



2023-0514
2023-0533
2023-0524
2023-0546
2023-0520

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC.. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 -(0)

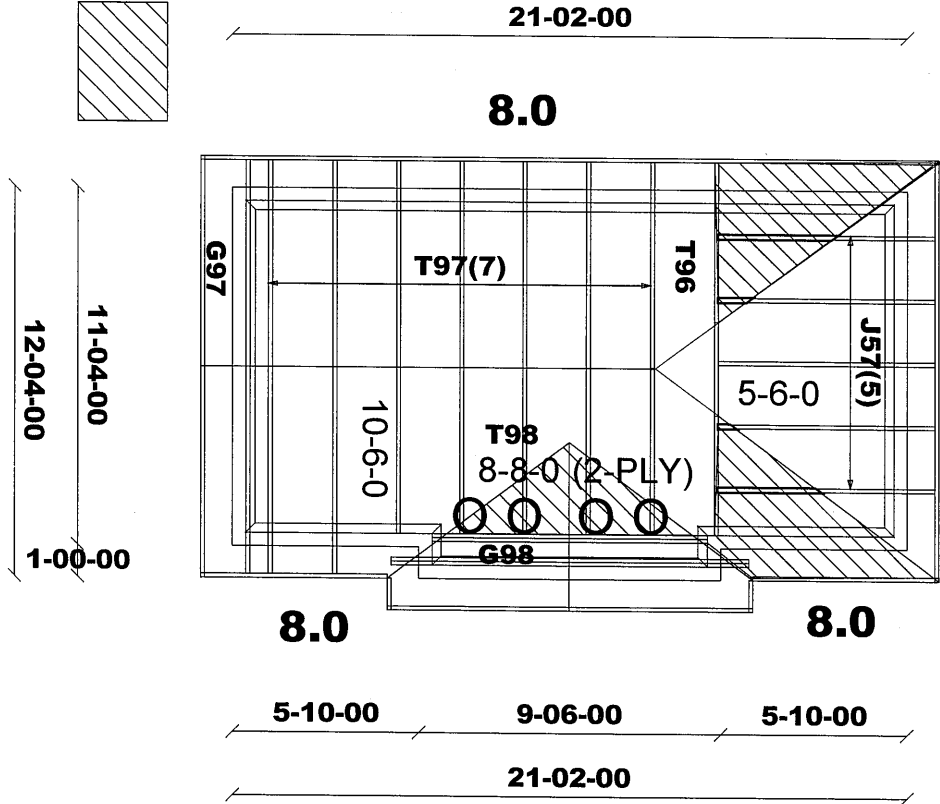
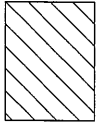
UNIT 1
DG-3
EL:B
404995

11-04-00
6.0
1-00-00

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES
CONVENTIONAL
FRAMING



All conventional framing to conform with Part 9 of O.B.C. 2012. Roof
rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c
with a vertical post to the truss at each cross point. Vertical posts longer
than 6' to have lateral bracing so that the distance between the post end
points and lateral bracing does not exceed 6'.

GARAGE



Job Track: **50469**

Plan Log: **200774**

Layout ID: **404995**

Builder / Location:
BAYVIEW WELLINGTON / INNISFIL

Project: **ALCONA SHORES**

Date: **2023-03-28**

Model / Elevation:
BLOCK 1 - DG-3 EL:B UNIT1

Designer: S.V

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC. AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC. IF UTILIZED FOR ANY OTHER PURPOSE.

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ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

UNIT 3
DG-1
EL:B (REV)
404952

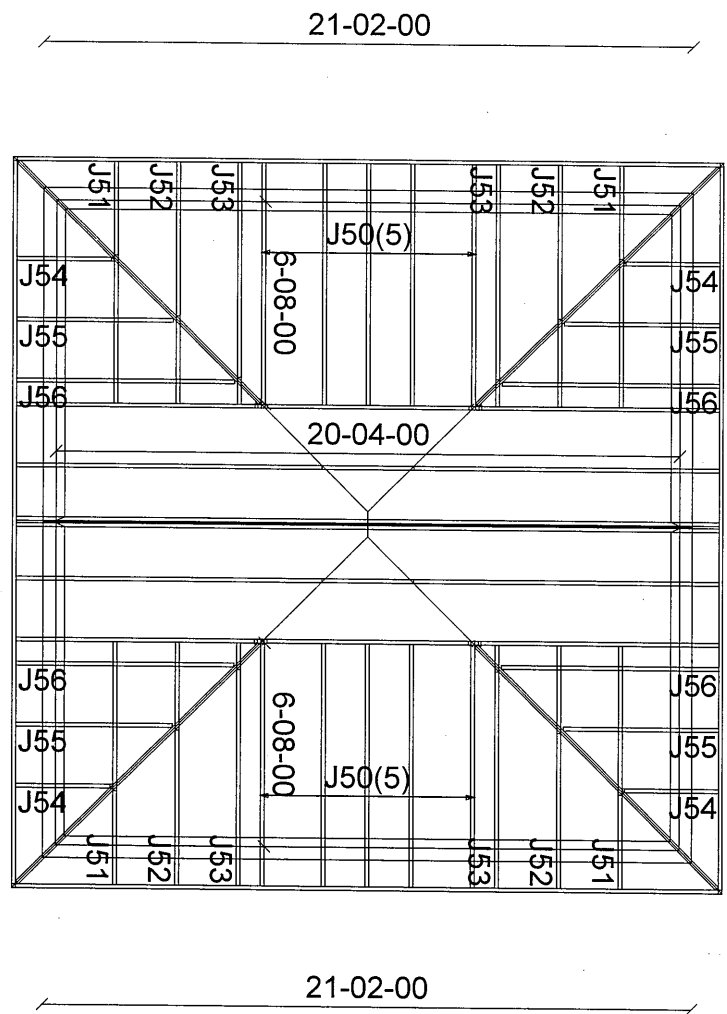
UNIT 2
DG-1
EL:B
404952

6/12 PITCHES (TYP.)
UNLESS NOTED

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES
CONVENTIONAL
FRAMING



GARAGE

		Job Track: 50469		Builder / Location: BAYVIEW WELLINGTON / INNISFIL		Model / Elevation: BLOCK 1 -DG-1 EL:B UNIT 3 PAIRED WITH DG-1 EL:B UNIT 2	
Plan Log: 200774		Project: ALCONA SHORES		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
Layout ID: 404994		Date: 2023-03-28		Sales: Mario Dicano		Designer: S.V	
						Mtek ver 8.5.3.233	



UNIT 6
DG-1
EL:A2 (REV)
404952

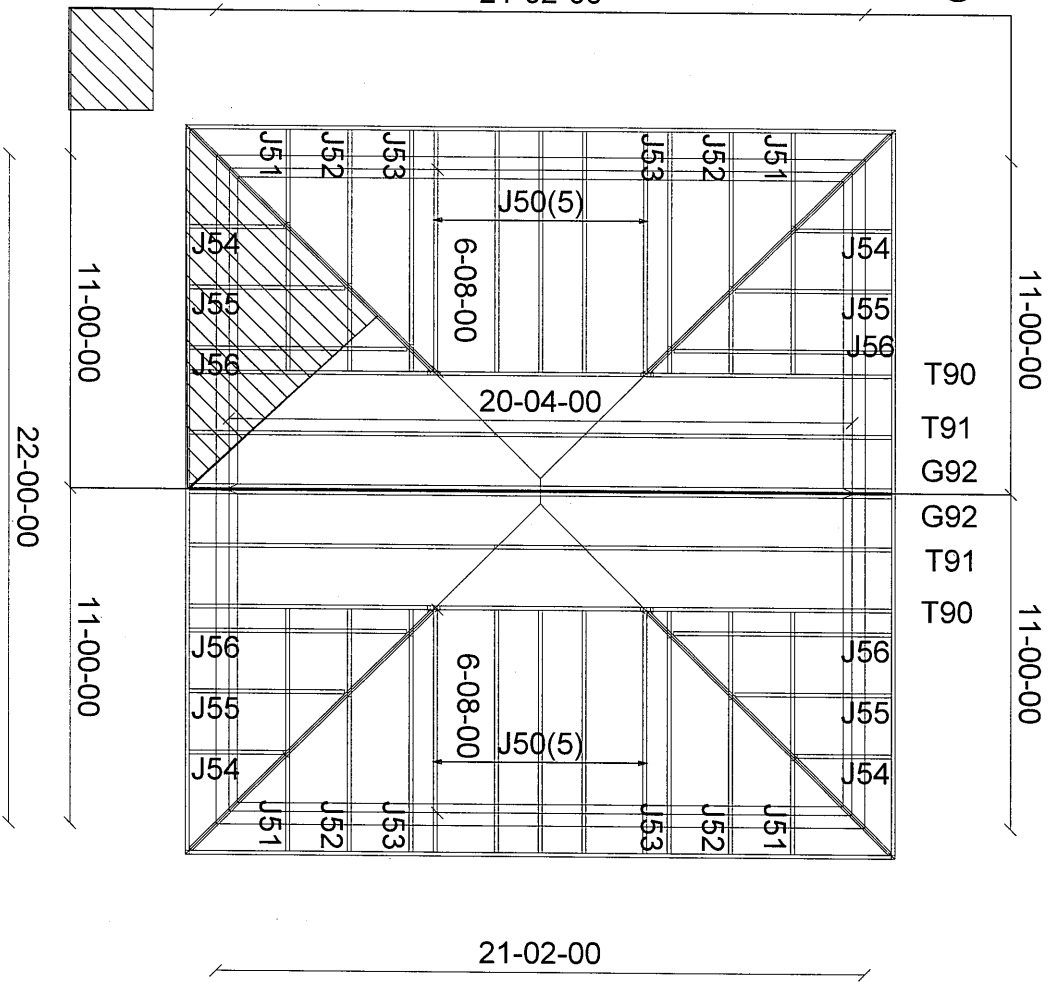
UNIT 5
DG-1
EL:A
404952

6/12 PITCHES (TYP.)
UNLESS NOTED

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES
CONVENTIONAL
FRAMING



All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

GARAGE

**TAMARACK**
TRUSSES INC.

Job Track: **50469**

Plan Log: **200893**

Layout ID: **404951**

Builder / Location:
BAYVIEW WELLINGTON / INNISFIL

Project: **ALCONA SHORES**

Date: **2023-03-28**

Sales: **Mario DiCaro**

Designer: **S.V.**

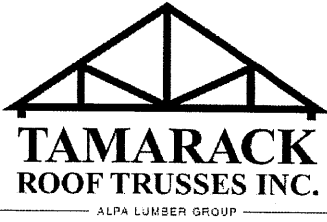
PURPOSE: THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC. IF UTILIZED FOR ANY OTHER PURPOSE.

Model / Elevation:
**BLOCK 2-DG-1 EL:A2 UNIT 6 PAIRED
WITH DG-1 EL:A UNIT 5**

WITH DG-1 EL:A UNIT 5

Intek ver 6.3.3.233

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B UNIT 1

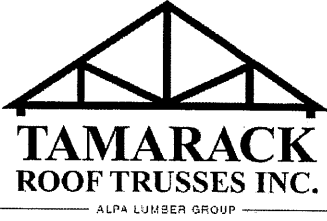
Job Track: 50469
 PlanLog: 200774
 Layout ID: 404966
 Ref #
 Page: 1 of 3
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T48 Hip Girder	8 /12	19-10-00	6-04-05	2 x 4 2 x 6	1-03-08	1-04-13 1-08-13	198.07 126.67		
	1	T49 Common	8 /12	19-10-00	8-02-02	2 x 4	1-03-08	1-04-13 1-08-13	86.62 55.00		
	1	T50 Half Hip	8 /12	19-10-00	8-03-11	2 x 4	1-03-08	1-04-13 8-03-11	100.37 62.83		
	1 2-ply	T51A Hip Girder	8 /12	19-10-00	3-11-13	2 x 4 2 x 6	1-03-08	1-04-13 1-08-02	188.34 121.33		
	1	T52A Hip	8 /12	19-05-00	5-03-13	2 x 4		1-08-02 1-08-02	81.33 52.00		
	1	T53A Hip	8 /12	19-05-00	6-07-13	2 x 4		1-08-02 1-08-02	84.06 55.33		
	2	T54 Common	8 /12	19-10-00	8-01-13	2 x 4	1-03-08	1-08-02 1-04-13	172.97 110.00		
	1	T54A Common	8 /12	19-05-00	8-01-13	2 x 4		1-08-02 1-04-13	83.71 53.67		
	1	T54AC Hip	8 /12	19-05-00	7-11-13	2 x 4		1-08-02 1-04-13	83.5 53.67		
	1	T55 Half Hip	8 /12	19-10-00	7-07-00	2 x 4	1-03-08	1-04-13 7-07-00	96.35 59.50		
	1	T56 Half Hip	8 /12	19-10-00	8-07-00	2 x 4	1-03-08	1-04-13 8-07-00	101.96 64.33		
	1	T57 Half Hip	8 /12	19-10-00	9-07-00	2 x 4	1-03-08	1-04-13 9-07-00	111.62 70.00		
	1	T47 Scissor	8 /12 4 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	56.66 37.67		
	2	T47A Scissor	8 /12 4 /12	13-01-00	5-10-13	2 x 4	1-03-08	1-04-13 1-04-13	107.2 71.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B UNIT 1

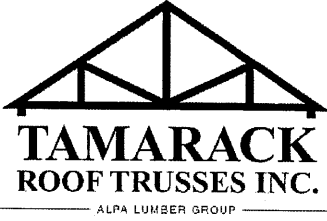
Job Track: 50469
 PlanLog: 200774
 Layout ID: 404966
 Ref #
 Page: 2 of 3
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G58 GABLE	8 /12	13-00-00	5-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	57.01 36.33		
	1 2-ply	T59 Common Girder	8 /12	13-00-00	5-08-13	2 x 4 2 x 6		1-04-13 1-04-13	130.01 83.00		
	1	T62Z Common Girder	8 /12	10-06-00	5-00-13	2 x 4	1-03-08	1-04-13 1-07-02	45.42 29.67		
	4	T62 Common	8 /12	10-06-00	5-00-13	2 x 4	1-03-08	1-04-13 1-07-02	181.68 118.67		
	2	T62A Common	8 /12	10-01-00	5-00-13	2 x 4		1-04-13 1-07-02	81.86 55.00		
	2	T61A Common	10 /12	12-06-00	8-06-03	2 x 4	1-03-08	1-07-11 4-11-11	125.46 78.00		
	1	T15 Half Hip	10 /12	12-09-08	11-08-05	2 x 6 2 x 4	1-03-08	1-07-11 12-03-10	88.01 54.00		
	4	T16 Monopitch	10 /12	12-09-08	12-03-10	2 x 4	1-03-08	1-07-11 12-03-10	310.71 193.33		
	1	G60A GABLE	10 /12	6-06-00	6-00-03	2 x 4	1-03-08	1-07-11 4-11-11	36.44 24.17		
	1	T36 Flat	0 /12	3-10-08	2-04-13	2 x 4		2-04-13 2-04-13	16.05 10.00		
	1	T37 Half Hip	10 /12	3-10-08	3-08-13	2 x 4		3-01-09 3-08-13	22 15.83		
	1	T38 Half Hip	10 /12	3-10-08	5-00-13	2 x 4		3-01-09 5-00-13	28.27 19.00		
	1	T39 Jack-Open	10 /12	3-10-08	6-04-05	2 x 4		3-01-09 6-04-05	23.48 15.17		
	1 2-ply	T390 Jack-Closed Girder	10 /12	3-10-08	6-04-05	2 x 4 2 x 6		3-01-09 6-04-05	51.09 33.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B UNIT 1

Job Track: 50469
 PlanLog: 200774
 Layout ID: 404966
 Ref #
 Page: 3 of 3
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	J4 Jack-Open	10 / 12	4-01-09	5-01-00	2 x 4	1-03-08	1-07-11 5-01-00	79.02 55.00		
	2	J6 Jack-Open	8 / 12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	28.1 18.00		
	1 2-ply	J60 Jack-Closed Girder	8 / 12	3-10-08	3-11-13	2 x 4 2 x 6		1-04-13 3-11-13	40.7 26.00		
	1	J7 Jack-Open	8 / 12	1-10-15	2-08-02	2 x 4	1-03-08 1-11-09	1-04-13 2-08-02	10.12 6.67		
	1	J8 Jack-Open	8 / 12	1-10-15	2-08-02	2 x 4	1-03-08 1-01	1-04-13 2-08-02	7.98 5.33		

TOTAL # TRUSS= 53

TOTAL BFT OF ALL TRUSSES= 1869.5

BFT.

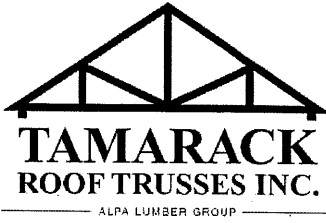
TOTAL WEIGHT OF ALL TRSSES 2916.15 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	
9	Hardware	LJS26DS	
3	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 18

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B- UNIT2

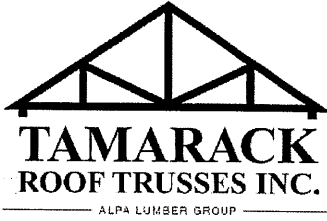
Job Track: 50469
 PlanLog: 200774
 Layout ID: 404967
 Ref #
 Page: 1 of 2
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T1 Roof Special	10 /12	27-10-00	9-09-03	2 x 4	1-03-08	10-14 1-02-00	588.29 368.33		
	1	G1 GABLE	10 /12	27-10-00	9-09-03	2 x 4	1-03-08	10-14 1-02-00	128.02 82.50		
	5	T1A Roof Special	10 /12	27-00-00	9-09-03	2 x 4		10-14 1-02-00	569.19 359.17		
	1	G1A GABLE	10 /12	27-00-00	9-09-03	2 x 4		10-14 1-02-00	126.91 82.67		
	6	T22 Monopitch	10 /12	10-01-08	10-00-15	2 x 4	1-03-08	1-07-11 10-00-15	344.78 219.00		
	1	T32 Half Hip Girder	8 /12	9-08-00	4-10-07	2 x 4	1-03-08	1-04-13 4-10-07	44.39 28.67		
	1	T33 Half Hip	8 /12	9-08-00	6-06-07	2 x 4	1-03-08	1-04-13 6-06-07	52.52 34.00		
	1	T28 Monopitch	8 /12	9-08-00	7-10-02	2 x 4	1-03-08	1-04-13 7-10-02	48.31 31.83		
	1 3-ply	T29 Monopitch Girder	8 /12	9-08-00	7-10-02	2 x 4 2 x 6		1-04-13 7-10-02	173.6 112.00		
	1	T40 Half Hip Girder	8 /12	10-06-00	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	45.9 29.67		
	1 3-ply	T41 Half Hip Girder	8 /12	10-06-00	4-06-08	2 x 4 2 x 6		1-04-13 4-06-08	160.56 102.50		
	5	J3 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	75.68 48.33		
	4	J6 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	56.2 36.00		
	1	J7 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 1-11-09	1-04-13 2-08-02	10.12 6.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B- UNIT2

Job Track: 50469
 PlanLog: 200774
 Layout ID: 404967
 Ref #
 Page: 2 of 2
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J8 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 1-01	1-04-13 2-08-02	7.98 5.33		

TOTAL # TRUSS= 39

TOTAL BFT OF ALL TRUSSES= 1546.67

BFT.

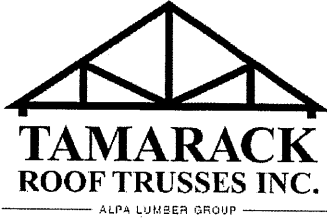
TOTAL WEIGHT OF ALL TRSSES 2432.44 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 10

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B2- UNIT3

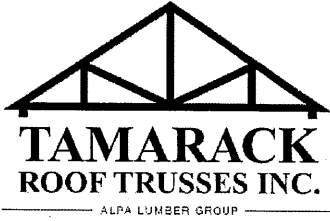
Job Track: 50469
 PlanLog: 200774
 Layout ID: 404968
 Ref #
 Page: 1 of 2
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T1 Roof Special	10 /12	27-10-00	9-09-03	2 x 4	1-03-08	10-14 1-02-00	588.29 368.33		
	1	G1 GABLE	10 /12	27-10-00	9-09-03	2 x 4	1-03-08	10-14 1-02-00	128.02 82.50		
	5	T1A Roof Special	10 /12	27-00-00	9-09-03	2 x 4		10-14 1-02-00	569.19 359.17		
	1	G1A GABLE	10 /12	27-00-00	9-09-03	2 x 4		10-14 1-02-00	126.91 82.67		
	3	T42 Common	8 /12	10-06-00	4-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	132.14 84.50		
	1	G42 GABLE	8 /12	10-06-00	4-10-13	2 x 4	1-03-08	1-04-13 1-03-02	45.66 30.17		
	2	T42X Common	8 /12	10-06-00	4-10-13	2 x 4	1-03-08	1-04-13 1-04-13	86.02 54.67		
	1 3-ply	T41 Half Hip Girder	8 /12	10-06-00	4-06-08	2 x 4 2 x 6		1-04-13 4-06-08	160.56 102.50		
	1	T32 Half Hip Girder	8 /12	9-08-00	4-10-07	2 x 4	1-03-08	1-04-13 4-10-07	44.39 28.67		
	1	T33 Half Hip	8 /12	9-08-00	6-06-07	2 x 4	1-03-08	1-04-13 6-06-07	52.52 34.00		
	1	T28 Monopitch	8 /12	9-08-00	7-10-02	2 x 4	1-03-08	1-04-13 7-10-02	48.31 31.83		
	1 3-ply	T29 Monopitch Girder	8 /12	9-08-00	7-10-02	2 x 4 2 x 6		1-04-13 7-10-02	173.6 112.00		
	6	T22 Monopitch	10 /12	10-01-08	10-00-15	2 x 4	1-03-08	1-07-11 10-00-15	344.78 219.00		
	1	G23 GABLE	10 /12	9-10-00	9-10-00	2 x 4	1-03-08	1-07-11 9-10-00	57.93 36.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B2- UNIT3

Job Track: 50469
 PlanLog: 200774
 Layout ID: 404968
 Ref #
 Page: 2 of 2
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	J3 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	75.68 48.33		

TOTAL # TRUSS= 39

TOTAL BFT OF ALL TRUSSES= 1675.01 BFT.

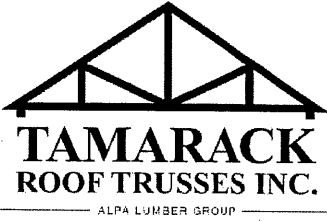
TOTAL WEIGHT OF ALL TRSSES 2633.99 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 10

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B2- UNIT4

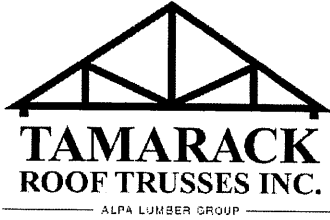
Job Track: 50469
 PlanLog: 200774
 Layout ID: 404969
 Ref #
 Page: 1 of 2
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T1 Roof Special	10 /12	27-10-00	9-09-03	2 x 4	1-03-08	10-14 1-02-00	588.29 368.33		
	1	G1 GABLE	10 /12	27-10-00	9-09-03	2 x 4	1-03-08	10-14 1-02-00	128.02 82.50		
	1	G1A GABLE	10 /12	27-00-00	9-09-03	2 x 4		10-14 1-02-00	126.91 82.67		
	5	T1A Roof Special	10 /12	27-00-00	9-09-03	2 x 4		10-14 1-02-00	569.19 359.17		
	1	T40 Half Hip Girder	8 /12	10-06-00	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	45.9 29.67		
	1 3-ply	T41 Half Hip Girder	8 /12	10-06-00	4-06-08	2 x 4 2 x 6		1-04-13 4-06-08	160.56 102.50		
	1	T26 Half Hip Girder	8 /12	9-08-00	5-01-00	2 x 4	1-03-08	1-04-13 5-01-00	46.78 30.83		
	1	T27 Half Hip	8 /12	9-08-00	6-09-00	2 x 4	1-03-08	1-04-13 6-09-00	53.44 35.67		
	1	T28 Monopitch	8 /12	9-08-00	7-10-02	2 x 4	1-03-08	1-04-13 7-10-02	48.31 31.83		
	1 3-ply	T29 Monopitch Girder	8 /12	9-08-00	7-10-02	2 x 4 2 x 6		1-04-13 7-10-02	173.6 112.00		
	6	T22 Monopitch	10 /12	10-01-08	10-00-15	2 x 4	1-03-08	1-07-11 10-00-15	344.78 219.00		
	1	G23 GABLE	10 /12	9-10-00	9-10-00	2 x 4	1-03-08	1-07-11 9-10-00	57.93 36.67		
	5	J4 Jack-Open	10 /12	4-01-09	5-01-00	2 x 4	1-03-08	1-07-11 5-01-00	79.02 55.00		
	4	J6 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	56.2 36.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B2- UNIT4

Job Track: 50469
 PlanLog: 200774
 Layout ID: 404969
 Ref #
 Page: 2 of 2
 Date: 03-27-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J7 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 1-11-09	1-04-13 2-08-02	10.12 6.67		
	1	J8 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 1-01	1-04-13 2-08-02	7.98 5.33		

TOTAL # TRUSS= 40

TOTAL BFT OF ALL TRUSSES= 1593.84

BFT.

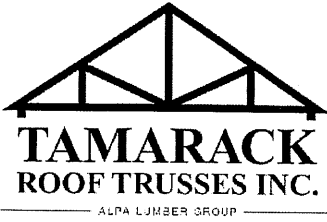
TOTAL WEIGHT OF ALL TRSSES 2497.01 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 10

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B-UNIT5

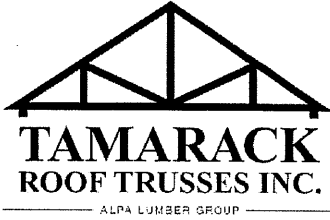
Job Track: 50469
 PlanLog: 200774
 Layout ID: 404977
 Ref #
 Page: 1 of 3
 Date: 03-28-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T10 Half Hip Girder	8 /12	19-10-00	6-04-05	2 x 4 2 x 6	1-03-08	1-04-13 6-04-05	202.29 128.00		
	1	T11 Half Hip	8 /12	19-10-00	8-00-05	2 x 4	1-03-08	1-04-13 8-00-05	98.8 62.17		
	1	T12 Hip	8 /12	19-10-00	9-08-05	2 x 4	1-03-08	1-04-13 8-04-13	114.41 72.83		
	1	T13A Hip	8 /12	19-10-00	11-04-05	2 x 4	1-03-08	1-04-13 8-04-13	104.64 64.83		
	1 2-ply	T43 Half Hip Girder	8 /12	19-10-00	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 5-03-13	204.63 129.33		
	1	T44 Half Hip	8 /12	19-10-00	6-07-13	2 x 4	1-03-08	1-04-13 6-07-13	98.37 64.50		
	5	T45 Half Hip	8 /12	19-10-00	7-11-13	2 x 4	1-03-08	1-04-13 7-11-13	492.86 310.83		
	1	T46 Hip	8 /12	19-10-00	9-03-13	2 x 4	1-03-08	1-04-13 8-11-08	109.35 69.33		
	3	T47 Scissor	8 /12 4 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	169.98 113.00		
	1	T17 Common	8 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	58.63 37.00		
	1	T18 Common Girder	8 /12	10-06-00	5-01-00	2 x 4	1-03-08	1-04-13 1-09-02	46.86 30.50		
	1	T19 Half Hip	8 /12	10-06-00	5-04-05	2 x 4	1-03-08	1-04-13 5-04-05	50.11 31.83		
	1	T20 Half Hip	8 /12	10-06-00	7-00-05	2 x 4	1-03-08	1-04-13 7-00-05	56.74 36.33		
	2	T21 Monopitch	8 /12	10-06-00	8-04-13	2 x 4	1-03-08	1-04-13 8-04-13	103.99 65.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B-UNIT5

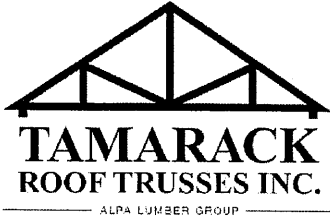
Job Track: 50469
 PlanLog: 200774
 Layout ID: 404977
 Ref #
 Page: 2 of 3
 Date: 03-28-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T21A Jack-Closed	8 /12	10-01-00	8-04-13	2 x 4		1-04-13 8-04-13	98.44 63.67		
	1	G14 GABLE	10 /12	12-06-00	10-03-15	2 x 4	1-03-08	1-07-11 10-03-15	78.49 50.00		
	1	T15 Half Hip	10 /12	12-09-08	11-08-05	2 x 6 2 x 4	1-03-08	1-07-11 12-03-10	88.01 54.00		
	4	T16 Monopitch	10 /12	12-09-08	12-03-10	2 x 4	1-03-08	1-07-11 12-03-10	310.71 193.33		
	1	T36 Flat	0 /12	3-10-08	2-04-13	2 x 4		2-04-13 2-04-13	16.05 10.00		
	1	T37 Half Hip	10 /12	3-10-08	3-08-13	2 x 4		3-01-09 3-08-13	22 15.83		
	1	T38 Half Hip	10 /12	3-10-08	5-00-13	2 x 4		3-01-09 5-00-13	28.27 19.00		
	1	T39 Jack-Open	10 /12	3-10-08	6-04-05	2 x 4		3-01-09 6-04-05	23.48 15.17		
	1 2-ply	T390 Jack-Closed Girder	10 /12	3-10-08	6-04-05	2 x 4 2 x 6		3-01-09 6-04-05	51.09 33.00		
	1	P1 Piggyback	8 /12	9-11-08	2-08-00	2 x 4		11-11	32.01 21.50		
	2	P2 Piggyback	8 /12	9-11-08	3-09-11	2 x 4		11-11	60.92 40.00		
	1	P2C Piggyback	8 /12	9-11-08	3-07-13	2 x 4		11-11	30.27 20.00		
	5	J4 Jack-Open	10 /12	4-01-09	5-01-00	2 x 4	1-03-08	1-07-11 5-01-00	79.02 55.00		
	3	J5 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	57.22 37.00		

DELIVERY SHIPLIST

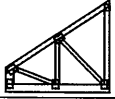


Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 1
 Lot #:
 Elevation: EL:B-UNIT5

Job Track: 50469
 PlanLog: 200774
 Layout ID: 404977
 Ref #
 Page: 3 of 3
 Date: 03-28-2023
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	J500 Jack-Closed Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	65.45 41.33		

TOTAL # TRUSS= 51

TOTAL BFT OF ALL TRUSSES= 1884.31

BFT.

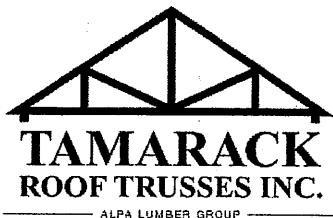
TOTAL WEIGHT OF ALL TRSSES 2953.09 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
6	Hardware	LUS24	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 10

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: DG- 3
 Lot #:
 Elevation: EL:B UNIT1

Job Track: 50469
 PlanLog: 200774
 Layout ID: 404995
 Ref #
 Page: 1 of 1
 Date: 08/13/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T96 Hip Girder	8/12	10-06-00	3-11-00	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	46.56 30.50		
	7	T97 Common	8/12	10-06-00	4-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	308.32 197.17		
	1	G97 GABLE	8/12	10-06-00	4-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	45.66 30.17		
	1 2-ply	T98 Common Girder	6/12	8-08-00	3-04-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	69.73 44.33		
	1 2-ply	G98 GABLE	6/12	8-08-00	3-04-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	69.17 46.33		
	5	J57 Jack-Open	6/12	5-06-00	3-11-00	2 x 4	1-03-08	1-02-00 3-11-00	79.44 50.00		

TOTAL # TRUSS= 18

TOTAL BFT OF ALL TRUSSES= 398.5

BFT.

TOTAL WEIGHT OF ALL TRSSES 618.89 LBS

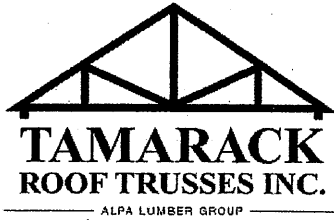
HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	

TOTAL NUMBER OF
ITEMS=

4

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: DG-1
 Lot #:
 Elevation: EL:B UNIT3 OR EL:B UNIT2

Job Track: 50469
 PlanLog: 200893
 Layout ID: 404952
 Ref #
 Page: 1 of 1
 Date: 08/20/2019
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T90 Hip Girder	6 /12	20-04-00	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	20-04-00	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	20-04-00	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	6-08-00	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	5-09-07	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	1-10-08	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

TOTAL # TRUSS= 20

TOTAL BFT OF ALL TRUSSES= 309.67

BFT.

TOTAL WEIGHT OF ALL TRSSES 487.86 LBS

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: DG-1
 Lot #:
 Elevation: EL:B2 UNIT5 OR EL:B UNIT4

Job Track: 50469
 PlanLog: 200893
 Layout ID: 404952
 Ref #
 Page: 1 of 1
 Date: 08/20/2019
 Designer:
 Sales Rep: Mario DiCano



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T90 Hip Girder	6 /12	20-04-00	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	20-04-00	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	20-04-00	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	6-08-00	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	5-09-07	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	1-10-08	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

TOTAL # TRUSS= 20

TOTAL BFT OF ALL TRUSSES= 309.67

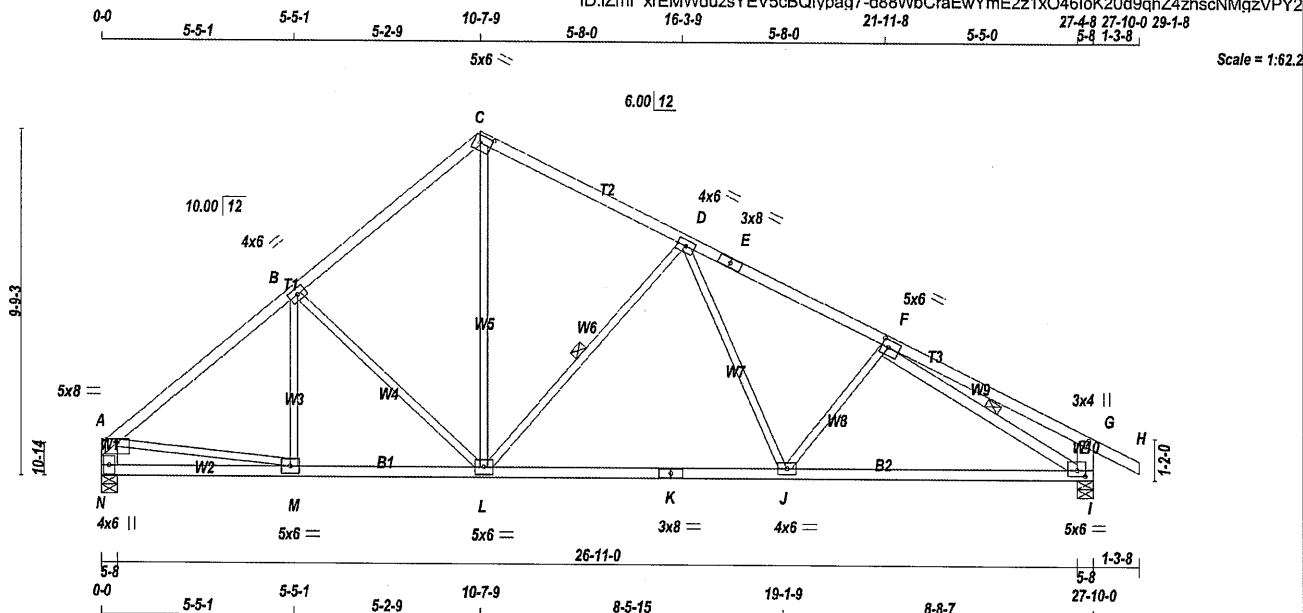
BFT.

TOTAL WEIGHT OF ALL TRSSES 487.86 LBS

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T1	15	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:23 2023 Page 1
ID: IZml_xrEMWduzysYEY5cRQ5ynagZ_d88WbCraEwYmEz71yC46lqK20d9ghZ4zbscaNMAzjDy2



TOTAL WEIGHT = $15 \times 118 = 1777 \text{ lb}$

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT F - I	2x3	DRY	No.2	SPF
	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

J	T	TYPE	PLATES	W	LEN	Y	X
A	B	TMVW-p	MT20	5.0	8.0	Edge	
C	B	TMVW-t	MT20	4.0	6.0		
D	C	TTW-I	MT20	5.0	6.0	2.50	3.75
E	D	TMVW-t	MT20	4.0	6.0		
F	E	TS-I	MT20	3.0	8.0		
G	F	TMVW-t	MT20	5.0	6.0	2.50	2.25
H	G	TMV+p	MT20	3.0	4.0	2.00	1.25
I	H	BMVW1-t	MT20	5.0	6.0	2.00	2.75
J	I	BMVW-t	MT20	4.0	6.0		
K	J	BS-t	MT20	3.0	8.0		
L	K	BMVWVW-t	MT20	5.0	6.0		
M	L	BMVW-t	MT20	5.0	6.0		
N	M	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	2062	0	2062	0	0	5-8	5-8
I	2238	0	2238	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1437	1064 / 0	0 / 0	0 / 0	0 / 0	373 / 0	0 / 0
I	1557	1167 / 0	0 / 0	0 / 0	0 / 0	389 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N. 1

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-L, F-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. LC (UNBRAC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	-2369 / 0	-129.6	-129.6	0.60 (1)	3.85	M-B	-101 / 65	0.04 (1)	
B-C	-1927 / 0	-129.6	-129.6	0.54 (1)	4.25	B-L	-585 / 0	0.62 (1)	
C-D	-1654 / 0	-129.6	-129.6	0.54 (1)	4.50	L-C	0 / 1369	0.31 (1)	
D-E	-2654 / 0	-129.6	-129.6	0.58 (1)	3.69	L-D	-1100 / 0	0.57 (1)	
E-F	-2654 / 0	-129.6	-129.6	0.58 (1)	3.69	D-J	0 / 523	0.12 (1)	
F-G	0 / 31	-129.6	-129.6	0.55 (1)	10.00	J-F	-382 / 0	0.14 (1)	
G-H	0 / 39	-129.6	-129.6	0.17 (1)	10.00	A-M	0 / 1870	0.42 (1)	
N-A	-2017 / 0	0.0	0.0	0.13 (1)	7.12	F-I	-3109 / 0	0.68 (1)	
I-G	-480 / 0	0.0	0.0	0.03 (1)	7.81				
N-M	0 / 0	-18.5	-18.5	0.11 (4)	10.00				
M-L	0 / 1859	-18.5	-18.5	0.40 (1)	10.00				
L-K	0 / 2170	-18.5	-18.5	0.52 (1)	10.00				
K-J	0 / 2170	-18.5	-18.5	0.52 (1)	10.00				
J-I	0 / 2608	-18.5	-18.5	0.59 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.60/1.00 (A-B:1) , BC=0.59/1.00 (I-J:1) ,
WB=0.68/1.00 (F-I:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.73 (K) (INPUT = 1.00)

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040019

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (J) (INPUT = 0.90)

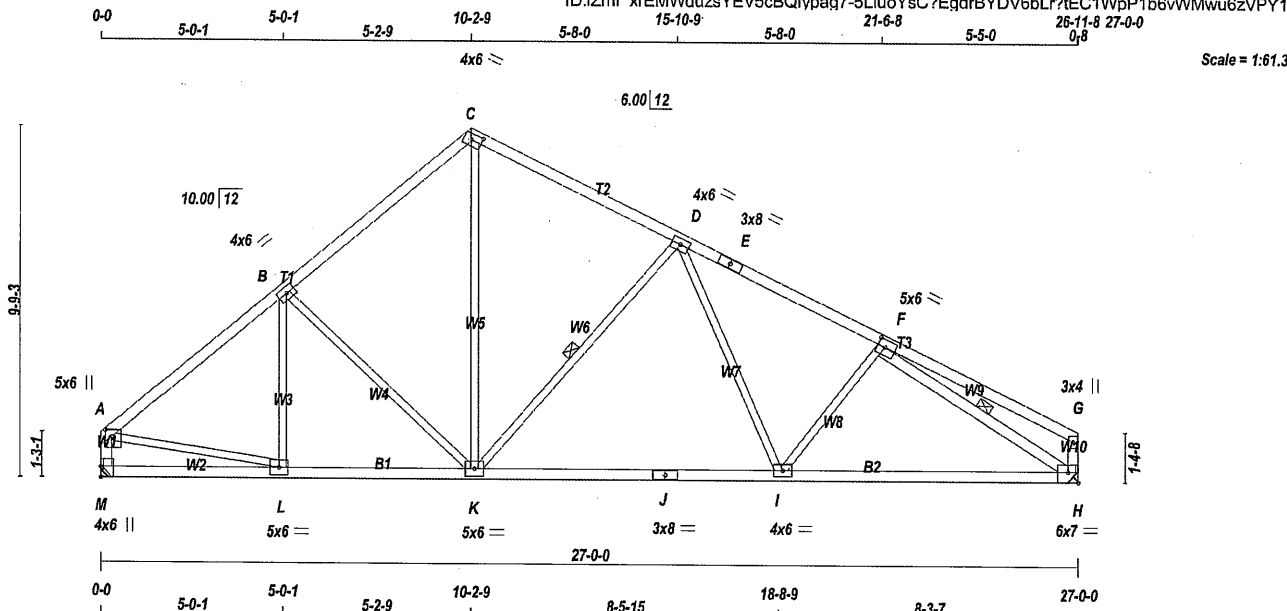
JSI METAL= 0.14 (C) (INPUT = 1.00)

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T1A	15	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 15 X 114 = 1706 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
M - A	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.25	2.00
B	TMVW-t	MT20	4.0	6.0		
C	TTW-t	MT20	4.0	6.0	2.00	3.50
D	TMVW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.50
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	6.0	7.0	3.50	Edge
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMV1-t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
M	2000	0	2000	0
H	2000	0	2000	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M, H. MINIMUM BEARING LENGTH AT JOINT M = 3-8, JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1394	1032 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0
H	1394	1032 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-K, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-2143 / 0	-129.6 -129.6	0.51 (1)	L-B	-220 / 29	0.10 (1)	
B-C	-1808 / 0	-129.6 -129.6	0.50 (1)	B-K	-467 / 0	0.50 (1)	
C-D	-1555 / 0	-129.6 -129.6	0.54 (1)	K-C	0 / 1246	0.28 (1)	
D-E	-2462 / 0	-129.6 -129.6	0.57 (1)	K-D	-1042 / 0	0.54 (1)	
E-F	-2462 / 0	-129.6 -129.6	0.57 (1)	D-I	0 / 421	0.09 (1)	
F-G	0 / 30	-129.6 -129.6	0.49 (1)	I-F	-269 / 15	0.10 (1)	
M-A	-1958 / 0	0.0 0.0	0.20 (1)	A-L	0 / 1714	0.39 (1)	
H-G	-280 / 0	0.0 0.0	0.03 (1)	F-H	-2894 / 0	0.60 (1)	
M-L	0 / 0	-18.5 -18.5	0.10 (4)				
L-K	0 / 1686	-18.5 -18.5	0.38 (1)				
K-J	0 / 2042	-18.5 -18.5	0.49 (1)				
J-I	0 / 2042	-18.5 -18.5	0.49 (1)				
I-H	0 / 2372	-18.5 -18.5	0.55 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.22")

CSI: TC=0.57/1.00 (D-F:1), BC=0.55/1.00 (H-I:1), WB=0.60/1.00 (F-H:1), SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1967 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)

JSI METAL= 0.71 (J) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040020



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	G1A	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

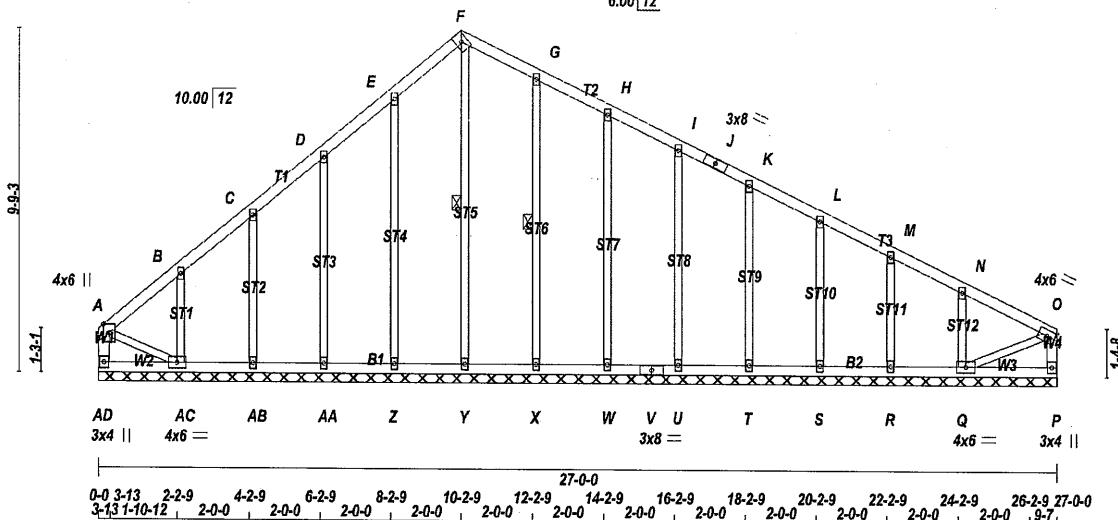
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0-0 3-13 2-2-9 4-2-9 6-2-9 8-2-9 10-2-9 12-2-9 14-2-9 16-2-9 18-2-9 20-2-9 22-2-9 24-2-9 26-2-9 27-0-0
3-13 1-10-12 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 8-15 0-8

4x6 //

6.00 | 12

Scale = 1:62.6



TOTAL WEIGHT = 3 X 127 = 381 lb

LUMBER				LUMBER			
N. L. G. A. RULES	CHORDS	SIZE		DESCR.			
A - F	2x4	DRY	No.2	SPF			
F - J	2x4	DRY	No.2	SPF			
J - O	2x4	DRY	No.2	SPF			
AD - A	2x4	DRY	No.2	SPF			
P - O	2x4	DRY	No.2	SPF			
AD - V	2x4	DRY	No.2	SPF			
V - P	2x4	DRY	No.2	SPF			
ALL WEBS				2x3	DRY	No.2	SPF
ALL GABLE WEBS				2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.							

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B, C, D, E, G, H, I, K, L, M, N						
B	TMVW+w	MT20	2.0	4.0		
F	TTW+h	MT20	4.0	6.0		
J	TS-l	MT20	3.0	8.0		
O	TMVW-t	MT20	4.0	6.0	Edge	
P	BMV1+p	MT20	3.0	4.0		
Q	BMVW1-t	MT20	4.0	6.0		
R, S, T, U, W, X, Y, Z, AA, AB						
R	BMV1+w	MT20	2.0	4.0		
V	BS-l	MT20	3.0	8.0		
AC	BMVW1-t	MT20	4.0	6.0		
AD	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Y, G-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-47 / 0	-129.6 -129.6	0.09 (1)	6.25	Y-F	-183 / 0	0.12 (1)
B-C	-59 / 0	-129.6 -129.6	0.09 (1)	6.25	Z-E	-285 / 0	0.37 (1)
C-D	-43 / 0	-129.6 -129.6	0.06 (1)	6.25	AA-D	-254 / 0	0.18 (1)
D-E	-37 / 0	-129.6 -129.6	0.07 (1)	6.25	AB-C	-242 / 0	0.08 (1)
E-F	-52 / 0	-129.6 -129.6	0.07 (1)	6.25	AC-B	-305 / 0	0.06 (1)
F-G	-39 / 0	-129.6 -129.6	0.08 (1)	6.25	X-G	-302 / 0	0.15 (1)
G-H	-27 / 0	-129.6 -129.6	0.08 (1)	6.25	W-H	-250 / 0	0.28 (1)
H-I	-33 / 0	-129.6 -129.6	0.06 (1)	6.25	U-I	-259 / 0	0.20 (1)
I-J	-36 / 0	-129.6 -129.6	0.06 (1)	6.25	T-K	-256 / 0	0.13 (1)
J-K	-36 / 0	-129.6 -129.6	0.06 (1)	6.25	S-L	-264 / 0	0.09 (1)
K-L	-40 / 0	-129.6 -129.6	0.06 (1)	6.25	R-M	-228 / 0	0.05 (1)
L-M	-42 / 0	-129.6 -129.6	0.06 (1)	6.25	Q-N	-349 / 0	0.06 (1)
M-N	-59 / 0	-129.6 -129.6	0.11 (1)	6.25	A-AC	0 / 53	0.01 (1)
N-O	-46 / 0	-129.6 -129.6	0.11 (1)	6.25	Q-O	0 / 57	0.01 (1)
AD-A	-177 / 0	0.0 0.0	0.02 (1)	7.81			
P-O	-186 / 0	0.0 0.0	0.02 (1)	7.81			

AD-AC	0 / 0	-18.5 -18.5	0.02 (4)	10.00
AC-AB	0 / 39	-18.5 -18.5	0.02 (4)	10.00
AB-AA	0 / 34	-18.5 -18.5	0.02 (4)	10.00
AA-Z	0 / 30	-18.5 -18.5	0.02 (4)	10.00
Z-Y	0 / 27	-18.5 -18.5	0.02 (4)	10.00
Y-X	0 / 25	-18.5 -18.5	0.02 (4)	10.00
X-W	0 / 27	-18.5 -18.5	0.02 (4)	10.00
W-V	0 / 30	-18.5 -18.5	0.02 (4)	10.00
V-U	0 / 30	-18.5 -18.5	0.02 (4)	10.00
U-T	0 / 32	-18.5 -18.5	0.02 (4)	10.00
T-S	0 / 35	-18.5 -18.5	0.02 (4)	10.00
S-R	0 / 39	-18.5 -18.5	0.02 (4)	10.00
R-Q	0 / 44	-18.5 -18.5	0.03 (4)	10.00
Q-P	0 / 0	-18.5 -18.5	0.03 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 38.2	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (N-O:1), BC=0.03/1.00 (Q-R:4), WB=0.37/1.00 (E-Z:1), SSI=0.14/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (A) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

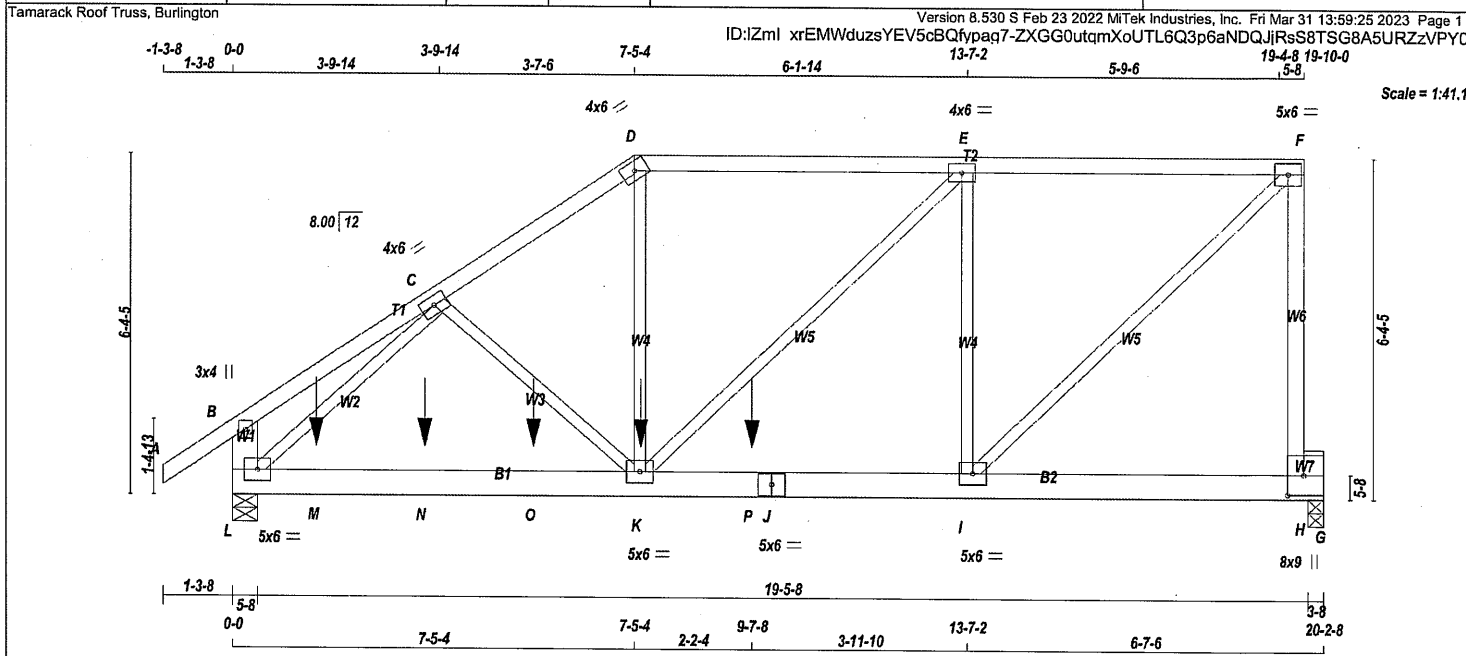
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040002



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T10	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:25 2023 Page 1			



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF H - F 2x4 DRY No.2 SPF L - B 2x6 DRY No.2 SPF L - J 2x6 DRY No.2 SPF J - G 2x6 DRY No.2 SPF H - G 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A - D 1 12 TOP D - F 1 12 TOP F - H 1 12 TOP L - B 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS L - J 2 12 SIDE(183.1) J - G 2 12 TOP WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 2x6 2 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG DOWN HORZ UPLIFT IN-SX IN-SX JT 2146 0 3094 0 5-8 5-8 L 2226 0 2226 0 3-8 3-8 G UNFACTORED REACTIONS 1ST LCASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL L 2146 1650 / 0 0 / 0 0 / 0 496 / 0 0 / 0 G 1548 1163 / 0 0 / 0 0 / 0 385 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS FACTORED WEBS MEMB. MAX. FORCE VERT. LOAD LC1 MAX. MAX. UNBRAC. MEMB. MAX. FORCE MAX. (LBS) (PLF) CSI (LC) LENGTH (LBS) CSI (LC) FR-TO FROM TO A-B 0 / 49 -129.6 -129.6 0.10 (1) 10.00 C-K 0 / 75 0.01 (4) B-C 0 / 27 -129.6 -129.6 0.14 (1) 10.00 K-D 0 / 1228 0.15 (1) C-D -3131 / 0 -129.6 -129.6 0.18 (1) 5.07 L-C -3453 / 0 0.69 (1) D-E -2599 / 0 -129.6 -129.6 0.53 (1) 4.93 I-F 0 / 2985 0.37 (1) E-F -2154 / 0 -129.6 -129.6 0.51 (1) 5.32 K-E 0 / 626 0.08 (1) H-F -2374 / 0 0.0 0.0 0.90 (1) 7.29 I-E -1430 / 0 0.44 (1) L-B -369 / 0 0.0 0.0 0.01 (1) 7.81 L-M 0 / 2522 -18.5 -18.5 0.52 (1) 10.00 M-N 0 / 2522 -18.5 -18.5 0.52 (1) 10.00 N-O 0 / 2522 -18.5 -18.5 0.52 (1) 10.00 O-K 0 / 2522 -18.5 -18.5 0.52 (1) 10.00 K-P 0 / 2154 -18.5 -18.5 0.50 (1) 10.00 P-J 0 / 2154 -18.5 -18.5 0.50 (1) 10.00 J-I 0 / 2154 -18.5 -18.5 0.50 (1) 10.00 I-H 0 / 0 -18.5 -18.5 0.31 (1) 10.00 H-G 0 / 0 -18.5 -18.5 0.13 (1) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN. K 7-6-12 -185 -185 - FRONT VERT TOTAL C1 M 1-6-12 -185 -185 - FRONT VERT TOTAL C1 N 3-6-12 -185 -185 - FRONT VERT TOTAL C1 O 5-6-12 -185 -185 - FRONT VERT TOTAL C1 P 9-7-8 -778 -778 - FRONT VERT TOTAL C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.2 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 51.6 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.67") CALCULATED VERT. DEFL.(LL) = L/999 (0.07") ALLOWABLE DEFL.(TL) = L/360 (0.67") CALCULATED VERT. DEFL.(TL) = L/999 (0.12") CSI: TC=0.90/1.00 (F-H:1), BC=0.52/1.00 (K-L:1), WB=0.69/1.00 (C-L:1), SSI=0.37/1.00 (G-H:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (C) (INPUT = 0.90) JSI METAL= 0.52 (J) (INPUT = 1.00) LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2 CONTINUED ON PAGE 2			
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STRUCTURAL COMPONENT ONLY
DWG # TR23040021



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T10	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

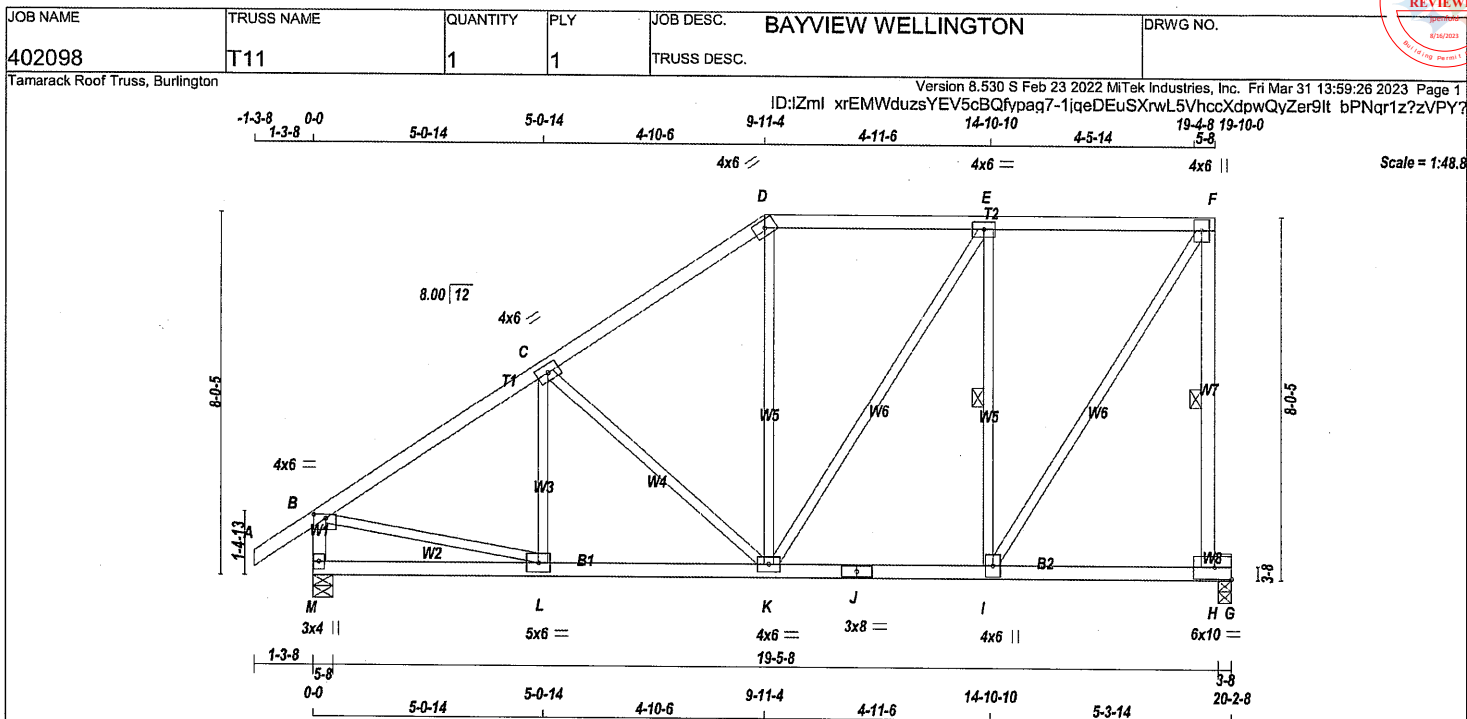
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW+p	MT20	8.0	9.0	4.50	3.50
I	BMVW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	5.0	6.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040021



TOTAL WEIGHT = 99 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	4.0	6.0	1.00	3.25
C TMVW-t	MT20	4.0	6.0		
D TTW-h	MT20	4.0	6.0		
E TMVW-t	MT20	4.0	6.0		
F TMVW+p	MT20	4.0	6.0		
H BMVW-t	MT20	6.0	10.0	3.00	Edge
I BMVW+t	MT20	4.0	6.0		
J BS-t	MT20	3.0	8.0		
K BMVW-t	MT20	4.0	6.0		
L BMVW-t	MT20	5.0	6.0		
M BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
M	1674	0	1674	0	5-8	5-8
G	1449	0	1449	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED UNBRACED LENGTH (FR-TO)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 49	-129.6 -129.6	0.17 (1)	10.00	L-C	-179 / 47	0.06 (1)
B-C	-1605 / 0	-129.6 -129.6	0.64 (1)	4.37	C-K	-536 / 0	0.44 (1)
C-D	-1214 / 0	-129.6 -129.6	0.59 (1)	4.93	K-D	0 / 172	0.04 (1)
D-E	-974 / 0	-129.6 -129.6	0.55 (1)	5.41	K-E	0 / 264	0.06 (1)
E-F	-835 / 0	-129.6 -129.6	0.55 (1)	5.74	I-E	-1017 / 0	0.41 (1)
H-F	-1541 / 0	0.0 0.0	0.45 (1)	5.30	I-F	0 / 1540	0.35 (1)
M-B	-1636 / 0	0.0 0.0	0.17 (1)	6.47	B-L	0 / 1401	0.32 (1)
M-L	0 / 0	-18.5 -18.5	0.11 (4)	10.00			
L-K	0 / 1371	-18.5 -18.5	0.28 (1)	10.00			
K-J	0 / 835	-18.5 -18.5	0.39 (1)	10.00			
J-I	0 / 835	-18.5 -18.5	0.39 (1)	10.00			
I-H	0 / 0	-18.5 -18.5	0.74 (1)	10.00			
H-G	0 / 0	-18.5 -18.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	38.2	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL)= L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL)= L/999 (0.24")

CSI: TC=0.64/1.00 (B-C:1), BC=0.74/1.00 (H-I:1), WB=0.44/1.00 (C-K:1), SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



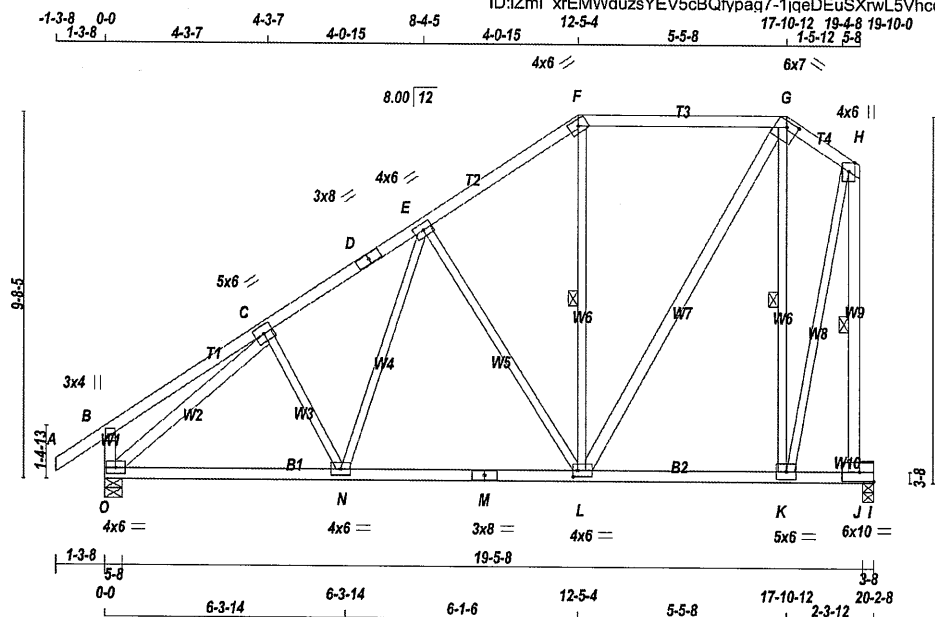
STRUCTURAL COMPONENT ONLY
DWG # TR23040022

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:58.3



TOTAL WEIGHT = 116 lb

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
J	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	5.0	6.0	
D	TS-t	MT20	3.0	8.0	
E	TMVW-t	MT20	4.0	6.0	
F	TTW-h	MT20	4.0	6.0	
G	TTWV-h	MT20	6.0	7.0	1.75 3.50
H	TMVW+p	MT20	4.0	6.0	Edge
J	BMVW-t	MT20	6.0	10.0	3.00 Edge
K	BMVW-t	MT20	5.0	6.0	
L	BMVWVW-t	MT20	4.0	6.0	2.00 1.50
M	BS-t	MT20	3.0	8.0	
N	BMVW-t	MT20	4.0	6.0	
O	BMVW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
GROSS REACTION		GROSS REACTION				BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1674	0	1674	0	0	5-8	5-8
I	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
I	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O. I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-L, G-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)
FR-TO		FROM	TO	LENGTH	FR-TO		CSI (LC)
A-B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C-N	-204 / 7
B-C	0 / 28	-129.6	-129.6	0.29 (1)	10.00	N-E	0 / 326
C-D	-1475 / 0	-129.6	-129.6	0.35 (1)	4.98	E-L	-702 / 0
D-E	-1475 / 0	-129.6	-129.6	0.35 (1)	4.98	L-F	-55 / 59
E-F	-943 / 0	-129.6	-129.6	0.32 (1)	5.93	L-G	0 / 852
F-G	-758 / 0	-129.6	-129.6	0.69 (1)	5.5	K-G	-986 / 0
G-H	-418 / 0	-129.6	-129.6	0.08 (1)	6.25	O-C	-1811 / 0
O-B	-396 / 0	0.0	0.0	0.04 (1)	7.81	K-H	0 / 1415
J-H	-1731 / 0	0.0	0.0	0.57 (1)	5.07		
O-N	0 / 1328	-18.5	-18.5	0.31 (1)	10.00		
N-M	0 / 1131	-18.5	-18.5	0.27 (1)	10.00		
M-L	0 / 1131	-18.5	-18.5	0.27 (1)	10.00		
L-K	0 / 336	-18.5	-18.5	0.17 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.74 (1)	10.00		
J-I	0 / 0	-18.5	-18.5	0.31 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.69/1.00 (F-G:1), BC=0.74/1.00 (J-K:1),
WB=0.89/1.00 (E-L:1), SSI=0.57/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

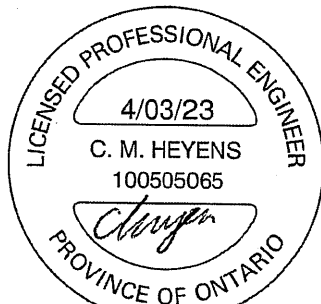
NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.43 (M) (INPUT = 1.00)

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

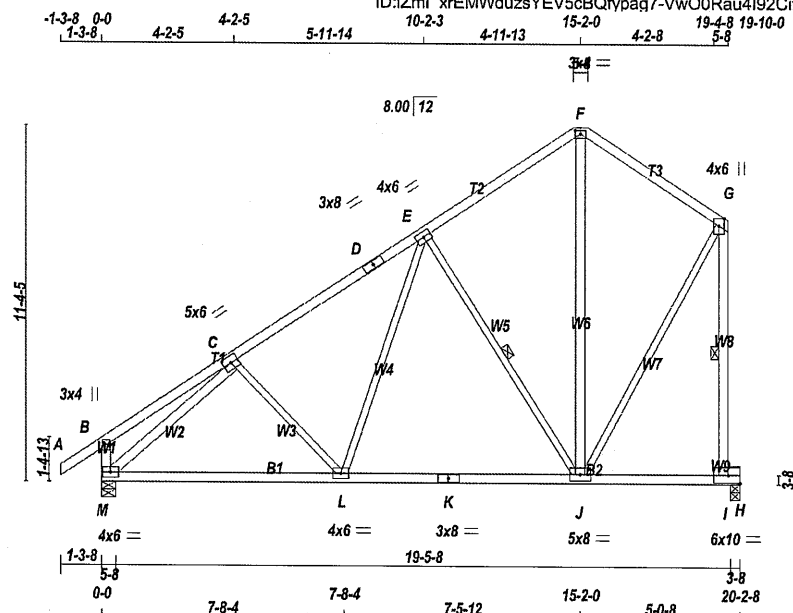


STRUCTURAL COMPONENT ONLY
DWG # TR23040023



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T13A	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:70.3

TOTAL WEIGHT = 106 lb

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
J - F	2x4	DRY	No.2	SPF	
M - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-p	MT20	3.0	4.0	1.25	2.00
G	TMWW+p	MT20	4.0	6.0	Edge	
I	BMVW-t	MT20	6.0	10.0	Edge	
J	BMVWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	8.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	IN-SX	BRG	IN-SX
M	1674	0	0	1674	0	0	5-8	5-8	
H	1449	0	0	1449	0	0	3-8	3-8	

UNFACTORED REACTIONS		1ST LOSE		MAX /MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
M	1164	877 / 0	0 / 0	0 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0 / 49	-129.6 -129.6	0.17 (1)	C-L	-313 / 0
B-C	0 / 45	-129.6 -129.6	0.46 (1)	L-E	0 / 385
C-D	-1351 / 0	-129.6 -129.6	0.70 (1)	E-J	-919 / 0
D-E	-1351 / 0	-129.6 -129.6	0.70 (1)	J-F	0 / 153
E-F	-672 / 0	-129.6 -129.6	0.60 (1)	M-C	-1838 / 0
F-G	-626 / 0	-129.6 -129.6	0.50 (1)	J-G	0 / 1041
M-B	-361 / 0	0.0 0.0	0.04 (1)		
I-G	-1551 / 0	0.0 0.0	0.52 (1)		
M-L	0 / 1345	-18.5 -18.5	0.35 (1)		
L-K	0 / 1012	-18.5 -18.5	0.40 (1)		
K-J	0 / 1012	-18.5 -18.5	0.40 (1)		
J-I	0 / 0	-18.5 -18.5	0.74 (1)		
I-H	0 / 0	-18.5 -18.5	0.31 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.70/1.00 (C-E:1), BC=0.74/1.00 (I-J:1), WB=0.67/1.00 (C-M:1), SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



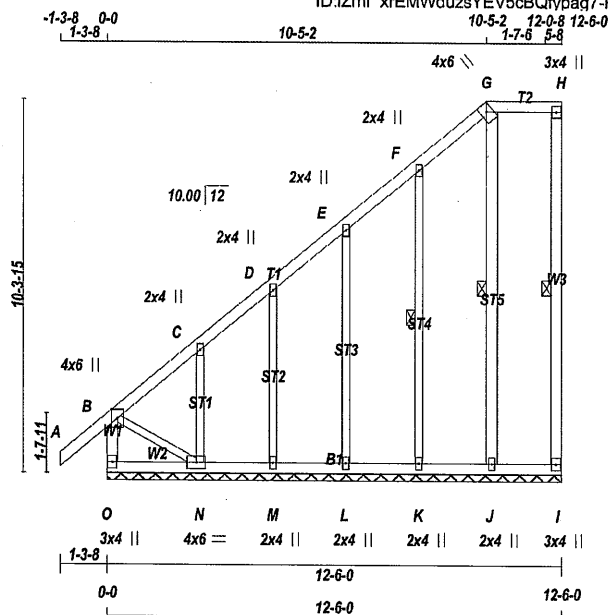
STRUCTURAL COMPONENT ONLY
DWG # TR23040024

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	G14	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:61.0

TOTAL WEIGHT = 78 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
O - B	2x4	DRY	No.2	SPF	
A - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
O - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
J - G	2x4	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
ST1	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2-0-0 OC.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F						
C	TMW+w	MT20	2.0	4.0		
G	TTW+h	MT20	4.0	6.0	Edge	
H	TMV+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, L, M						
N	BMW1+w	MT20	2.0	4.0		
J	BMWW1-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, G-J, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
O-B	-342 / 0	0.0 0.0	0.04 (1)	J-G	-233 / 0	0.12 (1)	
A-B	0 / 57	-129.6 -129.6	0.18 (1)	K-F	-280 / 0	0.14 (1)	
B-C	-12 / 0	-129.6 -129.6	0.10 (1)	L-E	-258 / 0	0.23 (1)	
C-D	-31 / 0	-129.6 -129.6	0.10 (1)	M-D	-233 / 0	0.10 (1)	
D-E	-11 / 0	-129.6 -129.6	0.06 (1)	N-C	-331 / 0	0.08 (1)	
E-F	-7 / 0	-129.6 -129.6	0.07 (1)	B-N	0 / 26	0.01 (1)	
F-G	-20 / 0	-129.6 -129.6	0.07 (1)				
G-H	0 / 0	-129.6 -129.6	0.09 (1)				
I-H	-135 / 0	0.0 0.0	0.07 (1)				
O-N	0 / 0	-18.5 -18.5	0.03 (4)				
N-M	0 / 14	-18.5 -18.5	0.03 (4)				
M-L	0 / 10	-18.5 -18.5	0.02 (4)				
L-K	0 / 7	-18.5 -18.5	0.02 (4)				
K-J	0 / 3	-18.5 -18.5	0.02 (4)				
J-I	0 / 0	-18.5 -18.5	0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1), BC=0.03/1.00 (M-N:4), WB=0.23/1.00 (E-L:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

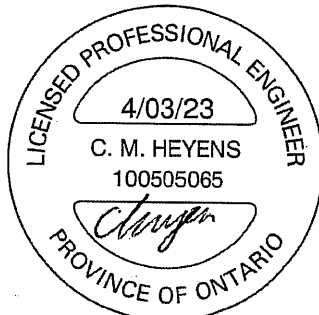
NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
MT20	650	371	1747	768	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (C) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

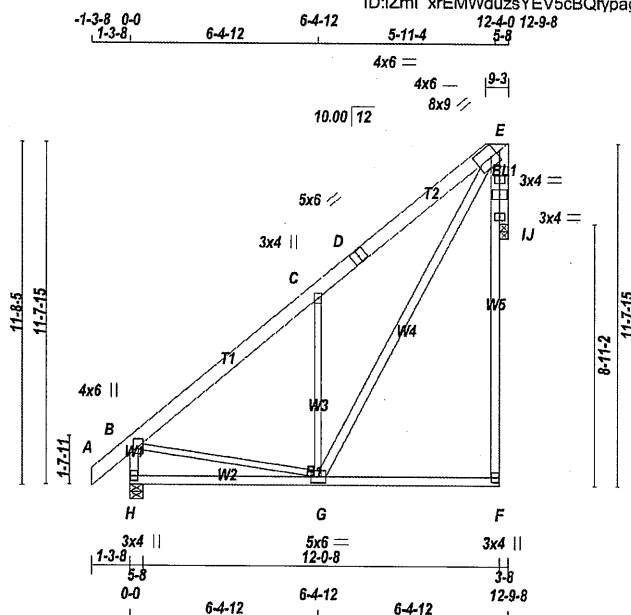


STRUCTURAL COMPONENT ONLY
DWG # TR23040003

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T15	2	1	TRUSS DESC. BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:75.2

TOTAL WEIGHT = 2 X 88 = 176 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

BT	TYPE	PLATES	W	LEN	Y	X
C	MTWV+p	MT20	4.0	6.0		
C	MTW-w	MT20	3.0	4.0		
C	TS-t	MT20	5.0	6.0		
F	MTWV1-t	MT20	8.0	9.0	4.50	2.75
F	KP-p	MT20	4.0	6.0	3.50	2.50
F	KP-p	MT20	4.0	6.0		
F	BMV+p	MT20	3.0	4.0		
G	BMVWV-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J(E)	915	0	915	0	0	3-8	3-8
H	1123	0	1123	0	0	5-8	5-8

(** SEE "BEARING NOTE" **)

UNFACTORED REACTIONS

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	638	472 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0
H	780	593 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 80	-129.6	-129.6	0.10 (1)	10.00	G-E	-969 / 0 0.74 (1)
B-C	-678 / 0	-129.6	-129.6	0.30 (1)	6.25	G-E	0 / 1131 0.18 (1)
C-D	-753 / 0	-129.6	-129.6	0.30 (1)	6.25	B-G	0 / 576 0.13 (1)
D-E	-753 / 0	-129.6	-129.6	0.30 (1)	6.25	E-J	-916 / 0 0.17 (1)
F-I	0 / 46	0.0	0.0	0.14 (1)	10.00	I-J	0 / 47 0.00 (1)
I-E	0 / 46	0.0	0.0	0.14 (1)	10.00		
H-B	-1075 / 0	0.0	0.0	0.11 (1)	7.61		
H-G	0 / 0	-18.5	-18.5	0.21 (4)	10.00		
G-F	0 / 11	-18.5	-18.5	0.21 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.30/1.00 (C-E:1), BC=0.21/1.00 (F-G:4)
 , WB=0.74/1.00 (C-G:1), SSI=0.23/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

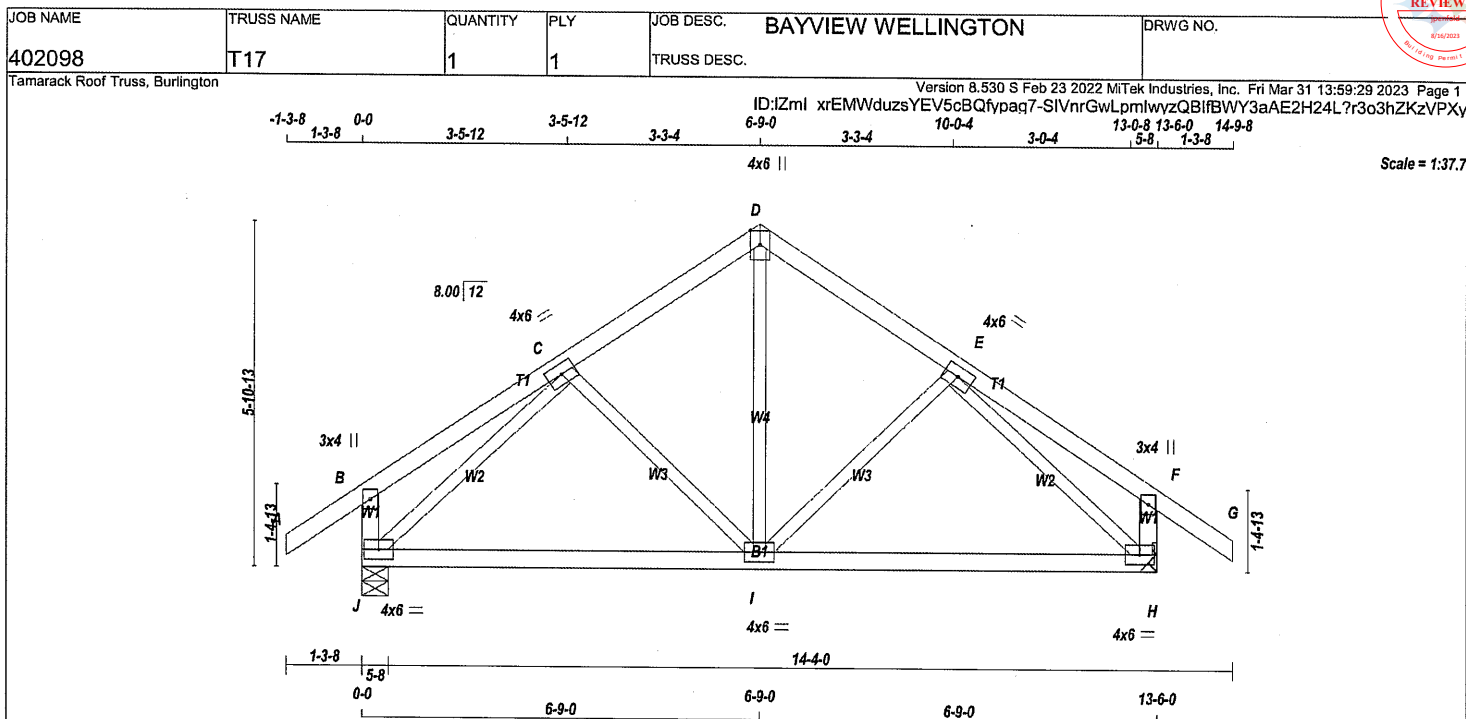
JSI GRIP= 0.83 (G) (INPUT = 0.90)

JSI METAL= 0.44 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR23040025

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	SPF	DRY	NO.2	SPF
A - D	2x4	DRY	No.2	SPF	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMV+p	MT20	3.0	4.0			
C	TMVW-t	MT20	4.0	6.0			
D	TTW+p	MT20	4.0	6.0	Edge		
E	TMVW-t	MT20	4.0	6.0			
F	TMV+p	MT20	3.0	4.0			
H	BMVW1-t	MT20	4.0	6.0			
I	BMVW1-t	MT20	4.0	6.0			
J	BMVW1-t	MT20	4.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS				FACTORED				MAXIMUM FACTORED				INPUT				REQD			
JT	GROSS	REACTION		GROSS	REACTION			GROSS	REACTION			BRG	BRG	IN-SX	IN-SX	BRG	BRG	IN-SX	IN-SX
JT	VERT	HORZ		DOWN	HORZ			DOWN	HORZ			5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8
H	1178	0		1178	0			1178	0			5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8
H	1178	0		1178	0			1178	0			MECHANICAL	MECHANICAL	MECHANICAL	MECHANICAL	MECHANICAL	MECHANICAL	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3'-8".

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
J	818	621 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0	
H	818	621 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)		FORCE (LBS)	MAX. FACTORED	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	I-D	0 / 488
B-C	0 / 27	-129.6	-129.6	0.23 (1)	10.00	I-E	-240 / 0
C-D	-775 / 0	-129.6	-129.6	0.18 (1)	6.25	C-I	-240 / 0
D-E	-775 / 0	-129.6	-129.6	0.18 (1)	6.25	J-C	-1114 / 0
E-F	0 / 27	-129.6	-129.6	0.23 (1)	10.00	E-H	-1114 / 0
F-G	0 / 49	-129.6	-129.6	0.17 (1)	10.00		
J-B	-347 / 0	0.0	0.0	0.04 (1)	7.81		
H-F	-347 / 0	0.0	0.0	0.04 (1)	7.81		
J-I	0 / 793	-18.5	-18.5	0.29 (4)	10.00		
I-H	0 / 793	-18.5	-18.5	0.29 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.23/1.00 (E-F:1), BC=0.29/1.00 (I-J:4), WB=0.40/1.00 (E-H:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (J) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



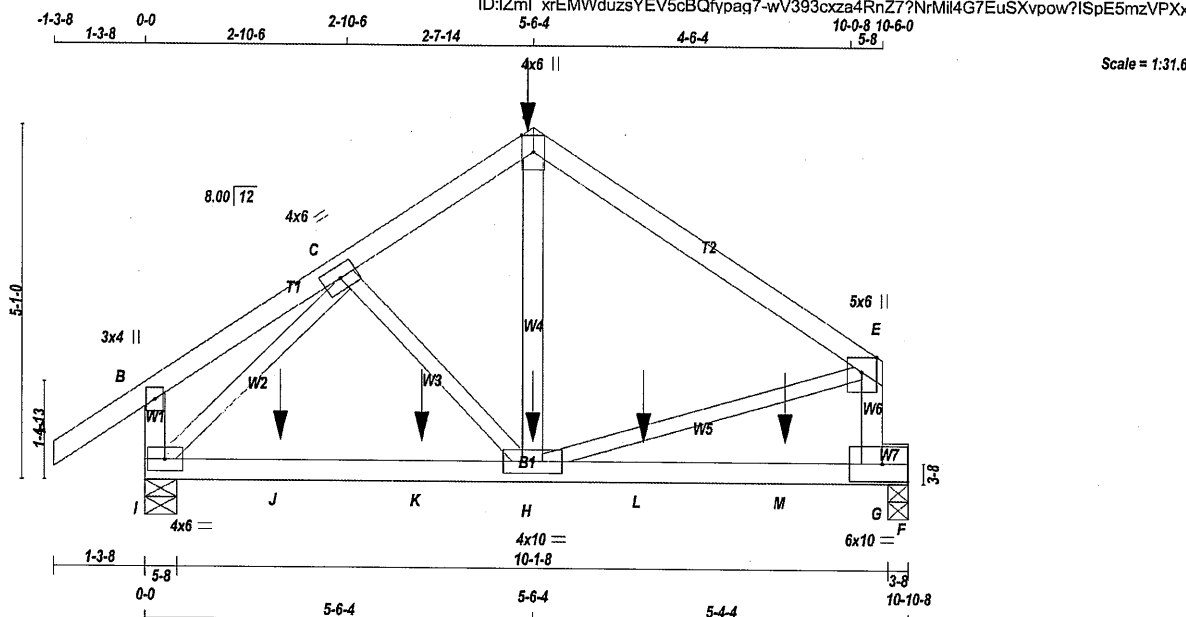
STRUCTURAL COMPONENT ONLY
DWG # TR23040027



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T18	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 47 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	

ALL WEBS	EXCEPT	H - D	2x4	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMVW-t	MT20	6.0	10.0	3.00	4.25
H	BMVW-t	MT20	4.0	10.0		
I	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
I	VERT	DOWN	IN-SX	IN-SX
F	1449	0	5-8	5-8
F	1238	0	3-8	3-8

UNFACTORED REACTIONS

UNFACTORED REACTIONS							
	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1004	777 / 0	0 / 0	0 / 0	0 / 0	227 / 0	0 / 0
F	860	652 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	C-H	-21 / 24
B-C	0 / 20	-129.6	-129.6	0.15 (1)	10.00	H-D	0 / 165
C-D	-1244 / 0	-129.6	-129.6	0.18 (1)	5.53	I-C	-1505 / 0
D-E	-1224 / 0	-129.6	-129.6	0.68 (1)	4.68	H-E	0 / 1060
I-B	-319 / 0	0.0	0.0	0.04 (1)	7.81		
G-E	-1296 / 0	0.0	0.0	0.15 (1)	7.05		
I-J	0 / 1033	-18.5	-18.5	0.47 (1)	10.00		
J-K	0 / 1033	-18.5	-18.5	0.47 (1)	10.00		
K-H	0 / 1033	-18.5	-18.5	0.47 (1)	10.00		
H-L	0 / 0	-18.5	-18.5	0.69 (1)	10.00		
L-M	0 / 0	-18.5	-18.5	0.69 (1)	10.00		
M-G	0 / 0	-18.5	-18.5	0.69 (1)	10.00		
G-F	0 / 0	-18.5	-18.5	0.29 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-6-4	-40	-40	---	FRONT	VERT	DEAD	---	C1
D	5-6-4	-94	-94	---	FRONT	VERT	TOTAL	---	C1
D	5-6-4	-440	-440	---	FRONT	VERT	SNOW	---	C1
H	5-6-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1
J	1-11-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1
K	3-11-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1
L	7-1-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1
M	9-1-4	-16	-16	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/989 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/659 (0.20")

CSI: TC=0.68/1.00 (D-E:1), BC=0.69/1.00 (G-H:1), WB=0.42/1.00 (C-I:1), SSI=0.53/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	MAX	MIN	MAX
	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.52 (E) (INPUT = 1.00)

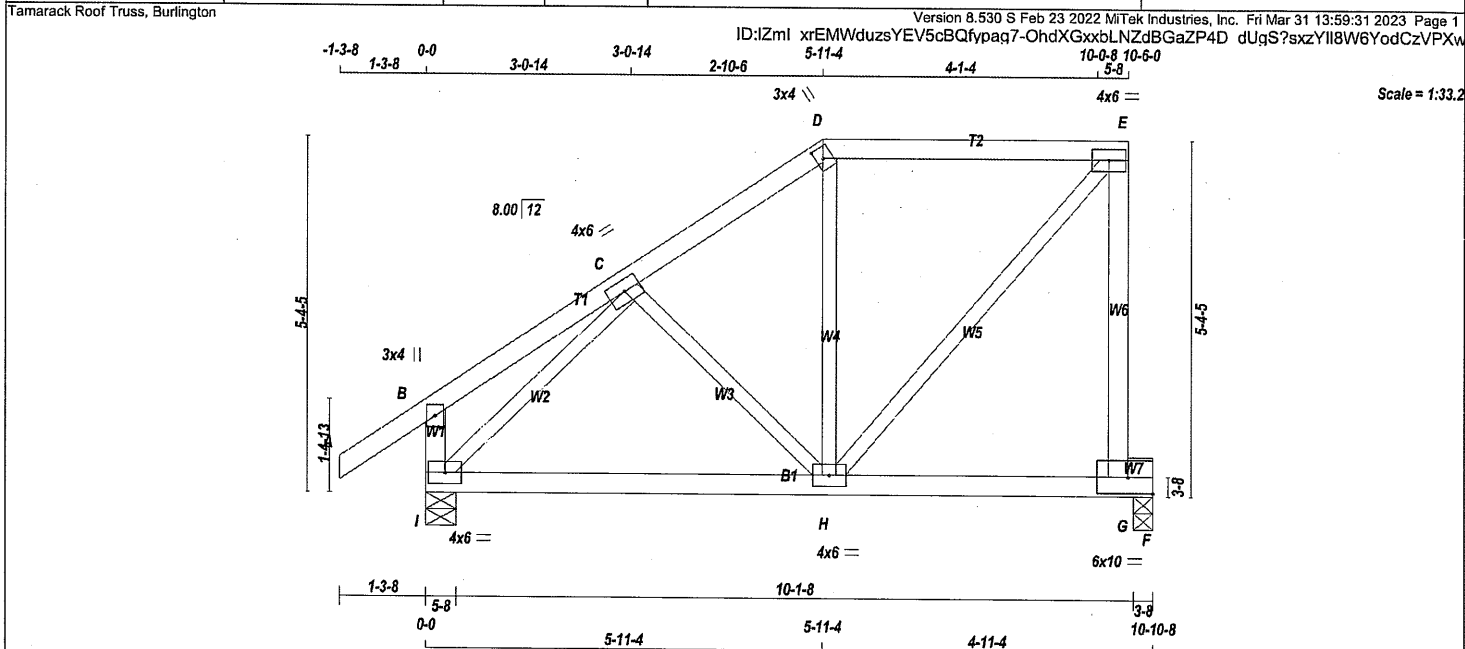
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040028



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T19	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:			
A - D	2x4	DRY	No.2	FACTORED				TOP CH.			
D - E	2x4	DRY	No.2	GROSS REACTION				LL = 38.2 PSF			
G - E	2x4	DRY	No.2	DOWN				DL = 6.0 PSF			
I - B	2x4	DRY	No.2	UP				LL = 0.0 PSF			
I - F	2x4	DRY	No.2	DOWN				DL = 7.4 PSF			
G - F	2x4	DRY	No.2	UP				TOTAL LOAD = 51.6 PSF			

ALL WEBS 2x3 DRY No.2 SPF EXCEPT				UNFACTORED REACTIONS				SPACING = 24.0 IN. C/C			
DRY: SEASONED LUMBER.				1ST LCASE				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
				MAX./MIN. COMPONENT REACTIONS				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
				COMBINED				THIS DESIGN COMPLIES WITH:			
				SNOW				- PART 9 OF BCBC 2018, ABC 2019			
				LIVE				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
				PERM.LIVE				- CSA 086-14			
				WIND				- TPIC 2014			
				DEAD				(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD			
				SOIL				ALLOWABLE DEFL.(LL)= L/360 (0.36")			
								CALCULATED VERT. DEFL.(LL)= L/999 (0.06")			
								ALLOWABLE DEFL.(TL)= L/360 (0.36")			
								CALCULATED VERT. DEFL.(TL)= L/999 (0.10")			
								CSI: TC=0.47/1.00 (D-E:1), BC=0.38/1.00 (G-H:1), WB=0.25/1.00 (C-I:1), SSI=0.30/1.00 (F-G:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
								COMPANION LIVE LOAD FACTOR = 1.00			
								AUTOSOLVE RIGHT HEEL ONLY			
								TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 650 371 1747 788 1987 1873			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.80 (H) (INPUT = 0.90)			
								JSI METAL= 0.20 (H) (INPUT = 1.00)			
								LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2			

PLATES (table is in inches)				BRACING			
JT	TYPE	PLATES	W LEN Y X	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.			
B	TMV+p	MT20	3.0 4.0	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.			
C	TMVW-t	MT20	4.0 6.0	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.			
D	TTW+h	MT20	3.0 4.0 2.00 1.25	LOADING			
E	TMVW-t	MT20	4.0 6.0	TOTAL LOAD CASES: (4)			
G	BMVW-t	MT20	6.0 10.0 3.00 Edge	CHORDS			
H	BMVW-t	MT20	4.0 6.0	MEMB. MAX. FACTORED FORCE (LBS)			
I	BMVW-t	MT20	4.0 6.0	VERT. LOAD LC1 MAX. MAX. UNBRACED LENGTH			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				CHORDS				WEBS				RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD	
				MAX. FACTORED				MAX. FACTORED					
MEMB.				FORCE				MEMB.				FORCE	
(LBS)				(PLF)				UNBRAC				(LBS)	
FR-TO				FROM TO				LENGTH FR-TO				CSI (LC)	
A - B				0 / 49				-129.6 -129.6 0.17 (1)				10.00 C - H	
B - C				0 / 24				-129.6 -129.6 0.18 (1)				10.00 H - D	
C - D				-563 / 0				-129.6 -129.6 0.18 (1)				6.25 H - E	
D - E				-444 / 0				-129.6 -129.6 0.47 (1)				6.25 I - C	
G - E				-799 / 0				0.0 0.0 0.40 (1)				7.81	
I - B				-326 / 0				0.0 0.0 0.03 (1)				7.81	
I - H				0 / 602				-18.5 -18.5 0.26 (1)				10.00	
H - G				0 / 0				-18.5 -18.5 0.38 (1)				10.00	
G - F				0 / 0				-18.5 -18.5 0.16 (1)				10.00	
ALLOWABLE DEFL.(LL)= L/360 (0.36")												CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06")	
ALLOWABLE DEFL.(TL)= L/360 (0.36")												ALLOWABLE DEFL.(TL)= L/ 999 (0.10")	
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.10")													
CSI: TC=0.47/1.00 (D-E:1), BC=0.38/1.00 (G-H:1)												WB=0.25/1.00 (C-I:1), SI=0.30/1.00 (F-G:1)	
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10												COMP=1.10 SHEAR=1.10 TENS= 1.10	
COMPANION LIVE LOAD FACTOR = 1.00													

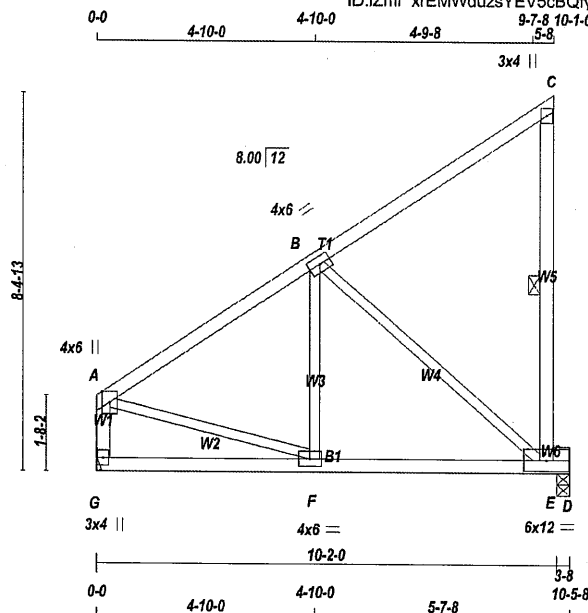
STRUCTURAL COMPONENT ONLY
DWG # TR23040030

STRUCTURAL COMPONENT ONLY
DWG # TR23040031



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T21A	2	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington



Scale = 1:49.0

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	Edge
B	TMVW-t	MT20	4.0	6.0	
C	TMV+p	MT20	3.0	4.0	
E	BMVW-t	MT20	6.0	12.0	
F	BMVW-t	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	774	0	774	0	0	MECHANICAL
D	727	0	727	0	0	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	539	399 / 0	0 / 0	0 / 0	0 / 0	140 / 0	0 / 0
D	507	372 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. UNBRACED LENGTH	
		FROM	TO			FR-TO	CS (LC)
A-B	-619 / 0	-129.6	-129.6	0.58 (1)	6.25	F-B	0 / 111
B-C	-43 / 0	-129.6	-129.6	0.56 (1)	6.25	B-E	-735 / 0
E-C	-263 / 0	0.0	0.0	0.08 (1)	6.25	A-F	0 / 575
G-A	-754 / 0	0.0	0.0	0.08 (1)	7.81		
G-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00		
F-E	0 / 554	-18.5	-18.5	0.46 (1)	10.00		
E-D	0 / 0	-18.5	-18.5	0.15 (1)	10.00		

TOTAL WEIGHT = 2 X 49 = 98 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 965 (0.13")

CSI: TC=0.58/1.00 (A-B:1), BC=0.46/1.00 (E-F:1), WB=0.64/1.00 (B-E:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (A) (INPUT = 0.90)
JSI METAL= 0.36 (A) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040032

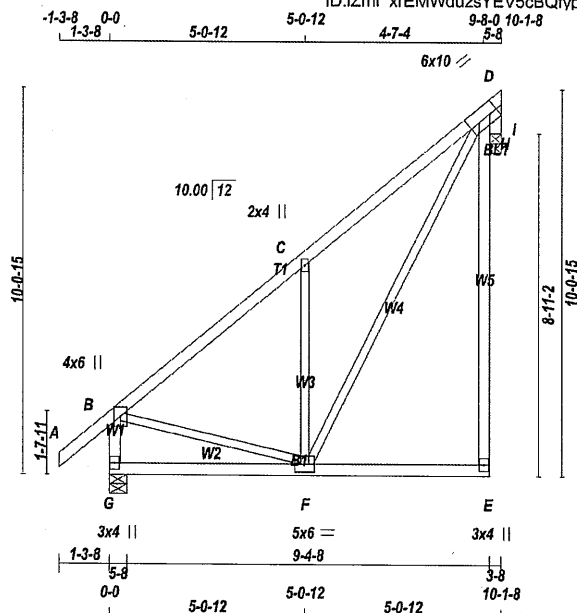


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T22	18	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:57.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	

BEARING BLOCKS	BL1	2x4	DRY	No.2	SPF
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ALL WEBS	2x3	DRY	No.2	SPF	EXCEPT
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DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW+w	MT20	2.0	4.0		
D	TMVWK1-t	MT20	6.0	10.0	Edge	2.50
E	BMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
I(D)	VERT	HORZ	DOWN	HORZ
G	919	0	919	0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I(D)	COMBINED						
G	500	370 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I(D), G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	UNBRAC LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX
FR-TO			FROM	TO			FR-TO			
A-B	0 / 57	-129.6	-129.6	0.18	(1)	10.00	F-C	-773 / 0	0.41	(1)
B-C	-493 / 0	-129.6	-129.6	0.40	(1)	6.25	F-D	0 / 864	0.19	(1)
C-D	-563 / 0	-129.6	-129.6	0.40	(1)	6.25	B-F	0 / 430	0.10	(1)
E-H	0 / 36	0.0	0.0	0.14	(1)	10.00	D-I	-732 / 0	0.07	(1)
H-D	0 / 36	0.0	0.0	0.14	(1)	10.00	H-I	0 / 146	0.00	(1)
G-B	-879 / 0	0.0	0.0	0.09	(1)	7.81				
G-F	0 / 0	-18.5	-18.5	0.12	(4)	10.00				
F-E	0 / 11	-18.5	-18.5	0.12	(4)	10.00				

TOTAL WEIGHT = 18 X 57 = 1034 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.40/1.00 (C-D:1), BC=0.12/1.00 (E-F:4), WB=0.41/1.00 (C-F:1), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.77 (F) (INPUT = 0.90)

JSI METAL = 0.41 (C) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

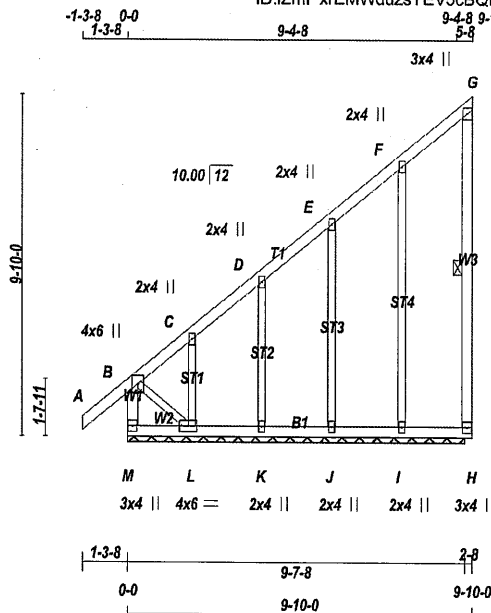


STRUCTURAL COMPONENT ONLY
DWG # TR23040033



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	G23	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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Scale: 3/16"=1'

TOTAL WEIGHT = 2 X 58 = 116 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
M - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F						
C	TMVW+w	MT20	2.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K						
I	BMV1+w	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
M-B	-382 / 0	0.0	0.04 (1)	I-F	-293 / 0	0.38 (1)	
A-B	0 / 57	-129.6	-129.6 0.18 (1)	J-E	-247 / 0	0.17 (1)	
B-C	-63 / 0	-129.6	-129.6 0.17 (1)	K-D	-274 / 0	0.09 (1)	
C-D	-7 / 0	-129.6	-129.6 0.07 (1)	L-C	-161 / 0	0.03 (1)	
D-E	-12 / 0	-129.6	-129.6 0.07 (1)	B-L	0 / 23	0.01 (1)	
E-F	-1 / 0	-129.6	-129.6 0.07 (1)				
F-G	-16 / 0	-129.6	-129.6 0.07 (1)				
H-G	-111 / 0	0.0	0.05 (1)				
M-L	0 / 0	-18.5	-18.5 0.02 (4)				
L-K	0 / 13	-18.5	-18.5 0.02 (4)				
K-J	0 / 7	-18.5	-18.5 0.01 (4)				
J-I	0 / 4	-18.5	-18.5 0.02 (4)				
I-H	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1), BC=0.02/1.00 (H-I:4), WB=0.38/1.00 (F-I:1), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

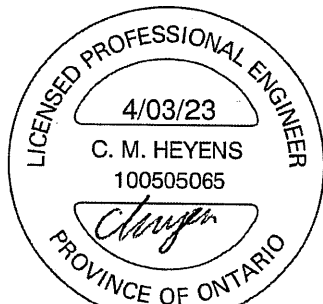
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

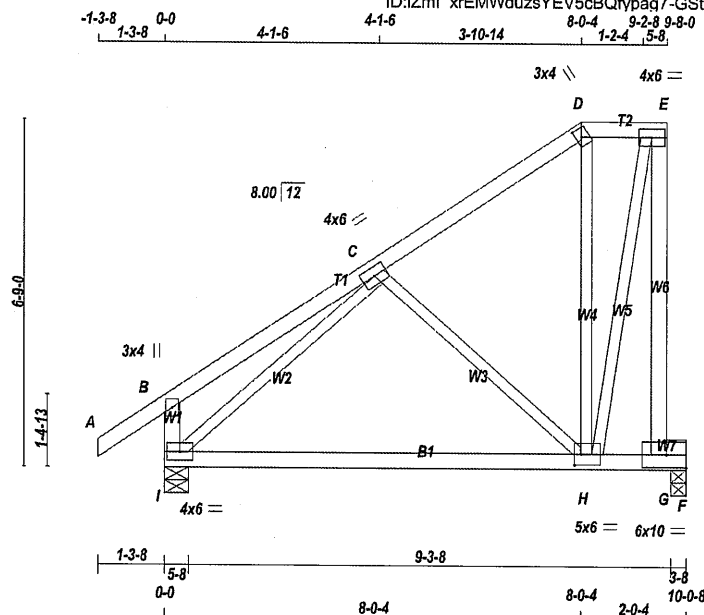


STRUCTURAL COMPONENT ONLY
DWG # TR23040004



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T27	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:42.8

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+tp	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0		Edge
H	BMVW-t	MT20	5.0	6.0	2.50	1.50
I	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
F	696	0	0	3-8
I	921	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX /MIN. COMPONENT REACTIONS
JT	COMBINED
F	486
I	639

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
FR-TO											
A-B	0 / 49	-129.6	-129.6	0.17	(1)	10.00	C-H	-434 / 0	0.23	(1)	
B-C	0 / 35	-129.6	-129.6	0.35	(1)	10.00	H-D	-173 / 5	0.13	(1)	
C-D	-274 / 0	-129.6	-129.6	0.35	(1)	6.25	H-E	0 / 844	0.19	(1)	
D-E	-196 / 0	-129.6	-129.6	0.06	(1)	6.25	I-C	-712 / 0	0.35	(1)	
G-E	-926 / 0	0.0	0.0	0.89	(1)	7.81					
I-B	-374 / 0	0.0	0.0	0.04	(1)	7.81					
I-H	0 / 520	-18.5	-18.5	0.32	(4)	10.00					
H-G	0 / 0	-18.5	-18.5	0.35	(1)	10.00					
G-F	0 / 0	-18.5	-18.5	0.15	(1)	10.00					

TOTAL WEIGHT = 53 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 38.2	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.89/1.00 (E-G:1), BC=0.35/1.00 (G-H:1), WB=0.35/1.00 (C-I:1), SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (H) (INPUT = 0.90)
JSI METAL= 0.23 (H) (INPUT = 1.00)

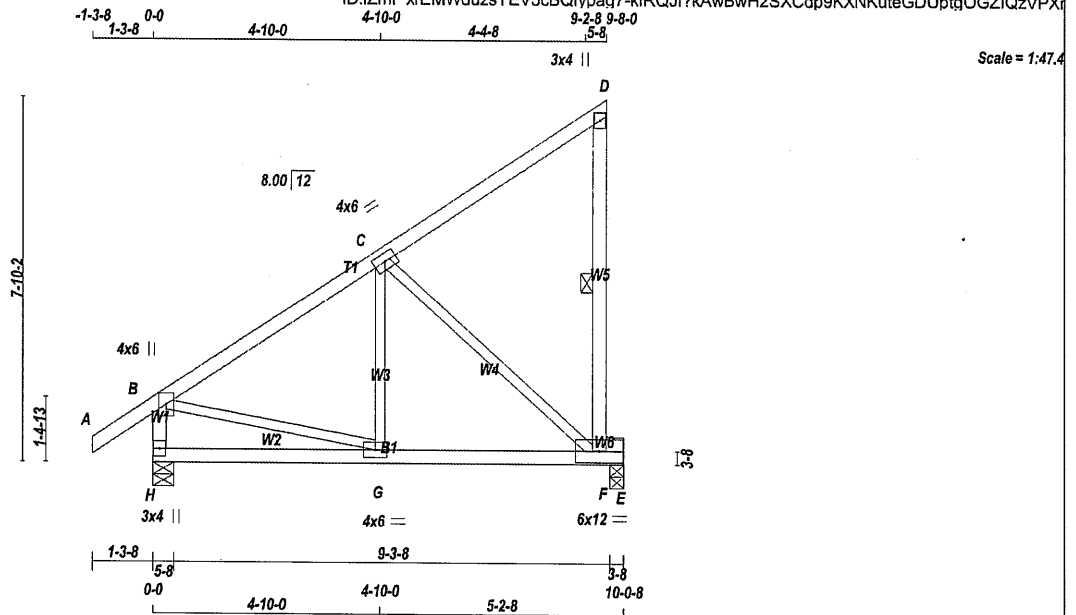
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040035

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T28	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 3 X 48 = 145 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge		
C	TMVW-t	MT20	4.0	6.0			
D	TMV+p	MT20	3.0	4.0			
F	BMVW-t	MT20	6.0	12.0			
G	BMVW-t	MT20	4.0	6.0			
H	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	921	0	921	0
E	696	0	696	0

UNFACTORED REACTIONS

1ST LOASE	MAX /MIN. COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	639	488 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
E	486	356 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO				
A-B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	G-C	0 / 114	0.04 (4)	
B-C	-611 / 0	-129.6	-129.6	0.40 (1)	6.25	C-F	-728 / 0	0.52 (1)	
C-D	-43 / 0	-129.6	-129.6	0.39 (1)	6.25	B-G	0 / 557	0.13 (1)	
F-D	-236 / 0	0.0	0.0	0.06 (1)	6.25				
H-B	-899 / 0	0.0	0.0	0.09 (1)	7.81				
H-G	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
G-F	0 / 544	-18.5	-18.5	0.44 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.15 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.40/1.00 (B-C:1), BC=0.44/1.00 (F-G:1), WB=0.52/1.00 (C-F:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)

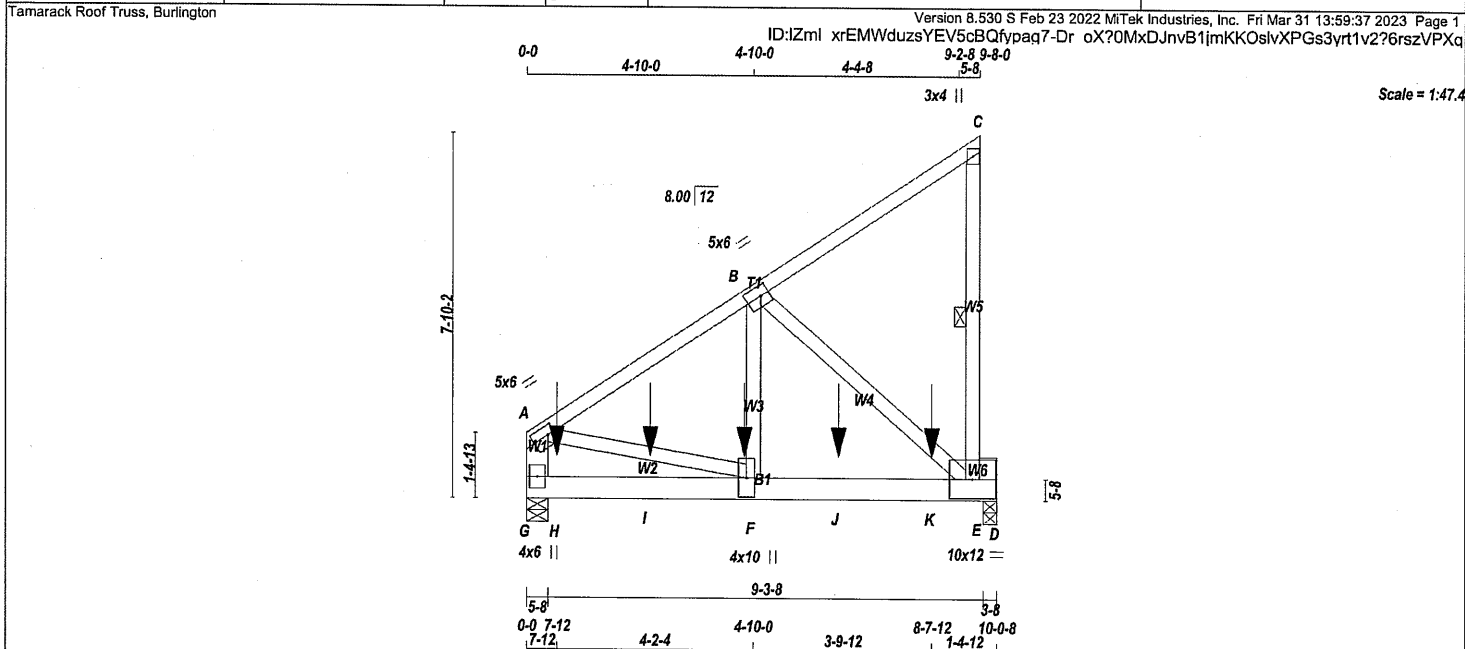
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040036



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T29	3	3	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - D	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-E	12	TOP
G-A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G-D	6	SIDE(676.5)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	
2x6	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	6032	0	6032	0	5-8	5-8
D	5317	0	5317	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX /MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
G	4199	3136 / 0	0 / 0	0 / 0	0 / 0	1063 / 0	0 / 0
D	3701	2762 / 0	0 / 0	0 / 0	0 / 0	940 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1 / 2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)
	FROM	TO	FROM	TO				FROM	TO	
A-B	-5083 / 0		-129.6	-129.6	0.22 (1)	4.92	F-B	0 / 5794		0.31 (1)
B-C	-39 / 0		-129.6	-129.6	0.15 (1)	6.25	B-E	-5656 / 0		0.91 (1)
E-C	-244 / 0		0.0	0.0	0.02 (1)	6.25	A-F	0 / 4351		0.23 (1)
G-A	-3963 / 0		0.0	0.0	0.09 (1)	7.81				
G-H	0 / 0		-18.5	-18.5	0.64 (1)	10.00				
H-I	0 / 0		-18.5	-18.5	0.64 (1)	10.00				
I-F	0 / 0		-18.5	-18.5	0.64 (1)	10.00				
F-J	0 / 4261		-18.5	-18.5	0.92 (1)	10.00				
J-K	0 / 4261		-18.5	-18.5	0.92 (1)	10.00				
K-E	0 / 4261		-18.5	-18.5	0.92 (1)	10.00				
E-D	0 / 0		-18.5	-18.5	0.19 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	4-7-12	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1
H	7-12	-1381	-1381	—	FRONT	VERT	TOTAL	—	C1
I	2-7-12	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1
J	6-7-12	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1
K	8-7-12	-1379	-1379	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 9 X 58 = 521 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.2	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 769 (0.16")

CSI: TC=0.22/1.00 (A-B:1), BC=0.92/1.00 (E-F:1), WB=0.91/1.00 (B-E:1), SSI=0.54/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

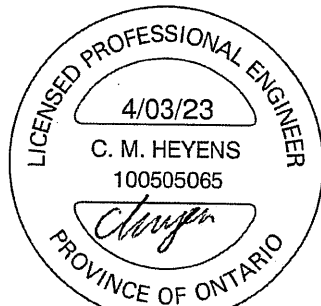
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.64 (F) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR23040037



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T29	3	3	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	1.75
B	TMVW-t	MT20	5.0	6.0	2.50	2.25
C	TMV+p	MT20	3.0	4.0		
E	BMVW-t	MT20	10.0	12.0		
F	BMVW-t	MT20	4.0	10.0		
G	BMV1+p	MT20	4.0	6.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040037

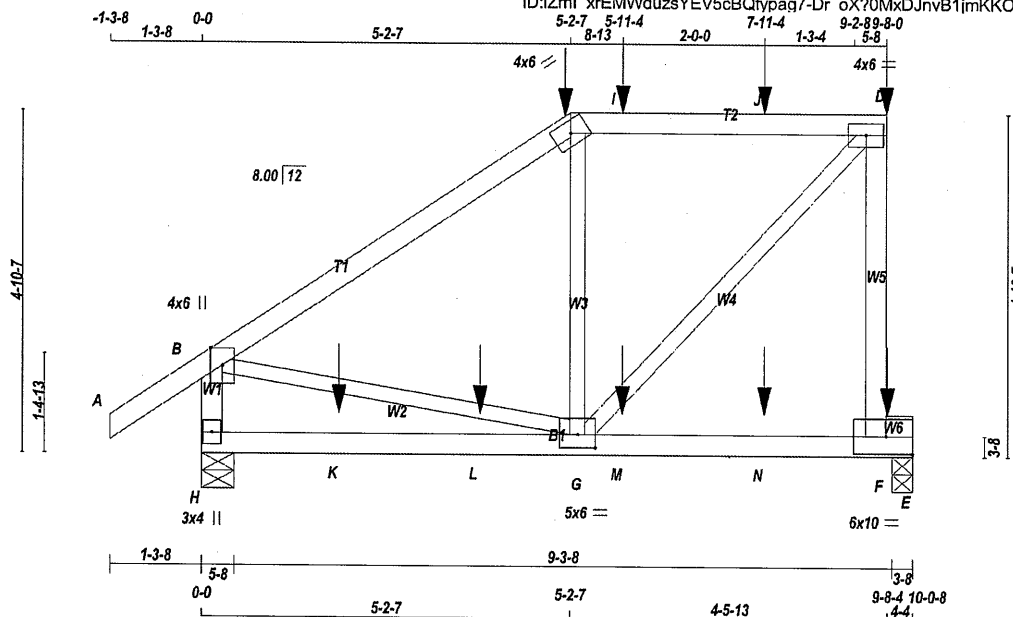


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T32	2	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:31.4

TOTAL WEIGHT = 2 X 44 = 89 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW-h	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
F	BMVW-t	MT20	6.0	10.0	Edge	
G	BMVWV-t	MT20	5.0	6.0	2.25	3.00
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	1202	0	1202	0	5-8	5-8
E	1279	0	1279	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	835	633 / 0	0 / 0	0 / 0	0 / 0	203 / 0	0 / 0
E	890	666 / 0	0 / 0	0 / 0	0 / 0	224 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED				MEMB.	WEBS MAX. FACTORED FORCE (LBS)
		VERT.	LOAD LC1	MAX	MAX.		
FR-TO							
A-B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	G-C	-614 / 0
B-C	-904 / 0	-129.6	-129.6	0.72 (1)	5.13	G-D	0 / 1064
C-I	-727 / 0	-129.6	-129.6	0.75 (1)	5.35	B-G	0 / 766
I-J	-727 / 0	-129.6	-129.6	0.75 (1)	5.35		
J-D	-727 / 0	-129.6	-129.6	0.75 (1)	5.35		
F-D	-1348 / 0	0.0	0.0	0.54 (1)	6.94		
H-B	-1169 / 0	0.0	0.0	0.13 (1)	7.34		
H-K	0 / 0	-18.5	-18.5	0.26 (4)	10.00		
K-L	0 / 0	-18.5	-18.5	0.26 (4)	10.00		
L-G	0 / 0	-18.5	-18.5	0.26 (4)	10.00		
G-M	0 / 0	-18.5	-18.5	0.72 (1)	10.00		
M-N	0 / 0	-18.5	-18.5	0.72 (1)	10.00		
N-F	0 / 0	-18.5	-18.5	0.72 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.30 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-2-7	-30	-30		FRONT	VERT	DEAD		C1
C	5-2-7	-191	-191		FRONT	VERT	SNOW		C1
D	9-8-0	-128	-128		BACK	VERT	TOTAL		C1
F	9-8-4	-19	-19		BACK	VERT	TOTAL		C1
I	5-11-4	-95	-95		BACK	VERT	TOTAL		C1
J	7-11-4	-83	-83		BACK	VERT	TOTAL		C1
K	1-11-4	-14	-14		BACK	VERT	TOTAL		C1
L	3-11-4	-14	-14		BACK	VERT	TOTAL		C1
M	5-11-4	-14	-14		BACK	VERT	TOTAL		C1
N	7-11-4	-14	-14		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 786 (0.15")

CSI: TC=0.75/1.00 (C-D:1), BC=0.72/1.00 (F-G:1)
, WB=0.26/1.00 (D-G:1), SSI=0.55/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN
MT20	650	371	1747	788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

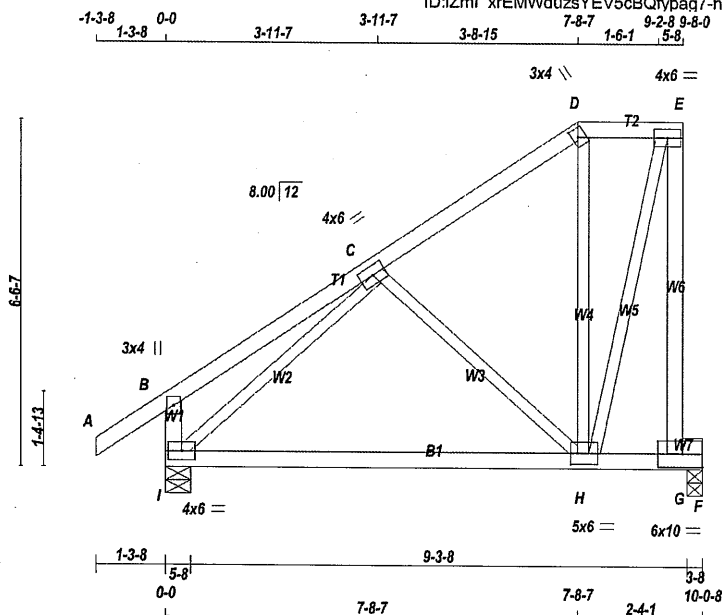


STRUCTURAL COMPONENT ONLY
DWG # TR23040038

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T33	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:41.5

TOTAL WEIGHT = $2 \times 53 = 105 \text{ lb}$

LUMBER

N. L. G. A. RULES			
CHORDS	SIZE		
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
G - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS	2x3	DRY
EXCEPT		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	4.50
H	BMWWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	696	0	696	0	0	3-8	3-8
I	921	0	921	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	486	356 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
I	639	488 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F. I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO				
A-B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C-H	-405 / 0	0.20 (1)	
B-C	0 / 33	-129.6	-129.6	0.32 (1)	10.00	H-D	-168 / 7	0.12 (1)	
C-D	-306 / 0	-129.6	-129.6	0.32 (1)	6.25	H-E	0 / 784	0.18 (1)	
D-E	-224 / 0	-129.6	-129.6	0.09 (1)	6.25	I-C	-723 / 0	0.33 (1)	
G-E	-877 / 0	0.0	0.0	0.77 (1)	7.81				
I-B	-367 / 0	0.0	0.0	0.04 (1)	7.81				
I-H	0 / 525	-18.5	-18.5	0.30 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.35 (1)	10.00				
G-F	0 / 0	-18.5	-18.5	0.15 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.77/1.00 (E-G:1), BC=0.35/1.00 (G-H:1)
 , WB=0.33/1.00 (C-I:1), SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

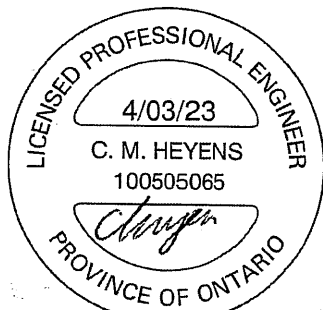
NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (H) (INPUT = 0.90)
JSI METAL= 0.21 (H) (INPUT = 1.00)

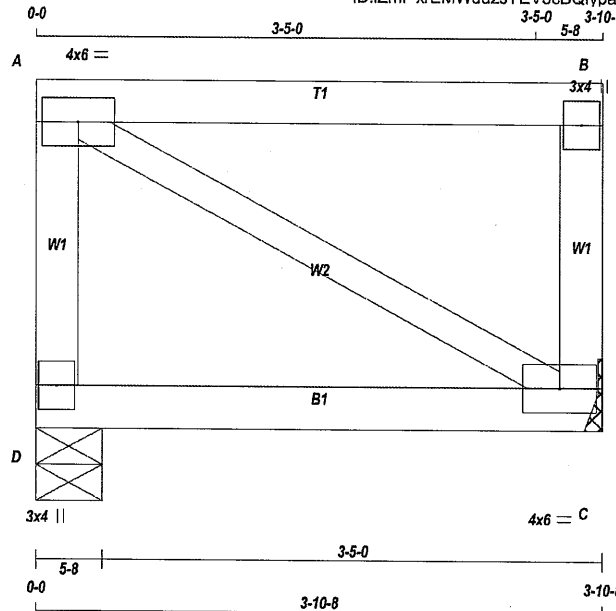
LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040039

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T36	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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Scale = 1:15.2

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF
D - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMV+P	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	6.0		
D	BMV1+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
D	287	0	287	0
C	287	0	287	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

1ST LCASE	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	D	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
	C	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
D-A	-251 / 0	0.0	0.03 (1)	A-C	0 / 0	0.00 (1)	
A-B	0 / 0	-129.6	-129.6 0.33 (1)				
C-B	-251 / 0	0.0	0.03 (1)				
D-C	0 / 0	-18.5	-18.5 0.08 (4)				

TOTAL WEIGHT = 2 X 16 = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (A-B:1), BC=0.08/1.00 (C-D:4), WB=0.00/1.00 (A-C:1), SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (B) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)

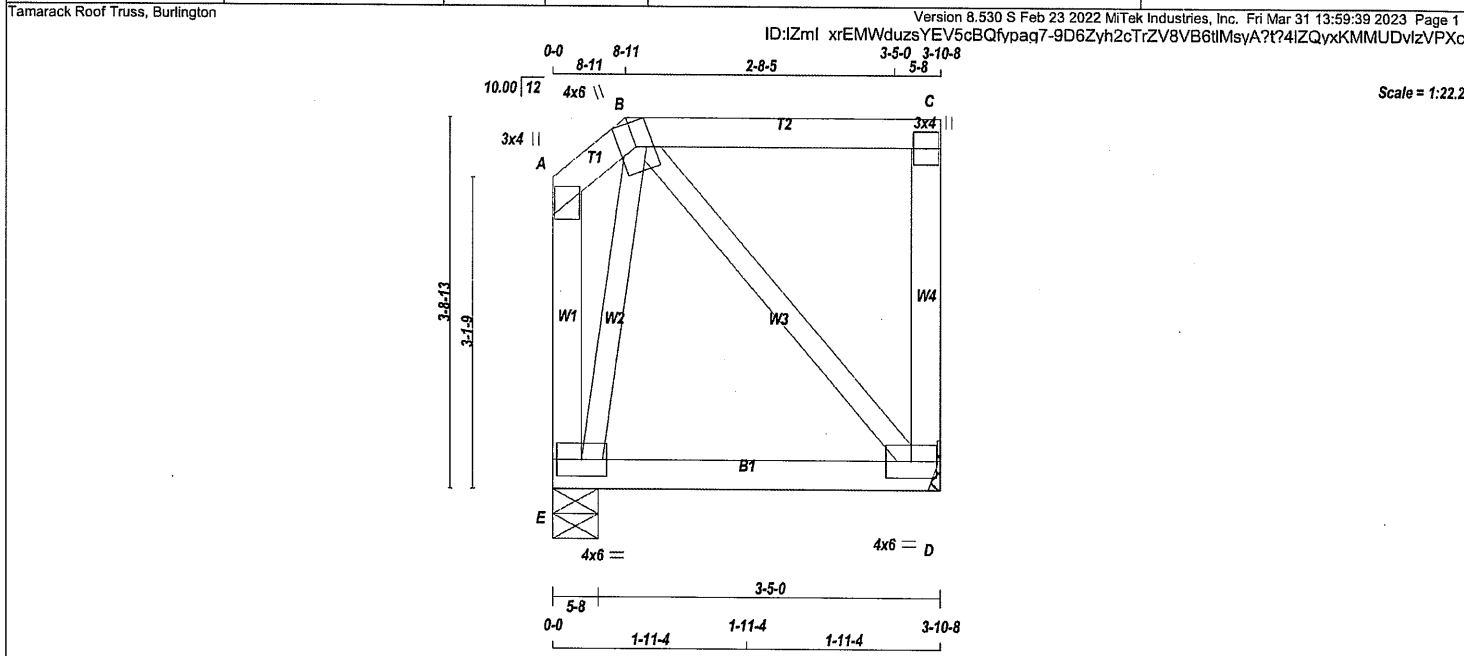
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040040



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T37	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 22 = 44 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
E - A	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY: SEASONED LUMBER.				SPF	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TTWW+m	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	6.0		
E	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
D	287	0	287	0	0	MECHANICAL			
E	287	0	287	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
D	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0	
E	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS			
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	MAX. FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 0	-129.6	-129.6	0.01 (1)	10.00	B-D	-68 / 0	0.02 (1)	
B-C	0 / 0	-129.6	-129.6	0.21 (1)	10.00	E-B	-206 / 0	0.05 (1)	
D-C	-201 / 0	0.0	0.0	0.04 (1)	7.81				
E-A	-50 / 0	0.0	0.0	0.01 (1)	7.81				
E-D	0 / 45	-18.5	-18.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.08/1.00 (D-E:4)
WB=0.05/1.00 (B-E:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

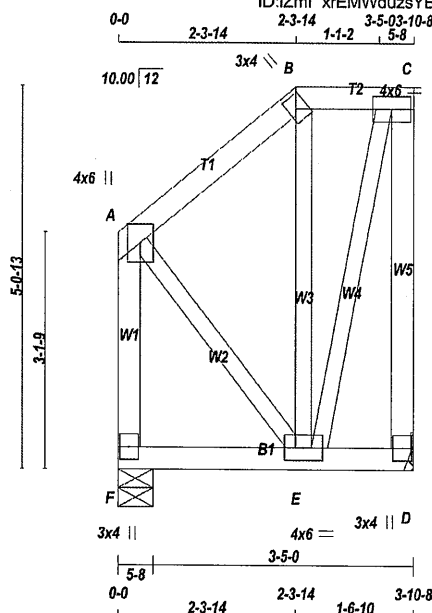


STRUCTURAL COMPONENT ONLY
DWG # TR23040041



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T38	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:29.2

TOTAL WEIGHT = 2 X 28 = 57 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
	GROSS REACTION		GROSS REACTION		BRG		BRG	
D	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
D	287	0	287	0	0	MECHANICAL		
F	287	0	287	0	0	5-8	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
D	200	148 / 0	0 / 0	0 / 0	0 / 0	0 / 0	52 / 0
F	200	148 / 0	0 / 0	0 / 0	0 / 0	0 / 0	52 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-74 / 0	-129.6 -129.6	0.09 (1)	E-B	-204 / 0	0.08 (1)	
B-C	-52 / 0	-129.6 -129.6	0.04 (1)	E-C	0 / 181	0.04 (1)	
D-C	-273 / 0	0.0 0.0	0.12 (1)	A-E	0 / 87	0.02 (1)	
F-A	-264 / 0	0.0 0.0	0.05 (1)				
F-E	0 / 0	-18.5 -18.5	0.02 (4)				
E-D	0 / 0	-18.5 -18.5	0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.12/1.00 (C-D:1), BC=0.02/1.00 (E-F:4), WB=0.09/1.00 (B-E:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)
JSI METAL= 0.07 (A) (INPUT = 1.00)

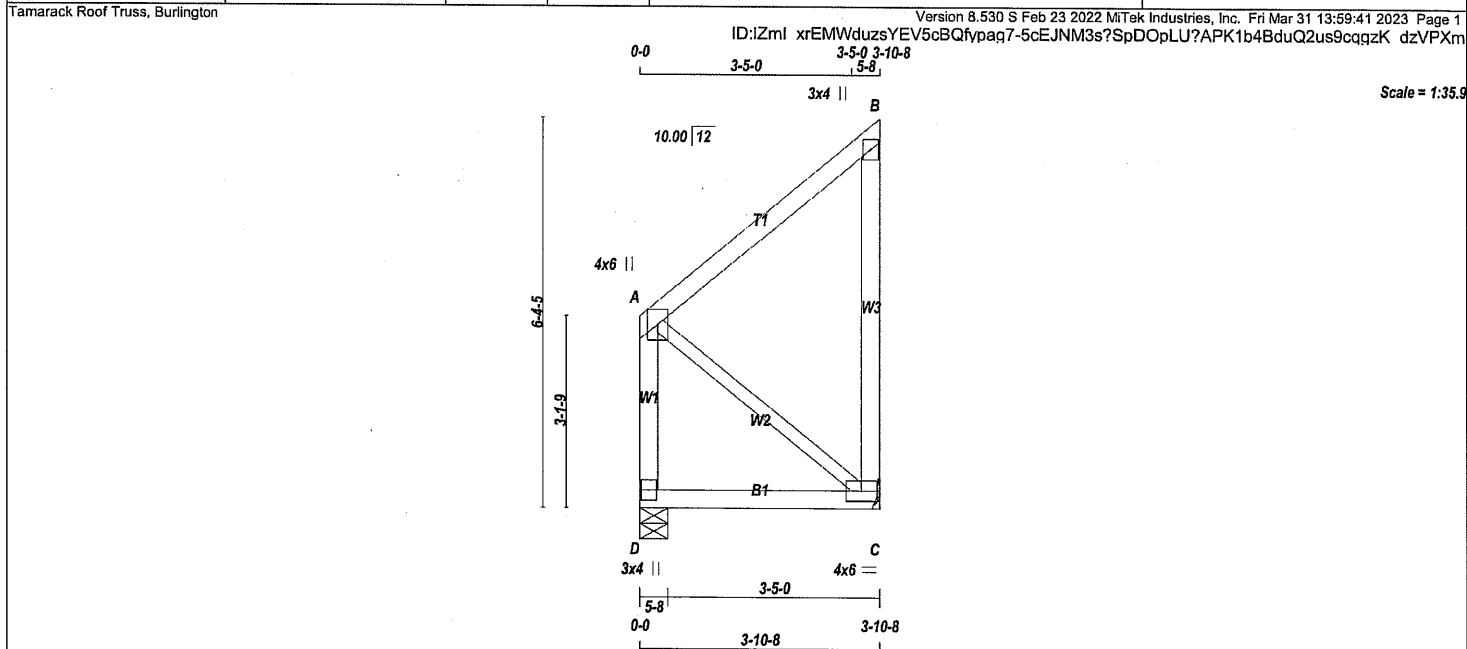
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040042



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T39	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 23 = 47 lb

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
C	287	0	287	0	MECHANICAL	
D	287	0	287	0	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
C	200	148 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
D	200	148 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 0	-129.6	-129.6	0.33 (1)	10.00	0 / 0	0.00 (1)
C-B	-251 / 0	0.0	0.0	0.20 (1)	7.81		
D-A	-251 / 0	0.0	0.0	0.04 (1)	7.81		
D-C	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	38.2	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (A-B:1), BC=0.08/1.00 (C-D:4), WB=0.00/1.00 (A-C:1), SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

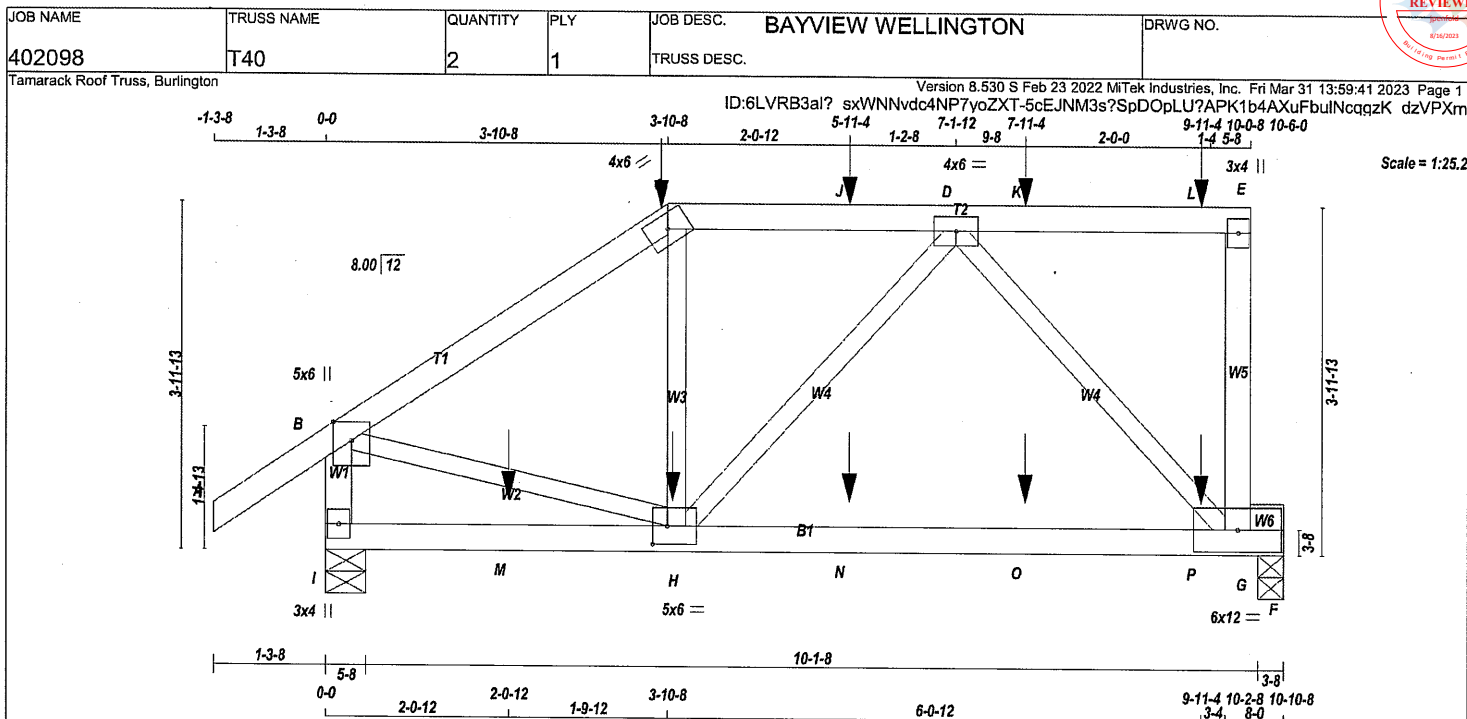
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (A) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

STRUCTURAL COMPONENT ONLY
DWG # TR23040043



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-h	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
G	BMVW-t	MT20	6.0	12.0		
H	BMVW-t	MT20	5.0	6.0	2.50	2.00
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
I	1292	0	1292	0	5-8	5-8
F	1195	0	1195	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	UNBRAC			
I	898	678 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0
F	833	615 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	
FR-TO				FR-TO			
A-B	0 / 49	-129.6	-129.6 0.19 (1)	H-C	-83 / 104	0.03 (4)	
B-C	-1128 / 0	-129.6	-129.6 0.40 (1)	B-H	0 / 970	0.24 (1)	
C-J	-932 / 0	-129.6	-129.6 0.40 (1)	H-D	0 / 171	0.05 (4)	
J-D	-932 / 0	-129.6	-129.6 0.40 (1)	D-G	-1224 / 0	0.50 (1)	
D-K	0 / 0	-129.6	-129.6 0.38 (1)				
K-L	0 / 0	-129.6	-129.6 0.38 (1)				
L-E	0 / 0	-129.6	-129.6 0.38 (1)				
G-E	-295 / 0	0.0	0.0 0.08 (1)				
I-B	-1307 / 0	0.0	0.0 0.15 (1)				
I-M	0 / 0	-18.5	-18.5 0.34 (4)				
M-H	0 / 0	-18.5	-18.5 0.34 (4)				
H-N	0 / 823	-18.5	-18.5 0.81 (1)				
N-O	0 / 823	-18.5	-18.5 0.81 (1)				
O-P	0 / 823	-18.5	-18.5 0.81 (1)				
P-G	0 / 823	-18.5	-18.5 0.81 (1)				
G-F	0 / 0	-18.5	-18.5 0.28 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-190	-190	122	FRONT	VERT	TOTAL	---	C1
H	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
J	5-11-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
K	7-11-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
L	9-11-4	-102	-102	---	FRONT	VERT	TOTAL	---	C1
M	2-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
N	5-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
O	7-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
P	9-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	38.2	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL)= L/903 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL)= L/413 (0.32")
CSI: TC=0.40/1.00 (C-D:1), BC=0.81/1.00 (G-H:1), WB=0.50/1.00 (D-G:1), SSI=0.51/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

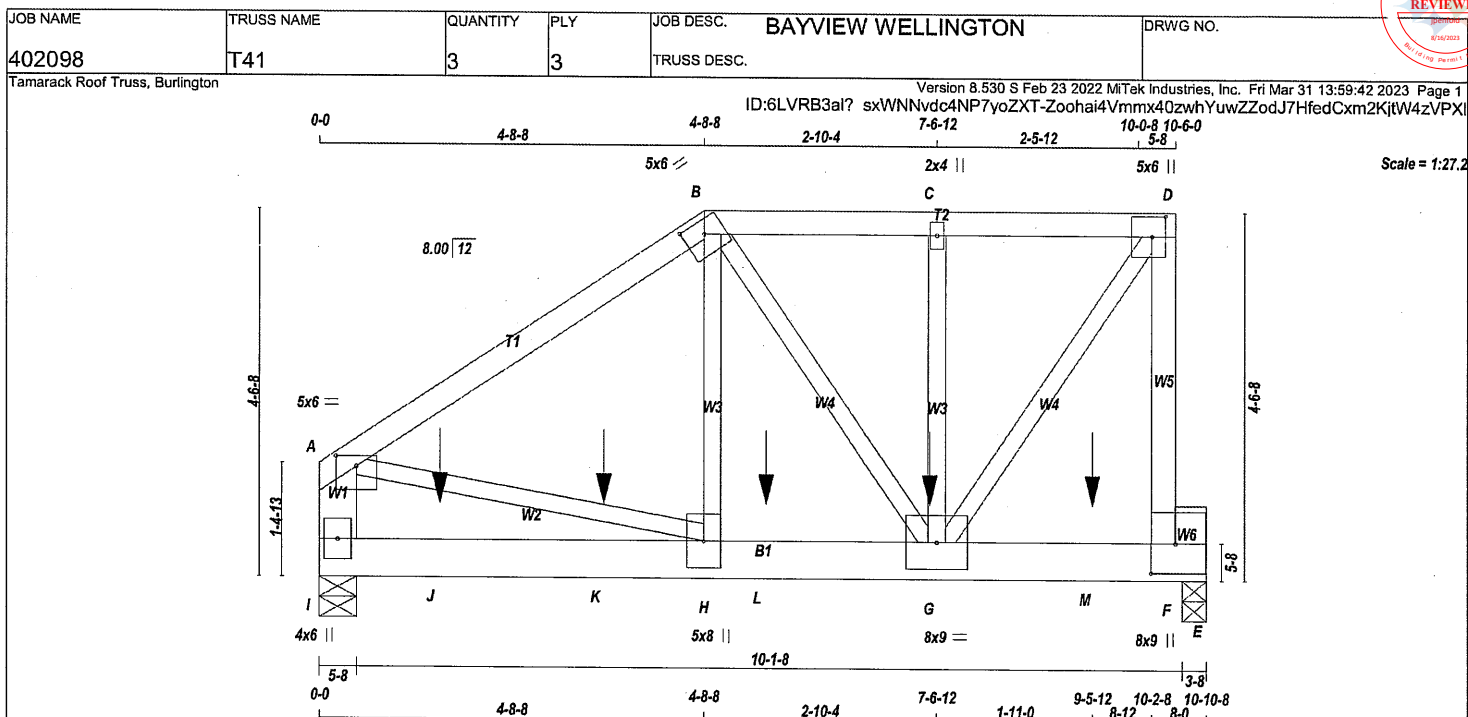
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (H) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

STRUCTURAL COMPONENT ONLY
DWG # TR23040044



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
F - D	2x4	DRY	No.2
I - A	2x6	DRY	No.2
I - E	2x6	DRY	No.2
F - E	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	TOP
B-D 1	12	TOP
D-F 1	12	TOP
I-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
I-E 2	7	SIDE(758.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x6 2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX		IN-SX	
I		5705	0	5705	0	5-8		5-8	
E		5764	0	5764	0	3-8		3-8	

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS	
JT		COMBINED	SNOW	LIVE	PERM.LIVE
I		3971	2967 / 0	0 / 0	0 / 0
E		4013	2995 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-5221 / 0	A-B	0 / 3508
B-C	-3675 / 0	B-C	0 / 4430
C-D	-3675 / 0	B-G	-1324 / 0
F-D	-5382 / 0	G-C	-395 / 0
I-A	-4109 / 0	G-D	0 / 6377
I-J	0 / 0		
J-K	0 / 0		
K-H	0 / 0		
H-L	0 / 4426		
L-G	0 / 4426		
G-M	0 / 0		
M-F	0 / 0		
F-E	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX-	MAX+	BACK	VERT	TOTAL				
G	7-5-12	-1379	-1379		BACK	VERT	TOTAL			C1	
J	1-5-12	-1379	-1379		BACK	VERT	TOTAL			C1	
K	3-5-12	-1379	-1379		BACK	VERT	TOTAL			C1	
L	5-5-12	-1379	-1379		BACK	VERT	TOTAL			C1	
M	9-5-12	-1379	-1379		BACK	VERT	TOTAL			C1	

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 9 X 57 = 509 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.54/1.00 (D-F:1), BC=0.57/1.00 (G-H:1), WB=0.48/1.00 (D-G:1), SSI=0.58/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

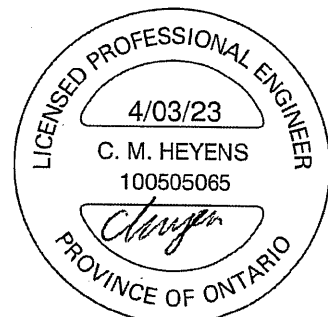
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.53 (H) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR23040045



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T41	3	3	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

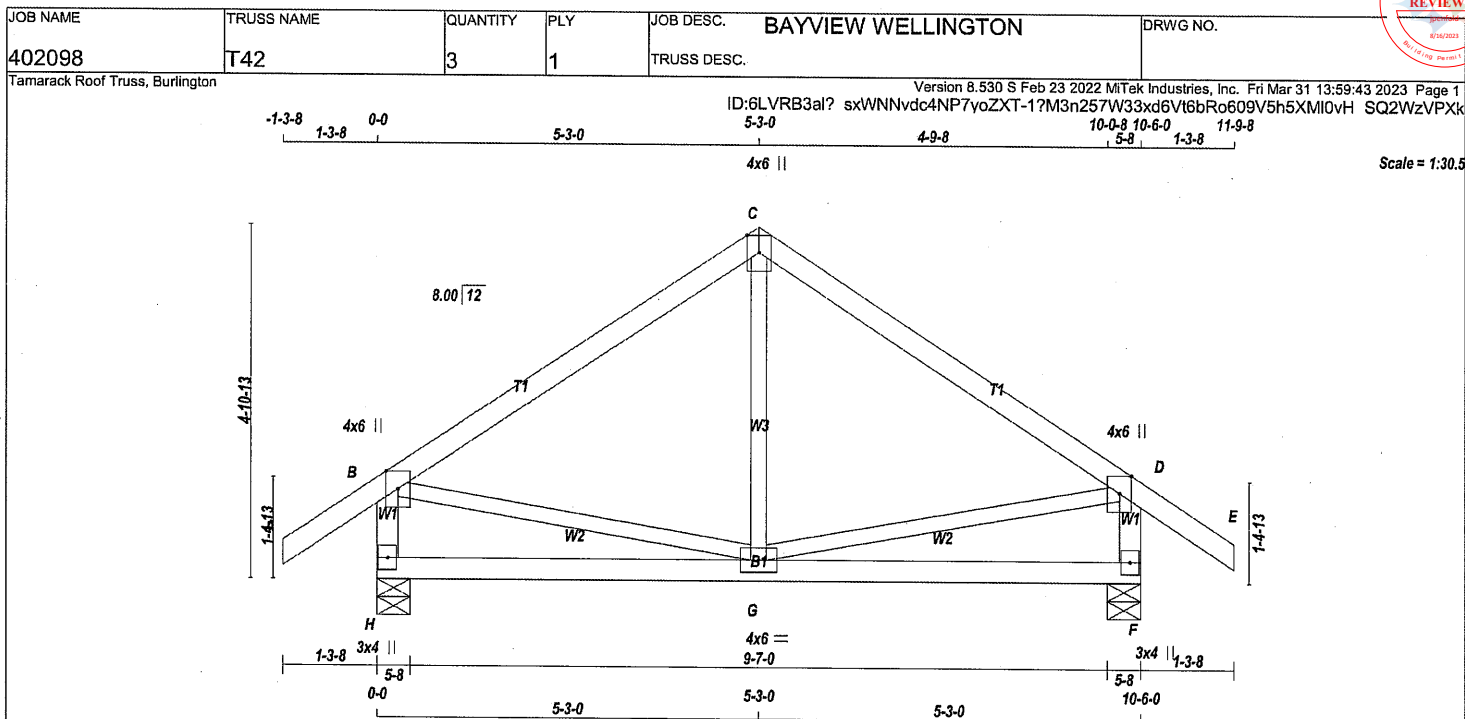
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:42 2023 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TTWW-h	MT20	5.0	6.0	2.00	3.00
C	TMVW+w	MT20	2.0	4.0		
D	TMVW+p	MT20	5.0	6.0	3.00	2.00
F	BMVW+p	MT20	8.0	9.0	4.25	3.50
G	BMVWW-t	MT20	8.0	9.0		
H	BMVWW-t	MT20	5.0	8.0		
I	BMV1+p	MT20	4.0	6.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040045



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	956	0	956	0	5-8	5-8
F	956	0	956	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
H	663	506 / 0	0 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0	0 / 0
F	663	506 / 0	0 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	LC1 MAX (LC)
FR-TO				FR-TO			
A-B	0 / 49	-129.6	-129.6 0.17 (1)	G-C	-70 / 82	0.03 (4)	
B-C	-551 / 0	-129.6	-129.6 0.46 (1)	B-G	0 / 468	0.11 (1)	
C-D	-551 / 0	-129.6	-129.6 0.46 (1)	G-D	0 / 468	0.11 (1)	
D-E	0 / 49	-129.6	-129.6 0.17 (1)				
H-B	-918 / 0	0.0	0.0 0.09 (1)				
F-D	-918 / 0	0.0	0.0 0.09 (1)				
H-G	0 / 0	-18.5	-18.5 0.14 (4)				
G-F	0 / 0	-18.5	-18.5 0.14 (4)				

TOTAL WEIGHT = 3 X 44 = 132 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.46/1.00 (B-C:1), BC=0.14/1.00 (G-H:4), WB=0.11/1.00 (D-G:1), SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (G) (INPUT = 0.90)
JSI METAL= 0.39 (D) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

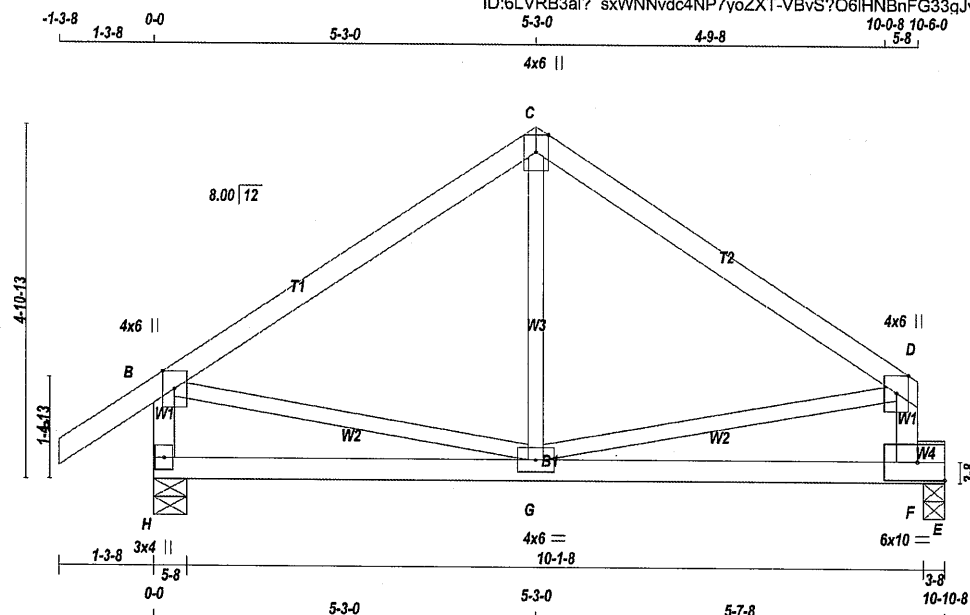


STRUCTURAL COMPONENT ONLY
DWG # TR23040046



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T42X	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:30.5

LUMBER			DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BMVW-t	MT20	6.0	10.0	Edge	
G	BMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
H	983	0	983	0	5-8	5-8		
E	758	0	758	0	3-8	3-8		

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
H	682	520 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
E	529	387 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 49	-129.6	-129.6 0.17 (1)	G-C	-8 / 98	0.03 (4)	
B-C	-606 / 0	-129.6	-129.6 0.64 (1)	B-G	0 / 515	0.12 (1)	
C-D	-606 / 0	-129.6	-129.6 0.64 (1)	G-D	0 / 515	0.12 (1)	
H-B	-958 / 0	0.0	0.0 0.10 (1)				
F-D	-780 / 0	0.0	0.0 0.08 (1)				
H-G	0 / 0	-18.5	-18.5 0.18 (4)				
G-F	0 / 0	-18.5	-18.5 0.38 (1)				
F-E	0 / 0	-18.5	-18.5 0.16 (1)				

TOTAL WEIGHT = 2 X 43 = 86 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/986 (0.13")

CSI: TC=0.64/1.00 (B-C:1), BC=0.38/1.00 (F-G:1), WB=0.12/1.00 (D-G:1), SS=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)

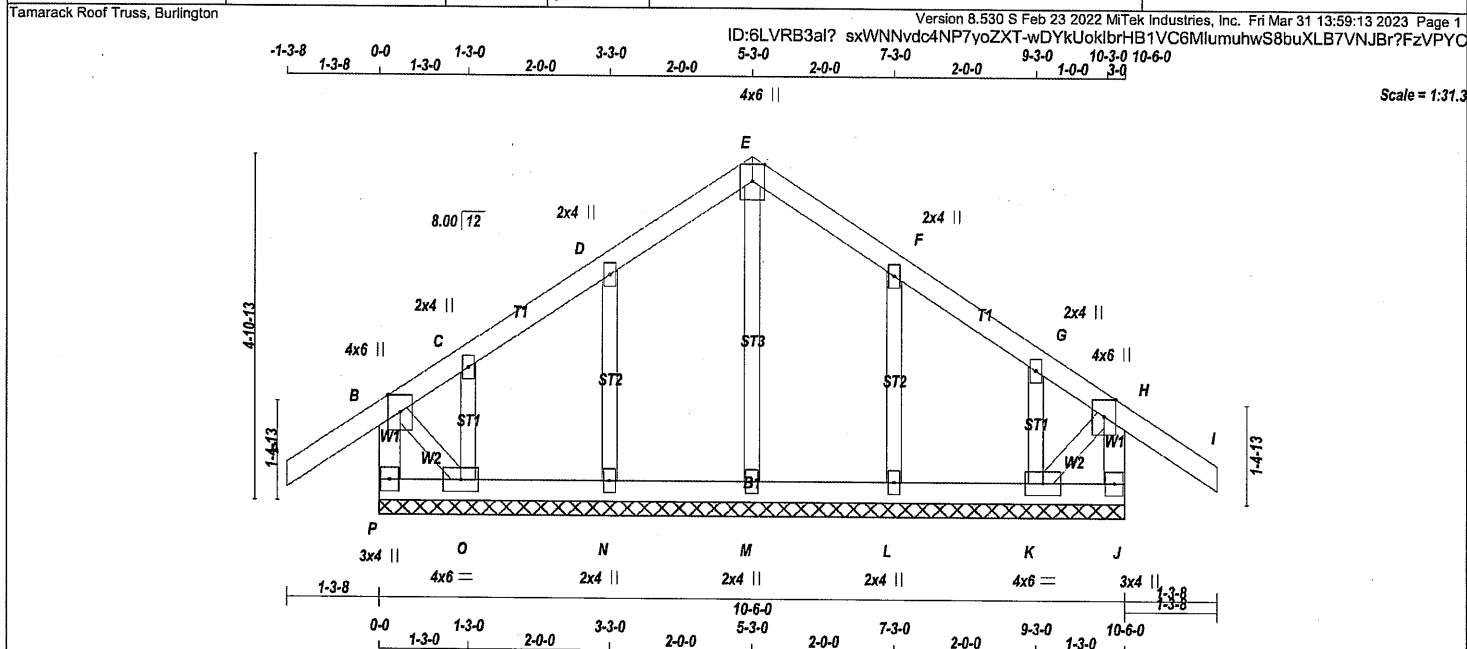
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040047



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	G42	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMVW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMVWV1-t	MT20	4.0	6.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVWV1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
FR-TO					FR-TO		
A-B	0 / 49	-129.6	-129.6 0.17 (1)	10.00	M-E	-206 / 0	0.07 (1)
B-C	-70 / 0	-129.6	-129.6 0.17 (1)	6.25	N-D	-315 / 0	0.07 (1)
C-D	0 / 0	-129.6	-129.6 0.08 (1)	10.00	O-C	-83 / 0	0.01 (1)
D-E	-20 / 0	-129.6	-129.6 0.08 (1)	6.25	L-F	-315 / 0	0.07 (1)
E-F	-20 / 0	-129.6	-129.6 0.08 (1)	6.25	K-G	-83 / 0	0.01 (1)
F-G	0 / 0	-129.6	-129.6 0.08 (1)	10.00	B-O	0 / 20	0.00 (1)
G-H	-70 / 0	-129.6	-129.6 0.17 (1)	6.25	K-H	0 / 20	0.00 (1)
H-I	0 / 49	-129.6	-129.6 0.17 (1)	10.00			
P-B	-371 / 0	0.0	0.0 0.04 (1)	7.81			
J-H	-371 / 0	0.0	0.0 0.04 (1)	7.81			
P-O	0 / 0	-18.5	-18.5 0.01 (4)	10.00			
O-N	0 / 11	-18.5	-18.5 0.02 (4)	10.00			
N-M	0 / 3	-18.5	-18.5 0.02 (4)	10.00			
M-L	0 / 3	-18.5	-18.5 0.02 (4)	10.00			
L-K	0 / 11	-18.5	-18.5 0.02 (4)	10.00			
K-J	0 / 0	-18.5	-18.5 0.01 (4)	10.00			

TOTAL WEIGHT = 46 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1), BC=0.02/1.00 (K-L:4), WB=0.07/1.00 (E-M:1), SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

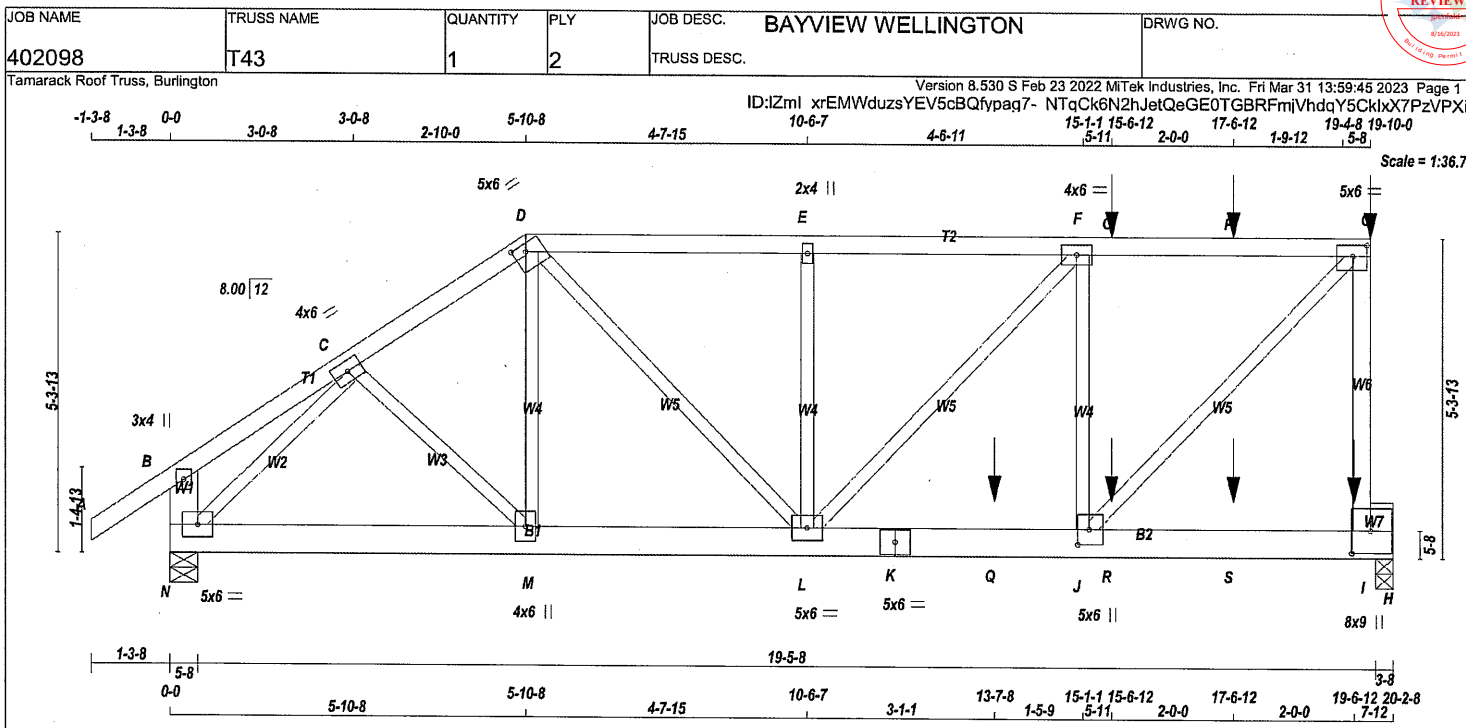
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040005



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF I - O 2x4 DRY No.2 SPF N - B 2x6 DRY No.2 SPF N - K 2x6 DRY No.2 SPF K - H 2x6 DRY No.2 SPF I - H 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS: (0.122"x3") SPIRAL NAILS A-D 1 12 TOP D-G 1 12 SIDE(61.0) G-I 1 12 TOP N-B 2 12 TOP BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS N-K 2 12 TOP K-H 2 12 SIDE(122.0) WEBS: (0.122"x3") SPIRAL NAILS 2x3 6 2x6 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX N 2315 0 2315 0 0 5-8 5-8 H 3358 0 3358 0 0 3-8 3-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL N 1609 1219 / 0 0 / 0 0 / 0 390 / 0 0 / 0 H 2335 1756 / 0 0 / 0 0 / 0 579 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE VERT. LOAD LC1 MAX. MAX. FACTORED MEMB. (LBS) (PLF) CSI (LC) UNBRAC (LBS) MAX. FACTORED FR-TO FROM TO LENGTH FR-TO A-B 0 / 49 -129.6 -129.6 0.10 (1) 10.00 C-M 0 / 160 0.02 (1) B-C 0 / 20 -129.6 -129.6 0.08 (1) 10.00 M-D -71 / 59 0.01 (1) C-D -2463 / 0 -129.6 -129.6 0.10 (1) 5.65 N-C -2715 / 0 0.37 (1) D-E -2990 / 0 -129.6 -129.6 0.23 (1) 5.09 J-G 0 / 4169 0.52 (1) E-F -2991 / 0 -129.6 -129.6 0.25 (1) 5.07 D-L 0 / 1404 0.17 (1) F-O -2886 / 0 -129.6 -129.6 0.25 (1) 5.14 J-F -810 / 0 0.16 (1) O-P -2886 / 0 -129.6 -129.6 0.25 (1) 5.14 L-E -638 / 0 0.13 (1) P-G -2886 / 0 -129.6 -129.6 0.25 (1) 5.14 L-F 0 / 154 0.02 (1) I-G -3345 / 0 0.0 0.0 0.77 (1) 6.38 N-B -331 / 0 0.0 0.0 0.01 (1) 7.81 N-M 0 / 1913 -18.5 -18.5 0.16 (1) 10.00 M-L 0 / 2026 -18.5 -18.5 0.22 (1) 10.00 L-K 0 / 2886 -18.5 -18.5 0.55 (1) 10.00 K-Q 0 / 2886 -18.5 -18.5 0.55 (1) 10.00 Q-J 0 / 2886 -18.5 -18.5 0.55 (1) 10.00 J-R 0 / 0 -18.5 -18.5 0.46 (1) 10.00 R-S 0 / 0 -18.5 -18.5 0.46 (1) 10.00 S-I 0 / 0 -18.5 -18.5 0.46 (1) 10.00 I-H 0 / 0 -18.5 -18.5 0.19 (1) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN. G 19-10-0 -49 -49 --- BACK VERT TOTAL --- C1 I 19-6-12 -189 -189 --- BACK VERT TOTAL --- C1 O 15-6-12 -17 -17 --- BACK VERT TOTAL --- C1 P 17-6-12 -17 -17 --- BACK VERT TOTAL --- C1 Q 13-7-8 -1126 -1126 --- BACK VERT TOTAL --- C1 R 15-6-12 -186 -186 --- BACK VERT TOTAL --- C1 S 17-6-12 -186 -186 --- BACK VERT TOTAL --- C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.2 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 51.6 PSF SPACING = 24.0 IN./C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.67") CALCULATED VERT. DEFL.(LL)= L/999 (0.07") ALLOWABLE DEFL.(TL)= L/360 (0.67") CALCULATED VERT. DEFL.(TL)= L/999 (0.12") CSI: TC=0.77/1.00 (G-I-1), BC=0.55/1.00 (J-L-1), WB=0.52/1.00 (G-J-1), SSI=0.56/1.00 (H-I-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (J) (INPUT = 0.90) JSI METAL= 0.52 (J) (INPUT = 1.00) LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2 CONTINUED ON PAGE 2			
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STRUCTURAL COMPONENT ONLY
DWG # TR23040048



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T43	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

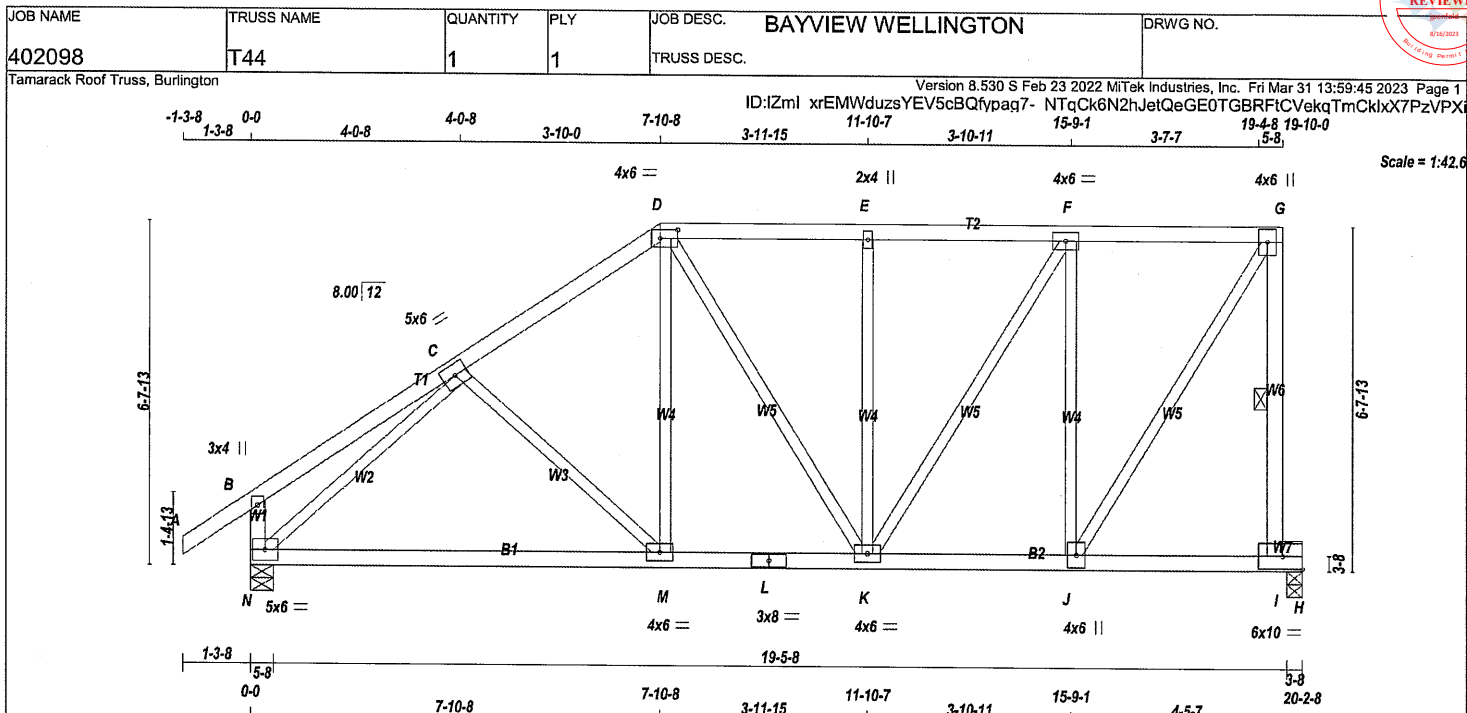
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:45 2023 Page 2
ID:IZml_xrEMWduzsYEV5cBQfypag7- NTqCk6N2hJetQeGE0TGBRFmjVhdqY5CklxX7PzVPX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-h	MT20	5.0	6.0	1.50	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.25	2.75
I	BMVW+p	MT20	8.0	9.0	4.50	3.75
J	BMWW+t	MT20	5.0	6.0	3.00	2.25
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040048



LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	5.0	6.0	
D	TTWV-t	MT20	4.0	6.0	2.00 4.00
E	TMVW-t	MT20	2.0	4.0	
F	TMVW-t	MT20	4.0	6.0	
G	TMVW+p	MT20	4.0	6.0	
I	BMVW-t	MT20	6.0	10.0	Edge
J	BMVW-t	MT20	4.0	6.0	
K	BMVW-t	MT20	4.0	6.0	
L	BS-t	MT20	3.0	8.0	
M	BMVW-t	MT20	4.0	6.0	
N	BMVW-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UPLIFT	BRG	BRG	IN-SX
N	1674	0	0	1674	0	0	5-8	5-8	
H	1449	0	0	1449	0	0	3-8	3-8	

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 49	-129.6 -129.6	0.17 (1)	10.00	C-M	-231 / 2	0.12 (1)
B-C	0 / 32	-129.6 -129.6	0.32 (1)	10.00	M-D	0 / 319	0.07 (1)
C-D	-1412 / 0	-129.6 -129.6	0.36 (1)	5.05	N-C	-1808 / 0	0.86 (1)
D-E	-1174 / 0	-129.6 -129.6	0.28 (1)	5.53	J-G	0 / 1621	0.36 (1)
E-F	-1174 / 0	-129.6 -129.6	0.29 (1)	5.51	D-K	-26 / 42	0.03 (4)
F-G	-875 / 0	-129.6 -129.6	0.29 (1)	6.16	J-F	-1057 / 0	0.77 (1)
I-G	-1580 / 0	0.0 0.0	0.32 (1)	5.25	K-E	-551 / 0	0.40 (1)
N-B	-374 / 0	0.0 0.0	0.04 (1)	7.81	K-F	0 / 573	0.13 (1)

N-M	0 / 1316	-18.5 -18.5	0.36 (1)	10.00
M-L	0 / 1151	-18.5 -18.5	0.33 (4)	10.00
L-K	0 / 1151	-18.5 -18.5	0.33 (4)	10.00
K-J	0 / 875	-18.5 -18.5	0.38 (1)	10.00
J-I	0 / 0	-18.5 -18.5	0.74 (1)	10.00
I-H	0 / 0	-18.5 -18.5	0.31 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.36/1.00 (C-D-1), BC=0.74/1.00 (I-J-1), WB=0.86/1.00 (C-N-1), SSI=0.57/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

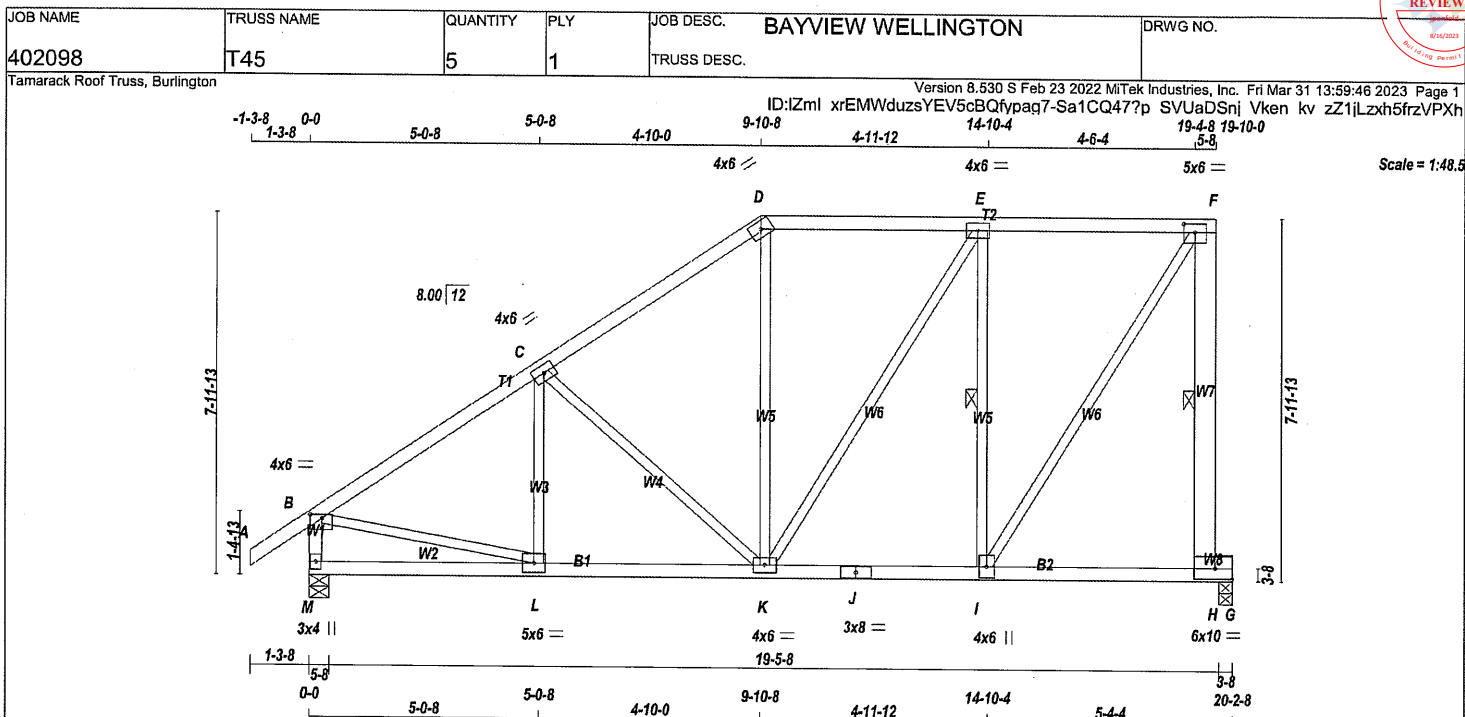
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.39 (J) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

STRUCTURAL COMPONENT ONLY
DWG # TR23040049



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
H - F	2x6	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	4.0	6.0	1.00 3.25
C	TMVW-t	MT20	4.0	6.0	
D	TTW-h	MT20	4.0	6.0	
E	TMVW-t	MT20	4.0	6.0	
F	TMVW-t	MT20	5.0	6.0	2.25 3.00
H	BMVW-t	MT20	6.0	10.0	2.75 Edge
I	BMVW-t	MT20	4.0	6.0	
J	BS-t	MT20	3.0	8.0	
K	BMVW-t	MT20	4.0	6.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	IN-SX	BRG	IN-SX
M	1674	0	0	1674	0	0	5-8	5-8	
G	1449	0	0	1449	0	0	3-8	3-8	

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0 / 49	-129.6	-129.6 0.17 (1)	10.00	L-C	-181 / 46	0.06 (1)
B-C	-1605 / 0	-129.6	-129.6 0.63 (1)	4.39	C-K	-529 / 0	0.43 (1)
C-D	-1220 / 0	-129.6	-129.6 0.59 (1)	4.93	K-D	0 / 182	0.04 (1)
D-E	-979 / 0	-129.6	-129.6 0.56 (1)	5.37	K-E	0 / 244	0.05 (1)
E-F	-851 / 0	-129.6	-129.6 0.56 (1)	5.68	I-E	-1005 / 0	0.40 (1)
H-F	-1538 / 0	0.0	0.0 0.28 (1)	6.25	I-F	0 / 1540	0.35 (1)
M-B	-1636 / 0	0.0	0.0 0.17 (1)	6.47	B-L	0 / 1402	0.32 (1)
M-L	0 / 0	-18.5	-18.5 0.11 (4)	10.00			
L-K	0 / 1371	-18.5	-18.5 0.28 (1)	10.00			
K-J	0 / 851	-18.5	-18.5 0.40 (1)	10.00			
J-I	0 / 851	-18.5	-18.5 0.40 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.74 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.31 (1)	10.00			

TOTAL WEIGHT = 5 X 103 = 517 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL)= L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL)= L/999 (0.24")

CSI: TC=0.63/1.00 (B-C:1), BC=0.74/1.00 (H-I:1), WB=0.43/1.00 (C-K:1), SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

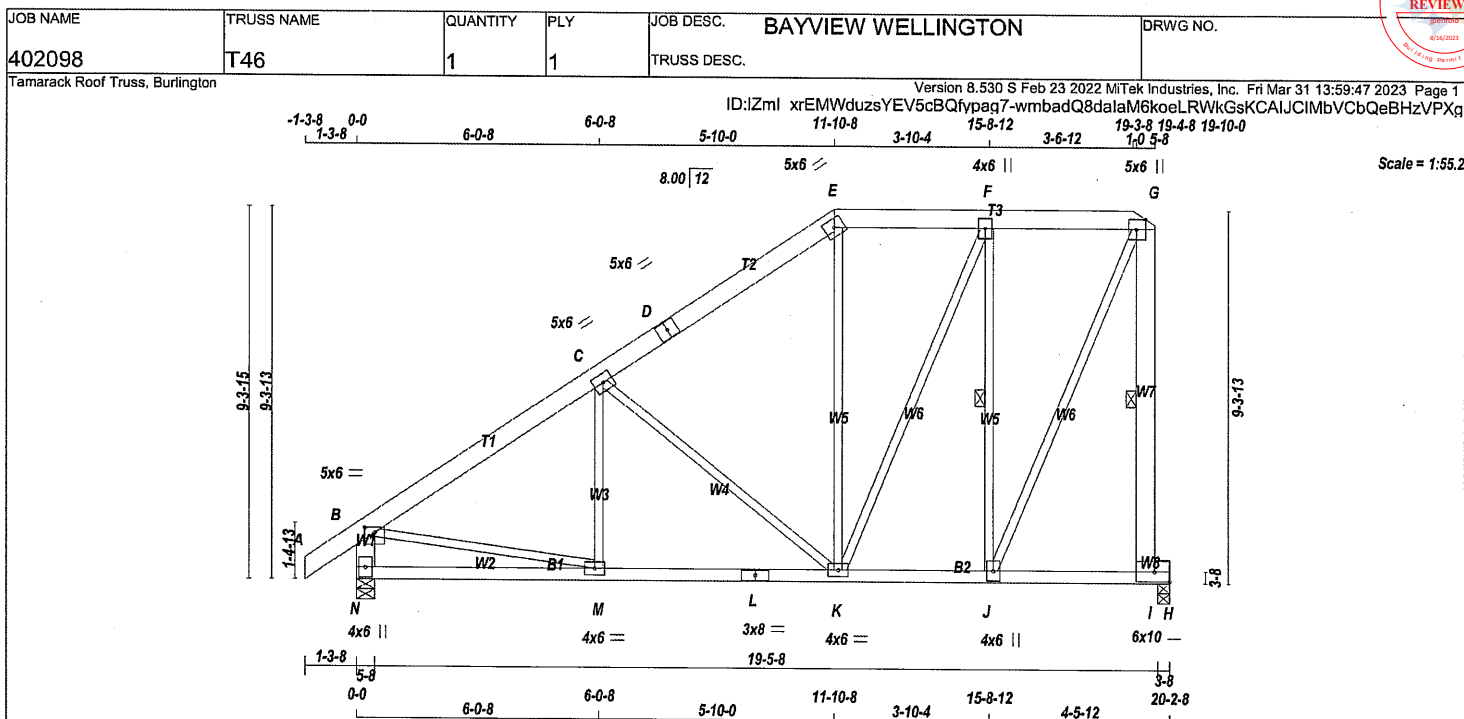
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040050



TOTAL WEIGHT = 127 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TTW-h	MT20	5.0	6.0		
F	TMVW+t	MT20	4.0	6.0		
G	TMVW+p	MT20	5.0	6.0	Edge	2.75
I	BMVW-t	MT20	6.0	10.0	2.75	Edge
J	BMVW+t	MT20	4.0	6.0		
K	BMVW+t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1680	0	1680	0	5-8	5-8
H	1449	0	1449	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1168	880 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 51	-129.6 -129.6	0.09 (1)	10.00	M-C	-90 / 90	0.04 (1)
B-C	-1616 / 0	-129.6 -129.6	0.38 (1)	5.68	C-K	-746 / 0	0.90 (1)
C-D	-1031 / 0	-129.6 -129.6	0.37 (1)	6.25	K-E	0 / 72	0.03 (4)
D-E	-1031 / 0	-129.6 -129.6	0.37 (1)	6.25	B-M	0 / 1403	0.32 (1)
E-F	-815 / 0	-129.6 -129.6	0.12 (1)	6.25	J-G	0 / 1491	0.34 (1)
F-G	-622 / 0	-129.6 -129.6	0.13 (1)	6.25	J-F	-1062 / 0	0.58 (1)
N-B	-1636 / 0	0.0 0.0	0.11 (1)	7.40	K-F	0 / 500	0.11 (1)
I-G	-1578 / 0	0.0 0.0	0.39 (1)	6.01			
N-M	0 / 0	-18.5 -18.5	0.17 (4)	10.00			
M-L	0 / 1385	-18.5 -18.5	0.32 (1)	10.00			
L-K	0 / 1385	-18.5 -18.5	0.32 (1)	10.00			
K-J	0 / 622	-18.5 -18.5	0.33 (1)	10.00			
J-I	0 / 0	-18.5 -18.5	0.74 (1)	10.00			
I-H	0 / 0	-18.5 -18.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.39/1.00 (G-I:1), BC=0.74/1.00 (I-J:1), WB=0.90/0.90 (C-K:1), SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)

JSI METAL= 0.53 (L) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



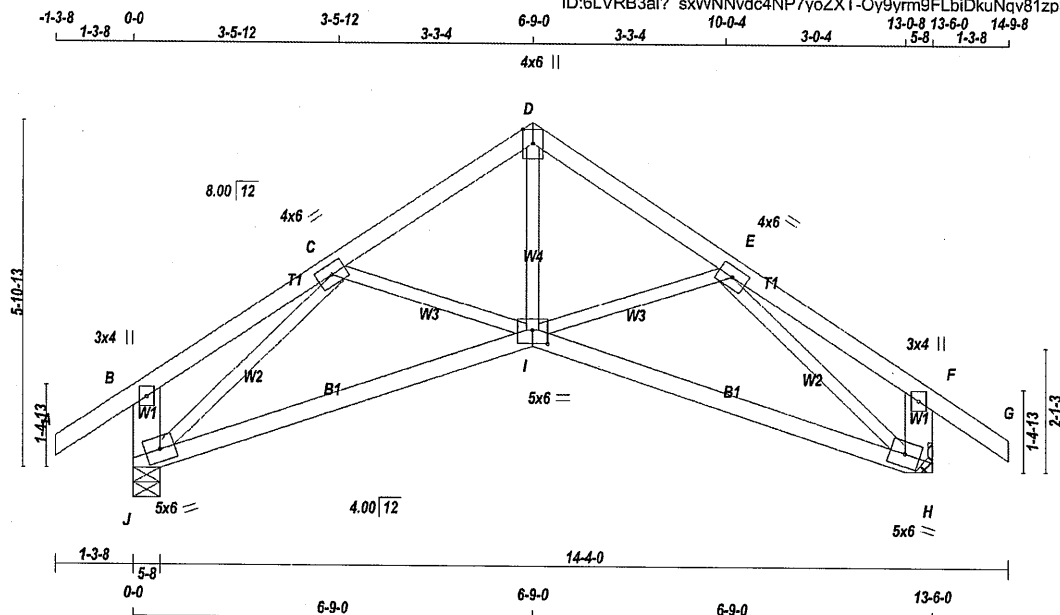
STRUCTURAL COMPONENT ONLY
DWG # TR23040051

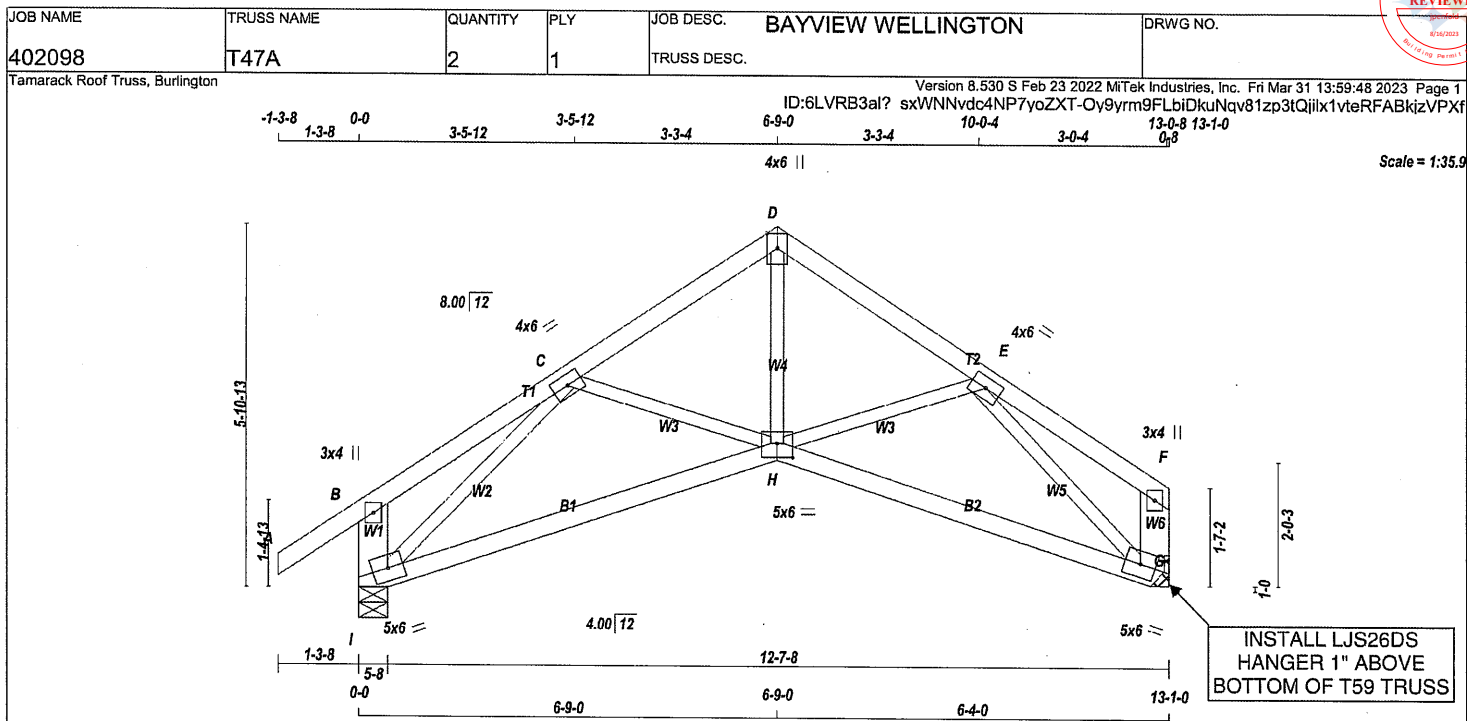


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T47	4	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER				DESCR.	
N. L. G. A. RULES				BUILDING DESIGNER	
CHORDS	SIZE	LUMBER		BEARINGS	
A - D	2x4	DRY	No.2	FACTORED	MAXIMUM FACTORED
D - F	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION
I - B	2x6	DRY	No.2	VERT	HORZ
G - F	2x6	DRY	No.2	DOWN	UPLIFT
I - H	2x4	DRY	No.2	IN-SX	IN-SX
H - G	2x4	DRY	No.2	MECHANICAL	
ALL WEBS	2x3	DRY	No.2	A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWV-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMWV-t	MT20	5.0	6.0		
H	BMWV-p	MT20	5.0	6.0	2.75	3.00
I	BMWV-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
I	797	605 / 0	0 / 0
G	675	500 / 0	0 / 0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	797	605 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
G	675	500 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 49	-129.6	-129.6 0.17 (1)	H-D	0 / 857
B-C	0 / 26	-129.6	-129.6 0.21 (1)	H-E	-55 / 31
C-D	-1120 / 0	-129.6	-129.6 0.18 (1)	C-H	-146 / 14
D-E	-1117 / 0	-129.6	-129.6 0.17 (1)	I-C	-1512 / 0
E-F	0 / 25	-129.6	-129.6 0.19 (1)	E-G	-1450 / 0
I-B	-345 / 0	0.0	0.0 0.02 (1)		
G-F	-147 / 0	0.0	0.0 0.01 (1)		
I-H	0 / 1106	-18.5	-18.5 0.32 (1)		
H-G	0 / 1015	-18.5	-18.5 0.29 (1)		

TOTAL WEIGHT = 2 X 55 = 111 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.21/1.00 (B-C:1), BC=0.32/1.00 (H-I:1), WB=0.51/1.00 (C-I:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

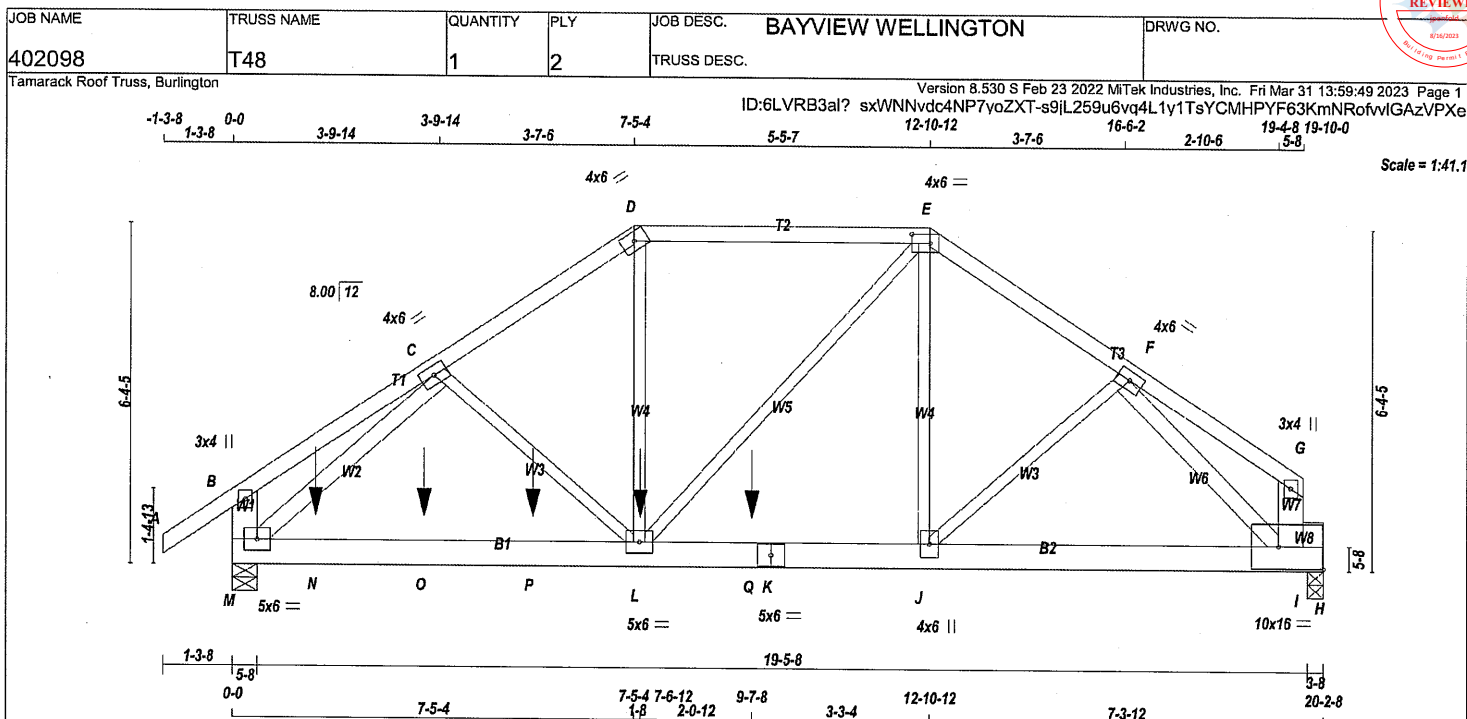
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040053



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x4	DRY	No.2	SPF	SPF
D - E	2x4	DRY	No.2	SPF	SPF
E - G	2x4	DRY	No.2	SPF	SPF
M - B	2x6	DRY	No.2	SPF	SPF
I - G	2x6	DRY	No.2	SPF	SPF
M - K	2x6	DRY	No.2	SPF	SPF
K - H	2x6	DRY	No.2	SPF	SPF
I - H	2x6	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
EXCEPT					
M - C	2x4	DRY	No.2	SPF	SPF
F - I	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-E 1	12	TOP
E-G 1	12	TOP
M-B 2	12	TOP
I-G 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M-K 2	12	SIDE(183.1)
K-H 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
M	3094	0	3094	0	5-8	5-8
H	2226	0	2226	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	UNBRAC			
M	2146	1650 / 0	0 / 0	0 / 0	0 / 0	496 / 0	0 / 0
H	1548	1163 / 0	0 / 0	0 / 0	0 / 0	385 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 49	-129.6 -129.6	0.10 (1)	C-L	0 / 70	0.01 (4)	
B-C	0 / 28	-129.6 -129.6	0.14 (1)	L-D	0 / 1179	0.15 (1)	
C-D	-3122 / 0	-129.6 -129.6	0.19 (1)	L-E	0 / 565	0.07 (1)	
D-E	-2590 / 0	-129.6 -129.6	0.42 (1)	J-E	0 / 508	0.06 (1)	
E-F	-2679 / 0	-129.6 -129.6	0.16 (1)	J-F	0 / 241	0.03 (1)	
F-G	0 / 30	-129.6 -129.6	0.13 (1)	M-C	-3447 / 0	0.49 (1)	
M-B	-368 / 0	0.0 0.0	0.01 (1)	F-I	-2982 / 0	0.37 (1)	
I-G	-155 / 0	0.0 0.0	0.01 (1)				
M-N	0 / 2518	-18.5 -18.5	0.50 (1)				
N-O	0 / 2518	-18.5 -18.5	0.50 (1)				
O-P	0 / 2518	-18.5 -18.5	0.50 (1)				
P-L	0 / 2518	-18.5 -18.5	0.50 (1)				
L-Q	0 / 2213	-18.5 -18.5	0.48 (1)				
Q-K	0 / 2213	-18.5 -18.5	0.48 (1)				
K-J	0 / 2213	-18.5 -18.5	0.48 (1)				
J-I	0 / 2026	-18.5 -18.5	0.44 (1)				
I-H	0 / 0	-18.5 -18.5	0.13 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	7-6-12	-185	-185	BACK	VERT	TOTAL	---	C1
N	1-6-12	-185	-185	BACK	VERT	TOTAL	---	C1
O	3-6-12	-185	-185	BACK	VERT	TOTAL	---	C1
P	5-6-12	-185	-185	BACK	VERT	TOTAL	---	C1
Q	9-7-8	-778	-778	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.42/1.00 (D-E:1), BC=0.50/1.00 (L-M:1), WB=0.49/1.00 (C-M:1), SSI=0.37/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

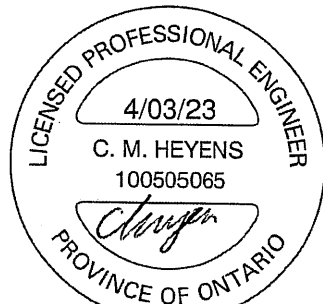
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.47 (K) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR23040054



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T48	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TTWW-l	MT20	4.0	6.0	2.00	4.00
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVWW-l	MT20	10.0	16.0	5.00	Edge
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

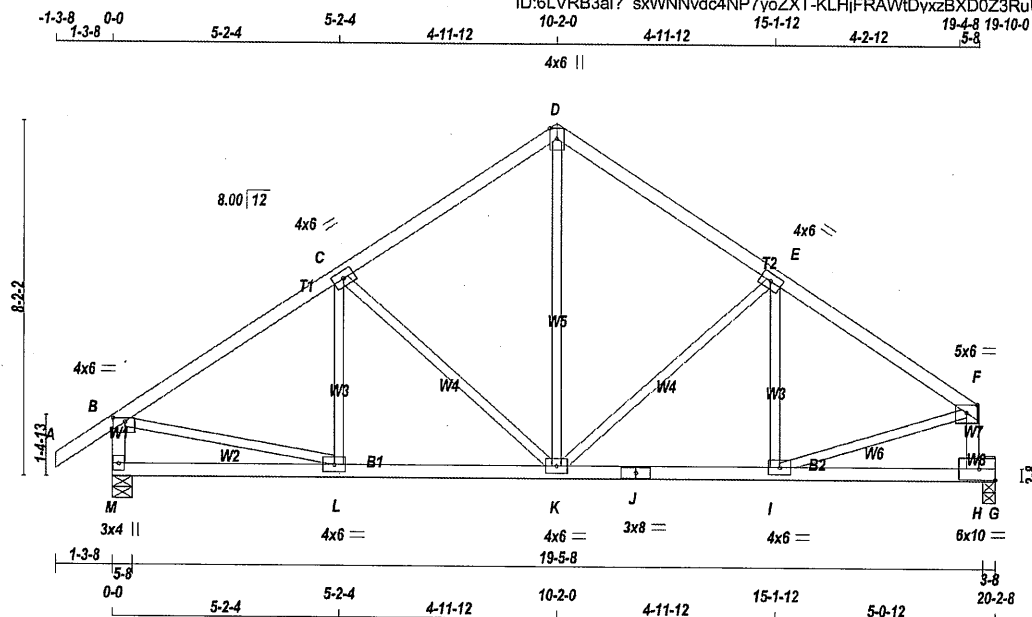


STRUCTURAL COMPONENT ONLY
DWG # TR23040054



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T49	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:6LVRB3al? sxWNNvdc4NP7yoZXT-KLHjFRAWtDyxzBXD0Z3RuUyf7WLvVq9xuZfloczVPXd



Scale = 1:50.8

TOTAL WEIGHT = 87 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMVW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMVW-t	MT20	6.0	10.0	3.00	4.50
I	BMVW-t	MT20	4.0	6.0		
J	BS-l	MT20	3.0	8.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
M	1674	0	1674	0	5-8	5-8
G	1449	0	1449	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 49	-129.6	-129.6 0.17 (1)	10.00	L-C	-174 / 49	0.06 (1)
B-C	-1603 / 0	-129.6	-129.6 0.67 (1)	4.32	C-K	-552 / 0	0.46 (1)
C-D	-1201 / 0	-129.6	-129.6 0.62 (1)	4.90	K-D	0 / 791	0.18 (1)
D-E	-1196 / 0	-129.6	-129.6 0.56 (1)	5.03	K-E	-548 / 0	0.46 (1)
E-F	-1600 / 0	-129.6	-129.6 0.59 (1)	4.45	I-E	-137 / 54	0.05 (1)
M-B	-1635 / 0	0.0	0.0 0.17 (1)	6.48	B-L	0 / 1399	0.31 (1)
H-F	-1550 / 0	0.0	0.0 0.17 (1)	6.61	I-F	0 / 1428	0.32 (1)
M-L	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
L-K	0 / 1370	-18.5	-18.5 0.28 (1)	10.00			
K-J	0 / 1367	-18.5	-18.5 0.45 (1)	10.00			
J-I	0 / 1367	-18.5	-18.5 0.45 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.74 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.67/1.00 (B-C:1), BC=0.74/1.00 (H-I:1), WB=0.46/1.00 (C-K:1), SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

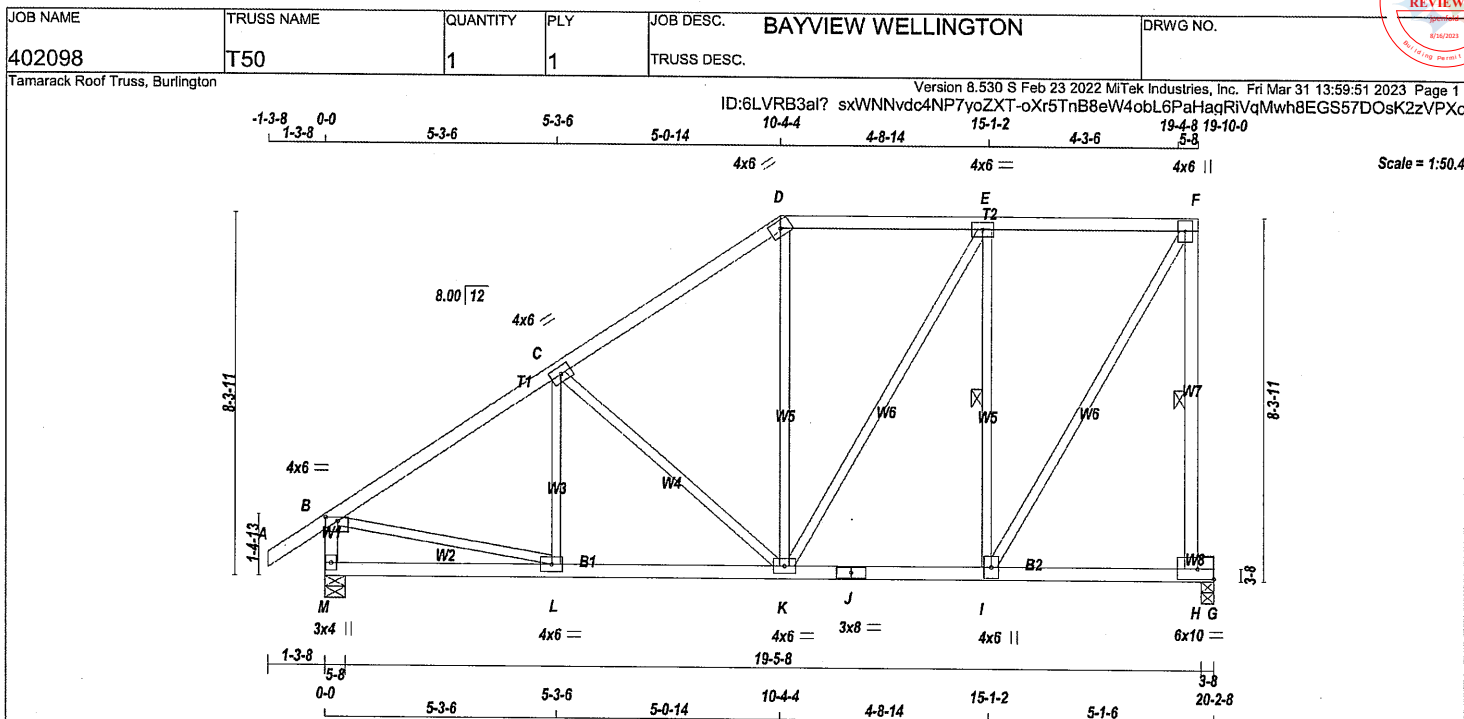
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.40 (J) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

STRUCTURAL COMPONENT ONLY
DWG # TR23040055



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	4.0	6.0 1.00 3.25
C	TMVW-t	MT20	4.0	6.0
D	TTW-h	MT20	4.0	6.0
E	TMVW-t	MT20	4.0	6.0
F	TMVW+p	MT20	4.0	6.0
H	BMVW-t	MT20	6.0	10.0 2.75 Edge
I	BMVW-t	MT20	4.0	6.0
J	BS-t	MT20	3.0	8.0
K	BMVW-t	MT20	4.0	6.0
L	BMVW-t	MT20	4.0	6.0
M	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	1674	0	1674	0	0	5-8	5-8	5-8	5-8
G	1449	0	1449	0	0	3-8	3-8	3-8	3-8

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FR-TO	LENGTH	FR-TO			
A-B	0 / 49	-129.6	-129.6 0.17 (1)	10.00	L-C	-163 / 56	0.06 (1)
B-C	-1603 / 0	-129.6	-129.6 0.70 (1)	4.26	C-K	-580 / 0	0.52 (1)
C-D	-1175 / 0	-129.6	-129.6 0.65 (1)	4.87	K-D	0 / 148	0.03 (4)
D-E	-939 / 0	-129.6	-129.6 0.50 (1)	5.59	K-E	0 / 316	0.07 (1)
E-F	-782 / 0	-129.6	-129.6 0.50 (1)	6.00	I-E	-1032 / 0	0.45 (1)
H-F	-1549 / 0	0.0	0.0 0.48 (1)	5.29	I-F	0 / 1527	0.34 (1)
M-B	-1635 / 0	0.0	0.0 0.17 (1)	6.48	B-L	0 / 1399	0.31 (1)
M-L	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
L-K	0 / 1371	-18.5	-18.5 0.29 (1)	10.00			
K-J	0 / 782	-18.5	-18.5 0.37 (1)	10.00			
J-I	0 / 782	-18.5	-18.5 0.37 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.74 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.31 (1)	10.00			

TOTAL WEIGHT = 100 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL)= L/999 (0.22")

CSI: TC=0.70/1.00 (B-C:1), BC=0.74/1.00 (H-I:1), WB=0.52/1.00 (C-K:1), SI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

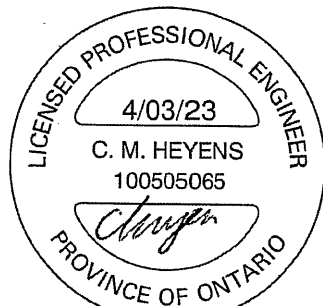
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1747	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

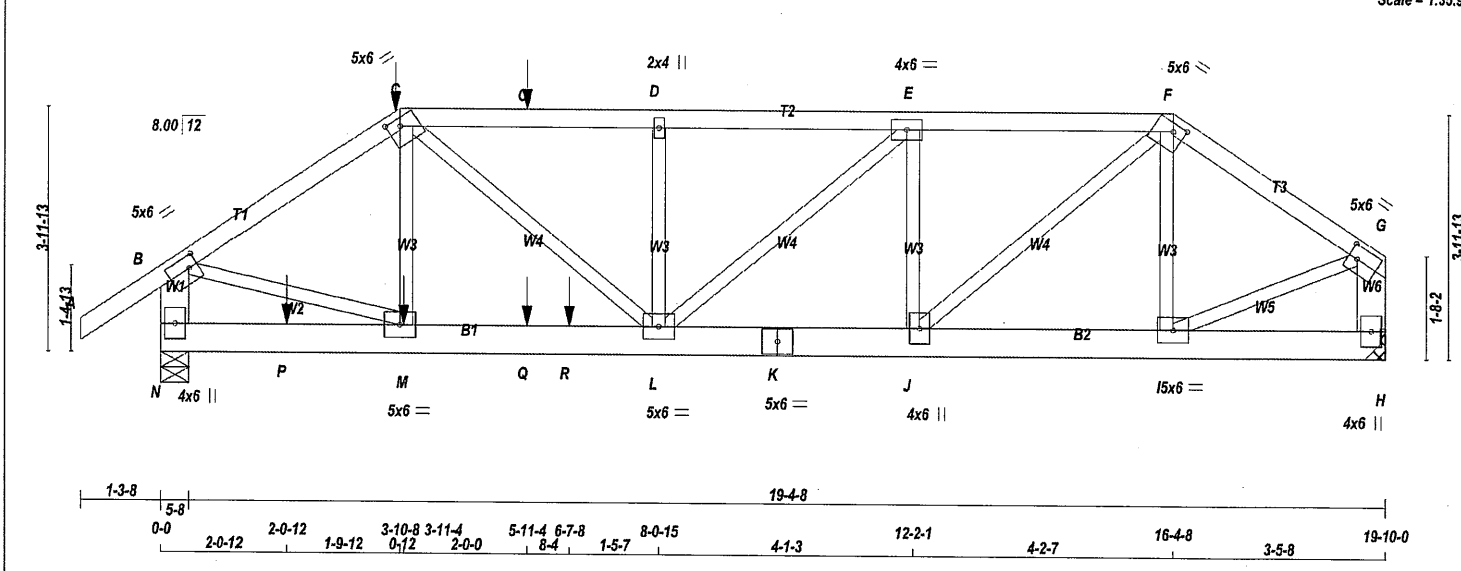


STRUCTURAL COMPONENT ONLY
DWG # TR23040056



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T51A	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:6LVRB3al? sxWNNvdc4NP7yoZXT-GkOTg7CmPqCfCvHc8 5vzv14jU6lznEELt8PtVzVPXb
Scale = 1:35.9



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. ALL WEBS EXCEPT 2x3 DRY No.2 SPF

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
N - B	2x4	DRY No.2	SPF
H - G	2x6	DRY No.2	SPF
N - K	2x6	DRY No.2	SPF
K - H	2x6	DRY No.2	SPF

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(61.0)
C - F 1	12	SIDE(61.0)
F - G 1	12	TOP
N - B 2	12	TOP
H - G 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N - K 2	12	SIDE(183.1)
K - H 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT 2566	DOWN 2566	0	5-8
H	VERT 1859	DOWN 1859	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4'-0".

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
N	1782 1350 / 0 0 / 0 0 / 0 432 / 0 0 / 0
H	1294 967 / 0 0 / 0 0 / 0 327 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 49	-129.6 -129.6	0.10 (1)	M-C	-129 / 67	0.02 (5)
B-C	-2811 / 0	-129.6 -129.6	0.22 (1)	I-F	-444 / 0	0.05 (1)
C-O	-3341 / 0	-129.6 -129.6	0.28 (1)	B-M	0 / 2407	0.30 (1)
O-D	-3341 / 0	-129.6 -129.6	0.28 (1)	I-G	0 / 1668	0.21 (1)
D-E	-3341 / 0	-129.6 -129.6	0.23 (1)	J-F	0 / 1598	0.20 (1)
E-F	-2771 / 0	-129.6 -129.6	0.18 (1)	C-L	0 / 1329	0.16 (1)
F-G	-1891 / 0	-129.6 -129.6	0.16 (1)	J-E	-1068 / 0	0.13 (1)
N-B	-2579 / 0	0.0 0.0	0.09 (1)	L-D	-653 / 0	0.08 (1)
H-G	-1839 / 0	0.0 0.0	0.07 (1)	L-E	0 / 759	0.09 (1)
N-P	0 / 0	-18.5 -18.5	0.08 (1)			
P-M	0 / 0	-18.5 -18.5	0.08 (1)			
M-Q	0 / 2331	-18.5 -18.5	0.39 (1)			
Q-R	0 / 2331	-18.5 -18.5	0.39 (1)			
R-L	0 / 2331	-18.5 -18.5	0.39 (1)			
L-K	0 / 2771	-18.5 -18.5	0.25 (1)			
K-J	0 / 2771	-18.5 -18.5	0.25 (1)			
J-I	0 / 1557	-18.5 -18.5	0.14 (1)			
I-H	0 / 0	-18.5 -18.5	0.02 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	3-10-8	-190	-190	122	BACK	VERT	TOTAL	—	C1
M	3-11-4	-14	-14	—	BACK	VERT	TOTAL	—	C1
O	5-11-4	-83	-83	—	BACK	VERT	TOTAL	—	C1
P	2-0-12	-6	-6	—	BACK	VERT	TOTAL	—	C1
Q	5-11-4	-14	-14	—	BACK	VERT	TOTAL	—	C1
R	6-7-8	-601	-601	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH.	LL	DL
	LL	= 38.2 PSF	DL = 6.0 PSF
	LL	= 0.0 PSF	DL = 7.4 PSF
	TOTAL LOAD	= 51.6 PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.28/1.00 (C-D:1), BC=0.39/1.00 (L-M:1), WB=0.30/1.00 (B-M:1), SSI=0.20/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR23040057



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T51A	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:52 2023 Page 2
ID:6LVRB3al? sxWNNvdc4NP7yoZXT-GkOTg7CmPqCfCVhc8 5vzv14jJ6lznEELt8PtVzVPXb

PLATES (table is in inches)

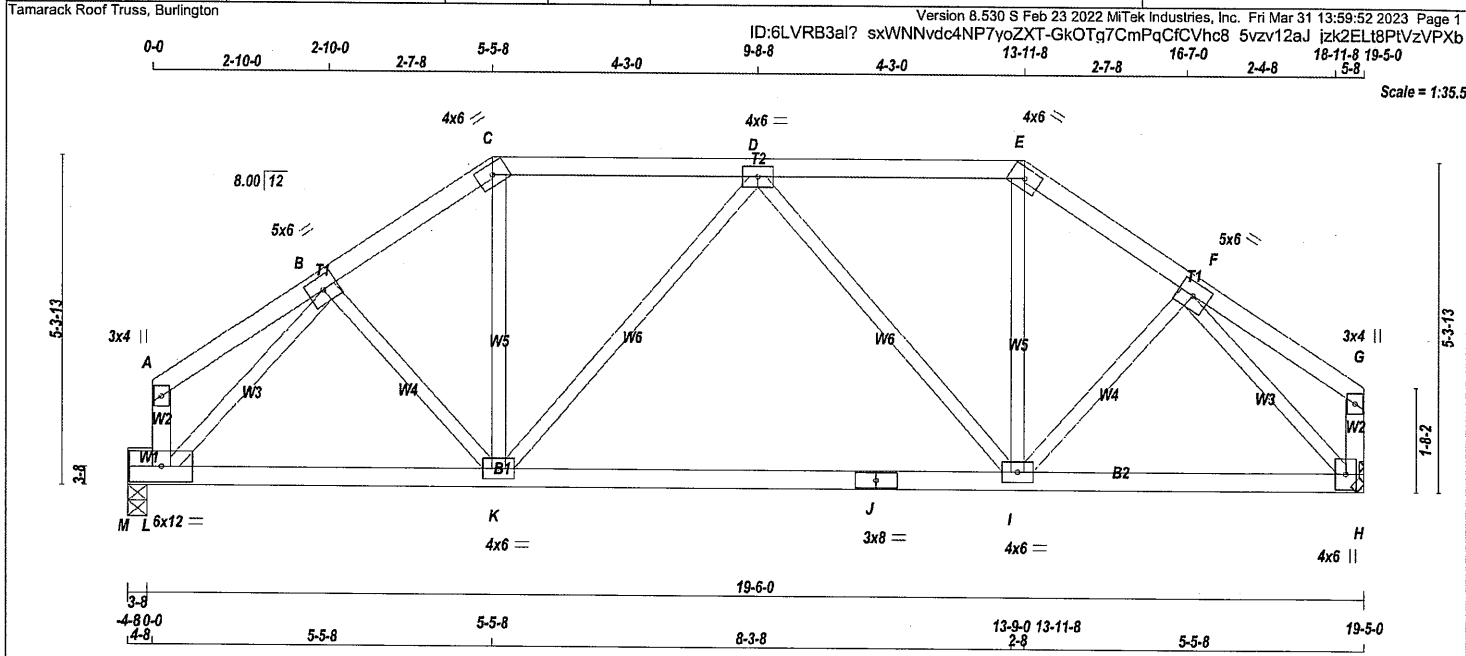
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	1.75
C	TTWW-h	MT20	5.0	6.0	1.50	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMWV-t	MT20	4.0	6.0		
F	TTWW-h	MT20	5.0	6.0	1.50	2.25
G	TMVW-t	MT20	5.0	6.0	2.50	1.75
H	BMV1+p	MT20	4.0	6.0		
I	BMWV-t	MT20	5.0	6.0		
J	BMWV+t	MT20	4.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWV-t	MT20	5.0	6.0		
N	BMV1+p	MT20	4.0	6.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040057



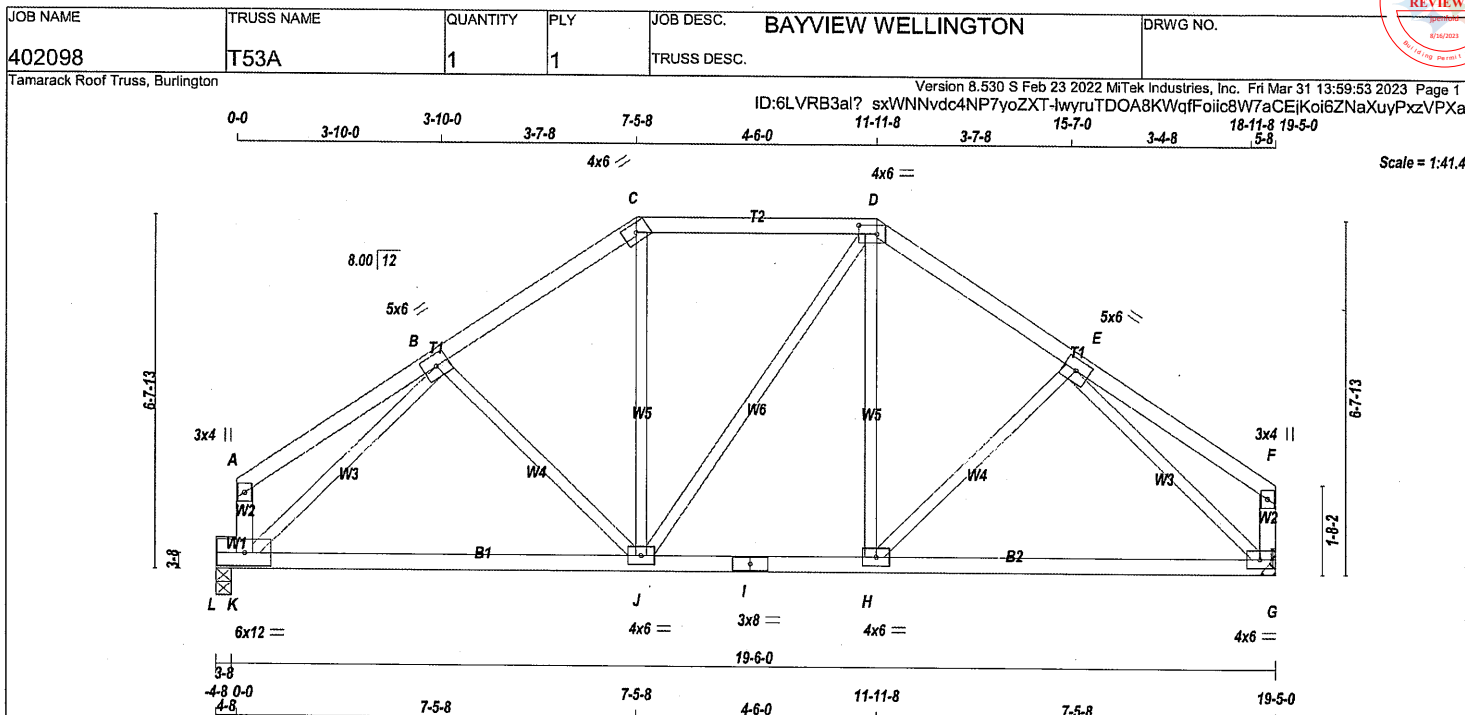
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T52A	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - E 2x4 DRY No.2 SPF E - G 2x4 DRY No.2 SPF L - A 2x4 DRY No.2 SPF H - G 2x4 DRY No.2 SPF M - J 2x4 DRY No.2 SPF M - L 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX M 1418 0 1418 0 0 3-8 3-8 H 1466 0 1466 0 0 MECHANICAL A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8. UNFACTORED REACTIONS 1ST LCASE MAX. MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL H 989 728 / 0 0 / 0 0 / 0 261 / 0 0 / 0 H 1021 756 / 0 0 / 0 0 / 0 265 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) UNBRACED LENGTH FR-TO A-B 0 / 19 -129.6 -129.6 0.13 (1) 10.00 B-K 0 / 126 0.03 (1) B-C -1514 / 0 -129.6 -129.6 0.15 (1) 5.18 K-C 0 / 475 0.11 (1) C-D -1251 / 0 -129.6 -129.6 0.41 (1) 5.18 I-E 0 / 426 0.10 (1) D-E -1175 / 0 -129.6 -129.6 0.41 (1) 5.31 I-F 0 / 101 0.02 (4) E-F -1425 / 0 -129.6 -129.6 0.15 (1) 5.30 L-B -1777 / 0 0.50 (1) F-G 0 / 19 -129.6 -129.6 0.13 (1) 10.00 F-H -1689 / 0 0.48 (1) L-A -142 / 0 0.0 0.0 0.02 (1) 7.81 K-D -377 / 0 0.29 (1) H-G -142 / 0 0.0 0.0 0.02 (1) 7.81 D-I -495 / 0 0.39 (1) WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) UNBRACED LENGTH FR-TO M-L 0 / 0 -18.5 -18.5 0.30 (1) 10.00 L-K 0 / 1157 -18.5 -18.5 0.91 (1) 10.00 K-J 0 / 1491 -18.5 -18.5 0.52 (1) 10.00 J-I 0 / 1491 -18.5 -18.5 0.52 (1) 10.00 I-H 0 / 1099 -18.5 -18.5 0.27 (4) 10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.2 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 51.6 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.66") CALCULATED VERT. DEFL.(LL)= L/999 (0.16") ALLOWABLE DEFL.(TL)= L/360 (0.66") CALCULATED VERT. DEFL.(TL)= L/864 (0.28") CSI: TC=0.41/1.00 (C-D:1), BC=0.91/1.00 (K-L:1), WB=0.50/1.00 (B-L:1), SSI=0.55/1.00 (L-M:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (H) (INPUT = 0.90) JSI METAL= 0.50 (J) (INPUT = 1.00) LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2			
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STRUCTURAL COMPONENT ONLY
DWG # TR23040058



TOTAL WEIGHT = 84 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVtp	MT20	3.0	4.0		
B	TMVW-t	MT20	5.0	6.0		
C	TTW-h	MT20	4.0	6.0		
D	TTW-t	MT20	4.0	6.0	2.00	4.00
E	TMVW-t	MT20	5.0	6.0		
F	TMVtp	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	6.0	12.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	1418	0	1418	0	3-8	3-8
G	1486	0	1486	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	989	728 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
G	1021	756 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 30	-129.6 -129.6	0.28 (1)	10.00	B-J	-129 / 22	0.06 (1)
B-C	-1388 / 0	-129.6 -129.6	0.32 (1)	5.14	J-C	0 / 277	0.06 (1)
C-D	-1132 / 0	-129.6 -129.6	0.48 (1)	5.25	J-D	0 / 92	0.02 (1)
D-E	-1328 / 0	-129.6 -129.6	0.32 (1)	5.23	H-D	0 / 168	0.04 (1)
E-F	0 / 31	-129.6 -129.6	0.28 (1)	10.00	H-E	-142 / 19	0.07 (1)
K-A	-186 / 0	0.0 0.0	0.02 (1)	7.81	K-B	-1752 / 0	0.80 (1)
G-F	-186 / 0	0.0 0.0	0.02 (1)	7.81	E-G	-1693 / 0	0.78 (1)
L-K	0 / 0	-18.5 -18.5	0.30 (1)	10.00			
K-J	0 / 1219	-18.5 -18.5	0.92 (1)	10.00			
J-I	0 / 1080	-18.5 -18.5	0.51 (1)	10.00			
I-H	0 / 1080	-18.5 -18.5	0.51 (1)	10.00			
H-G	0 / 1178	-18.5 -18.5	0.34 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 486 (0.49")

CS1: TC=0.48/1.00 (C-D:1), BC=0.92/1.00 (J-K:1), WB=0.80/1.00 (B-K:1), SSI=0.55/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

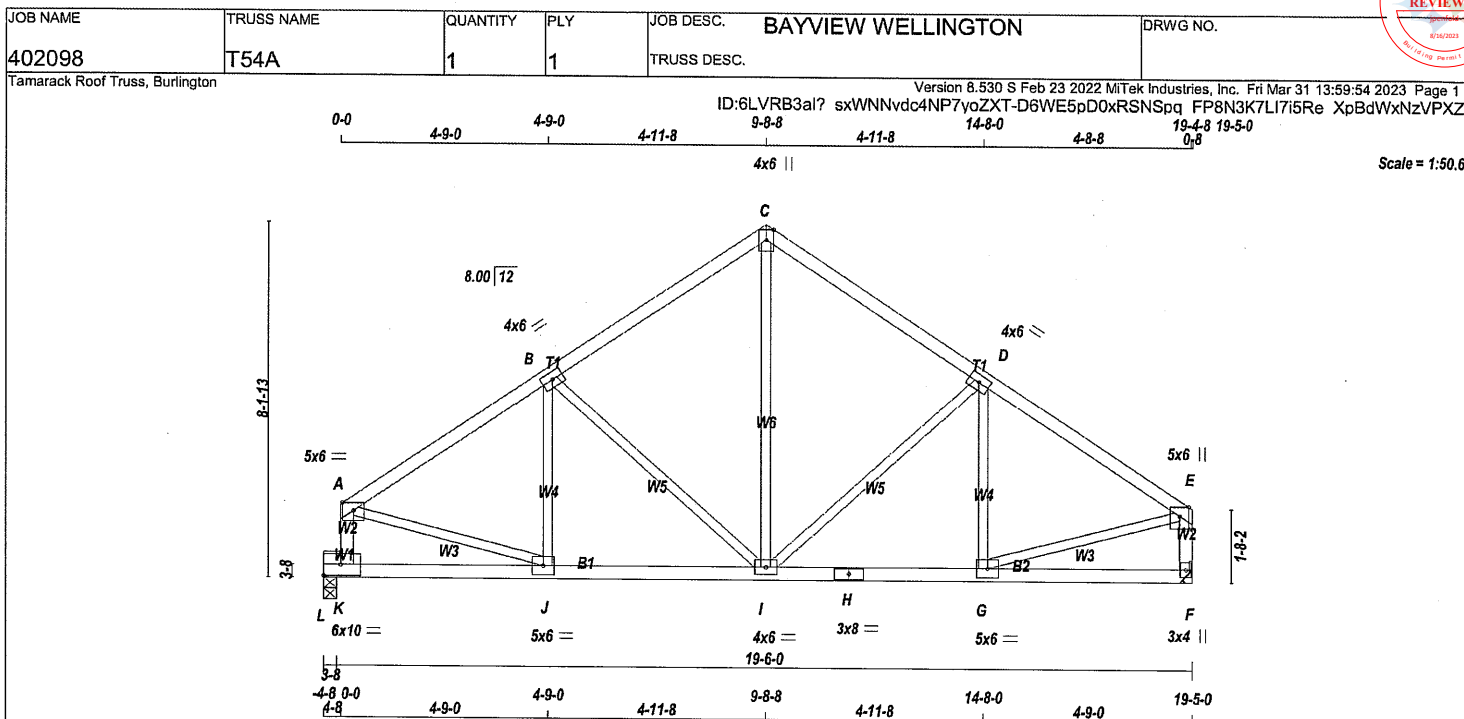
JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.60 (I) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040059





LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
K - A	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
L - K	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	6.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-t	MT20	5.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	6.0	10.0	3.00	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
L	1418	0	1418	0	0	3-8	3-8	
F	1466	0	1466	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
L	989	728 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
F	1021	756 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-1576 / 0	-129.6	-129.6 0.59 (1)	4.47	J-B	-115 / 60	0.04 (1)
B-C	-1153 / 0	-129.6	-129.6 0.56 (1)	5.10	B-I	-570 / 0	0.47 (1)
C-D	-1153 / 0	-129.6	-129.6 0.56 (1)	5.09	I-C	0 / 739	0.17 (1)
D-E	-1470 / 0	-129.6	-129.6 0.58 (1)	4.60	I-D	-452 / 0	0.37 (1)
K-A	-1514 / 0	0.0	0.0 0.16 (1)	6.68	G-D	-254 / 24	0.09 (1)
F-E	-1430 / 0	0.0	0.0 0.15 (1)	6.82	A-J	0 / 1401	0.32 (1)
					G-E	0 / 1310	0.29 (1)
L-K	0 / 0	-18.5	-18.5 0.30 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.72 (1)	10.00			
J-I	0 / 1347	-18.5	-18.5 0.44 (1)	10.00			
I-H	0 / 1259	-18.5	-18.5 0.26 (1)	10.00			
H-G	0 / 1259	-18.5	-18.5 0.26 (1)	10.00			
G-F	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

TOTAL WEIGHT = 84 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.59/1.00 (A-B:1), BC=0.72/1.00 (J-K:1), WB=0.47/1.00 (B-I:1), SSI=0.55/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.60 (E) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040061

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

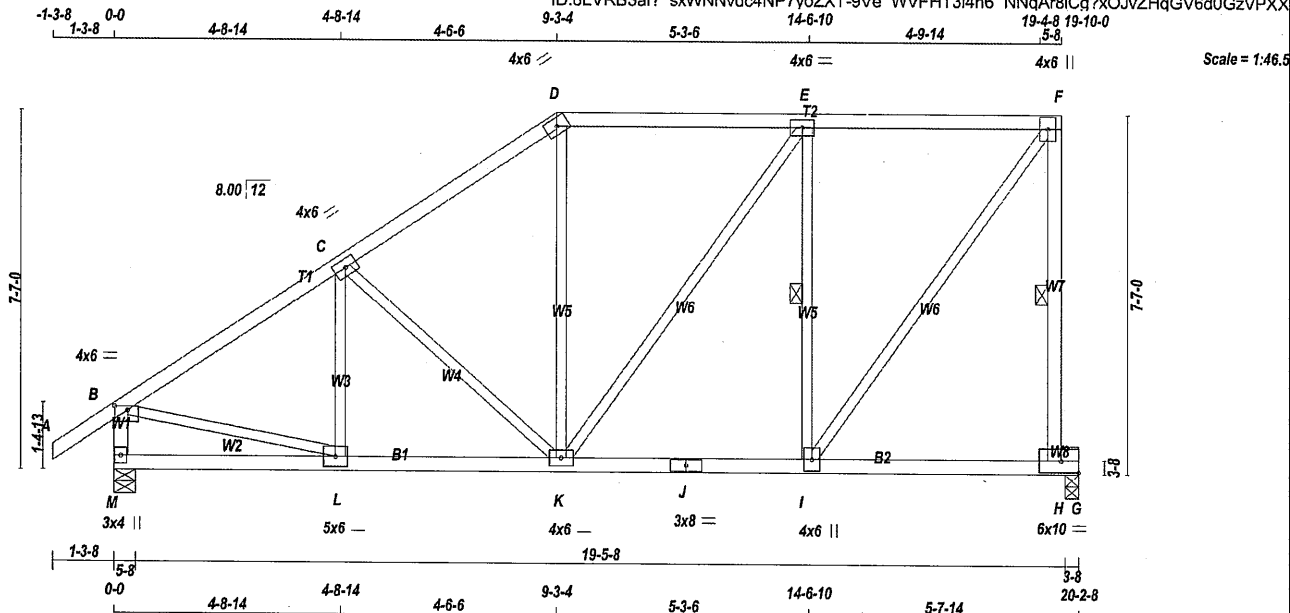


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T55	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID:6LVRB3al? sxWNNvdC4NP7yoZXT-9Ve WVFTHT3i4h6 NNqAr8ICq?xOJvZHqGV6d0GzVPXX



TOTAL WEIGHT = 96 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2

LUMBER			
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMVW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-p	MT20	4.0	6.0		
H	BMVW-t	MT20	6.0	10.0		Edge
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
M	1674	0	1674	0	5-8	5-8
G	1449	0	1449	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
M	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 49	-129.6 -129.6	0.17 (1)	L-C	-206 / 33	0.06 (1)	
B-C	-1605 / 0	-129.6 -129.6	0.54 (1)	C-K	-464 / 0	0.33 (1)	
C-D	-1275 / 0	-129.6 -129.6	0.51 (1)	K-D	0 / 207	0.05 (1)	
D-E	-1028 / 0	-129.6 -129.6	0.64 (1)	K-E	0 / 178	0.04 (1)	
E-F	-925 / 0	-129.6 -129.6	0.64 (1)	I-E	-993 / 0	0.36 (1)	
H-F	-1531 / 0	0.0 0.0	0.40 (1)	I-F	0 / 1569	0.35 (1)	
M-B	-1638 / 0	0.0 0.0	0.17 (1)	B-L	0 / 1403	0.32 (1)	
M-L	0 / 0	-18.5 -18.5	0.10 (4)				
L-K	0 / 1368	-18.5 -18.5	0.28 (1)				
K-J	0 / 925	-18.5 -18.5	0.41 (1)				
J-I	0 / 925	-18.5 -18.5	0.41 (1)				
I-H	0 / 0	-18.5 -18.5	0.74 (1)				
H-G	0 / 0	-18.5 -18.5	0.31 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.2	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/925 (0.26")

CSI: TC=0.64/1.00 (D-E:1), BC=0.74/1.00 (H-E:1), WB=0.36/1.00 (E-I:1), SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

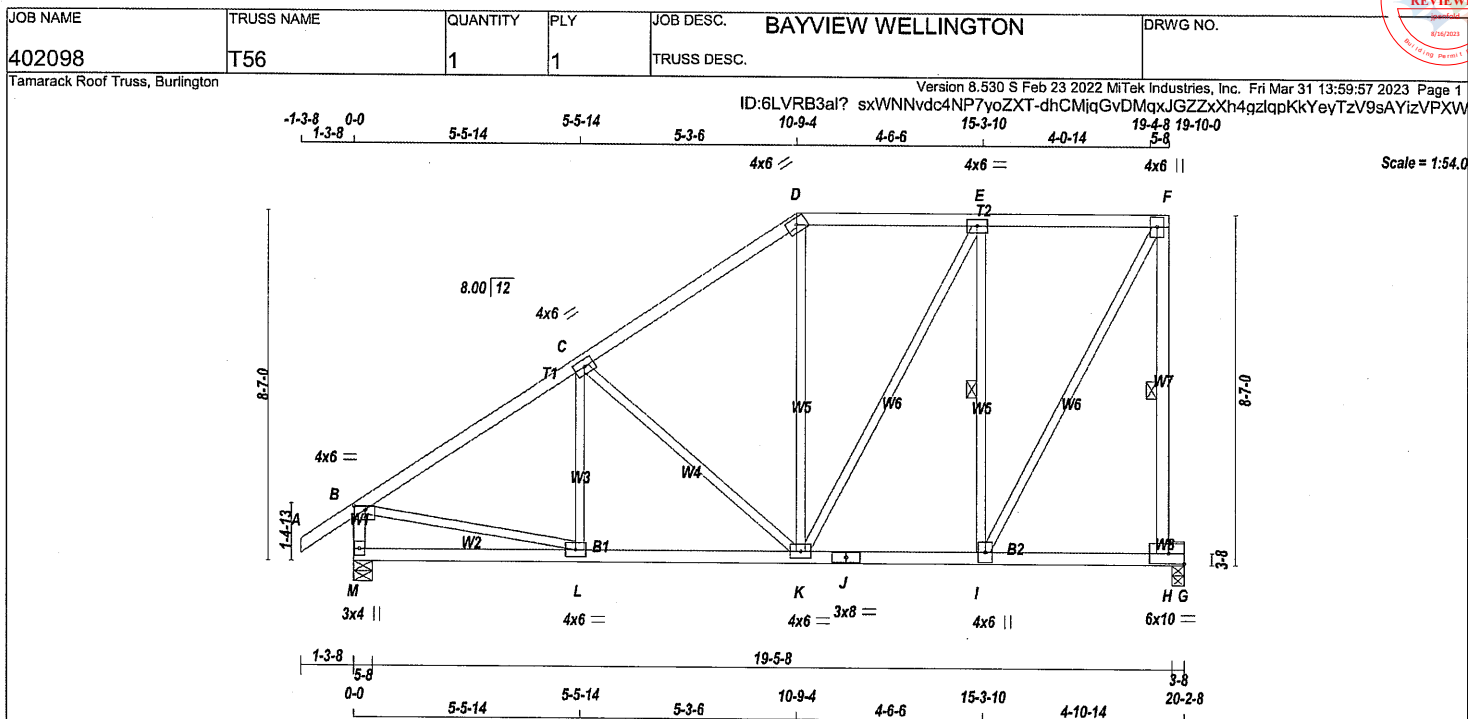
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.42 (I) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040063



TOTAL WEIGHT = 102 lb

LUMBER			LUMBER	DESCR.
N. L. G. A. RULES	CHORDS	SIZE		
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMVW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW-t	MT20	6.0	10.0	3.00	4.50
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
M	1674	0	1674	0	5-8	5-8
G	1449	0	1449	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
M	1164	877 / 0	0 / 0	0 / 0	287 / 0	0 / 0	0 / 0
G	1010	744 / 0	0 / 0	0 / 0	266 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 49	-129.6	-129.6 0.17 (1)	L-C	-148 / 64	0.06 (1)	
B-C	-1600 / 0	-129.6	-129.6 0.77 (1)	C-K	-623 / 0	0.62 (1)	
C-D	-1134 / 0	-129.6	-129.6 0.70 (1)	K-D	0 / 125	0.03 (4)	
D-E	-904 / 0	-129.6	-129.6 0.45 (1)	K-E	0 / 366	0.08 (1)	
E-F	-733 / 0	-129.6	-129.6 0.45 (1)	I-E	-1047 / 0	0.49 (1)	
H-F	-1556 / 0	0.0	0.0 0.52 (1)	I-F	0 / 1518	0.34 (1)	
M-B	-1633 / 0	0.0	0.0 0.17 (1)	B-L	0 / 1396	0.31 (1)	
M-L	0 / 0	-18.5	-18.5 0.14 (4)				
L-K	0 / 1370	-18.5	-18.5 0.29 (1)				
K-J	0 / 733	-18.5	-18.5 0.36 (1)				
J-I	0 / 733	-18.5	-18.5 0.36 (1)				
I-H	0 / 0	-18.5	-18.5 0.74 (1)				
H-G	0 / 0	-18.5	-18.5 0.31 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.77/1.00 (B-C:1), BC=0.74/1.00 (H-I:1), WB=0.62/1.00 (C-K:1), SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

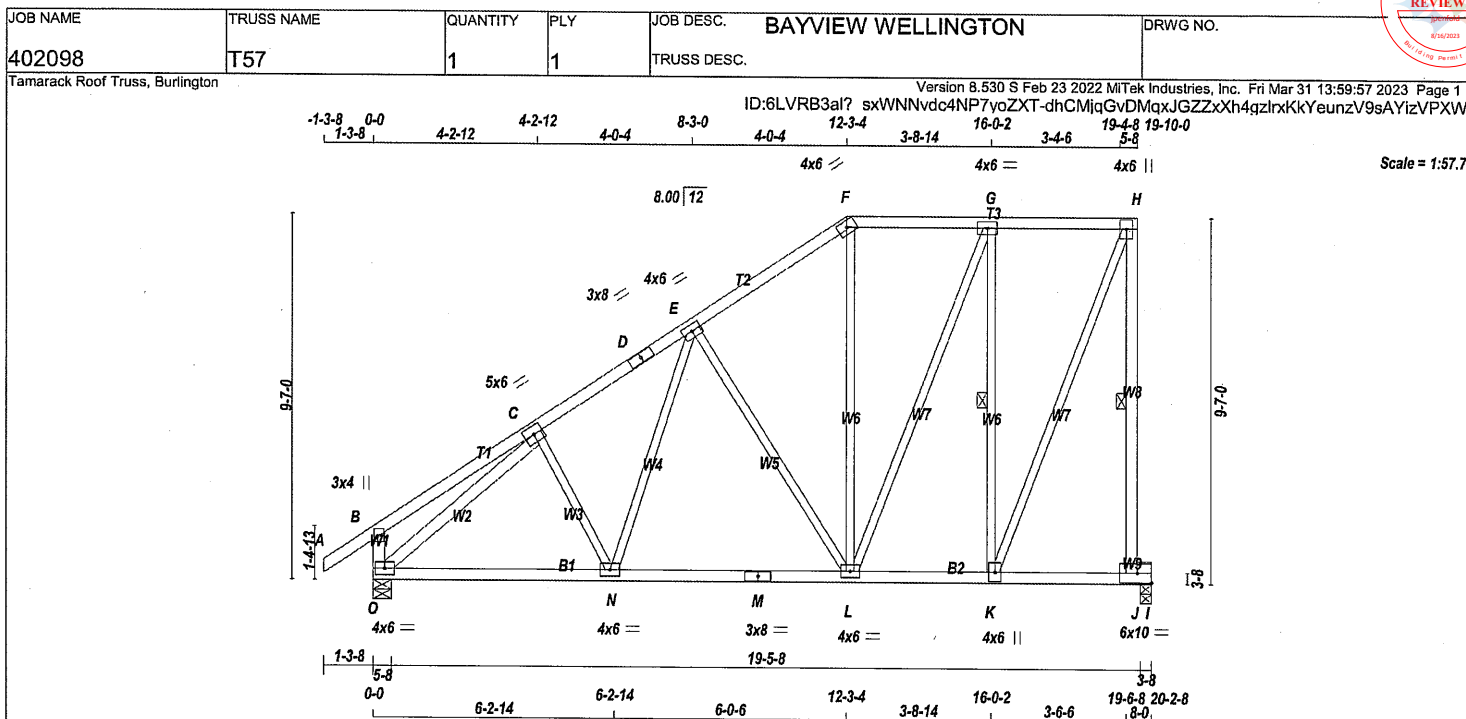
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040064



LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
O - C	2x4	DRY	No.2	SPF	

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-t	MT20	5.0	6.0		
D TS-t	MT20	3.0	8.0		
E TMVW-t	MT20	4.0	6.0		
F TTW-h	MT20	4.0	6.0		
G TMVW-t	MT20	4.0	6.0		
H TMVW+p	MT20	4.0	6.0		
J BMVW-t	MT20	8.0	10.0	3.00	4.50
K BMVW+t	MT20	4.0	6.0		
L BMVW-t	MT20	4.0	6.0		
M BS-t	MT20	3.0	8.0		
N BMVW-t	MT20	4.0	6.0		
O BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
O		1674	0	1674	0	0	5-8	5-8	
I		1449	0	1449	0	0	3-8	3-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0		
I	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-J, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CS (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C-N	-194 / 9
B-C	0 / 28	-129.6	-129.6	0.28 (1)	10.00	N-E	0 / 323
C-D	-1482 / 0	-129.6	-129.6	0.34 (1)	4.99	E-L	-693 / 0
D-E	-1482 / 0	-129.6	-129.6	0.34 (1)	4.99	L-F	0 / 119
E-F	-957 / 0	-129.6	-129.6	0.31 (1)	5.92	O-C	-1814 / 0
F-G	-771 / 0	-129.6	-129.6	0.30 (1)	6.25	K-H	0 / 1510
G-H	-574 / 0	-129.6	-129.6	0.29 (1)	6.25	K-G	-1101 / 0
J-H	-1590 / 0	0.0	0.0	0.69 (1)	5.24	L-G	0 / 541
O-B	-393 / 0	0.0	0.0	0.04 (1)	7.81		
O-N	0 / 1328	-18.5	-18.5	0.31 (1)	10.00		
N-M	0 / 1137	-18.5	-18.5	0.28 (1)	10.00		
M-L	0 / 1137	-18.5	-18.5	0.28 (1)	10.00		
L-K	0 / 574	-18.5	-18.5	0.31 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.74 (1)	10.00		
J-I	0 / 0	-18.5	-18.5	0.31 (1)	10.00		

TOTAL WEIGHT = 113 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.69/1.00 (H-J:1), BC=0.74/1.00 (J-K:1), WB=0.85/1.00 (E-L:1), SS=0.57/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

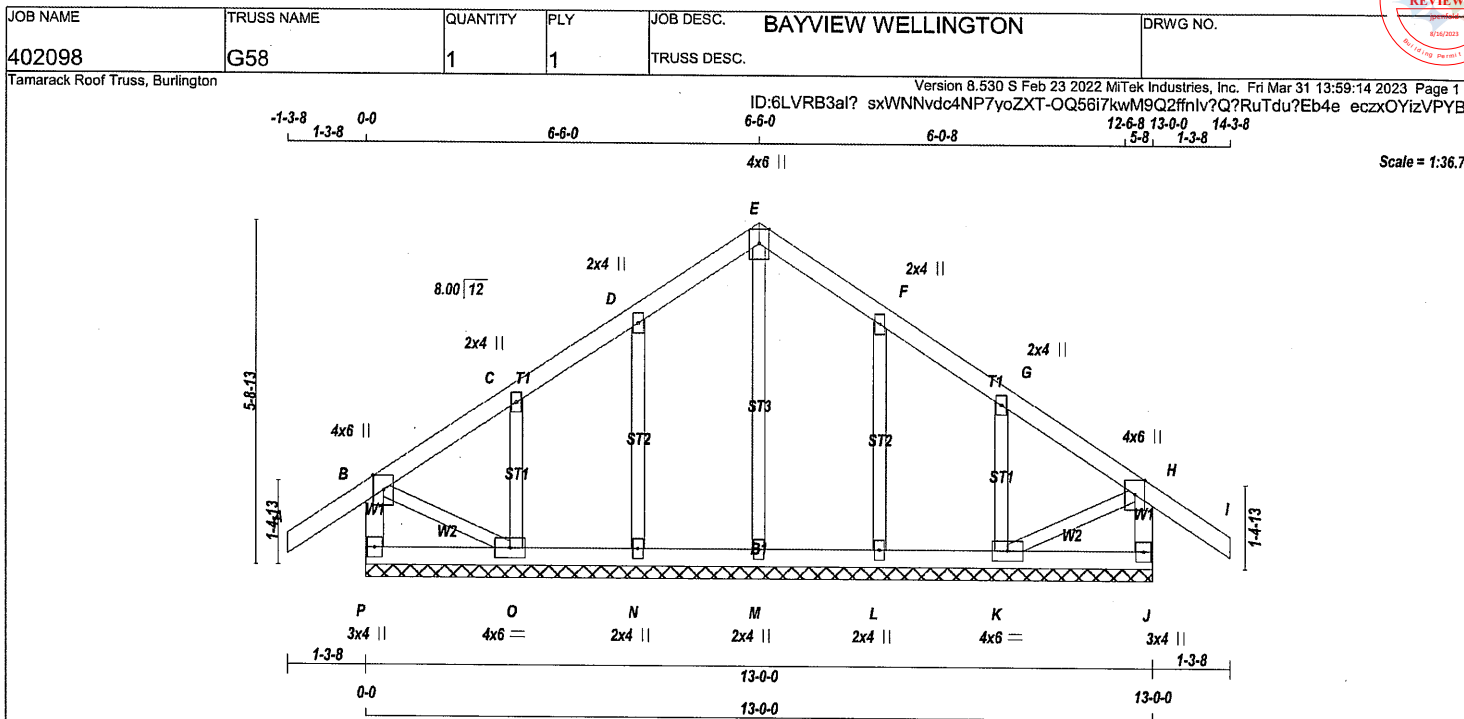
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

STRUCTURAL COMPONENT ONLY
DWG # TR23040065



TOTAL WEIGHT = 57 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

ALL GABLE WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMVW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-345 / 0	0.0 0.0 0.04 (1)	7.81	M-E	-194 / 0	0.10 (1)	
A-B	0 / 49	-129.6 -129.6 0.17 (1)	10.00	N-D	-275 / 0	0.08 (1)	
B-C	-27 / 0	-129.6 -129.6 0.09 (1)	6.25	O-C	-319 / 0	0.06 (1)	
C-D	-37 / 0	-129.6 -129.6 0.09 (1)	6.25	L-F	-275 / 0	0.08 (1)	
D-E	-38 / 0	-129.6 -129.6 0.07 (1)	6.25	K-G	-319 / 0	0.06 (1)	
E-F	-38 / 0	-129.6 -129.6 0.07 (1)	6.25	B-O	0 / 39	0.01 (1)	
F-G	-37 / 0	-129.6 -129.6 0.09 (1)	6.25	K-H	0 / 39	0.01 (1)	
G-H	-27 / 0	-129.6 -129.6 0.09 (1)	6.25				
H-I	0 / 49	-129.6 -129.6 0.17 (1)	10.00				
J-H	-345 / 0	0.0 0.0 0.04 (1)	7.81				
P-O	0 / 0	-18.5 -18.5 0.03 (4)	10.00				
O-N	0 / 26	-18.5 -18.5 0.03 (4)	10.00				
N-M	0 / 21	-18.5 -18.5 0.02 (4)	10.00				
M-L	0 / 21	-18.5 -18.5 0.02 (4)	10.00				
L-K	0 / 26	-18.5 -18.5 0.03 (4)	10.00				
K-J	0 / 0	-18.5 -18.5 0.03 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (H-I:1), BC=0.03/1.00 (N-O:4), WB=0.10/1.00 (E-M:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

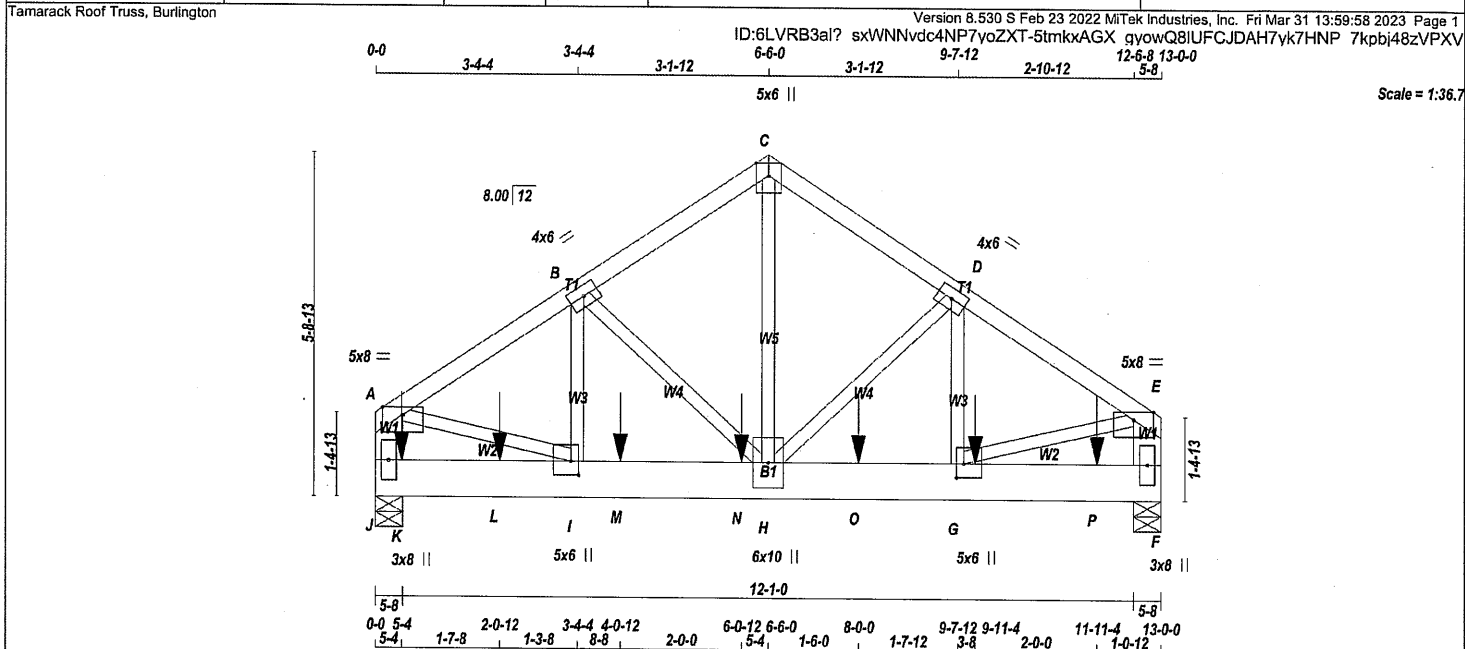
JSI GRIP= 0.21 (G) (INPUT = 0.90)
JSI METAL= 0.16 (C) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

STRUCTURAL COMPONENT ONLY
DWG # TR23040006



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T59	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS				SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
J - A	2x6	DRY	No.2	SPF	
F - E	2x6	DRY	No.2	SPF	
J - F	2x8	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
J - A	3	SIDE(195.8)
F - E	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F	11	SIDE(244.1)
WEBS : (0.122"x3") SPIRAL NAILS		
D - G	1	SIDE(3.9)
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	IN-SX	BRG	IN-SX
J	6382	0	0	6382	0	0	5-8	5-8	5-8
F	5079	0	0	5079	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	4440	3329 / 0	0 / 0	0 / 0	0 / 0	1111 / 0	0 / 0
F	3532	2656 / 0	0 / 0	0 / 0	0 / 0	876 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD	MAX. (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	-5611 / 0	-129.6	-129.6 0.23 (1)	H-C	0 / 4642
B-C	-4517 / 0	-129.6	-129.6 0.17 (1)	H-D	-993 / 0
C-D	-4518 / 0	-129.6	-129.6 0.17 (1)	G-D	0 / 695
D-E	-5329 / 0	-129.6	-129.6 0.22 (1)	B-H	-1324 / 0
J-A	-4601 / 0	0.0	0.0 0.16 (1)	I-B	0 / 1088
F-E	-4380 / 0	0.0	0.0 0.16 (1)	A-I	0 / 4854
				G-E	0 / 4612
J-K	0 / 0	-18.5	-18.5 0.32 (1)		
K-L	0 / 0	-18.5	-18.5 0.32 (1)		
L-I	0 / 0	-18.5	-18.5 0.32 (1)		
I-M	0 / 4680	-18.5	-18.5 0.43 (1)		
M-N	0 / 4680	-18.5	-18.5 0.43 (1)		
N-H	0 / 4680	-18.5	-18.5 0.43 (1)		
H-O	0 / 4446	-18.5	-18.5 0.51 (1)		
O-G	0 / 4446	-18.5	-18.5 0.51 (1)		
G-P	0 / 0	-18.5	-18.5 0.18 (1)		
P-F	0 / 0	-18.5	-18.5 0.18 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	9-11-4	-660	-660	---	BACK	VERT	TOTAL	---	C1
K	5-4	-1011	-1011	---	BACK	VERT	TOTAL	---	C1
L	2-0-12	-1006	-1006	---	BACK	VERT	TOTAL	---	C1
M	4-0-12	-1006	-1006	---	BACK	VERT	TOTAL	---	C1
N	6-0-12	-1006	-1006	---	BACK	VERT	TOTAL	---	C1
O	8-0-0	-1279	-1279	---	BACK	VERT	TOTAL	---	C1
P	11-11-4	-660	-660	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 71 = 143 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.23/1.00 (A-B:1), BC=0.51/1.00 (G-H:1), WB=0.60/1.00 (A-I:1), SSI=0.52/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)

JSI METAL= 0.82 (I) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR23040066



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T59	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID:6LVRB3aI? sxWNNvdc4NP7yoZXT-5tmkxAGX gyowQ8IUFCJDAH7yk7HNP 7kpbj48zVPXV

PLATES (table is in inches)

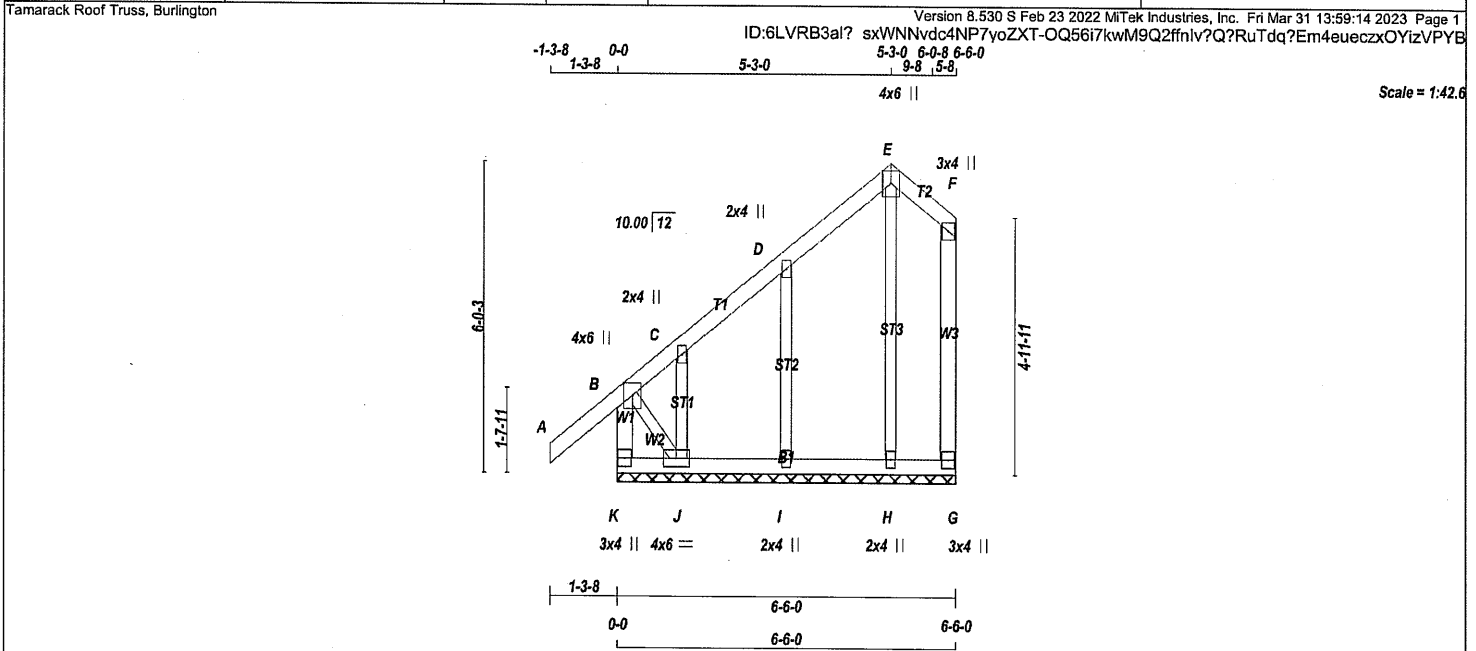
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMVW-t	MT20	4.0	6.0		
C	TTW+p	MT20	5.0	6.0	Edge	
D	TMVW-t	MT20	4.0	6.0		
E	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1+p	MT20	3.0	8.0		
G	BMVW+t	MT20	5.0	6.0	2.75	1.50
H	BMVW+t	MT20	6.0	10.0		
I	BMVW+t	MT20	5.0	6.0	2.75	1.50
J	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040066

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	G60A	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. K - B 2x4 DRY No.2 SPF A - E 2x4 DRY No.2 SPF E - F 2x4 DRY No.2 SPF G - F 2x4 DRY No.2 SPF K - G 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF ALL GABLE WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2-0-0 OC.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4)				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.2 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 51.6 PSF SPACING = 24.0 IN./C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.18/1.00 (A-B:1), BC=0.02/1.00 (I-J:4), WB=0.10/1.00 (E-H:1), SSI=0.11/1.00 (A-B:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 768 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.23 (B) (INPUT = 0.90) JSI METAL= 0.17 (D) (INPUT = 1.00)				PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW+p MT20 4.0 6.0 Edge C TMW+w MT20 2.0 4.0 D TMW+w MT20 2.0 4.0 E TTW+p MT20 4.0 6.0 Edge F TMV+p MT20 3.0 4.0 G BMV1+p MT20 3.0 4.0 H BMW1+w MT20 2.0 4.0 I BMW1+w MT20 2.0 4.0 J BMW1-t MT20 4.0 6.0 K BMV1+p MT20 3.0 4.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. FACTORED VERT. LOAD (PLF) MAX. FACTORED VERT. LOAD (PLF) UNBRACED LENGTH (FT) FR-TO K-B -372 / 0 0.0 0.0 0.04 (1) 7.81 A-B 0 / 57 -129.6 -129.6 0.18 (1) 10.00 B-C -76 / 0 -129.6 -129.6 0.17 (1) 6.25 C-D 0 / 6 -129.6 -129.6 0.09 (1) 10.00 D-E -19 / 0 -129.6 -129.6 0.08 (1) 6.25 E-F 0 / 0 -129.6 -129.6 0.03 (1) 10.00 G-F -81 / 0 0.0 0.0 0.04 (1) 7.81 WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED VERT. LOAD (PLF) UNBRACED LENGTH (FT) FR-TO H-E -187 / 0 0.10 (1) I-D -314 / 0 0.09 (1) J-C -79 / 0 0.01 (1) B-J 0 / 15 0.00 (1)			
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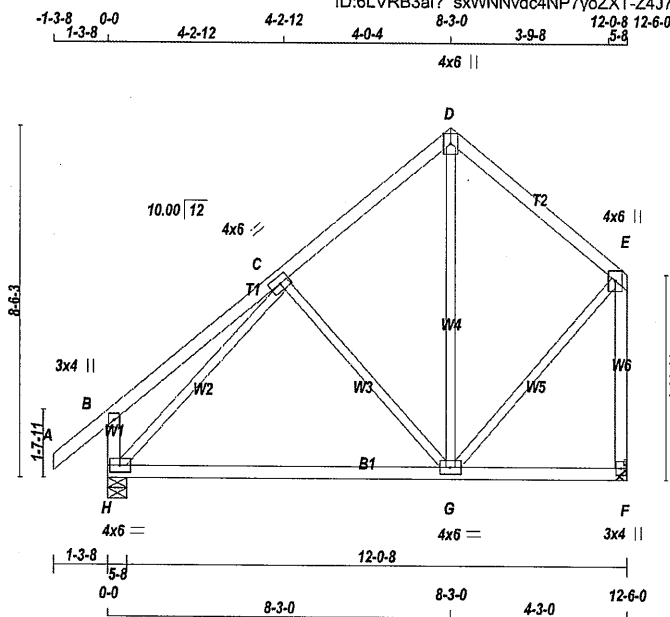
STRUCTURAL COMPONENT ONLY
DWG # TR23040007

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T61A	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

ID:6LVRB3al? sxWNNvdc4NP7yoZXT-Z4J78WH9I 4fYajy2yYmOqFu8Xp6sMGyTLHcbzVPXU Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:59 2023 Page 1



TOTAL WEIGHT = 2 X 63 = 125 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	926	0	926	0	MECHANICAL	
H	1105	0	1105	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	645	478 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
H	768	584 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 57	-129.6	-129.6 0.18 (1)	C-G	-369 / 0	0.26 (1)	
B-C	0 / 42	-129.6	-129.6 0.37 (1)	G-D	0 / 98	0.03 (4)	
C-D	-472 / 0	-129.6	-129.6 0.28 (1)	H-C	-877 / 0	0.59 (1)	
D-E	-431 / 0	-129.6	-129.6 0.30 (1)	G-E	0 / 490	0.11 (1)	
H-B	-380 / 0	0.0	0.0 0.04 (1)				
F-E	-913 / 0	0.0	0.0 0.41 (1)				
H-G	0 / 575	-18.5	-18.5 0.30 (4)				
G-F	0 / 0	-18.5	-18.5 0.27 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	38.2	
BOT CH.	LL	PSF
	0.0	
	DL	PSF
	7.4	
TOTAL LOAD	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.41/1.00 (E-F:1), BC=0.30/1.00 (G-H:4), WB=0.59/1.00 (C-H:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (G) (INPUT = 0.90)
JSI METAL= 0.25 (E) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



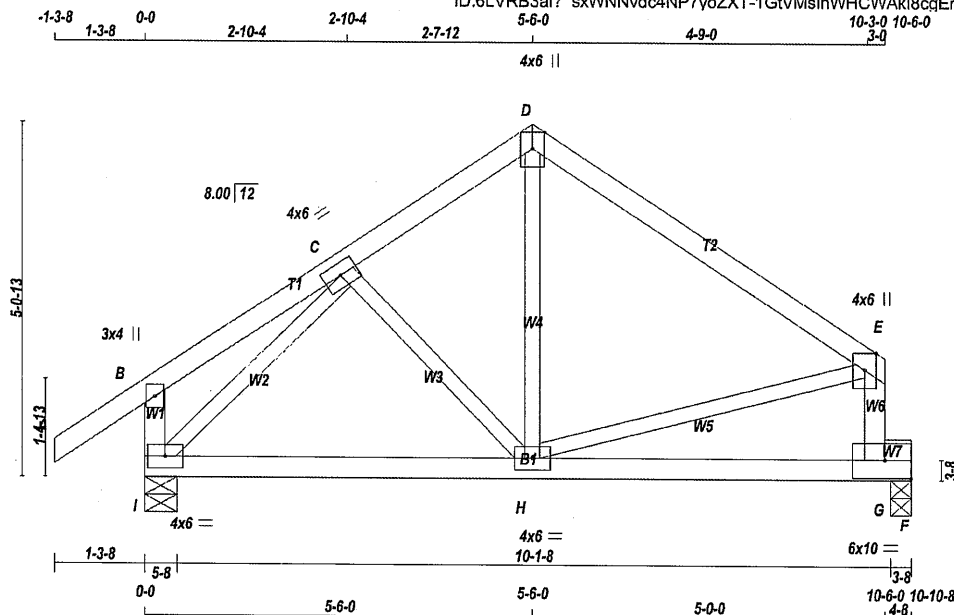
STRUCTURAL COMPONENT ONLY
DWG # TR23040067



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T62	4	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID:6LVRB3al? sxWNNvdc4NP7yoZXT-1GtVMslnWHCWAKl8cqEnlbMNyYqmrPGPB74q91zVPXT



Scale = 1:31.5

TOTAL WEIGHT = 4 X 45 = 182 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
I - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW+p	MT20	4.0	6.0	Edge	
G	BMVW-t	MT20	6.0	10.0	3.00	Edge
H	BMWW-t	MT20	4.0	6.0		
I	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQRD	
	GROSS REACTION		GROSS REACTION		BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	
I	983	0	983	0	5-8	5-8	
F	758	0	758	0	3-8	3-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
I	682	520 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
F	529	387 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	CS1 (LC)
FR-TO				FR-TO			
A-B	0 / 49	-129.6	-129.6 0.17 (1)	10.00	C-H	-166 / 0	0.05 (1)
B-C	0 / 22	-129.6	-129.6 0.15 (1)	10.00	H-D	0 / 186	0.04 (1)
C-D	-610 / 0	-129.6	-129.6 0.15 (1)	6.25	I-C	-878 / 0	0.23 (1)
D-E	-585 / 0	-129.6	-129.6 0.58 (1)	6.25	H-E	0 / 505	0.11 (1)
I-B	-316 / 0	0.0	0.0 0.03 (1)	7.81			
G-E	-784 / 0	0.0	0.0 0.08 (1)	7.81			
I-H	0 / 602	-18.5	-18.5 0.26 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.38 (1)	10.00			
G-F	0 / 0	-18.5	-18.5 0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.58/1.00 (D-E:1), BC=0.38/1.00 (G-H:1), WB=0.23/1.00 (C-I:1), SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.71 (H) (INPUT = 0.90)

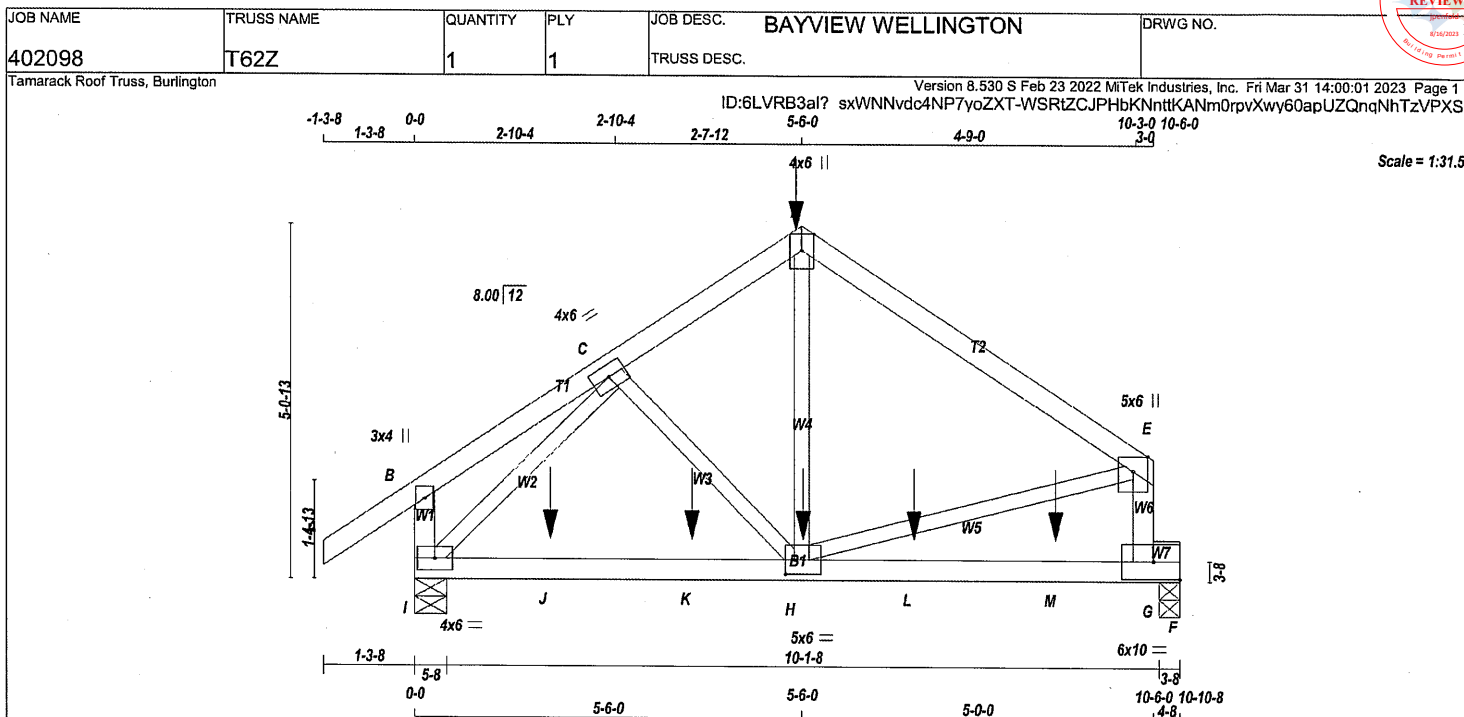
JSI METAL = 0.35 (E) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040068

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



TOTAL WEIGHT = 45 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
I - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMVW-t	MT20	6.0	10.0	Edge	
H	BMVW-t	MT20	5.0	6.0	2.50	1.50
I	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UPLIFT		
I	1470	0	1470	0	5-8	5-8
F	1257	0	1257	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE	WIND	DEAD			
I	1021	777 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0	0 / 0	0 / 0
F	876	650 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 49	-129.6 -129.6	0.19 (1)	10.00	C-H	-11 / 33	0.01 (4)
B-C	0 / 20	-129.6 -129.6	0.15 (1)	10.00	H-D	0 / 155	0.06 (4)
C-D	-1278 / 0	-129.6 -129.6	0.17 (1)	5.47	I-C	-1537 / 0	0.42 (1)
D-E	-1258 / 0	-129.6 -129.6	0.69 (1)	4.60	H-E	0 / 1086	0.27 (1)
I-B	-319 / 0	0.0 0.0	0.04 (1)	7.81			
G-E	-1313 / 0	0.0 0.0	0.15 (1)	7.02			
I-J	0 / 1054	-18.5 -18.5	0.48 (1)	10.00			
J-K	0 / 1054	-18.5 -18.5	0.48 (1)	10.00			
K-H	0 / 1054	-18.5 -18.5	0.48 (1)	10.00			
H-L	0 / 0	-18.5 -18.5	0.70 (1)	10.00			
L-M	0 / 0	-18.5 -18.5	0.70 (1)	10.00			
M-G	0 / 0	-18.5 -18.5	0.70 (1)	10.00			
G-F	0 / 0	-18.5 -18.5	0.29 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-6-0	-69	-69	---	FRONT	VERT	DEAD	---	C1
D	5-6-0	-100	-100	---	BACK	VERT	TOTAL	---	C1
D	5-6-0	-438	-438	---	FRONT	VERT	SNOW	---	C1
H	5-6-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
J	1-11-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
K	3-11-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
L	7-1-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
M	9-1-4	-16	-16	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/628 (0.21")

CSI: TC=0.69/1.00 (D-E:1), BC=0.70/1.00 (G-H:1), WB=0.42/1.00 (C-I:1), SSI=0.54/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.53 (E) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

STRUCTURAL COMPONENT ONLY
DWG # TR23040070



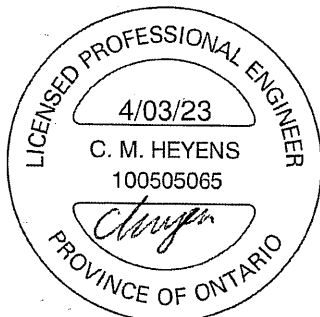
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T90	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:47:26 2023 Page 2	
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SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	1-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
U	3-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
V	5-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
W	8-8-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
X	11-7-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
Y	14-4-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
Z	16-4-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AA	18-4-12	-25	-25	---	BACK	VERT	TOTAL	---	C1

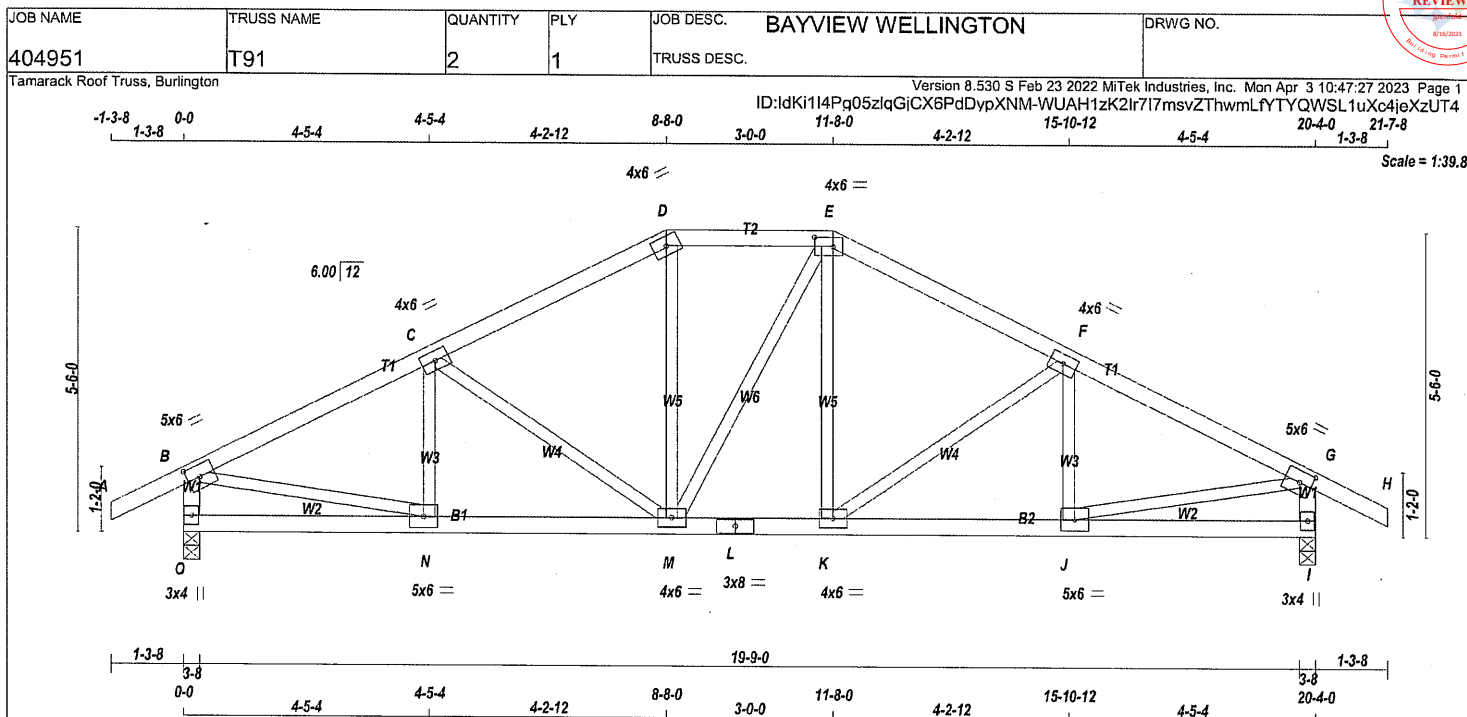
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040330



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TTW-t	MT20	4.0	6.0	2.00	4.00
F	TMVW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
O	1682	0	1682	0	3-8	3-8
I	1682	0	1682	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1169	881/0	0/0	0/0	0/0	289/0	0/0
I	1169	881/0	0/0	0/0	0/0	289/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.
FR-TO				FR-TO			
A-B	0/39	-129.6	-129.6 0.17 (1)	10.00	M-D	0/320	0.07 (1)
B-C	-1977/0	-129.6	-129.6 0.35 (1)	4.46	M-E	0/3	0.00 (1)
C-D	-1640/0	-129.6	-129.6 0.33 (1)	4.83	K-E	0/317	0.07 (1)
D-E	-1445/0	-129.6	-129.6 0.17 (1)	5.27	C-M	-434/0	0.20 (1)
E-F	-1638/0	-129.6	-129.6 0.33 (1)	4.83	N-C	-265/15	0.05 (1)
F-G	-1977/0	-129.6	-129.6 0.35 (1)	4.46	B-N	0/1826	0.41 (1)
G-H	0/39	-129.6	-129.6 0.17 (1)	10.00	K-F	-437/0	0.21 (1)
O-B	-1644/0	0.0	0.0 0.17 (1)	6.46	J-F	-263/16	0.05 (1)
I-G	-1644/0	0.0	0.0 0.17 (1)	6.46	J-G	0/1827	0.41 (1)
O-N	0/0	-18.5	-18.5 0.08 (4)	10.00			
N-M	0/1793	-18.5	-18.5 0.32 (1)	10.00			
M-L	0/1444	-18.5	-18.5 0.26 (1)	10.00			
L-K	0/1444	-18.5	-18.5 0.26 (1)	10.00			
K-J	0/1794	-18.5	-18.5 0.33 (1)	10.00			
J-I	0/0	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.35/1.00 (F-G-1), BC=0.33/1.00 (J-K-1),
WB=0.41/1.00 (G-J-1), SS=0.24/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

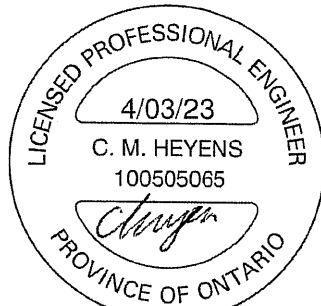
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.50 (G) (INPUT = 1.00)



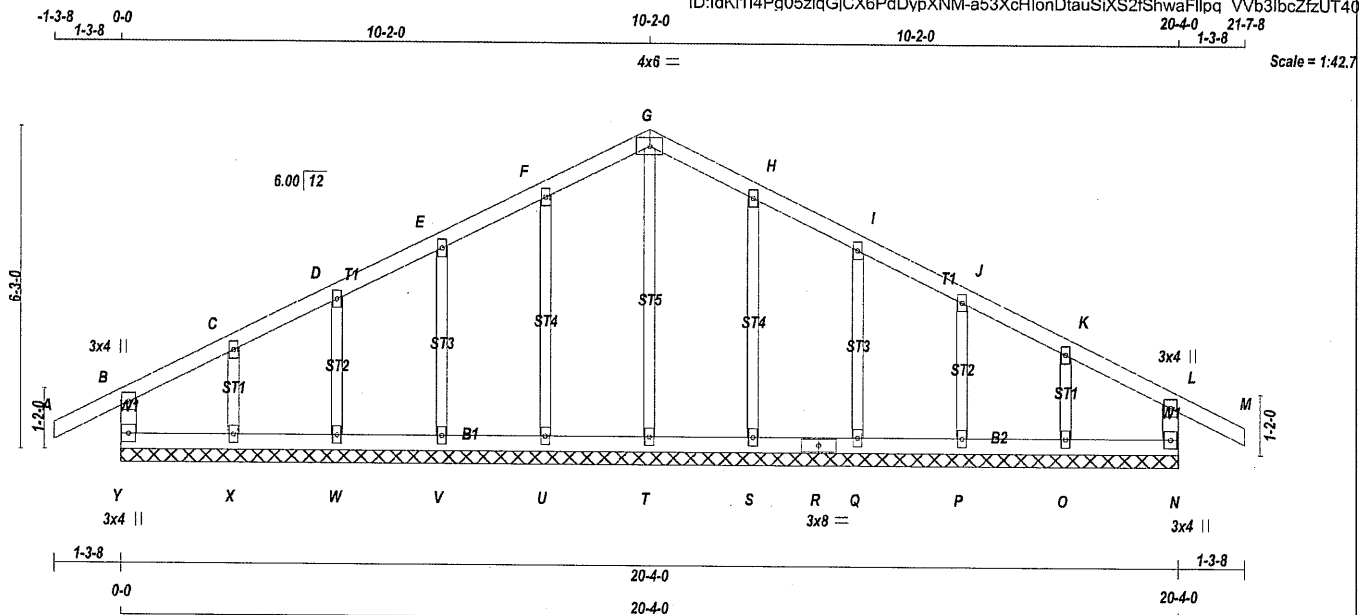
STRUCTURAL COMPONENT ONLY
DWG # TR23040331

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	G92	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:47:25 2023 Page 1
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TOTAL WEIGHT = 2 X 80 = 159 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C, D, E, F, H, I, J, K					
C	TMV+hw	MT20	2.0	4.0	
G	TTW+p	MT20	4.0	6.0	
L	TMV+p	MT20	3.0	4.0	
N	BMV1+p	MT20	3.0	4.0	
O, P, Q, S, T, U, V, W, X					
O	BMV1+hw	MT20	2.0	4.0	
R	BS-t	MT20	3.0	8.0	
Y	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
Y-B	-332 / 0	0.0	0.02 (1)	T-G	-257 / 0	0.16 (1)	
A-B	0 / 39	-129.6	-129.6 0.17 (1)	U-F	-262 / 0	0.11 (1)	
B-C	-39 / 0	-129.6	-129.6 0.07 (1)	V-E	-256 / 0	0.07 (1)	
C-D	-28 / 0	-129.6	-129.6 0.06 (1)	W-D	-256 / 0	0.05 (1)	
D-E	-22 / 0	-129.6	-129.6 0.06 (1)	X-C	-259 / 0	0.04 (1)	
E-F	-17 / 0	-129.6	-129.6 0.06 (1)	S-H	-262 / 0	0.11 (1)	
F-G	-13 / 0	-129.6	-129.6 0.06 (1)	Q-I	-256 / 0	0.07 (1)	
G-H	-13 / 0	-129.6	-129.6 0.06 (1)	P-J	-256 / 0	0.05 (1)	
H-I	-17 / 0	-129.6	-129.6 0.06 (1)	O-K	-259 / 0	0.04 (1)	
I-J	-22 / 0	-129.6	-129.6 0.06 (1)				
J-K	-28 / 0	-129.6	-129.6 0.06 (1)				
K-L	-39 / 0	-129.6	-129.6 0.07 (1)				
L-M	0 / 39	-129.6	-129.6 0.17 (1)				
N-L	-332 / 0	0.0	0.02 (1)				
Y-X	0 / 34	-18.5	-18.5 0.02 (1)				
X-W	0 / 25	-18.5	-18.5 0.02 (4)				
W-V	0 / 19	-18.5	-18.5 0.02 (4)				
V-U	0 / 15	-18.5	-18.5 0.02 (4)				
U-T	0 / 12	-18.5	-18.5 0.02 (4)				
T-S	0 / 12	-18.5	-18.5 0.02 (4)				
S-R	0 / 15	-18.5	-18.5 0.02 (4)				
R-Q	0 / 15	-18.5	-18.5 0.02 (4)				
Q-P	0 / 19	-18.5	-18.5 0.02 (4)				
P-O	0 / 25	-18.5	-18.5 0.02 (4)				
O-N	0 / 34	-18.5	-18.5 0.02 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (L-M:1), BC=0.02/1.00 (N-O:1), WB=0.16/1.00 (G-T:1), SSI=0.12/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (I) (INPUT = 0.90)

JSI METAL= 0.11 (L) (INPUT = 1.00)

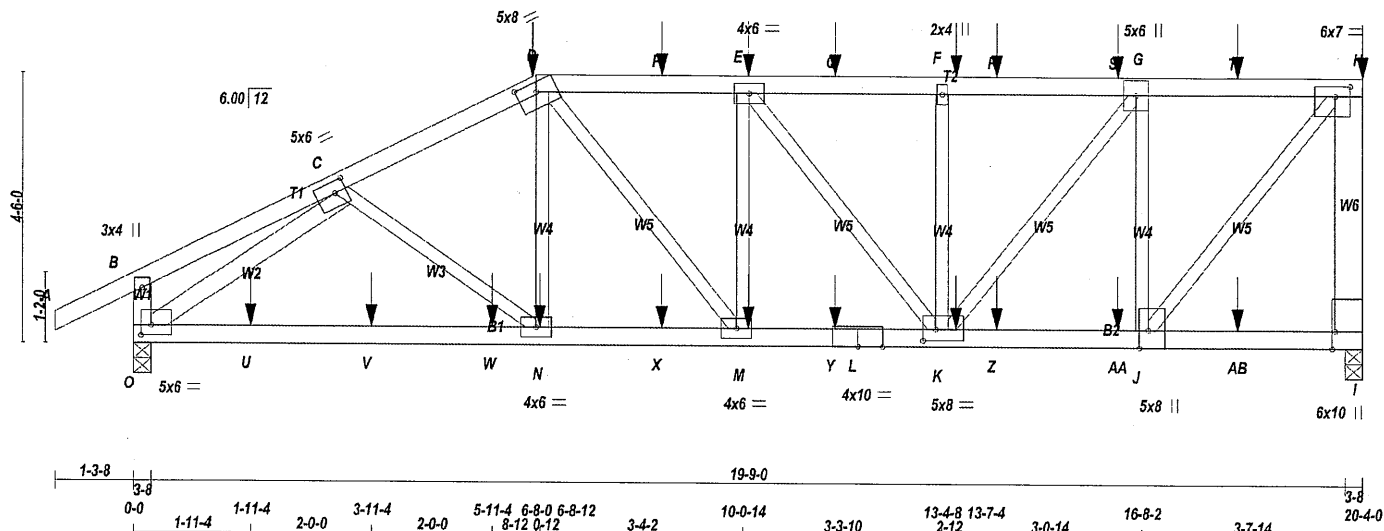


STRUCTURAL COMPONENT ONLY
DWG # TR23040329

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME 404958	TRUSS NAME T93	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:44:58 2023 Page 1 ID:1phxEr21g7yfr80b1u1TXym hD-Opgym0WbO3LRlszHLqW Ew ?PUSL2sfyGwUKvgzUT6J	
1-3-8 0-0 3-5-4 3-5-4 3-2-12 6-8-0 3-4-14 10-0-14 3-3-10 13-4-8 13-7-4 3-0-14 16-8-2 3-7-14 20-4-0 Scale = 1:36.7					



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

ALL WEBS				
EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW-h	MT20	5.0	8.0	2.00	4.00
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	TMVW-t	MT20	6.0	7.0	2.00	3.00
I	BMV1-t	MT20	6.0	10.0	Edge	0.50
J	BMVW-t	MT20	5.0	8.0	Edge	1.75
K	BMVW-t	MT20	5.0	8.0	2.25	2.50
L	BS-t	MT20	4.0	10.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	6.0		
O	BMVW-t	MT20	5.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	
I	VERT 3091	DOWN 3091	0	3-8	3-8
O	HORZ 2885	HORZ 2885	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2152	1606 / 0	0 / 0	0 / 0	0 / 0	546 / 0	0 / 0
O	2012	1480 / 0	0 / 0	0 / 0	0 / 0	533 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 39	-129.6 -129.6	0.19 (1)	10.00	C-N	0 / 310	0.08 (1)
B-C	0 / 17	-129.6 -129.6	0.20 (1)	10.00	N-D	0 / 206	0.08 (4)
C-D	-3919 / 0	-129.6 -129.6	0.43 (1)	3.16	O-C	-4057 / 0	0.89 (1)
D-P	-3855 / 0	-129.6 -129.6	0.80 (1)	2.92	J-H	0 / 3379	0.84 (1)
P-E	-3855 / 0	-129.6 -129.6	0.60 (1)	2.92	D-M	0 / 583	0.14 (1)
E-Q	-3403 / 0	-129.6 -129.6	0.54 (1)	3.17	J-G	-2411 / 0	0.75 (1)
Q-F	-3403 / 0	-129.6 -129.6	0.54 (1)	3.17	M-E	-367 / 9	0.11 (1)
F-R	-3403 / 0	-129.6 -129.6	0.59 (1)	3.09	K-G	0 / 1922	0.48 (1)
R-S	-3403 / 0	-129.6 -129.6	0.59 (1)	3.09	E-K	-733 / 0	0.35 (1)
S-G	-3403 / 0	-129.6 -129.6	0.59 (1)	3.09	K-F	-808 / 0	0.25 (1)
G-T	-2216 / 0	-129.6 -129.6	0.50 (1)	3.94			
T-H	-2216 / 0	-129.6 -129.6	0.50 (1)	3.94			
I-H	-3053 / 0	0.0	0.0	6.4 (1)	5.99		
O-B	-355 / 0	0.0	0.0	0.04 (1)	7.81		

O-U	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
U-V	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
V-W	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
W-N	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
N-X	0 / 3488	-18.5	-18.5	0.80 (1)	10.00
X-M	0 / 3488	-18.5	-18.5	0.80 (1)	10.00
M-Y	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
Y-L	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
L-K	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
K-Z	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
Z-AA	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
AA-J	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
J-AB	0 / 0	-18.5	-18.5	0.10 (4)	10.00
AB-I	0 / 0	-18.5	-18.5	0.10 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
D	6-8-0	-561	-561	FRONT	VERT	TOTAL	---	C1
E	10-2-0	-133	-133	FRONT	VERT	TOTAL	---	C1
F	13-7-4	-133	-133	FRONT	VERT	TOTAL	---	C1
H	20-4-0	-174	-174	FRONT	VERT	TOTAL	---	C1
K	13-7-4	-26	-26	FRONT	VERT	TOTAL	---	C1
M	10-2-0	-26	-26	FRONT	VERT	TOTAL	---	C1
N	6-8-12	-26	-26	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.64/1.00 (H-I:1), BC=0.80/1.00 (M-N:1), WB=0.89/1.00 (C-O:1), SSI=0.41/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

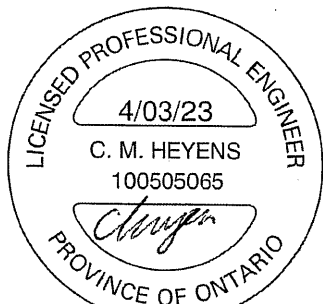
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.94 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR23040327



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T93	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	8-8-12	-133	-133	---	FRONT	VERT	TOTAL	---	C1
Q	11-7-4	-133	-133	---	FRONT	VERT	TOTAL	---	C1
R	14-3-4	-133	-133	---	FRONT	VERT	TOTAL	---	C1
S	16-3-4	-133	-133	---	FRONT	VERT	TOTAL	---	C1
T	18-3-4	-133	-133	---	FRONT	VERT	TOTAL	---	C1
U	1-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
V	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
W	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	8-8-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Y	11-7-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	14-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AA	16-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AB	18-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

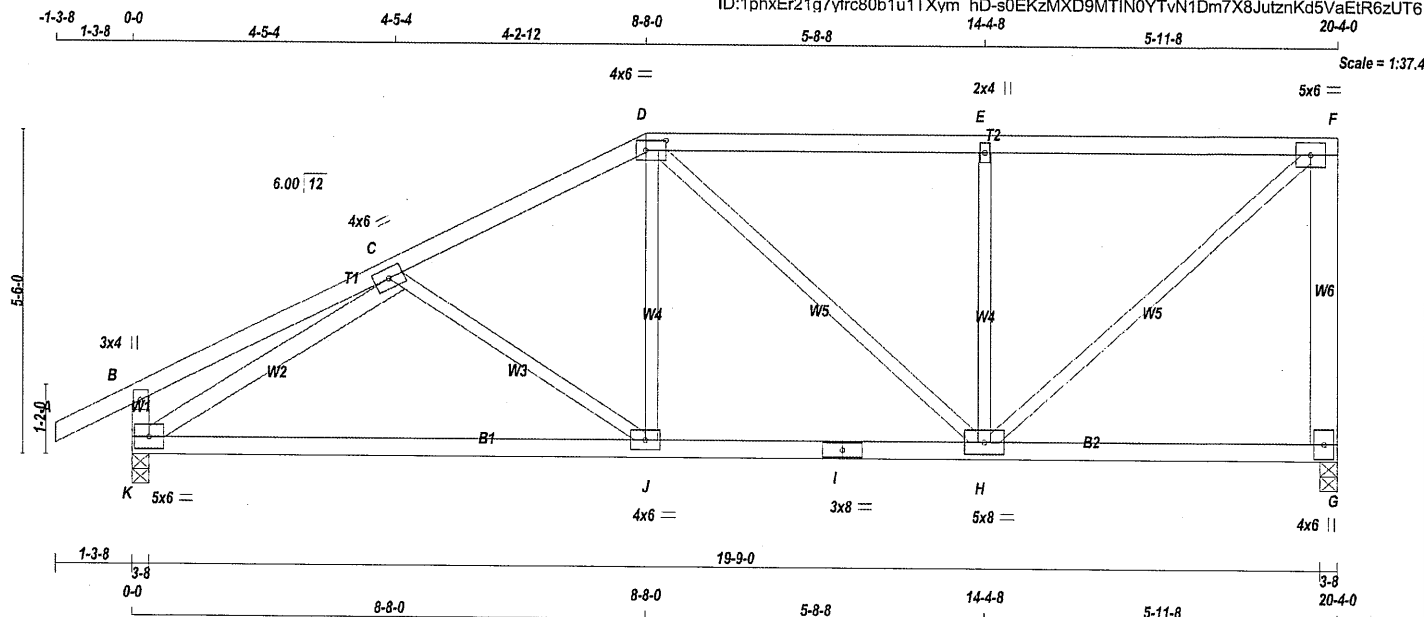


STRUCTURAL COMPONENT ONLY
DWG # TR23040327



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T94	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 89 = 177 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV-t	MT20	4.0	6.0		
D	TTWV-t	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	4.0	6.0		
H	BMWVW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	8.0		
J	BMWV-t	MT20	4.0	6.0		
K	BMWV1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
G	1506	0	1506	0	3-8	3-8
K	1682	0	1682	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1050	777 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 39	-129.6 -129.6	0.17 (1)	10.00	C-J	-356 / 0	0.18 (1)
B-C	0 / 29	-129.6 -129.6	0.38 (1)	10.00	J-D	0 / 363	0.08 (1)
C-D	-1659 / 0	-129.6 -129.6	0.44 (1)	4.61	K-C	-2133 / 0	0.72 (1)
D-E	-1327 / 0	-129.6 -129.6	0.82 (1)	4.13	H-F	0 / 1763	0.40 (1)
E-F	-1327 / 0	-129.6 -129.6	0.83 (1)	4.13	D-H	-185 / 0	0.22 (1)
G-F	-1457 / 0	0.0 0.0	0.50 (1)	7.81	H-E	-938 / 0	0.43 (1)
K-B	-395 / 0	0.0 0.0	0.04 (1)	7.81			
K-J	0 / 1750	-18.5 -18.5	0.45 (1)	10.00			
J-I	0 / 1464	-18.5 -18.5	0.39 (1)	10.00			
I-H	0 / 1464	-18.5 -18.5	0.39 (1)	10.00			
H-G	0 / 0	-18.5 -18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.21")

CSI: TC=0.83/1.00 (E-F:1), BC=0.45/1.00 (J-K:1), WB=0.72/1.00 (C-K:1), SSI=0.37/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

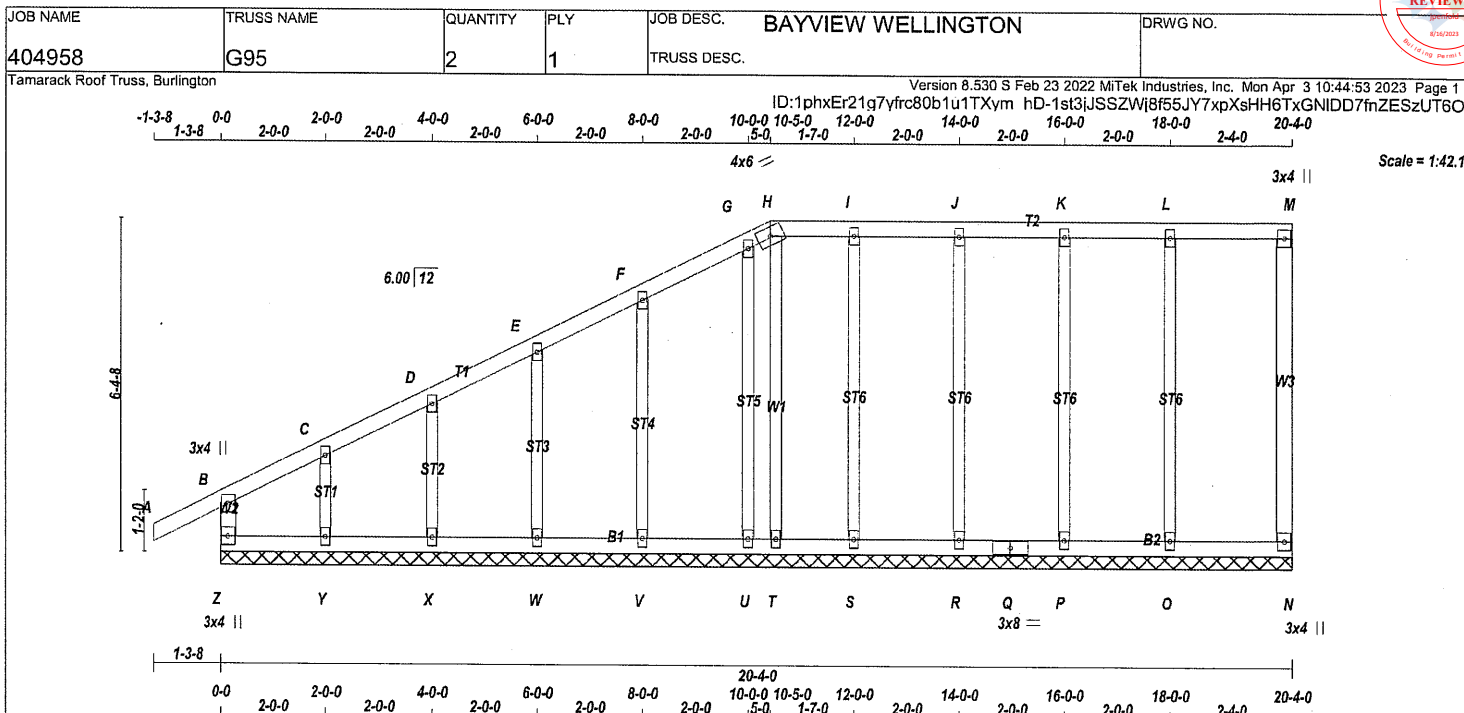
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.44 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR23040328

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



TOTAL WEIGHT = 2 X 95 = 190 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
Z - B	2x4	DRY	No.2
A - H	2x4	DRY	No.2
H - M	2x4	DRY	No.2
N - M	2x4	DRY	No.2
Z - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, L						
C	TMW+w	MT20	2.0	4.0		
H	TTW-h	MT20	4.0	6.0		
M	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, R, S, T, U, V, W, X, Y						
O	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
Z	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO			
Z-B	-317 / 0	0.0	0.0 0.02 (1)	Y-C	-249 / 0	7.81	0.04 (1)
A-B	0 / 39	-129.6	-129.6 0.17 (1)	X-D	-258 / 0	10.00	0.05 (1)
B-C	-36 / 0	-129.6	-129.6 0.06 (1)	W-E	-256 / 0	6.25	0.07 (1)
C-D	-25 / 0	-129.6	-129.6 0.06 (1)	V-F	-266 / 0	6.25	0.11 (1)
D-E	-19 / 0	-129.6	-129.6 0.06 (1)	U-G	-159 / 0	6.25	0.10 (1)
E-F	-14 / 0	-129.6	-129.6 0.06 (1)	S-I	-230 / 0	6.25	0.15 (1)
F-G	-13 / 0	-129.6	-129.6 0.06 (1)	R-J	-267 / 0	6.25	0.17 (1)
G-H	-11 / 0	-129.6	-129.6 0.04 (1)	P-K	-249 / 0	6.25	0.16 (1)
H-I	-5 / 0	-129.6	-129.6 0.04 (1)	O-L	-304 / 0	10.00	0.20 (1)
I-J	-5 / 0	-129.6	-129.6 0.06 (1)	T-H	-120 / 0	10.00	0.08 (1)
J-K	-5 / 0	-129.6	-129.6 0.06 (1)				
K-L	-5 / 0	-129.6	-129.6 0.08 (1)				
L-M	-5 / 0	-129.6	-129.6 0.08 (1)				
N-M	-136 / 0	0.0	0.0 0.04 (1)				
Z-Y	0 / 32	-18.5	-18.5 0.02 (1)				
Y-X	0 / 23	-18.5	-18.5 0.02 (4)				
X-W	0 / 17	-18.5	-18.5 0.02 (4)				
W-V	0 / 13	-18.5	-18.5 0.02 (4)				
V-U	0 / 9	-18.5	-18.5 0.02 (4)				
U-T	0 / 7	-18.5	-18.5 0.02 (4)				
T-S	0 / 5	-18.5	-18.5 0.01 (4)				
S-R	0 / 5	-18.5	-18.5 0.02 (4)				
R-Q	0 / 5	-18.5	-18.5 0.02 (4)				
Q-P	0 / 5	-18.5	-18.5 0.02 (4)				
P-O	0 / 5	-18.5	-18.5 0.02 (4)				
O-N	0 / 5	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1), BC=0.02/1.00 (O-P:4), WB=0.20/1.00 (L-O:1), SSI=0.13/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

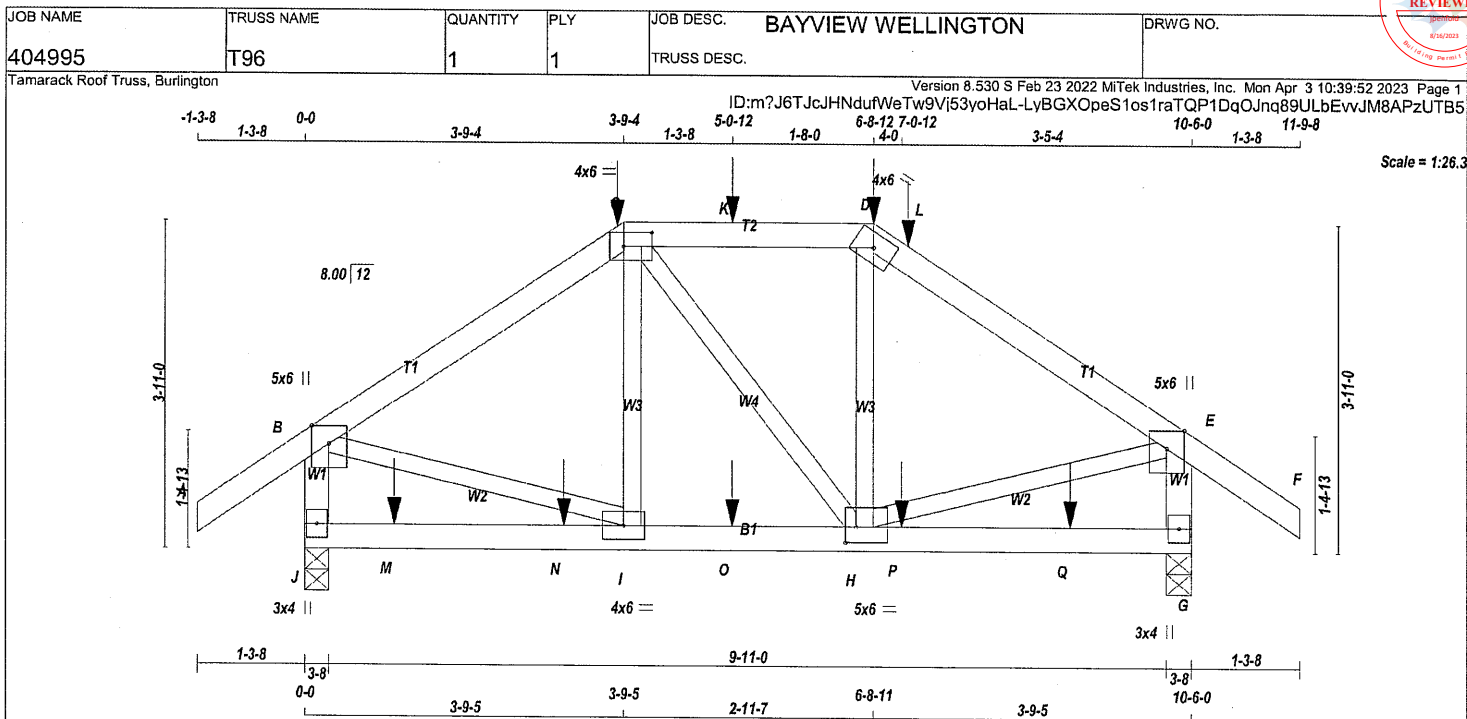
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (E) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR23040319

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

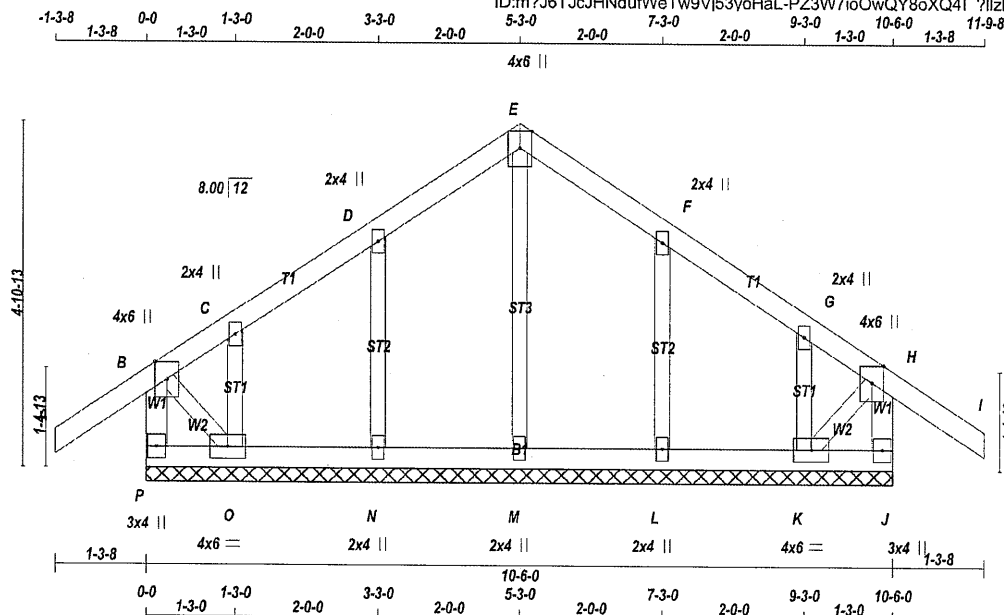




JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G97	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:31.3

TOTAL WEIGHT = 46 lb

LUMBER					DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER			
A - E	2x4	DRY	No.2		SPF	
E - I	2x4	DRY	No.2		SPF	
P - B	2x4	DRY	No.2		SPF	
J - H	2x4	DRY	No.2		SPF	
P - J	2x4	DRY	No.2		SPF	
ALL WEBS					2x3	DRY
ALL GABLE WEBS					2x3	DRY
DRY: SEASONED LUMBER.					No.2	SPF
GABLE STUDS SPACED AT 2-0-0 OC.						

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMVW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (LC))	UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (CSI (LC))
FR-TO					FR-TO		
A-B	0 / 49	-129.6	-129.6 0.17 (1)	10.00	M-E	-206 / 0	0.07 (1)
B-C	-70 / 0	-129.6	-129.6 0.17 (1)	6.25	N-D	-315 / 0	0.07 (1)
C-D	0 / 0	-129.6	-129.6 0.08 (1)	10.00	O-C	-83 / 0	0.01 (1)
D-E	-20 / 0	-129.6	-129.6 0.08 (1)	6.25	L-F	-315 / 0	0.07 (1)
E-F	-20 / 0	-129.6	-129.6 0.08 (1)	6.25	K-G	-83 / 0	0.01 (1)
F-G	0 / 0	-129.6	-129.6 0.08 (1)	10.00	B-O	0 / 20	0.00 (1)
G-H	-70 / 0	-129.6	-129.6 0.17 (1)	6.25	K-H	0 / 20	0.00 (1)
H-I	0 / 49	-129.6	-129.6 0.17 (1)	10.00			
P-B	-371 / 0	0.0	0.0 0.04 (1)	7.81			
J-H	-371 / 0	0.0	0.0 0.04 (1)	7.81			
P-O	0 / 0	-18.5	-18.5 0.01 (4)	10.00			
O-N	0 / 11	-18.5	-18.5 0.02 (4)	10.00			
N-M	0 / 3	-18.5	-18.5 0.02 (4)	10.00			
M-L	0 / 3	-18.5	-18.5 0.02 (4)	10.00			
L-K	0 / 11	-18.5	-18.5 0.02 (4)	10.00			
K-J	0 / 0	-18.5	-18.5 0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (H-I:1), BC=0.02/1.00 (N-O:4), WB=0.07/1.00 (E-M:1), SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)



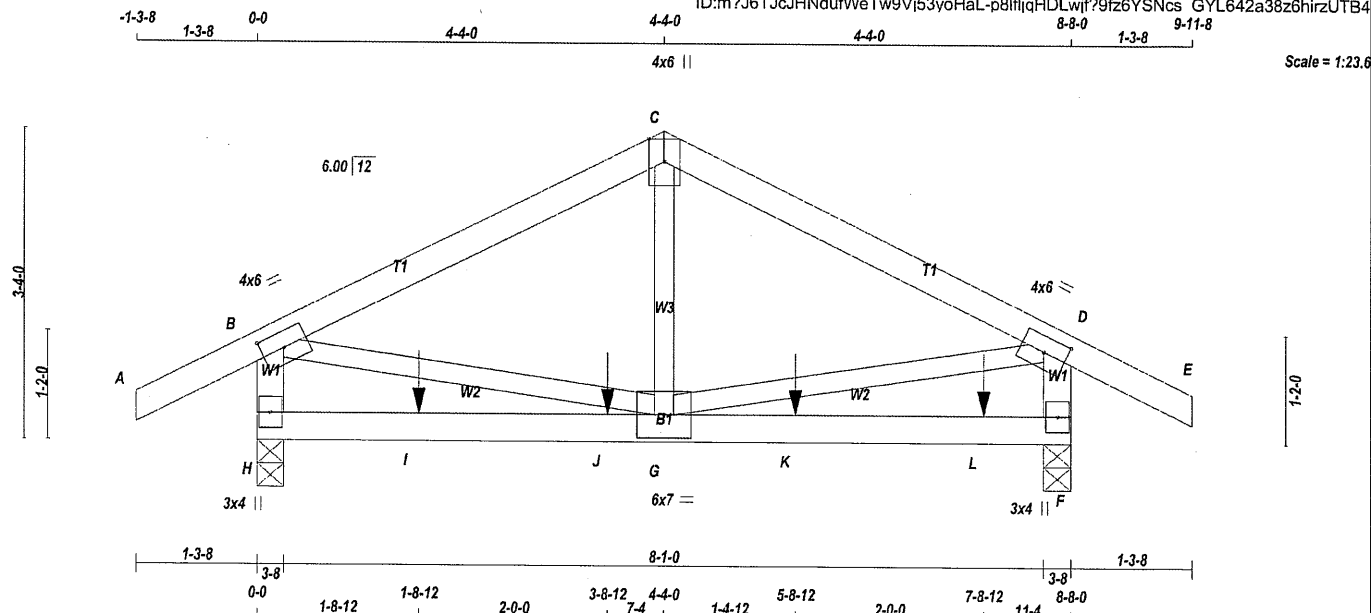
STRUCTURAL COMPONENT ONLY
DWG # TR23040313

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T98	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 35 = 70 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
A - C	12	TOP
C - E	12	TOP
H - B	12	TOP
F - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H - F	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	2521	0	2521	0
F	2864	0	2864	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	WIND	DEAD
H	1746	1355 / 0	0 / 0	0 / 0	391 / 0
F	1983	1540 / 0	0 / 0	0 / 0	444 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO								
A - B	0 / 39	-129.6	-129.6	0.09 (1)	10.00	G - C	0 / 1721	0.21 (1)
B - C	-2552 / 0	-129.6	-129.6	0.27 (1)	5.36	B - G	0 / 2327	0.29 (1)
C - D	-2552 / 0	-129.6	-129.6	0.27 (1)	5.36	G - D	0 / 2327	0.29 (1)
D - E	0 / 39	-129.6	-129.6	0.09 (1)	10.00			
H - B	-2050 / 0	0.0	0.0	0.11 (1)	7.72			
F - D	-2050 / 0	0.0	0.0	0.11 (1)	7.72			
H - I	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
I - J	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
J - G	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
G - K	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
K - L	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
L - F	0 / 0	-18.5	-18.5	0.85 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	1-8-12	-649	-649	---	FRONT	VERT	TOTAL	---	C1
J	3-8-12	-649	-649	---	FRONT	VERT	TOTAL	---	C1
K	5-8-12	-649	-649	---	FRONT	VERT	TOTAL	---	C1
L	7-8-12	-649	-649	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 54.3 P.S.F. 'G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL)= L/837 (0.12")

CSI: TC=0.27/1.00 (C-D:1), BC=0.85/1.00 (G-H:1), WB=0.29/1.00 (B-G:1), SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

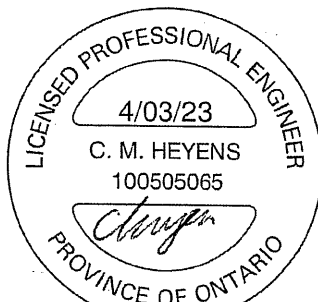
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (G) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR23040318

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



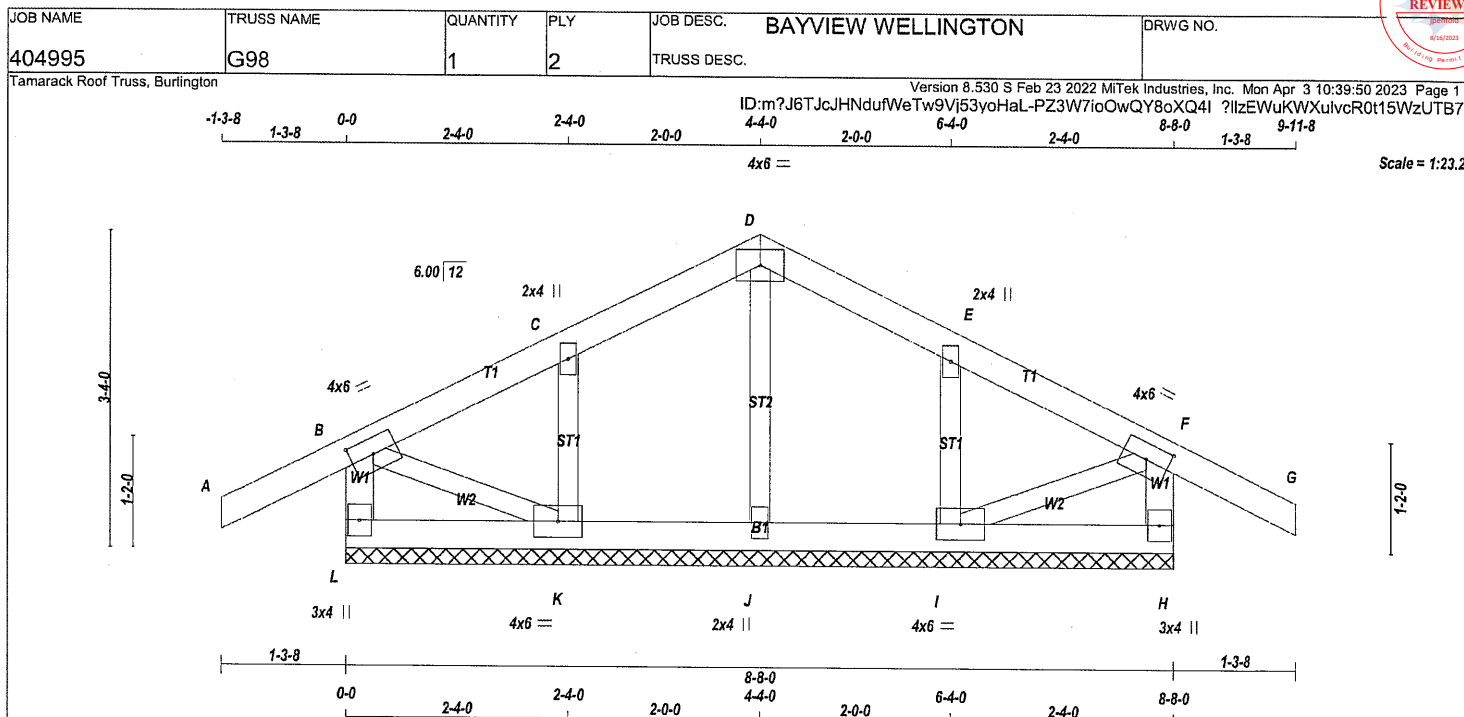
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T98	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTVW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	6.0	2.00	3.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	6.0	7.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

STRUCTURAL COMPONENT ONLY
DWG # TR23040318



TOTAL WEIGHT = 2 X 35 = 69 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	TOP
D - G	12	TOP
L - B	12	TOP
H - F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - H	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	4.0	6.0	2.00	3.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMW-t	MT20	4.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
A-B	0 / 39	-129.6	-129.6 0.08 (1)	J-D	-188 / 0	10.00	0.02 (1)
B-C	-6 / 0	-129.6	-129.6 0.05 (1)	K-C	-345 / 0	10.00	0.03 (1)
C-D	-25 / 0	-129.6	-129.6 0.05 (1)	L-E	-345 / 0	6.25	0.03 (1)
D-E	-25 / 0	-129.6	-129.6 0.05 (1)	B-K	0 / 20	6.25	0.00 (1)
E-F	-6 / 0	-129.6	-129.6 0.05 (1)	I-F	0 / 20	10.00	0.00 (1)
F-G	0 / 39	-129.6	-129.6 0.08 (1)				
L-B	-306 / 0	0.0	0.0 0.02 (1)			7.81	
H-F	-306 / 0	0.0	0.0 0.02 (1)			7.81	
L-K	0 / 0	-18.5	-18.5 0.01 (4)			10.00	
K-J	0 / 7	-18.5	-18.5 0.01 (4)			10.00	
J-I	0 / 7	-18.5	-18.5 0.01 (4)			10.00	
I-H	0 / 0	-18.5	-18.5 0.01 (4)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (F-G:1), BC=0.01/1.00 (H-I:4), WB=0.03/1.00 (E-I:1), SSI=0.06/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

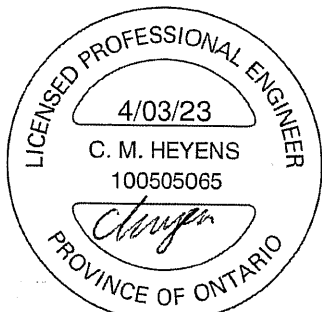
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (C) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR23040314

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G98	1	2	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	BMWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040314

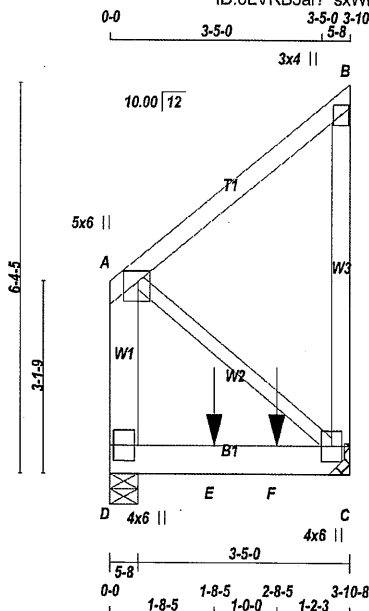


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
402098	T390	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:35.9

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS					
D - A	2x6	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A	12	TOP
A-B	12	TOP
B-C	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
D	943	0	943	0
C	1142	0	1142	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4'-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	656	495 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
C	793	600 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
D-A	-251 / 0	0.0	0.01 (1)	A-C	0 / 0	0.00 (1)	
A-B	0 / 0	-129.6	-129.6 (1)				
C-B	-251 / 0	0.0	0.09 (1)				
D-E	0 / 0	-18.5	-18.5 (1)				
E-F	0 / 0	-18.5	-18.5 (1)				
F-C	0 / 0	-18.5	-18.5 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-8-5	-524	-524	---	FRONT	VERT	TOTAL	---	C1
F	2-8-5	-524	-524	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 27 = 110 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.18/1.00 (A-B:1), BC=0.42/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR23040071

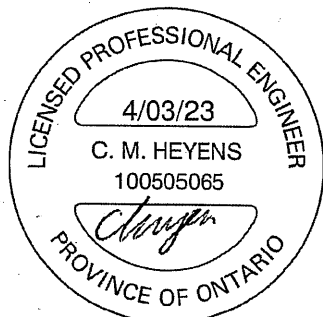


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	T390	2	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	4.0	6.0		

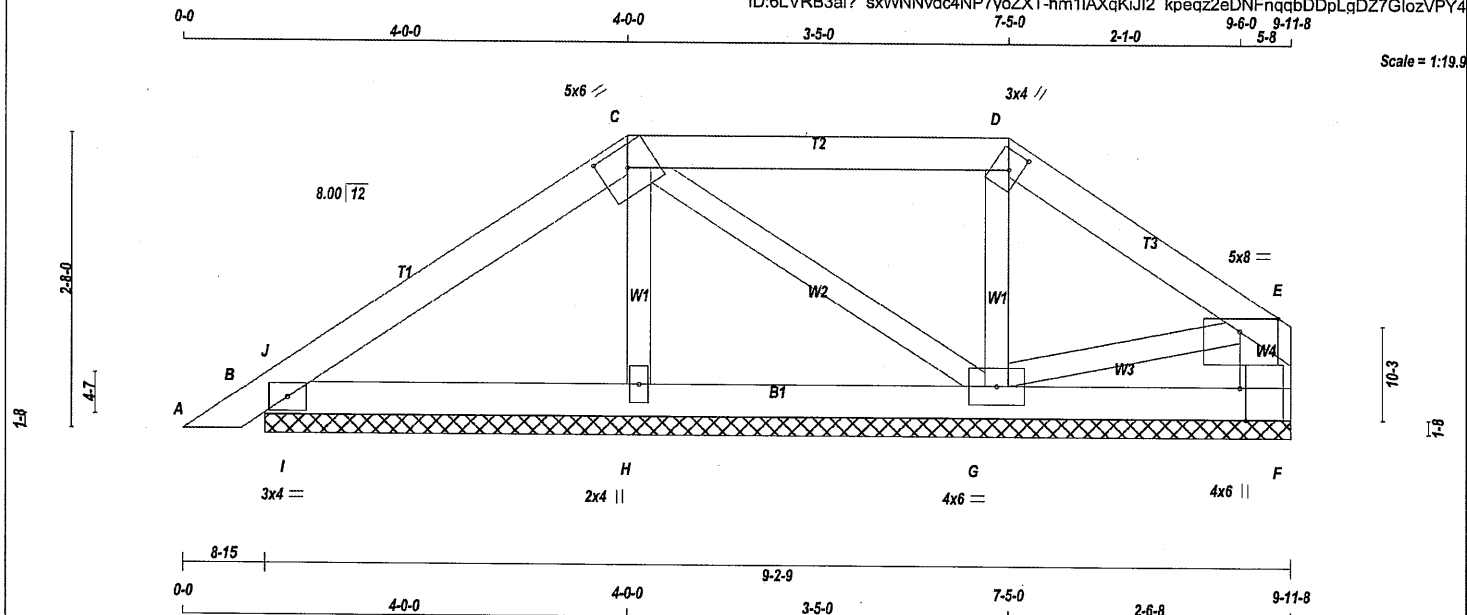


STRUCTURAL COMPONENT ONLY
DWG # TR23040071



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	P1	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:21 2023 Page 1
ID:6LVRB3al? sxWNNvdc4NP7yoZXT-hm1lAXqKIJl2 kpeqz2eDNFngqbDDpLgDZ7GlozVPY4



TOTAL WEIGHT = 32 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x6	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-h	MT20	5.0	6.0	2.25	3.00
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TWVWm	MT20	5.0	8.0	1.50	4.00
F	BMV1+p	MT20	4.0	6.0	Edge	0.75
G	BMVWW1-t	MT20	4.0	6.0		
H	BMV1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
F	240	0	0	240	0	0	0	9-2-9	9-2-9
B	391	0	0	391	0	0	0	9-2-9	9-2-9
H	397	0	0	397	0	0	0	9-2-9	9-2-9
G	423	0	0	423	0	0	0	9-2-9	9-2-9

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	166	129 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	270	214 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
H	278	197 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
G	296	215 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 22	-129.6	-129.6 0.04 (1)	H-C	-277 / 0	0.04 (1)	
B-J	-54 / 0	-129.6	-129.6 0.06 (1)	C-G	-83 / 0	0.02 (1)	
J-C	-142 / 0	-129.6	-129.6 0.16 (1)	G-D	-347 / 0	0.06 (1)	
C-D	-30 / 0	-129.6	-129.6 0.26 (1)	G-E	0 / 62	0.01 (1)	
D-E	-74 / 0	-129.6	-129.6 0.14 (1)	I-J	-350 / 0	0.00 (1)	
F-E	-218 / 0	0.0	0.0 0.01 (1)				
B-I	0 / 111	-18.5	-18.5 0.16 (1)				
I-H	0 / 111	-18.5	-18.5 0.16 (1)				
H-G	0 / 98	-18.5	-18.5 0.08 (1)				
G-F	0 / 0	-18.5	-18.5 0.03 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-D:1), BC=0.16/1.00 (B-I:1), WB=0.06/1.00 (D-G:1), SSI=0.26/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

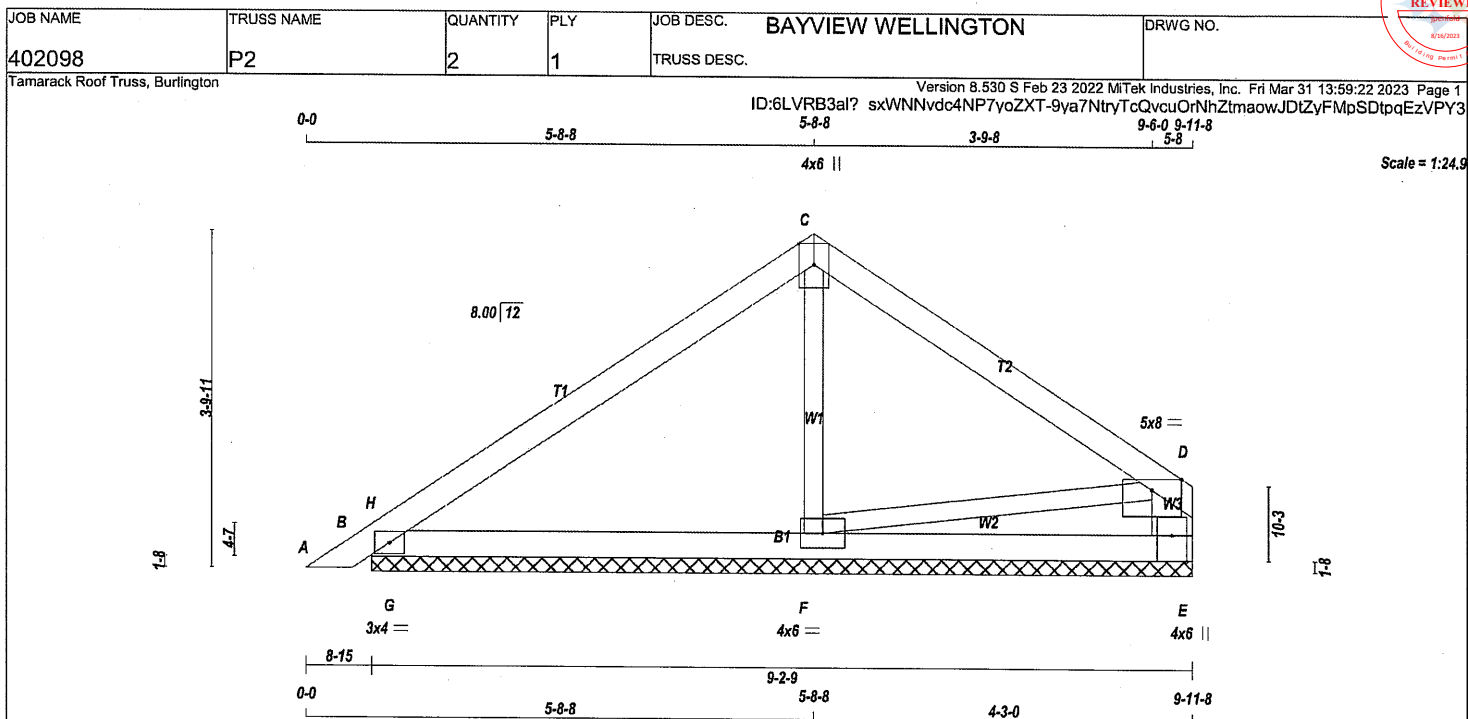
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040016



LUMBER				LUMBER	DESCR.
N. L. G. A. RULES	CHORDS	SIZE			
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x6	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVWm	MT20	5.0	8.0	1.50	4.00
E	BMV1+p	MT20	4.0	6.0	Edge	2.00
F	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E		418	0	418	0	0	9-2-9	9-2-9	
B		542	0	542	0	0	9-2-9	9-2-9	
F		490	0	490	0	0	9-2-9	9-2-9	

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	289	227 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
B	375	296 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
F	346	233 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 22	-129.6 -129.6	0.04 (1)	10.00	F-C	-327 / 0	0.07 (1)
B-H	-31 / 80	-129.6 -129.6	0.20 (1)	6.25	F-D	0 / 166	0.04 (1)
H-C	-215 / 0	-129.6 -129.6	0.39 (1)	6.25	G-H	-786 / 0	0.00 (1)
C-D	-198 / 0	-129.6 -129.6	0.40 (1)	6.25			
E-D	-406 / 0	0.0 0.0	0.03 (1)	7.81			
B-G	0 / 165	-18.5 -18.5	0.34 (1)	10.00			
G-F	0 / 165	-18.5 -18.5	0.34 (1)	10.00			
F-E	0 / 0	-18.5 -18.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (C-D:1), BC=0.34/1.00 (F-G:1), WB=0.07/1.00 (C-F:1), SSI=0.59/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

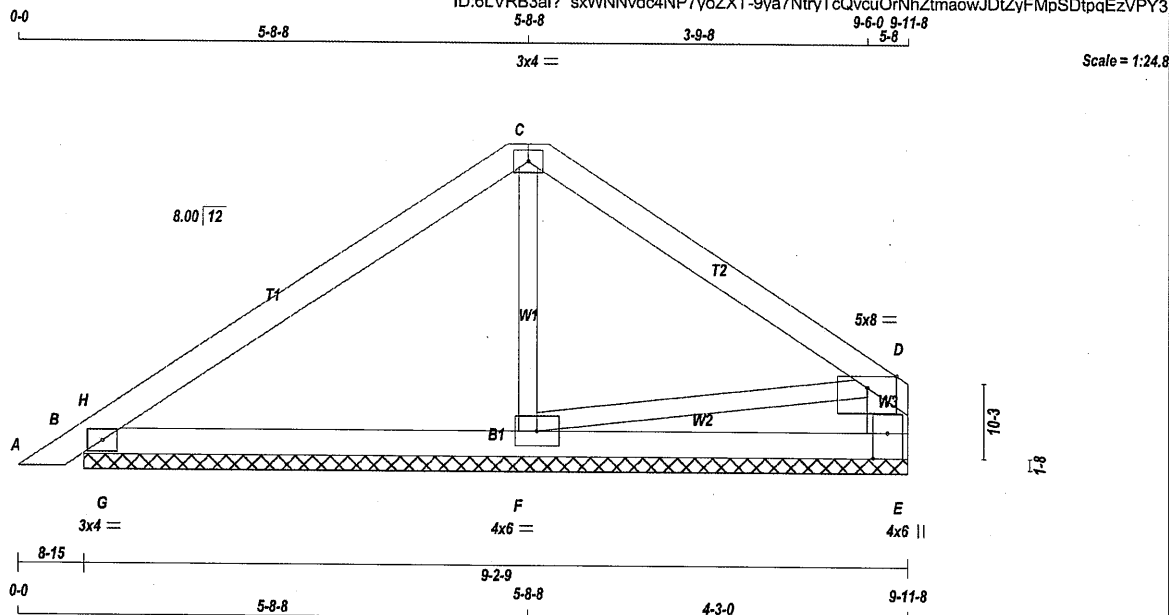


STRUCTURAL COMPONENT ONLY
DWG # TR23040017



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	P2C	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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TOTAL WEIGHT = 31 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x6	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS 2x3 DRY			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0	
C	TTW-p	MT20	3.0	4.0	
D	TMVWm	MT20	5.0	8.0	Edge
E	BMV1+p	MT20	4.0	6.0	3.50 2.00
F	BMVW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS	REACT	GROSS	REACT	BRG	IN-SX	BRG	IN-SX
E	418	0	418	0	0	9-2-9	9-2-9	9-2-9	9-2-9
B	542	0	542	0	0	9-2-9	9-2-9	9-2-9	9-2-9
F	490	0	490	0	0	9-2-9	9-2-9	9-2-9	9-2-9

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	289	227 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
B	375	296 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
F	346	233 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM			FR-TO		
A-B	0 / 22	-129.6	-129.6	0.04 (1)	10.00	F-C	-327 / 0
B-H	-31 / 80	-129.6	-129.6	0.20 (1)	6.25	F-D	0 / 166
H-C	-215 / 0	-129.6	-129.6	0.39 (1)	6.25	G-H	-786 / 0
C-D	-198 / 0	-129.6	-129.6	0.40 (1)	6.25		
E-D	-406 / 0	0.0	0.0	0.03 (1)	7.81		
B-G	0 / 165	-18.5	-18.5	0.34 (1)	10.00		
G-F	0 / 165	-18.5	-18.5	0.34 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.16 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (C-D:1), BC=0.34/1.00 (B-G:1), WB=0.07/1.00 (C-F:1), SSI=0.59/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

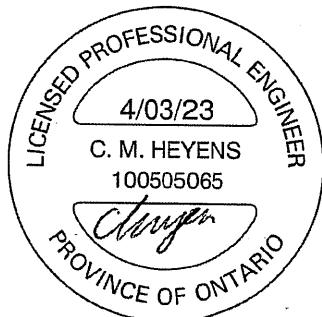
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

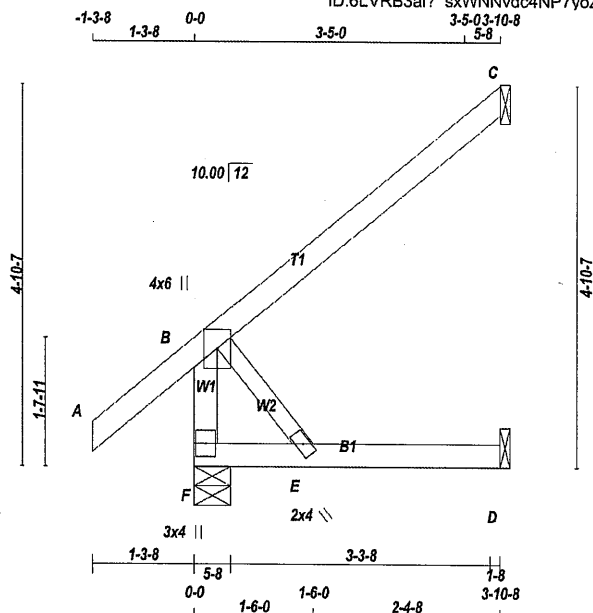


STRUCTURAL COMPONENT ONLY
DWG # TR23040018



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J3	10	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

ID:6LVRB3al? sxWNNvdc4NP7yoZXT-scfUvTI26TYvGpMVTjxEz6?IBPYzp6jnrday48zVPYA



Scale = 1:28.1

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
F	467	0	467	0
C	251	0	251	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	322	254/0	0/0	0/0	0/0	69/0	0/0
C	171	148/0	0/0	0/0	0/0	23/0	0/0
D	29	0/0	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO	F-B	-431/0	0.0	0.0	0.05 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/57	-129.6	-129.6	0.18 (5)	10.00				
B-C	0/0	-129.6	-129.6	0.33 (1)	10.00				
F-E	0/0	-18.5	-18.5	0.07 (4)	10.00				
E-D	0/0	-18.5	-18.5	0.08 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

TOTAL WEIGHT = 10 X 15 = 151 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.33/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)

JSI METAL= 0.11 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040008

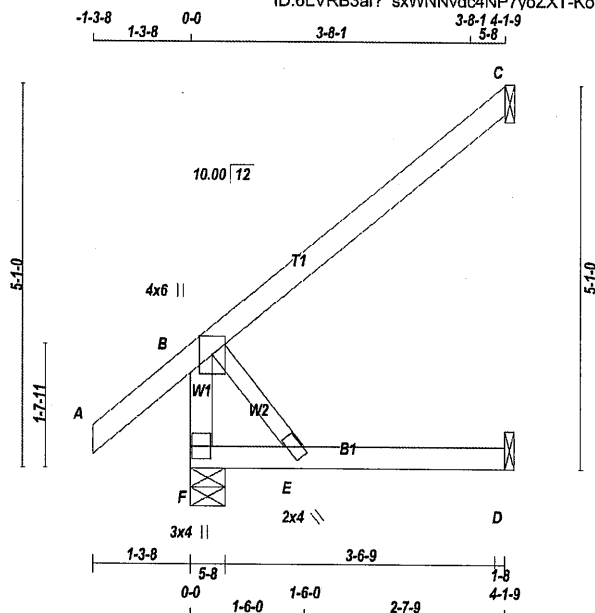


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J4	15	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

ID:6LVRB3al? sxWNNvdc4NP7yoZXT-KoDs7pmBtmgmuzwh1QSTWJYwFpu1YZzx4HQVcazVPY9

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Scale = 1:29.2

TOTAL WEIGHT = 15 X 16 = 237 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
F	485	485	5-8	5-8
C	268	268	1-8	1-8
D	38	43	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	336	264 / 0	0 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
C	183	158 / 0	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	31	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO										
F-B	-447 / 0	0.0	0.0	0.05 (1)	7.81	10.00				
A-B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	10.00				
B-C	0 / 0	-129.6	-129.6	0.38 (1)	10.00	10.00				
F-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00	10.00				
E-D	0 / 0	-18.5	-18.5	0.09 (4)	10.00	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.2 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 51.6 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.38/1.00 (B-C:1), BC=0.09/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



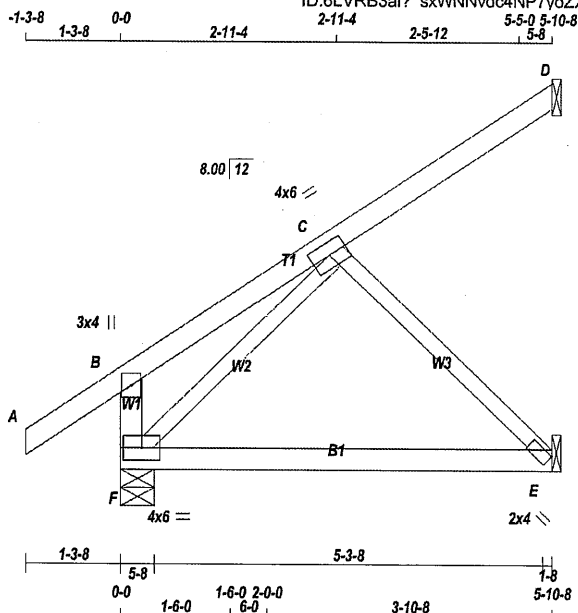
STRUCTURAL COMPONENT ONLY
DWG # TR23040009



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J5	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

ID:6LVRB3al? sxWNNvdc4NP7yoZXT-o?nEK9npe4odW7Vtb8zi3X48yCDpH7e4lx9280zVPY8

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TOTAL WEIGHT = 3 X 24 = 72 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
F - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
E	BMW1+w	MT20	2.0	4.0	Edge	0.50
F	BMW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
F	606	0	0	606	0	5-8	5-8		
D	154	0	0	154	0	1-8	1-8		
E	286	0	0	286	0	1-8	1-8		

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	419	326 / 0	0 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
D	105	91 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
E	201	137 / 0	0 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
F-B	-313 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B-C	0 / 26	-129.6	-129.6	0.19 (1)	10.00
C-D	-24 / 0	-129.6	-129.6	0.18 (1)	6.25
F-E	0 / 228	-18.5	-18.5	0.19 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.2 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.19/1.00 (B-C:1), BC=0.19/1.00 (E-F:4), WB=0.10/1.00 (C-E:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (E) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

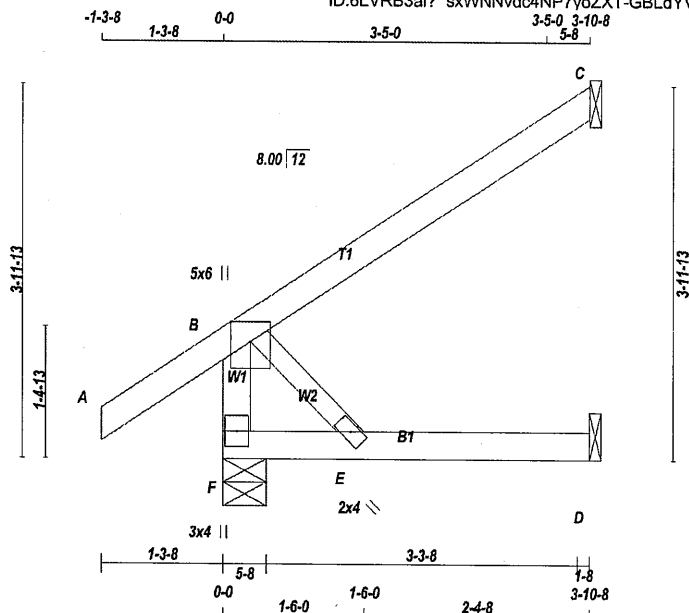


STRUCTURAL COMPONENT ONLY
DWG # TR23040010



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J6	10	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

ID:6LVRB3al? sxWNNvdc4NP7yoZXT-GBLdYVnRPOwU7G438rUxbkdGRcag0TSEXbvchTzVPY7
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TOTAL WEIGHT = 10 X 14 = 141 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	465	0	465	0	5-8	5-8
C	251	0	251	0	1-8	1-8
D	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS					
		1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
F	321	253 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	171	148 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
F-B	-429 / 0	0.0	0.0	0.04 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 49	-129.6	-129.6	0.18 (5)			
B-C	0 / 0	-129.6	-129.6	0.33 (1)			
F-E	0 / 0	-18.5	-18.5	0.07 (4)			
E-D	0 / 0	-18.5	-18.5	0.08 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.2 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 51.6 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.33/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (F) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)

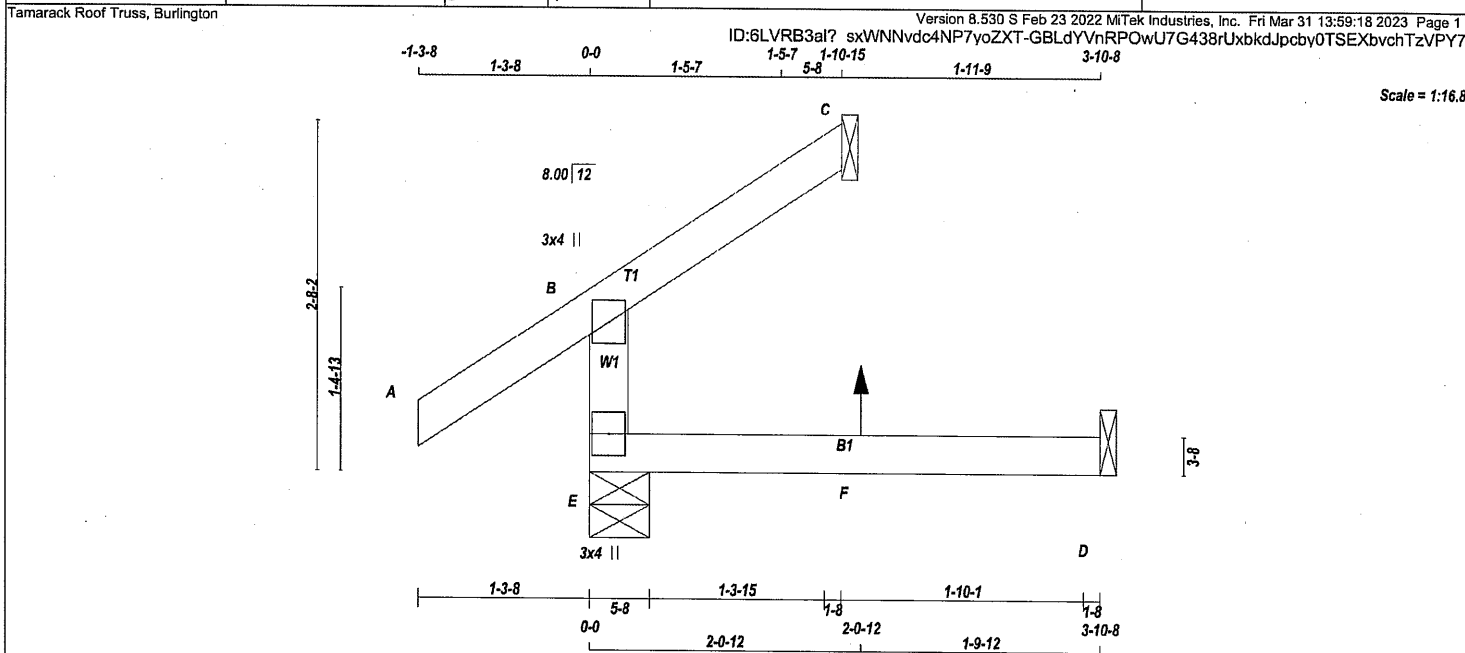
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040011



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J7	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER				DESCR.	SPF	SPF
N. L. G. A. RULES	CHORDS	SIZE	DRY			
E - B	2x4	DRY	No.2			
A - C	2x4	DRY	No.2			
E - D	2x4	DRY	No.2			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
E	390	0	0	390	0	5-8	5-8	5-8	5-8
C	74	0	74	0	-33	1-8	1-8	1-8	1-8
D	24	0	35	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	270	211 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
C	52	39 / -28	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	20	0 / -7	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE	VERT.	LOAD	MEMB.	FORCE	MAX	CS
(LBS)	(PLF)	LC1	LC2	(LBS)	(PLF)	MAX	CS
FR-TO		FROM	TO	FR-TO			
E-B	-353 / 0	0.0	0.0	0.06 (5)	7.81		
A-B	0 / 49	-129.6	-129.6	0.18 (5)	10.00		
B-C	-28 / 0	-129.6	-129.6	0.13 (6)	6.25		
E-F	0 / 0	-18.5	-18.5	0.06 (4)	10.00		
F-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	6	1	11	BACK	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.18/1.00 (A-B:5), BC=0.06/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

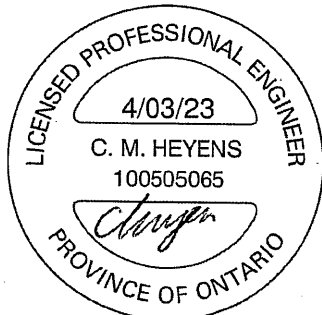
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

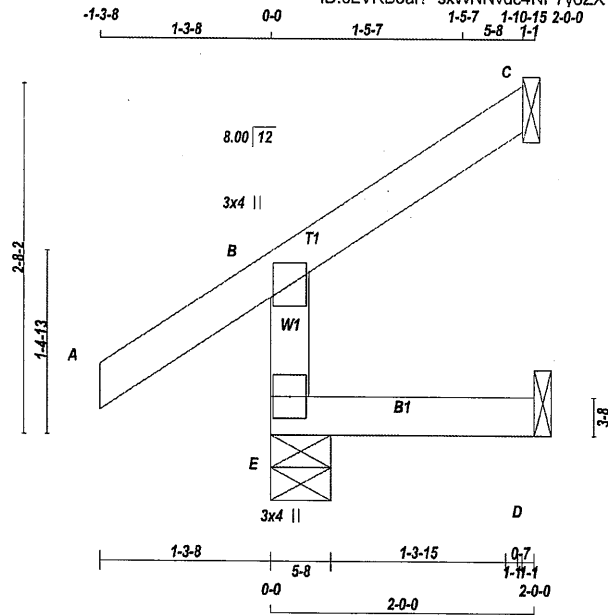
JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040012

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J8	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:19 2023 Page 1					
ID:6LVRB3al? sxWNNvdc4NP7yoZXT-kNv?Iro3Ah2KIQfGIZ?A8yAUd0xNlviNmFe9DvzVPY6					
-1-3-8 1-3-8 0-0 1-5-7 1-5-7 1-10-15 2-0-0 5-8 1-1					
Scale = 1:16.8					



Scale = 1:16.8

TOTAL WEIGHT = 3 X 8 = 24 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+tp	MT20	3.0	4.0		
E	BMV1+tp	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX
E	384	0	384	0	0	5-8	5-8	5-8	5-8
C	72	0	72	0	-34	1-8	1-8	1-8	1-8
D	7	0	18	0	-6	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		SNOW		LIVE		PERM.LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	264	215 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0	E	264	215 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
C	49	41 / -26	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0	C	49	41 / -26	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
D	7	0 / -11	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	D	7	0 / -11	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
E-B	-354 / 0	0.0	0.0 0.05 (5)	7.81			
A-B	0 / 49	-129.6	-129.6 0.17 (1)	10.00			
B-C	-29 / 0	-129.6	-129.6 0.14 (1)	6.25			
E-D	0 / 0	-18.5	-18.5 0.05 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	6.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.4	PSF
TOTAL LOAD	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.17/1.00 (A-B:1), BC=0.05/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

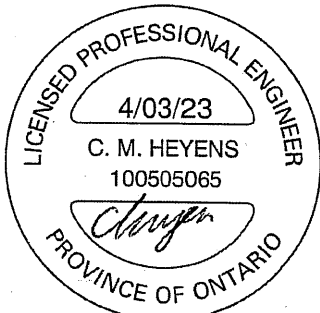
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



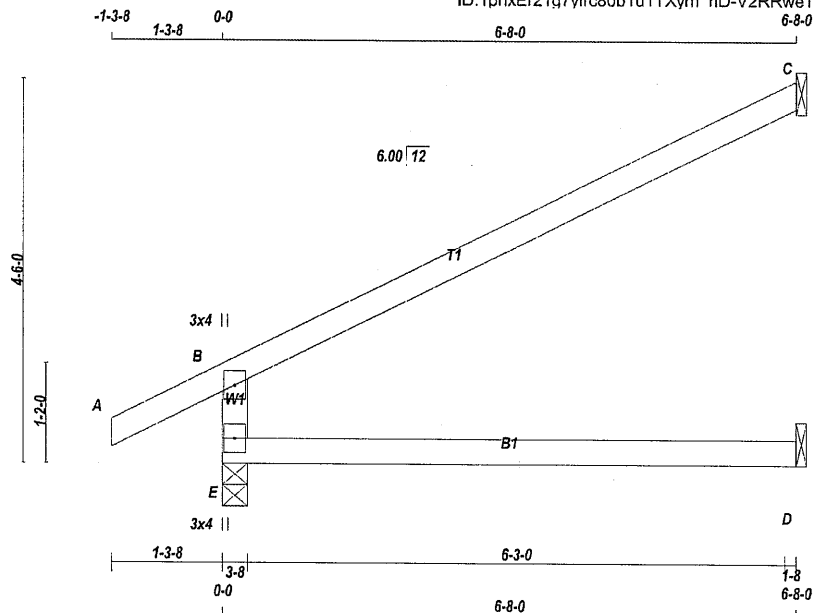
STRUCTURAL COMPONENT ONLY
DWG # TR23040013



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J50	18	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:44:54 2023 Page 1
ID:1phxEr21g7yfr80b1u1TXym hD-V2RRweT4Kqr?GFqW6qS234pJgtF76FaMLJW6mvzUT6N



Scale = 1:25.8

TOTAL WEIGHT = 18 X 19 = 337 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	INPUT BRG UPLIFT IN-SX	REQD BRG IN-SX
E	789	0	789	0	3-8	3-8
C	324	0	324	0	1-8	1-8
D	51	0	57	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	547	422 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
C	221	191 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-716 / 0	0.0	0.0	0.14 (4)	7.81		
A-B	0 / 39	-129.6	-129.6	0.17 (1)	10.00		
B-C	-48 / 0	-129.6	-129.6	0.76 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.17 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.76/1.00 (B-C:1), BC=0.17/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

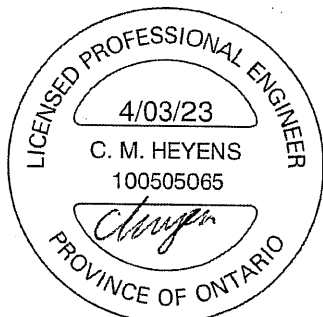
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

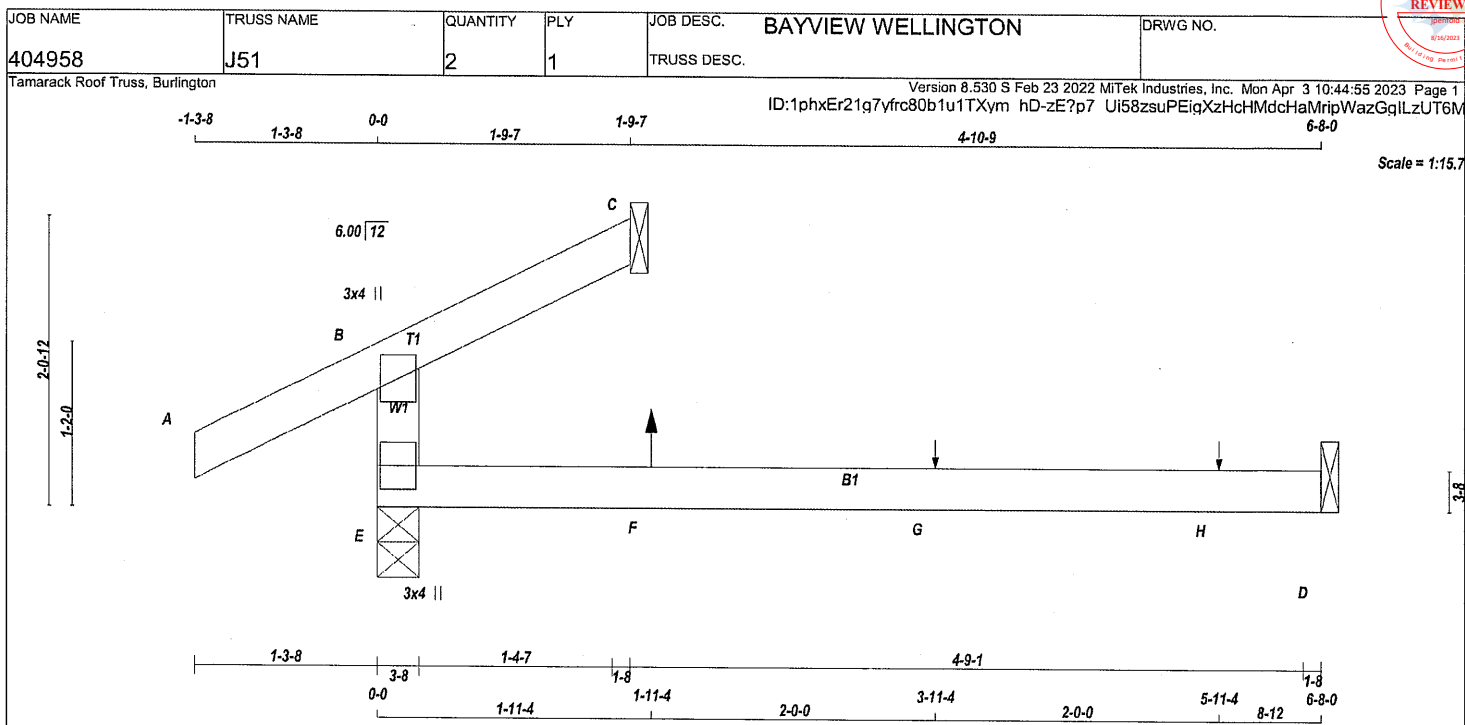
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR23040320

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



LUMBER

N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	386	0	386	0	3-8	3-8
C	86	0	86	0	1-8	1-8
D	49	0	59	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
E	268	206 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	63	30 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
D	39	0 / -3	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
E-B	-324 / 0	0.0	0.0 0.12 (4)	7.81	
A-B	0 / 39	-129.6	-129.6 0.17 (1)	10.00	
B-C	-15 / 14	-129.6	-129.6 0.12 (4)	6.25	
E-F	0 / 0	-18.5	-18.5 0.17 (4)	10.00	
F-G	0 / 0	-18.5	-18.5 0.17 (4)	10.00	
G-H	0 / 0	-18.5	-18.5 0.17 (4)	10.00	
H-D	0 / 0	-18.5	-18.5 0.17 (4)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	5-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 12 = 25 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.17/1.00 (A-B:1), BC=0.17/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



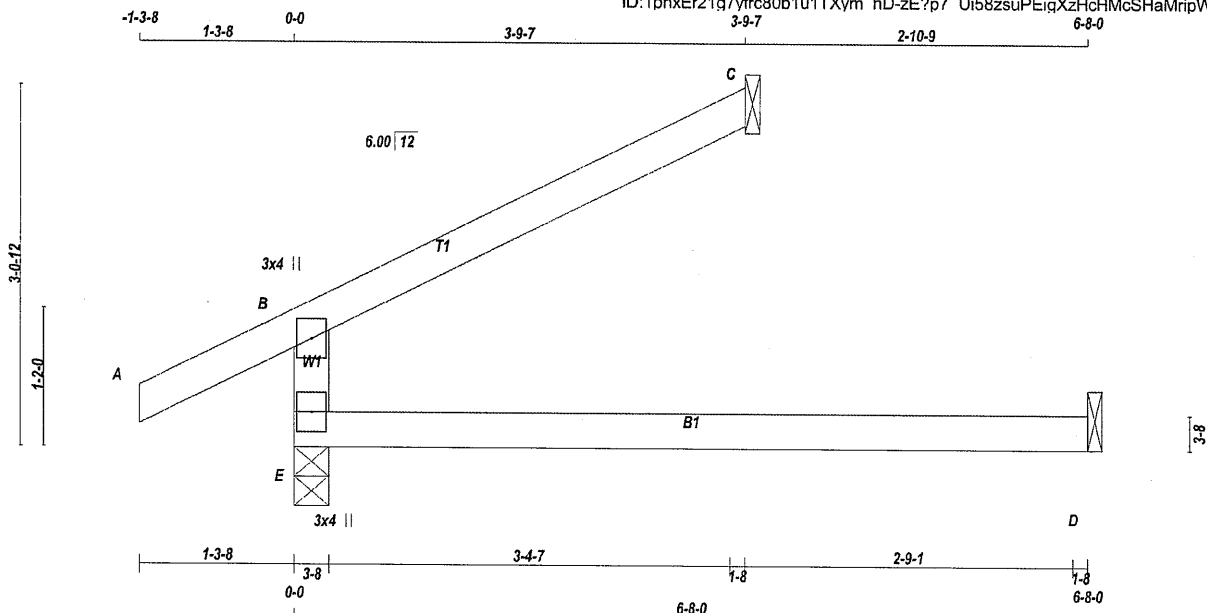
STRUCTURAL COMPONENT ONLY
DWG # TR23040321

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J52	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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ID:1phxEr21q7yfrC80b1u1TXym hD-zE?p7 Ui58zsuPEigXzHcHMcSHaMripWazGqILzUT6M



Scale = 1:18.7

TOTAL WEIGHT = 2 X 15 = 30 lb [MIF]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	555	0	555	0	3-8	3-8
C	184	0	184	0	1-8	1-8
D	51	0	57	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) E, C

UNFACTORED REACTIONS

JT	1ST LC CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	387	284 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
C	126	109 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO		
E-B	-483 / 0	0.0	0.0 0.15 (4)	7.81			
A-B	0 / 39	-129.6	-129.6 0.17 (1)	10.00			
B-C	-27 / 0	-129.6	-129.6 0.24 (1)	6.25			
E-D	0 / 0	-18.5	-18.5 0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.24/1.00 (B-C:1), BC=0.17/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR23040322

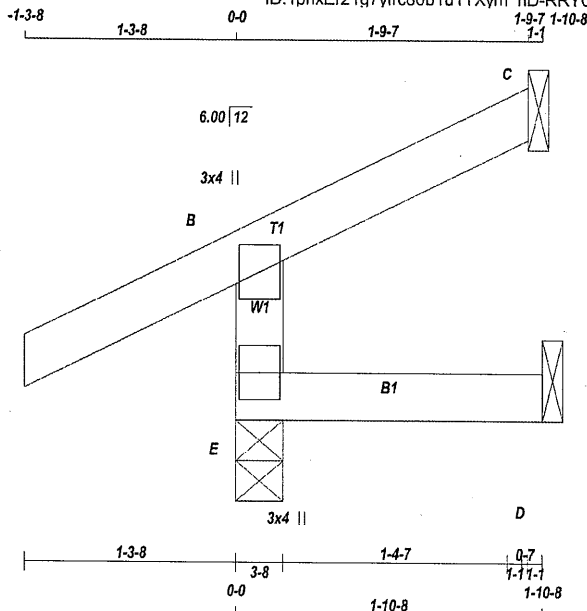
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J54	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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Scale = 1:13.6

TOTAL WEIGHT = 2 X 7 = 14 lb

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE		LUMBER		DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
E	375	0	375	0	3-8	3-8
C	62	0	62	0	1-8	1-8
D	5	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	258	211 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	43	36 / -27	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	5	0 / -12	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MAX. FACTORED UNBRACED LENGTH (LC)
FR-TO				
E-B	-345 / 0	0.0	0.0	0.05 (5)
A-B	0 / 39	-129.6	-129.6	0.17 (1)
B-C	-24 / 0	-129.6	-129.6	0.13 (1)
E-D	0 / 0	-18.5	-18.5	0.05 (5)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.17/1.00 (A-B:1), BC=0.05/1.00 (D-E:5)
WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

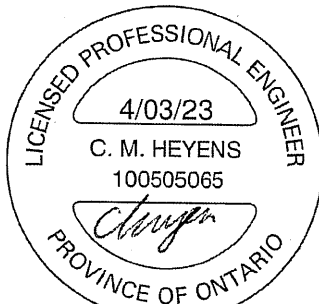
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



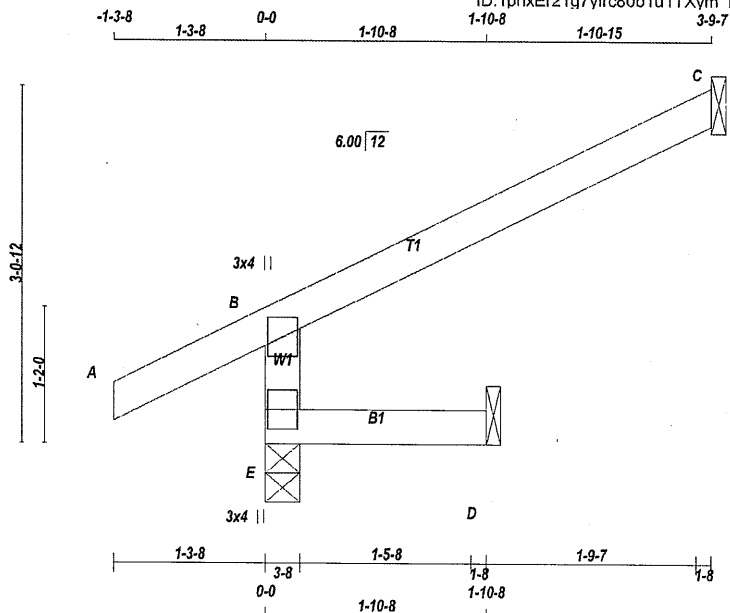
STRUCTURAL COMPONENT ONLY
DWG # TR23040324

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J55	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:18.9

TOTAL WEIGHT = 2 X 10 = 19 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG UPLIFT IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	502	0	502	0	3-8	3-8
C	184	0	184	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	344	284 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	126	109 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
FR-TO							
E-B	-483 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 39	-129.6	-129.6	0.17 (1)	10.00		
B-C	-27 / 0	-129.6	-129.6	0.32 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.32/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

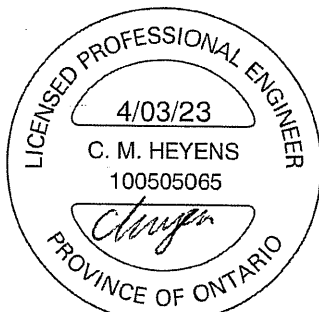
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR23040325

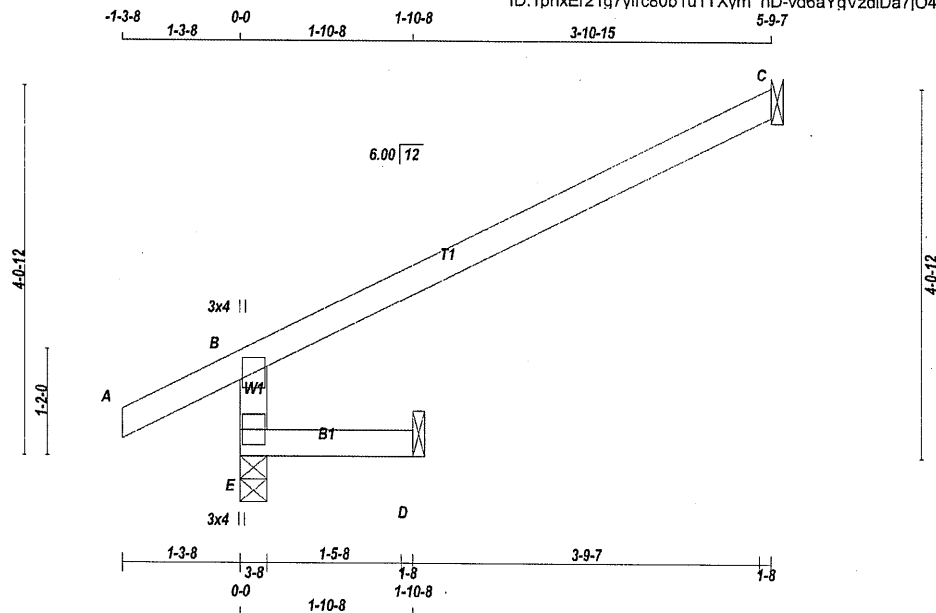
LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J56	2	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale: 1/2"=1'

TOTAL WEIGHT = 2 X 12 = 24 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2	DESCR.	SPF
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
E	664	0	664	0
C	281	0	281	0
D	16	0	17	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
E	455	380 / 0	0 / 0	0 / 0	75 / 0	0 / 0
C	192	166 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
E-B		-645 / 0	0.0	0.0	0.01 (1)	E-B		7.81	7.81
A-B		0 / 39	-129.6	-129.6	0.17 (1)	A-B		10.00	10.00
B-C		-42 / 0	-129.6	-129.6	0.74 (1)	B-C		6.25	6.25
E-D		0 / 0	-18.5	-18.5	0.02 (4)	E-D		10.00	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.74/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)



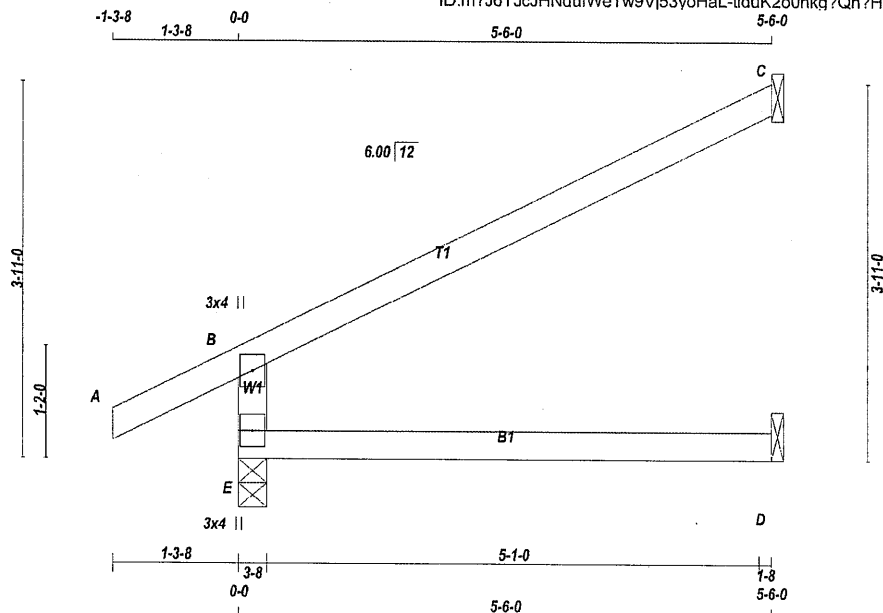
STRUCTURAL COMPONENT ONLY
DWG # TR23040326

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	J57	5	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 5 X 16 = 79 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	681	0	681	0	0	3-8	3-8
C	267	0	267	0	0	1-8	1-8
D	42	0	47	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	472	366 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
C	182	158 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH
FR-TO		FROM	TO		FR-TO		
E-B	-621 / 0	0.0	0.0	0.11 (4)	7.81		
A-B	0 / 39	-129.6	-129.6	0.17 (1)	10.00		
B-C	-40 / 0	-129.6	-129.6	0.67 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.11 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.67/1.00 (B-C:1), BC=0.11/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

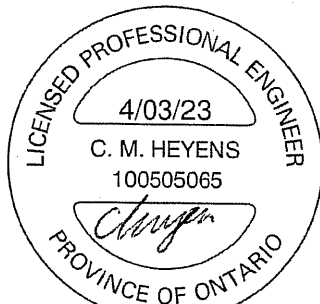
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

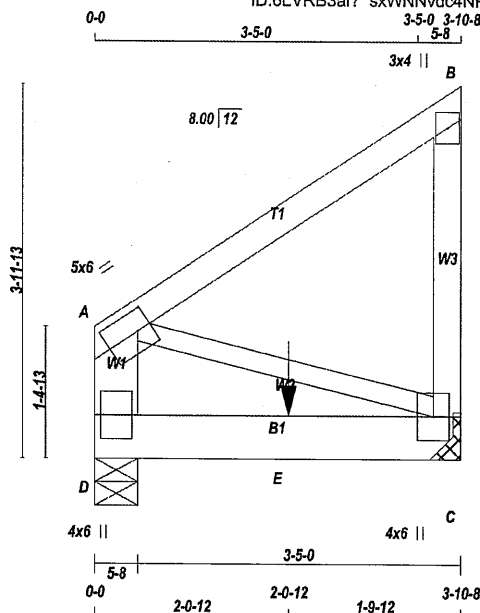


STRUCTURAL COMPONENT ONLY
DWG # TR23040315

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J60	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Fri Mar 31 13:59:20 2023 Page 1
ID:6LVRB3al? sxWNNvdc4NP7yoZXT-CaTNzBphx?ABNaESGXPg9ifQBByUMyX vOjLzVPY5

Scale = 1:23.5

TOTAL WEIGHT = 2 X 20 = 41 lb

LUMBER				DESCR.	
CHORDS	SIZE	LUMBER		SPF	
D - A	2x6	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 2	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
D	815	0	815	0	5-8	5-8
C	888	0	888	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS					SOIL
	SNOW	LIVE	PERM.LIVE	WIND	DEAD			
D	566	431/0	0/0	0/0	135/0	0/0		0/0
C	616	470/0	0/0	0/0	146/0	0/0		0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. CS1 (LC)
FR-TO				FR-TO			
D-A	-251/0	0.0	0.0 0.01 (1)	A-C	0/0	7.81	0.00 (1)
A-B	0/0	-129.6	-129.6 0.18 (1)			10.00	
C-B	-251/0	0.0	0.0 0.03 (1)			7.81	
D-E	0/0	-18.5	-18.5 0.41 (1)			10.00	
E-C	0/0	-18.5	-18.5 0.41 (1)			10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-0-12	-782	-782	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.18/1.00 (A-B:1), BC=0.41/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

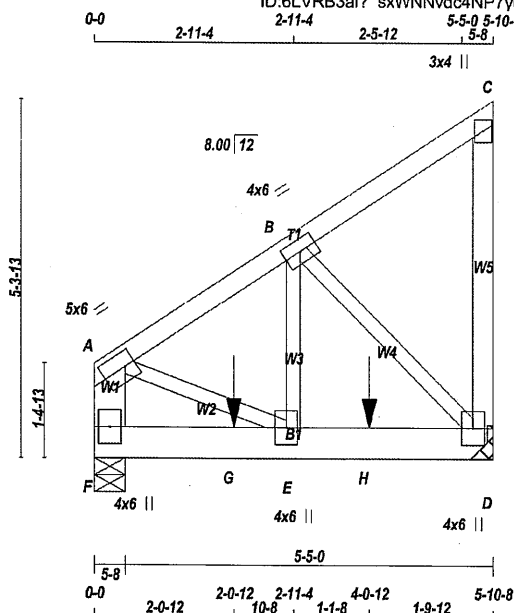
LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

STRUCTURAL COMPONENT ONLY
DWG # TR23040014



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J500	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:32.7

TOTAL WEIGHT = 2 X 33 = 65 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	F - A	2x6	DRY	No.2	SPF
	A - C	2x4	DRY	No.2	SPF
	D - C	2x4	DRY	No.2	SPF
	F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMVW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
F	1545	1545	0	5-8
D	1644	1644	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1073	819 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0
D	1141	872 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
F-A	-1331 / 0	0.0	0.0	0.05 (1)	A-E	0 / 1239	0.15 (1)
A-B	-1388 / 0	-129.6	-129.6	0.10 (1)	E-B	0 / 1502	0.19 (1)
B-C	-22 / 0	-129.6	-129.6	0.09 (1)	B-D	-1664 / 0	0.24 (1)
D-C	-150 / 0	0.0	0.0	0.03 (1)			
F-G	0 / 0	-18.5	-18.5	0.17 (1)			
G-E	0 / 0	-18.5	-18.5	0.17 (1)			
E-H	0 / 1173	-18.5	-18.5	0.27 (1)			
H-D	0 / 1173	-18.5	-18.5	0.27 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-803	-803	---	FRONT	VERT	TOTAL	---	C1
H	4-0-12	-803	-803	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	51.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.10/1.00 (A-B:1), BC=0.27/1.00 (D-E:1), WB=0.24/1.00 (B-D:1), SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	MAX MIN	MAX MIN	MAX MIN
	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)
JSI METAL= 0.26 (D) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR23040015

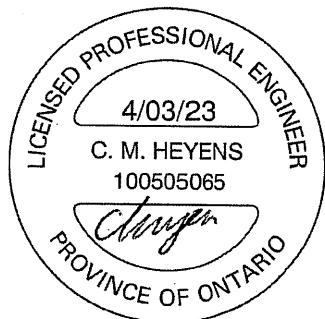


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402098	J500	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Mar 31 13:59:20 2023 Page 2
ID:6LVRB3aI? sxWNNvdc4NP7yoZXT-CaTNzBphx?ABNaESGGXPg9iqcQD8UJHX vOjILzVPY5

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040015

TECH-NOTES

TN 15-001

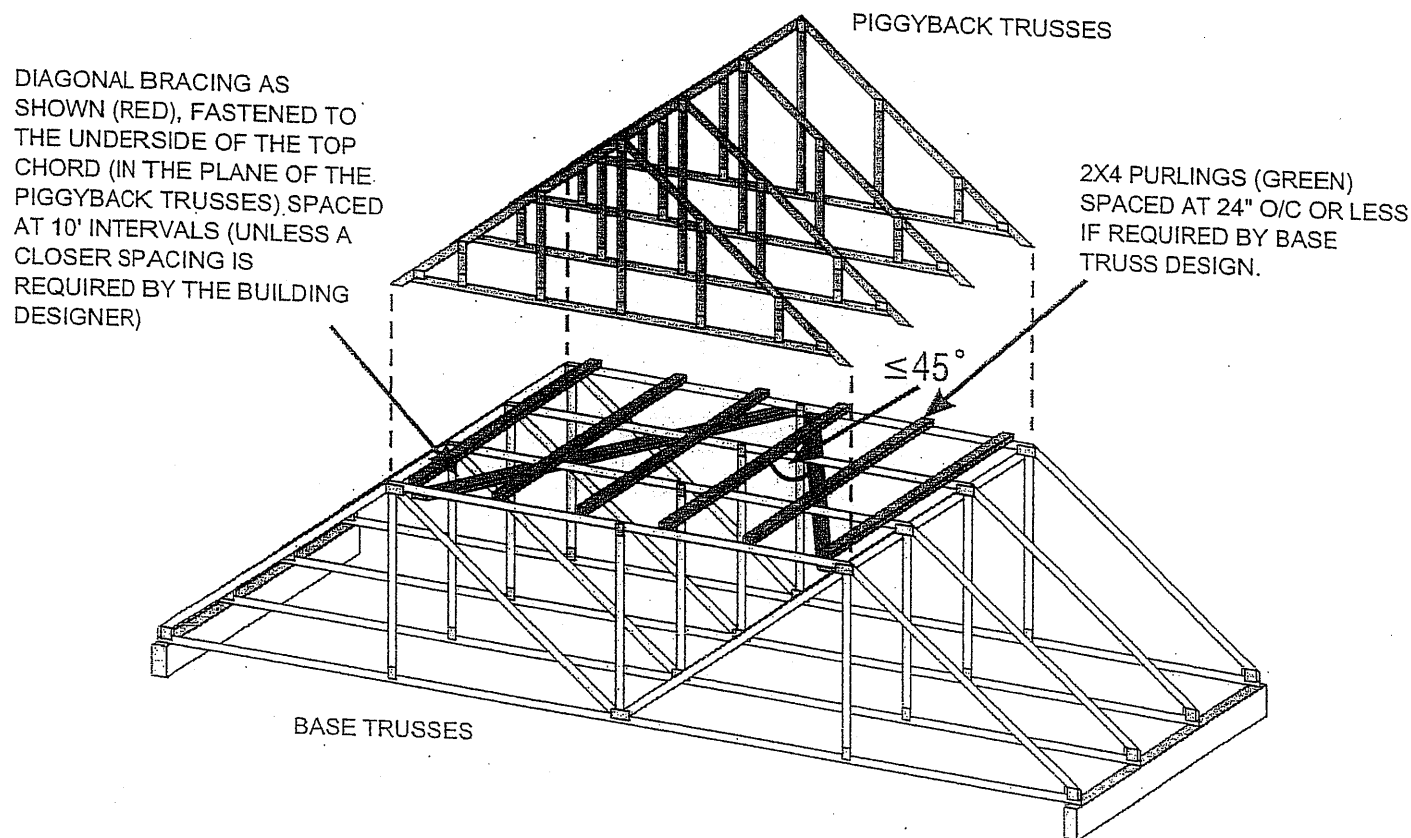
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

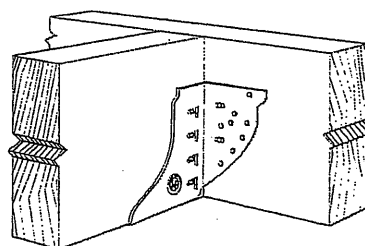
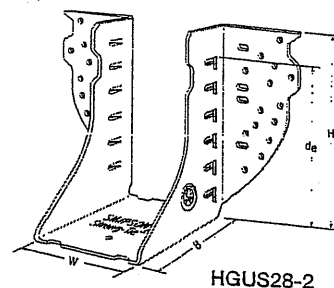
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

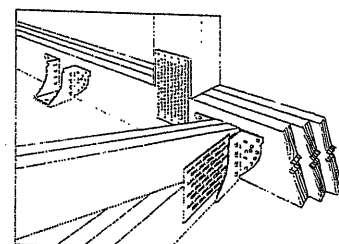
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



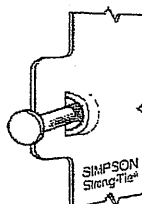
Typical HGUS Installation



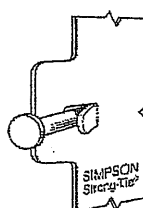
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1½	5¾	5	4¾ ₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3¾ ₁₆	5¾ ₁₆	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½ ₁₆	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6¾ ₁₆	5¾ ₁₆	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3¾ ₁₆	7¾ ₁₆	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½ ₁₆	7¼	4	6¾	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6¾ ₁₆	7¾ ₁₆	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7¾	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3¾ ₁₆	9¾ ₁₆	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½ ₁₆	9¼	4	8¾	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6¾ ₁₆	9¾ ₁₆	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6¾ ₁₆	10¾	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6¾ ₁₆	12¾	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

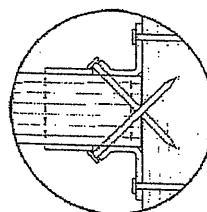
1. d_g is the distance from the seat of the hanger to the highest joist nail.



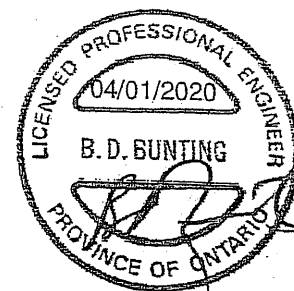
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

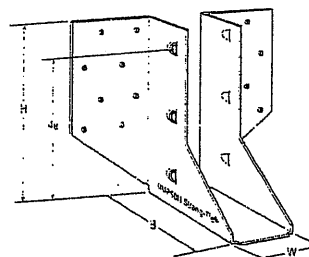
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

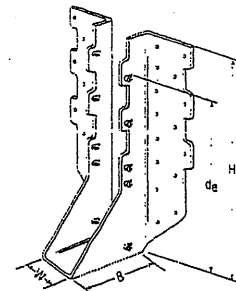
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

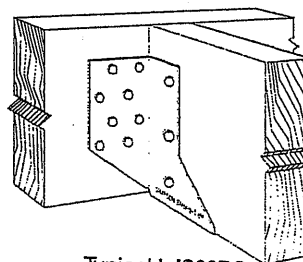
- See current catalogue for options



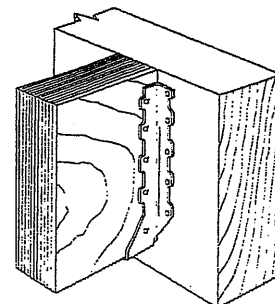
LJS26DS



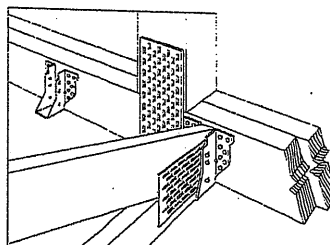
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



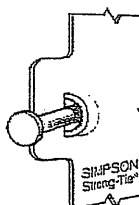
Typical HUS
Installation



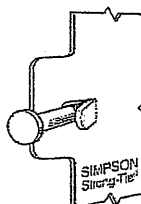
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LJS26DS	18	1 3/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 5/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 5/8	9 3/32	3	7 3/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

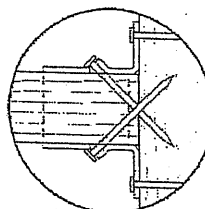
1. d_e is the distance from the seat of the hanger to the highest joist nail.



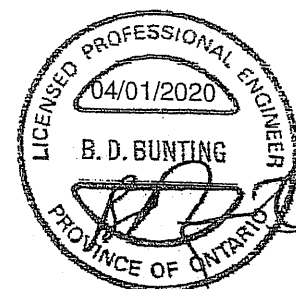
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

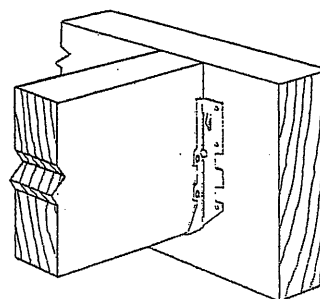
Finish: G90 galvanized

Design:

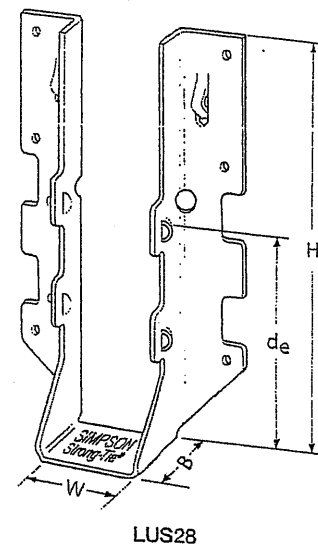
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



Typical LUS Installation

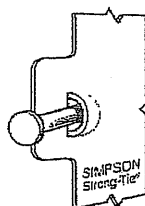


Options:

- These hangers cannot be modified

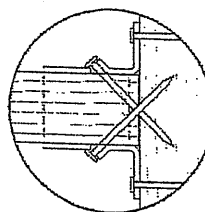
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1⅞	3½	1¾	1⅝	(4) 10d	(2) 10d	(K _o =1.15)	(K _o =1.00)	(K _o =1.15)	(K _o =1.00)
LUS24-2	18	3⅞	3½	2	1⅝	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¾	3⅞	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¼	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8⅞	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

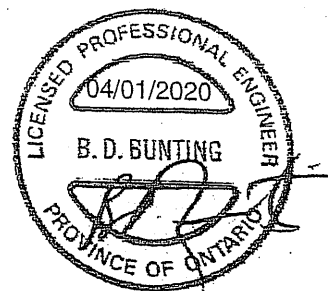


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

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BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

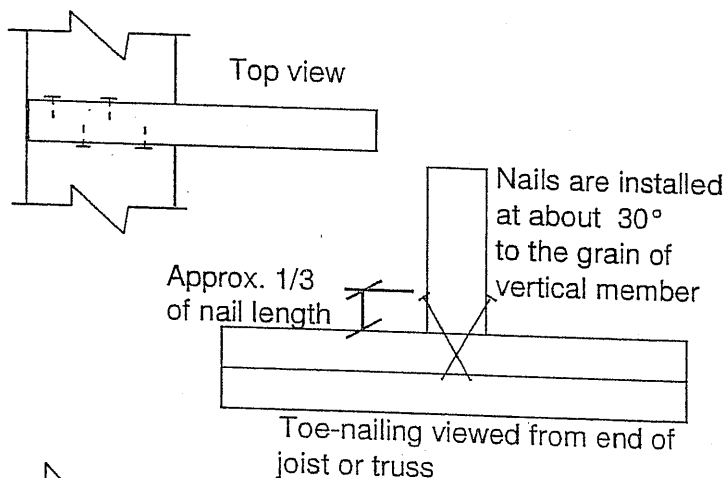
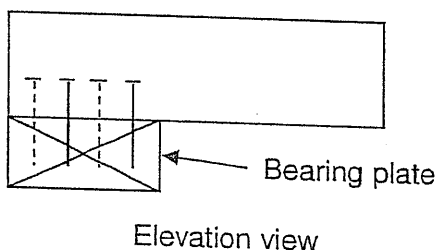
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

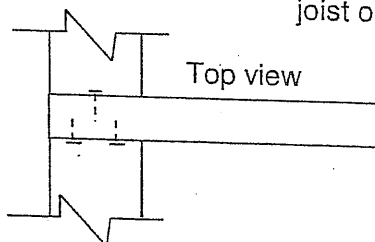
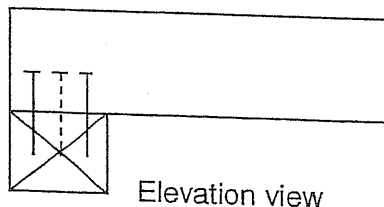
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



December 2, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF					
	2	2	3	3	3
2x6 SPF					
	4	4	4	5	5
2x4 D. FIR					
	2	2	2	2	2
2x6 D. FIR					
	3	3	3	4	4

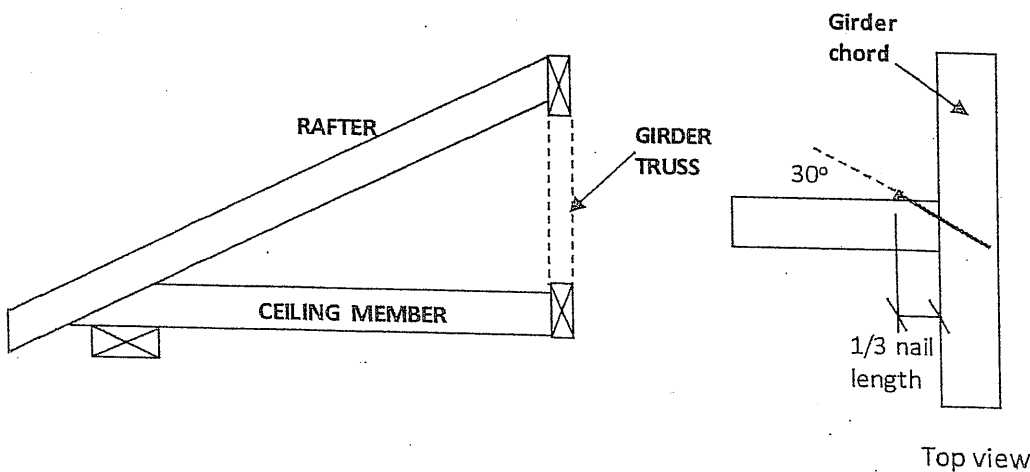


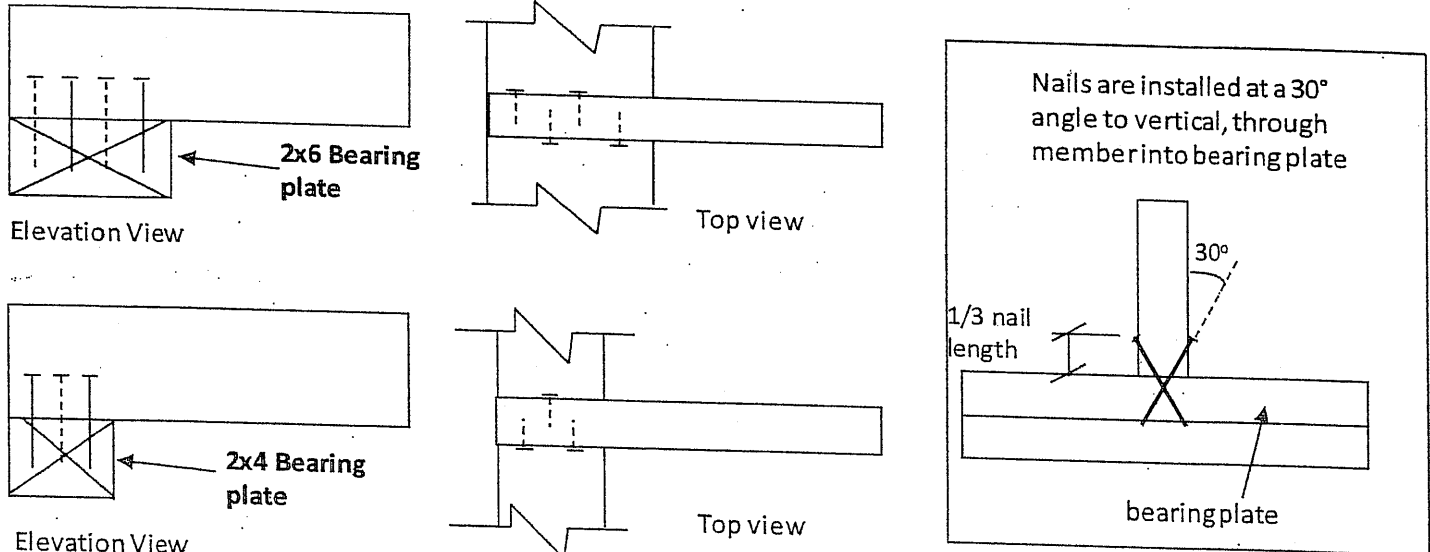
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

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TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

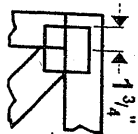
THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

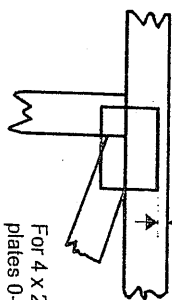
Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless X, Y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



0-1/16"



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MITek 20/20 software or upon request.

PLATE SIZE

4 X 4

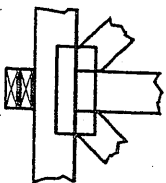
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or L bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

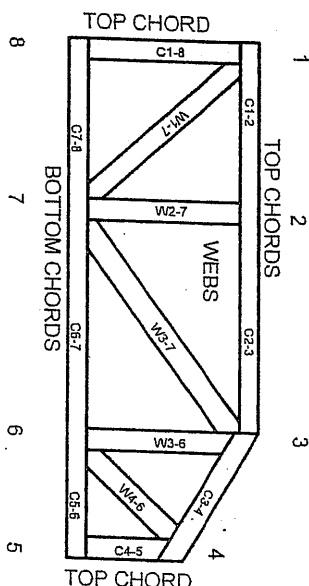
ANSI/ITP1: National Design Specification for Metal Plate Connected Wood Truss Construction.

DSB-89: Design Standard for Bracing, Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate

Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

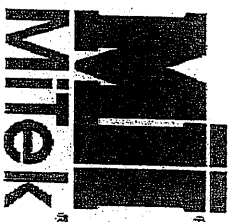
ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T or L bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.