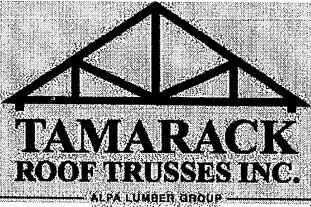


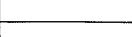


ALL ROOF SLOPES ARE 10:12
UNLESS OTHERWISE NOTED

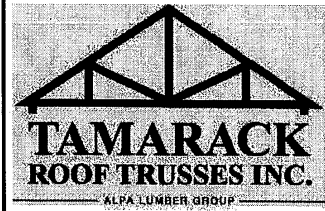
 TAMARACK BUILDING SOLUTIONS, INC. Member of the Tamarack Group	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Division: Block 148B / UNITS 1-3	Visit ref 83333
	Plan Log: 204746	Request: ALCONA SHORES	Scope: FINISHING CONCRETE THE PROPERTY OF TAMARACK ROOF TILERS INC. SHALL NOT BE REIMBURSED, PUBLISHED, OR REPRODUCED WITHOUT THE WRITTEN PERMISSION OF TAMARACK ROOF TILERS INC. AND WILL BE REFACTORED BY TAMARACK ROOF TILERS INC. NOT UTILIZED FOR ANY OTHER PURPOSE.	
Layered ID: 418815	Date: 2024-03-04	Designer: xx	Designer: xx	

		DELIVERY SHIPLIST	
TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: ALCONA SHORES Location: INNISFIL Model: BLOCK 148B Lot #: Elevation: EL: B UNIT 1 BLK 148B	Job Track: 50469 PlanLog: 204746 Layout ID: 418816 Ref # Page: 1 of 3 Date: 03-24-2023 Designer: Sales Rep: Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T98Z Jack-Open Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T99 Jack-Closed Structural Gable	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	47.01 32.00		
	1	T162 Hip	10 /12	23-06-00	5-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	115.4 71.83		
	1	T163 Hip	10 /12	23-06-00	6-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	125.54 80.83		
	1	T164 Hip	10 /12	23-06-00	7-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	126.63 79.17		
	1	T165 Hip	10 /12	23-06-00	8-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	133.18 83.67		
	1 2-ply	T170 Hip Girder	10 /12	21-10-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	218.6 139.33		
	1	T171 Hip	10 /12	21-10-00	6-06-07	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	114.66 73.17		
	1	T172 Hip	10 /12	21-10-00	8-02-07	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	122.4 78.33		
	1	T173 Hip	10 /12	21-10-00	9-10-07	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	130.51 83.33		
	1	T174 Common	10 /12	21-10-00	10-11-06	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	129.12 80.00		
	5	T176 Common	10 /12	12-00-00	6-07-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	290.49 185.00		
	1 2-ply	T181X Hip Girder	10 /12	24-00-00	4-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	206.19 132.67		
	1	T182 Hip	10 /12	23-06-00	9-01-04	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	134.9 84.50		

DELIVERY SHIPLIST



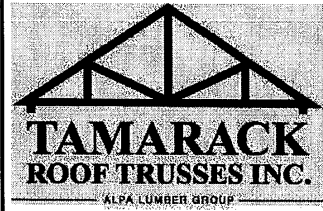
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148B
 Lot #:
 Elevation: EL: B UNIT 1 BLK 148B

Job Track: 50469
 PlanLog: 204746
 Layout ID: 418816
 Ref #
 Page: 2 of 3
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T183A Hip	10 / 12	23-01-00	10-01-04	2 x 4 2 x 6		1-11-14 2-00-11	133.92 83.00		
	6	T184 Piggyback Base	10 / 12	23-01-00	9-01-04	2 x 4 2 x 6		1-06-14 2-00-11	751.39 475.00		
	4	T185 Roof Special	10 / 12	23-00-00	10-11-06	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	553.26 344.67		
	2	T186 Common	10 / 12	13-00-00	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	124.2 79.00		
	1 2-ply	T187 Common Girder	10 / 12	13-00-00	7-00-11	2 x 4 2 x 6		1-07-11 1-07-11	142.16 93.00		
	2	T188 Common	10 / 12	17-04-00	8-10-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	177.52 113.33		
	1	G188 GABLE	10 / 12	17-04-00	8-10-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	89.86 58.83		
	1	P164 Piggyback	10 / 12	5-07-01	2-00-00	2 x 4			15.13 11.33		
	3	P165 Piggyback	10 / 12	5-07-01	2-03-15	2 x 4			39.68 26.00		
	1	P166 Piggyback	10 / 12	5-07-01	1-08-00	2 x 4			14.71 10.67		
	4	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	67.18 42.67		
	4	J6 Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	60.39 38.67		
	1	J7 Jack-Open Girder	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J8 Jack-Open	10 / 12	1-10-08	3-01-09	2 x 4	1-03-08	1-07-11 3-02-07	9.79 7.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148B
 Lot #:
 Elevation: EL: B UNIT 1 BLK 148B

Job Track: 50469
 PlanLog: 204746
 Layout ID: 418816
 Ref #
 Page: 3 of 3
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
---------	------------	--------------	-------	------	--------	--------	---------------------------	------------------------------	--------------	---------------------	--------------------

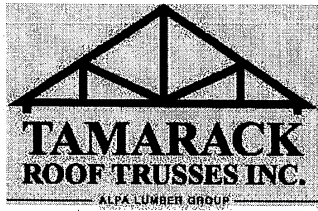
TOTAL # TRUSS= 55 TOTAL BFT OF ALL TRUSSES= 2633 BFT. TOTAL WEIGHT OF ALL TRSSES 4144.23 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
10	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 12

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148B
 Lot #:
 Elevation: EL: B2 UNIT 2 BLK 148B

Job Track: 50469
 PlanLog: 204746
 Layout ID: 418817
 Ref #
 Page: 1 of 1
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T52 Piggyback Base	6 /12	43-00-00	9-10-07	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1511.5 893.33		
	1	G52 GABLE	6 /12	43-00-00	9-10-07	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	264.13 170.50		
	5	T57 Piggyback Base	6 /12	45-05-00	9-10-13	2 x 6	1-03-08 1-03-08	1-02-00 1-07-11	1507.77 928.33		
	1	G57A GABLE	6 /12	44-05-00	9-10-13	2 x 6	1-03-08	1-02-00 1-07-11	274.08 173.00		
	5	P41 Piggyback	6 /12	8-02-04	2-00-09	2 x 4			93.42 61.67		
	1	P43 Piggyback	6 /12	8-02-04	1-04-04	2 x 4			20.28 14.50		
	6	P45 Piggyback	6 /12	7-02-04	1-09-09	2 x 4			91.24 64.00		

TOTAL # TRUSS= 24

TOTAL BFT OF ALL TRUSSES= 2305.33 BFT.

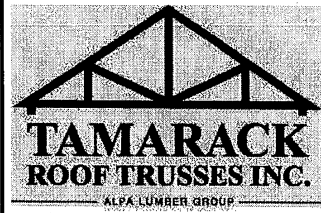
TOTAL WEIGHT OF ALL TRSSES 3762.43 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
40	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 40

DELIVERY SHIPLIST

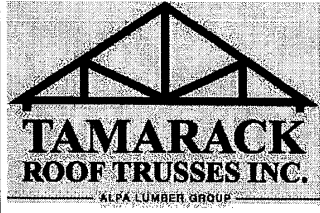


Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: ALCONA SHORES
 Location: INNISFIL
 Model: BLOCK 148B
 Lot #:
 Elevation: EL: B UNIT 3 BLK 148B

Job Track: 50469
 PlanLog: 204746
 Layout ID: 418818
 Ref #
 Page: 1 of 2
 Date: 03-24-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T98 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T1012 Half Hip Girder	10 /12	19-08-00	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	191.43 122.00		
	1	T102 Half Hip	10 /12	19-08-00	5-01-04	2 x 4 2 x 6	1-03-08	1-07-11 5-01-04	99.23 64.00		
	1	T103 Half Hip	10 /12	19-08-00	6-01-04	2 x 4 2 x 6	1-03-08	1-07-11 6-01-04	106.73 68.00		
	1	T104 Half Hip	10 /12	19-08-00	7-01-04	2 x 4 2 x 6	1-03-08	1-07-11 7-01-04	113.33 72.83		
	1	T105 Half Hip	10 /12	19-08-00	8-01-04	2 x 4 2 x 6	1-03-08	1-07-11 8-01-04	109.22 70.00		
	11	T106 Half Hip	10 /12	19-08-00	9-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-01-04	1354.1 852.50		
	1	T107 Half Hip	10 /12	19-08-00	10-01-04	2 x 4 2 x 6	1-03-08	1-07-11 10-01-04	136.01 86.17		
	4	T110A Common	10 /12	19-08-00	10-00-08	2 x 4 2 x 6	1-03-08	1-07-11 2-00-11	469.67 293.33		
	1	G110 GABLE	10 /12	20-02-00	10-00-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	105.59 69.00		
	2	T111 Common	10 /12	9-02-00	5-05-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	84.43 55.00		
	1	G111 GABLE	10 /12	9-02-00	5-05-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	46.72 31.50		
	2	T112 Common	10 /12	13-00-00	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	124.2 79.00		
	1	G112 GABLE	10 /12	13-00-00	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	65.8 43.50		

		DELIVERY SHIPLIST	
TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: ALCONA SHORES Location: INNISFIL Model: BLOCK 148B Lot #: Elevation: EL: B UNIT 3 BLK 148B	Job Track: 50469 PlanLog: 204746 Layout ID: 418818 Ref # Page: 2 of 2 Date: 03-24-2023 Designer: Sales Rep: Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	P101 Piggyback	10 /12	10-08-08	2-00-00	2 x 4		2-00-00	34.79 23.67		
	1	P102 Piggyback	10 /12	10-08-08	3-00-00	2 x 4		3-00-00	36.12 23.83		
	1	P107 Piggyback	10 /12	10-08-08	1-08-00	2 x 4		1-08-00	33.89 22.00		
	1	P122 Piggyback	10 /12	10-08-08	4-00-00	2 x 4		2-07-07	40.21 27.17		
	2	P123 Piggyback	10 /12	10-08-08	5-00-00	2 x 4		2-07-07	85.78 55.00		
	2	P124 Piggyback	10 /12	10-08-08	5-09-04	2 x 4		2-07-07	81.17 51.67		
	1	P125 Piggyback	10 /12	10-08-08	3-04-00	2 x 4		2-07-07	40.42 26.50		
	4	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	67.18 42.67		

TOTAL # TRUSS= 44

TOTAL BFT OF ALL TRUSSES= 2217.01

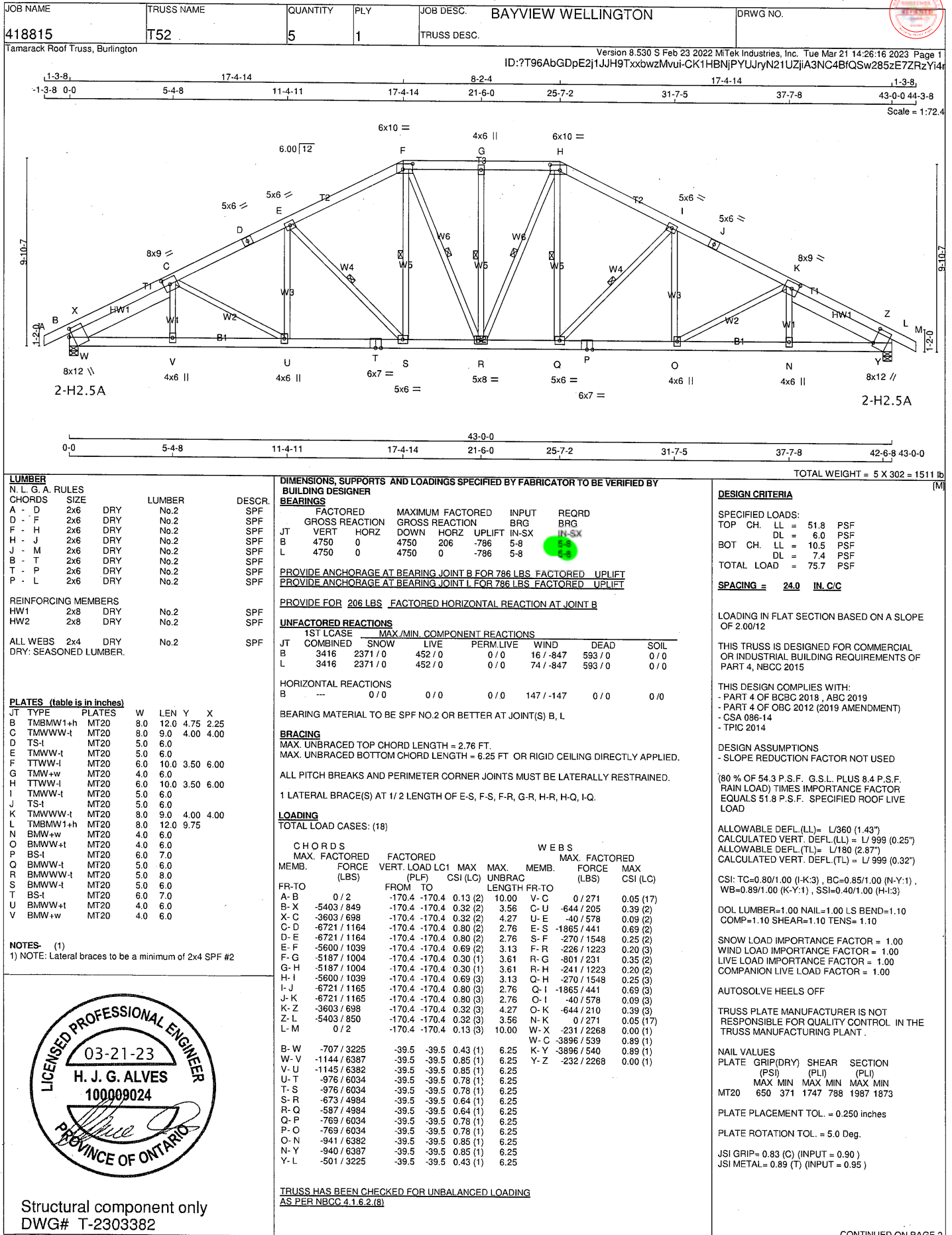
BFT.

TOTAL WEIGHT OF ALL TRSSES 3484.39 LBS

HARDWARE

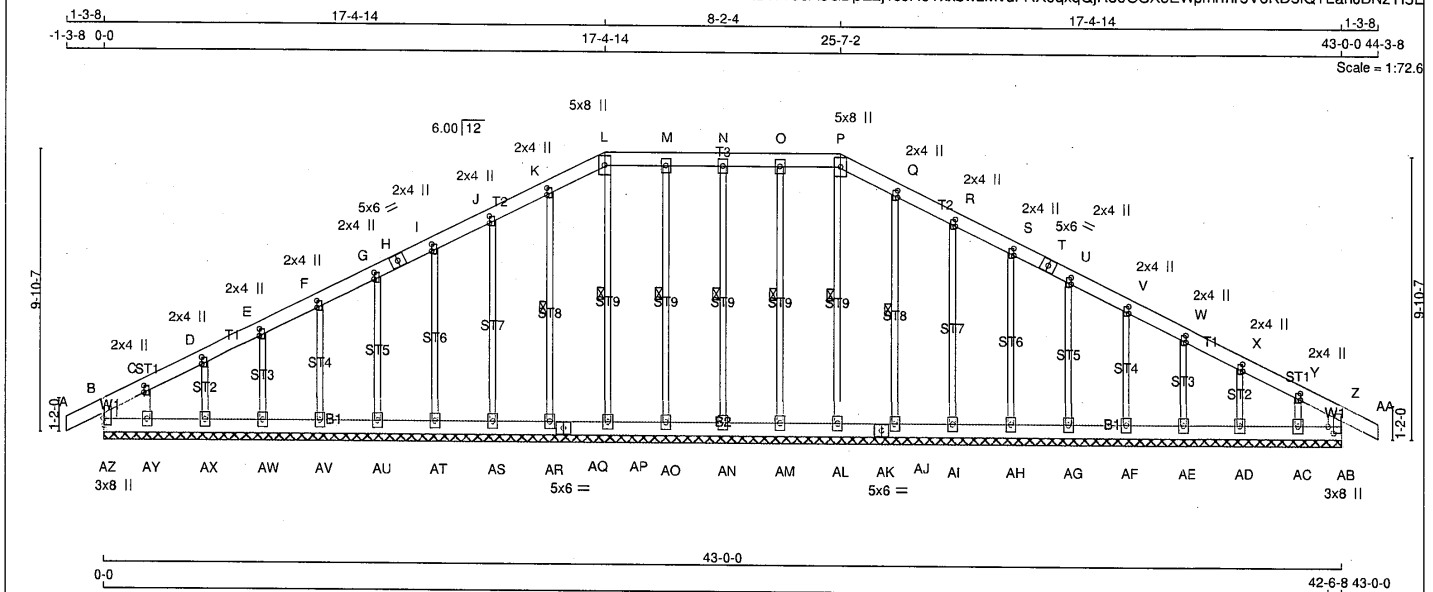
QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
2	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 3



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T52	5	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.530 S Feb 23 2022 Mittek Industries, Inc. Tue Mar 21 14:26:16 2023 Page 2					
ID:7T96AbGDpE2j1JJH9TxxbwzMvui-CK1HBNjPYUJryN21UZiIA3NC4BfQSw285zE7ZRzYi4r					
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.					
LICENSED PROFESSIONAL ENGINEER 03-21-23 H. J. G. ALVES 100009024 PROVINCE OF ONTARIO Structural component only DWG# T-2303382					

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	G52	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER DESCR.

AZ- B 2x6 DRY No.2 SPF

A - H 2x6 DRY No.2 SPF

H - L 2x6 DRY No.2 SPF

L - P 2x6 DRY No.2 SPF

P - T 2x6 DRY No.2 SPF

T - AA 2x6 DRY No.2 SPF

AB- Z 2x6 DRY No.2 SPF

AZ- AQ 2x6 DRY No.2 SPF

AQ- AK 2x6 DRY No.2 SPF

AK- AB 2x6 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

ALL GABLE WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B, Z, AB, AZ

C, D, E, Q, R, S, U, V, W, X, Y

C TMW+w MT20 2.0 4.0 2.75

F, G, I, J, K

F TMW+w MT20 2.0 4.0 3.00

H TS-t MT20 5.0 6.0

L TTW+p MT20 5.0 8.0

M, N, O

M TMW+w MT20 4.0 6.0

P TTW+p MT20 5.0 8.0

T TS-t MT20 5.0 6.0

AB TMBMV1+p MT20 3.0 8.0 2.75 2.25

AC, AD, AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AR, AS, AT, AU, AV, AW, AX, AY

AC BMW1+w MT20 4.0 6.0

AK BS-t MT20 5.0 6.0

AQ BS-t MT20 5.0 6.0

AZ TMBMV1+p MT20 3.0 8.0 2.75 0.25

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

LICENSED PROFESSIONAL ENGINEER

03-21-23

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only

DWG# T-2303359

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF P-AL, O-AM, N-AN, M-AO, L-AP, K-AR, Q-AJ.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

VERT. LOAD

LC1

MAX

MAX.

MEMB.

MAX. FACTORED

WEBS

MAX. FACTORED

MEMB.

MAX. FACTORED

MEMB.

FORCE

VERT. LOAD

LC1

MAX

UNBRAC

MEMB.

FORCE

MAX

FORCE

MAX

FORCE

MAX

FORCE

(LBS)

(LBS)

(PLF)

(LC)

(LC)

LENGTH

(LBS)

(LBS)

(LC)

(LBS)

(LBS)

(LC)

FR-TO

FROM TO

FR-TO

AZ-B

-368 / 0

0.0

0.0

0.04 (1)

7.81

AL-P

-261 / 0

0.16 (1)

A-B

0 / 40

-129.6

-129.6

0.09 (1)

10.00

AM-O

-266 / 0

0.16 (1)

B-C

-95 / 0

-129.6

-129.6

0.08 (1)

6.25

AN-N

-258 / 0

0.16 (1)

C-D

-48 / 0

-129.6

-129.6

0.03 (1)

6.25

AO-M

-266 / 0

0.16 (1)

D-E

-42 / 0

-129.6

-129.6

0.03 (1)

6.25

AP-L

-261 / 0

0.16 (1)

E-F

-33 / 0

-129.6

-129.6

0.03 (1)

6.25

AR-K

-258 / 0

0.12 (1)

F-G

-26 / 0

-129.6

-129.6

0.03 (1)

6.25

AS-J

-257 / 0

0.27 (1)

G-H

-21 / 0

-129.6

-129.6

0.03 (1)

6.25

AT-I

-257 / 0

0.18 (1)

H-I

-21 / 0

-129.6

-129.6

0.03 (1)

6.25

AV-G

-257 / 0

0.12 (1)

I-J

-16 / 0

-129.6

-129.6

0.03 (1)

6.25

AW-F

-256 / 0

0.08 (1)

J-K

-12 / 0

-129.6

-129.6

0.03 (1)

6.25

AX-E

-254 / 0

0.05 (1)

K-L

-9 / 0

-129.6

-129.6

0.03 (1)

10.00

AY-D

-270 / 0

0.04 (1)

L-M

-5 / 0

-129.6

-129.6

0.03 (1)

10.00

AY-C

-135 / 0

0.02 (1)

M-N

-5 / 0

-129.6

-129.6

0.03 (1)

10.00

AJ-Q

-258 / 0

0.12 (1)

N-O

-5 / 0

-129.6

-129.6

0.03 (1)

10.00

AI-R

-257 / 0

0.27 (1)

O-P

-5 / 0

-129.6

-129.6

0.03 (1)

10.00

AH-S

-257 / 0

0.18 (1)

P-Q

-9 / 0

-129.6

-129.6

0.03 (1)

10.00

AG-U

-257 / 0

0.12 (1)

Q-R

-12 / 0

-129.6

-129.6

0.03 (1)

6.25

AF-V

-256 / 0

0.08 (1)

R-S

-16 / 0

-129.6

-129.6

0.03 (1)

6.25

AE-W

-254 / 0

0.05 (1)

S-T

-21 / 0

-129.6

-129.6

0.03 (1)

6.25

AD-X

-270 / 0

0.04 (1)

T-U

-21 / 0

-129.6

-129.6

0.03 (1)

6.25

AC-Y

-135 / 0

0.02 (1)

U-V

-26 / 0

-129.6

-129.6

0.03 (1)

6.25

V-W

-33 / 0

-129.6

-129.6

0.03 (1)

6.25

W-X

-42 / 0

-129.6

-129.6

0.03 (1)

6.25

X-Y

-48 / 0

-129.6

-129.6

0.03 (1)

6.25

Y-Z

-95 / 0

-129.6

-129.6

0.08 (1)

6.25

Z-AA

0 / 40

-129.6

-129.6

0.09 (1)

10.00

AB-Z

-368 / 0

0.0

0.0

0.04 (1)

7.81

AZ-AY

0 / 59

-18.5

-18.5

0.04 (1)

10.00

AY-AX

0 / 49

-18.5

-18.5

0.01 (1)

10.00

AX-AW

0 / 37

-18.5

-18.5

0.01 (1)

10.00

AW-AV

0 / 29

-18.5

-18.5

0.01 (1)

10.00

AV-AU

0 / 23

-18.5

-18.5

0.01 (1)

10.00

AU-AT

0 / 18

-18.5

-18.5

0.01 (1)

10.00

AT-AS

0 / 14

-18.5

-18.5

0.01 (1)

10.00

AS-AR

0 / 11

-18.5

-18.5

0.01 (1)

10.00

AR-AQ

0 / 8

-18.5

-18.5

0.01 (1)

10.00

AQ-AP

0 / 8

-18.5

-18.5

0.01 (1)

10.00

AP-AO

0 / 5

-18.5

-18.5

0.01 (1)

10.00

AO-AN

0 / 5

-18.5

-18.5

0.01 (1)

10.00

AN-AM

0 / 5

-18.5

-18.5

0.01 (1)

10.00

AM-AL

0 / 5

-18.5

-18.5

0.01 (1)

10.00

AL-AK

0 / 8

-18.5

-18.5

0.01 (1)

10.00

AK-AJ

0 / 8

-18.5

-18.5

0.01 (1)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (Z-AA:1), BC=0.04/1.00 (AB-AC:1), WB=0.27/1.00 (R-AI:1), SSI=0.10/1.00 (Z-AA:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.37 (C) (INPUT = 0.90)

JSI METAL= 0.13 (AB) (INPUT = 0.95)

CONTINUED ON PAGE 5



Structural component only
DWG# T-2303359

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	G52	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



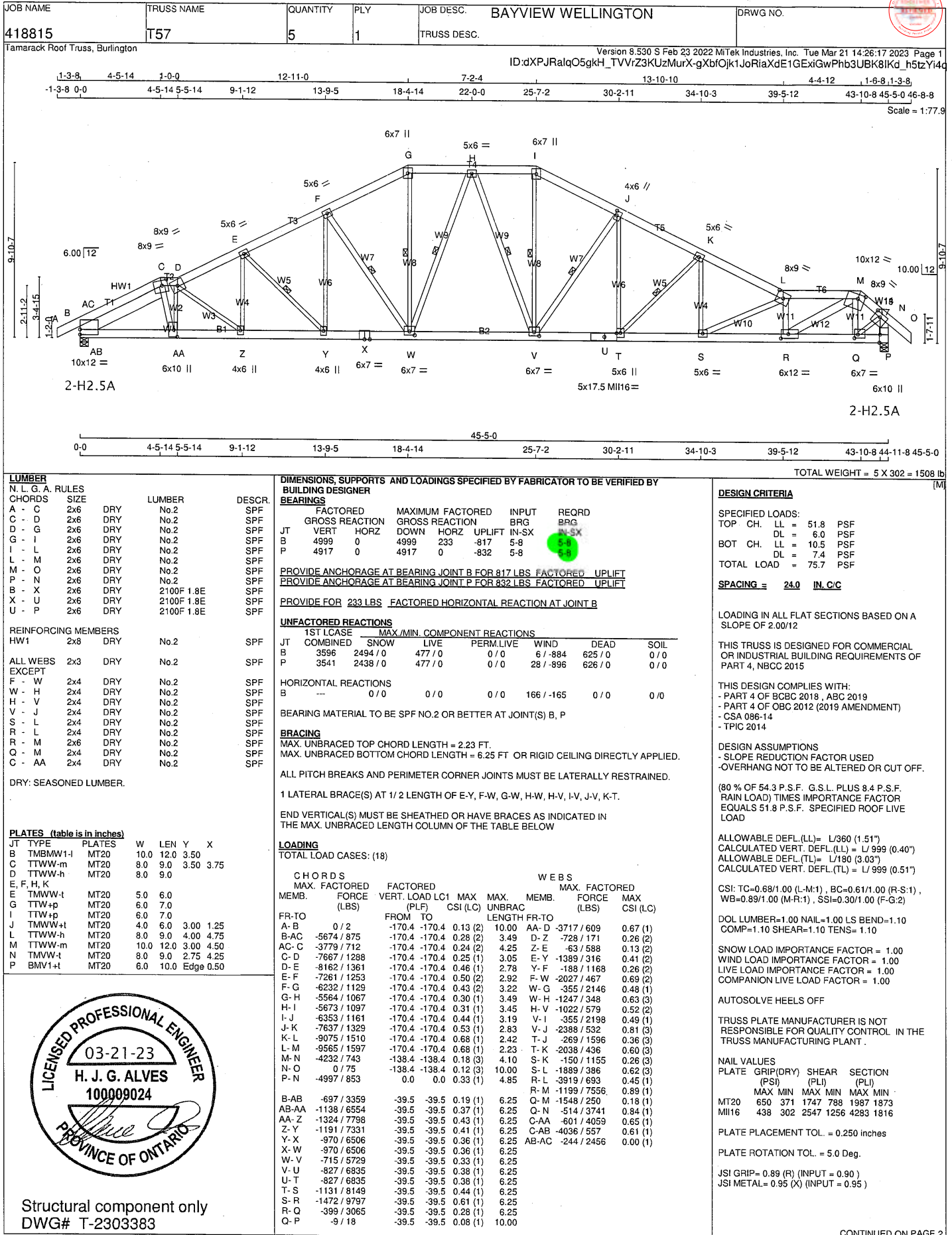
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:25:51 2023 Page 2
 ID: ?T96AbGDpE2j1JJH9TxxbwzMvui-RX0QxgQjRo9OSXeEWomrfnr5V6KD5iQTLahJBNzYi5E

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC. LENGTH
FR-TO		FROM TO		FR-TO			
AJ-AI	0 / 11	-18.5 -18.5	0.01 (4)	10.00			
AI-AH	0 / 14	-18.5 -18.5	0.01 (4)	10.00			
AH-AG	0 / 18	-18.5 -18.5	0.01 (4)	10.00			
AG-AF	0 / 23	-18.5 -18.5	0.01 (4)	10.00			
AF-AE	0 / 29	-18.5 -18.5	0.01 (4)	10.00			
AE-AD	0 / 37	-18.5 -18.5	0.01 (1)	10.00			
AD-AC	0 / 49	-18.5 -18.5	0.01 (1)	10.00			
AC-AB	0 / 59	-18.5 -18.5	0.04 (1)	10.00			



Structural component only
 DWG# T-2303359

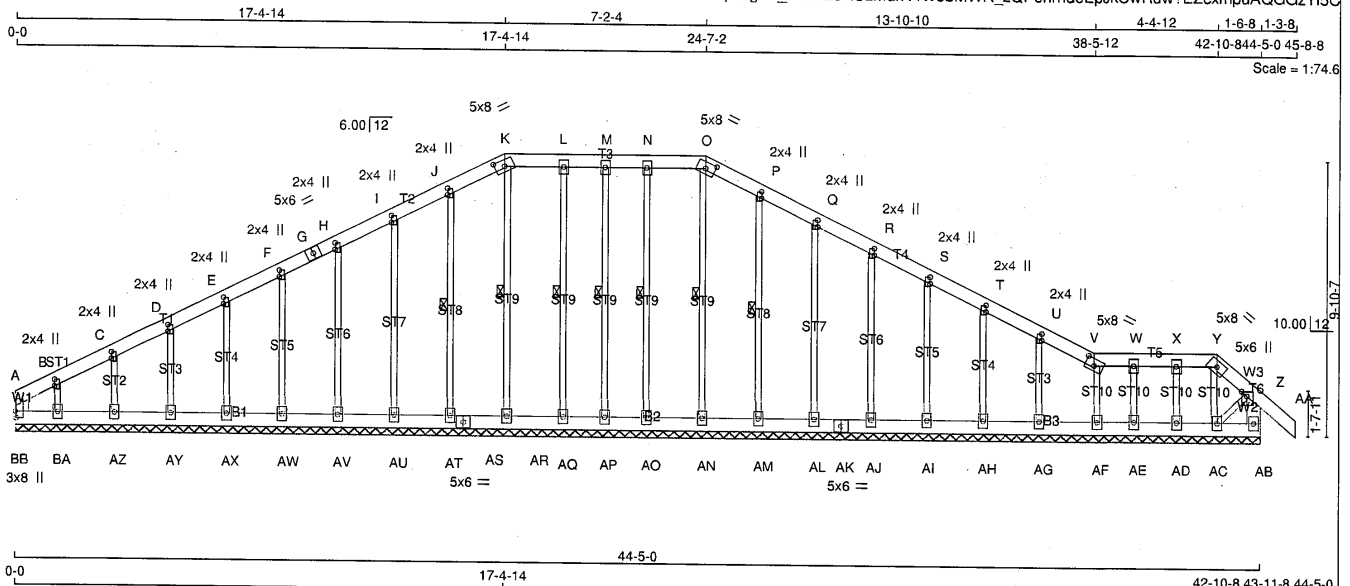


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																																																																				
418815	T57	5	1	BAYVIEW WELLINGTON																																																																																					
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:17 2023 Page 2																																																																																					
				ID:dXPJRalgO5qkH TVViZ3KUzMurX-qXbfOjk1JoRiaXdE1GExiGwPhb3UBK8IKd h5tzY14q																																																																																					
PLATES (table is in inches) <table><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>Q</td><td>BMWW-t</td><td>MT20</td><td>6.0</td><td>7.0</td><td>2.50</td><td>2.25</td></tr><tr><td>R</td><td>BMWW-t</td><td>MT20</td><td>6.0</td><td>12.0</td><td>3.00</td><td>5.00</td></tr><tr><td>S</td><td>BMWW-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>T</td><td>BMWW+t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>U</td><td>BS-t</td><td>MII16</td><td>5.0</td><td>17.5</td><td>2.50</td><td>8.25</td></tr><tr><td>V</td><td>BMWWW-t</td><td>MT20</td><td>6.0</td><td>7.0</td><td>2.50</td><td>3.50</td></tr><tr><td>W</td><td>BMWWW-t</td><td>MT20</td><td>6.0</td><td>7.0</td><td>2.50</td><td>3.50</td></tr><tr><td>X</td><td>BS-t</td><td>MT20</td><td>6.0</td><td>7.0</td><td></td><td></td></tr><tr><td>Y</td><td>BMWW+t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>Z</td><td>BMWW+t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>AA</td><td>BMWW+t</td><td>MT20</td><td>6.0</td><td>10.0</td><td>4.75</td><td>2.25</td></tr></tbody></table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES: (1) 1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2						JT	TYPE	PLATES	W	LEN	Y	X	Q	BMWW-t	MT20	6.0	7.0	2.50	2.25	R	BMWW-t	MT20	6.0	12.0	3.00	5.00	S	BMWW-t	MT20	5.0	6.0			T	BMWW+t	MT20	5.0	6.0			U	BS-t	MII16	5.0	17.5	2.50	8.25	V	BMWWW-t	MT20	6.0	7.0	2.50	3.50	W	BMWWW-t	MT20	6.0	7.0	2.50	3.50	X	BS-t	MT20	6.0	7.0			Y	BMWW+t	MT20	4.0	6.0			Z	BMWW+t	MT20	4.0	6.0			AA	BMWW+t	MT20	6.0	10.0	4.75	2.25
JT	TYPE	PLATES	W	LEN	Y	X																																																																																			
Q	BMWW-t	MT20	6.0	7.0	2.50	2.25																																																																																			
R	BMWW-t	MT20	6.0	12.0	3.00	5.00																																																																																			
S	BMWW-t	MT20	5.0	6.0																																																																																					
T	BMWW+t	MT20	5.0	6.0																																																																																					
U	BS-t	MII16	5.0	17.5	2.50	8.25																																																																																			
V	BMWWW-t	MT20	6.0	7.0	2.50	3.50																																																																																			
W	BMWWW-t	MT20	6.0	7.0	2.50	3.50																																																																																			
X	BS-t	MT20	6.0	7.0																																																																																					
Y	BMWW+t	MT20	4.0	6.0																																																																																					
Z	BMWW+t	MT20	4.0	6.0																																																																																					
AA	BMWW+t	MT20	6.0	10.0	4.75	2.25																																																																																			
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING <u>AS PER NBCC 4.1.6.2 (8)</u> WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.																																																																																									
 Structural component only DWG# T-2303383																																																																																									

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
418815	G57A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:25:53 2023 Page 1
ID:dXPJRalqO5gkH_TVvRz3KUZmUrX-Nv8bMWR_zQP6hrndeEpJkCwRuw?EZcxmpuAQGzYi5C



TOTAL WEIGHT = 274 lb
[M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
BB- A	2x6 DRY	No.2	SPF
A - K	2x6 DRY	No.2	SPF
G - K	2x6 DRY	No.2	SPF
K - O	2x6 DRY	No.2	SPF
O - V	2x6 DRY	No.2	SPF
V - Y	2x6 DRY	No.2	SPF
Y - AA	2x6 DRY	No.2	SPF
AB- Z	2x6 DRY	No.2	SPF
BB- AS	2x6 DRY	No.2	SPF
AS- AK	2x6 DRY	No.2	SPF
AK- AB	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-AN, N-AO, M-AP, L-AQ, J-AT, P-AM, K-AR.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 38.2 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 51.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (Z-AA:1), BC=0.01/1.00 (AG-AH:4), WB=0.27/1.00 (I-AU:1), SS=0.09/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (Q) (INPUT = 0.90)
JSI METAL= 0.12 (P) (INPUT = 0.95)

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A				
B, C, D, E, F, I, J, P, Q, R, S, T, U				
B TMW+w	MT20	2.0	4.0	3.00
G TS-t	MT20	5.0	6.0	
H TMW+w	MT20	2.0	4.0	2.75
K TTW-h	MT20	5.0	8.0	2.75 4.00
L, M, N, W, X				
L TMW+w	MT20	4.0	6.0	
O TTW-h	MT20	5.0	8.0	2.75 4.00
V TTW-h	MT20	5.0	8.0	3.00 4.00
Y TTW-h	MT20	5.0	8.0	
Z TMVW+p	MT20	5.0	6.0	2.00 2.25
AB BMV1+p	MT20	4.0	6.0	
AD BMWV1+t	MT20	4.0	6.0	
AD, AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ, AR, AT, AU, AV, AW, AX, AY, AZ, BA				
AD BMW1+w	MT20	4.0	6.0	
AK BS-t	MT20	5.0	6.0	
AS BS-t	MT20	5.0	6.0	
BB				
BB TMBMV1+p	MT20	3.0	8.0	2.75 0.25

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO			
BB-A	-71 / 0	0.0	0.0 0.00 (1)	7.81	AN-O	-253 / 0	0.15 (1)
A-B	0 / 9	-129.6	-129.6 0.02 (1)	10.00	AO-N	-249 / 0	0.15 (1)
B-C	0 / 23	-129.6	-129.6 0.03 (1)	10.00	AP-M	-198 / 0	0.12 (1)
C-D	0 / 30	-129.6	-129.6 0.03 (1)	10.00	AQ-L	-249 / 0	0.15 (1)
D-E	0 / 40	-129.6	-129.6 0.03 (1)	10.00	AT-J	-282 / 0	0.13 (1)
E-F	0 / 46	-129.6	-129.6 0.03 (1)	10.00	AU-I	-257 / 0	0.27 (1)
F-G	0 / 52	-129.6	-129.6 0.04 (1)	10.00	AV-H	-256 / 0	0.18 (1)
G-H	0 / 52	-129.6	-129.6 0.04 (1)	10.00	AW-F	-257 / 0	0.12 (1)
H-I	0 / 57	-129.6	-129.6 0.04 (1)	10.00	AX-E	-256 / 0	0.08 (1)
I-J	0 / 61	-129.6	-129.6 0.04 (1)	10.00	AY-D	-254 / 0	0.05 (1)
J-K	0 / 54	-129.6	-129.6 0.04 (1)	10.00	AZ-C	-265 / 0	0.04 (1)
K-L	0 / 61	-129.6	-129.6 0.04 (1)	10.00	BA-B	-224 / 0	0.13 (1)
L-M	0 / 60	-129.6	-129.6 0.04 (1)	10.00	AM-P	-282 / 0	0.27 (1)
M-N	0 / 60	-129.6	-129.6 0.04 (1)	10.00	AL-Q	-257 / 0	0.13 (1)
N-O	0 / 61	-129.6	-129.6 0.04 (1)	10.00	AJ-R	-256 / 0	0.18 (1)
O-P	0 / 54	-129.6	-129.6 0.04 (1)	10.00	AI-S	-257 / 0	0.12 (1)
P-Q	0 / 61	-129.6	-129.6 0.04 (1)	10.00	AH-T	-256 / 0	0.08 (1)
Q-R	0 / 57	-129.6	-129.6 0.04 (1)	10.00	AG-U	-273 / 0	0.06 (1)
R-S	0 / 52	-129.6	-129.6 0.03 (1)	10.00	AF-V	-160 / 0	0.03 (1)
S-T	0 / 46	-129.6	-129.6 0.03 (1)	10.00	AE-W	-212 / 0	0.03 (1)
T-U	0 / 39	-129.6	-129.6 0.04 (1)	10.00	AC-Y	-115 / 0	0.02 (1)
U-V	0 / 41	-129.6	-129.6 0.04 (1)	10.00	AD-Z	-35 / 0	0.01 (1)
V-W	0 / 34	-129.6	-129.6 0.02 (1)	10.00	AD-X	-206 / 0	0.03 (1)
W-X	0 / 34	-129.6	-129.6 0.02 (1)	10.00	AR-K	-253 / 0	0.15 (1)
X-Y	0 / 35	-129.6	-129.6 0.02 (1)	10.00			
Y-Z	-18 / 0	-129.6	-129.6 0.09 (1)	6.25			
Z-AA	0 / 60	-129.6	-129.6 0.10 (1)	10.00			
AB-Z	-329 / 0	0.0	0.0 0.02 (1)	7.81			
BB-BA	0 / 0	-18.5	-18.5 0.01 (4)	10.00			
BA-AZ	-17 / 0	-18.5	-18.5 0.01 (4)	6.25			
AZ-AY	-28 / 0	-18.5	-18.5 0.01 (4)	6.25			
AY-AX	-36 / 0	-18.5	-18.5 0.01 (4)	6.25			
AX-AW	-42 / 0	-18.5	-18.5 0.01 (4)	6.25			
AW-AV	-46 / 0	-18.5	-18.5 0.01 (4)	6.25			
AV-AU	-51 / 0	-18.5	-18.5 0.01 (4)	6.25			
AU-AT	-54 / 0	-18.5	-18.5 0.01 (4)	6.25			
AT-AS	-58 / 0	-18.5	-18.5 0.01 (4)	6.25			
AS-AR	-58 / 0	-18.5	-18.5 0.01 (4)	6.25			
AR-AQ	-60 / 0	-18.5	-18.5 0.01 (4)	6.25			
AQ-AP	-60 / 0	-18.5	-18.5 0.01 (4)	6.25			
AP-AO	-60 / 0	-18.5	-18.5 0.01 (4)	6.25			
AO-AN	-60 / 0	-18.5	-18.5 0.01 (4)	6.25			
AN-AM	-58 / 0	-18.5	-18.5 0.01 (4)	6.25			
AM-AL	-54 / 0	-18.5	-18.5 0.01 (4)	6.25			

03-21-23

H. J. G. ALVES

100009024

Structural component only

DWG# T-2303360

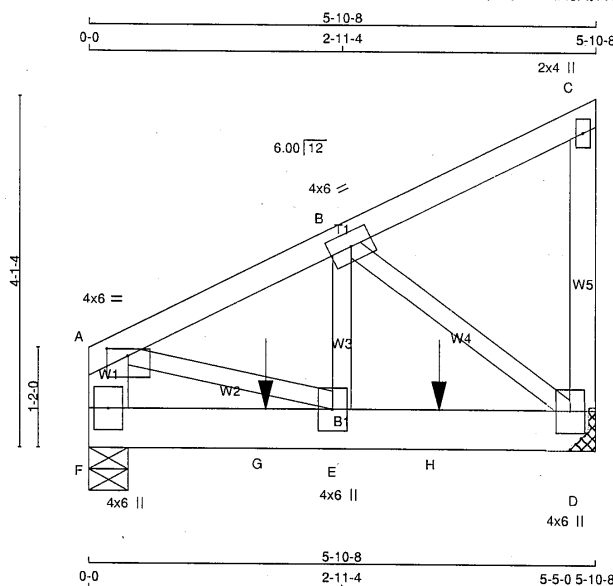
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																																																																																								
418815	G57A	1	1	BAYVIEW WELLINGTON																																																																																																									
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:25:53 2023 Page 2																																																																																																									
				ID:dXPJRalgO5qkH TVVrZ3KUzMurX-Nv8bMWR_zQP6hrndeEpJkCwRuW?EZcxmpuAQGGzY15C																																																																																																									
NOTES: (1) 1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2		LOADING TOTAL LOAD CASES: (4) <table><thead><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. UNBRAC (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. UNBRAC (LC)</th><th>LENGTH FR-TO</th></tr></thead><tbody><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>AL-AK</td><td>-51 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AK-AJ</td><td>-51 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AJ-AI</td><td>-46 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AI-AH</td><td>-42 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AH-AG</td><td>-36 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AG-AF</td><td>-27 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AF-AE</td><td>-34 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AE-AD</td><td>-34 / 0</td><td>-18.5 -18.5</td><td>0.00 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AD-AC</td><td>-34 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>AC-AB</td><td>0 / 0</td><td>-18.5 -18.5</td><td>0.01 (4)</td><td></td><td></td><td></td><td>10.00</td></tr></tbody></table>				CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC (LC)	LENGTH FR-TO	FR-TO		FROM TO						AL-AK	-51 / 0	-18.5 -18.5	0.01 (4)				6.25	AK-AJ	-51 / 0	-18.5 -18.5	0.01 (4)				6.25	AJ-AI	-46 / 0	-18.5 -18.5	0.01 (4)				6.25	AI-AH	-42 / 0	-18.5 -18.5	0.01 (4)				6.25	AH-AG	-36 / 0	-18.5 -18.5	0.01 (4)				6.25	AG-AF	-27 / 0	-18.5 -18.5	0.01 (4)				6.25	AF-AE	-34 / 0	-18.5 -18.5	0.01 (4)				6.25	AE-AD	-34 / 0	-18.5 -18.5	0.00 (4)				6.25	AD-AC	-34 / 0	-18.5 -18.5	0.01 (4)				6.25	AC-AB	0 / 0	-18.5 -18.5	0.01 (4)				10.00
CHORDS				WEBS																																																																																																									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC (LC)	LENGTH FR-TO																																																																																																						
FR-TO		FROM TO																																																																																																											
AL-AK	-51 / 0	-18.5 -18.5	0.01 (4)				6.25																																																																																																						
AK-AJ	-51 / 0	-18.5 -18.5	0.01 (4)				6.25																																																																																																						
AJ-AI	-46 / 0	-18.5 -18.5	0.01 (4)				6.25																																																																																																						
AI-AH	-42 / 0	-18.5 -18.5	0.01 (4)				6.25																																																																																																						
AH-AG	-36 / 0	-18.5 -18.5	0.01 (4)				6.25																																																																																																						
AG-AF	-27 / 0	-18.5 -18.5	0.01 (4)				6.25																																																																																																						
AF-AE	-34 / 0	-18.5 -18.5	0.01 (4)				6.25																																																																																																						
AE-AD	-34 / 0	-18.5 -18.5	0.00 (4)				6.25																																																																																																						
AD-AC	-34 / 0	-18.5 -18.5	0.01 (4)				6.25																																																																																																						
AC-AB	0 / 0	-18.5 -18.5	0.01 (4)				10.00																																																																																																						



Structural component only
DWG# T-2303360

JOB NAME 418815	TRUSS NAME T98	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:18 2023 Page 1
ID:3QHkcHYZQvP6BVkZHOAtV7zNZde-8j92c3kf45ZZBhCQb_IAFUSjp?ThwxxRZHjEdJzYI4p



Scale = 1:24.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(P/LF)
TOP CHORDS - (0 122"x3")	SPRINK NAILS	TOP
A - A	12	TOP
A - C	12	TOP
BOTTOM CHORDS - (0 122"x3")	SPRINK NAILS	TOP
F - D	12	TOP
WEBS - (0 122"x3")	SPRINK NAILS	SIDE(S)
2x3	1	SIDE(S)
2x4	1	SIDE(S)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GRINDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP: COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE: PLY SHOWN IS THE EQUIVALENT UCL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	6.0		
C	TMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
F	1470	0	1470	0	5-8	5-8
D	1618	0	1618	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1020	780 / 0	0 / 0	0 / 0	0 / 0	241 / 0	0 / 0
F	1123	858 / 0	0 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)
		FROM	TO				FROM	TO	
FR-TO									
F-A	-1246 / 0	0.0	0.0	0.04 (1)	7.81	E-B	0 / 1419	0.18 (1)	
A-B	-1578 / 0	-129.6	-129.6	0.08 (1)	6.25	D-C	-140 / 0	0.01 (1)	
B-C	-19 / 0	-129.6	-129.6	0.07 (1)	6.25	A-E	0 / 1474	0.18 (1)	
						B-D	-1825 / 0	0.22 (1)	
F-G	0 / 0	-18.5	-18.5	0.16 (1)	10.00				
G-E	0 / 0	-18.5	-18.5	0.16 (1)	10.00				
E-H	0 / 1425	-18.5	-18.5	0.29 (1)	10.00				
H-D	0 / 1425	-18.5	-18.5	0.29 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-779	-779	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-779	-779	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.2	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.08/1.00 (A-B:1), BC=0.29/1.00 (D-E:1),
WB=0.22/1.00 (B-D:1), SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (D) (INPUT = 0.90)
JSI METAL= 0.32 (D) (INPUT = 0.95)



Structural component only
DWG# T-2303384

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T98	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:18 2023 Page 2
ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-8j92c3kf45ZZBhCQb IAFUSjp?ThwxxRZHIEJzY14p

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMWW1+l	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES: (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2303384

JOB NAME
418815

Tamarack Roof Truss, Burlington

TRUSS NAME
T98Z

QUANTITY
1

PLY
2

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:19 2023 Page 1

ID:3QHkcHYZQvP6BVkzH0AtV7zNZde-cvjQpPIHrPhQpmc9hHPoh?uSConfNRbnxTo9mzY14o

Scale = 1:24.2

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT D - C	2x3	DRY	No.2	SPF
	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS

CHORDS	ROWS	SURFACE	LOAD (PLF)
TOP CHORDS (0 122"x3")	SPR	NAILED	TOP
F-A	2	12	TOP
A-C	1	12	TOP
BOTTOM CHORDS (0 122"x3")	SPR	NAILED	SIDE (0.0)
F-D	2	12	SIDE (0.0)
WEBS (0 122"x3")	SPR	NAILED	
2x3	1	6	
2x4	1	6	

NAILED TO BE DRIVEN FROM ONE SIDE ONLY

ORDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	6.0		
C	TMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	1769	0	1769	0	5-8	5-8
D	1961	0	1961	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LC CASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	1229	934 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0
D	1362	1035 / 0	0 / 0	0 / 0	0 / 0	326 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. CSI (LC)	MAX. UNBRAC	MEMB.	WEBS		MAX. CSI (LC)
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)				MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO							FR-TO			
F-A	-1490 / 0	0.0	0.0	0.05 (1)	7.81		E-B	0 / 1837	0.23 (1)	
A-B	-1933 / 0	-129.6	-129.6	0.09 (1)	6.21		D-C	-143 / 0	0.01 (1)	
B-C	-17 / 0	-129.6	-129.6	0.07 (1)	6.25		A-E	0 / 1801	0.22 (1)	
							B-D	-2230 / 0	0.27 (1)	
F-G	0 / 0	-18.5	-18.5	0.20 (1)	10.00					
G-E	0 / 0	-18.5	-18.5	0.20 (1)	10.00					
E-H	0 / 1741	-18.5	-18.5	0.36 (1)	10.00					
H-D	0 / 1741	-18.5	-18.5	0.36 (1)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-1002	-1002	---	FRONT	VERT	TOTAL	---	C1
H	4-0-12	-1002	-1002	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	38.2	PSF
DL	6.0	PSF
BOT CH.	LL	PSF
LL	0.0	PSF
DL	7.4	PSF
TOTAL LOAD	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")

CALCULATED VERT. DEFL. (LL) = L/999 (0.01")

ALLOWABLE DEFL. (TL) = L/360 (0.19")

CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.09/1.00 (A-B:1), BC=0.36/1.00 (D-E:1), WB=0.27/1.00 (B-D:1), SSI=0.40/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (D) (INPUT = 0.90)

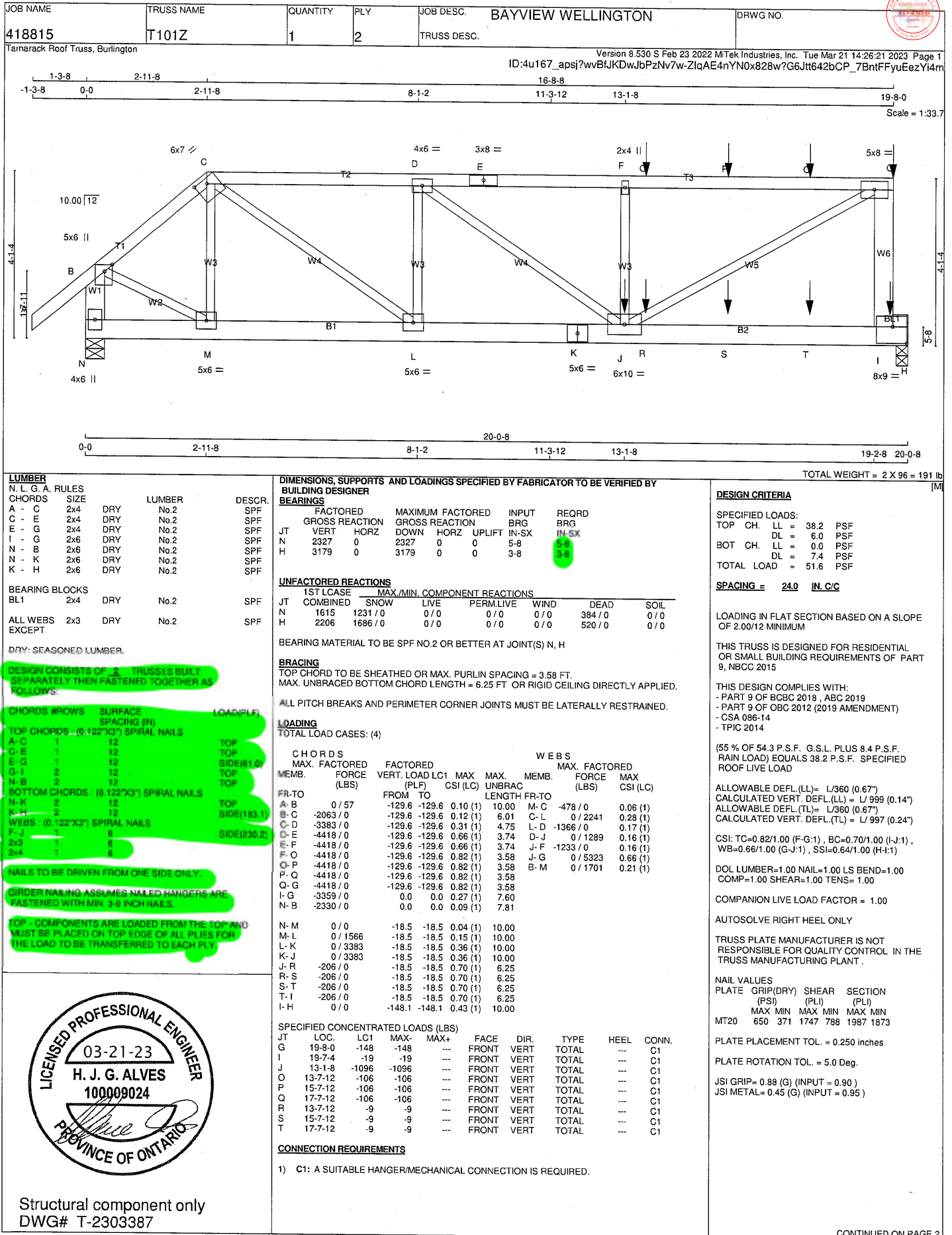
JSI METAL = 0.39 (D) (INPUT = 0.95)

Structural component only

DWG# T-2303385

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																												
418815	T98Z	1	2	BAYVIEW WELLINGTON																													
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:19 2023 Page 2																													
				ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-cvjQpPIHrPhQpmc9hHPoh?uSOonfNRbnxTo9mzYi4o																													
PLATES (table is in inches) <table><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>D</td><td>BMWV1+t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>E</td><td>BMWV1+t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>F</td><td>BMWV1+p</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr></tbody></table> NOTES: (1) 1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2						JT	TYPE	PLATES	W	LEN	Y	X	D	BMWV1+t	MT20	4.0	6.0			E	BMWV1+t	MT20	4.0	6.0			F	BMWV1+p	MT20	4.0	6.0		
JT	TYPE	PLATES	W	LEN	Y	X																											
D	BMWV1+t	MT20	4.0	6.0																													
E	BMWV1+t	MT20	4.0	6.0																													
F	BMWV1+p	MT20	4.0	6.0																													
 Structural component only DWG# T-2303385																																	





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T101Z	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

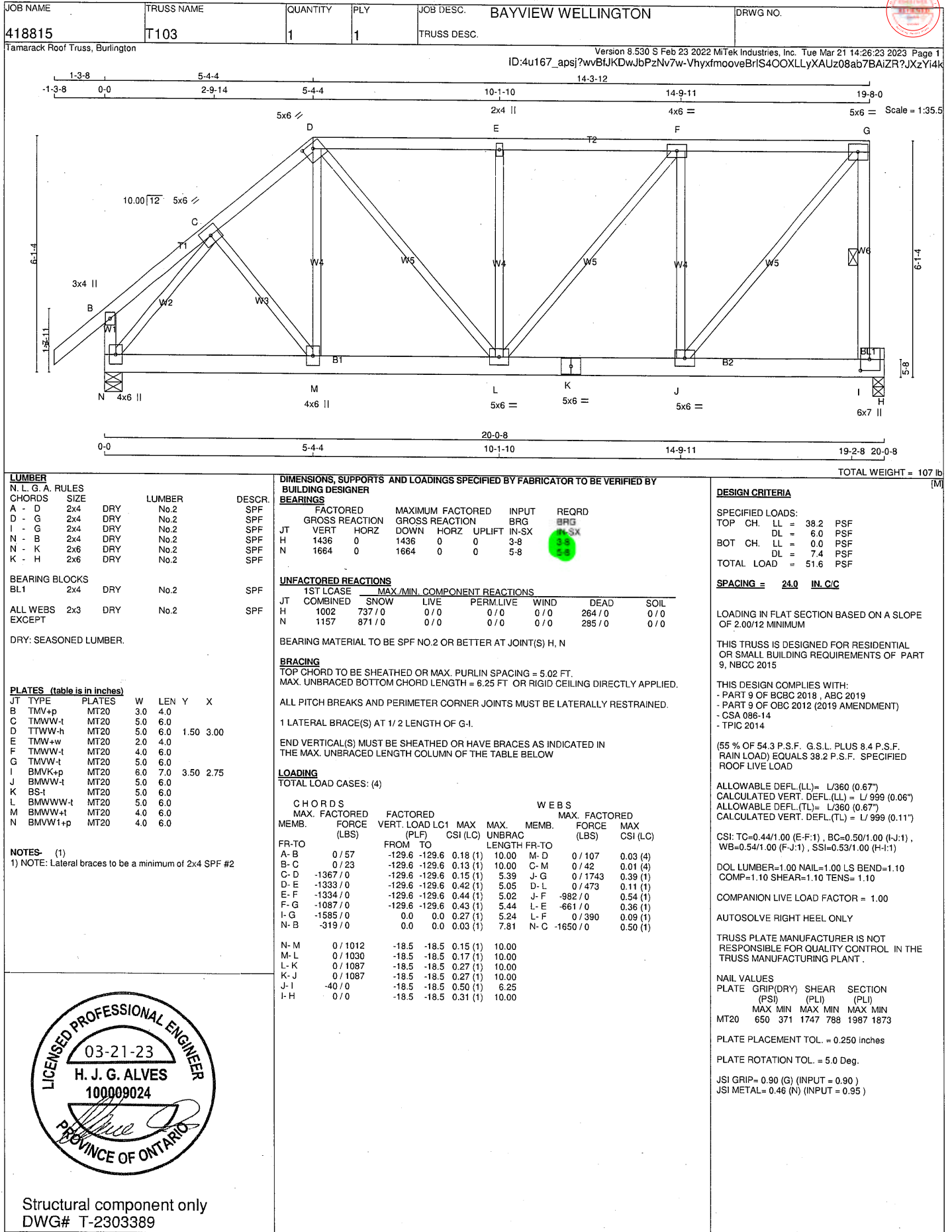
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:21 2023 Page 2
ID:4u167 apsi?wvBfJKDwJbPzNv7w-ZlqAE4nYN0x828w?G6Jtt642bCP_7BntFFyuEezYi4m

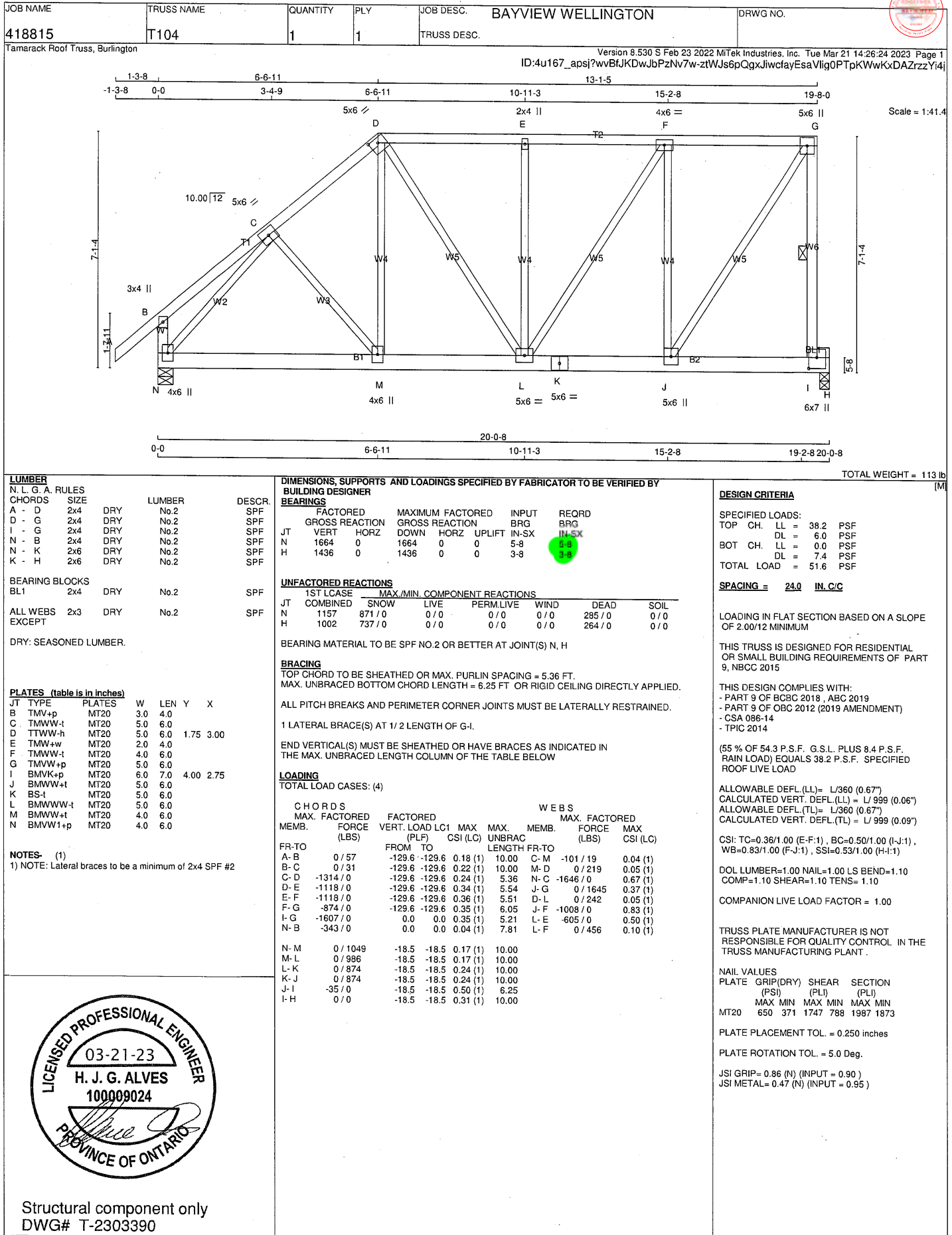
PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-h	MT20	6.0	7.0	1.50	3.50
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	8.0		
I	BMVK-t	MT20	8.0	9.0	5.00	4.25
J	BMWW-t	MT20	6.0	10.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	4.0	6.0		

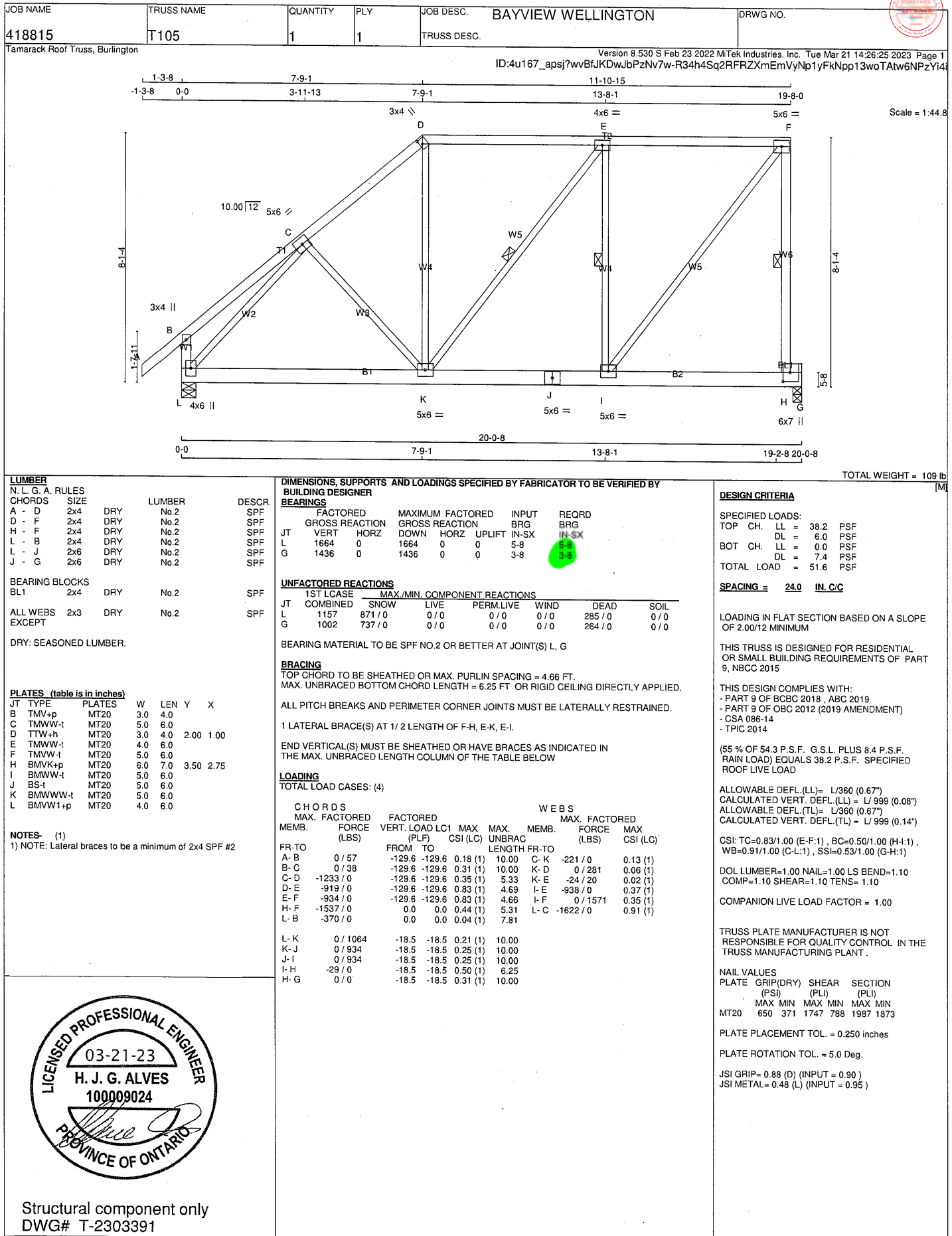
NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

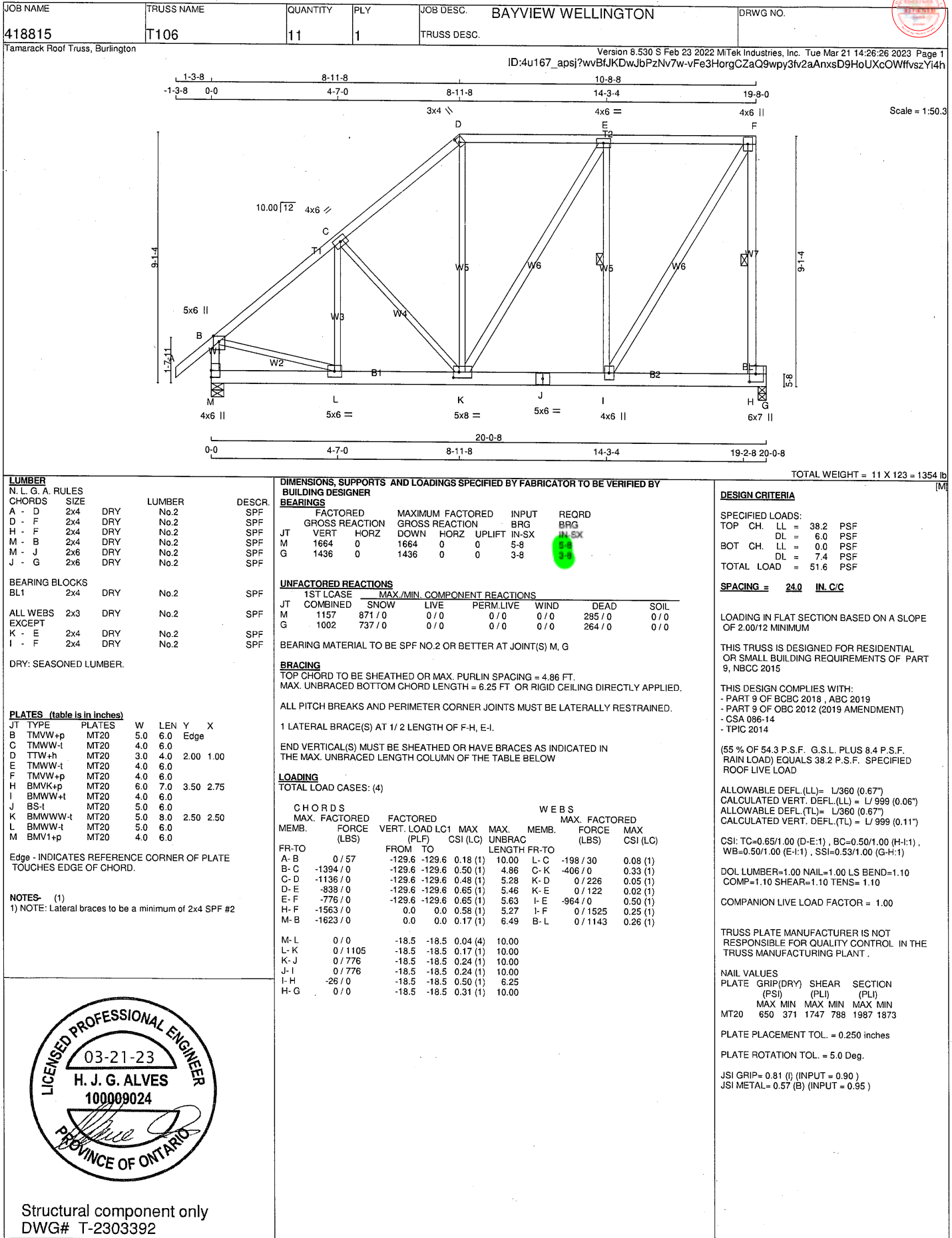
LICENSED PROFESSIONAL ENGINEER
03-21-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

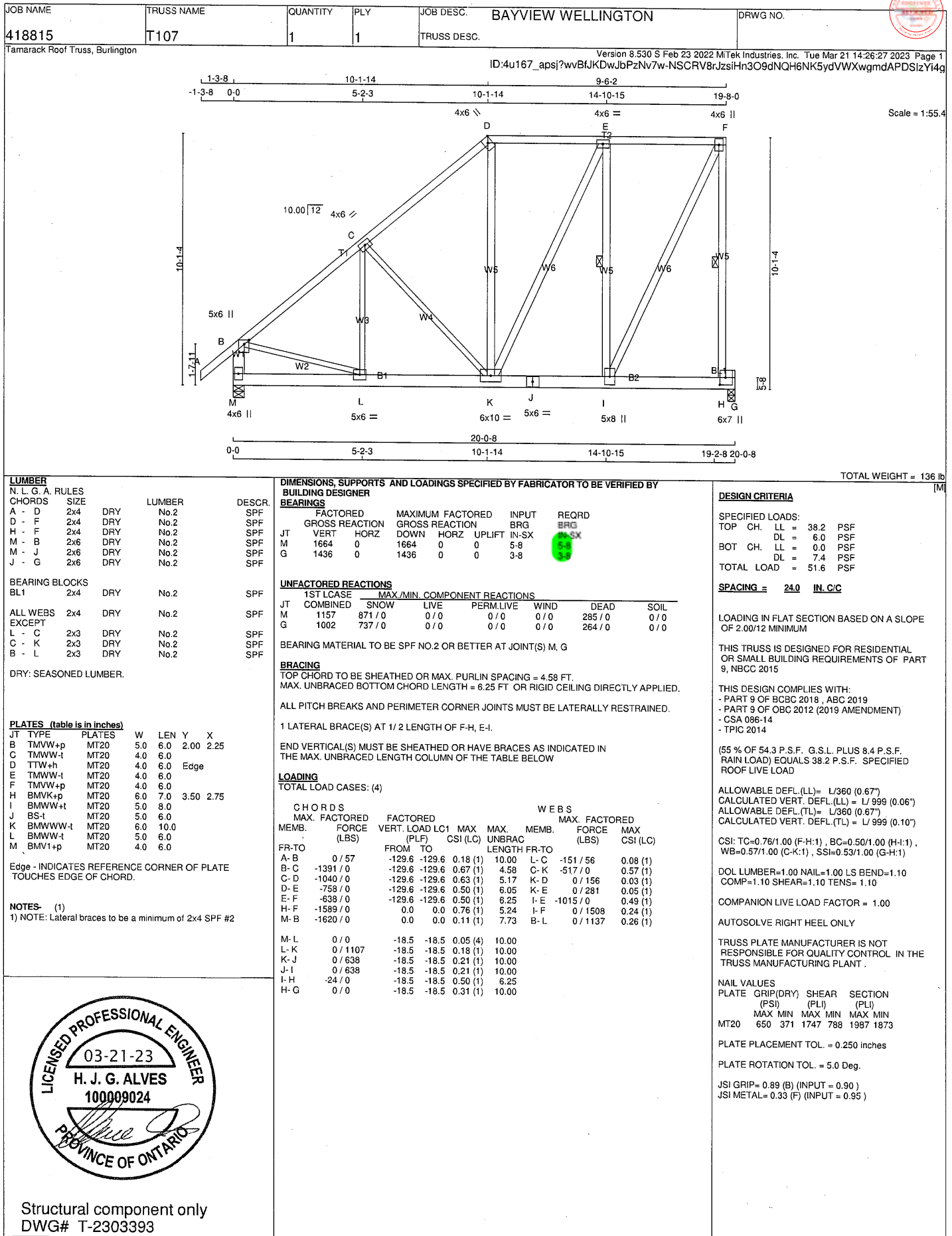
Structural component only
DWG# T-2303387









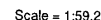


Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:28 2023 Page 1
ID:3QHkcHYZQvP6BVKzH0AIV7zNZde-relaiUsxkAa8ODzl B4xWfbrtH71nnGPRvsg8m kZy4f



Structural component only
DWG# T-2303394

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:25:54 2023 Page 1
ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-r5izZsSckjYzJ?MpBxKYHQtbMKLGI1?v1YvzoizYi5B



Structural component only
DWG# T-2303361

JOB NAME
418815

Tamarack Roof Truss, Burlington

TRUSS NAME
T111

QUANTITY
2

PLY
1

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Tue Mar 21 14:26:30 2023 Page 1

ID:3QHkcHYZQvP6BVkzH0AtV7zNZde-o1ta79uBFn4seX6kIVz_k0yiaqdIK09CJ8dt2dzY14d

Scale = 1:34.1

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	859	0	859	0	5-8	5-8
F	859	0	859	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	596	456 / 0	0 / 0	0 / 0	0 / 0	139 / 0	0 / 0
F	596	456 / 0	0 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD			MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO	PLF				
FR-TO								
A-B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	G-C	-74 / 68	0.03 (1)
B-C	-406 / 0	-129.6	-129.6	0.35 (1)	6.25	B-G	0 / 324	0.07 (1)
C-D	-406 / 0	-129.6	-129.6	0.35 (1)	6.25	G-D	0 / 324	0.07 (1)
D-E	0 / 57	-129.6	-129.6	0.18 (1)	10.00			
H-B	-826 / 0	0.0	0.0	0.09 (1)	7.81			
F-D	-826 / 0	0.0	0.0	0.09 (1)	7.81			
H-G	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
G-F	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL) = L/360 (0.31")

CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.35/1.00 (C-D:1), BC=0.11/1.00 (G-H:4), WB=0.07/1.00 (B-G:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (G) (INPUT = 0.90)

JSI METAL= 0.30 (D) (INPUT = 0.95)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-l	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

Structural component only

DWG# T-2303395

JOB NAME
418815

Tamarack Roof Truss, Burlington

TRUSS NAME
G111

QUANTITY
1

PLY
1

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:25:55 2023 Page 1

ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-JIGLnBTEV1gx9x?lfrmpd?m6jhY1ZD2GCfWK9zY15A

Scale = 1:33.6

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" O.C.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G					
C TMW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	6.0	Edge	
H TMVW+p	MT20	4.0	6.0	Edge	
J BMV1+p	MT20	3.0	4.0		
K BMW1-t	MT20	4.0	6.0		
L, M, N					
L BMW1+w	MT20	2.0	4.0		
O BMW1-t	MT20	4.0	6.0		
P BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MEMB.
FR-TO				FR-TO			
P-B	-394 / 0	0.0	0.0	M-E	-210 / 0	0.09 (1)	
A-B	0 / 57	-129.6	-129.6	N-D	-278 / 0	0.06 (1)	
B-C	-83 / 0	-129.6	-129.6	O-C	-11 / 0	0.00 (1)	
C-D	0 / 14	-129.6	-129.6	L-F	-278 / 0	0.06 (1)	
D-E	-26 / 0	-129.6	-129.6	K-G	-11 / 0	0.00 (1)	
E-F	-26 / 0	-129.6	-129.6	B-O	0 / 22	0.00 (1)	
F-G	0 / 14	-129.6	-129.6	K-H	0 / 22	0.00 (1)	
G-H	-83 / 0	-129.6	-129.6				
H-I	0 / 57	-129.6	-129.6				
J-H	-394 / 0	0.0	0.0				
P-O	0 / 0	-18.5	-18.5				
O-N	0 / 16	-18.5	-18.5				
N-M	0 / 8	-18.5	-18.5				
M-L	0 / 8	-18.5	-18.5				
L-K	0 / 16	-18.5	-18.5				
K-J	0 / 0	-18.5	-18.5				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1), BC=0.02/1.00 (M-N:4), WB=0.09/1.00 (E-M:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (H) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 0.95)

Structural component only
DWG# T-2303362

JOB NAME

118815

TRUSS NAME

G112

QUANTITY

1

PLY

1

JOB DESC.

BAYVIEW WELLINGTON

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTK Industries, Inc. Tue Mar 21 14:25:56 2023 Page 1

ID:AyJW?V_dk3D9O4qx_WrxbZNV4b-nUqj_XTsGLohZJWBjMM0MrYx71om?MCVsO4sbzYI59

1-3-8

6-6-0

6-6-0

6-6-0

1-3-8

-1-3-8

0-0

6-6-0

6-6-0

12-8-8

13-0-0

14-3-8

4x6 II

Scale = 1:41.9

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

T - B

2x4

DRY

No.2

SPF

A - F

2x4

DRY

No.2

SPF

F - K

2x4

DRY

No.2

SPF

L - J

2x4

DRY

No.2

SPF

T - L

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

ALL GABLE WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT

2-0-0 OC.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

B

TMVW+p

MT20

4.0

6.0

Edge

C, D, E, G, H, I

C

TMVW+w

MT20

2.0

4.0

F

TTW+p

MT20

4.0

6.0

Edge

J

TMVW+p

MT20

4.0

6.0

Edge

L

BMV1+p

MT20

3.0

4.0

M

BMVW1-t

MT20

4.0

6.0

N, O, P, Q, R

N

BMV1+w

MT20

2.0

4.0

S

BMVW1-t

MT20

4.0

6.0

T

BMV1+p

MT20

3.0

4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

VERT. LOAD (PLF)

LC1

MAX

CS1 (LC)

UNBRAC

LENGTH

FR-TO

FR-TO

MEMB.

MAX. FACTORED FORCE (LBS)

MAX

CS1 (LC)

T-B

-408 / 0

0.0

0.0

0.04 (1)

7.81

P-F

-195 / 0

0.16 (1)

A-B

0 / 57

-129.6

-129.6

0.18 (1)

10.00

Q-E

-303 / 0

0.13 (1)

B-C

-94 / 0

-129.6

-129.6

0.17 (1)

6.25

R-D

-213 / 0

0.05 (1)

C-D

-8 / 0

-129.6

-129.6

0.04 (1)

10.00

S-C

-23 / 0

0.00 (1)

D-E

-14 / 0

-129.6

-129.6

0.08 (1)

6.25

O-G

-303 / 0

0.13 (1)

E-F

-34 / 0

-129.6

-129.6

0.08 (1)

6.25

N-H

-213 / 0

0.05 (1)

F-G

-34 / 0

-129.6

-129.6

0.08 (1)

6.25

M-I

-23 / 0

0.00 (1)

G-H

-14 / 0

-129.6

-129.6

0.08 (1)

6.25

B-S

0 / 33

0.01 (1)

H-I

-8 / 0

-129.6

-129.6

0.04 (1)

10.00

M-J

0 / 33

0.01 (1)

I-J

-94 / 0

-129.6

-129.6

0.17 (1)

6.25

J-K

0 / 57

-129.6

-129.6

0.18 (1)

10.00

L-J

-408 / 0

0.0

0.0

0.04 (1)

7.81

T-S

0 / 0

-18.5

-18.5

0.01 (4)

10.00

S-R

0 / 24

-18.5

-18.5

0.01 (4)

10.00

R-Q

0 / 18

-18.5

-18.5

0.02 (4)

10.00

Q-P

0 / 13

-18.5

-18.5

0.02 (4)

10.00

P-O

0 / 13

-18.5

-18.5

0.02 (4)

10.00

O-N

0 / 18

-18.5

-18.5

0.02 (4)

10.00

N-M

0 / 24

-18.5

-18.5

0.01 (4)

10.00

M-L

0 / 0

-18.5

-18.5

0.01 (4)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1), BC=0.02/1.00 (N-O:4), WB=0.16/1.00 (F-P:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)

JSI METAL= 0.16 (E) (INPUT = 0.95)

03-21-23

H. J. G. ALVES

100009024

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

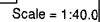
Structural component only

DWG# T-2303363

TOTAL WEIGHT = 66 lb

[M]

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:32 2023 Page 1
ID:82TzEwrlebruLmbXiL F9GzNTA6-kP?KYrvRnPKatrG6Qw0SpR1 DeA7CE5VnS6 7VzYi4H



15	16
[M]	

DESCR	SPF	SPF	SPF	SPF	SPF	SPF

SPF
SPF

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

LOADING
TOTAL LOAD CASES: (4)

LOADING
TOTAL LOAD CASES: (4)

DESIGNER		FACTORED		MAXIMUM FACTORED		INPUT	RECORD
BEARINGS		GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1948	0	1945	0	0	5-8	5-8
H	1720	0	1720	0	0	3-8	3-8

UNFACTORED REACTIONS						
1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
O	1355	1018 / 0	0 / 0	0 / 0	0 / 0	337 / 0
H	1199	884 / 0	0 / 0	0 / 0	0 / 0	315 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLT)	LC1 MAX CSI (L)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (L)
FR-TO		FROM-TO			FR-TO		
A-B	0/57	-129.6	-129.6 0.18 (1)	10.00	N-C	-285/13	0.10 (1)
B-C	-1704/0	-129.6	-129.6 0.46 (1)	4.55	C-M	0/1207	0.27 (1)
C-D	-2199/0	-129.6	-129.6 0.59 (1)	3.92	M-D	-729/0	0.26 (1)
D-E	-2199/0	-129.6	-129.6 0.59 (1)	3.89	M-E	0/20	0.00 (1)
E-F	-2185/0	-129.6	-129.6 0.60 (1)	3.91	K-E	-749/0	0.27 (1)
F-G	-1712/0	-129.6	-129.6 0.36 (1)	4.69	K-F	0/1179	0.27 (1)
O-B	-1919/0	0.0	0.0 0.13 (1)	7.25	J-F	-208/23	0.07 (1)
I-G	-1993/0	0.0	0.0 0.22 (1)	5.97	B-N	0/1358	0.31 (1)
					J-G	0/1623	0.37 (1)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (KIP)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 57	-129.6	-129.6 0.18 (1)	4.00	N-C	-265 / 13	0.10 (1)
B-C	-1704 / 0	-129.6	-129.6 0.46 (1)	10.55	C-M	0 / 1207	0.27 (1)
C-D	-2199 / 0	-129.6	-129.6 0.59 (1)	3.92	M-D	-729 / 0	0.26 (1)
D-E	-2199 / 0	-129.6	-129.6 0.59 (1)	3.89	M-E	0 / 20	0.00 (1)
E-F	-2185 / 0	-129.6	-129.6 0.60 (1)	3.91	K-E	-746 / 0	0.27 (1)
F-G	-1712 / 0	-129.6	-129.6 0.36 (1)	4.69	K-F	0 / 1179	0.27 (1)
O-B	-1919 / 0	0.0	0.0 0.13 (1)	7.25	J-F	-208 / 23	0.07 (1)
I-G	-1993 / 0	0.0	0.0 0.22 (1)	5.97	B-N	0 / 1358	0.31 (1)
					J-G	0 / 1623	0.37 (1)
O-N	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
N-M	0 / 1300	-18.5	-18.5 0.19 (1)	10.00			
M-L	0 / 2185	-18.5	-18.5 0.32 (1)	10.00			
L-K	0 / 2185	-18.5	-18.5 0.32 (1)	10.00			
K-J	0 / 1306	-18.5	-18.5 0.30 (1)	10.00			
J-I	-177 / 0	-18.5	-18.5 0.60 (1)	6.25			
I-H	0 / 0	-18.5	-18.5 0.37 (1)	10.00			

SPECIFIED LOADS:			
TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.60/1.00 (E-F:1), BC=0.60/1.00 (I-J:1),
WB=0.37/1.00 (G-J:1), SSI=0.63/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

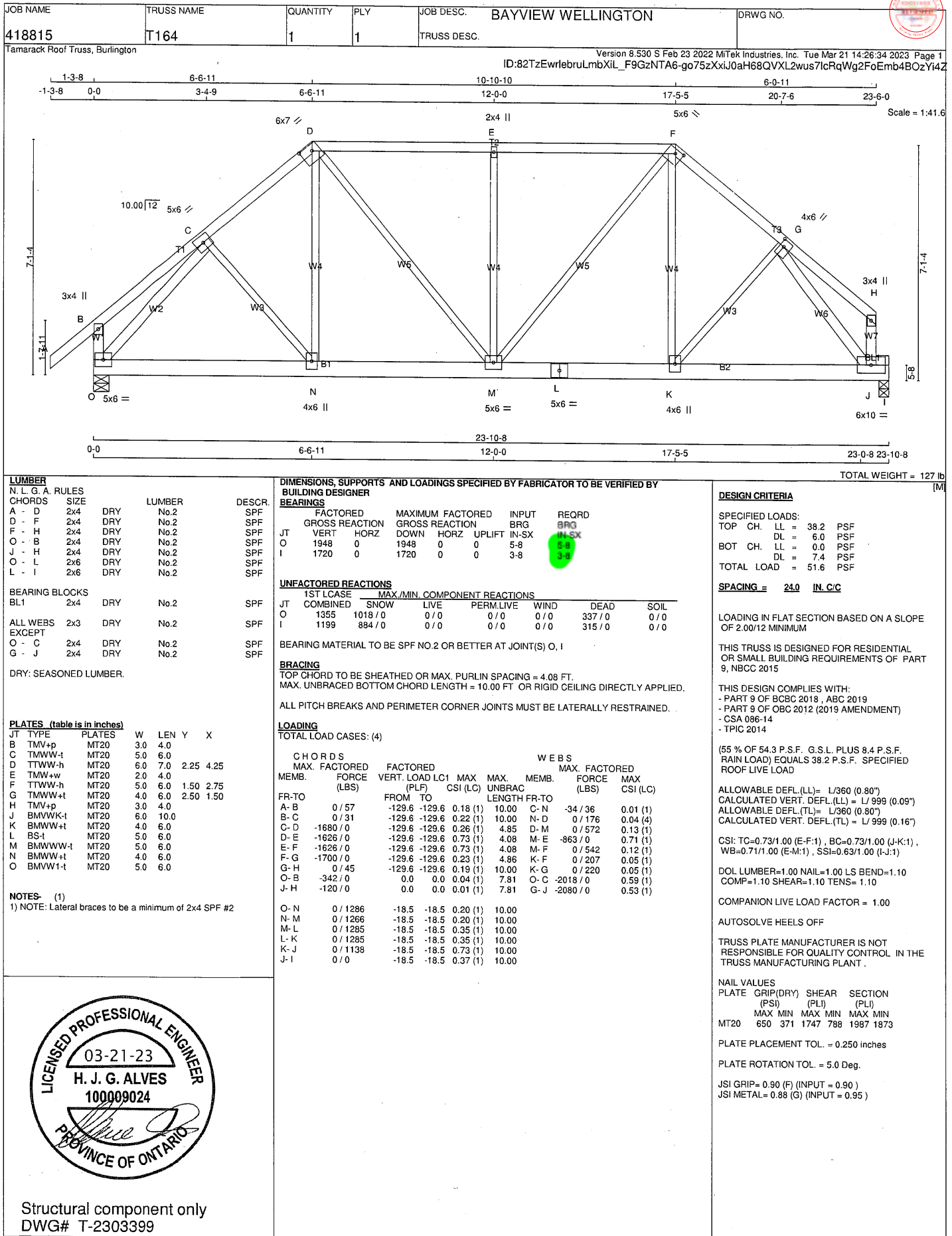
PLATE PLACEMENT TOL. = 0.250 inches

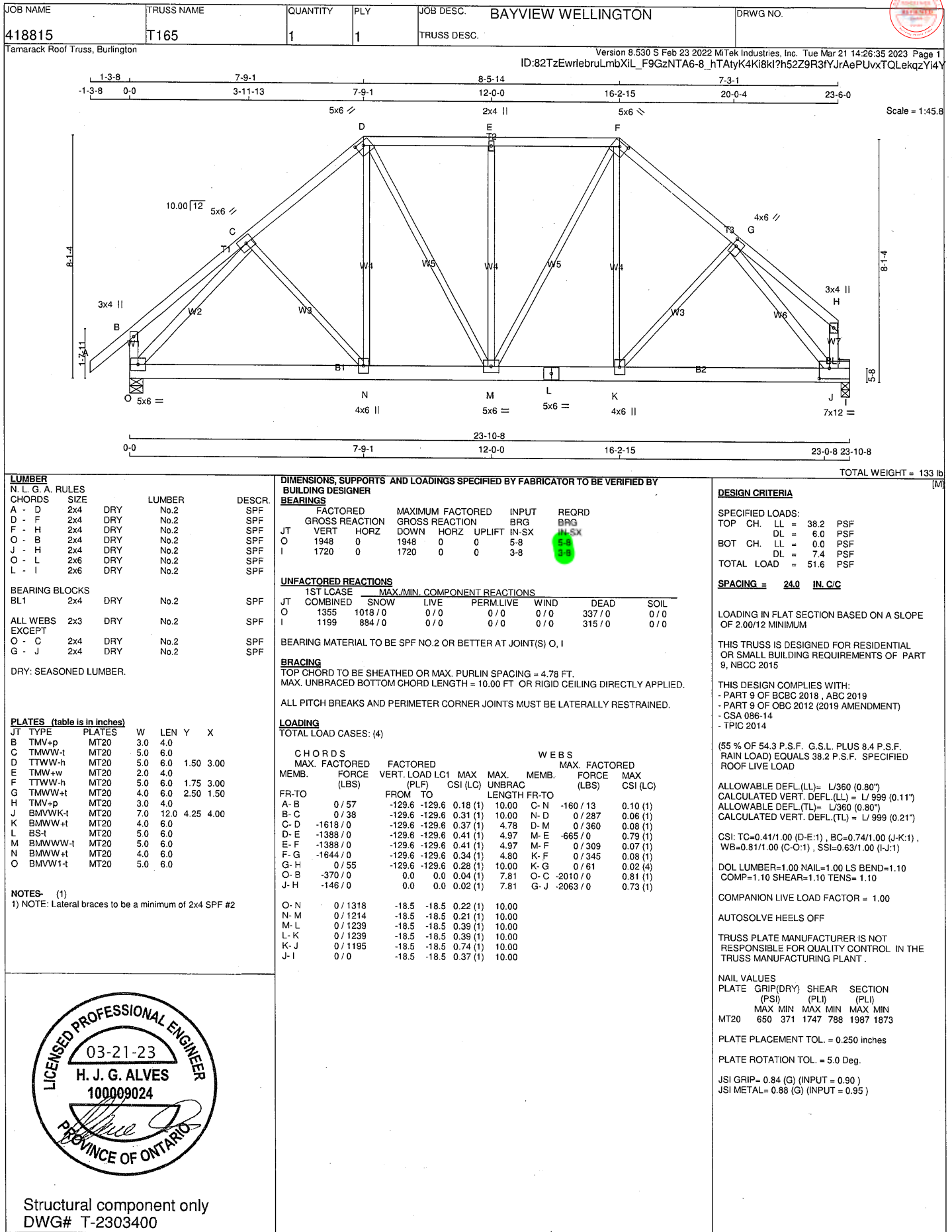
PLATE ROTATION TOL. = 5.0 Deg.

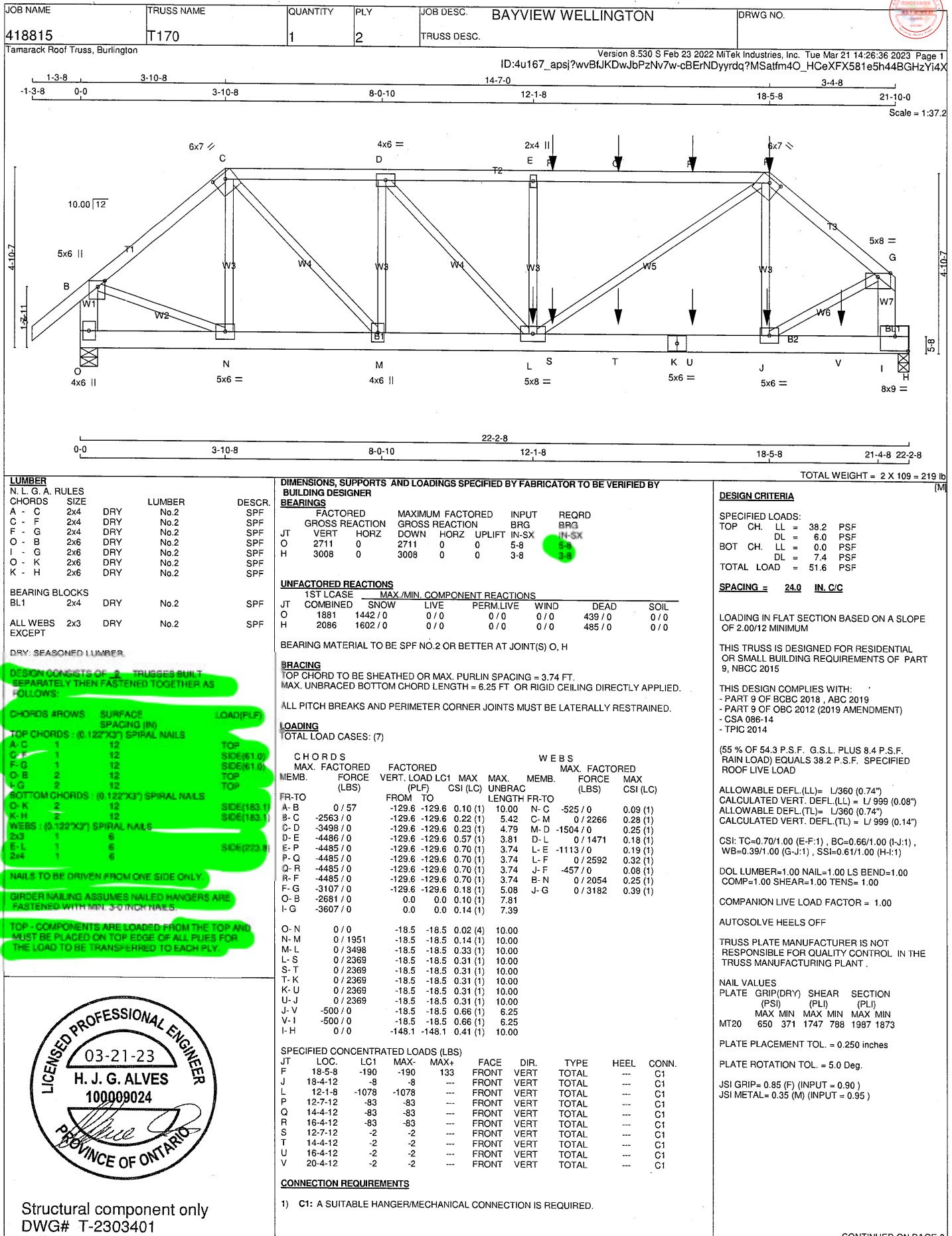
JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.43 (L) (INPUT = 0.95)



Structural component only
DWG# T-2303397







JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T170	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:36 2023 Page 2

ID:4u167 apsj?wvBfJKDwJbPzNv7w-cBErNDyyrdq?MSatfm4O HCeXFX581e5h44BGHzYi4X

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-h	MT20	6.0	7.0	1.75	3.75
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-h	MT20	6.0	7.0	1.50	3.50
G	TMVW-p	MT20	5.0	8.0	Edge	
I	BMVK-t	MT20	8.0	9.0	4.75	4.25
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

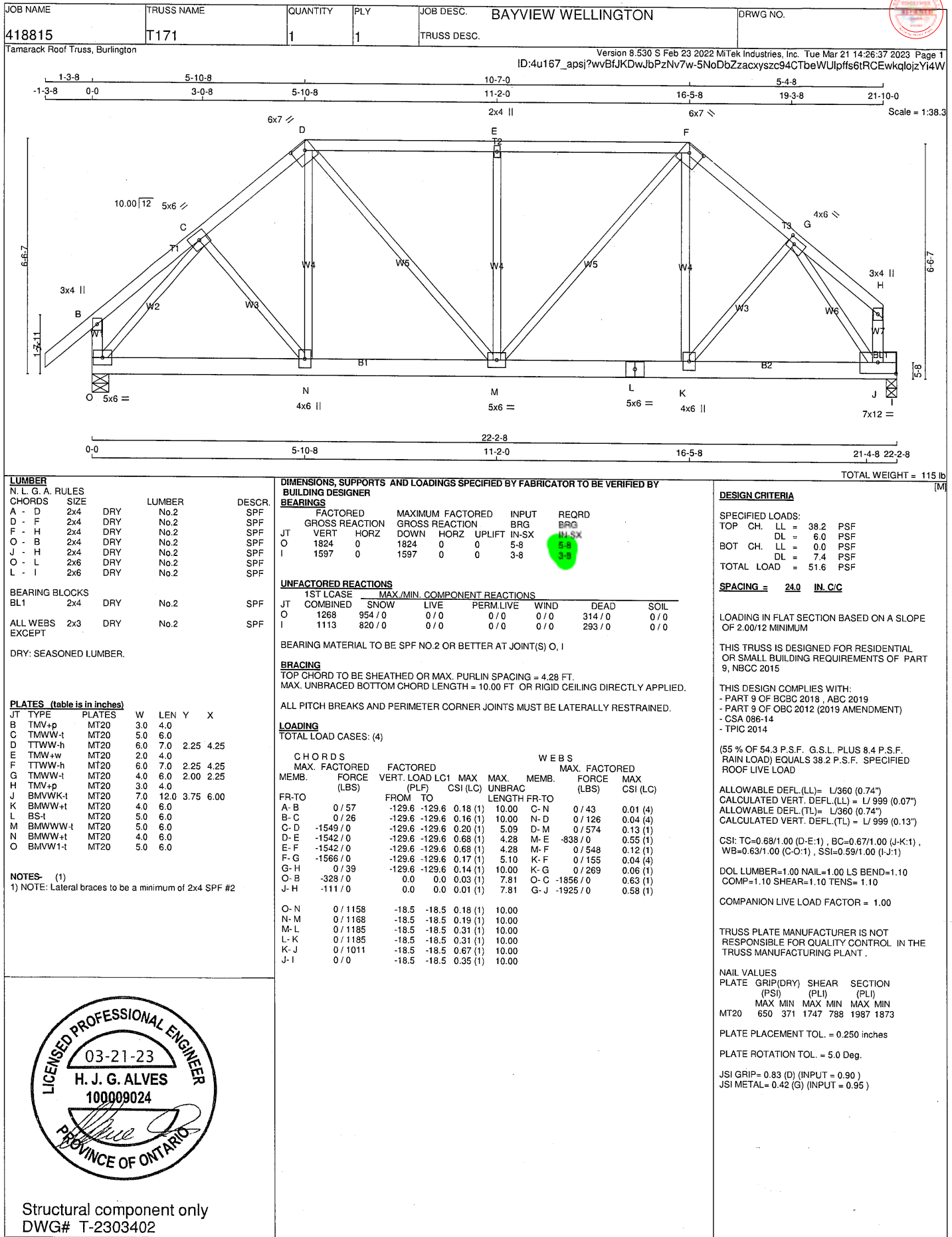
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

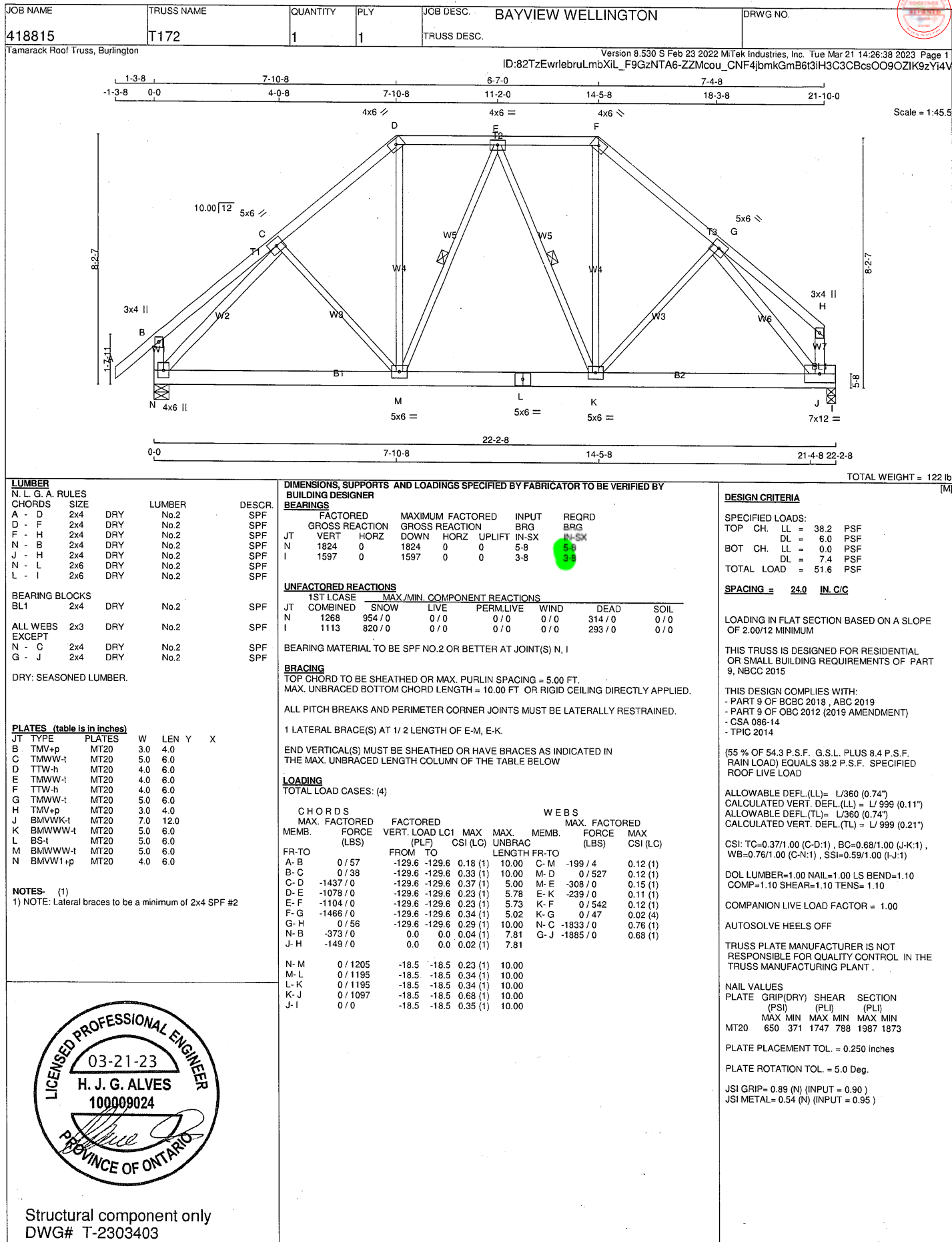
CONNECTION REQUIREMENTS

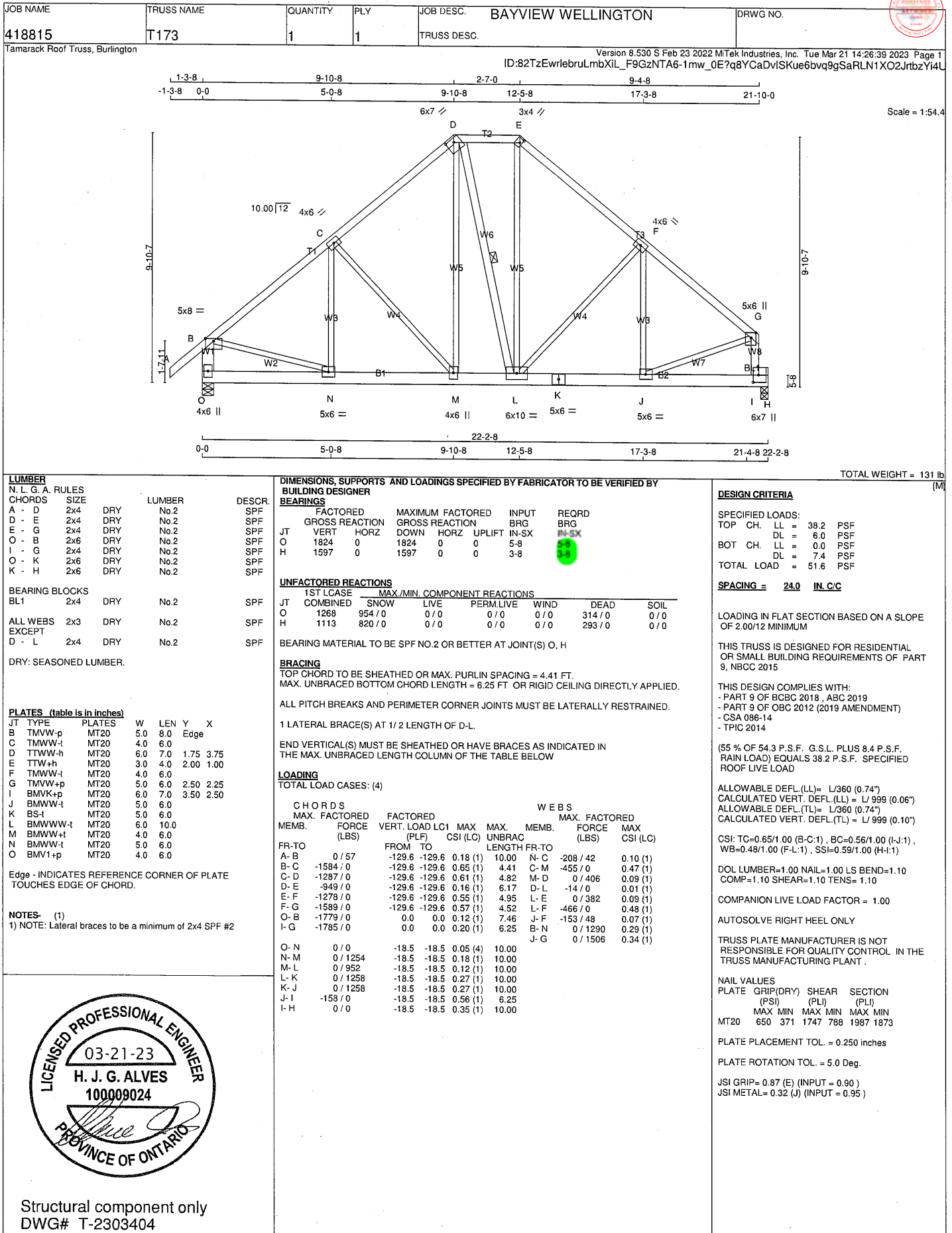
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2303401

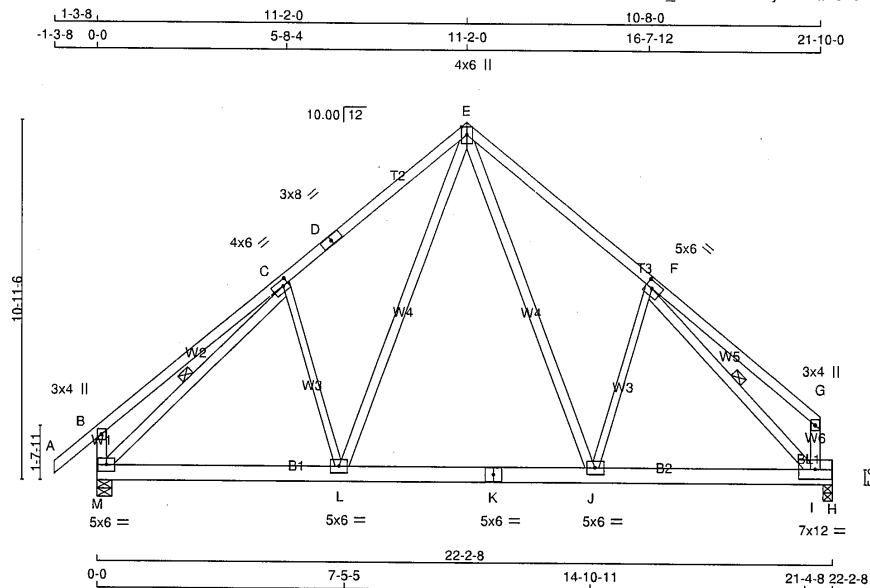






JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T174	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Tue Mar 21 14:26:40 2023 Page 1
ID:82TzEwrebruLmbXiL_F9GzNTA6-VyUMDa?SvsKRq3tfub9L87NHsSuS4pBgci2PP2zYi4T



Scale = 1:63.2

TOTAL WEIGHT = 129 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF

BEARING BLOCKS	SIZE	LUMBER	DESCR.
BL1	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
C - L	2x3	DRY	No.2
J - F	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	6.0	2.00	2.00
D TS-t	MT20	3.0	8.0		
E TTWW-m	MT20	4.0	6.0	Edge	
F TMWW-t	MT20	5.0	6.0	2.50	2.50
G TMV+p	MT20	3.0	4.0		
I BMVWK-t	MT20	7.0	12.0		
J BMWW-t	MT20	5.0	6.0		
K BS-t	MT20	5.0	6.0		
L BMWW-t	MT20	5.0	6.0		
M BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
M	1824	0	1824	0	5-8
H	1597	0	1597	0	3-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
M	1268	954 / 0	0 / 0	0 / 0	314 / 0	0 / 0	
H	1113	820 / 0	0 / 0	0 / 0	293 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 57	-129.6	-129.6 0.18 (1)	10.00	C-L	-518 / 0	0.34 (1)
B-C	0 / 57	-129.6	-129.6 0.68 (1)	10.00	L-E	0 / 642	0.10 (1)
C-D	-1471 / 0	-129.6	-129.6 0.84 (1)	4.15	E-J	0 / 699	0.11 (1)
D-E	-1471 / 0	-129.6	-129.6 0.84 (1)	4.15	J-F	-345 / 0	0.23 (1)
E-F	-1491 / 0	-129.6	-129.6 0.78 (1)	4.28	M-C	-1807 / 0	0.55 (1)
F-G	0 / 83	-129.6	-129.6 0.63 (1)	10.00	F-I	-1917 / 0	0.53 (1)
M-B	-448 / 0	0.0	0.0 0.05 (1)	7.81			
I-G	-220 / 0	0.0	0.0 0.02 (1)	7.81			
M-L	0 / 1243	-18.5	-18.5 0.22 (1)	10.00			
L-K	0 / 868	-18.5	-18.5 0.28 (1)	10.00			
K-J	0 / 868	-18.5	-18.5 0.28 (1)	10.00			
J-I	0 / 1211	-18.5	-18.5 0.70 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.35 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.84/1.00 (C-E:1), BC=0.70/1.00 (I-J:1),
WB=0.55/1.00 (C-M:1), SS=0.59/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

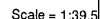
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 0.95)



Structural component only
DWG# T-2303405

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Tue Mar 21 14:26:41 2023 Page 1
ID:82TzEwrlebruLmbXiL F9GzNTA6-z82kRw05qATISDSrBjgahKvdRGI 8pJuarMoxvI lzYi4S



LUMBER

BUILDING BEARINGS

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8

DRY-SEASONED LUMBER

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0		Edge
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 57	-129.6	-129.6 0.18 (1)	10.00	I-D	0 / 418	0.09 (1)
B-C	0 / 28	-129.6	-129.6 0.14 (1)	10.00	E-I	-181 / 0	0.07 (1)
C-D	-593 / 0	-129.6	-129.6 0.14 (1)	6.25	C-I	-181 / 0	0.07 (1)
D-E	-593 / 0	-129.6	-129.6 0.14 (1)	6.25	J-C	-887 / 0	0.33 (1)
E-F	0 / 28	-129.6	-129.6 0.18 (1)	10.00	E-H	-887 / 0	0.33 (1)
F-G	0 / 57	-129.6	-129.6 0.18 (1)	10.00			
J-B	-328 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-328 / 0	0.0	0.0 0.03 (1)	7.81			
J-I	0 / 548	-18.5	-18.5 0.22 (4)	10.00			
I-H	0 / 548	-18.5	-18.5 0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.18/1.00 (B-C:1), BC=0.22/1.00 (H-I:4),
WB=0.33/1.00 (C-J:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 0.95)

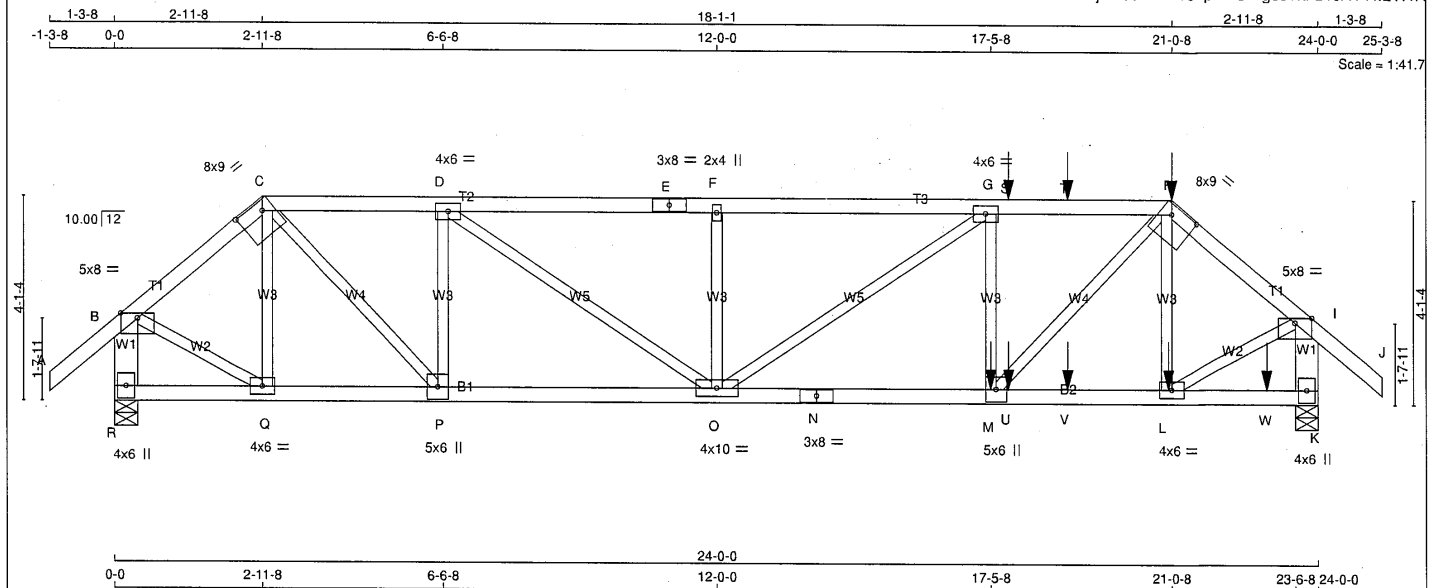


Structural component only
DWG# T-2303406

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T181X	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:42 2023 Page 1

ID:3QHkcHYZQvP6BVkzH0AtV7zNZde-RKc6eG1jRTb94N1170BpDYSkzgcYkFz40XVTWzY14R



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
R - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS

CHORDS	SPACING (IN)	LOAD (PLF)
TOP CHORDS (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-E	12	TOP
E-H	12	TOP
H-J	12	TOP
R-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS (0.122"x3") SPIRAL NAILS		
R-N	12	TOP
N-K	12	TOP
WEBS (0.122"x3") SPIRAL NAILS		
2x3	6	TOP
2x4	4	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

ORDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	2633	0	2633	0
K	4104	0	4104	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1830	1381 / 0
K	2852	2152 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX	WEBS	MAX. FACTORED	MAX
	MEMB.	FORCE	VERT. LOAD	CS (LC)		MEMB.	FORCE
FR-TO					FR-TO		
A-B	0 / 57	-129.6	-129.6	0.10 (1)	10.00	Q-C	-685 / 0
B-C	-2311 / 0	-129.6	-129.6	0.13 (1)	5.76	C-P	0 / 2475
C-D	-3450 / 0	-129.6	-129.6	0.24 (1)	4.79	P-D	-1718 / 0
D-E	-5014 / 0	-129.6	-129.6	0.39 (1)	3.99	D-O	0 / 1907
E-F	-5014 / 0	-129.6	-129.6	0.39 (1)	3.99	O-F	-722 / 0
F-G	-5014 / 0	-129.6	-129.6	0.39 (1)	3.99	O-G	-453 / 0
G-S	-5385 / 0	-129.6	-129.6	0.33 (1)	3.88	M-G	-599 / 0
S-T	-5385 / 0	-129.6	-129.6	0.33 (1)	3.88	M-H	0 / 3630
T-H	-5385 / 0	-129.6	-129.6	0.33 (1)	3.88	H-L	-1103 / 0
H-I	-3817 / 0	-129.6	-129.6	0.16 (1)	4.70	B-Q	0 / 1923
I-J	0 / 57	-129.6	-129.6	0.10 (1)	10.00	L-I	0 / 3176
R-B	-2613 / 0	0.0	0.0	0.10 (1)	7.81		
K-I	-4073 / 0	0.0	0.0	0.15 (1)	7.05		

R-Q	0 / 0	-18.5	-18.5	0.02 (4)	10.00
Q-P	0 / 1750	-18.5	-18.5	0.17 (1)	10.00
P-O	0 / 3451	-18.5	-18.5	0.34 (1)	10.00
O-N	0 / 5386	-18.5	-18.5	0.52 (1)	10.00
N-M	0 / 5386	-18.5	-18.5	0.52 (1)	10.00
M-U	0 / 2891	-18.5	-18.5	0.29 (1)	10.00
U-V	0 / 2891	-18.5	-18.5	0.29 (1)	10.00
V-L	0 / 2891	-18.5	-18.5	0.29 (1)	10.00
L-W	0 / 0	-18.5	-18.5	0.04 (4)	10.00
W-K	0 / 0	-18.5	-18.5	0.04 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	21-0-8	-28	-28	---	FRONT	VERT	DEAD	---	C1
H	21-0-8	-112	-112	---	BACK	VERT	TOTAL	---	C1
H	21-0-8	-176	-176	---	FRONT	VERT	SNOW	---	C1
L	20-11-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
M	17-5-8	-1347	-1347	---	BACK	VERT	TOTAL	---	C1
S	17-9-12	-106	-106	---	BACK	VERT	TOTAL	---	C1
T	18-11-12	-106	-106	---	BACK	VERT	TOTAL	---	C1
U	17-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	18-11-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	22-11-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL)= L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.39/1.00 (F-G-I), BC=0.52/1.00 (M-O-I), WB=0.45/1.00 (H-M-I), SSI=0.23/1.00 (G-H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.82 (N) (INPUT = 0.95)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2303407



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T181X	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:42 2023 Page 2

ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-RKc6eG1jRTb94N11?0BpDYskqccYkFz40XVTwzYi4F

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge	
C	TTWW-h	MT20	8.0	9.0	2.25	6.25
D	TMWV-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWV-t	MT20	4.0	6.0		
H	TTWW-h	MT20	8.0	9.0	2.25	6.25
I	TMWV-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	4.0	6.0		
L	BMWV-t	MT20	4.0	6.0		
M	BMWV+t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWVW-t	MT20	4.0	10.0		
P	BMWV+t	MT20	5.0	6.0		
Q	BMWV-t	MT20	4.0	6.0		
R	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

CONNECTION REQUIREMENTS

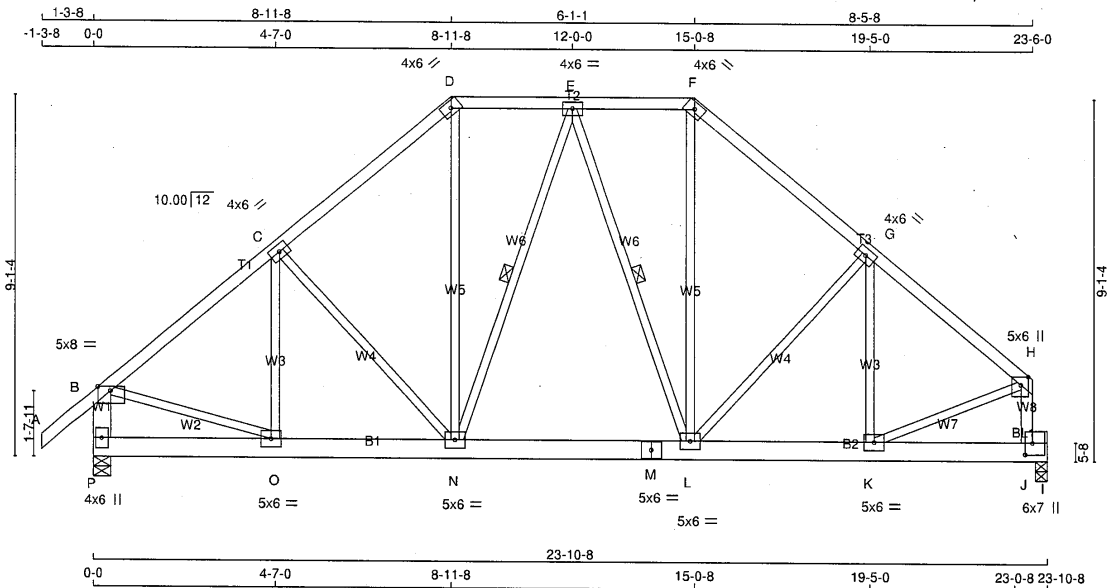
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2303407

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T182	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:43 2023 Page 1
ID:3QHkcHYZQvP6BVkzH0AtV7zNZde-vX9Urc2LBnj0hXcEZki2ml?w4whHCl7lgH30NzYi4Q



Scale = 1:52.3

TOTAL WEIGHT = 135 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF

BEARING BLOCKS	SIZE	LUMBER	DESCR.
BL1	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C, E, G						
C	TMVW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
F	TTW-h	MT20	4.0	6.0		
H	TMVW-p	MT20	5.0	6.0	2.50	2.25
J	BMVW-p	MT20	6.0	7.0	3.50	2.25
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMV1-p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX
P	1948	0	1948	0	5-8
I	1720	0	1720	0	3-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1355	1018 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0
I	1199	884 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B-C	-1723 / 0	-129.6	-129.6	0.53 (1)	4.45
C-D	-1531 / 0	-129.6	-129.6	0.50 (1)	4.68
D-E	-1145 / 0	-129.6	-129.6	0.19 (1)	5.71
E-F	-1132 / 0	-129.6	-129.6	0.19 (1)	5.74
F-G	-1509 / 0	-129.6	-129.6	0.45 (1)	4.80
G-H	-1730 / 0	-129.6	-129.6	0.46 (1)	4.53
P-B	-1901 / 0	0.0	0.0	0.13 (1)	7.28
J-H	-1956 / 0	0.0	0.0	0.22 (1)	6.02
P-O	0 / 0	-18.5	-18.5	0.04 (4)	10.00
O-N	0 / 1357	-18.5	-18.5	0.19 (1)	10.00
N-M	0 / 1216	-18.5	-18.5	0.18 (1)	10.00
M-L	0 / 1216	-18.5	-18.5	0.18 (1)	10.00
L-K	0 / 1362	-18.5	-18.5	0.28 (1)	10.00
K-J	-173 / 0	-18.5	-18.5	0.60 (1)	6.25
J-I	0 / 0	-18.5	-18.5	0.37 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 39.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.60/1.00 (J-K:1) ,
WB=0.37/1.00 (H-K:1) , SSI=0.63/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

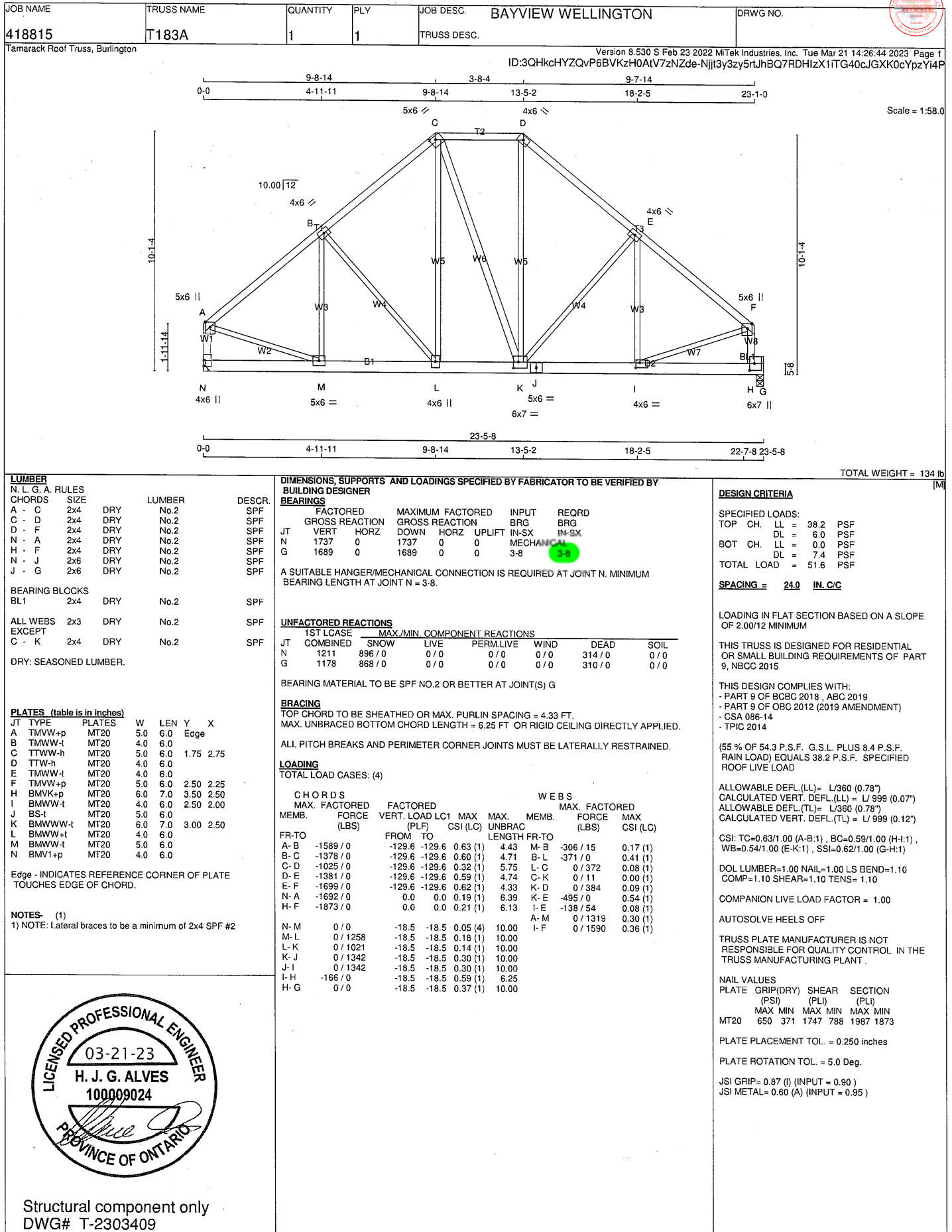
PLATE PLACEMENT TOL. = 0.250 inches

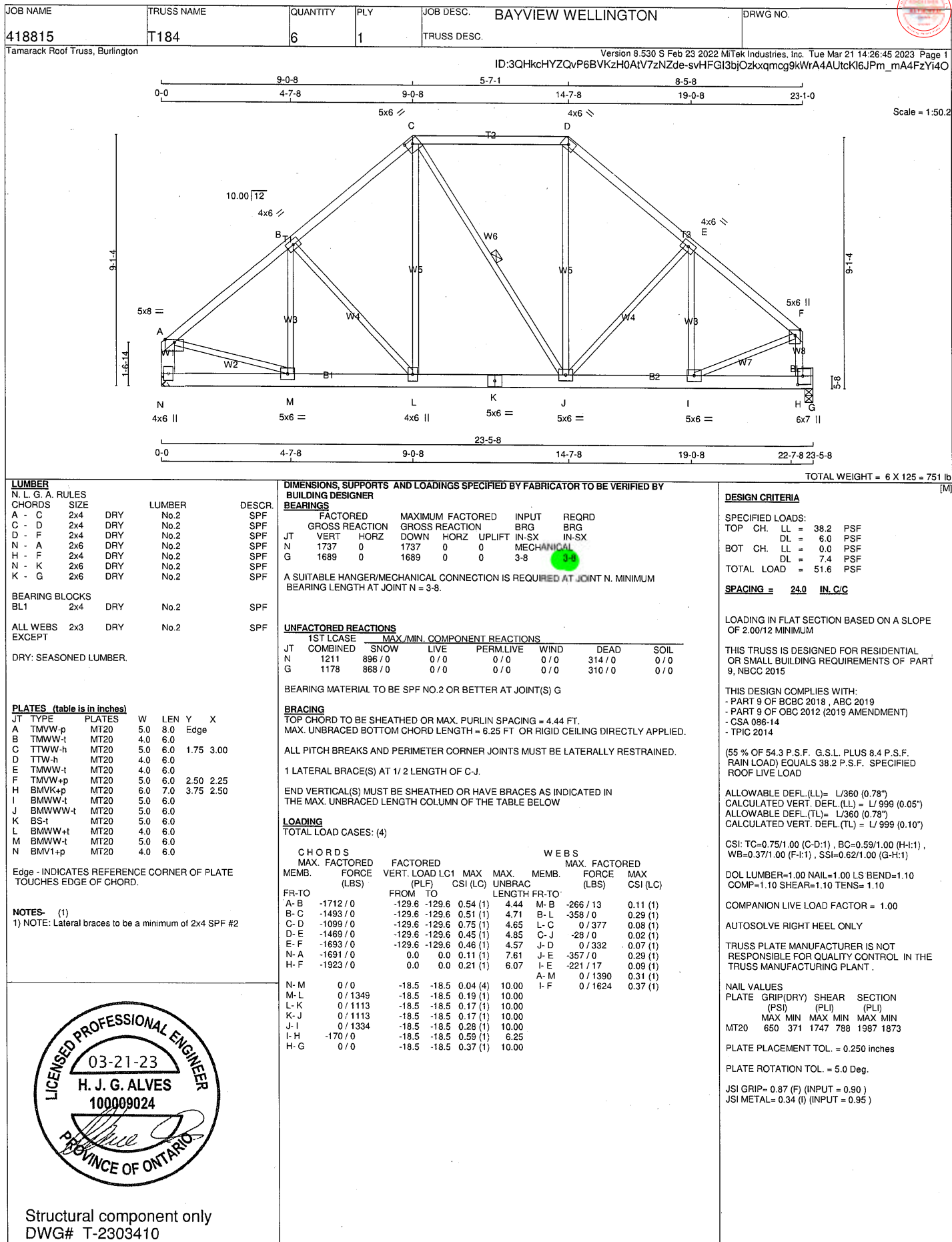
PLATE ROTATION TOL. = 5.0 Deg.

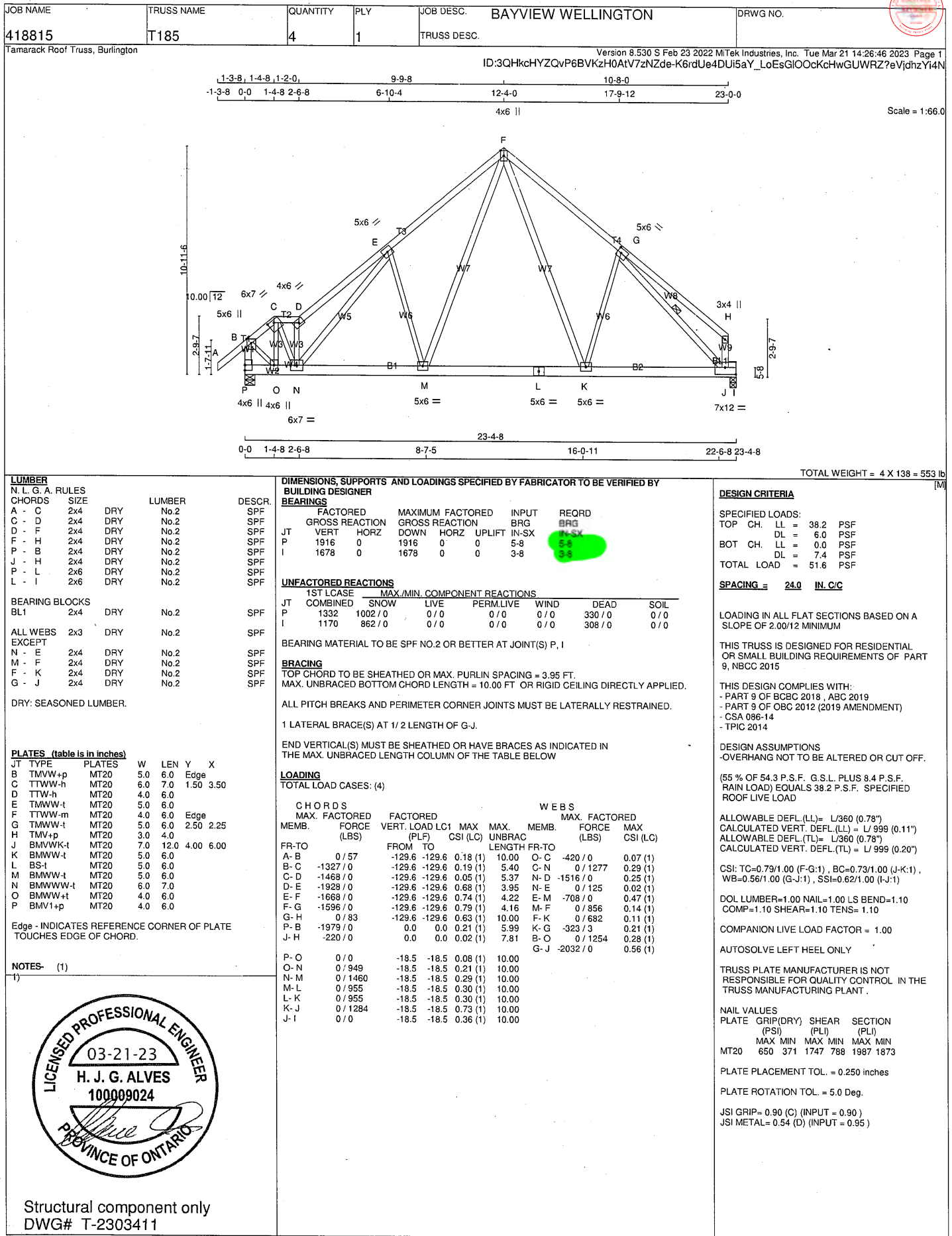
JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.35 (K) (INPUT = 0.95)



Structural component only
DWG# T-2303408







JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

TRUSS DESC.

DRWG NO.

418815

T186

2

1

BAYVIEW WELLINGTON

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:47 2023 Page 1

ID:3QHkcHYZQvP6BVkZH0AtV7zNZde-olP?hz5rF0DRA8w?oan_wb9eOhNyD?4iDIFG98zYi4M

1-3-8

6-6-0

6-6-0

1-3-8

-1-3-8

0-0

3-4-4

6-6-0

9-7-12

13-0-0

14-3-8

4x6 II

Scale = 1:41.9

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMVW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BMVW1-t	MT20	4.0	6.0	
J	BMVW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
J	1143	0	1143	0
H	1143	0	1143	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN. COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	793	603 / 0	0 / 0	0 / 0	191 / 0	0 / 0
H	793	603 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 57	-129.6 -129.6	0.18 (1)	10.00	I-D	0 / 468	
B-C	0 / 31	-129.6 -129.6	0.22 (1)	10.00	I-E	-207 / 0	
C-D	-654 / 0	-129.6 -129.6	0.17 (1)	6.25	C-I	-207 / 0	
D-E	-654 / 0	-129.6 -129.6	0.17 (1)	6.25	J-C	-972 / 0	
E-F	0 / 31	-129.6 -129.6	0.22 (1)	10.00	E-H	-972 / 0	
F-G	0 / 57	-129.6 -129.6	0.18 (1)	10.00			
J-B	-340 / 0	0.0	0.04 (1)	7.81			
H-F	-340 / 0	0.0	0.04 (1)	7.81			
J-I	0 / 610	-18.5	-18.5	0.26 (4)	10.00		
I-H	0 / 610	-18.5	-18.5	0.26 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.43")

CALCULATED VERT. DEFL. (LL) = L/999 (0.02")

ALLOWABLE DEFL. (TL) = L/360 (0.43")

CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.22/1.00 (B-C:1), BC=0.26/1.00 (I-J:4), WB=0.41/1.00 (E-H:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(F5I) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (C) (INPUT = 0.90)

JSI METAL= 0.22 (C) (INPUT = 0.95)

TOTAL WEIGHT = 2 X 62 = 124 lb

[M/F]

LICENSED PROFESSIONAL ENGINEER

03-21-23

H. J. G. ALVES

100009024

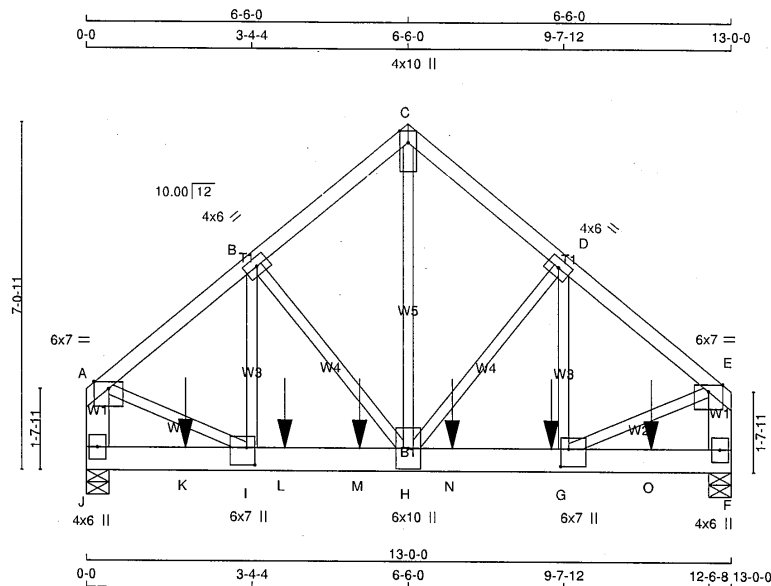
PROVINCE OF ONTARIO

Structural component only

DWG# T-2303412

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T187	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:48 2023 Page 1
ID:3QHkcHYZQvP6BVkzH0AtV7zNZde-GUzNvJ6U0JLloUBMHIDTtpj5esyOHsSy_qhazY14L



Scale = 1:42.2

TOTAL WEIGHT = 2 X 71 = 142 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
J - A	2x6 DRY	No.2	SPF		
F - E	2x6 DRY	No.2	SPF		
J - F	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS.

CHORDS SHOWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS - (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-E	12	TOP
J-A	12	TOP
F-E	12	TOP
BOTTOM CHORDS - (0.122"x3") SPIRAL NAILS		
J-F	12	SIDE (183 L)
WEBS - (0.122"x3") SPIRAL NAILS		
G-I	6	SIDE (265 L)
O-F	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PILES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
J	5988	0	5988	0
F	6251	0	6251	0

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	4167	3117 / 0	0 / 0	0 / 0	0 / 0	1050 / 0	0 / 0
F	4350	3254 / 0	0 / 0	0 / 0	0 / 0	1096 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		
A-B	-5766 / 0	-129.6 -129.6 0.24 (1)	3.88	H-C	0 / 5415	0.67 (1)
B-C	-4523 / 0	-129.6 -129.6 0.17 (1)	4.38	H-D	-1486 / 0	0.32 (1)
C-D	-4523 / 0	-129.6 -129.6 0.17 (1)	4.38	G-D	0 / 1461	0.18 (1)
D-E	-5683 / 0	-129.6 -129.6 0.24 (1)	3.91	B-H	-1590 / 0	0.34 (1)
J-A	-5512 / 0	0.0 0.0 0.20 (1)	6.25	I-B	0 / 1596	0.20 (1)
F-E	-5436 / 0	0.0 0.0 0.20 (1)	6.29	A-I	0 / 4727	0.59 (1)
				G-E	0 / 4660	0.58 (1)
J-K	0 / 0	-18.5 -18.5 0.34 (1)	10.00			
K-L	0 / 0	-18.5 -18.5 0.34 (1)	10.00			
L-M	0 / 4442	-18.5 -18.5 0.60 (1)	10.00			
M-H	0 / 4442	-18.5 -18.5 0.60 (1)	10.00			
H-N	0 / 4378	-18.5 -18.5 0.59 (1)	10.00			
N-G	0 / 4378	-18.5 -18.5 0.59 (1)	10.00			
G-O	0 / 0	-18.5 -18.5 0.47 (1)	10.00			
O-F	0 / 0	-18.5 -18.5 0.47 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	9-4-12	-1196	-1196	---	FRONT	VERT	TOTAL	---	C1
K	2-0-4	-1196	-1196	---	FRONT	VERT	TOTAL	---	C1
L	4-0-4	-1196	-1196	---	FRONT	VERT	TOTAL	---	C1
M	5-6-4	-1196	-1196	---	FRONT	VERT	TOTAL	---	C1
N	7-4-12	-1196	-1196	---	FRONT	VERT	TOTAL	---	C1
O	11-4-12	-1196	-1196	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.2 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.24/1.00 (A-B:1), BC=0.60/1.00 (H-I:1),
WB=0.67/1.00 (C-H:1), SSI=0.63/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)

JSI METAL= 0.67 (I) (INPUT = 0.95)



Structural component only
DWG# T-2303413

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	T187	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:48 2023 Page 2
ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-GUzNvJ6U0JLl0IUBMHIDTpij5esyOHsSy qhazYi4L

PLATES (table is in inches)

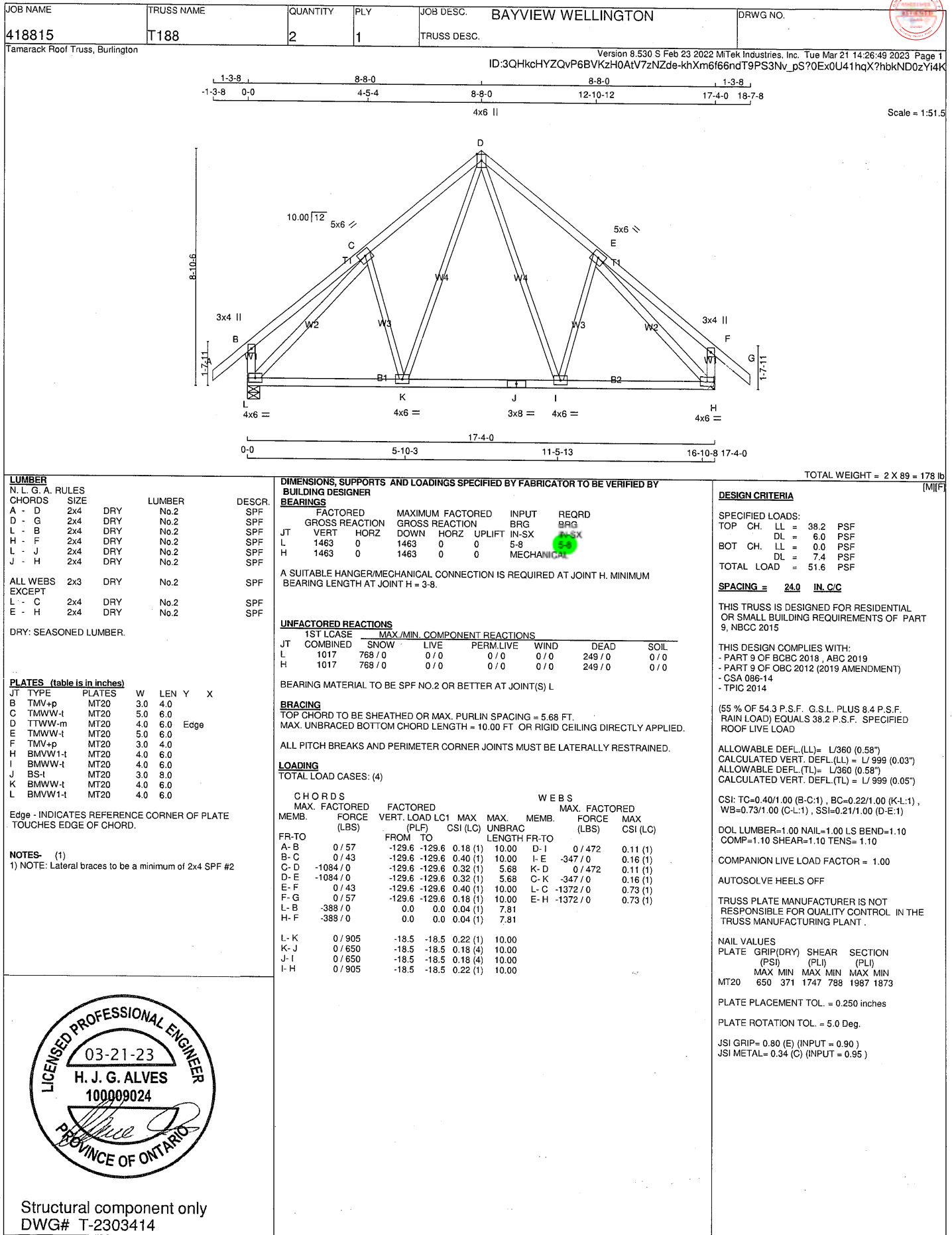
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	7.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	10.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-p	MT20	6.0	7.0	Edge	
F	BMV1+p	MT20	4.0	6.0		
G	BMWW+t	MT20	6.0	7.0	4.25	2.00
H	BMWW+t	MT20	6.0	10.0		
I	BMWW-t	MT20	6.0	7.0	4.25	2.00
J	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2303413



JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

DRWG NO.

418815

G188

1

1

BAYVIEW WELLINGTON

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:25:57 2023 Page 1

ID:QAT1O_rFuS1W_JAGIV03rfzNAqV-GgO6CtU1ewXASSOt4Fv256bXN1VQ_LkW8dP1zY158

1-3-8

8-8-0

8-8-0

8-8-0

1-3-8

-1-3-8

0-0

8-8-0

8-8-0

17-0-8

17-4-0

18-7-8

4x6 II

Scale = 1:52.3

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

SPF

ALL WEBS

2x3

DRY

No.2

SPF

ALL GABLE WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT

2-0-0

OC.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMWw+p

MT20

4.0

6.0

Edge

C

D, E, F, H, I, J, K

MT20

2.0

4.0

L

TMWw+p

MT20

4.0

6.0

Edge

N

BMV1+p

MT20

3.0

4.0

O

BMWW1-t

MT20

4.0

6.0

P, Q, R, T, U, V, W

MT20

2.0

4.0

S

BS-t

MT20

3.0

8.0

X

BMWW1-t

MT20

4.0

6.0

Y

BMV1+p

MT20

3.0

4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

FORCE

VERT. LOAD

LC1

MAX

CS1 (LC)

UNBRAC

LENGTH

FR-TO

Y-B

-422 / 0

0.0

0.0

0.04 (1)

7.81

T-G

-190 / 0

0.09 (1)

A-B

0 / 57

-129.6

-129.6

0.18 (1)

10.00

U-F

-294 / 0

0.27 (1)

B-C

-105 / 0

-129.6

-129.6

0.17 (1)

6.25

V-E

-254 / 0

0.12 (1)

C-D

-20 / 0

-129.6

-129.6

0.05 (1)

6.25

W-D

-232 / 0

0.05 (1)

D-E

-31 / 0

-129.6

-129.6

0.06 (1)

6.25

X-C

-36 / 0

0.01 (1)

E-F

-24 / 0

-129.6

-129.6

0.07 (1)

6.25

R-H

-294 / 0

0.27 (1)

F-G

-39 / 0

-129.6

-129.6

0.07 (1)

6.25

Q-I

-254 / 0

0.12 (1)

G-H

-39 / 0

-129.6

-129.6

0.07 (1)

6.25

P-J

-232 / 0

0.05 (1)

H-I

-24 / 0

-129.6

-129.6

0.07 (1)

6.25

O-K

-36 / 0

0.01 (1)

I-J

-31 / 0

-129.6

-129.6

0.06 (1)

6.25

B-X

0 / 45

0.01 (1)

J-K

-20 / 0

-129.6

-129.6

0.05 (1)

6.25

O-L

0 / 45

0.01 (1)

K-L

-105 / 0

-129.6

-129.6

0.17 (1)

6.25

L-M

0 / 57

-129.6

-129.6

0.18 (1)

10.00

N-L

-422 / 0

0.0

0.0

0.04 (1)

7.81

Y-X

0 / 0

-18.5

-18.5

0.01 (4)

10.00

X-W

0 / 32

-18.5

-18.5

0.01 (4)

10.00

W-V

0 / 26

-18.5

-18.5

0.02 (4)

10.00

V-U

0 / 21

-18.5

-18.5

0.02 (4)

10.00

U-T

0 / 17

-18.5

-18.5

0.02 (4)

10.00

T-S

0 / 17

-18.5

-18.5

0.02 (4)

10.00

S-R

0 / 17

-18.5

-18.5

0.02 (4)

10.00

R-Q

0 / 21

-18.5

-18.5

0.02 (4)

10.00

Q-P

0 / 26

-18.5

-18.5

0.02 (4)

10.00

P-O

0 / 32

-18.5

-18.5

0.01 (4)

10.00

O-N

0 / 0

-18.5

-18.5

0.01 (4)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.

LL

=

38.2

PSF

DL

=

6.0

PSF

BOT CH.

LL

=

0.0

PSF

DL

=

7.4

PSF

TOTAL LOAD

=

51.6

PSF

SPACING

=

24.0

IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.18/1.00 (L-M:1), BC=0.02/1.00 (P-Q:4), WB=0.27/1.00 (H-R:1), SS1=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION

(FSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (H) (INPUT = 0.90)

JSI METAL= 0.16 (H) (INPUT = 0.95)

03-21-23

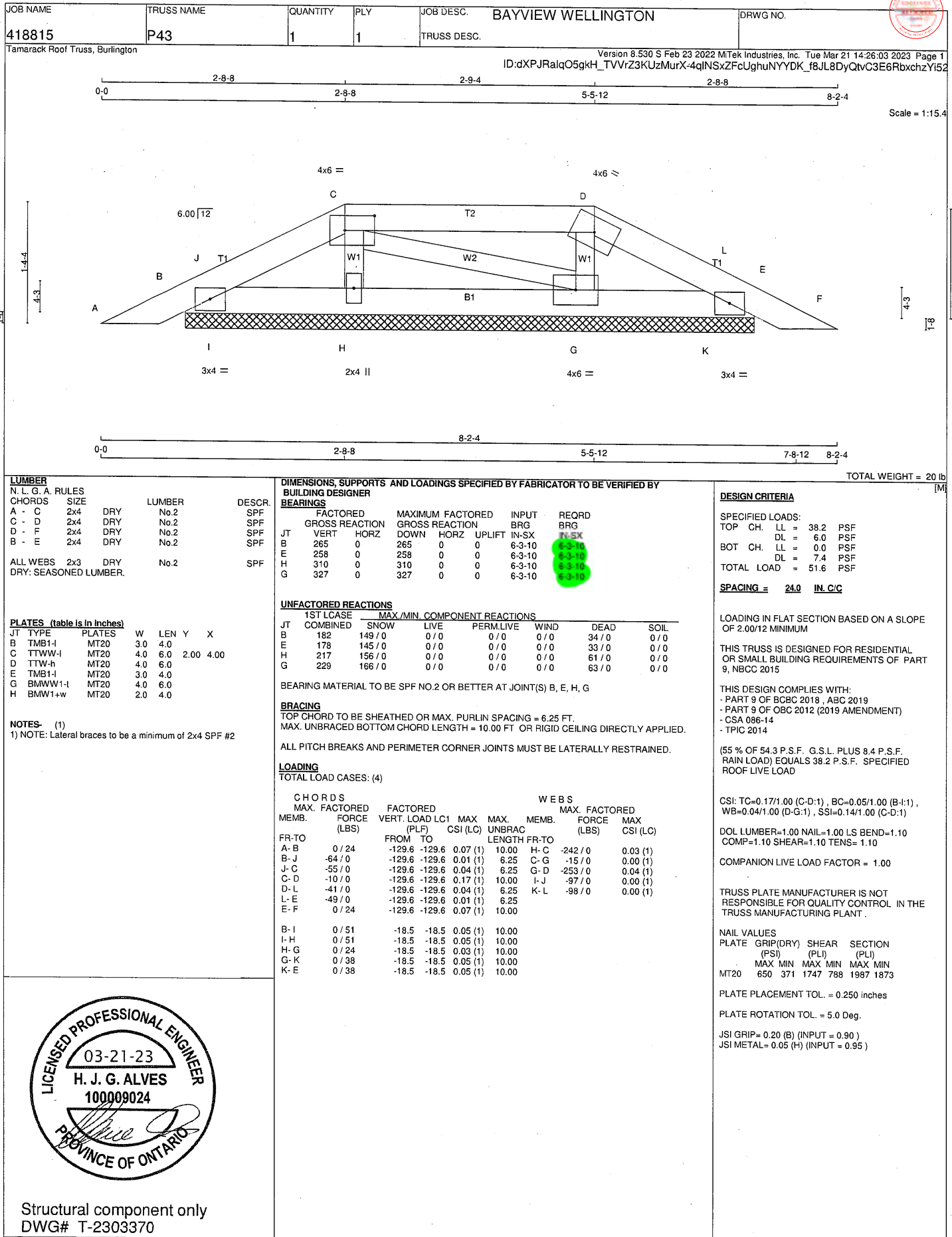
H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

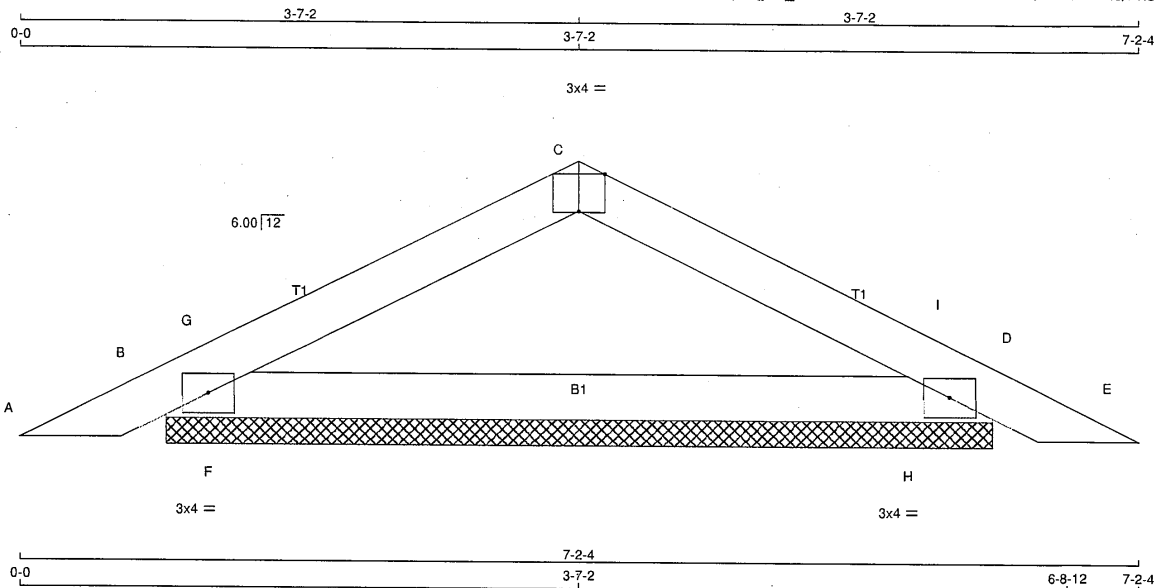
Structural component only

DWG# T-2303364



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	P45	6	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:05 2023 Page 1
ID:dXPJRalqO5gkH_TVvZ3KUzMurX-1Dt7caV86wP8hiwL117DkQT113zN66XZl42hazY150



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
B	506	0	0	5-3-10
D	506	0	0	5-3-10

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	351	270 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
D	351	270 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
MEMB.							
FR-TO							
A-B	0 / 24	-129.6	-129.6	0.07 (1)	10.00	F-G	0 / 90
B-G	-453 / 0	-129.6	-129.6	0.08 (4)	6.25	H-I	0 / 90
G-C	-365 / 0	-129.6	-129.6	0.15 (1)	6.25		
C-I	-365 / 0	-129.6	-129.6	0.15 (1)	6.25		
I-D	-453 / 0	-129.6	-129.6	0.08 (4)	6.25		
D-E	0 / 24	-129.6	-129.6	0.07 (1)	10.00		
B-F	0 / 335	-18.5	-18.5	0.08 (1)	10.00		
F-H	0 / 335	-18.5	-18.5	0.14 (1)	10.00		
H-D	0 / 335	-18.5	-18.5	0.08 (1)	10.00		

TOTAL WEIGHT = 6 X 15 = 91 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-G:1), BC=0.14/1.00 (F-H:1), WB=0.00/1.00 (F-G:1), SSI=0.12/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.37 (D) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 0.95)

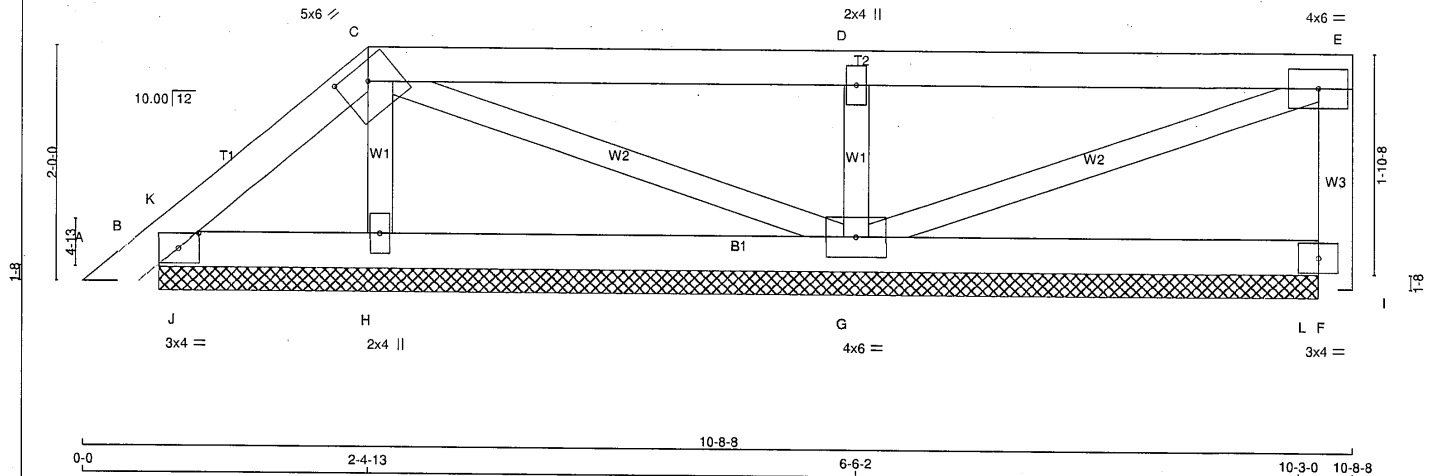
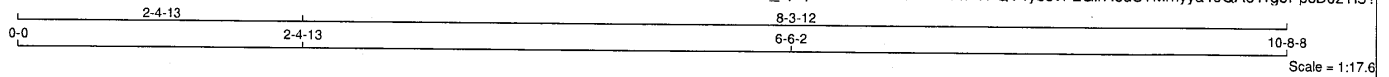


Structural component only
DWG# T-2303371

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	P101	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Tue Mar 21 14:26:06 2023 Page 1

ID:4u167_apsj?wvBfJKDwJbPzNv7w-VPQV4yb8vP2GlrH6uSYMmya49QA6YrgoPpcD0zYi5?



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50 2.00
C	TTWW-h	MT20	5.0	6.0	1.75 3.00
D	TMW-w	MT20	2.0	4.0	
E	TMW-I	MT20	4.0	6.0	
F	BVMH	MT20	3.0	4.0	
G	BMWW1-t	MT20	4.0	6.0	
H	BMW1-w	MT20	2.0	4.0	

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		REQD	
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
B	222	0	222	0	0	9-9-7	9-9-7	9-9-7
H	329	0	329	0	0	9-9-7	9-9-7	9-9-7
G	768	0	768	0	0	9-9-7	9-9-7	9-9-7
L	244	0	244	0	0	9-9-7	9-9-7	9-9-7

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	153	122 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
B	231	159 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
H	533	410 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
L	171	120 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 20	-129.6	-129.6 0.03 (1)	10.00	H-C	-235 / 0	0.03 (1)
B-K	-22 / 0	-129.6	-129.6 0.02 (1)	6.25	C-G	-63 / 3	0.02 (1)
K-C	-56 / 0	-129.6	-129.6 0.04 (1)	6.25	G-D	-674 / 0	0.10 (1)
C-D	-12 / 37	-129.6	-129.6 0.39 (1)	6.25	G-E	-39 / 13	0.01 (1)
D-E	-12 / 36	-129.6	-129.6 0.39 (1)	6.25	J-K	-160 / 0	0.00 (1)
I-F	0 / 0	0.0	0.0 0.00 (1)	10.00			
F-E	-192 / 0	0.0	0.0 0.02 (1)	7.81			
B-J	0 / 39	-18.5	-18.5 0.05 (1)	10.00			
J-H	0 / 39	-18.5	-18.5 0.05 (1)	10.00			
H-G	0 / 23	-18.5	-18.5 0.07 (4)	10.00			
G-L	0 / 0	-18.5	-18.5 0.08 (1)	10.00			
L-F	0 / 0	-18.5	-18.5 0.08 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (C-D:1), BC=0.08/1.00 (F-L:1), WB=0.10/1.00 (D-G:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

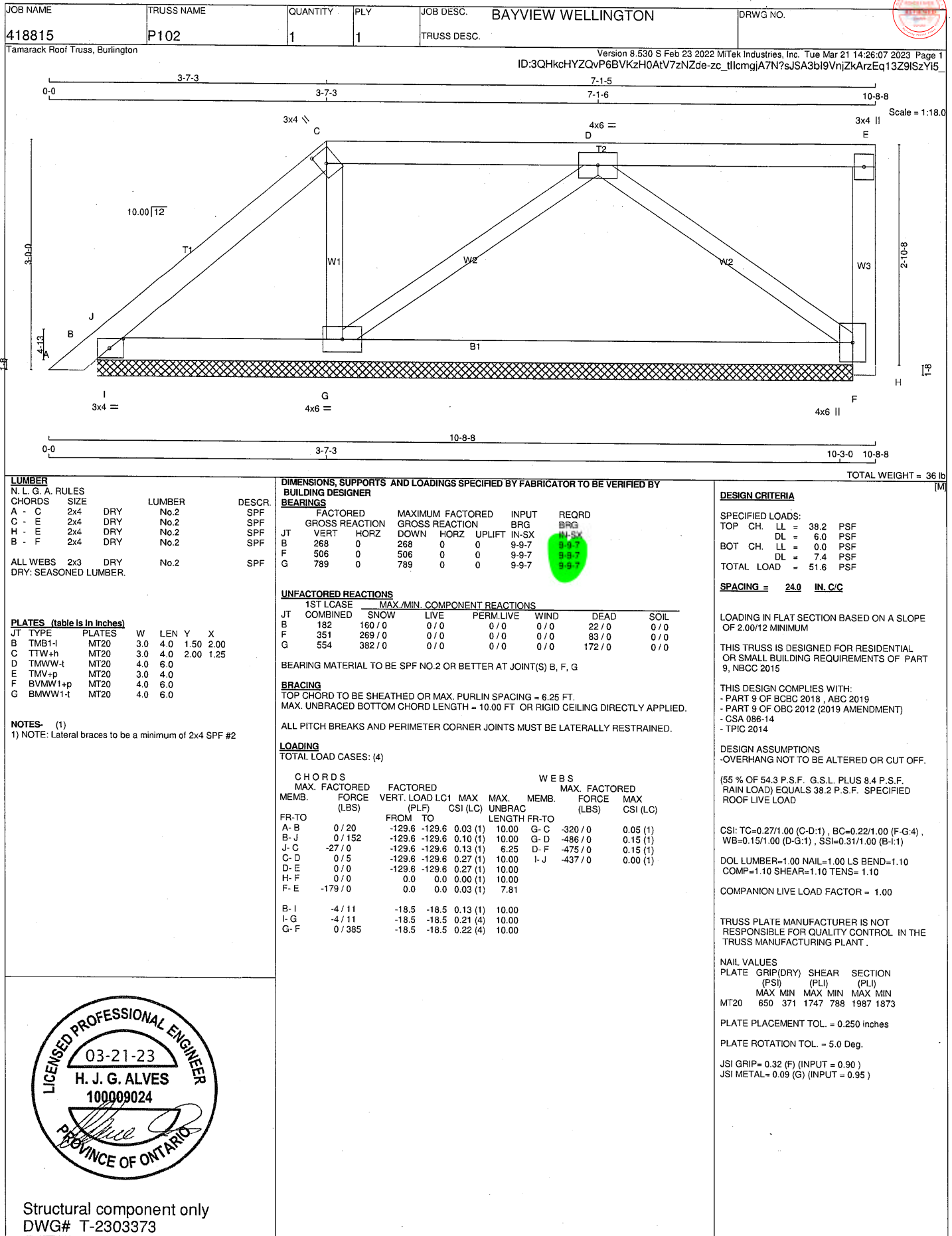
PLATE PLACEMENT TOL. = 0.250 inches

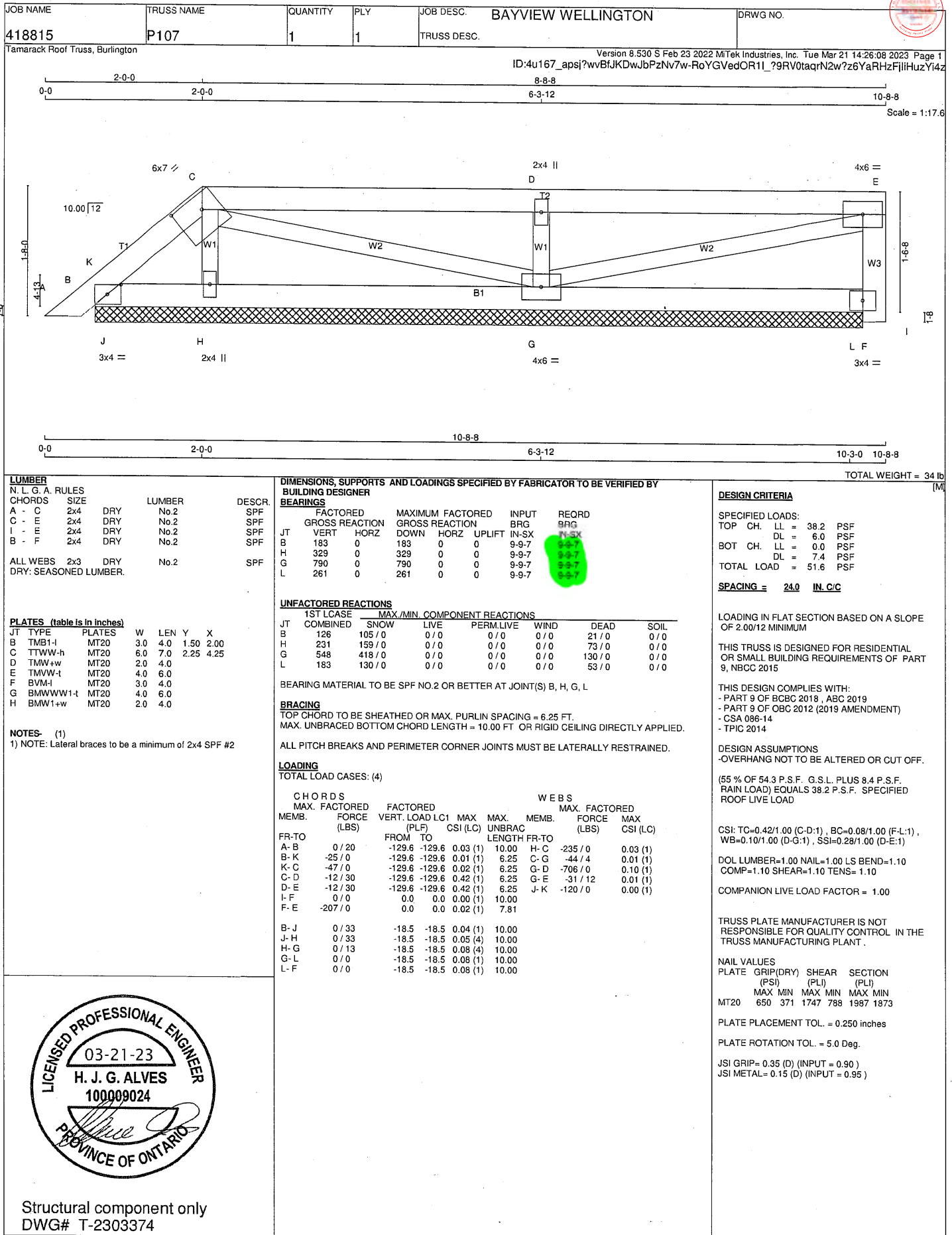
PLATE ROTATION TOL. = 5.0 Deg.

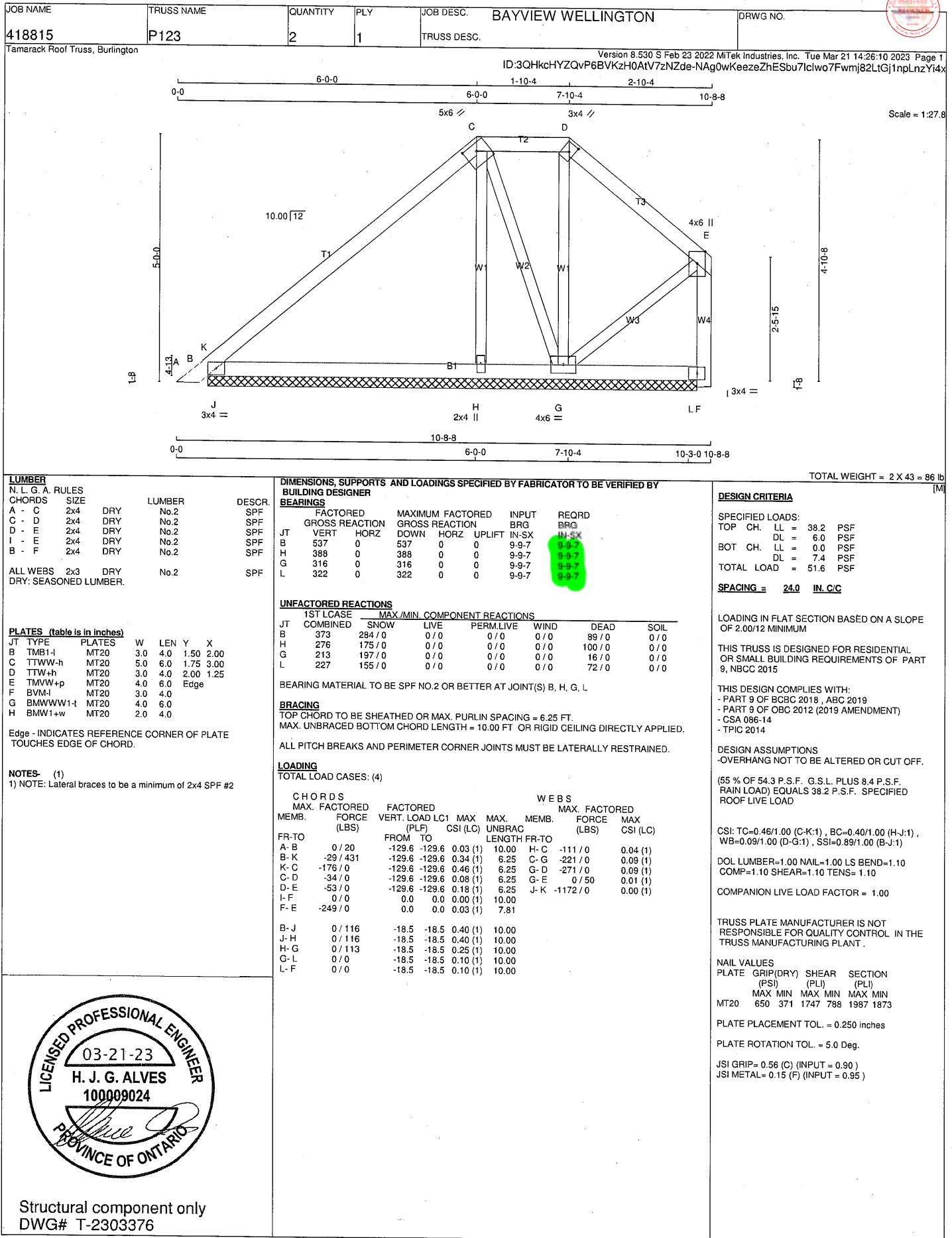
JSI GRIP= 0.34 (D) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 0.95)



Structural component only
DWG# T-2303372





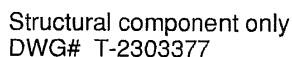


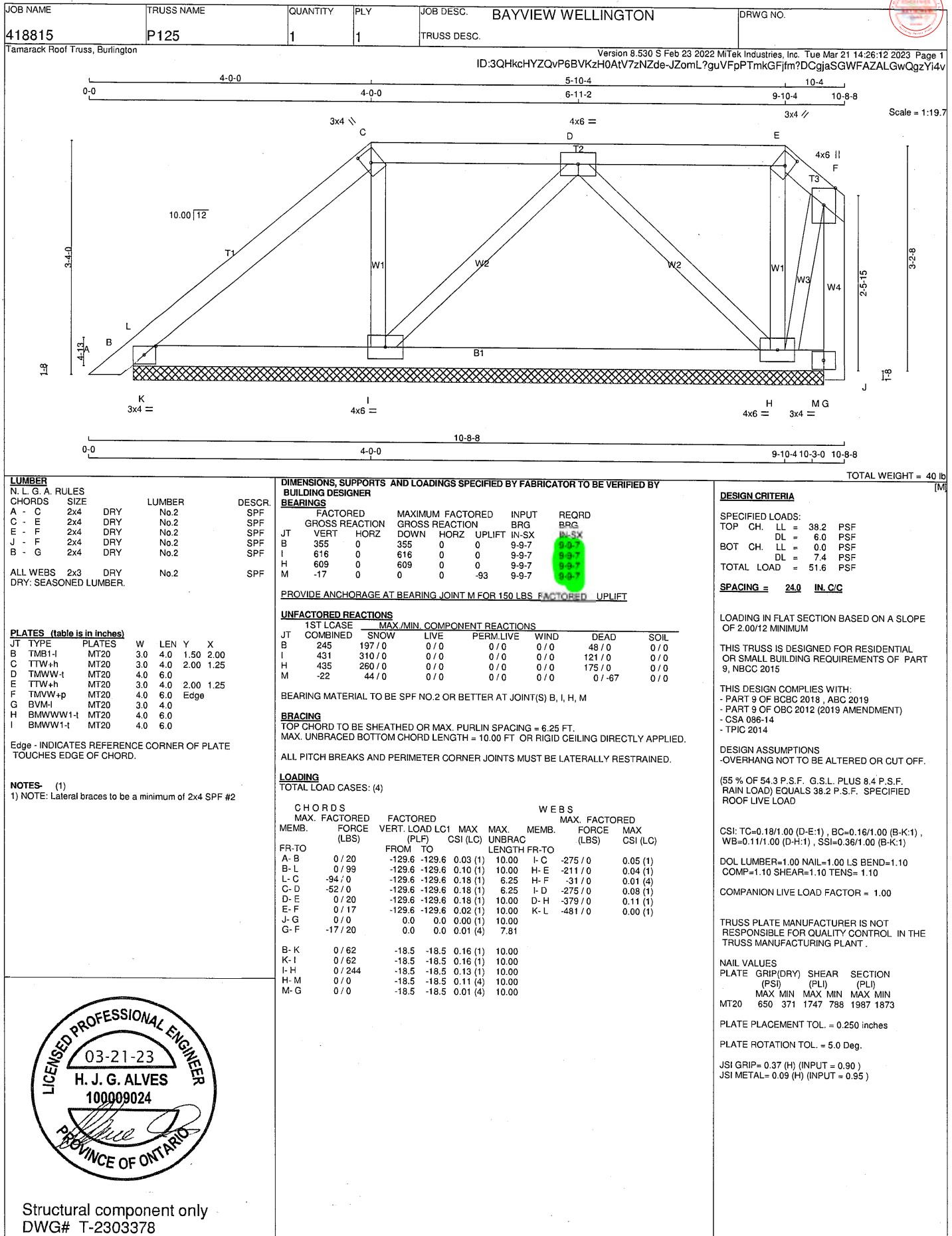
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:11 2023 Page 1
ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-rNEO8afGkyhYsc94h07XT?qaSmA6Nnm8PvhXMDpYi4w

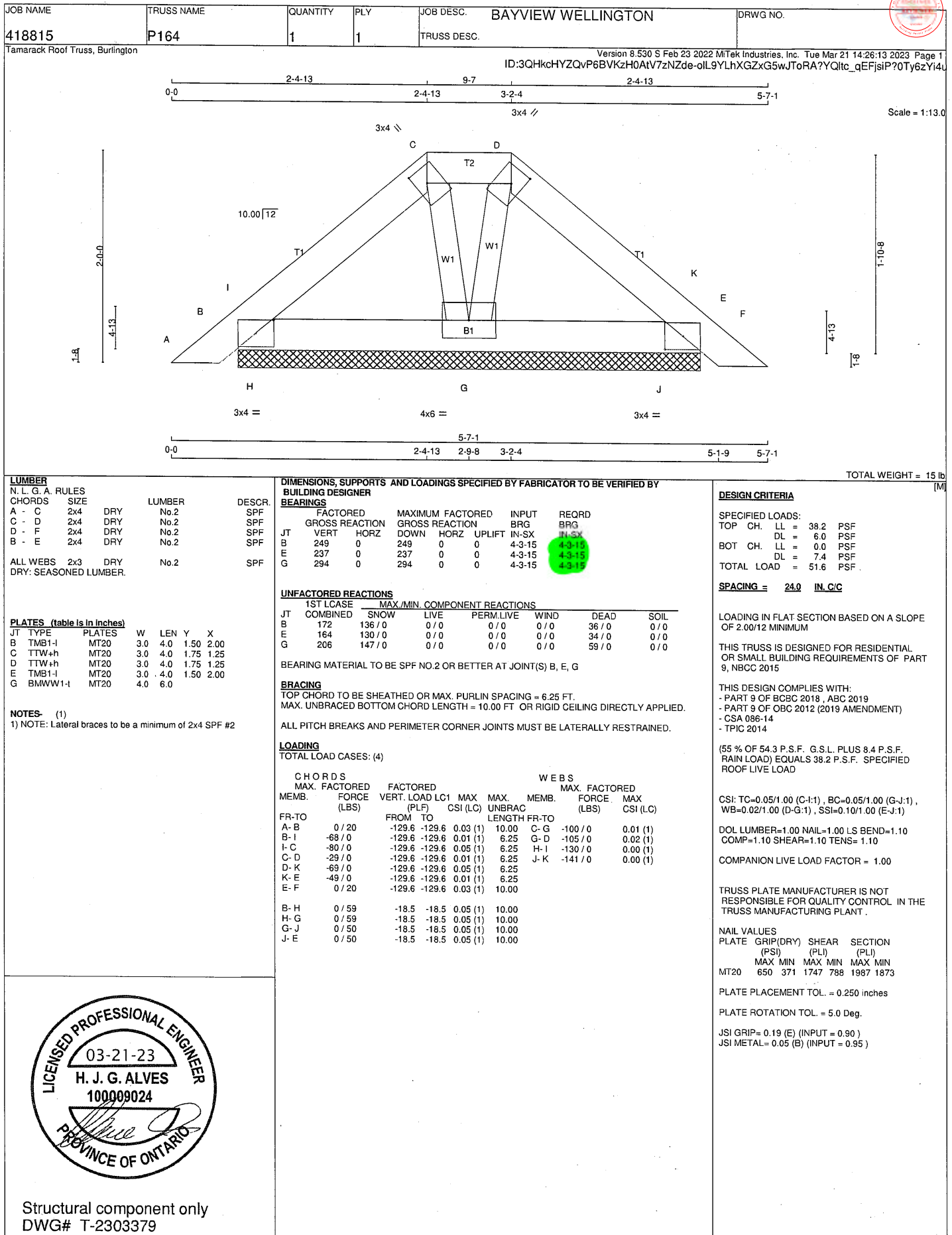


TOTAL WEIGHT = $2 \times 41 = 81$ lb

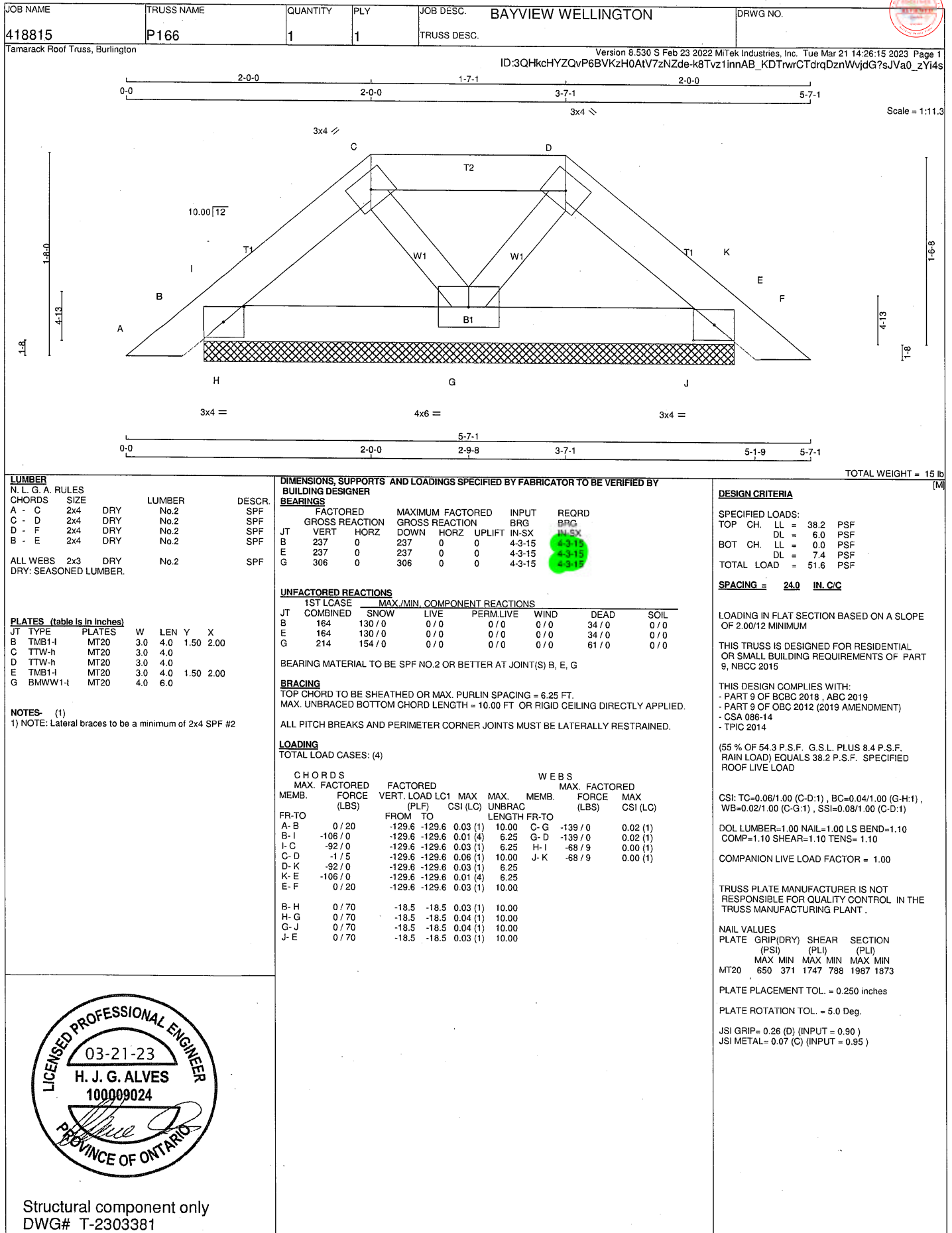
JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 0.95)







JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418815	P165	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:14 2023 Page 1	
				ID:3QHkcHYZQvP6BVkzH0AtV7zNZde-GyvXmhh91137i4ufM8hE5el1bN9v_AKrefl1UYzY14t	
TOTAL WEIGHT = 3 X 13 = 40 lb					
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER A - C 2x4 DRY No.2 C - E 2x4 DRY No.2 B - D 2x4 DRY No.2 DRY: SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX B 390 0 390 0 0 4-3-15 4-3-15 D 390 0 390 0 0 4-3-15 4-3-15		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.2 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 51.6 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.10/1.00 (C-I:1), BC=0.09/1.00 (F-H:1), WB=0.00/1.00 (F-G:1), SSI=0.08/1.00 (C-I:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 798 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.27 (B) (INPUT = 0.90) JSI METAL= 0.08 (D) (INPUT = 0.95)	
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMB1-I MT20 3.0 4.0 1.50 2.00 C TT-p MT20 3.0 4.0 Edge 2.00 D TMB1-I MT20 3.0 4.0 1.50 2.00 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES- (1) 1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2		UNFACTORED REACTIONS 1ST LCASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL B 271 206 / 0 0 / 0 0 / 0 64 / 0 0 / 0 D 271 206 / 0 0 / 0 0 / 0 64 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX. FACTORED (PLF) CSI (LC) UNBRAC LENGTH FR-TO A-B 0 / 20 -129.6 -129.6 0.03 (1) 10.00 F-G -5 / 90 0.00 (1) B-G -301 / 0 -129.6 -129.6 0.05 (4) 6.25 H-I -5 / 90 0.00 (1) G-C -204 / 0 -129.6 -129.6 0.10 (1) 6.25 C-I -204 / 0 -129.6 -129.6 0.10 (1) 6.25 I-D -301 / 0 -129.6 -129.6 0.05 (4) 6.25 D-E 0 / 20 -129.6 -129.6 0.03 (1) 10.00 B-F 0 / 162 -18.5 -18.5 0.04 (1) 10.00 F-H 0 / 162 -18.5 -18.5 0.09 (1) 10.00 H-D 0 / 162 -18.5 -18.5 0.04 (1) 10.00			
Structural component only DWG# T-2303380					



JOB NAME

418815

TRUSS NAME

J1

QUANTITY

8

PLY

1

JOB DESC.

BAYVIEW WELLINGTON

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:25:58 2023 Page 1

ID:3QHkcHYZQvP6BVkzH0AtV7zNZde-ktXUPDV6oy2OocgaQnOURGd8CxhUEXNVyAtAxTzYI57

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	716	0	716	0	5-8	
C	286	0	286	0	1-8	
D	45	0	50	0	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		SNOW	LIVE	PERM.LIVE	IN-SX			
E	496	384 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	
C	195	168 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0	
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS				
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC1 MIN	UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO					
E-B	-652 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 39	-129.6	-129.6	0.17 (1)	10.00		
B-C	-43 / 0	-129.6	-129.6	0.76 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.76/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)

JSI METAL= 0.18 (B) (INPUT = 0.95)

03-21-23

H. J. G. ALVES

100009024

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

Structural component only

DWG# T-2303365

JOB NAME
418815

TRUSS NAME
J6

QUANTITY
4

PLY
1

JOB DESC.
BAYVIEW WELLINGTON

TRUSS DESC.

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Tue Mar 21 14:25:59 2023 Page 1

ID:3QHkcHYZQvP6BVkzH0AtV7zNZde-C3VscZWkZGAFQmFm_UwJ_TAPiK1UzOdeBqdkTwzYI56

Scale = 1:27.3

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	467	0	467	0	5-8	5-8
C	251	0	251	0	1-8	1-8
D	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	322	254 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
C	171	148 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO F-B	-431 / 0	0.0	0.0	0.05 (1)
A-B	0 / 57	-129.6	-129.6	0.18 (5)
B-C	0 / 0	-129.6	-129.6	0.33 (1)
F-E	0 / 0	-18.5	-18.5	0.07 (4)
E-D	0 / 0	-18.5	-18.5	0.08 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 51.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")

CALCULATED VERT. DEFL. (LL) = L/999 (0.00")

ALLOWABLE DEFL. (TL) = L/360 (0.19")

CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.33/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PS)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)

JSI METAL= 0.11 (B) (INPUT = 0.95)

Structural component only
DWG# T-2303366

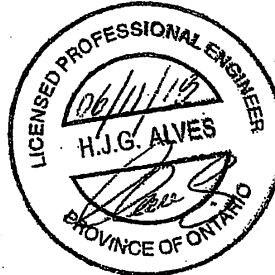
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Mar 21 14:26:01 2023 Page 1
ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-8Sdc1FX?4tQz4fO96vvB3uFqb8kwRl6xe86vYozY154

Scale = 1:18.7

TOTAL WEIGHT = 10 lb

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 0.95)

Structural component only
DWG# T-2303368



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering *em and General Safety notes.

T-1900218

Feb 09, 2018



TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

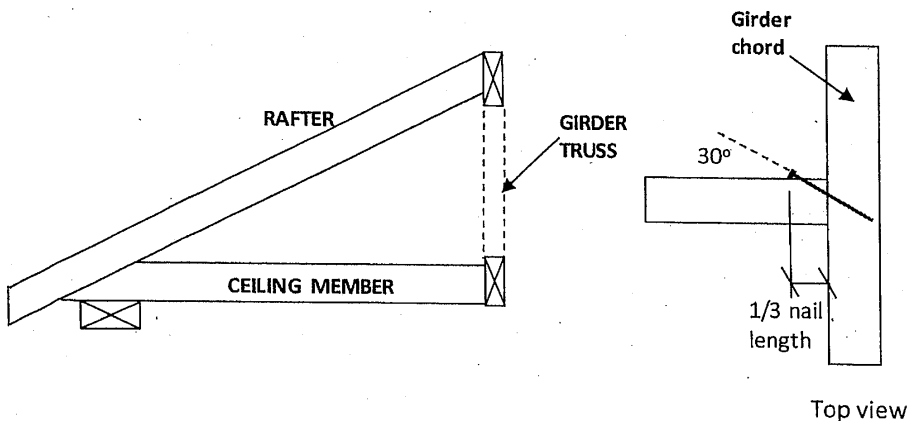


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

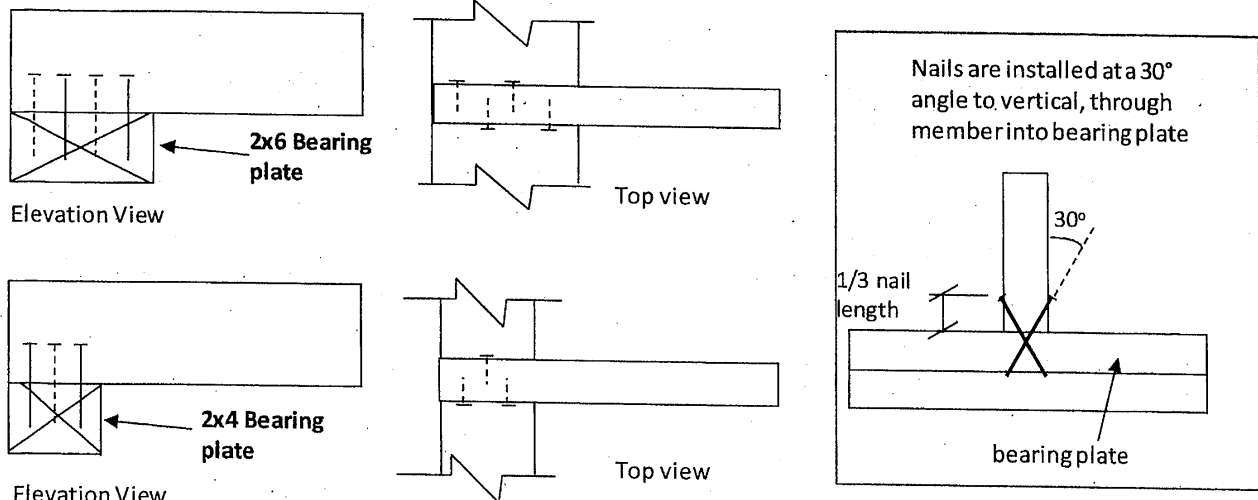
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485





LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

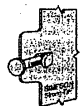
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

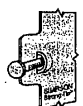
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on pp. 105–107.



Double-Shear Nailing Top View



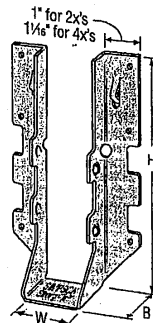
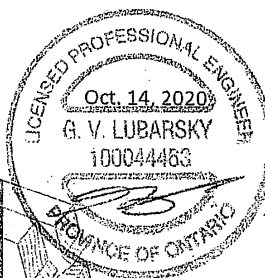
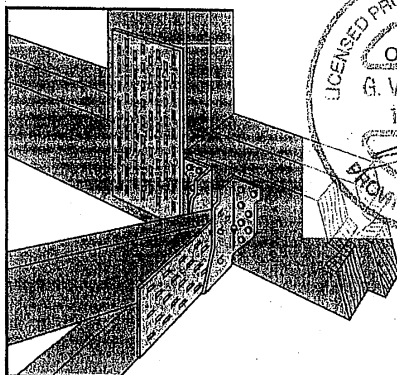
Double-Shear Nailing Side View; Do not bend tab



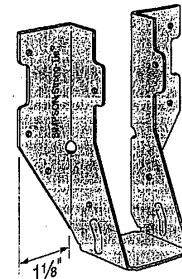
Dome Double-Shear Nailing Side View (available on some models)

Typical HUS26 Installation with Reduced Heel Height

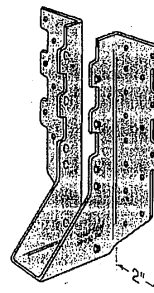
(Truss Designer to provide fastener quantity for connecting multiple members together)



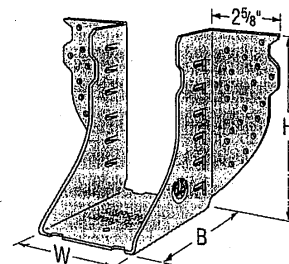
LUS28



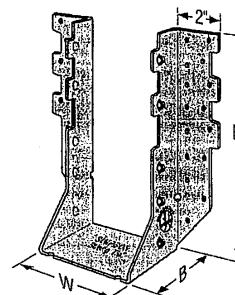
LU26L



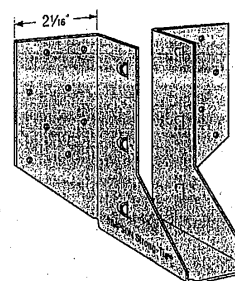
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS



TECHNICAL BULLETIN

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

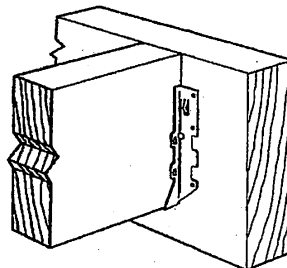
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

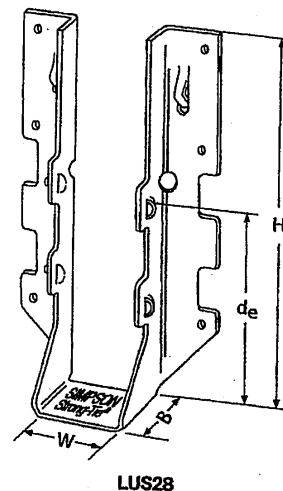
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

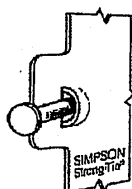


Typical LUS Installation

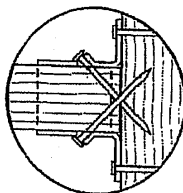


Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/16	3 1/4	1 3/4	1 15/16	(4) 10d	(2) 10d		710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 15/16	(4) 16d	(2) 16d		835	2020	590	1435
LUS26	18	1 1/16	4 1/4	1 3/4	3 5/8	(4) 10d	(4) 10d		1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3 1/4	(4) 16d	(4) 16d		1720	2595	1545	2340
LUS28	18	1 1/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d		1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d		1720	3325	1545	2375
LUS210	18	1 1/16	7 15/16	1 3/4	3 3/4	(8) 10d	(4) 10d		1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d		2580	4500	2320	3195
LUS210-3	18	4 1/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d		2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com



TECHNICAL BULLETIN

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

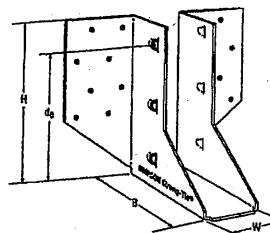
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

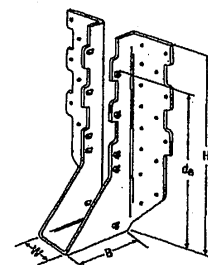
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

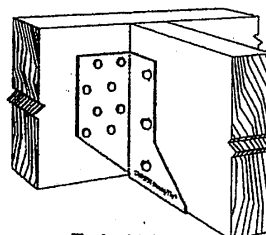
- See current catalogue for options



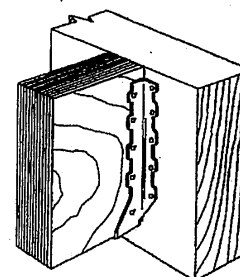
LJS26DS



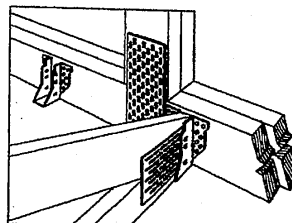
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



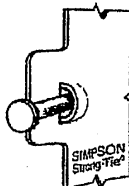
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

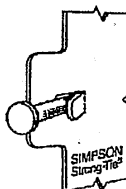
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 3/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

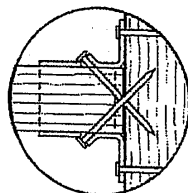


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com



TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

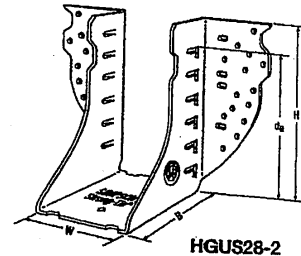
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

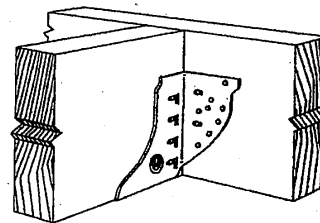
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

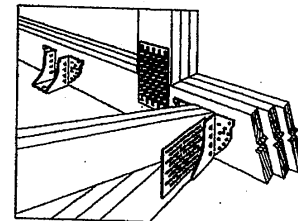
- See current catalogue for options



HGUS28-2



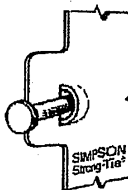
Typical HGUS
Installation



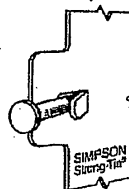
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	8625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10845
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

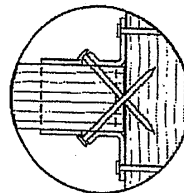
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

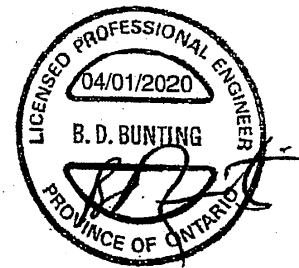


**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHGUS20 3/20 exp. 6/22



(800) 999-5099
strongtie.com

**H/TSP****Seismic and Hurricane Ties**

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

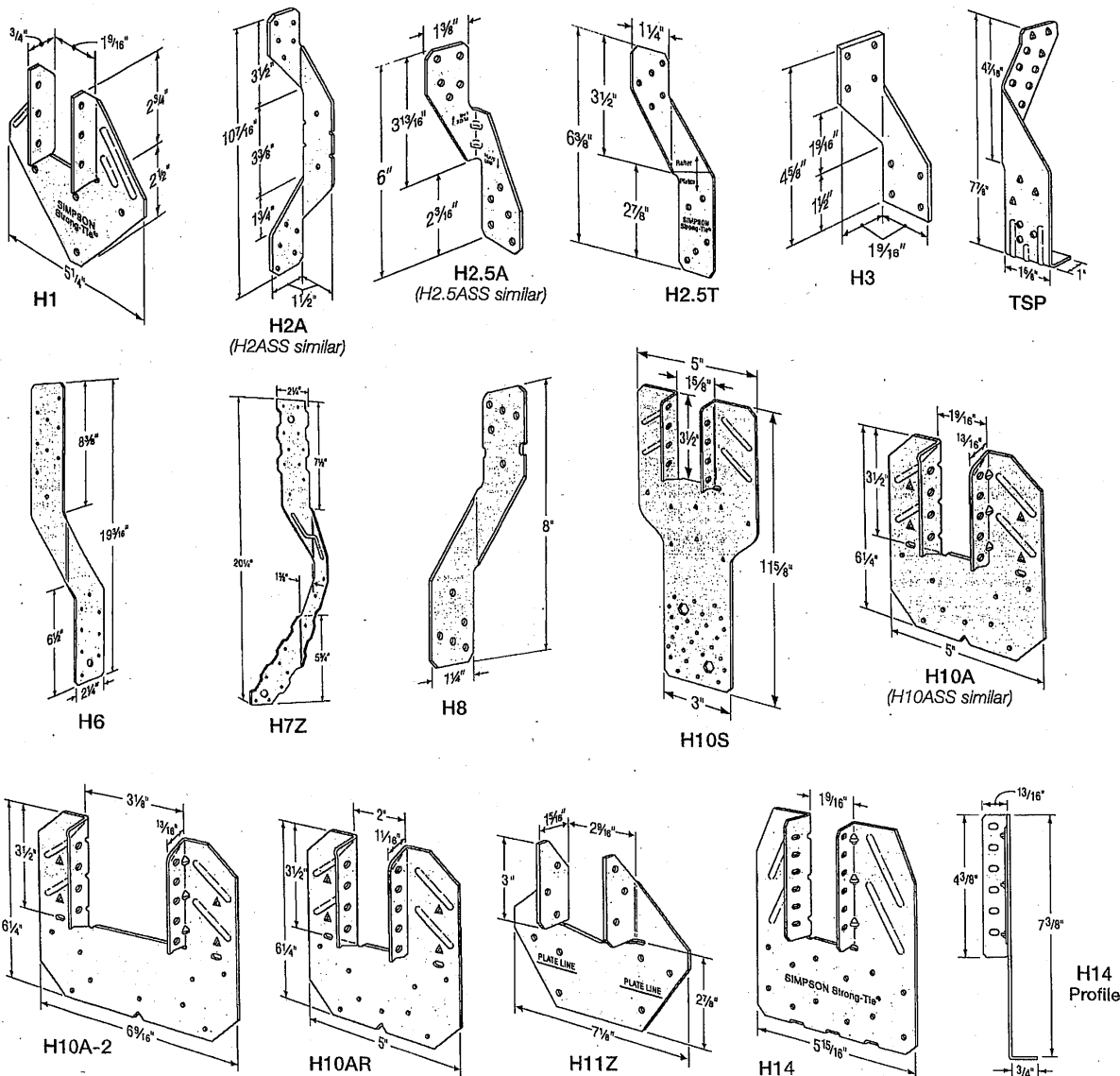
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized, H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

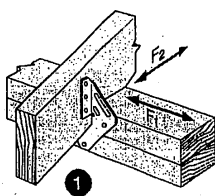
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12; use H10A sloped loads for field bent installation.



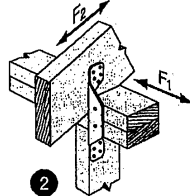


H/TSP

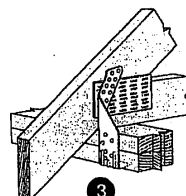
Seismic and Hurricane Ties (cont.)



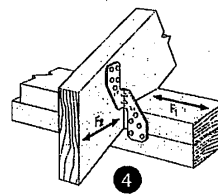
1 H1 Installation



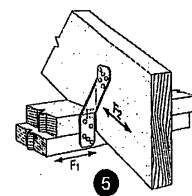
2 H2A Installation



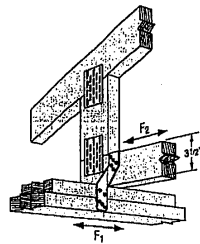
3 TSP Installation



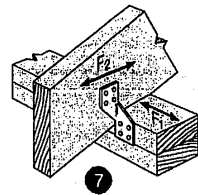
4 H2.5A Installation
(Nails into both top plates)



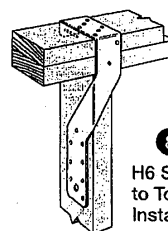
5 H2.5T Installation
(Nails into both top plates)



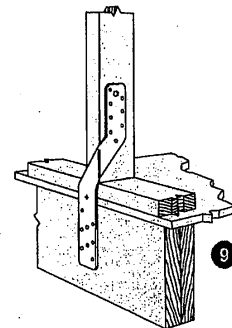
6 H2.5T Installation



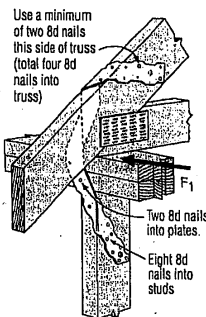
7 H3 Installation
(Nails into upper top plate)



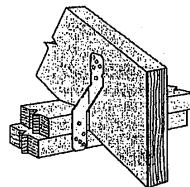
8 H6 Stud to Top Plate Installation



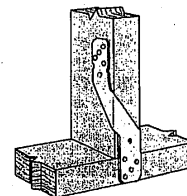
9 H6 Stud to Band Joist Installation



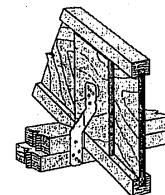
10 H7Z Installation



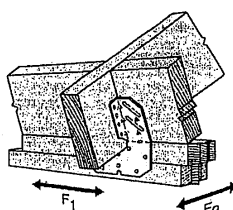
11 H8 Attaching Rafter to Double Top Plates



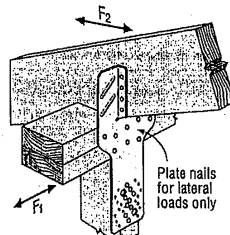
12 H8 attaching Stud to Sill
(4) 8d into plate, (5) 8d into stud



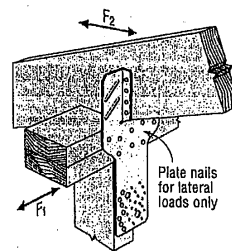
13 H8 attaching I-Joist to Double Top Plates



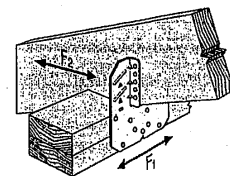
14 H10A Field-Bent Installation



15 H10S Installation

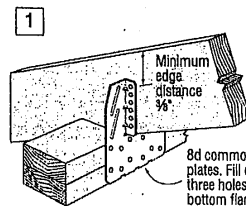


16 H10S Installation with Stud Offset

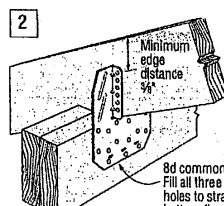


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

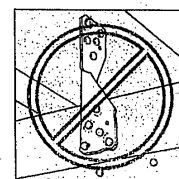


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

(800) 999-5099
strongtie.com

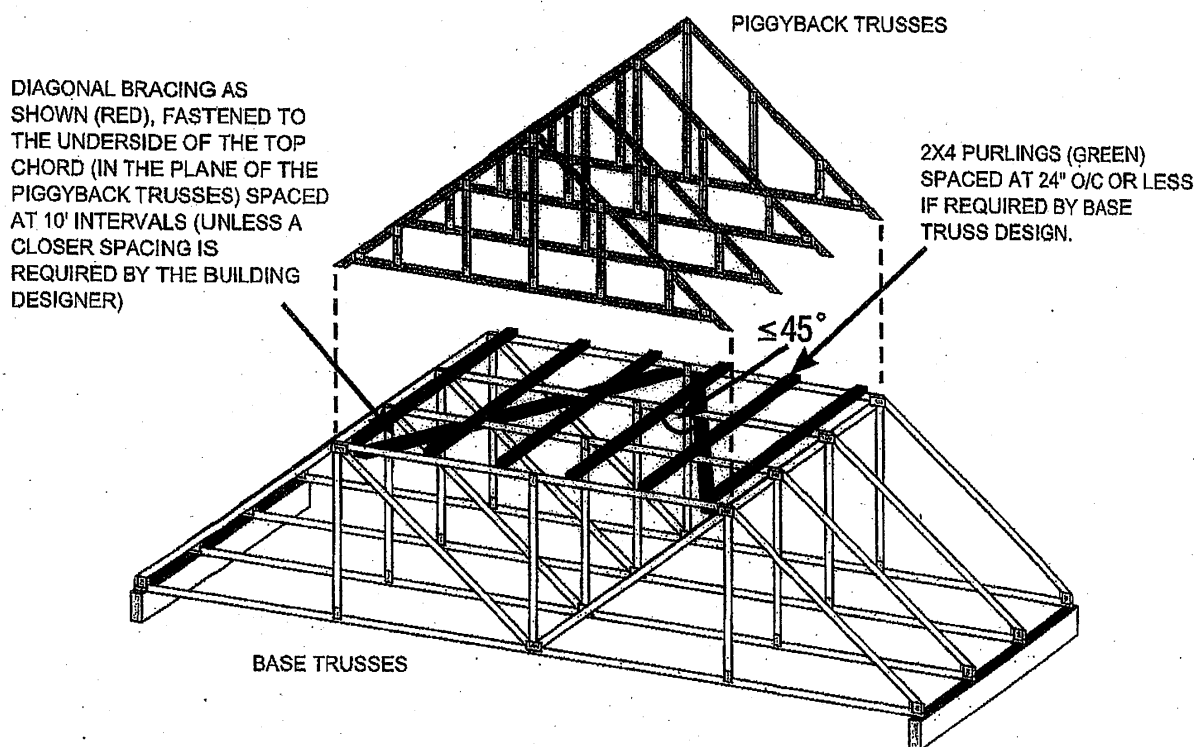


Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

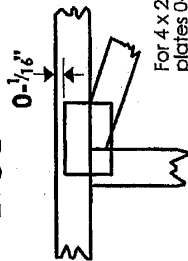
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x-y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



* Plate location details available in Mitek software or upon request.

PLATE SIZE

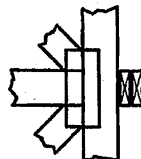
4 X 4
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, 1 or Eliminator bracing if indicated.

BEARING



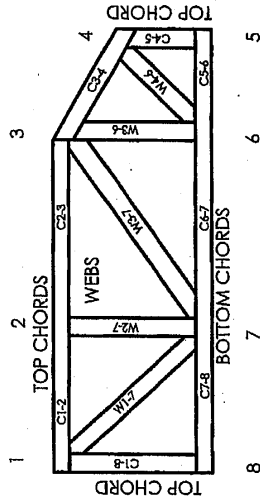
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 Mitek® All Rights Reserved

Mitek
POWER TO PERFORM.™

Mitek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing members may require bracing, or alternative 1, 1 or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.