ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

UNIT 1
RL-6E- KINGFISHER 6
ELV (A)
404883

UNIT 2
RL-4-KINGFISHER4
ELV (A) REV
404884

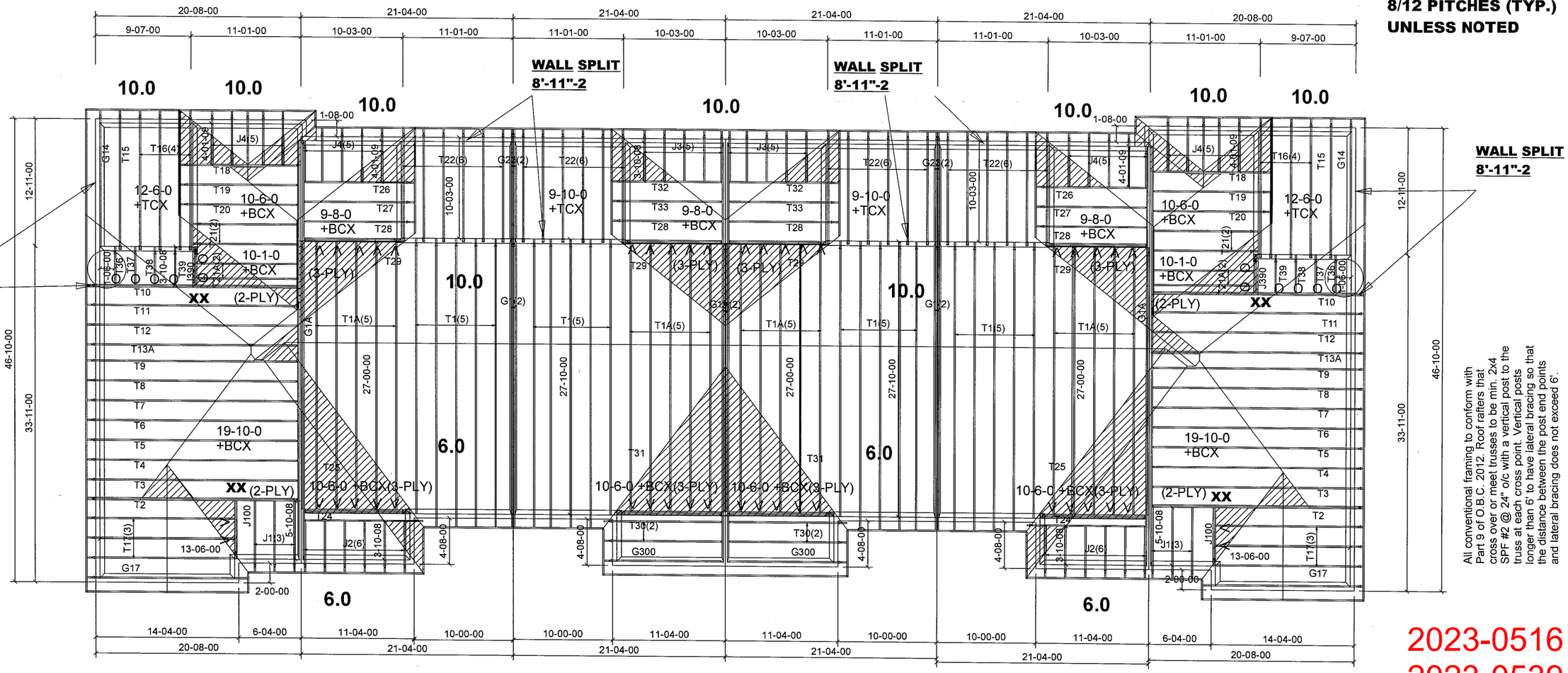
UNIT3
RL-4- KINGFISHER 4
ELV (A2)
404885

UNIT4
RL-4-KINGFISHER 4
ELV (A2)REV
404885

UNIT5
RL-4- KINGFISHER 4
ELV (A)
404884

UNIT6
RL-6E- KINGFISHER 6
ELV (A) REV
404883

8/12 PITCHES (TYP.)
UNLESS NOTED



DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

2023-0516
2023-0539
2023-0538
2023-0526
2023-0542
2023-0534

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023
Building Permit Plans

TAMARACK LUMBER INC.
ZEPA LUMBER GROUP

Job Track:	50469
Plan Log:	200893
Layout ID:	402463

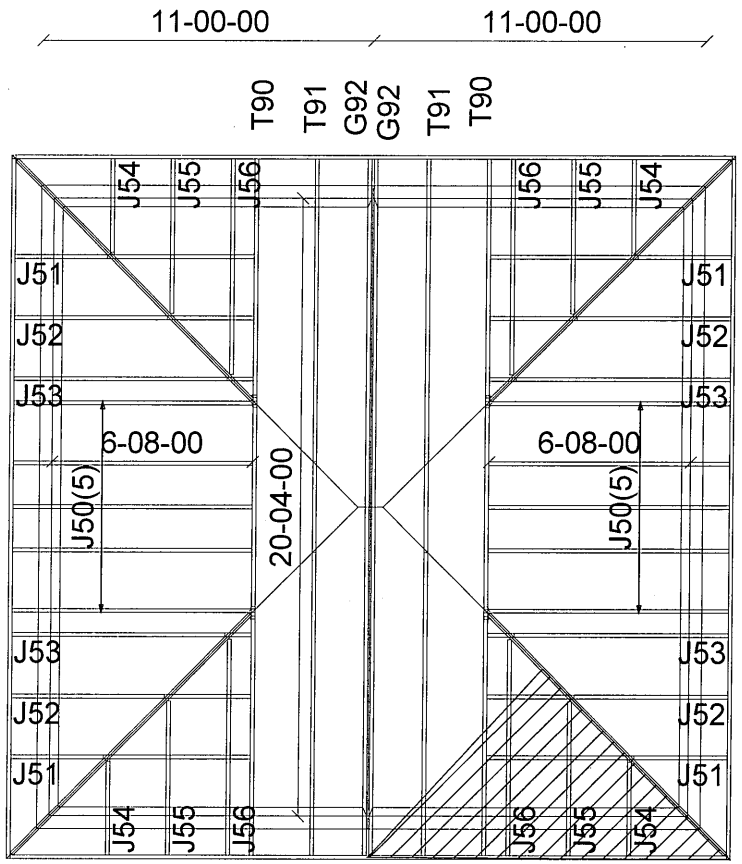
Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**
Model / Elevation: **BLOCK 2 / UNIT 1-6**
Mitek ver 8.5.3.233
Project: **ALCONA SHORES**
Date: 2023-03-28 | Sales: **Mario DiCano** | Designer: wassim/s.v.e

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

UNIT 2
DG-1
EL:A
404952

UNIT 1
DG-1
EL:A2 (REV)
404952

6/12 PITCHES (TYP.)
UNLESS NOTED



DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

GARAGE

TAMARACK LUMBER INC. / ALPA LUMBER GROUP	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 2 -DG-1 EL:A UNIT 2 PAIRED WITH DG-1 EL:A2 UNIT 1
	Plan Log: 200893	Project: ALCONA SHORES	
	Layout ID: 404957	Date: 2023-03-28 Sales: Mario DiCano Designer: S.V	

Mitek ver 8.5.3.233

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

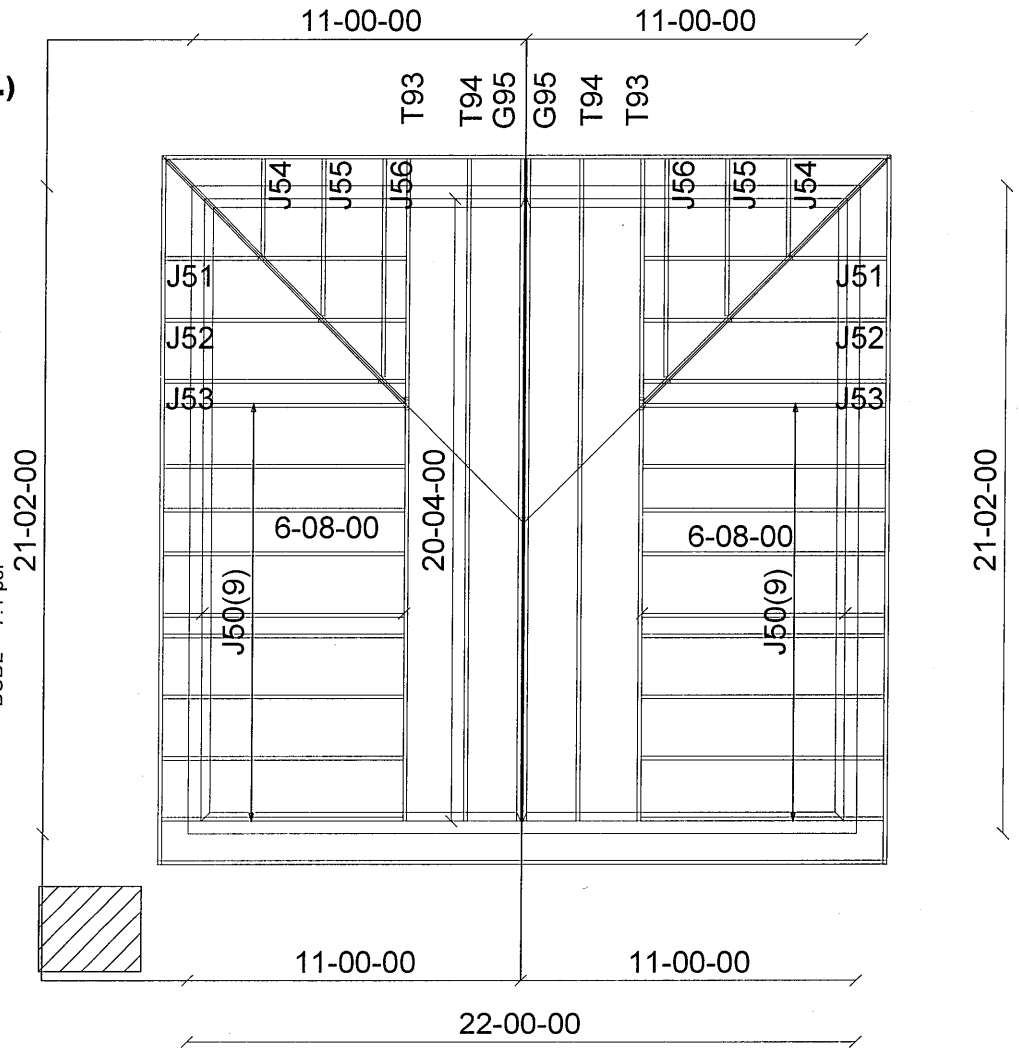
UNIT 4
DG-1
EL:A3 (REV)
404959

UNIT 3
DG-1
EL:A3
404959

6/12 PITCHES (TYP.)
UNLESS NOTED

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf



DENOTES CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

GARAGE

TAMARACK LUMBER INC.	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 2 -DG-1 EL:A3 UNIT 4 PAIRED WITH DG-1 EL:A3 UNIT 3
	Plan Log: 200893	Project: ALCONA SHORES	
	Layout ID: 404958	Date: 2023-03-28 Sales: Mario DiCano Designer: S.V	

Mitek ver 8.5.3.233

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

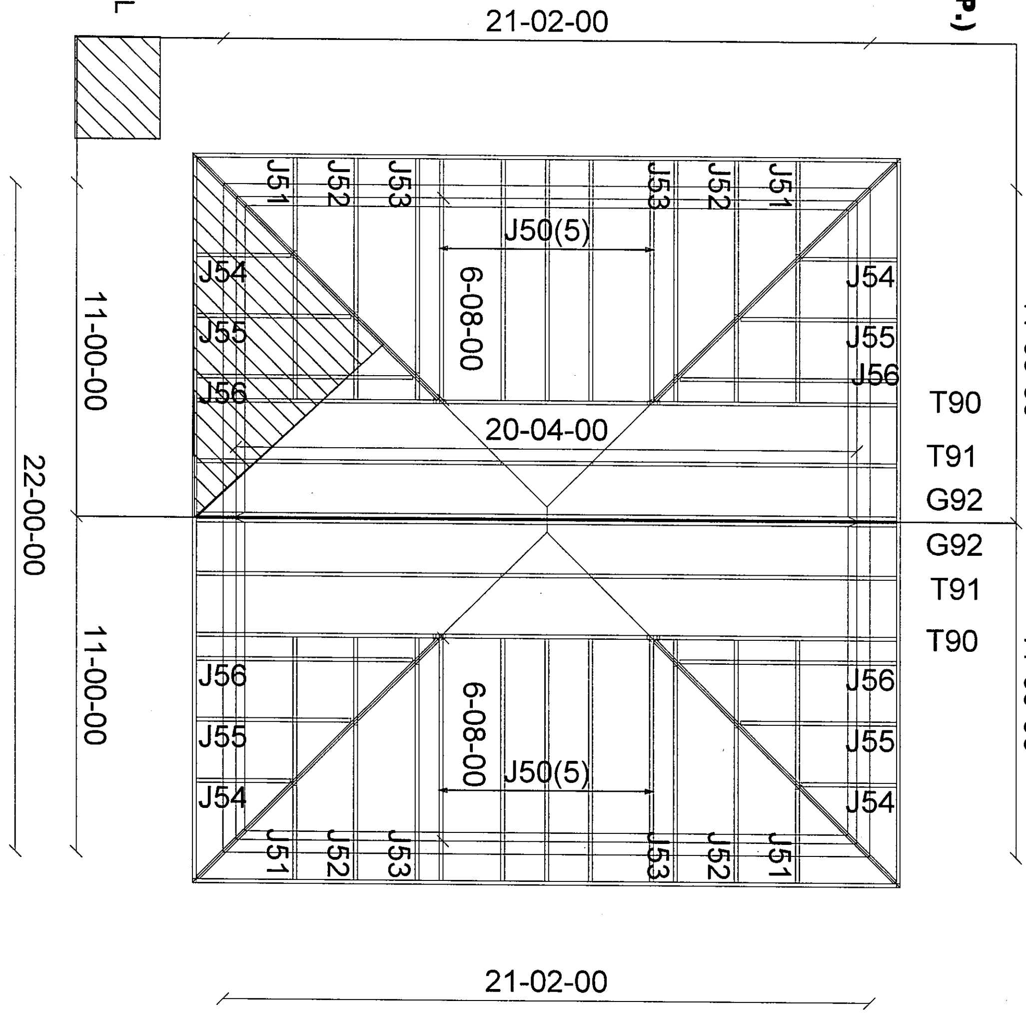
ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

6/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES
CONVENTIONAL
FRAMING

UNIT 6
DG-1
EL:A2 (REV)
404952

UNIT 5
DG-1
EL:A
404952



GARAGE

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

TAMARACK LUMBER INC.

Job Track: 50469
Plan Log: 200893
Layout ID: 404951

Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 2023-03-28
Sales: Mario DiCano
Designer: S.V

Model / Elevation: BLOCK 2 -DG-1 EL:A2 UNIT 6 PAIRED WITH DG-1 EL:A UNIT 5

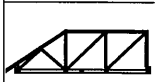
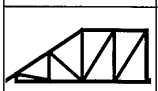
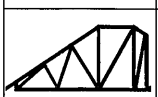
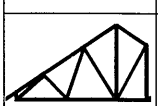
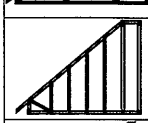
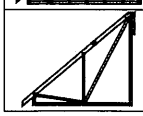
Mitek ver 8.5.3.233

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404883
Location:	INNISFIL	Ref #	
Model:	BLOCK 2	Page:	1 of 3
Lot #:		Date:	03-28-2023
Elevation:	A- UNIT1 OR UNIT6	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

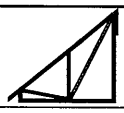
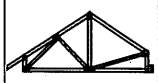
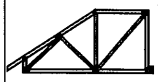
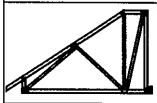
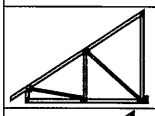
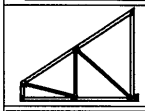
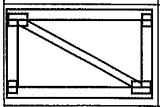
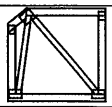
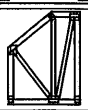
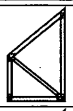
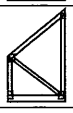
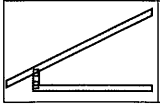
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T2 Half Hip Girder	8 /12	19-10-00	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	187.38 118.00		
	1	T3 Half Hip	8 /12	19-10-00	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	88.2 56.67		
	1	T4 Half Hip	8 /12	19-10-00	6-01-04	2 x 4	1-03-08	1-04-13 6-01-04	94.68 59.83		
	1	T5 Half Hip	8 /12	19-10-00	7-01-04	2 x 4	1-03-08	1-04-13 7-01-04	93.62 59.67		
	1	T6 Half Hip	8 /12	19-10-00	8-01-04	2 x 4	1-03-08	1-04-13 8-01-04	99.24 62.17		
	1	T7 Hip	8 /12	19-10-00	9-01-04	2 x 4	1-03-08	1-04-13 8-11-08	104.97 66.50		
	1	T8 Hip	8 /12	19-10-00	10-01-04	2 x 4	1-03-08	1-04-13 8-11-08	119.08 75.17		
	1	T9 Hip	8 /12	19-10-00	11-01-04	2 x 4	1-03-08	1-04-13 8-11-08	130.81 82.67		
	1 2-ply	T10 Half Hip Girder	8 /12	19-10-00	6-04-05	2 x 4 2 x 6	1-03-08	1-04-13 6-04-05	202.29 128.00		
	1	T11 Half Hip	8 /12	19-10-00	8-00-05	2 x 4	1-03-08	1-04-13 8-00-05	98.8 62.17		
	1	T12 Hip	8 /12	19-10-00	9-08-05	2 x 4	1-03-08	1-04-13 8-04-13	114.41 72.83		
	1	T13A Common	8 /12	19-10-00	11-04-05	2 x 4	1-03-08	1-04-13 8-04-13	106.34 65.83		
	1	G14 GABLE	10 /12	12-06-00	10-03-15	2 x 4	1-03-08	1-07-11 10-03-15	78.49 50.00		
	1	T15 Half Hip	10 /12	12-09-08	11-08-05	2 x 6 2 x 4	1-03-08	1-07-11 12-03-10	88.01 54.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404883
Location:	INNISFIL	Ref #	
Model:	BLOCK 2	Page:	2 of 3
Lot #:		Date:	03-28-2023
Elevation:	A- UNIT1 OR UNIT6	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

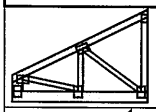
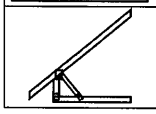
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	T16 Monopitch	10 /12	12-09-08	12-03-10	2 x 4	1-03-08	1-07-11 12-03-10	310.71 193.33		
	3	T17 Common	8 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	175.89 111.00		
	1	G17 GABLE	8 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	59.32 37.00		
	1	T18 Common Girder	8 /12	10-06-00	5-01-00	2 x 4	1-03-08	1-04-13 1-09-02	46.86 30.50		
	1	T19 Half Hip	8 /12	10-06-00	5-04-05	2 x 4	1-03-08	1-04-13 5-04-05	50.11 31.83		
	1	T20 Half Hip	8 /12	10-06-00	7-00-05	2 x 4	1-03-08	1-04-13 7-00-05	56.74 36.33		
	2	T21 Monopitch	8 /12	10-06-00	8-04-13	2 x 4	1-03-08	1-04-13 8-04-13	103.99 65.00		
	2	T21A Jack-Closed	8 /12	10-01-00	8-04-13	2 x 4		1-04-13 8-04-13	98.44 63.67		
	1	T36 Flat	0 /12	3-10-08	2-04-13	2 x 4		2-04-13 2-04-13	16.05 10.00		
	1	T37 Half Hip	10 /12	3-10-08	3-08-13	2 x 4		3-01-09 3-08-13	22 15.83		
	1	T38 Half Hip	10 /12	3-10-08	5-00-13	2 x 4		3-01-09 5-00-13	28.27 19.00		
	1	T39 Jack-Open	10 /12	3-10-08	6-04-05	2 x 4		3-01-09 6-04-05	23.48 15.17		
	1 2-ply	J390 Jack-Closed Girder	10 /12	3-10-08	6-04-05	2 x 4 2 x 6		3-01-09 6-04-05	51.09 33.00		
	3	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	50.38 32.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404883
Location:	INNISFIL	Ref #	
Model:	BLOCK 2	Page:	3 of 3
Lot #:		Date:	03-28-2023
Elevation:	A- UNIT1 OR UNIT6	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**1** **2-ply**	**J100** **Jack-Closed Girder**	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	**5**	**J4** **Jack-Open**	10 /12	**4-01-09**	5-01-00	2 x 4	1-03-08	1-07-11 5-01-00	79.02 55.00		

TOTAL # TRUSS= **47** TOTAL BFT OF ALL TRUSSES= **1799.84** BFT. TOTAL WEIGHT OF ALL TRSSES **2837.08** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
6	Hardware	LUS24	

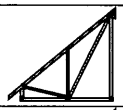
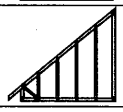
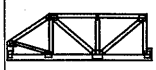
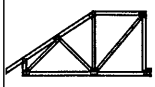
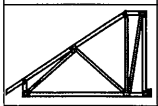
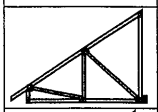
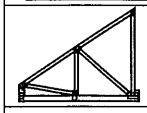
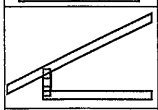
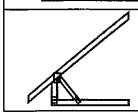
TOTAL NUMBER OF ITEMS= **10**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404884
Location:	INNISFIL	Ref #	
Model:	BLOCK 2	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	A- UNIT2 OR UNIT5	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	T1 Roof Special	10 /12	**27-10-00**	9-09-03	2 x 4	1-03-08	10-14 1-02-00	588.29 368.33		
	1	G1 GABLE	10 /12	**27-10-00**	9-09-03	2 x 4	1-03-08	10-14 1-02-00	128.02 82.50		
	5	T1A Roof Special	10 /12	**27-00-00**	9-09-03	2 x 4		10-14 1-02-00	569.19 359.17		
	1	G1A GABLE	10 /12	**27-00-00**	9-09-03	2 x 4		10-14 1-02-00	126.91 82.67		
	6	T22 Monopitch	10 /12	**10-01-08**	10-00-15	2 x 4	1-03-08	1-07-11 10-00-15	364.2 229.00		
	1	G23 GABLE	10 /12	**9-10-00**	9-10-00	2 x 4	1-03-08	1-07-11 9-10-00	57.93 36.67		
	1	T24 Half Hip Girder	8 /12	**10-06-00**	3-01-04	2 x 4	1-03-08	1-04-13 3-01-04	44.63 28.83		
	1 3-ply	T25 Half Hip Girder	8 /12	**10-06-00**	3-06-04	2 x 4 2 x 6		1-04-13 3-06-04	168.59 110.50		
	1	T26 Half Hip Girder	8 /12	**9-08-00**	5-01-00	2 x 4	1-03-08	1-04-13 5-01-00	46.78 30.83		
	1	T27 Half Hip	8 /12	**9-08-00**	6-09-00	2 x 4	1-03-08	1-04-13 6-09-00	53.44 35.67		
	1	T28 Monopitch	8 /12	**9-08-00**	7-10-02	2 x 4	1-03-08	1-04-13 7-10-02	48.31 31.83		
	1 3-ply	T29 Monopitch Girder	8 /12	**9-08-00**	7-10-02	2 x 4 2 x 6		1-04-13 7-10-02	173.6 112.00		
	6	J2 Jack-Open	6 /12	**3-10-08**	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	71.78 44.00		
	5	J4 Jack-Open	10 /12	**4-01-09**	5-01-00	2 x 4	1-03-08	1-07-11 5-01-00	79.02 55.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404884
Location:	INNISFIL	Ref #	
Model:	BLOCK 2	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	A- UNIT2 OR UNIT5	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **40** TOTAL BFT OF ALL TRUSSES= **1607** BFT. TOTAL WEIGHT OF ALL TRSSES **2520.69** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	

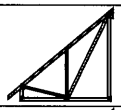
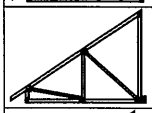
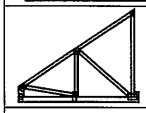
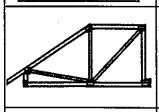
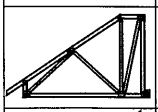
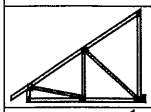
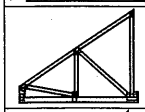
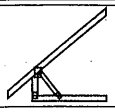
TOTAL NUMBER OF ITEMS= **10**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404885
Location:	INNISFIL	Ref #	
Model:	BLOCK 2	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	A2- UNIT3 OR UNIT4	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G1 GABLE	10 /12	**27-10-00**	9-09-03	2 x 4	1-03-08	10-14 1-02-00	128.02 82.50		
	5	T1 Roof Special	10 /12	**27-10-00**	9-09-03	2 x 4	1-03-08	10-14 1-02-00	588.29 368.33		
	5	T1A Roof Special	10 /12	**27-00-00**	9-09-03	2 x 4		10-14 1-02-00	569.19 359.17		
	1	G1A GABLE	10 /12	**27-00-00**	9-09-03	2 x 4		10-14 1-02-00	126.91 82.67		
	6	T22 Monopitch	10 /12	**10-01-08**	10-00-15	2 x 4	1-03-08	1-07-11 10-00-15	364.2 229.00		
	1	G23 GABLE	10 /12	**9-10-00**	9-10-00	2 x 4	1-03-08	1-07-11 9-10-00	57.93 36.67		
	1	G300 GABLE	8 /12	**10-06-00**	8-04-13	2 x 4	1-03-08	1-04-13 8-04-13	54.82 34.83		
	2	T30 Monopitch	8 /12	**10-06-00**	8-04-13	2 x 4	1-03-08	1-04-13 8-04-13	103.99 65.00		
	1 3-ply	T31 Monopitch Girder	8 /12	**10-06-00**	8-04-13	2 x 4 2 x 6		1-04-13 8-04-13	187.45 118.00		
	1	T32 Half Hip Girder	8 /12	**9-08-00**	4-10-07	2 x 4	1-03-08	1-04-13 4-10-07	44.39 28.67		
	1	T33 Half Hip	8 /12	**9-08-00**	6-06-07	2 x 4	1-03-08	1-04-13 6-06-07	52.52 34.00		
	1	T28 Monopitch	8 /12	**9-08-00**	7-10-02	2 x 4	1-03-08	1-04-13 7-10-02	48.31 31.83		
	1 3-ply	T29 Monopitch Girder	8 /12	**9-08-00**	7-10-02	2 x 4 2 x 6		1-04-13 7-10-02	173.6 112.00		
	5	J3 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	75.68 48.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404885
Location:	INNISFIL	Ref #	
Model:	BLOCK 2	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	A2- UNIT3 OR UNIT4	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **36** TOTAL BFT OF ALL TRUSSES= **1631** BFT. TOTAL WEIGHT OF ALL TRSSES **2575.29** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **10**

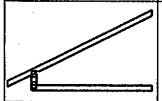
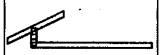
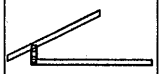
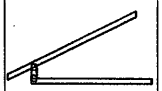
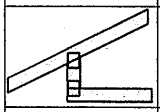
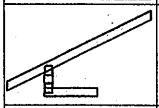
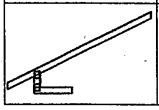
TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	ALCONA SHORES
Location:	INNISFIL
Model:	DG-1
Lot #:	
Elevation:	EL:A UNIT2 OR EL:A2 UNIT1

Job Track:	50469
PlanLog:	200893
Layout ID:	404952
Ref #	
Page:	1 of 1
Date:	08/20/2019
Designer:	
Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T90 Hip Girder	6 /12	**20-04-00**	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	**20-04-00**	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	**20-04-00**	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	**6-08-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	**5-09-07**	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	**1-10-08**	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

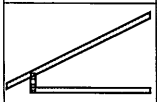
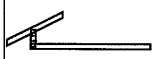
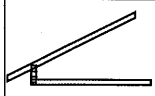
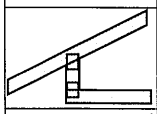
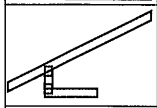
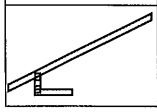
TOTAL # TRUSS= **20** TOTAL BFT OF ALL TRUSSES= **309.67** BFT. TOTAL WEIGHT OF ALL TRSSES **487.86** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404959
Location:	INNISFIL	Ref #	
Model:	BLOCK 2	Page:	1 of 1
Lot #:		Date:	03-28-2023
Elevation:	EL:A3 UNIT4 OR EL:B3 UNIT3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T93 Half Hip Girder	6 /12	**20-04-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	91.12 58.00		
	1	T94 Half Hip	6 /12	**20-04-00**	5-06-00	2 x 4	1-03-08	1-02-00 5-06-00	85.62 53.33		
	1	G95 GABLE	6 /12	**20-04-00**	6-04-08	2 x 4	1-03-08	1-02-00 6-04-08	95.24 59.67		
	9	J50 Jack-Open	6 /12	**6-08-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	168.36 108.00		
	1	J51 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	12.48 8.00		
	1	J52 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	15.03 9.33		
	1	J53 Jack-Open	6 /12	**5-09-07**	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	17.58 11.33		
	1	J54 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	1	J55 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J56 Jack-Open	6 /12	**1-10-08**	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	12.12 8.00		

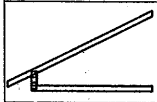
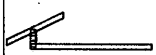
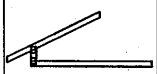
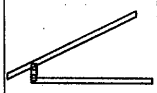
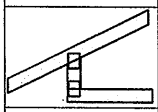
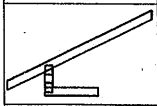
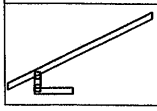
TOTAL # TRUSS= **18** TOTAL BFT OF ALL TRUSSES= **326.33** BFT. TOTAL WEIGHT OF ALL TRSSES **514.15** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404952
Location:	INNISFIL	Ref #	
Model:	DG-1	Page:	1 of 1
Lot #:		Date:	08/20/2019
Elevation:	EL:A2 UNIT6 OR EL:A UNIT5	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T90 Hip Girder	6 /12	**20-04-00**	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	**20-04-00**	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	**20-04-00**	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	**6-08-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	**5-09-07**	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	**1-10-08**	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

TOTAL # TRUSS= **20** TOTAL BFT OF ALL TRUSSES= **309.67** BFT. TOTAL WEIGHT OF ALL TRSSES **487.86** LBS

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T1	20	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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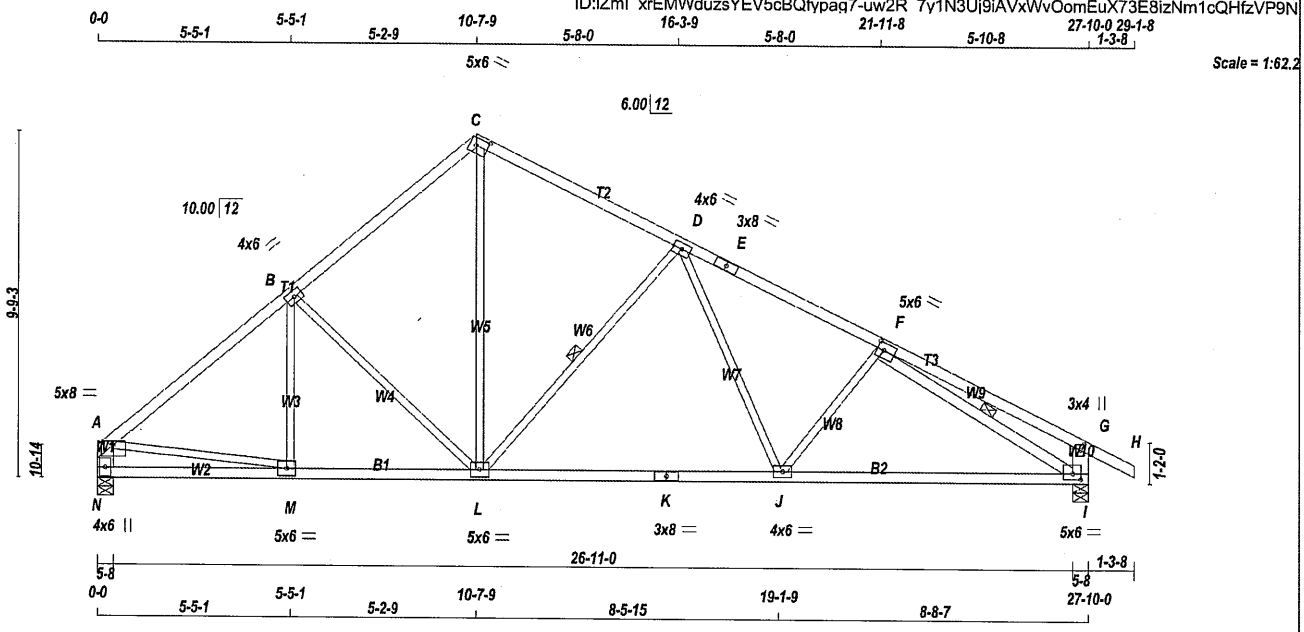

TOTAL WEIGHT = 20 X 118 = 2369 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW-l	MT20	5.0	6.0	2.50	3.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	5.0	6.0	2.50	2.25
G	TMV+p	MT20	3.0	4.0	2.00	1.25
I	BMVW1-t	MT20	5.0	6.0	2.00	2.75
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2062	0	2062	0	0	5-8	5-8
I	2238	0	2238	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1437	1064 / 0	0 / 0	0 / 0	0 / 0	373 / 0	0 / 0
I	1557	1167 / 0	0 / 0	0 / 0	0 / 0	389 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2369 / 0	-129.6	-129.6	0.60	(1)	3.85	M- B	-101 / 65	0.04 (1)
B- C	-1927 / 0	-129.6	-129.6	0.54	(1)	4.25	B- L	-585 / 0	0.62 (1)
C- D	-1654 / 0	-129.6	-129.6	0.54	(1)	4.50	L- C	0 / 1369	0.31 (1)
D- E	-2654 / 0	-129.6	-129.6	0.58	(1)	3.69	L- D	-1100 / 0	0.57 (1)
E- F	-2654 / 0	-129.6	-129.6	0.58	(1)	3.69	D- J	0 / 523	0.12 (1)
F- G	0 / 31	-129.6	-129.6	0.55	(1)	10.00	J- F	-382 / 0	0.14 (1)
G- H	0 / 39	-129.6	-129.6	0.17	(1)	10.00	A- M	0 / 1870	0.42 (1)
N- A	-2017 / 0	0.0	0.0	0.13	(1)	7.12	F- I	-3109 / 0	0.68 (1)
I- G	-480 / 0	0.0	0.0	0.03	(1)	7.81			
N- M	0 / 0	-18.5	-18.5	0.11	(4)	10.00			
M- L	0 / 1859	-18.5	-18.5	0.40	(1)	10.00			
L- K	0 / 2170	-18.5	-18.5	0.52	(1)	10.00			
K- J	0 / 2170	-18.5	-18.5	0.52	(1)	10.00			
J- I	0 / 2608	-18.5	-18.5	0.59	(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.60/1.00 (A-B:1) , BC=0.59/1.00 (I-J:1) , WB=0.68/1.00 (F-I:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.73 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040084

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	G1	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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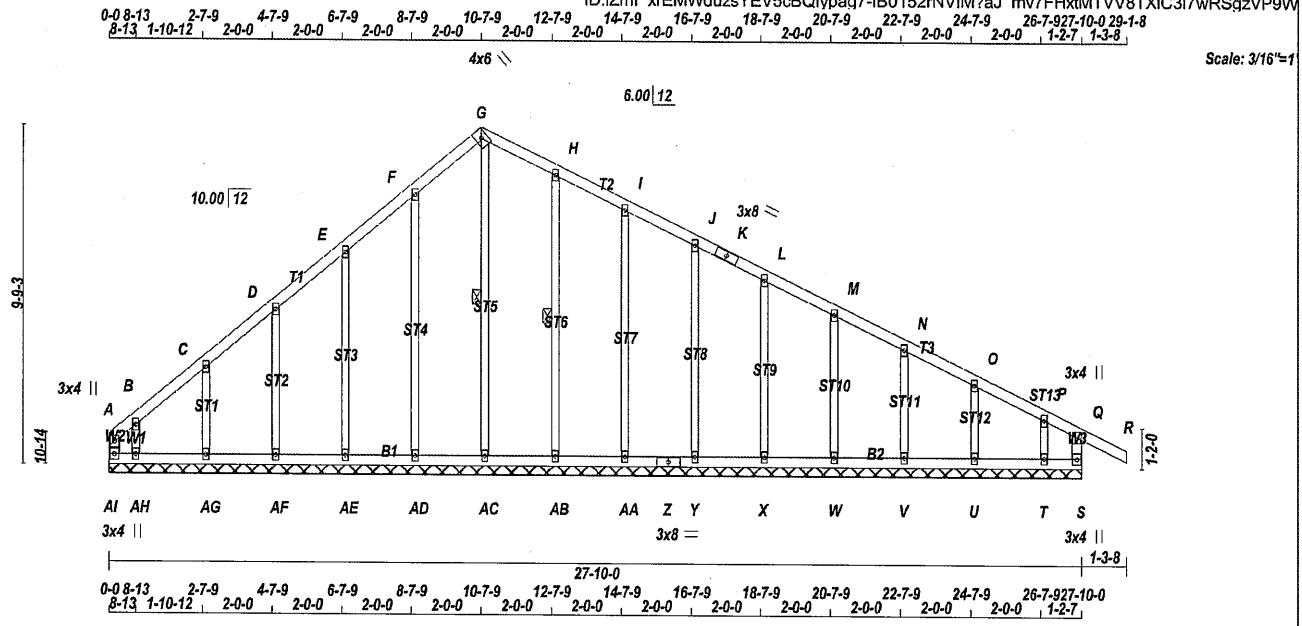

TOTAL WEIGHT = 4 X 128 = 512 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - R	2x4	DRY	No.2	SPF
AI - A	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
AI - Z	2x4	DRY	No.2	SPF
Z - S	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, C, D, E, F, H, I, J, L, M, N, O, P						
B	TMW+w	MT20	2.0	4.0		
G	TTW+h	MT20	4.0	6.0		
K	TS-t	MT20	3.0	8.0		
Q	TMV+p	MT20	3.0	4.0		
S	BMV1+p	MT20	3.0	4.0		
T, U, V, W, X, Y, AA, AB, AC, AD, AE, AF, AG, AH						
T	BMW1+w	MT20	2.0	4.0		
Z	BS-t	MT20	3.0	8.0		
AI	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-AC, H-AB.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-31 / 0	-129.6	-129.6	0.04 (1)	6.25
B- C	-27 / 0	-129.6	-129.6	0.06 (1)	6.25
C- D	-21 / 0	-129.6	-129.6	0.06 (1)	6.25
D- E	-13 / 0	-129.6	-129.6	0.06 (1)	6.25
E- F	-8 / 0	-129.6	-129.6	0.06 (1)	10.00
F- G	-3 / 0	-129.6	-129.6	0.06 (1)	10.00
G- H	0 / 0	-129.6	-129.6	0.07 (1)	10.00
H- I	-1 / 0	-129.6	-129.6	0.07 (1)	10.00
I- J	-5 / 0	-129.6	-129.6	0.06 (1)	10.00
J- K	-8 / 0	-129.6	-129.6	0.06 (1)	10.00
K- L	-8 / 0	-129.6	-129.6	0.06 (1)	10.00
L- M	-11 / 0	-129.6	-129.6	0.06 (1)	6.25
M- N	-15 / 0	-129.6	-129.6	0.06 (1)	6.25
N- O	-23 / 0	-129.6	-129.6	0.07 (1)	6.25
O- P	-22 / 0	-129.6	-129.6	0.07 (1)	6.25
P- Q	-73 / 0	-129.6	-129.6	0.14 (1)	6.25
Q- R	0 / 39	-129.6	-129.6	0.17 (1)	10.00
AI- A	-43 / 0	0.0	0.0	0.02 (1)	7.81
S- Q	-344 / 0	0.0	0.0	0.06 (1)	7.81
AI-AH	0 / 39	-18.5	-18.5	0.02 (1)	10.00
AH-AG	0 / 24	-18.5	-18.5	0.02 (4)	10.00
AG-AF	0 / 15	-18.5	-18.5	0.02 (4)	10.00
AF-AE	0 / 10	-18.5	-18.5	0.01 (4)	10.00
AE-AD	0 / 6	-18.5	-18.5	0.01 (4)	10.00
AD-AC	0 / 3	-18.5	-18.5	0.01 (4)	10.00
AC-AB	0 / 0	-18.5	-18.5	0.01 (4)	10.00
AB-AA	0 / 2	-18.5	-18.5	0.01 (4)	10.00
AA- Z	0 / 4	-18.5	-18.5	0.01 (4)	10.00
Z- Y	0 / 4	-18.5	-18.5	0.01 (4)	10.00
Y- X	0 / 7	-18.5	-18.5	0.01 (4)	10.00
X- W	0 / 10	-18.5	-18.5	0.01 (4)	10.00
W- V	0 / 14	-18.5	-18.5	0.01 (4)	10.00
V- U	0 / 19	-18.5	-18.5	0.02 (4)	10.00
U- T	0 / 26	-18.5	-18.5	0.02 (4)	10.00
T- S	0 / 31	-18.5	-18.5	0.06 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AC- G	-267 / 0	0.17 (1)
AD- F	-255 / 0	0.33 (1)
AE- E	-257 / 0	0.18 (1)
AF- D	-253 / 0	0.09 (1)
AG- C	-261 / 0	0.05 (1)
AB- H	-273 / 0	0.13 (1)
AA- I	-255 / 0	0.28 (1)
Y- J	-258 / 0	0.20 (1)
X- L	-258 / 0	0.13 (1)
W- M	-258 / 0	0.08 (1)
V- N	-253 / 0	0.06 (1)
U- O	-276 / 0	0.05 (1)
T- P	-92 / 0	0.01 (1)
AH- B	-183 / 0	0.03 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (Q-R:1) , BC=0.06/1.00 (S-T:1) , WB=0.33/1.00 (F-AD:1) , SSI=0.12/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (J) (INPUT = 0.90)
JSI METAL= 0.14 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040072

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T1A	20	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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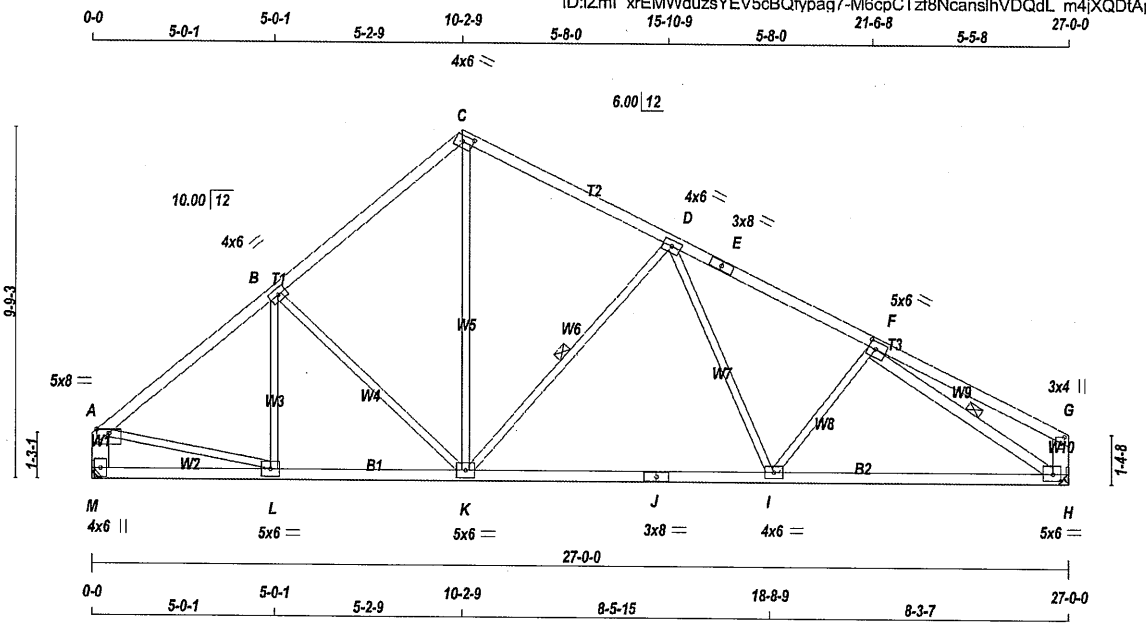

TOTAL WEIGHT = 20 X 115 = 2300 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMVW-t	MT20	4.0	6.0		
C	TTW-l	MT20	4.0	6.0	2.00	3.50
D	TMVW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.50
G	TMV+p	MT20	3.0	4.0	2.00	1.25
H	BMVW1-t	MT20	5.0	6.0	2.25	2.75
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2000	0	2000	0	0	MECHANICAL	
H	2000	0	2000	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M, H. MINIMUM BEARING LENGTH AT JOINT M = 3-8, JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1394	1032 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0
H	1394	1032 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2143 / 0	-129.6	-129.6	0.51 (1)	4.11	L- B	-220 / 29	0.10 (1)
B- C	-1808 / 0	-129.6	-129.6	0.50 (1)	4.43	B- K	-467 / 0	0.50 (1)
C- D	-1555 / 0	-129.6	-129.6	0.54 (1)	4.61	K- C	0 / 1246	0.28 (1)
D- E	-2462 / 0	-129.6	-129.6	0.57 (1)	3.81	K- D	-1042 / 0	0.54 (1)
E- F	-2462 / 0	-129.6	-129.6	0.57 (1)	3.81	D- I	0 / 421	0.09 (1)
F- G	0 / 30	-129.6	-129.6	0.49 (1)	10.00	I- F	-269 / 15	0.10 (1)
M- A	-1958 / 0	0.0	0.0	0.13 (1)	7.19	A- L	0 / 1714	0.39 (1)
H- G	-280 / 0	0.0	0.0	0.02 (1)	7.81	F- H	-2894 / 0	0.58 (1)
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
L- K	0 / 1686	-18.5	-18.5	0.38 (1)	10.00			
K- J	0 / 2042	-18.5	-18.5	0.49 (1)	10.00			
J- I	0 / 2042	-18.5	-18.5	0.49 (1)	10.00			
I- H	0 / 2372	-18.5	-18.5	0.55 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.57/1.00 (D-F:1) , BC=0.55/1.00 (H-I:1) , WB=0.58/1.00 (F-H:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.71 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040085

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	G1A	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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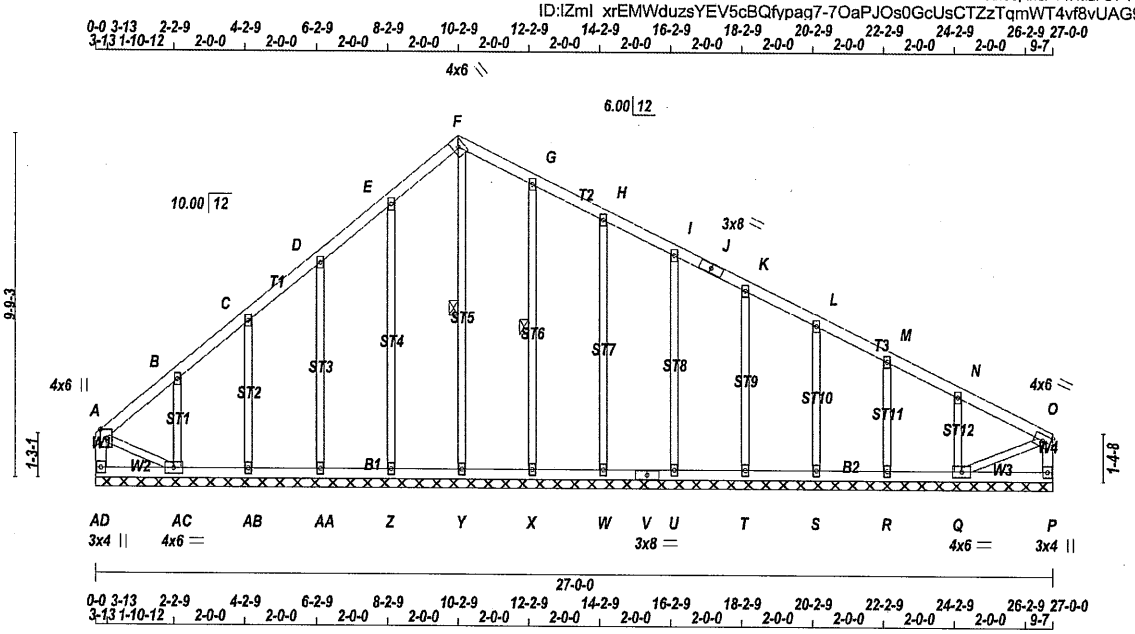

TOTAL WEIGHT = 4 X 127 = 508 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
AD- A	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AD- V	2x4	DRY	No.2	SPF
V - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B, C, D, E, G, H, I, K, L, M, N						
B	TMW+w	MT20	2.0	4.0		
F	TTW+h	MT20	4.0	6.0		
J	TS-t	MT20	3.0	8.0		
O	TMVW-t	MT20	4.0	6.0		Edge
P	BMV1+p	MT20	3.0	4.0		
Q	BMWW1-t	MT20	4.0	6.0		
R, S, T, U, W, X, Y, Z, AA, AB						
R	BMW1+w	MT20	2.0	4.0		
V	BS-t	MT20	3.0	8.0		
AC	BMWW1-t	MT20	4.0	6.0		
AD	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-Y, G-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-47 / 0	-129.6	-129.6	0.09	(1)	6.25	Y- F	-183 / 0	0.12 (1)
B- C	-59 / 0	-129.6	-129.6	0.09	(1)	6.25	Z- E	-285 / 0	0.37 (1)
C- D	-43 / 0	-129.6	-129.6	0.06	(1)	6.25	AA- D	-254 / 0	0.18 (1)
D- E	-37 / 0	-129.6	-129.6	0.07	(1)	6.25	AB- C	-242 / 0	0.08 (1)
E- F	-52 / 0	-129.6	-129.6	0.07	(1)	6.25	AC- B	-305 / 0	0.06 (1)
F- G	-39 / 0	-129.6	-129.6	0.08	(1)	6.25	X- G	-302 / 0	0.15 (1)
G- H	-27 / 0	-129.6	-129.6	0.08	(1)	6.25	W- H	-250 / 0	0.28 (1)
H- I	-33 / 0	-129.6	-129.6	0.06	(1)	6.25	U- I	-259 / 0	0.20 (1)
I- J	-36 / 0	-129.6	-129.6	0.06	(1)	6.25	T- K	-256 / 0	0.13 (1)
J- K	-36 / 0	-129.6	-129.6	0.06	(1)	6.25	S- L	-264 / 0	0.09 (1)
K- L	-40 / 0	-129.6	-129.6	0.06	(1)	6.25	R- M	-228 / 0	0.05 (1)
L- M	-42 / 0	-129.6	-129.6	0.06	(1)	6.25	Q- N	-349 / 0	0.06 (1)
M- N	-59 / 0	-129.6	-129.6	0.11	(1)	6.25	A-AC	0 / 53	0.01 (1)
N- O	-46 / 0	-129.6	-129.6	0.11	(1)	6.25	Q- O	0 / 57	0.01 (1)
AD- A	-177 / 0	0.0	0.0	0.02	(1)	7.81			
P- O	-186 / 0	0.0	0.0	0.02	(1)	7.81			
AD-AC	0 / 0	-18.5	-18.5	0.02	(4)	10.00			
AC-AB	0 / 39	-18.5	-18.5	0.02	(4)	10.00			
AB-AA	0 / 34	-18.5	-18.5	0.02	(4)	10.00			
AA- Z	0 / 30	-18.5	-18.5	0.02	(4)	10.00			
Z- Y	0 / 27	-18.5	-18.5	0.02	(4)	10.00			
Y- X	0 / 25	-18.5	-18.5	0.02	(4)	10.00			
X- W	0 / 27	-18.5	-18.5	0.02	(4)	10.00			
W- V	0 / 30	-18.5	-18.5	0.02	(4)	10.00			
V- U	0 / 30	-18.5	-18.5	0.02	(4)	10.00			
U- T	0 / 32	-18.5	-18.5	0.02	(4)	10.00			
T- S	0 / 35	-18.5	-18.5	0.02	(4)	10.00			
S- R	0 / 39	-18.5	-18.5	0.02	(4)	10.00			
R- Q	0 / 44	-18.5	-18.5	0.03	(4)	10.00			
Q- P	0 / 0	-18.5	-18.5	0.03	(4)	10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (N-O:1) , BC=0.03/1.00 (Q-R:4) , WB=0.37/1.00 (E-Z:1) , SSI=0.14/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (A) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040073

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T2	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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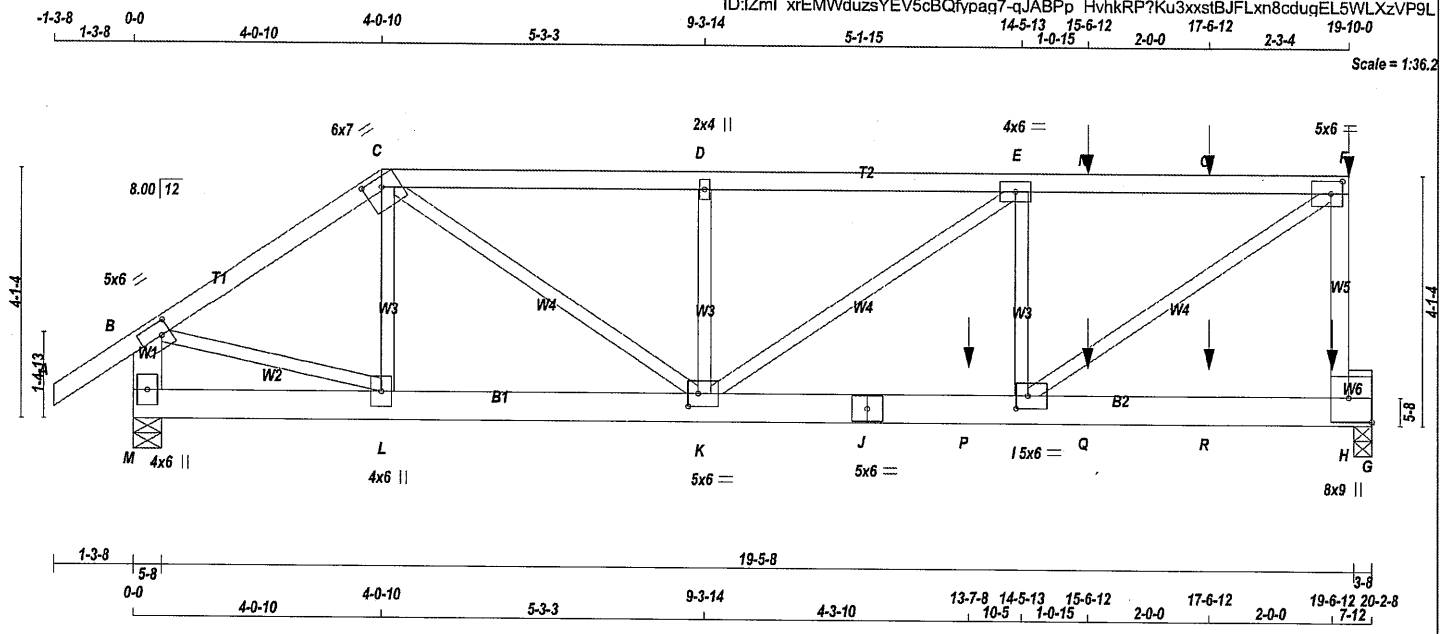

TOTAL WEIGHT = 4 X 94 = 375 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	TOP
J- G	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY
DWG # TR23040086

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2274	0	2274	0	0	5-8	5-8
G	3072	0	3072	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1580	1196 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0
G	2138	1600 / 0	0 / 0	0 / 0	0 / 0	538 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.10 (1)	10.00	L- C	-404 / 0	0.05 (1)
B- C	-2376 / 0	-129.6	-129.6	0.23 (1)	5.56	B- L	0 / 2030	0.25 (1)
C- D	-3619 / 0	-129.6	-129.6	0.32 (1)	4.63	I- F	0 / 4761	0.59 (1)
D- E	-3619 / 0	-129.6	-129.6	0.44 (1)	4.45	C- K	0 / 2027	0.25 (1)
E- N	-3906 / 0	-129.6	-129.6	0.51 (1)	4.21	I- E	-784 / 0	0.10 (1)
N- O	-3906 / 0	-129.6	-129.6	0.51 (1)	4.21	K- D	-689 / 0	0.09 (1)
O- F	-3906 / 0	-129.6	-129.6	0.51 (1)	4.21	K- E	-354 / 0	0.13 (1)
H- F	-3307 / 0	0.0	0.0	0.41 (1)	6.40			
M- B	-2237 / 0	0.0	0.0	0.08 (1)	7.81			
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
L- K	0 / 1963	-18.5	-18.5	0.16 (1)	10.00			
K- J	0 / 3906	-18.5	-18.5	0.50 (1)	10.00			
J- P	0 / 3906	-18.5	-18.5	0.50 (1)	10.00			
P- I	0 / 3906	-18.5	-18.5	0.50 (1)	10.00			
I- Q	0 / 0	-18.5	-18.5	0.42 (1)	10.00			
Q- R	0 / 0	-18.5	-18.5	0.42 (1)	10.00			
R- H	0 / 0	-18.5	-18.5	0.42 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.18 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	19-10-0	-139	-139	---	FRONT	VERT	TOTAL	---	C1
H	19-6-12	-24	-24	---	FRONT	VERT	TOTAL	---	C1
N	15-6-12	-106	-106	---	FRONT	VERT	TOTAL	---	C1
O	17-6-12	-106	-106	---	FRONT	VERT	TOTAL	---	C1
P	13-7-8	-1126	-1126	---	FRONT	VERT	TOTAL	---	C1
Q	15-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	17-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.51/1.00 (E-F:1) , BC=0.50/1.00 (I-K:1) , WB=0.59/1.00 (F-I:1) , SSI=0.51/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.51 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402463	T2	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW-h	MT20	6.0	7.0	1.75	3.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.25
H	BMVW+p	MT20	8.0	9.0	4.75	Edge
I	BMWW-t	MT20	5.0	6.0	2.50	2.25
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW+t	MT20	4.0	6.0		
M	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

STRUCTURAL COMPONENT ONLY
DWG # TR23040086

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T3	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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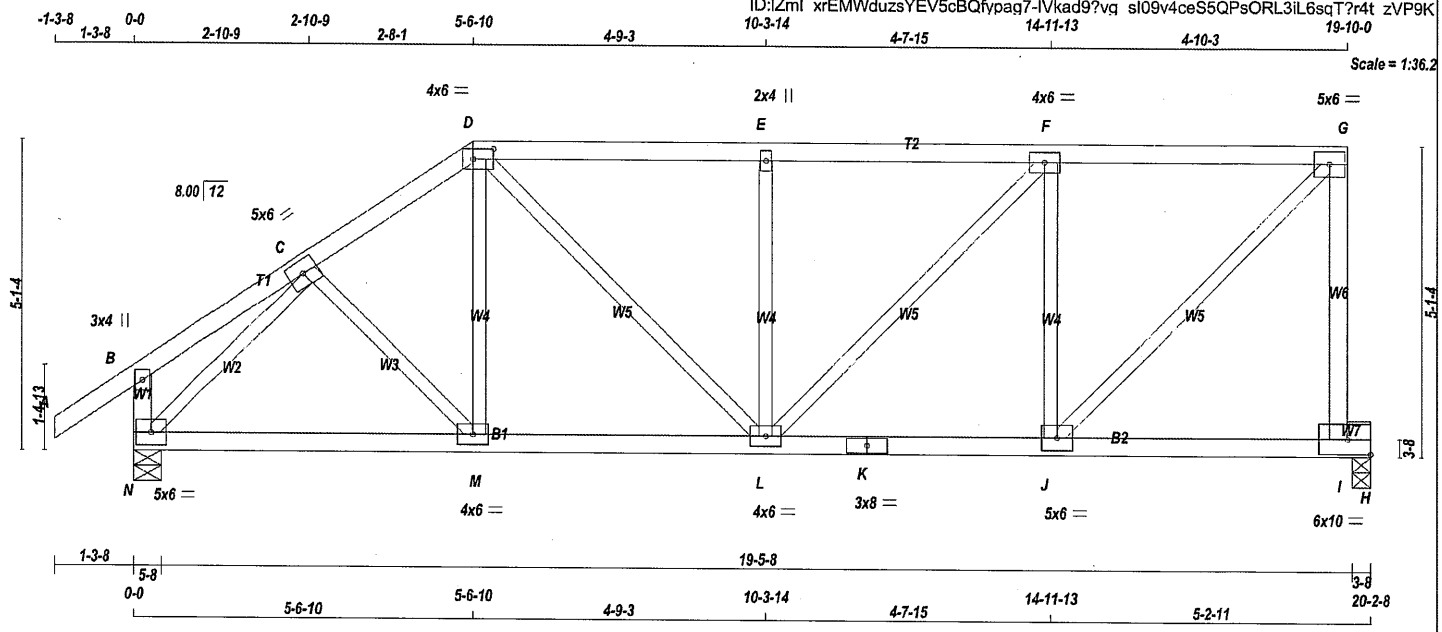


TOTAL WEIGHT = 2 X 88 = 176 lb [M]

LUMBER

N. L. G. A. RULES

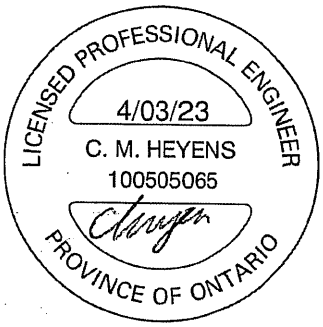
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-I	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
I	BMVW-t	MT20	6.0	10.0	3.00	Edge
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040087

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1674	0	1674	0	0	5-8	5-8
H	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- M	0 / 51	0.02 (4)
B- C	0 / 20	-129.6	-129.6	0.14 (1)	10.00	M- D	0 / 96	0.03 (4)
C- D	-1547 / 0	-129.6	-129.6	0.16 (1)	5.13	N- C	-1804 / 0	0.48 (1)
D- E	-1621 / 0	-129.6	-129.6	0.43 (1)	4.67	J- G	0 / 1832	0.41 (1)
E- F	-1621 / 0	-129.6	-129.6	0.45 (1)	4.64	D- L	0 / 498	0.11 (1)
F- G	-1301 / 0	-129.6	-129.6	0.43 (1)	5.08	J- F	-1008 / 0	0.39 (1)
I- G	-1546 / 0	0.0	0.0	0.68 (1)	6.62	L- E	-661 / 0	0.25 (1)
N- B	-322 / 0	0.0	0.0	0.03 (1)	7.81	L- F	0 / 460	0.10 (1)
N- M	0 / 1240	-18.5	-18.5	0.28 (1)	10.00			
M- L	0 / 1270	-18.5	-18.5	0.28 (1)	10.00			
L- K	0 / 1301	-18.5	-18.5	0.46 (1)	10.00			
K- J	0 / 1301	-18.5	-18.5	0.46 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.74 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.68/1.00 (G-I:1) , BC=0.74/1.00 (I-J:1) , WB=0.48/1.00 (C-N:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T4	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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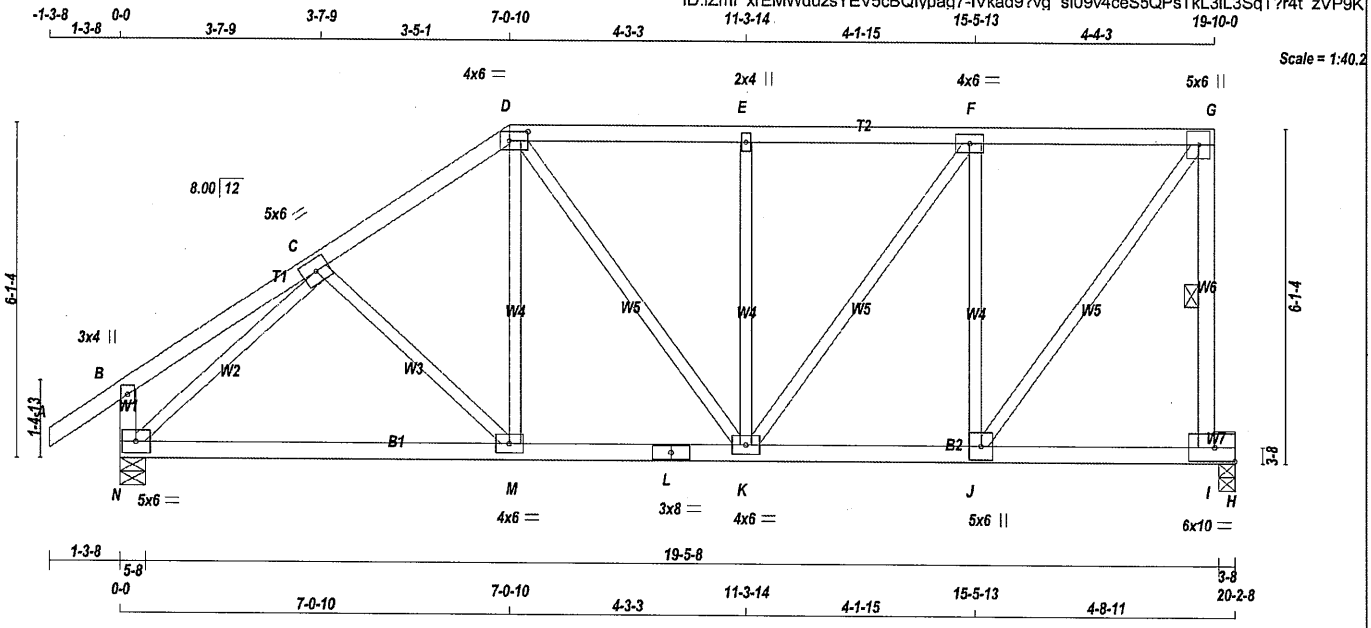

TOTAL WEIGHT = 2 X 95 = 189 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-I	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	5.0	6.0		
I	BMVW-t	MT20	6.0	10.0	3.00	4.50
J	BMWW+t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1674	0	1674	0	0	5-8	5-8
H	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- M	-140 / 18	0.06 (1)
B- C	0 / 28	-129.6	-129.6	0.25 (1)	10.00	M- D	0 / 243	0.05 (1)
C- D	-1469 / 0	-129.6	-129.6	0.28 (1)	5.08	N- C	-1813 / 0	0.70 (1)
D- E	-1311 / 0	-129.6	-129.6	0.33 (1)	5.23	J- G	0 / 1670	0.38 (1)
E- F	-1312 / 0	-129.6	-129.6	0.34 (1)	5.21	D- K	0 / 186	0.04 (1)
F- G	-1001 / 0	-129.6	-129.6	0.33 (1)	5.79	J- F	-1041 / 0	0.61 (1)
I- G	-1567 / 0	0.0	0.0	0.28 (1)	5.27	K- E	-590 / 0	0.35 (1)
N- B	-355 / 0	0.0	0.0	0.04 (1)	7.81	K- F	0 / 534	0.12 (1)
N- M	0 / 1299	-18.5	-18.5	0.33 (1)	10.00			
M- L	0 / 1201	-18.5	-18.5	0.31 (1)	10.00			
L- K	0 / 1201	-18.5	-18.5	0.31 (1)	10.00			
K- J	0 / 1001	-18.5	-18.5	0.40 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.74 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.74/1.00 (I-J:1) , WB=0.70/1.00 (C-N:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.43 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040088

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T5	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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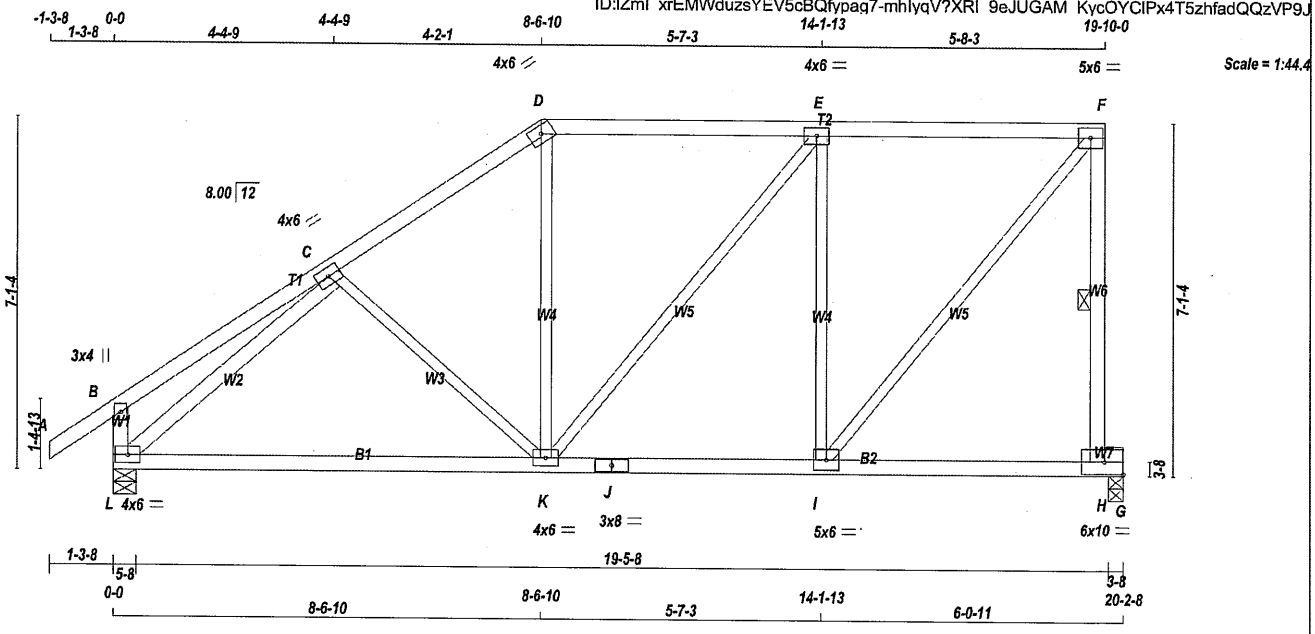

TOTAL WEIGHT = 2 X 94 = 187 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW-t	MT20	6.0	10.0		Edge
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1674	0	1674	0	0	5-8	5-8
G	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- K	-305 / 0	0.18 (1)
B- C	0 / 36	-129.6	-129.6	0.38 (1)	10.00	K- D	0 / 244	0.05 (1)
C- D	-1353 / 0	-129.6	-129.6	0.43 (1)	5.02	L- C	-1795 / 0	0.72 (1)
D- E	-1098 / 0	-129.6	-129.6	0.75 (1)	4.68	I- F	0 / 1609	0.36 (1)
E- F	-1030 / 0	-129.6	-129.6	0.74 (1)	4.81	K- E	0 / 108	0.02 (1)
H- F	-1516 / 0	0.0	0.0	0.35 (1)	5.33	I- E	-989 / 0	0.86 (1)
L- B	-390 / 0	0.0	0.0	0.04 (1)	7.81			
L- K	0 / 1321	-18.5	-18.5	0.39 (1)	10.00			
K- J	0 / 1030	-18.5	-18.5	0.41 (1)	10.00			
J- I	0 / 1030	-18.5	-18.5	0.41 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.74 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 798 (0.30")

CSI: TC=0.75/1.00 (D-E:1) , BC=0.74/1.00 (H-I:1) , WB=0.86/1.00 (E-I:1) , SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040089

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T6	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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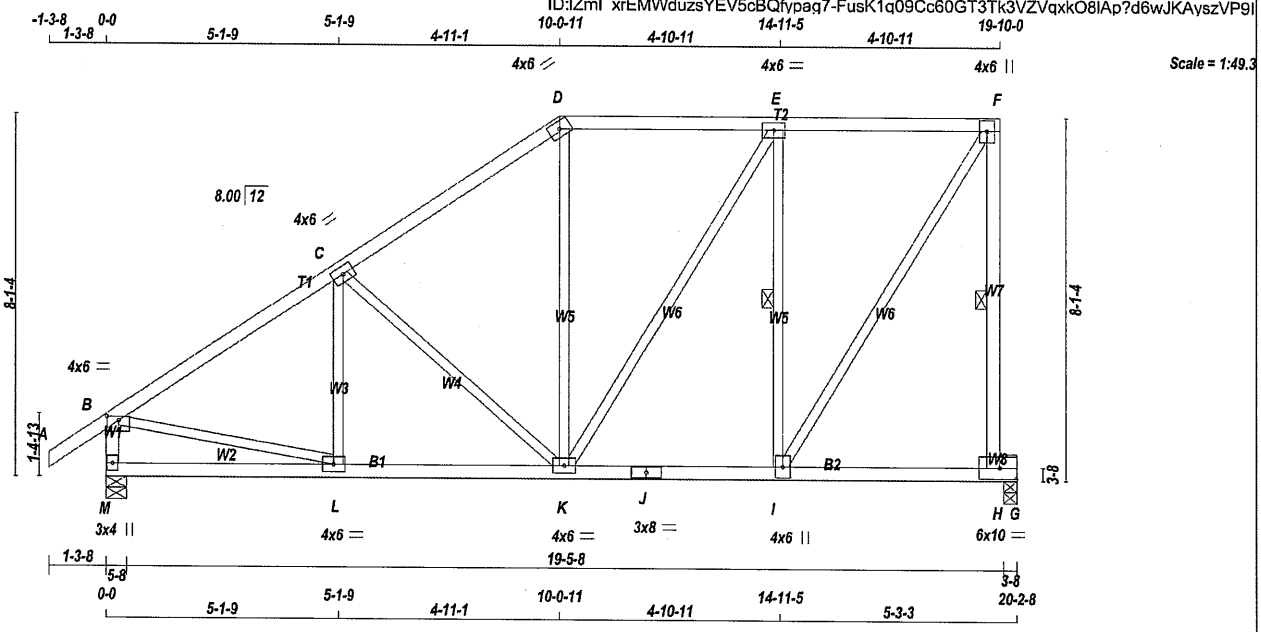

TOTAL WEIGHT = 2 X 99 = 198 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW-t	MT20	6.0	10.0	3.00	4.50
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1674	0	1674	0	0	5-8	5-8
G	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00		L- C	-174 / 50	0.06 (1)
B- C	-1605 / 0	-129.6	-129.6	0.65 (1)	4.35		C- K	-548 / 0	0.46 (1)
C- D	-1203 / 0	-129.6	-129.6	0.61 (1)	4.91		K- D	0 / 165	0.04 (1)
D- E	-964 / 0	-129.6	-129.6	0.54 (1)	5.46		K- E	0 / 279	0.06 (1)
E- F	-820 / 0	-129.6	-129.6	0.53 (1)	5.81		I- E	-1021 / 0	0.42 (1)
H- F	-1543 / 0	0.0	0.0	0.46 (1)	5.30		I- F	0 / 1536	0.35 (1)
M- B	-1636 / 0	0.0	0.0	0.17 (1)	6.47		B- L	0 / 1401	0.32 (1)
M- L	0 / 0	-18.5	-18.5	0.12 (4)	10.00				
L- K	0 / 1371	-18.5	-18.5	0.28 (1)	10.00				
K- J	0 / 820	-18.5	-18.5	0.38 (1)	10.00				
J- I	0 / 820	-18.5	-18.5	0.38 (1)	10.00				
I- H	0 / 0	-18.5	-18.5	0.74 (1)	10.00				
H- G	0 / 0	-18.5	-18.5	0.31 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.65/1.00 (B-C:1) , BC=0.74/1.00 (H-I:1) , WB=0.46/1.00 (C-K:1) , SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040090

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T7	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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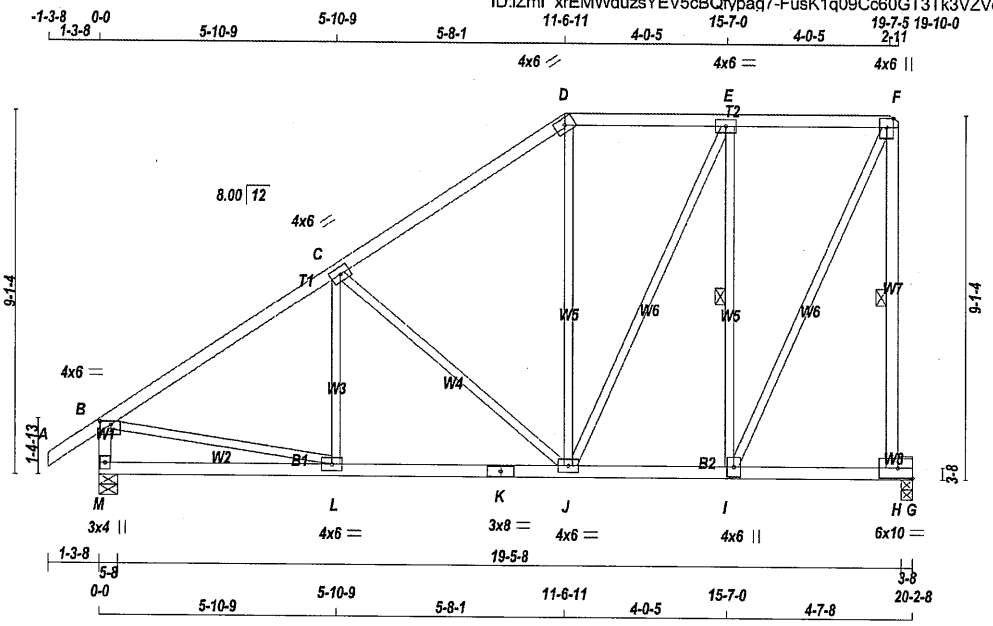

TOTAL WEIGHT = 2 X 105 = 210 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVW-t	MT20	6.0	10.0		Edge
I	BMWW+t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

STRUCTURAL COMPONENT ONLY
DWG # TR23040091

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1674	0	1674	0	0	5-8	5-8
G	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-1592 / 0	-129.6	-129.6	0.90 (1)	3.82
C- D	-1056 / 0	-129.6	-129.6	0.82 (1)	4.61
D- E	-836 / 0	-129.6	-129.6	0.37 (1)	6.12
E- F	-647 / 0	-129.6	-129.6	0.36 (1)	6.25
M- B	-1631 / 0	0.0	0.0	0.17 (1)	6.48
H- F	-1573 / 0	0.0	0.0	0.61 (1)	5.26
M- L	0 / 0	-18.5	-18.5	0.16 (4)	10.00
L- K	0 / 1366	-18.5	-18.5	0.31 (1)	10.00
K- J	0 / 1366	-18.5	-18.5	0.31 (1)	10.00
J- I	0 / 647	-18.5	-18.5	0.33 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.74 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.31 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-120 / 79	0.05 (1)
C- J	-702 / 0	0.83 (1)
J- D	0 / 80	0.03 (4)
J- E	0 / 458	0.10 (1)
I- E	-1075 / 0	0.58 (1)
B- L	0 / 1389	0.31 (1)
I- F	0 / 1509	0.34 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.90/1.00 (B-C:1) , BC=0.74/1.00 (H-I:1) , WB=0.83/1.00 (C-J:1) , SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.51 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T8	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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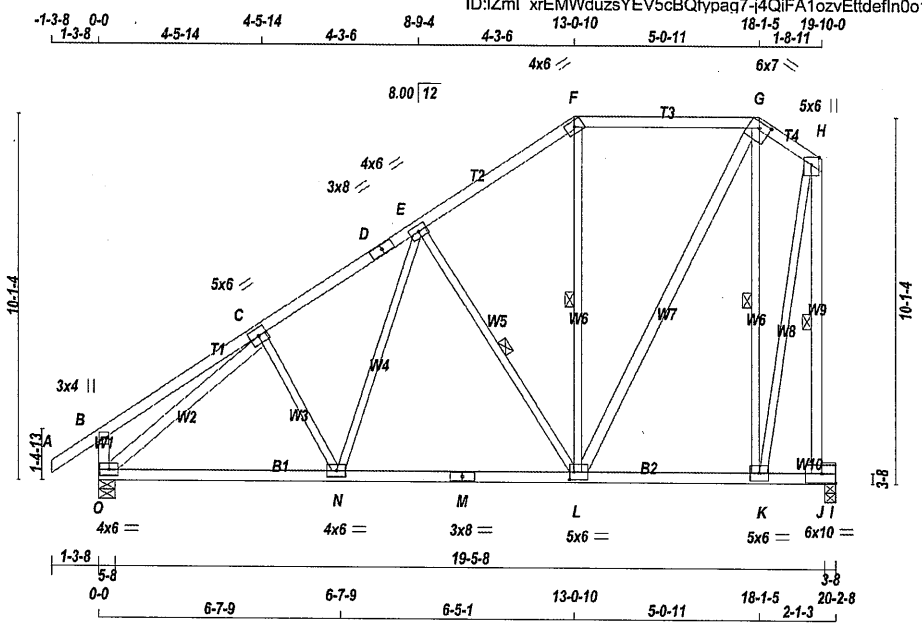

TOTAL WEIGHT = 2 X 119 = 238 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-h	MT20	4.0	6.0		
G	TTWW-h	MT20	6.0	7.0	1.75	3.50
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW-t	MT20	6.0	10.0	3.00	Edge
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	1.50
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

STRUCTURAL COMPONENT ONLY
DWG # TR23040092

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1674	0	1674	0	0	5-8	5-8
I	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
I	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L, F-L, G-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- N	-242 / 0	0.08 (1)
B- C	0 / 30	-129.6	-129.6	0.32 (1)	10.00	N- E	0 / 367	0.08 (1)
C- D	-1457 / 0	-129.6	-129.6	0.39 (1)	4.95	E- L	-750 / 0	0.34 (1)
D- E	-1457 / 0	-129.6	-129.6	0.39 (1)	4.95	L- F	-77 / 51	0.05 (1)
E- F	-877 / 0	-129.6	-129.6	0.36 (1)	6.04	L- G	0 / 909	0.15 (1)
F- G	-701 / 0	-129.6	-129.6	0.59 (1)	6.01	K- G	-1060 / 0	0.74 (1)
G- H	-340 / 0	-129.6	-129.6	0.07 (1)	6.25	O- C	-1805 / 0	0.76 (1)
O- B	-406 / 0	0.0	0.0	0.04 (1)	7.81	K- H	0 / 1515	0.34 (1)
J- H	-1790 / 0	0.0	0.0	0.69 (1)	4.99			
O- N	0 / 1332	-18.5	-18.5	0.32 (1)	10.00			
N- M	0 / 1103	-18.5	-18.5	0.28 (1)	10.00			
M- L	0 / 1103	-18.5	-18.5	0.28 (1)	10.00			
L- K	0 / 291	-18.5	-18.5	0.13 (4)	10.00			
K- J	0 / 0	-18.5	-18.5	0.74 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.69/1.00 (H-J:1) , BC=0.74/1.00 (J-K:1) , WB=0.76/1.00 (C-O:1) , SSI=0.57/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T9	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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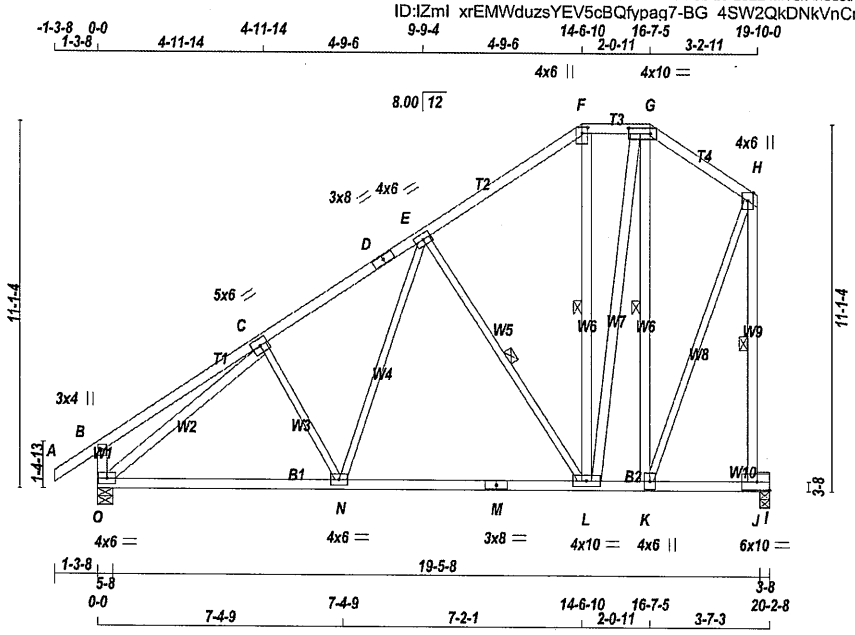

TOTAL WEIGHT = 2 X 131 = 262 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - N	2x3	DRY	No.2	SPF
N - E	2x3	DRY	No.2	SPF
E - L	2x3	DRY	No.2	SPF
K - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	2.25	2.00
G	TTWW-I	MT20	4.0	10.0	2.00	7.75
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMVW-t	MT20	6.0	10.0	3.00	Edge
K	BMWW+t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	10.0		
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1674	0	1674	0	0	5-8	5-8
I	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
I	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L, F-L, G-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- N	-330 / 0	0.14 (1)
B- C	0 / 34	-129.6	-129.6	0.41 (1)	10.00	N- E	0 / 470	0.11 (1)
C- D	-1409 / 0	-129.6	-129.6	0.49 (1)	4.87	E- L	-871 / 0	0.49 (1)
D- E	-1409 / 0	-129.6	-129.6	0.49 (1)	4.87	L- F	-8 / 48	0.01 (4)
E- F	-703 / 0	-129.6	-129.6	0.44 (1)	6.25	L- G	0 / 722	0.12 (1)
F- G	-555 / 0	-129.6	-129.6	0.10 (1)	6.25	K- G	-759 / 0	0.48 (1)
G- H	-529 / 0	-129.6	-129.6	0.24 (1)	6.25	O- C	-1790 / 0	0.96 (1)
O- B	-432 / 0	0.0	0.0	0.04 (1)	7.81	K- H	0 / 1210	0.27 (1)
J- H	-1631 / 0	0.0	0.0	0.63 (1)	5.19			
O- N	0 / 1337	-18.5	-18.5	0.36 (1)	10.00			
N- M	0 / 1029	-18.5	-18.5	0.32 (4)	10.00			
M- L	0 / 1029	-18.5	-18.5	0.32 (4)	10.00			
L- K	0 / 429	-18.5	-18.5	0.34 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.74 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.63/1.00 (H-J:1) , BC=0.74/1.00 (J-K:1) , WB=0.96/1.00 (C-O:1) , SSI=0.57/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040093

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T10	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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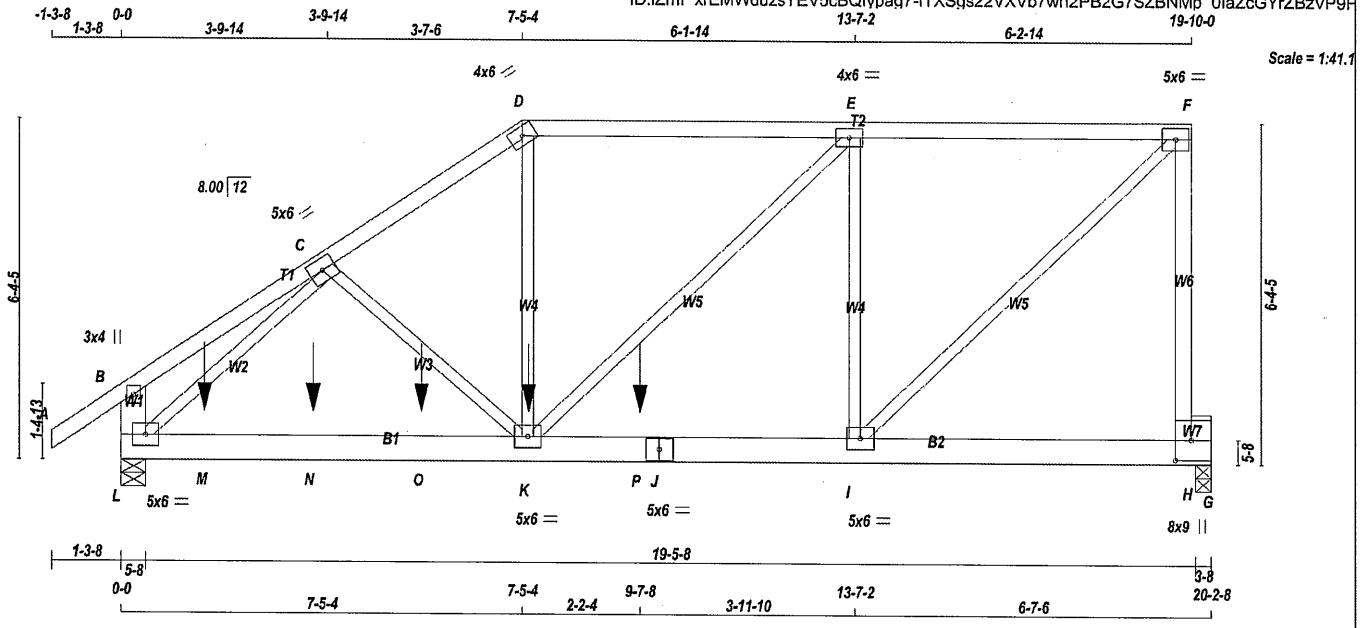

TOTAL WEIGHT = 4 X 101 = 405 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- H	1	12	TOP
L- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- J	2	12	SIDE(183.1)
J- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040094

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	3147	0	3147	0	0	5-8	5-8
G	2273	0	2273	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2182	1677 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
G	1581	1188 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	0 / 49	-129.6	-129.6	0.10 (1)	10.00		C- K	0 / 89	0.01 (4)
B- C	0 / 27	-129.6	-129.6	0.14 (1)	10.00		K- D	0 / 1276	0.16 (1)
C- D	-3217 / 0	-129.6	-129.6	0.18 (1)	5.01		L- C	-3537 / 0	0.70 (1)
D- E	-2671 / 0	-129.6	-129.6	0.53 (1)	4.87		I- F	0 / 3066	0.38 (1)
E- F	-2213 / 0	-129.6	-129.6	0.51 (1)	5.27		K- E	0 / 645	0.08 (1)
H- F	-2431 / 0	0.0	0.0	0.93 (1)	7.23		I- E	-1444 / 0	0.45 (1)
L- B	-370 / 0	0.0	0.0	0.01 (1)	7.81				
L- M	0 / 2583	-18.5	-18.5	0.54 (1)	10.00				
M- N	0 / 2583	-18.5	-18.5	0.54 (1)	10.00				
N- O	0 / 2583	-18.5	-18.5	0.54 (1)	10.00				
O- K	0 / 2583	-18.5	-18.5	0.54 (1)	10.00				
K- P	0 / 2213	-18.5	-18.5	0.51 (1)	10.00				
P- J	0 / 2213	-18.5	-18.5	0.51 (1)	10.00				
J- I	0 / 2213	-18.5	-18.5	0.51 (1)	10.00				
I- H	0 / 0	-18.5	-18.5	0.31 (1)	10.00				
H- G	0 / 0	-18.5	-18.5	0.13 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	7-6-12	-185	-185	—	BACK	VERT	TOTAL	—	C1
M	1-6-12	-185	-185	—	BACK	VERT	TOTAL	—	C1
N	3-6-12	-185	-185	—	BACK	VERT	TOTAL	—	C1
O	5-6-12	-185	-185	—	BACK	VERT	TOTAL	—	C1
P	9-7-8	-848	-848	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.93/1.00 (F-H:1) , BC=0.54/1.00 (K-L:1) , WB=0.70/1.00 (C-L:1) , SSI=0.38/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.56 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402463	T10	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW+p	MT20	8.0	9.0	4.50	3.50
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	5.0	6.0		

STRUCTURAL COMPONENT ONLY
DWG # TR23040094

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T11	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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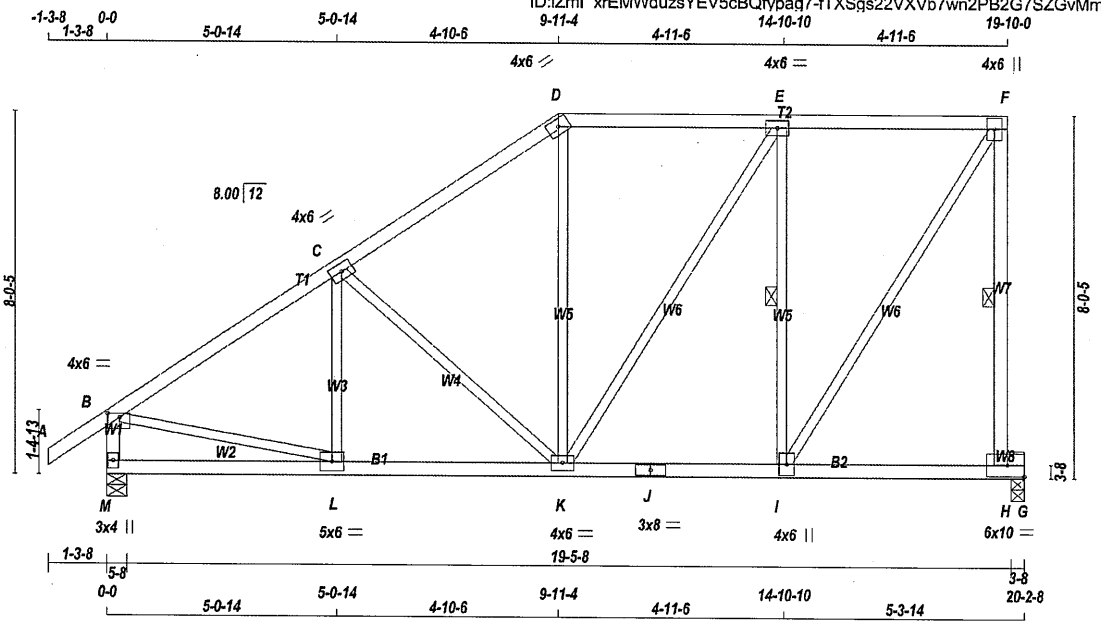

TOTAL WEIGHT = 2 X 99 = 198 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW-t	MT20	6.0	10.0	3.00	Edge
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1674	0	1674	0	0	5-8	5-8
G	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-1605 / 0	-129.6	-129.6	0.64 (1)	4.37
C- D	-1214 / 0	-129.6	-129.6	0.59 (1)	4.93
D- E	-974 / 0	-129.6	-129.6	0.55 (1)	5.41
E- F	-835 / 0	-129.6	-129.6	0.55 (1)	5.74
H- F	-1541 / 0	0.0	0.0	0.45 (1)	5.30
M- B	-1636 / 0	0.0	0.0	0.17 (1)	6.47
M- L	0 / 0	-18.5	-18.5	0.11 (4)	10.00
L- K	0 / 1371	-18.5	-18.5	0.28 (1)	10.00
K- J	0 / 835	-18.5	-18.5	0.39 (1)	10.00
J- I	0 / 835	-18.5	-18.5	0.39 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.74 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.31 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-179 / 47	0.06 (1)
C- K	-536 / 0	0.44 (1)
K- D	0 / 172	0.04 (1)
K- E	0 / 264	0.06 (1)
I- E	-1017 / 0	0.41 (1)
I- F	0 / 1540	0.35 (1)
B- L	0 / 1401	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.74/1.00 (H-I:1) , WB=0.44/1.00 (C-K:1) , SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040095

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T12	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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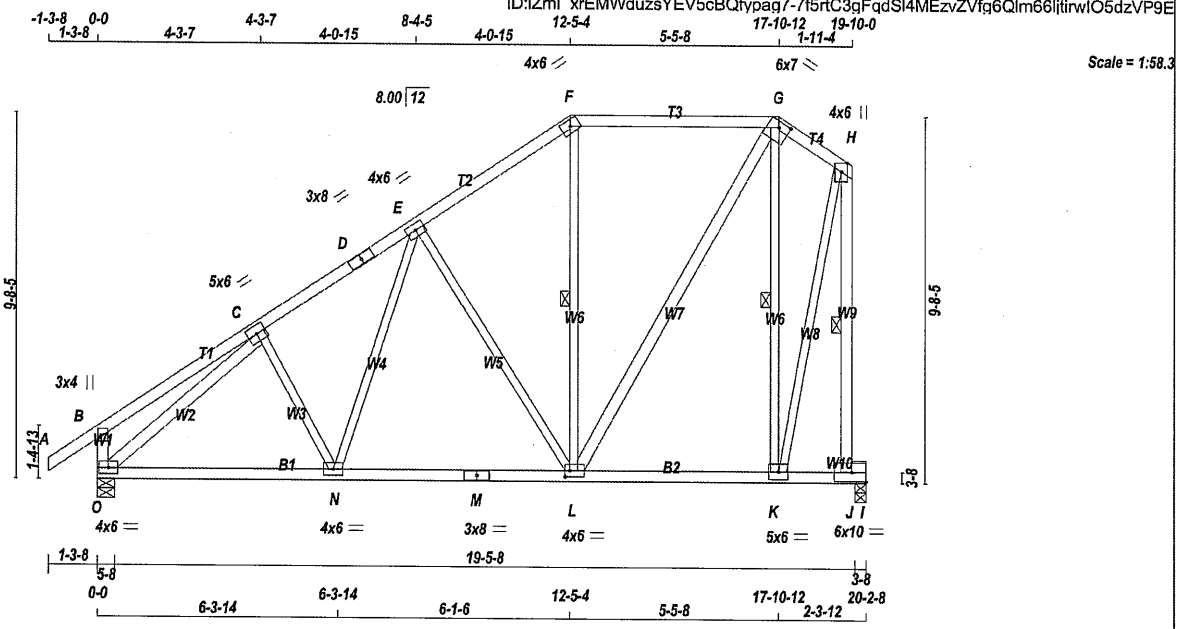

TOTAL WEIGHT = 2 X 116 = 232 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-h	MT20	4.0	6.0		
G	TTWW-h	MT20	6.0	7.0	1.75	3.50
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMVW-t	MT20	6.0	10.0	3.00	4.50
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	6.0	2.00	1.50
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040096

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1674	0	1674	0	0	5-8	5-8
I	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
I	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-L, G-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- N	-204 / 7	0.07 (1)
B- C	0 / 28	-129.6	-129.6	0.29 (1)	10.00	N- E	0 / 326	0.07 (1)
C- D	-1475 / 0	-129.6	-129.6	0.35 (1)	4.98	E- L	-702 / 0	0.89 (1)
D- E	-1475 / 0	-129.6	-129.6	0.35 (1)	4.98	L- F	-55 / 59	0.03 (1)
E- F	-943 / 0	-129.6	-129.6	0.32 (1)	5.93	L- G	0 / 852	0.14 (1)
F- G	-758 / 0	-129.6	-129.6	0.69 (1)	5.53	K- G	-986 / 0	0.62 (1)
G- H	-418 / 0	-129.6	-129.6	0.08 (1)	6.25	O- C	-1811 / 0	0.69 (1)
O- B	-396 / 0	0.0	0.0	0.04 (1)	7.81	K- H	0 / 1415	0.32 (1)
J- H	-1731 / 0	0.0	0.0	0.57 (1)	5.07			
O- N	0 / 1328	-18.5	-18.5	0.31 (1)	10.00			
N- M	0 / 1131	-18.5	-18.5	0.27 (1)	10.00			
M- L	0 / 1131	-18.5	-18.5	0.27 (1)	10.00			
L- K	0 / 336	-18.5	-18.5	0.17 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.74 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.69/1.00 (F-G:1) , BC=0.74/1.00 (J-K:1) , WB=0.89/1.00 (E-L:1) , SSI=0.57/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.43 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T13A	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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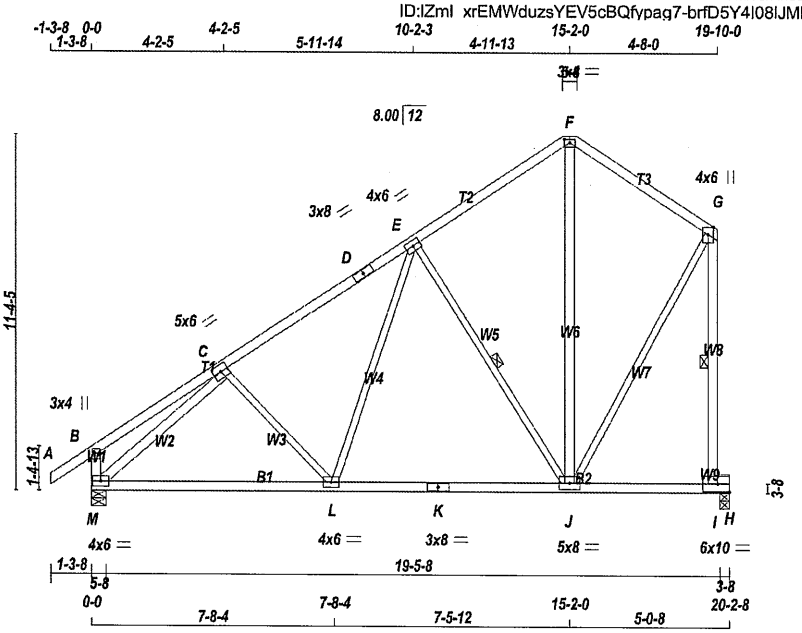

Scale = 1:70.3

TOTAL WEIGHT = 2 X 106 = 213 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-p	MT20	3.0	4.0	1.25	2.00
G	TMVW+p	MT20	4.0	6.0	Edge	
I	BMVW-t	MT20	6.0	10.0	3.00	Edge
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1674	0	1674	0	0	5-8	5-8
H	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- L		-313 / 0	0.14 (1)
B- C		0 / 45	-129.6	-129.6	0.46 (1)	10.00	L- E		0 / 385	0.09 (1)
C- D		-1351 / 0	-129.6	-129.6	0.70 (1)	4.61	E- J		-919 / 0	0.56 (1)
D- E		-1351 / 0	-129.6	-129.6	0.70 (1)	4.61	J- F		0 / 153	0.03 (4)
E- F		-672 / 0	-129.6	-129.6	0.60 (1)	6.09	M- C		-1838 / 0	0.67 (1)
F- G		-626 / 0	-129.6	-129.6	0.50 (1)	6.25	J- G		0 / 1041	0.23 (1)
M- B		-361 / 0	0.0	0.0	0.04 (1)	7.81				
I- G		-1551 / 0	0.0	0.0	0.52 (1)	5.29				
M- L		0 / 1345	-18.5	-18.5	0.35 (1)	10.00				
L- K		0 / 1012	-18.5	-18.5	0.40 (1)	10.00				
K- J		0 / 1012	-18.5	-18.5	0.40 (1)	10.00				
J- I		0 / 0	-18.5	-18.5	0.74 (1)	10.00				
I- H		0 / 0	-18.5	-18.5	0.31 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.70/1.00 (C-E:1) , BC=0.74/1.00 (I-J:1) , WB=0.67/1.00 (C-M:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040097

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	G14	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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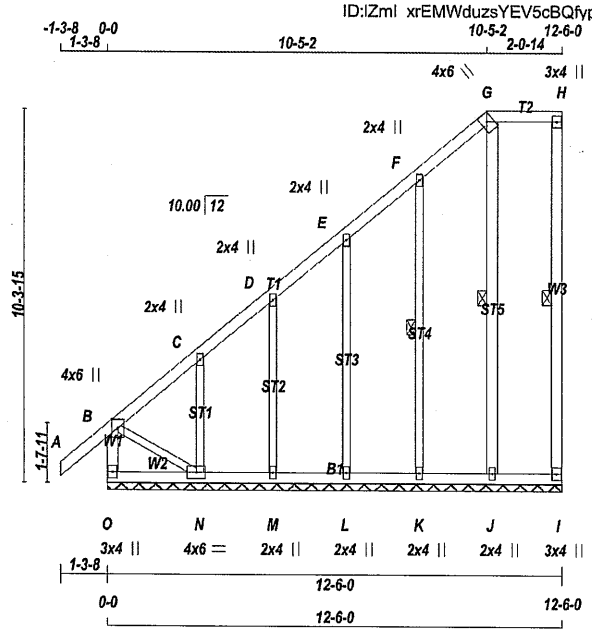

Scale = 1:61.0

TOTAL WEIGHT = 2 X 78 = 157 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - G	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F						
C	TMW+w	MT20	2.0	4.0		
G	TTW+h	MT20	4.0	6.0	Edge	
H	TMV+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, L, M						
J	BMW1+w	MT20	2.0	4.0		
N	BMWW1-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, G-J, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- B	-342 / 0	0.0	0.0	0.04 (1)	7.81	J- G	-233 / 0	0.12 (1)
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	K- F	-280 / 0	0.14 (1)
B- C	-12 / 0	-129.6	-129.6	0.10 (1)	6.25	L- E	-258 / 0	0.23 (1)
C- D	-31 / 0	-129.6	-129.6	0.10 (1)	6.25	M- D	-233 / 0	0.10 (1)
D- E	-11 / 0	-129.6	-129.6	0.06 (1)	6.25	N- C	-331 / 0	0.08 (1)
E- F	-7 / 0	-129.6	-129.6	0.07 (1)	10.00	B- N	0 / 26	0.01 (1)
F- G	-20 / 0	-129.6	-129.6	0.07 (1)	6.25			
G- H	0 / 0	-129.6	-129.6	0.09 (1)	10.00			
I- H	-135 / 0	0.0	0.0	0.07 (1)	6.25			
O- N	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
N- M	0 / 14	-18.5	-18.5	0.03 (4)	10.00			
M- L	0 / 10	-18.5	-18.5	0.02 (4)	10.00			
L- K	0 / 7	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 3	-18.5	-18.5	0.02 (4)	10.00			
J- I	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.03/1.00 (M-N:4) , WB=0.23/1.00 (E-L:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (C) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040074

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T15	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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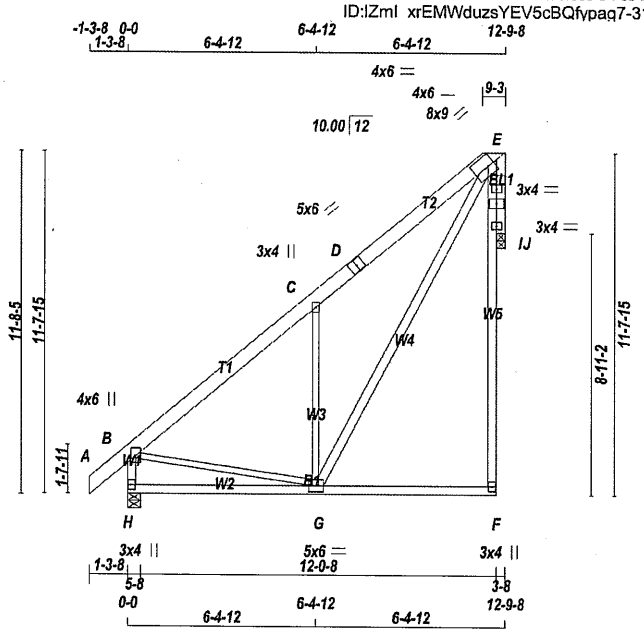

TOTAL WEIGHT = 2 X 88 = 176 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		
C	TMW+w	MT20	3.0	4.0		
D	TS-t	MT20	5.0	6.0		
E	TMVW1-t	MT20	8.0	9.0	4.50	2.75
E	KP-p	MT20	4.0	6.0	3.50	2.50
E	KP-p	MT20	4.0	6.0		
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(E)	915	0	915	0	0	3-8	3-8 (** SEE "BEARING NOTE" **)
H	1123	0	1123	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	638	472 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0
H	780	593 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-129.6	-129.6	0.10 (1)	10.00	G- C	-969 / 0	0.74 (1)
B- C	-678 / 0	-129.6	-129.6	0.30 (1)	6.25	G- E	0 / 1131	0.18 (1)
C- D	-753 / 0	-129.6	-129.6	0.30 (1)	6.25	B- G	0 / 576	0.13 (1)
D- E	-753 / 0	-129.6	-129.6	0.30 (1)	6.25	E- J	-916 / 0	0.17 (1)
F- I	0 / 46	0.0	0.0	0.14 (1)	10.00	I- J	0 / 47	0.00 (1)
I- E	0 / 46	0.0	0.0	0.14 (1)	10.00			
H- B	-1075 / 0	0.0	0.0	0.11 (1)	7.61			
H- G	0 / 0	-18.5	-18.5	0.21 (4)	10.00			
G- F	0 / 11	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.30/1.00 (C-E:1) , BC=0.21/1.00 (F-G:4) , WB=0.74/1.00 (C-G:1) , SSI=0.23/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (G) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040098

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T16	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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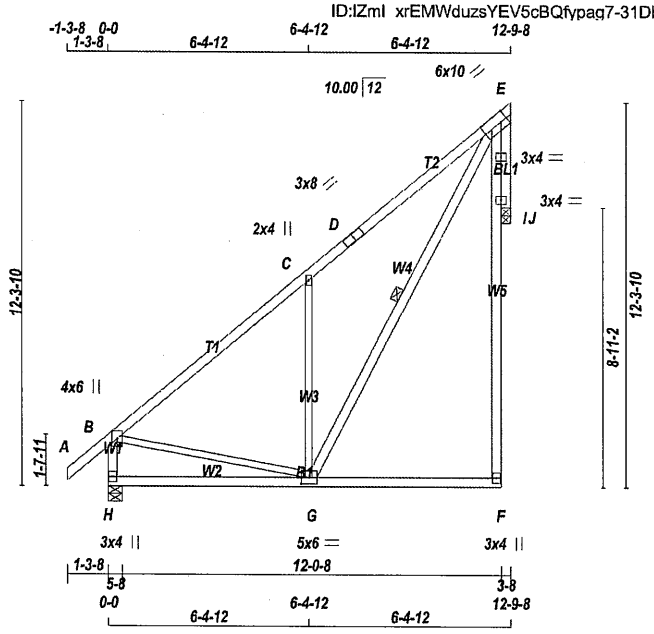

TOTAL WEIGHT = 8 X 78 = 621 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVWK1-t	MT20	6.0	10.0	Edge	2.50
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(E)	915	0	915	0	0	3-8	3-8 (** SEE "BEARING NOTE" **)
H	1116	0	1116	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	638	472 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0
H	775	589 / 0	0 / 0	0 / 0	0 / 0	186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.6	-129.6	0.18 (1)		10.00
B- C	-665 / 0	-129.6	-129.6	0.68 (1)		5.94
C- D	-757 / 0	-129.6	-129.6	0.68 (1)		5.69
D- E	-757 / 0	-129.6	-129.6	0.68 (1)		5.69
F- I	0 / 46	0.0	0.0	0.14 (1)		10.00
I- E	0 / 46	0.0	0.0	0.14 (1)		10.00
H- B	-1068 / 0	0.0	0.0	0.11 (1)		7.63
H- G	0 / 0	-18.5	-18.5	0.21 (4)		10.00
G- F	0 / 11	-18.5	-18.5	0.21 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
G- C	-987 / 0	0.82 (1)
G- E	0 / 1140	0.18 (1)
B- G	0 / 570	0.13 (1)
E- J	-916 / 0	0.18 (1)
I- J	0 / 45	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.68/1.00 (B-C:1) , BC=0.21/1.00 (F-G:4) , WB=0.82/1.00 (C-G:1) , SSI=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (G) (INPUT = 0.90)
JSI METAL= 0.53 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040099

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T17	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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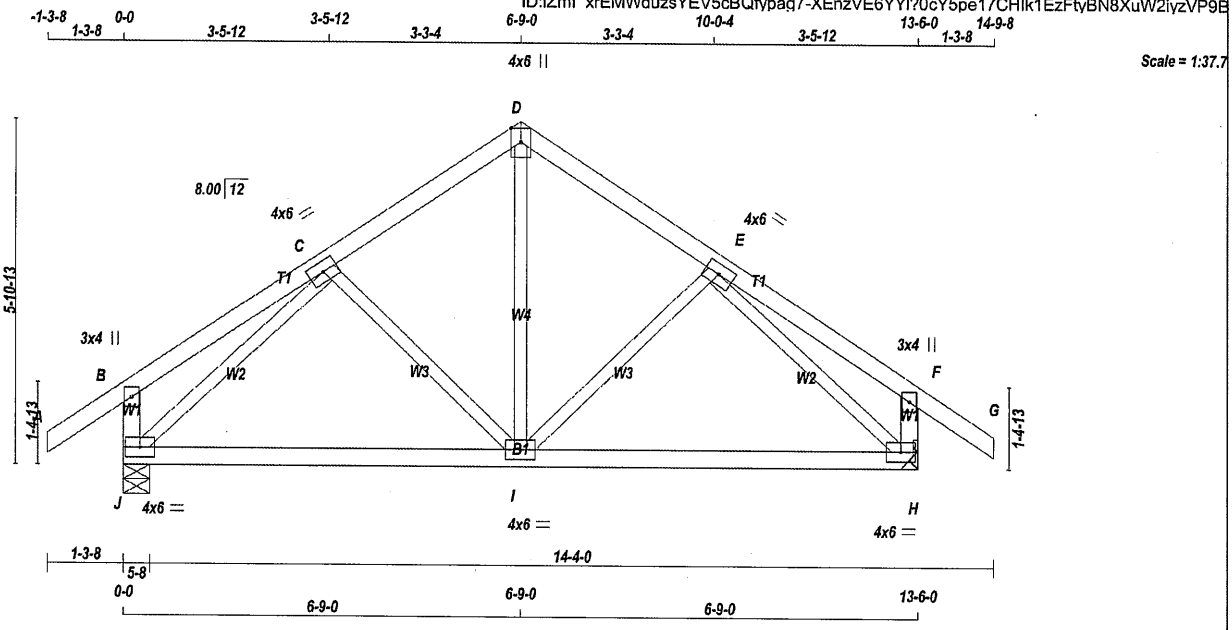

TOTAL WEIGHT = 6 X 59 = 352 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1178	0	1178	0	0	5-8	5-8
H	1178	0	1178	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	818	621 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0
H	818	621 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	0 / 27	-129.6	-129.6	0.23 (1)	10.00
C- D	-775 / 0	-129.6	-129.6	0.18 (1)	6.25
D- E	-775 / 0	-129.6	-129.6	0.18 (1)	6.25
E- F	0 / 27	-129.6	-129.6	0.23 (1)	10.00
F- G	0 / 49	-129.6	-129.6	0.17 (1)	10.00
J- B	-347 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-347 / 0	0.0	0.0	0.04 (1)	7.81
J- I	0 / 793	-18.5	-18.5	0.29 (4)	10.00
I- H	0 / 793	-18.5	-18.5	0.29 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 488	0.11 (1)
I- E	-240 / 0	0.09 (1)
C- I	-240 / 0	0.09 (1)
J- C	-1114 / 0	0.40 (1)
E- H	-1114 / 0	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.23/1.00 (E-F:1) , BC=0.29/1.00 (I-J:4) , WB=0.40/1.00 (E-H:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (J) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040100

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	G17	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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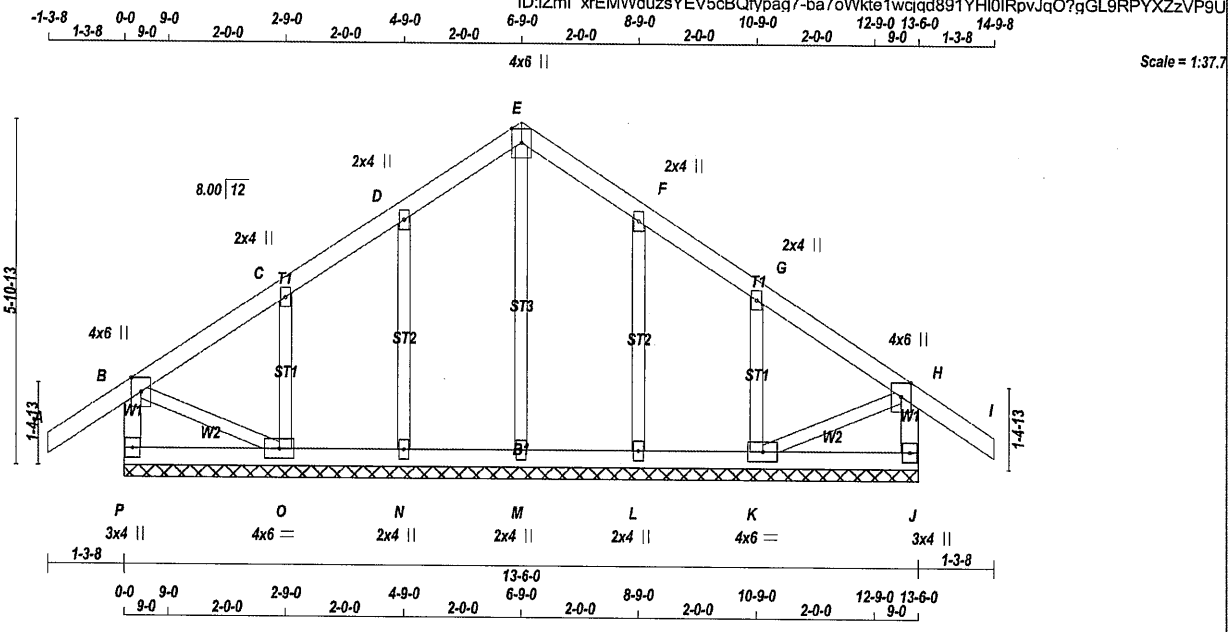

TOTAL WEIGHT = 2 X 59 = 119 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00		M- E	-196 / 0	0.10 (1)
B- C	-27 / 0	-129.6	-129.6	0.11 (1)	6.25		N- D	-266 / 0	0.08 (1)
C- D	-42 / 0	-129.6	-129.6	0.11 (1)	6.25		O- C	-345 / 0	0.07 (1)
D- E	-38 / 0	-129.6	-129.6	0.07 (1)	6.25		L- F	-266 / 0	0.08 (1)
E- F	-38 / 0	-129.6	-129.6	0.07 (1)	6.25		K- G	-345 / 0	0.07 (1)
F- G	-42 / 0	-129.6	-129.6	0.11 (1)	6.25		B- O	0 / 39	0.01 (1)
G- H	-27 / 0	-129.6	-129.6	0.11 (1)	6.25		K- H	0 / 39	0.01 (1)
H- I	0 / 49	-129.6	-129.6	0.17 (1)	10.00				
P- B	-359 / 0	0.0	0.0	0.04 (1)	7.81				
J- H	-359 / 0	0.0	0.0	0.04 (1)	7.81				
P- O	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
O- N	0 / 27	-18.5	-18.5	0.03 (4)	10.00				
N- M	0 / 22	-18.5	-18.5	0.02 (4)	10.00				
M- L	0 / 22	-18.5	-18.5	0.02 (4)	10.00				
L- K	0 / 27	-18.5	-18.5	0.03 (4)	10.00				
K- J	0 / 0	-18.5	-18.5	0.03 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.03/1.00 (K-L:4) , WB=0.10/1.00 (E-M:1) , SSI=0.13/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (C) (INPUT = 0.90)
JSI METAL= 0.18 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040075

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T18	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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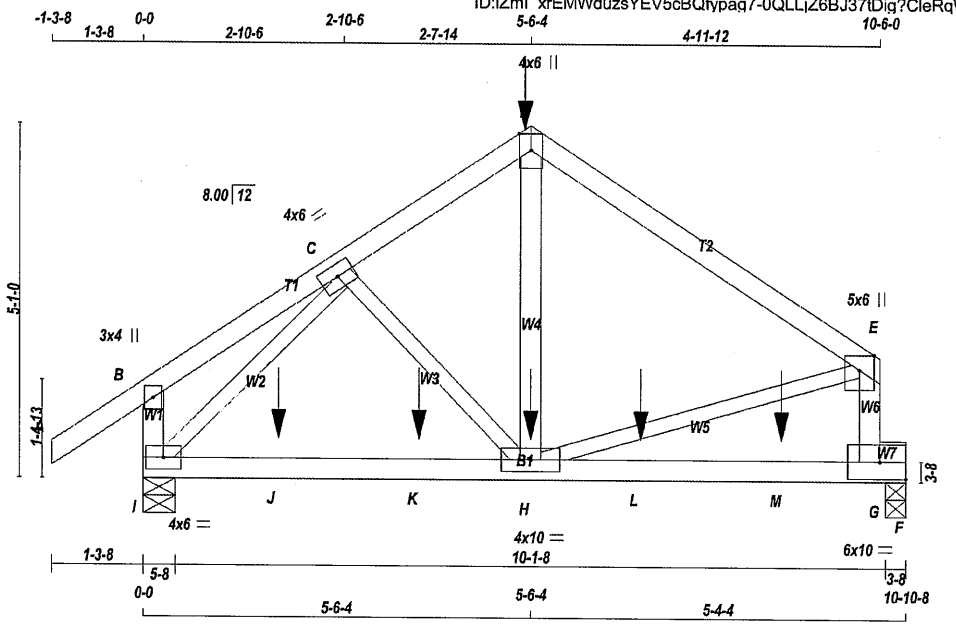

TOTAL WEIGHT = 2 X 47 = 94 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMVW-t	MT20	6.0	10.0	3.00	4.50
H	BMWWW-t	MT20	4.0	10.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1449	0	1449	0	0	5-8	5-8
F	1238	0	1238	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1004	777 / 0	0 / 0	0 / 0	0 / 0	227 / 0	0 / 0
F	860	652 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 49	-129.6	-129.6	0.19 (1)	10.00		C- H		-21 / 24	0.01 (4)
B- C		0 / 20	-129.6	-129.6	0.15 (1)	10.00		H- D		0 / 165	0.04 (4)
C- D		-1244 / 0	-129.6	-129.6	0.18 (1)	5.53		I- C		-1505 / 0	0.42 (1)
D- E		-1224 / 0	-129.6	-129.6	0.68 (1)	4.68		H- E		0 / 1060	0.26 (1)
I- B		-319 / 0	0.0	0.0	0.04 (1)	7.81					
G- E		-1296 / 0	0.0	0.0	0.15 (1)	7.05					
I- J		0 / 1033	-18.5	-18.5	0.47 (1)	10.00					
J- K		0 / 1033	-18.5	-18.5	0.47 (1)	10.00					
K- H		0 / 1033	-18.5	-18.5	0.47 (1)	10.00					
H- L		0 / 0	-18.5	-18.5	0.69 (1)	10.00					
L- M		0 / 0	-18.5	-18.5	0.69 (1)	10.00					
M- G		0 / 0	-18.5	-18.5	0.69 (1)	10.00					
G- F		0 / 0	-18.5	-18.5	0.29 (1)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-6-4	-40	-40	---	FRONT	VERT	DEAD	---	C1
D	5-6-4	-94	-94	---	BACK	VERT	TOTAL	---	C1
D	5-6-4	-440	-440	---	FRONT	VERT	SNOW	---	C1
H	5-6-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
J	1-11-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
K	3-11-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
L	7-1-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
M	9-1-4	-16	-16	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 659 (0.20")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.69/1.00 (G-H:1) , WB=0.42/1.00 (C-I:1) , SSI=0.53/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.52 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040101

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T19	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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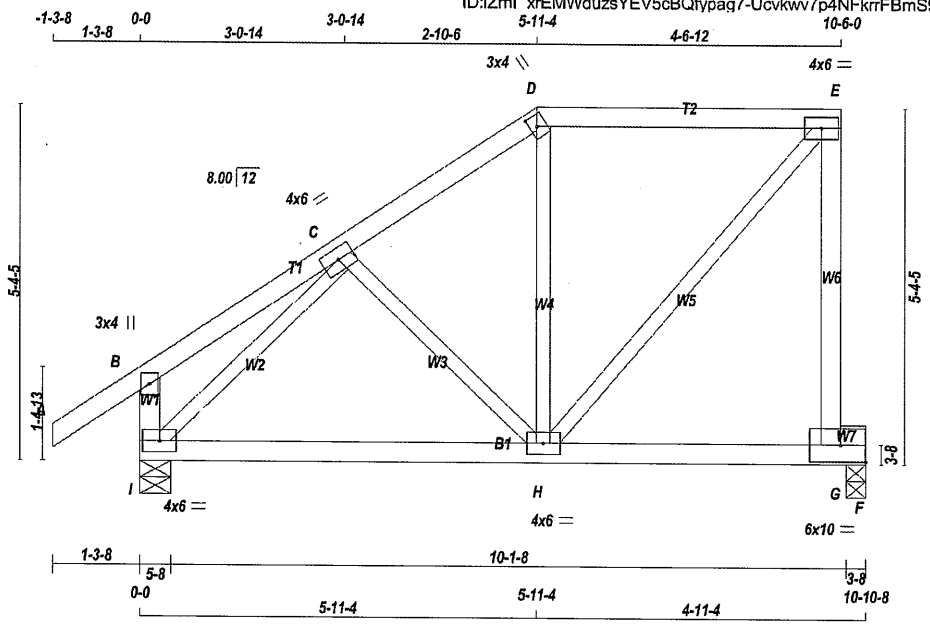

Scale = 1:33.2

TOTAL WEIGHT = 2 X 50 = 100 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0		Edge
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	758	0	758	0	0	3-8	3-8
I	983	0	983	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	529	387 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0
I	682	520 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- H	-217 / 0	0.07 (1)
B- C	0 / 24	-129.6	-129.6	0.18 (1)	10.00	H- D	-146 / 25	0.06 (1)
C- D	-563 / 0	-129.6	-129.6	0.18 (1)	6.25	H- E	0 / 673	0.15 (1)
D- E	-444 / 0	-129.6	-129.6	0.47 (1)	6.25	I- C	-864 / 0	0.25 (1)
G- E	-799 / 0	0.0	0.0	0.40 (1)	7.81			
I- B	-326 / 0	0.0	0.0	0.03 (1)	7.81			
I- H	0 / 602	-18.5	-18.5	0.26 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.38 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.47/1.00 (D-E:1) , BC=0.38/1.00 (G-H:1) , WB=0.25/1.00 (C-I:1) , SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (H) (INPUT = 0.90)
JSI METAL= 0.20 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040102

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T20	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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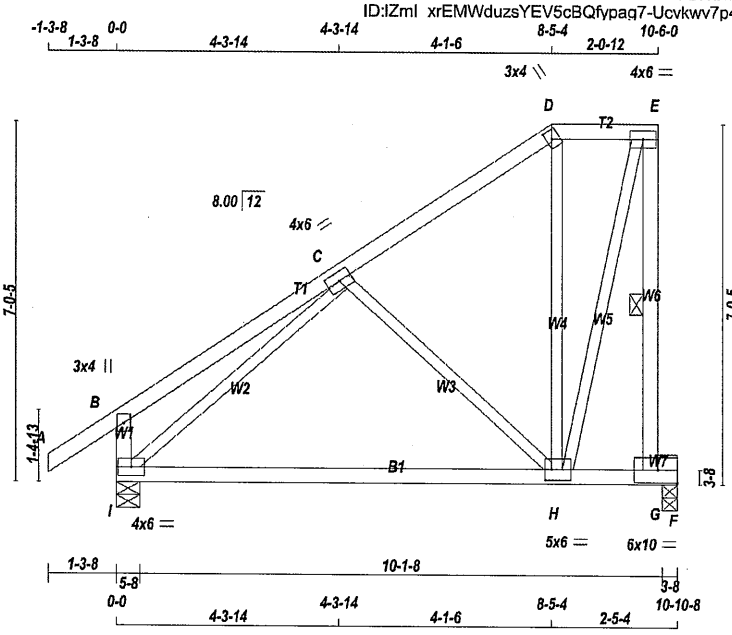

TOTAL WEIGHT = 2 X 57 = 113 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	Edge
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	758	0	758	0	0	3-8	3-8
I	983	0	983	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	529	387 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0
I	682	520 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- H	-458 / 0	0.26 (1)	
B- C	0 / 36	-129.6	-129.6	0.39 (1)	10.00	H- D	-173 / 9	0.15 (1)	
C- D	-328 / 0	-129.6	-129.6	0.39 (1)	6.25	H- E	0 / 854	0.19 (1)	
D- E	-238 / 0	-129.6	-129.6	0.09 (1)	6.25	I- C	-781 / 0	0.45 (1)	
G- E	-952 / 0	0.0	0.0	0.21 (1)	6.25				
I- B	-393 / 0	0.0	0.0	0.04 (1)	7.81				
I- H	0 / 576	-18.5	-18.5	0.36 (4)	10.00				
H- G	0 / 0	-18.5	-18.5	0.38 (1)	10.00				
G- F	0 / 0	-18.5	-18.5	0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 981 (0.13")

CSI: TC=0.39/1.00 (C-D:1) , BC=0.38/1.00 (G-H:1) , WB=0.45/1.00 (C-I:1) , SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.23 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040103

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T21	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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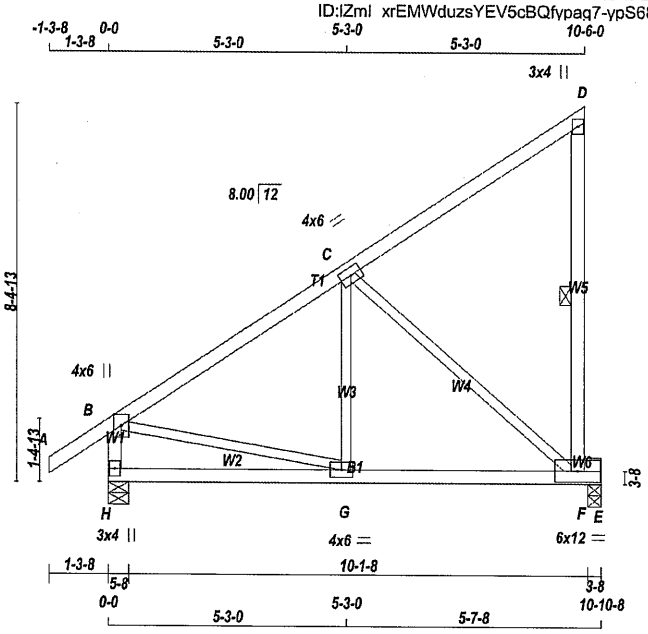

Scale = 1:49.0

TOTAL WEIGHT = 4 X 52 = 208 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	983	0	983	0	0	5-8	5-8
E	758	0	758	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	682	520 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
E	529	387 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	G- C	0 / 124	0.04 (4)
B- C	-674 / 0	-129.6	-129.6	0.64 (1)	5.99	C- F	-796 / 0	0.69 (1)
C- D	-46 / 0	-129.6	-129.6	0.60 (1)	6.25	B- G	0 / 612	0.14 (1)
F- D	-257 / 0	0.0	0.0	0.08 (1)	6.25			
H- B	-958 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
G- F	0 / 600	-18.5	-18.5	0.48 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 962 (0.14")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.48/1.00 (F-G:1) , WB=0.69/1.00 (C-F:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040104

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T21A	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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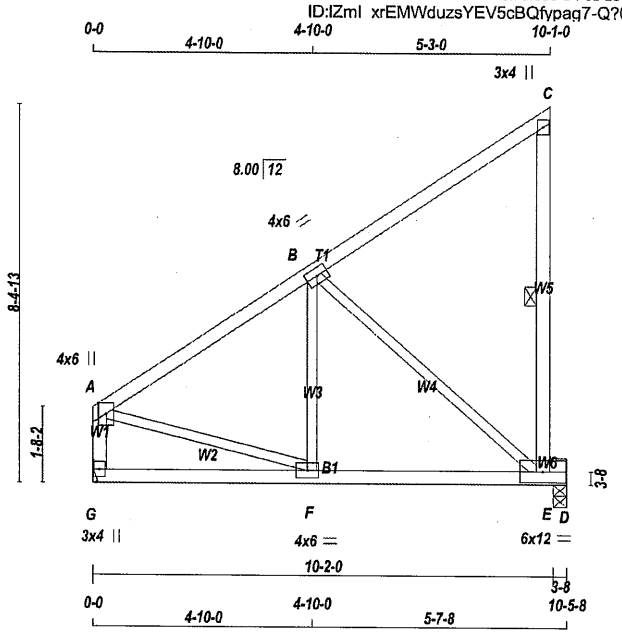

TOTAL WEIGHT = 4 X 49 = 197 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	6.0	12.0		
F	BMWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040105

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	774	0	774	0	0	MECHANICAL	
D	727	0	727	0	0	3-8	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	539	399 / 0	0 / 0	0 / 0	0 / 0	140 / 0	0 / 0
D	507	372 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-619 / 0	-129.6	-129.6	0.58 (1)	6.25	F- B	0 / 111	0.04 (4)
B- C	-43 / 0	-129.6	-129.6	0.56 (1)	6.25	B- E	-735 / 0	0.64 (1)
E- C	-263 / 0	0.0	0.0	0.08 (1)	6.25	A- F	0 / 575	0.13 (1)
G- A	-754 / 0	0.0	0.0	0.08 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
F- E	0 / 554	-18.5	-18.5	0.46 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.15 (1)	10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 965 (0.13")

CSI: TC=0.58/1.00 (A-B:1) , BC=0.46/1.00 (E-F:1) , WB=0.64/1.00 (B-E:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (A) (INPUT = 0.90)
JSI METAL= 0.36 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T22	24	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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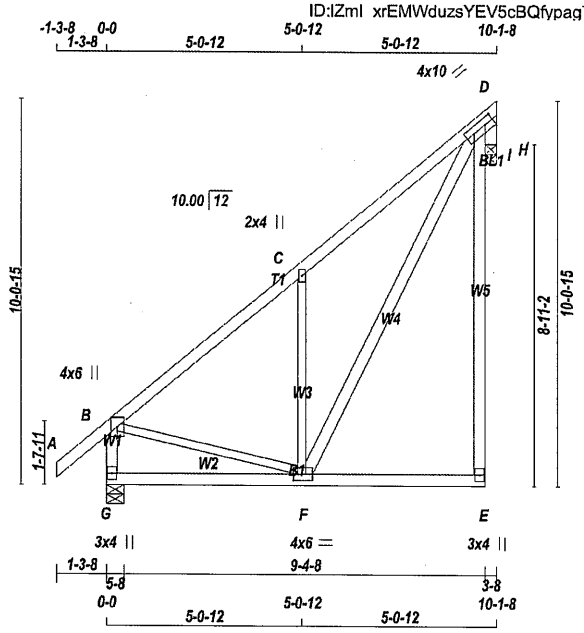

Scale = 1:57.6

TOTAL WEIGHT = 24 X 61 = 1457 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TMVWK1-t	MT20	4.0	10.0	2.00	2.75
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0	2.00	1.50
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	919	0	919	0	0	5-8	5-8
H	718	0	718	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	637	487 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
H	500	370 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00		F- C	-771 / 0	0.41 (1)
B- C	-495 / 0	-129.6	-129.6	0.40 (1)	6.25		F- D	0 / 863	0.14 (1)
C- D	-561 / 0	-129.6	-129.6	0.40 (1)	6.25		B- F	0 / 431	0.10 (1)
E- I	0 / 36	0.0	0.0	0.14 (1)	10.00		D- H	-732 / 0	0.00 (1)
I- D	0 / 36	0.0	0.0	0.14 (1)	10.00		I- H	0 / 146	0.00 (1)
G- B	-879 / 0	0.0	0.0	0.09 (1)	7.81				
G- F	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
F- E	0 / 11	-18.5	-18.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.40/1.00 (C-D:1) , BC=0.13/1.00 (E-F:4) , WB=0.41/1.00 (C-F:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040106

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	G23	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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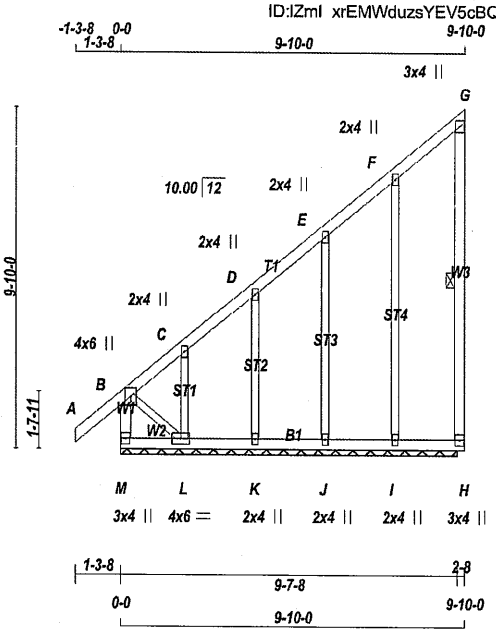

TOTAL WEIGHT = 4 X 58 = 232 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
M - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F						
C	TMW+w	MT20	2.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMWW1-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-382 / 0	0.0	0.0	0.04 (1)	7.81	I- F	-293 / 0	0.38 (1)
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	J- E	-247 / 0	0.17 (1)
B- C	-63 / 0	-129.6	-129.6	0.17 (1)	6.25	K- D	-274 / 0	0.09 (1)
C- D	-7 / 0	-129.6	-129.6	0.07 (1)	10.00	L- C	-161 / 0	0.03 (1)
D- E	-12 / 0	-129.6	-129.6	0.07 (1)	6.25	B- L	0 / 23	0.01 (1)
E- F	-1 / 0	-129.6	-129.6	0.07 (1)	10.00			
F- G	-16 / 0	-129.6	-129.6	0.07 (1)	6.25			
H- G	-111 / 0	0.0	0.0	0.05 (1)	6.25			
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
L- K	0 / 13	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 7	-18.5	-18.5	0.01 (4)	10.00			
J- I	0 / 4	-18.5	-18.5	0.02 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (H-I:4) , WB=0.38/1.00 (F-I:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040076

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T24	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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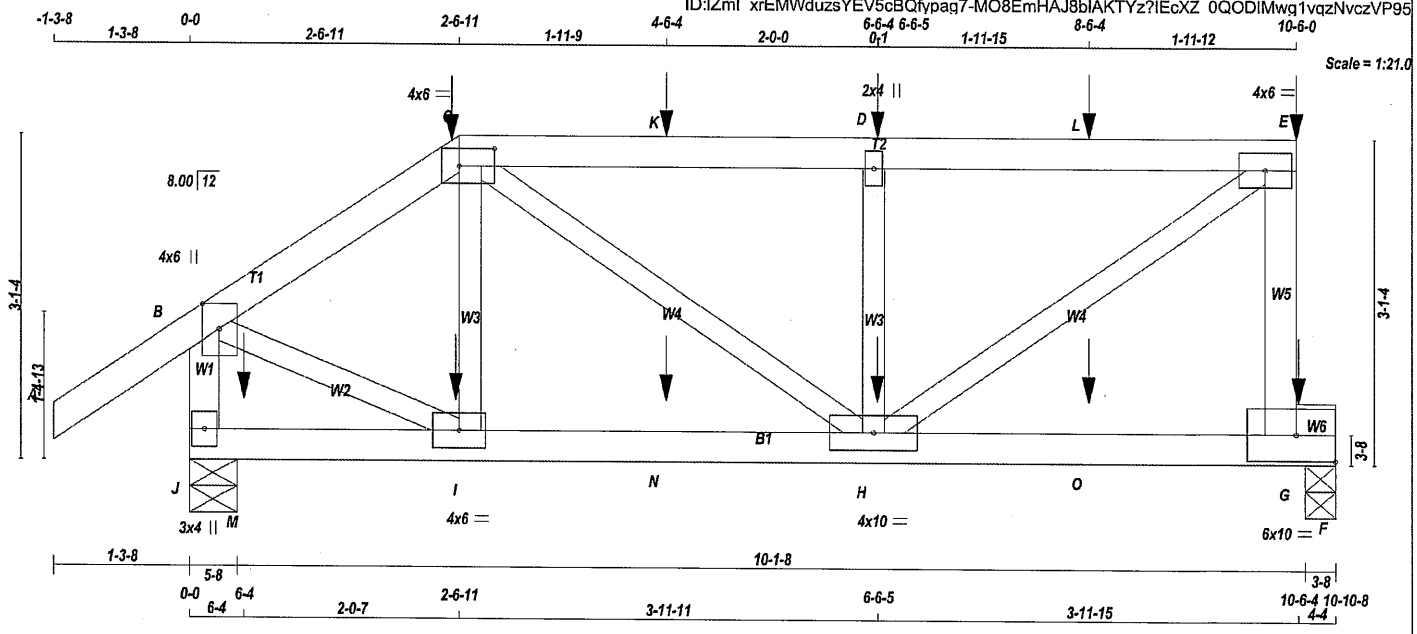

TOTAL WEIGHT = 2 X 45 = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	4.50
H	BMWWW-t	MT20	4.0	10.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

STRUCTURAL COMPONENT ONLY
DWG # TR23040107

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1275	0	1275	0	0	5-8	5-8
F	1081	0	1081	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	888	663 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
F	755	551 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
	A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00
	B- C	-994 / 0	-129.6	-129.6	0.17 (1)	6.02
	C- K	-1138 / 0	-129.6	-129.6	0.44 (1)	5.29
	K- D	-1138 / 0	-129.6	-129.6	0.44 (1)	5.29
	D- L	-1139 / 0	-129.6	-129.6	0.44 (1)	5.29
	L- E	-1139 / 0	-129.6	-129.6	0.44 (1)	5.29
	G- E	-1145 / 0	0.0	0.0	0.19 (1)	7.40
	J- B	-1233 / 0	0.0	0.0	0.14 (1)	7.19
	J- M	0 / 0	-18.5	-18.5	0.04 (4)	10.00
	M- I	0 / 0	-18.5	-18.5	0.04 (4)	10.00
	I- N	0 / 814	-18.5	-18.5	0.33 (1)	10.00
	N- H	0 / 814	-18.5	-18.5	0.33 (1)	10.00
	H- O	0 / 0	-18.5	-18.5	0.60 (1)	10.00
	O- G	0 / 0	-18.5	-18.5	0.60 (1)	10.00
	G- F	0 / 0	-18.5	-18.5	0.25 (1)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
	I- C	-294 / 0	0.06 (1)
	H- D	0 / 399	0.10 (1)
	H- E	-765 / 0	0.15 (1)
	B- I	0 / 1390	0.34 (1)
		0 / 890	0.22 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-6-11	-20	-20	---	FRONT	VERT	DEAD	---	C1
C	2-6-11	-46	-46	---	BACK	VERT	TOTAL	---	C1
C	2-6-11	-94	-94	---	FRONT	VERT	SNOW	---	C1
D	6-6-4	-40	-40	---	BACK	VERT	TOTAL	---	C1
E	10-6-0	-85	-85	---	BACK	VERT	TOTAL	---	C1
G	10-6-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
H	6-6-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
I	2-6-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
K	4-6-4	-40	-40	---	BACK	VERT	TOTAL	---	C1
L	8-6-4	-40	-40	---	BACK	VERT	TOTAL	---	C1
M	6-4	-13	-13	---	BACK	VERT	TOTAL	---	C1
N	4-6-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
O	8-6-4	-10	-10	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.60/1.00 (G-H:1) , WB=0.34/1.00 (E-H:1) , SSI=0.46/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T25	2	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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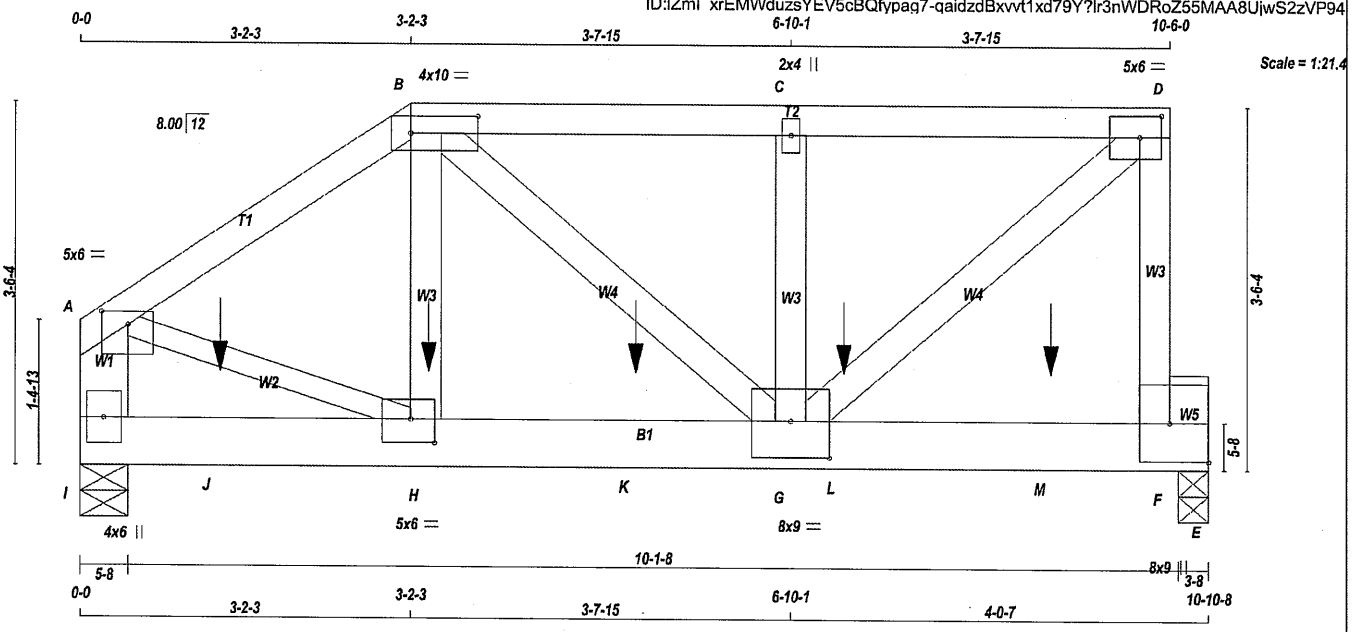

TOTAL WEIGHT = 6 X 56 = 337 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
A - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- D	1	12	TOP
D- F	1	12	TOP
I- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- E	2	7	SIDE(549.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	4	
G- B	1	6	
C- G	1	6	
G- D	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040108

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	5840	0	5840	0	0	5-8	5-8
E	5630	0	5630	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	4064	3037 / 0	0 / 0	0 / 0	0 / 0	1027 / 0	0 / 0
E	3919	2925 / 0	0 / 0	0 / 0	0 / 0	995 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	-5681 / 0	-129.6	-129.6	0.12 (1)	4.79	H- B		0 / 2098	0.11 (1)
B- C	-5562 / 0	-129.6	-129.6	0.11 (1)	4.84	B- G		0 / 984	0.05 (1)
C- D	-5562 / 0	-129.6	-129.6	0.11 (1)	4.84	G- C		-522 / 0	0.02 (1)
F- D	-4983 / 0	0.0	0.0	0.30 (1)	6.44	G- D		0 / 7334	0.39 (1)
I- A	-4786 / 0	0.0	0.0	0.10 (1)	7.77	A- H		0 / 4924	0.37 (1)
I- J	0 / 0	-18.5	-18.5	0.31 (1)	10.00				
J- H	0 / 0	-18.5	-18.5	0.31 (1)	10.00				
H- K	0 / 4814	-18.5	-18.5	0.56 (1)	10.00				
K- G	0 / 4814	-18.5	-18.5	0.56 (1)	10.00				
G- L	0 / 0	-18.5	-18.5	0.63 (1)	10.00				
L- M	0 / 0	-18.5	-18.5	0.63 (1)	10.00				
M- F	0 / 0	-18.5	-18.5	0.63 (1)	10.00				
F- E	0 / 0	-18.5	-18.5	0.20 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	3-4-4	-1379	-1379	---	FRONT	VERT	TOTAL	---	C1
J	1-4-4	-1379	-1379	---	FRONT	VERT	TOTAL	---	C1
K	5-4-4	-1379	-1379	---	FRONT	VERT	TOTAL	---	C1
L	7-4-4	-1379	-1379	---	FRONT	VERT	TOTAL	---	C1
M	9-4-4	-1379	-1379	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.30/1.00 (D-F:1) , BC=0.63/1.00 (F-G:1) , WB=0.39/1.00 (D-G:1) , SSI=0.68/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.45 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402463	T25	2	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TTWW-l	MT20	4.0	10.0	2.00	7.75
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	5.0	6.0	2.50	2.50
F	BMVW+p	MT20	8.0	9.0	4.50	Edge
G	BMWWW-t	MT20	8.0	9.0	4.25	4.50
H	BMWW-t	MT20	5.0	6.0	2.75	2.75
I	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

STRUCTURAL COMPONENT ONLY
DWG # TR23040108

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T26	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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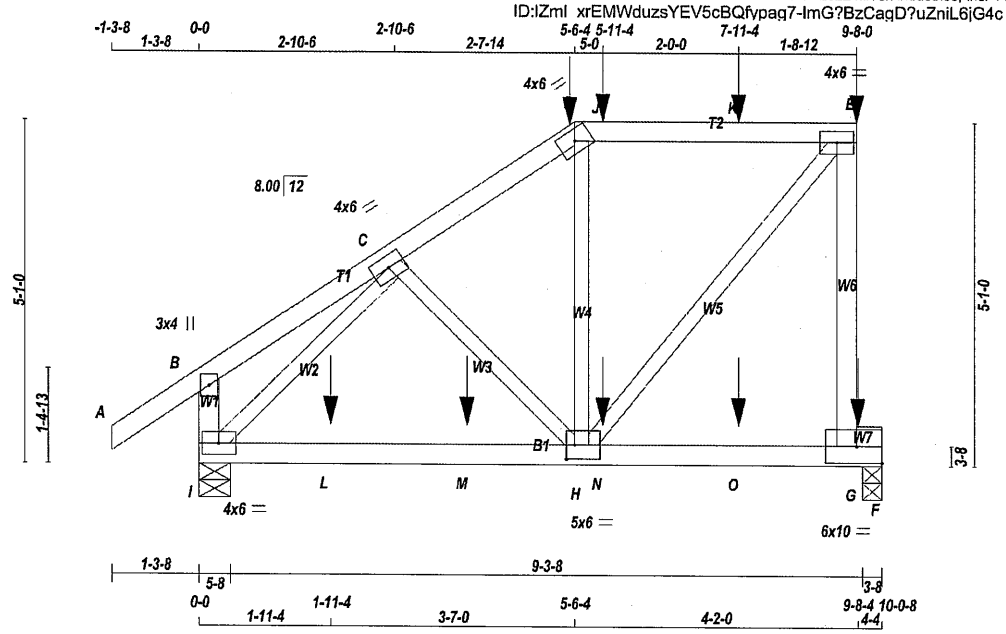

TOTAL WEIGHT = 2 X 47 = 94 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0		Edge
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

STRUCTURAL COMPONENT ONLY
DWG # TR23040109

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1239	0	1239	0	0	5-8	5-8
F	1375	0	1375	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	861	651 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0
F	956	718 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	C- H	-96 / 13	0.03 (1)
B- C	0 / 22	-129.6	-129.6	0.17 (1)	10.00	H- D	-487 / 0	0.19 (1)
C- D	-944 / 0	-129.6	-129.6	0.18 (1)	6.13	H- E	0 / 1170	0.29 (1)
D- J	-755 / 0	-129.6	-129.6	0.68 (1)	5.51	I- C	-1213 / 0	0.34 (1)
J- K	-755 / 0	-129.6	-129.6	0.68 (1)	5.51			
K- E	-755 / 0	-129.6	-129.6	0.68 (1)	5.51			
G- E	-1463 / 0	0.0	0.0	0.65 (1)	6.73			
I- B	-317 / 0	0.0	0.0	0.04 (1)	7.81			
I- L	0 / 833	-18.5	-18.5	0.41 (1)	10.00			
L- M	0 / 833	-18.5	-18.5	0.41 (1)	10.00			
M- H	0 / 833	-18.5	-18.5	0.41 (1)	10.00			
H- N	0 / 0	-18.5	-18.5	0.77 (1)	10.00			
N- O	0 / 0	-18.5	-18.5	0.77 (1)	10.00			
O- G	0 / 0	-18.5	-18.5	0.77 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.32 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-6-4	-35	-35	---	FRONT	VERT	DEAD	---	C1
D	5-6-4	-220	-220	---	FRONT	VERT	SNOW	---	C1
E	9-8-0	-139	-139	---	FRONT	VERT	TOTAL	---	C1
G	9-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
J	5-11-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
K	7-11-4	-94	-94	---	FRONT	VERT	TOTAL	---	C1
L	1-11-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1
M	3-11-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1
N	5-11-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1
O	7-11-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 808 (0.15")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.77/1.00 (G-H:1) , WB=0.34/1.00 (C-I:1) , SSI=0.59/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.34 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T27	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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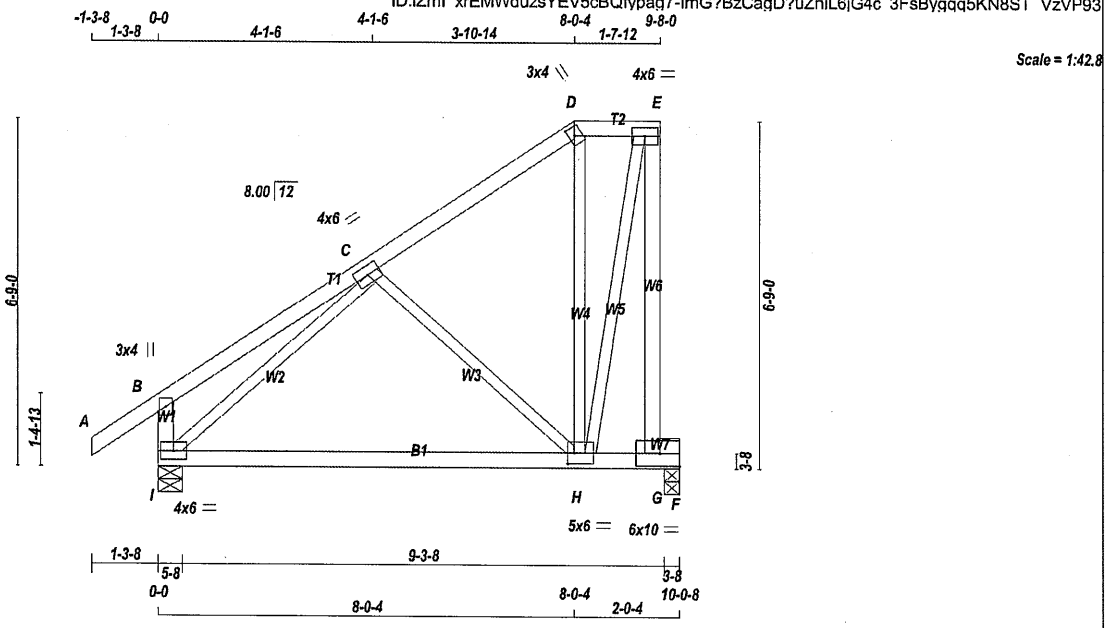

TOTAL WEIGHT = 2 X 53 = 107 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	Edge
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	696	0	696	0	0	3-8	3-8
I	921	0	921	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	486	356 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
I	639	488 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- H	-434 / 0		0.23 (1)
B- C	0 / 35	-129.6	-129.6	0.35 (1)	10.00	H- D	-173 / 5		0.13 (1)
C- D	-274 / 0	-129.6	-129.6	0.35 (1)	6.25	H- E	0 / 844		0.19 (1)
D- E	-196 / 0	-129.6	-129.6	0.06 (1)	6.25	I- C	-712 / 0		0.35 (1)
G- E	-926 / 0	0.0	0.0	0.89 (1)	7.81				
I- B	-374 / 0	0.0	0.0	0.04 (1)	7.81				
I- H	0 / 520	-18.5	-18.5	0.32 (4)	10.00				
H- G	0 / 0	-18.5	-18.5	0.35 (1)	10.00				
G- F	0 / 0	-18.5	-18.5	0.15 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.89/1.00 (E-G:1) , BC=0.35/1.00 (G-H:1) , WB=0.35/1.00 (C-I:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (H) (INPUT = 0.90)
JSI METAL= 0.23 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040110

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T28	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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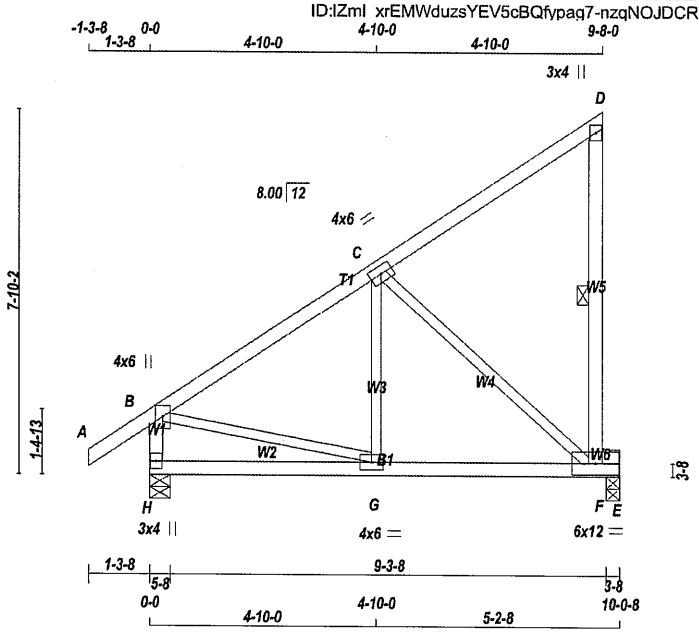

Scale = 1:47.4

TOTAL WEIGHT = 4 X 48 = 193 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	921	0	921	0	0	5-8	5-8
E	696	0	696	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	639	488 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
E	486	356 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00		G- C	0 / 114	0.04 (4)
B- C	-611 / 0	-129.6	-129.6	0.40 (1)	6.25		C- F	-728 / 0	0.52 (1)
C- D	-43 / 0	-129.6	-129.6	0.39 (1)	6.25		B- G	0 / 557	0.13 (1)
F- D	-236 / 0	0.0	0.0	0.06 (1)	6.25				
H- B	-899 / 0	0.0	0.0	0.09 (1)	7.81				
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
G- F	0 / 544	-18.5	-18.5	0.44 (1)	10.00				
F- E	0 / 0	-18.5	-18.5	0.15 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.44/1.00 (F-G:1) , WB=0.52/1.00 (C-F:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040111

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T29	4	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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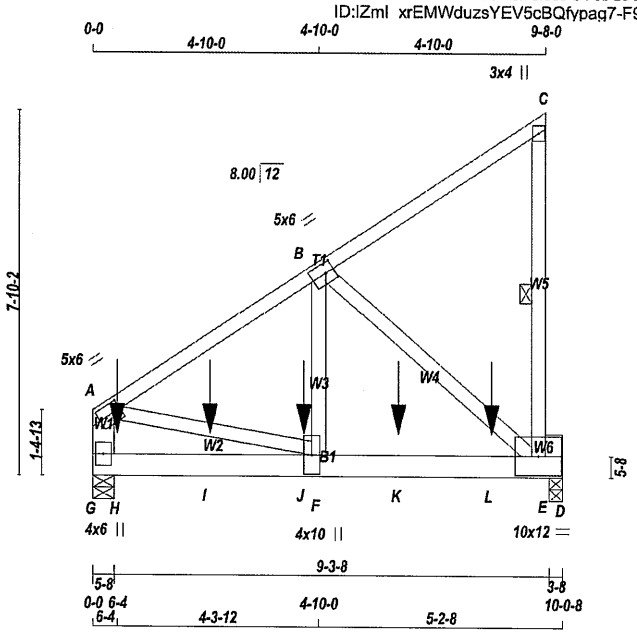

Scale = 1:47.4

TOTAL WEIGHT = 12 X 58 = 694 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - D	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
G- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- D	2	5	SIDE(824.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040112

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	6194	0	6194	0	0	5-8	5-8
D	5157	0	5157	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	4311	3220 / 0	0 / 0	0 / 0	0 / 0	1092 / 0	0 / 0
D	3590	2678 / 0	0 / 0	0 / 0	0 / 0	911 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-5054 / 0	-129.6	-129.6	0.22 (1)	4.93	F- B	0 / 5757	0.31 (1)	
B- C	-39 / 0	-129.6	-129.6	0.15 (1)	6.25	B- E	-5624 / 0	0.91 (1)	
E- C	-244 / 0	0.0	0.0	0.02 (1)	6.25	A- F	0 / 4326	0.23 (1)	
G- A	-3942 / 0	0.0	0.0	0.09 (1)	7.81				
G- H	0 / 0	-18.5	-18.5	0.69 (1)	10.00				
H- I	0 / 0	-18.5	-18.5	0.69 (1)	10.00				
I- J	0 / 0	-18.5	-18.5	0.69 (1)	10.00				
J- F	0 / 0	-18.5	-18.5	0.69 (1)	10.00				
F- K	0 / 4237	-18.5	-18.5	0.90 (1)	10.00				
K- L	0 / 4237	-18.5	-18.5	0.90 (1)	10.00				
L- E	0 / 4237	-18.5	-18.5	0.90 (1)	10.00				
E- D	0 / 0	-18.5	-18.5	0.18 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	6-4	-1382	-1382	---	BACK	VERT	TOTAL	---	C1
I	2-6-4	-1379	-1379	---	BACK	VERT	TOTAL	---	C1
J	4-6-4	-1379	-1379	---	BACK	VERT	TOTAL	---	C1
K	6-6-4	-1379	-1379	---	BACK	VERT	TOTAL	---	C1
L	8-6-4	-1379	-1379	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 803 (0.15")

CSI: TC=0.22/1.00 (A-B:1) , BC=0.90/1.00 (E-F:1) , WB=0.91/1.00 (B-E:1) , SSI=0.76/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.64 (F) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T29	4	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:IZml_xrEMWduzsYEV5cBQfypag7-F9OlceDqCqFco4skE8IYhP8lw?VYIbrdqSxa2NzVP91

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMWW-t	MT20	5.0	6.0	2.50	2.25
C	TMV+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	10.0	12.0		
F	BMWW+t	MT20	4.0	10.0		
G	BMV1+p	MT20	4.0	6.0		

STRUCTURAL COMPONENT ONLY
DWG # TR23040112

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T30	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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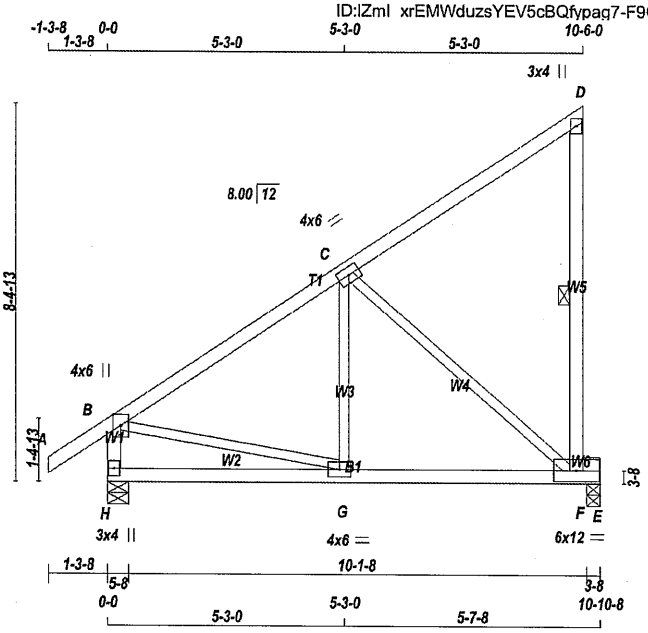

Scale = 1:49.0

TOTAL WEIGHT = 4 X 52 = 208 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	983	0	983	0	0	5-8	5-8
E	758	0	758	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	682	520 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
E	529	387 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	G- C	0 / 124	0.04 (4)
B- C	-674 / 0	-129.6	-129.6	0.64 (1)	5.99	C- F	-796 / 0	0.69 (1)
C- D	-46 / 0	-129.6	-129.6	0.60 (1)	6.25	B- G	0 / 612	0.14 (1)
F- D	-257 / 0	0.0	0.0	0.08 (1)	6.25			
H- B	-958 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
G- F	0 / 600	-18.5	-18.5	0.48 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 962 (0.14")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.48/1.00 (F-G:1) , WB=0.69/1.00 (C-F:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040113

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T31	2	3	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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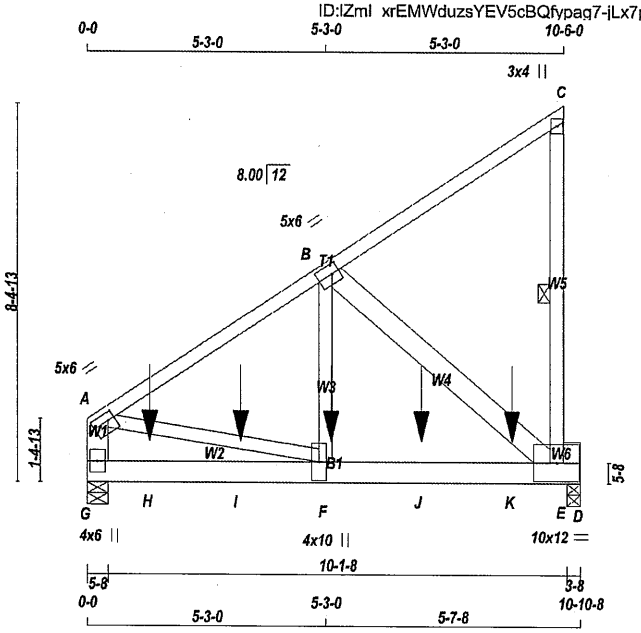


Scale = 1:49.0

TOTAL WEIGHT = 6 X 67 = 400 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - D	2x6	DRY	2100F 1.8E	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - E	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
G- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- D	2	7	SIDE(667.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040114

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	5838	0	5838	0	0	5-8	5-8
D	5632	0	5632	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	4063	3036 / 0	0 / 0	0 / 0	0 / 0	1027 / 0	0 / 0
D	3920	2925 / 0	0 / 0	0 / 0	0 / 0	995 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	-5569 / 0	-129.6	-129.6	0.28 (1)	4.69	F- B	0 / 6275	0.34 (1)
B- C	-44 / 0	-129.6	-129.6	0.19 (1)	6.25	B- E	-6148 / 0	0.77 (1)
E- C	-262 / 0	0.0	0.0	0.03 (1)	6.25	A- F	0 / 4753	0.25 (1)
G- A	-4261 / 0	0.0	0.0	0.09 (1)	7.81			
G- H	0 / 0	-18.5	-18.5	0.41 (1)	10.00			
H- I	0 / 0	-18.5	-18.5	0.41 (1)	10.00			
I- F	0 / 0	-18.5	-18.5	0.41 (1)	10.00			
F- J	0 / 4670	-18.5	-18.5	0.51 (1)	10.00			
J- K	0 / 4670	-18.5	-18.5	0.51 (1)	10.00			
K- E	0 / 4670	-18.5	-18.5	0.51 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.20 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	5-4-8	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1
H	1-4-8	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1
I	3-4-8	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1
J	7-4-8	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1
K	9-4-8	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 877 (0.15")

CSI: TC=0.28/1.00 (A-B:1) , BC=0.51/1.00 (E-F:1) , WB=0.77/1.00 (B-E:1) , SSI=0.57/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

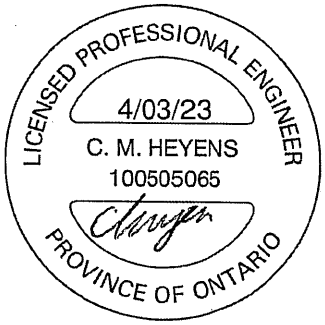
JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.70 (F) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T31	2	3	BAYVIEW WELLINGTON TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.00	1.75
B	TMWW-t	MT20	5.0	6.0	2.50	2.00
C	TMV+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	10.0	12.0		
F	BMWW+t	MT20	4.0	10.0		
G	BMV1+p	MT20	4.0	6.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040114

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T32	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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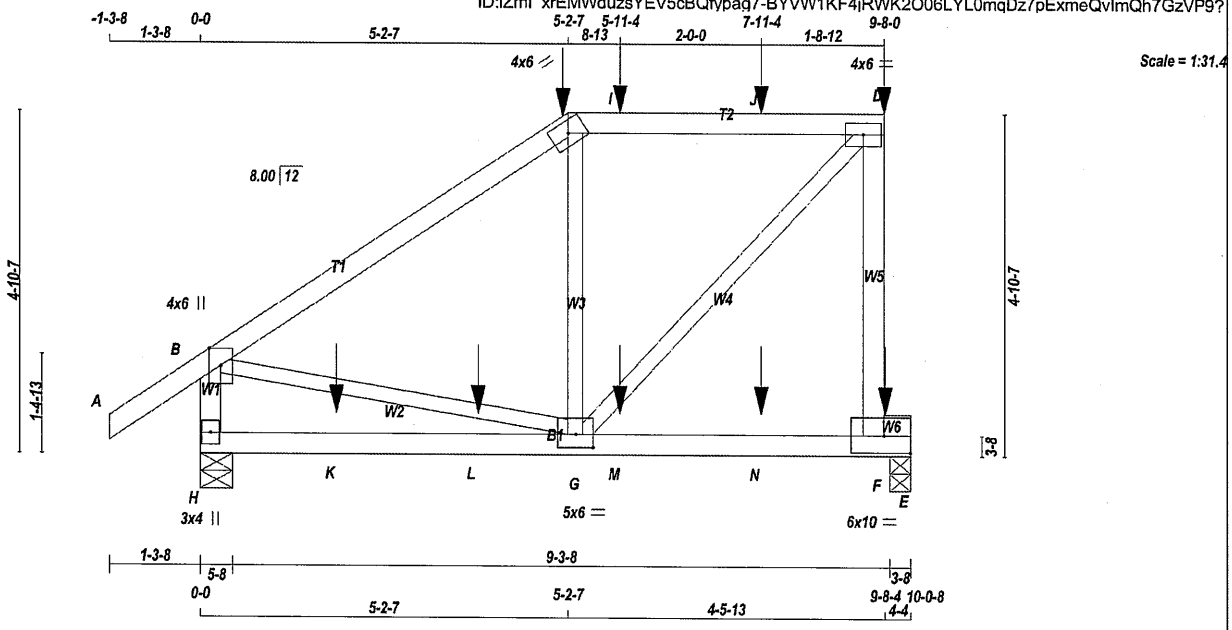

TOTAL WEIGHT = 2 X 44 = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW-h	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
F	BMVW-t	MT20	6.0	10.0	3.00	4.50
G	BMWWW-t	MT20	5.0	6.0	2.25	3.00
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040115

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1202	0	1202	0	0	5-8	5-8
E	1279	0	1279	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	835	633 / 0	0 / 0	0 / 0	0 / 0	203 / 0	0 / 0
E	890	666 / 0	0 / 0	0 / 0	0 / 0	224 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	G- C	-614 / 0	0.22 (1)
B- C	-904 / 0	-129.6	-129.6	0.72 (1)	5.13	G- D	0 / 1064	0.26 (1)
C- I	-727 / 0	-129.6	-129.6	0.75 (1)	5.35	B- G	0 / 766	0.19 (1)
I- J	-727 / 0	-129.6	-129.6	0.75 (1)	5.35			
J- D	-727 / 0	-129.6	-129.6	0.75 (1)	5.35			
F- D	-1348 / 0	0.0	0.0	0.54 (1)	6.94			
H- B	-1169 / 0	0.0	0.0	0.13 (1)	7.34			
H- K	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
K- L	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
L- G	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
G- M	0 / 0	-18.5	-18.5	0.72 (1)	10.00			
M- N	0 / 0	-18.5	-18.5	0.72 (1)	10.00			
N- F	0 / 0	-18.5	-18.5	0.72 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.30 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-2-7	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	5-2-7	-191	-191	---	FRONT	VERT	SNOW	---	C1
D	9-8-0	-128	-128	---	BACK	VERT	TOTAL	---	C1
F	9-8-4	-19	-19	---	BACK	VERT	TOTAL	---	C1
I	5-11-4	-95	-95	---	BACK	VERT	TOTAL	---	C1
J	7-11-4	-83	-83	---	BACK	VERT	TOTAL	---	C1
K	1-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
L	3-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
M	5-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
N	7-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 786 (0.15")

CSI: TC=0.75/1.00 (C-D:1) , BC=0.72/1.00 (F-G:1) , WB=0.26/1.00 (D-G:1) , SSI=0.55/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T33	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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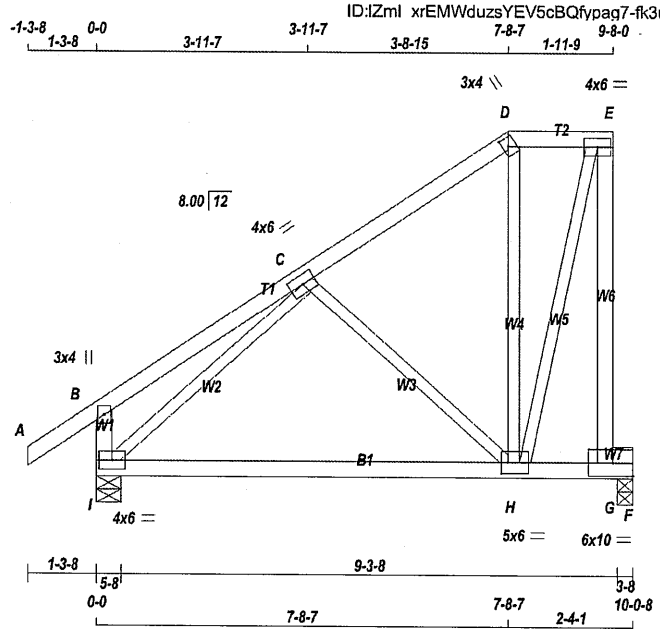

TOTAL WEIGHT = 2 X 53 = 105 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0		Edge
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	696	0	696	0	0	3-8	3-8
I	921	0	921	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	486	356 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
I	639	488 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00		C- H	-405 / 0	0.20 (1)
B- C	0 / 33	-129.6	-129.6	0.32 (1)	10.00		H- D	-168 / 7	0.12 (1)
C- D	-306 / 0	-129.6	-129.6	0.32 (1)	6.25		H- E	0 / 784	0.18 (1)
D- E	-224 / 0	-129.6	-129.6	0.09 (1)	6.25		I- C	-723 / 0	0.33 (1)
G- E	-877 / 0	0.0	0.0	0.77 (1)	7.81				
I- B	-367 / 0	0.0	0.0	0.04 (1)	7.81				
I- H	0 / 525	-18.5	-18.5	0.30 (4)	10.00				
H- G	0 / 0	-18.5	-18.5	0.35 (1)	10.00				
G- F	0 / 0	-18.5	-18.5	0.15 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.77/1.00 (E-G:1) , BC=0.35/1.00 (G-H:1) , WB=0.33/1.00 (C-I:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (H) (INPUT = 0.90)
JSI METAL= 0.21 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040116

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T36	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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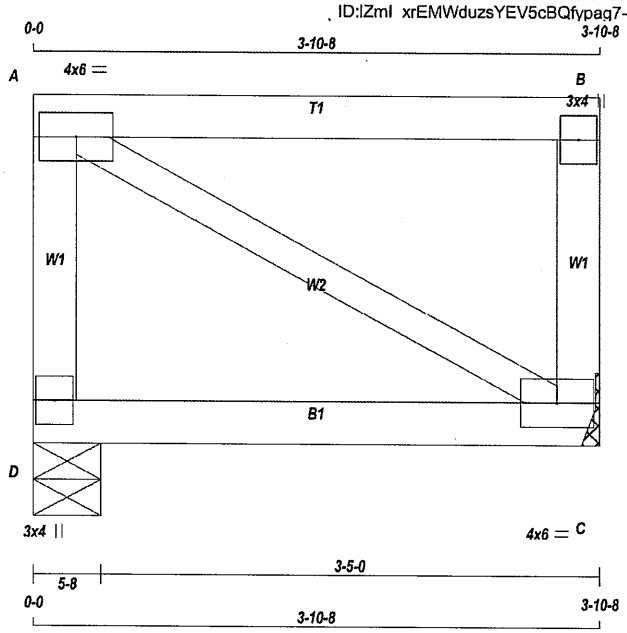

Scale = 1:15.2

TOTAL WEIGHT = 2 X 16 = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	287	0	287	0	0	5-8	5-8
C	287	0	287	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
C	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-251 / 0	0.0	0.0	0.03 (1)		7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-129.6	-129.6	0.33 (1)		10.00			
C- B	-251 / 0	0.0	0.0	0.03 (1)		7.81			
D- C	0 / 0	-18.5	-18.5	0.08 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.08/1.00 (C-D:4) , WB=0.00/1.00 (A-C:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (B) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040117

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T37	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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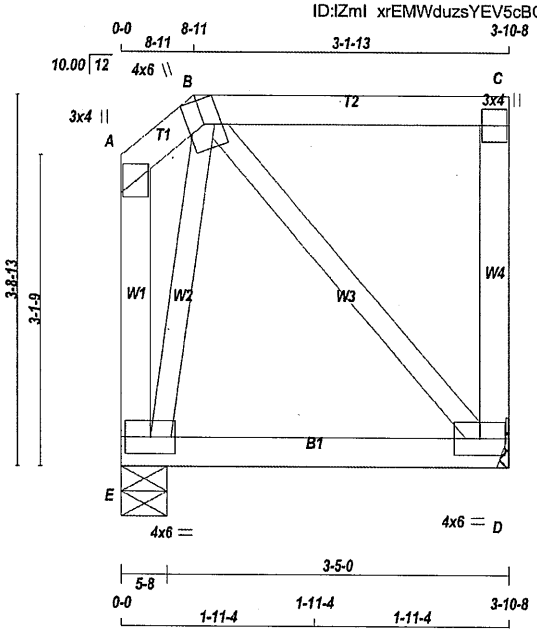

Scale = 1:22.2

TOTAL WEIGHT = 2 X 22 = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TTWW+m	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	6.0		
E	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	287	0	287	0	0	MECHANICAL	
E	287	0	287	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
E	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 0	-129.6	-129.6	0.01 (1)	10.00	B- D	-68 / 0	0.02 (1)
B- C	0 / 0	-129.6	-129.6	0.21 (1)	10.00	E- B	-206 / 0	0.05 (1)
D- C	-201 / 0	0.0	0.0	0.04 (1)	7.81			
E- A	-50 / 0	0.0	0.0	0.01 (1)	7.81			
E- D	0 / 45	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.05/1.00 (B-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040118

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T38	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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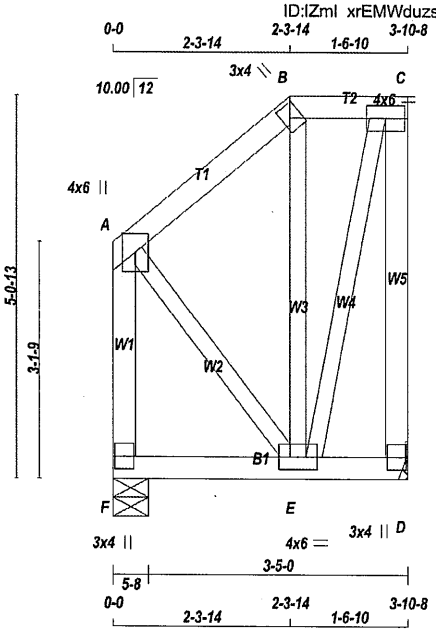

Scale = 1:29.2

TOTAL WEIGHT = 2 X 28 = 57 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	287	0	287	0	0	MECHANICAL	
F	287	0	287	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
F	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-74 / 0	-129.6	-129.6	0.09 (1)	6.25	E- B	-204 / 0	0.08 (1)
B- C	-52 / 0	-129.6	-129.6	0.04 (1)	6.25	E- C	0 / 181	0.04 (1)
D- C	-273 / 0	0.0	0.0	0.12 (1)	7.81	A- E	0 / 87	0.02 (1)
F- A	-264 / 0	0.0	0.0	0.05 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (C-D:1) , BC=0.02/1.00 (E-F:4) , WB=0.08/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)
JSI METAL= 0.07 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040119

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023 BUILDING PERMIT REQUIRED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	T39	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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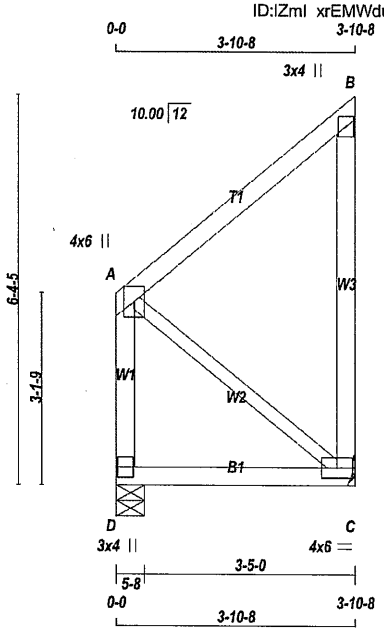

Scale = 1:35.9

TOTAL WEIGHT = 2 X 23 = 47 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	287	0	287	0	0	MECHANICAL	
D	287	0	287	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
D	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 0	-129.6	-129.6	0.33 (1)	10.00	A- C	0 / 0	0.00 (1)
C- B	-251 / 0	0.0	0.0	0.20 (1)	7.81			
D- A	-251 / 0	0.0	0.0	0.04 (1)	7.81			
D- C	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.08/1.00 (C-D:4) , WB=0.00/1.00 (A-C:1) , SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (A) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040120

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T90	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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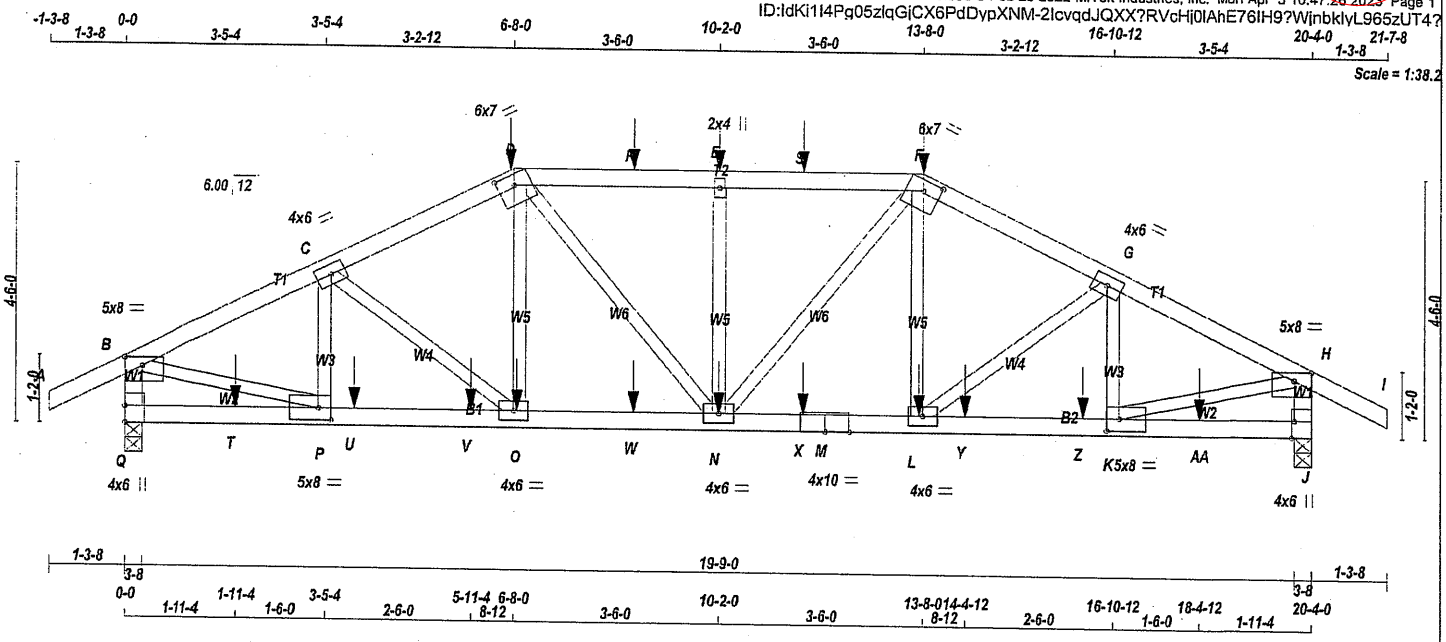

TOTAL WEIGHT = 2 X 86 = 173 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-h	MT20	6.0	7.0	2.25	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-h	MT20	6.0	7.0	2.25	3.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-p	MT20	5.0	8.0	1.75	3.50
J	BMV1+t	MT20	4.0	6.0	Edge	0.50
K	BMWW-t	MT20	5.0	8.0	2.50	2.50
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	4.0	10.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.50	2.50
Q	BMV1+t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

STRUCTURAL COMPONENT ONLY
DWG # TR23040330

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2969	0	2969	0	0	3-8	3-8
J	2969	0	2969	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2070	1527 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0
J	2070	1527 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.19 (1)	10.00
B- C	-3802 / 0	-129.6	-129.6	0.42 (1)	3.22
C- D	-4046 / 0	-129.6	-129.6	0.44 (1)	3.09
D- R	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67
R- E	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67
E- S	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67
S- F	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67
F- G	-4046 / 0	-129.6	-129.6	0.44 (1)	3.09
G- H	-3802 / 0	-129.6	-129.6	0.42 (1)	3.22
H- I	0 / 39	-129.6	-129.6	0.19 (1)	10.00
Q- B	-2921 / 0	0.0	0.0	0.32 (1)	4.98
J- H	-2921 / 0	0.0	0.0	0.32 (1)	4.98
Q- T	0 / 0	-18.5	-18.5	0.10 (4)	10.00
T- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00
P- U	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
U- V	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
V- O	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
O- W	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
W- N	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
N- X	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
X- M	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
M- L	0 / 3599	-18.5	-18.5	0.70 (1)	10.00
L- Y	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
Y- Z	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
Z- K	0 / 3416	-18.5	-18.5	0.66 (1)	10.00
K-AA	0 / 0	-18.5	-18.5	0.10 (4)	10.00
AA- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- D	0 / 143	0.05 (4)
L- F	0 / 143	0.05 (4)
N- E	-1021 / 0	0.32 (1)
N- F	0 / 737	0.18 (1)
D- N	0 / 737	0.18 (1)
C- O	0 / 231	0.06 (1)
P- C	-754 / 0	0.14 (1)
B- P	0 / 3520	0.87 (1)
L- G	0 / 231	0.06 (1)
K- G	-754 / 0	0.14 (1)
K- H	0 / 3520	0.87 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-8-0	-561	-561	---	BACK	VERT	TOTAL	---	C1
E	10-2-0	-133	-133	---	BACK	VERT	TOTAL	---	C1
F	13-8-0	-561	-561	---	BACK	VERT	TOTAL	---	C1
L	13-7-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
N	10-2-0	-26	-26	---	BACK	VERT	TOTAL	---	C1
O	6-8-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
R	8-8-12	-133	-133	---	BACK	VERT	TOTAL	---	C1
S	11-7-4	-133	-133	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.69/1.00 (D-E:1) , BC=0.70/1.00 (L-N:1) , WB=0.87/1.00 (B-P:1) , SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.81 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
404951	T90	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	1-11-4	-25	-25	—	BACK	VERT	TOTAL	—	C1
U	3-11-4	-26	-26	—	BACK	VERT	TOTAL	—	C1
V	5-11-4	-26	-26	—	BACK	VERT	TOTAL	—	C1
W	8-8-12	-26	-26	—	BACK	VERT	TOTAL	—	C1
X	11-7-4	-26	-26	—	BACK	VERT	TOTAL	—	C1
Y	14-4-12	-26	-26	—	BACK	VERT	TOTAL	—	C1
Z	16-4-12	-26	-26	—	BACK	VERT	TOTAL	—	C1
AA	18-4-12	-25	-25	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040330

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T91	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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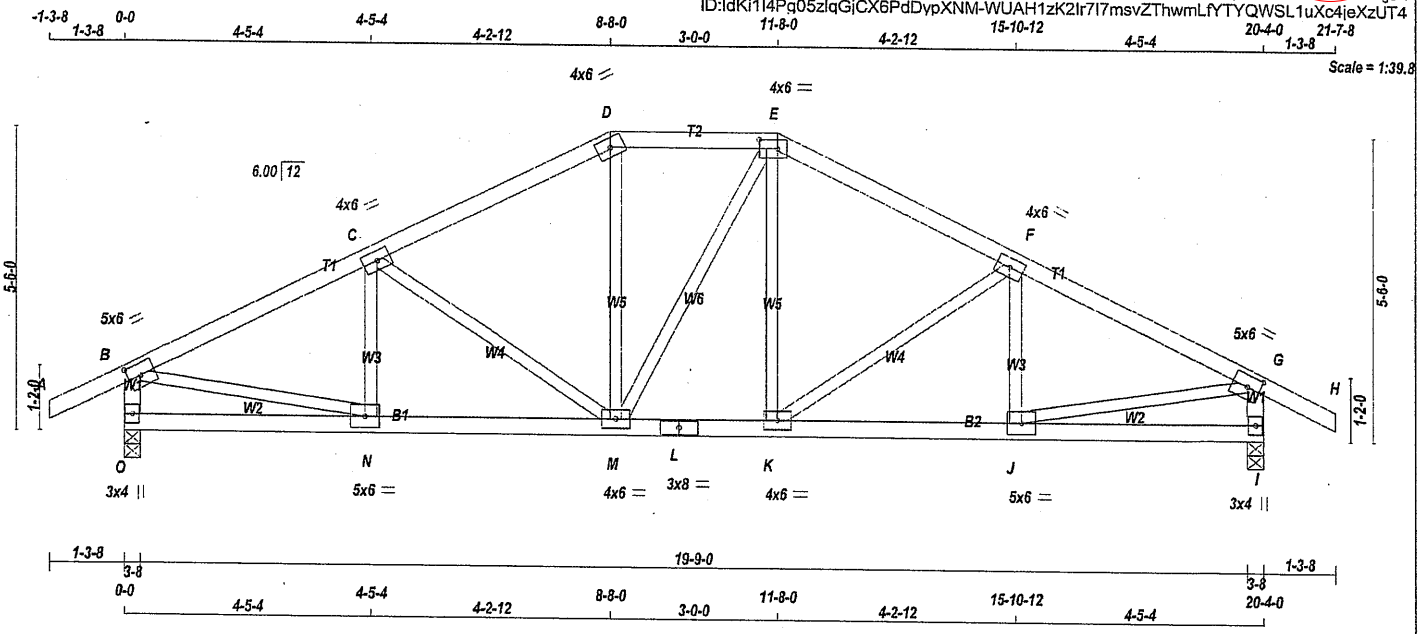

TOTAL WEIGHT = 2 X 86 = 172 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TTWW-I	MT20	4.0	6.0	2.00	4.00
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

STRUCTURAL COMPONENT ONLY
DWG # TR23040331

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1682	0	1682	0	0	3-8	3-8
I	1682	0	1682	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
I	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-1977 / 0	-129.6	-129.6	0.35 (1)	4.46
C- D	-1640 / 0	-129.6	-129.6	0.33 (1)	4.83
D- E	-1445 / 0	-129.6	-129.6	0.17 (1)	5.27
E- F	-1638 / 0	-129.6	-129.6	0.33 (1)	4.83
F- G	-1977 / 0	-129.6	-129.6	0.35 (1)	4.46
G- H	0 / 39	-129.6	-129.6	0.17 (1)	10.00
O- B	-1644 / 0	0.0	0.0	0.17 (1)	6.46
I- G	-1644 / 0	0.0	0.0	0.17 (1)	6.46
O- N	0 / 0	-18.5	-18.5	0.08 (4)	10.00
N- M	0 / 1793	-18.5	-18.5	0.32 (1)	10.00
M- L	0 / 1444	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 1444	-18.5	-18.5	0.26 (1)	10.00
K- J	0 / 1794	-18.5	-18.5	0.33 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- D	0 / 320	0.07 (1)
M- E	0 / 3	0.00 (1)
K- E	0 / 317	0.07 (1)
C- M	-434 / 0	0.20 (1)
N- C	-265 / 15	0.05 (1)
B- N	0 / 1826	0.41 (1)
K- F	-437 / 0	0.21 (1)
J- F	-263 / 16	0.05 (1)
J- G	0 / 1827	0.41 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.35/1.00 (F-G:1) , BC=0.33/1.00 (J-K:1) , WB=0.41/1.00 (G-J:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.50 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	G92	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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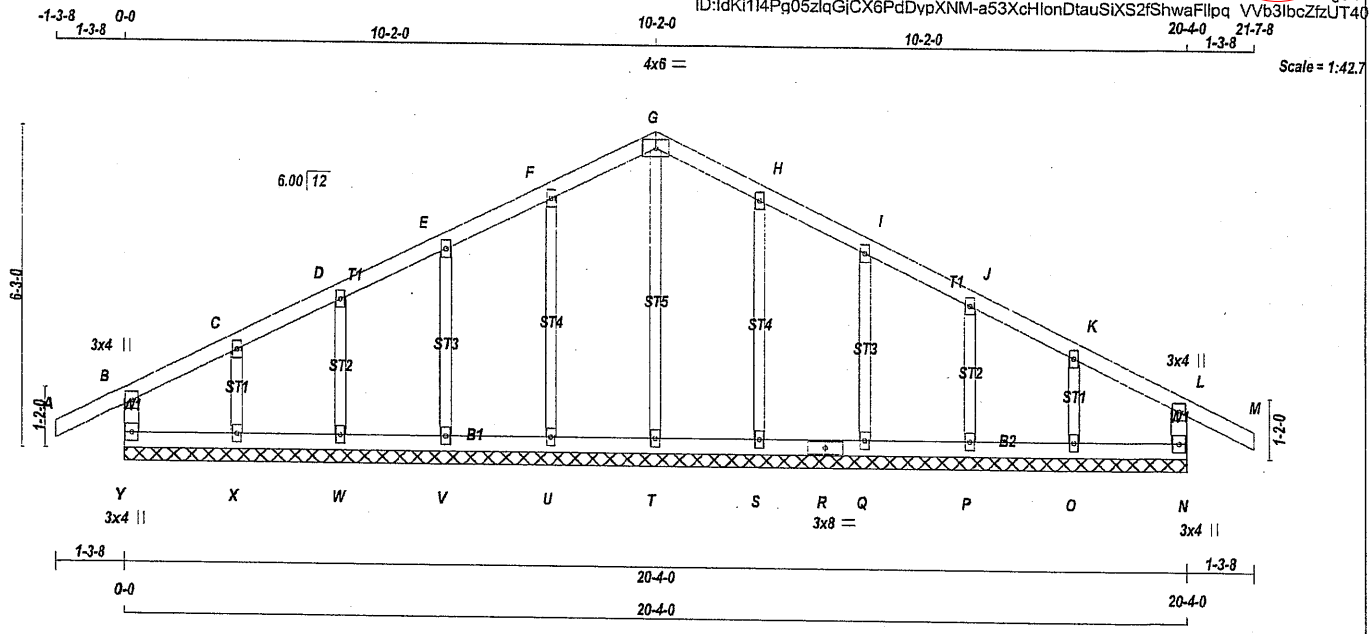

TOTAL WEIGHT = 2 X 80 = 159 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	6.0		
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, Q, S, T, U, V, W, X						
O	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	8.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
Y-B	-332 / 0	0.0	0.0	0.02 (1)	7.81
A-B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B-C	-39 / 0	-129.6	-129.6	0.07 (1)	6.25
C-D	-28 / 0	-129.6	-129.6	0.06 (1)	6.25
D-E	-22 / 0	-129.6	-129.6	0.06 (1)	6.25
E-F	-17 / 0	-129.6	-129.6	0.06 (1)	6.25
F-G	-13 / 0	-129.6	-129.6	0.06 (1)	6.25
G-H	-13 / 0	-129.6	-129.6	0.06 (1)	6.25
H-I	-17 / 0	-129.6	-129.6	0.06 (1)	6.25
I-J	-22 / 0	-129.6	-129.6	0.06 (1)	6.25
J-K	-28 / 0	-129.6	-129.6	0.06 (1)	6.25
K-L	-39 / 0	-129.6	-129.6	0.07 (1)	6.25
L-M	0 / 39	-129.6	-129.6	0.17 (1)	10.00
N-L	-332 / 0	0.0	0.0	0.02 (1)	7.81
Y-X	0 / 34	-18.5	-18.5	0.02 (1)	10.00
X-W	0 / 25	-18.5	-18.5	0.02 (4)	10.00
W-V	0 / 19	-18.5	-18.5	0.02 (4)	10.00
V-U	0 / 15	-18.5	-18.5	0.02 (4)	10.00
U-T	0 / 12	-18.5	-18.5	0.02 (4)	10.00
T-S	0 / 12	-18.5	-18.5	0.02 (4)	10.00
S-R	0 / 15	-18.5	-18.5	0.02 (4)	10.00
R-Q	0 / 15	-18.5	-18.5	0.02 (4)	10.00
Q-P	0 / 19	-18.5	-18.5	0.02 (4)	10.00
P-O	0 / 25	-18.5	-18.5	0.02 (4)	10.00
O-N	0 / 34	-18.5	-18.5	0.02 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-G	-257 / 0	0.16 (1)
U-F	-262 / 0	0.11 (1)
V-E	-256 / 0	0.07 (1)
W-D	-256 / 0	0.05 (1)
X-C	-259 / 0	0.04 (1)
S-H	-262 / 0	0.11 (1)
Q-I	-256 / 0	0.07 (1)
P-J	-256 / 0	0.05 (1)
O-K	-259 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (L-M:1) , BC=0.02/1.00 (N-O:1) , WB=0.16/1.00 (G-T:1) , SSI=0.12/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (I) (INPUT = 0.90)
JSI METAL= 0.11 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040329

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T93	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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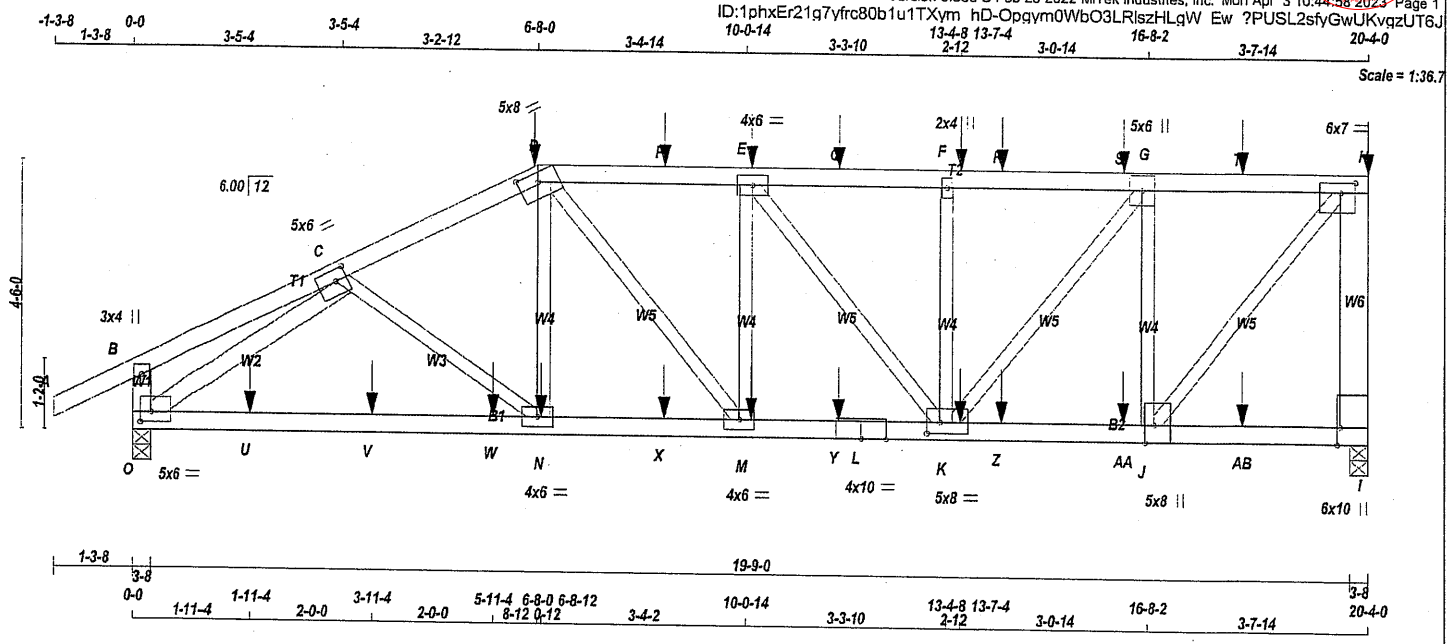


TOTAL WEIGHT = 2 X 94 = 187 lb [M]

LUMBER
N. L. G. A. RULES

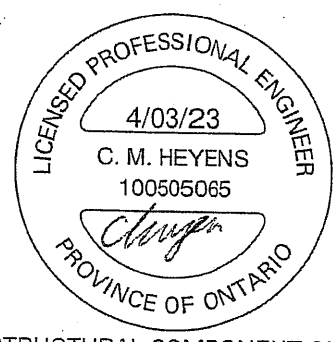
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW-h	MT20	5.0	8.0	2.00	4.00
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	5.0	6.0		
H	TMVW-t	MT20	6.0	7.0	2.00	3.00
I	BMV1+t	MT20	6.0	10.0	Edge	0.50
J	BMWW+t	MT20	5.0	8.0	Edge	1.75
K	BMWWW-t	MT20	5.0	8.0	2.25	2.50
L	BS-t	MT20	4.0	10.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	5.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040327

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	3091	0	3091	0	0	3-8	3-8
O	2885	0	2885	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2152	1606 / 0	0 / 0	0 / 0	0 / 0	546 / 0	0 / 0
O	2012	1480 / 0	0 / 0	0 / 0	0 / 0	533 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.19 (1)	10.00
B- C	0 / 17	-129.6	-129.6	0.20 (1)	10.00
C- D	-3919 / 0	-129.6	-129.6	0.43 (1)	3.16
D- P	-3855 / 0	-129.6	-129.6	0.60 (1)	2.92
P- E	-3855 / 0	-129.6	-129.6	0.60 (1)	2.92
E- Q	-3403 / 0	-129.6	-129.6	0.54 (1)	3.17
Q- F	-3403 / 0	-129.6	-129.6	0.54 (1)	3.17
F- R	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
R- S	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
S- G	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
G- T	-2216 / 0	-129.6	-129.6	0.50 (1)	3.94
T- H	-2216 / 0	-129.6	-129.6	0.50 (1)	3.94
I- H	-3053 / 0	0.0	0.0	0.64 (1)	5.99
O- B	-355 / 0	0.0	0.0	0.04 (1)	7.81
O- U	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
U- V	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
V- W	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
W- N	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
N- X	0 / 3488	-18.5	-18.5	0.80 (1)	10.00
X- M	0 / 3488	-18.5	-18.5	0.80 (1)	10.00
M- Y	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
Y- L	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
L- K	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
K- Z	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
Z-AA	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
AA- J	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
J-AB	0 / 0	-18.5	-18.5	0.10 (4)	10.00
AB- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 310	0.08 (1)
N- D	0 / 206	0.08 (4)
O- C	-4057 / 0	0.89 (1)
J- H	0 / 3379	0.84 (1)
D- M	0 / 583	0.14 (1)
J- G	-2411 / 0	0.75 (1)
M- E	-367 / 9	0.11 (1)
K- G	0 / 1922	0.48 (1)
E- K	-733 / 0	0.35 (1)
K- F	-808 / 0	0.25 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-8-0	-561	-561	—	FRONT	VERT	TOTAL	—	C1
E	10-2-0	-133	-133	—	FRONT	VERT	TOTAL	—	C1
F	13-7-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
H	20-4-0	-174	-174	—	FRONT	VERT	TOTAL	—	C1
K	13-7-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
M	10-2-0	-26	-26	—	FRONT	VERT	TOTAL	—	C1
N	6-8-12	-26	-26	—	FRONT	VERT	TOTAL	—	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.64/1.00 (H-I:1) , BC=0.80/1.00 (M-N:1) , WB=0.89/1.00 (C-O:1) , SSI=0.41/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.94 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T93	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	8-8-12	-133	-133	—	FRONT	VERT	TOTAL	—	C1
Q	11-7-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
R	14-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
S	16-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
T	18-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
U	1-11-4	-25	-25	—	FRONT	VERT	TOTAL	—	C1
V	3-11-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
W	5-11-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
X	8-8-12	-26	-26	—	FRONT	VERT	TOTAL	—	C1
Y	11-7-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
Z	14-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
AA	16-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
AB	18-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040327

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T94	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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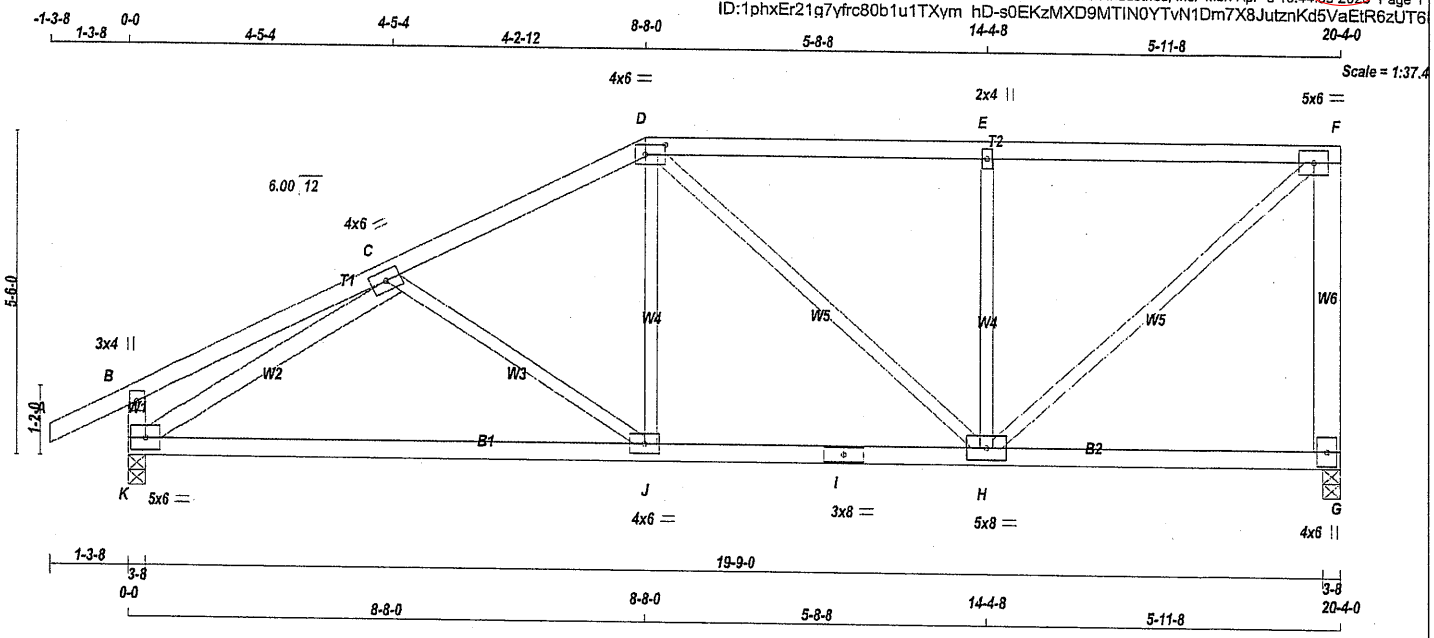

TOTAL WEIGHT = 2 X 89 = 177 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-t	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	4.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1506	0	1506	0	0	3-8	3-8
K	1682	0	1682	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1050	777 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	C- J	-356 / 0	0.18 (1)
B- C	0 / 29	-129.6	-129.6	0.38 (1)	10.00	J- D	0 / 363	0.08 (1)
C- D	-1659 / 0	-129.6	-129.6	0.44 (1)	4.61	K- C	-2133 / 0	0.72 (1)
D- E	-1327 / 0	-129.6	-129.6	0.82 (1)	4.13	H- F	0 / 1763	0.40 (1)
E- F	-1327 / 0	-129.6	-129.6	0.83 (1)	4.13	D- H	-185 / 0	0.22 (1)
G- F	-1457 / 0	0.0	0.0	0.50 (1)	7.81	H- E	-938 / 0	0.43 (1)
K- B	-395 / 0	0.0	0.0	0.04 (1)	7.81			
K- J	0 / 1750	-18.5	-18.5	0.45 (1)	10.00			
J- I	0 / 1464	-18.5	-18.5	0.39 (1)	10.00			
I- H	0 / 1464	-18.5	-18.5	0.39 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.83/1.00 (E-F:1) , BC=0.45/1.00 (J-K:1) , WB=0.72/1.00 (C-K:1) , SSI=0.37/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.44 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040328

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	G95	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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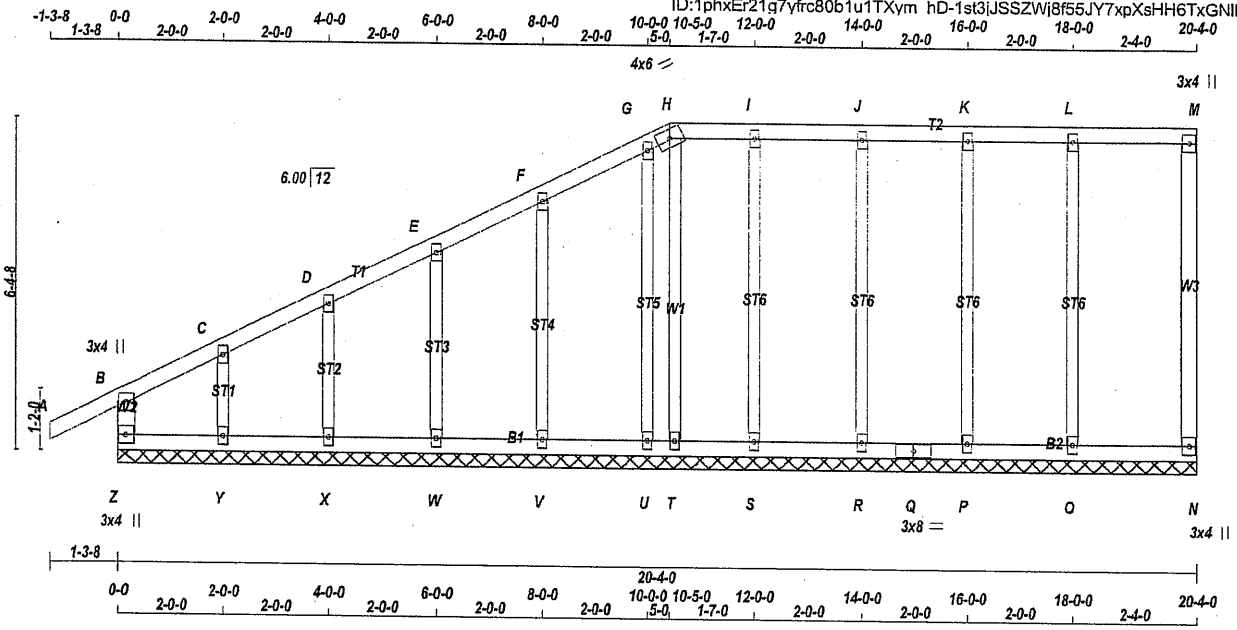

TOTAL WEIGHT = 2 X 95 = 190 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Z - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
Z - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, L						
C	TMW+w	MT20	2.0	4.0		
H	TTW-h	MT20	4.0	6.0		
M	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, R, S, T, U, V, W, X, Y						
O	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
Z	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
Z- B	-317 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-36 / 0	-129.6	-129.6	0.06 (1)	6.25
C- D	-25 / 0	-129.6	-129.6	0.06 (1)	6.25
D- E	-19 / 0	-129.6	-129.6	0.06 (1)	6.25
E- F	-14 / 0	-129.6	-129.6	0.06 (1)	6.25
F- G	-13 / 0	-129.6	-129.6	0.06 (1)	6.25
G- H	-11 / 0	-129.6	-129.6	0.04 (1)	6.25
H- I	-5 / 0	-129.6	-129.6	0.04 (1)	10.00
I- J	-5 / 0	-129.6	-129.6	0.06 (1)	10.00
J- K	-5 / 0	-129.6	-129.6	0.06 (1)	10.00
K- L	-5 / 0	-129.6	-129.6	0.08 (1)	10.00
L- M	-5 / 0	-129.6	-129.6	0.08 (1)	10.00
N- M	-136 / 0	0.0	0.0	0.04 (1)	7.81
Z- Y	0 / 32	-18.5	-18.5	0.02 (1)	10.00
Y- X	0 / 23	-18.5	-18.5	0.02 (4)	10.00
X- W	0 / 17	-18.5	-18.5	0.02 (4)	10.00
W- V	0 / 13	-18.5	-18.5	0.02 (4)	10.00
V- U	0 / 9	-18.5	-18.5	0.02 (4)	10.00
U- T	0 / 7	-18.5	-18.5	0.01 (4)	10.00
T- S	0 / 5	-18.5	-18.5	0.01 (4)	10.00
S- R	0 / 5	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 5	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 5	-18.5	-18.5	0.02 (4)	10.00
P- O	0 / 5	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 5	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-249 / 0	0.04 (1)
X- D	-258 / 0	0.05 (1)
W- E	-256 / 0	0.07 (1)
V- F	-266 / 0	0.11 (1)
U- G	-159 / 0	0.10 (1)
S- I	-230 / 0	0.15 (1)
R- J	-267 / 0	0.17 (1)
P- K	-249 / 0	0.16 (1)
O- L	-304 / 0	0.20 (1)
T- H	-120 / 0	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.02/1.00 (O-P:4) , WB=0.20/1.00 (L-O:1) , SSI=0.13/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (E) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040319

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T96	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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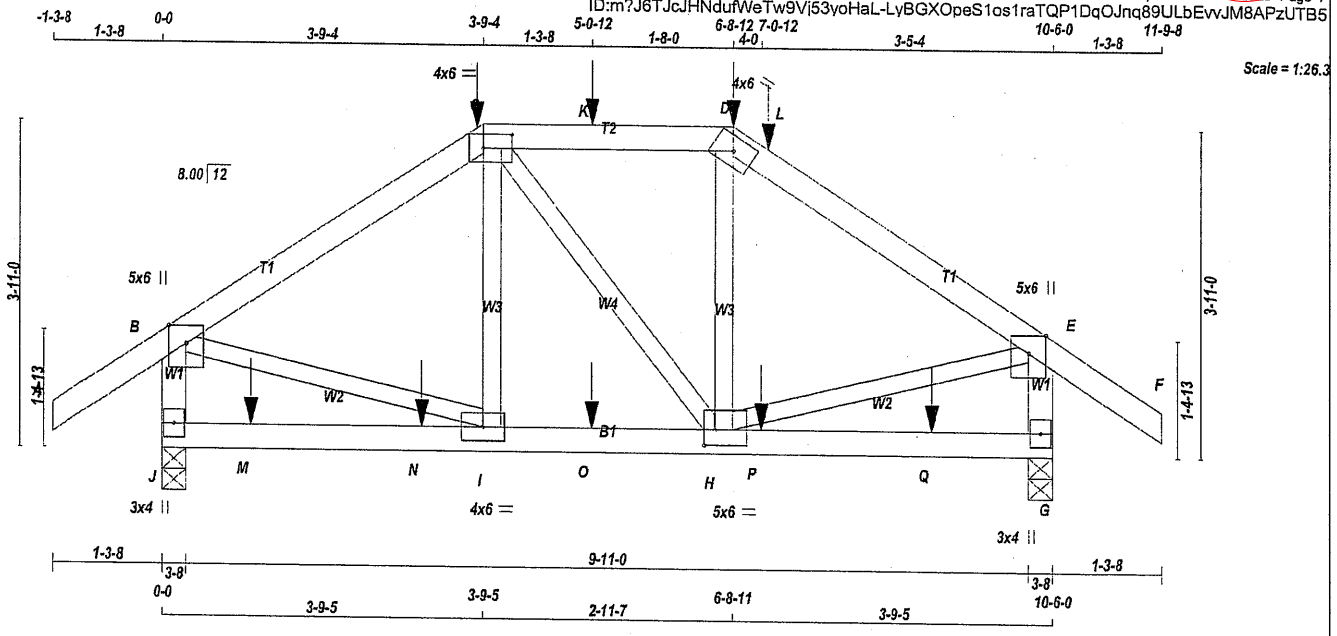


TOTAL WEIGHT = 47 lb [M]

LUMBER
N. L. G. A. RULES

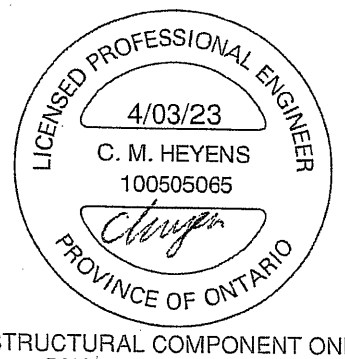
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-l	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040316

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1516	0	1516	0	0	3-8	3-8
G	1569	0	1569	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1053	800 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
G	1088	834 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	I- C	-198 / 50	0.05 (1)
B- C	-1323 / 0	-129.6	-129.6	0.39 (1)	5.09	C- H	0 / 66	0.02 (1)
C- K	-1133 / 0	-129.6	-129.6	0.38 (1)	5.40	H- D	-261 / 43	0.07 (1)
K- D	-1133 / 0	-129.6	-129.6	0.38 (1)	5.40	B- I	0 / 1140	0.28 (1)
D- L	-1338 / 0	-129.6	-129.6	0.44 (1)	4.98	H- E	0 / 1188	0.29 (1)
L- E	-1338 / 0	-129.6	-129.6	0.44 (1)	4.98			
E- F	0 / 49	-129.6	-129.6	0.19 (1)	10.00			
J- B	-1465 / 0	0.0	0.0	0.17 (1)	6.72			
G- E	-1520 / 0	0.0	0.0	0.17 (1)	6.62			
J- M	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
M- N	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
N- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
I- O	0 / 1093	-18.5	-18.5	0.24 (1)	10.00			
O- H	0 / 1093	-18.5	-18.5	0.24 (1)	10.00			
H- P	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
P- Q	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
Q- G	0 / 0	-18.5	-18.5	0.12 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-9-4	-34	-34	---	FRONT	VERT	DEAD	---	C1
C	3-9-4	-217	-217	---	FRONT	VERT	SNOW	---	C1
D	6-8-12	-34	-34	---	FRONT	VERT	DEAD	---	C1
D	6-8-12	-217	-217	---	FRONT	VERT	SNOW	---	C1
K	5-0-12	-94	-94	---	BACK	VERT	TOTAL	---	C1
L	7-0-12	-123	-123	---	BACK	VERT	TOTAL	---	C1
M	1-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
N	3-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
O	5-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
P	7-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
Q	9-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.24/1.00 (H-I:1) , WB=0.29/1.00 (E-H:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.60 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T97	7	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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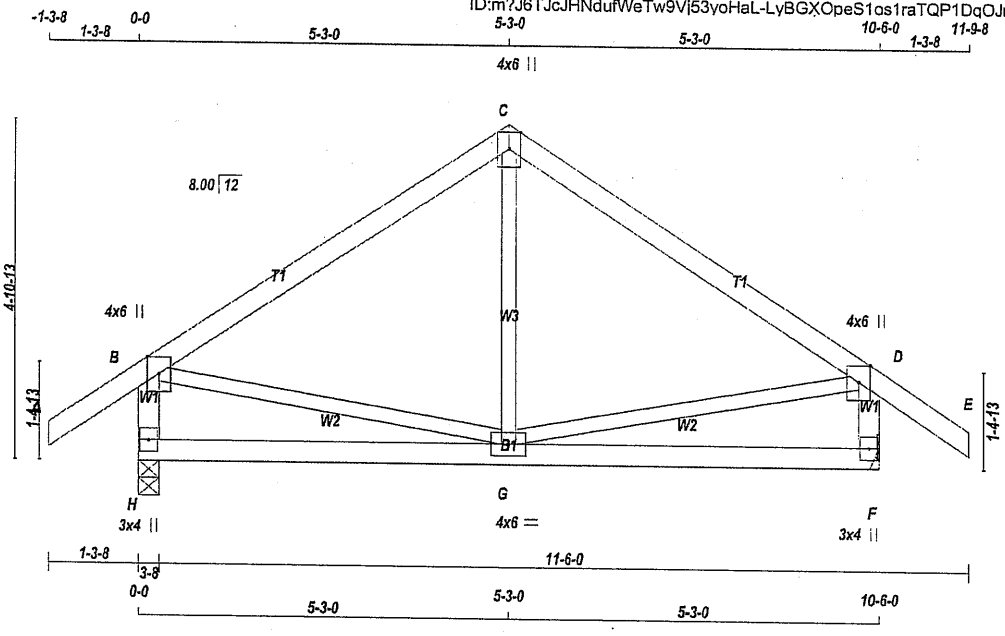

TOTAL WEIGHT = 7 X 44 = 308 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	956	0	956	0	0	3-8	3-8
F	956	0	956	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	663	506 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
F	663	506 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	G- C	-70 / 82	0.03 (4)
B- C	-551 / 0	-129.6	-129.6	0.46 (1)	6.25	B- G	0 / 468	0.11 (1)
C- D	-551 / 0	-129.6	-129.6	0.46 (1)	6.25	G- D	0 / 468	0.11 (1)
D- E	0 / 49	-129.6	-129.6	0.17 (1)	10.00			
H- B	-918 / 0	0.0	0.0	0.09 (1)	7.81			
F- D	-918 / 0	0.0	0.0	0.09 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.46/1.00 (C-D:1) , BC=0.14/1.00 (F-G:4) , WB=0.11/1.00 (D-G:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (G) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040317

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G97	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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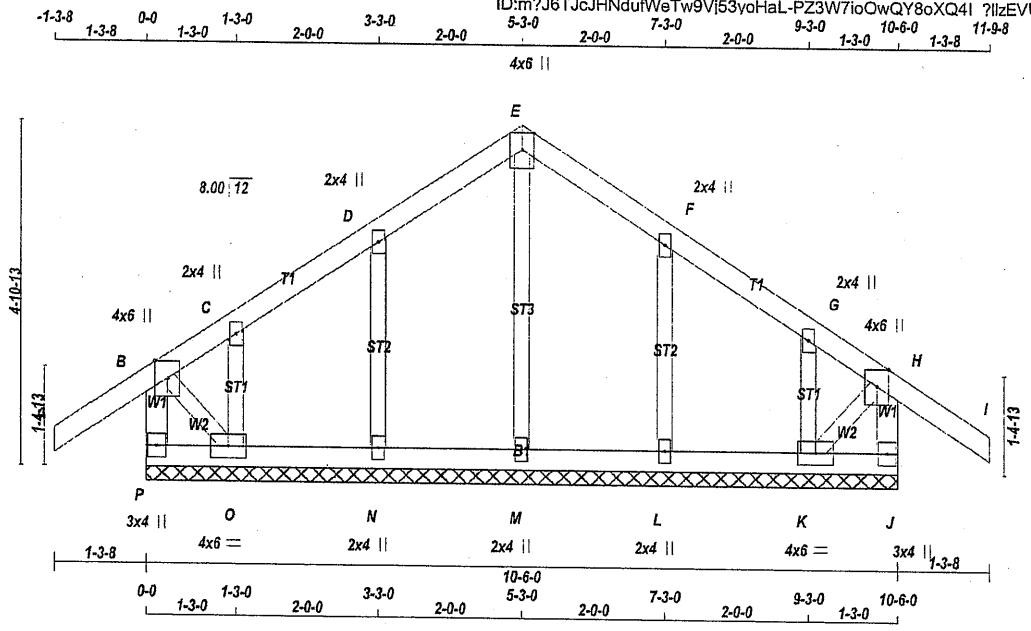

TOTAL WEIGHT = 46 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-70 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	0 / 0	-129.6	-129.6	0.08 (1)	10.00
D- E	-20 / 0	-129.6	-129.6	0.08 (1)	6.25
E- F	-20 / 0	-129.6	-129.6	0.08 (1)	6.25
F- G	0 / 0	-129.6	-129.6	0.08 (1)	10.00
G- H	-70 / 0	-129.6	-129.6	0.17 (1)	6.25
H- I	0 / 49	-129.6	-129.6	0.17 (1)	10.00
P- B	-371 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-371 / 0	0.0	0.0	0.04 (1)	7.81
P- O	0 / 0	-18.5	-18.5	0.01 (4)	10.00
O- N	0 / 11	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 3	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 3	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 11	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-206 / 0	0.07 (1)
N- D	-315 / 0	0.07 (1)
O- C	-83 / 0	0.01 (1)
L- F	-315 / 0	0.07 (1)
K- G	-83 / 0	0.01 (1)
B- O	0 / 20	0.00 (1)
K- H	0 / 20	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (H-I:1) , BC=0.02/1.00 (N-O:4) , WB=0.07/1.00 (E-M:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040313

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T98	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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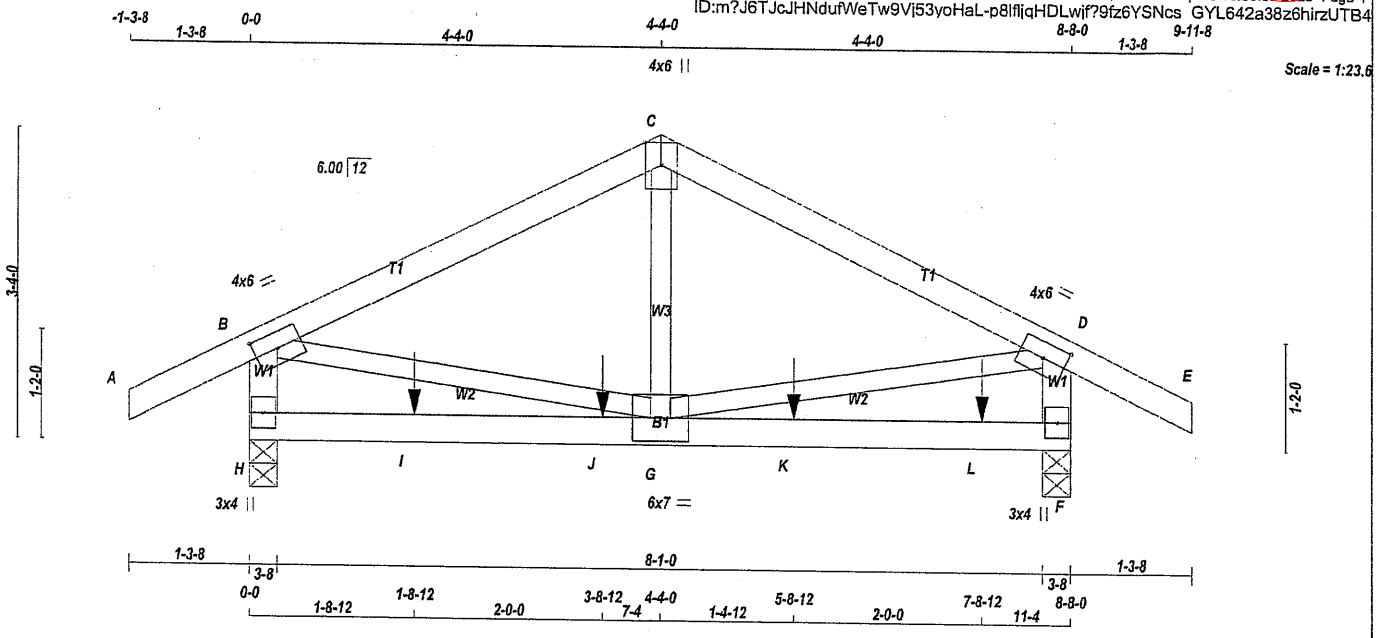

TOTAL WEIGHT = 2 X 35 = 70 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
H- B	1	12	TOP
F- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- F	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040318

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	2521	0	2521	0	0	3-8	3-8
F	2864	0	2864	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1746	1355 / 0	0 / 0	0 / 0	0 / 0	391 / 0	0 / 0
F	1983	1540 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.09 (1)	10.00	G- C	0 / 1721	0.21 (1)
B- C	-2552 / 0	-129.6	-129.6	0.27 (1)	5.36	B- G	0 / 2327	0.29 (1)
C- D	-2552 / 0	-129.6	-129.6	0.27 (1)	5.36	G- D	0 / 2327	0.29 (1)
D- E	0 / 39	-129.6	-129.6	0.09 (1)	10.00			
H- B	-2050 / 0	0.0	0.0	0.11 (1)	7.72			
F- D	-2050 / 0	0.0	0.0	0.11 (1)	7.72			
H- I	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
I- J	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
J- G	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
G- K	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
K- L	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
L- F	0 / 0	-18.5	-18.5	0.85 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	1-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
J	3-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
K	5-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
L	7-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 837 (0.12")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.85/1.00 (G-H:1) , WB=0.29/1.00 (B-G:1) , SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (G) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
404995	T98	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	6.0	2.00	3.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	6.0	7.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040318

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G98	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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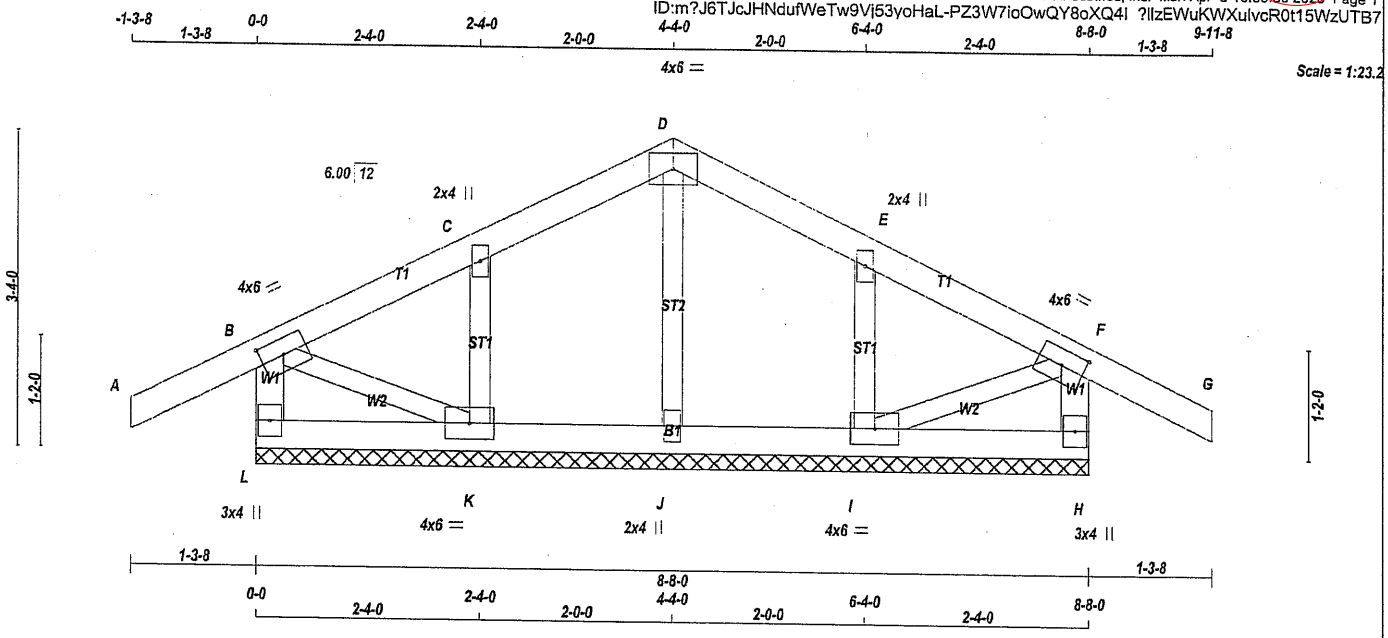

TOTAL WEIGHT = 2 X 35 = 69 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- D 1	12	TOP
D- G 1	12	TOP
L- B 1	12	TOP
H- F 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
L- H 1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040314

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.08 (1)	10.00	J- D	-188 / 0	0.02 (1)
B- C	-6 / 0	-129.6	-129.6	0.05 (1)	10.00	K- C	-345 / 0	0.03 (1)
C- D	-25 / 0	-129.6	-129.6	0.05 (1)	6.25	I- E	-345 / 0	0.03 (1)
D- E	-25 / 0	-129.6	-129.6	0.05 (1)	6.25	B- K	0 / 20	0.00 (1)
E- F	-6 / 0	-129.6	-129.6	0.05 (1)	10.00	I- F	0 / 20	0.00 (1)
F- G	0 / 39	-129.6	-129.6	0.08 (1)	10.00			
L- B	-306 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-306 / 0	0.0	0.0	0.02 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
K- J	0 / 7	-18.5	-18.5	0.01 (4)	10.00			
J- I	0 / 7	-18.5	-18.5	0.01 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (F-G:1) , BC=0.01/1.00 (H-I:4) , WB=0.03/1.00 (E-I:1) , SSI=0.06/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (C) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
404995	G98	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	BMWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040314

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	G300	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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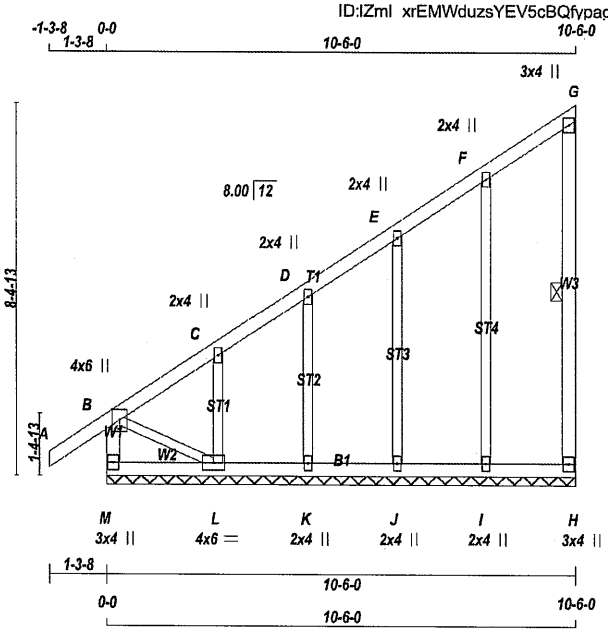

Scale = 1:49.8

TOTAL WEIGHT = 2 X 55 = 110 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
M - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F						
C	TMW+w	MT20	2.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMWW1-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-329 / 0		0.0	0.0	0.03 (1)	7.81	I- F	-291 / 0	0.25 (1)
A- B	0 / 49		-129.6	-129.6	0.17 (1)	10.00	J- E	-256 / 0	0.13 (1)
B- C	-10 / 0		-129.6	-129.6	0.10 (1)	10.00	K- D	-236 / 0	0.07 (1)
C- D	-24 / 0		-129.6	-129.6	0.10 (1)	6.25	L- C	-327 / 0	0.06 (1)
D- E	-7 / 0		-129.6	-129.6	0.06 (1)	10.00	B- L	0 / 24	0.01 (1)
E- F	-2 / 0		-129.6	-129.6	0.07 (1)	10.00			
F- G	-14 / 0		-129.6	-129.6	0.07 (1)	6.25			
H- G	-110 / 0		0.0	0.0	0.03 (1)	6.25			
M- L	0 / 0		-18.5	-18.5	0.03 (4)	10.00			
L- K	0 / 12		-18.5	-18.5	0.03 (4)	10.00			
K- J	0 / 7		-18.5	-18.5	0.01 (4)	10.00			
J- I	0 / 4		-18.5	-18.5	0.02 (4)	10.00			
I- H	0 / 0		-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.03/1.00 (K-L:4) , WB=0.25/1.00 (F-I:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (C) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040077

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	J1	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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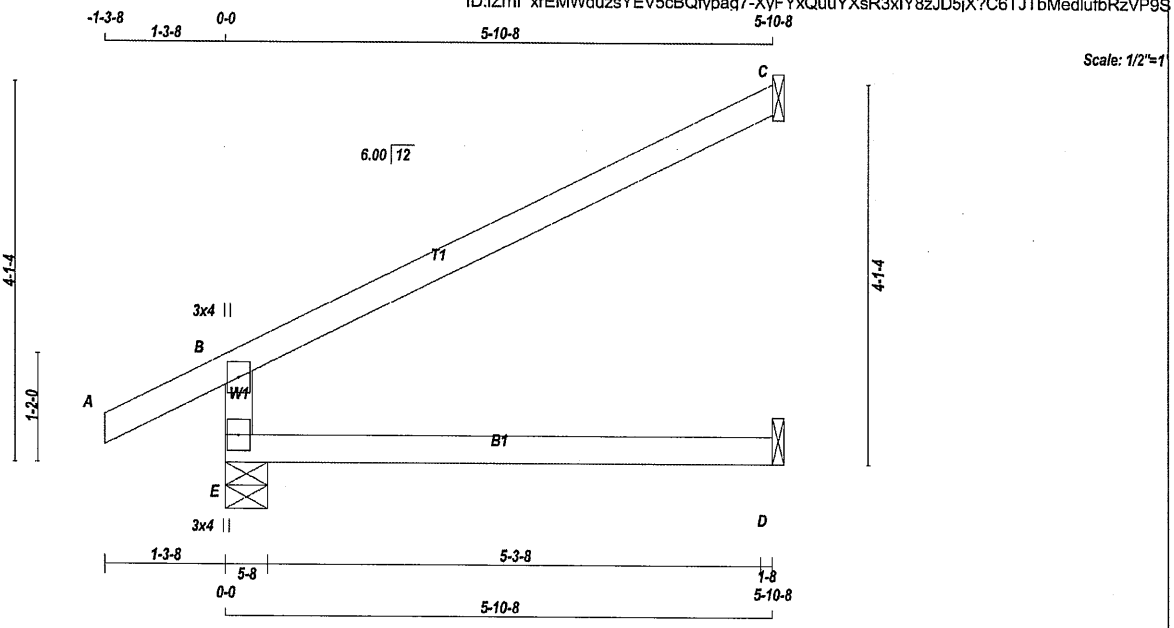

TOTAL WEIGHT = 6 X 17 = 101 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	716	0	716	0	0	5-8	5-8
C	286	0	286	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	496	384 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	195	168 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-652 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-43 / 0	-129.6	-129.6	0.76 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.76/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040078

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	J2	12	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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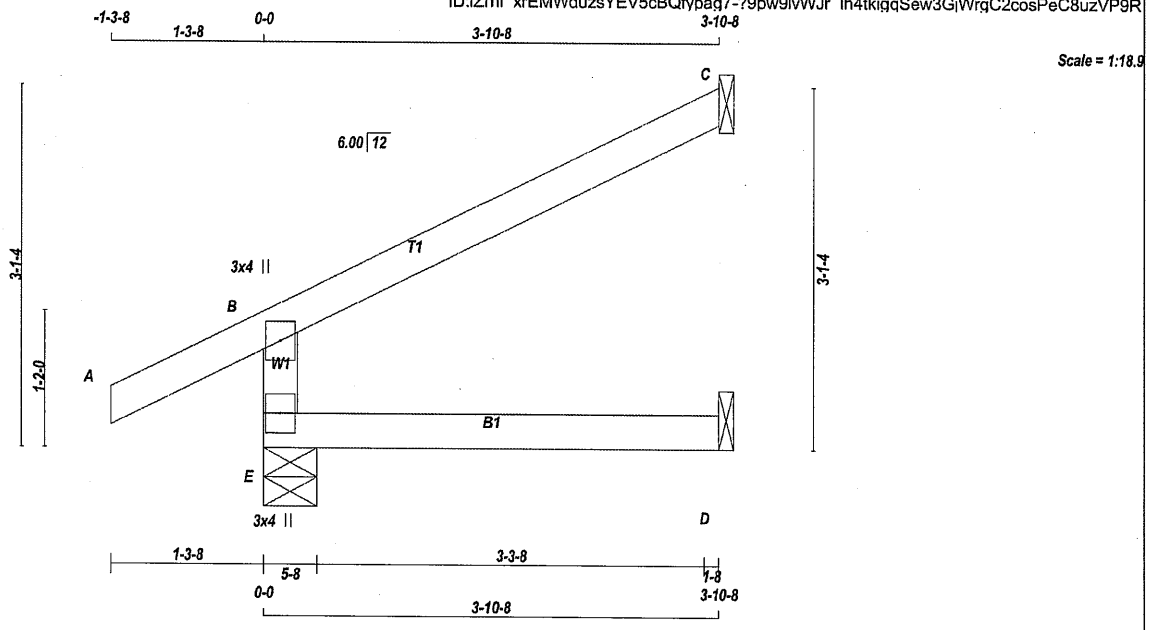

TOTAL WEIGHT = 12 X 12 = 144 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	531	0	531	0	0	5-8	5-8
C	189	0	189	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	367	289 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	129	111 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-490 / 0	0.0	0.0		0.05 (4)	7.81			
A- B	0 / 39	-129.6	-129.6		0.17 (5)	10.00			
B- C	-28 / 0	-129.6	-129.6		0.33 (6)	6.25			
E- D	0 / 0	-18.5	-18.5		0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (B-C:6) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.22/1.00 (B-C:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040079

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	J3	10	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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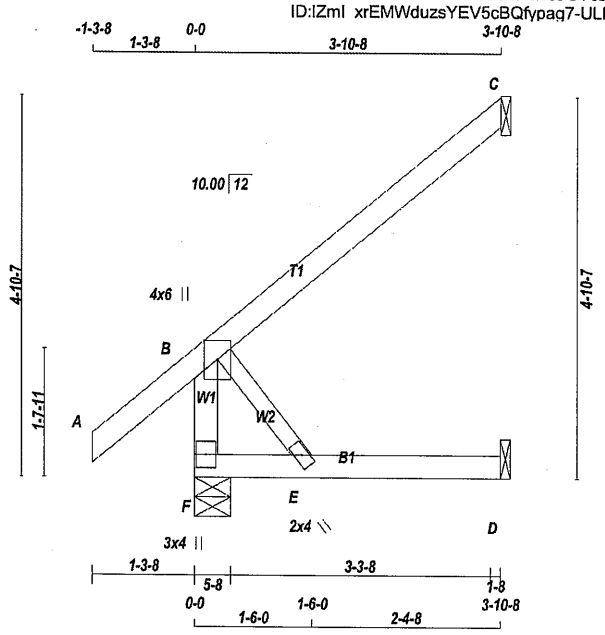

TOTAL WEIGHT = 10 X 15 = 151 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	467	0	467	0	0	5-8	5-8
C	251	0	251	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	322	254 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
C	171	148 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-431 / 0	0.0	0.0	0.05 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 57	-129.6	-129.6	0.18 (5)		10.00			
B- C	0 / 0	-129.6	-129.6	0.33 (1)		10.00			
F- E	0 / 0	-18.5	-18.5	0.07 (4)		10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040080

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	J4	20	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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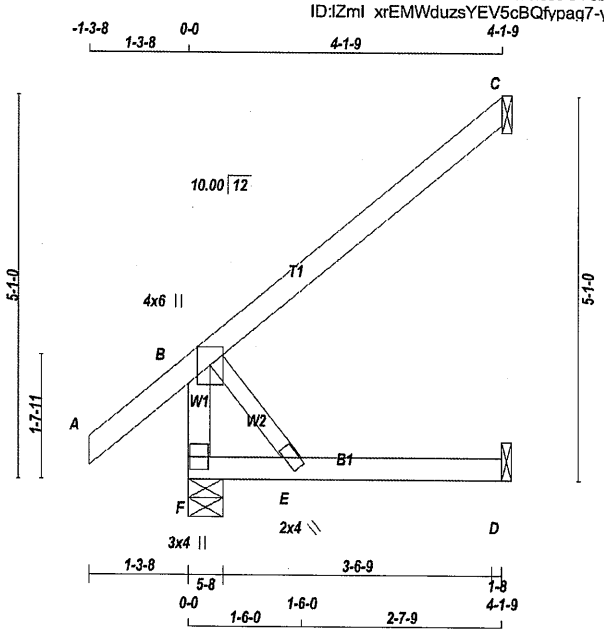

TOTAL WEIGHT = 20 X 16 = 316 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	485	0	485	0	0	5-8	5-8
C	268	0	268	0	0	1-8	1-8
D	38	0	43	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	336	264 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
C	183	158 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	31	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-447 / 0	0.0	0.0	0.05 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00			
B- C	0 / 0	-129.6	-129.6	0.38 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.09/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040081

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J50	18	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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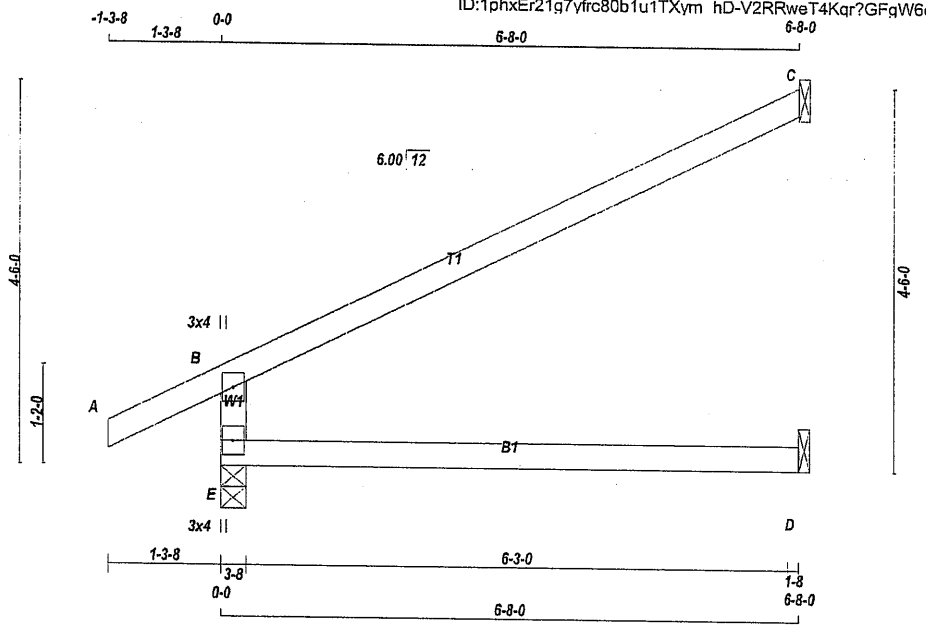

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	789	0	789	0	0	3-8	3-8
C	324	0	324	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	547	422 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
C	221	191 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-716 / 0	0.0	0.0	0.14 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-48 / 0	-129.6	-129.6	0.76 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

TOTAL WEIGHT = 18 X 19 = 337 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.76/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040320

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J51	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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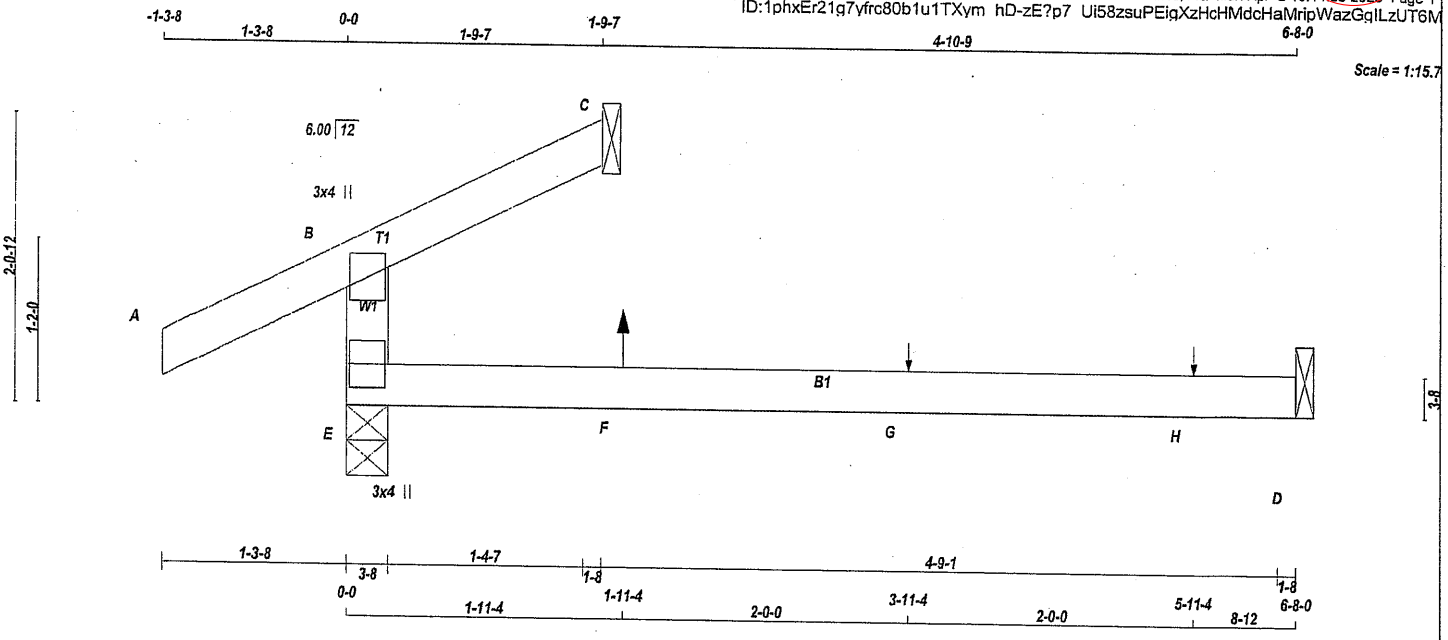

TOTAL WEIGHT = 2 X 12 = 25 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	386	0	386	0	0	3-8	3-8
C	86	0	86	0	0	1-8	1-8
D	49	0	59	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	268	206 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	63	30 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
D	39	0 / -3	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-324 / 0	0.0	0.0	0.12 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-15 / 14	-129.6	-129.6	0.12 (4)	6.25			
E- F	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
F- G	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
H- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	—	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	—	C1
H	5-11-4	1	1	---	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040321

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J52	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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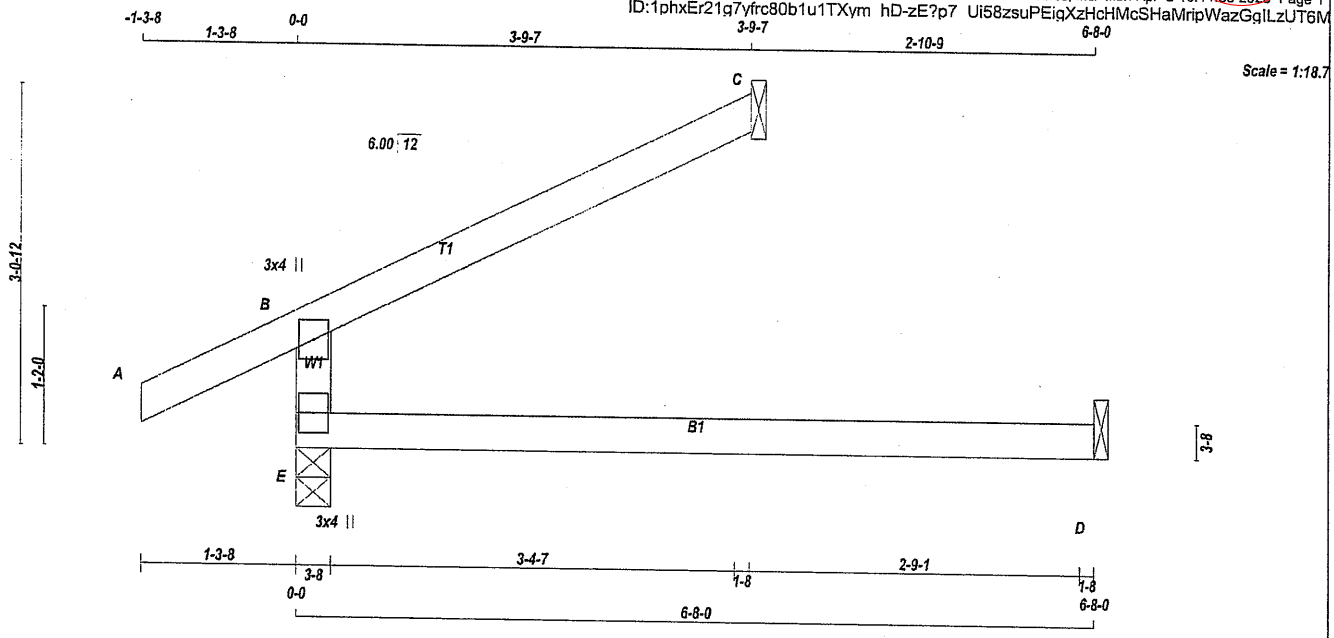

TOTAL WEIGHT = 2 X 15 = 30 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	555	0	555	0	0	3-8	3-8
C	184	0	184	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	387	284 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
C	126	109 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-483 / 0	0.0	0.0	0.15 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-27 / 0	-129.6	-129.6	0.24 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040322

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J53	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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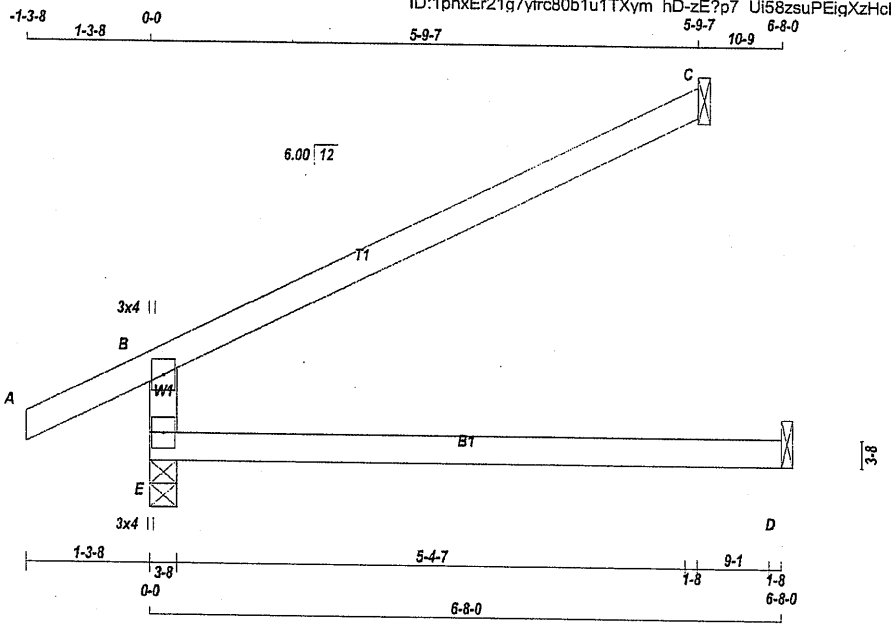

TOTAL WEIGHT = 2 X 18 = 35 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	718	0	718	0	0	3-8	3-8
C	281	0	281	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	498	380 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
C	192	166 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-645 / 0	0.0	0.0	0.15 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-42 / 0	-129.6	-129.6	0.57 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040323

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J54	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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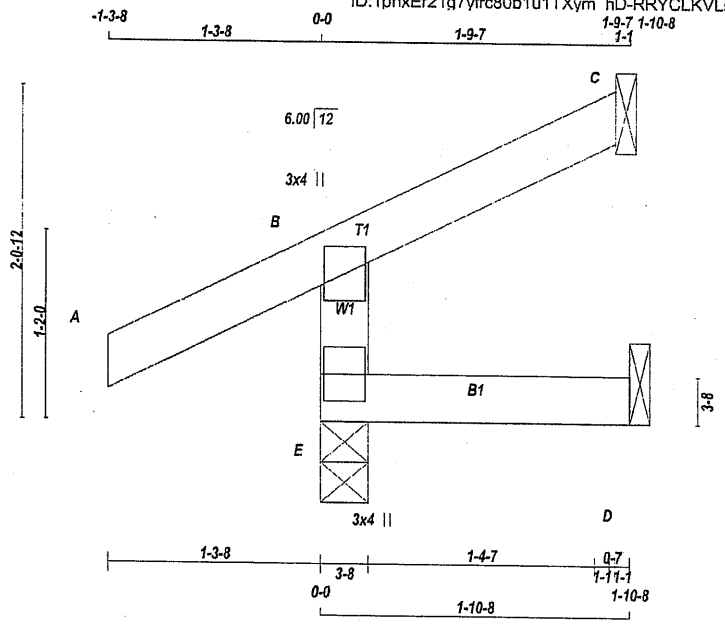

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	375	0	375	0	0	3-8	3-8
C	62	0	62	0	-36	1-8	1-8
D	5	0	17	0	-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	258	211 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	43	36 / -27	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	5	0 / -12	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-345 / 0	0.0	0.0	0.05 (5)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-24 / 0	-129.6	-129.6	0.13 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.05 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040324

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOTAL WEIGHT = 2 X 7 = 14 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.05/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J55	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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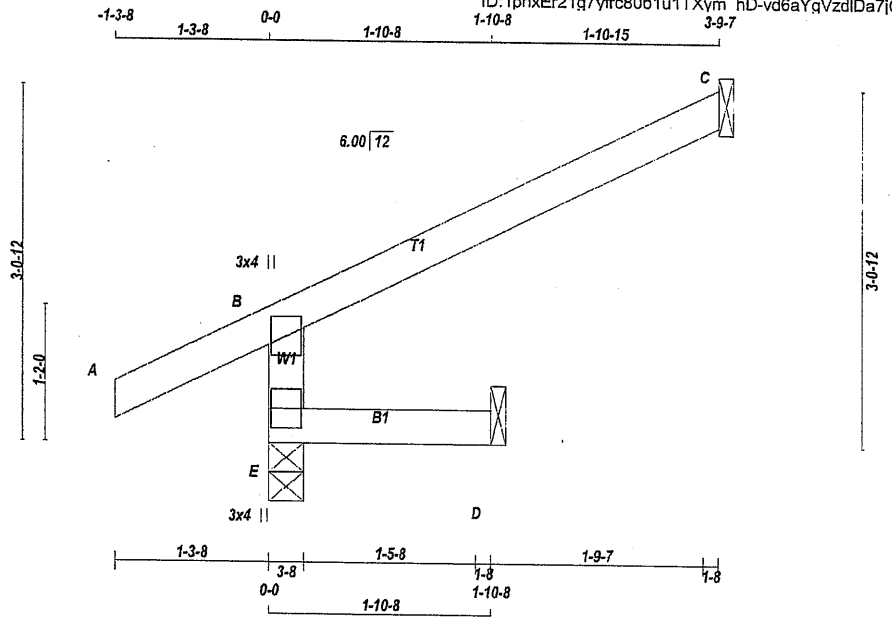

TOTAL WEIGHT = 2 X 10 = 19 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	502	0	502	0	0	3-8	3-8
C	184	0	184	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	344	284 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	126	109 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-483 / 0	0.0	0.0	0.01 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-27 / 0	-129.6	-129.6	0.32 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040325

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J56	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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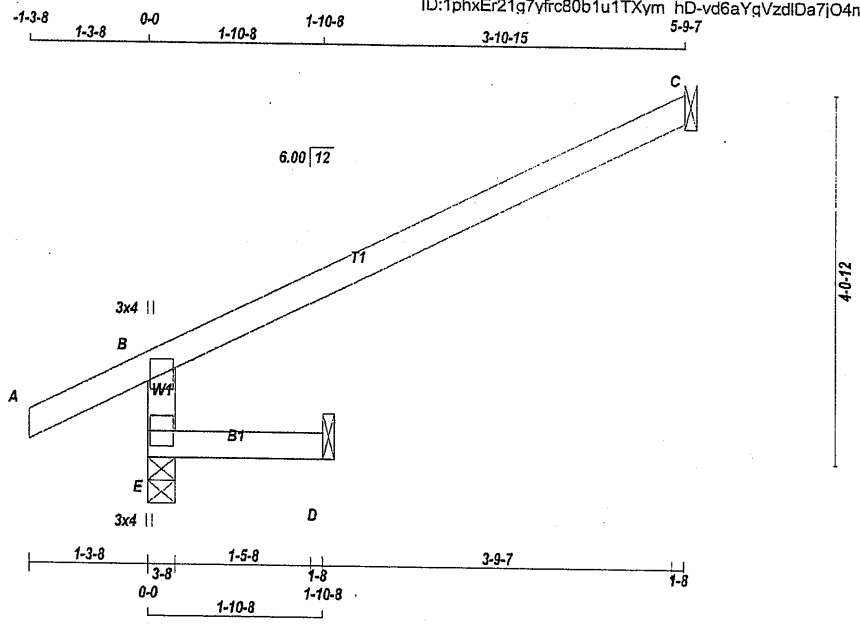

TOTAL WEIGHT = 2 X 12 = 24 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	664	0	664	0	0	3-8	3-8
C	281	0	281	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	455	380 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
C	192	166 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-645 / 0	0.0	0.0	0.01 (1)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-42 / 0	-129.6	-129.6	0.74 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.74/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040326

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 2
REVIEWED
jpenfold
8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	J57	5	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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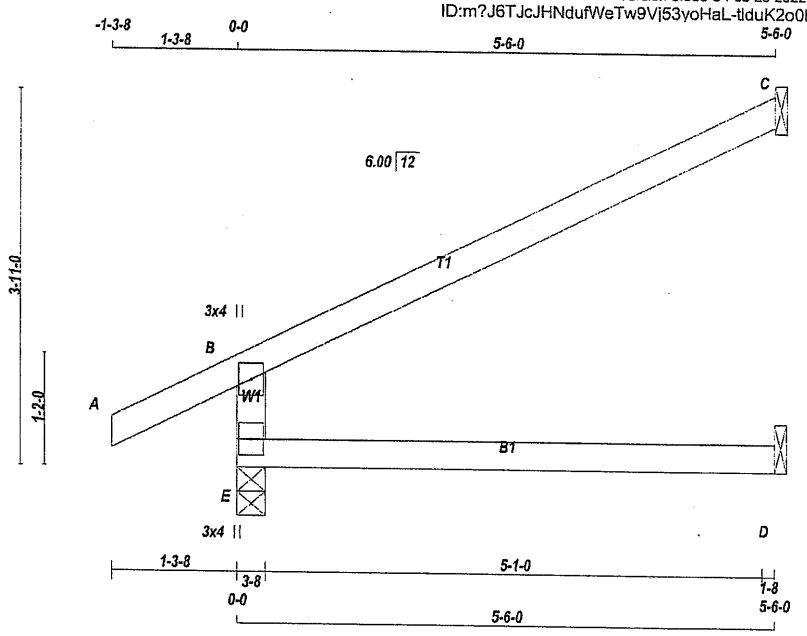

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	681	0	681	0	0	3-8	3-8
C	267	0	267	0	0	1-8	1-8
D	42	0	47	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	472	366 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
C	182	158 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-621 / 0	0.0	0.0	0.11 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-40 / 0	-129.6	-129.6	0.67 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

TOTAL WEIGHT = 5 X 16 = 79 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.11/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040315

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 2 REVIEWED jpenfold 8/15/2023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	J100	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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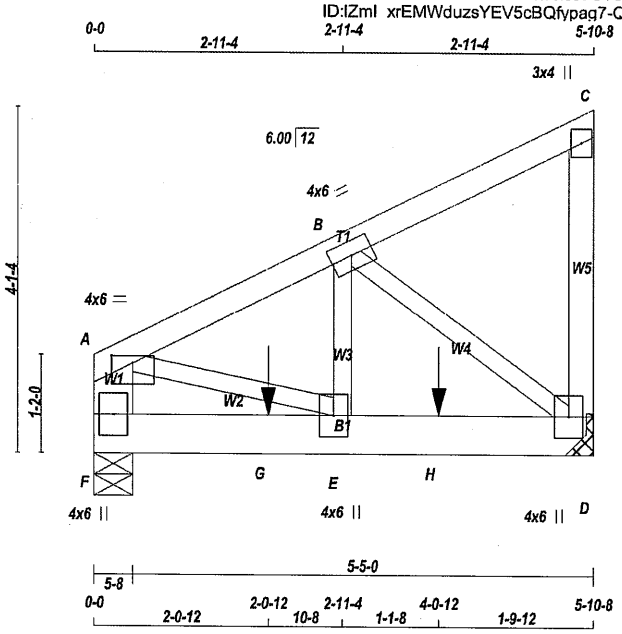

Scale = 1:26.1

TOTAL WEIGHT = 4 X 29 = 117 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040082

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1545	0	1545	0	0	5-8	5-8
D	1644	0	1644	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1072	819 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
D	1141	872 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1319 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 1577	0.20 (1)
A- B	-1689 / 0	-129.6	-129.6	0.09 (1)	6.25	E- B	0 / 1475	0.18 (1)
B- C	-16 / 0	-129.6	-129.6	0.08 (1)	6.25	B- D	-1918 / 0	0.23 (1)
D- C	-155 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
E- H	0 / 1524	-18.5	-18.5	0.30 (1)	10.00			
H- D	0 / 1524	-18.5	-18.5	0.30 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-803	-803	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-803	-803	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.09/1.00 (A-B:1) , BC=0.30/1.00 (D-E:1) , WB=0.23/1.00 (B-D:1) , SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (D) (INPUT = 0.90)
JSI METAL= 0.34 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	J100	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040082

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	J390	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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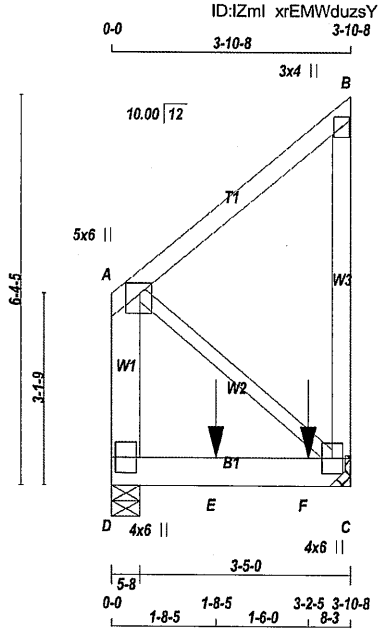


Scale = 1:35.9

TOTAL WEIGHT = 4 X 27 = 110 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040083

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	846	0	846	0	0	5-8	5-8
C	1241	0	1241	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	588	443 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
C	863	651 / 0	0 / 0	0 / 0	0 / 0	212 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-251 / 0	0.0	0.0	0.01 (1)		7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-129.6	-129.6	0.18 (1)		10.00			
C- B	-251 / 0	0.0	0.0	0.09 (1)		7.81			
D- E	0 / 0	-18.5	-18.5	0.36 (1)		10.00			
E- F	0 / 0	-18.5	-18.5	0.36 (1)		10.00			
F- C	0 / 0	-18.5	-18.5	0.36 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-8-5	-524	-524	---	BACK	VERT	TOTAL	---	C1
F	3-2-5	-527	-527	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.36/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.33/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402463	J390	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 14:25:41 2023 Page 2

ID:IZmI_xrEMWduzsYEV5cBQfypag7-QkV3nnxPcmMtYYbJNoO9FZhqGkofPPLEYNtskCzVP9O

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	4.0	6.0		

STRUCTURAL COMPONENT ONLY
DWG # TR23040083

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

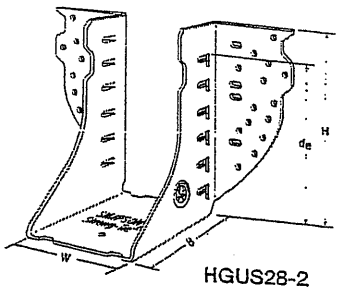
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

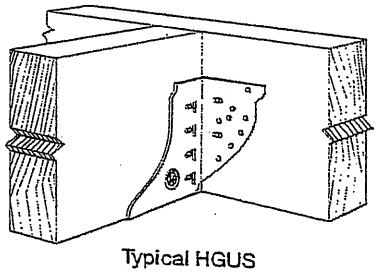
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

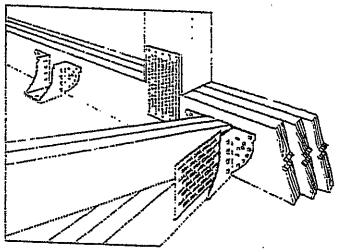
Options:

- See current catalogue for options



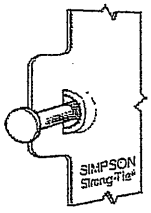
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

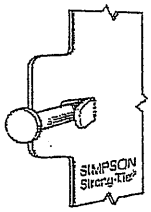
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅜	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 3/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 ½	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 3/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅜	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 3/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 ¼	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 3/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅜	9 1/8	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 3/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9 ¼	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 3/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 3/16	10 5/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 3/16	12 5/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

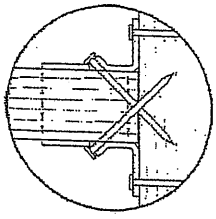


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

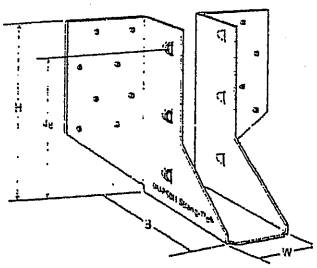
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

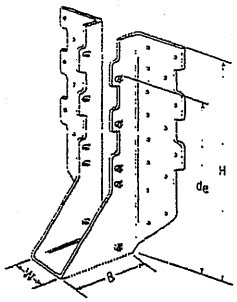
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

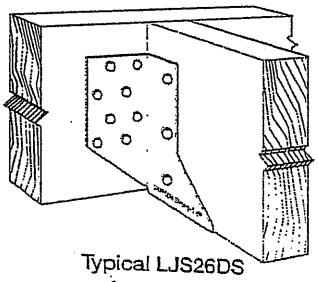
- See current catalogue for options



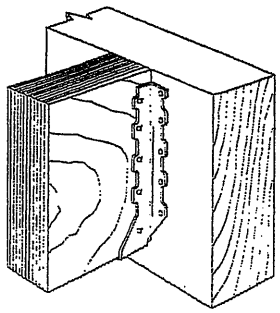
LJS26DS



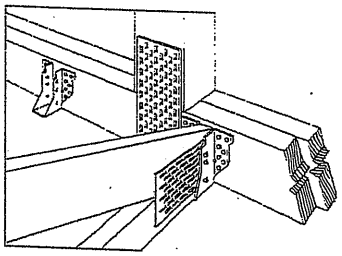
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



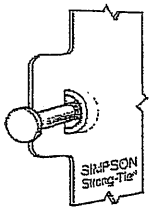
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

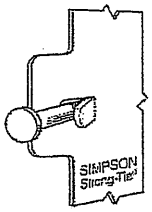
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 3/16	5	3½	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 5/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 5/8	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

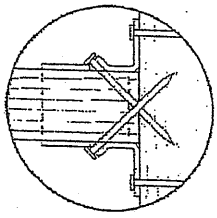


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

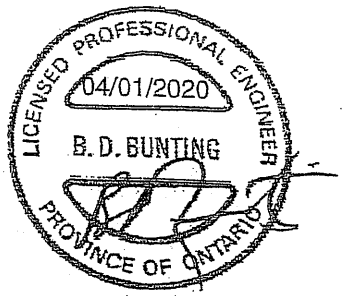
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

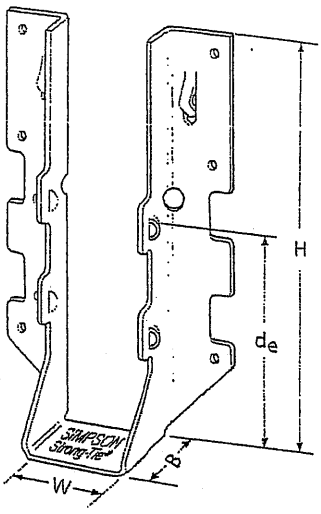
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

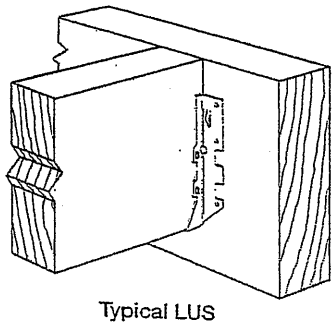
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

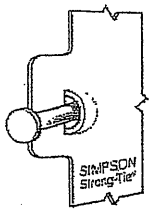


LUS28


Typical LUS Installation

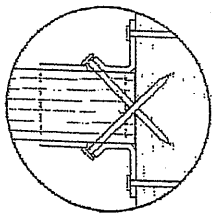
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

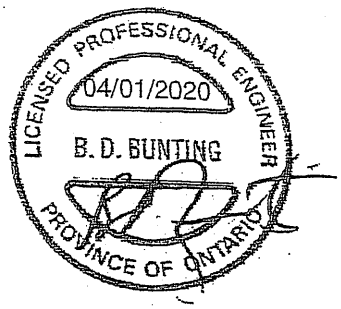
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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(800) 999-5099
strongtie.com

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

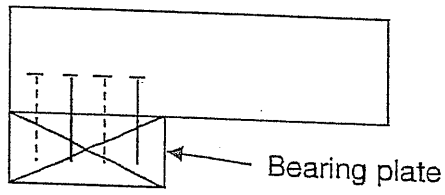
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

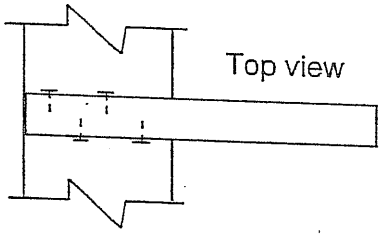
NOTES:

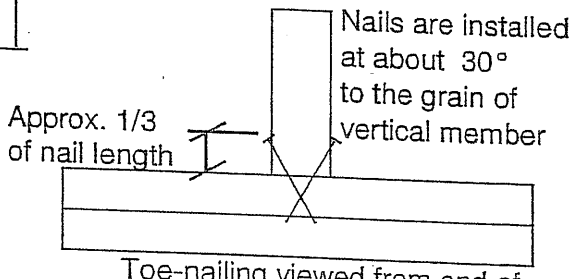
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



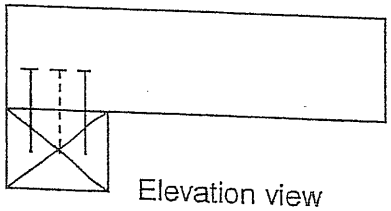
Elevation view



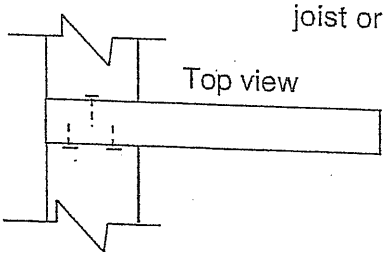


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



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Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

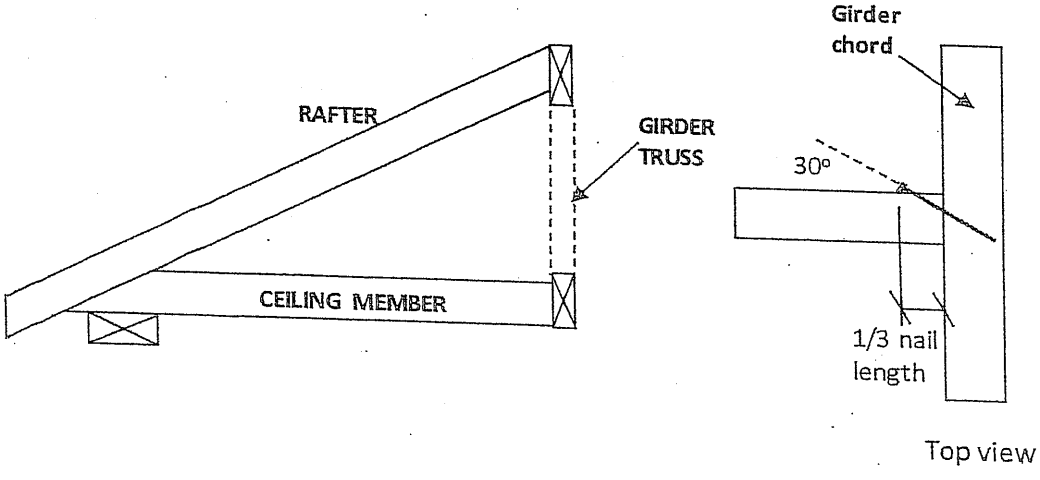

Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

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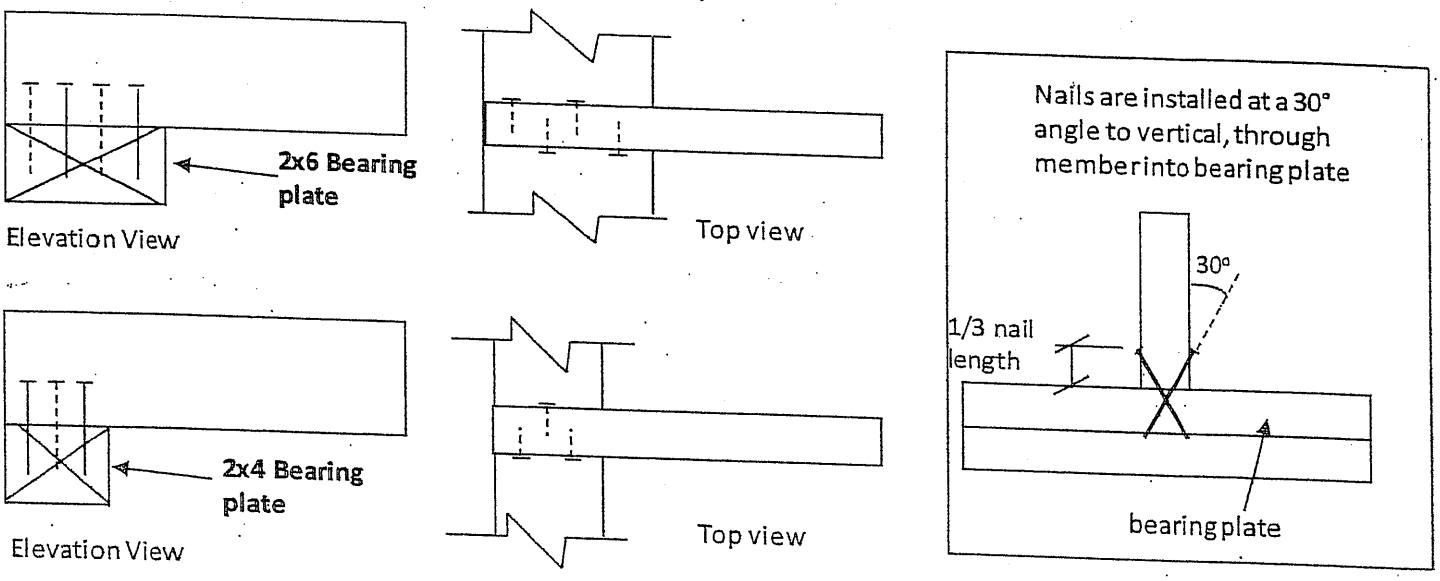
December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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EWP DESIGN INC.

(905) 832-2250 FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

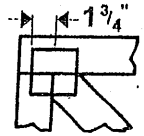
SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

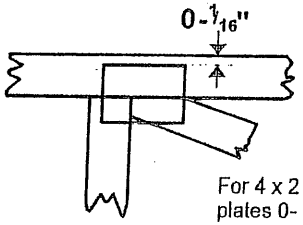
February 1, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

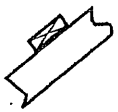
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 X 4

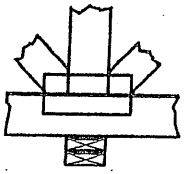
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

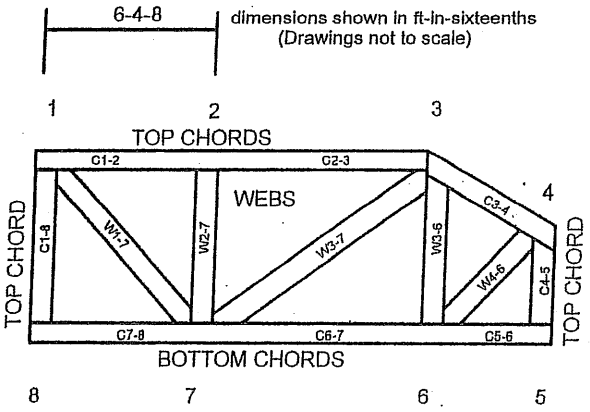


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek®

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.