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Dob Track: 50465
Plan Log: 204529
Layout ID: 417701

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Plan Log: 204529
Layout ID: 417701

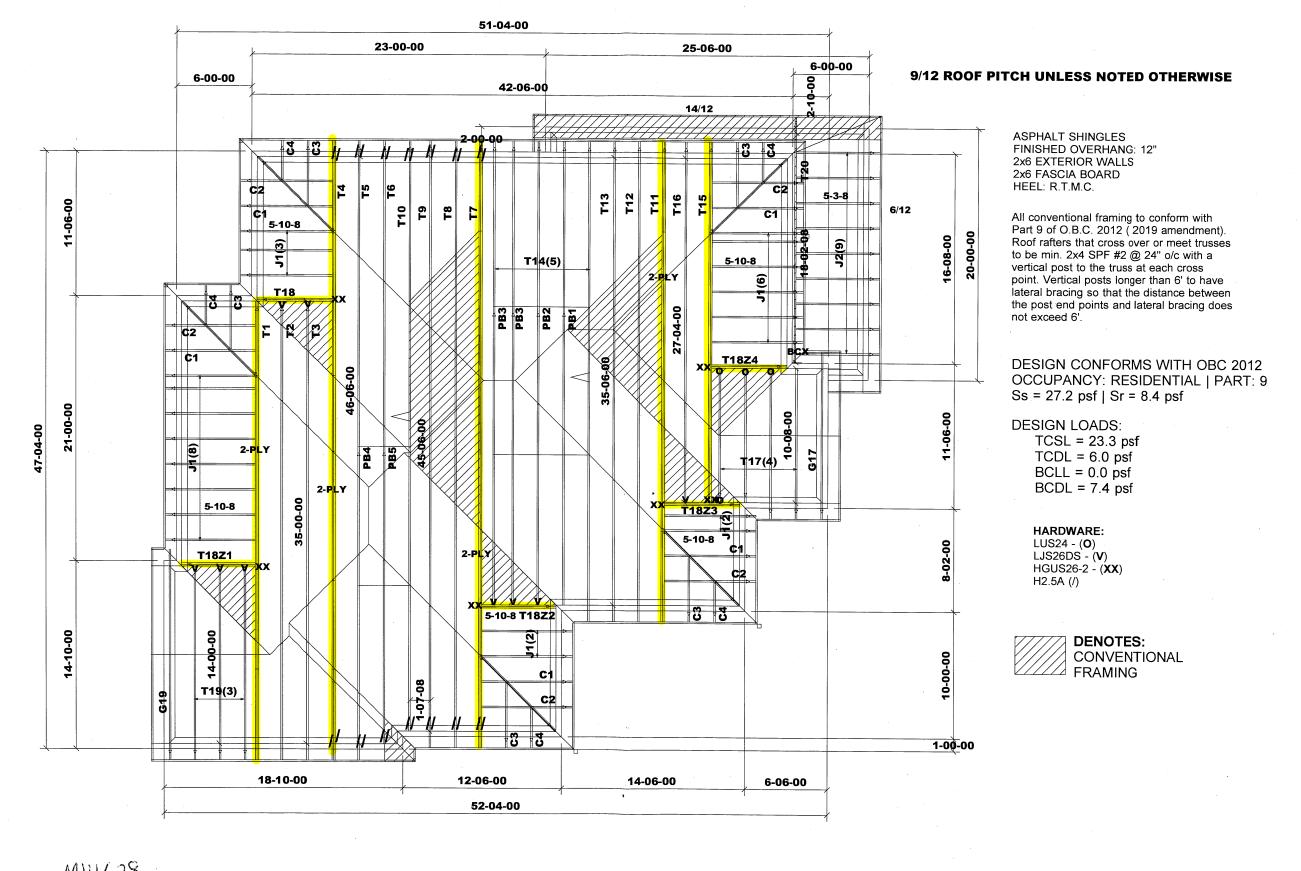
Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD
S42-19C/A-LOT 48 (OPT.WITH COFF)

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Date: 2022-02-15

Date: 2022-02-15

Sales: Rick DiCiano

Designer: YPG
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Job Track: 50465
Plan Log: 204529
Layout ID: 417704

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DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER

Builder: BAYVIEW WELLINGTON HOMES

Project: GREEN VALLEY EAST

Location: BRADFORD

Model: Lot #:

Lot #:
Elevation: A-LOT 48 (OPT.WITH COFF)

S42-19C

Job Track:

50465

PlanLog: Layout ID: 204529 417701

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Date:

02-15-2022

Designer:

Sales Rep: Rick DiCiano

Roof Trusses

· · · · · · · · · · · · · · · · · · ·	QTY	MARK		<u> </u>			OVERHANG	HEEL HEIGHT	LBS.	BUNDLE#	LOAD BY
PROFILE	PLY	TYPE	PITCH	SPAN	HEIGHT	LUMBER	LEFT RIGHT	LEFT RIGHT	BFT.	STACK#	REMARKS
	1 2-ply	T1S Hip Girder	9 /12	35-00-00	5-11-02	2 x 4 2 x 6	1-03-08	1-06-04 1-06-04	374.94 242.67		
	1	T2S Hip	9 /12	35-00-00	7-05-02	2 x 4	1-03-08	1-06-04 1-06-04	171.91 110.00		
	1	T3S Roof Special	9 /12	35-00-00	8-11-02	2 x 4	1-03-08	1-06-04 1-06-04	188.27 121.17		
	1 2-ply	T4S Roof Special Girder	9 /12	46-06-00	10-05-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	674.21 421.33		
	1	T5S Piggyback Base	9 /12	46-06-00	10-02-00	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	342.48 212.00		
	1	T6 Piggyback Base	9 /12	46-06-00	10-02-00	2 x 6	1-03-08	1-06-04 9-04	294.56 180.67		
	1 2-ply	T7 Hip Girder	9 /12	45-06-00	5-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	562.02 337.33		
	1	T8 Hip	9 /12	45-06-00	7-05-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	266.95 158.17		
	1	T9 Hip	9 /12	45-06-00	8-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	285.65 174.83		
	1	T10 Hip	9 /12	45-06-00	10-02-00	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	282.44 171.00		
	1 2-ply	T11 Hip Girder	9 /12	35-06-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	370.96 232.67		
	1	T12 Hip	9 /12	35-06-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	166.22 104.83		
	1	T13 Hip	9 /12	35-06-00	8-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	172.16 108.67		-
	5	T14 Hip	9 /12	35-06-00	10-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	885.23 548.33		-



DELIVERY SHIPLIST

Lumber Yard:

TAMARACK LUMBER

Builder:

BAYVIEW WELLINGTON HOMES

Project:

GREEN VALLEY EAST

Location:

BRADFORD

Model:

Elevation:

Lot #:

S42-19C

A-LOT 48 (OPT.WITH COFF)

Job Track:

50465

PlanLog: Layout ID:

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Date:

02-15-2022

Designer:

Sales Rep: Rick DiCiano

Roof Trusses

	QTY	MARK					OVERHANG	HEEL HEIGHT	LBS.	BUNDLE#	LOAD BY
PROFILE	PLY	TYPE	PITCH	SPAN	HEIGHT	LUMBER	LEFT RIGHT	LEFT RIGHT	BFT.	STACK#	REMARKS
	1 2-ply	T15 Hip Girder	9 /12	27-04-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	276.46 176.00		
	1	T16 Hip	9 /12	27-04-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	122.05 76.83		
	4	T17 Common	9 /12	10-08-00	5-06-04	2 x 4		1-06-04 1-06-04	170.13 111.33		
	1	G17 GABLE	9 /12	10-08-00	5-06-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	49.09 32.50		
	1 2-ply	T18 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z1 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z2 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z3 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z4 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	3	T19S Roof Special	9 /12	14-00-00	6-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	198.19 137.00		
	1	G19 GABLE	9 /12	14-00-00	6-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	65.2 43.17		
	1	T20 Half Hip Girder	14 /12	18-02-08	3-07-14	2 x 6	1-03-08	1-03-08 2-11-15	105.3 66.67		-
	1	PB1 Piggyback	9 /12	11-09-00	1-06-00	2 x 4			34.2 22.17		
	1	PB2 Piggyback	9 /12	11-09-00	3-00-00	2 x 4			34.5 22.17		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER

Builder: **BAYVIEW WELLINGTON HOMES**

Project: **GREEN VALLEY EAST**

Location: **BRADFORD** Model: S42-19C

Lot #:

Elevation: A-LOT 48 (OPT.WITH COFF) Job Track:

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Date:

02-15-2022

Designer:

Sales Rep: Rick DiCiano

Roof Trusses

ROOTIF								I		BUNDLE#	LOAD BY
PROFILE	QTY PLY	MARK TYPE	РІТСН	SPAN	HEIGHT	LUMBER	OVERHANG LEFT	HEEL HEIGHT LEFT	LBS. BFT.	STACK#	REMARKS
	PLY	TYPE					RIGHT	RIGHT	Dri.	STACK#	REWARKS
	2	PB3	9 /12	11-09-00	4-04-14	2 x 4			62.99		
		Piggyback	9/12	11-09-00	4-04-14	2 7 4			40.00		
	1	PB4							32.36		
	-	Piggyback	9 /12	11-00-00	1-09-06	2 x 4			22.33		
	1	PB5	0.440	44 00 00	4.00.00				31.9		
		Piggyback	9 /12	11-00-00	1-06-00	2 x 4			20.83		
	21	J1						1-06-04	386.38		
		Jack-Open	9 /12	5-10-08	5-11-02	2 x 4	1-03-08	5-11-02	252.00		
	9	J2						4-03	131.34		
	_	Jack-Open	6 /12	5-03-08	3-07-07	2 x 4	1-03-08	2-11-15	84.00		
	5	C1				_	1-03-08	1-06-04	84.19		
	_	Jack-Open	9 /12	3-09-07	4-04-05	2 x 4	2-01-01	4-04-05	55.00		
	5	C2		4 00 05	0.40.05		1-03-08	1-06-04	69.37		
		Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	4-01-01	2-10-05	48.33		
	5	C3					1-03-08	1-06-04	61.11		
		Jack-Open	9 /12	1-10-08	4-04-05	2 x 4	1-10-15	2-11-02	41.67		
	5	C4					1-03-08	1-06-04	46.56		
		Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-01	2-10-05	35.00		

TOTAL #TRUSS= 101

TOTAL BFT OF ALL TRUSSES= 4625.67

BFT.

TOTAL WEIGHT OF ALL TRSSES 7329.71 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
28	Hardware	H2.5A	
6	Hardware	HGUS26-2	
9	Hardware	LJS26DS	
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 47



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER

Builder: BAYVIEW WELLINGTON HOMES

Project: GREEN VALLEY EAST

Location: BRADFORD Model: S42-19C

Lot #:

Elevation: A-LOT 48

Job Track:

50465

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Date:

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Designer:

Sales Rep: Rick DiCiano

Roof Trusses

	QTY	MARK					OVERHANG	HEEL HEIGHT	LBS.	BUNDLE#	LOAD BY
PROFILE	PLY	TYPE	PITCH	SPAN	HEIGHT	LUMBER	LEFT RIGHT	LEFT RIGHT	BFT.	STACK#	REMARKS
	1 2-ply	T1 Hip Girder	9 /12	35-00-00	5-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	421.62 254.67		
	1	T2 Hip	9 /12	35-00-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	156.74 99.00		
	1	T3 Roof Special	9 /12	35-00-00	8-11-02	2 x 4	1-03-08	1-06-04 1-06-04	173.74 110.33		
	1 2-ply	T4 Roof Special Girder	9 /12	46-06-00	10-05-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	646.25 398.67		
	1	T5 Piggyback Base	9 /12	46-06-00	10-02-00	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	340.88 206.67		
	1	T6 Piggyback Base	9 /12	46-06-00	10-02-00	2 x 6	1-03-08	1-06-04 9-04	326.25 198.67		
	1 2-ply	T7 Hip Girder	9 /12	45-06-00	5-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	562.02 337.33		
	1	T8 Hip	9 /12	45-06-00	7-05-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	296.73 174.67		
	1	T9 Hip	9 /12	45-06-00	8-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	308.35 188.00		
	1	T10 Hip	9 /12	45-06-00	10-02-00	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	305.76 184.00		
	1 2-ply	T11 Hip Girder	9 /12	35-06-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	378.14 236.67		
	1	T12 Hip	9 /12	35-06-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	169.6 106.83		
	1	T13 Hip	9 /12	35-06-00	8-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	175.18 110.33		
	5	T14 Hip	9 /12	35-06-00	10-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	950.91 583.33		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER

Builder: BAYVIEW WELLINGTON HOMES

Project: GREEN VALLEY EAST

Location: BRADFORD Model: S42-19C

Lot #:

Elevation: A-LOT 48

Job Track:

50465

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Date:

02-15-2022

Designer:

Sales Rep: Rick DiCiano

Roof Trusses

	QTY	MARK					OVERHANG	HEEL HEIGHT	LBS.	BUNDLE#	LOAD BY
PROFILE	PLY	TYPE	PITCH	SPAN	HEIGHT	LUMBER	LEFT RIGHT	LEFT RIGHT	BFT.	STACK#	REMARKS
	1 2-ply	T15 Hip Girder	9 /12	27-04-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	283.41 178.00		
	1	T16 Hip	9 /12	27-04-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	125.5 78.83		
	1	G17 GABLE	9 /12	10-08-00	5-06-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	49.09 32.50		
	. 4	T17 Common	9 /12	10-08-00	5-06-04	2 x 4		1-06-04 1-06-04	170.13 111.33		
	1 2-ply	T18 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		·
	1	G19 GABLE	9 /12	14-00-00	6-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	65.2 43.17		
	1 2-ply	T18Z1 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z2 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z3 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	1 2-ply	T18Z4 Jack-Closed Girder	9 /12	5-10-08	5-11-02	2 x 4 2 x 6		1-06-04 5-11-02	66.08 43.00		
	3	T19 Common	9 /12	14-00-00	6-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	189.71 120.50		
	1	T20 Half Hip Girder	14 /12	18-02-08	3-07-14	2 x 6	1-03-08	1-03-08 2-11-15	105.3 66.67		
	1	PB1 Piggyback	9 /12	11-09-00	1-06-00	2 x 4			34.2 22.17		
	1	PB2 Piggyback	9 /12	11-09-00	3-00-00	2 x 4			34.5 22.17		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER

Builder: **BAYVIEW WELLINGTON HOMES**

Project: **GREEN VALLEY EAST**

BRADFORD Location: Model: S42-19C

Lot #:

Elevation: A-LOT 48 Job Track:

Layout ID:

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Date: 02-15-2022

Designer:

Sales Rep: Rick DiCiano

Roof Trusses

KOOI II	4330	<u> </u>									
	QTY	MARK					OVERHANG	HEEL HEIGHT	LBS.	BUNDLE#	LOAD BY
PROFILE	PLY	TYPE	РІТСН	SPAN	HEIGHT	LUMBER	LEFT RIGHT	LEFT RIGHT	BFT.	STACK#	REMARKS
	2	PB3 Piggyback	9 /12	11-09-00	4-04-14	2 x 4			62.99 40.00		
	1	PB4 Piggyback	9 /12	11-00-00	1-09-06	2 x 4			32.36 22.33		
	1	PB5 Piggyback	9 /12	11-00-00	1-06-00	2 x 4			31.9 20.83		
	21	J1 Jack-Open	9 /12	5-10-08	5-11-02	2 x 4	1-03-08	1-06-04 5-11-02	526.76 346.50		
	9	J2 Jack-Open	6 /12	5-03-08	3-07-07	2 x 4	1-03-08	4-03 2-11-15	131.34 84.00		
	5	C1 Jack-Open	9 /12	3-09-07	4-04-05	2 x 4	1-03-08 2-01-01	1-06-04 4-04-05	84.19 55.00		
	5	C2 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 4-01-01	1-06-04 2-10-05	69.37 48.33		
	5	C3 Jack-Open	9 /12	1-10-08	4-04-05	2 x 4	1-03-08 1-10-15	1-06-04 2-11-02	61.11 41.67		
	5	C4 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	46.56 35.00		

TOTAL #TRUSS= 101

TOTAL BFT OF ALL TRUSSES= 4773.17

BFT.

TOTAL WEIGHT OF ALL TRSSES 7646.17 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
28	Hardware	H2.5A	
6	Hardware	HGUS26-2	
9	Hardware	LJS26DS	
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 47

JOB DESC. BAYVIEW WELLINGTON PLY DRWG NO. 417704 T1 2 TRUSS DESC. HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:15 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-BPKulCklhGcGhqKF9eta1fd8W9wmPptpwyi7n6znTsy 5-10-8 1-3-8 23-3-0 5-10-8 1-3-8 Scale = 1:61. 3x6 || 3x6 ||1 8x9 \\ 5x6 5x6 = 8x9 // D G н Ε 9.00 12 5x8 💸 цŧ-⊠ K AB AC AD ΑE AG АН М Q 0 N L 3x6 | 5x6 5x6 =3x6 || 6x10 = 6x10 =5x6 = 1-11-4 2-0-0 16-11-4 14-1-8 1-3-8 35-0-0 1-3-8 TOTAL WEIGHT = 2 X 211 = 422 lb LUMBE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER N. L. G. A. RULES CHORDS SIZE **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED SPF SPF SPF MAXIMUM FACTORED GROSS REACTION DOWN HORZ UPLI A C F C 2x6 DRY No.2 REQRD SPECIFIED LOADS: DRY DRY DRY DL = Tr = 20: 2x6 No.2 GROSS REACTION VERT HORZ BRG BRG TOP CH 325 PSF 2x6 HORZ UPLIFT IN-SX 6.0 0.0 7.4 PSF IN-SX 2x6 No.2 SPF R 4978 4978 4427 5-8 5-8 BOT CH. RKRP В 2x6 DRY No.2 SPF DL PSF 2x6 DRY TOTAL LOAD 2100F 1.8E 2x6 SPF M K 2x6 DRY 2100F 1.8F SPE UNFACTORED REACTIONS SPACING = 24.0 IN. C/C M 2x6 2100F 1.8E MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERMILIVE V COMBINED WIND DEAD SOIL 0/0 ALL WEBS 2x4 DRY No.2 SPF 2492 / 0 2237 / 0 0/0 3484 992/0 LOADING IN FLAT SECTION BASED ON A SLOPE EXCEPT 0/0 0/0 856 / 0 0/0 OF 6.00/12 DRY: SEASONED LUMBER. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) CHORDS #ROWS SURFACE LOAD(PLF) ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. SPACING (IN) CSA 086-14 TOP CHORDS : (0.122"X3") SPIRAL NAILS A-C 2 12 LOADING TOTAL LOAD CASES: (4) - TPIC 2014 A-C C-F F-H SIDE(122.0) 12 SIDE(61.0) 5 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. 12 TOP CHORDS WEBS RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED MAX. FACTORED FACTORED MAX. FACTORED ROOF LIVE LOAD TOF MEMB. FORCE VERT. LOAD LC1 MAX MAX MAX CSI (LC) ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL)= L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL)= L/999 (0.30") (PLF) FROM TO K-I TOP (LBS) CSI (LC) (LBS) BOTTOM CHORDS: (0.122"X3") SPIRAL NAILS FR-TO LENGTH FR-TO R- P P- M SIDE(197.8) Q- C C- O 0.03 (4) 0.38 (1) 0.18 (1) A-B 0 / 48 -112.4 -112.4 0.05 (1) 10.00 0/210 0.26 0.47 0.47 B- C -112.4 -112.4 -112.4 -112.4 12 SIDE(197.8) -6080 / 0 0 / 4280 TOP 3.82 -1008 / 0 WEBS: (0.122"X3") SPIRAL NAILS -112.4 -112.4 -8429 / 0 3.82 N- G -914/0 CSI: TC=0.47/1.00 (C-D:1) , BC=0.36/1.00 (N-O:1) , WB=0.48/1.00 (H-N:1) , SSI=0.19/1.00 (C-D:1) 0.47 0.47 3.82 3.82 N- H L- H 0/5425 -552/0 2x4 T- U -8429 / 0 -112.4 -112.4 U- V V- D D- W G-N -112.4 -112.4 -112.4 -112.4 6 SIDE(327.8) 8429 / 0 0.10 (1) -8429 / 0 0.47 (1 0 / 4286 0.38 (1) 0.43 (1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 3.82 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. B- Q O- E -8429 / 0 -112.4 -112.4 0.36 3.86 0/4920 W-X X-E E-Y Y-F -112.4 -112.4 -112.4 -112.4 0.36 (1) 0.36 (1) 3.86 3.86 8429 / 0 0.07 (1) GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. 0/320 E-N 0.03 (1) COMPANION LIVE LOAD FACTOR = 1.00 -8741 / 0 -112.4 -112.4 0.34 (1 3.84 3.84 3.84 -8741 / 0 -112.4 -112.4 AUTOSOLVE HEELS OFF TOP - COMPONENTS ARE LOADED FROM THE TOP AND F-G G-H H-I -112.4 -112.4 -112.4 -112.4 MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. -8741 / 0 0.46 (1) 3.78 -112.4 -112.4 -112.4 -112.4 -5296 / 0 0.24 4 87 0.05 (1) 0.0 0.0 0.18(1)K-1 -4388 / 0 0.0 6.85 PLATE GRIP(DRY) SHEAR SECTION R-Z -18.5 -18.5 -18.5 0.10 (1) 10.00 (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 PROFESSIONAL CHARLES HAVES Z-AA AA- Q 0/0 -18.5 0.10(1) 10.00 0.10 (1) 0.26 (1) 0.26 (1) 0/0 -185 -185 Q-AB AB-AC 0 / 4856 -18.5 0 / 4856 -18.510.00 PLATE PLACEMENT TOL. = 0.250 inches AC-AD 0.26 (1) 0.26 (1) 10.00 10.00 0 / 4856 -185 -185 AD- P P-AE 0 / 4856 0 / 4856 PLATE ROTATION TOL. = 5.0 Deg -18.5 -18.50.26(1)10.00 AE- O 0 / 4856 -18.5-18.50.26 (1) 10.00 JSI GRIP= 0.76 (L) (INPUT = 0.90) JSI METAL= 0.57 (I) (INPUT = 1.00) O-AF AF-AG AG-AH 0 / 8573 0 / 8573 -18.5 -18.5 -18.5 0.36 100009024 -18.5 0.36(1) 10.00 0 / 8573 -18.5 -18.5 0.36 (1) 10.00 AH- N N- M M- L 0 / 8573 0 / 4212 0 / 4212 -18.5 -18.5 -18.5 -18.5 0.36 (1) 0.13 (1) 10.00 10.00 FOUND OF ONTARD -18.5-18.50.13 (1) 10.00 1 - K 0/0 SPECIFIED CONCENTRATED LOADS (LBS) DIR. VERT VERT TYPE TOTAL TOTAL CONN. C1 C1 C1 C1 MAX--269 MAX+ 2020s LOC LC₁ FACE HEEL 5-10-8 20-10-8 -269 -1403 FRONT Structural component only -1403 ___ 5-11-4 -164 -164 __ FRONT VERT TOTAL DWG# T-2204180 [M] 6-11-4 CONTINUED ON PAGE 2

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPE	CIFIED CON	ICENTRA	TED LOA	DS (LBS)					
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CON
T	8-11-4	-15	-15		FRONT	VERT	TOTAL		C1
U	10-11-4	-15	-15		FRONT	VERT	TOTAL		C1
٧	12-11-4	-15	-15		FRONT	VERT.	TOTAL		C1
w	14-11-4	-15	-15		FRONT	VERT	TOTAL		C1
X	16-11-4	-15	-15		FRONT	VERT	TOTAL		C1
Υ	18-11-4	-15	-15		FRONT	VERT	TOTAL		C1
Z	1-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AA	3-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AB	6-11-4	-164	-164		FRONT	VERT .	TOTAL		C1
AC	8-11-4	-164	-164		FRONT	VERT	TOTAL		C1
AD	10-11-4	-164	-164		FRONT	VERT	TOTAL		C1
ΑE	12-11-4	-164	-164		FRONT	VERT	TOTAL		C1
AF	14-11-4	-164	-164		FRONT	VERT	TOTAL		C1
AG	16-11-4	-164	-164		FRONT	VERT	TOTAL		C1
AH	18-11-4	-164	-164		FRONT	VERT	TOTAL		Čí

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB DESC. QUANTITY PLY **BAYVIEW WELLINGTON** DRWG NO. 417701 2 TRUSS DESC. T1S HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:35:07 2022 Page 1 $ID_JvMhHdWEgsSI57rNnlib4czW6nI-jAVeKAJUgPsEwK6XHKbuKcbT7GzZeZlcXj5bHzznGTchanner (New York North New York North North$ 5-10-8 23-3-0 5-10-8 Scale = 1:61. 2x4 || 6x10 \\ 3x6 5x6 = 7x8 // D E G Н 9.00 12 4x4 | 5-11-2 5x8 WHO) | 1-6-6 K | 4-6-6 1-6-4 P 0 ₩ X 6x12 AF AG Ah Al U ΑJ AL 5x6 || 5x6 = w s R М 3x6 || 5x6 =5x6 = 5x6 = 6x12 3x6 || 3x6 || 3x6 || 4x6 || 1-11-4 2-0-0 16-11-4 1-9-0 10-9-0 1-7-8 35-0-0 1-3-8 TOTAL WEIGHT = 2 X 187 = 375 lb IMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING BEARINGS FACTORED **BUILDING DESIGNER DESIGN CRITERIA** CHORDS SIZE LUMBER DESCR DRY DRY DRY No.2 No.2 SPF MAXIMUM FACTORED INPUT REQRD SPECIFIED LOADS: GROSS REACTION GROSS REACTION BBG BRG CH. 325 PSE Н 2x4 6.0 0.0 7.4 No.2 SPF VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX 2x4 2x6 2x4 SPF SPF SPF DRY No.2 No.2 5-8 5-8 0 BOT CH. LL **PSF** DRY 4173 5-8 DI PSE TOTAL No.2 2x6 2x6 DRY DRY No.2 No.2 SPF UNFACTORED REACTIONS
1ST LCASE ____MA SPACING = 24.0 IN. C/C DRY 2x6 No.2 SPF ./MIN. COMPONENT REACTIONS SNOW LIVE 0/0 2x4 DRY No.2 SPF COMBINED PERM.LIVE DEAD WIND SPF 2135 / 0 0/0 0/0 904 / 0 827 / 0 0/0 LOADING IN FLAT SECTION BASED ON A SLOPE 2093 / 0 0/0 0/0 0/0 OF 6.00/12 ALL WEBS DRY 2x3 No 2 SPF BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, L THIS TRUSS IS DESIGNED FOR RESIDENTIAL No.2 SPF E Q OR SMALL BUILDING REQUIREMENTS OF PART BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED. 2x6 DRY No.2 SPF DRY SPF THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) DBY: SEASONED LUMBER MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT CSA 086-14 DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. - TPIC 2014 FOLLOWS: LOADING TOTAL LOAD CASES: (4) DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. CHORDS #ROWS SURFACE LOAD(PLF) SPACING (IN) CHORDS MAX. FACTORED WEBS (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. FACTORED MAX. FACTORED RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED A- B B- F F- H SIDE(61.0) MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. **FORCE** MAX ROOF LIVE LOAD SIDE(61.0) TOP TOP (LBS) (PLF) FROM TO CSI (LC) UNBRAC LENGTH FR-TO ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/987 (0.43") A- B B- Z Z-AA H-K 5234 / 0 -112.4 -112.4 12 12 0.69 (1) 3.70 W-R -544 / 34 -112.4 -112.4 -112.4 -112.4 0.57 0.57 Q-G P-G TOP 6435 / 0 0/3306 -2919/0 BOTTOM CHORDS: (0.122"X3") SPIRAL NAILS 0/138 6435 / 0 0- H -112.4 -112.4 0.57 3.36 0.02 (4) C-AB AB-AC AC- D D-AD -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 3.02 3.02 -270 / 0 0 / 4252 0.05 (1) 0.53 (1) U- X U- R SIDE(183.1) 7624 / 0 0.66 CSI: TC=0.78/1.00 (D-E:1) , BC=0.51/1.00 (P-Q:1) , WB=0.69/1.00 (D-T:1) , SSI=0.21/1.00 (B-C:1) SIDE(183.1) TOP -7624 / 0 Q-N 7624 / 0 S-Q Q-E 0.66 (1) 3.02 0/2091 0.32 (1) TOP DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 12 -9575 / 0 -112.4 -112.4 0.78 (1) 2.60 -588 / 0 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 0.78 (1) 0.78 (1) 0.78 (1) 0.54 (1) 2.60 2.60 0 / 3353 -2666 / 0 I- M TOP AD-AF -9575 / 0 B-WEBS (0.122"X3") SPIRAL NAILS 0.69 (1) W-B SIDE(9.9) E-F F-G -9615 / 0 2.81 -2339 / 0 0.61 (1) COMPANION LIVE LOAD FACTOR = 1.00 2x3 -9615 / 0 0.54 2.81 3.36 0 / 1798 0 / 4003 G- H H- I I- J E-S SIDE(336.6) -7404 / 0 0.37 AUTOSOLVE HEELS OFF 5896 / 0 -112.4 -112.4 0.36 (1) 3.77 T-Q 0 / 7623 0.50 (1) TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. 2x6 -5990 / 0 -112.4 -112.4 -112.4 -112.4 0.19 (1) 3.84 D- Q 0/2600 0.32 (1 0.08 -225 / 0 0 / 4796 0.0 0.0 0.15(1) 6.94 N-J 0.42 (1) L-J 4029 / 0 0.0 0.0 PROFESSIONAL ENGINEERS H. J. G. ALVES X-AF AF-AG -18.5 -18.5 0.07 (4) 10.00 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873 -18.5 0/0 0.07(4)10.00 -18.5 -18.5 -18.5 -18.5 0.07 (4) 0.30 (1) AG- W 0/0 10.00 W-AH AH-AI 0 / 4170 0 / 4170 -18.5 -18.5 0.30(1) 10.00 -18.5 -18.5 0 / 4170 -18.5 0.30 (1) 10.00 PLATE PLACEMENT TOL. = 0.250 inches V- U U-AJ 0 / 6436 -18.5 PLATE ROTATION TOL. = 5.0 Deg 0 / 6436 -18.5 -18.5 0.45 (1) 10.00 100009024 AJ- T 0 / 6436 -18.5 -18.5 0.45 (1) 10.00 0 / 181 0 / 181 0 / 181 -18.5 -18.5 -18.5 T-AK AK-AL -18.5 0.05 (4) JSI GRIP= 0.88 (C) (INPUT = 0.90) -18.5 10.00 JSI METAL= 0.70 (J) (INPUT = 1.00) AL-S S-R Q-P P-O M-N -18.50.05 (4) 10.00 POWNEE OF ONTARIO 0 / 0 0 / 7404 -18.5 -18.5 -18.5 -18.5 10.00 0.51 (1) 10.00 0 / 4699 -18.5-18.50.33 (1) 10.00 -18.5 0.0 0.34 (1) 0 / 4957 -18.5 10.00 0 / 15 10.00 -231 / 0 N-I 0.0 0.0 0.14 (1 7.81 0 / 184 M- L -18.5 0.01 (1) 10.00 Structural component only DWG# T-2204204 CONTINUED ON PAGE 2

417701

T1S

2

JOB DESC. BAYVIEW WELLINGTON

TRUSS DESC. HOMES

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:35:08 2022 Page 2 ID:JvMhHdWEgsSl57rNnlib4czW6nl-BN30XWK6Ri 5YUhjr267tq8etqJoN0?lmNr8qPznGTn

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

	PLA	TES (table is i	n inches)				
	JT		LATES	W	LEN	Υ	Х
١,	Α	TMVW-p	MT20	5.0	8.0	Edge	
٠	В	TTWW+m	MT20	6.0	10.0	Edge	2.00
	С	TMWW-t	MT20	4.0	4.0		
	D	TMWW-t	MT20	4.0	6.0		
	E	TMW+w	MT20	2.0	4.0		
	F	TS-t	MT20	3.0	6.0		
,	G	TMWW-t	MT20	5.0	6.0		
ı	Н	TTWW+m	MT20	7.0	8.0	Edge	2.50
	ı	TMVW+p	MT20	4.0	4.0	1.00	2.00
١,	J	TMVW-t	MT20	5.0	6.0	2.25	2.75
ı	L	BMVW1+p	MT20	4.0	6.0		
	M ·	BMV+p	MT20	3.0	6.0		
	N	BVMWW-I	MT20	6.0	12.0	3.75	8.00
	0	BMWW-t	MT20	5.0	6.0		
	Ρ	BMWW+t	MT20	5.0	6.0	3.00	2.25
	Q	BWMWWW*-I		8.0	12.0	5.00	4.50
	R	NP+w	MT20	3.0	6.0		
	s	BMW+w	MT20	3.0	6.0		
1	Т	BMWWW-t	MT20	6.0	12.0	3.00	4.25
	U	BS-t	MT20	5.0	6.0		
	٧	BMWW-t	MT20	5.0	6.0		
	W	BMWW-t	MT20	5.0	6.0	2.50	2.75
	Χ	BMV1+p	MT20	3.0	6.0		
1	Υ	NP+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

١											
	SPE	CIFIED CO	NCENTRA	TED LOA	DS (LBS)						
l	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	
ı	В	5-10-8	-347	-347		FRONT	VERT	TOTAL		C1	
l	С	10-11-4	-93	-93		FRONT	VERT	TOTAL		C1	
I	S	20-10-8	-1388	-1388		FRONT	VERT	TOTAL		C1	
ı	U	12-11-4	-22	-22		FRONT	VERT	TOTAL		C1	
ı	V	10-11-4	-22	-22		FRONT	VERT	TOTAL		C1	
ļ	W	5-11-4	-22	-22		FRONT	VERT	TOTAL		C1	
l	Z	6-11-4	-93	-93		FRONT	VERT	TOTAL		C1	
l	AA	8-11-4	-9 3	-93		FRONT	VERT	TOTAL		C1	
ļ	AB	12-11-4	-93	- 9 3		FRONT	VERT	TOTAL		C1	
	AC	14-11-4	-93	- 9 3		FRONT	VERT	TOTAL		C1	
	AD	16-11-4	-93	-93		FRONT	VERT	TOTAL		C1	
	ΑE	18-11-4	-93	-93		FRONT	VERT	TOTAL		C1	
	ΑF	1-11-4	-29	-29		FRONT	VERT	TOTAL		C1	
	AG	3-11-4	-29	-29		FRONT	VERT	TOTAL		C1	
	AH	6-11-4	-22	-22		FRONT	VERT	TOTAL		C1	
	Al	8-11-4	-22	-22		FRONT	VERT	TOTAL		C1	
	AJ	14-11-4	-22	-22		FRONT	VERT	TOTAL		C1	
	AK	16-11-4	-22	-22		FRONT	VERT	TOTAL		C1	
	AL	18-11-4	-22	-22		FRONT	VERT	TOTAL		C1	

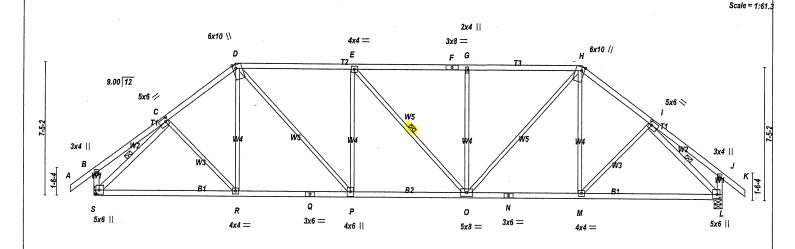
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only DWG# T-2204204 V/L

JOB DESC. BAYVIEW WELLINGTON PLY DRWG NO. 417704 **T2** TRUSS DESC. HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:16 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-fbuGVYlwSak7L_vSjLOpZs9IRZDY8BZy9cRhJYznTsX 1-3-8 7-10-8 19-3-0



LUMBE N I G A BULES CHORDS LUMBER DESCR SPF SPF SPF DRY D F 2x4 No.2 2100F 1.8E 2x4 DRY 2x4 2x4 2x4 2x4 H K B DRY 2100F 1.8E No.2 SPF SPF SPF SPF SPF SPF DRY No.2 No.2 ZDOL 2x4 2x4 2x4 2x4 DRY No.2 N DRY No 2 ALL WEBS 2x3 DRY No.2 SPF EXCEPT SPF No.2 DRY 2x4 No.2 SPF

DRY: SEASONED LUMBER.

1-3-8

PL	ATES (table i	s in inches)				
JT	TYPE	PLATES	W	LEN	Υ.	Х
В	TMV+p	MT20	3.0	4.0		
С	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	6.0	10.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0	-	
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
Н	TTWW+m	MT20	6.0	10.0	Edge	2.00
1	TMWW-t	MT20	5.0	6.0	2.50	1.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1+p	MT20	5.0	6.0	2.75	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
0	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
s	BMVW1+p	MT20	5.0	6.0	2.75	2.00
i						

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only DWG# T-2204181

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
	THE LOCAL COLUMN STATE OF THE STATE OF THE SERVICE
BUILDING DESIGNER	
REARINGS	

35-0-0

35-0-0

ΞA	RINGS						
	FACTOR	ED	MAXIMUN	/ FACTO	RED	INPUT	REORD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	2446	0	2446	0	0	MECHANIC	AL
	2446	0	2446	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT S. MINIMUM BEARING LENGTH AT JOINT S = 3-8.

UNFACTORED REACTIONS | MAX./MIN. COMPONENT REACTIONS | SNOW LIVE PERM.LIVE WIND | 228 / 0 0 / 0 0 / 0 0 / 0 | 226 / 0 0 / 0 0 / 0 0 / 0 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / COMBINED 486 / 0 1712

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, C-S, I-L.

1226 / 0

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

<u>LOADING</u> TOTAL LOAD CASES: (4)

СН	ORDS					W E	BS	
MAX	C. FACTORED	FACTOR	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LOA	AD LC1	MAX	MAX.	MEMB.		MAX
	(LBS)				UNBRAC		(LBS)	
FR-TO		FROM	TO		LENGTH	FR-TO		
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00	C-R	0 / 65	0.02 (4)
B- C	0 / 30	-112.4	-112.4	0.27(1)	10.00	R-D	0/129	0.04 (4)
	-2508 / 0	-112.4	-112.4	0.38 (1)	4.00	D- P	0/1200	0.27 (1)
D- E	-2786 / 0			0.52 (1)		P-E	-780 / 0	0.77(1)
E-F	-2785 / 0	-112.4	-112.4	0.52 (1)	4.47	E-O	-2/0	0.00(1)
F- G	-2785 / 0			0.52 (1)		0- G	-780 / 0	0.77 (1)
G- H	-2785 / 0			0.52 (1)		O- H	0/1198	0.27(1)
H- I	-2508 / 0			0.38 (1)		M- H	0 / 130	0.04 (4)
l- J	0 / 30			0.27 (1)		M-I	0 / 65	0.02 (4)
J- K	0 / 47			0.15 (1)		S-C	-2839 / 0	0.48 (1)
	-326 / 0	0.0		0.03 (1)		I- L	-2839 / 0	0.48 (1)
L- J	-326 / 0	0.0	0.0	0.03 (1)	7.81			
_								
S-R	0 / 1953			0.46 (1)				
R-Q	0 / 1981			0.46 (1)				
Q-P	0 / 1981			0.46 (1)				
P- O	0 / 2787			0.52 (1)				
0- N	0 / 1981			0.46 (1)				
N- M	0 / 1981			0.46 (1)				
M- L	0 / 1953	-18.5	-18.5	0.46 (1)	10.00			

DESIGN CRITERIA

SPEC	IFIED	LOAD	os:		
TOP	CH.	LL	=	32.5	PSF
		DL		6.0	PSF
BOT	CH.			0.0	PSF
		DL		7.4	PSF
TOTA	L LO	AD	=	45.9	PSF

7-10-8

1-3-8

1-3-8 TOTAL WEIGHT = 157 lb

SPACING = 24.0 IN. C/C

0/0

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) CSA 086-14 - TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL)= L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL)= L/999 (0.25")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.52/1.00 (O-P:1) , WB=0.77/1.00 (E-P:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 MT20

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

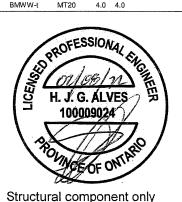
JSI GRIP= 0.80 (H) (INPUT = 0.90) JSI METAL= 0.69 (L) (INPUT = 1.00)

JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. HOMES T2S 417701 1 Version 8.420 S Jan 21 2021 MITTER Industries, Inc. Tue Feb 8 08:35:08 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-BN30XWK6Ri_5YUhjr267tq8iagJtNz_lmNr8qPznGTn Tamarack Roof Truss, Burlington 7-10-8 19-3-0 1-3-8 Scale = 1:62. 6x10 \\ 4x4 = 4x4 =3x6 = 3x4 || н ⁷х8 // C D E G 9.00 12 5x6 // 4x4 💸 В 3x4 || J 5x6 <> 3y4 || W12 K 徶 1.6.4 B2 W7 Bro 4x4 = V U s R Ν 5x6 = 3x6 = 6x10 = 4x4 =4x6 || 3x4 || 3x4 || 22-7-8 10-9-0 1-7-8 35-0-0 1-3-8 TOTAL WEIGHT = 172 lb LUMBER DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY N. L. G. A. RULES **BUILDING DÉSIGNER DESIGN CRITERIA** BEARINGS FACTORED CHORDS SIZE LUMBER DESCR No.2 No.2 No.2 No.2 SPF MAXIMUM FACTORED INPUT REQRD SPECIFIED LOADS: ACFHWMWTRQNN REACTION BRG BR HORZ UPLIFT IN-SX IN-0 0 MECHANICAL LL = LL = DRY GROSS REACTION GROSS REACTION BRG TOP CH. 32.5 PSF VERT 2288 6.0 0.0 7.4 PSF PSF Н 2x4 SPF SPF SPF SPF SPF HORZ DOWN IN-SX 2x4 2x4 2x4 DRY DRY DRY 2288 2449 0 0 PSF AKTRG No.2 5-8 DL No.2 TOTAL LOAD 2x4 2x4 2x4 2x4 No.2 No.2 A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT W. MINIMUM BEARING LENGTH AT JOINT W = 3-8. SPACING = 24.0 IN. C/C SPF DRY No.2 O J M 2x4 2x4 No.2 No.2 SPF SPF DRY LOADING IN FLAT SECTION BASED ON A SLOPE UNFACTORED REACTIONS

1ST LCASE MAX

JT COMBINED SNOW 2x4 DRY No.2 SPE OF 6.00/12 MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE WIND ALL WEBS EXCEPT W - B 2x3 DRY No.2 SPF LIVE 0/0 SOIL THIS TRUSS IS DESIGNED FOR RESIDENTIAL 1135/0 0/0 0/0 469 / 0 0/0 OR SMALL BUILDING REQUIREMENTS OF PART W - B O - M O - K S - Q DRY 2x4 No.2 SPF 1228 / 0 0/0 0/0 0/0 2x4 2x4 No.2 No.2 SPF BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) 2x4 DRY No.2 SPF BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT. DRY: SEASONED LUMBER. - CSA 086-14 MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED. - TPIC 2014

	PL	ATES (table is	s in inches)					
	JT	TYPE	PLATES	w	LEN	Υ	Х	
	Α, (G, J						
	Α	TMV+p	MT20	3.0	4.0			
	В		MT20	5.0	6.0	2.50	2.25	
ŀ	С		MT20	6.0	10.0	Edge	2.00	
	D, I							
i	D	TMWW-t	MT20	4.0	4.0			
	F		MT20	3.0	6.0			
	Н	TTWW+m	MT20	7.0	8.0	Edge		
	K	TMVW-t	MT20	5.0	6.0	2.25	2.50	
	М	BMVW1-t	MT20	4.0	6.0			
	N	BMV+p	MT20	3.0	4.0			
	0	BVMWWW-I		6.0	12.0	3.00	4.75	
	Р	BMWW-t	MT20	4.0	4.0			
I	Q	BVMWWW-I		6.0	12.0	3.00	4.25	
I	R	BMV+p	MT20	3.0	4.0			
	S	BMWWW-t	MT20	6.0	10.0	2.50	3.25	
	Т	BS-t	MT20	3.0	6.0			
	U	BMWW+t	MT20	4.0	6.0			



DWG# T-2204205

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

ATERAL BRACE(S) AT 1/2 LENGTH OF B-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

IOIAL	TOTAL LOAD CASES: (4)							
	ORDS					W E	BS	
MAX	C. FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LC			MAX.	MEMB.		MAX
	(LBS)			CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM			LENGTH			
A-B	0 / 30			0.27 (1)	10.00	B-V		0.02 (4)
	-2503 / 0			0.38 (1)		V-C		0.04 (4)
C- D	-2641 / 0			0.53 (1)		Q-H	0/1236	0.28 (1)
D- E	-2832 / 0			0.54 (1)		P- H	0/334	0.08 (1)
E-F	-3023 / 0			0.52 (1)		P-1	-337 / 0	0.15 (1)
F- G	-3023 / 0			0.52 (1)		1-0	-302/0	0.08 (1)
G-H	-3032 / 0			0.51 (1)			-2834 / 0	0.48 (1)
H- I	-2880 / 0			0.37 (1)		O- M	-105/0	0.01 (1)
I- J	-2896 / 0			0.32 (1)		0- K	0/2313	0.37 (1)
J- K	-2942 / 0			0.24 (1)		C- n	0/1167	0.26 (1)
K-L	0 / 47			0.15 (1)			-747 / 0	0.74 (1)
	-171 / 0	0.0		0.02 (1)		U-D	-898/0	0.89 (1)
M-K	-2377 / 0	0.0	0.0	0.25 (1)	5.54	D-S		0.08 (1)
w-v	0./1050	10.5	40.5	0 45 (4)	40.00	S-Q		0.46 (1)
V- U	0 / 1950 0 / 1977	-18.5 -18.5		0.45 (1)		E-Q	0/296	0.07 (1)
U- T	0 / 2641	-18.5 -18.5		0.46 (1)				
T- S	0 / 2641	-18.5 -18.5		0.48 (1)				
S- R	0 / 57	-18.5		0.48 (1) 0.12 (4)				
R-Q	0 / 40	0.0		0.12 (4)				
Q-G	-588 / 0	0.0		0.39 (1)				
Q-P	0 / 2285	-18.5		0.46 (1)				
P- O	0 / 2546	-18.5		0.50 (1)				
N- O	0 / 15	0.0		0.12 (1)				
0-J	-51 / 0	0.0		0.12 (1)				
N- M	0 / 87	-18.5		0.02 (1)				
1	5 / 6/	, 0.0	. 0.0	5.54 (1)				

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL)= L/999 (0.28")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.50/1.00 (O-P:1) , WB=0.89/1.00 (D-U:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Dea.

JSI GRIP= 0.89 (K) (INPUT = 0.90) JSI METAL= 0.78 (T) (INPUT = 1.00)

ЈОВ NAME 417701	TRUSS NAME T2S	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.			DRWG NO.
amarack Roof Truss, Burling	ton				Vers ID:JvMhHdWEgsSl57r		ek Industries, Inc. Tue Feb 8 08:35:09 202 206yAeGvOldMP1gtK4f66QDu 1ahN
PLATES (table is in inches)	. W LENY X					·	
W BMVW1-t MT20 Edge - INDICATES REFERE TOUCHES EDGE OF CHOI	5.0 6.0 2.50 2.75						
	RD.						
NOTES- (1) 1) Lateral braces to be a mir	imum of 2X4 SPF #2.						
						·	
			2				
	•						
						-	
•							
_		-					
PROFES	SIONALE						
S Corl	16/11/2 K						
H. J. G	SSIONAL ELGINAL ELGINA						
							·
Pound	OF ON ARIO						
No.	ON.						
Structural con DWG# T-220	nponent only						

JOB DESC. **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. 417704 ТЗ HOMES Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 07:46:31 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-5BIHK50OOST_5CB5SwH38hqCtpmNQgOO;2kly_znHBM Tamarack Roof Truss, Burlington 2-10-8 7-0-0 9-10-8 14-3-0 1-0-0 1-3-8 Scale = 1:62. 2x4 || 5x8 ~ 5x8 \\ F c 5x6 > F 5x6 // 9.00 12 G 4x4 // В 5x6 < Н 3x4 || 1-6-4 W2 P N R ٥ 0 L 4x6 || 3x6 = 3x6 = 5x6 || 5x6 = 4x4 = 4x10 =4x4 = 5x6 =35-0-0 1-3-8 35-0-0 TOTAL WEIGHT = DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY LUMBER N. L. G. A. CHORDS MIF BUILDING DESIGNER DESIGN CRITERIA LUMBER DESCR BEARINGS FACTORED SIZE - C - E - F SPECIFIED LOADS: MAXIMUM FACTORED INPUT REQRD ACHFGGKGP DRY No.2 SPF LOADS: LL = 32.5 DL = 6.0 LL = 0.0 DL = 7.4 AD = 45.9 2x4 2x4 DRY SPF GROSS REACTION VERT HORZ GROSS REACTION DOWN HORZ U BRG IN-SX BRG IN-SX 2100F 1.8E CH. PSF UPLIFT PSF BOT CH. - G 2x4 DRY No.2 SPF 2291 2291 0 MECHANICAL SPF SPF SPF SPF SPF 2x4 2x4 DRY DRY No.2 No.2 2446 ō 2446 ō ō 5-8 5-8 PSF TOTAL LOAD A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT S. MINIMUM DRY 2x4 No.2 DRY No 2 BEARING LENGTH AT JOINT S = 3-8. SPACING = 24.0 IN. C/C DRY N 2x4 No.2 LOADING IN ALL FLAT SECTIONS BASED ON A ALL WEBS EXCEPT DRY No.2 SPF UNFACTORED REACTIONS SLOPE OF 6.00/12 2x3 MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE C - O O - E H - K WIND SOIL 0/0 0/0 THIS TRUSS IS DESIGNED FOR RESIDENTIAL DRY SPF DEAD 2x4 No.2 COMBINED 2x4 2x4 DRY No.2 No.2 SPF 1606 1137 / 0 0/0 0/0 0/0 469 / 0 OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K THIS DESIGN COMPLIES WITH: DRY: SEASONED LUMBER. - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT. - CSA 086-14 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. - TPIC 2014 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED 1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O. ROOF LIVE LOAD

١	PLATES (table is in inches)						
1	JT	TYPE	PLATES	W	LEN	Υ	X
1	A	TMVW-p	MT20	5.0	8.0	1.50	Edge
١	В	TMWW-t	MT20	4.0	4.0	2.00	1.50
1	С	TTWW+m	MT20	5.0	8.0	Edge	2.00
I	D	TMW+w	MT20	2.0	4.0	-	
l	E	TTWW-m	MT20	5.0	8.0	Edge	
١	F	TTWW+m	MT20	5.0	6.0	3.00	2.25
ı	G	TTW-m	MT20	5.0	6.0	Edge	
-	Н	TMWW-t	MT20	5.0	6.0	2.50	1.75
ı	1	TMV+p	MT20	3.0	4.0		
1	K	BMVW1+p	MT20	5.0	6.0	2.75	
	L	BMWWW-t	MT20	5.0	6.0	2.25	1.50
ı	M	BMWW-t	MT20	4.0	4.0		
ı	Ν	BS-t	MT20	3.0	6.0		
	0	BMWWW-t	MT20	4.0	10.0		
i	Р	BS-t	MT20	3.0	6.0		
	Q	BMWW-t	MT20	4.0	4.0		
	R	BMWW-t	MT20	5.0	6.0		2.25
	s	BMV1+t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



Structural component only DWG# T-2204182

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

	HORDS					WE	BS		
M/	XX. FACTORED	FACTOR	ED				MAX. FACTO	RED	
MEMB	. FORCE	VERT. LOA	D LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF	•) (CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TC) ' '	FROM 1	Ó		LENGTH				
A-B	-2534 / 0	-112.4 -	112.4	0.46 (1)	3.92	R- B	-396 / 0	0.17(1)	
B- C	-2406 / 0	-112.4 -	112.4	0.44 (1)	4.02	B-Q	-240 / 0	0.22(1)	
C- D	-2410 / 0	-112.4 -	112.4	0.53(1)	4.69	Q-C	0 / 284	0.06(1)	
D- E	-2410 / 0	-112.4 -	112.4	0.53(1)	4.69	C- O	0/813	0.13(1)	
E-F	-2497 / 0	-112.4 -	112.4	0.19 (1)	4.22	O- D	-984 / 0	0.51(1)	
F-G	-2017 / 0	-112.4 -	112.4	0.06(1)	4.74	0- E	0/647	0.10(1)	
G- H	-2526 / 0	-112.4 -	112.4	0.23 (1)	4.16	M- E	0/540	0.12(1)	
H- I	0 / 24	-112.4 -	112.4	0.19 (1)	10.00	L- G	0 / 1255	0.28 (1)	
l- J	0 / 47	-112.4 -	112.4	0.15(1)	10.00	L- H	0/137	0.03(1)	
S-A	-2249 / 0	0.0	0.0	0.24(1)	5.68	A-R	0/2115	0.48 (1)	
K-I	-311 / 0	0.0	0.0	0.03(1)	7.81	H- K	-2818 / 0	0.86 (1)	
						F-L	-1248 / 0	0.98 (1)	
S-R	0/0	-18.5	-18.5	0.10 (4)	10.00	M- F	-498 / 0	0.49(1)	
R-Q	0 / 2058	-18.5	-18.5	0.41 (1)	10.00				
Q-P	0 / 1892	-18.5	-18.5	0.42 (1)	10.00				
P-0	0 / 1892	-18.5	-18.5	0.42 (1)	10.00				
0- N	0 / 1998	-18.5	-18.5	0.44 (1)	10.00				
N- M	0 / 1998	-18.5	-18.5	0.44 (1)	10.00				
M- L	0 / 2197	-18.5	-18.5	0.44 (1)	10.00				
L- K	0 / 1902	-18.5	-18.5	0.42 (1	10.00				
NO.	NOTES-(1)								
	Lateral braces to be a minimum of 2V4 CDE #2								

Lateral braces to be a minimum of 2X4 SPF #2

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.23")

CSI: TC=0.53/1.00 (D-E:1) , BC=0.44/1.00 (L-M:1) , WB=0.98/1.00 (F-L:1) , SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873 MT20

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90) JSI METAL= 0.70 (N) (INPUT = 1.00)

JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. HOMES 417701 T3S Tamarack Roof Truss, Burlingtor Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:44:18 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-4Blxza_6viH3X0kQ3Y_59owfCO0hFmYiEy4F8XznGLE 9-10-8 14-3-0 2-10-8 7-0-0 Scale = 1:64. 5x6 = 4x4 = 3x4 || 5x8 > c D E 5x6 > G ^{5x6 ≥} 9.00 12 н 5x6 <> 5x8 1 5x8 🔷 W15 J W14 1-6-4 W2 B2 WA 0 6x12 == T R 6x12 = 5x6 =V U s M 4x6 | 3x6 5x6 = 4x4 =6x10 == 3x4 || 4x4 == 3x4 || 4x6 =22-7-8 10-9-0 1-7-8 35-0-0 1-3-8 TOTAL WEIGHT = 188 Ib LUMBER N. L. G. A CHORDS DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY A RIIIES BUILDING DESIGNER **DESIGN CRITERIA** DESCR BEARINGS FACTORED A C F G H W SPF SPF SPF C 2x4 DRY No.2 MAXIMUM FACTORED INPLIT REORD SPECIFIED LOADS: DRY DRY DRY REACTION GROSS REACTION BRG DOWN HORZ UPLIFT IN-SX No.2 GROSS LL = DL = LL = 32.5 PSF CH. 2x4 2x4 2x4 2x4 2x4 2x4 2x4 No.2 VERT G H K A IN-SX 6.0 0.0 7.4 PSF No.2 SPF 2288 0 2288 0 0 MECHANICAL BOT CH LL DL PSF PSF DRY No.2 No.2 SPF SPF SPF 2449 Ö = DRY DRY DRY TOTAL LOAD 45.9 PSF A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT W. MINIMUM L -W -R -M -No.2 SPF SPF SPF No.2 BEARING LENGTH AT JOINT W = 3-8. SPACING = 24.0 IN. C/C 2x4 2x4 DRY DRY No.2 No.2 R Ν 2x4 DRY No.2 SPF LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 SPF 2x4 DRY No.2 UNFACTORED REACTIONS
1ST LCASE ____MA MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERMITIVE V SOIL 0/0 0/0 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART COMBINED WIND DEAD ALL WEBS 2x3 DRY No.2 SPF 1604 1714 1135 / 0 0/0 0/0 0/0 EXCEPT 1228 / 0 9. NBCC 2015 486 / 0 DRY No.2 SPF C S SQ 2x4 DRY No.2 SPF BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 2x4 DRY No.2 SPF 2x4 DRY No.2 <u>BRACING</u>
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT. - PART 9 OF OBC 2012 (2019 AMENDMENT) DRY: SEASONED LUMBER MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. - TPIC 2014 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, D-S. (55.% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD PLATES (table is in inches)
JT TYPE PLATES END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LEN Y TMVW-p TMWW-t 8.0 1.50 Edge 2.00 1.50 MT20 MT20 ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL)= L/999 (0.28") 5.0 LOADING TOTAL LOAD CASES: (4) TTWW-m MT20 6.0 4.0 8.0 6.0 6.0 6.0 2.00 2.00 TMWW-t MT20 4.0 3.0 5.0 5.0 5.0 5.0 TMV+p TTWW-m Edge 3.50 2.50 2.75 CHORDS MT20 WEBS MT20 MT20 MT20 MAX. FACTORED EMB. FORCE (LBS) FACTORED VERT. LOAD LC1 (PLF) C FROM TO MAX. FACTORED FORCE MA G TTWW-m TTW-m TMVW-t мемв. МЕМВ. MAX. CSI: TC=0.64/1.00 (C-D:1) , BC=0.56/1.00 (N-O:1) , WB=0.67/1.00 (G-O:1) , SSI=0.34/1.00 (C-D:1) Edge 2.50 2.50 MAX CSI (LC) UNBRAC (LBS) CSI (LC) FROM TO -112.4 -112.4 0.46 (1) -112.4 -112.4 0.44 (1) LENGTH FR-TO 3.92 V- B 4.02 B- U 5.0 FR-TO TMVW-t MT20 8.0 BMVW1-t BMV+p BVMWW-l 6.0 4.0 12.0 4.00 7.75 MT20 MT20 4.0 3.0 A- B B- C C- D E- F F- G H- I -2531 / 0 0.16(1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 -2399 / 0 -244 / 0 0.23 (1 COMP=1.10 SHEAR=1.10 TENS= 1.10 3.72 3.71 4.15 0.64 (1) 0.63 (1) 0/274 0/810 0.06 (1) MT20 6.0 -2364 / 0 -112.4 -112.4 6.0 2.25 1.50 4.0 12.0 3.00 4.75 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 BMWWW-t MT20 5.0 4.0 -2426 / 0 -2431 / 0 C-S S-D COMPANION LIVE LOAD FACTOR = 1.00 BMWW-t BVMWWW-I 0.28 (1) -890 / 0 0.46 (1) Q R S S-Q D-Q Q-F P-F 0/2356 0/83 0/885 MT20 6.0 -2795 / 0 0.21 (1) 4 00 0.38 (1 AUTOSOLVE LEFT HEEL ONLY -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 4.41 3.49 3.36 4.0 10.0 3.00 3.00 -2393 / 0 -2954 / 0 0.07 MT20 3.0 BMWWW-t TRUSS PLATE MANUFACTURER IS NOT 0.20(1)RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. I- J -3572 / 0 0.40 (1) 0/659 0.15 (1 P- G O- H O- I -112.4 -112.4 0.0 0.0 0.15 (1) -712/0 0/1441 0.49 (1) 0.32 (1) J- K 0 / 47 10.00 W-A PROFESSIONAL ENGRED H. J. G. ALVES -2357 / 0 0.0 0.0 -688 / 0 L-J 0.25(1)5.56 0.40 (1) NAIL VALUES 0.48 (1) 0/2112 -18.5 -18.5 -18.5 N- L N- J W- V -18.5 0 / 2056 -18.5 0.39 (1) 10.00 0 / 2906 U- T T- S S- R R- Q 0 / 1887 -185 0 41 (1) 10.00 G- O -1283 / 0 0.67(1) 0 / 1887 0 / 47 -18.5 -18.5 -18.5 0.41 (1) 0.19 (4) -18.5 10.00 PLATE PLACEMENT TOL. = 0.250 inches 0 / 52 0.0 0.0 0.07 (1) 10.00 -499 / 0 0.0 -18.5 0.0 -18.5 0.08 (1) 0.37 (1) 6.25 PLATE ROTATION TOL. = 5.0 Deg. 0 / 2245 100009024 0 / 2577 -18.5 -18.5 0.48 (1) 10.00 JSI GRIP= 0.90 (G) (INPUT = 0.90) JSI METAL= 0.66 (J) (INPUT = 1.00) 0 / 3010 0 / 15 -18.5 0.0 -18.5 0.0 0.56 (1) 0.16 (1) 10.00 FOUNDE OF ONTRE 0 / 56 0.0 0.0 0.16(1) 10.00 0 / 100 -18.5 0.02(1) 10.00 Structural component only DWG# T-2204206 CONTINUED ON PAGE 2

PLY JOB DESC. BAYVIEW WELLINGTON DRWG NO. TRUSS DESC. HOMES 1 417701 T3S Version 8.420 S Jan 21 2021 MITek Industries, Inc. Tue Feb 8 08:44:18 2022 Page 2 ID:JvMhHdWEgsSl57rNnlib4czW6nl-4Blxza_6viH3X0kQ3Y_59owfCO0hFmYiEy4F8XznGLB Tamarack Roof Truss, Burlington
 PLATES (table is in inches)

 JT
 TYPE
 PLATES

 T
 BS-t
 MT20

 U
 BMWW-t
 MT20

 V
 BMWW-t
 MT20

 W
 BMV1+t
 MT20
 LEN Y X 6.0 4.0 6.0 2.50 2.50 6.0 3.50 W 3.0 4.0 5.0 4.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2. PROFESSIONAL FINGUES TO THE PROPERTY OF THE PR 100009024 PONNOE OF ONTARIO Structural component only DWG# T-2204206 WV

JOB DESC. **BAYVIEW WELLINGTON IPLY** DRWG NO. 417704 **T4** 2 TRUSS DESC. HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:19 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-3AaO7anplV6h9Sd1OTxWBVnqtmHBLa6PrZgLwtznTsU 5-10-8 1-3-8 1-6-12 2-0-0 7-11-4 6-0-0 1-3-8 Scale = 1:84. 7x8 = 3x6 || 6x10 // Н 7x8 \\ 4x6 || 6x10 // 9.00 12 10-5-2 5-11-2 6x10 / 5x8 📏 5.2.2

AA Τ U s R P ۵ n M 3x6 || 5x8 = 7x8 || 4x6 || 7x8 =8x9 || 4x6 || 6x10 =3x6 || 2425A ZH25A

1-11-4 2-0-0 2-0-0 1-6-0 2-0-0 1-11-4

35-1-8

1-3-8 TOTAL WEIGHT = 2 X 323 = 646 lb

	LUMBER					DIN
	N. L. G. A. R					BU
	CHORDS	SIZE		LUMBER	DESCR.	BE
	A - C	2x6	DRY	No.2	SPF	
	C - E	2x6	DRY	No.2	SPF	
ı	E - F	2x6	DRY	No.2	SPF	JT
	F - H	2x6	DRY	No.2	SPF	U
	H - I	2x6	DRY	No.2	SPF	M
	1 - J	2x6	DRY	No.2	SPF	
	J - L	2x6	DRY	No.2	SPF	PR
	U - B	2x6	DRY	No.2	SPF	PR
Ì	M - K	2x6	DRY	No.2	SPF	
	U - M	2x6	DRY	2100F 1.8E	SPF	PR
						_
	ALL WEBS	2x4	DRY	No.2	SPF	UN
	EXCEPT					
	E - Q	2x6	DRY	2100F 1.8E	SPF	JT
						υ
	DRV- SEASO	JVIEDII	IMPED			N.4

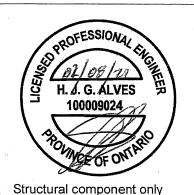
DESIGN CONSISTS OF $\underline{2}$ TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	S #ROWS	SURFACE	LOAD(PLF)
TOD 011	0000 /0 /	SPACING (IN)	
TOP CH	OHDS : (0.1	22"X3") SPIRAL NAILS	
A-C	2	12	SIDE(140.3)
C-E	2	12	SIDE(70.1)
E-F	2	12	TOP `
F- H	2	12	TOP
H- I	2	12	TOP
I-J	2	12	TOP
J- L	2	12	TOP
U- B	2	12	TOP
M-K	2	12	TOP
BOTTON	I CHORDS	: (0.122"X3") SPIRAL NAIL	.S
U- M	2	` 12	SIDE(227.4)
WEBS:	(0.122"X3")	SPIRAL NAILS	
04	,		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

D- S 2x6

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.



DWG# T-2204183

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DÉSIGNER	
READINGS	

м	HINGS						
	FACTO		MAXIMU		INPUT BRG	REQRE	
	GROSS RI	EACTION	GROSS	REACTIC		BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	7480	0	7480	-356	-819	5-8	5-8
	5325	0	5325	0	-734	5-8	5-8

PROVIDE FOR 356 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED	

9111	NI ACTORED REACTIONS											
	1ST LCASE	MAX./	MIN. COMPO	NENT REACTIO	NS							
JT	COMBINED) SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
U	5359	3613 / 0	488 / 0	0/0	428 / -893	1258 / 0	0/0					
М	3846	2555 / 0	488 / 0	0/0	238 / -832	803 / 0	0/0					
HOF	HORIZONTAL REACTIONS											
U		0/0	0/0	0/0	254 / -254	0/0	0 /0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

 $\frac{\text{BRACING}}{\text{MAX. UNBRACED TOP CHORD LENGTH}} = 3.11 \text{ FT.} \\ \text{MAX. UNBRACED BOTTOM CHORD LENGTH} = 6.25 \text{ FT} \text{ OR RIGID CEILING DIRECTLY APPLIED.} \\$

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, F-Q, F-P, G-P, H-P, H-O, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

SIDE(503.9)

LOADING TOTAL LOAD CASES: (18)

	ORDS X. FACTORED	FACTO	DED			W E	BS MAX. FACTO	DED
MEMB.		VERT. LO		MAN	MAX.	мемв.		MAX
MICINIO.	(LBS)				WAX. UNBRAC		(LBS)	
FR-TO	(LDG)	FROM			LENGTH			CSI (LC)
A-B	0 / 64	-145.3		0.06 (2)	10.00	T- C	-451 / 153	0.08 (7)
B- C	-9162 / 941	-145.3		0.41 (1)	3.72	C-S	-695 / 6753	0.60 (1)
C- V	-12103 / 1361	-145.3		0.38 (1)	3.27	D-R	-381 / 1168	0.00 (1)
V- W	-12103 / 1361	-145.3		0.38 (1)		R-E	-575 / 323	0.17 (14)
W-D	-12103 / 1361	-145.3		0.38 (1)	3.27		-8183 / 1028	0.10 (1)
D-E	-12957 / 1610	-145.3		0.35 (1)	3.11	Q-F	-625 / 5604	0.52 (1)
E-F	-8557 / 1211	-145.3		0.40 (1)	3.84		-1671 / 144	0.54 (2)
F- G	-6280 / 1033	-145.3		0.40 (1)	4.57	P- G	-925 / 269	0.23 (10)
G-H	-6273 / 1033		-145.3	0.21 (1)	4.57	P-H	-407 / 2884	0.25 (10)
H- I	-6381 / 951	-145.3		0.45 (1)	4.30	O- H	-130 / 738	0.23 (2)
i- J	-5110 / 774	-145.3		0.45 (1)		0-1	-836 / 340	0.07 (3)
J- K	-6156 / 819	-145.3		0.23 (1)	4.59	N-J	-384 / 3150	0.13 (3)
K-L	0 / 64	-145.3		0.06 (3)	10.00	B-T	-580 / 7414	0.66 (1)
U- B	-7375 / 840	0.0	0.0	0.26 (1)	5.52	N- K	-538 / 5003	0.44 (1)
M- K	-5229 / 745	0.0	0.0	0.19 (1)	6.39		-1656 / 534	0.29 (1)
	02207710	0.0	0.0	0.10 (1)	0.00		-3905 / 568	0.52 (1)
U- X	-341 / 349	-39.5	-39.5	0.06 (6)	6.25		00007000	0.02 (1)
X-Y	-341 / 349	-39.5	-39.5	0.06 (6)	6.25			
Y-T	-341 / 349	-39.5	-39.5	0.06 (6)	6.25			
T- Z	-759 / 7303	-39.5	-39.5	0.27 (1)	6.25			
Z-AA	-759 / 7303	-39.5	-39.5	0.27(1)	6.25			
AA-S	-759 / 7303	-39.5	-39.5	0.27 (1)	6.25			
S-R	-1253 / 12104	-39.5	-39.5	0.35 (1)	6.25			
R-Q	-1511 / 12972	-39.5	-39.5	0.39 (1)	6.25			
Q-P	-750 / 6928	-39.5	-39.5	0.21 (1)	6.25			
P- 0	-497 / 5105	-39.5	-39.5	0.17(1)	6.25			
0- N	-630 / 5747	-39.5	-39.5	0.19(1)	6.25			
N-M	-7 / 16	-39.5	-39.5	0.05 (17	10.00			

DESIGN CRITERIA

SPEC	IFIED	LOA	os:		
TOP	CH.	LL	=	43.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	==	7.4	PSF
TOTA	L LO	AD	=	67.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 9.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018 , ABC 2019 - PART 4 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

- TPIC 2014

- SLOPE REDUCTION FACTOR USED - PERCENTAGE OF GROUND SNOW LOAD IS

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE

ALLOWABLE DEFL.(LL) = L/360 (1.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL) = L/180 (3.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.45/1.00 (H-I:1) , BC=0.39/1.00 (Q-R:1) , WB=0.66/1.00 (B-T:1) , SSI=0.17/1.00 (D-E:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI) (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 MT20

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (T) (INPUT = 0.90) JSI METAL= 0.75 (E) (INPUT = 1.00)

JOB DESC. PLY **BAYVIEW WELLINGTON** TRUSS DESC. 2

Tamarack Roof Truss, Burlington

417704

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:19 2022 Page 2 ID:JvMhHdWEgsSl57rNnlib4czW6nl-3AaO7anplV6h9Sd1OTxWBVnqtmHBLa6PrZqLwtznTsU

DRWG NO.

W 6.0 8.0 7.0 7.0 6.0 5.0 6.0 7.0 4.0 7.0 5.0 3.0 5.0 3.0 GHIJKMZOPQRSHU BMWW+t BMWW+t BMWW-t BMV1+p MT20 6.0

T4

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPEC	DIFIED CO	NCENTRA	TED LOA	DS (LBS)					
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C.	5-10-8	-226	-426		FRONT	VERT	TOTAL		C1
S	11-4-8	-1880	-1880		FRONT	VERT	TOTAL		C1
T	5-11-4	-164	-164		FRONT	VERT	TOTAL		C1
٧	7-5-4	-2	-80		FRONT	VERT	TOTAL		C1
w	9-5-4	-2	-80		FRONT	VERT	TOTAL		C1
X	1-11-4	-29	-29		FRONT	VERT	TOTAL		C1
Υ	3-11-4	-29	-29		FRONT	VERT	TOTAL		C1
Z	7-5-4	-164	-164		FRONT	VERT	TOTAL		C1
AA	9-5-4	-164	-164		FRONT	VERT	TOTAL		Ct

HOMES

CONNECTION REQUIREMENTS

C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only DWG# T-2204183 W

JOB DESC. **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. 417701 T4S 2 HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:35:12 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-48IXNtNdVxUX16?U4uB31flO_HjhJnSLg?pMzAznGT 1-3-8 5-10-8 1-6-12 2-0-0 7-11-4 6-0-0 4-10-8 Scale = 1:87.0 6x10 \\ 4x6 || 5x8 // F Н 5x6 <> 7x8 \\ 8x9 \\ 4x6 || 5x8 // 9.00 12 D E 5x8 // K 10.5.6 10.5 AE AF 5x8 > 4x6 || 6x10 // 77. W19 M_N В 1-6-4 W10 s R AG AH ΑI ΑJ v ^U AA X AD AC AR Z γ W P 0 4x6 || 5x8 = 6x10 == 8x12 == 6x10 || 5x8 = 5x8 || 4x6 | 6x10 =6x12 =3x6 || 3x6 || 5x6 = 4x6 || 2H25A 6x12 =-ZH2574 1-11-4 2-0-0 2-0-0 1-6-0 2-0-0 1-11-4 22-3-0 10-9-0 1-7-8 1-3-8 46-6-0 1-3-8 TOTAL WEIGHT = 2 X 337 = 674 lb LUMBER DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY N.I. G. A. BULES **BUILDING DESIGNER DESIGN CRITERIA** BEARINGS FACTORED CHORDS LUMBER DESCR A -C -F -H -- C - E - H SPECIFIED LOADS: 2x6 No.2 SPF MAXIMUM FACTORED INPUT REORD DRY GROSS REACTION DOWN HORZ UPLIFT 2x6 No.2 GROSS REACTION BRG IN-SX LL = DL = LL = DL = AD = CH. 43.5 2x6 2x6 DRY SPF HORZ IN-SX 6.0 PSF No.2 SPF AD 7175 0 7175 -356 -817 5-8 5-8 BOT CH. 10.5 7.4 2x6 DRY No.2 SPF 0 5280 ō 5280 ō -731 5-8 5-8 DL TOTAL LOAD 2x6 2x6 DRY No.2 SPF SPF J -K -AD-AD-AA-X -67.3 PSF NBM DRY PROVIDE ANCHORAGE AT BEARING JOINT AD FOR 817 LBS FACTORED UPLIF PROVIDE ANCHORAGE AT BEARING JOINT O FOR 731 LBS FACTORED UPLIFT No.2 2x6 No.2 SPF SPACING = 24.0 IN. C/C SPF 244 DRY AA X U DRY 2100F1.8E PROVIDE FOR 356 LBS FACTORED HORIZONTAL REACTION AT JOINT AD 2x6 DRY 2100F1.8E SPF LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 9.00/12 SPF UNFACTORED REACTIONS 2x6 DRY 2100F 1.8F DRY ã 2100F 1.8E MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015 2x4 COMBINED WIND L 0 No.2 SPF DEAD SOIL 2x6 DRY No.2 SPF 5151 3441 / 0 492 / 0 0/0 428 / -892 238 / -834 1217/0 0/0 - H DRY SPF 2520 / 0 0/0 ALL WEBS 2x4 DRY SPF THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT) No.2 HORIZONTAL REACTIONS EXCEPT W - T 0/0 0/0 254 / -254 0 /0 DRY No.2 SPE BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AD, O DRY: SEASONED LUMBER. TPIC 2014 BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.14 FT. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS DESIGN ASSUMPTIONS SLOPE REDUCTION FACTOR USED
PERCENTAGE OF GROUND SNOW LOAD IS MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. FOLLOWS: ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. USER-DEFINED CHORDS #ROWS SURFACE LOAD(PLF) OVERHANG NOT TO BE ALTERED OR CUT OFF. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Y, G-W, H-T. 2x6 DRY SPF No.2 T-BRACE AT E-Y, F-W, G-T SPACING (IN) "X3") SPIRAL NAILS (80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE A- C C- E E- F F- H SIDE(140.3) SIDE(70.1) TOP TOP 12 12 12 12 FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% 2 2 ALLOWABLE DEFL.(LL)= L/360 (1.55")
CALCULATED VERT. DEFL.(LL)= L/999 (0.30")
ALLOWABLE DEFL.(TL)= L/180 (3.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41") H- .I TOP J- K K- N AD- B 12 TOP TOP END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW 12 TOP O- M TOP LOADING TOTAL LOAD CASES: (18) BOTTOM CHORDS AD- AA 2 (0.122"X3") SPIRAL NAILS CSI: TC=0.45/1.00 (D-E:1), BC=0.38/1.00 (Y-Z:1), SIDE(227.4) 2 12 12 WB=0.86/1.00 (E-Y:1), SSI=0.20/1.00 (C-D:2) AA- X X- U T- Q P- O TOP TOP TOP CHORDS W E B S MAX. FACTORED 12 12 MAX. FACTORED DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 MEMB. VERT. LOAD LC1 MAX MAX. MEMB. FORCE FORCE MAX COMP=1.00 SHEAR=1.00 TENS= 1.00 (PLF) FROM TO -145.3 -145.3 CSI (LC) UNBRAC LENGTH FR-TO 12 TOP (LBS) (LBS) CSI (LC) SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00 L-P 12 FR-TO A-B B-C C-AE 0.06 (2) AC- C -848 / 153 0.14 (7) 10.00 C-AB D- Z Z- E E- Y Y- F 0.40 (1) 0.40 (1) 0.40 (1) -8719 / 938 -145.3 -145.3 -693 / 6672 -375 / 1263 0.59 (1) 0.17 (14) 3.81 -11681 / 1357 -11681 / 1357 -145.3 -145.3 -145.3 -145.3 PROFESSIONAL ENGINE BU DILOS 23 H. J.G. ALVES AE-AF 3.30 -643 / 322 0.11(1)AF- D -11681 / 1357 -145.3 -145.3 0.40 (1) 3.30 -7899 / 1021 0.86 (1) AUTOSOLVE HEELS OFF D- E E- F F- G -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 0.45 0.40 -627 / 5424 -1563 / 148 -12604 / 1601 0.30 (2) TRUSS PLATE MANUFACTURER IS NOT -6199 / 1029 RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. 0.21 (1) 4.59 W-G -310/5730.08 (10) G- H H- I I- J -5608 / 903 -6982 / 1071 -7856 / 1094 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 4.79 4.44 4.24 0.20 0.15 -1382 / 395 -170 / 1127 0.10 (1) 0.13 (1) R-K -507 / 4003 0.35 (1) NAIL VALUES J- K K- L L- M M- N -6365 / 900 -145.3 -145.3 0.06 (1 4.70 4.21 B- I -383 / 165 -577 / 7055 0.03 (3) 0.62 (1) PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) -145.3 -145.3 -145.3 -145.3 0.23 (1) 0.20 (1) -7527 / 995 100009024 (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 -7924 / 1049 4.13 AB-D -1848 / 529 0.32 (1) Q- O -263 / 52 Q- M -734 / 6183 W- T -688 / 6275 G- T -1653 / 259 0 / 64 -145.3 -145.3 0.06 (3) 0.25 (1) 10.00 -263 / 52 -734 / 6183 0 02 11 FOF ONT ARIO 0.0 0.0 0.25 (1) 0.28 (1) 5.63 5.31 AD- B -7049 / 838 POVI O- M -5099 / 708 PLATE PLACEMENT TOL = 0.250 inches -688 / 6275 0.41(1) 0.27 (2) 0.18 (1) AD-AG AG-AH AH-AC -39.5 -39.5 -39.5 S-J J-R -1337 / 290 -3902 / 541 -341 / 349 -39.5 0.05 (17) 6.25 PLATE ROTATION TOL. = 5.0 Deg. -341 / 349 -341 / 349 -39.5 0.05 (17) 0.05 (17) 0.05 (17) 6.25 0.35 (1) 6.25 JSI GRIP= 0.87 (AB) (INPUT = 0.90) JSI METAL= 0.94 (E) (INPUT = 1.00) -39.50.22 (1) AC-AI -757 / 6938 -39.5 -39.5 6.25 -39.5 -39.5 Structural component only AJ-AB -757 / 6938 -39.5 -39.5 -39.5 0.22 (1) 6.25

DWG# T-2204207 1/1

-1249 / 11682

-39.5 0.36 (1)

6.25

JOB DESC.

TOP

417701

T4S

BAYVIEW WELLINGTON

TRUSS DESC. HOMES

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:35:12 2022 Page 2 ID:JvMhHdWEgsSl57rNnlib4c2W6nl-48IXNtNdVxUX16?U4uB31flO HjhJnSLg?pMzAznGTT

V- H 1 12 WEBS : (0.122"X3") SPIRAL NAILS AC- C 1 6 D- AB 1 6 2x4 1 6 2x6 2 6

SIDE(10.2) SIDE(489.7

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLA	TES (table is	in inches)				
JT		PLATES	W	LEN	Υ	Х
В	TMVW-t	MT20	6.0	10.0		
С	TTWW+m	MT20	8.0	9.0	3.75	2.25
D	TMWW+t	MT20	4.0	6.0		
E	TTWW+m	MT20	7.0	8.0		
F	TTWW+m	MT20	6.0	10.0	4.00	1.75
G	TMWW+t	MT20	4.0	6.0		
Н	TTV+m	MT20	5.0	8.0	Edge	
1	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	5.0	8.0		
K	TTW+m	MT20	5.0	8.0	Edge	
L	TMVW+p	MT20	4.0	6.0		
М	TMVW-t	MT20	5.0	8.0		
0	BMVW1+p	MT20	4.0	6.0		
Р	BMV+p	MT20	3.0	6.0		
Q	BVMWW-I	MT20	6.0	12.0	4.00	7.75
R	BMWWW-t	MT20	5.0	8.0		
s	BMWW+t	MT20	4.0	6.0		
T	BVMWWW-I	MT20	8.0	12.0	4.50	4.50
٧	BMV+p	MT20	3.0	6.0		
W	BMWWW-t	MT20	6.0	12.0	3.00	4.00
Х	BS-t	MT20	5.0	6.0		
Y	BMWW-t	MT20	6.0	10.0		
Z	BMWW+t	MT20	4.0	6.0		
AA	BS-t	MT20	6.0	10.0		
AB	BMWW+t	MT20	5.0	8.0	4.00	2.00
AC	BMWW-t	MT20	5.0	8.0	2.50	3.00
AD	BMV1+t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.

.7) CH	ORDS					WE	3 S	
· MA	C. FACTORED	FACTOR	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LO.	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
AA-Z	-1249 / 11682	-39.5	-39.5	0.36(1)	6.25			
Z-Y	-1502 / 12622	-39.5	-39.5	0.38 (1)	6.25			
Y-X	-746 / 6787	-39.5	-39.5	0.22(1)	6.25			
X-W	-746 / 6787	-39.5	-39.5	0.22 (1)	6.25			
W-V	-5 / 42	-39.5	-39.5	0.04 (4)	10.00			
V- U	0/0	-39.5	-39.5	0.00 (17	10.00			
T-S	-667 / 6298	-39.5	-39.5	0.19(1)	6.25			
S-R	-792 / 7166	-39.5	-39.5	0.22 (1)	6.25			
R-Q	-745 / 6393	-39.5	-39.5	0.19(1)	6.25			
P-Q	0 / 37	0.0	0.0	0.17(1)	10.00			
Q-L	0 / 231	0.0	0.0	0.19(1)	10.00			
P-O	-27 / 215	-39.5	-39.5	0.02 (1)	6.25			
V-T	0 / 127	0.0	0.0	0.04 (1)	10.00			
T-H	-515 / 3634	0.0	0.0	0.35 (1)	6.25			

SPECIFIED CONCENTRATED LOADS (LBS)										
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	
С	5-10-8	-294	-504		FRONT	VERT	TOTAL		C1	
AB	11-4-8	-1843	-1843		FRONT	VERT	TOTAL		C1	
AC	5-11-4	-22	-22		FRONT	VERT	TOTAL		C1	
ΑE	7-5-4	-71	-158		FRONT	VERT	TOTAL		C1	
ΑF	9-5-4	-71	-158		FRONT	VERT	TOTAL		C1	
AG	1-11-4	-29	-29		FRONT	VERT	TOTAL	***	C1	
AH	3-11-4	-29	-29		FRONT	VERT	TOTAL		C1	
Αl	7-5-4	-22	-22		FRONT	VERT	TOTAL		C1	
ΑJ	9-5-4	-22	-22		FRONT	VERT	TOTAL		C1	

CONNECTION REQUIREMENTS

2

LOADING TOTAL LOAD CASES: (18)

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, OPC3, BASED ON THE (MAIN WIND PORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF BRSPECTIVELY. PSF AND 5.0 PSF RESPECTIVELY.



DWG# T-2204207 WV

PLY JOB DESC. **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. 417704 **T**5 HOMES Famarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:21 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-?Zi9YGo3H6MPPInPVu__Gwt8QazqpRfiJt9S?mznTsS 1-3-8 7-10-8 11-6-0 3-7-8 1-0-0 3-0-0 1-3-8 Scale = 1:84. 3x6 || 8x9 // 7x8 = G 8x9 \\ 7x8 \\ 5x6 == D 5x6 > 9.00 12 6x10 // 5x8 // 10-1-12 10-2-0 8x9 // 6x10 > М N W s Υ U т Q 0 8x9 WB= 5x6 =4x6 || 5x8 || 5x6 == 5x8 || 6x10 == 4x6 || 4x6 || 6x12 =7x8 || 2H25A ZH2574 46-6-0 1-3-8 46-6-0 1-3-8 TOTAL WEIGHT = 342 lb LUMBER DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY N. L. G. A. BULES BUILDING DESIGNER BEARINGS FACTORED **DESIGN CRITERIA** CHORDS SIZE 2x6 LUMBER A D F MAXIMUM FACTORED INPUT GROSS REACTION BRG DOWN HORZ UPLIFT IN-SX No.2 No.2 SPF REQRD SPECIFIED LOADS: GROSS REACTION VERT HORZ DRY DRY DRY 2x6 LL = 43.5 DL = 6.0 LL = 10.5 BRG IN-SX 2x6 2x6 2x6 2x6 2x6 2x6 TOP CH. PSF G I K No.2 No.2 SPF 6.0 10.5 7.4 PSF PSF G z o 4505 0 4505 -347 -822 -739 5-8 5-8 DRY No.2 SPF 4505 ñ 4505 IKLZOZ DL = PSF DRY No.2 SPF TOTAL LOAD 67.3 N B M No.2 PROVIDE ANCHORAGE AT BEARING JOINT Z FOR 822 LBS PROVIDE ANCHORAGE AT BEARING JOINT O FOR 739 LBS 2x6 DRY No.2 SPE SPACING = 24.0 IN. C/C SPF 2x6 DRY No.2 DRY 2x6 2100F 1.8E PROVIDE FOR 347 LBS FACTORED HORIZONTAL REACTION AT JOINT Z S 2x6 2100F 1.8E SPF LOADING IN ALL FLAT SECTIONS BASED ON A s -2x6 2100F 1.8E SPF UNFACTORED REACTIONS SLOPE OF 9.00/12 MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM LIVE V ALL WEBS EXCEPT 2x4 DRY No.2 SPF COMBINED PERM.LIVE WIND DEAD SOIL THIS TRUSS IS DESIGNED FOR COMMERCIAL 3273 3273 2144 / 0 2144 / 0 0/0 0/0 OR INDUSTRIAL BUILDING REQUIREMENTS OF Ü 2x6 DRY 2100F 1.8E 2100F 1.8E 488 / 0 243 / -836 640 / 0 0/0 PART 4, NBCC 2015 2x4 HORIZONTAL REACTIONS THIS DESIGN COMPLIES WITH: DRY: SEASONED LUMBER. 0/0 0/0 248 / -248 - PART 4 OF BCBC 2018 , ABC 2019 - PART 4 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 0/0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, O - TPIC 2014 DIVIDITION OF THE PROPERTY OF PLATES (table is in inches)
JT TYPE PLATES DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED W LEN Y Х 4.0 10.0 3.00 4.00 9.0 3.75 2.50 TMV+p TMWW-t TTWW+m BCDHFG MT20 MT20 3.0 6.0 8.0 5.0 7.0 3.0 8.0 5.0 6.0 7.0 - PERCENTAGE OF GROUND SNOW LOAD IS ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. MT20 6.0 8.0 8.0 TMWW-t MT20 ATERAL BRACE(S) AT 1/2 LENGTH OF D-X, E-X, E-V, F-U, G-U, G-T, H-T, I-T, I-R, C-Z. (80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE TTWW+m TTWW-m MT20 MT20 4.25 2.75 2.75 3.50 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN 6.0 9.0 6.0 9.0 TMW+w MT20 THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW MT20 MT20 3.75 2.50 2.50 2.75 TTWW+m ALLOWABLE DEFL.(LL)= L/360 (1.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/180 (3.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41") LOADING TTWW+m MT20 TTW+m MT20 MT20 8.0 10.0 2.50 5.00 LMOP TMVW-t BMV1+t 8.0 Edge 1.50 12.0 2.75 3.25 MT20 BMWWW-t MT20 6.0 CSI: TC=0.64/1.00 (E-F:1) , BC=0.38/1.00 (U-V:1) , WB=0.78/1.00 (J-R:3) , SSI=0.32/1.00 (D-E:2)

R, Ý BMWW+t Q, Q S T 6.0 6.0 4.0 BS-t MT20 5.0 BMWWW-t 6.0 5.0 10.0 8.0 MT20 . V W BMWW+t 3.25 2.50 BMWW-t MT20 5.0 6.0 BS-t MT20 8.0



Structural component only DWG# T-2204184

TOTAL LOAD CASES: (18)												
СН	CHORDS WEBS											
MA)	K. FACTORED						MAX. FACTO	RED				
MEMB.	FORCE	VERT. LC			MAX.	MEMB	FORCE	MAX				
	(LBS)			CSI (LC)			(LBS)	CSI (LC)				
FR-TO		FROM			LENGTH							
A-B	0 / 64	-145.3		0.11 (2)	10.00	C-Y		0.06 (3)				
B-C	-26 / 153	-145.3		0.14 (2)		Y- D	-34 / 271	0.04 (17)				
C-D	-5141 / 1047	-145.3		0.28 (1)		D- X	-548 / 3043	0.49 (1)				
D-E	-6023 / 1253	-145.3		0.57 (1)			-2131 / 462	0.48 (1)				
E-F	-6970 / 1380	-145.3		0.64 (1)		E-V	-296 / 1510	0.24 (1)				
F-G	-6389 / 1303	-145.3		0.35 (1)		V-F	-997 / 280	0.62 (1)				
G-H	-4958 / 1060	-145.3		0.46 (1)			-3938 / 869	0.44 (1)				
H- I	-4958 / 1060	-145.3		0.46 (1)			-718/3594	0.58 (1)				
I- J	-5132 / 1012		-145.3	0.31 (1)			-685 / 447	0.44 (2)				
J- K	-5648 / 988		-145.3	0.34 (1)			-1023 / 291	0.46 (10)				
K-L	-4403 / 788	-145.3		0.09(1)	4.11	T- I	-418/2076	0.33 (2)				
L-M M-N	-4866 / 811	-145.3		0.22 (1)		R-I	-260 / 980	0.16 (3)				
	0 / 64		-145.3	0.11 (3)		R-J	-931 / 361	0.78 (3)				
	-474 / 204	0.0	0.0	0.03 (2)		Q-J	-49/365	0.06 (6)				
O-M	-4413 / 746	0.0	0.0	0.29 (1)	5.14	Q-K	-313 / 152	0.10 (3)				
Z- Y	005 / 0005	00.5	00.5	0.07 (4)			-5523 / 909	0.51 (1)				
Y- X	-825 / 3885 -709 / 4083	-39.5		0.27 (1)			-3709 / 624	0.53 (1)				
X- W	-1059 / 6024	-39.5	-39.5	0.27 (1)			-436 / 2812	0.45 (1)				
W-V			-39.5	0.34 (1)		P- M	-549 / 4035	0.65 (1)				
V- U	-1059 / 6024 -1191 / 6991	-39.5	-39.5	0.34 (1)								
U- T	-787 / 5165	-39.5 -39.5	-39.5	0.38 (1)								
T- S	-513 / 4077	-39.5	-39.5	0.30 (1)								
S-R	-513 / 4077			0.25 (1)								
R-Q	-513 / 4 0 / / -611 / 4562	-39.5 -39.5	-39.5 -39.5	0.25 (1)								
Q-P	-652 / 4756	-39.5 -39.5	-39.5	0.25 (1)								
P-0	-652 / 4756 -7 / 16	-39.5 -39.5	-39.5	0.28 (1)								
1 0	-1 / 10	-39.5	-39.5	0.04 (1)	10.00							

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1 nn WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 MT20

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (P) (INPUT = 0.90) JSI METAL= 0.85 (W) (INPUT = 1.00)

JOB DESC. **BAYVIEW WELLINGTON** PLY DRWG NO. TRUSS DESC. HOMES **T**5 1

417704 Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:21 2022 Page 2 ID:JvMhHdWEqsSl57rNnlib4czW6nl-?Zi9YGo3H6MPPInPVu Gwt8QazqpRfiJt9S?mznTsS

 PLATES
 (table is in inches)

 JT
 TYPE
 PLATES

 X
 BMWW+t
 MT20

 Z
 BMVW1-t
 MT20
 W LEN Y X 5.0 8.0 4.00 2.00 7.0 8.0 3.25 3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C.D.C., BASSED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only DWG# T-2204184 2/7

JOB DESC. **BAYVIEW WELLINGTON** PLY DRWG NO TRUSS DESC. 417701 T5S 1 HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:35:13 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-YKsvbDNFGFdOfFahdbilatrXoh2g2DiUvfYvVdznGT 7-10-8 1-3-8 11-6-0 3-7-8 1-0-0 3-0-0 1-3-8 Scale = 1:90. 7x8 \\ 4x6 || 5x8 || G Н 1 6x10 \\ 8x9 \\ 4x6 || 5x6 <> ח 9.00 12 W9 5x8 // 6x10 // 6x10 // 8x9 || 4x6 || 3x4 || 1996 W19 W12 No 1-6-4 1-6-4 B3 W10 s У $w^{V}_{8x12} =$ ΑE AD AB AA Z ٥ P 5x6 =6x10 5x6 = 5x6 = 6x10 =4x6 || 4x6 || 5x8 || 6x12 == 3x6 || 3x6 || 5x6 || 7x8 || 2/2574 7x12 =24257 34-0-0 10-10-8 1-7-8 1-3-8 46-6-0 1-3-8 TOTAL WEIGHT = 342 lb LUMBER DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY N. L. G. A. BULES **BUILDING DESIGNER** DESIGN CRITERIA CHORDS LUMBER SIZE DESCR A D F FACTORED No.2 No.2 SPF SPF MAXIMUM FACTORED INPUT REORD SPECIFIED LOADS: GROSS REACTION DOWN HORZ L BRG IN-SX BRG IN-SX 2x6 DRY GROSS REACTION 43.5 PSF CH. LL = DL = 2x6 2x6 2x6 DRY No.2 No.2 SPF G HORZ UPLIFT 6.0 PSF Ġ ΑE 4508 0 4508 -347 -820 5-8 5-8 BOT CH. LL 10.5 7.4 PSF I K I -K -DRY No.2 SPF 4524 ō 4524 0 -737 5-8 5-8 SPF SPF SPF SPF SPF 2x6 2x6 DRY No.2 No.2 TOTAL LOAD 67.3 PSF L -OBN PROVIDE ANCHORAGE AT BEARING JOINT AE FOR 820 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT P FOR 737 LBS FACTORED UPLIFT 2x6 DRY No.2 SPACING = 24.0 IN. C/C 2×4 DRY No.2 AE-AC-Y 2100F 1.8E 2100F 1.8E 2100F 1.8E DRY 2x6 PROVIDE FOR 347 LBS FACTORED HORIZONTAL REACTION AT JOINT AE 2x6 2x6 SPF LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 9.00/12 DRY SPE UNFACTORED REACTIONS ii 2x6 2x4 DRY SPF 2100F 1.8E MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015 αa COMBINED WIND DEAD No.2 SOIL 2x6 DRY No.2 SPF 3276 2142 / 0 491 / 0 0/0 432 / -895 642 / 0 646 / 0 0/0 3289 2146 / 0 242 / -837 ALL WEBS EXCEPT 2x4 DRY No.2 SPF THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT) HORIZONTAL REACTIONS 2100F 1.8E 2×4 DRY SPF 0/0 0/0 248 / -248 0 /0 - Ū No.2 2x6 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AE, P DRY: SEASONED LUMBER. TPIC 2014 MAX, UNBRACED TOP CHORD LENGTH = 2.79 FT. DESIGN ASSLIMPTIONS SLOPE REDUCTION FACTOR USED
PERCENTAGE OF GROUND SNOW LOAD IS MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED PLATES (table is in inches)
JT TYPE PLATES MAX. UNBRACED INTERIOR CHORD LENGTH = 6.25 FT USER-DEFINED LEN Y Х OVERHANG NOT TO BE ALTERED OR CUT OFF. TMV+p TMWW-t TTWW+m MT20 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. 10.0 3.00 4.75 9.0 3.75 2.50 (80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE MT20 8.0 4.0 6.0 7.0 4.0 5.0 5.0 1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-AB, E-AB, E-AA, F-Z, G-Z, G-X, H-X, C-AE, I-U, H-U. E TMWW+t MT20 6.0 MT20 MT20 10.0 5.00 2.25 8.0 3.75 2.50 TTWW+m END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX, UNBRACED LENGTH COLUMN OF THE TABLE BELOW 8.0 6.0 ALLOWABLE DEFL.(LL) = L/360 (1.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL) = L/180 (3.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.46") TMWW+t MT20 TTW+p TMWW-t TTWW+m MT20 MT20 8.0 6.0 LOADING TOTAL LOAD CASES: (18) 10.0 MT20 6.0 TTW+h MT20 5.0 4.0 8.0 4.25 1.75 CHORDS WEBS TMVW+p MT20 MAX. FACTORED FACTORED MAX. FACTORED CSI; TC=0.64/1.00 (E-F:1), BC=0.40/1.00 (S-T:1), MT20 4.00 2.75 MEMB. VERT. LOAD LC1 MAX MAX. WB=0.86/1.00 (J-U:1) , SSI=0.32/1.00 (D-E:2) 8.0 7.0 9.0 FORCE MEMB. FORCE (PLF) CSI (LC) FROM TO -145.3 -145.3 0.11 (2) BMVW1+t MT20 8.0 Edge 3.50 (LBS) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) BMV+p BVMWW-l BMWWW-t MT20 MT20 6.0 12.0 3.75 7.25 O FR-TO DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 A-B B-C C-D E-F 0 / 64 C-AD AD- D D-AB AB- E 10.00 0/352 0.06 (3) COMP=1.10 SHEAR=1.10 TENS= 1.10 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -27 / 154 -5140 / 1043 -6030 / 1250 0.14 (2) 0.28 (1) 0.57 (1) -32 / 266 -547 / 3051 -2126 / 458 0.08 (3) 0.04 (17) 0.49 (1) 0.48 (1) 10.0 2.50 5.00 6.25 3.66 S MT20 6.0 SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 BMWW-3.08 E-AA AA- F F- Z 0.24 (1) LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00 -6972 / 1374 0.64 (1) 2.79 -291 / 1503 F- G G- H H- I -6398 / 1298 -4974 / 1056 0.35 (1) 0.47 (1) 3.23 3.50 -1012 / 280 PROFESSIONAL ENGINE H. J. G. ALVES -3928 / 863 0.69(1)Z-G G-X X-H 4566 / 911 0.44 (1) 3.66 -718 / 3591 0.58 (1) AUTOSOLVE HEELS OFF -5710 / 1077 -6715 / 1104 -145.3 -145.3 -145.3 -145.3 3.46 3.16 3.71 -664 / 452 -863 / 199 0.42 (2) 0.39 (10) 0.32 (1 -145.3 -145.3 TRUSS PLATE MANUFACTURER IS NOT -145.3 -145.3 -145.3 -145.3 K-L RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. -5411 / 882 0.13 (1) U- J T- .I -1385 / 406 0.86 (1) L- M M- N N- O -6346 / 1003 -6515 / 999 0 / 64 -145.3 -145.3 -145.3 0.52 (1) 0.53 (1) 0.11 (3) 2.98 -115/983 0.16 (1) 0.53 (1) -502 / 3325 S-L S-M NAIL VALUES 10.00 -4/85 0.01 (2) -5517 / 905 AE- B -479 / 205 0.0 0.0 0.03 (2) 7.81 AE- C R- P 0.85 (1) 0.02 (1) GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) -4369 / 714 0.0 0.0 -218 / 51 -668 / 4995 100009024 (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 R- N T- K K- S W- U 0.80 (1) AE-AD -822 / 3880 -39.5 -39.5 0.27 (1) 6.25 -1604 / 344 0.41 (1) 0.43 (1) OF ONTARIO -707 / 4084 -707 / 4084 -39.5 -39.5 0.26 (1) 6.25 6.25 AD-AC -39.5 ROVING -39.5 0 / 131 0.07 (1) PLATE PLACEMENT TOL = 0.250 inches AB-AA -511 / 2834 -1056 / 6031 -39.5-39.5 0.35 (1) 6.25 11-1 0.51 (1) AA- Z Z- Y Y- X X- W W- V -39.5 -39.5 -39.5 -39.5 -39.5 X- U H- U -1184 / 6994 0.38 (1) 6.25 PLATE ROTATION TOL. = 5.0 Deg. 0.38 (1) 0.31 (1) 0.31 (1) 0.08 (4) -783 / 5173 -783 / 5173 6.25 -1203/276 0.62(2)JSI GRIP= 0.90 (K) (INPUT = 0.90) JSI METAL= 0.93 (Y) (INPUT = 1.00) -39.56.25 -5 / 42 0 / 0 -39.5 -39.5 -39.5 10.00 -39.5 0.00 (17) 10.00 Structural component only U-T -711 / 5397 -39.5 -39.5 0.29 (1) 6.25 DWG# T-2204208 1/1 -930 / 6800 -39.5 0.40 (1) CONTINUED ON PAGE 2 PLY JOB DESC.

417701

1

BAYVIEW WELLINGTON

DRWG NO.

Tamarack Roof Truss, Burlington

T5S

TRUSS DESC. HOMES

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LEN Y X 12.0 5.00 5.00 6.0 8.0 3.0 6.0 5.0 5.0 4.0 5.0 4.0 6.0 12.0 2.50 3.25 6.0 8.0 6.0 6.0 6.0 3.75 2.50 2.50 2.00 AC BS-t AD BMWW+t AE BMVW1-t MT20 MT20 MT20 MT20 6.0 10.0 2.50 5.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING TOTAL LOAD CASES: (18)

W E B S MAX. FACTORED IEMB. FORCE MAX (LBS) CSI (LC) CHORDS FACTORED VERT. LOAD LC1 MAX MAX. MEMB. (PLF) CSI (LC) UNBRAC FROM TO LENGTH FR-TO -39.5 39.5 0.29 (1) 6.25 0.0 0.0 0.30 (1) 7.81 -39.5 -39.5 0.03 (1) 6.25 MAX. FACTORED MB. FORCE (LBS) MEMB. FR-TO S-R Q-R R-M Q-P -679 / 5172 0 / 37 -23 / 312 -26 / 178

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF [7.5] PSF AT [30-0-0] FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPC9, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST [0-0] FT-IN-SX AWAY FROM EAVE-TRUSS UPLIET IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



DWG# T-2204208

JOB DESC. BAYVIEW WELLINGTON PLY DRWG NO. 417704 **T6** 1 TRUSS DESC. HOMES Version 8.420 S Jan 21 2021 Mirk Industries, Inc. Mon Feb 7 17:21:22 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-TiFXmcph1QUG0vMb3cVDp7PJo_JQYtLrXXv?XCznTsR Tamarack Roof Truss, Burlington 1-3-8 9-10-8 11-6-0 12-6-0 1-7-8 Scale = 1:82. 7x8 \\ 4x6 || 8x9 // 6x10 = 8x9 \\ G 3x6 || D 9.00 12 5x6 // 5x6 > 10-1-12 C 8-11-2 6x10 // W 1-6-4 uw₂ K 4. 161 R N U Q s P 0 М L 5x6 = 7x8 || 6x10 == 7x8 == 4x6 || 5x6 8x9 = 5x6 || 4x6 || 3x6 || 6x10 =2H25A ZHISA 46-6-0 1-3-8 TOTAL WEIGHT = 326 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED A -D -F -G -SPF SPF SPF MAXIMUM FACTORED INPUT GROSS REACTION BRG DOWN HORZ UPLIFT IN-SX D F 2x6 DRY No.2 SPECIFIED LOADS: DRY DRY DRY LL = DL = LL = DL = AD = GROSS REACTION HEEL BRG CH. 43.5 Ġ VERT HORZ 6.0 10.5 7.4 PSF PSF IN-SX WEDGE 2x6 No.2 SPF 4505 0 4505 4297 -327 -822 -694 BOT CH. 2x6 2x6 DRY No.2 SPF SPF SPF 2x4 R PSF UURN TOTAL LOAD 67.3 2100F 1.8E DRY PROVIDE ANCHORAGE AT BEARING JOINT U FOR 822 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT K FOR 694 LBS FACTORED 2x6 Ν 2x6 DRY 2100F 1 8F SPACING = 24.0 IN. C/C 2100F 1.8E PROVIDE FOR 327 LBS FACTORED HORIZONTAL REACTION AT JOINT U ALL WEBS 2x4 DRY No.2 SPE UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

LIVE PERM.LIVE PERM.LIVE PERM.LIVE NO. (1997) LOADING IN ALL FLAT SECTIONS BASED ON A EXCEPT SLOPE OF 9.00/12 2x6 DRY 2100F 1.8E SPF WIND 432 / -895 264 / -794 LIVE 488 / 0 SOIL THIS TRUSS IS DESIGNED FOR COMMERCIAL DRY: SEASONED LUMBER. 640 / 0 0/0 OR INDUSTRIAL BUILDING REQUIREMENTS OF 3131 2020 / 0 488 / 0 0/0 623 / 0 0/0 HORIZONTAL REACTIONS THIS DESIGN COMPLIES WITH: 0/0 0/0 - PART 4 OF BCBC 2018 , ABC 2019 - PART 4 OF OBC 2012 (2019 AMENDMENT) 0/0 228 / -234 0/0 0 / 0 PLATES (table is in inches)
JT TYPE PLATES LEN Y BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K CSA 086-14 10.0 2.50 5.00 6.0 2.50 2.75 9.0 3.75 2.50 ВС 6.0 5.0 8.0 TMVW-t MT20 - TPIC 2014 TTWW+m MT20 MAX. UNBRACED TOP CHORD LENGTH = 2.89 FT. DESIGN ASSUMPTIONS 6.0 10.0 3.25 5.00 EFGH TMW+w MT20 3.0 6.0 7.0 4.0 8.0 5.0 7.0 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. SLOPE REDUCTION FACTOR USED TTWW-m TTW+m TMWW+t - PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED. 8.0 6.0 9.0 MT20 Edge ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. MT20 MT20 MT20 TTWW+ 3.75 2.50 1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-S, D-Q, E-Q, H-O, I-O, I-M, G-P, H-P, F-Q, F-P. (80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE J K 6.0 2.50 2.75 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW TMBH1-I MT20 8.0 6.0 LMZOPOR BMW+w MT20 3.0 4.0 5.0 5.0 8.0 6.0 5.0 4.0 6.0 LOAD BMWW+ LOADING TOTAL LOAD CASES: (18) 6.0 ALLOWABLE DEFL.(LL) = L/360 (1.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL) = L/180 (3.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35") BS-t MT20 6.0 3.00 2.25 9.0 4.00 3.00 10.0 3.00 4.00 6.0 9.0 RMWW₊₁ MT20 BMWWW-t BMWWW-t MT20 MT20 CHORDS WEBS 6.0 6.0 10.0 3.00 MAX. FACTORED FACTORED VERT. LOAD LC1 MAX BS-t MT20 MAX. FACTORED RMW/W MEMB. CSI: TC=0.60/1.00 (J-W:1) , BC=0.36/1.00 (L-V:1) , WB=0.88/1.00 (J-M:3) , SSI=0.49/1.00 (K-V:2) FORCE MAX BMWW-t (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FROM TO -145.3 -145.3 -145.3 -145.3 U BMV1+t MT20 7.0 8.0 5.50 FR-TO FROM LENGTH FR-TO 0.11 (2) 0.40 (1) 0 / 64 -709 / 189 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 10.00 T-C C-S 0.17 (7) Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. -5086 / 961 3.58 -230 / 220 0.14 (2) 0.06 (5) COMP=1.10 SHEAR=1.10 TENS= 1.10 0.39 (1) 0.52 (1) 0.52 (1) S-D Q-H I C- D -122 / 403 -406 / 2416 -5021 / 1067 -145.3 -145.3 D- E E- F F- G -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 -5336 / 1159 0.39 (1) SNOW LOAD IMPORTANCE FACTOR = 1 00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00 -5336 / 1159 3.33 -1029 / 304 0.34(2)0.73 (2) -6406 / 1349 0.20 (1) 3.38 -1587 / 406 0.45 (1) 0.44 (1) 0.55 (1) G- H H- I -5175 / 1123 -4945 / 1058 -145.3 -145.3 -145.3 -145.3 3.47 3.55 -193/876 0.14 (3) PROFESSIONAL ENGINE BULLES IN H. J. G. ALVES 0.88 (3) 0.05 (17) -5146 / 982 -145.3 -145.3 3.43 M-J -957 / 391 AUTOSOLVE RIGHT HEEL ONLY L-J B-T P-G P-H Q-F F-P 0.60 (1) J- W -5957 / 985 -145.3 -145.3 W-K U-B -640 / 4184 0.67(1) TRUSS PLATE MANUFACTURER IS NOT 0.29 (1) RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. -4399 / 834 0.0 0.0 5.15 -671 / 3380 0.54(1) -424 / 740 -1244 / 256 U-T T-S-R R-Q-P P-O N-M M-L L-V V-K -304 / 320 -39.5 0.66(3)-833 / 4103 -39.5 -39.5 -39.5 -39.5 0.24(1)6.25 -3614/872 0.52 (1) NAIL VALUES -676 / 3977 0.23 (1) 6 25 V-W -27 / 1010 PLATE GRIP(DRY) SHEAR SECTION -676 / 3977 -972 / 5867 -39.5 -39.5 -39.5 0.23 (1) 0.35 (1) (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 100009024 -39.5 6.25 -737 / 4945 -39.5 -39.5 0.27 (1) 6.25 -531 / 4086 -531 / 4086 -671 / 4761 0.26 (1) 0.26 (1) 0.29 (1) -39.5 -39.5 -39.5 -39.5 OF ON ARIO POVIK PLATE PLACEMENT TOL. = 0.250 inches -39.5 -39.5 6.25 -39.5 -39.5 -39.5 -39.5 0.36 (1) 0.36 (1) -671 / 4761 PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.80 (F) (INPUT = 0.90) JSI METAL= 0.78 (B) (INPUT = 1.00) TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8) Structural component only DWG# T-2204185 //L CONTINUED ON PAGE 2

PLY JOB DESC. BAYVIEW WELLINGTON

TRUSS DESC. HOMES

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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPGQ, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE TRUS SUPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0

PSF AND 5.0 PSF RESPECTIVELY.



Structural component only DWG# T-2204185

JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. HOMES 417704 **T7** Tamarack Roof Truss, Burlingtor Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 07:57:57 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-09lclfKhrEvfOdexe?axyLEQXLuM9zJNknvb0DznH0e 1-3-8 5-10-8 31-8-4 2-0-12 5-10-8 Scale = 1:79. 8x12 \\ 5x6 = 5x6 =3x6 || 5x6 = 5x6 = 8x12 // D C E G Н 9.00 12 5-11-2 5-11-6 6x10 / 6x10 < B κ 1-6-4 B ۵ n v W S M L 6x10 = 5x12.5 Mii16 =5x17.5 MII16= 6x10 | 6x10 = 5x6 = 5x6 = 6x10 || 6x10 = 6x10 | 6x10 || 5x17.5 MII16= 2H25A 2H25A 35-7-8 1-11-4 , 2-0-0 , 2-0-0 , 2-0-0 , 1-11-4 45-6-0 1-3-8 TOTAL WEIGHT = 2 X 281 = 562 II LUMBER DIMENSIONS SUPPORTS AND LOADINGS SPECIFIED BY FARRICATOR TO BE VERIFIED BY N. L. G. A CHORDS BUILDING DESIGNER RITES **DESIGN CRITERIA** LUMBER DESCF SIZE BEARINGS SPECIFIED LOADS: SPF SPF SPF REQRD A C DRY No.2 FACTORED MAXIMUM FACTORED INPUT 2x6 2x6 DRY DRY GROSS VERT REACTION HORZ GROSS REACTION BRG DOWN HORZ UPLIFT IN-SX BRG IN-SX CH. 43.5 LL = DL = K 6.0 PSF = 10.5 = 7.4 = 67.3 W-2x6 DRY No.2 SPF W 5536 0 5593 -217 -870 5-8 5-8 BOT CH. LL DL 2x6 2x6 DRY No.2 SPF ō -934 W 2100F 1.8E 2100F 1.8E TOTAL LOAD PSF PROVIDE ANCHORAGE AT BEARING JOINT W FOR 870 LBS FACTORED UPLIF PROVIDE ANCHORAGE AT BEARING JOINT L FOR 934 LBS FACTORED UPLIFT 2x6 DRY SPF Q O õ 246 DRY 2100F 1 8F SPE SPACING = 24.0 IN. C/C 2100F 1.8E PROVIDE FOR 217 LBS FACTORED HORIZONTAL REACTION AT JOINT W ALL WEBS 2x4 DRY No.2 SPF LOADING IN FLAT SECTION BASED ON A SLOPE UNFACTORED REACTIONS
1ST LCASE ____MA MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE V THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015 DRY: SEASONED LUMBER. WIND COMBINED DEAD SOIL 2705 / 0 4309 / 0 487/0 0/0 483 / -930 400 / -993 839 / 0 1482 / 0 0/0 DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS 6278 510/0 THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT) FOLLOWS: HORIZONTAL REACTIONS 0/0 0/0 155 / -155 0/0 0 /0 **CHORDS #ROWS** SURFACE LOAD(PLF) SPACING (IN) CSA 086-14 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L TOP CHORDS: (0.122"X3") SPIRAL NAILS TPIC 2014 A- C C- I DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS 12 SIDE(70.1) MAX_UNBRACED TOP CHORD LENGTH = 2.91 FT. SIDE(70.1) SIDE(140.3) TOP TOP MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. USER-DEFINED. BOTTOM CHORDS: (0.122"X3") SPIRAL NAILS (80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR TOP TOP 1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-P, D-S, G-R, E-B. Q- T O- Q TOP END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE SIDE(227.4) WEBS : (0.122"X3") SPIRAL NAILS LOADING TOTAL LOAD CASES: (22) ALLOWABLE DEFL.(LL)= L/360 (1.52") CALCULATED VERT. DEFL.(LL)= L/999 (0.34") ALLOWABLE DEFL.(TL)= L/180 (3.03") CALCULATED VERT. DEFL.(TL)= L/999 (0.46") 2x4 H- N SIDE(1008.5) CHORDS WEBS NAILS TO BE DRIVEN FROM ONE SIDE ONLY. MAX, FACTORED FACTORED MAX. FACTORED FORCE VERT. LOAD LC1 MAX FORCE CSI: TC=0.61/1.00 (G-H:3) , BC=0.41/1.00 (P-R:3) , WB=0.78/1.00 (J-M:3) , SSI=0.17/1.00 (G-H:3) GIRDER NAILING ASSUMES NAILED HANGERS ARE CSI (LC) UNBRAC CSI (LC) (LBS) (PLF) FROM TO (LBS) FASTENED WITH MIN. 3-0 INCH NAILS. FR-TO ΤΌ LENGTH FR-TO A-B B-C C-D D-E E-F -145.3 -145.3 0.06 (2) -145.3 -145.3 0.34 (2) -145.3 -145.3 0.25 (2) 0 / 64 -6577 / 1011 -8107 / 1336 V- C C- U DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 -713 / 158 0.12 (7) - COMPONENTS ARE LOADED FROM THE TOP AND -735 / 4960 0.44 (3) MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. 4.08 U-D -3803 / 634 -145.3 -145.3 -145.3 -145.3 -145.3 -145.3 N- H N- I M- I SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 -11551 / 1757 -13521 / 1899 0.46 (2) 3.34 -1496 / 632 0.26 (2) -810 / 7853 0.69 (2) F- G -13521 / 1899 0.57 (2) 3.01 -827 / 167 0.14 (10) G- H H- X 0.61 (3) 0.40 (3) -659 / 5322 -730 / 8856 -14148 / 1815 -145.3 -145.3 291 B- V COMPANION LIVE LOAD FACTOR = 1.00 -145.3 -145.3 -145.3 -145.3 PROFESSIONAL ENGINE SH 02/08/22 H. J. G. ALVES X-1 -13305 / 1450 P-H AUTOSOLVE HEELS OFF 0.40(3)3.17 -589 / 1235 0.11(2)0.41 (3) 0.09 (2) 0.48 (3) -145.3 -145.3 -145.3 -145.3 -10944 / 1099 0.49 (3) 3.36 D-S -597 / 4646 J- K W- B 0 / 64 -5490 / 892 P-G S-E R-G -505 / 443 -2752 / 447 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. 0.06 (3) 10.00 0.0 0.0 L-J -8703 / 956 0.0 0.0 0.31 (3) 5.11 -975 / 151 0.16 (8) -232 / 2713 -982 / 282 0.24 (3) 0.16 (10) -39.5 -39.5 NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION V- U -722 / 5231 -39.50.16(2) 6.25 (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873. -1135 / 8108 -39.5 -39.5 -39.5 0.25 (2) 6.25 100009024 S-R -1556 / 11551 -39.5 -39.5 0.34 (2) 6.25 MT20 R- Q Q- P P- O -1579 / 14148 -1579 / 14148 -39.5 -39.5 0.41 (3) 0.41 (3) 6.25 MII16 438 302 2547 1256 4283 1816 -39.5 -39.5 6.25 6.25 POLINCE OF ONTARIO PLATE PLACEMENT TOL. = 0.250 inches -1179 / 13305 -39.5 0.40 (3) O- N N- Y Y- M -39.5 -39.5 -39.5 -39.5 -1179 / 13305 -39.5 0.40 (3) 6 25 -700 / 8715 -700 / 8715 -700 / 8715 -7 / 16 -39.5 -39.5 -39.5 -39.5 0.28 (3) 0.28 (3) 0.05 (17) 6.25 6.25 PLATE ROTATION TOL. = 5.0 Deg. M- Z Z-AA AA- L JSI GRIP= 0.85 (J) (INPUT = 0.90) JSI METAL= 0.81 (B) (INPUT = 1.00) 10.00 -7 / 16 -7 / 16 0.05 (17) -39.5 -39.5 Structural component only DWG# T-2204186 // CONTINUED ON PAGE 2

417704

QUANTITY

PLY

2

JOB DESC.

BAYVIEW WELLINGTON

TRUSS DESC. HOMES

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 07:57:57 2022 Page 2 ID:JvMhHdWEgsSl57rNnlib4czW6nl-09lclfKhrEvfOdexe?axyLEQXLuM9zJNknvb0DznH0e

DRWG NO.

 PLATES
 (table is in inches)

 JT
 TYPE
 PLATES

 B
 TMVW-t
 MT20

 C
 TTWW+m
 MT20

 D
 TMWW-t
 MT20
 LEN Y X 10.0 2.75 4.75 12.0 3.75 2.50 6.0 2.50 2.50 6.0 6.0 6.0 BCDEFGHLJL TMWW-t TMW+w TMWW-t MT20 MT20 MT20 MT20 6.0 2.50 2.50 1.00 2.75 4.75 10.0 Edge 0.50 10.0 17.5 10.0 17.5 10.0 2.50 2.50 12.5 10.0 2.50 2.50 17.5 10.0 2.50 2.50 17.5 10.0 2.50 4.75 10.0 2.50 4.75 10.0 2.50 4.75 10.0 2.50 4.75 10.0 2.50 4.75 10.0 2.50 4.75 TMWW-t TTWW+m TMVW-t MT20 MT20 MT20 MT20 MT20 BMV1+t MT20 MT20 MII16 MNOP RMWW-t BMWW+t BS-t BMWW-t MT20 MII16 MT20 MT20 BS-t BMWWW-t QRSTUV BMWW-t BS-t BMWW+t BMWW-t MII16 MT20 MT20 10.0 3.00 4.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

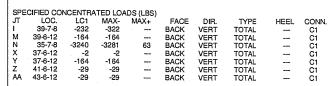
6.0

10.0 5.50

MT20

w BMV1+t

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



CONNECTION REQUIREMENTS

C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

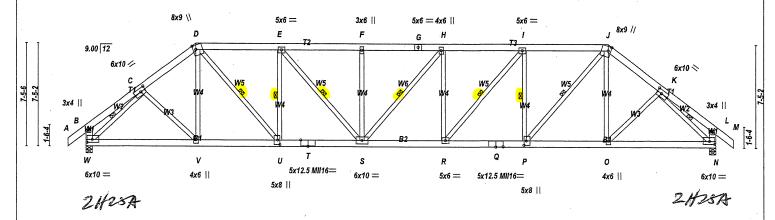
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, O.PO., BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



DWG# T-2204186

QUANTITY PLY JOB DESC. **BAYVIEW WELLINGTON** DRWG NO TRUSS DESC. 417704 **T8** 1 HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:26 2022 Page 1 $ID: JvMhHdWEgsSI57rNnlib4czW6nl-MWV2bzsC5e? iVWgNIRZ9zza_gbg7Ui7RS9tCgzznTsNlib4czW6nl-MWV2bzsC5e? iVWgNIRZ9zza_gbg7Ui7RS9tCgzznTsNlib4czw6ui7W6zyw6nl-MWV2bzyw6nl-MWV2bzyw6nl-MWV2bzyw6nl-MWV2bzyw6nl-MWV2byw6nl-MWV2byw6nl-MWV2byw6nl-MWV2byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02byw6nl-MWW02by$ 1-3-8 7-10-8 29-9-0 7-10-8 Scale = 1:79.



<u>45-6-0</u> 45-6-0 1-3-8 TOTAL WEIGHT = 297 lb

ł					
	LUMBER	=0			
1	N. L. G. A. F				
ı	CHORDS	SIZE		LUMBER	DESCR.
ı	A - D	2x6	DRY	No.2	SPF
ı	D - G	2x6	DRY	No.2	SPF
ı	G - J	2x6	DRY	No.2	SPF
ı	J - M	2x6	DRY	No.2	SPF
1	W - B	2x6	DRY	No.2	SPF
ı	N - L	2x6	DRY	No.2	SPF
ı	W - T	2x6	DRY	2100F 1.8E	SPF
ı	T - Q	2x6	DRY	2100F 1.8E	SPF
ı	Q - N	2x6	DRY	2100F 1.8E	SPF
١	1				
ı	ALL WEBS	2x4	DRY	No.2	SPF
ı	EXCEPT				

DRY: SEASONED LUMBER.

l	PL/	ATES (table i	s in inches)				
۱	JT	TYPE	PLATES	W	LEN	Υ	Х
۱	В	TMV+p	MT20	3.0	4.0		
ļ	С	TMWW-t	MT20	6.0	10.0	3.00	4.25
ı	D	TTWW+m	MT20	8.0	9.0	3.75	2.25
I	Е	TMWW-t	MT20	5.0	6.0		
I	F	TMW+w	MT20	3.0	6.0		
ı	G	TS-t	MT20	5.0	6.0		
	Н	TMWW+t	MT20	4.0	6.0		
1		TMWW-t	MT20	5.0	6.0		
	J	TTWW+m	MT20	8.0	9.0	3.75	2.25
	K	TMWW-t	MT20	6.0	10.0	3.00	4.25
	L	TMV+p	MT20	3.0	4.0		
	N	BMVW1-t	MT20	6.0	10.0	2.50	4.50
	0	BMWW+t	MT20	4.0	6.0		
١	Р	BMWW+t	MT20	5.0	8.0	4.00	2.00
ı	Q	BS-t	MII16	5.0	12.5		6.25
ı	R	BMWW-t	MT20	5.0	6.0		
	S	BMWWW-t	MT20	6.0	10.0		
ı	Т	BS-t	MII16	5.0	12.5	Edge	6.25
	U	BMWW+t	MT20	5.0	8.0	4.00	2.00
	٧	BMWW+t	MT20	4.0	6.0		
	W	BMVW1-t	MT20	6.0	10.0	2.50	4.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

NOTES-PROFESSIONAL ENGINE PROFESSIONAL ENGINE BY OLIOS/12 H. J. G. ALVES 100009024 POWER OF ONT ARIO

Structural component only DWG# T-2204187 //

MENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
UILDING DESIGNER
EARINGS
EATHAGO

:AR	INGS						
FACTORED			MAXIMUM FACTORED			INPUT	REQRD
GROSS REACTION			GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	4412	0	4412	-263	-835	5-8	5-8
	4412	0	4412	0	-835	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 835 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT N FOR 835 LBS FACTORED UPLIFT

PROVIDE FOR 263 LBS FACTORED HORIZONTAL REACTION AT JOINT W

UNF/	UNFACTORED REACTIONS											
	1ST LCASE	MAX./	MIN. COMPON									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
W	3205	2101/0	478 / 0	0/0	454 / -898	627 / 0	0/0					
N	3205	2101 / 0	478 / 0	0/0	302 / -898	627 / 0	0/0					
HOR W	IZONTAL RE	ACTIONS 0 / 0	. 0/0	0/0	188 / -188	0/0	0 /0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

 $\frac{\text{BRACING}}{\text{MAX. UNBRACED TOP CHORD LENGTH}} = 2.87 \text{ FT.} \\ \text{MAX. UNBRACED BOTTOM CHORD LENGTH} = 6.25 \text{ FT} \text{ OR RIGID CEILING DIRECTLY APPLIED.} \\$

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-W, K-N, J-P, D-U, I-P, E-U, I-R, E-S, H-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

JT W N

LOADING TOTAL LOAD CASES: (18)

		,								
	ORDS	FACTO	200	WEBS						
							MAX. FACTO			
MEMB.	FORCE	VERT. LO			MAX.	MEMB		MAX		
	(LBS)			CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
A- B	0 / 64	-145.3	-145.3	0.11 (2)	10.00	C- V	0/622	0.10 (3)		
B- C	-34 / 154	-145.3	-145.3	0.14 (2)	6.25	V- D	-197 / 278	0.12 (10)		
C- D	-5006 / 1064	-145.3	-145.3	0.28 (2)	3.71	O- J	-175 / 297	0.11 (9)		
D-E	-5911 / 1289	-145.3	-145.3	0.60(2)	3.15	O- K	-63 / 622	0.10(2)		
E-F	-6800 / 1434	-145.3	-145.3	0.67(1)		W-C	-5385 / 925	0.83 (1)		
F-G	-6800 / 1434	-145.3	-145.3				-5386 / 926	0.83 (1)		
G-H	-6800 / 1434	-145.3	-145.3			P-J	-631/3119	0.50 (2)		
H-I	-6810 / 1437	-145.3					-632/3123	0.50 (3)		
I- J	-5909 / 1288	-145.3					-2149 / 521	0.49 (2)		
-J- K	-5006 / 1064	-145.3					-2142/519	0.49 (3)		
K-L	-33 / 154	-145.3				R- I	-296 / 1541	0.25 (2)		
L- M	0 / 64	-145.3	-145.3				-290 / 1523	0.24 (3)		
W-B	-478 / 205	0.0	0.0				-935 / 278	0.58 (2)		
N- L	-477 / 205	0.0	0.0				-887 / 252	0.54 (10)		
				0.00 (0)		S- H	-133 / 104	0.05 (3)		
W-V	-780 / 3788	-39.5	-39.5	0.26 (1)	6.25			0.00 (0)		
V- U	-667 / 3977	-39.5	-39.5							
U- T	-1038 / 5912	-39.5	-39.5							
T-S	-1038 / 5912	-39.5	-39.5	0.33 (1)	6.25					
S-R	-1165 / 6810	-39.5	-39.5							
R-Q	-974 / 5910	-39.5	-39.5							
Q-P	-974 / 5910	-39.5	-39.5							
P- 0	-559 / 3977	-39.5	-39.5	0.26 (1)	6.25					
O- N -	-636 / 3788	-39.5	-39.5	0.26 (1)						
				. '						
				••						
TOUGO	LIAO DEEN OU									

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

	IFIED	LOAD	S:		
TOP	CH.	LL		43.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL		7.4	PSF
TOTA	L LO	AD	=	67.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014

DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR USED - PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE

ALLOWABLE DEFL.(LL) = L/360 (1.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.31")
ALLOWABLE DEFL.(TL) = L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.67/1.00 (E-F:1) , BC=0.37/1.00 (R-S:1) , WB=0.83/1.00 (K-N:1) , SSI=0.32/1.00 (D-E:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 438 302 2547 1256 4283 1816 MII16

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (N) (INPUT = 0.90) JSI METAL= 0.77 (K) (INPUT = 1.00)

RUSS NAME
TRUSS NAME
TRUSS NAME
TRUSS NAME
TRUSS DESC.
BAYVIEW WELLINGTON
TRUSS DESC.
HOMES

Version 8.420 S Jan 21 2021 MITEK Industries, Inc. Mon Feb 7 17:21:26 2022 Page 2
ID:JvMhHdWEgsSI57rNnlib4czW6nl-MWV2bzsC5e?ivWgNIRZ9zza gbg7Ui7RS9tCgzznTsN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPC9, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE-TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



JOB DESC PLY **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. Т9 417704 HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MITEk Industries, Inc. Mon Feb 7 17:21:27 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-qi3QpJtqsy7Z7gFZs94OWB69c?xEDB3ahpcmCQznTsM 9-10-8 ,1-3-8, 25-9-0 9-10-8 Scale = 1:79.4 8x9 \\ 4x6 || 3x6 || 5x6 =4x6 || 8x9 // D Ε F G Н 9.00 12 5x6 // 5x6 > 6x10 // 6x10 > L T s P U R Q 0 N M 5x6 =7x8 | 6x10 == 4x6 || 5x6 || 5x6 == 5x8 == 4x6 || 6x10 == 7x8 || 5x6 || 21125 K 211254 45-6-0 1-3-8 45-6-0 1-3-8 TOTAL WEIGHT = 308 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DÉSIGNER **DESIGN CRITERIA** SIZE LUMBER DESCR A -D -G -DG SPF MAXIMUM FACTORED GROSS REACTION No.2 INPUT REQRD SPECIFIED LOADS: LUAUS: LL = 43.5 DL = 6.0 LL = 10.5 DL = 7.4 AD = 67.3 DRY DRY DRY DRY No.2 BRG IN-SX PSF PSF 2x6 **GROSS REACTION** BRG CH. HORZ 0 HORZ -310 2x6 No 2 SPF VERT DOWN UPLIFT IN-SX SPF BOT CH. -821 5-8 PSF W -M -BKTP 2x6 No.2 SPF 4412 0 4412 0 -821 5-8 5-8 SPF SPF SPF 2x6 DRY No.2 TOTAL LOAD W 2x6 DRY DRY PROVIDE ANCHORAGE AT BEARING JOINT W FOR 821 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT M FOR 821 LBS FACTORED UPLIFT 2x6 No.2 SPACING = 24.0 IN. C/C М 2x6 DRY No.2 SPF PROVIDE FOR 310 LBS FACTORED HORIZONTAL REACTION AT JOINT W ALL WEBS 2x4 DRY SPF No.2 LOADING IN FLAT SECTION BASED ON A SLOPE EXCEPT UNFACTORED REACTIONS MAX 1ST LCASE COMBINED ./MIN. COMPONENT REACTIONS
LIVE PERM.LIVE WIND DRY: SEASONED LUMBER. DEAD SOIL THIS TRUSS IS DESIGNED FOR COMMERCIAL 3205 2101/0 478 / 0 0/0 439 / -888 627 / 0 0/0 OR INDUSTRIAL BUILDING REQUIREMENTS OF 3205 2101/0 0/0 272 / -888 PART 4, NBCC 2015 HORIZONTAL REACTIONS THIS DESIGN COMPLIES WITH-PLATES (table is in inches)
JT TYPE PLATES
B TMVW-t MT20 - PART 4 OF BCBC 2018 , ABC 2019 - PART 4 OF OBC 2012 (2019 AMENDMENT) 0/0 0/0 0/0 221 / -221 0/0 0 /0 LEN Y X 10.0 2.75 5.00 6.0 2.50 2.75 BCD BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W. M. - CSA 086-14 TMWW-t MT20 5.0 - TPIC 2014 TTWW+m 8.0 4.0 3.0 5.0 4.0 8.0 5.0 6.0 7.0 6.0 3.75 2.50 TMWW+I XX. UNBRACED TOP CHORD LENGTH = 3.11 FT. DESIGN ASSUMPTIONS MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. - SLOPE REDUCTION FACTOR USED - PERCENTAGE OF GROUND SNOW LOAD IS MT20 6.0 6.0 6.0 9.0 3.75 2.50 6.0 2.50 2.75 10.0 2.75 5.00 8.0 Edge 1.50 10.0 3.00 4.75 G TS-t MT20 TMWW+t TTWW+m ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. USER-DEFINED. (80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR TMWW-t MT20 1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-U, D-S, E-S, E-R, F-R, H-R, H-Q, I-Q, I-Q. MT20 MT20 TMVW-t BMV1+t BMWW-t ID VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW MT20 4.0 5.0 6.0 BMWW+t MT20 BS-t BMWW+t LOADING TOTAL LOAD CASES: (18) 2.75 2.00 MT20 6.0 BMWWW-t MT20

NOPQRST 5.0 5.0 5.0 5.0 4.0 BMWW+t 2.75 2.00 BMWW+t MT20 6.0 BMWW-t 10.0 3.00 4.75 8.0 5.50 MT20

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

PROFESSIONAL ENGINE BY DE 106/12 H. J. G. ALVES 100009024 OF ON ARIO POUN

Structural component only DWG# T-2204188

С	HORDS			WEBS					
M	XX. FACTORED	FACTORED				MAX. FACTO	RED		
MEME	. FORCE	VERT, LOAD LO	C1 MAX	MAX.	MEMB.		MAX		
	(LBS)	(PLF)	CSI (LC)			(LBS)	CSI (LC)		
FR-TC		FROM TO	()	LENGTH		(250)	00. (20)		
A-B	0 / 64	-145.3 -145.	3 0.11 (2)			-681 / 184	0.17 (7)		
B- C	-4971 / 963	-145.3 -145.			Ċ- Ŭ	-235 / 291	0.14 (2)		
C-D	-4886 / 1064	-145.3 -145.			U- D	-112/429	0.07 (5)		
D- E	-5287 / 1181	-145.3 -145.			D-S	-511 / 2557	0.41 (3)		
E-F	-5699 / 1229	-145.3 -145.				-1776 / 464	0.60 (3)		
F-G	-5699 / 1229	-145.3 -145.			E-R	-175 / 878	0.14 (3)		
G-H	-5699 / 1229	-145.3 -145.			R-F	-926 / 263	0.30 (10)		
H- I	-5287 / 1181	-145.3 -145.			R- H	-175 / 878	0.14 (2)		
l- J	-4886 / 1064	-145.3 -145.				-1771 / 464	0.60 (2)		
J- K	-4971 / 963	-145.3 -145.			Q-1	-511/2557	0.41 (2)		
K-L	0 / 64	-145.3 -145.			õ-i	-112/429	0.07 (6)		
W-B	-4311 / 835	0.0 0.			Ō- J	-251 / 291	0.15 (8)		
M- K	-4311 / 836	0.0 0.			N- J	-677 / 184	0.17 (7)		
			,		B- V	-641 / 4089	0.66 (1)		
W-V	-294 / 302	-39.5 -39.	5 0.09 (4)	6.25	N-K	-641 / 4089	0.66 (1)		
V-U	-789 / 4010	-39.5 -39.							
U- T	-629 / 3869	-39.5 -39.							
T-S	-629 / 3869	-39.5 -39.							
S-R	-881 / 5287	-39.5 -39.							
R-Q	-823 / 5287	-39.5 -39.							
Q-P	-511 / 3869	-39.5 -39.							
P-0	-511 / 3869	-39.5 -39	5 0.54 (1)	6.25					
0- N	-613 / 4010	-39.5 -39							
N-M	-7 / 16	-39.5 -39							

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING

ALLOWABLE DEFL.(LL) = L/360 (1.52") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25") ALLOWABLE DEFL.(TL) = L/180 (3.03") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.66/1.00 (F-H:1) , BC=0.70/1.00 (Q-R:1) , WB=0.66/1.00 (K-N:1) , SSI=0.35/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.90) JSI METAL= 0.76 (K) (INPUT = 1.00)

Temera Rod Trus, Sufrigion Te	OB NAME TR	USS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WEL	LINGTON		DRWG NO.		
ID:JvMhHdWEqsSl57rNnlib4czW6nl-qi3QpJtqsy7Z7qFZs94OWB69c?xEDB3ahpcmC0		9	1	1	TRUSS DESC	LICIVILO					
	amarack Roof Truss, Burlington					ID: MhHdW	Version 8.4	120 S Jan 21 2021 MIT	ek Industries, Inc. 1	Mon Feb 7 17:2	1:27 2022
WIND LADA APPLIED IS DERIVED FROM RETERENCE VELICATIVE PRESSURE OF \$1.75 PSF AT 180-00 PT-MISK REPERENCE VERBITA MOVE GRADE AND USING EXTERNAL PEAK COMPRISED AND USING COMPRISED AND USIN						ID JOYNA II ICVV	Eggolo711411115-10	SERVOIT GIOGROTIGSY	121g1 23040WD	OSC: XLDDSA	i ipunio Qz
WIND LOAD APPLED EDERVICE PROMISED VELOCITY PROSESSING OF 17.5) PSF AT TO COEFFICIENTS, CELL, BASED ON THE PARM VINDE PROSESSING OF STEMBLIFFENAL OF THE PARM VINDE PRESSURE IS MASED ON DESIGN (CATEGORY 3), BULLDRIA MAY BE LOCATED ON OF THE PARM VINDE PRESSURE IS MASED ON TOP AND SOTTEM CHARGO SEAD LOADS OF SO PSF ASSPECTIVELY.											
COEPTICIENTS, CO.S., BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM, INTERNAL WIND COPEN TERMAN, MAIN TRUSS IS DESIRED TO SELECTOR AT LESS THAT HAS AN ANY FROM ENVE TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.3 PSF ASSPECTIVELY.			WIND LOAD AF {30-0-0} FT-IN-	PPLIED IS DE SX REFERE	ERIVED FROM REFE NCE HEIGHT ABOVE	RENCE VELOCITY P GRADE AND USING	PRESSURE OF { 7.5 G EXTERNAL PEAK	5) PSF AT			
FROM EAVIC TRUSS DELIFF IS BASED ON TOP AND BOTTOM CHIEFED DEAD LOADS OF 58 RSF AND 5.0 PSP RESPECTIVELY.	•		WIND PRESSL	S, CpCg, BAS JRE IS BASE	SED ON THE (MAIN D ON DESIGN (CAT	WIND FORCE RESIS EGORY 2}. BUILDING	TING SYSTEM}.IN MAY BE LOCATE	TERNAL D ON			
O ALL SUPERINCE!			FROM EAVE.T	RUSS UPLIF	T IS BASED ON TOP	O BE LOCATED AT I	LEAST (0-0) FT-IN- ORD DEAD LOADS	OF 5.0			
			FOR AND 5.01	FOF RESPE	STIVELY.			-			
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Structural component only DWG# T-2204188 WW

JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. 417704 T10 HOMES Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:28 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nI-Ivdo0fuSdGFQkqqiQscd2OflgPHhyeBkwTMJksznTsL Tamarack Roof Truss, Burlington 1-3-8 11-6-0 .22-6-0 Scale = 1:81.0 8x9 \\ 3x6 || 5x6 = 4x6 || 9.00 12 8x9 // F Н G 5x6 //

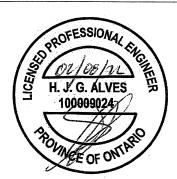
5x6 <> 5x6 // 5x6 <> D c κ 10-2-0 10-1-12 6x10 // 6x10 > М [4] 8 Q W s R 0 N 5x6 5x6 =7x8 || 6x10 == 4x6 || 6x10 == 5x6 || 4x6 || 6x10 == 7x8 || 2014 2H257 45-6-0

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
l - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

ı	PLATES (table is in inches)								
į	JT	TYPE	PLATES	W	LEN	Υ	Χ		
į	В	TMVW-t	MT20	6.0	10.0	2.75	5.00		
l	С	TMWW-t	MT20	5.0	6.0	2.50	2.75		
١	D, 0	a, J							
ı	D	TS-t	MT20	5.0	6.0				
ĺ	Ε	TTWW+m	MT20	8.0	9.0	3.75	2.50		
ı	F	TMW+w	MT20	3.0	6.0				
1	Н	TMWW+t	MT20	4.0	6.0				
	1	TTWW+m	MT20	8.0	9.0	3.75	2.50		
ı	K	TMWW-t	MT20	5.0	6.0	2.50	2.75		
	L	TMVW-t	MT20	6.0	10.0	2.75	5.00		
i	N	BMV1+t	MT20	7.0	8.0	Edge	1.50		
ı	0	BMWW-t	MT20	6.0	10.0				
ı	Р	BMWW+t	MT20	4.0	6.0				
ļ	Q	BŞ-t	MT20	5.0	6.0				
ı	R	BMWW+t	MT20	5.0	6.0	3.00	2.25		
ı	S	BMWWW-t	MT20	6.0	10.0				
i	T	BS-t	MT20	5.0	6.0				
i	U	BMWW+t	MT20	4.0	6.0				
	٧	BMWW-t	MT20	6.0	10.0				
ļ	W	BMV1+t	MT20	7.0	8.0	5.50			
ı									

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD



Structural component only DWG# T-2204189 ///

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED	BY FABRICATOR TO	BE VERIFIED BY
BUILDING DESIGNER			
PEADINGS			

45-6-0

EΑ	RINGS						
	FACTO	RED	MAXIMU	MAXIMUM FACTORED			REQRD
	GROSS F	REACTION	GROSS	GROSS REACTION			BRG
•	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	4412	0	4412	-347	-808	5-8	5-8
	4412	0	4412	0	-808	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 808 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT N FOR 808 LBS FACTORED UPLIFT

PROVIDE FOR 347 LBS FACTORED HORIZONTAL REACTION AT JOINT W

UNF	UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS									
N N		SNOW 2101 / 0 2101 / 0	LIVE 478 / 0 478 / 0	PERM.LIVE 0 / 0 0 / 0	WIND 425 / -879 250 / -879	DEAD 627 / 0 627 / 0	SOIL 0/0 0/0			
HOF W	RIZONTAL RE	ACTIONS 0/0	0/0	0/0	248 / -248	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.18 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U, E-S, F-S, H-S, H-R, I-R, I-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

N

LOADING TOTAL LOAD CASES: (18)

		. ,						
	ORDS C. FACTORED	FACTORED		W E B S MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD LC1		MAX.	MEMB		MAX	
	(LBS)			UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH				
A- B	0 / 64	-145.3 -145.3	0.11 (2)	10.00	V- C	-564 / 170	0.18 (7)	
B- C	-5044 / 959	-145.3 -145.3	0.56 (2)	3.43	C- U	-487 / 299	0.43 (2)	
C- D	-4756 / 1040	-145.3 -145.3	0.53(2)	3.54	U- E	-153 / 619	0.10 (2)	
D- E	-4756 / 1040	-145.3 -145.3	0.53 (2)	3.54	E-S	-435 / 2137	0.35 (14)	
E-F	-4898 / 1110	-145.3 -145.3	0.75 (1)		S-F	-1268 / 361	0.57 (10)	
F-G	-4898 / 1111	-145.3 -145.3	0.76(1)		S-H	-254 / 238	0.21 (3)	
G-H	-4898 / 1111	-145.3 -145.3	0.76 (1)			-1397 / 414	0.65 (2)	
H- I	-4902 / 1112	-145.3 -145.3		3.18	R-I	-438 / 2146	0.36 (13)	
i- J	-4755 / 1040	-145.3 -145.3	0.53 (3)	3.54	P- I	-151 / 614	0.10 (3)	
J- K	-4755 / 1040	-145.3 -145.3	0.53 (3)	3.54	P-K	-488 / 300	0.43 (3)	
K-L	-5044 / 960	-145.3 -145.3			0- K	-559 / 170		
L- M	0 / 64	-145.3 -145.3	0.11 (3)	10.00	B- V		0.18 (7)	
W-B	-4302 / 826					-623 / 4138	0.67 (1)	
		0.0 0.0	0.28 (1)	5.20	O- L	-623 / 4138	0.67 (1)	
N- L	-4302 / 826	0.0 0.0	0.28 (1)	5.20				
W-V	-332 / 340	-39.5 -39.5	0.12 (4)	6.25				
V- U	-799 / 4078	-39.5 -39.5		6.25				
U- T								
	-583 / 3760	-39.5 -39.5	0.56 (1)	6.25				
T-S	-583 / 3760	-39.5 -39.5		6.25				
S-R	-732 / 4903	-39.5 -39.5		6.25				
R-Q	-463 / 3760	-39.5 -39.5	0.55 (1)					
Q-P	-463 / 3760	-39.5 -39.5				`		
P-O	-599 / 4078	-39.5 -39.5	0.59 (1)	6.25				
O- N	-7 / 16	-39.5 -39.5	0.12 (4)	10.00				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS: LL = 43.5 DL = 6.0 LL = 10.5 DL = 7.4 AD = 67.3 PSF CH. 6.0 PSF BOT CH. PSF PSF PSF TOTAL LOAD

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

1-3-8 TOTAL WEIGHT = 306 lb

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE

ALLOWABLE DEFL.(LL)= L/360 (1.52") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.21") ALLOWABLE DEFL.(TL)= L/180 (3.03") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.28")

CSI: TC=0.76/1.00 (H-I:1) , BC=0.69/1.00 (R-S:1) , WB=0.67/1.00 (L-O:1) , SSI=0.40/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 MT20

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90) JSI METAL= 0.77 (L) (INPUT = 1.00)

в NAME 17704	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	BAYVIEW WELLINGTON HOMES		DRWG NO.		
marack Roof Truss, Bu					Version 8.420 S Jan 21 20 ID:JvMhHdWEgsSI57rNnlib4czW6nI-lvd	21 MiTe do0fuS	I k Industries, Inc. Mo dGFQkqqlQscd20	n Feb 7 17:21:28 20 OflgPHhyeBkwTM	22 Pag Jkszn
OTES- (1) Lateral braces to be a	minimum of 2X4 SPF #2.	WIND LOAD AI (30-0-0) FT-IN- COEFFICIENT WIND PRESSI (OPEN TERRA FROM EAVE.T PSF AND 5.0	PPLIED IS DERIN SX REFERENCE S, CpCg, BASED JRE IS BASED C ININ, AND TRUSS RUSS UPLIFT IS PSF RESPECTIN	/ED FROM REFER HEIGHT ABOVE ON THE (MAIN W IN DESIGN (CATE: IS DESIGNED TO BASED ON TOP /ELY.	IENCE VELOCITY PRESSURE OF { 7.5} PSF AT GRADE AND USING EXTERNAL PEAK INID FORCE RESISTING SYSTEM, INTERNAL GORY 2}. BUILDING MAY BE LOCATED ON I BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY AND BOTTOM CHORD DEAD LOADS OF 5.0				
				No.					
				÷					
	l	ļ							
201	ESSIONA								
EWSE)	FESSIONAL FIGURES CONTROL FOR THE STATE OF T								
1 7									
Roun	OF ONTARIO								
Structural of	component only 2204189 W								

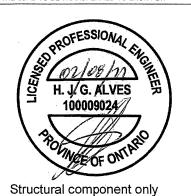
JOB DESC. BAYVIEW WELLINGTON PLY DRWG NO. TRUSS DESC. 417704 T11 2 HOMES Famarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:30 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-EHkYRLvi9tV8_8z8XHe57pkl9C2rQcQ0NnrQplznTsJ 5-10-8 1-3-8 22-6-4 1-2-12 5-10-8 Scale = 1:61.7 4x6 | 7x8 \\ 4x4 =2x4 || 3x6 = 4x4 =4x6 || 7x8 // D F н G 9.00 12 5x8 🖊 5x8 < ₩ M s Q AA V u 0 R N 3x6 || 5x6 =5x6 3x6 || 5x8 || 5x8 5x8 = 5x8 || 5x8 == 5x6 5x6 == 27-5-8 11-4, 1-2-0; 2-0-0 2-0-0 1-11-4 1-3-8 35-6-0 1-3-8 TOTAL WEIGHT = 2 X 189 = 378 II LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS, AND LOADINGS SPECIFIED BY FARRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED DESIGN CRITERIA SIZE LUMBER CGJ DRY 2x4 2100F 1.8E SPF SPF MAXIMUM FACTORED INPUT REORD SPECIFIED LOADS: 2x4 2x4 GROSS REACTION VERT HORZ GROSS REACTION BRG DOWN HORZ UPLIFT IN-SX BRG IN-SX DRY No 2 PSF PSF CH. LL = DL = DRY JT W 6.0 0.0 7.4 2x4 2x6 2x6 J -W -M -2100F 1.8E SPF 2725 2725 0 0 5-8 5-8 5-8 BOT CH. LL = PSF DRY No.2 No.2 SPF SPF М 3562 ñ DL TOTAL LOAD BKSQ DRY 45.9 PSF SPF 2x6 No.2 UNFACTORED REACTIONS
1ST LCASE MAX
JT COMBINED SNOW 2x6 DRY No.2 SPF SPACING = 24.0 IN. C/C ō-No.2 SPF C./MIN. COMPONENT REACTIONS WIND DEAD LIVE PERM.LIVE SOIL ALL WEBS 2x3 DRY No.2 SPF 1368 / 0 0/0 0/0 0/0 538 / 0 0/0 LOADING IN FLAT SECTION BASED ON A SLOPE EXCEPT 1777 / 0 SPF N - K B - V THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W. M. 2x4 DRY No.2 DRY: SEASONED LUMBER. BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) FOLLOWS: ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. - CSA 086-14 LOADING TOTAL LOAD CASES: (4) TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD CHORDS WERS

CHORD	S #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CH	ORDS: (0.1	22"X3") SPIRAL NAILS	;
A-C	1 `	12	TOP
C-G	1	12	TOP
G-J	1	12	SIDE(61.0)
J- L	1	12	SIDE(65.9)
W-B	2	12	TOP
M-K	2	12	TOP
BOTTON	M CHORDS	: (0.122"X3") SPIRAL N	IAILS
W-S	2	12	TOP
S-Q	2	12	TOP
Q-M	2	12	SIDE(183.1)
WEBS:	(0.122"X3")	SPIRAL NAILS	
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



DWG# T-2204190

MA	X. FACTORED	FACTO	RED				MAX. FACT	ORED	
MEMB				MAX	MAX	MEMB			
	(LBS)				UNBRAC		(LBS)		C)
FR-TO	()	FROM		(,	LENGTH			33. (2	-0,
A- B	0 / 47			0.05 (1)		V-C		0.08 (1)
B- C	-2932 / 0			0.29 (1)		N- J		0.02 (
C- D	-2981 / 0			0.14(1)		N- K	0/3256	0.29 (
D-E	-4048 / 0				4.50	B- V	0 / 2382	0.21 (
E-F	-4526 / 0			0.28 (1)			-1150/0	0.30 (
F- G	-4526 / 0			0.28 (1)			0/1752	0.22 (
G-H	-4526 / 0			0.28 (1)		U- D	-1637/0	0.43 (
H- I	-4477 / 0				4.30	C- U	0/1768	0.22 (1)
I- X	-3839 / 0			0.16 (1)		P-1			
X-J	-3839 / 0			0.16 (1)		D- T		0.20	
J- K	-4008 / 0			0.32 (1)		P-H		0.16	
K-L	0 / 47			0.05 (1)		T-E	-1117/0	0.29	
W-B	-2684 / 0	0.0		0.10(1)		R- H	0/73	0.01	
M-K	-3490 / 0	0.0		0.13 (1)		E-R		0.09	
				٠,		R-F		0.14	
W-V	0/0	-18.5	-18.5	0.04 (4)	10.00				(- /
V- U	0 / 2337	-18.5	-18.5	0.18 (1)	10.00				
U- T	0 / 2981	-18.5	-18.5	0.22 (1)	10.00				
T-S	0 / 4048	-18.5	-18.5	0.29 (1)	10.00				
S-R	0 / 4048	-18.5		0.29 (1)					
R-Q	0 / 4477	-18.5		0.32 (1)					
Q-P	0 / 4477	-18.5		0.32 (1)					
P- 0	0 / 3839	-18.5		0.28 (1)					
0- Y	0 / 3200	-18.5		0.25 (1)					
Y-N	0 / 3200	-18.5	-18.5	0.25 (1	10.00				
N-Z	0 / 0	-18.5		0.07 (4					
Z-AA	0/0	-18.5		0.07 (4					
AA- M	0/0	-18.5	-18.5	0.07 (4	10.00				
SPEC	IFIED CONCEN	TRATED LO	DADS (L	BS)					
JT		C1 MAX	MAX	(+ F	ACE	DIR.	TYPE	HEEL	CONN.
J	29-7-8 -2	75 -275		BA	CK V	ERT	TOTAL		C1
N	29-6-12 -1	64 -164		BA	CK V	ERT	TOTAL		C1
0		55 -255	, .	BA	CK V	ERT	TOTAL		C1
X	28-4-12 -	15 -15	, .	BA	CK V	ERT	TOTAL		C1
Υ		64 -164				ERT	TOTAL		C1
Z		29 -29				ERT	TOTAL		C1
AA	33-6-12 -	29 -29		BA	CK V	ERT	TOTAL		C1
1									

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

ALLOWABLE DEFL.(LL)= U/360 (1.18")
CALCULATED VERT. DEFL.(LL) = U/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.18")
CALCULATED VERT. DEFL.(TL)= U/999 (0.20")

CSI: TC=0.32/1.00 (J-K:1) , BC=0.32/1.00 (P-R:1) , WB=0.43/1.00 (D-U:1) , SSI=0.13/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (D) (INPUT = 0.90) JSI METAL= 0.40 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLY JOB DESC. BAYVIEW WELLINGTON DRWG NO. TRUSS DESC. HOMES 2 T11

417704
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTTek Industries, Inc. Mon Feb 7 17:21:30 2022 Page 2 ID:JvMhHdWEgsSI57rNnlib4czW6nI-EHkYRLvi9tV8 8z8XHe57pkl9C2rQcQ0NnrQplznTsJ

PLATES (table is in inches)
JT TYPE PLATES
B TMVW+t MT20
C TTWW+m MT20
E TMWW+t MT20
E TMWW+t MT20
F TMW+w MT20
G TS-t MT20
H TMWW+t MT20
J TTWW+m MT20
J TTWW+m MT20
K TMWW+t MT20
J TTWW+m MT20
J TTWW+m MT20
J TTWW+m MT20
N BMW+t MT20
N BMW+t MT20
N BMW+t MT20
N BMW+t MT20
P BMWW+t MT20
P BMWW+t MT20
P BMWW+t MT20
B S-t MT20
R BMWW+t MT20
S BS-t MT20
T BMWW+t MT20
U BMW+t MT20
U BMWW+t MT20 W 5.0 7.0 4.0 4.0 2.0 4.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 3.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only DWG# T-2204190 W

JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO TRUSS DESC. HOMES 417704 T12 Tamarack Roof Truss, Burlingtor Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:31 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-jUlxfgwLwBd?blYK5?9Kg0HsxcMZ9x2AcRazLBznTsl , 1-3-8 , 7-10-8 19-9-0 7-10-8 7x8 \\ 4x4 =2x4 || 3x6 = 4x4 =7x8 // D E F Н G 9.00 12 5x6 // 5x6 > 3x4 || 3x4 || K L R P Т s Q 0 5x8 = 3x6 == 5x8 = 4x4 = 4x10 = 4x6 || 4x4 4x6 || 3x6 35-6-0 1-3-8 35-6-0 1-3-8 TOTAL WEIGHT = 170 lb LUMBER DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY N. L. G. A CHORDS BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCF BEARINGS FACTORED SPF SPF SPF SPF Ā D G D G I 2x4 DRY No.2 MAXIMUM FACTORED INPLIT REORD SPECIFIED LOADS: DRY DRY DRY No.2 No.2 GROSS VERT REACTION HORZ GROSS REACTION DOWN HORZ UPLIFT BRG IN-SX LL = DL = LL = DL = DAD = BRG CH. PSF IN-SX 6.0 PSF 2x4 No.2 2479 0 2479 0 0 5-8 5-8 5-8 BOT CH. 0.0 PSF . U M U 2x6 2x6 DRY No.2 No.2 SPF SPF SPF 2479 ŏ B K R P DRY TOTAL LOAD 45.9 PSF 2x4 DRY No.2 R 2×4 DRY No 2 SPF UNFACTORED REACTIONS
1ST LCASE MA
JT COMBINED SNOW SPACING = 24.0 IN. C/C MIN. COMPONENT REACTIONS SNOW WIND PERM.LIVE DEAD SOIL LIVE ALL WEBS 2x3 DRY No.2 SPF 1735 1242 / 0 0/0 0/0 0/0 LOADING IN FLAT SECTION BASED ON A SLOPE EXCEPT 1242 / 0 OF 6.00/12 SPF No.2 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M 2x4 DRY No.2 SPF DRY: SEASONED LUMBER. BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 3.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. PLATES (table is in inches)
JT TYPE PLATES
B TMV+p MT20 LEN Y **TPIC 2014** TMV+p TMWW-t ВС 3.0 5.0 7.0 4.0 2.0 3.0 4.0 7.0 5.0 4.0 MT20 6.0 2.25 2.25 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED 8.0 4.0 4.0 6.0 TTWW+m MT20 Edge 2.50 THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW TMWW-t TMW+w ROOF LIVE LOAD LOADING TOTAL LOAD CASES: (4) MT20 ALLOWABLE DEFL.(LL)= L/360 (1.18")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.18")
CALCULATED VERT. DEFL.(TL)= L/999 (0.26") G H TS-t MT20 TMWW-t TTWW+m 4.0 8.0 MT20 CHORDS WEBS Edge 2.50 2.25 2.25 TMWW-t MT20 6.0 MAX. FACTORED FACTORED MAX. FACTORED TMV+p BMVW1-t BMWW-t 4.0 8.0 4.0 VERT. LOAD LC1 MAX MAX.
(PLF) CSI (LC) UNBRAC
FROM TO LENGTH MT20 MT20 FORCE (LBS) KMXOP MEMB. CSI: TC=0.57/1.00 (F-H:1), BC=0.48/1.00 (Q-S:1), (LBS) CSI (LC) FR-TO MT20 LENGTH FR-TO WB=0.91/1.00 (E-S:1) , SSI=0.26/1.00 (H-I:1) A- B B- C C- E E- F -ROM 10 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 6.0 6.0 10.0 0.15 (1) 0.27 (1) 0.39 (1) C T D S E Q F H BMWW+t MT20 0 / 47 10.00 0/67 0.02 (4) 0/30 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 10.00 0/121 0/1215 0.04 (4) 0.27 (1) BMWWW-t Q R S T -2553 / 0 3.97 COMP=1.10 SHEAR=1.10 TENS= 1.10 3.0 4.0 4.0 6.0 -924 / 0 0 / 364 0.91 (1) 0.08 (1) BS-t MT20 -2713 / 0 0.55 (1) 3.65 BMWW+t MT20 MT20 0.57 (1) 0.57 (1) COMPANION LIVE LOAD FACTOR = 1.00 -2919 / 0 -505/0 3.51 0.50 (1) G- H H- I I- J J- K -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 BMVW1-t MT20 5.0 8.0 -2919 / 0 0.57 (1) 3.51 0/364 0.08 (1) AUTOSOLVE HEELS OFF -2713 / 0 -2553 / 0 0.55 (1) 3.65 3.97 O- H O- I -924/0 0/1215 0.91 (1) 0.27 (1) Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. 0 / 30 -112.4 -112.4 0.27 (1) 10.00 N- I 0/121 0.04 (4) K- L U- B M- K N- J 14- I -112.4 -112.4 0.0 0.0 0.15 (1) 0.02 (1) 10.00 7.81 0.02 (4) 0.47 (1) 0/67 0.47 (1 7.81 -326 / 0 0.0 0.0 0.02(1) -2883 / 0 NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN U- T T- S S- R 0 / 1984 -18.5 -18.5 10.00 PROFESSIONAL ENGINE W 01/08/27 H. d. G./ALVES / FO -18.5 -18.5 -18.5 0.46 (1) 0 / 2016 10.00 0 / 2713 -18.5 0.48 (1 10.00 MT20 650 371 1747 788 1987 1873 R- Q Q- P P- O O- N N- M 0 / 2713 -18.5 -18.5 -18.5 -18.5 -18.5 0.48 (1) 10.00 10.00 PLATE PLACEMENT TOL. = 0.250 inches 0/2713-18.5 0.48 (1) 10.00 0 / 2016 0 / 1984 -18.5 -18.5 -18.5 0.46 (1) 10.00 PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.80 (J) (INPUT = 0.90) JSI METAL= 0.82 (R) (INPUT = 1.00) 100009024 POLINGE OF ONT ARIO Structural component only DWG# T-2204191

JOB DESC. **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. 417704 T13 HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:32 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-BgsJs0xzhUlrDR7WfigZDEq2l0jauXAJr5KXudznTsH 9-10-8 , 1-3-8 9-10-8 15-9-0 Scale = 1:64. 5x8 = 4x4 =2x4 || 5x8 ≥ ם Ε F G 9.00 12 4x4 🖊 4x4 📎 Н C 5x8 == 5x8 = В W2 W2 R2 ğ Q Ν s 0 R М L 3x6 || 3x6 3x6 || 5x6 == 4x4 = 5x8 == 4x4 =5x6 = 4x6 || 3x6 =35-6-0 1-3-8 35-6-0 1-3-8 TOTAL WEIGHT = 175 lb LUMBER N. L. G. A. CHORDS DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED DESIGN CRITERIA SIZE LUMBER 2x4 2x4 2x4 2x6 No.2 No.2 No.2 No.2 SPF SPF SPF SPF D G J B DRY A D G T MAXIMUM FACTORED INPUT REORD SPECIFIED LOADS: DRY DRY DRY GROSS REACTION VERT HORZ GROSS REACTION BRG DOWN HORZ UPLIFT IN-SX BRG IN-SX CH. 32.5 LL = 6.0 0.0 7.4 PSF T K 2479 2479 0 0 5-8 5-8 BOT CH. LL PSF 2x6 2x4 2x4 DRY No.2 SPF SPF SPF SPF ñ ō 5-8 PSF PSF à DRY DRY No.2 No.2 TOTAL LOAD 45.9 Q -N -UNFACTORED REACTIONS
1ST LCASE MAX
JT COMBINED SNOW 2x4 DRY No.2 SPACING = 24.0 IN. C/C MIN. COMPONENT REACTIONS ALL WEBS 2x3 DRY No.2 SPF WIND DEAD SOIL UVE 0/0 PERM.LIVE EXCEPT 1735 1242 / 0 0/0 492 / 0 492 / 0 0/0 LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 DRY No 2 SPF 1242 / 0 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K DRY: SEASONED LUMBER. TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) PLATES (table is in inches) ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LEN Y PLATES Х 8.0 4.0 8.0 Edge 2.00 1.50 TMVW-n MT20 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, E-O, F-O. TPIC 2014 CDE TMWW-t TTWW-m (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD MT20 5.0 Edge 3.50 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN TMWW-t MT20 4.0 2.0 5.0 4.0 5.0 3.0 4.0 4.0 8.0 4.0 8.0 6.0 6.0 4.0 THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW FGH MT20 MT20 MT20 MT20 TMW+w TTWW-m LOADING TOTAL LOAD CASES: (4) Edge 3.50 2.00 1.50 ALLOWABLE DEFL.(LL)= L/360 (1.18")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.18")
CALCULATED VERT. DEFL.(TL)= L/999 (0.21") TMWW-t MT20 MT20 TMVW-p Edge BMV1+p BMWW-t 5.0 2.50 2.00 MT20 4.0 3.0 5.0 BMWW-t MT20 MT20 MT20 BS-t BMWWW-t CSI: TC=0.47/1.00 (B-C:1), BC=0.43/1.00 (O-P:1), 8.0 WB=0.35/1.00 (B-S:1), SSI=0.28/1.00 (D-E:1) 4.0 3.0 4.0 5.0 BMWW+t MT20 6.0 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 BMWW-t 6.0 S MT20 2.50 2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

BMV1+p



Structural component only DWG# T-2204192

CH	ORDS					WE	BS	
	(. FACTORED	FACTO	RED				MAX. FACTO	ORFO
MEMB.	FORCE	VERT. LO		MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)			CSI (LC)			(LBS)	
FR-TO	, ,	FROM			LENGTH		(<i>-</i>)	()
A-B	0 / 47			0.15 (1)		S-C	-399 / 0	0.17(1)
B- C	-2577 / 0			0.47 (1)		C-R	-239 / 0	0.22(1)
C- D	-2450 / 0	-112.4	-112.4	0.45 (1)	3.98	R- D	0/265	0.06 (1)
D- E	-2377 / 0	-112.4	-112.4	0.43 (1)	4.03	D- P	0/860	0.19 (1)
E-F	-2375 / 0	-112.4	-112.4	0.40 (1)	4.08	P-E	-635 / 0	0.33 (1)
F-G	-2375 / 0	-112.4	-112.4	0.43 (1)	4.04	E- O	-4/0	0.00(1)
G- H	-2451 / 0			0.45 (1)		0- F	-634 / 0	0.33 (1)
H- I	-2577 / 0			0.47 (1)		O- G	0/855	0.19(1)
l- J	0 / 47			0.15 (1)		M-G	0/268	0.06 (1)
T-B	-2438 / 0	0.0		0.17 (1)		M- H	-238 / 0	0.22(1)
K-I	-2438 / 0	0.0	0.0	0.16 (1)	6.61	L- H	-399 / 0	0.17 (1)
						B-S	0 / 2150	0.35 (1)
T-S	0/0	-18.5		0.10 (4)		L-I	0/2150	0.35 (1)
S-R	0 / 2093	-18.5		0.39 (1)				
R-Q	0 / 1927	-18.5	-18.5	0.36 (1)				
Q-P	0 / 1927	-18.5	-18.5	0.36 (1)				
P-O	0 / 2377	-18.5						
0- N	0 / 1928	-18.5	-18.5	0.37 (1)				
N- M	0 / 1928	-18.5	-18.5	0.37 (1)				
M- L	0 / 2093	-18.5	-18.5	0.39 (1)				
L-K	0/0	-18.5	-18.5	0.10 (4)	10.00			

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (S) (INPUT = 0.90) JSI METAL= 0.57 (N) (INPUT = 1.00)

QUANTITY PLY JOB DESC. **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. 417704 5 1 HOMES

Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:33 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-fsQh3MybSotirbijCQBolRM9LQ25dz?T3l34Q3znTsG

1-3-8 11-10-8 11-9-0 11-10-8 Scale = 1:66. 5x8 = 2x4 || 5x8 < 9.00 12 G 3x6 // 3x6 > Н 5x8 = W2 W2 ۵ n s P L 3x6 =3x6 = 3x6 || 5x6 =4x4 =4x10 =5x6 = 3x6 || 35-6-0

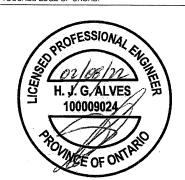
			T
ULES			1
SIZE		LUMBER	DESCR.
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x6	DRY	No.2	SPF
2x6	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
			i
2x3			SPF
2x3		No.2	SPF
2x3	DRY	No.2	SPF
2x3	DRY	No.2	SPF
	SIZE 2x4 2x4 2x4 2x4 2x6 2x6 2x6 2x6 2x4 2x4 2x4 2x4 2x4	SIZE 2x4 DRY 2x4 DRY 2x4 DRY 2x4 DRY 2x4 DRY 2x6 DRY 2x6 DRY 2x4 DRY 2x3 DRY 2x3 DRY 2x3 DRY 2x3 DRY	SIZE

1-3-8

DRY: SEASONED LUMBER.

PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	Χ				
В	TMVW-p	MT20	5.0	8.0	Edge					
	TMWW-t	MT20	4.0	4.0	2.00					
	TS-t	MT20	3.0	6.0						
	TTWW-m	MT20	5.0	8.0	1.75	3.25				
	TMW+w	MT20	2.0	4.0						
	TTWW-m	MT20	5.0	8.0	1.75	3.25				
Н	TS-t	MT20	3.0	6.0						
1	TMWW-t	MT20	4.0	4.0	2.00	1.50				
J	TMVW-p	MT20	5.0	8.0	Edge					
L	BMV1+p	MT20	3.0	6.0						
M	BMWW-t	MT20	5.0	6.0	2.50	2.25				
N	BMWW-t	MT20	4.0	4.0						
0	BS-t	MT20	3.0	6.0						
Р	BMWWW-t	MT20	4.0	10.0						
Q	BS-t	MT20	3.0	6.0						
R	BMWW-t	MT20	4.0	4.0						
S	BMWW-t	MT20	5.0	6.0	2.50	2.25				
T	BMV1+p	MT20	3.0	6.0						

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



Structural component only DWG# T-2204193

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
DEADINGS	

35-6-0

BEA	RINGS						
-	FACTORED		MAXIMU	M FACTO	INPUT	REORD	
	GROSS F	REACTION	GROSS	REACTIC	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Τ	2479	0	2479	0	0	5-8	5-8
L	2479	0	2479	0	0 -	5-8	5-8

	1ST LCASE	MAX./	MIN. COMPON	ENT REACTION	NS.					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND					
T	1735	1242 / 0	0/0	0/0	0/0					
L	1735	1242 / 0	0/0	0/0	0/0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

DEAD 492 / 0

492 / 0

0/0

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

СН	ORDS				WE	BS	
	. FACTORED	FACTORED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LOAD LO	1 MAX	MAX.	MEMB		MAX
	(LBS)	(PLF)				(LBS)	CSI (LC)
FR-TO	(250)	FROM TO		LENGTH		(LDC)	001 (20)
A- B	0 / 47	-112.4 -112.4				-300 / 20	0.17 (1)
B- C	-2614 / 0	-112.4 -112.4			C-R	-435/0	0.20 (1)
C- D	-2322 / 0	-112.4 -112.4			R-E	0 / 409	0.20 (1)
	-2322 / 0					0 / 527	0.07 (1)
E-F	-2086 / 0	-112.4 -112.4			P-F	-808/0	0.44 (1)
F- G	-2086 / 0	-112.4 -112.4			P- G		0.08 (1)
G-H	-2322 / 0	-112.4 -112.4				0 / 409	0.07 (1)
H- I	-2322 / 0	-112.4 -112.4			N- I	-435 / 0	0.20 (1)
i- J	-2614 / 0	-112.4 -112.4			M- I	-300/20	0.17 (1)
J- K	0 / 47	-112.4 -112.4			B-S	0/2171	0.35 (1)
	-2432 / 0	0.0 0.0			M-J	0/2171	0.35 (1)
L- J	-2432 / 0	0.0 0.0			141- 0	0/21/1	0.33 (1)
	L10L1 0	0.0 0.0	0.10(1)	0.02			
T-S	0/0	-18.5 -18.5	0.16 (4)	10.00			
S-R	0 / 2129	-18.5 -18.5					
R-Q	0 / 1821	-18.5 -18.5					
Q-P	0 / 1821		0.36 (1)				
P- 0	0 / 1821	-18.5 -18.5					
O- N	0 / 1821	-18.5 -18.5					
N- M	0 / 2129	-18.5 -18.5					
M- L	0/0		5 0.16 (4)				
141 L	0/0	-10.5 -10.6	0.10 (4)	10.00			

NOTES-(1) 1) Lateral braces to be a minimum of 2X4 SPF #2.

DESIGN CRITERIA

SPECIFIED LOADS:										
TOP	CH.	LL	=	32.5	PSF					
		DL	=	6.0	PSF					
BOT	CH.	LL	=	0.0	PSF					
		DL		7.4	PSF					
TOTA	L LO	AD	=	45.9	PSF					

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE

1-3-8 TOTAL WEIGHT = 5 X 190 = 951 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.18")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.18")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.70/1.00 (I-J:1) , BC=0.42/1.00 (M-N:1) , WB=0.44/1.00 (F-P:1) , SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

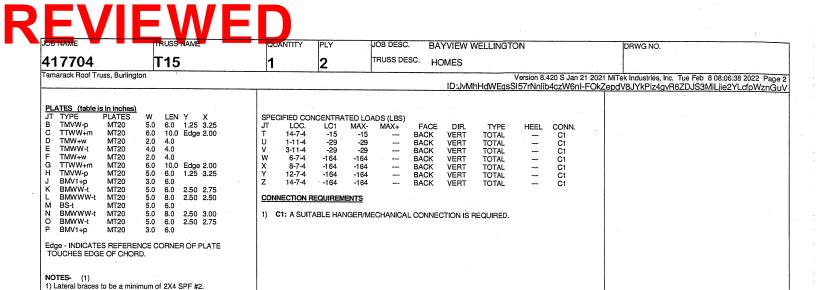
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

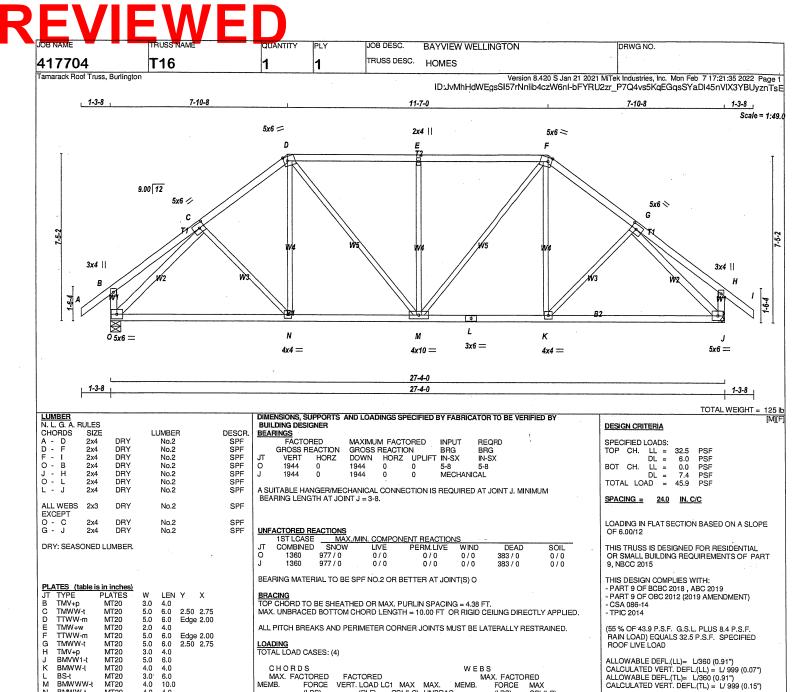
JSI GRIP= 0.79 (M) (INPUT = 0.90) JSI METAL= 0.56 (Q) (INPUT = 1.00)

JOB DESC. **BAYVIEW WELLINGTON** DRWG NO. 417704 T15 2 TRUSS DESC. HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:06:38 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-FOkZepdV8JYkPiz4gvR6ZDJS3MiLjie2YLcfpWznGuV 5-10-8 1-3-8 8-12 14-10-4 5-10-8 Scale: 1/4"=1 6x10 // 2x4 || 6x10 \\ 4x4 = F G 9.00 12 5x6 5x6 = В 1-6-4 W2 W2 \aleph U V χ М Z ٥ N L 3x6 || 5x6 == 3x6 || 5x6 5x8 == 5x8 = 5x6 = 1-11-4 2-0-0 2-0-0 10-7-4 10-9-8 27-4-0 1-3-8 TOTAL WEIGHT = 2 X 142 = 283 lb [M] LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED SIZE MAXIMUM FACTORED GROSS REACTION DOWN HORZ UPLII - C No.2 No.2 DRY SPF INPUT REQRD SPECIFIED LOADS: 2x4 2x4 DRY SPF GROSS REACTION VERT HORZ LL = DL = LL = DL = AD = 32.5 PSF BRG CH. No.2 UPLIFT IN-SX 6.0 IN-SX PSF - B 2x6 DRY No.2 SPF 3879 0 3879 5-8 5-8 BOT CH. 2x6 2x6 DRY DRY SPF SPF SPF No.2 TOTAL LOAD 45.9 PSF M - J 2x6 DRY No.2 A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0. SPACING = 24.0 IN. C/C SPF ALL WEBS 2x3 DRY No.2 EXCEPT 0 DRY No 2 SPE LOADING IN FLAT SECTION BASED ON A SLOPE UNFACTORED REACTIONS
1ST LCASE MA OF 6.00/12 MAX./MIN. COMPONENT REACTIONS SOIL 0/0 0/0 SNOW DRY: SEASONED LUMBER. COMBINED WIND DEAD THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 778 / 0 679 / 0 DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS 2440 1761 / 0 0/0 0/0 0/0 9. NBCC 2015 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 CHORDS #ROWS SURFACE BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. LOAD(PLF) - PART 9 OF OBC 2012 (2019 AMENDMENT) SPACING (IN) TOP CHORDS: (0.122"X3") SPIRAL NAILS TPIC 2014 A- C C- G G- I P- B SIDE(61.0) SIDE(61.0) 12 12 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED TOP TOP LOADING TOTAL LOAD CASES: (4) 12 ROOF LIVE LOAD J- H TOP ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18") BOTTOM CHORDS: (0.122"X3") SPIRAL NAILS P-M SIDE(183.1) CHORDS WEBS 12 SIDE(183.1) MAX. FACTORED FACTORED MAX. FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC) FROM TO FORCE WEBS: (0.122"X3") SPIRAL NAILS MEMB. MAX FORCE 2x3 F- L (LBS) CSI (LC) UNBRAC CSI (LC) (LBS) CSI: TC=0.61/1.00 (B-C:1), BC=0.47/1.00 (L-N:1), SIDE(205.6) FR-TO LENGTH FR-TO -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 0.08 (1) 0.61 (1) 10.00 0/149 0/2198 2x4 A-B B-C-Q-R D-S-E-T F-G 0 / 47 0.03 (4) WB=0.38/1.00 (G-L:1) , SSI=0.15/1.00 (C-D:1) 0.27 (1) NAILS TO BE DRIVEN FROM ONE SIDE ONLY. DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 -5001 / 0 0.33 (1) 4.04 N- D L- F -594 / 0 0.15 (1 -5001 / 0 0.33 (1) 4.04 -537 / 0 0 14 (1 COMP=1.00 SHEAR=1.00 TENS= 1.00 4.04 4.06 4.06 L-G K-G B-O -112.4 -112.4 -112.4 -112.4 0.33 (1) 0.27 (1) GIRDER NAILING ASSUMES NAILED HANGERS ARE -5001 / 0 0/3042 FASTENED WITH MIN. 3-0 INCH NAILS. -5001 / 0 486/0 COMPANION LIVE LOAD FACTOR = 1.00 0.13(1)-5001 / 0 -112.4 -112.4 0.27(1) 0/3602 0.32 (1 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 4.03 4.03 4.00 0.25 (1) 0.25 (1) K- H N- E 0 / 3203 -287 / 0 TOP - COMPONENTS ARE LOADED FROM THE TOP AND -5160 / C AUTOSOLVE HEELS OFF MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. 0.10 (1) -5160 / 0 0.32 (1) E-L 0/58 TRUSS PLATE MANUFACTURER IS NOT G-H H-I P-B -112.4 -112.4 -112.4 -112.4 RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. -3942 / 0 0.56 (1) 4 25 0 / 47 0.0 0.0 0.14 (1) J- H -3441 / 0 0.0 0.0 0.12 (1) NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION P- U U- V -18.5 -18.5 0.07 (4) 10.00 PROFESSIONAL ANGELOW PROFESSIO (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 0/0 -18.5 -18.5 0.07 (4) 10.00 V- O O- W -18.5 -18.5 -18.5 -18.5 0.07 (4) 0.33 (1) 0/0 10.00 1987 1873 0- W W- X -18.5 -18.5 -18.5 -18.5 0 / 3541 0.33(1) 10.00 PLATE PLACEMENT TOL. = 0.250 inches -18.5 0.33 (1) -18.5 0.47 (1) -18.5 0.47 (1) X- N N- Y 0 / 3541 10.00 0 / 5133 0 / 5133 0 / 5133 PLATE ROTATION TOL. = 5.0 Deg. N- Y Y- M M- Z Z- L L- K K- J -18.5 10.00 JSI GRIP= 0.77 (H) (INPUT = 0.90) JSI METAL= 0.60 (M) (INPUT = 1.00) -18.5-18.5 0.47 (1) 10.00 0 / 5133 0 / 3140 -18.5 0.47 (1) 100009024 -18.5-18.5 0.23 (1) 10.00 0/0 -18.5 -18.5 0.04 (4) 10.00 SPECIFIED CONCENTE TED LOADS (LBS) POLYNOE OF ONT ARIO JT C D LOC LC₁ MAX-MAX+ FACE DIR. TYPE HEEL CONN BACK BACK BACK 5-10-8 10-7-4 -269 -15 -269 -15 VERT VERT TOTAL C1 C1 C1 C1 C1 C1 C1 C1 C1 -1040 16-6-8 10-7-4 -1040 VERT TOTAL -164 -164 -164 -164 TOTAL ZOGES __ BACK VERT BACK VERT 6-7-4 8-7-4 12-7-4 -25 -25 VERT TOTAL Structural component only -15 -15 -15 -15 BACK VERT TOTAL DWG# T-2204194 //L CONTINUED ON PAGE 2





Structural component only DWG# T-2204194 M



6.0 4.0 6.0 6.0 4.0 6.0 4.0 6.0 10.0 BMWW-t BS-t BMWWW-t 4.0 3.0 4.0 MT20 BMWW-t MT20 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only DWG# T-2204195

(LC)
(LC)
` '
5 (1)
5 (4)
3 (1)
9 (1)
3 (1)
5 (4)
5 (1)
2 (1)
2 (1)
500000000000000000000000000000000000000

CSI: TC=0.53/1.00 (D-E:1) , BC=0.38/1.00 (J-K:1) , WB=0.82/1.00 (G-J:1), SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90) JSI METAL= 0.48 (G) (INPUT = 1.00)

JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. HOMES T17

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTTek Industries, Inc. Mon Feb 7 17:21:36 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-3R6qiO_TkjFHi3RluYIVN4_l8d8yqPEvmjlk1OznTsD

Scale = 1:34

4x4 = В 9.00 12 4x4 || 4x4 || С 1-6-4 1-6-4 W Ε D 3x4 || 4x10 = 3x4 || 10-8-0 10-8-0

LUMBER N. L. G. A. RULES CHORDS SIZE SIZE LUMBER DESCR SPF SPF SPF SPF SPF No.2 No.2 No.2 No.2 2x4 2x4 DRY DRY 2x4 A 244 DRY No.2 No.2 ALL WEBS 2x3 DRY No.2 SPF

DBY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES LEN Y TMVW+p MT20 MT20 MT20 4.0 4.0 4.0 4.0 4.0 4.0 1.00 2.00 2.25 2.00 TTW-p TMVW+p 1.00 2.00 BMV1+p BMWWW-t MT20 3.0 4.0 4.0 10.0 BMV1+p

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEA	HINGS						
	FACTO	RED	MAXIMU	M FACTO	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	698	0	698	0	0	5-8	5-8
D	698	0	698	0	0	MECHAN	IICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	IENT REACTION	4S		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	489	346 / 0	0/0	0/0	0/0	143 / 0	0/0
D	489	346 / 0	0/0	0/0	0/0	143 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

 $\frac{\textbf{BRACING}}{\textbf{TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING} = 6.25 \, \text{FT.} \\ \textbf{MAX. UNBRACED BOTTOM CHORD LENGTH} = 10.00 \, \text{FT. OR RIGID CEILING DIRECTLY APPLIED.} \\ \end{cases}$

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHO	ORDS		WEBS								
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED			
MEMB.	FORCE	VERT. LO	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX			
	(LBS)	(PL	.F) (CSI (LC)	UNBRAC)	(LBS)	CSI (LC)			
FR-TO		FROM	TO .		LENGTH	FR-TO	, ,	` '			
A-B	-463 / 0	-112.4	-112.4	0.41 (1)	6.25	E-B	-44 / 85	0.03 (4)			
B- C	-463 / 0	-112.4	-112.4	0.41 (1)	6.25	A-E	0/379	0.09 (1)			
F- A	-660 / 0	0.0	0.0	0.07(1)	7.81	E-C	0/379	0.09 (1)			
D- C	-660 / 0	0.0	0.0	0.07(1)	7.81						
F-E	0/0	-18.5	-18.5	0.15 (4)	10.00						
E- D	0/0	-18.5	-18.5	0.15 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS: LL = DL = LL = TOP CH. 32.5 PSF 6.0 PSF PSF BOT CH. DL = AD = TOTAL LOAD

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 4 X 43 = 170 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.41/1.00 (A-B:1) , BC=0.15/1.00 (D-E:4) , WB=0.09/1.00 (C-E:1) , SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (C) (INPUT = 0.90) JSI METAL= 0.16 (C) (INPUT = 1.00)



Structural component only DWG# T-2204196

JOB DESC. BAYVIEW WELLINGTON DRWG NO. TRUSS DESC. 417704 G17 HOMES

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:05 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-TUk6Boc12CLhUIZKZXhEdYCW17bo3NhLdOHbQhznTsi

1-3-8 5-4-0 1-3-8 4x4 = Scale = 1:34.1 Ε 2x4 || 2x4 || 9.00 12 D 2x4 || 2x4 || G 4x4 || P o N М L κ 3x4 || 4x4 =2x4 || 2x4 || 2x4 || 4x4 = 3x4 || 10-8-0

LUMBER | LUMBER | N. L. G. A. RULES | CHORDS | SIZE | P - B | 2x4 | A - E | 2x4 | J - H | 2x4 | P - J | 2x4 LUMBER DRY SPF SPF SPF SPF No.2 No.2 DRY DRY No.2 No.2 DRY No.2 ALL WEBS 2x3 ALL GABLE WEBS No.2 SPF DRY No.2 SPF DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

١.	PLA	ATES (table	is in inches)				
П	JT	TYPE	PLATES	W	LEN	Υ	X
١	В	TMVW+p	MT20	4.0	4.0	1.00	2.00
	C, [D, F, G					
	С	TMW+w	MT20	2.0	4.0		
	Е	TTW-p	MT20	4.0	4.0	2.25	2.00
		TMVW+p	MT20	4.0	4.0	1.00	2.00
	J	BMV1+p	MT20	3.0	4.0		
	K	BMWW1-t	MT20	4.0	4.0		
		л, N					
	L	BMW1+w	MT20	2.0	4.0		
	0	BMWW1-t	MT20	4.0	4.0		
1	Ρ	BMV1+p	MT20	3.0	4.0		
1							

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
BEARINGS	

10-8-0

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

 $\frac{\textbf{BRACING}}{\textbf{TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING} = 6.25 \, \text{FT.} \\ \textbf{MAX. UNBRACED BOTTOM CHORD LENGTH} = 10.00 \, \text{FT. OR RIGID CEILING DIRECTLY APPLIED.} \\$

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

		R D S FACTORED	FACTORED			WE	BS MAX. FACTO	RED	
	MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX	
		(LBS)		CSI (LC			(LBS)	CSI (LC)	
	FR-TO		FROM TO						
i	P-B	-328 / 0		0.03 (1		M-E	-175/0	0.08 (1)	
ı	A-B	0 / 47	-112.4 -112			N- D	-273 / 0	0.07(1)	
I	B- C	-66 / 0	-112.4 -112				-80 / 0	0.01(1)	
	C-D	-2/0 .	-112.4 -112			L-F	-273 / 0	0.07(1)	
	D- E	-22 / 0	-112.4 -112				-80 / 0	0.01(1)	
ı	E-F	-22 / 0	-112.4 -112			B-O		0.00(1)	
	F-G	-2 / 0	-112.4 -112			K- H	0/21	0.00(1)	
	G-H	-66 / 0	-112.4 -112						
	H-I	0 / 47	-112.4 -112						
	J- H	-328 / 0	0.0	0.03 (7.81				
	P- 0	0/0		3.5 0.01 (4					
	0- N	0 / 12		3.5 0.02 (4					
	N- M	0/5		3.5 0.02 (4					
	M- L	0/5	-18.5 -18						
	L-K	0 / 12		3.5 0.02 (4					
	K-J	0/0	-18.5 -18	3.5 0.01 (4	10.00				

DESIGN CRITERIA

1-3-8

SPECIFIED LOADS: LL = 32.5 PSF DL = 6.0 PSF LL = 0.0 PSF DL = 7.4 PSF AD = 45.9 PSF TOP CH. BOT CH. TOTAL LOAD

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 49 L

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (H-I:1) , BC=0.02/1.00 (K-L:4) , WB=0.08/1.00 (E-M:1) , SSI=0.10/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (H) (INPUT = 0.90) JSI METAL= 0.14 (F) (INPUT = 1.00)



Structural component only DWG# T-2204171

REVIEWED

JOB NAME

TRUSS NAME

QUANTITY

PLY

2

JOB DESC. TRUSS DESC. BAYVIEW WELLINGTON

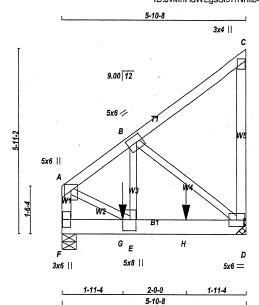
HOMES

DRWG NO.

Scale = 1:35.3

417704
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Mon Feb 7 17:21:37 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-XefCvk?5V1N8JD0URFGkwHX_w1RXZmo2_N1IZrznTsC



LUMBER N. L. G. A. RULES CHORDS SIZE SIZE LUMBER DESCR DRY DRY SPF No.2 No.2 2x4 DRY No.2 SPF - D 2100F 1.8E SPF ALL WEBS 2x3 DRY SPF No.2 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.1	SPACING (IN) 22"X3") SPIRAL NAILS	
F-A 1	12	TOP
A- C 1	12	TOP
C- D 1	12	TOP
BOTTOM CHORDS:	(0.122"X3") SPIRAL NAILS	
F-D 2	12	SIDE(197.8
WEBS: (0.122"X3")	SPIRAL NAILS	
B-F 1	2	SIDE/435 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

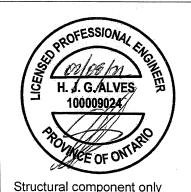
2x3

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PL/	ATES (table	is in inches)				
JT	TYPE	PLATES	W	LEN	Υ	Х
Α	TMVW+p	MT20	5.0	6.0	Edge	
В	TMWW-t	MT20	5.0	6.0	2.50	2.50
С	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		



DWG# T-2204197 [//]

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEA	RINGS						
	FACTO	RED	MAXIMU	M FACTO	ORED	INPUT	REQRE
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
Т	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	2762	0	2762	0	0	5-8	5-8
)	2708	0	2708	0	0	MECHANIC	CAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

	UNF	ACTORED RE						•
į		1ST LCASE	MAX.	MIN. COMPOR	VENT REACTION	NS		
i	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	F	1932	1388 / 0	0/0	0/0	0/0	544 / 0	0/0
	D	1895	1357 / 0	0/0	0/0	0/0	538 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	ORDS	FACTOR	RED			WE	BS MAX. FACT	ORED	
MEMB.	FORCE	VERT. LO	AD LC1	MAX	MAX.	MEMB			
	(LBS)	(PL		CSI (LC)			(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH				
F- A	-2737 / 0	0.0		0.15 (1)		A-E	0 / 2387	0.30	(1)
A-B	-2653 / 0	-112.4	-112.4	0.10(1)	5.49	E-B	0 / 2876	0.36	(1)
	-18 / 0	-112.4	-112.4	0.10(1)	6.25	B- D	-2720 / 0	0.45	(1)
D- C	-173 / 0	0.0	0.0	0.05(1)	7.81				
F- G	0/0	-18.5		0.16(1)					
G-E	0/0	· -18.5		0.16(1)					
E-H	0 / 2145			0.38 (1)					
H- D	0 / 2145	-18.5	-18.5	0.38 (1)	10.00				
	TED CONCENT		ADS (LE	3S)					
JT	LOC. LC		MAX:	+ F.	ACE I	DIR.	TYPE	HEEL	CONN.
	1-11-4 -169		-	FR	ONT VI	ERT	TOTAL		C1
Н	3-11-4 -159	1 -1591	-	- FR	ONT VI	ERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPEC	IFIED	LOAD	S:		
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL		7.4	PSF
TOTA	L LO	AD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 2 X 33 = 66 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.15/1.00 (A-F:1) , BC=0.38/1.00 (D-E:1) , WB=0.45/1.00 (B-D:1) , SSI=0.81/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90) JSI METAL= 0.48 (A) (INPUT = 1.00)

CONTINUED ON PAGE 2

117704	TRUSS NAME T18	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	BAYVIEW WELLING HOMES			DRWG NO.		
amarack Roof Truss, Burlin					ID:JvMhHdWEgsS	Version 8.420 S Jan 21 3157rNnlib4czW6nl-XefC	2021 MIT Vk?5V1	ek Industries, Inc. M N8JD0URFGkwH	on Feb 7 17:21:3 X w1RXZmo2	7 2022 Page N1IZrznT
PLATES (table is in inche	s) W LENY X									
E BMWW+t MT20 F BMV1+p MT20 Edge - INDICATES REFER	5.0 8.0 4.25 2.25 3.0 6.0 RENCE CORNER OF PLATE									
	RENCE CORNER OF PLATE ORD.									
NOTES- (1) 1) Lateral braces to be a m	ninimum of 2X4 SPF #2.									
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		-								
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H. J.	G. ALVES									
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Polyc	F OF ONTARIO						-			
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417704

T18Z1

JOB DESC.

TRUSS DESC.

BAYVIEW WELLINGTON

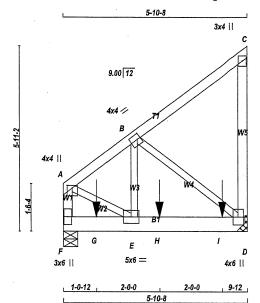
HOMES

DRWG NO.

Scale = 1:35.2

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:38 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-?qDa740kGKV?xMag?znzSV4AYRqxIH7CD1nr5HznTsB



LUMBER N. L. G. A. RULES CHORDS F - A LUMBER DRY 2x4 2x4 No.2 No.2 SPF ACCD DRY DRY SPF 2x4 2100F 1.8E ALL WEBS 2x3 DRY DRY: SEASONED LUMBER. No.2 SPF

DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.1	22"X3") SPIRAL NAILS	
F- A 1	12	TOP
A- C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS	: (0.122"X3") SPIRAL NAILS	
F- D 2	12	SIDE(0.0)
WEBS: (0.122"X3")	SPIRAL NAILS	
00		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

	JT	TYPE	PLATES	W	LEN	Υ	Χ	
	Α	TMVW+p	MT20	4.0	4.0	1.00	2.00	
	В	TMWW-t	MT20	4.0	4.0	2.00	1.25	
	С	TMV+p	MT20	3.0	4.0			
ı	D	BMVW1+p	MT20	4.0	6.0			
	Ε	BMWW-t	MT20	5.0	6.0			



Structural component only DWG# T-2204198 ///

DIMENSIONS, SUPI	PORTS AND LOA	DINGS SPECIFIED B	Y FABRICATOR TO	BE VERIFIED BY
BUILDING DESIGN	ER			

BEARINGS	
	EQRD
GROSS REACTION GROSS REACTION BRG B	RG
JT VERT HORZ DOWN HORZ UPLIFT IN-SX II	V-SX
F 1897 0 1897 0 0 5-8 5	-8
D 2033 0 2033 0 0 MECHANICA	L.

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	NS.		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1323	972 / 0	0/0	0/0	0/0	351 / 0	0/0
D	1418	1042 / 0	0/0	0/0	0/0	376 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

CHORD TO BE SHEATHED OR MAX. PUBLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING TOTAL LOAD CASES: (4)

		(- ,									
	ORDS C. FACTOR	ED EA	СТОЕ	RED				W E	EBS MAX. FAC	IORED	
MEMB.					MAX	: 1	MAX.	МЕМВ			
	(LBS						UNBRAC				
FR-TO	,						LENGTH				,
									0 / 1339		(1)
A- B	-1472 / 0	-1	12.4	-112.4	0.10 (1)	6.25	E-B	0 / 1424 -1526 / 0	0.18	
B- C	-20 / 0	-1	12.4	-112.4	0.10 (1)	6.25	B- D	-1526/0	0.25	(1)
D- C	-169 / 0		0.0	0.0	0.05 (1)	7.81				
F- G	0/0	_	18.5	-185	0 11 /	11)	10.00				
	0/0										
	0 / 12										
	0 / 12										
I- D	0 / 12	03 -	18.5	-18.5	0.18	(1)	10.00				
SPECII	FIED CONC	ENTRATE	חוח	ADS (I F	121						
JT	LOC.			MAX-		FΔ	CE E	DIR.	TYPE	HEEL	CONN.
	1-0-12							RT	TOTAL		C1
Н	3-0-12					RC	ONT VE	RT	TOTAL		C1
	5-0-12					FRC	ONT VE	RT	TOTAL		C1

CONNECTION REQUIREMENTS

C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTA	L LO	AD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 2 X 33 = 66 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) CSA 086-14

- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.18/1.00 (D-E:1) , WB=0.25/1.00 (B-D:1) , SSI=0.41/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90) JSI METAL= 0.27 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

OB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON		DRWG NO.	
117704 amarack Roof Truss, Burl	T18Z1	1	2	TRUSS DESC.	HOMES Version 8.420 S JID:JvMhHdWEgsSI57rNnlib4czW6r	lan 21 2021 MiT	ek Industries, Inc. Mon Fe	b 7 17:21:38 2022 P
					ID JvMhHdWEgsSl57rNnlib4czW6r	il-?qDa740kG	iKV?xMag?znzSV4AY	<u>'RaxIH7CD1nr5Hz</u>
LATES (table is in inch T TYPE PLATE BMV1+p MT20	es) S W LEN Y X 3.0 6.0							
OTES- (1)	minimum of 2X4 SPF #2.							
) Lateral braces to be a l								
	·							
		/.	•					
		-						
		-		٠				
						-		
							*	
								-
PROF	ESSIONALEN							
\$ O	ESSIONAL ENGLANDERS OF ALVES							
H. J.	G. ALVES							
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Rowy	F OF ONTARIO		•				*** · · · · · · · · · · · · · · · · · ·	
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417704

T18Z2

PLY

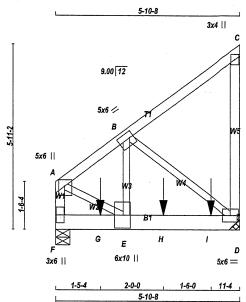
JOB DESC. TRUSS DESC. **BAYVIEW WELLINGTON**

HOMES

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:14:37 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-kYAfAbQyLUumMJ47D0XRUEfPgSA9SlIZhM8W7GznGn0

DRWG NO.



TOTAL WEIGHT = 2 X 33 = 66 lb

Scale = 1:35.2

LUMBER				
N. L. G. A. F	ULES			
CHORDS	SIZE		LUMBER	DESCR.
F-A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRV- SEASO	ングドロコ	IMBER		

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

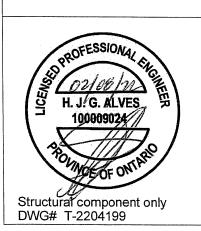
CHORDS	#ROWS	SURFACE	LOAD(PLF)
i			
TOP CHO	RDS: (0.12	2"X3") SPIRAL NA	ILS
F-A	1	12	TOP
A- C	1	12	TOP
C-D	1	12	TOP
BOTTOM	CHORDS:	(0.122"X3") SPIRA	
	2		TOP
	400m/om/		101
MER2:(0	.122"X3") S	PIRAL NAILS	
	TOP CHOI F- A A- C C- D BOTTOM (F- D	F- A 1 A- C 1 C- D 1 BOTTOM CHORDS: F- D 2	SPACING (IN) TOP CHORDS: (0.122"X3") SPIRAL NA F- A 1 12 A- C 1 12 C- D 1 12 BOTTOM CHORDS: (0.122"X3") SPIRA

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

ı	PLATES (table is in inches)								
	JT		PLATES	W	LEN	Υ	Х		
į	Α	TMVW+p	MT20	5.0	6.0	2.50	2.25		
ĺ	В	TMWW-t	MT20	5.0	6.0	2.25	1.50		
	С	TMV+p	MT20	3.0	4.0				
	D	BMVW1-t	MT20	5.0	6.0				
	E	BMWW+t	MT20	6.0	10.0	5.00	2.75		
	F	BMV1+p	MT20	3.0	6.0				

NOTES-1) Lateral braces to be a minimum of 2X4 SPF #2.



DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
	THE TOTAL OF THE PARTY OF THE P
BUILDING DESIGNER	

BEA	RINGS						
	FACTO	MAXIMU	M FACT	ORED	INPUT	REORD	
	GROSS R	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	3657	0	3657	0	0	5-8	5-8
D	4494	0	4494	0	0	MECHA	NICAL

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNF	UNFACTORED REACTIONS									
	1ST LCASE	MAX.//	MIN. COMPO	NENT REACTION	1S					
JT	COMBINED	NONS C	LIVE	PERM.LIVE	MIND	DEAD	SOIL			
F	2557	1843 / 0	0/0	0/0	0/0	714/0	0/0			
D	3142	2266 / 0	0/0	0/0	0/0	876 / 0	0/0			

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHURDS						W E	:BS		
MAX	K. FACTORED	FACTOR	ED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOA	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLI	=) (CSI (LC)	UNBRAC)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO		, ,	
F-A	-3415 / 0	0.0	0.0	0.19(1)	6.32	A-E	0/2993	0.37(1)	
A-B	-3335 / 0	-112.4 -	112.4	0.11(1)	5.02	E-B	0/3715	0.46 (1)	
B-C	-16 / 0	-112.4 -	112.4	0.10(1)	6.25	B- D	-3411/0	0.56 (1)	
D- C	-176 / 0	0.0	0.0	0.05 (1)	7.81			. ,	
F- G	0/0	-18.5	-18.5	0.31 (1)	10.00				
G-E	0/0	-18.5	-18.5	0.31 (1)	10.00				
E- H	0 / 2689	-18.5	-18.5	0.49 (1)	10.00				
H-1	0 / 2689	-18.5	-18.5	0.49 (1)	10.00				
I D	0.7.2680	-195	-10 E	0.40./1)	10.00				

SPEC	CIFIED CON	NCENTRA	TED LOA	DS (LBS)					
JΤ	LOC.	LC1	MAX-	MÀX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-5-4	-1720	-1720	****	TOP	VERT	TOTAL		C1
Н	3-5-4	-1720	-1720		TOP	VERT	TOTAL		C1
i	4-11-4	-1720	-1720		TOP	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:								
TOP	CH.	LL	=	32.5	PS			
		DL		6.0	PS			
BOT	CH.	LL	=	0.0	PS			
		DL		7.4	PS			
TOTA	L LO	AD	=	45.9	PS			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14 -TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.19/1.00 (A-F:1) , BC=0.49/1.00 (D-E:1) , WB=0.56/1.00 (B-D:1) , SSI=0.92/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (A) (INPUT = 0.90) JSI METAL= 0.40 (E) (INPUT = 1.00)

T18Z3

PLY

JOB DESC.

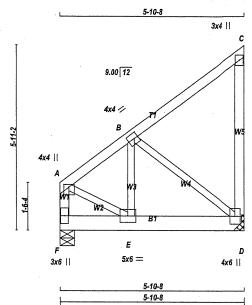
BAYVIEW WELLINGTON

TRUSS DESC. HOMES

417704 Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITHS Industries, Inc. Mon Feb 7 17:21:40 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-yDLKYI1_oyljAgk37OpRXw9W_EX0mDnVgLGyA9znTs9

DRWG NO.



LUMBER N. L. G. A. RULES CHORDS SIZE F - A 2x4 LUMBER SIZE DESCR 2x4 2x4 No.2 No.2 DRY DRY SPF D DRY 2100F 1.8E SPF ALL WEBS 2x3 DRY DRY: SEASONED LUMBER. SPF No.2

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

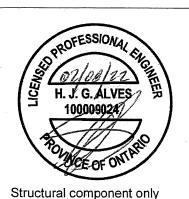
CHORD	S #ROWS	SURFACE	LOAD(PLF)
TOP CH	ORDS : (0.1	SPACING (IN) 22"X3") SPIRAL NAILS	
F-A	1	12	TOP
A- C	1	12	TOP
C-D	1	12	TOP
BOTTO	M CHORDS	: (0.122"X3") SPIRAL NAILS	
F-D	2	12	TOP
WEBS:	(0.122"X3")	SPIRAL NAILS	
202	4	. 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

	PL	PLATES (table is in inches)									
	JT	TYPE	PLATES	W	LEN	Υ	Х				
į	Α	TMVW+p	MT20	4.0	4.0	1.00	2.00				
	В	TMWW-t	MT20	4.0	4.0	2.00	1.50				
	С	TMV+p	MT20	3.0	4.0						
	D	BMVW1+p	MT20	4.0	6.0						
	E	BMWW-t	MT20	5.0	6.0						
	-	D M / 4	NATOO.	0.0	0.0						

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2. NOTES-



DWG# T-2204200

		and the second second second		
DIMENSIONS, SUPPORTS	AND LOADINGS	SPECIFIED BY EARS	ICATOD TO BE VEDICI	ED DV
DUIL DING BEGIONED	rate compared.	O. 2011 12D D 1 1 AD1	MOATOR TO DE VERMI	

BEA	RINGS						
	FACTO	RED	MAXIMUM FACTORED			INPUT	REORE
	GROSS R	EACTION	GROSS REACTION			BRG	BRG
IT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
=	385	0 .	385	0	0	5-8	5-8
)	385	0	385	0	0	MECHANIC	CAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED	

	1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
F	270	191 / 0	0/0	0/0	0/0	79 / 0	0/0			
D	270	191 / 0	0/0	0/0	0/0	79 / 0	0/0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING TOTAL LOAD CASES: (4)

CHO	CHORDS				WEBS				
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LC	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PI	_F) (CSI (LC)	UNBRAC)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
F- A	-360 / 0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 262	0.03(1)	
A-B	-259 / 0	-112.4	-112.4	0.10(1)	6.25	E-B	-67 / 39	0.01 (4)	
B- C	-23 / 0	-112.4	-112.4	0.10(1)	6.25	B- D	-299 / 0	0.05 (1)	
D- C	-165 / 0	0.0	0.0	0.05(1)	7.81				
F-E	0/0	-18.5	-18.5	0.00 (4)	10.00	,			
E-D	0 / 236	-18.5	-18.5	0.01 (1)	10.00				

TOTAL WEIGHT = 2 X 33 = 66 lb

Scale = 1:35.3

DESIGN CRITERIA

SPEC	ルトドロ	LUAI	JO:		
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	==	7.4	PSF
TOT		AD.		450	DOE

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.01/1.00 (D-E:1) , WB=0.05/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (A) (INPUT = 0.90) JSI METAL= 0.05 (D) (INPUT = 1.00)

417704

T18Z4

JOB DESC. **BAYVIEW WELLINGTON**

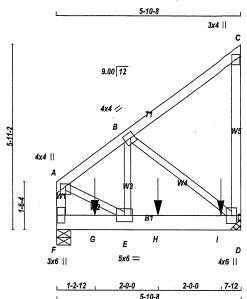
TRUSS DESC. HOMES

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:41 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-QPvjl52cZFuaoqJFg5Lg47ihoesmVe0ev??VicznTs8

DRWG NO.

Scale = 1:35.2



LUMBER N I G A BULES SIZE LUMBER DRY A 2x4 No.2 No.2 SPF 2x4 DRY 2x4 2x6 SPF 2100F 1.8E ALL WERS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.

DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
	SPACING (IN)	
TOP CHORDS: (0.1:	22"X3") SPIRAL NAILS	
F- A 1	12	TOP
A- C 1	12	TOP
C- D 1	12	TOP
BOTTOM CHORDS:	(0.122"X3") SPIRAL NA	JLS
F- D 2	12	SIDE(0.0)
WEBS: (0.122"X3")	SPIRAL NAILS	
2x3 1	6	

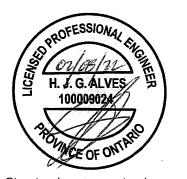
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PL	ATES (table)						
JT	TYPE	PLATES	W	LEN	Υ	Х	
Α	TMVW+p	MT20	4.0	4.0	1.00	2.00	
В	TMWW-t	MT20	4.0	4.0	2.00	1.50	
С	TMV+p	MT20	3.0	4.0			
D	BMVW1+p	MT20	4.0	6.0			
Ε	BMWW-t	MT20	5.0	6.0			



Structural component only DWG# T-2204201

DIMENSIONS, SUPPORTS /	ND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY	7
BUILDING DESIGNER	· · · · · · · · · · · · · · · · · · ·	
BEARINGS		

EΑ	RINGS						
	FACTO	RED	MAXIMU	M FACTO	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIC	N	BRG	BRG
Γ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	1303	0	1303	0	0	5-8	5-8
	1508	0	1508	0	0	MECHANI	CAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	NS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	911	659 / 0	0/0	0/0	0/0	252 / 0	0/0
D	1054	762 / 0	0/0	0/0	0/0	292 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

JT

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CH	ORDS							WE	BS			
MAX	C. FACTOR	RED	FACTO	RED					MAX. I	ACTO	RED	
MEMB.	FOR	ICE V	ERT. LC	AD LC1	MAX	MA.	X.	MEMB.	. FC	RCE	MAX	
	(LBS	3)	(PI	LF) (CSI (LC) UNI	BRAC	3	(Li	3S)	CSI (L	_C)
FR-TO	•	•	FROM					FR-TO		-,		/
F- A	-1130 / 0		0.0	0.0	0.06 () :	7.81	A-E	0/9	51	0.12 (1)
A-B	-1034 / 0		-112.4								0.11 (
B- C	-21 / 0		-112.4	-112.4	0.09 (í) (6.25	B- D	-1084/0)	0.18 (
D- C	-168 / 0		0.0	0.0	0.05 () :	7.81					,
					•	•						
F- G	0/0		-18.5	-18.5	0.07 (1) 10	0.00					
G-E	0/0		-18.5	-18.5	0.07	í) 10	0.00					
E- H	0 / 85	4	-18.5	-18.5	0.11 (í) 10	0.00					
H- I	0 / 85	4		-18.5			0.00					
I- D	0 / 85	4		-18.5								
						,						
SPECIF	FIED CONC	ENTR	ATED LC	ADS (LE	35)							
JT	LOC.	LC1	MAX-			FACE		DIR.	TYPE	H	IEEL	CONN.
	1-2-12		-475			ACK		ERT	TOTAL			C1
	3-2-12					ACK		ERT	TOTAL			C1
i	5-2-12	-477		_		ACK		ERT	TOTAL			C1
l												

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPEC	IFIED	Loai	os:		
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL		7.4	PSF
TOTA	L LO	AD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART

TOTAL WEIGHT = 2 X 33 = 66 It

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.11/1.00 (D-E:1) , WB=0.18/1.00 (B-D:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90) JSI METAL= 0.19 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

OB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON		DRWG NO.	
417704 Tamarack Roof Truss, Burli	T18Z4	1	2	TRUSS DESC.	HOMES Version 8.420 S Jan	21 2021 MiTe	k Industries, Inc. Mon Feb 7 17:21:41 2	2022 Pac
· ."	· · · · · · · · · · · · · · · · · · ·	T .			Version 8.420 S Jan ID:JvMhHdWEgsSl57rNnlib4czWe	3nl-QPvjl52	cZFuaoqJFg5Lg47ihoesmVe0ev?	?Viczn
LATES (table is in inche TYPE PLATES BMV1+p MT20	<u>rs)</u> 3 W LEN Y X 3.0 6.0							
	ninimum of 2X4 SPF #2.			•				
Lateral braces to be a n	ninimum of 2X4 SPF #2.							
		,						
		-				-		
					•			
OF	ESSIONAL							
ED PRO	THE							
	ESSIONAL ENGLANGE G. ALVES							
100	009024							
12	MDo!							
Chine	E OF ONTARIO							
					•			
DWG# T-22	emponent only 204201 M_{\odot}				•			

QUANTITY JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO. TRUSS DESC. HOMES T19 3 417704 1 Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:42 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-ubT5yR3EKZ0RQ_uREpsvdLEqf29_E1bn8el3E2znTs7 Tamarack Roof Truss, Burlington 1-3-8 7-0-0 1-3-8

> 4x4 = D 9.00 12 4x6 // 4x6 <> E. 3x4 || 3x4 || Н 4x10 = 4x4 = 14-0-0 14-0-0 1-3-8 1-3-8

LUMBER				
N. L. G. A. F	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				1

DRY: SEASONED LUMBER.

PL	ATES (table i	s in inches)				
JT	TYPE	PLATES	W	LEN	Υ	Χ
В	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
1	BMWWW-t	MT20	4.0	10.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
DEADWOO	

EA	RINGS						
	FACTO	RED	MAXIMU	M FACTO	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIC	N	BRG	BRG
Т	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	1072	0	1072	0	0	5-8	5-8
ł	1072	0	1072	0	0	MECHANIC	CAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	VENT REACTION	1 S		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	748	544 / 0	0/0	0/0	0/0	204 / 0	0/0
Н	748	544 / 0	0/0	0/0	0/0	204 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS

	RDS					WE	BS		
MAX.	FACTORED	FACTORE	D				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX .	
	(LBS)	(PLF)	(CSI (LC)	UNBRAC)	(LBS)	CSI (LC)	
FR-TO	` '	FROM TO			LENGTH	FR-TO		. ,	
A-B	0 / 47	-112.4 -1	12.4	0.15 (1)	10.00	I- D	0 / 465	0.10(1)	
B- C	0 / 27	-112.4 -1	12.4	0.22 (1)	10.00	I-E	-208 / 0	0.09(1)	
C- D	-669 / 0	-112.4 -1	12.4	0.17(1)	6.25	C- I	-208 / 0	0.09(1)	
D- E	-669 / 0	-112.4 -1	12.4	0.17 (1)	6.25	J- C	-971 / 0	0.41 (1)	
E-F	0 / 27	-112.4 -1	12.4	0.22 (1)	10.00	E-H	-971 / 0	0.41 (1)	
F- G	0 / 47	-112.4 -1	12.4	0.15(1)	10.00				
J- B	-305 / 0	0.0	0.0	0.03(1)	7.81				
H- F	-305 / 0	0.0	0.0	0.03 (1)	7.81				
J- I	0 / 655	-18.5 -	18.5	0.30 (4)	10.00				
I- H	0 / 655	-18.5 -	18.5	0.30 (4)	10.00				



SPEC	SPECIFIED LOADS:				
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTA	L LO	AD	=	45.9	PSF

SPACING = 24.0 IN, C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

TOTAL WEIGHT = 3 X 63 = 190 lb [M][F]

Scale = 1:41.3

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.30/1.00 (I-J:4) , WB=0.41/1.00 (C-J:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

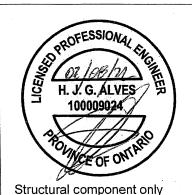
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

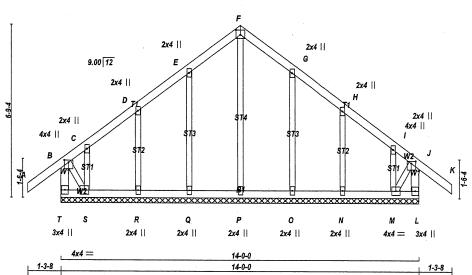
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90) JSI METAL= 0.22 (H) (INPUT = 1.00)



QUANTITY JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO. 417704 G19 TRUSS DESC. HOMES Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:06 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-xgIUP7dfpVTY5S8X7FCT9llhnXx1op1Us209z8znTsh Tamarack Roof Truss, Burlington 1-3-8 7-0-0 1-3-8 4x4 = Scale = 1:43. F



LUMBER				
N. L. G. A. F	ULES			
CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE	WEBS			
	2x3	DRY	No.2	SPF
DRY- SEAS	JVIEDII	IMBER		

GABLE STUDS SPACED AT 2-0-0 OC.

PL/	PLATES (table is in inches)						
·JT	TYPE	PLATES	W	LEN	Υ	Х	
В	TMVW+p	MT20	4.0	4.0	1.00	2.00	
C, t	D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0			
F	TTW-p	MT20	4.0	4.0	2.25	2.00	
	TMVW+p	MT20	4.0	4.0	1.00	2.00	
L	BMV1+p	MT20	3.0	4.0			
M	BMWW1-t	MT20	4.0	4.0			
N, (O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0			
S	BMWW1-t	MT20	· 4.0	4.0			
T	BMV1+p	MT20	3.0	4.0			
1							

NOTES-1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **BEARINGS**

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

<u>LOADING</u> TOTAL LOAD CASES: (4)

ı		ORDS				WE	BS		
١	MAX.	FACTORED	FACTORED				MAX. FACTO	RED	
ı	MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
ı		(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
ı	FR-TO	•	FROM TO		LENGTH	FR-TO			
į	T-B	-342 / 0	0.0 0.0	0.04(1)	7.81	P-F	-180/0	0.13(1)	
Ì	A-B	0 / 47	-112.4 -112.4	0.15(1)	10.00	Q-E	-253 / 0	0.10 (1)	
	B- C	-83 / 0	-112.4 -112.4	0.15(1)	6.25	R- D	-228 / 0	0.05 (1)	
	C- D	-13 / 0	-112.4 -112.4	0.05(1)	6.25	S-C	-51 / 0	0.01 (1)	
Į	D-E	-11 / 0	-112.4 -112.4	0.06(1)	6.25	O- G	-253 / 0	0.10 (1)	
-	E-F	-21 / 0	-112.4 -112.4	0.06 (1)	6.25	N- H	-228/0	0.05 (1)	
	F-G	-21 / 0	-112.4 -112.4	0.06(1)	6.25	M- I	-51/0	0.01 (1)	
1	G- H	-11 / 0	-112.4 -112.4	0.06(1)	6.25	B-S	0/29	0.01 (1)	
1	H- I	-13 / 0	-112.4 -112.4	0.05 (1)	6.25	M- J	0/29	0.01 (1)	
	I- J	-83 / 0	-112.4 -112.4	0.15 (1)	6.25			. ,	
ı	J- K	0 / 47	-112.4 -112.4	0.15 (1)	10.00				
ı	L-J	-342 / 0	0.0 0.0	0.04(1)	7.81				
ı									
į	T-S	0/0	-18.5 -18.5						
	S-R	0 / 16	-18.5 -18.5	0.02 (4)	10.00				
	R-Q	0 / 10	-18.5 -18.5	0.02 (4)	10.00				
	Q-P	0/6	-18.5 -18.5	0.01 (4)	10.00				
	P- O	0/6	-18.5 -18.5						
	O- N	0 / 10	-18.5 -18.5	0.02 (4)	10.00				
	N-M	0 / 16	-18.5 -18.5	0.02 (4)	10.00				
	M-L	0/0	-18.5 -18.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPEC	IFIED	LOA	os:		
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTA	1 10	AΠ	_	45 9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 65 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (R-S:4) , WB=0.13/1.00 (F-P:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (J) (INPUT = 0.90) JSI METAL= 0.13 (E) (INPUT = 1.00)



Structural component only DWG# T-2204172

417701

T19S

QUANTITY 3

PLY

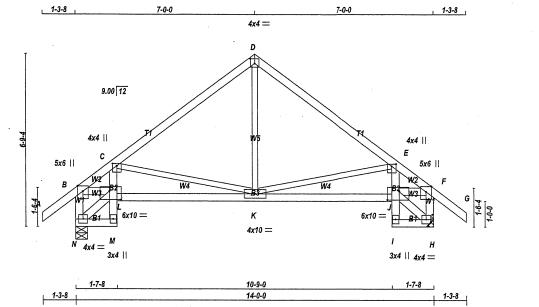
JOB DESC. TRUSS DESC. HOMES

BAYVIEW WELLINGTON

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Feb 8 08:35:14 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-0XQHoZOt1YlFGP8tBIDX64Ol15PanoVe8JIS13znGTr

DRWG NO.



TOTAL WEIGHT = 3 X 66 = 198 II MIF

Scale = 1:43.1

ULES			į.
SIZE		LUMBER	DESCR.
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x3 2x3 2x3	DRY DRY DRY	No.2 No.2 No.2	SPF SPF SPF
	SIZE 2x4 2x4 2x4 2x4 2x4 2x4 2x4 2x4 2x4 2x4	SIZE 2x4 DRY	SIZE LUMBER 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES	(table	is in	inches	Ĺ
IT TO (D)			ATEO	-

JT	TYPE	PLATES	W	LEN	Υ	Х
В	TMVW+p	MT20	5.0	6.0	Edge	
С	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	5.0	6.0	Edge	
Н	BMVW1-t	MT20	4.0	4.0	_	
1	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	6.0	10.0	4.00	5.50
K	BMWWW-t	MT20	4.0	10.0		
L	BVMWW-I	MT20	6.0	10.0	4.00	5.50
М	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only DWG# T-2204209

DIMENSIONS SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
DINILIADIONO, SUFFORIS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
DUIL DINO DECIONED	
BUILDING DESIGNER	·
DE 1 511100	

BEA	RINGS						
	FACTO	RED	MAXIMU	M FACTO	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIC	N .	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	1072	0	1072	.0	0	5-8	5-8
4	1072	0	1072	0	0	MECHANI	CAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM, LIVE	WIND	DEAD	SOIL			
N	748	544 / 0	0/0	0/0	0/0	204 / 0	0/0			
H.	748	544 / 0	0/0	0/0	0/0	204 / 0	0/0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

<u>BRACING</u>
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CH	ORDS					WE	BS		
MAX	. FACTORED	FACTO	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LC	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PI	_F) (CSI (LC)	UNBRAC	כ .	(LBS)	CSI (LC)	
R-TO	\—— - <i>/</i>				LENGTH		(/	()	
A- B	0 / 47	-112.4	-112.4	0.15(1)	10.00	K- D	0/360	0.08(1)	
	-1422 / 0							0.37(1)	
	-760 / 0							0.37 (1)	
D- E	-760 / 0	-112.4	-112.4	0.41 (1)	6.25	N- L	-52 / 0	0.01 (1)	
E-F	-1422 / 0	-112.4	-112.4	0.33 (1)	5.09	B- L	0/1199	0.19 (1)	
F- G	0 / 47	-112.4	-112.4	0.15 (1)	10.00	J- H	-52 / 0		
N-B	-1026 / 0	0.0	0.0	0.11 (1)	7.74	J-F	0/1199		
	-1026 / 0								
NI NA	0 / 41	40.5	40.5	0.04 (4)	10.00				
	0 / 41								
M- L	0 / 15			0.07 (1)					
L-C	0 / 56								
L- K	0 / 1241								
	0 / 1241			0.29 (1)					
I- J									
J- E	0 / 56	0.0	0.0	0.07(1)	10.00				
I- H	0 / 41	-18.5	-18.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPEC	IFIED	LOA	os:		
TOP	CH.	LL	=	32.5	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTA	L LO	AD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.29/1.00 (K-L:1) , WB=0.37/1.00 (C-K:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (L) (INPUT = 0.90) JSI METAL= 0.55 (F) (INPUT = 1.00)

JOB DESC. BAYVIEW WELLINGTON DRWG NO. TRUSS DESC. 417704 T20 1 HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:43 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-Mn1TAn3s5t8I18TeoWN89Ynz9SOHzPfxNIUcmUznTs6 1-3-8 1-5-8 16-0-12 Scale = 1:33.0 5x6 = 5x6 = 3x6 || 5x6 = 16x10 = 8x9 \\ F G Н F 14.00 12 5x8 // 3-7-14 ÌW4 5-8 Z _M X γ AA L AB AC ΑD 0 N P 3x6 || 5x6 = 5x6 == 5x6 = 5x8 = 6x10 =7x12 =19-1-0 1-3-8 18-2-8 10-8 TOTAL WEIGHT = 105 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY LUMBER N. L. G. A. RULES BUILDING DESIGNER BEARINGS FACTORED DESIGN CRITERIA CHORDS LUMBER 2x4 2x6 2x6 2x4 DRY ACEJP C No.2 No.2 SPF MAXIMUM FACTORED INPUT REORD SPECIFIED LOADS: GROSS REACTION VERT HORZ GROSS REACTION BRG DOWN HORZ UPLIFT IN-SX BRG IN-SX DRY CH. 32.5 PSF LL = DL = DRY DRY No.2 No.2 6.0 0.0 7.4 PSF BOT CH. SPF 2388 0 2388 0 0 5-8 5-8 5-8 LL PSF В 2x6 DRY No.2 SPF 2133 5-8 TOTAL LOAD 2x6 2x6 DRY DRY 2100F 1.8E 2100F 1.8E SPF 45.9 PSF UNFACTORED REACTIONS
1ST LCASE MAX
JT COMBINED SNOW 2x4 DRY No.2 SPF SPACING = 24.0 IN. C/C (./MIN. COMPONENT REACTIONS ALL WEBS 2x4 No.2 WIND SOIL LIVE PERM.LIVE DEAD EXCEPT 1672 1195 / 0 0/0 0/0 0/0 477 / 0 0/0 LOADING IN FLAT SECTION BASED ON A SLOPE 1051 / 0 OF 6.00/12 DRY: SEASONED LUMBER. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P. I TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) PLATES (table is in inches)
JT TYPE PLATES LEN TMVW-t 5.0 8.0 8.0 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. 4.50 3.25 TTWW+m MT20 6.0 LOADING TOTAL LOAD CASES: (4) TMWW-t MT20 5.0 - TPIC 2014 TS-t TMW+w MT20 3.0 6.0 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. Ġ TMWW-t MT20 5.0 6.0 CHORDS WEBS MT20 MT20 MT20 MT20 10.0 Edge 12.0 3.75 3.75 10.0 TMVW-t 6.0 7.0 6.0 5.0 5.0 5.0 MAX. FACTORED FACTORED MAX. FACTORED MEMB. VERT. LOAD LC1 MAX MAX. (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD **FORCE** MEMB. FORCE MAX BMWW-t (PLF) M TO (LBS) CSI (LC) UNBRAC (LBS) CSI (LC) 6.0 8.0 6.0 FROM TO -112.4 -112.4 -112.4 -112.4 MT20 FR-TO BMWWW-t BMWW-t MT20 O-O-H N G 0 / 60 0.18 (1) A- B B- C 10.00 -391 / 0 0.05(1) 4.53 4.17 4.17 3.95 0/1425 0/4218 0/2816 ALLOWABLE DEFL.(LL)= L/360 (0.64") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.14") ALLOWABLE DEFL.(TL)= L/360 (0.64") CALCULATED VERT. DEFL.(TL)= L/917 (0.25") 2.50 2.00 0.25 (1) 0.75 (1) MT20 -1997/00.22 (1) -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 5.0 6.0 C- Q Q- D BMWW-t MT20 -3661 / 0 0.31 (1) BMV1+p 0.31 (1) 0.28 (1) 0.50 (1) D-R R-E 0.16 (1 4263 / 0 -1248 / 0 -4263 / 0 -4263 / 0 -4263 / 0 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 N- D M- G D- M M- F -1248/0 -1225/0 0/748 0/707 Edge - INDICATES REFERENCE CORNER OF PLATE 0.28 (1) 3.95 3.95 0.16 (1) E-S S-F F-T TOUCHES EDGE OF CHORD. CSI: TC=0.36/1.00 (H-J:1), BC=0.75/1.00 (J-K:1), WB=0.75/1.00 (H-K:1), SSI=0.86/1.00 (I-J:1) 0.13 (1) 0.13 (1) 3.95 0.28 (1) -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 0.09 (1) -4263 / 0 0.28 (1 3 94 -723 / 0 T- U U- G G- V V- W NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2. -4263 / 0 -4263 / 0 0.28 (1) 0.28 (1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 3.94 COMP=1.00 SHEAR=1.00 TENS= 1.00 -3625 / 0 0.32 (1 4.17 -3625 / 0 -112.4 -112.4 -112.4 -112.4 0.32 (1) COMPANION LIVE LOAD FACTOR = 1.00 W-H J- H P- B -2534 / 00.0 0.0 0.36 (1) 5.33 AUTOSOLVE RIGHT HEEL ONLY 0.18 (1) -2447 / N 0.0 0.0 6.56 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE P- O O- X X- N N- Y Y- Z -18.5 0.05(1) 10.00 0.11 (1) 0.11 (1) 0.28 (1) 0 / 1241 -18.5 -18.510.00 TRUSS MANUFACTURING PLANT. 0 / 1241 -18.5 -18.5 -18.5 -18.5 10.00 PROFESSIONAL ENGINEERS HIS H. J. G./ALVES NAIL VALUES PLATE GRIP(DRY) SHEAR (PLI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 0 / 3661 -18.5-18.5 0.28 (1) 10.00 -18.5 -18.5 -18.5 -18.5 0.28 (1) 10.00 7- M 0 / 3661 AA- L -18.5 10.00 0 / 3625 -18.50.36(1) 0/3625 0/3625 0/3625 0/0 0/0 0/0 L-AB AB- K K-AC AC-AD AD- J J- I -18.5 -18.5 -18.5 0.36 (1) 0.36 (1) 0.36 (1) 0.75 (1) -18.5 -18.5 10.00 PLATE PLACEMENT TOL. = 0.250 inches -18.5 10.00 -18.5 -18.5 0.75 (1) 10.00 PLATE ROTATION TOL. = 5.0 Deg. -18.5 0.75 (1) 100009024 JSI GRIP= 0.79 (N) (INPUT = 0.90) -18.5-18.5 0.70(1) 10.00 JSI METAL= 0.67 (J) (INPUT = 1.00) SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ OVINCE OF ONT ARIO LC1 -107 LOC. 1-5-8 FACE DIR TYPE HEEL CONN. BACK BACK BACK BACK CDNOQRST -107 VERT TOTAL C1 C1 C1 C1 C1 -101 -50 -50 -107 -101 -50 -50 5-6-4 5-6-4 1-6-4 3-6-4 7-6-4 VERT VERT TOTAL <u>-</u> VERT TOTAL. -101 -101 -101 VERT VERT TOTAL TOTAL BACK BACK BACK BACK -101 -101 Structural component only -101 VERT TOTAL Ċ1 DWG# T-2204203 4/2 CONTINUED ON PAGE 2

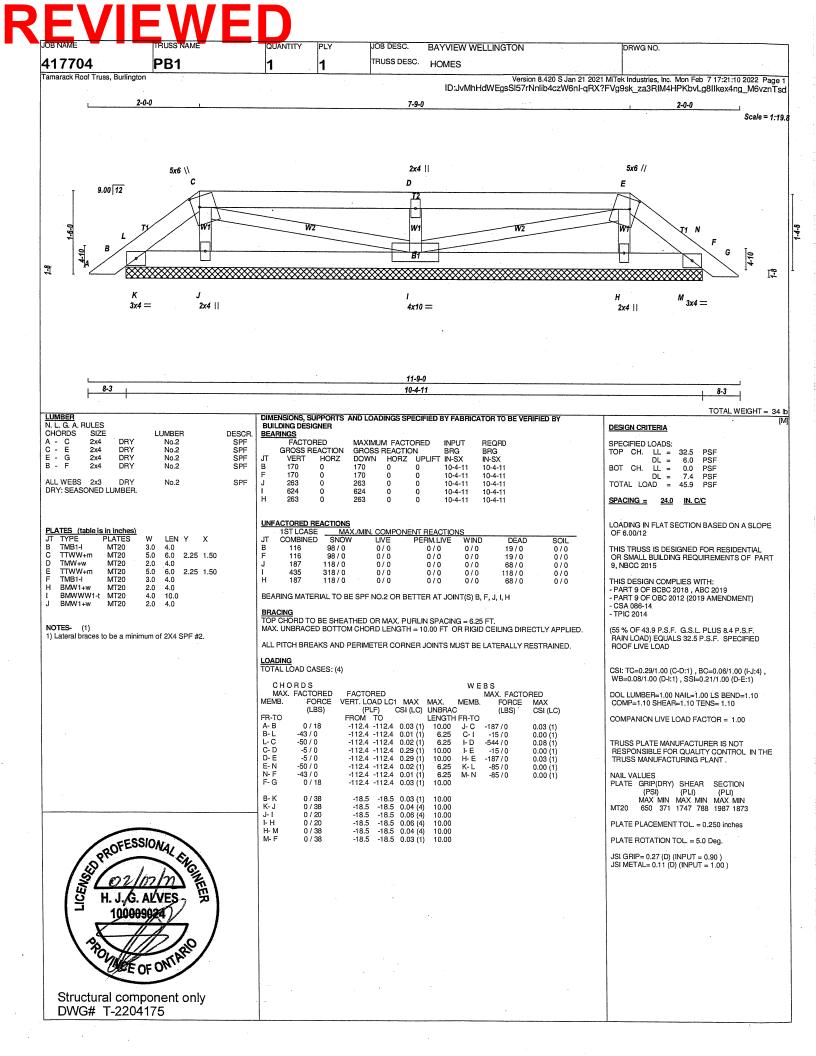
JOB DESC. PLY BAYVIEW WELLINGTON DRWG NO. TRUSS DESC. HOMES 417704 T20 1 Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MITek Industries, Inc. Mon Feb 7 17:21:43 2022 Page 2 ID:JvMhHdWEgsSI57rNnlib4czW6nl-Mn1TAn3s5t8I18TeoWN89Ynz9SOHzPfxNlUcmUznTs6 SPECIFIED CONCENTRATED LOADS (LBS)
JT LOC. LC1 MAX- MAX+
U 13-6-4 -101 -101 --V 15-6-4 -101 -101 --W 17-6-4 -50 -50 --Z 9-6-4 -50 -50 --AA 11-6-4 -50 -50 --AC 15-6-4 -50 -50 --AD 17-6-4 --AD 17-6-4 --AD 17-6-4 --AD 17-6-4 --AD 17-6-4 --AD 17-6-4 --CONN. C1 TYPE
TOTAL
TOTAL HEEL

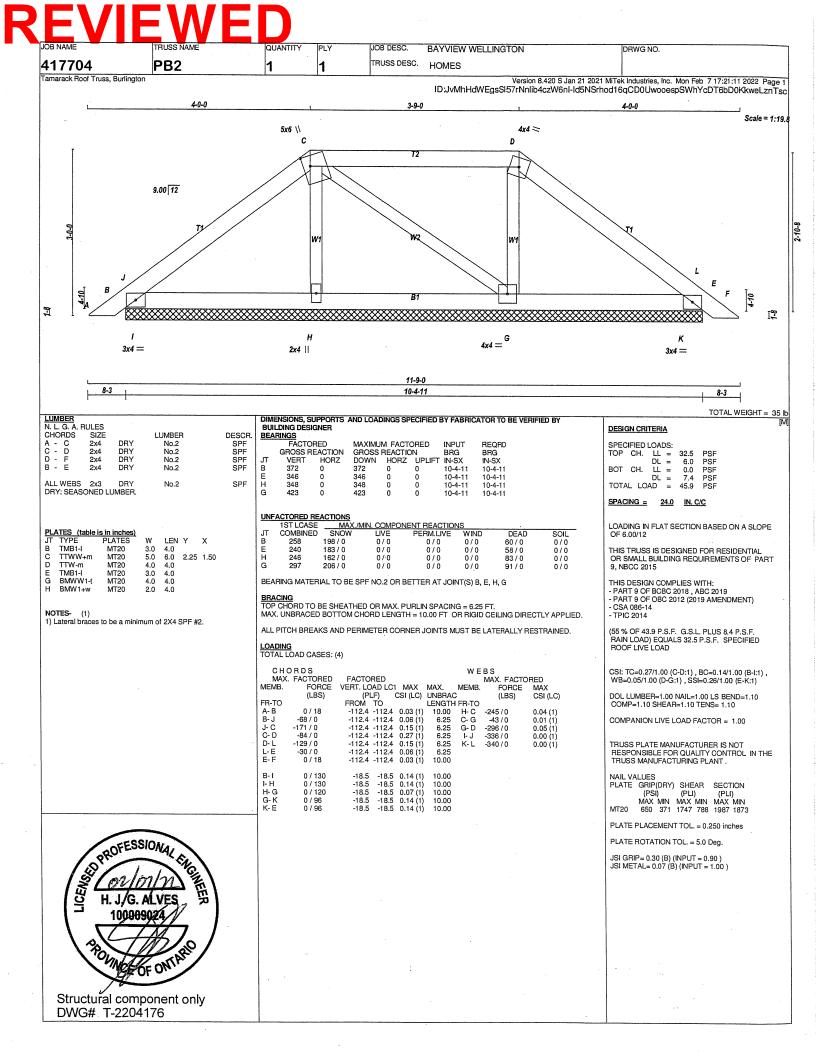
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

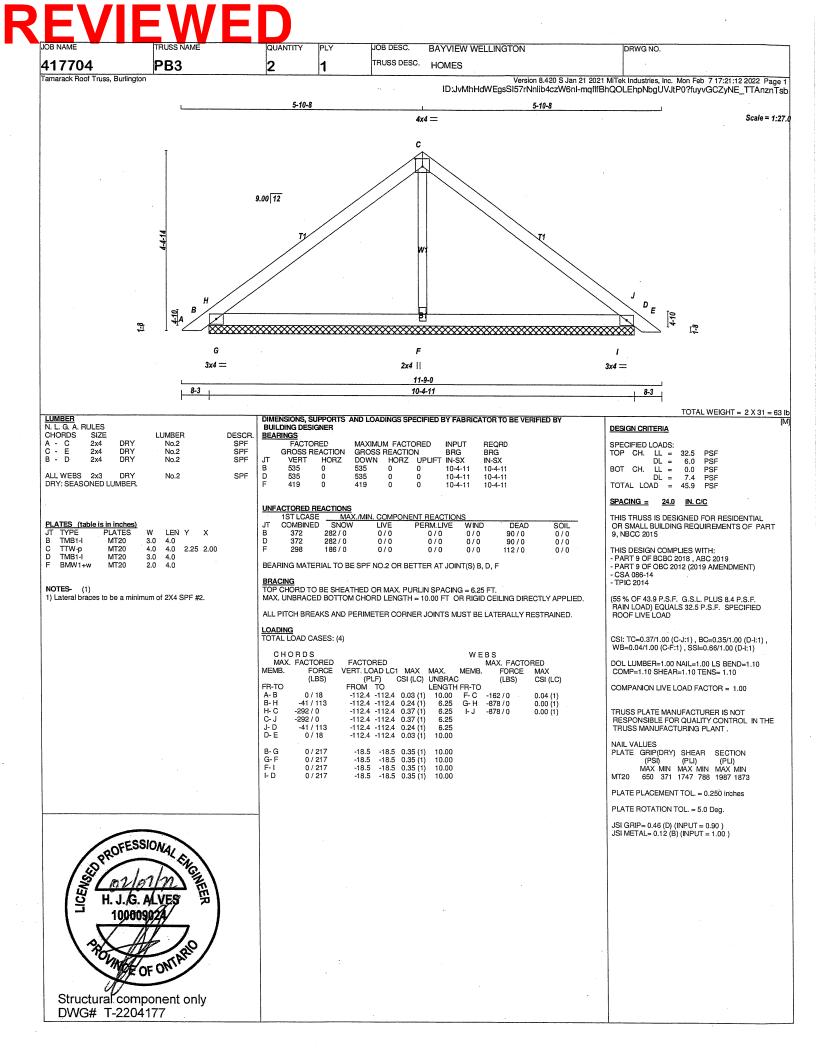
CONNECTION REQUIREMENTS

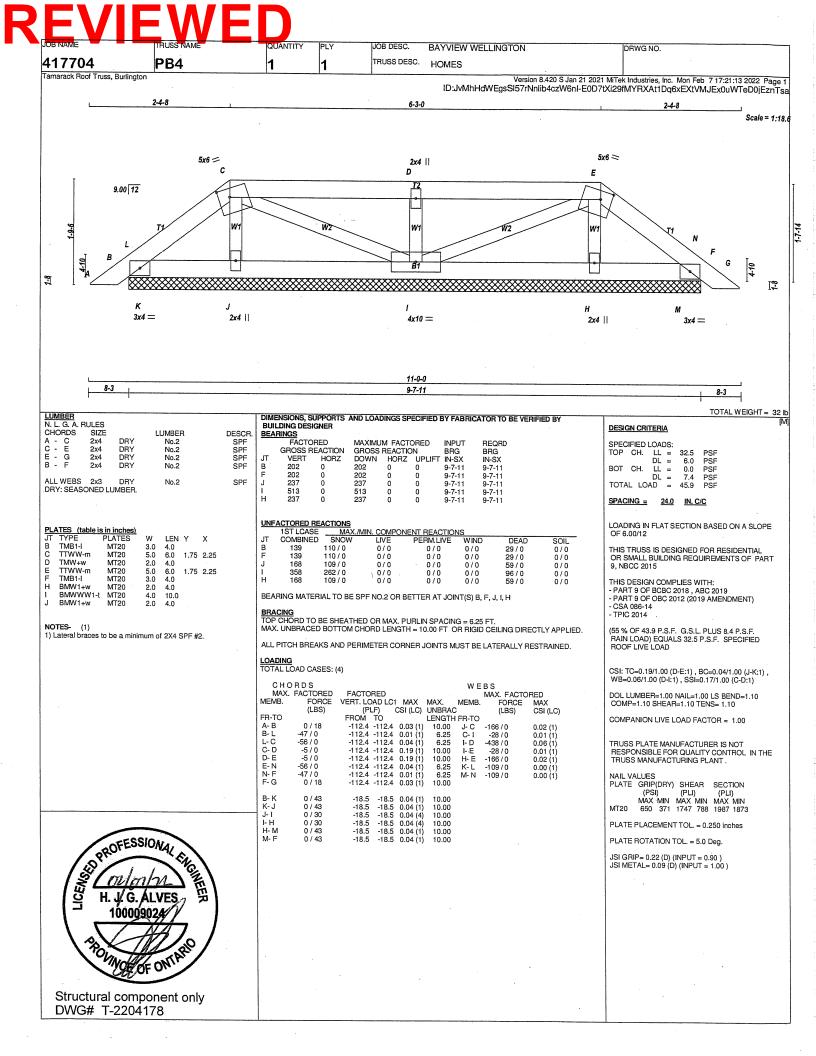


Structural component only DWG# T-2204203 //L









JOB DESC. PLY BAYVIEW WELLINGTON DRWG NO. TRUSS DESC. 417704 PB5 1 HOMES Version 8.420 S Jan 21 2021 MTk Industries, Inc. Mon Feb 7 17:21:14 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nI-iCnV4sjgwyUP3gl3bwMLUR41VlfPgS1gilyaFgznTsZ Tamarack Roof Truss, Burlington 2-0-0 7-0-0 Scale = 1:18. 5x6 // 5x6 \\ 2x4 || D С Ε 9.00 12 W1 W١ N 3 [* 2x4 || 4x10 =3x4 == 2x4 || 3x4 = 11-0-0 8-3 9-7-11 8-3 TOTAL WEIGHT = 32 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER LUMBER N. L. G. A. RULES CHORDS SIZE **DESIGN CRITERIA** BEARINGS FACTORED SIZE LUMBER DESCR DRY No.2 No.2 SPF MAXIMUM FACTORED REQRD SPECIFIED LOADS: GROSS REACTION DOWN HORZ L 171 0 0 171 0 0 LL = DL = LL = DRY GROSS REACTION 2x4 2x4 BRG BRG TOP CH. 32.5 PSF UPLIFT IN-SX 9-7-11 6.0 0.0 7.4 PSF PSF G No.2 SPF VERT HORZ IN-SX SPF 171 171 BOT CH. 9-7-11 9-7-11 PSF 9-7-11 DL ALL WEBS DRY 2x3 No.2 SPF 242 242 Ó 0 9-7-11 TOTAL LOAD 45.9 DRY: SEASONED LUMBER. SPACING = 24.0 IN. C/C UNFACTORED REACTIONS
1ST LCASE MA LOADING IN FLAT SECTION BASED ON A SLOPE PLATES (table is in inches)
JT TYPE PLATES MAX OF 6.00/12 LEN Y COMBINED WIND DEAD SOIL 4.0 6.0 4.0 97 / 0 97 / 0 TMB1-I 117 0/0 0/0 THIS TRUSS IS DESIGNED FOR RESIDENTIAL 21/0 21/0 2.25 1.50 TWW+m OR SMALL BUILDING REQUIREMENTS OF PART 2.0 5.0 3.0 0/0 0/0 0/0 0/0 62 / 0 106 / 0 TMW+w MT20 172 110 / 0 0/0 0/0 9. NBCC 2015 6.0 4.0 4.0 TTWW+m TMB1-I MT20 MT20 2.25 1.50 I H 288 / 0 110 / 0 THIS DESIGN COMPLIES WITH: 0/0 0/0 0/0 62/0 Н 2.0 BMW1+w MT20 PART 9 OF BCBC 2018, ABC 2019 10.0 BMWWW1-t MT20 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H - PART 9 OF OBC 2012 (2019 AMENDMENT) BMW1+w CSA 086-14 - TPIC 2014 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2. (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED ROOF LIVE LOAD LOADING TOTAL LOAD CASES: (4) CSI: TC=0.23/1.00 (C-D:1) , BC=0.05/1.00 (I-J:4) , WB=0.07/1.00 (D-I:1) , SSI=0.19/1.00 (D-E:1) WEBS MAX. FACTORED FACTORED MAX. FACTORED DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 FACTORED VERT. LOAD LC1 MAX MAX.

(PLF) CSI (LC) UNBR
FROM TO
-112.4 -112.4 0.00 (1) 6.2
-112.4 -112.4 0.02 (1) 6.2
-112.4 -112.4 0.23 (1) 10.0 FORCE MEMB. MEMB. COMP=1.10 SHEAR=1.10 TENS= 1.10 CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) (LBS) FR-TO COMPANION LIVE LOAD FACTOR = 1.00 J- C C- I I- D I- E 0 / 18 0.02 (1) A-B B-C C-D-E N-F F-G 10.00 -172 / 0 -43 / 0 -48 / 0 6.25 6.25 -16/0 -491/0 0.02 (1) 0.00 (1) 0.07 (1) TRUSS PLATE MANUFACTURER IS NOT 0.00(1) -4/0 -4/0 10.00 -16/0 RESPONSIBLE FOR QUALITY CONTROL IN THE -112.4 -112.4 -112.4 -112.4 -112.4 -112.4 0.23 (1) 0.02 (1) 0.00 (1) 10.00 6.25 6.25 H- E K- L M- N 0.02 (1) TRUSS MANUFACTURING PLANT. -81 / 0 0.00 (1) NAIL VALUES -43 / 0 PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 0 / 18 -112.4 -112.4 0.03 (1) 10.00 -18.5 -18.5 -18.5 -18.5 0 / 37 -18.5 0.03 (1) 10.00 -18.5 -18.5 -18.5 0 / 37 0 / 20 0 / 20 0.03 (4) 0.05 (4) 0.05 (4) 10.00 10.00 MT20 650 371 1747 788 1987 1873 J- I I- H H- M M- F PLATE PLACEMENT TOL. = 0.250 inches PROFESSIONAL CHARLES THE PROFESSIONAL CHARLES 0 / 37 -18.5 -18.5 0.03 (4) 10.00 0 / 37 PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.25 (D) (INPUT = 0.90) JSI METAL= 0.10 (D) (INPUT = 1.00) 100009024 OF ONTARIO Structural component only DWG# T-2204179

417704

Tamarack Roof Truss, Burlington

J1

21

PLY

JOB DESC.

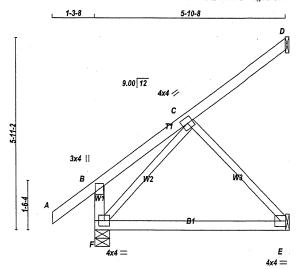
BAYVIEW WELLINGTON

TRUSS DESC. HOMES

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:07 2022 Page 1 ID:JvMhHdWEgsSI57rNnlib4czW6nl-PsrscTeHapbOjbjjhykiizHsNxEYXHxe5imiVaznTsg

DRWG NO.

Scale = 1:33.9



5-10-8 5-10-8

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
F D	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASO			140.2	311
,DH 1. SEAS	DIVED L	UIVIDEN.		

 PLATES
 (table is in inches)

 JT
 TYPE
 PLATES

 B
 TMV+p
 MT20

 C
 TMWW-t
 MT20

W 3.0 4.0 4.0 4.0 LEN Y 4.0 4.0 4.0 BMW1-t MT20 Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER**

BEARINGS											
	FACTO	RED	MAXIMU	M FACTO	INPUT	REQRD					
	GROSS R	EACTION	GROSS	REACTIC	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
F	537	0	537	0	0	5-8	5-8				
D	134	0	134	0	0	1-8	1-8				
E	252	0	252	0	0	1-8	1-8				

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

UNF	UNFACTORED REACTIONS								
	1ST LCASE	MAX./N	IN. COMPO	NENT REACTION	NS.				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
F	373	279 / 0	0/0	0/0	0/0	94 / 0	0/0		
D	92	77 / 0	0/0	0/0	0/0	14/0	0/0		
E	179	115/0	0/0	0/0	0/0	64 / 0	0/0		

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СНС	RDS				WE	BS		
MAX.	FACTORED	FACTORED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
F-B	-272 / 0	0.0 0.0	0.03 (1)	7.81	C-E	-268 / 0	0.09(1)	
A-B	0 / 47	-112.4 -112.4	0.15(1)	10.00	F- C	-277 / 0	0.08(1)	
B- C	0 / 25	-112.4 -112.4	0.16 (1)	10.00				
C- D	-23 / 0	-112.4 -112.4	0.16 (1)	6.25				
F. F	0 / 180	-185 -185	0 19 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

 SPECIFIED LOADS:

 TOP CH. LL = 32.5 PSF

 DL = 6.0 PSF

 BOT CH. LL = 0.0 PSF

 DL = 7.4 PSF

 TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART

TOTAL WEIGHT = 21 X 25 = 527 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.19/1.00 (E-F:4) , WB=0.09/1.00 (C-E:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

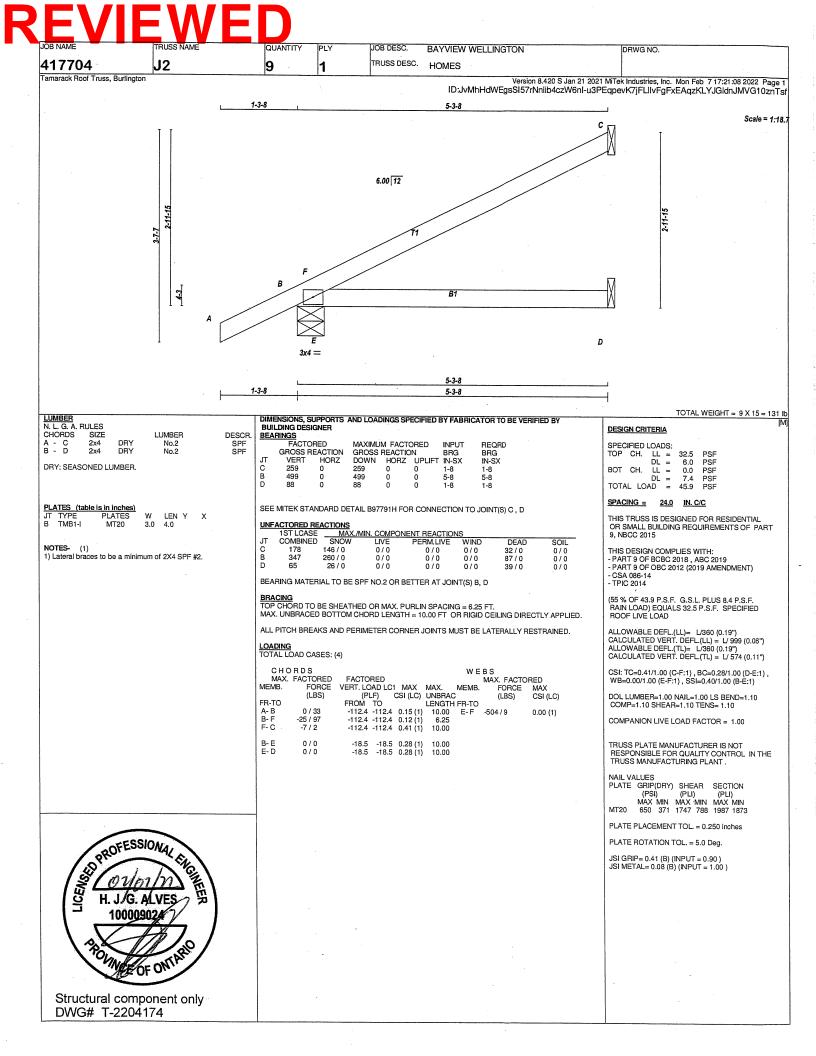
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

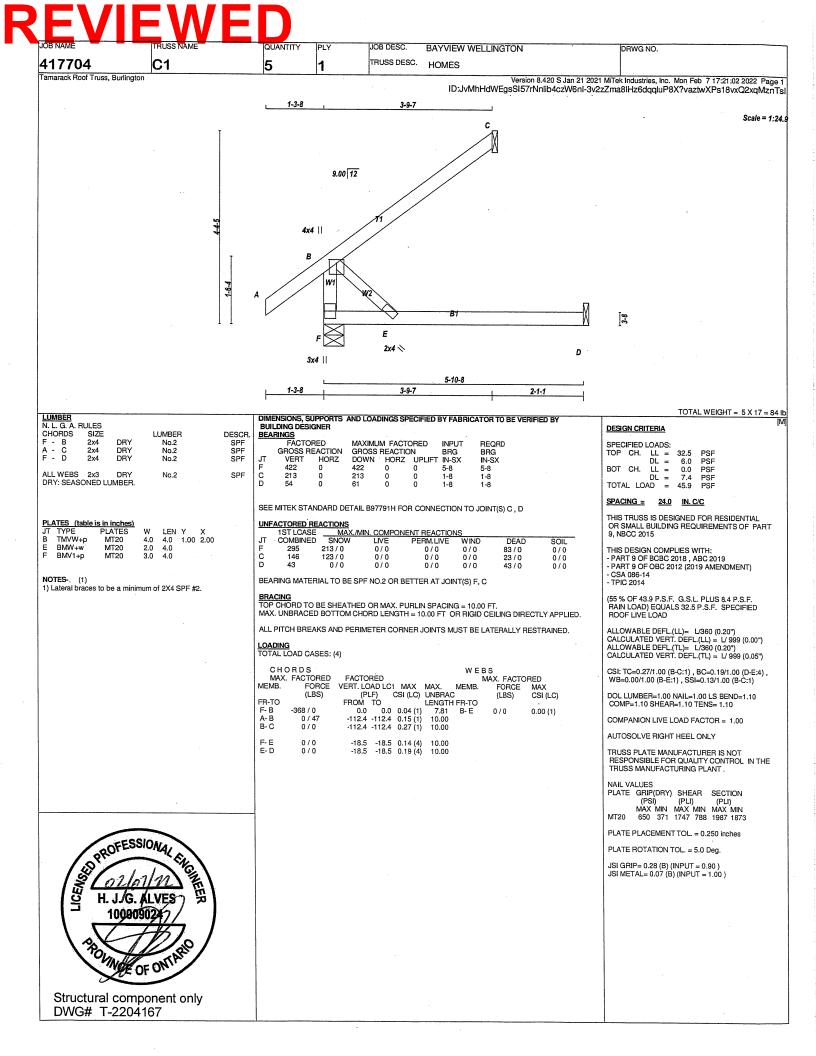
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (C) (INPUT = 0.90) JSI METAL= 0.10 (B) (INPUT = 1.00)





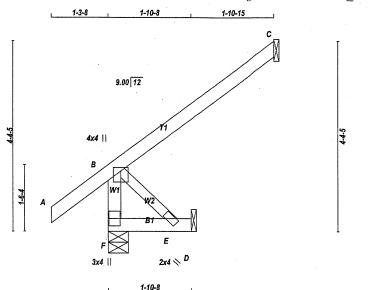


JOB DESC. PLY **BAYVIEW WELLINGTON** DRWG NO. 417704 C2 TRUSS DESC. HOMES Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:03 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-X5cLn6bmWa5zE_PyS6fmX77AXKtebUO2A4oVMpznTsk 1-3-8 1-9-7 Scale = 1:17. C 9.00 12 4x4 || W1 3-8 E G Н 2x4 💉 D 1-11-4 2-0-0 1-11-4 1-9-7 4-1-1 TOTAL WEIGHT = 5 X 14 = 69 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** BEARINGS FACTORED SIZE LUMBER DESCR JRD F - B A - C F SPF SPF SPF 2x4 DRY No.2 MAXIMUM FACTORED INPUT REQRD SPECIFIED LOADS: DRY DRY DL = DL = DL = DL = AD = GROSS REACTION GROSS REACTION PSF PSF PSF BRG BRG CH. 32.5 No.2 HORZ UPLIFT 6.0 0.0 7.4 VERT DOWN HORZ IN-SX IN-SX 370 370 41 BOT CH. ALL WEBS 2x3 DRY No.2 SPF . CD 1-8 1-8 61 TOTAL LOAD SPACING = 24.0 IN. C/C SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D THIS TRUSS IS DESIGNED FOR RESIDENTIAL PLATES (table is in inches)
JT TYPE PLATES
B TMVW+p MT20 UNFACTORED REACTIONS OR SMALL BUILDING REQUIREMENTS OF PART W 4.0 LEN Y 1ST LCASE MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE 9, NBCC 2015 1.00 2.00 WIND SOIL 0/0 0/0 4.0 COMBINED DEAD BMW+w 259 28 182 / 0 23 / 0 MT20 2.0 0/0 77/0 THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) BMV1+p 0/0 0/0 07.0 - CSA 086-14 NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C <u>BRACING</u>
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. ROOF LIVE LOAD LOADING TOTAL LOAD CASES: (4) ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05") CHORDS WEBS MAX. FACTORED FACTORED MAX. FACTORED FORCE MAX VERT. LOAD LC1 MAX (PLF) CSI (LC) (PLF) CSI (LC) (PLF) TROM TO 0.0 0.03 (1) -112.4 -112.4 0.15 (1) -112.4 -112.4 0.15 (1) MEMB. MAX CSI (LC) UNBRAC CSI: TC=0.15/1.00 (A-B:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1) (LBS) (LBS) CSI (LC) LENGTH FR-TO 7.81 B- E -315 / 0 B-E 0.00 (1) 10.00 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 B-C -36 / 0 6.25 F- E E- G G- H -18.5 -18.5 -18.5 0.13 (4) 0.19 (4) 0.19 (4) 10.00 COMPANION LIVE LOAD FACTOR = 1.00 -18.5 10.00 0/0 0/0 -18.5 10.00 AUTOSOLVE RIGHT HEEL ONLY H- D TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. SPECIFIED CONCENTRATED LOADS (LBS) LOC. 1-11-4 MAX-MAX+ FACE DIR. VERT TYPE CONN. BACK TOTAL C1 C1 NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION VERT TOTAL CONNECTION REQUIREMENTS (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 PROFESSIONAL ENGINE BU ON IN INC. H. J. G. ALVES 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.24 (B) (INPUT = 0.90) JSI METAL= 0.06 (B) (INPUT = 1.00) 100009024 POLINGE OF ONTARIO Structural component only

JOB DESC. BAYVIEW WELLINGTON PLY DRWG NO. TRUSS DESC. HOMES 417704 C3 5

Tamarack Roof Truss, Burlingtor

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:04 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-?HAj_SbPHuDqs8_80qA?4KfJNjFVKxeBPkX2uFznTsj



1-10-15

LUMBER
N. L. G. A. RULES
CHORDS SIZE
F - B 2x4 SIZE LUMBER DESCR SPF SPF SPF - B DRY No.2 DRY DRY No.2 ALL WEBS 2x3 DRY DRY: SEASONED LUMBER. No.2 SPE

PLATES (table is in inches)
JT TYPE PLATES
B TMVW+p MT20 LEN 4.0 4.0 W 4.0 1.00 2.00 BMW+w MT20 BMV1+p MT20 3.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

1-10-8

BEARINGS INPUT BRG FACTORED MAXIMUM FACTORED REQRD GROSS REACTION VERT HORZ GROSS REACTION DOWN HORZ U BRG IN-SX UPLIFT IN-SX 385 0 385 5-8 1-8 213 17 213 19 1-8 1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	ASE MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
F	266	213/0	0/0	0/0	0/0	53 / 0	0/0		
С	146	123 / 0	0/0	0/0	0/0	23 / 0	0/0		
D	14	0/0	0/0	0/0	0/0	14/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

<u>LOADING</u> TOTAL LOAD CASES: (5)

	RDS FACTORED	FACTOR	RED		W E B S MAX. FACTORED				
MEMB.	FORCE	VERT. LO	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (CSI (LC)	UNBRAC)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO	, ,		
F-B	-368 / 0	0.0	0.0	0.04(1)	7.81	B-E	0/0	0.00(1)	
A-B	0 / 47	-112.4	-112.4	0.15 (1)	10.00			` '	
B-C	0/0	-112.4	-112.4	0.27 (1)	10.00				
F-E	0/0			0.02 (4)	10.00				
E- D	0/0	-18.5	-18.5	0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

TOTAL LOAD

SPECIFIED LOADS: LL = 32.5 DL = 6.0 LL = 0.0 DL = 7.4 AD = 45.9 PSF PSF CH. BOT CH. PSF PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

PSF

TOTAL WEIGHT = 5 X 12 = 61 lb

Scale = 1:25.2

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
CSA 086 14 - CSA 086-14 - TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

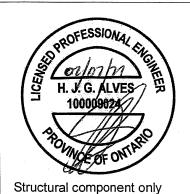
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

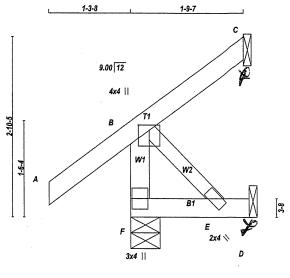
JSI GRIP= 0.28 (B) (INPUT = 0.90) JSI METAL= 0.07 (B) (INPUT = 1.00)



JOB DESC. **BAYVIEW WELLINGTON** PLY DRWG NO. 417704 C4 5 TRUSS DESC. **HOMES**

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon Feb 7 17:21:04 2022 Page 1 ID:JvMhHdWEgsSl57rNnlib4czW6nl-?HAj_SbPHuDqs8_80qA?4KfLHjFVKxeBPkX2uFznTsj



1-10-8 1-3-8 1-9-7

LUMBER
N. L. G. A. RULES
CHORDS SIZE
F - B 2x4 LUMBER DESCR DRY DRY DRY SPF SPF SPF No.2 No.2 - B 2x4 No.2 ALL WEBS 2x3 DRY DRY: SEASONED LUMBER. SPF

 PLATES
 (table is in inches)

 JT
 TYPE
 PLATES

 B
 TMVW+p
 MT20

 E
 BMW+w
 MT20

 F
 BMV1+p
 MT20
 LEN Y 4.0 1 4.0 4.0 W 4.0 2.0 Y X 1.00 2.00

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS FACTORED MAXIMUM FACTORED GROSS REACTION DOWN HORZ UPLII INPUT REQRD GROSS REACTION VERT HORZ BRG BRG IN-SX UPLIFT IN-SX 333 333 41 5-8 1-8 5-8 1-8 41 17 19 1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	101 FOASE	IVIAA./IV	IIIN. COMPO	NENT REACTION	<i>1</i> 2		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	230	182 / 0	0/0	0/0	0/0	48 / 0	0/0
С	28	23 / -35	0/0	0/0	0/0	4/0	0/0
D	14	0/0	0/0	0/0	0/0	14/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

. C D

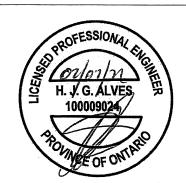
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

CHC	RDS				WEBS					
MAX.	FACTORED	FACTORE	ED				MAX. FACTO	RED		
MEMB.	FORCE	VERT. LOAI	D LC1	MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PLF) (CSI (LC)	UNBRAC)	(LBS)	CSI (LC)		
FR-TO		FROM T	Ö		LENGTH	FR-TO				
F-B	-315 / 0	0.0	0.0	0.03(1)	7.81	B-E	0/0	0.00(1)		
A-B	0 / 47	-112.4 -1	112.4	0.15(1)	10.00					
B- C	-36 / 0	-112.4 -1	12.4	0.15(1)	6.25					
F-E	0/0			0.02 (4)	10.00					
E- D	0/0	-18.5	-18.5	0.02 (4)	10.00					

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



Structural component only DWG# T-2204170

DESIGN CRITERIA

SPECIFIED LOADS: LL = 32.5 DL = 6.0 LL = 0.0 DL = 7.4 AD = 45.9 PSF TOP CH. PSF PSF PSF BOT CH.

TOTAL LOAD

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 5 X 9 = 47 lb

Scale = 1:17.

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL. IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR (PLI) (PLI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90) JSI METAL= 0.06 (B) (INPUT = 1.00)





Alves Engineering Services Inc.

5208 Easton road Burlington, Ontario L7L 6N6 (289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes. Feb 09, 2018

7-1800213

REVIEWED MiTek[®]

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020

Expiry:

APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

A Marine Process	ALESTRATION OF THE PARTY OF THE					
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	. 36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:		Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter	(in.)	0.160	0.152	0.144	0.122	0.120
Length	(in.)	3.50	3.50	3.00	3.00	3.25
			SE SE NASINE	Miniminante	ENAIG EST	
2x4 SPF		2	2 .	3	3	3
2x6 SPF		4	4	4	5	5
2x4 D. FI	R	2	2	2	2	2
2x6 D. FI	R	3,	3	3	4	4

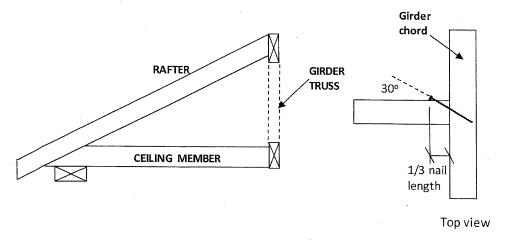


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



December 21, 2020

REVIEWED MiTek®

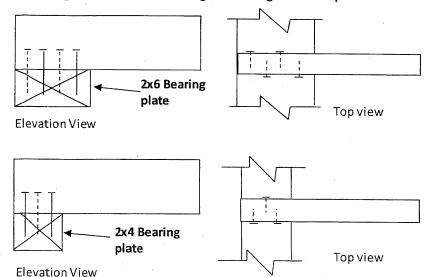
STANDARD DETAIL MSD2015-H

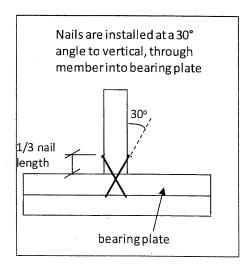
Issued: SEPTEMBER 22, 2020

Expiry: **APRIL 30, 2022**

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift





NOTES:

- 1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
- 2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to wind or earthquake loads do not exceed the withdrawal resistance of the toe-nails. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is NOT permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
- **3.** Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
- 4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).
- 5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
- 6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
- 7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
- **8.** Nail values in this table comply with CSA O86-19, Clause 12.9.



Page 2 of 2

December 21, 2020

SIMPSON Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217-218.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18-20.

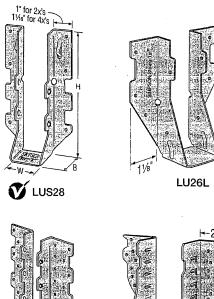
Installation:

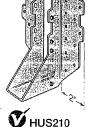
Plated Truss Connectors

- · Use all specified fasteners; see General Notes.
- · Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- · Not designed for welded or nailer applications.
- \bullet With single ply 2x carrying members, use 10d x 1 ½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

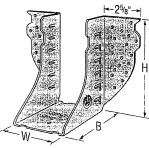
Options:

- · LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105-107.





HUS210 (HUS26, HUS28, and HHUS similar)





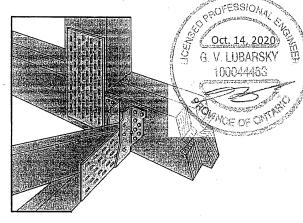


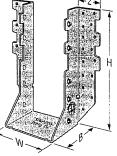
Double-Shear Nailing

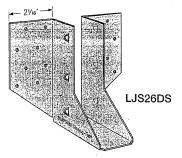


Typical HUS26 Installation with Reduced Heel Height

(Truss Designer to provide fastener quantity for connecting multiple members together)







TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

LUS28

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge Finish: G90 galvanized

Design:

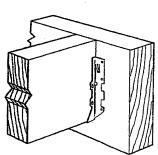
- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

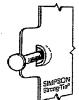
These hangers cannot be modified



Typical LUS Installation

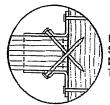
							-		. п	staliauor	ı		
ĺ			Dimens	ions (in	.)	Fast	Fasteners		Factored Resistance (ib.)				
Model	Ga.	 	7		,		T	D.F	īr-L	S-P-F			
No.	ua.	W	н	В	d _a ^T	Face	Joist	Uplift	Normal	Uplift	Normal		
11100	 				40	. 400	Just	(K ₀ =1.15)	(K ₀ =1.00)	$(K_0=1.15)$	(K ₀ =1.00)		
LUS24	18	19/16	31/8	134	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155		
LUS24-2	18	31/6	31/6	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435		
LUS26	18	1%6	43/4	13/4	3%	(4) 10d	(4) 10d	1420	2170	1290	1630		
LUS26-2	18	31/8	47/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920		
LUS26-3	18	4%	43/16	2	31/4	(4) 16d	(4) 16d	1720	2595	1545	2340		
LUS28	18	1%	6%	13/4	3¾	(6) 10d	(6) 10d	1420	2520	1290			
LUS28-2	18	31/8	7	2	4	(6) 16d	(4) 16d	1720	3325		1790		
LUS28-3	18	4%	61/4	2	31/4	(6) 16d	(4) 16d	1720		1545	2575		
LUS210	18	19/16	7 13/16	13/4	37/8	(8) 10d	(4) 10d		3325	1545	2375		
Ш8210-2	18	31/6	9			··-		1420	2785	1290	2210		
				2	6	(8) 16d	(6) 16d	2580	4500	2320	3195		
LUS210-3	18	4%	83/16	2	51/4	(8) 16d	(6) 16d	2580	3345	2320	2375		

^{1.} d_{e} is the distance from the seat of the hanger to the highest joist nail.

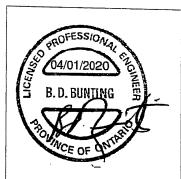


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.





This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tia Company Inc.

T-SPECLUS20 3/20 exp. 6/22

TECHNICAL BULLETIN

HUS/LJS - Double Shear Joist Hangers

SIMPSON Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table Finish: G90 galvanized

Design:

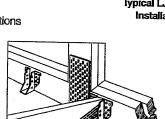
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

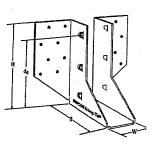
Installation:

Options:

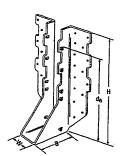
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x $3\frac{1}{2}$ " long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

See current catalogue for options

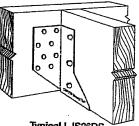




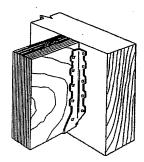
LJS26DS



HUS210 (HUS26, HUS28, similar)



Typical LJS26DS Installation



Typical HUS Installation

Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

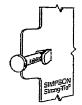
		D	Dimensions (in.)				teners	Factored Resistance (lb.)				
Model	Ga.		1					D.F	ir-L	S-	P-F	
No.	ua.	W	Н	В	d _e †	Face	Joist	Uplift (K ₀ =1.15)	Normal (K _p =1.00)	Uplift (K ₀ =1.15)	Normal (K _p =1.00)	
1 teache	10	401	<u> </u>					lb.	lb.	lb.	lb.	
LJS26DS	18	19/16	5	3½	45/B	(16) 16d	(6) 16d	2055	4265	1460	4115	
HUS26	16	1%	53/8	3	315/16	(14) 16d	(6) 16d	2705	4940	2065	3875	
HUS28	16	1%	73/32	3	63/32	(22) 16d	(8) 16d	3605	5365	2675	4345	
HUS210	16	15%	93/32	3	731/32	(30) 16d	(10) 16d	4505	5795	4010		
HUS1.81/10	16	113/16	9	3	8	(30) 16d					4740	
t d in the dia						(30) 100	(10) 16d	4505	6450	4010	5200	

the distance from the seat of the hanger to the highest joist nail.

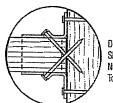


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

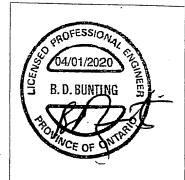
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.





This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

TECHNICAL BULLETIN

HGUS - Double Shear Joist Hangers

SIMPSON Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge **Finish:** G90 galvanized

Design:

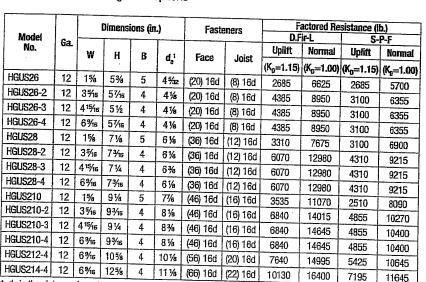
- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%.
 No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

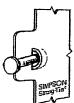
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



See current catalogue for options

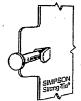


1. d_{e} is the distance from the seat of the hanger to the highest joist nail.

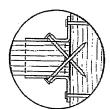


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

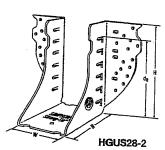
U.S. Patent 5.603.580

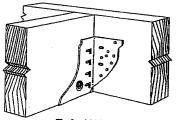


Double Shear Nailing Side View. Do not bend tab back.

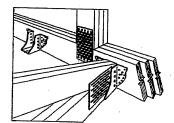


Double Shear Nailing Top View.





Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)



(800) 999-5099 strongtie.com



This technical bulletin is effective until June 30; 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relled upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

TECHNICAL BULLETIN

HHUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge Finish: G90 galvanized

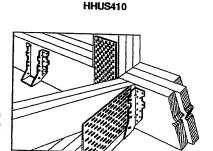
Design:

- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

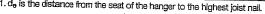
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

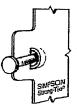


Options:

See current catalogue for options

	1	0	imensi	ons (in.	}	Faste	eners	F	actored Re	sistance (I	b.)
Model	Ga.	<u> </u>	Т	, ` 				D.Fir-L		S-P-F	
No.	ua.	w	н	В	d _e 1	Face	Joist	Uplift	Normal	Uplift	Normal
					u _e	1 400	Just	(K _p =1.15)	(K _p =1.00)	(K _n =1.15)	(K _D =1.00)
HHUS26-2	14	3%6	5 13/16	3	315/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	35/16	77/32	3	65/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	35/16	93/32	3	8	(30) 16d	(10) 16d	4670	9660	4235	
HHUS210-3	14	411/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670		7000
HHUS210-4	14	61/4	829/32	3	727/32	(30) 16d	(10) 16d	4670		4235	6865
HHUS46	14	3%	513/32	3	315/16	(14) 16d			10155	4235	7210
HHUS48						· · · · · · · · · · · · · · · · · · ·	(6) 16d	2540	7335	2065	5205
	14	3%	71/8	3	61/6	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3%	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	51/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	
HHUS7.25/10	14	71/4	9	35/16	729/32	(30) 16d	(10) 16d	4670			7210
l.d₀ is the dista	nce fo	om the co			ł_ · · ·]			40/0	10155	3370	7210



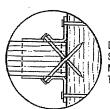


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

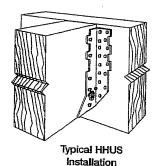
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.







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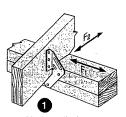
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T-SPECHHUS20 3/20 exp. 6/22

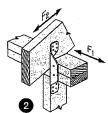
Simpson Strong-Tie Wood Construction Connectors — Canadian Limit States Design

SIMPSON Smondaile

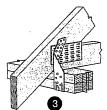
Seismic and Hurricane Ties (cont.)



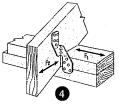
H1 Installation



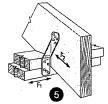
H2A Installation



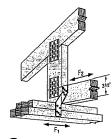
TSP Installation



H2.5A Installation (Nails into both top plates)



H2.5T Installation (Nails into both top plates)



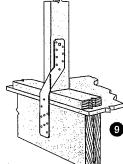
6 H2.5T Installation



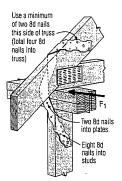
H3 Installation (Nails into upper top plate)



H6 Stud to Top Plate Installation



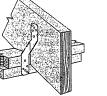
H6 Stud to **Band Joist** Installation



10 H7Z Installation



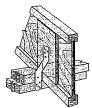
H8 Attaching Top Plates



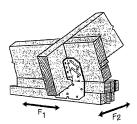
Rafter to Double



H8 attaching Stud to Sill ((4) 8d into plate, (5) 8d into stud)

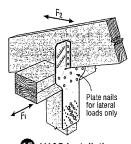


H8 attaching I-Joist to Double Top Plates

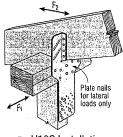


C-C-CAN2018 @2017 SIMPSON STRONG-TIE COMPANY INC.

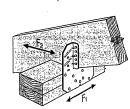
14 H10A Field-Bent Installation



15 H10S Installation

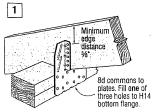


H10S Installation with Stud Offset

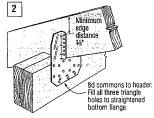


H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

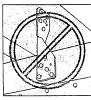


H14 Installation to Double Top Plates



H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.



H/TSP

SIMPSON Strong-Tie

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

- H10AR The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS Popular ties now available in stainless steel

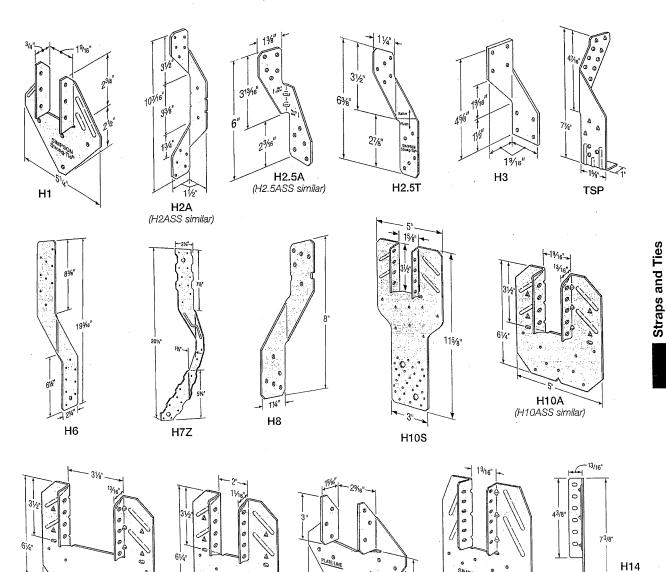
Material: See table

H10A-2

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

- · Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- · Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails.
 Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



H₁₁Z

H₁₀AR

Profile

- 3/4" -

TECHNICAL BULLETIN

H - Seismic and Hurricane Ties

SIMPSON Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge Finish: G90 galvanized

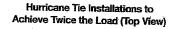
Design: • Factored resistances are in accordance with CSA 086-14

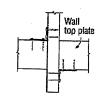
 Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0 .

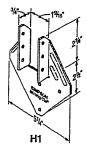


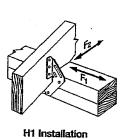


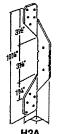


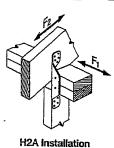
Install diagonally across from each other for minimum 2x truss.

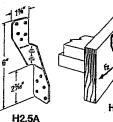
Nailing into both sides of a single ply 2x truss may cause the wood to split.



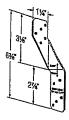


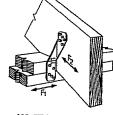


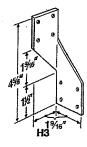


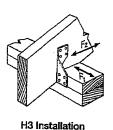


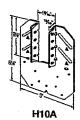


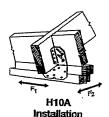








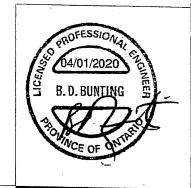




H2.5T H2.5T Installation (Nails into both top plates)

			Fasteners		Factored Resistance (lb.)							
Model	_		7 200011010			D.Fir-L		S-P-F				
No.	Ga.				Volift	Nor	mai	t Indian	Nor	mai		
	To Rafter		To Plates	To Studs	Ohing	F ₁	F ₂	Uplift	F ₁	F ₂		
						$(K_0=1.15)$			(K _n =1.15			
H1	18	(6) 8d x 11/2"	(4) 8d		740	685	300	680	485	215		
H2A	18	(5) 8d x 11/2"	(2) 8d x 1½"	(5) 8d x 11/2"	830	220	75	590	155	55		
H2.5A	18	(5) 8d	(5) 8d	_	805	160	160	755	160	160		
H2.5T	18	(5) 8d	(5) 8d	_	835	175	240	740	160	210		
НЗ	18	(4) 8d	(4) 8d		740	180	265	615	125	190		
H10A	18	(9) 10d x 11/2"	(9) 10d x 11/2"		1735	795	410	1505	565	300		

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Factored resistances are for one anchor. A
 minimum rafter thickness of 2½" must be used
 when framing anchors are installed on each side of
 the joist and on the same side of the plate.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.





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T-SPECH20 3/20 exp. 6/22

TECHNICAL BULLETIN

TC - Truss Connectors

SIMPSON Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 11/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge Finish: G90 galvanized

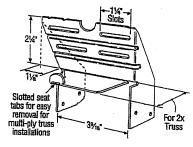
Design: Factored resistances are in accordance with CSA 086-14

Installation:

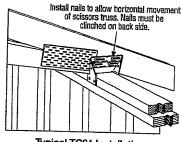
- · Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss-allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

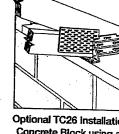
Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen° screws into the top and face of masonry wall. See optional load tables and installation details.



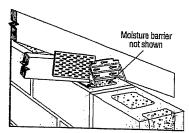
TC24 U.S. Patent 4,932,173



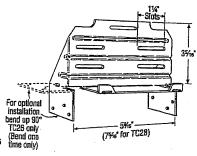
Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws



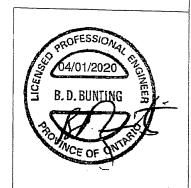
TC26 (TC28 Similiar)

	Fas	teners	Factored Resistance		
Model			D.Fir-L	S-P-F	
No.	Truss	Wall Plates	Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)	
			lb.	lb.	
TC24	(4) 10d	(4) 10d	605	430	
TC26	(5) 10d	(6) 10d	1015	720	
TC28	(5) 10d	(6) 10d	1015	720	

Optional TC Installation Table

	Fas	teners	Factored Resistance		
Model			D.Fir-L	S-P-F	
No.	Truss	Wall Plates	Uplift (K _D =1.15)	Uplift (K ₀ =1.15)	
		_	lb.	lb.	
TC26	(5) 10d	(6) 10d x 11/2"	810	660	
1020	(5) 10d	(6) 10d	930	660	

- Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
- Grout strength is 15 MPa minimum.
- Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
- 4. TC26 fastened to grouted concrete block with (6) 1/16" x 21/4" Titen screws has a factored uplift resistance of 275 lb.





This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see stronglie.com.

SIMPSON Strong-Tie

Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

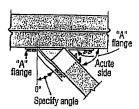
			F:	isteners .	Factored Resistance					
	Min.	Minimum		1	D.F	ir-L	S-	P-F		
Model No.	Heel Height	Header			Uplift	Normal	Uplift	Normal (Kp = 1.00)		
110.	(in.)	Size	Header	Joist	$(K_D = 1.15)$	$(K_0 = 1.00)$	$(K_0 = 1.15)$			
				1	lb.	lb.	lb.	lb.		
					kN	kN	kN	kN		
HTU26 (Min.)	37/8	(2) 2x4	(10) 16d	(14) 10d x 11/2"	1740	3340	1235	2370		
,				(11) 100 X 172	7.74	14,86	5.49	10.54		
HTU26 (Max.)	6½	(2) 2x4	(10) 16d	(20) 10d x 11/2"	2470	4015	1755	2850		
,				(20) 100 % 172	10.99	17.86	7.81	12.68		
HTU28 (Max.)	3%	(2) 2x6	(20) 16d	(26) 10d x 11/2"	4150	6395	2945	4540		
			(/	(20) 100 x 172	18.46	28.45	13.10	20.19		
HTU210 (Max.)	71/4	(2) 2x6	(20) 16d	(32) 10d x 11/2"	4150	6395	2945	4540		
		1-7 -70	(20) 100	(UE) 100 X 172	18.45	28.45	13.10	20.19		

See table footnotes on p. 260.

Hanger Options

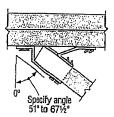
See Hanger Options Information on pp. 125–127. Skewed Seat

- Skewable up to 67½°
- · Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°

C-C-CAN2018 @2017 SIMPSON STRONG-TIE COMPANY INC.



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

		F	asteners		Factored	Resistance	
	Skew			D.	Fir-L	S-	P-F
Model	Angle		.]	Uplift	Normai	Uplift	Normal
No.	(Degrees)	Header	Joist	(KD=1.15)	(KD=1.00)	(KD=1.15)	(KD=1.00)
		İ	}	lbs	lbs	lbs	ibs
				kN	kN	kN	kN
	< 51	(20) 16d	(14) 10d x 1 1/2"	1835	4110	1300	2905
HTU26		(,	11.17.100 # 172	8.16	18.28	5.78	12.92
	51-671/2	(20) 16d	(12) 10d x 1 1/2"	1350	3620	955	2560
		()	112/104/1/2	6.01	18.10	4.25	11.39
	< 51	(26) 16d	(20) 10d x 11/4"	2810	4270	1985	3030
HTU28		(20) 100	(25) 100 x 1 /2	12.50	18.99	8.83	13.48
	51-671/2	(26) 16d	(17) 10d x 11/2"	2075	3930	1465	2780
			(117 100 X 172	9.23	17.48	6.52	12.37
	< 51	(32) 16d	(26) 10d x 1 ½"	3785	4430	2675	3135
HTU210		(,	(20) 10th 172	16.84	19.71	11.90	13,95
	51~671/2	(32) 16d	(22) 10d x 11/2"	2795	4240	1980	3000
		(,	(az) 100 x 172	12.43	18.86	8.81	13.35
	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
HTU26-2		(,	(1.7.104	9.52	16.53	6.74	11,68
	51~67½	(20) 16d	(12) 10d	1610	3920	1140	2785
		· · · · · · · · · · · · · · · · · · ·	(12) 100	7.16	17.44	5.07	12.39
	< 51	(26) 16d	(20) 10d	3960	5425	2815	3855
HTU28-2		,	1237 100	17.62	24.13	12.52	17.15
	51-671/2	(26) 16d	(17) 10d	2385	5425	1695	.3855
-		/ou	1.17 100	10.61	24.13	7.54	17.15
	< 51	(32) 16d	(26) 10d	5025	6890	3570	4890
HTU210-2		,,	(=0) 100	22.35	30.65	15.88	21.75
	51-671/2	(36) 16d	(22) 10d	3145	6680	2225	4745
		(30) 100	(45) TVU	13.99	29.72	9.90	21.10

- 1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
- 2. Reduced heel heights are not permitted for skewed HTU's.
- 3. Nails: 16d = 0.162" dia. x 3½" long, 10d x 1½" = 0.148" dia. x 1½" long, 10d = 0.148" dia. x 3" long. See pp. 27–28 for other nail sizes and information.





TECH-NOTES

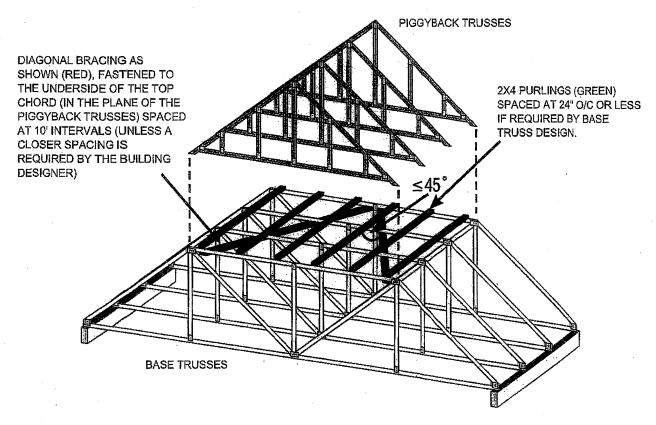
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclalmer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the technote are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this technote to offer guidance where it is not currently readily available.

Simpson Strong-Tie® Wood Construction Connectors - Canadian Limit States Design

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

SIMPSON Strong:Tie

C-C-CAN2020 @ 2020 SIMPSON STRONG-TIE COMPANY INC.

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

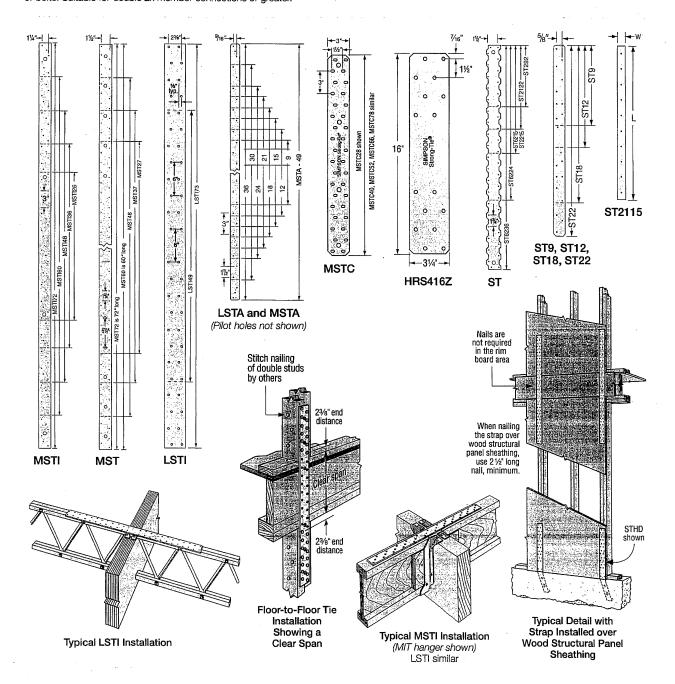
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

Installation: Use all specified fasteners; see General Notes

Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

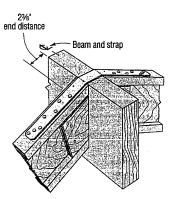


Strap Ties (cont.)

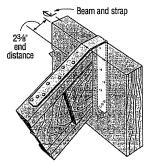
These products are available with additional corrosion protection. For more information, see p. 20.

Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

ſ				nsions				ile Resistance	
	Model		(1	in.)	Fasteners		ir-L		P-F
-	No.	Ga.			(Total)	$(K_D = 1.00)$	(K _D = 1.15)	$(K_D = 1.00)$	(K _D = 1.15)
			w	L		lb.	lb.	lb.	lb.
ŀ				·		kN	kN	kN	kN
i	LSTA9		11/4	9	(6) 10d	600	690	555	635
ŀ		-				2.67	3.07	2.47	2.82
	LSTA12		11/4	12	(8) 10d	800 3.56	920 4.09	735 3.27	845 3.76
ŀ		-				1000	1150	920	1060
	LSTA15	.	11/4	15	(10) 10d	4.45	5.12	4.09	4.72
ŀ		-				1200	1380	1105	1270
	LSTA18		11/4	18	(12) 10d	5.34	6.14	4.92	5.65
t		ľ		64	40.401	1400	1610	1290	1485
	LSTA21	- 00	11⁄4	21	(14) 10d	6.23	7.16	5.74	6.61
Ī	LCTAGA	20	11/4	24	(16) 104	1600	1840	1475	1695
-	LSTA24		1 74	24	(16) 10d	7.12	8.19	6.56	7.54
ſ	ST292		21/1s	95/16	(8) 8d	585	675	535	615
L	31292		2715	3716	(0) 00	2.60	3.00	2.38	2.74
	ST2122		21/16	12 ¹³ /15	(12) 8d	940	1085	865	995
ļ	012122		2710	12 /10	(12) 00	4.18	4.83	3.85	4.43
	ST2115		3/4	165/16	(8) 8d	670	770	615	710
-					(-,	2.98	3.43	2.74	3.16
ı	ST2215		21/16	165/16	(16) 8d	1335	1540	1235	1420
-						5.94	6.85	5.49	6.32
	LSTA30	*	11/4	30	(20) 10d	2235	2465	2075	2385
}		1				9.94 2465	10.97 2465	9.23 2465	10.61 2465
1	LSTA36		11/4	36	(24) 10d	10.97	10.97	10.97	10.97
ŀ			· · · · · · · · · · · · · · · · · · ·			3115	3580	2852	3280
	LSTI49		3¾	49	(32) 10d x 11/2"	13.86	15.93	12.69	14.59
+					<u> </u>	4670	5370	4280	4920
ļ	LSTI73		3¾	73	(48) 10d x 11/2"	20.77	23.89	19.04	21.89
-					(2) (2)	670	770	625	715
	MSTA9	40	11/4	9	(6) 10d	2.98	3.43	2.78	3.18
. [MOTALO	18	41/	12	(O) 10d	895	1030	830	955
•	MSTA12		11/4	12	(8) 10d	3.98	4.58	3.69	4.25
	MSTA15		11/4	15	(10) 10d	1120	1285	1040	1195
	WIGHTIO		1 /4	19	(10) 100	4.98	5.72	4.63	5.32
.	MSTA18		11/4	18	(12) 10d	1340	1545	1245	1430
۱ ً					(,	5.96	6.87	5.54	6.36
	MSTA21		11/4	21	(14) 10d	1565	1800	1455	1670
						6.96 1790	8.01 2060	6.47 1660	7.43 1910
3	MSTA24		11/4	24	(16) 10d	7.96	9.16	7.38	8.50
					 	2470	2840	2260	2595
	MSTA30		11/4	30	(20) 10d	10.99	12.63	10.05	11.54
		1			40.11.11	2965	3070	2710	3070
3	MSTA36		11/4	36	(24) 10d	13.19	13.66	12.06	13.66
	MOTA 40	1	417	40	(00) 04	2725	2725	2545	2725
	MSTA49		11/4	49	. (28) 8d	12.12	12.12	11.32	12.12
	CTC015		91/-	165/-	(16) 04	1405	1615	1300	1500
	ST6215		21/16	165/16	(16) 8d	6.25	7.18	5.78	6.67
	ST6224	16	21/16	235/16	(24) 8d	2305	2650	2155	2475
	010227	, ,	2/10	20/10	(24) 00	10.25	11.79	9.59	11.01
	ST9		11/4	9	(6) 8d	525	605	490	560
	5.5			ļ	1	2.34	2.69	2.18	2.49
	ST12		11/4	11%	(8) 8d	700	805	650	750
		-		-	''	3.11	3.58	2.89	3.34
	ST18		11/4	173/4	(12) 8d	1050	1210 5.38	975 4.34	1125 5.00
		-		-		4.67 1580	5.38 1790	1465	1685
		1 .	-11/4	21%	(18) 8d	1000	1 1/50	1400	1 1000



Typical LSTA Installation (hanger not shown) Bend strap one time only



Typical LSTA Installation (hanger not shown) Bend strap one time only

- 1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- 2. Use half of the nails in each member being connected to achieve the listed resistances.
- 3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.

Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

		Dimer	nsions	Ĺ	Factored Tensile Resistance					
		(iı	1.)		D.F	ir–L	S-I	P-F		
Model No.	Ga.			Fasteners (Total)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$		
NO.		W	L	(1014)	lb.	lb.	lb.	tb.		
				Ī	kN	kN	kN	kN		
			001/	(00) 104	3955	4545	3615	4155		
MSTC28		. 3	281/4	(32) 10d	17.59	20.22	16.08	18.48		
	1 [401/	(40) 104	5930	6820	5420	6235		
MSTC40	16	3	401/4	(48) 10d	26.38	30.34	24.11	27.74		
			501/	(EA) 10d	6670	6940	6100	6940		
MSTC52		3	521/4	(54) 10d	29.67	30.87	27.14	30.87		
A POTOGO		_	053/	(CC) 10d	8515	8565	7455	8565		
MSTC66		3	65%	(66) 10d	37.88	38.10	33.16	38.10		
1107070	1 [773/	(SS) 10d	8515	8565	7455	8565		
MSTC78	14	3	77%	(66) 10d	37.88	38.10	33.16	38.10		
	1 1	044	00127	(00) 04	3735	4295	3270	3760		
ST6236	236 21/16	3313/16	(36) 8d	16.61	19.11	14.55	16.73			
		01/		(00) 101 11/1	2825	3250	2475	2850		
MSTI26		21/16	26	(22) 10d x 1 ½"	12.57	14.46	11.01	12.68		
	1 1			(00) 40 1 41/1	4110	4725	3600	4140		
MSTI36		21/16	36	(32) 10d x 1½"	18.28	21.02	16.01	18.42		
	1 1		10	(44) 40 1 41/8	5650	6500	4955	5695		
MSTI48		21/16	48	(44) 10d x 1 ½"	25.13	28.91	22.04	25.33		
		241		(50) 40 4 4 1/1	7195	7360	6305	7250		
MSTI60		21/16	60	(56) 10d x 1 ½"	32.01	32.74	28.05	32.25		
	1 .	01/	70	(CO) 104 × 11/8	7360	7360	7240	7360		
MSTI72	12	21/16	72	(68) 10d x 1 ½"	32.74	32.74	32.21	32.74		
		04/	0.7	(00) 84	2685	3090	2355	2710		
MST27		21/16	27	(26) 8d	11.94	13.75	10.48	12.06		
MOTOT	1	017	071/	(30) 24	3930	4515	3440	3960		
MST37		21/16	371/2	(38) 8d	17.48	20.08	15.30	17.62		
HOTIO		01/	40	(EO) 0d	5170	5945	4530	5210		
MST48		21/16	48	(50) 8d	23.00	26.45	20.15	23.18		
11004407	1	01/	10	(16) 1/8 × 11/8 CDC	2400	2760	2120	2440		
нк5416∠	RS416Z 31/4	16	(16) 1/4" x 11/2" SDS	10.68	12.28	9.43	10.85			
MOTOO		01/		(CA) 0d	6620	7610	5800	6670		
MST60		21/16	60	(64) 8d	29.45	33.85	25.80	29.67		
MOTTO	10	01/	70	(70) 0d	8065	9135	7065	8125		
MST72		21/16	72	. (78) 8d	35.88	40.64	31.43	36.14		

Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

36.14

23/4" end distance

G. V. LUBARSKY
100044463

Typical MSTL Installation (MIT hanger shown)
LSTI similar

Straps and Ties

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Use half of the nails in each member being connected to achieve the listed resistances.

^{3.} Nails: 10d = 0.148" dia. \times 3" long, 10d \times 1½" = 0.148" dia. \times 1½" long, 8d = 0.131" dia. \times 2½" long. See pp. 22–23 for other nail sizes and information.

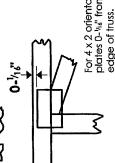
EW

Symbols

PLATE LOCATION AND ORIENTATION



Dimensions are in ff-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth. Center plate on joint unless x, y offsets are indicated.



For 4 x 2 orientation, locate plates 0-1/14" from outside edge of truss.

required direction of slots in connector plates. This symbol indicates the

*Plate location details available in MiTek software or upon request

PLATE SIZE

4 4 ×

width measured perpendicular to slots. Second dimension is The first dimension is the plate the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated

BEARING



indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

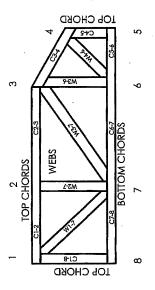
industry Standards:

for Light Metal Plate Connected Wood Trusses Truss Design Procedures and Specifications <u>::</u>

Design Standard for Bracing.
Bullding Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses. DSB-89: BCSI:

Numbering System

dimensions shown in ft-in-sixteenths or mm (Drawings not to scale) 6-4-8



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MITek Engineering Reference Sheet: MII-7473C rev. 10-'08 POWER 10 PERFORM."

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide furse spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building 4
- Cut members to bear tightly against each other. 'n
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC. ٥,
- Design assumes trusses will be suitably protected from the environment in accord with TPIC 7
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others. 16. Do not cut or alter truss member or plate without prior
 - Install and load vertically unless indicated otherwise. approval of an engineer.
- 18. Use of green or treated lumber may pose unacceptable environmental, health ar performance risks. Consult with project engineer before use.
- 19. Review all partions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with TPIC Quality Criteria.