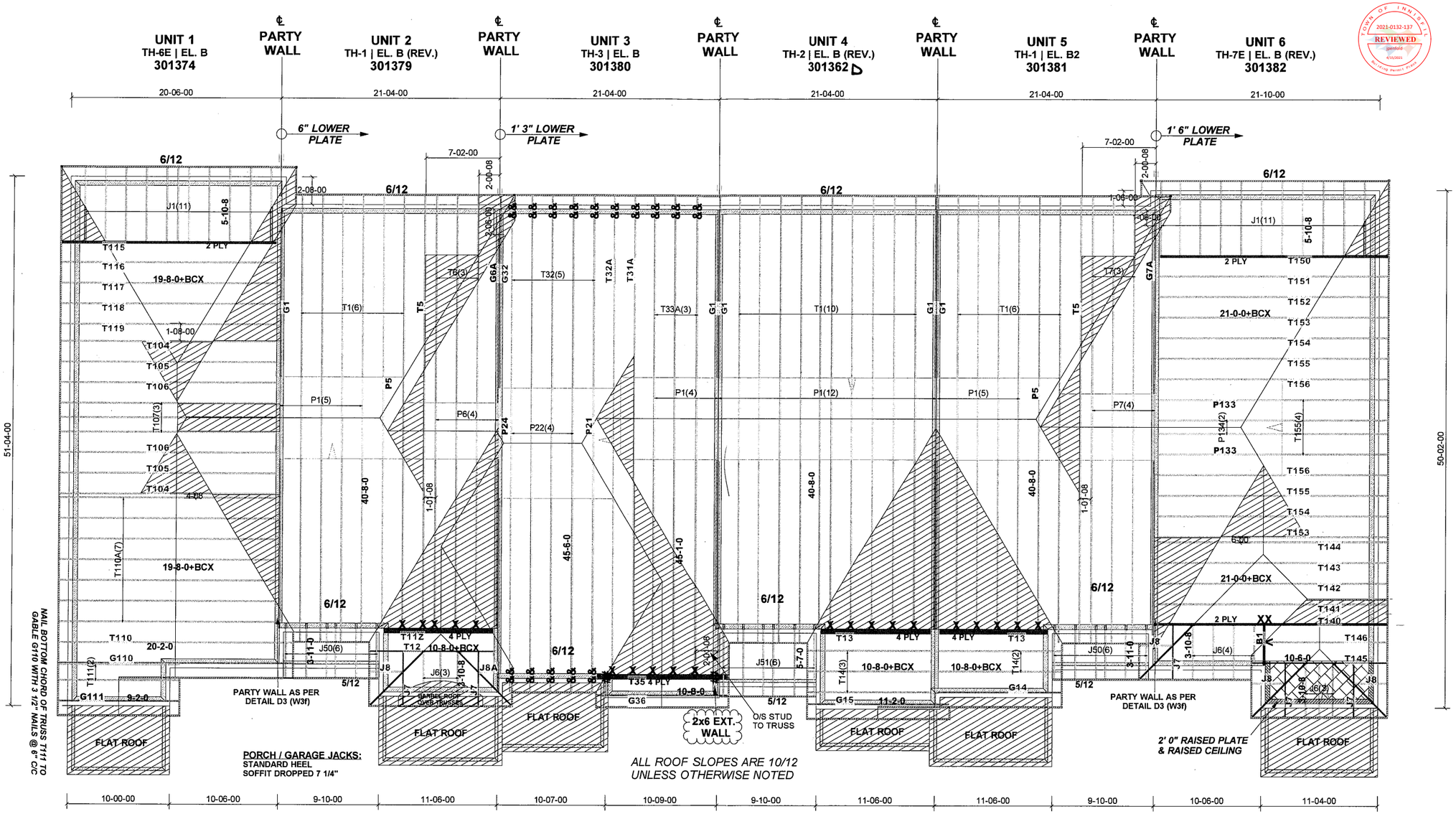


ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE (BRICK)
2" CLADDING ALLOWANCE (STUCCO)
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

T - 180523

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012 / 2018
OCCUPANCY: RESIDENTIAL | PART: 9
GROUND SNOW LOAD Ss=54.3psf Sr=8.4psf
DESIGN LOADS
TCDL (6psf)
BCLL (10.5psf)
BCDL (7psf)

DENOTES:
CONVENTIONAL FRAMING

HARDWARE:
LJS26DS - (**V**)
HGUS26 - (**X**)
HGUS26-2 - (**XX**)
TS22 x2 - (**##**)
TBE6 x2 - (**&&**)

BEAMS:
B1: 2 - 2x10 SPF #2

M11,903

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 146 / UNITS 1-6	Mitek ver 7.5.0
Layout ID: 297085	Project: ALCONA SHORES		
Plan Log: 97625	Date: 08/24/18	Designer: colinh	

TAMARACK LUMBER INC
ALPA LUMBER GROUP

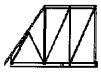
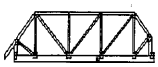
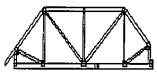
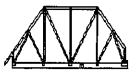
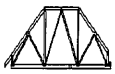
Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 301374 **LOCATION:** INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO **SUB-BUILDER:**
MODEL: TH-6E **ELEVATION:** B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	**T104** HALF HIP	10.00 / 0.00	**19-08-00**	07-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	226.64 / 143.66		
	2	**T105** HALF HIP	10.00 / 0.00	**19-08-00**	08-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	218.46 / 138.00		
	2	**T106** PIGGYBACK	10.00 / 0.00	**19-08-00**	09-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	246.18 / 153.00		
	3	**T107** HALF HIP	10.00 / 0.00	**19-08-00**	10-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	405.54 / 253.50		
	1	**T110** COMMON	10.00 / 0.00	**20-02-00**	10-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	103.31 / 65.33		
	1	**G110** COMMON	10.00 / 0.00	**20-02-00**	10-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	105.59 / 69.00		
	7	**T110A** COMMON	10.00 / 0.00	**19-08-00**	10-00-08	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	791.21 / 490.00		
	2	**T111** COMMON	10.00 / 0.00	**09-02-00**	05-05-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	84.44 / 55.00		
	1	**G111** COMMON	10.00 / 0.00	**09-02-00**	05-05-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	46.71 / 31.50		
	1 / 2 Ply	**T115** HIP GIRDER	10.00 / 0.00	**19-08-00**	04-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	194.04 / 124.66		
	1	**T116** HIP	10.00 / 0.00	**19-08-00**	05-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	95.24 / 61.00		
	1	**T117** HIP	10.00 / 0.00	**19-08-00**	06-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	104.69 / 66.67		
	1	**T118** HIP	10.00 / 0.00	**19-08-00**	07-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	105.65 / 68.00		
	1	**T119** HIP	10.00 / 0.00	**19-08-00**	08-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	105.85 / 68.50		
	11	**J1** JACK-OPEN	6.00 / 0.00	**05-10-08**	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	184.69 / 117.37		

TOTAL # TRUSS= **38.00** TOTAL BFT OF ALL TRUSSES= **1905.19** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3018.24** LBS.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

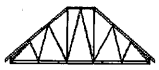
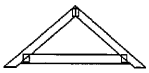
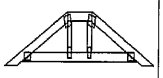
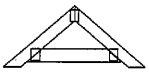
Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301379 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	6	T1 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	1252.08 / 757.98		
	1	G1 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	217.69 / 133.50		
	1	T5 HIP	6.00 / 0.00	40-08-00	07-08-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	206.59 / 127.83		
	3	T6 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 00-00-00	01-02-00 / 01-02-00	633.30 / 390.51		
	1	G6A PIGGYBACK	6.00 / 0.00	38-02-00	09-04-03	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 01-02-00	200.07 / 124.33		
	1 / 4 Ply	T11Z COMMON	10.00 / 0.00	10-08-00	06-03-08	2 X 6	2 X 8	00-00-00 / 00-00-00	01-07-11 / 00-07-04	332.08 / 208.00		
	1	T12 HIP GIRDER	10.00 / 0.00	10-08-00	04-10-07	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 00-05-08	56.88 / 38.00		
	5	P1 PIGGYBACK	6.00 / 0.00	06-00-10	01-10-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	85.10 / 56.65		
	1	P5 PIGGYBACK	6.00 / 0.00	06-00-10	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	18.42 / 12.33		
	4	P6 PIGGYBACK	6.00 / 0.00	03-06-10	01-02-13	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	43.96 / 32.00		
	3	J6 JACK-OPEN	10.00 / 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-10-07	45.69 / 29.01		
	2	J7 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 02-01-01	01-07-11 / 00-03-08	21.30 / 14.66		
	1	J8 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 00-01-01	01-07-11 / 00-03-08	8.37 / 6.00		
	1	J8A JACK-OPEN	10.00 / 0.00	01-04-08	03-01-09	2 X 4	2 X 4	00-00-00 / -00-01-01	00-03-08 / 00-03-08	6.47 / 4.67		
	6	J50 JACK-OPEN	5.00 / 0.00	03-11-00	02-00-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-05-06 / 02-00-05	66.24 / 40.02		

TOTAL # TRUSS= **40.00** TOTAL BFT OF ALL TRUSSES= **1975.49** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3194.24** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	

TOTAL # ITEMS= **5.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

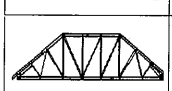
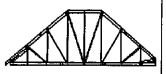
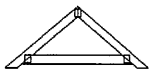
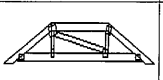
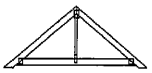
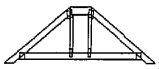
Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301380 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G1 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	217.69 / 133.50		
	1	T31A HIP	6.00 / 0.00	45-01-00	08-02-07	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 01-04-08	264.47 / 159.33		
	5	T32 PIGGYBACK	6.00 / 0.00	45-06-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	1436.65 / 850.85		
	1	G32 PIGGYBACK	6.00 / 0.00	45-06-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	287.34 / 181.17		
	1	T32A HIP	6.00 / 0.00	45-01-00	09-10-07	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 01-04-08	281.51 / 169.17		
	3	T33A PIGGYBACK	6.00 / 0.00	45-01-00	09-04-03	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 03-02-07	835.83 / 500.49		
	1 / 4 Ply	T35 COMMON	10.00 / 0.00	10-08-00	05-10-08	2 X 6	2 X 6	01-03-08 / 01-00-08	01-05-03 / 01-05-03	320.48 / 201.32		
	1	G36 COMMON	10.00 / 0.00	10-08-00	05-10-08	2 X 4	2 X 4	01-00-08 / 01-00-08	01-05-03 / 01-05-03	49.49 / 31.67		
	4	P1 PIGGYBACK	6.00 / 0.00	06-00-10	01-10-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	68.08 / 45.32		
	1	P21 PIGGYBACK	6.00 / 0.00	08-09-10	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	27.72 / 17.67		
	4	P22 PIGGYBACK	6.00 / 0.00	08-09-10	02-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	100.92 / 60.00		
	1	P24 PIGGYBACK	6.00 / 0.00	08-09-10	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	26.04 / 17.33		

TOTAL # TRUSS= **27.00** TOTAL BFT OF ALL TRUSSES= **2367.82** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3916.22** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	
30	Hangers	TBE6	
4	Hangers	TS22	

TOTAL # ITEMS= **39.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

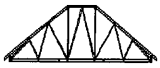
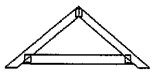
Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 301362 **LOCATION:** INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO **SUB-BUILDER:**
MODEL: TH-2 **ELEVATION:** B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	10	T1 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	2086.80 / 1263.30		
	2	G1 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	435.38 / 267.00		
	1 / 4 Ply	T13 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 6	2 X 8	00-00-00 / 00-00-00	01-07-11 / 00-07-04	395.20 / 249.32		
	3	T14 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 00-03-08	174.93 / 111.99		
	1	G15 COMMON	10.00 / 0.00	11-02-00	06-03-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	54.26 / 37.00		
	12	P1 PIGGYBACK	6.00 / 0.00	06-00-10	01-10-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	204.24 / 135.96		
	6	J51 JACK-OPEN	5.00 / 0.00	05-07-00	02-09-05	2 X 4	2 X 4	01-03-08 / 00-00-00	00-05-06 / 02-08-11	90.00 / 55.98		

TOTAL # TRUSS= **38.00** TOTAL BFT OF ALL TRUSSES= **2120.55** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3440.81** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	

TOTAL # ITEMS= **5.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

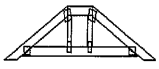
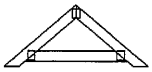
DATE	08/24/18
SALES REP	Mario

REVIEWED

JOB TRACK: 42067 **LAYOUT ID:** 301381 **LOCATION:** INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO **SUB-BUILDER:**
MODEL: TH-1 **ELEVATION:** B2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	6	T1 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	1252.08 / 757.98		
	1	G1 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	217.69 / 133.50		
	1	T5 HIP	6.00 / 0.00	40-08-00	07-08-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	206.59 / 127.83		
	3	T7 PIGGYBACK	6.00 / 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	647.46 / 401.01		
	1	G7A PIGGYBACK	6.00 / 0.00	39-02-00	09-04-03	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	213.29 / 130.83		
	1 / 4 Ply	T13 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 6	2 X 6	00-00-00 / 00-00-00	01-07-11 / 00-05-08	374.76 / 233.32		
	2	T14 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 00-03-08	116.62 / 74.66		
	1	G14 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 10-06-06	67.19 / 42.50		
	5	P1 PIGGYBACK	6.00 / 0.00	06-00-10	01-10-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	85.10 / 56.65		
	1	P5 PIGGYBACK	6.00 / 0.00	06-00-10	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	18.42 / 12.33		
	4	P7 PIGGYBACK	6.00 / 0.00	04-06-10	01-05-13	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	53.60 / 34.68		
	6	J50 JACK-OPEN	5.00 / 0.00	03-11-00	02-00-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-05-06 / 02-00-05	66.24 / 40.02		

TOTAL # TRUSS= **35.00** TOTAL BFT OF ALL TRUSSES= **2045.31** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3319.04** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	

TOTAL # ITEMS= **5.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

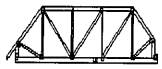
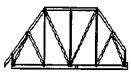
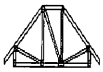
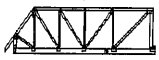
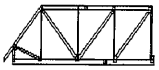
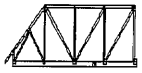
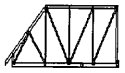
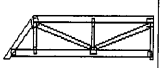
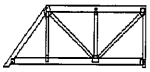
Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301382 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-7E ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T140 HIP GIRDER	10.00 0.00	21-00-00	04-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	212.52 134.00		
	1	T141 HIP	10.00 0.00	21-00-00	06-06-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	111.88 70.50		
	1	T142 HIP	10.00 0.00	21-00-00	08-02-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	116.24 72.67		
	1	T143 HIP	10.00 0.00	21-00-00	09-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	124.06 78.33		
	1	T144 COMMON	10.00 0.00	21-00-00	10-07-03	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	119.94 74.00		
	1	T145 HIP GIRDER	10.00 0.00	10-06-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	50.89 32.83		
	1	T146 COMMON	10.00 0.00	10-06-00	08-00-03	2 X 4	2 X 4	01-03-08 01-03-08	03-07-11 03-07-11	54.75 34.67		
	1 2 Ply	T150 HALF HIP	10.00 0.00	21-00-00	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	209.48 132.66		
	1	T151 HALF HIP	10.00 0.00	21-00-00	05-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	103.13 64.50		
	1	T152 HALF HIP	10.00 0.00	21-00-00	06-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	111.36 70.17		
	2	T153 HALF HIP	10.00 0.00	21-00-00	07-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	235.70 147.00		
	2	T154 HALF HIP	10.00 0.00	21-00-00	08-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	249.28 157.66		
	6	T155 HALF HIP	10.00 0.00	21-00-00	09-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	766.92 481.02		
	2	T156 HALF HIP	10.00 0.00	21-00-00	10-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	279.54 172.34		
	2	P133 PIGGYBACK	10.00 0.00	11-04-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	77.72 51.34		
	2	P134 PIGGYBACK	10.00 0.00	11-04-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	83.60 54.66		
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	7	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	106.61 67.69		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301382 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-7E ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	J7 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	31.95 21.99		
	3	J8 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 00-01-01	01-07-11 00-03-08	25.11 18.00		

TOTAL # TRUSS= **52.00** TOTAL BFT OF ALL TRUSSES= **2053.40** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3255.37** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LJS26DS	
1	Hangers	HGUS26-2	

TOTAL # ITEMS= **2.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T1	42	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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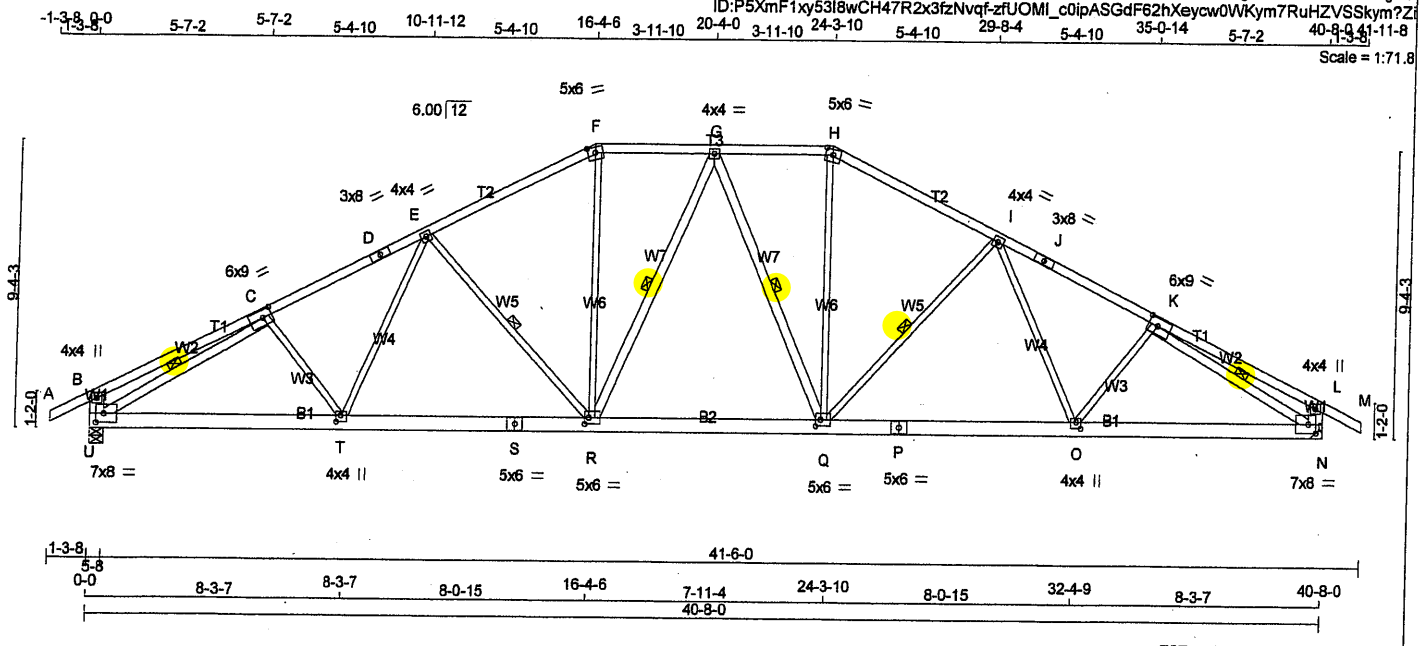

TOTAL WEIGHT = 42 X 209 = 8765 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
G - Q	2x4	DRY	No.2	SPF
U - C	2x4	DRY	2100F 1.8E	SPF
K - N	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	6.0	9.0	3.00	3.75
D	TS-t	MT20	3.0	8.0		
E, G, I						
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	5.0	6.0	2.25	3.00
H	TTW-m	MT20	5.0	6.0	2.25	3.00
J	TS-t	MT20	3.0	8.0		
K	TMWW-t	MT20	6.0	9.0	3.00	3.75
L	TMV+p	MT20	4.0	4.0		
N	BMVW1-t	MT20	7.0	8.0	3.50	3.00
O	BMWW+t	MT20	4.0	4.0	2.50	1.75
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0	2.50	2.00
R	BMWWW-t	MT20	5.0	6.0	2.50	1.50
S	BS-t	MT20	5.0	6.0		
T	BMWW+t	MT20	4.0	4.0	2.50	1.75
U	BMVW1-t	MT20	7.0	8.0	3.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3595	0	3595	0	0	5-8	5-8
N	3595	0	3595	0	0	MECHANICAL - HGUS26	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2630	1658 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0
N	2630	1658 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, G-R, G-Q, I-Q, C-U, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 39	-129.7	-129.7	0.17 (1)	10.00
B-C	0 / 28	-129.7	-129.7	0.47 (1)	10.00
C-D	-5144 / 0	-129.7	-129.7	0.88 (1)	2.43
D-E	-5144 / 0	-129.7	-129.7	0.88 (1)	2.43
E-F	-4215 / 0	-129.7	-129.7	0.78 (1)	2.78
F-G	-3756 / 0	-129.7	-129.7	0.40 (1)	3.31
G-H	-3756 / 0	-129.7	-129.7	0.40 (1)	3.31
H-I	-4215 / 0	-129.7	-129.7	0.78 (1)	2.78
I-J	-5144 / 0	-129.7	-129.7	0.88 (1)	2.43
J-K	-5144 / 0	-129.7	-129.7	0.88 (1)	2.43
K-L	0 / 28	-129.7	-129.7	0.47 (1)	10.00
L-M	0 / 39	-129.7	-129.7	0.17 (1)	10.00
U-B	-468 / 0	0.0	0.0	0.03 (1)	7.81
N-L	-468 / 0	0.0	0.0	0.03 (1)	7.81
U-T	0 / 4661	-38.5	-38.5	0.72 (1)	10.00
T-S	0 / 4426	-38.5	-38.5	0.65 (1)	10.00
S-R	0 / 4426	-38.5	-38.5	0.65 (1)	10.00
R-Q	0 / 3887	-38.5	-38.5	0.59 (1)	10.00
Q-P	0 / 4426	-38.5	-38.5	0.65 (1)	10.00
P-O	0 / 4426	-38.5	-38.5	0.65 (1)	10.00
O-N	0 / 4661	-38.5	-38.5	0.72 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-T	-95 / 144	0.03 (3)
T-E	0 / 468	0.11 (2)
E-R	-1025 / 0	0.48 (1)
R-F	0 / 1378	0.31 (1)
R-G	-332 / 0	0.16 (1)
G-Q	-332 / 0	0.16 (1)
Q-H	0 / 1378	0.31 (1)
Q-I	-1025 / 0	0.48 (1)
I-O	0 / 468	0.11 (2)
O-K	-95 / 144	0.03 (3)
U-C	-5537 / 0	0.67 (1)
K-N	-5537 / 0	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.42")

CSI: TC=0.88/1.00 (I-K:1) , BC=0.72/1.00 (N-O:1) , WB=0.67/1.00 (C-U:1) , SSI=0.30/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (R) (INPUT = 0.90)
SSI METAL= 0.91 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805174
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	G1	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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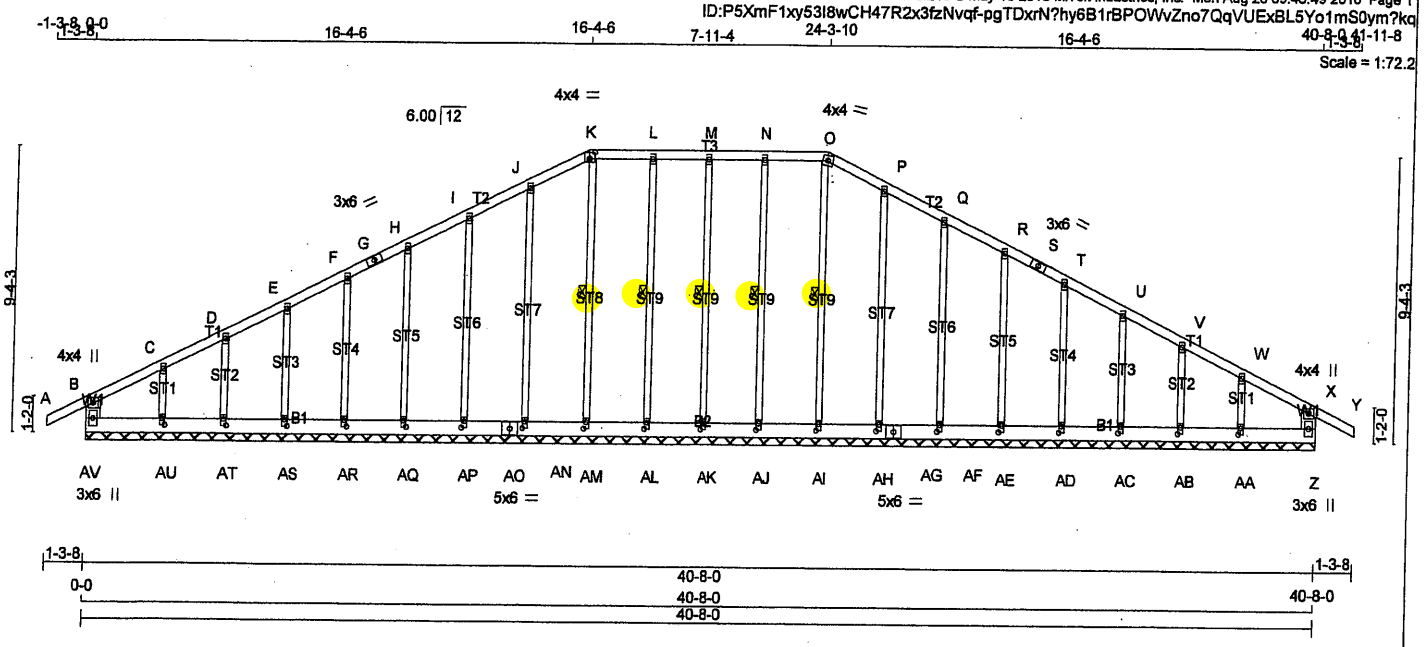

TOTAL WEIGHT = 8 X 218 = 1742 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AV- B	2x6	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - O	2x4	DRY	No.2	SPF
O - S	2x4	DRY	No.2	SPF
S - Y	2x4	DRY	No.2	SPF
Z - X	2x6	DRY	No.2	SPF
AV- AO	2x6	DRY	No.2	SPF
AO- AG	2x6	DRY	No.2	SPF
AG- Z	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C, D, E, F, H, I, J, L, M, N, P, Q, R, T, U, V, W						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
K	TTW-l	MT20	4.0	4.0	2.50	2.00
O	TTW-m	MT20	4.0	4.0		
S	TS-t	MT20	3.0	6.0		
X	TMV+p	MT20	4.0	4.0		
Z	BMV1+p	MT20	3.0	6.0		
AA, AB, AC, AD, AE, AF, AH, AI, AJ, AK, AL, AM, AN, AP, AQ, AR, AS, AT, AU						
AA	BMW1+w	MT20	2.0	4.0	2.50	1.00
AG	BS-t	MT20	5.0	6.0		
AO	BS-t	MT20	5.0	6.0		
AV	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF O-AI, N-AJ, M-AK, L-AL, K-AM.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO								
AV- B	-367 / 0	0.0	0.0	0.02 (1)	7.81	AI- O	-253 / 0	0.14 (1)
A- B	0 / 39	-129.7	-129.7	0.17 (1)	10.00	AJ- N	-259 / 0	0.14 (1)
B- C	-66 / 0	-129.7	-129.7	0.09 (1)	6.25	AK- M	-235 / 0	0.13 (1)
C- D	-54 / 0	-129.7	-129.7	0.07 (1)	6.25	AL- L	-259 / 0	0.14 (1)
D- E	-45 / 0	-129.7	-129.7	0.06 (1)	6.25	AM- K	-253 / 0	0.14 (1)
E- F	-41 / 0	-129.7	-129.7	0.06 (1)	6.25	AN- J	-253 / 0	0.34 (1)
F- G	-37 / 0	-129.7	-129.7	0.06 (1)	6.25	AP- I	-259 / 0	0.24 (1)
G- H	-37 / 0	-129.7	-129.7	0.06 (1)	6.25	AQ- H	-258 / 0	0.16 (1)
H- I	-34 / 0	-129.7	-129.7	0.06 (1)	6.25	AR- F	-257 / 0	0.11 (1)
I- J	-32 / 0	-129.7	-129.7	0.06 (1)	6.25	AS- E	-259 / 0	0.07 (1)
J- K	-28 / 0	-129.7	-129.7	0.06 (1)	6.25	AT- D	-250 / 0	0.05 (1)
K- L	-23 / 0	-129.7	-129.7	0.06 (1)	6.25	AU- C	-279 / 0	0.04 (1)
L- M	-23 / 0	-129.7	-129.7	0.06 (1)	6.25	AH- P	-253 / 0	0.34 (1)
M- N	-23 / 0	-129.7	-129.7	0.06 (1)	6.25	AF- Q	-259 / 0	0.24 (1)
N- O	-23 / 0	-129.7	-129.7	0.06 (1)	6.25	AE- R	-258 / 0	0.16 (1)
O- P	-28 / 0	-129.7	-129.7	0.06 (1)	6.25	AD- T	-257 / 0	0.11 (1)
P- Q	-32 / 0	-129.7	-129.7	0.06 (1)	6.25	AC- U	-259 / 0	0.07 (1)
Q- R	-34 / 0	-129.7	-129.7	0.06 (1)	6.25	AB- V	-250 / 0	0.05 (1)
R- S	-37 / 0	-129.7	-129.7	0.06 (1)	6.25	AA- W	-279 / 0	0.04 (1)
S- T	-37 / 0	-129.7	-129.7	0.06 (1)	6.25			
T- U	-41 / 0	-129.7	-129.7	0.06 (1)	6.25			
U- V	-45 / 0	-129.7	-129.7	0.06 (1)	6.25			
V- W	-54 / 0	-129.7	-129.7	0.07 (1)	6.25			
W- X	-66 / 0	-129.7	-129.7	0.09 (1)	6.25			
X- Y	0 / 39	-129.7	-129.7	0.17 (1)	10.00			
Z- X	-367 / 0	0.0	0.0	0.02 (1)	7.81			
AV-AU	0 / 56	-38.5	-38.5	0.02 (1)	10.00			
AU-AT	0 / 46	-38.5	-38.5	0.02 (2)	10.00			
AT-AS	0 / 41	-38.5	-38.5	0.01 (2)	10.00			
AS-AR	0 / 37	-38.5	-38.5	0.01 (2)	10.00			
AR-AQ	0 / 33	-38.5	-38.5	0.01 (2)	10.00			
AQ-AP	0 / 31	-38.5	-38.5	0.01 (2)	10.00			
AP-AO	0 / 28	-38.5	-38.5	0.01 (2)	10.00			
AO-AN	0 / 28	-38.5	-38.5	0.01 (2)	10.00			
AN-AM	0 / 26	-38.5	-38.5	0.01 (2)	10.00			
AM-AL	0 / 23	-38.5	-38.5	0.01 (2)	10.00			
AL-AK	0 / 23	-38.5	-38.5	0.01 (2)	10.00			
AK-AJ	0 / 23	-38.5	-38.5	0.01 (2)	10.00			
AJ-AI	0 / 23	-38.5	-38.5	0.01 (2)	10.00			
AI-AH	0 / 26	-38.5	-38.5	0.01 (2)	10.00			
AH-AG	0 / 28	-38.5	-38.5	0.01 (2)	10.00			
AG-AF	0 / 28	-38.5	-38.5	0.01 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (X-Y:1) , BC=0.02/1.00 (Z-AA:1) , WB=0.34/1.00 (J-AN:1) , SSI=0.12/1.00 (X-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (O) (INPUT = 0.90)
JSI METAL= 0.12 (W) (INPUT = 1.00)

DWG NO. TAM T1805165
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	G1	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AF-AE	0 / 31	-38.5	-38.5	0.01 (2)	10.00			
AE-AD	0 / 33	-38.5	-38.5	0.01 (2)	10.00			
AD-AC	0 / 37	-38.5	-38.5	0.01 (2)	10.00			
AC-AB	0 / 41	-38.5	-38.5	0.01 (2)	10.00			
AB-AA	0 / 46	-38.5	-38.5	0.02 (2)	10.00			
AA- Z	0 / 56	-38.5	-38.5	0.02 (1)	10.00			

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM TIB05165
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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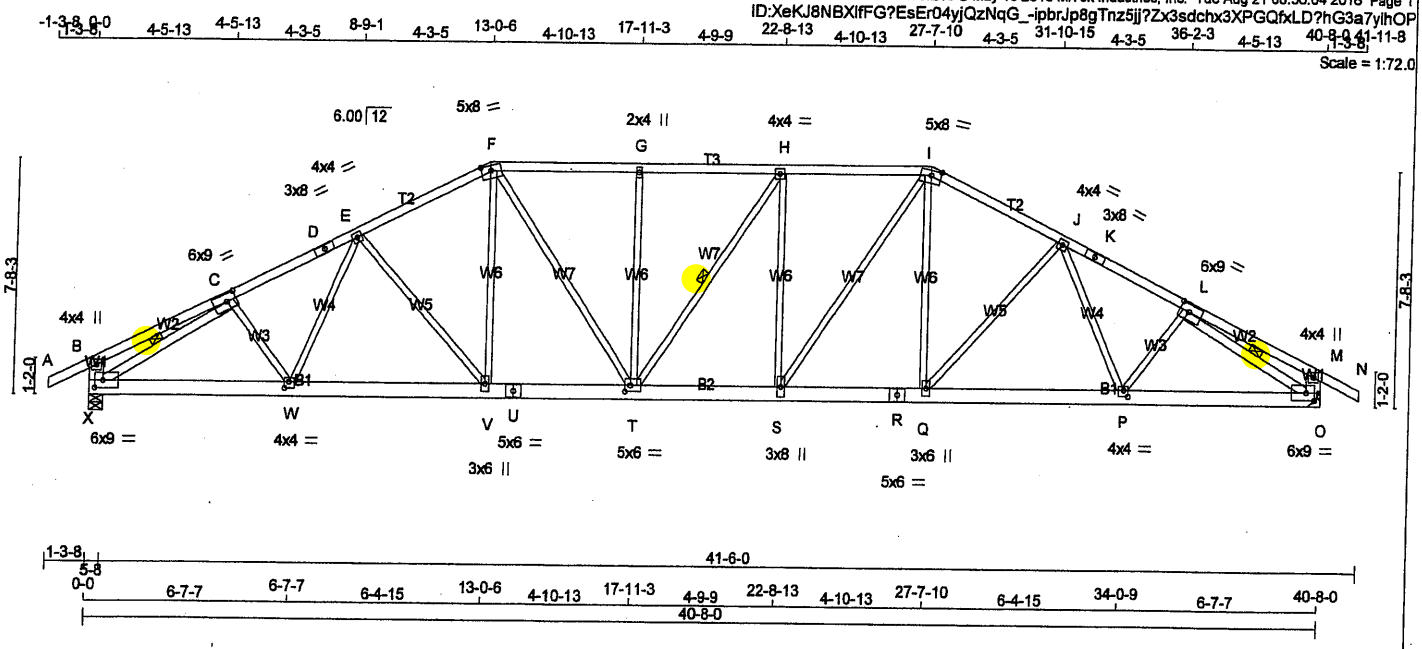

TOTAL WEIGHT = 2 X 207 = 413 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
X - C	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	6.0	9.0	3.00	3.75
D	TS-t	MT20	3.0	8.0		
E, H, J						
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	3.75
G	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.00	3.75
K	TS-t	MT20	3.0	8.0		
L	TMWW-t	MT20	6.0	9.0	3.00	3.75
M	TMV+p	MT20	4.0	4.0		
O	BMVW1-t	MT20	6.0	9.0	3.00	3.25
P	BMWW-t	MT20	4.0	4.0	2.50	1.75
Q	BMWW+t	MT20	3.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	3.0	8.0		
T	BMWWW-t	MT20	5.0	6.0	2.50	2.00
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	3.0	6.0		
W	BMWW-t	MT20	4.0	4.0	2.50	1.75
X	BMVW1-t	MT20	6.0	9.0	3.00	3.25

PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO
H.J.G. ALVES
08/21/18

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	
X	3595	0	3595	0	0	5-8	5-8
O	3595	0	3595	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT O.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2630	1658 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0
O	2630	1658 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-T, C-X, L-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.7	-129.7	0.17 (1)	10.00
B- C	0 / 20	-129.7	-129.7	0.30 (1)	10.00
C- D	-5168 / 0	-129.7	-129.7	0.61 (1)	2.66
D- E	-5168 / 0	-129.7	-129.7	0.61 (1)	2.66
E- F	-4675 / 0	-129.7	-129.7	0.57 (1)	2.83
F- G	-4728 / 0	-129.7	-129.7	0.65 (1)	2.70
G- H	-4728 / 0	-129.7	-129.7	0.61 (1)	2.75
H- I	-4736 / 0	-129.7	-129.7	0.65 (1)	2.70
I- J	-4674 / 0	-129.7	-129.7	0.57 (1)	2.83
J- K	-5169 / 0	-129.7	-129.7	0.61 (1)	2.66
K- L	-5169 / 0	-129.7	-129.7	0.61 (1)	2.66
L- M	0 / 20	-129.7	-129.7	0.30 (1)	10.00
M- N	0 / 39	-129.7	-129.7	0.17 (1)	10.00
X- B	-415 / 0	0.0	0.0	0.03 (1)	7.81
O- M	-415 / 0	0.0	0.0	0.03 (1)	7.81
X- W	0 / 4518	-38.5	-38.5	0.66 (1)	10.00
W- V	0 / 4588	-38.5	-38.5	0.63 (1)	10.00
V- U	0 / 4168	-38.5	-38.5	0.58 (1)	10.00
U- T	0 / 4168	-38.5	-38.5	0.58 (1)	10.00
T- S	0 / 4736	-38.5	-38.5	0.62 (1)	10.00
S- R	0 / 4166	-38.5	-38.5	0.57 (1)	10.00
R- Q	0 / 4166	-38.5	-38.5	0.57 (1)	10.00
Q- P	0 / 4587	-38.5	-38.5	0.63 (1)	10.00
P- O	0 / 4518	-38.5	-38.5	0.66 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- W	0 / 224	0.05 (2)
W- E	0 / 198	0.05 (3)
E- V	-653 / 0	0.55 (1)
V- F	0 / 753	0.17 (1)
F- T	0 / 1005	0.23 (1)
T- G	-676 / 0	0.69 (1)
T- H	-15 / 0	0.01 (1)
S- H	-671 / 0	0.69 (1)
S- I	0 / 1022	0.23 (1)
Q- I	0 / 741	0.17 (1)
Q- J	-654 / 0	0.55 (1)
J- P	0 / 198	0.05 (3)
P- L	0 / 224	0.05 (2)
X- C	-5456 / 0	0.81 (1)
L- O	-5457 / 0	0.81 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	61.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.48")

CSI: TC=0.65/1.00 (F-G:1) , BC=0.66/1.00 (W-X:1) , WB=0.81/1.00 (L-O:1) , SSI=0.29/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.91 (L) (INPUT = 1.00)

DWG NO. TAM T1305401
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	T6	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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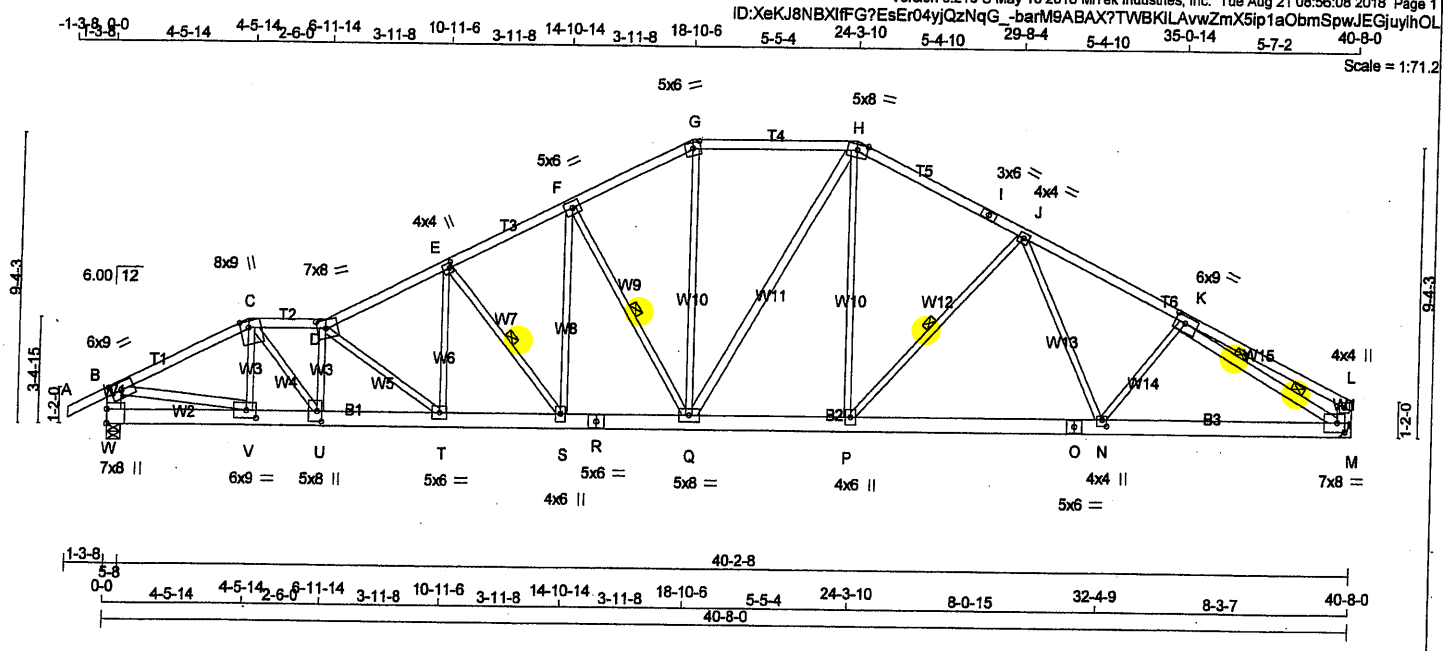

TOTAL WEIGHT = 3 X 211 = 633 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
W - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	3.00	4.00
C	TTWW+m	MT20	8.0	9.0	Edge	3.25
D	TTWW-m	MT20	7.0	8.0	3.25	3.25
E	TMWW+t	MT20	4.0	4.0	2.00	1.25
F	TMWW-t	MT20	5.0	6.0		
G	TTW-m	MT20	5.0	6.0	2.25	3.00
H	TTWW-m	MT20	5.0	8.0	Edge	
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	4.0	4.0		
K	TMWW-t	MT20	6.0	9.0	2.75	4.25
L	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	7.0	8.0	3.50	3.00
N	BMWW+t	MT20	4.0	4.0	2.50	1.75
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW+t	MT20	5.0	8.0	4.00	1.75
V	BMWW-t	MT20	6.0	9.0	3.00	4.00
W	BMV1+t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. AIKES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3595	0	3595	0	0	5-8	5-8
M	3419	0	3419	0	0	MECHANICAL - HGUS26	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2630	1658 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0
M	2510	1554 / 0	427 / 0	0 / 0	0 / 0	529 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-P, E-S, F-Q.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF K-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.7	-129.7	0.17 (1)	10.00
B- C	-5076 / 0	-129.7	-129.7	0.70 (1)	2.56
C- D	-6383 / 0	-129.7	-129.7	0.53 (1)	2.37
D- E	-6129 / 0	-129.7	-129.7	0.70 (1)	2.30
E- F	-5195 / 0	-129.7	-129.7	0.54 (1)	2.71
F- G	-4331 / 0	-129.7	-129.7	0.45 (1)	3.06
G- H	-3868 / 0	-129.7	-129.7	0.72 (1)	2.86
H- I	-4209 / 0	-129.7	-129.7	0.78 (1)	2.77
I- J	-4209 / 0	-129.7	-129.7	0.78 (1)	2.77
J- K	-5144 / 0	-129.7	-129.7	0.89 (1)	2.42
K- L	0 / 28	-129.7	-129.7	0.47 (1)	10.00
W- B	-3509 / 0	0.0	0.0	0.22 (1)	5.44
M- L	-294 / 0	0.0	0.0	0.02 (1)	7.81
W- V	0 / 0	-38.5	-38.5	0.07 (2)	10.00
V- U	0 / 4518	-38.5	-38.5	0.68 (1)	10.00
U- T	0 / 6465	-38.5	-38.5	0.91 (1)	10.00
T- S	0 / 5505	-38.5	-38.5	0.72 (1)	10.00
S- R	0 / 4647	-38.5	-38.5	0.62 (1)	10.00
R- Q	0 / 4647	-38.5	-38.5	0.62 (1)	10.00
Q- P	0 / 3747	-38.5	-38.5	0.54 (1)	10.00
P- O	0 / 4425	-38.5	-38.5	0.64 (1)	10.00
O- N	0 / 4425	-38.5	-38.5	0.64 (1)	10.00
N- M	0 / 4660	-38.5	-38.5	0.71 (1)	10.00

WEBS MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)
V- C	-533 / 0	0.10 (1)
C- U	0 / 3005	0.68 (1)
U- D	-2381 / 0	0.45 (1)
Q- G	0 / 1372	0.31 (1)
Q- H	0 / 237	0.04 (1)
P- H	0 / 1030	0.23 (1)
P- J	-1031 / 0	0.49 (1)
J- N	0 / 475	0.11 (2)
N- K	-92 / 145	0.03 (3)
B- V	0 / 4601	0.74 (1)
K- M	-5536 / 0	0.73 (1)
D- T	-1208 / 0	0.49 (1)
T- E	0 / 943	0.21 (1)
E- S	-1381 / 0	0.38 (1)
S- F	0 / 1193	0.27 (1)
F- Q	-1591 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 864 (0.56")

CSI: TC=0.89/1.00 (J-K:1) , BC=0.91/1.00 (T-U:1) , WB=0.74/0.90 (B-V:1) , SSI=0.30/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.91 (K) (INPUT = 1.00)

DWG NO. TAM T1805402
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	G6A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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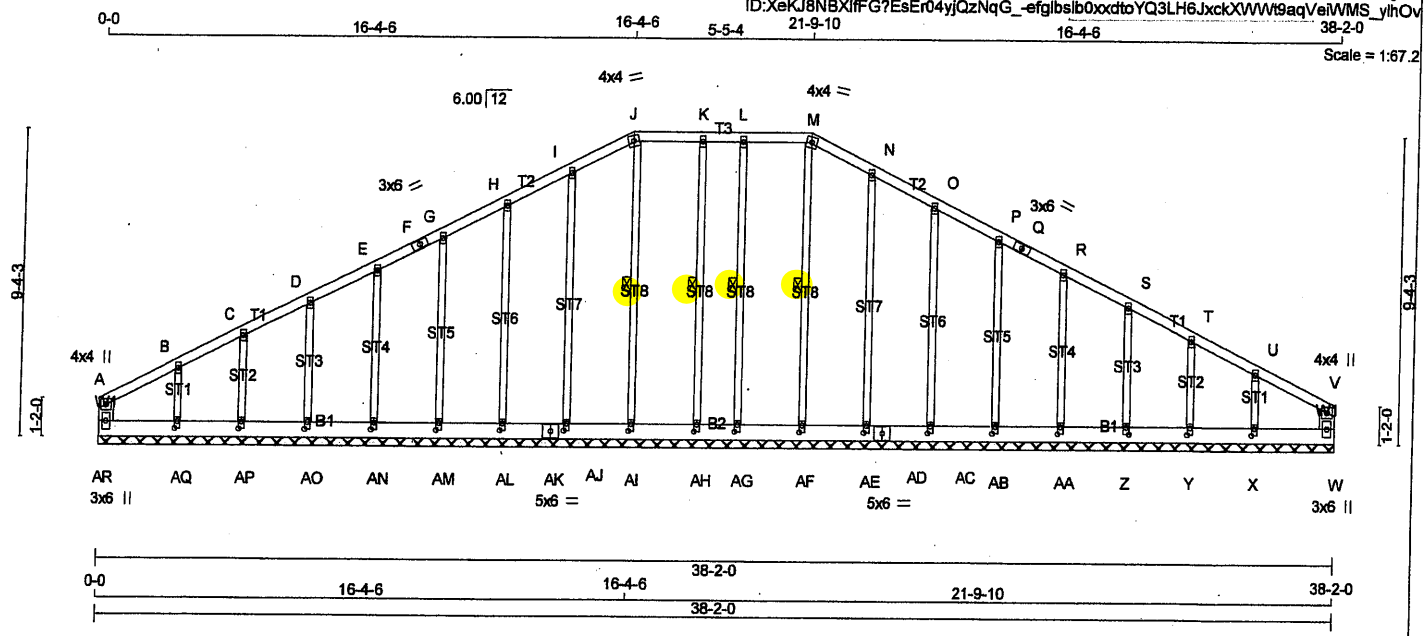

TOTAL WEIGHT = 200 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AR- A	2x6	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
M - Q	2x4	DRY	No.2	SPF
Q - V	2x4	DRY	No.2	SPF
W - V	2x6	DRY	No.2	SPF
AR- AK	2x6	DRY	No.2	SPF
AK- AD	2x6	DRY	No.2	SPF
AD- W	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	4.0	4.0		
B, C, D, E, G, H, I, K, L, N, O, P, R, S, T, U						
B	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
M	TTW-m	MT20	4.0	4.0		
Q	TS-t	MT20	3.0	6.0		
V	TMV+p	MT20	4.0	4.0		
W	BMV1+p	MT20	3.0	6.0		
X, Y, Z, AA, AB, AC, AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ						
X	BMW1+w	MT20	2.0	4.0	2.50	1.00
AD	BS-t	MT20	5.0	6.0		
AK	BS-t	MT20	5.0	6.0		
AR	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF M-AF, L-AG, K-AH, J-AI.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
AR- A	-195 / 0	0.0	0.0	0.04 (1)		7.81
A- B	-85 / 0	-129.7	-129.7	0.08 (1)		6.25
B- C	-77 / 0	-129.7	-129.7	0.07 (1)		6.25
C- D	-67 / 0	-129.7	-129.7	0.06 (1)		6.25
D- E	-63 / 0	-129.7	-129.7	0.06 (1)		6.25
E- F	-59 / 0	-129.7	-129.7	0.06 (1)		6.25
F- G	-59 / 0	-129.7	-129.7	0.06 (1)		6.25
G- H	-56 / 0	-129.7	-129.7	0.06 (1)		6.25
H- I	-54 / 0	-129.7	-129.7	0.06 (1)		6.25
I- J	-50 / 0	-129.7	-129.7	0.06 (1)		6.25
J- K	-43 / 0	-129.7	-129.7	0.06 (1)		6.25
K- L	-43 / 0	-129.7	-129.7	0.05 (1)		6.25
L- M	-43 / 0	-129.7	-129.7	0.06 (1)		6.25
M- N	-50 / 0	-129.7	-129.7	0.06 (1)		6.25
N- O	-54 / 0	-129.7	-129.7	0.06 (1)		6.25
O- P	-56 / 0	-129.7	-129.7	0.06 (1)		6.25
P- Q	-59 / 0	-129.7	-129.7	0.06 (1)		6.25
Q- R	-59 / 0	-129.7	-129.7	0.06 (1)		6.25
R- S	-63 / 0	-129.7	-129.7	0.06 (1)		6.25
S- T	-67 / 0	-129.7	-129.7	0.06 (1)		6.25
T- U	-77 / 0	-129.7	-129.7	0.07 (1)		6.25
U- V	-85 / 0	-129.7	-129.7	0.08 (1)		6.25
W- V	-195 / 0	0.0	0.0	0.04 (1)		7.81
AR-AQ	0 / 76	-38.5	-38.5	0.02 (2)		10.00
AQ-AP	0 / 66	-38.5	-38.5	0.02 (2)		10.00
AP-AO	0 / 61	-38.5	-38.5	0.02 (2)		10.00
AO-AN	0 / 56	-38.5	-38.5	0.02 (2)		10.00
AN-AM	0 / 53	-38.5	-38.5	0.02 (2)		10.00
AM-AL	0 / 50	-38.5	-38.5	0.02 (2)		10.00
AL-AK	0 / 48	-38.5	-38.5	0.02 (2)		10.00
AK-AJ	0 / 48	-38.5	-38.5	0.02 (2)		10.00
AJ-AI	0 / 46	-38.5	-38.5	0.02 (2)		10.00
AI-AH	0 / 43	-38.5	-38.5	0.02 (2)		10.00
AH-AG	0 / 43	-38.5	-38.5	0.01 (2)		10.00
AG-AF	0 / 43	-38.5	-38.5	0.02 (2)		10.00
AF-AE	0 / 46	-38.5	-38.5	0.02 (2)		10.00
AE-AD	0 / 48	-38.5	-38.5	0.02 (2)		10.00
AD-AC	0 / 48	-38.5	-38.5	0.02 (2)		10.00
AC-AB	0 / 50	-38.5	-38.5	0.02 (2)		10.00
AB-AA	0 / 53	-38.5	-38.5	0.02 (2)		10.00
AA- Z	0 / 56	-38.5	-38.5	0.02 (2)		10.00
Z- Y	0 / 61	-38.5	-38.5	0.02 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AF- M	-249 / 0	0.14 (1)
AG- L	-211 / 0	0.12 (1)
AH- K	-211 / 0	0.12 (1)
AJ- I	-252 / 0	0.34 (1)
AL- H	-259 / 0	0.24 (1)
AM- G	-258 / 0	0.16 (1)
AN- E	-257 / 0	0.11 (1)
AO- D	-259 / 0	0.07 (1)
AP- C	-248 / 0	0.05 (1)
AQ- B	-287 / 0	0.04 (1)
AE- N	-252 / 0	0.34 (1)
AC- O	-259 / 0	0.24 (1)
AB- P	-258 / 0	0.16 (1)
AA- R	-257 / 0	0.11 (1)
Z- S	-259 / 0	0.07 (1)
Y- T	-248 / 0	0.05 (1)
X- U	-287 / 0	0.04 (1)
AI- J	-249 / 0	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1) , BC=0.02/1.00 (AQ-AR:2) , WB=0.34/1.00 (I-AJ:1) , SSI=0.11/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (M) (INPUT = 0.90)
JSI METAL= 0.12 (U) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805392
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	G6A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- X	0 / 66	-38.5	-38.5	0.02 (2)	10.00			
X- W	0 / 76	-38.5	-38.5	0.02 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
06/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1605392
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	T7	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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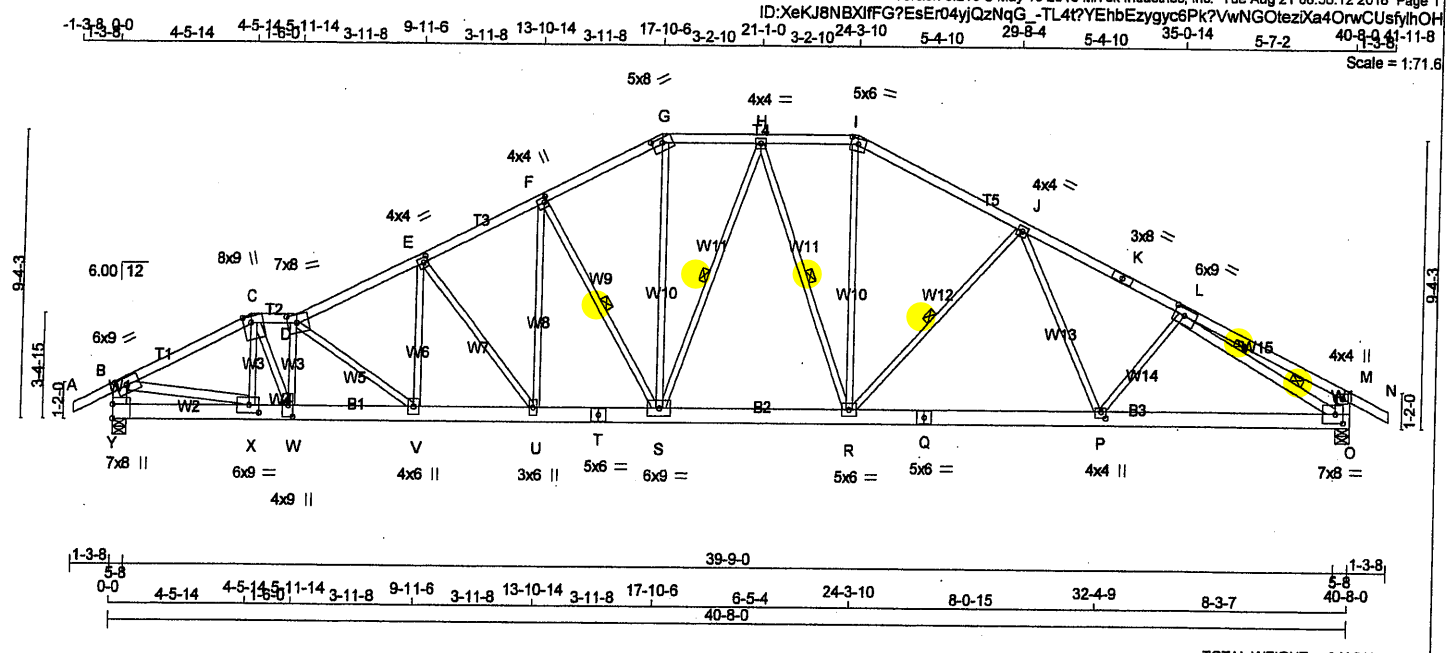


TOTAL WEIGHT = 3 X 216 = 648 lb

[M][F]

LUMBER

N. L. G. A. RULES

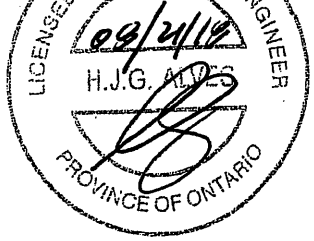
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
Y - T	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0	3.00	4.00
C	TTWW+m	MT20	8.0	9.0	Edge	3.25
D	TTWW-m	MT20	7.0	8.0	3.25	3.50
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMWW+t	MT20	4.0	4.0	2.00	1.25
G	TTW-h	MT20	5.0	8.0	2.00	4.00
H	TMWW-t	MT20	4.0	4.0		
I	TTW-m	MT20	5.0	6.0	2.25	3.00
J	TMWW-t	MT20	4.0	4.0		
K	TS-t	MT20	3.0	8.0		
L	TMWW-t	MT20	6.0	9.0	2.75	4.25
M	TMV+p	MT20	4.0	4.0		
O	BMVW1-t	MT20	7.0	8.0	3.50	3.00
P	BMWW+t	MT20	4.0	4.0	2.50	1.75
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	6.0	9.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW+t	MT20	3.0	6.0		
V	BMWW+t	MT20	4.0	6.0		
W	BMWW+t	MT20	4.0	9.0	4.50	1.75
X	BMWW-t	MT20	6.0	9.0	3.00	4.00
Y	BMV1+t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	3595	0	3595	0	0	5-8	5-8
O	3595	0	3595	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2630	1658 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0
O	2630	1658 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-S, J-R, H-S, H-R.
2 LATERAL BRACE(S) AT 1/3 LENGTH OF L-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.7	-129.7	1	0.17	10.00
B- C	-5081 / 0	-129.7	-129.7	1	0.70	2.56
C- D	-5608 / 0	-129.7	-129.7	1	0.35	2.73
D- E	-5761 / 0	-129.7	-129.7	1	0.64	2.45
E- F	-5036 / 0	-129.7	-129.7	1	0.52	2.78
F- G	-4291 / 0	-129.7	-129.7	1	0.45	3.08
G- H	-3834 / 0	-129.7	-129.7	1	0.30	3.39
H- I	-3754 / 0	-129.7	-129.7	1	0.29	3.42
I- J	-4213 / 0	-129.7	-129.7	1	0.78	2.78
J- K	-5142 / 0	-129.7	-129.7	1	0.88	2.43
K- L	-5142 / 0	-129.7	-129.7	1	0.88	2.43
L- M	0 / 28	-129.7	-129.7	1	0.47	10.00
M- N	0 / 39	-129.7	-129.7	1	0.17	10.00
Y- B	-3512 / 0	0.0	0.0	1	0.22	5.69
O- M	-469 / 0	0.0	0.0	1	0.03	7.81
Y- X	0 / 0	-38.5	-38.5	2	0.07	10.00
X- W	0 / 4525	-38.5	-38.5	1	0.68	10.00
W- V	0 / 5693	-38.5	-38.5	1	0.83	10.00
V- U	0 / 5175	-38.5	-38.5	1	0.68	10.00
U- T	0 / 4505	-38.5	-38.5	1	0.60	10.00
T- S	0 / 4505	-38.5	-38.5	1	0.60	10.00
S- R	0 / 3876	-38.5	-38.5	1	0.57	10.00
R- Q	0 / 4425	-38.5	-38.5	1	0.64	10.00
Q- P	0 / 4425	-38.5	-38.5	1	0.64	10.00
P- O	0 / 4659	-38.5	-38.5	1	0.72	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
X- C	-458 / 0	0.09 (1)
C- W	0 / 2586	0.58 (1)
W- D	-2490 / 0	0.47 (1)
D- V	-652 / 0	0.27 (1)
V- E	0 / 613	0.14 (1)
E- U	-1080 / 0	0.76 (1)
U- F	0 / 958	0.22 (1)
F- S	-1369 / 0	0.59 (1)
S- G	0 / 1524	0.34 (1)
R- I	0 / 1405	0.32 (1)
R- J	-1026 / 0	0.48 (1)
J- P	0 / 469	0.11 (2)
P- L	-94 / 145	0.03 (3)
B- X	0 / 4605	0.74 (1)
L- O	-5534 / 0	0.73 (1)
S- H	-128 / 12	0.08 (1)
H- R	-369 / 0	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 952 (0.51")

CSI: TC=0.88/1.00 (J-L:1) , BC=0.83/1.00 (V-W:1) , WB=0.76/1.00 (E-U:1) , SSI=0.30/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (X) (INPUT = 0.90)
JSI METAL= 0.91 (L) (INPUT = 1.00)

DESIGNED BY TAM T1805403
STRUCTURAL
ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	G7A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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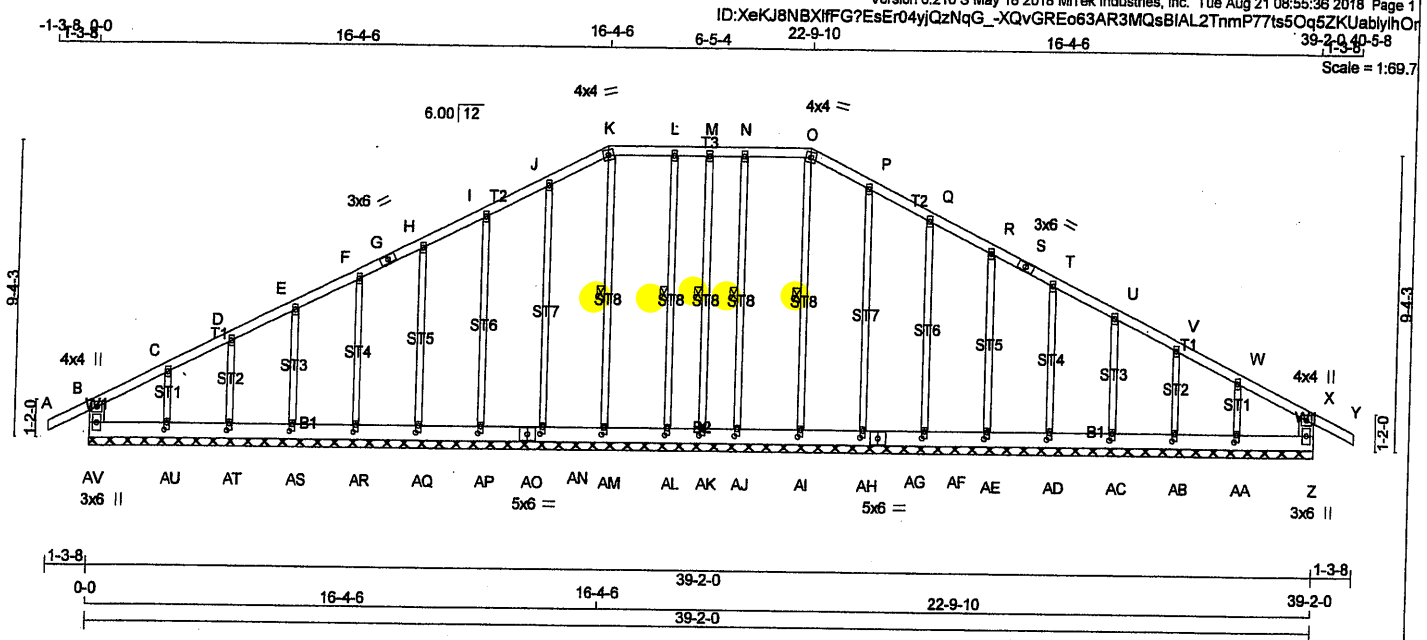

TOTAL WEIGHT = 213 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AV- B	2x6	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - O	2x4	DRY	No.2	SPF
O - S	2x4	DRY	No.2	SPF
S - Y	2x4	DRY	No.2	SPF
Z - X	2x6	DRY	No.2	SPF
AV- AO	2x6	DRY	No.2	SPF
AO- AG	2x6	DRY	No.2	SPF
AG- Z	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C, D, E, F, H, I, J, L, M, N, P, Q, R, T, U, V, W						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
K	TTW-m	MT20	4.0	4.0		
O	TTW-m	MT20	4.0	4.0		
S	TS-t	MT20	3.0	6.0		
X	TMV+p	MT20	4.0	4.0		
Z	BMV1+p	MT20	3.0	6.0		
AA, AB, AC, AD, AE, AF, AH, AI, AJ, AK, AL, AM, AN, AP, AQ, AR, AS, AT, AU						
AA	BMW1+w	MT20	2.0	4.0	2.50	1.00
AG	BS-t	MT20	5.0	6.0		
AO	BS-t	MT20	5.0	6.0		
AV	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF O-AI, N-AJ, M-AK, L-AL, K-AM.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AV- B	-368 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 39	-129.7	-129.7	0.17 (1)	10.00
B- C	-67 / 0	-129.7	-129.7	0.09 (1)	6.25
C- D	-56 / 0	-129.7	-129.7	0.07 (1)	6.25
D- E	-47 / 0	-129.7	-129.7	0.06 (1)	6.25
E- F	-43 / 0	-129.7	-129.7	0.06 (1)	6.25
F- G	-39 / 0	-129.7	-129.7	0.06 (1)	6.25
G- H	-39 / 0	-129.7	-129.7	0.06 (1)	6.25
H- I	-36 / 0	-129.7	-129.7	0.06 (1)	6.25
I- J	-34 / 0	-129.7	-129.7	0.06 (1)	6.25
J- K	-30 / 0	-129.7	-129.7	0.06 (1)	6.25
K- L	-25 / 0	-129.7	-129.7	0.06 (1)	6.25
L- M	-25 / 0	-129.7	-129.7	0.04 (1)	6.25
M- N	-25 / 0	-129.7	-129.7	0.04 (1)	6.25
N- O	-25 / 0	-129.7	-129.7	0.06 (1)	6.25
O- P	-30 / 0	-129.7	-129.7	0.06 (1)	6.25
P- Q	-34 / 0	-129.7	-129.7	0.06 (1)	6.25
Q- R	-36 / 0	-129.7	-129.7	0.06 (1)	6.25
R- S	-39 / 0	-129.7	-129.7	0.06 (1)	6.25
S- T	-39 / 0	-129.7	-129.7	0.06 (1)	6.25
T- U	-43 / 0	-129.7	-129.7	0.06 (1)	6.25
U- V	-47 / 0	-129.7	-129.7	0.06 (1)	6.25
V- W	-56 / 0	-129.7	-129.7	0.07 (1)	6.25
W- X	-67 / 0	-129.7	-129.7	0.09 (1)	6.25
X- Y	0 / 39	-129.7	-129.7	0.17 (1)	10.00
Z- X	-368 / 0	0.0	0.0	0.02 (1)	7.81
AV-AU	0 / 57	-38.5	-38.5	0.02 (1)	10.00
AU-AT	0 / 48	-38.5	-38.5	0.02 (2)	10.00
AT-AS	0 / 43	-38.5	-38.5	0.01 (2)	10.00
AS-AR	0 / 38	-38.5	-38.5	0.01 (2)	10.00
AR-AQ	0 / 35	-38.5	-38.5	0.01 (2)	10.00
AQ-AP	0 / 32	-38.5	-38.5	0.01 (2)	10.00
AP-AO	0 / 30	-38.5	-38.5	0.01 (2)	10.00
AO-AN	0 / 30	-38.5	-38.5	0.01 (2)	10.00
AN-AM	0 / 28	-38.5	-38.5	0.01 (2)	10.00
AM-AL	0 / 25	-38.5	-38.5	0.01 (2)	10.00
AL-AK	0 / 25	-38.5	-38.5	0.01 (2)	10.00
AK-AJ	0 / 25	-38.5	-38.5	0.01 (2)	10.00
AJ-AI	0 / 25	-38.5	-38.5	0.01 (2)	10.00
AI-AH	0 / 28	-38.5	-38.5	0.01 (2)	10.00
AH-AG	0 / 30	-38.5	-38.5	0.01 (2)	10.00
AG-AF	0 / 30	-38.5	-38.5	0.01 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AI- O	-259 / 0	0.14 (1)
AJ- N	-209 / 0	0.12 (1)
AK- M	-129 / 0	0.07 (1)
AL- L	-209 / 0	0.12 (1)
AN- J	-253 / 0	0.34 (1)
AP- I	-259 / 0	0.24 (1)
AQ- H	-258 / 0	0.16 (1)
AR- F	-257 / 0	0.11 (1)
AS- E	-259 / 0	0.07 (1)
AT- D	-250 / 0	0.05 (1)
AU- C	-279 / 0	0.04 (1)
AH- P	-253 / 0	0.34 (1)
AF- Q	-259 / 0	0.24 (1)
AE- R	-258 / 0	0.16 (1)
AD- T	-257 / 0	0.11 (1)
AC- U	-259 / 0	0.07 (1)
AB- V	-250 / 0	0.05 (1)
AA- W	-279 / 0	0.04 (1)
AM- K	-259 / 0	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (X-Y:1) , BC=0.02/1.00 (Z-AA:1) , WB=0.34/1.00 (J-AN:1) , SSI=0.12/1.00 (X-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (O) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/11/18
H.J.G. AWES
PROVINCE OF ONTARIO

DWG NO. TAM T1805393
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	G7A	1	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AF-AE	0 / 32	-38.5	-38.5	0.01 (2)	10.00			
AE-AD	0 / 35	-38.5	-38.5	0.01 (2)	10.00			
AD-AC	0 / 38	-38.5	-38.5	0.01 (2)	10.00			
AC-AB	0 / 43	-38.5	-38.5	0.01 (2)	10.00			
AB-AA	0 / 48	-38.5	-38.5	0.02 (2)	10.00			
AA- Z	0 / 57	-38.5	-38.5	0.02 (1)	10.00			

LICENSED PROFESSIONAL ENGINEER
05/21/18
H.J.G. ALLES
PROVINCE OF ONTARIO

DWG NO. TAM T1805393
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	T11Z	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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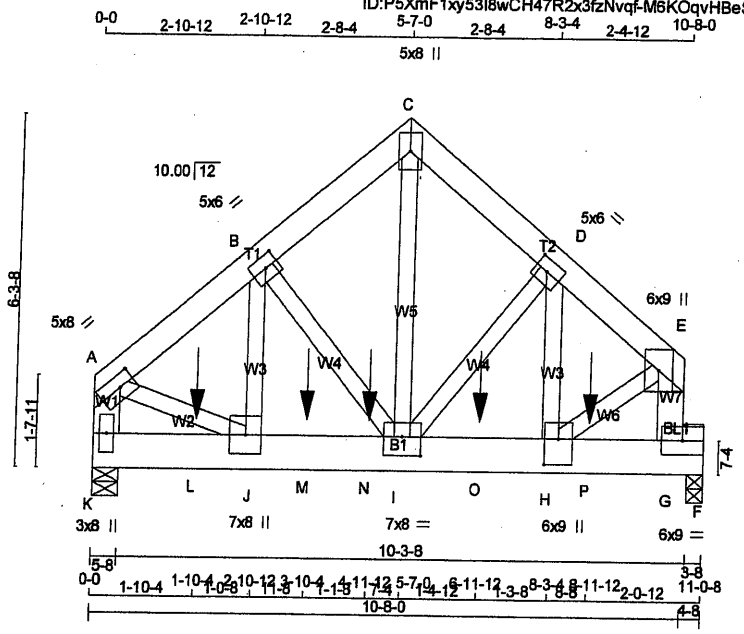

TOTAL WEIGHT = 4 X 83 = 332 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
K - F	2x8	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
K- A	2	12	TOP
G- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- F	4	4	SIDE(2403.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.25
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TTW+p	MT20	5.0	8.0		
D	TMWW-t	MT20	5.0	6.0	2.50	2.75
E	TMVW+p	MT20	6.0	9.0		
G	BMVK-t	MT20	6.0	9.0		
H	BMWW+t	MT20	6.0	9.0	5.50	3.00
I	BMWWW-t	MT20	7.0	8.0	4.00	4.00
J	BMWW+t	MT20	7.0	8.0		
K	BMV1+p	MT20	3.0	8.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	9899	0	9899	0	0	5-8	5-8
F	9171	0	9171	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	7233	4597 / 0	1166 / 0	0 / 0	0 / 0	1469 / 0	0 / 0
F	6707	4243 / 0	1096 / 0	0 / 0	0 / 0	1369 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-9458 / 0	-129.7	-129.7	0.10 (1)	5.05
B- C	-7393 / 0	-129.7	-129.7	0.09 (1)	5.57
C- D	-7382 / 0	-129.7	-129.7	0.10 (1)	5.56
D- E	-8796 / 0	-129.7	-129.7	0.10 (1)	5.19
K- A	-8900 / 0	0.0	0.0	0.16 (1)	6.57
G- E	-11129 / 0	0.0	0.0	0.21 (1)	6.00
K- L	0 / 0	-38.5	-38.5	0.24 (1)	10.00
L- J	0 / 0	-38.5	-38.5	0.24 (1)	10.00
J- M	0 / 7210	-38.5	-38.5	0.43 (1)	10.00
M- N	0 / 7210	-38.5	-38.5	0.43 (1)	10.00
N- I	0 / 7210	-38.5	-38.5	0.43 (1)	10.00
I- O	0 / 6680	-38.5	-38.5	0.36 (1)	10.00
O- H	0 / 6680	-38.5	-38.5	0.36 (1)	10.00
H- P	-1729 / 0	-38.5	-38.5	0.73 (1)	6.25
P- G	-1729 / 0	-38.5	-38.5	0.73 (1)	6.25
G- F	0 / 0	-168.2	-168.2	0.52 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 2989	0.12 (1)
B- I	-2394 / 0	0.12 (1)
I- C	0 / 8955	0.36 (1)
I- D	-1532 / 0	0.08 (1)
H- D	0 / 1960	0.08 (1)
A- J	0 / 7661	0.31 (1)
H- E	0 / 9842	0.40 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	1-10-4	-2583	-2583	—	BACK	VERT	TOTAL
M	3-10-4	-2583	-2583	—	BACK	VERT	TOTAL
N	4-11-12	-2465	-2465	—	BACK	VERT	TOTAL
O	6-11-12	-2463	-2463	—	BACK	VERT	TOTAL
P	8-11-12	-2463	-2463	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.21/1.00 (E-G:1) , BC=0.73/1.00 (G-H:1) , WB=0.40/1.00 (E-H:1) , SSI=0.87/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.63 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. REES
PROVINCE OF ONTARIO

DWG NO. TAM 11805404
STRUCTURAL
COMPONENT SHEET 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	T11Z	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:P5XmF1xy53l8wCH47R2x3fzNvqf-M6KOqvHBeSTO9Zvtea3R5DQENFMATTo_mYAh?RylhOD

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3541.7 lbs FACTORED DOWN AT 1-10-4, 3541.7 lbs FACTORED DOWN AT 3-10-4, 3368.7 lbs FACTORED DOWN AT 4-11-12, AND 3365.8 lbs FACTORED DOWN AT 6-11-12, AND 3365.8 lbs FACTORED DOWN AT 8-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 17805404
STRUCTURAL

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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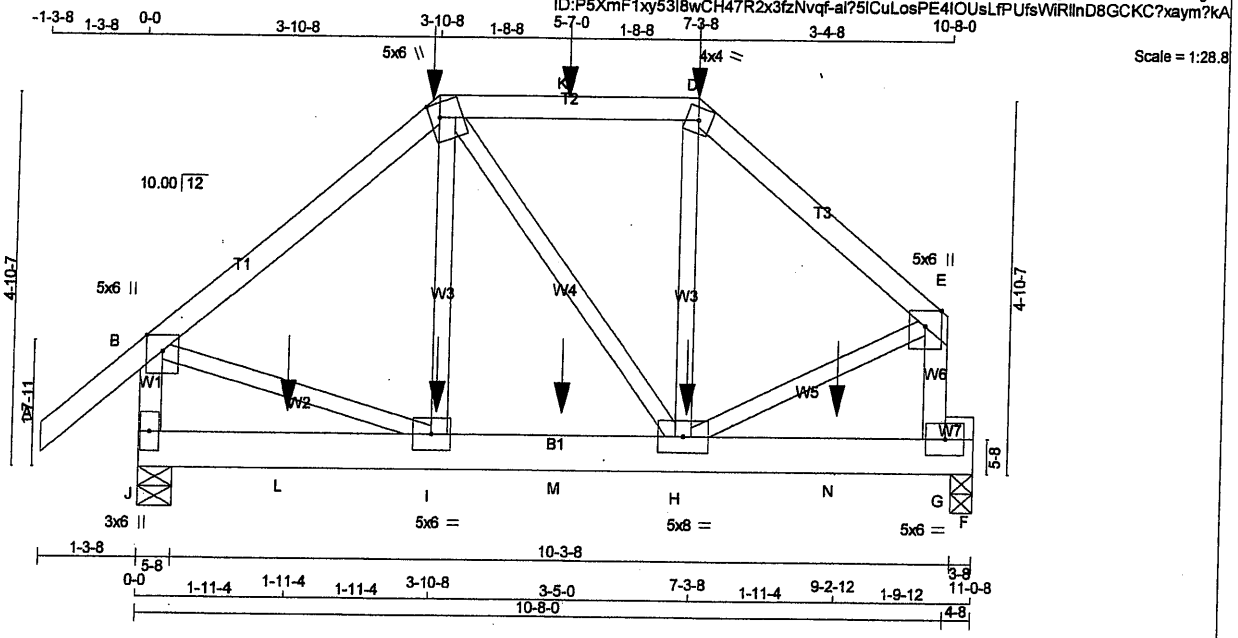

TOTAL WEIGHT = 2 X 57 = 114 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMVW-t	MT20	5.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 292.9 lbs FACTORED DOWN AT 3-10-8, AND 121.8 lbs FACTORED DOWN AT 5-7-0, AND 292.9 lbs FACTORED DOWN AT 7-3-8 ON TOP CHORD, AND 36.3 lbs FACTORED DOWN AT 1-11-4, 45.9 lbs FACTORED DOWN AT 3-11-4, 45.9 lbs FACTORED DOWN AT 5-7-0, AND 45.9 lbs FACTORED DOWN AT 7-2-12, AND 36.3 lbs FACTORED DOWN AT 9-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1532	0	1532	0	0	5-8	5-8
F	1315	0	1315	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1114	722 / 0	165 / 0	0 / 0	0 / 0	226 / 0	0 / 0
F	967	593 / 0	166 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.20 (1)	10.00
B- C	-1158 / 0	-129.7	-129.7	0.41 (1)	5.33
C- K	-902 / 0	-129.7	-129.7	0.46 (1)	5.73
K- D	-902 / 0	-129.7	-129.7	0.46 (1)	5.73
D- E	-1179 / 0	-129.7	-129.7	0.31 (1)	5.46
J- B	-1447 / 0	0.0	0.0	0.17 (1)	6.76
G- E	-1403 / 0	0.0	0.0	0.17 (1)	6.84
J- L	0 / 0	-38.5	-38.5	0.08 (2)	10.00
L- I	0 / 0	-38.5	-38.5	0.08 (2)	10.00
I- M	0 / 885	-38.5	-38.5	0.22 (1)	10.00
M- H	0 / 885	-38.5	-38.5	0.22 (1)	10.00
H- N	0 / 0	-38.5	-38.5	0.36 (1)	10.00
N- G	0 / 0	-38.5	-38.5	0.36 (1)	10.00
G- F	0 / 0	-38.5	-38.5	0.22 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
I- C	-106 / 162	0.04 (3)
C- H	0 / 30	0.01 (1)
H- D	-35 / 180	0.05 (3)
B- I	0 / 928	0.23 (1)
H- E	0 / 998	0.25 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-202	-202	—	FRONT	VERT	TOTAL
D	7-3-8	-202	-202	—	FRONT	VERT	TOTAL
H	7-2-12	-33	-33	—	FRONT	VERT	TOTAL
I	3-11-4	-33	-33	—	FRONT	VERT	TOTAL
K	5-7-0	-83	-83	—	FRONT	VERT	TOTAL
L	1-11-4	-22	-26	—	FRONT	VERT	TOTAL
M	5-7-0	-33	-33	—	FRONT	VERT	TOTAL
N	9-2-12	-22	-26	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
09/20/16
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.46/1.00 (C-D:1) , BC=0.36/1.00 (G-H:1) , WB=0.25/1.00 (E-H:1) , SSI=0.53/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)

DWG NO. TAM T1605726
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297080	T13	4	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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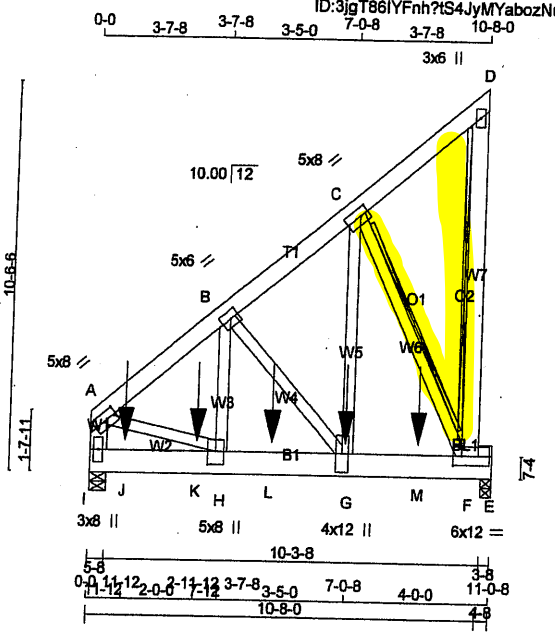

TOTAL WEIGHT = 16 X 99 = 1581 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
I - E	2x8	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
I- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- E	3	4	SIDE(1549.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.25
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	5.0	8.0	2.50	2.75
D	TMV+p	MT20	3.0	6.0		
F	BMVWK-t	MT20	6.0	12.0	3.00	2.75
G	BMWW+t	MT20	4.0	12.0		
H	BMWW+t	MT20	5.0	8.0	4.25	2.50
I	BMV1+p	MT20	3.0	8.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	10672	0	10672	0	0	5-8	5-8
E	8920	0	8920	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	7791	4964 / 0	1228 / 0	0 / 0	0 / 0	1598 / 0	0 / 0
E	6514	4141 / 0	1034 / 0	0 / 0	0 / 0	1339 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT D-F, C-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-8746 / 0	-129.7	-129.7	0.11 (1)	5.21
B- C	-5468 / 0	-129.7	-129.7	0.07 (1)	6.25
C- D	-32 / 0	-129.7	-129.7	0.04 (1)	6.25
F- D	-180 / 0	0.0	0.0	0.01 (1)	7.81
I- A	-7795 / 0	0.0	0.0	0.14 (1)	6.92
I- J	0 / 0	-38.5	-38.5	0.37 (1)	10.00
J- K	0 / 0	-38.5	-38.5	0.37 (1)	10.00
K- H	0 / 0	-38.5	-38.5	0.37 (1)	10.00
H- L	0 / 6684	-38.5	-38.5	0.39 (1)	10.00
L- G	0 / 6684	-38.5	-38.5	0.39 (1)	10.00
G- M	0 / 4269	-38.5	-38.5	0.82 (1)	10.00
M- F	0 / 4269	-38.5	-38.5	0.82 (1)	10.00
F- E	0 / 0	-38.5	-38.5	0.51 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	0 / 4637	0.19 (1)
B- G	-3750 / 0	0.28 (1)
G- C	0 / 11600	0.47 (1)
C- F	-9687 / 0	0.38 (1)
A- H	0 / 6953	0.28 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	6-11-12	-2595	-2595	—	BACK	VERT	TOTAL
J	11-12	-2595	-2595	—	BACK	VERT	TOTAL
K	2-11-12	-2595	-2595	—	BACK	VERT	TOTAL
L	4-11-12	-2595	-2595	—	BACK	VERT	TOTAL
M	8-11-12	-2595	-2595	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	38.2	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.14/1.00 (A-I:1) , BC=0.82/1.00 (F-G:1) , WB=0.47/1.00 (C-G:1) , SSI=0.85/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.60 (H) (INPUT = 1.00)

DWG NO. TAM T1805262
STRUCTURAL
COMPONENT ONLY
½

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297080	T13	4	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3557.1 lbs FACTORED DOWN AT 11-12, 3556.7 lbs FACTORED DOWN AT 2-11-12, 3556.7 lbs FACTORED DOWN AT 4-11-12, AND 3556.7 lbs FACTORED DOWN AT 6-11-12, AND 3556.7 lbs FACTORED DOWN AT 8-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805262
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297080	T14	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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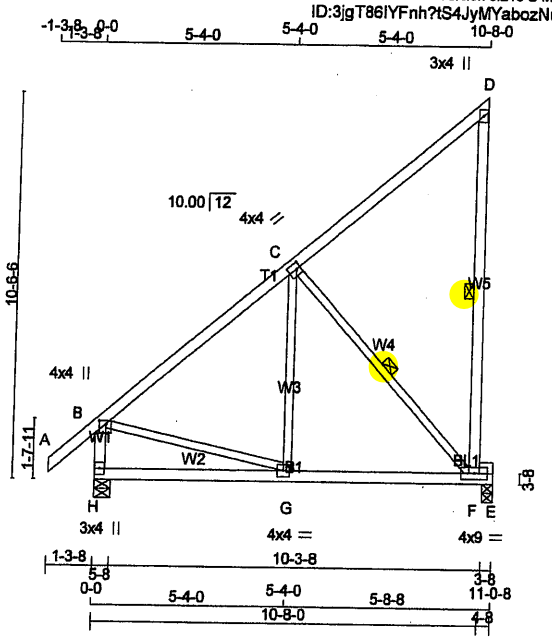


TOTAL WEIGHT = 10 X 58 = 583 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMV+p	MT20	3.0	4.0		
F	BMVWK-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1107	0	1107	0	0	5-8	5-8
E	881	0	881	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	803	527 / 0	116 / 0	0 / 0	0 / 0	160 / 0	0 / 0
E	649	394 / 0	116 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

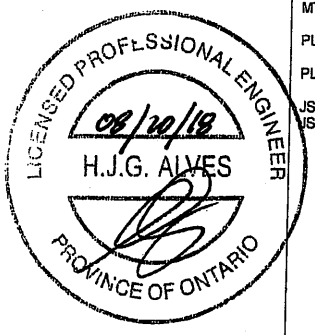
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-680 / 0	-129.7	-129.7	0.67 (1)	5.91
C- D	-50 / 0	-129.7	-129.7	0.62 (1)	6.25
F- D	-264 / 0	0.0	0.0	0.14 (1)	6.25
H- B	-1048 / 0	0.0	0.0	0.11 (1)	7.68
H- G	0 / 0	-38.5	-38.5	0.34 (2)	10.00
G- F	0 / 564	-38.5	-38.5	0.71 (1)	10.00
F- E	0 / 0	-38.5	-38.5	0.26 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 318	0.08 (3)
C- F	-848 / 0	0.34 (1)
B- G	0 / 581	0.13 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 621 (0.21")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.71/1.00 (F-G:1) , WB=0.34/1.00 (C-F:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

DWG NO. TAM #1805263
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297080	G14	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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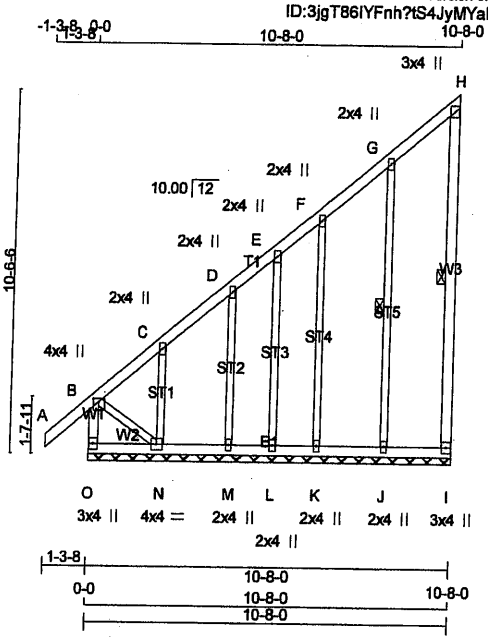

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, G						
C	TMW+w	MT20	2.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, L, M						
J	BMW1+w	MT20	2.0	4.0		
N	BMWW1-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
O- B	-316 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-10 / 0	-129.7	-129.7	0.08 (1)	6.25
C- D	-26 / 0	-129.7	-129.7	0.08 (1)	6.25
D- E	-17 / 0	-129.7	-129.7	0.04 (1)	6.25
E- F	0 / 0	-129.7	-129.7	0.04 (1)	10.00
F- G	0 / 3	-129.7	-129.7	0.07 (1)	10.00
G- H	-16 / 0	-129.7	-129.7	0.07 (1)	6.25
I- H	-110 / 0	0.0	0.0	0.06 (1)	6.25
O- N	0 / 0	-38.5	-38.5	0.03 (3)	10.00
N- M	0 / 13	-38.5	-38.5	0.03 (3)	10.00
M- L	0 / 8	-38.5	-38.5	0.02 (2)	10.00
L- K	0 / 6	-38.5	-38.5	0.02 (3)	10.00
K- J	0 / 3	-38.5	-38.5	0.03 (3)	10.00
J- I	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- G	-299 / 0	0.15 (1)
K- F	-209 / 0	0.19 (1)
L- E	-141 / 0	0.08 (1)
M- D	-202 / 0	0.08 (1)
N- C	-299 / 0	0.06 (1)
B- N	0 / 25	0.01 (1)

TOTAL WEIGHT = 2 X 67 = 134 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.03/1.00 (M-N:3) , WB=0.19/1.00 (F-K:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.16 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805261
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	G15	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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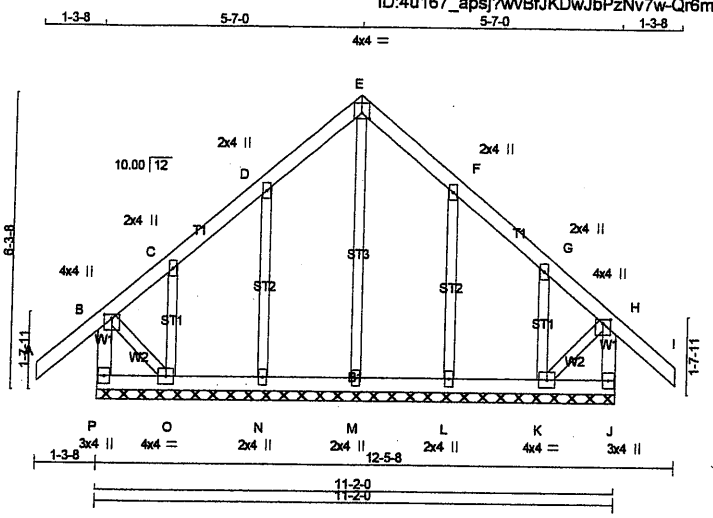

TOTAL WEIGHT = 2 X 54 = 109 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
P- B	-389 / 0	0.0	0.0	4 (1)	0.04	7.81
A- B	0 / 57	-129.7	-129.7	4 (1)	0.18	10.00
B- C	-78 / 0	-129.7	-129.7	4 (1)	0.17	6.25
C- D	-8 / 0	-129.7	-129.7	4 (1)	0.08	10.00
D- E	-33 / 0	-129.7	-129.7	4 (1)	0.08	6.25
E- F	-33 / 0	-129.7	-129.7	4 (1)	0.08	6.25
F- G	-8 / 0	-129.7	-129.7	4 (1)	0.08	10.00
G- H	-78 / 0	-129.7	-129.7	4 (1)	0.17	6.25
H- I	0 / 57	-129.7	-129.7	4 (1)	0.18	10.00
J- H	-389 / 0	0.0	0.0	4 (1)	0.04	7.81
P- O	0 / 0	-38.5	-38.5	2 (3)	0.02	10.00
O- N	0 / 18	-38.5	-38.5	2 (3)	0.02	10.00
N- M	0 / 11	-38.5	-38.5	2 (3)	0.02	10.00
M- L	0 / 11	-38.5	-38.5	2 (3)	0.02	10.00
L- K	0 / 18	-38.5	-38.5	2 (3)	0.02	10.00
K- J	0 / 0	-38.5	-38.5	2 (3)	0.02	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
M- E	-194 / 0	0.12 (1)
N- D	-314 / 0	0.10 (1)
O- C	-122 / 0	0.02 (1)
L- F	-314 / 0	0.10 (1)
K- G	-122 / 0	0.02 (1)
B- O	0 / 29	0.01 (1)
K- H	0 / 29	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (K-L:3) , WB=0.12/1.00 (E-M:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (E) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T18055545
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T31A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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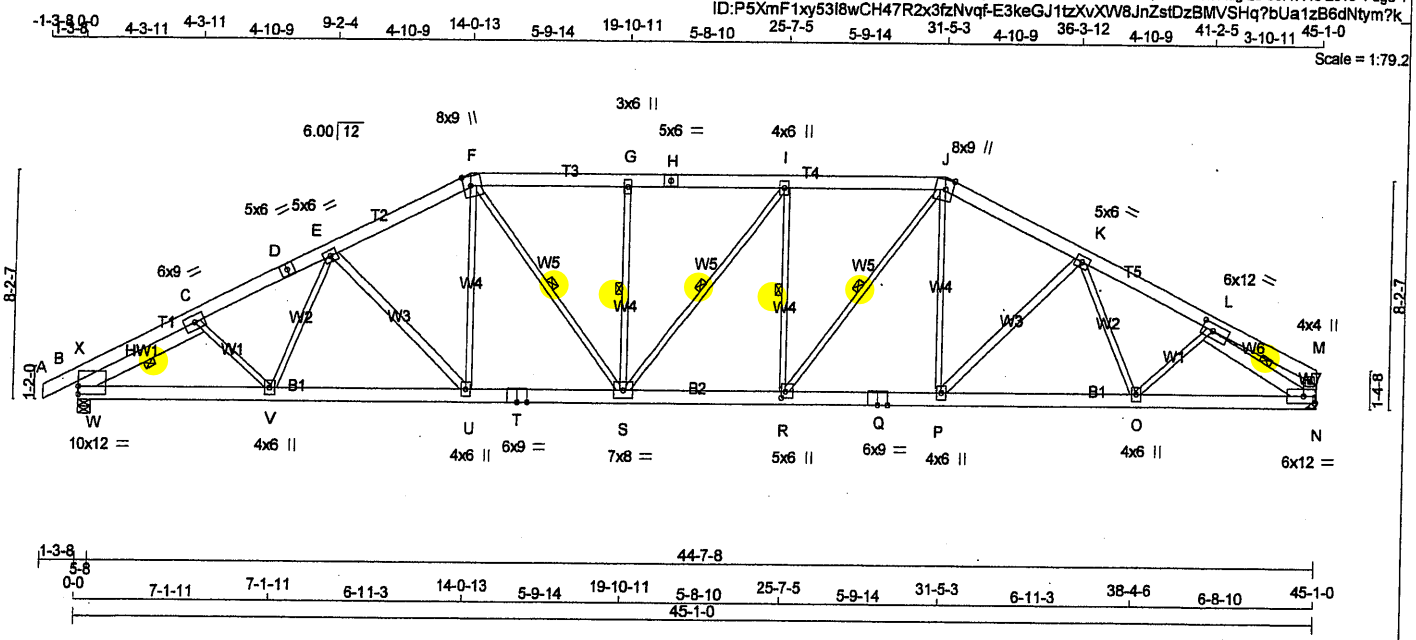

TOTAL WEIGHT = 2 X 264 = 529 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	2100F 1.8E	SPF
T - Q	2x6	DRY	2100F 1.8E	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - U	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
L - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.50	
C	TMWW-t	MT20	6.0	9.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW+m	MT20	8.0	9.0		Edge 3.25
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	8.0	9.0		Edge 3.25
K	TMWW-t	MT20	5.0	6.0		
L	TMWW-t	MT20	6.0	12.0	3.00	5.00
M	TMV+p	MT20	4.0	4.0		
N	BMVW1-t	MT20	6.0	12.0	3.00	5.00
O, P, U, V						
O	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	6.0	9.0		
R	BMWW+t	MT20	5.0	6.0	2.75	1.75
S	BMWWW-t	MT20	7.0	8.0		
T	BS-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4946	0	5337	220	-1099	5-8	3.5
N	4709	0	5030	0	-1033	MECHANICAL	

2-TBEG
-HGUS26

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1099 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1033 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 220 LBS FACTORED HORIZONTAL REACTION AT JOINT B OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3556	2740 / 0	473 / 0	0 / 0	193 / -1173	603 / 0	0 / 0
N	3395	2550 / 0	473 / 0	0 / 0	81 / -1115	586 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	157 / -135	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-S, G-S, I-S, I-R, J-R, L-N, C-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00
B- X	-5835 / 1065	-170.4	-170.4	0.41 (2)	3.30
X- C	-3383 / 768	-170.4	-170.4	0.31 (2)	4.34
C- D	-7786 / 1718	-170.4	-170.4	0.68 (2)	2.61
D- E	-7786 / 1718	-170.4	-170.4	0.68 (2)	2.61
E- F	-6888 / 1623	-170.4	-170.4	0.61 (2)	2.86
F- G	-6887 / 1698	-170.4	-170.4	0.71 (2)	2.73
G- H	-6888 / 1698	-170.4	-170.4	0.72 (2)	2.71
H- I	-6888 / 1698	-170.4	-170.4	0.72 (2)	2.71
I- J	-6826 / 1689	-170.4	-170.4	0.73 (3)	2.72
J- K	-6737 / 1594	-170.4	-170.4	0.60 (3)	2.92
K- L	-7379 / 1629	-170.4	-170.4	0.64 (3)	2.73
L- M	-20 / 86	-170.4	-170.4	0.19 (3)	6.25
N- M	-339 / 135	0.0	0.0	0.02 (3)	7.81
B- W	-766 / 2998	-38.5	-38.5	0.15 (2)	6.25
W- V	-1551 / 6858	-38.5	-38.5	0.40 (2)	6.25
V- U	-1481 / 6957	-38.5	-38.5	0.39 (2)	6.25
U- T	-1187 / 6131	-38.5	-38.5	0.35 (2)	6.25
T- S	-1187 / 6131	-38.5	-38.5	0.35 (2)	6.25
S- R	-1294 / 6827	-38.5	-38.5	0.38 (3)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 638	0.14 (3)
V- E	-322 / 218	0.15 (10)
E- U	-1194 / 422	0.86 (2)
U- F	-229 / 1144	0.26 (13)
F- S	-471 / 2252	0.51 (3)
S- G	-1275 / 382	0.49 (2)
G- I	-479 / 523	0.30 (3)
R- I	-1690 / 471	0.66 (2)
R- J	-501 / 2377	0.53 (2)
P- J	-195 / 985	0.22 (14)
P- K	-993 / 376	0.71 (3)
K- O	-462 / 156	0.22 (9)
O- L	-44 / 911	0.21 (2)
L- N	-7715 / 1561	0.66 (3)
W- X	-415 / 3012	0.00 (1)
W- C	-4802 / 977	0.46 (2)

DWG NO. TAM T1805T1A
STRUCTURAL COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/180 (3.01")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.73/1.00 (I-J:3) , BC=0.40/1.00 (V-W:2) , WB=0.86/0.90 (E-U:2) , SSI=0.42/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
SIMETAL= 0.97 (L) (INPUT = 1.00)

1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T31A	2	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- Q	-993 / 5995	-38.5	-38.5	0.34 (3)	6.25			
Q- P	-993 / 5995	-38.5	-38.5	0.34 (3)	6.25			
P- O	-1231 / 6681	-38.5	-38.5	0.38 (3)	6.25			
O- N	-1240 / 6218	-38.5	-38.5	0.37 (3)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.6} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. ADES
PROVINCE OF ONTARIO

DWG NO. TAM 1605179
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T32	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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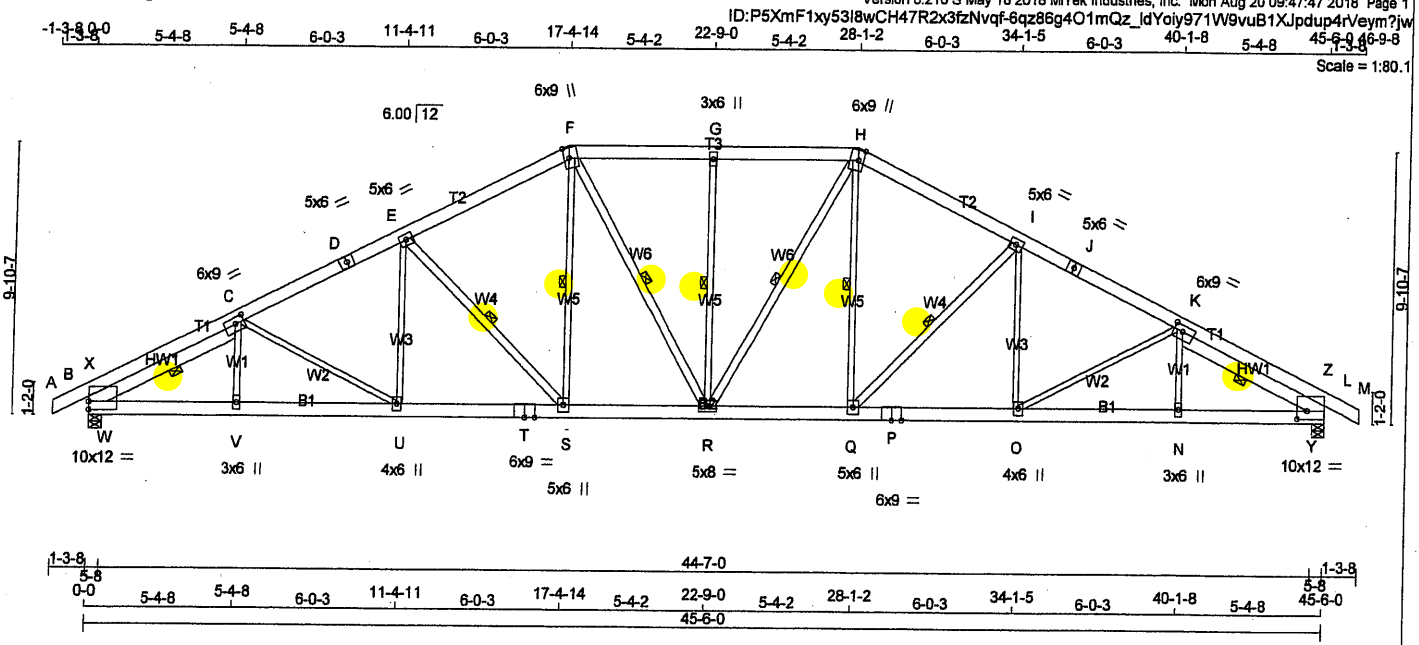


TOTAL WEIGHT = 10 X 287 = 2873 lb

LUMBER

N. L. G. A. RULES

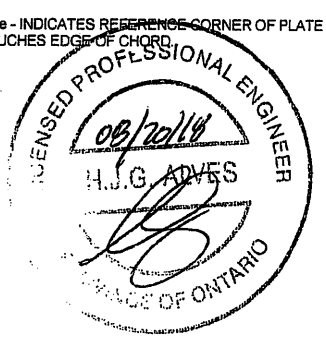
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - S	2x4	DRY	No.2	SPF
F - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.50	
C	TMWWW-t	MT20	6.0	9.0	2.75	4.00
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.25
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWWW-t	MT20	6.0	9.0	2.75	4.00
L	TMBMW1-I	MT20	10.0	12.0	3.50	4.50
N	BMW+w	MT20	3.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW+t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	5.0	6.0		
T	BS-t	MT20	6.0	9.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4990	0	5151	263	-1066	5-8	5-8 2TBEG
L	4990	0	5151	0	-1066	5-8	5-8 2TBEG

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1066 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1066 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 263 LBS FACTORED HORIZONTAL REACTION AT JOINT B OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3587	2608 / 0	478 / 0	0 / 0	172 / -1152	608 / 0	0 / 0
L	3587	2608 / 0	478 / 0	0 / 0	92 / -1152	608 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	188 / -188	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.47 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q, C-W, K-Y.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)		10.00
B- X	-5735 / 1092	-170.4	-170.4	0.48 (2)		3.23
X- C	-3413 / 835	-170.4	-170.4	0.40 (2)		4.22
C- D	-7208 / 1612	-170.4	-170.4	0.89 (2)		2.47
D- E	-7208 / 1612	-170.4	-170.4	0.89 (2)		2.47
E- F	-6122 / 1461	-170.4	-170.4	0.77 (2)		2.91
F- G	-5806 / 1425	-170.4	-170.4	0.57 (2)		3.18
G- H	-5806 / 1425	-170.4	-170.4	0.57 (2)		3.18
H- I	-6122 / 1461	-170.4	-170.4	0.77 (3)		2.91
I- J	-7208 / 1612	-170.4	-170.4	0.89 (3)		2.47
J- K	-7208 / 1612	-170.4	-170.4	0.89 (3)		2.47
K- Z	-3413 / 834	-170.4	-170.4	0.40 (3)		4.22
Z- L	-5735 / 1091	-170.4	-170.4	0.48 (3)		3.23
L- M	0 / 2	-170.4	-170.4	0.13 (3)		10.00
B- W	-853 / 3040	-38.5	-38.5	0.16 (1)		6.25
W- V	-1555 / 6811	-38.5	-38.5	0.37 (2)		6.25
V- U	-1556 / 6808	-38.5	-38.5	0.39 (2)		6.25
U- T	-1358 / 6472	-38.5	-38.5	0.35 (1)		6.25
T- S	-1358 / 6472	-38.5	-38.5	0.35 (1)		6.25
S- R	-977 / 5444	-38.5	-38.5	0.29 (1)		6.25
R- Q	-841 / 5444	-38.5	-38.5	0.29 (1)		6.25
Q- P	-1094 / 6472	-38.5	-38.5	0.35 (1)		6.25
P- O	-1094 / 6472	-38.5	-38.5	0.35 (1)		6.25
O- N	-1295 / 6808	-38.5	-38.5	0.39 (3)		6.25
N- Y	-1293 / 6811	-38.5	-38.5	0.37 (3)		6.25
Y- L	-590 / 3040	-38.5	-38.5	0.16 (1)		6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	0 / 257	0.06 (17)
C- U	-509 / 313	0.44 (2)
U- E	-46 / 509	0.11 (5)
E- S	-1807 / 555	0.67 (2)
S- F	-330 / 1526	0.34 (2)
F- R	-327 / 1564	0.25 (3)
R- G	-1321 / 396	0.78 (2)
R- H	-328 / 1564	0.25 (2)
Q- H	-330 / 1526	0.34 (3)
Q- I	-1807 / 555	0.67 (3)
O- I	-42 / 509	0.11 (6)
O- K	-509 / 305	0.44 (3)
N- K	0 / 257	0.06 (17)
W- X	-367 / 2828	0.00 (1)
W- C	-4602 / 850	0.59 (1)
K- Y	-4603 / 853	0.59 (1)
Y- Z	-367 / 2828	0.00 (1)

DWG NO. TAM 1180518O

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

[M]

SPECIFIED LOADS:

TOP CH.	LL	=	51.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09; CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.89/1.00 (I-K:3) , BC=0.39/1.00 (N-O:3) , WB=0.78/0.90 (G-R:2) , SSI=0.40/1.00 (G-H:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.95 (T) (INPUT = 1.00)

1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T32	10	1	TRUSS DESC.	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.6} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T18057B0
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	G32	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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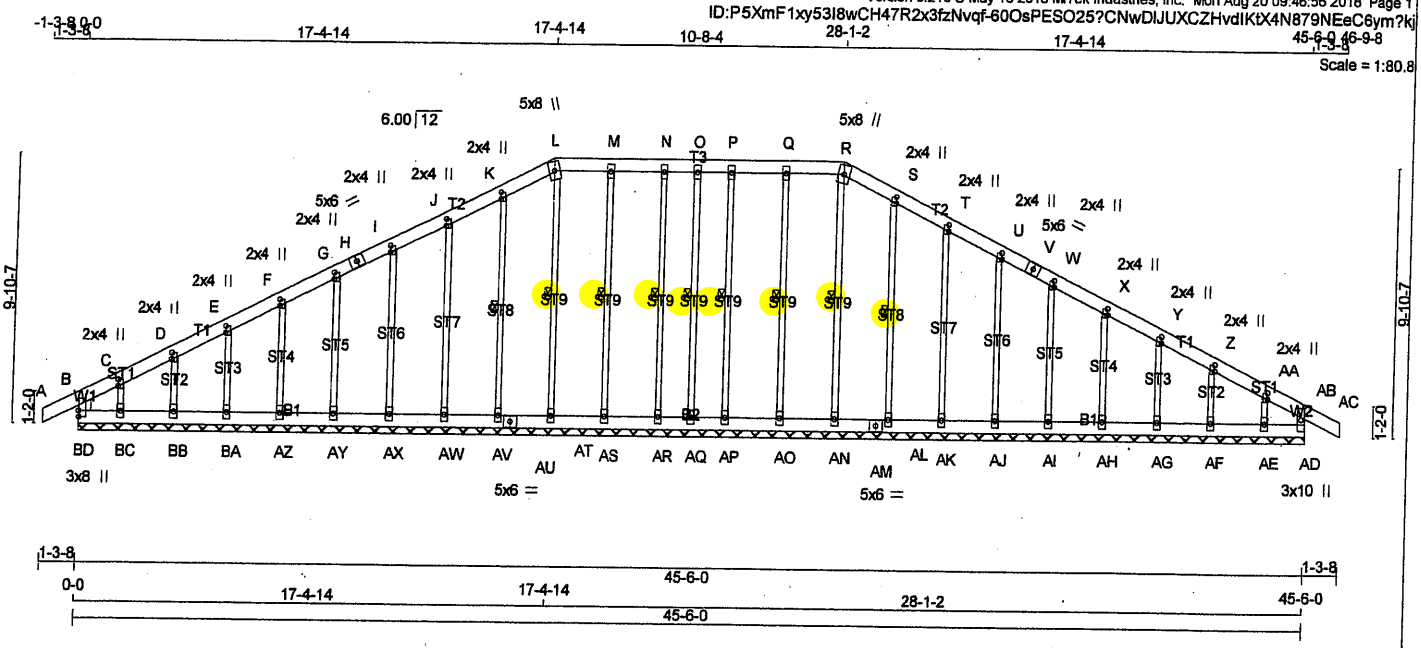

TOTAL WEIGHT = 2 X 287 = 575 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
BD- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - R	2x6	DRY	No.2	SPF
R - V	2x6	DRY	No.2	SPF
V - AC	2x6	DRY	No.2	SPF
AD- AB	2x4	DRY	No.2	SPF
BD- AU	2x6	DRY	No.2	SPF
AU- AM	2x6	DRY	No.2	SPF
AM- AD	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, AB, AD, BD						
B						
C, D, E, F, G, I, J, K, S, T, W, Z, AA						
C	TMW+w	MT20	2.0	4.0	3.00	
H	TS-t	MT20	5.0	6.0		
L	TTW+m	MT20	5.0	8.0		
M, N, O, P, Q						
M	TMW+w	MT20	3.0	6.0		
R	TTW+m	MT20	5.0	8.0		
U, X, Y						
U	TMW+w	MT20	2.0	4.0	2.75	
V	TS-t	MT20	5.0	6.0		
AD	TMBMV1+p	MT20	3.0	10.0		
AE, AF, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR, AS, AT, AV, AW, AX, AY, AZ, BA, BB, BC						
AE	BMW1+w	MT20	3.0	6.0		
AM	BS-t	MT20	5.0	6.0		
AU	BS-t	MT20	5.0	6.0		
BD	TMBMV1+p	MT20	3.0	8.0	2.75	0.25

LICENSED PROFESSIONAL ENGINEER
05/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF R-AN, Q-AO, P-AP, O-AQ, N-AR, M-AS, K-AV, S-AL, L-AT.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
BD- B	-366 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 40	-129.7	-129.7	0.09 (1)	10.00
B- C	-92 / 0	-129.7	-129.7	0.07 (1)	6.25
C- D	-46 / 0	-129.7	-129.7	0.03 (1)	6.25
D- E	-40 / 0	-129.7	-129.7	0.03 (1)	6.25
E- F	-30 / 0	-129.7	-129.7	0.03 (1)	6.25
F- G	-24 / 0	-129.7	-129.7	0.03 (1)	6.25
G- H	-18 / 0	-129.7	-129.7	0.03 (1)	6.25
H- I	-18 / 0	-129.7	-129.7	0.03 (1)	6.25
I- J	-13 / 0	-129.7	-129.7	0.03 (1)	6.25
J- K	-9 / 0	-129.7	-129.7	0.03 (1)	10.00
K- L	-6 / 0	-129.7	-129.7	0.03 (1)	10.00
L- M	-2 / 0	-129.7	-129.7	0.03 (1)	10.00
M- N	-2 / 0	-129.7	-129.7	0.03 (1)	10.00
N- O	-2 / 0	-129.7	-129.7	0.02 (1)	10.00
O- P	-2 / 0	-129.7	-129.7	0.02 (1)	10.00
P- Q	-2 / 0	-129.7	-129.7	0.03 (1)	10.00
Q- R	-2 / 0	-129.7	-129.7	0.03 (1)	10.00
R- S	-6 / 0	-129.7	-129.7	0.03 (1)	10.00
S- T	-9 / 0	-129.7	-129.7	0.03 (1)	10.00
T- U	-13 / 0	-129.7	-129.7	0.03 (1)	6.25
U- V	-18 / 0	-129.7	-129.7	0.03 (1)	6.25
V- W	-18 / 0	-129.7	-129.7	0.03 (1)	6.25
W- X	-24 / 0	-129.7	-129.7	0.03 (1)	6.25
X- Y	-30 / 0	-129.7	-129.7	0.03 (1)	6.25
Y- Z	-40 / 0	-129.7	-129.7	0.03 (1)	6.25
Z-AA	-45 / 0	-129.7	-129.7	0.03 (1)	6.25
AA-AB	-97 / 0	-129.7	-129.7	0.09 (1)	6.25
AB-AC	0 / 40	-129.7	-129.7	0.09 (1)	10.00
AD-AB	-379 / 0	0.0	0.0	0.05 (1)	7.81
BD-BC	0 / 56	-38.5	-38.5	0.04 (1)	10.00
BC-BB	0 / 46	-38.5	-38.5	0.02 (2)	10.00
BB-BA	0 / 35	-38.5	-38.5	0.02 (2)	10.00
BA-AZ	0 / 27	-38.5	-38.5	0.01 (2)	10.00
AZ-AY	0 / 21	-38.5	-38.5	0.01 (2)	10.00
AY-AX	0 / 16	-38.5	-38.5	0.01 (2)	10.00
AX-AW	0 / 12	-38.5	-38.5	0.01 (2)	10.00
AW-AV	0 / 8	-38.5	-38.5	0.01 (3)	10.00
AV-AU	0 / 5	-38.5	-38.5	0.01 (3)	10.00
AU-AT	0 / 5	-38.5	-38.5	0.01 (3)	10.00
AT-AS	0 / 2	-38.5	-38.5	0.01 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AN- R	-265 / 0	0.16 (1)
AO- Q	-269 / 0	0.16 (1)
AP- P	-203 / 0	0.12 (1)
AQ- O	-166 / 0	0.10 (1)
AR- N	-203 / 0	0.12 (1)
AS- M	-269 / 0	0.16 (1)
AV- K	-258 / 0	0.12 (1)
AW- J	-257 / 0	0.27 (1)
AX- I	-257 / 0	0.18 (1)
AY- G	-257 / 0	0.12 (1)
AZ- F	-256 / 0	0.08 (1)
BA- E	-254 / 0	0.05 (1)
BB- D	-269 / 0	0.04 (1)
BC- C	-136 / 0	0.02 (1)
AL- S	-258 / 0	0.12 (1)
AK- T	-257 / 0	0.27 (1)
AJ- U	-257 / 0	0.18 (1)
AI- W	-257 / 0	0.12 (1)
AH- X	-256 / 0	0.08 (1)
AG- Y	-254 / 0	0.05 (1)
AF- Z	-270 / 0	0.04 (1)
AE-AA	-122 / 0	0.02 (1)
AT- L	-265 / 0	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (AB-AC:1) , BC=0.04/1.00 (BC-BD:1) , WB=0.27/1.00 (T-AK:1) , SSI=0.10/1.00 (AB-AC:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (AA) (INPUT = 0.90)
JSI METAL= 0.14 (AD) (INPUT = 1.00)

DWG NO. TAM 11805167
STRUCTURAL
COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	G32	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AS-AR	0 / 2	-38.5	-38.5	0.01 (3)	10.00			
AR-AQ	0 / 2	-38.5	-38.5	0.01 (3)	10.00			
AQ-AP	0 / 2	-38.5	-38.5	0.01 (3)	10.00			
AP-AO	0 / 2	-38.5	-38.5	0.01 (3)	10.00			
AO-AN	0 / 2	-38.5	-38.5	0.01 (3)	10.00			
AN-AM	0 / 5	-38.5	-38.5	0.01 (3)	10.00			
AM-AL	0 / 5	-38.5	-38.5	0.01 (3)	10.00			
AL-AK	0 / 8	-38.5	-38.5	0.01 (3)	10.00			
AK-AJ	0 / 12	-38.5	-38.5	0.01 (2)	10.00			
AJ-AI	0 / 16	-38.5	-38.5	0.01 (2)	10.00			
AI-AH	0 / 21	-38.5	-38.5	0.01 (2)	10.00			
AH-AG	0 / 27	-38.5	-38.5	0.01 (2)	10.00			
AG-AF	0 / 35	-38.5	-38.5	0.02 (2)	10.00			
AF-AE	0 / 46	-38.5	-38.5	0.02 (2)	10.00			
AE-AD	0 / 55	-38.5	-38.5	0.03 (1)	10.00			

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805767
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T32A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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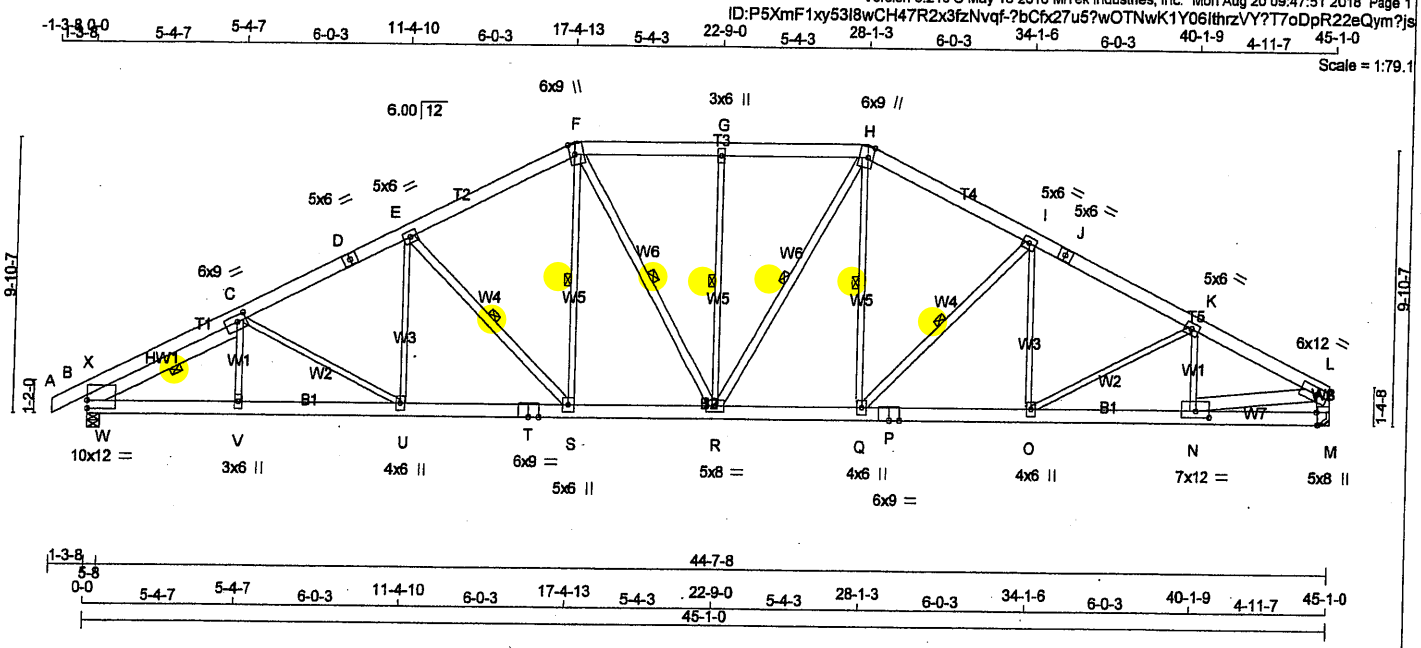

TOTAL WEIGHT = 2 X 282 = 563 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
B - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - S	2x4	DRY	No.2	SPF
F - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - I	2x4	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X	
B	TMBMW1-I	MT20	10.0	12.0	3.50		
C	TMWWW-t	MT20	6.0	9.0	2.75	4.00	
D	TS-t	MT20	5.0	6.0			
E, I, K							
E	TMWW-t	MT20	5.0	6.0			
F	TTWW+m	MT20	6.0	9.0			Edge 2.25
G	TMW+w	MT20	3.0	6.0			
H	TTWW+m	MT20	6.0	9.0			Edge 2.25
J	TS-t	MT20	5.0	6.0			
L	TMWW-t	MT20	6.0	12.0			Edge
M	BMV1+t	MT20	5.0	8.0			Edge 0.50
N	BMWW-t	MT20	7.0	12.0	2.75	6.00	
O, Q, U							
O	BMWW+t	MT20	4.0	6.0			
P	BS-t	MT20	6.0	9.0			
R	BMWWW-t	MT20	5.0	8.0			
S	BMWW+t	MT20	5.0	6.0			
T	BS-t	MT20	6.0	9.0			
V	BMW+w	MT20	3.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

PROFESSIONAL ENGINEER
LICENSED
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG
B	4946	0	5125	266	-1059	5-8	5-8 - 2TBEC
M	4709	0	4812	0	-992	MECHANICAL	2 HGUS26

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1059 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 992 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 266 LBS FACTORED HORIZONTAL REACTION AT JOINT B OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3556	2598 / 0	473 / 0	0 / 0	171 / -1144	603 / 0	0 / 0
M	3395	2404 / 0	473 / 0	0 / 0	98 / -1085	586 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	190 / -168	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q, C-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00	V- C	0 / 258	0.06 (17)
B- X	-5704 / 1083	-170.4	-170.4	0.48 (2)	3.25	C- U	-514 / 304	0.45 (2)
X- C	-3395 / 830	-170.4	-170.4	0.40 (2)	4.24	U- E	-41 / 511	0.11 (5)
C- D	-7122 / 1598	-170.4	-170.4	0.89 (2)	2.48	E- S	-1809 / 555	0.67 (2)
D- E	-7122 / 1598	-170.4	-170.4	0.89 (2)	2.48	S- F	-330 / 1528	0.34 (2)
E- F	-6033 / 1446	-170.4	-170.4	0.77 (2)	2.92	F- R	-317 / 1514	0.24 (3)
F- G	-5702 / 1408	-170.4	-170.4	0.57 (2)	3.21	R- G	-1323 / 396	0.78 (3)
G- H	-5702 / 1408	-170.4	-170.4	0.57 (2)	3.21	R- H	-336 / 1594	0.26 (2)
H- I	-5978 / 1429	-170.4	-170.4	0.74 (3)	2.98	Q- H	-306 / 1413	0.32 (3)
I- J	-6953 / 1557	-170.4	-170.4	0.85 (3)	2.58	Q- I	-1646 / 521	0.61 (3)
J- K	-6953 / 1557	-170.4	-170.4	0.85 (3)	2.58	O- I	-119 / 388	0.09 (6)
K- L	-7019 / 1485	-170.4	-170.4	0.60 (3)	2.87	O- K	-216 / 480	0.19 (3)
M- L	-4704 / 1008	0.0	0.0	0.30 (3)	4.75	N- K	-971 / 310	0.20 (3)
						N- L	-1255 / 6414	0.76 (3)
B- W	-852 / 3024	-38.5	-38.5	0.16 (2)	6.25	W- X	-362 / 2797	0.00 (1)
W- V	-1547 / 6770	-38.5	-38.5	0.37 (2)	6.25	W- C	-4551 / 842	0.58 (1)
V- U	-1548 / 6767	-38.5	-38.5	0.39 (2)	6.25			
U- T	-1349 / 6395	-38.5	-38.5	0.34 (1)	6.25			
T- S	-1349 / 6395	-38.5	-38.5	0.34 (1)	6.25			
S- R	-968 / 5364	-38.5	-38.5	0.28 (1)	6.25			
R- Q	-824 / 5315	-38.5	-38.5	0.28 (1)	6.25			

DWG NO. TAM 1160581
STRUCTURAL COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 51.8 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 75.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(60 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/180 (3.01")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.89/1.00 (C-E:2) , BC=0.39/1.00 (U-V:2) , WB=0.78/0.90 (G-R:3) , SSI=0.40/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (R) (INPUT = 0.90)
JSI METAL= 0.94 (T) (INPUT = 1.00)

1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T32A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
Q- P	-1072 / 6236	-38.5	-38.5	0.34 (1)	6.25			
P- O	-1072 / 6236	-38.5	-38.5	0.34 (1)	6.25			
O- N	-1217 / 6313	-38.5	-38.5	0.37 (3)	6.25			
N- M	-10 / 20	-38.5	-38.5	0.06 (3)	10.00			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.6} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ADAMES
PROVINCE OF ONTARIO

DWG NO. TAM T2605181
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T33A	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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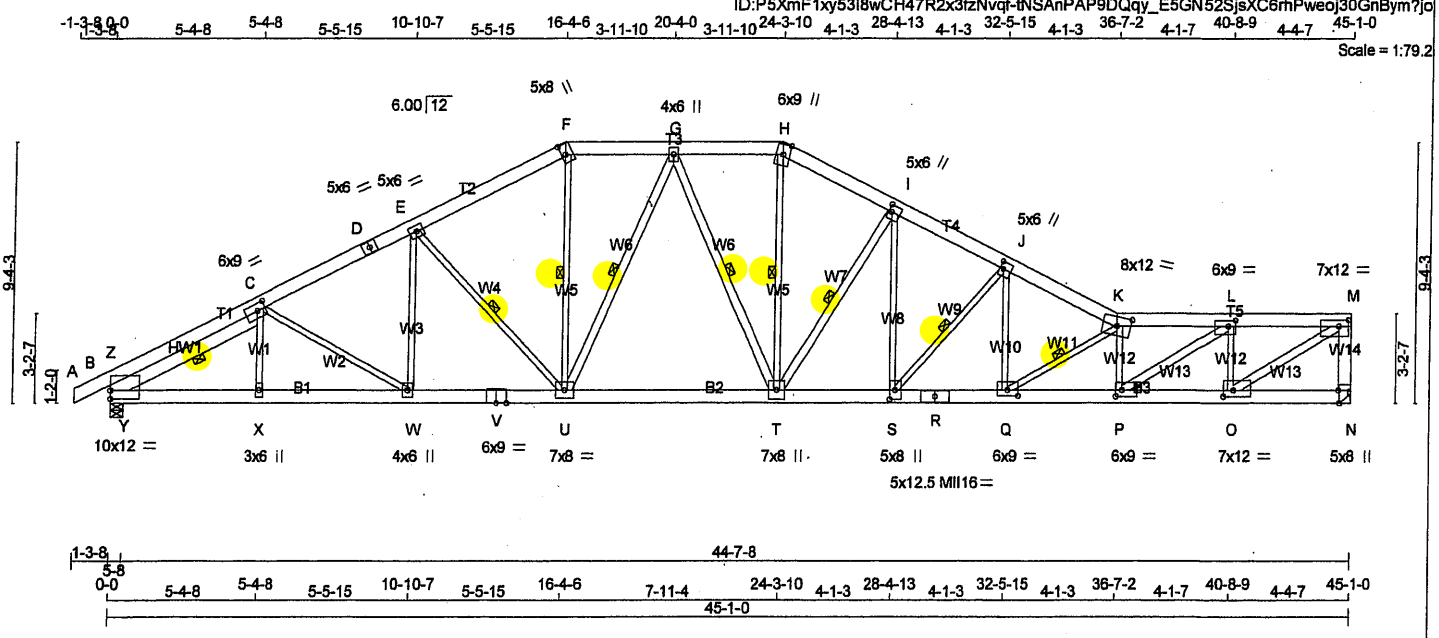


TOTAL WEIGHT = 6 X 279 = 1672 lb [M]

LUMBER

N. L. G. A. RULES

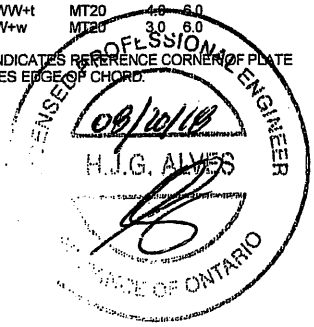
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
B - V	2x6	DRY	2100F 1.8E	SPF
V - R	2x6	DRY	2100F 1.8E	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - L	2x4	DRY	2100F 1.8E	SPF
O - M	2x4	DRY	2100F 1.8E	SPF
U - G	2x4	DRY	No.2	SPF
G - T	2x4	DRY	No.2	SPF
T - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
C	TMWWW-t	MT20	6.0	9.0	3.00	3.75
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTW+h	MT20	5.0	8.0	4.50	1.75
G	TMWW+t	MT20	4.0	6.0		
H	TTW+m	MT20	6.0	9.0	Edge	
I	TMWW+t	MT20	5.0	6.0	3.00	1.25
J	TMWW+t	MT20	5.0	6.0	3.00	1.50
K	TTWW-m	MT20	8.0	12.0	4.00	5.75
L	TMWW-t	MT20	6.0	9.0	2.50	3.25
M	TMVW-t	MT20	7.0	12.0	2.75	4.25
N	BMV1+t	MT20	5.0	8.0	Edge	0.50
O	BMWW-t	MT20	7.0	12.0	2.75	4.25
P	BMWW-t	MT20	6.0	9.0	2.75	2.75
Q	BMWW-t	MT20	6.0	9.0	2.50	4.50
R	BS-t	MII16	5.0	12.5		
S	BMWW+t	MT20	5.0	8.0	4.00	2.25
T	BMWWW+t	MT20	7.0	8.0		
U	BMWWW-t	MT20	7.0	8.0		
V	BS-t	MT20	6.0	9.0		
W	BMWW+t	MT20	4.0	6.0		
X	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	4709	0	4885	0	-1008	MECHANICAL	HGUS26 - 2TDE6
B	4946	0	4946	301	-1017	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1008 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1017 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 301 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3395	2453 / 0	473 / 0	0 / 0	131 / -1097	586 / 0	0 / 0
B	3556	2480 / 0	473 / 0	0 / 0	150 / -1114	603 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	215 / -117	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 1.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-U, F-U, H-T, G-U, G-T, I-T, J-S, K-Q, C-Y.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)				FR-TO		
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00	X- C	0 / 252	0.06 (17)
B- Z	-5569 / 1030	-170.4	-170.4	0.46 (2)	3.33	C- W	-522 / 328	0.38 (2)
Z- C	-3302 / 808	-170.4	-170.4	0.39 (2)	4.32	W- E	-74 / 475	0.11 (2)
C- D	-7168 / 1528	-170.4	-170.4	0.69 (2)	2.78	E- U	-1637 / 514	0.74 (2)
D- E	-7168 / 1528	-170.4	-170.4	0.69 (2)	2.78	U- F	-430 / 2079	0.47 (1)
E- F	-6232 / 1392	-170.4	-170.4	0.60 (2)	3.07	T- H	-490 / 2371	0.53 (1)
F- G	-5558 / 1332	-170.4	-170.4	0.32 (1)	3.48	P- K	-3285 / 742	0.56 (1)
G- H	-5963 / 1462	-170.4	-170.4	0.34 (3)	3.34	P- L	-1158 / 5750	0.43 (1)
H- I	-6670 / 1560	-170.4	-170.4	0.40 (3)	3.14	O- L	-4152 / 957	0.71 (3)
I- J	-8138 / 1819	-170.4	-170.4	0.51 (3)	2.76	O- M	-1719 / 8312	0.62 (3)
J- K	-10015 / 2142	-170.4	-170.4	0.72 (3)	2.22	U- G	-1678 / 387	0.78 (3)
K- L	-11715 / 2469	-170.4	-170.4	0.81 (3)	1.76	G- T	-907 / 717	0.41 (2)
L- M	-7039 / 1514	-170.4	-170.4	0.47 (3)	2.95	T- I	-2871 / 756	0.85 (3)
N- M	-4800 / 1026	0.0	0.0	0.44 (3)	4.70	S- I	-475 / 2258	0.51 (3)
						S- J	-2854 / 689	0.76 (3)
B- Y	-822 / 2935	-38.5	-38.5	0.16 (1)	6.25	Q- J	-390 / 2198	0.49 (3)
Y- X	-1468 / 6667	-38.5	-38.5	0.37 (1)	6.25	Q- K	-3508 / 772	0.71 (3)
X- W	-1469 / 6664	-38.5	-38.5	0.38 (1)	6.25	Y- Z	-332 / 2776	0.00 (1)
W- V	-1284 / 6431	-38.5	-38.5	0.36 (1)	6.25	Y- C	-4525 / 783	0.58 (1)
V- U	-1284 / 6431	-38.5	-38.5	0.36 (1)	6.25			
U- T	-981 / 5915	-38.5	-38.5	0.35 (1)	6.25			
T- S	-1291 / 7284	-38.5	-38.5	0.42 (1)	6.25			
S- R	-1740 / 8986	-38.5	-38.5	0.48 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.51")
ALLOWABLE DEFL.(TL)= L/180 (3.01")
CALCULATED VERT. DEFL.(TL) = L/ 846 (0.64")

CSI: TC=0.81/1.00 (K-L:3) , BC=0.72/1.00 (P-Q:3) , WB=0.85/0.90 (I-T:3) , SSI=0.36/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656
MII16	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 1.00 (K) (INPUT = 1.00)

DWG NO. TAM STRUCTURAL M605182 1/2
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T33A	6	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- Q	-1740 / 8986	-38.5	-38.5	0.48 (1)	6.25			
Q- P	-2383 / 11838	-38.5	-38.5	0.72 (3)	6.25			
P- O	-1398 / 7039	-38.5	-38.5	0.49 (3)	6.25			
O- N	-22 / 58	-38.5	-38.5	0.04 (6)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.6} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MYES
PROVINCE OF ONTARIO

DWG NO. TAM T1805182
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T35	2	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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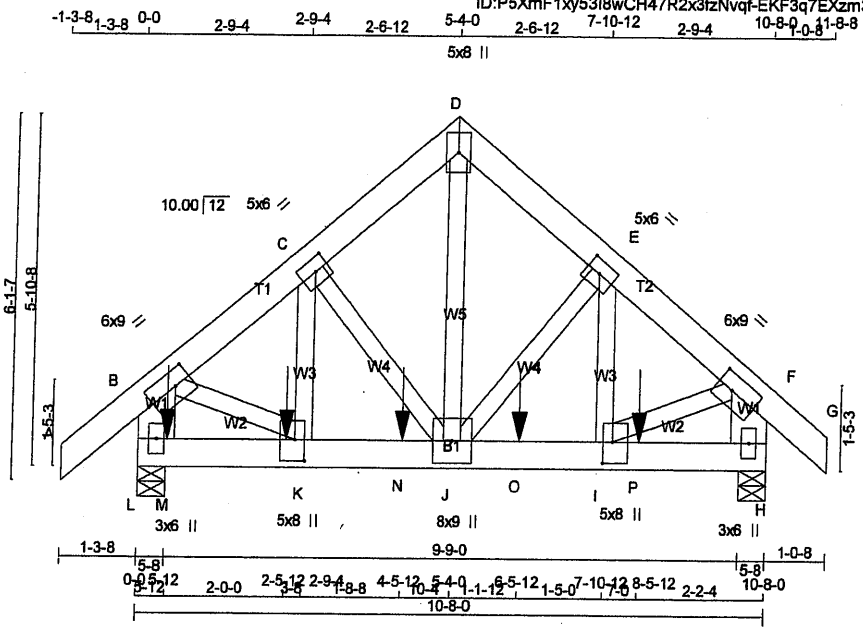

TOTAL WEIGHT = 8 X 80 = 641 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
L - B	2x8	DRY	No.2	SPF
H - F	2x8	DRY	No.2	SPF
L - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- G	2	12	TOP
L- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : MITRSLK-G006 SCREWS			
L- H	2	4	SIDE(3422.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES. SCREWS TO BE DRIVEN FROM ONE SIDE ONLY. DO NOT SINK SCREW HEADS INTO LUMBER.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL A 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

BOLTING MAY BE OMITTED IN PANELS THAT ARE CONNECTED WITH SCREWS.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	3.00	3.25
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTW+p	MT20	5.0	8.0		
E	TMWW-t	MT20	5.0	6.0	2.50	2.75
F	TMVW-t	MT20	6.0	9.0	3.00	3.25
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	8.0	4.25	2.00
J	BMWWW+t	MT20	8.0	9.0		
K	BMWW+t	MT20	5.0	8.0	4.25	2.00
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	15331	0	15331	290	-3183	5-8	5-8 -2+522
H	11659	0	11659	0	-2417	5-8	5-8 -2+522

PROVIDE ANCHORAGE AT BEARING JOINT L FOR 3183 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 2417 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 290 LBS FACTORED HORIZONTAL REACTION AT JOINT L ok

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	10993	7773 / 0	1417 / 0	0 / 0	368 / -3433	1803 / 0	0 / 0
H	8359	5915 / 0	1074 / 0	0 / 0	341 / -2607	1370 / 0	0 / 0
HORIZONTAL REACTIONS							
L	—	0 / 0	0 / 0	0 / 0	207 / -205	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 93	-170.4	-170.4	0.05 (2)	10.00
B- C	-12181 / 2581	-170.4	-170.4	0.17 (3)	4.52
C- D	-9409 / 2103	-170.4	-170.4	0.15 (3)	5.03
D- E	-9414 / 2104	-170.4	-170.4	0.15 (2)	5.04
E- F	-11699 / 2482	-170.4	-170.4	0.16 (2)	4.60
F- G	0 / 77	-170.4	-170.4	0.03 (3)	10.00
L- B	-11407 / 2384	0.0	0.0	0.15 (1)	6.71
H- F	-10932 / 2286	0.0	0.0	0.15 (1)	6.82
L- M	-271 / 278	-38.5	-38.5	0.17 (2)	6.25
M- K	-271 / 278	-38.5	-38.5	0.17 (2)	6.25
K- N	-1973 / 9276	-38.5	-38.5	0.27 (1)	6.25
N- J	-1973 / 9276	-38.5	-38.5	0.27 (1)	6.25
J- O	-1777 / 8909	-38.5	-38.5	0.27 (1)	6.25
O- I	-1777 / 8909	-38.5	-38.5	0.27 (1)	6.25
I- P	-9 / 19	-38.5	-38.5	0.14 (3)	10.00
P- H	-9 / 19	-38.5	-38.5	0.14 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-2528 / 11405	0.46 (1)
J- E	-2662 / 719	0.13 (3)
I- E	-695 / 3331	0.13 (2)
C- J	-3250 / 839	0.15 (2)
K- C	-853 / 4111	0.17 (3)
B- K	-1962 / 9758	0.39 (1)
I- F	-1889 / 9372	0.38 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	2-5-12	-3574	-3574	543	BACK	VERT	TOTAL
M	5-12	-3437	-3437	509	BACK	VERT	TOTAL
N	4-5-12	-3477	-3477	525	BACK	VERT	TOTAL
O	6-5-12	-3477	-3477	525	BACK	VERT	TOTAL
P	8-5-12	-3477	-3477	525	BACK	VERT	TOTAL

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/180 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.17/1.00 (B-C:3) , BC=0.27/1.00 (J-K:1) , WB=0.46/1.00 (D-J:1) , SSI=0.64/1.00 (H-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.84 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1180518 3
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T35	2	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4783.3 lbs FACTORED DOWN AND 1001.0 lbs FACTORED UP AT 5-12, 4991.9 lbs FACTORED DOWN AND 1045.7 lbs FACTORED UP AT 2-5-12, 4846.3 lbs FACTORED DOWN AND 1020.4 lbs FACTORED UP AT 4-5-12, AND 4846.3 lbs FACTORED DOWN AND 1020.4 lbs FACTORED UP AT 6-5-12, AND 4846.3 lbs FACTORED DOWN AND 1020.4 lbs FACTORED UP AT 8-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.6} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805183
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	G36	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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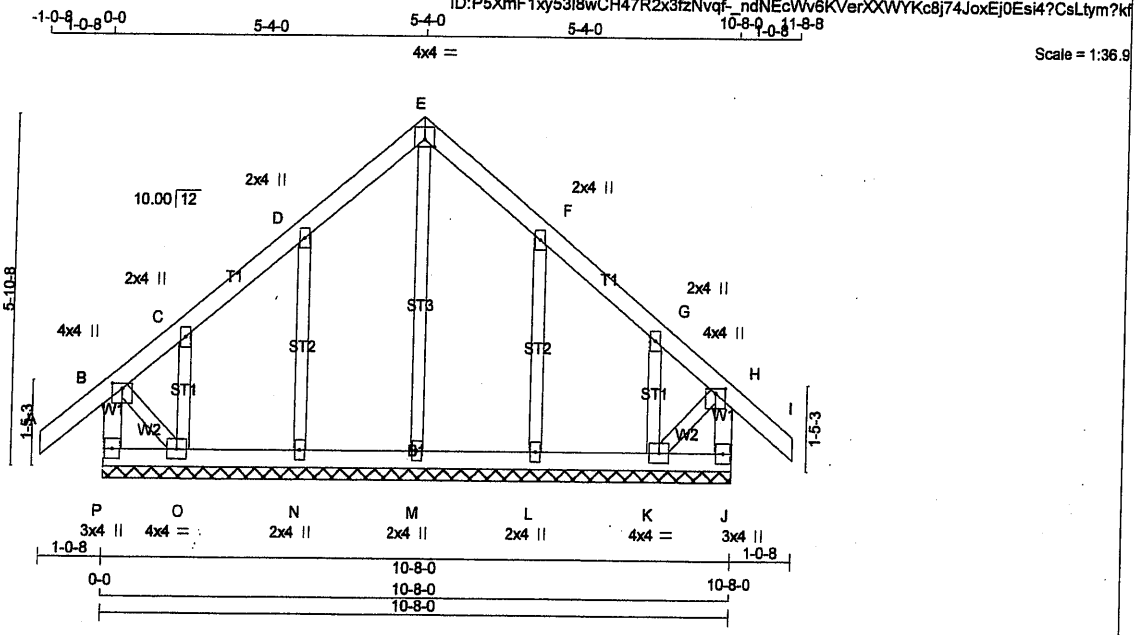

TOTAL WEIGHT = 2 X 49 = 99 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-332 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 47	-129.7	-129.7	0.12 (1)	10.00
B- C	-75 / 0	-129.7	-129.7	0.11 (1)	6.25
C- D	-20 / 0	-129.7	-129.7	0.08 (1)	6.25
D- E	-42 / 0	-129.7	-129.7	0.08 (1)	6.25
E- F	-42 / 0	-129.7	-129.7	0.08 (1)	6.25
F- G	-20 / 0	-129.7	-129.7	0.08 (1)	6.25
G- H	-75 / 0	-129.7	-129.7	0.11 (1)	6.25
H- I	0 / 47	-129.7	-129.7	0.12 (1)	10.00
J- H	-332 / 0	0.0	0.0	0.03 (1)	7.81
P- O	0 / 0	-38.5	-38.5	0.02 (3)	10.00
O- N	0 / 26	-38.5	-38.5	0.03 (2)	10.00
N- M	0 / 18	-38.5	-38.5	0.03 (3)	10.00
M- L	0 / 18	-38.5	-38.5	0.03 (3)	10.00
L- K	0 / 26	-38.5	-38.5	0.03 (2)	10.00
K- J	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-184 / 0	0.09 (1)
N- D	-309 / 0	0.08 (1)
O- C	-131 / 0	0.02 (1)
L- F	-309 / 0	0.08 (1)
K- G	-131 / 0	0.02 (1)
B- O	0 / 40	0.01 (1)
K- H	0 / 40	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (H-I:1) , BC=0.03/1.00 (K-L:2) , WB=0.09/1.00 (E-M:1) , SSI=0.10/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (E) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805168
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T104	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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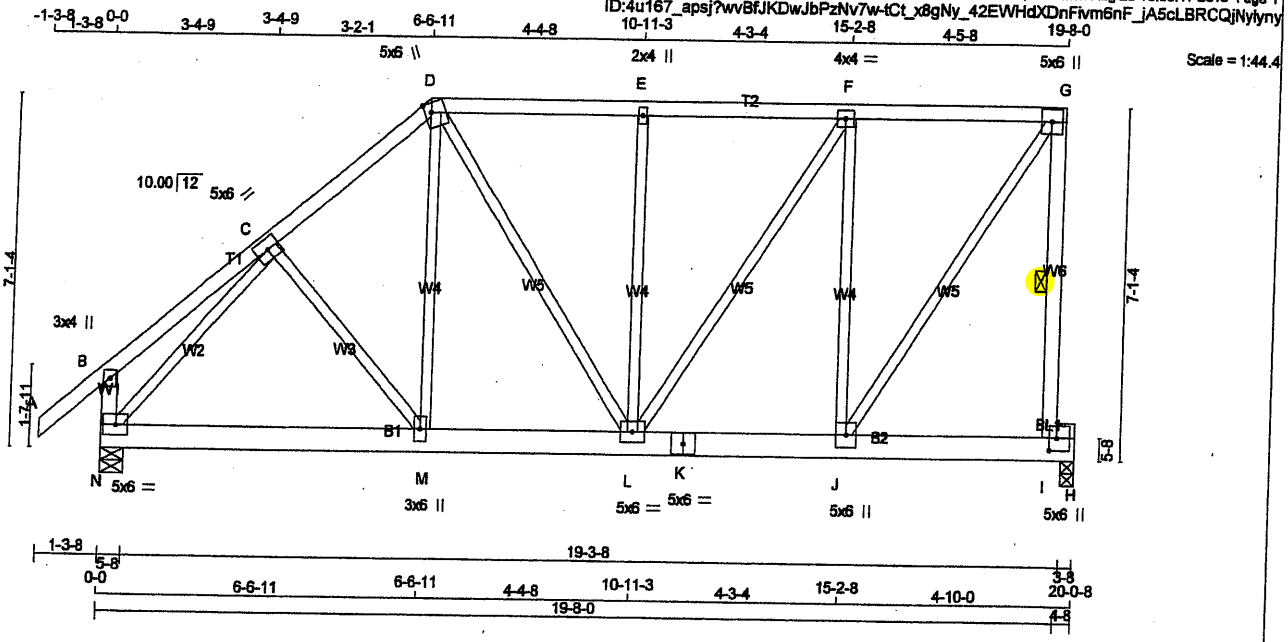


TOTAL WEIGHT = 2 X 113 = 227 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	5.0	6.0		
I	BMVK+p	MT20	5.0	6.0	3.00	1.75
J	BMWW+t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1864	0	1864	0	0	5-8	5-8
H	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
H	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

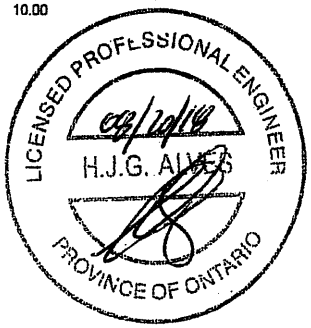
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 30	-129.7	-129.7	0.21 (1)	10.00
C- D	-1499 / 0	-129.7	-129.7	0.25 (1)	5.09
D- E	-1267 / 0	-129.7	-129.7	0.35 (1)	5.27
E- F	-1268 / 0	-129.7	-129.7	0.36 (1)	5.25
F- G	-992 / 0	-129.7	-129.7	0.35 (1)	5.77
I- G	-1791 / 0	0.0	0.0	0.39 (1)	4.99
N- B	-343 / 0	0.0	0.0	0.04 (1)	7.81
N- M	0 / 1169	-38.5	-38.5	0.24 (2)	10.00
M- L	0 / 1131	-38.5	-38.5	0.23 (2)	10.00
L- K	0 / 992	-38.5	-38.5	0.29 (1)	10.00
K- J	0 / 992	-38.5	-38.5	0.29 (1)	10.00
J- I	-39 / 0	-38.5	-38.5	0.57 (1)	6.25
I- H	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-66 / 70	0.03 (1)
M- D	0 / 353	0.08 (2)
N- C	-1834 / 0	0.75 (1)
J- G	0 / 1866	0.42 (1)
D- L	0 / 251	0.06 (1)
J- F	-1056 / 0	0.87 (1)
L- E	-604 / 0	0.49 (1)
L- F	0 / 514	0.12 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.39/1.00 (G-I:1) , BC=0.57/1.00 (I-J:1) , WB=0.87/1.00 (F-J:1) , SSI=0.60/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)

DWG NO. TAM 11805277
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T105	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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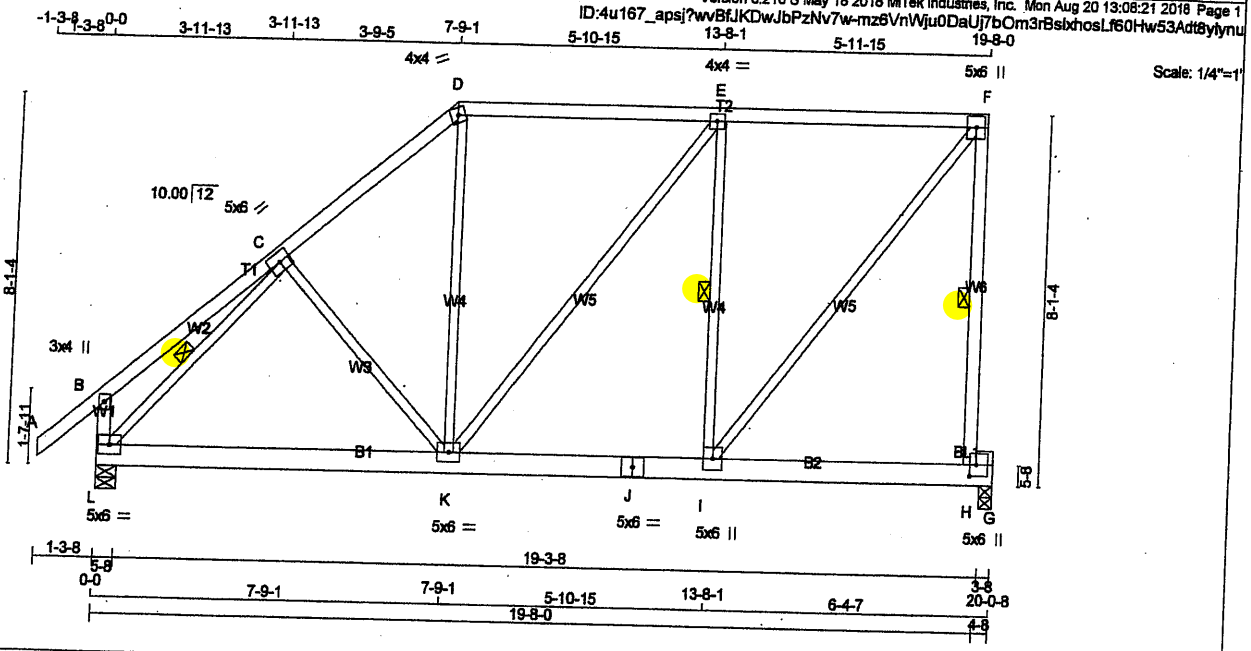

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0		
H	BMVK+p	MT20	5.0	6.0	3.00	1.75
I	BMWW+t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1864	0	1864	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, E-I, C-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 37	-129.7	-129.7	0.31 (1)	10.00
C- D	-1410 / 0	-129.7	-129.7	0.36 (1)	5.06
D- E	-1057 / 0	-129.7	-129.7	0.84 (1)	4.43
E- F	-1057 / 0	-129.7	-129.7	0.84 (1)	4.43
H- F	-1699 / 0	0.0	0.0	0.48 (1)	5.10
L- B	-371 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 1181	-38.5	-38.5	0.30 (2)	10.00
K- J	0 / 1057	-38.5	-38.5	0.32 (1)	10.00
J- I	0 / 1057	-38.5	-38.5	0.32 (1)	10.00
I- H	-32 / 0	-38.5	-38.5	0.57 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-192 / 42	0.12 (1)
K- D	0 / 401	0.09 (2)
K- E	0 / 55	0.01 (3)
I- E	-957 / 0	0.38 (1)
I- F	0 / 1777	0.40 (1)
L- C	-1801 / 0	0.44 (1)

TOTAL WEIGHT = 4 X 109 = 437 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.84/1.00 (E-F:1) , BC=0.57/1.00 (H-I:1) , WB=0.44/1.00 (C-L:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.44 (C) (INPUT = 1.00)

DWG NO. TAM T1805278
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T106	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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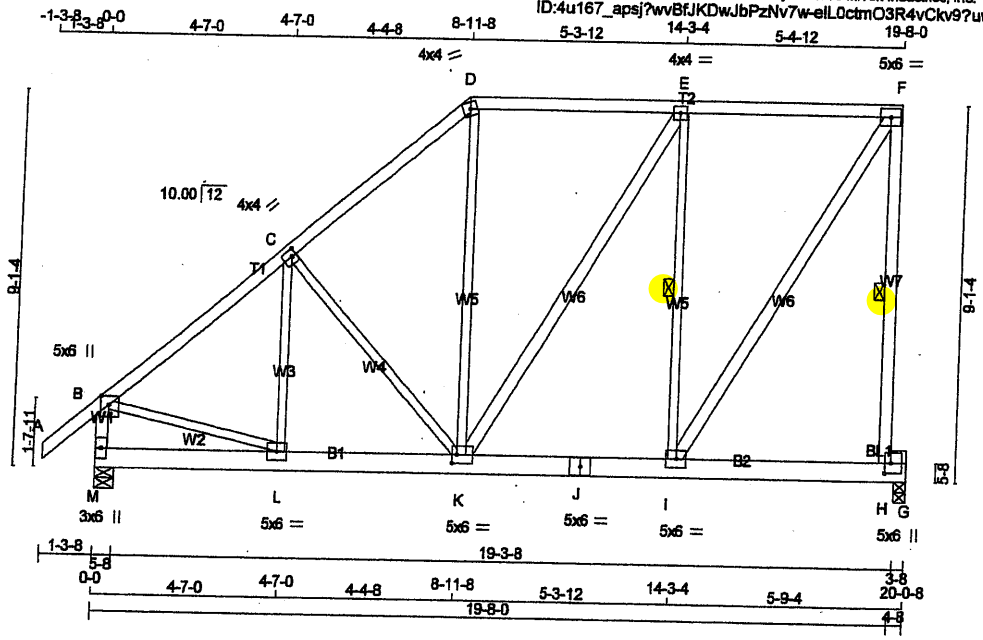

TOTAL WEIGHT = 10 X 123 = 1231 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVK+p	MT20	5.0	6.0	3.00	1.75
I	BMVW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0	2.50	1.50
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1864	0	1864	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1586 / 0	-129.7	-129.7	0.52 (1)	4.61
C- D	-1288 / 0	-129.7	-129.7	0.49 (1)	5.03
D- E	-955 / 0	-129.7	-129.7	0.66 (1)	5.19
E- F	-879 / 0	-129.7	-129.7	0.65 (1)	5.36
H- F	-1736 / 0	0.0	0.0	0.64 (1)	5.05
M- B	-1786 / 0	0.0	0.0	0.19 (1)	6.25
M- L	0 / 0	-38.5	-38.5	0.06 (2)	10.00
L- K	0 / 1252	-38.5	-38.5	0.20 (1)	10.00
K- J	0 / 879	-38.5	-38.5	0.31 (1)	10.00
J- I	0 / 879	-38.5	-38.5	0.31 (1)	10.00
I- H	-29 / 0	-38.5	-38.5	0.57 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-137 / 120	0.06 (1)
C- K	-453 / 0	0.37 (1)
K- D	0 / 332	0.07 (2)
K- E	0 / 149	0.02 (1)
I- E	-986 / 0	0.51 (1)
I- F	0 / 1727	0.28 (1)
B- L	0 / 1296	0.29 (1)

LICENSED PROFESSIONAL ENGINEER
09/10/19
H.J.G. MAES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.66/1.00 (D-E:1) , BC=0.57/1.00 (H-I:1) , WB=0.51/1.00 (E-I:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)

DWG NO. TAM T1805279
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T107	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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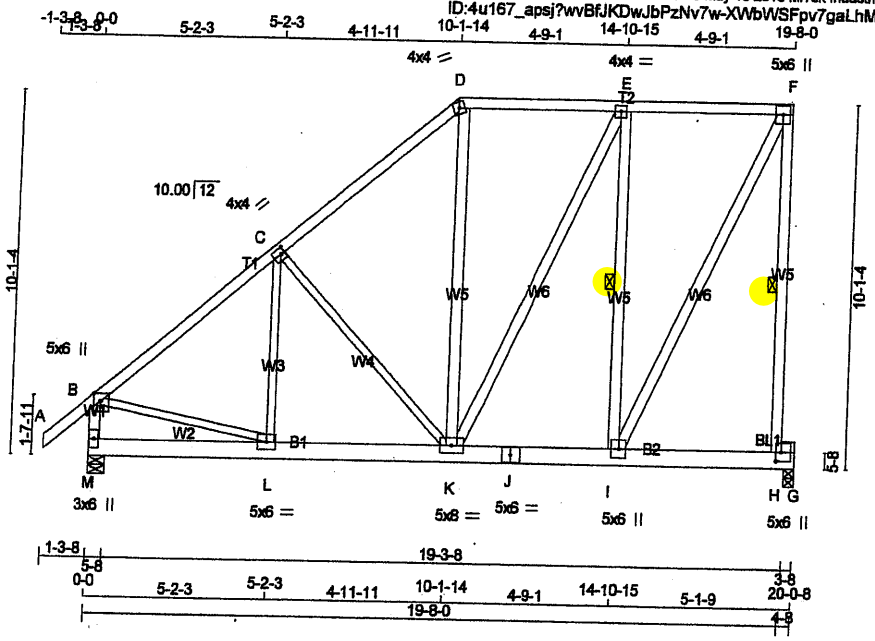

TOTAL WEIGHT = 5 X 135 = 676 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0		
H	BMVK+p	MT20	5.0	6.0	3.00	1.75
I	BMWW+t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1864	0	1864	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	736 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1583 / 0	-129.7	-129.7	0.69 (1)	4.33
C- D	-1177 / 0	-129.7	-129.7	0.64 (1)	4.92
D- E	-865 / 0	-129.7	-129.7	0.51 (1)	5.76
E- F	-724 / 0	-129.7	-129.7	0.50 (1)	6.16
H- F	-1769 / 0	0.0	0.0	0.85 (1)	5.02
M- B	-1778 / 0	0.0	0.0	0.19 (1)	6.26
M- L	0 / 0	-38.5	-38.5	0.09 (3)	10.00
L- K	0 / 1255	-38.5	-38.5	0.21 (1)	10.00
K- J	0 / 724	-38.5	-38.5	0.26 (1)	10.00
J- I	0 / 724	-38.5	-38.5	0.26 (1)	10.00
I- H	-27 / 0	-38.5	-38.5	0.57 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-69 / 175	0.04 (3)
C- K	-580 / 0	0.64 (1)
K- D	0 / 266	0.04 (2)
K- E	0 / 331	0.05 (1)
I- E	-1059 / 0	0.51 (1)
I- F	0 / 1708	0.27 (1)
B- L	0 / 1289	0.29 (1)

LICENSED PROFESSIONAL ENGINEER
08/10/14
H.J.G. MYERS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.85/1.00 (F-H:1) , BC=0.57/1.00 (H-I:1) , WB=0.64/1.00 (C-K:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.35 (F) (INPUT = 1.00)

DWG NO. TAM T1805280
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T110	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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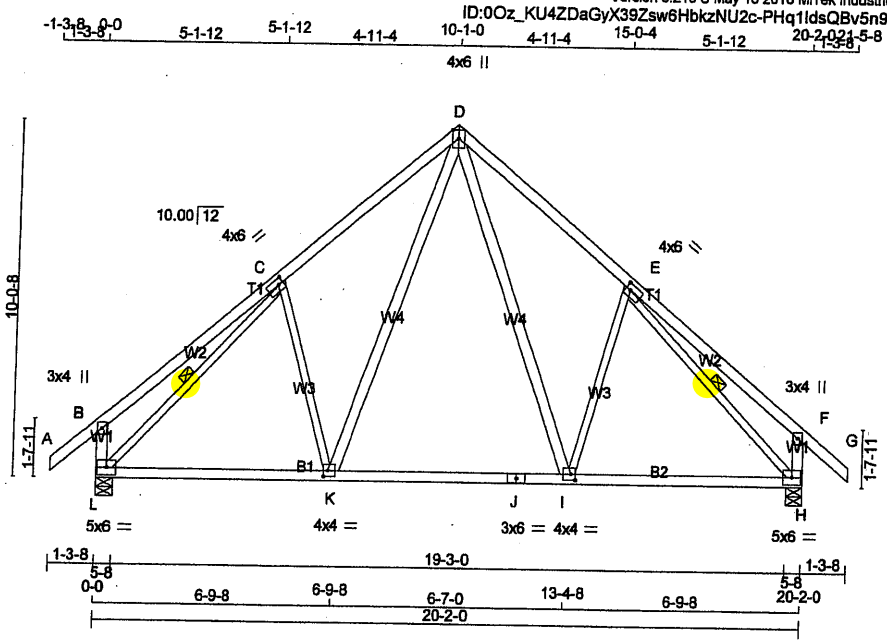

TOTAL WEIGHT = 4 X 103 = 413 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
E - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1875	0	1875	0	0	5-8	5-8
H	1875	0	1875	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1367	877 / 0	212 / 0	0 / 0	0 / 0	279 / 0	0 / 0
H	1367	877 / 0	212 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 50	-129.7	-129.7	0.54 (1)	10.00
C- D	-1488 / 0	-129.7	-129.7	0.46 (1)	4.84
D- E	-1488 / 0	-129.7	-129.7	0.46 (1)	4.84
E- F	0 / 50	-129.7	-129.7	0.54 (1)	10.00
F- G	0 / 57	-129.7	-129.7	0.18 (1)	10.00
L- B	-425 / 0	0.0	0.0	0.05 (1)	7.81
H- F	-425 / 0	0.0	0.0	0.05 (1)	7.81
L- K	0 / 1229	-38.5	-38.5	0.45 (2)	10.00
K- J	0 / 875	-38.5	-38.5	0.41 (2)	10.00
J- I	0 / 875	-38.5	-38.5	0.41 (2)	10.00
I- H	0 / 1229	-38.5	-38.5	0.45 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 707	0.11 (1)
I- E	-405 / 20	0.24 (1)
K- D	0 / 707	0.11 (1)
C- K	-405 / 20	0.24 (1)
L- C	-1822 / 0	0.67 (1)
E- H	-1822 / 0	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.45/1.00 (K-L:2) , WB=0.67/1.00 (E-H:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.49 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. GIVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805281
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G110	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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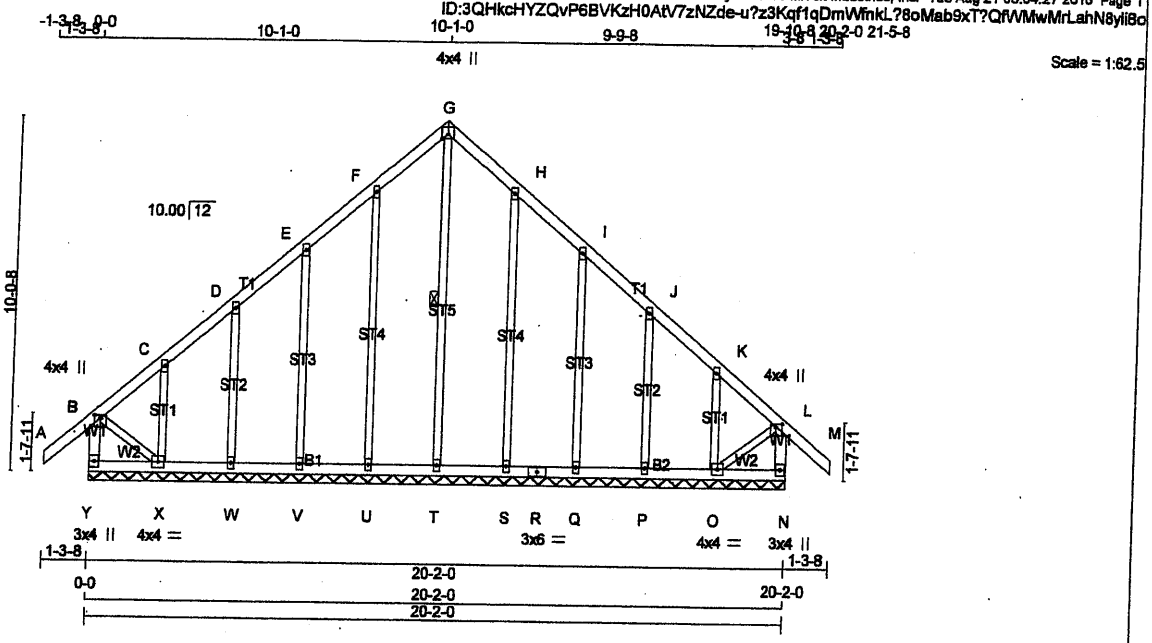

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	1.50	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, Q, S, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-353 / 0	0.0	0.0	4	0.04 (1)	7.81
A- B	0 / 57	-129.7	-129.7	4	0.18 (1)	10.00
B- C	-46 / 0	-129.7	-129.7	4	0.07 (1)	6.25
C- D	-55 / 0	-129.7	-129.7	4	0.07 (1)	6.25
D- E	-45 / 0	-129.7	-129.7	4	0.06 (1)	6.25
E- F	-36 / 0	-129.7	-129.7	4	0.07 (1)	6.25
F- G	-52 / 0	-129.7	-129.7	4	0.07 (1)	6.25
G- H	-52 / 0	-129.7	-129.7	4	0.07 (1)	6.25
H- I	-36 / 0	-129.7	-129.7	4	0.07 (1)	6.25
I- J	-45 / 0	-129.7	-129.7	4	0.06 (1)	6.25
J- K	-55 / 0	-129.7	-129.7	4	0.07 (1)	6.25
K- L	-46 / 0	-129.7	-129.7	4	0.07 (1)	6.25
L- M	0 / 57	-129.7	-129.7	4	0.18 (1)	10.00
N- L	-353 / 0	0.0	0.0	4	0.04 (1)	7.81
Y- X	0 / 0	-38.5	-38.5	4	0.03 (3)	10.00
X- W	0 / 40	-38.5	-38.5	4	0.03 (2)	10.00
W- V	0 / 35	-38.5	-38.5	4	0.03 (2)	10.00
V- U	0 / 31	-38.5	-38.5	4	0.03 (2)	10.00
U- T	0 / 27	-38.5	-38.5	4	0.03 (2)	10.00
T- S	0 / 27	-38.5	-38.5	4	0.03 (2)	10.00
S- R	0 / 31	-38.5	-38.5	4	0.03 (2)	10.00
R- Q	0 / 31	-38.5	-38.5	4	0.03 (2)	10.00
Q- P	0 / 35	-38.5	-38.5	4	0.03 (2)	10.00
P- O	0 / 40	-38.5	-38.5	4	0.03 (2)	10.00
O- N	0 / 0	-38.5	-38.5	4	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-174 / 0	0.12 (1)
U- F	-294 / 0	0.41 (1)
V- E	-251 / 0	0.19 (1)
W- D	-249 / 0	0.09 (1)
X- C	-284 / 0	0.06 (1)
S- H	-294 / 0	0.41 (1)
Q- I	-251 / 0	0.19 (1)
P- J	-249 / 0	0.09 (1)
O- K	-284 / 0	0.06 (1)
B- X	0 / 57	0.01 (1)
O- L	0 / 57	0.01 (1)

TOTAL WEIGHT = 106 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.03/1.00 (W-X:2) , WB=0.41/1.00 (H-S:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (G) (INPUT = 0.90)
JSI METAL= 0.16 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/24/18
H.J.G. AI
PROVINCE OF ONTARIO

DWG NO. TAM T1805377
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T110A	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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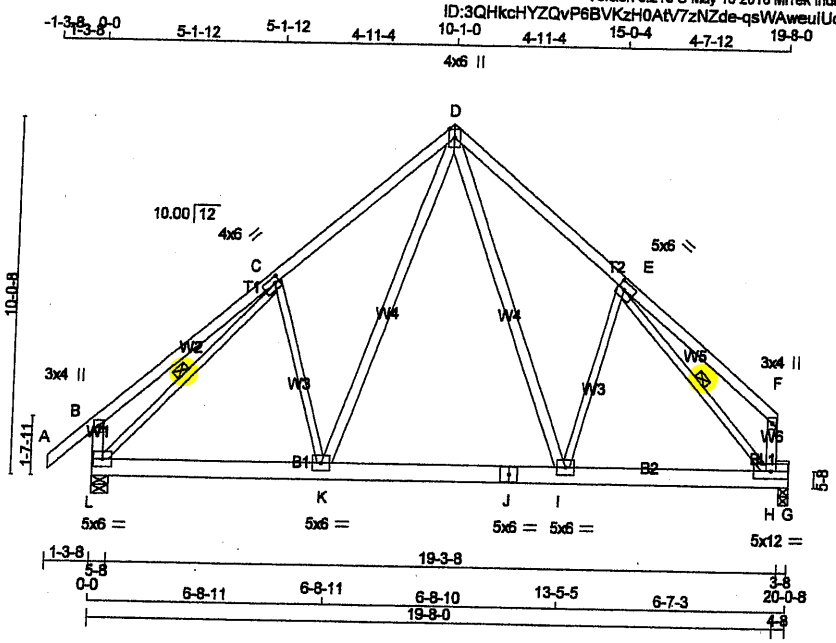

TOTAL WEIGHT = 12 X 113 = 1356 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0	2.50	2.25
F	TMV+p	MT20	3.0	4.0		
H	BMVWK-t	MT20	5.0	12.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1864	0	1864	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	1 (1)	0.18	10.00
B- C	0 / 50	-129.7	-129.7	1 (1)	0.54	10.00
C- D	-1489 / 0	-129.7	-129.7	1 (1)	0.65	4.49
D- E	-1510 / 0	-129.7	-129.7	1 (1)	0.60	4.56
E- F	0 / 74	-129.7	-129.7	1 (1)	0.50	10.00
L- B	-425 / 0	0.0	0.0	1 (1)	0.04	7.81
H- F	-198 / 0	0.0	0.0	1 (1)	0.02	7.81
L- K	0 / 1230	-38.5	-38.5	1 (1)	0.26	10.00
K- J	0 / 875	-38.5	-38.5	1 (1)	0.32	10.00
J- I	0 / 875	-38.5	-38.5	1 (1)	0.32	10.00
I- H	0 / 1194	-38.5	-38.5	1 (1)	0.71	10.00
H- G	0 / 0	-38.5	-38.5	1 (1)	0.35	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-424 / 14	0.23 (1)
K- D	0 / 686	0.11 (1)
D- I	0 / 745	0.12 (1)
I- E	-234 / 74	0.13 (1)
L- C	-1808 / 0	0.64 (1)
E- H	-1928 / 0	0.63 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.65/1.00 (C-D:1) , BC=0.71/1.00 (H-I:1) , WB=0.64/1.00 (C-L:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.50 (E) (INPUT = 1.00)

DWG NO. TAM T1105282
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	T111	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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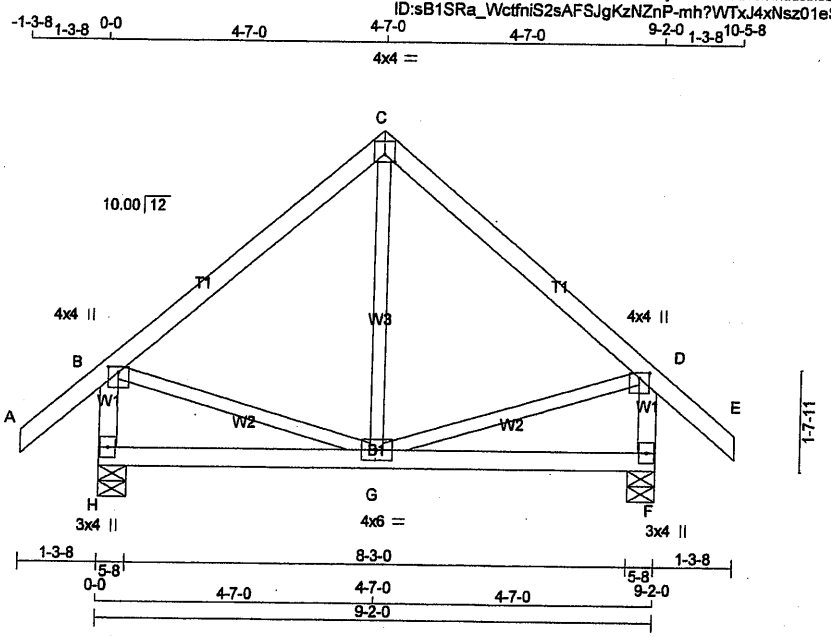

TOTAL WEIGHT = 2 X 42 = 84 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	950	0	950	0	0	5-8	5-8
F	950	0	950	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	688	456 / 0	96 / 0	0 / 0	0 / 0	136 / 0	0 / 0
F	688	456 / 0	96 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-473 / 0	-129.7	-129.7	0.35 (1)	6.25
C- D	-473 / 0	-129.7	-129.7	0.35 (1)	6.25
D- E	0 / 57	-129.7	-129.7	0.18 (1)	10.00
H- B	-883 / 0	0.0	0.0	0.09 (1)	7.81
F- D	-883 / 0	0.0	0.0	0.09 (1)	7.81
H- G	0 / 0	-38.5	-38.5	0.18 (3)	10.00
G- F	0 / 0	-38.5	-38.5	0.18 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 191	0.05 (3)
B- G	0 / 377	0.08 (1)
G- D	0 / 377	0.08 (1)

LICENSED PROFESSIONAL ENGINEER
09/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.18/1.00 (F-G:3) , WB=0.08/1.00 (B-G:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

DWG NO. TAM T1605405
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	G111	1	1	TRUSS DESC.	

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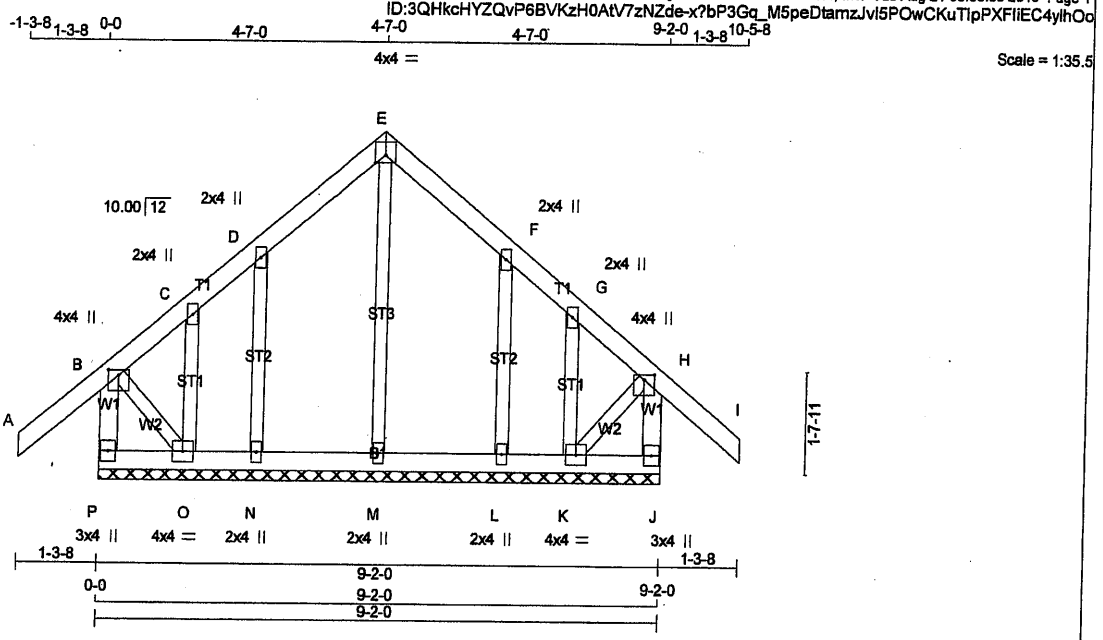

TOTAL WEIGHT = 47 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-394 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-83 / 0	-129.7	-129.7	0.17 (1)	6.25
C- D	0 / 14	-129.7	-129.7	0.07 (1)	10.00
D- E	-26 / 0	-129.7	-129.7	0.07 (1)	6.25
E- F	-26 / 0	-129.7	-129.7	0.07 (1)	6.25
F- G	0 / 14	-129.7	-129.7	0.07 (1)	10.00
G- H	-83 / 0	-129.7	-129.7	0.17 (1)	6.25
H- I	0 / 57	-129.7	-129.7	0.18 (1)	10.00
J- H	-394 / 0	0.0	0.0	0.04 (1)	7.81
P- O	0 / 0	-38.5	-38.5	0.01 (3)	10.00
O- N	0 / 16	-38.5	-38.5	0.02 (2)	10.00
N- M	0 / 8	-38.5	-38.5	0.03 (3)	10.00
M- L	0 / 8	-38.5	-38.5	0.03 (3)	10.00
L- K	0 / 16	-38.5	-38.5	0.02 (2)	10.00
K- J	0 / 0	-38.5	-38.5	0.01 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-210 / 0	0.09 (1)
N- D	-278 / 0	0.06 (1)
O- C	-11 / 0	0.00 (1)
L- F	-278 / 0	0.06 (1)
K- G	-11 / 0	0.00 (1)
B- O	0 / 22	0.00 (1)
K- H	0 / 22	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.03/1.00 (M-N:3) , WB=0.09/1.00 (E-M:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (E) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805394
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T115	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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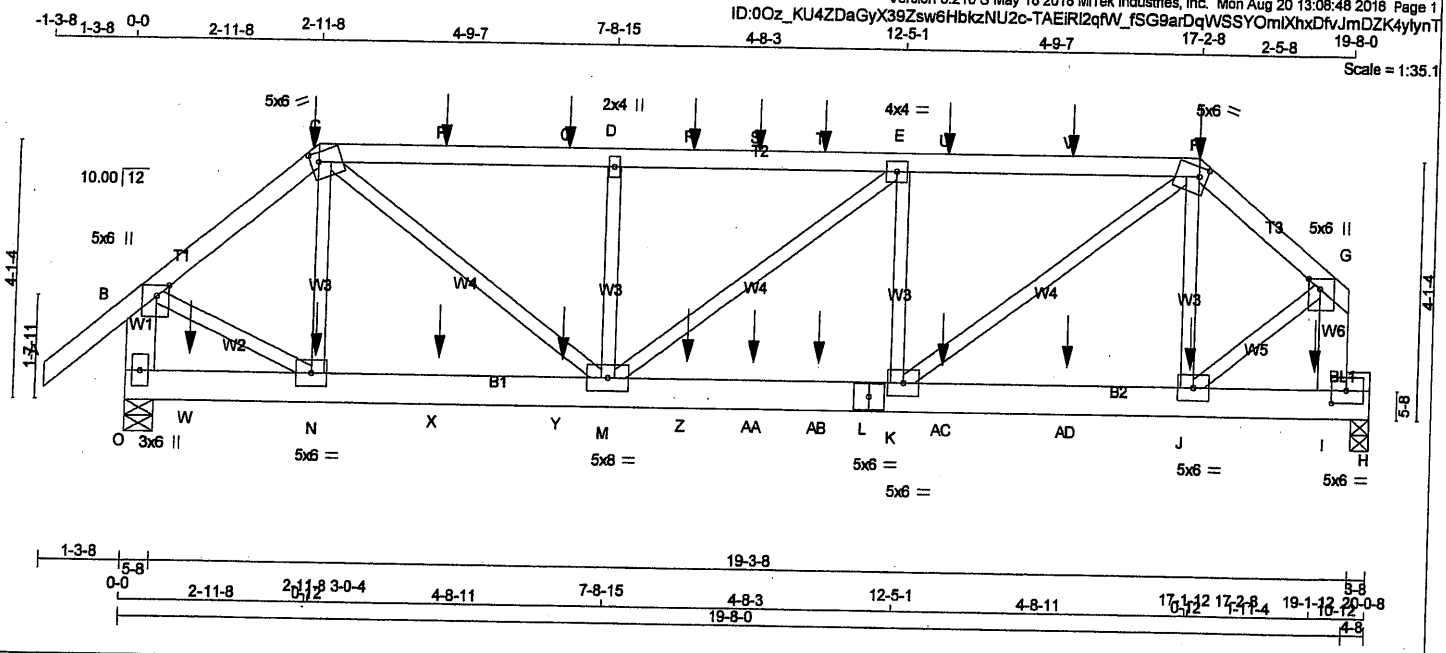

TOTAL WEIGHT = 2 X 97 = 194 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
O- B	2	12	TOP
I- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	SIDE(183.1)
L- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
N- C	1	6	SIDE(12.6)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	1.75	1.50
G	TMVW+p	MT20	5.0	6.0	2.00	2.25
I	BMVK-t	MT20	5.0	6.0	2.50	2.75
J, K, N						
J	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	3138	0	3138	0	0	5-8	5-8
H	2971	0	2971	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2287	1489 / 0	374 / 0	0 / 0	0 / 0	424 / 0	0 / 0
H	2176	1382 / 0	378 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.10 (1)	10.00
B- C	-2861 / 0	-129.7	-129.7	0.14 (1)	5.29
C- P	-3945 / 0	-129.7	-129.7	0.46 (1)	4.25
P- Q	-3945 / 0	-129.7	-129.7	0.46 (1)	4.25
Q- D	-3945 / 0	-129.7	-129.7	0.46 (1)	4.25
D- R	-3945 / 0	-129.7	-129.7	0.47 (1)	4.24
R- S	-3945 / 0	-129.7	-129.7	0.47 (1)	4.24
S- T	-3945 / 0	-129.7	-129.7	0.47 (1)	4.24
T- E	-3945 / 0	-129.7	-129.7	0.47 (1)	4.24
E- U	-3908 / 0	-129.7	-129.7	0.47 (1)	4.25
U- V	-3908 / 0	-129.7	-129.7	0.47 (1)	4.25
V- F	-3908 / 0	-129.7	-129.7	0.47 (1)	4.25
F- G	-2826 / 0	-129.7	-129.7	0.10 (1)	5.36
O- B	-3086 / 0	0.0	0.0	0.11 (1)	7.81
I- G	-3730 / 0	0.0	0.0	0.15 (1)	7.29
O- W	0 / 0	-38.5	-38.5	0.04 (2)	10.00
W- N	0 / 0	-38.5	-38.5	0.04 (2)	10.00
N- X	0 / 2174	-38.5	-38.5	0.18 (1)	10.00
X- Y	0 / 2174	-38.5	-38.5	0.18 (1)	10.00
Y- M	0 / 2174	-38.5	-38.5	0.18 (1)	10.00
M- Z	0 / 3908	-38.5	-38.5	0.34 (1)	10.00
Z-AA	0 / 3908	-38.5	-38.5	0.34 (1)	10.00
AA-AB	0 / 3908	-38.5	-38.5	0.34 (1)	10.00
AB- L	0 / 3908	-38.5	-38.5	0.34 (1)	10.00
L- K	0 / 3908	-38.5	-38.5	0.34 (1)	10.00
K-AC	0 / 2147	-38.5	-38.5	0.28 (1)	10.00
AC-AD	0 / 2147	-38.5	-38.5	0.28 (1)	10.00
AD- J	0 / 2147	-38.5	-38.5	0.28 (1)	10.00
J- I	-517 / 0	-38.5	-38.5	0.65 (1)	6.25
I- H	0 / 0	-168.2	-168.2	0.40 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-579 / 28	0.07 (1)
C- M	0 / 2241	0.28 (1)
M- D	-1128 / 0	0.14 (1)
M- E	0 / 47	0.01 (1)
K- E	-1166 / 0	0.15 (1)
K- F	0 / 2227	0.28 (1)
J- F	-501 / 38	0.06 (1)
B- N	0 / 2558	0.29 (1)
I- G	0 / 3180	0.39 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-28	-28	—	FRONT	VERT	DEAD
C	2-11-8	-148	-148	—	FRONT	VERT	TOTAL
C	2-11-8	-176	-176	—	FRONT	VERT	SNOW
F	17-2-8	-28	-28	—	FRONT	VERT	DEAD
F	17-2-8	-112	-112	—	FRONT	VERT	TOTAL
F	17-2-8	-176	-176	—	FRONT	VERT	SNOW
I	19-1-12	-43	-43	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G
PROVINCE OF ONTARIO

DWG NO. TAM 11805285
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.47/1.00 (E-F:1) , BC=0.65/1.00 (I-J:1) , WB=0.39/1.00 (G-J:1) , SSI=0.60/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.38 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T115	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 515.8 lbs FACTORED DOWN AT 2-11-8, 462.1 lbs FACTORED DOWN AT 17-2-8, 156.1 lbs FACTORED DOWN AT 5-0-4, 156.1 lbs FACTORED DOWN AT 7-0-4, 156.1 lbs FACTORED DOWN AT 9-0-4, 156.1 lbs FACTORED DOWN AT 10-1-0, 156.1 lbs FACTORED DOWN AT 11-1-12, AND 156.1 lbs FACTORED DOWN AT 13-1-12, AND 156.1 lbs FACTORED DOWN AT 15-1-12 ON TOP CHORD, AND 54.9 lbs FACTORED DOWN AT 1-0-4, 61.9 lbs FACTORED DOWN AT 3-0-4, 54.9 lbs FACTORED DOWN AT 5-0-4, 54.9 lbs FACTORED DOWN AT 7-0-4, 54.9 lbs FACTORED DOWN AT 9-0-4, 54.9 lbs FACTORED DOWN AT 10-1-0, 54.9 lbs FACTORED DOWN AT 11-1-12, 54.9 lbs FACTORED DOWN AT 13-1-12, 54.9 lbs FACTORED DOWN AT 15-1-12, AND 61.9 lbs FACTORED DOWN AT 17-1-12, AND 61.1 lbs FACTORED DOWN AT 19-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	17-1-12	-44	-44	—	FRONT	VERT	TOTAL
N	3-0-4	-44	-44	—	FRONT	VERT	TOTAL
P	5-0-4	-106	-106	—	FRONT	VERT	TOTAL
Q	7-0-4	-106	-106	—	FRONT	VERT	TOTAL
R	9-0-4	-106	-106	—	FRONT	VERT	TOTAL
S	10-1-0	-106	-106	—	FRONT	VERT	TOTAL
T	11-1-12	-106	-106	—	FRONT	VERT	TOTAL
U	13-1-12	-106	-106	—	FRONT	VERT	TOTAL
V	15-1-12	-106	-106	—	FRONT	VERT	TOTAL
W	1-0-4	-38	-38	—	FRONT	VERT	TOTAL
X	5-0-4	-38	-38	—	FRONT	VERT	TOTAL
Y	7-0-4	-38	-38	—	FRONT	VERT	TOTAL
Z	9-0-4	-38	-38	—	FRONT	VERT	TOTAL
AA	10-1-0	-38	-38	—	FRONT	VERT	TOTAL
AB	11-1-12	-38	-38	—	FRONT	VERT	TOTAL
AC	13-1-12	-38	-38	—	FRONT	VERT	TOTAL
AD	15-1-12	-38	-38	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. [illegible]
PROVINCE OF ONTARIO

DWG NO. TAM 1805285
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T116	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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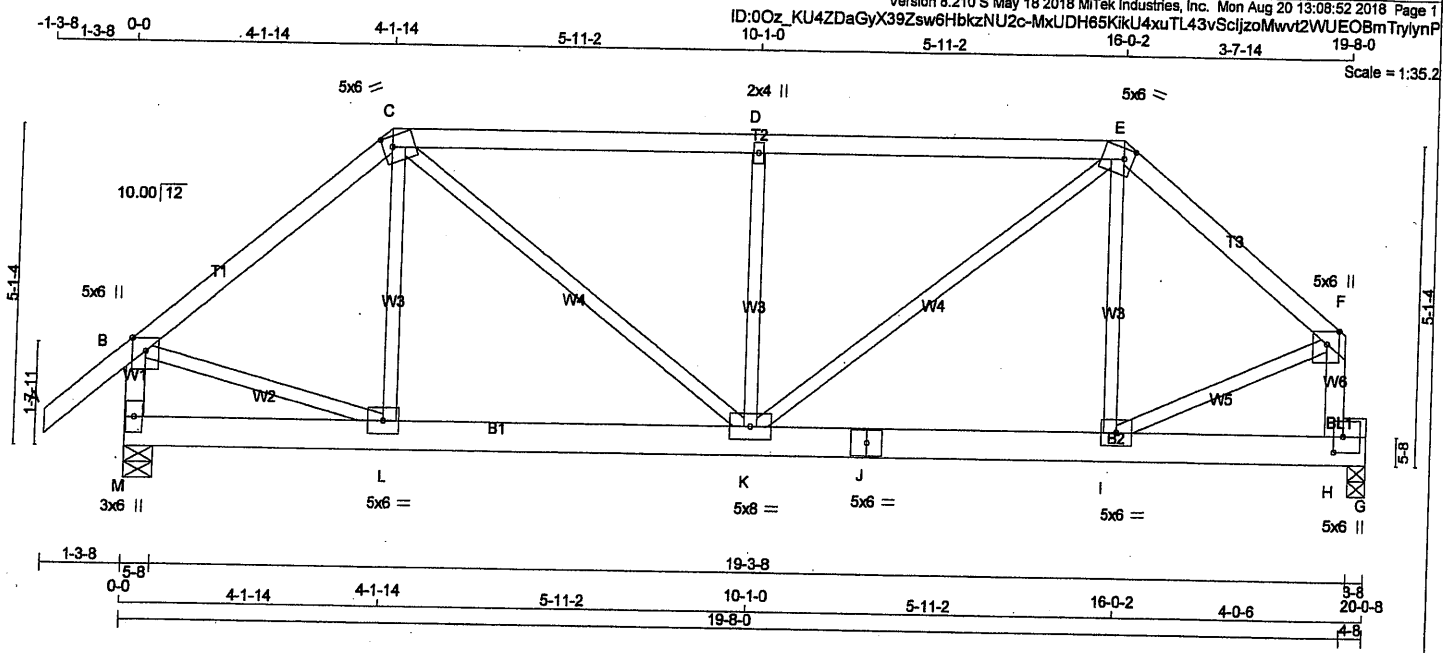

TOTAL WEIGHT = 95 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	1.75
F	TMVW+p	MT20	5.0	6.0	2.50	2.25
H	BMVK+p	MT20	5.0	6.0	3.00	1.75
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1864	0	1864	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1576 / 0	-129.7	-129.7	0.45 (1)	4.71
C- D	-1900 / 0	-129.7	-129.7	0.91 (1)	3.37
D- E	-1900 / 0	-129.7	-129.7	0.91 (1)	3.37
E- F	-1581 / 0	-129.7	-129.7	0.35 (1)	4.85
M- B	-1808 / 0	0.0	0.0	0.19 (1)	6.22
H- F	-1860 / 0	0.0	0.0	0.21 (1)	6.15
M- L	0 / 0	-38.5	-38.5	0.09 (2)	10.00
L- K	0 / 1205	-38.5	-38.5	0.22 (1)	10.00
K- J	0 / 1210	-38.5	-38.5	0.32 (1)	10.00
J- I	0 / 1210	-38.5	-38.5	0.32 (1)	10.00
I- H	-165 / 0	-38.5	-38.5	0.57 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS

MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		
L- C	-108 / 141	0.04 (1)
C- K	0 / 888	0.20 (1)
K- D	-939 / 0	0.34 (1)
K- E	0 / 882	0.20 (1)
I- E	-68 / 146	0.03 (3)
B- L	0 / 1256	0.28 (1)
I- F	0 / 1501	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.91/1.00 (D-E:1) , BC=0.57/1.00 (H-I:1) , WB=0.34/1.00 (F-I:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.62 (B) (INPUT = 1.00)

DWG NO. TAM 17805286
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T117	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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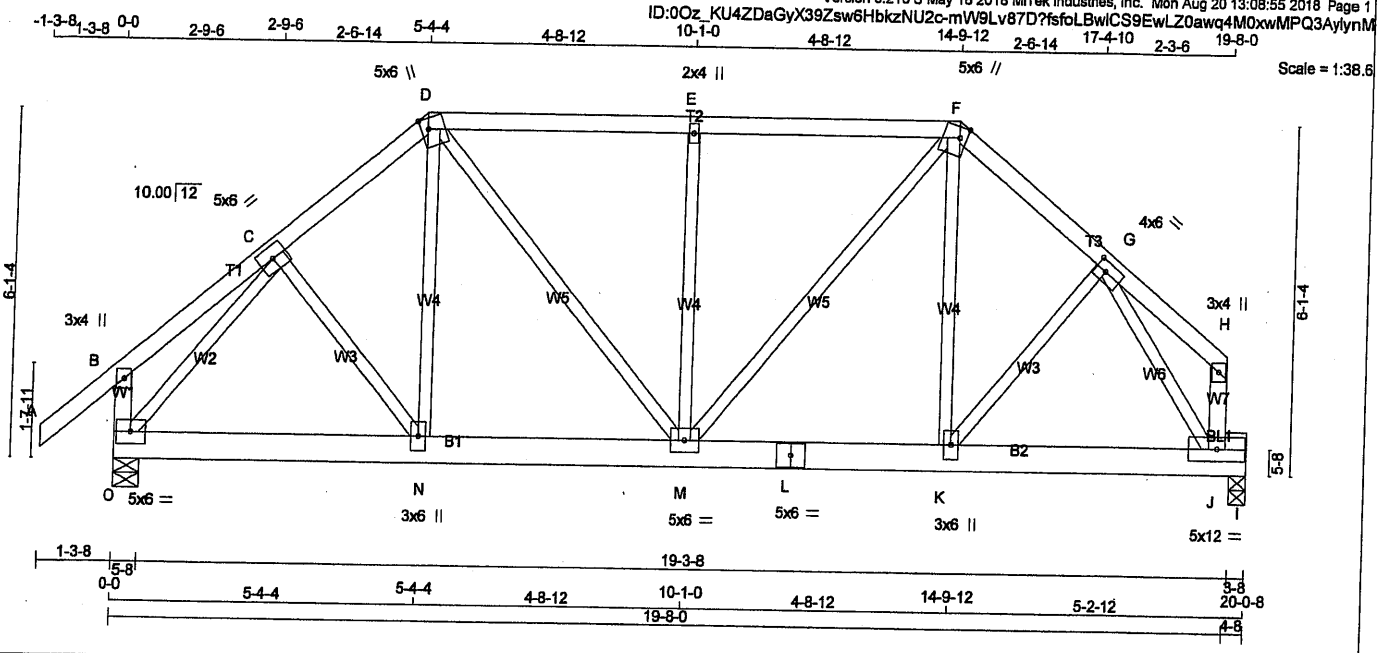

TOTAL WEIGHT = 105 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVWK-t	MT20	5.0	12.0		
K	BMWW+t	MT20	3.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1864	0	1864	0	0	5-8	5-8
I	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
I	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	1	0.18 (1)	10.00
B- C	0 / 22	-129.7	-129.7	1	0.13 (1)	10.00
C- D	-1558 / 0	-129.7	-129.7	1	0.16 (1)	5.13
D- E	-1526 / 0	-129.7	-129.7	1	0.53 (1)	4.60
E- F	-1526 / 0	-129.7	-129.7	1	0.53 (1)	4.60
F- G	-1575 / 0	-129.7	-129.7	1	0.14 (1)	5.12
G- H	0 / 33	-129.7	-129.7	1	0.11 (1)	10.00
O- B	-317 / 0	0.0	0.0	1	0.03 (1)	7.81
J- H	-101 / 0	0.0	0.0	1	0.01 (1)	7.81
O- N	0 / 1129	-38.5	-38.5	1	0.20 (1)	10.00
N- M	0 / 1178	-38.5	-38.5	1	0.21 (1)	10.00
M- L	0 / 1195	-38.5	-38.5	1	0.34 (1)	10.00
L- K	0 / 1195	-38.5	-38.5	1	0.34 (1)	10.00
K- J	0 / 972	-38.5	-38.5	1	0.68 (1)	10.00
J- I	0 / 0	-38.5	-38.5	1	0.35 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 104	0.02 (3)
N- D	0 / 225	0.05 (2)
D- M	0 / 545	0.12 (1)
M- E	-744 / 0	0.41 (1)
M- F	0 / 518	0.12 (1)
K- F	0 / 248	0.06 (2)
K- G	0 / 351	0.08 (1)
O- C	-1846 / 0	0.55 (1)
G- J	-1929 / 0	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.53/1.00 (E-F:1) , BC=0.68/1.00 (J-K:1) , WB=0.55/1.00 (C-O:1) , SSI=0.60/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)

DWG NO. T1805287
STRUCTURAL COMPONENTS IV

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T118	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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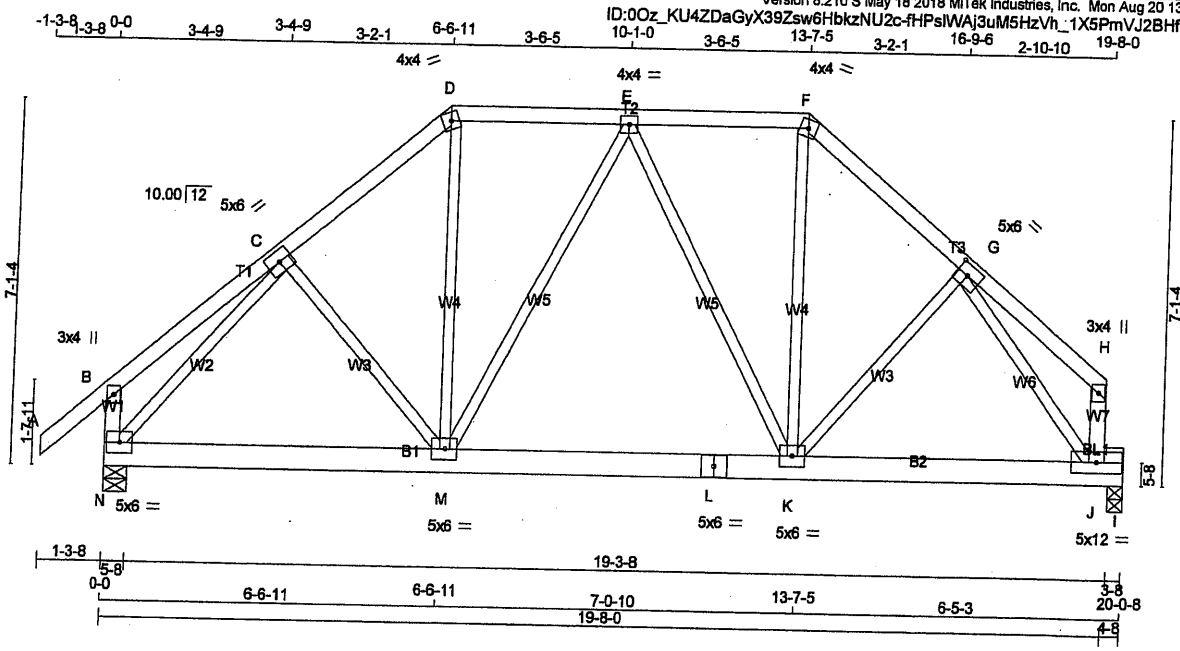

TOTAL WEIGHT = 106 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVWK-t	MT20	5.0	12.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1864	0	1864	0	0	5-8	5-8
I	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
I	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 30	-129.7	-129.7	0.21 (1)	10.00
C- D	-1491 / 0	-129.7	-129.7	0.24 (1)	5.10
D- E	-1128 / 0	-129.7	-129.7	0.27 (1)	5.62
E- F	-1152 / 0	-129.7	-129.7	0.27 (1)	5.58
F- G	-1518 / 0	-129.7	-129.7	0.21 (1)	5.11
G- H	0 / 44	-129.7	-129.7	0.18 (1)	10.00
N- B	-344 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-123 / 0	0.0	0.0	0.01 (1)	7.81
N- M	0 / 1163	-38.5	-38.5	0.25 (1)	10.00
M- L	0 / 1280	-38.5	-38.5	0.37 (1)	10.00
L- K	0 / 1280	-38.5	-38.5	0.37 (1)	10.00
K- J	0 / 1033	-38.5	-38.5	0.69 (1)	10.00
J- I	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-67 / 71	0.03 (1)
M- D	0 / 588	0.13 (1)
M- E	-333 / 0	0.37 (1)
E- K	-280 / 0	0.31 (1)
K- F	0 / 602	0.14 (1)
K- G	0 / 180	0.04 (2)
N- C	-1826 / 0	0.74 (1)
G- J	-1890 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.27/1.00 (E-F:1) , BC=0.69/1.00 (J-K:1) , WB=0.74/1.00 (C-N:1) , SSI=0.60/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)

DWG NO. TAM #1805288
STRUCTURAL COMPONENTS INC.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T119	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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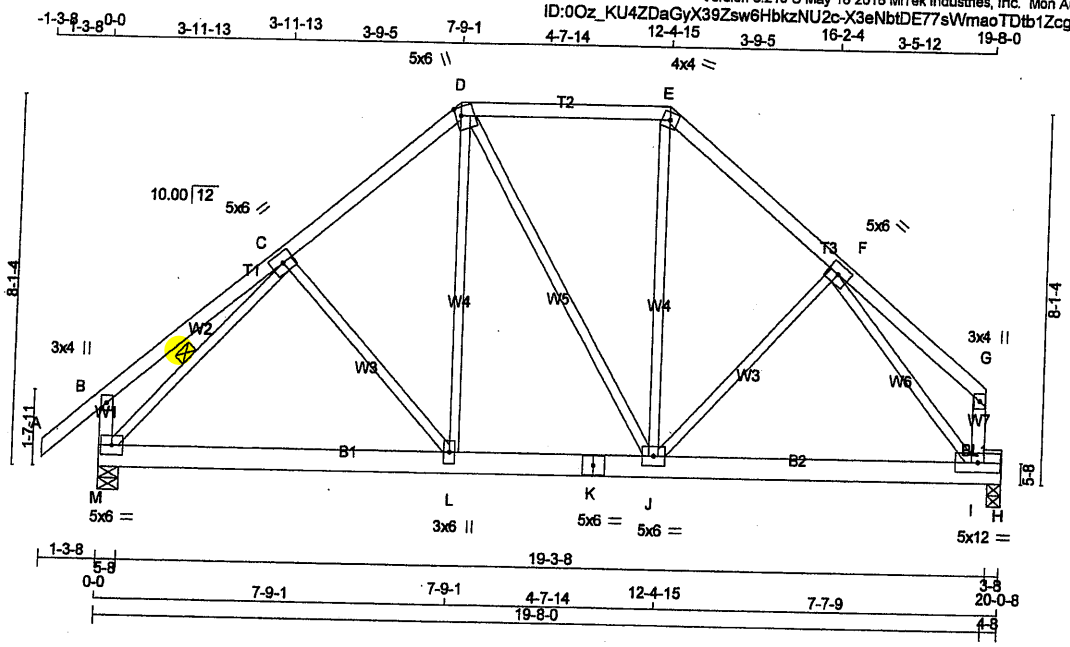

TOTAL WEIGHT = 106 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVWK-t	MT20	5.0	12.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW+t	MT20	3.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG
M	1864	0	1864	0	0	5-8	5-8
H	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
H	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 37	-129.7	-129.7	0.31 (1)	10.00
C- D	-1407 / 0	-129.7	-129.7	0.36 (1)	5.07
D- E	-1086 / 0	-129.7	-129.7	0.51 (1)	5.27
E- F	-1443 / 0	-129.7	-129.7	0.32 (1)	5.07
F- G	0 / 55	-129.7	-129.7	0.28 (1)	10.00
M- B	-371 / 0	0.0	0.0	0.04 (1)	7.81
I- G	-147 / 0	0.0	0.0	0.02 (1)	7.81
M- L	0 / 1179	-38.5	-38.5	0.31 (2)	10.00
L- K	0 / 1054	-38.5	-38.5	0.39 (1)	10.00
K- J	0 / 1054	-38.5	-38.5	0.39 (1)	10.00
J- I	0 / 1074	-38.5	-38.5	0.69 (1)	10.00
I- H	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-193 / 41	0.12 (1)
L- D	0 / 352	0.08 (2)
D- J	0 / 62	0.01 (1)
J- E	0 / 408	0.09 (2)
J- F	0 / 101	0.02 (3)
M- C	-1797 / 0	0.44 (1)
F- I	-1856 / 0	0.91 (1)

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.51/1.00 (D-E:1) , BC=0.69/1.00 (I-J:1) , WB=0.91/1.00 (F-I:1) , SSI=0.60/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.45 (F) (INPUT = 1.00)

DWG NO. TAM 11805289
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T140	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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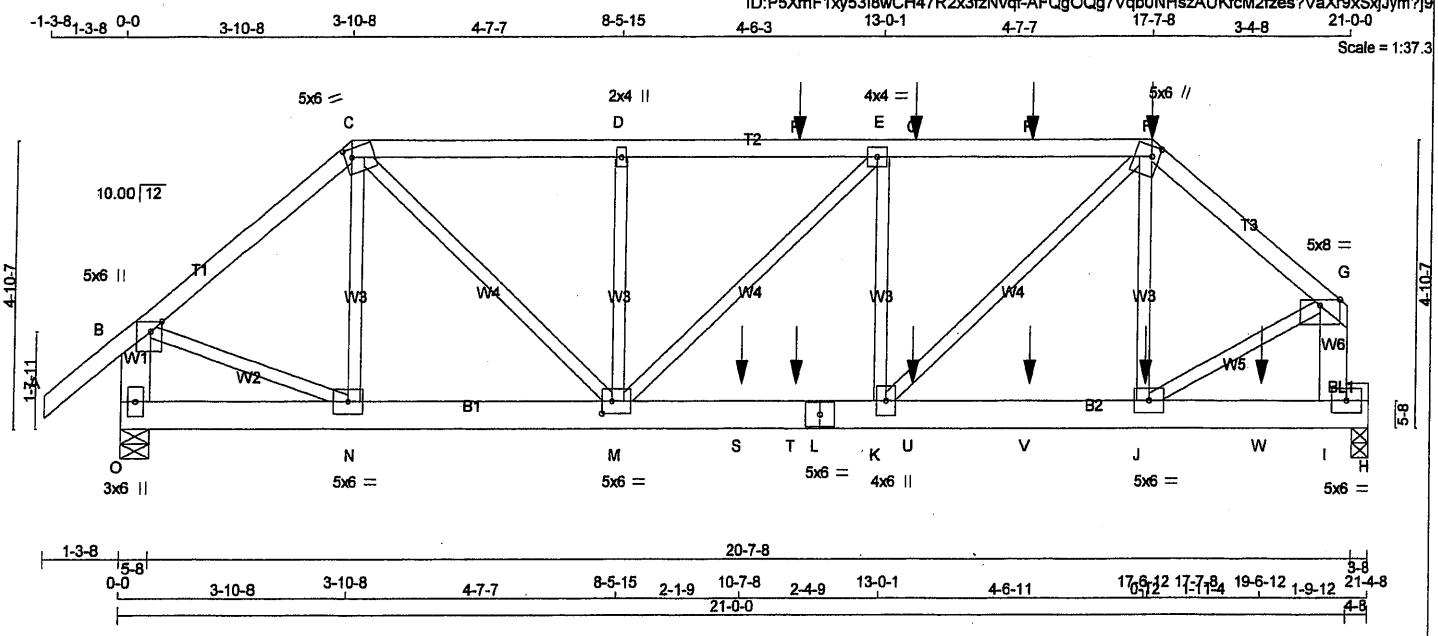


TOTAL WEIGHT = 2 X 106 = 213 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
O- B	2	12	TOP
I- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	SIDE(0.0)
L- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-p	MT20	5.0	8.0	Edge	
I	BMVK-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0	2.50	2.00
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2630	0	2630	0	0	5-8	5-8
H	2804	0	2804	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1911	1254 / 0	288 / 0	0 / 0	0 / 0	368 / 0	0 / 0
H	2045	1325 / 0	332 / 0	0 / 0	0 / 0	388 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.10 (1)	10.00
B- C	-2419 / 0	-129.7	-129.7	0.22 (1)	5.55
C- D	-3402 / 0	-129.7	-129.7	0.25 (1)	4.82
D- P	-3402 / 0	-129.7	-129.7	0.34 (1)	4.68
P- E	-3402 / 0	-129.7	-129.7	0.34 (1)	4.68
E- Q	-3549 / 0	-129.7	-129.7	0.35 (1)	4.60
Q- R	-3549 / 0	-129.7	-129.7	0.35 (1)	4.60
R- F	-3549 / 0	-129.7	-129.7	0.35 (1)	4.60
F- G	-2607 / 0	-129.7	-129.7	0.18 (1)	5.29
O- B	-2555 / 0	0.0	0.0	0.09 (1)	7.81
I- G	-3282 / 0	0.0	0.0	0.13 (1)	7.66
O- N	0 / 0	-38.5	-38.5	0.03 (2)	10.00
N- M	0 / 1842	-38.5	-38.5	0.23 (1)	10.00
M- S	0 / 3550	-38.5	-38.5	0.54 (1)	10.00
S- T	0 / 3550	-38.5	-38.5	0.54 (1)	10.00
T- L	0 / 3550	-38.5	-38.5	0.54 (1)	10.00
L- K	0 / 3550	-38.5	-38.5	0.54 (1)	10.00
K- U	0 / 2140	-38.5	-38.5	0.26 (1)	10.00
U- V	0 / 2140	-38.5	-38.5	0.26 (1)	10.00
V- J	0 / 2140	-38.5	-38.5	0.26 (1)	10.00
J- W	-455 / 0	-38.5	-38.5	0.61 (1)	6.25
W- I	-455 / 0	-38.5	-38.5	0.61 (1)	6.25
I- H	0 / 0	-168.2	-168.2	0.38 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-468 / 2	0.08 (1)
C- M	0 / 2171	0.27 (1)
M- D	-641 / 0	0.11 (1)
M- E	-209 / 0	0.07 (1)
K- E	-781 / 0	0.13 (1)
K- F	0 / 1963	0.24 (1)
J- F	-389 / 78	0.07 (1)
B- N	0 / 1939	0.24 (1)
J- G	0 / 2878	0.36 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	17-7-8	-202	-202	—	FRONT	VERT	TOTAL
J	17-6-12	-26	-26	—	FRONT	VERT	TOTAL
P	11-6-12	-83	-83	—	FRONT	VERT	TOTAL
Q	13-6-12	-83	-83	—	FRONT	VERT	TOTAL
R	15-6-12	-83	-83	—	FRONT	VERT	TOTAL
S	10-7-8	-623	-623	—	FRONT	VERT	TOTAL
T	11-6-12	-21	-21	—	FRONT	VERT	TOTAL
U	13-6-12	-21	-21	—	FRONT	VERT	TOTAL
V	15-6-12	-21	-21	—	FRONT	VERT	TOTAL
W	19-6-12	-11	-16	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.61/1.00 (I-J:1) , WB=0.36/1.00 (G-J:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

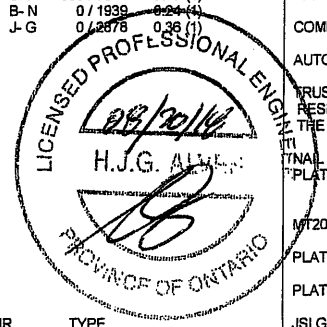
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.43 (L) (INPUT = 1.00)



DWG NO. TAM T1805193
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T140	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 121.6 lbs FACTORED DOWN AT 11-6-12, 121.6 lbs FACTORED DOWN AT 13-6-12, AND 121.6 lbs FACTORED DOWN AT 15-6-12, AND 292.9 lbs FACTORED DOWN AT 17-7-8 ON TOP CHORD, AND 872.4 lbs FACTORED DOWN AT 10-7-8, 30.9 lbs FACTORED DOWN AT 11-6-12, 30.9 lbs FACTORED DOWN AT 13-6-12, 30.9 lbs FACTORED DOWN AT 15-6-12, AND 38.0 lbs FACTORED DOWN AT 17-6-12, AND 23.7 lbs FACTORED DOWN AT 19-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
09/10/18
H.J.G. AMES
PROVINCE OF ONTARIO

DWG NO. TAM 1805793
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T141	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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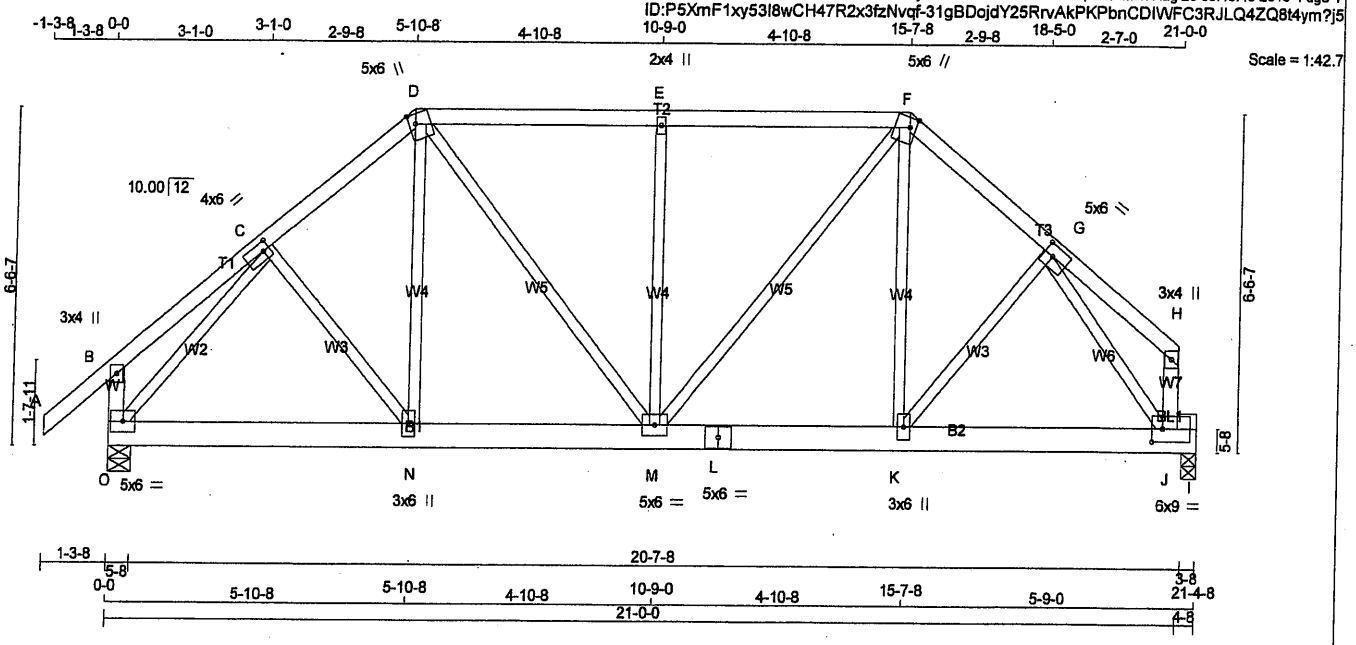


TOTAL WEIGHT = 112 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVWK-t	MT20	6.0	9.0	3.00	2.50
K	BMWW+t	MT20	3.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1749	0	1749	0	0	3-8	3-8
O	1976	0	1976	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0
O	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 25	-129.7	-129.7	0.16 (1)	10.00
C- D	-1677 / 0	-129.7	-129.7	0.20 (1)	4.93
D- E	-1610 / 0	-129.7	-129.7	0.57 (1)	4.43
E- F	-1610 / 0	-129.7	-129.7	0.57 (1)	4.43
F- G	-1697 / 0	-129.7	-129.7	0.17 (1)	4.94
G- H	0 / 38	-129.7	-129.7	0.14 (1)	10.00
O- B	-332 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-112 / 0	0.0	0.0	0.01 (1)	7.81
O- N	0 / 1242	-38.5	-38.5	0.23 (1)	10.00
N- M	0 / 1268	-38.5	-38.5	0.23 (2)	10.00
M- L	0 / 1287	-38.5	-38.5	0.38 (1)	10.00
L- K	0 / 1287	-38.5	-38.5	0.38 (1)	10.00
K- J	0 / 1084	-38.5	-38.5	0.73 (1)	10.00
J- I	0 / 0	-38.5	-38.5	0.38 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- D	0 / 270	0.06 (2)
D- M	0 / 550	0.12 (1)
M- E	-768 / 0	0.50 (1)
M- F	0 / 520	0.12 (1)
K- F	0 / 294	0.07 (2)
C- N	0 / 98	0.02 (3)
O- C	-1986 / 0	0.69 (1)
K- G	0 / 312	0.07 (1)
G- J	-2062 / 0	0.63 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.57/1.00 (E-F:1) , BC=0.73/1.00 (J-K:1) , WB=0.69/1.00 (C-O:1) , SSI=0.64/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

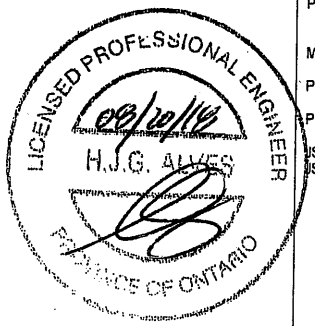
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (C) (INPUT = 0.90)
SI METAL= 0.49 (G) (INPUT = 1.00)



DWG NO. TAM T1805194
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T142	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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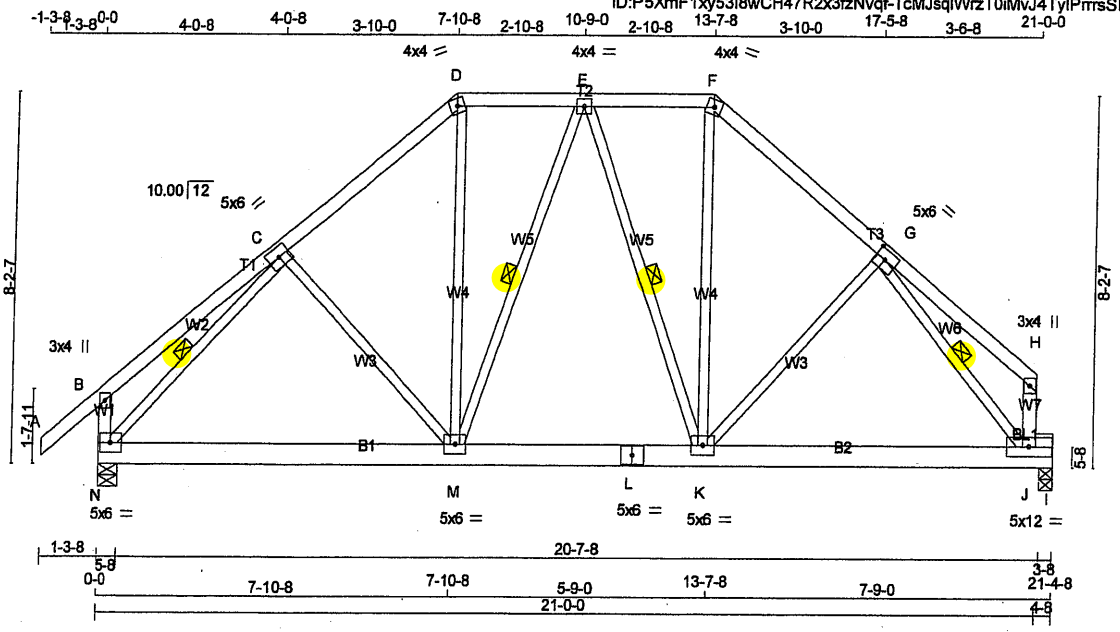


TOTAL WEIGHT = 116 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVWK-t	MT20	5.0	12.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1976	0	1976	0	0	5-8	5-8
I	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
I	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, E-K, C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 38	-129.7	-129.7	0.32 (1)	10.00
C- D	-1547 / 0	-129.7	-129.7	0.37 (1)	4.86
D- E	-1164 / 0	-129.7	-129.7	0.16 (1)	5.71
E- F	-1192 / 0	-129.7	-129.7	0.16 (1)	5.66
F- G	-1579 / 0	-129.7	-129.7	0.34 (1)	4.88
G- H	0 / 56	-129.7	-129.7	0.29 (1)	10.00
N- B	-374 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-151 / 0	0.0	0.0	0.02 (1)	7.81
N- M	0 / 1278	-38.5	-38.5	0.32 (2)	10.00
M- L	0 / 1255	-38.5	-38.5	0.42 (1)	10.00
L- K	0 / 1255	-38.5	-38.5	0.42 (1)	10.00
K- J	0 / 1164	-38.5	-38.5	0.74 (1)	10.00
J- I	0 / 0	-38.5	-38.5	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-180 / 48	0.11 (1)
M- D	0 / 613	0.14 (1)
M- E	-270 / 0	0.12 (1)
E- K	-185 / 0	0.09 (1)
K- F	0 / 630	0.14 (1)
K- G	0 / 112	0.03 (3)
N- C	-1943 / 0	0.48 (1)
G- J	-2000 / 0	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.74/1.00 (J-K:1) , WB=0.48/1.00 (C-N:1) , SSI=0.64/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

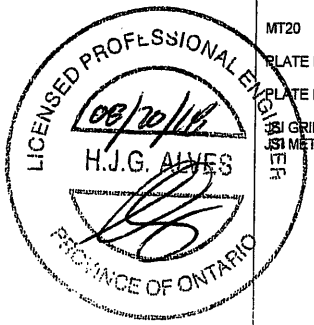
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.86 (G) (INPUT = 0.90)
SI METAL= 0.48 (G) (INPUT = 1.00)



DWG NO. TAM 11805795
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T143	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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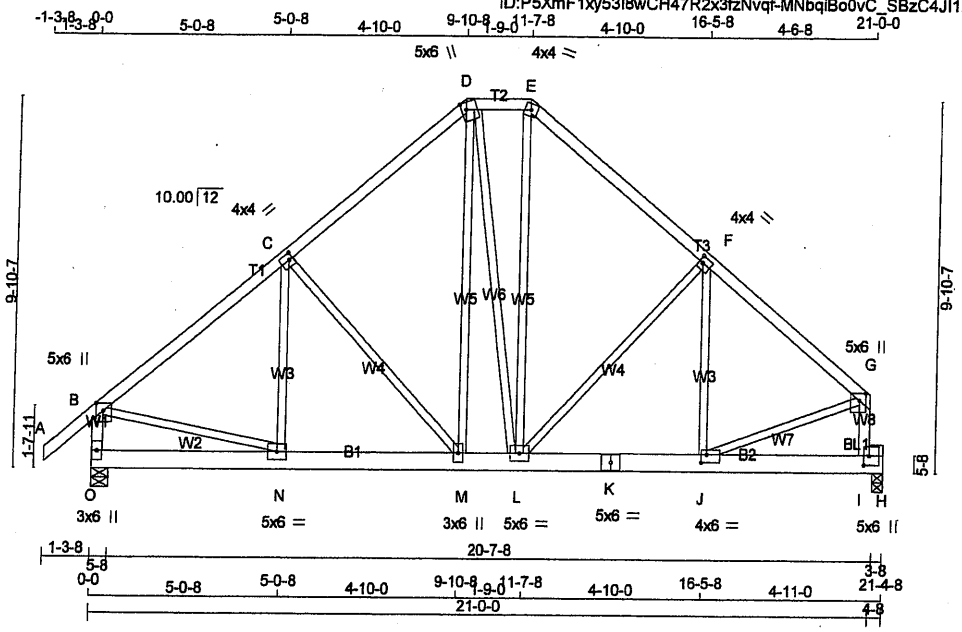


TOTAL WEIGHT = 124 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.25	2.00
I	BMVK+p	MT20	5.0	6.0	3.00	1.75
J	BMWW-t	MT20	4.0	6.0	2.50	1.75
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1976	0	1976	0	0	5-8	5-8
H	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
H	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

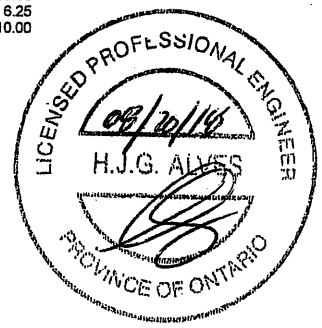
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1720 / 0	-129.7	-129.7	0.66 (1)	4.25
C- D	-1358 / 0	-129.7	-129.7	0.61 (1)	4.71
D- E	-1010 / 0	-129.7	-129.7	0.08 (1)	6.14
E- F	-1356 / 0	-129.7	-129.7	0.55 (1)	4.83
F- G	-1724 / 0	-129.7	-129.7	0.58 (1)	4.36
O- B	-1892 / 0	0.0	0.0	0.20 (1)	6.10
I- G	-1913 / 0	0.0	0.0	0.21 (1)	6.07
O- N	0 / 0	-38.5	-38.5	0.08 (3)	10.00
N- M	0 / 1358	-38.5	-38.5	0.22 (1)	10.00
M- L	0 / 1007	-38.5	-38.5	0.14 (1)	10.00
L- K	0 / 1361	-38.5	-38.5	0.32 (1)	10.00
K- J	0 / 1361	-38.5	-38.5	0.32 (1)	10.00
J- I	-170 / 0	-38.5	-38.5	0.61 (1)	6.25
I- H	0 / 0	-38.5	-38.5	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-112 / 160	0.05 (1)
C- M	-529 / 0	0.54 (1)
M- D	0 / 479	0.11 (1)
D- L	0 / 20	0.00 (3)
L- E	0 / 486	0.11 (1)
L- F	-530 / 0	0.54 (1)
J- F	-70 / 158	0.04 (3)
B- N	0 / 1398	0.31 (1)
J- G	0 / 1627	0.37 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.61/1.00 (I-J:1) , WB=0.54/1.00 (F-L:1) , SSI=0.64/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.38 (J) (INPUT = 1.00)

DWG NO. TAM T1805196
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T144	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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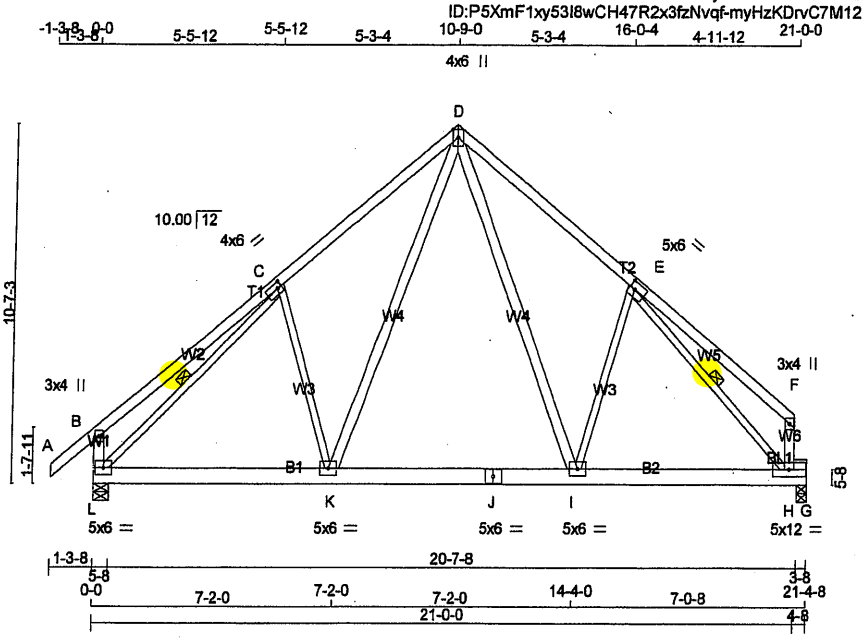

TOTAL WEIGHT = 3 X 120 = 360 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0	2.50	2.00
F	TMV+p	MT20	3.0	4.0		
H	BMVWK-t	MT20	5.0	12.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1976	0	1976	0	0	5-8	5-8
G	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
G	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 54	-129.7	-129.7	0.62 (1)	10.00
C- D	-1608 / 0	-129.7	-129.7	0.78 (1)	4.15
D- E	-1629 / 0	-129.7	-129.7	0.72 (1)	4.26
E- F	0 / 79	-129.7	-129.7	0.57 (1)	10.00
L- B	-441 / 0	0.0	0.0	0.05 (1)	7.81
H- F	-213 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 1332	-38.5	-38.5	0.28 (1)	10.00
K- J	0 / 941	-38.5	-38.5	0.35 (1)	10.00
J- I	0 / 941	-38.5	-38.5	0.35 (1)	10.00
I- H	0 / 1296	-38.5	-38.5	0.76 (1)	10.00
H- G	0 / 0	-38.5	-38.5	0.38 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-465 / 10	0.28 (1)
K- D	0 / 748	0.12 (1)
D- I	0 / 807	0.13 (1)
I- E	-275 / 69	0.17 (1)
L- C	-1943 / 0	0.77 (1)
E- H	-2063 / 0	0.75 (1)

LICENSED PROFESSIONAL ENGINEER
Aug/20/18
H.J.G. AIVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.78/1.00 (C-D:1) , BC=0.76/1.00 (H-I:1) , WB=0.77/1.00 (C-L:1) , SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.53 (E) (INPUT = 1.00)

DWG NO. TAM T1805197
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T145	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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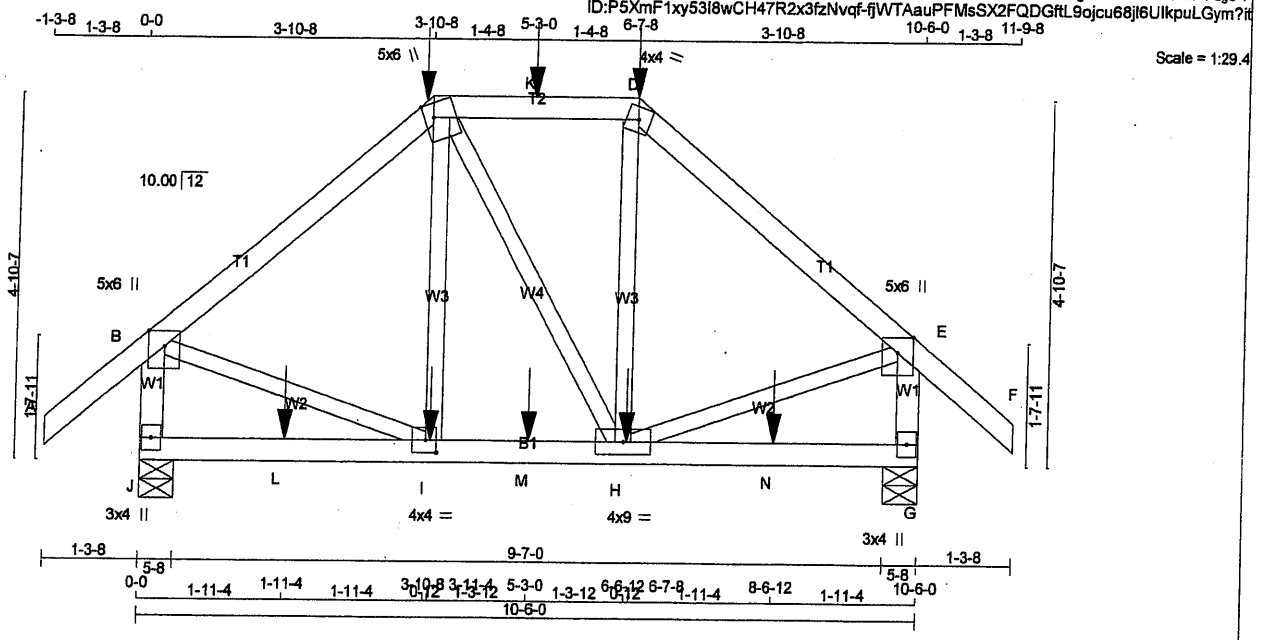


TOTAL WEIGHT = 51 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 292.9 lbs FACTORED DOWN AT 3-10-8, AND 121.6 lbs FACTORED DOWN AT 5-3-0, AND 292.9 lbs FACTORED DOWN AT 6-7-8 ON TOP CHORD, AND 36.3 lbs FACTORED DOWN AT 1-11-4, 45.9 lbs FACTORED DOWN AT 3-11-4, 45.9 lbs FACTORED DOWN AT 5-3-0, AND 45.9 lbs FACTORED DOWN AT 6-6-12, AND 36.3 lbs FACTORED DOWN AT 8-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1492	0	1492	0	0	5-8	5-8
G	1492	0	1492	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1085	704 / 0	160 / 0	0 / 0	0 / 0	220 / 0	0 / 0
G	1085	704 / 0	160 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.20 (1)	10.00
B- C	-1104 / 0	-129.7	-129.7	0.40 (1)	5.44
C- K	-844 / 0	-129.7	-129.7	0.32 (1)	6.15
K- D	-844 / 0	-129.7	-129.7	0.32 (1)	6.15
D- E	-1104 / 0	-129.7	-129.7	0.40 (1)	5.44
E- F	0 / 57	-129.7	-129.7	0.20 (1)	10.00
J- B	-1418 / 0	0.0	0.0	0.17 (1)	6.81
G- E	-1417 / 0	0.0	0.0	0.16 (1)	6.81
J- L	0 / 0	-38.5	-38.5	0.15 (3)	10.00
L- I	0 / 0	-38.5	-38.5	0.15 (3)	10.00
I- M	0 / 844	-38.5	-38.5	0.23 (2)	10.00
M- H	0 / 844	-38.5	-38.5	0.23 (2)	10.00
H- N	0 / 0	-38.5	-38.5	0.15 (3)	10.00
N- G	0 / 0	-38.5	-38.5	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-73 / 161	0.04 (3)
C- H	0 / 1	0.00 (3)
H- D	-74 / 163	0.04 (3)
B- I	0 / 890	0.22 (1)
H- E	0 / 890	0.22 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-202	-202	—	FRONT	VERT	TOTAL
D	6-7-8	-202	-202	—	FRONT	VERT	TOTAL
H	6-6-12	-33	-33	—	FRONT	VERT	TOTAL
I	3-11-4	-33	-33	—	FRONT	VERT	TOTAL
K	5-3-0	-83	-83	—	FRONT	VERT	TOTAL
L	1-11-4	-22	-26	—	FRONT	VERT	TOTAL
M	5-3-0	-33	-33	—	FRONT	VERT	TOTAL
N	8-6-12	-22	-26	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.23/1.00 (H-I:2) , WB=0.22/1.00 (B-I:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

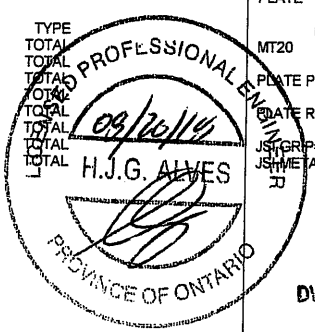
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI:GRIP= 0.89 (I) (INPUT = 0.90)
JSI:METAL= 0.46 (B) (INPUT = 1.00)



DWG NO. TAM 11805198
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T146	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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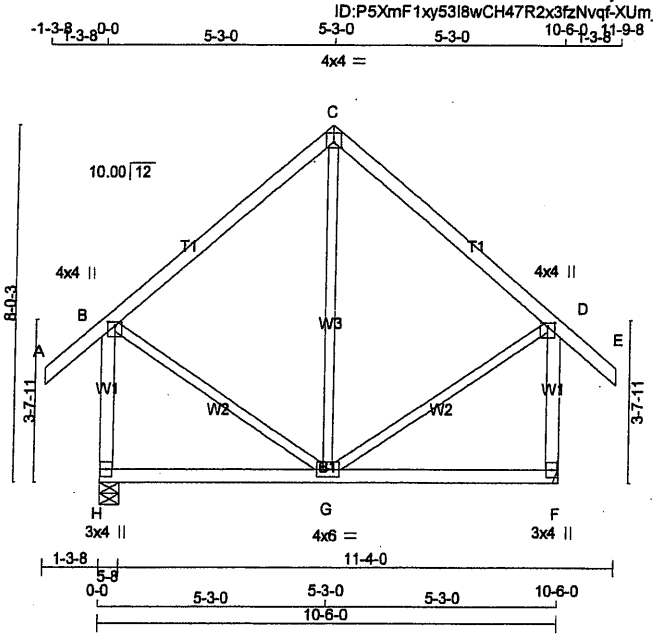


TOTAL WEIGHT = 55 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1062	0	1062	0	0	5-8	5-8
F	1062	0	1062	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	771	507 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0
F	771	507 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

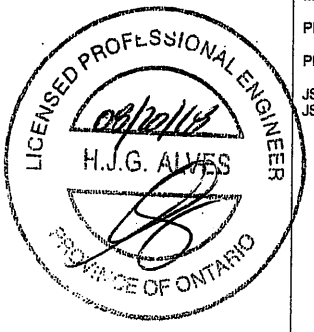
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)		10.00
B- C	-414 / 0	-129.7	-129.7	0.46 (1)		6.25
C- D	-414 / 0	-129.7	-129.7	0.46 (1)		6.25
D- E	0 / 57	-129.7	-129.7	0.18 (1)		10.00
H- B	-986 / 0	0.0	0.0	0.22 (1)		7.81
F- D	-986 / 0	0.0	0.0	0.22 (1)		7.81
H- G	0 / 0	-38.5	-38.5	0.24 (3)		10.00
G- F	0 / 0	-38.5	-38.5	0.24 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-150 / 149	0.18 (1)
B- G	0 / 376	0.08 (1)
G- D	0 / 376	0.08 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.24/1.00 (G-H:3) , WB=0.18/1.00 (C-G:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

DWG NO. TAM T1805799
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	T150	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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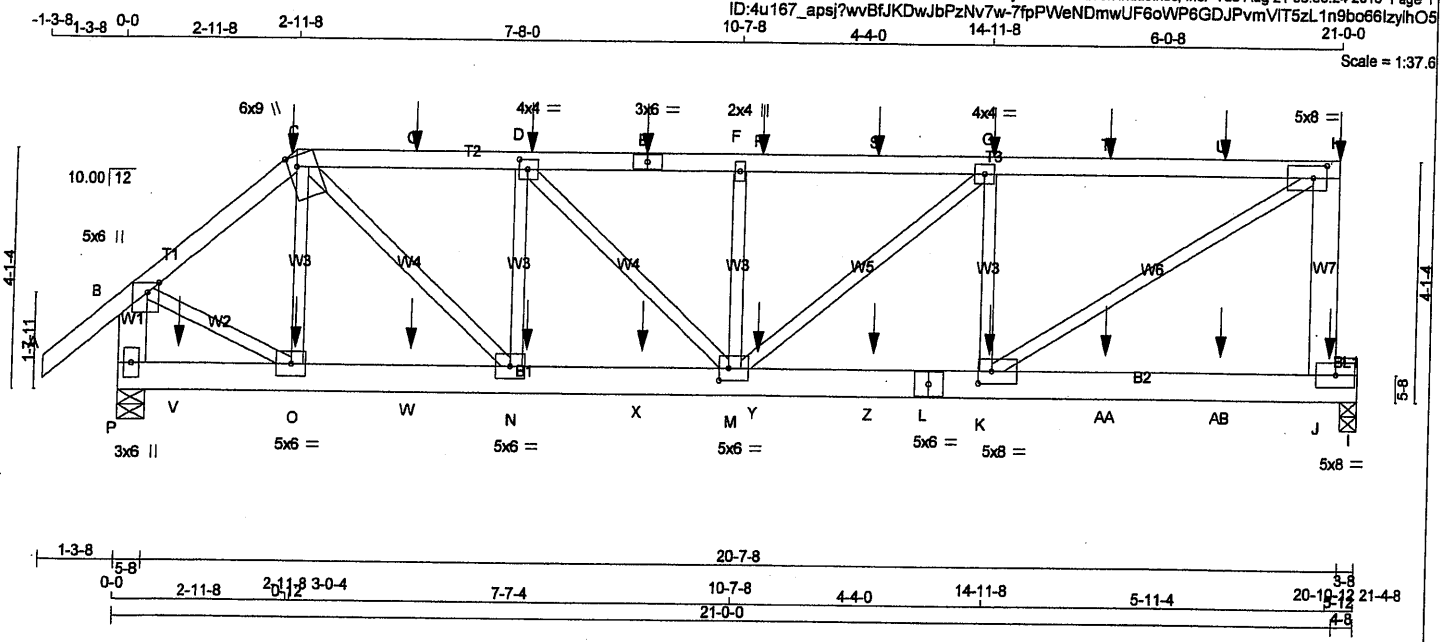

TOTAL WEIGHT = 2 X 105 = 209 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	SIDE(61.0)
H- J	2	12	TOP
P- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- L	2	12	SIDE(183.1)
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
O- C	1	6	SIDE(12.6)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	8.0	2.50	2.75
J	BMVK-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	8.0	2.50	2.75
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0	2.50	2.00
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	3194	0	3194	0	0	5-8	5-8
I	3051	0	3051	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	2330	1510 / 0	385 / 0	0 / 0	0 / 0	435 / 0	0 / 0
I	2238	1413 / 0	399 / 0	0 / 0	0 / 0	426 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.10 (1)	10.00
B- C	-2906 / 0	-129.7	-129.7	0.14 (1)	5.26
C- Q	-3811 / 0	-129.7	-129.7	0.28 (1)	4.56
Q- D	-3811 / 0	-129.7	-129.7	0.28 (1)	4.56
D- E	-4192 / 0	-129.7	-129.7	0.29 (1)	4.37
E- F	-4192 / 0	-129.7	-129.7	0.29 (1)	4.37
F- R	-4192 / 0	-129.7	-129.7	0.59 (1)	3.90
R- S	-4192 / 0	-129.7	-129.7	0.59 (1)	3.90
S- G	-4192 / 0	-129.7	-129.7	0.59 (1)	3.90
G- T	-3843 / 0	-129.7	-129.7	0.63 (1)	4.07
T- U	-3843 / 0	-129.7	-129.7	0.63 (1)	4.07
U- H	-3843 / 0	-129.7	-129.7	0.63 (1)	4.07
J- H	-3155 / 0	0.0	0.0	0.25 (1)	7.77
P- B	-3129 / 0	0.0	0.0	0.12 (1)	7.80
P- V	0 / 0	-38.5	-38.5	0.03 (3)	10.00
V- O	0 / 0	-38.5	-38.5	0.03 (3)	10.00
O- W	0 / 2206	-38.5	-38.5	0.17 (1)	10.00
W- N	0 / 2206	-38.5	-38.5	0.17 (1)	10.00
N- X	0 / 3812	-38.5	-38.5	0.33 (1)	10.00
X- M	0 / 3812	-38.5	-38.5	0.33 (1)	10.00
M- Y	0 / 3843	-38.5	-38.5	0.46 (1)	10.00
Y- Z	0 / 3843	-38.5	-38.5	0.46 (1)	10.00
Z- L	0 / 3843	-38.5	-38.5	0.46 (1)	10.00
L- K	0 / 3843	-38.5	-38.5	0.46 (1)	10.00
K-AA	-193 / 0	-38.5	-38.5	0.67 (1)	6.25
AA-AB	-193 / 0	-38.5	-38.5	0.67 (1)	6.25
AB- J	-193 / 0	-38.5	-38.5	0.67 (1)	6.25
J- I	0 / 0	-168.2	-168.2	0.42 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-644 / 0	0.08 (1)
M- F	-677 / 0	0.09 (1)
M- G	0 / 461	0.06 (1)
K- G	-1606 / 0	0.20 (1)
K- H	0 / 4743	0.59 (1)
B- Q	0 / 2395	0.30 (1)
N- O	-1304 / 0	0.16 (1)
C- N	0 / 2219	0.27 (1)
P- V	0 / 535	0.07 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-28	-28	—	FRONT	VERT	DEAD
C	2-11-8	-148	-148	—	FRONT	VERT	TOTAL
C	2-11-8	-176	-176	—	FRONT	VERT	SNOW
D	7-0-4	-106	-106	—	FRONT	VERT	TOTAL
E	9-0-4	-106	-106	—	FRONT	VERT	TOTAL
G	15-0-4	-106	-106	—	FRONT	VERT	TOTAL
H	21-0-0	-146	-146	—	FRONT	VERT	TOTAL
J	20-10-12	-52	-52	—	FRONT	VERT	TOTAL
K	15-0-4	-38	-38	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.63/1.00 (G-H:1) , BC=0.67/1.00 (J-K:1) , WB=0.59/1.00 (H-K:1) , SSI=0.62/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.47 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO
DATE: 08/21/18
DWG NO. T AM T1805406
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	T150	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 515.8 lbs FACTORED DOWN AT 2-11-8, 156.1 lbs FACTORED DOWN AT 5-0-4, 156.1 lbs FACTORED DOWN AT 7-0-4, 156.1 lbs FACTORED DOWN AT 9-0-4, 156.1 lbs FACTORED DOWN AT 11-0-4, 156.1 lbs FACTORED DOWN AT 13-0-4, 156.1 lbs FACTORED DOWN AT 15-0-4, 156.1 lbs FACTORED DOWN AT 17-0-4, AND 156.1 lbs FACTORED DOWN AT 19-0-4, AND 214.0 lbs FACTORED DOWN AT 21-0-0 ON TOP CHORD, AND 54.9 lbs FACTORED DOWN AT 1-0-4, 61.9 lbs FACTORED DOWN AT 3-0-4, 54.9 lbs FACTORED DOWN AT 5-0-4, 54.9 lbs FACTORED DOWN AT 7-0-4, 54.9 lbs FACTORED DOWN AT 9-0-4, 54.9 lbs FACTORED DOWN AT 11-0-4, 54.9 lbs FACTORED DOWN AT 13-0-4, 54.9 lbs FACTORED DOWN AT 15-0-4, 54.9 lbs FACTORED DOWN AT 17-0-4, AND 54.9 lbs FACTORED DOWN AT 19-0-4, AND 74.3 lbs FACTORED DOWN AT 20-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	7-0-4	-38	-38	—	FRONT	VERT	TOTAL
O	3-0-4	-44	-44	—	FRONT	VERT	TOTAL
Q	5-0-4	-106	-106	—	FRONT	VERT	TOTAL
R	11-0-4	-106	-106	—	FRONT	VERT	TOTAL
S	13-0-4	-106	-106	—	FRONT	VERT	TOTAL
T	17-0-4	-106	-106	—	FRONT	VERT	TOTAL
U	19-0-4	-106	-106	—	FRONT	VERT	TOTAL
V	1-0-4	-38	-38	—	FRONT	VERT	TOTAL
W	5-0-4	-38	-38	—	FRONT	VERT	TOTAL
X	9-0-4	-38	-38	—	FRONT	VERT	TOTAL
Y	11-0-4	-38	-38	—	FRONT	VERT	TOTAL
Z	13-0-4	-38	-38	—	FRONT	VERT	TOTAL
AA	17-0-4	-38	-38	—	FRONT	VERT	TOTAL
AB	19-0-4	-38	-38	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. MOES
PROVINCE OF ONTARIO

BY B.M.L. TAM 71805406
STRUCTURAL

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T151	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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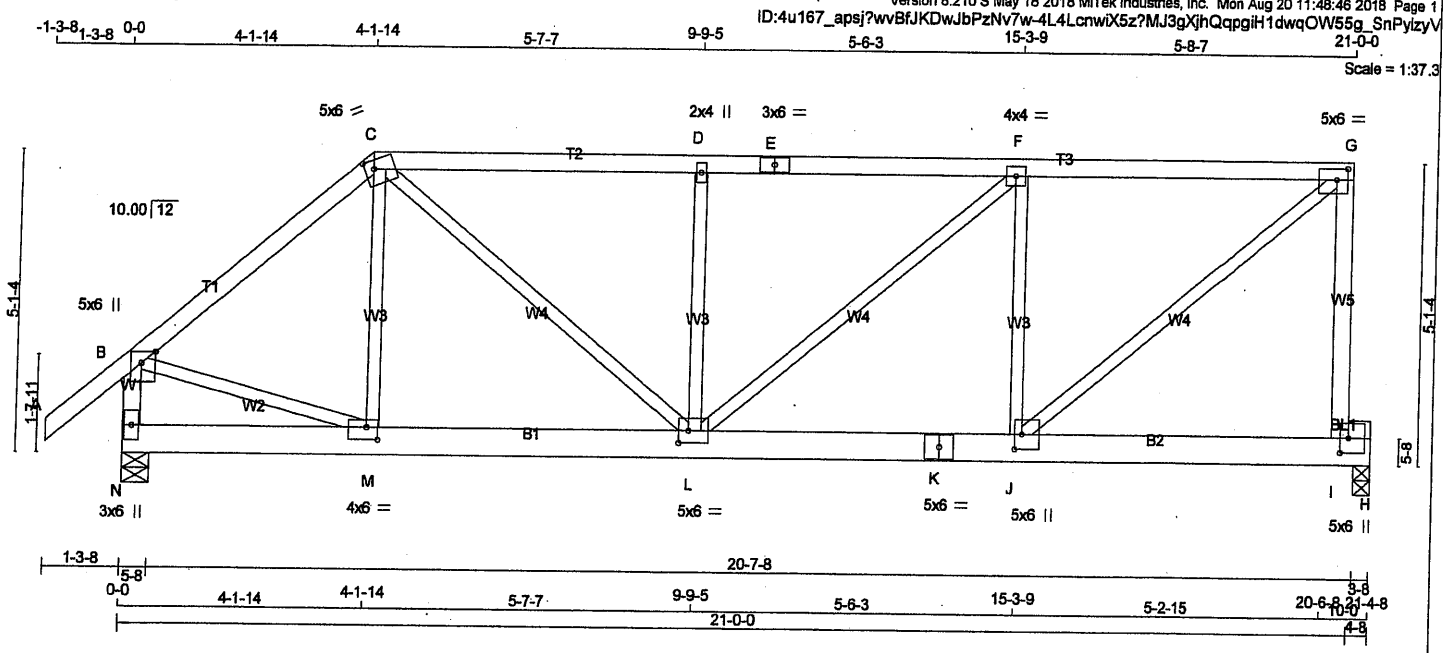

TOTAL WEIGHT = 103 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0	2.25	2.25
I	BMVK+p	MT20	5.0	6.0	3.00	1.75
J	BMWW+t	MT20	5.0	6.0	3.00	1.50
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW-t	MT20	4.0	6.0	2.50	2.25
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1976	0	1976	0	0	5-8	5-8
H	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
H	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1706 / 0	-129.7	-129.7	0.46 (1)	4.55
C- D	-2089 / 0	-129.7	-129.7	0.67 (1)	3.88
D- E	-2089 / 0	-129.7	-129.7	0.68 (1)	3.83
E- F	-2089 / 0	-129.7	-129.7	0.68 (1)	3.83
F- G	-1802 / 0	-129.7	-129.7	0.66 (1)	4.11
I- G	-1842 / 0	0.0	0.0	0.75 (1)	6.17
N- B	-1920 / 0	0.0	0.0	0.20 (1)	6.07
N- M	0 / 0	-38.5	-38.5	0.09 (2)	10.00
M- L	0 / 1304	-38.5	-38.5	0.23 (1)	10.00
L- K	0 / 1802	-38.5	-38.5	0.42 (1)	10.00
K- J	0 / 1802	-38.5	-38.5	0.42 (1)	10.00
J- I	-57 / 0	-38.5	-38.5	0.61 (1)	6.25
I- H	0 / 0	-38.5	-38.5	0.38 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-135 / 129	0.05 (1)
B- M	0 / 1360	0.31 (1)
J- G	0 / 2414	0.54 (1)
C- L	0 / 1024	0.23 (1)
J- F	-1046 / 0	0.37 (1)
L- D	-780 / 0	0.28 (1)
L- F	0 / 378	0.09 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.75/1.00 (G-I:1) , BC=0.61/1.00 (I-J:1) , WB=0.54/1.00 (G-J:1) , SSI=0.64/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.67 (J) (INPUT = 1.00)

DWG NO. TAM 11605231
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T152	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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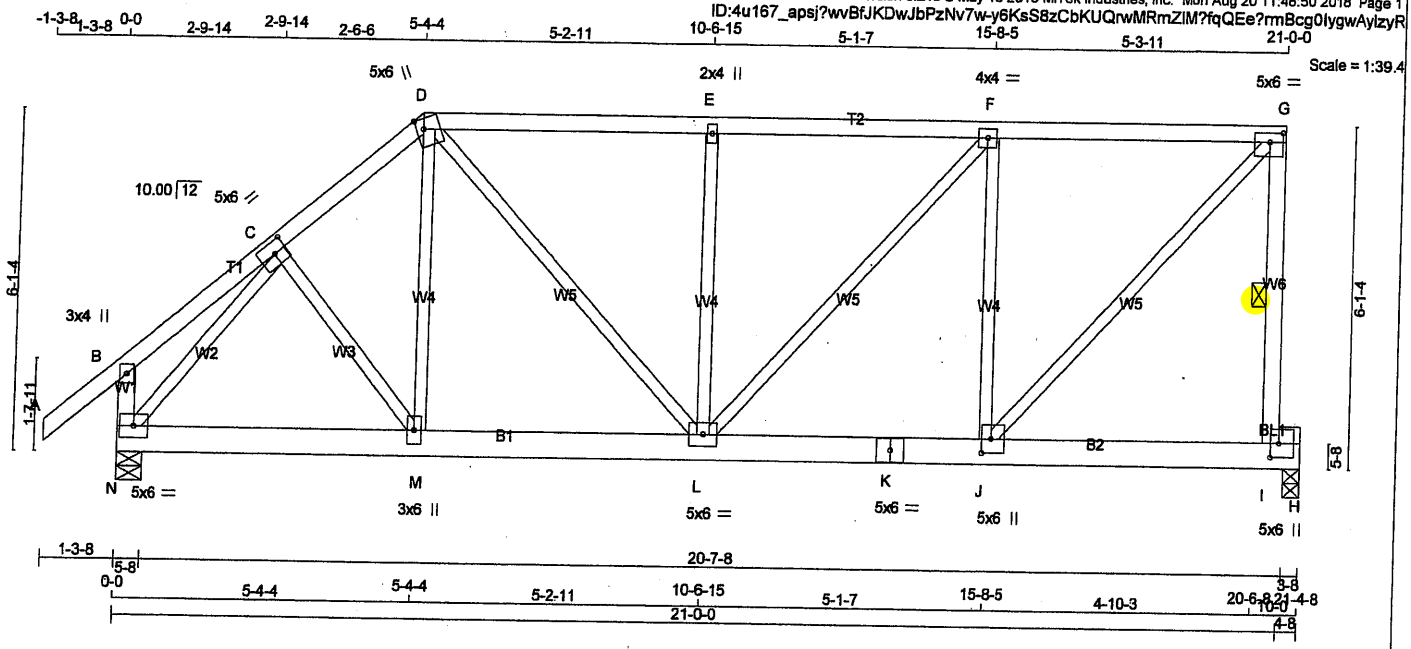


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0	2.00	2.75
I	BMVK+p	MT20	5.0	6.0	3.00	1.75
J	BMWW+t	MT20	5.0	6.0	3.00	2.00
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1749	0	1749	0	0	3-8	3-8
N	1976	0	1976	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0
N	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

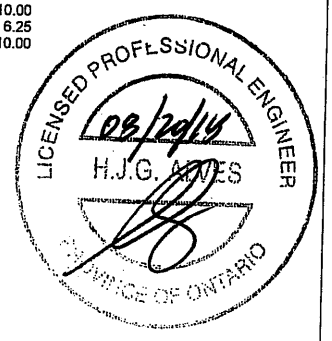
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 22	-129.7	-129.7	0.13 (1)	10.00
C- D	-1698 / 0	-129.7	-129.7	0.16 (1)	4.95
D- E	-1726 / 0	-129.7	-129.7	0.54 (1)	4.39
E- F	-1726 / 0	-129.7	-129.7	0.56 (1)	4.36
F- G	-1420 / 0	-129.7	-129.7	0.54 (1)	4.74
I- G	-1865 / 0	0.0	0.0	0.32 (1)	4.91
N- B	-320 / 0	0.0	0.0	0.03 (1)	7.81
N- M	0 / 1221	-38.5	-38.5	0.22 (1)	10.00
M- L	0 / 1286	-38.5	-38.5	0.23 (1)	10.00
L- K	0 / 1420	-38.5	-38.5	0.37 (1)	10.00
K- J	0 / 1420	-38.5	-38.5	0.37 (1)	10.00
J- I	-47 / 0	-38.5	-38.5	0.61 (1)	6.25
I- H	0 / 0	-38.5	-38.5	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- D	0 / 219	0.05 (3)
J- G	0 / 2160	0.49 (1)
D- L	0 / 652	0.15 (1)
J- F	-1085 / 0	0.59 (1)
L- E	-724 / 0	0.40 (1)
L- F	0 / 459	0.10 (1)
C- M	0 / 121	0.03 (3)
N- C	-1990 / 0	0.60 (1)



TOTAL WEIGHT = 111 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.56/1.00 (E-F:1) , BC=0.61/1.00 (I-J:1) , WB=0.60/1.00 (C-N:1) , SSI=0.64/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.53 (J) (INPUT = 1.00)

DWG NO. TAM T1805232
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T153	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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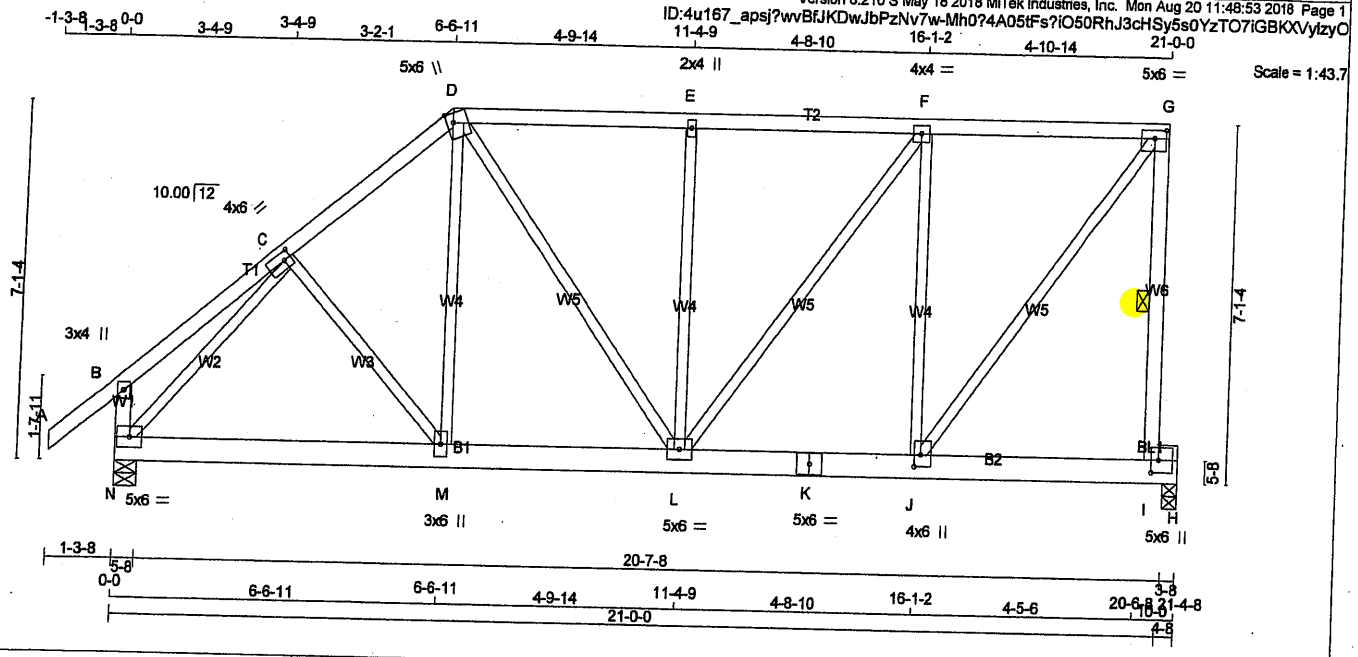

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0	2.00	2.75
I	BMVK+p	MT20	5.0	6.0	3.00	1.75
J	BMWW+t	MT20	4.0	6.0	2.75	1.50
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1976	0	1976	0	0	5-8	5-8
H	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
H	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)		10.00
B- C	0 / 30	-129.7	-129.7	0.21 (1)		10.00
C- D	-1644 / 0	-129.7	-129.7	0.25 (1)		4.90
D- E	-1453 / 0	-129.7	-129.7	0.44 (1)		4.86
E- F	-1454 / 0	-129.7	-129.7	0.45 (1)		4.83
F- G	-1148 / 0	-129.7	-129.7	0.44 (1)		5.32
I- G	-1890 / 0	0.0	0.0	0.41 (1)		4.88
N- B	-344 / 0	0.0	0.0	0.04 (1)		7.81
N- M	0 / 1262	-38.5	-38.5	0.25 (1)		10.00
M- L	0 / 1241	-38.5	-38.5	0.25 (2)		10.00
L- K	0 / 1148	-38.5	-38.5	0.33 (1)		10.00
K- J	0 / 1148	-38.5	-38.5	0.33 (1)		10.00
J- I	-41 / 0	-38.5	-38.5	0.61 (1)		6.25
I- H	0 / 0	-38.5	-38.5	0.38 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-39 / 81	0.02 (3)
M- D	0 / 341	0.08 (2)
N- C	-1980 / 0	0.81 (1)
J- G	0 / 2019	0.45 (1)
D- L	0 / 364	0.08 (1)
J- F	-1122 / 0	0.92 (1)
L- E	-667 / 0	0.55 (1)
L- F	0 / 532	0.12 (1)

TOTAL WEIGHT = 2 X 118 = 236 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.45/1.00 (E-F:1) , BC=0.61/1.00 (I-J:1) , WB=0.92/1.00 (F-J:1) , SSI=0.64/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.89 (J) (INPUT = 0.90)
USI METAL= 0.54 (J) (INPUT = 1.00)
ER

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AIVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805233
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T154	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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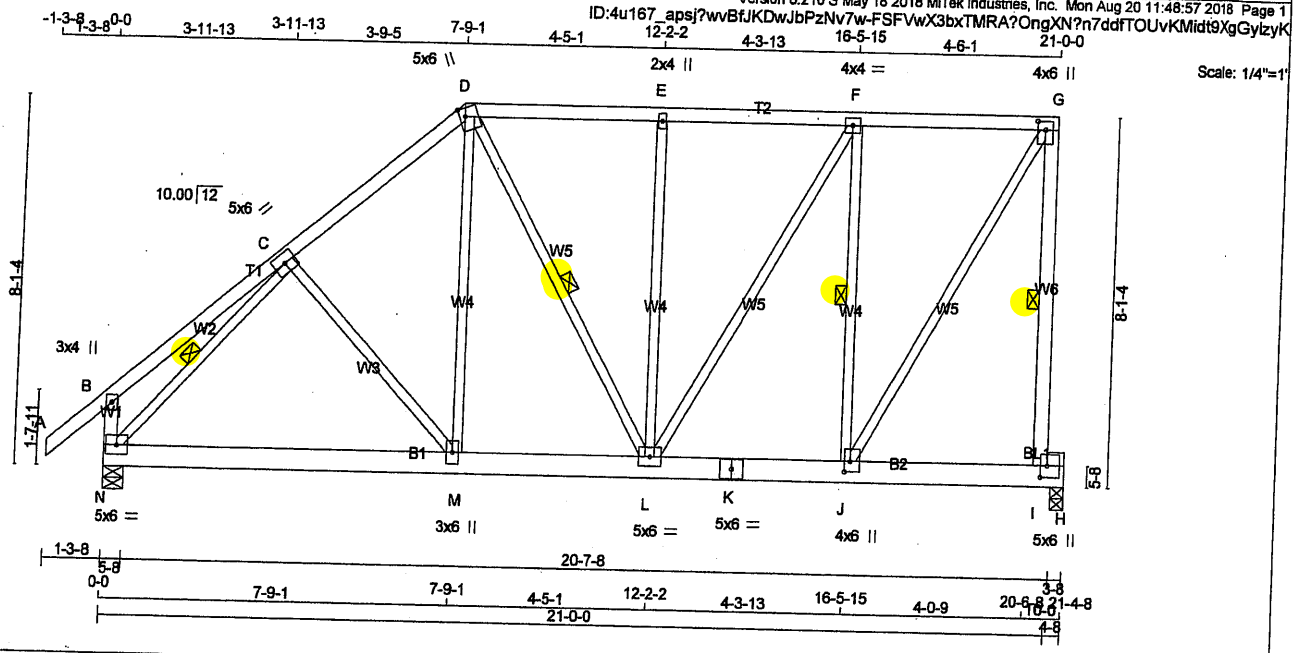

TOTAL WEIGHT = 2 X 125 = 249 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0	2.25	2.00
I	BMVK+p	MT20	5.0	6.0	3.00	1.75
J	BMWW+t	MT20	4.0	6.0	2.75	1.50
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1976	0	1976	0	0	5-8	5-8
H	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
H	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I, C-N, D-L, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	1 (1)	0.18	10.00
B- C	0 / 37	-129.7	-129.7	1 (1)	0.31	10.00
C- D	-1568 / 0	-129.7	-129.7	1 (1)	0.36	4.85
D- E	-1237 / 0	-129.7	-129.7	1 (1)	0.35	5.32
E- F	-1237 / 0	-129.7	-129.7	1 (1)	0.36	5.29
F- G	-943 / 0	-129.7	-129.7	1 (1)	0.35	5.88
I- G	-1918 / 0	0.0	0.0	1 (1)	0.54	4.86
N- B	-371 / 0	0.0	0.0	1 (1)	0.04	7.81
N- M	0 / 1285	-38.5	-38.5	2 (2)	0.30	10.00
M- L	0 / 1179	-38.5	-38.5	2 (2)	0.29	10.00
L- K	0 / 943	-38.5	-38.5	2 (1)	0.30	10.00
K- J	0 / 943	-38.5	-38.5	2 (1)	0.30	10.00
J- I	-36 / 0	-38.5	-38.5	2 (1)	0.61	6.25
I- H	0 / 0	-38.5	-38.5	2 (1)	0.38	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-167 / 51	0.10 (1)
M- D	0 / 465	0.10 (2)
N- C	-1959 / 0	0.48 (1)
J- G	0 / 1944	0.44 (1)
D- L	-18 / 116	0.03 (1)
J- F	-1155 / 0	0.46 (1)
L- E	-611 / 0	0.73 (1)
L- F	0 / 604	0.14 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.54/1.00 (G-I:1) , BC=0.61/1.00 (I-J:1) , WB=0.73/1.00 (E-L:1) , SSI=0.64/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.89 (G) (INPUT = 0.90)
LSI METAL= 0.48 (C) (INPUT = 1.00)

DWG NO. TAM T1805234
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T155	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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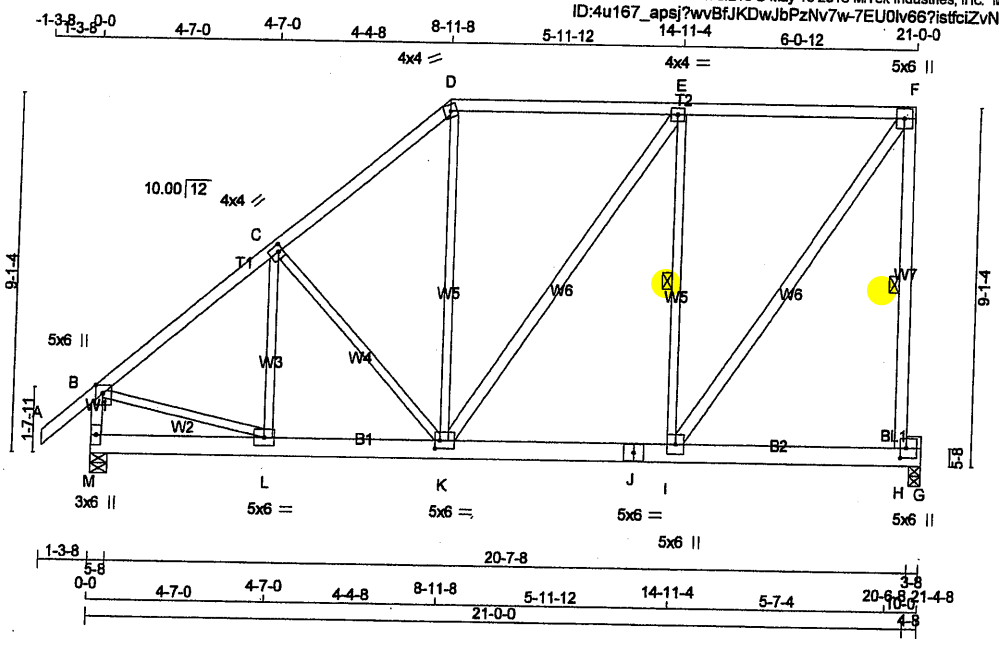

TOTAL WEIGHT = 6 X 128 = 767 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0		
H	BMVK+p	MT20	5.0	6.0	3.00	1.75
I	BMWW+t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0	2.50	1.50
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1976	0	1976	0	0	5-8	5-8
G	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
G	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1718 / 0	-129.7	-129.7	0.53 (1)	4.46
C- D	-1439 / 0	-129.7	-129.7	0.50 (1)	4.80
D- E	-1072 / 0	-129.7	-129.7	0.86 (1)	4.30
E- F	-1032 / 0	-129.7	-129.7	0.86 (1)	4.37
H- F	-1829 / 0	0.0	0.0	0.67 (1)	4.95
M- B	-1897 / 0	0.0	0.0	0.20 (1)	6.10
M- L	0 / 0	-38.5	-38.5	0.07 (2)	10.00
L- K	0 / 1353	-38.5	-38.5	0.21 (1)	10.00
K- J	0 / 1032	-38.5	-38.5	0.36 (1)	10.00
J- I	0 / 1032	-38.5	-38.5	0.36 (1)	10.00
I- H	-31 / 0	-38.5	-38.5	0.61 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-167 / 106	0.07 (1)
C- K	-430 / 0	0.35 (1)
K- D	0 / 392	0.09 (2)
K- E	0 / 72	0.01 (1)
I- E	-1027 / 0	0.53 (1)
I- F	0 / 1863	0.30 (1)
B- L	0 / 1401	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.86/1.00 (D-E:1) , BC=0.61/1.00 (H-I:1) , WB=0.53/1.00 (E-I:1) , SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.39 (I) (INPUT = 1.00)

DWG NO. TAM 1805235
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T156	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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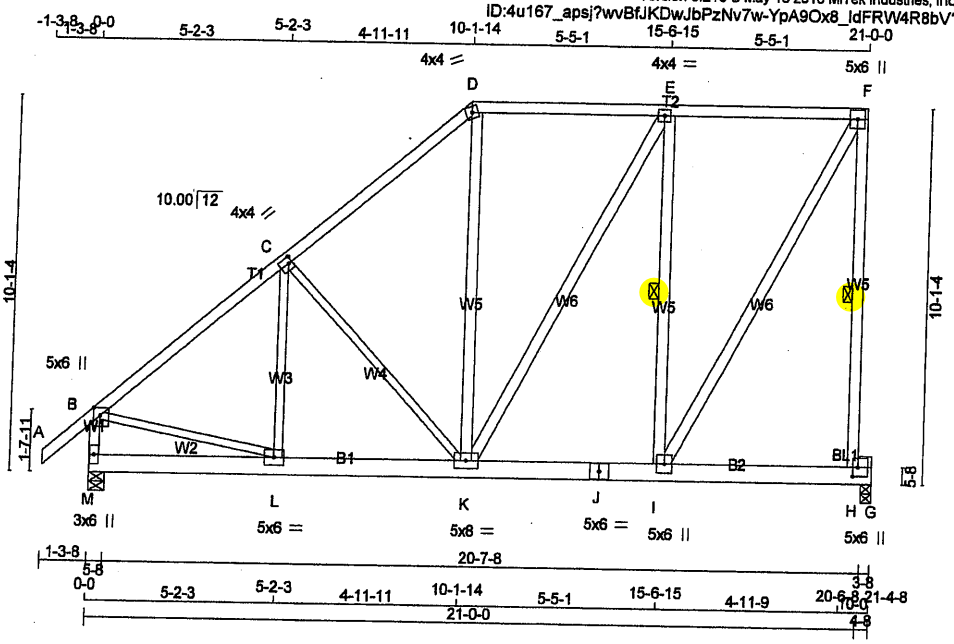

TOTAL WEIGHT = 2 X 140 = 280 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0		
H	BMVK+p	MT20	5.0	6.0	3.00	1.75
I	BMWW+t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1976	0	1976	0	0	5-8	5-8
G	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
G	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1719 / 0	-129.7	-129.7	0.71 (1)	4.17
C- D	-1330 / 0	-129.7	-129.7	0.65 (1)	4.68
D- E	-984 / 0	-129.7	-129.7	0.68 (1)	5.07
E- F	-859 / 0	-129.7	-129.7	0.67 (1)	5.36
H- F	-1859 / 0	0.0	0.0	0.89 (1)	4.92
M- B	-1890 / 0	0.0	0.0	0.20 (1)	6.10
M- L	0 / 0	-38.5	-38.5	0.08 (3)	10.00
L- K	0 / 1359	-38.5	-38.5	0.22 (1)	10.00
K- J	0 / 859	-38.5	-38.5	0.31 (1)	10.00
J- I	0 / 859	-38.5	-38.5	0.31 (1)	10.00
I- H	-28 / 0	-38.5	-38.5	0.61 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-96 / 163	0.05 (1)
C- K	-559 / 0	0.62 (1)
K- D	0 / 326	0.05 (2)
K- E	0 / 261	0.04 (1)
I- E	-1099 / 0	0.53 (1)
I- F	0 / 1823	0.29 (1)
B- L	0 / 1396	0.31 (1)

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.89/1.00 (F-H:1) , BC=0.61/1.00 (H-I:1) , WB=0.62/1.00 (C-K:1) , SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.37 (F) (INPUT = 1.00)

DWG NO. TAM 1180536
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	P1	56	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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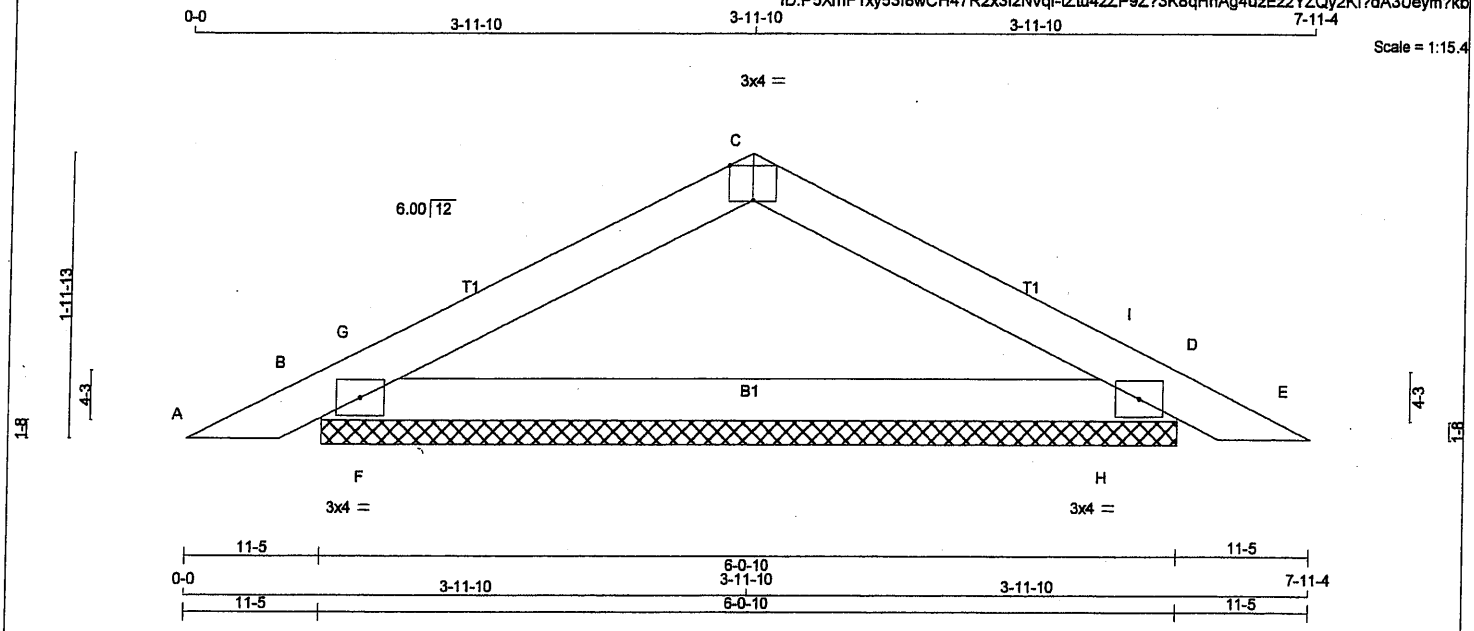

TOTAL WEIGHT = 56 X 17 = 953 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	623	0	623	0	0	6-0-10	6-0-10
D	623	0	623	0	0	6-0-10	6-0-10

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	451	298 / 0	64 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	451	298 / 0	64 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	F- G	0 / 294	0.00 (1)
B- G	-613 / 0	-129.7	-129.7	0.16 (2)	6.25	H- I	0 / 294	0.00 (1)
G- C	-449 / 0	-129.7	-129.7	0.23 (1)	6.25			
C- I	-449 / 0	-129.7	-129.7	0.23 (1)	6.25			
I- D	-613 / 0	-129.7	-129.7	0.16 (2)	6.25			
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00			
B- F	0 / 417	-38.5	-38.5	0.11 (3)	10.00			
F- H	0 / 417	-38.5	-38.5	0.23 (1)	10.00			
H- D	0 / 417	-38.5	-38.5	0.11 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-I:1) , BC=0.23/1.00 (F-H:1) , WB=0.00/1.00 (F-G:1) , SSI=0.21/1.00 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches .

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1180569
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	P5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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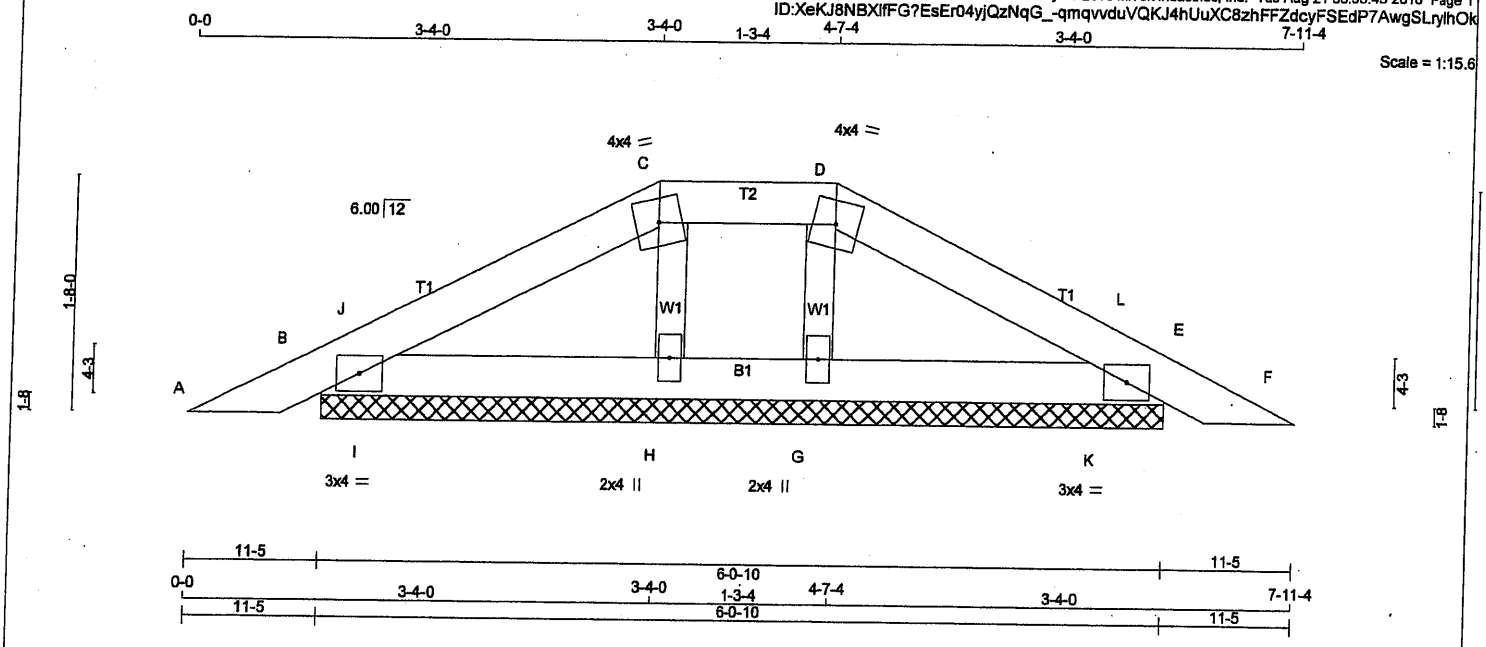

TOTAL WEIGHT = 2 X 18 = 37 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1+w	MT20	2.0	4.0	
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	332	0	332	0	0	6-0-10	6-0-10
E	332	0	332	0	0	6-0-10	6-0-10
H	290	0	290	0	0	6-0-10	6-0-10
G	290	0	290	0	0	6-0-10	6-0-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	237	169 / 0	25 / 0	0 / 0	0 / 0	43 / 0	0 / 0
E	237	169 / 0	25 / 0	0 / 0	0 / 0	43 / 0	0 / 0
H	214	129 / 0	39 / 0	0 / 0	0 / 0	46 / 0	0 / 0
G	214	129 / 0	39 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24		-129.7	-129.7	0.07 (1)		10.00
B- J	-69 / 0		-129.7	-129.7	0.02 (3)		6.25
J- C	-62 / 0		-129.7	-129.7	0.09 (1)		6.25
C- D	-41 / 0		-129.7	-129.7	0.04 (1)		6.25
D- L	-62 / 0		-129.7	-129.7	0.09 (1)		6.25
L- E	-69 / 0		-129.7	-129.7	0.02 (3)		6.25
E- F	0 / 24		-129.7	-129.7	0.07 (1)		10.00
B- I	0 / 56		-38.5	-38.5	0.09 (1)		10.00
I- H	0 / 56		-38.5	-38.5	0.09 (1)		10.00
H- G	0 / 41		-38.5	-38.5	0.06 (1)		10.00
G- K	0 / 56		-38.5	-38.5	0.09 (1)		10.00
K- E	0 / 56		-38.5	-38.5	0.09 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-185 / 0	0.03 (1)
G- D	-185 / 0	0.03 (1)
I- J	-141 / 25	0.00 (1)
K- L	-141 / 25	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09; CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (D-L:1) , BC=0.09/1.00 (G-K:1) , WB=0.03/1.00 (D-G:1) , SSI=0.12/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)

DWG NO. TAM T1805396
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	P6	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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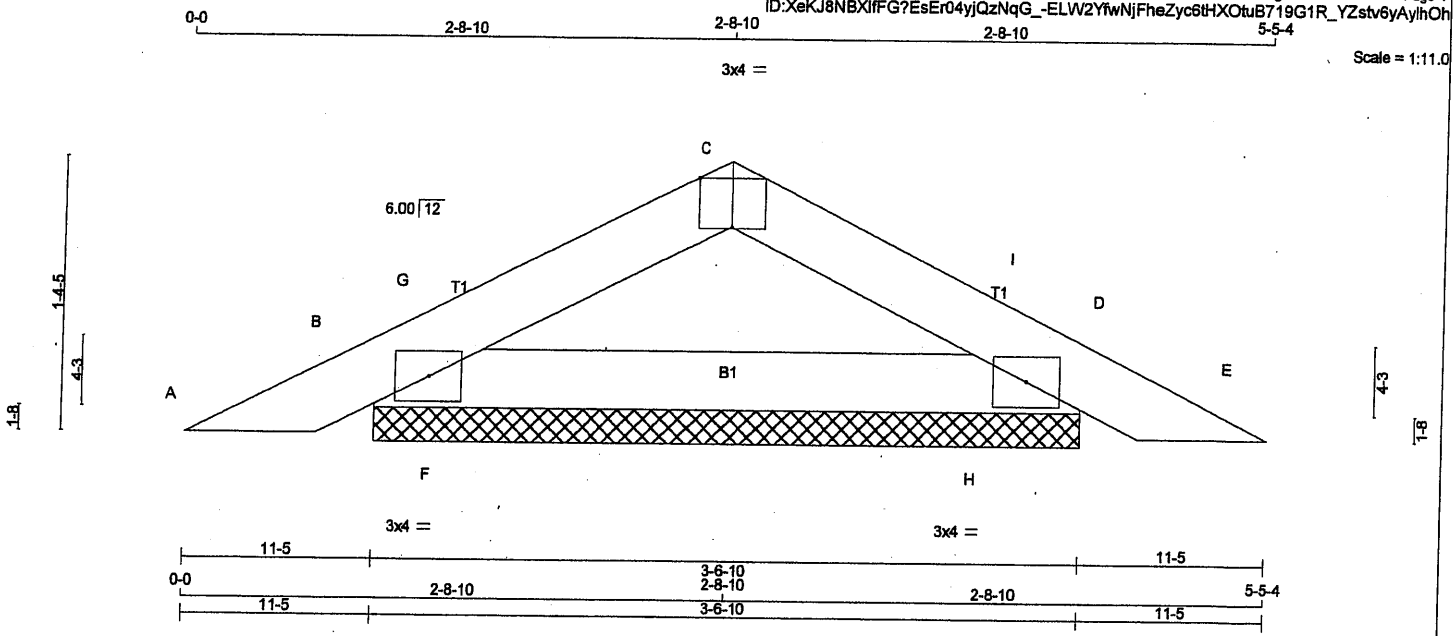


TOTAL WEIGHT = 4 X 11 = 44 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	412	0	412	0	0	3-6-10	3-6-10
D	412	0	412	0	0	3-6-10	3-6-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	297	203 / 0	37 / 0	0 / 0	0 / 0	57 / 0	0 / 0
D	297	203 / 0	37 / 0	0 / 0	0 / 0	57 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- G	-335 / 0	-129.7	-129.7	0.06 (2)	6.25
G- C	-255 / 0	-129.7	-129.7	0.08 (1)	6.25
C- I	-255 / 0	-129.7	-129.7	0.08 (1)	6.25
I- D	-335 / 0	-129.7	-129.7	0.06 (2)	6.25
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- F	0 / 241	-38.5	-38.5	0.05 (1)	10.00
F- H	0 / 241	-38.5	-38.5	0.10 (1)	10.00
H- D	0 / 241	-38.5	-38.5	0.05 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 109	0.00 (1)
H- I	0 / 109	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-G:1) , BC=0.10/1.00 (F-H:1) , WB=0.00/1.00 (F-G:1) , SSI=0.09/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

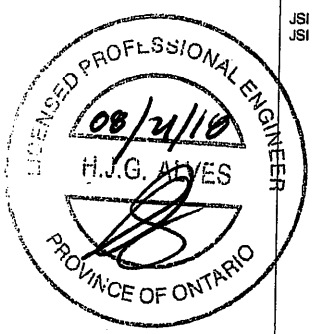
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



DWG NO. TAM T1805396
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	P7	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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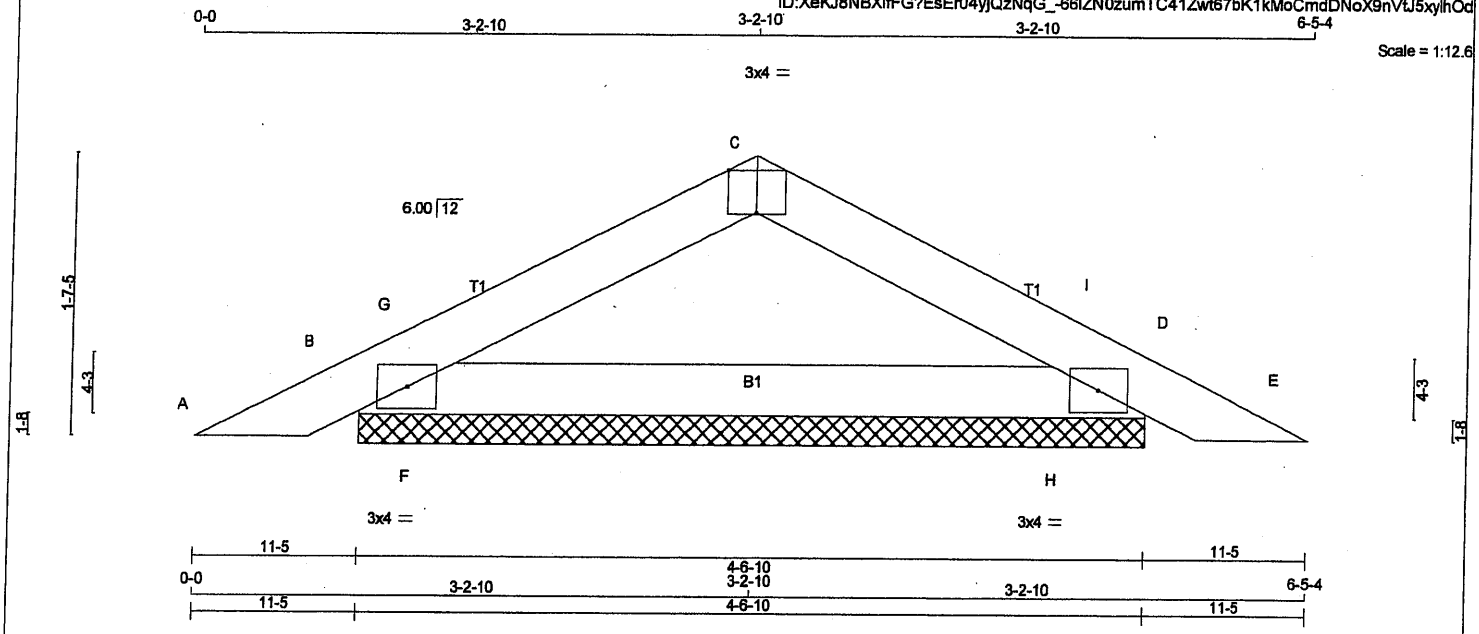

TOTAL WEIGHT = 4 X 13 = 54 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	496	0	496	0	0	4-6-10	4-6-10
D	496	0	496	0	0	4-6-10	4-6-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	359	241 / 0	48 / 0	0 / 0	0 / 0	70 / 0	0 / 0
D	359	241 / 0	48 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- G	-444 / 0	-129.7	-129.7	0.10 (2)	6.25
G- C	-333 / 0	-129.7	-129.7	0.13 (1)	6.25
C- I	-333 / 0	-129.7	-129.7	0.13 (1)	6.25
I- D	-444 / 0	-129.7	-129.7	0.10 (2)	6.25
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- F	0 / 312	-38.5	-38.5	0.06 (3)	10.00
F- H	0 / 312	-38.5	-38.5	0.14 (1)	10.00
H- D	0 / 312	-38.5	-38.5	0.06 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 173	0.00 (1)
H- I	0 / 173	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-I:1) , BC=0.14/1.00 (F-H:1) , WB=0.00/1.00 (F-G:1) , SSI=0.13/1.00 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (D) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ARES
PROVINCE OF ONTARIO

H.J.G. TAM T1805397
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	P21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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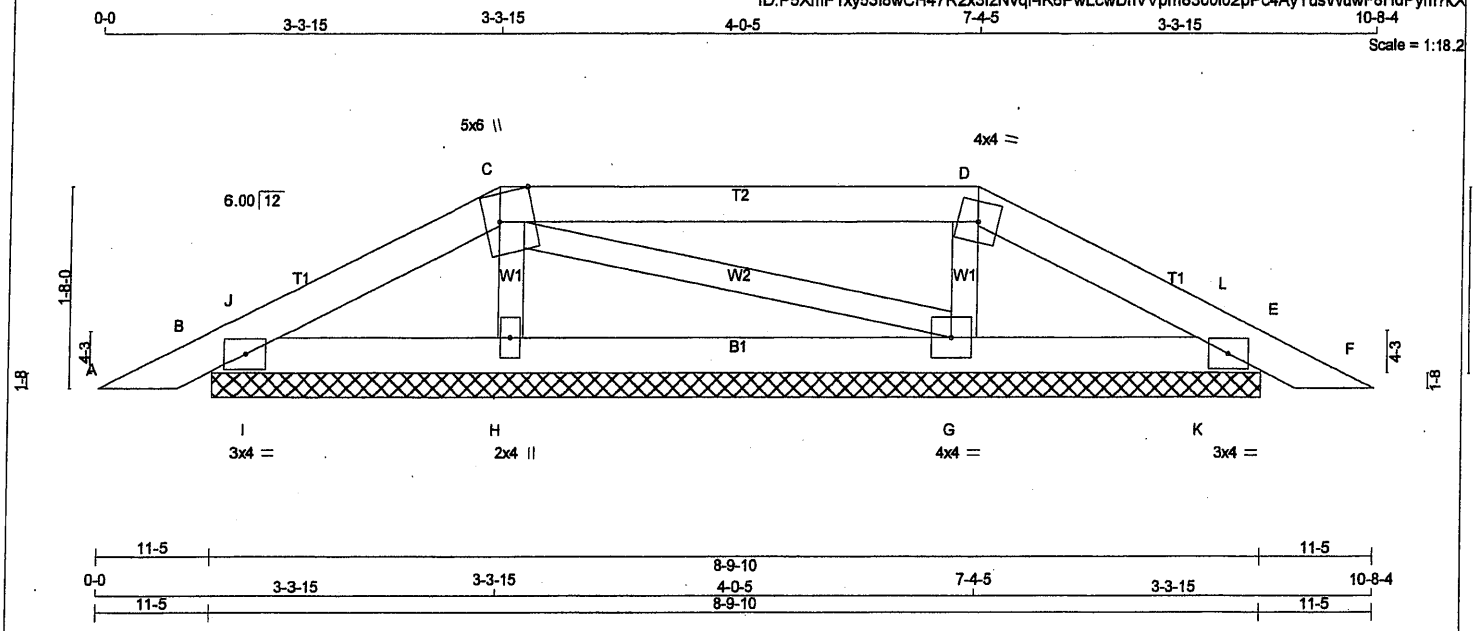


TOTAL WEIGHT = 2 X 28 = 55 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	Edge	3.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	335	0	335	0	0	8-9-10	8-9-10
E	326	0	326	0	0	8-9-10	8-9-10
H	514	0	514	0	0	8-9-10	8-9-10
G	533	0	533	0	0	8-9-10	8-9-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	236	178 / 0	18 / 0	0 / 0	0 / 0	40 / 0	0 / 0
E	230	173 / 0	18 / 0	0 / 0	0 / 0	39 / 0	0 / 0
H	381	222 / 0	75 / 0	0 / 0	0 / 0	85 / 0	0 / 0
G	395	233 / 0	75 / 0	0 / 0	0 / 0	86 / 0	0 / 0

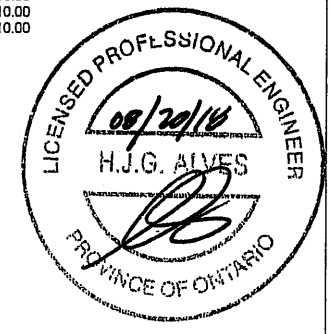
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	H- C	-348 / 0	0.05 (1)
B- J	-82 / 0	-129.7	-129.7	0.01 (3)	6.25	C- G	-18 / 0	0.00 (1)
J- C	-81 / 0	-129.7	-129.7	0.09 (1)	6.25	G- D	-362 / 0	0.05 (1)
C- D	-26 / 0	-129.7	-129.7	0.36 (1)	6.25	I- J	-153 / 0	0.00 (1)
D- L	-63 / 0	-129.7	-129.7	0.09 (1)	6.25	K- L	-154 / 0	0.00 (1)
L- E	-63 / 0	-129.7	-129.7	0.01 (3)	6.25			
E- F	0 / 24	-129.7	-129.7	0.07 (1)	10.00			
B- I	0 / 72	-38.5	-38.5	0.09 (1)	10.00			
I- H	0 / 72	-38.5	-38.5	0.09 (1)	10.00			
H- G	0 / 43	-38.5	-38.5	0.09 (2)	10.00			
G- K	0 / 56	-38.5	-38.5	0.09 (1)	10.00			
K- E	0 / 56	-38.5	-38.5	0.09 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.36/1.00 (C-D:1) , BC=0.09/1.00 (B-I:1) , WB=0.05/1.00 (D-G:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)

DWG NO. TAM 176057 70
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	P22	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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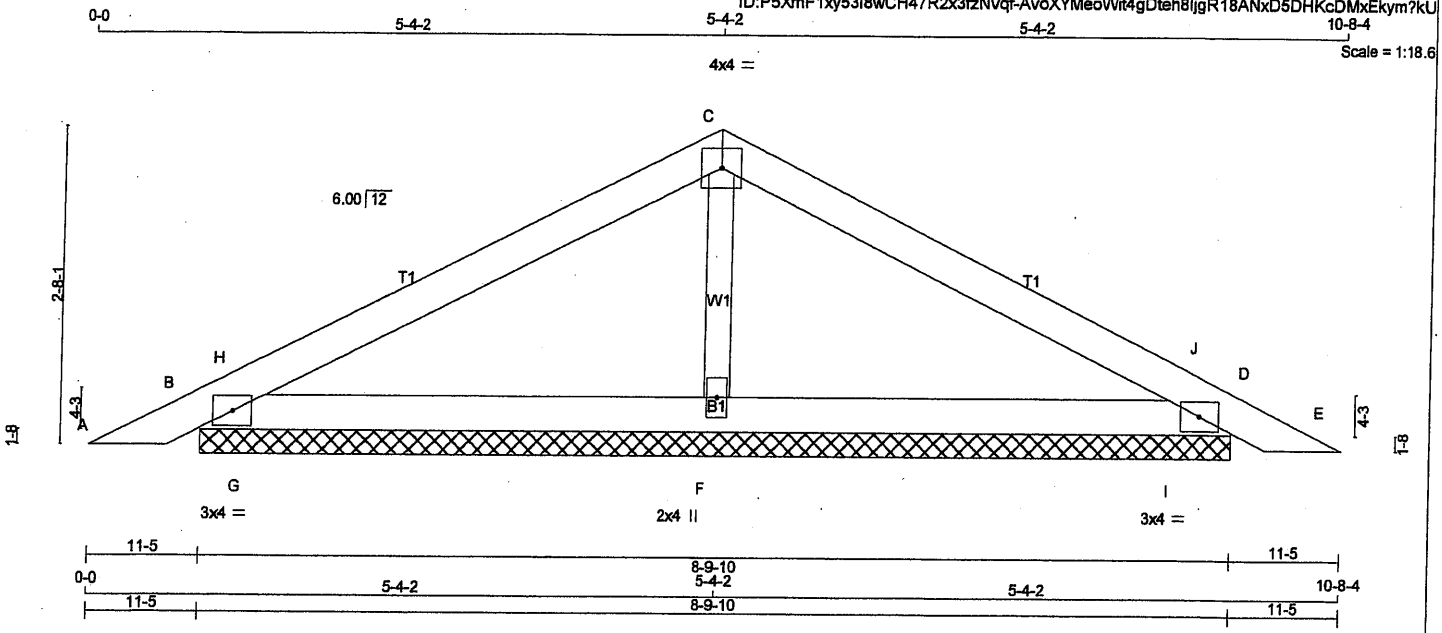


TOTAL WEIGHT = 8 X 25 = 202 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	525	0	525	0	0	8-9-10	8-9-10
D	525	0	525	0	0	8-9-10	8-9-10
F	658	0	658	0	0	8-9-10	8-9-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	375	264 / 0	42 / 0	0 / 0	0 / 0	69 / 0	0 / 0
D	375	264 / 0	42 / 0	0 / 0	0 / 0	69 / 0	0 / 0
F	491	279 / 0	101 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

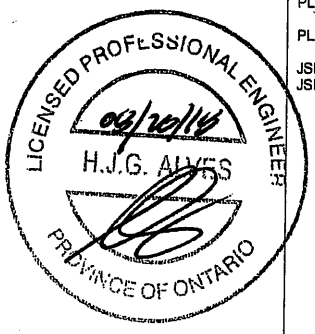
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- H	-99 / 0	-129.7	-129.7	0.11 (1)	6.25
H- C	-194 / 0	-129.7	-129.7	0.30 (1)	6.25
C- J	-194 / 0	-129.7	-129.7	0.30 (1)	6.25
J- D	-99 / 0	-129.7	-129.7	0.11 (1)	6.25
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- G	0 / 166	-38.5	-38.5	0.29 (1)	10.00
G- F	0 / 166	-38.5	-38.5	0.29 (1)	10.00
F- I	0 / 166	-38.5	-38.5	0.29 (1)	10.00
I- D	0 / 166	-38.5	-38.5	0.29 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-313 / 0	0.05 (1)
G- H	-481 / 31	0.00 (1)
I- J	-481 / 31	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-J:1) , BC=0.29/1.00 (D-I:1) , WB=0.05/1.00 (C-F:1) , SSI=0.37/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

DWG NO. TAM T1805171
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	P24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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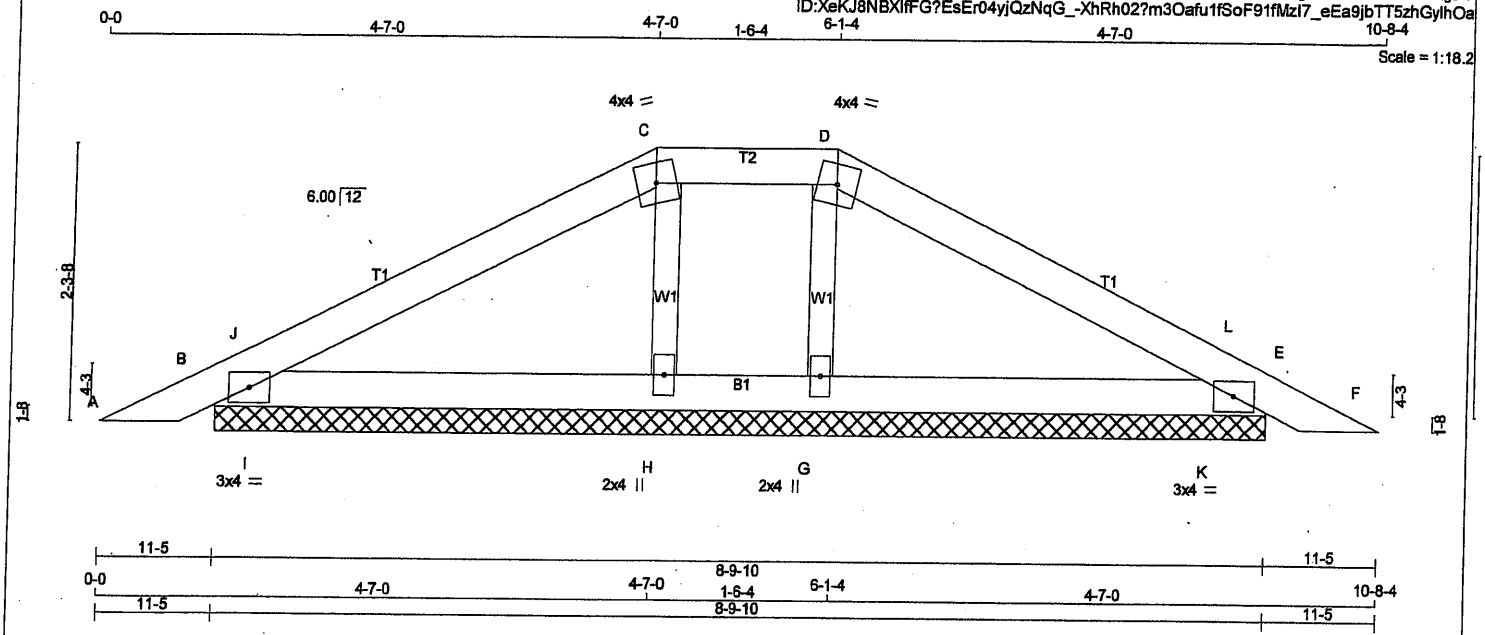

TOTAL WEIGHT = 26 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1+w	MT20	2.0	4.0	
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	456	0	456	0	0	8-9-10	8-9-10
E	456	0	456	0	0	8-9-10	8-9-10
H	398	0	398	0	0	8-9-10	8-9-10
G	398	0	398	0	0	8-9-10	8-9-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	327	228 / 0	38 / 0	0 / 0	0 / 0	61 / 0	0 / 0
E	327	228 / 0	38 / 0	0 / 0	0 / 0	61 / 0	0 / 0
H	294	176 / 0	54 / 0	0 / 0	0 / 0	64 / 0	0 / 0
G	294	176 / 0	54 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- J	-94 / 0	-129.7	-129.7	0.04 (3)	6.25
J- C	-126 / 0	-129.7	-129.7	0.21 (1)	6.25
C- D	-96 / 0	-129.7	-129.7	0.05 (1)	6.25
D- L	-126 / 0	-129.7	-129.7	0.21 (1)	6.25
L- E	-94 / 0	-129.7	-129.7	0.04 (3)	6.25
E- F	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- I	0 / 109	-38.5	-38.5	0.19 (1)	10.00
I- H	0 / 109	-38.5	-38.5	0.19 (1)	10.00
H- G	0 / 96	-38.5	-38.5	0.13 (1)	10.00
G- K	0 / 109	-38.5	-38.5	0.19 (1)	10.00
K- E	0 / 109	-38.5	-38.5	0.19 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-245 / 0	0.04 (1)
G- D	-245 / 0	0.04 (1)
I- J	-299 / 48	0.00 (1)
K- L	-299 / 48	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-J:1) , BC=0.19/1.00 (H-I:1) , WB=0.04/1.00 (C-H:1) , SSI=0.25/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805398
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	P133	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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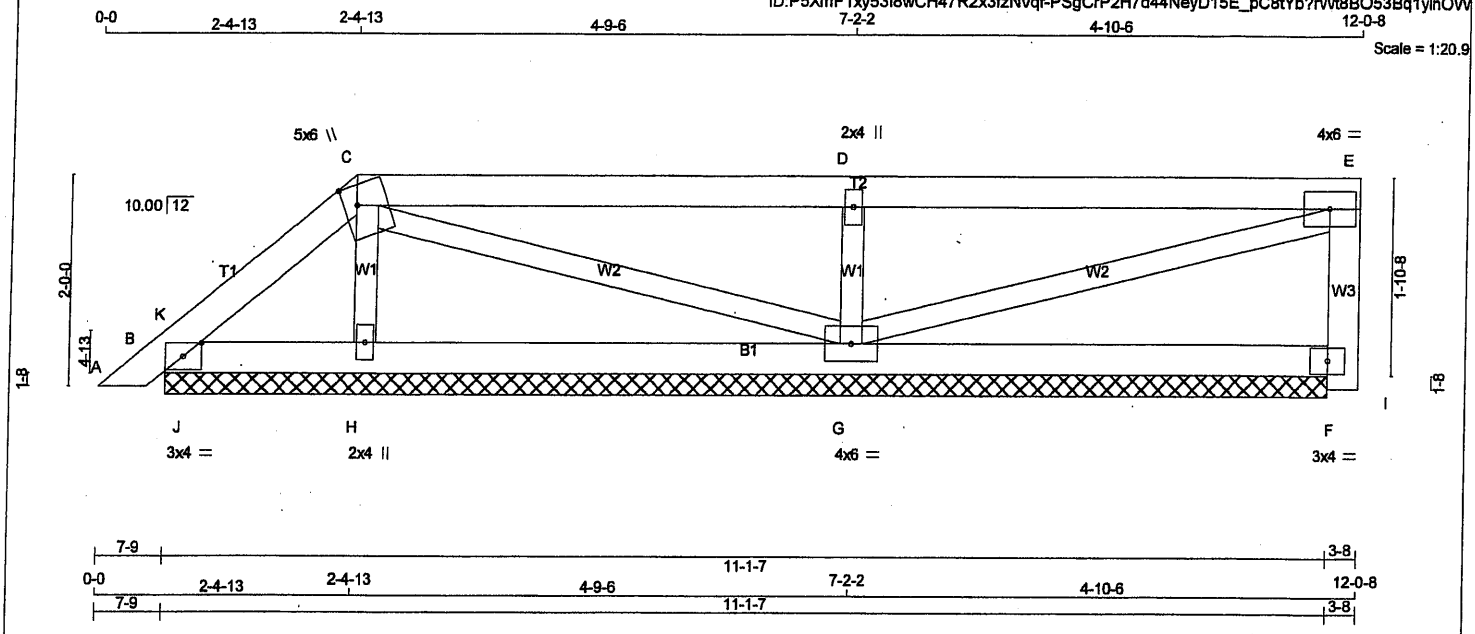

TOTAL WEIGHT = 2 X 39 = 78 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BVM-I	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	-77	0	0	0	-91	11-1-7	11-1-7
H	551	0	551	0	0	11-1-7	11-1-7
G	1515	0	1515	0	0	11-1-7	11-1-7

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	-67	0 / -8	0 / -35	0 / 0	0 / 0	0 / -24	0 / 0
H	414	225 / 0	92 / 0	0 / 0	0 / 0	97 / 0	0 / 0
G	1109	696 / 0	183 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- K	0 / 472	-129.7	-129.7	0.08 (1)	10.00
K- C	0 / 411	-129.7	-129.7	0.12 (1)	10.00
C- D	0 / 921	-129.7	-129.7	0.70 (1)	10.00
D- E	0 / 920	-129.7	-129.7	0.70 (1)	10.00
I- F	0 / 0	0.0	0.0	0.00 (1)	10.00
F- E	0 / 91	0.0	0.0	0.02 (3)	10.00
B- J	-314 / 0	-38.5	-38.5	0.06 (1)	6.25
J- H	-314 / 0	-38.5	-38.5	0.08 (3)	6.25
H- G	-340 / 0	-38.5	-38.5	0.21 (2)	6.25
G- F	0 / 0	-38.5	-38.5	0.21 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-397 / 0	0.06 (1)
C- G	-610 / 0	0.25 (1)
G- D	-797 / 0	0.12 (1)
G- E	-968 / 0	0.39 (1)
J- K	-201 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.70/1.00 (C-D:1) , BC=0.21/1.00 (G-H:2) , WB=0.39/1.00 (E-G:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.81 (G) (INPUT = 0.90)
SSI METAL= 0.23 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/21/18
H.J.G. ALDES
PROVINCE OF ONTARIO

DWG NO. TAM T7805399
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297085	P134	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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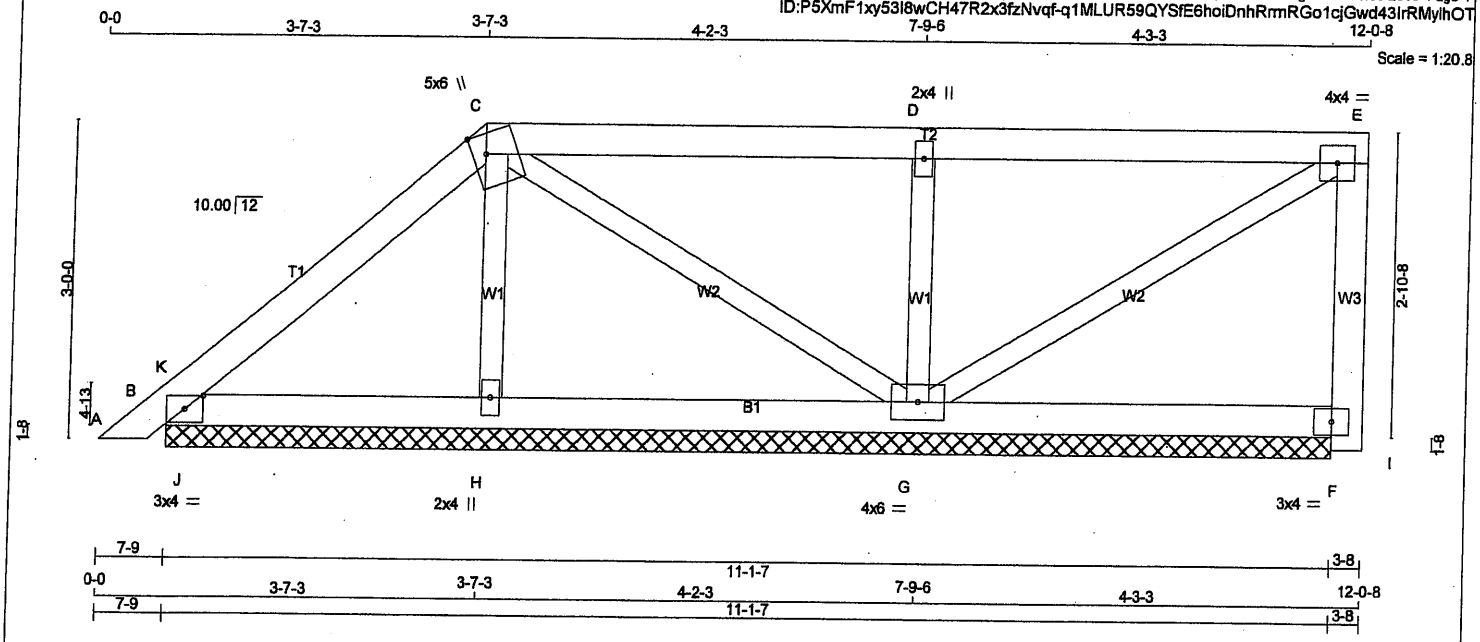


TOTAL WEIGHT = 2 X 42 = 84 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	BVM-I	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	223	0	223	0	0	11-1-7	11-1-7
H	411	0	411	0	0	11-1-7	11-1-7
G	1355	0	1355	0	0	11-1-7	11-1-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	155	124 / 0	7 / 0	0 / 0	0 / 0	24 / 0	0 / 0
H	311	163 / 0	73 / 0	0 / 0	0 / 0	75 / 0	0 / 0
G	991	626 / 0	160 / 0	0 / 0	0 / 0	205 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- K	0 / 234	-129.7	-129.7	0.08 (1)	10.00
K- C	0 / 113	-129.7	-129.7	0.16 (1)	10.00
C- D	0 / 446	-129.7	-129.7	0.48 (1)	10.00
D- E	0 / 446	-129.7	-129.7	0.48 (1)	10.00
I- F	0 / 0	0.0	0.0	0.00 (1)	10.00
F- E	0 / 82	0.0	0.0	0.01 (3)	10.00
B- J	-93 / 0	-38.5	-38.5	0.12 (1)	6.25
J- H	-93 / 0	-38.5	-38.5	0.12 (1)	6.25
H- G	-102 / 0	-38.5	-38.5	0.14 (3)	6.25
G- F	0 / 0	-38.5	-38.5	0.14 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-230 / 0	0.04 (1)
C- G	-403 / 0	0.15 (1)
G- D	-690 / 0	0.12 (1)
G- E	-521 / 0	0.20 (1)
J- K	-359 / 12	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (C-D:1) , BC=0.14/1.00 (G-H:3) , WB=0.20/1.00 (E-G:1) , SSI=0.29/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (C) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)

DWG NO. TAM 71805400
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie®

HHUS/HGUS

See Hanger Options Information on pp. 125–127.

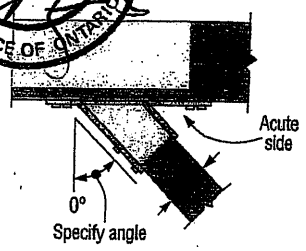
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes									
LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅝	2¹⁵⁄₁₆	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅞	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705 / 12.03	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	(16) 10d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅞	5	4⅛	(20) 16d	(8) 16d	2685 / 11.94	6625 / 29.47	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7¹⁵⁄₁₆	3	6¹⁵⁄₁₆	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅞	5	6⅛	(36) 16d	(12) 16d	3310 / 14.72	7675 / 34.14	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[1]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.72	2020 / 8.99	590 / 2.62	1435 / 6.38
SS ■ LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS26-2	14	3⅝	5⅜	3	3⅝	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
SS ■ LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS28-2	14	3⅝	7⅝	3	6⅛	(22) 16d	(6) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⅝	7⅞	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
■ HHUS210-2	14	3⅝	9⅜	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⅝	9⅞	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4⅝	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4⅝	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-3	14	4⅛	9	3	7⅝	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4⅝	9¼	4	8⅜	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⅝	7⅝	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-4	14	6⅛	8⅞	3	7⅞	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⅝	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⅝	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⅝	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
■ LUS46	18	3⅝	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS46	14	3⅝	5¼	3	3⅝	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
■ LUS48	18	3⅝	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ LUS410	18	3⅝	8¾	2	5⅝	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.

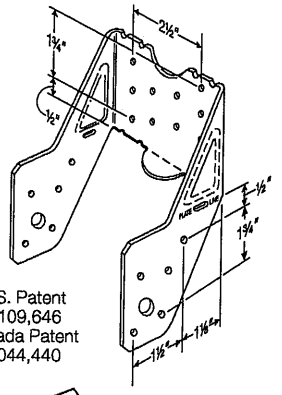
TBE – Truss Bearing Enhancers

SIMPSON Strong-Tie®

One size works with any number of girder plys. The TBE transfers load from the truss or girder to plates for bearing-limited conditions, and provides exceptional uplift capacity. Replaces nail-on scabs that provide lower load transfer, or in some cases, an additional ply when needed for bearing. The table lists factored resistances for TBE4 used on 2x4 and TBE6 used on 2x6 top plates. The tables give the different resistances calculated for TBE with and without wood bearing.

Material: 18 gauge

Finish: G90 galvanized

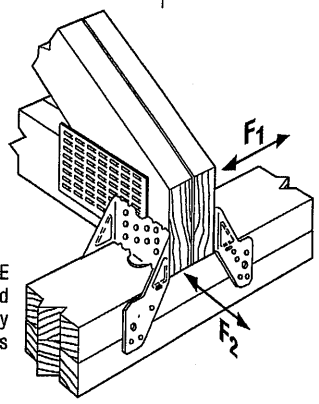


TBE4
(TBE6 similar)

U.S. Patent
5,109,646
Canada Patent
2,044,440

Design:

- Factored resistances are in accordance with CSA 086-14.
- Factored uplift resistances have been increased 15% for short term load duration with no further increase allowed; reduce resistances by 15% for standard term load duration.
- Factored resistances are determined by nail shear calculations or tests of the metal connectors. The attached wood members must be designed to with stand the loads imposed by the nails.



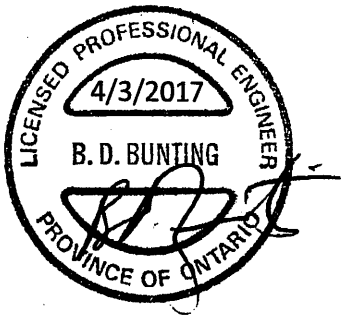
Two TBE installed with two ply girder truss

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia x 3" long common wire, 10d x 1½" = 0.148" dia x 1½" long.
- TBE must be installed in pairs.
- Top plate size is 2x4 for TBE4, 2x6 for TBE6. Use alternate installation for TBE4 and TBE6 on larger plates or pre-sheathed walls.
- Do not use TBEs in end-grain-bearing applications.
- See current catalogue for alternate installations.

Model No.	No. of Plies	Fasteners		TBE Only Factored Resistance				Combined TBE and Wood Bearing Factored Resistance	
		Truss	Plate	Uplift (K_D = 1.15)	Normal (K_D = 1.00)	Lateral (K_D = 1.15) F_1	Lateral (K_D = 1.15) F_2	Normal[2] (K_D = 1.00)	TBL[4]
				lb.	lb.	lb.	lb.	lb.	in.
D.Fir-L									
TBE4	1	(20) 10d x 1½"	(20) 10d x 1½"	1605	3540	655	1415	7820	6.44
	2	(20) 10d	(20) 10d	1605	3660	655	1415	12180	5.00
	3	(20) 10d	(20) 10d	1605	3660	655	1415	16445	4.50
	4	(20) 10d	(20) 10d	1605	3660	655	1415	20705	4.25
TBE6	1	(20) 10d x 1½"	(20) 10d x 1½"	1760	3540	490	1745	10235	8.41
	2	(20) 10d	(20) 10d	1760	3860	490	1745	17250	7.09
	3	(20) 10d	(20) 10d	1760	3860	490	1745	23945	6.56
	4	(20) 10d	(20) 10d	1760	3860	490	1745	30640	6.29
S-P-F									
TBE4	1	(20) 10d x 1½"	(20) 10d x 1½"	1605	3220	615	1415	6445	6.99
	2	(20) 10d	(20) 10d	1605	3440	615	1415	9890	5.37
	3	(20) 10d	(20) 10d	1605	3440	615	1415	13120	4.74
	4	(20) 10d	(20) 10d	1605	3440	615	1415	16345	4.43
TBE6	1	(20) 10d x 1½"	(20) 10d x 1½"	1760	3220	490	1585	8290	8.99
	2	(20) 10d	(20) 10d	1760	3440	490	1585	13680	7.42
	3	(20) 10d	(20) 10d	1760	3440	490	1585	18750	6.78
	4	(20) 10d	(20) 10d	1760	3440	490	1585	23820	6.46

1. Factored resistances are for two TBEs only. Wood factored bearing resistance may be added as shown in the table.
2. Factored bearing resistances shown assume Q_r'/A_b and Q_r'/A_b= 812 psi (5.60 MPa) for D.Fir-L and 614 psi (4.24 MPa) for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 for required bearing reinforcement when compression loads are applied to both sides of truss member.
3. Use lower of top plate or wood truss species.
4. Total bearing length, TBL, equals the plate width plus simulated bearing length provided by the TBE. TBE4 = 3½" plate width; TBE6 = 5½" plate width.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TS/LTS/MTS/HTS

Twist Straps

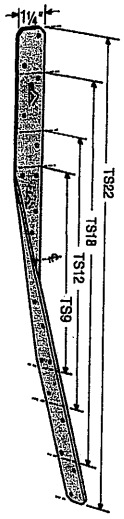
Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3"-bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

Material: LTS — 18 gauge; MTS/TS — 16 gauge; HTS — 14 gauge

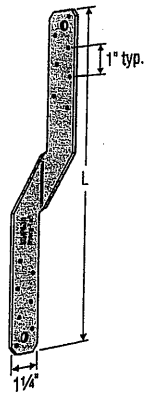
Finish: Galvanized. Some products available in stainless steel and ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

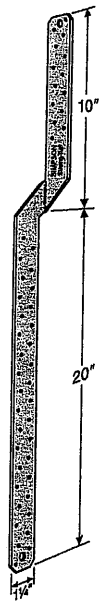
- Use all specified fasteners; see General Notes
- TS should be installed in pairs to reduce eccentricity
- When LTS/MTS is installed as truss-to-top plate tie, install (3) 10d x 1½" nails to the underside of the plate and (3) 10d x 1½" nails into the edge of the double top plate
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after model number, such as MTS16-REV



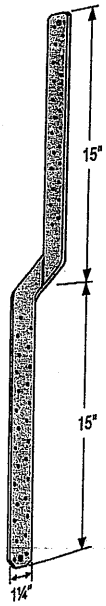
TS



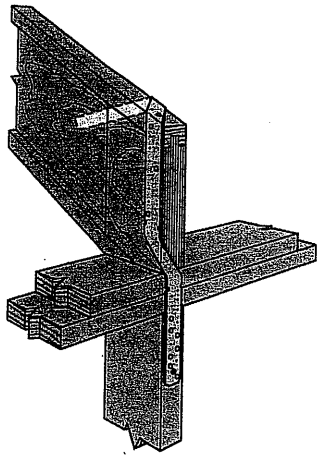
LTS12
(MTS and HTS similar)



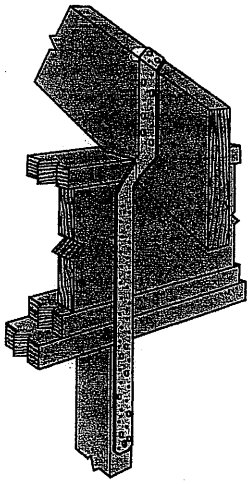
MTS30
(HTS30 similar)



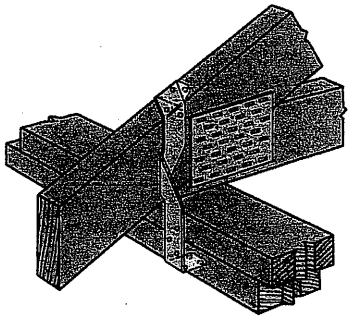
HTS30C
(MTS30C similar)



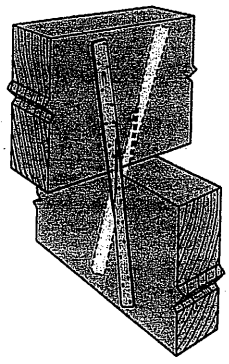
MTS30 Installation with I-Joist Rafter



Typical MTS30 Installation



MTS Installation as a Truss-to-Top Plate Tie



Typical TS Installation

TS/LTS/MTS/HTS

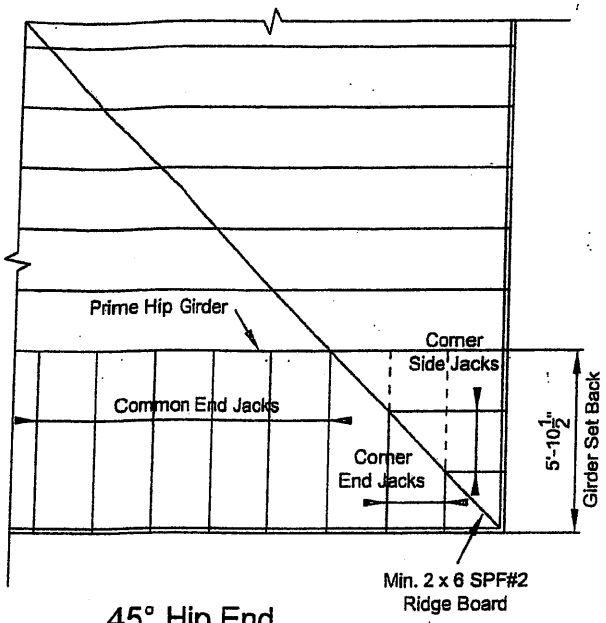
Twist Straps (cont.)

SS These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	L (in.)	Fasteners (Total)	Factored Resistance (K_D = 1.15) D.Fir-L	Factored Resistance (K_D = 1.15) S-P-F
			lb.	lb.
			kN	kN
TS9	9	(8) 16d	1125	1040
			5.00	4.63
TS12	11⅞	(10) 16d	1410	1300
			6.27	5.78
TS18	17¾	(14) 16d	1970	1820
			8.76	8.10
TS22	21⅝	(18) 16d	2125	2125
			9.45	9.45
SS LTS12	12	(12) 10d x 1½"	1015	720
			4.52	3.20
LTS16	16	(12) 10d x 1½"	1015	720
			4.52	3.20
SS LTS18	18	(12) 10d x 1½"	1015	720
			4.52	3.20
LTS20	20	(12) 10d x 1½"	1015	720
			4.52	3.20
MTS12	12	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS16	16	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS18	18	(14) 10d x 1½"	1570	1180
			6.98	5.25
SS MTS20	20	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS30	30	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS24C	24	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS30C	30	(14) 10d x 1½"	1570	1180
			6.98	5.25
HTS16	16	(16) 10d x 1½"	2050	1455
			9.12	6.47
HTS20	20	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS24	24	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS28	28	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS30	30	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS30C	30	(24) 10d x 1½"	2050	1455
			9.12	6.47

1. LTS12 through LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading. No further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided on some straps.
7. When used as a truss-to-top plate tie multiply the tabulated values by 0.95 for LTS and 0.74 for MTS. HTS cannot be used in this application.
8. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.



45° Hip End

LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

LICENSED PROFESSIONAL ENGINEER
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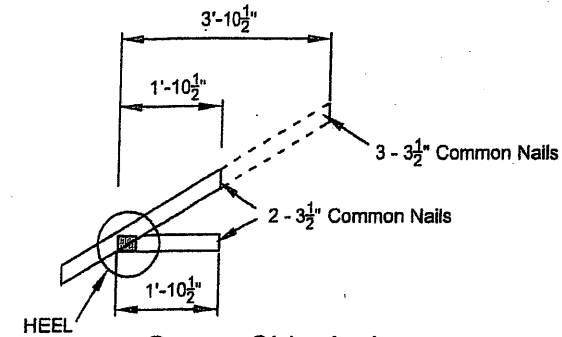
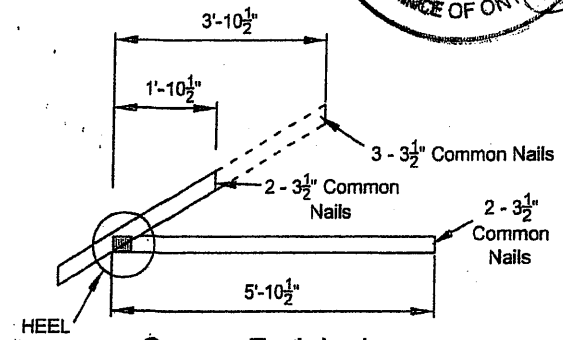
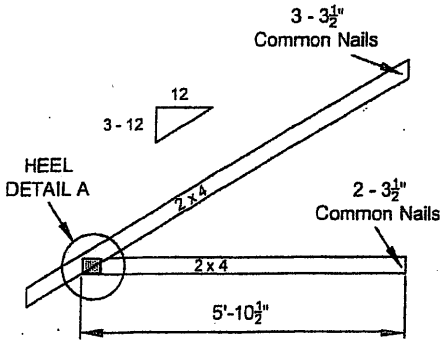


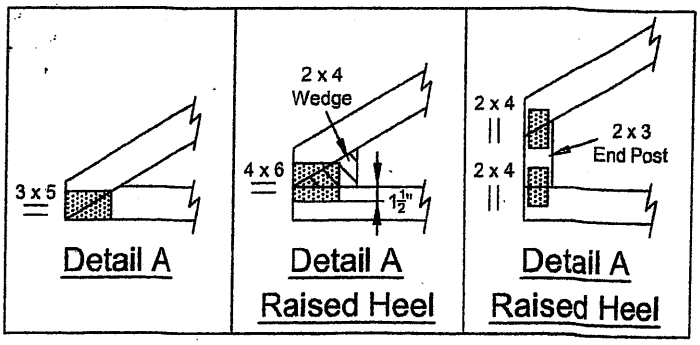
Corner Side Jacks



Corner End Jacks

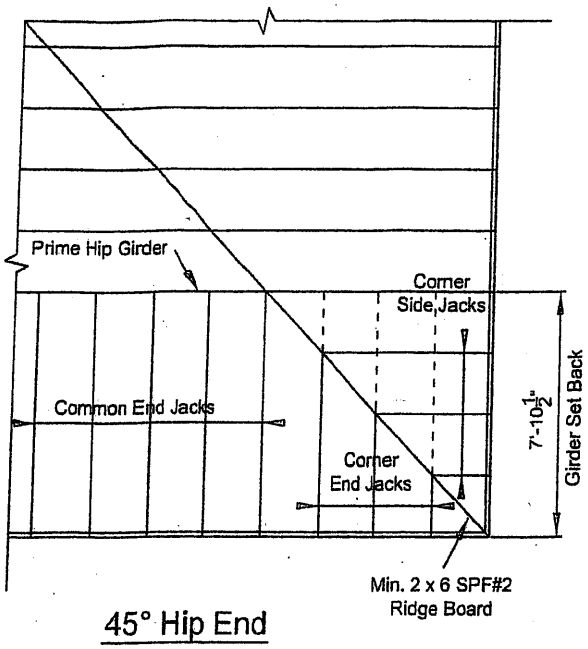


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

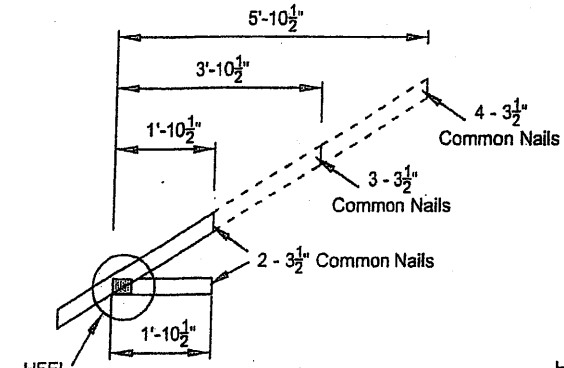
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

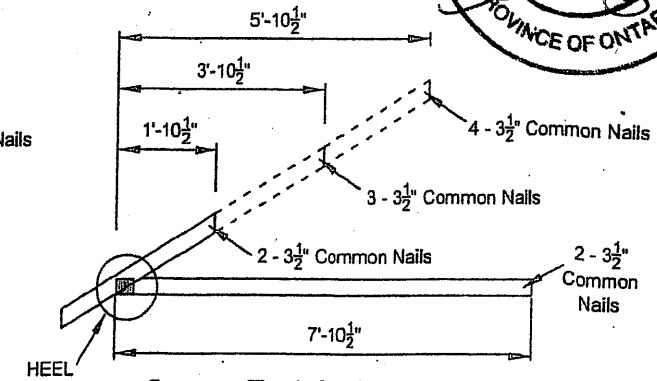
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

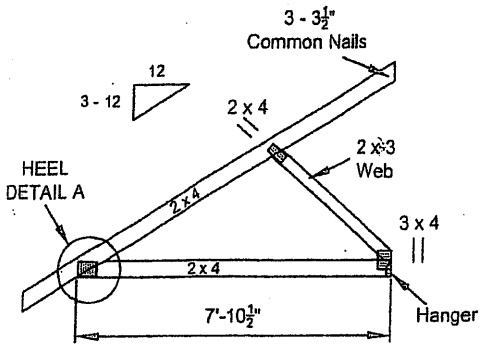
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



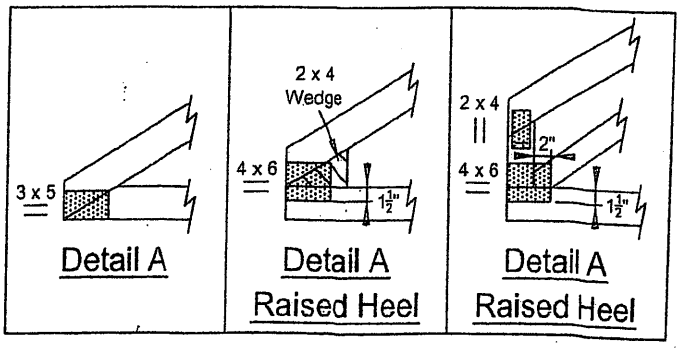
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

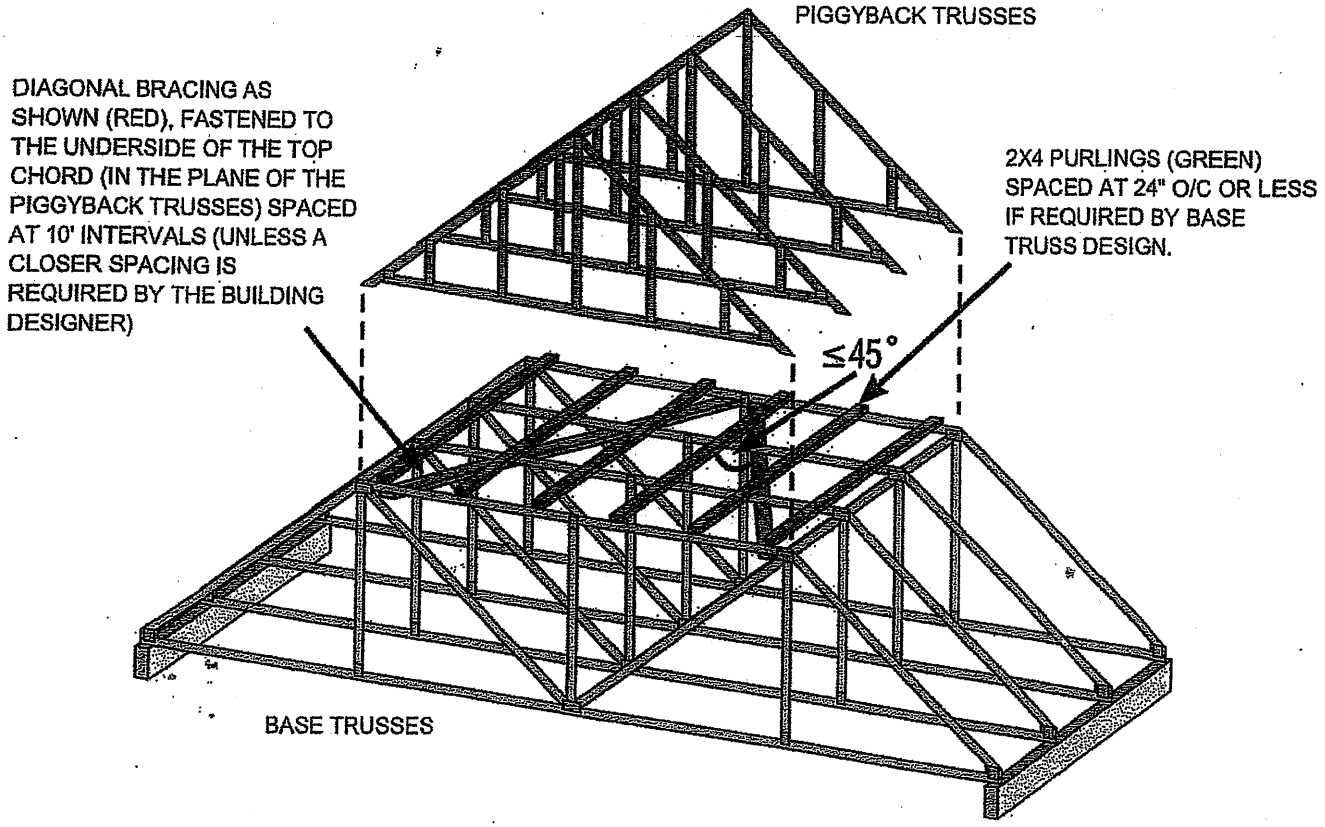
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

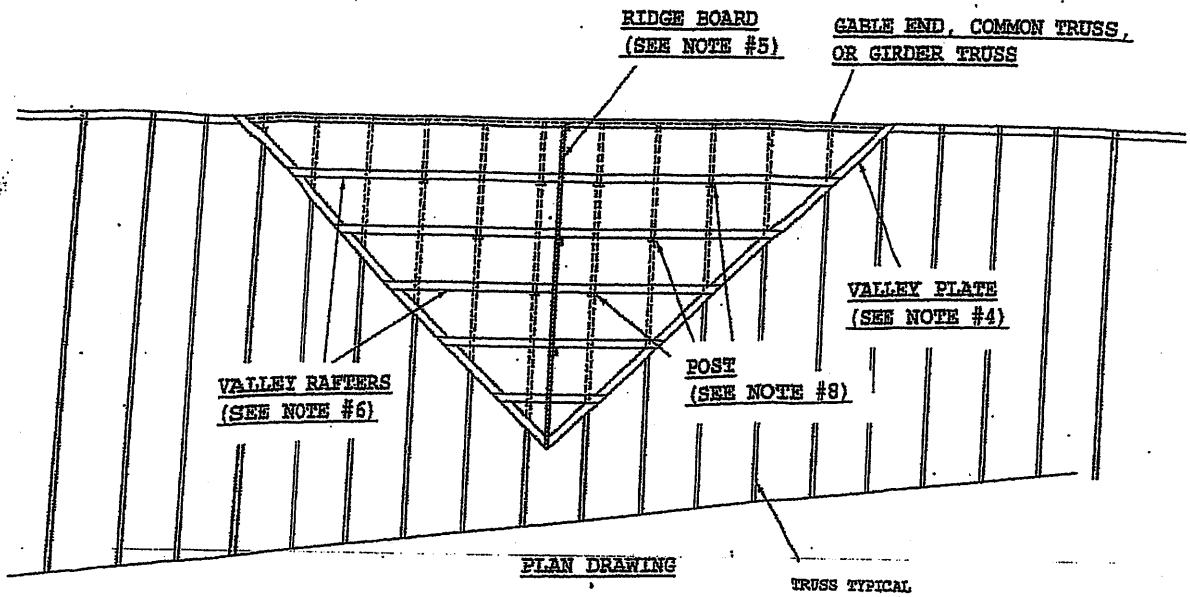


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

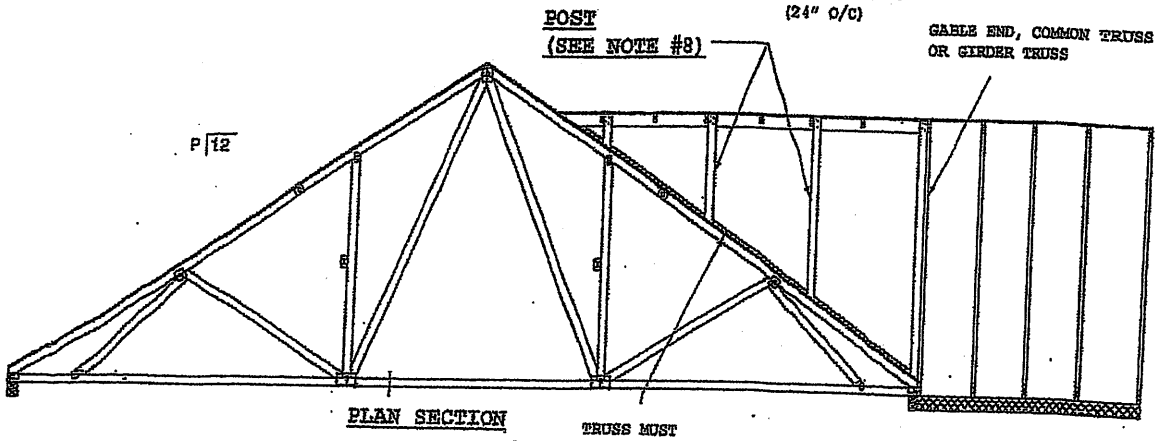
SKETCH FROM BCSI-CANADA 2013

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

TRUSS MUST
BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

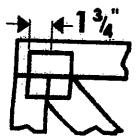
(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-574
S. KATSOULAKOS
PROVINCE OF ONTARIO

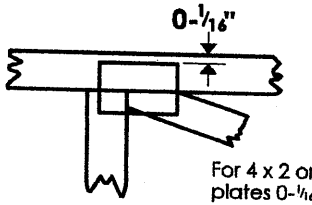
DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

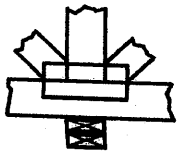
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

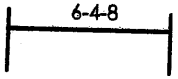
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

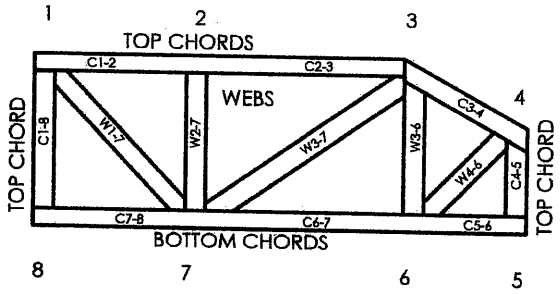
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

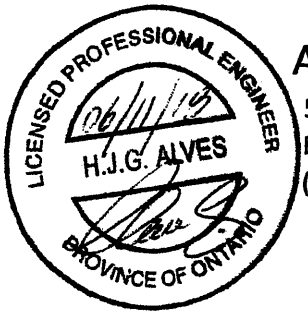
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018