTORED UPLIFT

PROVIDE FOR 55 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	262	179 / -17	0 / 0	0 / 0	0 / -197	83 / 0	0 / 0
B	395	292 / 0	0 / 0	0 / 0	0 / -239	103 / 0	0 / 0

HORIZONTAL REACTIONS

B	---	0 / 0	0 / 0	0 / 0	39 / -2	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (19)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX.	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	0 / 33	-112.4 -112.4	0.16 (1)		10.00	F- C	-111 / 42	0.02 (12)
B- H	-611 / 317	-112.4 -112.4	0.17 (13)		6.25	F- D	-292 / 448	0.11 (1)
H- C	-510 / 314	-112.4 -112.4	0.07 (7)		6.25	G- H	-10 / 94	0.00 (1)
C- I	-437 / 298	-112.4 -112.4	0.42 (1)		6.25			
I- J	-437 / 298	-112.4 -112.4	0.42 (1)		6.25			
J- D	-437 / 298	-112.4 -112.4	0.42 (1)		6.25			
E- D	-346 / 152	0.0 0.0	0.04 (1)		7.81			
B- G	-267 / 446	-18.5 -18.5	0.11 (1)		6.25			
G- F	-267 / 446	-18.5 -18.5	0.11 (1)		6.25			
F- K	-5 / 14	-18.5 -18.5	0.08 (12)		10.00			
K- L	-5 / 14	-18.5 -18.5	0.08 (12)		10.00			
L- E	-5 / 14	-18.5 -18.5	0.08 (12)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-0	-2	-2	225	FRONT	VERT	TOTAL	---	C1
F	1-10-12	8	-3	23	FRONT	VERT	TOTAL	---	C1
I	3-10-12	1	1	78	FRONT	VERT	TOTAL	---	C1
J	5-7-12	1	-2	65	FRONT	VERT	TOTAL	---	C1
K	3-10-12	8	-3	23	FRONT	VERT	TOTAL	---	C1
L	5-7-12	8	-3	26	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {7.5} PSF AT {20-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.11/1.00 (B-G:1) , WB=0.11/1.00 (D-F:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
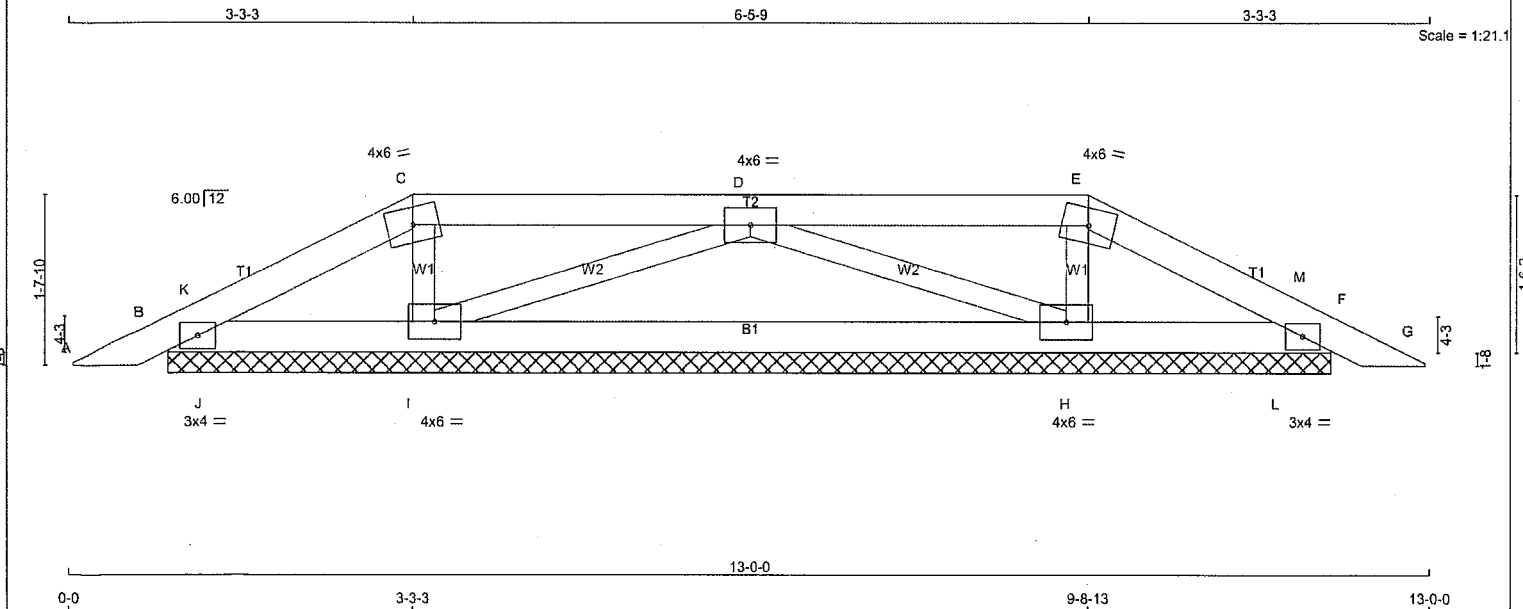JSI METAL= 0.17 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	PB1	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:46 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-tLaxR2qqCoQ2OAX9UQcJn7oTBexoc2RqCwxJaezUnxd

2-4-13　　8-2-4

Scale = 1:17.6

10.00 [12

5x6 \\   C   2x4 || D   4x6 =   E

T2

T1   W1   W2   W1   W2   W3

J   B1

B

I   H   G   F
3x4 =   2x4 || 4x6 =   3x4 ||

10-7-0

0-0   2-4-13   6-5-7   10-7-0

TOTAL WEIGHT = 2 X 34 = 68 lb   [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMV1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	205	0	205	0	0	9-11-7	1-8
B	206	0	206	0	0	9-11-7	1-8
H	277	0	277	0	0	9-11-7	1-8
G	674	0	674	0	0	9-11-7	1-8

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	144	101 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
B	142	114 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
H	196	127 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
G	472	339 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CSI (LC)	MAX.	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO				FR-TO		
A - B	0 / 17	-112.4 -112.4	0.02 (1)		10.00	H - C	-198 / 0	0.03 (1)
B - J	-41 / 0	-112.4 -112.4	0.01 (1)		6.25	C - G	-37 / 0	0.01 (1)
J - C	-64 / 0	-112.4 -112.4	0.04 (1)		6.25	G - D	-575 / 0	0.08 (1)
C - D	0 / 2	-112.4 -112.4	0.32 (1)		10.00	G - E	-2 / 0	0.00 (1)
D - E	0 / 2	-112.4 -112.4	0.32 (1)		10.00	I - J	-130 / 0	0.00 (1)
F - E	-175 / 0	0.0 0.0	0.02 (1)		7.81			
B - I	0 / 46	-18.5 -18.5	0.05 (1)		10.00			
I - H	0 / 46	-18.5 -18.5	0.05 (1)		10.00			
H - G	0 / 33	-18.5 -18.5	0.08 (4)		10.00			
G - F	0 / 0	-18.5 -18.5	0.08 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-D:1) , BC=0.08/1.00 (G-H:4) , WB=0.08/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	PB2	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:47 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-LX8JfOrSz6Yv?J6L287YKLKfT2FzLV2zRahs74zUnxc

Scale = 1:18.1

TOTAL WEIGHT = 2 X 36 = 71 lb

10.00 √12

3x4 \\ — C
4x6 = — D
3x4 || — E

T2

T1, W1, W2, W2, W3

B, I, A

H (3x4 =) G (4x6 =) F (4x6 =)

B1

3-7-3   6-11-13
3-0-0
4-13
2-10-8
1-8
10-7-0
0-0   3-7-3   10-7-0

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMWW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	439	0	439	0	0	9-11-7	1-8
B	234	0	234	0	0	9-11-7	1-8
G	690	0	690	0	0	9-11-7	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	306	225 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
B	160	137 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
G	488	319 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

	CHORDS MAX. FACTORED	FACTORED					WEBS MAX. FACTORED	
MEMB. FR-TO	FORCE (LBS)	VERT. LOAD (PLF) FROM TO	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-112.4 -112.4	0.02 (1)		10.00	G- C	-273 / 0	0.05 (1)
B- I	0 / 130	-112.4 -112.4	0.09 (1)		10.00	G- D	-408 / 0	0.12 (1)
I- C	-27 / 0	-112.4 -112.4	0.11 (1)		6.25	D- F	-401 / 0	0.12 (1)
C- D	0 / 4	-112.4 -112.4	0.22 (1)		10.00	H- I	-381 / 0	0.00 (1)
D- E	0 / 0	-112.4 -112.4	0.22 (1)		10.00			
F- E	-153 / 0	0.0 0.0	0.02 (1)		7.81			
B- H	-3 / 12	-18.5 -18.5	0.12 (1)		10.00			
H- G	-3 / 12	-18.5 -18.5	0.20 (4)		10.00			
G- F	0 / 324	-18.5 -18.5	0.21 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-D:1) , BC=0.21/1.00 (F-G:4) ,
WB=0.12/1.00 (D-G:1) , SSI=0.27/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS: 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
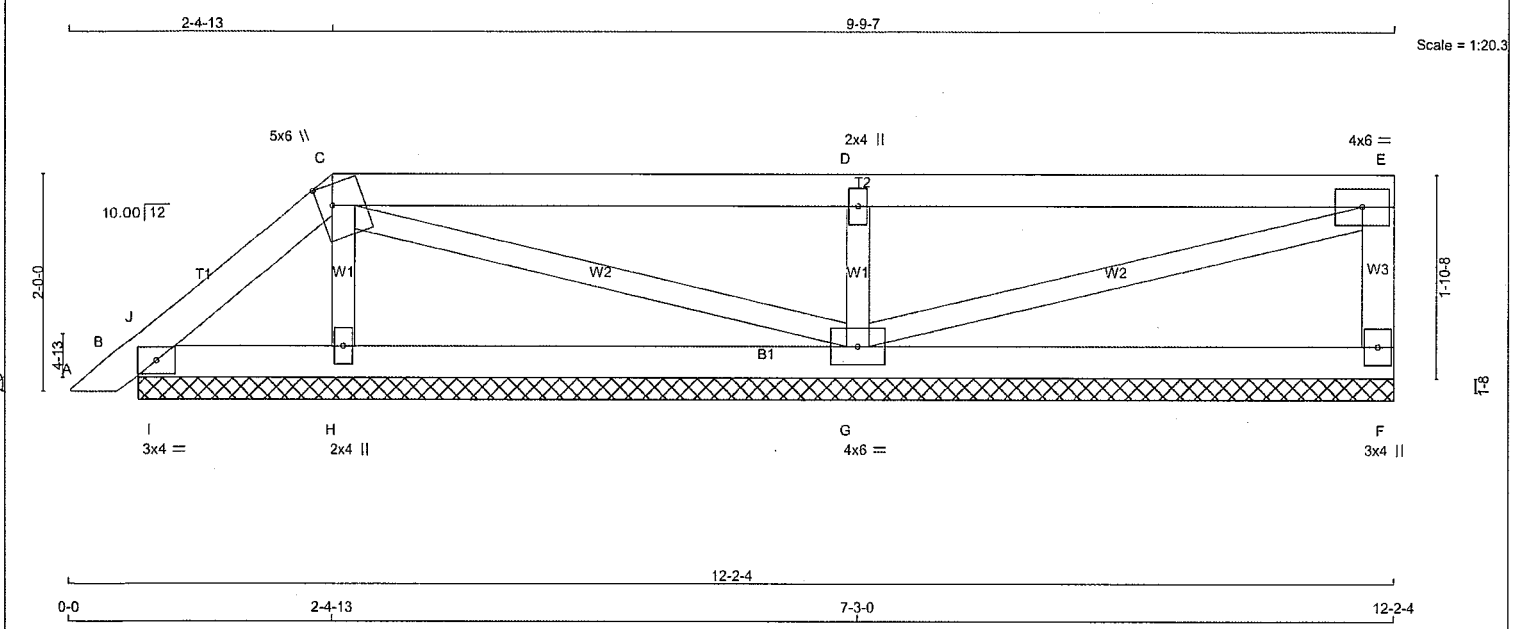JSI METAL= 0.08 (G) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	PB3	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:49 2024 Page 1
ID:AgdClOSevh3uN4XyI113N2zyjH4-HwG343siUjodFdFjAY90PmQusswepPwGuuAzBzzUnxa

Scale = 1:21.9

4-7-13   5-11-4

3x4 \\   4x6 =

10.00 ⌐12

TOTAL WEIGHT = 2 X 36 = 73 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TMVW-l	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	415	0	415	0	0	9-11-7	1-8
B	376	0	376	0	0	9-11-7	1-8
F	571	0	571	0	0	9-11-7	1-8

UNFACTORED REACTIONS

	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	289	214 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
B	260	203 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0
F	404	263 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

	CHORDS					WEBS		
	MAX. FACTORED	FACTORED					MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	0 / 17	-112.4 -112.4	0.02 (1)		10.00	F- C	-450 / 0	0.10 (1)
B- H	0 / 133	-112.4 -112.4	0.14 (1)		10.00	F- D	0 / 85	0.02 (1)
H- C	-128 / 0	-112.4 -112.4	0.22 (1)		6.25	G- H	-581 / 0	0.00 (1)
C- D	-73 / 0	-112.4 -112.4	0.68 (1)		6.25			
E- D	-377 / 0	0.0 0.0	0.08 (1)		7.81			
B- G	0 / 87	-18.5 -18.5	0.20 (1)		10.00			
G- F	0 / 87	-18.5 -18.5	0.20 (1)		10.00			
F- E	0 / 0	-18.5 -18.5	0.15 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.68/1.00 (C-D:1) , BC=0.20/1.00 (F-G:1) , WB=0.10/1.00 (C-F:1) , SSI=0.44/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

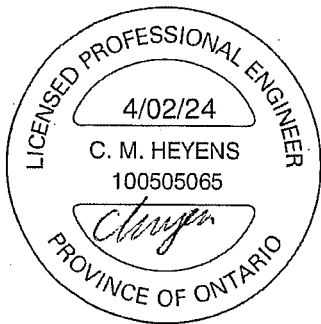TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

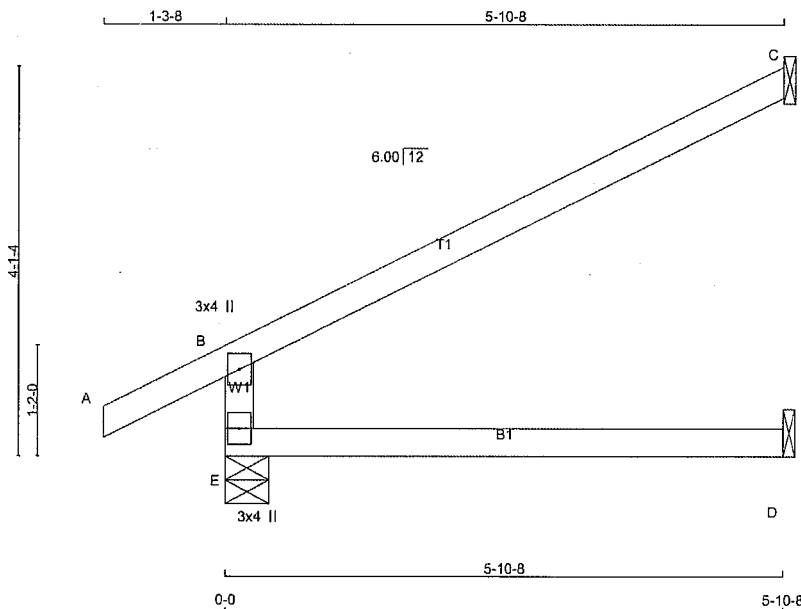JSI GRIP= 0.53 (C) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	PB4	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-I6qSHPtKF1wUsnqwjGgFxzy9XFFYYt?Q7YvWjPzUnxZ

5-8-0   5-8-0

Scale = 1:18.4

4x6 =
6.00 ⌐12
2-10-0

TOTAL WEIGHT = 5 X 27 = 134 lb

11-4-0

0-0   5-8-0   11-4-0

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	6.0		
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	450	0	450	0	0	9-5-6	1-8
D	450	0	450	0	0	9-5-6	1-8
F	528	0	528	0	0	9-5-6	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	312	237 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
D	312	237 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
F	372	250 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MAX. FACTORED		FACTORED					MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD LC1	MAX.	MAX.		MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO			
A- B	0 / 20	-112.4 -112.4	0.06 (1)	10.00		F- C	-288 / 0	0.05 (1)
B- H	-59 / 0	-112.4 -112.4	0.13 (1)	6.25		G- H	-507 / 0	0.00 (1)
H- C	-181 / 0	-112.4 -112.4	0.30 (1)	6.25		I- J	-507 / 0	0.00 (1)
C- J	-181 / 0	-112.4 -112.4	0.30 (1)	6.25				
J- D	-59 / 0	-112.4 -112.4	0.13 (1)	6.25				
D- E	0 / 20	-112.4 -112.4	0.06 (1)	10.00				
B- G	0 / 153	-18.5 -18.5	0.28 (1)	10.00				
G- F	0 / 153	-18.5 -18.5	0.28 (1)	10.00				
F- I	0 / 153	-18.5 -18.5	0.28 (1)	10.00				
I- D	0 / 153	-18.5 -18.5	0.28 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING =   24.0   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-H:1) , BC=0.28/1.00 (B-G:1) , WB=0.05/1.00 (C-F:1) , SSI=0.37/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION			
	(PSI)	(PLI)	(PLI)			
	MAX MIN	MAX MIN	MAX MIN			
MT20	650 371	1747 788	1987 1873			

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	PB5	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-ElOqVluy0K2LUxP6HzCUUBVMzfdvHIsZMCf4GszUnxY

3-3-3   6-5-9   3-3-3

Scale = 1:21.1

4x6 =   4x6 =   4x6 =
C   D   E
6.00 [12
1-7-10
T1   T2
K   W1   W2   W2   W1   T1   M
B   F   1-6-2
B1   G
4-3   4-3
1-8   1-8

J   I   H   L
3x4 =   4x6 =   4x6 =   3x4 =

13-0-0

0-0   3-3-3   9-8-13   13-0-0

TOTAL WEIGHT = 35 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES   (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMB1-I	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW1-t	MT20	4.0	6.0		

NOTES·   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	173	0	173	0	0	11-1-6	1-8
F	173	0	173	0	0	11-1-6	1-8
I	650	0	650	0	0	11-1-6	1-8
H	650	0	650	0	0	11-1-6	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	118	100 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
F	118	100 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
I	456	316 / 0	0 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	456	316 / 0	0 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MEMB. MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT.LOAD LC1 (PLF) FROM TO	MAX CSI (LC)	MAX. UNBRAC LENGTH		MEMB. MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)	
A- B	0 / 20	-112.4 -112.4	0.06 (1)	10.00	I- C	-299 / 0	0.04 (1)
B- K	0 / 128	-112.4 -112.4	0.03 (1)	10.00	I- D	-608 / 0	0.14 (1)
K- C	0 / 112	-112.4 -112.4	0.08 (1)	10.00	D- H	-608 / 0	0.14 (1)
C- D	0 / 127	-112.4 -112.4	0.19 (1)	10.00	H- E	-299 / 0	0.04 (1)
D- E	0 / 127	-112.4 -112.4	0.19 (1)	10.00	J- K	-164 / 0	0.00 (1)
E- M	0 / 112	-112.4 -112.4	0.08 (1)	10.00	L- M	-164 / 0	0.00 (1)
M- F	0 / 128	-112.4 -112.4	0.03 (1)	10.00			
F- G	0 / 20	-112.4 -112.4	0.06 (1)	10.00			
B- J	-102 / 0	-18.5 -18.5	0.07 (1)	6.25			
J- I	-102 / 0	-18.5 -18.5	0.12 (4)	6.25			
I- H	0 / 440	-18.5 -18.5	0.15 (1)	10.00			
H- L	-102 / 0	-18.5 -18.5	0.12 (4)	6.25			
L- F	-102 / 0	-18.5 -18.5	0.07 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   24.0   IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (D-E:1) , BC=0.15/1.00 (H-I:1) , WB=0.14/1.00 (D-H:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

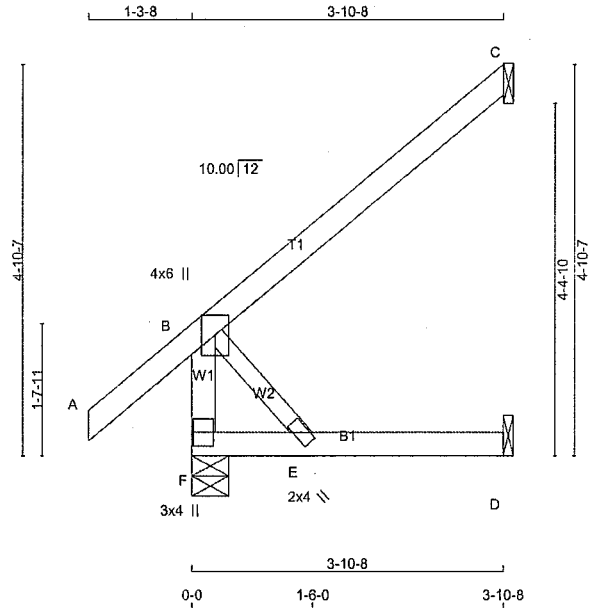JSI GRIP= 0.36 (H) (INPUT = 0.90)
JSI METAL= 0.13 (H) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	PB6	15	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AqdClOSevh3uN4Xyl113N2zyjH4-AhVavRwDYyI3jEZVPOEyZcaipTJUID9spW8BKkzUnxW
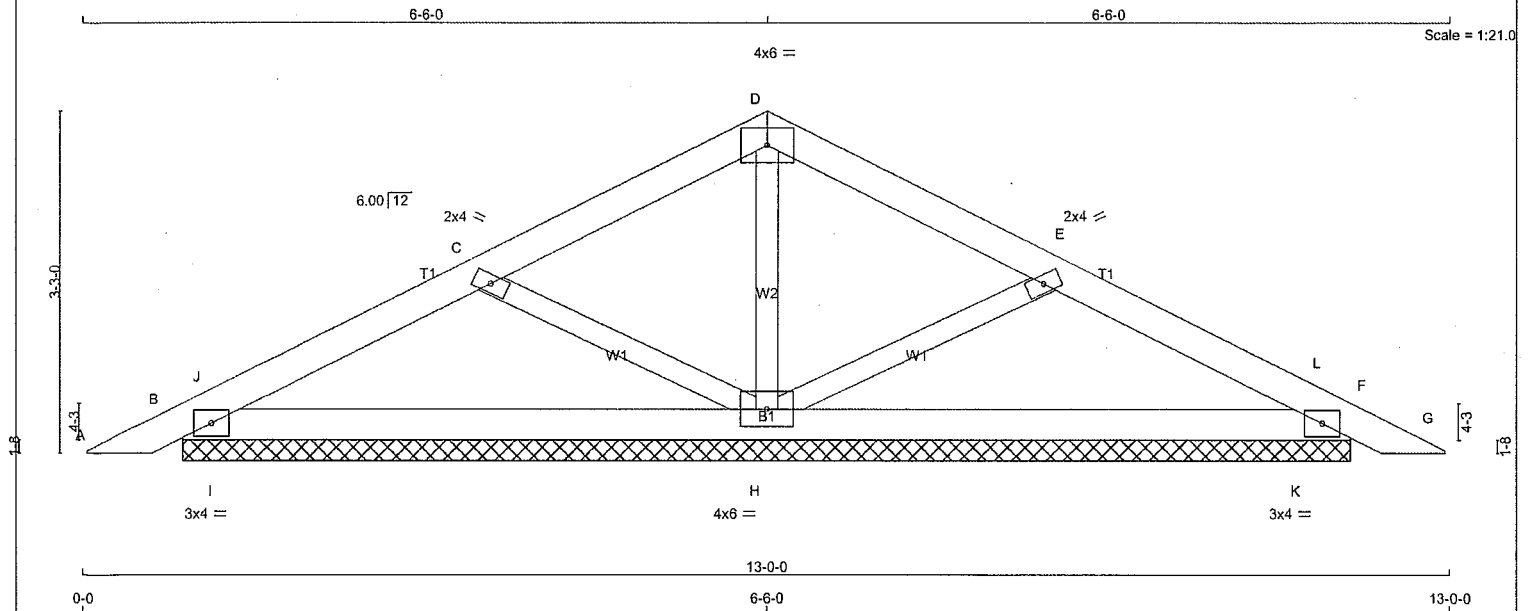
TOTAL WEIGHT = 15 X 36 = 538 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMB1-l	MT20	3.0	4.0		
H	BMWWW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	356	0	356	0	0	11-1-6	1-8
F	356	0	356	0	0	11-1-6	1-8
H	933	0	933	0	0	11-1-6	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	248	183 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
F	248	183 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
H	653	466 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-112.4 -112.4	0.06 (1)	10.00	H- D	-405 / 0	0.07 (1)
B- J	-263 / 0	-112.4 -112.4	0.07 (4)	6.25	H- E	-417 / 0	0.08 (1)
J- C	-187 / 0	-112.4 -112.4	0.14 (1)	6.25	C- H	-417 / 0	0.08 (1)
C- D	0 / 181	-112.4 -112.4	0.17 (1)	10.00	I- J	-4 / 86	0.00 (1)
D- E	0 / 181	-112.4 -112.4	0.17 (1)	10.00	K- L	-4 / 86	0.00 (1)
E- L	-187 / 0	-112.4 -112.4	0.14 (1)	6.25			
L- F	-263 / 0	-112.4 -112.4	0.07 (4)	6.25			
F- G	0 / 20	-112.4 -112.4	0.06 (1)	10.00			
B- I	0 / 189	-18.5 -18.5	0.05 (1)	10.00			
I- H	0 / 189	-18.5 -18.5	0.14 (4)	10.00			
H- K	0 / 189	-18.5 -18.5	0.14 (4)	10.00			
K- F	0 / 189	-18.5 -18.5	0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-D:1) , BC=0.14/1.00 (H-I:4) , WB=0.08/1.00 (C-H:1) , SSI=0.13/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	PB7	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:54 2024 Page 1
ID:AgdCIOSevh3uN4Xyl113N2zyjH4-et3y7nwrJFQwLO8hy6lB6p7o1tg8UgA?2AtksAzUnxV

2-4-13   9-9-7

Scale = 1:20.3

5x6 \\   2x4 || 4x6 =
C   D   E
10.00 [12]
2-0-0
T1   W1   W2   W1   W2   W3
J   1-10-8
4-13
B
A   B1
1-8   1-8

I   H   G   F
3x4 =   2x4 || 4x6 =   3x4 ||

12-2-4

0-0   2-4-13   7-3-0   12-2-4

TOTAL WEIGHT = 3 X 39 = 117 lb   [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	245	0	245	0	0	11-6-11	1-8
B	204	0	204	0	0	11-6-11	1-8
H	321	0	321	0	0	11-6-11	1-8
G	801	0	801	0	0	11-6-11	1-8

UNFACTORED REACTIONS
MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	172	121 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
B	140	115 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
H	228	146 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
G	561	402 / 0	0 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM TO (PLF)	LC1 MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-112.4 -112.4	0.02 (1)	10.00	H- C	-232 / 0	0.03 (1)
B- J	-38 / 0	-112.4 -112.4	0.01 (1)	6.25	C- G	-34 / 0	0.01 (1)
J- C	-67 / 0	-112.4 -112.4	0.04 (1)	6.25	G- D	-687 / 0	0.10 (1)
C- D	0 / 0	-112.4 -112.4	0.46 (1)	10.00	G- E	0 / 0	0.00 (1)
D- E	0 / 0	-112.4 -112.4	0.46 (1)	10.00	I- J	-138 / 0	0.00 (1)
F- E	-209 / 0	0.0 0.0	0.02 (1)	7.81			
B- I	0 / 48	-18.5 -18.5	0.05 (1)	10.00			
I- H	0 / 48	-18.5 -18.5	0.06 (4)	10.00			
H- G	0 / 32	-18.5 -18.5	0.11 (4)	10.00			
G- F	0 / 0	-18.5 -18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
        DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
        DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46/1.00 (C-D:1) , BC=0.11/1.00 (G-H:4) , WB=0.10/1.00 (D-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX MIN	SHEAR (PLI) MAX MIN	SECTION (PLI) MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040118

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	J1	13	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:35 2024 Page 1
ID:AqdClOSevh3uN4Xyl113N2zyjH4-iDQn8HiwoQ1cZTB1LcvkqpqVdCARX50CginEhnzUnxo

1-3-8   5-10-8

Scale = 1:23.3

6.00 ⌐12

4-1-4

3-8-9
4-1-4

3x4 ||

B

W1

1-2-0

A

B1

E

3x4 ||

D

5-10-8

0-0   5-10-8

TOTAL WEIGHT = 13 X 17 = 218 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES   (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES-   (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	629	0	629	0	0	5-8	1-8
C	248	0	248	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	438	327 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	170	143 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MAX. FACTORED	FACTORED					MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO				LENGTH	FR-TO	
E- B	-565 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00			
B- C	-37 / 0	-112.4	-112.4	0.66 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP. CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT. CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING =   24.0   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
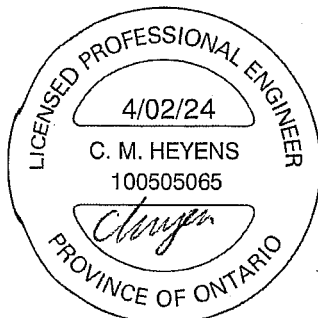
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION	
	(PSI)	(PLI)	(PLI)	
	MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (E) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	J2	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:36 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-AQ 9LdiYYj9TBdmEvKQzN0NmDcWRGYGMvMWnEDzUnxn

Scale = 1:27.5

1-3-8   3-10-8

10.00 | 12

T1

4x6 ||

B

W1   W2

B1

A

F   E

3x4 || 2x4 \\   D

3-10-8

0-0   1-6-0   3-10-8

4-10-7   4-4-10   4-10-7   1-7-11

TOTAL WEIGHT = 7 X 15 = 106 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		Edge
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	409	0	409	0	0	5-8	1-8
C	218	0	218	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS
1ST LCASE   MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	284	216 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
C	149	126 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
F- B	-374 / 0	0.0 0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 50	-112.4 -112.4	0.16 (5)	10.00			
B- C	0 / 0	-112.4 -112.4	0.29 (1)	10.00			
F- E	0 / 0	-18.5 -18.5	0.07 (4)	10.00			
E- D	0 / 0	-18.5 -18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING =   24.0   IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) ,
WB=0.00/1.00 (B-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

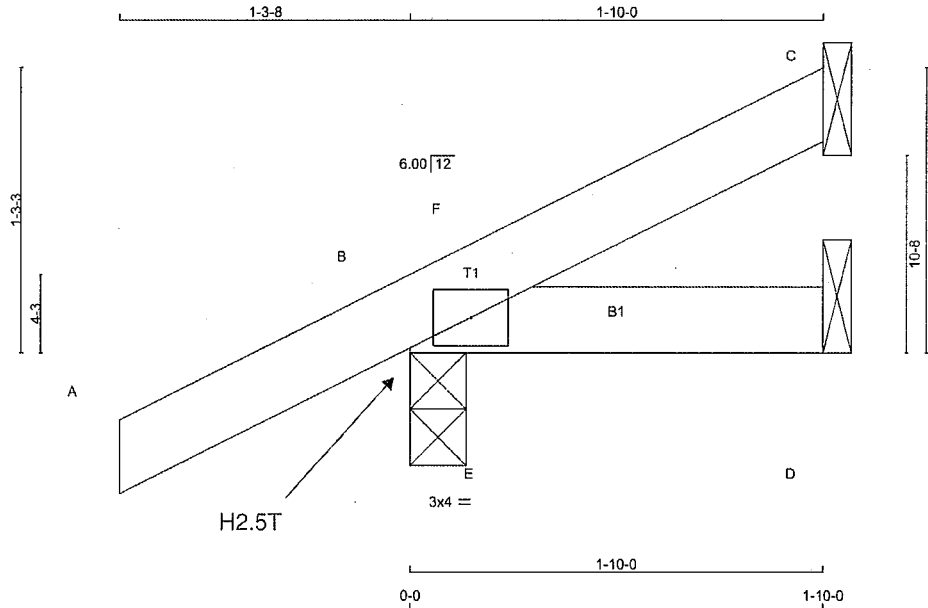JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	J3W	17	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Tue Apr 2 11:02:38 2024 Page 1
ID:AgdClOSevh3uN4Xyl113N2zyjH4-6o5vmJko4LPBQxwc0ITRSRS8TQ59kPCfMg?uI6zUnxl

1-3-8   6-7-0

Scale: 1/2"=1'

6.00 ⌐12   4x6 ≈

4-0-11   4-3

T1

W1   W2

B   I

A

3x4 = H   B1   G   F   3x4 ≈

2x4 || E

H2.5T

7-5-0

0-0   3-8-8   7-5-0

TOTAL WEIGHT = 17 X 24 = 403 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
F	BMW+w	MT20	3.0	4.0	
G	BMW+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	177	0	181	0	-82	1-8	1-8
B	638	0	638	210	-226	3-0	1-8
E	309	0	309	0	-201	5-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 226 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 201 LBS FACTORED UPLIFT

PROVIDE FOR 210 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	121	101 / 0	0 / 0	0 / 0	11 / -72	20 / 0	0 / 0
B	445	329 / 0	0 / 0	0 / 0	0 / -236	116 / 0	0 / 0
E	219	140 / 0	0 / 0	0 / 0	0 / -194	79 / 0	0 / 0

HORIZONTAL REACTIONS

| B | --- | 0 / 0 | 0 / 0 | 0 / 0 | 150 / -24 | 0 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS					WEBS		
MAX. FACTORED	FACTORED				MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX FROM TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	FORCE (LBS)	MAX CSI(LC)
FR-TO		FROM TO					
A- B	0 / 33	-112.4 -112.4	0.15 (1)	10.00	G- C	-208 / 257	0.06 (1)
B- I	-644 / 181	-112.4 -112.4	0.04 (1)	6.25	C- F	-720 / 419	0.16 (1)
I- C	-646 / 229	-112.4 -112.4	0.13 (1)	6.25	H- I	-225 / 85	0.00 (1)
C- D	-46 / 0	-112.4 -112.4	0.18 (1)	6.25			
B- H	-343 / 588	-18.5 -18.5	0.20 (1)	6.25			
H- G	-343 / 588	-18.5 -18.5	0.22 (1)	6.25			
G- F	-343 / 588	-18.5 -18.5	0.51 (1)	6.25			
F- E	0 / 0	-18.5 -18.5	0.42 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT {20-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.18/1.00 (C-D:1) , BC=0.51/1.00 (F-G:1) , WB=0.16/1.00 (C-F:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (B) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 0.95)

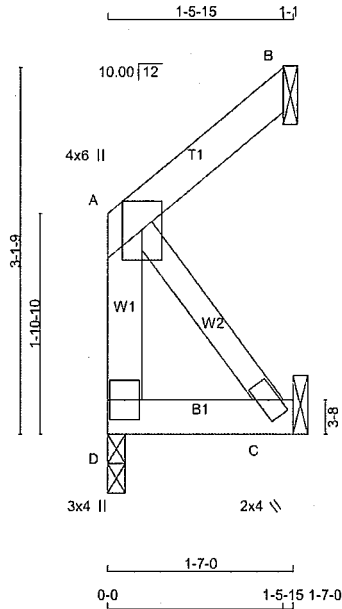LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040106

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	J4	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-a?fHzflRreX225UoaS_g?f?ECqVkTu?obKlRqYzUnxk

1-3-8   5-5-8

Scale = 1:18.7

6.00 [12

T1

B1

3-0-15

2-8-4   3-0-15

4-3

3x4 =

5-5-8

0-0   5-5-8

TOTAL WEIGHT = 5 X 15 = 75 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	267	0	267	0	0	1-8	1-8
B	510	0	510	0	0	3-8	1-8
D	90	0	90	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	184	150 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	355	265 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	67	27 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

	CHORDS						WEBS		
	MAX. FACTORED	FACTORED						MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A- B	0 / 33	-112.4	-112.4	0.15 (1)	10.00	E- F	-533 / 9	0.00 (1)	
B- F	-26 / 107	-112.4	-112.4	0.13 (1)	6.25				
F- C	-8 / 2	-112.4	-112.4	0.43 (1)	10.00				
B- E	0 / 0	-18.5	-18.5	0.30 (1)	10.00				
E- D	0 / 0	-18.5	-18.5	0.30 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH;
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 525 (0.12")

CSI: TC=0.43/1.00 (C-F:1) , BC=0.30/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.42/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
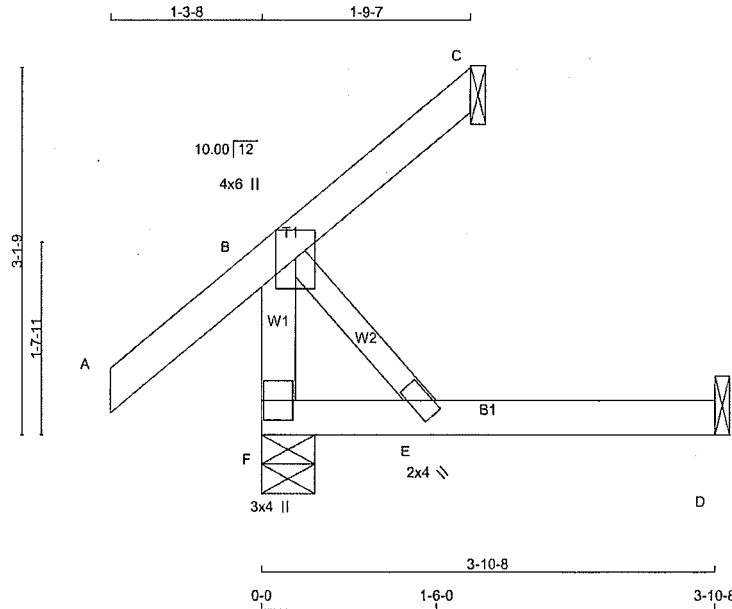JSI METAL= 0.08 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437026	J5W	8	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-2BDgB I3cyfvgF3?89VvXsYSDDpCCKzxq U N zUnxi

1-3-8   5-7-0

Scale = 1:21.3

6.00 [12]   2x4 ≈

TOTAL WEIGHT = 8 X 20 = 156 lb   [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
F	BMW+w	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	153	0	157	0	-71	1-8	1-8
B	573	0	573	185	-200	3-0	1-8
E	267	0	267	0	-174	5-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 200 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 174 LBS FACTORED UPLIFT

PROVIDE FOR 185 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	105	87 / 0	0 / 0	0 / 0	9 / -62	18 / 0	0 / 0
B	399	297 / 0	0 / 0	0 / 0	0 / -209	102 / 0	0 / 0
E	189	121 / 0	0 / 0	0 / 0	0 / -168	68 / 0	0 / 0

HORIZONTAL REACTIONS

| B | --- | 0 / 0 | 0 / 0 | 0 / 0 | 132 / -21 | 0 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS						WEBS		
MAX. FACTORED		FACTORED				MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 FROM TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO						
A- B	0 / 33	-112.4 -112.4	0.15 (1)		10.00	C- F	-435 / 212	0.08 (1)
B- H	-553 / 155	-112.4 -112.4	0.15 (7)		6.25	G- H	-242 / 228	0.00 (1)
H- C	-349 / 38	-112.4 -112.4	0.22 (1)		6.25			
C- D	-39 / 0	-112.4 -112.4	0.13 (1)		6.25			
B- G	-168 / 344	-18.5 -18.5	0.12 (1)		6.25			
G- F	-168 / 344	-18.5 -18.5	0.41 (1)		6.25			
F- E	0 / 0	-18.5 -18.5	0.36 (1)		10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {20-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 692 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 448 (0.17")

CSI: TC=0.22/1.00 (C-H:1) , BC=0.41/1.00 (F-G:1) , WB=0.08/1.00 (C-F:1) , SSI=0.22/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 0.95)

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ID:AgdClOSevh3uN4Xyl113N2zyjH4- aLQcgnJ8ZvdvYDNFaXNdHdlY1XWgFlEHlz5RtzUnxh

1-3-8 4-7-0

Scale = 1:18.7

6.00 [12

3-0-11

4-3

T1

C

B F

A

3x4 = E

H2.5T

B1

D

5-5-0

0-0 5-5-0

TOTAL WEIGHT = 7 X 15 = 104 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	265	0	270	0	-127	1-8	1-8
B	507	0	507	161	-175	3-0	1-8
D	90	0	90	0	-80	5-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 175 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 161 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	182	149 / 0	0 / 0	0 / 0	11 / -112	33 / 0	0 / 0
B	353	264 / 0	0 / 0	0 / 0	0 / -182	89 / 0	0 / 0
D	66	27 / 0	0 / 0	0 / 0	0 / -82	40 / 0	0 / 0

HORIZONTAL REACTIONS

B	---	0 / 0	0 / 0	0 / 0	115 / -17	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS					WEBS		
MAX. FACTORED		FACTORED			MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A- B	0 / 33	-112.4 -112.4	0.15 (1)	10.00	E- F	-600 / 165	0.00 (1)
B- F	-158 / 143	-112.4 -112.4	0.15 (3)	6.25			
F- C	-69 / 4	-112.4 -112.4	0.43 (1)	6.25			
B- E	0 / 0	-18.5 -18.5	0.29 (1)	10.00			
E- D	0 / 0	-18.5 -18.5	0.29 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {20-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 950 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 536 (0.12")

CSI: TC=0.43/1.00 (C-F:1) , BC=0.29/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.42/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
4/02/24
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR24040109

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
437026	J7	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:AqdClOSevh3uN4Xyl113N2zyjH4-Tmvop0oxvt1UXioapl2c9VAyURt8Pi?OWyjf JzUnxg

1-3-8   4-5-8

Scale = 1:16.3

2-6-15   4-3   6.00 [12]   T1   2-2-4   2-6-15

C   F   B   B1   A   E   D

3x4 =

4-5-8

0-0   4-5-8

TOTAL WEIGHT = 6 X 13 = 75 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	217	0	217	0	0	1-8	1-8
B	444	0	444	0	0	3-8	1-8
D	75	0	75	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	149	122 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
B	309	233 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
D	55	23 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			LENGTH	FR-TO		
A- B	0 / 33	-112.4 -112.4	0.15 (1)		10.00	E- F	-367 / 6	0.00 (1)
B- F	-20 / 54	-112.4 -112.4	0.07 (1)		6.25			
F- C	-5 / 2	-112.4 -112.4	0.29 (1)		10.00			
B- E	0 / 0	-18.5 -18.5	0.21 (1)		10.00			
E- D	0 / 0	-18.5 -18.5	0.21 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 945 (0.06")

CSI: TC=0.29/1.00 (C-F:1) , BC=0.21/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.29/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437026	J8	3	1	**BAYVIEW WELLINGTON** TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:AgdCIOSevh3uN4Xyl113N2zyjH4-P90ZEipCRUIBm0yyxj54EwFK9EcttcUhzGCl2CzUnxe

Scale = 1:9.8

1-3-8 · 1-10-0

6.00 [12

C
F
B
T1
B1
A
E
D
3x4 =

H2.5T

1-3-3 · 10-8 · 1-3-3 · 4-3

1-10-0
0-0 · 1-10-0

TOTAL WEIGHT = 3 X 6 = 19 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER		DESCR.
A - C	2x4	DRY	No.2		SPF
B - D	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	63	0	64	0	-35	1-8	1-8
B	329	0	329	72	-101	3-0	1-8
D	0	0	14	0	-22	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 72 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	43	35 / -13	0 / 0	0 / 0	4 / -31	9 / 0	0 / 0
B	227	180 / 0	0 / 0	0 / 0	0 / -102	47 / 0	0 / 0
D	0	0 / -19	0 / 0	0 / 0	0 / -22	10 / 0	0 / 0

HORIZONTAL REACTIONS

| B | --- | 0 / 0 | 0 / 0 | 0 / 0 | 51 / -4 | 0 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (13)

CHORDS						WEBS		
MAX. FACTORED		FACTORED				MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT.LOAD (PLF)	LC1 FROM TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 33	-112.4 -112.4	0.15 (1)	10.00		E- F	-23 / 153	0.00 (1)
B- F	-99 / 0	-112.4 -112.4	0.14 (1)	6.25				
F- C	-19 / 0	-112.4 -112.4	0.04 (13)	6.25				
B- E	0 / 0	-18.5 -18.5	0.06 (13)	10.00				
E- D	0 / 0	-18.5 -18.5	0.06 (13)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {20-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.06/1.00 (B-E:13) , WB=0.00/1.00 (E-F:1) , SSI=0.15/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
437026	C1	2	1	**BAYVIEW WELLINGTON** TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-HekeVFf2VVf1i0SSqUM1DAD83?AcLkGm IYZ5SzUnxr

1-5-15 1-1

Scale = 1:18.8

10.00 ⌐12

4x6 ‖ T1

A

B

W1 W2

B1

C

D

3x4 ‖ 2x4 \\

1-7-0

0-0   1-5-15 1-7-0

TOTAL WEIGHT = 2 X 8 = 15 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		Edge
C	BMW1+w	MT20	2.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	97	0	97	0	0	1-13	1-8
B	84	0	84	0	0	1-8	1-8
C	13	0	14	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	68	49 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
B	58	49 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
C	10	0 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

	C H O R D S						W E B S		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM TO (PLF)		CSI (LC)	MAX UNBRAC		MEMB. LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D - A	-84 / 0	0.0	0.0	0.01 (1)	7.81	A- C		0 / 0	0.00 (1)
A - B	0 / 0	-112.4	-112.4	0.04 (1)	10.00				
D - C	0 / 0	-18.5	-18.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = **24.0** IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
 OR SMALL BUILDING REQUIREMENTS OF PART
 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
 RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
 ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.04/1.00 (A-B:1) , BC=0.01/1.00 (C-D:4) ,
 WB=0.00/1.00 (A-C:1) , SSI=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL IN THE
 TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 0.95)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.		DRWG NO.
437026	C2	2	1	**BAYVIEW WELLINGTON**		
Tamarack Roof Truss, Burlington				TRUSS DESC.		

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ID:AgdClOSevh3uN4Xyl113N2zyjH4-D1sPwxhl16vlxKcrnvOVlblSmprzoem3R21g9LzUnxp

1-3-8   1-9-7

Scale = 1:18.8

10.00 $\overline{12}$

4x6 ‖

3-1-9

1-7-11

TOTAL WEIGHT = 2 X 12 = 24 lb
[M]

3-10-8

0-0   1-6-0   3-10-8

LUMBER
N. L. G. A. RULES
CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		Edge
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
  TOUCHES EDGE OF CHORD.

NOTES· (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	352	0	352	0	0	5-8	1-8
C	40	0	40	0	-51	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED   UPLIFT

UNFACTORED REACTIONS
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	245	183 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	27	23 / -35	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

	CHORDS					WEBS		
	MAX. FACTORED	FACTORED			MAX.		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO				
F- B	-317 / 0	0.0 0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)	
A- B	0 / 50	-112.4 -112.4	0.16 (5)	10.00				
B- C	-39 / 0	-112.4 -112.4	0.15 (5)	6.25				
F- E	0 / 0	-18.5 -18.5	0.07 (4)	10.00				
E- D	0 / 0	-18.5 -18.5	0.08 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING =   **24.0**   **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) ,
WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
  COMP=1.10 SHEAR=1.10 TENS:1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)

EWP DESIGN INC.

(905) 832-2250   FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

February 1, 2019

REVIEWED

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

RAFTER

GIRDER TRUSS

CEILING MEMBER

Girder chord

30°

1/3 nail length

Top view
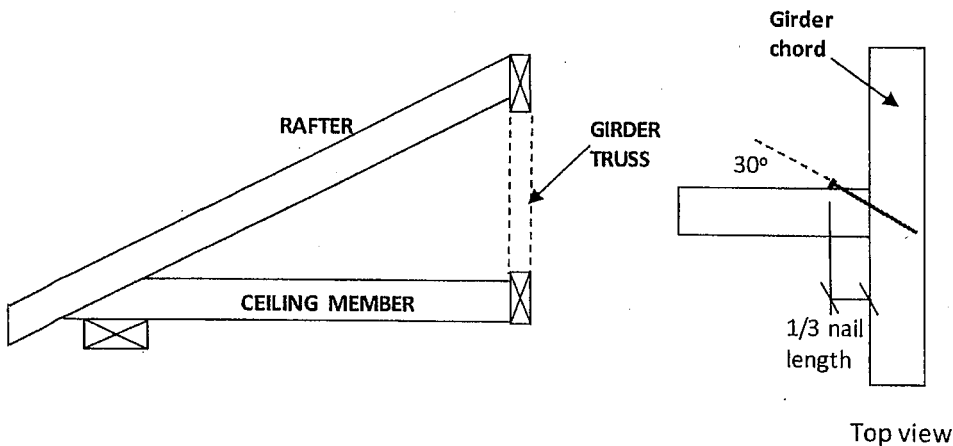
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

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TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift

Elevation View — 2x6 Bearing plate

Top view

Elevation View — 2x4 Bearing plate

Top view

Nails are installed at a 30° angle to vertical, through member into bearing plate

30°

1/3 nail length

bearing plate
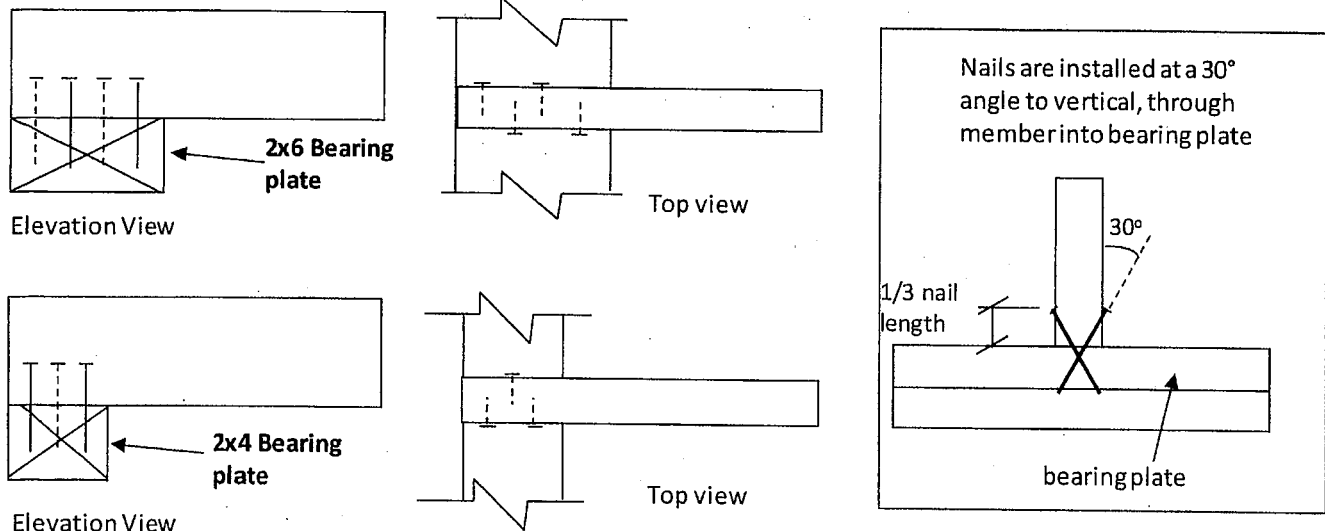
NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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LUS — Double-Shear Joist Hangers

All LUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:
- These hangers cannot be modified
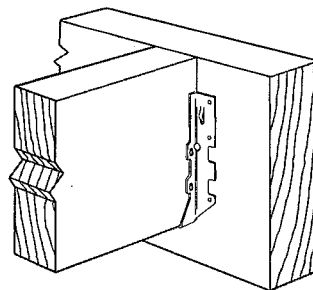
Typical LUS Installation

LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir–L		S-P-F	
		W	H	B	d_e^1	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⁹⁄₁₆	3⅛	1¾	1 ¹⁵⁄₁₆	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 ¹³⁄₁₆	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⁹⁄₁₆	4¾	1¾	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4³⁄₁₆	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⁹⁄₁₆	7 ¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8³⁄₁₆	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.
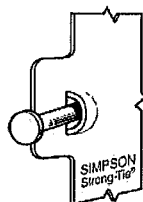
Dome double-shear nailing prevents tabs breaking off (available on some models).

US Patent 5,603,580

Double-shear nailing top view.

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HUS/LJS — Double-Shear Joist Hangers

SIMPSON Strong-Tie

All hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options
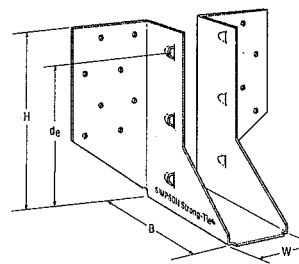
LJS26DS
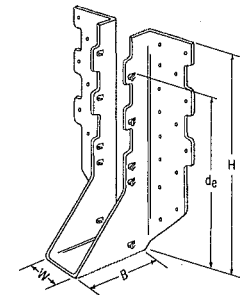
HUS210
(HUS26, HUS28, similar)
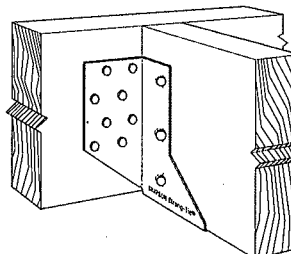
Typical LJS26DS Installation

Typical HUS Installation

Typical HUS Installation
(Truss designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir–L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅝	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6³⁄₃₂	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.
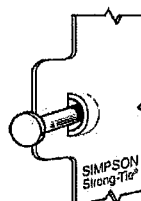
Dome double-shear nailing prevents tabs breaking off (available on some models).

US Patent 5,603,580
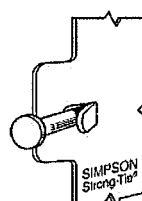
Double-shear nailing side view. Do not bend tab back.

Double-shear nailing top view.
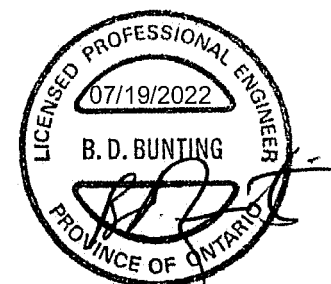
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HGUS — Double-Shear Joist Hangers

SIMPSON Strong-Tie

All HGUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:
- See current catalogue for options
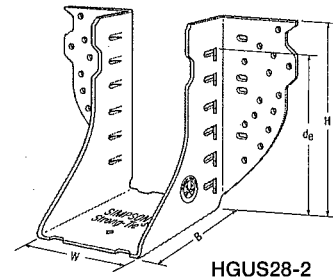
HGUS28-2
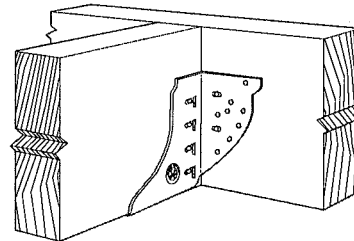
Typical HGUS Installation

Typical HGUS Installation
(Truss designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir–L		S-P-F	
		W	H	B	d_e^1	Face	Joist	Uplift	Normal	Uplift	Normal
								$(K_D=1.15)$	$(K_D=1.00)$	$(K_D=1.15)$	$(K_D=1.00)$
HGUS26	12	1⅝	5⅜	5	4 5⁄32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5⁄16	5 7⁄16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15⁄16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9⁄16	5 7⁄16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5⁄16	7 3⁄16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15⁄16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9⁄16	7 3⁄16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5⁄16	9 9⁄16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15⁄16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9⁄16	9 9⁄16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9⁄16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9⁄16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

Dome double-shear nailing prevents tabs breaking off (available on some models).
SIMPSON Strong-Tie
US Patent 5,603,580

Double-shear nailing side view. Do not bend tab back.
SIMPSON Strong-Tie

Double-shear nailing top view.

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H — Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge

Finish: G90 galvanized

Design:
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Factored resistances have been increased 15%. No further increase is permitted.

Installation:
- Use all specified fasteners
- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)
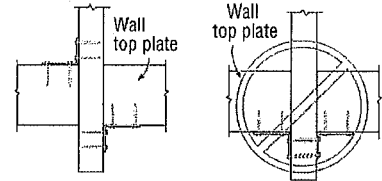
Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.
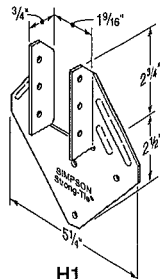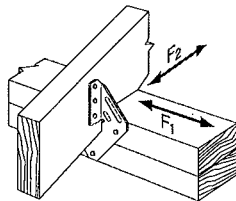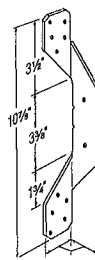
H1   H1 Installation   H2A   H2A Installation   H2.5A   H2.5A Installation
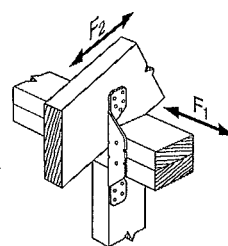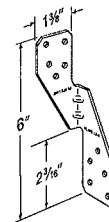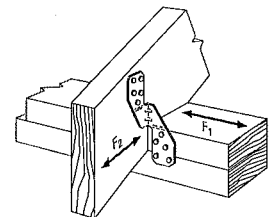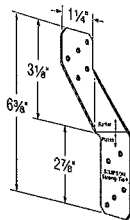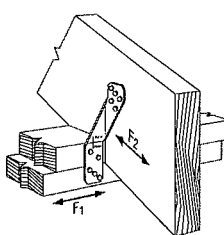
H2.5T   H2.5T Installation (Nails into both top plates)   H3   H3 Installation   H10A   H10A Installation
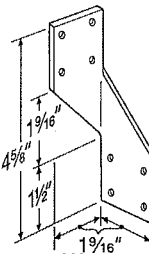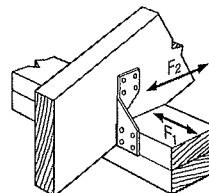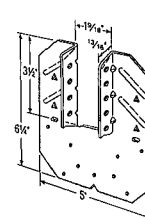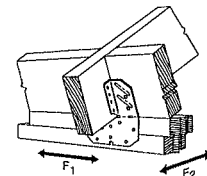
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir–L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F_1	F_2		F_1	F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

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Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

DIAGONAL BRACING AS SHOWN (RED), FASTENED TO THE UNDERSIDE OF THE TOP CHORD (IN THE PLANE OF THE PIGGYBACK TRUSSES) SPACED AT 10' INTERVALS (UNLESS A CLOSER SPACING IS REQUIRED BY THE BUILDING DESIGNER)

PIGGYBACK TRUSSES

2X4 PURLINGS (GREEN) SPACED AT 24" O/C OR LESS IF REQUIRED BY BASE TRUSS DESIGN.

≤45°
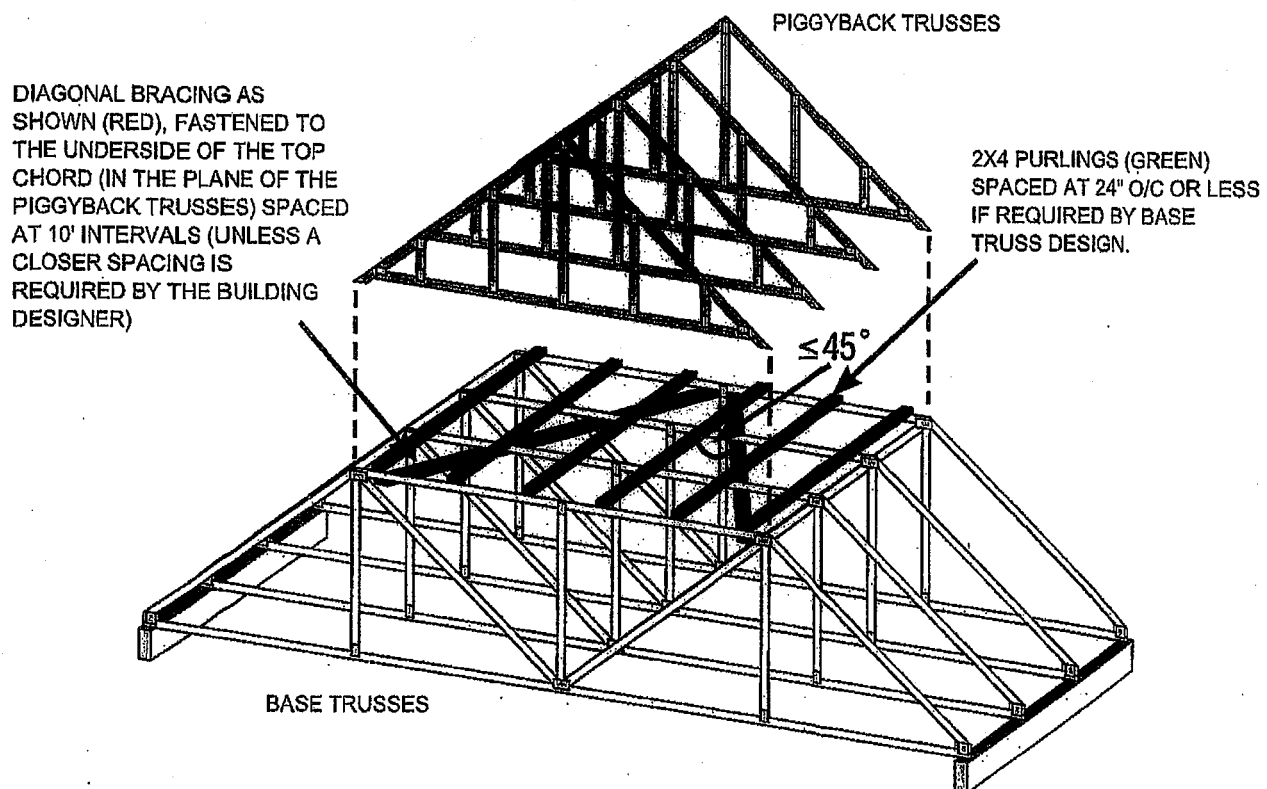
BASE TRUSSES

NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

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Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x,y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.

↑ 1 3/4"

0-¹⁄₁₆"

For 4 x 2 orientation, locate plates 0-¹⁄₁₆" from outside edge of truss.
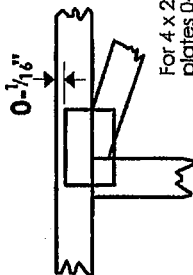
This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING
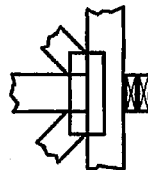
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 | dimensions shown in ft-in-sixteenths or mm
(Drawings not to scale)

TOP CHORD

TOP CHORDS

WEBS

BOTTOM CHORDS

TOP CHORD

C1-2, C2-3, C3-4, C4-5, C5-6, C6-7, C7-8, C1-8
W1-7, W2-7, W3-6, W4-5, W1-3

1 2 3 4 5 6 7 8
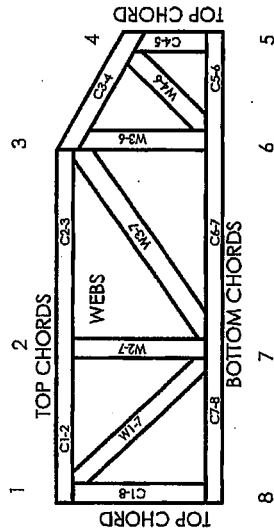
JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: Mii-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.

2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.

3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.

4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

5. Cut members to bear tightly against each other.

6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.

7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.

8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.

10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.

11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.

12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.

13. Top chords must be sheathed or purlins provided at spacing indicated on design.

14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.

15. Connections not shown are the responsibility of others.

16. Do not cut or alter truss member or plate without prior approval of an engineer.

17. Install and load vertically unless indicated otherwise.

18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.

19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.

20. Design assumes manufacture in accordance with TPIC Quality Criteria.

REVIEWED

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STANDARD DETAIL MSD2015-P

Issued: **APRIL 27, 2022**
Expiry: **APRIL 30, 2024**
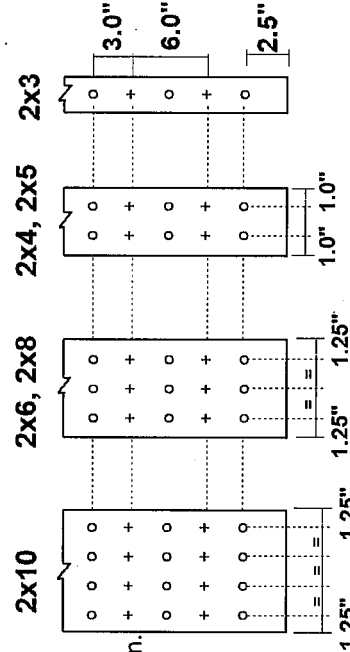
ALTERNATIVE WEB BRACING SOLUTIONS

The scab brace detail shown on this page provides an alternative method of bracing compression webs of single ply trusses. Where the original design calls for web bracing, the scab-brace is an acceptable alternative provided that the factored axial force in the web member does not exceed the tabulated values shown below. This detail applies to web lengths of 4.0 ft. to 10.0 ft. only. For intermediate web lengths, do not interpolate, use the tabulated value of the longer length. (ex. For a 6.25 ft. web, use the tabulated values for a 6.5 ft. web)

Top Chord
Bottom Chord
90% Web Length
Scab Brace

SCAB BRACE DETAIL 1-PLY TRUSS

Maximum factored web force, lbs (1-Ply Truss)

Web size	2x3	2x4	2x5	2x6	2x8+
4.0	4331	6064	7796	9529	12561
4.5	3794	5312	6829	8347	11003
5.0	3285	4599	5913	7227	9527
5.5	2823	3952	5081	6210	8186
6.0	2415	3381	4347	5313	7003
6.5	2063	2888	3713	4538	5982
7.0	1763	2468	3174	3879	5113
7.5	1510	2114	2718	3322	4379
8.0	1297	1816	2335	2854	3762
8.5	1117	1564	2011	2458	3240
9.0	966	1353	1740	2126	2803
9.5	840	1176	1512	1848	2436
10.0	733	1027	1320	1614	2127

WEB LENGTH (Ft.)

SCAB CONNECTION: 1-PLY TRUSS

2x3 — 3.0" / 6.0" / 2.5"
2x4, 2x5 — 1.0" / 1.0"
2x6, 2x8 — 1.25" / 1.25"
2x10 — 1.25" / 1.25"

+ 0.122" dia. x 3.0" nail driven from front face
o 0.122" dia. x 3.0" nail driven from back face

Note: Connect scabs to truss along their entire length.

NOTES:

1. This detail **CANNOT** be used to repair damaged webs.
2. Scab and web sizes must be equal (i.e. use a 2x6 scab on a 2x6 web, etc.).
3. Scab & web lumber must be DRY (≤ 19% moisture content) at time of installation.
4. Scab must cover minimum 90% of the entire length of web.
5. For 2x12 webs use 2x10 nail pattern, but with 5 rows of nails instead of 4 rows.
6. This detail is for webs loaded axially only (not for axial/bending members).
7. Web and scab lumber shall be SPF No. 2 (or better) grade.
8. Tabulated resistances are for standard load duration only (K_D=1.0) and DRY service conditions (K_S =1.0). Do not use detail for WET service applications.
9. This detail shall be used only in conjunction with sealed MiTek truss drawings.

PEO
Certificate No. 10889485
C. Cordogiannis
90553736
2022-05-03
LICENSED PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO

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STANDARD DETAIL MSD2015-P

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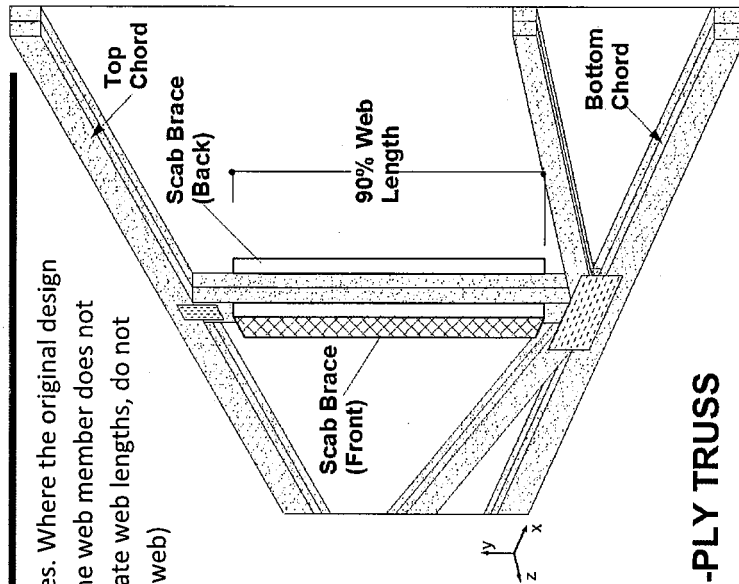## ALTERNATIVE WEB BRACING SOLUTIONS

The scab brace detail shown on this page provides an alternative method of bracing compression webs of 2-PLY trusses. Where the original design calls for web bracing, the scab-brace is an acceptable alternative provided that the maximum factored axial force in the web member does not exceed the tabulated values shown below. This detail applies to web lengths of 4.0 Ft. to 10.0 Ft. only. For intermediate web lengths, do not interpolate, use the tabulated value of the longer length. (ex. For a 6.25 ft. web, use the tabulated values for a 6.5 ft. web)

SCAB BRACE DETAIL 2-PLY TRUSS

Maximum factored web force, lbs (2-Ply Truss)

WEB LENGTH (Ft.)	2x3	2x4	2x5	2x6	2x8+
4.0	8663	12128	15593	19058	25122
4.5	7588	10623	13659	16694	22006
5.0	6570	9198	11826	14455	19054
5.5	5645	7903	10162	12420	16371
6.0	4830	6762	8694	10626	14007
6.5	4126	5776	7426	9077	11965
7.0	3526	4937	6347	7758	10226
7.5	3020	4228	5436	6644	8758
8.0	2594	3632	4670	5708	7524
8.5	2235	3128	4022	4916	6480
9.0	1933	2706	3479	4253	5606
9.5	1680	2352	3024	3696	4872
10.0	1467	2054	2640	3227	4254

SCAB CONNECTION: 2-PLY TRUSS

+ MITEK MIFLK006 Screw @ 8 in. cc
Note: Connect scabs to truss along their entire length.
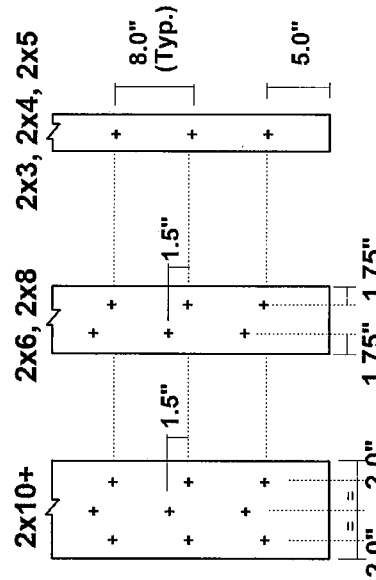
NOTES:

1. This detail CANNOT be used to repair damaged webs.
2. Scab sizes must be equal to web size (i.e. use a 2x6 scab on a 2x6 web, etc.).
3. Scabs & web lumber must be DRY (≤ 19% moisture content) at time of installation.
4. Scabs must cover 90% of the entire length of web and installed on both faces.
5. This detail shall NOT apply to vertical webs used for girder load transfer.
6. Web & scab lumber to be SPF No. 2 (or better) grade.
7. This detail is for webs loaded axially only (not for axial/bending members).
8. Ensure scabs will not interfere with incoming trusses, prior to using this detail.
9. Tabulated resistances are for standard load duration only (K_D=1.0) and DRY service conditions (K_S =1.0). Do not use detail for WET service applications.
10. This detail shall be used only in conjunction with sealed MiTek truss drawings.

LICENSED PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO
2022-05-03
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