

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD

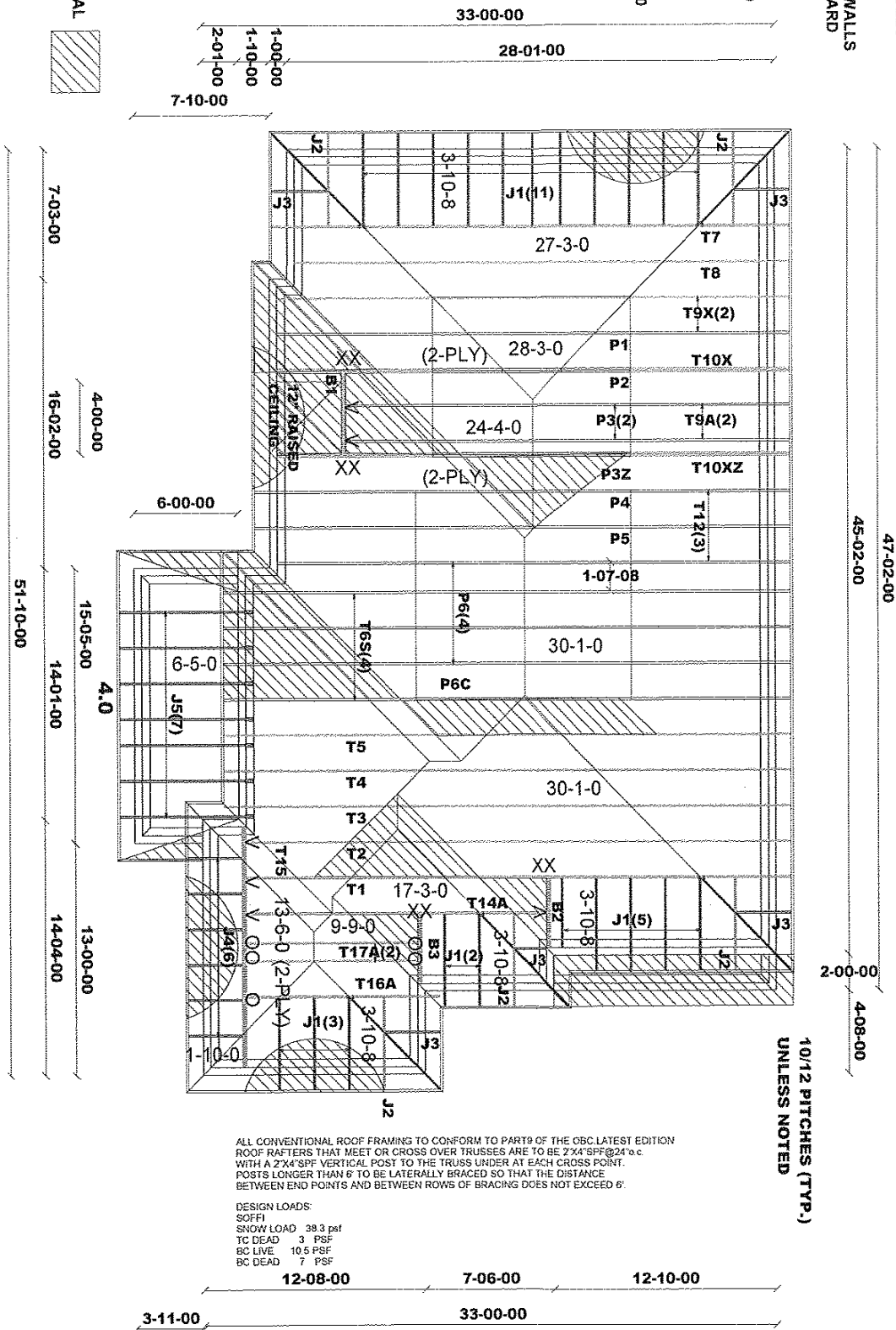
HARDWARE:

HGUS26-2-(XX)
LUS26DS (V)
LUS24 (O)

DENOTE:

B1.B2.B3-2-2X10

T-160686
DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4" SPP @ 24" o.c
WITH A 2"x4" SPP VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

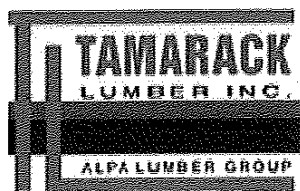
DESIGN LOADS:
SOFFIT
SNOW LOAD 38.3 psf
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

10/12 PITCHES (TYP.)
UNLESS NOTED

Town of Innisfil Certified Model
09/03/2018 9:23:41 AM kgervais

		Job Track: 42067		Builder / Location: BAYVIEW WELLINGTON INNISFIL		Model / Elevation: S39-2 COR STARLING 2/ EL-A	
Layout ID: 272847		Project: ALCONA SHORES		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.		Meek ver 7.5.0	
Plan Log: 88256		Date: 9/18/2017		Designer: EN			

3-11-00 12-08-00 7-06-00 12-10-00
33-00-00



Delivery Shiplist

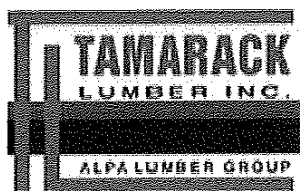
DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272847 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS SUB-BUILDER:
 MODEL: S39-2 CORNER ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T1 HIP GIRDER	10.00 0.00	30-01-00	04-10-07	2 X 6 2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	174.05 106.00		
	1	T2 HIP	10.00 0.00	30-01-00	06-06-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	133.99 84.83		
	1	T3 HIP	10.00 0.00	30-01-00	08-02-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.79 89.33		
	1	T4 HIP	10.00 0.00	30-01-00	09-10-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	157.06 99.33		
	1	T5 HIP	10.00 0.00	30-01-00	09-00-14	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	151.60 95.83		
	4	T6S PIGGYBACK	10.00 0.00	30-01-00	08-02-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	585.88 372.68		
	1	T7 HIP GIRDER	10.00 0.00	27-03-00	04-10-07	2 X 6 2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	153.72 94.67		
	1	T8 HIP	10.00 0.00	27-03-00	06-06-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	125.99 79.33		
	2	T9A PIGGYBACK	10.00 0.00	24-04-00	08-02-07	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 04-00-14	231.82 146.00		
	2	T9X HIP	10.00 0.00	28-03-00	08-02-07	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 00-09-11	267.36 166.66		
	1 2 Ply	T10X PIGGYBACK	10.00 0.00	28-03-00	08-02-07	2 X 4 2 X 6	01-03-08 00-00-00	01-07-11 00-09-11	302.80 189.34		
	1 2 Ply	T10XZ PIGGYBACK	10.00 0.00	28-03-00	08-02-07	2 X 4 2 X 6	01-03-08 00-00-00	01-07-11 00-09-11	302.80 189.34		
	3	T12 PIGGYBACK	10.00 0.00	28-03-00	08-02-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	389.91 246.00		
	1	T14A HIP GIRDER	10.00 0.00	17-03-00	04-10-07	2 X 4 2 X 6	00-00-00 00-00-00	01-07-11 03-04-08	89.84 58.33		
	1 2 Ply	T15 HIP GIRDER	10.00 0.00	13-06-00	03-02-00	2 X 4 2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	147.04 97.66		
	1	T16A HIP GIRDER	10.00 0.00	09-09-00	04-10-07	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 03-04-08	48.77 32.00		
	2	T17A COMMON	10.00 0.00	09-09-00	06-06-14	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 03-04-08	97.02 62.34		
	1	P1 PIGGYBACK	10.00 0.00	10-02-09	01-06-06	2 X 4 2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	34.25 22.17		



Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272847 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS SUB-BUILDER:
 MODEL: S39-2 CORNER ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	P2	10.00	10-02-09	03-02-06	2 X 4	2 X 4	00-00-00	00-04-13	70.14		
	2 Ply	PIGGYBACK	0.00							47.00		
	2	P3	10.00	10-02-09	04-07-14	2 X 4	2 X 4	00-00-00	00-04-13	63.92		
		PIGGYBACK	0.00							40.00		
	1	P3Z	10.00	10-02-09	04-07-14	2 X 4	2 X 4	00-00-00	00-04-13	63.92		
	2 Ply	PIGGYBACK	0.00							40.00		
	1	P4	10.00	11-02-09	02-10-03	2 X 4	2 X 4	00-00-00	00-04-13	37.61		
		PIGGYBACK	0.00							24.67		
	1	P5	10.00	11-02-09	04-06-03	2 X 4	2 X 4	00-00-00	00-04-13	40.57		
		PIGGYBACK	0.00							26.50		
	4	P6	10.00	11-02-09	05-00-14	2 X 4	2 X 4	00-00-00	00-04-13	139.72		
		PIGGYBACK	0.00							90.00		
	1	P6C	10.00	11-02-09	04-10-06	2 X 4	2 X 4	00-00-00	00-04-13	34.62		
		PIGGYBACK	0.00							21.17		
	21	J1	10.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08	01-07-11	319.83		
		JACK-OPEN	0.00							203.07		
	5	J2	10.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08	01-07-11	53.25		
		JACK-OPEN	0.00							36.65		
	5	J3	10.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08	01-07-11	41.85		
		JACK-OPEN	0.00							30.00		
	6	J4	10.00	01-10-00	03-02-00	2 X 4	2 X 4	01-03-08	01-07-11	59.22		
		JACK-OPEN	0.00							42.00		
	7	J5	4.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08	00-03-15	131.95		
		JACK-OPEN	0.00							85.19		

TOTAL # TRUSS= 86.00

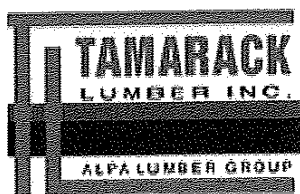
TOTAL BFT OF ALL TRUSSES=

2918.09 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4592.29 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
6	Hangers	LJS26DS	
5	Hangers	LUS24	
4	Hangers	HGUS26-2	

TOTAL # ITEMS= 15.00



Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272848 LOCATION:
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS SUB-BUILDER:
 MODEL: S39-2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T1 HIP GIRDER	10.00 0.00	30-01-00	04-10-07	2 X 6 2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	174.05 106.00		
	1	T2 HIP	10.00 0.00	30-01-00	06-06-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	133.99 84.83		
	1	T3 HIP	10.00 0.00	30-01-00	08-02-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.79 89.33		
	1	T4 HIP	10.00 0.00	30-01-00	09-10-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	157.06 99.33		
	1	T5 HIP	10.00 0.00	30-01-00	09-00-14	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	151.60 95.83		
	4	T6S PIGGYBACK	10.00 0.00	30-01-00	08-02-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	585.88 372.68		
	1	T7 HIP GIRDER	10.00 0.00	27-03-00	04-10-07	2 X 6 2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	153.72 94.67		
	1	T8 HIP	10.00 0.00	27-03-00	06-06-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	125.99 79.33		
	6	T9A PIGGYBACK	10.00 0.00	24-04-00	08-02-07	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 04-00-14	695.46 438.00		
	1	T12 PIGGYBACK	10.00 0.00	28-03-00	08-02-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	136.26 85.34		
	1	T14A HIP GIRDER	10.00 0.00	17-03-00	04-10-07	2 X 4 2 X 6	00-00-00 00-00-00	01-07-11 03-04-08	89.84 58.33		
	1 2 Ply	T150 HALF HIP	10.00 0.00	13-06-00	03-02-00	2 X 4 2 X 6	00-00-00 00-00-00	01-07-11 03-02-00	136.34 90.00		
	4	T17A COMMON	10.00 0.00	09-09-00	06-06-14	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 03-04-08	194.04 124.68		
	1	G17A COMMON	10.00 0.00	09-09-00	06-06-14	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 03-04-08	51.08 33.17		
	1 2 Ply	T18 HIP GIRDER	10.00 0.00	27-03-00	07-08-03	2 X 4 2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	287.24 180.34		
	1 2 Ply	T19 PIGGYBACK	10.00 0.00	28-03-00	08-02-07	2 X 4 2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	316.54 200.68		
	1 2 Ply	T20A COMMON	10.00 0.00	13-02-08	07-09-00	2 X 4 2 X 6	00-00-00 00-00-00	01-10-03 02-07-12	152.80 100.34		
	2	T21 COMMON	10.00 0.00	11-00-00	06-01-00	2 X 4 2 X 4	01-03-08 01-03-08	01-06-00 01-06-00	106.60 70.66		



Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067	LAYOUT ID: 272848	LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORE	SUB-BUILDER:	
MODEL: S39-2	ELEVATION: B	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	T22	0.00	03-08-00	01-08-00	2 X 4 2 X 4	00-00-00	01-08-00	51.64		
	2 Ply	FLAT GIRDER	0.00				00-00-00	01-08-00	37.32		
	3	P3	10.00	10-02-09	04-07-14	2 X 4 2 X 4	00-00-00	00-04-13	95.88		
		PIGGYBACK	0.00				00-00-00	00-04-13	60.00		
	1	P5Z	10.00	11-02-09	04-06-03	2 X 4 2 X 4	00-00-00	00-04-13	81.14		
	2 Ply	PIGGYBACK	0.00				00-00-00	00-04-13	53.00		
	4	P6	10.00	11-02-09	05-00-14	2 X 4 2 X 4	00-00-00	00-04-13	139.72		
		PIGGYBACK	0.00				00-00-00	00-04-13	90.00		
	1	P6C	10.00	11-02-09	04-10-06	2 X 4 2 X 4	00-00-00	00-04-13	34.62		
		PIGGYBACK	0.00				00-00-00	00-04-13	21.17		
	1	P7	10.00	10-02-09	01-00-02	2 X 4 2 X 4	00-00-00	00-04-13	33.40		
		PIGGYBACK	0.00				00-00-00	00-04-13	22.50		
	1	P8	10.00	10-02-09	02-08-02	2 X 4 2 X 4	00-00-00	00-04-13	34.42		
		PIGGYBACK	0.00				00-00-00	00-04-13	22.83		
	1	P9	10.00	10-02-09	04-04-02	2 X 4 2 X 4	00-00-00	00-04-13	37.65		
		PIGGYBACK	0.00				00-00-00	00-04-13	23.33		
	18	J1	10.00	03-10-08	04-10-07	2 X 4 2 X 4	01-03-08	01-07-11	274.14		
		JACK-OPEN	0.00				00-00-00	04-10-07	174.06		
	4	J2	10.00	03-10-08	03-01-09	2 X 4 2 X 4	01-03-08	01-07-11	42.60		
		JACK-OPEN	0.00				-02-01-01	00-03-08	29.32		
	4	J3	10.00	01-10-08	03-01-09	2 X 4 2 X 4	01-03-08	01-07-11	33.48		
		JACK-OPEN	0.00				-00-01-01	00-03-08	24.00		
	7	J4	10.00	01-10-00	03-02-00	2 X 4 2 X 4	01-03-08	01-07-11	69.09		
		JACK-OPEN	0.00				00-00-00	03-02-00	49.00		

TOTAL # TRUSS= 84.00

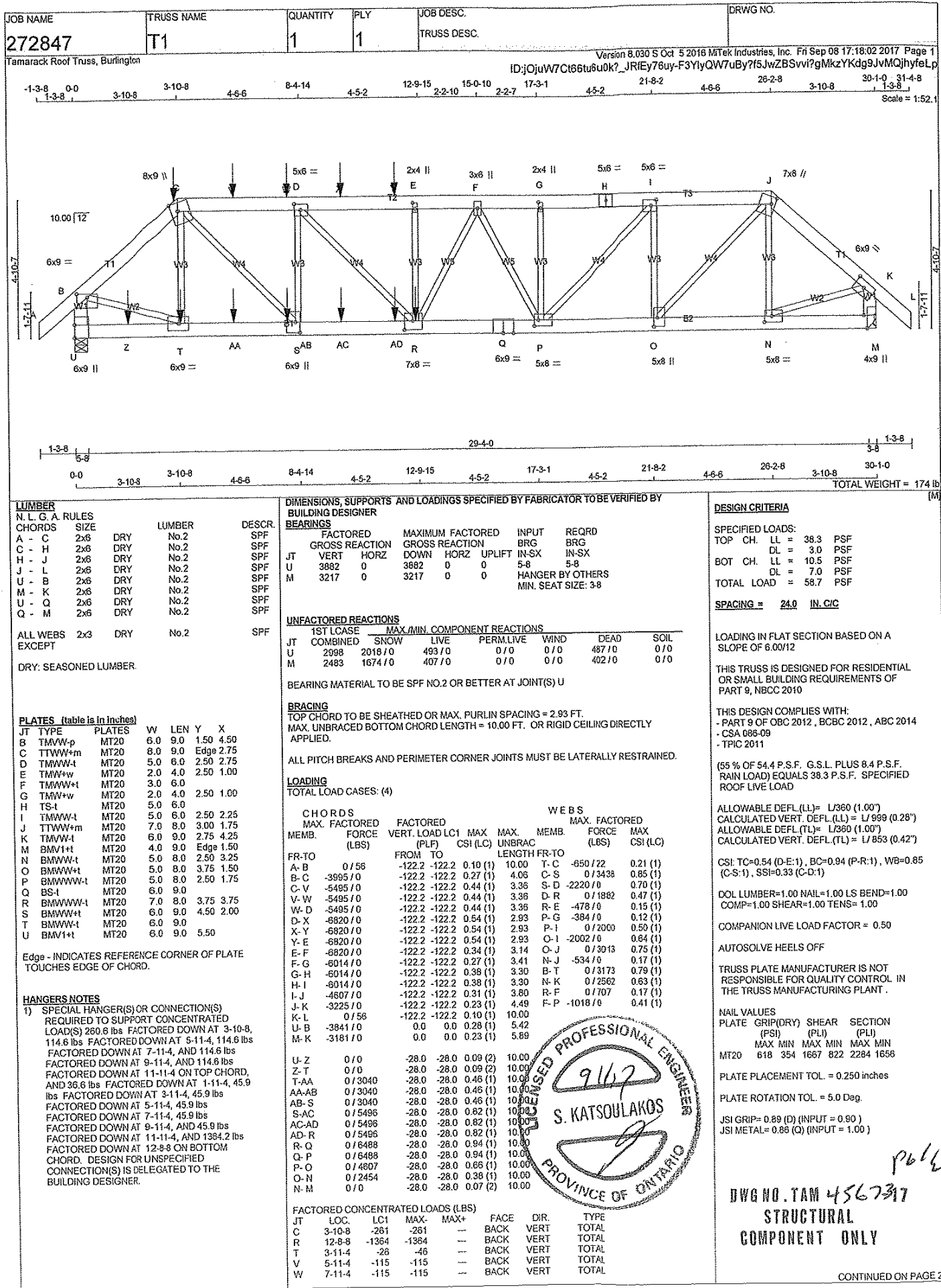
TOTAL BFT OF ALL TRUSSES=

3010.07 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4718.06 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
10	Hangers	LJS26DS	
5	Hangers	LUS24	
2	Hangers	LUS26-2	
4	Hangers	HGUS26-2	

TOTAL # ITEMS= 21.00



JOB NAME 272847	TRUSS NAME T1	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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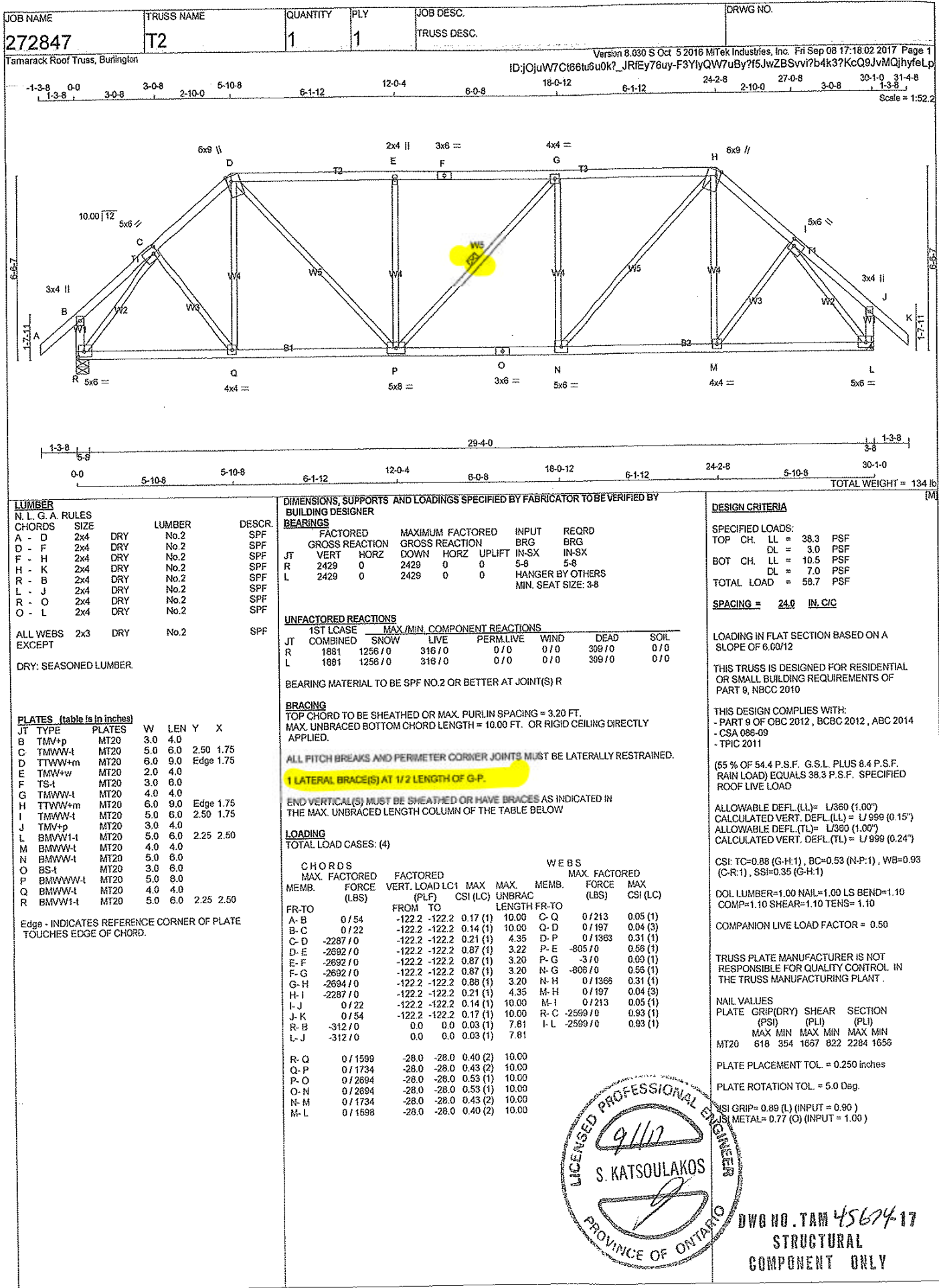
Tamarack Roof Truss, Burlington

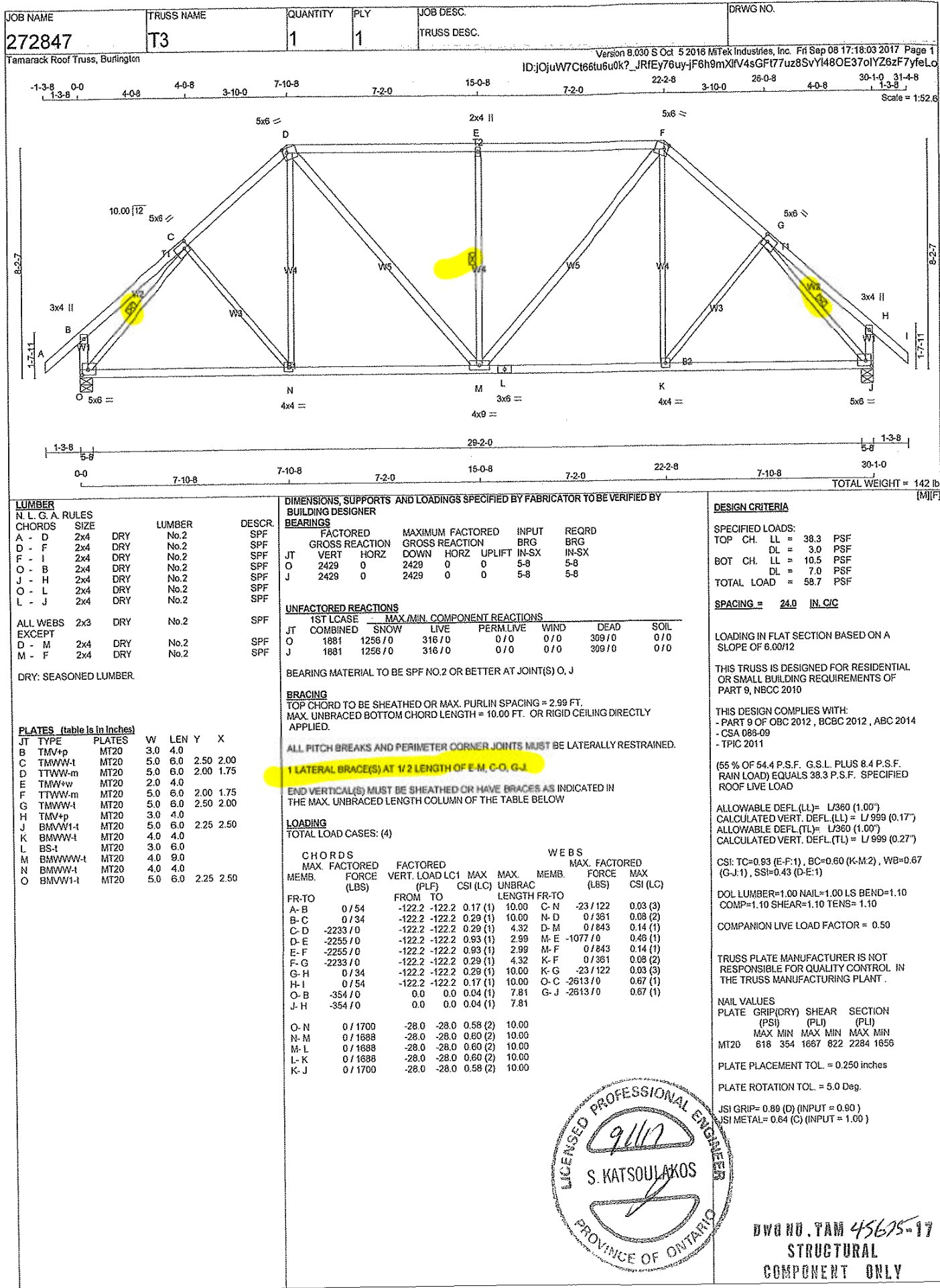
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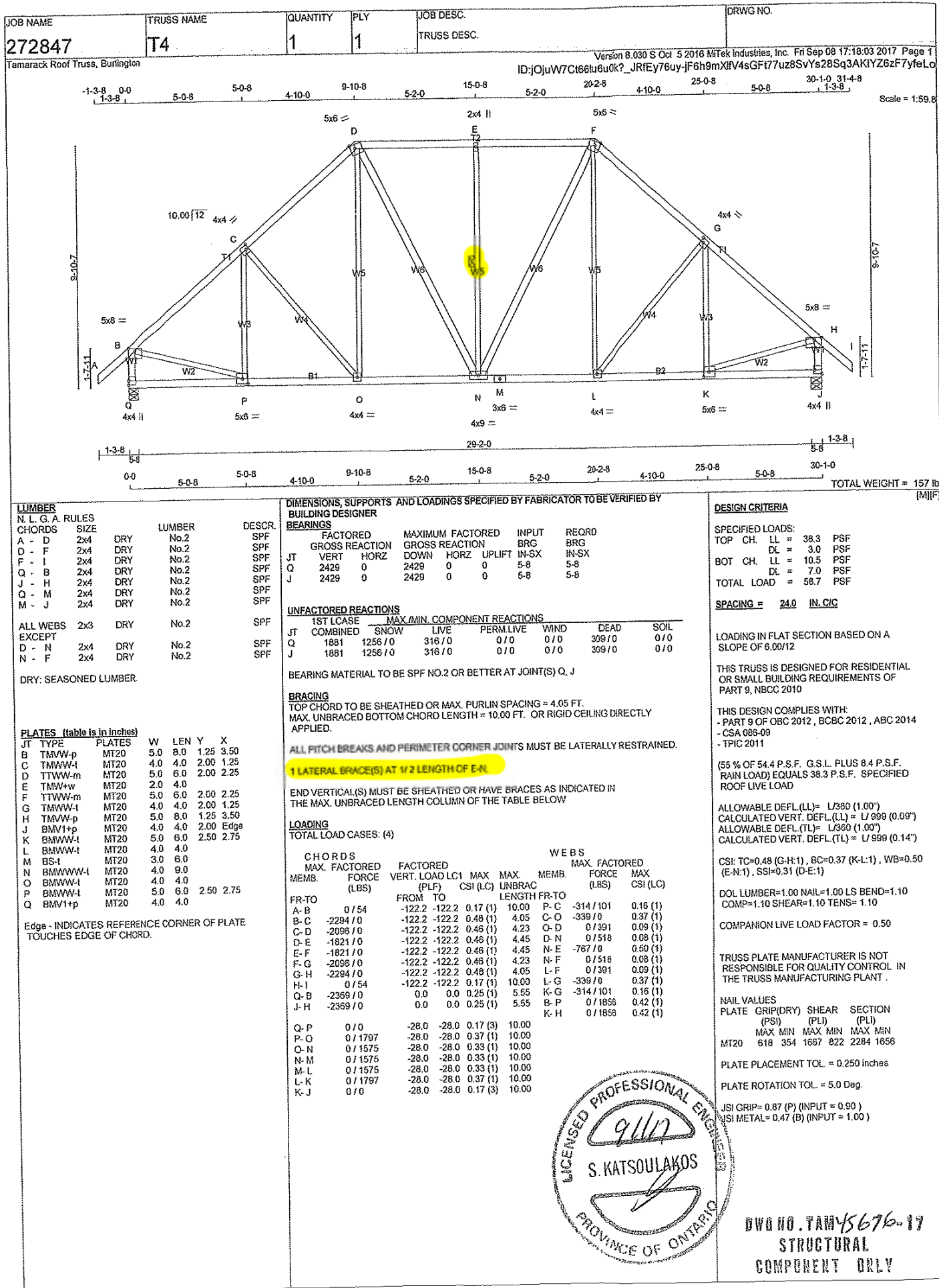
FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	9-11-4	-115	-115	—	BACK	VERT	TOTAL
Y	11-11-4	-115	-115	—	BACK	VERT	TOTAL
Z	1-11-4	-14	-37	—	BACK	VERT	TOTAL
AA	5-11-4	-26	-46	—	BACK	VERT	TOTAL
AB	7-11-4	-26	-46	—	BACK	VERT	TOTAL
AC	9-11-4	-28	-46	—	BACK	VERT	TOTAL
AD	11-11-4	-26	-46	—	BACK	VERT	TOTAL

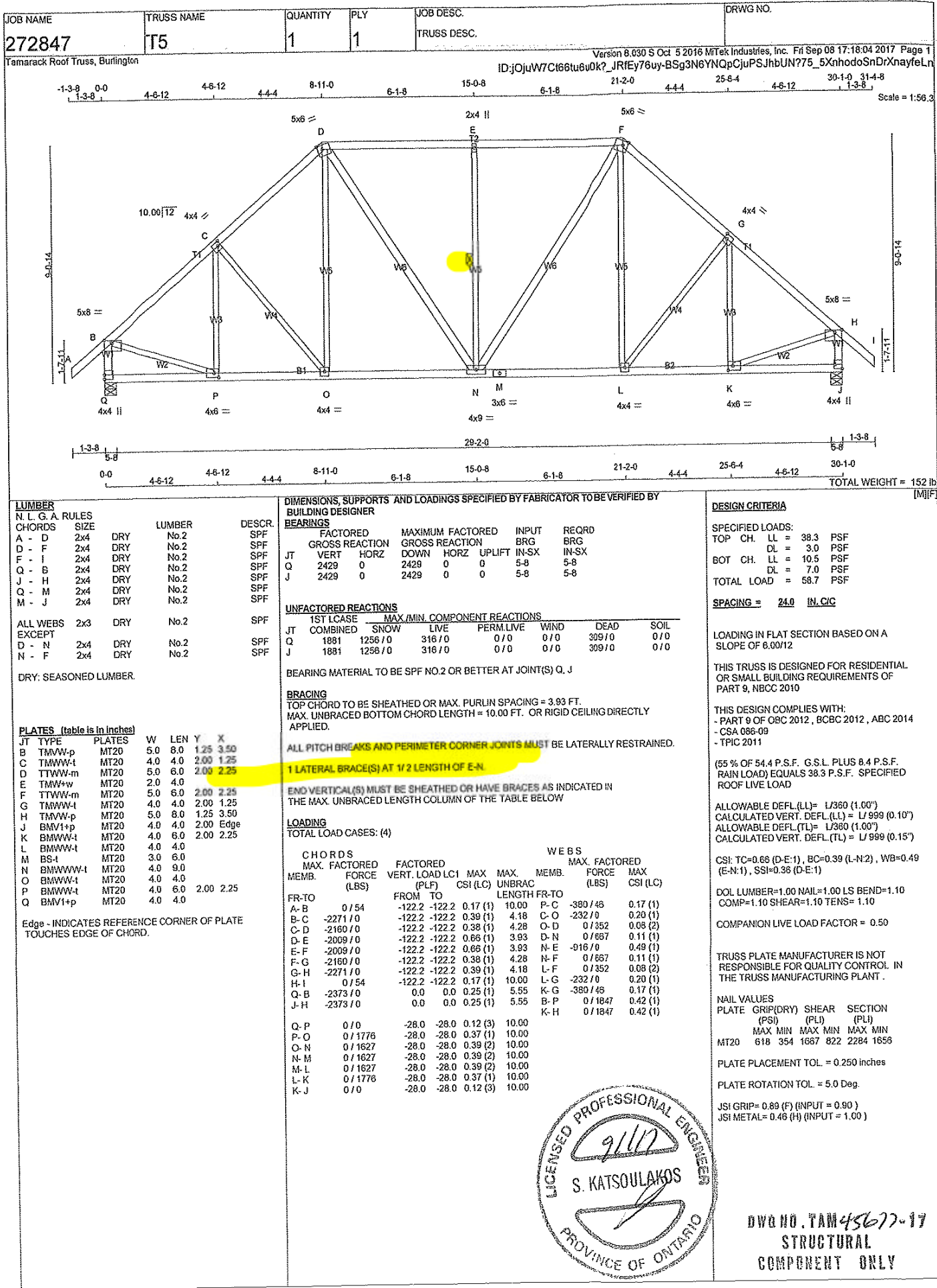


DWG NO. TAM 45673-17
 STRUCTURAL
 COMPONENT ONLY









JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
272847	T6S	4	1			

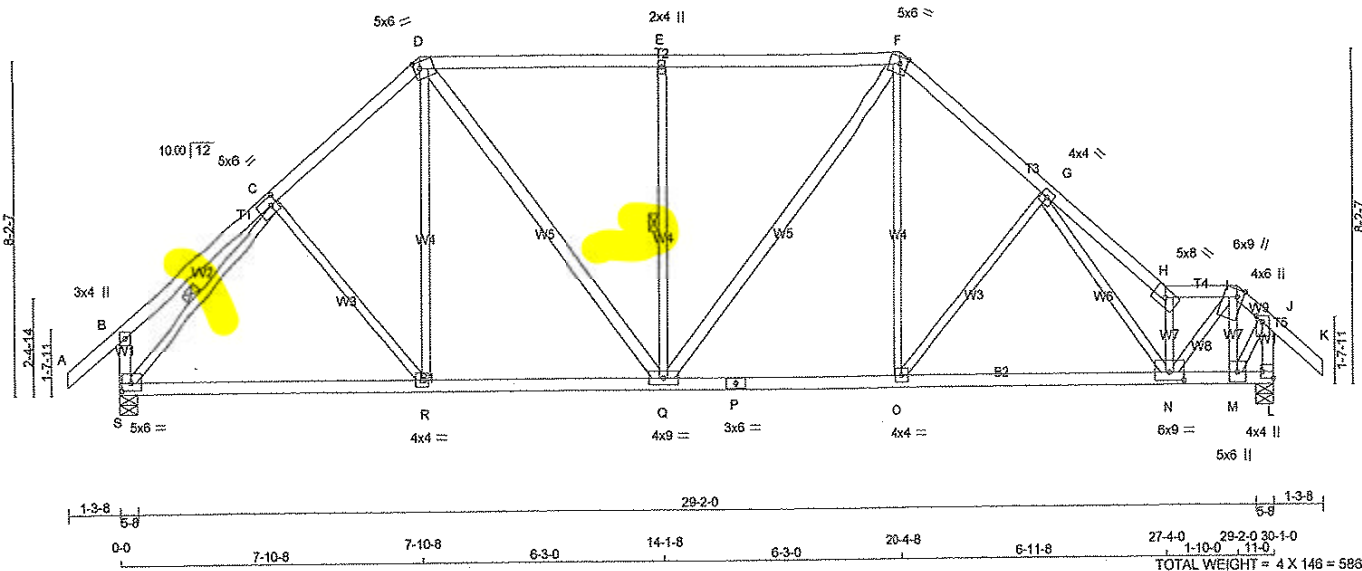
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M/Tek Industries, Inc. Fri Sep 08 17:18:04 2017 Page 1

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1-3-8 0-0 4-1-0 4-1-0 3-9-8 7-10-8 6-3-0 14-1-8 6-3-0 20-4-8 3-9-8 24-2-0 3-2-0 27-4-0 29-2-0 30-1-0 31-4-8 1-10-0 1-1-0 1-3-8

Scale = 1:54.0



TOTAL WEIGHT = 4 X 146 = 586 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - Q	2x4	DRY	No.2
Q - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-t	MT20	5.0	6.0 2.50 2.00
D	TTWW-m	MT20	5.0	6.0 2.00 1.75
E	TMW+w	MT20	2.0	4.0
F	TTWW-m	MT20	5.0	6.0 2.00 2.25
G	TMWW-t	MT20	4.0	4.0
H	TTW-h	MT20	5.0	8.0
I	TTWW+m	MT20	6.0	9.0 Edge 1.75
J	TMVW+p	MT20	4.0	6.0 1.75 1.75
L	BMV1+p	MT20	4.0	4.0 2.00 Edge
M	BMWW-t	MT20	5.0	6.0
N	BMWW-t	MT20	6.0	9.0 2.50 4.50
O	BMWW-t	MT20	4.0	4.0
P	BS-t	MT20	3.0	6.0
Q	BMWW-t	MT20	4.0	9.0
R	BMWW-t	MT20	4.0	4.0
S	BMWW-t	MT20	5.0	6.0 2.25 2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG
L	2433	0	2433	0	0	5-8	5-8	5-8	5-8
S	2425	0	2425	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE
L	1884	1259 / 0	316 / 0	0 / 0	0 / 0	309 / 0	0 / 0	L	1884	1259 / 0	316 / 0
S	1879	1254 / 0	316 / 0	0 / 0	0 / 0	309 / 0	0 / 0	S	1879	1254 / 0	316 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, C-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	CSI (LBS)	MAX. UNBRAC	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	CSI (LBS)	MAX. UNBRAC
FR-TO		FROM	TO		LENGTH	FR-TO		FROM	TO		LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	R-D	0 / 346	0.08 (2)			
B-C	0 / 34	-122.2	-122.2	0.29 (1)	10.00	D-Q	0 / 853	0.14 (1)			
C-D	-2227 / 0	-122.2	-122.2	0.29 (1)	4.32	Q-E	-934 / 0	0.40 (1)			
D-E	-2212 / 0	-122.2	-122.2	0.70 (1)	3.69	Q-F	0 / 518	0.08 (1)			
E-F	-2212 / 0	-122.2	-122.2	0.70 (1)	3.69	O-F	0 / 685	0.15 (1)			
F-G	-2488 / 0	-122.2	-122.2	0.30 (1)	4.13	N-H	-2452 / 0	0.38 (1)			
G-H	-3438 / 0	-122.2	-122.2	0.34 (1)	3.52	M-I	-1179 / 0	0.18 (1)			
H-I	-2559 / 0	-122.2	-122.2	0.12 (1)	4.24	M-J	0 / 1593	0.36 (1)			
I-J	-1377 / 0	-122.2	-122.2	0.14 (1)	5.40	N-I	0 / 2567	0.58 (1)			
J-K	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C-R	-28 / 120	0.03 (3)			
S-B	-357 / 0	0.0	0.0	0.04 (1)	7.81	S-C	-2605 / 0	0.67 (1)			
L-J	-2449 / 0	0.0	0.0	0.26 (1)	5.47	O-G	-624 / 0	0.41 (1)			
						G-N	0 / 688	0.15 (1)			
S-R	0 / 1698	-28.0	-28.0	0.56 (2)	10.00						
R-Q	0 / 1693	-28.0	-28.0	0.57 (2)	10.00						
Q-P	0 / 1891	-28.0	-28.0	0.49 (2)	10.00						
P-O	0 / 1891	-28.0	-28.0	0.49 (2)	10.00						
O-N	0 / 2293	-28.0	-28.0	0.55 (1)	10.00						
N-M	0 / 924	-28.0	-28.0	0.26 (2)	10.00						
M-L	0 / 0	-28.0	-28.0	0.04 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (1.00")
CALCULATED VERT. DEFL (LL) = L/999 (0.18")
ALLOWABLE DEFL (TL) = L/360 (1.00")
CALCULATED VERT. DEFL (TL) = L/999 (0.29")

CSI: TC=0.70 (D-E-1), BC=0.57 (Q-R-2), WB=0.67 (C-S-1), SS=0.37 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

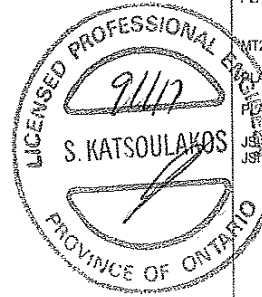
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

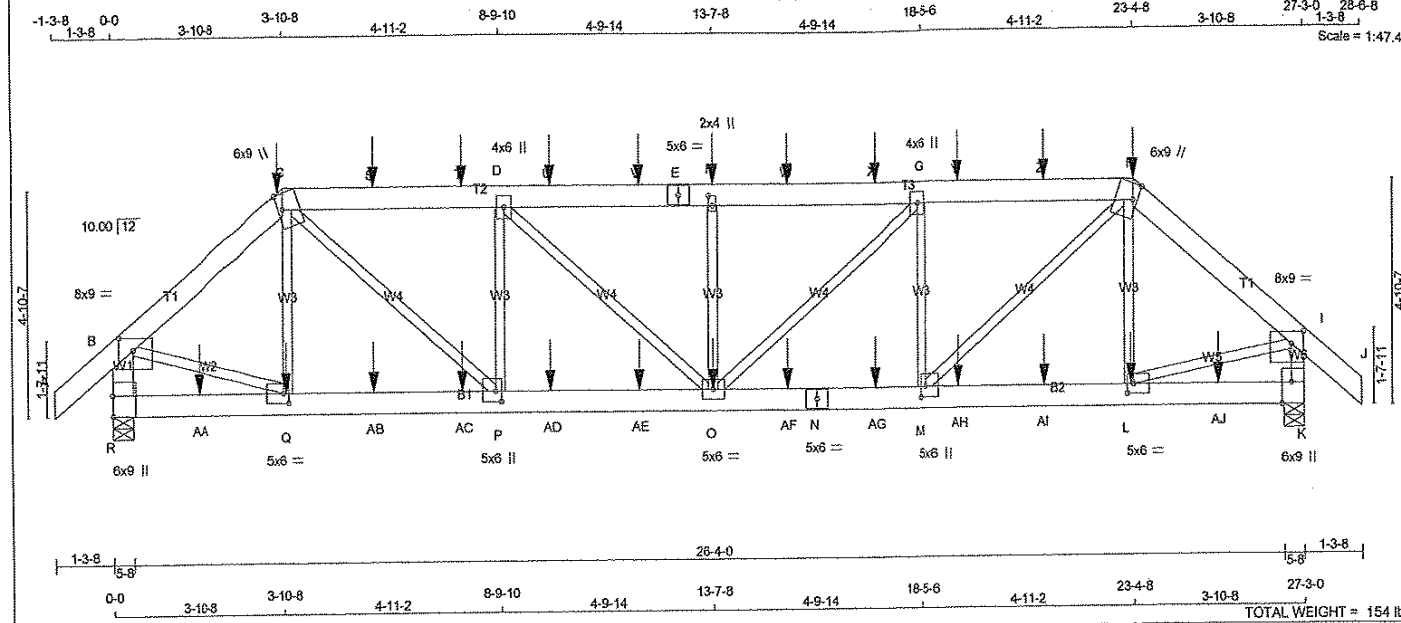
JST GRIP= 0.90 (N) (INPUT = 0.90)
JST METAL= 0.74 (H) (INPUT = 1.00)



DWG NO. TAM 4367817
STRUCTURAL
COMPONENT ONLY

JOB NAME 272847	TRUSS NAME T7	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington
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 ID: jOjuW7Ct66tu6u0k?_JRfEy76uy-BSg3N6YNQpCjuPSJhbUN7751KXjlobnSnDrXnayfeLn



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2 SPF
C - E	2x6	DRY	No.2 SPF
E - H	2x6	DRY	No.2 SPF
H - J	2x6	DRY	No.2 SPF
R - B	2x6	DRY	No.2 SPF
K - I	2x4	DRY	No.2 SPF
R - N	2x6	DRY	No.2 SPF
N - K	2x6	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWm	MT20	8.0	9.0	3.25	3.75
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D	TMVW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVW+w	MT20	2.0	4.0	2.50	1.00
G	TMVW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.00	1.25
I	TMVW-p	MT20	8.0	9.0	3.00	3.25
K	BMV1+t	MT20	6.0	9.0	Edge 2.50	
L	BMVW-t	MT20	5.0	6.0	2.50	1.75
M	BMVW+t	MT20	5.0	6.0	2.75	1.50
N	BS-t	MT20	5.0	6.0		
O	BMVWVW-t	MT20	5.0	6.0		
P	BMVW+t	MT20	5.0	6.0	2.75	1.50
Q	BMVW-t	MT20	5.0	6.0	2.50	1.50
R	BMV1+p	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 280.6 lbs FACTORED DOWN AT 3-10-8, 114.6 lbs FACTORED DOWN AT 5-11-4, 114.6 lbs FACTORED DOWN AT 7-11-4, 114.6 lbs FACTORED DOWN AT 9-11-4, 114.6 lbs FACTORED DOWN AT 11-11-4, 114.6 lbs FACTORED DOWN AT 13-7-8, 114.6 lbs FACTORED DOWN AT 15-3-12, 114.6 lbs FACTORED DOWN AT 17-3-12, 114.6 lbs FACTORED DOWN AT 19-3-12, AND 114.6 lbs FACTORED DOWN AT 21-3-12, AND 260.6 lbs FACTORED DOWN AT 23-4-8 ON TOP CHORD, AND 36.6 lbs FACTORED DOWN AT 1-11-4, 45.9 lbs FACTORED DOWN AT 3-11-4, 45.9 lbs FACTORED DOWN AT 5-11-4, 45.9 lbs FACTORED DOWN AT 7-11-4, 45.9 lbs FACTORED DOWN AT 9-11-4, 45.9 lbs FACTORED DOWN AT 11-11-4, 45.9 lbs FACTORED DOWN AT 13-7-8, 45.9 lbs FACTORED DOWN AT 15-3-12, 45.9 lbs FACTORED DOWN AT 17-3-12, 45.9 lbs FACTORED DOWN AT 19-3-12, 45.9 lbs FACTORED DOWN AT 21-3-12, AND 45.9 lbs FACTORED DOWN AT 23-3-12, AND 36.6 lbs FACTORED DOWN AT 25-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
R	3157	0	0	5-8
K	3157	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
JT	2449	1630 / 0	415 / 0 0 / 0 404 / 0 0 / 0
R	2449	1630 / 0	415 / 0 0 / 0 404 / 0 0 / 0
K	2449	1630 / 0	415 / 0 0 / 0 404 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	WEBS	MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	CS1 (LC)	MEMB.	FORCE	
	(LBS)	(PLF)	CS1 (LC)		(LBS)	
FR-TO		FROM TO	LENGTH	FR-TO		
A-B	0 / 56	-122.2 -122.2	0.10 (1)	10.00	Q-C -463 / 94	0.15 (1)
B-C	-3150 / 0	-122.2 -122.2	0.23 (1)	4.53	C-P 0 / 2491	0.62 (1)
C-S	-4260 / 0	-122.2 -122.2	0.41 (1)	3.79	P-D -1463 / 0	0.46 (1)
S-T	-4260 / 0	-122.2 -122.2	0.41 (1)	3.79	D-O 0 / 762	0.19 (1)
T-D	-4260 / 0	-122.2 -122.2	0.41 (1)	3.79	O-F -337 / 0	0.27 (1)
D-U	-4823 / 0	-122.2 -122.2	0.45 (1)	3.55	O-G 0 / 762	0.19 (1)
U-V	-4823 / 0	-122.2 -122.2	0.45 (1)	3.55	M-G -1463 / 0	0.46 (1)
V-E	-4823 / 0	-122.2 -122.2	0.45 (1)	3.55	M-H 0 / 2493	0.62 (1)
E-F	-4823 / 0	-122.2 -122.2	0.45 (1)	3.55	L-H -484 / 93	0.15 (1)
F-W	-4823 / 0	-122.2 -122.2	0.45 (1)	3.55	B-Q 0 / 2502	0.62 (1)
W-X	-4823 / 0	-122.2 -122.2	0.45 (1)	3.55	L-I 0 / 2501	0.62 (1)
X-G	-4823 / 0	-122.2 -122.2	0.45 (1)	3.55		
G-Y	-4260 / 0	-122.2 -122.2	0.41 (1)	3.79		
Y-Z	-4260 / 0	-122.2 -122.2	0.41 (1)	3.79		
Z-H	-4260 / 0	-122.2 -122.2	0.41 (1)	3.79		
H-I	-3149 / 0	-122.2 -122.2	0.23 (1)	4.53		
I-J	0 / 56	-122.2 -122.2	0.10 (1)	10.00		
R-B	-3116 / 0	0.0	0.0	0.22 (1)	5.94	
K-I	-3115 / 0	0.0	0.0	0.35 (1)	4.82	
R-AA	0 / 0	-28.0 -28.0	0.10 (2)	10.00		
AA-Q	0 / 0	-28.0 -28.0	0.10 (2)	10.00		
Q-AB	0 / 2398	-28.0 -28.0	0.37 (1)	10.00		
AB-AC	0 / 2398	-28.0 -28.0	0.37 (1)	10.00		
AC-P	0 / 2398	-28.0 -28.0	0.37 (1)	10.00		
P-AD	0 / 4261	-28.0 -28.0	0.64 (1)	10.00		
AD-AE	0 / 4261	-28.0 -28.0	0.64 (1)	10.00		
AE-O	0 / 4261	-28.0 -28.0	0.64 (1)	10.00		
O-AF	0 / 4261	-28.0 -28.0	0.64 (1)	10.00		
AF-N	0 / 4261	-28.0 -28.0	0.64 (1)	10.00		
N-AG	0 / 4261	-28.0 -28.0	0.64 (1)	10.00		
AG-M	0 / 4261	-28.0 -28.0	0.64 (1)	10.00		
M-AH	0 / 2397	-28.0 -28.0	0.37 (1)	10.00		
AH-AI	0 / 2397	-28.0 -28.0	0.37 (1)	10.00		
AI-L	0 / 2397	-28.0 -28.0	0.37 (1)	10.00		
L-AJ	0 / 0	-28.0 -28.0	0.09 (2)	10.00		
AJ-K	0 / 0	-28.0 -28.0	0.09 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.3 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL)= L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL)= L/999 (0.27")

CSI: TC=0.45 (O-F:1), BC=0.64 (O-P:1), WB=0.62 (B-Q:1), SSI=0.35 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN
MT20	618 354	1697 822
		2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SLIP GRIP= 0.88 (Q) (INPUT = 0.90)
SLIP METAL= 0.73 (N) (INPUT = 1.00)

DWG NO. TAM45679-1
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE



DRWG NO. TAM45679-17
 STRUCTURAL
 COMPONENT ONLY
 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272847	T7	1	1	TRUSS DESC.	

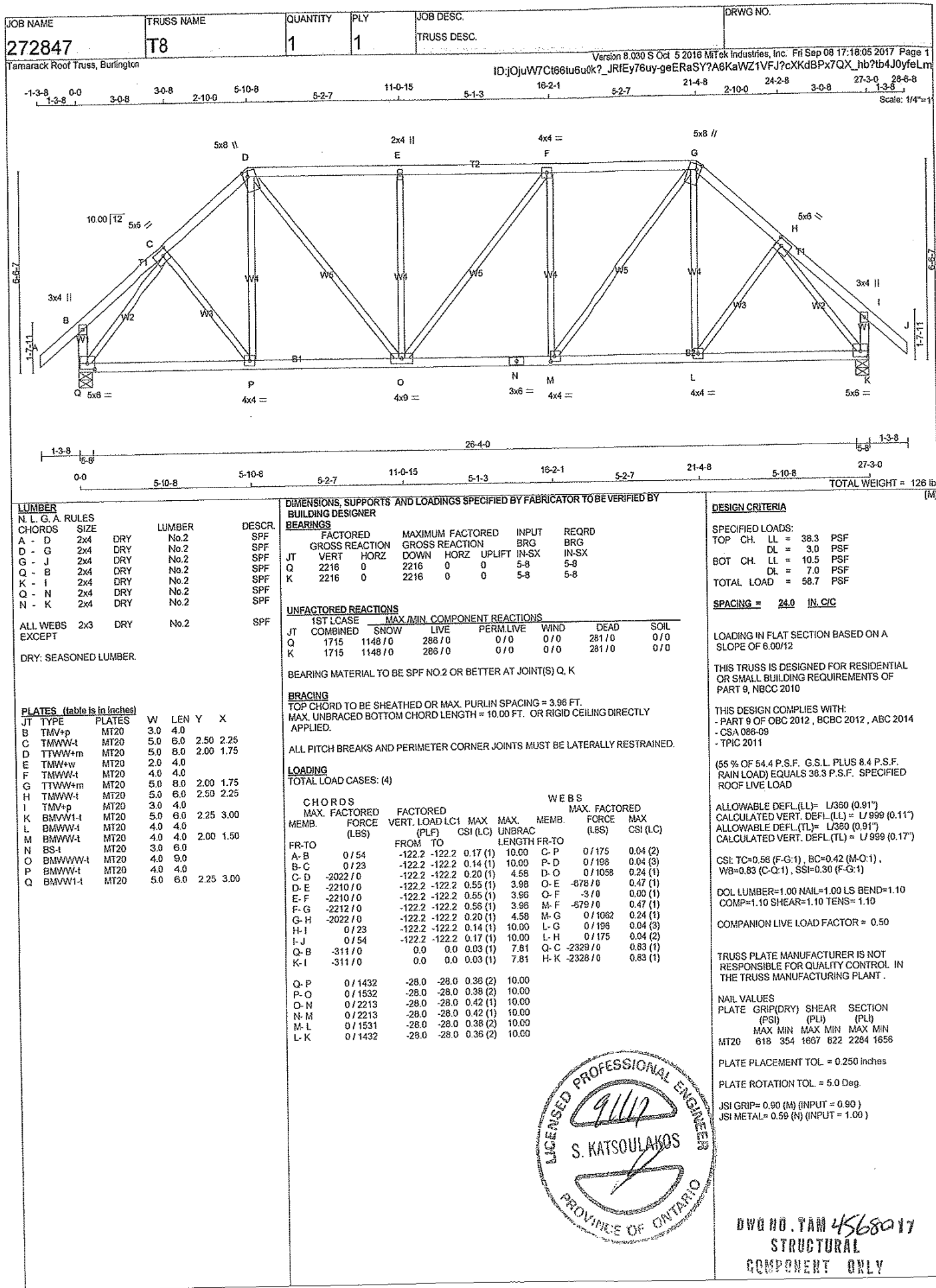
Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
C	3-10-8	-281	-281	—	FRONT	VERT
F	13-7-8	-115	-115	—	FRONT	VERT
H	23-4-8	-281	-281	—	FRONT	VERT
L	23-3-12	-26	-46	—	FRONT	VERT
O	13-7-8	-26	-46	—	FRONT	VERT
Q	3-11-4	-26	-46	—	FRONT	VERT
S	5-11-4	-115	-115	—	FRONT	VERT
T	7-11-4	-115	-115	—	FRONT	VERT
U	9-11-4	-115	-115	—	FRONT	VERT
V	11-11-4	-115	-115	—	FRONT	VERT
W	15-3-12	-115	-115	—	FRONT	VERT
X	17-3-12	-115	-115	—	FRONT	VERT
Y	19-3-12	-115	-115	—	FRONT	VERT
Z	21-3-12	-115	-115	—	FRONT	VERT
AA	1-11-4	-14	-37	—	FRONT	VERT
AB	5-11-4	-26	-46	—	FRONT	VERT
AC	7-11-4	-26	-46	—	FRONT	VERT
AD	9-11-4	-26	-46	—	FRONT	VERT
AE	11-11-4	-26	-46	—	FRONT	VERT
AF	15-3-12	-26	-46	—	FRONT	VERT
AG	17-3-12	-26	-46	—	FRONT	VERT
AH	19-3-12	-26	-46	—	FRONT	VERT
AI	21-3-12	-26	-46	—	FRONT	VERT
AJ	25-3-12	-14	-37	—	FRONT	VERT



DWG NO. TAM 43019-17
STRUCTURAL
COMPONENT ONLY



JOB NAME 272847 **TRUSS NAME** T9A **QUANTITY** 2 **PLY** 1 **JOB DESC.** TRUSS DESC.
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Fri Sep 08 17:18:05 2017 Page 1
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Scale = 1:47.0

N. L. G. A. RULES	LUMBER	DESCR.
A - D	2x4 DRY No.2	SPF
D - F	2x4 DRY No.2	SPF
F - G	2x4 DRY No.2	SPF
M - B	2x4 DRY No.2	SPF
H - G	2x4 DRY No.2	SPF
M - J	2x4 DRY No.2	SPF
J - H	2x4 DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
B TMV+p MT20	3.0	4.0		
C TMWW-t MT20	4.0	6.0	2.00	1.50
E TTWW+m MT20	5.0	6.0	2.25	1.50
D TMW+w MT20	2.0	4.0		
F TTWW+m MT20	5.0	6.0	2.25	1.50
G TMVV+p MT20	5.0	6.0	Edge	
H BMV1+p MT20	3.0	4.0		
I BMWW-t MT20	4.0	4.0	2.00	1.50
J BS-I MT20	3.0	6.0		
K BMWWW-l MT20	5.0	6.0		
L BMWW-l MT20	4.0	4.0		
M BMWW-l MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED DOWN	INPUT BRG IN-SX	REQD BRG IN-SX
JT VERT 1997	0	0	5-8
M 1997	0	0	5-8
H 1828	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

1ST CASE COMBINED	MAX/MIN SNOW LIVE PERM.LIVE WIND DEAD SOIL
JT 1543	1036 / 0 256 / 0 0 / 0 0 / 0 252 / 0 0 / 0
M 1429	930 / 0 256 / 0 0 / 0 0 / 0 243 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K, C-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C-L	-118 / 86	0.08 (1)	
B-C	0 / 35	-122.2	-122.2	0.30 (1)	10.00	L-D	0 / 382	0.09 (2)	
C-D	-1668 / 0	-122.2	-122.2	0.27 (1)	4.68	D-K	0 / 333	0.07 (1)	
D-E	-1451 / 0	-122.2	-122.2	0.55 (1)	4.71	K-E	-861 / 0	0.37 (1)	
E-F	-1451 / 0	-122.2	-122.2	0.55 (1)	4.71	K-F	0 / 890	0.20 (1)	
F-G	-1218 / 0	-122.2	-122.2	0.41 (1)	5.30	I-F	-512 / 38	0.67 (1)	
M-B	-353 / 0	0.0	0.0	0.04 (1)	7.81	M-C	-2044 / 0	0.52 (1)	
H-G	-1774 / 0	0.0	0.0	0.49 (1)	6.26	I-G	0 / 1161	0.26 (1)	
M-L	0 / 1330	-28.0	-28.0	0.52 (2)	10.00				
L-K	0 / 1255	-28.0	-28.0	0.52 (2)	10.00				
K-J	0 / 927	-28.0	-28.0	0.31 (2)	10.00				
J-I	0 / 927	-28.0	-28.0	0.31 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.21 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF DL = 3.0 PSF
BOT CH. LL = 10.5 PSF DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/999 (0.27")

CSI: TC=0.55 (E-F:1), BC=0.52 (K-L:2), WB=0.67 (F-I:1), SSI=0.34 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

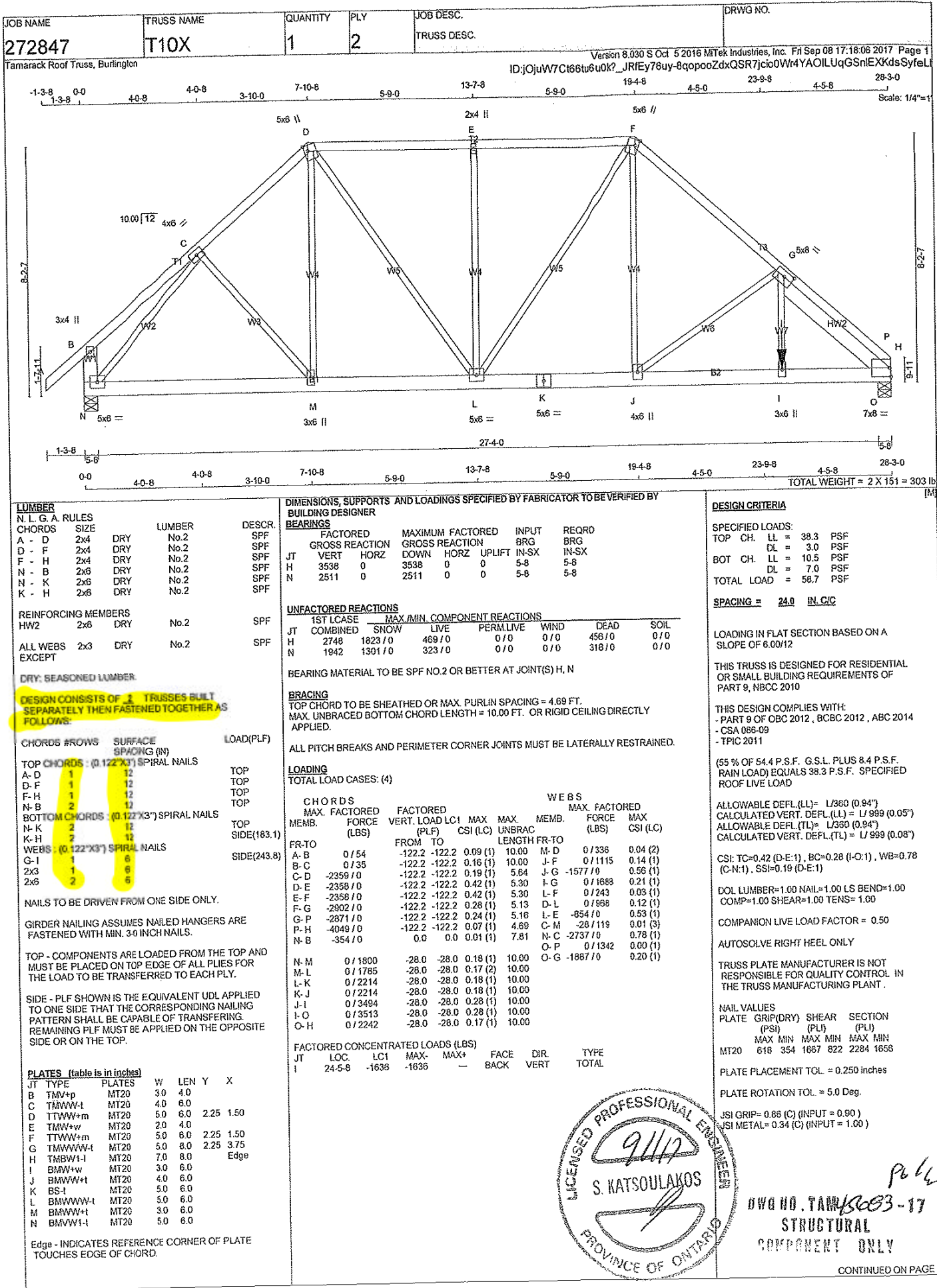
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

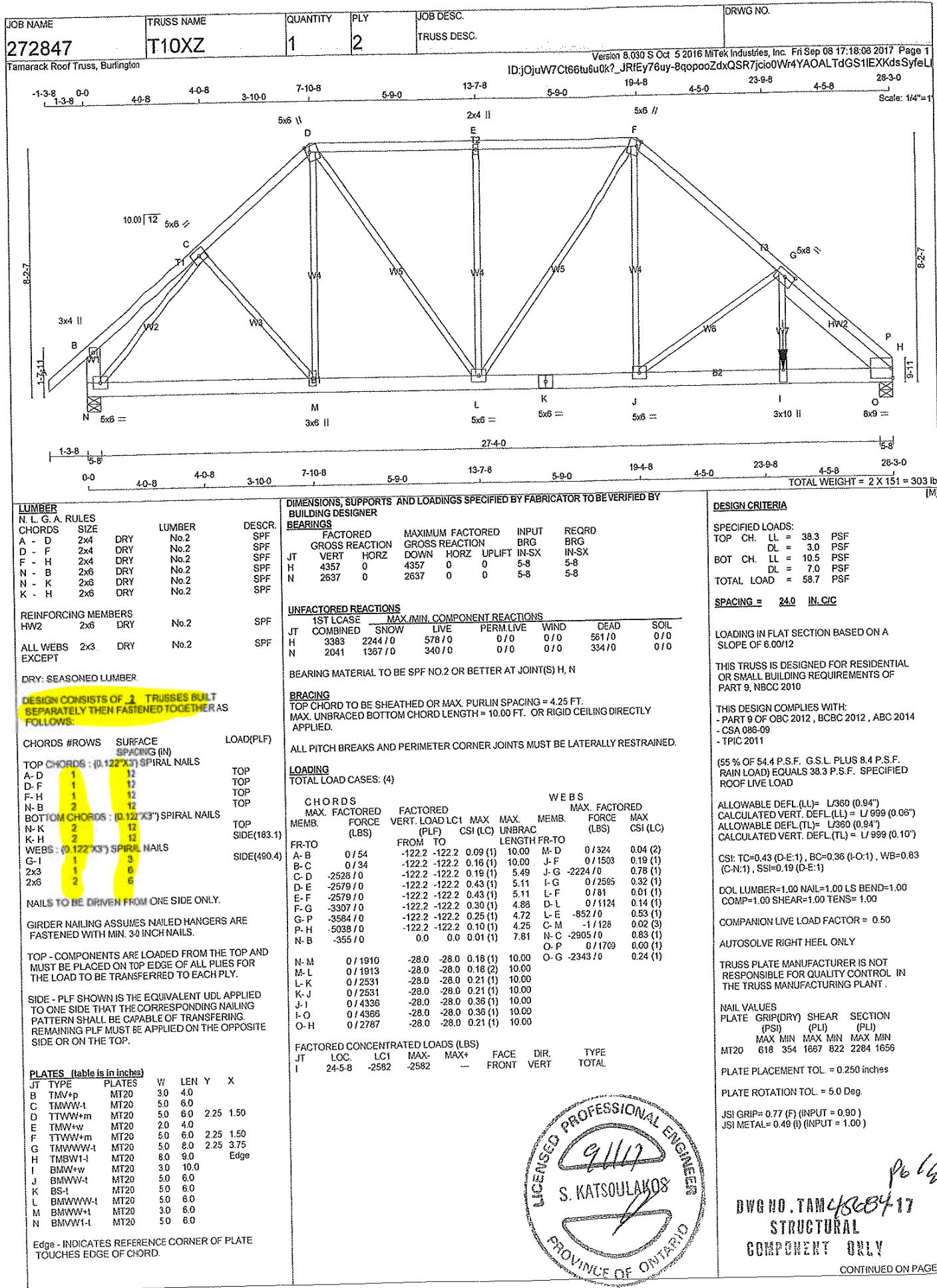
NAIL VALUES

PLATE GRIP(DRY)

[illegible]



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272847	T10X	1	2	TRUSS DESC.	
Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Fri Sep 08 17:18:06 2017 Page 2 Tamarack Roof Truss, Burlington ID: jOjuW7Ct66tu6u0k? JRFEy76uy-8qopooZdxQSR7jcl0Ww4YAOILUqGSnlEXKdsSylL					
HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1636.2 lbs FACTORED DOWN AT 24-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.					
 DWG NO. TAM 48083-17 STRUCTURAL COMPONENT ONLY					



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272847	T10XZ	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:JQiuW7Ct86tu6u0K7 JRfEV76uy-8qopooZdxQSR7jcl0Wt4YAOALtdGS1iEXKdsSyfeL

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2581.6 lbs FACTORED DOWN AT 24-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM45004-17
STRUCTURAL
COMPONENT ONLY

JOB NAME

272847

Tamarack Roof Truss, Burlington

TRUSS NAME

T12

QUANTITY

3

PLY

1

JOB DESC.

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ID:jOjuW7Ct66tu6u0K?_JRfEy76uy-8qopooZdxQSR7jcio0VW4YAKELQOGVGIEKKdsSyfeL

DRWG NO.

Scale = 1:49.2

LUMBER

N.L.G.A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF

ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN Y	X
B TMV+p	MT20	3.0	4.0	
C TMMW-l	MT20	5.0	6.0	2.50 2.25
D TTWW+m	MT20	5.0	6.0	2.00 1.75
E TMV+w	MT20	2.0	4.0	
F TTWW+m	MT20	5.0	6.0	2.00 1.75
G TMMW-l	MT20	5.0	6.0	2.50 2.25
H TMV+p	MT20	3.0	4.0	
J BMVW-l	MT20	5.0	6.0	2.25 3.00
K BMW-l	MT20	4.0	4.0	
L BS-l	MT20	3.0	6.0	
M BMWWW-l	MT20	4.0	6.0	
N BMW-l	MT20	4.0	4.0	
O BMVW-l	MT20	5.0	6.0	2.25 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORIZ		
O	2291	0	2291	0	5-8	5-8
J	2291	0	2291	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX / MIN COMPONENT REACTIONS		WIND PERM LIVE	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE			
O	1774	1186 / 0	297 / 0	0 / 0	0 / 0	291 / 0	0 / 0
J	1774	1186 / 0	297 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, C-G, J-I

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		FROM	TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					
A-B	0 / 154	-122.2	-122.2	0.17 (1)	10.00
B-C	0 / 135	-122.2	-122.2	0.29 (1)	10.00
C-D	-2052 / 0	-122.2	-122.2	0.28 (1)	4.48
D-E	-1972 / 0	-122.2	-122.2	0.68 (1)	3.91
E-F	-1972 / 0	-122.2	-122.2	0.68 (1)	3.91
F-G	-2052 / 0	-122.2	-122.2	0.28 (1)	4.48
G-H	0 / 135	-122.2	-122.2	0.29 (1)	10.00
H-I	0 / 154	-122.2	-122.2	0.17 (1)	10.00
O-B	-354 / 0	0.0	0.0	0.04 (1)	7.81
J-H	-354 / 0	0.0	0.0	0.04 (1)	7.81
O-N	0 / 1582	-28.0	-28.0	0.56 (2)	10.00
N-M	0 / 1550	-28.0	-28.0	0.57 (2)	10.00
M-L	0 / 1550	-28.0	-28.0	0.57 (2)	10.00
L-K	0 / 1550	-28.0	-28.0	0.57 (2)	10.00
K-J	0 / 1582	-28.0	-28.0	0.56 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF DL = 3.0 PSF

BOT CH. LL = 10.5 PSF DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

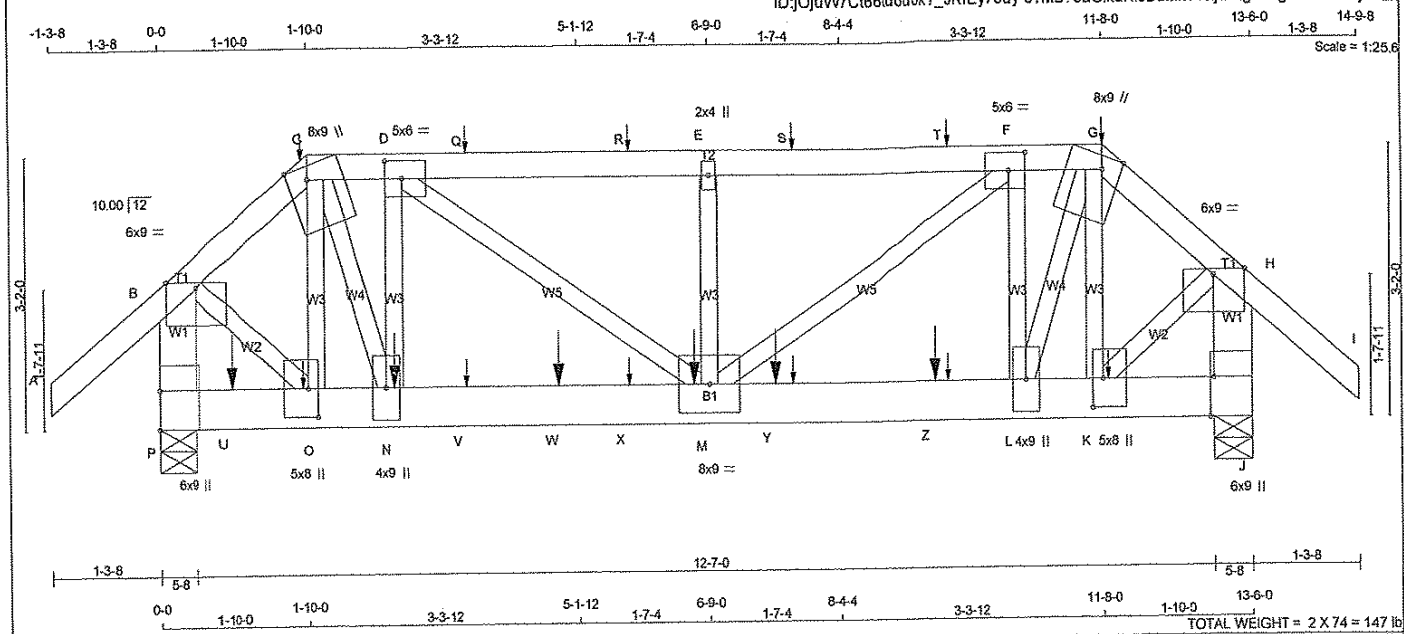
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED

[illegible]



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
P - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
P - J	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-G	1	12	SIDE(61.0)
G-I	1	12	SIDE(61.0)
P-B	2	12	TOP
J-H	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
P-J	2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS			
D-N	2	6	SIDE(648.8)
E-M	1	6	SIDE(0.6)
2x3	1	6	
F-L	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	6.0	9.0	Edge	
C TTTWV+m	MT20	8.0	9.0	Edge	2.75
D TMWV-t	MT20	5.0	6.0	2.50	2.50
E TMWV+w	MT20	2.0	4.0		
F TMWV-t	MT20	5.0	6.0	2.50	2.50
G TTTWV+m	MT20	8.0	9.0	Edge	2.75
H TMWV-p	MT20	6.0	9.0	Edge	
J BMV1+t	MT20	6.0	9.0	Edge	0.50
K BMWV+t	MT20	5.0	8.0	4.00	1.50
L BMWV+t	MT20	4.0	9.0		
M BMWVW-t	MT20	8.0	9.0		
N BMWVW+t	MT20	4.0	9.0		
O BMWVW-t	MT20	5.0	8.0	4.00	1.50
P BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					
BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT
P	8061	0	8061	0	0
J	4165	0	4165	0	0

UNFACTORED REACTIONS					
	1ST LCASE	MAX/MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
P	6184	4235 / 0	970 / 0	0 / 0	0 / 0
J	3190	2192 / 0	495 / 0	0 / 0	503 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING				TOTAL LOAD CASES: (7)			
CHORDS				WEBS			
	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. UNBRACED LENGTH (FT)		MAX. FACTORED MEMB. FORCE (LBS)	MAX. MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 54	-122.2 -122.2	0.09 (1)	10.00	O-C	-835 / 0	0.08 (1)
B-C	-6076 / 0	-122.2 -122.2	0.21 (1)	3.60	C-N	0 / 4831	0.60 (1)
C-D	-6307 / 0	-122.2 -122.2	0.14 (1)	3.81	N-D	-967 / 0	0.09 (1)
D-Q	-7399 / 0	-122.2 -122.2	0.33 (1)	3.37	L-F	-2728 / 0	0.26 (1)
Q-R	-7399 / 0	-122.2 -122.2	0.33 (1)	3.37	L-G	0 / 3809	0.47 (1)
R-E	-7399 / 0	-122.2 -122.2	0.33 (1)	3.37	K-G	-1403 / 0	0.13 (1)
E-S	-7399 / 0	-122.2 -122.2	0.31 (1)	3.39	B-O	0 / 5448	0.67 (1)
S-T	-7399 / 0	-122.2 -122.2	0.31 (1)	3.39	K-H	0 / 3191	0.39 (1)
T-F	-7399 / 0	-122.2 -122.2	0.31 (1)	3.39	M-E	-499 / 0	0.05 (1)
F-G	-4006 / 0	-122.2 -122.2	0.10 (1)	4.67	D-M	0 / 1349	0.17 (1)
G-H	-3576 / 0	-122.2 -122.2	0.13 (1)	4.85	M-F	0 / 4169	0.52 (1)
H-I	0 / 54	-122.2 -122.2	0.09 (1)	10.00			
P-B	-7147 / 0	0.0	0.0	2.25 (1)	5.59		
J-H	-4329 / 0	0.0	0.0	0.16 (1)	6.89		
P-U	0 / 0	-28.0 -28.0	0.30 (1)	10.00			
U-O	0 / 0	-28.0 -28.0	0.30 (1)	10.00			
C-N	0 / 1457	-28.0 -28.0	0.58 (1)	10.00			
N-V	0 / 6309	-28.0 -28.0	0.92 (1)	10.00			
V-W	0 / 6309	-28.0 -28.0	0.92 (1)	10.00			
W-X	0 / 6309	-28.0 -28.0	0.92 (1)	10.00			
X-M	0 / 6309	-28.0 -28.0	0.92 (1)	10.00			
M-Y	0 / 4007	-28.0 -28.0	0.48 (1)	10.00			
Y-Z	0 / 4007	-28.0 -28.0	0.48 (1)	10.00			
Z-L	0 / 4007	-28.0 -28.0	0.48 (1)	10.00			
L-K	0 / 2646	-28.0 -28.0	0.31 (1)	10.00			
K-J	0 / 0	-28.0 -28.0	0.14 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX.	FACE	DIR.
C	11-10-0	---	---	175	FRONT
G	11-8-0	---	---	175	FRONT
K	11-8-12	---	---	---	FRONT
M	6-6-12	-704	-704	---	BACK
N	2-10-12	-3189	-3189	---	BACK
O	1-9-4	---	---	---	FRONT
Q	3-9-4	---	---	175	FRONT
R	5-9-4	---	---	175	FRONT
S	7-9-4	---	---	175	FRONT
T	9-8-12	---	---	175	FRONT

DESIGN CRITERIA		
SPECIFIED LOADS:		
TOP CH.	LL	= 38.3 PSF
	DL	= 3.0 PSF
BOT CH.	LL	= 10.5 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.45")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (0.45")
CALCULATED VERT. DEFL. (TL) = L/999 (0.15")

CSI: TC=0.33 (D-E:1), BC=0.92 (M-N:1), WB=0.67 (B-O:1), SSI=0.51 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX MIN	MAX MIN	MAX MIN		
MT20	618 354	1667 822	2284 1858		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.60 (O) (INPUT = 1.00)

DWG NO. TAM 4807-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272847	T15	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 174.7 lbs FACTORED UP AT 1-10-0, 174.7 lbs FACTORED UP AT 3-9-4, 174.7 lbs FACTORED UP AT 5-9-4, 174.7 lbs FACTORED UP AT 7-9-4, AND 174.7 lbs FACTORED UP AT 9-8-12, AND 174.7 lbs FACTORED UP AT 11-8-0 ON TOP CHORD, AND 2402.1 lbs FACTORED DOWN AT 10-12, AT 1-9-4, 3188.8 lbs FACTORED DOWN AT 2-10-12, AT 3-9-4, 1836.0 lbs FACTORED DOWN AT 4-10-12, AT 5-9-4, 704.3 lbs FACTORED DOWN AT 6-6-12, 704.3 lbs FACTORED DOWN AT 7-6-12, AT 7-9-4, 1024.4 lbs FACTORED DOWN AT 9-6-12, AND AT 9-8-12, AND AT 11-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	10-12	-2402	-2402	---	BACK	VERT	TOTAL
V	3-9-4	---	---	---	FRONT	VERT	TOTAL
W	4-10-12	-1836	-1836	---	BACK	VERT	TOTAL
X	5-9-4	---	---	---	FRONT	VERT	TOTAL
Y	7-6-12	-704	-704	---	BACK	VERT	TOTAL
Y	7-9-4	---	---	---	FRONT	VERT	TOTAL
Z	9-6-12	-1024	-1024	---	BACK	VERT	TOTAL
Z	9-8-12	---	---	---	FRONT	VERT	TOTAL

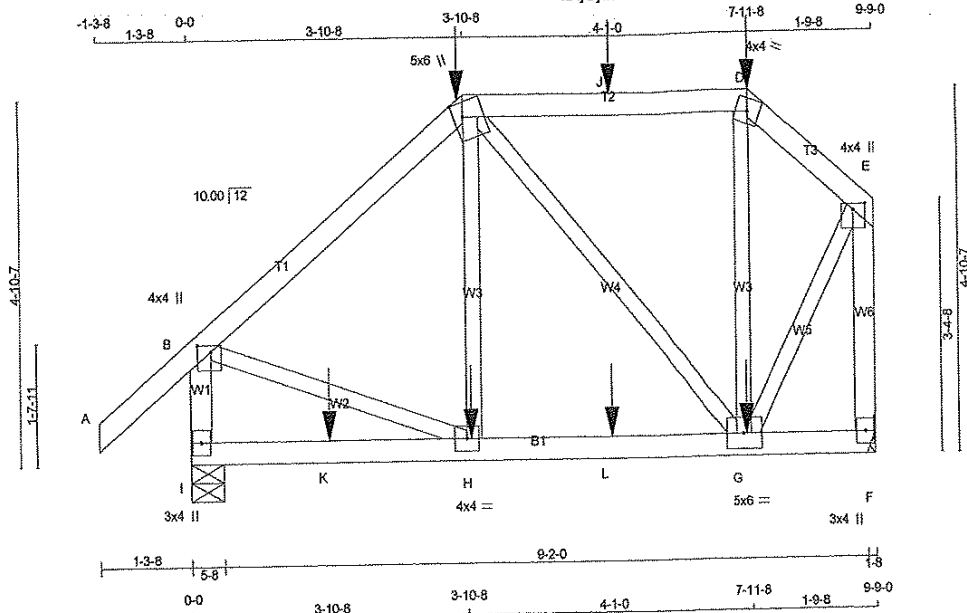


DWG NO. TAM 45607-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
272847	T16A	1	1			

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 49 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	5.0	6.0		
H	BMVWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 280.6 lbs FACTORED DOWN AT 3-10-8, AND 118.1 lbs FACTORED DOWN AT 5-11-0, AND 118.1 lbs FACTORED DOWN AT 7-11-8 ON TOP CHORD, AND 38.6 lbs FACTORED DOWN AT 1-11-4, 45.9 lbs FACTORED DOWN AT 3-11-4, AND 45.9 lbs FACTORED DOWN AT 5-11-0, AND 45.9 lbs FACTORED DOWN AT 7-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								
<u>BEARINGS</u>								
	FACTORED		MAXIMUM FACTORED			INPUT	REQD	
	GROSS	REACTION	GROSS	REACTION		BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
I	1168	0	1168	0	0	5-8	5-8	
F	1052	0	1052	0	0	HANGER BY OTHERS		
						MIN. SEAT SIZE: 1-8		

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN	COMPONENT REACTIONS	LIVE	PERM LIVE	WIND	DEAD	SOIL
I	898	613 / 0	141 / 0	0 / 0	0 / 0	143 / 0	0 / 0	0 / 0
F	820	537 / 0	143 / 0	0 / 0	0 / 0	140 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		FACTORED		MEMB.		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	LC1 MAX	UNBRAC	FORCE (LBS)	MAX	CS1 (LC)	CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 54	-122.2	-122.2	0.19 (1)	10.00	H-C	-24 / 242	0.08 (3)	
B-C	-798 / 0	-122.2	-122.2	0.37 (1)	6.21	C-G	-357 / 0	0.22 (1)	
C-J	-375 / 0	-122.2	-122.2	0.56 (1)	6.25	G-D	-213 / 90	0.08 (1)	
J-D	-375 / 0	-122.2	-122.2	0.56 (1)	6.25	B-H	0 / 642	0.16 (1)	
D-E	-497 / 0	-122.2	-122.2	0.08 (1)	6.25	G-E	0 / 717	0.18 (1)	
I-B	-1118 / 0	0.0	0.0	0.13 (1)	7.48				
F-E	-1039 / 0	0.0	0.0	0.21 (1)	7.68				
I-K	0 / 0	-28.0	-28.0	0.16 (3)	10.00				
K-H	0 / 0	-28.0	-28.0	0.16 (3)	10.00				
H-L	0 / 609	-28.0	-28.0	0.22 (2)	10.00				
L-G	0 / 609	-28.0	-28.0	0.22 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.08 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE
C	3-10-8	-261	-261		BACK	VERT	TOTAL
D	7-11-8	-118	-118		BACK	VERT	TOTAL
G	7-10-12	-26	-46		BACK	VERT	TOTAL
H	3-11-4	-26	-46		BACK	VERT	TOTAL
J	5-11-0	-115	-115		BACK	VERT	TOTAL
K	1-11-4	-14	-37		BACK	VERT	TOTAL
L	5-11-0	-26	-46		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.32")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.32")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CS1: TC=0.56 (C-D:1), BC=0.22 (G-H:2), WB=0.22 (C-G:1), SS=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

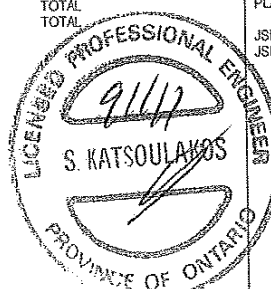
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)



DWG NO. TAM 4566617
STRUCTURAL
COMPONENT ONLY

JOB NAME

272847

TRUSS NAME

T17A

QUANTITY

2

PLY

1

JOB DESC.

TRUSS DESC.

DRWG NO.

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Fri Sep 08 17:18:08 2017 Page 1

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Tamarack Roof Truss, Burlington

Scale = 1:41.0

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMVV+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMVV1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT 732	DOWN 732	IN-SX	IN-SX
F	HORZ 0	HORZ 0	UPLIFT 0	UPLIFT 0
H	VERT 902	DOWN 902	HANGER BY OTHERS	HANGER BY OTHERS
			MIN. SEAT SIZE: 1-8	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
F	COMBINED	SNOW 373 / 0	LIVE 102 / 0	PERM. LIVE 0 / 0	0 / 0
H		SNOW 479 / 0	LIVE 102 / 0	PERM. LIVE 0 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 54	-122.2 -122.2 0.17 (1)	C-G	-201 / 20 0.08 (1)
B-C	0 / 27	-122.2 -122.2 0.17 (1)	G-D	0 / 191 0.04 (3)
C-D	-400 / 0	-122.2 -122.2 0.13 (1)	H-C	-870 / 0 0.24 (1)
D-E	-374 / 0	-122.2 -122.2 0.23 (1)	G-E	0 / 387 0.08 (1)
H-B	-306 / 0	0.0 0.0 0.03 (1)		
F-E	-701 / 0	0.0 0.0 0.14 (1)		
H-G	0 / 412	-28.0 -28.0 0.26 (2)		
G-F	0 / 0	-28.0 -28.0 0.22 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL)= L/999 (0.08")

CSI: TC=0.23 (D-E:1), BC=0.26 (G-H:2), WB=0.24 (C-H:1), SSI=0.14 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

9/1/17

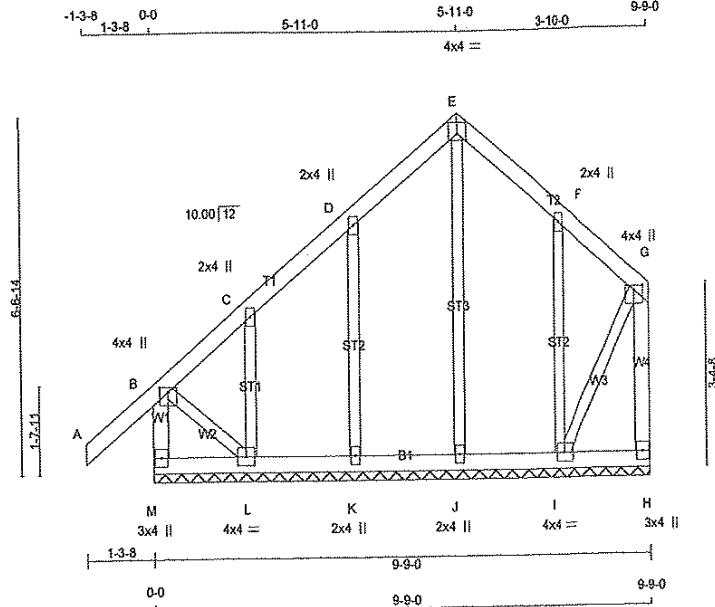
S. KATSOULAKOS

LICENSED PROFESSIONAL ENGINEER

PROVING OF ONTARIO

OWN NO. TAM 45608-17

STRUCTURAL COMPONENT ONLY



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
M - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.00 2.00
C, D, F				
C	TMW+w	MT20	2.0	4.0
E	TTW-p	MT20	4.0	4.0 1.50 2.00
G	TMVW+p	MT20	4.0	4.0 1.00 2.00
H	BMV1+p	MT20	3.0	4.0
I	BMW1-w	MT20	4.0	4.0
J	BMW1+w	MT20	2.0	4.0
K	BMW1-w	MT20	2.0	4.0
L	BMW1-w	MT20	4.0	4.0
M	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (LBS)	MAX. UNBRACED LENGTH (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LBS)	LC1 MAX (LBS)
FR-TO						FR-TO			
M-B	-372 / 0	0.0	0.0	0.04 (1)	7.81	J-E	-183 / 0	0.12 (1)	0.12 (1)
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	K-D	-293 / 0	0.10 (1)	0.10 (1)
B-C	-86 / 0	-122.2	-122.2	0.16 (1)	6.25	L-C	-153 / 0	0.03 (1)	0.03 (1)
C-D	-10 / 0	-122.2	-122.2	0.08 (1)	6.25	I-F	-286 / 0	0.10 (1)	0.10 (1)
D-E	-33 / 0	-122.2	-122.2	0.08 (1)	6.25	B-L	0 / 28	0.01 (1)	0.01 (1)
E-F	-32 / 0	-122.2	-122.2	0.07 (1)	6.25	I-G	0 / 35	0.01 (1)	0.01 (1)
F-G	-4 / 0	-122.2	-122.2	0.07 (1)	10.00				
H-G	-120 / 0	0.0	0.0	0.02 (1)	7.81				
M-L	0 / 0	-28.0	-28.0	0.03 (2)	10.00				
L-K	0 / 18	-28.0	-28.0	0.03 (2)	10.00				
K-J	0 / 12	-28.0	-28.0	0.02 (2)	10.00				
J-I	0 / 12	-28.0	-28.0	0.03 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.03 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1), BC=0.03 (K-L:2), WB=0.12 (E-J:1), SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.30 (E) (INPUT = 0.90)

JSI METAL= 0.08 (D) (INPUT = 1.00)



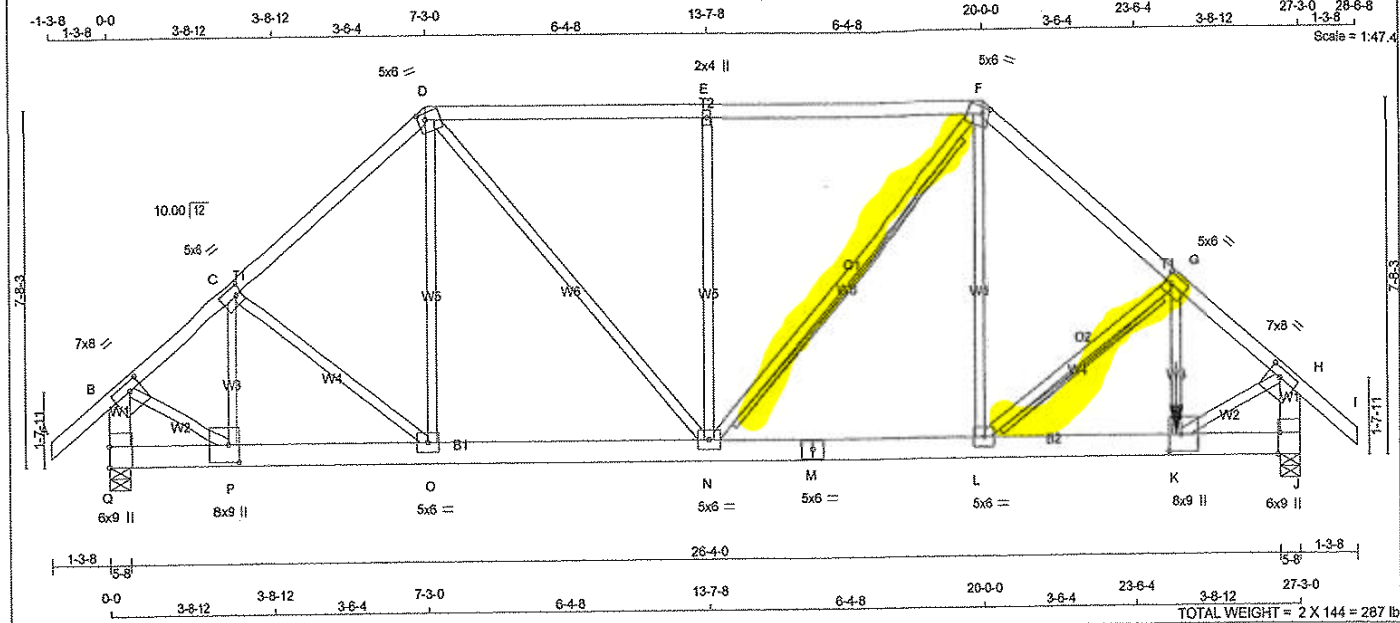
DRWG NO. TAM 45704-17

STRUCTURAL

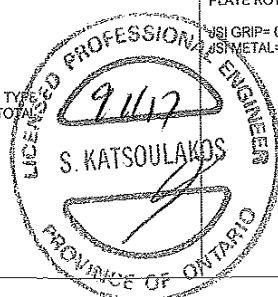
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272848	T18	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - I 2x4 DRY No.2 SPF Q - B 2x6 DRY No.2 SPF J - H 2x6 DRY No.2 SPF Q - M 2x6 DRY No.2 SPF M - J 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY, SEASONED LUMBER DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-D 1 12 TOP D-F 1 12 TOP F-I 1 12 TOP Q-B 2 12 TOP J-H 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS Q-M 2 3 TOP M-J 2 3 SIDE(915.6) WEBS : (0.122"x3") SPIRAL NAILS G-K 2 2 SIDE(895.6) 2x3 1 2 O-C 1 2 O-D 1 2 L-F 1 2 L-G 1 2 P-B 1 2 K-H 1 2 N-F 1 2 N-D 1 2 N-E 1 2 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/4 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES (table is in inches) JT TYPE PLATES W L E N Y X B TMWV-I MT20 7.0 8.0 2.25 3.25 C TMWW-I MT20 5.0 6.0 2.50 1.75 D TMWW-m MT20 5.0 6.0 1.75 2.00 E TMW-w MT20 2.0 4.0 F TMWW-m MT20 5.0 6.0 1.75 2.00 G TMWW-I MT20 5.0 6.0 2.50 1.75 H TMWV-I MT20 7.0 8.0 2.25 3.25 J BMV1-I MT20 6.0 9.0 Edge 0.50 K BMWW-I MT20 8.0 9.0 4.50 3.25 L BMWW-I MT20 5.0 6.0 M BS-I MT20 5.0 6.0				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION JT VERT 2927 J 8447 MAXIMUM FACTORED GROSS REACTION DOWN 0 HORIZ 8447 INPUT BRG IN-SX 5-8 REQRD BRG IN-SX 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL Q 2265 1517 / 0 377 / 0 0 / 0 370 / 0 0 / 0 J 6531 4382 / 0 1083 / 0 0 / 0 0 / 0 1656 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 2x4 DRY SPF No.2 T-BRACE AT G-L, F-N FASTEN T AND I BRACES TO HOLLOW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) CHORDS FACTORED VERT. LOAD LC1 MAX MEMB. FORCE FACTORED WEBS MAX. FORCE FACTORED MEMB. (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO MEMB. FORCE FACTORED (LBS) CSI (LC) FR-TO FROM TO A-B 0 / 54 -122.2 -122.2 0.09 (1) 10.00 P-C -846 / 0 0.10 (1) B-C -2621 / 0 -122.2 -122.2 0.19 (1) 5.41 C-O 0 / 210 0.03 (1) C-D -2924 / 0 -122.2 -122.2 0.22 (1) 5.18 O-D 0 / 224 0.03 (3) D-E -3320 / 0 -122.2 -122.2 0.57 (1) 4.43 L-F 0 / 2510 0.31 (1) E-F -3320 / 0 -122.2 -122.2 0.57 (1) 4.43 L-G -3558 / 0 0.58 (1) F-G -4507 / 0 -122.2 -122.2 0.30 (1) 4.28 K-G 0 / 3975 0.49 (1) G-H -8095 / 0 -122.2 -122.2 0.35 (1) 3.20 B-P 0 / 2239 0.28 (1) H-I 0 / 54 -122.2 -122.2 0.09 (1) 10.00 K-H 0 / 6817 0.84 (1) Q-B -2871 / 0 0.0 0.0 0.11 (1) 7.81 N-F -231 / 0 0.12 (1) J-H -8223 / 0 0.0 0.0 0.30 (1) 5.25 D-N 0 / 1673 0.21 (1) N-E -944 / 0 0.49 (1) Q-P 0 / 0 -28.0 -28.0 0.02 (1) 10.00 P-O 0 / 2052 -28.0 -28.0 0.17 (1) 10.00 O-N 0 / 2219 -28.0 -28.0 0.18 (1) 10.00 N-M 0 / 3473 -28.0 -28.0 0.29 (1) 10.00 M-L 0 / 3473 -28.0 -28.0 0.29 (1) 10.00 L-K 0 / 6245 -28.0 -28.0 0.60 (1) 10.00 K-J 0 / 0 -28.0 -28.0 0.19 (1) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR K 24-5-8 -6943 -6943 -- BACK VERT				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.3 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 58.7 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, ECBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.91") CALCULATED VERT. DEFL.(LL) = L/999 (0.08") ALLOWABLE DEFL.(TL) = L/360 (0.91") CALCULATED VERT. DEFL.(TL) = L/999 (0.12") CSI: TC=0.57 (E-F:1), BC=0.60 (K-L:1), WB=0.84 (H-K:1), SSI=0.21 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 0.50 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1658 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. CSI GRIP= 0.90 (C) (INPUT = 0.90) SSI METAL= 0.78 (H) (INPUT = 1.00)			
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W6 NO. TAM 4569817
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272848	T18	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
N	BMWWW-t	MT20	5.0	6.0	
O	BMWWW-t	MT20	5.0	6.0	
P	BMWWW-t	MT20	8.0	9.0	4.50 3.25
Q	BMV1-t	MT20	6.0	9.0	5.50

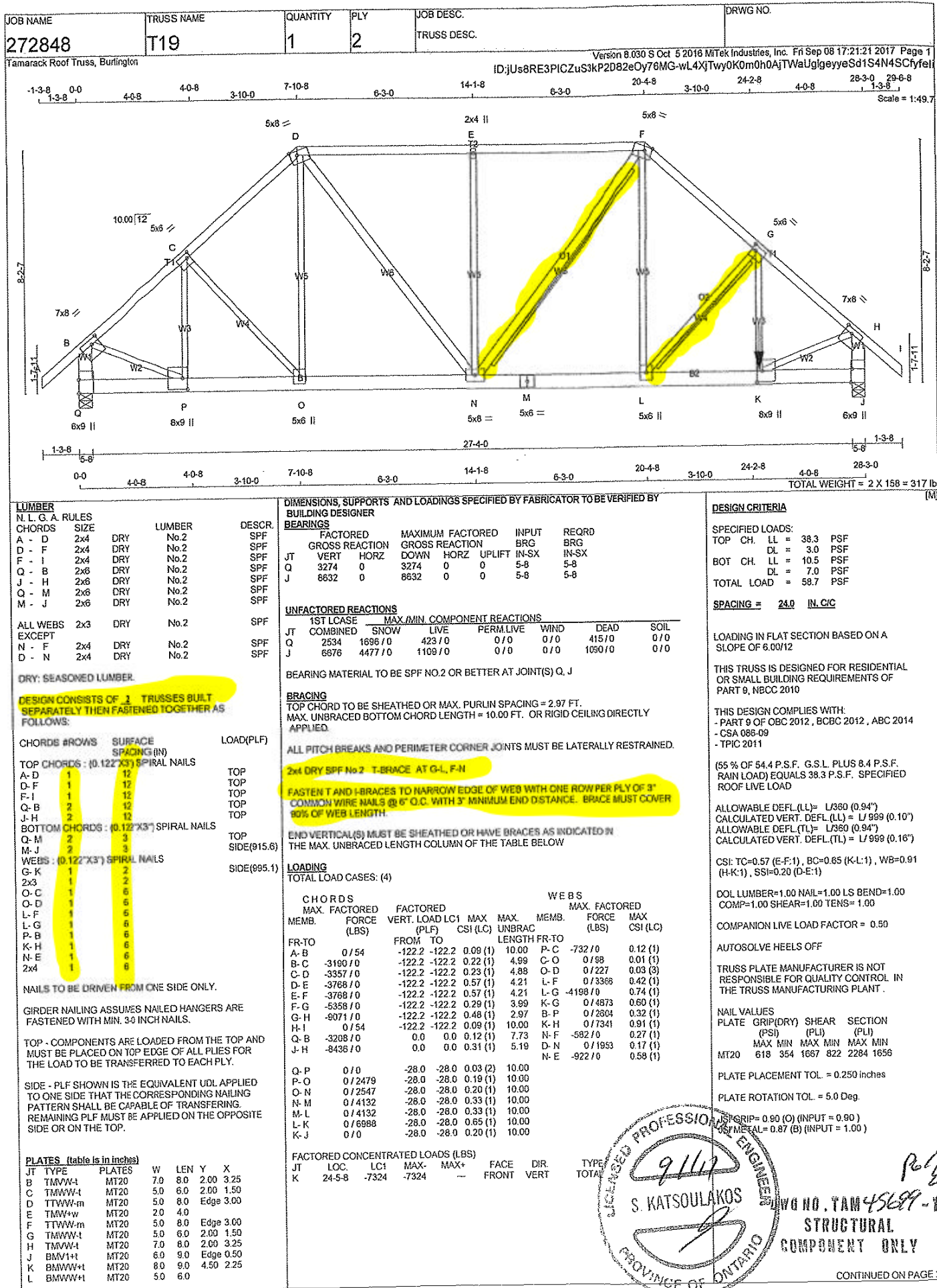
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6942.8 lbs FACTORED DOWN AT 24-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



91117
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4869817
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272848	T19	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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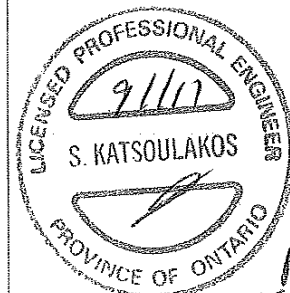
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	8.0	9.0	4.50	2.25
Q	BMV-t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 7324.2 lbs FACTORED DOWN AT
24-5-8 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.



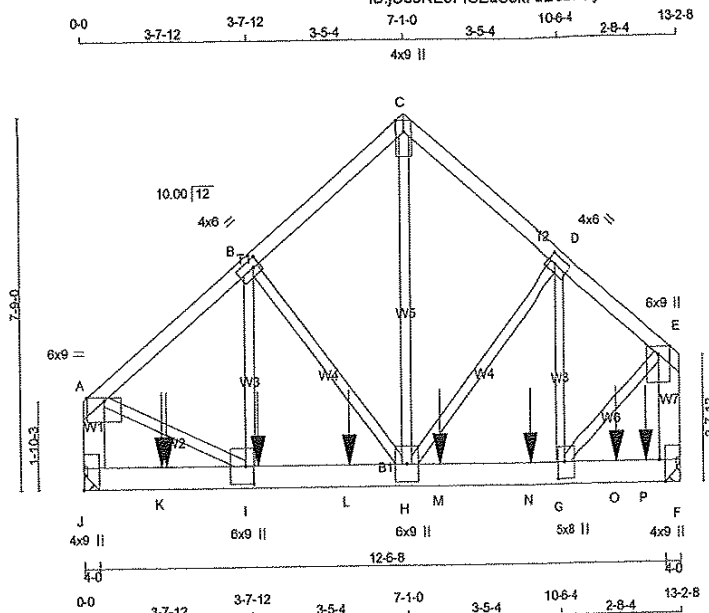
DWG NO. TAM 45699.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272848	T20A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:45.9

TOTAL WEIGHT = 2 X 76 = 153 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
J - A	2x6	DRY	No.2	SPF	
F - E	2x6	DRY	No.2	SPF	
J - F	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122' X 3') SPIRAL NAILS			
A - C	1	12	TOP
C - E	1	12	TOP
J - A	2	12	TOP
F - E	2	12	TOP
BOTTOM CHORDS : (0.122' X 3') SPIRAL NAILS			
J - F	2	12	SIDE(183.1)
WEBS : (0.122' X 3') SPIRAL NAILS			
B - I	1	6	SIDE(286.4)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-9 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	9.0	Edge	
B	TMVW-t	MT20	4.0	6.0	2.00	1.75
C	TTW+p	MT20	4.0	9.0	Edge	
D	TMVW-t	MT20	4.0	6.0	2.00	2.00
E	TMVW+p	MT20	6.0	9.0	Edge	
F	BMV1+t	MT20	4.0	9.0	Edge	1.50
G	BMVW+t	MT20	5.0	8.0	4.25	2.00
H	BMVW+t	MT20	6.0	9.0		
I	BMVW+t	MT20	6.0	9.0	4.50	2.00
J	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ
J	6971	0	6971	0
F	7352	0	7352	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW			
J	5401	3804 / 0	909 / 0	0 / 0	889 / 0
F	5697	3801 / 0	959 / 0	0 / 0	938 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. MAX. UNBRAC	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CS1 (LC)	FR-TO	
A-B	-5920 / 0	-122.2 -122.2	0.27 (1)	I-B	0 / 2205
B-C	-4312 / 0	-122.2 -122.2	0.18 (1)	B-H	-2076 / 0
C-D	-4306 / 0	-122.2 -122.2	0.19 (1)	H-C	0 / 5127
D-E	-4757 / 0	-122.2 -122.2	0.14 (1)	H-D	-608 / 0
J-A	-5785 / 0	0.0 0.0	0.22 (1)	G-D	0 / 389
F-E	-6232 / 0	0.0 0.0	0.29 (1)	A-I	0 / 4901
				G-E	0 / 4762
J-K	0 / 0	-28.0 -28.0	0.73 (1)		
K-I	0 / 0	-28.0 -28.0	0.73 (1)		
I-L	0 / 4563	-28.0 -28.0	0.61 (1)		
L-H	0 / 4563	-28.0 -28.0	0.61 (1)		
H-M	0 / 3668	-28.0 -28.0	0.57 (1)		
M-N	0 / 3668	-28.0 -28.0	0.57 (1)		
N-G	0 / 3668	-28.0 -28.0	0.57 (1)		
G-O	0 / 0	-28.0 -28.0	0.38 (1)		
O-P	0 / 0	-28.0 -28.0	0.38 (1)		
P-F	0 / 0	-28.0 -28.0	0.38 (1)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	3-9-12	-1800	-1800		BACK	VERT	TOTAL
K	1-8-8	-769	-769		FRONT	VERT	TOTAL
K	1-9-12	-1800	-1800		BACK	VERT	TOTAL
L	5-9-12	-1800	-1800		BACK	VERT	TOTAL
M	7-9-12	-1800	-1800		BACK	VERT	TOTAL
N	9-9-12	-1800	-1800		BACK	VERT	TOTAL
O	11-9-12	-1800	-1800		BACK	VERT	TOTAL
P	12-5-8	-773	-773		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 58.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.29 (E-F:1), BC=0.73 (I-J:1), WB=0.63 (C-H:1), SS=0.62 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

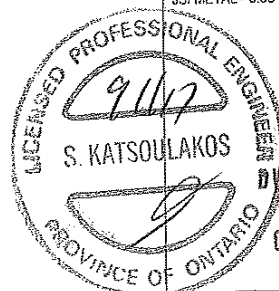
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

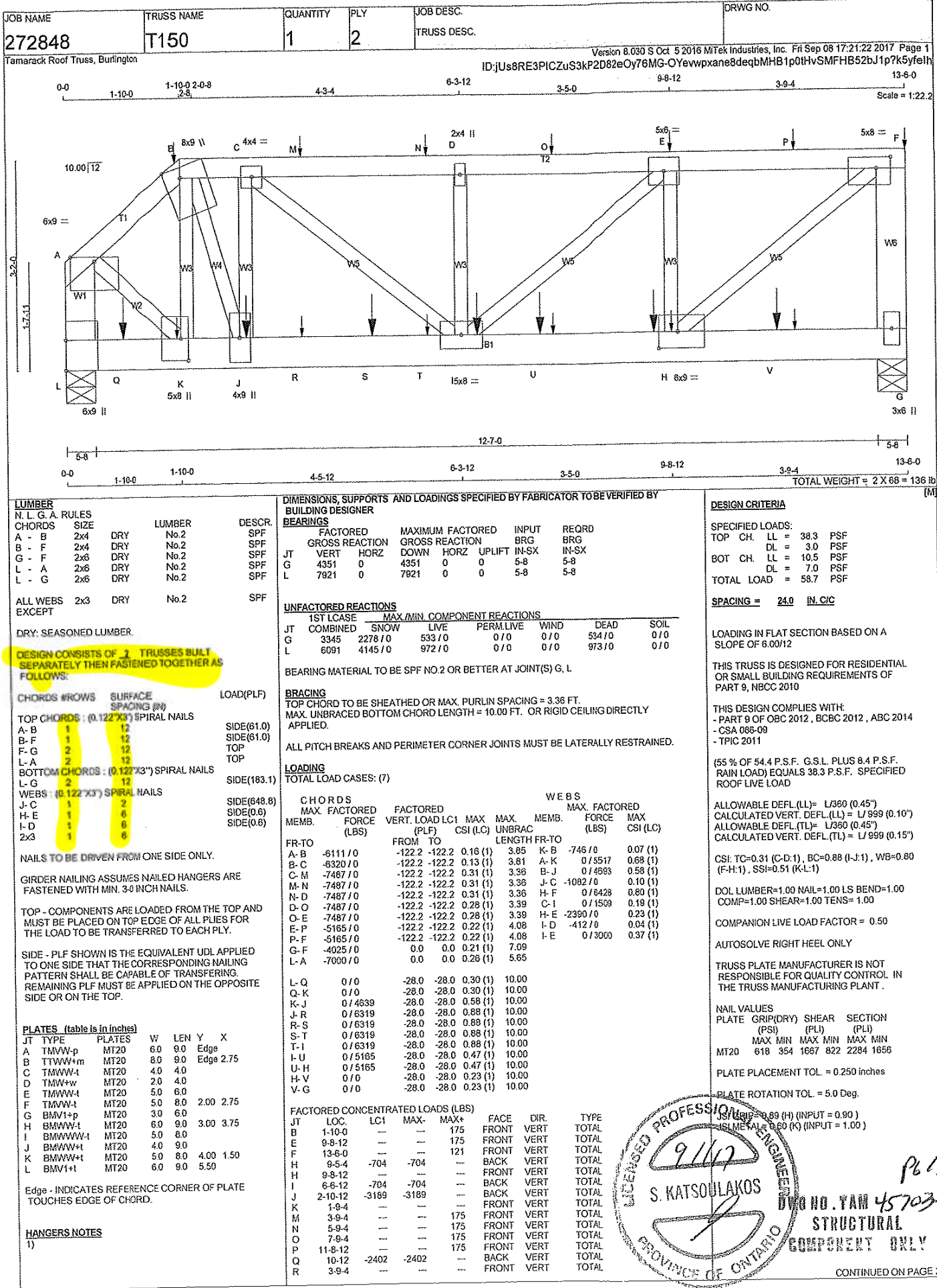


DRW NO. TAM 45702 17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272848	T20A	1	2	TRUSS DESC.	
Tamarack Roof Truss, Burlington Version 8.030 S Oct 5 2016 MITek Industries, Inc. Fri Sep 08 17:22:30 2017 Page 2 ID:jUs8RE3PICZuS3kP2D82eOy78MG-PdlcC3mEmXjzoHoRGTFInD6XcmUa7vPcxchb79yfeHd					
HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 769.1 lbs FACTORED DOWN AT 1-8-8, 1799.6 lbs FACTORED DOWN AT 1-9-12, 1799.6 lbs FACTORED DOWN AT 3-9-12, 1799.6 lbs FACTORED DOWN AT 5-9-12, 1799.6 lbs FACTORED DOWN AT 7-9-12, 1799.6 lbs FACTORED DOWN AT 9-9-12, AND 1799.6 lbs FACTORED DOWN AT 11-9-12, AND 772.6 lbs FACTORED DOWN AT 12-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.					
 P62 DWG NO. TAM 45700-17 STRUCTURAL COMPONENT ONLY					

[illegible]



JOB NAME 272848	TRUSS NAME T150	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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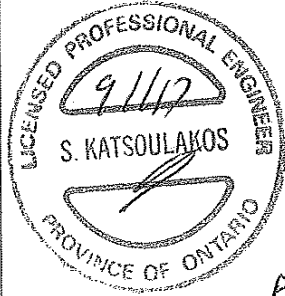
Tamarack Roof Truss, Burlington
Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Fri Sep 08 17:21:22 2017 Page 2

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HANGERS NOTES

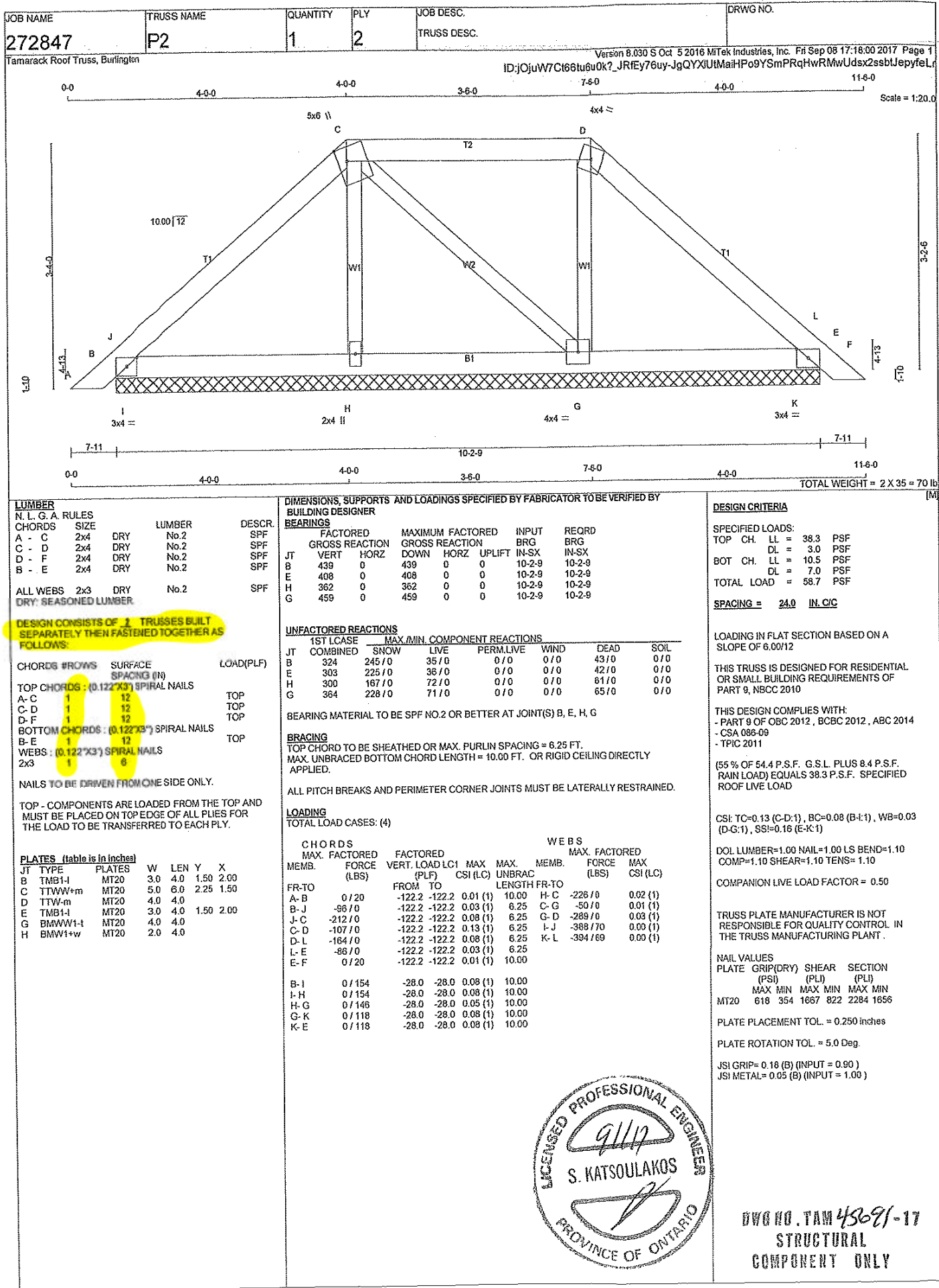
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 174.7 lbs FACTORED UP AT 1-10-0, 174.7 lbs FACTORED UP AT 3-9-4, 174.7 lbs FACTORED UP AT 5-9-4, 174.7 lbs FACTORED UP AT 7-9-4, 174.7 lbs FACTORED UP AT 9-8-12, AND 121.0 lbs FACTORED UP AT 11-8-12, AND 121.0 lbs FACTORED UP AT 13-6-0 ON TOP CHORD, AND 2402.1 lbs FACTORED DOWN AT 10-12, AT 1-9-4, 3188.8 lbs FACTORED DOWN AT 2-10-12, AT 3-9-4, 1838.0 lbs FACTORED DOWN AT 4-10-12, AT 5-9-4, 704.3 lbs FACTORED DOWN AT 6-6-12, 704.3 lbs FACTORED DOWN AT 7-6-12, AT 7-9-4, 704.3 lbs FACTORED DOWN AT 9-5-4, AT 9-8-12, AND 704.3 lbs FACTORED DOWN AT 11-5-4, AND AT 11-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

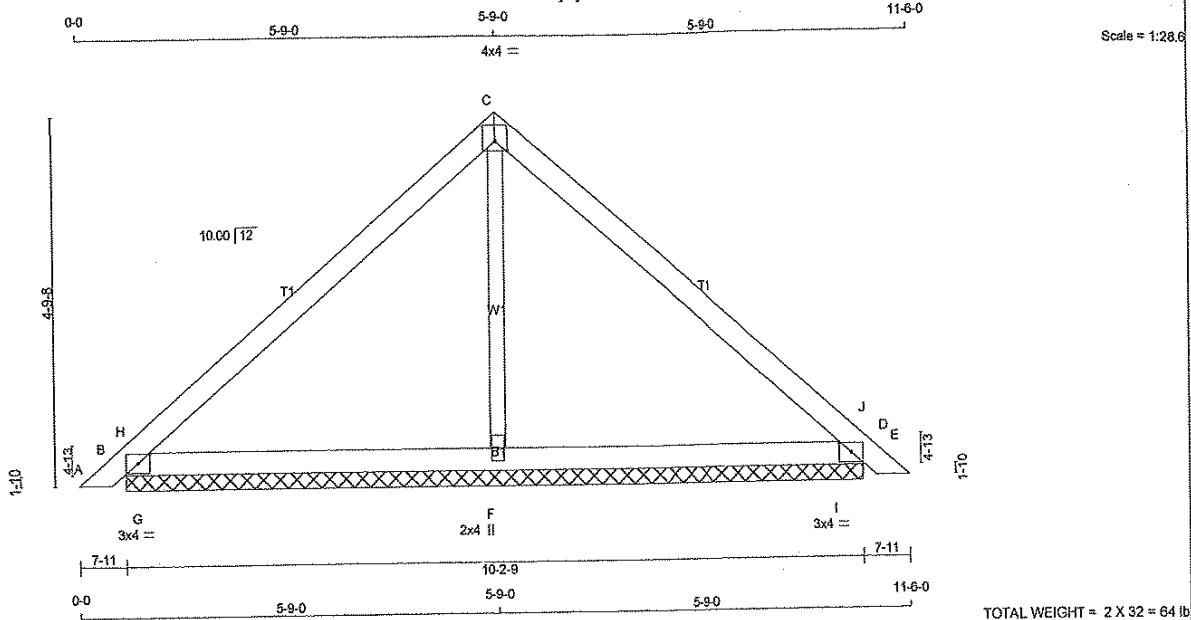
FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	4-10-12	-1838	-1836	---	BACK	VERT	TOTAL
T	5-9-4	---	---	---	FRONT	VERT	TOTAL
U	7-6-12	-704	-704	---	BACK	VERT	TOTAL
U	7-9-4	---	---	---	FRONT	VERT	TOTAL
V	11-5-4	-704	-704	---	BACK	VERT	TOTAL
V	11-8-12	---	---	---	FRONT	VERT	TOTAL



PG 2
DWG NO. TAM 45703.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272847	P1	1	1		
Tamarack Roof Truss, Burlington					
Version 6.030 S Oct 5 2016 MTek Industries, Inc. Fri Sep 08 17:16:00 2017 Page 1 IDjOjuW7Ct66tu6u0K?_JRfEy76uy-JgQYXUitMailHPo9YSmPRqHwOpwUXswCsstJepyfaLr					
Scale = 1:19.0					
TOTAL WEIGHT = 34 lb					
LUMBER	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				
N. L. G. A. RULES					
CHORDS SIZE	LUMBER	DESCR.	BEARINGS		
A - C 2x4 DRY No.2	SPF				
C - E 2x4 DRY No.2	SPF				
E - G 2x4 DRY No.2	SPF				
B - F 2x4 DRY No.2	SPF				
ALL WEBS 2x3 DRY No.2	SPF				
DRY: SEASONED LUMBER.					
UNFACTORED REACTIONS					
1ST CASE MAX/MIN COMPONENT REACTIONS					
JT TYPE PLATES W LEN Y X					
B TMB1-I MT20 3.0 4.0 1.50 2.00					
D TTWW+m MT20 5.0 6.0 2.25 1.50					
C TMW+w MT20 2.0 4.0 1.50 2.00					
E TTWW+m MT20 5.0 6.0 2.25 1.50					
F TMB1-I MT20 3.0 4.0 1.50 2.00					
H BMW1+w MT20 2.0 4.0 1.50 2.00					
I BMWWW1-I MT20 4.0 6.0 2.25 1.50					
J BMW1+w MT20 2.0 4.0 1.50 2.00					
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H					
BRACING					
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.					
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.					
LOADING					
TOTAL LOAD CASES: (4)					
CHORDS FACTORED FORCE (LBS) FROM TO					
MEMB. MAX. FACTORED FORCE (LBS) FROM TO					
FR-TO A-B 0/20 -122.2 -122.2 0.03 (1) 10.00					
B-L -45/10 -122.2 -122.2 0.01 (1) 6.25					
L-C -81/0 -122.2 -122.2 0.02 (1) 6.25					
C-D -7/0 -122.2 -122.2 0.29 (1) 10.00					
D-E -7/0 -122.2 -122.2 0.29 (1) 10.00					
E-N -61/0 -122.2 -122.2 0.02 (1) 6.25					
N-F -45/10 -122.2 -122.2 0.01 (1) 6.25					
F-G 0/20 -122.2 -122.2 0.03 (1) 10.00					
WEBS FACTORED FORCE (LBS) FROM TO					
MEMB. MAX. FACTORED FORCE (LBS) FROM TO					
J-C -192/0 0.03 (1) 10.00					
C-I -21/0 0.01 (1) 6.25					
I-D -572/0 0.08 (1) 10.00					
D-E -21/0 0.01 (1) 6.25					
E-H -192/0 0.03 (1) 10.00					
H-K -103/0 0.00 (1) 6.25					
K-L -103/0 0.00 (1) 6.25					
M-N -103/0 0.00 (1) 6.25					
B-K 0/44 -28.0 -28.0 0.03 (1) 10.00					
K-J 0/44 -28.0 -28.0 0.06 (2) 10.00					
J-I 0/27 -28.0 -28.0 0.08 (2) 10.00					
I-H 0/27 -28.0 -28.0 0.08 (2) 10.00					
H-M 0/44 -28.0 -28.0 0.06 (2) 10.00					
M-F 0/44 -28.0 -28.0 0.03 (1) 10.00					
DESIGN CRITERIA					
SPECIFIED LOADS:					
TOP CH. LL = 38.3 PSF					
DL = 3.0 PSF					
BOT CH. LL = 10.5 PSF					
DL = 7.0 PSF					
TOTAL LOAD = 58.7 PSF					
SPACING = 24.0 IN./C					
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12					
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010					
THIS DESIGN COMPLIES WITH:					
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014					
- CSA 086-09					
- TPIC 2011					
(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD					
CSI: TC=0.29 (C-D:1), BC=0.08 (I-J:2), WB=0.08 (D-I:1), SSI=0.22 (C-D:1)					
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10					
COMPANION LIVE LOAD FACTOR = 0.50					
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .					
NAIL VALUES					
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)					
MAX MIN MAX MIN MAX MIN					
MT20 618 354 1657 822 2284 1855					
PLATE PLACEMENT TOL. = 0.250 inches					
PLATE ROTATION TOL. = 5.0 Deg.					
JSI GRIP= 0.30 (D) (INPUT = 0.90)					
JSI METAL= 0.11 (D) (INPUT = 1.00)					
DWG NO. TAM 45692-17					
STRUCTURAL COMPONENT ONLY					





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
B - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.00
C	TTW-p	MT20	4.0	4.0	1.50 2.00
D	TMB1-I	MT20	3.0	4.0	1.50 2.00
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER								
<u>BEARINGS</u>								
	FACTORED		MAXIMUM FACTORED		INPUT	REQRD		
	GROSS REACTION		GROSS REACTION					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
B	612	0	612	0	0	10-2-9	10-2-9	
D	612	0	612	0	0	10-2-9	10-2-9	
F	445	0	445	0	0	10-2-9	10-2-9	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX / MIN LIVE		PERM. LIVE		WIND		DEAD		SOIL	
	SNOW											
B	454	338 / 0	54 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0	63 / 0	0 / 0	0 / 0	0 / 0
D	454	338 / 0	54 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0	63 / 0	0 / 0	0 / 0	0 / 0
F	382	190 / 0	106 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0	86 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 20	-122.2 -122.2	0.03 (1)	10.00	F-C	-124 / 30	0.04 (1)
B-H	-153 / 170	-122.2 -122.2	0.28 (1)	6.25	G-H	-994 / 112	0.00 (1)
H-C	-334 / 0	-122.2 -122.2	0.39 (1)	6.25	I-J	-994 / 112	0.00 (1)
C-J	-334 / 0	-122.2 -122.2	0.39 (1)	6.25			
J-D	-153 / 170	-122.2 -122.2	0.28 (1)	6.25			
D-E	0 / 20	-122.2 -122.2	0.03 (1)	10.00			
B-G	0 / 239	-28.0 -28.0	0.36 (1)	10.00			
G-F	0 / 239	-28.0 -28.0	0.36 (1)	10.00			
F-I	0 / 239	-28.0 -28.0	0.36 (1)	10.00			
I-D	0 / 239	-28.0 -28.0	0.36 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39 (C-J-1), BC=0.36 (B-G-1), WB=0.04 (C-F-1), SSI=0.76 (D-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (D) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)



DRWG NO. TAM45692-17
STRUCTURAL
COMPONENT ONLY

JOB NAME

272847

TRUSS NAME

P3Z

QUANTITY

1

PLY

2

JOB DESC.

TRUSS DESC.

DRWG NO.

Tamarack Roof Truss, Burlington

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ID:QjuW7Ct66tu6u0K?_JRfEy76uy-nl_wk5VV7uq81xk0TwgNUta3KpGbNR75FdsAFyfeLq

0-0

5-9-0

5-9-0

5-9-0

11-6-0

4x4

Scale = 1:28.6

10.00

12

4-9-8

1-10

4-13

1-10

7-11

3x4

10-2-0

5-9-0

5-9-0

5-9-0

11-6-0

7-11

3x4

1-10

4-13

1-10

TOTAL WEIGHT = 2 X 32 = 64 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

SPF

A - C

2x4

DRY

No.2

SPF

C - E

2x4

DRY

No.2

SPF

B - D

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS

BROWS

SURFACE

LOAD(PLF)

TOP CHORDS : (0.122'X3') SPIRAL NAILS

A - C

1

12

TOP

C - E

1

12

TOP

BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS

B - D

1

12

TOP

WEBS : (0.122'X3') SPIRAL NAILS

2x3

1

8

TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

B

TMB1-I

MT20

3.0

4.0

1.50

2.00

C

TTW-p

MT20

4.0

4.0

1.50

2.00

D

TMB1-I

MT20

3.0

4.0

1.50

2.00

F

BMW1-w

MT20

2.0

4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQRD

GROSS REACTION

GROSS REACTION

BRG

BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

B

613

0

613

0

0

10-2-9

10-2-9

D

613

0

613

0

0

10-2-9

10-2-9

F

442

0

442

0

0

10-2-9

10-2-9

UNFACTORED REACTIONS

1ST LCASE

MAX / MIN

COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM LIVE

WIND

DEAD

SOIL

B

455

339 / 0

54 / 0

0 / 0

0 / 0

63 / 0

0 / 0

D

455

339 / 0

54 / 0

0 / 0

0 / 0

63 / 0

0 / 0

F

380

187 / 0

105 / 0

0 / 0

0 / 0

86 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

VERT. LOAD

LC1

MAX

MEMB.

FORCE

(PLF)

CSI (LC)

UNBRAC

MEMB.

FORCE

MAX

CSI (LC)

FR-TO

FROM

TO

LENGTH

FR-TO

A-B

0 / 20

-122.2

-122.2

0.01 (1)

10.00

F-C

-122 / 30

0.02 (1)

B-H

-152 / 160

-122.2

-122.2

0.13 (1)

6.25

G-H

-984 / 111

0.00 (1)

H-C

-336 / 0

-122.2

-122.2

0.19 (1)

6.25

I-J

-984 / 111

0.00 (1)

C-J

-336 / 0

-122.2

-122.2

0.19 (1)

6.25

J-D

-152 / 160

-122.2

-122.2

0.13 (1)

6.25

D-E

0 / 20

-122.2

-122.2

0.01 (1)

10.00

B-G

0 / 241

-28.0

-28.0

0.18 (1)

10.00

G-F

0 / 241

-28.0

-28.0

0.18 (1)

10.00

F-I

0 / 241

-28.0

-28.0

0.18 (1)

10.00

I-D

0 / 241

-28.0

-28.0

0.18 (1)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (C-H:1), BC=0.18 (F-G:1), WB=0.02 (C-F:1), SSI=0.38 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)

JSI METAL= 0.07 (B) (INPUT = 1.00)

9/11/17

S. KATSOUKAKOS

PROVINCE OF ONTARIO

DWG NO. TAM 45693-17

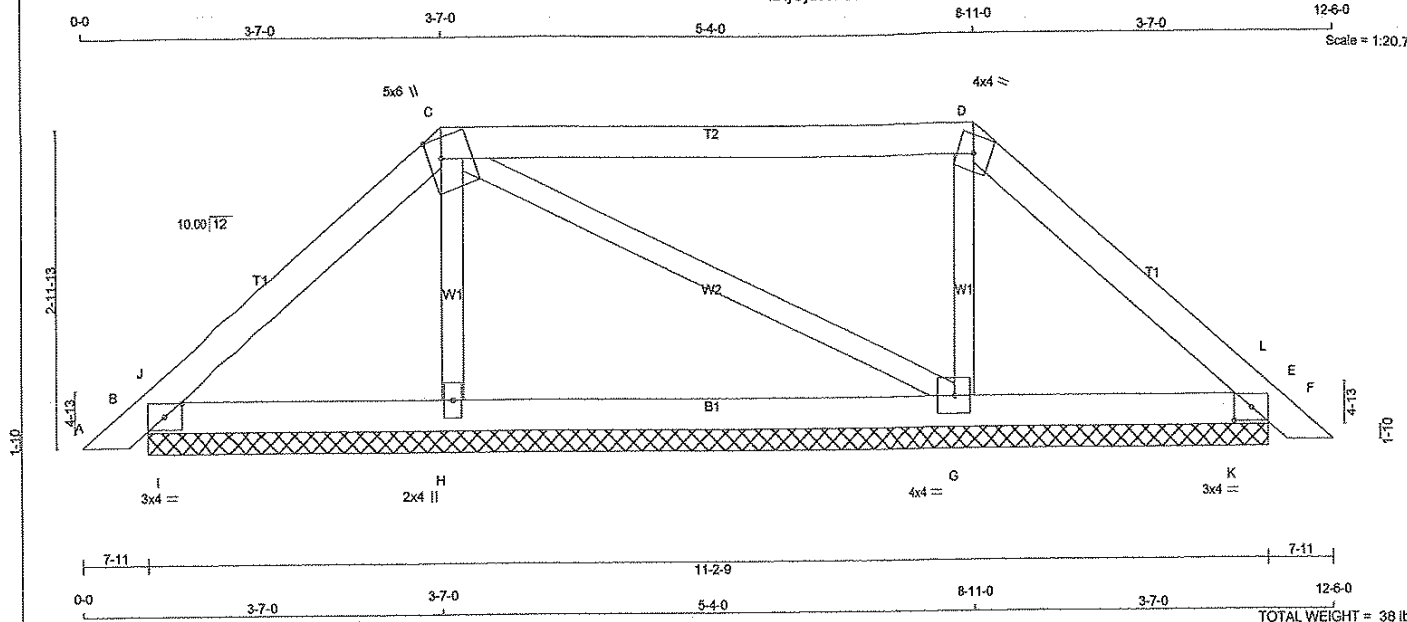
STRUCTURAL

COMPONENT ONLY

JOB NAME 272847	TRUSS NAME P4	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: jOjuW7Cl66tu6u0k?_JRlEy76uy-nt_wk5VV7uq81xk0TwgNUTUpKpubMi75FdsAFyfeLq



<u>LUMBER</u>						
N. L. G. A. RULES						
CHORDS		SIZE		LUMBER	DESCR.	
A - C	2x4	DRY	No.2			SPF
C - D	2x4	DRY	No.2			SPF
D - F	2x4	DRY	No.2			SPF
B - E	2x4	DRY	No.2			SPF
ALL WEBS		2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMB1-H	MT20	3.0	4.0	1.50 2.00
C TTWW+m	MT20	5.0	6.0	2.25 1.50
D TTW-m	MT20	4.0	4.0	
E TMB1-H	MT20	3.0	4.0	1.50 2.00
G BMWW1-H	MT20	4.0	4.0	
H BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
B	386	0	386	0
E	361	0	361	0
H	508	0	508	0
G	564	0	564	0

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
B	276
E	259
H	418
G	466

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX
MEMB.	(LBS)	(PLF)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO									
A-B	0/20	-122.2	-122.2	0.03	(1)	10.00	H-C	-346/0	0.06 (1)
B-J	-74/0	-122.2	-122.2	0.06	(1)	6.25	C-G	-35/0	0.02 (1)
J-C	-187/0	-122.2	-122.2	0.12	(1)	6.25	G-D	-386/0	0.07 (1)
C-D	-90/0	-122.2	-122.2	0.59	(1)	6.25	I-J	-333/0	0.00 (1)
D-L	-148/0	-122.2	-122.2	0.12	(1)	6.25	K-L	-338/0	0.00 (1)
L-E	-32/0	-122.2	-122.2	0.06	(1)	6.25			
E-F	0/20	-122.2	-122.2	0.03	(1)	10.00			
B-I	0/136	-28.0	-28.0	0.13	(1)	10.00			
I-H	0/136	-28.0	-28.0	0.14	(2)	10.00			
H-G	0/122	-28.0	-28.0	0.14	(2)	10.00			
G-K	0/106	-28.0	-28.0	0.14	(2)	10.00			
K-E	0/106	-28.0	-28.0	0.13	(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(5% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.59 (C-D:1), BC=0.14 (H-I:2), WB=0.07 (D-G:1), SSI=0.26 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

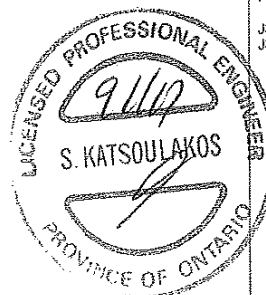
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1867
	822	2284	1656

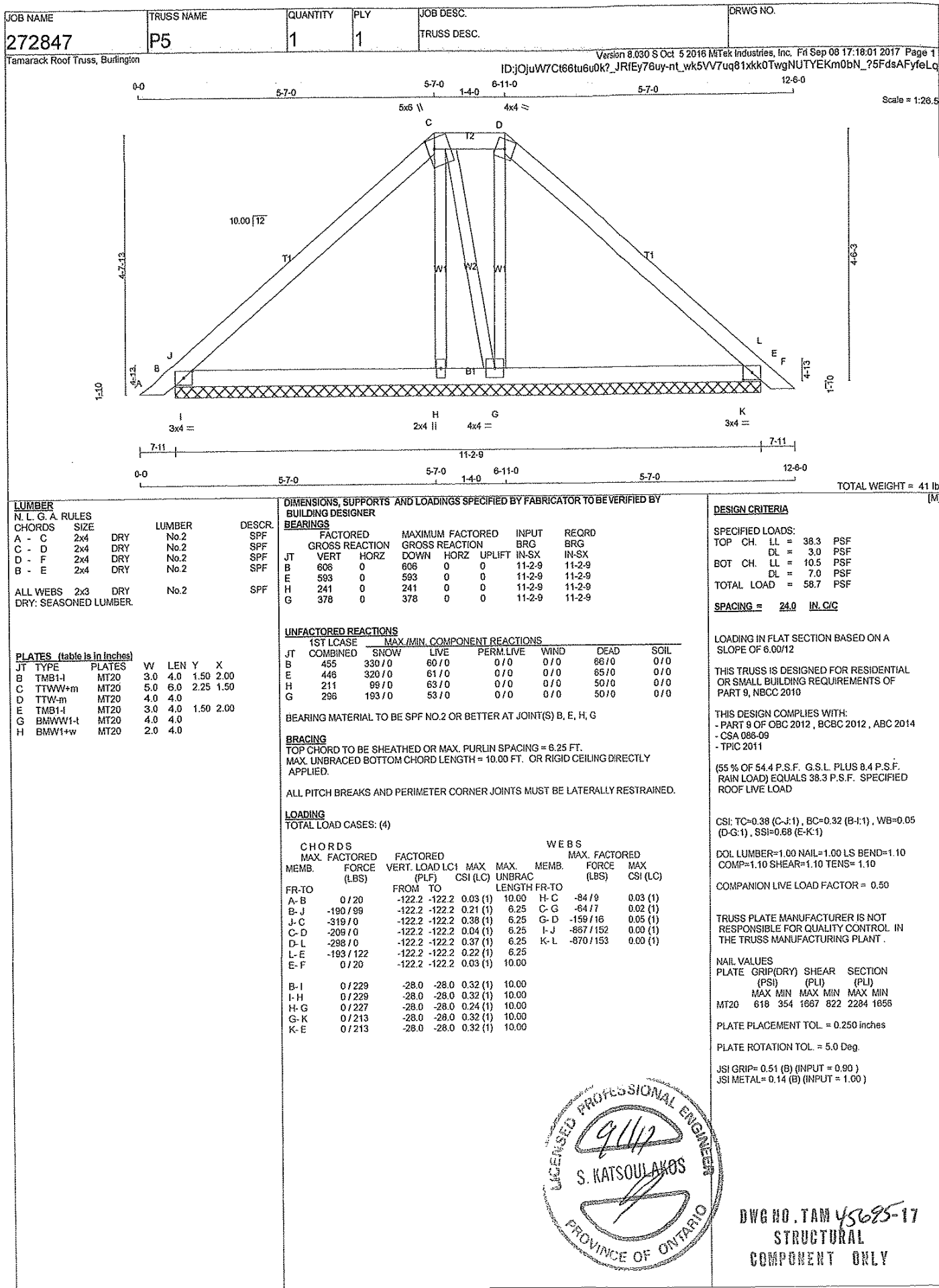
PLATE PLACEMENT TOL = 0.250 inches

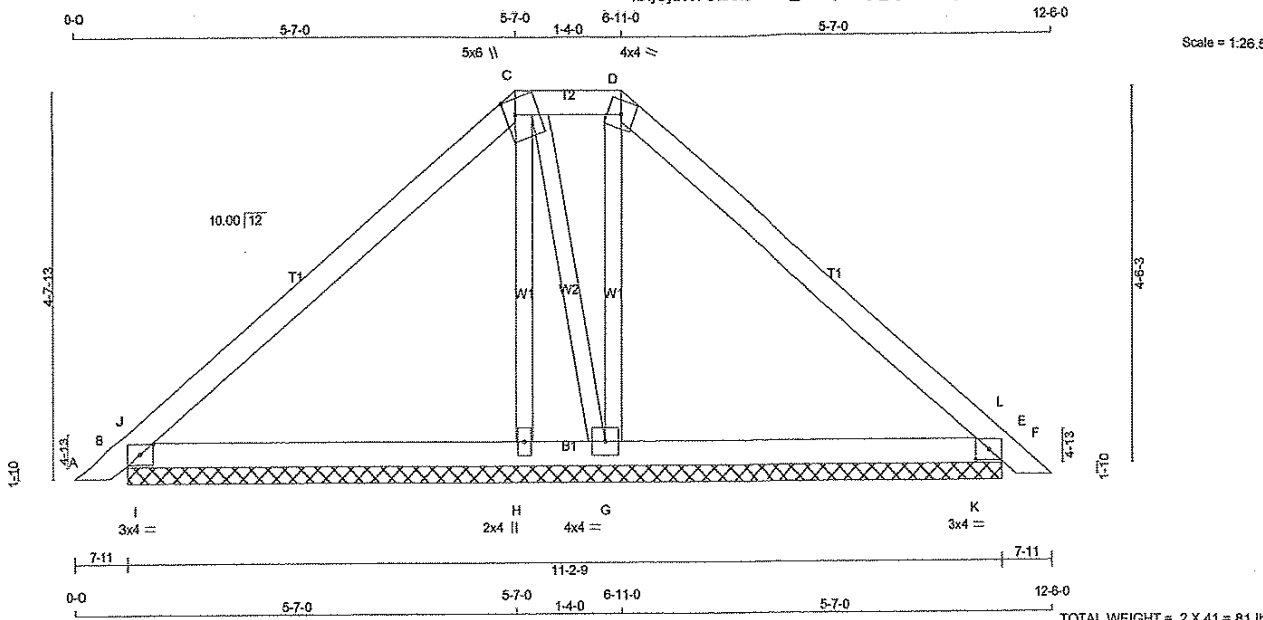
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DWG NO. TAM45694-17
STRUCTURAL
COMPONENT ONLY





LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY No.2 SPF
C - D 2x4 DRY No.2 SPF
D - F 2x4 DRY No.2 SPF
B - E 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
DRY, SEASONED LUMBER

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (in)	LOAD (PLF)
TOP CHORDS: (0.122'X3') SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	TOP
D-F	1	12	TOP
BOTTOM CHORDS: (0.122'X3') SPIRAL NAILS			
B-E	1	12	TOP
WEBS: (0.122'X3') SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWV1-I	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	608	0	608	0	0	11-2-9
E	594	0	594	0	0	11-2-9
H	240	0	240	0	0	11-2-9
G	376	0	376	0	0	11-2-9

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE			
B	456	331 / 0	59 / 0	0 / 0	66 / 0	0 / 0
E	447	322 / 0	60 / 0	0 / 0	65 / 0	0 / 0
H	210	98 / 0	63 / 0	0 / 0	50 / 0	0 / 0
G	295	191 / 0	53 / 0	0 / 0	50 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 20	-122.2 -122.2	0.01 (1)	H-C	-84 / 8	0.01 (1)	
B-J	-189 / 91	-122.2 -122.2	0.10 (1)	C-G	-64 / 6	0.01 (1)	
J-C	-321 / 0	-122.2 -122.2	0.19 (1)	G-D	-158 / 16	0.02 (1)	
C-D	-211 / 0	-122.2 -122.2	0.02 (1)	I-J	-858 / 151	0.00 (1)	
D-L	-300 / 0	-122.2 -122.2	0.19 (1)	K-L	-861 / 151	0.00 (1)	
L-E	-191 / 114	-122.2 -122.2	0.11 (1)				
E-F	0 / 20	-122.2 -122.2	0.01 (1)				
B-I	0 / 231	-28.0 -28.0	0.16 (1)				
I-H	0 / 231	-28.0 -28.0	0.16 (1)				
H-G	0 / 229	-28.0 -28.0	0.12 (1)				
G-K	0 / 215	-28.0 -28.0	0.16 (1)				
K-E	0 / 215	-28.0 -28.0	0.16 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (C-J-1), BC=0.16 (B-I-1), WB=0.02 (D-G-1), SS=0.34 (E-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	818	354	1667
	822		2284

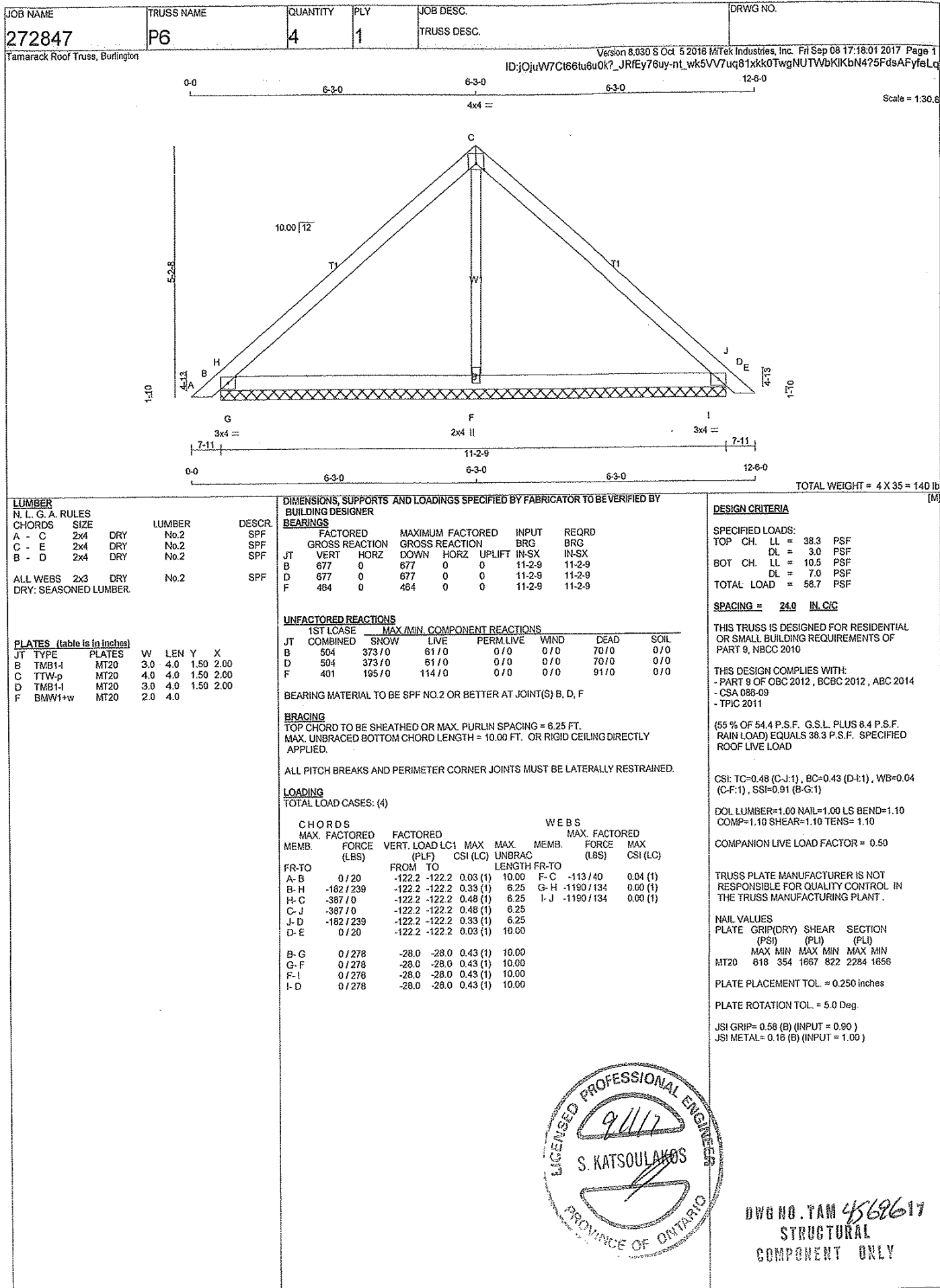
PLATE PLACEMENT TOL. = 0.250 inches

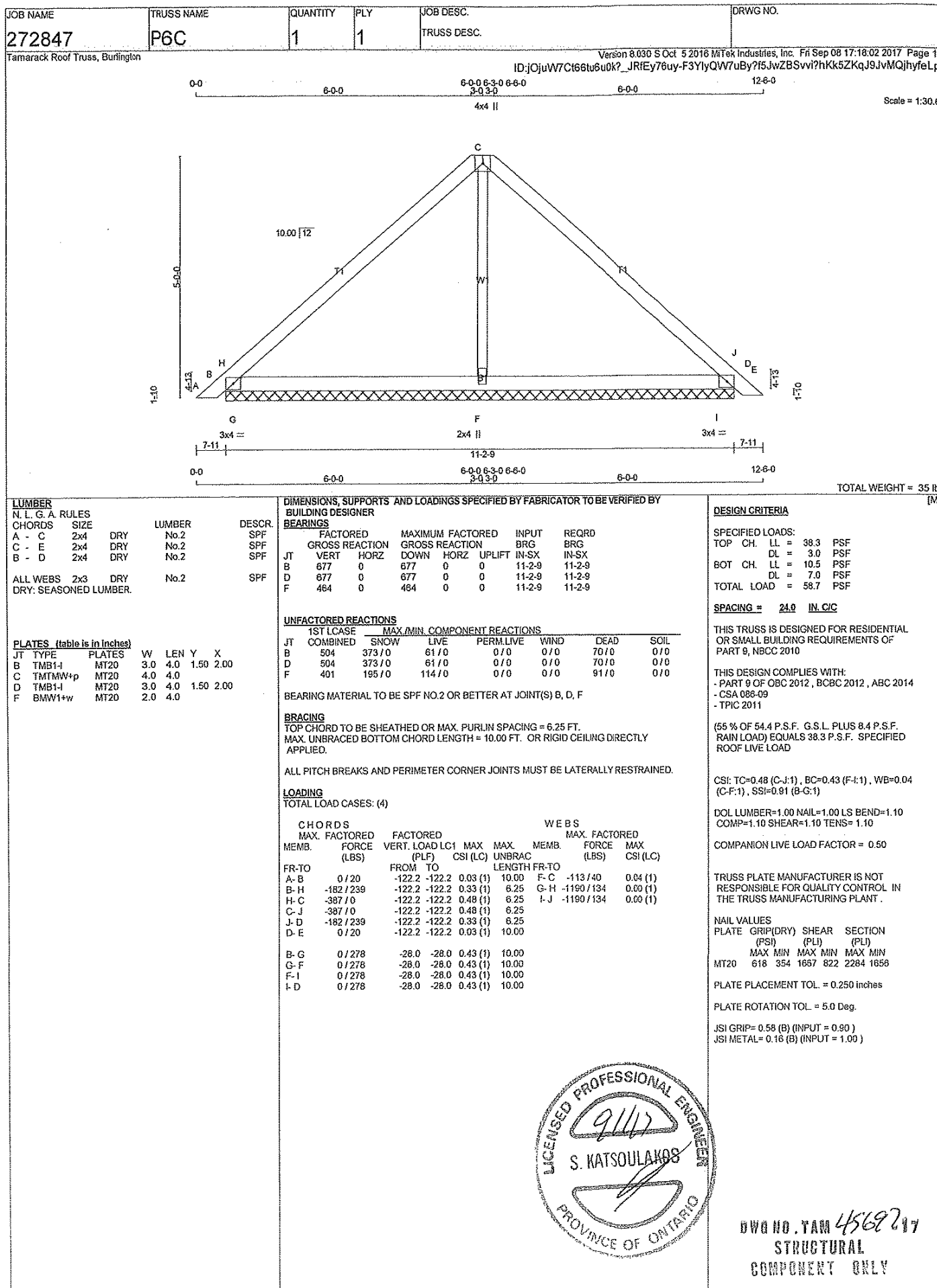
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



DWG NO. TAM 45705-17
STRUCTURAL
COMPONENT ONLY



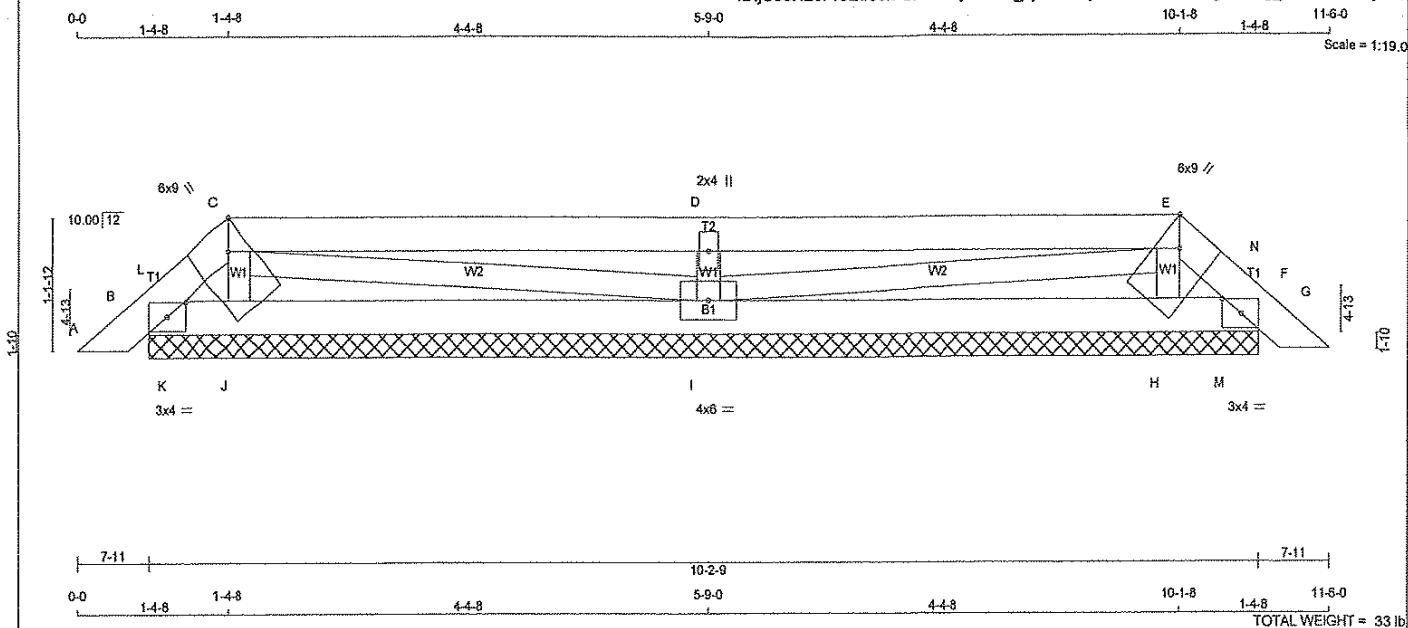


JOB NAME 272848	TRUSS NAME P7	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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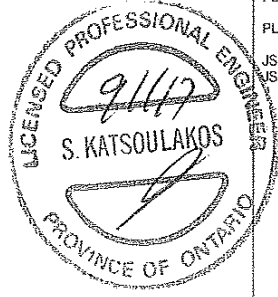
Tamarack Roof Truss, Burlington

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LUMBER N. L. G. A. RULES CHOROS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - E 2x4 DRY No.2 SPF E - G 2x4 DRY No.2 SPF B - F 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER				PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMB1-H MT20 3.0 4.0 1.50 2.00 C, E, H, J C TTBWW1*H MT20 6.0 9.0 Edge 2.25 D TMVW*W MT20 2.0 4.0 E TTBWW1*H MT20 6.0 9.0 2.75 Edge F TMB1-H MT20 3.0 4.0 1.50 2.00 I BMVWV1-H MT20 4.0 6.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG DOWN HORZ UPLIFT IN-SX IN-SX JT 138 0 138 0 -45 10-2-9 10-2-9 B 138 0 138 0 -45 10-2-9 10-2-9 F 299 0 299 0 0 10-2-9 10-2-9 J 794 0 794 0 0 10-2-9 10-2-9 H 299 0 299 0 0 10-2-9 10-2-9 PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS. FACTORED UPLIFT UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL B 75 107/0 0/1-24 0/0 0/0 0/0 0/0 F 75 107/0 0/1-24 0/0 0/0 0/0 0/0 J 267 118/0 85/0 0/0 0/0 66/0 0/0 I 607 419/0 93/0 0/0 0/0 95/0 0/0 H 267 118/0 85/0 0/0 0/0 66/0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MAX. FACTORED FORCE MAX. FACTORED FORCE MEMB. (LBS) VERT. LOAD LC1 MAX. MAX. MEMB. (LBS) MAX. FACTORED FORCE FR-TO FROM TO CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) A-B 0/20 -122.2 -122.2 0.03 (1) 10.00 J-C -174/10 0.02 (1) B-L -58/44 -122.2 -122.2 0.03 (3) 6.25 C-I -4/1 0.00 (1) L-C -62/0 -122.2 -122.2 0.02 (2) 6.25 I-D -688/0 0.09 (1) C-D 0/1 -122.2 -122.2 0.40 (1) 10.00 H-E -4/1 0.00 (1) D-E 0/1 -122.2 -122.2 0.40 (1) 10.00 E-H -174/10 0.02 (1) E-N -62/0 -122.2 -122.2 0.02 (2) 6.25 K-L -84/0 0.00 (1) N-F -58/44 -122.2 -122.2 0.03 (3) 6.25 M-N -84/0 0.00 (1) F-G 0/20 -122.2 -122.2 0.03 (1) 10.00 B-K -4/31 -28.0 -28.0 0.01 (1) 10.00 K-J -4/31 -28.0 -28.0 0.09 (2) 10.00 J-I -3/6 -28.0 -28.0 0.11 (3) 10.00 I-H -3/6 -28.0 -28.0 0.11 (3) 10.00 H-M -4/31 -28.0 -28.0 0.09 (2) 10.00 M-F -4/31 -28.0 -28.0 0.01 (1) 10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 38.3 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 58.7 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.40 (C-D-1), BC=0.11 (I-J-3), WB=0.09 (D-I-1), SSI=0.28 (C-D-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.35 (D) (INPUT = 0.90) JSI METAL= 0.12 (D) (INPUT = 1.00)			
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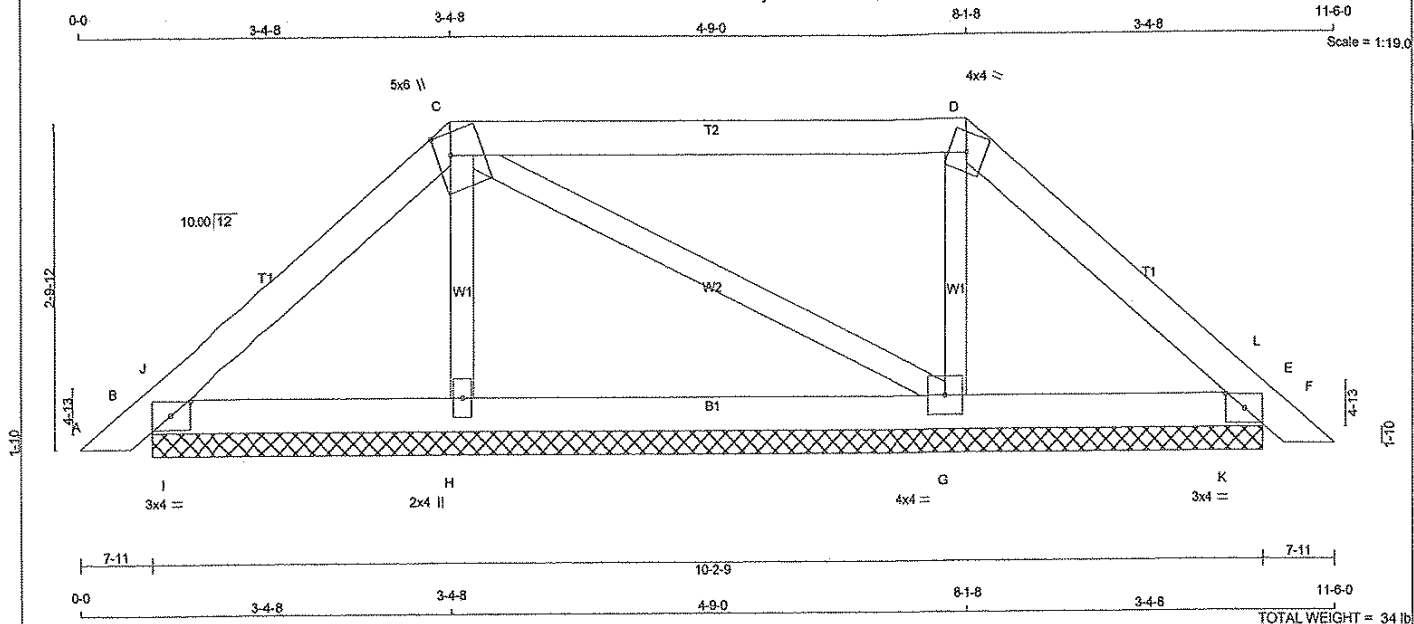
DRW NO. TAM 45706-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272848	P8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:Jus8RE3PICZuS3kP2D82eOy76MG-S9W9V7vKF1uvPXR_Am?LxSCVWVYIsjO4IriKufDyfelj



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B TMB1-I	MT20	3.0	4.0	1.50 2.00
C TTWW+m	MT20	5.0	6.0	2.25 1.50
D TTW-m	MT20	4.0	4.0	
E TMB1-I	MT20	3.0	4.0	1.50 2.00
G BMW1-I	MT20	4.0	4.0	
H BMW1+I	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	363	0	363	0	0	10-2-9	10-2-9
E	339	0	339	0	0	10-2-9	10-2-9
H	456	0	456	0	0	10-2-9	10-2-9
G	511	0	511	0	0	10-2-9	10-2-9

UNFACTORED REACTIONS

1ST LCASE		MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	260	211 / 0	20 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	244	195 / 0	20 / 0	0 / 0	0 / 0	29 / 0	0 / 0
H	374	212 / 0	87 / 0	0 / 0	0 / 0	75 / 0	0 / 0
G	412	247 / 0	87 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO												
A-B	0 / 20	-122.2	-122.2	0.03 (1)	10.00							
B-J	-82 / 0	-122.2	-122.2	0.05 (1)	6.25							
J-C	-170 / 0	-122.2	-122.2	0.11 (1)	6.25							
C-D	-80 / 0	-122.2	-122.2	0.47 (1)	6.25							
D-L	-133 / 0	-122.2	-122.2	0.11 (1)	6.25							
L-E	-42 / 0	-122.2	-122.2	0.04 (1)	6.25							
E-F	0 / 20	-122.2	-122.2	0.03 (1)	10.00							
B-I	0 / 124	-28.0	-28.0	0.11 (1)	10.00							
I-H	0 / 124	-28.0	-28.0	0.11 (2)	10.00							
H-G	0 / 110	-28.0	-28.0	0.11 (2)	10.00							
G-K	0 / 95	-28.0	-28.0	0.11 (2)	10.00							
K-E	0 / 95	-28.0	-28.0	0.11 (1)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	58.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47 (C-D:1), BC=0.11 (H-I:2), WB=0.06 (D-G:1), SSI=0.23 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



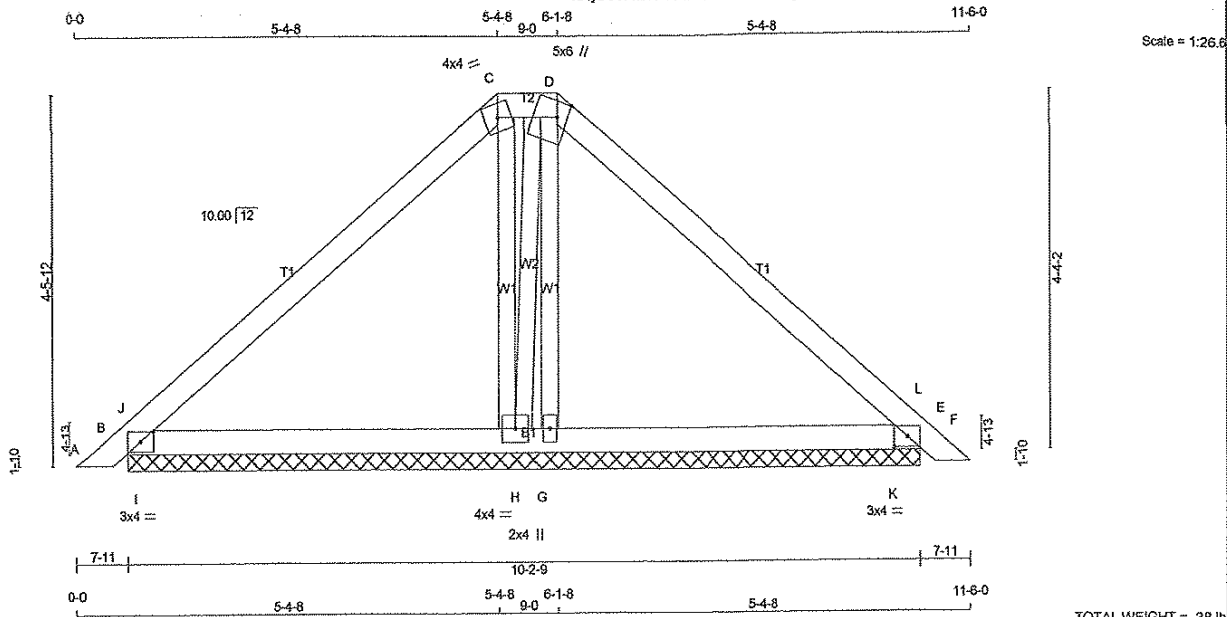
DRWG NO. TAM 45707-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272848	P9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 38 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
B - E	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	586	0	586	0	10-2-9	10-2-9
E	580	0	560	0	10-2-9	10-2-9
H	344	0	344	0	10-2-9	10-2-9
G	198	0	198	0	10-2-9	10-2-9

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	425	307 / 0	56 / 0	0 / 0	0 / 0	62 / 0	0 / 0
E	420	305 / 0	55 / 0	0 / 0	0 / 0	61 / 0	0 / 0
H	279	164 / 0	61 / 0	0 / 0	0 / 0	54 / 0	0 / 0
G	166	89 / 0	42 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 20	-122.2	-122.2	0.03 (1)	H-C	-123 / 14	0.03 (1)
B-J	-175 / 100	-122.2	-122.2	0.19 (1)	H-D	-60 / 8	0.02 (1)
J-C	-280 / 0	-122.2	-122.2	0.34 (1)	G-D	-63 / 8	0.02 (1)
C-D	-191 / 0	-122.2	-122.2	0.01 (1)	I-J	-804 / 140	0.00 (1)
D-L	-279 / 0	-122.2	-122.2	0.34 (1)	K-L	-829 / 121	0.00 (1)
L-E	-161 / 120	-122.2	-122.2	0.21 (1)			
E-F	0 / 20	-122.2	-122.2	0.03 (1)			
B-I	0 / 200	-28.0	-28.0	0.30 (1)			
I-H	0 / 200	-28.0	-28.0	0.30 (1)			
H-G	0 / 198	-28.0	-28.0	0.26 (1)			
G-K	0 / 199	-28.0	-28.0	0.30 (1)			
K-E	0 / 199	-28.0	-28.0	0.30 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	58.7	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-J-1), BC=0.30 (G-K-1), WB=0.03 (C-H-1), ESI=0.84 (E-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

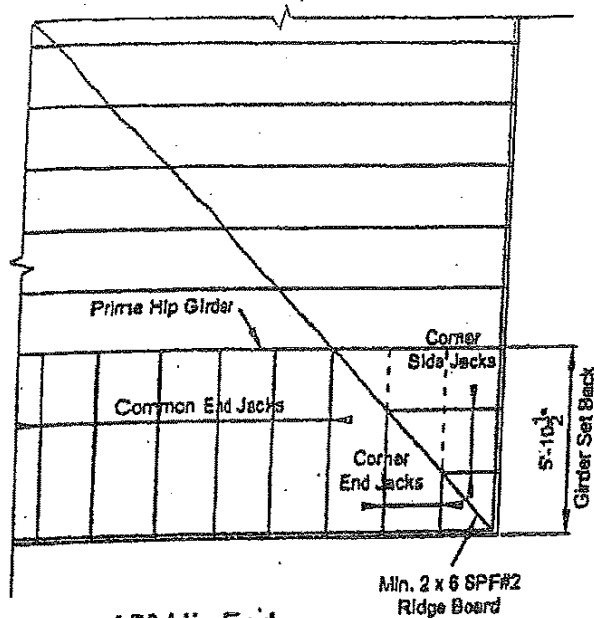
JSI GRIP= 0.47 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



DRWG NO. TAM 4570817
STRUCTURAL
COMPONENT ONLY

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

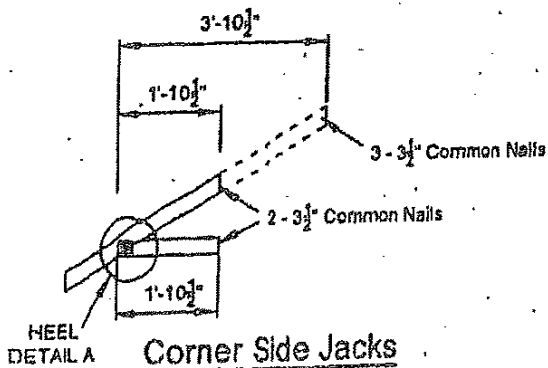
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD:

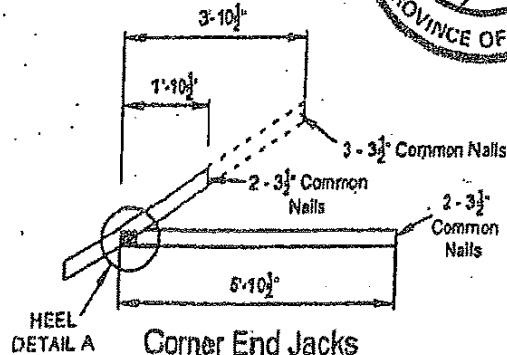
TOP CHORD LIVE LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

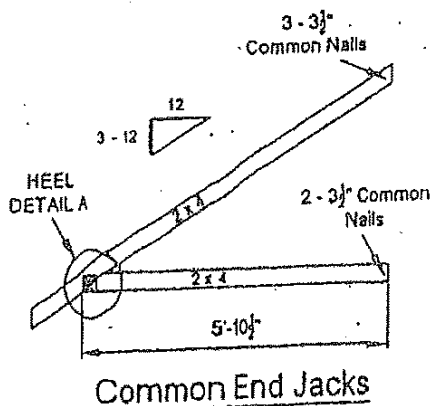
DWG NO TAM 3495.14
 STRUCTURAL
 COMPONENT ONLY



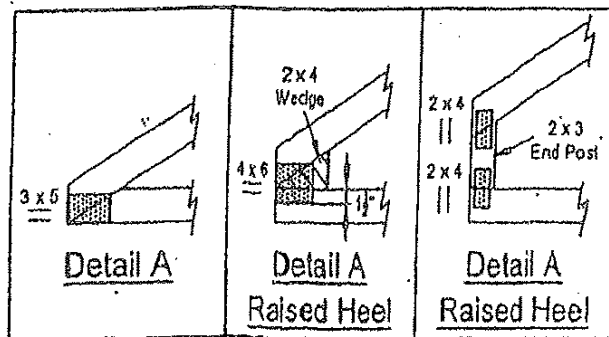
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
 (TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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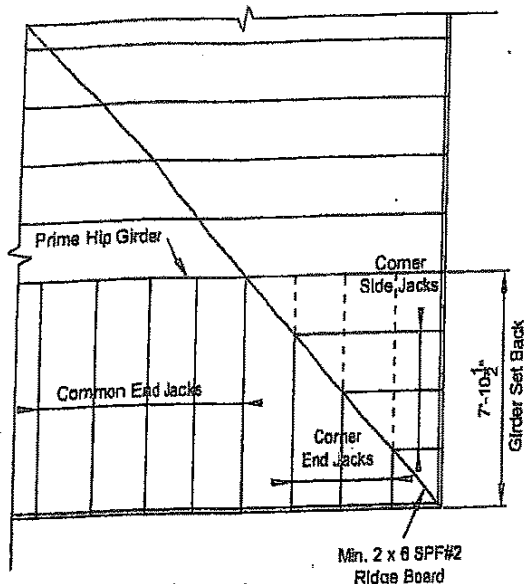
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

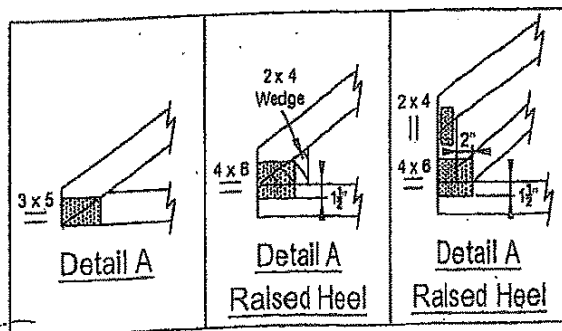
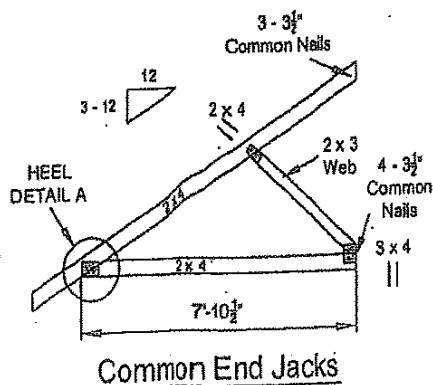
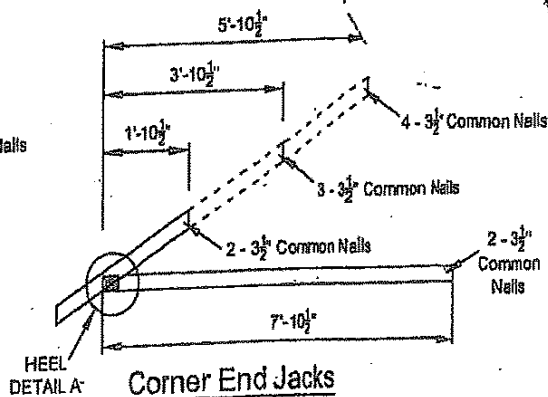
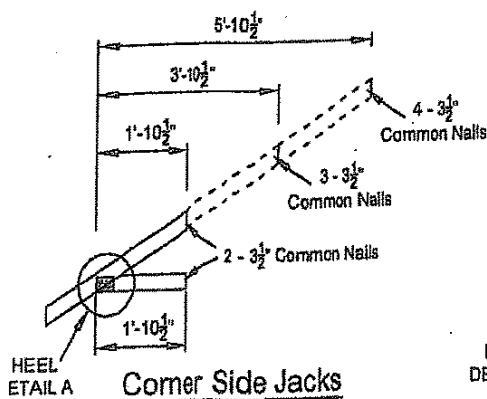
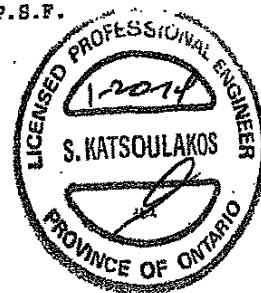
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.



45° Hip End

DWG NO TAM 3503 14
STRUCTURAL
COMPONENT ONLY



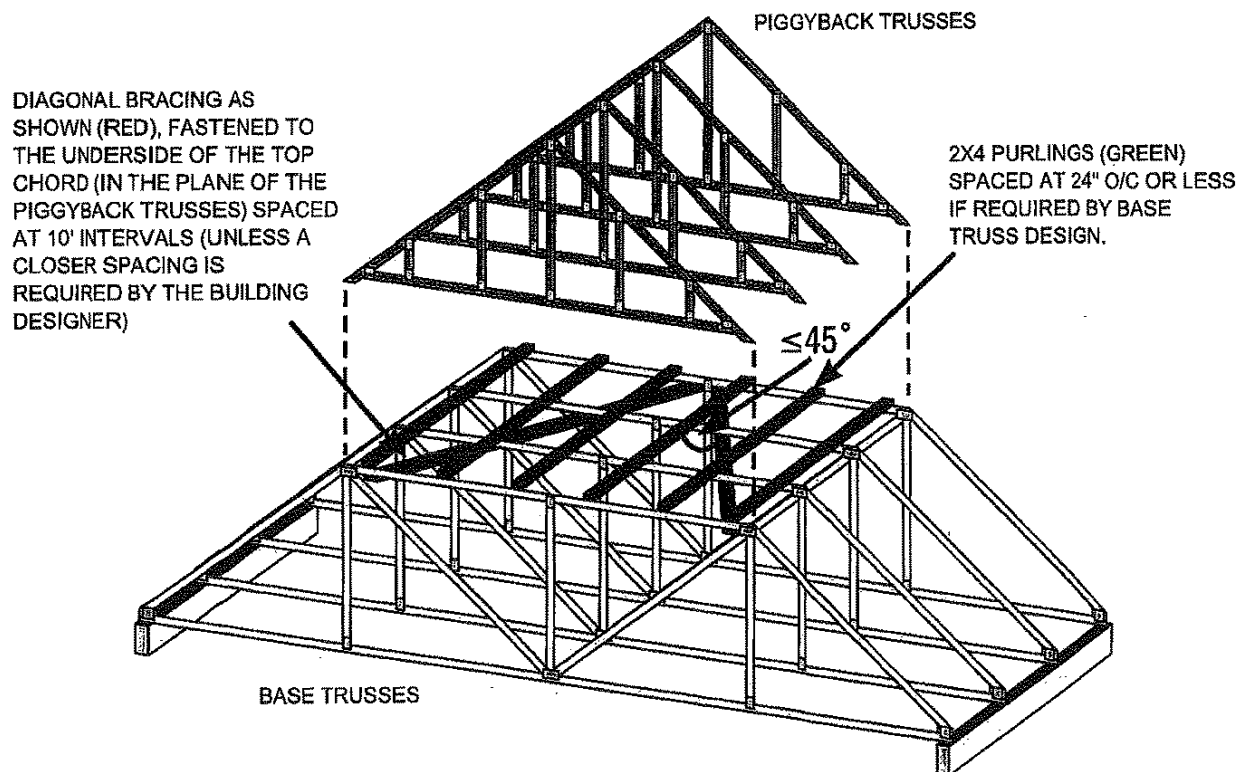
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

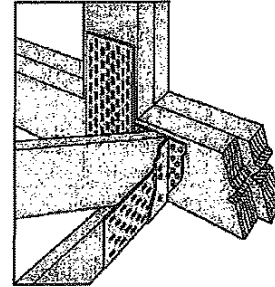
FACE MOUNT HANGERS

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

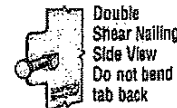
Model No.	Ga	Dimensions (In.)				Fasteners		Factored Resistance			
		W	H	B	d _g ^a	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lbs	lbs	lbs	lbs
								kN	kN	kN	kN
DOUBLE 2x SIZES											
LUS24-2	18	3 3/8	3 3/8	2	1 1/2	4-16d	2-16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
LUS26-2	18	3 3/8	4 1/4	2	4	4-16d	4-16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 3/8	5 1/4	3	3 3/8	14-16d	6-16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 3/8	5 1/4	4	4 1/4	20-16d	8-16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3 3/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 3/8	7 1/4	3	5 1/4	22-16d	8-16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 3/8	7 1/4	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 3/8	9	2	8	8-16d	6-16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 3/8	9 1/4	3	8	30-16d	10-16d	4745	9660	4310	7000
								21.11	42.97	19.17	31.14
HGUS210-2	12	3 3/8	9 1/4	4	8 1/4	46-16d	16-16d	6840	14845	4855	10400
								30.43	65.14	21.60	46.26
TRIPLE 2x SIZES											
HGUS26-3	12	4 1/8	5 1/4	4	4 1/4	20-16d	8-16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/4	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/4	30-16d	10-16d	4745	10545	4310	7485
								21.11	46.91	19.17	33.29
HGUS210-3	12	4 1/8	9 1/4	4	8 1/4	46-16d	16-16d	6840	14845	4855	10400
								30.43	65.14	21.60	46.26
QUADRUPLE 2x SIZES											
HGUS26-4	12	6 1/8	5 1/4	4	4 1/4	20-16d	8-16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 1/4	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	9	3	7 1/4	30-16d	10-16d	4745	10545	4310	7485
								21.11	46.91	19.17	33.29
HGUS210-4	12	6 1/8	9 1/4	4	8 1/4	46-16d	16-16d	6840	14845	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/4	4	10 1/4	56-16d	20-16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/4	4	11 1/4	66-16d	22-16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x SIZES											
LUS46	18	3 3/8	4 1/4	2	3 3/8	4-16d	4-16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 3/8	5 1/4	3	3 3/8	14-16d	6-16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 3/8	5 1/4	4	4 1/4	20-16d	8-16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 3/8	6 1/4	2	3 3/8	6-16d	4-16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 3/8	7 1/4	3	6 1/4	22-16d	8-16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 3/8	7 1/4	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 3/8	8 1/4	2	5 1/4	8-16d	6-16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 3/8	9	4	8 1/4	46-16d	16-16d	6840	14845	4855	10400
								30.43	65.14	21.60	46.26
HGUS412	12	3 3/8	10 1/4	4	10 1/4	56-16d	20-16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 3/8	12 1/4	4	11 1/4	66-16d	22-16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80



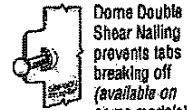
Typical HUS26 Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)



Double Shear Nailing Top View



Double Shear Nailing Side View
Do not bend tab back



Dome Double Shear Nailing prevents tabs breaking off (available on some models)



- Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
- Designer must ensure that hanger is compatible with truss when reduced heel height is used.
- dg is the distance from the bearing seat to the top joist nail.
- Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-N10FORTRUSS and/or see installation notes.
- NAILS: 16d = 0.162" dia. x 3 3/4" long. See pages 22-23 for other nail sizes and information.

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS Standard & Double Shear Joist Hangers

SIMPSON
Strong-Tie

This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed cost,
or a combination of these features.

Most hangers in this series have double shear nailing – an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

MATERIAL: See tables below and on page 155

FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating;

see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

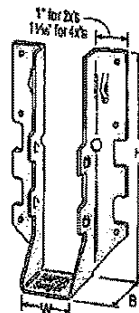
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With 3x carrying members, use 16d x 2 1/4" nails into the header and 16d commons into the joist with no reduction in resistance. With 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value.

OPTIONS: • LUS and LUL hangers cannot be modified.

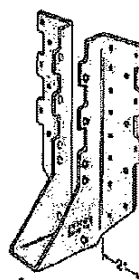
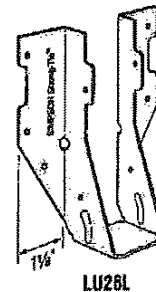
- HUS hangers available with the header flanges turned in for 2-2x (3 1/4") and 4x only, with no load reduction. See HUSC Concealed Flange Illustration.
- Concealed flanges are not available for HGUS.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See hanger options on page 230.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

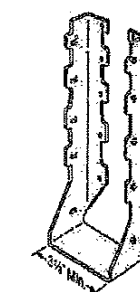
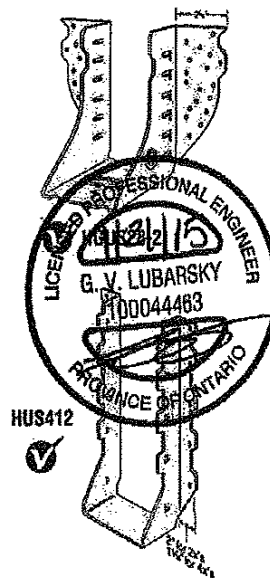
These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.



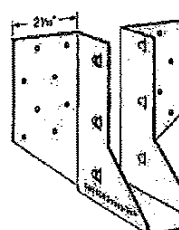
LUS28

HUS210
(HUS26, HUS28,
and HHUS similar)

LU26L

HUSC
Concealed Flanges
(not available for HHUS,
HGUS and HUS2x)

HUS412



LJS26DS

Model No.	Ga	Dimensions (In)				Fasteners		Factored Resistance			
		W	H	B	d _a ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _p = 1.15)	(K _p = 1.00)	(K _p = 1.15)	(K _p = 1.00)
								lbs	lbs	lbs	lbs
				kN	kN	kN	kN				
SINGLE 2x SIZES											
LUS24	18	1½	3¼	1¼	2¼	4-10d	2-10d	710	1625	645	1155
								316	7.23	2.87	5.14
LU24L	22	1½	3	1¼	2½	4-10d	2-10dx1½	360	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1½	5	1¼	4¾	6-10d	4-10dx1½	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1½	4¾	1¼	3¾	4-10d	4-10d	1420	2170	1290	1690
								6.32	9.65	5.74	7.25
HUS26	16	1½	5¾	3	3¾	14-16d	6-16d	2705	4940	2085	3875
								11.30	21.97	9.20	17.24
LJS26DS	18	1½	5	3½	4¾	16-16d	6-16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1½	5¾	5	4¾	20-16d	8-16d	2685	6625	2685	5700
								11.96	29.51	11.96	25.35
LU28L	20	1½	6¾	1¼	5¾	8-10d	8-10dx1½	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1½	6¾	1¼	3¾	8-10d	4-10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1½	7¾	3	6¾	22-16d	8-16d	3605	5365	2675	4345
								16.04	23.88	11.90	19.33
HGUS28	12	1½	7¾	5	6¾	36-16d	12-16d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
LU210L	20	1½	8	1¼	7¾	10-10d	6-10dx1½	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1½	7¾	1¼	3¾	8-10d	4-10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

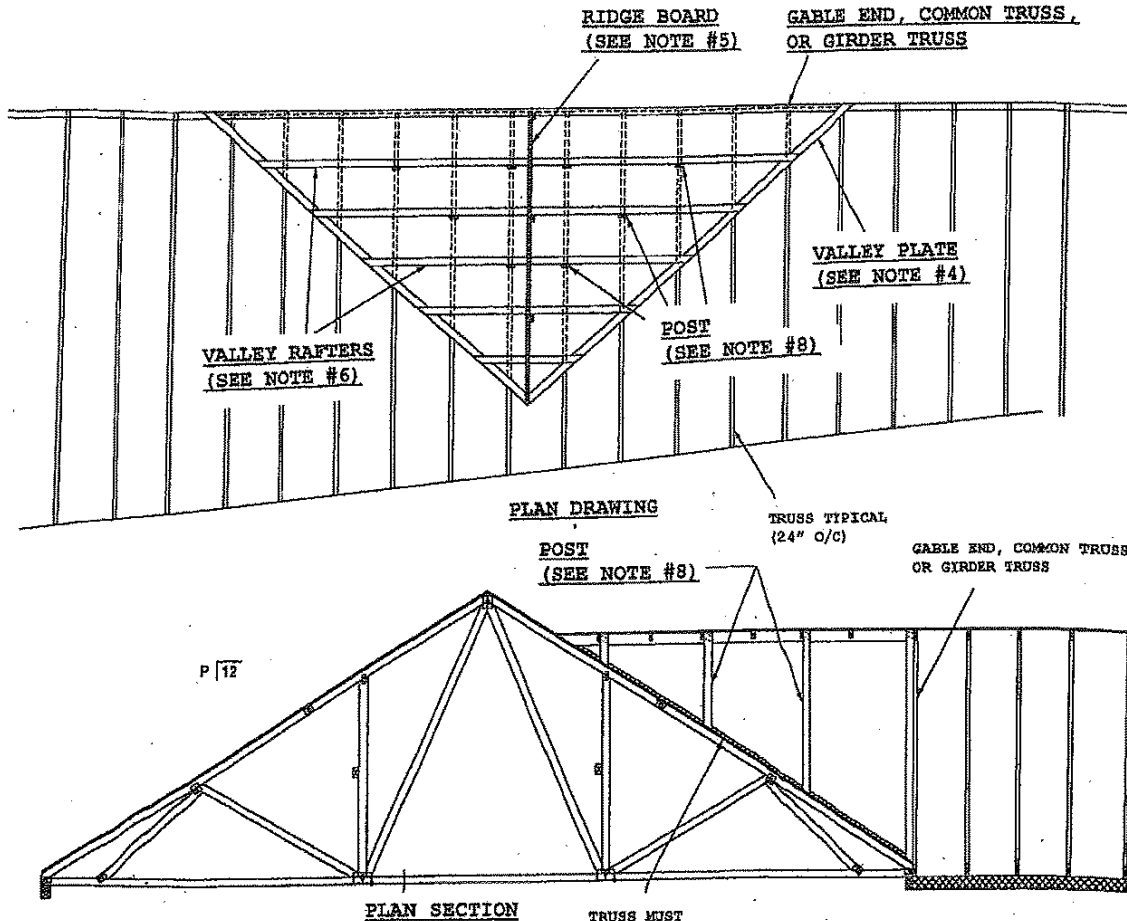
See footnotes on page 155.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BEAM. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUGGER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

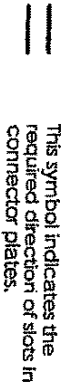
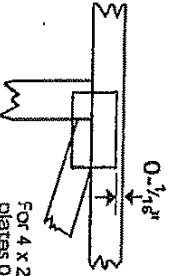
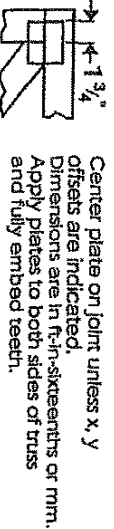
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.9 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAN 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in Mitek software or upon request.

PLATE SIZE

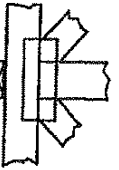
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



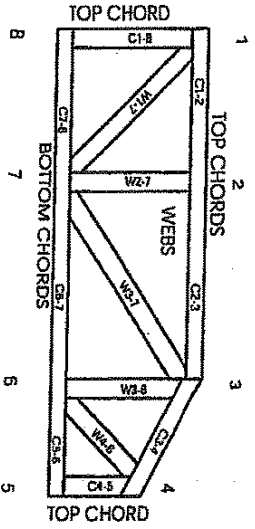
BEARING



Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in feet-inches-eighths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:
11996-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet Mill-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

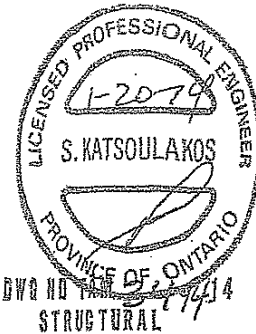
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For truss bracing, individual lateral braces themselves may require bracing, or alternative I, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plying requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of the design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario

NOL 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.