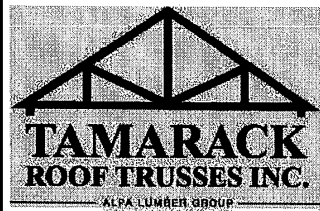


DELIVERY SHIPLIST



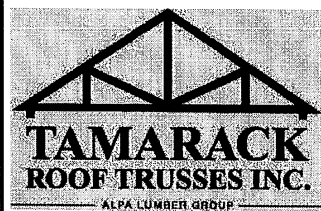
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148A
 Lot #:
 Elevation: EL: A2 UNIT 1 BLK 148A

Job Track: 50469
 PlanLog: 204745
 Layout ID: 418811
 Ref #
 Page: 1 of 3
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T97Z Jack-Open Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	4	T147A Common	10 /12	21-00-00	10-05-15	2 x 4 2 x 6	1-03-08	1-07-11 1-10-03	496.15 306.67		
	1	T147S Roof Special	10 /12 4 /12	21-06-00	10-05-15	2 x 4	1-03-08 1-00-08	1-07-11 1-05-03	105.37 66.50		
	2	T148S Scissor	10 /12 4 /12	10-06-00	6-00-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	99.85 63.33		
	1 2-ply	T150Z2 Half Hip Girder	10 /12	21-00-00	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	205.36 130.00		
	1	T151 Half Hip	10 /12	21-00-00	5-01-04	2 x 4 2 x 6	1-03-08	1-07-11 5-01-04	103.97 64.67		
	1	T152 Half Hip	10 /12	21-00-00	6-01-04	2 x 4 2 x 6	1-03-08	1-07-11 6-01-04	111.35 70.17		
	1	T153 Half Hip	10 /12	21-00-00	7-01-04	2 x 4 2 x 6	1-03-08	1-07-11 7-01-04	117.85 73.50		
	1	T154A Half Hip	10 /12	20-07-00	8-01-04	2 x 4 2 x 6		1-11-14 8-01-04	123.22 76.83		
	7	T155 Piggyback Base	10 /12	21-00-00	9-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-01-04	900.59 565.83		
	4	T155A Half Hip	10 /12	20-07-00	9-01-04	2 x 4 2 x 6		1-11-14 9-01-04	503.09 312.00		
	1	T156A Half Hip	10 /12	20-07-00	10-01-04	2 x 4 2 x 6		1-11-14 10-01-04	136.63 83.17		
	1 2-ply	T158 Common Girder	10 /12	11-00-00	6-00-03	2 x 4 2 x 6		1-05-03 1-05-03	119.45 77.33		
	1	G158 GABLE	10 /12	11-00-00	6-00-03	2 x 4	1-00-08 1-00-08	1-05-03 1-05-03	51.14 34.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148A
 Lot #:
 Elevation: EL: A2 UNIT 1 BLK 148A

Job Track: 50469
 PlanLog: 204745
 Layout ID: 418811
 Ref #
 Page: 2 of 3
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T159 Common	10/12	14-10-00	7-09-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	138.88 89.33		
	1	G159 GABLE	10/12	14-10-00	7-09-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	73.57 47.67		
	1	P135 Piggyback	10/12	12-00-08	2-00-00	2 x 4		2-00-00	38.87 25.67		
	1	P136 Piggyback	10/12	12-00-08	3-00-00	2 x 4		3-00-00	41.8 27.33		
	1	P137 Piggyback	10/12	12-00-08	4-00-00	2 x 4		3-01-07	46.82 31.17		
	1	P138 Piggyback	10/12	12-00-08	5-00-00	2 x 4		3-01-07	47.01 30.67		
	1	P139 Piggyback	10/12	12-00-08	6-00-00	2 x 4		3-01-07	49.75 32.33		
	1	P140 Piggyback	10/12	12-00-08	1-08-00	2 x 4		1-08-00	37.99 24.67		
	1	P141 Piggyback	10/12	12-00-08	3-04-00	2 x 4		3-01-07	42.88 28.67		
	1	P142 Piggyback	10/12	12-00-08	5-00-00	2 x 4		3-01-07	47.01 30.67		
	1	P143 Piggyback	10/12	12-00-08	6-06-15	2 x 4		3-01-07	46.29 29.33		
	4	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	67.18 42.67		
	6	J47 Jack-Open	4/12	7-01-00	3-01-03	2 x 4	1-03-08	3-15 2-08-04	128.87 83.00		

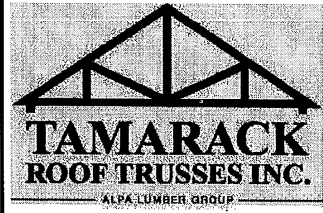
TOTAL # TRUSS= 52

TOTAL BFT OF ALL TRUSSES= 2484.85

BFT.

TOTAL WEIGHT OF ALL TRSSES 3939.31 LBS

DELIVERY SHIPLIST



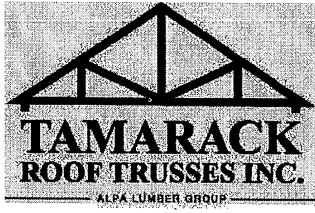
Lumber Yard: TAMARACK LUMBER
Builder: BAYVIEW WELLINGTON
Project: ALCONA SHORES
Location: INNISFIL
Model: BLOCK 148A
Lot #:
Elevation: EL: A2 UNIT 1 BLK 148A

Job Track: 50469
PlanLog: 204745
Layout ID: 418811
Ref #
Page: 3 of 3
Date: 03-24-2023
Designer:
Sales Rep: Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
8	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 9

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST		
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: ALCONA SHORES Location: INNISFIL Model: BLOCK 148A Lot #: Elevation: EL: A UNIT 2 BLK 148A	Job Track: 50469 PlanLog: 204745 Layout ID: 418812 Ref # Page: 1 of 1 Date: 03-24-2023 Designer: Sales Rep: Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T52 Piggyback Base	6 /12	43-00-00	9-10-07	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1385.6 824.17		
	1	G52 GABLE	6 /12	43-00-00	9-10-07	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	264.13 170.50		
	5	T56 Piggyback Base	6 /12	45-05-00	9-10-13	2 x 6	1-03-08 1-03-08	1-02-00 1-07-11	1494.6 918.33		
	1	G56A GABLE	6 /12	43-08-00	9-10-13	2 x 6	1-03-08	1-02-00 1-07-11	271.39 172.00		
	5	P41 Piggyback	6 /12	8-02-04	2-00-09	2 x 4			88.11 56.67		
	1	P43 Piggyback	6 /12	8-02-04	1-04-04	2 x 4			20.28 14.50		
	6	P44 Piggyback	6 /12	6-05-04	1-07-05	2 x 4			80.37 52.00		
	6	J47 Jack-Open	4 /12	7-01-00	3-01-03	2 x 4	1-03-08	3-15 2-08-04	127.49 80.00		

TOTAL # TRUSS= 30

TOTAL BFT OF ALL TRUSSES= 2288.17 BFT.

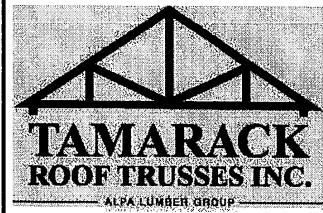
TOTAL WEIGHT OF ALL TRSSES 3731.98 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
40	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 40

DELIVERY SHIPLIST



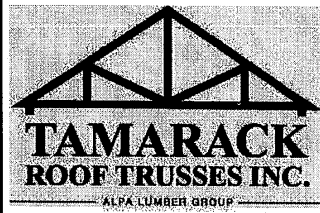
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148A
 Lot #:
 Elevation: EL: A UNIT 3 BLK 148A

Job Track: 50469
 PlanLog: 204745
 Layout ID: 418813
 Ref #
 Page: 1 of 3
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT. RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T95 Jack-Open Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	44.83 30.67		
	1 2-ply	T97 Jack-Open Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T161X Hip Girder	10 /12	24-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	237.29 153.33		
	1	T162 Hip	10 /12	23-06-00	5-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	115.4 71.83		
	1	T163 Hip	10 /12	23-06-00	6-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	125.54 80.83		
	1	T164 Hip	10 /12	23-06-00	7-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	126.63 79.17		
	1	T165 Hip	10 /12	23-06-00	8-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	133.98 84.33		
	1 2-ply	T166 Hip Girder	10 /12	23-06-00	9-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	281.56 175.33		
	1	T167A Hip	10 /12	18-09-00	10-01-04	2 x 4 2 x 6		5-07-03 2-00-11	122.12 76.83		
	5	T168A Piggyback Base	10 /12	18-09-00	9-01-04	2 x 4 2 x 6		5-07-03 2-00-11	570.59 363.33		
	2	T169 Common	10 /12	16-04-00	8-05-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	168.62 106.67		
	1	G169 GABLE	10 /12	16-04-00	8-05-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.98 53.33		
	1 2-ply	T170 Hip Girder	10 /12	21-10-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	223.12 142.00		
	1	T171 Hip	10 /12	21-10-00	6-06-07	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	114.66 73.17		

DELIVERY SHIPLIST



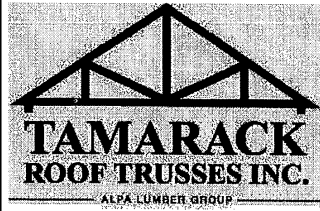
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148A
 Lot #:
 Elevation: EL: A UNIT 3 BLK 148A

Job Track: 50469
 PlanLog: 204745
 Layout ID: 418813
 Ref #
 Page: 2 of 3
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T172 Hip	10 /12	21-10-00	8-02-07	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	122.4 78.33		
	1	T173 Hip	10 /12	21-10-00	9-10-07	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	130.51 83.33		
	1	T174 Common	10 /12	21-10-00	10-11-06	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	129.12 80.00		
	4	T174X Common	10 /12	22-10-00	10-11-06	2 x 4 2 x 6		9-11 2-00-11	543.61 333.33		
	1 2-ply	T175X Common Girder	10 /12	22-10-00	10-11-06	2 x 6		9-11 2-00-11	289.71 181.33		
	2	T176 Common	10 /12	12-00-00	6-07-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	116.2 74.00		
	3	T176S Scissor	10 /12 4 /12	12-00-00	6-07-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	177.56 117.00		
	1	G177 Roof Special Structural Gable	10 /12 4 /12	22-08-08	11-00-03	2 x 4 2 x 6	1-03-08	1-05-10 1-07-11	168.47 108.33		
	1	T178 Common	10 /12	13-02-08	11-00-03	2 x 4		1-05-10 9-06-11	81.82 52.33		
	1 2-ply	T179 Common Girder	10 /12	13-02-08	11-00-03	2 x 6		1-05-10 9-06-11	233.28 143.00		
	2	T180S Scissor	10 /12 4 /12	13-00-00	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	126.15 80.67		
	6	T190 Monopitch	4 /12	8-11-00	3-08-08	2 x 4	1-03-08	3-15 3-03-10	183.96 121.00		
	1	P161 Piggyback	10 /12	6-01-01	2-00-00	2 x 4			16.42 12.00		
	3	P162 Piggyback	10 /12	6-01-01	2-06-07	2 x 4			47.87 29.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148A
 Lot #:
 Elevation: EL: A UNIT 3 BLK 148A

Job Track: 50469
 PlanLog: 204745
 Layout ID: 418813
 Ref #
 Page: 3 of 3
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	P163 Piggyback	10 /12	6-01-01	1-10-15	2 x 4			16.31 10.67		
	4	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	67.18 42.67		
	4	J6 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	60.9 38.67		
	1	J7 Jack-Open Girder	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J8 Jack-Open	10 /12	1-10-08	3-01-09	2 x 4	1-03-08	1-07-11 3-02-07	9.79 7.00		
	7	J46 Jack-Open	4 /12	5-11-00	2-08-08	2 x 4	1-03-08	3-15 2-03-10	123.72 75.83		

TOTAL # TRUSS= 72

TOTAL BFT OF ALL TRUSSES= 3205.31

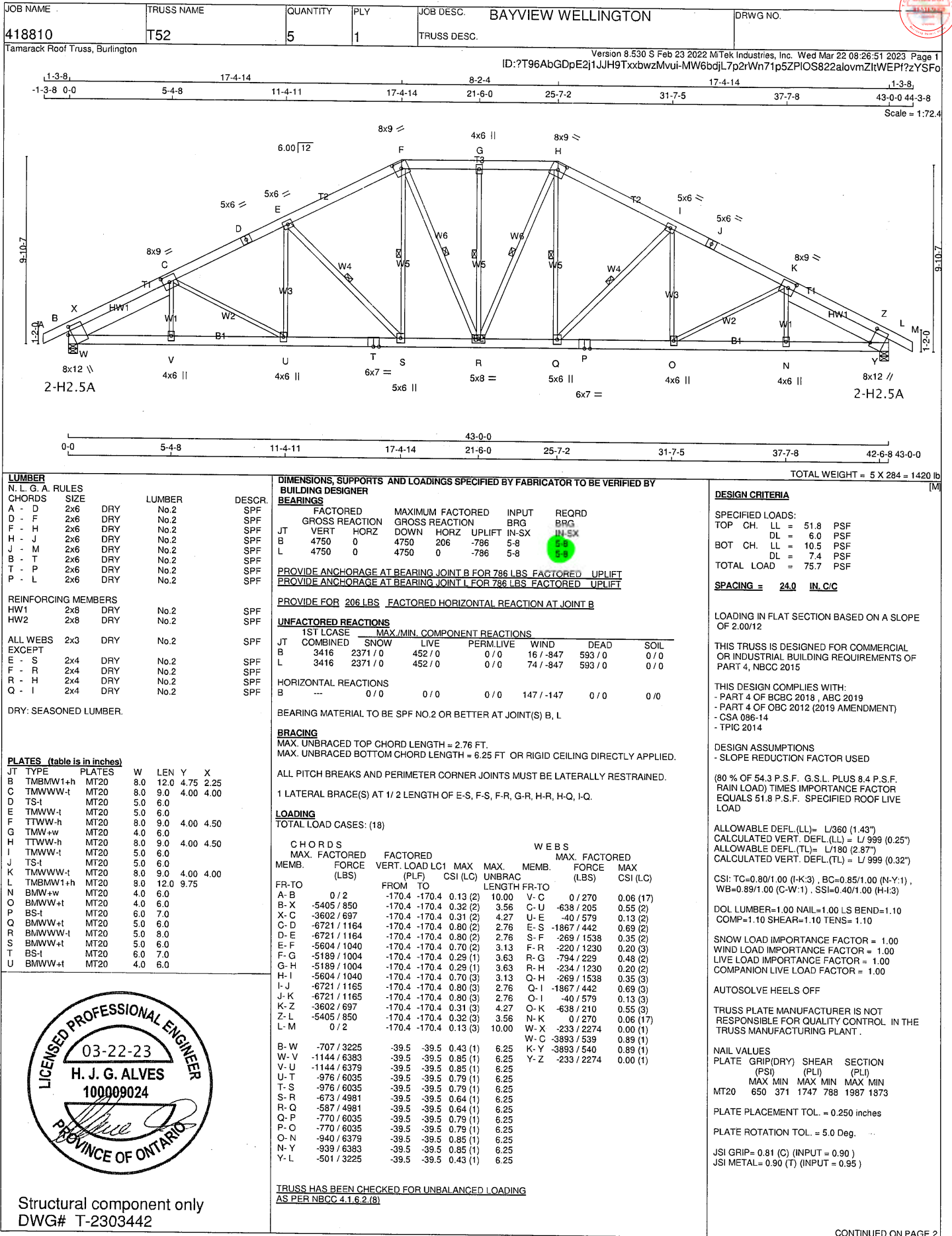
BFT.

TOTAL WEIGHT OF ALL TRSSES 5061.71 LBS

HARDWARE

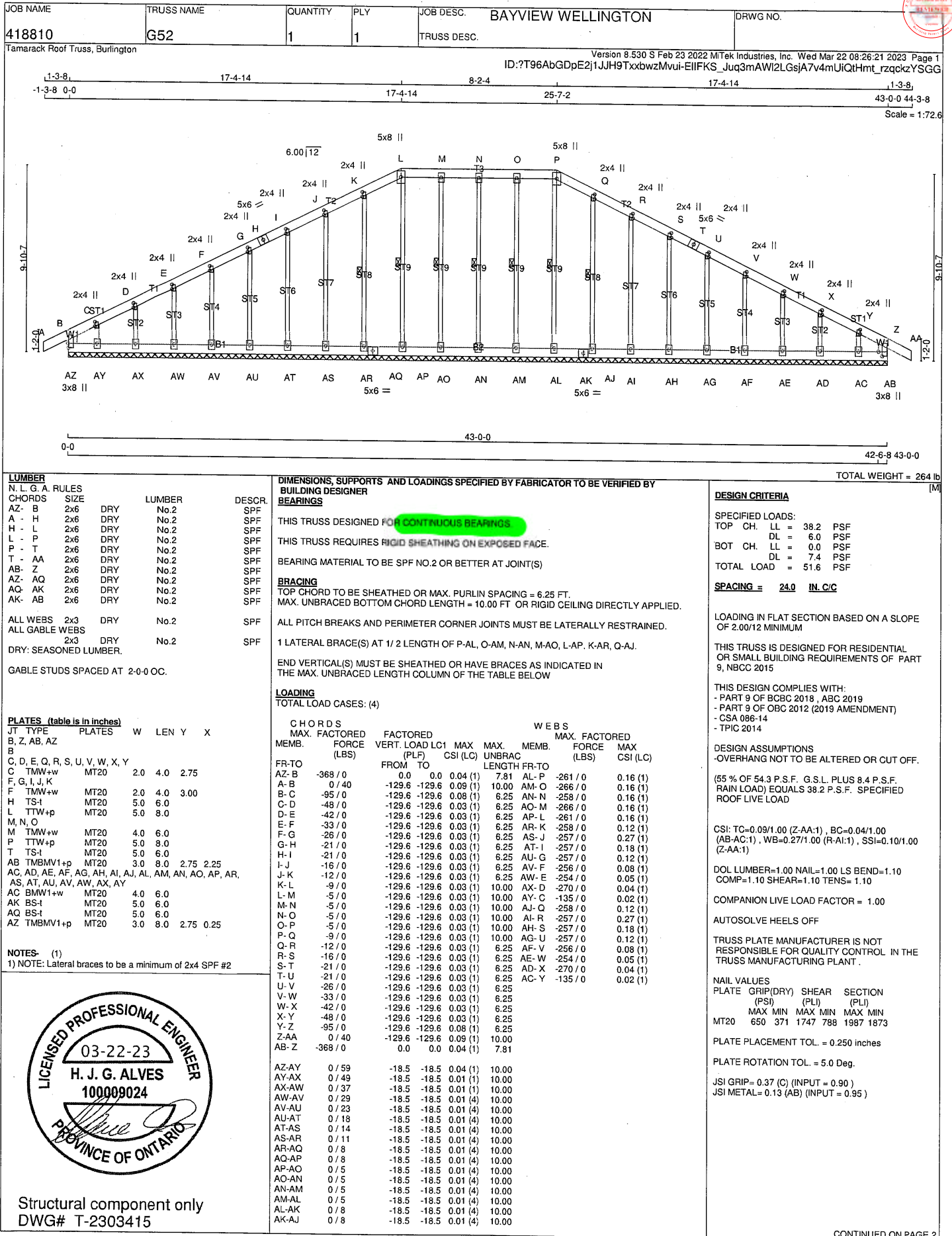
QTY	TYPE	MODEL	LENGTH
4	Hardware	HGUS26-2	
13	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 17





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.														
418810	T52	5	1	BAYVIEW WELLINGTON															
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:51 2023 Page 2															
				ID: ?T96AbGDpE2j1JJH9TxxbwzMvui-MW6bdjL7p2rWn71p5ZPIOS822alovmZlIWEPf?zYSFo															
PLATES (table is in inches) <table border="1"><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>V</td><td>BMW+w</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr></tbody></table> NOTES: (1) 1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2		JT	TYPE	PLATES	W	LEN	Y	X	V	BMW+w	MT20	4.0	6.0			WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.			
JT	TYPE	PLATES	W	LEN	Y	X													
V	BMW+w	MT20	4.0	6.0															
																			
Structural component only DWG# T-2303442																			



JOB NAME 418810	TRUSS NAME G52	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:22 2023 Page 2 ID: ?T96AbGDpE2j1JJH9TxxbwzMvui-ixsdYo?xf7BdogKEu_OyjKRfVt2fck00DVjn8AzYSGF	

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX
FR-TO		FROM TO			FR-TO				
AJ-AI	0 / 11	-18.5	-18.5	0.01 (4)	10.00				
AI-AH	0 / 14	-18.5	-18.5	0.01 (4)	10.00				
AH-AG	0 / 18	-18.5	-18.5	0.01 (4)	10.00				
AG-AF	0 / 23	-18.5	-18.5	0.01 (4)	10.00				
AF-AE	0 / 29	-18.5	-18.5	0.01 (4)	10.00				
AE-AD	0 / 37	-18.5	-18.5	0.01 (1)	10.00				
AD-AC	0 / 49	-18.5	-18.5	0.01 (1)	10.00				
AC-AB	0 / 59	-18.5	-18.5	0.04 (1)	10.00				



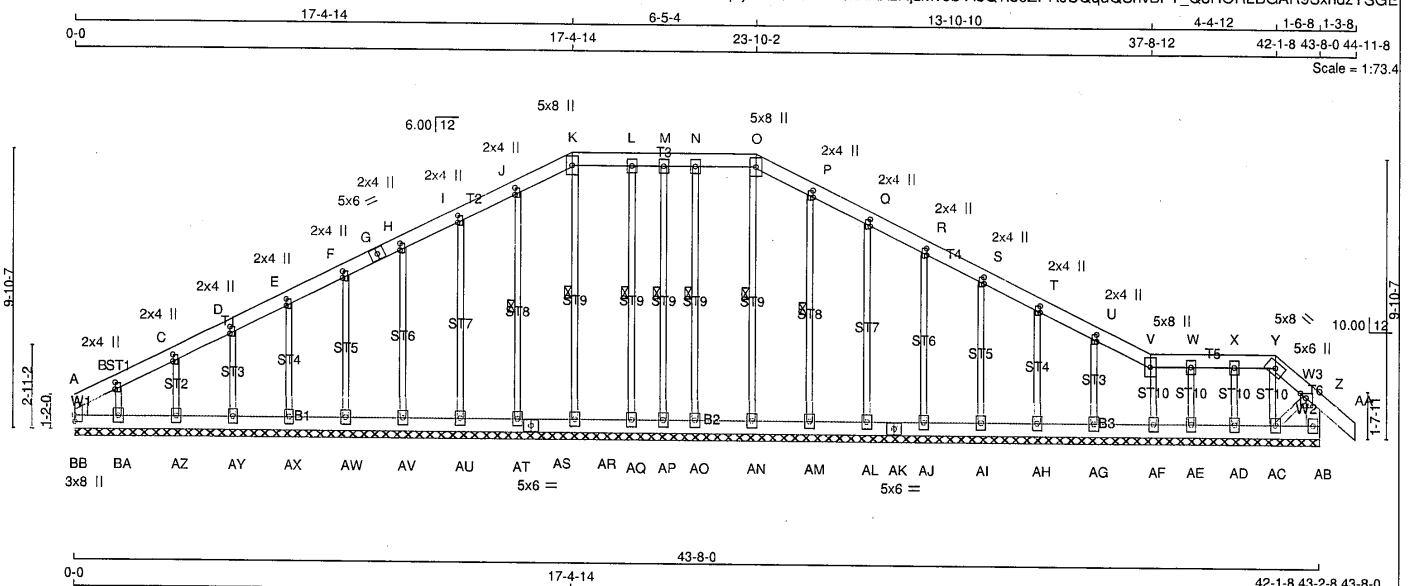
Structural component only
DWG# T-2303415

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T56	5	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:53 2023 Page 2	
				ID:DWqvyXmQmbvwFJkHAXzHjzMv9b-IvEL2ONNLf5E0RBBC RDTIEPRNU3Nd?aLqIWtzYSFm	
PLATES (table is in inches)		LOADING			
JT TYPE PLATES W LEN Y X		TOTAL LOAD CASES: (18)			
P BMV1-t MT20 8.0 9.0 Edge 3.50		C H O R D S			
Q BMWW-t MT20 6.0 7.0 3.00 2.75		MAX. FACTORED FACTORED WEBS			
R BMWW-t MT20 6.0 12.0 3.00 5.00		MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. MAX. FORCE MAX			
S, Y, Z		(LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC)			
S BMWW-t MT20 5.0 6.0		FR-TO FROM TO LENGTH FR-TO			
T BMWW-t MT20 5.0 6.0		Q- P -9 / 18 -39.5 -39.5 0.09 (1) 10.00			
U BS-t MII16 5.0 17.5 2.50 8.25					
V BMWW-t MT20 6.0 10.0 2.50 5.00					
W BMWW-t MT20 6.0 10.0 2.50 5.00					
X BS-t MII16 5.0 17.5					
AA BMWW-t MT20 6.0 10.0 3.25 3.00					
AB BMWW-t MT20 4.0 6.0					
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.		TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING			
		AS PER NBCC 4.1.6.2 (8)			
NOTES- (1)		WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.			
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2					
					
Structural component only DWG# T-2303443					

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	G56A	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:23 2023 Page 1

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
BB- A	2x6	DRY	No.2
A - G	2x6	DRY	No.2
G - K	2x6	DRY	No.2
K - O	2x6	DRY	No.2
O - V	2x6	DRY	No.2
V - Y	2x6	DRY	No.2
Y - AA	2x6	DRY	No.2
AB- Z	2x6	DRY	No.2
BB- AS	2x6	DRY	No.2
AS- AK	2x6	DRY	No.2
AK- AB	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A				
B, C, D, T				
B TMW-w	MT20	2.0	4.0	2.75
E, F, H, I, J, P, Q, R, S, U				
E TMW-w	MT20	2.0	4.0	3.00
G TS-t	MT20	5.0	6.0	
K, O, V				
K TTW-p	MT20	5.0	8.0	
L, M, N, W, X				
L TMW-w	MT20	4.0	6.0	
Y TTW-h	MT20	5.0	8.0	
Z TMW-w	MT20	5.0	6.0	2.00 2.25
AB BMV1+p	MT20	4.0	6.0	
AC BMWV1+t	MT20	4.0	6.0	
AD, AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ, AR, AT, AU, AV, AW, AX, AY, AZ, BA				
AD BMW1-w	MT20	4.0	6.0	
AK BS-t	MT20	5.0	6.0	
AS BS-t	MT20	5.0	6.0	
BB				
BB TBMV1+p	MT20	3.0	8.0	2.75 0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-AN, N-AQ, M-AP, L-AQ, J-AT, P-AM, K-AR.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	VERT. LOAD (PLF)	FR-TO	VERT. LOAD (PLF)
BB-A	-71 / 0	AN-O	-254 / 0
A-B	0 / 9	AO-N	-216 / 0
B-C	0 / 23	AP-M	-165 / 0
C-D	0 / 30	AQ-L	-216 / 0
D-E	0 / 40	AT-J	-282 / 0
E-F	0 / 46	AU-I	-257 / 0
F-G	0 / 52	AV-H	-256 / 0
G-H	0 / 52	AW-F	-257 / 0
H-I	0 / 57	AX-E	-256 / 0
I-J	0 / 61	AY-D	-254 / 0
J-K	0 / 54	AZ-C	-265 / 0
K-L	0 / 61	BA-B	-224 / 0
L-M	0 / 60	AM-P	-282 / 0
M-N	0 / 60	AL-Q	-257 / 0
N-O	0 / 61	AJ-R	-256 / 0
O-P	0 / 54	AI-S	-257 / 0
P-Q	0 / 61	AH-T	-256 / 0
Q-R	0 / 57	AG-U	-273 / 0
R-S	0 / 52	AF-V	-160 / 0
S-T	0 / 46	AE-W	-212 / 0
T-U	0 / 39	AC-Z	-35 / 0
U-V	0 / 41	AR-K	-254 / 0
V-W	0 / 34		
W-X	0 / 34		
X-Y	0 / 35		
Y-Z	-18 / 0		
Z-AA	0 / 60		
AB-Z	-329 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (Z-AA:1), BC=0.01/1.00 (AG-AH:4), WB=0.27/1.00 (I-AU:1), SS=0.09/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (P) (INPUT = 0.90)

JSI METAL= 0.12 (P) (INPUT = 0.95)



Structural component only
DWG# T-2303416

CONTINUED ON PAGE 2

JOB NAME 418810	TRUSS NAME G56A	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			



Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:23 2023 Page 2
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NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

LOADING
TOTAL LOAD CASES: (4)

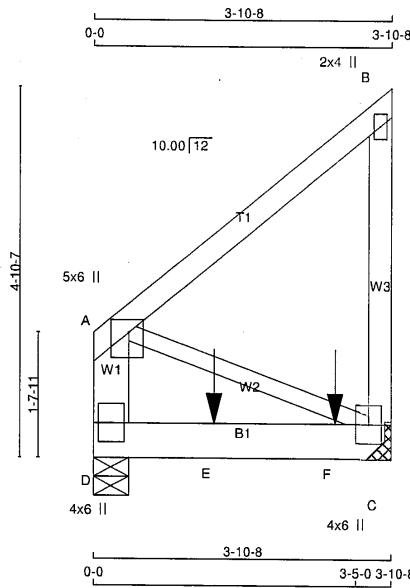
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM	TO		FR-TO		
AL-AK	-51 / 0	-18.5	-18.5 0.01 (4)	6.25			
AK-AJ	-51 / 0	-18.5	-18.5 0.01 (4)	6.25			
AJ-AI	-46 / 0	-18.5	-18.5 0.01 (4)	6.25			
AI-AH	-42 / 0	-18.5	-18.5 0.01 (4)	6.25			
AH-AG	-36 / 0	-18.5	-18.5 0.01 (4)	6.25			
AG-AF	-27 / 0	-18.5	-18.5 0.01 (4)	6.25			
AF-AE	-34 / 0	-18.5	-18.5 0.01 (4)	6.25			
AE-AD	-34 / 0	-18.5	-18.5 0.00 (4)	6.25			
AD-AC	-34 / 0	-18.5	-18.5 0.01 (4)	6.25			
AC-AB	0 / 0	-18.5	-18.5 0.01 (4)	10.00			



Structural component only
DWG# T-2303416

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T95	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Mar 22 09:26:54 2023 Page 1
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Scale = 1:27.3

TOTAL WEIGHT = 2 X 22 = 45 lb [M]

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	D - A	2x6	DRY	No.2	SPF
	A - B	2x4	DRY	No.2	SPF
	D - C	2x6	DRY	No.2	SPF
	ALL WEBS	2x4	DRY	No.2	SPF
	EXCEPT				
	A - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	WEBS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3') SPIRAL NAILS			
D-A	2	12	TOP
A-B	1	12	TOP
BOTTOM CHORDS: (0.122'X3') SPIRAL NAILS			
D-C	2	12	SIDE (0.0)
WEBS: (0.122'X3') SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

GRIPER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS

TOP COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PILES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMW+w	MT20	2.0	4.0		
C	BMWW+1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	DOWN	IN-SX	IN-SX
D	1051	0	5-8	5-8
C	1593	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS				
			SNOW	LIVE	PERM.LIVE	WIND	DEAD
D	729	559 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0
C	1105	848 / 0	0 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX.	MAX.	CS1 (LC)	UNBRAC LENGTH	FR-TO	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX.	CS1 (LC)
FR-TO														
D-A	-242 / 0		0.0	0.0	0.01 (1)	7.81				C-B	-232 / 0		0.03 (1)	
A-B	-7 / 0		-129.6	-129.6	0.16 (1)	10.00				A-C	0 / 5		0.00 (1)	
D-E	0 / 0		-18.5	-18.5	0.46 (1)	10.00								
E-F	0 / 0		-18.5	-18.5	0.46 (1)	10.00								
F-C	0 / 0		-18.5	-18.5	0.46 (1)	10.00								

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-6-12	-727	-727	---	FRONT	VERT	TOTAL	---	C1
F	3-1-12	-730	-730	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.16/1.00 (A-B:1), BC=0.46/1.00 (C-D:1), WB=0.03/1.00 (B-C:1), SS=0.45/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (B) (INPUT = 0.90)

JSI METAL= 0.06 (B) (INPUT = 0.95)



Structural component only
DWG# T-2303444

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T95	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:54 2023 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	4.0	6.0		

NOTES: (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2303444



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T97	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:55 2023 Page 2
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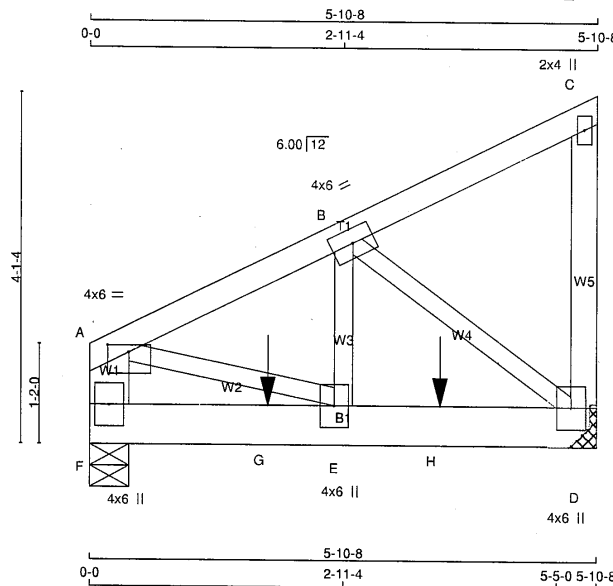
PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
D	BMWW1+t	MT20	4.0	6.0	
E	BMWW+t	MT20	4.0	6.0	
F	BMV1+p	MT20	4.0	6.0	

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2303445

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T97Z	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					



Scale = 1:24.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x6 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS EXCEPT D - C	2x3 DRY	No.2	SPF		
	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS (0.122"x3") SPIRAL NAILS		TOP
F - A 2	12	TOP
A - C 1	12	TOP
BOTTOM CHORDS (0.122"x3") SPIRAL NAILS		SIDE (0.0)
F - D 2	12	SIDE (0.0)
WEBS (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GRIPER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.0 INCH NAILS.

TOP COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
JT	1597	0	1597	0	5-8	IN-SX
F	1763	0	1763	0	MECHANICAL	
D						

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	1108	845/0	0/0	0/0	0/0	263/0	0/0
D	1224	933/0	0/0	0/0	0/0	290/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	MEMB.	WEBS		MAX. FACTORED
	FORCE (LBS)	VERT. LOAD LC1	MAX	CS1 (LC)			FORCE (LBS)	MAX	
FR-TO									
F-A	-1349/0	0.0	0.0	0.05 (1)	7.81	D-C	-142/0	0.01 (1)	
A-B	-1729/0	-129.6	-129.6	0.08 (1)	6.25	E-B	0/1596	0.20 (1)	
B-C	-18/0	-129.6	-129.6	0.07 (1)	6.25	A-E	0/1612	0.20 (1)	
						B-D	-1996/0	0.24 (1)	
F-G	0/0	-18.5	-18.5	0.17 (1)	10.00				
G-E	0/0	-18.5	-18.5	0.17 (1)	10.00				
E-H	0/1559	-18.5	-18.5	0.32 (1)	10.00				
H-D	0/1559	-18.5	-18.5	0.32 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-873	-873	---	FRONT	VERT	TOTAL	C1
H	4-0-12	-873	-873	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.08/1.00 (A-B:1), BC=0.32/1.00 (D-E:1),
WB=0.24/1.00 (B-D:1), SS=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (D) (INPUT = 0.90)
JSI METAL= 0.35 (D) (INPUT = 0.95)

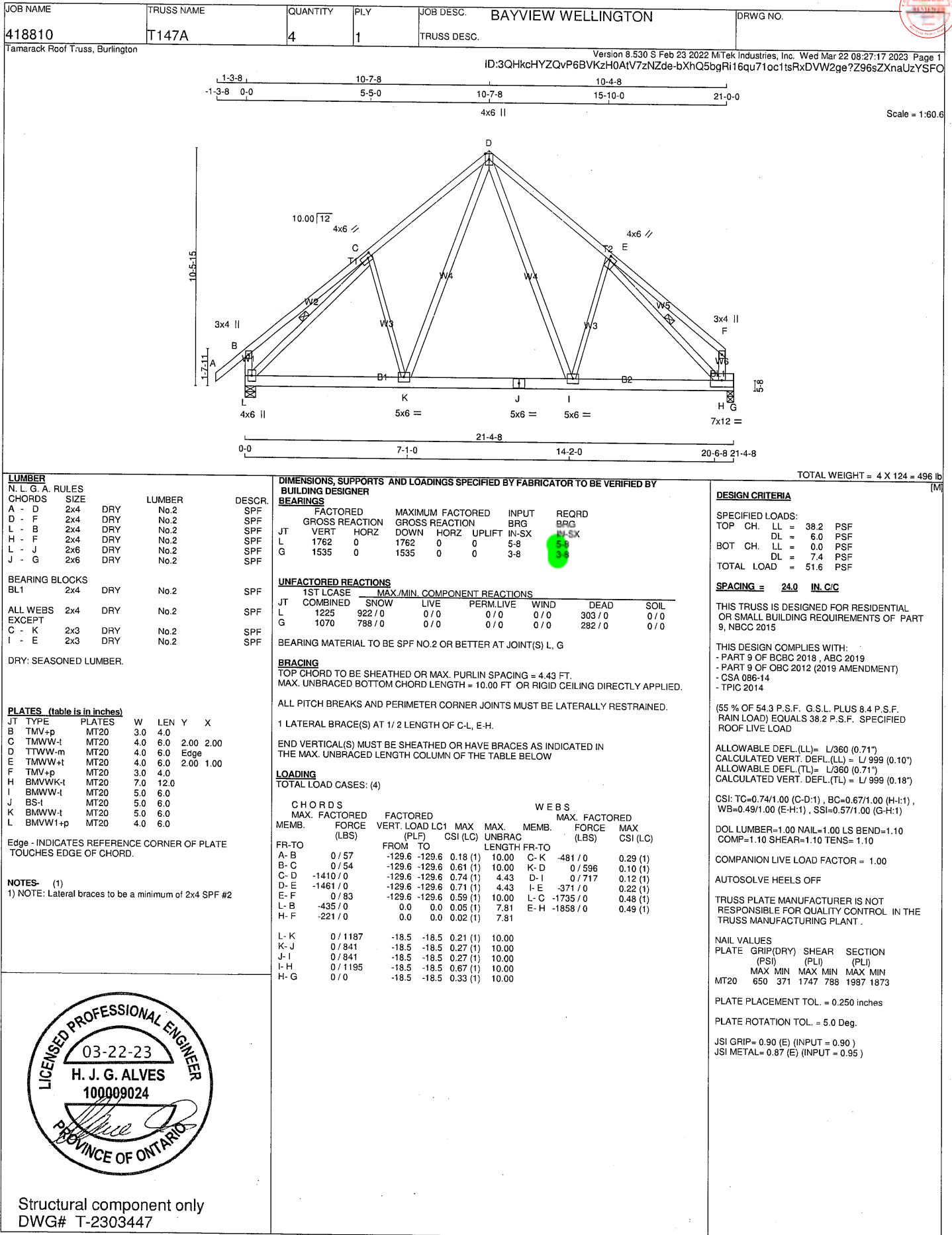


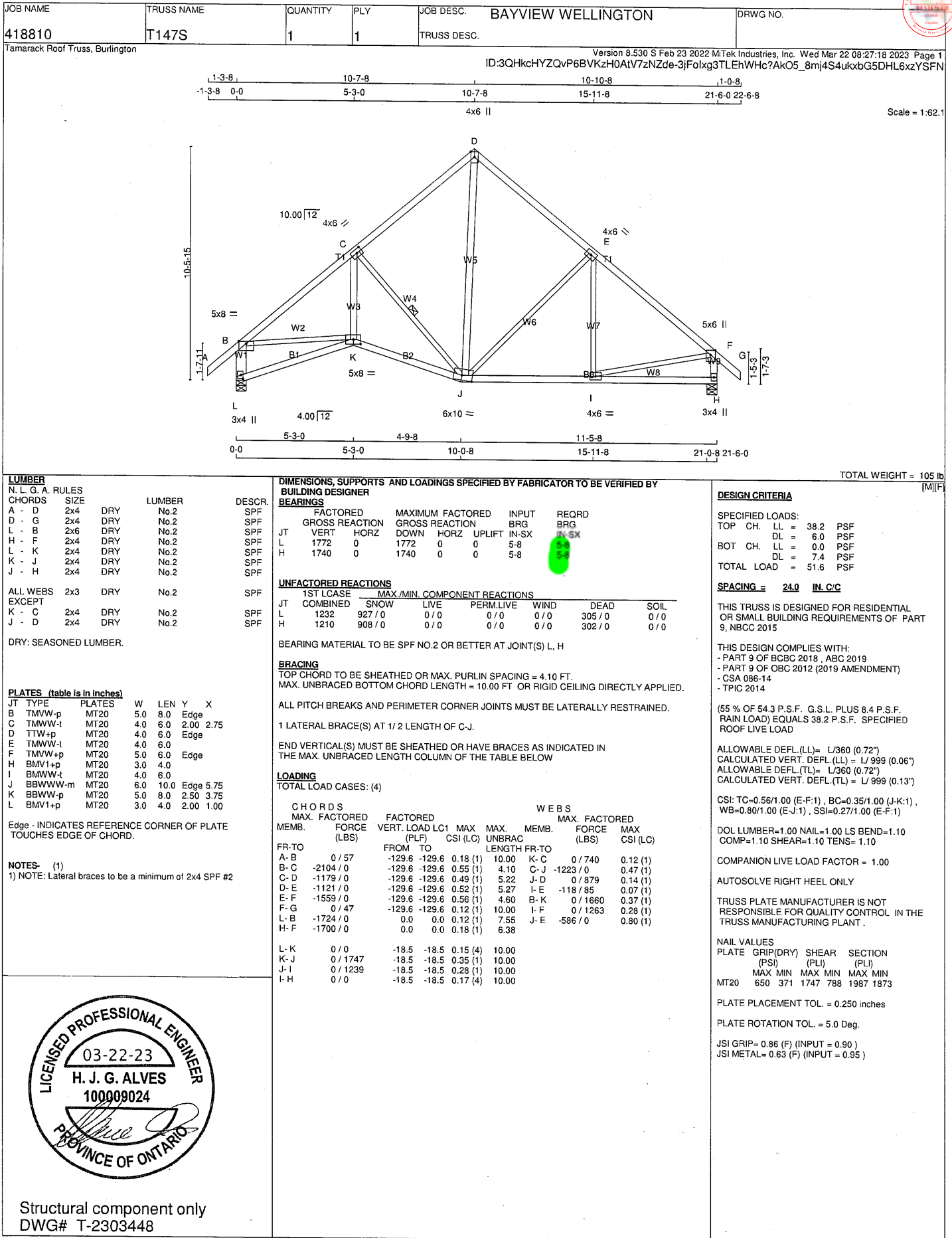
Structural component only
DWG# T-2303446

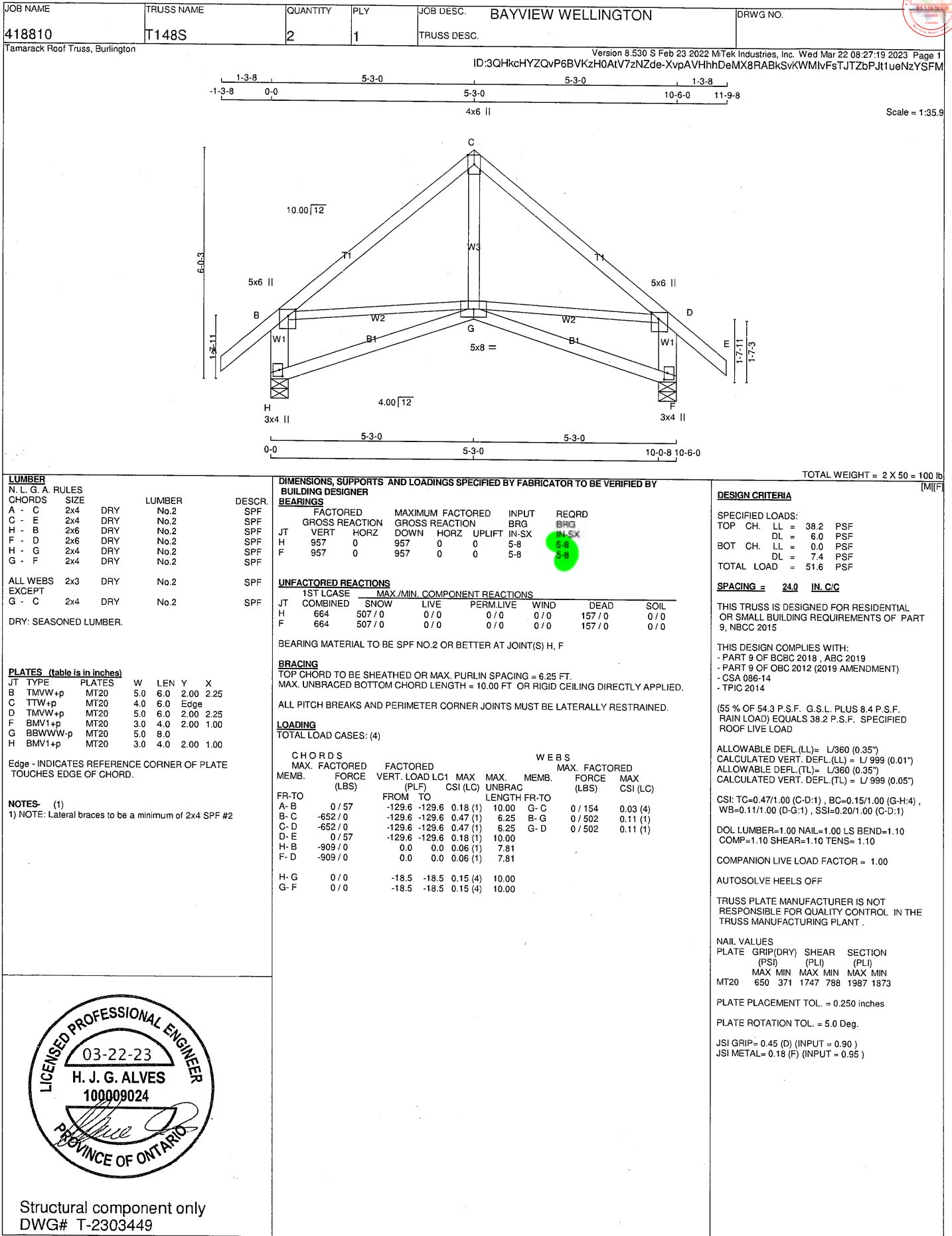
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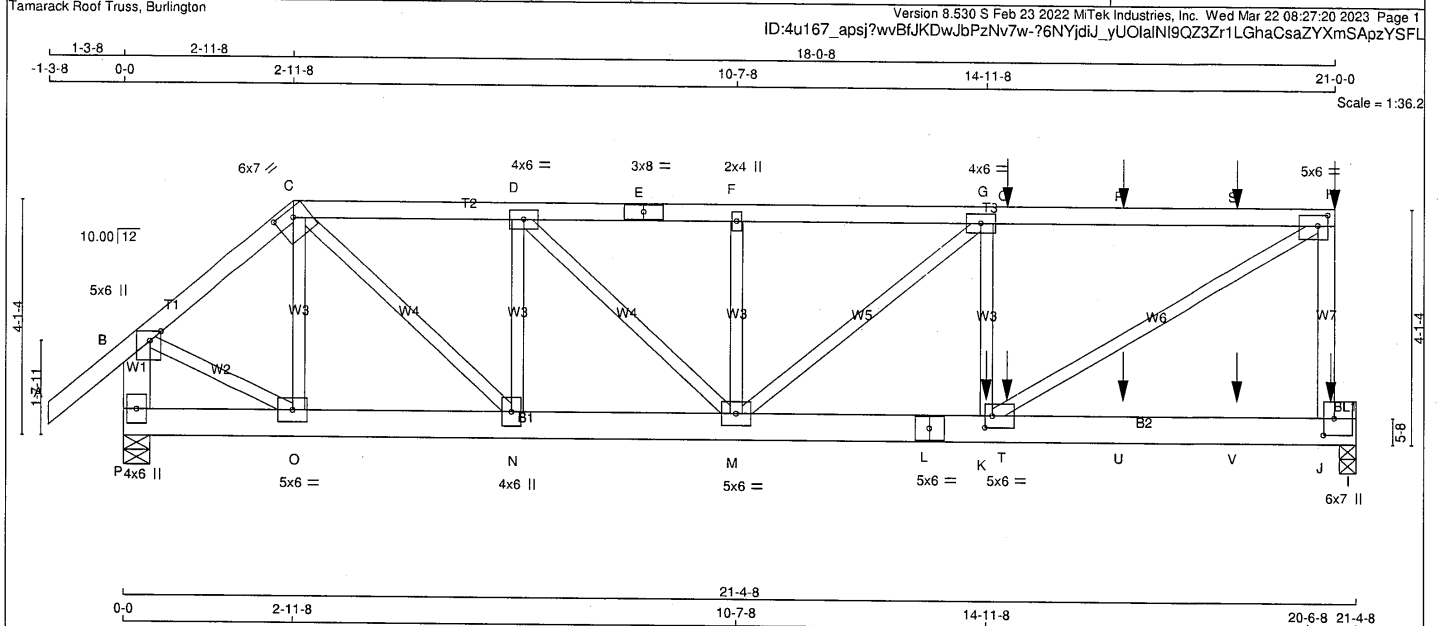
JOB NAME 418810	TRUSS NAME T97Z	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.																												
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:15 2023 Page 2 ID:j4cXy5eFullmbVHL?ZV kuzNZHa-f8ZfgweAAQs6fptQVcqOMW8JCF4jXjdqPF2hVczYSFQ																													
PLATES (table is in inches) <table><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>D</td><td>BMWW1+t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>E</td><td>BMWW+t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>F</td><td>BMV1+p</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr></tbody></table> NOTES: (1) 1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2						JT	TYPE	PLATES	W	LEN	Y	X	D	BMWW1+t	MT20	4.0	6.0			E	BMWW+t	MT20	4.0	6.0			F	BMV1+p	MT20	4.0	6.0		
JT	TYPE	PLATES	W	LEN	Y	X																											
D	BMWW1+t	MT20	4.0	6.0																													
E	BMWW+t	MT20	4.0	6.0																													
F	BMV1+p	MT20	4.0	6.0																													
 Structural component only DWG# T-2303446																																	







JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T150Z2	1	2	BAYVIEW WELLINGTON	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - E 2x4 DRY No.2 SPF E - H 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF P - B 2x6 DRY No.2 SPF L - I 2x6 DRY No.2 SPF BEARING BLOCKS BL1 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS & ROWS SURFACE LOADS (PLF) SPACING (IN) TOP CHORDS (0.122'X3') SPIRAL NAILS A - C 1 12 TOP C - E 1 12 TOP E - H 1 12 SIDE (61.0) H - J 1 12 TOP P - B 2 12 TOP BOTTOM CHORDS (0.122'X3') SPIRAL NAILS P - L 2 12 TOP L - I 2 12 SIDE (183.1) WEBS (0.122'X3') SPIRAL NAILS G - K 1 6 SIDE (272.0) 2x3 1 6 2x4 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. ORDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQ'D GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX P 2395 0 2395 0 0 5-8 5-8 I 3443 0 3443 0 0 3-8 3-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL P 1664 1257 / 0 0 / 0 0 / 0 407 / 0 0 / 0 I 2396 1791 / 0 0 / 0 0 / 0 606 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC) MAX. UNBRAC LENGTH FR-TO WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. CSI (LC) A-B 0 / 57 FROM TO B-C -2117 / 0 -129.6 -129.6 0.10 (1) 10.00 O-C -557 / 0 0.07 (1) C-D -3187 / 0 -129.6 -129.6 0.12 (1) 5.96 M-F -350 / 0 0.04 (1) D-E -4028 / 0 -129.6 -129.6 0.20 (1) 5.00 M-G -894 / 0 0.25 (1) E-F -4028 / 0 -129.6 -129.6 0.22 (1) 4.53 K-H -607 / 0 0.08 (1) F-G -4028 / 0 -129.6 -129.6 0.22 (1) 4.53 K-H 0 / 5699 0.71 (1) G-H -4028 / 0 -129.6 -129.6 0.55 (1) 4.04 B-O 0 / 1744 0.22 (1) H-I -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 N-D -1406 / 0 0.18 (1) I-J -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 C-N 0 / 2187 0.27 (1) J-K -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 D-M 0 / 1182 0.15 (1) K-L -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 L-M -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 M-N -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 N-O -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 O-P -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 P-Q -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 Q-R -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 R-S -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 S-T -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 T-U -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 U-V -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 V-W -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 W-X -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 X-Y -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 Y-Z -4706 / 0 -129.6 -129.6 0.70 (1) 3.66 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN. H 21-0-0 -148 -148 --- BACK VERT TOTAL --- C1 J 20-11-4 -25 -25 --- BACK VERT TOTAL --- C1 K 14-11-8 -1209 -1209 --- BACK VERT TOTAL --- C1 O 15-3-12 -106 -106 --- BACK VERT TOTAL --- C1 R 17-3-12 -106 -106 --- BACK VERT TOTAL --- C1 S 19-3-12 -106 -106 --- BACK VERT TOTAL --- C1 T 15-3-12 -21 -21 --- BACK VERT TOTAL --- C1 U 17-3-12 -21 -21 --- BACK VERT TOTAL --- C1 V 19-3-12 -21 -21 --- BACK VERT TOTAL --- C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.2 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 51.6 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.71") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.12") ALLOWABLE DEFL.(TL)= L/360 (0.71") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.21") CSI: TC=0.70/1.00 (G-H-I), BC=0.66/1.00 (J-K-I), WB=0.71/1.00 (H-K-I), SSI=0.70/1.00 (I-J-I) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.88 (H) (INPUT = 0.90) JSI METAL= 0.55 (K) (INPUT = 0.95)			
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Structural component only
DWG# T-2303450



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T150Z2	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Mar 22 08:27:20 2023 Page 2
ID:4u167 apsj?wvBfJKDwJbPzNv7w-76NYdiJ_yUOlalNI9QZ3Zr1LGhaCsaZYXmSApZYSFL

PLATES (table is in inches)

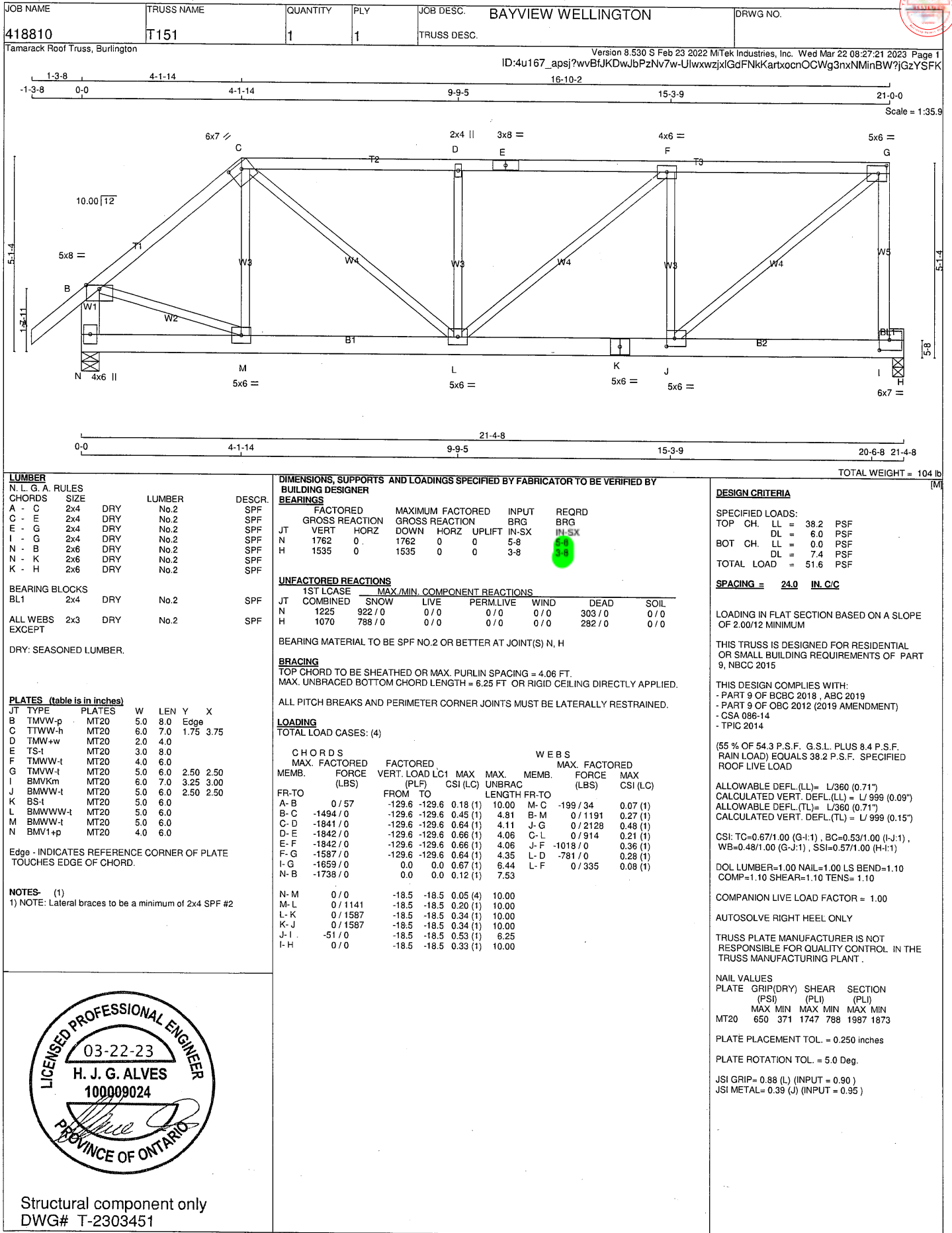
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-h	MT20	6.0	7.0	1.75	3.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.25	2.00
J	BMVK+p	MT20	6.0	7.0	3.50	2.25
K	BMWW-t	MT20	5.0	6.0	2.50	1.50
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	4.0	6.0		

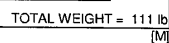
NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2303450





NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
<u>BEARINGS</u>						
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	1535	0	1535	0	3-8	3-8
H	1762	0	1762	0	5-8	5-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1070	788 / 0	0 / 0	0 / 0	0 / 0	282 / 0	0 / 0
N	1225	922 / 0	0 / 0	0 / 0	0 / 0	303 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H. N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 57	-129.6	-129.6 0.18 (1)	10.00	M-D	0 / 91	0.03 (4)
B-C	0 / 22	-129.6	-129.6 0.13 (1)	10.00	J-G	0 / 1904	0.43 (1)
C-D	-1489 / 0	-129.6	-129.6 0.16 (1)	5.21	D-L	0 / 590	0.13 (1)
D-E	-1522 / 0	-129.6	-129.6 0.52 (1)	4.64	L-E	-1047 / 0	0.57 (1)
E-F	-1522 / 0	-129.6	-129.6 0.54 (1)	4.60	L-F	-725 / 0	0.40 (1)
F-G	-1251 / 0	-129.6	-129.6 0.53 (1)	4.99	L-F	0 / 407	0.09 (1)
I-G	-1677 / 0	0.0	0.0 0.29 (1)	5.13	C-M	0 / 53	0.02 (4)
N-B	-319 / 0	0.0	0.0 0.03 (1)	7.81	N-C	-1776 / 0	0.52 (4)
N-M	0 / 1089	-18.5	-18.5 0.16 (1)	10.00			
M-L	0 / 1124	-18.5	-18.5 0.19 (1)	10.00			
L-K	0 / 1251	-18.5	-18.5 0.30 (1)	10.00			
K-J	0 / 1251	-18.5	-18.5 0.30 (1)	10.00			
J-I	-43 / 0	-18.5	-18.5 0.53 (1)	6.25			
I-H	0 / 0	-18.5	-18.5 0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL =	38.2	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.53/1.00 (I-J:1) ,
WB=0.57/1.00 (F-J:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

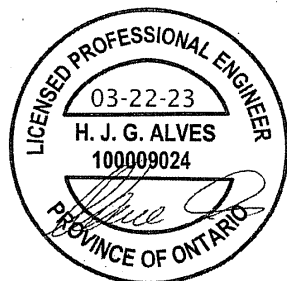
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

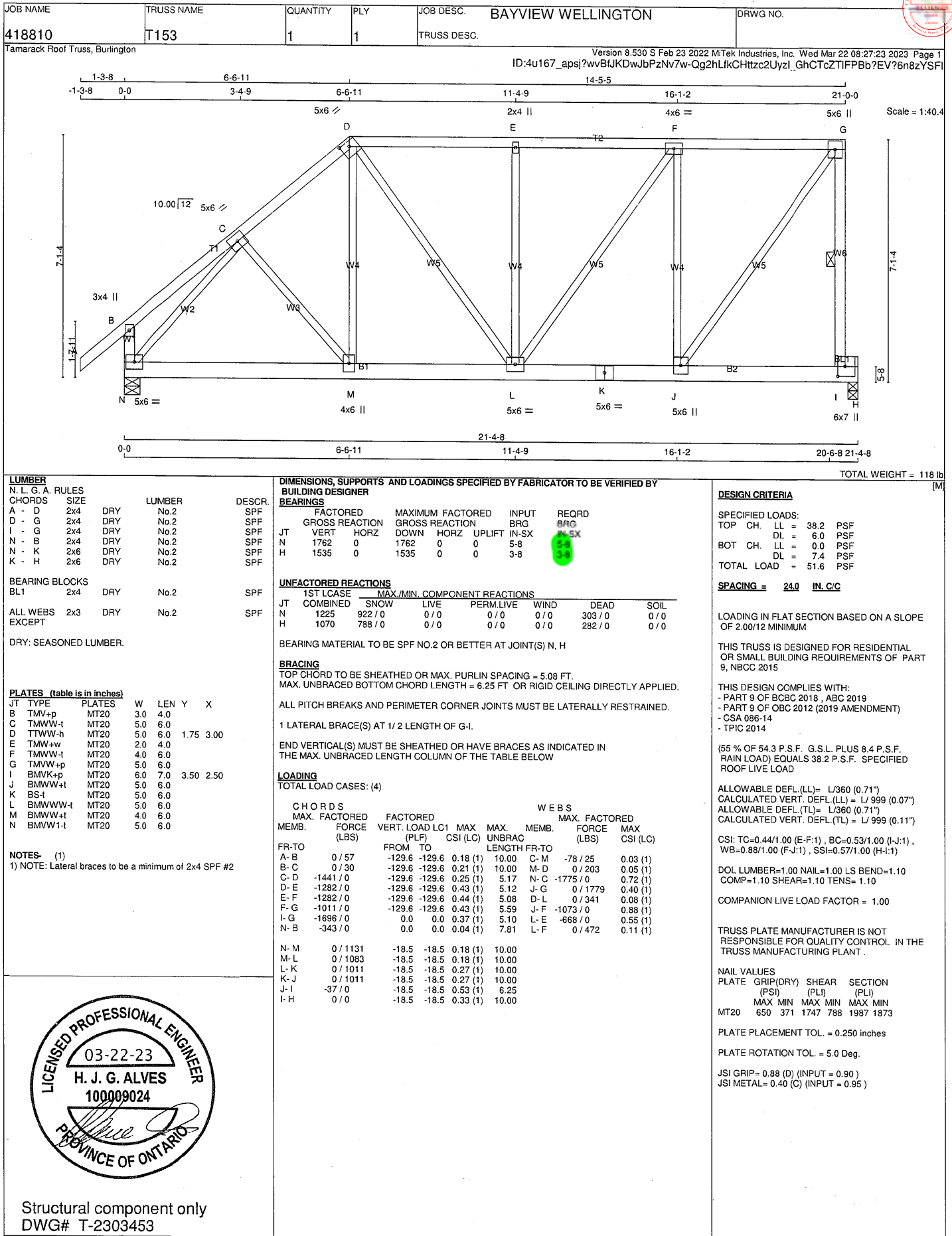
PLATE PLACEMENT TOL. = 0.250 inches

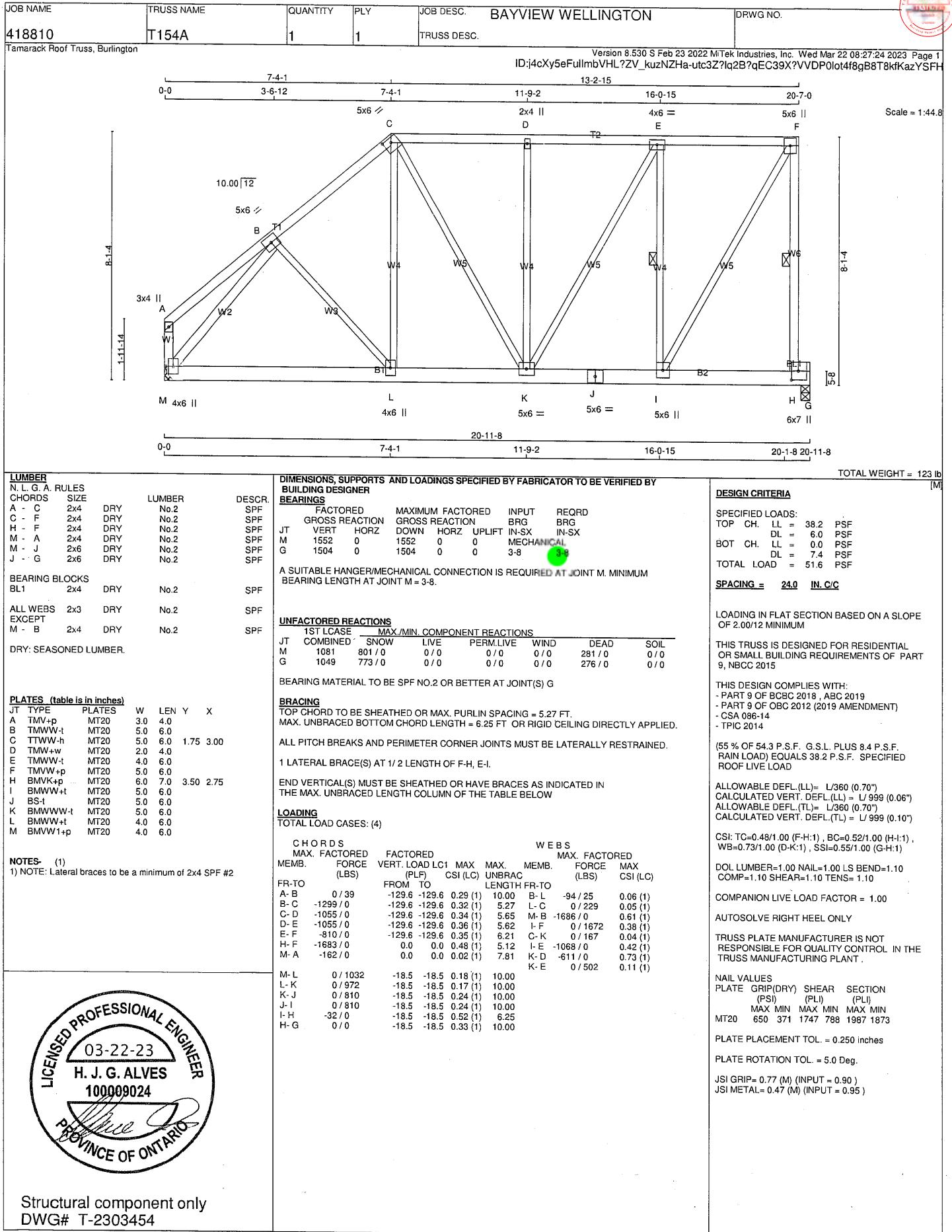
PLATE ROTATION TOL. = 5.0 Deg.

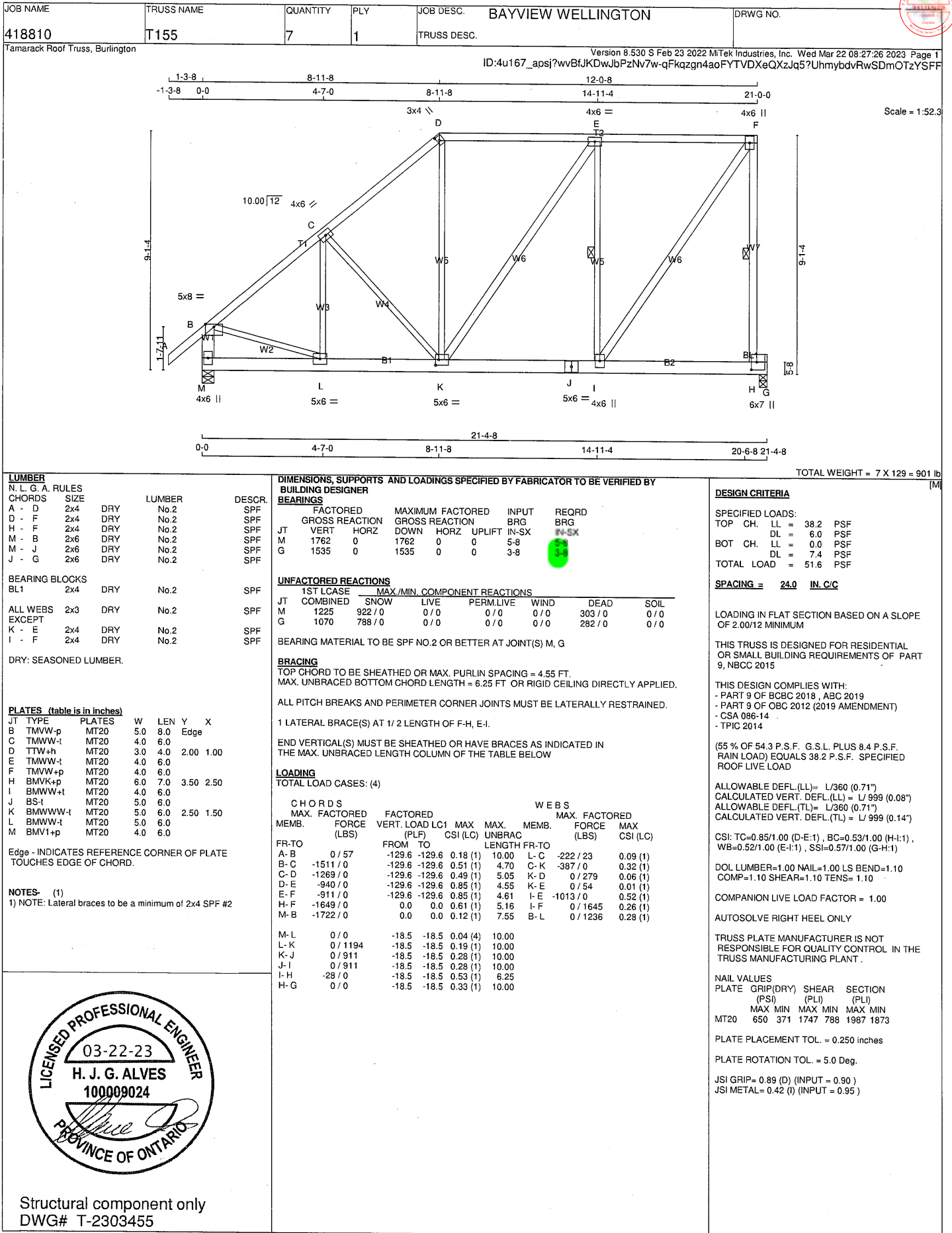
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JSI METAL= 0.49 (N) (INPUT = 0.95)

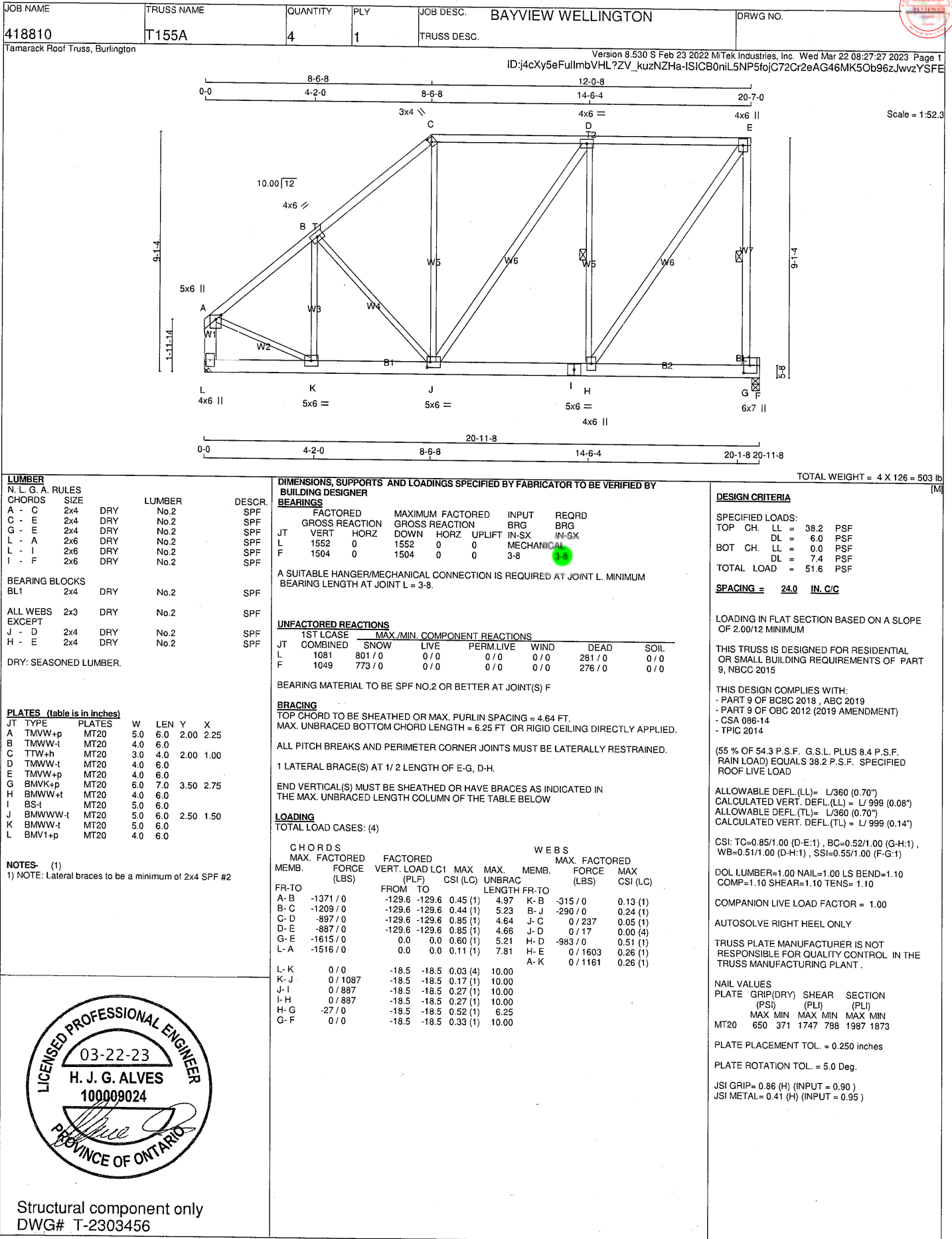


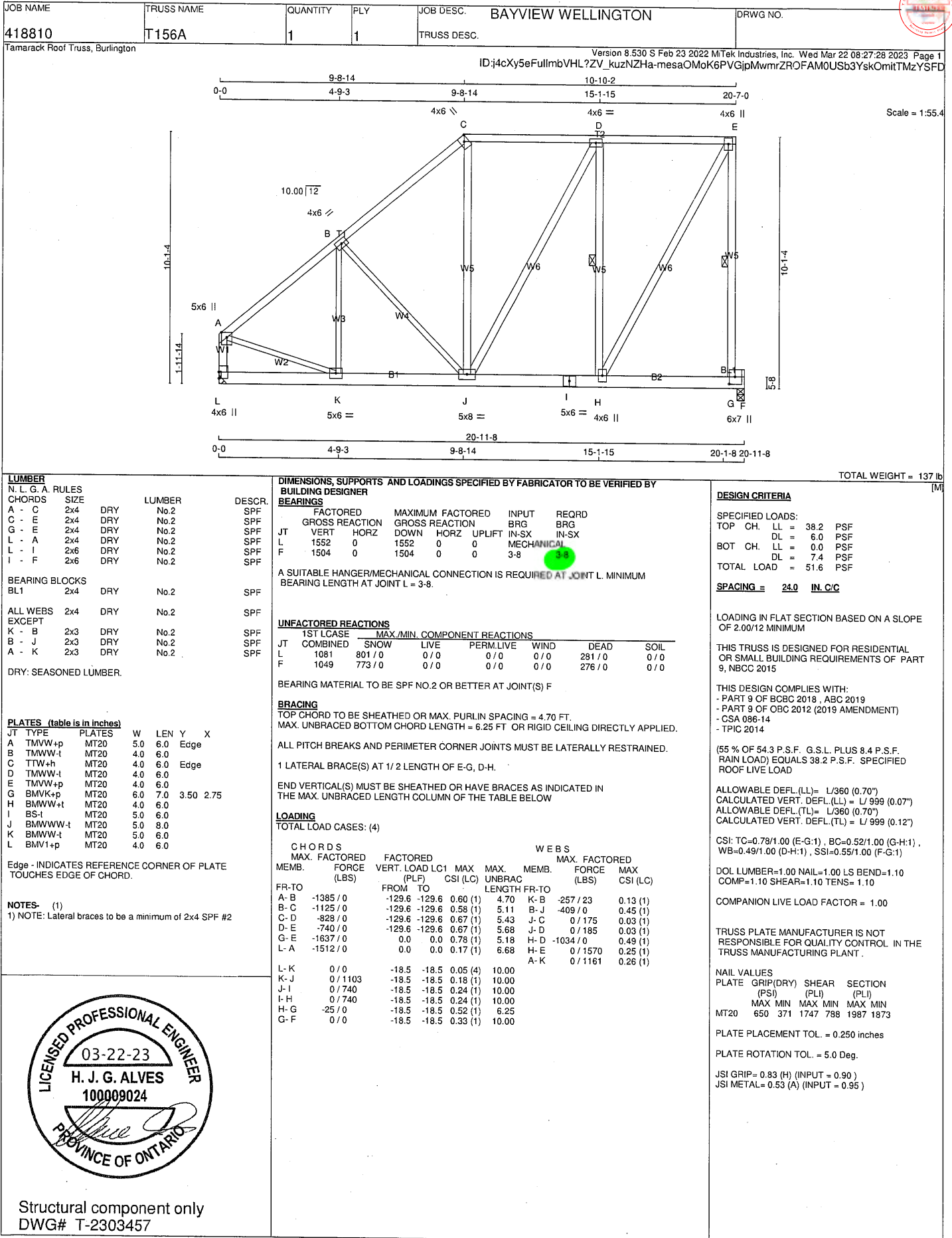
Structural component only
DWG# T-2303452

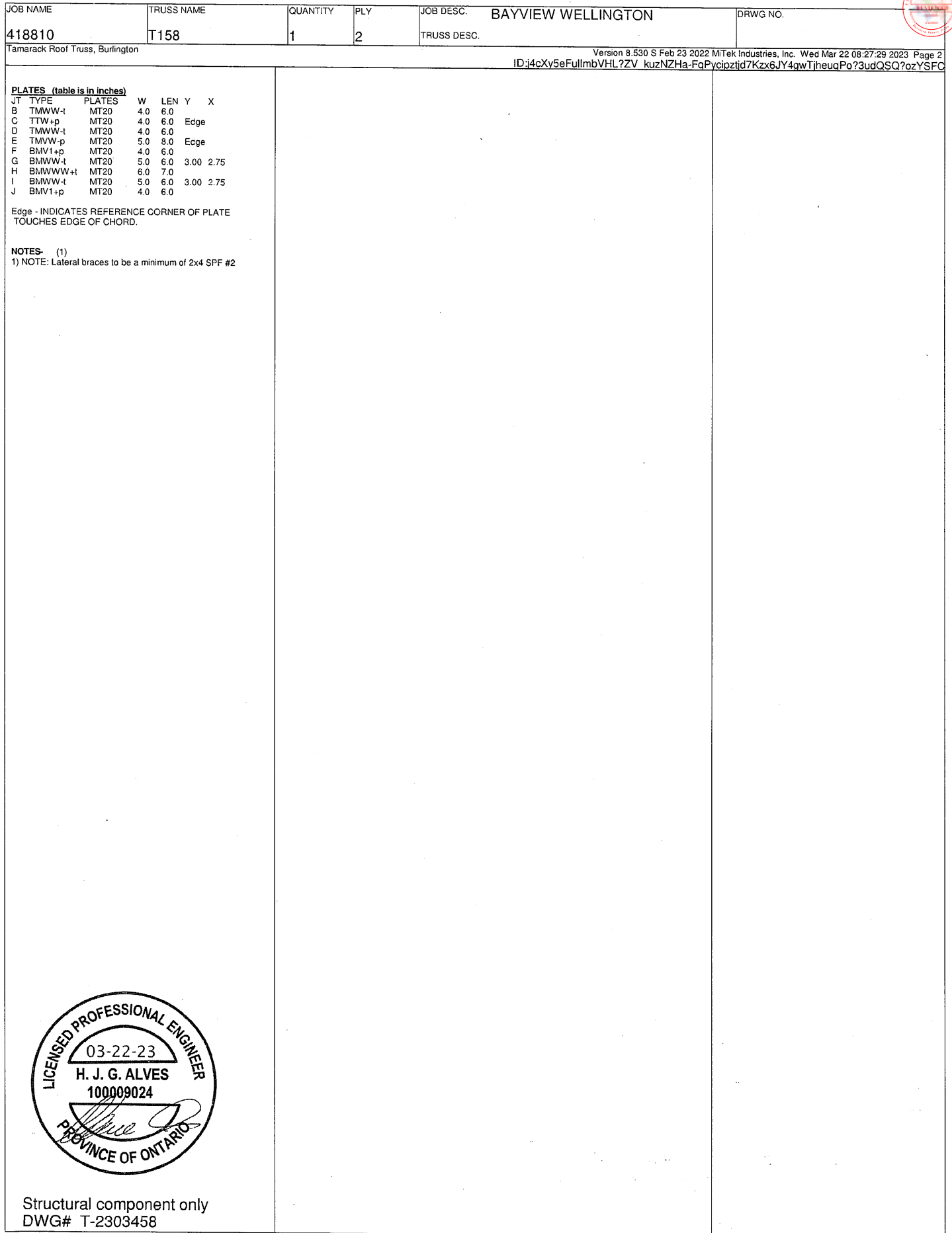












JOB NAME
418810

Tamarack Roof Truss, Burlington

TRUSS NAME
G158

QUANTITY
1

PLY
1

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:24 2023 Page 1

ID:j4cXy5eFullmbVHL?ZV_kuzNZHa-eK_Nzu1BAIRL1_Td0PQqolXbVhky4hBJgpCUD3zYSGD

Scale = 1:35.6

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED UNBRACED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
P-B	-336 / 0	0.0 0.0 0.03 (1)	7.81	M-E	-181 / 0
A-B	0 / 47	-129.6 -129.6 0.12 (1)	10.00	N-D	-309 / 0
B-C	-72 / 0	-129.6 -129.6 0.11 (1)	6.25	O-C	-149 / 0
C-D	-22 / 0	-129.6 -129.6 0.08 (1)	6.25	L-F	-309 / 0
D-E	-44 / 0	-129.6 -129.6 0.08 (1)	6.25	K-G	-149 / 0
E-F	-44 / 0	-129.6 -129.6 0.08 (1)	6.25	B-O	0 / 41
F-G	-22 / 0	-129.6 -129.6 0.08 (1)	6.25	K-H	0 / 41
G-H	-72 / 0	-129.6 -129.6 0.11 (1)	6.25		
H-I	0 / 47	-129.6 -129.6 0.12 (1)	10.00		
J-H	-336 / 0	0.0 0.0 0.03 (1)	7.81		
P-O	0 / 0	-18.5 -18.5 0.01 (4)	10.00		
O-N	0 / 27	-18.5 -18.5 0.02 (4)	10.00		
N-M	0 / 19	-18.5 -18.5 0.02 (4)	10.00		
M-L	0 / 19	-18.5 -18.5 0.02 (4)	10.00		
L-K	0 / 27	-18.5 -18.5 0.02 (4)	10.00		
K-J	0 / 0	-18.5 -18.5 0.01 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 096-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (H-I:1), BC=0.02/1.00 (N-O:4), WB=0.10/1.00 (E-M:1), SSI=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.22 (F) (INPUT = 0.90)

JSI METAL = 0.16 (D) (INPUT = 0.95)

PLATES (table is in inches)

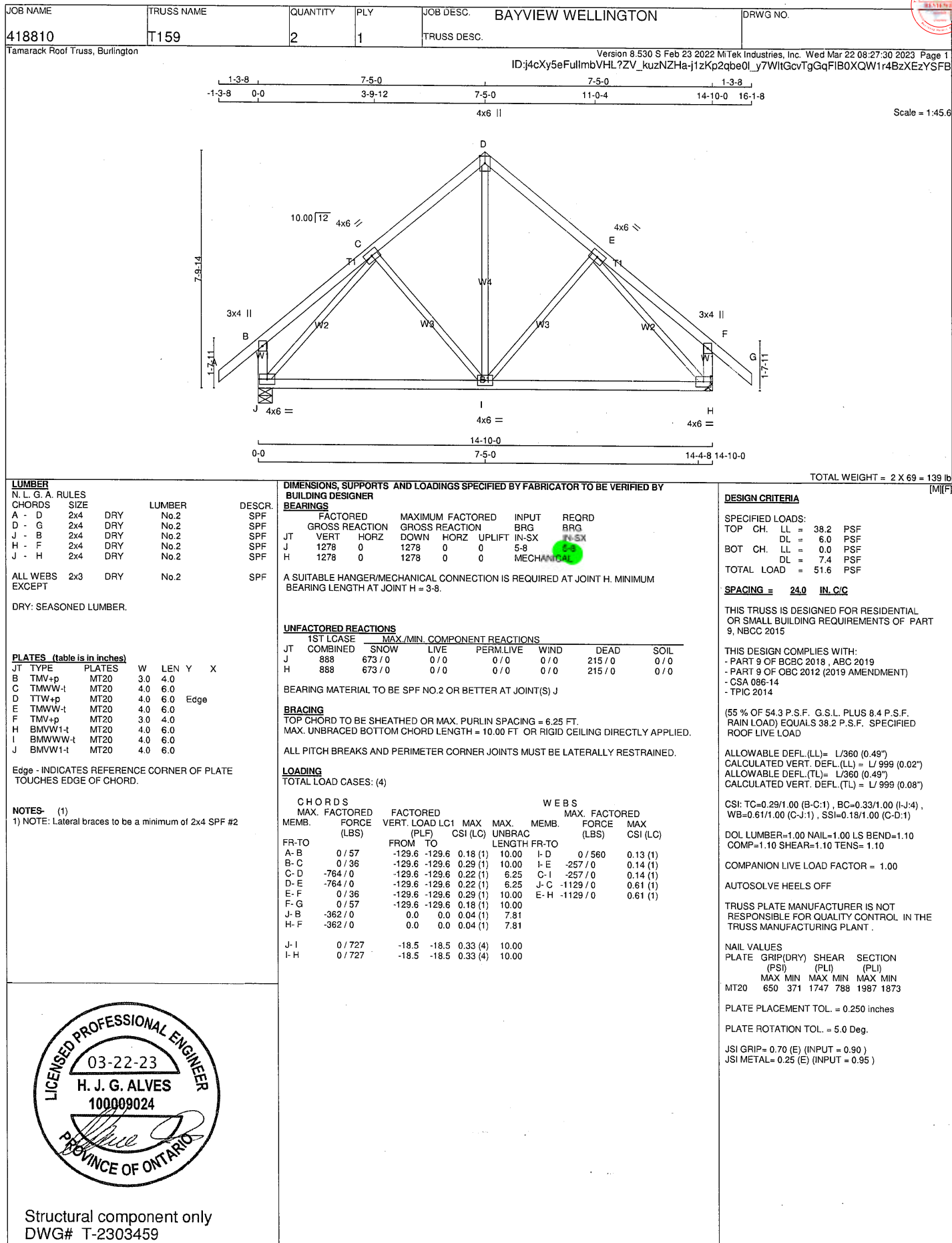
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G					
C TMW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	6.0	Edge	
H TMVW+p	MT20	4.0	6.0	Edge	
J BMV1+p	MT20	3.0	4.0		
K BMWV1-t	MT20	4.0	6.0		
L, M, N					
L BMW1+w	MT20	2.0	4.0		
O BMWV1-t	MT20	4.0	6.0		
P BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

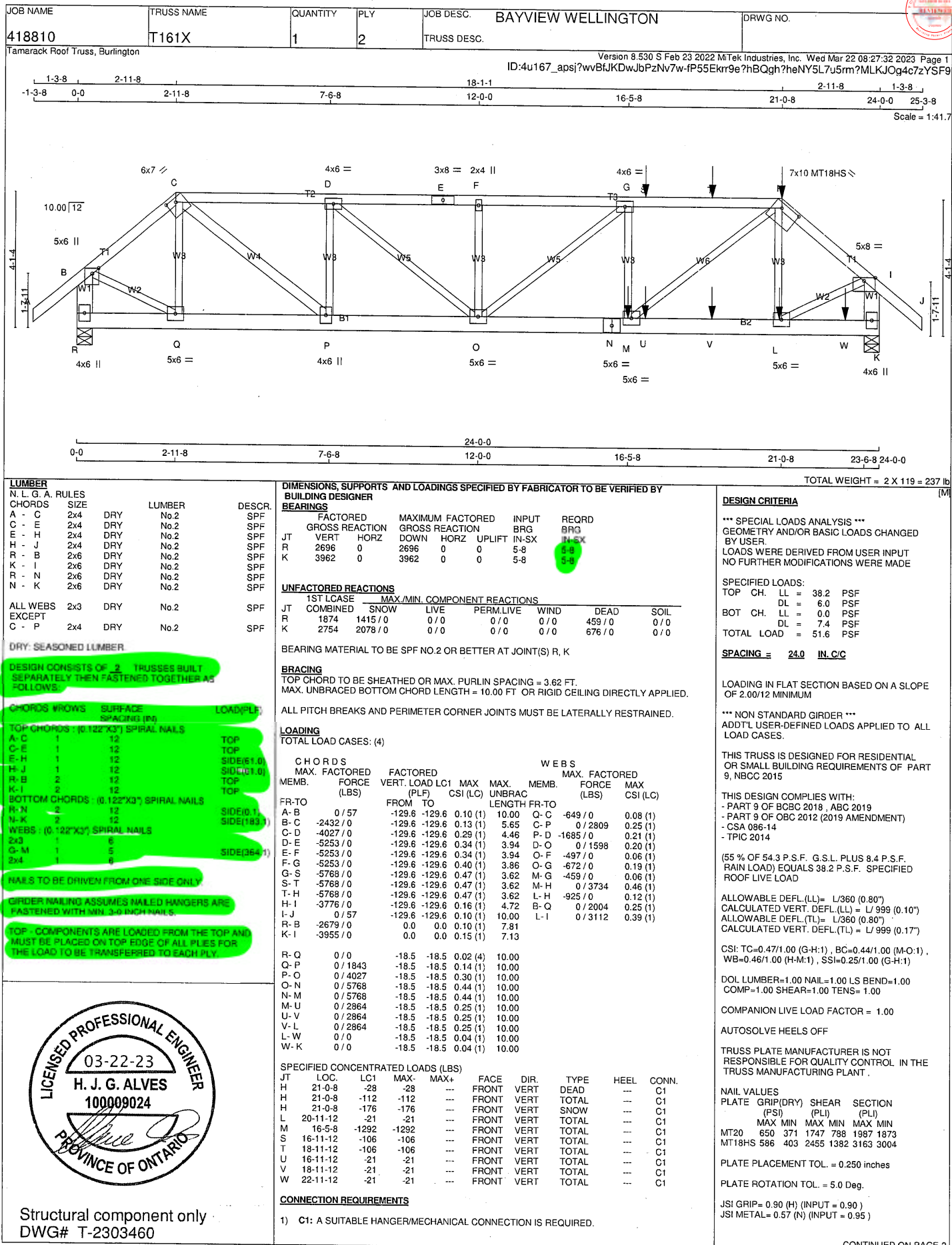
NOTES: (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

Structural component only
DWG# T-2303417



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																																																																																																																																																
418810	G159	1	1	BAYVIEW WELLINGTON																																																																																																																																																																	
Tamarack Roof Truss, Burlington																																																																																																																																																																					
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:25 2023 Page 1																																																																																																																																																																					
ID:j4cXy5eFullmbVHL?ZV_kuzNZHa-6WYmAq1px2ZCf82pa6xfKz3IM54np6dTvtX1IVzYSGC																																																																																																																																																																					
Scale = 1:46.2																																																																																																																																																																					
TOTAL WEIGHT = 74 lb																																																																																																																																																																					
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER T - B 2x4 DRY No.2 A - F 2x4 DRY No.2 F - K 2x4 DRY No.2 L - J 2x4 DRY No.2 T - L 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 ALL GABLE WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2'-0" OC.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4)		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.2 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 51.6 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.18/1.00 (A-B:1), BC=0.02/1.00 (M-N:4), WB=0.21/1.00 (F-P:1), SSI=0.11/1.00 (A-B:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.25 (B) (INPUT = 0.90) JSI METAL= 0.15 (E) (INPUT = 0.95)																																																																																																																																																																	
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW+p MT20 4.0 6.0 Edge C, D, E, G, H, I C TMW+w MT20 2.0 4.0 F TTW+p MT20 4.0 6.0 Edge J TMVW+p MT20 4.0 6.0 Edge L BMV1+p MT20 3.0 4.0 M BMWV1-t MT20 4.0 6.0 N, O, P, Q, R N BMV1+w MT20 2.0 4.0 S BMWV1-t MT20 4.0 6.0 T BMV1+p MT20 3.0 4.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES: (1) 1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2		<table border="1"><thead><tr><th colspan="2">CHORDS</th><th colspan="2">FACTORED</th><th colspan="2">WEBS</th></tr><tr><th>MEMB.</th><th>FORCE (LBS)</th><th>VERT. LOAD (PLF)</th><th>MAX. CSI (LC)</th><th>MEMB.</th><th>FORCE (LBS)</th><th>MAX. CSI (LC)</th></tr></thead><tbody><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td></tr><tr><td>T-B</td><td>-402 / 0</td><td>0.0 0.0 0.04 (1)</td><td>7.81</td><td>P-F</td><td>-194 / 0</td><td>0.21 (1)</td></tr><tr><td>A-B</td><td>0 / 57</td><td>-129.6 -129.6 0.18 (1)</td><td>10.00</td><td>Q-E</td><td>-291 / 0</td><td>0.17 (1)</td></tr><tr><td>B-C</td><td>-93 / 0</td><td>-129.6 -129.6 0.17 (1)</td><td>6.25</td><td>R-D</td><td>-267 / 0</td><td>0.08 (1)</td></tr><tr><td>C-D</td><td>-24 / 0</td><td>-129.6 -129.6 0.06 (1)</td><td>6.25</td><td>S-C</td><td>-113 / 0</td><td>0.02 (1)</td></tr><tr><td>D-E</td><td>-24 / 0</td><td>-129.6 -129.6 0.07 (1)</td><td>6.25</td><td>O-G</td><td>-291 / 0</td><td>0.17 (1)</td></tr><tr><td>E-F</td><td>-37 / 0</td><td>-129.6 -129.6 0.07 (1)</td><td>6.25</td><td>N-H</td><td>-267 / 0</td><td>0.08 (1)</td></tr><tr><td>F-G</td><td>-37 / 0</td><td>-129.6 -129.6 0.07 (1)</td><td>6.25</td><td>M-I</td><td>-113 / 0</td><td>0.02 (1)</td></tr><tr><td>G-H</td><td>-24 / 0</td><td>-129.6 -129.6 0.07 (1)</td><td>6.25</td><td>B-S</td><td>0 / 42</td><td>0.01 (1)</td></tr><tr><td>H-I</td><td>-24 / 0</td><td>-129.6 -129.6 0.06 (1)</td><td>6.25</td><td>M-J</td><td>0 / 42</td><td>0.01 (1)</td></tr><tr><td>I-J</td><td>-93 / 0</td><td>-129.6 -129.6 0.17 (1)</td><td>6.25</td><td></td><td></td><td></td></tr><tr><td>J-K</td><td>0 / 57</td><td>-129.6 -129.6 0.18 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>L-J</td><td>-402 / 0</td><td>0.0 0.0 0.04 (1)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>T-S</td><td>0 / 0</td><td>-18.5 -18.5 0.01 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>S-R</td><td>0 / 26</td><td>-18.5 -18.5 0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>R-Q</td><td>0 / 20</td><td>-18.5 -18.5 0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>Q-P</td><td>0 / 15</td><td>-18.5 -18.5 0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>P-O</td><td>0 / 15</td><td>-18.5 -18.5 0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>O-N</td><td>0 / 20</td><td>-18.5 -18.5 0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>N-M</td><td>0 / 26</td><td>-18.5 -18.5 0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>M-L</td><td>0 / 0</td><td>-18.5 -18.5 0.01 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></tbody></table>				CHORDS		FACTORED		WEBS		MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	FR-TO		FROM TO		FR-TO			T-B	-402 / 0	0.0 0.0 0.04 (1)	7.81	P-F	-194 / 0	0.21 (1)	A-B	0 / 57	-129.6 -129.6 0.18 (1)	10.00	Q-E	-291 / 0	0.17 (1)	B-C	-93 / 0	-129.6 -129.6 0.17 (1)	6.25	R-D	-267 / 0	0.08 (1)	C-D	-24 / 0	-129.6 -129.6 0.06 (1)	6.25	S-C	-113 / 0	0.02 (1)	D-E	-24 / 0	-129.6 -129.6 0.07 (1)	6.25	O-G	-291 / 0	0.17 (1)	E-F	-37 / 0	-129.6 -129.6 0.07 (1)	6.25	N-H	-267 / 0	0.08 (1)	F-G	-37 / 0	-129.6 -129.6 0.07 (1)	6.25	M-I	-113 / 0	0.02 (1)	G-H	-24 / 0	-129.6 -129.6 0.07 (1)	6.25	B-S	0 / 42	0.01 (1)	H-I	-24 / 0	-129.6 -129.6 0.06 (1)	6.25	M-J	0 / 42	0.01 (1)	I-J	-93 / 0	-129.6 -129.6 0.17 (1)	6.25				J-K	0 / 57	-129.6 -129.6 0.18 (1)	10.00				L-J	-402 / 0	0.0 0.0 0.04 (1)	7.81				T-S	0 / 0	-18.5 -18.5 0.01 (4)	10.00				S-R	0 / 26	-18.5 -18.5 0.02 (4)	10.00				R-Q	0 / 20	-18.5 -18.5 0.02 (4)	10.00				Q-P	0 / 15	-18.5 -18.5 0.02 (4)	10.00				P-O	0 / 15	-18.5 -18.5 0.02 (4)	10.00				O-N	0 / 20	-18.5 -18.5 0.02 (4)	10.00				N-M	0 / 26	-18.5 -18.5 0.02 (4)	10.00				M-L	0 / 0	-18.5 -18.5 0.01 (4)	10.00			
CHORDS		FACTORED		WEBS																																																																																																																																																																	
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R-Q	0 / 20	-18.5 -18.5 0.02 (4)	10.00																																																																																																																																																																		
Q-P	0 / 15	-18.5 -18.5 0.02 (4)	10.00																																																																																																																																																																		
P-O	0 / 15	-18.5 -18.5 0.02 (4)	10.00																																																																																																																																																																		
O-N	0 / 20	-18.5 -18.5 0.02 (4)	10.00																																																																																																																																																																		
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M-L	0 / 0	-18.5 -18.5 0.01 (4)	10.00																																																																																																																																																																		
		Structural component only DWG# T-2303418																																																																																																																																																																			



JOB NAME

418810

TRUSS NAME

T161X

QUANTITY

1

PLY

2

JOB DESC.

BAYVIEW WELLINGTON

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:32 2023 Page 2

ID:4u167_apsi?vvBfJKDwJbPzNv7w-fP55Ekrr9e?hBQgh?heNY5L7u5rm?MLKJQg4c7zYSF9

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-h	MT20	6.0	7.0	1.50	3.50
D	TMVW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	TTWW-h	MT18HS	7.0	10.0	1.75	6.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	4.0	6.0		
L, M, Q						
L	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMVW-t	MT20	4.0	6.0		
R	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE

TOUCHES EDGE OF CHORD.

NOTES: (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER

03-22-23

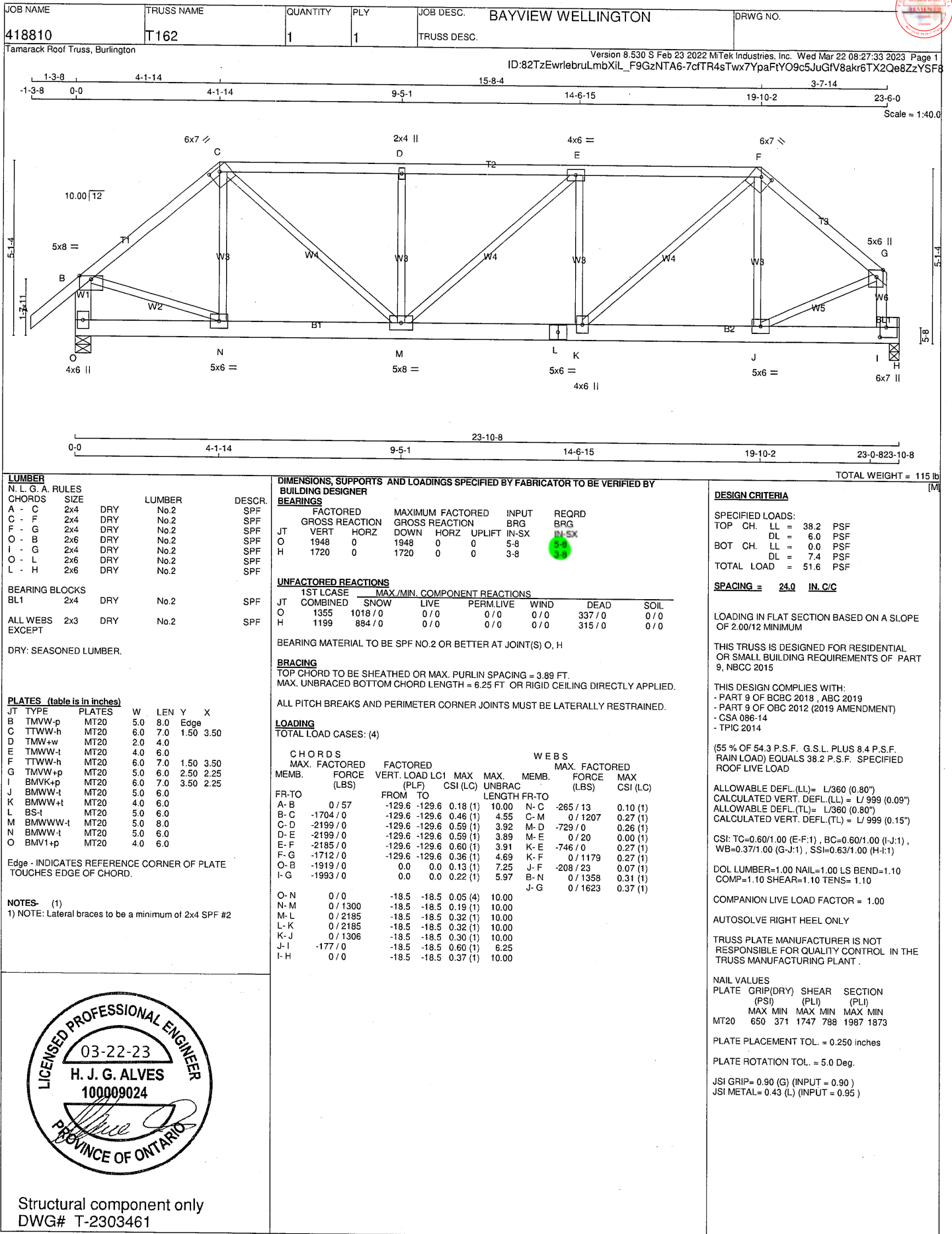
H. J. G. ALVES

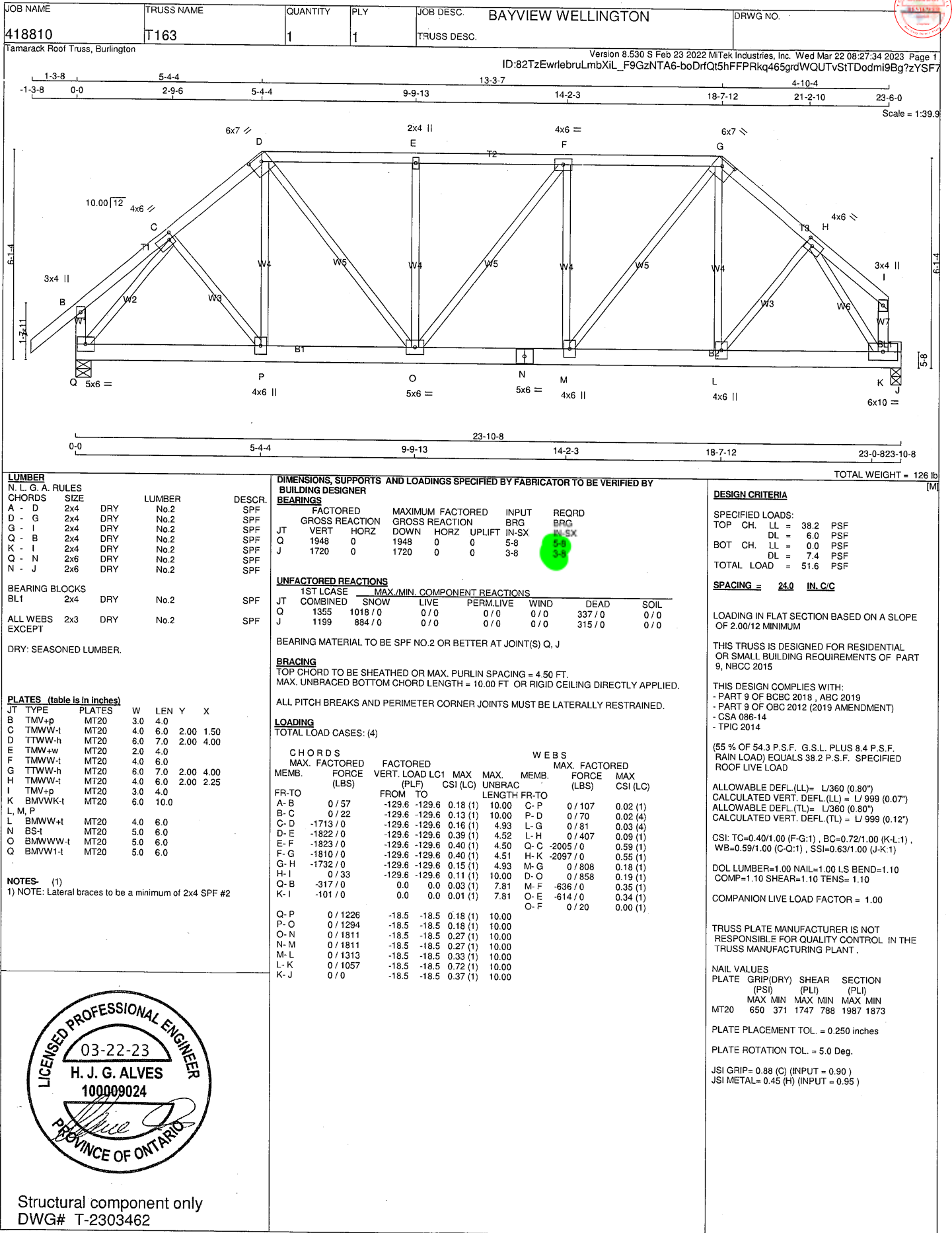
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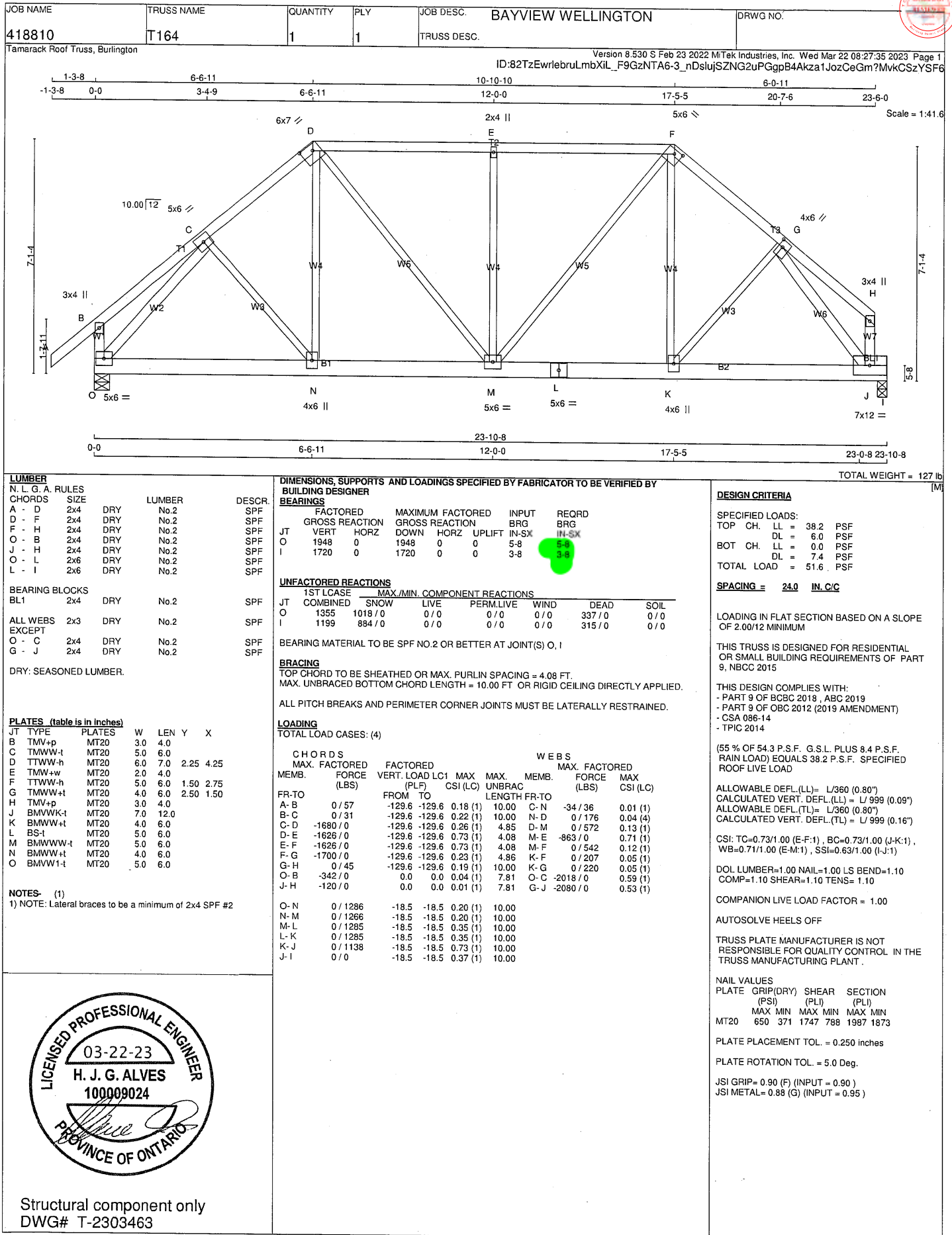
PROVINCE OF ONTARIO

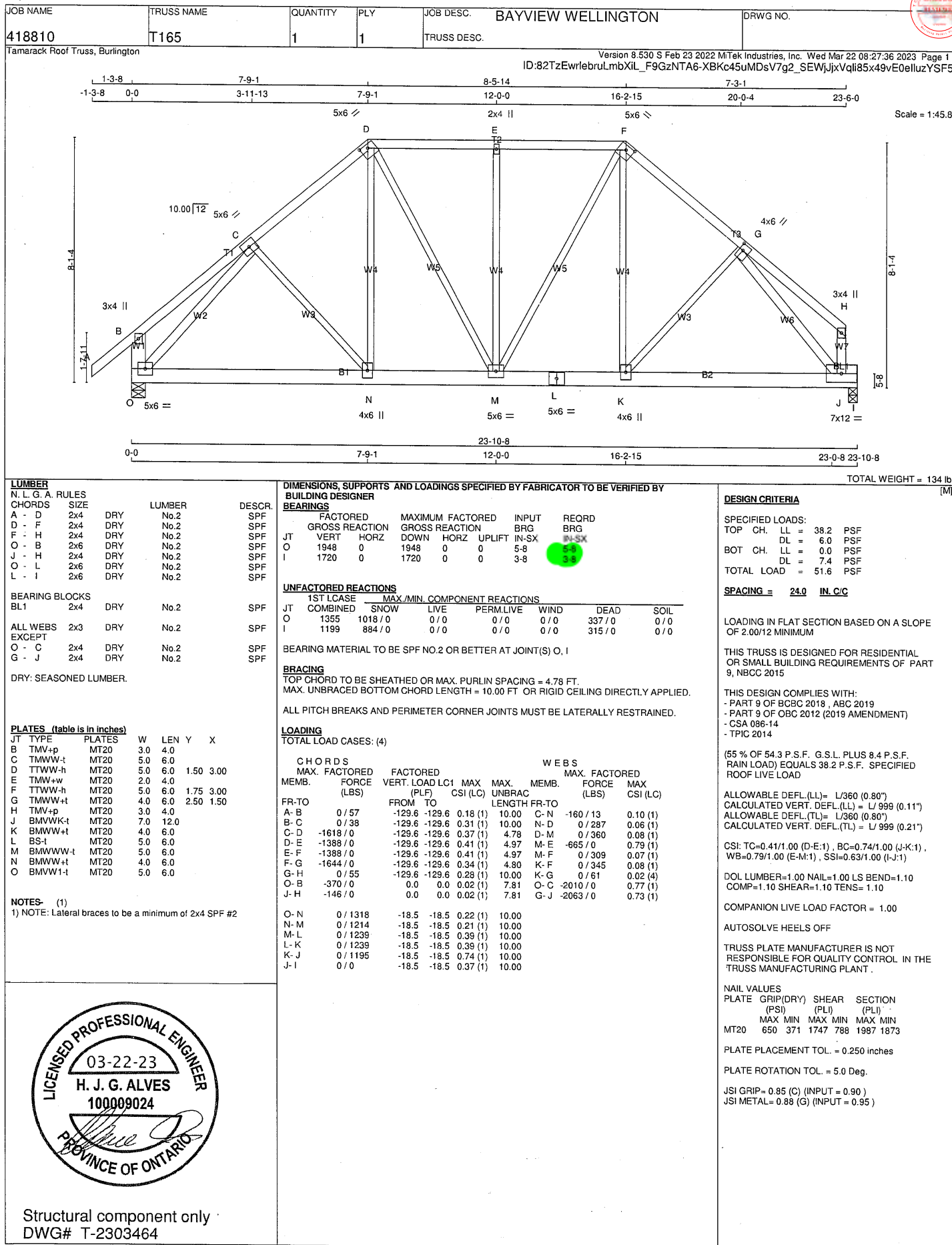
Structural component only

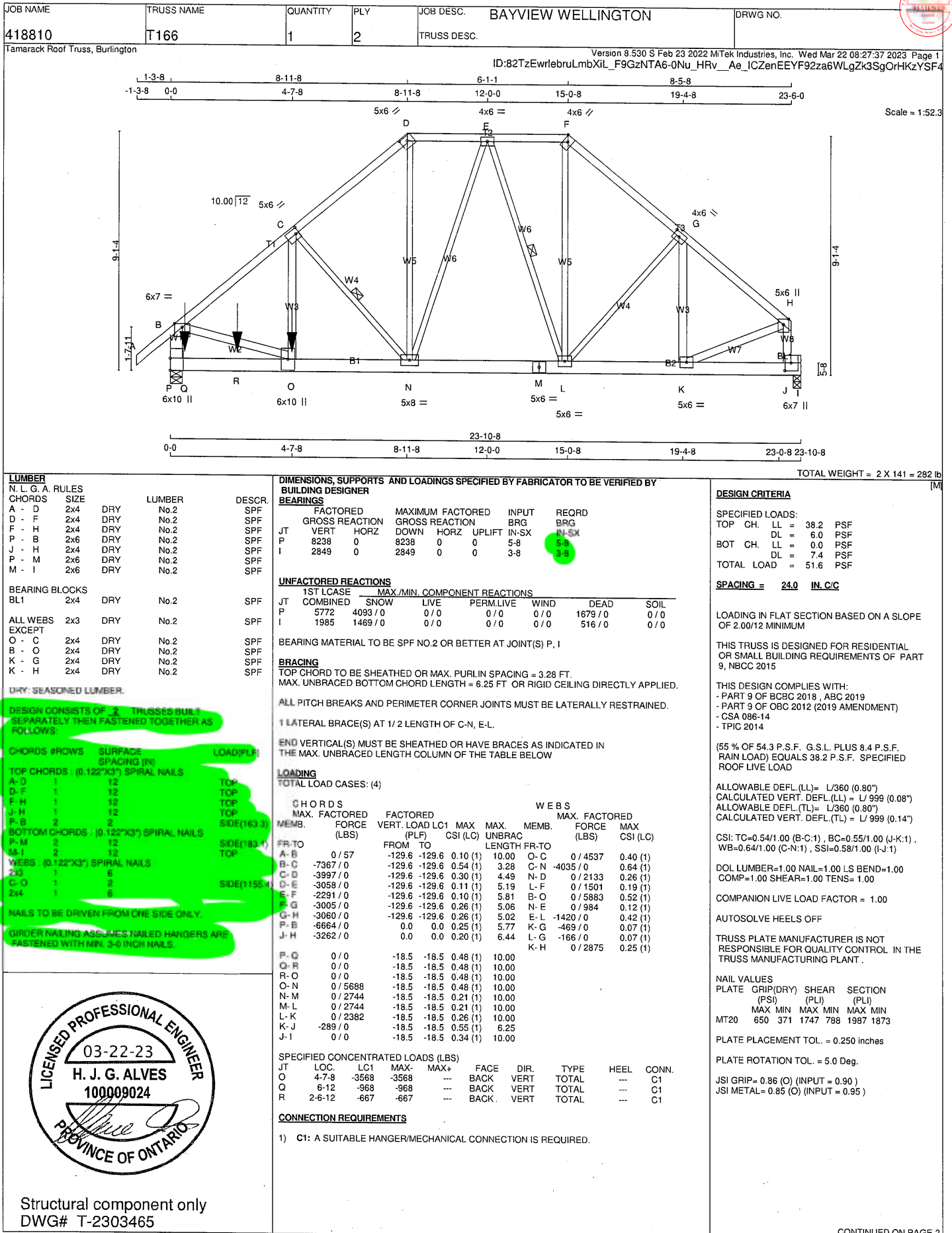
DWG# T-2303460













JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T166	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:37 2023 Page 2
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PLATES (table is in inches)

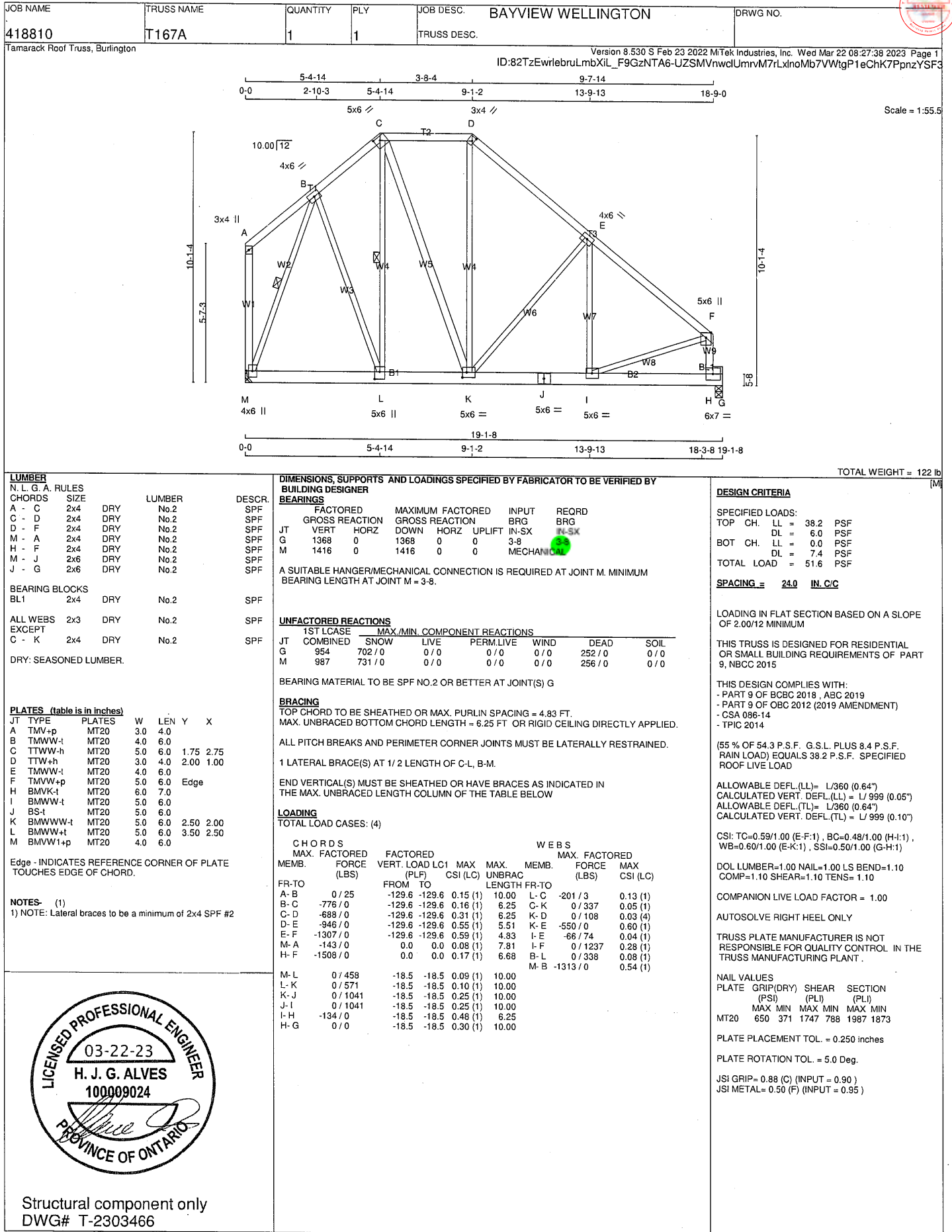
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMWW-t	MT20	5.0	6.0	2.50	1.50
D	TTW-h	MT20	5.0	6.0	2.00	3.00
E	TMWW-t	MT20	4.0	6.0		
F	TTW+h	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	6.0	2.00	2.75
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVK+p	MT20	6.0	7.0	3.50	2.75
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	6.0	10.0		
P	BMV1+t	MT20	6.0	10.0	5.50	

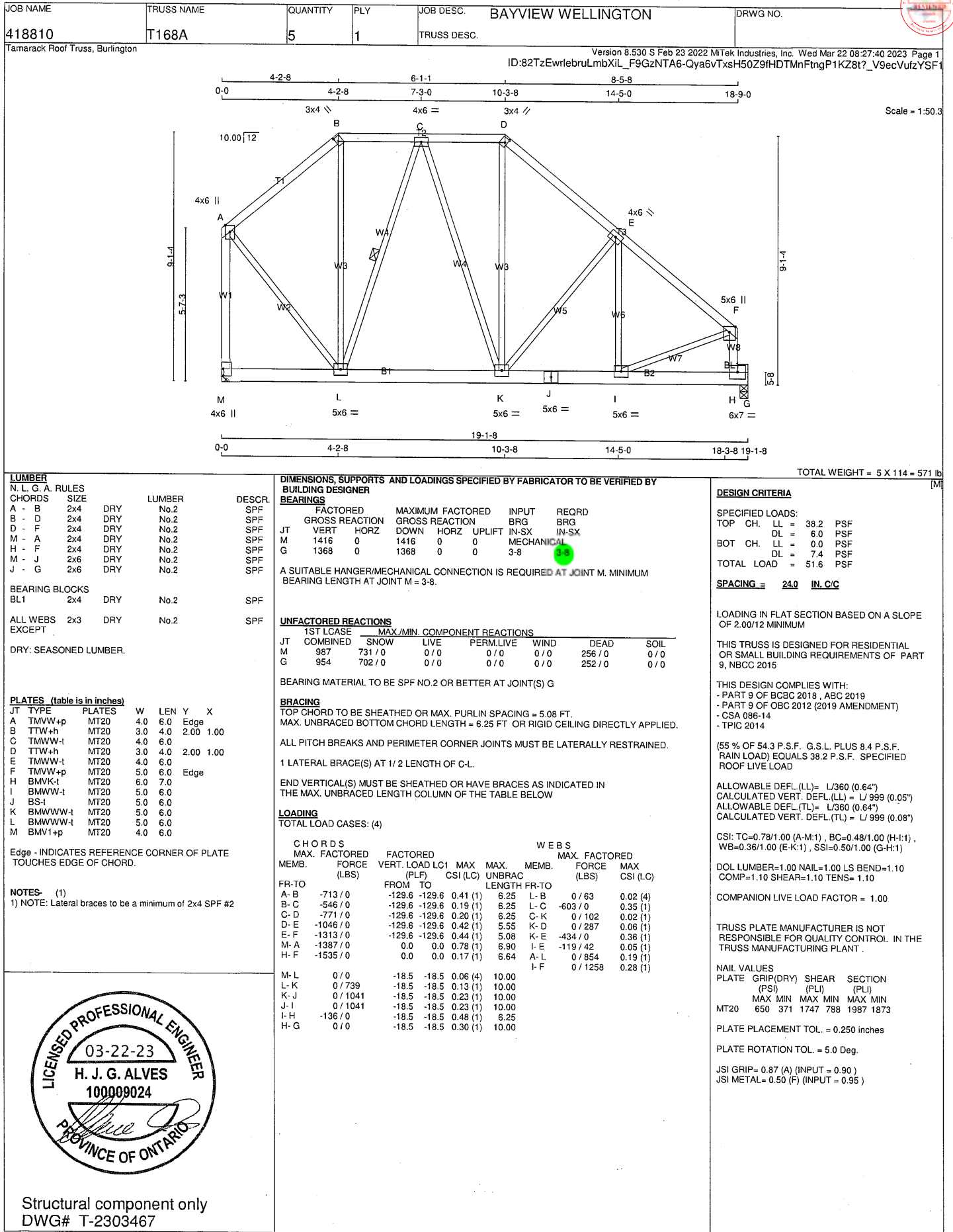
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2303465



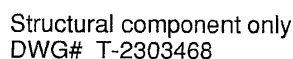


Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:41 2023 Page 1
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TOTAL WEIGHT = $2 \times 84 = 169$ lb

JSI GRIP= 0.76 (C) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 0.95)



JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

DRWG NO.

418810

G169

1

1

BAYVIEW WELLINGTON

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:26 2023 Page 1

ID: dQRJcissEbKAe0Zf5XnL8YzNr43-bj68N92RiMh3HHd?7pSutAcw6VQwYZRc77hbHyzYSGE

1-3-8

8-2-0

8-2-0

8-2-0

1-3-8

-1-3-8

0-0

16-4-0

17-7-8

4x6 II

Scale = 1:49.9

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

U - B

2x4

DRY

No.2

SPF

A - F

2x4

DRY

No.2

SPF

F - K

2x4

DRY

No.2

SPF

L - J

2x4

DRY

No.2

SPF

U - O

2x4

DRY

No.2

SPF

O - L

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

ALL GABLE WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT

2-0-0

OC.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMVW+p

MT20

4.0

6.0

Edge

C, D, E, G, H, I

C

TMW+w

MT20

2.0

4.0

F

TTW+p

MT20

4.0

6.0

Edge

J

TMVW+p

MT20

4.0

6.0

Edge

L

BMV1+p

MT20

3.0

4.0

M

BMWW1-t

MT20

4.0

6.0

N, P, Q, R, S

N

BMW1+w

MT20

2.0

4.0

O

BS-t

MT20

3.0

8.0

T

BMWW1-t

MT20

4.0

6.0

U

BMV1+p

MT20

3.0

4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

VERT. LOAD (PLF)

LC1

MAX

CS1 (LC)

UNBRAC

WEBS

MEMB.

MAX. FACTORED FORCE (LBS)

MAX

CS1 (LC)

FR-TO

FROM

TO

LENGTH

FR-TO

FROM

TO

U-B

-350 / 0

0.0

0.0

0.04 (1)

7.81

Q-F

-175 / 0

0.24 (1)

A-B

0 / 57

-129.6

-129.6

0.18 (1)

10.00

R-E

-297 / 0

0.23 (1)

B-C

-40 / 0

-129.6

-129.6

0.08 (1)

6.25

S-D

-238 / 0

0.09 (1)

C-D

-52 / 0

-129.6

-129.6

0.08 (1)

6.25

T-C

-293 / 0

0.06 (1)

D-E

-34 / 0

-129.6

-129.6

0.08 (1)

6.25

P-G

-297 / 0

0.23 (1)

E-F

-51 / 0

-129.6

-129.6

0.08 (1)

6.25

N-H

-238 / 0

0.09 (1)

F-G

-51 / 0

-129.6

-129.6

0.08 (1)

6.25

M-I

-293 / 0

0.06 (1)

G-H

-34 / 0

-129.6

-129.6

0.08 (1)

6.25

B-T

0 / 51

0.01 (1)

H-I

-52 / 0

-129.6

-129.6

0.08 (1)

6.25

M-J

0 / 51

0.01 (1)

I-J

-40 / 0

-129.6

-129.6

0.08 (1)

6.25

J-K

0 / 57

-129.6

-129.6

0.18 (1)

10.00

L-J

-350 / 0

0.0

0.0

0.04 (1)

7.81

U-T

0 / 0

-18.5

-18.5

0.02 (4)

10.00

T-S

0 / 35

-18.5

-18.5

0.02 (4)

10.00

S-R

0 / 30

-18.5

-18.5

0.02 (4)

10.00

R-Q

0 / 26

-18.5

-18.5

0.02 (4)

10.00

Q-P

0 / 26

-18.5

-18.5

0.02 (4)

10.00

P-O

0 / 30

-18.5

-18.5

0.02 (4)

10.00

O-N

0 / 30

-18.5

-18.5

0.02 (4)

10.00

N-M

0 / 35

-18.5

-18.5

0.02 (4)

10.00

M-L

0 / 0

-18.5

-18.5

0.02 (4)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- T-PC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (J-K:1), BC=0.02/1.00 (M-N:4), WB=0.24/1.00 (F-Q:1), SSI=0.11/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (I) (INPUT = 0.90)

JSI METAL= 0.16 (E) (INPUT = 0.95)

03-22-23

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only

DWG# T-2303419



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T170	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:42 2023 Page 2	
				ID:4u167_apsj?wwBfJKDwJbPzNv7w-MKItK9z6piGHOzRcanplyCImI7CiLv1ocy5cyYzYSF?	

PLATES (table is in inches)

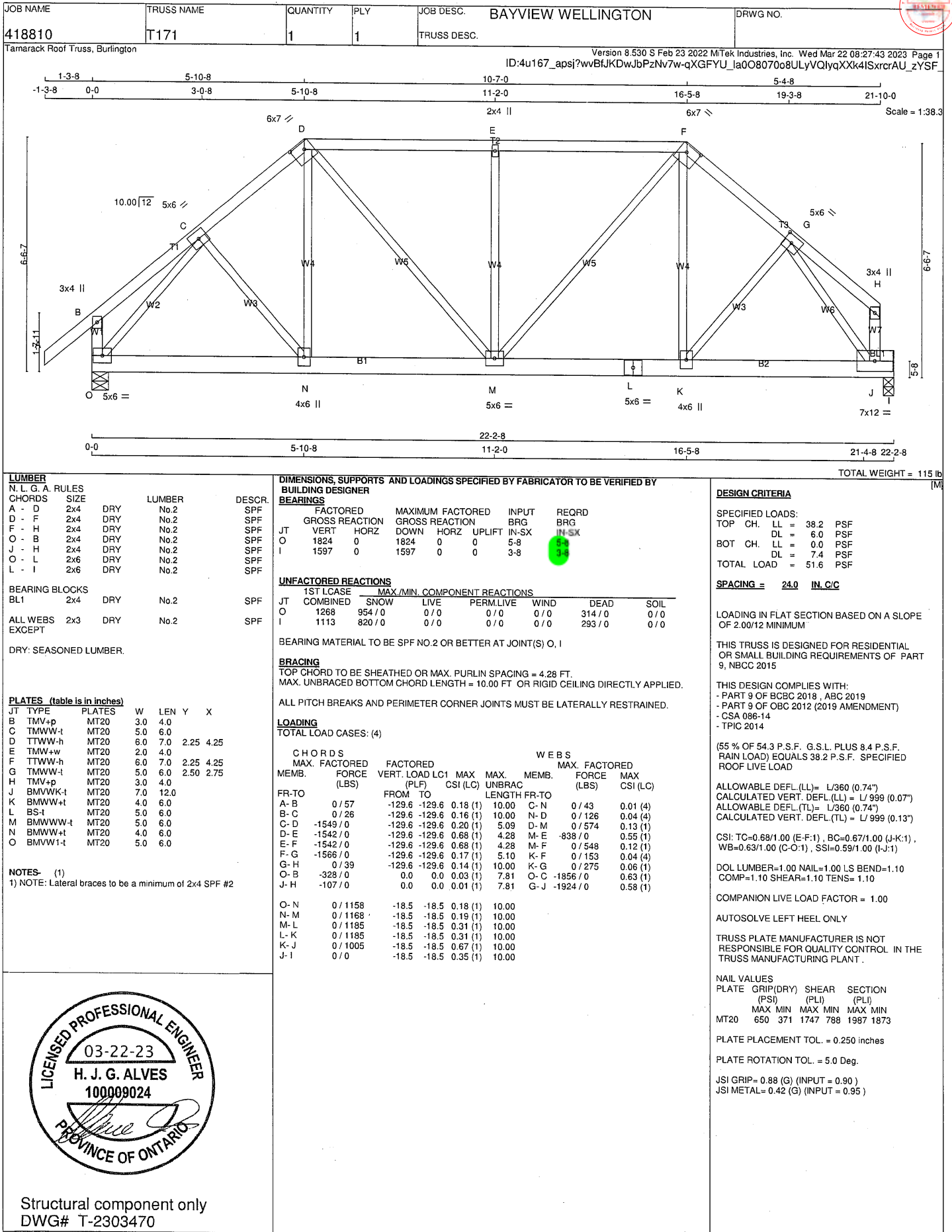
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-h	MT20	6.0	7.0	1.75	3.75
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-h	MT20	6.0	7.0	1.50	3.50
G	TMVW+p	MT20	5.0	6.0	2.00	2.25
I	BMVK-t	MT20	9.0	9.0	4.75	4.00
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

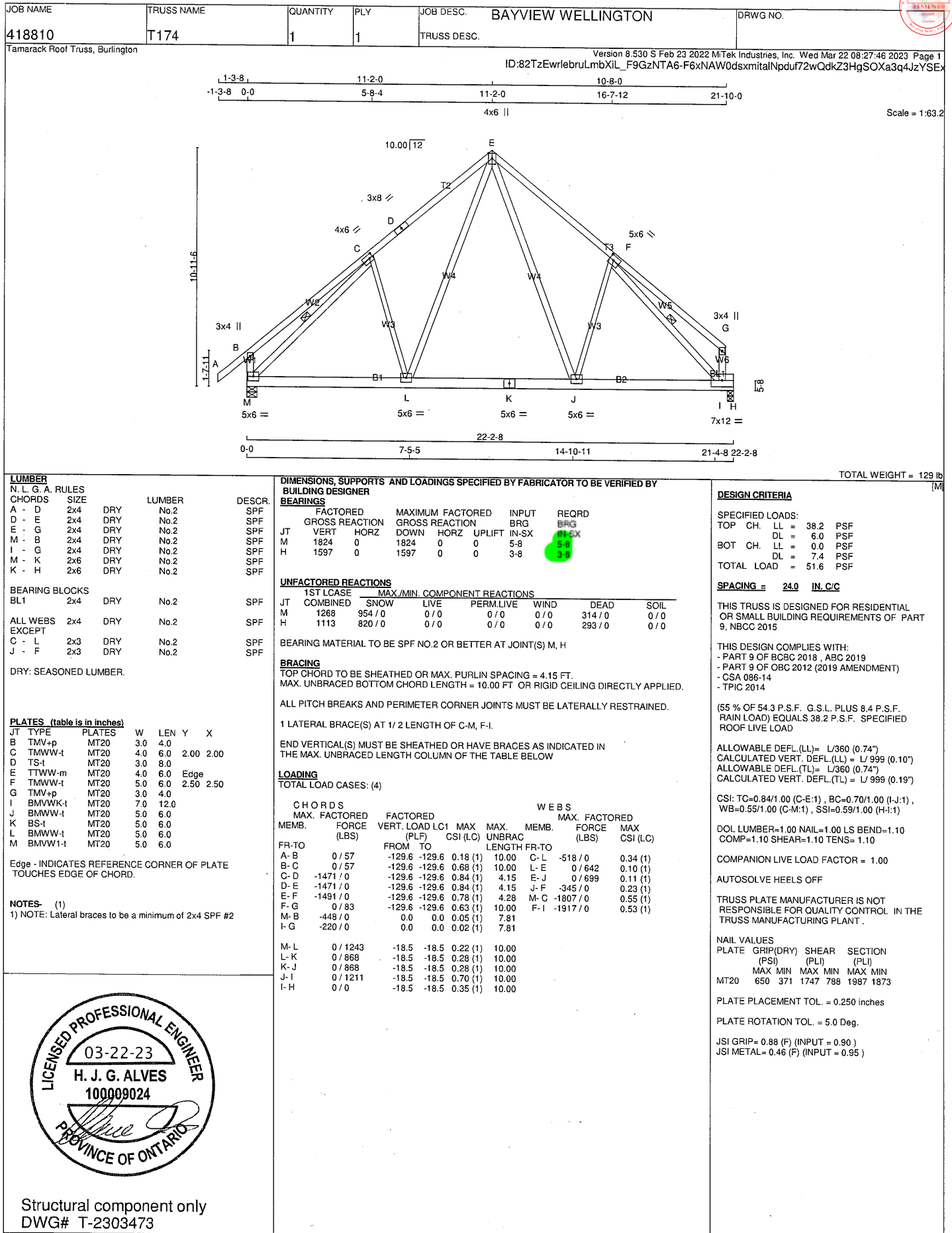
NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2303469





JOB NAME: 418810

TRUSS NAME: T174X

QUANTITY: 4

PLY: 1

JOB DESC.: BAYVIEW WELLINGTON

TRUSS DESC.:

DRWG NO.:

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:48 2023 Page 1

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Tamarack Roof Truss, Burlington

Scale = 1:62.9

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
A - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
-----	-----	-----	------	-----

BEARING BLOCKS

BL1	2x4	DRY	No.2	SPF
-----	-----	-----	------	-----

ALL WEBS EXCEPT

B - K	2x3	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT		
A	1719	0	1719	0	3-8	3-8
G	1671	0	1671	0	3-8	3-8

UNFACTORED REACTIONS

JT	MAX / MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	1198	887 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0
G	1165	858 / 0	0 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. FORCE (LBS)	VERT. LOAD (LBS)			MAX. FORCE (LBS)	VERT. LOAD (LBS)
FR-TO						FR-TO		
A-M	-1895 / 0	-129.6	-129.6	0.42 (1)	4.33	B-K	-710 / 0	0.47 (1)
M-B	-1636 / 0	-129.6	-129.6	0.87 (1)	4.05	K-D	0 / 882	0.14 (1)
B-C	-1680 / 0	-129.6	-129.6	0.82 (1)	4.00	D-I	0 / 676	0.11 (1)
C-D	-1680 / 0	-129.6	-129.6	0.82 (1)	4.00	I-E	-325 / 2	0.21 (1)
D-E	-1585 / 0	-129.6	-129.6	0.79 (1)	4.17	E-H	-2020 / 0	0.56 (1)
E-F	0 / 83	-129.6	-129.6	0.63 (1)	10.00	L-M	-31 / 138	0.00 (1)
H-F	-220 / 0	0.0	0.0	0.02 (1)	7.81	L-B	-235 / 0	0.19 (1)
A-L	0 / 1272	-18.5	-18.5	0.24 (1)	10.00			
L-K	0 / 1440	-18.5	-18.5	0.30 (1)	10.00			
K-J	0 / 948	-18.5	-18.5	0.29 (1)	10.00			
J-I	0 / 948	-18.5	-18.5	0.29 (1)	10.00			
I-H	0 / 1277	-18.5	-18.5	0.73 (1)	10.00			
H-G	0 / 0	-18.5	-18.5	0.36 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.87/1.00 (B-M-1), BC=0.73/1.00 (H-I-1), WB=0.56/1.00 (E-H-1), SSI=0.62/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(FSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

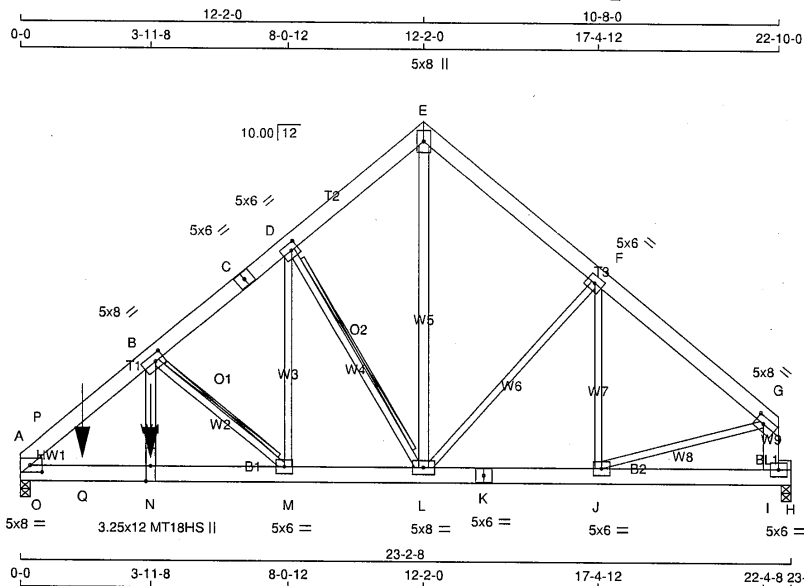
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.48 (E) (INPUT = 0.95)

Structural component only

DWG# T-2303474

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T175X	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF		
C - E	2x6 DRY	No.2	SPF		
E - G	2x6 DRY	No.2	SPF		
I - K	2x6 DRY	No.2	SPF		
A - K	2x6 DRY	2100F 1.8E	SPF		
K - H	2x6 DRY	2100F 1.8E	SPF		

BEARING BLOCKS	SIZE	LUMBER	DESCR.
BL1	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
N - B	2x4 DRY	No.2	SPF
L - E	2x4 DRY	No.2	SPF

DRY, SEASONED LUMBER

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS.

CHORDS	SPACING (IN)	LOADS
TOP CHORDS : (0 122"x3") SPIRAL NAILS		
A - C	2	12
C - E	2	12
E - G	2	12
I - K	2	12
BOTTOM CHORDS : (0 122"x3") SPIRAL NAILS		
A - K	2	12
K - H	2	12
WEBS : (0 122"x3") SPIRAL NAILS		
2x3	1	5
2x4	1	5

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

ORDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.8 INCH NAILS.

TOP COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT	VERT	DOWN	UPLIFT	IN-SX	
A	6867	0	6867	0	3-8
H	2705	0	2705	0	3-8

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
A	4774	3597 / 0 0 / 0 0 / 0 1177 / 0 0 / 0
H	1883	1407 / 0 0 / 0 0 / 0 476 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) A (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-M, D-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-P	-10663	0	-129.6	-129.6	0.30	(1)	3.53						
P-B	-8623	0	-129.6	-129.6	0.26	(1)	3.95						
B-C	-4350	0	-129.6	-129.6	0.15	(1)	5.36						
C-D	-4350	0	-129.6	-129.6	0.15	(1)	5.36						
D-E	-2608	0	-129.6	-129.6	0.11	(1)	6.25						
E-F	-2614	0	-129.6	-129.6	0.15	(1)	6.25						
F-G	-2919	0	-129.6	-129.6	0.16	(1)	6.22						
I-G	-2978	0	0.0	0.0	0.11	(1)	7.81						
A-O	0	6539	-18.5	-18.5	0.33	(1)	10.00						
O-Q	0	6539	-18.5	-18.5	0.33	(1)	10.00						
Q-N	0	6539	-18.5	-18.5	0.33	(1)	10.00						
N-M	0	6539	-18.5	-18.5	0.26	(1)	10.00						
M-L	0	3398	-18.5	-18.5	0.10	(1)	10.00						
L-K	0	2277	-18.5	-18.5	0.13	(1)	10.00						
K-J	0	2277	-18.5	-18.5	0.13	(1)	10.00						
J-I	-441	0	-18.5	-18.5	0.32	(1)	6.25						
I-H	0	0	-148.1	-148.1	0.20	(1)	10.00						

SPECIFIED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
N	3-11-8	-3586	-3586	---	---	FRONT	VERT	TOTAL	---	C1
Q	1-10-12	-655	-655	---	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.30/1.00 (A-P:1), BC=0.33/1.00 (A-O:1), WB=0.89/1.00 (D-L:1), SS=0.55/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873
MT18HS 586 403 2455 1382 3163 3004

PLATE PLACEMENT TOL. = 0.250 inches

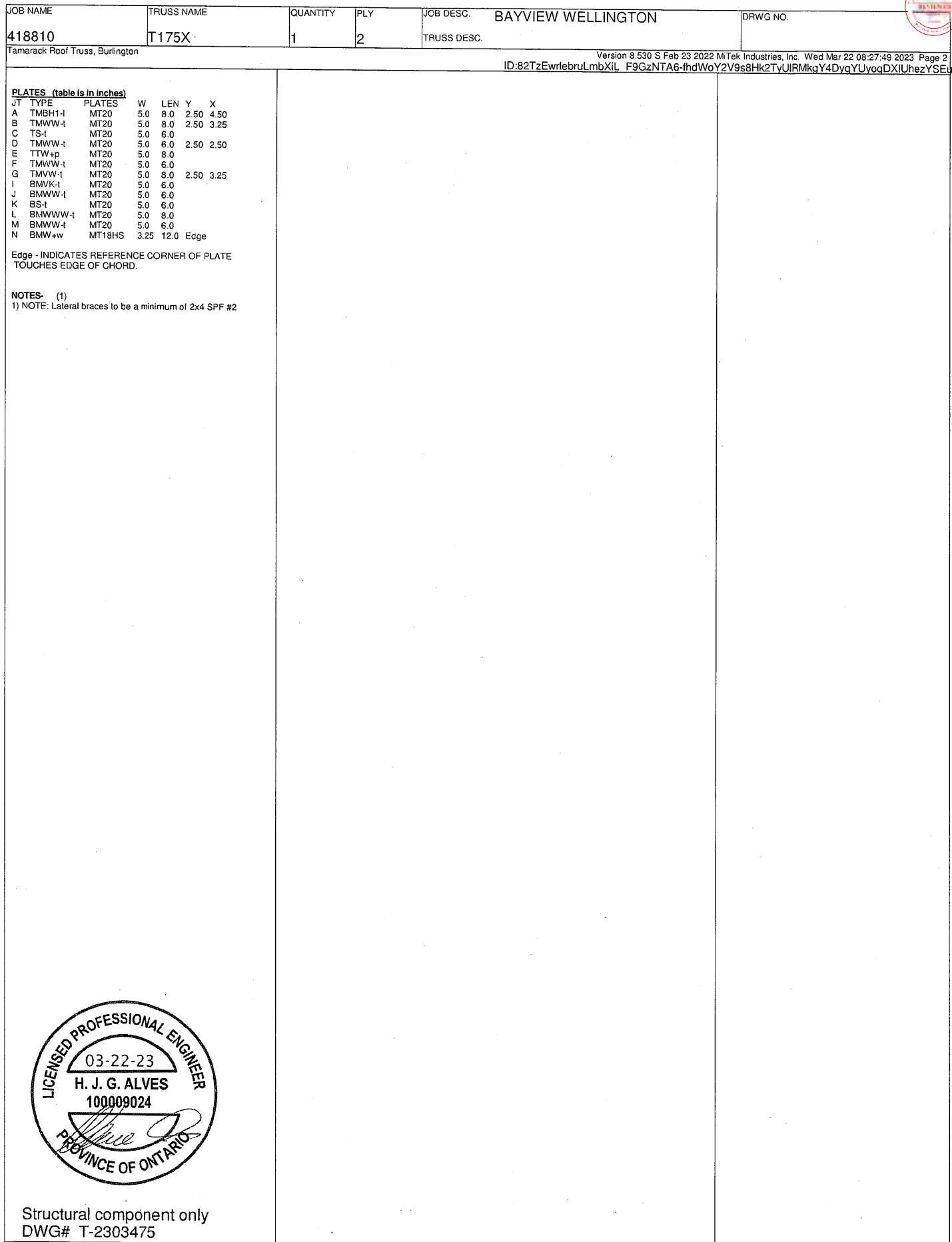
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.79 (N) (INPUT = 0.95)



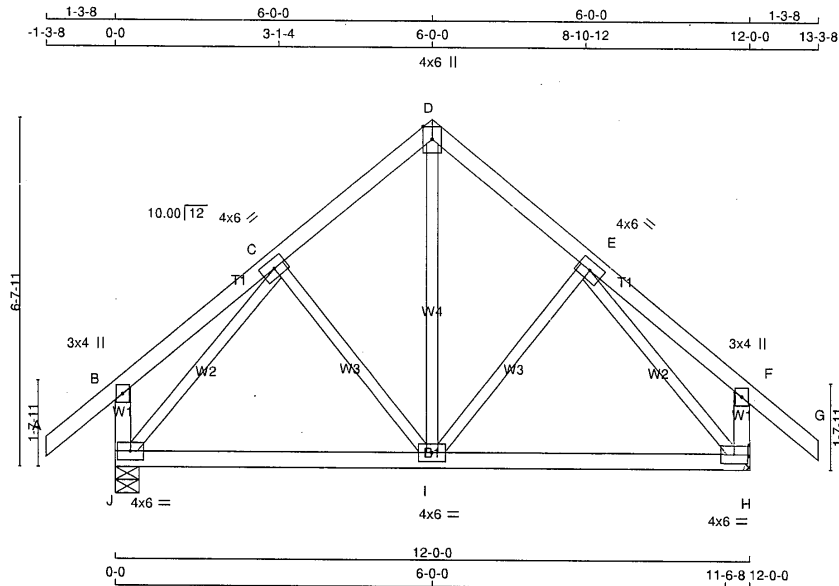
Structural component only
DWG# T-2303475

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T176	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:50 2023 Page 1
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Scale = 1:39.5

TOTAL WEIGHT = 2 X 58 = 116 lb [M/F]

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWV-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMWV1-t	MT20	4.0	6.0		
I	BMWV1-t	MT20	4.0	6.0		
J	BMWV1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
J	1068	0	0	5-8
H	1068	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	742	564 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
H	742	564 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS							
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)		
FR-TO		FROM TO	CSI (LC)	UNBRAC	LENGTH	FR-TO					
A-B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	I-D	0 / 418	0.09 (1)			
B-C	0 / 23	-129.6	-129.6	0.18 (1)	10.00	I-E	-181 / 0	0.07 (1)			
C-D	-593 / 0	-129.6	-129.6	0.14 (1)	6.25	C-I	-181 / 0	0.07 (1)			
D-E	-593 / 0	-129.6	-129.6	0.14 (1)	6.25	J-C	-887 / 0	0.33 (1)			
E-F	0 / 23	-129.6	-129.6	0.18 (1)	10.00	E-H	-887 / 0	0.33 (1)			
F-G	0 / 57	-129.6	-129.6	0.18 (1)	10.00						
J-B	-328 / 0	0.0	0.0	0.03 (1)	7.81						
H-F	-328 / 0	0.0	0.0	0.03 (1)	7.81						
J-I	0 / 548	-18.5	-18.5	0.22 (4)	10.00						
I-H	0 / 548	-18.5	-18.5	0.22 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 39.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 39.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.18/1.00 (B-C:1), BC=0.22/1.00 (H-I:4), WB=0.33/1.00 (C-J:1), SS=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

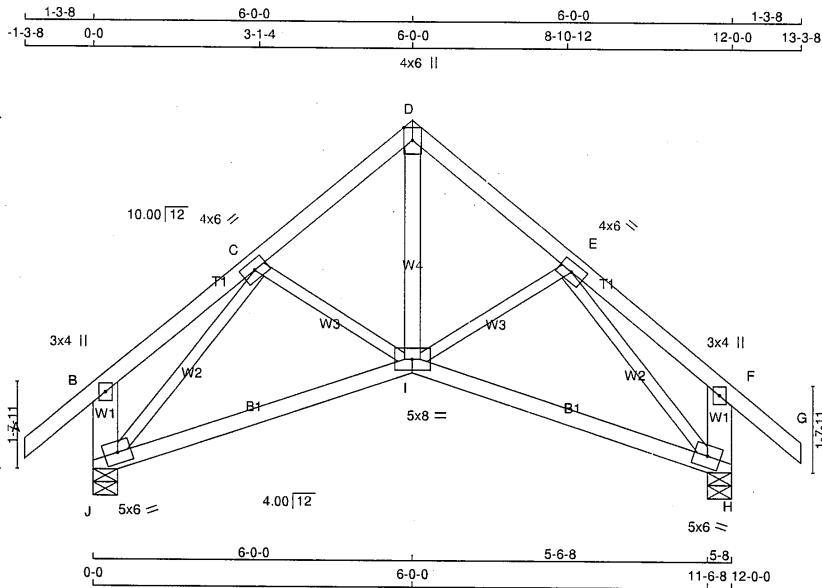
JSI GRIP= 0.59 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 0.95)



Structural component only
DWG# T-2303476

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T176S	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:51 2023 Page 1
ID:82TzEwrlbruLmbXil_F9GzNTA6-b3kGDE4mhTO?zLdLcAUqp5dSfInVyzw7grnbmWzYSEs



Scale = 1:39.4

TOTAL WEIGHT = 3 X 59 = 178 lb [M/F]

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWV-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMWV-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	5.0	6.0	
I	BBWVW-p	MT20	5.0	8.0	
J	BMVW1-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
J	1068	0	0	5-8
H	1068	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	742	564 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
H	742	564 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 57	-129.6 -129.6	0.18 (1)	10.00	I-D	0 / 690	0.11 (1)
B-C	0 / 27	-129.6 -129.6	0.17 (1)	10.00	I-E	-110 / 8	0.03 (1)
C-D	-806 / 0	-129.6 -129.6	0.14 (1)	6.25	C-I	-110 / 8	0.03 (1)
D-E	-806 / 0	-129.6 -129.6	0.14 (1)	6.25	J-C	-1144 / 0	0.41 (1)
E-F	0 / 27	-129.6 -129.6	0.17 (1)	10.00	E-H	-1144 / 0	0.41 (1)
F-G	0 / 57	-129.6 -129.6	0.18 (1)	10.00			
J-B	-332 / 0	0.0	0.02 (1)	7.81			
H-F	-331 / 0	0.0	0.02 (1)	7.81			
J-I	0 / 729	-18.5 -18.5	0.23 (4)	10.00			
I-H	0 / 729	-18.5 -18.5	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.40")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.40")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.08")

CSI: TC=0.18/1.00 (A-B:1), BC=0.23/1.00 (I-J:4), WB=0.41/1.00 (C-J:1), SS=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 798 1987 1873

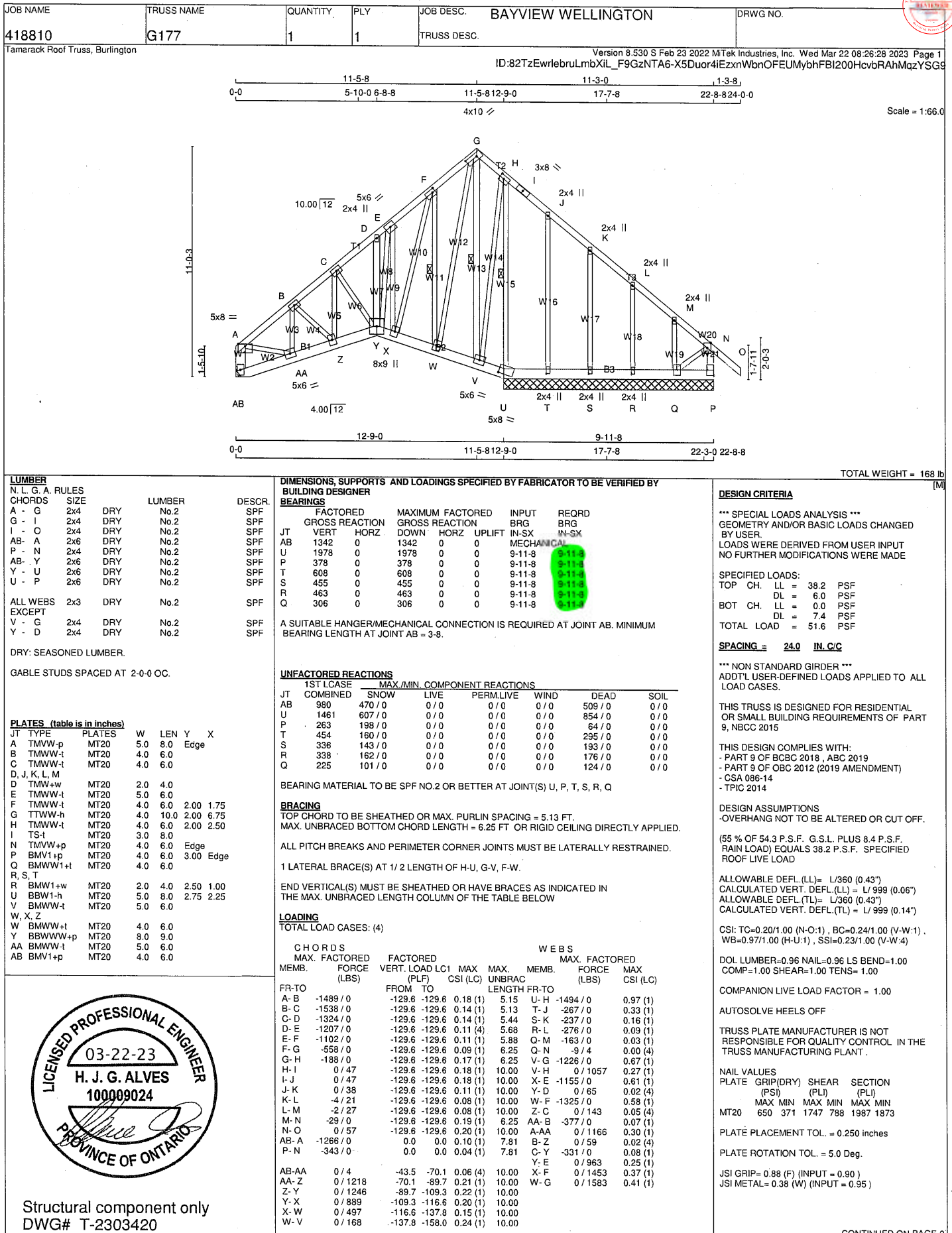
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.60 (E) (INPUT = 0.90)
JSI METAL = 0.25 (C) (INPUT = 0.95)



Structural component only
DWG# T-2303477





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	G177	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:28 2023 Page 2
ID:82TzEwrlbruLmbXiL F9GzNTA6-X5Duor4iEzxnWbnOFEUMybhFBI200HcvbRAhMqzYSG9

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRAC LENGTH FR-TO	LC1 MAX CSI (LC)
FR-TO		FROM TO					
V-U	-97 / 0	-158.0 -168.5	0.21 (1)			6.25	
U-T	-26 / 1	-168.5 -142.1	0.09 (4)			6.25	
T-S	-23 / 2	-142.1 -117.0	0.09 (4)			6.25	
S-R	-19 / 2	-117.0 -91.9	0.04 (4)			6.25	
R-O	-13 / 3	-91.9 -66.8	0.04 (4)			6.25	
Q-P	0 / 0	-66.8 -43.5	0.03 (4)			10.00	



Structural component only
DWG# T-2303420

JOB NAME
418810

TRUSS NAME
T178

QUANTITY
1

PLY
1

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Mar 22 08:27:52 2023 Page 1

ID:82TzEwrlbruLmbXIL_F9GzNTA6-3GIIRZ5OSnWsbVCXA73MJAW99jIhQRGvVW8lzzYSEr

Tamarack Roof Truss, Burlington

Scale = 1:66.4

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
H - A	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT	F - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWV-t	MT20	4.0	6.0		Edge
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	6.0	Edge	
E	BMV1+p	MT20	3.0	4.0		
F	BMVWV-t	MT20	4.0	10.0		
G	BMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
H	978	0	978	0	0	MECHANICAL
E	978	0	978	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	COMBINED	505 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
E	COMBINED	505 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-F, C-F, D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						
A-B	-763 / 0	-129.6	-129.6	0.58 (1)	5.94	0 / 109
B-C	-242 / 0	-129.6	-129.6	0.55 (1)	6.25	-717 / 0
C-D	-182 / 0	-129.6	-129.6	0.05 (1)	6.25	-152 / 13
H-A	-936 / 0	0.0	0.0	0.10 (1)	7.81	0 / 642
E-D	-966 / 0	0.0	0.0	0.44 (1)	6.25	0 / 749
H-G	0 / 0	-18.5	-18.5	0.16 (4)	10.00	
G-F	0 / 631	-18.5	-18.5	0.20 (4)	10.00	
F-E	0 / 0	-18.5	-18.5	0.06 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.58/1.00 (A-B:1), BC=0.20/1.00 (F-G:4), WB=0.33/1.00 (B-F:1), SS=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (A) (INPUT = 0.90)
JSI METAL= 0.42 (A) (INPUT = 0.95)

Structural component only
DWG# T-2303478

JOB NAME
418810

Tamarack Roof Truss, Burlington

TRUSS NAME
T179

QUANTITY
1

PLY
2

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:27:53 2023 Page 1

ID:82TzEwrlbruLmbXIL_F9GzNTA6-YSs1ev50D5fjDfnjibWlvWjIMZ_pQq?Q89GhqPzYSEq

Scale = 1:66.4

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD/PLY
TOP CHORDS: (Ø 122"x3") SPIRAL NAILS		
A-D	2	12
D-E	2	12
J-A	2	12
F-E	2	12
BOTTOM CHORDS: (Ø 122"x3") SPIRAL NAILS		
J-F	2	12
WEBS: (Ø 122"x3") SPIRAL NAILS		
B-I	1	6
D-E	1	6
J-A	1	6
F-E	1	6
G-D	1	6
D-E	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 5149	DOWN 5149	0	0
J	HORIZ 0	HORIZ 0	MECHANICAL	MECHANICAL
F	VERT 5194	DOWN 5194	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 4-0, JOINT F = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
J	3592 2685 / 0 0 / 0 0 / 0 0 897 / 0 0 / 0
F	3613 2708 / 0 0 / 0 0 / 0 0 905 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.01 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT C-G, E-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		
MEMB.	MAX. FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MEMB.	MAX. FORCE (LBS)	
FR-TO		FROM	TO	FR-TO		
A-B	-5182 / 0	-129.6	-129.6 0.15 (1)	5.01	I-B	0 / 2126
B-C	-3443 / 0	-129.6	-129.6 0.09 (1)	5.93	B-H	-1956 / 0
C-D	-1131 / 0	-129.6	-129.6 0.08 (1)	6.25	H-C	0 / 4528
D-E	-1096 / 0	-129.6	-129.6 0.02 (1)	6.25	C-G	-4047 / 0
J-A	-4525 / 0	0.0	0.0 0.16 (1)	6.77	G-D	0 / 1101
F-E	-5159 / 0	0.0	0.0 0.55 (1)	7.81	A-I	0 / 4085
					G-E	0 / 4425
J-K	0 / 0	-18.5	-18.5 0.41 (1)	10.00		
K-I	0 / 0	-18.5	-18.5 0.41 (1)	10.00		
I-L	0 / 3974	-18.5	-18.5 0.56 (1)	10.00		
L-M	0 / 3974	-18.5	-18.5 0.56 (1)	10.00		
M-H	0 / 3974	-18.5	-18.5 0.56 (1)	10.00		
H-N	0 / 2678	-18.5	-18.5 0.51 (1)	10.00		
N-G	0 / 2678	-18.5	-18.5 0.51 (1)	10.00		
G-F	0 / 0	-18.5	-18.5 0.01 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	11-4-12	-972	-972	---	FRONT	VERT	TOTAL	---	C1
I	3-9-12	-972	-972	---	FRONT	VERT	TOTAL	---	C1
K	1-9-12	-972	-972	---	FRONT	VERT	TOTAL	---	C1
L	5-9-12	-972	-972	---	FRONT	VERT	TOTAL	---	C1
M	7-4-12	-972	-972	---	FRONT	VERT	TOTAL	---	C1
N	9-4-12	-972	-972	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.55/1.00 (E-F:1), BC=0.56/1.00 (H-I:1), WB=0.56/1.00 (C-H:1), SSI=0.68/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 798

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.72 (I) (INPUT = 0.95)

Structural component only
DWG# T-2303479

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	T179	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Mar 22 08:27:54 2023 Page 2
ID:82TzEwrlebruLmbXiL_F9GzNTA6-0eQPsF6e_OnaqpMwHl1XRkFt5zK29HFZNP?FMrzYSEp

PLATES (table is in inches)

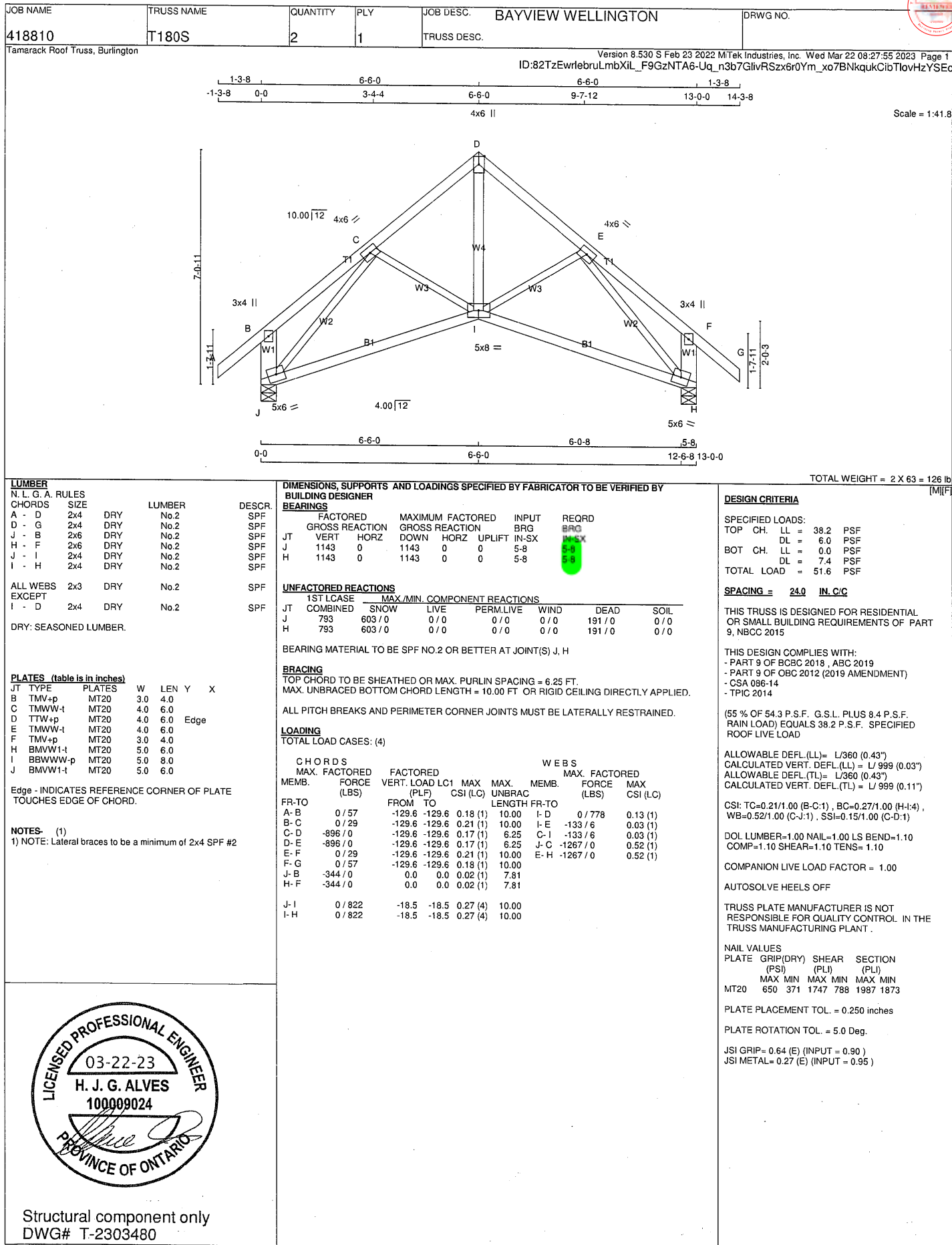
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	7.0	2.75	2.25
B	TMWW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.25
D	TTW+p	MT20	5.0	6.0	3.75	2.50
E	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BMV1+p	MT20	4.0	6.0		
G	BMWWW-t	MT20	6.0	10.0	2.50	5.00
H	BMWW-t	MT20	4.0	10.0		
I	BMWW-t	MT20	5.0	8.0	4.00	1.75
J	BMV1+p	MT20	4.0	6.0		

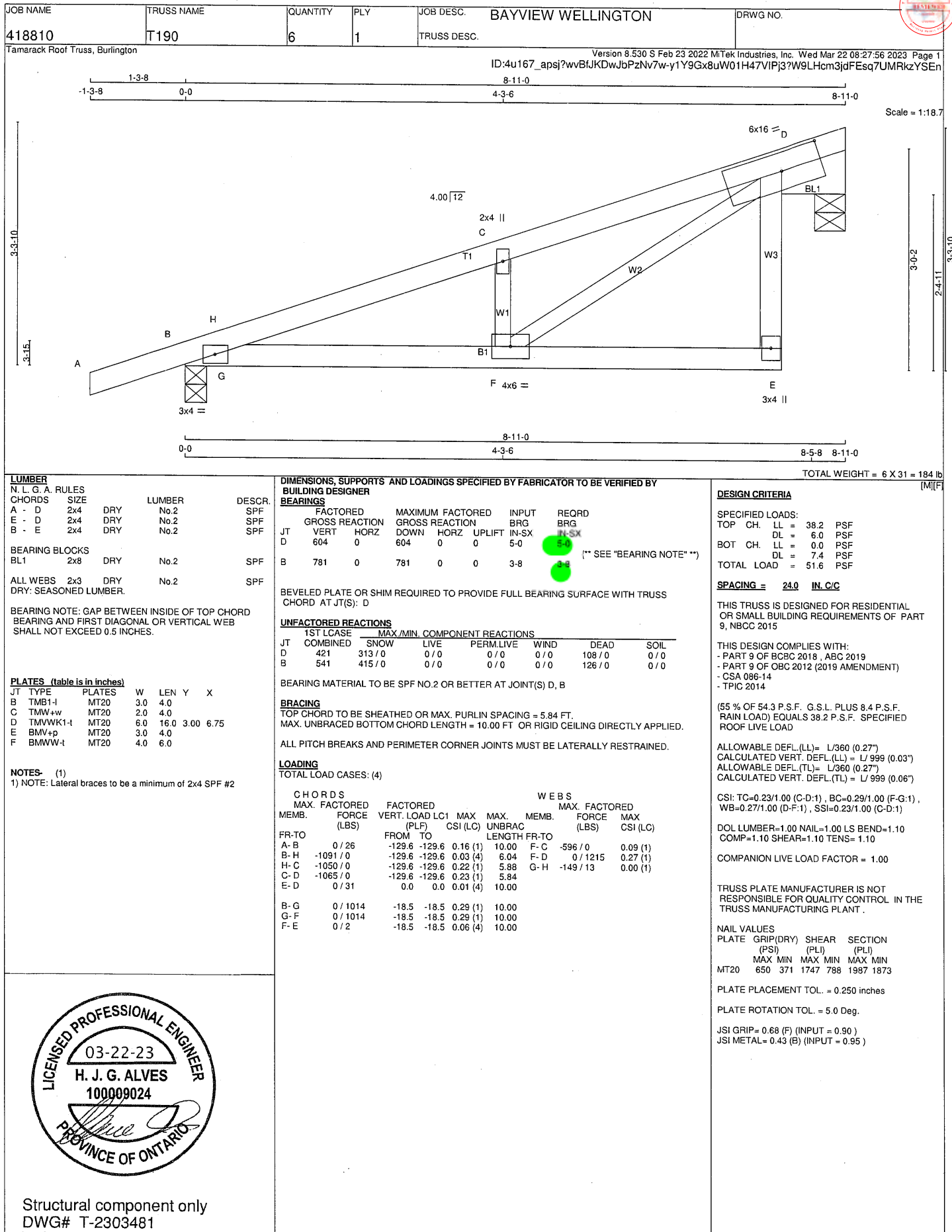
NOTES: (1)

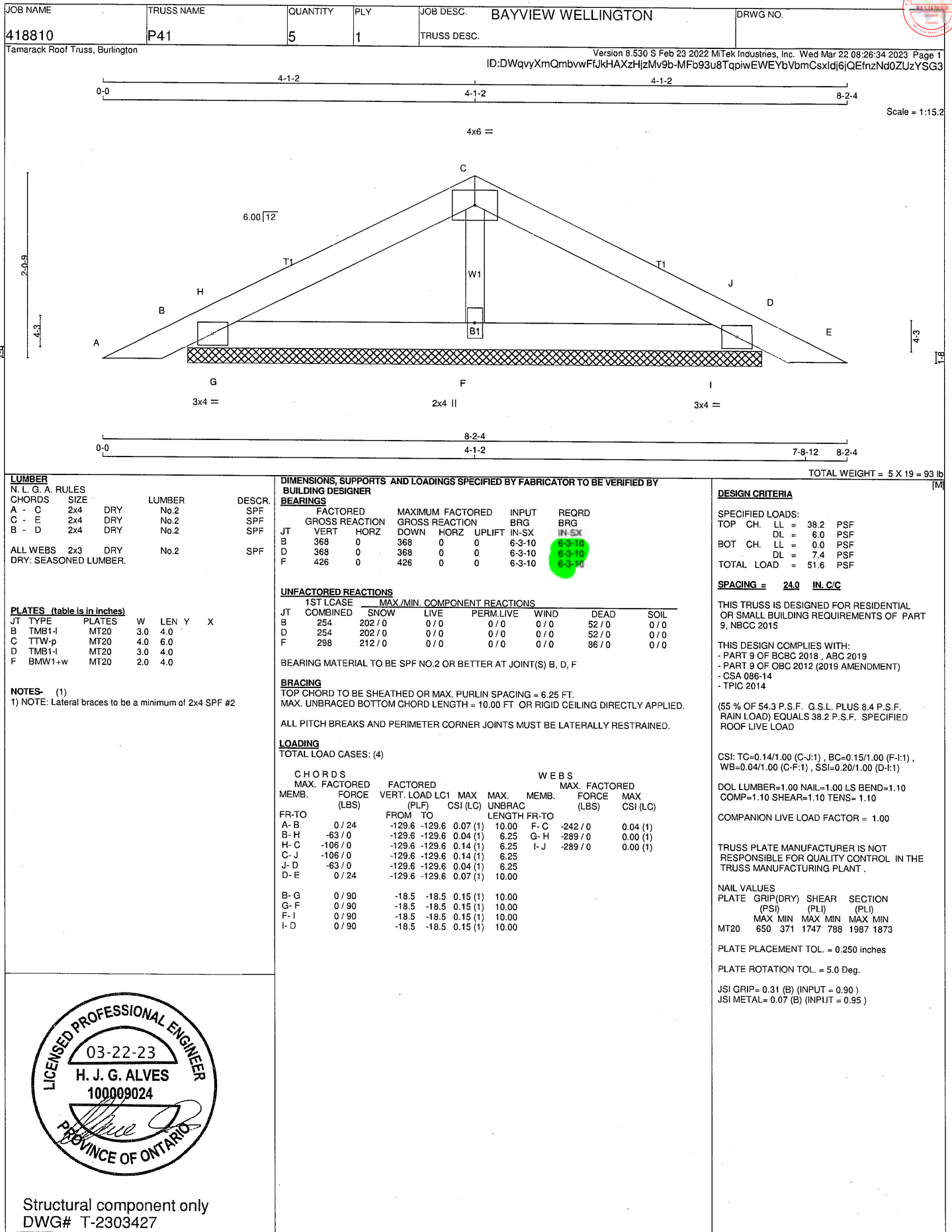
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

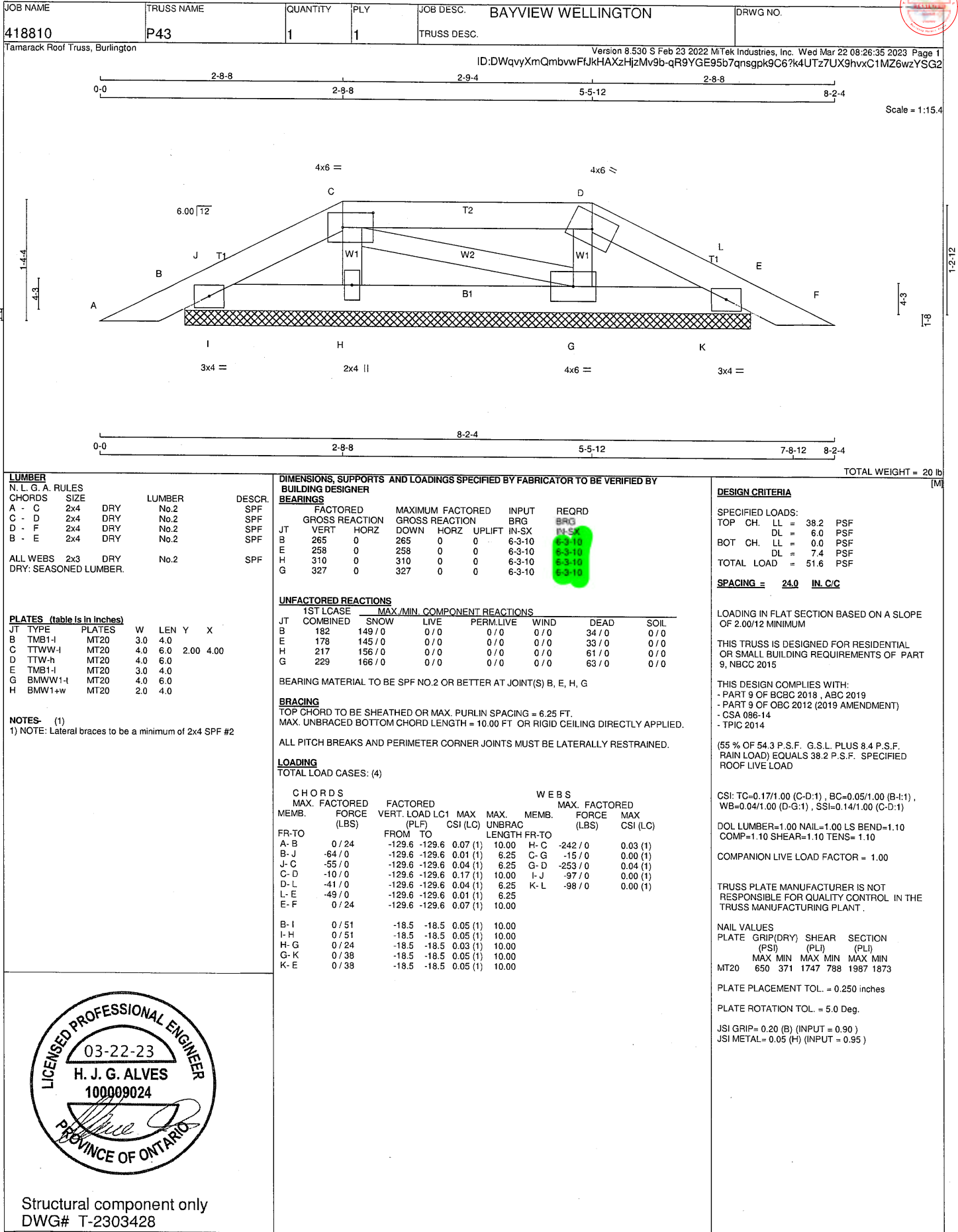


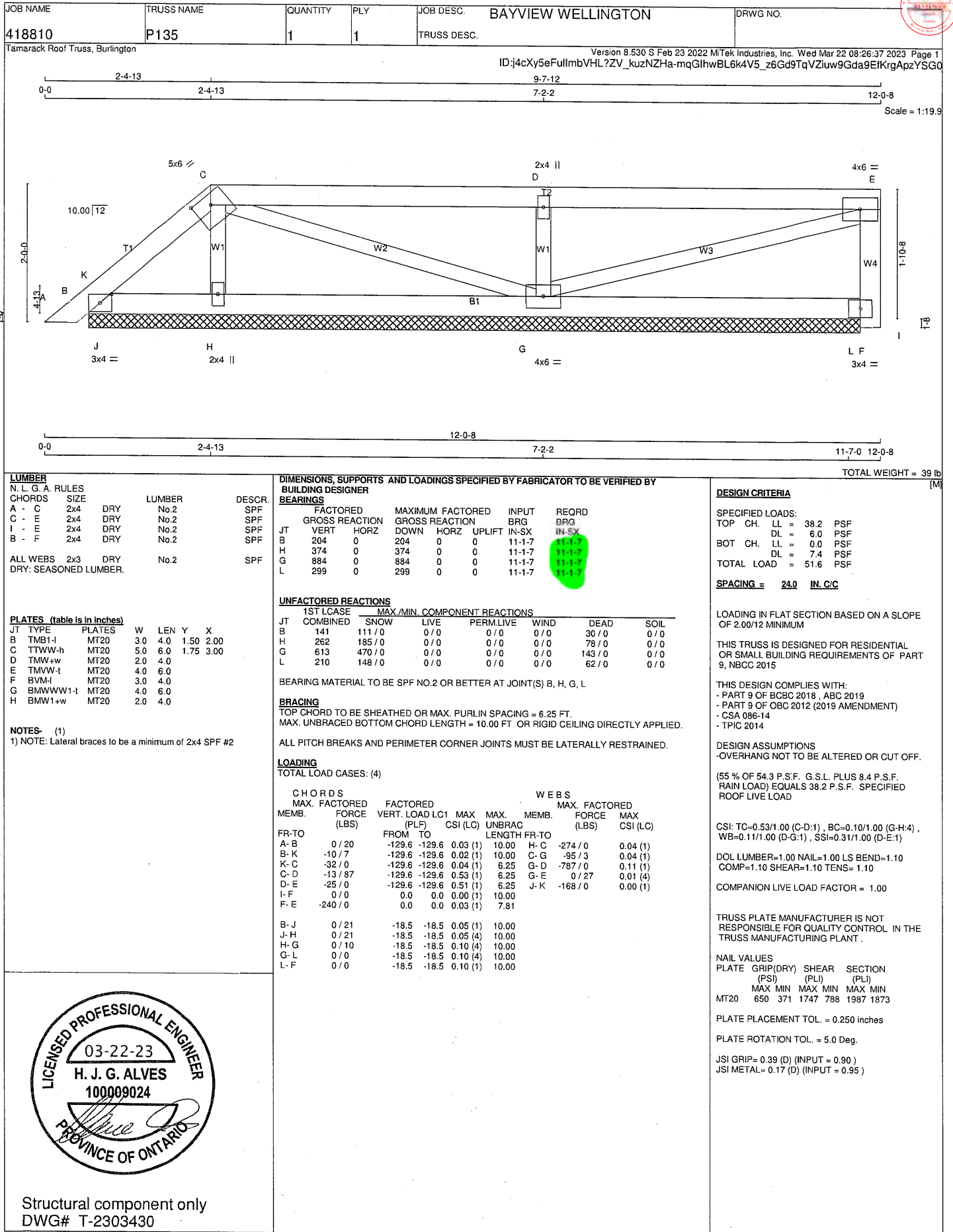
Structural component only
DWG# T-2303479







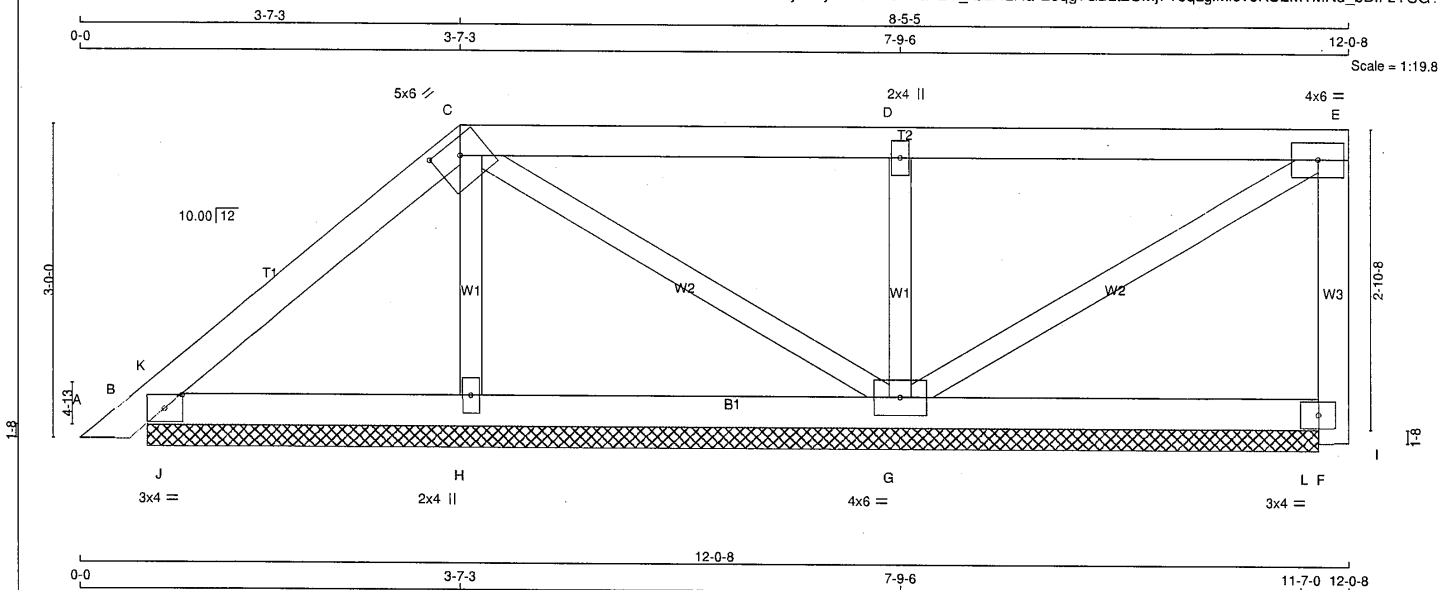




JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418810	P136	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:38 2023 Page 1
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TOTAL WEIGHT = 42 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

C - E 2x4 DRY

I - E 2x4 DRY

B - F 2x4 DRY

ALL WEBS 2x3 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
B	338	0	338	0
H	359	0	359	0
G	828	0	828	0
L	237	0	237	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	234	181/0	0/0	0/0	0/0	53/0
H	253	171/0	0/0	0/0	0/0	81/0
G	572	451/0	0/0	0/0	0/0	121/0
L	167	110/0	0/0	0/0	0/0	57/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MAX. UNBRACED LENGTH (FR-TO)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)
FR-TO						FR-TO			
A-B	0/20	-129.6	-129.6	0.03 (1)	10.00	H-C	-239/0	0.04 (1)	
B-K	-27/28	-129.6	-129.6	0.06 (1)	6.25	C-G	-121/0	0.05 (1)	
K-C	-104/0	-129.6	-129.6	0.14 (1)	6.25	G-D	-684/0	0.12 (1)	
C-D	-17/42	-129.6	-129.6	0.40 (1)	6.25	G-E	-43/19	0.02 (1)	
D-E	-17/42	-129.6	-129.6	0.40 (1)	6.25	J-K	-371/0	0.00 (1)	
I-F	0/0	0.0	0.0	0.00 (1)	10.00				
F-E	-184/0	0.0	0.0	0.03 (1)	7.81				
B-J	0/71	-18.5	-18.5	0.13 (1)	10.00				
J-H	0/71	-18.5	-18.5	0.13 (1)	10.00				
H-G	0/62	-18.5	-18.5	0.07 (1)	10.00				
G-L	0/0	-18.5	-18.5	0.07 (1)	10.00				
L-F	0/0	-18.5	-18.5	0.07 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (C-D:1), BC=0.13/1.00 (B-J:1), WB=0.12/1.00 (D-G:1), SSI=0.28/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

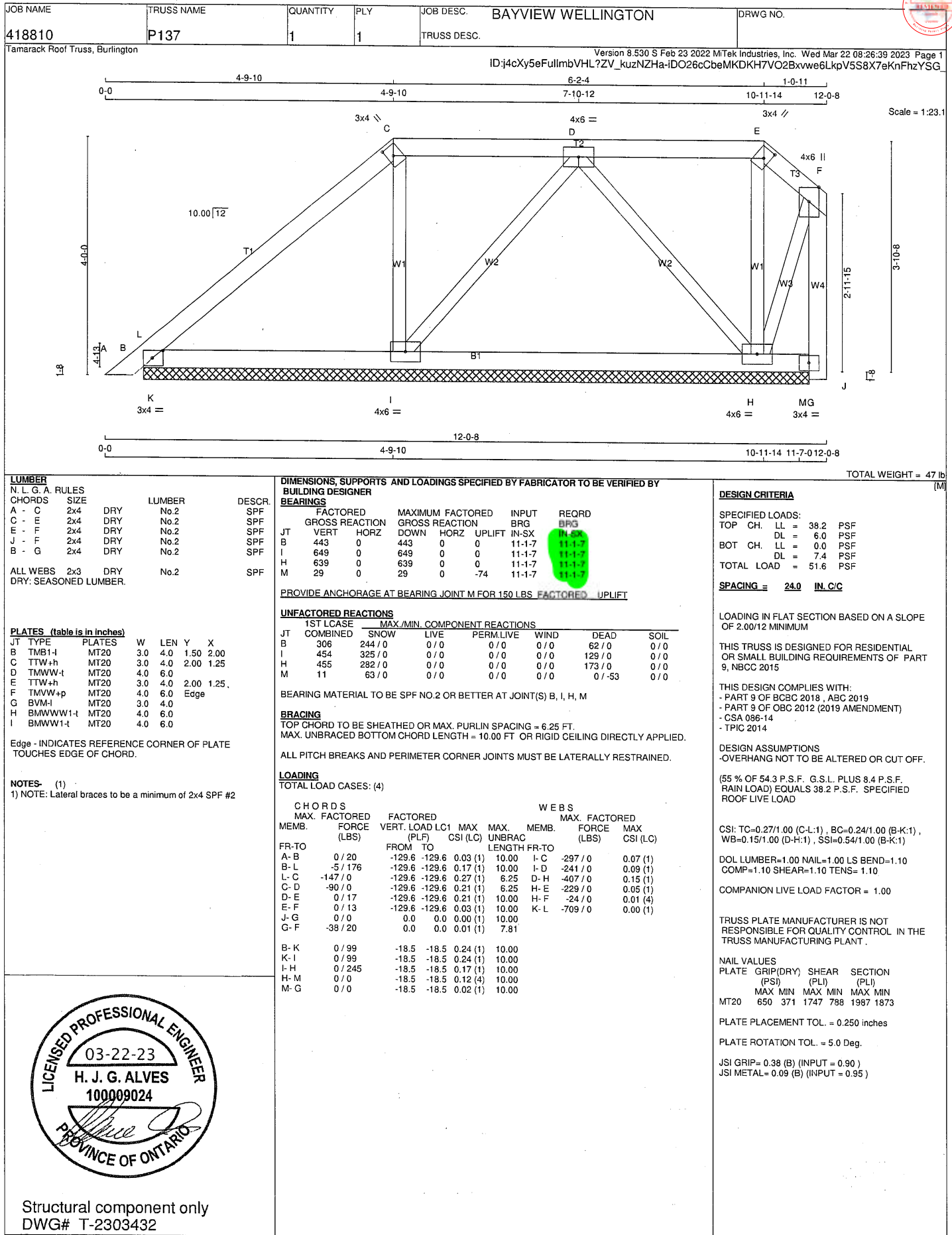
PLATE ROTATION TOL. = 5.0 Deg.

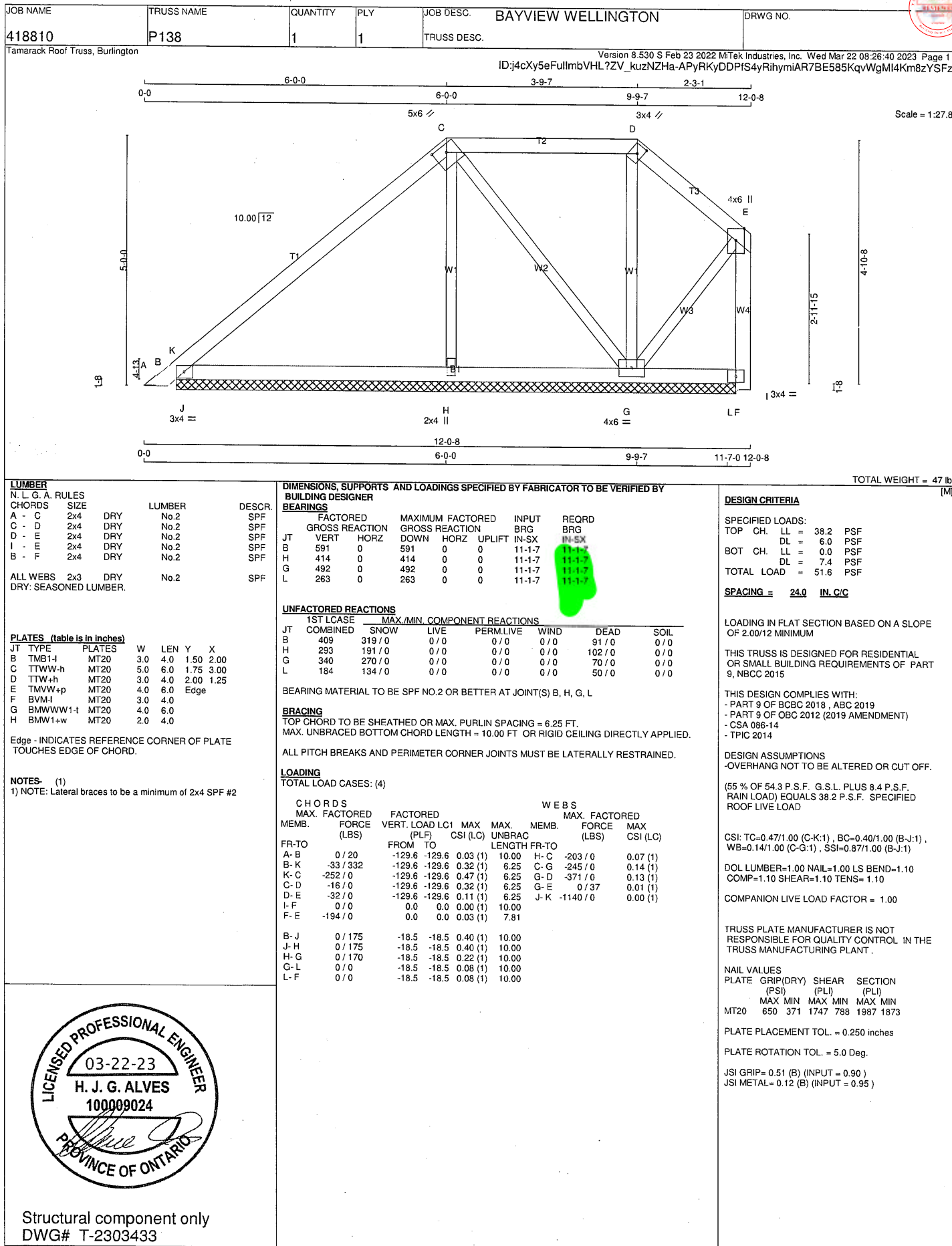
JSI GRIP= 0.34 (D) (INPUT = 0.90)

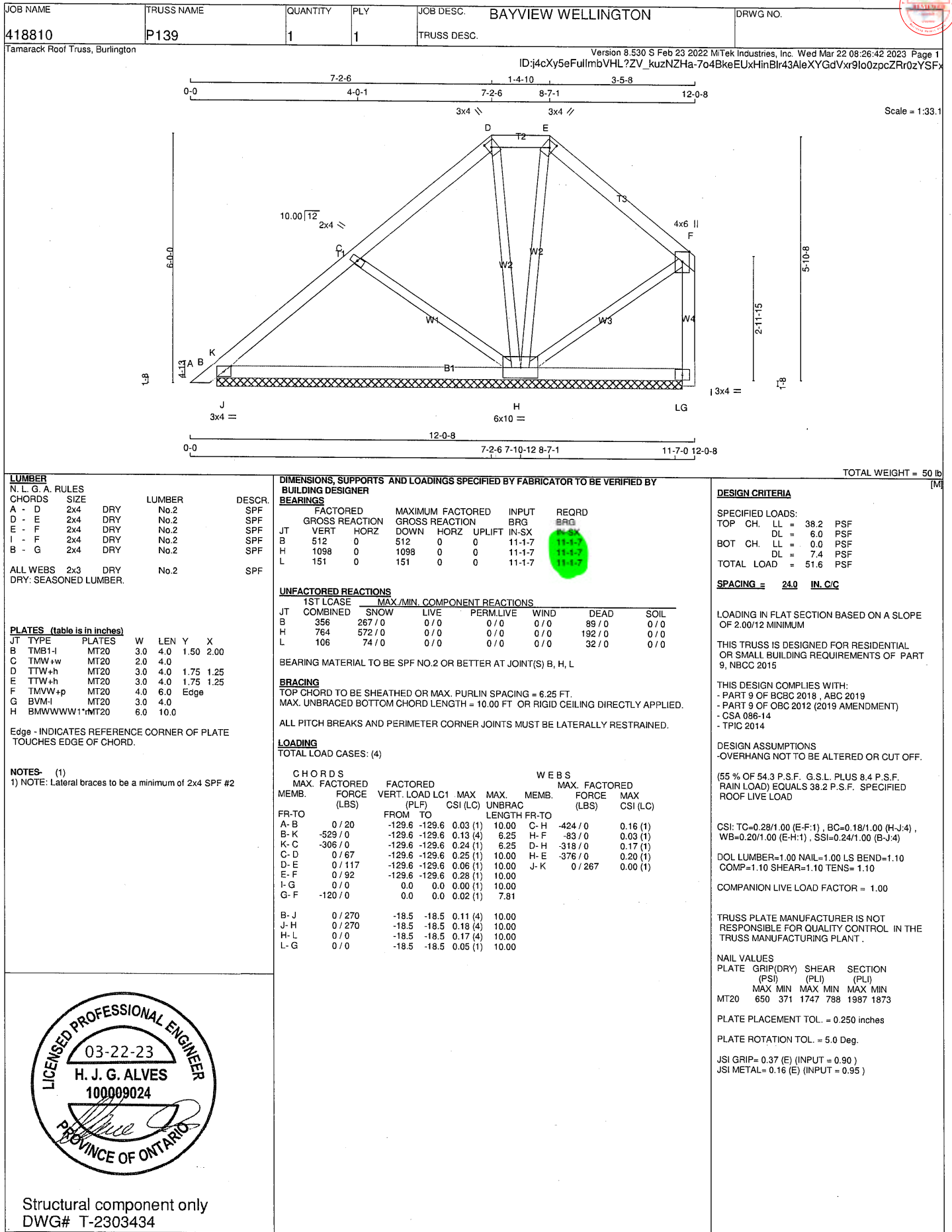
JSI METAL= 0.14 (D) (INPUT = 0.95)

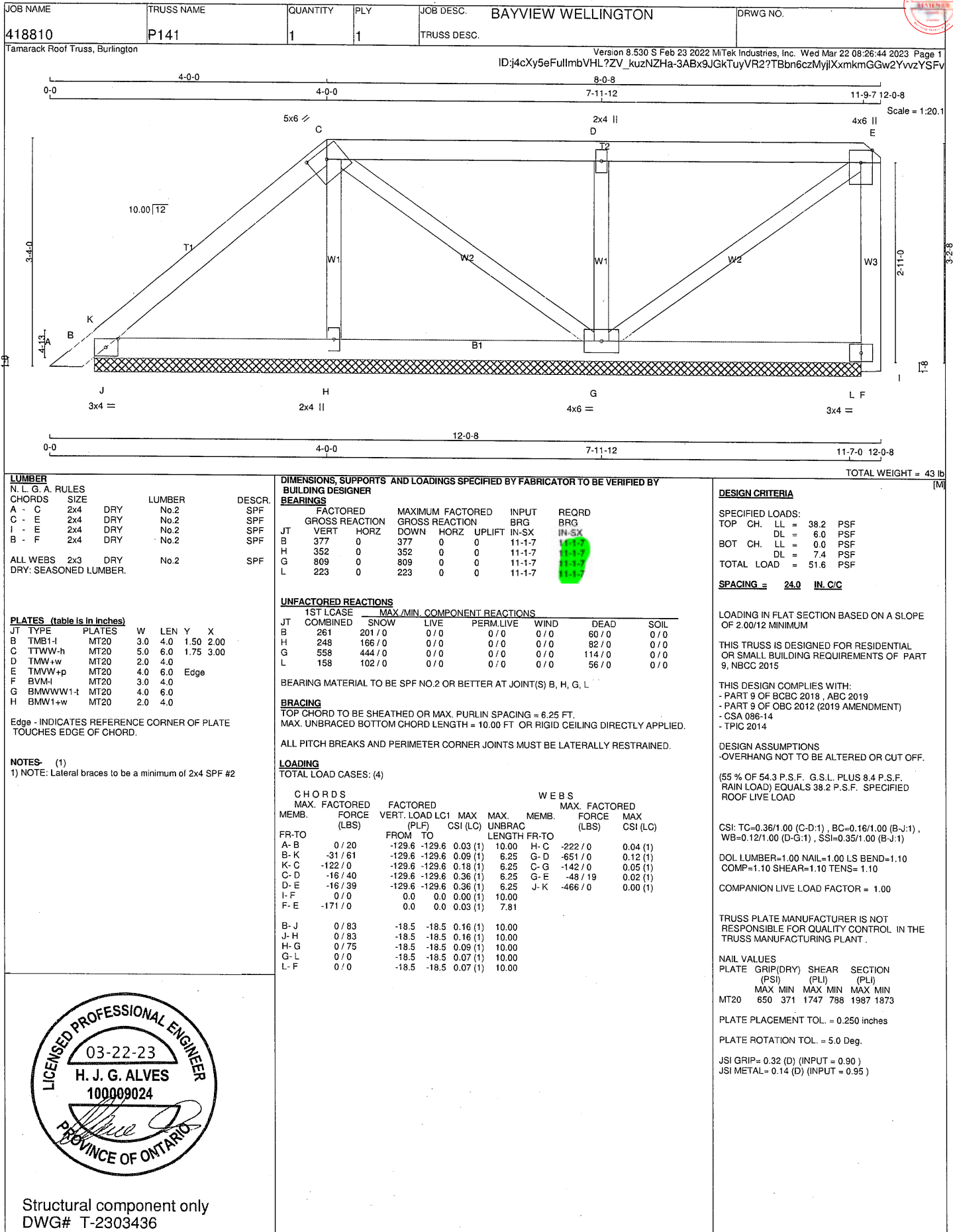


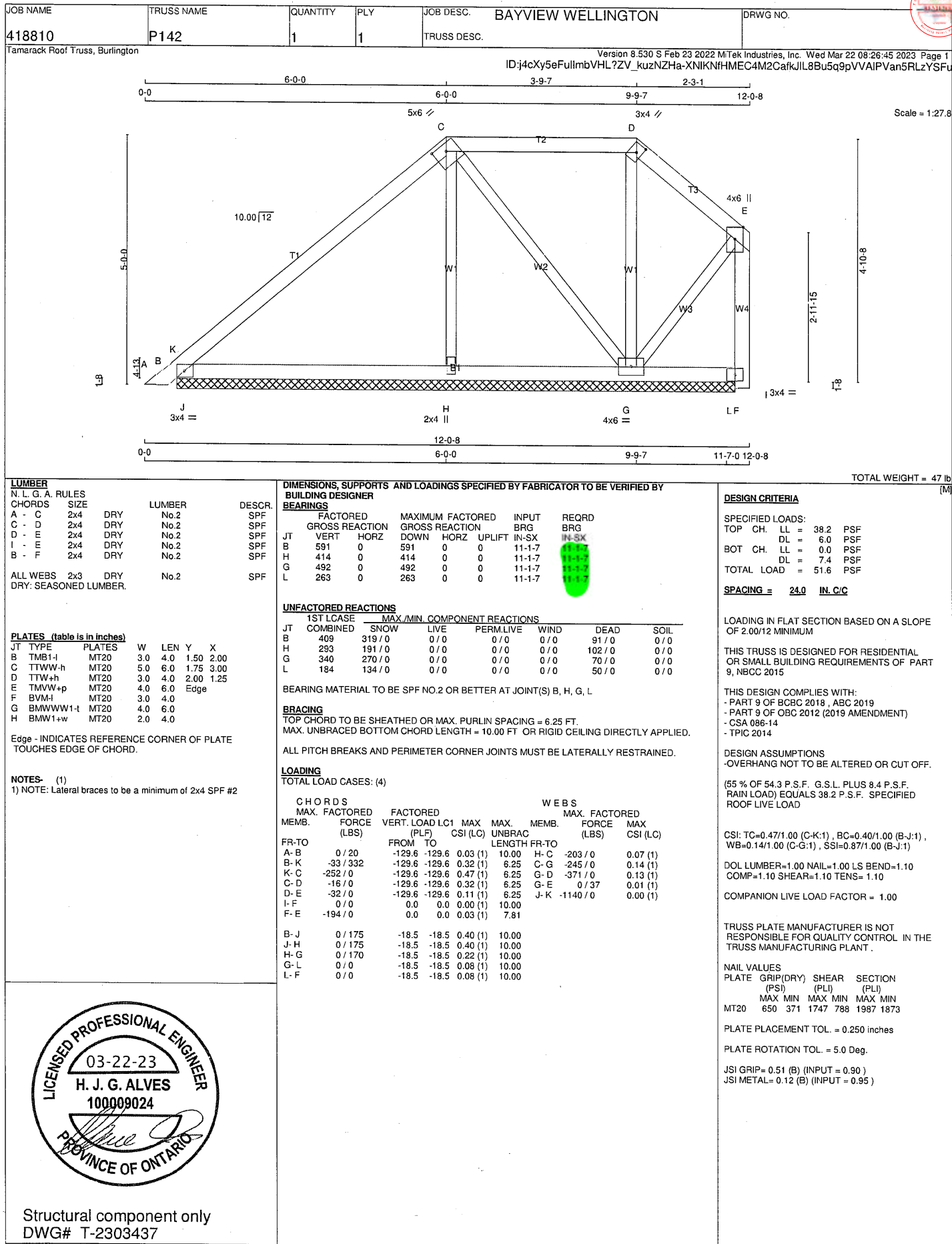
Structural component only
DWG# T-2303431

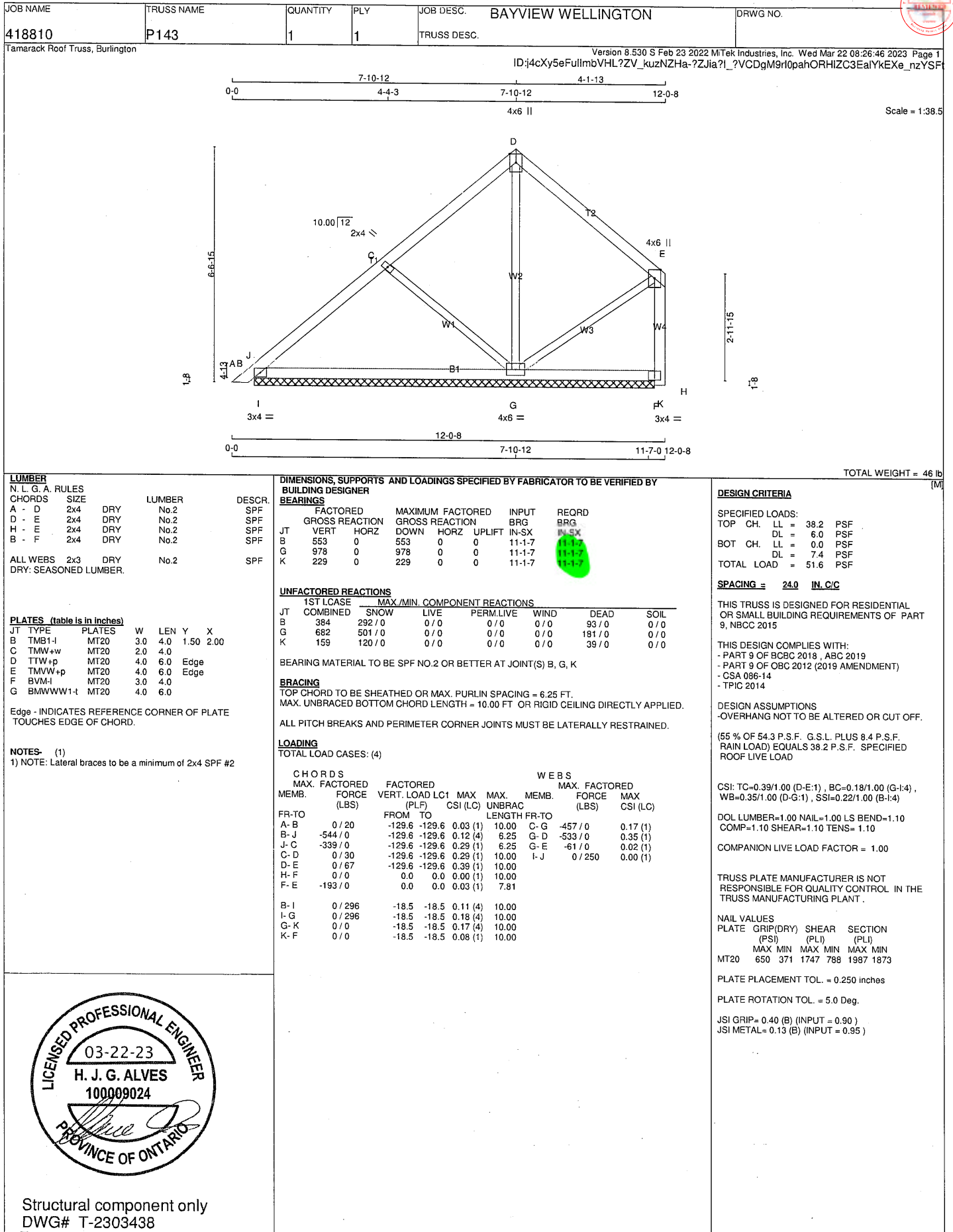








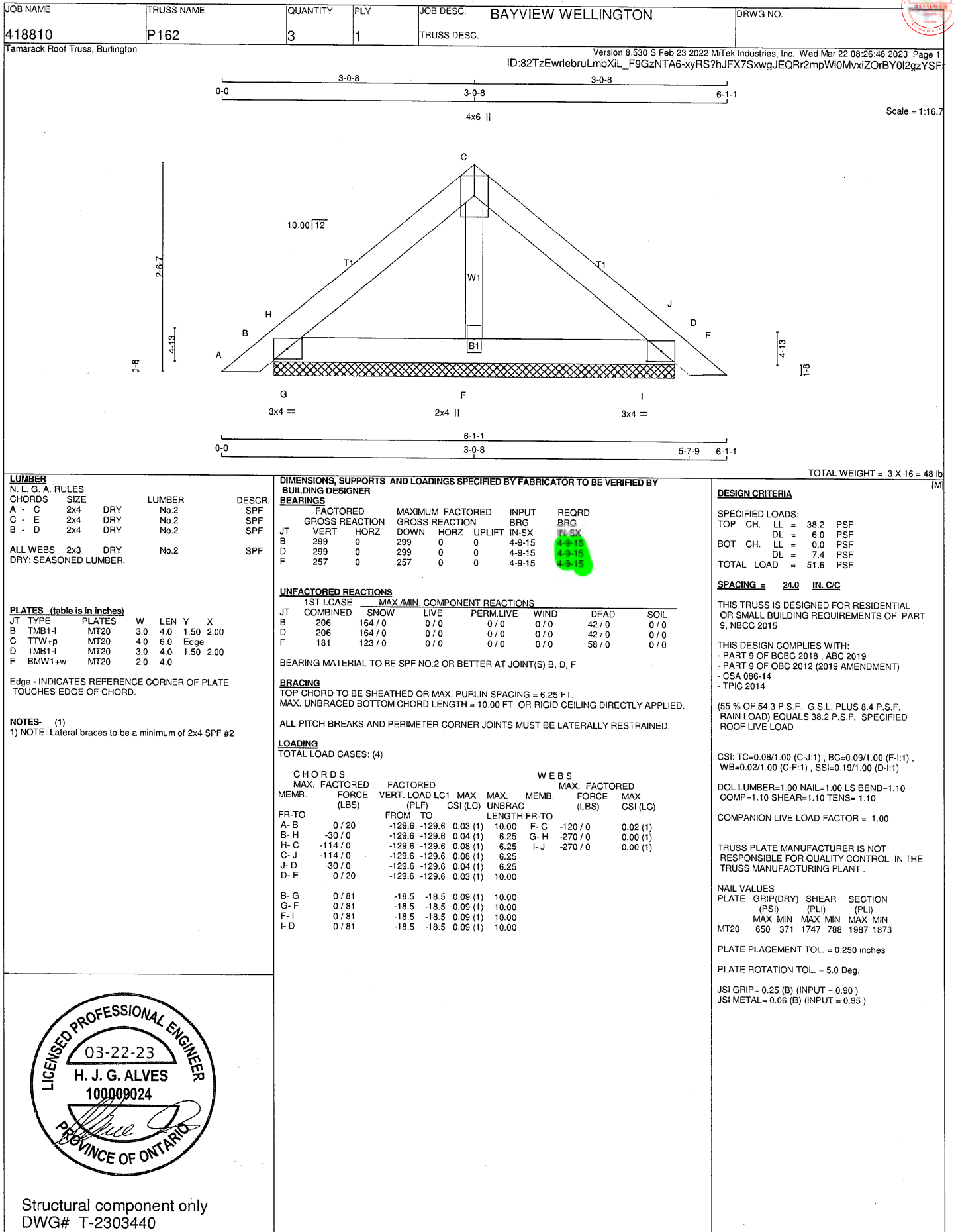


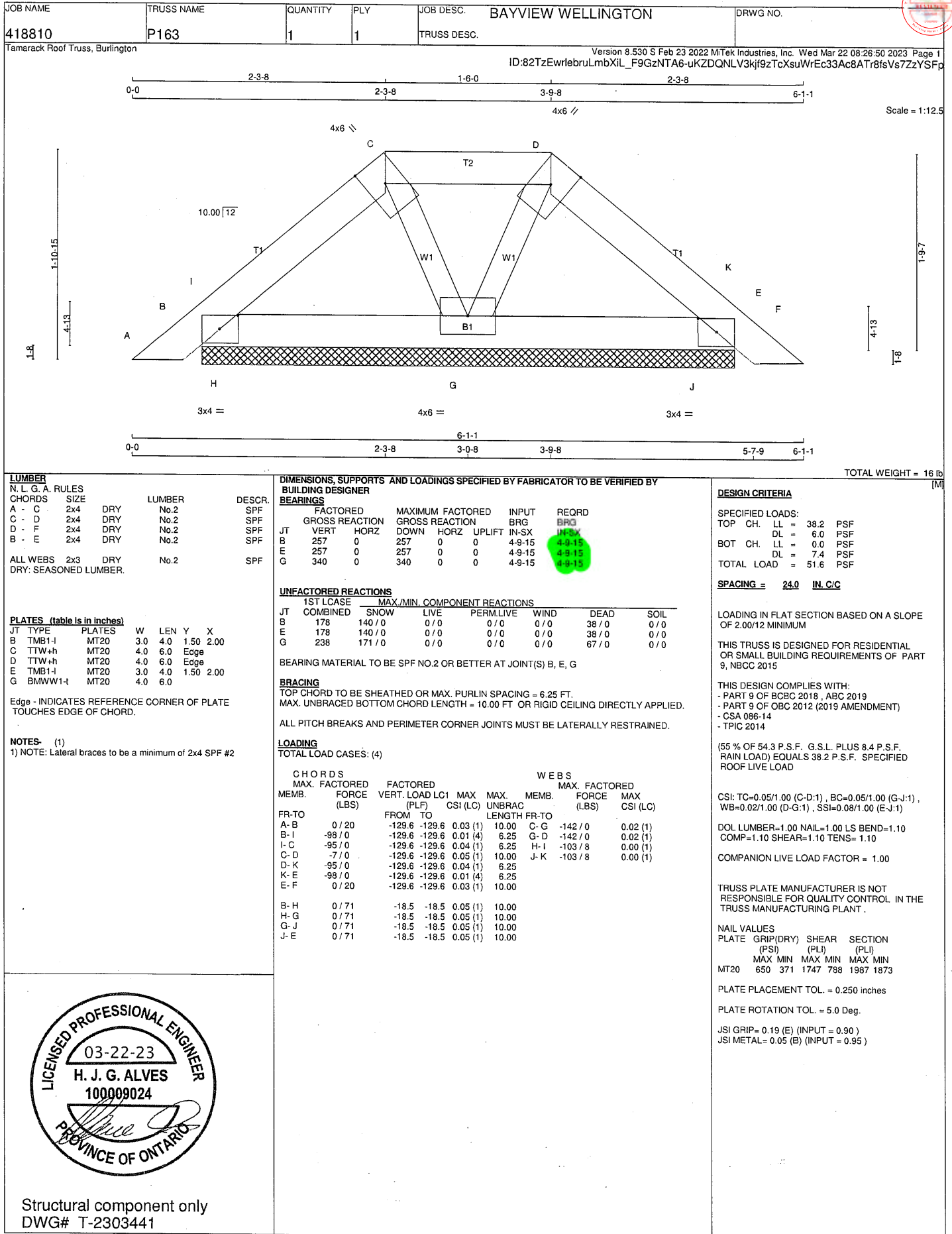


Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:47 2023 Page 1
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Structural component only
DWG# T-2303439





JOB NAME
418810

Tamarack Roof Truss, Burlington

TRUSS NAME
J1

QUANTITY
8

PLY
1

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:28 2023 Page 1

ID:4cXy5eFullmbVHL?ZV_kuzNZHa-X5Duor4IEzxnWbnOFEUMybh7SI3g0WjvbRAhMqzYSG9

1-3-8 5-10-8

-1-3-8 0-0 5-10-8 5-10-8

Scale = 1:23.1

6.00' | 12

3x4 ||

B

WT

E

3x4 ||

D

5-10-8

5-5-0 5-10-8

TOTAL WEIGHT = 8 X 17 = 134 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	716	0	716	0	5-8	5-8
C	286	0	286	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LC CASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	496	384 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	195	168 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX				MEMB. MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
		FROM TO	PLF	CSI (LC)	UNBRAC LENGTH		
FR-TO							
E-B	-652 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 39	-129.6	-129.6	0.17 (1)	10.00		
B-C	-43 / 0	-129.6	-129.6	0.76 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.76/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)

JSI METAL= 0.18 (B) (INPUT = 0.95)

Structural component only

DWG# T-2303421

JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

DRWG NO.

418810

J6

4

1

BAYVIEW WELLINGTON

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:29 2023 Page 1

ID:j4cXy5eFullmbVHL?ZV_kuzNZHa-?InG0B4K?H3d8lMaoy0bVpEOyiQglzz2q5vFuGzYSG8

1-3-8

3-10-8

1-3-8

0-0

3-10-8

3-10-8

4-10-7

1-7-11

10.00

12

4x6 II

T1

B

W1

W2

B1

A

F

3x4 II

E

2x4

D

3-10-8

0-0

1-6-0

3-5-0

3-10-8

Scale = 1:27.3

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

SPF

F - B

2x4

DRY

No.2

SPF

A - C

2x4

DRY

No.2

SPF

F - D

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMVW+p

MT20

4.0

6.0

Edge

E

BMW+w

MT20

2.0

4.0

F

BMV1+p

MT20

3.0

4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

MAXIMUM FACTORED

GROSS REACTION

INPUT

BRG

REQD

BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

F

467

0

467

0

0

5-8

C

251

0

251

0

0

1-8

D

36

0

40

0

0

1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE

MAX/MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

F

322

254/0

0/0

0/0

0/0

69/0

0/0

C

171

148/0

0/0

0/0

0/0

23/0

0/0

D

29

0/0

0/0

0/0

0/0

29/0

0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB.

MAX. FACTORED

FORCE

VERT. LOAD

LC1

MAX

CS1 (LC)

UNBRAC

MEMB.

MAX. FACTORED

FORCE

MAX

CS1 (LC)

FR-TO

FROM TO

LENGTH

FR-TO

F-B

-431/0

0.0

0.0

0.05 (1)

7.81

B-E

0/0

0.00 (1)

A-B

0/57

-129.6

-129.6

0.18 (5)

10.00

B-C

0/0

-129.6

-129.6

0.33 (1)

10.00

F-E

0/0

-18.5

-18.5

0.08 (4)

10.00

E-D

0/0

-18.5

-18.5

0.08 (4)

10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.33/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)

JSI METAL= 0.11 (B) (INPUT = 0.95)

03-22-23

H. J. G. ALVES

100009024

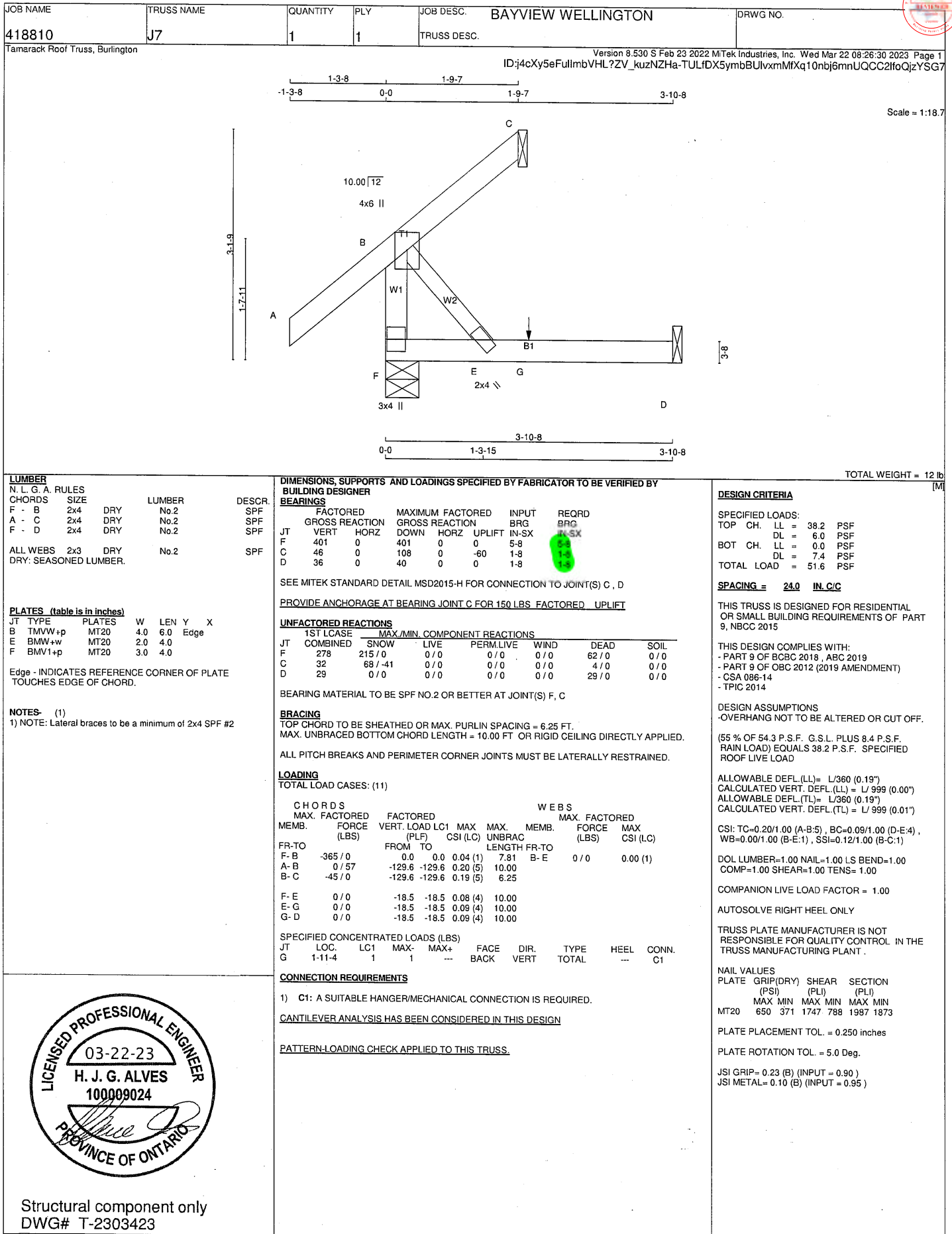
PROVINCE OF ONTARIO

Structural component only

DWG# T-2303422

TOTAL WEIGHT = 4 X 15 = 61 lb

[M]



JOB NAME
418810

Tamarack Roof Truss, Burlington

TRUSS NAME
J8

QUANTITY
1

PLY
1

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Mar 22 08:26:31 2023 Page 1

ID:j4cXy5eFullmbVHL?ZV_kuzNZHa-xgv1Rt6aXUJLN3WzwN23aEJnrW76DtsLHPOMz9zYSG6

Scale = 1:18.7

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	382	0	382	0	5-8	5-8
C	46	0	46	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
F	263	215/0	0/0	0/0	0/0	48/0	0/0
C	32	27/-41	0/0	0/0	0/0	4/0	0/0
D	14	0/0	0/0	0/0	0/0	14/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	VERT. LOAD LC1	MAX. CSI (LC)			MAX. FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO						FR-TO		
F-B	-365/0	0.0	0.0	0.04 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/57	-129.6	-129.6	0.18 (1)	10.00			
B-C	-45/0	-129.6	-129.6	0.17 (1)	6.25			
F-E	0/0	-18.5	-18.5	0.02 (4)	10.00			
E-D	0/0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	DL
LL	38.2	PSF
DL	6.0	PSF

BOT CH.

LL	DL
LL	0.0
DL	7.4

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.18/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

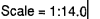
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 0.95)

Structural component only

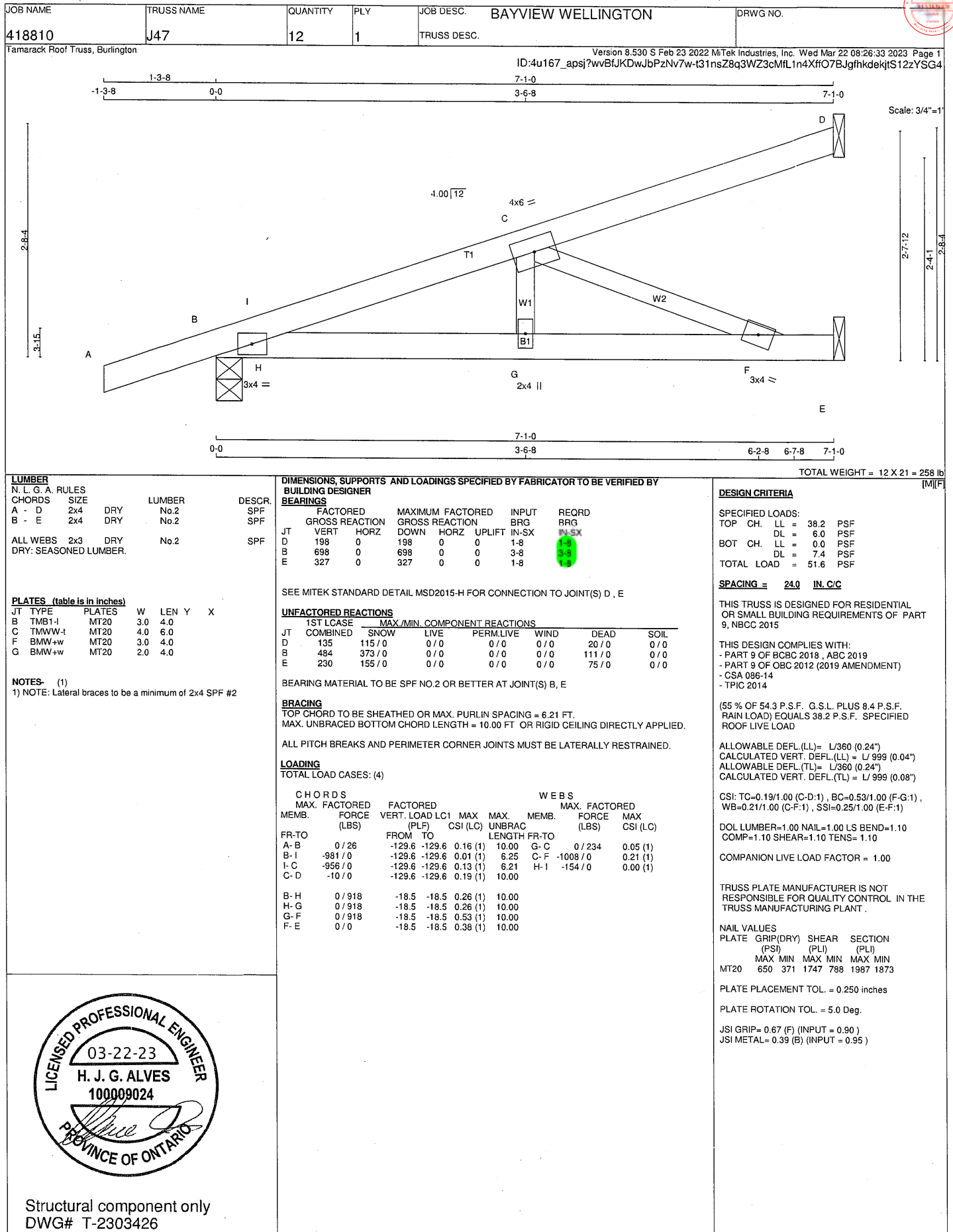
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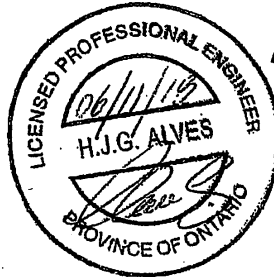


M][F]

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

Structural component only
DWG# T-2303425





Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufacturer's responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300213

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

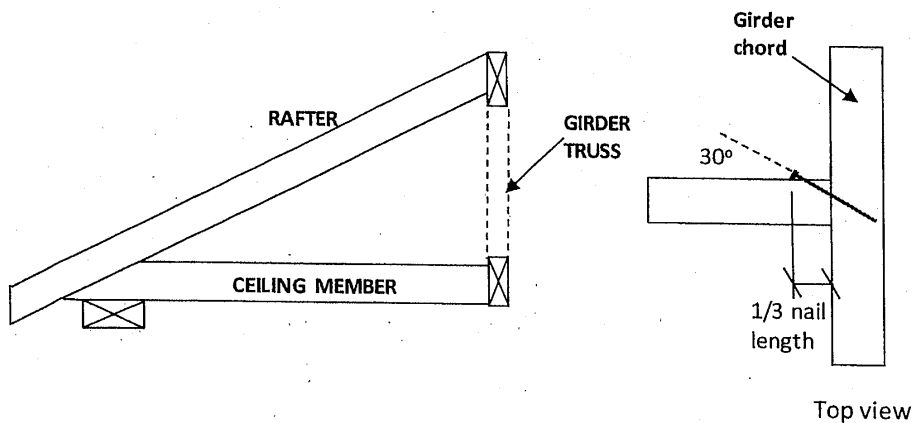
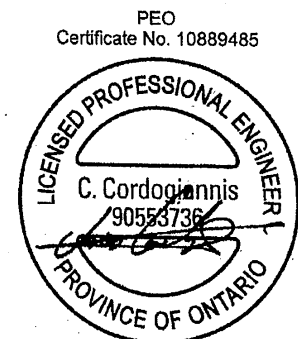


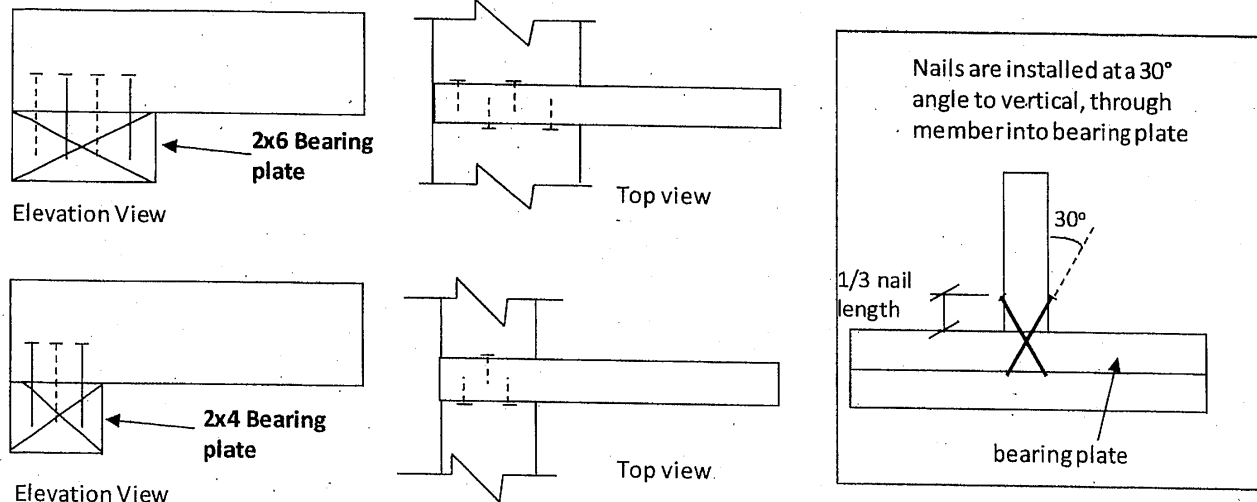
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_0 factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

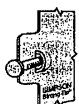
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

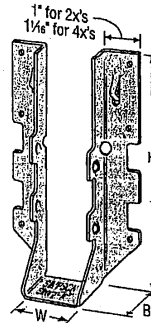
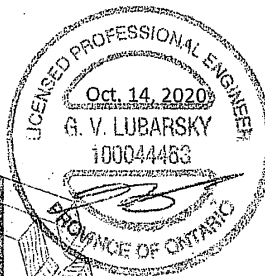
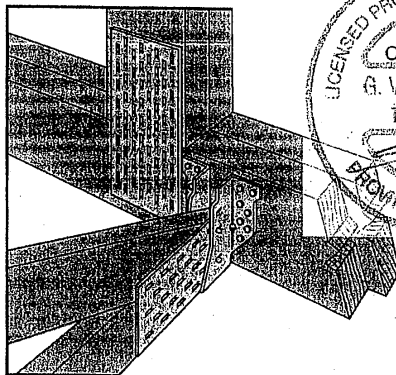


Double-Shear Nailing Side View; Do not bend tab

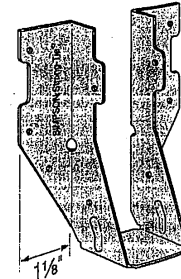


Dome Double-Shear Nailing Side View (available on some models)

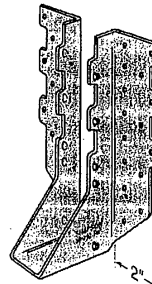
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



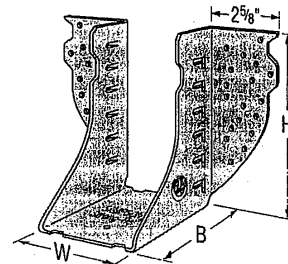
✓ LUS28



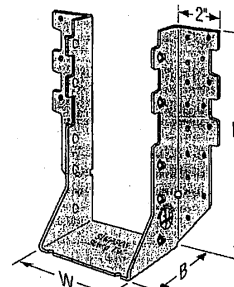
LU26L



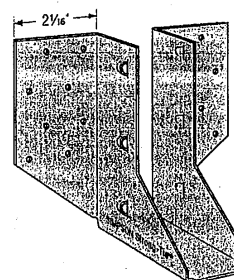
✓ HUS210
(HUS26, HUS28, and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



LJS26DS

TECHNICAL BULLETIN

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

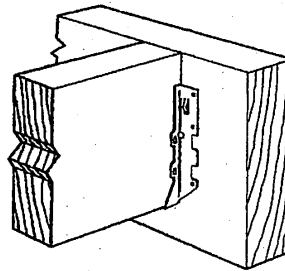
- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

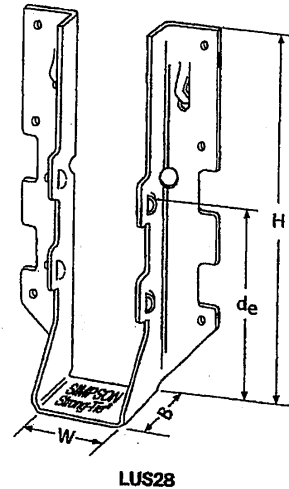
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

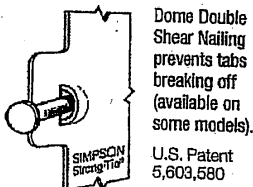


Typical LUS Installation

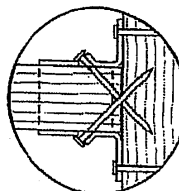


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1½	3½	1¾	1 15/16	(4) 10d	(2) 10d	(K _u =1.15)	(K _n =1.00)	(K _u =1.15)	(K _n =1.00)
LUS24-2	18	3½	3½	2	1 15/16	(4) 16d	(2) 16d	710	1630	645	1155
LUS26	18	1½	4¾	1¾	3¾	(4) 10d	(4) 10d	835	2020	590	1435
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1420	2170	1290	1630
LUS26-3	18	4¾	4¾	2	3¾	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1545	2340
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¾	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 15/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8 3/8	2	5¾	(8) 16d	(6) 16d	2580	3345	2320	2375

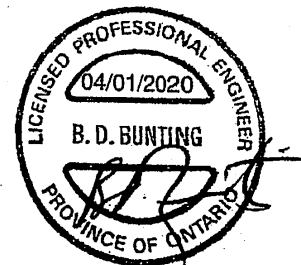
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com



TECHNICAL BULLETIN

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

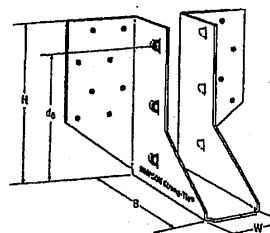
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

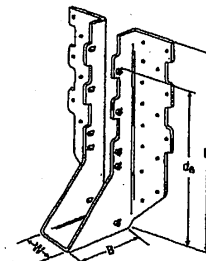
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

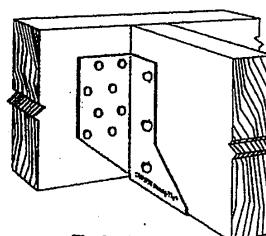
- See current catalogue for options



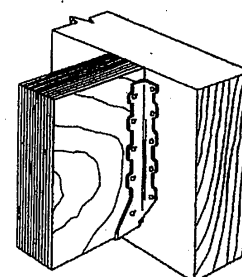
LJS26DS



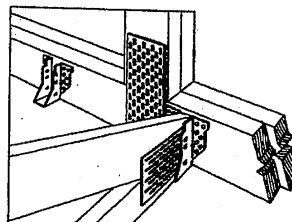
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



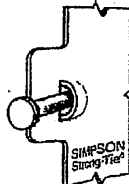
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

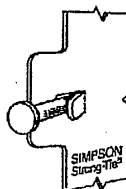
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _a ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 1/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_a is the distance from the seat of the hanger to the highest joist nail.

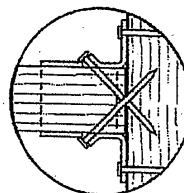


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

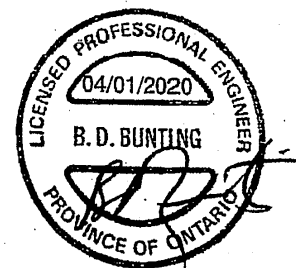
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

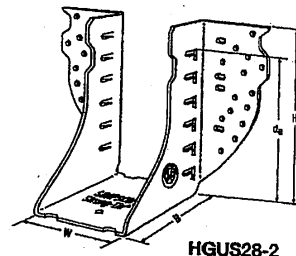
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

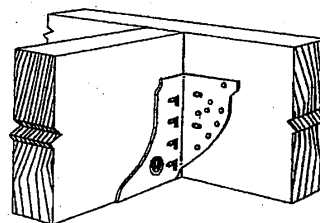
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

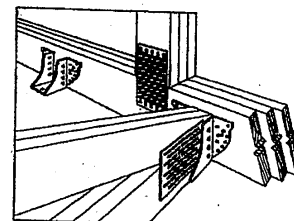
- See current catalogue for options



HGUS28-2



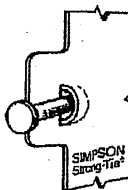
Typical HGUS
Installation



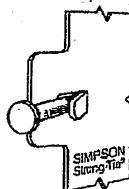
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	8625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

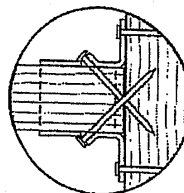
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

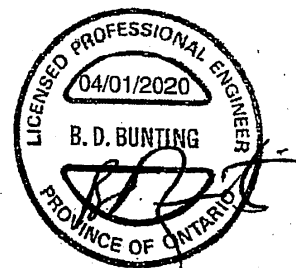


LIMIT
STATES
DESIGN

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T-SPECHGUS20 3/20 exp. 6/22



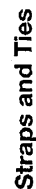
(800) 999-5099
strongtie.com

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

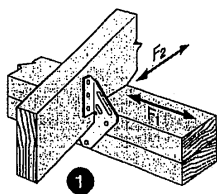
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp.20–24 or visit strongtie.com.

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.

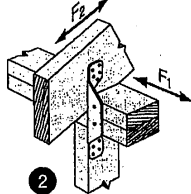


H/TSP

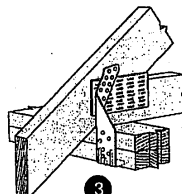
Seismic and Hurricane Ties (cont.)



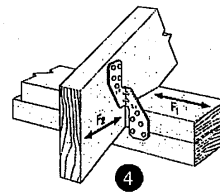
1 H1 Installation



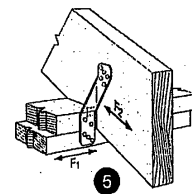
2 H2A Installation



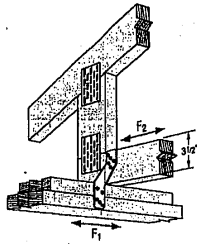
3 TSP Installation



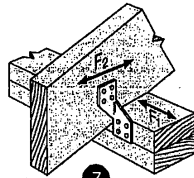
4 H2.5A Installation
(Nails into both top plates)



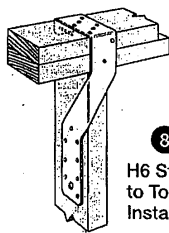
5 H2.5T Installation
(Nails into both top plates)



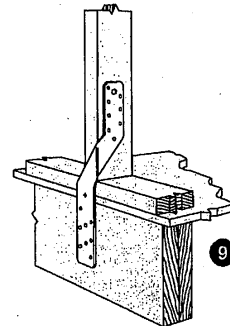
6 H2.5T Installation



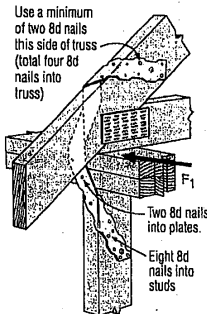
7 H3 Installation
(Nails into upper top plate)



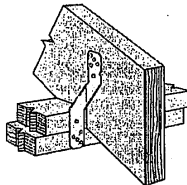
8 H6 Stud to Top Plate Installation



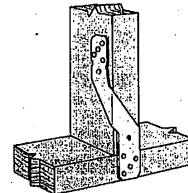
9 H6 Stud to Band Joist Installation



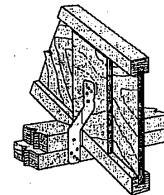
10 H7Z Installation



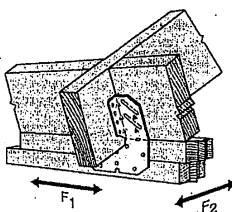
11 H8 Attaching Rafter to Double Top Plates



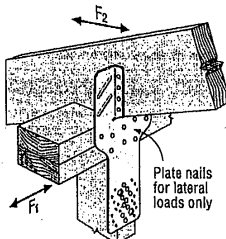
12 H8 attaching Stud to Sill
(4) 8d into plate, (5) 8d into stud



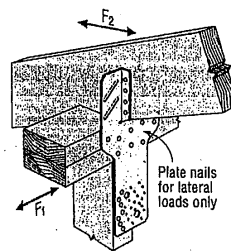
13 H8 attaching I-Joist to Double Top Plates



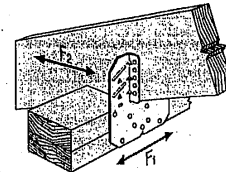
14 H10A Field-Bent Installation



15 H10S Installation

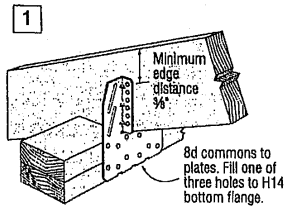


16 H10S Installation with Stud Offset

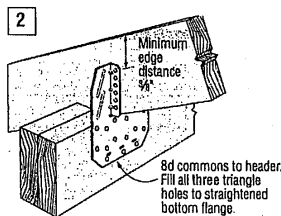


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

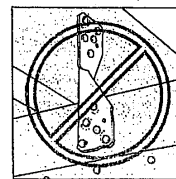


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.



TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

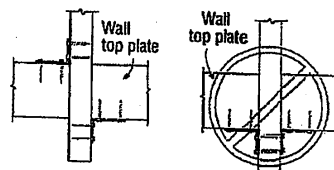
Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

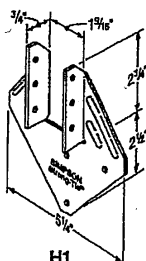
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

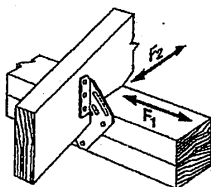


Install diagonally across from each other for minimum 2x truss.

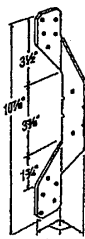
Nailing into both sides of a single ply 2x truss may cause the wood to split.



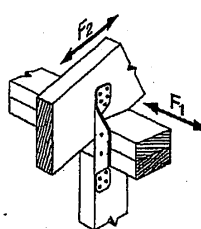
H1



H1 Installation



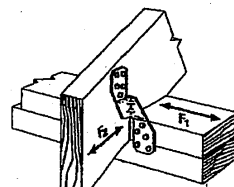
H2A



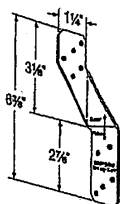
H2A Installation



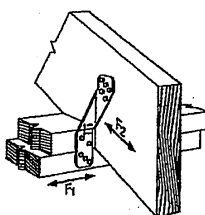
H2.5A



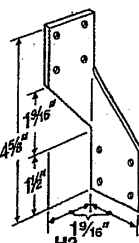
H2.5A Installation



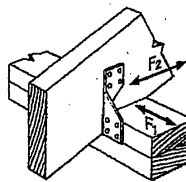
H2.5T



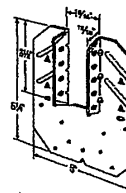
H2.5T Installation
(Nails into both top plates)



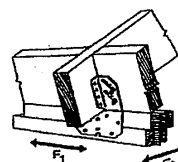
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

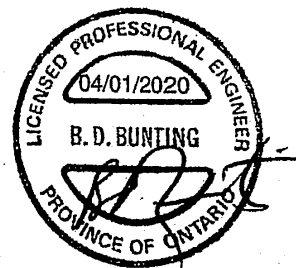


LIMIT
STATES
DESIGN

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T-SPECH20 3/20 exp. 6/22



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strongtie.com



TECH-NOTES

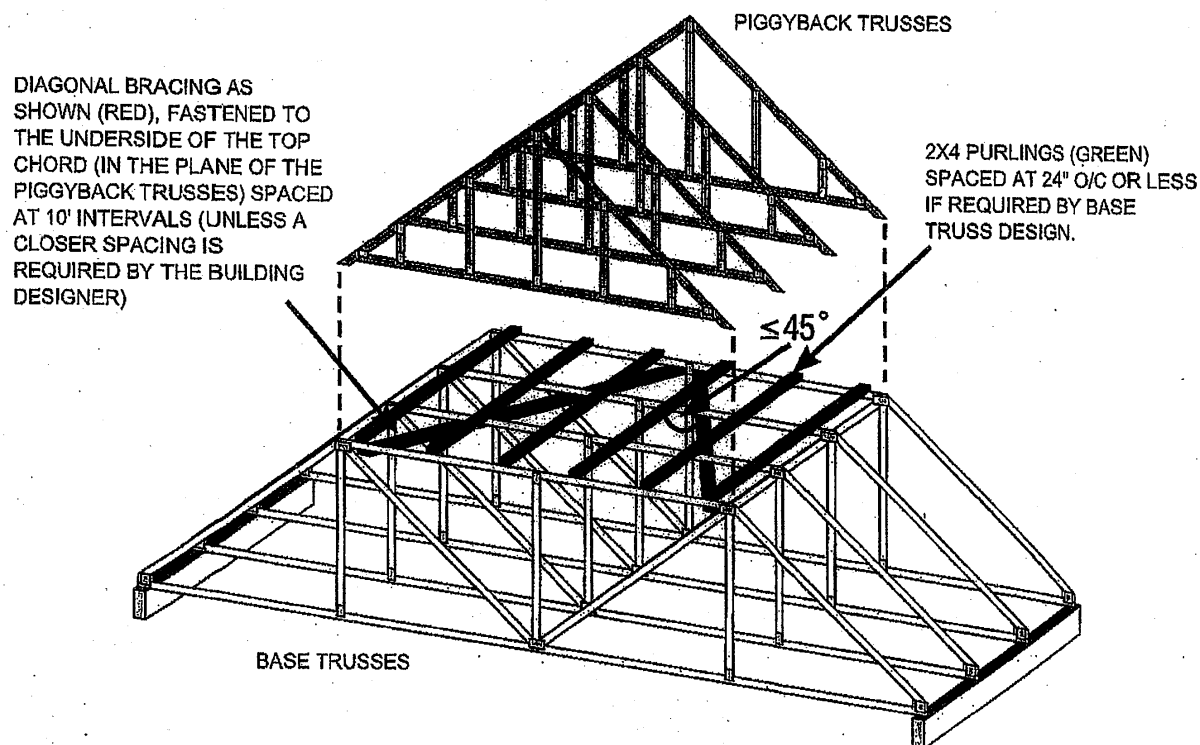
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

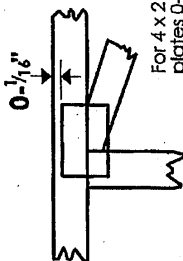
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

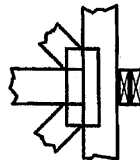
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, I or Eliminator bracing if indicated.

BEARING



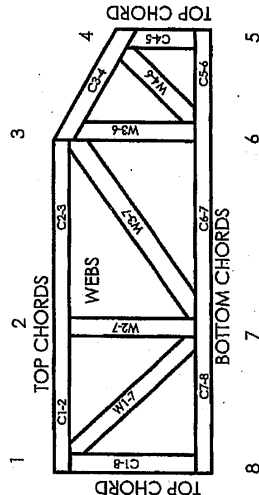
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet: MIL-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See 9C31.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

