UNIT1
RL-5E - KINGFISHER 5
ELV (B)
405036

UNIT2
RL-2 - KINGFISHER 2
ELV (B) REV
405037

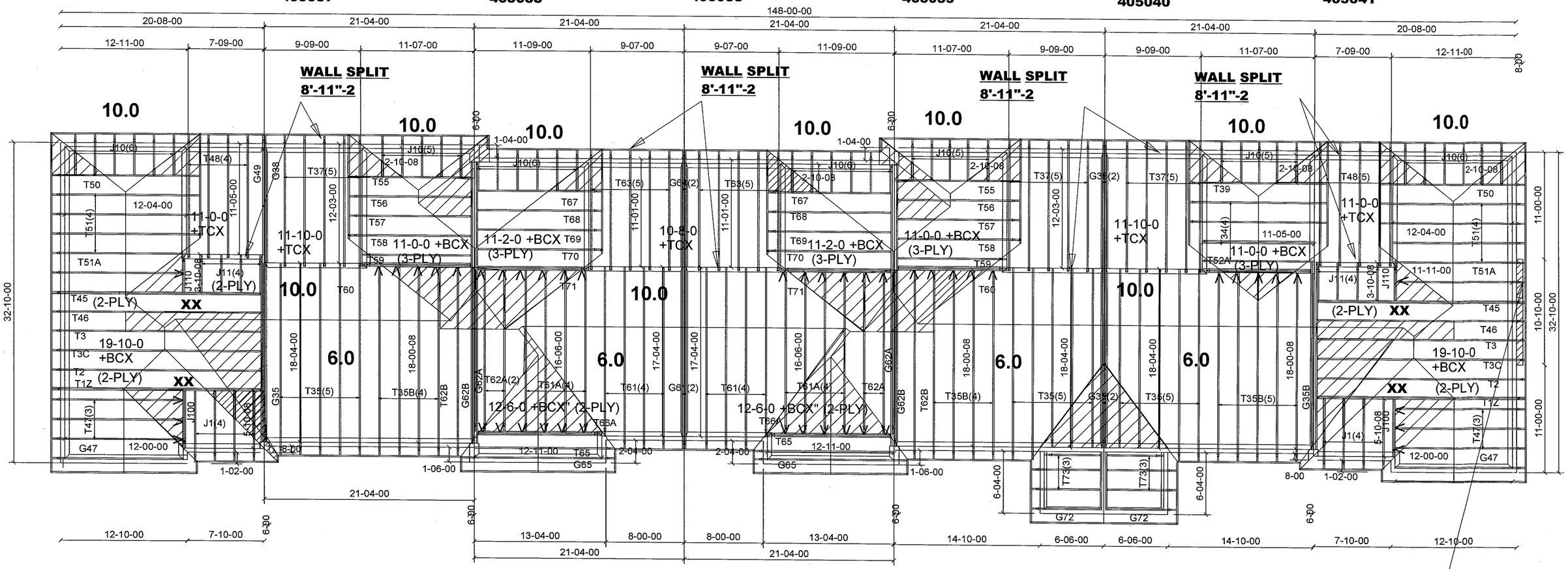
UNIT3
RL-1 - KINGFISHER 1
ELV (B)
405038

UNIT4
RL-1 - KINGFISHER 1
ELV (B) REV
405038

UNIT5
RL-2- KINGFISHER 2
ELV (B2)
405039

UNIT6
RL-2 - KINGFISHER 2
ELV (B2) REV
405040

UNIT7
RL-5E - KINGFISHER 5
ELV (B) REV
405041



ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS -(V)
LUS24 -(O)
HGUS26-2 -(XX)

8/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES CONVENTIONAL FRAMING

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

SIDE UPGRADE
CONV. FRAMING

TAMARACK LUMBER INC.
A LP/L LUMBER GROUP

Job Track: **50469**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **BLOCK 5 / UNIT 1-7**	Mitek ver 8.5.3.233
Plan Log: **200925**	Project: **ALCONA SHORES**		
Layout ID: **402645**	Date: 2023-03-28 Sales: **Mario DiCano** Designer: wassim/s.v.e		

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

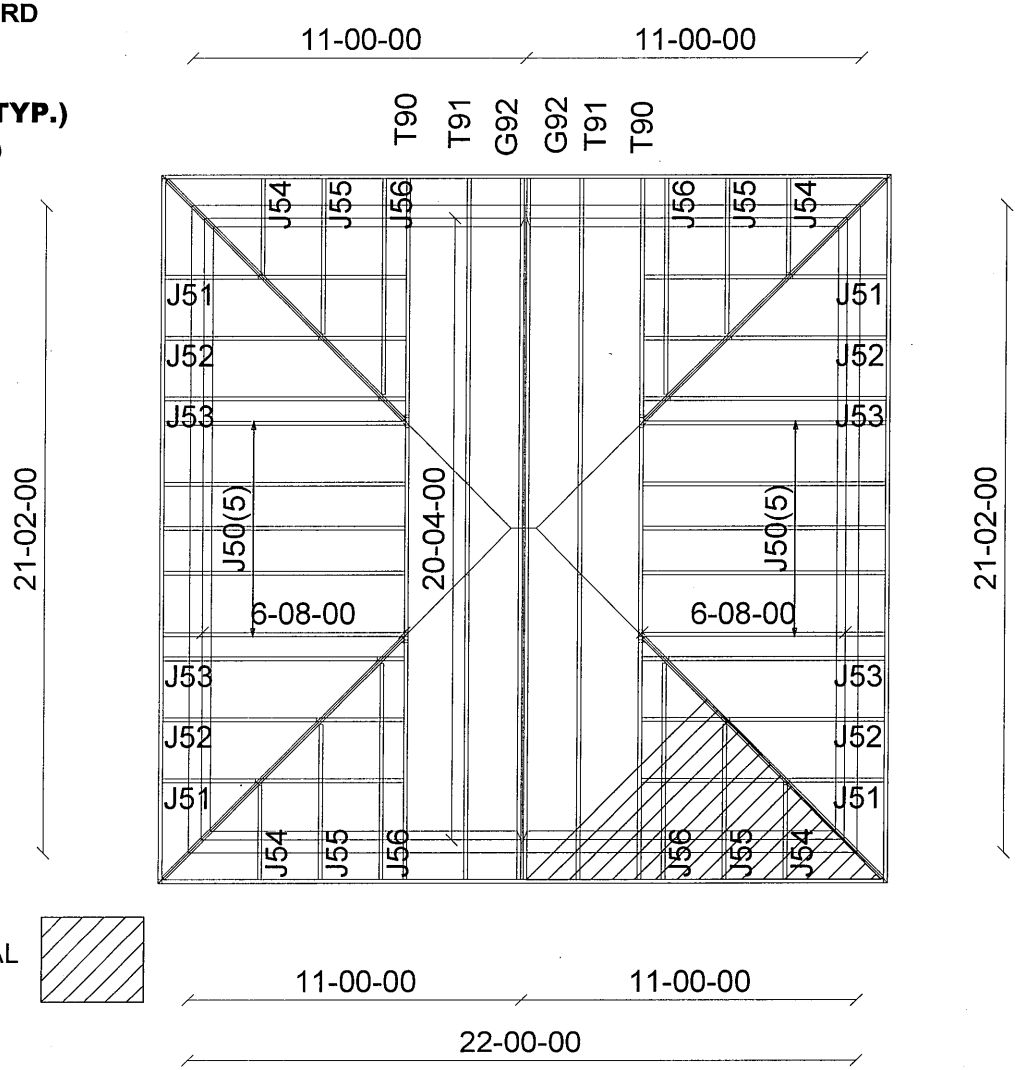
UNIT 2
DG-1
EL:B (REV)
404952

UNIT 1
DG-1
EL:B2
404952

6/12 PITCHES (TYP.)
UNLESS NOTED

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf



All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DENOTES CONVENTIONAL FRAMING

GARAGE

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **50469**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **BLOCK 5 -DG-1 EL:B UNIT 2 PAIRED WITH DG-1 EL:B2 UNIT 1**
	Plan Log: **200925**	Project: **ALCONA SHORES**	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Layout ID: **405054**	Date: **2023-03-28** Sales: Mario DiCano Designer: S.V	Mitek ver 8.5.3.233

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

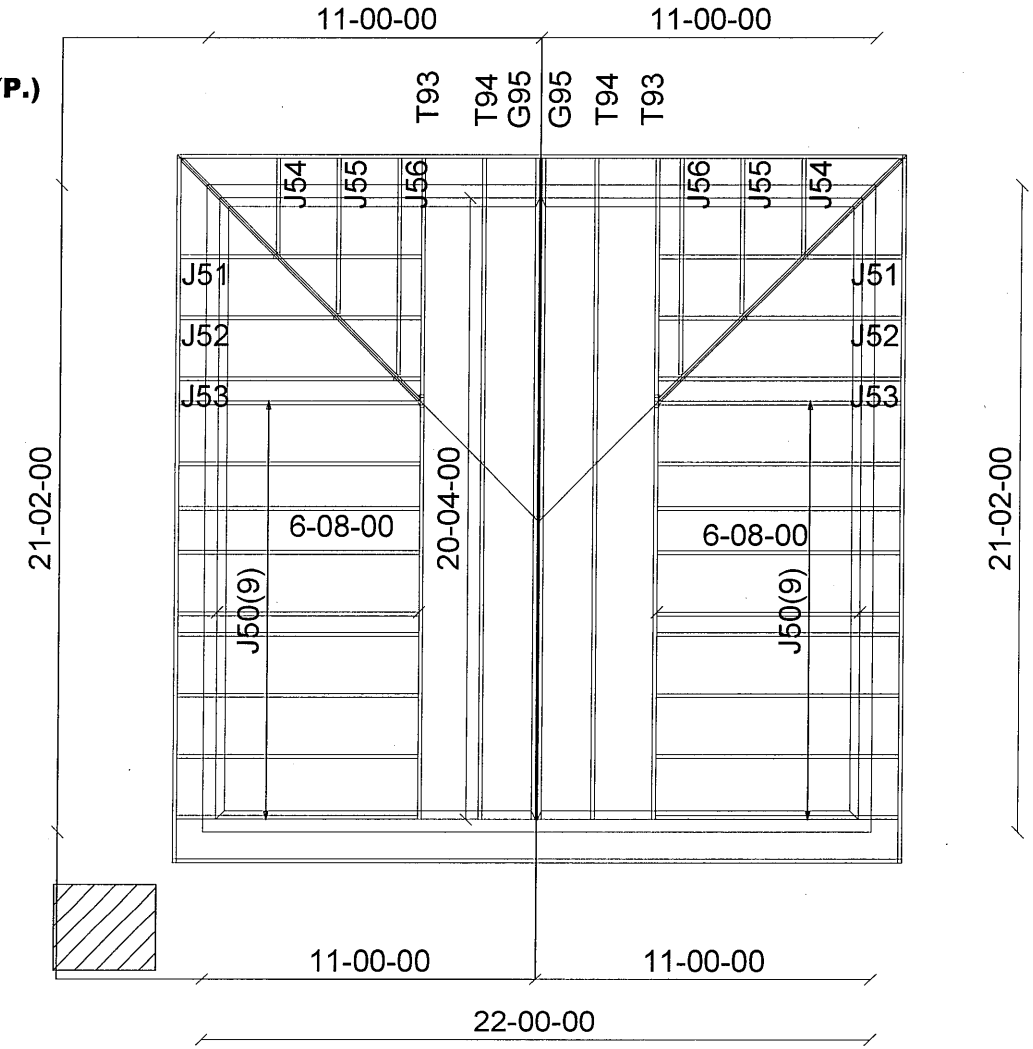
UNIT 4
DG-1
EL:B3 (REV)
404959

UNIT 3
DG-1
EL:B3
404959

6/12 PITCHES (TYP.)
UNLESS NOTED

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf



All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DENOTES CONVENTIONAL FRAMING

GARAGE

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **50469**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **BLOCK 5 -DG-1 EL:B3 UNIT 4 PAIRED WITH DG-1 EL:B3 UNIT 3**
	Plan Log: **200925**	Project: **ALCONA SHORES**	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Layout ID: **405052**	Date: **2023-03-28** Sales: Mario DiCano Designer: S.V	Mitek ver 8.5.3.233

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

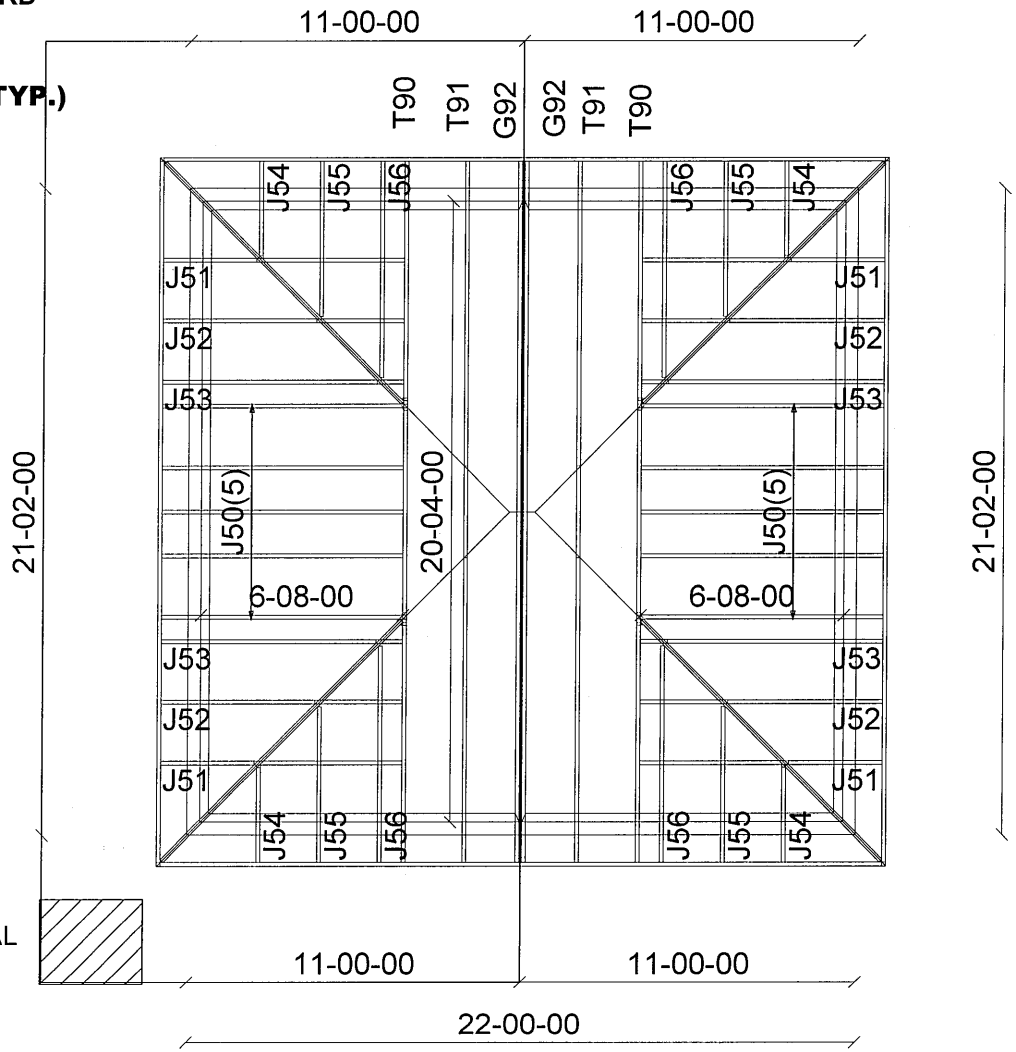
UNIT 6
DG-1
EL:B (REV)
404952

UNIT 5
DG-1
EL:B
404952

6/12 PITCHES (TYP.)
UNLESS NOTED

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf



DENOTES CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

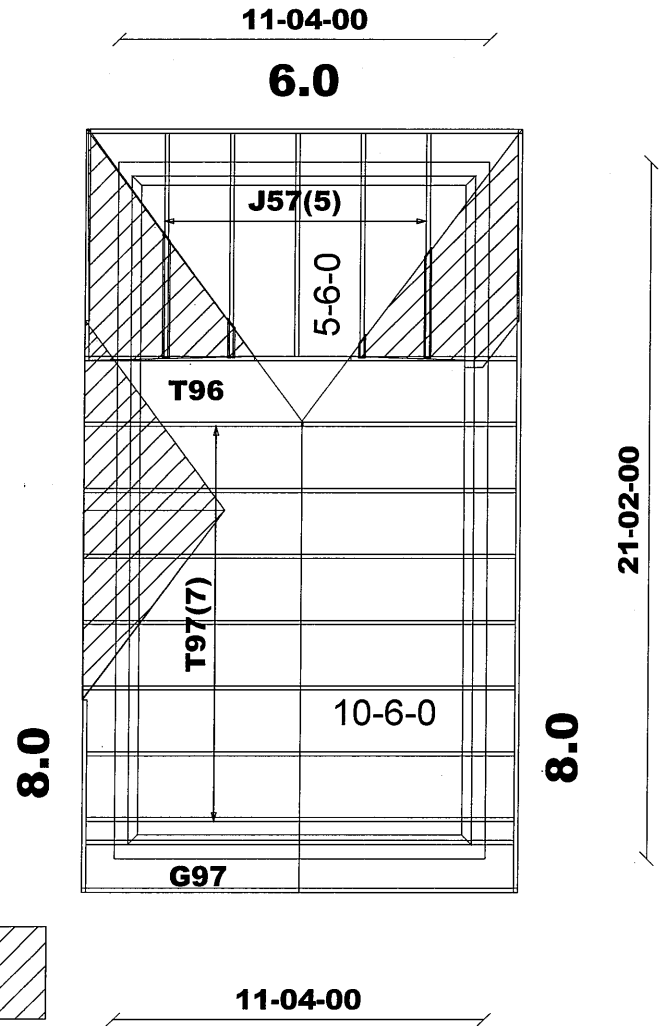
GARAGE

TAMARACK LUMBER INC.	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 5 -DG-1 EL:B UNIT 6 PAIRED WITH DG-1 EL:B UNIT 5
	Plan Log: 200925	Project: ALCONA SHORES	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Layout ID: 405050	Date: 2023-03-28 Sales: Mario DiCano Designer: S.V	Mitek ver 8.5.3.233

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

UNIT 7
DG-2
EL:B UPGRADE (REV)
405047

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.



DENOTES CONVENTIONAL FRAMING

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

GARAGE

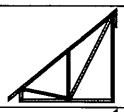
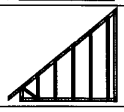
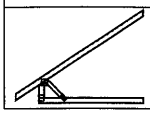
TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 5 - DG-2 EL:B UPGRADE UNIT7
	Plan Log: 200925	Project: ALCONA SHORES	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Layout ID: 405047	Date: 2023-03-28 Sales: Mario DiCano Designer: S.V	Mitek ver 8.5.3.233

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405036
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B- UNIT1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

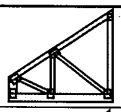
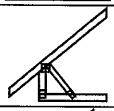
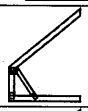
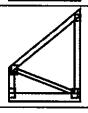
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1Z Hip Girder	8 /12	**19-10-00**	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 1-08-02	188.13 121.33		
	1	T2 Hip	8 /12	**19-10-00**	6-07-13	2 x 4	1-03-08	1-04-13 1-08-02	88.33 58.17		
	1	T3 Common	8 /12	**19-10-00**	8-01-13	2 x 4	1-03-08	1-04-13 1-08-02	86.49 55.00		
	1	T3C Hip	8 /12	**19-10-00**	7-11-13	2 x 4	1-03-08	1-04-13 1-08-02	86.28 55.00		
	1 2-ply	T45 Half Hip Girder	8 /12	**19-10-00**	5-01-05	2 x 4 2 x 6	1-03-08	1-04-13 5-01-05	198.9 128.67		
	1	T46 Half Hip	8 /12	**19-10-00**	6-09-05	2 x 4	1-03-08	1-04-13 6-09-05	90.19 58.00		
	3	T47 Common	8 /12	**12-00-00**	5-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	159 95.50		
	1	G47 GABLE	8 /12	**12-00-00**	5-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	52.42 32.83		
	4	T48 Monopitch	10 /12	**11-03-08**	11-00-10	2 x 4	1-03-08	1-07-11 11-00-10	272.5 170.67		
	1	G49 GABLE	10 /12	**11-00-00**	10-09-11	2 x 4	1-03-08	1-07-11 10-09-11	70.31 45.33		
	1	T50 Hip Girder	8 /12	**12-04-00**	4-00-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	52.2 33.67		
	4	T51 Common	8 /12	**12-04-00**	5-06-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	217 139.33		
	1	T51A Common	8 /12	**11-11-00**	5-06-02	2 x 4	1-03-08	1-04-13 1-04-13	51.52 33.50		
	4	J1 Jack-Open	8 /12	**5-10-08**	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	76.29 49.33		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405036
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B- UNIT1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	J100 Jack-Open Girder	8 /12	**5-10-08**	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.52 41.33		
	6	J10 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	75.07 50.00		
	4	J11 Jack-Open	10 /12	**3-10-08**	5-01-05	2 x 4		1-10-09 5-01-05	54.61 36.00		
	1 2-ply	J110 Jack-Closed Girder	10 /12	**3-10-08**	5-01-05	2 x 4 2 x 6		1-10-09 5-01-05	44.37 29.33		

TOTAL # TRUSS= **41** TOTAL BFT OF ALL TRUSSES= **1232.99** BFT. TOTAL WEIGHT OF ALL TRSSES **1928.12** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

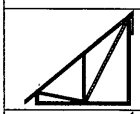
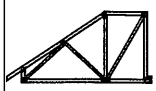
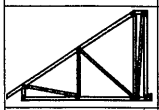
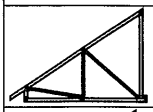
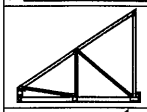
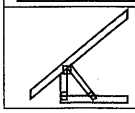
TOTAL NUMBER OF ITEMS= **6**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405037
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B- UNIT2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T35 Roof Special	6 /12	**18-04-00**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	392.11 249.17		
	1	G35 GABLE	6 /12	**18-04-00**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	82.95 52.83		
	4	T35B Roof Special	6 /12	**18-00-08**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	311.27 197.33		
	1	T62B Half Hip	6 /12	**18-00-08**	7-00-08	2 x 4	1-03-08	1-02-00 7-00-08	82.46 52.67		
	1	G62B GABLE	6 /12	**18-00-08**	7-00-08	2 x 4	1-03-08	1-02-00 7-00-08	86.13 56.67		
	5	T37 Monopitch	10 /12	**12-01-08**	11-08-15	2 x 4	1-03-08	1-07-11 11-08-15	367.16 225.83		
	1	G38 GABLE	10 /12	**11-10-00**	11-06-00	2 x 4	1-03-08	1-07-11 11-06-00	72.61 45.67		
	1	T55 Hip Girder	8 /12	**11-00-00**	4-00-07	2 x 4	1-03-08	1-04-13 1-08-02	47.68 31.83		
	1	T56 Common	8 /12	**11-00-00**	5-02-08	2 x 4	1-03-08	1-04-13 1-08-02	47.14 30.83		
	1	T57 Half Hip	8 /12	**11-00-00**	6-03-02	2 x 4	1-03-08	1-04-13 6-03-02	54.84 34.83		
	1	T58 Half Hip	8 /12	**11-00-00**	7-11-02	2 x 4	1-03-08	1-04-13 7-11-02	63.95 41.83		
	1	T59 Monopitch	8 /12	**11-00-00**	8-08-13	2 x 4	1-03-08	1-04-13 8-08-13	54.21 34.33		
	1 3-ply	T60 Monopitch Girder	8 /12	**11-00-00**	8-08-13	2 x 4 2 x 6		1-04-13 8-08-13	180.18 113.00		
	5	J10 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	62.56 41.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405037
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B- UNIT2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **31** TOTAL BFT OF ALL TRUSSES= **1208.49** BFT. TOTAL WEIGHT OF ALL TRSSES **1905.26** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	

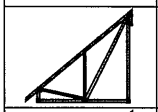
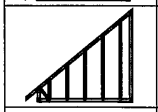
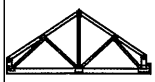
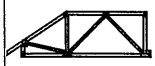
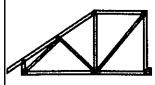
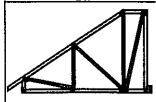
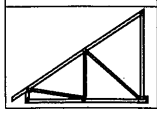
TOTAL NUMBER OF ITEMS= **5**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405038
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B - UNIT3 OR UNIT4	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

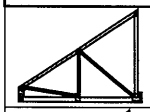
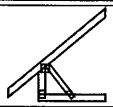
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	T61 Roof Special	6 /12	**17-04-00**	6-08-15	2 x 4	1-03-08	1-02-00 1-07-04	288.19 188.00		
	1	G61 GABLE	6 /12	**17-04-00**	6-08-15	2 x 4	1-03-08	1-02-00 1-07-04	73.81 49.50		
	4	T61A Roof Special	6 /12	**16-06-00**	6-08-15	2 x 4		1-02-00 1-07-04	273.7 175.33		
	2	T62A Half Hip	6 /12	**16-06-00**	7-00-08	2 x 4		1-02-00 7-00-08	153.7 97.67		
	1	G62A GABLE	6 /12	**16-06-00**	7-00-08	2 x 4		1-02-00 7-00-08	82.16 53.33		
	5	T63 Monopitch	10 /12	**10-11-08**	10-09-04	2 x 4	1-03-08	1-07-11 10-09-04	330.02 206.67		
	1	G64 GABLE	10 /12	**10-08-00**	10-06-06	2 x 4	1-03-08	1-07-11 10-06-06	66.47 42.00		
	1	T65 Common	8 /12	**12-11-00**	5-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	56.44 36.33		
	1	G65 GABLE	8 /12	**12-11-00**	5-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	56.62 36.33		
	1 3-ply	T66A Common Girder	8 /12	**12-06-00**	5-08-08	2 x 4 2 x 6		1-04-13 1-08-02	185.84 120.50		
	1	T67 Half Hip Girder	8 /12	**11-02-00**	4-00-07	2 x 4	1-03-08	1-04-13 4-00-07	47.99 31.00		
	1	T68 Half Hip	8 /12	**11-02-00**	5-08-07	2 x 4	1-03-08	1-04-13 5-08-07	53.22 34.83		
	1	T69 Half Hip	8 /12	**11-02-00**	7-04-07	2 x 4	1-03-08	1-04-13 7-04-07	61.78 39.17		
	1	T70 Monopitch	8 /12	**11-02-00**	8-10-02	2 x 4	1-03-08	1-04-13 8-10-02	54.95 35.00		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405038
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B - UNIT3 OR UNIT4	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	**1** **3-ply**	**T71** **Monopitch** **Girder**	8 /12	**11-02-00**	8-10-02	2 x 4 2 x 6		1-04-13 8-10-02	182.72 115.00		
	**6**	**J10** **Jack-Open**	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	75.07 50.00		

TOTAL # TRUSS= **36** TOTAL BFT OF ALL TRUSSES= **1310.66** BFT. TOTAL WEIGHT OF ALL TRSSES **2042.68** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
12	Hardware	LJS26DS	

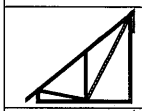
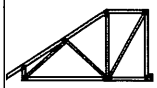
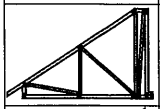
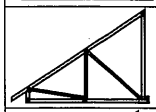
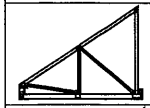
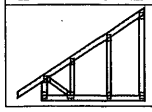
TOTAL NUMBER OF ITEMS= **12**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405039
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B2- UNIT5	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

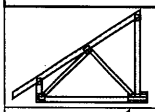
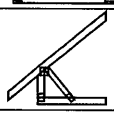
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T62B Half Hip	6 /12	**18-00-08**	7-00-08	2 x 4	1-03-08	1-02-00 7-00-08	82.46 52.67		
	1	G62B GABLE	6 /12	**18-00-08**	7-00-08	2 x 4	1-03-08	1-02-00 7-00-08	86.13 56.67		
	5	T35 Roof Special	6 /12	**18-04-00**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	392.11 249.17		
	1	G35 GABLE	6 /12	**18-04-00**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	82.95 52.83		
	4	T35B Roof Special	6 /12	**18-00-08**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	311.27 197.33		
	5	T37 Monopitch	10 /12	**12-01-08**	11-08-15	2 x 4	1-03-08	1-07-11 11-08-15	367.16 225.83		
	1	G38 GABLE	10 /12	**11-10-00**	11-06-00	2 x 4	1-03-08	1-07-11 11-06-00	72.61 45.67		
	1	T55 Hip Girder	8 /12	**11-00-00**	4-00-07	2 x 4	1-03-08	1-04-13 1-08-02	47.68 31.83		
	1	T56 Common	8 /12	**11-00-00**	5-02-08	2 x 4	1-03-08	1-04-13 1-08-02	47.14 30.83		
	1	T57 Half Hip	8 /12	**11-00-00**	6-03-02	2 x 4	1-03-08	1-04-13 6-03-02	54.84 34.83		
	1	T58 Half Hip	8 /12	**11-00-00**	7-11-02	2 x 4	1-03-08	1-04-13 7-11-02	63.95 41.83		
	1	T59 Monopitch	8 /12	**11-00-00**	8-08-13	2 x 4	1-03-08	1-04-13 8-08-13	54.21 34.33		
	1 3-ply	T60 Monopitch Girder	8 /12	**11-00-00**	8-08-13	2 x 4 2 x 6		1-04-13 8-08-13	180.18 113.00		
	1	G72 GABLE	8 /12	**5-08-00**	5-02-02	2 x 4	1-03-08	1-04-13 5-02-02	28.02 18.00		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405039
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B2- UNIT5	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	T73 Monopitch	8 /12	5-08-00	5-02-02	2 x 4	1-03-08	1-04-13 5-02-02	87.87 58.00		
	5	J10 Jack-Open	10 /12	2-10-08	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	62.56 41.67		

TOTAL # TRUSS= **35** TOTAL BFT OF ALL TRUSSES= **1284.49** BFT. TOTAL WEIGHT OF ALL TRSSES **2021.15** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	

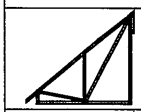
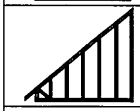
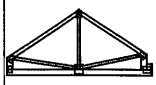
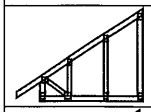
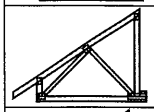
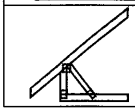
TOTAL NUMBER OF ITEMS= **5**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405040
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	1 of 1
Lot #:		Date:	03-28-2023
Elevation:	EL:B2- UNIT6	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T35 Roof Special	6 /12	**18-04-00**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	392.11 249.17		
	1	G35 GABLE	6 /12	**18-04-00**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	82.95 52.83		
	5	T35B Roof Special	6 /12	**18-00-08**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	389.08 246.67		
	1	G35B GABLE	6 /12	**18-00-08**	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	82.52 52.83		
	5	T37 Monopitch	10 /12	**12-01-08**	11-08-15	2 x 4	1-03-08	1-07-11 11-08-15	367.16 225.83		
	1	G38 GABLE	10 /12	**11-10-00**	11-06-00	2 x 4	1-03-08	1-07-11 11-06-00	72.61 45.67		
	1	T39 Hip Girder	8 /12	**11-05-00**	4-00-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49.58 31.83		
	4	T34 Common	8 /12	**11-05-00**	5-02-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	203.25 127.33		
	1 3-ply	T52A Common Girder	8 /12	**11-00-00**	5-02-08	2 x 4 2 x 6		1-04-13 1-08-02	156.28 102.00		
	1	G72 GABLE	8 /12	**5-08-00**	5-02-02	2 x 4	1-03-08	1-04-13 5-02-02	28.02 18.00		
	3	T73 Monopitch	8 /12	**5-08-00**	5-02-02	2 x 4	1-03-08	1-04-13 5-02-02	87.87 58.00		
	5	J10 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	62.56 41.67		

TOTAL # TRUSS= **35** TOTAL BFT OF ALL TRUSSES= **1251.83** BFT. TOTAL WEIGHT OF ALL TRSSES **1974** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	

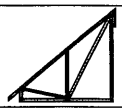
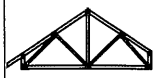
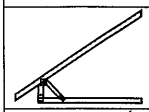
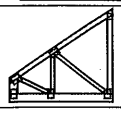
TOTAL NUMBER OF ITEMS= **5**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405041
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B - UNIT7	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

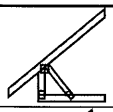
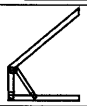
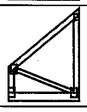
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1Z Hip Girder	8 /12	**19-10-00**	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 1-08-02	188.13 121.33		
	1	T2 Hip	8 /12	**19-10-00**	6-07-13	2 x 4	1-03-08	1-04-13 1-08-02	88.33 58.17		
	1	T3 Common	8 /12	**19-10-00**	8-01-13	2 x 4	1-03-08	1-04-13 1-08-02	86.49 55.00		
	1	T3C Hip	8 /12	**19-10-00**	7-11-13	2 x 4	1-03-08	1-04-13 1-08-02	86.28 55.00		
	1 2-ply	T45 Half Hip Girder	8 /12	**19-10-00**	5-01-05	2 x 4 2 x 6	1-03-08	1-04-13 5-01-05	198.9 128.67		
	1	T46 Half Hip	8 /12	**19-10-00**	6-09-05	2 x 4	1-03-08	1-04-13 6-09-05	90.19 58.00		
	3	T47 Common	8 /12	**12-00-00**	5-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	159 95.50		
	1	G47 GABLE	8 /12	**12-00-00**	5-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	52.42 32.83		
	5	T48 Monopitch	10 /12	**11-03-08**	11-00-10	2 x 4	1-03-08	1-07-11 11-00-10	340.63 213.33		
	1	T50 Hip Girder	8 /12	**12-04-00**	4-00-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	52.2 33.67		
	4	T51 Common	8 /12	**12-04-00**	5-06-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	217 139.33		
	1	T51A Common	8 /12	**11-11-00**	5-06-02	2 x 4	1-03-08	1-04-13 1-04-13	51.52 33.50		
	4	J1 Jack-Open	8 /12	**5-10-08**	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	76.29 49.33		
	1 2-ply	J100 Jack-Open Girder	8 /12	**5-10-08**	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.52 41.33		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405041
Location:	INNISFIL	Ref #	
Model:	BLOCK 5	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:B - UNIT7	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	6	J10 Jack-Open	10 /12	2-10-08	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	75.07 50.00		
	4	J11 Jack-Open	10 /12	3-10-08	5-01-05	2 x 4		1-10-09 5-01-05	54.61 36.00		
	1 2-ply	J110 Jack-Closed Girder	10 /12	3-10-08	5-01-05	2 x 4 2 x 6		1-10-09 5-01-05	44.37 29.33		

TOTAL # TRUSS= **41** TOTAL BFT OF ALL TRUSSES= **1230.32** BFT. TOTAL WEIGHT OF ALL TRSSES **1925.94** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

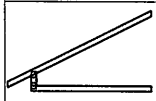
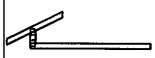
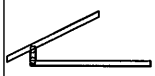
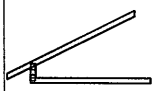
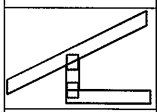
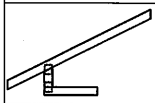
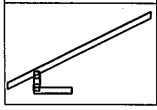
TOTAL NUMBER OF ITEMS= **6**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404952
Location:	INNISFIL	Ref #	
Model:	DG-1	Page:	1 of 1
Lot #:		Date:	08/20/2019
Elevation:	EL:B UNIT2 OR EL:B2 UNIT1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T90 Hip Girder	6 /12	**20-04-00**	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	**20-04-00**	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	**20-04-00**	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	**6-08-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	**5-09-07**	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	**1-10-08**	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

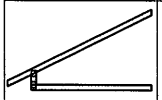
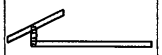
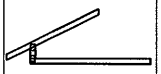
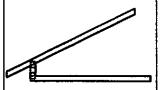
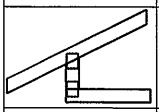
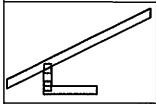
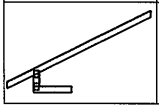
TOTAL # TRUSS= **20** TOTAL BFT OF ALL TRUSSES= **309.67** BFT. TOTAL WEIGHT OF ALL TRSSES **487.86** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404959
Location:	INNISFIL	Ref #	
Model:	DG-1	Page:	1 of 1
Lot #:		Date:	08/20/2019
Elevation:	EL:B3 UNIT4 OR EL:B3 UNIT3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T93 Half Hip Girder	6 /12	**20-04-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	84.42 52.33		
	1	T94 Half Hip	6 /12	**20-04-00**	5-06-00	2 x 4	1-03-08	1-02-00 5-06-00	85.62 53.33		
	1	G95 GABLE	6 /12	**20-04-00**	6-04-08	2 x 4	1-03-08	1-02-00 6-04-08	95.24 59.67		
	9	J50 Jack-Open	6 /12	**6-08-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	168.36 108.00		
	1	J51 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	12.48 8.00		
	1	J52 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	15.03 9.33		
	1	J53 Jack-Open	6 /12	**5-09-07**	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	17.58 11.33		
	1	J54 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	1	J55 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J56 Jack-Open	6 /12	**1-10-08**	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	12.12 8.00		

TOTAL # TRUSS= **18** TOTAL BFT OF ALL TRUSSES= **320.66** BFT. TOTAL WEIGHT OF ALL TRSSES **507.45** LBS

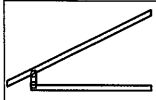
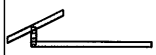
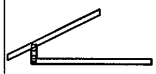
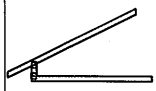
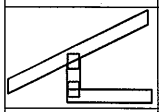
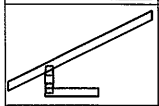
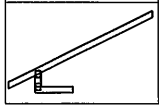
TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404952
Location:	INNISFIL	Ref #	
Model:	DG-1	Page:	1 of 1
Lot #:		Date:	08/20/2019
Elevation:	EL:B UNIT6 OR EL:B UNIT5	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T90 Hip Girder	6 /12	**20-04-00**	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	**20-04-00**	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	**20-04-00**	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	**6-08-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	**5-09-07**	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	**1-10-08**	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

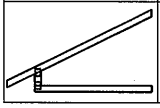
TOTAL # TRUSS= **20** TOTAL BFT OF ALL TRUSSES= **309.67** BFT. TOTAL WEIGHT OF ALL TRSSES **487.86** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200925
Project:	ALCONA SHORES	Layout ID:	405047
Location:	INNISFIL	Ref #	
Model:	DG-2	Page:	1 of 1
Lot #:		Date:	08/15/2019
Elevation:	EL:B UPGRADE - UNIT7	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T96 Hip Girder	8 /12	**10-06-00**	3-11-00	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	46.56 30.50		
	7	T97 Common	8 /12	**10-06-00**	4-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	308.32 197.17		
	1	G97 GABLE	8 /12	**10-06-00**	4-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	45.66 30.17		
	5	J57 Jack-Open	6 /12	**5-06-00**	3-11-00	2 x 4	1-03-08	1-02-00 3-11-00	79.44 50.00		

TOTAL # TRUSS= **14** TOTAL BFT OF ALL TRUSSES= **307.84** BFT. TOTAL WEIGHT OF ALL TRSSES **479.99** LBS

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T1Z	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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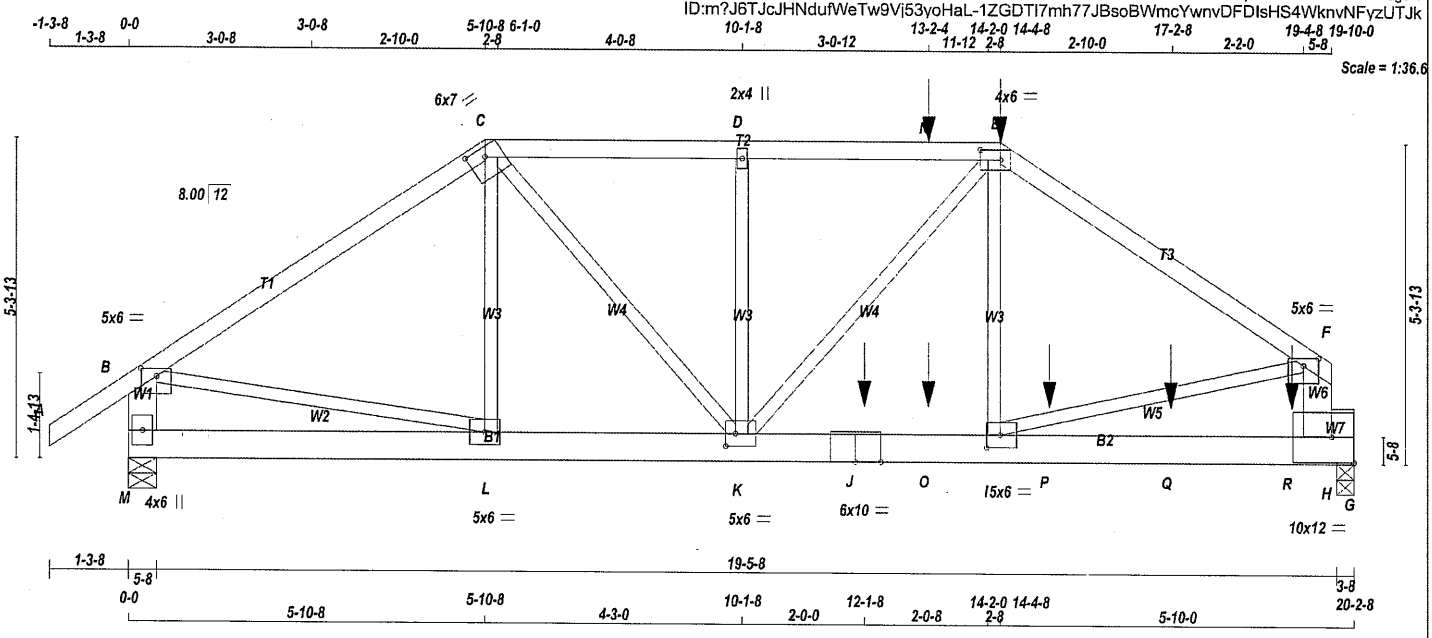

TOTAL WEIGHT = 4 X 98 = 391 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(61.0)
E- F	1	12	SIDE(61.0)
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
I- E	1	6	SIDE(28.3)
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040277

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2863	0	2863	0	0	5-8	5-8
G	3897	0	3897	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1986	1520 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0
G	2705	2061 / 0	0 / 0	0 / 0	0 / 0	644 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.10 (1)	10.00
B- C	-3221 / 0	-129.6	-129.6	0.56 (1)	4.55
C- D	-4112 / 0	-129.6	-129.6	0.27 (1)	4.43
D- N	-4112 / 0	-129.6	-129.6	0.27 (1)	4.43
N- E	-4112 / 0	-129.6	-129.6	0.27 (1)	4.43
E- F	-4750 / 0	-129.6	-129.6	0.58 (1)	3.89
M- B	-2796 / 0	0.0	0.0	0.10 (1)	7.81
H- F	-3900 / 0	0.0	0.0	0.14 (1)	7.17
M- L	0 / 0	-18.5	-18.5	0.04 (1)	10.00
L- K	0 / 2667	-18.5	-18.5	0.35 (1)	10.00
K- J	0 / 3975	-18.5	-18.5	0.76 (1)	10.00
J- O	0 / 3975	-18.5	-18.5	0.76 (1)	10.00
O- I	0 / 3975	-18.5	-18.5	0.76 (1)	10.00
I- P	0 / 0	-18.5	-18.5	0.54 (1)	10.00
P- Q	0 / 0	-18.5	-18.5	0.54 (1)	10.00
Q- R	0 / 0	-18.5	-18.5	0.54 (1)	10.00
R- H	0 / 0	-18.5	-18.5	0.54 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.22 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
L- C	-494 / 0	0.10 (1)
I- E	0 / 1307	0.16 (1)
I- F	0 / 4050	0.50 (1)
B- L	0 / 2714	0.34 (1)
K- D	-651 / 0	0.13 (1)
K- E	0 / 210	0.03 (1)
C- K	0 / 2213	0.27 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	14-4-8	-54	-54	---	FRONT	VERT	DEAD	---	C1
E	14-4-8	-339	-339	---	FRONT	VERT	SNOW	---	C1
J	12-1-8	-1362	-1362	---	FRONT	VERT	TOTAL	---	C1
N	13-2-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1
O	13-2-4	-186	-186	---	FRONT	VERT	TOTAL	---	C1
P	15-2-4	-186	-186	---	FRONT	VERT	TOTAL	---	C1
Q	17-2-4	-186	-186	---	FRONT	VERT	TOTAL	---	C1
R	19-2-4	-186	-186	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.58/1.00 (E-F:1) , BC=0.76/1.00 (I-K:1) , WB=0.50/1.00 (F-I:1) , SSI=0.65/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T1Z	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-h	MT20	6.0	7.0	1.75	3.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-l	MT20	4.0	6.0	2.00	4.00
F	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMVW-t	MT20	10.0	12.0	5.00	4.50
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	6.0	10.0		
K	BMWWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	4.0	6.0		

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.78 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040277

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T2	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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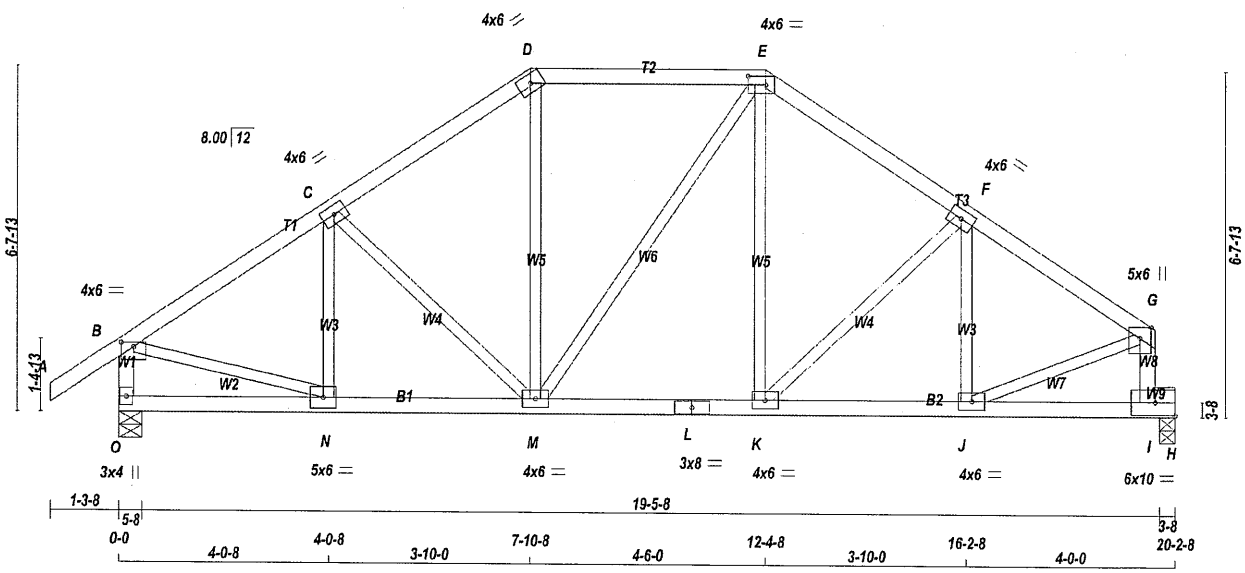

TOTAL WEIGHT = 2 X 90 = 180 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TTWW-l	MT20	4.0	6.0	2.00	4.00
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMVW-t	MT20	6.0	10.0		Edge
J	BMWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1674	0	1674	0	0	5-8	5-8
H	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-1586 / 0	-129.6	-129.6	0.38 (1)	4.80
C- D	-1401 / 0	-129.6	-129.6	0.36 (1)	5.05
D- E	-1140 / 0	-129.6	-129.6	0.48 (1)	5.24
E- F	-1385 / 0	-129.6	-129.6	0.33 (1)	5.13
F- G	-1608 / 0	-129.6	-129.6	0.33 (1)	4.83
O- B	-1640 / 0	0.0	0.0	0.17 (1)	6.47
I- G	-1598 / 0	0.0	0.0	0.17 (1)	6.53
O- N	0 / 0	-18.5	-18.5	0.06 (4)	10.00
N- M	0 / 1347	-18.5	-18.5	0.26 (1)	10.00
M- L	0 / 1130	-18.5	-18.5	0.23 (1)	10.00
L- K	0 / 1130	-18.5	-18.5	0.23 (1)	10.00
K- J	0 / 1364	-18.5	-18.5	0.42 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.74 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.31 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
M- D	0 / 282	0.06 (1)
M- E	0 / 18	0.00 (1)
K- E	0 / 254	0.06 (1)
N- C	-290 / 0	0.07 (1)
B- N	0 / 1394	0.31 (1)
C- M	-293 / 0	0.15 (1)
K- F	-331 / 0	0.16 (1)
J- F	-211 / 14	0.05 (1)
J- G	0 / 1457	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.74/1.00 (I-J:1) , WB=0.33/1.00 (G-J:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.64 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040278

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED jlandolfo 4/16/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T3	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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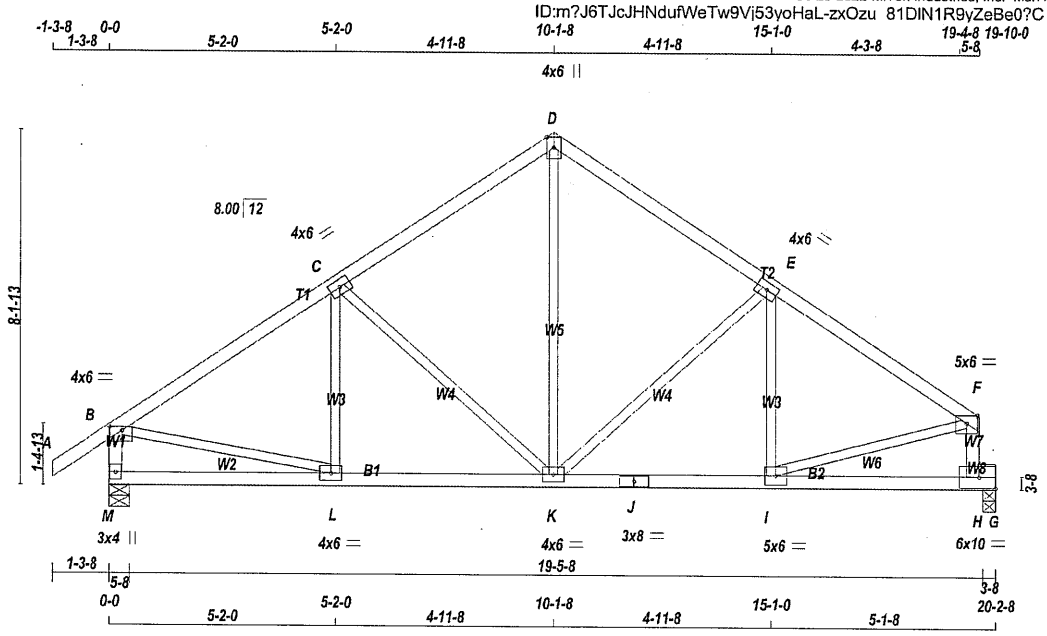

TOTAL WEIGHT = 2 X 86 = 173 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMVW-t	MT20	6.0	10.0	3.00	4.50
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1674	0	1674	0	0	5-8	5-8
G	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-1603 / 0	-129.6	-129.6	0.66 (1)	4.33
C- D	-1205 / 0	-129.6	-129.6	0.61 (1)	4.90
D- E	-1201 / 0	-129.6	-129.6	0.56 (1)	5.02
E- F	-1619 / 0	-129.6	-129.6	0.59 (1)	4.42
M- B	-1635 / 0	0.0	0.0	0.17 (1)	6.48
H- F	-1548 / 0	0.0	0.0	0.17 (1)	6.62
M- L	0 / 0	-18.5	-18.5	0.12 (4)	10.00
L- K	0 / 1370	-18.5	-18.5	0.28 (1)	10.00
K- J	0 / 1382	-18.5	-18.5	0.45 (1)	10.00
J- I	0 / 1382	-18.5	-18.5	0.45 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.74 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.31 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
L- C	-175 / 49	0.06 (1)
C- K	-548 / 0	0.45 (1)
K- D	0 / 798	0.18 (1)
K- E	-565 / 0	0.47 (1)
I- E	-121 / 59	0.04 (1)
B- L	0 / 1399	0.31 (1)
I- F	0 / 1438	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.74/1.00 (H-I:1) , WB=0.47/1.00 (E-K:1) , SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040279

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED jlandolo 4/04/2023 Building Permit #

[M]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T3C	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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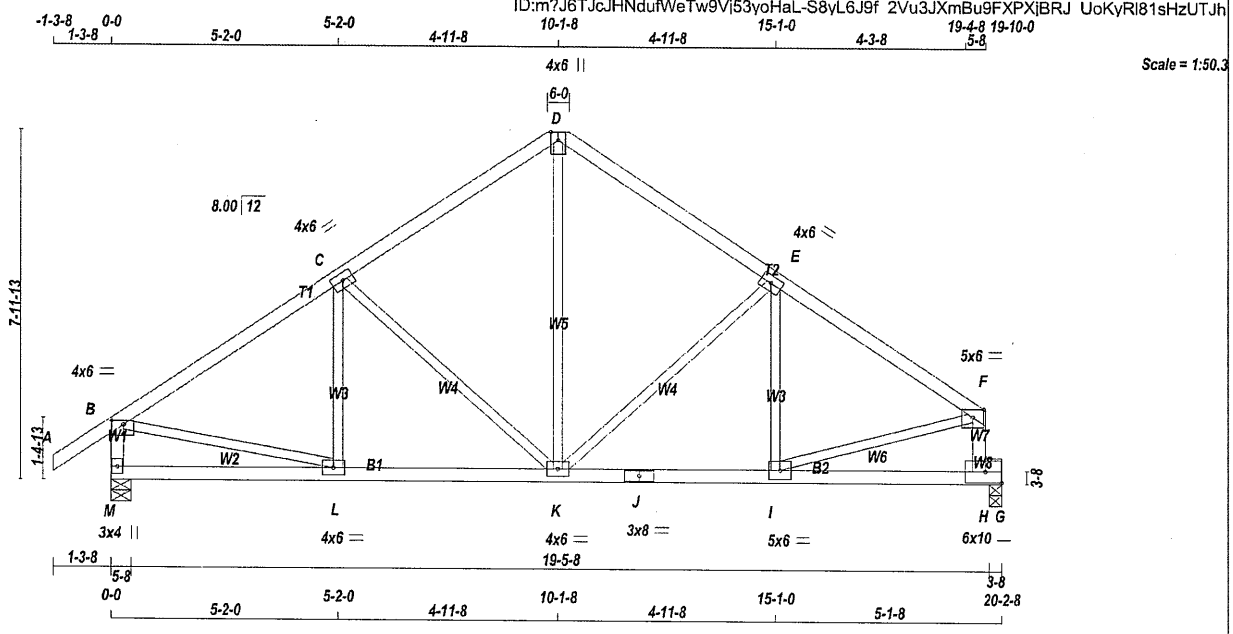

TOTAL WEIGHT = 2 X 86 = 173 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMVW-t	MT20	6.0	10.0		Edge
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1674	0	1674	0	0	5-8	5-8
G	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-1603 / 0	-129.6	-129.6	0.66 (1)	4.33
C- D	-1205 / 0	-129.6	-129.6	0.61 (1)	4.90
D- E	-1201 / 0	-129.6	-129.6	0.56 (1)	5.02
E- F	-1619 / 0	-129.6	-129.6	0.59 (1)	4.42
M- B	-1635 / 0	0.0	0.0	0.17 (1)	6.48
H- F	-1548 / 0	0.0	0.0	0.17 (1)	6.62
M- L	0 / 0	-18.5	-18.5	0.12 (4)	10.00
L- K	0 / 1370	-18.5	-18.5	0.28 (1)	10.00
K- J	0 / 1382	-18.5	-18.5	0.45 (1)	10.00
J- I	0 / 1382	-18.5	-18.5	0.45 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.74 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.31 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
L- C	-175 / 49	0.06 (1)
C- K	-548 / 0	0.45 (1)
K- D	0 / 798	0.18 (1)
K- E	-565 / 0	0.47 (1)
I- E	-121 / 59	0.04 (1)
B- L	0 / 1399	0.31 (1)
I- F	0 / 1438	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.74/1.00 (H-I:1) , WB=0.47/1.00 (E-K:1) , SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040280

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan Review

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T34	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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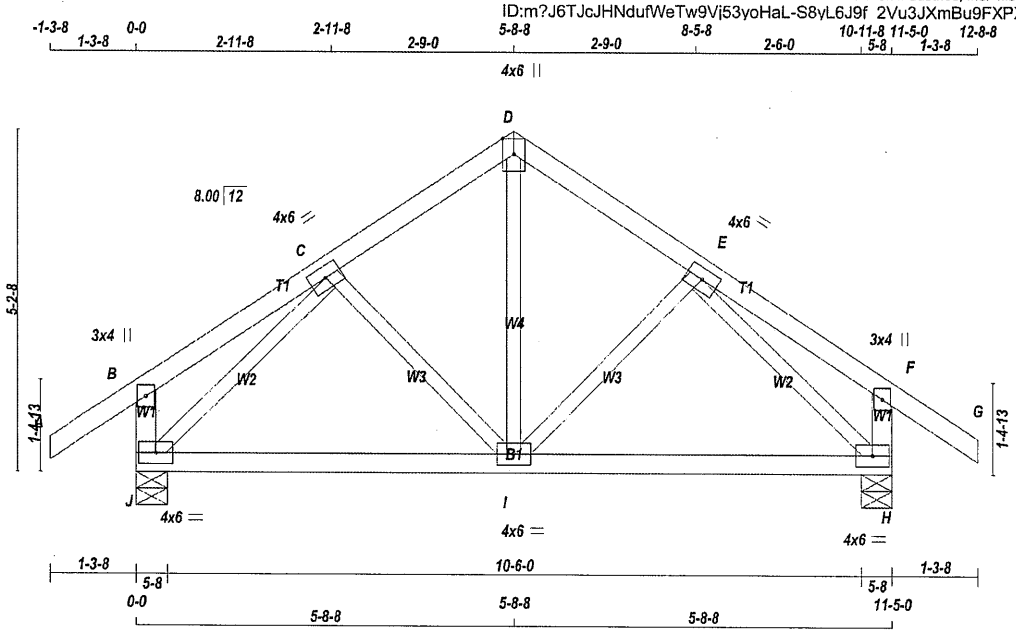

TOTAL WEIGHT = 4 X 51 = 203 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1024	0	1024	0	0	5-8	5-8
H	1024	0	1024	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	711	541 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
H	711	541 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	0 / 22	-129.6	-129.6	0.16 (1)	10.00
C- D	-632 / 0	-129.6	-129.6	0.12 (1)	6.25
D- E	-632 / 0	-129.6	-129.6	0.12 (1)	6.25
E- F	0 / 22	-129.6	-129.6	0.16 (1)	10.00
F- G	0 / 49	-129.6	-129.6	0.17 (1)	10.00
J- B	-322 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-322 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 631	-18.5	-18.5	0.21 (4)	10.00
I- H	0 / 631	-18.5	-18.5	0.21 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 385	0.09 (1)
I- E	-178 / 0	0.05 (1)
C- I	-178 / 0	0.05 (1)
J- C	-913 / 0	0.25 (1)
E- H	-913 / 0	0.25 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.21/1.00 (H-I:4) , WB=0.25/1.00 (E-H:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (E) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040281

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T35	15	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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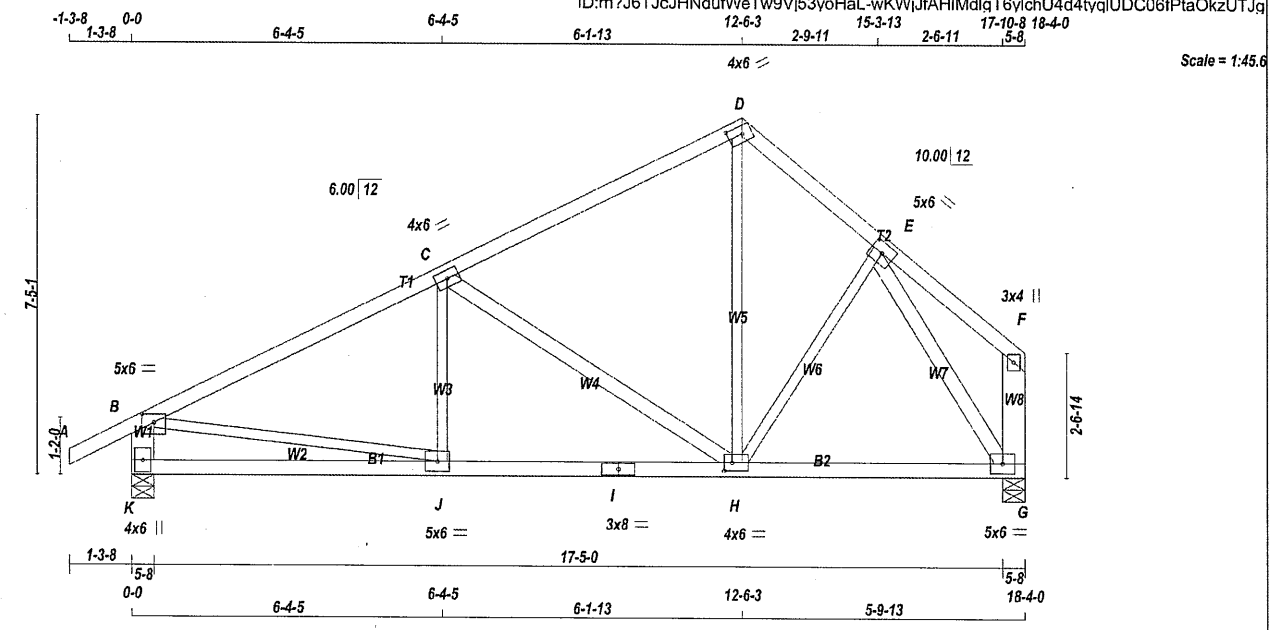

TOTAL WEIGHT = 15 X 84 = 1266 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - H	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTW-l	MT20	4.0	6.0	2.00	3.50
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	5.0	6.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	2.00
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1534	0	1534	0	0	5-8	5-8
G	1358	0	1358	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1066	804 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
G	946	701 / 0	0 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	
B- C	-1704 / 0	-129.6	-129.6	0.72 (1)	4.06	
C- D	-913 / 0	-129.6	-129.6	0.66 (1)	5.32	
D- E	-1028 / 0	-129.6	-129.6	0.14 (1)	6.04	
E- F	0 / 27	-129.6	-129.6	0.17 (1)	10.00	
K- B	-1484 / 0	0.0	0.0	0.10 (1)	7.81	
G- F	-143 / 0	0.0	0.0	0.01 (1)	7.81	
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00	
J- I	0 / 1563	-18.5	-18.5	0.34 (1)	10.00	
I- H	0 / 1563	-18.5	-18.5	0.34 (1)	10.00	
H- G	0 / 724	-18.5	-18.5	0.20 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-89 / 89	0.03 (4)
C- H	-945 / 0	0.70 (1)
H- D	0 / 580	0.13 (1)
H- E	0 / 81	0.02 (4)
B- J	0 / 1577	0.35 (1)
E- G	-1376 / 0	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.34/1.00 (H-J:1) , WB=0.70/1.00 (C-H:1) , SSI=0.36/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.50 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040282

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G35	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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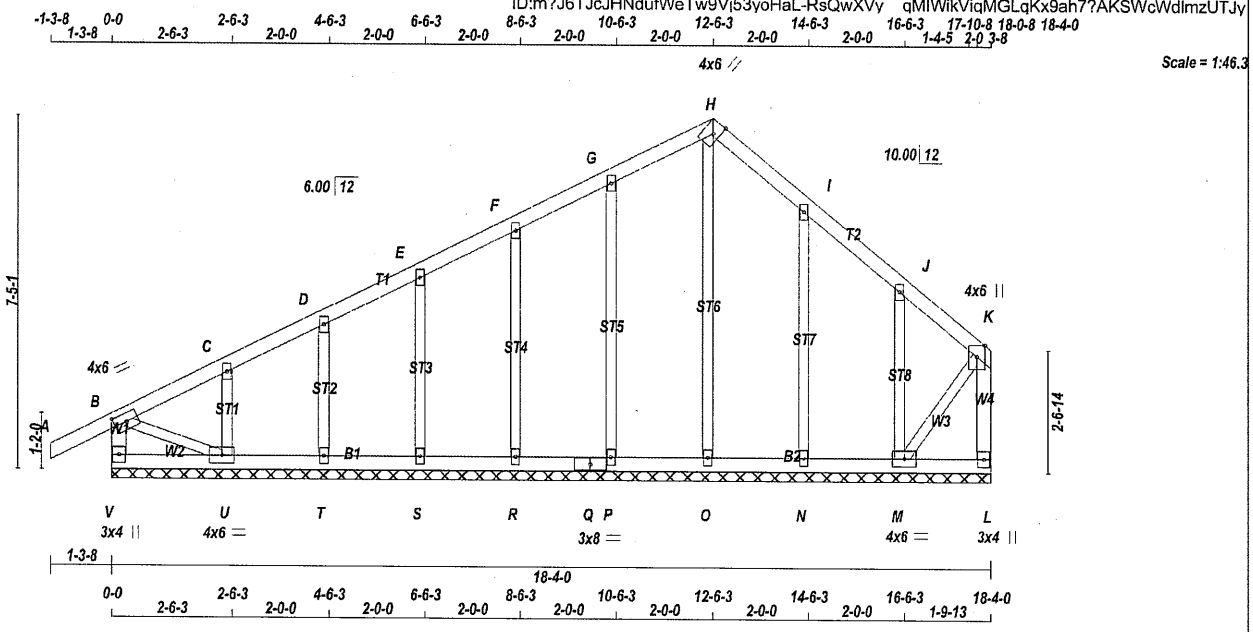

TOTAL WEIGHT = 3 X 83 = 249 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
V - Q	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C, D, E, F, G, I, J						
C	TMW+w	MT20	2.0	4.0		
H	TTW+h	MT20	4.0	6.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	6.0		
N, O, P, R, S, T						
N	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
U	BMWW1-t	MT20	4.0	6.0		
V	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-35 / 0	-129.6	-129.6	0.09 (1)	6.25
C- D	-43 / 0	-129.6	-129.6	0.09 (1)	6.25
D- E	-29 / 0	-129.6	-129.6	0.06 (1)	6.25
E- F	-27 / 0	-129.6	-129.6	0.06 (1)	6.25
F- G	-19 / 0	-129.6	-129.6	0.08 (1)	6.25
G- H	-31 / 0	-129.6	-129.6	0.08 (1)	6.25
H- I	-42 / 0	-129.6	-129.6	0.07 (1)	6.25
I- J	-30 / 0	-129.6	-129.6	0.07 (1)	6.25
J- K	-25 / 0	-129.6	-129.6	0.06 (1)	6.25
V- B	-336 / 0	0.0	0.0	0.03 (1)	7.81
L- K	-156 / 0	0.0	0.0	0.02 (1)	7.81
V- U	0 / 0	-18.5	-18.5	0.02 (4)	10.00
U- T	0 / 33	-18.5	-18.5	0.03 (4)	10.00
T- S	0 / 28	-18.5	-18.5	0.02 (4)	10.00
S- R	0 / 23	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 20	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 20	-18.5	-18.5	0.02 (4)	10.00
P- O	0 / 17	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 20	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 25	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
O- H	-192 / 0	0.19 (1)
P- G	-304 / 0	0.20 (1)
R- F	-247 / 0	0.11 (1)
S- E	-263 / 0	0.08 (1)
T- D	-238 / 0	0.05 (1)
U- C	-321 / 0	0.05 (1)
N- I	-283 / 0	0.15 (1)
M- J	-265 / 0	0.07 (1)
B- U	0 / 46	0.01 (1)
M- K	0 / 48	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.03/1.00 (T-U:4) , WB=0.20/1.00 (G-P:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (M) (INPUT = 0.90)
JSI METAL= 0.15 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040261

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED jlandolfo 4/3/2023 Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T35B	13	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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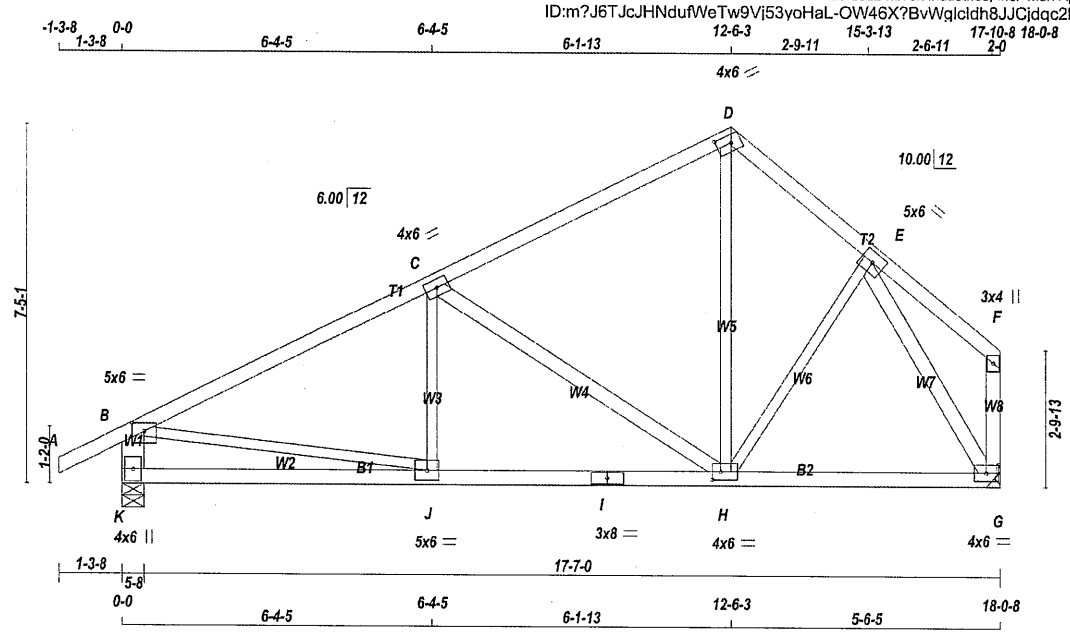

TOTAL WEIGHT = 13 X 82 = 1068 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTW-I	MT20	4.0	6.0	2.00	3.50
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	2.00
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1512	0	1512	0	0	5-8	5-8
G	1336	0	1336	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1051	793 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0
G	931	689 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	
B- C	-1667 / 0	-129.6	-129.6	0.72 (1)	4.11	
C- D	-869 / 0	-129.6	-129.6	0.66 (1)	5.43	
D- E	-977 / 0	-129.6	-129.6	0.13 (1)	6.16	
E- F	0 / 27	-129.6	-129.6	0.15 (1)	10.00	
K- B	-1462 / 0	0.0	0.0	0.09 (1)	7.81	
G- F	-124 / 0	0.0	0.0	0.02 (1)	7.81	
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00	
J- I	0 / 1530	-18.5	-18.5	0.33 (1)	10.00	
I- H	0 / 1530	-18.5	-18.5	0.33 (1)	10.00	
H- G	0 / 650	-18.5	-18.5	0.18 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-83 / 91	0.03 (4)
C- H	-951 / 0	0.70 (1)
H- D	0 / 526	0.12 (1)
H- E	0 / 146	0.03 (1)
B- J	0 / 1543	0.35 (1)
E- G	-1339 / 0	0.44 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.33/1.00 (H-J:1) , WB=0.70/1.00 (C-H:1) , SSI=0.36/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.49 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040283

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G35B	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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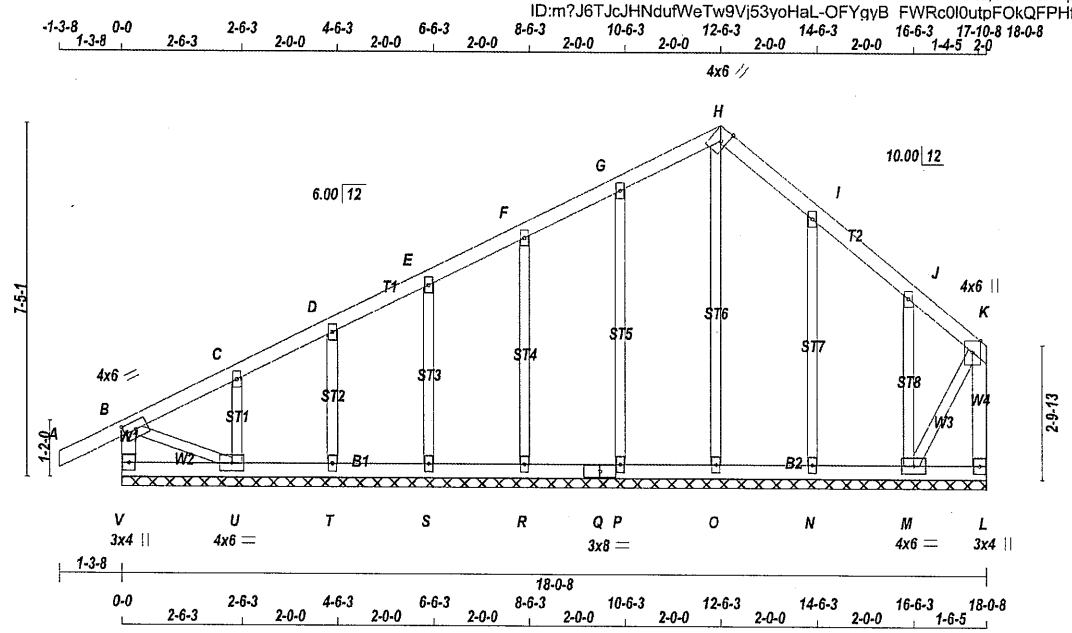

TOTAL WEIGHT = 83 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
V - Q	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C, D, E, F, G, I, J						
C	TMW+w	MT20	2.0	4.0		
H	TTW+h	MT20	4.0	6.0	Edge	1.50
K	TMVW+p	MT20	4.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	6.0		
N, O, P, R, S, T						
N	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
U	BMWW1-t	MT20	4.0	6.0		
V	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040262

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	
B- C	-32 / 0	-129.6	-129.6	0.09 (1)	6.25	
C- D	-40 / 0	-129.6	-129.6	0.09 (1)	6.25	
D- E	-26 / 0	-129.6	-129.6	0.06 (1)	6.25	
E- F	-24 / 0	-129.6	-129.6	0.06 (1)	6.25	
F- G	-15 / 0	-129.6	-129.6	0.08 (1)	6.25	
G- H	-27 / 0	-129.6	-129.6	0.08 (1)	6.25	
H- I	-39 / 0	-129.6	-129.6	0.07 (1)	6.25	
I- J	-24 / 0	-129.6	-129.6	0.07 (1)	6.25	
J- K	-20 / 0	-129.6	-129.6	0.05 (1)	6.25	
V- B	-333 / 0	0.0	0.0	0.03 (1)	7.81	
L- K	-140 / 0	0.0	0.0	0.02 (1)	7.81	
V- U	0 / 0	-18.5	-18.5	0.02 (4)	10.00	
U- T	0 / 30	-18.5	-18.5	0.03 (4)	10.00	
T- S	0 / 25	-18.5	-18.5	0.02 (4)	10.00	
S- R	0 / 20	-18.5	-18.5	0.02 (4)	10.00	
R- Q	0 / 17	-18.5	-18.5	0.02 (4)	10.00	
Q- P	0 / 17	-18.5	-18.5	0.02 (4)	10.00	
P- O	0 / 14	-18.5	-18.5	0.01 (4)	10.00	
O- N	0 / 17	-18.5	-18.5	0.02 (4)	10.00	
N- M	0 / 22	-18.5	-18.5	0.02 (4)	10.00	
M- L	0 / 0	-18.5	-18.5	0.01 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- H	-196 / 0	0.19 (1)
P- G	-304 / 0	0.20 (1)
R- F	-247 / 0	0.11 (1)
S- E	-263 / 0	0.08 (1)
T- D	-238 / 0	0.05 (1)
U- C	-321 / 0	0.05 (1)
N- I	-289 / 0	0.15 (1)
M- J	-242 / 0	0.06 (1)
B- U	0 / 43	0.01 (1)
M- K	0 / 50	0.01 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.03/1.00 (T-U:4) , WB=0.20/1.00 (G-P:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (M) (INPUT = 0.90)
JSI METAL= 0.15 (I) (INPUT = 1.00)

TOWN OF INNISFIL
Block 5
REVIEWED
Penfold
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T37	15	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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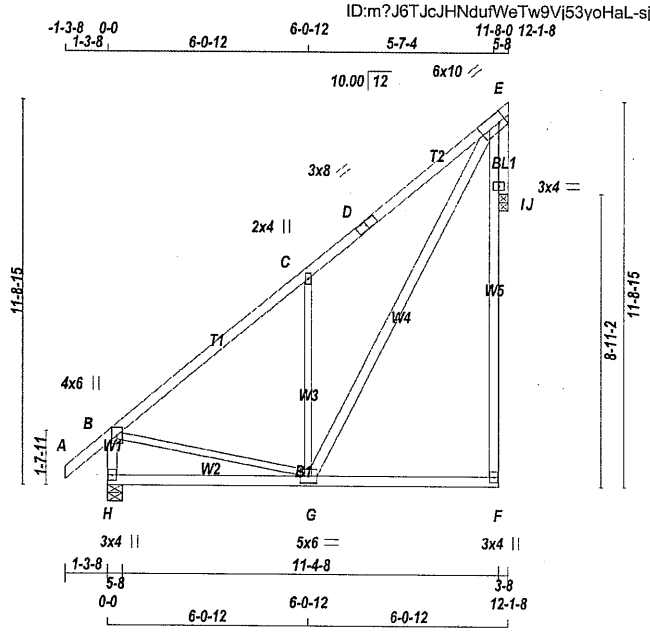

TOTAL WEIGHT = 15 X 73 = 1101 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVWK1-t	MT20	6.0	10.0	Edge	2.50
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(E)	866	0	866	0	0	3-8	3-8 (** SEE "BEARING NOTE" **)
H	1067	0	1067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	603	447 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
H	741	564 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	-622 / 0	-129.6	-129.6	0.60 (1)	6.25
C- D	-708 / 0	-129.6	-129.6	0.60 (1)	6.05
D- E	-708 / 0	-129.6	-129.6	0.60 (1)	6.05
F- I	0 / 44	0.0	0.0	0.14 (1)	10.00
I- E	0 / 44	0.0	0.0	0.14 (1)	10.00
H- B	-1021 / 0	0.0	0.0	0.11 (1)	7.75
H- G	0 / 0	-18.5	-18.5	0.18 (4)	10.00
G- F	0 / 11	-18.5	-18.5	0.18 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-933 / 0	0.69 (1)
G- E	0 / 1070	0.17 (1)
B- G	0 / 535	0.12 (1)
E- J	-867 / 0	0.13 (1)
I- J	0 / 53	0.00 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.18/1.00 (F-G:4) , WB=0.69/1.00 (C-G:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.50 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040284

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G38	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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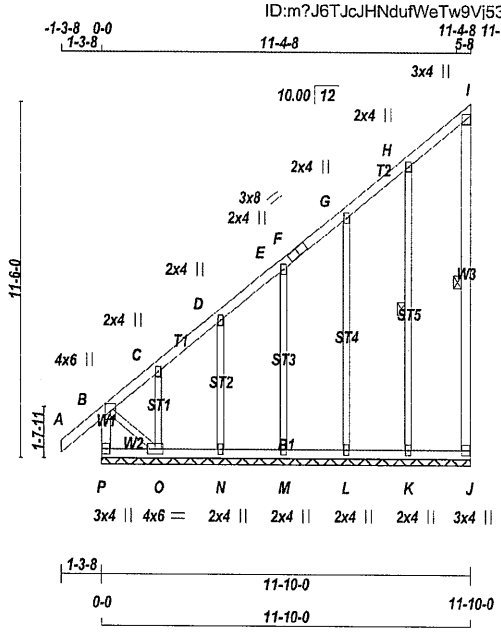

TOTAL WEIGHT = 3 X 73 = 218 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, G, H						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
I	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N						
K	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-386 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	-67 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	-10 / 0	-129.6	-129.6	0.07 (1)	6.25
D- E	-15 / 0	-129.6	-129.6	0.07 (1)	6.25
E- F	-8 / 0	-129.6	-129.6	0.06 (1)	10.00
F- G	-8 / 0	-129.6	-129.6	0.06 (1)	10.00
G- H	-1 / 0	-129.6	-129.6	0.07 (1)	10.00
H- I	-15 / 0	-129.6	-129.6	0.07 (1)	6.25
J- I	-112 / 0	0.0	0.0	0.08 (1)	6.25
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 16	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 10	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 6	-18.5	-18.5	0.01 (4)	10.00
L- K	0 / 3	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- H	-291 / 0	0.19 (1)
L- G	-253 / 0	0.33 (1)
M- E	-253 / 0	0.17 (1)
N- D	-273 / 0	0.09 (1)
O- C	-161 / 0	0.03 (1)
B- O	0 / 26	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (J-K:4) , WB=0.33/1.00 (G-L:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.15 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040263

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T39	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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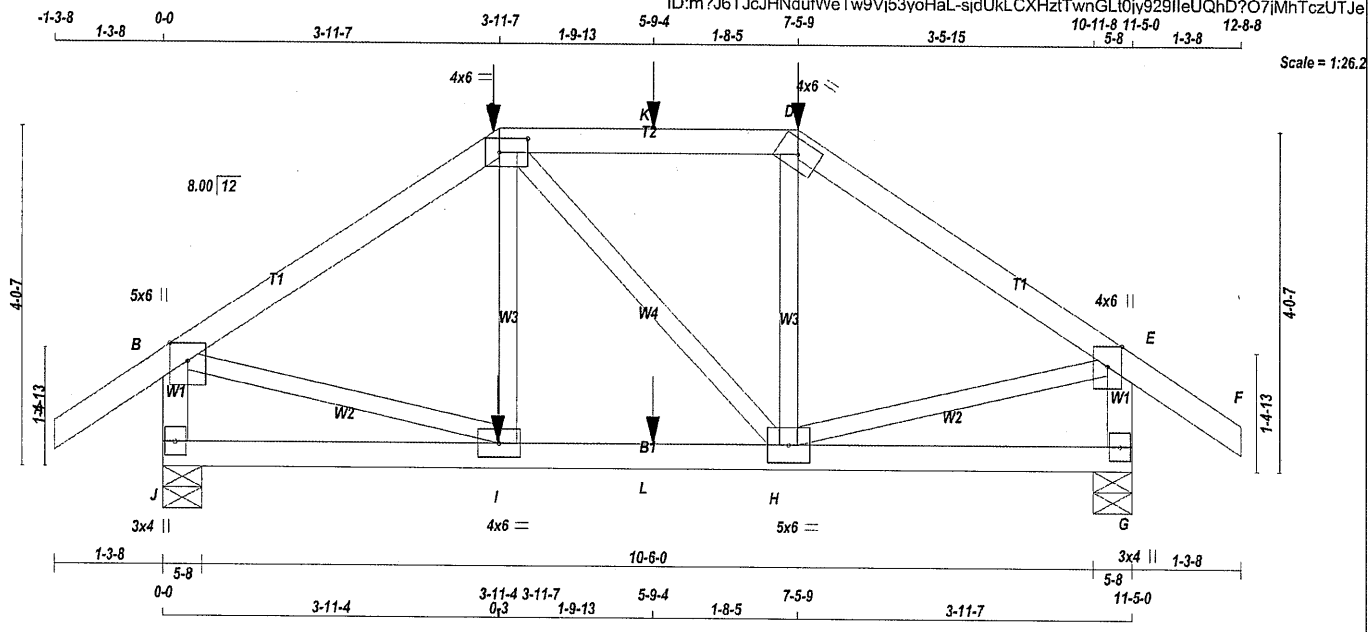

TOTAL WEIGHT = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1264	0	1264	0	0	5-8	5-8
G	1243	0	1243	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	876	675 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0
G	861	665 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00
B- C	-1024 / 0	-129.6	-129.6	0.41 (1)	5.57
C- K	-826 / 0	-129.6	-129.6	0.39 (1)	6.07
K- D	-826 / 0	-129.6	-129.6	0.39 (1)	6.07
D- E	-1001 / 0	-129.6	-129.6	0.41 (1)	5.63
E- F	0 / 49	-129.6	-129.6	0.19 (1)	10.00
J- B	-1231 / 0	0.0	0.0	0.14 (1)	7.19
G- E	-1209 / 0	0.0	0.0	0.14 (1)	7.25
J- I	0 / 0	-18.5	-18.5	0.07 (4)	10.00
I- L	0 / 847	-18.5	-18.5	0.18 (1)	10.00
L- H	0 / 847	-18.5	-18.5	0.18 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-143 / 41	0.04 (1)
C- H	-31 / 0	0.01 (1)
H- D	-117 / 45	0.03 (1)
B- I	0 / 881	0.22 (1)
H- E	0 / 859	0.21 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-7	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	3-11-7	-45	-45	---	BACK	VERT	TOTAL	---	C1
C	3-11-7	-95	-95	---	FRONT	VERT	SNOW	---	C1
D	7-5-9	-15	-15	---	FRONT	VERT	DEAD	---	C1
D	7-5-9	-95	-95	---	FRONT	VERT	SNOW	---	C1
I	3-11-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
K	5-9-4	-39	-39	---	BACK	VERT	TOTAL	---	C1
L	5-9-4	-6	-6	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.18/1.00 (H-I:1) , WB=0.22/1.00 (B-I:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.57 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040285

TOWN OF INNISFIL
Block 5
REVIEWED
jlendvoi
4/3/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T45	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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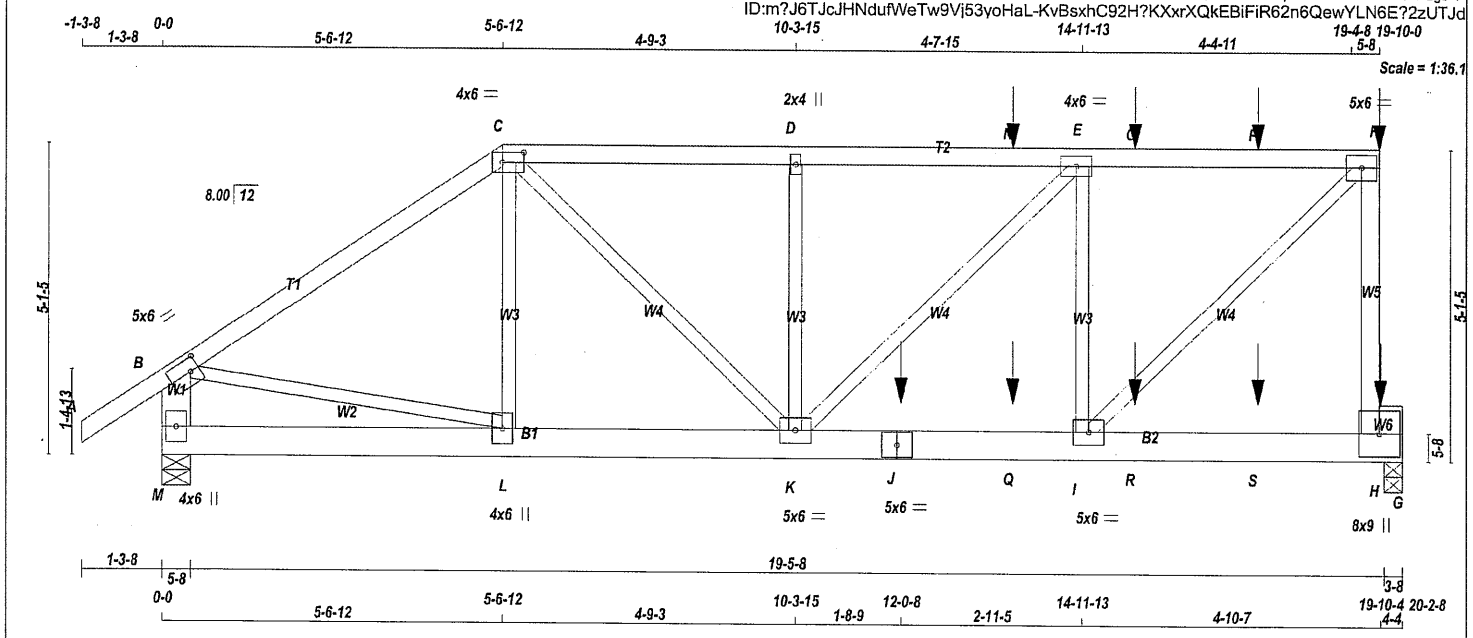

TOTAL WEIGHT = 4 X 99 = 398 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- G	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040286

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2053	0	2053	0	0	5-8	5-8
G	2398	0	2398	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1427	1078 / 0	0 / 0	0 / 0	0 / 0	349 / 0	0 / 0
G	1670	1244 / 0	0 / 0	0 / 0	0 / 0	426 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.10 (1)	10.00	
B- C	-2081 / 0	-129.6	-129.6	0.44 (1)	5.55	
C- D	-2510 / 0	-129.6	-129.6	0.23 (1)	5.44	
D- N	-2510 / 0	-129.6	-129.6	0.35 (1)	5.27	
N- E	-2510 / 0	-129.6	-129.6	0.35 (1)	5.27	
E- O	-2120 / 0	-129.6	-129.6	0.34 (1)	5.63	
O- P	-2120 / 0	-129.6	-129.6	0.34 (1)	5.63	
P- F	-2120 / 0	-129.6	-129.6	0.34 (1)	5.63	
H- F	-2581 / 0	0.0	0.0	0.54 (1)	7.06	
M- B	-2000 / 0	0.0	0.0	0.07 (1)	7.81	
M- L	0 / 0	-18.5	-18.5	0.03 (4)	10.00	
L- K	0 / 1724	-18.5	-18.5	0.17 (1)	10.00	
K- J	0 / 2120	-18.5	-18.5	0.34 (1)	10.00	
J- Q	0 / 2120	-18.5	-18.5	0.34 (1)	10.00	
Q- I	0 / 2120	-18.5	-18.5	0.34 (1)	10.00	
I- R	0 / 0	-18.5	-18.5	0.33 (1)	10.00	
R- S	0 / 0	-18.5	-18.5	0.33 (1)	10.00	
S- H	0 / 0	-18.5	-18.5	0.33 (1)	10.00	
H- G	0 / 0	-18.5	-18.5	0.14 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
L- C	-243 / 37	0.05 (1)
B- L	0 / 1756	0.22 (1)
I- F	0 / 2963	0.37 (1)
C- K	0 / 1106	0.14 (1)
I- E	-1371 / 0	0.26 (1)
K- D	-656 / 0	0.12 (1)
K- E	0 / 556	0.07 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	19-10-0	-128	-128	---	FRONT	VERT	TOTAL	---	C1
H	19-10-4	-19	-19	---	FRONT	VERT	TOTAL	---	C1
J	12-0-8	-485	-485	---	FRONT	VERT	TOTAL	---	C1
N	13-10-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
O	15-10-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
P	17-10-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
Q	13-10-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
R	15-10-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
S	17-10-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.54/1.00 (F-H:1) , BC=0.34/1.00 (I-K:1) , WB=0.37/1.00 (F-I:1) , SSI=0.40/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402645	T45	2	2	TRUSS DESC.	

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit File

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW-l	MT20	4.0	6.0	2.00	4.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW+p	MT20	8.0	9.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	4.0	6.0		
M	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040286

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T46	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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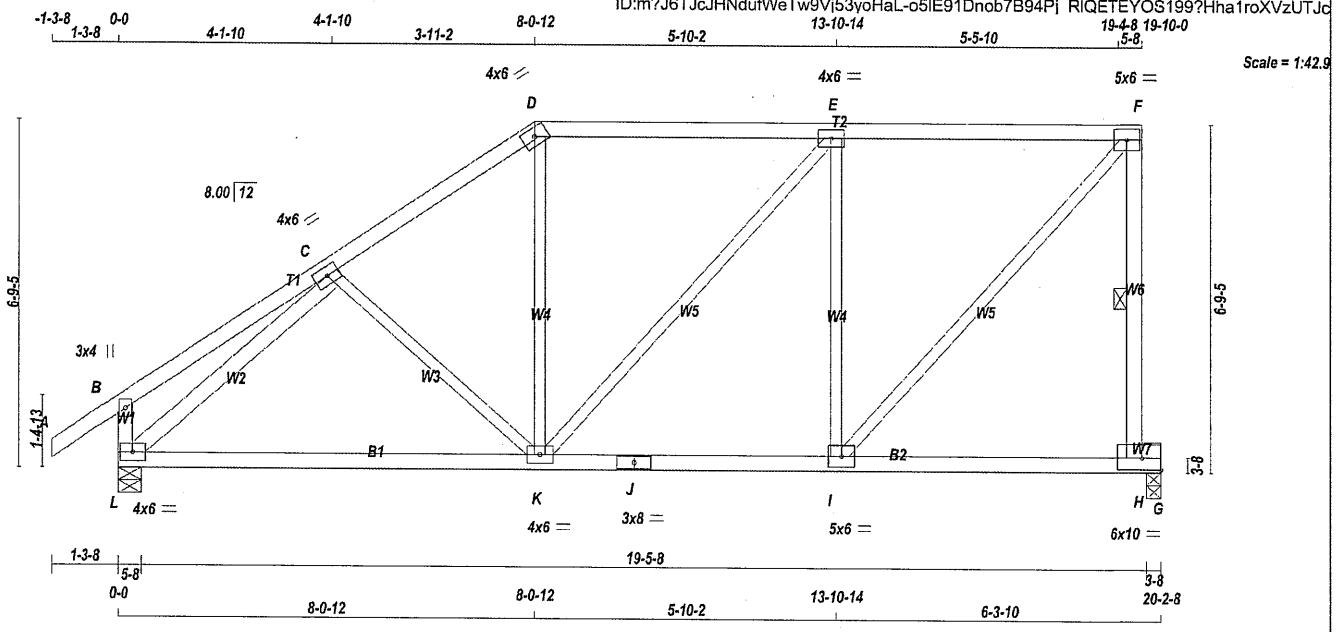

TOTAL WEIGHT = 2 X 92 = 184 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW-t	MT20	6.0	10.0	2.75	4.50
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1674	0	1674	0	0	5-8	5-8
G	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	0 / 34	-129.6	-129.6	0.34 (1)	10.00
C- D	-1391 / 0	-129.6	-129.6	0.38 (1)	5.04
D- E	-1132 / 0	-129.6	-129.6	0.82 (1)	4.37
E- F	-1113 / 0	-129.6	-129.6	0.82 (1)	4.41
H- F	-1510 / 0	0.0	0.0	0.32 (1)	5.35
L- B	-378 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 1315	-18.5	-18.5	0.37 (1)	10.00
K- J	0 / 1113	-18.5	-18.5	0.43 (1)	10.00
J- I	0 / 1113	-18.5	-18.5	0.43 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.74 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.31 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-253 / 0	0.14 (1)
K- D	0 / 266	0.06 (1)
L- C	-1800 / 0	0.64 (1)
I- F	0 / 1650	0.37 (1)
K- E	0 / 36	0.01 (4)
I- E	-967 / 0	0.74 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 741 (0.33")

CSI: TC=0.82/1.00 (E-F:1) , BC=0.74/1.00 (H-I:1) , WB=0.74/1.00 (E-I:1) , SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.42 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040287

TOWN OF INNISFIL Block 5 REVIEWED jlandolo 4/3/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T47	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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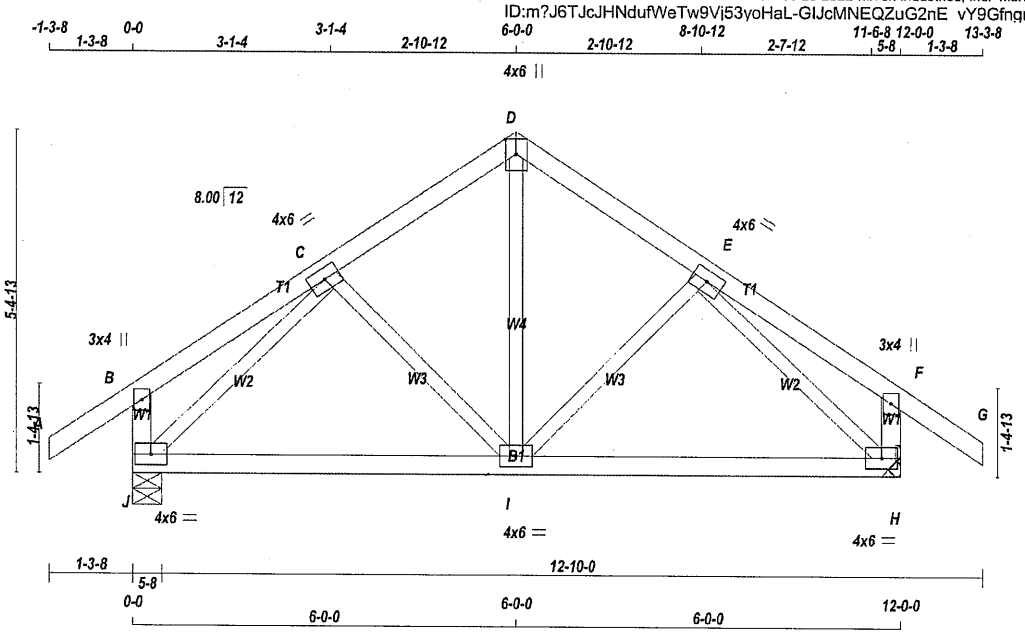

TOTAL WEIGHT = 6 X 53 = 318 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1067	0	1067	0	0	5-8	5-8
H	1067	0	1067	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	563 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
H	741	563 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
B- C	0 / 24	-129.6	-129.6	0.18 (1)	10.00	
C- D	-672 / 0	-129.6	-129.6	0.14 (1)	6.25	
D- E	-672 / 0	-129.6	-129.6	0.14 (1)	6.25	
E- F	0 / 24	-129.6	-129.6	0.18 (1)	10.00	
F- G	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
J- B	-329 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-329 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 676	-18.5	-18.5	0.23 (4)	10.00	
I- H	0 / 676	-18.5	-18.5	0.23 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 414	0.09 (1)
I- E	-195 / 0	0.06 (1)
C- I	-195 / 0	0.06 (1)
J- C	-969 / 0	0.29 (1)
E- H	-969 / 0	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.23/1.00 (I-J:4) , WB=0.29/1.00 (C-J:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (E) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040288

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED gandolfo 4/10/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G47	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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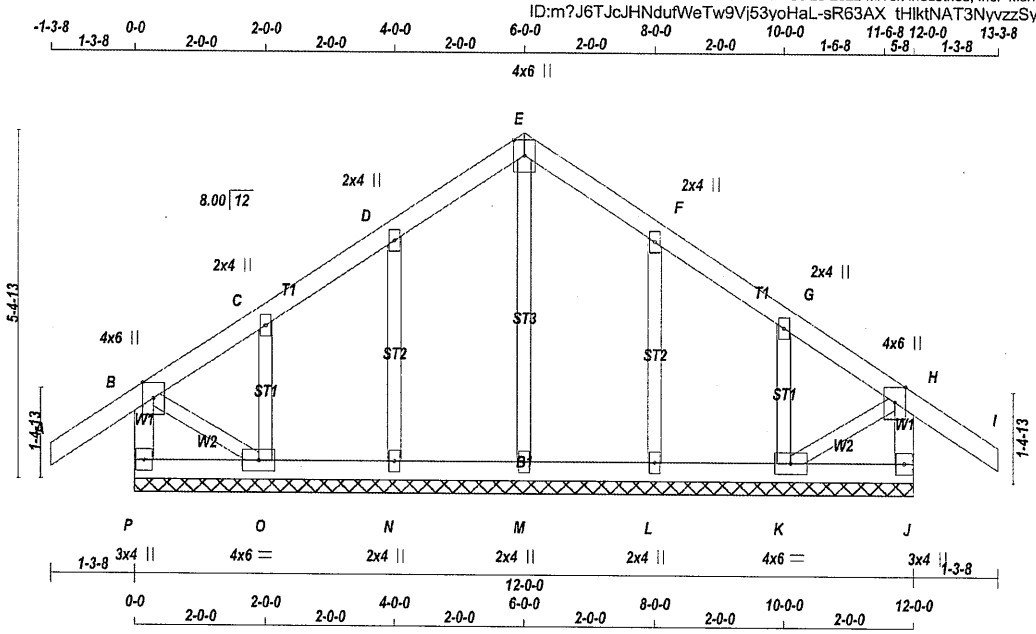

TOTAL WEIGHT = 2 X 52 = 105 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-58 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	-10 / 0	-129.6	-129.6	0.08 (1)	10.00
D- E	-29 / 0	-129.6	-129.6	0.08 (1)	6.25
E- F	-29 / 0	-129.6	-129.6	0.08 (1)	6.25
F- G	-10 / 0	-129.6	-129.6	0.08 (1)	10.00
G- H	-58 / 0	-129.6	-129.6	0.17 (1)	6.25
H- I	0 / 49	-129.6	-129.6	0.17 (1)	10.00
P- B	-385 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-385 / 0	0.0	0.0	0.04 (1)	7.81
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 18	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 11	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 11	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 18	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-197 / 0	0.08 (1)
N- D	-310 / 0	0.08 (1)
O- C	-176 / 0	0.03 (1)
L- F	-310 / 0	0.08 (1)
K- G	-176 / 0	0.03 (1)
B- O	0 / 27	0.01 (1)
K- H	0 / 27	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (H-I:1) , BC=0.02/1.00 (N-O:4) , WB=0.08/1.00 (E-M:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (F) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040264

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED jlandolo 4/16/2023 Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T48	9	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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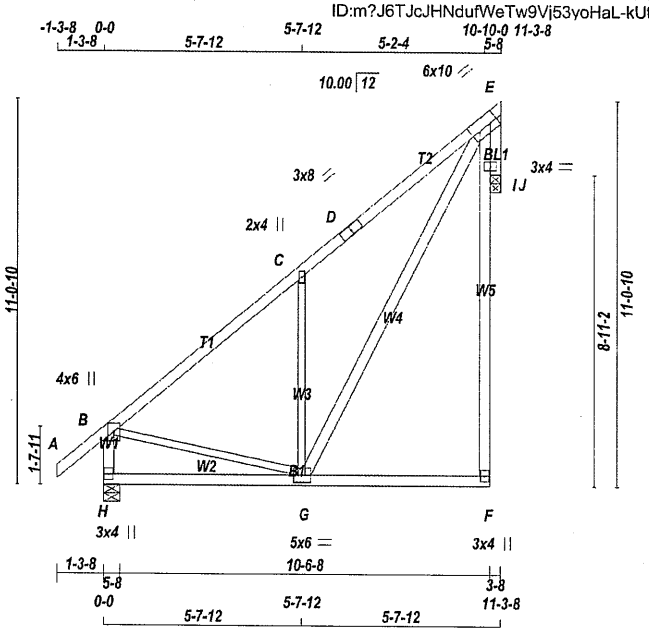

TOTAL WEIGHT = 9 X 68 = 613 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVWK1-t	MT20	6.0	10.0	Edge	2.50
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(E)	804	0	804	0	0	3-8	3-8
							(** SEE "BEARING NOTE" **)
H	1005	0	1005	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	560	415 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
H	698	532 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	-569 / 0	-129.6	-129.6	0.51 (1)	6.25
C- D	-646 / 0	-129.6	-129.6	0.51 (1)	6.25
D- E	-646 / 0	-129.6	-129.6	0.51 (1)	6.25
F- I	0 / 40	0.0	0.0	0.14 (1)	10.00
I- E	0 / 40	0.0	0.0	0.14 (1)	10.00
H- B	-962 / 0	0.0	0.0	0.10 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00
G- F	0 / 11	-18.5	-18.5	0.16 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-865 / 0	0.56 (1)
G- E	0 / 984	0.16 (1)
B- G	0 / 491	0.11 (1)
E- J	-807 / 0	0.09 (1)
I- J	0 / 69	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.51/1.00 (B-C:1) , BC=0.16/1.00 (F-G:4) , WB=0.56/1.00 (C-G:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (G) (INPUT = 0.90)
JSI METAL= 0.46 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040289

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
jlandolo
4/04/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G49	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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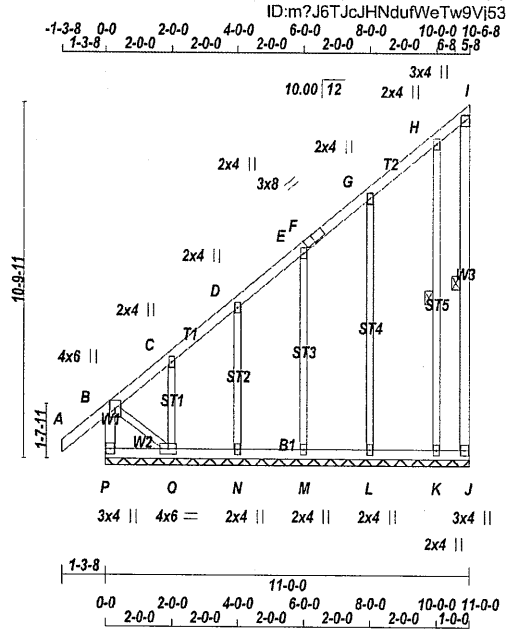

TOTAL WEIGHT = 70 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, G, H						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0	Edge 3.25	
I	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N						
K	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-388 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	-61 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	-10 / 0	-129.6	-129.6	0.07 (1)	6.25
D- E	-14 / 0	-129.6	-129.6	0.07 (1)	6.25
E- F	-5 / 0	-129.6	-129.6	0.06 (1)	10.00
F- G	-5 / 0	-129.6	-129.6	0.06 (1)	10.00
G- H	-10 / 0	-129.6	-129.6	0.06 (1)	6.25
H- I	-14 / 0	-129.6	-129.6	0.03 (1)	6.25
J- I	-50 / 0	0.0	0.0	0.03 (1)	6.25
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 15	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 9	-18.5	-18.5	0.01 (4)	10.00
M- L	0 / 5	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 2	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-181 / 0	0.03 (1)
M- D	-271 / 0	0.10 (1)
M- E	-251 / 0	0.18 (1)
L- G	-271 / 0	0.37 (1)
K- H	-209 / 0	0.14 (1)
B- O	0 / 24	0.01 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (N-O:4) , WB=0.37/1.00 (G-L:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040265

TOWN OF INNISFIL Block 5 REVIEWED Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T50	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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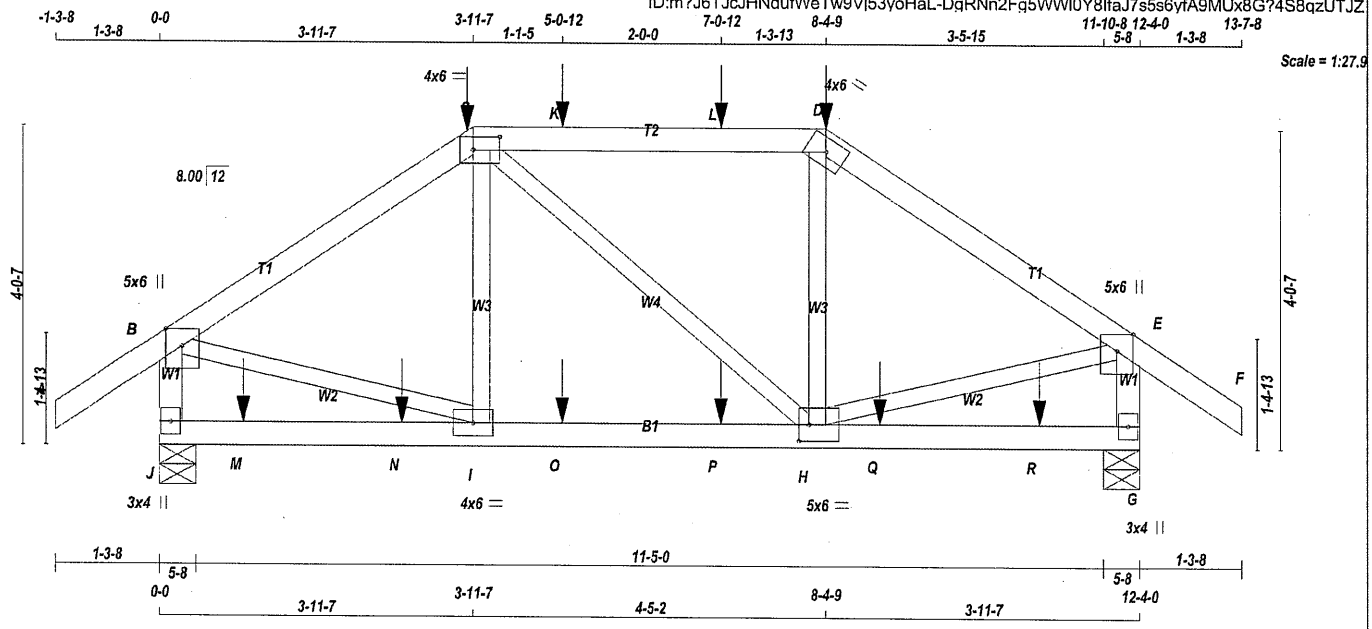

TOTAL WEIGHT = 2 X 52 = 104 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040290

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1335	0	1335	0	0	5-8	5-8
G	1332	0	1332	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	927	705 / 0	0 / 0	0 / 0	0 / 0	222 / 0	0 / 0
G	925	704 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00
B- C	-1109 / 0	-129.6	-129.6	0.42 (1)	5.40
C- K	-916 / 0	-129.6	-129.6	0.61 (1)	5.35
K- L	-916 / 0	-129.6	-129.6	0.61 (1)	5.35
L- D	-916 / 0	-129.6	-129.6	0.61 (1)	5.35
D- E	-1111 / 0	-129.6	-129.6	0.42 (1)	5.40
E- F	0 / 49	-129.6	-129.6	0.19 (1)	10.00
J- B	-1297 / 0	0.0	0.0	0.15 (1)	7.05
G- E	-1294 / 0	0.0	0.0	0.15 (1)	7.06
J- M	0 / 0	-18.5	-18.5	0.09 (4)	10.00
M- N	0 / 0	-18.5	-18.5	0.09 (4)	10.00
N- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00
I- O	0 / 917	-18.5	-18.5	0.21 (1)	10.00
O- P	0 / 917	-18.5	-18.5	0.21 (1)	10.00
P- H	0 / 917	-18.5	-18.5	0.21 (1)	10.00
H- Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00
Q- R	0 / 0	-18.5	-18.5	0.10 (4)	10.00
R- G	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-146 / 52	0.04 (1)
C- H	-1 / 2	0.00 (4)
H- D	-140 / 54	0.04 (1)
B- I	0 / 953	0.24 (1)
H- E	0 / 954	0.24 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-7	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	3-11-7	-95	-95	---	FRONT	VERT	SNOW	---	C1
D	8-4-9	-15	-15	---	FRONT	VERT	DEAD	---	C1
D	8-4-9	-95	-95	---	FRONT	VERT	SNOW	---	C1
K	5-0-12	-39	-39	---	FRONT	VERT	TOTAL	---	C1
L	7-0-12	-39	-39	---	FRONT	VERT	TOTAL	---	C1
M	1-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
N	3-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
O	5-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
P	7-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
Q	9-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
R	11-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.61/1.00 (C-D:1) , BC=0.21/1.00 (H-I:1) , WB=0.24/1.00 (E-H:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (H) (INPUT = 0.90)
JSI METAL= 0.50 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T50	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOWN OF INNISFIL
Block 5
REVIEWED
jpendola
4/10/2023
Building Permit File

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040290

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T51	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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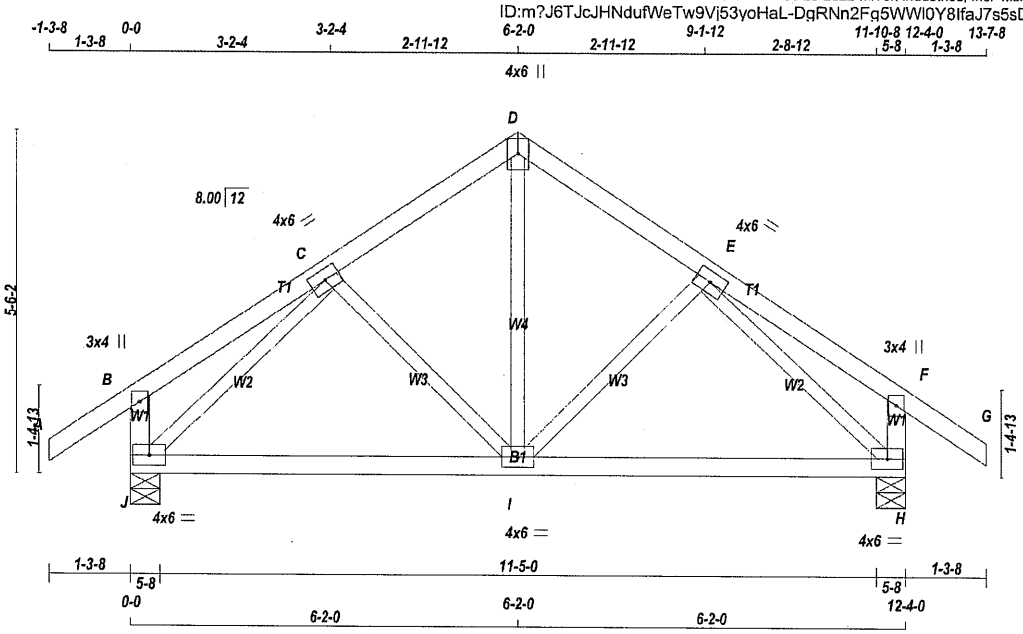

TOTAL WEIGHT = 8 X 54 = 434 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1092	0	1092	0	0	5-8	5-8
H	1092	0	1092	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	758	576 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	758	576 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	0 / 25	-129.6	-129.6	0.19 (1)	10.00
C- D	-695 / 0	-129.6	-129.6	0.15 (1)	6.25
D- E	-695 / 0	-129.6	-129.6	0.15 (1)	6.25
E- F	0 / 25	-129.6	-129.6	0.19 (1)	10.00
F- G	0 / 49	-129.6	-129.6	0.17 (1)	10.00
J- B	-333 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-333 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 702	-18.5	-18.5	0.24 (4)	10.00
I- H	0 / 702	-18.5	-18.5	0.24 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
I- D	0 / 430	0.10 (1)
I- E	-205 / 0	0.07 (1)
C- I	-205 / 0	0.07 (1)
J- C	-1001 / 0	0.31 (1)
E- H	-1001 / 0	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.24/1.00 (I-J:4) , WB=0.31/1.00 (C-J:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (E) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040291

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T51A	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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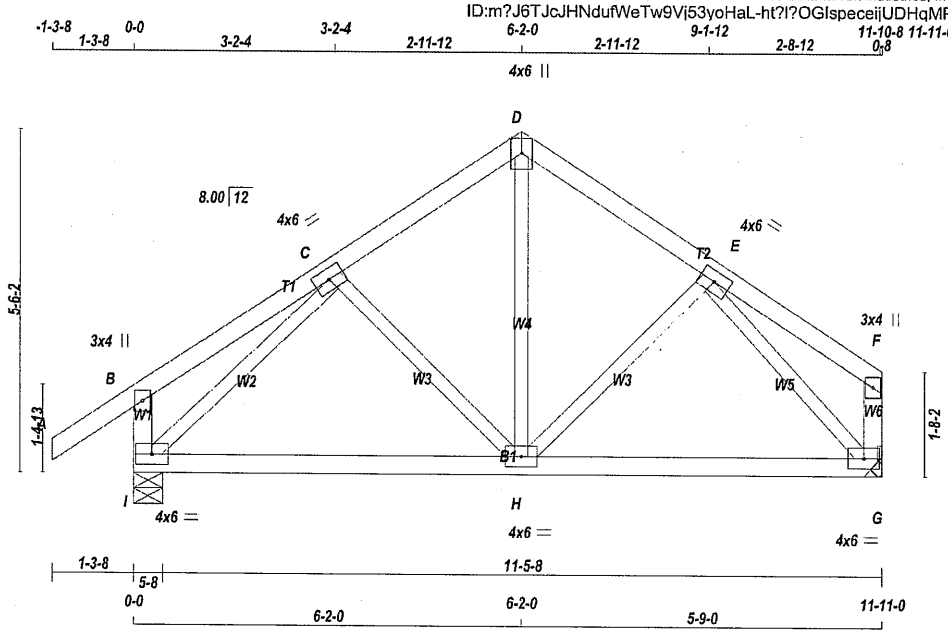

TOTAL WEIGHT = 2 X 52 = 103 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1061	0	1061	0	0	5-8	5-8
G	883	0	883	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	736	560 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0
G	615	455 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	0 / 25	-129.6	-129.6	0.19 (1)	10.00
C- D	-650 / 0	-129.6	-129.6	0.15 (1)	6.25
D- E	-646 / 0	-129.6	-129.6	0.13 (1)	6.25
E- F	0 / 25	-129.6	-129.6	0.17 (1)	10.00
I- B	-332 / 0	0.0	0.0	0.03 (1)	7.81
G- F	-127 / 0	0.0	0.0	0.01 (1)	7.81
I- H	0 / 671	-18.5	-18.5	0.23 (4)	10.00
H- G	0 / 607	-18.5	-18.5	0.22 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	0 / 375	0.08 (1)
H- E	-124 / 11	0.04 (1)
C- H	-215 / 0	0.07 (1)
I- C	-957 / 0	0.29 (1)
E- G	-938 / 0	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.23/1.00 (H-I:4) , WB=0.29/1.00 (C-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (E) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040292

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T52A	1	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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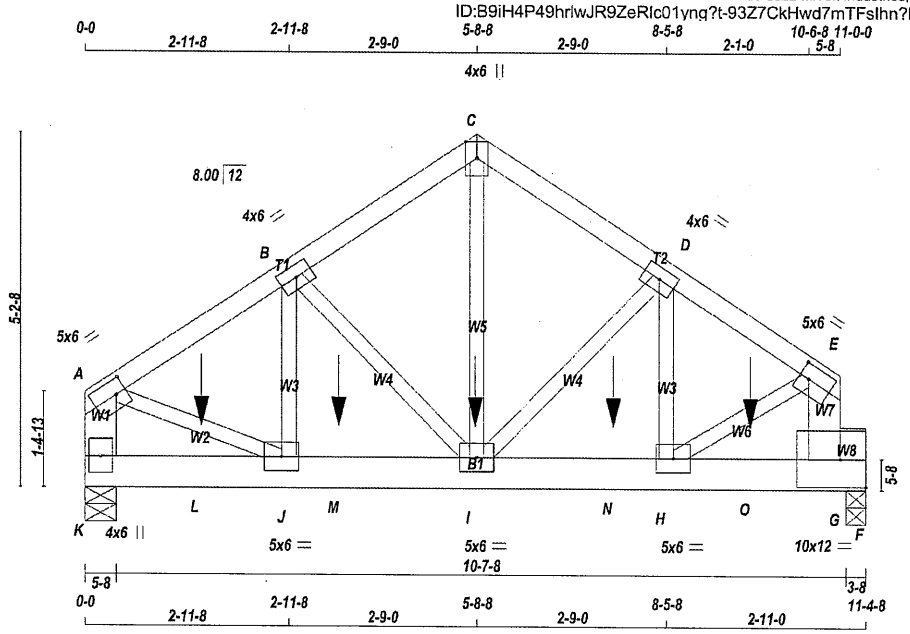

TOTAL WEIGHT = 3 X 58 = 174 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
K - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
G- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- F	2	11	SIDE(366.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040293

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	4134	0	4134	0	0	5-8	5-8
F	4092	0	4092	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	2876	2157 / 0	0 / 0	0 / 0	0 / 0	719 / 0	0 / 0
F	2847	2131 / 0	0 / 0	0 / 0	0 / 0	716 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4181 / 0	-129.6	-129.6	0.08 (1)	5.45	
B- C	-3353 / 0	-129.6	-129.6	0.06 (1)	5.94	
C- D	-3349 / 0	-129.6	-129.6	0.06 (1)	5.94	
D- E	-4251 / 0	-129.6	-129.6	0.06 (1)	5.43	
K- A	-3672 / 0	0.0	0.0	0.08 (1)	7.81	
G- E	-4290 / 0	0.0	0.0	0.10 (1)	7.81	
K- L	0 / 0	-18.5	-18.5	0.17 (1)	10.00	
L- J	0 / 0	-18.5	-18.5	0.17 (1)	10.00	
J- M	0 / 3488	-18.5	-18.5	0.23 (1)	10.00	
M- I	0 / 3488	-18.5	-18.5	0.23 (1)	10.00	
I- N	0 / 3544	-18.5	-18.5	0.29 (1)	10.00	
N- H	0 / 3544	-18.5	-18.5	0.29 (1)	10.00	
H- O	0 / 0	-18.5	-18.5	0.34 (1)	10.00	
O- G	0 / 0	-18.5	-18.5	0.34 (1)	10.00	
G- F	0 / 0	-18.5	-18.5	0.14 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
I- C	0 / 3385	0.25 (1)
B- I	-1041 / 0	0.09 (1)
J- B	0 / 824	0.06 (1)
A- J	0 / 3680	0.28 (1)
I- D	-1123 / 0	0.10 (1)
H- D	0 / 963	0.07 (1)
H- E	0 / 3964	0.30 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	5-8-4	-916	-916	---	FRONT	VERT	TOTAL	---	C1
L	1-8-4	-916	-916	---	FRONT	VERT	TOTAL	---	C1
M	3-8-4	-916	-916	---	FRONT	VERT	TOTAL	---	C1
N	7-8-4	-916	-916	---	FRONT	VERT	TOTAL	---	C1
O	9-8-4	-916	-916	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.10/1.00 (E-G:1) , BC=0.34/1.00 (G-H:1) , WB=0.30/1.00 (E-H:1) , SSI=0.41/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.33 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T52A	1	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-t	MT20	5.0	6.0	2.50	1.75
G	BMVW-t	MT20	10.0	12.0	5.00	4.50
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040293

TOWN OF INNISFIL
Block 5
REVIEWED
jbenfold
4/16/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T55	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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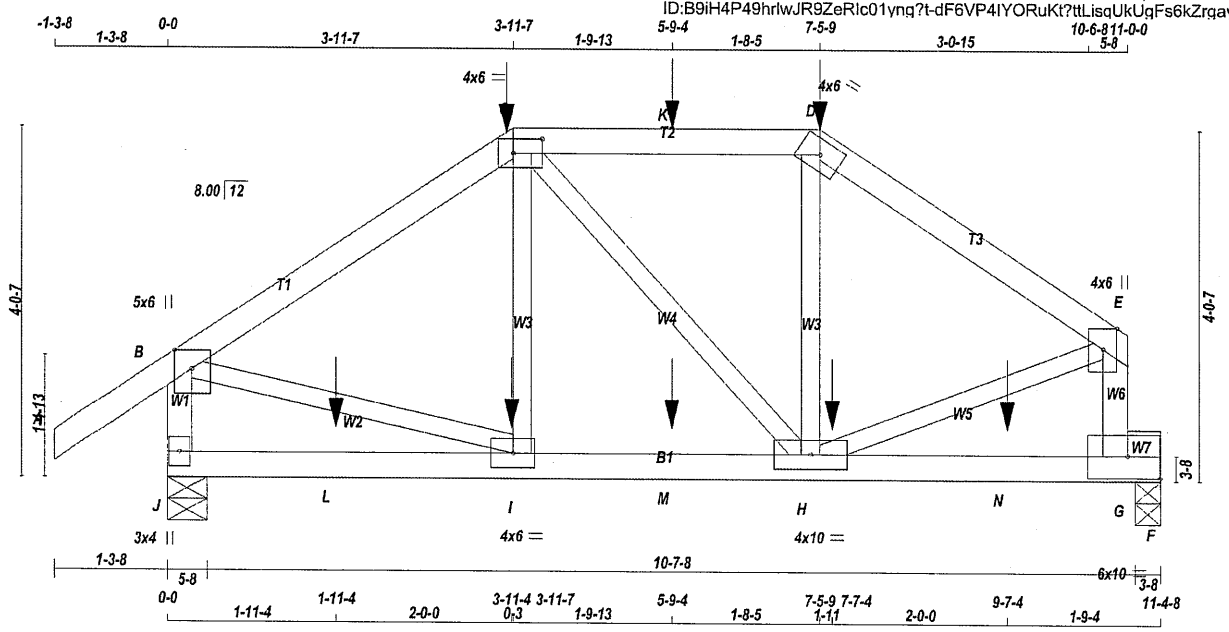

TOTAL WEIGHT = 2 X 48 = 95 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMVW-t	MT20	6.0	10.0	3.00	4.50
H	BMWWW-t	MT20	4.0	10.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040294

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1292	0	1292	0	0	5-8	5-8
F	1070	0	1070	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	897	684 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
F	745	552 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00
B- C	-1049 / 0	-129.6	-129.6	0.41 (1)	5.52
C- K	-896 / 0	-129.6	-129.6	0.39 (1)	5.88
K- D	-896 / 0	-129.6	-129.6	0.39 (1)	5.88
D- E	-1083 / 0	-129.6	-129.6	0.33 (1)	5.59
J- B	-1250 / 0	0.0	0.0	0.14 (1)	7.16
G- E	-1168 / 0	0.0	0.0	0.14 (1)	7.34
J- L	0 / 0	-18.5	-18.5	0.09 (4)	10.00
L- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00
I- M	0 / 866	-18.5	-18.5	0.34 (1)	10.00
M- H	0 / 866	-18.5	-18.5	0.34 (1)	10.00
H- N	0 / 0	-18.5	-18.5	0.60 (1)	10.00
N- G	0 / 0	-18.5	-18.5	0.60 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.25 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-186 / 35	0.05 (1)
C- H	0 / 45	0.01 (1)
H- D	-107 / 50	0.03 (1)
B- I	0 / 901	0.22 (1)
H- E	0 / 960	0.24 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-7	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	3-11-7	-45	-45	---	FRONT	VERT	TOTAL	---	C1
C	3-11-7	-95	-95	---	FRONT	VERT	SNOW	---	C1
D	7-5-9	-15	-15	---	FRONT	VERT	DEAD	---	C1
D	7-5-9	-44	-44	---	FRONT	VERT	TOTAL	---	C1
D	7-5-9	-95	-95	---	FRONT	VERT	SNOW	---	C1
H	7-7-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
I	3-11-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
K	5-9-4	-39	-39	---	FRONT	VERT	TOTAL	---	C1
L	1-11-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
M	5-9-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
N	9-7-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.60/1.00 (G-H:1) , WB=0.24/1.00 (E-H:1) , SSI=0.46/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.56 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T56	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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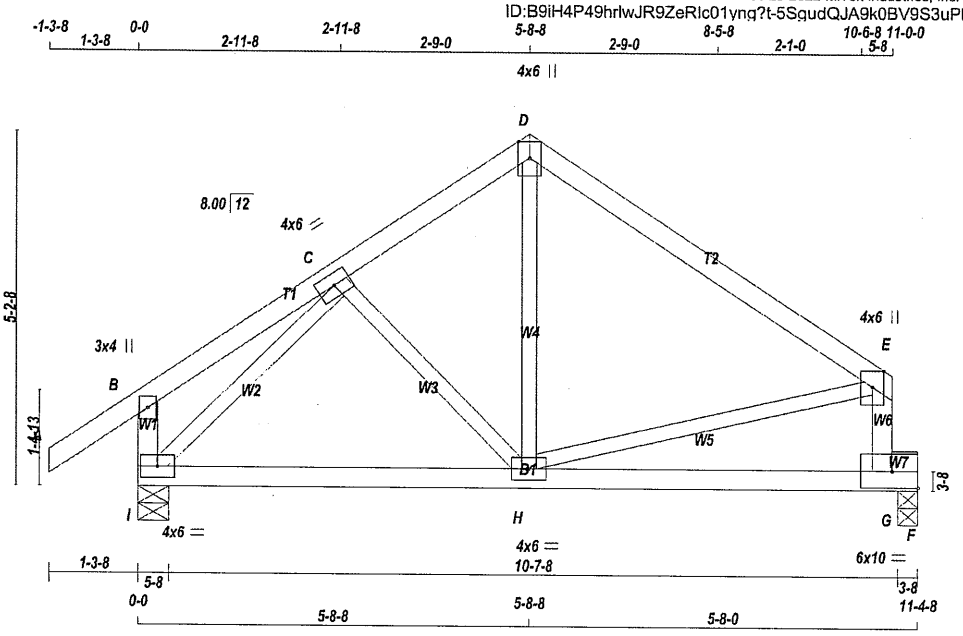

TOTAL WEIGHT = 2 X 47 = 94 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMVW-t	MT20	6.0	10.0	3.00	Edge
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	795	0	795	0	0	3-8	3-8
I	1020	0	1020	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
I	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	0 / 23	-129.6	-129.6	0.16 (1)	10.00
C- D	-647 / 0	-129.6	-129.6	0.16 (1)	6.25
D- E	-621 / 0	-129.6	-129.6	0.65 (1)	6.12
I- B	-321 / 0	0.0	0.0	0.03 (1)	7.81
G- E	-818 / 0	0.0	0.0	0.09 (1)	7.81
I- H	0 / 640	-18.5	-18.5	0.28 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.40 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	-176 / 0	0.05 (1)
H- D	0 / 203	0.05 (1)
I- C	-926 / 0	0.25 (1)
H- E	0 / 533	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 940 (0.15")

CSI: TC=0.65/1.00 (D-E:1) , BC=0.40/1.00 (G-H:1) , WB=0.25/1.00 (C-I:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (H) (INPUT = 0.90)
JSI METAL= 0.37 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040295

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
jlandolo
4/04/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T57	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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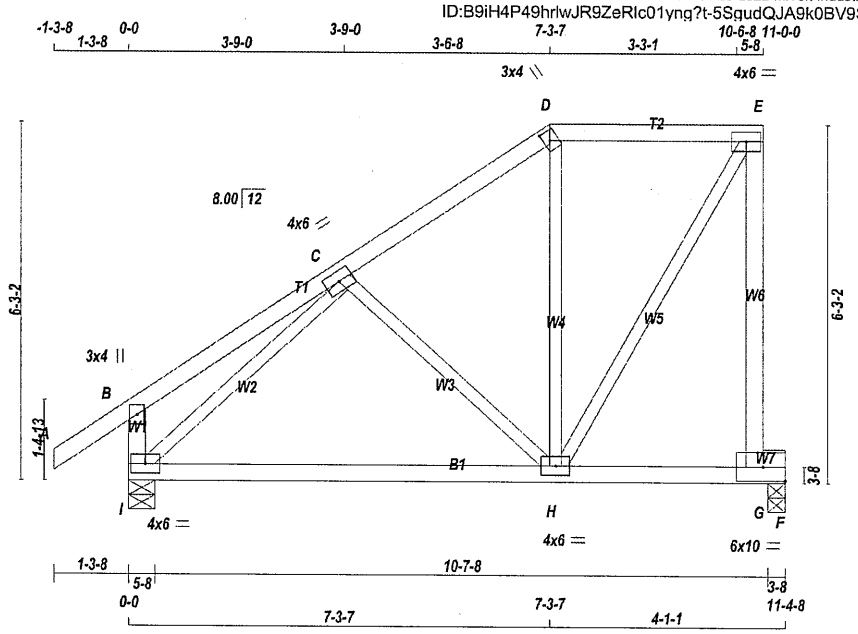

TOTAL WEIGHT = 2 X 55 = 110 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	Edge
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040296

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	795	0	795	0	0	3-8	3-8
I	1020	0	1020	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
I	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
B- C	0 / 31	-129.6	-129.6	0.28 (1)	10.00	
C- D	-496 / 0	-129.6	-129.6	0.29 (1)	6.25	
D- E	-383 / 0	-129.6	-129.6	0.31 (1)	6.25	
G- E	-873 / 0	0.0	0.0	0.68 (1)	7.81	
I- B	-357 / 0	0.0	0.0	0.04 (1)	7.81	
I- H	0 / 635	-18.5	-18.5	0.28 (1)	10.00	
H- G	0 / 0	-18.5	-18.5	0.40 (1)	10.00	
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C- H	-340 / 0	0.15 (1)
H- D	-164 / 19	0.10 (1)
H- E	0 / 741	0.17 (1)
I- C	-881 / 0	0.36 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.68/1.00 (E-G:1) , BC=0.40/1.00 (G-H:1) , WB=0.36/1.00 (C-I:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.23 (H) (INPUT = 1.00)

[M]

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T58	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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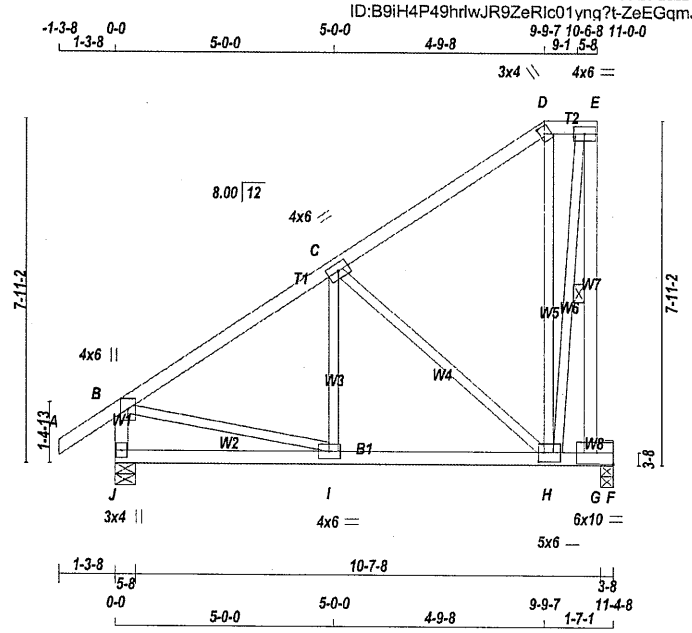

TOTAL WEIGHT = 2 X 64 = 128 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	4.50
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1020	0	1020	0	0	5-8	5-8
F	795	0	795	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
F	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-716 / 0	-129.6	-129.6	0.55 (1)	6.08
C- D	-217 / 0	-129.6	-129.6	0.53 (1)	6.25
D- E	-140 / 0	-129.6	-129.6	0.03 (1)	6.25
G- E	-1044 / 0	0.0	0.0	0.30 (1)	6.15
J- B	-983 / 0	0.0	0.0	0.10 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.12 (4)	10.00
I- H	0 / 631	-18.5	-18.5	0.16 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.40 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-26 / 86	0.03 (4)
C- H	-657 / 0	0.52 (1)
H- D	-215 / 0	0.26 (1)
H- E	0 / 977	0.22 (1)
B- I	0 / 646	0.15 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.40/1.00 (G-H:1) , WB=0.52/1.00 (C-H:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040297

TOWN OF INNISFIL Block 5 REVIEWED 8/16/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T59	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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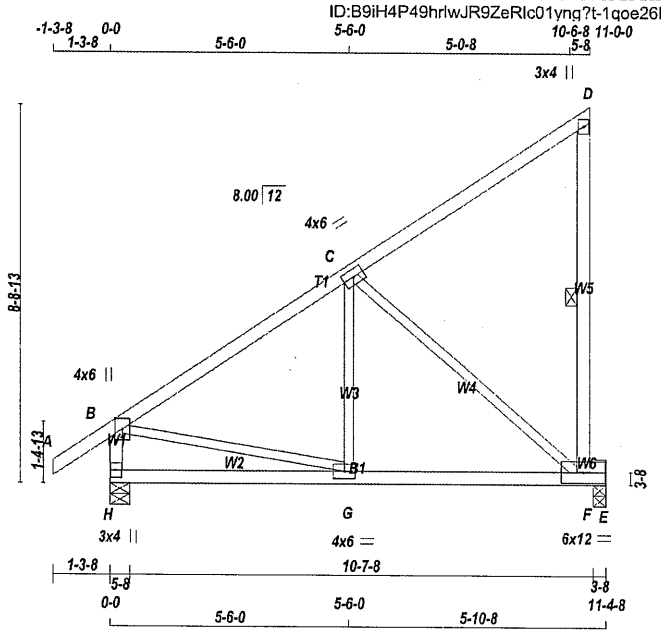

TOTAL WEIGHT = 2 X 54 = 108 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1020	0	1020	0	0	5-8	5-8
E	795	0	795	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
E	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-712 / 0	-129.6	-129.6	0.71 (1)	5.66
C- D	-49 / 0	-129.6	-129.6	0.66 (1)	6.25
F- D	-269 / 0	0.0	0.0	0.09 (1)	6.25
H- B	-993 / 0	0.0	0.0	0.10 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.20 (4)	10.00
G- F	0 / 633	-18.5	-18.5	0.51 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 131	0.05 (4)
C- F	-836 / 0	0.81 (1)
B- G	0 / 645	0.15 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 881 (0.15")

CSI: TC=0.71/1.00 (B-C:1) , BC=0.51/1.00 (F-G:1) , WB=0.81/1.00 (C-F:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (B) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040298

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T60	2	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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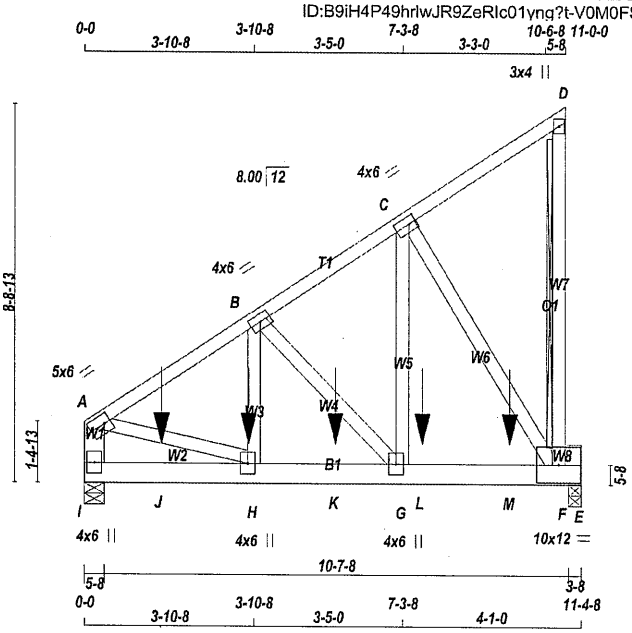

TOTAL WEIGHT = 6 X 73 = 441 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
I- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- E	2	11	SIDE(466.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040299

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	4086	0	4086	0	0	5-8	5-8
E	4140	0	4140	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2842	2131 / 0	0 / 0	0 / 0	0 / 0	711 / 0	0 / 0
E	2881	2157 / 0	0 / 0	0 / 0	0 / 0	724 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4161 / 0	-129.6	-129.6	0.11 (1)		5.42
B- C	-2732 / 0	-129.6	-129.6	0.09 (1)		6.25
C- D	-25 / 0	-129.6	-129.6	0.08 (1)		6.25
F- D	-195 / 0	0.0	0.0	0.02 (1)		7.81
I- A	-3432 / 0	0.0	0.0	0.07 (1)		7.81
I- J	0 / 0	-18.5	-18.5	0.25 (1)		10.00
J- H	0 / 0	-18.5	-18.5	0.25 (1)		10.00
H- K	0 / 3477	-18.5	-18.5	0.40 (1)		10.00
K- G	0 / 3477	-18.5	-18.5	0.40 (1)		10.00
G- L	0 / 2279	-18.5	-18.5	0.53 (1)		10.00
L- M	0 / 2279	-18.5	-18.5	0.53 (1)		10.00
M- F	0 / 2279	-18.5	-18.5	0.53 (1)		10.00
F- E	0 / 0	-18.5	-18.5	0.14 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- H	0 / 3590	0.19 (1)
H- B	0 / 1562	0.08 (1)
B- G	-1735 / 0	0.16 (1)
G- C	0 / 4599	0.25 (1)
C- F	-4261 / 0	0.86 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	3-9-0	-916	-916	---	FRONT	VERT	TOTAL	---	C1
J	1-9-0	-916	-916	---	FRONT	VERT	TOTAL	---	C1
K	5-9-0	-916	-916	---	FRONT	VERT	TOTAL	---	C1
L	7-9-0	-916	-916	---	FRONT	VERT	TOTAL	---	C1
M	9-9-0	-916	-916	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.53/1.00 (F-G:1) , WB=0.86/1.00 (C-F:1) , SSI=0.48/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.52 (H) (INPUT = 1.00)

CONTINUED ON PAGE 2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402645	T60	2	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:30:57 2023 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.00
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	10.0	12.0		
G	BMWW+t	MT20	4.0	6.0		
H	BMWW+t	MT20	4.0	6.0		
I	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040299

TOWN OF INNISFIL
Block 5
REVIEWED
jlenfold
4/16/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T61	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:30:58 2023 Page 1
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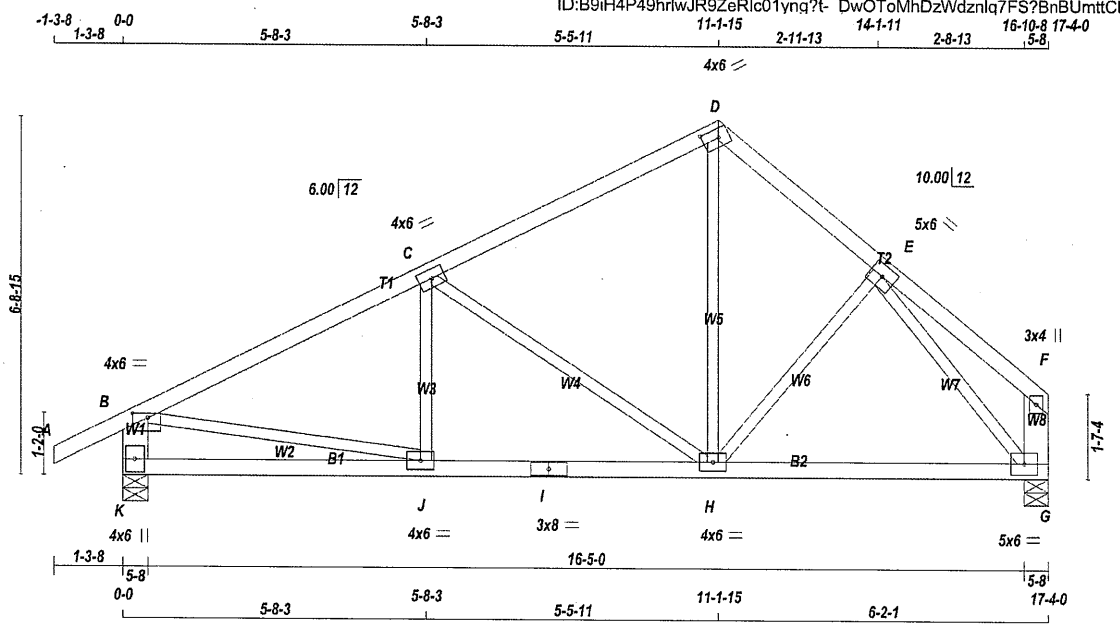

TOTAL WEIGHT = 8 X 73 = 588 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	6.0		
D	TTW-I	MT20	4.0	6.0	2.00	3.50
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	5.0	6.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1460	0	1460	0	0	5-8	5-8
G	1284	0	1284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1015	766 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0
G	895	662 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-1601 / 0	-129.6	-129.6	0.56 (1)	4.52
C- D	-947 / 0	-129.6	-129.6	0.52 (1)	5.59
D- E	-1073 / 0	-129.6	-129.6	0.15 (1)	5.93
E- F	0 / 28	-129.6	-129.6	0.18 (1)	10.00
K- B	-1413 / 0	0.0	0.0	0.09 (1)	7.81
G- F	-155 / 0	0.0	0.0	0.01 (1)	7.81
K- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00
J- I	0 / 1466	-18.5	-18.5	0.32 (1)	10.00
I- H	0 / 1466	-18.5	-18.5	0.32 (1)	10.00
H- G	0 / 866	-18.5	-18.5	0.23 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
J- C	-118 / 65	0.03 (1)
C- H	-794 / 0	0.61 (1)
H- D	0 / 650	0.15 (1)
H- E	-100 / 16	0.04 (1)
B- J	0 / 1483	0.33 (1)
E- G	-1385 / 0	0.51 (1)

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.32/1.00 (H-J:1) , WB=0.61/1.00 (C-H:1) , SSI=0.32/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.46 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040300

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan Review

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G61	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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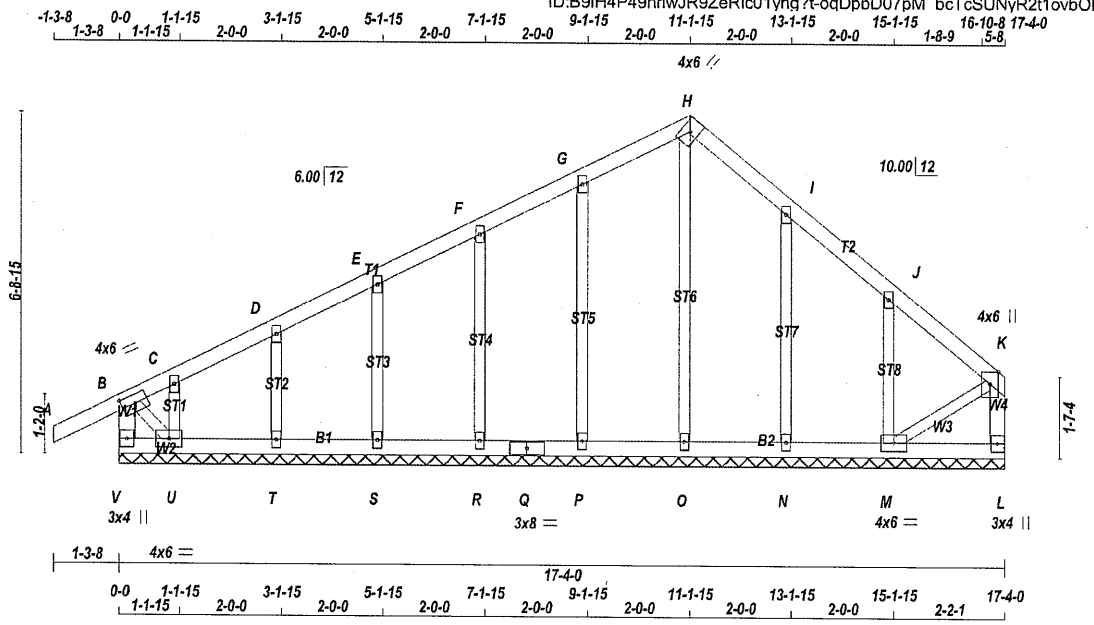

TOTAL WEIGHT = 2 X 74 = 148 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
V - Q	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C, D, E, F, G, I, J						
C	TMW+w	MT20	2.0	4.0		
H	TTW+h	MT20	4.0	6.0		
K	TMVW+p	MT20	4.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	6.0		
N, O, P, R, S, T						
N	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
U	BMWW1-t	MT20	4.0	6.0		
V	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040266

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	17 (1)	10.00	
B- C	-93 / 0	-129.6	-129.6	16 (1)	6.25	
C- D	-35 / 0	-129.6	-129.6	07 (1)	6.25	
D- E	-36 / 0	-129.6	-129.6	07 (1)	6.25	
E- F	-29 / 0	-129.6	-129.6	06 (1)	6.25	
F- G	-21 / 0	-129.6	-129.6	08 (1)	6.25	
G- H	-32 / 0	-129.6	-129.6	08 (1)	6.25	
H- I	-43 / 0	-129.6	-129.6	07 (1)	6.25	
I- J	-38 / 0	-129.6	-129.6	08 (1)	6.25	
J- K	-29 / 0	-129.6	-129.6	08 (1)	6.25	
V- B	-399 / 0	0.0	0.0	04 (1)	7.81	
L- K	-164 / 0	0.0	0.0	02 (1)	7.81	
V- U	0 / 0	-18.5	-18.5	01 (4)	10.00	
U- T	0 / 38	-18.5	-18.5	02 (4)	10.00	
T- S	0 / 31	-18.5	-18.5	02 (4)	10.00	
S- R	0 / 26	-18.5	-18.5	02 (4)	10.00	
R- Q	0 / 22	-18.5	-18.5	02 (4)	10.00	
Q- P	0 / 22	-18.5	-18.5	02 (4)	10.00	
P- O	0 / 18	-18.5	-18.5	02 (4)	10.00	
O- N	0 / 21	-18.5	-18.5	02 (4)	10.00	
N- M	0 / 27	-18.5	-18.5	02 (4)	10.00	
M- L	0 / 0	-18.5	-18.5	02 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
O- H	-192 / 0	0.14 (1)
P- G	-304 / 0	0.15 (1)
R- F	-248 / 0	0.08 (1)
S- E	-254 / 0	0.06 (1)
T- D	-275 / 0	0.05 (1)
U- C	-72 / 0	0.01 (1)
N- I	-273 / 0	0.11 (1)
M- J	-296 / 0	0.06 (1)
M- K	0 / 41	0.01 (1)
B- U	0 / 54	0.01 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.02/1.00 (M-N:4) , WB=0.15/1.00 (G-P:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (B) (INPUT = 0.90)
JSI METAL= 0.16 (J) (INPUT = 1.00)

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T61A	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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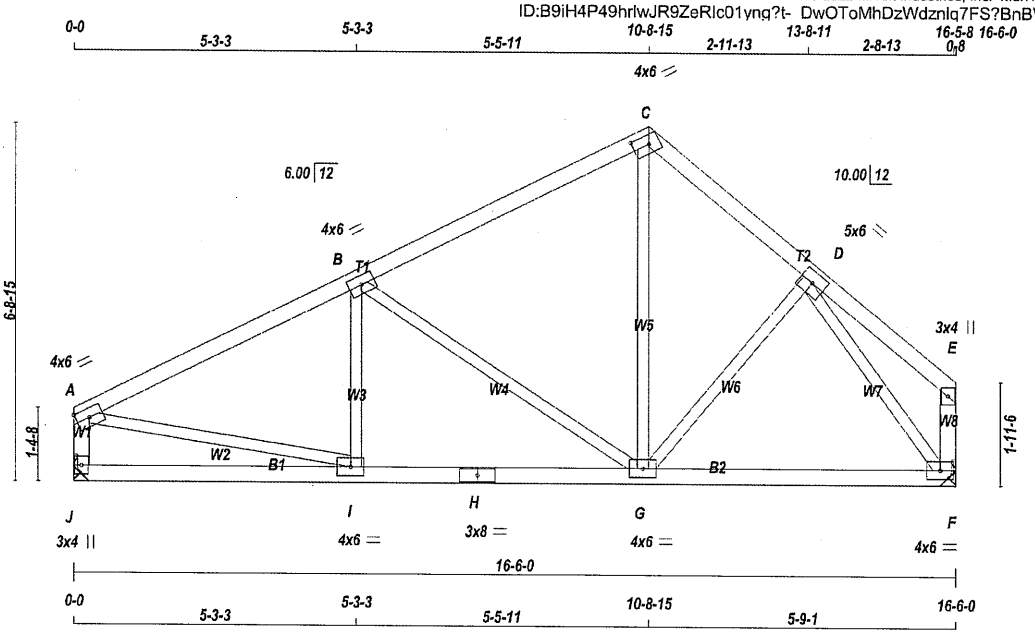

TOTAL WEIGHT = 8 X 68 = 547 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	4.0	6.0		
C	TTW-l	MT20	4.0	6.0	2.00	3.50
D	TMWW-t	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1222	0	1222	0	0	MECHANICAL	
F	1222	0	1222	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	852	631 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0
F	852	631 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1432 / 0	-129.6	-129.6	0.51 (1)	4.81
B- C	-852 / 0	-129.6	-129.6	0.48 (1)	5.90
C- D	-965 / 0	-129.6	-129.6	0.14 (1)	6.18
D- E	0 / 28	-129.6	-129.6	0.16 (1)	10.00
J- A	-1180 / 0	0.0	0.0	0.12 (1)	7.35
F- E	-127 / 0	0.0	0.0	0.01 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00
I- H	0 / 1315	-18.5	-18.5	0.29 (1)	10.00
H- G	0 / 1315	-18.5	-18.5	0.29 (1)	10.00
G- F	0 / 727	-18.5	-18.5	0.20 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-167 / 50	0.04 (1)
B- G	-711 / 0	0.55 (1)
G- C	0 / 530	0.12 (1)
G- D	-10 / 34	0.01 (4)
A- I	0 / 1342	0.30 (1)
D- F	-1278 / 0	0.44 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.51/1.00 (A-B:1) , BC=0.29/1.00 (G-I:1) , WB=0.55/1.00 (B-G:1) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.43 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040301

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan Review

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T62A	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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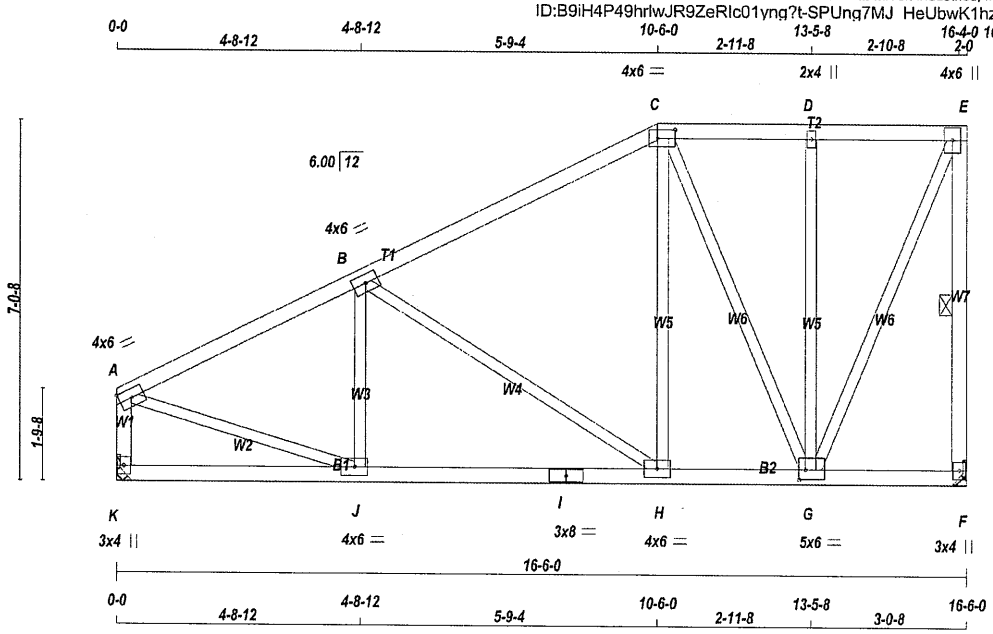

TOTAL WEIGHT = 4 X 82 = 328 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	4.0	6.0		
C	TTWW-l	MT20	4.0	6.0	2.00	4.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.25	1.50
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040302

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1222	0	1222	0	0	MECHANICAL	
K	1222	0	1222	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, K. MINIMUM BEARING LENGTH AT JOINT F = 3-8, JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	852	631 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0
K	852	631 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1297 / 0	-129.6	-129.6	0.49 (1)	5.02	
B- C	-808 / 0	-129.6	-129.6	0.51 (1)	5.97	
C- D	-468 / 0	-129.6	-129.6	0.13 (1)	6.25	
D- E	-468 / 0	-129.6	-129.6	0.13 (1)	6.25	
F- E	-1194 / 0	0.0	0.0	0.27 (1)	5.85	
K- A	-1187 / 0	0.0	0.0	0.13 (1)	7.33	
K- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00	
J- I	0 / 1198	-18.5	-18.5	0.25 (1)	10.00	
I- H	0 / 1198	-18.5	-18.5	0.25 (1)	10.00	
H- G	0 / 698	-18.5	-18.5	0.16 (1)	10.00	
G- F	0 / 0	-18.5	-18.5	0.04 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-268 / 31	0.07 (1)
B- H	-605 / 0	0.57 (1)
H- C	0 / 425	0.10 (1)
A- J	0 / 1255	0.28 (1)
C- G	-573 / 0	0.60 (1)
G- D	-472 / 0	0.40 (1)
G- E	0 / 1138	0.26 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.51/1.00 (B-C:1) , BC=0.25/1.00 (H-J:1) , WB=0.60/1.00 (C-G:1) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (G) (INPUT = 0.90)
JSI METAL= 0.42 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G62A	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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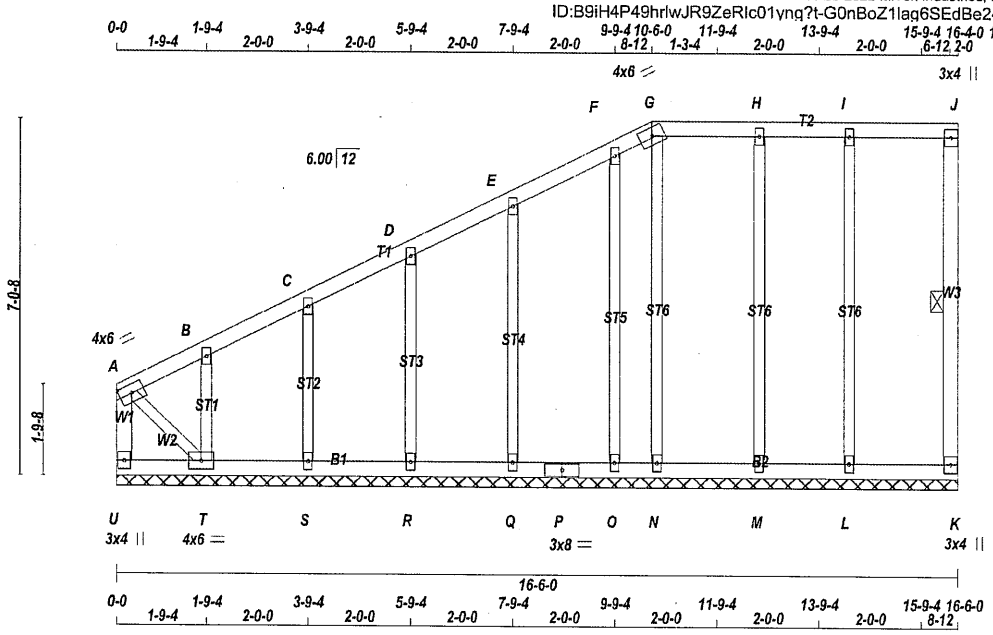

TOTAL WEIGHT = 2 X 82 = 164 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B, C, D, E, F, H, I						
B	TMW+w	MT20	2.0	4.0		
G	TTW-h	MT20	4.0	6.0		
J	TMV+p	MT20	3.0	4.0		
K	BMV1+p	MT20	3.0	4.0		
L, M, N, O, Q, R, S						
L	BMW1+w	MT20	2.0	4.0		
P	BS-t	MT20	3.0	8.0		
T	BMWW1-t	MT20	4.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040267

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-15 / 0	-129.6	-129.6	0.06 (1)	6.25	
B- C	-18 / 0	-129.6	-129.6	0.06 (1)	6.25	
C- D	-13 / 0	-129.6	-129.6	0.06 (1)	6.25	
D- E	-8 / 0	-129.6	-129.6	0.06 (1)	10.00	
E- F	-8 / 0	-129.6	-129.6	0.06 (1)	10.00	
F- G	-18 / 0	-129.6	-129.6	0.04 (1)	6.25	
G- H	0 / 0	-129.6	-129.6	0.07 (1)	10.00	
H- I	0 / 0	-129.6	-129.6	0.07 (1)	10.00	
I- J	0 / 0	-129.6	-129.6	0.07 (1)	10.00	
K- J	-117 / 0	0.0	0.0	0.03 (1)	6.25	
U- A	-116 / 0	0.0	0.0	0.01 (1)	7.81	
U- T	0 / 0	-18.5	-18.5	0.02 (4)	10.00	
T- S	0 / 16	-18.5	-18.5	0.02 (4)	10.00	
S- R	0 / 11	-18.5	-18.5	0.01 (4)	10.00	
R- Q	0 / 7	-18.5	-18.5	0.02 (4)	10.00	
Q- P	0 / 4	-18.5	-18.5	0.02 (4)	10.00	
P- O	0 / 4	-18.5	-18.5	0.02 (4)	10.00	
O- N	0 / 2	-18.5	-18.5	0.01 (4)	10.00	
N- M	0 / 0	-18.5	-18.5	0.01 (4)	10.00	
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00	
L- K	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- E	-268 / 0	0.13 (1)
R- D	-256 / 0	0.08 (1)
S- C	-257 / 0	0.06 (1)
T- B	-263 / 0	0.04 (1)
O- F	-207 / 0	0.15 (1)
N- G	-129 / 0	0.11 (1)
M- H	-272 / 0	0.23 (1)
L- I	-275 / 0	0.23 (1)
A- T	0 / 31	0.01 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2 .

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (I-J:1) , BC=0.02/1.00 (K-L:4) , WB=0.23/1.00 (I-L:1) , SSI=0.13/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (A) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 1.00)

TOWN OF INNISFIL
Block 5
REVIEWED
jlandolo
4/04/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T62B	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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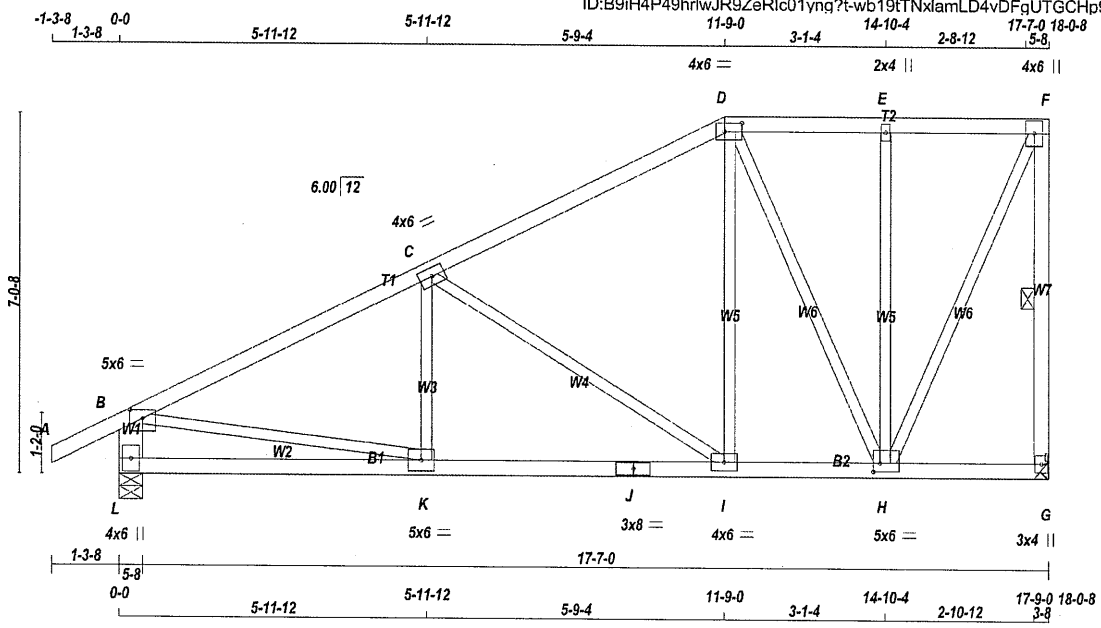

TOTAL WEIGHT = 2 X 88 = 176 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-l	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1336	0	1336	0	0	MECHANICAL	
L	1512	0	1512	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	931	689 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0
L	1051	793 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-1684 / 0	-129.6	-129.6	0.63 (1)	4.30
C- D	-947 / 0	-129.6	-129.6	0.58 (1)	5.46
D- E	-541 / 0	-129.6	-129.6	0.15 (1)	6.25
E- F	-541 / 0	-129.6	-129.6	0.15 (1)	6.25
G- F	-1308 / 0	0.0	0.0	0.29 (1)	5.65
L- B	-1466 / 0	0.0	0.0	0.09 (1)	7.81
L- K	0 / 0	-18.5	-18.5	0.16 (4)	10.00
K- J	0 / 1543	-18.5	-18.5	0.32 (1)	10.00
J- I	0 / 1543	-18.5	-18.5	0.32 (1)	10.00
I- H	0 / 817	-18.5	-18.5	0.17 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-99 / 87	0.03 (4)
C- I	-877 / 0	0.82 (1)
I- D	0 / 569	0.13 (1)
B- K	0 / 1558	0.35 (1)
D- H	-661 / 0	0.70 (1)
H- E	-496 / 0	0.42 (1)
H- F	0 / 1267	0.29 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.63/1.00 (B-C:1) , BC=0.32/1.00 (I-K:1) , WB=0.82/1.00 (C-I:1) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.52 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040303

TOWN OF INNISFIL Block 5 REVIEWED jpendola 4/16/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G62B	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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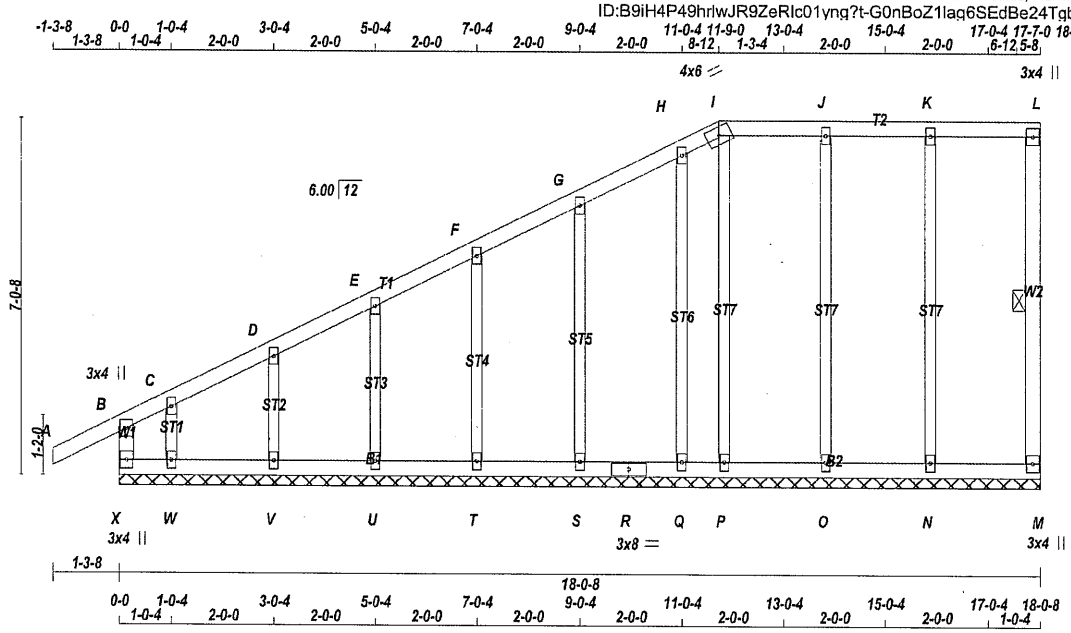

TOTAL WEIGHT = 2 X 86 = 172 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
X - R	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, F, G, H, J, K					
C	TMW+w	MT20	2.0	4.0	
I	TTW-h	MT20	4.0	6.0	
L	TMV+p	MT20	3.0	4.0	
M	BMV1+p	MT20	3.0	4.0	
N, O, P, Q, S, T, U, V, W					
N	BMW1+w	MT20	2.0	4.0	
R	BS-t	MT20	3.0	8.0	
X	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF L-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-77 / 0	-129.6	-129.6	0.14 (1)	6.25
C- D	-24 / 0	-129.6	-129.6	0.07 (1)	6.25
D- E	-24 / 0	-129.6	-129.6	0.07 (1)	6.25
E- F	-16 / 0	-129.6	-129.6	0.06 (1)	6.25
F- G	-12 / 0	-129.6	-129.6	0.06 (1)	6.25
G- H	-11 / 0	-129.6	-129.6	0.06 (1)	6.25
H- I	-6 / 0	-129.6	-129.6	0.05 (1)	10.00
I- J	-3 / 0	-129.6	-129.6	0.06 (1)	10.00
J- K	-3 / 0	-129.6	-129.6	0.07 (1)	10.00
K- L	-3 / 0	-129.6	-129.6	0.07 (1)	10.00
M- L	-123 / 0	0.0	0.0	0.02 (1)	6.25
X- B	-346 / 0	0.0	0.0	0.06 (1)	7.81
X- W	0 / 31	-18.5	-18.5	0.06 (1)	10.00
W- V	0 / 28	-18.5	-18.5	0.02 (4)	10.00
V- U	0 / 20	-18.5	-18.5	0.02 (4)	10.00
U- T	0 / 15	-18.5	-18.5	0.01 (4)	10.00
T- S	0 / 11	-18.5	-18.5	0.02 (4)	10.00
S- R	0 / 8	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 8	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 6	-18.5	-18.5	0.01 (4)	10.00
P- O	0 / 3	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 3	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 3	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- G	-264 / 0	0.13 (1)
T- F	-258 / 0	0.08 (1)
U- E	-253 / 0	0.06 (1)
V- D	-275 / 0	0.05 (1)
W- C	-81 / 0	0.01 (1)
Q- H	-172 / 0	0.13 (1)
P- I	-180 / 0	0.15 (1)
O- J	-273 / 0	0.23 (1)
N- K	-290 / 0	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.06/1.00 (W-X:1) , WB=0.25/1.00 (K-N:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (E) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040268

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T63	10	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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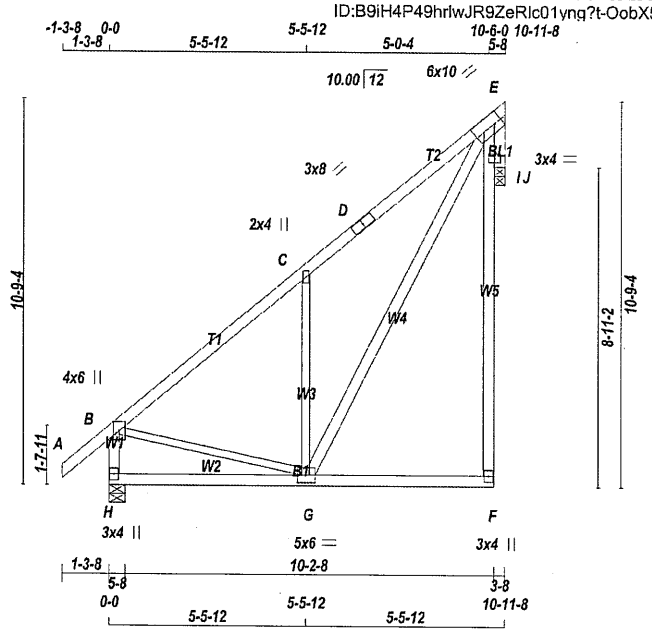

TOTAL WEIGHT = 10 X 66 = 660 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVWK1-t	MT20	6.0	10.0	Edge	2.50
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(E)	779	0	779	0	0	3-8	3-8
							(** SEE "BEARING NOTE" **)
H	980	0	980	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	543	402 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0
H	680	519 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	-547 / 0	-129.6	-129.6	0.48 (1)	6.25
C- D	-622 / 0	-129.6	-129.6	0.48 (1)	6.25
D- E	-622 / 0	-129.6	-129.6	0.48 (1)	6.25
F- I	0 / 39	0.0	0.0	0.14 (1)	10.00
I- E	0 / 39	0.0	0.0	0.14 (1)	10.00
H- B	-938 / 0	0.0	0.0	0.10 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00
G- F	0 / 11	-18.5	-18.5	0.15 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-839 / 0	0.51 (1)
G- E	0 / 949	0.15 (1)
B- G	0 / 474	0.11 (1)
E- J	-784 / 0	0.09 (1)
I- J	0 / 80	0.00 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

[M][F]

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.15/1.00 (F-G:4) , WB=0.51/1.00 (C-G:1) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (G) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040304

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G64	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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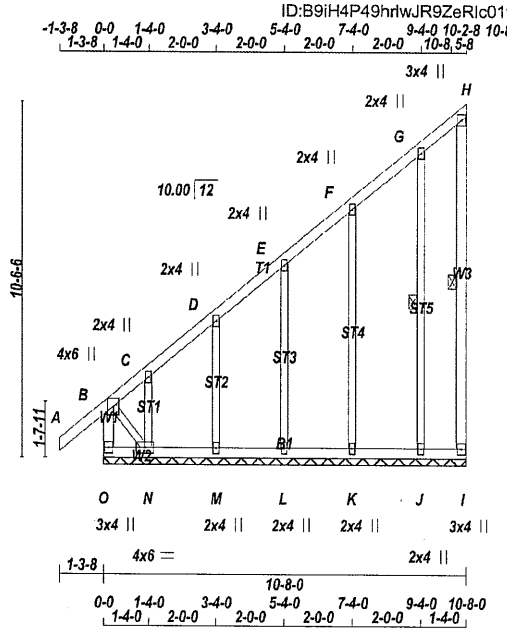

TOTAL WEIGHT = 2 X 66 = 133 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F, G						
C	TMW+w	MT20	2.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, L, M						
J	BMW1+w	MT20	2.0	4.0		
N	BMWW1-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
O- B	-386 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	-83 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	-10 / 0	-129.6	-129.6	0.07 (1)	6.25
D- E	-15 / 0	-129.6	-129.6	0.07 (1)	6.25
E- F	-6 / 0	-129.6	-129.6	0.06 (1)	10.00
F- G	-8 / 0	-129.6	-129.6	0.06 (1)	10.00
G- H	-14 / 0	-129.6	-129.6	0.04 (1)	6.25
I- H	-71 / 0	0.0	0.0	0.04 (1)	6.25
O- N	0 / 0	-18.5	-18.5	0.01 (4)	10.00
N- M	0 / 16	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 10	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 6	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 2	-18.5	-18.5	0.02 (4)	10.00
J- I	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- E	-250 / 0	0.15 (1)
M- D	-275 / 0	0.08 (1)
N- C	-101 / 0	0.02 (1)
K- F	-266 / 0	0.30 (1)
J- G	-234 / 0	0.14 (1)
B- N	0 / 28	0.01 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (M-N:4) , WB=0.30/1.00 (F-K:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040269

TOWN OF INNISFIL Block 5 REVIEWED jpenfold 4/04/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T65	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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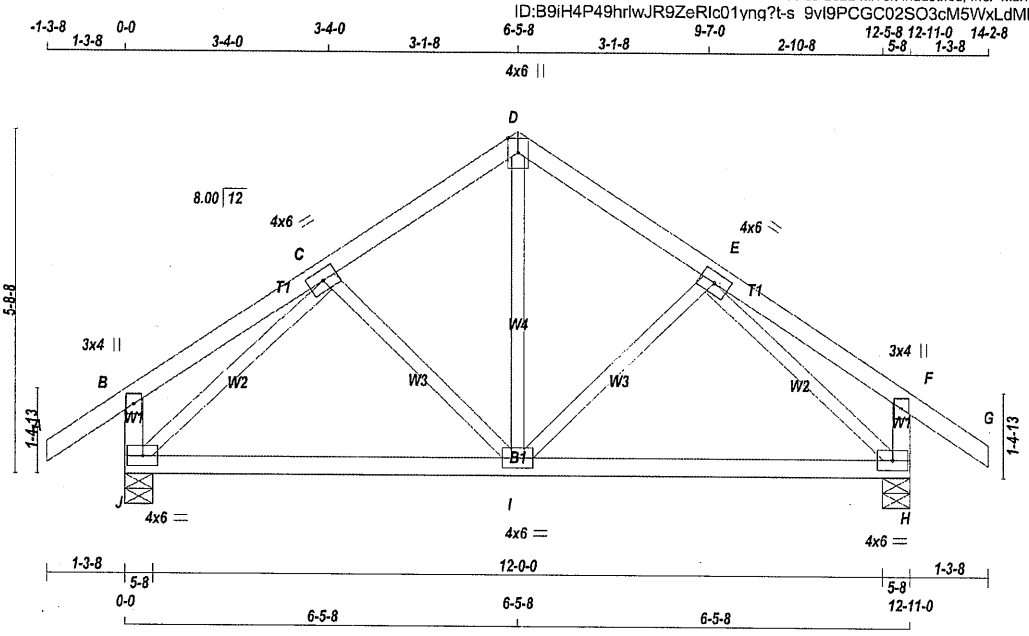

TOTAL WEIGHT = 2 X 56 = 113 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1135	0	1135	0	0	5-8	5-8
H	1135	0	1135	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	788	599 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0
H	788	599 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
B- C	0 / 26	-129.6	-129.6	0.21 (1)	10.00	
C- D	-735 / 0	-129.6	-129.6	0.16 (1)	6.25	
D- E	-735 / 0	-129.6	-129.6	0.16 (1)	6.25	
E- F	0 / 26	-129.6	-129.6	0.21 (1)	10.00	
F- G	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
J- B	-340 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-340 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 747	-18.5	-18.5	0.26 (4)	10.00	
I- H	0 / 747	-18.5	-18.5	0.26 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
I- D	0 / 459	0.10 (1)
I- E	-222 / 0	0.08 (1)
C- I	-222 / 0	0.08 (1)
J- C	-1058 / 0	0.35 (1)
E- H	-1058 / 0	0.35 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.26/1.00 (I-J:4) , WB=0.35/1.00 (E-H:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (C) (INPUT = 0.90)
JSI METAL= 0.23 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040305

TOWN OF INNISFIL
Block 5
REVIEWED
jlandolo
4/04/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G65	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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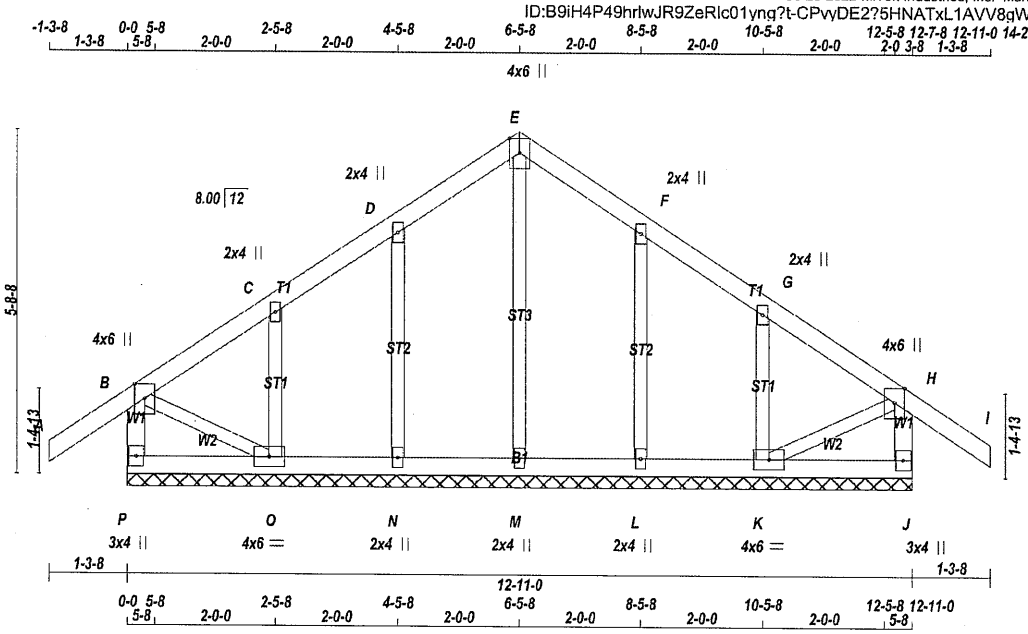

TOTAL WEIGHT = 2 X 57 = 113 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-27 / 0	-129.6	-129.6	0.09 (1)	6.25
C- D	-36 / 0	-129.6	-129.6	0.09 (1)	6.25
D- E	-38 / 0	-129.6	-129.6	0.07 (1)	6.25
E- F	-38 / 0	-129.6	-129.6	0.07 (1)	6.25
F- G	-36 / 0	-129.6	-129.6	0.09 (1)	6.25
G- H	-27 / 0	-129.6	-129.6	0.09 (1)	6.25
H- I	0 / 49	-129.6	-129.6	0.17 (1)	10.00
P- B	-343 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-343 / 0	0.0	0.0	0.04 (1)	7.81
P- O	0 / 0	-18.5	-18.5	0.03 (4)	10.00
O- N	0 / 26	-18.5	-18.5	0.03 (4)	10.00
N- M	0 / 21	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 21	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 26	-18.5	-18.5	0.03 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-194 / 0	0.09 (1)
N- D	-276 / 0	0.08 (1)
O- C	-314 / 0	0.06 (1)
L- F	-276 / 0	0.08 (1)
K- G	-314 / 0	0.06 (1)
B- O	0 / 39	0.01 (1)
K- H	0 / 39	0.01 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.03/1.00 (N-O:4) , WB=0.09/1.00 (E-M:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (C) (INPUT = 0.90)
JSI METAL= 0.16 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040270

TOWN OF INNISFIL Block 5 REVIEWED jpendola 4/14/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T66A	2	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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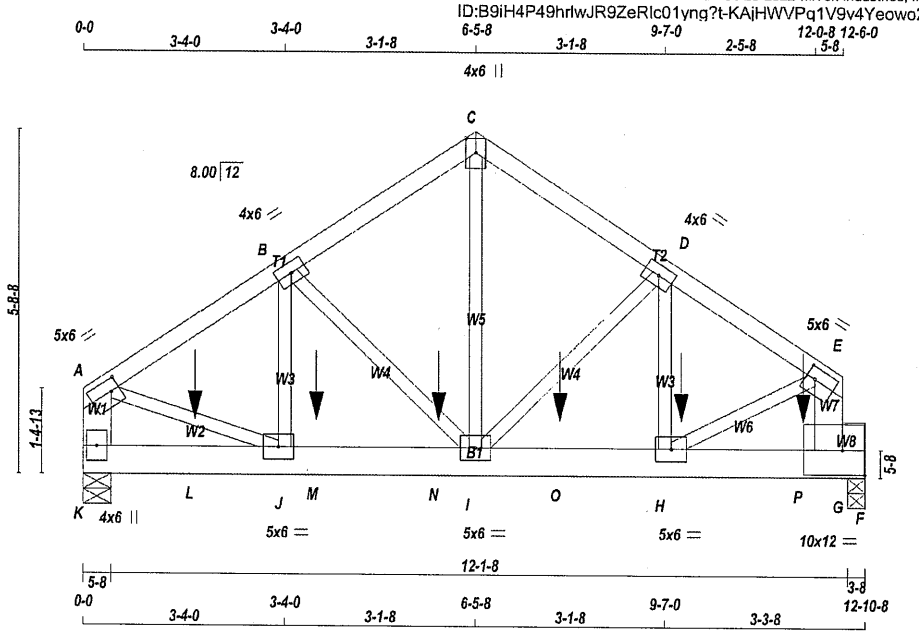

TOTAL WEIGHT = 6 X 65 = 389 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
K - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
G- E	2	8	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- F	2	11	SIDE(490.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040306

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	4356	0	4356	0	0	5-8	5-8
F	4725	0	4725	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	3030	2274 / 0	0 / 0	0 / 0	0 / 0	756 / 0	0 / 0
F	3287	2465 / 0	0 / 0	0 / 0	0 / 0	823 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4628 / 0	-129.6	-129.6	0.09 (1)	5.22	
B- C	-3671 / 0	-129.6	-129.6	0.07 (1)	5.73	
C- D	-3666 / 0	-129.6	-129.6	0.08 (1)	5.73	
D- E	-4757 / 0	-129.6	-129.6	0.08 (1)	5.19	
K- A	-3919 / 0	0.0	0.0	0.09 (1)	7.81	
G- E	-4545 / 0	0.0	0.0	0.10 (1)	7.81	
K- L	0 / 0	-18.5	-18.5	0.17 (1)	10.00	
L- J	0 / 0	-18.5	-18.5	0.17 (1)	10.00	
J- M	0 / 3864	-18.5	-18.5	0.26 (1)	10.00	
M- N	0 / 3864	-18.5	-18.5	0.26 (1)	10.00	
N- I	0 / 3864	-18.5	-18.5	0.26 (1)	10.00	
I- O	0 / 3970	-18.5	-18.5	0.28 (1)	10.00	
O- H	0 / 3970	-18.5	-18.5	0.28 (1)	10.00	
H- P	0 / 0	-18.5	-18.5	0.42 (1)	10.00	
P- G	0 / 0	-18.5	-18.5	0.42 (1)	10.00	
G- F	0 / 0	-18.5	-18.5	0.16 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 3702	0.28 (1)
B- I	-1183 / 0	0.13 (1)
J- B	0 / 920	0.07 (1)
A- J	0 / 4033	0.30 (1)
I- D	-1334 / 0	0.14 (1)
H- D	0 / 1138	0.09 (1)
H- E	0 / 4332	0.32 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	9-10-4	-837	-837	---	BACK	VERT	TOTAL	---	C1
L	1-10-4	-837	-837	---	BACK	VERT	TOTAL	---	C1
M	3-10-4	-837	-837	---	BACK	VERT	TOTAL	---	C1
N	5-10-4	-837	-837	---	BACK	VERT	TOTAL	---	C1
O	7-10-4	-837	-837	---	BACK	VERT	TOTAL	---	C1
P	11-10-4	-837	-837	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.10/1.00 (E-G:1) , BC=0.42/1.00 (G-H:1) , WB=0.32/1.00 (E-H:1) , SSI=0.47/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.37 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

TOWN OF INNISFIL
Block 5
REVIEWED
jlandolfo
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402645	T66A	2	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	1.75
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-t	MT20	5.0	6.0	2.25	1.75
G	BMVW-t	MT20	10.0	12.0	5.00	4.50
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040306

TOWN OF INNISFIL
Block 5
REVIEWED
Greenfield
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T67	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:31:03 2023 Page 1
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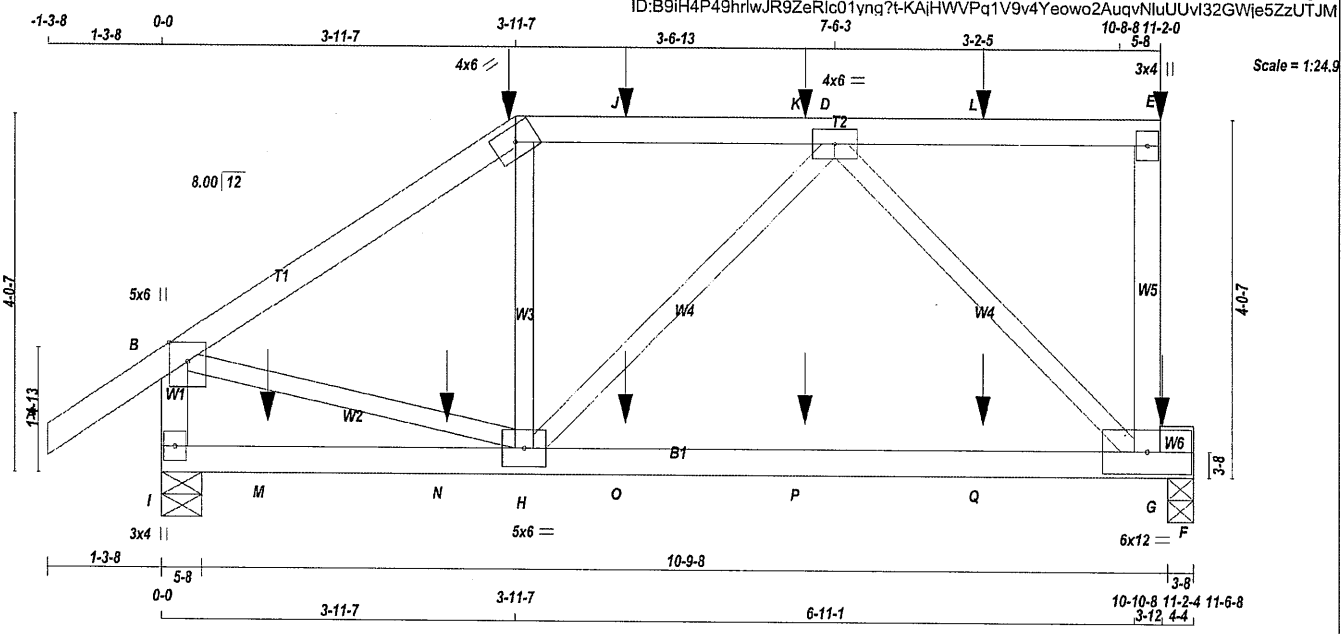

TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-h	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	6.0	12.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040307

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1229	0	1229	0	0	5-8	5-8
F	1119	0	1119	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	854	648 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
F	780	578 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00
B- C	-1038 / 0	-129.6	-129.6	0.41 (1)	5.54
C- J	-860 / 0	-129.6	-129.6	0.38 (1)	6.00
J- K	-860 / 0	-129.6	-129.6	0.38 (1)	6.00
K- D	-860 / 0	-129.6	-129.6	0.38 (1)	6.00
D- L	0 / 0	-129.6	-129.6	0.36 (1)	10.00
L- E	0 / 0	-129.6	-129.6	0.36 (1)	10.00
G- E	-319 / 0	0.0	0.0	0.08 (1)	7.81
I- B	-1238 / 0	0.0	0.0	0.14 (1)	7.18
I- M	0 / 0	-18.5	-18.5	0.34 (4)	10.00
M- N	0 / 0	-18.5	-18.5	0.34 (4)	10.00
N- H	0 / 0	-18.5	-18.5	0.34 (4)	10.00
H- O	0 / 775	-18.5	-18.5	0.76 (1)	10.00
O- P	0 / 775	-18.5	-18.5	0.76 (1)	10.00
P- Q	0 / 775	-18.5	-18.5	0.76 (1)	10.00
Q- G	0 / 775	-18.5	-18.5	0.76 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.26 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-48 / 73	0.03 (4)
B- H	0 / 891	0.22 (1)
H- D	0 / 127	0.04 (4)
D- G	-1110 / 0	0.50 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-7	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	3-11-7	-95	-95	---	FRONT	VERT	SNOW	---	C1
E	11-2-0	-84	-84	---	BACK	VERT	TOTAL	---	C1
G	11-2-4	-11	-11	---	BACK	VERT	TOTAL	---	C1
J	5-2-4	-39	-39	---	BACK	VERT	TOTAL	---	C1
K	7-2-4	-39	-39	---	BACK	VERT	TOTAL	---	C1
L	9-2-4	-39	-39	---	BACK	VERT	TOTAL	---	C1
M	1-2-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
N	3-2-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
O	5-2-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
P	7-2-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
Q	9-2-4	-6	-6	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 891 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 403 (0.34")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.76/1.00 (G-H:1) , WB=0.50/1.00 (D-G:1) , SSI=0.48/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T67	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOWN OF INNISFIL
Block 5
REVIEWED
Plan/Folio
Building Permit Plan

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040307

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T68	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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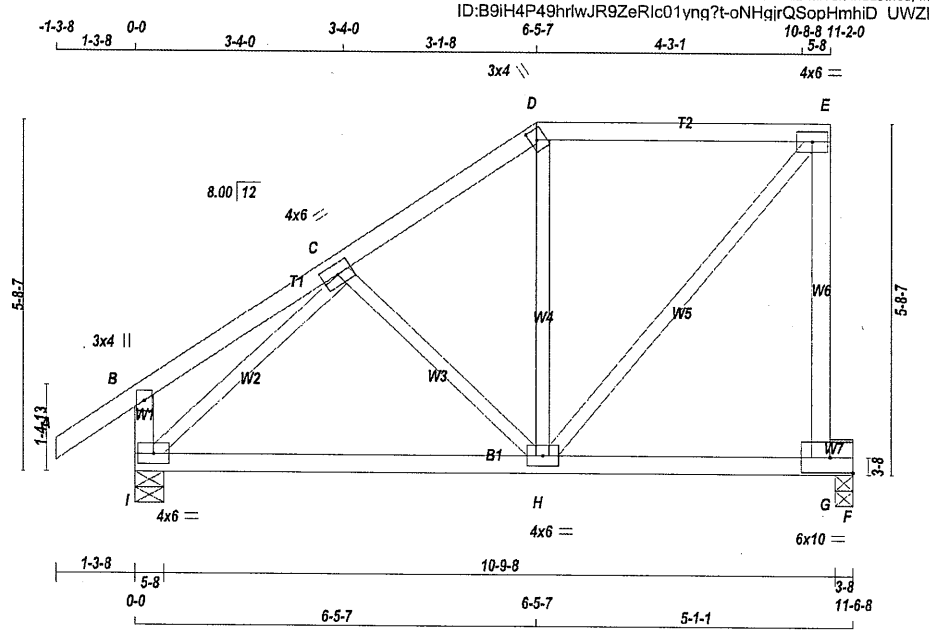

TOTAL WEIGHT = 2 X 53 = 106 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	Edge
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	807	0	807	0	0	3-8	3-8
I	1032	0	1032	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	563	413 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
I	717	546 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)		10.00
B- C	0 / 27	-129.6	-129.6	0.22 (1)		10.00
C- D	-592 / 0	-129.6	-129.6	0.22 (1)		6.25
D- E	-466 / 0	-129.6	-129.6	0.50 (1)		6.25
G- E	-851 / 0	0.0	0.0	0.51 (1)		7.81
I- B	-338 / 0	0.0	0.0	0.03 (1)		7.81
I- H	0 / 652	-18.5	-18.5	0.28 (1)		10.00
H- G	0 / 0	-18.5	-18.5	0.41 (1)		10.00
G- F	0 / 0	-18.5	-18.5	0.17 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C- H	-254 / 0	0.09 (1)
H- D	-154 / 27	0.08 (1)
H- E	0 / 719	0.16 (1)
I- C	-923 / 0	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.51/1.00 (E-G:1) , BC=0.41/1.00 (G-H:1) , WB=0.31/1.00 (C-I:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.21 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040308

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T69	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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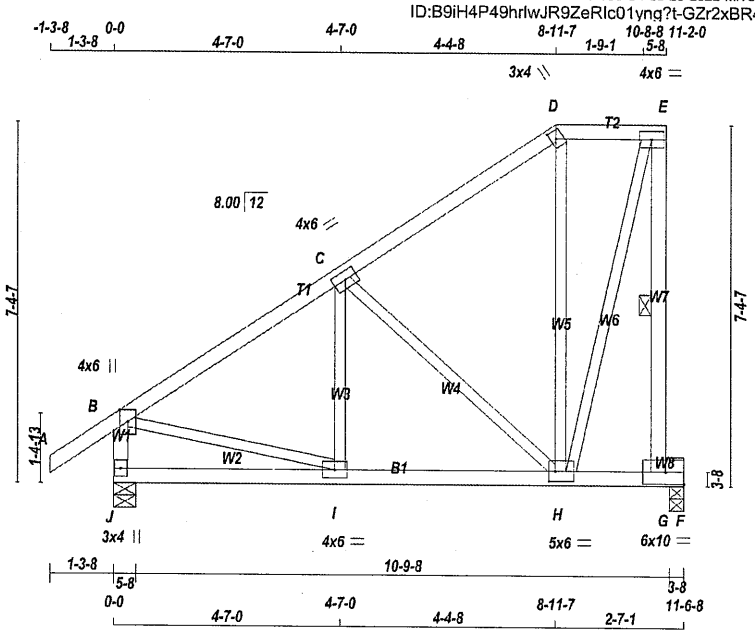

TOTAL WEIGHT = 2 X 62 = 124 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	4.50
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1032	0	1032	0	0	5-8	5-8
F	807	0	807	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	717	546 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
F	563	413 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-746 / 0	-129.6	-129.6	0.46 (1)	6.20
C- D	-333 / 0	-129.6	-129.6	0.44 (1)	6.25
D- E	-240 / 0	-129.6	-129.6	0.11 (1)	6.25
G- E	-949 / 0	0.0	0.0	0.23 (1)	6.25
J- B	-995 / 0	0.0	0.0	0.10 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00
I- H	0 / 653	-18.5	-18.5	0.18 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.41 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-72 / 67	0.02 (4)
C- H	-558 / 0	0.36 (1)
H- D	-193 / 0	0.19 (1)
H- E	0 / 842	0.19 (1)
B- I	0 / 671	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.41/1.00 (G-H:1) , WB=0.36/1.00 (C-H:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040309

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED jlenfold 8/16/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T70	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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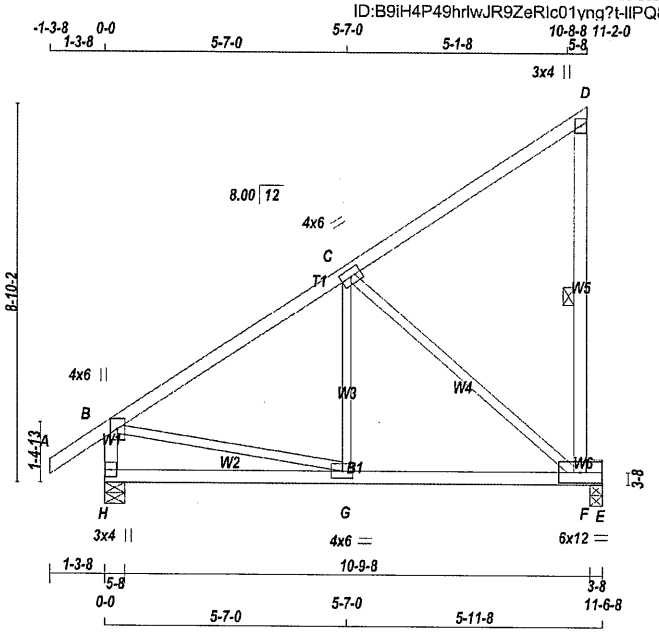

TOTAL WEIGHT = 2 X 55 = 110 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1032	0	1032	0	0	5-8	5-8
E	807	0	807	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	717	546 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
E	563	413 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-725 / 0	-129.6	-129.6	0.73 (1)	5.55
C- D	-49 / 0	-129.6	-129.6	0.68 (1)	6.25
F- D	-273 / 0	0.0	0.0	0.10 (1)	6.25
H- B	-1005 / 0	0.0	0.0	0.10 (1)	7.80
H- G	0 / 0	-18.5	-18.5	0.20 (4)	10.00
G- F	0 / 644	-18.5	-18.5	0.51 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 133	0.05 (4)
C- F	-850 / 0	0.86 (1)
B- G	0 / 656	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 856 (0.16")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.51/1.00 (F-G:1) , WB=0.86/1.00 (C-F:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040310

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED 8/16/2023 Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T71	2	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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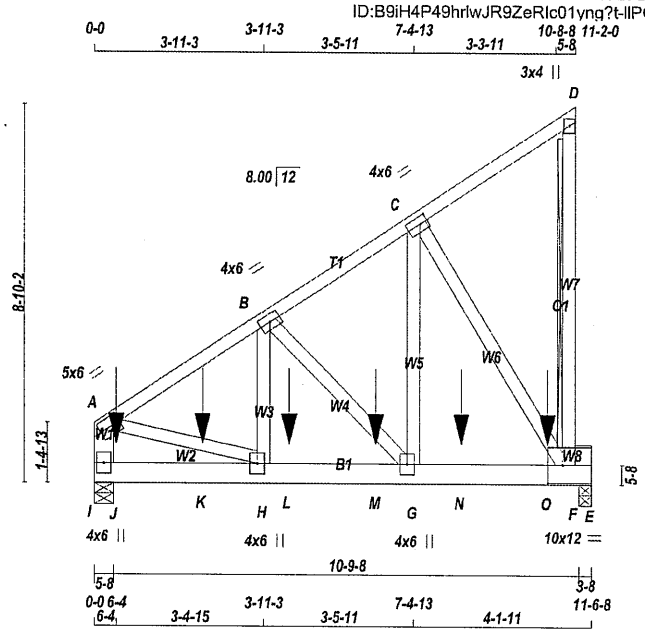

TOTAL WEIGHT = 6 X 74 = 447 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
I- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- E	2	8	SIDE(366.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040311

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	4626	0	4626	0	0	5-8	5-8
E	4262	0	4262	0	0	3-8	3-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	3218	2414 / 0	0 / 0	0 / 0	0 / 0	804 / 0	0 / 0
E	2965	2223 / 0	0 / 0	0 / 0	0 / 0	742 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3976 / 0	-129.6	-129.6	0.11 (1)	5.52
B- C	-2593 / 0	-129.6	-129.6	0.09 (1)	6.25
C- D	-26 / 0	-129.6	-129.6	0.08 (1)	6.25
F- D	-198 / 0	0.0	0.0	0.02 (1)	7.81
I- A	-3280 / 0	0.0	0.0	0.07 (1)	7.81
I- J	0 / 0	-18.5	-18.5	0.20 (1)	10.00
J- K	0 / 0	-18.5	-18.5	0.20 (1)	10.00
K- H	0 / 0	-18.5	-18.5	0.20 (1)	10.00
H- L	0 / 3324	-18.5	-18.5	0.40 (1)	10.00
L- M	0 / 3324	-18.5	-18.5	0.40 (1)	10.00
M- G	0 / 3324	-18.5	-18.5	0.40 (1)	10.00
G- N	0 / 2163	-18.5	-18.5	0.53 (1)	10.00
N- O	0 / 2163	-18.5	-18.5	0.53 (1)	10.00
O- F	0 / 2163	-18.5	-18.5	0.53 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.15 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- H	0 / 3429	0.18 (1)
H- B	0 / 1482	0.08 (1)
B- G	-1677 / 0	0.16 (1)
G- C	0 / 4323	0.23 (1)
C- F	-4037 / 0	0.85 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	6-4	-840	-840	---	FRONT	VERT	TOTAL	---	C1
K	2-6-4	-837	-837	---	FRONT	VERT	TOTAL	---	C1
L	4-6-4	-837	-837	---	FRONT	VERT	TOTAL	---	C1
M	6-6-4	-837	-837	---	FRONT	VERT	TOTAL	---	C1
N	8-6-4	-837	-837	---	FRONT	VERT	TOTAL	---	C1
O	10-6-4	-837	-837	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.53/1.00 (F-G:1) , WB=0.85/1.00 (C-F:1) , SSI=0.43/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.50 (H) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T71	2	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:B9iH4P49hrlwJR9ZeRIc01yng?t-DyzoMtTK5kfLY9xZ9e662g38QWu0r0ceA8hrEKzUTJI

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit #

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	10.0	12.0	5.00	4.25
G	BMWW+t	MT20	4.0	6.0		
H	BMWW+t	MT20	4.0	6.0		
I	BMV1+p	MT20	4.0	6.0		

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040311

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	G72	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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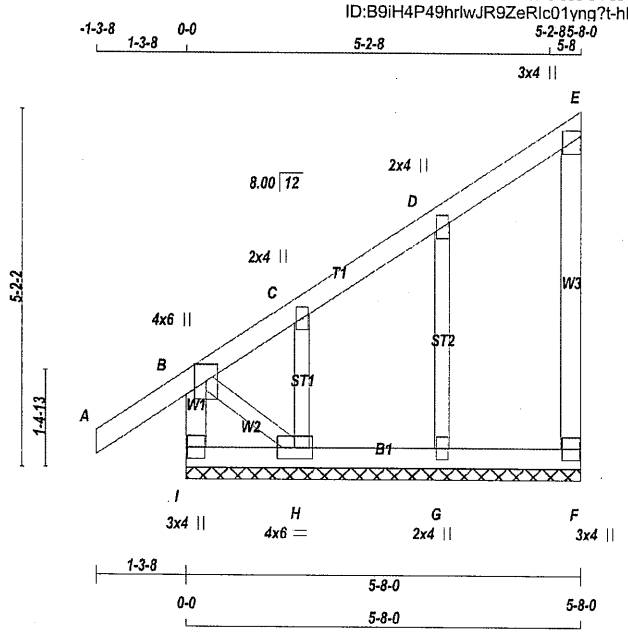

TOTAL WEIGHT = 2 X 28 = 56 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMWW1-t	MT20	2.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
I- B	-366 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-53 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	0 / 4	-129.6	-129.6	0.08 (1)	10.00
D- E	-16 / 0	-129.6	-129.6	0.08 (1)	6.25
F- E	-106 / 0	0.0	0.0	0.05 (1)	7.81
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00
H- G	0 / 7	-18.5	-18.5	0.02 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- D	-312 / 0	0.07 (1)
H- C	-137 / 0	0.02 (1)
B- H	0 / 15	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.02/1.00 (G-H:4) , WB=0.07/1.00 (D-G:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040271

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
jlandolo
4/04/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	T73	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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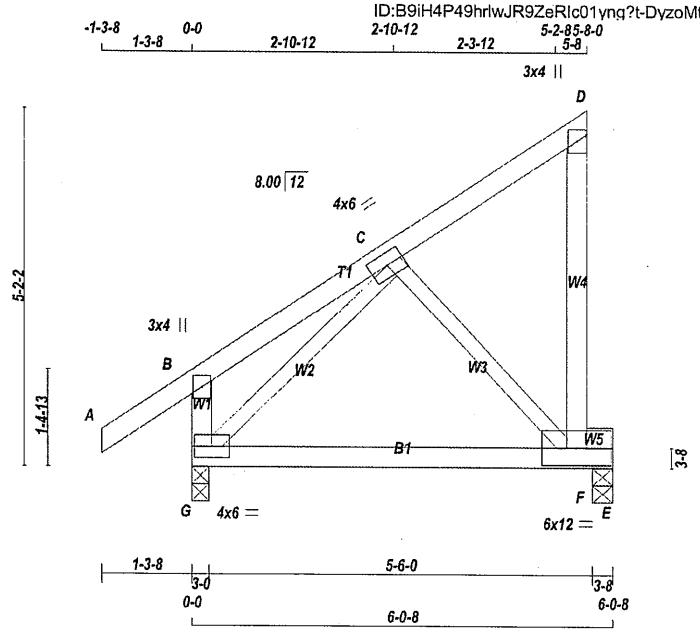

TOTAL WEIGHT = 6 X 29 = 176 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	624	0	624	0	0	3-0	3-0
E	400	0	400	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	432	335 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
E	280	203 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
B- C	0 / 25	-129.6	-129.6	0.17 (1)	10.00	
C- D	-24 / 0	-129.6	-129.6	0.13 (1)	6.25	
F- D	-144 / 0	0.0	0.0	0.06 (1)	7.81	
G- B	-313 / 0	0.0	0.0	0.03 (1)	7.81	
G- F	0 / 220	-18.5	-18.5	0.26 (1)	10.00	
F- E	0 / 0	-18.5	-18.5	0.08 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C- F	-313 / 0	0.08 (1)
G- C	-320 / 0	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 582 (0.12")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.26/1.00 (F-G:1) , WB=0.09/1.00 (C-G:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (G) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040312

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T90	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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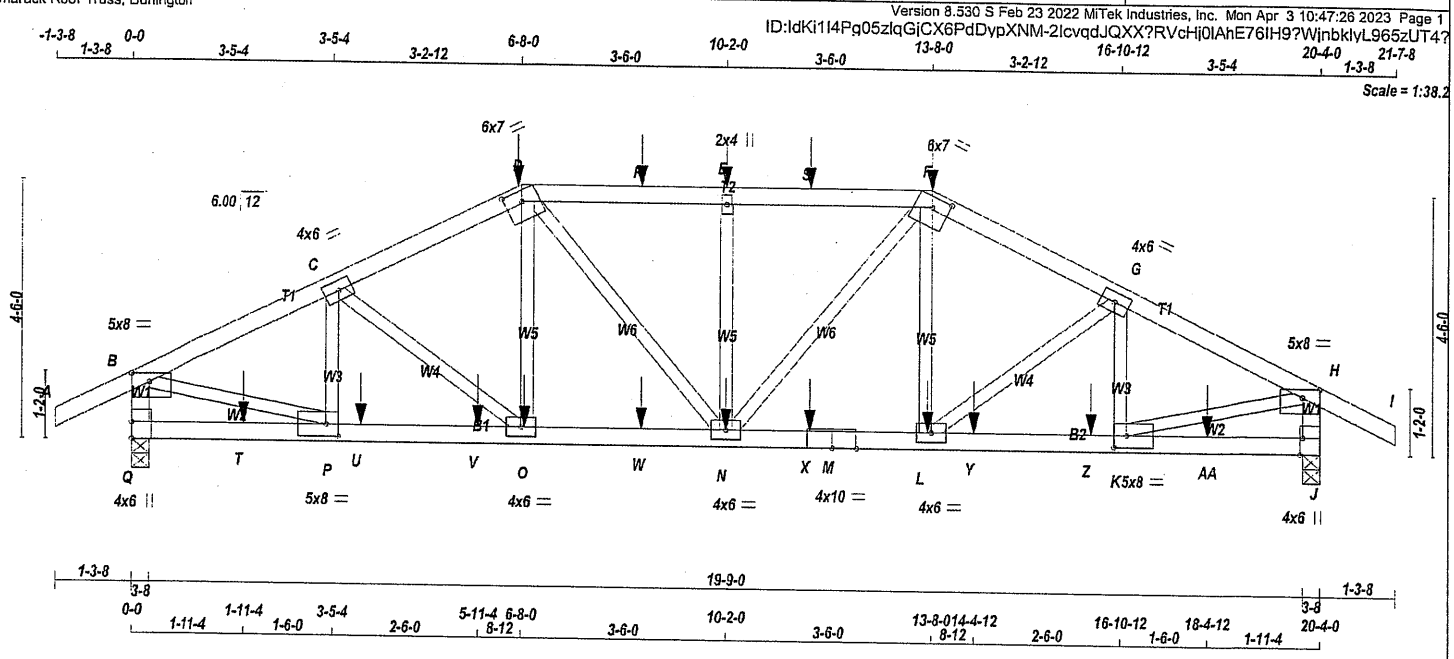

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-h	MT20	6.0	7.0	2.25	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-h	MT20	6.0	7.0	2.25	3.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-p	MT20	5.0	8.0	1.75	3.50
J	BMV1+t	MT20	4.0	6.0	Edge	0.50
K	BMWW-t	MT20	5.0	8.0	2.50	2.50
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	4.0	10.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.50	2.50
Q	BMV1+t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

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C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040330

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2969	0	2969	0	0	3-8	3-8
J	2969	0	2969	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2070	1527 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0
J	2070	1527 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.19 (1)	10.00
B- C	-3802 / 0	-129.6	-129.6	0.42 (1)	3.22
C- D	-4046 / 0	-129.6	-129.6	0.44 (1)	3.09
D- R	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67
R- E	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67
E- S	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67
S- F	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67
F- G	-4046 / 0	-129.6	-129.6	0.44 (1)	3.09
G- H	-3802 / 0	-129.6	-129.6	0.42 (1)	3.22
H- I	0 / 39	-129.6	-129.6	0.19 (1)	10.00
Q- B	-2921 / 0	0.0	0.0	0.32 (1)	4.98
J- H	-2921 / 0	0.0	0.0	0.32 (1)	4.98
Q- T	0 / 0	-18.5	-18.5	0.10 (4)	10.00
T- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00
P- U	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
U- V	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
V- O	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
O- W	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
W- N	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
N- X	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
X- M	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
M- L	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
L- Y	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
Y- Z	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
Z- K	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
K-AA	0 / 0	-18.5	-18.5	0.10 (4)	10.00
AA- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- D	0 / 143	0.05 (4)
L- F	0 / 143	0.05 (4)
N- E	-1021 / 0	0.32 (1)
N- F	0 / 737	0.18 (1)
D- N	0 / 737	0.18 (1)
C- O	0 / 231	0.06 (1)
P- C	-754 / 0	0.14 (1)
B- P	0 / 3520	0.87 (1)
L- G	0 / 231	0.06 (1)
K- G	-754 / 0	0.14 (1)
K- H	0 / 3520	0.87 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-8-0	-561	-561	—	BACK	VERT	TOTAL	—	C1
E	10-2-0	-133	-133	—	BACK	VERT	TOTAL	—	C1
F	13-8-0	-561	-561	—	BACK	VERT	TOTAL	—	C1
L	13-7-4	-26	-26	—	BACK	VERT	TOTAL	—	C1
N	10-2-0	-26	-26	—	BACK	VERT	TOTAL	—	C1
O	6-8-12	-26	-26	—	BACK	VERT	TOTAL	—	C1
R	8-8-12	-133	-133	—	BACK	VERT	TOTAL	—	C1
S	11-7-4	-133	-133	—	BACK	VERT	TOTAL	—	C1

TOTAL WEIGHT = 2 X 86 = 173 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.69/1.00 (D-E:1) , BC=0.70/1.00 (L-N:1) , WB=0.87/1.00 (B-P:1) , SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.81 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T90	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOWN OF INNISFIL
Block 5
REVIEWED
permission
4/16/2023
Building Permit Plan

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	1-11-4	-25	-25	—	BACK	VERT	TOTAL	—	C1
U	3-11-4	-26	-26	—	BACK	VERT	TOTAL	—	C1
V	5-11-4	-26	-26	—	BACK	VERT	TOTAL	—	C1
W	8-8-12	-26	-26	—	BACK	VERT	TOTAL	—	C1
X	11-7-4	-26	-26	—	BACK	VERT	TOTAL	—	C1
Y	14-4-12	-26	-26	—	BACK	VERT	TOTAL	—	C1
Z	16-4-12	-26	-26	—	BACK	VERT	TOTAL	—	C1
AA	18-4-12	-25	-25	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040330

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T91	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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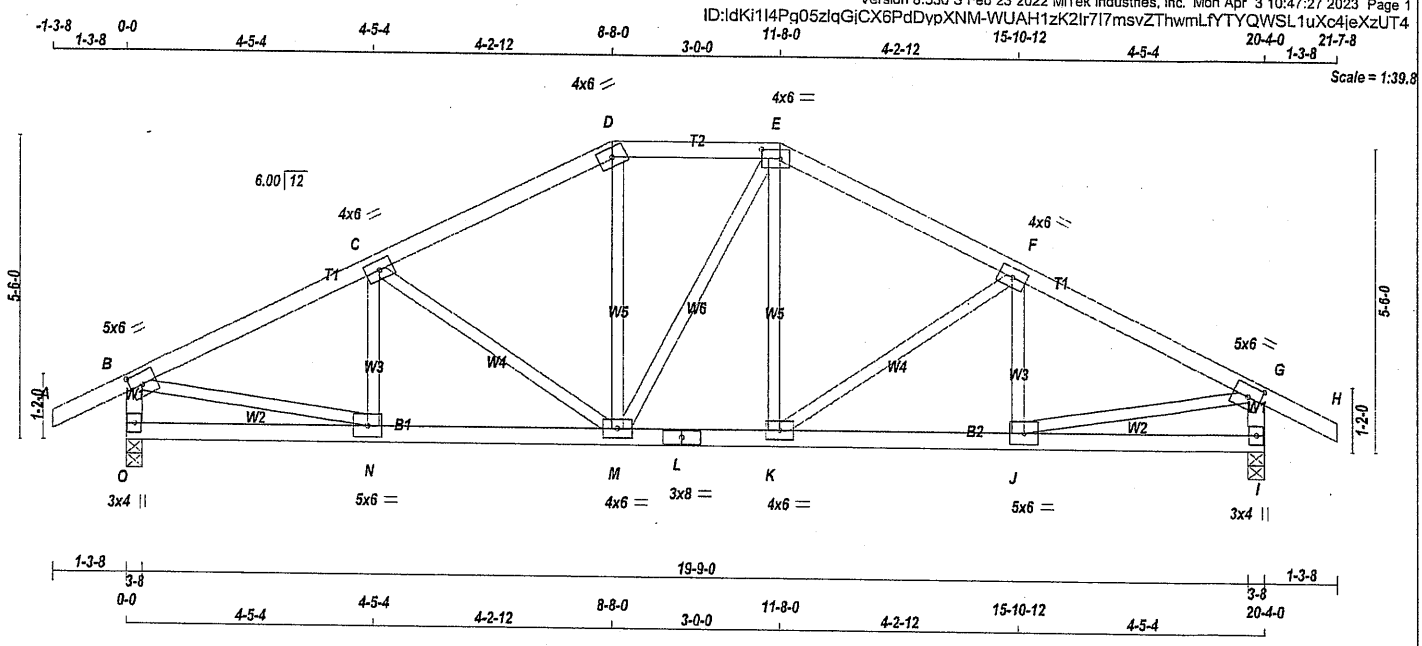

TOTAL WEIGHT = 2 X 86 = 172 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TTWW-I	MT20	4.0	6.0	2.00	4.00
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1682	0	1682	0	0	3-8	3-8
I	1682	0	1682	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
I	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-1977 / 0	-129.6	-129.6	0.35 (1)	4.46
C- D	-1640 / 0	-129.6	-129.6	0.33 (1)	4.83
D- E	-1445 / 0	-129.6	-129.6	0.17 (1)	5.27
E- F	-1638 / 0	-129.6	-129.6	0.33 (1)	4.83
F- G	-1977 / 0	-129.6	-129.6	0.35 (1)	4.46
G- H	0 / 39	-129.6	-129.6	0.17 (1)	10.00
O- B	-1644 / 0	0.0	0.0	0.17 (1)	6.46
I- G	-1644 / 0	0.0	0.0	0.17 (1)	6.46
O- N	0 / 0	-18.5	-18.5	0.08 (4)	10.00
N- M	0 / 1793	-18.5	-18.5	0.32 (1)	10.00
M- L	0 / 1444	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 1444	-18.5	-18.5	0.26 (1)	10.00
K- J	0 / 1794	-18.5	-18.5	0.33 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- D	0 / 320	0.07 (1)
M- E	0 / 3	0.00 (1)
K- E	0 / 317	0.07 (1)
C- M	-434 / 0	0.20 (1)
N- C	-265 / 15	0.05 (1)
B- N	0 / 1826	0.41 (1)
K- F	-437 / 0	0.21 (1)
J- F	-263 / 16	0.05 (1)
J- G	0 / 1827	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.35/1.00 (F-G:1) , BC=0.33/1.00 (J-K:1) , WB=0.41/1.00 (G-J:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.50 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040331

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	G92	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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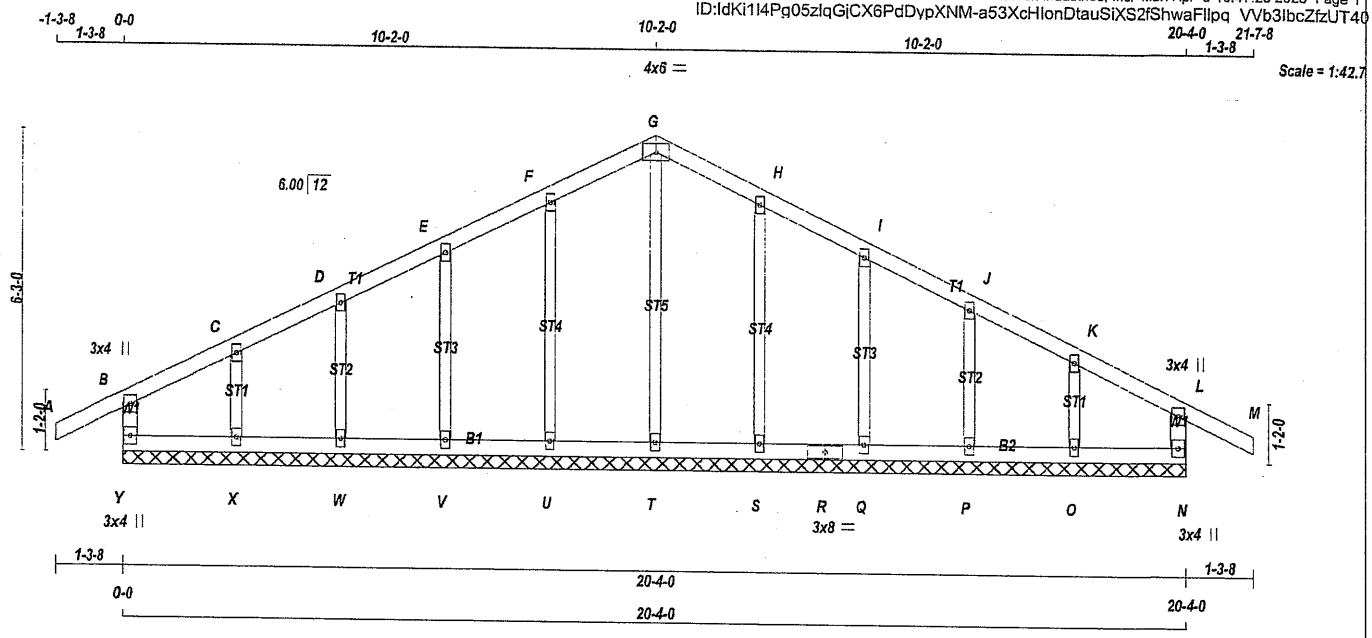

TOTAL WEIGHT = 2 X 80 = 159 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	6.0		
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, Q, S, T, U, V, W, X						
O	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	8.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y-B	-332 / 0	0.0	0.0		0.02 (1)	7.81
A-B	0 / 39	-129.6	-129.6		0.17 (1)	10.00
B-C	-39 / 0	-129.6	-129.6		0.07 (1)	6.25
C-D	-28 / 0	-129.6	-129.6		0.06 (1)	6.25
D-E	-22 / 0	-129.6	-129.6		0.06 (1)	6.25
E-F	-17 / 0	-129.6	-129.6		0.06 (1)	6.25
F-G	-13 / 0	-129.6	-129.6		0.06 (1)	6.25
G-H	-13 / 0	-129.6	-129.6		0.06 (1)	6.25
H-I	-17 / 0	-129.6	-129.6		0.06 (1)	6.25
I-J	-22 / 0	-129.6	-129.6		0.06 (1)	6.25
J-K	-28 / 0	-129.6	-129.6		0.06 (1)	6.25
K-L	-39 / 0	-129.6	-129.6		0.07 (1)	6.25
L-M	0 / 39	-129.6	-129.6		0.17 (1)	10.00
N-L	-332 / 0	0.0	0.0		0.02 (1)	7.81
Y-X	0 / 34	-18.5	-18.5		0.02 (1)	10.00
X-W	0 / 25	-18.5	-18.5		0.02 (4)	10.00
W-V	0 / 19	-18.5	-18.5		0.02 (4)	10.00
V-U	0 / 15	-18.5	-18.5		0.02 (4)	10.00
U-T	0 / 12	-18.5	-18.5		0.02 (4)	10.00
T-S	0 / 12	-18.5	-18.5		0.02 (4)	10.00
S-R	0 / 15	-18.5	-18.5		0.02 (4)	10.00
R-Q	0 / 15	-18.5	-18.5		0.02 (4)	10.00
Q-P	0 / 19	-18.5	-18.5		0.02 (4)	10.00
P-O	0 / 25	-18.5	-18.5		0.02 (4)	10.00
O-N	0 / 34	-18.5	-18.5		0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-G	-257 / 0	0.16 (1)
U-F	-262 / 0	0.11 (1)
V-E	-256 / 0	0.07 (1)
W-D	-256 / 0	0.05 (1)
X-C	-259 / 0	0.04 (1)
S-H	-262 / 0	0.11 (1)
Q-I	-256 / 0	0.07 (1)
P-J	-256 / 0	0.05 (1)
O-K	-259 / 0	0.04 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (L-M:1) , BC=0.02/1.00 (N-O:1) , WB=0.16/1.00 (G-T:1) , SSI=0.12/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (I) (INPUT = 0.90)
JSI METAL= 0.11 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040329

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T93	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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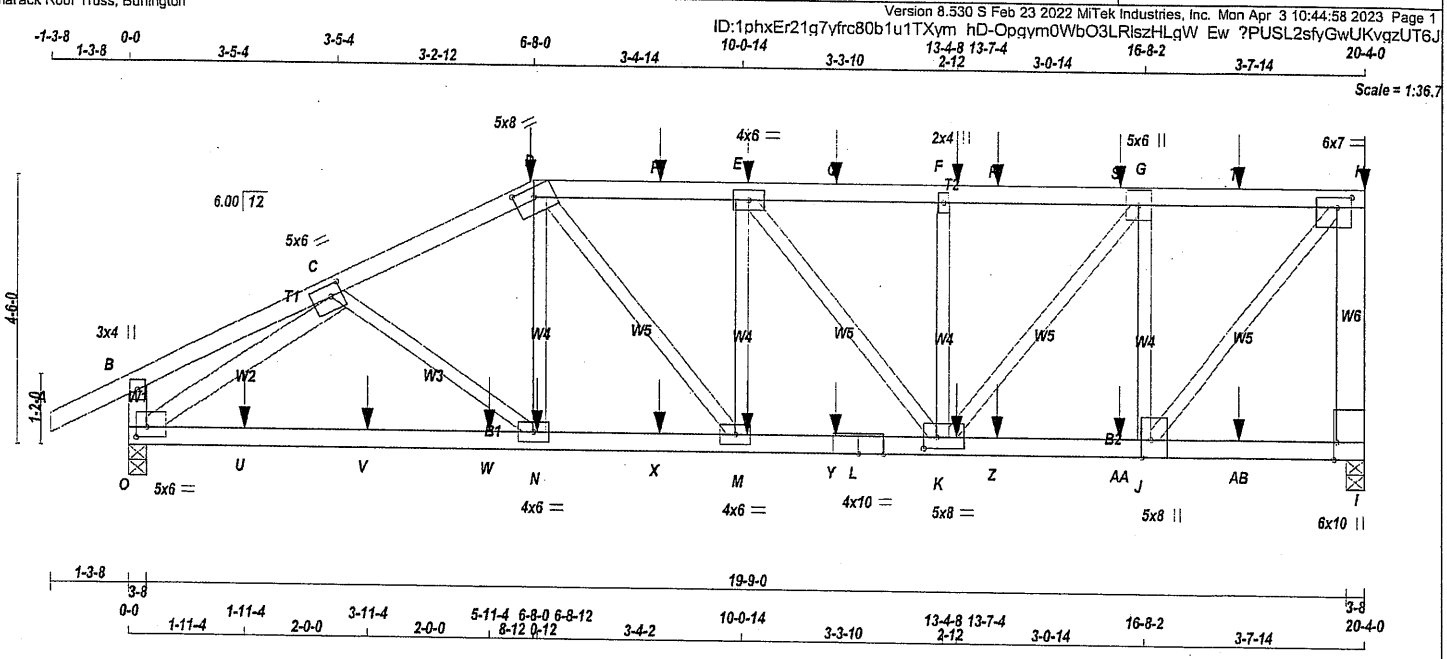

TOTAL WEIGHT = 2 X 94 = 187 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW-h	MT20	5.0	8.0	2.00	4.00
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	5.0	6.0		
H	TMVW-t	MT20	6.0	7.0	2.00	3.00
I	BMV1+t	MT20	6.0	10.0	Edge	0.50
J	BMWW+t	MT20	5.0	8.0	Edge	1.75
K	BMWWW-t	MT20	5.0	8.0	2.25	2.50
L	BS-t	MT20	4.0	10.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	5.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040327

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	3091	0	3091	0	0	3-8	3-8
O	2885	0	2885	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2152	1606 / 0	0 / 0	0 / 0	0 / 0	546 / 0	0 / 0
O	2012	1480 / 0	0 / 0	0 / 0	0 / 0	533 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.19 (1)	10.00
B- C	0 / 17	-129.6	-129.6	0.20 (1)	10.00
C- D	-3919 / 0	-129.6	-129.6	0.43 (1)	3.16
D- P	-3855 / 0	-129.6	-129.6	0.60 (1)	2.92
P- E	-3855 / 0	-129.6	-129.6	0.60 (1)	2.92
E- Q	-3403 / 0	-129.6	-129.6	0.54 (1)	3.17
Q- F	-3403 / 0	-129.6	-129.6	0.54 (1)	3.17
F- R	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
R- S	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
S- G	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
G- T	-2216 / 0	-129.6	-129.6	0.50 (1)	3.94
T- H	-2216 / 0	-129.6	-129.6	0.50 (1)	3.94
I- H	-3053 / 0	0.0	0.0	0.64 (1)	5.99
O- B	-355 / 0	0.0	0.0	0.04 (1)	7.81
O- U	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
U- V	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
V- W	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
W- N	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
N- X	0 / 3488	-18.5	-18.5	0.80 (1)	10.00
X- M	0 / 3488	-18.5	-18.5	0.80 (1)	10.00
M- Y	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
Y- L	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
L- K	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
K- Z	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
Z-AA	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
AA- J	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
J-AB	0 / 0	-18.5	-18.5	0.10 (4)	10.00
AB- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 310	0.08 (1)
N- D	0 / 206	0.08 (4)
O- C	-4057 / 0	0.89 (1)
J- H	0 / 3379	0.84 (1)
D- M	0 / 583	0.14 (1)
J- G	-2411 / 0	0.75 (1)
M- E	-367 / 9	0.11 (1)
K- G	0 / 1922	0.48 (1)
E- K	-733 / 0	0.35 (1)
K- F	-808 / 0	0.25 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-8-0	-561	-561	—	FRONT	VERT	TOTAL	—	C1
E	10-2-0	-133	-133	—	FRONT	VERT	TOTAL	—	C1
F	13-7-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
H	20-4-0	-174	-174	—	FRONT	VERT	TOTAL	—	C1
K	13-7-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
M	10-2-0	-26	-26	—	FRONT	VERT	TOTAL	—	C1
N	6-8-12	-26	-26	—	FRONT	VERT	TOTAL	—	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.64/1.00 (H-I:1) , BC=0.80/1.00 (M-N:1) , WB=0.89/1.00 (C-O:1) , SSI=0.41/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.94 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T93	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	8-8-12	-133	-133	—	FRONT	VERT	TOTAL	—	C1
Q	11-7-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
R	14-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
S	16-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
T	18-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
U	1-11-4	-25	-25	—	FRONT	VERT	TOTAL	—	C1
V	3-11-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
W	5-11-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
X	8-8-12	-26	-26	—	FRONT	VERT	TOTAL	—	C1
Y	11-7-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
Z	14-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
AA	16-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
AB	18-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040327

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T94	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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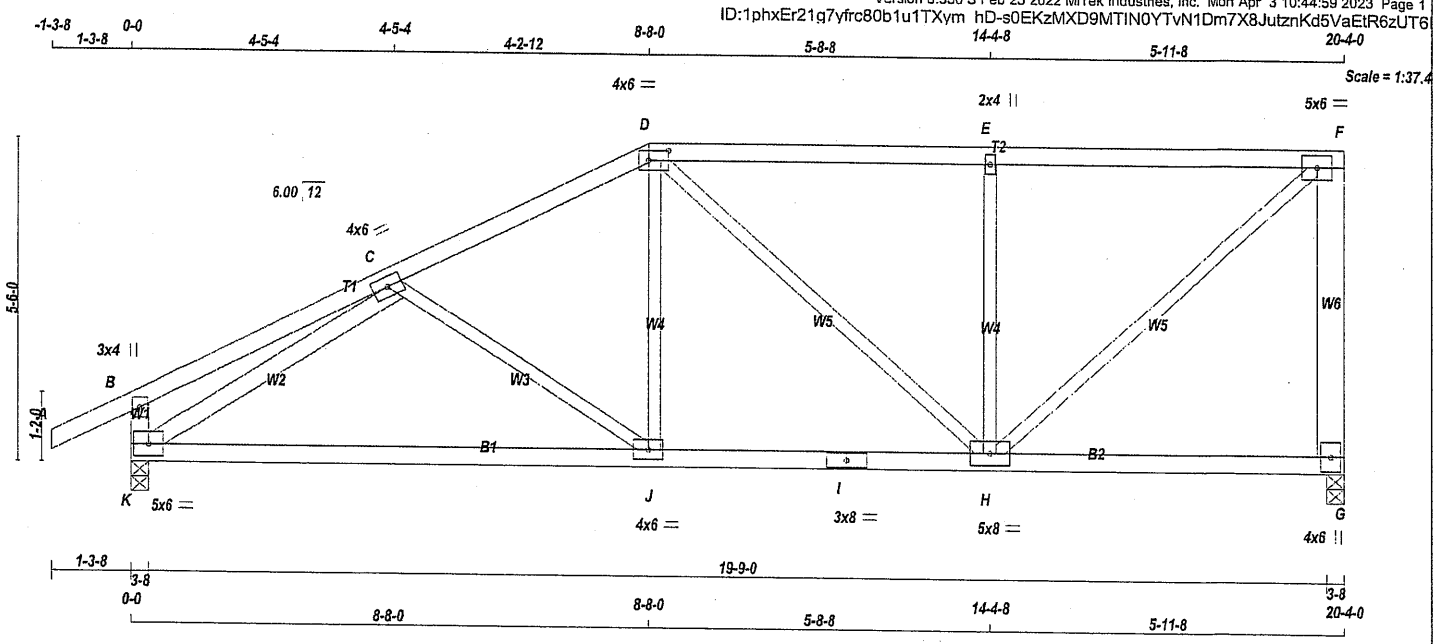

TOTAL WEIGHT = 2 X 89 = 177 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-I	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	4.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1506	0	1506	0	0	3-8	3-8
K	1682	0	1682	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1050	777 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	0 / 29	-129.6	-129.6	0.38 (1)	10.00
C- D	-1659 / 0	-129.6	-129.6	0.44 (1)	4.61
D- E	-1327 / 0	-129.6	-129.6	0.82 (1)	4.13
E- F	-1327 / 0	-129.6	-129.6	0.83 (1)	4.13
G- F	-1457 / 0	0.0	0.0	0.50 (1)	7.81
K- B	-395 / 0	0.0	0.0	0.04 (1)	7.81
K- J	0 / 1750	-18.5	-18.5	0.45 (1)	10.00
J- I	0 / 1464	-18.5	-18.5	0.39 (1)	10.00
I- H	0 / 1464	-18.5	-18.5	0.39 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-356 / 0	0.18 (1)
J- D	0 / 363	0.08 (1)
K- C	-2133 / 0	0.72 (1)
H- F	0 / 1763	0.40 (1)
D- H	-185 / 0	0.22 (1)
H- E	-938 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.83/1.00 (E-F:1) , BC=0.45/1.00 (J-K:1) , WB=0.72/1.00 (C-K:1) , SSI=0.37/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.44 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040328

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	G95	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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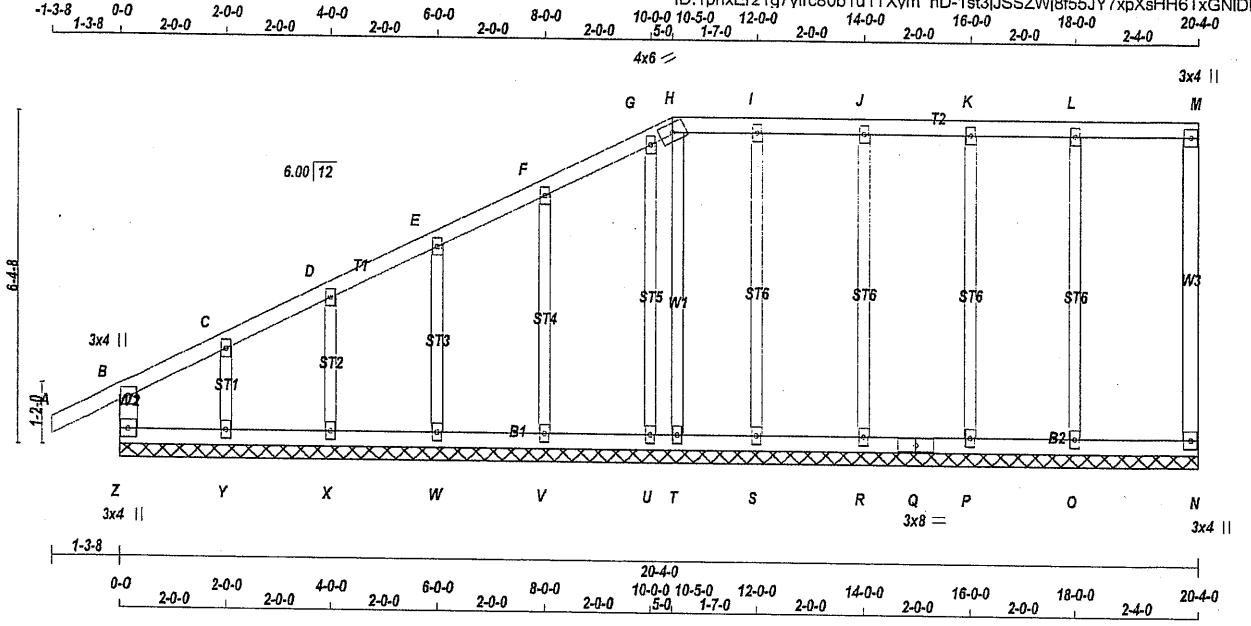

TOTAL WEIGHT = 2 X 95 = 190 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Z - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
Z - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, L						
C	TMW+w	MT20	2.0	4.0		
H	TTW-h	MT20	4.0	6.0		
M	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, R, S, T, U, V, W, X, Y						
O	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
Z	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
Z- B	-317 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-36 / 0	-129.6	-129.6	0.06 (1)	6.25
C- D	-25 / 0	-129.6	-129.6	0.06 (1)	6.25
D- E	-19 / 0	-129.6	-129.6	0.06 (1)	6.25
E- F	-14 / 0	-129.6	-129.6	0.06 (1)	6.25
F- G	-13 / 0	-129.6	-129.6	0.06 (1)	6.25
G- H	-11 / 0	-129.6	-129.6	0.04 (1)	6.25
H- I	-5 / 0	-129.6	-129.6	0.04 (1)	10.00
I- J	-5 / 0	-129.6	-129.6	0.06 (1)	10.00
J- K	-5 / 0	-129.6	-129.6	0.06 (1)	10.00
K- L	-5 / 0	-129.6	-129.6	0.08 (1)	10.00
L- M	-5 / 0	-129.6	-129.6	0.08 (1)	10.00
N- M	-136 / 0	0.0	0.0	0.04 (1)	7.81
Z- Y	0 / 32	-18.5	-18.5	0.02 (1)	10.00
Y- X	0 / 23	-18.5	-18.5	0.02 (4)	10.00
X- W	0 / 17	-18.5	-18.5	0.02 (4)	10.00
W- V	0 / 13	-18.5	-18.5	0.02 (4)	10.00
V- U	0 / 9	-18.5	-18.5	0.02 (4)	10.00
U- T	0 / 7	-18.5	-18.5	0.01 (4)	10.00
T- S	0 / 5	-18.5	-18.5	0.01 (4)	10.00
S- R	0 / 5	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 5	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 5	-18.5	-18.5	0.02 (4)	10.00
P- O	0 / 5	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 5	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-249 / 0	0.04 (1)
X- D	-258 / 0	0.05 (1)
W- E	-256 / 0	0.07 (1)
V- F	-266 / 0	0.11 (1)
U- G	-159 / 0	0.10 (1)
S- I	-230 / 0	0.15 (1)
R- J	-267 / 0	0.17 (1)
P- K	-249 / 0	0.16 (1)
O- L	-304 / 0	0.20 (1)
T- H	-120 / 0	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.02/1.00 (O-P:4) , WB=0.20/1.00 (L-O:1) , SSI=0.13/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (E) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040319

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T96	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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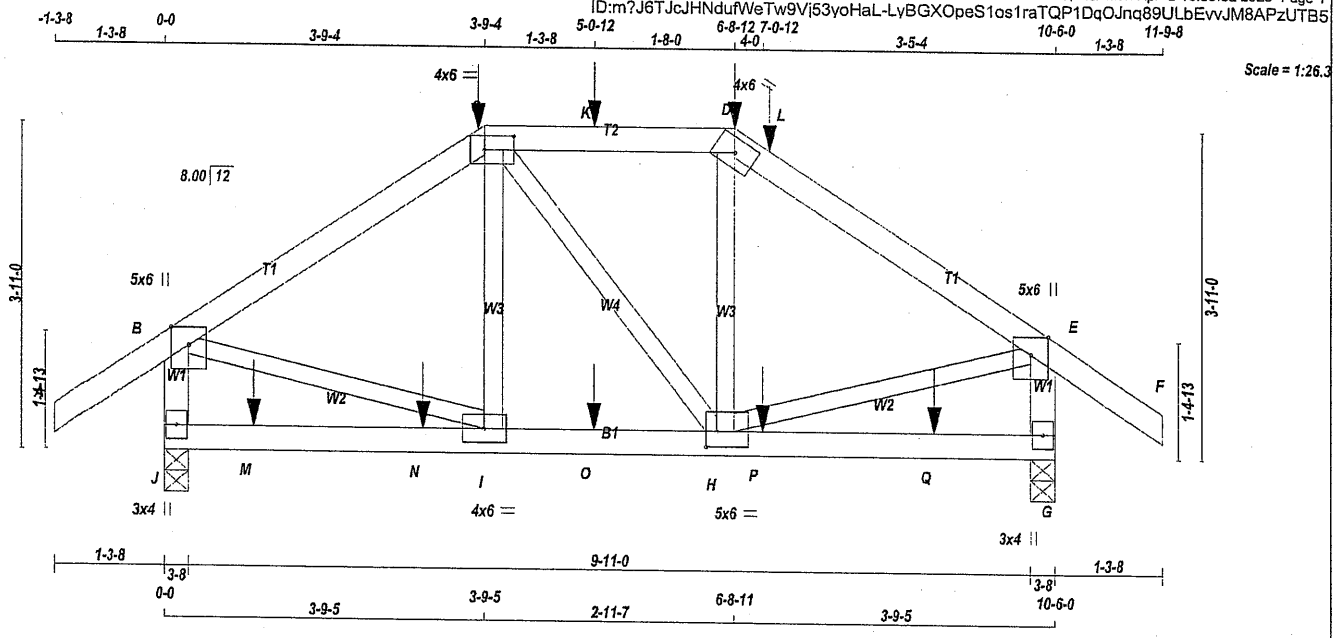

TOTAL WEIGHT = 47 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040316

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1516	0	1516	0	0	3-8	3-8
G	1569	0	1569	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1053	800 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
G	1088	834 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00
B- C	-1323 / 0	-129.6	-129.6	0.39 (1)	5.09
C- K	-1133 / 0	-129.6	-129.6	0.38 (1)	5.40
K- D	-1133 / 0	-129.6	-129.6	0.38 (1)	5.40
D- L	-1338 / 0	-129.6	-129.6	0.44 (1)	4.98
L- E	-1338 / 0	-129.6	-129.6	0.44 (1)	4.98
E- F	0 / 49	-129.6	-129.6	0.19 (1)	10.00
J- B	-1465 / 0	0.0	0.0	0.17 (1)	6.72
G- E	-1520 / 0	0.0	0.0	0.17 (1)	6.62
J- M	0 / 0	-18.5	-18.5	0.11 (4)	10.00
M- N	0 / 0	-18.5	-18.5	0.11 (4)	10.00
N- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00
I- O	0 / 1093	-18.5	-18.5	0.24 (1)	10.00
O- H	0 / 1093	-18.5	-18.5	0.24 (1)	10.00
H- P	0 / 0	-18.5	-18.5	0.12 (4)	10.00
P- Q	0 / 0	-18.5	-18.5	0.12 (4)	10.00
Q- G	0 / 0	-18.5	-18.5	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-198 / 50	0.05 (1)
C- H	0 / 66	0.02 (1)
H- D	-261 / 43	0.07 (1)
B- I	0 / 1140	0.28 (1)
H- E	0 / 1188	0.29 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-9-4	-34	-34	---	FRONT	VERT	DEAD	---	C1
C	3-9-4	-217	-217	---	FRONT	VERT	SNOW	---	C1
D	6-8-12	-34	-34	---	FRONT	VERT	DEAD	---	C1
D	6-8-12	-217	-217	---	FRONT	VERT	SNOW	---	C1
K	5-0-12	-94	-94	---	BACK	VERT	TOTAL	---	C1
L	7-0-12	-123	-123	---	BACK	VERT	TOTAL	---	C1
M	1-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
N	3-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
O	5-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
P	7-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
Q	9-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.24/1.00 (H-I:1) , WB=0.29/1.00 (E-H:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.60 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T97	7	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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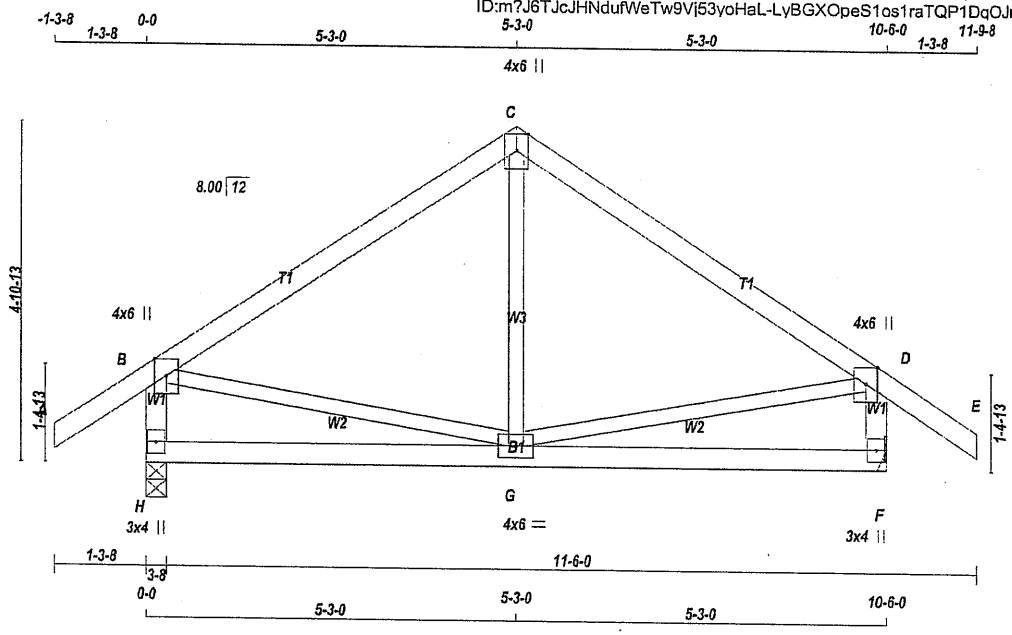

TOTAL WEIGHT = 7 X 44 = 308 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	956	0	956	0	0	3-8	3-8
F	956	0	956	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	663	506 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
F	663	506 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-551 / 0	-129.6	-129.6	0.46 (1)	6.25
C- D	-551 / 0	-129.6	-129.6	0.46 (1)	6.25
D- E	0 / 49	-129.6	-129.6	0.17 (1)	10.00
H- B	-918 / 0	0.0	0.0	0.09 (1)	7.81
F- D	-918 / 0	0.0	0.0	0.09 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-70 / 82	0.03 (4)
B- G	0 / 468	0.11 (1)
G- D	0 / 468	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.46/1.00 (C-D:1) , BC=0.14/1.00 (F-G:4) , WB=0.11/1.00 (D-G:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (G) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040317

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED Building Permit Plan 4/06/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G97	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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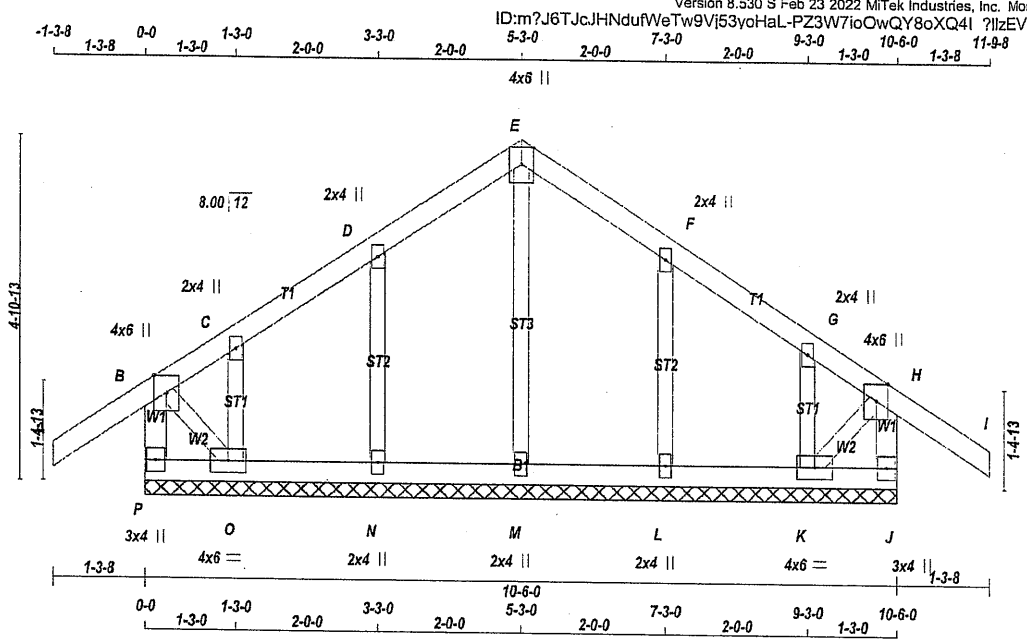

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-70 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	0 / 0	-129.6	-129.6	0.08 (1)	10.00
D- E	-20 / 0	-129.6	-129.6	0.08 (1)	6.25
E- F	-20 / 0	-129.6	-129.6	0.08 (1)	6.25
F- G	0 / 0	-129.6	-129.6	0.08 (1)	10.00
G- H	-70 / 0	-129.6	-129.6	0.17 (1)	6.25
H- I	0 / 49	-129.6	-129.6	0.17 (1)	10.00
P- B	-371 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-371 / 0	0.0	0.0	0.04 (1)	7.81
P- O	0 / 0	-18.5	-18.5	0.01 (4)	10.00
O- N	0 / 11	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 3	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 3	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 11	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-206 / 0	0.07 (1)
N- D	-315 / 0	0.07 (1)
O- C	-83 / 0	0.01 (1)
L- F	-315 / 0	0.07 (1)
K- G	-83 / 0	0.01 (1)
B- O	0 / 20	0.00 (1)
K- H	0 / 20	0.00 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOTAL WEIGHT = 46 lb
[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (H-I:1) , BC=0.02/1.00 (N-O:4) , WB=0.07/1.00 (E-M:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040313

TOWN OF INNISFIL Block 5 REVIEWED 4/16/2023 Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T98	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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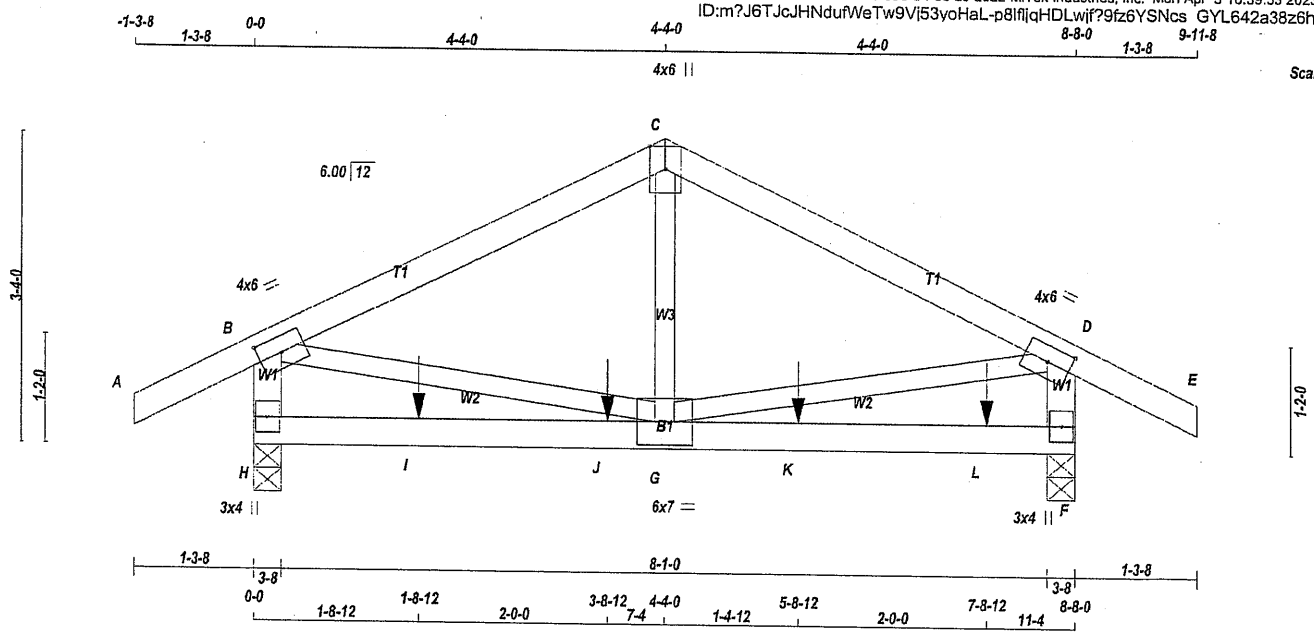

TOTAL WEIGHT = 2 X 35 = 70 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
H- B	1	12	TOP
F- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- F	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040318

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2521	0	2521	0	0	3-8	3-8
F	2864	0	2864	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1746	1355 / 0	0 / 0	0 / 0	0 / 0	391 / 0	0 / 0
F	1983	1540 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.09 (1)	10.00
B- C	-2552 / 0	-129.6	-129.6	0.27 (1)	5.36
C- D	-2552 / 0	-129.6	-129.6	0.27 (1)	5.36
D- E	0 / 39	-129.6	-129.6	0.09 (1)	10.00
H- B	-2050 / 0	0.0	0.0	0.11 (1)	7.72
F- D	-2050 / 0	0.0	0.0	0.11 (1)	7.72
H- I	0 / 0	-18.5	-18.5	0.85 (1)	10.00
I- J	0 / 0	-18.5	-18.5	0.85 (1)	10.00
J- G	0 / 0	-18.5	-18.5	0.85 (1)	10.00
G- K	0 / 0	-18.5	-18.5	0.85 (1)	10.00
K- L	0 / 0	-18.5	-18.5	0.85 (1)	10.00
L- F	0 / 0	-18.5	-18.5	0.85 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 1721	0.21 (1)
B- G	0 / 2327	0.29 (1)
G- D	0 / 2327	0.29 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	1-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
J	3-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
K	5-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
L	7-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 837 (0.12")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.85/1.00 (G-H:1) , WB=0.29/1.00 (B-G:1) , SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (G) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
404995	T98	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	6.0	2.00	3.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	6.0	7.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040318

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G98	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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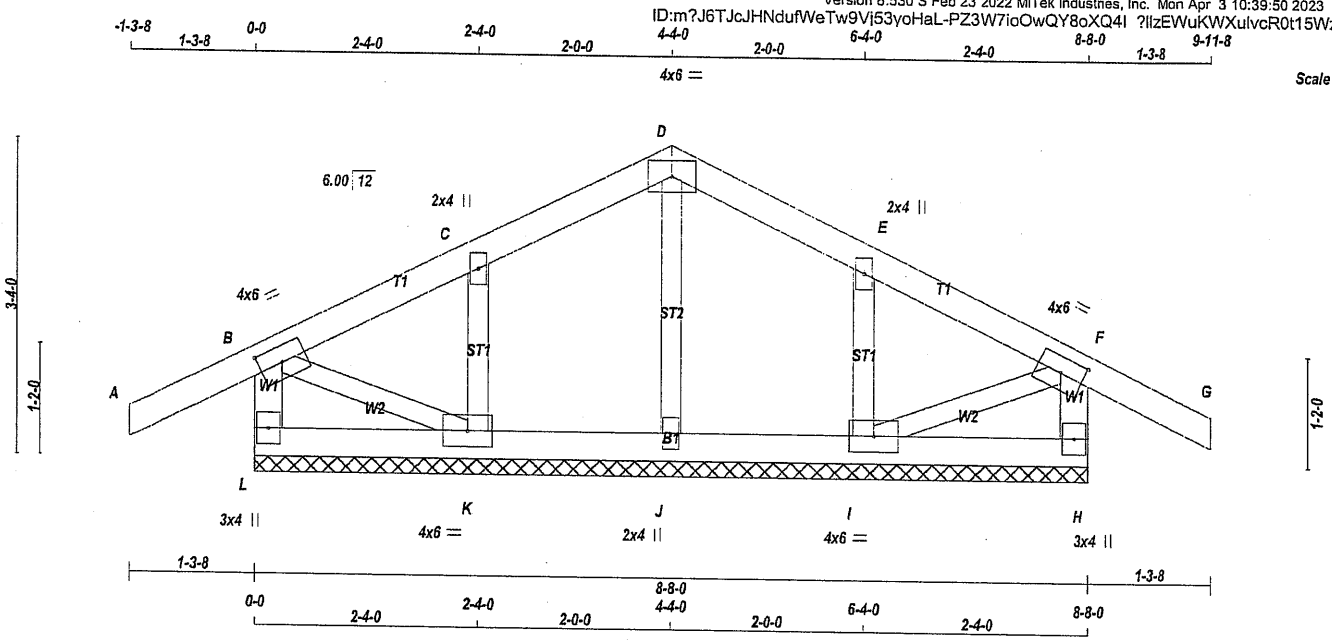

TOTAL WEIGHT = 2 X 35 = 69 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
L- B	1	12	TOP
H- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- H	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040314

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.08 (1)	10.00
B- C	-6 / 0	-129.6	-129.6	0.05 (1)	10.00
C- D	-25 / 0	-129.6	-129.6	0.05 (1)	6.25
D- E	-25 / 0	-129.6	-129.6	0.05 (1)	6.25
E- F	-6 / 0	-129.6	-129.6	0.05 (1)	10.00
F- G	0 / 39	-129.6	-129.6	0.08 (1)	10.00
L- B	-306 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-306 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 0	-18.5	-18.5	0.01 (4)	10.00
K- J	0 / 7	-18.5	-18.5	0.01 (4)	10.00
J- I	0 / 7	-18.5	-18.5	0.01 (4)	10.00
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-188 / 0	0.02 (1)
K- C	-345 / 0	0.03 (1)
I- E	-345 / 0	0.03 (1)
B- K	0 / 20	0.00 (1)
I- F	0 / 20	0.00 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (F-G:1) , BC=0.01/1.00 (H-I:4) , WB=0.03/1.00 (E-I:1) , SSI=0.06/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (C) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G98	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:39:51 2023 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	BMWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040314

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	J1	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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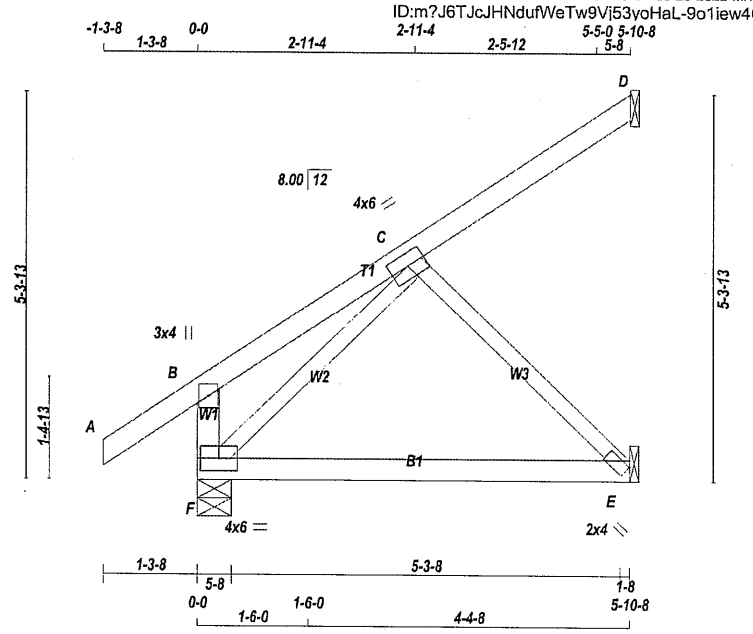

TOTAL WEIGHT = 8 X 24 = 193 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
E	BMW1+w	MT20	2.0	4.0	Edge	0.50
F	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	606	0	606	0	0	5-8	5-8
D	154	0	154	0	0	1-8	1-8
E	286	0	286	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	419	326 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
D	105	91 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
E	201	137 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-313 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	0 / 26	-129.6	-129.6	0.19 (1)	10.00
C- D	-24 / 0	-129.6	-129.6	0.18 (1)	6.25
F- E	0 / 228	-18.5	-18.5	0.19 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- E	-326 / 0	0.10 (1)
F- C	-331 / 0	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.19/1.00 (E-F:4) , WB=0.10/1.00 (C-E:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (E) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040272

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
jlandolo
4/04/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	J10	39	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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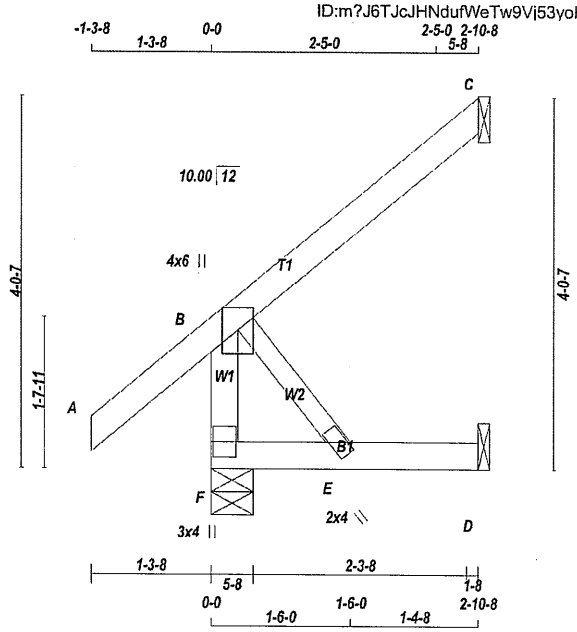

TOTAL WEIGHT = 39 X 13 = 488 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	393	0	393	0	0	5-8	5-8
C	186	0	186	0	0	1-8	1-8
D	27	0	30	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	271	216 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	127	110 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-366 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	0 / 0	-129.6	-129.6	0.18 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.04 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.04/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040273

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED 4/16/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	J11	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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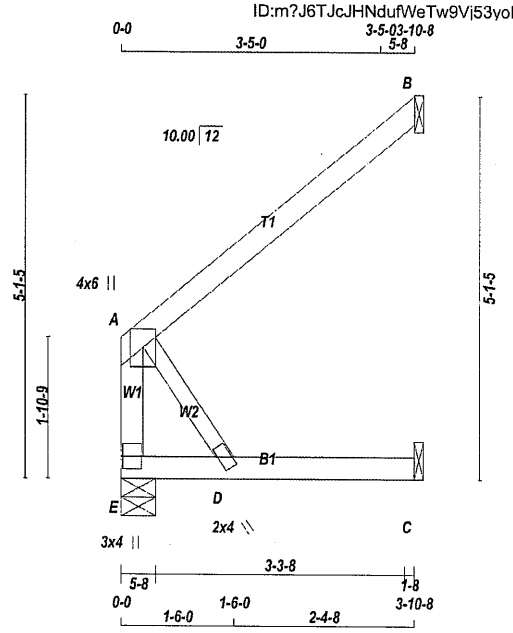

TOTAL WEIGHT = 8 X 14 = 109 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	287	0	287	0	0	5-8	5-8
B	251	0	251	0	0	1-8	1-8
C	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
B	171	148 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
C	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- A	-251 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 0	-129.6	-129.6	0.33 (1)	10.00
E- D	0 / 0	-18.5	-18.5	0.07 (4)	10.00
D- C	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- D	0 / 0	0.00 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.08/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (A) (INPUT = 0.90)
JSI METAL= 0.07 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040274

TOWN OF INNISFIL Block 5 REVIEWED jpendola 4/16/2023 Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J50	18	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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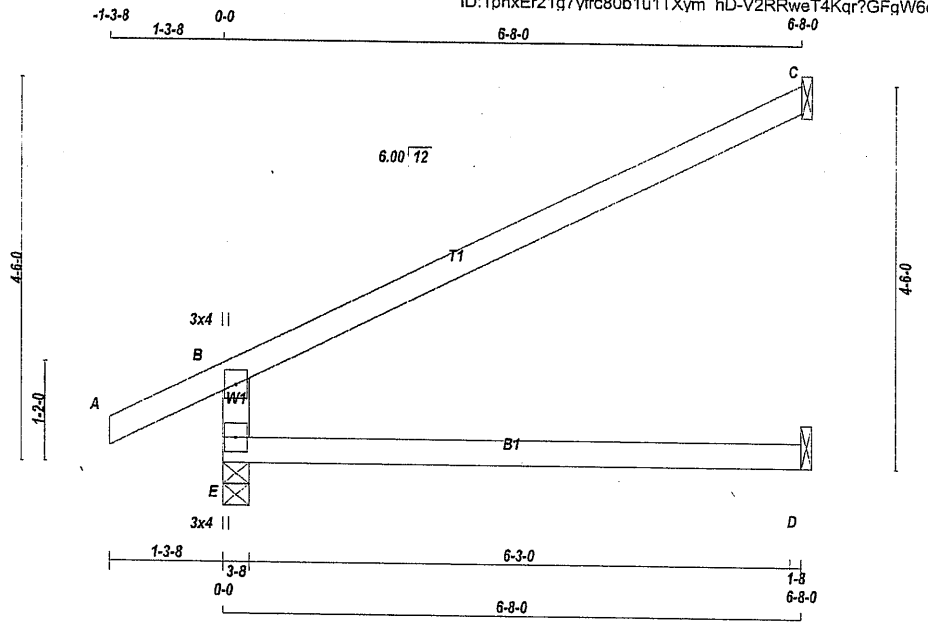

TOTAL WEIGHT = 18 X 19 = 337 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	789	0	789	0	0	3-8	3-8
C	324	0	324	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	547	422 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
C	221	191 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
E- B	-716 / 0	0.0	0.0	0.14 (4)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-48 / 0	-129.6	-129.6	0.76 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.76/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040320

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J51	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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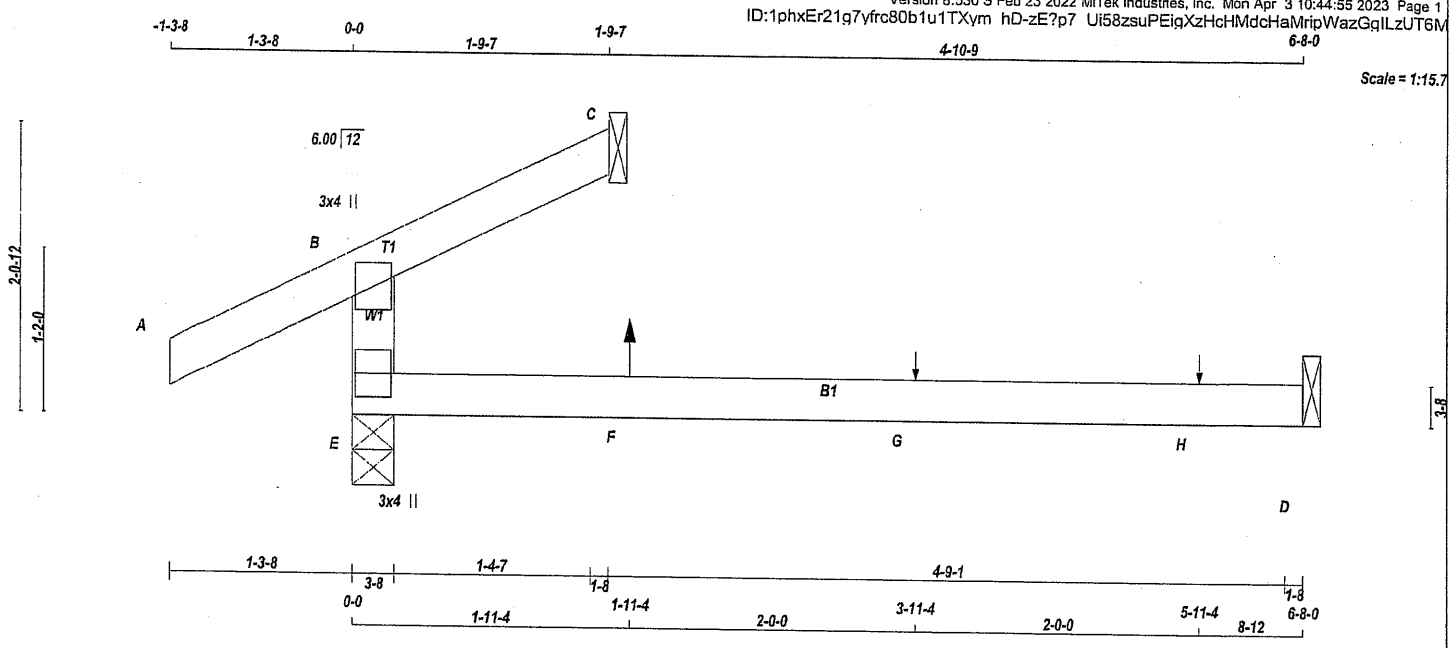

TOTAL WEIGHT = 2 X 12 = 25 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	386	0	386	0	0	3-8	3-8
C	86	0	86	0	0	1-8	1-8
D	49	0	59	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	268	206 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	63	30 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
D	39	0 / -3	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
E- B	-324 / 0	0.0	0.0	0.12 (4)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-15 / 14	-129.6	-129.6	0.12 (4)	6.25
E- F	0 / 0	-18.5	-18.5	0.17 (4)	10.00
F- G	0 / 0	-18.5	-18.5	0.17 (4)	10.00
G- H	0 / 0	-18.5	-18.5	0.17 (4)	10.00
H- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	—	C1
G	3-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1
H	5-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040321

TOWN OF INNISFIL
Block 5
REVIEWED
Permission
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J52	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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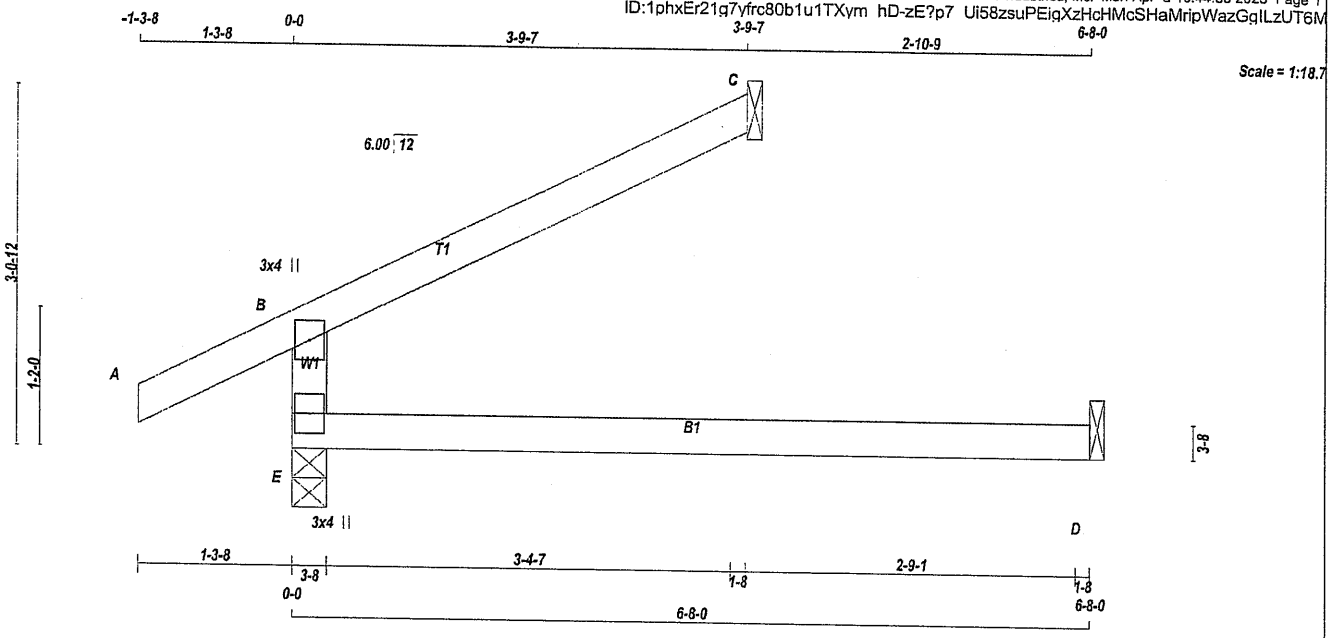

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	555	0	555	0	0	3-8	3-8
C	184	0	184	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	387	284 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
C	126	109 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
E- B	-483 / 0	0.0	0.0	0.15 (4)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-27 / 0	-129.6	-129.6	0.24 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

TOTAL WEIGHT = 2 X 15 = 30 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040322

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J53	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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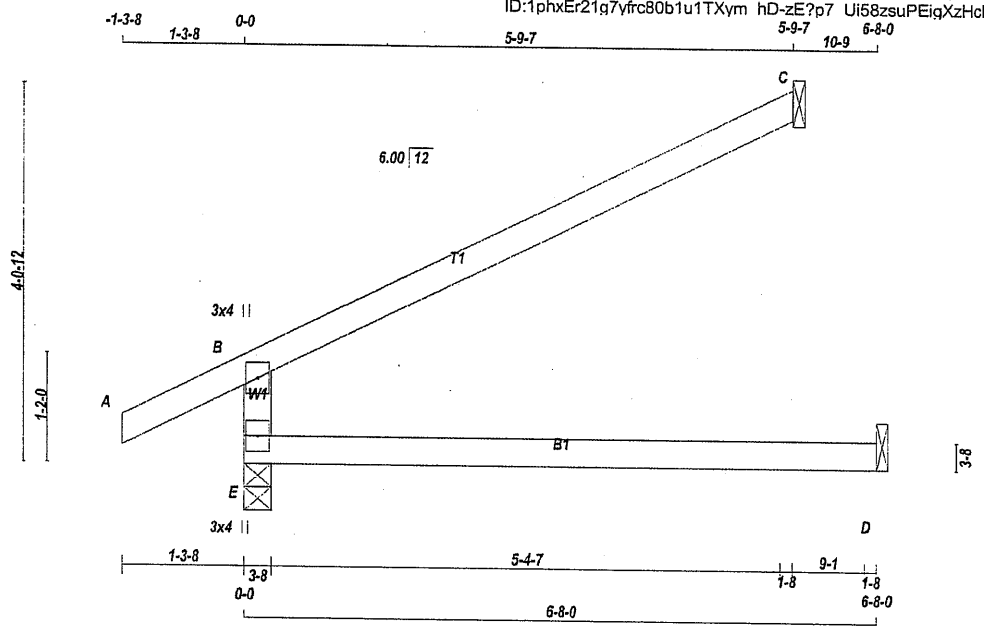

TOTAL WEIGHT = 2 X 18 = 35 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	718	0	718	0	0	3-8	3-8
C	281	0	281	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	498	380 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
C	192	166 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
E- B	-645 / 0	0.0	0.0	0.15 (4)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-42 / 0	-129.6	-129.6	0.57 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040323

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J54	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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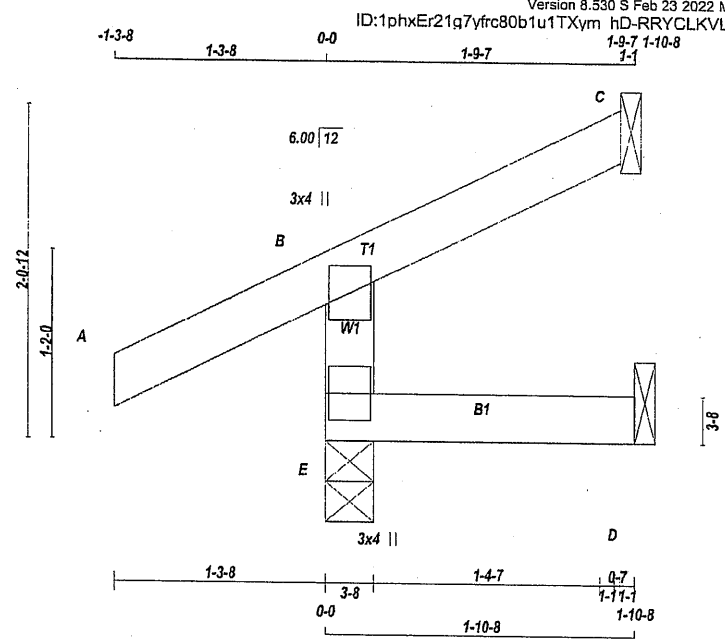

TOTAL WEIGHT = 2 X 7 = 14 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	375	0	375	0	0	3-8	3-8
C	62	0	62	0	-36	1-8	1-8
D	5	0	17	0	-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	258	211 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	43	36 / -27	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	5	0 / -12	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-345 / 0	0.0	0.0	0.05 (5)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-24 / 0	-129.6	-129.6	0.13 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.05 (5)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.05/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040324

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J55	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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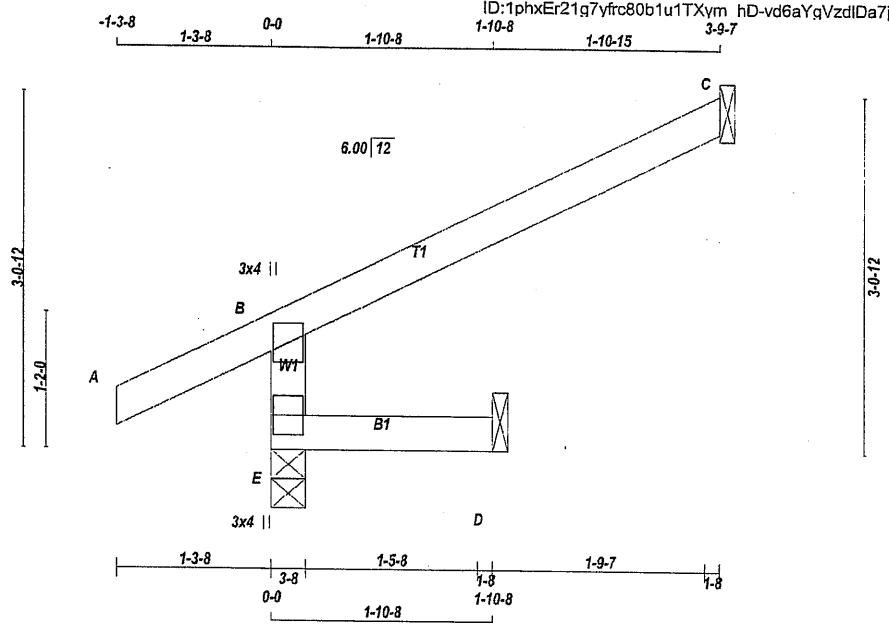

TOTAL WEIGHT = 2 X 10 = 19 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	502	0	502	0	0	3-8	3-8
C	184	0	184	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	344	284 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	126	109 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
E- B	-483 / 0	0.0	0.0	0.01 (4)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-27 / 0	-129.6	-129.6	0.32 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040325

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J56	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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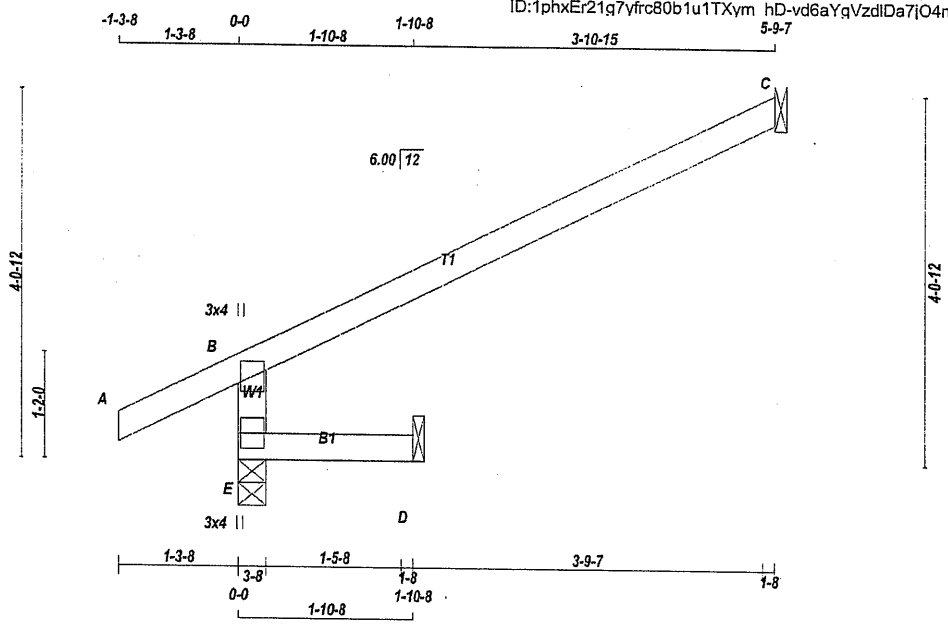

TOTAL WEIGHT = 2 X 12 = 24 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	664	0	664	0	0	3-8	3-8
C	281	0	281	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	455	380 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
C	192	166 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-645 / 0	0.0	0.0	0.01 (1)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-42 / 0	-129.6	-129.6	0.74 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.74/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040326

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 5
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	J57	5	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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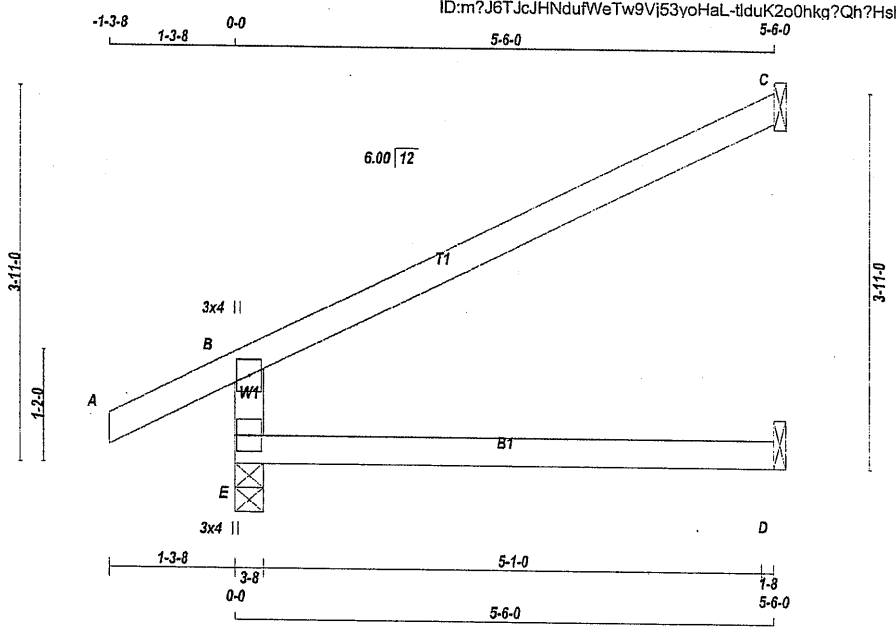

TOTAL WEIGHT = 5 X 16 = 79 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	681	0	681	0	0	3-8	3-8
C	267	0	267	0	0	1-8	1-8
D	42	0	47	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	472	366 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
C	182	158 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
E- B	-621 / 0	0.0	0.0	0.11 (4)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-40 / 0	-129.6	-129.6	0.67 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.11/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040315

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 5 REVIEWED Building Permit Plan 6/16/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	J100	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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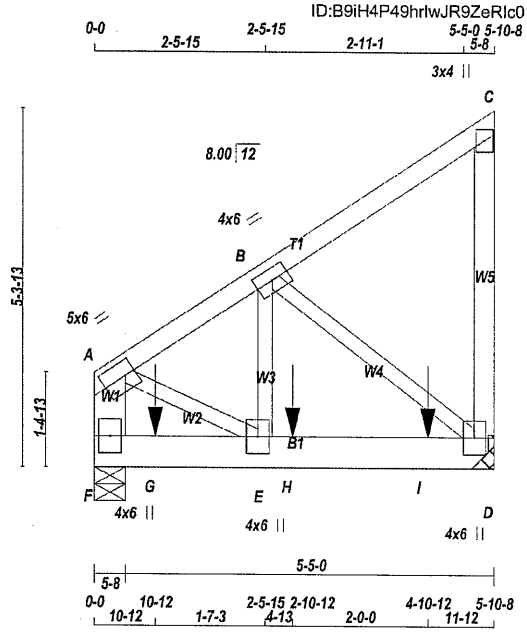

TOTAL WEIGHT = 4 X 32 = 129 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
D- C	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040275

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1986	0	1986	0	0	MECHANICAL	
F	2031	0	2031	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1377	1058 / 0	0 / 0	0 / 0	0 / 0	319 / 0	0 / 0
F	1408	1082 / 0	0 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	-1567 / 0	-129.6	-129.6	0.10 (1)	6.25
B- C	-20 / 0	-129.6	-129.6	0.10 (1)	6.25
D- C	-182 / 0	0.0	0.0	0.04 (1)	7.81
F- A	-1526 / 0	0.0	0.0	0.05 (1)	7.81
F- G	0 / 0	-18.5	-18.5	0.18 (1)	10.00
G- E	0 / 0	-18.5	-18.5	0.18 (1)	10.00
E- H	0 / 1327	-18.5	-18.5	0.36 (1)	10.00
H- I	0 / 1327	-18.5	-18.5	0.36 (1)	10.00
I- D	0 / 1327	-18.5	-18.5	0.36 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 1460	0.18 (1)
B- D	-1688 / 0	0.25 (1)
A- E	0 / 1429	0.18 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	10-12	-727	-727	---	BACK	VERT	TOTAL	---	C1
H	2-10-12	-726	-726	---	BACK	VERT	TOTAL	---	C1
I	4-10-12	-726	-726	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.36/1.00 (D-E:1) , WB=0.25/1.00 (B-D:1) , SSI=0.46/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (E) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402645	J100	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:30:38 2023 Page 2
ID:B9iH4P49hrlwJR9ZeRlc01yng?t-ZNirGy68wq?SaiD_y25JNZMBvp2?Y3INW7ApjWzUTJI

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040275

TOWN OF INNISFIL
Block 5
REVIEWED
jlandolfo
4/10/2023
Building Permit #

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	J110	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:30:38 2023 Page 1
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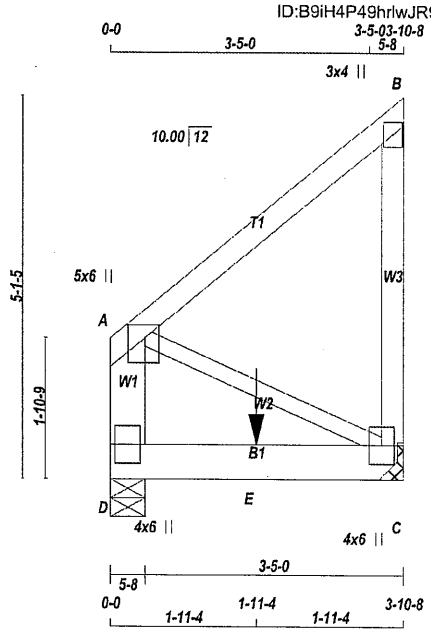

TOTAL WEIGHT = 4 X 23 = 93 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		

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STRUCTURAL COMPONENT ONLY
DWG # TR23040276

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	719	0	719	0	0	5-8	5-8
C	719	0	719	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	500	376 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0
C	500	376 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
D- A	-251 / 0	0.0	0.0	0.01 (1)	7.81
A- B	0 / 0	-129.6	-129.6	0.18 (1)	10.00
C- B	-251 / 0	0.0	0.0	0.05 (1)	7.81
D- E	0 / 0	-18.5	-18.5	0.32 (1)	10.00
E- C	0 / 0	-18.5	-18.5	0.32 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11-4	-600	-600	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.32/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402645	J110	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

TOWN OF INNISFIL
Block 5
REVIEWED
jpentola
4/10/2023
Building Permit Plan

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	4.0	6.0		

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4/03/23
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100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040276

TOWN OF INNISFIL
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REVIEWED
Building Permit Plan

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

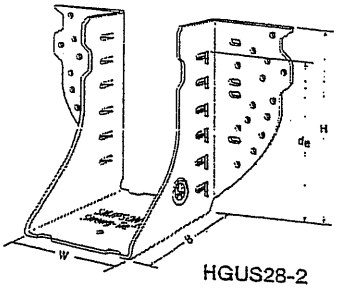
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

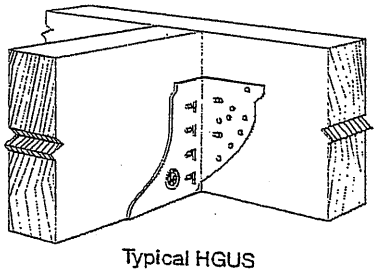
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

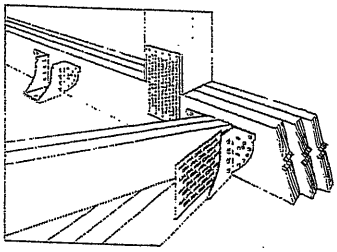
Options:

- See current catalogue for options



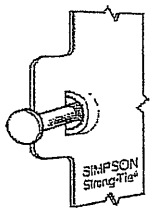
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

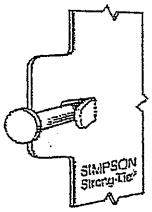
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅜	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 3/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 3/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 3/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 3/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 3/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

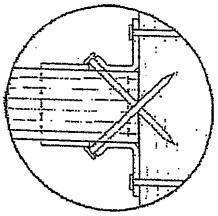


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TOWN OF TINTAGEL
Block 5
REVIEWED
Building Permit Plan

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

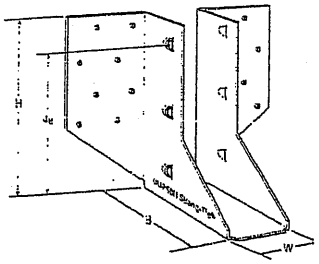
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

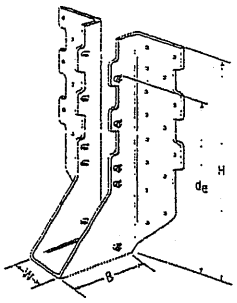
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

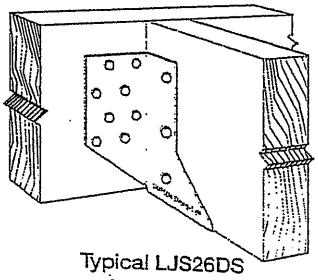
- See current catalogue for options



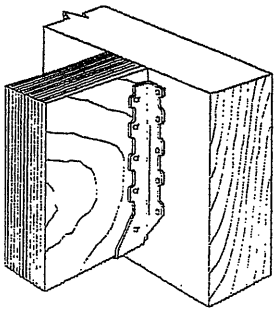
LJS26DS



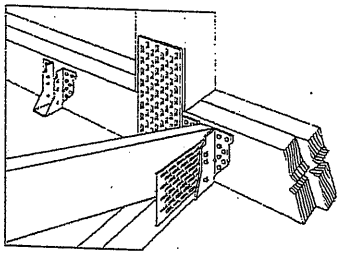
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



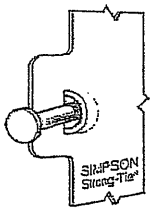
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

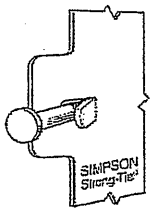
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 3/16	5	3½	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 5/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 5/8	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

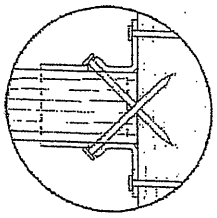


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
strongtie.com

TOWN OF INNISFIL
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Building Permit Plan

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

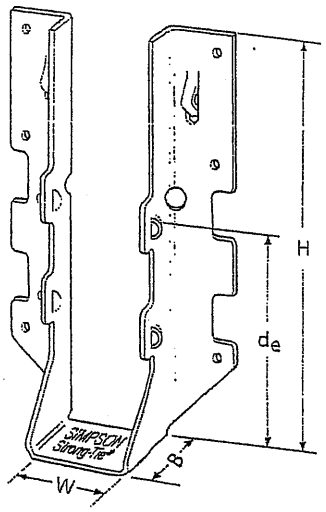
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

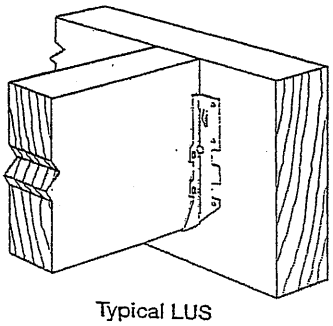
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



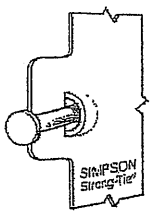
LUS28



Typical LUS Installation

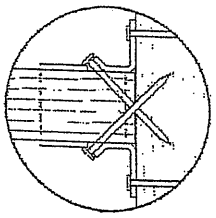
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
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BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

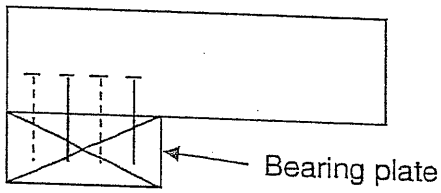
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

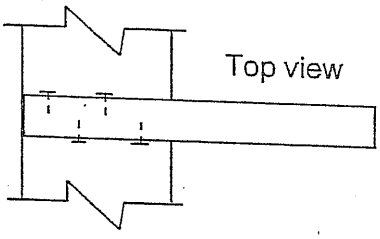
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

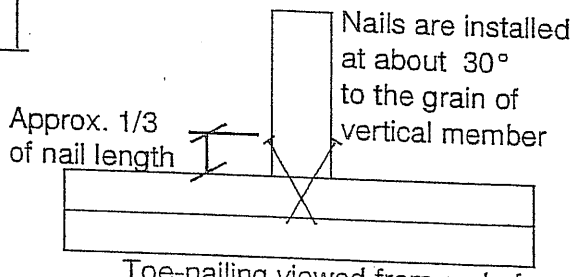
Toe-nailing on 2x6 Bearing Plate



Elevation view

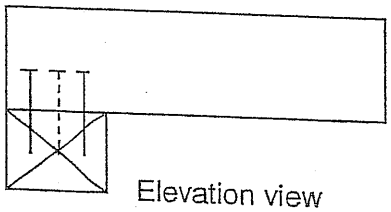


Top view

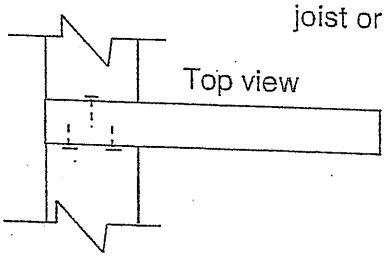


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



Top view

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MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

TOWN OF INNISFIL
Block 5
REVIEWED

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			LATERAL		WITHDRAWAL	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
	MINIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

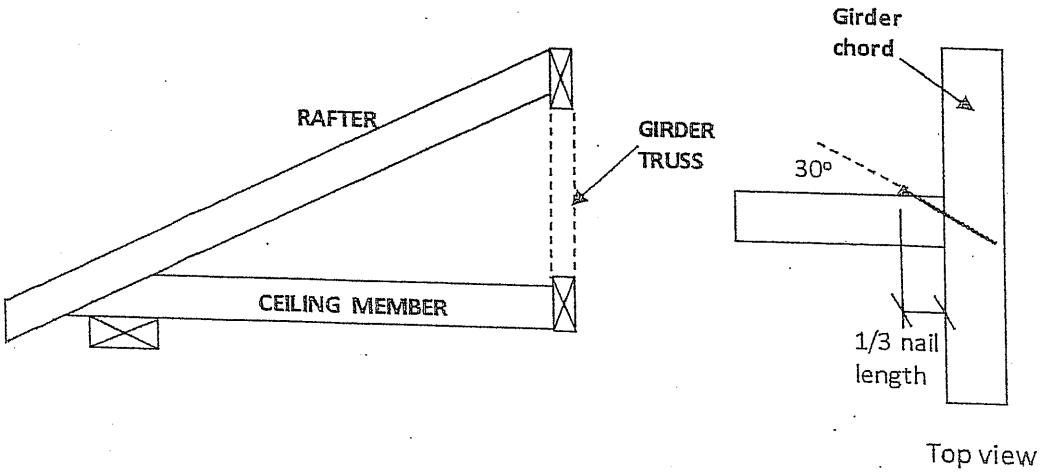

Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

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December 21, 2020

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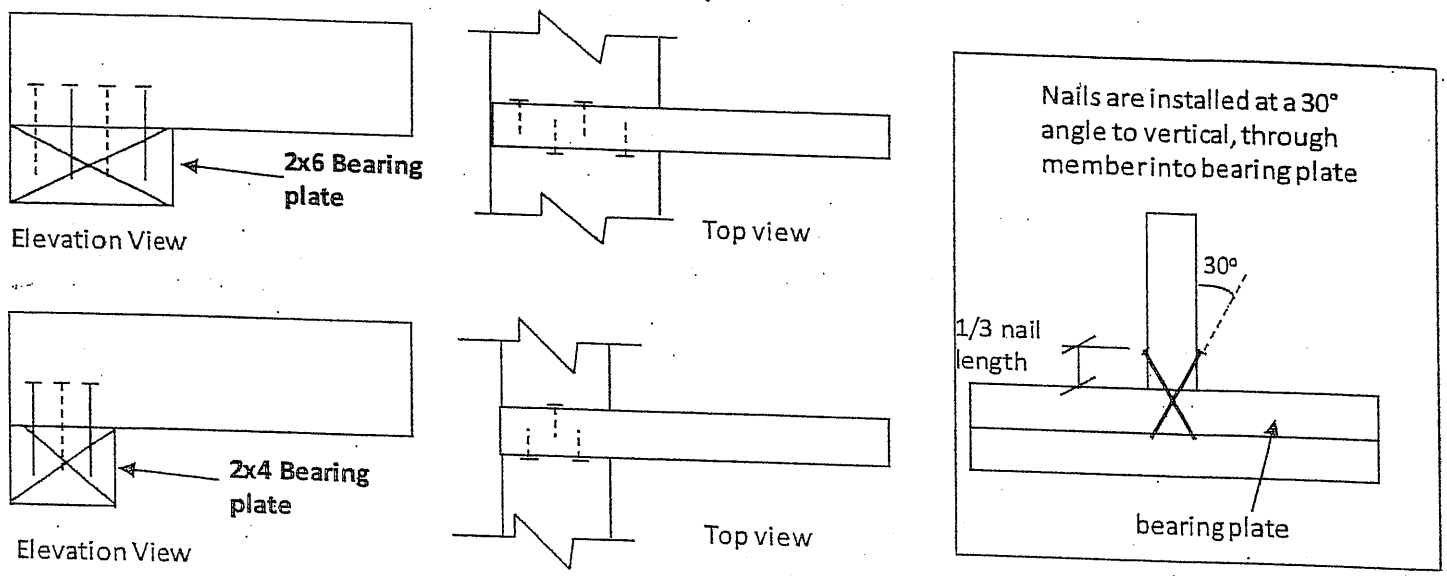
TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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December 21, 2020

TOWN OF INNISFIL
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Building Permit Plan

EWP DESIGN INC.

(905) 832-2250 FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

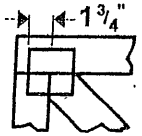
1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

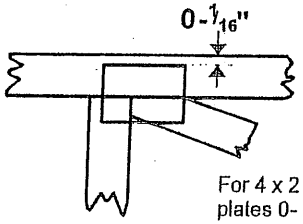
February 1, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

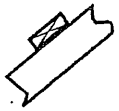
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

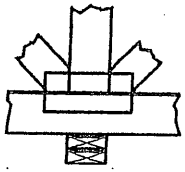
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

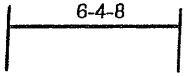


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

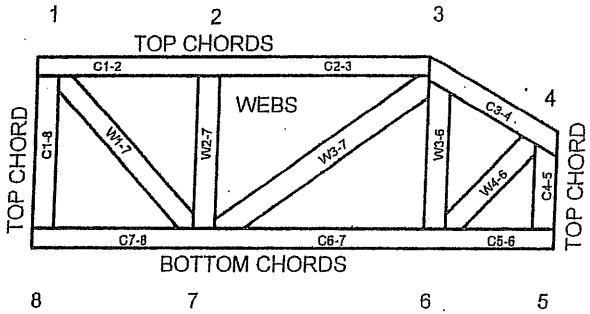
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.