All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

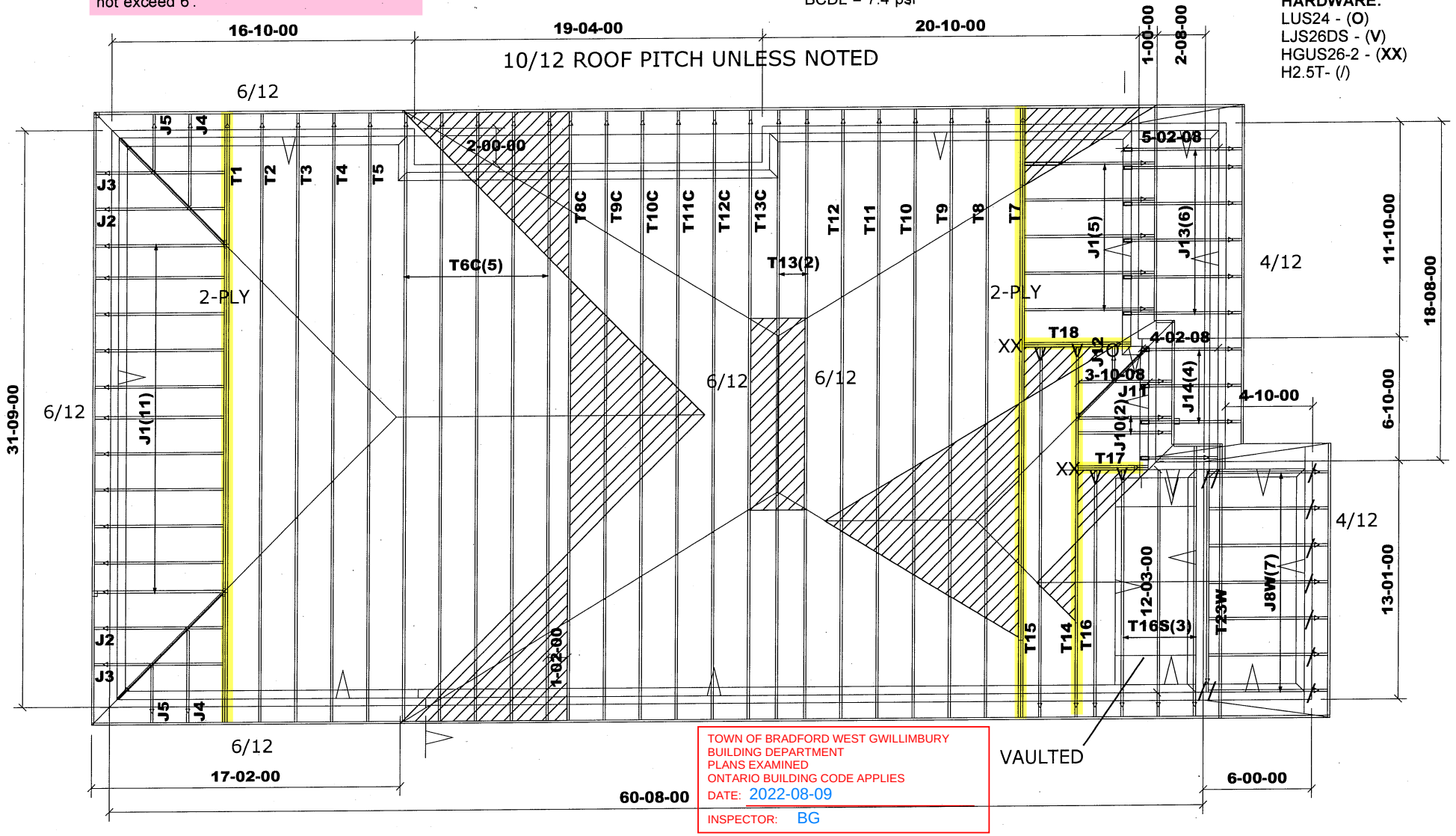
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T- (/)



TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2022-08-09
INSPECTOR: BG

TAMARACK ROOF TRUSSES INC.

Job Track: 50465
Plan Log: 205567
Layout ID: 423565

Builder / Location: BAYVIEW WELLINGTON / BRADFORD
Project: GREEN VALLEY EAST
Date: 2022-06-28 | Sales: Rick DiCiano | Designer: JG

Model / Elevation: S38-20 / A-STD. OR 5 BED (NO COFF)

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.5.3.233

REVIEWED

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

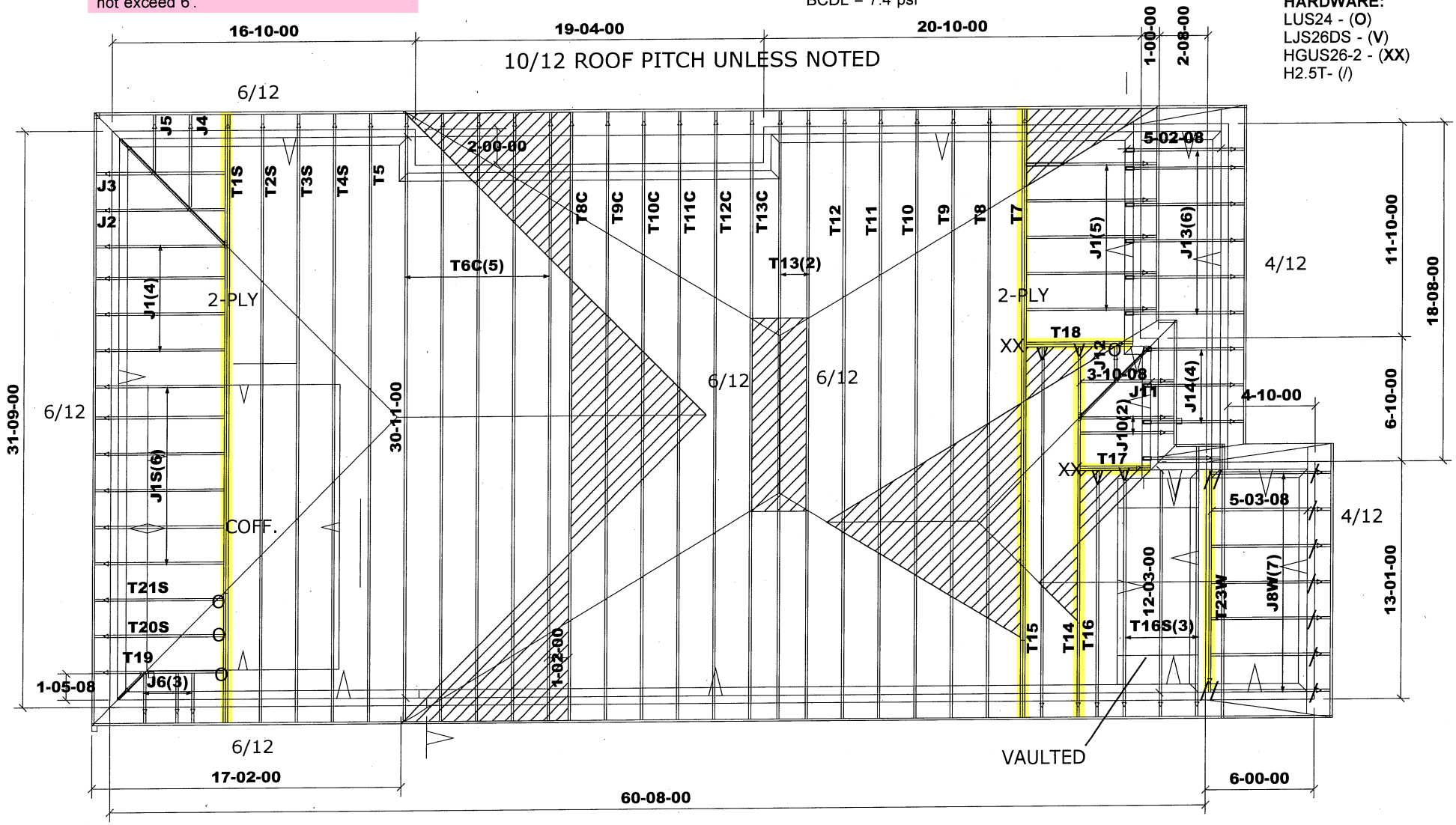
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
H2.5T- (/)



TAMARACK ROOF TRUSSES INC.	Job Track: 50465	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-20 / A-OPT.WITH COFF.
	Plan Log: 205567	Project: GREEN VALLEY EAST	
	Layout ID: 423564	Date: 2022-06-28 Sales: Rick DiCiano Designer: JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.5.3.233

REVIEWED

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

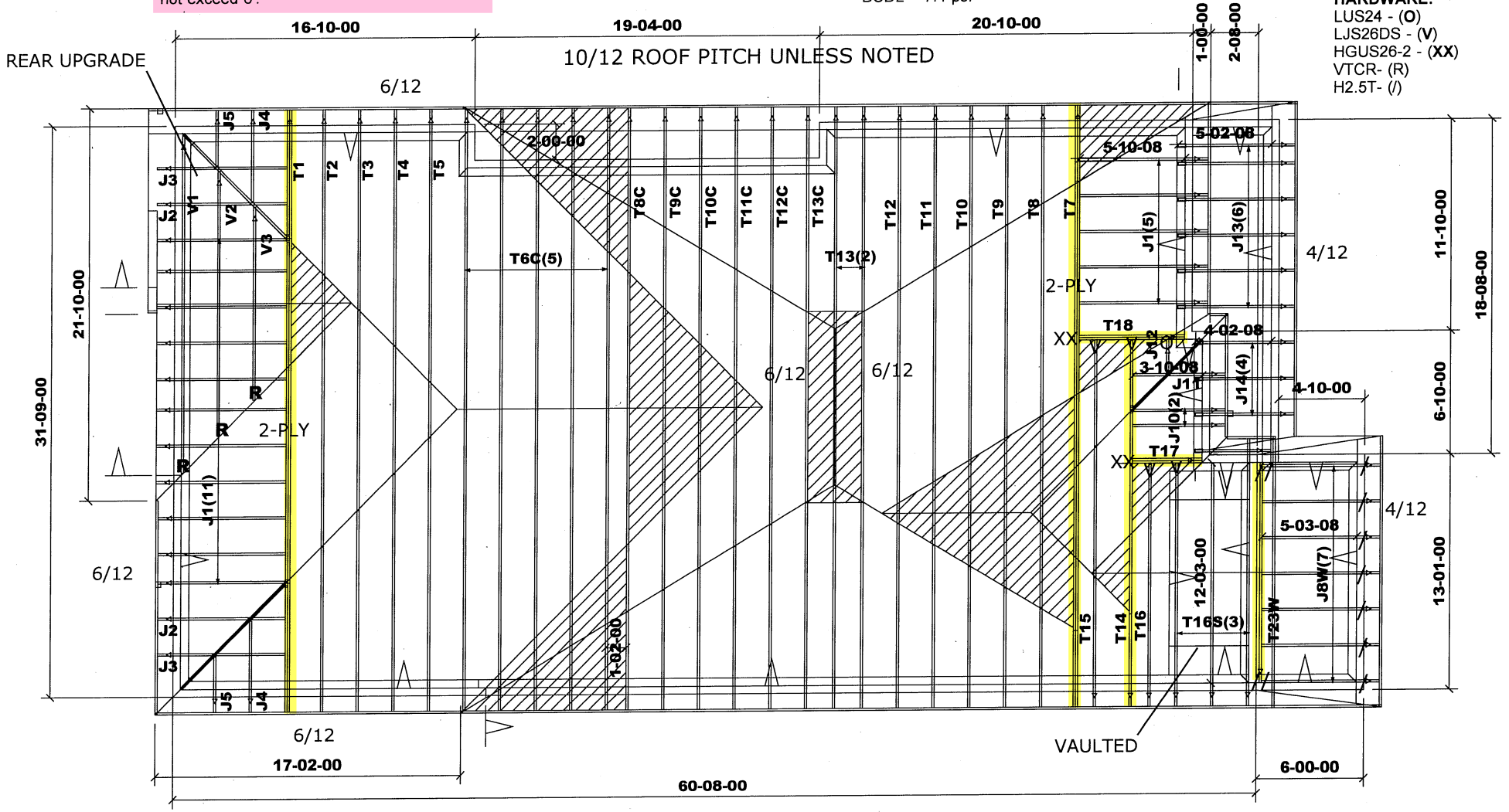
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
VTCR- (R)
H2.5T- (/)



TAMARACK ROOF TRUSSES INC.	Job Track: 50465	Builder / Location: BAYVIEW WELLINGTON / BRADFORD		Model / Elevation: S38-20 / A-REAR UPGRADE
	Plan Log: 205567	Project: GREEN VALLEY EAST		
	Layout ID: 423566	Date: 2022-06-28	Sales: Rick DiCiano	Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.3.233

REVIEWED

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

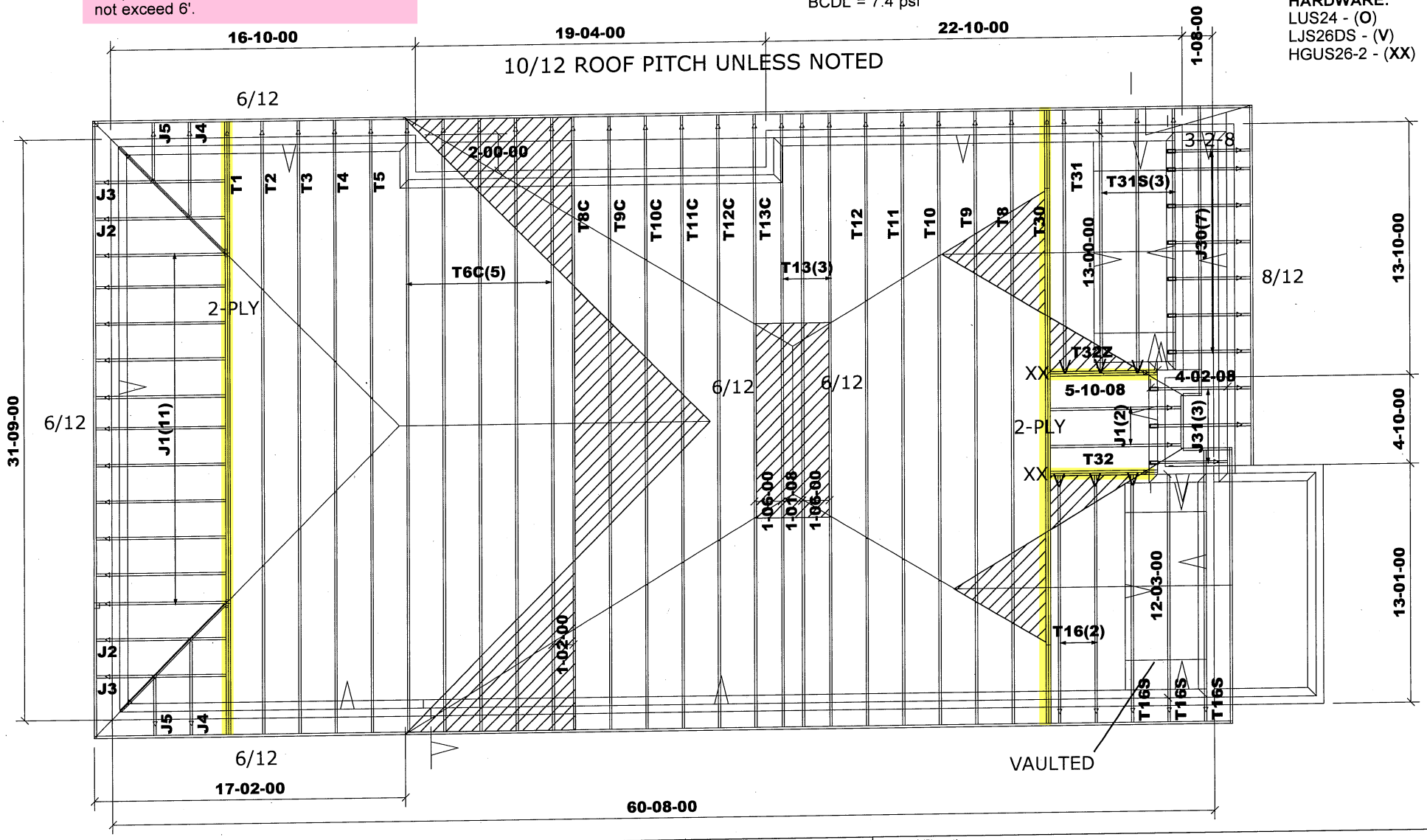
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)



TAMARACK ROOF TRUSSES INC.	Job Track: 50465	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-20 / B-STD OR 5 BED (NO COFF)
	Plan Log: 205567	Project: GREEN VALLEY EAST	
	Layout ID: 423568	Date: 2022-06-28 Sales: Rick DiCiano Designer: JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.5.3.233

REVIEWED

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

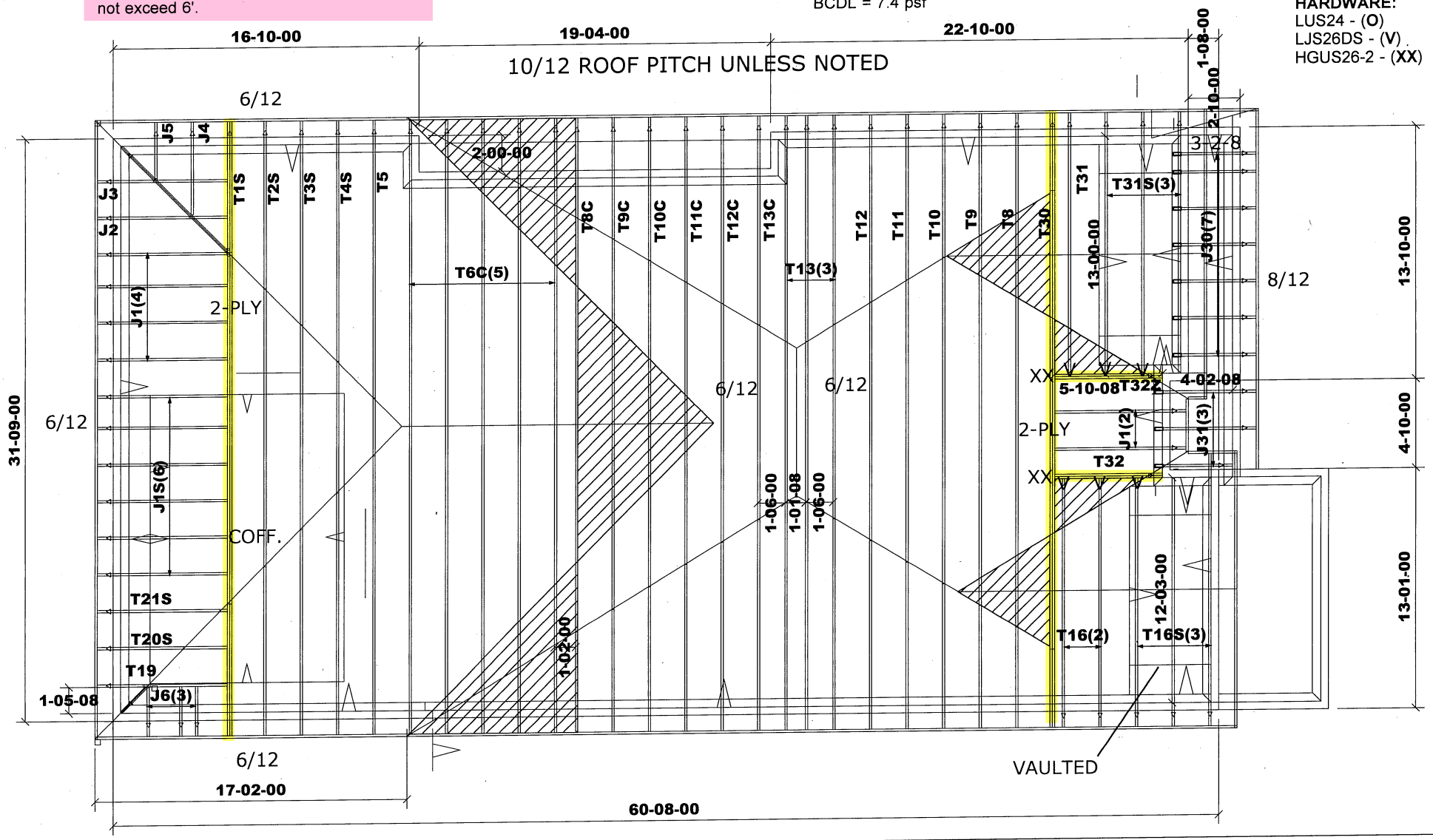
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)



TAMARACK ROOF TRUSSES INC.	Job Track: 50465	Builder / Location: BAYVIEW WELLINGTON / BRADFORD		Model / Elevation: S38-20 / B-OPT.WITH COFF.
	Plan Log: 205567	Project: GREEN VALLEY EAST		
	Layout ID: 423567	Date: 2022-06-28	Sales: Rick DiCiano	Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.5.3.233

REVIEWED

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

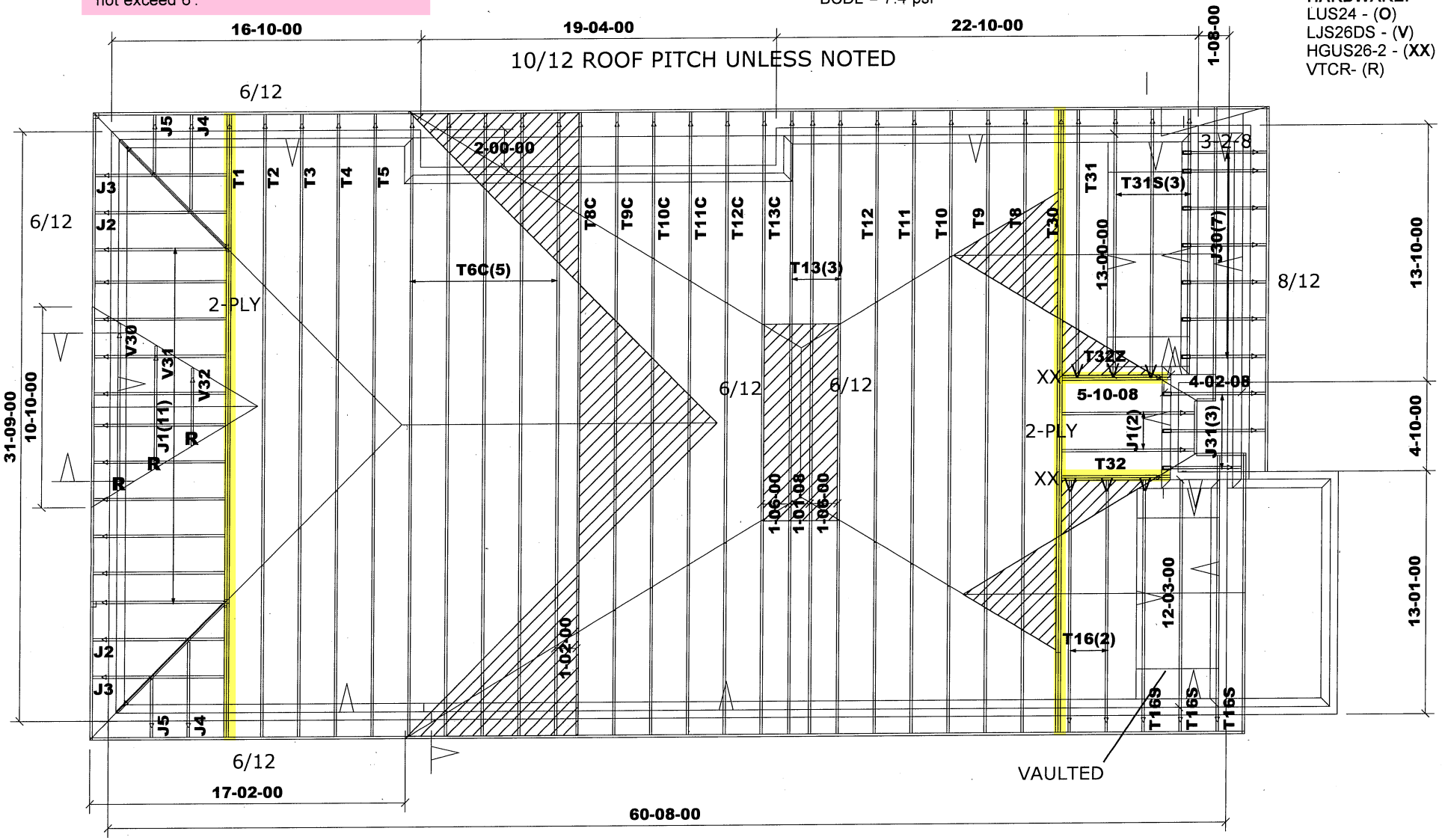
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
VTCR- (R)



TAMARACK ROOF TRUSSES INC.	Job Track: 50465	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-20 / B-REAR UPGRADE
	Plan Log: 205567	Project: GREEN VALLEY EAST	
	Layout ID: 423569	Date: 2022-06-28 Sales: Rick DiCiano Designer: JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

REVIEWED

Mitek ver 8.5.3.233

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

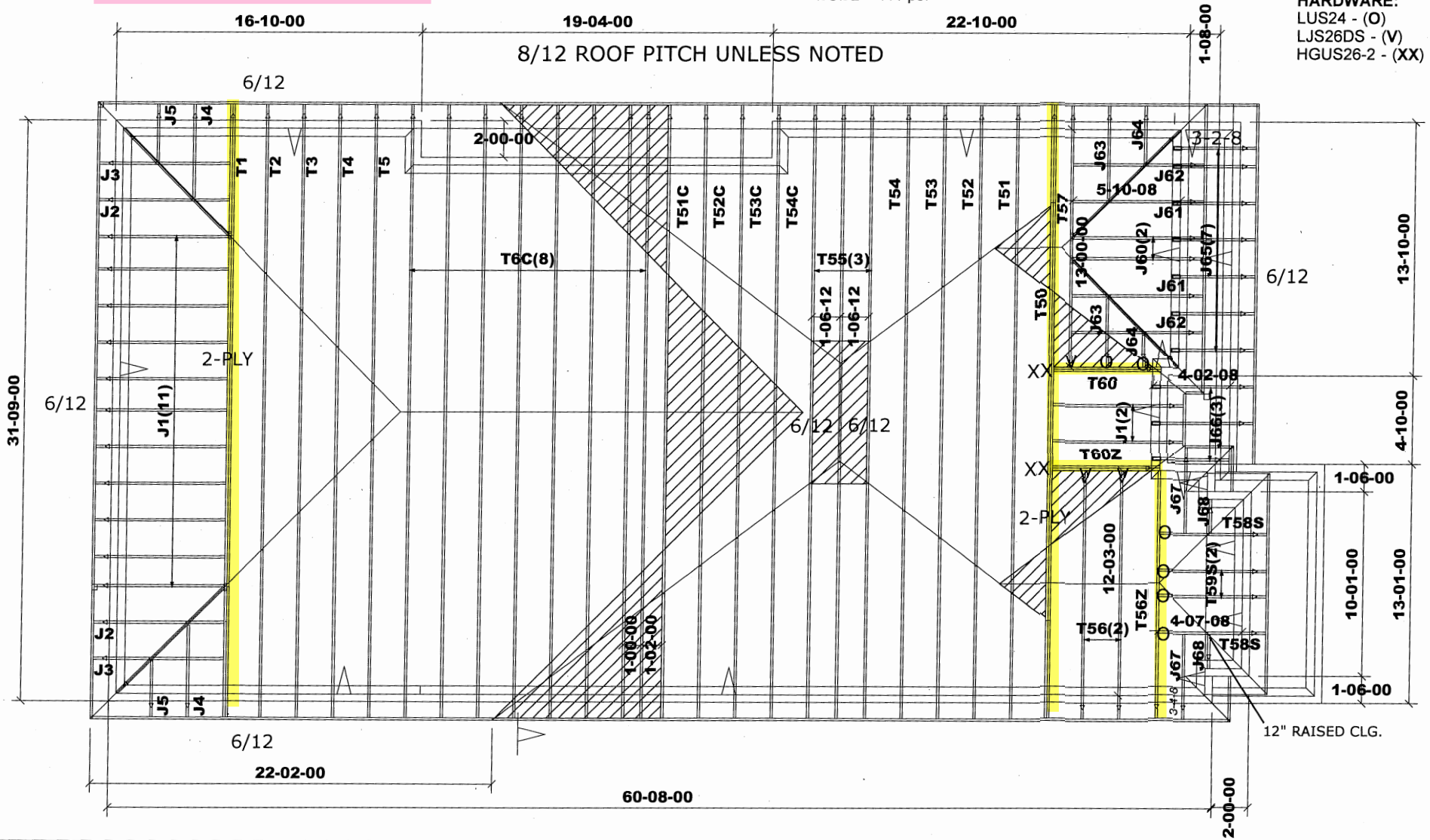
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)



TAMARACK ROOF TRUSSES INC.	Job Track: 50465	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-20 / C-STD OR 5 BED(NOCOFF)	
	Plan Log: 205567	Project: GREEN VALLEY EAST		
	Layout ID: 423571	Date: 2022-06-28	Sales: Rick DiCiano	Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

REVIEWED

Mitek ver 8.5.3.233

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

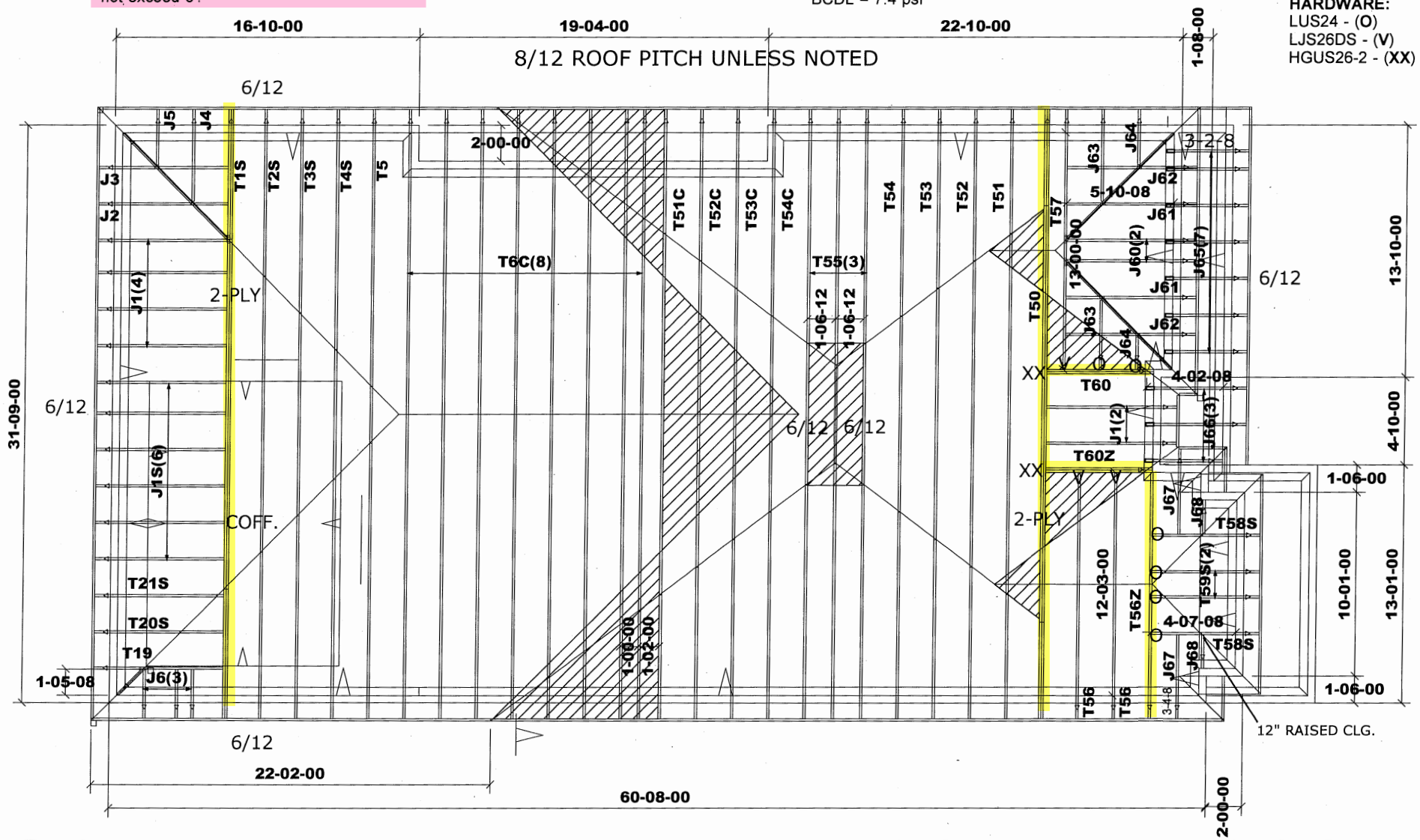
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)



TAMARACK ROOF TRUSSES INC.	Job Track: **50465**	Builder / Location: **BAYVIEW WELLINGTON / BRADFORD**	Model / Elevation: **S38-20 / C-OPT.WITH COFF**
	Plan Log: **205567**	Project: **GREEN VALLEY EAST**	
	Layout ID: **423570**	Date: **2022-06-28** Sales: Rick DiCiano Designer: JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.5.3.233

REVIEWED

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

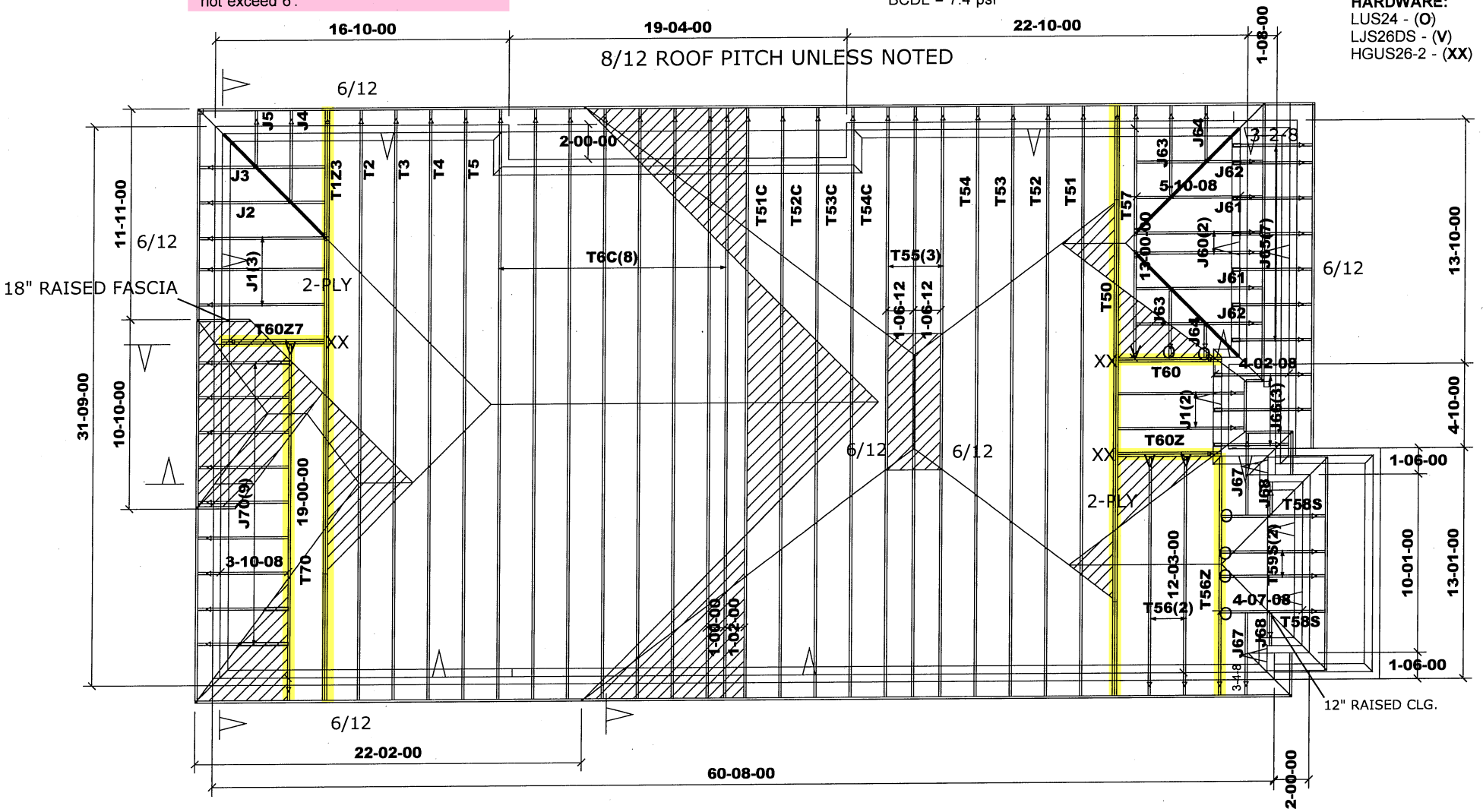
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)



TAMARACK ROOF TRUSSES INC.	Job Track: 50465	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-20 / C-REAR UPGRADE
	Plan Log: 205567	Project: GREEN VALLEY EAST	
	Layout ID: 423572	Date: 2022-06-28 Sales: Rick DiCiano Designer: JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.5.3.233

REVIEWED

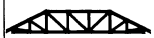
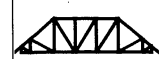
TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	GREEN VALLEY EAST
Location:	BRADFORD
Model:	S38-20
Lot #:	
Elevation:	A-STD. OR 5 BED (NO COFF)

Job Track:	50465
PlanLog:	205567
Layout ID:	423565
Ref #	
Page:	1 of 3
Date:	06-28-2022
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	**30-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	277.92 172.67		
	1	T2 Hip	6 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.38 79.83		
	1	T3 Hip	6 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.27 79.33		
	1	T4 Hip	6 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.71 79.33		
	1	T5 Hip	6 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.41 84.17		
	5	T6C Common	6 /12	**30-11-00**	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	629.51 392.50		
	1 2-ply	T7 Hip Girder	10 /12	**30-11-00**	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	341.08 206.67		
	1	T8 Hip	10 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T8C Hip	10 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T9 Hip	10 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T9C Hip	10 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T10 Hip	10 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T10C Hip	10 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T11 Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		

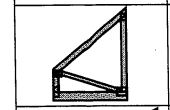
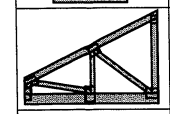
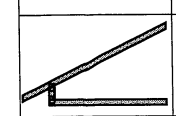
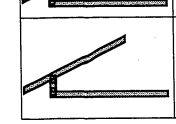
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423565
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	2 of 3
Lot #:		Date:	06-28-2022
Elevation:	A-STD. OR 5 BED (NO COFF)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T11C Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		
	1	T12 Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	1	T12C Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	2	T13 Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	341.66 214.67		
	1	T13C Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	170.83 107.33		
	1	T14 Hip Girder	10 /12	**19-01-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	102.49 65.33		
	1	T15 Hip	10 /12	**19-01-00**	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	90.06 57.67		
	1	T16 Common	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.1 38.17		
	3	T16S Roof Special	10 /12 10 /12	**12-03-00**	6-08-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	182.61 118.50		
	1 2-ply	T17 Jack-Closed Girder	10 /12	**3-10-08**	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1 2-ply	T18 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T23W Flat Girder	0 /12	**12-03-00**	2-01-02	2 x 6		2-01-02 2-01-02	58.33 39.00		
	16	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	2	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		

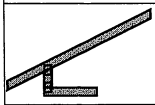
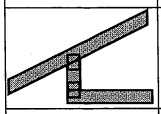
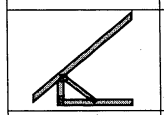
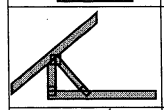
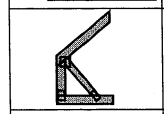
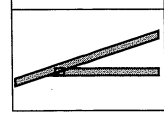
REVIEWED

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423565
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	3 of 3
Lot #:		Date:	06-28-2022
Elevation:	A-STD. OR 5 BED (NO COFF)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	7	J8W Jack-Open	4 /12	**5-03-08**	2-06-00	2 x 4	1-03-08	3-15 2-01-02	98.41 65.33		
	2	J10 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	30.9 20.33		
	1	J11 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J12 Jack-Open	10 /12	**1-10-08**	3-01-09	2 x 4		1-07-11 3-02-07	7.79 5.67		
	6	J13 Jack-Open	4 /12	**5-02-08**	2-05-11	2 x 4	1-03-08	3-15 2-00-12	83.18 52.00		
	4	J14 Jack-Open	4 /12	**4-02-08**	2-01-11	2 x 4	1-03-08	3-15 1-08-12	46.08 29.33		

TOTAL # TRUSS= **82** TOTAL BFT OF ALL TRUSSES= **3154.81** BFT. TOTAL WEIGHT OF ALL TRSSES **4999.03** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
7	Hardware	H2.5T	
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
1	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **14**

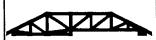
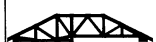
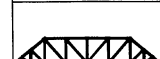
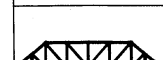
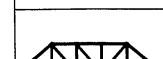
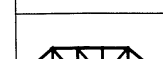
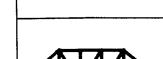
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423564
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	1 of 4
Lot #:		Date:	06-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	6 /12	**30-11-00**	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	293.17 190.00		
	1	T2S Hip	6 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.39 87.00		
	1	T3S Hip	6 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	135 87.83		
	1	T4S Hip	6 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	138.98 88.50		
	1	T5 Hip	6 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	136.86 86.17		
	5	T6C Common	6 /12	**30-11-00**	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	629.51 392.50		
	1 2-ply	T7 Hip Girder	10 /12	**30-11-00**	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	347.72 214.00		
	1	T8 Hip	10 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T8C Hip	10 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T9 Hip	10 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T9C Hip	10 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T10 Hip	10 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T10C Hip	10 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T11 Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		

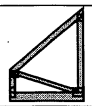
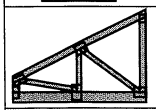
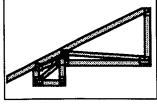
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423564
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	2 of 4
Lot #:		Date:	06-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T11C Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		
	1	T12 Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	1	T12C Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	2	T13 Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	341.66 214.67		
	1	T13C Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	170.83 107.33		
	1	T14 Hip Girder	10 /12	**19-01-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	102.49 65.33		
	1	T15 Hip	10 /12	**19-01-00**	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	90.06 57.67		
	1	T16 Common	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.1 38.17		
	3	T16S Roof Special	10 /12 10 /12	**12-03-00**	6-08-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	182.61 118.50		
	1 2-ply	T17 Jack-Closed Girder	10 /12	**3-10-08**	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1 2-ply	T18 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T19 Half Hip Girder	6 /12	**5-10-08**	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	23.24 16.50		
	1	T20S Half Hip	6 /12	**5-10-08**	2-10-12	2 x 4	1-03-08	1-02-00 1-10-12	26.14 19.50		
	1	T21S Half Hip	6 /12	**5-10-08**	3-10-12	2 x 4	1-03-08	1-02-00 2-10-12	26.46 18.67		

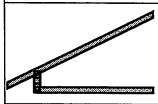
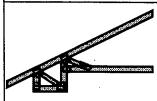
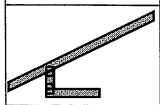
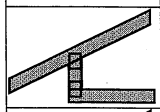
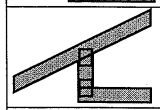
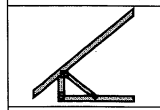
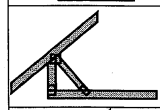
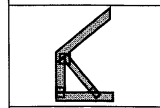
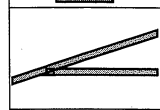
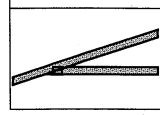
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423564
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	3 of 4
Lot #:		Date:	06-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T23W Flat Girder	0 /12	**12-03-00**	2-01-02	2 x 6		2-01-02 2-01-02	58.33 39.00		
	9	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	6	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	122.34 88.00		
	1	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	1	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	3	J6 Jack-Open	6 /12	**1-05-08**	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	18.38 14.00		
	7	J8W Jack-Open	4 /12	**5-03-08**	2-06-00	2 x 4	1-03-08	3-15 2-01-02	98.41 65.33		
	2	J10 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	30.9 20.33		
	1	J11 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J12 Jack-Open	10 /12	**1-10-08**	3-01-09	2 x 4		1-07-11 3-02-07	7.79 5.67		
	6	J13 Jack-Open	4 /12	**5-02-08**	2-05-11	2 x 4	1-03-08	3-15 2-00-12	83.18 52.00		
	4	J14 Jack-Open	4 /12	**4-02-08**	2-01-11	2 x 4	1-03-08	3-15 1-08-12	46.08 29.33		

REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423564
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	4 of 4
Lot #:		Date:	06-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **83** TOTAL BFT OF ALL TRUSSES= **3261.65** BFT. TOTAL WEIGHT OF ALL TRSSES **5107.08** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
1	Hardware	LUS24	
7	Hardware	H2.5T	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **14**

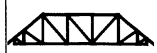
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423566
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	1 of 4
Lot #:		Date:	06-28-2022
Elevation:	A-REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	30-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	277.92 172.67		
	1	T2 Hip	6 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.38 79.83		
	1	T3 Hip	6 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.27 79.33		
	1	T4 Hip	6 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.71 79.33		
	1	T5 Hip	6 /12	30-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	136.86 86.17		
	5	T6C Common	6 /12	30-11-00	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	629.51 392.50		
	1 2-ply	T7 Hip Girder	10 /12	30-11-00	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	341.08 206.67		
	1	T8 Hip	10 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T8C Hip	10 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T9 Hip	10 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T9C Hip	10 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T10 Hip	10 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T10C Hip	10 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T11 Hip	10 /12	30-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		

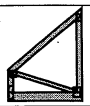
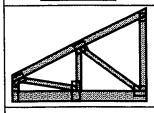
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423566
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	2 of 4
Lot #:		Date:	06-28-2022
Elevation:	A-REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T11C Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		
	1	T12 Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	1	T12C Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.8 90.83		
	2	T13 Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	341.66 214.67		
	1	T13C Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.82 103.67		
	1	T14 Hip Girder	10 /12	**19-01-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	102.49 65.33		
	1	T15 Hip	10 /12	**19-01-00**	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	90.06 57.67		
	1	T16 Common	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.1 38.17		
	3	T16S Roof Special	10 /12 10 /12	**12-03-00**	6-08-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	182.61 118.50		
	1 2-ply	T17 Jack-Closed Girder	10 /12	**3-10-08**	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1 2-ply	T18 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T23W Flat Girder	0 /12	**12-03-00**	2-01-02	2 x 6		2-01-02 2-01-02	58.33 39.00		
	1	V1 Valley	6 /12	**18-09-00**	4-08-04	2 x 4			57.29 37.50		
	1	V2 Valley	6 /12	**14-09-00**	3-08-04	2 x 4			38.27 24.67		

REVIEWED

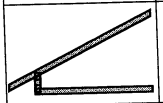
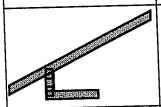
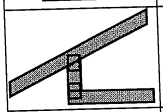
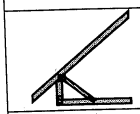
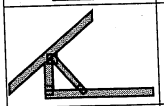
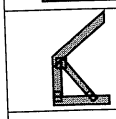
DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	GREEN VALLEY EAST
Location:	BRADFORD
Model:	S38-20
Lot #:	
Elevation:	A-REAR UPGRADE

Job Track:	50465
PlanLog:	205567
Layout ID:	423566
Ref #	
Page:	3 of 4
Date:	06-28-2022
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	V3 Valley	6 /12	10-09-00	2-08-04	2 x 4			25.97 16.83		
	16	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	2	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	7	J8W Jack-Open	4 /12	5-03-08	2-06-00	2 x 4	1-03-08	3-15 2-01-02	98.41 65.33		
	2	J10 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	30.9 20.33		
	1	J11 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J12 Jack-Open	10 /12	1-10-08	3-01-09	2 x 4		1-07-11 3-02-07	7.79 5.67		
	6	J13 Jack-Open	4 /12	5-02-08	2-05-11	2 x 4	1-03-08	3-15 2-00-12	83.18 52.00		
	4	J14 Jack-Open	4 /12	4-02-08	2-01-11	2 x 4	1-03-08	3-15 1-08-12	46.08 29.33		

TOTAL # TRUSS= **85** TOTAL BFT OF ALL TRUSSES= **3228.48** BFT. TOTAL WEIGHT OF ALL TRSSES **5110.15** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
7	Hardware	H2.5T	
2	Hardware	HGUS26-2	

REVIEWED

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423566
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	4 of 4
Lot #:		Date:	06-28-2022
Elevation:	A-REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
1	Hardware	LUS24	
3		VTCR	

TOTAL NUMBER OF ITEMS= **17**

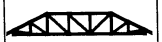
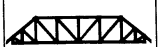
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423568
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	1 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-STD OR 5 BED (NO COFF.)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	30-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	277.92 172.67		
	1	T2 Hip	6 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.38 79.83		
	1	T3 Hip	6 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.27 79.33		
	1	T4 Hip	6 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.71 79.33		
	1	T5 Hip	6 /12	30-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.41 84.17		
	5	T6C Common	6 /12	30-11-00	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	629.51 392.50		
	1	T8 Hip	10 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T8C Hip	10 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T9 Hip	10 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T9C Hip	10 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T10 Hip	10 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T10C Hip	10 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T11 Hip	10 /12	30-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		
	1	T11C Hip	10 /12	30-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		

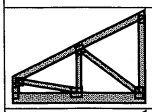
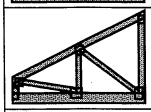
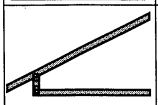
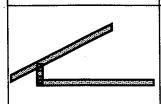
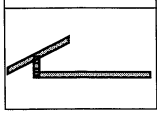
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423568
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	2 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-STD OR 5 BED (NO COFF.)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T12 Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	1	T12C Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	3	T13 Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	512.48 322.00		
	1	T13C Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	170.83 107.33		
	2	T16 Common	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.2 76.33		
	3	T16S Roof Special	10 /12 10 /12	**12-03-00**	6-08-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	182.61 118.50		
	1 2-ply	T30 Hip Girder	10 /12	**30-11-00**	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	339.47 208.00		
	1	T31 Common	10 /12	**13-00-00**	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	62.1 39.50		
	3	T31S Roof Special	10 /12 10 /12	**13-00-00**	7-00-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	190.97 124.00		
	1 2-ply	T32 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T32Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	13	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	218.33 138.67		
	2	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		

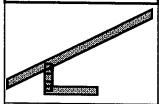
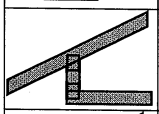
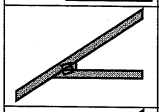
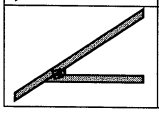
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423568
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	3 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-STD OR 5 BED (NO COFF.)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	7	J30 Jack-Open	8 /12	**3-02-08**	3-04-03	2 x 4	1-03-08	4-07 2-06-02	73.82 51.33		
	3	J31 Jack-Open	8 /12	**4-02-08**	4-00-03	2 x 4	1-03-08	4-07 3-02-02	39.17 26.00		

TOTAL # TRUSS= **71** TOTAL BFT OF ALL TRUSSES= **3175.14** BFT. TOTAL WEIGHT OF ALL TRSSES **5028.26** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **8**

REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423567
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	1 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-OPT.WITH COFF.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	6 /12	**30-11-00**	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	293.17 190.00		
	1	T2S Hip	6 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.39 85.67		
	1	T3S Hip	6 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	135 87.83		
	1	T4S Hip	6 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	138.87 89.83		
	1	T5 Hip	6 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	136.86 86.17		
	5	T6C Common	6 /12	**30-11-00**	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	629.51 392.50		
	1	T8 Hip	10 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T8C Hip	10 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T9 Hip	10 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T9C Hip	10 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T10 Hip	10 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T10C Hip	10 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T11 Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		
	1	T11C Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		

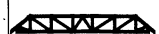
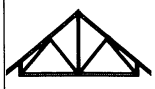
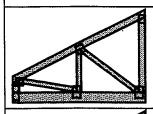
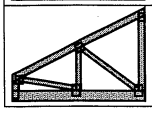
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423567
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	2 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-OPT.WITH COFF.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T12 Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	1	T12C Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.8 90.83		
	3	T13 Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	512.48 322.00		
	1	T13C Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.82 103.67		
	2	T16 Common	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.2 76.33		
	3	T16S Roof Special	10 /12 10 /12	**12-03-00**	6-08-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	182.61 118.50		
	1	T19 Half Hip Girder	6 /12	**5-10-08**	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	23.24 16.50		
	1	T20S Half Hip	6 /12	**5-10-08**	2-10-12	2 x 4	1-03-08	1-02-00 1-10-12	25.02 18.50		
	1	T21S Half Hip	6 /12	**5-10-08**	3-10-12	2 x 4	1-03-08	1-02-00 2-10-12	25.33 17.67		
	1 2-ply	T30 Hip Girder	10 /12	**30-11-00**	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	375.14 226.67		
	1	T31 Common	10 /12	**13-00-00**	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	62.09 39.50		
	3	T31S Roof Special	10 /12 10 /12	**13-00-00**	7-00-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	196.5 128.00		
	1 2-ply	T32 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T32Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		

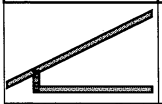
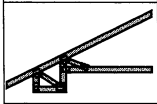
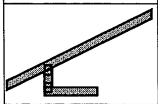
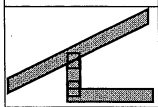
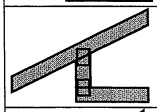
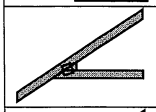
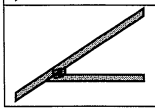
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423567
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	3 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-OPT.WITH COFF.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	6	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		
	6	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	122.34 88.00		
	1	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	1	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	3	J6 Jack-Open	6 /12	**1-05-08**	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	18.38 14.00		
	7	J30 Jack-Open	8 /12	**3-02-08**	3-04-03	2 x 4	1-03-08	4-07 2-06-02	73.82 51.33		
	3	J31 Jack-Open	8 /12	**4-02-08**	4-00-03	2 x 4	1-03-08	4-07 3-02-02	39.17 26.00		

TOTAL # TRUSS= **72** TOTAL BFT OF ALL TRUSSES= **3287.99** BFT. TOTAL WEIGHT OF ALL TRSSES **5154.64** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **8**

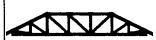
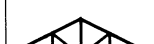
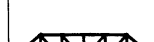
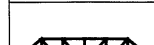
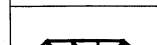
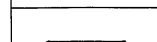
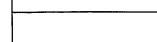
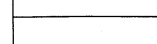
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423569
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	1 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	**30-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	277.92 172.67		
	1	T2 Hip	6 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.38 79.83		
	1	T3 Hip	6 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.27 79.33		
	1	T4 Hip	6 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.71 79.33		
	1	T5 Hip	6 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.41 84.17		
	5	T6C Common	6 /12	**30-11-00**	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	629.51 392.50		
	1	T8 Hip	10 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T8C Hip	10 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	137.74 88.83		
	1	T9 Hip	10 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T9C Hip	10 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.72 92.33		
	1	T10 Hip	10 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T10C Hip	10 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.09 90.50		
	1	T11 Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		
	1	T11C Hip	10 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	173.38 108.00		

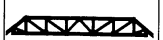
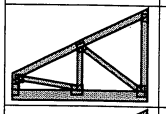
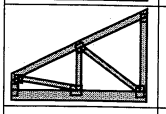
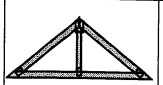
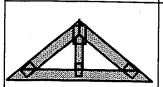
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423569
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	2 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T12 Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	1	T12C Hip	10 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.66 94.50		
	3	T13 Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	512.48 322.00		
	1	T13C Hip	10 /12	**30-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	170.83 107.33		
	2	T16 Common	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.2 76.33		
	3	T16S Roof Special	10 /12 10 /12	**12-03-00**	6-08-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	182.61 118.50		
	1 2-ply	T30 Hip Girder	10 /12	**30-11-00**	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	339.47 208.00		
	1	T31 Common	10 /12	**13-00-00**	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	62.1 39.50		
	3	T31S Roof Special	10 /12 10 /12	**13-00-00**	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	190.97 124.00		
	1 2-ply	T32 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T32Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	V30 Valley	10 /12	**8-11-14**	3-08-15	2 x 4			27.37 18.00		
	1	V31 Valley	10 /12	**6-07-01**	2-08-15	2 x 4			17.83 11.50		
	1	V32 Valley	10 /12	**4-02-04**	1-07-07	2 x 4			9.82 7.17		

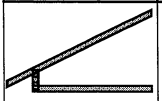
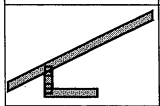
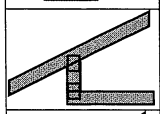
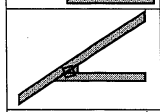
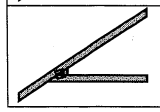
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423569
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	3 of 3
Lot #:		Date:	06-28-2022
Elevation:	B-REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	13	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	218.33 138.67		
	2	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	7	J30 Jack-Open	8 /12	**3-02-08**	3-04-03	2 x 4	1-03-08	4-07 2-06-02	73.82 51.33		
	3	J31 Jack-Open	8 /12	**4-02-08**	4-00-03	2 x 4	1-03-08	4-07 3-02-02	39.17 26.00		

TOTAL # TRUSS= **74** TOTAL BFT OF ALL TRUSSES= **3211.81** BFT. TOTAL WEIGHT OF ALL TRSSES **5083.28** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LJS26DS	
3		VTCR	

TOTAL NUMBER OF ITEMS= **11**

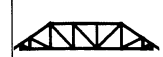
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423570
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	1 of 4
Lot #:		Date:	06-28-2022
Elevation:	C-OPT.WITH COFF.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	6 /12	**30-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	292.54 189.33		
	1	T2S Hip	6 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.61 85.67		
	1	T3S Hip	6 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	135.28 86.50		
	1	T4S Hip	6 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.12 88.50		
	1	T5 Hip	6 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.41 84.17		
	8	T6C Common	6 /12	**30-11-00**	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1007.22 628.00		
	1	T19 Half Hip Girder	6 /12	**5-10-08**	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	23.24 16.50		
	1	T20S Half Hip	6 /12	**5-10-08**	2-10-12	2 x 4	1-03-08	1-02-00 1-10-12	25.02 18.50		
	1	T21S Half Hip	6 /12	**5-10-08**	3-10-12	2 x 4	1-03-08	1-02-00 2-10-12	25.33 17.67		
	1 2-ply	T50 Hip Girder	8 /12	**30-11-00**	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	336.29 206.00		
	1	T51 Hip	8 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.95 84.83		
	1	T51C Hip	8 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.95 84.83		
	1	T52 Hip	8 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.82 85.00		
	1	T52C Hip	8 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.82 85.00		

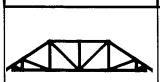
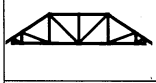
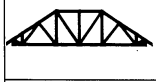
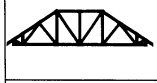
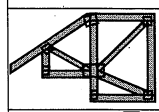
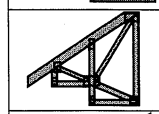
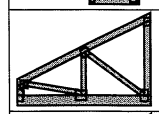
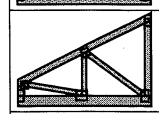
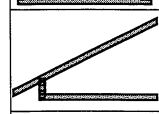
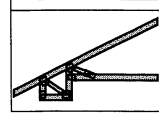
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423570
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	2 of 4
Lot #:		Date:	06-28-2022
Elevation:	C-OPT.WITH COFF.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T53 Hip	8 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	131.06 83.17		
	1	T53C Hip	8 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	131.06 83.17		
	1	T54 Hip	8 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.29 91.00		
	1	T54C Hip	8 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.29 91.00		
	3	T55 Hip	8 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	419.18 260.00		
	2	T56 Common	8 /12	**12-03-00**	5-05-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	122.1 78.33		
	1	T56Z Common Girder	8 /12	**12-03-00**	5-05-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	61.05 39.17		
	1	T57 Hip Girder	8 /12	**13-00-00**	5-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	59.54 37.33		
	2	T58S Half Hip Girder	8 /12	**4-07-08**	3-07-13	2 x 4	1-03-08	1-04-13 3-07-13	55.15 38.33		
	2	T59S Half Hip	8 /12	**4-07-08**	4-11-13	2 x 6 2 x 4	1-03-08	1-04-13 4-11-13	70.21 46.00		
	1 2-ply	T60 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T60Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	6	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		
	6	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	122.34 88.00		

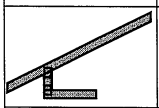
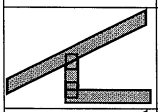
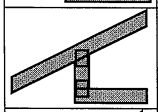
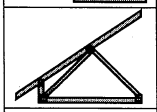
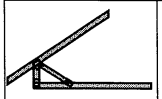
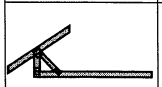
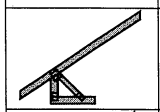
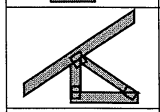
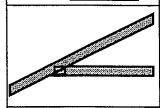
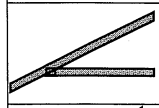
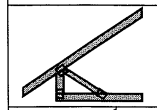
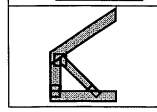
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423570
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	3 of 4
Lot #:		Date:	06-28-2022
Elevation:	C-OPT.WITH COFF.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	1	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	3	J6 Jack-Open	6 /12	**1-05-08**	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	18.38 14.00		
	2	J60 Jack-Open	8 /12	**5-10-08**	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	48.44 29.33		
	2	J61 Jack-Open	8 /12	**3-09-08**	3-11-02	2 x 4	1-03-08 2-01-00	1-04-13 3-11-02	33.12 20.67		
	2	J62 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.74 16.67		
	2	J63 Jack-Open	8 /12	**1-10-08**	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.21 15.33		
	2	J64 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	18.34 11.33		
	7	J65 Jack-Open	6 /12	**3-02-08**	2-06-15	2 x 4	1-03-08	4-03 1-11-07	66.93 46.67		
	3	J66 Jack-Open	6 /12	**4-02-08**	3-00-15	2 x 4	1-03-08	4-03 2-05-07	35.93 24.00		
	2	J67 Jack-Open	8 /12	**3-04-08**	3-07-13	2 x 4	1-03-08	1-04-13 3-07-13	26.29 16.67		
	2	J68 Jack-Open	8 /12	**1-10-08**	2-07-13	2 x 4		1-04-13 2-07-13	14.43 10.00		

REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423570
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	4 of 4
Lot #:		Date:	06-28-2022
Elevation:	C-OPT.WITH COFF.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **85** TOTAL BFT OF ALL TRUSSES= **3065.34** BFT. TOTAL WEIGHT OF ALL TRSSES **4814.03** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **11**

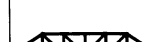
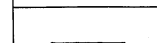
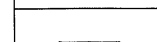
REVIEWED

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423571
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	1 of 3
Lot #:		Date:	06-28-2022
Elevation:	C-STD OR 5 BED (NO COFF)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	30-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	277.92 172.67		
	1	T2 Hip	6 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.38 79.83		
	1	T3 Hip	6 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.27 79.33		
	1	T4 Hip	6 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.71 79.33		
	1	T5 Hip	6 /12	30-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.41 84.17		
	8	T6C Common	6 /12	30-11-00	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1007.22 628.00		
	1 2-ply	T50 Hip Girder	8 /12	30-11-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	336.29 206.00		
	1	T51 Hip	8 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.95 84.83		
	1	T51C Hip	8 /12	30-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.95 84.83		
	1	T52 Hip	8 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.82 85.00		
	1	T52C Hip	8 /12	30-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.82 85.00		
	1	T53 Hip	8 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	131.06 83.17		
	1	T53C Hip	8 /12	30-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	131.06 83.17		
	1	T54 Hip	8 /12	30-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.29 91.00		

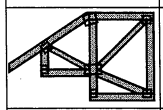
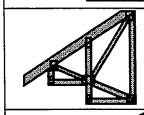
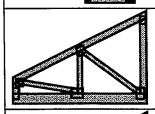
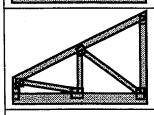
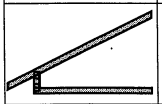
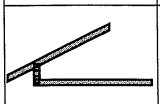
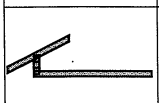
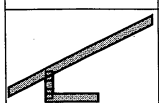
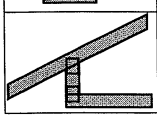
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423571
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	2 of 3
Lot #:		Date:	06-28-2022
Elevation:	C-STD OR 5 BED (NO COFF)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T54C Hip	8 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.29 91.00		
	3	T55 Hip	8 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	419.18 260.00		
	2	T56 Common	8 /12	**12-03-00**	5-05-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	122.1 78.33		
	1	T56Z Common Girder	8 /12	**12-03-00**	5-05-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	61.05 39.17		
	1	T57 Hip Girder	8 /12	**13-00-00**	5-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	59.54 37.33		
	2	T58S Half Hip Girder	8 /12	**4-07-08**	3-07-13	2 x 4	1-03-08	1-04-13 3-07-13	55.15 38.33		
	2	T59S Half Hip	8 /12	**4-07-08**	4-11-13	2 x 6 2 x 4	1-03-08	1-04-13 4-11-13	70.21 46.00		
	1 2-ply	T60 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T60Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	13	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	218.33 138.67		
	2	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		

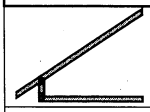
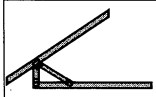
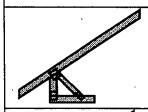
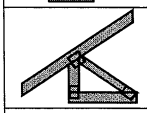
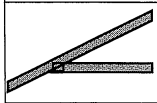
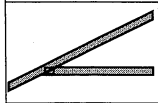
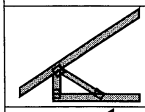
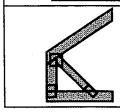
REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423571
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	3 of 3
Lot #:		Date:	06-28-2022
Elevation:	C-STD OR 5 BED (NO COFF)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	J60 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	35.66 21.33		
	2	J61 Jack-Open	8 /12	3-09-08	3-11-02	2 x 4	1-03-08 2-01-00	1-04-13 3-11-02	33.12 20.67		
	2	J62 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.74 16.67		
	2	J63 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.21 15.33		
	2	J64 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	18.34 11.33		
	7	J65 Jack-Open	6 /12	3-02-08	2-06-15	2 x 4	1-03-08	4-03 1-11-07	66.93 46.67		
	3	J66 Jack-Open	6 /12	4-02-08	3-00-15	2 x 4	1-03-08	4-03 2-05-07	35.93 24.00		
	2	J67 Jack-Open	8 /12	3-04-08	3-07-13	2 x 4	1-03-08	1-04-13 3-07-13	26.29 16.67		
	2	J68 Jack-Open	8 /12	1-10-08	2-07-13	2 x 4		1-04-13 2-07-13	14.43 10.00		

TOTAL # TRUSS= **84** TOTAL BFT OF ALL TRUSSES= **2965.16** BFT. TOTAL WEIGHT OF ALL TRSSES **4705.52** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **11**

REVIEWED

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
Builder:	BAYVIEW WELLINGTON	PlanLog:	205567
Project:	GREEN VALLEY EAST	Layout ID:	423572
Location:	BRADFORD	Ref #	
Model:	S38-20	Page:	1 of 4
Lot #:		Date:	06-28-2022
Elevation:	C-REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1Z3 Hip Girder	6 /12	**30-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	277.92 174.67		
	1	T2 Hip	6 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.38 79.83		
	1	T3 Hip	6 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.27 79.33		
	1	T4 Hip	6 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.71 79.33		
	1	T5 Hip	6 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.41 84.17		
	8	T6C Common	6 /12	**30-11-00**	8-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1007.22 628.00		
	1 2-ply	T50 Hip Girder	8 /12	**30-11-00**	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	336.29 206.00		
	1	T51 Hip	8 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.95 84.83		
	1	T51C Hip	8 /12	**30-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.95 84.83		
	1	T52 Hip	8 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.82 85.00		
	1	T52C Hip	8 /12	**30-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.82 85.00		
	1	T53 Hip	8 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	131.06 83.17		
	1	T53C Hip	8 /12	**30-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	131.06 83.17		
	1	T54 Hip	8 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.29 91.00		

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Roof Trusses

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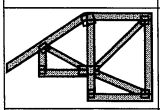
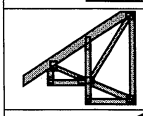
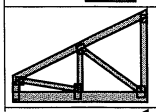
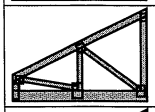
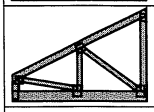
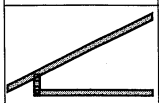
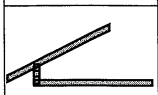
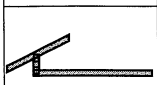
	1	T54C Hip	8 /12	**30-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.29 91.00		
	3	T55 Hip	8 /12	**30-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	419.18 260.00		
	2	T56 Common	8 /12	**12-03-00**	5-05-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	122.1 78.33		
	1	T56Z Common Girder	8 /12	**12-03-00**	5-05-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	61.05 39.17		
	1	T57 Hip Girder	8 /12	**13-00-00**	5-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	59.54 37.33		
	2	T58S Half Hip Girder	8 /12	**4-07-08**	3-07-13	2 x 4	1-03-08	1-04-13 3-07-13	55.15 38.33		
	2	T59S Half Hip	8 /12	**4-07-08**	4-11-13	2 x 6 2 x 4	1-03-08	1-04-13 4-11-13	70.21 46.00		
	1 2-ply	T60 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T60Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T60Z7 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T70 Hip Girder	6 /12	**19-00-00**	3-11-13	2 x 4 2 x 6	1-03-08	1-02-00 2-08-00	86.66 56.00		
	5	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	1	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	1	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		

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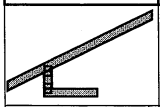
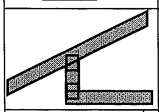
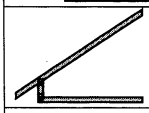
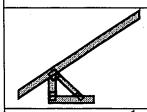
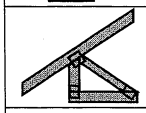
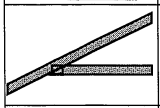
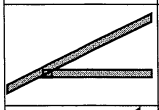
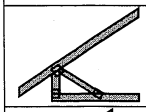
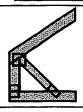
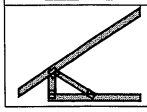
	1	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	2	J60 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	35.66 21.33		
	2	J61 Jack-Open	8 /12	3-09-08	3-11-02	2 x 4	1-03-08 2-01-00	1-04-13 3-11-02	33.12 20.67		
	2	J62 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.74 16.67		
	2	J63 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.21 15.33		
	2	J64 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	18.34 11.33		
	7	J65 Jack-Open	6 /12	3-02-08	2-06-15	2 x 4	1-03-08	4-03 1-11-07	66.93 46.67		
	3	J66 Jack-Open	6 /12	4-02-08	3-00-15	2 x 4	1-03-08	4-03 2-05-07	35.93 24.00		
	2	J67 Jack-Open	8 /12	3-04-08	3-07-13	2 x 4	1-03-08	1-04-13 3-07-13	26.29 16.67		
	2	J68 Jack-Open	8 /12	1-10-08	2-07-13	2 x 4		1-04-13 2-07-13	14.43 10.00		
	9	J70 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	129.59 81.00		

TOTAL # TRUSS= **84** TOTAL BFT OF ALL TRUSSES= **3029.16** BFT. TOTAL WEIGHT OF ALL TRSSES **4802.75** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

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		Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **13**

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423566	T1	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 10:35:30 2022 Page 1
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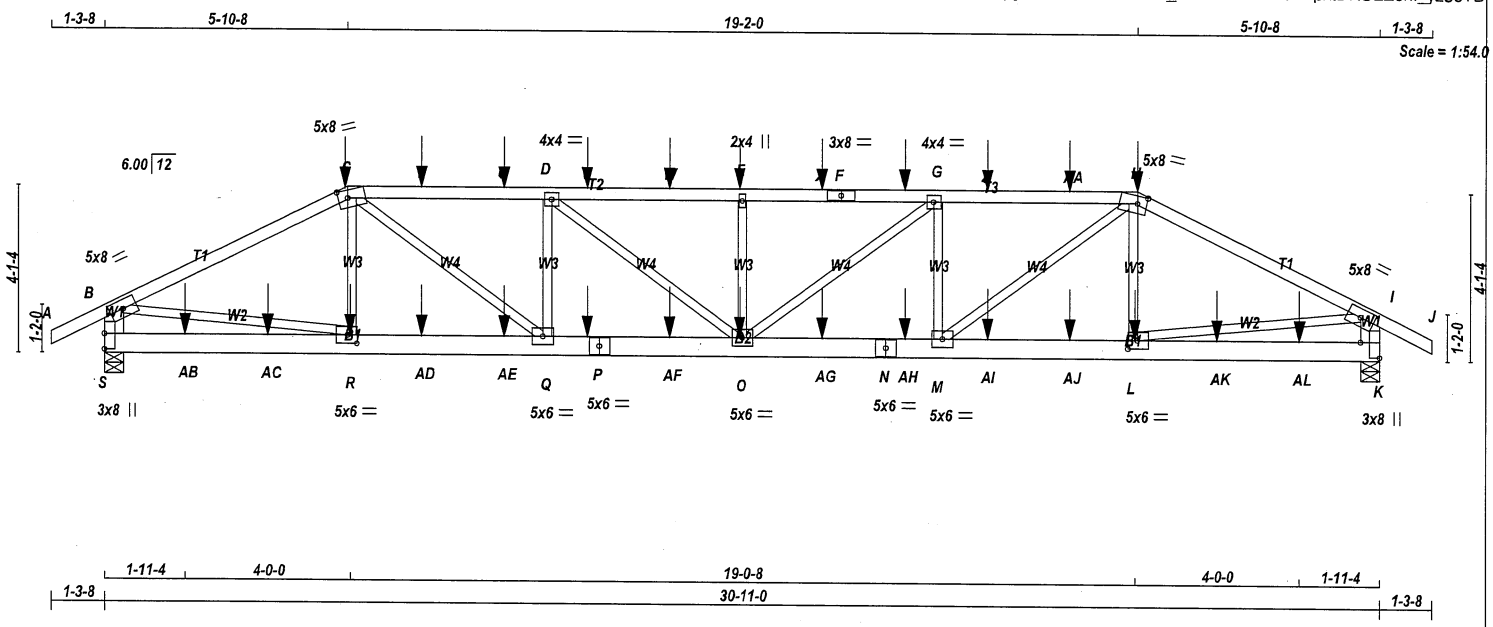


TOTAL WEIGHT = 2 X 139 = 278 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

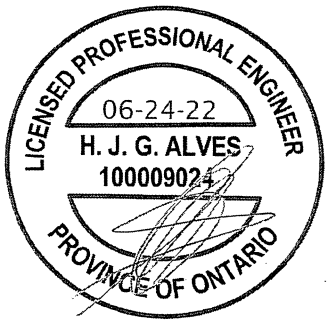
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(183.1)
P- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



Structural component only
DWG# T-2215215

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3425	0	3425	0	0	5-8	5-8
K	3433	0	3433	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2402	1693 / 0	0 / 0	0 / 0	0 / 0	709 / 0	0 / 0
K	2407	1696 / 0	0 / 0	0 / 0	0 / 0	711 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.08 (1)	10.00
B- C	-5074 / 0	-112.4	-112.4	0.60 (1)	3.76
C- T	-6596 / 0	-112.4	-112.4	0.56 (1)	3.32
T- U	-6596 / 0	-112.4	-112.4	0.56 (1)	3.32
U- D	-6596 / 0	-112.4	-112.4	0.56 (1)	3.32
D- V	-7190 / 0	-112.4	-112.4	0.59 (1)	3.15
V- W	-7190 / 0	-112.4	-112.4	0.59 (1)	3.15
W- E	-7190 / 0	-112.4	-112.4	0.59 (1)	3.15
E- X	-7190 / 0	-112.4	-112.4	0.59 (1)	3.15
X- F	-7190 / 0	-112.4	-112.4	0.59 (1)	3.15
F- Y	-7190 / 0	-112.4	-112.4	0.59 (1)	3.15
Y- G	-7190 / 0	-112.4	-112.4	0.59 (1)	3.15
G- Z	-6602 / 0	-112.4	-112.4	0.56 (1)	3.32
Z-AA	-6602 / 0	-112.4	-112.4	0.56 (1)	3.32
AA- H	-6602 / 0	-112.4	-112.4	0.56 (1)	3.32
H- I	-5087 / 0	-112.4	-112.4	0.61 (1)	3.75
I- J	0 / 34	-112.4	-112.4	0.08 (1)	10.00
S- B	-3345 / 0	0.0	0.0	0.12 (1)	7.61
K- I	-3352 / 0	0.0	0.0	0.12 (1)	7.60
S-AB	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AB-AC	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AC- R	0 / 0	-18.5	-18.5	0.07 (4)	10.00
R-AD	0 / 4524	-18.5	-18.5	0.33 (1)	10.00
AD-AE	0 / 4524	-18.5	-18.5	0.33 (1)	10.00
AE- Q	0 / 4524	-18.5	-18.5	0.33 (1)	10.00
Q- P	0 / 6597	-18.5	-18.5	0.49 (1)	10.00
P-AF	0 / 6597	-18.5	-18.5	0.49 (1)	10.00
AF- O	0 / 6597	-18.5	-18.5	0.49 (1)	10.00
O-AG	0 / 6603	-18.5	-18.5	0.49 (1)	10.00
AG- N	0 / 6603	-18.5	-18.5	0.49 (1)	10.00
N-AH	0 / 6603	-18.5	-18.5	0.49 (1)	10.00
AH- M	0 / 6603	-18.5	-18.5	0.49 (1)	10.00
M-AI	0 / 4536	-18.5	-18.5	0.33 (1)	10.00
AI-AJ	0 / 4536	-18.5	-18.5	0.33 (1)	10.00
AJ- L	0 / 4536	-18.5	-18.5	0.33 (1)	10.00
L-AK	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AK-AL	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AL- K	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-406 / 47	0.05 (1)
C- Q	0 / 2613	0.32 (1)
Q- D	-1436 / 0	0.18 (1)
D- O	0 / 755	0.09 (1)
O- E	-803 / 0	0.10 (1)
O- G	0 / 747	0.09 (1)
M- G	-1431 / 0	0.18 (1)
M- H	0 / 2606	0.32 (1)
L- H	-405 / 50	0.05 (1)
B- R	0 / 4573	0.57 (1)
L- I	0 / 4585	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.61/1.00 (H-I:1) , BC=0.49/1.00 (O-Q:1) , WB=0.57/1.00 (I-L:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.62 (P) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423566	T1	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.25	2.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	2.75
I	TMVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	8.0	4.50	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	8.0	4.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-308	-308	---	FRONT	VERT	TOTAL	---	C1
E	15-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
H	25-0-8	-308	-308	---	FRONT	VERT	TOTAL	---	C1
L	24-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
O	15-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	11-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	7-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
U	9-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
V	11-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
W	13-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
X	17-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Y	19-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Z	21-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AA	23-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AD	7-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AE	9-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	13-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	17-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	19-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	21-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	23-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	26-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	28-11-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100090244
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215215

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423572	T1Z3	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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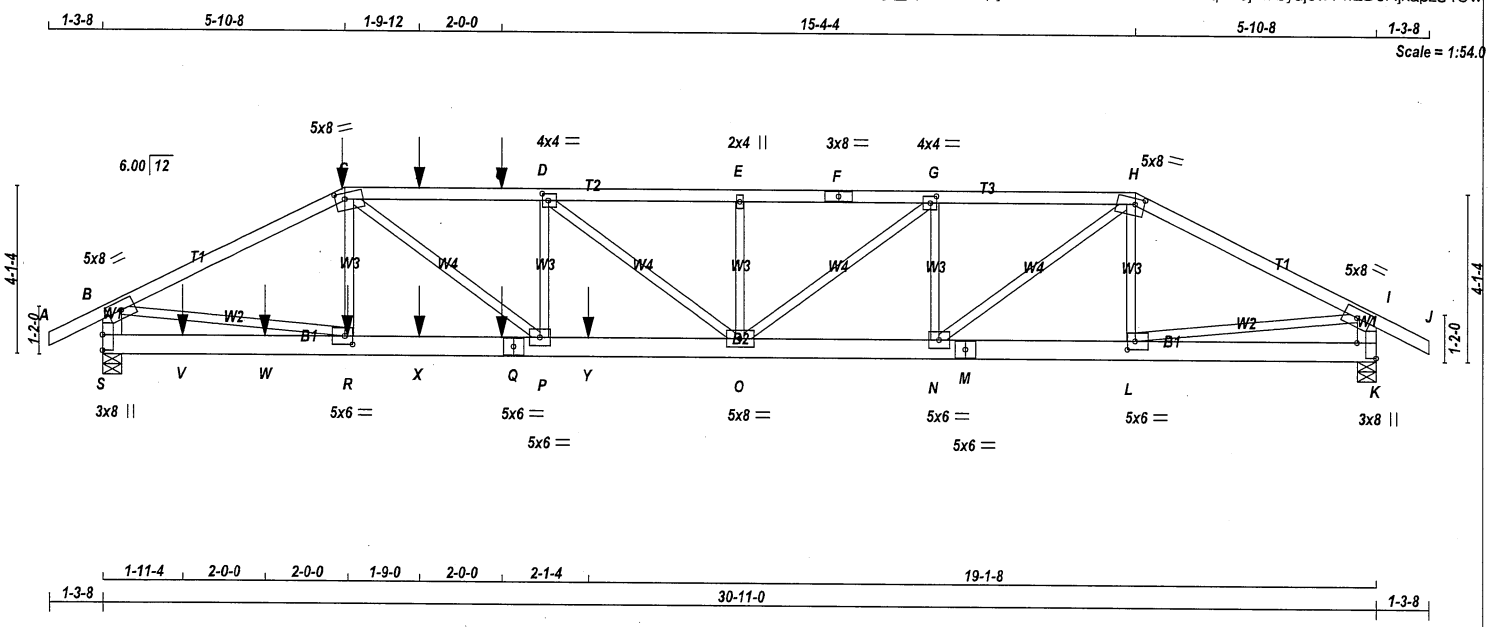

TOTAL WEIGHT = 2 X 139 = 278 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- Q	2	12	SIDE(183.1)
Q- M	2	12	SIDE(0.0)
M- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2215259

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3966	0	3966	0	0	5-8	5-8
K	3058	0	3058	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2775	1986 / 0	0 / 0	0 / 0	0 / 0	790 / 0	0 / 0
K	2138	1541 / 0	0 / 0	0 / 0	0 / 0	597 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34		-112.4	-112.4	0.08	(1)	10.00	R- C	-665 / 0	0.08 (1)
B- C	-5997 / 0		-112.4	-112.4	0.69	(1)	3.43	C- P	0 / 3683	0.46 (1)
C- T	-8262 / 0		-112.4	-112.4	0.65	(1)	2.90	P- D	-535 / 0	0.07 (1)
T- U	-8262 / 0		-112.4	-112.4	0.65	(1)	2.90	D- O	-424 / 0	0.13 (1)
U- D	-8262 / 0		-112.4	-112.4	0.65	(1)	2.90	O- E	-455 / 0	0.06 (1)
D- E	-7929 / 0		-112.4	-112.4	0.57	(1)	3.06	O- G	0 / 1987	0.25 (1)
E- F	-7929 / 0		-112.4	-112.4	0.50	(1)	3.14	N- G	-1840 / 0	0.23 (1)
F- G	-7929 / 0		-112.4	-112.4	0.50	(1)	3.14	N- H	0 / 3006	0.37 (1)
G- H	-6367 / 0		-112.4	-112.4	0.40	(1)	3.58	L- H	-389 / 0	0.05 (1)
H- I	-4469 / 0		-112.4	-112.4	0.56	(1)	4.00	B- R	0 / 5405	0.67 (1)
I- J	0 / 34		-112.4	-112.4	0.08	(1)	10.00	L- I	0 / 4028	0.50 (1)
S- B	-3865 / 0		0.0	0.0	0.14	(1)	7.20			
K- I	-3004 / 0		0.0	0.0	0.11	(1)	7.81			
S- V	0 / 0		-18.5	-18.5	0.07	(1)	10.00			
V- W	0 / 0		-18.5	-18.5	0.07	(1)	10.00			
W- R	0 / 0		-18.5	-18.5	0.07	(1)	10.00			
R- X	0 / 5342		-18.5	-18.5	0.52	(1)	10.00			
X- Q	0 / 5342		-18.5	-18.5	0.52	(1)	10.00			
Q- P	0 / 5342		-18.5	-18.5	0.52	(1)	10.00			
P- Y	0 / 8262		-18.5	-18.5	0.96	(1)	10.00			
Y- O	0 / 8262		-18.5	-18.5	0.96	(1)	10.00			
O- N	0 / 6368		-18.5	-18.5	0.49	(1)	10.00			
N- M	0 / 3984		-18.5	-18.5	0.31	(1)	10.00			
M- L	0 / 3984		-18.5	-18.5	0.31	(1)	10.00			
L- K	0 / 0		-18.5	-18.5	0.04	(4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-308	-308	---	FRONT	VERT	TOTAL	---	C1
Q	9-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	7-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
U	9-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
V	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
W	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	7-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	11-9-8	-1271	-1271	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 890 (0.42")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.96/1.00 (O-P:1) , WB=0.67/1.00 (B-R:1) , SSI=0.50/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.61 (R) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423572	T1Z3	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	1.75	2.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TTWW-m	MT20	5.0	8.0	1.75	2.75
I	TMVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	8.0	4.50	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	3.0	8.0	4.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2215259

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T1S	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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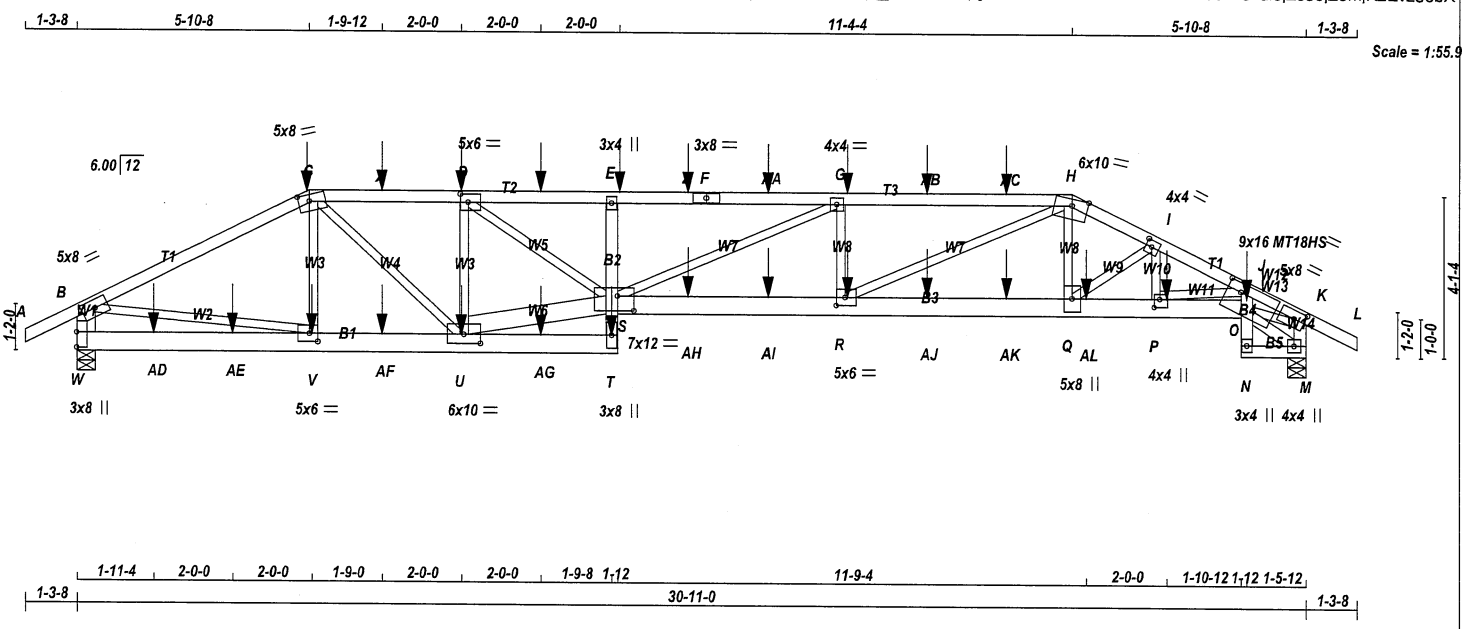

TOTAL WEIGHT = 2 X 147 = 293 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - L	2x4	DRY	2100F 1.8E	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - E	2x4	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(65.9)
C- F	1	12	SIDE(65.9)
F- H	1	12	SIDE(65.9)
H- L	1	12	TOP
M- K	1	12	TOP
W- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(183.1)
S- O	2	12	SIDE(183.1)
T- E	1	12	TOP
J- N	1	12	SIDE(27.2)
N- M	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

Structural component only
DWG# T-2215186

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3635	0	3635	0	0	5-8	5-8
M	4187	0	4187	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2549	1797 / 0	0 / 0	0 / 0	0 / 0	751 / 0	0 / 0
M	2930	2095 / 0	0 / 0	0 / 0	0 / 0	835 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 34	-112.4	-112.4	0.05 (1)	10.00	V- C	-482 / 16	0.06 (1)
B- C	-5441 / 0	-112.4	-112.4	0.34 (1)	4.74	C- U	0 / 2701	0.33 (1)
C- X	-6780 / 0	-112.4	-112.4	0.19 (1)	4.47	U- D	-3496 / 0	0.44 (1)
X- D	-6780 / 0	-112.4	-112.4	0.19 (1)	4.47	U- S	0 / 6809	0.44 (1)
D- Y	-10637 / 0	-112.4	-112.4	0.26 (1)	3.64	D- S	0 / 4691	0.58 (1)
Y- E	-10637 / 0	-112.4	-112.4	0.26 (1)	3.64	S- G	0 / 243	0.03 (1)
E- Z	-10734 / 0	-112.4	-112.4	0.56 (1)	3.38	R- G	-1295 / 0	0.12 (1)
Z- F	-10734 / 0	-112.4	-112.4	0.56 (1)	3.38	R- H	0 / 3760	0.47 (1)
F-AA	-10734 / 0	-112.4	-112.4	0.56 (1)	3.38	Q- H	0 / 1476	0.18 (1)
AA- G	-10734 / 0	-112.4	-112.4	0.56 (1)	3.38	B- V	0 / 4904	0.61 (1)
G-AB	-10515 / 0	-112.4	-112.4	0.55 (1)	3.42	O- M	-702 / 0	0.04 (1)
AB-AC	-10515 / 0	-112.4	-112.4	0.55 (1)	3.42	O- K	0 / 7406	0.92 (1)
AC- H	-10515 / 0	-112.4	-112.4	0.55 (1)	3.42	P- J	-430 / 0	0.04 (1)
H- I	-7909 / 0	-112.4	-112.4	0.09 (1)	4.29	P- I	0 / 506	0.06 (1)
I- J	-8684 / 0	-112.4	-112.4	0.28 (1)	3.90	Q- I	-995 / 0	0.09 (1)
J- K	-8414 / 0	-112.4	-112.4	0.27 (1)	3.96			
K- L	0 / 34	-112.4	-112.4	0.05 (1)	10.00			
W- B	-3552 / 0	0.0	0.0	0.13 (1)	7.43			
M- K	-3793 / 0	0.0	0.0	0.21 (1)	6.05			
W-AD	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AD-AE	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AE- V	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
V-AF	0 / 4850	-18.5	-18.5	0.34 (1)	10.00			
AF- U	0 / 4850	-18.5	-18.5	0.34 (1)	10.00			
U-AG	0 / 266	-18.5	-18.5	0.04 (1)	10.00			
AG- T	0 / 266	-18.5	-18.5	0.04 (1)	10.00			
T- S	0 / 156	0.0	0.0	0.21 (1)	10.00			
S- E	-815 / 0	0.0	0.0	0.21 (1)	7.81			
S-AH	0 / 10515	-18.5	-18.5	0.81 (1)	10.00			
AH-AI	0 / 10515	-18.5	-18.5	0.81 (1)	10.00			
AI- R	0 / 10515	-18.5	-18.5	0.81 (1)	10.00			
R-AJ	0 / 7109	-18.5	-18.5	0.58 (1)	10.00			
AJ-AK	0 / 7109	-18.5	-18.5	0.58 (1)	10.00			
AK- Q	0 / 7109	-18.5	-18.5	0.58 (1)	10.00			
Q-AL	0 / 7844	-18.5	-18.5	0.66 (1)	10.00			
AL- P	0 / 7844	-18.5	-18.5	0.66 (1)	10.00			
P- O	0 / 8267	-18.5	-18.5	0.69 (1)	10.00			
N- O	0 / 15	0.0	0.0	0.42 (1)	10.00			
O- J	0 / 186	0.0	0.0	0.44 (1)	10.00			
N- M	0 / 590	-18.5	-18.5	0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 669 (0.55")

CSI: TC=0.56/1.00 (E-G:1) , BC=0.81/1.00 (R-S:1) , WB=0.92/1.00 (K-O:1) , SSI=0.37/1.00 (J-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.76 (H) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T1S	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.25
D	TMWW-t	MT20	5.0	6.0	2.50	2.25
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	10.0	2.00	4.75
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J						
J	TMBVWWW*-t	MT18HS	9.0	16.0	2.75	4.25
K	TMVW-t	MT20	5.0	8.0	2.50	3.25
M	BMVW1+p	MT20	4.0	4.0		
N	BMV+p	MT20	3.0	4.0		
O						
P	BMWW+t	MT20	4.0	4.0	2.50	1.75
Q	BMWW+t	MT20	5.0	8.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BVMWWW-t	MT20	7.0	12.0	4.50	5.00
T	BMV+p	MT20	3.0	8.0		
U	BMWWW-t	MT20	6.0	10.0	2.75	5.00
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1+p	MT20	3.0	8.0	4.50	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-308	-308	---	FRONT	VERT	TOTAL	---	C1
D	9-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
E	13-8-4	-94	-94	---	FRONT	VERT	TOTAL	---	C1
G	19-4-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
O	29-5-4	-237	-237	---	FRONT	VERT	TOTAL	---	C1
P	27-4-12	-243	-243	---	FRONT	VERT	TOTAL	---	C1
R	19-4-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
T	13-5-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
U	9-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	7-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Y	11-8-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Z	15-4-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
AA	17-4-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
AB	21-4-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
AC	23-4-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
AD	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
AE	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	7-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	11-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	15-4-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AI	17-4-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AJ	21-4-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AK	23-4-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AL	25-4-12	-243	-243	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2215186

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423566	T2	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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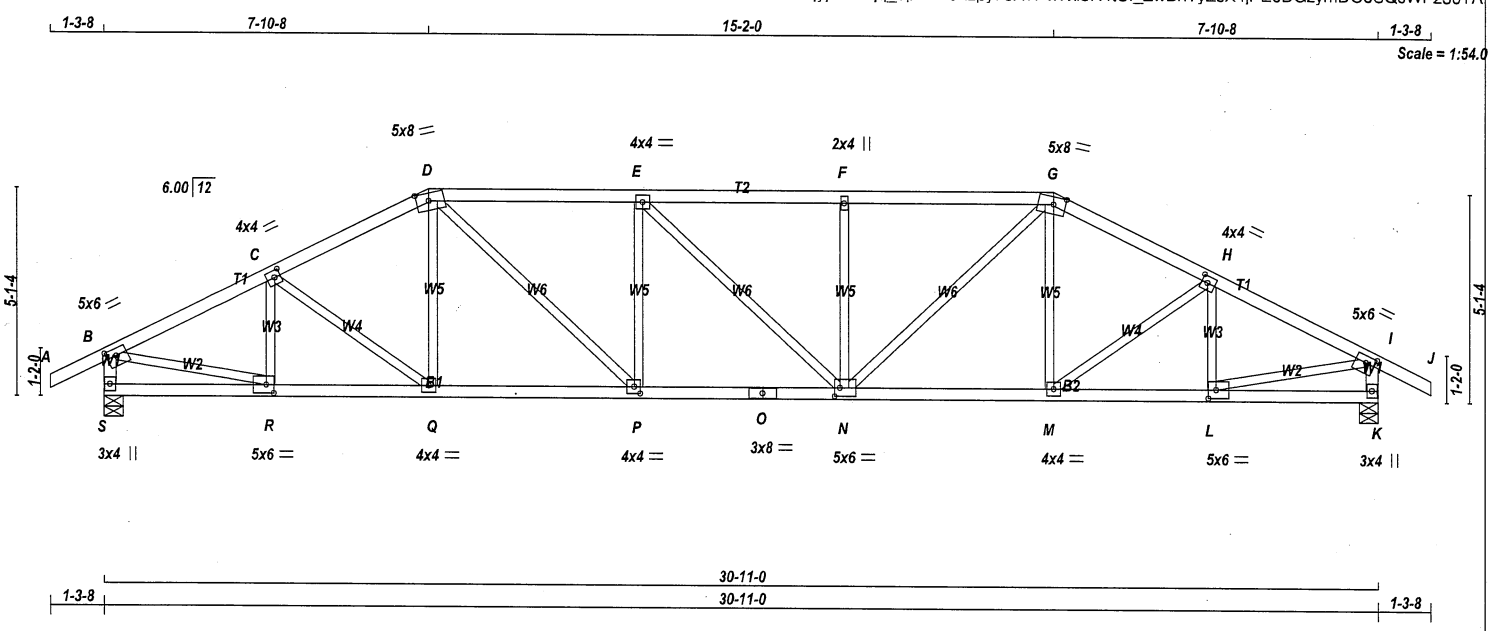

TOTAL WEIGHT = 126 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.25	3.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-t	MT20	5.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	1.50
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215216

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2177	0	2177	0	0	5-8	5-8
K	2177	0	2177	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1523	1092 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1523	1092 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00		R- C	-476 / 0	0.09 (1)
B- C	-2810 / 0	-112.4	-112.4	0.30 (1)	3.91		C- Q	-66 / 0	0.03 (1)
C- D	-2795 / 0	-112.4	-112.4	0.29 (1)	3.92		Q- D	0 / 134	0.03 (4)
D- E	-3227 / 0	-112.4	-112.4	0.47 (1)	3.49		D- P	0 / 1026	0.23 (1)
E- F	-3226 / 0	-112.4	-112.4	0.43 (1)	3.54		P- E	-613 / 0	0.24 (1)
F- G	-3225 / 0	-112.4	-112.4	0.47 (1)	3.49		E- N	-3 / 0	0.00 (1)
G- H	-2795 / 0	-112.4	-112.4	0.29 (1)	3.92		N- F	-612 / 0	0.24 (1)
H- I	-2810 / 0	-112.4	-112.4	0.30 (1)	3.91		N- G	0 / 1023	0.23 (1)
I- J	0 / 34	-112.4	-112.4	0.15 (1)	10.00		M- G	0 / 136	0.04 (4)
S- B	-2137 / 0	0.0	0.0	0.22 (1)	5.80		M- H	-65 / 0	0.03 (1)
K- I	-2137 / 0	0.0	0.0	0.22 (1)	5.80		L- H	-477 / 0	0.09 (1)
							B- R	0 / 2588	0.58 (1)
S- R	0 / 0	-18.5	-18.5	0.07 (4)	10.00		L- I	0 / 2588	0.58 (1)
R- Q	0 / 2531	-18.5	-18.5	0.45 (1)	10.00				
Q- P	0 / 2481	-18.5	-18.5	0.45 (1)	10.00				
P- O	0 / 3228	-18.5	-18.5	0.57 (1)	10.00				
O- N	0 / 3228	-18.5	-18.5	0.57 (1)	10.00				
N- M	0 / 2481	-18.5	-18.5	0.45 (1)	10.00				
M- L	0 / 2531	-18.5	-18.5	0.46 (1)	10.00				
L- K	0 / 0	-18.5	-18.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.47/1.00 (F-G:1) , BC=0.57/1.00 (N-P:1) , WB=0.58/1.00 (B-R:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.99 (O) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T2S	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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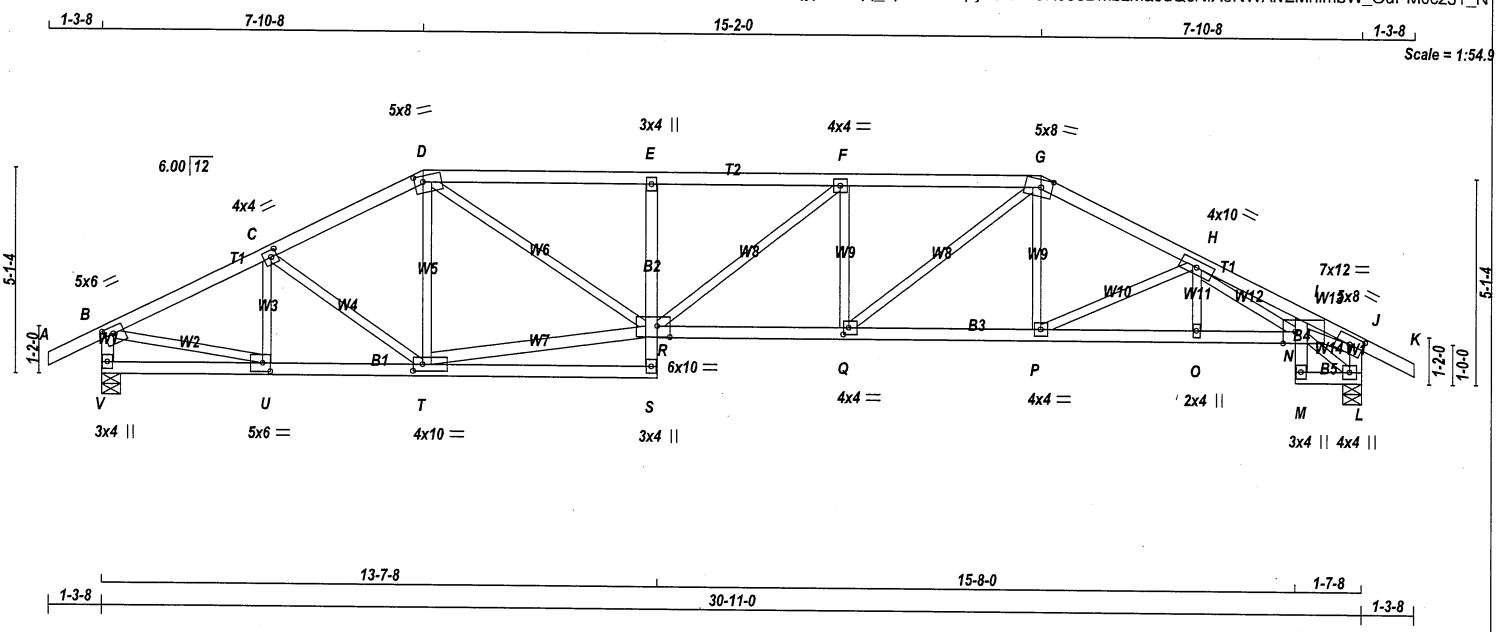

TOTAL WEIGHT = 132 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	2100F 1.8E	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - E	2x4	DRY	No.2	SPF
R - N	2x4	DRY	2100F 1.8E	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	1.75	2.50
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.25	3.25
H	TMWWW-t	MT20	4.0	10.0		
I						
I	TMBVWWW*-I	MT20	7.0	12.0	Edge	3.50
J	TMVW-t	MT20	5.0	8.0	2.50	4.00
L	BMVW1+p	MT20	4.0	4.0		
M	BMV+p	MT20	3.0	4.0		
N						
O	BMW+w	MT20	2.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BVMWWW-I	MT20	6.0	10.0	3.25	3.75
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	4.0	10.0	2.00	2.75
U	BMWW-t	MT20	5.0	6.0	2.50	2.25
V	BMV1+p	MT20	3.0	4.0		

Structural component only
DWG# T-2215187

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2173	0	2173	0	0	5-8	5-8
L	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1520	1090 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0
L	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.10 (1)	10.00
B- C	-2802 / 0	-112.4	-112.4	0.16 (1)	4.89
C- D	-2795 / 0	-112.4	-112.4	0.16 (1)	4.90
D- E	-4072 / 0	-112.4	-112.4	0.68 (1)	2.91
E- F	-4102 / 0	-112.4	-112.4	0.54 (1)	3.04
F- G	-4016 / 0	-112.4	-112.4	0.51 (1)	3.11
G- H	-3492 / 0	-112.4	-112.4	0.17 (1)	4.48
H- I	-4549 / 0	-112.4	-112.4	0.28 (1)	3.89
I- J	-4169 / 0	-112.4	-112.4	0.25 (1)	4.06
J- K	0 / 34	-112.4	-112.4	0.10 (1)	10.00
V- B	-2133 / 0	0.0	0.0	0.22 (1)	5.81
L- J	-1966 / 0	0.0	0.0	0.20 (1)	6.01
V- U	0 / 0	-18.5	-18.5	0.07 (4)	10.00
U- T	0 / 2524	-18.5	-18.5	0.48 (1)	10.00
T- S	0 / 112	-18.5	-18.5	0.13 (4)	10.00
S- R	0 / 47	0.0	0.0	0.16 (1)	10.00
R- E	-645 / 0	0.0	0.0	0.17 (1)	7.81
R- Q	0 / 4016	-18.5	-18.5	0.34 (1)	10.00
Q- P	0 / 3115	-18.5	-18.5	0.27 (1)	10.00
P- O	0 / 3811	-18.5	-18.5	0.32 (1)	10.00
O- N	0 / 3811	-18.5	-18.5	0.32 (1)	10.00
M- N	0 / 15	0.0	0.0	0.40 (1)	10.00
N- I	0 / 20	0.0	0.0	0.41 (1)	10.00
M- L	0 / 296	-18.5	-18.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-483 / 0	0.09 (1)
C- T	-56 / 0	0.02 (1)
T- D	-274 / 0	0.11 (1)
T- R	0 / 2404	0.39 (1)
D- R	0 / 1927	0.43 (1)
R- F	0 / 110	0.02 (1)
Q- F	-629 / 0	0.16 (1)
Q- G	0 / 1148	0.26 (1)
P- G	0 / 444	0.10 (1)
P- H	-784 / 0	0.25 (1)
O- H	0 / 62	0.02 (4)
B- U	0 / 2580	0.58 (1)
N- L	-357 / 0	0.05 (1)
N- J	0 / 3636	0.82 (1)
H- N	0 / 406	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 874 (0.42")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.48/1.00 (T-U:1) , WB=0.82/1.00 (J-N:1) , SSI=0.41/1.00 (I-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.76 (J) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T2S	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215187

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423566	T3	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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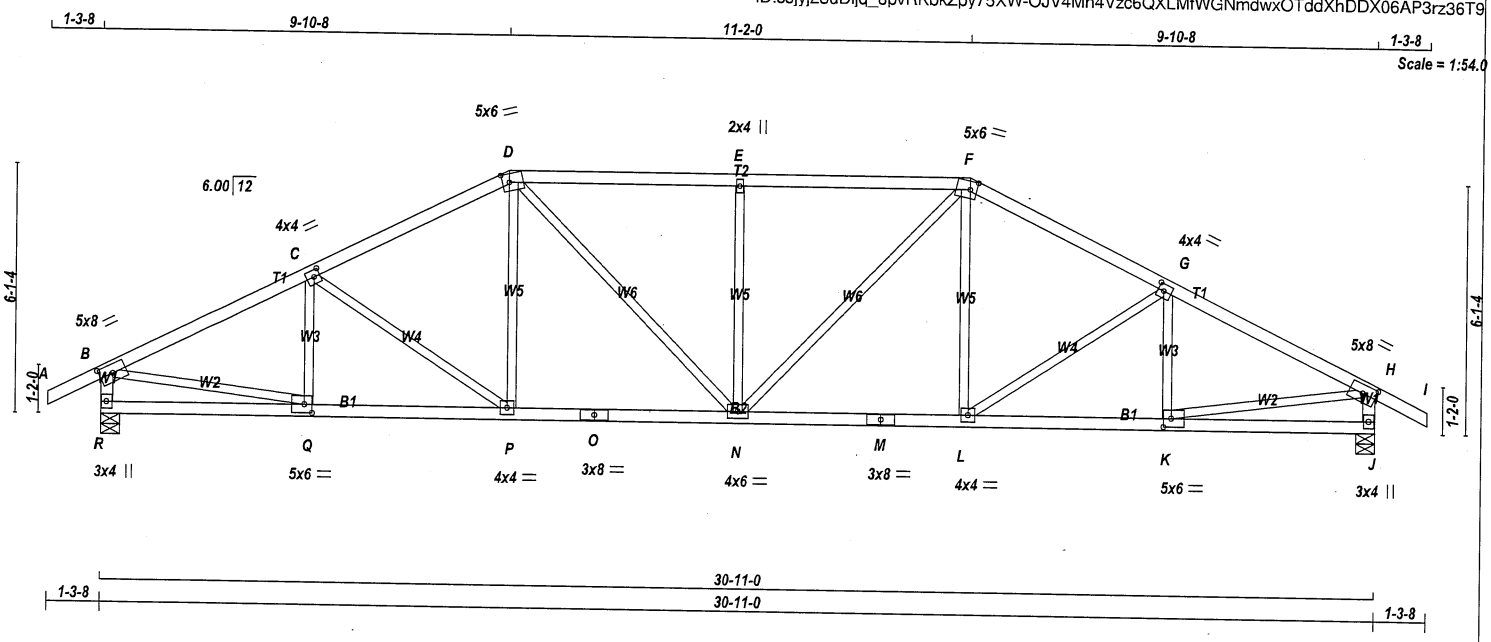

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	4.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0	2.50	4.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215217

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2177	0	2177	0	0	5-8	5-8
J	2177	0	2177	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1523	1092 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	1523	1092 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	-2906 / 0	-112.4	-112.4	0.46 (1)	3.67
C- D	-2655 / 0	-112.4	-112.4	0.43 (1)	3.85
D- E	-2769 / 0	-112.4	-112.4	0.56 (1)	3.60
E- F	-2769 / 0	-112.4	-112.4	0.56 (1)	3.60
F- G	-2655 / 0	-112.4	-112.4	0.43 (1)	3.85
G- H	-2906 / 0	-112.4	-112.4	0.46 (1)	3.67
H- I	0 / 34	-112.4	-112.4	0.15 (1)	10.00
R- B	-2132 / 0	0.0	0.0	0.22 (1)	5.81
J- H	-2132 / 0	0.0	0.0	0.22 (1)	5.81
R- Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00
Q- P	0 / 2624	-18.5	-18.5	0.48 (1)	10.00
P- O	0 / 2352	-18.5	-18.5	0.44 (1)	10.00
O- N	0 / 2352	-18.5	-18.5	0.44 (1)	10.00
N- M	0 / 2352	-18.5	-18.5	0.44 (1)	10.00
M- L	0 / 2352	-18.5	-18.5	0.44 (1)	10.00
L- K	0 / 2624	-18.5	-18.5	0.48 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-358 / 0	0.08 (1)
C- P	-335 / 0	0.21 (1)
P- D	0 / 294	0.07 (1)
D- N	0 / 601	0.14 (1)
N- E	-769 / 0	0.45 (1)
N- F	0 / 601	0.14 (1)
L- F	0 / 294	0.07 (1)
L- G	-335 / 0	0.21 (1)
K- G	-358 / 0	0.08 (1)
B- Q	0 / 2661	0.60 (1)
K- H	0 / 2661	0.60 (1)

TOTAL WEIGHT = 126 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.48/1.00 (P-Q:1) , WB=0.60/1.00 (B-Q:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.72 (O) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T3S	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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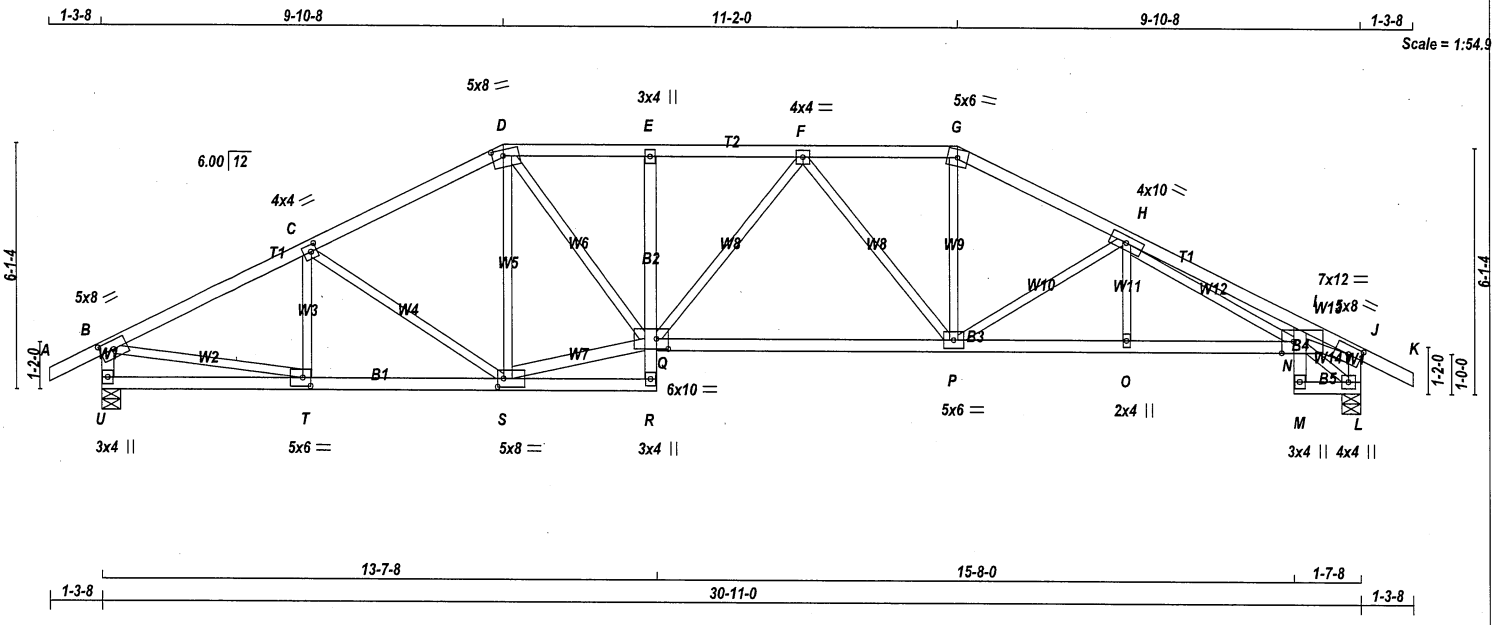

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	2100F 1.8E	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	2100F 1.8E	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	4.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	1.75	3.25
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	5.0	6.0		
H	TMWWW-t	MT20	4.0	10.0		
I						
I	TMBVWWW*-I	MT20	7.0	12.0	Edge	3.50
J	TMVW-t	MT20	5.0	8.0	2.50	3.75
L	BMVW1+p	MT20	4.0	4.0		
M	BMV+p	MT20	3.0	4.0		
N						
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BVMWWW-I	MT20	6.0	10.0	3.00	3.50
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	5.0	8.0	2.50	1.75
T	BMWW-t	MT20	5.0	6.0	2.50	2.25
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2215188

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2173	0	2173	0	0	5-8	5-8
L	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1520	1090 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0
L	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.10 (1)		10.00
B- C	-2900 / 0	-112.4	-112.4	0.26 (1)		4.72
C- D	-2652 / 0	-112.4	-112.4	0.25 (1)		4.90
D- E	-3201 / 0	-112.4	-112.4	0.27 (1)		3.70
E- F	-3213 / 0	-112.4	-112.4	0.28 (1)		3.70
F- G	-2847 / 0	-112.4	-112.4	0.26 (1)		3.91
G- H	-3189 / 0	-112.4	-112.4	0.24 (1)		4.57
H- I	-4701 / 0	-112.4	-112.4	0.35 (1)		3.78
I- J	-4280 / 0	-112.4	-112.4	0.20 (1)		4.07
J- K	0 / 34	-112.4	-112.4	0.10 (1)		10.00
U- B	-2130 / 0	0.0	0.0	0.22 (1)		5.81
L- J	-1961 / 0	0.0	0.0	0.20 (1)		6.01
U- T	0 / 0	-18.5	-18.5	0.09 (4)		10.00
T- S	0 / 2618	-18.5	-18.5	0.48 (1)		10.00
S- R	0 / 60	-18.5	-18.5	0.08 (4)		10.00
R- Q	0 / 27	0.0	0.0	0.08 (1)		10.00
Q- E	-432 / 0	0.0	0.0	0.12 (1)		7.81
Q- P	0 / 3210	-18.5	-18.5	0.32 (1)		10.00
P- O	0 / 3535	-18.5	-18.5	0.34 (1)		10.00
O- N	0 / 3535	-18.5	-18.5	0.31 (1)		10.00
M- N	0 / 15	0.0	0.0	0.41 (1)		10.00
N- I	-188 / 0	0.0	0.0	0.32 (1)		7.81
M- L	0 / 303	-18.5	-18.5	0.06 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-356 / 0	0.08 (1)
C- S	-334 / 0	0.21 (1)
S- D	-350 / 0	0.20 (1)
S- Q	0 / 2371	0.38 (1)
D- Q	0 / 1427	0.32 (1)
Q- F	0 / 25	0.01 (4)
F- P	-594 / 0	0.39 (1)
P- G	0 / 1071	0.24 (1)
P- H	-846 / 0	0.35 (1)
O- H	0 / 70	0.02 (4)
B- T	0 / 2656	0.60 (1)
N- L	-366 / 0	0.05 (1)
N- J	0 / 3758	0.85 (1)
H- N	0 / 848	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 988 (0.38")

CSI: TC=0.35/1.00 (H-I:1) , BC=0.48/1.00 (S-T:1) , WB=0.85/1.00 (J-N:1) , SSI=0.42/1.00 (I-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.78 (J) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T3S	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:c3jyj23uDijq_8pvRKbkZpy75XW-3K066DmbzMa0uQcNfA9NWAI_CMnsma5_OuPM0cz31_N

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215188

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423566	T4	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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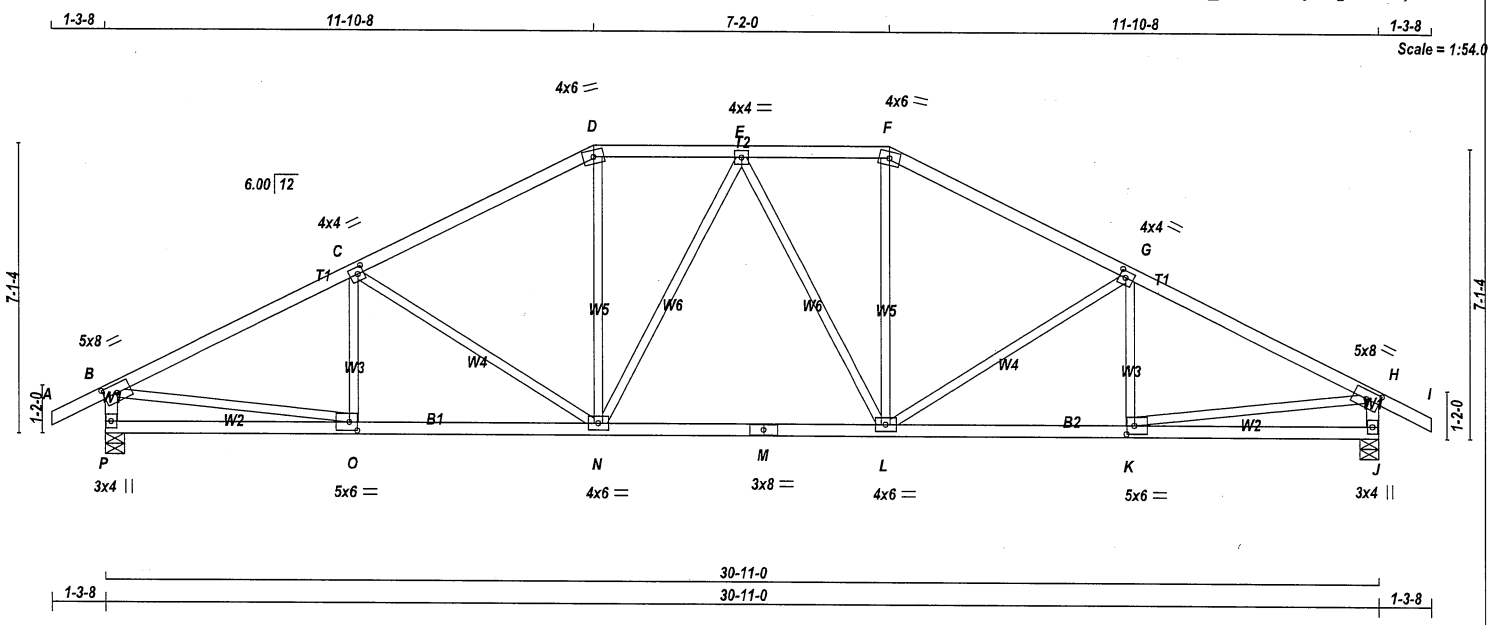

TOTAL WEIGHT = 128 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	4.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0	2.50	4.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2177	0	2177	0	0	5-8	5-8
J	2177	0	2177	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1523	1092 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	1523	1092 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	O- C	-267 / 26		0.07 (1)
B- C	-2946 / 0	-112.4	-112.4	0.68 (1)	3.38	C- N	-581 / 0		0.56 (1)
C- D	-2474 / 0	-112.4	-112.4	0.61 (1)	3.73	N- D	0 / 682		0.15 (1)
D- E	-2190 / 0	-112.4	-112.4	0.22 (1)	4.42	N- E	-270 / 0		0.32 (1)
E- F	-2190 / 0	-112.4	-112.4	0.22 (1)	4.42	E- L	-270 / 0		0.32 (1)
F- G	-2474 / 0	-112.4	-112.4	0.61 (1)	3.73	L- F	0 / 682		0.15 (1)
G- H	-2946 / 0	-112.4	-112.4	0.68 (1)	3.38	L- G	-581 / 0		0.56 (1)
H- I	0 / 34	-112.4	-112.4	0.15 (1)	10.00	K- G	-267 / 26		0.07 (1)
P- B	-2127 / 0	0.0	0.0	0.21 (1)	5.81	B- O	0 / 2693		0.61 (1)
J- H	-2127 / 0	0.0	0.0	0.21 (1)	5.81	K- H	0 / 2693		0.61 (1)
P- O	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
O- N	0 / 2666	-18.5	-18.5	0.52 (1)	10.00				
N- M	0 / 2312	-18.5	-18.5	0.46 (1)	10.00				
M- L	0 / 2312	-18.5	-18.5	0.46 (1)	10.00				
L- K	0 / 2666	-18.5	-18.5	0.52 (1)	10.00				
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.68/1.00 (B-C:1) , BC=0.52/1.00 (N-O:1) , WB=0.61/1.00 (B-O:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.77 (M) (INPUT = 1.00)

Structural component only
DWG# T-2215218

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T4S	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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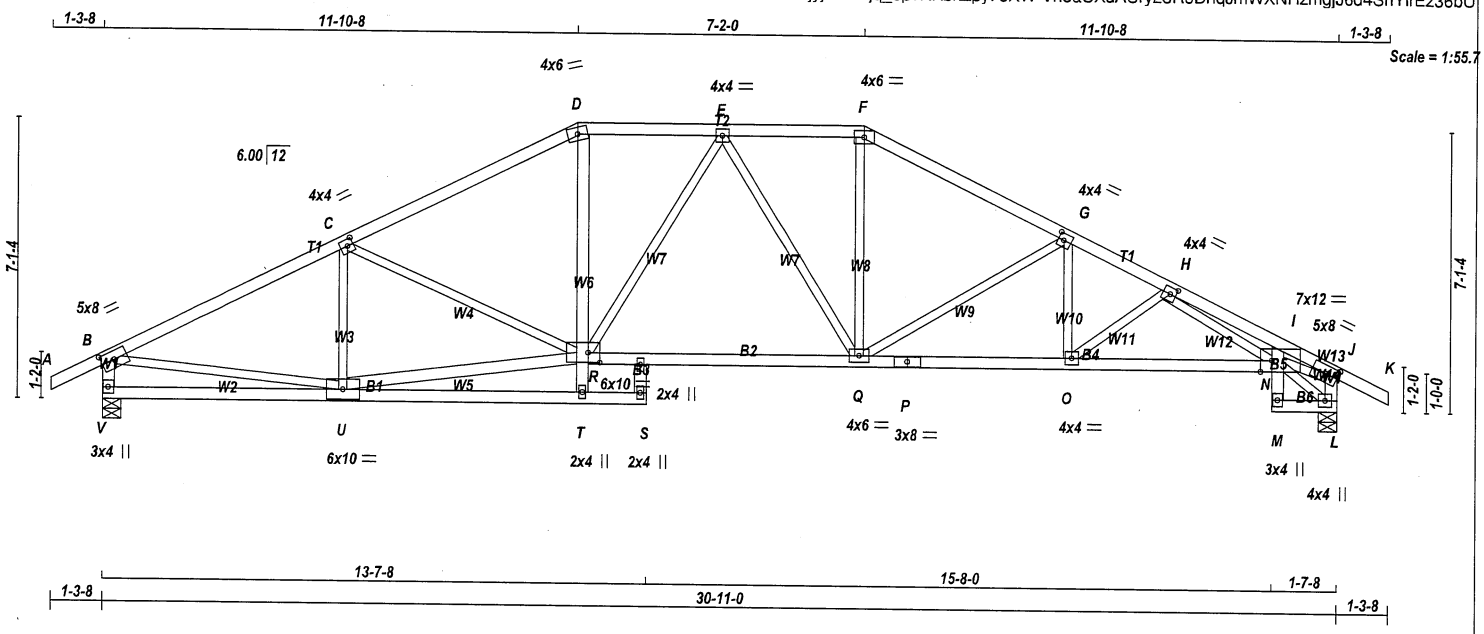

TOTAL WEIGHT = 139 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - D	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	4.00
C, G, H						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-l	MT20	4.0	6.0		
I						
I	TMBVWWW*-I	MT20	7.0	12.0	Edge	3.50
J	TMVW-t	MT20	5.0	8.0	2.50	4.00
L	BMVW1+p	MT20	4.0	4.0		
M	BMV+p	MT20	3.0	4.0		
N						
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BWMWWW*-I	MT20	6.0	10.0	3.00	3.50
S	NP+w	MT20	2.0	4.0		
T	BMW+w	MT20	2.0	4.0		
U	BMWWW-t	MT20	6.0	10.0		
V	BMV1+p	MT20	3.0	4.0		
W	NP+w	MT20	2.0	4.0		

Structural component only
DWG# T-2215189

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2192	0	2192	0	0	5-8	5-8
L	2192	0	2192	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1535	1090 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1535	1094 / 0	0 / 0	0 / 0	0 / 0	440 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	1	0.15 (1)	10.00
B- C	-2979 / 0	-112.4	-112.4	1	0.69 (1)	3.36
C- D	-2900 / 0	-112.4	-112.4	1	0.66 (1)	3.42
D- E	-2588 / 0	-112.4	-112.4	1	0.24 (1)	4.10
E- F	-2583 / 0	-112.4	-112.4	1	0.24 (1)	4.11
F- G	-2893 / 0	-112.4	-112.4	1	0.49 (1)	3.66
G- H	-3748 / 0	-112.4	-112.4	1	0.40 (1)	3.30
H- I	-4704 / 0	-112.4	-112.4	1	0.58 (1)	2.72
I- J	-4215 / 0	-112.4	-112.4	1	0.42 (1)	3.08
J- K	0 / 34	-112.4	-112.4	1	0.15 (1)	10.00
V- B	-2145 / 0	0.0	0.0	1	0.22 (1)	5.79
L- J	-1951 / 0	0.0	0.0	1	0.20 (1)	6.03
V- U	0 / 0	-18.5	-18.5	1	0.18 (4)	10.00
U- T	0 / 61	-18.5	-18.5	1	0.18 (4)	10.00
T- S	0 / 0	-18.5	-18.5	1	0.06 (4)	10.00
R- Q	0 / 2728	-18.5	-18.5	1	0.55 (1)	10.00
Q- P	0 / 3379	-18.5	-18.5	1	0.65 (1)	10.00
P- O	0 / 3379	-18.5	-18.5	1	0.65 (1)	10.00
O- N	0 / 3735	-18.5	-18.5	1	0.68 (1)	10.00
M- N	0 / 15	0.0	0.0	1	0.46 (1)	10.00
N- I	-116 / 0	0.0	0.0	1	0.35 (1)	7.81
M- L	0 / 336	-18.5	-18.5	1	0.06 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-696 / 0	0.18 (1)
T- R	0 / 80	0.10 (1)
R- D	0 / 870	0.23 (1)
Q- F	0 / 885	0.20 (1)
Q- G	-954 / 0	0.64 (1)
O- G	0 / 359	0.08 (1)
B- U	0 / 2722	0.61 (1)
U- R	0 / 2672	0.43 (1)
C- R	-150 / 0	0.12 (1)
R- E	-271 / 0	0.23 (1)
E- Q	-285 / 0	0.24 (1)
N- L	-405 / 0	0.06 (1)
N- J	0 / 3693	0.83 (1)
O- H	-449 / 0	0.09 (1)
H- N	0 / 634	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 956 (0.39")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.68/1.00 (N-O:1) , WB=0.83/1.00 (J-N:1) , SSI=0.46/1.00 (I-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 1.00 (P) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T4S	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215189

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T5	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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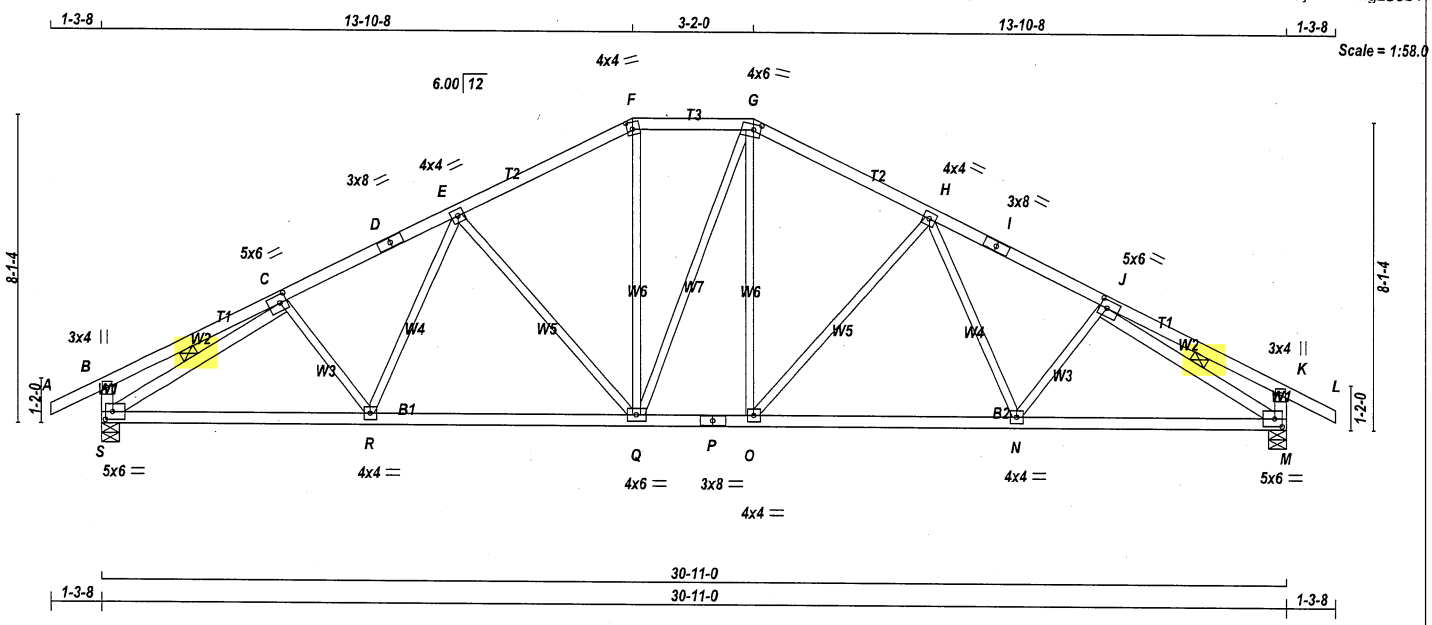

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	2.25	1.75
G	TTWW-m	MT20	4.0	6.0	1.75	2.25
H	TMWW-t	MT20	4.0	4.0		
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.25
N, O, R						
N	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	4.0	6.0		
S	BMVW1-t	MT20	5.0	6.0	2.50	2.25

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215190

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2177	0	2177	0	0	5-8	5-8
M	2177	0	2177	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1523	1092 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
M	1523	1092 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-S, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	C- R	-86 / 44	0.02 (1)	
B- C	0 / 21	-112.4	-112.4	0.30 (1)	10.00	R- E	0 / 230	0.05 (1)	
C- D	-2807 / 0	-112.4	-112.4	0.36 (1)	3.86	E- Q	-685 / 0	0.70 (1)	
D- E	-2807 / 0	-112.4	-112.4	0.36 (1)	3.86	Q- F	0 / 611	0.14 (1)	
E- F	-2240 / 0	-112.4	-112.4	0.33 (1)	4.27	Q- G	0 / 5	0.00 (1)	
F- G	-1990 / 0	-112.4	-112.4	0.19 (1)	4.63	O- G	0 / 606	0.14 (1)	
G- H	-2239 / 0	-112.4	-112.4	0.33 (1)	4.27	O- H	-687 / 0	0.70 (1)	
H- I	-2808 / 0	-112.4	-112.4	0.36 (1)	3.86	H- N	0 / 233	0.05 (1)	
I- J	-2808 / 0	-112.4	-112.4	0.36 (1)	3.86	N- J	-86 / 44	0.02 (1)	
J- K	0 / 21	-112.4	-112.4	0.30 (1)	10.00	S- C	-3106 / 0	0.52 (1)	
K- L	0 / 34	-112.4	-112.4	0.15 (1)	10.00	J- M	-3107 / 0	0.52 (1)	
S- B	-368 / 0	0.0	0.0	0.04 (1)	7.81				
M- K	-368 / 0	0.0	0.0	0.04 (1)	7.81				
S- R	0 / 2563	-18.5	-18.5	0.52 (1)	10.00				
R- Q	0 / 2424	-18.5	-18.5	0.49 (1)	10.00				
Q- P	0 / 1989	-18.5	-18.5	0.39 (1)	10.00				
P- O	0 / 1989	-18.5	-18.5	0.39 (1)	10.00				
O- N	0 / 2424	-18.5	-18.5	0.50 (1)	10.00				
N- M	0 / 2564	-18.5	-18.5	0.52 (1)	10.00				

TOTAL WEIGHT = 137 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.36/1.00 (H-J:1) , BC=0.52/1.00 (M-N:1) , WB=0.70/1.00 (H-O:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.70 (J) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T6C	5	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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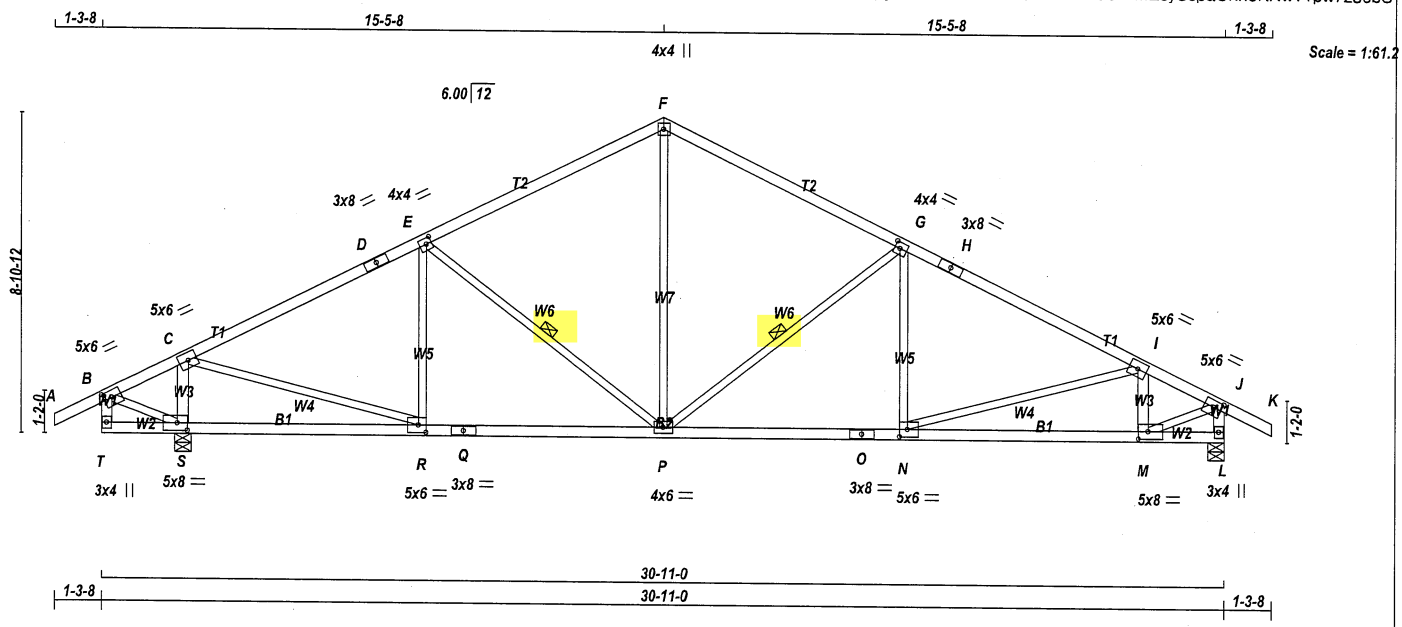

TOTAL WEIGHT = 5 X 126 = 630 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW+p	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.25	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	8.0	2.50	3.25
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMWW1-t	MT20	5.0	8.0	2.50	3.25
T	BMV+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215191

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2346	0	2346	0	0	5-8	5-8
L	2007	0	2007	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1641	1177 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
L	1404	1007 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING =3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-P, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	P- F	0 / 942	0.21 (1)
B- C	0 / 213	-112.4	-112.4	0.45 (1)	10.00	P- G	-928 / 0	0.45 (1)
C- D	-2021 / 0	-112.4	-112.4	0.65 (1)	4.00	N- G	0 / 127	0.04 (4)
D- E	-2021 / 0	-112.4	-112.4	0.65 (1)	4.00	N- I	0 / 190	0.04 (1)
E- F	-1741 / 0	-112.4	-112.4	0.66 (1)	4.20	M- I	-709 / 0	0.08 (1)
F- G	-1742 / 0	-112.4	-112.4	0.67 (1)	4.18	E- P	-377 / 0	0.18 (1)
G- H	-2497 / 0	-112.4	-112.4	0.73 (1)	3.56	R- E	-472 / 0	0.22 (1)
H- I	-2497 / 0	-112.4	-112.4	0.73 (1)	3.56	C- R	0 / 2041	0.46 (1)
I- J	-2263 / 0	-112.4	-112.4	0.32 (1)	4.24	S- C	-2162 / 0	0.24 (1)
J- K	0 / 34	-112.4	-112.4	0.15 (1)	10.00	B- S	-145 / 0	0.02 (1)
T- B	-22 / 0	0.0	0.0	0.00 (1)	7.81	M- J	0 / 2216	0.50 (1)
L- J	-1997 / 0	0.0	0.0	0.20 (1)	5.97			
T- S	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
S- R	-136 / 0	-18.5	-18.5	0.12 (4)	6.25			
R- Q	0 / 1820	-18.5	-18.5	0.39 (1)	10.00			
Q- P	0 / 1820	-18.5	-18.5	0.39 (1)	10.00			
P- O	0 / 2251	-18.5	-18.5	0.46 (1)	10.00			
O- N	0 / 2251	-18.5	-18.5	0.46 (1)	10.00			
N- M	0 / 2069	-18.5	-18.5	0.42 (1)	10.00			
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.73/1.00 (G-I:1) , BC=0.46/1.00 (N-P:1) , WB=0.50/1.00 (J-M:1) , SSI=0.32/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.64 (Q) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T7	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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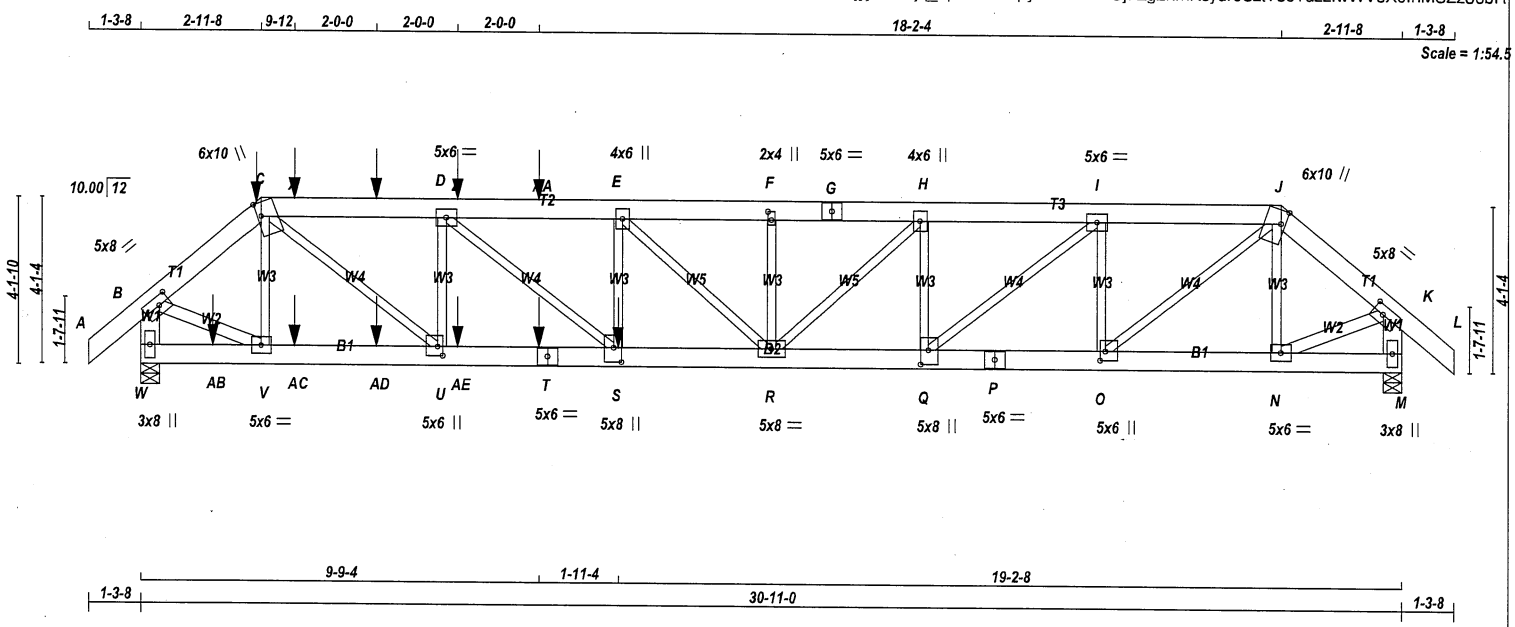

TOTAL WEIGHT = 2 X 174 = 348 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(61.0)
G- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(0.0)
T- P	2	12	SIDE(183.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- S	1	3	SIDE(553.2)
H- Q	1	3	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2215192

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	4728	0	4728	0	0	5-8	5-8
M	3425	0	3425	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3304	2392 / 0	0 / 0	0 / 0	0 / 0	912 / 0	0 / 0
M	2394	1731 / 0	0 / 0	0 / 0	0 / 0	663 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FROM-TO	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00	V- C	-1125 / 0	0.13 (1)
B- C	-4773 / 0	-112.4	-112.4	0.07 (1)	5.25	S- E	0 / 470	0.06 (1)
C- X	-7710 / 0	-112.4	-112.4	0.22 (1)	4.19	E- R	-1322 / 0	0.29 (1)
X- Y	-7710 / 0	-112.4	-112.4	0.22 (1)	4.19	R- F	-443 / 0	0.05 (1)
Y- D	-7710 / 0	-112.4	-112.4	0.22 (1)	4.19	R- H	0 / 2136	0.26 (1)
D- Z	-10485 / 0	-112.4	-112.4	0.32 (1)	3.58	Q- H	-1913 / 0	0.23 (1)
Z-AA	-10485 / 0	-112.4	-112.4	0.32 (1)	3.58	N- J	-792 / 0	0.09 (1)
AA- E	-10485 / 0	-112.4	-112.4	0.32 (1)	3.58	B- V	0 / 3879	0.34 (1)
E- F	-9537 / 0	-112.4	-112.4	0.23 (1)	3.83	N- K	0 / 2734	0.24 (1)
F- G	-9537 / 0	-112.4	-112.4	0.22 (1)	3.85	D- S	0 / 3630	0.45 (1)
G- H	-9537 / 0	-112.4	-112.4	0.22 (1)	3.85	C- U	0 / 5287	0.65 (1)
H- I	-8005 / 0	-112.4	-112.4	0.18 (1)	4.18	U- D	-3193 / 0	0.38 (1)
I- J	-5658 / 0	-112.4	-112.4	0.13 (1)	4.86	O- J	0 / 4015	0.50 (1)
J- K	-3363 / 0	-112.4	-112.4	0.06 (1)	6.02	Q- I	0 / 3071	0.38 (1)
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00	O- I	-2491 / 0	0.30 (1)
W- B	-4734 / 0	0.0	0.0	0.17 (1)	6.65			
M- K	-3433 / 0	0.0	0.0	0.12 (1)	7.53			
W-AB	0 / 0	-18.5	-18.5	0.05 (1)	10.00			
AB- V	0 / 0	-18.5	-18.5	0.05 (1)	10.00			
V-AC	0 / 3613	-18.5	-18.5	0.29 (1)	10.00			
AC-AD	0 / 3613	-18.5	-18.5	0.29 (1)	10.00			
AD- U	0 / 3613	-18.5	-18.5	0.29 (1)	10.00			
U-AE	0 / 7710	-18.5	-18.5	0.60 (1)	10.00			
AE- T	0 / 7710	-18.5	-18.5	0.60 (1)	10.00			
T- S	0 / 7710	-18.5	-18.5	0.60 (1)	10.00			
S- R	0 / 10485	-18.5	-18.5	0.78 (1)	10.00			
R- Q	0 / 8005	-18.5	-18.5	0.56 (1)	10.00			
Q- P	0 / 5658	-18.5	-18.5	0.41 (1)	10.00			
P- O	0 / 5658	-18.5	-18.5	0.41 (1)	10.00			
O- N	0 / 2546	-18.5	-18.5	0.21 (1)	10.00			
N- M	0 / 0	-18.5	-18.5	0.04 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-159	-159	---	FRONT	VERT	SNOW	---	C1
S	11-8-8	-1968	-1968	---	BACK	VERT	TOTAL	---	C1
T	9-9-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
X	3-9-4	-100	-100	---	BACK	VERT	TOTAL	---	C1
Y	5-9-4	-93	-93	---	BACK	VERT	TOTAL	---	C1
Z	7-9-4	-93	-93	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.78/1.00 (R-S:1) , WB=0.65/1.00 (C-U:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (U) (INPUT = 0.90)
JSI METAL= 0.76 (T) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	6.0	10.0	4.00	1.25
D	TMWW-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	6.0	10.0	4.00	1.25
K	TMVW-t	MT20	5.0	8.0	2.50	3.25
M	BMV1+p	MT20	3.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	5.0	6.0	2.75	1.50
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	8.0	4.25	2.25
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	5.0	8.0	4.25	2.25
T	BS-t	MT20	5.0	6.0		
U	BMWW+t	MT20	5.0	6.0	2.75	1.50
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	9-9-4	-93	-93	---	BACK	VERT	TOTAL	---	C1
AB	1-9-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	3-9-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	5-9-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	7-9-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215192

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T8	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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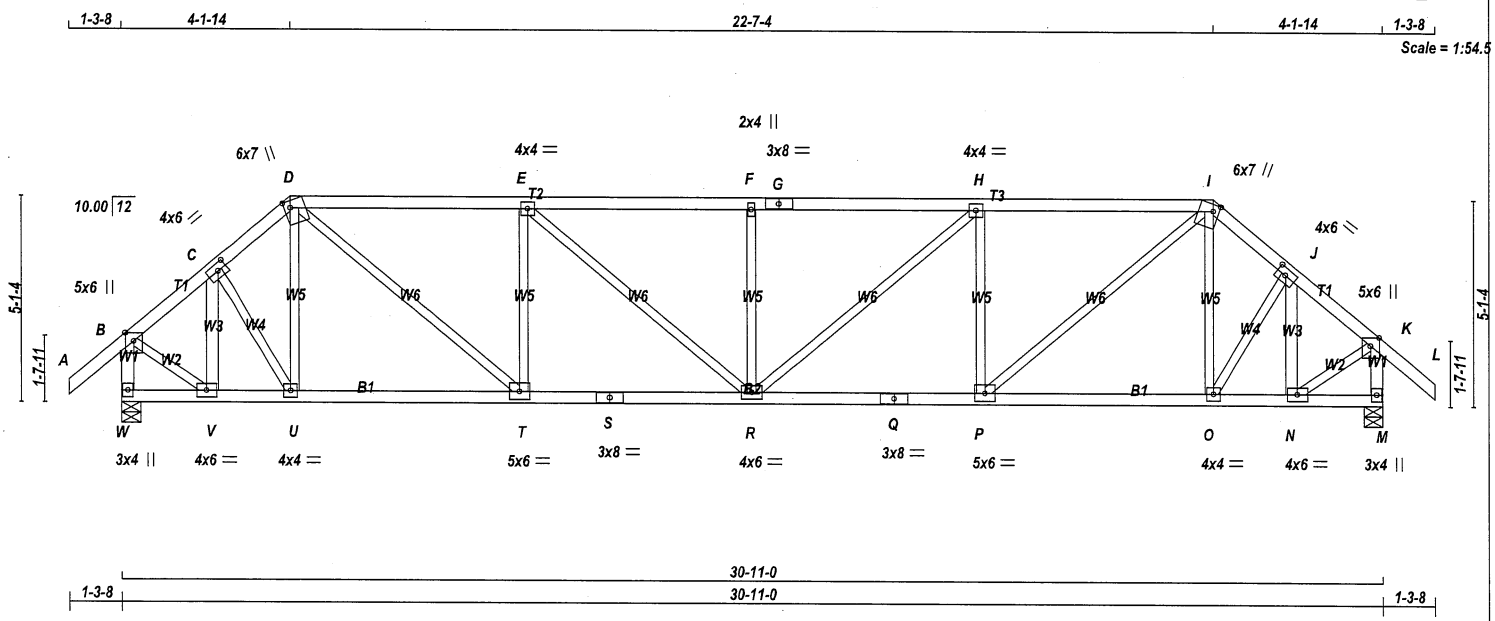

TOTAL WEIGHT = 138 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW+m	MT20	6.0	7.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.00	1.75
J	TMWW-t	MT20	4.0	6.0	2.00	2.75
K	TMVW+p	MT20	5.0	6.0	Edge	
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	8.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW-t	MT20	4.0	4.0		
V	BMWW-t	MT20	4.0	6.0		
W	BMV1+p	MT20	3.0	4.0		

Structural component only
DWG# T-2215193

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2180	0	2180	0	0	5-8	5-8
M	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
M	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	-1722 / 0	-112.4	-112.4	0.10 (1)		4.99
C- D	-2026 / 0	-112.4	-112.4	0.11 (1)		4.68
D- E	-2908 / 0	-112.4	-112.4	0.76 (1)		3.29
E- F	-3305 / 0	-112.4	-112.4	0.82 (1)		3.04
F- G	-3305 / 0	-112.4	-112.4	0.82 (1)		3.04
G- H	-3305 / 0	-112.4	-112.4	0.82 (1)		3.04
H- I	-2908 / 0	-112.4	-112.4	0.76 (1)		3.29
I- J	-2026 / 0	-112.4	-112.4	0.11 (1)		4.68
J- K	-1722 / 0	-112.4	-112.4	0.10 (1)		4.99
K- L	0 / 50	-112.4	-112.4	0.15 (1)		10.00
W- B	-2152 / 0	0.0	0.0	0.23 (1)		5.78
M- K	-2152 / 0	0.0	0.0	0.23 (1)		5.78
W- V	0 / 0	-18.5	-18.5	0.03 (4)		10.00
V- U	0 / 1333	-18.5	-18.5	0.28 (1)		10.00
U- T	0 / 1531	-18.5	-18.5	0.31 (1)		10.00
T- S	0 / 2909	-18.5	-18.5	0.52 (1)		10.00
S- R	0 / 2909	-18.5	-18.5	0.52 (1)		10.00
R- Q	0 / 2909	-18.5	-18.5	0.52 (1)		10.00
Q- P	0 / 2909	-18.5	-18.5	0.52 (1)		10.00
P- O	0 / 1531	-18.5	-18.5	0.31 (1)		10.00
O- N	0 / 1333	-18.5	-18.5	0.28 (1)		10.00
N- M	0 / 0	-18.5	-18.5	0.03 (4)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-780 / 0	0.12 (1)
C- U	0 / 374	0.08 (1)
U- D	-211 / 0	0.08 (1)
D- T	0 / 1799	0.40 (1)
T- E	-1055 / 0	0.41 (1)
E- R	0 / 523	0.12 (1)
R- F	-580 / 0	0.22 (1)
R- H	0 / 523	0.12 (1)
P- H	-1055 / 0	0.41 (1)
P- I	0 / 1799	0.40 (1)
O- I	-211 / 0	0.08 (1)
O- J	0 / 374	0.08 (1)
N- J	-780 / 0	0.12 (1)
B- V	0 / 1545	0.35 (1)
N- K	0 / 1545	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.82/1.00 (E-F:1) , BC=0.52/1.00 (R-T:1) , WB=0.41/1.00 (E-T:1) , SSI=0.30/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.91 (S) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423564	T8	1	1	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215193

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T8C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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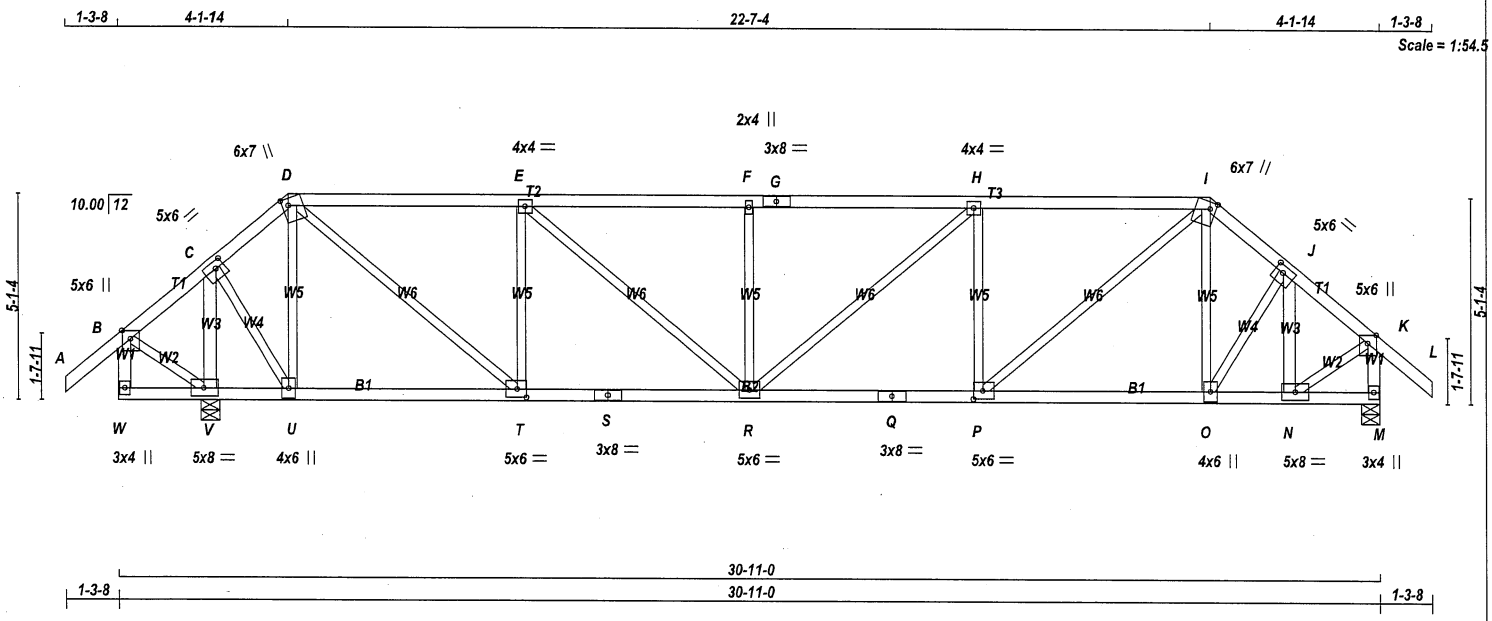

TOTAL WEIGHT = 138 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	5.0	6.0	2.00	2.50
D	TTWW+m	MT20	6.0	7.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.00	1.75
J	TMWW-t	MT20	5.0	6.0	2.00	2.50
K	TMVW+p	MT20	5.0	6.0	Edge	
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	5.0	8.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	3.0	8.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMWW+t	MT20	4.0	6.0		
V	BMWW1-t	MT20	5.0	8.0		
W	BMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2215194

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	2349	0	2349	0	0	5-8	5-8
M	2010	0	2010	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1643	1179 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
M	1406	1009 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	0 / 244	-112.4	-112.4	0.16 (1)		10.00
C- D	-825 / 0	-112.4	-112.4	0.13 (1)		6.25
D- E	-2168 / 0	-112.4	-112.4	0.67 (1)		3.84
E- F	-2762 / 0	-112.4	-112.4	0.74 (1)		3.38
F- G	-2762 / 0	-112.4	-112.4	0.74 (1)		3.38
G- H	-2762 / 0	-112.4	-112.4	0.74 (1)		3.38
H- I	-2562 / 0	-112.4	-112.4	0.72 (1)		3.53
I- J	-1832 / 0	-112.4	-112.4	0.10 (1)		4.87
J- K	-1567 / 0	-112.4	-112.4	0.09 (1)		5.19
K- L	0 / 50	-112.4	-112.4	0.15 (1)		10.00
W- B	-3 / 16	0.0	0.0	0.00 (4)		7.81
M- K	-1983 / 0	0.0	0.0	0.21 (1)		5.99
W- V	0 / 0	-18.5	-18.5	0.07 (1)		10.00
V- U	-168 / 0	-18.5	-18.5	0.09 (4)		6.25
U- T	0 / 583	-18.5	-18.5	0.16 (4)		10.00
T- S	0 / 2169	-18.5	-18.5	0.40 (1)		10.00
S- R	0 / 2169	-18.5	-18.5	0.40 (1)		10.00
R- Q	0 / 2562	-18.5	-18.5	0.46 (1)		10.00
Q- P	0 / 2562	-18.5	-18.5	0.46 (1)		10.00
P- O	0 / 1384	-18.5	-18.5	0.28 (1)		10.00
O- N	0 / 1215	-18.5	-18.5	0.26 (1)		10.00
N- M	0 / 0	-18.5	-18.5	0.03 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-2180 / 0	0.32 (1)
C- U	0 / 1435	0.32 (1)
U- D	-1145 / 0	0.44 (1)
D- T	0 / 2071	0.47 (1)
T- E	-1227 / 0	0.47 (1)
E- R	0 / 782	0.18 (1)
R- F	-580 / 0	0.22 (1)
R- H	0 / 263	0.06 (1)
P- H	-887 / 0	0.34 (1)
P- I	0 / 1539	0.35 (1)
O- I	-167 / 9	0.06 (1)
O- J	0 / 319	0.07 (1)
N- J	-708 / 0	0.10 (1)
B- V	-194 / 0	0.03 (1)
N- K	0 / 1408	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 .

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.74/1.00 (E-F:1) , BC=0.46/1.00 (P-R:1) , WB=0.47/1.00 (E-T:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.81 (S) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423564	T8C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215194

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T9	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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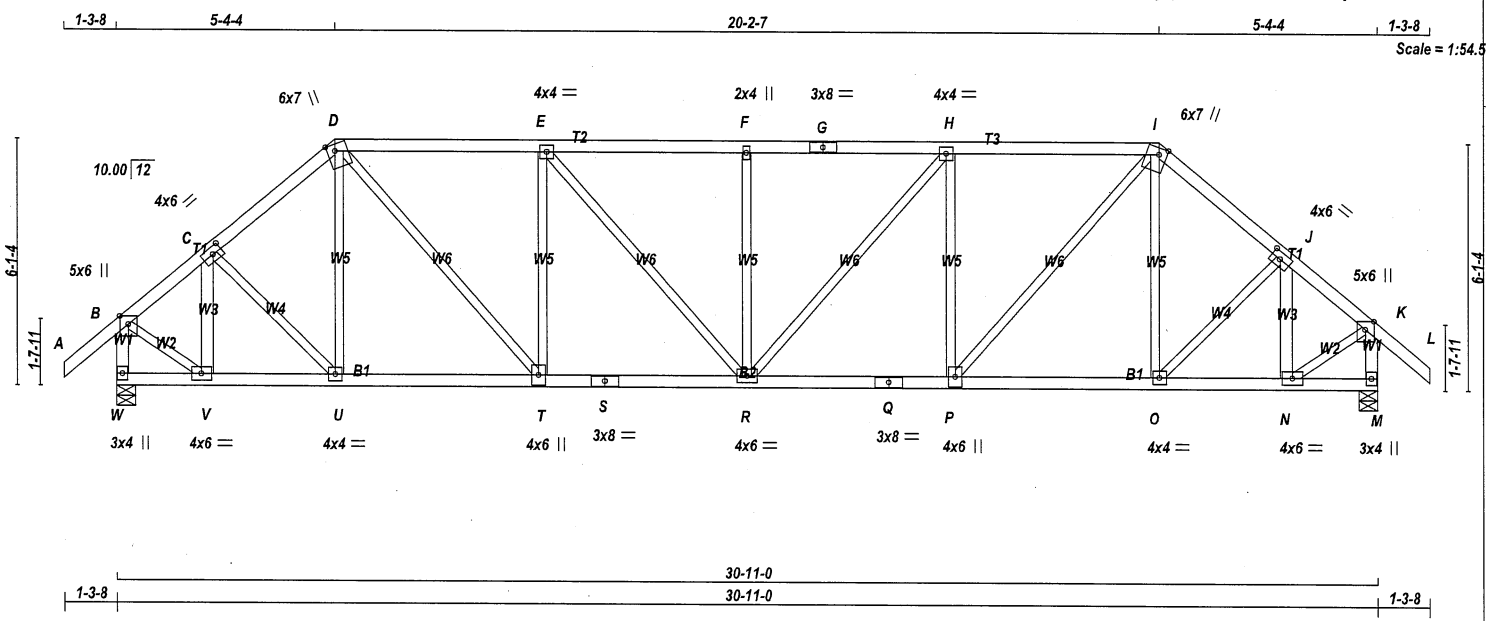

TOTAL WEIGHT = 145 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW+m	MT20	6.0	7.0	2.00	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.00	2.25
J	TMWW-t	MT20	4.0	6.0	2.00	2.75
K	TMVW+p	MT20	5.0	6.0	Edge	
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	8.0		
T	BMWW+t	MT20	4.0	6.0		
U	BMWW-t	MT20	4.0	4.0		
V	BMWW-t	MT20	4.0	6.0		
W	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215195

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2180	0	2180	0	0	5-8	5-8
M	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
M	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00		V- C	-759 / 0	0.11 (1)
B- C	-1731 / 0	-112.4	-112.4	0.16 (1)	4.91		C- U	0 / 289	0.07 (1)
C- D	-2053 / 0	-112.4	-112.4	0.19 (1)	4.57		U- D	-111 / 29	0.06 (1)
D- E	-2466 / 0	-112.4	-112.4	0.55 (1)	3.80		D- T	0 / 1378	0.31 (1)
E- F	-2727 / 0	-112.4	-112.4	0.58 (1)	3.61		T- E	-941 / 0	0.55 (1)
F- G	-2727 / 0	-112.4	-112.4	0.58 (1)	3.61		E- R	0 / 399	0.09 (1)
G- H	-2727 / 0	-112.4	-112.4	0.58 (1)	3.61		R- F	-517 / 0	0.30 (1)
H- I	-2466 / 0	-112.4	-112.4	0.55 (1)	3.80		R- H	0 / 399	0.09 (1)
I- J	-2053 / 0	-112.4	-112.4	0.19 (1)	4.57		P- H	-941 / 0	0.55 (1)
J- K	-1731 / 0	-112.4	-112.4	0.16 (1)	4.91		P- I	0 / 1378	0.31 (1)
K- L	0 / 50	-112.4	-112.4	0.15 (1)	10.00		O- I	-111 / 29	0.06 (1)
W- B	-2155 / 0	0.0	0.0	0.23 (1)	5.78		O- J	0 / 289	0.07 (1)
M- K	-2155 / 0	0.0	0.0	0.23 (1)	5.78		N- J	-759 / 0	0.11 (1)
							B- V	0 / 1565	0.35 (1)
W- V	0 / 0	-18.5	-18.5	0.02 (1)	10.00		N- K	0 / 1565	0.35 (1)
V- U	0 / 1350	-18.5	-18.5	0.27 (1)	10.00				
U- T	0 / 1555	-18.5	-18.5	0.30 (1)	10.00				
T- S	0 / 2466	-18.5	-18.5	0.44 (1)	10.00				
S- R	0 / 2466	-18.5	-18.5	0.44 (1)	10.00				
R- Q	0 / 2466	-18.5	-18.5	0.44 (1)	10.00				
Q- P	0 / 2466	-18.5	-18.5	0.44 (1)	10.00				
P- O	0 / 1555	-18.5	-18.5	0.30 (1)	10.00				
O- N	0 / 1350	-18.5	-18.5	0.27 (1)	10.00				
N- M	0 / 0	-18.5	-18.5	0.02 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.58/1.00 (F-H:1) , BC=0.44/1.00 (R-T:1) , WB=0.55/1.00 (H-P:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.75 (S) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423564	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:c3jyj23uDijq_8pvRKbkZpy75XW-Kx9rlaiw4hjBpMZN85QAmocLBBnvjuVzrd?03uz36bO

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215195

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T9C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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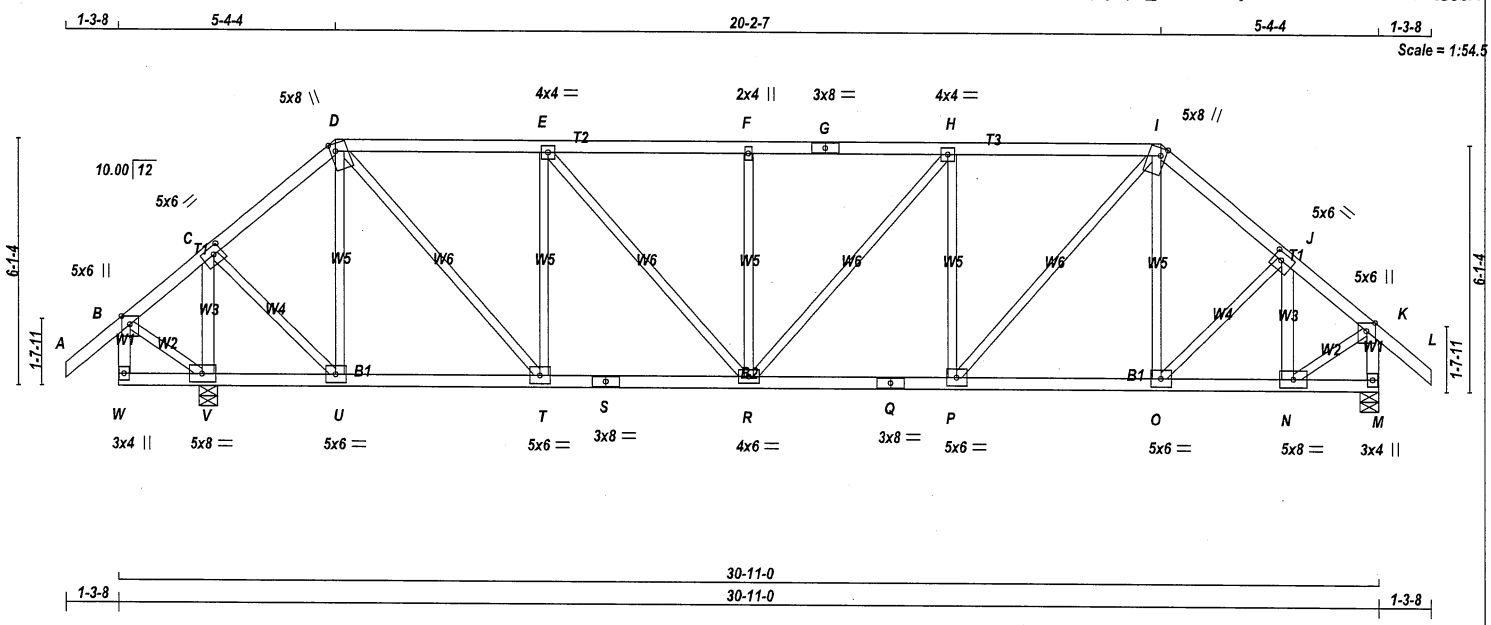

TOTAL WEIGHT = 145 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	5.0	6.0	2.25	2.50
D	TTWW+m	MT20	5.0	8.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	8.0	2.25	1.50
J	TMWW-t	MT20	5.0	6.0	2.25	2.50
K	TMVW+p	MT20	5.0	6.0	Edge	
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	5.0	8.0		
O, P, T, U						
O	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	8.0		
V	BMWW1-t	MT20	5.0	8.0		
W	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2215196

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	2349	0	2349	0	0	IN-SX 5-8	IN-SX 5-8
M	2010	0	2010	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1643	1179 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
M	1406	1009 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B-C	0 / 244	-112.4	-112.4	0.21 (1)		10.00
C-D	-1099 / 0	-112.4	-112.4	0.19 (1)		5.80
D-E	-1871 / 0	-112.4	-112.4	0.50 (1)		4.32
E-F	-2277 / 0	-112.4	-112.4	0.53 (1)		3.95
F-G	-2277 / 0	-112.4	-112.4	0.53 (1)		3.95
G-H	-2277 / 0	-112.4	-112.4	0.53 (1)		3.95
H-I	-2162 / 0	-112.4	-112.4	0.53 (1)		4.05
I-J	-1848 / 0	-112.4	-112.4	0.18 (1)		4.77
J-K	-1576 / 0	-112.4	-112.4	0.16 (1)		5.10
K-L	0 / 50	-112.4	-112.4	0.15 (1)		10.00
W-B	0 / 14	0.0	0.0	0.00 (4)		10.00
M-K	-1986 / 0	0.0	0.0	0.21 (1)		5.98
W-V	0 / 0	-18.5	-18.5	0.06 (1)		10.00
V-U	-160 / 0	-18.5	-18.5	0.07 (4)		6.25
U-T	0 / 806	-18.5	-18.5	0.17 (1)		10.00
T-S	0 / 1871	-18.5	-18.5	0.35 (1)		10.00
S-R	0 / 1871	-18.5	-18.5	0.35 (1)		10.00
R-Q	0 / 2162	-18.5	-18.5	0.39 (1)		10.00
Q-P	0 / 2162	-18.5	-18.5	0.39 (1)		10.00
P-O	0 / 1398	-18.5	-18.5	0.27 (1)		10.00
O-N	0 / 1232	-18.5	-18.5	0.25 (1)		10.00
N-M	0 / 0	-18.5	-18.5	0.02 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-2181 / 0	0.32 (1)
C-U	0 / 1374	0.31 (1)
U-D	-890 / 0	0.52 (1)
D-T	0 / 1610	0.36 (1)
T-E	-1113 / 0	0.65 (1)
E-R	0 / 622	0.14 (1)
R-F	-517 / 0	0.30 (1)
R-H	0 / 176	0.04 (1)
P-H	-773 / 0	0.45 (1)
P-I	0 / 1155	0.26 (1)
O-I	-74 / 39	0.04 (1)
O-J	0 / 235	0.05 (1)
N-J	-688 / 0	0.10 (1)
B-V	-185 / 0	0.03 (1)
N-K	0 / 1427	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.53/1.00 (F-H:1) , BC=0.39/1.00 (P-R:1) , WB=0.65/1.00 (E-T:1) , SSI=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.66 (S) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T9C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215196

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T10	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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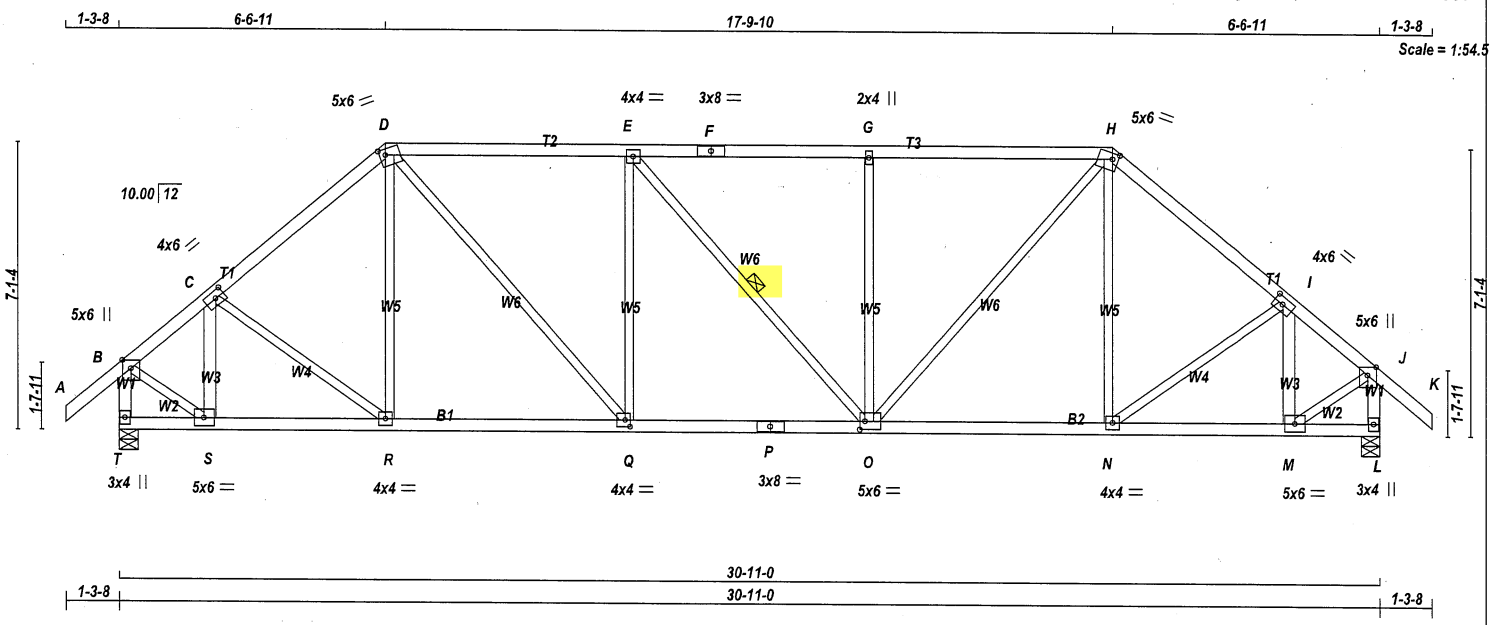

TOTAL WEIGHT = 143 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW-m	MT20	5.0	6.0	1.75	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	1.75
I	TMWW-t	MT20	4.0	6.0	2.00	2.75
J	TMVW+p	MT20	5.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2215197

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
						IN-SX	IN-SX
T	2180	0	2180	0	0	5-8	5-8
L	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-1746 / 0	-112.4	-112.4	0.29 (1)	4.72
C- D	-2047 / 0	-112.4	-112.4	0.35 (1)	4.41
D- E	-2277 / 0	-112.4	-112.4	0.71 (1)	3.71
E- F	-2275 / 0	-112.4	-112.4	0.70 (1)	3.71
F- G	-2275 / 0	-112.4	-112.4	0.70 (1)	3.71
G- H	-2275 / 0	-112.4	-112.4	0.70 (1)	3.72
H- I	-2048 / 0	-112.4	-112.4	0.35 (1)	4.41
I- J	-1745 / 0	-112.4	-112.4	0.29 (1)	4.72
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
T- B	-2159 / 0	0.0	0.0	0.23 (1)	5.78
L- J	-2159 / 0	0.0	0.0	0.23 (1)	5.78
T- S	0 / 0	-18.5	-18.5	0.03 (4)	10.00
S- R	0 / 1381	-18.5	-18.5	0.29 (1)	10.00
R- Q	0 / 1548	-18.5	-18.5	0.32 (1)	10.00
Q- P	0 / 2277	-18.5	-18.5	0.43 (1)	10.00
P- O	0 / 2277	-18.5	-18.5	0.43 (1)	10.00
O- N	0 / 1548	-18.5	-18.5	0.32 (1)	10.00
N- M	0 / 1381	-18.5	-18.5	0.29 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-758 / 0	0.11 (1)
C- R	0 / 205	0.05 (1)
R- D	-12 / 73	0.03 (4)
D- Q	0 / 1104	0.25 (1)
Q- E	-720 / 0	0.63 (1)
E- O	-3 / 0	0.00 (1)
O- G	-720 / 0	0.63 (1)
O- H	0 / 1101	0.25 (1)
N- H	-11 / 73	0.03 (4)
N- I	0 / 205	0.05 (1)
M- I	-759 / 0	0.11 (1)
B- S	0 / 1601	0.36 (1)
M- J	0 / 1600	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.71/1.00 (D-E:1) , BC=0.43/1.00 (O-Q:1) , WB=0.63/1.00 (E-Q:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.73 (P) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T10C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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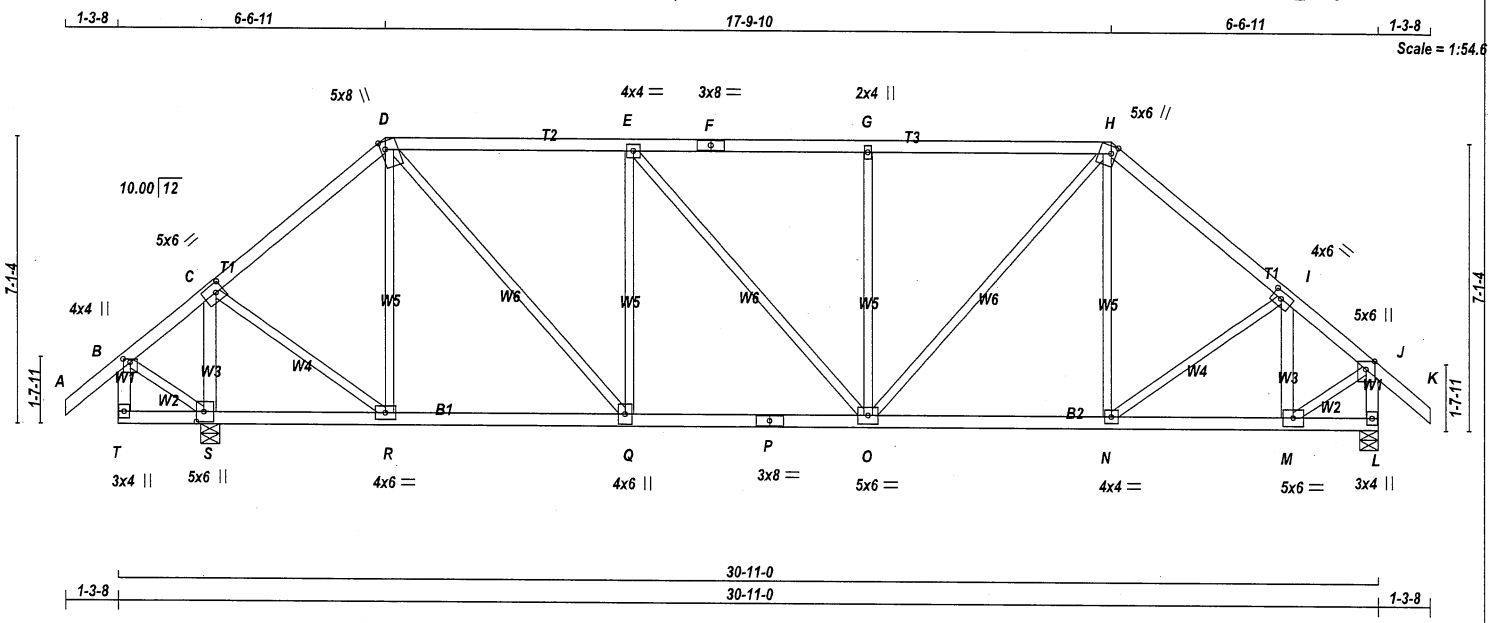

TOTAL WEIGHT = 143 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	5.0	8.0	Edge	1.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.75
J	TMVW+p	MT20	5.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	6.0		
S	BMWW1+t	MT20	5.0	6.0	3.00	2.25
T	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2215198

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2349	0	2349	0	0	5-8	5-8
L	2010	0	2010	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1643	1179 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
L	1406	1009 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	0 / 232	-112.4	-112.4	0.32 (1)		10.00
C- D	-1270 / 0	-112.4	-112.4	0.33 (1)		5.32
D- E	-1820 / 0	-112.4	-112.4	0.66 (1)		4.13
E- F	-1964 / 0	-112.4	-112.4	0.67 (1)		3.99
F- G	-1964 / 0	-112.4	-112.4	0.67 (1)		3.99
G- H	-1964 / 0	-112.4	-112.4	0.66 (1)		4.00
H- I	-1833 / 0	-112.4	-112.4	0.33 (1)		4.62
I- J	-1591 / 0	-112.4	-112.4	0.28 (1)		4.91
J- K	0 / 50	-112.4	-112.4	0.15 (1)		10.00
T- B	0 / 10	0.0	0.0	0.00 (4)		10.00
L- J	-1990 / 0	0.0	0.0	0.21 (1)		5.98
T- S	0 / 0	-18.5	-18.5	0.06 (1)		10.00
S- R	-133 / 0	-18.5	-18.5	0.10 (4)		6.25
R- Q	0 / 939	-18.5	-18.5	0.22 (1)		10.00
Q- P	0 / 1820	-18.5	-18.5	0.36 (1)		10.00
P- O	0 / 1820	-18.5	-18.5	0.36 (1)		10.00
O- N	0 / 1384	-18.5	-18.5	0.29 (1)		10.00
N- M	0 / 1262	-18.5	-18.5	0.27 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.03 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-2188 / 0	0.32 (1)
C- R	0 / 1330	0.30 (1)
R- D	-679 / 0	0.59 (1)
D- Q	0 / 1334	0.30 (1)
Q- E	-891 / 0	0.78 (1)
E- O	0 / 221	0.05 (1)
O- G	-721 / 0	0.63 (1)
O- H	0 / 878	0.20 (1)
N- H	0 / 82	0.03 (4)
N- I	0 / 150	0.03 (1)
B- S	-154 / 0	0.03 (1)
M- J	0 / 1463	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.67/1.00 (E-G:1) , BC=0.36/1.00 (O-Q:1) , WB=0.78/1.00 (E-Q:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.60 (P) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T11	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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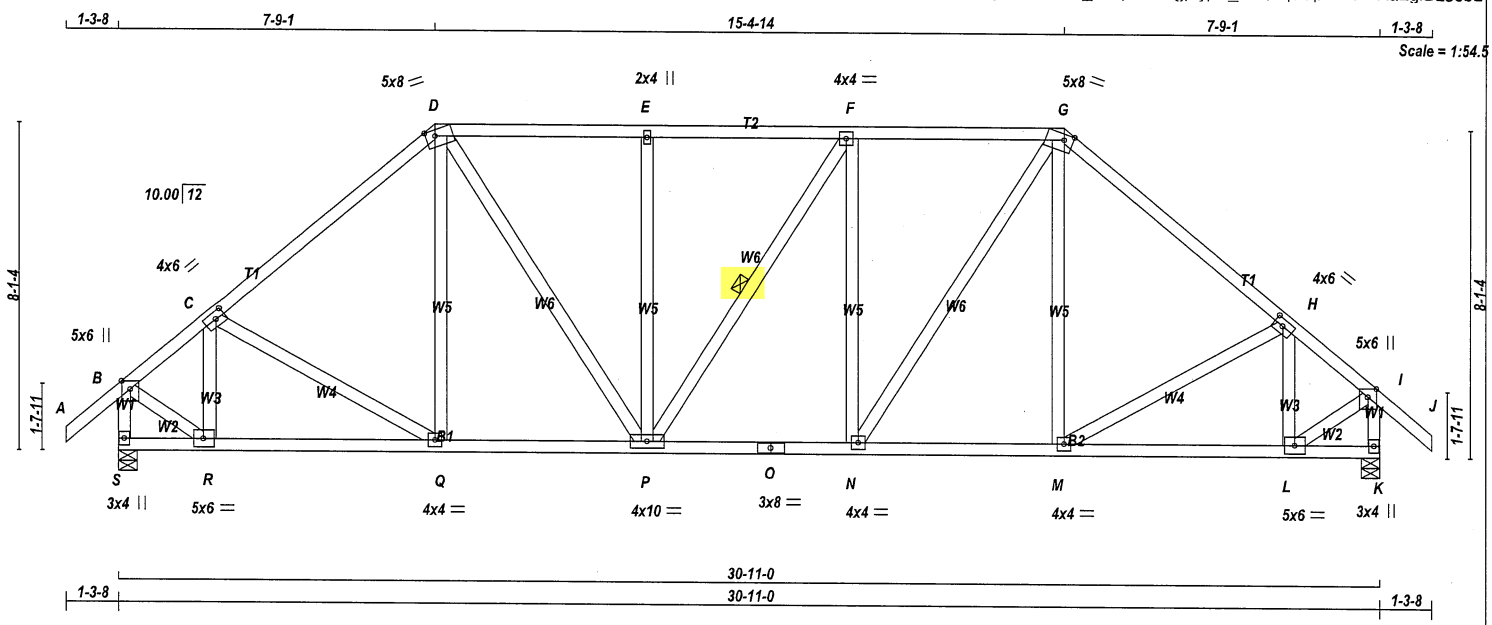

TOTAL WEIGHT = 173 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW-m	MT20	5.0	8.0	1.75	2.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	2.75
H	TMWW-t	MT20	4.0	6.0	2.00	2.75
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M, N, Q						
M	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	10.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2180	0	2180	0	0	5-8	5-8
K	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00		R- C	-759 / 0	0.11 (1)
B- C	-1771 / 0	-112.4	-112.4	0.47 (1)	4.39		C- Q	0 / 99	0.02 (1)
C- D	-2013 / 0	-112.4	-112.4	0.63 (1)	4.15		Q- D	0 / 98	0.02 (4)
D- E	-1989 / 0	-112.4	-112.4	0.49 (1)	4.24		M- G	0 / 98	0.02 (4)
E- F	-1989 / 0	-112.4	-112.4	0.49 (1)	4.23		M- H	0 / 99	0.02 (1)
F- G	-1990 / 0	-112.4	-112.4	0.49 (1)	4.23		L- H	-758 / 0	0.11 (1)
G- H	-2013 / 0	-112.4	-112.4	0.63 (1)	4.15		B- R	0 / 1656	0.27 (1)
H- I	-1771 / 0	-112.4	-112.4	0.47 (1)	4.39		L- I	0 / 1656	0.27 (1)
I- J	0 / 50	-112.4	-112.4	0.15 (1)	10.00		N- G	0 / 855	0.14 (1)
S- B	-2169 / 0	0.0	0.0	0.23 (1)	5.77		D- P	0 / 852	0.14 (1)
K- I	-2169 / 0	0.0	0.0	0.23 (1)	5.77		N- F	-621 / 0	0.56 (1)
							P- E	-620 / 0	0.56 (1)
S- R	0 / 0	-18.5	-18.5	0.08 (4)	10.00		P- F	-3 / 0	0.00 (1)
R- Q	0 / 1429	-18.5	-18.5	0.30 (1)	10.00				
Q- P	0 / 1517	-18.5	-18.5	0.31 (1)	10.00				
P- O	0 / 1990	-18.5	-18.5	0.36 (1)	10.00				
O- N	0 / 1990	-18.5	-18.5	0.36 (1)	10.00				
N- M	0 / 1517	-18.5	-18.5	0.31 (1)	10.00				
M- L	0 / 1429	-18.5	-18.5	0.30 (1)	10.00				
L- K	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.63/1.00 (C-D:1) , BC=0.36/1.00 (N-P:1) , WB=0.56/1.00 (F-N:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.65 (I) (INPUT = 1.00)

Structural component only
DWG# T-2215199

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T11C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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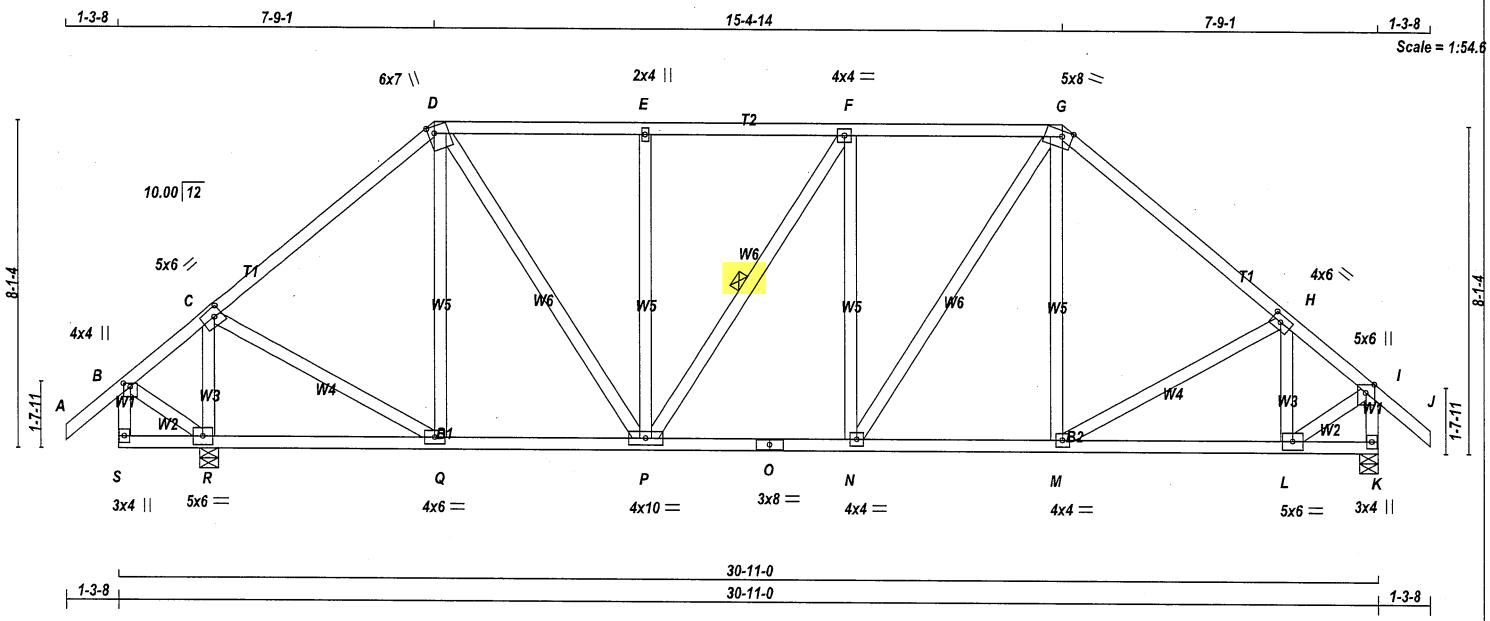

TOTAL WEIGHT = 173 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	6.0	7.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TMWW-t	MT20	4.0	6.0	2.00	2.75
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	10.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BMWW1-t	MT20	5.0	6.0		
S	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215200

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2349	0	2349	0	0	5-8	5-8
K	2010	0	2010	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1643	1179 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
K	1406	1009 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	0 / 211	-112.4	-112.4	0.48 (1)		10.00
C- D	-1371 / 0	-112.4	-112.4	0.56 (1)		4.85
D- E	-1599 / 0	-112.4	-112.4	0.46 (1)		4.67
E- F	-1599 / 0	-112.4	-112.4	0.46 (1)		4.65
F- G	-1710 / 0	-112.4	-112.4	0.47 (1)		4.52
G- H	-1793 / 0	-112.4	-112.4	0.59 (1)		4.37
H- I	-1616 / 0	-112.4	-112.4	0.46 (1)		4.57
I- J	0 / 50	-112.4	-112.4	0.15 (1)		10.00
S- B	-6 / 0	0.0	0.0	0.00 (1)		7.81
K- I	-2000 / 0	0.0	0.0	0.21 (1)		5.97
S- R	0 / 0	-18.5	-18.5	0.10 (4)		10.00
R- Q	-90 / 0	-18.5	-18.5	0.11 (4)		6.25
Q- P	0 / 1012	-18.5	-18.5	0.22 (1)		10.00
P- O	0 / 1710	-18.5	-18.5	0.32 (1)		10.00
O- N	0 / 1710	-18.5	-18.5	0.32 (1)		10.00
N- M	0 / 1348	-18.5	-18.5	0.28 (1)		10.00
M- L	0 / 1311	-18.5	-18.5	0.28 (1)		10.00
L- K	0 / 0	-18.5	-18.5	0.08 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-2194 / 0	0.33 (1)
C- Q	0 / 1273	0.20 (1)
Q- D	-521 / 0	0.47 (1)
M- G	0 / 106	0.03 (4)
M- H	0 / 41	0.01 (1)
L- H	-688 / 0	0.10 (1)
B- R	-104 / 0	0.01 (1)
L- I	0 / 1519	0.24 (1)
N- G	0 / 653	0.10 (1)
D- P	0 / 1060	0.17 (1)
N- F	-453 / 0	0.41 (1)
P- E	-621 / 0	0.56 (1)
P- F	-204 / 0	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.59/1.00 (G-H:1) , BC=0.32/1.00 (N-P:1) , WB=0.56/1.00 (E-P:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.60 (I) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T12	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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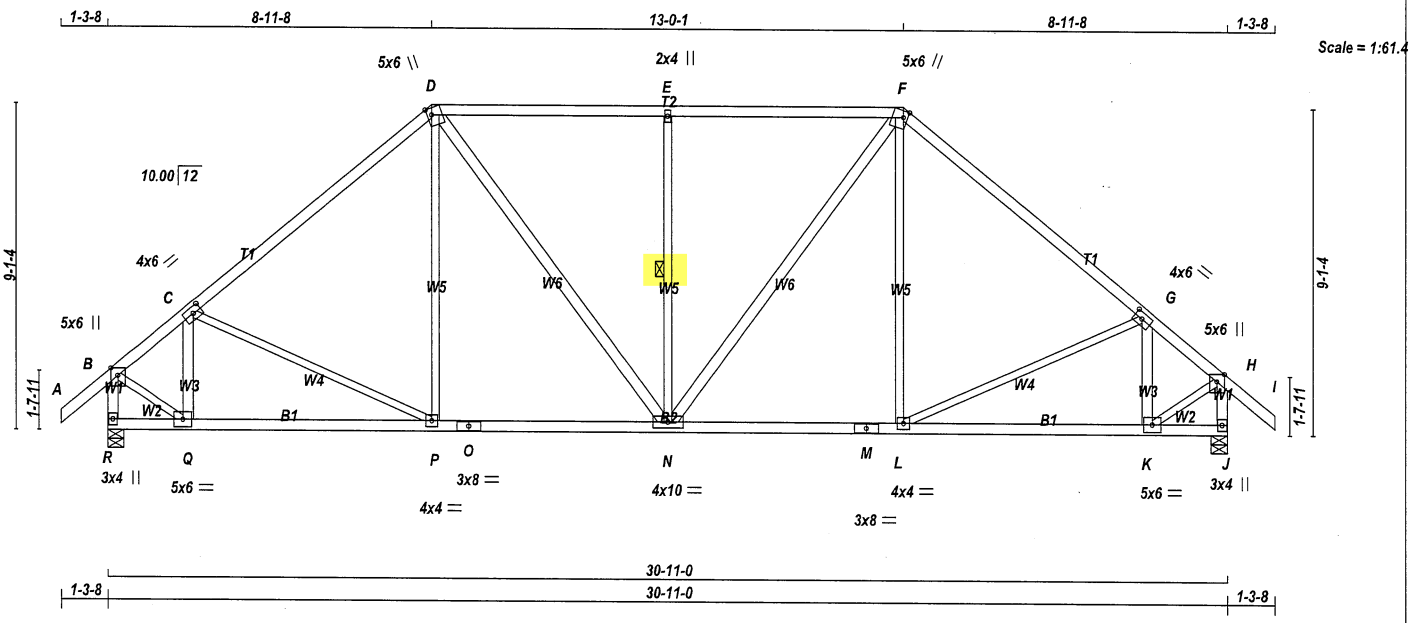

TOTAL WEIGHT = 151 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.75
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	10.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2215201

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
						IN-SX	IN-SX
R	2180	0	2180	0	0	5-8	5-8
J	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-1798 / 0	-112.4	-112.4	0.54 (1)	4.31
C- D	-1965 / 0	-112.4	-112.4	0.76 (1)	3.92
D- E	-1846 / 0	-112.4	-112.4	0.67 (1)	4.03
E- F	-1846 / 0	-112.4	-112.4	0.67 (1)	4.03
F- G	-1965 / 0	-112.4	-112.4	0.76 (1)	3.92
G- H	-1798 / 0	-112.4	-112.4	0.54 (1)	4.31
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00
R- B	-2176 / 0	0.0	0.0	0.23 (1)	5.76
J- H	-2176 / 0	0.0	0.0	0.23 (1)	5.76
R- Q	0 / 0	-18.5	-18.5	0.12 (4)	10.00
Q- P	0 / 1487	-18.5	-18.5	0.34 (1)	10.00
P- O	0 / 1474	-18.5	-18.5	0.34 (1)	10.00
O- N	0 / 1474	-18.5	-18.5	0.34 (1)	10.00
N- M	0 / 1474	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 1474	-18.5	-18.5	0.34 (1)	10.00
L- K	0 / 1487	-18.5	-18.5	0.34 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-776 / 0	0.12 (1)
C- P	-16 / 9	0.02 (1)
P- D	0 / 139	0.05 (4)
D- N	0 / 626	0.10 (1)
N- E	-897 / 0	0.48 (1)
N- F	0 / 626	0.10 (1)
L- F	0 / 139	0.05 (4)
L- G	-16 / 9	0.02 (1)
K- G	-776 / 0	0.12 (1)
B- Q	0 / 1723	0.39 (1)
K- H	0 / 1723	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.76/1.00 (C-D:1) , BC=0.34/1.00 (P-Q:1) , WB=0.48/1.00 (E-N:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.66 (B) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T12C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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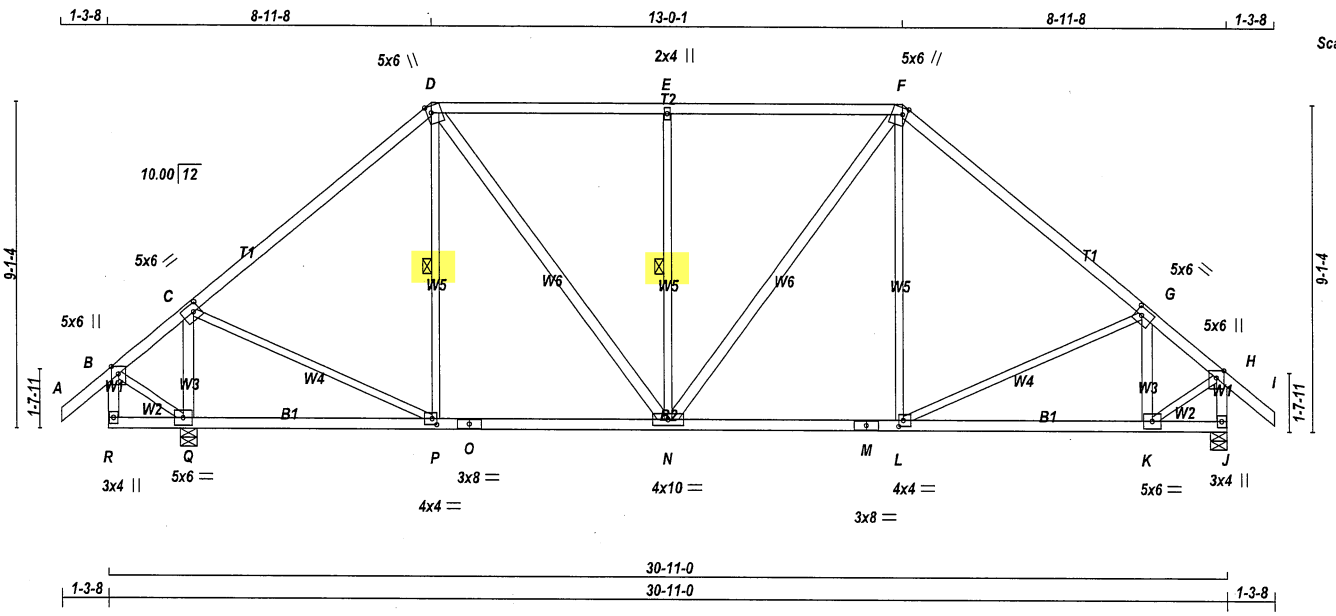

TOTAL WEIGHT = 151 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	10.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BMWW1-t	MT20	5.0	6.0		
R	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2215202

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2349	0	2349	0	0	5-8	5-8
J	2010	0	2010	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1643	1179 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
J	1406	1009 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-P, E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00		Q- C	-2211 / 0	0.33 (1)
B- C	0 / 183	-112.4	-112.4	0.71 (1)	10.00		C- P	0 / 1195	0.27 (1)
C- D	-1422 / 0	-112.4	-112.4	0.69 (1)	4.53		P- D	-379 / 0	0.20 (1)
D- E	-1549 / 0	-112.4	-112.4	0.66 (1)	4.37		D- N	0 / 841	0.14 (1)
E- F	-1549 / 0	-112.4	-112.4	0.66 (1)	4.37		N- E	-899 / 0	0.49 (1)
F- G	-1740 / 0	-112.4	-112.4	0.73 (1)	4.15		N- F	0 / 418	0.07 (1)
G- H	-1643 / 0	-112.4	-112.4	0.53 (1)	4.48		L- F	0 / 163	0.05 (4)
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00		L- G	-76 / 0	0.09 (1)
R- B	-14 / 0	0.0	0.0	0.00 (1)	7.81		K- G	-705 / 0	0.10 (1)
J- H	-2007 / 0	0.0	0.0	0.21 (1)	5.96		B- Q	-35 / 0	0.01 (1)
							K- H	0 / 1586	0.36 (1)
R- Q	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
Q- P	-30 / 0	-18.5	-18.5	0.13 (4)	6.25				
P- O	0 / 1050	-18.5	-18.5	0.26 (1)	10.00				
O- N	0 / 1050	-18.5	-18.5	0.26 (1)	10.00				
N- M	0 / 1301	-18.5	-18.5	0.31 (1)	10.00				
M- L	0 / 1301	-18.5	-18.5	0.31 (1)	10.00				
L- K	0 / 1368	-18.5	-18.5	0.32 (1)	10.00				
K- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.73/1.00 (F-G:1) , BC=0.32/1.00 (K-L:1) , WB=0.49/1.00 (E-N:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T13	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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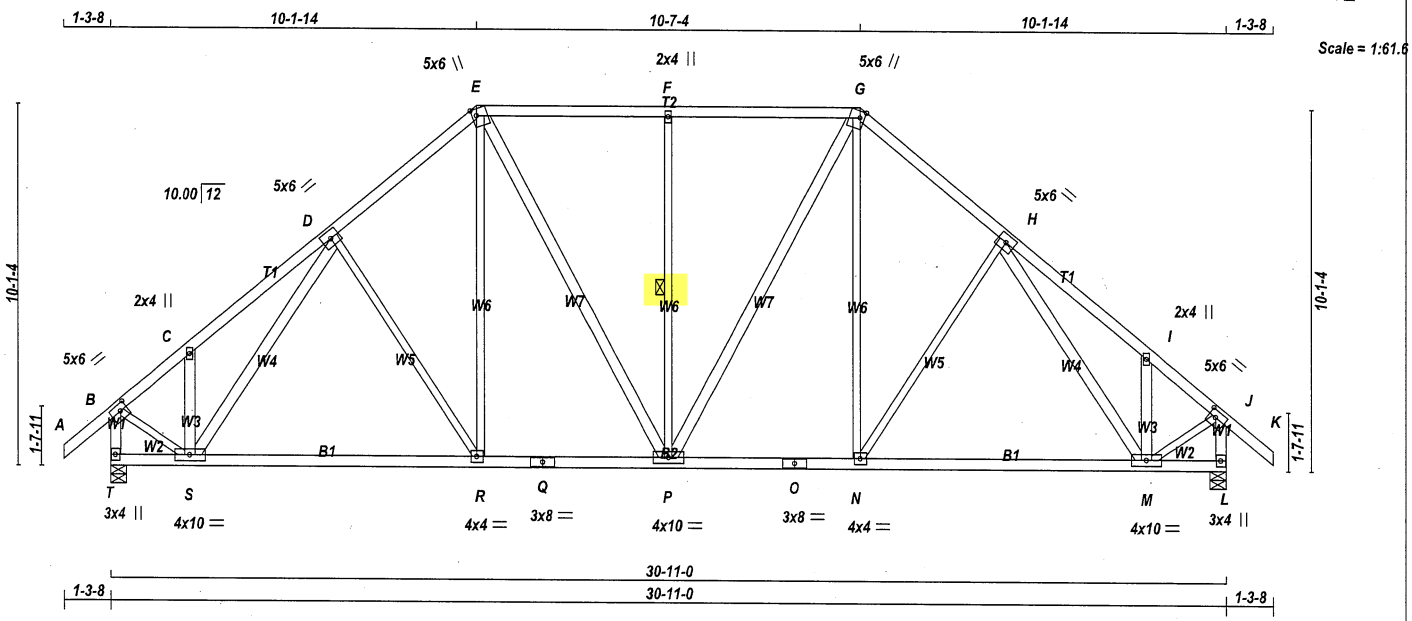

TOTAL WEIGHT = 2 X 171 = 342 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
D - R	2x3	DRY	No.2	SPF
R - E	2x3	DRY	No.2	SPF
P - F	2x3	DRY	No.2	SPF
N - G	2x3	DRY	No.2	SPF
N - H	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.50
C, F, I						
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	5.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.25	2.50
L	BMV1+p	MT20	3.0	4.0		
M, P, S						
M	BMWWW-t	MT20	4.0	10.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1)

Structural component only
DWG# T-2215203

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2180	0	2180	0	0	5-8	5-8
L	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1525	1094 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	-1765 / 0	-112.4	-112.4	0.12 (1)		4.94
C- D	-1756 / 0	-112.4	-112.4	0.25 (1)		4.80
D- E	-1878 / 0	-112.4	-112.4	0.27 (1)		4.66
E- F	-1634 / 0	-112.4	-112.4	0.44 (1)		4.68
F- G	-1634 / 0	-112.4	-112.4	0.44 (1)		4.68
G- H	-1878 / 0	-112.4	-112.4	0.27 (1)		4.66
H- I	-1756 / 0	-112.4	-112.4	0.25 (1)		4.80
I- J	-1765 / 0	-112.4	-112.4	0.12 (1)		4.94
J- K	0 / 50	-112.4	-112.4	0.15 (1)		10.00
T- B	-2189 / 0	0.0	0.0	0.23 (1)		5.74
L- J	-2189 / 0	0.0	0.0	0.23 (1)		5.74
T- S	0 / 0	-18.5	-18.5	0.19 (4)		10.00
S- R	0 / 1572	-18.5	-18.5	0.37 (1)		10.00
R- Q	0 / 1419	-18.5	-18.5	0.35 (1)		10.00
Q- P	0 / 1419	-18.5	-18.5	0.35 (1)		10.00
P- O	0 / 1419	-18.5	-18.5	0.35 (1)		10.00
O- N	0 / 1419	-18.5	-18.5	0.35 (1)		10.00
N- M	0 / 1572	-18.5	-18.5	0.37 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.19 (4)		10.00

WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
S- C	-337 / 0	0.05 (1)
S- D	-403 / 0	0.32 (1)
D- R	-282 / 0	0.33 (1)
R- E	0 / 373	0.08 (1)
E- P	0 / 452	0.07 (1)
P- F	-726 / 0	0.51 (1)
P- G	0 / 452	0.07 (1)
N- G	0 / 373	0.08 (1)
N- H	-282 / 0	0.33 (1)
H- M	-403 / 0	0.32 (1)
M- I	-337 / 0	0.05 (1)
B- S	0 / 1593	0.36 (1)
M- J	0 / 1593	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.44/1.00 (E-F:1) , BC=0.37/1.00 (R-S:1) , WB=0.51/1.00 (F-P:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T13C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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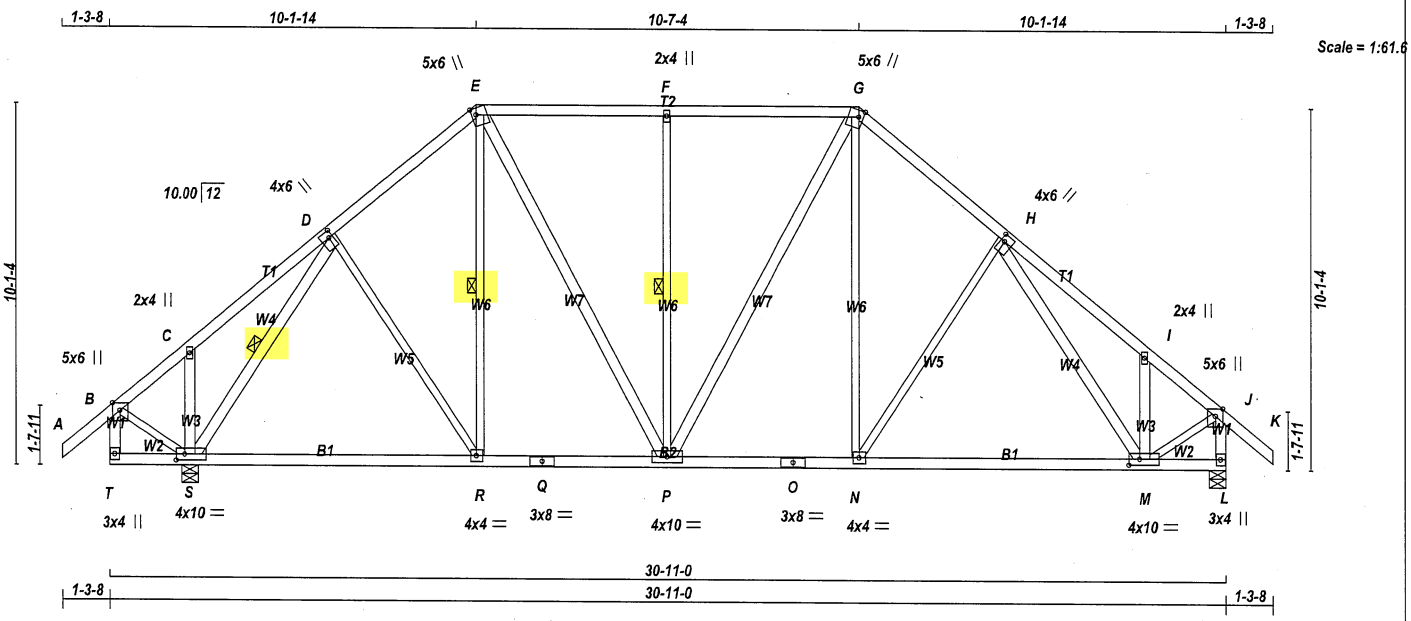

TOTAL WEIGHT = 171 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
D - R	2x3	DRY	No.2	SPF
R - E	2x3	DRY	No.2	SPF
P - F	2x3	DRY	No.2	SPF
N - G	2x3	DRY	No.2	SPF
N - H	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C, F, I						
C	TMW+w	MT20	2.0	4.0		
D	TMWW+t	MT20	4.0	6.0	2.25	1.25
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW+t	MT20	4.0	6.0	2.25	1.25
J	TMVW+p	MT20	5.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWWW-t	MT20	4.0	10.0	2.00	3.50
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	10.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWWW1-t	MT20	4.0	10.0	2.00	3.00
T	BMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2215204

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2349	0	2349	0	0	5-8	5-8
L	2010	0	2010	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1643	1179 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
L	1406	1009 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-S, E-R, F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 212	-112.4	-112.4	0.21 (1)	10.00
C- D	0 / 199	-112.4	-112.4	0.28 (1)	10.00
D- E	-1413 / 0	-112.4	-112.4	0.24 (1)	5.24
E- F	-1368 / 0	-112.4	-112.4	0.43 (1)	5.03
F- G	-1368 / 0	-112.4	-112.4	0.43 (1)	5.03
G- H	-1648 / 0	-112.4	-112.4	0.25 (1)	4.92
H- I	-1602 / 0	-112.4	-112.4	0.24 (1)	4.99
I- J	-1611 / 0	-112.4	-112.4	0.11 (1)	5.12
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
T- B	-27 / 0	0.0	0.0	0.00 (4)	7.81
L- J	-2020 / 0	0.0	0.0	0.21 (1)	5.94
T- S	0 / 0	-18.5	-18.5	0.21 (4)	10.00
S- R	0 / 907	-18.5	-18.5	0.29 (1)	10.00
R- Q	0 / 1059	-18.5	-18.5	0.27 (1)	10.00
Q- P	0 / 1059	-18.5	-18.5	0.27 (1)	10.00
P- O	0 / 1244	-18.5	-18.5	0.32 (1)	10.00
O- N	0 / 1244	-18.5	-18.5	0.32 (1)	10.00
N- M	0 / 1410	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-374 / 0	0.06 (1)
S- D	-2043 / 0	0.54 (1)
D- R	0 / 276	0.06 (1)
R- E	-100 / 59	0.07 (1)
E- P	0 / 648	0.10 (1)
P- F	-728 / 0	0.51 (1)
P- G	0 / 261	0.04 (1)
N- G	0 / 394	0.09 (1)
N- H	-308 / 0	0.36 (1)
H- M	-320 / 0	0.25 (1)
M- I	-338 / 0	0.05 (1)
B- S	-155 / 0	0.03 (1)
M- J	0 / 1455	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.34/1.00 (M-N:1) , WB=0.54/1.00 (D-S:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.84 (D) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423564	T13C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215204

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T14	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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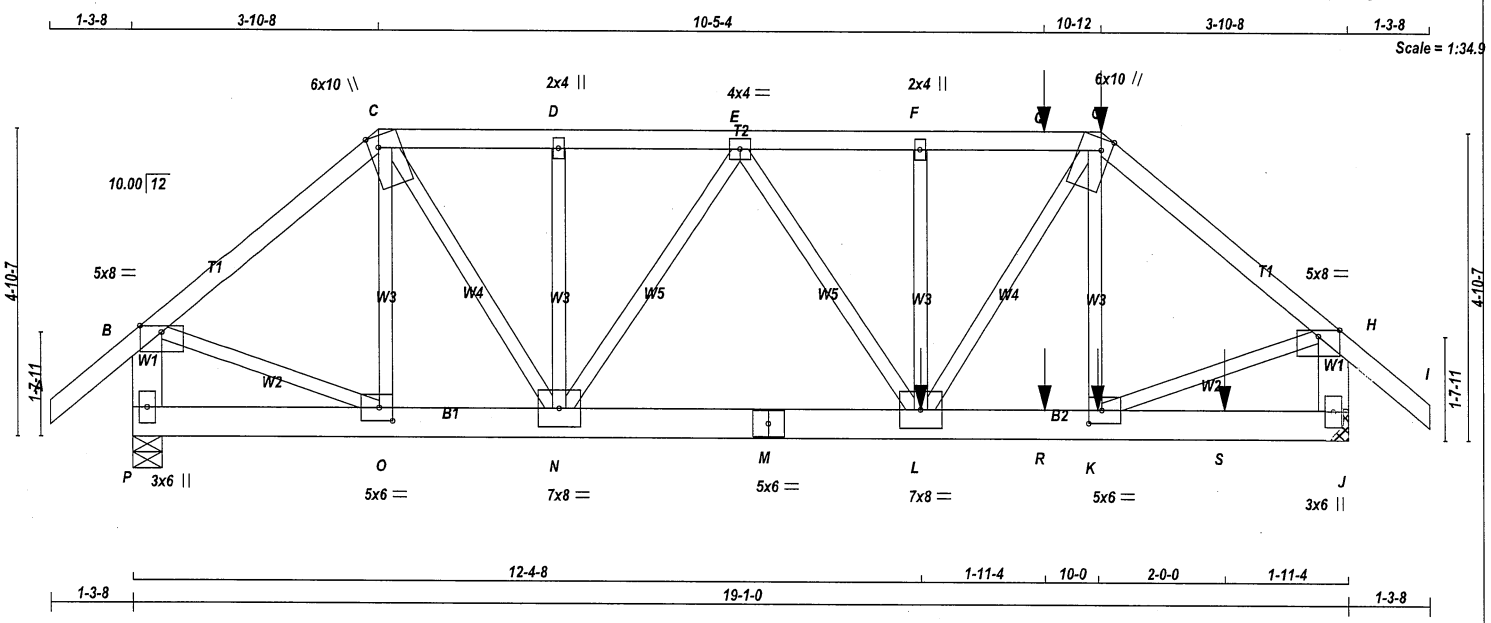

TOTAL WEIGHT = 102 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	10.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	10.0	Edge	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWWW-t	MT20	7.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	7.0	8.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215205

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1946	0	1946	0	0	5-8	5-8
J	2567	0	2567	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1359	991 / 0	0 / 0	0 / 0	0 / 0	367 / 0	0 / 0
J	1792	1308 / 0	0 / 0	0 / 0	0 / 0	484 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.17 (1)	10.00	O- C	-339 / 0	0.11 (1)
B- C	-1757 / 0	-112.4	-112.4	0.40 (1)	4.56	C- N	0 / 1304	0.32 (1)
C- D	-2036 / 0	-112.4	-112.4	0.17 (1)	4.55	N- D	-331 / 0	0.11 (1)
D- E	-2036 / 0	-112.4	-112.4	0.17 (1)	4.56	L- F	-362 / 0	0.12 (1)
E- F	-2699 / 0	-112.4	-112.4	0.21 (1)	4.00	L- G	0 / 1543	0.38 (1)
F- Q	-2699 / 0	-112.4	-112.4	0.31 (1)	3.87	K- G	-425 / 0	0.14 (1)
Q- G	-2699 / 0	-112.4	-112.4	0.31 (1)	3.87	B- O	0 / 1408	0.35 (1)
G- H	-2458 / 0	-112.4	-112.4	0.46 (1)	3.90	K- H	0 / 1970	0.49 (1)
H- I	0 / 50	-112.4	-112.4	0.17 (1)	10.00	N- E	-808 / 0	0.38 (1)
P- B	-1916 / 0	0.0	0.0	0.14 (1)	7.22	E- L	0 / 437	0.11 (1)
J- H	-2532 / 0	0.0	0.0	0.19 (1)	6.47			
P- O	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
O- N	0 / 1338	-18.5	-18.5	0.19 (1)	10.00			
N- M	0 / 2466	-18.5	-18.5	0.40 (1)	10.00			
M- L	0 / 2466	-18.5	-18.5	0.40 (1)	10.00			
L- R	0 / 1873	-18.5	-18.5	0.30 (1)	10.00			
R- K	0 / 1873	-18.5	-18.5	0.30 (1)	10.00			
K- S	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
S- J	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	15-2-8	-175	-175	---	FRONT	VERT	TOTAL	---	C1
K	15-1-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
L	12-4-8	-894	-894	---	FRONT	VERT	TOTAL	---	C1
Q	14-3-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
R	14-3-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
S	17-1-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.46/1.00 (G-H:1) , BC=0.40/1.00 (L-N:1) , WB=0.49/1.00 (H-K:1) , SSI=0.19/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.52 (M) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T15	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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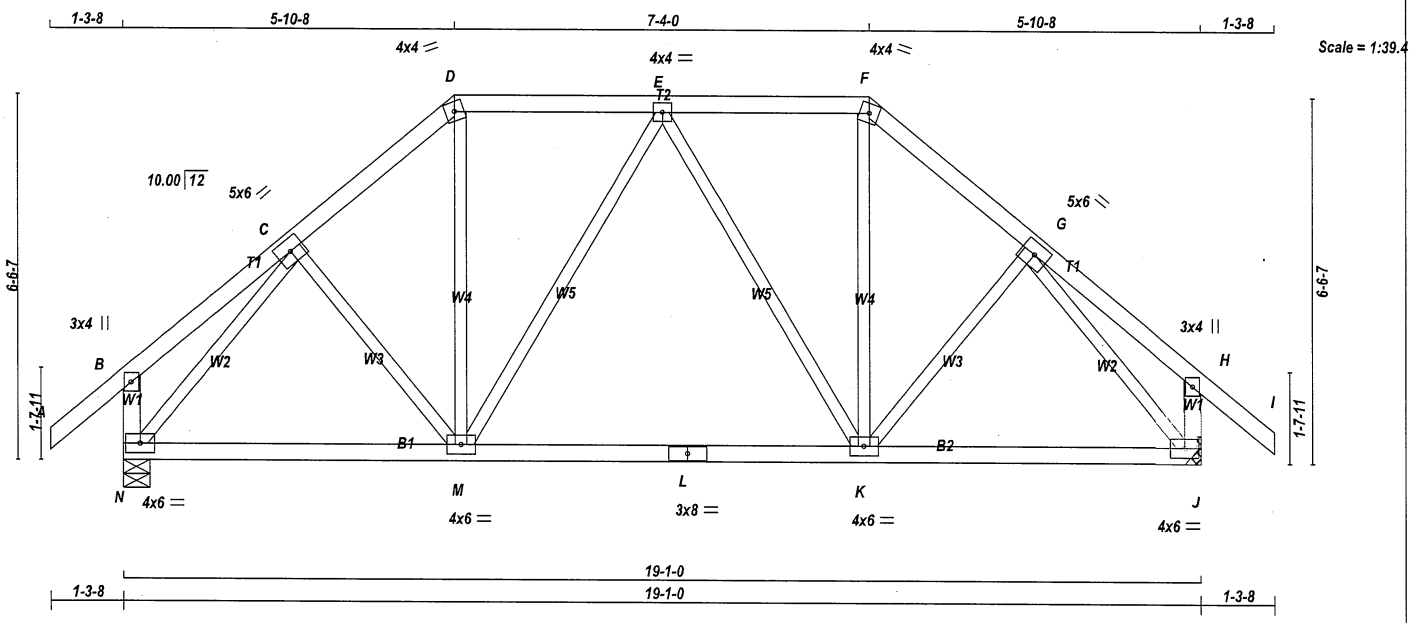

TOTAL WEIGHT = 90 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1405	0	1405	0	0	IN-SX	IN-SX
J	1405	0	1405	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	982	710 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	982	710 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	C- M	-22 / 32	0.01 (4)	
B- C	0 / 22	-112.4	-112.4	0.14 (1)	10.00	M- D	0 / 399	0.09 (1)	
C- D	-1102 / 0	-112.4	-112.4	0.12 (1)	5.91	M- E	-286 / 0	0.29 (1)	
D- E	-833 / 0	-112.4	-112.4	0.19 (1)	6.25	E- K	-286 / 0	0.29 (1)	
E- F	-833 / 0	-112.4	-112.4	0.19 (1)	6.25	K- F	0 / 399	0.09 (1)	
F- G	-1102 / 0	-112.4	-112.4	0.12 (1)	5.91	K- G	-22 / 32	0.01 (4)	
G- H	0 / 22	-112.4	-112.4	0.14 (1)	10.00	N- C	-1367 / 0	0.49 (1)	
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00	G- J	-1367 / 0	0.49 (1)	
N- B	-284 / 0	0.0	0.0	0.03 (1)	7.81				
J- H	-284 / 0	0.0	0.0	0.03 (1)	7.81				
N- M	0 / 840	-18.5	-18.5	0.24 (4)	10.00				
M- L	0 / 975	-18.5	-18.5	0.26 (1)	10.00				
L- K	0 / 975	-18.5	-18.5	0.26 (1)	10.00				
K- J	0 / 840	-18.5	-18.5	0.24 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.19/1.00 (D-E:1) , BC=0.26/1.00 (K-M:1) , WB=0.49/1.00 (C-N:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (J) (INPUT = 0.90)
JSI METAL= 0.38 (L) (INPUT = 1.00)

Structural component only
DWG# T-2215206

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T16	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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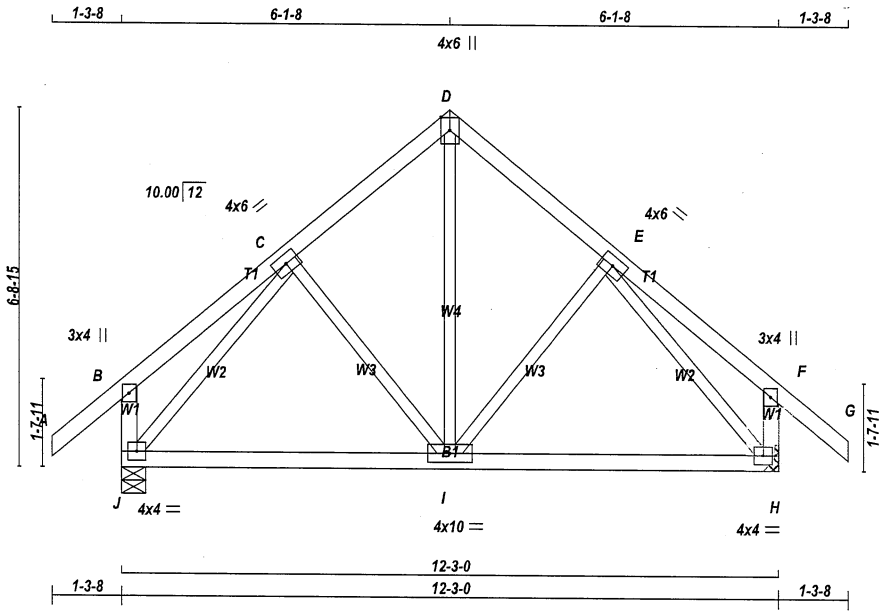

TOTAL WEIGHT = 59 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	10.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	958	0	958	0	0	5-8	5-8
H	958	0	958	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	669	488 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	669	488 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	1	0.15 (1)	10.00
B- C	0 / 25	-112.4	-112.4	1	0.17 (1)	10.00
C- D	-539 / 0	-112.4	-112.4	1	0.13 (1)	6.25
D- E	-539 / 0	-112.4	-112.4	1	0.13 (1)	6.25
E- F	0 / 25	-112.4	-112.4	1	0.17 (1)	10.00
F- G	0 / 50	-112.4	-112.4	1	0.15 (1)	10.00
J- B	-287 / 0	0.0	0.0	1	0.03 (1)	7.81
H- F	-287 / 0	0.0	0.0	1	0.03 (1)	7.81
J- I	0 / 496	-18.5	-18.5	4	0.23 (4)	10.00
I- H	0 / 496	-18.5	-18.5	4	0.23 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 388	0.09 (1)
I- E	-160 / 0	0.06 (1)
C- I	-160 / 0	0.06 (1)
J- C	-799 / 0	0.31 (1)
E- H	-799 / 0	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.23/1.00 (H-I:4) , WB=0.31/1.00 (C-J:1) . SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (H) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)

Structural component only
DWG# T-2215207

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T16S	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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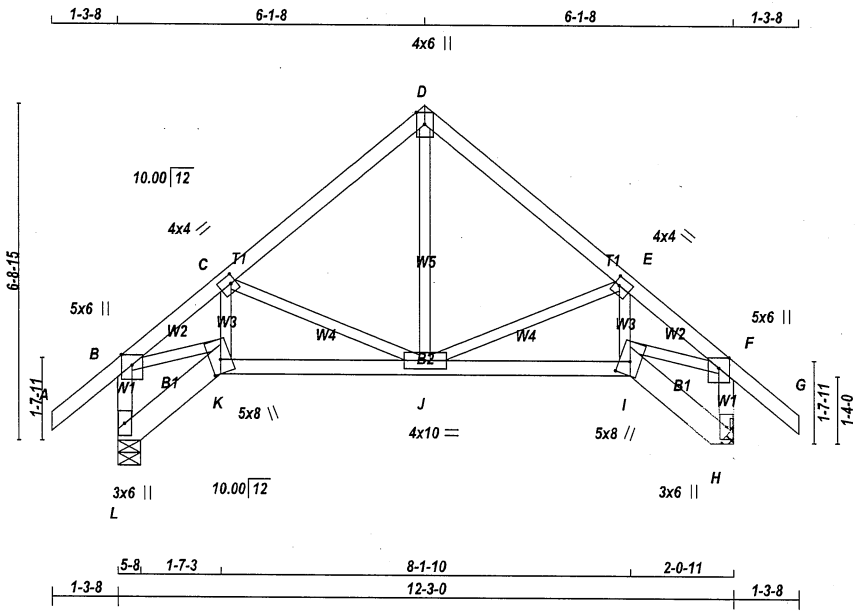

TOTAL WEIGHT = 3 X 61 = 183 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMWW+p	MT20	5.0	6.0	Edge	
H	BMW1+p	MT20	3.0	6.0		
I	BBWW+m	MT20	5.0	8.0	3.25	2.50
J	BMWWW-t	MT20	4.0	10.0		
K	BBWW+m	MT20	5.0	8.0	3.25	2.50
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	925	0	925	0	0	IN-SX	IN-SX
H	985	0	985	0	0	5-8 MECHANICAL	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	646	471 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
H	687	505 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-1115 / 0	-112.4	-112.4	0.16 (1)	5.82
C- D	-611 / 0	-112.4	-112.4	0.21 (1)	6.25
D- E	-608 / 0	-112.4	-112.4	0.22 (1)	6.25
E- F	-938 / 0	-112.4	-112.4	0.15 (1)	6.23
F- G	0 / 55	-112.4	-112.4	0.19 (1)	10.00
L- B	-906 / 0	0.0	0.0	0.10 (1)	7.81
L- K	0 / 0	-18.5	-18.5	0.01 (4)	10.00
K- J	0 / 882	-18.5	-18.5	0.19 (1)	10.00
J- I	0 / 720	-18.5	-18.5	0.16 (1)	10.00
I- H	-118 / 0	-18.5	-18.5	0.01 (4)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 375	0.08 (1)
J- E	-295 / 0	0.09 (1)
I- E	-22 / 34	0.01 (4)
C- J	-472 / 0	0.15 (1)
K- C	0 / 110	0.02 (1)
B- K	0 / 890	0.20 (1)
H- F	-900 / 0	0.09 (1)
I- F	0 / 812	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.22/1.00 (D-E:1) , BC=0.19/1.00 (J-K:1) , WB=0.20/1.00 (B-K:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL= 0.43 (K) (INPUT = 1.00)

Structural component only
DWG# T-2215208

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T17	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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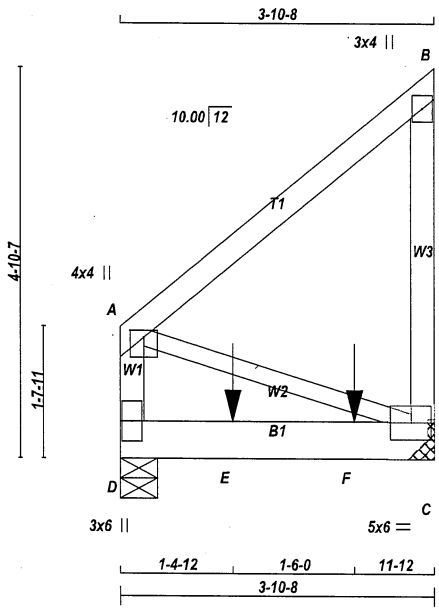

Scale = 1:27.5

TOTAL WEIGHT = 2 X 22 = 43 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2215209

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1109	0	1109	0	0	IN-SX	IN-SX
C	1304	0	1304	0	0	5-8	5-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	773	572 / 0	0 / 0	0 / 0	0 / 0	201 / 0	0 / 0
C	909	672 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-218 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-112.4	-112.4	0.16 (1)	10.00			
C- B	-218 / 0	0.0	0.0	0.04 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.45 (1)	10.00			
E- F	0 / 0	-18.5	-18.5	0.45 (1)	10.00			
F- C	0 / 0	-18.5	-18.5	0.45 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-4-12	-672	-672	---	BACK	VERT	TOTAL	---	C1
F	2-10-12	-654	-654	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 940 (0.05")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.45/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T17	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215209

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T18	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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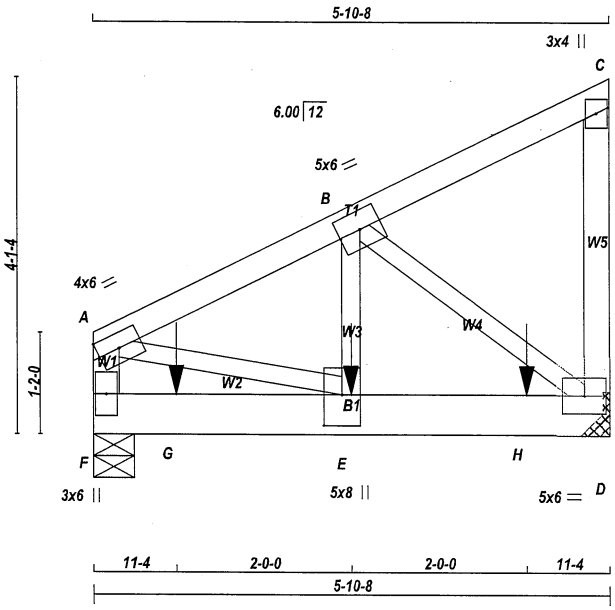

Scale = 1:25.4

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	11	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(420.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2215210

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1966	0	1966	0	0	IN-SX	IN-SX
D	2841	0	2841	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1372	1007 / 0	0 / 0	0 / 0	0 / 0	365 / 0	0 / 0
D	1983	1450 / 0	0 / 0	0 / 0	0 / 0	532 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1869 / 0	0.0	0.0		0.10 (1)	7.81	A- E	0 / 2339	0.29 (1)
A- B	-2519 / 0	-112.4	-112.4		0.09 (1)	5.62	E- B	0 / 2482	0.31 (1)
B- C	-10 / 0	-112.4	-112.4		0.07 (1)	6.25	B- D	-2846 / 0	0.34 (1)
D- C	-142 / 0	0.0	0.0		0.02 (1)	7.81			
F- G	0 / 0	-18.5	-18.5		0.03 (1)	10.00			
G- E	0 / 0	-18.5	-18.5		0.03 (1)	10.00			
E- H	0 / 2262	-18.5	-18.5		0.48 (1)	10.00			
H- D	0 / 2262	-18.5	-18.5		0.48 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-1777	-1777	---	BACK	VERT	TOTAL	---	C1
G	11-4	-70	-70	---	BACK	VERT	TOTAL	---	C1
H	4-11-4	-968	-968	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.10/1.00 (A-F:1) , BC=0.48/1.00 (D-E:1) , WB=0.34/1.00 (B-D:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.43 (E) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423564	T18	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	5.0	8.0	4.25	2.50
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215210

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T19	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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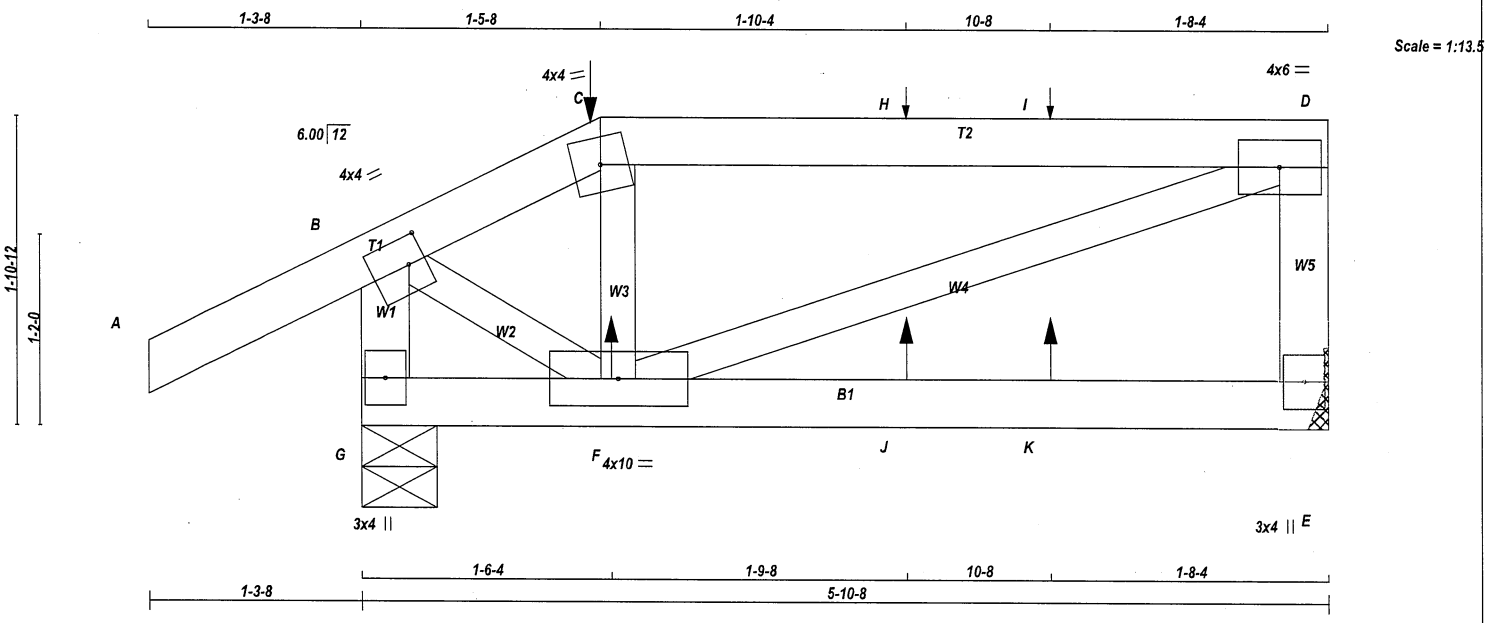


TOTAL WEIGHT = 23 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	10.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	348	0	348	0	0	MECHANICAL	
G	538	0	538	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	245	168 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
G	375	278 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-112.4	-112.4	0.16 (1)		10.00	F- C	-156 / 21	0.02 (1)
B- C	-270 / 0	-112.4	-112.4	0.16 (1)		6.25	F- D	0 / 216	0.05 (1)
C- H	-203 / 0	-112.4	-112.4	0.41 (1)		6.25	B- F	0 / 243	0.06 (1)
H- I	-203 / 0	-112.4	-112.4	0.41 (1)		6.25			
I- D	-203 / 0	-112.4	-112.4	0.41 (1)		6.25			
E- D	-323 / 0	0.0	0.0	0.04 (1)		7.81			
G- B	-530 / 0	0.0	0.0	0.06 (1)		7.81			
G- F	0 / 0	-18.5	-18.5	0.08 (4)		10.00			
F- J	0 / 0	-18.5	-18.5	0.08 (4)		10.00			
J- K	0 / 0	-18.5	-18.5	0.08 (4)		10.00			
K- E	0 / 0	-18.5	-18.5	0.08 (4)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-5-8	-2	-2	94	FRONT	VERT	TOTAL	---	C1
F	1-6-4	9	1	12	FRONT	VERT	TOTAL	---	C1
H	3-3-12	1	1	94	FRONT	VERT	TOTAL	---	C1
I	4-2-4	1	1	94	FRONT	VERT	TOTAL	---	C1
J	3-3-12	9	1	12	FRONT	VERT	TOTAL	---	C1
K	4-2-4	9	1	12	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.08/1.00 (F-G:4) , WB=0.06/1.00 (B-F:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

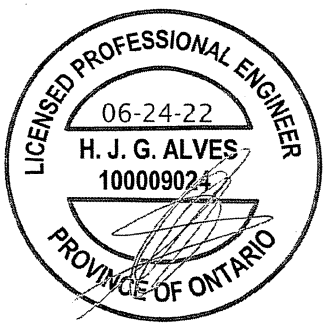
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215211

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T20S	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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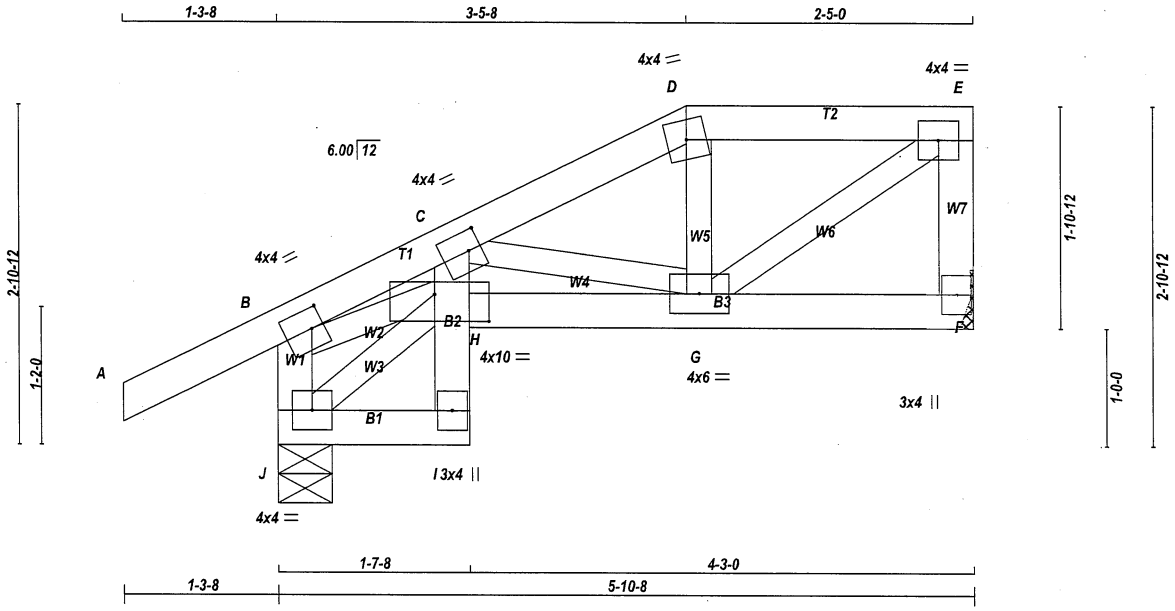

TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BVMWW-I	MT20	4.0	10.0	2.75	5.50
I	BMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	367	0	367	0	0	MECHANICAL	
J	555	0	555	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	257	181 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
J	386	289 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00		C- G	-364 / 0	0.06 (1)
B- C	-627 / 0	-112.4	-112.4	0.15 (1)	6.25		G- D	-110 / 4	0.02 (1)
C- D	-339 / 0	-112.4	-112.4	0.10 (1)	6.25		G- E	0 / 365	0.08 (1)
D- E	-296 / 0	-112.4	-112.4	0.11 (1)	6.25		J- H	-73 / 0	0.01 (1)
F- E	-348 / 0	0.0	0.0	0.04 (1)	7.81		B- H	0 / 532	0.12 (1)
J- B	-497 / 0	0.0	0.0	0.05 (1)	7.81				
J- I	0 / 58	-18.5	-18.5	0.02 (1)	10.00				
I- H	0 / 15	0.0	0.0	0.09 (1)	10.00				
H- C	0 / 117	0.0	0.0	0.11 (1)	10.00				
H- G	0 / 661	-18.5	-18.5	0.12 (1)	10.00				
G- F	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.12/1.00 (G-H:1) , WB=0.12/1.00 (B-H:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215212

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T21S	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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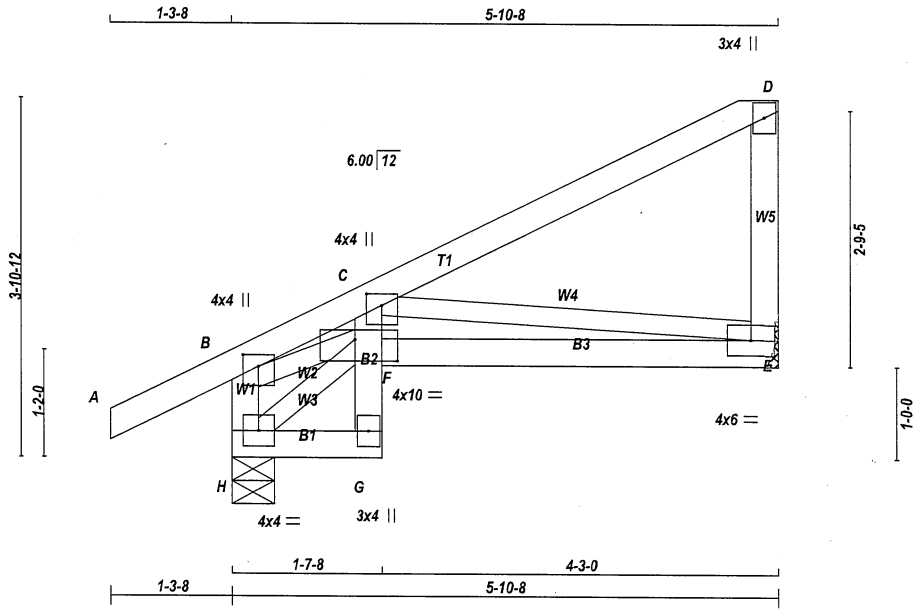

TOTAL WEIGHT = 26 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BVMWW-t	MT20	4.0	10.0	2.75	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	367	0	367	0	0	MECHANICAL	
H	555	0	555	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	257	181 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
H	386	289 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	
B- C	-856 / 0	-112.4	-112.4	0.16 (1)	6.25	
C- D	-11 / 0	-112.4	-112.4	0.30 (1)	6.25	
E- D	-222 / 0	0.0	0.0	0.03 (1)	7.81	
H- B	-480 / 0	0.0	0.0	0.05 (1)	7.81	
H- G	0 / 80	-18.5	-18.5	0.02 (1)	10.00	
G- F	0 / 15	0.0	0.0	0.12 (1)	10.00	
F- C	0 / 155	0.0	0.0	0.15 (1)	10.00	
F- E	0 / 956	-18.5	-18.5	0.21 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- E	-964 / 0	0.29 (1)
H- F	-101 / 0	0.01 (1)
B- F	0 / 779	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.21/1.00 (E-F:1) , WB=0.29/1.00 (C-E:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

Structural component only
DWG# T-2215213

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	T23W	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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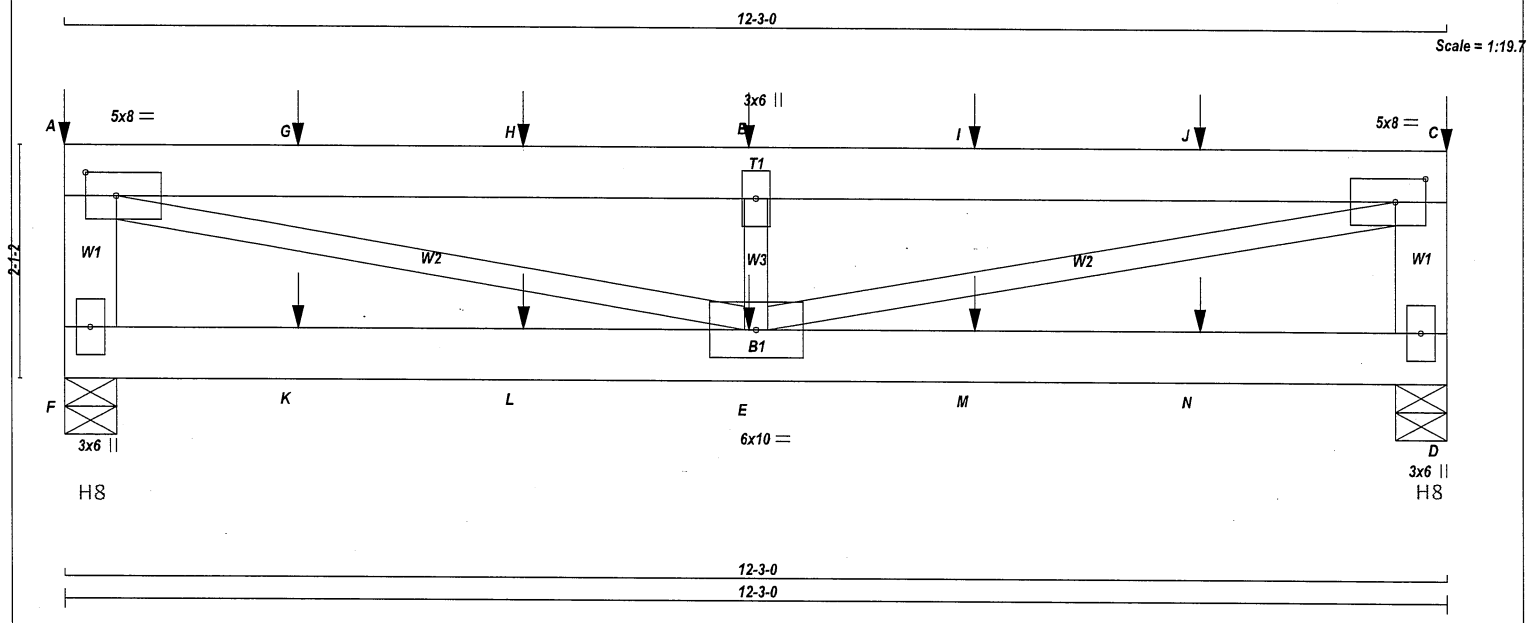

TOTAL WEIGHT = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.25
B	TMW+w	MT20	3.0	6.0		
C	TMVW-t	MT20	5.0	8.0	2.50	3.25
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	6.0	10.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

END VERTICALS ARE NOT EXPOSED TO WIND.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1543	0	1543	0	-773	5-8	5-8
D	1527	0	1527	0	-765	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 773 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 765 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1079	777 / 0	0 / 0	0 / 0	0 / -746	302 / 0	0 / 0
D	1068	769 / 0	0 / 0	0 / 0	0 / -739	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1399 / 656	0.0	0.0	0.10 (1)	7.81	A- E	-1576 / 3075	0.98 (7)
A- G	-2971 / 1523	-112.4	-112.4	0.53 (1)	4.22	E- B	-1290 / 501	0.20 (1)
G- H	-2971 / 1523	-112.4	-112.4	0.53 (1)	4.22	E- C	-1576 / 3075	0.98 (7)
H- B	-2971 / 1523	-112.4	-112.4	0.53 (1)	4.22			
B- I	-2971 / 1523	-112.4	-112.4	0.53 (1)	4.22			
I- J	-2971 / 1523	-112.4	-112.4	0.53 (1)	4.22			
J- C	-2971 / 1523	-112.4	-112.4	0.53 (1)	4.22			
D- C	-1386 / 651	0.0	0.0	0.10 (1)	7.81			
F- K	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
K- L	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
L- E	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
E- M	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
M- N	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
N- D	0 / 0	-18.5	-18.5	0.21 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-136	-136	65	FRONT	VERT	TOTAL	---	C1
B	6-0-12	-100	-100	64	FRONT	VERT	TOTAL	---	C1
C	12-3-0	-131	-131	65	FRONT	VERT	TOTAL	---	C1
E	6-0-12	-51	-51	40	TOP	VERT	TOTAL	---	C1
G	2-0-12	-100	-100	64	FRONT	VERT	TOTAL	---	C1
H	4-0-12	-100	-100	64	FRONT	VERT	TOTAL	---	C1
I	8-0-12	-100	-100	64	FRONT	VERT	TOTAL	---	C1
J	10-0-12	-100	-100	64	FRONT	VERT	TOTAL	---	C1
K	2-0-12	-51	-51	40	TOP	VERT	TOTAL	---	C1
L	4-0-12	-51	-51	40	TOP	VERT	TOTAL	---	C1
M	8-0-12	-51	-51	40	TOP	VERT	TOTAL	---	C1
N	10-0-12	-51	-51	40	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 679 (0.22")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.21/1.00 (E-F:1) , WB=0.98/1.00 (C-E:7) , SSI=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.53 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215214

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423564	T23W	1	1	TRUSS DESC.	

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215214

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423567	T30	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 11:14:49 2022 Page 1
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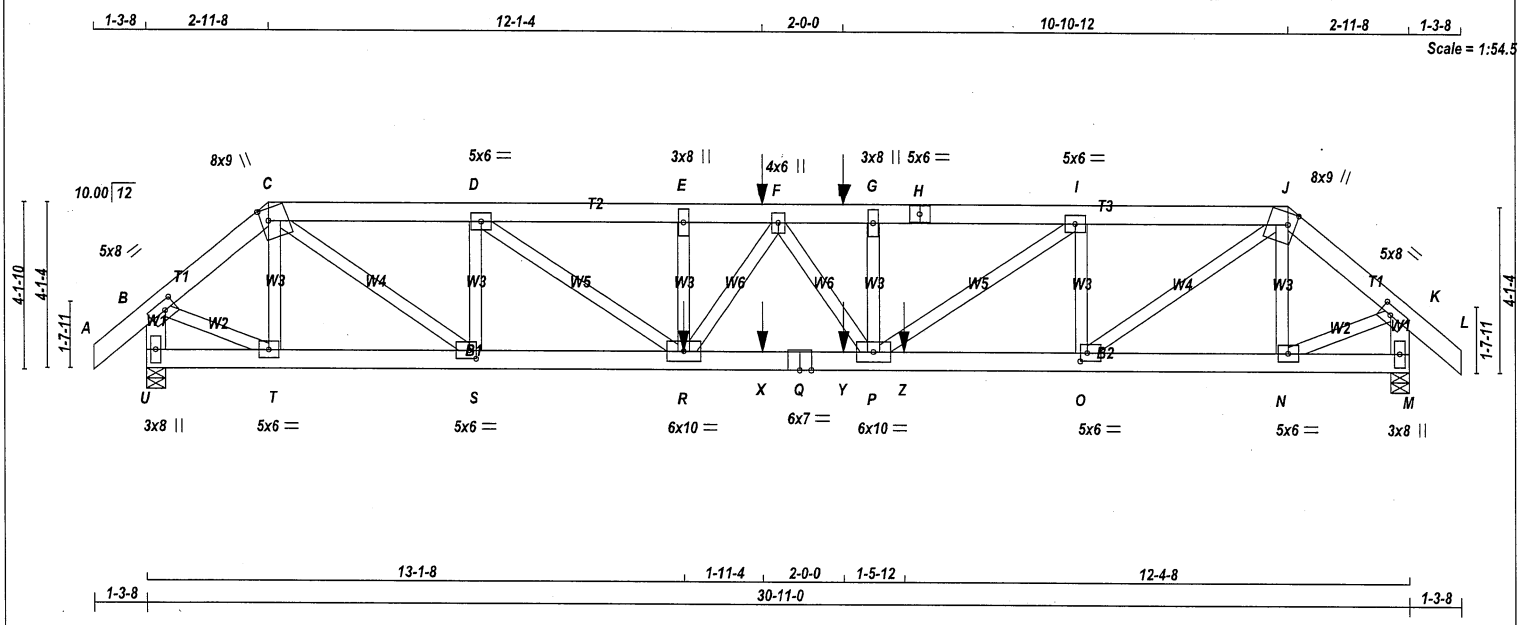

TOTAL WEIGHT = 2 X 188 = 375 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- H	2	12	SIDE(0.0)
H- J	2	12	TOP
J- L	2	12	TOP
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(197.8)
Q- M	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
E- R	1	6	SIDE(295.8)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2215224

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	4233	0	4233	0	0	5-8	5-8
M	4360	0	4360	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2955	2154 / 0	0 / 0	0 / 0	0 / 0	802 / 0	0 / 0
M	3044	2219 / 0	0 / 0	0 / 0	0 / 0	825 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00		T- C	-1030 / 0	0.09 (1)
B- C	-4253 / 0	-112.4	-112.4	0.07 (1)	5.50		C- S	0 / 5727	0.51 (1)
C- D	-7876 / 0	-112.4	-112.4	0.21 (1)	4.18		S- D	-3190 / 0	0.27 (1)
D- E	-11450 / 0	-112.4	-112.4	0.34 (1)	3.44		D- R	0 / 4398	0.39 (1)
E- V	-11450 / 0	-112.4	-112.4	0.26 (1)	3.50		R- E	-339 / 0	0.03 (1)
V- F	-11450 / 0	-112.4	-112.4	0.26 (1)	3.50		R- F	-523 / 0	0.06 (1)
F- W	-11696 / 0	-112.4	-112.4	0.29 (1)	3.44		F- P	-65 / 3	0.01 (1)
W- G	-11696 / 0	-112.4	-112.4	0.29 (1)	3.44		P- G	-411 / 0	0.04 (1)
G- H	-11696 / 0	-112.4	-112.4	0.36 (1)	3.39		P- I	0 / 4298	0.38 (1)
H- I	-11696 / 0	-112.4	-112.4	0.36 (1)	3.39		O- I	-3126 / 0	0.27 (1)
I- J	-8203 / 0	-112.4	-112.4	0.22 (1)	4.10		O- J	0 / 6020	0.53 (1)
J- K	-4373 / 0	-112.4	-112.4	0.07 (1)	5.44		N- J	-1126 / 0	0.10 (1)
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00		B- T	0 / 3451	0.31 (1)
U- B	-4238 / 0	0.0	0.0	0.15 (1)	6.94		N- K	0 / 3549	0.31 (1)
M- K	-4349 / 0	0.0	0.0	0.16 (1)	6.87				
U- T	0 / 0	-18.5	-18.5	0.02 (1)	10.00				
T- S	0 / 3207	-18.5	-18.5	0.11 (1)	10.00				
S- R	0 / 7876	-18.5	-18.5	0.28 (1)	10.00				
R- X	0 / 11732	-18.5	-18.5	0.39 (1)	10.00				
X- Q	0 / 11732	-18.5	-18.5	0.39 (1)	10.00				
Q- Y	0 / 11732	-18.5	-18.5	0.39 (1)	10.00				
Y- P	0 / 11732	-18.5	-18.5	0.39 (1)	10.00				
P- Z	0 / 8203	-18.5	-18.5	0.41 (1)	10.00				
Z- O	0 / 8203	-18.5	-18.5	0.41 (1)	10.00				
O- N	0 / 3295	-18.5	-18.5	0.12 (1)	10.00				
N- M	0 / 0	-18.5	-18.5	0.01 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	13-1-8	-1318	-1318	---	BACK	VERT	TOTAL	---	C1
V	15-0-12	-93	-93	---	BACK	VERT	TOTAL	---	C1
W	17-0-12	-93	-93	---	BACK	VERT	TOTAL	---	C1
X	15-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	17-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	18-6-8	-1396	-1396	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.36/1.00 (G-I:1) , BC=0.41/1.00 (O-P:1) , WB=0.53/1.00 (J-O:1) , SSI=0.60/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.87 (Q) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423567	T30	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	8.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	8.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	3.50	2.25
K	TMVW-t	MT20	5.0	8.0	2.50	3.25
M	BMV1+p	MT20	3.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BMWWW-t	MT20	6.0	10.0		
Q	BS-t	MT20	6.0	7.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2215224

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423567	T31	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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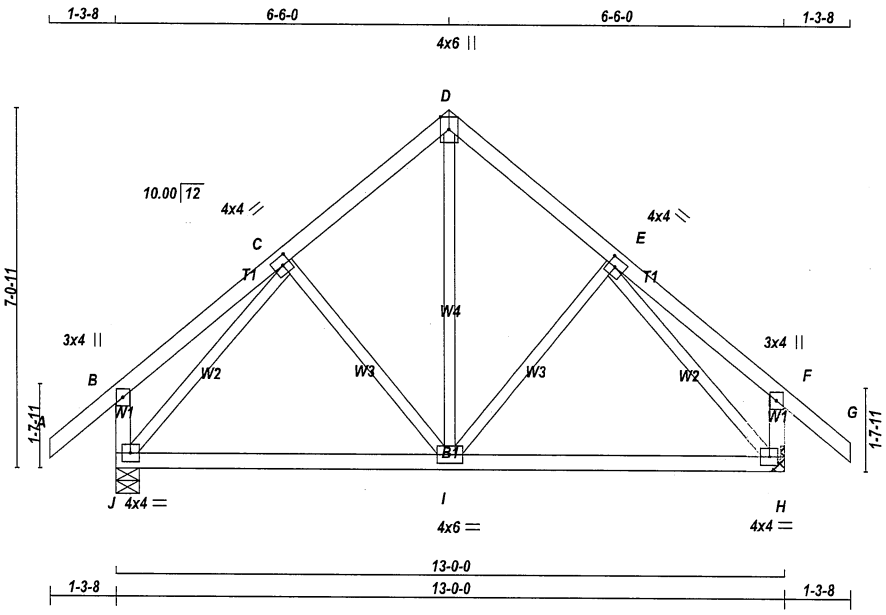

TOTAL WEIGHT = 62 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1007	0	1007	0	0	IN-SX	IN-SX
H	1007	0	1007	0	0	5-8	5-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	703	512 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
H	703	512 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4		0.15 (1)	10.00	C- I	-177 / 0	0.08 (1)
B- C	0 / 27	-112.4	-112.4		0.19 (1)	10.00	I- D	0 / 422	0.09 (1)
C- D	-579 / 0	-112.4	-112.4		0.15 (1)	6.25	I- E	-177 / 0	0.08 (1)
D- E	-579 / 0	-112.4	-112.4		0.15 (1)	6.25	J- C	-855 / 0	0.36 (1)
E- F	0 / 27	-112.4	-112.4		0.19 (1)	10.00	E- H	-855 / 0	0.36 (1)
F- G	0 / 50	-112.4	-112.4		0.15 (1)	10.00			
J- B	-295 / 0	0.0	0.0		0.03 (1)	7.81			
H- F	-295 / 0	0.0	0.0		0.03 (1)	7.81			
J- I	0 / 537	-18.5	-18.5		0.26 (4)	10.00			
I- H	0 / 538	-18.5	-18.5		0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.19/1.00 (E-F:1) , BC=0.26/1.00 (H-I:4) , WB=0.36/1.00 (E-H:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

Structural component only
DWG# T-2215225

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423567	T31S	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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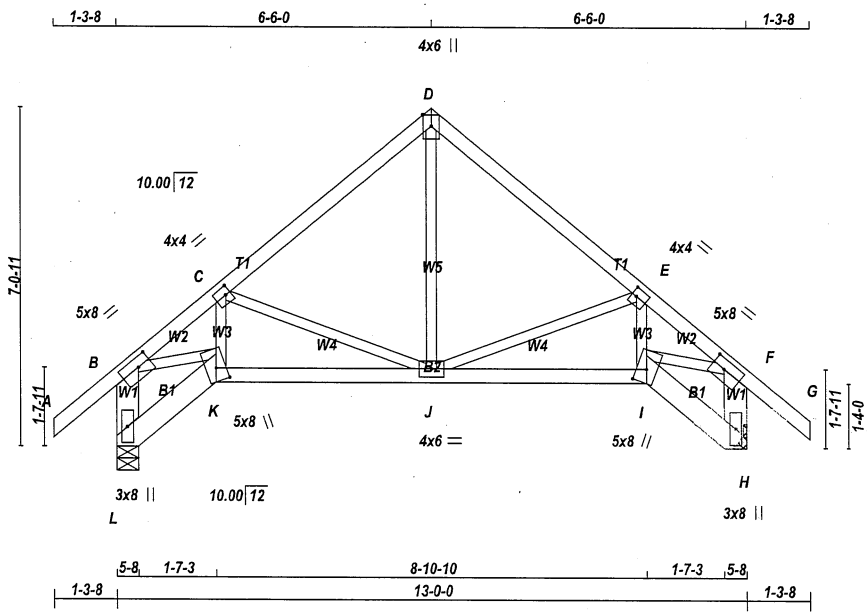


TOTAL WEIGHT = 3 X 66 = 197 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW-t	MT20	5.0	8.0	2.25	3.25
H	BMV1+p	MT20	3.0	8.0		
I	BBWW+m	MT20	5.0	8.0	3.25	2.50
J	BMWWW-t	MT20	4.0	6.0		
K	BBWW+m	MT20	5.0	8.0	3.25	2.50
L	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1007	0	1007	0	0	5-8	5-8
H	1007	0	1007	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	703	512 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
H	703	512 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	K- C	0 / 120	0.03 (1)
B- C	-1264 / 0	-112.4	-112.4	0.20 (1)	5.51	C- J	-521 / 0	0.19 (1)
C- D	-704 / 0	-112.4	-112.4	0.25 (1)	6.25	J- D	0 / 461	0.10 (1)
D- E	-704 / 0	-112.4	-112.4	0.25 (1)	6.25	J- E	-522 / 0	0.19 (1)
E- F	-1266 / 0	-112.4	-112.4	0.20 (1)	5.51	I- E	0 / 121	0.03 (1)
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00	B- K	0 / 1012	0.23 (1)
L- B	-988 / 0	0.0	0.0	0.07 (1)	7.81	I- F	0 / 1013	0.23 (1)
H- F	-988 / 0	0.0	0.0	0.07 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
K- J	0 / 1003	-18.5	-18.5	0.22 (1)	10.00			
J- I	0 / 1005	-18.5	-18.5	0.22 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.22/1.00 (I-J:1) , WB=0.23/1.00 (F-I:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

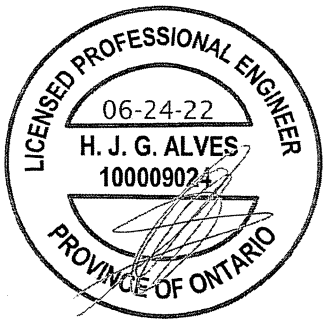
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (K) (INPUT = 0.90)
JSI METAL= 0.49 (K) (INPUT = 1.00)



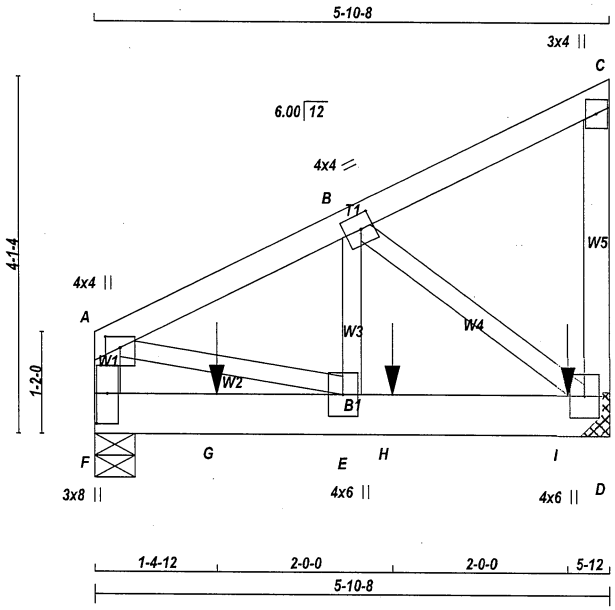
Structural component only
DWG# T-2215226

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423567	T32	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:c3jyj23uDijq_8pvRKbkZpy75XW-IL6FglerBjRZg1mEmvjBHbhpQy9VefcviHrHu2z35uH



Scale = 1:25.4

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	10	SIDE(124.2)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

Structural component only
DWG# T-2215227

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1594	0	1594	0	0	IN-SX	IN-SX
D	2024	0	2024	0	0	5-8 MECHANICAL	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1111	821 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0
D	1411	1041 / 0	0 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1183 / 0	0.0	0.0	0.07 (1)		7.81	A- E	0 / 1419	0.18 (1)
A- B	-1521 / 0	-112.4	-112.4	0.08 (1)		6.25	E- B	0 / 1343	0.17 (1)
B- C	-14 / 0	-112.4	-112.4	0.07 (1)		6.25	B- D	-1727 / 0	0.21 (1)
D- C	-135 / 0	0.0	0.0	0.02 (1)		7.81			
F- G	0 / 0	-18.5	-18.5	0.20 (1)		10.00			
G- E	0 / 0	-18.5	-18.5	0.20 (1)		10.00			
E- H	0 / 1372	-18.5	-18.5	0.24 (1)		10.00			
H- I	0 / 1372	-18.5	-18.5	0.24 (1)		10.00			
I- D	0 / 1372	-18.5	-18.5	0.24 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-12	-672	-672	---	BACK	VERT	TOTAL	---	C1
H	3-4-12	-654	-654	---	BACK	VERT	TOTAL	---	C1
I	5-4-12	-658	-658	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.24/1.00 (D-E:1) , WB=0.21/1.00 (B-D:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423567	T32	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:c3jvj23uDijq_8pvRKbkZpv75XW-IL6FqlerBjRZg1mEmvjBHbhpQy9VefcviHrHu2z35uH

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	8.0	4.25	1.50

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215227

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423567	T32Z	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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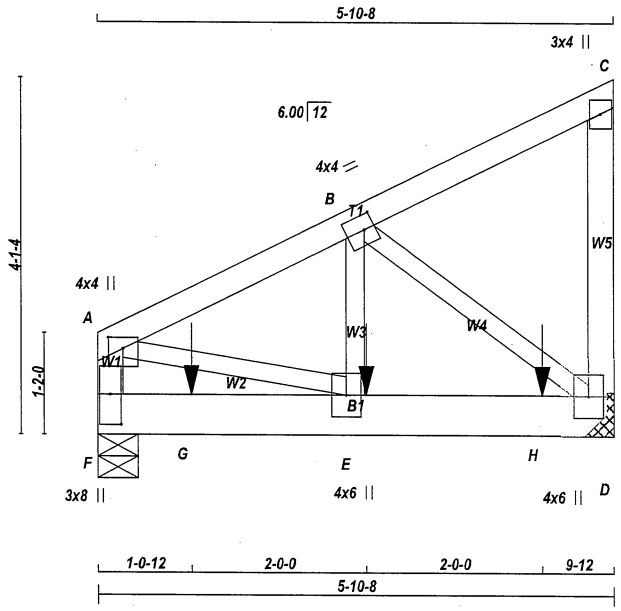

Scale = 1:25.4

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(74.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2215228

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1825	0	1825	0	0	IN-SX	IN-SX
D	1911	0	1911	0	0	5-8	5-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1273	937 / 0	0 / 0	0 / 0	0 / 0	335 / 0	0 / 0
D	1332	981 / 0	0 / 0	0 / 0	0 / 0	351 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1243 / 0	0.0	0.0	0.07 (1)		7.81	A- E	0 / 1500	0.19 (1)
A- B	-1609 / 0	-112.4	-112.4	0.07 (1)		6.25	E- B	0 / 1444	0.18 (1)
B- C	-13 / 0	-112.4	-112.4	0.07 (1)		6.25	B- D	-1825 / 0	0.22 (1)
D- C	-136 / 0	0.0	0.0	0.02 (1)		7.81			
F- G	0 / 0	-18.5	-18.5	0.22 (1)		10.00			
G- E	0 / 0	-18.5	-18.5	0.22 (1)		10.00			
E- H	0 / 1451	-18.5	-18.5	0.29 (1)		10.00			
H- D	0 / 1451	-18.5	-18.5	0.29 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-688	-688	---	FRONT	VERT	TOTAL	---	C1
G	1-0-12	-688	-688	---	FRONT	VERT	TOTAL	---	C1
H	5-0-12	-690	-690	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.29/1.00 (D-E:1) , WB=0.22/1.00 (B-D:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.33 (D) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423567	T32Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	8.0	4.25	1.50

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215228

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T50	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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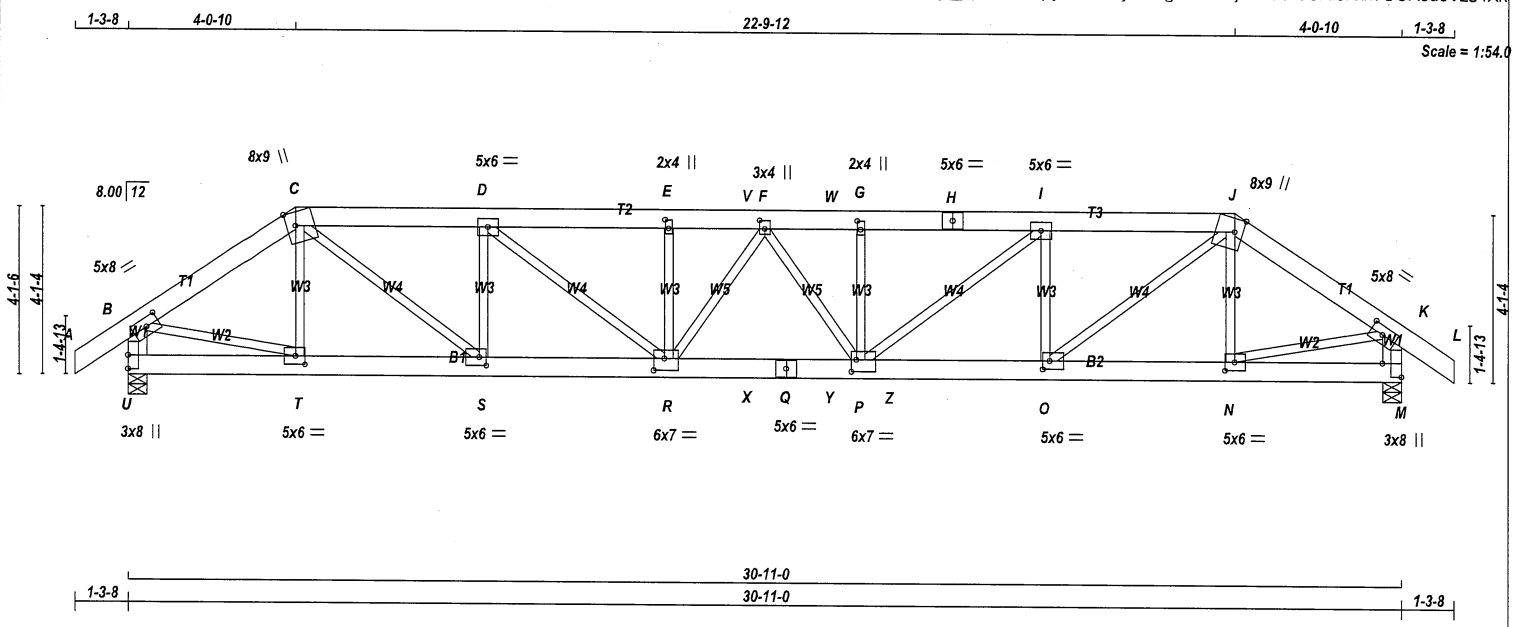

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- H	2	12	TOP
H- J	2	12	TOP
J- L	2	12	TOP
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	TOP
Q- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW+m	MT20	8.0	9.0	Edge	2.50
D	TMWW-t	MT20	5.0	6.0		

Structural component only
DWG# T-2215241

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
						IN-SX	IN-SX
U	2183	0	2183	0	0	5-8	5-8
M	2183	0	2183	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1528	1096 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
M	1528	1096 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00	T- C	-327 / 0	0.04 (1)
B- C	-2396 / 0	-112.4	-112.4	0.09 (1)	6.25	C- S	0 / 1940	0.24 (1)
C- D	-3502 / 0	-112.4	-112.4	0.10 (1)	5.89	S- D	-1132 / 0	0.14 (1)
D- E	-4197 / 0	-112.4	-112.4	0.10 (1)	5.49	D- R	0 / 896	0.11 (1)
E- V	-4197 / 0	-112.4	-112.4	0.05 (1)	5.55	R- E	-368 / 0	0.04 (1)
V- F	-4197 / 0	-112.4	-112.4	0.05 (1)	5.55	O- I	-1132 / 0	0.14 (1)
F- W	-4197 / 0	-112.4	-112.4	0.05 (1)	5.55	O- J	0 / 1939	0.24 (1)
W- G	-4197 / 0	-112.4	-112.4	0.05 (1)	5.55	N- J	-326 / 0	0.04 (1)
G- H	-4197 / 0	-112.4	-112.4	0.10 (1)	5.49	B- T	0 / 2034	0.25 (1)
H- I	-4197 / 0	-112.4	-112.4	0.10 (1)	5.49	N- K	0 / 2035	0.25 (1)
I- J	-3502 / 0	-112.4	-112.4	0.10 (1)	5.89	P- G	-368 / 0	0.04 (1)
J- K	-2397 / 0	-112.4	-112.4	0.09 (1)	6.25	R- F	-135 / 0	0.02 (1)
K- L	0 / 44	-112.4	-112.4	0.04 (1)	10.00	F- P	-135 / 0	0.02 (1)
U- B	-2154 / 0	0.0	0.0	0.08 (1)	7.81	P- I	0 / 895	0.11 (1)
M- K	-2154 / 0	0.0	0.0	0.08 (1)	7.81			
U- T	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
T- S	0 / 1979	-18.5	-18.5	0.15 (1)	10.00			
S- R	0 / 3502	-18.5	-18.5	0.26 (1)	10.00			
R- X	0 / 4270	-18.5	-18.5	0.31 (1)	10.00			
X- Q	0 / 4270	-18.5	-18.5	0.31 (1)	10.00			
Q- Y	0 / 4270	-18.5	-18.5	0.31 (1)	10.00			
Y- P	0 / 4270	-18.5	-18.5	0.31 (1)	10.00			
P- Z	0 / 3502	-18.5	-18.5	0.26 (1)	10.00			
Z- O	0 / 3502	-18.5	-18.5	0.26 (1)	10.00			
O- N	0 / 1980	-18.5	-18.5	0.15 (1)	10.00			
N- M	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

TOTAL WEIGHT = 2 X 168 = 336 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.10/1.00 (D-E:1) , BC=0.31/1.00 (P-R:1) , WB=0.25/1.00 (K-N:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (F) (INPUT = 0.90)
JSI METAL= 0.40 (Q) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T50	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TMWW+t	MT20	3.0	4.0	2.50	1.50
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	Edge	2.50
K	TMVW-t	MT20	5.0	8.0	2.50	3.75
M	BMV1+p	MT20	3.0	8.0	4.00	Edge
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BMWWW-t	MT20	6.0	7.0	3.50	1.50
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	6.0	7.0	3.50	3.00
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215241

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T51	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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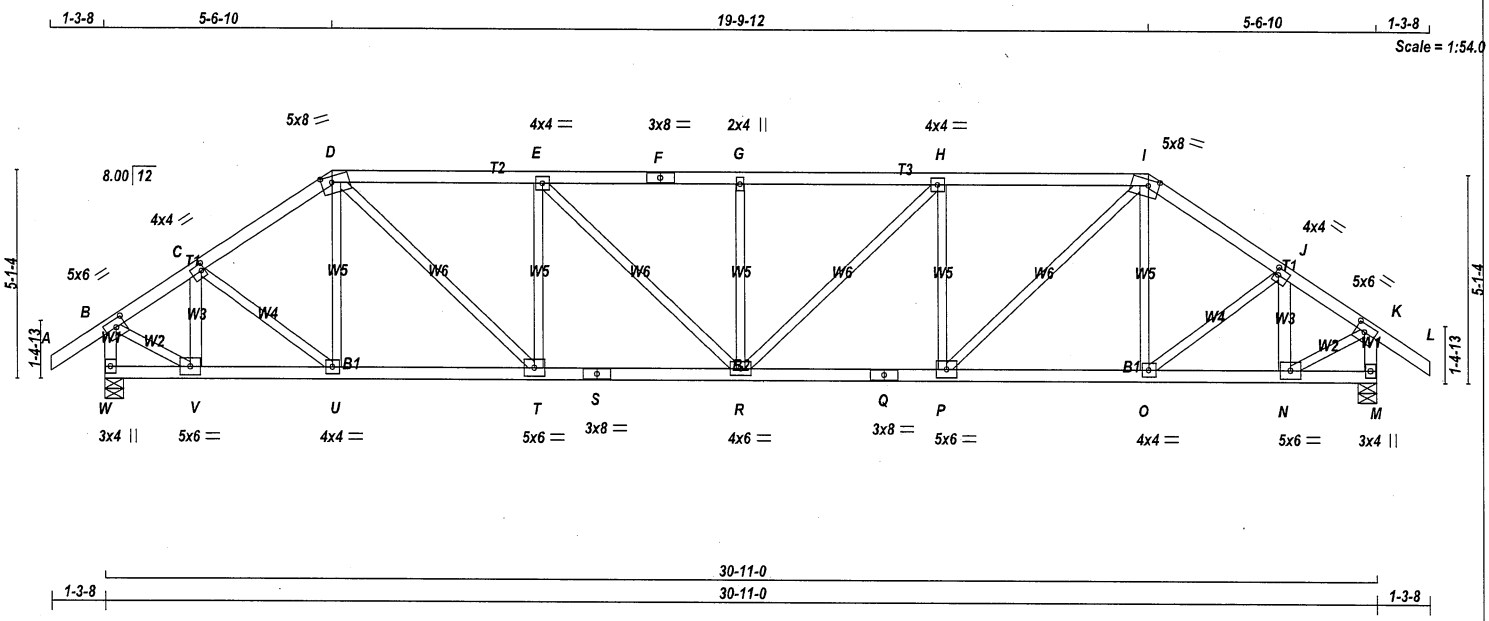

TOTAL WEIGHT = 134 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	5.0	8.0	1.75	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.00
J	TMWW-t	MT20	4.0	4.0	2.00	1.00
K	TMVW-t	MT20	5.0	6.0	2.25	2.75
M	BMV1+p	MT20	3.0	4.0		
N, P, T, V						
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	8.0		
U	BMWW-t	MT20	4.0	4.0		
W	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215242

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2178	0	2178	0	0	5-8	5-8
M	2178	0	2178	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
M	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-1974 / 0	-112.4	-112.4	0.18 (1)	4.63
C- D	-2344 / 0	-112.4	-112.4	0.23 (1)	4.28
D- E	-2987 / 0	-112.4	-112.4	0.58 (1)	3.47
E- F	-3291 / 0	-112.4	-112.4	0.61 (1)	3.29
F- G	-3291 / 0	-112.4	-112.4	0.61 (1)	3.29
G- H	-3291 / 0	-112.4	-112.4	0.61 (1)	3.29
H- I	-2987 / 0	-112.4	-112.4	0.58 (1)	3.47
I- J	-2345 / 0	-112.4	-112.4	0.23 (1)	4.28
J- K	-1977 / 0	-112.4	-112.4	0.18 (1)	4.63
K- L	0 / 43	-112.4	-112.4	0.15 (1)	10.00
W- B	-2151 / 0	0.0	0.0	0.22 (1)	5.78
M- K	-2151 / 0	0.0	0.0	0.22 (1)	5.78
W- V	0 / 0	-18.5	-18.5	0.03 (1)	10.00
V- U	0 / 1663	-18.5	-18.5	0.32 (1)	10.00
U- T	0 / 1930	-18.5	-18.5	0.36 (1)	10.00
T- S	0 / 2987	-18.5	-18.5	0.53 (1)	10.00
S- R	0 / 2987	-18.5	-18.5	0.53 (1)	10.00
R- Q	0 / 2988	-18.5	-18.5	0.53 (1)	10.00
Q- P	0 / 2988	-18.5	-18.5	0.53 (1)	10.00
P- O	0 / 1931	-18.5	-18.5	0.36 (1)	10.00
O- N	0 / 1665	-18.5	-18.5	0.32 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.03 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
V- C	-776 / 0	0.10 (1)
U- D	-107 / 31	0.04 (1)
O- I	-106 / 31	0.04 (1)
O- J	0 / 335	0.08 (1)
N- J	-775 / 0	0.10 (1)
B- V	0 / 1849	0.42 (1)
N- K	0 / 1850	0.42 (1)
P- I	0 / 1464	0.33 (1)
D- T	0 / 1464	0.33 (1)
P- H	-924 / 0	0.36 (1)
T- E	-924 / 0	0.36 (1)
R- H	0 / 425	0.10 (1)
E- R	0 / 425	0.10 (1)
R- G	-507 / 0	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.61/1.00 (E-G:1) , BC=0.53/1.00 (P-R:1) , WB=0.42/1.00 (K-N:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.90 (S) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T51C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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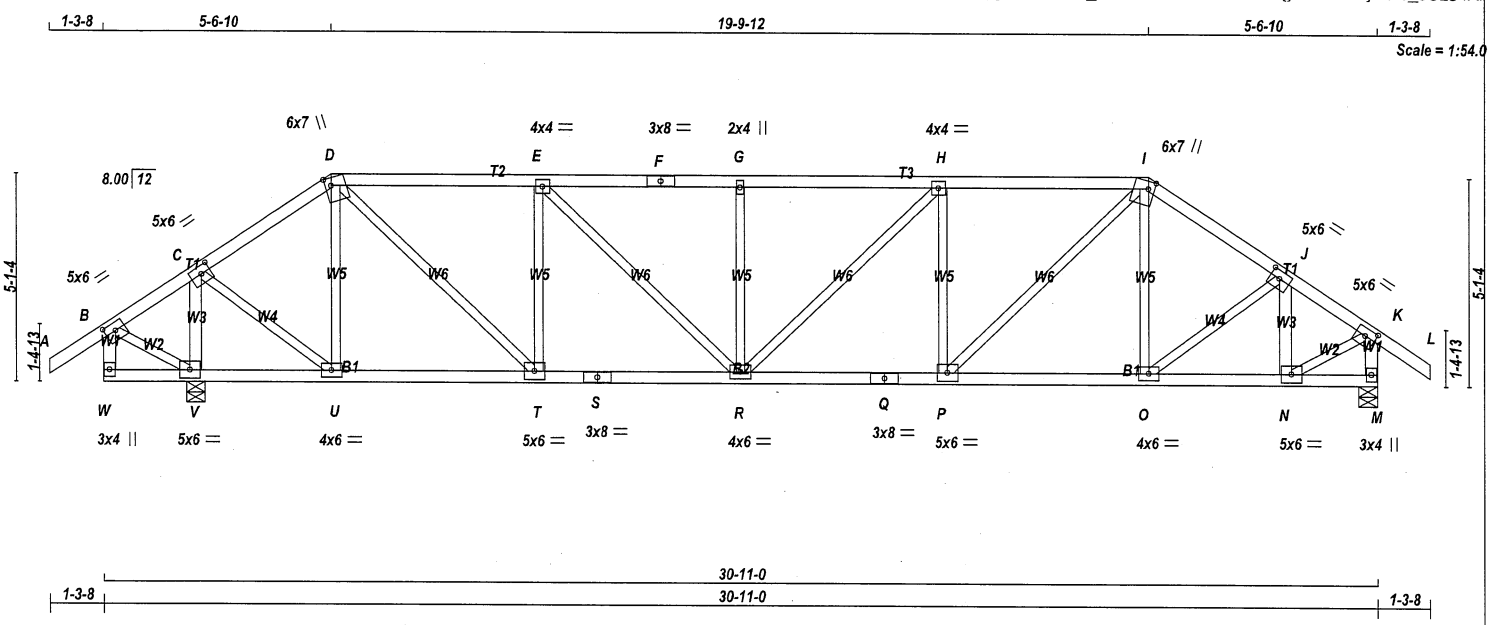

TOTAL WEIGHT = 134 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	3.00
C	TMWW-t	MT20	5.0	6.0	2.25	2.75
D	TTWW+m	MT20	6.0	7.0	2.25	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.25	1.75
J	TMWW-t	MT20	5.0	6.0	2.25	2.75
K	TMVW-t	MT20	5.0	6.0	2.25	3.00
M	BMV1+p	MT20	3.0	4.0		
N, P, T						
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	8.0		
U	BMWW-t	MT20	4.0	6.0		
V	BMWW1-t	MT20	5.0	6.0		
W	BMV+p	MT20	3.0	4.0		

NOTES- (1)
1)

Structural component only
DWG# T-2215243

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2348	0	2348	0	0	5-8	5-8
M	2009	0	2009	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1642	1178 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
M	1406	1008 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED (PLF) FROM	TO	VERT. LOAD LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00	V- C	-2178 / 0	0.27 (1)
B- C	0 / 262	-112.4	-112.4	0.23 (1)		10.00	C- U	0 / 1552	0.35 (1)
C- D	-1290 / 0	-112.4	-112.4	0.21 (1)		5.43	U- D	-859 / 0	0.33 (1)
D- E	-2272 / 0	-112.4	-112.4	0.51 (1)		3.99	O- I	-69 / 41	0.03 (1)
E- F	-2747 / 0	-112.4	-112.4	0.55 (1)		3.63	O- J	0 / 271	0.06 (1)
F- G	-2747 / 0	-112.4	-112.4	0.55 (1)		3.63	N- J	-703 / 0	0.09 (1)
G- H	-2747 / 0	-112.4	-112.4	0.55 (1)		3.63	B- V	-211 / 0	0.03 (1)
H- I	-2617 / 0	-112.4	-112.4	0.54 (1)		3.72	N- K	0 / 1688	0.38 (1)
I- J	-2108 / 0	-112.4	-112.4	0.21 (1)		4.49	P- I	0 / 1222	0.27 (1)
J- K	-1801 / 0	-112.4	-112.4	0.17 (1)		4.82	D- T	0 / 1716	0.39 (1)
K- L	0 / 43	-112.4	-112.4	0.15 (1)		10.00	P- H	-756 / 0	0.29 (1)
W- B	0 / 13	0.0	0.0	0.00 (4)		10.00	T- E	-1095 / 0	0.42 (1)
M- K	-1983 / 0	0.0	0.0	0.20 (1)		5.99	R- H	0 / 183	0.04 (1)
							E- R	0 / 666	0.15 (1)
W- V	0 / 0	-18.5	-18.5	0.06 (1)		10.00	R- G	-507 / 0	0.19 (1)
V- U	-190 / 0	-18.5	-18.5	0.07 (4)		6.25			
U- T	0 / 1034	-18.5	-18.5	0.20 (1)		10.00			
T- S	0 / 2272	-18.5	-18.5	0.41 (1)		10.00			
S- R	0 / 2272	-18.5	-18.5	0.41 (1)		10.00			
R- Q	0 / 2617	-18.5	-18.5	0.47 (1)		10.00			
Q- P	0 / 2617	-18.5	-18.5	0.47 (1)		10.00			
P- O	0 / 1735	-18.5	-18.5	0.33 (1)		10.00			
O- N	0 / 1519	-18.5	-18.5	0.29 (1)		10.00			
N- M	0 / 0	-18.5	-18.5	0.03 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.55/1.00 (E-G:1) , BC=0.47/1.00 (P-R:1) , WB=0.42/1.00 (E-T:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.79 (S) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T52	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 12:47:14 2022 Page 1
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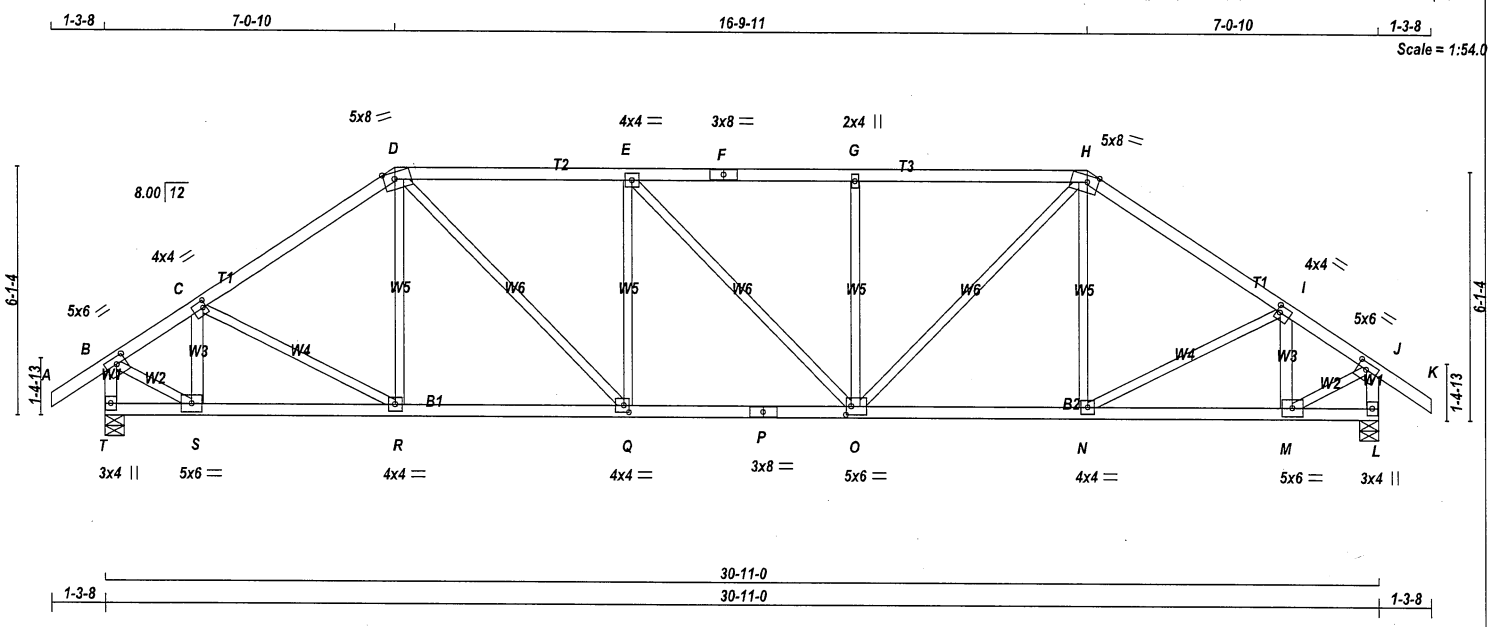

TOTAL WEIGHT = 134 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMVW-t	MT20	5.0	6.0	2.00	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215244

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2178	0	2178	0	0	5-8	5-8
L	2178	0	2178	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00
B- C	-2009 / 0	-112.4	-112.4	0.36 (1)		4.36
C- D	-2324 / 0	-112.4	-112.4	0.45 (1)		4.08
D- E	-2670 / 0	-112.4	-112.4	0.67 (1)		3.54
E- F	-2668 / 0	-112.4	-112.4	0.66 (1)		3.54
F- G	-2668 / 0	-112.4	-112.4	0.66 (1)		3.54
G- H	-2668 / 0	-112.4	-112.4	0.66 (1)		3.55
H- I	-2325 / 0	-112.4	-112.4	0.45 (1)		4.08
I- J	-2011 / 0	-112.4	-112.4	0.36 (1)		4.36
J- K	0 / 43	-112.4	-112.4	0.15 (1)		10.00
T- B	-2158 / 0	0.0	0.0	0.22 (1)		5.78
L- J	-2158 / 0	0.0	0.0	0.22 (1)		5.78
T- S	0 / 0	-18.5	-18.5	0.05 (4)		10.00
S- R	0 / 1718	-18.5	-18.5	0.34 (1)		10.00
R- Q	0 / 1910	-18.5	-18.5	0.37 (1)		10.00
Q- P	0 / 2670	-18.5	-18.5	0.48 (1)		10.00
P- O	0 / 2670	-18.5	-18.5	0.48 (1)		10.00
O- N	0 / 1911	-18.5	-18.5	0.38 (1)		10.00
N- M	0 / 1720	-18.5	-18.5	0.35 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.05 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-777 / 0	0.10 (1)
C- R	0 / 216	0.05 (1)
R- D	0 / 80	0.03 (4)
D- Q	0 / 1090	0.25 (1)
Q- E	-681 / 0	0.40 (1)
E- O	-2 / 0	0.00 (1)
O- G	-680 / 0	0.40 (1)
O- H	0 / 1087	0.24 (1)
N- H	0 / 81	0.03 (4)
N- I	0 / 215	0.05 (1)
M- I	-776 / 0	0.10 (1)
B- S	0 / 1910	0.43 (1)
M- J	0 / 1911	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.48/1.00 (O-Q:1) , WB=0.43/1.00 (J-M:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.84 (P) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T52C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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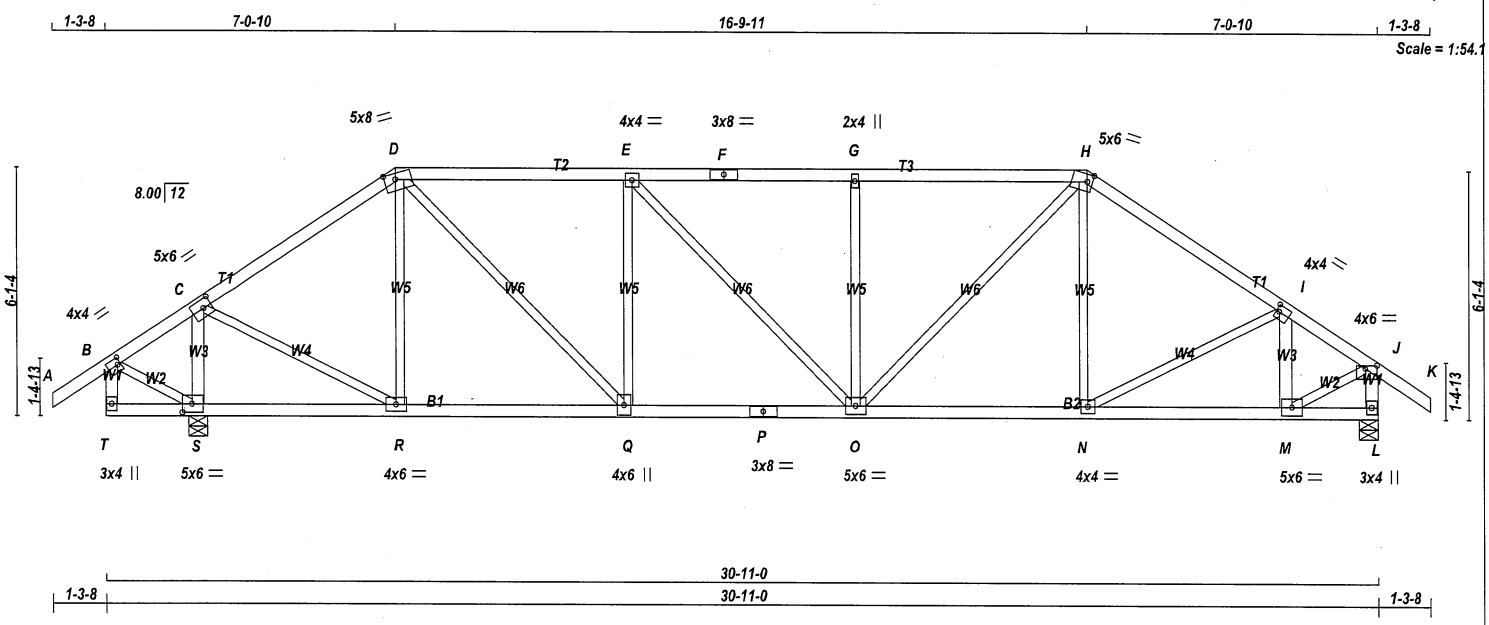

TOTAL WEIGHT = 134 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	1.75	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.25	1.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMVW-p	MT20	4.0	6.0	1.00	3.25
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	6.0		
S	BMWW1m	MT20	5.0	6.0	2.50	2.75
T	BMV+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215245

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2348	0	2348	0	0	5-8	5-8
L	2009	0	2009	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1642	1178 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
L	1406	1008 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM	TO		
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 231	-112.4	-112.4	0.39 (1)	10.00
C- D	-1501 / 0	-112.4	-112.4	0.42 (1)	4.86
D- E	-2140 / 0	-112.4	-112.4	0.61 (1)	3.96
E- F	-2299 / 0	-112.4	-112.4	0.62 (1)	3.82
F- G	-2299 / 0	-112.4	-112.4	0.62 (1)	3.82
G- H	-2299 / 0	-112.4	-112.4	0.62 (1)	3.83
H- I	-2077 / 0	-112.4	-112.4	0.42 (1)	4.29
I- J	-1835 / 0	-112.4	-112.4	0.35 (1)	4.53
J- K	0 / 43	-112.4	-112.4	0.15 (1)	10.00
T- B	-4 / 6	0.0	0.0	0.00 (4)	7.81
L- J	-1989 / 0	0.0	0.0	0.20 (1)	5.98
T- S	0 / 0	-18.5	-18.5	0.08 (4)	10.00
S- R	-139 / 0	-18.5	-18.5	0.10 (4)	6.25
R- Q	0 / 1211	-18.5	-18.5	0.25 (1)	10.00
Q- P	0 / 2140	-18.5	-18.5	0.40 (1)	10.00
P- O	0 / 2140	-18.5	-18.5	0.40 (1)	10.00
O- N	0 / 1704	-18.5	-18.5	0.34 (1)	10.00
N- M	0 / 1574	-18.5	-18.5	0.32 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.05 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
S- C	-2186 / 0	0.27 (1)
C- R	0 / 1535	0.35 (1)
R- D	-616 / 0	0.36 (1)
D- Q	0 / 1332	0.30 (1)
Q- E	-851 / 0	0.50 (1)
E- O	0 / 231	0.05 (1)
O- G	-681 / 0	0.40 (1)
O- H	0 / 853	0.19 (1)
N- H	0 / 89	0.03 (4)
N- I	0 / 146	0.03 (1)
M- I	-704 / 0	0.09 (1)
B- S	-154 / 0	0.02 (1)
M- J	0 / 1748	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.62/1.00 (E-G:1) , BC=0.40/1.00 (O-Q:1) , WB=0.50/1.00 (E-Q:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.69 (P) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T53	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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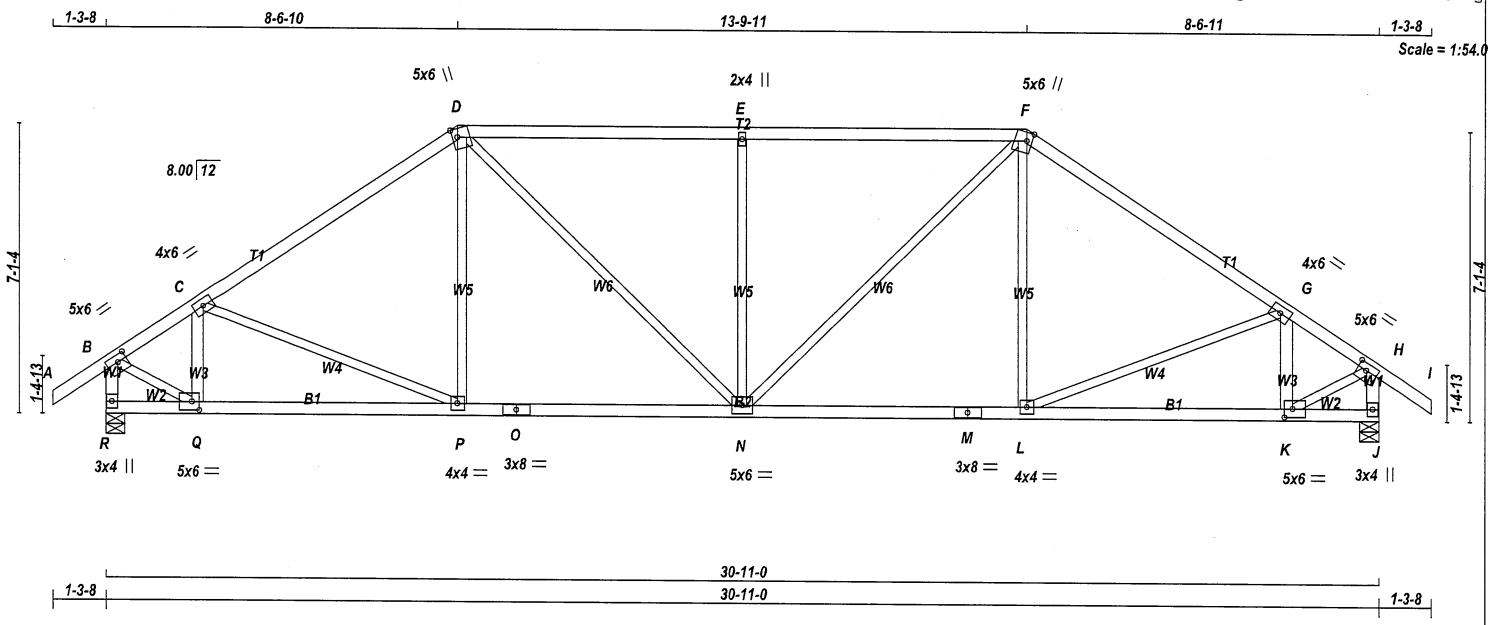

TOTAL WEIGHT = 131 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.00	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215246

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2178	0	2178	0	0	5-8	5-8
J	2178	0	2178	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)				FR-TO		
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	Q- C	-792 / 0	0.10 (1)
B- C	-2059 / 0	-112.4	-112.4	0.48 (1)	4.17	C- P	0 / 58	0.01 (1)
C- D	-2265 / 0	-112.4	-112.4	0.67 (1)	3.83	P- D	0 / 131	0.05 (4)
D- E	-2400 / 0	-112.4	-112.4	0.81 (1)	3.34	D- N	0 / 766	0.17 (1)
E- F	-2400 / 0	-112.4	-112.4	0.81 (1)	3.34	N- E	-955 / 0	0.83 (1)
F- G	-2265 / 0	-112.4	-112.4	0.67 (1)	3.83	N- F	0 / 765	0.17 (1)
G- H	-2062 / 0	-112.4	-112.4	0.48 (1)	4.17	L- F	0 / 131	0.05 (4)
H- I	0 / 43	-112.4	-112.4	0.15 (1)	10.00	L- G	0 / 56	0.01 (1)
R- B	-2167 / 0	0.0	0.0	0.22 (1)	5.77	K- G	-791 / 0	0.10 (1)
J- H	-2167 / 0	0.0	0.0	0.22 (1)	5.77	B- Q	0 / 1999	0.45 (1)
						K- H	0 / 2000	0.45 (1)
R- Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
Q- P	0 / 1799	-18.5	-18.5	0.39 (1)	10.00			
P- O	0 / 1854	-18.5	-18.5	0.40 (1)	10.00			
O- N	0 / 1854	-18.5	-18.5	0.40 (1)	10.00			
N- M	0 / 1855	-18.5	-18.5	0.40 (1)	10.00			
M- L	0 / 1855	-18.5	-18.5	0.40 (1)	10.00			
L- K	0 / 1801	-18.5	-18.5	0.39 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.40/1.00 (L-N:1) , WB=0.83/1.00 (E-N:1) , SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.53 (O) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T53C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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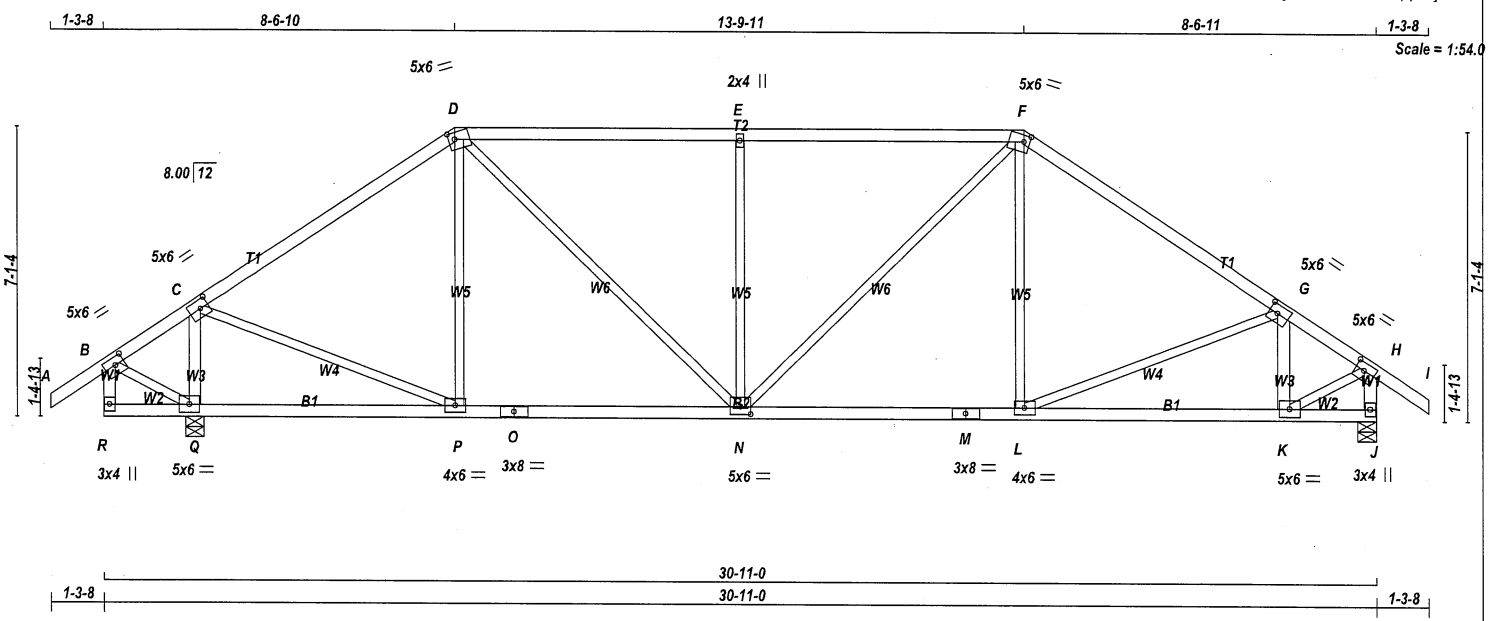

TOTAL WEIGHT = 131 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.75
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMVW-t	MT20	5.0	6.0	2.25	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	6.0	2.25	3.00
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW1-t	MT20	5.0	6.0		
R	BMV+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215247

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2348	0	2348	0	0	5-8	5-8
J	2009	0	2009	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1642	1178 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
J	1406	1008 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
	A- B	0 / 43	-112.4	-112.4	1	0.15 (1)	10.00	Q- C	-2205 / 0	0.27 (1)
	B- C	0 / 183	-112.4	-112.4	1	0.63 (1)	10.00	C- P	0 / 1467	0.33 (1)
	C- D	-1606 / 0	-112.4	-112.4	1	0.60 (1)	4.48	P- D	-422 / 0	0.37 (1)
	D- E	-2016 / 0	-112.4	-112.4	1	0.77 (1)	3.66	D- N	0 / 1012	0.23 (1)
	E- F	-2016 / 0	-112.4	-112.4	1	0.77 (1)	3.66	N- E	-957 / 0	0.84 (1)
	F- G	-2009 / 0	-112.4	-112.4	1	0.64 (1)	4.05	N- F	0 / 526	0.12 (1)
	G- H	-1886 / 0	-112.4	-112.4	1	0.47 (1)	4.34	L- F	0 / 139	0.05 (4)
	H- I	0 / 43	-112.4	-112.4	1	0.15 (1)	10.00	L- G	-17 / 13	0.01 (1)
	R- B	-11 / 0	0.0	0.0	1	0.00 (1)	7.81	K- G	-719 / 0	0.09 (1)
	J- H	-1998 / 0	0.0	0.0	1	0.21 (1)	5.97	B- Q	-67 / 0	0.01 (1)
								K- H	0 / 1838	0.41 (1)
	R- Q	0 / 0	-18.5	-18.5	1	0.13 (4)	10.00			
	Q- P	-60 / 0	-18.5	-18.5	1	0.12 (4)	6.25			
	P- O	0 / 1296	-18.5	-18.5	1	0.30 (1)	10.00			
	O- N	0 / 1296	-18.5	-18.5	1	0.30 (1)	10.00			
	N- M	0 / 1641	-18.5	-18.5	1	0.37 (1)	10.00			
	M- L	0 / 1641	-18.5	-18.5	1	0.37 (1)	10.00			
	L- K	0 / 1655	-18.5	-18.5	1	0.37 (1)	10.00			
	K- J	0 / 0	-18.5	-18.5	1	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.77/1.00 (D-E:1) , BC=0.37/1.00 (K-L:1) , WB=0.84/1.00 (E-N:1) , SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T54	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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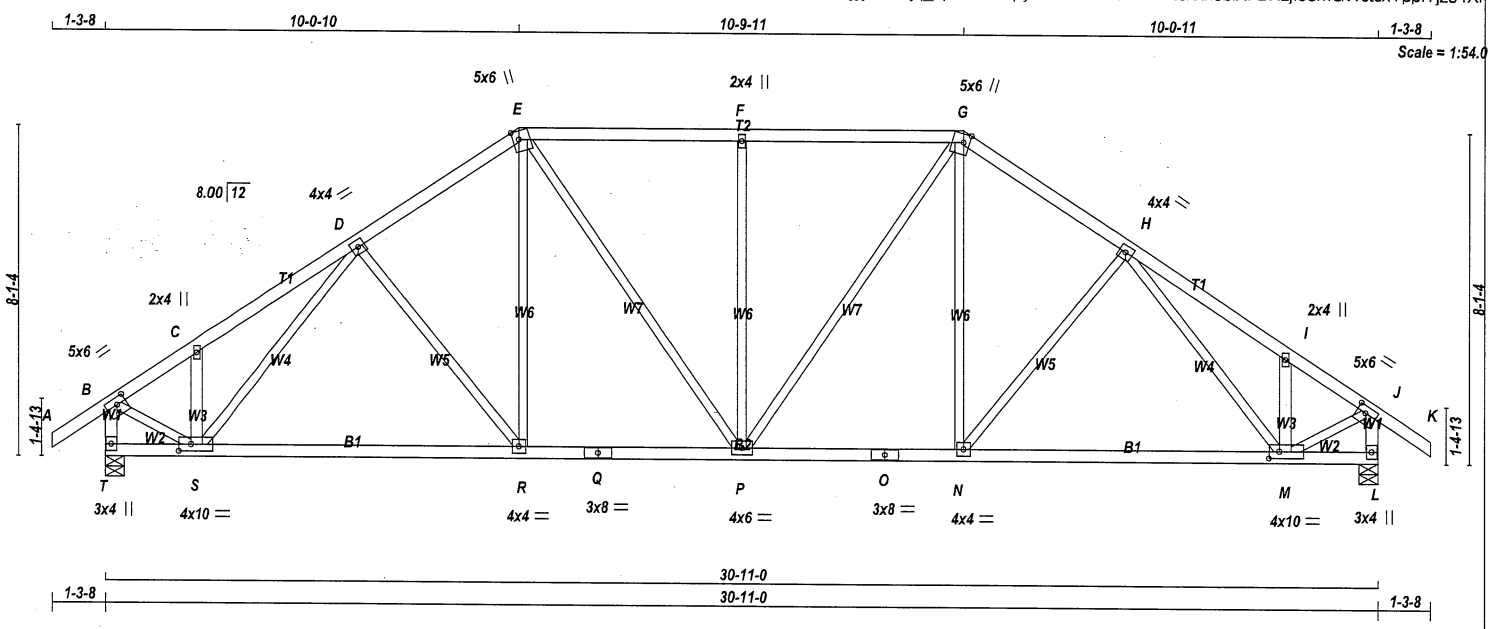

TOTAL WEIGHT = 143 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C, F, I						
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TTWW+m	MT20	5.0	6.0	Edge	1.75
H	TMWW-t	MT20	4.0	4.0		
J	TMVW-t	MT20	5.0	6.0	2.00	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWWW-t	MT20	4.0	10.0	2.00	3.00
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWWW-t	MT20	4.0	10.0	2.00	3.50
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only

DWG# T-2215248

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
						IN-SX	IN-SX
T	2178	0	2178	0	0	5-8	5-8
L	2178	0	2178	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 43	-112.4	-112.4	0.15 (1)		10.00
B- C		-2012 / 0	-112.4	-112.4	0.13 (1)		4.67
C- D		-2006 / 0	-112.4	-112.4	0.25 (1)		4.56
D- E		-2154 / 0	-112.4	-112.4	0.25 (1)		4.42
E- F		-2054 / 0	-112.4	-112.4	0.48 (1)		4.21
F- G		-2054 / 0	-112.4	-112.4	0.48 (1)		4.21
G- H		-2155 / 0	-112.4	-112.4	0.25 (1)		4.42
H- I		-2008 / 0	-112.4	-112.4	0.25 (1)		4.55
I- J		-2014 / 0	-112.4	-112.4	0.13 (1)		4.67
J- K		0 / 43	-112.4	-112.4	0.15 (1)		10.00
T- B		-2185 / 0	0.0	0.0	0.23 (1)		5.75
L- J		-2184 / 0	0.0	0.0	0.23 (1)		5.75
T- S		0 / 0	-18.5	-18.5	0.18 (4)		10.00
S- R		0 / 1946	-18.5	-18.5	0.43 (1)		10.00
R- Q		0 / 1773	-18.5	-18.5	0.40 (1)		10.00
Q- P		0 / 1773	-18.5	-18.5	0.40 (1)		10.00
P- O		0 / 1773	-18.5	-18.5	0.40 (1)		10.00
O- N		0 / 1773	-18.5	-18.5	0.40 (1)		10.00
N- M		0 / 1947	-18.5	-18.5	0.43 (1)		10.00
M- L		0 / 0	-18.5	-18.5	0.18 (4)		10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C		-353 / 0	0.04 (1)
S- D		-440 / 0	0.33 (1)
D- R		-291 / 0	0.23 (1)
R- E		0 / 369	0.08 (1)
E- P		0 / 495	0.11 (1)
P- F		-742 / 0	0.94 (1)
P- G		0 / 494	0.11 (1)
N- G		0 / 370	0.08 (1)
N- H		-292 / 0	0.23 (1)
H- M		-438 / 0	0.33 (1)
M- I		-353 / 0	0.04 (1)
B- S		0 / 1881	0.42 (1)
M- J		0 / 1882	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.48/1.00 (E-F:1) , BC=0.43/1.00 (M-N:1) , WB=0.94/1.00 (F-P:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.50 (Q) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T54C	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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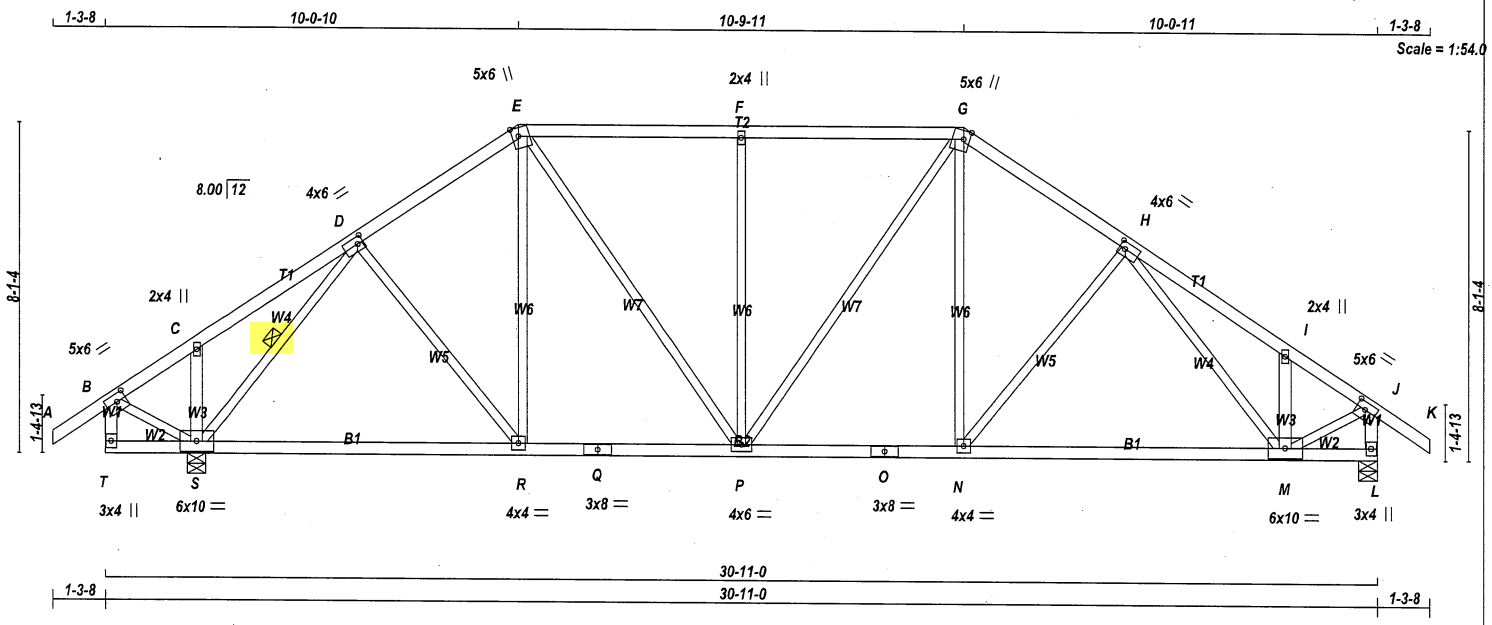

TOTAL WEIGHT = 143 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C, F, I						
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	6.0	2.00	1.75
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TTWW+m	MT20	5.0	6.0	Edge	1.75
H	TMWW-t	MT20	4.0	6.0	2.00	1.75
J	TMVW-t	MT20	5.0	6.0	2.25	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWWW-t	MT20	6.0	10.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWWW1-t	MT20	6.0	10.0		
T	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215249

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2348	0	2348	0	0	5-8	5-8
L	2009	0	2009	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1642	1178 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0
L	1406	1008 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00
B- C	0 / 223	-112.4	-112.4	0.24 (1)		10.00
C- D	0 / 201	-112.4	-112.4	0.26 (1)		10.00
D- E	-1612 / 0	-112.4	-112.4	0.22 (1)		5.00
E- F	-1720 / 0	-112.4	-112.4	0.46 (1)		4.55
F- G	-1720 / 0	-112.4	-112.4	0.46 (1)		4.55
G- H	-1891 / 0	-112.4	-112.4	0.24 (1)		4.67
H- I	-1833 / 0	-112.4	-112.4	0.24 (1)		4.74
I- J	-1838 / 0	-112.4	-112.4	0.13 (1)		4.85
J- K	0 / 43	-112.4	-112.4	0.15 (1)		10.00
T- B	-28 / 0	0.0	0.0	0.00 (4)		7.81
L- J	-2016 / 0	0.0	0.0	0.21 (1)		5.94
T- S	0 / 0	-18.5	-18.5	0.21 (4)		10.00
S- R	0 / 1135	-18.5	-18.5	0.33 (1)		10.00
R- Q	0 / 1318	-18.5	-18.5	0.31 (1)		10.00
Q- P	0 / 1318	-18.5	-18.5	0.31 (1)		10.00
P- O	0 / 1554	-18.5	-18.5	0.36 (1)		10.00
O- N	0 / 1554	-18.5	-18.5	0.36 (1)		10.00
N- M	0 / 1746	-18.5	-18.5	0.40 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.18 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-404 / 0	0.05 (1)
S- D	-2158 / 0	0.63 (1)
D- R	0 / 300	0.07 (1)
R- E	-109 / 54	0.14 (1)
E- P	0 / 706	0.16 (1)
P- F	-744 / 0	0.94 (1)
P- G	0 / 291	0.07 (1)
N- G	0 / 392	0.09 (1)
N- H	-321 / 0	0.25 (1)
H- M	-348 / 0	0.26 (1)
M- I	-353 / 0	0.04 (1)
B- S	-170 / 0	0.03 (1)
M- J	0 / 1719	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.46/1.00 (E-F:1) , BC=0.40/1.00 (M-N:1) , WB=0.94/1.00 (F-P:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (P) (INPUT = 0.90)
JSI METAL= 0.47 (D) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T55	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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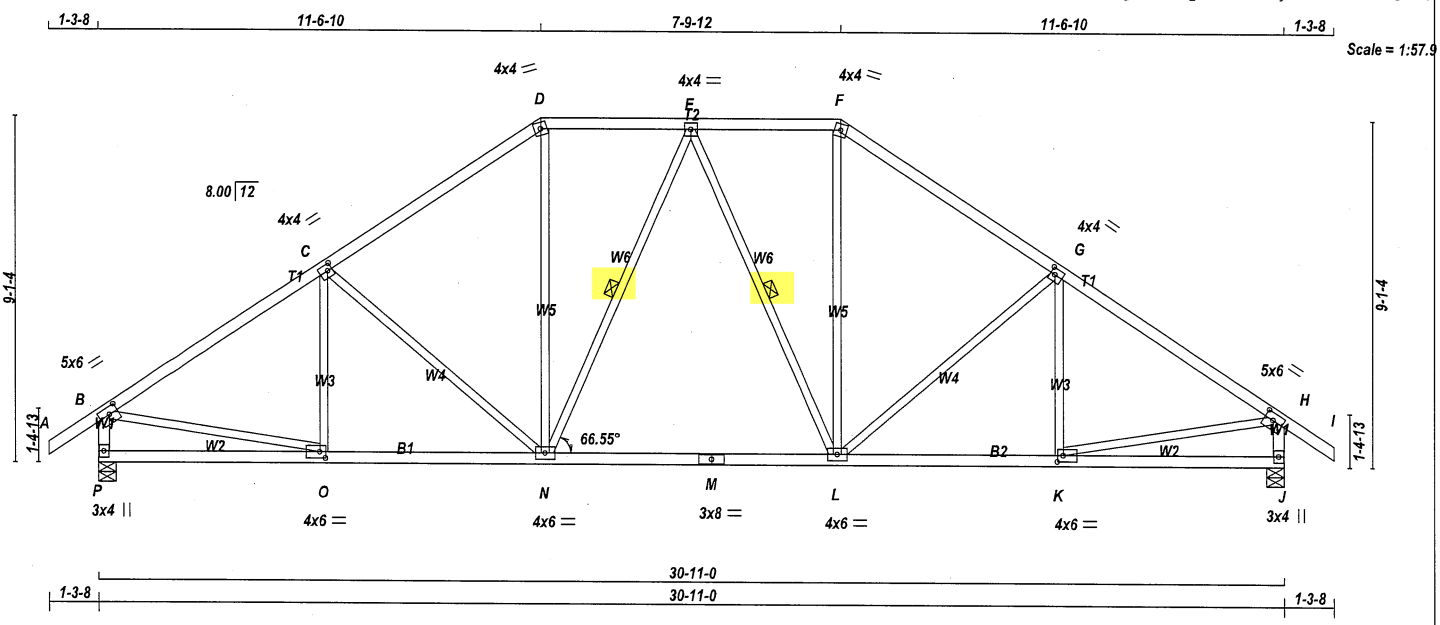

TOTAL WEIGHT = 3 X 140 = 419 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-t	MT20	5.0	6.0	2.25	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.75
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.75
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2178	0	2178	0	0	5-8	5-8
J	2178	0	2178	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	1524	1093 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	O- C	-264 / 21	0.11 (1)	
B- C	-2391 / 0	-112.4	-112.4	0.61 (1)	3.81	C- N	-471 / 0	0.56 (1)	
C- D	-2052 / 0	-112.4	-112.4	0.56 (1)	4.11	N- D	0 / 701	0.16 (1)	
D- E	-1677 / 0	-112.4	-112.4	0.23 (1)	4.90	N- E	-286 / 0	0.19 (1)	
E- F	-1677 / 0	-112.4	-112.4	0.23 (1)	4.90	E- L	-286 / 0	0.19 (1)	
F- G	-2052 / 0	-112.4	-112.4	0.56 (1)	4.11	L- F	0 / 701	0.16 (1)	
G- H	-2392 / 0	-112.4	-112.4	0.61 (1)	3.81	L- G	-472 / 0	0.56 (1)	
H- I	0 / 43	-112.4	-112.4	0.15 (1)	10.00	K- G	-263 / 21	0.11 (1)	
P- B	-2131 / 0	0.0	0.0	0.22 (1)	5.81	B- O	0 / 2059	0.46 (1)	
J- H	-2131 / 0	0.0	0.0	0.22 (1)	5.81	K- H	0 / 2059	0.46 (1)	
P- O	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
O- N	0 / 2025	-18.5	-18.5	0.43 (1)	10.00				
N- M	0 / 1790	-18.5	-18.5	0.39 (1)	10.00				
M- L	0 / 1790	-18.5	-18.5	0.39 (1)	10.00				
L- K	0 / 2026	-18.5	-18.5	0.43 (1)	10.00				
K- J	0 / 0	-18.5	-18.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.61/1.00 (G-H:1) , BC=0.43/1.00 (K-L:1) , WB=0.56/1.00 (G-L:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 1.00)

Structural component only
DWG# T-2215250

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T56	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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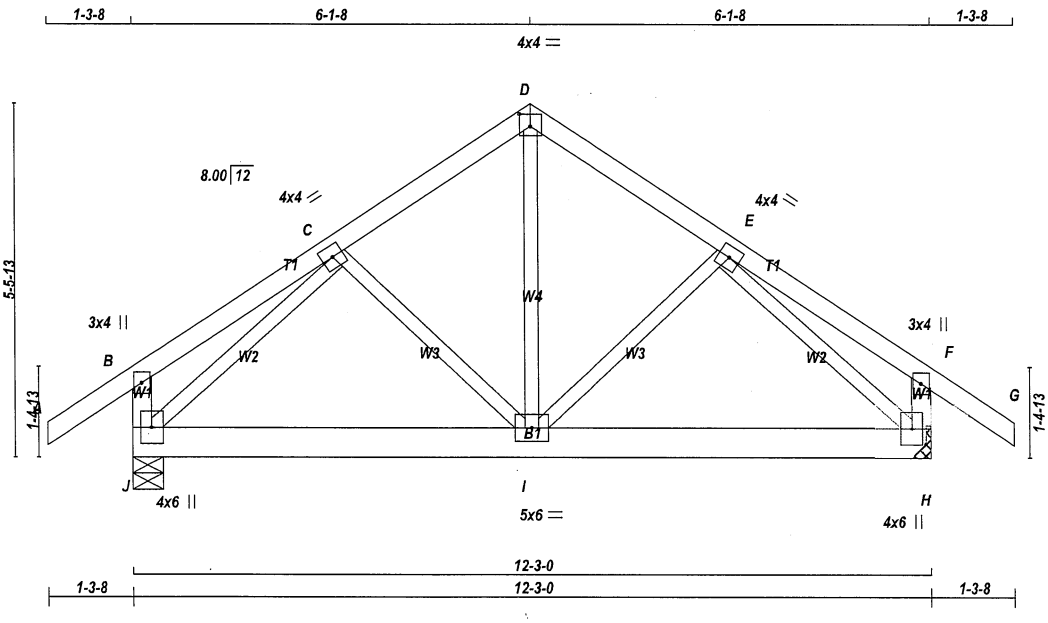

TOTAL WEIGHT = 2 X 61 = 122 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	956	0	956	0	0	5-8	5-8
H	956	0	956	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	668	487 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	668	487 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	I- D	0 / 390	0.09 (1)
B- C	0 / 21	-112.4	-112.4	0.16 (1)	10.00	I- E	-184 / 0	0.06 (1)
C- D	-615 / 0	-112.4	-112.4	0.13 (1)	6.25	C- I	-184 / 0	0.06 (1)
D- E	-615 / 0	-112.4	-112.4	0.13 (1)	6.25	J- C	-881 / 0	0.26 (1)
E- F	0 / 21	-112.4	-112.4	0.16 (1)	10.00	E- H	-881 / 0	0.26 (1)
F- G	0 / 43	-112.4	-112.4	0.15 (1)	10.00			
J- B	-287 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-287 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 625	-18.5	-18.5	0.12 (4)	10.00			
I- H	0 / 625	-18.5	-18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.12/1.00 (H-I:4) , WB=0.26/1.00 (C-J:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

Structural component only
DWG# T-2215251

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T56Z	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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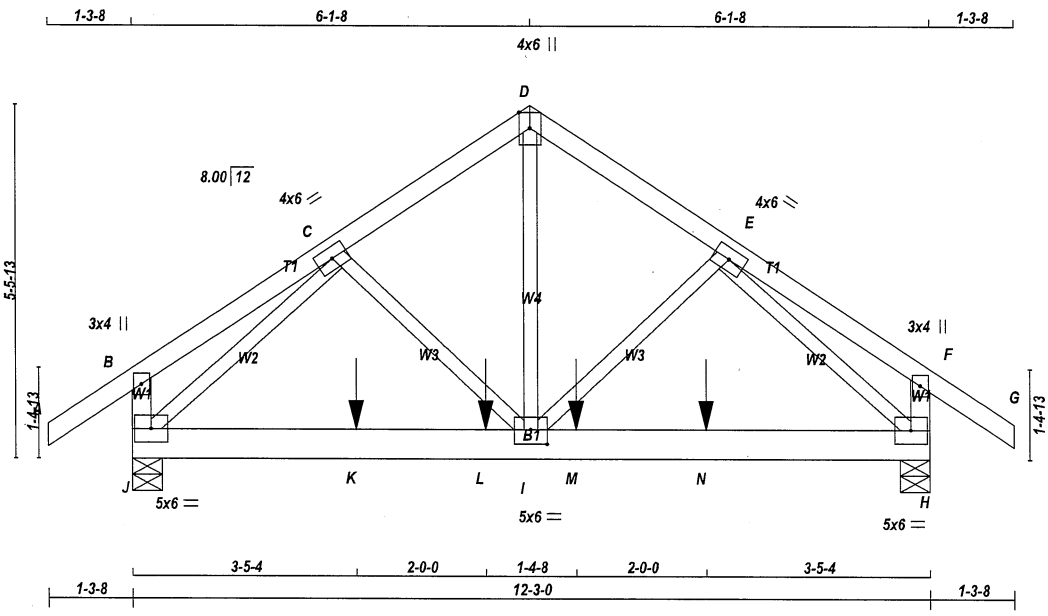

TOTAL WEIGHT = 61 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	6.0	2.75	3.00
J	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1591	0	1591	0	0	5-8	5-8
H	1591	0	1591	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1109	819 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0
H	1109	819 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00	I- D	0 / 1243	0.31 (1)
B- C	0 / 19	-112.4	-112.4	0.16 (1)	10.00	I- E	-28 / 30	0.01 (4)
C- D	-1390 / 0	-112.4	-112.4	0.19 (1)	5.29	C- I	-28 / 30	0.01 (4)
D- E	-1390 / 0	-112.4	-112.4	0.19 (1)	5.29	J- C	-1636 / 0	0.50 (1)
E- F	0 / 19	-112.4	-112.4	0.16 (1)	10.00	E- H	-1636 / 0	0.50 (1)
F- G	0 / 43	-112.4	-112.4	0.17 (1)	10.00			
J- B	-292 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-292 / 0	0.0	0.0	0.03 (1)	7.81			
J- K	0 / 1161	-18.5	-18.5	0.59 (1)	10.00			
K- L	0 / 1161	-18.5	-18.5	0.59 (1)	10.00			
L- I	0 / 1161	-18.5	-18.5	0.59 (1)	10.00			
I- M	0 / 1161	-18.5	-18.5	0.59 (1)	10.00			
M- N	0 / 1161	-18.5	-18.5	0.59 (1)	10.00			
N- H	0 / 1161	-18.5	-18.5	0.59 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	3-5-4	-244	-244	---	FRONT	VERT	TOTAL	---	C1
L	5-5-4	-197	-197	---	FRONT	VERT	TOTAL	---	C1
M	6-9-12	-197	-197	---	FRONT	VERT	TOTAL	---	C1
N	8-9-12	-244	-244	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.19/1.00 (D-E:1) , BC=0.59/1.00 (I-J:1) , WB=0.50/1.00 (E-H:1) , SSI=0.40/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

Structural component only
DWG# T-2215252

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T57	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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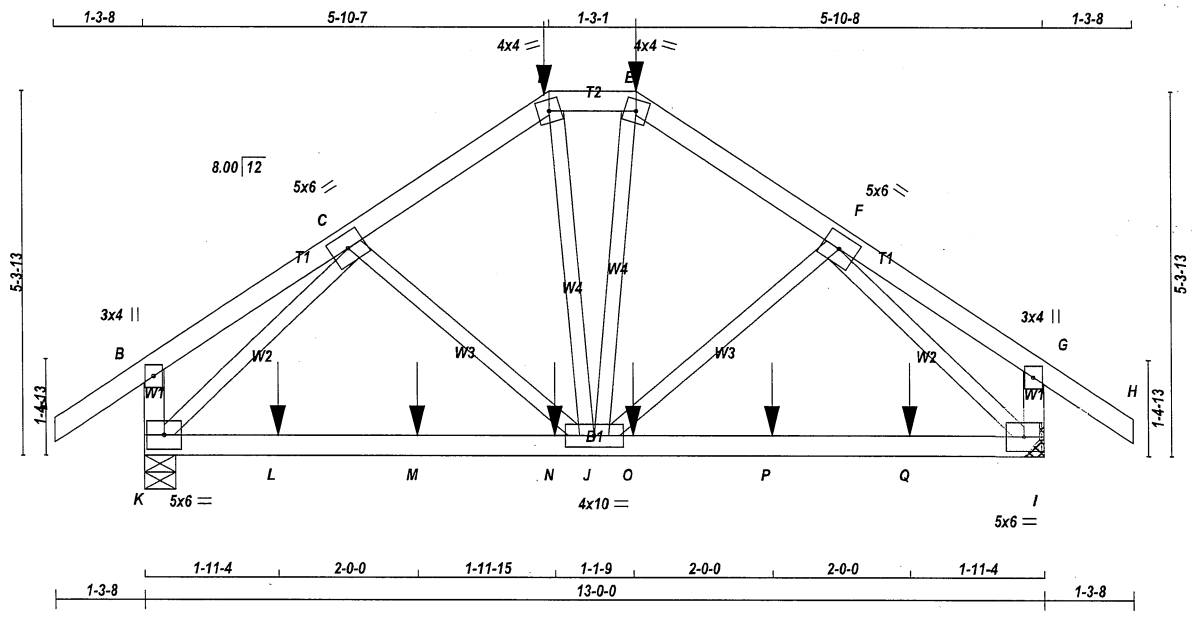


TOTAL WEIGHT = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TTW-m	MT20	4.0	4.0	
F	TMWW-t	MT20	5.0	6.0	
G	TMV+p	MT20	3.0	4.0	
I	BMVW1-t	MT20	5.0	6.0	
J	BMWWWW*-I	MT20	4.0	10.0	
K	BMVW1-t	MT20	5.0	6.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1722	0	1722	0	0	5-8	5-8
I	1721	0	1721	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1205	859 / 0	0 / 0	0 / 0	0 / 0	346 / 0	0 / 0
I	1205	859 / 0	0 / 0	0 / 0	0 / 0	346 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00	C- J	0 / 72	0.02 (4)
B- C	0 / 18	-112.4	-112.4	0.14 (1)	10.00	J- F	0 / 71	0.02 (4)
C- D	-1661 / 0	-112.4	-112.4	0.18 (1)	4.94	K- C	-1885 / 0	0.57 (1)
D- E	-1403 / 0	-112.4	-112.4	0.06 (1)	5.43	F- I	-1885 / 0	0.57 (1)
E- F	-1661 / 0	-112.4	-112.4	0.18 (1)	4.94	D- J	0 / 314	0.08 (1)
F- G	0 / 18	-112.4	-112.4	0.14 (1)	10.00	J- E	0 / 314	0.08 (1)
G- H	0 / 43	-112.4	-112.4	0.17 (1)	10.00			
K- B	-287 / 0	0.0	0.0	0.03 (1)	7.81			
I- G	-287 / 0	0.0	0.0	0.03 (1)	7.81			
K- L	0 / 1309	-18.5	-18.5	0.65 (1)	10.00			
L- M	0 / 1309	-18.5	-18.5	0.65 (1)	10.00			
M- N	0 / 1309	-18.5	-18.5	0.65 (1)	10.00			
N- J	0 / 1309	-18.5	-18.5	0.65 (1)	10.00			
J- O	0 / 1310	-18.5	-18.5	0.65 (1)	10.00			
O- P	0 / 1310	-18.5	-18.5	0.65 (1)	10.00			
P- Q	0 / 1310	-18.5	-18.5	0.65 (1)	10.00			
Q- I	0 / 1310	-18.5	-18.5	0.65 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-7	-275	-275	---	BACK	VERT	TOTAL	---	C1
E	7-1-8	-275	-275	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
M	3-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
N	5-11-3	-171	-171	---	BACK	VERT	TOTAL	---	C1
O	7-0-12	-171	-171	---	BACK	VERT	TOTAL	---	C1
P	9-0-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
Q	11-0-12	-29	-29	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.18/1.00 (E-F:1) , BC=0.65/1.00 (I-J:1) , WB=0.57/1.00 (F-I:1) , SSI=0.31/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

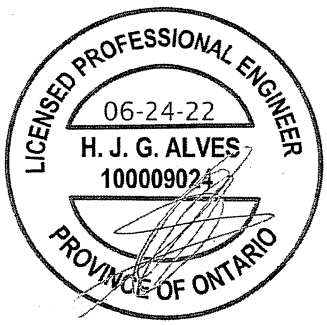
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)
JSI METAL= 0.42 (C) (INPUT = 1.00)



Structural component only
DWG# T-2215253

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T58S	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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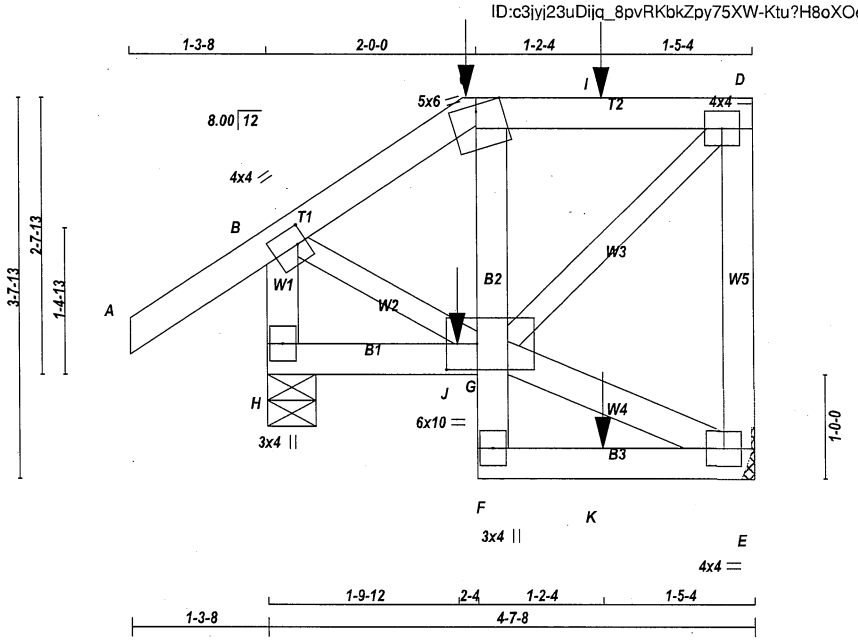

TOTAL WEIGHT = 2 X 28 = 55 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TTV-m	MT20	5.0	6.0	Edge	
D	TMVW-t	MT20	4.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-t	MT20	6.0	10.0	3.00	3.50
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	369	0	369	0	0	MECHANICAL	
H	498	0	498	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	259	182 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
H	347	258 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00	G- E	-8 / 0	0.00 (1)
B- C	-218 / 0	-112.4	-112.4	0.08 (1)	6.25	G- D	0 / 230	0.06 (1)
C- I	-167 / 0	-108.9	-108.9	0.22 (1)	6.25	B- G	0 / 204	0.05 (1)
I- D	-167 / 0	-108.9	-108.9	0.22 (1)	6.25			
E- D	-339 / 0	0.0	0.0	0.07 (1)	7.81			
H- B	-478 / 0	0.0	0.0	0.05 (1)	7.81			
H- J	0 / 0	-17.9	-17.9	0.03 (4)	10.00			
J- G	0 / 0	-17.9	-17.9	0.03 (4)	10.00			
F- G	0 / 33	0.0	0.0	0.02 (1)	10.00			
G- C	-201 / 0	0.0	0.0	0.01 (1)	7.81			
F- K	0 / 8	-17.9	-17.9	0.05 (4)	10.00			
K- E	0 / 8	-17.9	-17.9	0.05 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-0-0	-17	-17	---	FRONT	VERT	TOTAL	---	C1
I	3-2-4	-53	-53	---	FRONT	VERT	TOTAL	---	C1
J	1-9-12	-4	-4	---	FRONT	VERT	TOTAL	---	C1
K	3-2-4	-10	-10	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-0-0
RIGHT SETBACK = 0-0
END SETBACK = 2-0-0
END WALL WIDTH = 1-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.05/1.00 (E-F:4) , WB=0.06/1.00 (D-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215254

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T59S	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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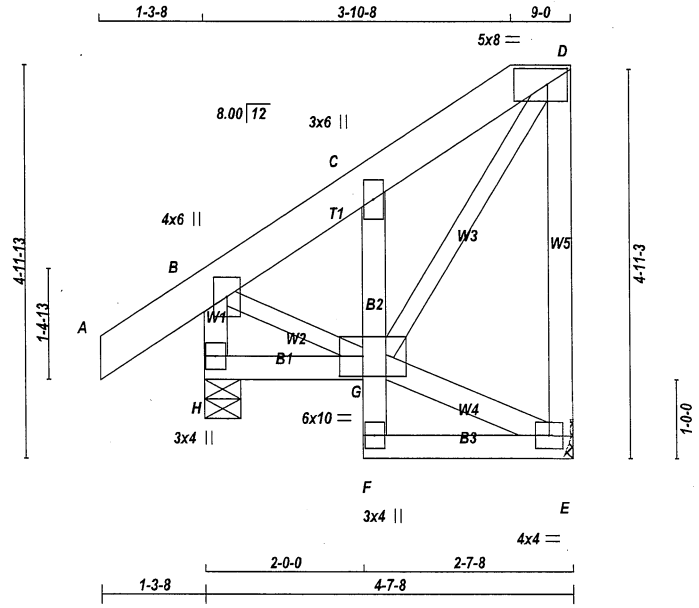

Scale = 1:27.9

TOTAL WEIGHT = 2 X 35 = 70 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	6.0		
D	TMVWm	MT20	5.0	8.0	2.50	3.00
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-I	MT20	6.0	10.0	3.00	3.50
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	303	0	303	0	0	MECHANICAL	
H	462	0	462	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	212	150 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
H	321	242 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 44	-112.4	-112.4	0.08 (1)	10.00	G- E	-16 / 0	0.00 (1)
B- C	-192 / 0	-112.4	-112.4	0.03 (1)	6.25	B- G	0 / 185	0.04 (1)
C- D	-185 / 0	-112.4	-112.4	0.05 (1)	6.25	G- D	0 / 282	0.06 (1)
E- D	-274 / 0	0.0	0.0	0.13 (1)	7.81			
H- B	-442 / 0	0.0	0.0	0.04 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
F- G	0 / 26	0.0	0.0	0.02 (1)	10.00			
G- C	-282 / 0	0.0	0.0	0.02 (1)	7.81			
F- E	0 / 15	-18.5	-18.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (D-E:1) , BC=0.03/1.00 (E-F:4) , WB=0.06/1.00 (D-G:1) , SSI=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215255

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T60	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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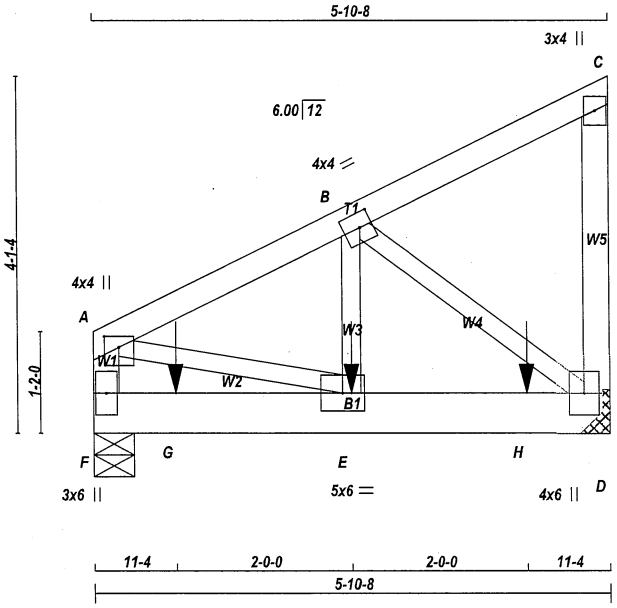

Scale = 1:25.4

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

Structural component only
DWG# T-2215256

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1106	0	1106	0	0	5-8	5-8
D	2052	0	2052	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	768	586 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
D	1432	1048 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-963 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 1125	0.14 (1)
A- B	-1201 / 0	-112.4	-112.4	0.08 (1)	6.25	E- B	0 / 979	0.12 (1)
B- C	-15 / 0	-112.4	-112.4	0.07 (1)	6.25	B- D	-1368 / 0	0.16 (1)
D- C	-133 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E- H	0 / 1087	-18.5	-18.5	0.44 (1)	10.00			
H- D	0 / 1087	-18.5	-18.5	0.44 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-253	-253	---	TOP	VERT	TOTAL	---	C1
G	11-4	-216	-216	---	TOP	VERT	TOTAL	---	C1
H	4-11-4	-1191	-1191	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.44/1.00 (D-E:1) , WB=0.16/1.00 (B-D:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (A) (INPUT = 0.90)
JSI METAL= 0.24 (D) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T60	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215256

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	T60Z	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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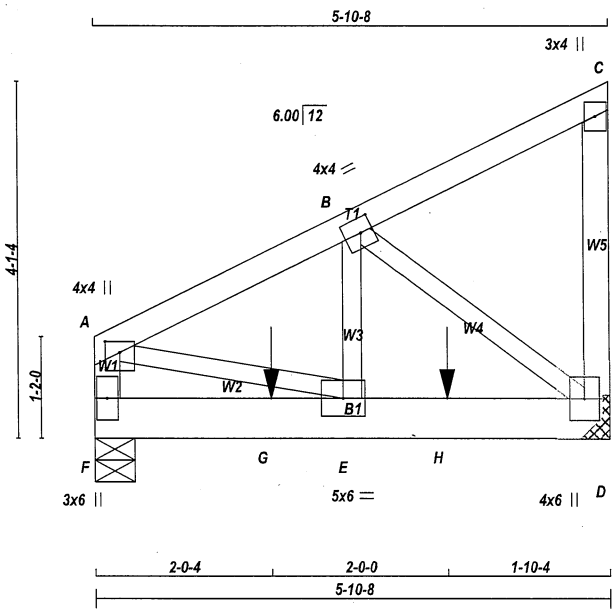

Scale = 1:25.4

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215257

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1296	0	1296	0	0	5-8	5-8
D	1349	0	1349	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	904	664 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0
D	941	692 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1095 / 0	0.0	0.0	0.06 (1)	7.81	A- E	0 / 1302	0.16 (1)
A- B	-1393 / 0	-112.4	-112.4	0.08 (1)	6.25	E- B	0 / 1198	0.15 (1)
B- C	-14 / 0	-112.4	-112.4	0.07 (1)	6.25	B- D	-1584 / 0	0.19 (1)
D- C	-134 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
E- H	0 / 1259	-18.5	-18.5	0.24 (1)	10.00			
H- D	0 / 1259	-18.5	-18.5	0.24 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-4	-653	-653	---	BACK	VERT	TOTAL	---	C1
H	4-0-4	-653	-653	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.24/1.00 (D-E:1) , WB=0.19/1.00 (B-D:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423570	T60Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215257

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423572	T60Z7	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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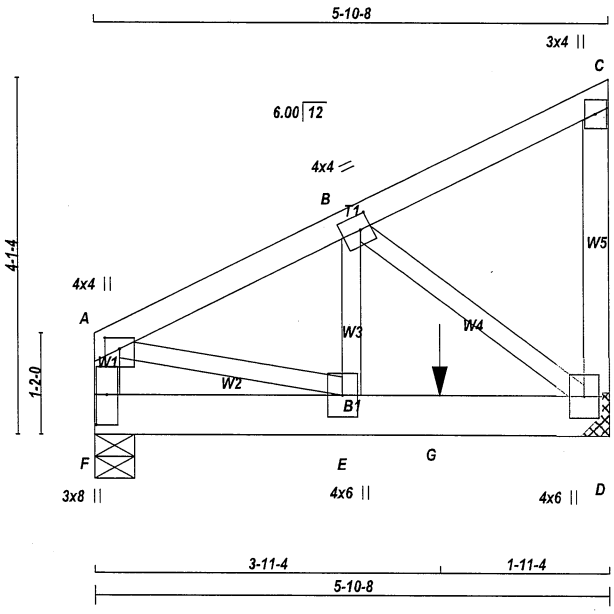

Scale = 1:25.4

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(58.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

Structural component only
DWG# T-2215260

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1329	0	1329	0	0	5-8	5-8
D	1841	0	1841	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	929	670 / 0	0 / 0	0 / 0	0 / 0	259 / 0	0 / 0
D	1286	935 / 0	0 / 0	0 / 0	0 / 0	350 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1274 / 0	0.0	0.0	0.07 (1)		7.81	A- E	0 / 1541	0.19 (1)
A- B	-1653 / 0	-112.4	-112.4	0.07 (1)		6.25	E- B	0 / 1495	0.18 (1)
B- C	-13 / 0	-112.4	-112.4	0.06 (1)		6.25	B- D	-1875 / 0	0.22 (1)
D- C	-136 / 0	0.0	0.0	0.02 (1)		7.81			
F- E	0 / 0	-134.4	-134.4	0.15 (1)		10.00			
E- G	0 / 1491	-134.4	-134.4	0.49 (1)		10.00			
G- D	0 / 1491	-18.5	-18.5	0.49 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	3-11-4	-1356	-1356		---	FRONT VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 3-11-4
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 4-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.49/1.00 (D-E:1) , WB=0.22/1.00 (B-D:1) , SSI=0.51/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.34 (D) (INPUT = 1.00)

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423572	T60Z7	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:c3jvj23uDijq_8pvRKbkZpy75XW-afxpmwWghAUTz4PNLsEbgpVx0WNdpJRMJxTH7Fz34Ov

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	8.0	4.25	1.50

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215260

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423572	T70	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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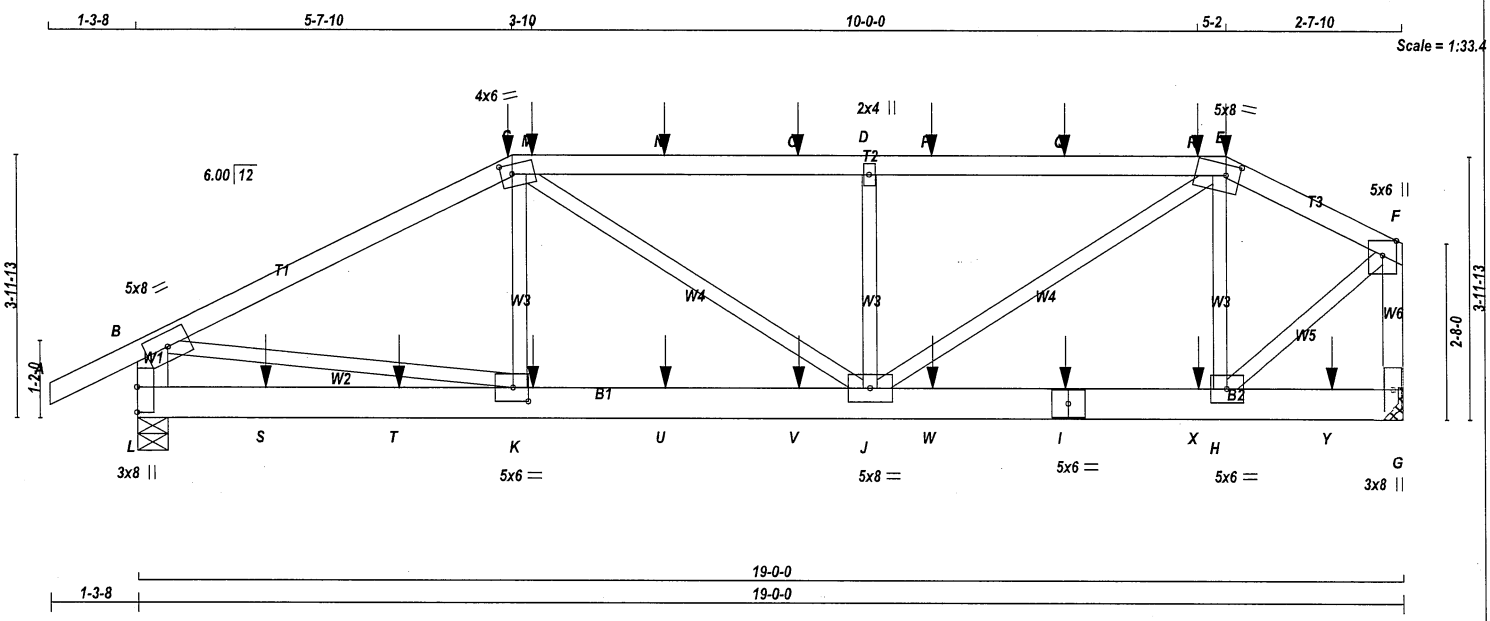


TOTAL WEIGHT = 87 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.50
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	8.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	8.0	4.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2215261

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	2015	0	2015	0	0	5-8	5-8
G	1963	0	1963	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1408	1021 / 0	0 / 0	0 / 0	0 / 0	387 / 0	0 / 0
G	1371	998 / 0	0 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.11 (1)	10.00
B- C	-2605 / 0	-112.4	-112.4	0.55 (1)	4.49
C- M	-2715 / 0	-112.4	-112.4	0.67 (1)	4.14
M- N	-2715 / 0	-112.4	-112.4	0.67 (1)	4.14
N- O	-2715 / 0	-112.4	-112.4	0.67 (1)	4.14
O- D	-2715 / 0	-112.4	-112.4	0.67 (1)	4.14
D- P	-2715 / 0	-112.4	-112.4	0.67 (1)	4.14
P- Q	-2715 / 0	-112.4	-112.4	0.67 (1)	4.14
Q- R	-2715 / 0	-112.4	-112.4	0.67 (1)	4.14
R- E	-2715 / 0	-112.4	-112.4	0.67 (1)	4.14
E- F	-1517 / 0	-112.4	-112.4	0.18 (1)	5.11
L- B	-1951 / 0	0.0	0.0	0.14 (1)	7.17
G- F	-1955 / 0	0.0	0.0	0.27 (1)	5.97
L- S	0 / 0	-18.5	-18.5	0.10 (4)	10.00
S- T	0 / 0	-18.5	-18.5	0.10 (4)	10.00
T- K	0 / 0	-18.5	-18.5	0.10 (4)	10.00
K- U	0 / 2325	-18.5	-18.5	0.37 (1)	10.00
U- V	0 / 2325	-18.5	-18.5	0.37 (1)	10.00
V- J	0 / 2325	-18.5	-18.5	0.37 (1)	10.00
J- W	0 / 1327	-18.5	-18.5	0.24 (1)	10.00
W- I	0 / 1327	-18.5	-18.5	0.24 (1)	10.00
I- X	0 / 1327	-18.5	-18.5	0.24 (1)	10.00
X- H	0 / 1327	-18.5	-18.5	0.24 (1)	10.00
H- Y	0 / 0	-18.5	-18.5	0.07 (4)	10.00
Y- G	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-142 / 103	0.04 (4)
J- D	-1090 / 0	0.27 (1)
J- E	0 / 1671	0.41 (1)
H- E	-964 / 0	0.23 (1)
B- K	0 / 2350	0.58 (1)
H- F	0 / 1761	0.44 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-7-10	-34	-34	---	FRONT	VERT	DEAD	---	C1
C	5-11-4	-99	-99	---	BACK	VERT	TOTAL	---	C1
C	5-7-10	-181	-181	---	FRONT	VERT	SNOW	---	C1
E	16-4-6	-109	-109	---	FRONT	VERT	SNOW	---	C1
I	13-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
K	5-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
N	7-11-4	-72	-72	---	BACK	VERT	TOTAL	---	C1
O	9-11-4	-72	-72	---	BACK	VERT	TOTAL	---	C1
P	11-11-4	-72	-72	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.67/1.00 (C-D:1) , BC=0.37/1.00 (J-K:1) , WB=0.58/1.00 (B-K:1) , SSI=0.48/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.66 (C) (INPUT = 1.00)

REVIEWED

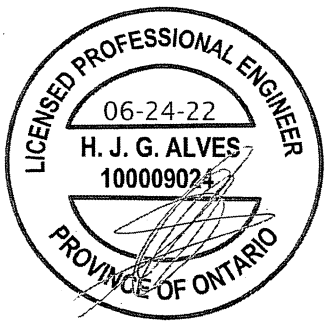
CONTINUED ON PAGE 2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	13-11-4	-72	-72	---	BACK	VERT	TOTAL	---	C1
R	15-11-4	-94	-94	---	BACK	VERT	TOTAL	---	C1
S	1-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
T	3-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
U	7-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
V	9-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
W	11-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
X	15-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
Y	17-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2215261

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423566	V1	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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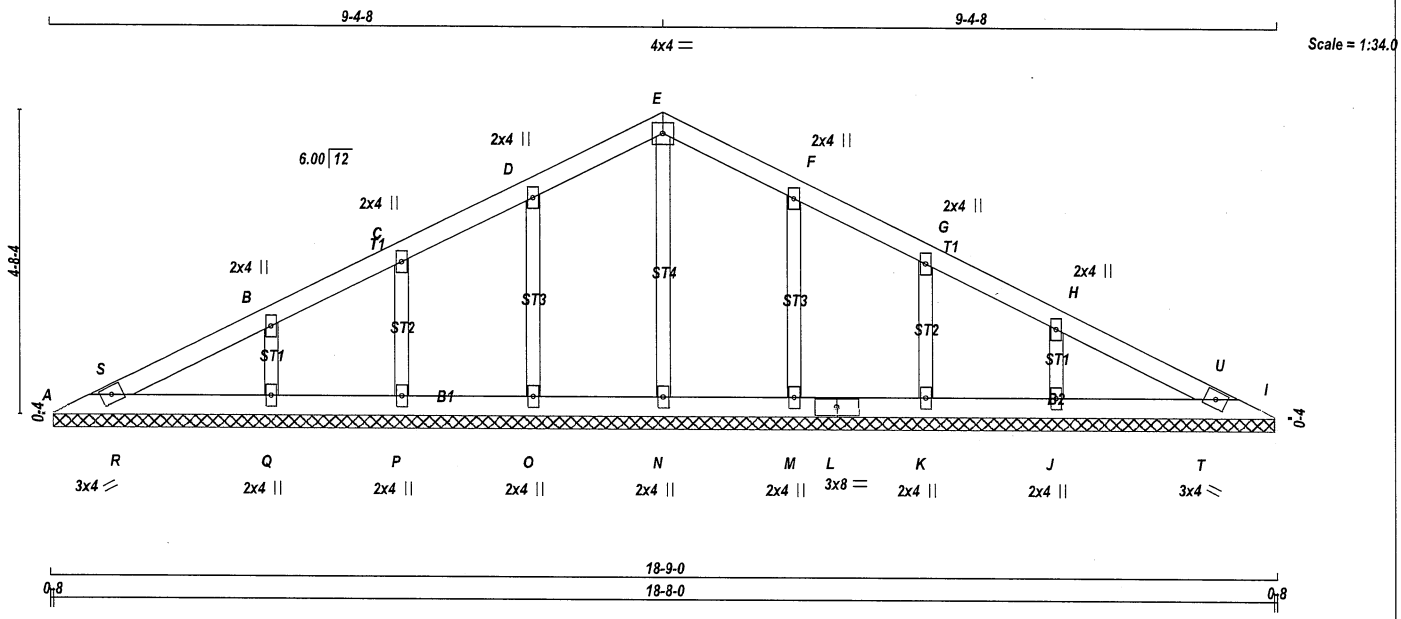

TOTAL WEIGHT = 57 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
A - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TBM1-h	MT20	3.0	4.0	
B, C, D, F, G, H					
B	TMW+w	MT20	2.0	4.0	
E	TTW-p	MT20	4.0	4.0	
I	TBM1-h	MT20	3.0	4.0	
J, K, M, N, O, P, Q					
J	BMW1+w	MT20	2.0	4.0	
L	BS-t	MT20	3.0	8.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	159	0	159	0	0	18-8-0 (6-8-4)	18-8-0
I	159	0	159	0	0	18-8-0 (6-8-4)	18-8-0
N	283	0	283	0	0	18-8-0 (6-8-4)	18-8-0
O	310	0	310	0	0	18-8-0 (6-8-4)	18-8-0
P	186	0	186	0	0	18-8-0 (6-8-4)	18-8-0
Q	425	0	425	0	0	18-8-0 (6-8-4)	18-8-0
M	310	0	310	0	0	18-8-0 (6-8-4)	18-8-0
K	186	0	186	0	0	18-8-0 (6-8-4)	18-8-0
J	425	0	425	0	0	18-8-0 (6-8-4)	18-8-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	111	80 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
I	111	80 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
N	199	136 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
O	217	156 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
P	131	92 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
Q	298	210 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
M	217	156 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
K	131	92 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
J	298	210 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I, N, O, P, Q, M, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- S	0 / 34	-112.4	-112.4	0.05 (1)	10.00	N- E	-250 / 0		0.08 (1)
S- B	0 / 70	-112.4	-112.4	0.12 (1)	10.00	O- D	-264 / 0		0.06 (1)
B- C	0 / 52	-112.4	-112.4	0.12 (1)	10.00	P- C	-182 / 0		0.03 (1)
C- D	0 / 76	-112.4	-112.4	0.08 (1)	10.00	Q- B	-309 / 0		0.04 (1)
D- E	0 / 65	-112.4	-112.4	0.08 (1)	10.00	M- F	-264 / 0		0.06 (1)
E- F	0 / 65	-112.4	-112.4	0.08 (1)	10.00	K- G	-182 / 0		0.03 (1)
F- G	0 / 76	-112.4	-112.4	0.08 (1)	10.00	J- H	-309 / 0		0.04 (1)
G- H	0 / 52	-112.4	-112.4	0.12 (1)	10.00	R- S	-95 / 3		0.00 (1)
H- U	0 / 70	-112.4	-112.4	0.12 (1)	10.00	T- U	-95 / 3		0.00 (1)
U- I	0 / 34	-112.4	-112.4	0.05 (1)	10.00				
A- R	-53 / 0	-18.5	-18.5	0.10 (1)	6.25				
R- Q	-43 / 0	-18.5	-18.5	0.10 (1)	6.25				
Q- P	-58 / 0	-18.5	-18.5	0.06 (1)	6.25				
P- O	-63 / 0	-18.5	-18.5	0.02 (4)	6.25				
O- N	-68 / 0	-18.5	-18.5	0.02 (4)	6.25				
N- M	-68 / 0	-18.5	-18.5	0.02 (4)	6.25				
M- L	-63 / 0	-18.5	-18.5	0.02 (4)	6.25				
L- K	-63 / 0	-18.5	-18.5	0.02 (4)	6.25				
K- J	-58 / 0	-18.5	-18.5	0.06 (1)	6.25				
J- T	-43 / 0	-18.5	-18.5	0.10 (1)	6.25				
T- I	-53 / 0	-18.5	-18.5	0.10 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (B-S:1) , BC=0.10/1.00 (Q-R:1) , WB=0.08/1.00 (E-N:1) , SSI=0.12/1.00 (B-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (E) (INPUT = 0.90)
JSI METAL= 0.13 (H) (INPUT = 1.00)

Structural component only
DWG# T-2215219

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423566	V2	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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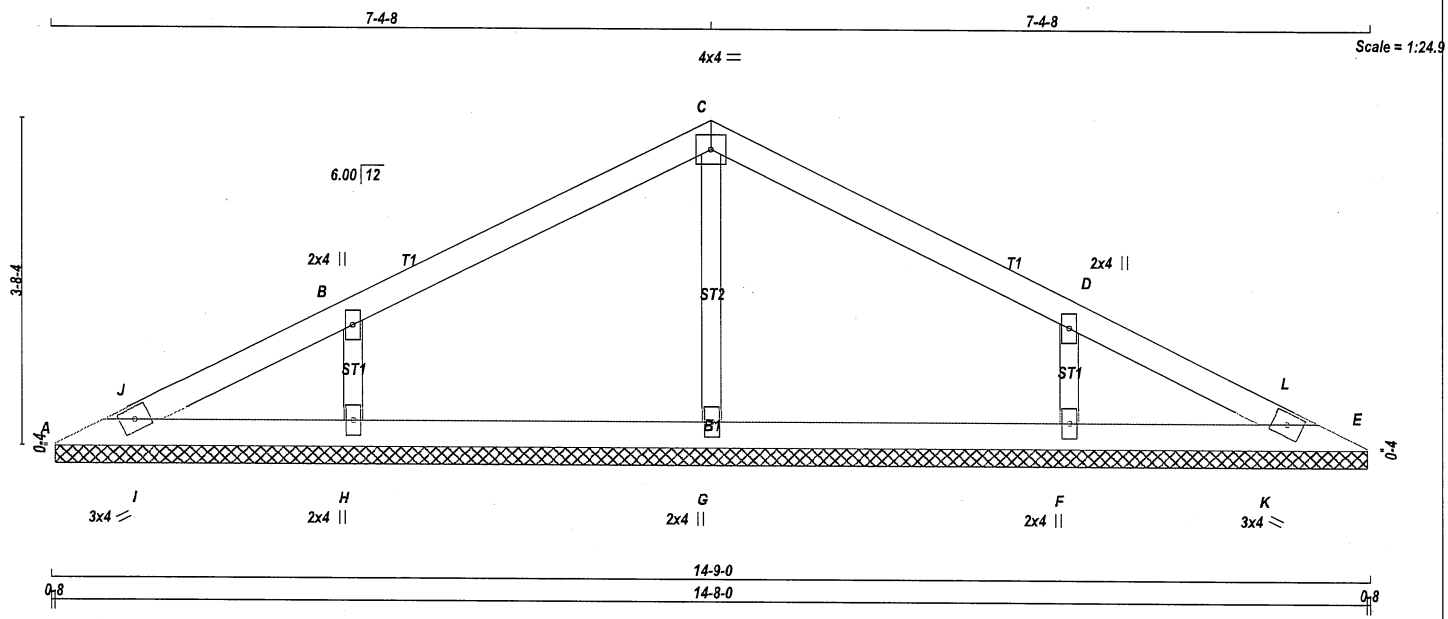

TOTAL WEIGHT = 38 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TBM1-h	MT20	3.0	4.0	
B	TMW+w	MT20	2.0	4.0	
C	TTW-p	MT20	4.0	4.0	
D	TMW+w	MT20	2.0	4.0	
E	TBM1-h	MT20	3.0	4.0	
F, G, H					
F	BMW1+w	MT20	2.0	4.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	148	0	148	0	0	14-8-0	14-8-0
E	148	0	148	0	0	14-8-0	14-8-0
G	454	0	454	0	0	14-8-0	14-8-0
H	586	0	586	0	0	14-8-0	14-8-0
F	586	0	586	0	0	14-8-0	14-8-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	103	74 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
E	103	74 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
G	321	212 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	409	296 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
F	409	296 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- J	-16 / 6	-112.4	-112.4	0.06	(1)	6.25	G- C	-388 / 0	0.08 (1)
J- B	0 / 45	-112.4	-112.4	0.26	(1)	10.00	H- B	-492 / 0	0.07 (1)
B- C	0 / 12	-112.4	-112.4	0.26	(1)	10.00	F- D	-492 / 0	0.07 (1)
C- D	0 / 12	-112.4	-112.4	0.26	(1)	10.00	I- J	-39 / 6	0.00 (1)
D- L	0 / 45	-112.4	-112.4	0.26	(1)	10.00	K- L	-39 / 6	0.00 (1)
L- E	-16 / 6	-112.4	-112.4	0.06	(1)	6.25			
A- I	-12 / 0	-18.5	-18.5	0.05	(1)	6.25			
I- H	-10 / 1	-18.5	-18.5	0.06	(4)	6.25			
H- G	-23 / 0	-18.5	-18.5	0.06	(4)	6.25			
G- F	-23 / 0	-18.5	-18.5	0.06	(4)	6.25			
F- K	-10 / 1	-18.5	-18.5	0.06	(4)	6.25			
K- E	-12 / 0	-18.5	-18.5	0.05	(1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (B-J:1) , BC=0.06/1.00 (G-H:4) , WB=0.08/1.00 (C-G:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

Structural component only
DWG# T-2215220

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423566	V3	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

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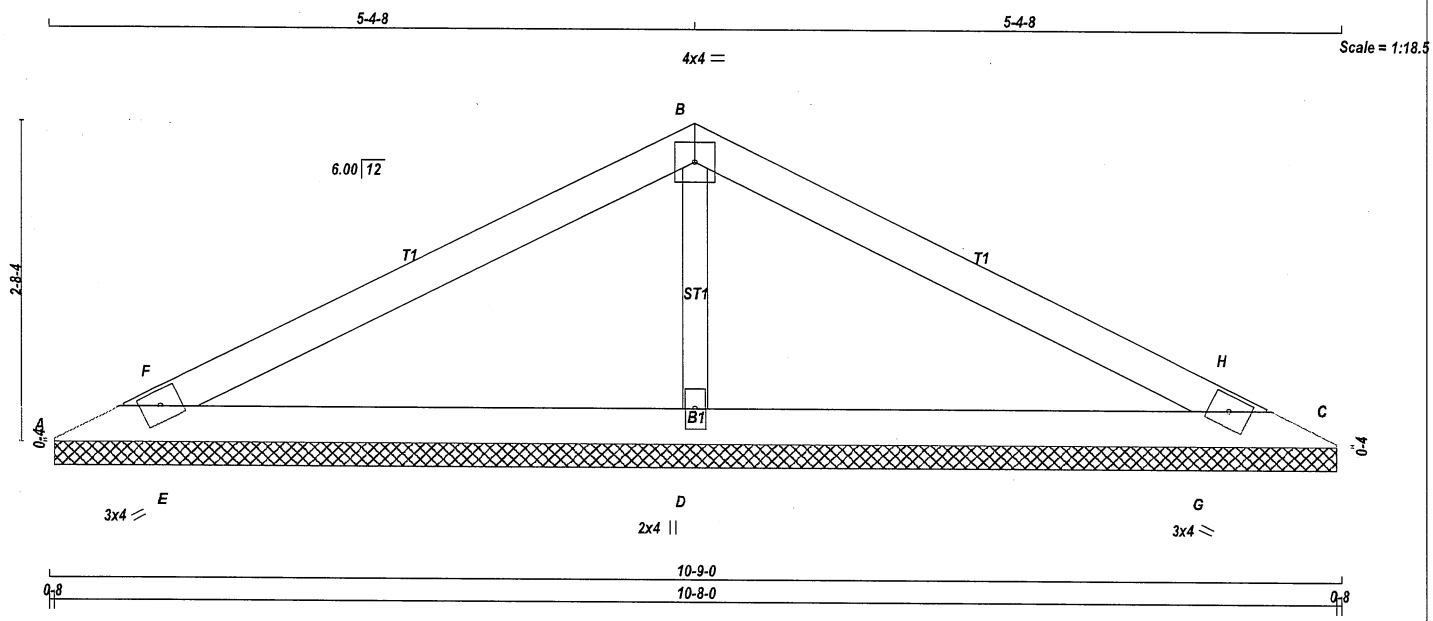

TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0		
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	52	0	52	0	0	10-8-0	10-8-0
C	52	0	52	0	0	10-8-0	10-8-0
D	1292	0	1292	0	0	10-8-0	10-8-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	37	24 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
C	37	24 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	905	644 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- F	0 / 654	-112.4	-112.4	0.21 (1)	10.00	D- B	-1040 / 0	0.17 (1)
F- B	0 / 653	-112.4	-112.4	0.41 (1)	10.00	E- F	-308 / 0	0.00 (1)
B- H	0 / 653	-112.4	-112.4	0.41 (1)	10.00	G- H	-308 / 0	0.00 (1)
H- C	0 / 654	-112.4	-112.4	0.21 (1)	10.00			
A- E	-624 / 0	-18.5	-18.5	0.27 (1)	6.25			
E- D	-591 / 0	-18.5	-18.5	0.27 (1)	6.25			
D- G	-591 / 0	-18.5	-18.5	0.27 (1)	6.25			
G- C	-624 / 0	-18.5	-18.5	0.27 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.41/1.00 (B-F:1) , BC=0.27/1.00 (D-E:1) , WB=0.17/1.00 (B-D:1) , SSI=0.18/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)

Structural component only.
DWG# T-2215221

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423569	V30	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

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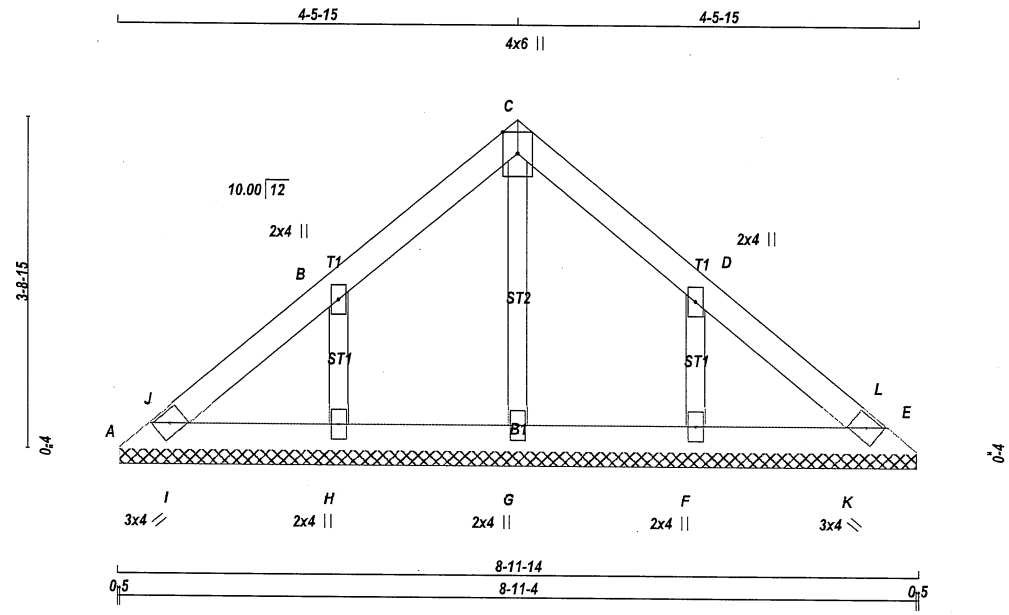

TOTAL WEIGHT = 27 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	123	0	123	0	0	8-11-4	8-11-4
E	123	0	123	0	0	8-11-4	8-11-4
G	213	0	213	0	0	8-11-4	8-11-4
H	356	0	356	0	0	8-11-4	8-11-4
F	356	0	356	0	0	8-11-4	8-11-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	86	63 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
E	86	63 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
G	151	98 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
H	249	178 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
F	249	178 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- J	-13 / 2	-112.4	-112.4	0.02 (1)	6.25	G- C	-203 / 0	0.04 (1)
J- B	0 / 27	-112.4	-112.4	0.09 (1)	10.00	H- B	-274 / 0	0.04 (1)
B- C	0 / 8	-112.4	-112.4	0.08 (1)	10.00	F- D	-274 / 0	0.04 (1)
C- D	0 / 8	-112.4	-112.4	0.08 (1)	10.00	I- J	-66 / 2	0.00 (1)
D- L	0 / 27	-112.4	-112.4	0.09 (1)	10.00	K- L	-66 / 2	0.00 (1)
L- E	-13 / 2	-112.4	-112.4	0.02 (1)	6.25			
A- I	-11 / 0	-18.5	-18.5	0.05 (1)	6.25			
I- H	-6 / 0	-18.5	-18.5	0.05 (1)	10.00			
H- G	-15 / 0	-18.5	-18.5	0.03 (1)	6.25			
G- F	-15 / 0	-18.5	-18.5	0.03 (1)	6.25			
F- K	-6 / 0	-18.5	-18.5	0.05 (1)	10.00			
K- E	-11 / 0	-18.5	-18.5	0.05 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (D-L:1) , BC=0.05/1.00 (E-K:1) , WB=0.04/1.00 (C-G:1) , SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215229

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423569	V31	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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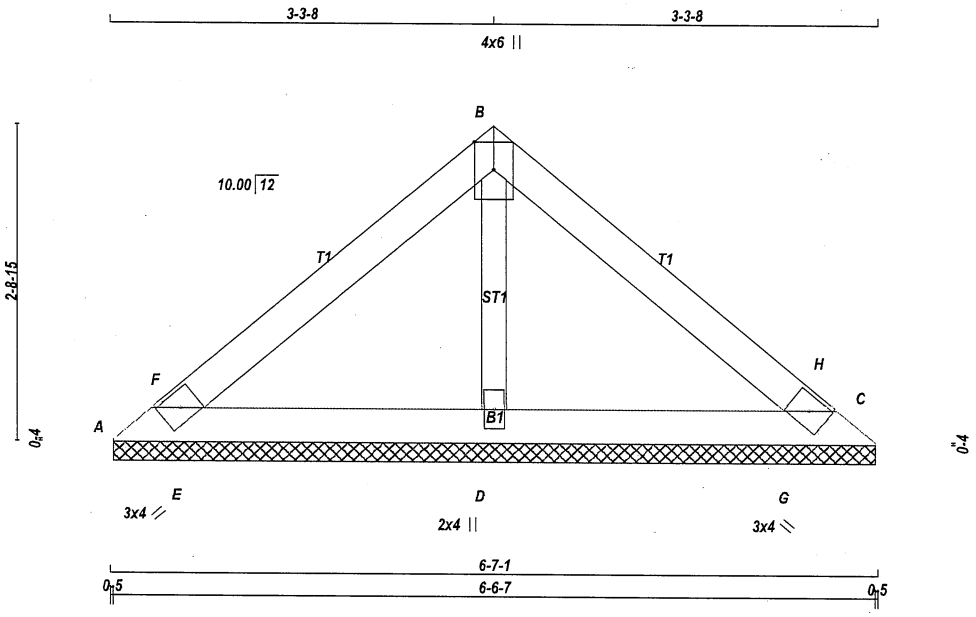

TOTAL WEIGHT = 18 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	4.0	6.0	Edge	
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	91	0	91	0	0	6-6-7	6-6-7
C	91	0	91	0	0	6-6-7	6-6-7
D	674	0	674	0	0	6-6-7	6-6-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	63	46 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
C	63	46 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	473	332 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- F	0 / 175	-112.4	-112.4	0.06 (1)		10.00	D- B	-497 / 0	0.08 (1)
F- B	0 / 169	-112.4	-112.4	0.14 (1)		10.00	E- F	-198 / 0	0.00 (1)
B- H	0 / 169	-112.4	-112.4	0.14 (1)		10.00	G- H	-198 / 0	0.00 (1)
H- C	0 / 175	-112.4	-112.4	0.06 (1)		10.00			
A- E	-166 / 0	-18.5	-18.5	0.12 (1)		6.25			
E- D	-135 / 0	-18.5	-18.5	0.12 (1)		6.25			
D- G	-135 / 0	-18.5	-18.5	0.12 (1)		6.25			
G- C	-166 / 0	-18.5	-18.5	0.12 (1)		6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (B-H:1) , BC=0.12/1.00 (D-G:1) , WB=0.08/1.00 (B-D:1) , SSI=0.11/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

Structural component only
DWG# T-2215230

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423569	V32	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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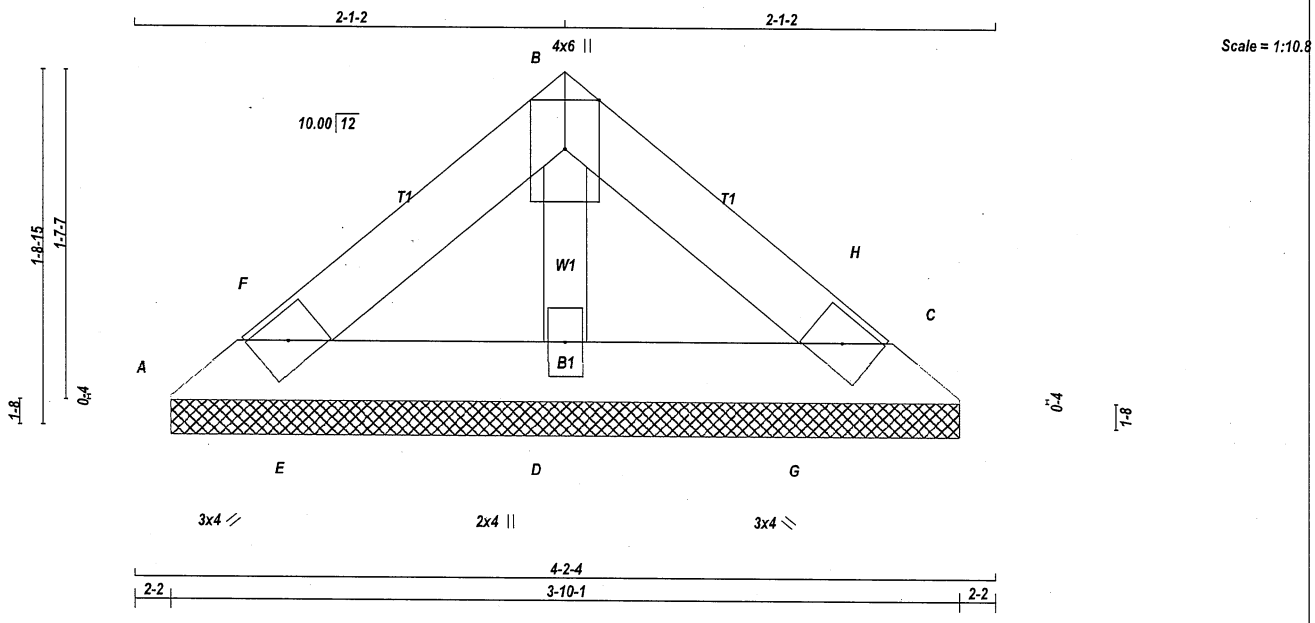


Scale = 1:10.8

TOTAL WEIGHT = 10 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	4.0	6.0	Edge	
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	86	0	86	0	0	3-10-1	3-10-1
C	86	0	86	0	0	3-10-1	3-10-1
D	330	0	330	0	0	3-10-1	3-10-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	60	44 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
C	60	44 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	232	162 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- F	0 / 36	-112.4	-112.4	0.02 (1)	10.00	D- B	-197 / 0		0.03 (1)
F- B	0 / 43	-112.4	-112.4	0.04 (1)	10.00	E- F	-96 / 0		0.00 (1)
B- H	0 / 43	-112.4	-112.4	0.04 (1)	10.00	G- H	-96 / 0		0.00 (1)
H- C	0 / 36	-112.4	-112.4	0.02 (1)	10.00				
A- E	-44 / 0	-18.5	-18.5	0.05 (1)	6.25				
E- D	-32 / 0	-18.5	-18.5	0.05 (1)	6.25				
D- G	-32 / 0	-18.5	-18.5	0.05 (1)	6.25				
G- C	-44 / 0	-18.5	-18.5	0.05 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (B-H:1) , BC=0.05/1.00 (D-E:1) , WB=0.03/1.00 (B-D:1) , SSI=0.05/1.00 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

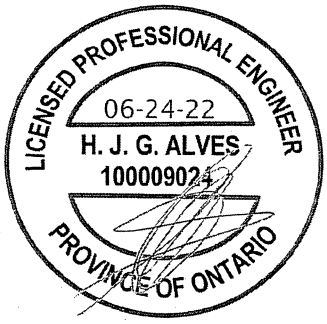
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (D) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)



Structural component only
DWG# T-2215231

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J1	9	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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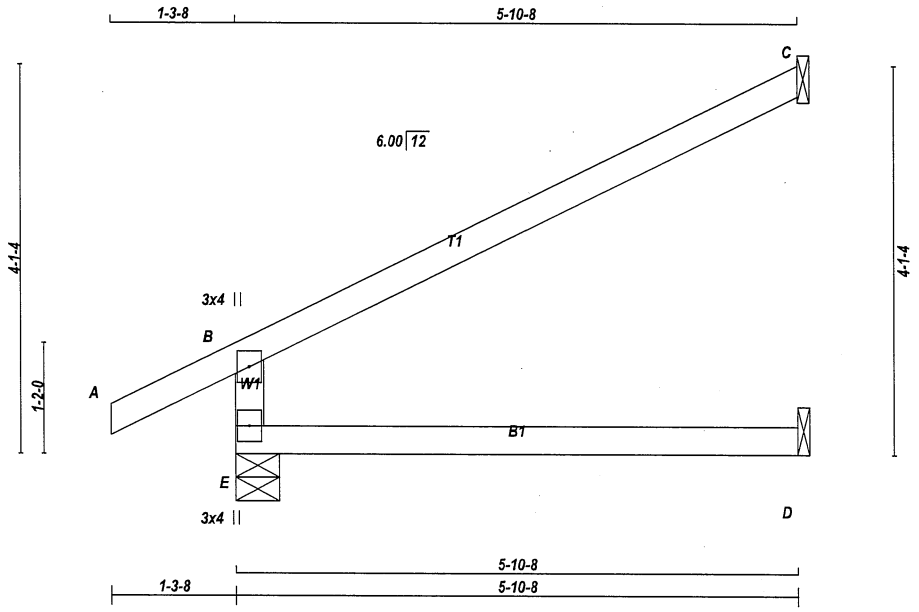

TOTAL WEIGHT = 9 X 17 = 151 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	629	0	629	0	0	5-8	5-8
C	248	0	248	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	438	327 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	170	143 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-565 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00			
B- C	-37 / 0	-112.4	-112.4	0.66 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (E) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215173

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J1S	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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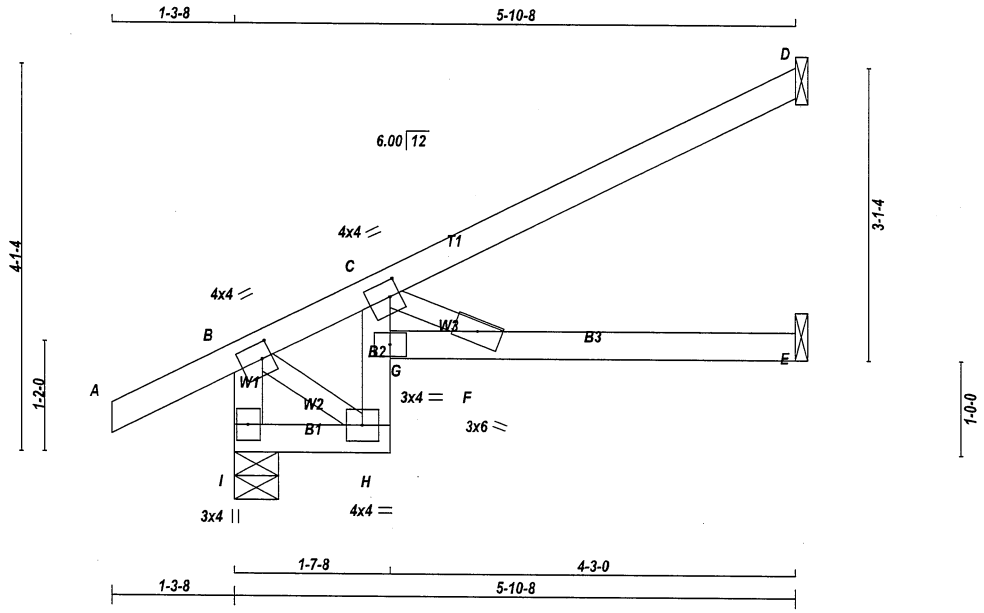

TOTAL WEIGHT = 6 X 20 = 122 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	6.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	555	0	555	0	0	5-8	5-8
D	248	0	248	0	0	1-8	1-8
E	119	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	386	289 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	171	138 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
E	86	42 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-541 / 0	0.0	0.0	0.05 (1)	7.81	B- H	0 / 248	0.06 (1)
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	C- F	-657 / 0	0.10 (1)
B- C	-273 / 0	-112.4	-112.4	0.14 (1)	6.25			
C- D	0 / 4	-112.4	-112.4	0.37 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
H- G	-111 / 0	0.0	0.0	0.29 (1)	7.81			
G- C	0 / 173	0.0	0.0	0.32 (1)	10.00			
G- F	0 / 574	-18.5	-18.5	0.49 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.40 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 853 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 451 (0.16")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.49/1.00 (F-G:1) , WB=0.10/1.00 (C-F:1) , SSI=0.28/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (C) (INPUT = 0.90)
JSI METAL= 0.21 (G) (INPUT = 1.00)

Structural component only
DWG# T-2215174

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J2	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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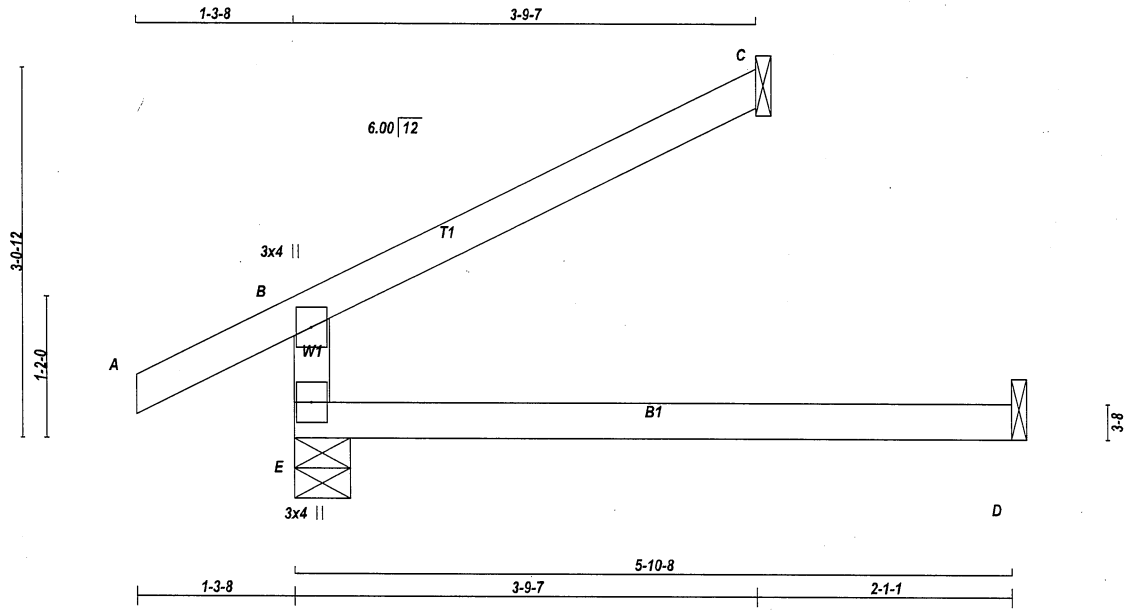

TOTAL WEIGHT = 14 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	482	0	482	0	0	5-8	5-8
C	160	0	160	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	337	242 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	109	92 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-418 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00			
B- C	-24 / 0	-112.4	-112.4	0.27 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215175

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J3	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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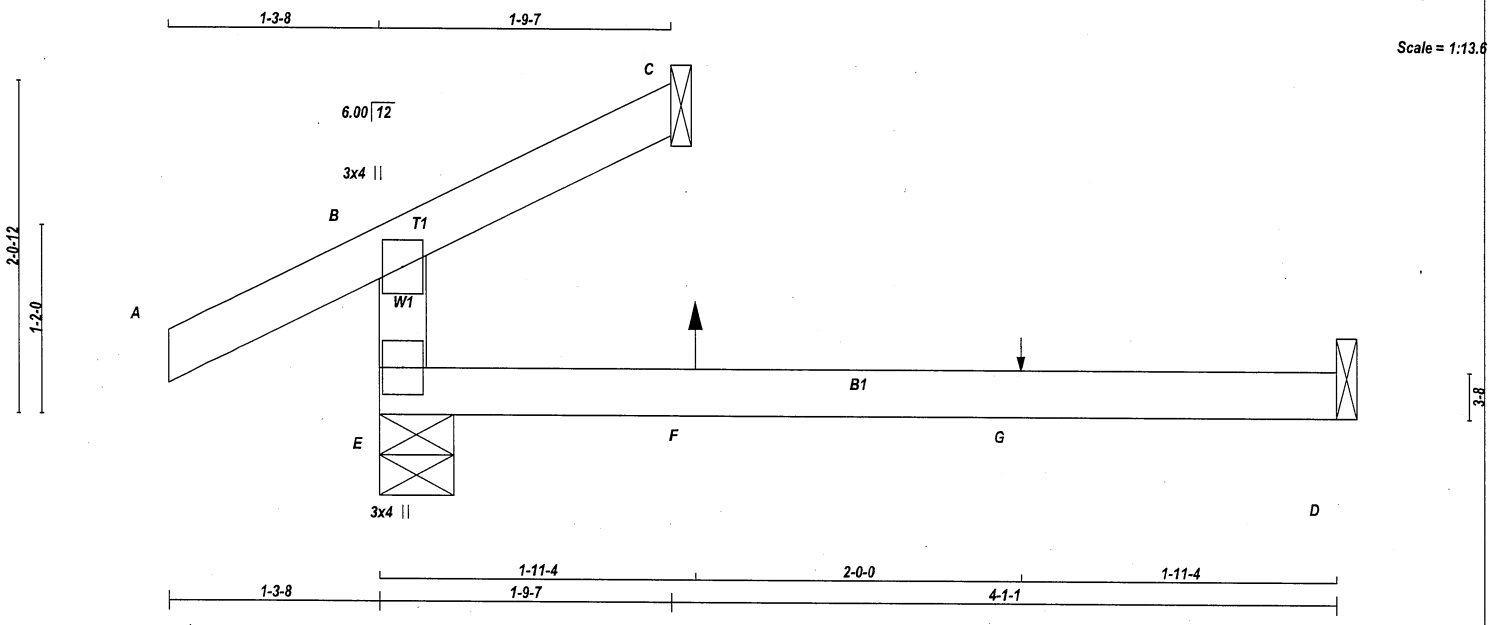


TOTAL WEIGHT = 12 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	340	0	340	0	0	5-8	5-8
C	71	0	71	0	0	1-8	1-8
D	43	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	237	175 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	52	26 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	MAX CSI (LC)
E- B	-284 / 0	0.0	0.0	0.11 (4)			7.81	
A- B	0 / 34	-112.4	-112.4	0.15 (1)			10.00	
B- C	-14 / 9	-112.4	-112.4	0.08 (4)			6.25	
E- F	0 / 0	-18.5	-18.5	0.14 (4)			10.00	
F- G	0 / 0	-18.5	-18.5	0.14 (4)			10.00	
G- D	0 / 0	-18.5	-18.5	0.14 (4)			10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	6	1	10	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

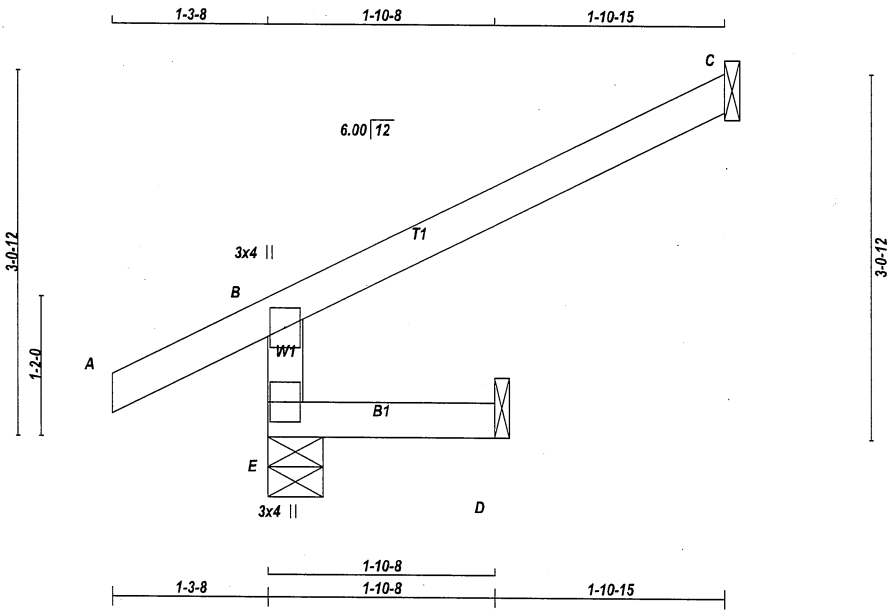
LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215176

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J4	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 10 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	438	0	438	0	0	5-8	5-8
C	160	0	160	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	302	242 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	109	92 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-418 / 0	0.0	0.0	0.01 (4)		7.81			
A- B	0 / 34	-112.4	-112.4	0.15 (1)		10.00			
B- C	-24 / 0	-112.4	-112.4	0.27 (1)		6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215177

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J5	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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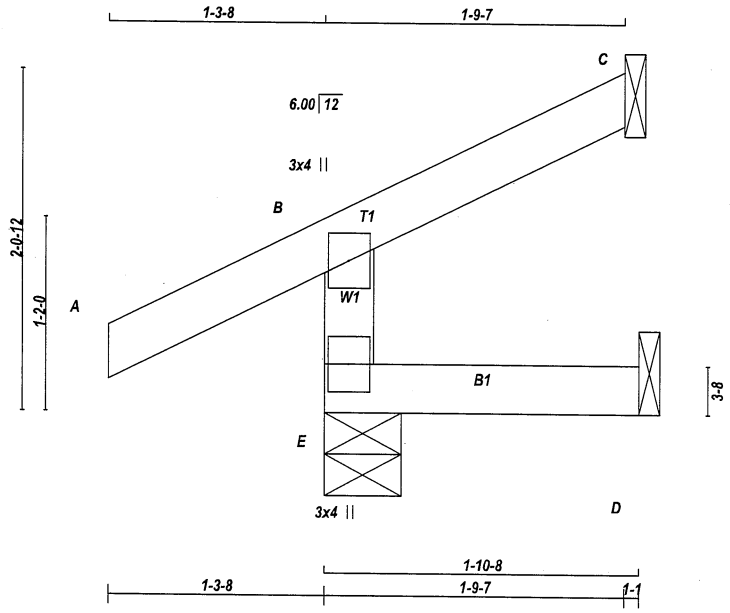

TOTAL WEIGHT = 7 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	328	0	328	0	0	5-8	5-8
C	54	0	54	0	-30	1-8	1-8
D	6	0	17	0	-5	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	226	179 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	37	31 / -23	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	6	0 / -10	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-299 / 0	0.0	0.0	0.05 (5)	7.81			
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00			
B- C	-21 / 0	-112.4	-112.4	0.11 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215178

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J6	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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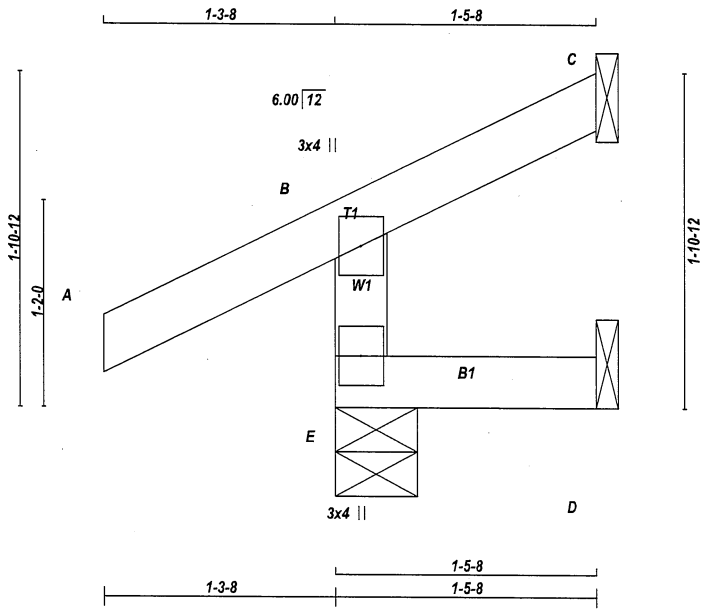

TOTAL WEIGHT = 3 X 6 = 18 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	319	0	319	0	0	5-8	5-8
C	27	0	27	0	-42	1-8	1-8
D	-2	0	12	0	-11	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	220	176 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	19	15 / -29	0 / 0	0 / 0	0 / 0	3 / 0	0 / 0
D	-0	0 / -12	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-290 / 0	0.0	0.0	0.04 (5)	7.81			
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00			
B- C	-25 / 0	-112.4	-112.4	0.11 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215179

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J8W	7	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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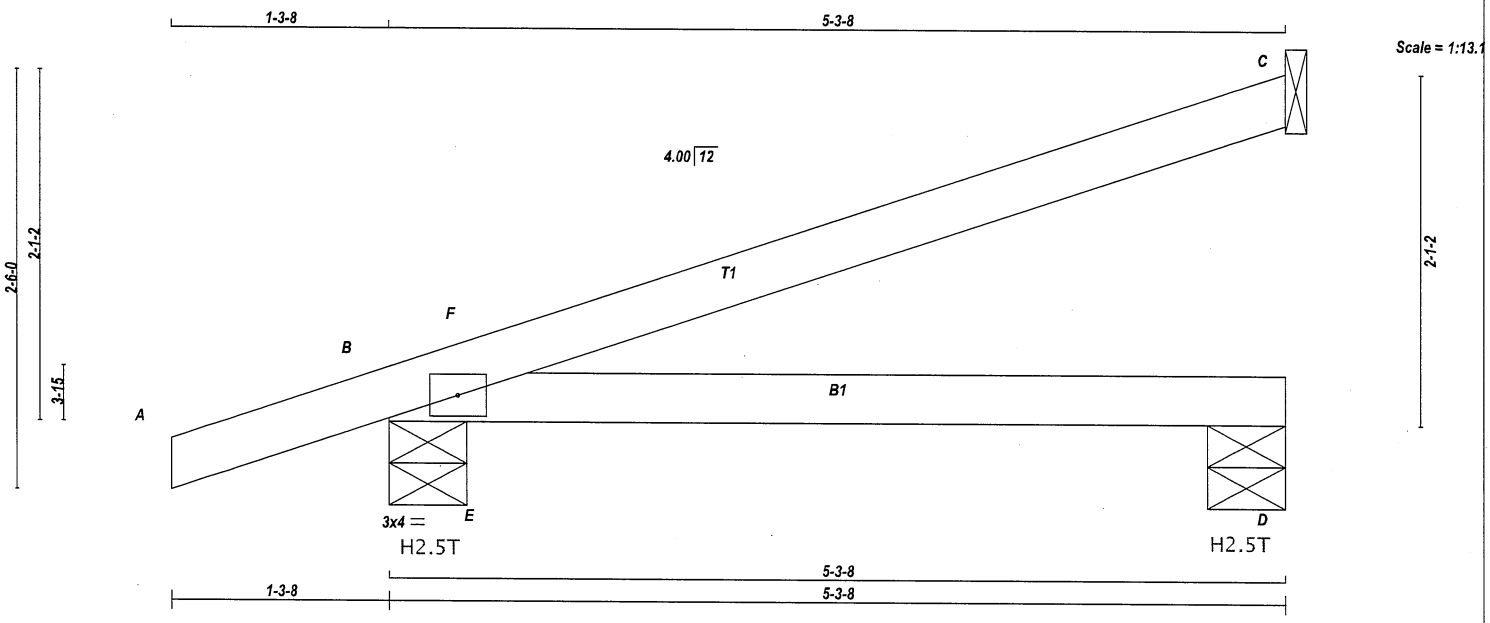

TOTAL WEIGHT = 7 X 14 = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	257	0	257	0	-108	1-8	1-8
B	497	0	497	104	-188	5-8	5-8
D	89	0	89	0	-74	5-8	5-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 188 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 104 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	177	144 / 0	0 / 0	0 / 0	0 / -98	33 / 0	0 / 0
B	346	259 / 0	0 / 0	0 / 0	0 / -190	87 / 0	0 / 0
D	66	27 / 0	0 / 0	0 / 0	0 / -77	38 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	CSI (LC)
A- B	0 / 22	-112.4	-112.4	0.14 (1)	10.00	E- F	-361 / 86		0.00 (1)
B- F	-75 / 17	-112.4	-112.4	0.06 (12)	6.25				
F- C	-42 / 2	-112.4	-112.4	0.40 (1)	6.25				
B- E	0 / 0	-18.5	-18.5	0.29 (1)	10.00				
E- D	0 / 0	-18.5	-18.5	0.29 (1)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 564 (0.11")

CSI: TC=0.40/1.00 (C-F:1) , BC=0.29/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.28/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215180

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423564	J10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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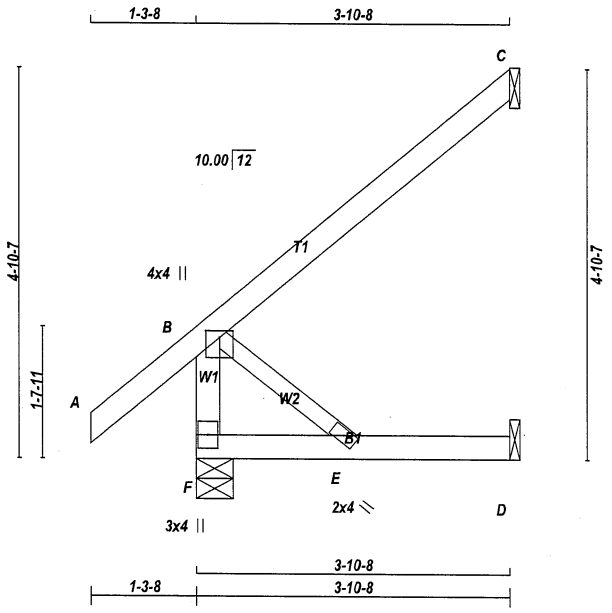


Scale = 1:27.5

TOTAL WEIGHT = 2 X 15 = 31 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	409	0	409	0	0	5-8	5-8
C	218	0	218	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	284	216 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
C	149	126 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-374 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 50	-112.4	-112.4	0.16 (5)	10.00			
B- C	0 / 0	-112.4	-112.4	0.29 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

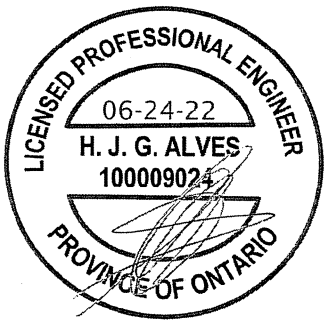
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215181

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J11	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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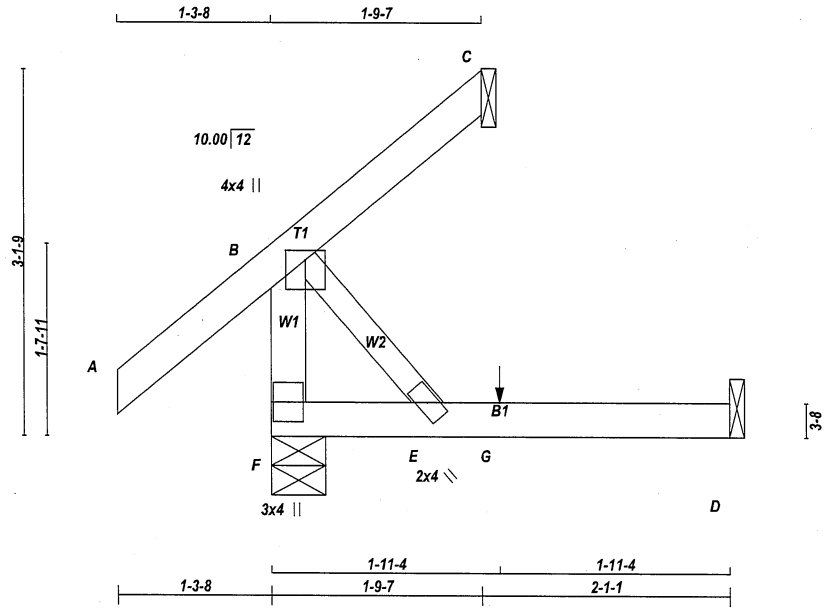


Scale = 1:18.8

TOTAL WEIGHT = 12 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	352	0	352	0	0	5-8	5-8
C	40	0	40	0	-51	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	245	183 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	27	23 / -35	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-317 / 0	0.0	0.0	0.03	(1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 50	-112.4	-112.4	0.17	(5)	10.00			
B- C	-39 / 0	-112.4	-112.4	0.16	(5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.07	(4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.08	(4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.08	(4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.17/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

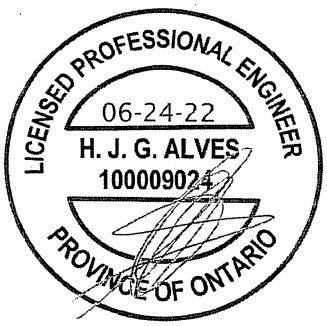
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215182

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J12	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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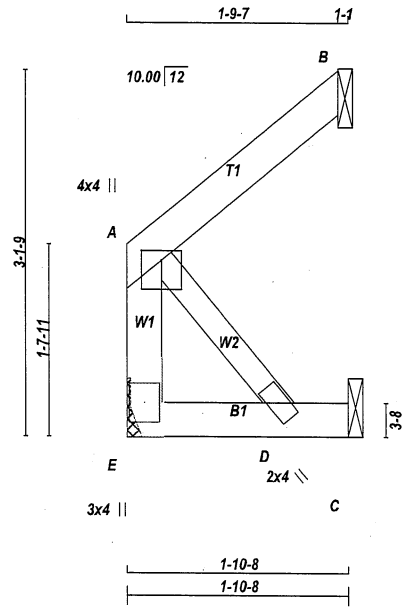

Scale = 1:18.8

TOTAL WEIGHT = 8 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	118	0	118	0	0	MECHANICAL	
B	100	0	100	0	0	1-8	1-8
C	17	0	19	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	83	58 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	69	58 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-100 / 0	0.0	0.0	0.01 (1)		7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-112.4	-112.4	0.06 (1)		10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)		10.00			
D- C	0 / 0	-18.5	-18.5	0.02 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215183

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J13	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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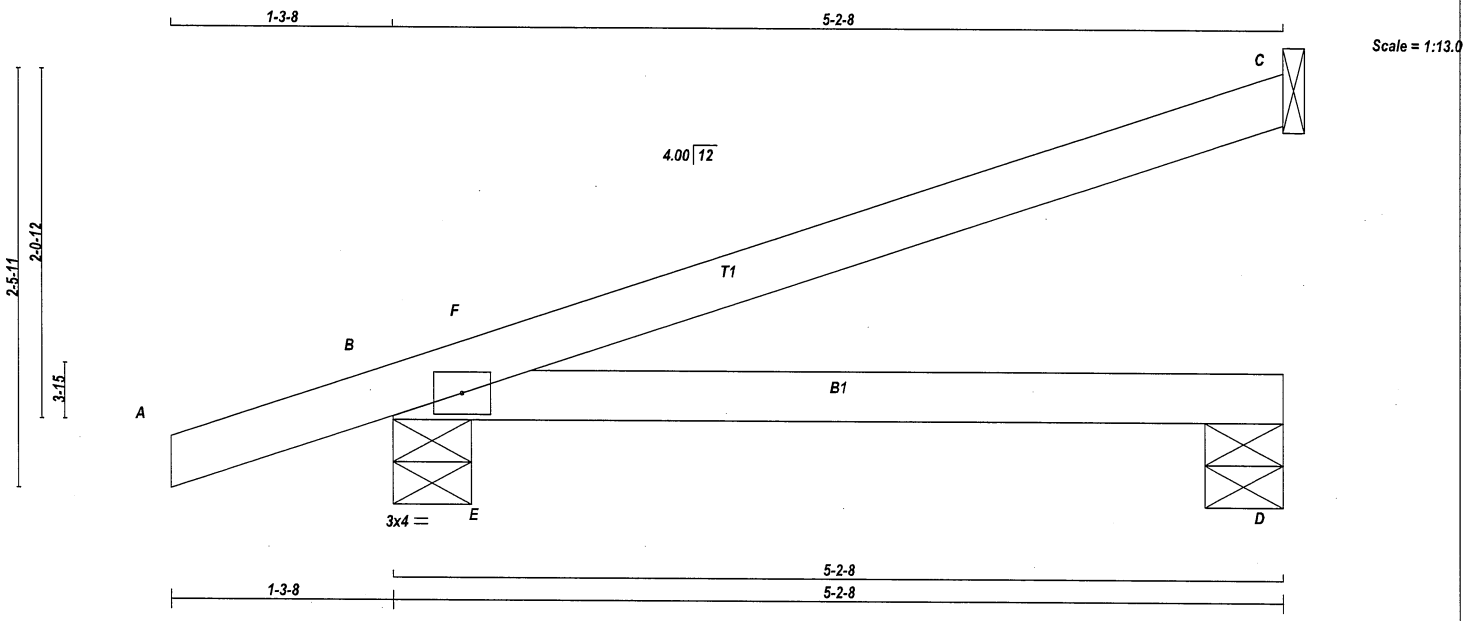

TOTAL WEIGHT = 6 X 14 = 83 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	253	0	253	0	0	1-8	1-8
B	491	0	491	0	0	5-8	5-8
D	88	0	88	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	174	142 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	342	256 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
D	65	27 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 22	-112.4	-112.4	0.14 (1)	10.00	E- F	-338 / 10	0.00 (1)	
B- F	-17 / 14	-112.4	-112.4	0.06 (4)	6.25				
F- C	-2 / 2	-112.4	-112.4	0.39 (1)	10.00				
B- E	0 / 0	-18.5	-18.5	0.28 (1)	10.00				
E- D	0 / 0	-18.5	-18.5	0.28 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 591 (0.11")

CSI: TC=0.39/1.00 (C-F:1) , BC=0.28/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.27/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215184

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423564	J14	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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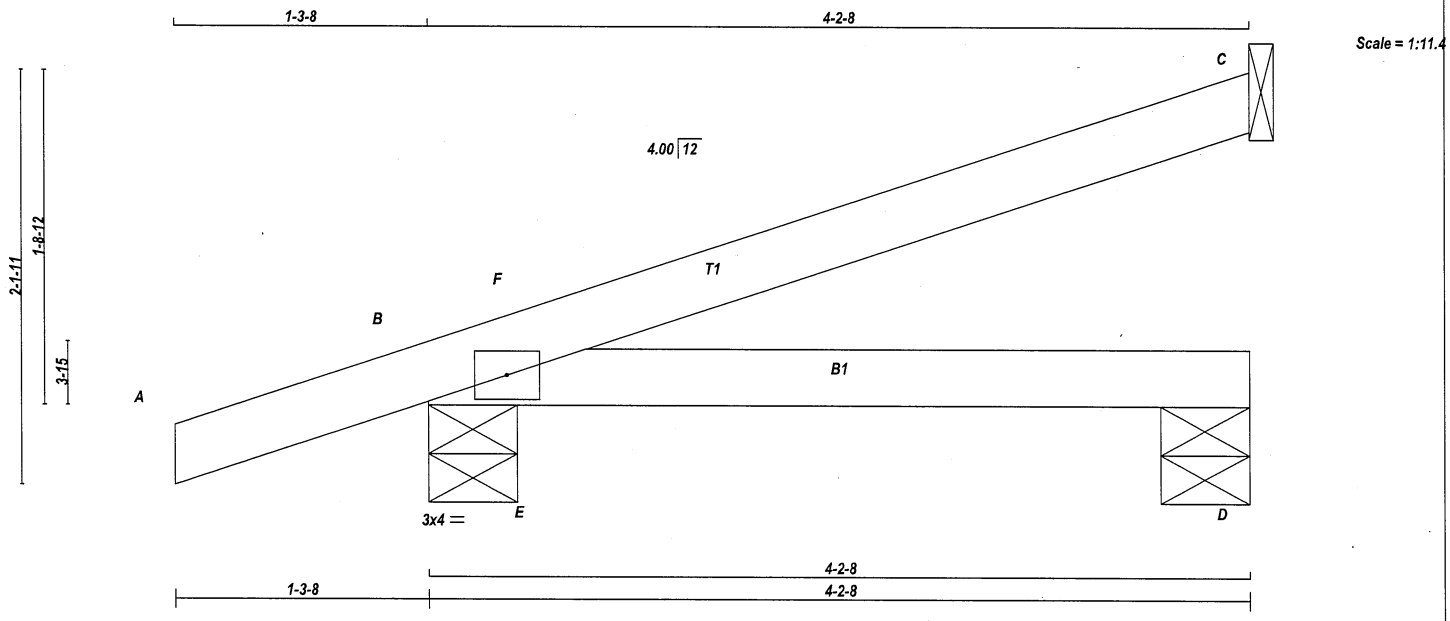


TOTAL WEIGHT = 4 X 12 = 46 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	203	0	203	0	0	1-8	1-8
B	426	0	426	0	0	5-8	5-8
D	73	0	73	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	140	114 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
B	296	224 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
D	53	23 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 22	-112.4	-112.4	0.14 (1)	10.00		E- F	-232 / 7	0.00 (1)
B- F	-13 / 0	-112.4	-112.4	0.05 (4)	6.25				
F- C	0 / 2	-112.4	-112.4	0.25 (1)	10.00				
B- E	0 / 0	-18.5	-18.5	0.19 (1)	10.00				
E- D	0 / 0	-18.5	-18.5	0.19 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.25/1.00 (C-F:1) , BC=0.19/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.19/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

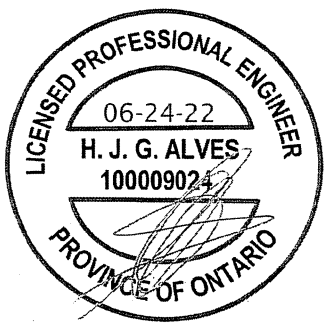
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215185

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423567	J30	7	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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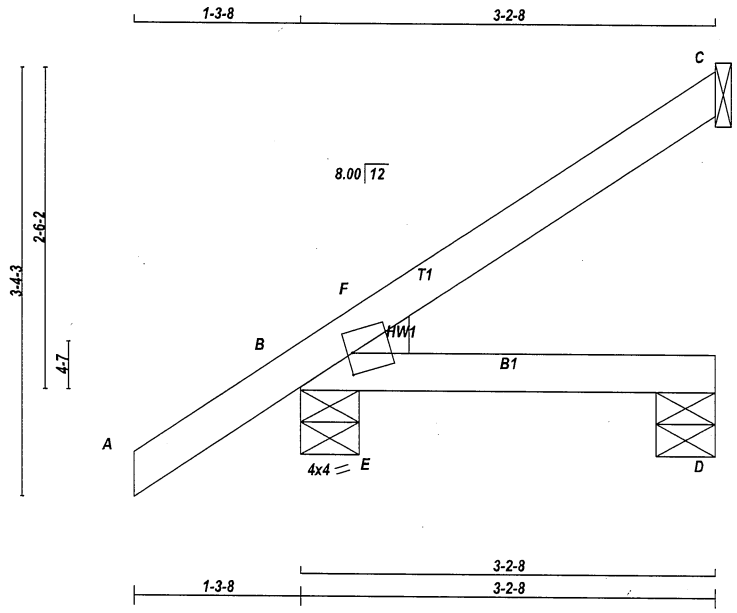

Scale = 1:17.2

TOTAL WEIGHT = 7 X 11 = 74 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	4.0	2.00	0.50

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	150	0	150	0	0	1-8	1-8	
B	364	0	364	0	0	5-8	5-8	2x4 L
D	60	0	60	0	0	5-8	5-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	103	84 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
B	253	193 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
D	44	20 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-112.4	-112.4	0.16	(5)	10.00	E- F	-155 / 0	0.00 (1)
B- F	-13 / 0	-112.4	-112.4	0.03	(4)	6.25			
F- C	0 / 3	-112.4	-112.4	0.14	(1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.13	(1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.13	(1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (A-B:5) , BC=0.13/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.12/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215222

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423567	J31	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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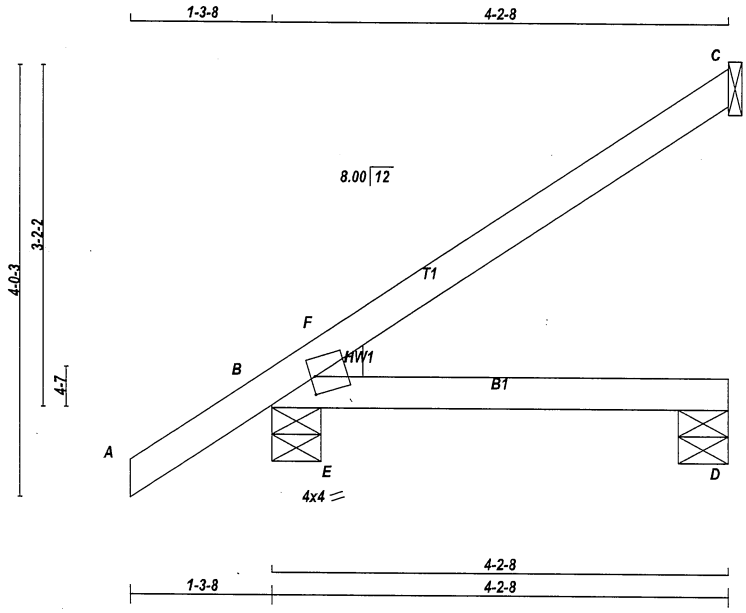

TOTAL WEIGHT = 3 X 13 = 39 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	4.0	2.00	0.50

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	200	0	200	0	0	1-8	1-8	
B	430	0	430	0	0	5-8	5-8	2x4 L
D	76	0	76	0	0	5-8	5-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	137	112 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	299	226 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	56	25 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-112.4	-112.4	0.15 (1)	10.00	E- F	-245 / 0	0.00 (1)
B- F	-18 / 4	-112.4	-112.4	0.04 (4)	6.25			
F- C	0 / 3	-112.4	-112.4	0.24 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.21 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.24/1.00 (C-F:1) , BC=0.21/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.19/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215223

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	J60	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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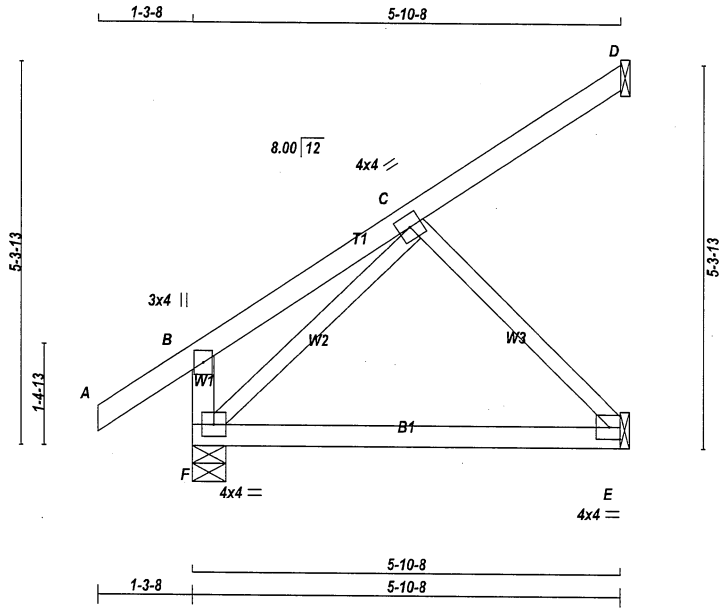

TOTAL WEIGHT = 2 X 24 = 48 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
E	BMW1-t	MT20	4.0	4.0		Edge
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	536	0	536	0	0	5-8	5-8
D	124	0	124	0	0	1-8	1-8
E	263	0	263	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	373	278 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
D	85	71 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
E	186	121 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-282 / 0	0.0	0.0	0.03 (1)	7.81	C- E	-285 / 0	0.09 (1)
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	F- C	-278 / 0	0.08 (1)
B- C	0 / 22	-112.4	-112.4	0.16 (1)	10.00			
C- D	-22 / 0	-112.4	-112.4	0.16 (1)	6.25			
F- E	0 / 194	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.19/1.00 (E-F:4) , WB=0.09/1.00 (C-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (F) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
10000902
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215232

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	J61	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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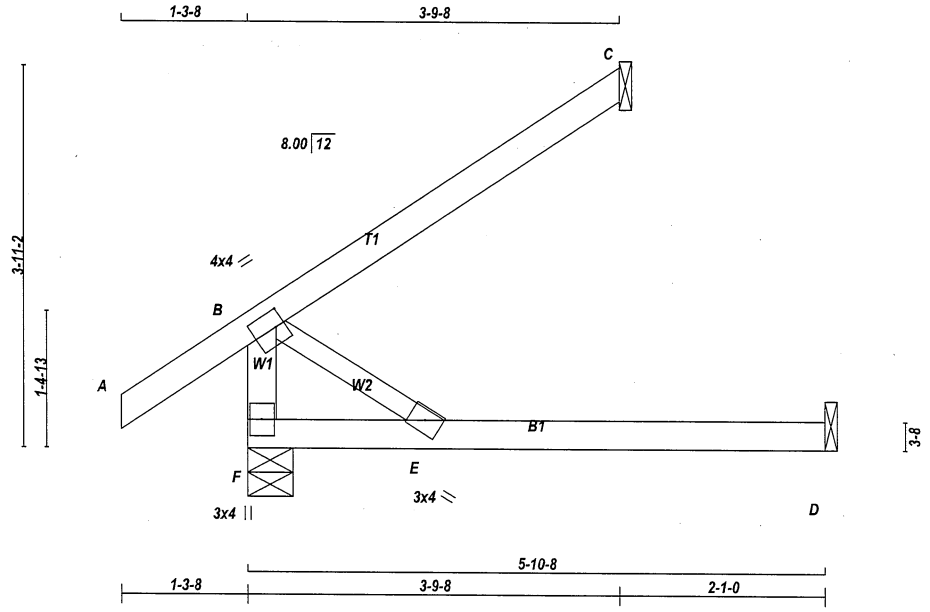

TOTAL WEIGHT = 2 X 17 = 33 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	422	0	422	0	0	5-8	5-8
C	213	0	213	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	295	212 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	146	123 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-367 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00			
B- C	0 / 0	-112.4	-112.4	0.27 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215233

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	J62	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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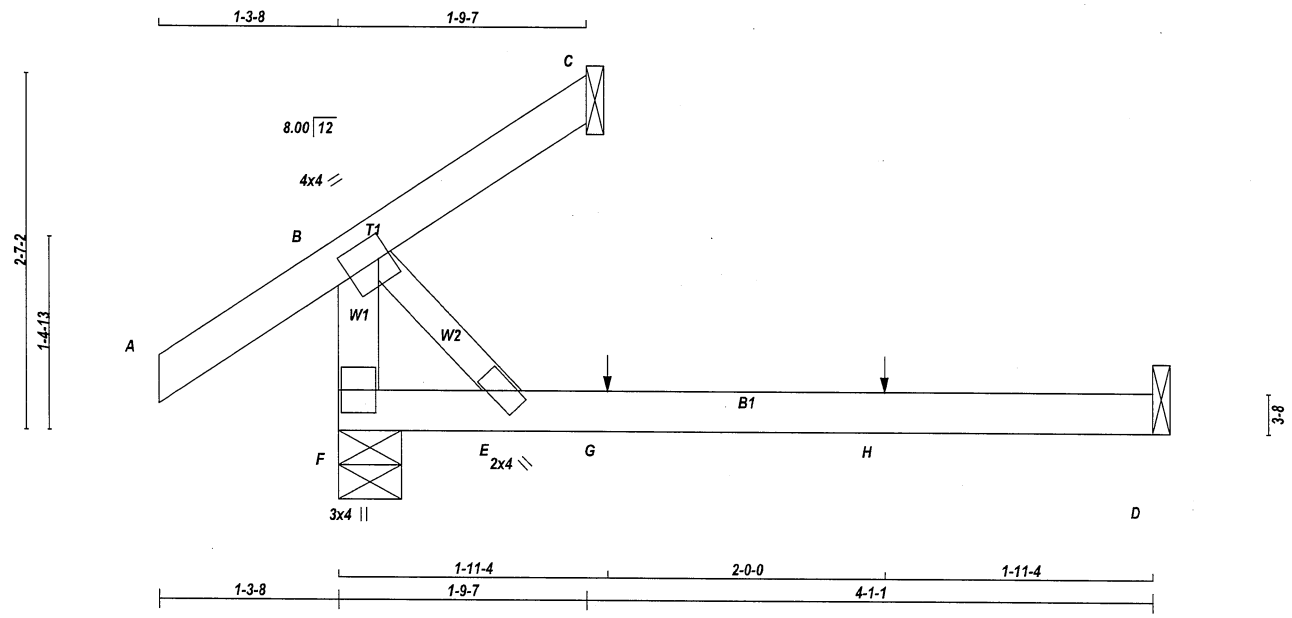

TOTAL WEIGHT = 2 X 13 = 27 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	368	0	368	0	0	5-8	5-8
C	41	0	41	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	258	181 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	28	24 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-314 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00			
B- C	-33 / 0	-112.4	-112.4	0.14 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
H- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215234

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	J63	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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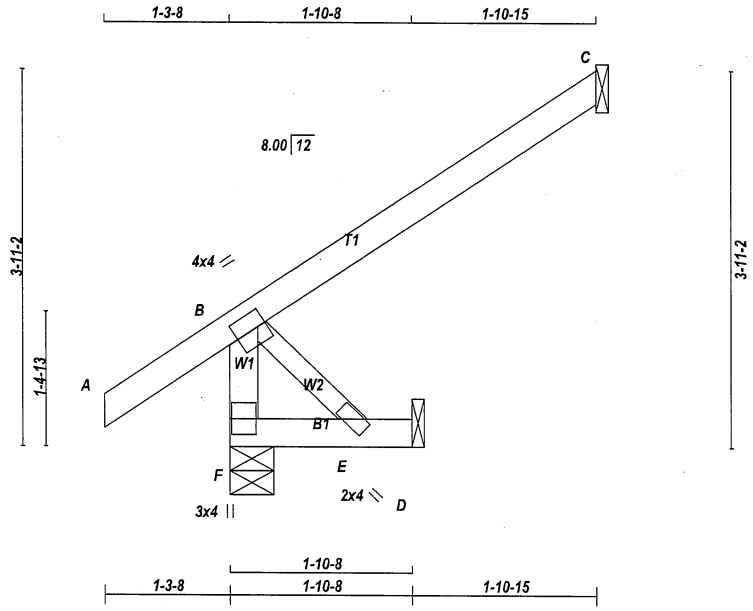

TOTAL WEIGHT = 2 X 12 = 23 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	384	0	384	0	0	5-8	5-8
C	213	0	213	0	0	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	265	212 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	146	123 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-367 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00			
B- C	0 / 0	-112.4	-112.4	0.27 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215235

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	J64	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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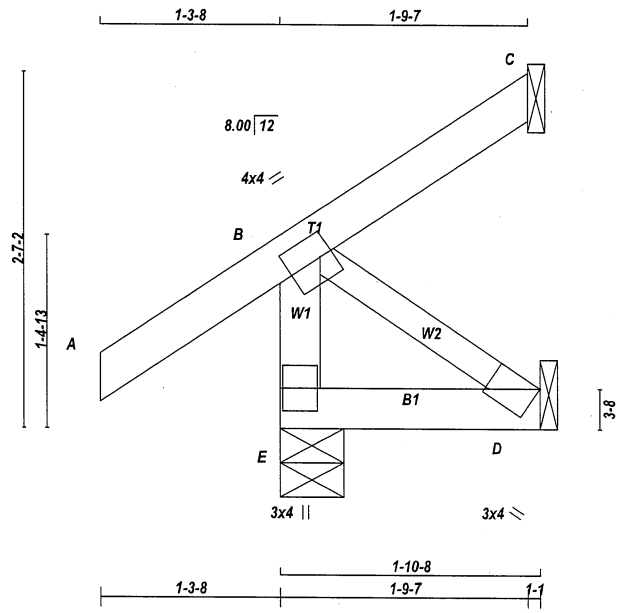

TOTAL WEIGHT = 2 X 9 = 18 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	BMW1+w	MT20	3.0	4.0	3.75	Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	330	0	330	0	0	5-8	5-8
C	41	0	41	0	-50	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	227	181 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	28	24 / -34	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MEMB. FR-TO	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-314 / 0	0.0	0.0	0.03 (1)	B- D	7.81	0 / 0	0.00 (1)
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00		
B- C	-33 / 0	-112.4	-112.4	0.14 (1)		6.25		
E- D	0 / 0	-18.5	-18.5	0.02 (4)		10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215236

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	J65	7	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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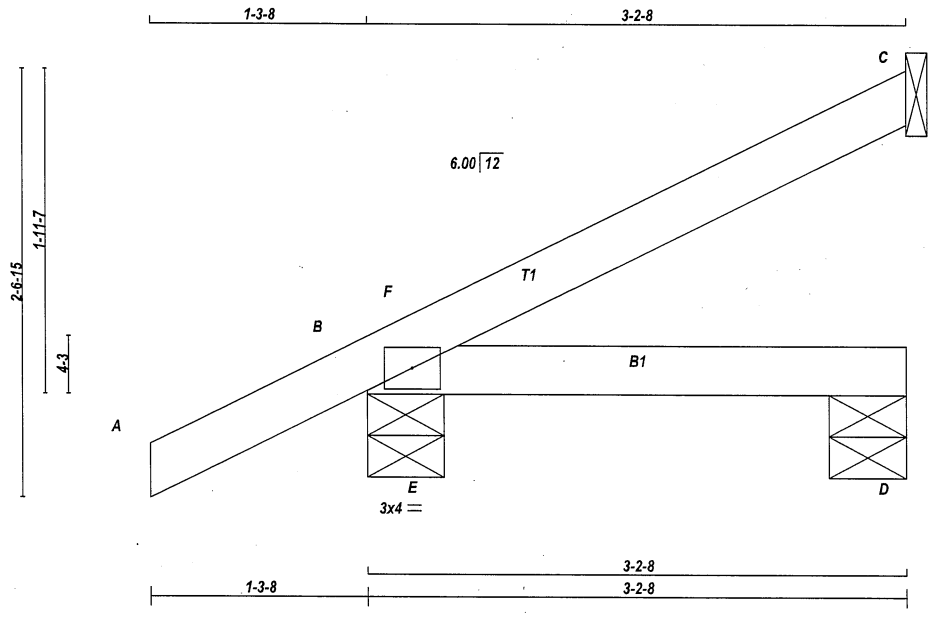

TOTAL WEIGHT = 7 X 10 = 67 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	155	0	155	0	0	1-8	1-8
B	363	0	363	0	0	5-8	5-8
D	55	0	55	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	106	87 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
B	252	192 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
D	41	17 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 33	-112.4	-112.4	0.15 (5)	10.00	E- F	-203 / 3	0.00 (1)
B- F	-14 / 10	-112.4	-112.4	0.02 (4)	6.25			
F- C	-1 / 2	-112.4	-112.4	0.14 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.11 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (A-B:5) , BC=0.11/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215237

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	J66	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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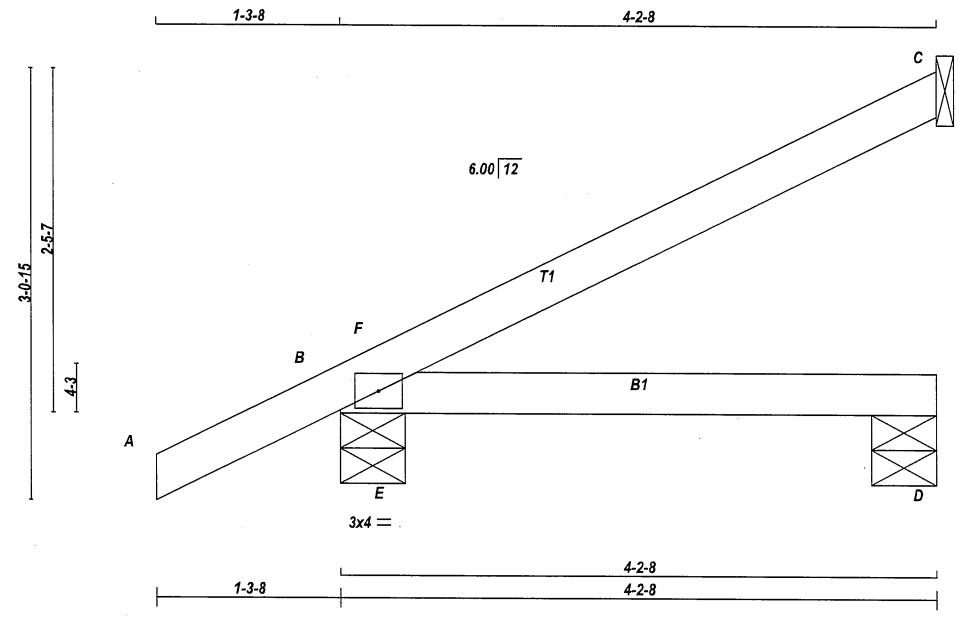

TOTAL WEIGHT = 3 X 12 = 36 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	205	0	205	0	0	1-8	1-8
B	428	0	428	0	0	5-8	5-8
D	71	0	71	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	141	115 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
B	297	225 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	52	22 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 33	-112.4	-112.4	0.15 (1)	10.00	E- F	-330 / 6	0.00 (1)
B- F	-19 / 43	-112.4	-112.4	0.05 (1)	6.25			
F- C	-4 / 2	-112.4	-112.4	0.25 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.18 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.25/1.00 (C-F:1) , BC=0.18/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches .

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215238

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423570	J67	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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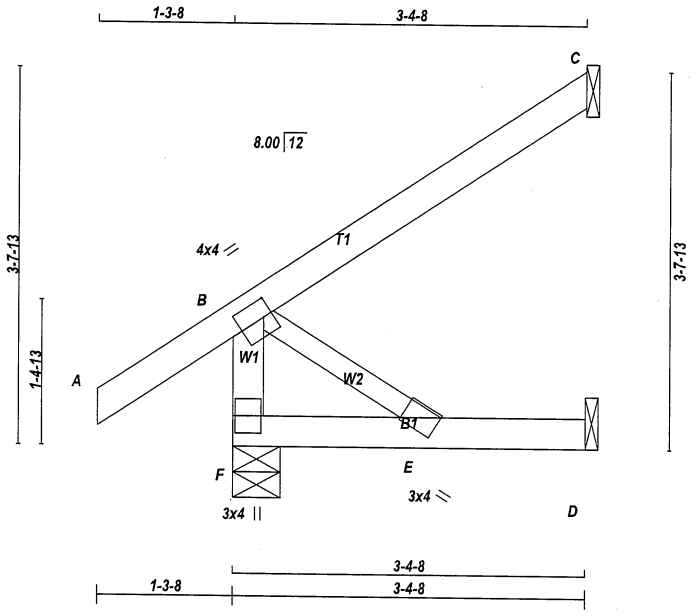

TOTAL WEIGHT = 2 X 13 = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	375	0	375	0	0	5-8	5-8
C	190	0	190	0	0	1-8	1-8
D	31	0	35	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	260	199 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	130	110 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-344 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 43	-112.4	-112.4	0.16 (5)	10.00			
B- C	0 / 0	-112.4	-112.4	0.22 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.06/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215239

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
423570	J68	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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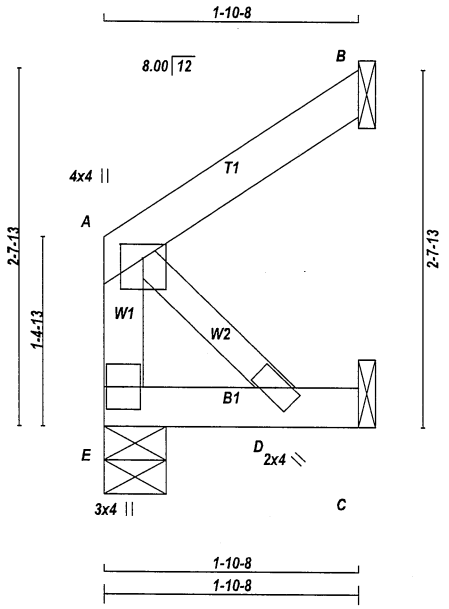


Scale = 1:16.3

TOTAL WEIGHT = 2 X 7 = 14 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	123	0	123	0	0	5-8	5-8
B	105	0	105	0	0	1-8	1-8
C	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	86	61 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	72	61 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-105 / 0	0.0	0.0	0.01 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-112.4	-112.4	0.07 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

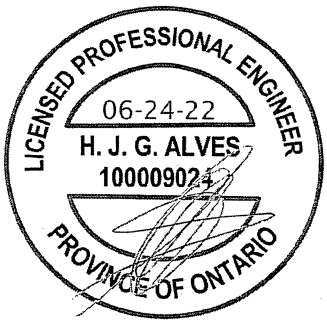
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2215240

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423572	J70	9	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 12:56:34 2022 Page 1
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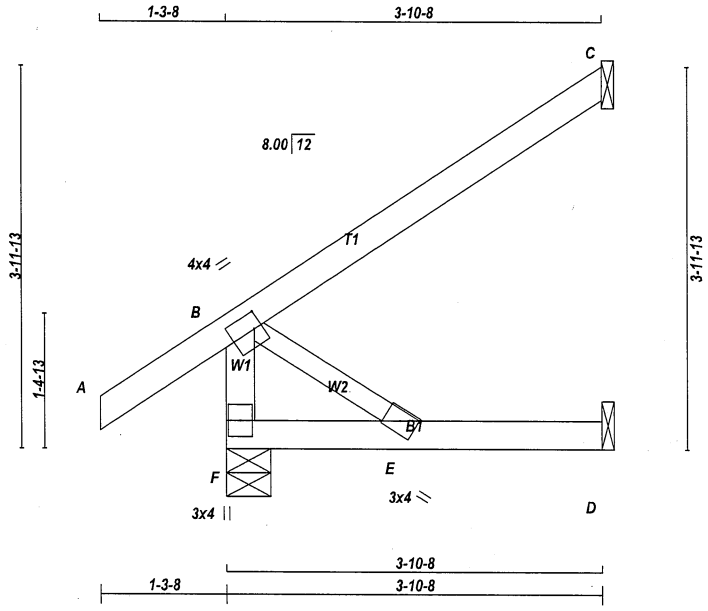


TOTAL WEIGHT = 9 X 14 = 130 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	408	0	408	0	0	5-8	5-8
C	218	0	218	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	283	215 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	149	126 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-372 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 43	-112.4	-112.4	0.16 (5)	10.00			
B- C	0 / 0	-112.4	-112.4	0.29 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06-24-22
H. J. G. ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2215258

REVIEWED

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1800218 Feb 09, 2018

REVIEWED

MiTek®

STANDARD DETAIL MSD2015-H

Issued: **SEPTEMBER 22, 2020**
Expiry: **APRIL 30, 2022**

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

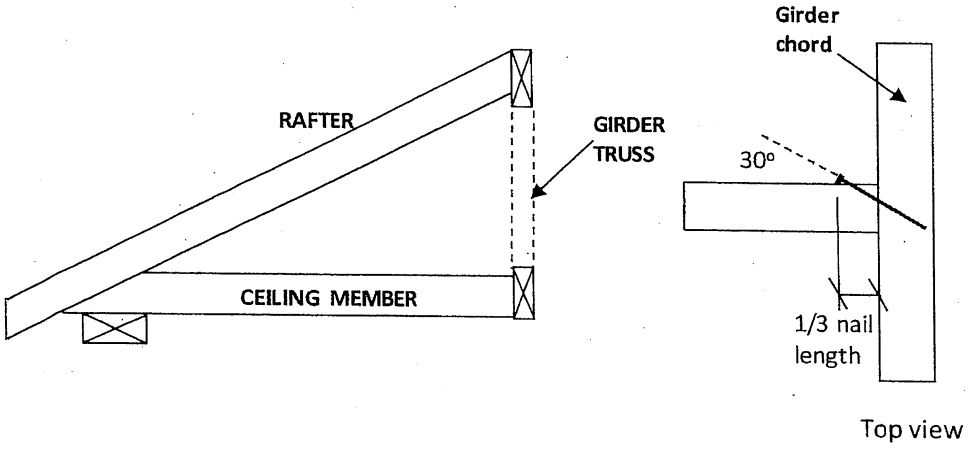


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

©2020 MiTek Canada Inc., 240 Stirling Crescent, Bradford, Ontario, L3Z 4L5 | (800) 268-3434, www.mitek.ca

REVIEWED

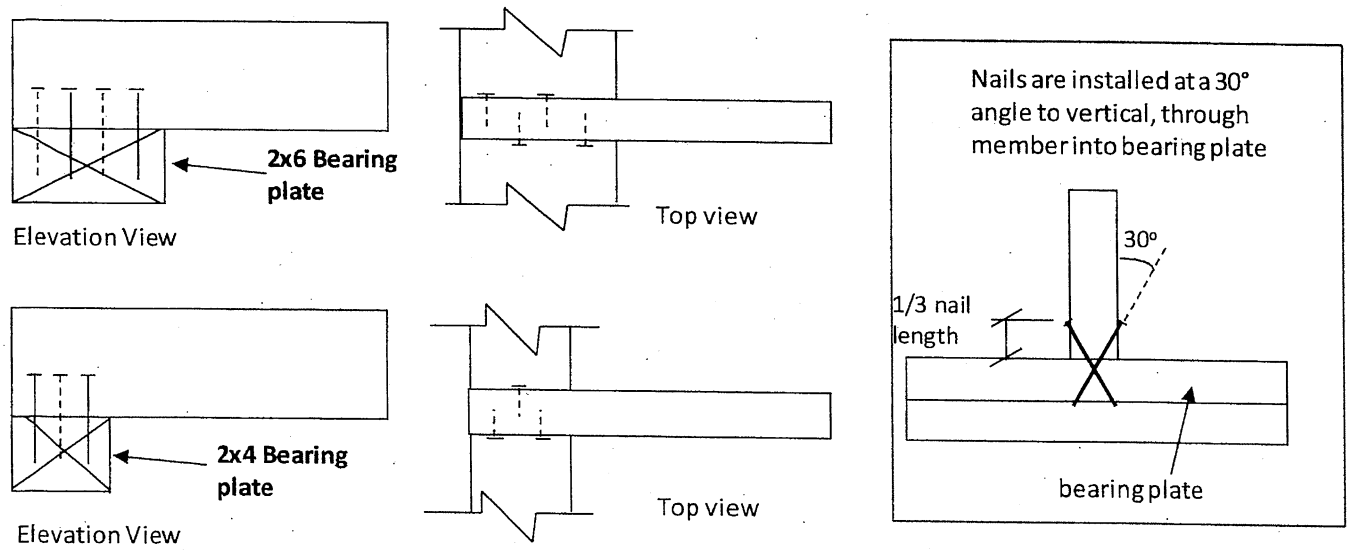
MiTek®

STANDARD DETAIL MSD2015-H

Issued: **SEPTEMBER 22, 2020**
Expiry: **APRIL 30, 2022**

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485

December 21, 2020

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REVIEWED

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie®

Standard and Double-Shear Joist Hangers

VALUE ENGINEERED

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

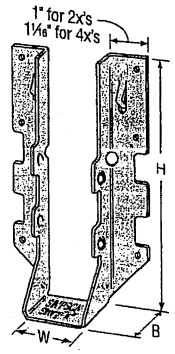
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

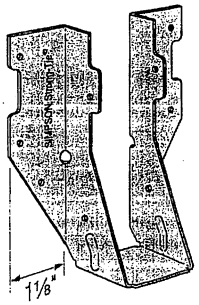
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

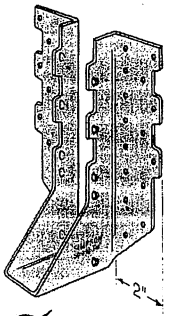
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



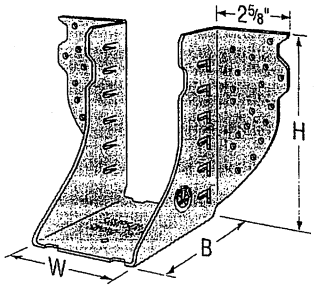
LUS28



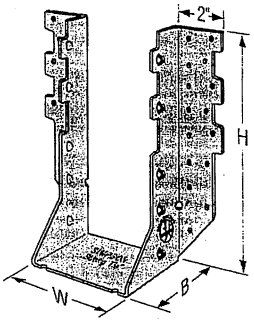
LU26L



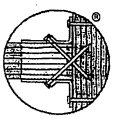
HUS210
(HUS26, HUS28, and HHUS similar)



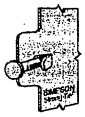
HGUS28-2



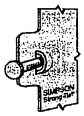
HHUS210-2



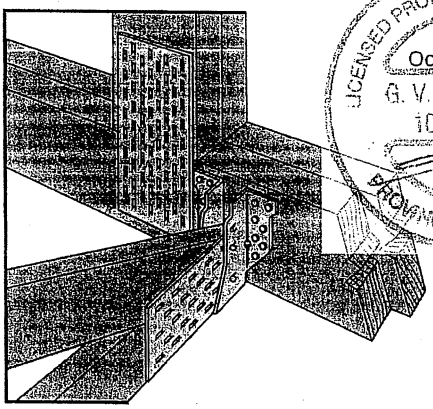
Double-Shear Nailing Top View



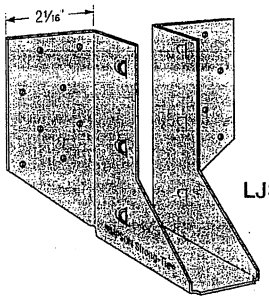
Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View *(available on some models)*



Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

LICENSED PROFESSIONAL ENGINEER
Oct. 14, 2020
G. V. LUBARSKY
100044463
PROVINCE OF ONTARIO

C-C-CAN2020 © 2020 SIMPSON STRONG-TIE COMPANY INC.

REVIEWED

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

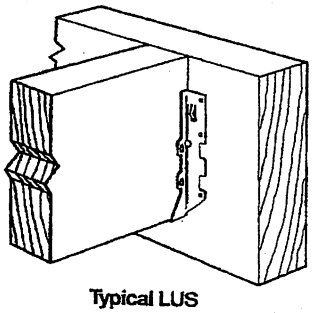
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

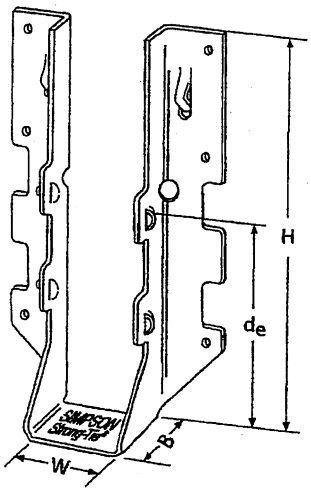
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



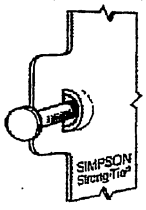
Typical LUS Installation



LUS28

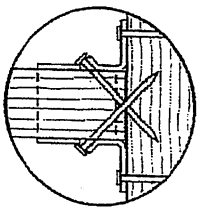
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

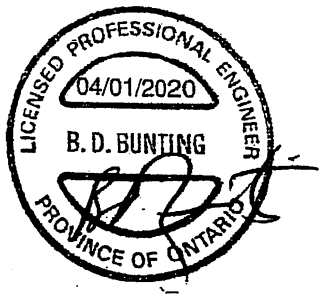


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

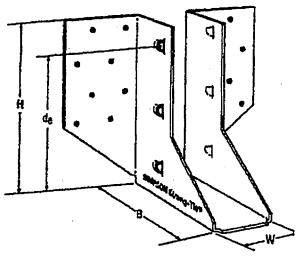
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

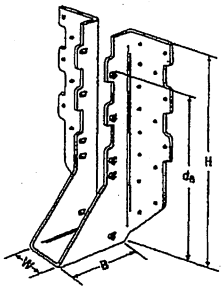
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

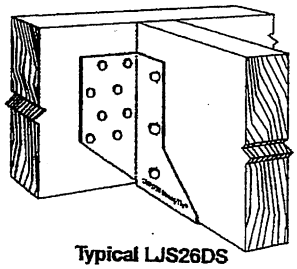
- See current catalogue for options



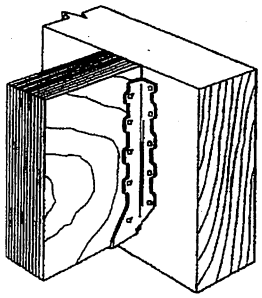
LJS26DS



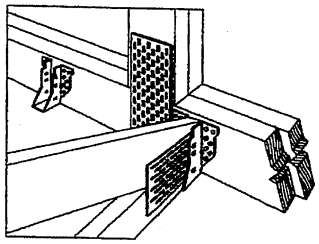
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



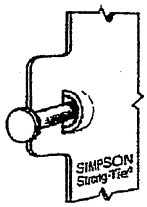
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

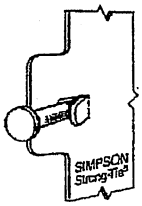
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

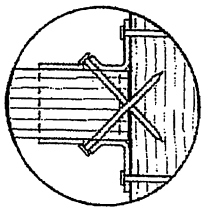


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

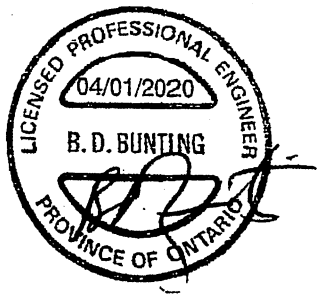
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

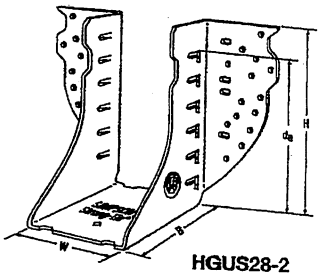
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

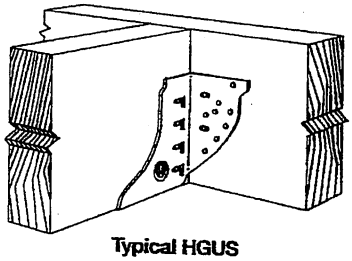
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

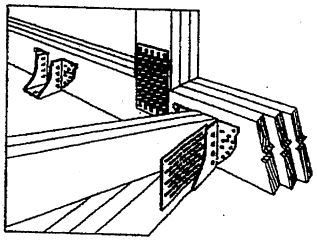
Options:

- See current catalogue for options



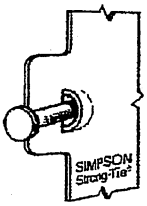
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

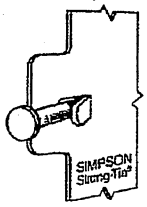
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⁵⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⁵⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

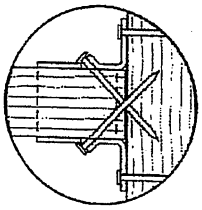


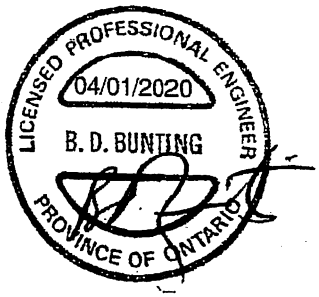
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

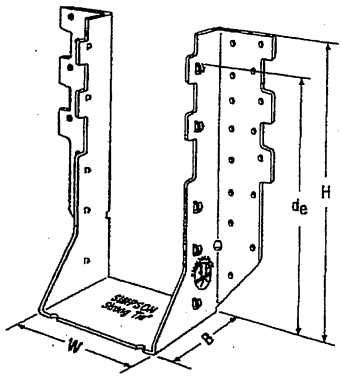
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

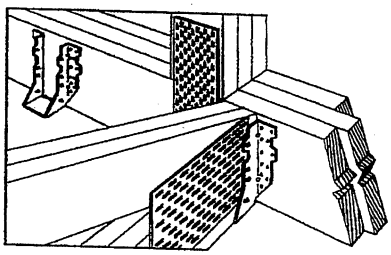
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HHUS410

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



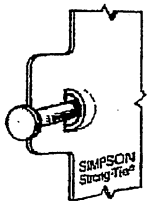
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Options:

- See current catalogue for options

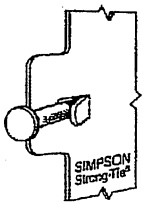
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

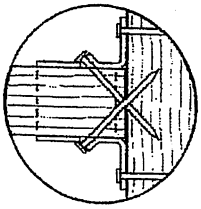


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

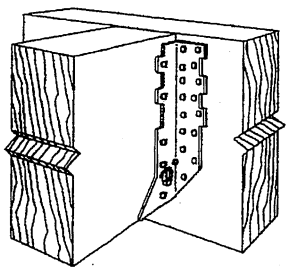
U.S. Patent 5,603,580



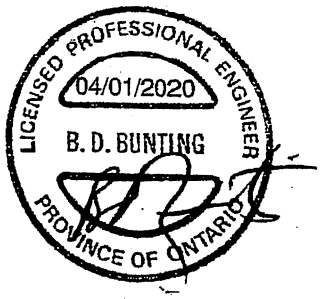
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HHUS Installation



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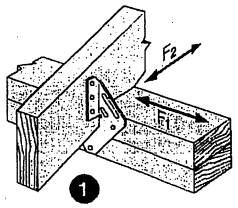
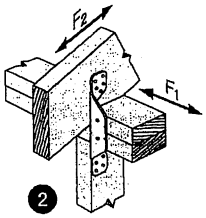
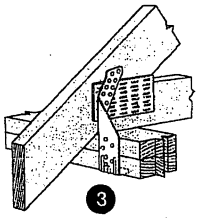
Seismic and Hurricane Ties (cont.)



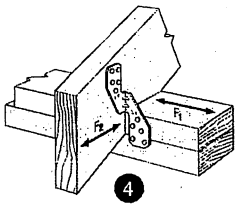
1 H1 Installation



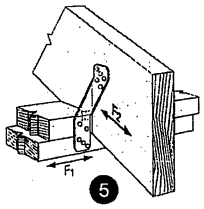
2 H2A Installation



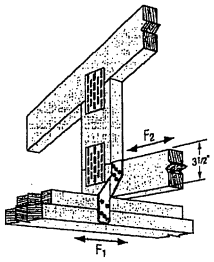
3 TSP Installation



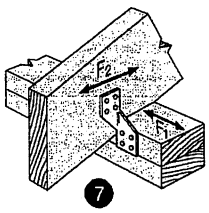
4 H2.5A Installation
(Nails into both top plates)



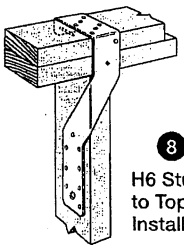
5 H2.5T Installation
(Nails into both top plates)



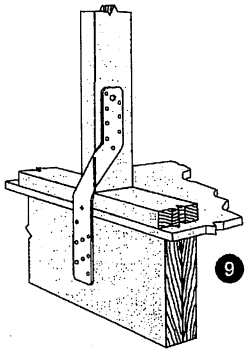
6 H2.5T Installation



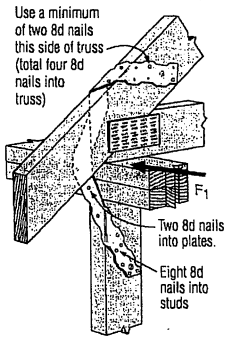
7 H3 Installation
(Nails into upper top plate)



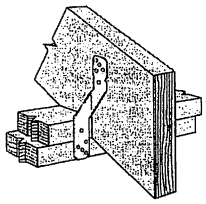
8 H6 Stud to Top Plate Installation



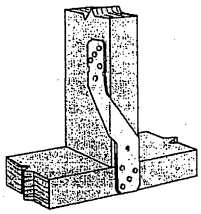
9 H6 Stud to Band Joist Installation



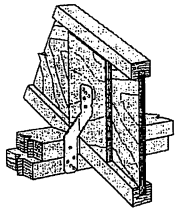
10 H7Z Installation



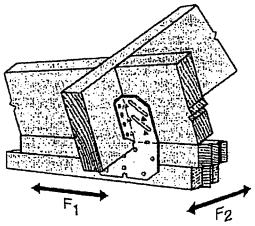
11 H8 Attaching Rafter to Double Top Plates



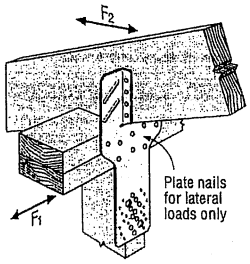
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



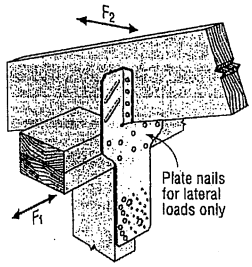
13 H8 attaching I-Joist to Double Top Plates



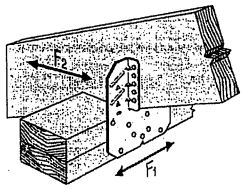
14 H10A Field-Bent Installation



15 H10S Installation



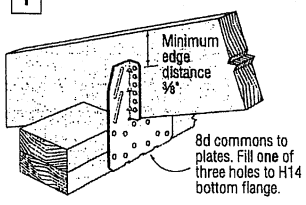
16 H10S Installation with Stud Offset



17 H10A Installation

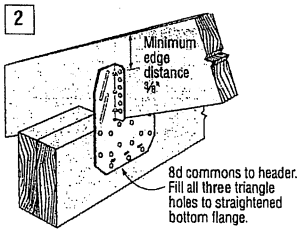
H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

1



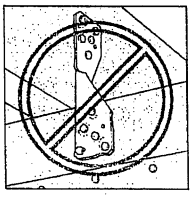
18 H14 Installation to Double Top Plates

2



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

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REVIEWED

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

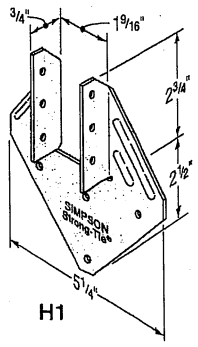
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

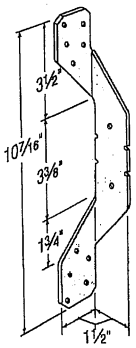
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit **strongtie.com**.

Installation:

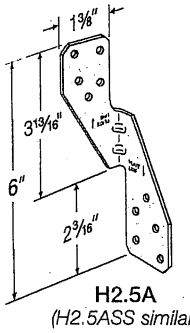
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



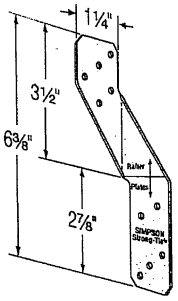
H1



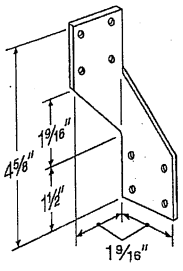
H2A
(H2ASS similar)



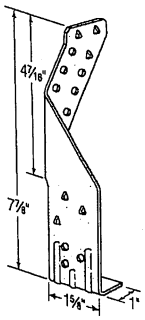
H2.5A
(H2.5ASS similar)



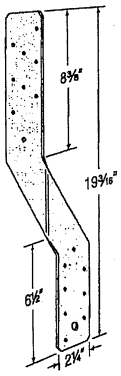
H2.5T



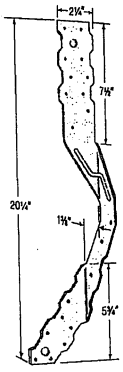
H3



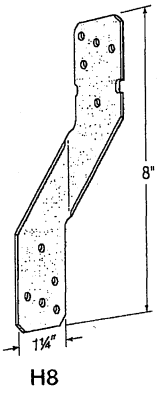
TSP



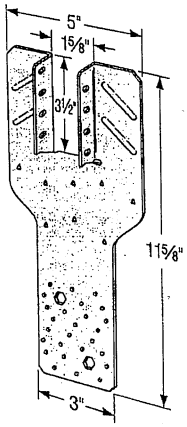
H6



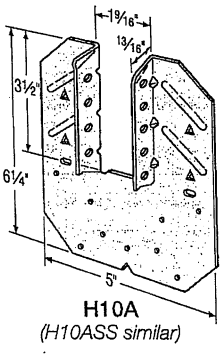
H7Z



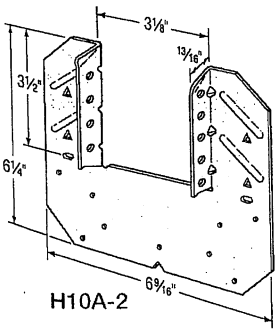
H8



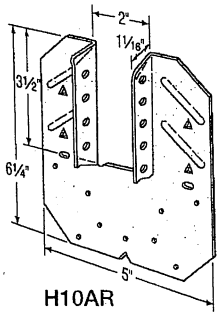
H10S



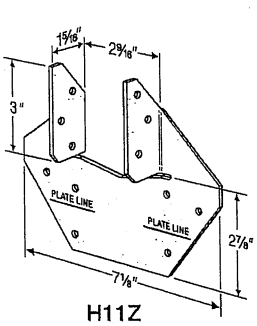
H10A
(H10ASS similar)



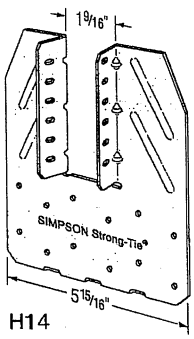
H10A-2



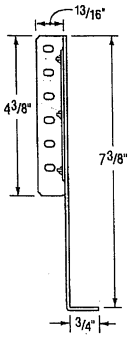
H10AR



H11Z



H14



H14 Profile

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REVIEWED

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON Strong-Tie®

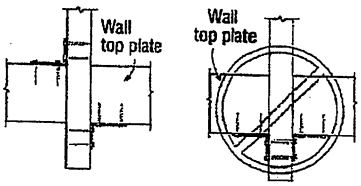
The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners
- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

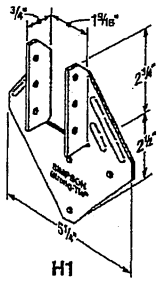
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

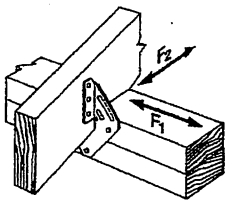
Hurricane Tie Installations to Achieve Twice the Load (Top View)

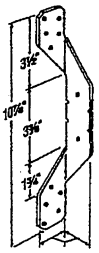


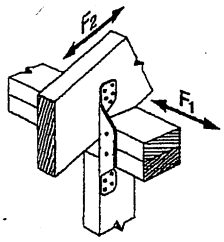
Install diagonally across from each other for minimum 2x truss.

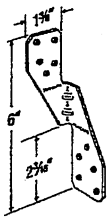
Nailing into both sides of a single ply 2x truss may cause the wood to split.

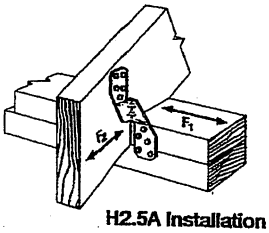

H1

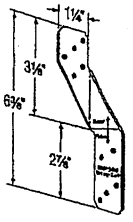

H1 Installation

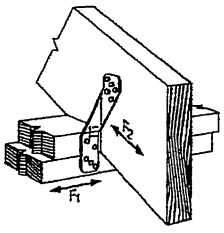

H2A

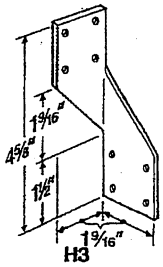

H2A Installation

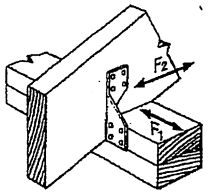

H2.5A

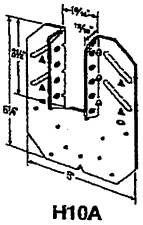

H2.5A Installation

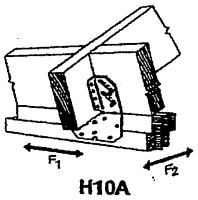

H2.5T


H2.5T Installation (Nails into both top plates)


H3

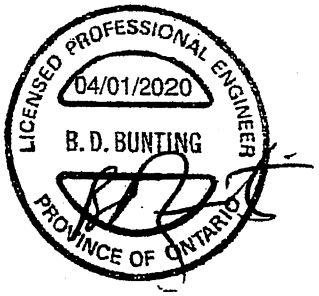

H3 Installation


H10A


H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
		To Rafter	To Plates	To Studs		F_1	F_2		F_1	F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

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TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON Strong-Tie®

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

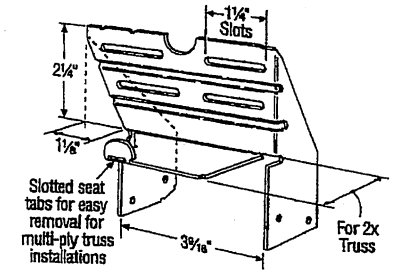
Design: Factored resistances are in accordance with CSA 086-14

Installation:

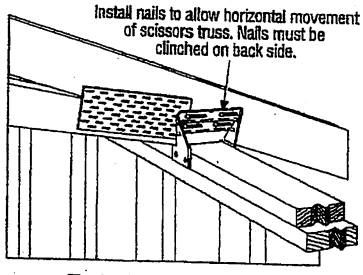
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

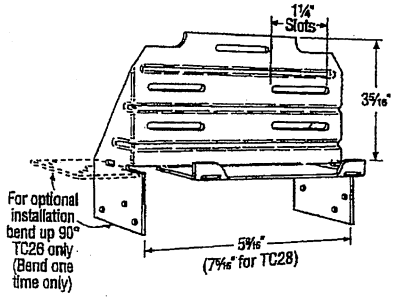
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



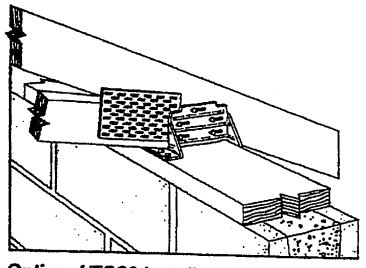
TC24
U.S. Patent 4,932,173



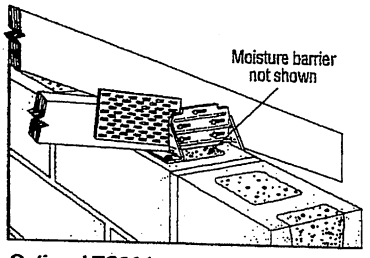
Typical TC24 Installation



TC26
(TC28 Similiar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
strongtie.com

REVIEWED

HTU

Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
					D.Fir-L		S-P-F	
			Header	Joist	Uplift (K_D = 1.15)	Normal (K_D = 1.00)	Uplift (K_D = 1.15)	Normal (K_D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3⅞	(2) 2x4	(10) 16d	(14) 10d x 1½"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5½	(2) 2x4	(10) 16d	(20) 10d x 1½"	2470	4015	1755	2850
					10.99	17.86	7.81	12.68
HTU28 (Max.)	3⅞	(2) 2x6	(20) 16d	(26) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19
HTU210 (Max.)	7¼	(2) 2x6	(20) 16d	(32) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19

See table footnotes on p. 260.

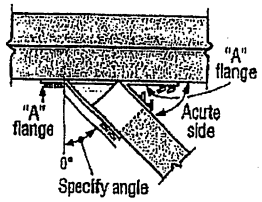
LICENSED PROFESSIONAL ENGINEER 5/29/19 B. D. BUNTING PROVINCE OF ONTARIO

Hanger Options

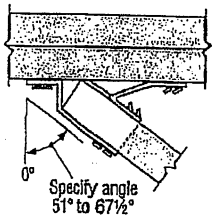
See Hanger Options information on pp. 125–127.

Skewed Seat

- Skewable up to 67½°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
				D.Fir-L		S-P-F	
		Header	Joist	Uplift (KD=1.15)	Normal (KD=1.00)	Uplift (KD=1.15)	Normal (KD=1.00)
				lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1½"	1835	4110	1300	2905
				8.16	18.28	5.78	12.92
	51–67½	(20) 16d	(12) 10d x 1½"	1350	3620	955	2560
				6.01	16.10	4.25	11.39
HTU28	< 51	(26) 16d	(20) 10d x 1½"	2810	4270	1985	3030
				12.50	18.99	8.83	13.48
	51–67½	(26) 16d	(17) 10d x 1½"	2075	3930	1465	2780
				9.23	17.48	6.52	12.37
HTU210	< 51	(32) 16d	(26) 10d x 1½"	3785	4430	2675	3135
				16.84	19.71	11.90	13.95
	51–67½	(32) 16d	(22) 10d x 1½"	2795	4240	1980	3000
				12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
				9.52	16.53	6.74	11.68
	51–67½	(20) 16d	(12) 10d	1610	3920	1140	2785
				7.16	17.44	5.07	12.39
HTU28-2	< 51	(26) 16d	(20) 10d	3960	5425	2815	3855
				17.62	24.13	12.52	17.15
	51–67½	(26) 16d	(17) 10d	2385	5425	1695	3855
				10.61	24.13	7.54	17.15
HTU210-2	< 51	(32) 16d	(26) 10d	5025	6890	3570	4890
				22.35	30.65	15.88	21.75
	51–67½	(36) 16d	(22) 10d	3145	6680	2225	4745
				13.99	29.72	9.90	21.10

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3½" long, 10d x 1½" = 0.148" dia. x 1½" long, 10d = 0.148" dia. x 3" long. See pp. 27–28 for other nail sizes and information.

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REVIEWED

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

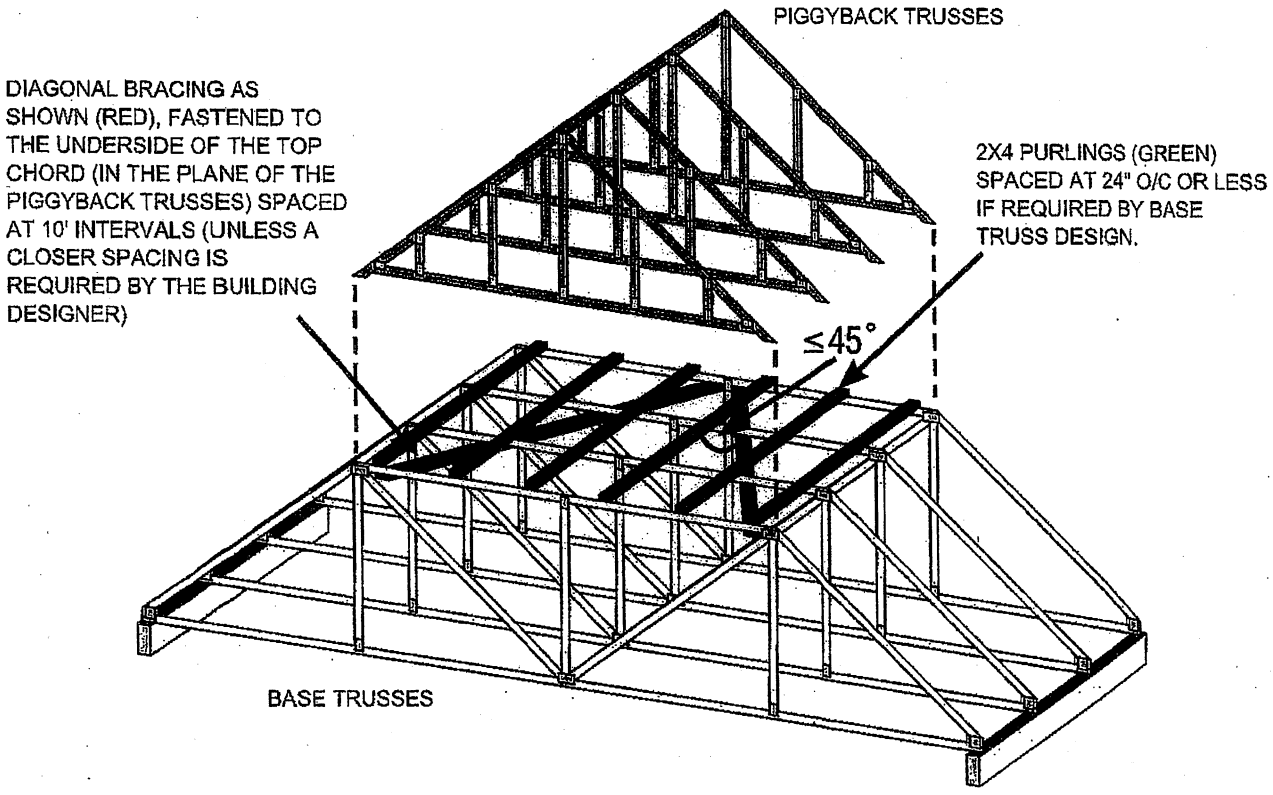
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

REVIEWED

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

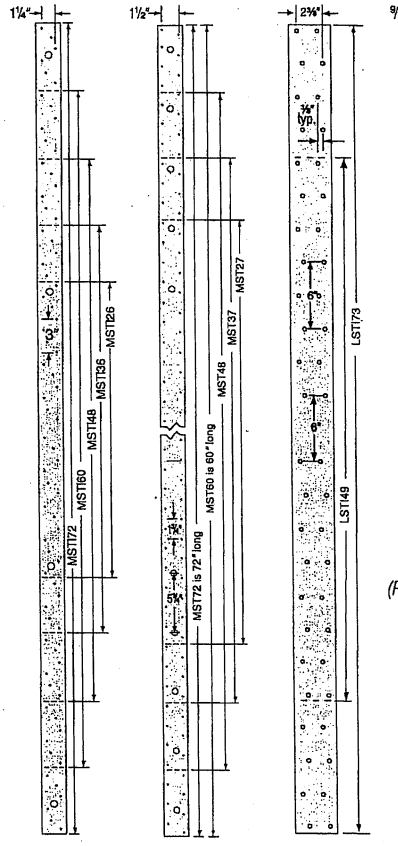
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

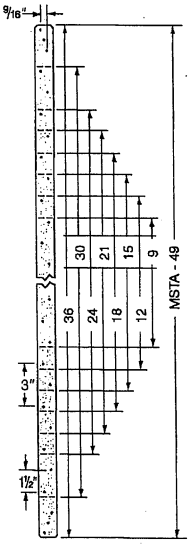
Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

Installation: Use all specified fasteners; see General Notes

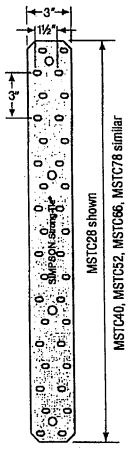
Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths



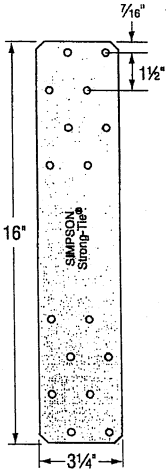
MSTI MST LSTI



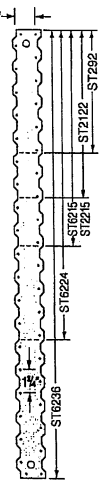
LSTA and MSTA
(Pilot holes not shown)



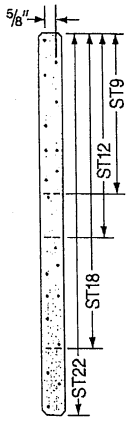
MSTC



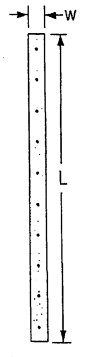
HRS416Z



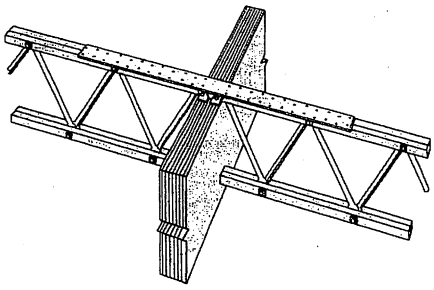
ST



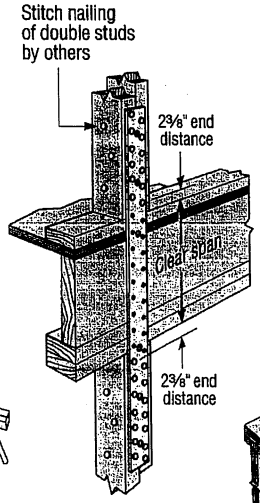
ST9, ST12, ST18, ST22



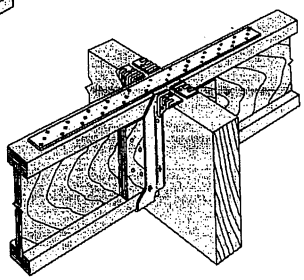
ST2115



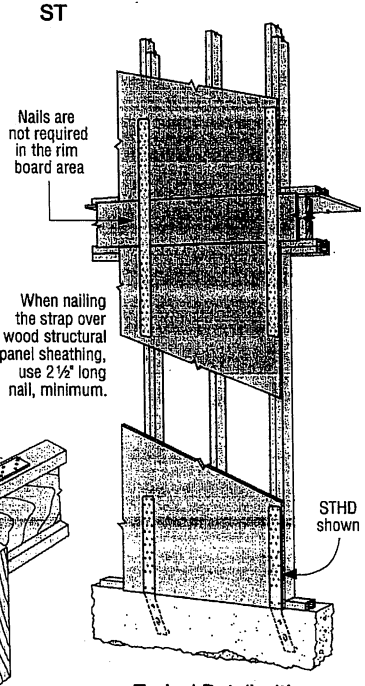
Typical LSTI Installation



Floor-to-Floor Tie Installation Showing a Clear Span



Typical MSTI Installation
(MIT hanger shown)
LSTI similar



Typical Detail with Strap Installed over Wood Structural Panel Sheathing

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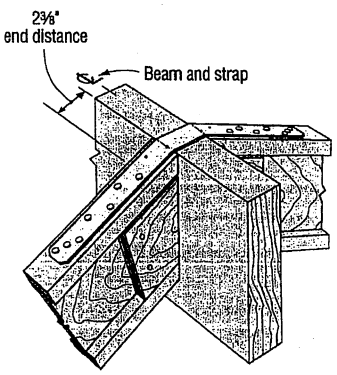
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties (cont.)

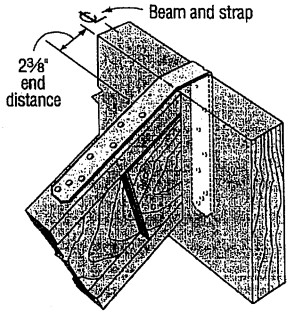
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.) W	Dimensions (in.) L	Fasteners (Total)	Factored Tensile Resistance D.Fir–L (K_D = 1.00) lb. / kN	D.Fir–L (K_D = 1.15) lb. / kN	S-P-F (K_D = 1.00) lb. / kN	S-P-F (K_D = 1.15) lb. / kN
LSTA9	20	1¼	9	(6) 10d	600 / 2.67	690 / 3.07	555 / 2.47	635 / 2.82
LSTA12		1¼	12	(8) 10d	800 / 3.56	920 / 4.09	735 / 3.27	845 / 3.76
LSTA15		1¼	15	(10) 10d	1000 / 4.45	1150 / 5.12	920 / 4.09	1060 / 4.72
LSTA18		1¼	18	(12) 10d	1200 / 5.34	1380 / 6.14	1105 / 4.92	1270 / 5.65
LSTA21		1¼	21	(14) 10d	1400 / 6.23	1610 / 7.16	1290 / 5.74	1485 / 6.61
LSTA24		1¼	24	(16) 10d	1600 / 7.12	1840 / 8.19	1475 / 6.56	1695 / 7.54
ST292		2⅛	9⅝	(8) 8d	585 / 2.60	675 / 3.00	535 / 2.38	615 / 2.74
ST2122		2⅛	12⅜	(12) 8d	940 / 4.18	1085 / 4.83	865 / 3.85	995 / 4.43
ST2115		¾	16⅝	(8) 8d	670 / 2.98	770 / 3.43	615 / 2.74	710 / 3.16
ST2215		2⅛	16⅝	(16) 8d	1335 / 5.94	1540 / 6.85	1235 / 5.49	1420 / 6.32
LSTA30	18	1¼	30	(20) 10d	2235 / 9.94	2465 / 10.97	2075 / 9.23	2385 / 10.61
LSTA36		1¼	36	(24) 10d	2465 / 10.97	2465 / 10.97	2465 / 10.97	2465 / 10.97
LSTI49		3¾	49	(32) 10d x 1½"	3115 / 13.86	3580 / 15.93	2852 / 12.69	3280 / 14.59
LSTI73		3¾	73	(48) 10d x 1½"	4670 / 20.77	5370 / 23.89	4280 / 19.04	4920 / 21.89
MSTA9		1¼	9	(6) 10d	670 / 2.98	770 / 3.43	625 / 2.78	715 / 3.18
SS MSTA12		1¼	12	(8) 10d	895 / 3.98	1030 / 4.58	830 / 3.69	955 / 4.25
MSTA15		1¼	15	(10) 10d	1120 / 4.98	1285 / 5.72	1040 / 4.63	1195 / 5.32
SS MSTA18		1¼	18	(12) 10d	1340 / 5.96	1545 / 6.87	1245 / 5.54	1430 / 6.36
MSTA21		1¼	21	(14) 10d	1565 / 6.96	1800 / 8.01	1455 / 6.47	1670 / 7.43
SS MSTA24		1¼	24	(16) 10d	1790 / 7.96	2060 / 9.16	1660 / 7.38	1910 / 8.50
MSTA30	16	1¼	30	(20) 10d	2470 / 10.99	2840 / 12.63	2260 / 10.05	2595 / 11.54
SS MSTA36		1¼	36	(24) 10d	2965 / 13.19	3070 / 13.66	2710 / 12.06	3070 / 13.66
MSTA49		1¼	49	(28) 8d	2725 / 12.12	2725 / 12.12	2545 / 11.32	2725 / 12.12
ST6215		2⅛	16⅝	(16) 8d	1405 / 6.25	1615 / 7.18	1300 / 5.78	1500 / 6.67
ST6224		2⅛	23⅝	(24) 8d	2305 / 10.25	2650 / 11.79	2155 / 9.59	2475 / 11.01
ST9		1¼	9	(6) 8d	525 / 2.34	605 / 2.69	490 / 2.18	560 / 2.49
ST12		1¼	11⅝	(8) 8d	700 / 3.11	805 / 3.58	650 / 2.89	750 / 3.34
ST18		1¼	17¾	(12) 8d	1050 / 4.67	1210 / 5.38	975 / 4.34	1125 / 5.00
ST22		1¼	21⅝	(18) 8d	1580 / 7.03	1790 / 7.96	1465 / 6.52	1685 / 7.50



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.

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REVIEWED

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

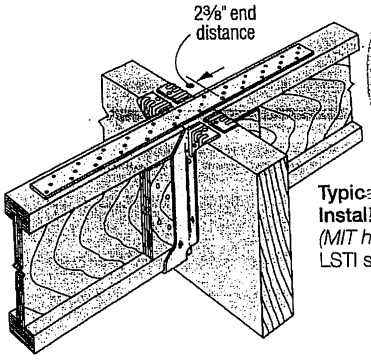
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.) W	Dimensions (in.) L	Fasteners (Total)	Factored Tensile Resistance D.Fir-L (K_D = 1.00) lb. / kN	Factored Tensile Resistance D.Fir-L (K_D = 1.15) lb. / kN	Factored Tensile Resistance S-P-F (K_D = 1.00) lb. / kN	Factored Tensile Resistance S-P-F (K_D = 1.15) lb. / kN
MSTC28	16	3	28¼	(32) 10d	3955 / 17.59	4545 / 20.22	3615 / 16.08	4155 / 18.48
MSTC40	16	3	40¼	(48) 10d	5930 / 26.38	6820 / 30.34	5420 / 24.11	6235 / 27.74
MSTC52	16	3	52¼	(54) 10d	6670 / 29.67	6940 / 30.87	6100 / 27.14	6940 / 30.87
MSTC66	14	3	65¾	(66) 10d	8515 / 37.88	8565 / 38.10	7455 / 33.16	8565 / 38.10
MSTC78	14	3	77¾	(66) 10d	8515 / 37.88	8565 / 38.10	7455 / 33.16	8565 / 38.10
ST6236	14	2⅛	33 13⁄16	(36) 8d	3735 / 16.61	4295 / 19.11	3270 / 14.55	3760 / 16.73
MSTI26	12	2⅛	26	(22) 10d x 1½"	2825 / 12.57	3250 / 14.46	2475 / 11.01	2850 / 12.68
MSTI36	12	2⅛	36	(32) 10d x 1½"	4110 / 18.28	4725 / 21.02	3600 / 16.01	4140 / 18.42
MSTI48	12	2⅛	48	(44) 10d x 1½"	5650 / 25.13	6500 / 28.91	4955 / 22.04	5695 / 25.33
MSTI60	12	2⅛	60	(56) 10d x 1½"	7195 / 32.01	7360 / 32.74	6305 / 28.05	7250 / 32.25
MSTI72	12	2⅛	72	(68) 10d x 1½"	7360 / 32.74	7360 / 32.74	7240 / 32.21	7360 / 32.74
MST27	12	2⅛	27	(26) 8d	2685 / 11.94	3090 / 13.75	2355 / 10.48	2710 / 12.06
MST37	12	2⅛	37½	(38) 8d	3930 / 17.48	4515 / 20.08	3440 / 15.30	3960 / 17.62
MST48	12	2⅛	48	(50) 8d	5170 / 23.00	5945 / 26.45	4530 / 20.15	5210 / 23.18
HRS416Z	12	3¼	16	(16) ¼" x 1½" SDS	2400 / 10.68	2760 / 12.28	2120 / 9.43	2440 / 10.85
MST60	10	2⅛	60	(64) 8d	6620 / 29.45	7610 / 33.85	5800 / 25.80	6670 / 29.67
MST72	10	2⅛	72	(78) 8d	8065 / 35.88	9135 / 40.64	7065 / 31.43	8125 / 36.14

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.



Typical MSTI Installation
(MIT hanger shown)
LSTI similar

LICENSED PROFESSIONAL ENGINEER
Jan. 5, 2021
G. V. LUBARSKY
100044463
PROVINCE OF ONTARIO

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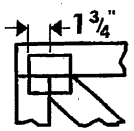
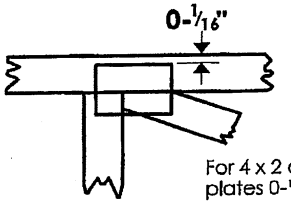
REVIEWED

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

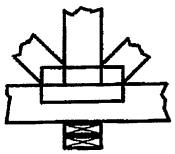
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

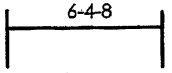


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

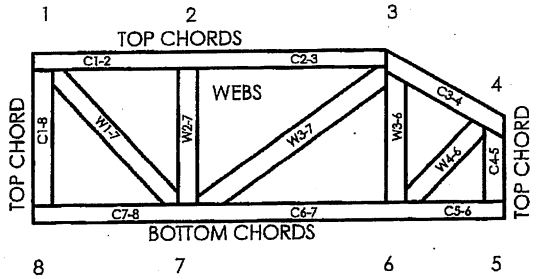
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

REVIEWED