

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with
Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses
to be min. 2x4 SPF #2 @ 24" o/c with a
vertical post to the truss at each cross
point. Vertical posts longer than 6' to have
lateral bracing so that the distance between
the post end points and lateral bracing does
not exceed 6'.

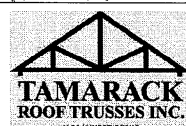
DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.86 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.47 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5A (/)

 DENOTES:
CONVENTIONAL
FRAMING

M14628



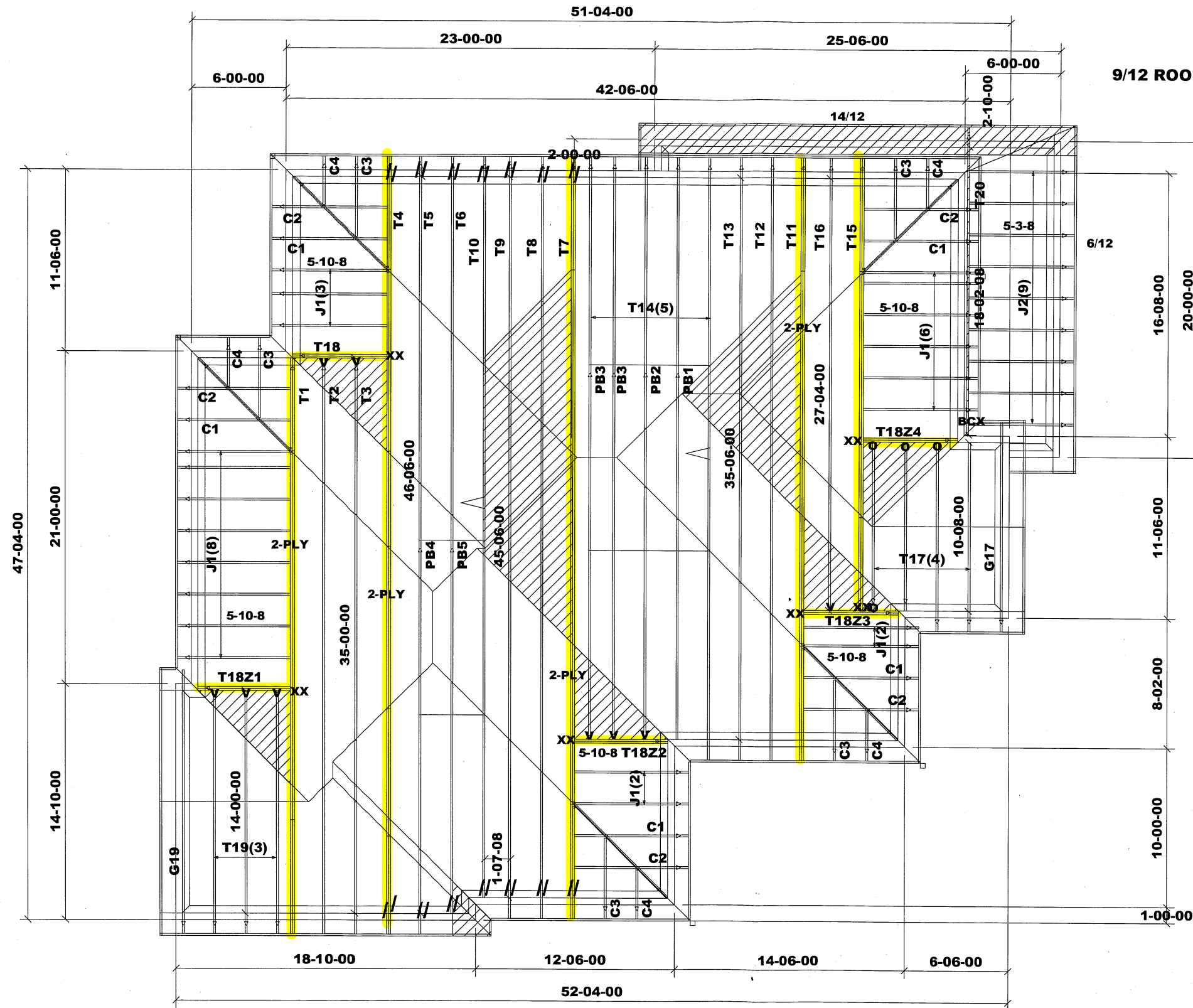
Job Track: **50465**
Plan Log: **204529**
Layout ID: **417701**

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD
Project: **GREEN VALLEY EAST**
Date: 2022-02-15 Sales: **Rick DiCiano** Designer: YPG

Model / Elevation:
S42-19C/A-LOT 48 (OPT.WITH COFF)

Mitek ver 8.4.2.286

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR
REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



9/12 ROOF PITCH UNLESS NOTED OTHERWISE

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with
Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses
to be min. 2x4 SPF #2 @ 24" o/c with a
vertical post to the truss at each cross
point. Vertical posts longer than 6' to have
lateral bracing so that the distance between
the post end points and lateral bracing does
not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5A (I)

 DENOTES:
CONVENTIONAL
FRAMING

M14628



Job Track: **50465**
Plan Log: **204529**
Layout ID: **417704**

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD
Project: **GREEN VALLEY EAST**

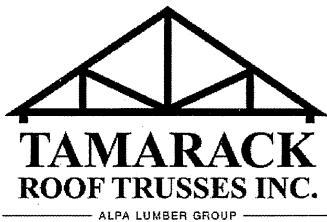
Model / Elevation:
S42-19C/A-Lot 48

Mitek ver 8.4.2.286

Date: 2022-02-15 Sales: Rick DiCiano Designer: JG

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DELIVERY SHIPLIST



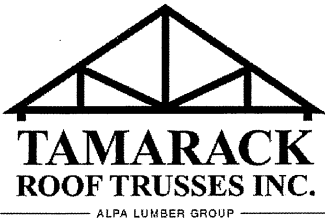
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON HOMES
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19C
 Lot #:
 Elevation: A-LOT 48 (OPT.WITH COFF)

Job Track: 50465
 PlanLog: 204529
 Layout ID: 417701
 Ref #: 12877
 Page: 1 of 3
 Date: 02-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	9 /12	35-00-00	5-11-02	2 x 4 2 x 6	1-03-08	1-06-04 1-06-04	374.94 242.67		
	1	T2S Hip	9 /12	35-00-00	7-05-02	2 x 4	1-03-08	1-06-04 1-06-04	171.91 110.00		
	1	T3S Roof Special	9 /12	35-00-00	8-11-02	2 x 4	1-03-08	1-06-04 1-06-04	188.27 121.17		
	1 2-ply	T4S Roof Special Girder	9 /12	46-06-00	10-05-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	674.21 421.33		
	1	T5S Piggyback Base	9 /12	46-06-00	10-02-00	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	342.48 212.00		
	1	T6 Piggyback Base	9 /12	46-06-00	10-02-00	2 x 6	1-03-08	1-06-04 9-04	294.56 180.67		
	1 2-ply	T7 Hip Girder	9 /12	45-06-00	5-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	562.02 337.33		
	1	T8 Hip	9 /12	45-06-00	7-05-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	266.95 158.17		
	1	T9 Hip	9 /12	45-06-00	8-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	285.65 174.83		
	1	T10 Hip	9 /12	45-06-00	10-02-00	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	282.44 171.00		
	1 2-ply	T11 Hip Girder	9 /12	35-06-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	370.96 232.67		
	1	T12 Hip	9 /12	35-06-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	166.22 104.83		
	1	T13 Hip	9 /12	35-06-00	8-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	172.16 108.67		
	5	T14 Hip	9 /12	35-06-00	10-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	885.23 548.33		

DELIVERY SHIPLIST



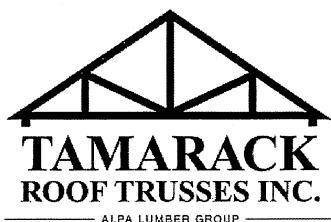
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON HOMES
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19C
 Lot #:
 Elevation: A-LOT 48 (OPT.WITH COFF)

Job Track: 50465
 PlanLog: 204529
 Layout ID: 417701
 Ref #: 12877
 Page: 2 of 3
 Date: 02-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T15 Hip Girder	9 /12	27-04-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	276.46 176.00		
	1	T16 Hip	9 /12	27-04-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	122.05 76.83		
	4	T17 Common	9 /12	10-08-00	5-06-04	2 x 4		1-06-04 1-06-04	170.13 111.33		
	1	G17 GABLE	9 /12	10-08-00	5-06-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	49.09 32.50		
	1 2-ply	T18 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z1 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z2 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z3 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z4 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	3	T19S Roof Special	9 /12	14-00-00	6-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	198.19 137.00		
	1	G19 GABLE	9 /12	14-00-00	6-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	65.2 43.17		
	1	T20 Half Hip Girder	14 /12	18-02-08	3-07-14	2 x 6	1-03-08	1-03-08 2-11-15	105.3 66.67		
	1	PB1 Piggyback	9 /12	11-09-00	1-06-00	2 x 4			34.2 22.17		
	1	PB2 Piggyback	9 /12	11-09-00	3-00-00	2 x 4			34.5 22.17		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON HOMES
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19C
 Lot #:
 Elevation: A-LOT 48 (OPT.WITH COFF)

Job Track: 50465
 PlanLog: 204529
 Layout ID: 417701
 Ref #: 12877
 Page: 3 of 3
 Date: 02-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB3 Piggyback	9 /12	11-09-00	4-04-14	2 x 4			62.99 40.00		
	1	PB4 Piggyback	9 /12	11-00-00	1-09-06	2 x 4			32.36 22.33		
	1	PB5 Piggyback	9 /12	11-00-00	1-06-00	2 x 4			31.9 20.83		
	21	J1 Jack-Open	9 /12	5-10-08	5-11-02	2 x 4	1-03-08	1-06-04 5-11-02	386.38 252.00		
	9	J2 Jack-Open	6 /12	5-03-08	3-07-07	2 x 4	1-03-08	4-03 2-11-15	131.34 84.00		
	5	C1 Jack-Open	9 /12	3-09-07	4-04-05	2 x 4	1-03-08 2-01-01	1-06-04 4-04-05	84.19 55.00		
	5	C2 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 4-01-01	1-06-04 2-10-05	69.37 48.33		
	5	C3 Jack-Open	9 /12	1-10-08	4-04-05	2 x 4	1-03-08 1-10-15	1-06-04 2-11-02	61.11 41.67		
	5	C4 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	46.56 35.00		

TOTAL # TRUSS= 101

TOTAL BFT OF ALL TRUSSES= 4625.67

BFT.

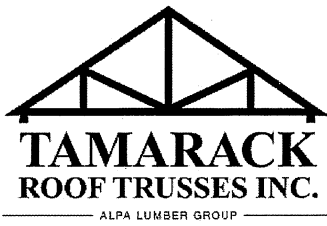
TOTAL WEIGHT OF ALL TRSSES 7329.71 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
28	Hardware	H2.5A	
6	Hardware	HGUS26-2	
9	Hardware	LJS26DS	
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 47

DELIVERY SHIPLIST



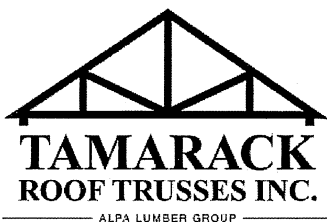
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON HOMES
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19C
 Lot #:
 Elevation: A-LOT 48

Job Track: 50465
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 Layout ID: 417704
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 Page: 1 of 3
 Date: 02-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9 /12	35-00-00	5-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	421.62 254.67		
	1	T2 Hip	9 /12	35-00-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	156.74 99.00		
	1	T3 Roof Special	9 /12	35-00-00	8-11-02	2 x 4	1-03-08	1-06-04 1-06-04	173.74 110.33		
	1 2-ply	T4 Roof Special Girder	9 /12	46-06-00	10-05-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	646.25 398.67		
	1	T5 Piggyback Base	9 /12	46-06-00	10-02-00	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	340.88 206.67		
	1	T6 Piggyback Base	9 /12	46-06-00	10-02-00	2 x 6	1-03-08	1-06-04 9-04	326.25 198.67		
	1 2-ply	T7 Hip Girder	9 /12	45-06-00	5-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	562.02 337.33		
	1	T8 Hip	9 /12	45-06-00	7-05-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	296.73 174.67		
	1	T9 Hip	9 /12	45-06-00	8-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	308.35 188.00		
	1	T10 Hip	9 /12	45-06-00	10-02-00	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	305.76 184.00		
	1 2-ply	T11 Hip Girder	9 /12	35-06-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	378.14 236.67		
	1	T12 Hip	9 /12	35-06-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	169.6 106.83		
	1	T13 Hip	9 /12	35-06-00	8-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	175.18 110.33		
	5	T14 Hip	9 /12	35-06-00	10-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	950.91 583.33		

DELIVERY SHIPLIST



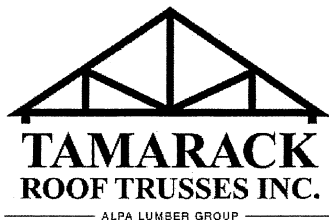
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON HOMES
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19C
 Lot #:
 Elevation: A-LOT 48

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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T15 Hip Girder	9 /12	27-04-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	283.41 178.00		
	1	T16 Hip	9 /12	27-04-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	125.5 78.83		
	1	G17 GABLE	9 /12	10-08-00	5-06-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	49.09 32.50		
	4	T17 Common	9 /12	10-08-00	5-06-04	2 x 4		1-06-04 1-06-04	170.13 111.33		
	1 2-ply	T18 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1	G19 GABLE	9 /12	14-00-00	6-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	65.2 43.17		
	1 2-ply	T18Z1 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z2 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z3 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z4 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	3	T19 Common	9 /12	14-00-00	6-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	189.71 120.50		
	1	T20 Half Hip Girder	14 /12	18-02-08	3-07-14	2 x 6	1-03-08	1-03-08 2-11-15	105.3 66.67		
	1	PB1 Piggyback	9 /12	11-09-00	1-06-00	2 x 4			34.2 22.17		
	1	PB2 Piggyback	9 /12	11-09-00	3-00-00	2 x 4			34.5 22.17		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON HOMES
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19C
 Lot #:
 Elevation: A-LOT 48

Job Track: 50465
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 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB3 Piggyback	9 /12	11-09-00	4-04-14	2 x 4			62.99 40.00		
	1	PB4 Piggyback	9 /12	11-00-00	1-09-06	2 x 4			32.36 22.33		
	1	PB5 Piggyback	9 /12	11-00-00	1-06-00	2 x 4			31.9 20.83		
	21	J1 Jack-Open	9 /12	5-10-08	5-11-02	2 x 4	1-03-08	1-06-04 5-11-02	526.76 346.50		
	9	J2 Jack-Open	6 /12	5-03-08	3-07-07	2 x 4	1-03-08	4-03 2-11-15	131.34 84.00		
	5	C1 Jack-Open	9 /12	3-09-07	4-04-05	2 x 4	1-03-08 2-01-01	1-06-04 4-04-05	84.19 55.00		
	5	C2 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 4-01-01	1-06-04 2-10-05	69.37 48.33		
	5	C3 Jack-Open	9 /12	1-10-08	4-04-05	2 x 4	1-03-08 1-10-15	1-06-04 2-11-02	61.11 41.67		
	5	C4 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	46.56 35.00		

TOTAL # TRUSS= 101

TOTAL BFT OF ALL TRUSSES= 4773.17

BFT.

TOTAL WEIGHT OF ALL TRSSES 7646.17 LBS

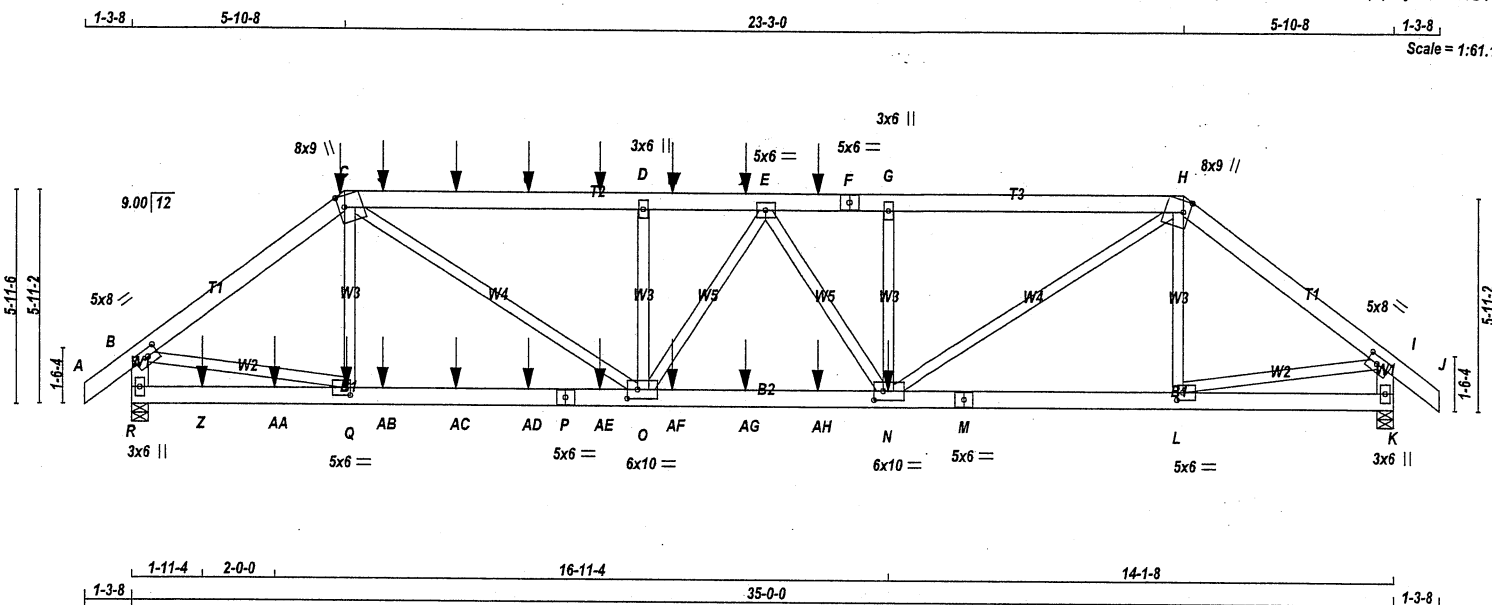
HARDWARE

QTY	TYPE	MODEL	LENGTH
28	Hardware	H2.5A	
6	Hardware	HGUS26-2	
9	Hardware	LJS26DS	
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 47

JOB NAME 417704	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Mon Feb 7 17:21:15 2022 Page 1
ID:JvMhHdWEgsSI57rNnlib4czW6nI-BPKuICkhGcGhKf9eta1fd8W9wmPtpwyI7n6znTsY



TOTAL WEIGHT = 2 X 211 = 422 lb [M]

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
M - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-J	2	12
R-B	2	12
K-I	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-P	2	12
P-M	2	12
M-K	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6
G-N	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	4978	0	5-8	5-8
K	4427	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
R	3484	2492 / 0	0 / 0	0 / 0	992 / 0	0 / 0
K	3094	2237 / 0	0 / 0	0 / 0	856 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 48	-112.4 -112.4	0.05 (1)	10.00	Q-C	0 / 210
B-C	-6080 / 0	-112.4 -112.4	0.26 (1)	4.60	C-O	0 / 4280
C-S	-8429 / 0	-112.4 -112.4	0.47 (1)	3.82	O-D	-1008 / 0
S-T	-8429 / 0	-112.4 -112.4	0.47 (1)	3.82	N-G	-914 / 0
T-U	-8429 / 0	-112.4 -112.4	0.47 (1)	3.82	N-H	0 / 5425
U-V	-8429 / 0	-112.4 -112.4	0.47 (1)	3.82	L-H	-552 / 0
V-D	-8429 / 0	-112.4 -112.4	0.47 (1)	3.82	L-I	0 / 4286
D-W	-8429 / 0	-112.4 -112.4	0.36 (1)	3.86	B-Q	0 / 4920
W-X	-8429 / 0	-112.4 -112.4	0.36 (1)	3.86	O-E	-274 / 0
X-E	-8429 / 0	-112.4 -112.4	0.36 (1)	3.86	E-N	0 / 320
E-Y	-8741 / 0	-112.4 -112.4	0.34 (1)	3.84		
Y-F	-8741 / 0	-112.4 -112.4	0.34 (1)	3.84		
F-G	-8741 / 0	-112.4 -112.4	0.34 (1)	3.84		
G-H	-8741 / 0	-112.4 -112.4	0.46 (1)	3.78		
H-I	-5296 / 0	-112.4 -112.4	0.24 (1)	4.87		
I-J	0 / 48	-112.4 -112.4	0.05 (1)	10.00		
R-B	-4965 / 0	0.0	0.0	1.18 (1)	6.52	
K-I	-4388 / 0	0.0	0.0	0.16 (1)	6.85	

R-Z	0 / 0	-18.5	-18.5	0.10 (1)	10.00
Z-AA	0 / 0	-18.5	-18.5	0.10 (1)	10.00
AA-Q	0 / 0	-18.5	-18.5	0.10 (1)	10.00
Q-AB	0 / 4856	-18.5	-18.5	0.26 (1)	10.00
AB-AC	0 / 4856	-18.5	-18.5	0.26 (1)	10.00
AC-AD	0 / 4856	-18.5	-18.5	0.26 (1)	10.00
AD-P	0 / 4856	-18.5	-18.5	0.26 (1)	10.00
P-AE	0 / 4856	-18.5	-18.5	0.26 (1)	10.00
AE-O	0 / 4856	-18.5	-18.5	0.26 (1)	10.00
O-AF	0 / 8573	-18.5	-18.5	0.36 (1)	10.00
AF-AG	0 / 8573	-18.5	-18.5	0.36 (1)	10.00
AG-AH	0 / 8573	-18.5	-18.5	0.36 (1)	10.00
AH-N	0 / 8573	-18.5	-18.5	0.36 (1)	10.00
N-M	0 / 4212	-18.5	-18.5	0.13 (1)	10.00
M-L	0 / 4212	-18.5	-18.5	0.13 (1)	10.00
L-K	0 / 0	-18.5	-18.5	0.03 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-269	-1403	-	FRONT	VERT	TOTAL	-	C1
N	20-10-8	-1403	-1403	-	FRONT	VERT	TOTAL	-	C1
Q	5-11-4	-164	-164	-	FRONT	VERT	TOTAL	-	C1
S	6-11-4	-15	-15	-	FRONT	VERT	TOTAL	-	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.47/1.00 (C-D:1), BC=0.36/1.00 (N-O:1), WB=0.48/1.00 (H-N:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	
	MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (L) (INPUT = 0.90)
JSI METAL= 0.57 (I) (INPUT = 1.00)



Structural component only
DWG# T-2204180 in

CONTINUED ON PAGE 2

JOB NAME 417704	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:15 2022 Page 2
ID:JvMhHdWEgsSI57rNnlib4czW6nl-BPKuICkIhGcGhqKF9eta1fd8W9wmPptpwyi7n6znTsY

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.50
C	TTWW+m	MT20	8.0	9.0	3.75	2.00
D	TMW+w	MT20	3.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.75	2.00
I	TMVW-t	MT20	5.0	8.0	2.50	3.50
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	6.0	10.0	3.00	3.00
O	BMWWW-t	MT20	6.0	10.0	3.00	3.50
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	8-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
U	10-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
V	12-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
W	14-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
X	16-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
Y	18-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
Z	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AA	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	6-11-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1
AC	8-11-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1
AD	10-11-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1
AE	12-11-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1
AF	14-11-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1
AG	16-11-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1
AH	18-11-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2204180

CONTINUED ON PAGE 2

JOB NAME 417701	TRUSS NAME T1S	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:35:08 2022 Page 2
ID:JvMhHdWEgsSI57rNnlib4czW6nl-BN30XWK6Ri 5YUhir267tq8etqJoN0?ImNr8qPznGTn

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-p	MT20	5.0	8.0	Edge	
B	TTWW+m	MT20	6.0	10.0	Edge	2.00
C	TMWW-t	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMWV+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.50
I	TMWV+p	MT20	4.0	4.0	1.00	2.00
J	TMWV-t	MT20	5.0	6.0	2.25	2.75
L	BMWV1+p	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BMWW-t	MT20	6.0	12.0	3.75	8.00
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	3.00	2.25
Q	BMWWWW-t	MT20	8.0	12.0	5.00	4.50
R	NP+sw	MT20	3.0	6.0		
S	BMW+sw	MT20	3.0	6.0		
T	BMWWWW-t	MT20	6.0	12.0	3.00	4.25
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMWW-t	MT20	5.0	6.0	2.50	2.75
X	BMV1+p	MT20	3.0	6.0		
Y	NP+sw	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-10-8	-347	-347	---	FRONT	VERT	TOTAL	---	C1
C	10-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
S	20-10-8	-1388	-1388	---	FRONT	VERT	TOTAL	---	C1
U	12-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
V	10-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
W	5-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
Z	6-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AA	8-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AB	12-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AC	14-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AD	16-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AE	18-11-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AF	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AG	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AH	6-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
AI	8-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
AJ	14-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
AK	16-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
AL	18-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

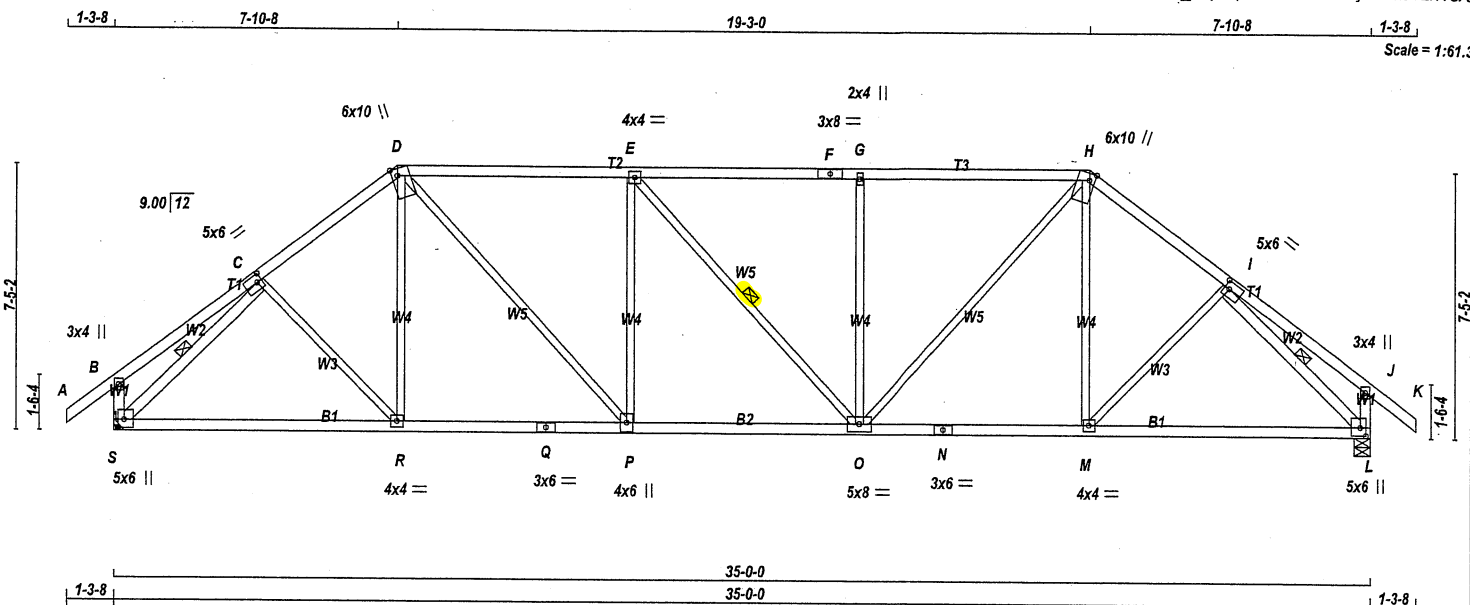
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2204204 *me*

JOB NAME 417704	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:16 2022 Page 1
ID:JvMhHdWEgSSi57rNnlib4czW6nH-fbuGVYlwSak7L_vSlOpZs9IRZDY8BZy9cRhJYznTsX



TOTAL WEIGHT = 157 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+t	MT20	5.0	6.0	2.50 1.75
D	TTVW+m	MT20	6.0	10.0	Edge 2.00
E	TMVW-t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	8.0	
G	TMV+w	MT20	2.0	4.0	
H	TTVW+m	MT20	6.0	10.0	Edge 2.00
I	TMVW-t	MT20	5.0	6.0	2.50 1.75
J	TMV+p	MT20	3.0	4.0	
L	BMVW1+p	MT20	5.0	6.0	2.75 2.00
M	BMVW-t	MT20	4.0	4.0	
N	BS-t	MT20	3.0	6.0	
O	BMVW+t	MT20	5.0	8.0	
P	BMVW-t	MT20	4.0	6.0	
Q	BS-t	MT20	3.0	6.0	
R	BMVW-t	MT20	4.0	4.0	
S	BMVW1+p	MT20	5.0	6.0	2.75 2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT	REQRD
GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	DOWN	HORZ	IN-SX	IN-SX
S	2446	0	0	MECHANICAL	
L	2446	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT S. MINIMUM BEARING LENGTH AT JOINT S = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1712	1226 / 0	0 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0
L	1712	1226 / 0	0 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 47	-112.4	-112.4 0.15 (1)	10.00	C-R	0 / 65	0.02 (4)
B-C	0 / 30	-112.4	-112.4 0.27 (1)	10.00	R-D	0 / 129	0.04 (4)
C-D	-2508 / 0	-112.4	-112.4 0.38 (1)	4.00	D-P	0 / 1200	0.27 (1)
D-E	-2786 / 0	-112.4	-112.4 0.52 (1)	4.47	P-E	-780 / 0	0.77 (1)
E-F	-2785 / 0	-112.4	-112.4 0.52 (1)	4.47	E-O	-2 / 0	0.00 (1)
F-G	-2785 / 0	-112.4	-112.4 0.52 (1)	4.47	O-G	-780 / 0	0.77 (1)
G-H	-2785 / 0	-112.4	-112.4 0.52 (1)	4.47	O-H	0 / 1198	0.27 (1)
H-I	-2508 / 0	-112.4	-112.4 0.38 (1)	4.00	M-H	0 / 130	0.04 (4)
I-J	0 / 30	-112.4	-112.4 0.27 (1)	10.00	M-I	0 / 65	0.02 (4)
J-K	0 / 47	-112.4	-112.4 0.15 (1)	10.00	S-C	-2839 / 0	0.48 (1)
S-B	-326 / 0	0.0	0.0 0.03 (1)	7.81	I-L	-2839 / 0	0.48 (1)
L-J	-326 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 1953	-18.5	-18.5 0.46 (1)	10.00			
R-Q	0 / 1981	-18.5	-18.5 0.46 (1)	10.00			
Q-P	0 / 1981	-18.5	-18.5 0.46 (1)	10.00			
P-O	0 / 2787	-18.5	-18.5 0.52 (1)	10.00			
O-N	0 / 1981	-18.5	-18.5 0.46 (1)	10.00			
N-M	0 / 1981	-18.5	-18.5 0.46 (1)	10.00			
M-L	0 / 1953	-18.5	-18.5 0.46 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.52/1.00 (D-E:1), BC=0.52/1.00 (O-P:1), WB=0.77/1.00 (E-P:1), SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (H) (INPUT = 0.90)
JSI METAL= 0.69 (L) (INPUT = 1.00)



Structural component only
DWG# T-2204181

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
417701	T2S	1	1	TRUSS DESC.	HOMES	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
W	BMVW1-t	MT20	5.0	6.0	2.50 2.75

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

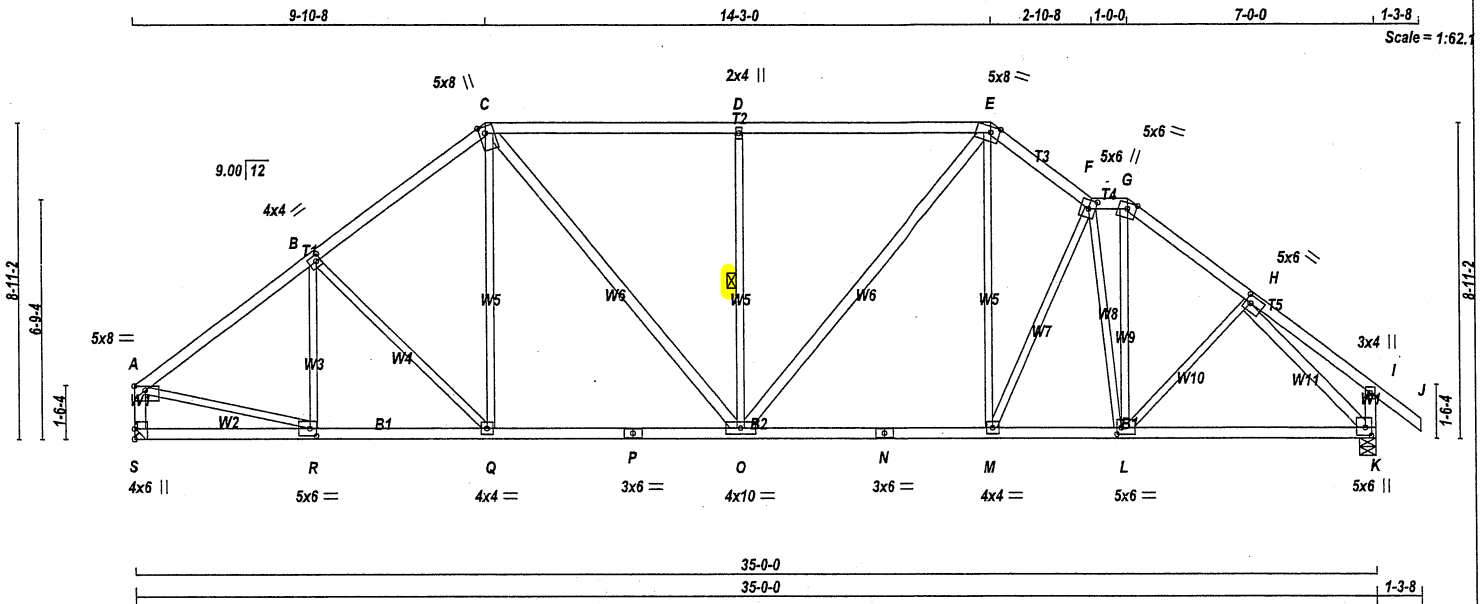
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2204205 *W*

JOB NAME 417704	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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TOTAL WEIGHT = 174 lb
[M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	2100F 1.8E	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
S - A	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
S - P	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
C - O	2x4	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	1.50	Edge
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	8.0	Edge	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TTWW+m	MT20	5.0	6.0	3.00	2.25
G	TTWW-m	MT20	5.0	6.0	Edge	
H	TMVW-t	MT20	5.0	6.0	2.50	1.75
I	TMVW-p	MT20	3.0	4.0		
K	BMVW1+p	MT20	5.0	6.0	2.75	2.00
L	BMVWV-t	MT20	5.0	6.0	2.25	1.50
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMVWV-t	MT20	4.0	10.0		
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
S	2291	0	0	MECHANICAL
K	2446	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT S. MINIMUM BEARING LENGTH AT JOINT S = 3'-8".

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1606	1137 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0
K	1712	1226 / 0	0 / 0	0 / 0	0 / 0	496 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-2534 / 0	-112.4 -112.4	0.46 (1)	3.92	R-B	-396 / 0	0.17 (1)
B-C	-2406 / 0	-112.4 -112.4	0.44 (1)	4.02	B-Q	-240 / 0	0.22 (1)
C-D	-2410 / 0	-112.4 -112.4	0.53 (1)	4.69	Q-C	0 / 284	0.06 (1)
D-E	-2410 / 0	-112.4 -112.4	0.53 (1)	4.69	C-O	0 / 813	0.13 (1)
E-F	-2497 / 0	-112.4 -112.4	0.19 (1)	4.22	O-D	-984 / 0	0.51 (1)
F-G	-2017 / 0	-112.4 -112.4	0.06 (1)	4.74	O-E	0 / 647	0.10 (1)
G-H	-2526 / 0	-112.4 -112.4	0.23 (1)	4.16	M-E	0 / 540	0.12 (1)
H-I	0 / 24	-112.4 -112.4	0.19 (1)	10.00	L-G	0 / 1255	0.28 (1)
I-J	0 / 47	-112.4 -112.4	0.15 (1)	10.00	L-H	0 / 137	0.03 (1)
S-A	-2249 / 0	0.0 0.0	0.24 (1)	5.68	A-R	0 / 2115	0.48 (1)
K-I	-311 / 0	0.0 0.0	0.03 (1)	7.81	H-K	-2818 / 0	0.86 (1)
					F-L	-1248 / 0	0.98 (1)
					M-F	-498 / 0	0.49 (1)
S-R	0 / 0	-18.5 -18.5	0.10 (4)	10.00			
R-Q	0 / 2058	-18.5 -18.5	0.41 (1)	10.00			
Q-P	0 / 1892	-18.5 -18.5	0.42 (1)	10.00			
P-O	0 / 1892	-18.5 -18.5	0.42 (1)	10.00			
O-N	0 / 1998	-18.5 -18.5	0.44 (1)	10.00			
N-M	0 / 1998	-18.5 -18.5	0.44 (1)	10.00			
M-L	0 / 2197	-18.5 -18.5	0.44 (1)	10.00			
L-K	0 / 1902	-18.5 -18.5	0.42 (1)	10.00			

NOTES-(1)

Lateral braces to be a minimum of 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 45.9	PSF	

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL)= L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL)= L/999 (0.23")

CSI: TC=0.53/1.00 (D-E:1), BC=0.44/1.00 (L-M:1), WB=0.98/1.00 (F-L:1), SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

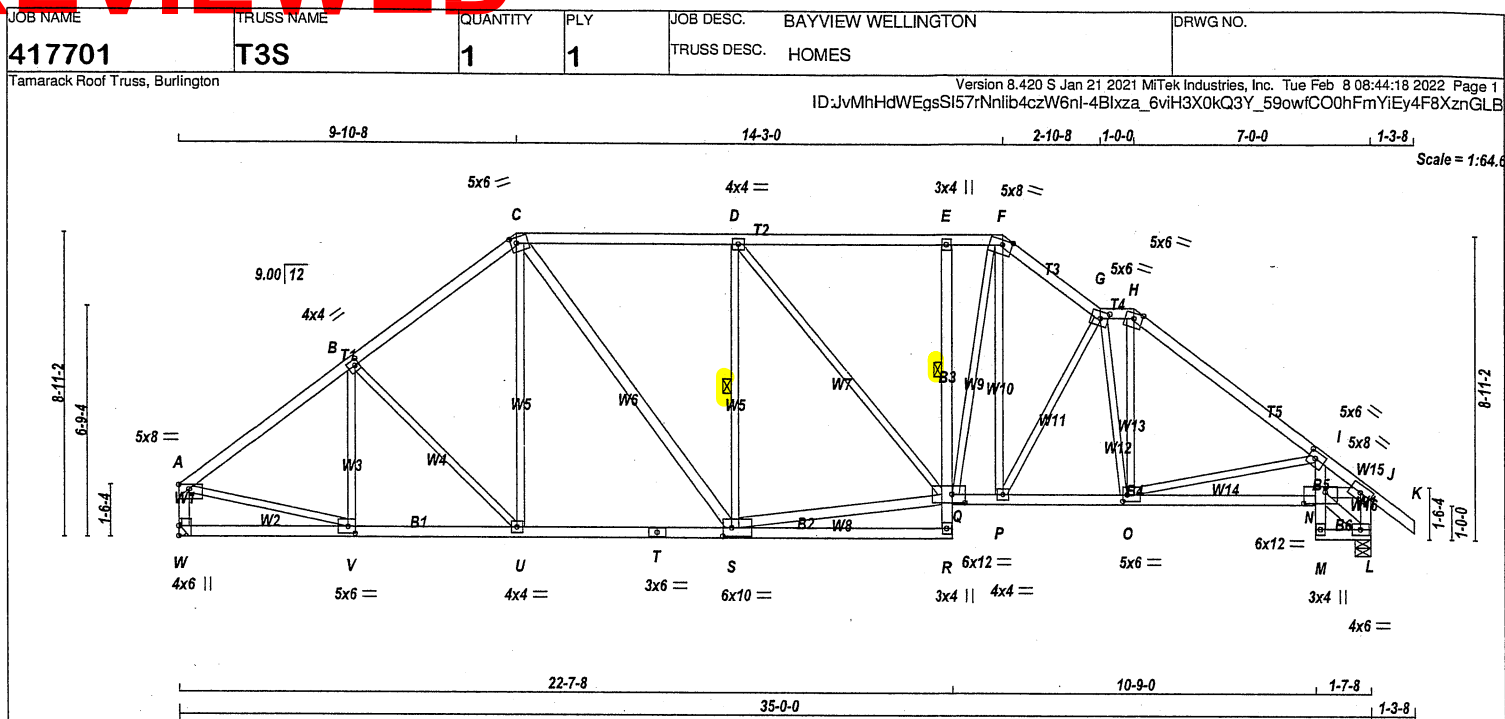
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (A) (INPUT = 0.90)
JSI METAL = 0.70 (N) (INPUT = 1.00)



Structural component only
DWG# T-2204182



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
W - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	1.50	Edge
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTVW-m	MT20	5.0	6.0	2.00	2.00
D	TMVW-t	MT20	4.0	4.0		
E	TMV-p	MT20	3.0	4.0		
F	TTVW-m	MT20	5.0	8.0	Edge	3.50
G	TTVW-m	MT20	5.0	6.0	2.50	2.75
H	TTVW-m	MT20	5.0	6.0	Edge	
I	TMVW-t	MT20	5.0	6.0	2.50	2.50
J	TMVW-t	MT20	5.0	8.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMV-p	MT20	3.0	4.0		
N	BMVWV-I	MT20	6.0	12.0	4.00	7.75
O	BMVWV-t	MT20	5.0	6.0	2.25	1.50
P	BMVW-t	MT20	4.0	4.0		
Q	BMVWVW-I	MT20	6.0	12.0	3.00	4.75
R	BMV-p	MT20	3.0	4.0		
S	BMVWVW-t	MT20	6.0	10.0	3.00	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
W	2288	0	2288	0
L	2449	0	2449	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT W. MINIMUM BEARING LENGTH AT JOINT W = 3-8.

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
W	1804	1135 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0
L	1714	1228 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, D-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	-2531 / 0	-112.4	-112.4	0.46 (1)	3.92	V-B	-392 / 0	0.16 (1)
B-C	-2399 / 0	-112.4	-112.4	0.44 (1)	4.02	B-U	-244 / 0	0.23 (1)
C-D	-2364 / 0	-112.4	-112.4	0.64 (1)	3.72	U-C	0 / 274	0.06 (1)
D-E	-2426 / 0	-112.4	-112.4	0.63 (1)	3.71	C-S	0 / 810	0.13 (1)
E-F	-2431 / 0	-112.4	-112.4	0.28 (1)	4.15	S-D	-890 / 0	0.46 (1)
F-G	-2795 / 0	-112.4	-112.4	0.21 (1)	4.00	S-Q	0 / 2356	0.38 (1)
G-H	-2393 / 0	-112.4	-112.4	0.07 (1)	4.41	D-Q	0 / 83	0.02 (1)
H-I	-2954 / 0	-112.4	-112.4	0.62 (1)	3.49	Q-F	0 / 885	0.20 (1)
I-J	-3572 / 0	-112.4	-112.4	0.40 (1)	3.36	P-F	0 / 659	0.15 (1)
J-K	0 / 47	-112.4	-112.4	0.15 (1)	10.00	P-G	-712 / 0	0.49 (1)
W-A	-2247 / 0	0.0	0.0	0.23 (1)	5.68	O-H	0 / 1441	0.32 (1)
L-J	-2357 / 0	0.0	0.0	0.25 (1)	5.56	O-I	-688 / 0	0.40 (1)
W-V	0 / 0	-18.5	-18.5	0.10 (4)	10.00	A-V	0 / 2112	0.48 (1)
V-U	0 / 2056	-18.5	-18.5	0.39 (1)	10.00	N-L	-127 / 0	0.01 (1)
U-T	0 / 1887	-18.5	-18.5	0.41 (1)	10.00	N-J	0 / 2906	0.47 (1)
T-S	0 / 1887	-18.5	-18.5	0.41 (1)	10.00	G-O	-1283 / 0	0.67 (1)
S-R	0 / 47	-18.5	-18.5	0.19 (4)	10.00			
R-Q	0 / 52	0.0	0.0	0.07 (1)	10.00			
Q-E	-499 / 0	0.0	0.0	0.08 (1)	8.25			
Q-P	0 / 2245	-18.5	-18.5	0.37 (1)	10.00			
P-O	0 / 2577	-18.5	-18.5	0.48 (1)	10.00			
O-N	0 / 3010	-18.5	-18.5	0.56 (1)	10.00			
M-N	0 / 15	0.0	0.0	0.16 (1)	10.00			
N-I	0 / 58	0.0	0.0	0.16 (1)	10.00			
M-L	0 / 100	-18.5	-18.5	0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.64/1.00 (C-D:1), BC=0.56/1.00 (N-O:1), WB=0.67/1.00 (G-O:1), SSI=0.34/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.66 (J) (INPUT = 1.00)



Structural component only
DWG# T-2204206

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
417701	T3S	1	1	TRUSS DESC.	HOMES	

Tamarack Roof Truss, Burlington

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ID:JvMhHdWEgsSI57rNnlib4czW6nl-4Blxza 6viH3X0kQ3Y 59owfCO0hFmYiEy4F8XznGLB

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	4.0	4.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1-t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2204206 *mm*

JOB NAME 417704	TRUSS NAME T4	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:JvMhHdWEgsSI57rNnlib4czW6nl-3AaO7anplV6h9Sd1OTxWBVnqtmHBLA6PrZgLwtznTstU

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	10.0		
C	TTWW+m	MT20	8.0	9.0	3.75	2.00
D	TMVW+t	MT20	4.0	6.0		
E	TTWW+m	MT20	7.0	8.0		
F	TTWW-m	MT20	7.0	8.0	3.25	2.50
G	TMVW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	10.0	4.00	1.75
I	TTWW+m	MT20	6.0	10.0		
J	TTW+m	MT20	5.0	8.0	Edge	
K	TMVW-t	MT20	5.0	8.0	2.50	3.50
M	BMV1+p	MT20	3.0	6.0		
N	BMVWW-t	MT20	6.0	10.0		
O	BMVWW+t	MT20	4.0	6.0		
P	BMVWW+t	MT20	8.0	9.0	2.50	2.50
Q	BMVWW-t	MT20	7.0	8.0		
R	BMVWW+t	MT20	4.0	6.0		
S	BMVWW+t	MT20	7.0	8.0		
T	BMVWW-t	MT20	5.0	8.0	2.50	2.25
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-226	-426	---	FRONT	VERT	TOTAL	---	C1
S	11-4-8	-1880	-1880	---	FRONT	VERT	TOTAL	---	C1
T	5-11-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1
V	7-5-4	-2	-80	---	FRONT	VERT	TOTAL	---	C1
W	9-5-4	-2	-80	---	FRONT	VERT	TOTAL	---	C1
X	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Y	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Z	7-5-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1
AA	9-5-4	-164	-164	---	FRONT	VERT	TOTAL	---	C1

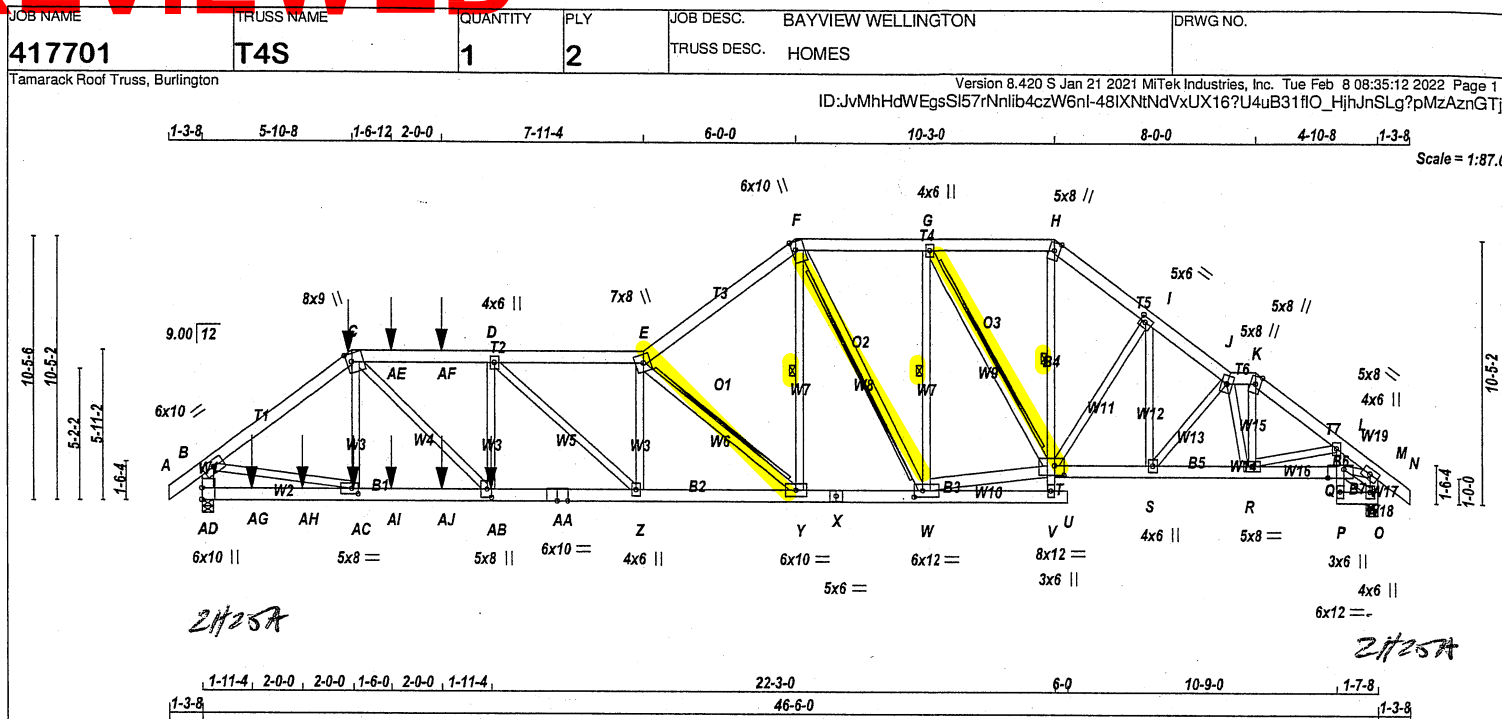
CONNECTION REQUIREMENTS
1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

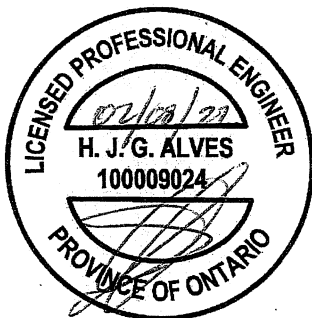
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM); INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2204183 *mr*



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS			DESIGN CRITERIA		
A - C 2x6 DRY No.2 SPF C - E 2x6 DRY No.2 SPF E - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF J - K 2x6 DRY No.2 SPF K - N 2x6 DRY No.2 SPF AD - B 2x6 DRY No.2 SPF O - M 2x4 DRY No.2 SPF AD - AA 2x6 DRY 2100F 1.8E SPF AA - X 2x6 DRY 2100F 1.8E SPF X - U 2x6 DRY 2100F 1.8E SPF T - Q 2x6 DRY 2100F 1.8E SPF P - L 2x4 DRY No.2 SPF P - O 2x6 DRY No.2 SPF V - H 2x4 DRY No.2 SPF			FACTORED MAXIMUM FACTORED INPUT REQ'D GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX AD 7175 0 7175 -356 -817 5-8 5-8 O 5280 0 5280 0 -731 5-8 5-8			SPECIFIED LOADS: TOP CH. LL = 43.5 PSF DL = 6.0 PSF BOT CH. LL = 10.5 PSF DL = 7.4 PSF TOTAL LOAD = 67.3 PSF		
ALL WEBS 2x4 DRY No.2 SPF EXCEPT W - T 2x6 DRY No.2 SPF DRY: SEASONED LUMBER.			PROVIDE ANCHORAGE AT BEARING JOINT AD FOR 817 LBS. FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT O FOR 731 LBS. FACTORED UPLIFT PROVIDE FOR 356 LBS. FACTORED HORIZONTAL REACTION AT JOINT AD			SPACING = 24.0 IN. C/C LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 9.00/12 THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 4 OF CBC 2018, ABC 2019 - PART 4 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014		
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 2 12 SIDE (140.3) C-E 2 12 SIDE (70.1) E-F 2 12 TOP F-H 2 12 TOP H-J 2 12 TOP J-K 2 12 TOP K-N 2 12 TOP AD-B 2 12 TOP O-M 1 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS AD-AA 2 12 SIDE (227.4) AA-X 2 12 TOP X-U 2 12 TOP T-Q 2 12 TOP P-O 2 12 TOP L-P 1 12 TOP			UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL AD 5151 3441 / 0 492 / 0 0 / 0 428 / -892 1217 / 0 0 / 0 O 3820 2520 / 0 498 / 0 0 / 0 238 / -834 801 / 0 0 / 0 HORIZONTAL REACTIONS AD --- 0 / 0 0 / 0 0 / 0 254 / -254 0 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AD, O			DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR USED - PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED. - OVERHANG NOT TO BE ALTERED OR CUT OFF. (80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.55") CALCULATED VERT. DEFL.(LL) = L/999 (0.30") ALLOWABLE DEFL.(TL)= L/180 (3.10") CALCULATED VERT. DEFL.(TL) = L/999 (0.41") CSI: TO=0.45/1.00 (D-E:1), BC=0.38/1.00 (Y-Z:1), WB=0.86/1.00 (E-Y:1), SSI=0.20/1.00 (C-D:2) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.87 (AB) (INPUT = 0.90) JSI METAL= 0.94 (E) (INPUT = 1.00)		
BRACING MAX. UNBRACED TOP CHORD LENGTH = 3.14 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Y, G-W, H-T. 2x6 DRY SPF No.2 T-BRACE AT E-Y, F-W, G-T FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (18)			CHORDS WEBS MAX. FACTORED FACTORED MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX MAX MEMB. FORCE MAX (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO A-B 0 / 64 -145.3 -145.3 0.06 (2) 10.00 AC-C -848 / 153 0.14 (7) B-C -8719 / 938 -145.3 -145.3 0.40 (1) 3.81 C-AB -693 / 6672 0.59 (1) C-AE -11681 / 1357 -145.3 -145.3 0.40 (1) 3.30 D-Z -375 / 1263 0.17 (14) AE-AF -11681 / 1357 -145.3 -145.3 0.40 (1) 3.30 Z-E -643 / 322 0.11 (1) AF-D -11681 / 1357 -145.3 -145.3 0.40 (1) 3.30 E-Y -7899 / 1021 0.86 (1) D-E -12604 / 1601 -145.3 -145.3 0.45 (1) 3.14 Y-F -627 / 5424 0.48 (1) E-F -8384 / 1206 -145.3 -145.3 0.40 (1) 3.88 F-W -1563 / 148 0.30 (2) F-G -6199 / 1029 -145.3 -145.3 0.21 (1) 4.59 W-G -310 / 573 0.08 (10) G-H -5608 / 903 -145.3 -145.3 0.20 (1) 4.79 T-I -1382 / 395 0.48 (1) H-I -6982 / 1071 -145.3 -145.3 0.15 (1) 4.44 S-I -170 / 1127 0.10 (1) I-J -7856 / 1094 -145.3 -145.3 0.13 (1) 4.24 R-K -507 / 4003 0.35 (1) J-K -6365 / 900 -145.3 -145.3 0.06 (1) 4.70 R-L -383 / 165 0.03 (3) K-L -7527 / 995 -145.3 -145.3 0.23 (1) 4.21 B-AC -577 / 7055 0.62 (1) L-M -7924 / 1049 -145.3 -145.3 0.20 (1) 4.13 AB-D -1848 / 529 0.32 (1) M-N 0 / 64 -145.3 -145.3 0.06 (3) 10.00 Q-O -263 / 52 0.02 (1) AD-B -7049 / 838 0.0 0.0 0.25 (1) 5.83 Q-M -734 / 6183 0.55 (1) O-M -5099 / 708 0.0 0.0 0.28 (1) 5.31 W-T -688 / 6275 0.41 (1) AD-AG -341 / 349 -39.5 -39.5 0.05 (17) 6.25 G-T -1653 / 259 0.27 (2) AG-AH -341 / 349 -39.5 -39.5 0.05 (17) 6.25 S-J -1337 / 290 0.18 (1) AH-AC -341 / 349 -39.5 -39.5 0.05 (17) 6.25 J-R -3902 / 541 0.35 (1) AC-AI -757 / 6938 -39.5 -39.5 0.22 (1) 6.25 AI-AJ -757 / 6938 -39.5 -39.5 0.22 (1) 6.25 AJ-AB -757 / 6938 -39.5 -39.5 0.22 (1) 6.25 AB-AA -1249 / 11682 -39.5 -39.5 0.36 (1) 6.25			CONTINUED ON PAGE 2		



Structural component only
DWG# T-2204207 1/2

JOB NAME 417701	TRUSS NAME T4S	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:JvMhHdWEgsSI57rNnlib4czW6nl-48IXNtNdVxUX16?U4uB31fIO HihJnSLq?pmZAzrGTJ

V-H 1 12
WEBS : (0.122"x3") SPIRAL NAILS
AC-C 1 6
D-AB 1 6
2x4 1 6
2x6 2 6

TOP
SIDE(10.2)
SIDE(48.7)

LOADING

TOTAL LOAD CASES: (18)

MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED CSI (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO				FR-TO		
AA-Z	-1249 / 11682	-39.5	-39.5	0.36 (1)	6.25			
Z-Y	-1502 / 12622	-39.5	-39.5	0.38 (1)	6.25			
Y-X	-746 / 6787	-39.5	-39.5	0.22 (1)	6.25			
X-W	-746 / 6787	-39.5	-39.5	0.22 (1)	6.25			
W-V	-5 / 42	-39.5	-39.5	0.04 (4)	10.00			
V-U	0 / 0	-39.5	-39.5	0.00 (17)	10.00			
T-S	-667 / 6298	-39.5	-39.5	0.19 (1)	6.25			
S-R	-792 / 7166	-39.5	-39.5	0.22 (1)	6.25			
R-Q	-745 / 6393	-39.5	-39.5	0.19 (1)	6.25			
P-Q	0 / 37	0.0	0.0	0.17 (1)	10.00			
Q-L	0 / 231	0.0	0.0	0.19 (1)	10.00			
P-O	-27 / 215	-39.5	-39.5	0.02 (1)	6.25			
V-T	0 / 127	0.0	0.0	0.04 (1)	10.00			
T-H	-515 / 3634	0.0	0.0	0.35 (1)	6.25			

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	10.0		
C	TTWW+m	MT20	8.0	9.0	3.75	2.25
D	TMVW-t	MT20	4.0	6.0		
E	TTWW+m	MT20	7.0	8.0		
F	TTWW+m	MT20	6.0	10.0	4.00	1.75
G	TMVW-t	MT20	4.0	6.0		
H	TTV+m	MT20	5.0	8.0	Edge	
I	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	5.0	8.0		
K	TTW+m	MT20	5.0	8.0	Edge	
L	TMVW+p	MT20	4.0	6.0		
M	TMVW-t	MT20	5.0	8.0		
O	BMVW1+p	MT20	4.0	6.0		
P	BMV+p	MT20	3.0	6.0		
Q	BYMWW-t	MT20	6.0	12.0	4.00	7.75
R	BMVW-t	MT20	5.0	8.0		
S	BMVW-t	MT20	4.0	6.0		
T	BYMWW-t	MT20	8.0	12.0	4.50	4.50
V	BMV+p	MT20	3.0	6.0		
W	BYMWW-t	MT20	6.0	12.0	3.00	4.00
X	BS-t	MT20	5.0	6.0		
Y	BMVW-t	MT20	6.0	10.0		
Z	BMVW-t	MT20	4.0	6.0		
AA	BS-t	MT20	6.0	10.0		
AB	BYMWW-t	MT20	5.0	8.0	4.00	2.00
AC	BMVW-t	MT20	5.0	8.0	2.50	3.00
AD	BMV1-t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-294	-504		FRONT	VERT	TOTAL	--	C1
AB	11-4-8	-1843	-1843		FRONT	VERT	TOTAL	--	C1
AC	5-11-4	-22	-22		FRONT	VERT	TOTAL	--	C1
AF	7-5-4	-71	-158		FRONT	VERT	TOTAL	--	C1
AE	9-5-4	-71	-158		FRONT	VERT	TOTAL	--	C1
AG	1-11-4	-29	-29		FRONT	VERT	TOTAL	--	C1
AH	3-11-4	-29	-29		FRONT	VERT	TOTAL	--	C1
AI	7-5-4	-22	-22		FRONT	VERT	TOTAL	--	C1
AJ	9-5-4	-22	-22		FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING

AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2204207 *W*

CONTINUED ON PAGE 2

REVIEWED

JOB NAME 417704	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
X	BMWW-t	MT20	5.0	8.0	4.00	2.00
Z	BMVW1-t	MT20	7.0	8.0	3.25	3.00


Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

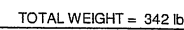
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2204184 *mn*

USMEETAL=0.95 (1) (INFOT=1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
417701	T5S	1	1	TRUSS DESC.	HOMES	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BWMWWW-t	MT20	8.0	12.0	5.00	5.00
W	BMW-w	MT20	3.0	6.0		
X	BMWWW-t	MT20	6.0	12.0	2.50	3.25
Y	BS-t	MT20	5.0	6.0		
Z	BMWW-t	MT20	5.0	8.0	3.75	2.50
AA	BMWW-t	MT20	4.0	6.0		
AB	BMWW-t	MT20	5.0	6.0	2.50	2.00
AC	BS-t	MT20	5.0	6.0		
AD	BMWW-t	MT20	4.0	6.0		
AE	BMWW-t	MT20	6.0	10.0	2.50	5.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX	FACTORED
(LBS)	(PLF)	CSI (LC)	UNBRAC	(LBS)	CSI (LC)		
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH
S-R	-679 / 5172	-39.5	-39.5	0.29 (1)	6.25		
Q-R	0 / 37	0.0	0.0	0.26 (1)	10.00		
R-M	-23 / 312	0.0	0.0	0.30 (1)	7.81		
Q-P	-26 / 178	-39.5	-39.5	0.03 (1)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2204208

Structural component only
DWG# T-2204185 *in*

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417704	T6	1	1	BAYVIEW WELLINGTON TRUSS DESC. HOMES	

Tamarack Roof Truss, Burlington

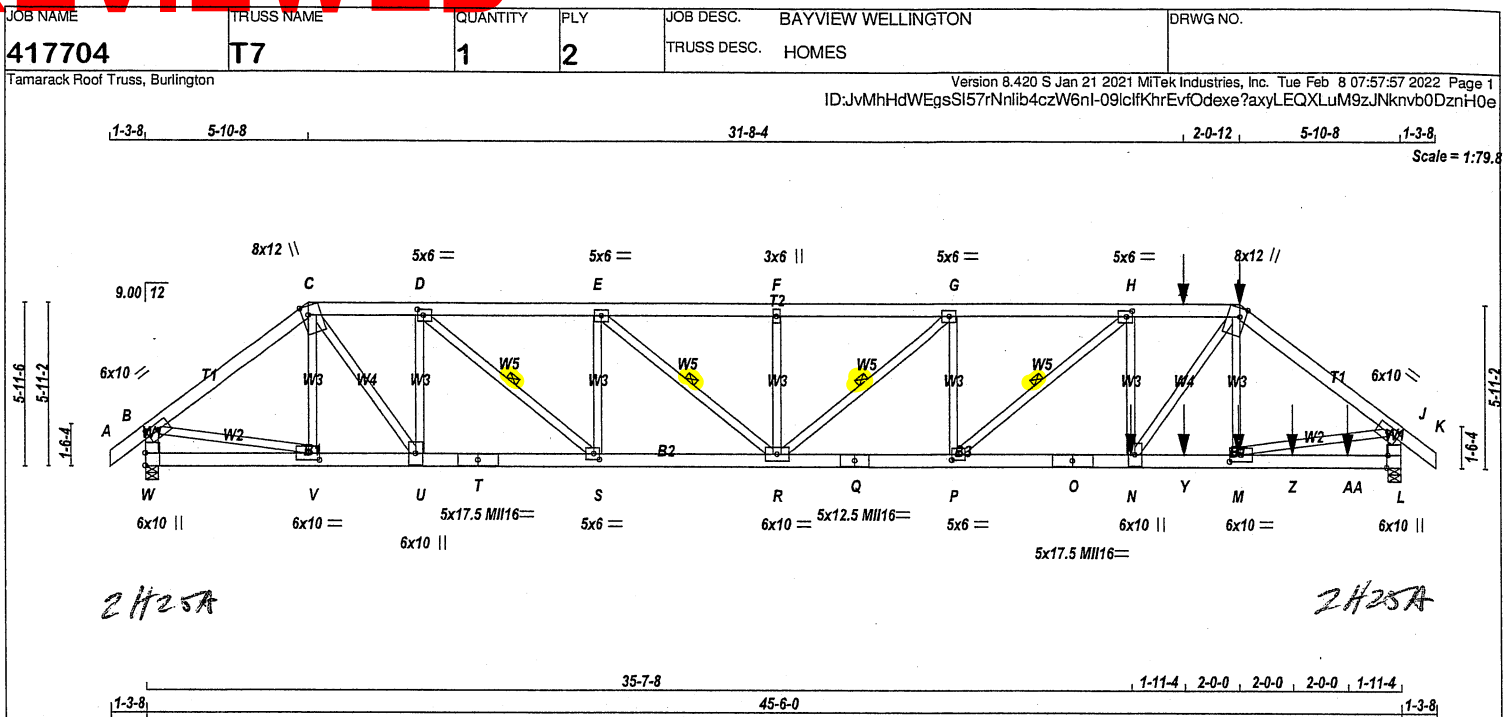
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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2204185 *en*



TOTAL WEIGHT = 2 X 281 = 562 lb

[M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x6	DRY	No.2	SPF
C - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - Q	2x6	DRY	2100F 1.8E	SPF
Q - O	2x6	DRY	2100F 1.8E	SPF
O - L	2x6	DRY	2100F 1.8E	SPF

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2 12	TOP
C - I	2 12	SIDE(70.1)
I - K	2 12	SIDE(140.3)
W - B	2 12	TOP
L - J	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W - T	2 12	TOP
Q - T	2 12	TOP
O - Q	2 12	TOP
O - L	2 12	SIDE(227.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1 6	
H - N	1 3	SIDE(1008.5)
D - U	1 3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2204186

DIMENSIONS, SUPPORTS, LOADS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BEARING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
W	5536	0		5593	-217	-870	5-8	5-8	
L	8791	0		8826	0	-934	5-8	5-8	

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 870 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 934 LBS FACTORED UPLIFT

PROVIDE FOR 217 LBS FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS									
1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
W	3993	2705 / 0	487 / 0	0 / 0	483 / -930	839 / 0	0 / 0		
L	6278	4309 / 0	510 / 0	0 / 0	400 / -993	1482 / 0	0 / 0		
HORIZONTAL REACTIONS									
W	--	0 / 0	0 / 0	0 / 0	155 / -155	0 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L									

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-P, D-S, G-R, E-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS		WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH FR-TO
A-B	0 / 64	-145.3 -145.3 0.06 (2)	10.00	V-C	-713 / 158	0.12 (7)
B-C	-6577 / 1011	-145.3 -145.3 0.34 (2)	4.36	C-U	-735 / 4960	0.44 (3)
C-D	-8107 / 1336	-145.3 -145.3 0.25 (2)	4.08	U-D	-3803 / 634	0.66 (3)
D-E	-11551 / 1757	-145.3 -145.3 0.46 (2)	3.34	N-H	-1496 / 632	0.26 (2)
E-F	-13521 / 1899	-145.3 -145.3 0.56 (2)	3.02	M-I	-810 / 7853	0.69 (2)
F-G	-13521 / 1899	-145.3 -145.3 0.57 (2)	3.01	M-I	-827 / 167	0.14 (10)
G-H	-14148 / 1815	-145.3 -145.3 0.61 (3)	2.91	B-V	-659 / 5322	0.47 (2)
H-X	-13305 / 1450	-145.3 -145.3 0.40 (3)	3.17	M-J	-730 / 8856	0.78 (3)
X-I	-13305 / 1450	-145.3 -145.3 0.40 (3)	3.17	P-H	-589 / 1235	0.11 (2)
I-J	-10944 / 1099	-145.3 -145.3 0.49 (3)	3.36	D-S	-597 / 4646	0.41 (3)
J-K	0 / 64	-145.3 -145.3 0.06 (3)	10.00	P-G	-505 / 443	0.09 (2)
W-B	-5490 / 892	0.0 0.0 0.20 (2)	6.26	S-E	-2752 / 447	0.48 (3)
L-J	-8703 / 956	0.0 0.0 0.31 (3)	5.11	R-G	-975 / 151	0.16 (8)
				E-R	-232 / 2713	0.24 (3)
				R-F	-982 / 282	0.16 (10)
W-V	-201 / 209	-39.5 -39.5 0.04 (17)	6.25			
V-U	-722 / 5231	-39.5 -39.5 0.16 (2)	6.25			
U-T	-1135 / 8108	-39.5 -39.5 0.25 (2)	6.25			
T-S	-1135 / 8108	-39.5 -39.5 0.25 (2)	6.25			
S-R	-1556 / 11551	-39.5 -39.5 0.34 (2)	6.25			
R-Q	-1579 / 14148	-39.5 -39.5 0.41 (3)	6.25			
Q-P	-1579 / 14148	-39.5 -39.5 0.41 (3)	6.25			
P-O	-1179 / 13305	-39.5 -39.5 0.40 (3)	6.25			
O-N	-1179 / 13305	-39.5 -39.5 0.40 (3)	6.25			
N-Y	-700 / 8715	-39.5 -39.5 0.28 (3)	6.25			
Y-M	-700 / 8715	-39.5 -39.5 0.28 (3)	6.25			
M-Z	-7 / 16	-39.5 -39.5 0.05 (17)	10.00			
Z-AA	-7 / 16	-39.5 -39.5 0.05 (17)	10.00			
AA-L	-7 / 16	-39.5 -39.5 0.05 (17)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 43.5 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 67.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.34")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.46")

CS1: TC=0.61/1.00 (G-H:3), BC=0.41/1.00 (P-R:3), WB=0.78/1.00 (J-M:3), SS1=0.17/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
M116 438 302 2547 1256 4283 1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.81 (B) (INPUT = 1.00)

CONTINUED ON PAGE



Structural component only
DWG# T-2204186

JOB NAME 417704	TRUSS NAME T7	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 07:57:57 2022 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	10.0	2.75	4.75
C	TTWW+m	MT20	8.0	12.0	3.75	2.50
D	TMVW-t	MT20	5.0	6.0	2.50	2.50
E	TMVW-t	MT20	5.0	6.0		
F	TMVW-w	MT20	3.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.50
I	TTWW+m	MT20	8.0	12.0	3.75	2.50
J	TMVW-t	MT20	6.0	10.0	2.75	4.75
L	BMV1-t	MT20	6.0	10.0	Edge	0.50
M	BMVW-t	MT20	6.0	10.0	3.00	4.75
N	BMVW-t	MT20	6.0	10.0		
O	BS-t	MI16	5.0	17.5		
P	BMVW-t	MT20	5.0	6.0	2.50	2.50
Q	BS-t	MI16	5.0	12.5		
R	BMVW-t	MT20	6.0	10.0		
S	BMVW-t	MT20	5.0	6.0	2.50	2.50
T	BS-t	MI16	5.0	17.5		
U	BMVW-t	MT20	6.0	10.0		
V	BMVW-t	MT20	6.0	10.0	3.00	4.75
W	BMV1-t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)


JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	39-7-8	-232	-322	---	BACK	VERT	TOTAL	---	C1
M	39-6-12	-164	-164	---	BACK	VERT	TOTAL	---	C1
N	35-7-8	-3240	-3281	63	BACK	VERT	TOTAL	---	C1
X	37-6-12	-2	-2	---	BACK	VERT	TOTAL	---	C1
Y	37-6-12	-164	-164	---	BACK	VERT	TOTAL	---	C1
Z	41-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
AA	43-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

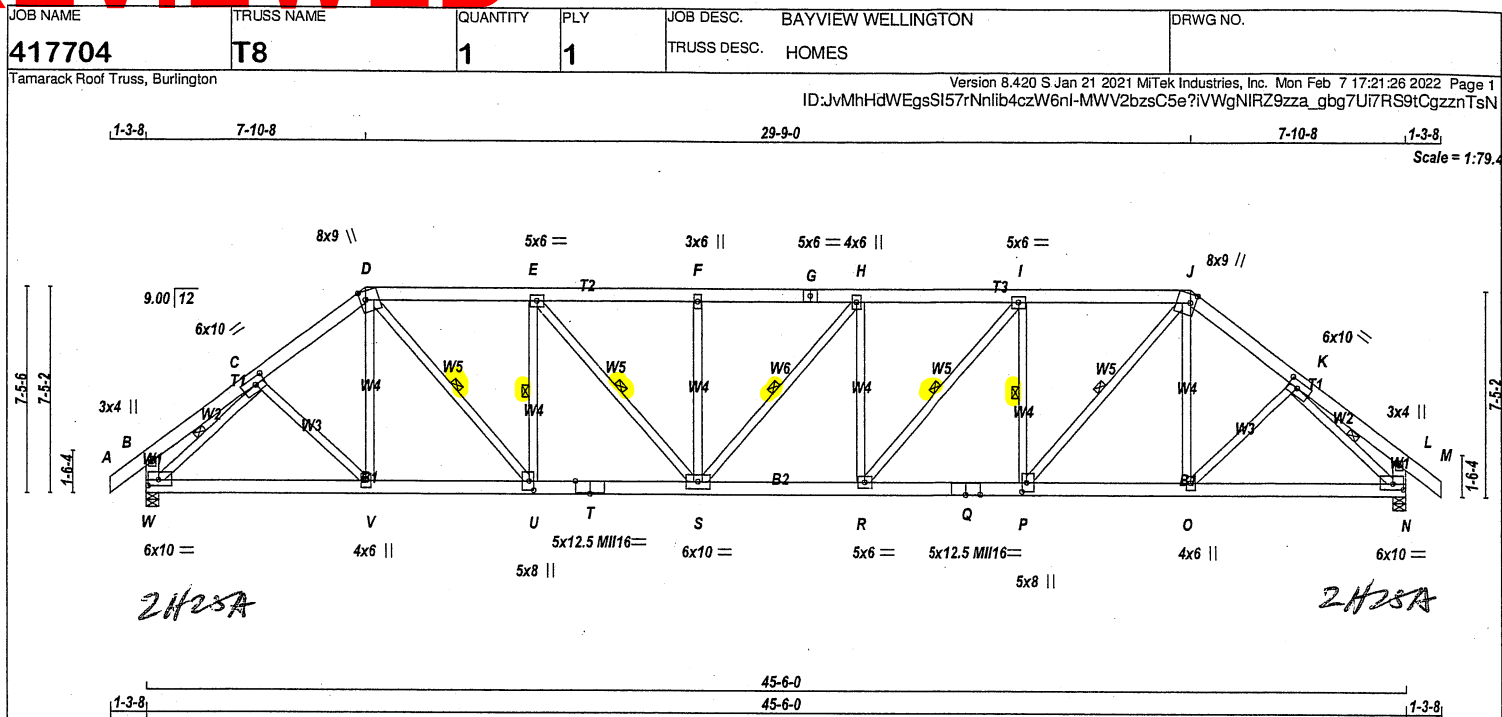
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2204186 *2/2*



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - Q	2x6	DRY	2100F 1.8E	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF

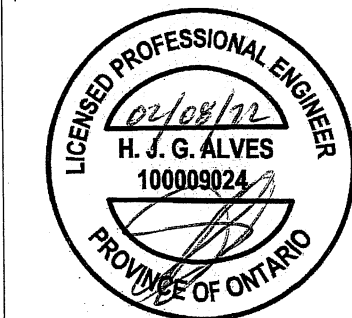
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	6.0	10.0	3.00	4.25
D	TTVW+m	MT20	8.0	9.0	3.75	2.25
E	TMVW+t	MT20	5.0	6.0		
F	TMVW+t	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMVW+t	MT20	4.0	6.0		
I	TMVW+t	MT20	5.0	6.0		
J	TTVW+m	MT20	8.0	9.0	3.75	2.25
K	TMVW+t	MT20	6.0	10.0	3.00	4.25
L	TMV+p	MT20	3.0	4.0		
N	BMVW+t	MT20	6.0	10.0	2.50	4.50
O	BMVW+t	MT20	4.0	6.0		
P	BMVW+t	MT20	5.0	8.0	4.00	2.00
Q	BS-t	MT20	5.0	12.5		6.25
R	BMVW+t	MT20	5.0	6.0		
S	BMVW+t	MT20	6.0	10.0		
T	BS-t	MT20	5.0	12.5	Edge	6.25
U	BMVW+t	MT20	5.0	8.0	4.00	2.00
V	BMVW+t	MT20	4.0	6.0		
W	BMVW+t	MT20	6.0	10.0	2.50	4.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)



Structural component only
DWG# T-2204187 1/12

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
W	4412	0	4412	-263
N	4412	0	4412	-835

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 835 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 835 LBS. FACTORED UPLIFT

PROVIDE FOR 263 LBS. FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3205	2101 / 0	478 / 0	0 / 0	454 / -898	627 / 0	0 / 0
N	3205	2101 / 0	478 / 0	0 / 0	302 / -898	627 / 0	0 / 0

HORIZONTAL REACTIONS

W	0 / 0	0 / 0	0 / 0	188 / -188	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-W, K-N, J-P, D-U, I-P, E-U, I-R, E-S, H-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO	0 / 64	-145.3	-145.3	0.11 (2)	C-V	0 / 622	0.10 (3)
B-C	-34 / 154	-145.3	-145.3	0.14 (2)	V-D	-197 / 278	0.12 (10)
C-D	-5006 / 1064	-145.3	-145.3	0.28 (2)	O-J	-175 / 297	0.11 (9)
D-E	-5911 / 1289	-145.3	-145.3	0.60 (2)	O-K	-63 / 622	0.10 (2)
E-F	-6800 / 1434	-145.3	-145.3	0.67 (1)	W-C	-5385 / 925	0.83 (1)
F-G	-6800 / 1434	-145.3	-145.3	0.56 (1)	K-N	-5386 / 926	0.83 (1)
G-H	-6800 / 1434	-145.3	-145.3	0.56 (1)	P-J	-631 / 3119	0.50 (2)
H-I	-6810 / 1437	-145.3	-145.3	0.66 (1)	D-U	-632 / 3123	0.50 (3)
I-J	-5909 / 1288	-145.3	-145.3	0.60 (3)	P-I	-2149 / 521	0.49 (2)
J-K	-5006 / 1064	-145.3	-145.3	0.28 (3)	U-E	-2142 / 519	0.49 (3)
K-L	-33 / 154	-145.3	-145.3	0.14 (3)	R-I	-296 / 1541	0.25 (2)
L-M	0 / 64	-145.3	-145.3	0.11 (3)	E-S	-290 / 1523	0.24 (3)
W-B	-478 / 205	0.0	0.0	0.03 (2)	R-H	-935 / 278	0.58 (2)
N-L	-477 / 205	0.0	0.0	0.03 (3)	S-F	-887 / 252	0.54 (10)
					S-H	-133 / 104	0.05 (3)
W-V	-780 / 3788	-39.5	-39.5	0.26 (1)			
V-U	-667 / 3977	-39.5	-39.5	0.26 (1)			
U-T	-1038 / 5912	-39.5	-39.5	0.33 (1)			
T-S	-1038 / 5912	-39.5	-39.5	0.33 (1)			
S-R	-1165 / 6810	-39.5	-39.5	0.37 (1)			
R-Q	-974 / 5910	-39.5	-39.5	0.33 (1)			
Q-P	-974 / 5910	-39.5	-39.5	0.33 (1)			
P-O	-559 / 3977	-39.5	-39.5	0.26 (1)			
O-N	-636 / 3788	-39.5	-39.5	0.26 (1)			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	43.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	67.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.52")
CALCULATED VERT. DEFL.(LL)= L/999 (0.31")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.40")

CSI: TC=0.87/1.00 (E-F:1), BC=0.37/1.00 (R-S:1), WB=0.83/1.00 (K-N:1), SSI=0.32/1.00 (D-E:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
MI16	438	302	1254


PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (N) (INPUT = 0.90)

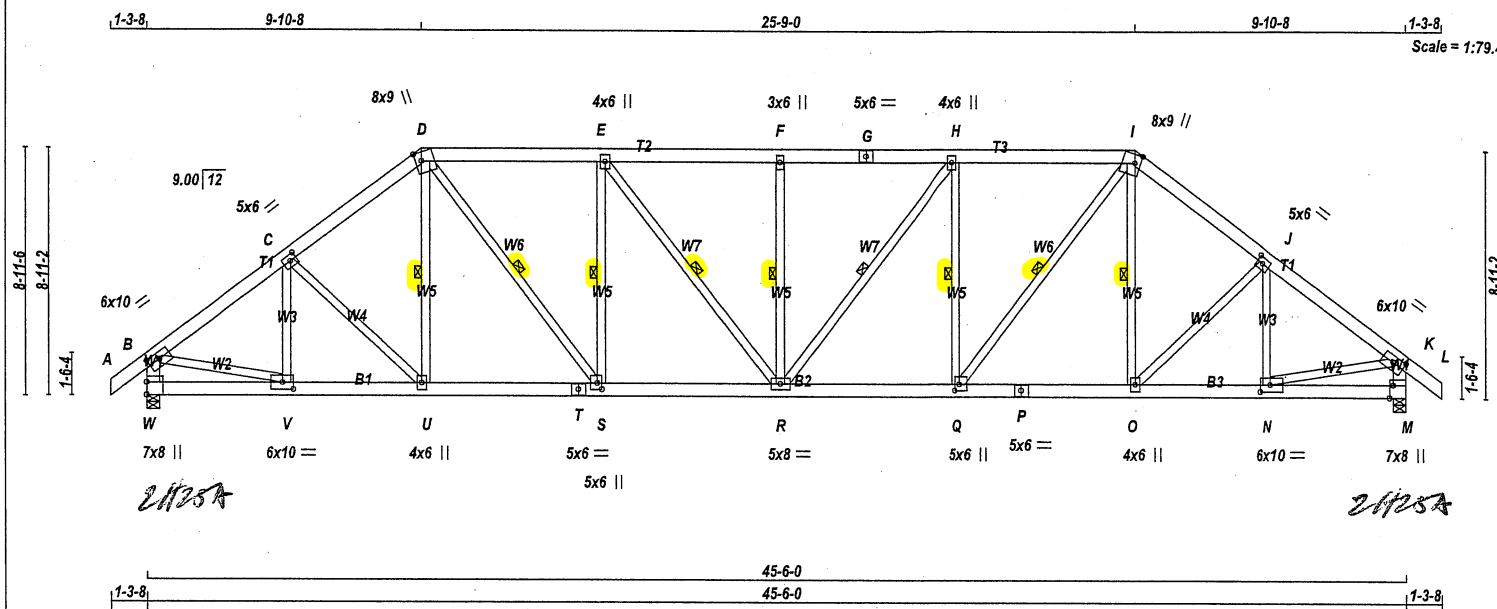
JSI METAL= 0.77 (K) (INPUT = 1.00)

REVIEWED

JOB NAME 417704	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:26 2022 Page 2 ID:JvMhHdWEgsSI57rNnlib4czW6nl-MWV2bzsC5e?iVWgNIRZ9zza_gbg7Ui7RS9tCqzznTsN				
<p>WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.</p>						
<div><p>Structural component only DWG# T-2204187 <i>ml</i></p></div>						

JOB NAME 417704	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Mon Feb 7 17:21:27 2022 Page 1
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TOTAL WEIGHT = 308 lb
(M)

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	6.0	10.0	2.75	5.00
C	TMW-t	MT20	5.0	6.0	2.50	2.75
D	TTW+m	MT20	8.0	9.0	3.75	2.50
E	TMW-t	MT20	4.0	6.0		
F	TMW-w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW-t	MT20	4.0	6.0		
I	TTW+m	MT20	8.0	9.0	3.75	2.50
J	TMW-t	MT20	5.0	6.0	2.50	2.75
K	TMW-t	MT20	6.0	10.0	2.75	5.00
M	BMV1-t	MT20	7.0	8.0	Edge	1.50
N	BMW-t	MT20	6.0	10.0	3.00	4.75
O	BMW-t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMW-t	MT20	5.0	6.0	2.75	2.00
R	BMW-t	MT20	5.0	8.0		
S	BMW-t	MT20	5.0	6.0	2.75	2.00
T	BS-t	MT20	5.0	6.0		
U	BMW-t	MT20	4.0	6.0		
V	BMW-t	MT20	6.0	10.0	3.00	4.75
W	BMV1-t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
1'



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	IN-SX	BRG	IN-SX
W	4412	0	4412	-310	-821	5-8	5-8	5-8	5-8
M	4412	0	4412	0	-821	5-8	5-8	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 821 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 821 LBS. FACTORED UPLIFT

PROVIDE FOR 310 LBS. FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
W	3205	2101 / 0	478 / 0	0 / 0	439 / -888	627 / 0	0 / 0
M	3205	2101 / 0	478 / 0	0 / 0	272 / -888	627 / 0	0 / 0

HORIZONTAL REACTIONS		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
W	0 / 0	0 / 0	0 / 0
M	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-U, D-S, E-S, E-R, F-R, H-R, H-Q, I-Q, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 64	-145.3 -145.3	0.11 (2)	10.00	V-C	-681 / 184	0.17 (7)
B-C	-4971 / 963	-145.3 -145.3	0.40 (2)	3.62	C-U	-235 / 291	0.14 (2)
C-D	-4886 / 1064	-145.3 -145.3	0.38 (2)	3.66	U-D	-112 / 429	0.07 (5)
D-E	-5287 / 1181	-145.3 -145.3	0.63 (2)	3.26	D-S	-511 / 2557	0.41 (3)
E-F	-5699 / 1229	-145.3 -145.3	0.66 (1)	3.11	S-E	-1776 / 464	0.60 (3)
F-G	-5699 / 1229	-145.3 -145.3	0.66 (1)	3.11	E-R	-175 / 878	0.14 (3)
G-H	-5699 / 1229	-145.3 -145.3	0.66 (1)	3.11	R-F	-926 / 263	0.30 (10)
H-I	-5287 / 1181	-145.3 -145.3	0.63 (3)	3.26	H-R	-175 / 878	0.14 (2)
I-J	-4886 / 1064	-145.3 -145.3	0.38 (3)	3.66	Q-H	-1771 / 464	0.60 (2)
J-K	-4971 / 963	-145.3 -145.3	0.40 (3)	3.62	Q-I	-511 / 2557	0.41 (2)
K-L	0 / 64	-145.3 -145.3	0.11 (3)	10.00	O-I	-112 / 429	0.07 (6)
W-B	-4311 / 835	0.0 0.0	0.28 (1)	5.20	C-J	-251 / 291	0.15 (8)
M-K	-4311 / 836	0.0 0.0	0.28 (1)	5.20	N-J	-677 / 184	0.17 (7)
					B-V	-641 / 4089	0.66 (1)
					N-K	-641 / 4089	0.66 (1)
W-V	-294 / 302	-39.5 -39.5	0.09 (4)	6.25			
V-U	-789 / 4010	-39.5 -39.5	0.56 (1)	6.25			
U-T	-629 / 3869	-39.5 -39.5	0.54 (1)	6.25			
T-S	-629 / 3869	-39.5 -39.5	0.54 (1)	6.25			
S-R	-881 / 5287	-39.5 -39.5	0.70 (1)	6.25			
R-Q	-823 / 5287	-39.5 -39.5	0.70 (1)	6.25			
Q-P	-511 / 3869	-39.5 -39.5	0.54 (1)	6.25			
P-O	-511 / 3869	-39.5 -39.5	0.54 (1)	6.25			
O-N	-613 / 4010	-39.5 -39.5	0.56 (1)	6.25			
N-M	-7 / 16	-39.5 -39.5	0.09 (4)	10.00			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 43.5 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 67.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.52")
CALCULATED VERT. DEFL.(LL)= L/999 (0.25")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.32")

CSI: TC=0.66/1.00 (F-H:1), BC=0.70/1.00 (Q-R:1), WB=0.66/1.00 (K-N:1), SSI=0.35/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.90)
JSI METAL= 0.76 (K) (INPUT = 1.00)

Structural component only
DWG# T-2204188

CONTINUED ON PAGE 2

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417704	T9	1	1	BAYVIEW WELLINGTON TRUSS DESC. HOMES	

Tamarack Roof Truss, Burlington

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ID:JvMhHdWEgsSI57rNnlib4czW6nl-q3QpJtqsy7Z7qFZs94OWB69c?xEDB3ahpcmCQznTsM

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2204188 *mn*

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
417704	T10	1	1	TRUSS DESC.	HOMES	

Tamarack Roof Truss, Burlington

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ID:JvMhHdWEgsSI57rNnlib4czW6nl-lvdo0fuSdGfQkqqlQscd2OfIqPHhyeBkwTMJksznTsL

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

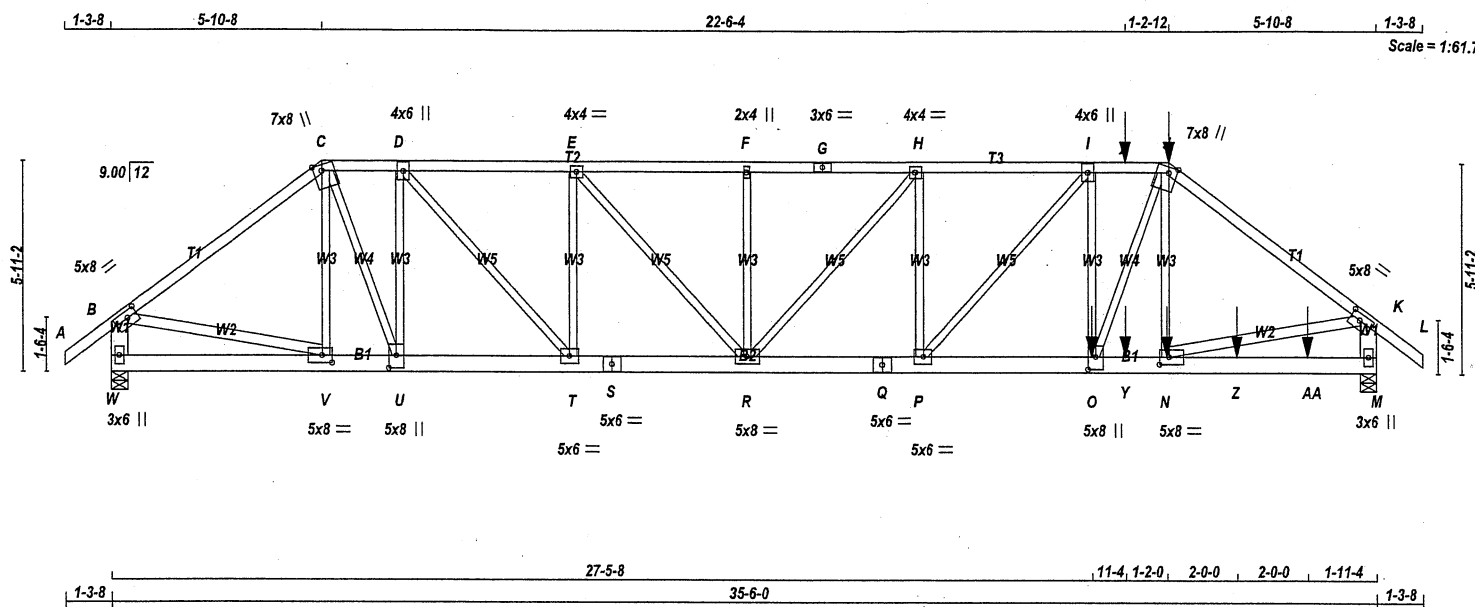


Structural component only
DWG# T-2204189 *in*

JOB NAME 417704	TRUSS NAME T11	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 189 = 378 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	2100F 1.8E
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	2100F 1.8E
W - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
W - S	2x6	DRY	No.2
S - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
N - K	2x4	DRY	No.2
B - V	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - G	12	TOP
G - J	12	SIDE(61.0)
J - L	12	SIDE(65.9)
W - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W - S	12	TOP
S - Q	12	TOP
Q - M	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
JT	2725	0	2725	0	5-8	5-8
W	3562	0	3562	0	5-8	5-8
M						

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	1906	1368 / 0	0 / 0	0 / 0	538 / 0	0 / 0
W	2494	1777 / 0	0 / 0	0 / 0	716 / 0	0 / 0
M						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. UNBRACED LENGTH (FR-TO)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC1)
FR-TO		FROM TO			FR-TO		
A-B	0 / 47	-112.4 -112.4	0.05 (1)	10.00	V-C	-294 / 0	0.08 (1)
B-C	-2992 / 0	-112.4 -112.4	0.29 (1)	6.05	N-J	-51 / 129	0.02 (4)
C-D	-2981 / 0	-112.4 -112.4	0.14 (1)	5.21	N-K	0 / 3256	0.29 (1)
D-E	-4048 / 0	-112.4 -112.4	0.26 (1)	4.50	B-V	0 / 2382	0.21 (1)
E-F	-4526 / 0	-112.4 -112.4	0.28 (1)	4.27	O-I	-1150 / 0	0.30 (1)
F-G	-4526 / 0	-112.4 -112.4	0.28 (1)	4.27	O-J	0 / 1752	0.22 (1)
G-H	-4526 / 0	-112.4 -112.4	0.28 (1)	4.27	U-D	-1637 / 0	0.43 (1)
H-I	-4477 / 0	-112.4 -112.4	0.28 (1)	4.30	C-U	0 / 1768	0.22 (1)
I-X	-3839 / 0	-112.4 -112.4	0.16 (1)	4.69	P-I	0 / 969	0.12 (1)
X-J	-3839 / 0	-112.4 -112.4	0.16 (1)	4.69	D-T	0 / 1621	0.20 (1)
J-K	-4008 / 0	-112.4 -112.4	0.32 (1)	5.37	P-H	-627 / 0	0.16 (1)
K-L	0 / 47	-112.4 -112.4	0.05 (1)	10.00	T-E	-1117 / 0	0.29 (1)
W-B	-2684 / 0	0.0	0.0	10.00	R-H	0 / 73	0.01 (1)
M-K	-3490 / 0	0.0	0.0	10.00	E-R	0 / 726	0.09 (1)
					R-F	-530 / 0	0.14 (1)
W-V	0 / 0	-18.5	-18.5	0.04 (4)			
V-U	0 / 2337	-18.5	-18.5	0.18 (1)			
U-T	0 / 2981	-18.5	-18.5	0.22 (1)			
T-S	0 / 4048	-18.5	-18.5	0.29 (1)			
S-R	0 / 4048	-18.5	-18.5	0.29 (1)			
R-Q	0 / 4477	-18.5	-18.5	0.32 (1)			
Q-P	0 / 4477	-18.5	-18.5	0.32 (1)			
P-O	0 / 3839	-18.5	-18.5	0.28 (1)			
O-Y	0 / 3200	-18.5	-18.5	0.25 (1)			
Y-N	0 / 3200	-18.5	-18.5	0.25 (1)			
N-Z	0 / 0	-18.5	-18.5	0.07 (4)			
Z-AA	0 / 0	-18.5	-18.5	0.07 (4)			
AA-M	0 / 0	-18.5	-18.5	0.07 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	29-7-8	-275			BACK	VERT	TOTAL		C1
N	29-6-12	-164			BACK	VERT	TOTAL		C1
O	27-5-8	-255			BACK	VERT	TOTAL		C1
X	28-4-12	-15			BACK	VERT	TOTAL		C1
Y	28-4-12	-164			BACK	VERT	TOTAL		C1
Z	31-6-12	-29			BACK	VERT	TOTAL		C1
AA	33-6-12	-29			BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.18")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
ALLOWABLE DEFL.(TL) = $L/360$ (1.18")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.20")

CSI: TC=0.32/1.00 (J-K:1), BC=0.32/1.00 (P-R:1), WB=0.43/1.00 (D-U:1), SS=0.13/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MIN	MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (D) (INPUT = 0.90)

JSI METAL= 0.40 (S) (INPUT = 1.00)



Structural component only
DWG# T-2204190

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
417704	T11	1	2	TRUSS DESC.	HOMES	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:30 2022 Page 2
ID:JvMhHdWEgsSI57rNnlib4czW6nl-EHKYRLvi9tV8 8z8XHe57pkI9C2rQcQ0NnrQplznTsJ

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.50
C	TTWW+m	MT20	7.0	8.0	Edge	2.75
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.75
K	TMVW-t	MT20	5.0	8.0	2.25	3.50
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	3.25
O	BMWW-t	MT20	5.0	8.0	4.25	2.50
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	8.0	4.25	2.50
V	BMWW-t	MT20	5.0	8.0	2.50	3.25
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

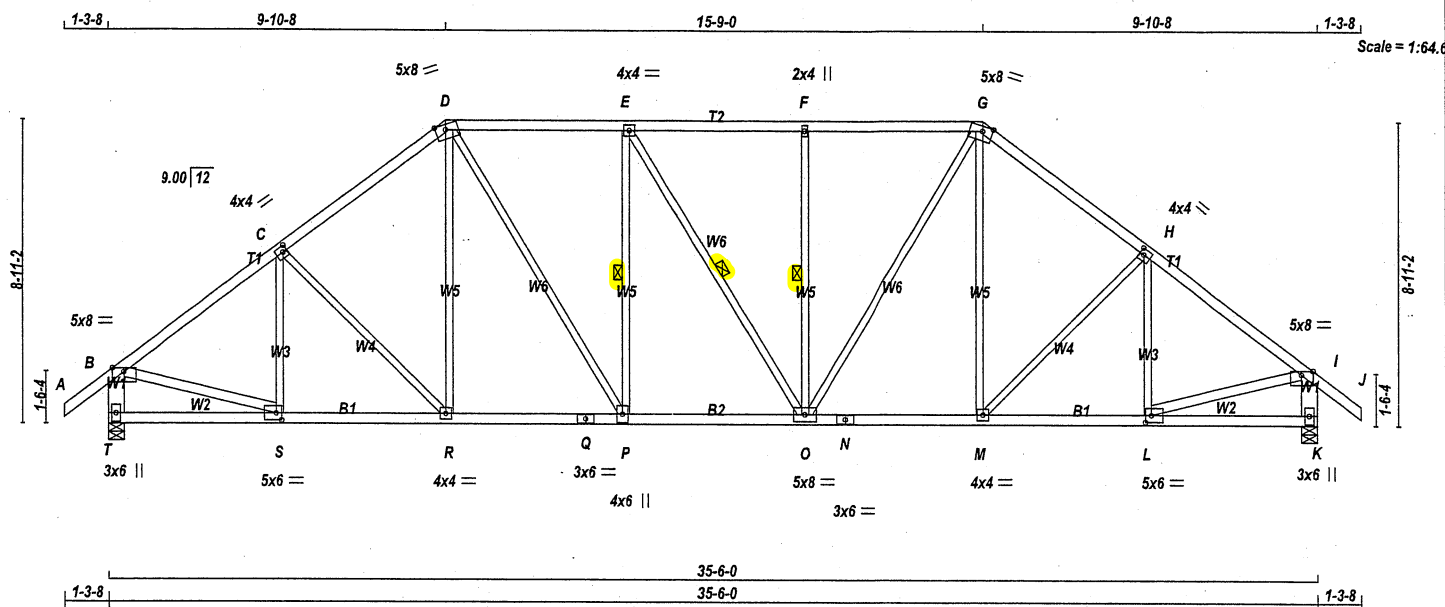


Structural component only
DWG# T-2204190

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.82 (R) (INPUT = 1.00)

JOB NAME 417704	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:32 2022 Page 1
ID:JvMhHdWEgsSI57rNnlib4czW6nl-BgsJs0xzhUlrDR7WfigZDEq210JauXAJr5KXudznTsh



TOTAL WEIGHT = 175 lb (M/F)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
T - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
T - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
B - S	2x4	DRY	No.2
L - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TTVW-m	MT20	5.0	8.0	Edge	3.50
E TMVW-t	MT20	4.0	4.0		
F TMVW-w	MT20	2.0	4.0		
G TTVW-m	MT20	5.0	8.0	Edge	3.50
H TMVW-t	MT20	4.0	4.0	2.00	1.50
I TMVW-p	MT20	5.0	8.0	Edge	
K BMV1+p	MT20	3.0	6.0		
L BMVW-t	MT20	5.0	6.0	2.50	2.00
M BMVW-t	MT20	4.0	4.0		
N BS-t	MT20	3.0	6.0		
O BMVW-t	MT20	5.0	8.0		
P BMVW-t	MT20	4.0	6.0		
Q BS-t	MT20	3.0	6.0		
R BMVW-t	MT20	4.0	4.0		
S BMVW-t	MT20	5.0	6.0	2.50	2.00
T BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
T	2479	0	0	2479	0	0	0	5-8	5-8
K	2479	0	0	2479	0	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1735	1242 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
K	1735	1242 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, E-O, F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 47	-112.4 -112.4	0.15 (1)	10.00	S-C	-399/ 0	0.17 (1)	
B-C	-2577 / 0	-112.4 -112.4	0.47 (1)	3.88	C-R	-239 / 0	0.22 (1)	
C-D	-2450 / 0	-112.4 -112.4	0.45 (1)	3.98	R-D	0 / 265	0.06 (1)	
D-E	-2377 / 0	-112.4 -112.4	0.43 (1)	4.03	D-P	0 / 860	0.19 (1)	
E-F	-2375 / 0	-112.4 -112.4	0.40 (1)	4.03	P-E	-635 / 0	0.33 (1)	
F-G	-2375 / 0	-112.4 -112.4	0.43 (1)	4.04	E-O	-4 / 0	0.00 (1)	
G-H	-2451 / 0	-112.4 -112.4	0.45 (1)	3.98	O-F	-634 / 0	0.33 (1)	
H-I	-2577 / 0	-112.4 -112.4	0.47 (1)	3.88	O-G	0 / 855	0.19 (1)	
I-J	0 / 47	-112.4 -112.4	0.15 (1)	10.00	M-G	0 / 268	0.06 (1)	
T-B	-2438 / 0	0.0	0.0	0.17 (1)	6.61	M-H	-238 / 0	0.22 (1)
K-I	-2438 / 0	0.0	0.0	0.16 (1)	6.61	L-H	-399 / 0	0.17 (1)
					B-S	0 / 2150	0.35 (1)	
					L-I	0 / 2150	0.35 (1)	
T-S	0 / 0	-18.5 -18.5	0.10 (4)	10.00				
S-R	0 / 2093	-18.5 -18.5	0.39 (1)	10.00				
R-Q	0 / 1927	-18.5 -18.5	0.36 (1)	10.00				
Q-P	0 / 1927	-18.5 -18.5	0.36 (1)	10.00				
P-O	0 / 2377	-18.5 -18.5	0.43 (1)	10.00				
O-N	0 / 1928	-18.5 -18.5	0.37 (1)	10.00				
N-M	0 / 1928	-18.5 -18.5	0.37 (1)	10.00				
M-L	0 / 2093	-18.5 -18.5	0.39 (1)	10.00				
L-K	0 / 0	-18.5 -18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 45.9	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.18")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.18")
CALCULATED VERT. DEFL.(TL)= L/999 (0.21")

CSI: TC=0.47/1.00 (B-C:1), BC=0.43/1.00 (O-P:1), WB=0.35/1.00 (B-S:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

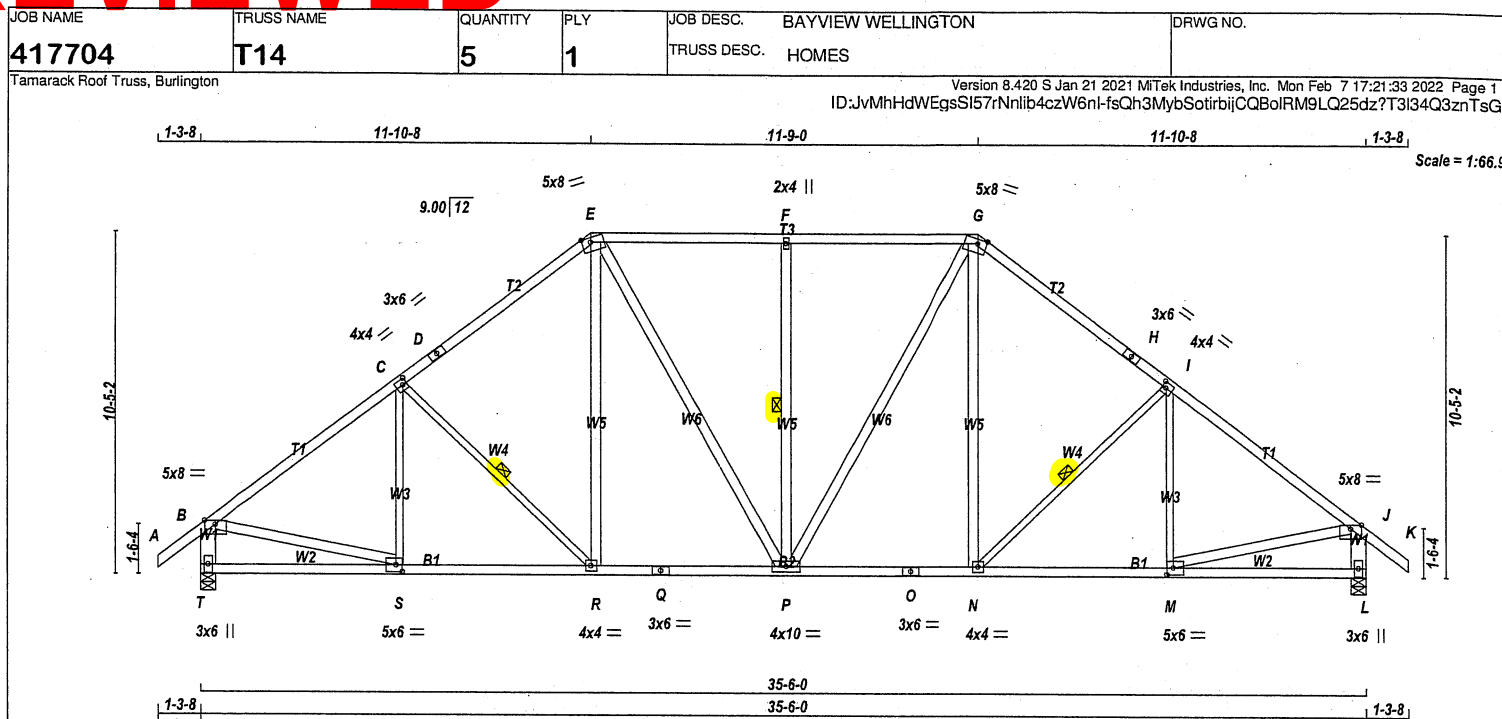
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (S) (INPUT = 0.90)
JSI METAL= 0.57 (N) (INPUT = 1.00)



Structural component only
DWG# T-2204192



TOTAL WEIGHT = 5 X 190 = 951 lb [M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
T - Q	2x4	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
S - C	2x3	DRY	No.2	SPF	
C - R	2x3	DRY	No.2	SPF	
N - I	2x3	DRY	No.2	SPF	
M - I	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	6.0		
E TTVW-m	MT20	5.0	8.0	1.75	3.25
F TMVW-w	MT20	2.0	4.0		
G TTVW-m	MT20	5.0	8.0	1.75	3.25
H TS-t	MT20	3.0	6.0		
I TMVW-t	MT20	4.0	4.0	2.00	1.50
J TMVW-p	MT20	5.0	8.0	Edge	
L BMV1+p	MT20	3.0	6.0		
M BMVW-t	MT20	5.0	6.0	2.50	2.25
N BMVW-t	MT20	4.0	4.0		
O BS-t	MT20	3.0	6.0		
P BMVW-t	MT20	4.0	10.0		
Q BS-t	MT20	3.0	6.0		
R BMVW-t	MT20	4.0	4.0		
S BMVW-t	MT20	5.0	6.0	2.50	2.25
T BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
T	2479	0	5-8	5-8
L	2479	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1735	1242 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
L	1735	1242 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	TO	TO	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)	FR-TO	CSI (LC)
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00	S-C	-300 / 20	0.17 (1)		
B-C	-2614 / 0	-112.4	-112.4	0.70 (1)	3.56	C-R	-435 / 0	0.20 (1)		
C-D	-2322 / 0	-112.4	-112.4	0.64 (1)	3.81	E-P	0 / 409	0.07 (1)		
D-E	-2322 / 0	-112.4	-112.4	0.64 (1)	3.81	E-P	0 / 527	0.08 (1)		
E-F	-2086 / 0	-112.4	-112.4	0.57 (1)	4.04	P-F	-808 / 0	0.44 (1)		
F-G	-2086 / 0	-112.4	-112.4	0.57 (1)	4.04	P-G	0 / 527	0.08 (1)		
G-H	-2322 / 0	-112.4	-112.4	0.64 (1)	3.81	N-G	0 / 409	0.07 (1)		
H-I	-2322 / 0	-112.4	-112.4	0.64 (1)	3.81	N-I	-435 / 0	0.20 (1)		
I-J	-2614 / 0	-112.4	-112.4	0.70 (1)	3.56	M-I	-300 / 20	0.17 (1)		
J-K	0 / 47	-112.4	-112.4	0.15 (1)	10.00	B-S	0 / 2171	0.35 (1)		
T-B	-2432 / 0	0.0	0.0	0.16 (1)	6.62	M-J	0 / 2171	0.35 (1)		
L-J	-2432 / 0	0.0	0.0	0.16 (1)	6.62					
T-S	0 / 0	-18.5	-18.5	0.16 (4)	10.00					
S-R	0 / 2129	-18.5	-18.5	0.42 (1)	10.00					
R-Q	0 / 1821	-18.5	-18.5	0.36 (1)	10.00					
Q-P	0 / 1821	-18.5	-18.5	0.36 (1)	10.00					
P-O	0 / 1821	-18.5	-18.5	0.36 (1)	10.00					
O-N	0 / 1821	-18.5	-18.5	0.36 (1)	10.00					
N-M	0 / 2129	-18.5	-18.5	0.42 (1)	10.00					
M-L	0 / 0	-18.5	-18.5	0.16 (4)	10.00					

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.70/1.00 (I-J-I), BC=0.42/1.00 (M-N-I), WB=0.44/1.00 (F-P-I), SI=0.32/1.00 (E-F-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (M) (INPUT = 0.90)
JSI METAL= 0.56 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2204193

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
417704	T15	1	2	TRUSS DESC.	HOMES	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:06:38 2022 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.25
C	TTWW+m	MT20	6.0	10.0	Edge	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	10.0	Edge	2.00
H	TMVW-p	MT20	5.0	6.0	1.25	3.25
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWWW-t	MT20	5.0	8.0	2.50	2.50
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0	2.50	3.00
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

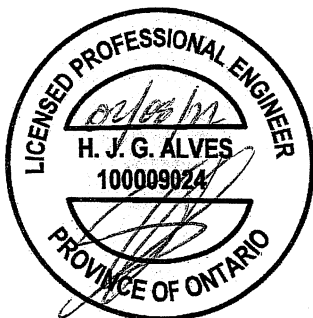
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	14-7-4	-15	-15	---	BACK	VERT	TOTAL	---	C1
U	1-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
V	3-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
W	6-7-4	-164	-164	---	BACK	VERT	TOTAL	---	C1
X	8-7-4	-164	-164	---	BACK	VERT	TOTAL	---	C1
Y	12-7-4	-164	-164	---	BACK	VERT	TOTAL	---	C1
Z	14-7-4	-164	-164	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

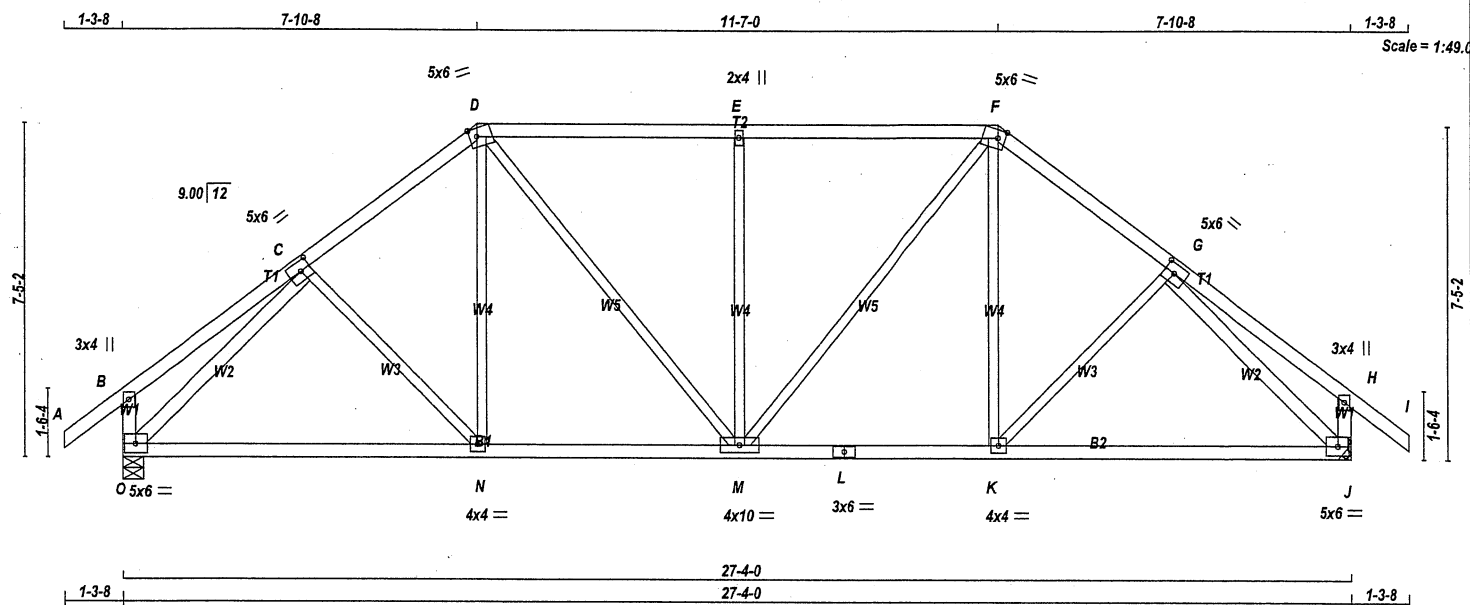
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2204194

JOB NAME 417704	TRUSS NAME T16	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:35 2022 Page 1
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TOTAL WEIGHT = 125 lb
(M/F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
O - C	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	6.0	Edge	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge	2.00
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVWW-t	MT20	4.0	10.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
O	1944	0	1944	0
J	1944	0	1944	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
O	1360	977 / 0	0 / 0	0 / 0
J	1360	977 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD	PLF	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)	CSI (LC)
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00	C-N	-80 / 32	0.05 (1)	
B-C	0 / 30	-112.4	-112.4	0.27 (1)	10.00	N-D	0 / 214	0.05 (4)	
C-D	-1812 / 0	-112.4	-112.4	0.25 (1)	4.74	D-M	0 / 560	0.13 (1)	
D-E	-1779 / 0	-112.4	-112.4	0.53 (1)	4.38	M-E	-797 / 0	0.79 (1)	
E-F	-1779 / 0	-112.4	-112.4	0.53 (1)	4.38	M-F	0 / 560	0.13 (1)	
F-G	-1812 / 0	-112.4	-112.4	0.25 (1)	4.74	K-F	0 / 214	0.05 (4)	
G-H	0 / 30	-112.4	-112.4	0.27 (1)	10.00	K-G	-80 / 32	0.05 (1)	
H-I	0 / 47	-112.4	-112.4	0.15 (1)	10.00	O-C	-2149 / 0	0.82 (1)	
O-B	-325 / 0	0.0	0.0	0.03 (1)	7.81	G-J	-2149 / 0	0.82 (1)	
J-H	-325 / 0	0.0	0.0	0.03 (1)	7.81				
O-N	0 / 1479	-18.5	-18.5	0.38 (1)	10.00				
N-M	0 / 1425	-18.5	-18.5	0.37 (1)	10.00				
M-L	0 / 1425	-18.5	-18.5	0.37 (1)	10.00				
L-K	0 / 1425	-18.5	-18.5	0.37 (1)	10.00				
K-J	0 / 1479	-18.5	-18.5	0.38 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 32.5	PSF
DL	= 6.0	PSF	
BOT CH.	LL	= 0.0	PSF
DL	= 7.4	PSF	
TOTAL LOAD	= 45.9	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.15")

CSI: TC=0.53/1.00 (D-E:1), BC=0.38/1.00 (J-K:1), WB=0.82/1.00 (G-J:1), SS=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.48 (G) (INPUT = 1.00)

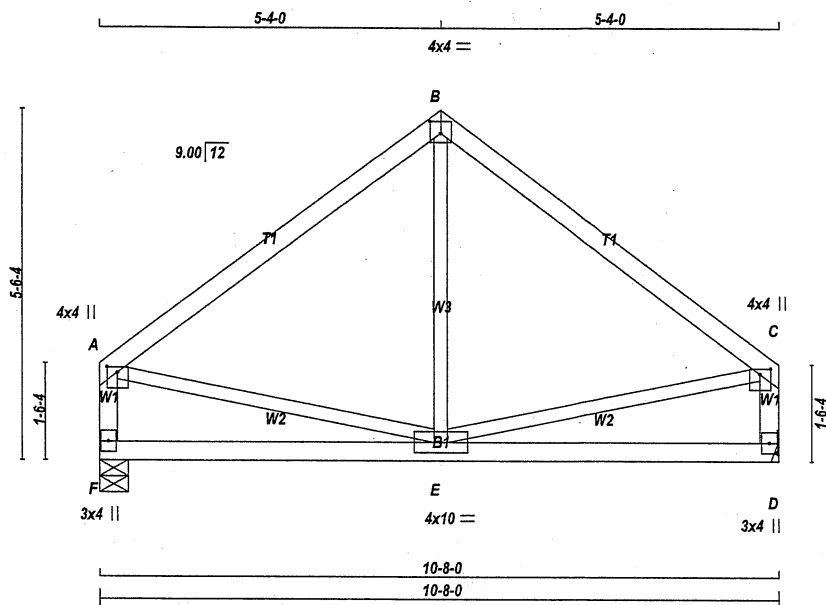


Structural component only
DWG# T-2204195

JOB NAME 417704	TRUSS NAME T17	QUANTITY 4	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 43 = 170 lb
(M/F)

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW+p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	10.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	VERT 698	DOWN 698	5-8	5-8
D	HORZ 0	HORZ 0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	489	346 / 0	0 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0
D	489	346 / 0	0 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-463 / 0	-112.4 -112.4	0.41 (1)	E-B	-44 / 85	0.03 (4)	
B-C	-463 / 0	-112.4 -112.4	0.41 (1)	A-E	0 / 379	0.09 (1)	
F-A	-660 / 0	0.0 0.0	0.07 (1)	E-C	0 / 379	0.09 (1)	
D-C	-660 / 0	0.0 0.0	0.07 (1)				
F-E	0 / 0	-18.5 -18.5	0.15 (4)				
E-D	0 / 0	-18.5 -18.5	0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 45.9 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.41/1.00 (A-B:1), BC=0.15/1.00 (D-E:4), WB=0.09/1.00 (C-E:1), SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

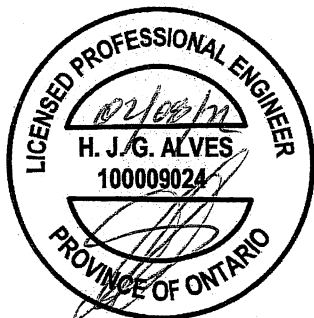
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 650 371	1747 788	1987 1873

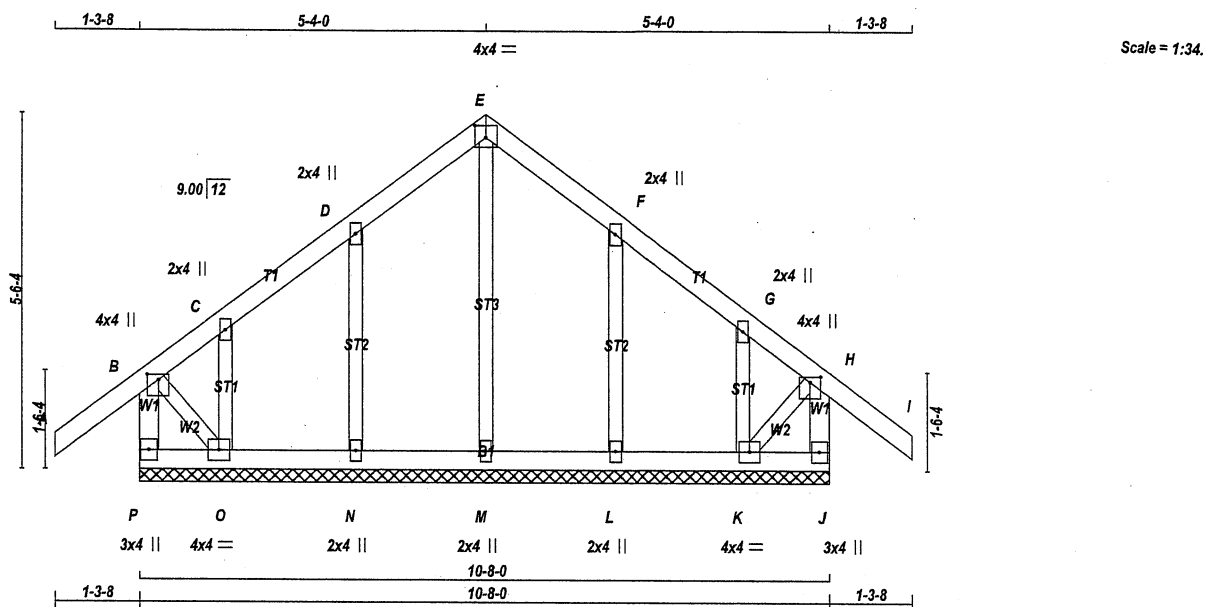
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (C) (INPUT = 0.90)
JSI METAL= 0.16 (C) (INPUT = 1.00)



Structural component only
DWG# T-2204196



TOTAL WEIGHT = 49 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS				
	2x3	DRY	No.2	SPF
DRY- SEASONED LUMBER				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW-w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	
	(LBS)	(PLF)	CSI (LC)	UNBRAC			MAX	
FR-TO		FROM	TO	LENGTH	FR-TO		CSI (LC)	
P-B	-328 / 0	0.0	0.0	0.03 (1)	7.81	M-E	-175 / 0	0.08 (1)
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00	N-D	-273 / 0	0.07 (1)
B-C	-66 / 0	-112.4	-112.4	0.15 (1)	6.25	O-C	-80 / 0	0.01 (1)
C-D	-2 / 0	-112.4	-112.4	0.07 (1)	10.00	L-F	-273 / 0	0.07 (1)
D-E	-22 / 0	-112.4	-112.4	0.07 (1)	6.25	K-G	-80 / 0	0.01 (1)
E-F	-22 / 0	-112.4	-112.4	0.07 (1)	6.25	B-O	0 / 21	0.00 (1)
F-G	-2 / 0	-112.4	-112.4	0.07 (1)	10.00	K-H	0 / 21	0.00 (1)
G-H	-66 / 0	-112.4	-112.4	0.15 (1)	6.25			
H-I	0 / 47	-112.4	-112.4	0.15 (1)	10.00			
J-H	-328 / 0	0.0	0.0	0.03 (1)	7.81			
P-O	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
O-N	0 / 12	-18.5	-18.5	0.02 (4)	10.00			
N-M	0 / 5	-18.5	-18.5	0.02 (4)	10.00			
M-L	0 / 5	-18.5	-18.5	0.02 (4)	10.00			
L-K	0 / 12	-18.5	-18.5	0.02 (4)	10.00			
K-J	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9. NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (H-I:1), BC=0.02/1.00 (K-L:4),
WB=0.08/1.00 (E-M:1), SSI=0.10/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

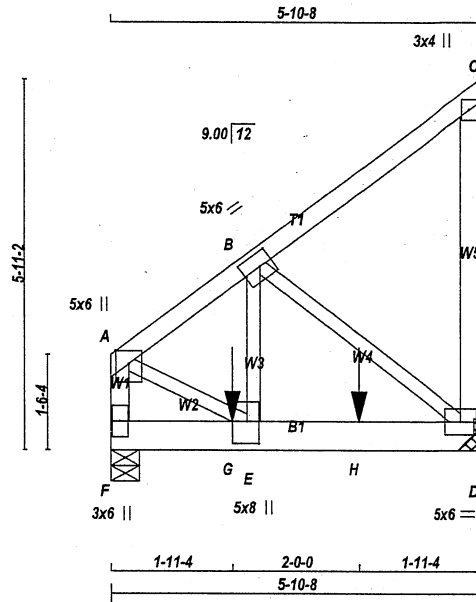
JSI GRIP= 0.24 (H) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 1.00)



Structural component only
DWG# T-2204171

JOB NAME 417704	TRUSS NAME T18	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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Scale = 1:35.2

TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE (197.8)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	2	SIDE (435.6)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	5.0	6.0	2.50	2.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	2762	0	2762	0
D	2708	0	2708	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1932	1388 / 0	0 / 0	0 / 0	0 / 0	544 / 0	0 / 0
D	1895	1357 / 0	0 / 0	0 / 0	0 / 0	538 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-2737 / 0	0.0	0.0 0.15 (1)	A-E	0 / 2387	0.30 (1)	
A-B	-2653 / 0	-112.4	-112.4 0.10 (1)	E-B	0 / 2876	0.36 (1)	
B-C	-18 / 0	-112.4	-112.4 0.10 (1)	B-D	-2720 / 0	0.45 (1)	
D-C	-173 / 0	0.0	0.0 0.05 (1)				
F-G	0 / 0	-18.5	-18.5 0.16 (1)				
G-E	0 / 0	-18.5	-18.5 0.16 (1)				
E-H	0 / 2145	-18.5	-18.5 0.38 (1)				
H-D	0 / 2145	-18.5	-18.5 0.38 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-1697	-1697	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	-1591	-1591	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 45.9	PSF	

SPACING = **24.0** IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.15/1.00 (A-F:1), BC=0.38/1.00 (D-E:1), WB=0.45/1.00 (B-D:1), SSI=0.81/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.48 (A) (INPUT = 1.00)



Structural component only
DWG# T-2204197

REVIEWED

JOB NAME 417704	TRUSS NAME T18	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	


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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMW+u	MT20	5.0	8.0	4.25	2.25
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

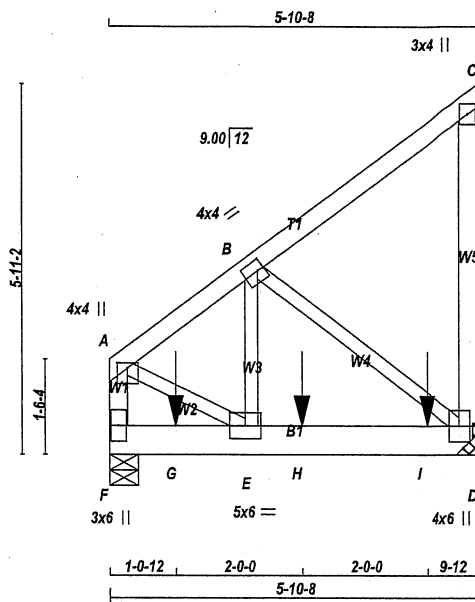


Structural component only
DWG# T-2204197 *m*

JOB NAME 417704	TRUSS NAME T18Z1	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
TRUSS DESC. HOMES					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - D	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQ'D	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
F	1897	0	1897	0	0	5-8	5-8		
D	2033	0	2033	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED						
F	1323	972 / 0	0 / 0	0 / 0	0 / 0	351 / 0	0 / 0
D	1418	1042 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 MAX CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-1564 / 0	0.0 0.0 0.09 (1)	7.81	A-E	0 / 1339	0.17 (1)	
A-B	-1472 / 0	-112.4 -112.4 0.10 (1)	6.25	E-B	0 / 1424	0.18 (1)	
B-C	-20 / 0	-112.4 -112.4 0.10 (1)	6.25	B-D	-1526 / 0	0.25 (1)	
D-C	-169 / 0	0.0 0.0 0.05 (1)	7.81				
F-G	0 / 0	-18.5 -18.5 0.11 (1)	10.00				
G-E	0 / 0	-18.5 -18.5 0.11 (1)	10.00				
E-H	0 / 1203	-18.5 -18.5 0.18 (1)	10.00				
H-I	0 / 1203	-18.5 -18.5 0.18 (1)	10.00				
I-D	0 / 1203	-18.5 -18.5 0.18 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-0-12	-734	-734	---	FRONT	VERT	TOTAL	---	C1
H	3-0-12	-734	-734	---	FRONT	VERT	TOTAL	---	C1
I	5-0-12	-735	-735	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.10/1.00 (A-B:1), BC=0.18/1.00 (D-E:1), WB=0.25/1.00 (B-D:1), SSI=0.41/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)

JSI METAL= 0.27 (D) (INPUT = 1.00)



Structural component only
DWG# T-2204198

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
417704	T18Z1	1	2	TRUSS DESC.	HOMES	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

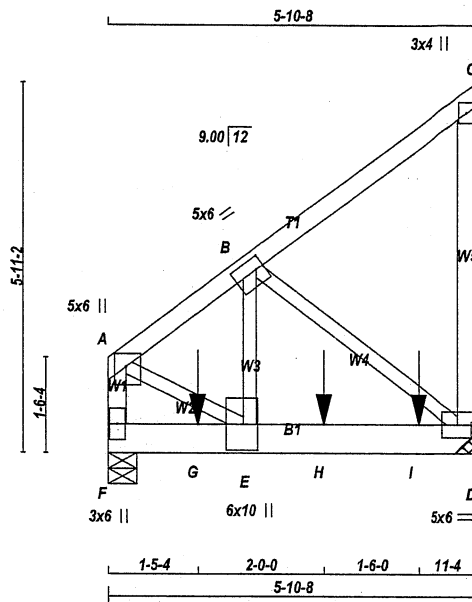
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2204198 *mr*

JOB NAME 417704	TRUSS NAME T18Z2	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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Scale = 1:35.2

TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.50	2.25
B	TMVW-t	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMVW-t	MT20	6.0	10.0	5.00	2.75
F	BMV1+p	MT20	3.0	6.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
F	3657	0	3657	0	0	5-8	5-8		
D	4494	0	4494	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
F	2557	1843 / 0	0 / 0	0 / 0	0 / 0	714 / 0	0 / 0	
D	3142	2266 / 0	0 / 0	0 / 0	0 / 0	876 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
F-A	-3415 / 0	0.0	0.0 0.19 (1)	A-E	0 / 2993
A-B	-3335 / 0	-112.4	-112.4 0.11 (1)	E-B	0 / 3715
B-C	-16 / 0	-112.4	-112.4 0.10 (1)	B-D	-3411 / 0
D-C	-176 / 0	0.0	0.0 0.05 (1)		
F-G	0 / 0	-18.5	-18.5 0.31 (1)		
G-E	0 / 0	-18.5	-18.5 0.31 (1)		
E-H	0 / 2689	-18.5	-18.5 0.49 (1)		
H-I	0 / 2689	-18.5	-18.5 0.49 (1)		
I-D	0 / 2689	-18.5	-18.5 0.49 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-5-4	-1720	-1720	---	TOP	VERT	TOTAL	---	C1
H	3-5-4	-1720	-1720	---	TOP	VERT	TOTAL	---	C1
I	4-11-4	-1720	-1720	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.19/1.00 (A-F:1), BC=0.49/1.00 (D-E:1), WB=0.56/1.00 (B-D:1), SSI=0.92/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

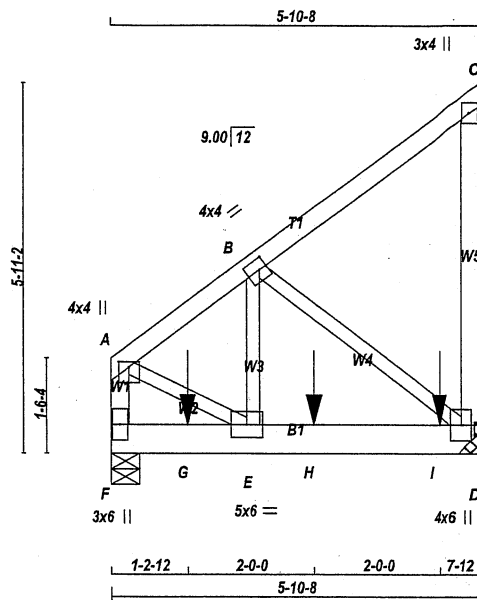
JSI GRIP= 0.73 (A) (INPUT = 0.90)
JSI METAL= 0.40 (E) (INPUT = 1.00)



Structural component only
DWG# T-2204199

JOB NAME 417704	TRUSS NAME T18Z4	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TMV+p	MT20	3.0	4.0	
D	BMVW1+p	MT20	4.0	6.0	
E	BMVW-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER								
BEARINGS								
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	1303	0	1303	0	0	5-8	5-8	
D	1508	0	1508	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	911	659 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
D	1054	762 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FORCE (LBS)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
F-A	-1130 / 0	0.0	0.0	0.06 (1)	A-E	0 / 951	0.12 (1)
A-B	-1034 / 0	-112.4	-112.4	0.10 (1)	E-B	0 / 886	0.11 (1)
B-C	-21 / 0	-112.4	-112.4	0.09 (1)	B-D	-1084 / 0	0.18 (1)
D-C	-168 / 0	0.0	0.0	0.05 (1)			
F-G	0 / 0	-18.5	-18.5	0.07 (1)			
G-E	0 / 0	-18.5	-18.5	0.07 (1)			
E-H	0 / 854	-18.5	-18.5	0.11 (1)			
H-I	0 / 854	-18.5	-18.5	0.11 (1)			
I-D	0 / 854	-18.5	-18.5	0.11 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-2-12	-475	-475	---	BACK	VERT	TOTAL	---	C1
H	3-2-12	-475	-475	---	BACK	VERT	TOTAL	---	C1
I	5-2-12	-477	-477	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	45.9	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.10/1.00 (A-B:1), BC=0.11/1.00 (D-E:1), WB=0.18/1.00 (B-D:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.


JSI GRIP= 0.56 (B) (INPUT = 0.90)

JSI METAL= 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2204201

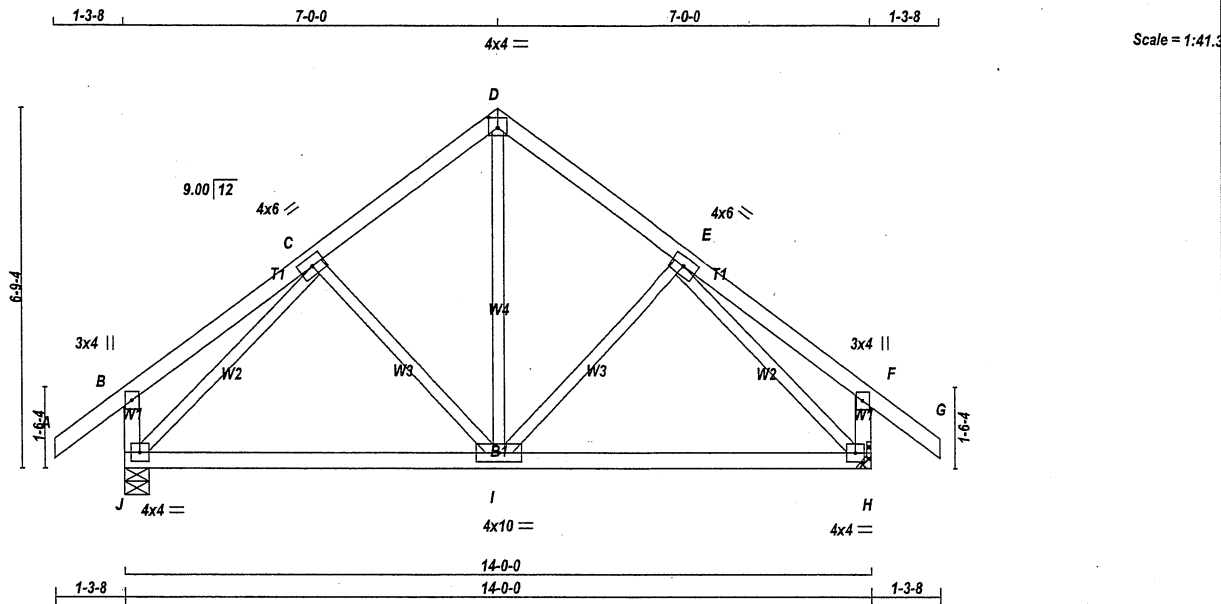
REVIEWED

JOB NAME 417704	TRUSS NAME T18Z4	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.														
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:41 2022 Page 2 ID:JvMhHdWEgsSI57rNnlib4czW6nI-QPvii52cZFuaocJFg5Lg47ihoesmVe0ev??VicznTs8																
<p>PLATES (table is in inches)</p> <table border="1"><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>F</td><td>BMV1+p</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr></tbody></table> <p>NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.</p>							JT	TYPE	PLATES	W	LEN	Y	X	F	BMV1+p	MT20	3.0	6.0		
JT	TYPE	PLATES	W	LEN	Y	X														
F	BMV1+p	MT20	3.0	6.0																
<div><p>Structural component only DWG# T-2204201 <i>W</i></p></div>																				

JOB NAME 417704	TRUSS NAME T19	QUANTITY 3	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 63 = 190 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	VERT	GROSS REACTION	BRG	BRG	TOP CH. LL =	32.5 PSF
D - G	2x4	DRY	No.2	SPF	DOWN	HORZ	DOWN	IN-SX	IN-SX	DL =	6.0 PSF
J - B	2x4	DRY	No.2	SPF	J	1072	0	1072	0	BOT CH. LL =	0.0 PSF
J - F	2x4	DRY	No.2	SPF	H	1072	0	1072	0	DL =	7.4 PSF
J - H	2x4	DRY	No.2	SPF						TOTAL LOAD =	45.9 PSF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3'-8".						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	4.0	10.0		
J	BMVW-t	MT20	4.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	748	544 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
H	748	544 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 47	-112.4	-112.4 0.15 (1)	I-D	0 / 465	0.10 (1)	10.00
B-C	0 / 27	-112.4	-112.4 0.22 (1)	I-E	-208 / 0	0.09 (1)	10.00
C-D	-669 / 0	-112.4	-112.4 0.17 (1)	C-I	-208 / 0	0.09 (1)	6.25
D-E	-669 / 0	-112.4	-112.4 0.17 (1)	J-C	-971 / 0	0.41 (1)	6.25
E-F	0 / 27	-112.4	-112.4 0.22 (1)	E-H	-971 / 0	0.41 (1)	10.00
F-G	0 / 47	-112.4	-112.4 0.15 (1)				10.00
J-B	-305 / 0	0.0	0.0 0.03 (1)				7.81
H-F	-305 / 0	0.0	0.0 0.03 (1)				7.81
J-I	0 / 655	-18.5	-18.5 0.30 (4)				10.00
I-H	0 / 655	-18.5	-18.5 0.30 (4)				10.00

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.22/1.00 (B-C:1), BC=0.30/1.00 (H-J:4), WB=0.41/1.00 (C-J:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

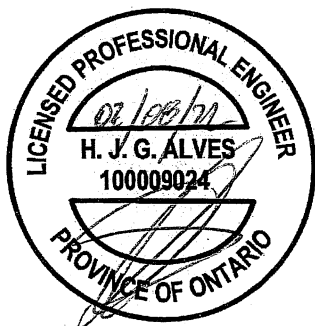
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (D) (INPUT = 0.90)
JSI METAL = 0.22 (H) (INPUT = 1.00)

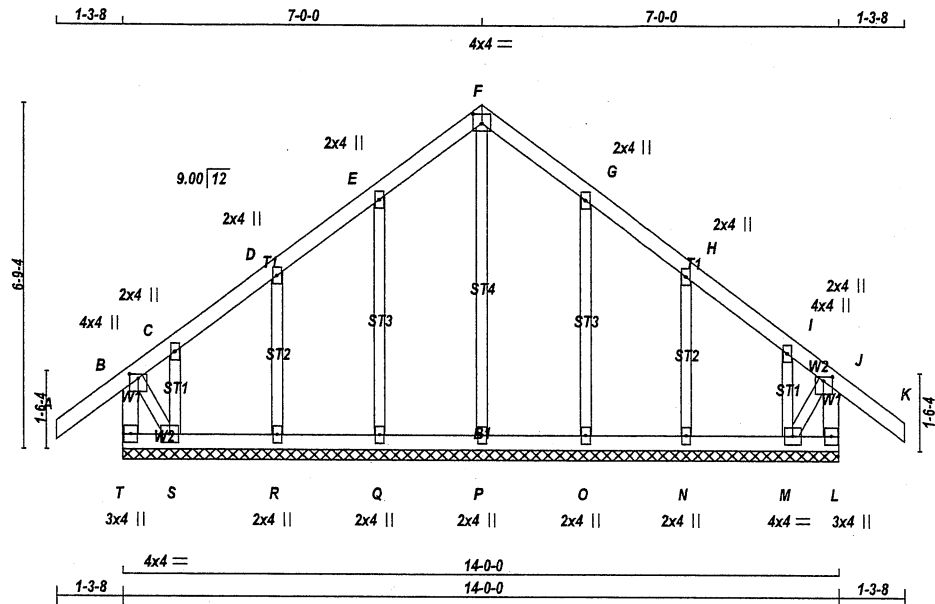


Structural component only
DWG# T-2204202

JOB NAME 417704	TRUSS NAME G19	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:06 2022 Page 1
ID:JvMhHdWEgsSI57rNnlib4czW6nl-xglUP7dtpVTY5S8X7FCT9llhnXx1op1Us209z8znTsh



Scale = 1:43.1

TOTAL WEIGHT = 65 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
T - B	2x4	DRY No.2
A - F	2x4	DRY No.2
F - K	2x4	DRY No.2
L - J	2x4	DRY No.2
T - L	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I					
C TMVW+w	MT20	2.0	4.0		
F TTW-p	MT20	4.0	4.0	2.25	2.00
J TMVW+p	MT20	4.0	4.0	1.00	2.00
L BMV1+p	MT20	3.0	4.0		
M BMVW1-t	MT20	4.0	4.0		
N, O, P, Q, R					
N BMV1+w	MT20	2.0	4.0		
S BMVW1-t	MT20	4.0	4.0		
T BMV1+p	MT20	3.0	4.0		

NOTES

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED			WEBS		
	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)
FR-TO			FROM TO	FR-TO		
T-B	-342 / 0	0.0	0.0	0.04 (1)	7.81	P-F -180 / 0
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00	Q-E -253 / 0
B-C	-83 / 0	-112.4	-112.4	0.15 (1)	6.25	R-D -228 / 0
C-D	-13 / 0	-112.4	-112.4	0.05 (1)	6.25	S-C -51 / 0
D-E	-11 / 0	-112.4	-112.4	0.06 (1)	6.25	O-G -253 / 0
E-F	-21 / 0	-112.4	-112.4	0.06 (1)	6.25	N-H -228 / 0
F-G	-11 / 0	-112.4	-112.4	0.06 (1)	6.25	M-I -51 / 0
G-H	-11 / 0	-112.4	-112.4	0.06 (1)	6.25	B-S 0 / 29
H-I	-13 / 0	-112.4	-112.4	0.05 (1)	6.25	M-J 0 / 29
I-J	-83 / 0	-112.4	-112.4	0.15 (1)	6.25	
J-K	0 / 47	-112.4	-112.4	0.15 (1)	10.00	
L-J	-342 / 0	0.0	0.0	0.04 (1)	7.81	
T-S	0 / 0	-18.5	-18.5	0.01 (4)	10.00	
S-R	0 / 16	-18.5	-18.5	0.02 (4)	10.00	
R-Q	0 / 10	-18.5	-18.5	0.02 (4)	10.00	
Q-P	0 / 6	-18.5	-18.5	0.01 (4)	10.00	
P-O	0 / 6	-18.5	-18.5	0.01 (4)	10.00	
O-N	0 / 10	-18.5	-18.5	0.02 (4)	10.00	
N-M	0 / 16	-18.5	-18.5	0.02 (4)	10.00	
M-L	0 / 0	-18.5	-18.5	0.01 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015.

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (R-S:4), WB=0.13/1.00 (F-P:1), SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

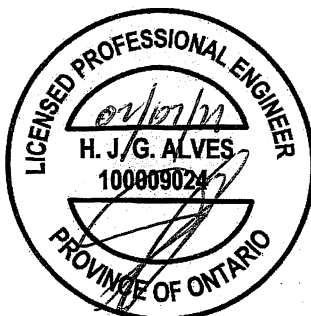
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (J) (INPUT = 0.90)

JSI METAL= 0.13 (E) (INPUT = 1.00)

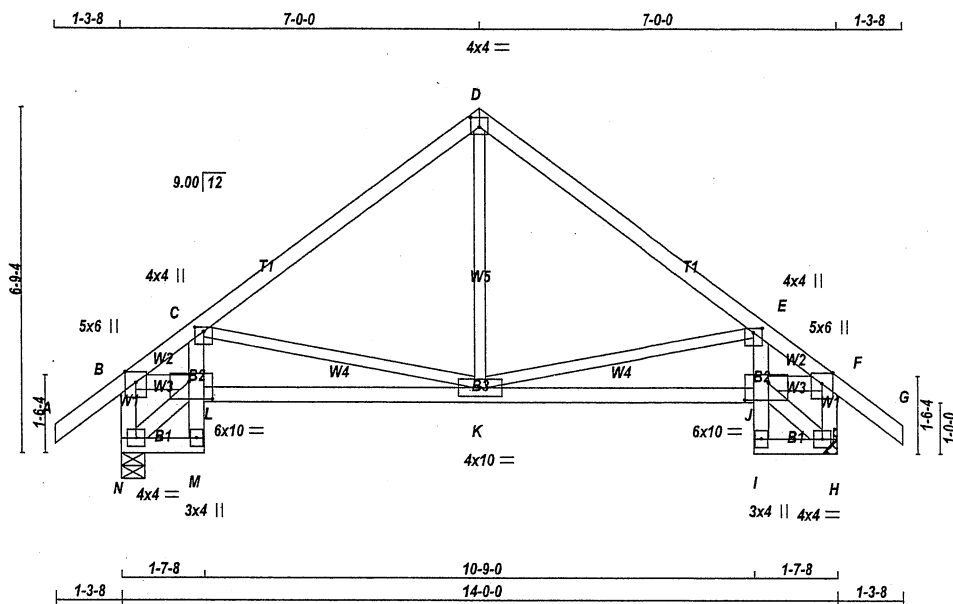


Structural component only
DWG# T-2204172

JOB NAME 417701	TRUSS NAME T19S	QUANTITY 3	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:35:14 2022 Page 1
ID:JvMhHdWEgsSI57rNnlib4czW6nl-0XQHozOtiYIFGP8tBIDX64OI15PanoVe8JIS13znGTh



TOTAL WEIGHT = 3 X 66 = 198 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
K - D	2x3	DRY	No.2	SPF
K - E	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TMVW+p	MT20	4.0	4.0	1.00 2.00	
D TTW-p	MT20	4.0	4.0	2.25 2.00	
E TMVW+p	MT20	4.0	4.0	1.00 2.00	
F TMVW+p	MT20	5.0	6.0	Edge	
H BMVW1-t	MT20	4.0	4.0		
I BMV-p	MT20	3.0	4.0		
J BVMVW1-t	MT20	6.0	10.0	4.00 5.50	
K BVMVW1-t	MT20	4.0	10.0		
L BVMVW1-t	MT20	6.0	10.0	4.00 5.50	
M BMV-p	MT20	3.0	4.0		
N BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
FACTORED				MAXIMUM FACTORED			INPUT	REQRD	
GROSS REACTION				GROSS REACTION			BRG	BRG	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
N	1072	0		1072	0	0	5-8	5-8	
H	1072	0		1072	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	748	544 / 0	0 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
H	748	544 / 0	0 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO				FR-TO			
A-B	0 / 47	-112.4	-112.4 0.15 (1)	K-D	0 / 360	0.08 (1)	
B-C	-1422 / 0	-112.4	-112.4 0.33 (1)	K-E	-677 / 0	0.37 (1)	
C-D	-780 / 0	-112.4	-112.4 0.41 (1)	K-F	-677 / 0	0.37 (1)	
D-E	-780 / 0	-112.4	-112.4 0.41 (1)	N-L	-52 / 0	0.01 (1)	
E-F	-1422 / 0	-112.4	-112.4 0.33 (1)	B-L	0 / 1199	0.19 (1)	
F-G	0 / 47	-112.4	-112.4 0.15 (1)	J-H	-52 / 0	0.01 (1)	
N-B	-1026 / 0	0.0	0.0 0.11 (1)	J-F	0 / 1199	0.19 (1)	
H-F	-1026 / 0	0.0	0.0 0.11 (1)				
N-M	0 / 41	-18.5	-18.5 0.01 (4)				
M-L	0 / 15	0.0	0.0 0.07 (1)				
L-C	0 / 56	0.0	0.0 0.07 (1)				
L-K	0 / 1241	-18.5	-18.5 0.29 (1)				
K-J	0 / 1241	-18.5	-18.5 0.29 (1)				
I-J	0 / 15	0.0	0.0 0.07 (1)				
J-E	0 / 56	0.0	0.0 0.07 (1)				
I-H	0 / 41	-18.5	-18.5 0.01 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.41/1.00 (C-D:1), BC=0.29/1.00 (K-L:1), WB=0.37/1.00 (C-K:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(FSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.72 (L) (INPUT = 0.90)
JSI METAL = 0.55 (F) (INPUT = 1.00)

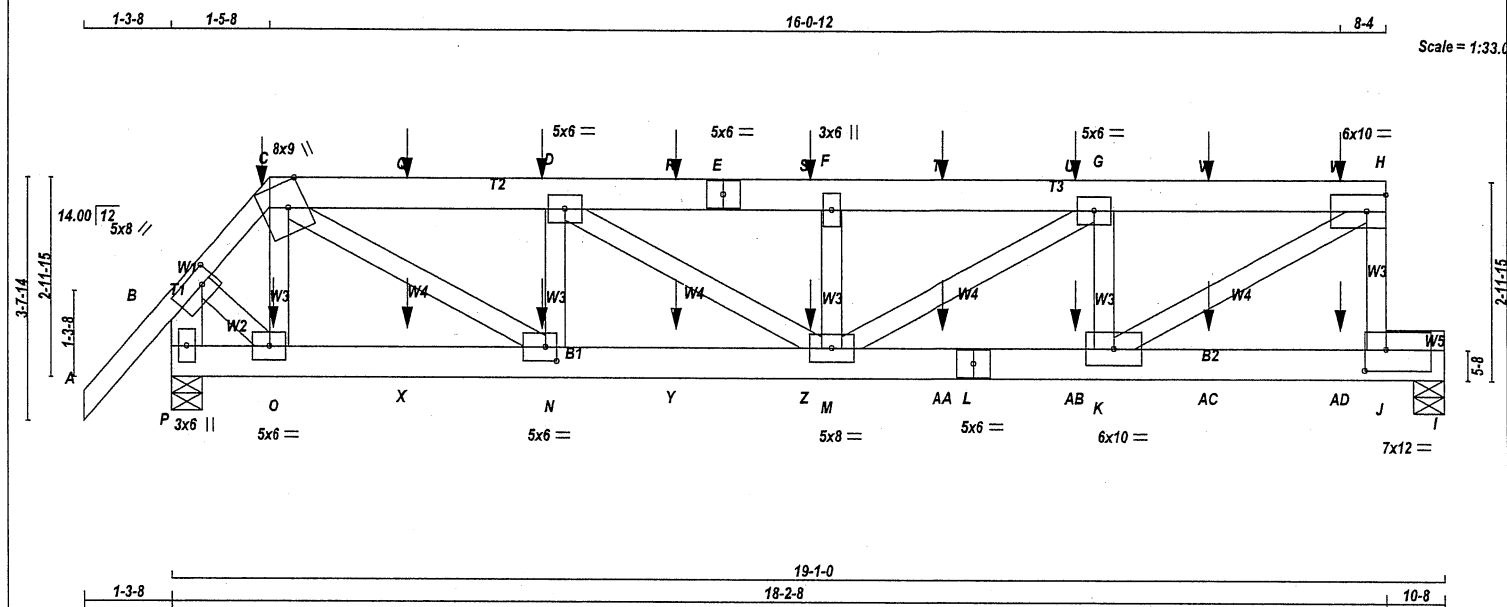


Structural component only
DWG# T-2204209

JOB NAME 417704	TRUSS NAME T20	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:43 2022 Page 1
ID:JvMhHdWEgSIS157rNnlib4czW6nl-Mn1TAn3s5t8l18TeoWN89Ynz9SOHxPfxNIUcmUznTs6



TOTAL WEIGHT = 105 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - E	2x6	DRY	No.2
E - H	2x6	DRY	No.2
J - I	2x4	DRY	No.2
P - B	2x6	DRY	No.2
P - L	2x6	DRY	2100F 1.8E
L - I	2x6	DRY	2100F 1.8E
J - I	2x4	DRY	No.2

ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	8.0	2.50	2.50
C TTWW+m	MT20	8.0	9.0	4.50	3.25
D TMVW-t	MT20	5.0	6.0		
E TS-t	MT20	5.0	6.0		
F TMVW+w	MT20	3.0	6.0		
G TMVW-t	MT20	5.0	6.0		
H TMVW-t	MT20	6.0	10.0	Edge	
J BMVW-t	MT20	7.0	12.0	3.75	3.75
K BMVW-t	MT20	6.0	10.0		
L BS-t	MT20	5.0	6.0		
M BMVW-t	MT20	5.0	8.0		
N BMVW-t	MT20	5.0	6.0	2.50	2.00
O BMVW-t	MT20	5.0	6.0		
P BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2388	0	2388	0	0	5-8	5-8
I	2133	0	2133	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1672	1195 / 0	0 / 0	0 / 0	0 / 0	0 / 0	477 / 0	0 / 0
I	1497	1051 / 0	0 / 0	0 / 0	0 / 0	0 / 0	446 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 60	-112.4 -112.4	0.18 (1)	10.00	O-C	-391 / 0	0.05 (1)
B-C	-1997 / 0	-112.4 -112.4	0.22 (1)	4.53	B-O	0 / 1425	0.25 (1)
C-Q	-3661 / 0	-112.4 -112.4	0.31 (1)	4.17	K-H	0 / 4218	0.75 (1)
Q-D	-3661 / 0	-112.4 -112.4	0.31 (1)	4.17	C-N	0 / 2816	0.50 (1)
D-R	-4263 / 0	-112.4 -112.4	0.28 (1)	3.95	K-G	-1248 / 0	0.16 (1)
R-E	-4263 / 0	-112.4 -112.4	0.28 (1)	3.95	N-D	-1225 / 0	0.16 (1)
E-S	-4263 / 0	-112.4 -112.4	0.28 (1)	3.95	M-G	0 / 748	0.13 (1)
S-F	-4263 / 0	-112.4 -112.4	0.28 (1)	3.95	D-M	0 / 707	0.13 (1)
F-T	-4263 / 0	-112.4 -112.4	0.28 (1)	3.94	M-F	-723 / 0	0.09 (1)
T-U	-4263 / 0	-112.4 -112.4	0.28 (1)	3.94			
U-G	-4263 / 0	-112.4 -112.4	0.28 (1)	3.94			
G-V	-3625 / 0	-112.4 -112.4	0.32 (1)	4.17			
V-W	-3625 / 0	-112.4 -112.4	0.32 (1)	4.17			
W-H	-3625 / 0	-112.4 -112.4	0.32 (1)	4.17			
J-H	-2534 / 0	0.0	0.0	5.33			
P-B	-2447 / 0	0.0	0.0	6.56			

P-O	0 / 0	-18.5	-18.5	0.05 (1)	10.00
O-X	0 / 1241	-18.5	-18.5	0.11 (1)	10.00
X-N	0 / 1241	-18.5	-18.5	0.11 (1)	10.00
N-Y	0 / 3661	-18.5	-18.5	0.28 (1)	10.00
Y-Z	0 / 3661	-18.5	-18.5	0.28 (1)	10.00
Z-M	0 / 3661	-18.5	-18.5	0.28 (1)	10.00
M-AA	0 / 3625	-18.5	-18.5	0.36 (1)	10.00
AA-L	0 / 3625	-18.5	-18.5	0.36 (1)	10.00
L-AB	0 / 3625	-18.5	-18.5	0.36 (1)	10.00
AB-K	0 / 3625	-18.5	-18.5	0.36 (1)	10.00
K-AC	0 / 0	-18.5	-18.5	0.75 (1)	10.00
AC-AD	0 / 0	-18.5	-18.5	0.75 (1)	10.00
AD-J	0 / 0	-18.5	-18.5	0.75 (1)	10.00
J-I	0 / 0	-18.5	-18.5	0.70 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-5-8	-107	-107	---	BACK	VERT	TOTAL	---	C1
D	5-6-4	-101	-101	---	BACK	VERT	TOTAL	---	C1
N	5-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1
O	1-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1
Q	3-6-4	-101	-101	---	BACK	VERT	TOTAL	---	C1
R	7-6-4	-101	-101	---	BACK	VERT	TOTAL	---	C1
S	9-6-4	-101	-101	---	BACK	VERT	TOTAL	---	C1
T	11-6-4	-101	-101	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL)= L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL)= L/917 (0.25")

CSI: TC=0.36/1.00 (H-J:1), BC=0.75/1.00 (J-K:1), WB=0.75/1.00 (H-K:1), SSI=0.86/1.00 (J-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (N) (INPUT = 0.90)
JSI METAL= 0.67 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2204203

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
417704	T20	1	1	TRUSS DESC.	HOMES	

Tamarack Roof Truss, Burlington

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ID:JvMhHdWEqsSI57rNnlib4czW6nl-Mn1TAn3s5t818TcoWN89Ynz9SOHzPfxNIUcmUznTs6

SPECIFIED CONCENTRATED LOADS (LBS)

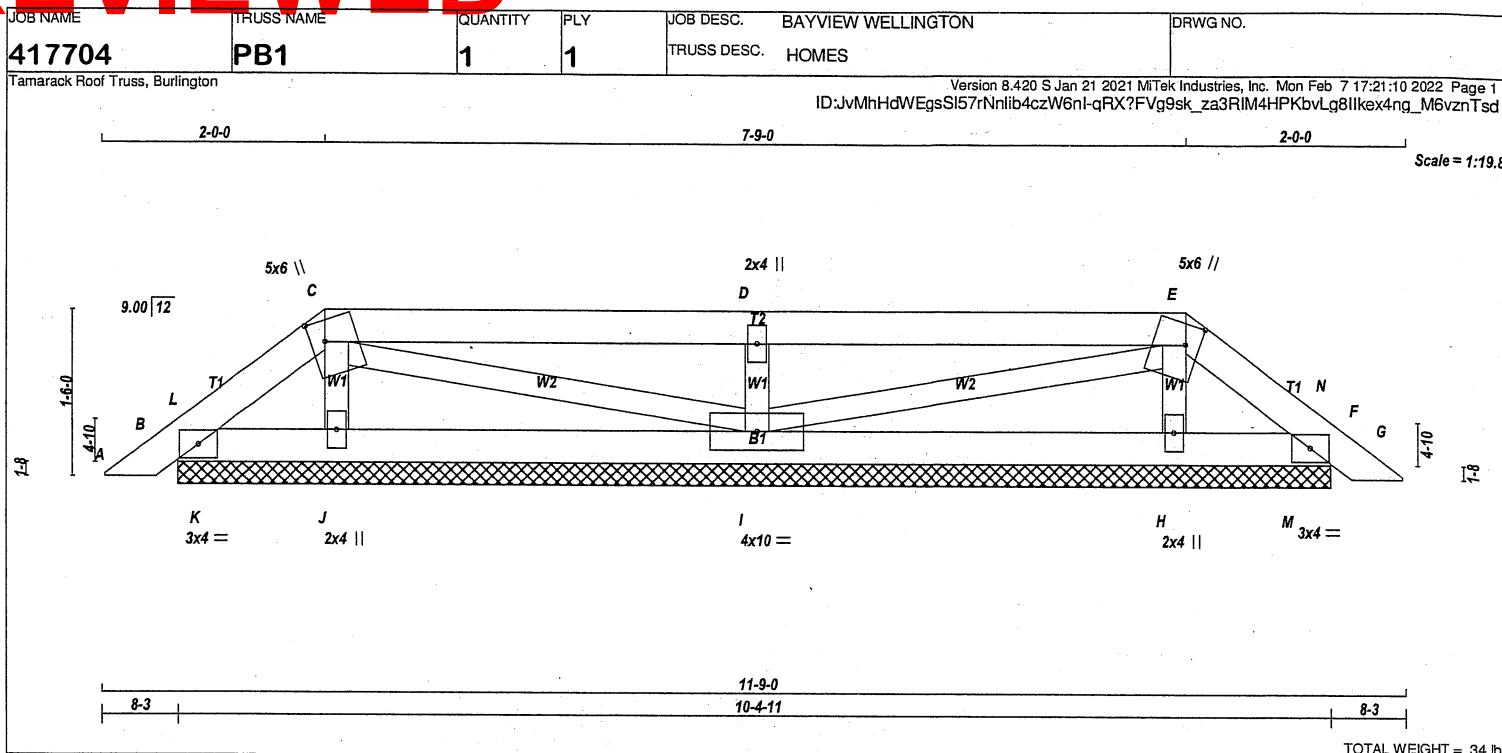
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	13-6-4	-101	-101	---	BACK	VERT	TOTAL	---	C1
V	15-6-4	-101	-101	---	BACK	VERT	TOTAL	---	C1
W	17-6-4	-113	-113	---	BACK	VERT	TOTAL	---	C1
X	3-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1
Y	7-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1
Z	9-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1
AA	11-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1
AB	13-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1
AC	15-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1
AD	17-6-4	-50	-50	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2204203 1/2



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS		
A - C	2x4	DRY	No.2	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG
C - E	2x4	DRY	No.2	SPF	VERT	DOWN	BRG
E - G	2x4	DRY	No.2	SPF	HORZ	HORZ	IN-SX
B - F	2x4	DRY	No.2	SPF		UPLIFT	IN-SX
ALL WEBS	2x3	DRY	No.2	SPF	JT		
DRY: SEASONED LUMBER.					B	170 0	10-4-11 10-4-11
					F	170 0	10-4-11 10-4-11
					J	263 0	10-4-11 10-4-11
					I	624 0	10-4-11 10-4-11
					H	263 0	10-4-11 10-4-11

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTWW+w	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMB1-I	MT20	3.0	4.0	
H	BMW1+w	MT20	2.0	4.0	
I	BMWW1-t	MT20	4.0	10.0	
J	BMW1+w	MT20	2.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS						
1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
B	116 98/0	0/0	0/0	0/0	19/0	0/0
F	116 98/0	0/0	0/0	0/0	19/0	0/0
J	187 118/0	0/0	0/0	0/0	68/0	0/0
I	435 318/0	0/0	0/0	0/0	118/0	0/0
H	187 118/0	0/0	0/0	0/0	68/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX CSI (LC)	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/18	-112.4 -112.4	0.03 (1)	J-C	-187/0	0.03 (1)	
B-L	-43/0	-112.4 -112.4	0.01 (1)	C-I	-15/0	0.00 (1)	
L-C	-50/0	-112.4 -112.4	0.02 (1)	I-D	-544/0	0.08 (1)	
C-D	-5/0	-112.4 -112.4	0.29 (1)	D-E	-15/0	0.00 (1)	
D-E	-5/0	-112.4 -112.4	0.29 (1)	E-N	-187/0	0.03 (1)	
E-N	-50/0	-112.4 -112.4	0.02 (1)	K-L	-85/0	0.00 (1)	
N-F	-43/0	-112.4 -112.4	0.01 (1)	M-N	-85/0	0.00 (1)	
F-G	0/18	-112.4 -112.4	0.03 (1)				
B-K	0/38	-18.5 -18.5	0.03 (1)				
K-J	0/38	-18.5 -18.5	0.04 (4)				
J-I	0/20	-18.5 -18.5	0.06 (4)				
I-H	0/20	-18.5 -18.5	0.06 (4)				
H-M	0/38	-18.5 -18.5	0.04 (4)				
M-F	0/38	-18.5 -18.5	0.03 (1)				

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1), BC=0.06/1.00 (I-J:4), WB=0.08/1.00 (D-I:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

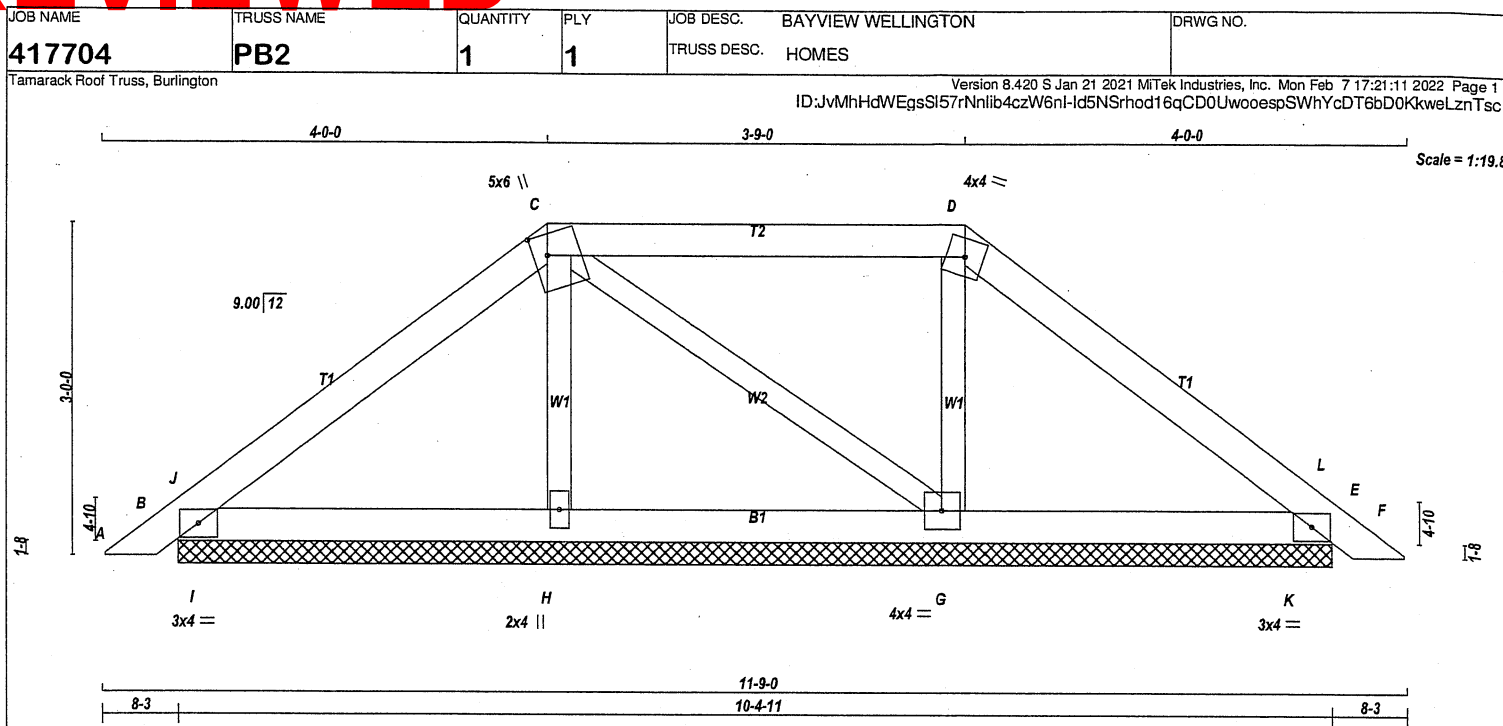
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

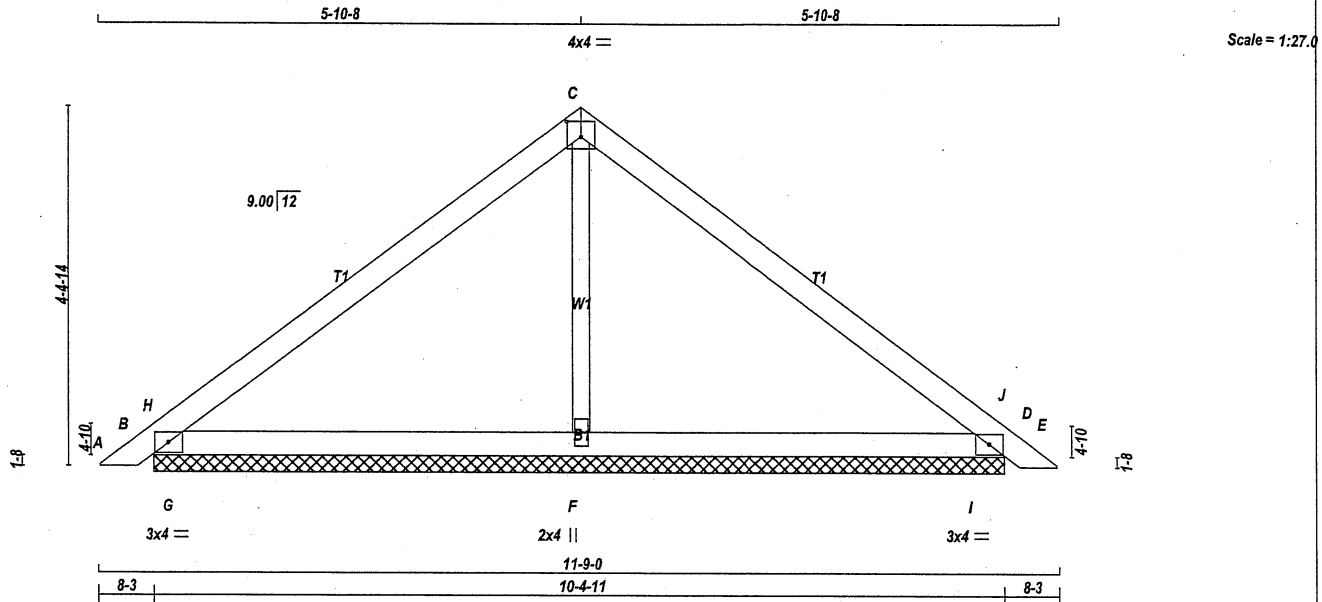


Structural component only
DWG# T-2204175



JOB NAME 417704	TRUSS NAME PB3	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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TOTAL WEIGHT = 2 X 31 = 63 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1-w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ	
B	535	0	535	0	10-4-11
D	535	0	535	0	10-4-11
F	419	0	419	0	10-4-11

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	372	282 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
D	372	282 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
F	298	186 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 18	-112.4	-112.4	0.03 (1)	10.00	F-C	-162 / 0
B-H	-41 / 113	-112.4	-112.4	0.24 (1)	6.25	G-H	-878 / 0
H-C	-292 / 0	-112.4	-112.4	0.37 (1)	6.25	I-J	-878 / 0
C-J	-292 / 0	-112.4	-112.4	0.37 (1)	6.25		
J-D	-41 / 113	-112.4	-112.4	0.24 (1)	6.25		
D-E	0 / 18	-112.4	-112.4	0.03 (1)	10.00		
B-G	0 / 217	-18.5	-18.5	0.35 (1)	10.00		
G-F	0 / 217	-18.5	-18.5	0.35 (1)	10.00		
F-I	0 / 217	-18.5	-18.5	0.35 (1)	10.00		
I-D	0 / 217	-18.5	-18.5	0.35 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (C-J:1), BC=0.35/1.00 (D-I:1), WB=0.04/1.00 (C-F:1), SSI=0.66/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

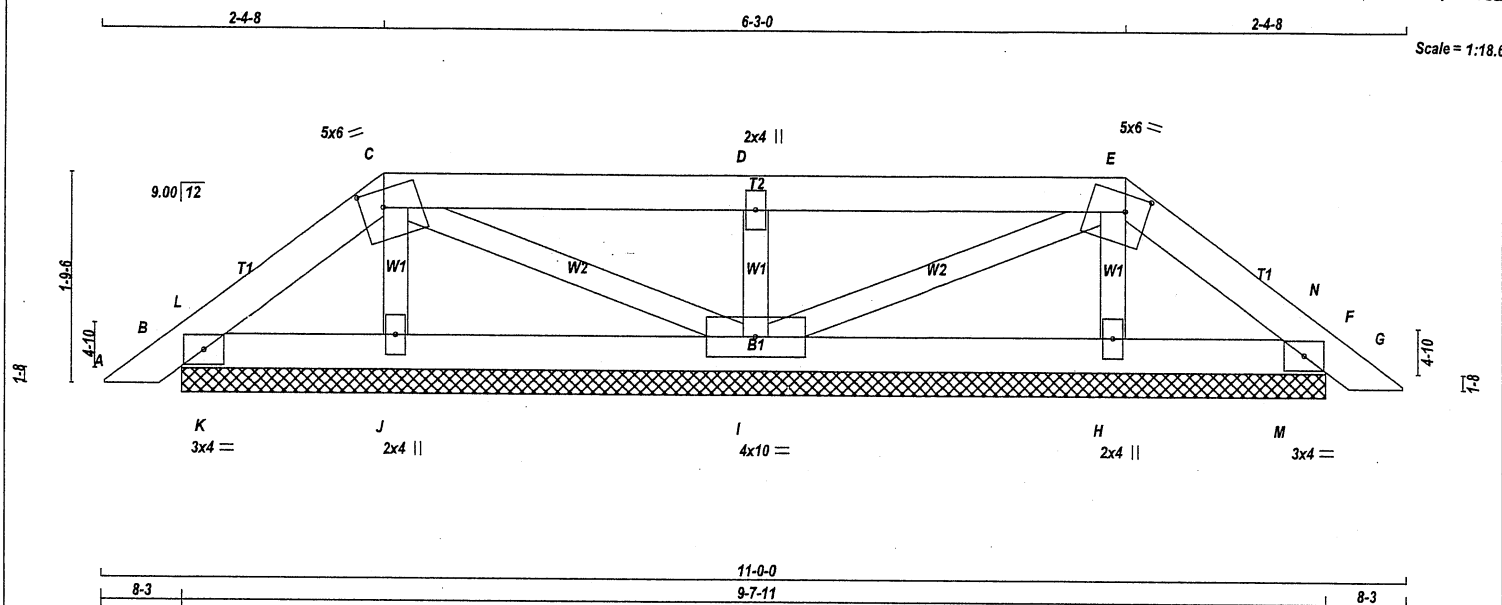
JSI GRIP= 0.46 (D) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



Structural component only
DWG# T-2204177

JOB NAME 417704	TRUSS NAME PB4	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
Tamarack Roof Truss, Burlington						

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TOTAL WEIGHT = 32 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	DOWN	GROSS REACTION	BRG	BRG	TOP CH. LL = 32.5 PSF	
C - E	2x4	DRY	No.2	SPF	VERT	0	DOWN	IN-SX	IN-SX	DL = 6.0 PSF	
E - G	2x4	DRY	No.2	SPF	HORZ	0	HORZ	9-7-11	9-7-11	BOT CH. LL = 0.0 PSF	
B - F	2x4	DRY	No.2	SPF				9-7-11	9-7-11	DL = 7.4 PSF	
ALL WEBS	2x3	DRY	No.2	SPF				9-7-11	9-7-11	TOTAL LOAD = 45.9 PSF	
DRY: SEASONED LUMBER.								9-7-11	9-7-11		

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.25
D	TTWW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	1.75	2.25
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMW1+w	MT20	4.0	10.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	139	110 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
F	139	110 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
J	168	109 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
I	358	262 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
H	168	109 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 18	-112.4 -112.4 0.03 (1)	10.00	J-C	-166 / 0	0.02 (1)	
B-L	-47 / 0	-112.4 -112.4 0.01 (1)	6.25	C-I	-28 / 0	0.01 (1)	
L-C	-56 / 0	-112.4 -112.4 0.04 (1)	6.25	I-D	-438 / 0	0.06 (1)	
C-D	-5 / 0	-112.4 -112.4 0.19 (1)	10.00	D-E	-28 / 0	0.01 (1)	
D-E	-5 / 0	-112.4 -112.4 0.19 (1)	10.00	E-H	-166 / 0	0.02 (1)	
E-N	-56 / 0	-112.4 -112.4 0.04 (1)	6.25	K-L	-109 / 0	0.00 (1)	
N-F	-47 / 0	-112.4 -112.4 0.01 (1)	6.25	M-N	-109 / 0	0.00 (1)	
F-G	0 / 18	-112.4 -112.4 0.03 (1)	10.00				
B-K	0 / 43	-18.5 -18.5 0.04 (1)	10.00				
K-J	0 / 43	-18.5 -18.5 0.04 (1)	10.00				
J-I	0 / 30	-18.5 -18.5 0.04 (1)	10.00				
I-H	0 / 30	-18.5 -18.5 0.04 (1)	10.00				
H-M	0 / 43	-18.5 -18.5 0.04 (1)	10.00				
M-F	0 / 43	-18.5 -18.5 0.04 (1)	10.00				

CSI: TC=0.19/1.00 (D-E:1), BC=0.04/1.00 (J-K:1), WB=0.06/1.00 (D-I:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

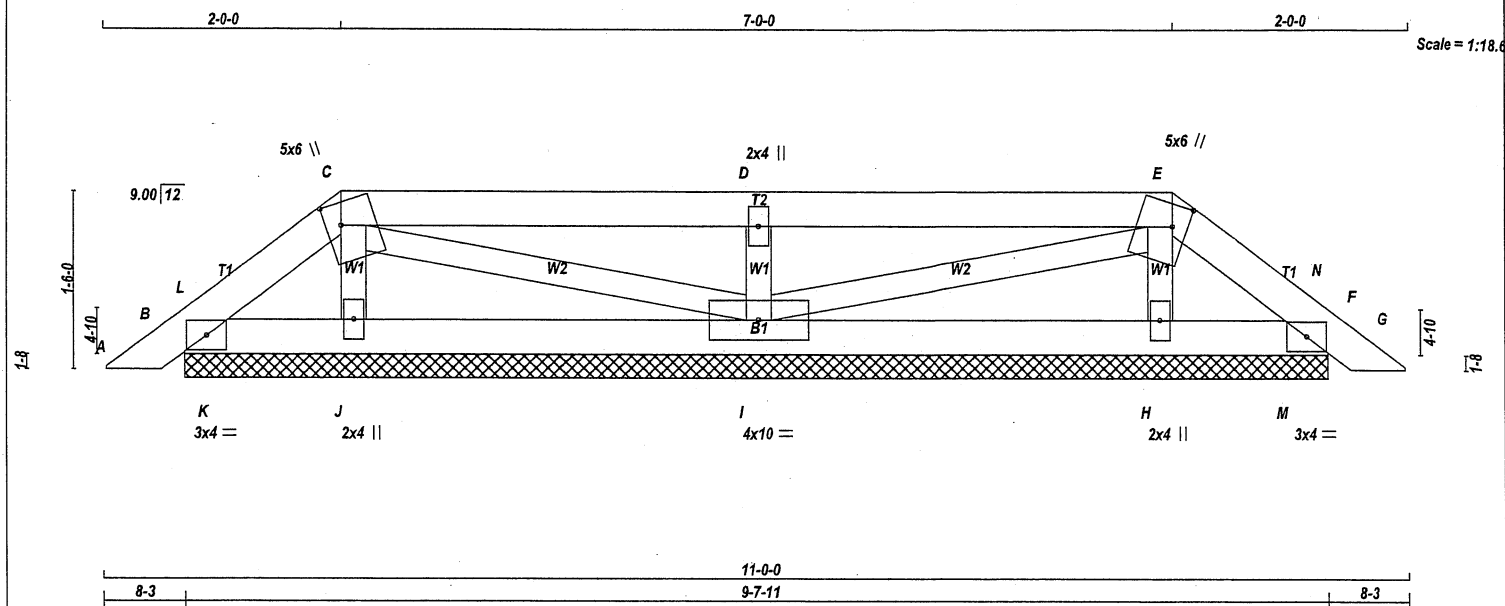
JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



Structural component only
DWG# T-2204178

JOB NAME 417704	TRUSS NAME PB5	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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TOTAL WEIGHT = 32 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTWW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-I	MT20	4.0	10.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES

- (1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	171	0	171	0	9-7-11	9-7-11
F	171	0	171	0	9-7-11	9-7-11
J	242	0	242	0	9-7-11	9-7-11
I	565	0	565	0	9-7-11	9-7-11
H	242	0	242	0	9-7-11	9-7-11

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	117	97/0	0/0	0/0	0/0	21/0	0/0
F	117	97/0	0/0	0/0	0/0	21/0	0/0
J	172	110/0	0/0	0/0	0/0	62/0	0/0
I	394	288/0	0/0	0/0	0/0	106/0	0/0
H	172	110/0	0/0	0/0	0/0	62/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 18	-112.4 -112.4	0.03 (1)	J-C	-172 / 0	0.02 (1)	
B-L	-43 / 0	-112.4 -112.4	0.00 (1)	C-I	-16 / 0	0.00 (1)	
L-C	-48 / 0	-112.4 -112.4	0.02 (1)	I-D	-491 / 0	0.07 (1)	
C-D	-4 / 0	-112.4 -112.4	0.23 (1)	D-E	-16 / 0	0.00 (1)	
D-E	-4 / 0	-112.4 -112.4	0.23 (1)	E-H	-172 / 0	0.02 (1)	
E-N	-48 / 0	-112.4 -112.4	0.02 (1)	K-L	-81 / 0	0.00 (1)	
N-F	-43 / 0	-112.4 -112.4	0.00 (1)	M-N	-81 / 0	0.00 (1)	
F-G	0 / 18	-112.4 -112.4	0.03 (1)				
B-K	0 / 37	-18.5 -18.5	0.03 (1)				
K-J	0 / 37	-18.5 -18.5	0.03 (4)				
J-I	0 / 20	-18.5 -18.5	0.05 (4)				
I-H	0 / 20	-18.5 -18.5	0.05 (4)				
H-M	0 / 37	-18.5 -18.5	0.03 (4)				
M-F	0 / 37	-18.5 -18.5	0.03 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS.				
TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1), BC=0.05/1.00 (I-J:4), WB=0.07/1.00 (D-I:1), SS=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

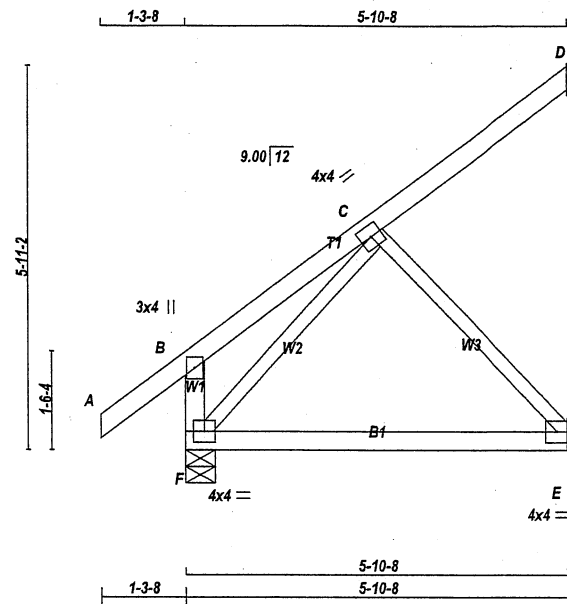
JSI GRIP = 0.25 (D) (INPUT = 0.90)
JSI METAL = 0.10 (D) (INPUT = 1.00)



Structural component only
DWG# T-2204179

JOB NAME 417704	TRUSS NAME J1	QUANTITY 21	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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TOTAL WEIGHT = 21 X 25 = 527 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS		No.2		SPF
F - B	2x4	DRY		SPF
A - D	2x4	DRY		SPF
F - E	2x4	DRY		SPF
ALL WEBS	2x3	DRY		SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	4.0	
E	BMV1-t	MT20	4.0	4.0	Edge
F	BMVW1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	DOWN	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
F	537	0	0	537	0	0	5-8	5-8	5-8
D	134	0	0	134	0	0	1-8	1-8	1-8
E	252	0	0	252	0	0	1-8	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	373	279 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
D	92	77 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
E	179	115 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS		FACTORED			
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC
FR-TO		FROM	TO		LENGTH	FR-TO		FROM	TO		LENGTH
F-B	-272 / 0	0.0	0.0	0.03 (1)	7.81	C-E	-268 / 0	0.09 (1)			
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00	F-C	-277 / 0	0.08 (1)			
B-C	0 / 25	-112.4	-112.4	0.16 (1)	10.00						
C-D	-23 / 0	-112.4	-112.4	0.16 (1)	6.25						
F-E	0 / 180	-18.5	-18.5	0.19 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 45.9 PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.16/1.00 (B-C:1), BC=0.19/1.00 (E-F:4), WB=0.09/1.00 (C-E:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

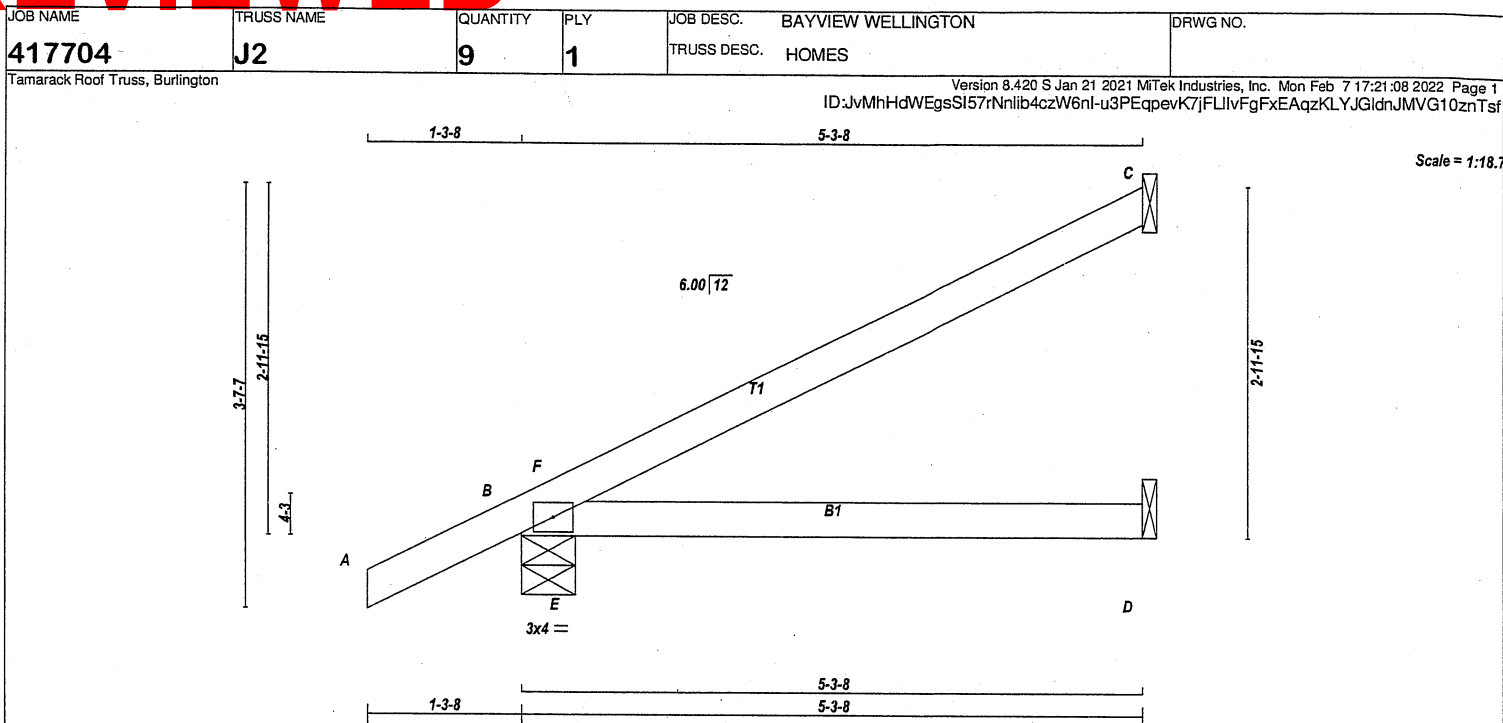
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (C) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2204173



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF	SPF	SPECIFIED LOADS:	TOP CH.	LL	PSF
A - C	2x4	DRY	No.2					DL = 6.0	PSF		
B - D	2x4	DRY	No.2					BOT CH.	LL	0.0	PSF
DRY: SEASONED LUMBER.								DL = 7.4	PSF		
								TOTAL LOAD	=	45.9	PSF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
C	178	146 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	347	260 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	65	26 / 0	0 / 0	0 / 0	39 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 33	-112.4 -112.4	0.15 (1)	10.00	E-F	-504 / 9	0.00 (1)
B-F	-25 / 97	-112.4 -112.4	0.12 (1)	6.25			
F-C	-7 / 2	-112.4 -112.4	0.41 (1)	10.00			
B-E	0 / 0	-18.5 -18.5	0.28 (1)	10.00			
E-D	0 / 0	-18.5 -18.5	0.28 (1)	10.00			

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/574 (0.11")

CSI: TC=0.41/1.00 (C-F:1), BC=0.28/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.40/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

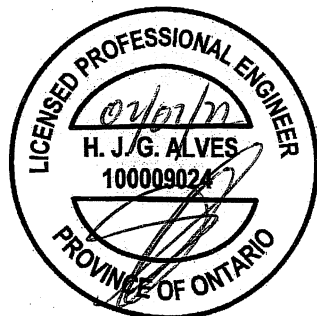
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

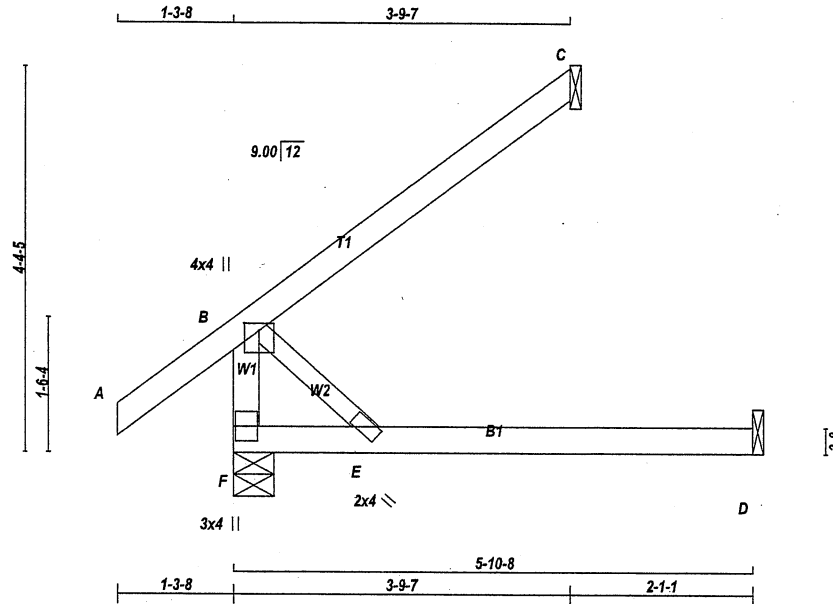
JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2204174

JOB NAME 417704	TRUSS NAME C1	QUANTITY 5	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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TOTAL WEIGHT = 5 X 17 = 84 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
F	422	0	422	0
C	213	0	213	0
D	54	0	61	0

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED							
F	295	213 / 0	0 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	146	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED HORIZ. LOAD (LC2)	UNBRACED LENGTH FR-TO	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC2)
F-B		-368 / 0	0.0	0.0	0.04 (1)	7.81	B-E		0 / 0	0.00 (1)
A-B		0 / 47	-112.4	-112.4	0.15 (1)	10.00				
B-C		0 / 0	-112.4	-112.4	0.27 (1)	10.00				
F-E		0 / 0	-18.5	-18.5	0.14 (4)	10.00				
E-D		0 / 0	-18.5	-18.5	0.19 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.27/1.00 (B-C:1), BC=0.19/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

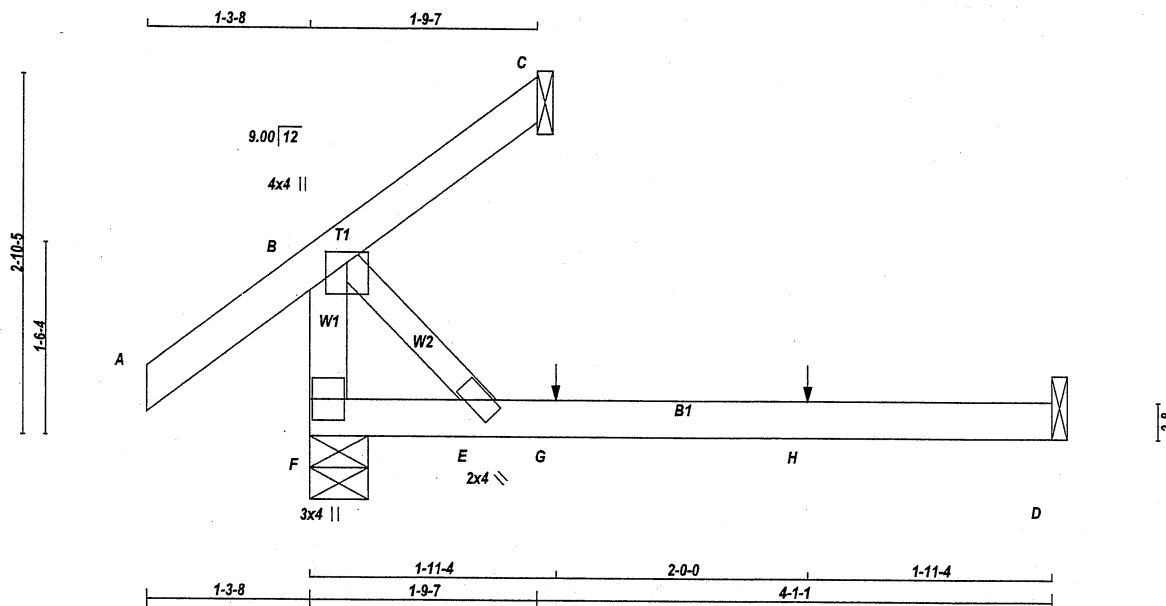


Structural component only
DWG# T-2204167

JOB NAME 417704	TRUSS NAME C2	QUANTITY 5	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. HOMES	

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ID:JvMhHdWEgsSI57rNnlib4czW6nl-X5cLn6bmWa5zE_PyS6fmX77AXKtebUO2A4oVMpznTsk

Scale = 1:17.4



TOTAL WEIGHT = 5 X 14 = 69 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			SPF
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	370	0	370	0	5-8	5-8
C	41	0	41	0	1-8	1-8
D	54	0	61	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) F, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	259	182 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	28	23 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
F-B	-315 / 0	0.0 0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 47	-112.4 -112.4	0.15 (1)	10.00			
B-C	-36 / 0	-112.4 -112.4	0.15 (1)	6.25			
F-E	0 / 0	-18.5 -18.5	0.13 (4)	10.00			
E-G	0 / 0	-18.5 -18.5	0.19 (4)	10.00			
G-H	0 / 0	-18.5 -18.5	0.19 (4)	10.00			
H-D	0 / 0	-18.5 -18.5	0.19 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.15/1.00 (A-B:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	MAX MIN	MAX MIN	MAX MIN
	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

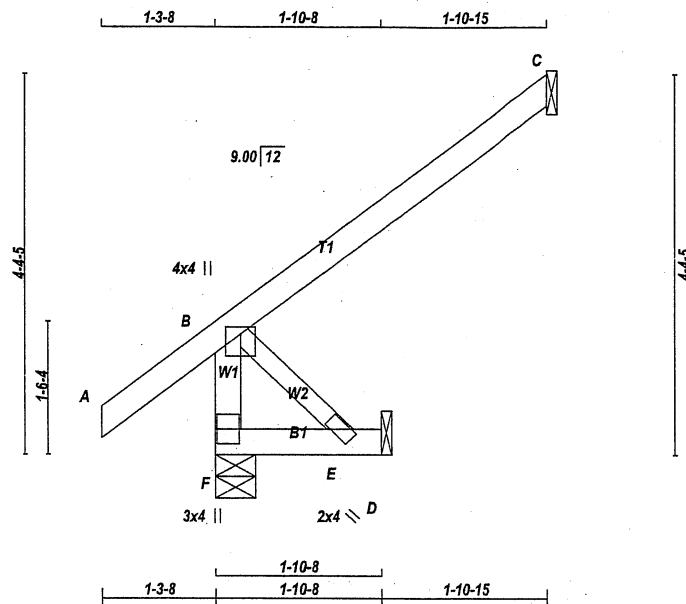


Structural component only
DWG# T-2204168

JOB NAME 417704	TRUSS NAME C3	QUANTITY 5	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:JvMhHdWEgsSI57rNnlib4czW6nl-?HAj_SbPhuQds8_80qA?4KfJNjFVKxeBPKX2uFznTsj



Scale = 1:25.2

TOTAL WEIGHT = 5 X 12 = 61 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

(1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
F	385	385	5-8	5-8
C	213	213	1-8	1-8
D	17	19	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
F	266	213 / 0	0 / 0	53 / 0	0 / 0
C	146	123 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED VERT. LOAD LC1 (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LC)
FR-TO					FR-TO		
F-B	-368 / 0	0.0	0.0	0.04 (1)	7.81	0 / 0	0.00 (1)
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00		
B-C	0 / 0	-112.4	-112.4	0.27 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	6.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.4	PSF
TOTAL LOAD	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.27/1.00 (B-C:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

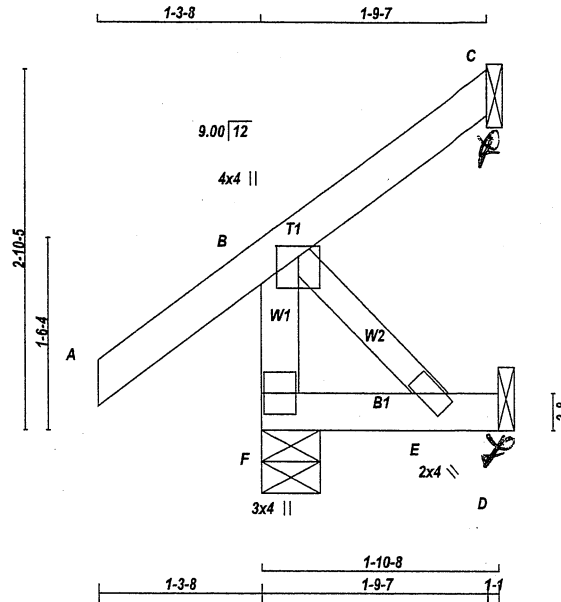


Structural component only
DWG# T-2204169

JOB NAME 417704	TRUSS NAME C4	QUANTITY 5	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC. HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:JvMhHdWEgsSI57rNnlib4czW6nl-?HAj_SbPHuDqs8_80qA?4KfLHjFVKxeBPKX2uFznTsj



TOTAL WEIGHT = 5 X 9 = 47 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
F	333	0	0	333	0	0	0	5-8	5-8
C	41	0	41	0	-50	1-8	1-8	1-8	1-8
D	17	0	19	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	230	182 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	28	23 / -35	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED				WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED (LC)	UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO						FR-TO	
F-B	-315 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00		
B-C	-36 / 0	-112.4	-112.4	0.15 (1)	6.25		
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

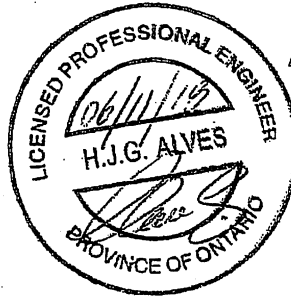
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2204170



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering *em and General Safety notes.

T-1800218

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	Length (in.)	Diameter (in.)	LATERAL RESISTANCE PER NAIL (lb)		WITHDRAWAL RESISTANCE PER NAIL (lb)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

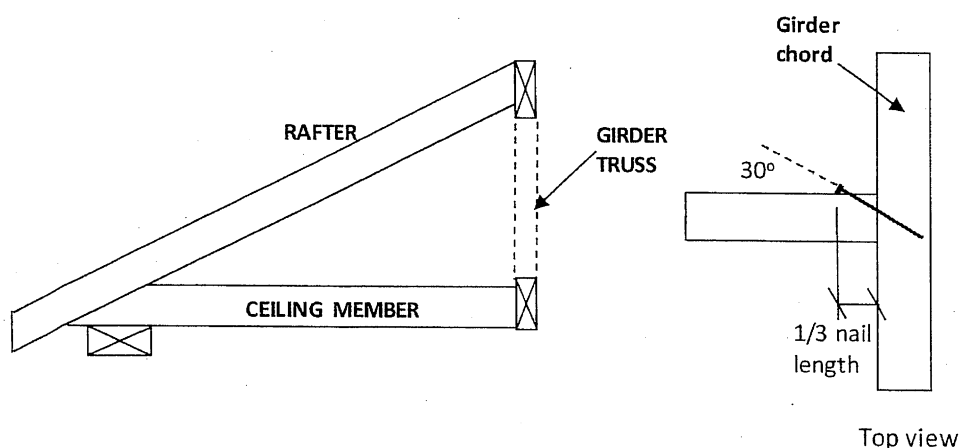


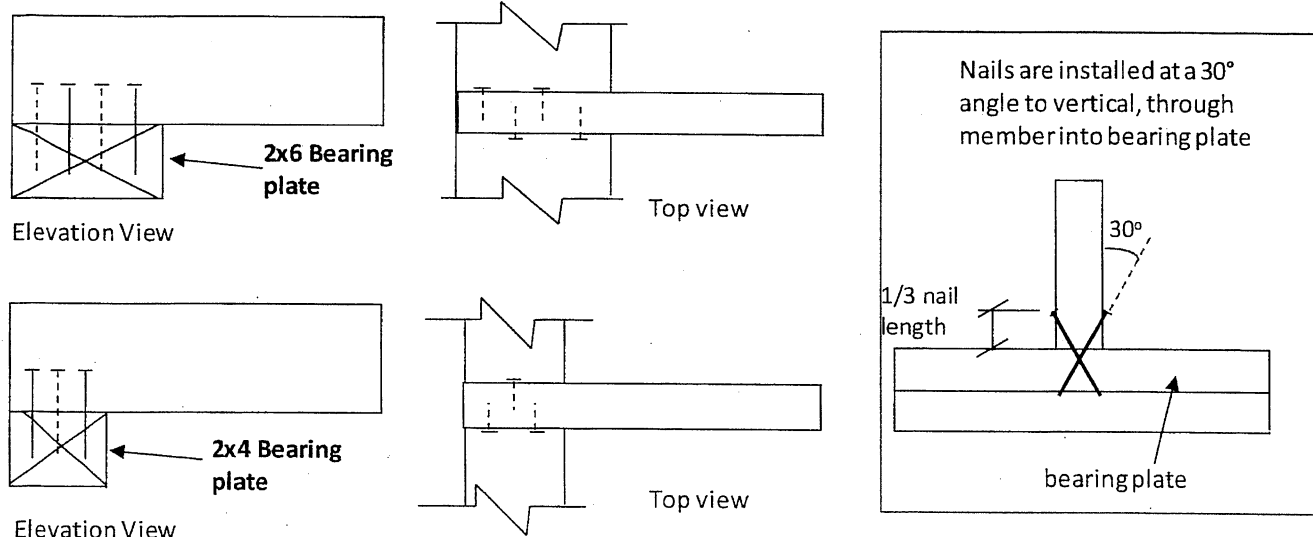
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_0 factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

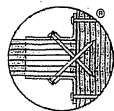
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

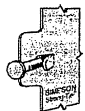
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

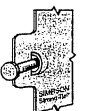
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear
Nailing
Top View

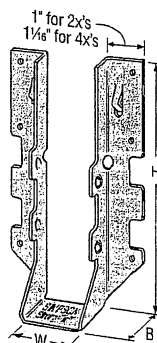
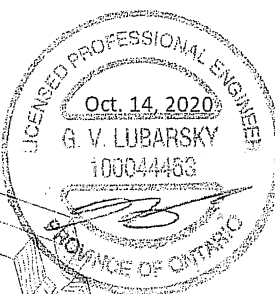
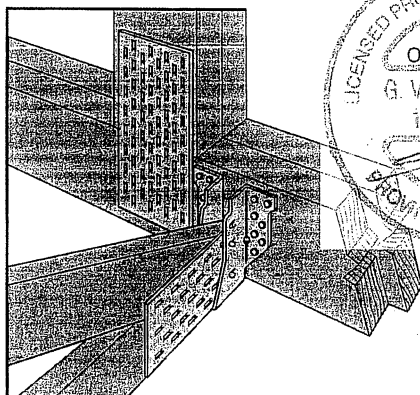


Double-Shear
Nailing
Side View;
Do not
bend tab

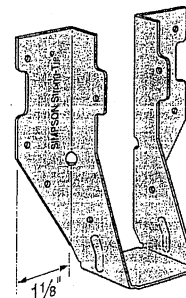


Dome Double-Shear
Nailing
Side View
(available on
some models)

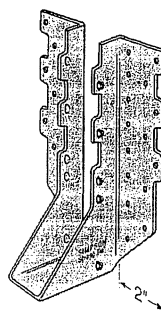
**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



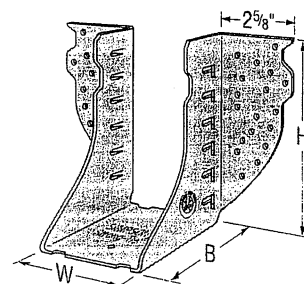
LUS28



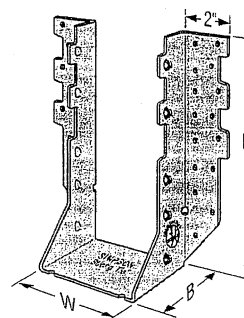
LU26L



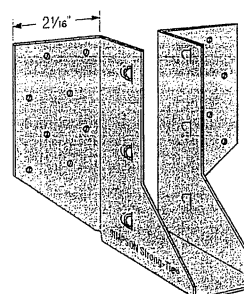
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS

TECHNICAL BULLETIN

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

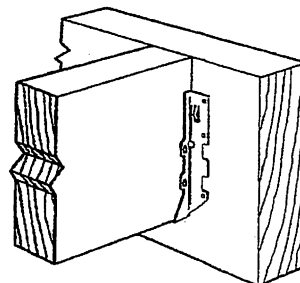
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

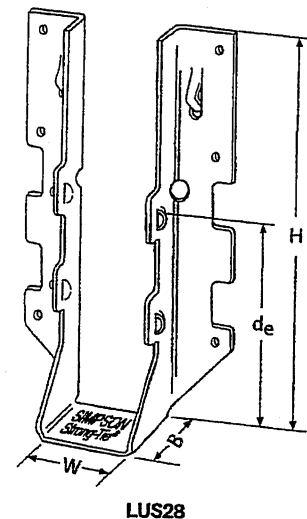
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

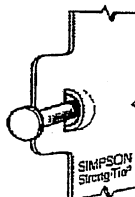


Typical LUS Installation



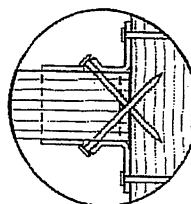
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _u =1.15)	(K _n =1.00)	(K _u =1.15)	(K _n =1.00)
LUS24	18	1½	3½	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3%	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4%	4¾	2	3¾	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6%	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4%	6¾	2	3¾	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 15/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4%	8¾	2	5¾	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

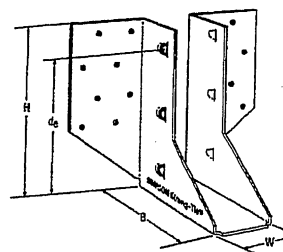
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

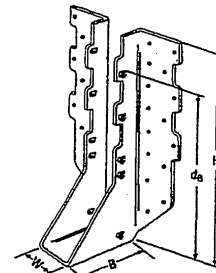
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

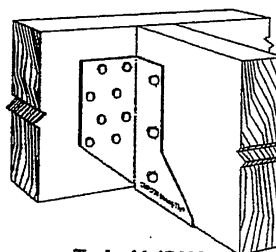
- See current catalogue for options



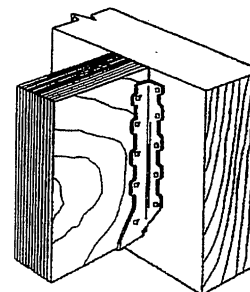
LJS26DS



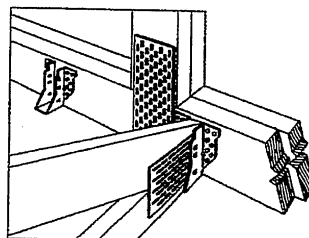
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



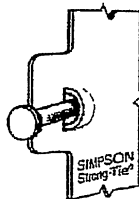
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

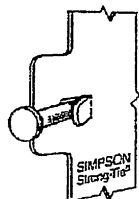
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15) lb.	Normal (K _n =1.00) lb.	Uplift (K _u =1.15) lb.	Normal (K _n =1.00) lb.
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

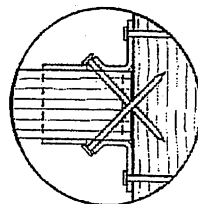


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

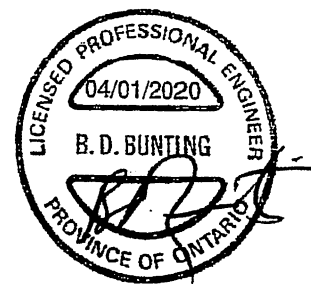
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

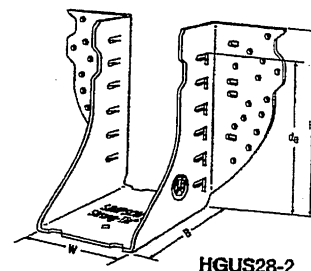
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

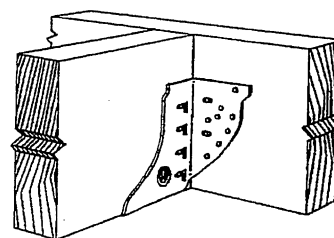
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

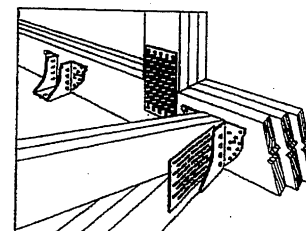
- See current catalogue for options



HGUS28-2



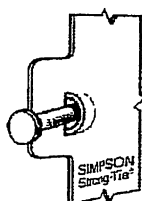
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

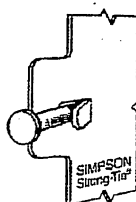
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

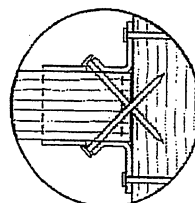


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

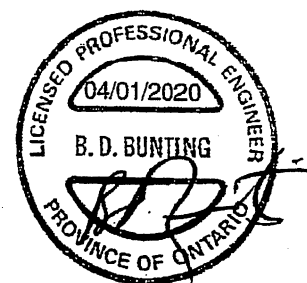
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

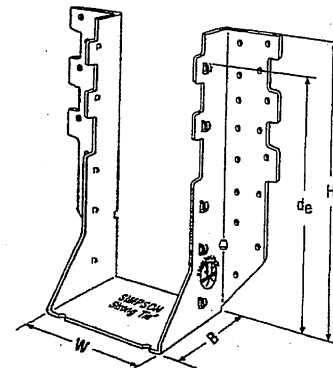
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

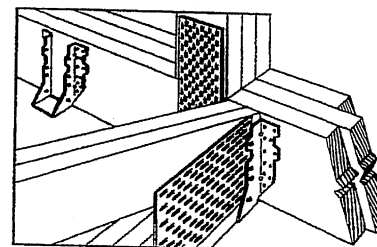
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



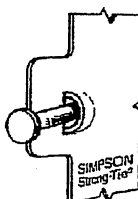
HHUS410



Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)

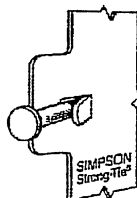
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
HHUS26-2	14	3⅞	5⅜	3	3⅜	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3⅞	7⅜	3	6⅜	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3⅞	9⅜	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4⅞	9	3	7⅜	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6⅞	8⅜	3	7⅜	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3⅞	5⅜	3	3⅜	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3⅞	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5⅞	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7⅞	9	3⅞	7⅜	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

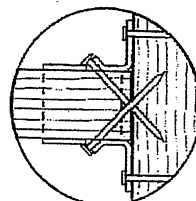


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

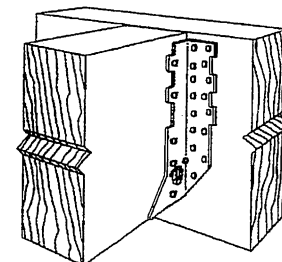
U.S. Patent
5,603,580



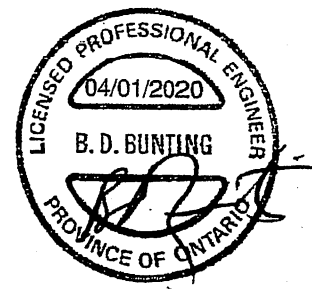
Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



Typical HHUS
Installation



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

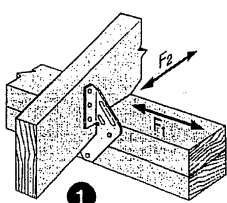
© 2020 Simpson Strong-Tie Company Inc.

T-SPECHHUS20 3/20 exp. 6/22

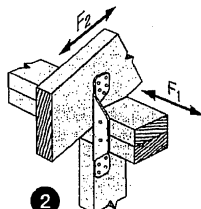
(800) 999-5099
strongtie.com

H/TSP

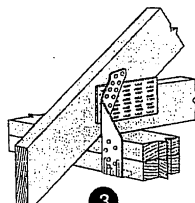
Seismic and Hurricane Ties (cont.)



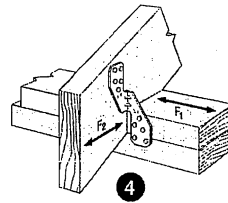
1 H1 Installation



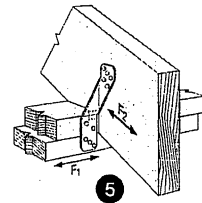
2 H2A Installation



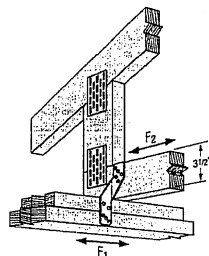
3 TSP Installation



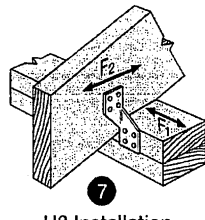
4 H2.5A Installation
(Nails into both top plates)



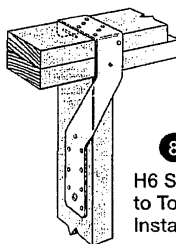
5 H2.5T Installation
(Nails into both top plates)



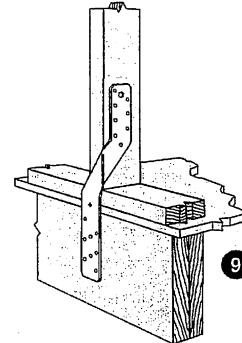
6 H2.5T Installation



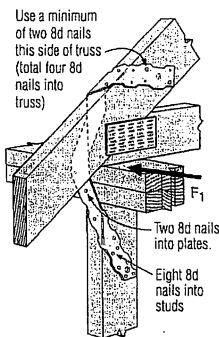
7 H3 Installation
(Nails into upper top plate)



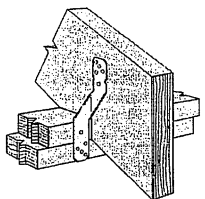
8 H6 Stud to Top Plate Installation



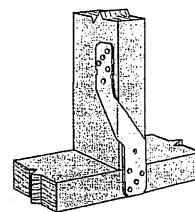
9 H6 Stud to Band Joist Installation



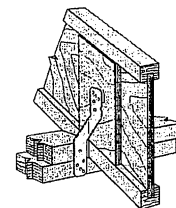
10 H7Z Installation



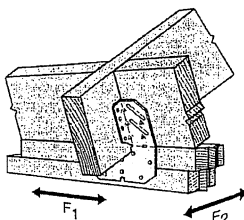
11 H8 Attaching Rafter to Double Top Plates



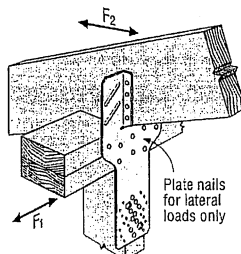
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



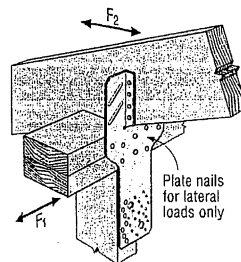
13 H8 attaching I-Joist to Double Top Plates



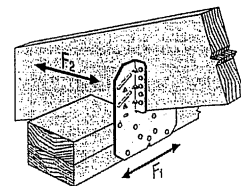
14 H10A Field-Bent Installation



15 H10S Installation

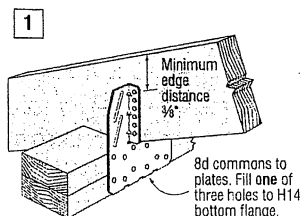


16 H10S Installation with Stud Offset

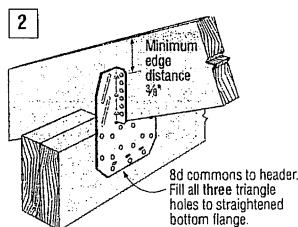


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

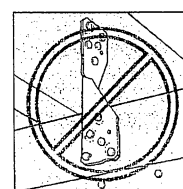


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

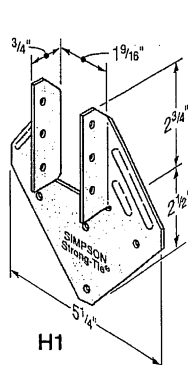
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

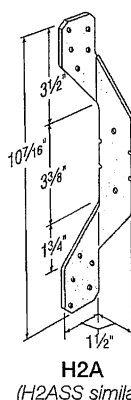
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp.20-24 or visit strongtie.com.

Installation:

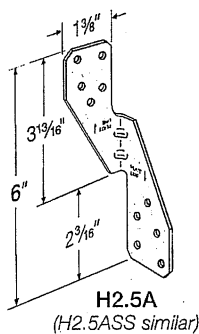
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12; use H10A sloped loads for field bent installation.



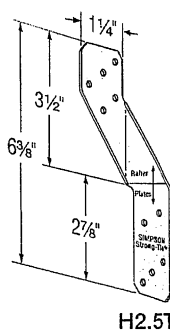
H1



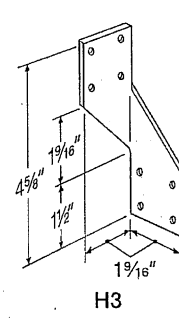
H2A
(H2ASS similar)



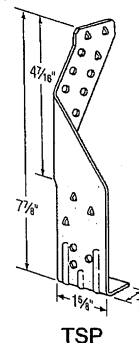
H2.5A
(H2.5ASS similar)



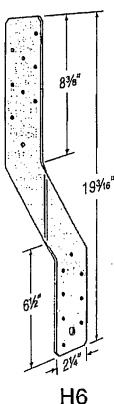
H2.5T



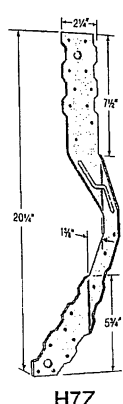
H3



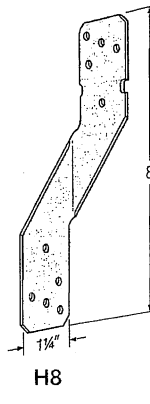
TSP



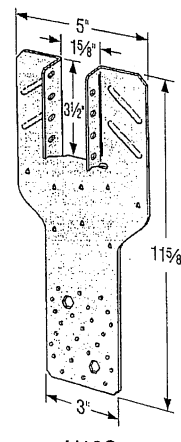
H6



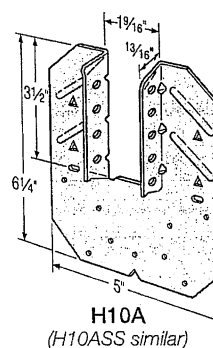
H7Z



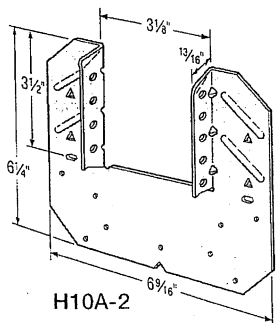
H8



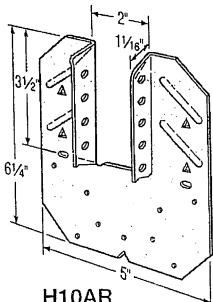
H10S



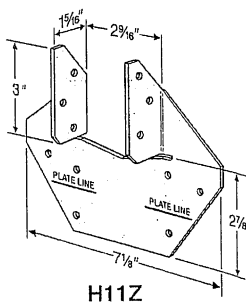
H10A
(H10ASS similar)



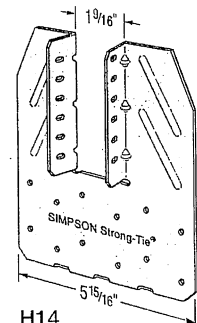
H10A-2



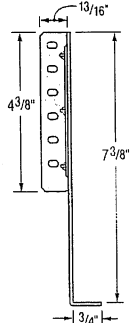
H10AR



H11Z



H14



H14
Profile

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA 086-14

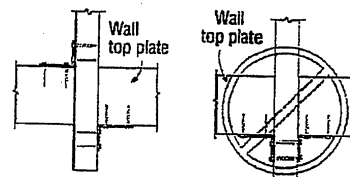
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

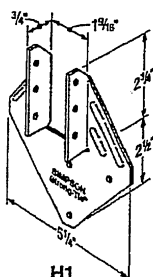
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

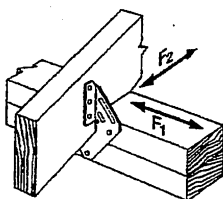


Install diagonally across from each other for minimum 2x truss.

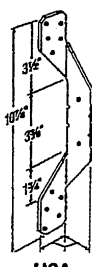
Nailing into both sides of a single ply 2x truss may cause the wood to split.



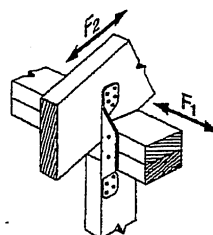
H1



H1 Installation



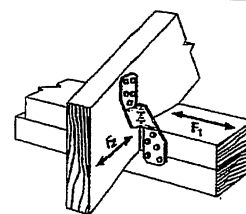
H2A



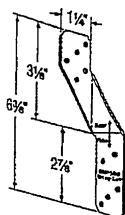
H2A Installation



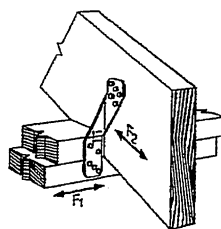
H2.5A



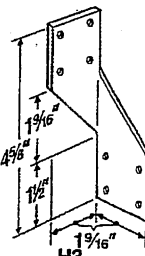
H2.5A Installation



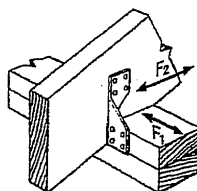
H2.5T



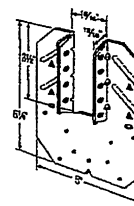
H2.5T Installation
(Nails into both top plates)



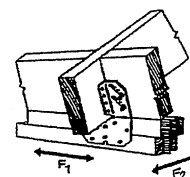
H3



H3 Installation



H10A



H10A Installation

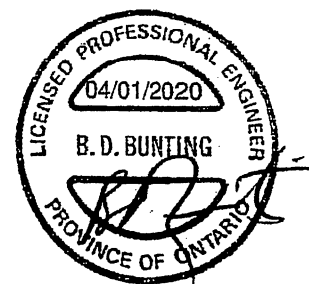
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
					(K _n =1.15)					(K _n =1.15)
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

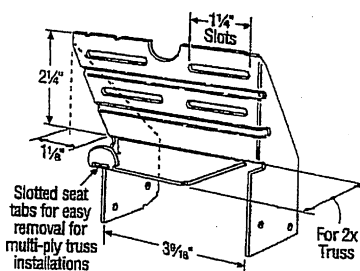
Design: Factored resistances are in accordance with CSA 086-14

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

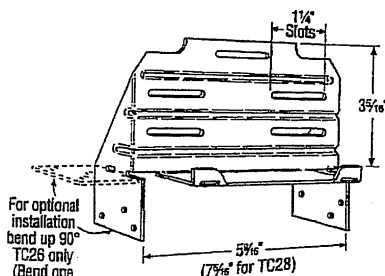
Optional TC Installation:

- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



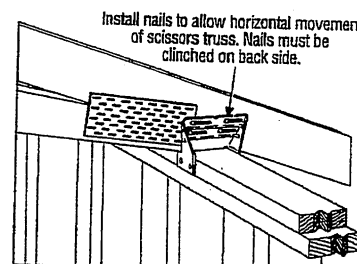
TC24

U.S. Patent 4,932,173

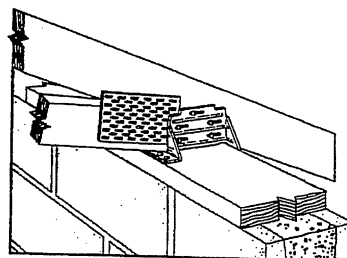


TC26

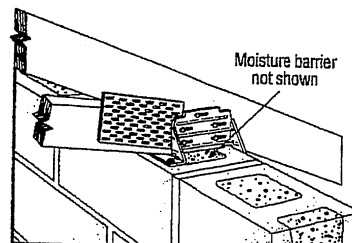
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



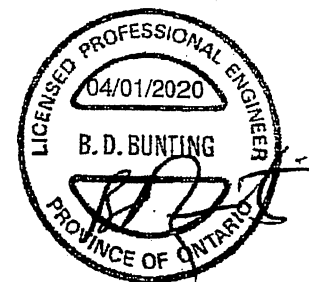
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



**LIMIT
STATES
DESIGN**

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T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HTU

SIMPSON
Strong-Tie

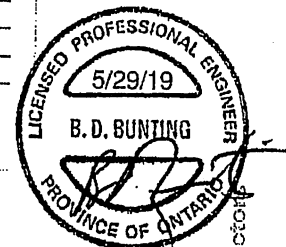
Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
			Header	Joist	D.Fir-L		S-P-F	
					Uplift	Normal	Uplift	Normal
					(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3 3/8	(2) 2x4	(10) 16d	(14) 10d x 1 1/2"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5 1/2	(2) 2x4	(10) 16d	(20) 10d x 1 1/2"	2470	4015	1755	2850
					10.99	17.86	7.81	12.68
HTU28 (Max.)	3 3/8	(2) 2x6	(20) 16d	(26) 10d x 1 1/2"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19
HTU210 (Max.)	7 1/4	(2) 2x6	(20) 16d	(32) 10d x 1 1/2"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19

See table footnotes on p. 260.

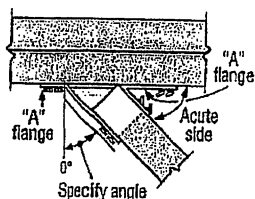


Hanger Options

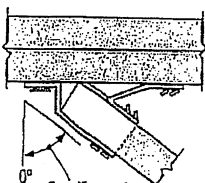
See Hanger Options Information on pp. 125-127.

Skewed Seat

- Skawable up to 67 1/2°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
		Header	Joist	D.Fir-L		S-P-F	
				Uplift	Normal	Uplift	Normal
				(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
				lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1 1/2"	1835	4110	1300	2905
	51-67 1/2	(20) 16d	(12) 10d x 1 1/2"	8.16	18.28	5.78	12.92
HTU28	< 51	(26) 16d	(20) 10d x 1 1/2"	1350	3620	955	2560
	51-67 1/2	(26) 16d	(17) 10d x 1 1/2"	6.01	16.10	4.25	11.39
HTU210	< 51	(32) 16d	(26) 10d x 1 1/2"	2810	4270	1985	3030
	51-67 1/2	(32) 16d	(22) 10d x 1 1/2"	12.50	18.99	8.83	13.48
HTU26-2	< 51	(20) 16d	(14) 10d	2075	3930	1465	2780
	51-67 1/2	(20) 16d	(12) 10d	9.23	17.48	6.52	12.37
HTU28-2	< 51	(26) 16d	(20) 10d	3785	4430	2675	3135
	51-67 1/2	(26) 16d	(17) 10d	16.84	19.71	11.90	13.95
HTU210-2	< 51	(32) 16d	(26) 10d	2795	4240	1980	3000
	51-67 1/2	(32) 16d	(22) 10d	12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
	51-67 1/2	(20) 16d	(12) 10d	9.52	16.53	6.74	11.68
HTU28-2	< 51	(26) 16d	(20) 10d	1610	3920	1140	2785
	51-67 1/2	(26) 16d	(17) 10d	7.16	17.44	5.07	12.39
HTU210-2	< 51	(32) 16d	(26) 10d	3960	5425	2815	3855
	51-67 1/2	(32) 16d	(22) 10d	17.62	24.13	12.52	17.15
HTU26-2	< 51	(20) 16d	(14) 10d	2385	5425	1695	3855
	51-67 1/2	(20) 16d	(12) 10d	10.61	24.13	7.54	17.15
HTU28-2	< 51	(26) 16d	(20) 10d	5025	6890	3570	4890
	51-67 1/2	(26) 16d	(17) 10d	22.35	30.65	15.88	21.75
HTU210-2	< 51	(32) 16d	(26) 10d	3145	6680	2225	4745
	51-67 1/2	(32) 16d	(22) 10d	13.99	29.72	9.90	21.10

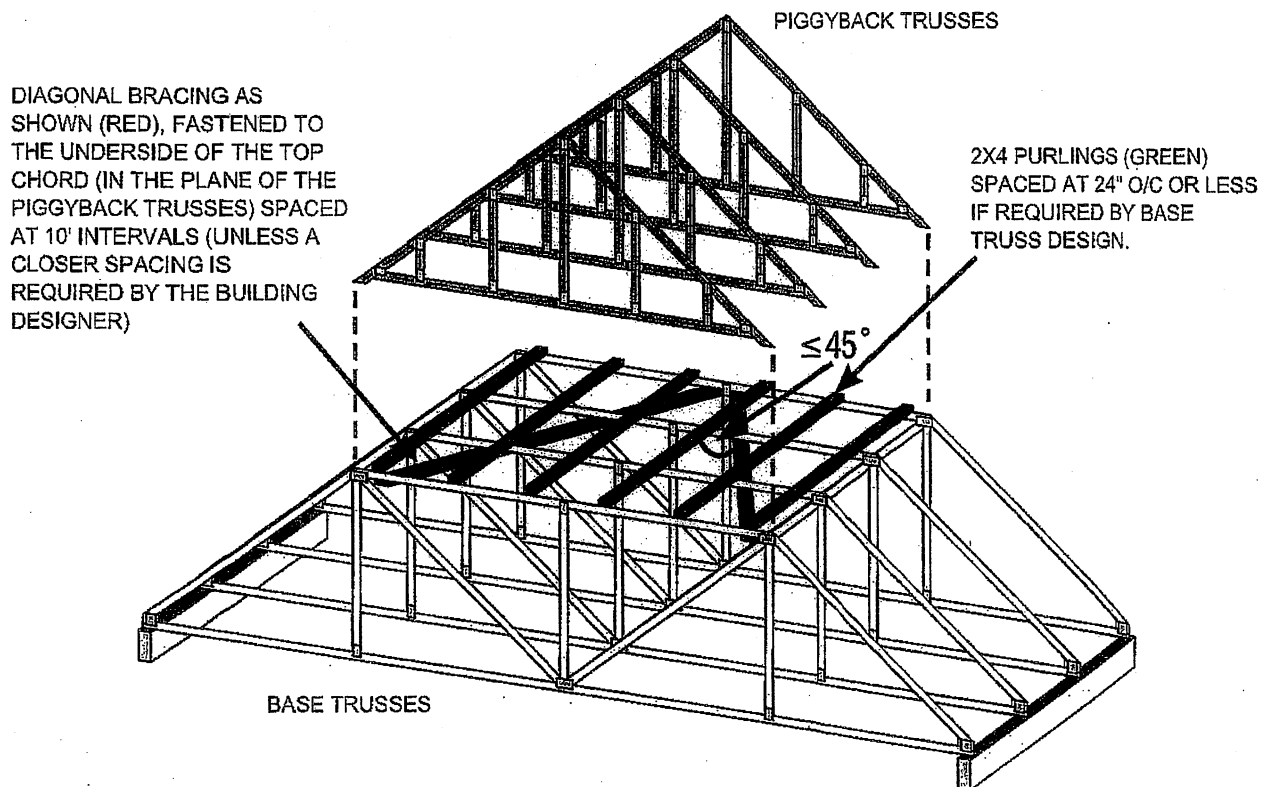
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3 1/2" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 10d = 0.148" dia. x 3" long. See pp. 27-28 for other nail sizes and information.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and **MSTA** — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and **MSTI** — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

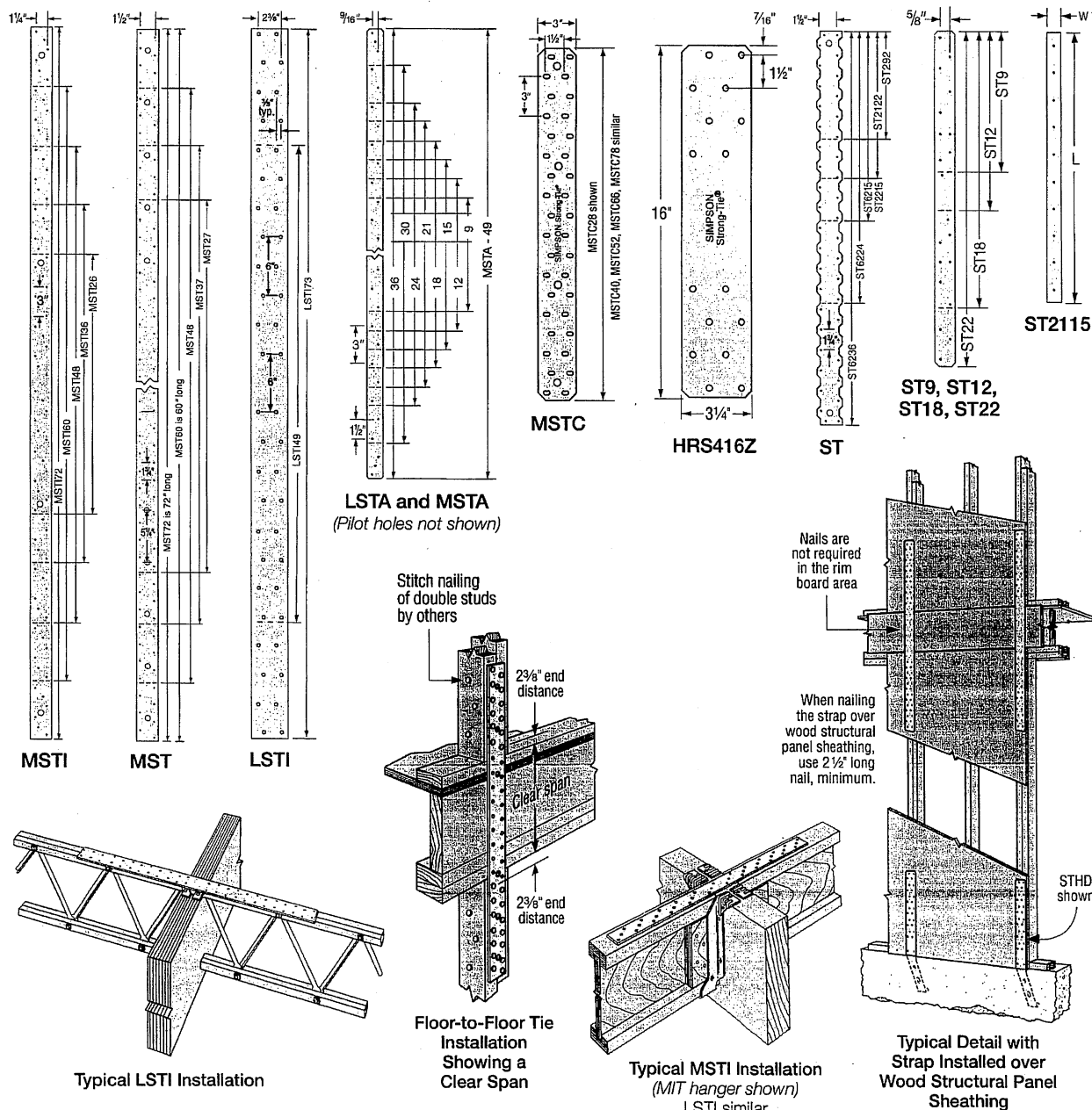
MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

Installation: Use all specified fasteners; see General Notes

Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

Straps and Ties



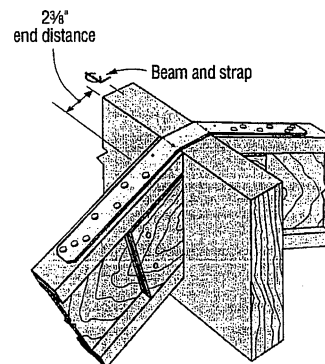
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties (cont.)

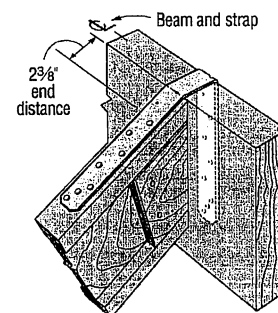
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366-370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
					D.Fir-L		S-P-F	
		W	L		(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
LSTA9	20	1¼	9	(6) 10d	600	690	555	635
					2.67	3.07	2.47	2.82
LSTA12		1¼	12	(8) 10d	800	920	735	845
					3.56	4.09	3.27	3.76
LSTA15		1¼	15	(10) 10d	1000	1150	920	1060
					4.45	5.12	4.09	4.72
LSTA18		1¼	18	(12) 10d	1200	1380	1105	1270
					5.34	6.14	4.92	5.65
LSTA21		1¼	21	(14) 10d	1400	1610	1290	1485
					6.23	7.16	5.74	6.61
LSTA24		1¼	24	(16) 10d	1600	1840	1475	1695
					7.12	8.19	6.56	7.54
ST292		2⅝	9⅝	(8) 8d	585	675	535	615
					2.60	3.00	2.38	2.74
ST2122		2⅝	12⅜	(12) 8d	940	1085	865	995
					4.18	4.83	3.85	4.43
ST2115	¾	16⅝	(8) 8d	670	770	615	710	
				2.98	3.43	2.74	3.16	
ST2215	2⅝	16⅝	(16) 8d	1335	1540	1235	1420	
				5.94	6.85	5.49	6.32	
LSTA30	18	1¼	30	(20) 10d	2235	2465	2075	2385
					9.94	10.97	9.23	10.61
LSTA36		1¼	36	(24) 10d	2465	2465	2465	2465
					10.97	10.97	10.97	10.97
LSTI49		¾	49	(32) 10d x 1½"	3115	3580	2852	3280
					13.86	15.93	12.69	14.59
LSTI73		¾	73	(48) 10d x 1½"	4670	5370	4280	4920
					20.77	23.89	19.04	21.89
MSTA9		1¼	9	(6) 10d	670	770	625	715
					2.98	3.43	2.78	3.18
MSTA12		1¼	12	(8) 10d	895	1030	830	955
					3.98	4.58	3.69	4.25
MSTA15		1¼	15	(10) 10d	1120	1285	1040	1195
					4.98	5.72	4.63	5.32
MSTA18		1¼	18	(12) 10d	1340	1545	1245	1430
					5.96	6.87	5.54	6.36
MSTA21	1¼	21	(14) 10d	1565	1800	1455	1670	
				6.96	8.01	6.47	7.43	
MSTA24	1¼	24	(16) 10d	1790	2060	1660	1910	
				7.96	9.16	7.38	8.50	
MSTA30	16	1¼	30	(20) 10d	2470	2840	2260	2595
					10.99	12.63	10.05	11.54
MSTA36		1¼	36	(24) 10d	2965	3070	2710	3070
					13.19	13.66	12.06	13.66
MSTA49		1¼	49	(28) 8d	2725	2725	2545	2725
					12.12	12.12	11.32	12.12
ST6215		2⅝	16⅝	(16) 8d	1405	1615	1300	1500
					6.25	7.18	5.78	6.67
ST6224		2⅝	23⅝	(24) 8d	2305	2650	2155	2475
					10.25	11.79	9.59	11.01
ST9		1¼	9	(6) 8d	525	605	490	560
					2.34	2.69	2.18	2.49
ST12		1¼	11⅝	(8) 8d	700	805	650	750
					3.11	3.58	2.89	3.34
ST18		1¼	17¾	(12) 8d	1050	1210	975	1125
					4.67	5.38	4.34	5.00
ST22	1¼	21⅝	(18) 8d	1580	1790	1465	1685	
				7.03	7.96	6.52	7.50	



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22-23 for other nail sizes and information.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

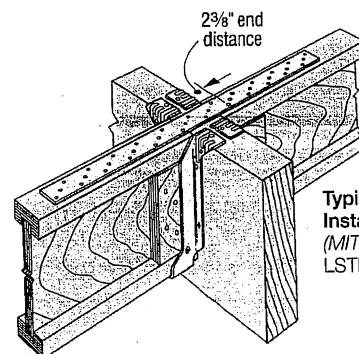
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

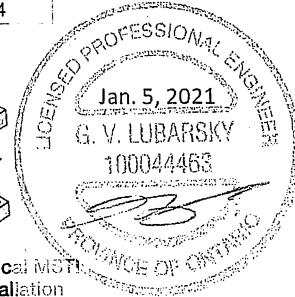
Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
					D.Fir-L		S-P-F	
		W	L		(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52		3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66	14	3	65¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78		3	77¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236		2⅙	33⅓⅙	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	12	2⅙	26	(22) 10d x 1 ½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2⅙	36	(32) 10d x 1 ½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48		2⅙	48	(44) 10d x 1 ½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2⅙	60	(56) 10d x 1 ½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72		2⅙	72	(68) 10d x 1 ½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2⅙	27	(26) 8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37		2⅙	37½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MST48		2⅙	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z		3¼	16	(16) ¼" x 1 ½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60	10	2⅙	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72		2⅙	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Use half of the nails in each member being connected to achieve the listed resistances.
- Nails: 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.



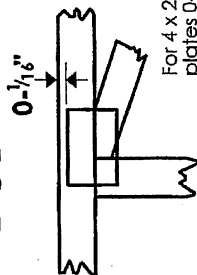
Typical MSTI
Installation
(MIT hanger shown)
LSTI similar



Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



*Plate location details available in Mitek software or upon request.

PLATE SIZE

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

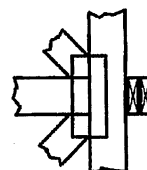
4 X 4

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



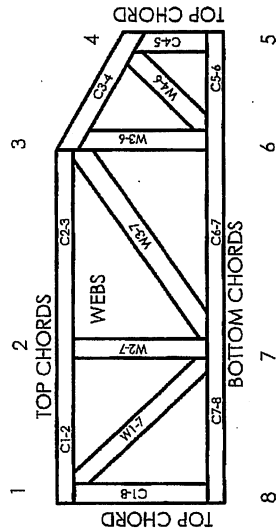
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM.™

Mitek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.