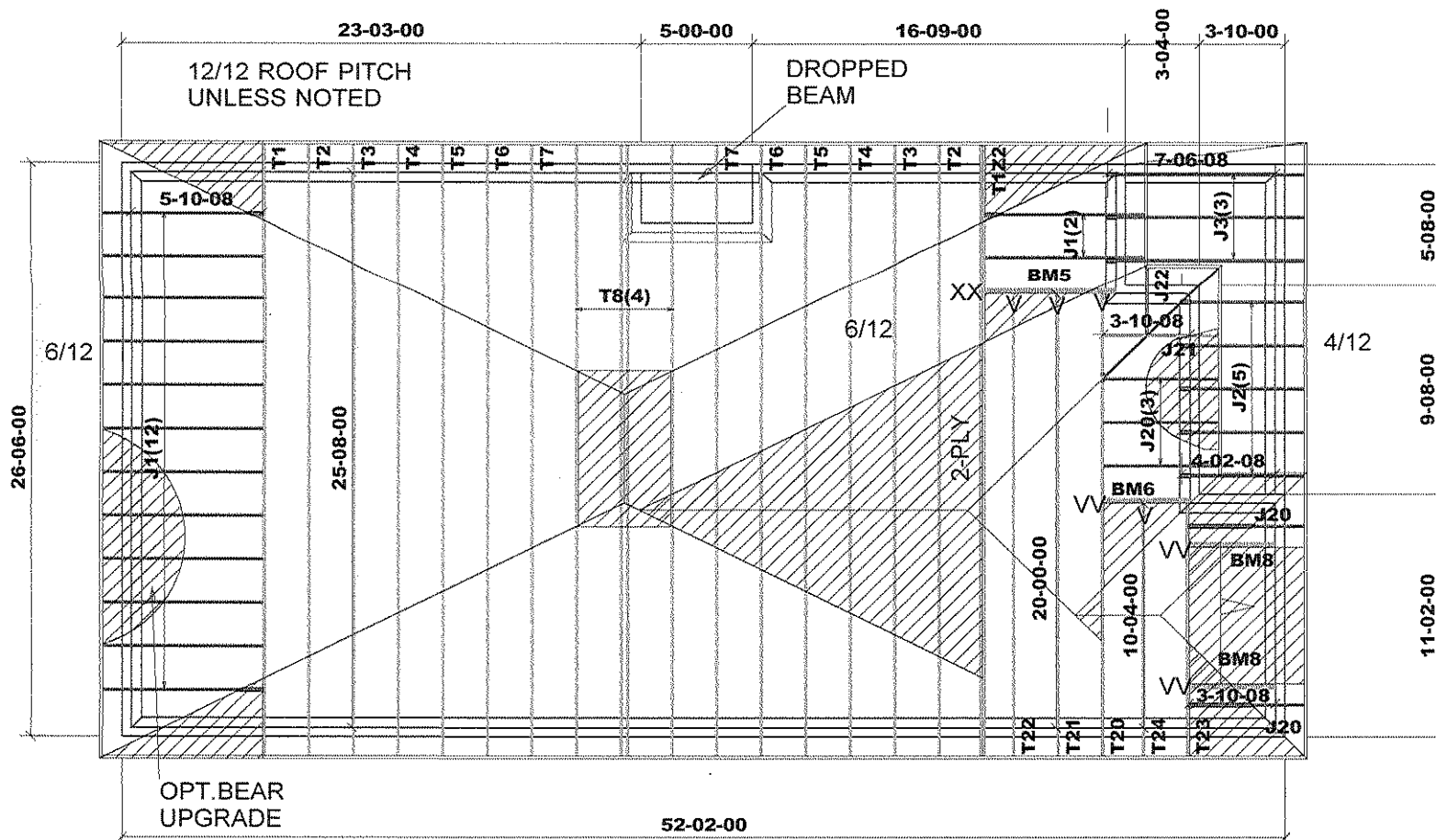


ALL BEAMS(BM):2-2X10

T-160596



12" FINISH O.H  
R.T.M.C  
2X6 EXTERIOR WALLS  
ASPHALT SHINGLES  
2X6 FASCIA BOARD

ALL CONVENTIONAL  
ROOF FRAMING TO  
CONFORM TO PART9 OF  
THE OBC.LATEST EDITION  
ROOF RAFTERS THAT  
MEET OR CROSS OVER  
TRUSSES ARE TO BE  
2"X4" SPF@24"o.c. WITH A  
2"X4"SPF VERTICAL POST  
TO THE TRUSS UNDER AT  
EACH CROSS POINT.  
POSTS LONGER THAN 6'  
TO BE Laterally  
BRACED SO THAT THE  
DISTANCE BETWEEN END  
POINTS AND BETWEEN  
ROWS OF BRACING DOES  
NOT EXCEED 6'.

DESIGN CONFORMS WITH  
THE RELEVANT SECTION  
OF THE LATEST EDITION  
OF O.B.C. PART.9

DESIGN LOADS:  
GROUND SNOW LOAD  
Ss= 2.6 kPa  
TC DEAD 3 PSF  
BC LIVE 10.5 PSF  
BC DEAD 7 PSF

DENOTES  
CONVENTIONAL  
FRAMING

HARDWARE  
LJS26DS(V)  
HGUS26-2(XX)  
LUS26-2(VV)

FINAL PLAN CHECKED

09/11/2017 JANE

M9784

Job Track: 42067

Layout ID: 272443

Plan Log: 87565

Builder / Location:

BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 8/25/2016

Designer: JG

Model / Elevation:

S32-9-15G / A

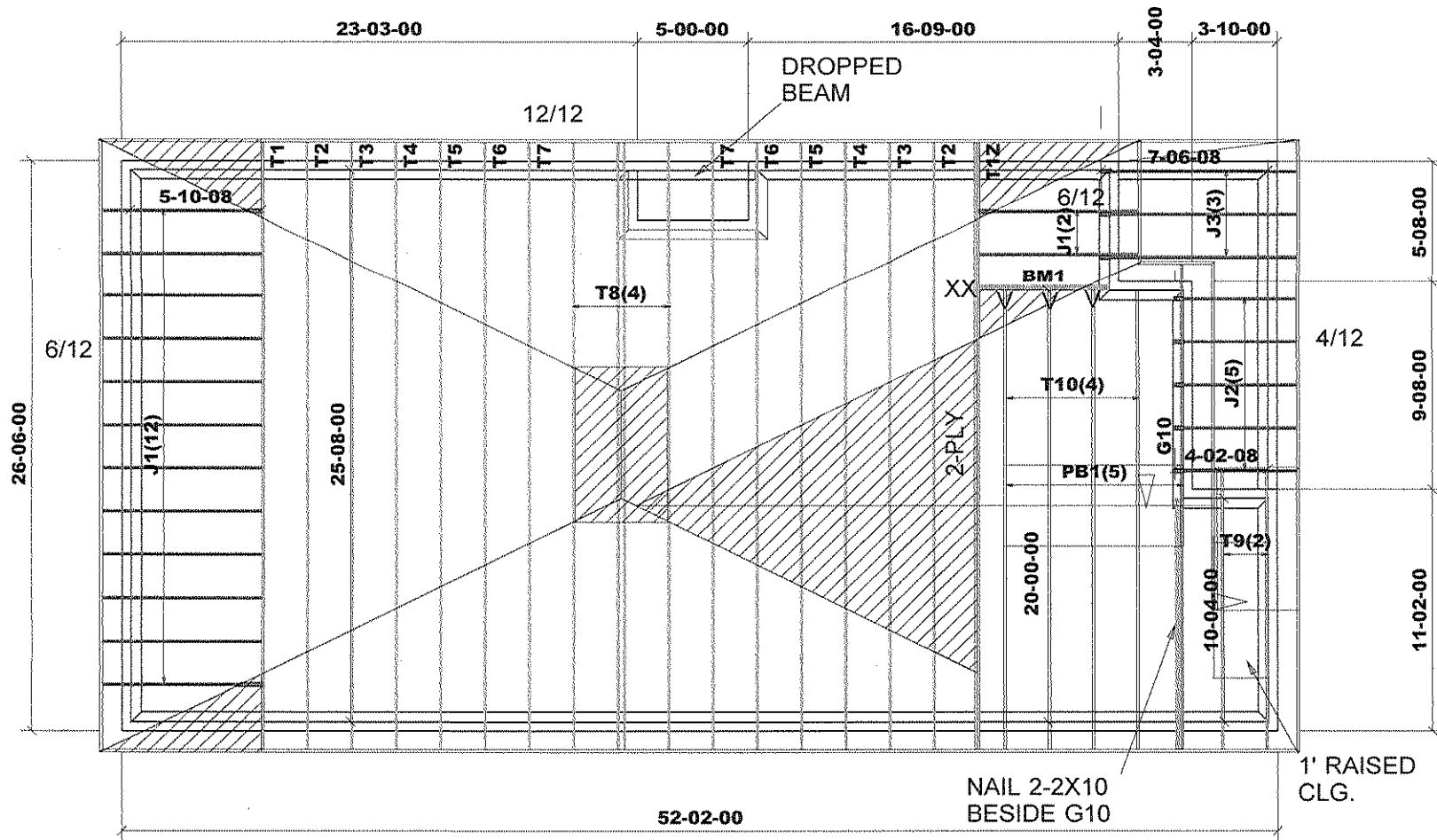
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Mitek ver 7.5.0

Town of Innisfil Certified Model

12/03/2018 9:05:00 AM kgervais

T-160278



12" FINISH O.H  
R.T.M.C  
2X6 EXTERIOR WALLS  
ASPHALT SHINGLES  
2X6 FASCIA BOARD

ALL CONVENTIONAL  
ROOF FRAMING TO  
CONFORM TO PART9 OF  
THE OBC.LATEST EDITION  
ROOF RAFTERS THAT  
MEET OR CROSS OVER  
TRUSSES ARE TO BE  
2"X4" SPF@24"o.c. WITH A  
2"X4"SPF VERTICAL POST  
TO THE TRUSS UNDER AT  
EACH CROSS POINT.  
POSTS LONGER THAN 6'  
TO BE Laterally  
BRACED SO THAT THE  
DISTANCE BETWEEN END  
POINTS AND BETWEEN  
ROWS OF BRACING DOES  
NOT EXCEED 6'.

DESIGN CONFORMS WITH  
THE RELEVANT SECTION  
OF THE LATEST EDITION  
OF O.B.C. PART.9

DESIGN LOADS:  
GROUND SNOW LOAD  
Ss= 2.6 kPa  
TC DEAD 3 PSF  
BC LIVE 10.5 PSF  
BC DEAD 7 PSF

DENOTES  
CONVENTIONAL  
FRAMING

HARDWARE  
LJS26DS(V)  
HGUS26-2(XX)

BM1: 2-2X10

FINAL PLAN CHECKED  
09/11/2017 JAVE

M9784



Job Track: **42067**  
Layout ID: **266137**  
Plan Log: **85812**

Builder / Location:

**BAYVIEW WELLINGTON / INNISFIL**

Project: **ALCONA SHORES**

Date: **8/25/2016** Designer: **JG**

Model / Elevation:

**S32-9-15G / B**

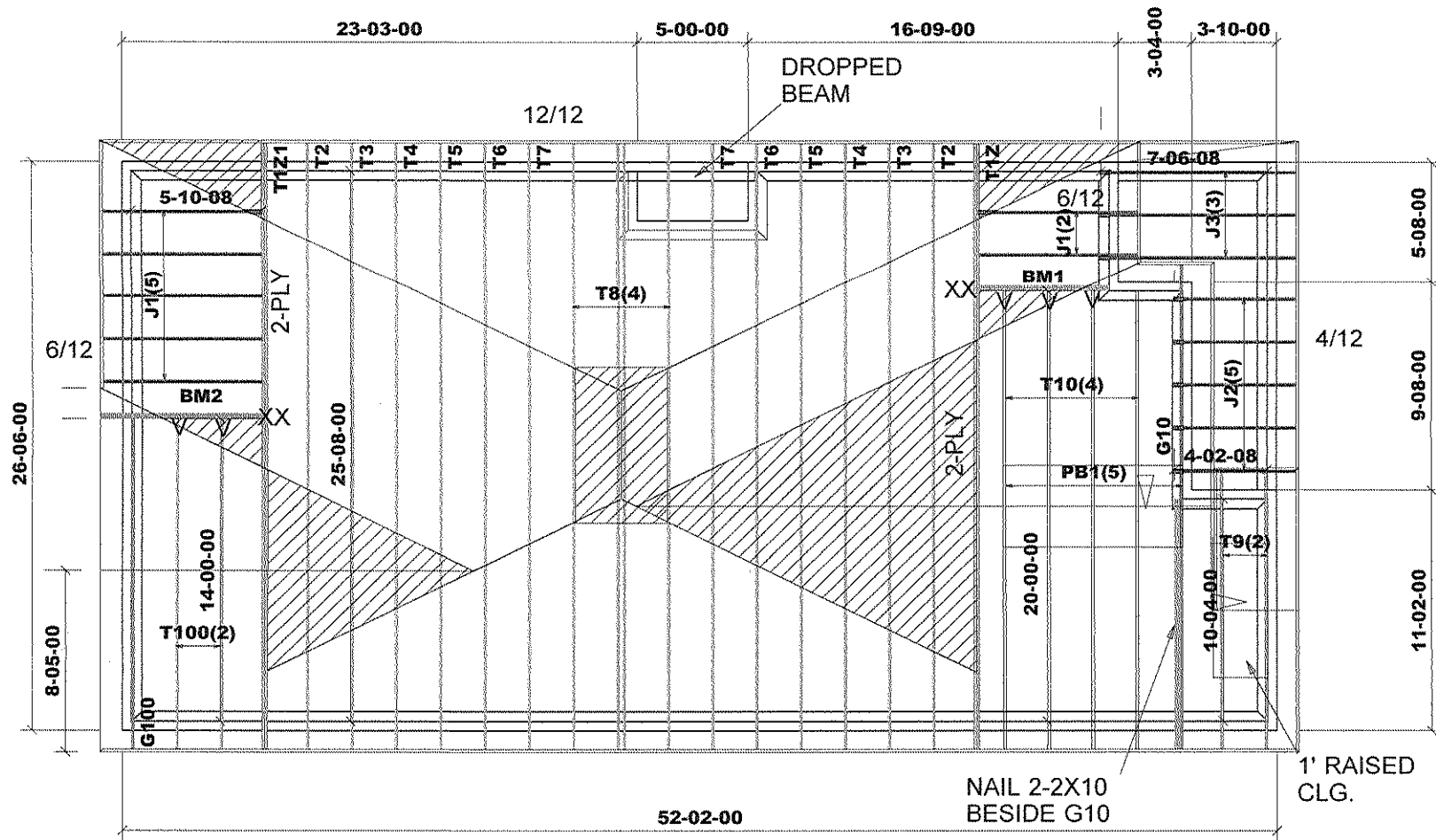
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Mitek ver 7.5.0

Town of Innisfil Certified Model

12/03/2018 9:05:07 AM kgervais

T-160278



12" FINISH O.H.  
R.T.M.C  
2X6 EXTERIOR WALLS  
ASPHALT SHINGLES  
2X6 FASCIA BOARD

ALL CONVENTIONAL  
ROOF FRAMING TO  
CONFORM TO PART9 OF  
THE OBC.LATEST EDITION  
ROOF RAFTERS THAT  
MEET OR CROSS OVER  
TRUSSES ARE TO BE  
2"X4" SPF@24"o.c. WITH A  
2"X4"SPF VERTICAL POST  
TO THE TRUSS UNDER AT  
EACH CROSS POINT.  
POSTS LONGER THAN 6'  
TO BE LATERALLY  
BRACED SO THAT THE  
DISTANCE BETWEEN END  
POINTS AND BETWEEN  
ROWS OF BRACING DOES  
NOT EXCEED 6'.

DESIGN CONFORMS WITH  
THE RELEVANT SECTION  
OF THE LATEST EDITION  
OF O.B.C. PART.9

DESIGN LOADS:  
GROUND SNOW LOAD  
Ss= 2.6 kPa  
TC DEAD 3 PSF  
BC LIVE 10.5 PSF  
BC DEAD 7 PSF

DENOTES  
CONVENTIONAL  
FRAMING

HARDWARE  
LJS26DS(V)  
HGUS26-2(XX)

BM1,2 : 2-2X10

FINAL PLAN CHECKED

09/11/2017 JANE

M9784



Job Track: 42067

Layout ID: 272442

Plan Log: 87565

Builder / Location:

BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 8/25/2016

Designer: JG

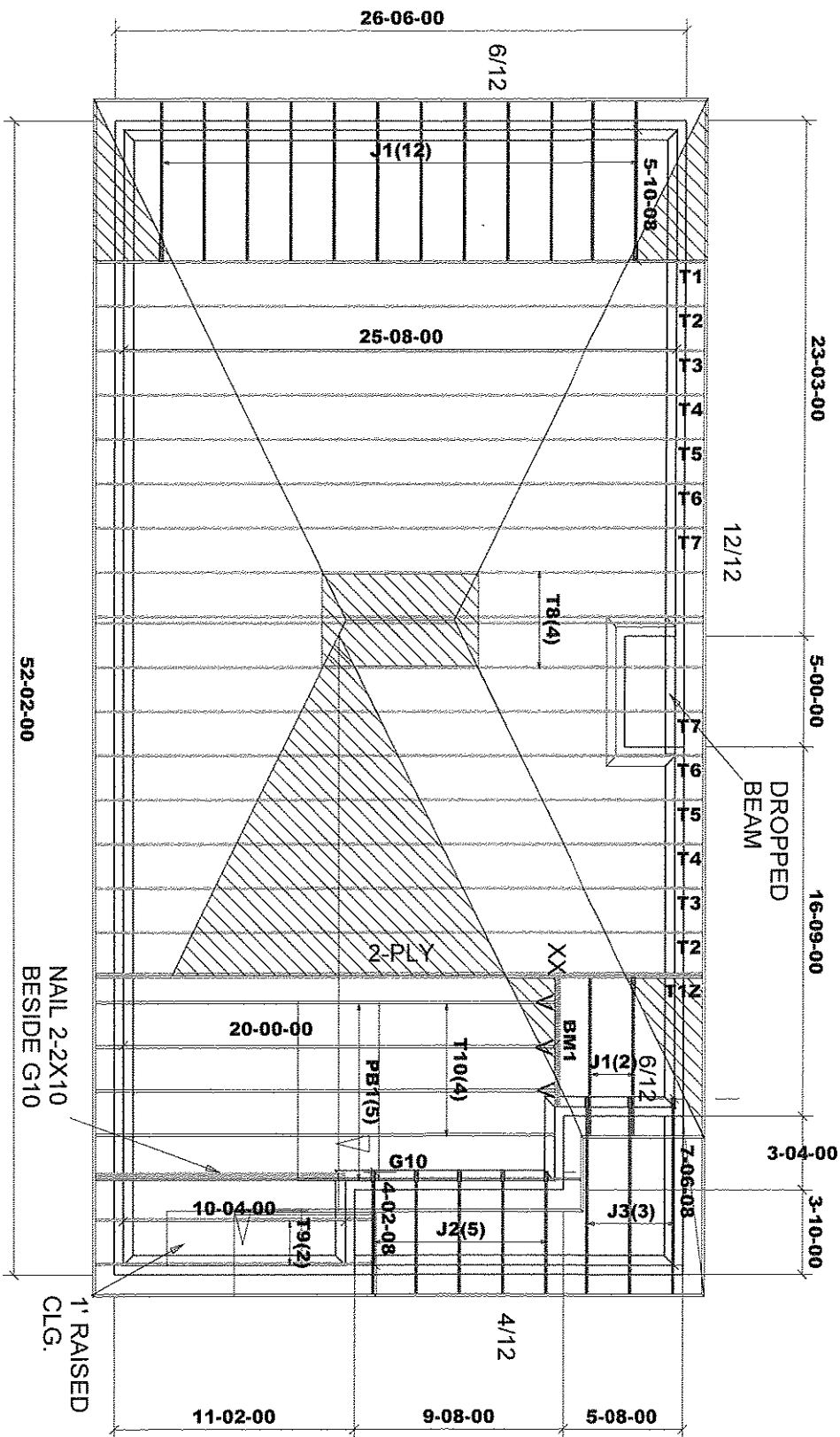
Model / Elevation:

S32-9-15G / B-REAR UPGRADE

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

T-160278



12" FINISH O.H  
R.T.M.C  
2X6 EXTERIOR WALLS  
ASPHALT SHINGLES  
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC, LATEST EDITION. ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4" SPF@24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH  
THE RELEVANT SECTION  
OF THE LATEST EDITION  
OF O.B.C. PART.9


DESIGN LOADS:  
GROUND SNOW LOAD  
Ss = 2.6 kPa  
TC DEAD 3 PSF  
BC LIVE 10.5 PSF  
BC DEAD 7 PSF

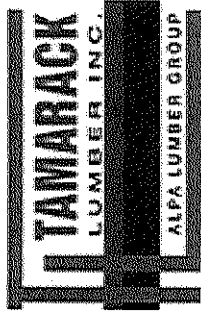
DENOTES  
 CONVENTIONAL  
 FRAMING



HARDWARE  
LJS26DS(V)  
HGUS26-2(XX)

BM1: 2-2X10

		<b>Job Track: 42067</b>		<b>Builder / Location:</b>	
<b>Layout ID: 266137</b>		<b>BAYVIEW WELLINGTON / INNISFIL</b>		<b>S32-9 LOT72 / B</b>	
<b>Plan Log: 85812</b>		<b>Project: ALCONA SHORES</b>			
<b>Date: 6/1/2016</b>		<b>Designer: JG</b>			
<p>THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC.. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.</p>					
<p>Mitek ver 7.5.0</p>					



## Delivery Shiplist

DATE	08/25/16
SALES REP	Mario

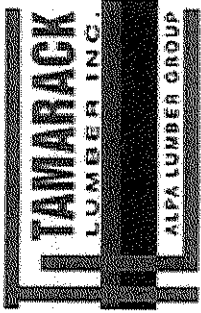
JOB TRACK: 42067	LAYOUT ID: 272443	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-9-15G	ELEVATION: A	

### ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT						
	1	T1 HIP GIRDER	12.00 0.00	25-08-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		147.02 91.67		
	1	T1Z2 HIP GIRDER	12.00 0.00	25-08-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		294.04 183.34		
	2	T2 HIP	12.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		216.88 137.34		
	2	T3 HIP	12.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		229.00 146.00		
	2	T4 HIP	12.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		255.08 163.00		
	2	T5 HIP	12.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		270.36 171.00		
	2	T6 HIP	12.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		273.24 173.00		
	2	T7 HIP	12.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		296.38 186.00		
	4	T8 HIP	12.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		615.64 382.68		
	1	T20 HIP GIRDER	12.00 0.00	20-00-00	05-09-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		101.01 62.50		
	1	T21 HIP	12.00 0.00	20-00-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		106.40 69.33		
	1	T22 HIP	12.00 0.00	20-00-00	09-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		105.10 67.67		
	1	T23 HIP GIRDER	12.00 0.00	10-04-00	05-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		54.68 35.67		
	1	T24 COMMON	12.00 0.00	10-04-00	07-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		49.91 31.83		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04		235.06 149.38		
	5	J2 JACK-OPEN	4.00 0.00	04-02-08	01-08-12	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-08-12		57.60 36.65		
	3	J3 JACK-OPEN	4.00 0.00	07-06-08	02-10-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-10-02		68.16 43.50		
	5	J20 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00		89.50 56.65		





Delivery Shiplist

DATE	08/25/16
SALES REP	Mario

JOB TRACK:42067	LAYOUT ID: 272443	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-9-15G	ELEVATION: A	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	J21 JACK-OPEN	12.00 0.00	03-10-08	03-07-15	2 X 4 2 X 4	01-03-08 -02-01-01	01-10-08 00-03-08	14.54 9.33		
	1	J22 JACK-OPEN	12.00 0.00	01-10-08	03-07-15	2 X 4 2 X 4	01-03-08 -00-01-01	01-10-08 00-03-08	10.86 7.50		

TOTAL # TRUSS= 53.00

TOTAL BFT OF ALL TRUSSES=

2204.04 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3490.46 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
3	Hangers	LUS26-2	

TOTAL # ITEMS= 8.00



## Delivery Shiplist

DATE	06/01/16
SALES REP	Mario

JOB TRACK:42067	LAYOUT ID: 266137	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-9 LOT 72	ELEVATION: B	

### ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O. C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T1 HIP GIRDER	12.00 0.00	25-08-00	04-01-04	2 X 6 2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	147.02 91.67		
	1 2 Ply	T1Z HIP GIRDER	12.00 0.00	25-08-00	04-01-04	2 X 6 2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	294.04 183.34		
	2	T2 HIP	12.00 0.00	25-08-00	05-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	216.88 137.34		
	2	T3 HIP	12.00 0.00	25-08-00	06-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	229.00 146.00		
	2	T4 HIP	12.00 0.00	25-08-00	07-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	255.08 163.00		
	2	T5 HIP	12.00 0.00	25-08-00	08-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	270.36 171.00		
	2	T6 HIP	12.00 0.00	25-08-00	09-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	273.24 173.00		
	2	T7 HIP	12.00 0.00	25-08-00	10-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	296.38 186.00		
	4	T8 HIP	12.00 0.00	25-08-00	11-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	615.64 382.68		
	2	T9 ROOF	12.00 0.00	10-04-00	07-00-08	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	121.16 83.34		
	4	T10 PIGGYBACK	12.00 0.00	20-00-00	10-00-00	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	445.16 284.00		
	1	G10 GABLE	12.00 0.00	20-00-00	10-00-00	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	115.19 74.00		
	5	PB1 PIGGYBACK	12.00 0.00	02-07-10	01-09-00	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	45.25 30.00		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	5	J2 JACK-OPEN	4.00 0.00	04-02-08	01-08-12	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 01-08-12	57.60 36.65		
	3	J3 JACK-OPEN	4.00 0.00	07-06-08	02-10-02	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 02-10-02	68.16 43.50		

TOTAL # TRUSS= 53.00

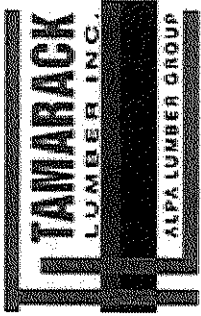
TOTAL BFT OF ALL TRUSSES=

2334.90 BFT.

TOTAL WEIGHT OF ALL TRUSSES= 3685.22 LBS.

### HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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**HARDWARE**

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

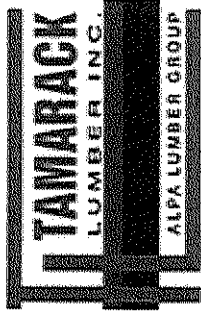
TOTAL # ITEMS= 4.00

**Delivery Shiplist**

JOB TRACK: 42067	LAYOUT ID: 266137	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-9 LOT 72	ELEVATION: B	

DATE	06/01/16
SALES REP	Mario





## Delivery Shiplist

DATE	08/25/16
SALES REP	Mario

JOB TRACK: 42067	LAYOUT ID: 272442	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-9-15G	ELEVATION: B-REAR	

### ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

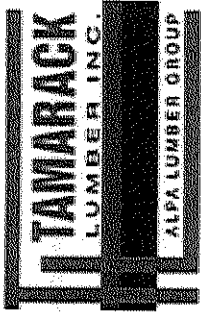
PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1Z HIP GIRDER	12.00 0.00	25-08-00	04-01-04	2 X 6 2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	294.04 183.34		
	1 2 Ply	T1Z1 HIP GIRDER	12.00 0.00	25-08-00	04-01-04	2 X 6 2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	294.04 183.34		
	2	T2 HIP	12.00 0.00	25-08-00	05-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	216.88 137.34		
	2	T3 HIP	12.00 0.00	25-08-00	06-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	229.00 146.00		
	2	T4 HIP	12.00 0.00	25-08-00	07-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	255.08 163.00		
	2	T5 HIP	12.00 0.00	25-08-00	08-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	270.36 171.00		
	2	T6 HIP	12.00 0.00	25-08-00	09-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	273.24 173.00		
	2	T7 HIP	12.00 0.00	25-08-00	10-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	296.38 186.00		
	4	T8 HIP	12.00 0.00	25-08-00	11-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	615.64 382.68		
	2	T9 ROOF	12.00 0.00	10-04-00	07-00-08	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	121.16 83.34		
	4	T10 PIGGYBACK	12.00 0.00	20-00-00	10-00-00	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	445.16 284.00		
	1	G10 GABLE	12.00 0.00	20-00-00	10-00-00	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	115.19 74.00		
	2	T100 COMMON	12.00 0.00	14-00-00	08-10-08	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	144.28 91.66		
	1	G100 COMMON	12.00 0.00	14-00-00	08-10-08	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	74.26 48.83		
	5	PB1 PIGGYBACK	12.00 0.00	02-07-10	01-09-00	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	45.25 30.00		
	7	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	117.53 74.69		
	5	J2 JACK-OPEN	4.00 0.00	04-02-08	01-08-12	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 01-08-12	57.60 36.65		
	3	J3 JACK-OPEN	4.00 0.00	07-06-08	02-10-02	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 02-10-02	68.16 43.50		

TOTAL # TRUSS= 50.00

TOTAL BFT OF ALL TRUSSES=

2492.37 BFT.

TOTAL WEIGHT OF ALL TRUSSES= 3933.25 LBS.



Delivery Shiplist

DATE	08/25/16
SALES REP	Mario

JOB TRACK: 42067	LAYOUT ID: 272442	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-9-15G	ELEVATION: B-REAR	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00

[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
266137	T1	1	1	TRUSS DESC.		

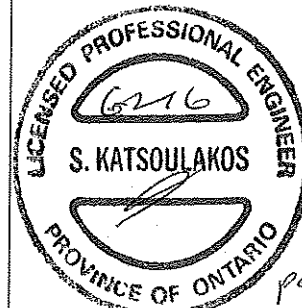
Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MiTek Industries, Inc. Thu Jun 02 08:37:20 2016 Page 2

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-321	-321	---	FRONT	VERT	TOTAL
F	15-11-4	-147	-147	---	FRONT	VERT	TOTAL
G	19-11-4	-147	-147	---	FRONT	VERT	TOTAL
H	23-5-4	-171	-171	---	FRONT	VERT	TOTAL
M	19-11-4	-40	-70	---	FRONT	VERT	TOTAL
N	15-11-4	-40	-70	---	FRONT	VERT	TOTAL
S	3-11-4	-147	-147	---	FRONT	VERT	TOTAL
T	5-11-4	-147	-147	---	FRONT	VERT	TOTAL
U	7-11-4	-147	-147	---	FRONT	VERT	TOTAL
V	9-11-4	-147	-147	---	FRONT	VERT	TOTAL
W	11-11-4	-147	-147	---	FRONT	VERT	TOTAL
X	13-11-4	-147	-147	---	FRONT	VERT	TOTAL
Y	17-11-4	-147	-147	---	FRONT	VERT	TOTAL
Z	21-11-4	-147	-147	---	FRONT	VERT	TOTAL
AA	1-11-4	-40	-70	---	FRONT	VERT	TOTAL
AB	3-11-4	-40	-70	---	FRONT	VERT	TOTAL
AC	5-11-4	-40	-70	---	FRONT	VERT	TOTAL
AD	7-11-4	-40	-70	---	FRONT	VERT	TOTAL
AE	9-11-4	-40	-70	---	FRONT	VERT	TOTAL
AF	11-11-4	-40	-70	---	FRONT	VERT	TOTAL
AG	13-11-4	-40	-70	---	FRONT	VERT	TOTAL
AH	17-11-4	-40	-70	---	FRONT	VERT	TOTAL
AI	21-11-4	-40	-70	---	FRONT	VERT	TOTAL
AJ	23-11-4	-40	-70	---	FRONT	VERT	TOTAL



DWG NO. TAM 25714-16  
STRUCTURAL  
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
266137	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

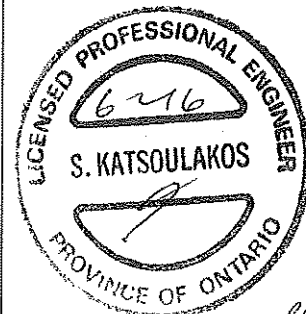
Version 8.000 S Jan 15 2016 Mittek Industries, Inc. Thu Jun 02 08:37:20 2016 Page 2  
ID:tdHlu4 B52F4VdpmVL2iFzAqfi-RiCq9Y7JDt4H2eUxQYznjqHumq NRXXxxZGI9zAOFz

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
R	BMV1+p	MT20	3.0	6.0		

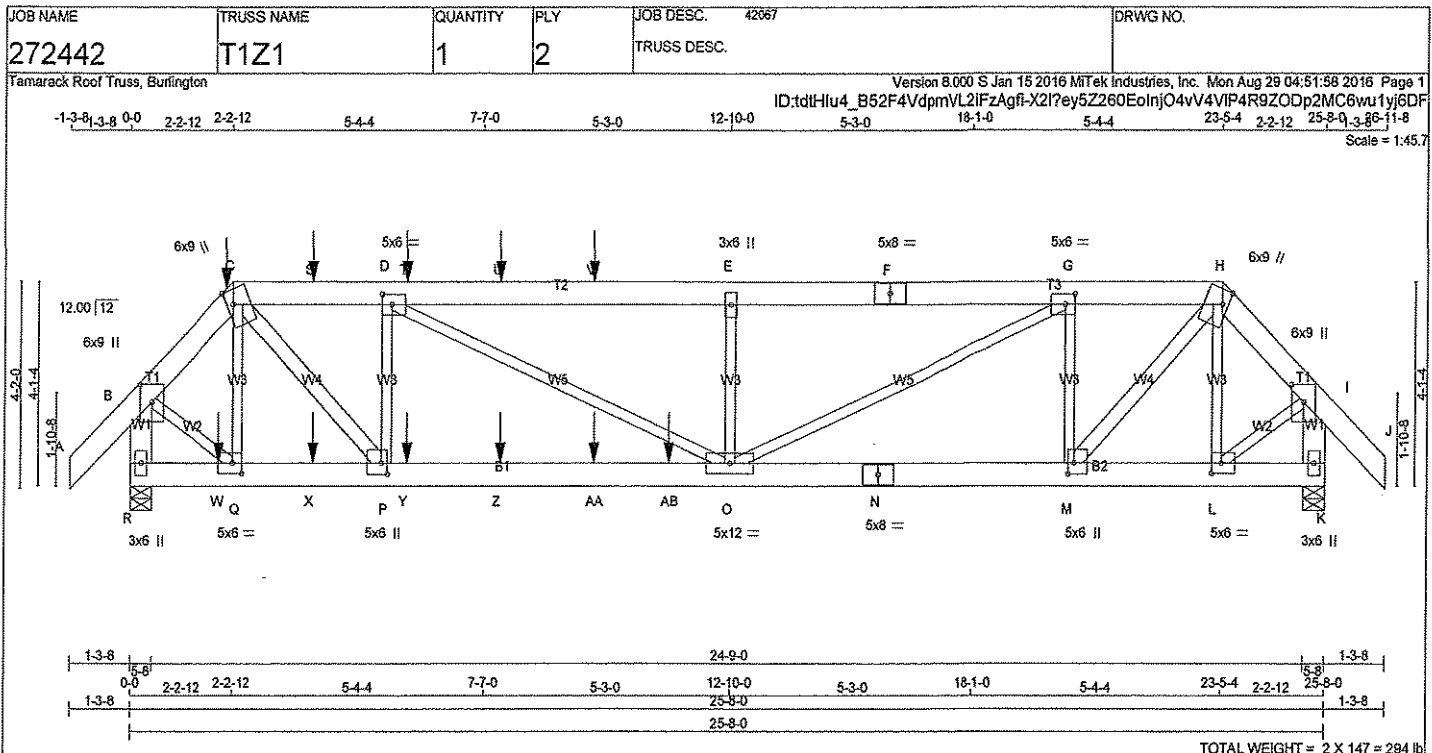
**HANGERS NOTES**

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 171.3 lbs FACTORED DOWN AT 2-2-12, AND 149.8 lbs FACTORED DOWN AT 2-2-12, AND 147.1 lbs FACTORED DOWN AT 3-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, AND 69.9 lbs FACTORED DOWN AT 3-11-4, AND 2763.9 lbs FACTORED DOWN AT 5-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 25715-18  
STRUCTURAL  
COMPONENT ONLY





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
R - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
R - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
C - P	2x4	DRY	No.2
M - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-J	2	12
R-B	2	12
K-I	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
R-N	2	12
N-K	2	12
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	1	6
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PILES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0	4.25	3.00
C	TTVW+m	MT20	6.0	9.0	3.50	1.50
D	TMVW+V	MT20	6.0	6.0	2.50	2.50
E	TMVW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW+V	MT20	5.0	6.0	2.50	2.50
H	TTVW+m	MT20	6.0	9.0	3.50	1.50
I	TMVW+p	MT20	6.0	9.0	4.25	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMVW+V	MT20	5.0	6.0	2.50	2.50
M	BMVW+V	MT20	5.0	6.0	2.75	1.50
N	BS-t	MT20	5.0	6.0		
O	BMVW+V	MT20	5.0	6.0	12.0	
P	BMVW+V	MT20	5.0	6.0	2.75	1.50
Q	BMVW+V	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
R	3945	0	0	5-8
K	3089	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	3053	2043 / 0	508 / 0	0 / 0	0 / 0	501 / 0	0 / 0
K	2379	1612 / 0	384 / 0	0 / 0	0 / 0	383 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 63	-122.2 -122.2	0.05 (1)	Q-C	-1405 / 0	0.17 (1)	
B-C	-3205 / 0	-122.2 -122.2	0.04 (1)	C-P	0 / 4118	0.36 (1)	
C-S	-4969 / 0	-122.2 -122.2	0.29 (1)	P-D	-2276 / 0	0.27 (1)	
S-D	-4969 / 0	-122.2 -122.2	0.29 (1)	D-O	0 / 2464	0.30 (1)	
D-T	-7178 / 0	-122.2 -122.2	0.38 (1)	O-E	-1033 / 0	0.12 (1)	
T-U	-7178 / 0	-122.2 -122.2	0.38 (1)	U-G	0 / 3712	0.45 (1)	
U-V	-7178 / 0	-122.2 -122.2	0.38 (1)	M-G	-2359 / 0	0.28 (1)	
V-E	-7178 / 0	-122.2 -122.2	0.38 (1)	E-H	0 / 3158	0.28 (1)	
E-F	-7178 / 0	-122.2 -122.2	0.31 (1)	L-H	-811 / 0	0.10 (1)	
F-G	-7178 / 0	-122.2 -122.2	0.31 (1)	B-Q	0 / 2577	0.32 (1)	
G-H	-3853 / 0	-122.2 -122.2	0.18 (1)	L-I	0 / 2015	0.25 (1)	
H-I	-2506 / 0	-122.2 -122.2	0.04 (1)				
I-J	0 / 63	-122.2 -122.2	0.05 (1)				
R-B	-3874 / 0	0.0	0.0	14 (1)			
K-I	-3089 / 0	0.0	0.0	11 (1)			

FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
W-Q	0 / 0	-28.0	-28.0	0.03 (1)	10.00	
Q-X	0 / 0	-28.0	-28.0	0.03 (1)	10.00	
X-P	0 / 2201	-28.0	-28.0	0.39 (1)	10.00	
P-Y	0 / 4972	-28.0	-28.0	0.39 (1)	10.00	
Y-Z	0 / 4972	-28.0	-28.0	0.79 (1)	10.00	
Z-AA	0 / 4972	-28.0	-28.0	0.79 (1)	10.00	
AA-AB	0 / 4972	-28.0	-28.0	0.79 (1)	10.00	
AB-O	0 / 4972	-28.0	-28.0	0.79 (1)	10.00	
O-N	0 / 3854	-28.0	-28.0	0.49 (1)	10.00	
N-M	0 / 3854	-28.0	-28.0	0.49 (1)	10.00	
M-L	0 / 1730	-28.0	-28.0	0.15 (1)	10.00	
L-K	0 / 0	-28.0	-28.0	0.04 (1)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-377	-377		FRONT	VERT	TOTAL
S	3-11-4	-147	-147		FRONT	VERT	TOTAL
T	5-11-4	-147	-147		FRONT	VERT	TOTAL
U	7-11-4	-147	-147		FRONT	VERT	TOTAL
V	9-11-4	-147	-147		FRONT	VERT	TOTAL
W	1-11-4	-40	-70		FRONT	VERT	TOTAL
X	3-11-4	-40	-70		FRONT	VERT	TOTAL
Y	5-11-4	-40	-70		FRONT	VERT	TOTAL

DESIGN CRITERIA

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*  
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.  
LOADS WERE DERIVED FROM USER INPUT  
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

\*\*\* NON STANDARD GIRDER \*\*\*  
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")  
ALLOWABLE DEFL.(TL) = L/360 (0.86")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.38 (D-E-I), BC=0.79 (O-P-I), WB=0.48 (G-O-I), SSI=0.50 (O-P-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

UTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.77 (G) (INPUT = 0.90)  
JSI METAL=0.73 (N) (INPUT = 1.00)



DWG NO. 39296-18

STRUCTURAL  
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 272442	TRUSS NAME T1Z1	QUANTITY 1	PLY 2	JOB DESC. 42067 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Mon Aug 29 04:51:58 2016 Page 2  
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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 377.4 lbs FACTORED DOWN AT 2-2-12, 147.1 lbs FACTORED DOWN AT 3-11-4, 147.1 lbs FACTORED DOWN AT 5-11-4, AND 147.1 lbs FACTORED DOWN AT 7-11-4, AND 147.1 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, AND 69.9 lbs FACTORED DOWN AT 9-11-4, AND 1658.0 lbs FACTORED DOWN AT 11-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Z	7-11-4	-40	-70	---	FRONT	VERT	TOTAL
AA	9-11-4	-40	-70	---	FRONT	VERT	TOTAL
AB	11-6-8	-1658	-1658	---	FRONT	VERT	TOTAL



9632  
DWG NO. TAM39296-18  
STRUCTURAL  
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272443	T1Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

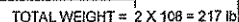
Version 8.000 S Jan 15 2016 MTek Industries, Inc. Thu Aug 25 16:51:00 2016 Page 2  
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# HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 171.3 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



*P622*  
DWG NO. TAM39290-18  
STRUCTURAL  
COMPONENT ONLY



DWG NO. TAM 25716-16  
STRUCTURAL  
COMPONENT ONLY

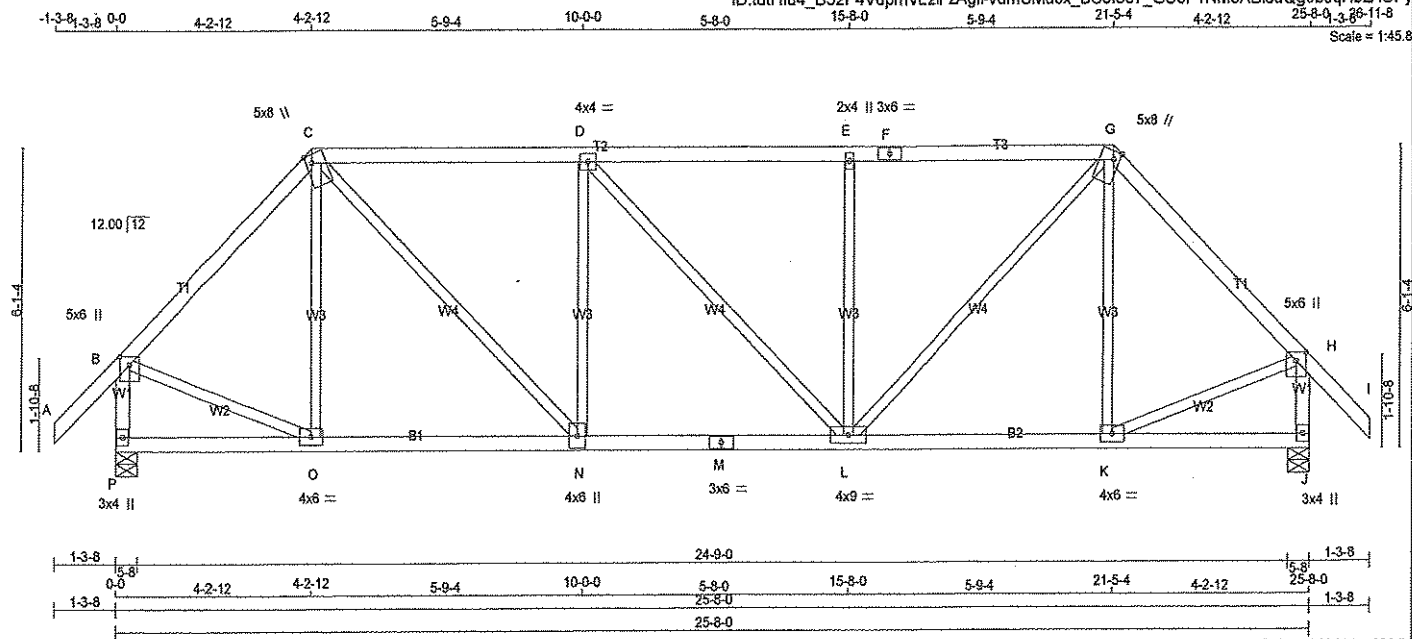
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
266137	T3	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MitTek Industries, Inc. Thu Jun 02 08:37:21 2016 Page 1

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Scale = 1:45.8



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - F	2x4 DRY	No.2	SPF		
F - G	2x4 DRY	No.2	SPF		
G - I	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
P - M	2x4 DRY	No.2	SPF		
M - J	2x4 DRY	No.2	SPF		

ALL WEBS EXCEPT	2x3 DRY	No.2	SPF
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DRY: SEASONED LUMBER.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.50
C	TTVW+m	MT20	5.0	8.0	2.00	1.50
D	TMWV-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TTVW+m	MT20	5.0	8.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	2.00	2.50
J	BMV1+p	MT20	3.0	4.0		
K	BMWV-t	MT20	4.0	6.0		
L	BMWV-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWV-t	MT20	4.0	6.0		
O	BMWV-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
P	2098	0	2098	0	0	5-8	3-5	3-5	
J	2098	0	2098	0	0	5-8	3-5	3-5	

#### UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1623	1088 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1623	1088 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.84 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LBS)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LBS)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	O-C	-263 / 109	0.15 (1)
B-C	-1697 / 0	-122.2	-122.2	0.47 (1)	4.58	C-N	0 / 1291	0.29 (1)
C-D	-2103 / 0	-122.2	-122.2	0.69 (1)	3.84	N-D	-755 / 0	0.44 (1)
D-E	-2100 / 0	-122.2	-122.2	0.68 (1)	3.85	D-L	-4 / 0	0.01 (1)
E-F	-2101 / 0	-122.2	-122.2	0.67 (1)	3.88	L-E	-752 / 0	0.44 (1)
F-G	-2101 / 0	-122.2	-122.2	0.67 (1)	3.88	L-G	0 / 1268	0.29 (1)
G-H	-1698 / 0	-122.2	-122.2	0.47 (1)	4.57	K-G	-262 / 109	0.15 (1)
H-I	0 / 60	-122.2	-122.2	0.17 (1)	10.00	B-O	0 / 1268	0.29 (1)
P-B	-2054 / 0	0.0	0.0	0.23 (1)	5.90	K-H	0 / 1268	0.29 (1)
J-H	-2054 / 0	0.0	0.0	0.23 (1)	5.90			
P-O	0 / 0	-28.0	-28.0	0.17 (2)	10.00			
O-N	0 / 1191	-28.0	-28.0	0.30 (2)	10.00			
N-M	0 / 2103	-28.0	-28.0	0.43 (1)	10.00			
M-L	0 / 2103	-28.0	-28.0	0.43 (1)	10.00			
L-K	0 / 1192	-28.0	-28.0	0.29 (2)	10.00			
K-J	0 / 0	-28.0	-28.0	0.17 (2)	10.00			

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 088-00  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =  $L/360$  (0.86")  
CALCULATED VERT. DEFL.(LL) =  $L/999$  (0.10")  
ALLOWABLE DEFL.(TL) =  $L/360$  (0.86")  
CALCULATED VERT. DEFL.(TL) =  $L/999$  (0.15")

CSI: TC=0.69 (C-D:1), BC=0.43 (L-N:1), WB=0.44 (D-N:1), SSI=0.33 (C-D:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

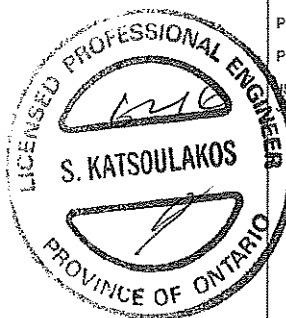
#### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

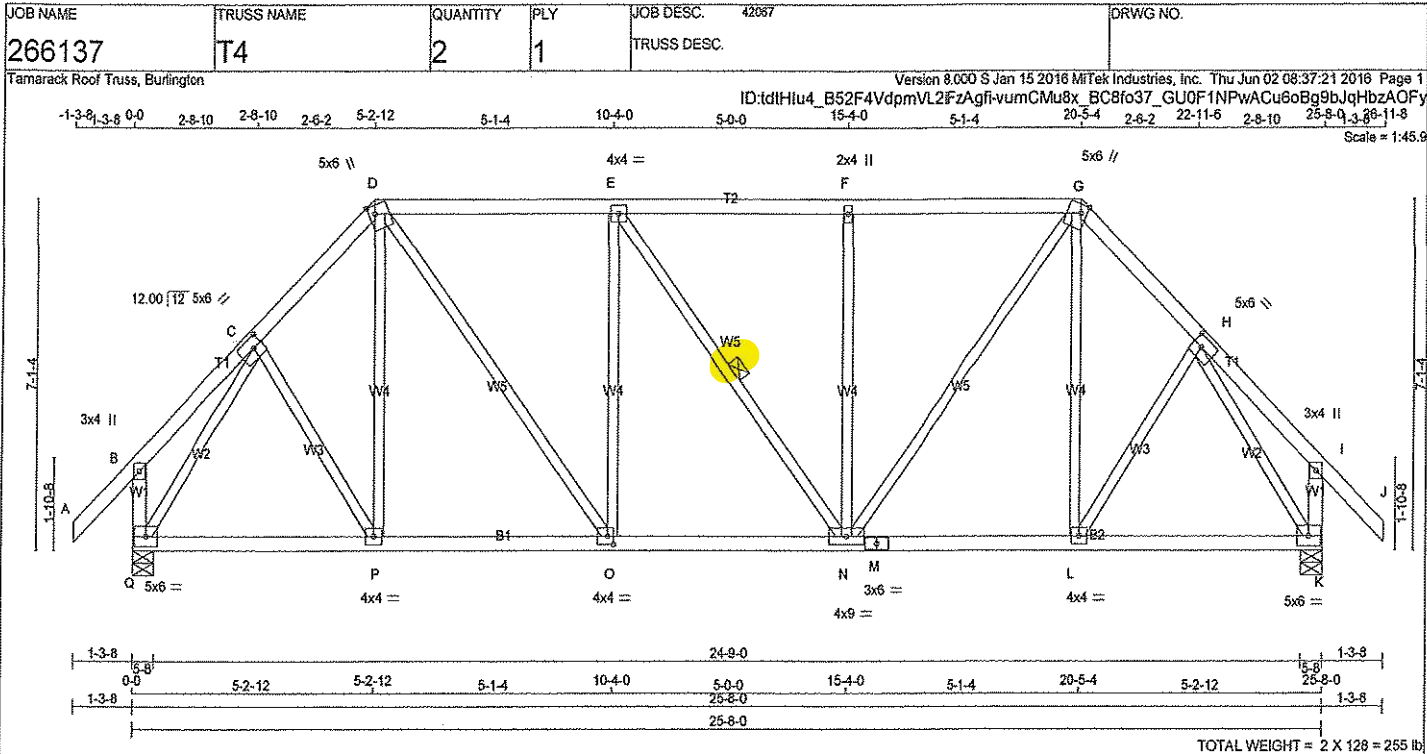
PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.89 (N) (INPUT = 0.90)  
SSI METAL= 0.63 (M) (INPUT = 1.00)



DWG NO. TAM 25717-16  
STRUCTURAL  
COMPONENT ONLY





**LUMBER**

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF
Q - B	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
Q - M	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
Q	2098	0	2098	0	0	5-8	2-4	2-4
K	2098	0	2098	0	0	5-8	2-4	2-4

**UNFACTORED REACTIONS**

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
Q	1623	1088 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
K	1623	1088 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.**

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX	UNBRACED	MAX. FACTORED		MAX	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)			MEMB.	FORCE (LBS)		CSI (LC)
FR-TO		FROM	TO	CSI (LC)	LENGTH	FR-TO		CSI (LC)	
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	C-P	0 / 183	0.04 (1)	
B-C	0 / 22	-122.2	-122.2	0.11 (1)	10.00	P-D	0 / 166	0.04 (3)	
C-D	-1722 / 0	-122.2	-122.2	0.15 (1)	4.94	D-O	0 / 1003	0.23 (1)	
D-E	-1798 / 0	-122.2	-122.2	0.50 (1)	4.38	O-E	-665 / 0	0.58 (1)	
E-F	-1797 / 0	-122.2	-122.2	0.50 (1)	4.38	E-N	-4 / 0	0.00 (1)	
F-G	-1796 / 0	-122.2	-122.2	0.49 (1)	4.41	N-F	-664 / 0	0.58 (1)	
G-H	-1722 / 0	-122.2	-122.2	0.15 (1)	4.94	N-G	0 / 999	0.22 (1)	
H-I	0 / 22	-122.2	-122.2	0.11 (1)	10.00	L-G	0 / 166	0.04 (3)	
I-J	0 / 60	-122.2	-122.2	0.17 (1)	10.00	L-H	0 / 183	0.04 (1)	
Q-B	-297 / 0	0.0	0.0	0.03 (1)	7.81	Q-C	-2058 / 0	0.78 (1)	
K-I	-297 / 0	0.0	0.0	0.03 (1)	7.81	H-K	-2056 / 0	0.78 (1)	
Q-P	0 / 1093	-28.0	-28.0	0.29 (2)	10.00				
P-O	0 / 1166	-28.0	-28.0	0.31 (2)	10.00				
O-N	0 / 1799	-28.0	-28.0	0.36 (1)	10.00				
N-M	0 / 1166	-28.0	-28.0	0.31 (2)	10.00				
M-L	0 / 1166	-28.0	-28.0	0.31 (2)	10.00				
L-K	0 / 1093	-28.0	-28.0	0.29 (2)	10.00				

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 090-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")  
ALLOWABLE DEFL.(TL) = L/360 (0.86")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.50 (D-E-1), BC=0.38 (N-O-1), WB=0.78 (H-K-1), SSI=0.29 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1667 822 2284 1656

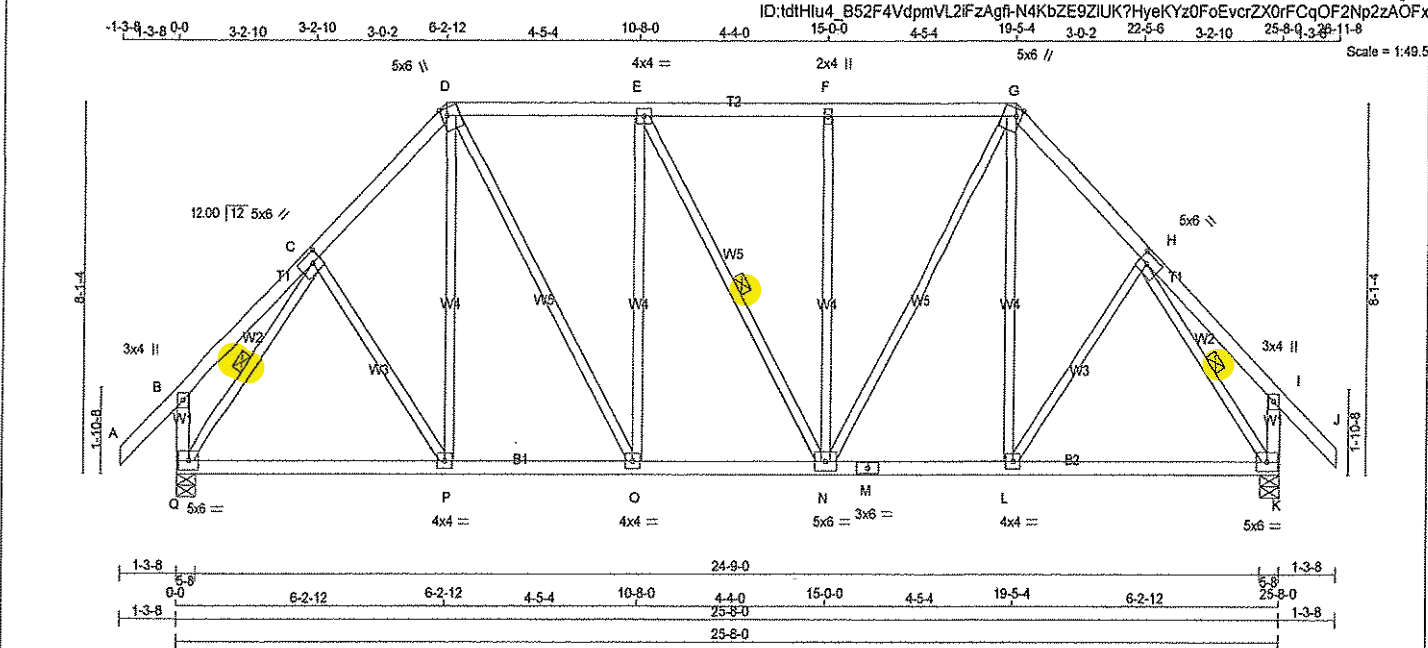
PLATE PLACEMENT TOL. = 0.250 inches

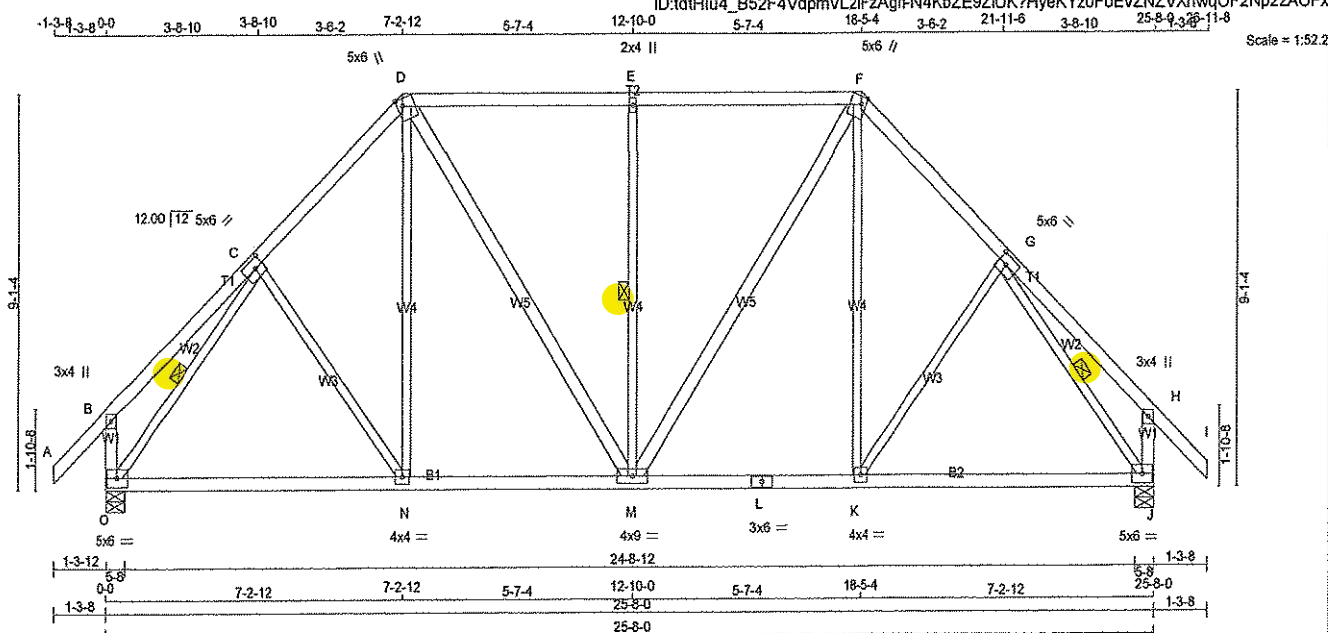
PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (H) (INPUT = 0.90)  
SSI METAL= 0.49 (H) (INPUT = 1.00)



DWG NO. TAM 25718-16  
STRUCTURAL  
COMPONENT ONLY





TOTAL WEIGHT =  $2 \times 137 \approx 273$  lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
J	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	3.0	4.0	
C	TMWW-I	MT20	5.0	6.0	2.50
D	TTWW+m	MT20	5.0	6.0	2.00
E	TMW+y	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	6.0	2.00
G	TMWW-I	MT20	5.0	6.0	2.50
H	TMV+p	MT20	3.0	4.0	
I	BMWW-I	MT20	5.0	6.0	
J	BMWW-I	MT20	4.0	4.0	
L	BS-I	MT20	3.0	6.0	
M	BMWWWW-I	MT20	4.0	9.0	
N	BMWW-I	MT20	4.0	4.0	
O	BMWW-I	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
GROSS REACTION		GROSS REACTION				BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
0	2098	0	2098	0	0	5-8	2-4
I	2098	0	2098	0	0	5-8	2-4

### UNFACTORED REACTIONS

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1623	1088 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1623	1088 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O. J.

## BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY  
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M. C-O, G-J

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

## LOADING

**TOTAL LOAD CASES: (4)**

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/60	-122.2	-122.2	0.17 (1)	10.00	C-N	-35/98	0.03 (1)	
B-C	0/35	-122.2	-122.2	0.25 (1)	10.00	N-D	0/328	0.07 (2)	
C-D	-1673/0	-122.2	-122.2	0.23 (1)	4.93	D-M	0/564	0.09 (1)	
D-E	-1459/0	-122.2	-122.2	0.52 (1)	4.74	M-E	-835/0	0.45 (1)	
E-F	-1459/0	-122.2	-122.2	0.52 (1)	4.74	M-F	0/564	0.09 (1)	
F-G	-1673/0	-122.2	-122.2	0.23 (1)	4.93	K-F	0/328	0.07 (2)	
G-H	0/35	-122.2	-122.2	0.25 (1)	10.00	K-G	-35/98	0.03 (1)	
H-I	0/60	-122.2	-122.2	0.17 (1)	10.00	O-C	-2046/0	0.55 (1)	
Q-B	-339/0	0.0	0.0	0.04 (1)	7.81	G-J	-2046/0	0.55 (1)	
J-H	-339/0	0.0	0.0	0.04 (1)	7.81				
O-N	0/1175	-28.0	-28.0	0.45 (2)	10.00				
N-M	0/1167	-28.0	-28.0	0.46 (2)	10.00				
M-L	0/1167	-28.0	-28.0	0.46 (2)	10.00				
L-K	0/1167	-28.0	-28.0	0.46 (2)	10.00				
K-J	0/1175	-28.0	-28.0	0.45 (2)	10.00				

## DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL	=	38.3 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	10.5 PSF
		DL	=	7.0 PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A  
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL  
OR SMALL BUILDING REQUIREMENTS OF  
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014  
- CSA 086-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)=  $L/360$  (0.86")  
CALCULATED VERT. DEFL.(LL)=  $L/999$  (0.12")  
ALLOWABLE DEFL.(TL)=  $L/360$  (0.86")  
CALCULATED VERT. DEFL.(TL)=  $L/999$  (0.20")

CSI: TC=0.52 (D-E:1), EC=0.46 (K-M:2), WB=0.55 (G-I:1), SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

## NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	822	2284	1656

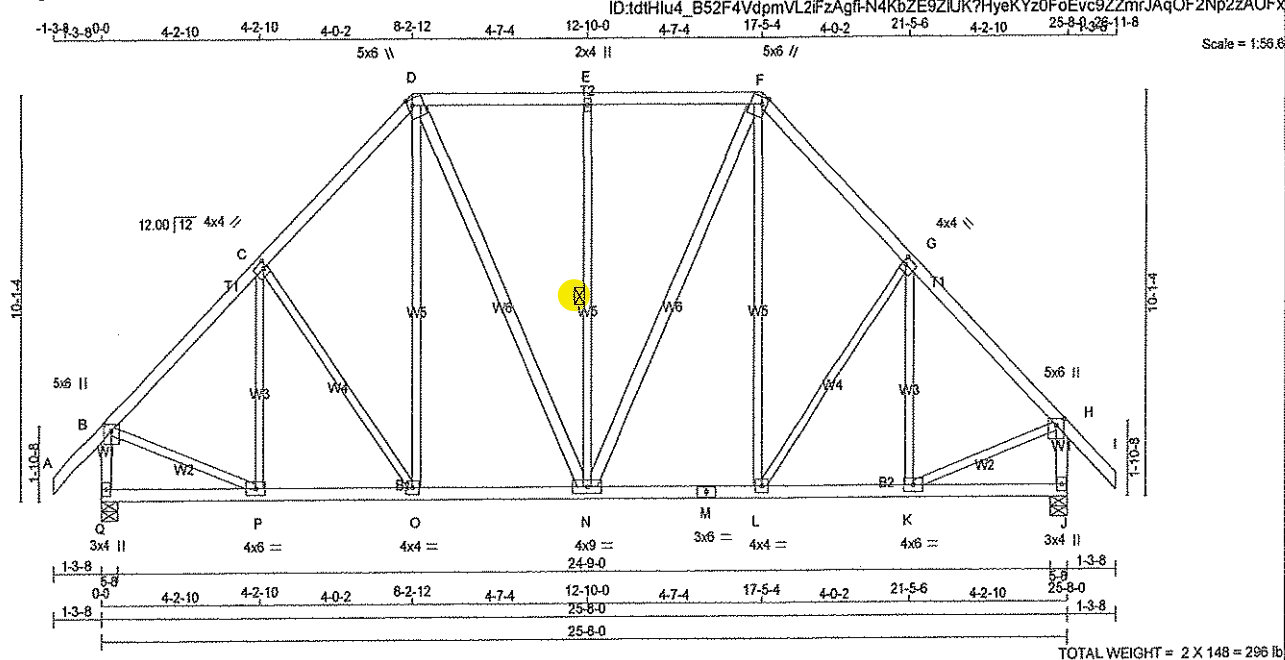
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90 )  
JSI METAL= 0.50 (G) (INPUT = 1.00 )



DWG NO. TAM25720-10  
STRUCTURAL  
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 2.00 2.50
C	TMVW-t	MT20	4.0	4.0 2.00 1.00
D	TTVW+m	MT20	5.0	6.0 2.00 1.50
E	TMVW+w	MT20	2.0	4.0
F	TTVW+m	MT20	5.0	6.0 2.00 1.50
G	TMVW-t	MT20	4.0	4.0 2.00 1.00
H	TMVW+p	MT20	5.0	6.0 2.00 2.50
J	BMV1+p	MT20	3.0	4.0
K	BMVW-t	MT20	4.0	6.0
L	BMVW-t	MT20	4.0	4.0
M	BS-t	MT20	3.0	6.0
N	BMVW-t	MT20	4.0	9.0
O	BMVW-t	MT20	4.0	4.0
P	BMVW-t	MT20	4.0	6.0
Q	BMV1+p	MT20	3.0	4.0

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REORD
JT	GROSS REACTION	DOWN	UP	BRG	BRG
Q	2088 0	2088 0	0 0	5-8	3-5
J	2088 0	2088 0	0 0	5-8	3-5

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM. LIVE WIND DEAD SOIL
Q	1623 1088 / 0	270 / 0	0 / 0 0 / 0 265 / 0 0 / 0
J	1623 1088 / 0	270 / 0	0 / 0 0 / 0 265 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 60	P-C	-320 / 61 0.19 (1)
B-C	-1703 / 0	C-O	-223 / 0 0.21 (1)
C-D	-1610 / 0	O-D	0 / 310 0.07 (2)
D-E	-1296 / 0	D-N	0 / 447 0.07 (1)
E-F	-1296 / 0	N-E	-679 / 0 0.47 (1)
F-G	-1610 / 0	N-F	0 / 447 0.07 (1)
G-H	-1703 / 0	L-F	0 / 310 0.07 (2)
H-I	0 / 60	L-G	-223 / 0 0.21 (1)
Q-B	-2048 / 0	K-G	-320 / 61 0.19 (1)
J-H	-2048 / 0	B-P	0 / 1311 0.30 (1)
		K-H	0 / 1311 0.30 (1)
Q-P	0 / 0		
P-O	0 / 1233		
O-N	0 / 1108		
N-M	0 / 1108		
M-L	0 / 1108		
L-K	0 / 1233		
K-J	0 / 0		

### DESIGN CRITERIA

**SPECIFIED LOADS:**  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN./C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")  
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")  
ALLOWABLE DEFL.(TL)= L/360 (0.88")  
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.08")

CSI: TC=0.34 (D-E:1), BC=0.25 (O-P:1), WB=0.47 (E-N:1), SSI=0.27 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

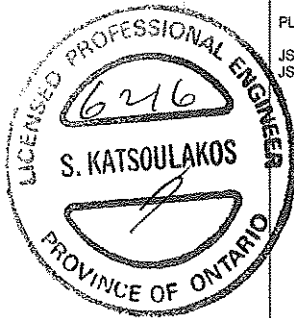
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)  
JSI METAL= 0.38 (H) (INPUT = 1.00)



DWG NO. TAM 25721-10  
STRUCTURAL  
COMPONENT ONLY

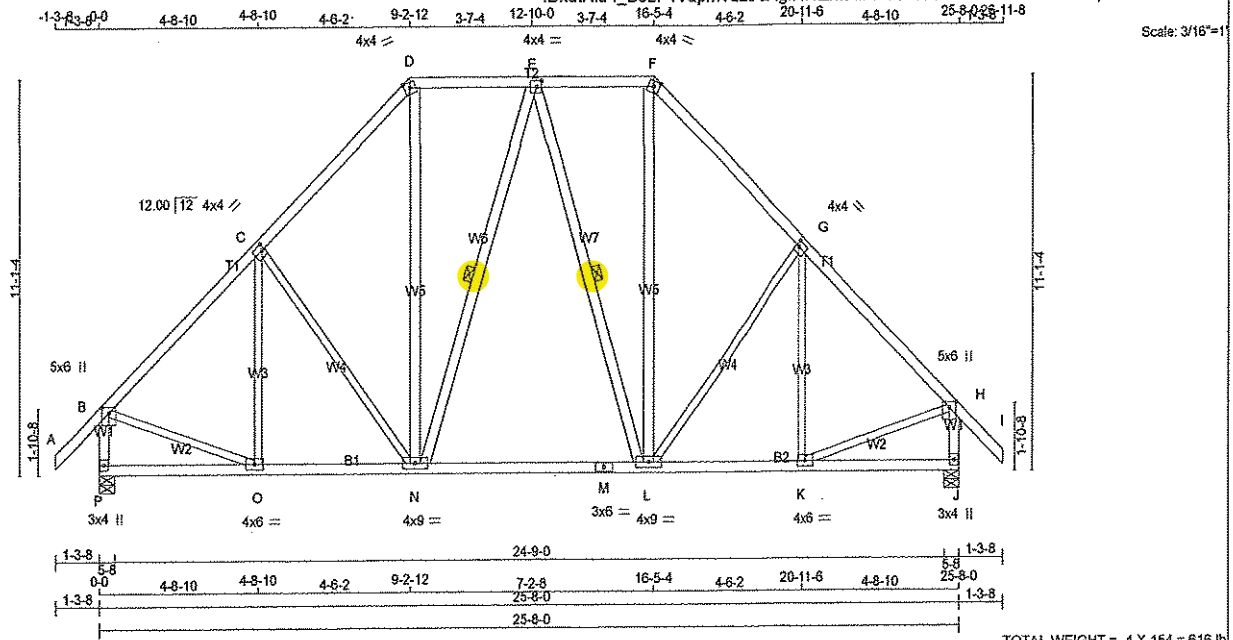


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
266137	T8	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MiTek Industries, Inc. Thu Jun 02 08:37:23 2016 Page 1

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TOTAL WEIGHT = 4 X 154 = 616 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
P - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - D	2x4	DRY	No.2	SPF	
N - E	2x4	DRY	No.2	SPF	
E - L	2x4	DRY	No.2	SPF	
L - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW-m	MT20	4.0	4.0	Edge	
G	TMWW-t	MT20	4.0	4.0	2.00	1.00
H	TMVW+p	MT20	5.0	6.0	2.00	2.50
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

##### BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
P	2098	0	2098	0
J	2098	0	2098	0

##### UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1623	1088 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1623	1088 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

##### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

##### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 60	-122.2	-122.2 0.17 (1)	10.00	O-C	-273 / 83	0.19 (1)
B-C	-1716 / 0	-122.2	-122.2 0.39 (1)	4.68	C-N	-310 / 0	0.38 (1)
C-D	-1554 / 0	-122.2	-122.2 0.37 (1)	4.88	N-D	0 / 679	0.11 (1)
D-E	-1070 / 0	-122.2	-122.2 0.20 (1)	5.87	E-N	-271 / 0	0.20 (1)
E-F	-1070 / 0	-122.2	-122.2 0.20 (1)	5.87	E-L	-277 / 0	0.20 (1)
F-G	-1554 / 0	-122.2	-122.2 0.37 (1)	4.88	L-F	0 / 685	0.11 (1)
G-H	-1716 / 0	-122.2	-122.2 0.39 (1)	4.68	L-G	-310 / 0	0.38 (1)
H-I	0 / 60	-122.2	-122.2 0.17 (1)	10.00	K-G	-273 / 83	0.19 (1)
P-B	-2041 / 0	0.0	0.0 0.23 (1)	5.92	B-O	0 / 1310	0.29 (1)
J-H	-2041 / 0	0.0	0.0 0.23 (1)	5.92	K-H	0 / 1310	0.29 (1)
P-O	0 / 0	-28.0	-28.0 0.13 (2)	10.00			
O-N	0 / 1246	-28.0	-28.0 0.34 (2)	10.00			
N-M	0 / 1153	-28.0	-28.0 0.33 (2)	10.00			
M-L	0 / 1153	-28.0	-28.0 0.33 (2)	10.00			
L-K	0 / 1246	-28.0	-28.0 0.34 (2)	10.00			
K-J	0 / 0	-28.0	-28.0 0.13 (2)	10.00			

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

SPECIFIED LOADS.				
TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD			=	58.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =  $L/360$  (0.86")  
CALCULATED VERT. DEFL. (LL) =  $L/999$  (0.08")  
ALLOWABLE DEFL. (TL) =  $L/360$  (0.86")  
CALCULATED VERT. DEFL. (TL) =  $L/999$  (0.14")

CSI: TC=0.39 (B-C-1), BC=0.34 (K-L-2), WB=0.38 (C-N-1), SSI=0.21 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (P) (INPUT = 0.90)  
JSI METAL= 0.36 (B) (INPUT = 1.00)



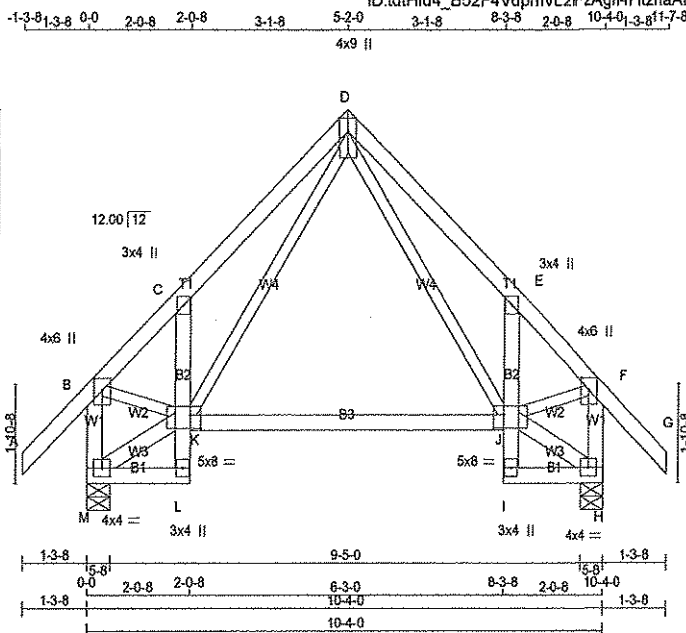
DRWG NO. TAM 25722-16  
STRUCTURAL  
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
266137	T9	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Thu Jun 02 08:37:23 2016 Page 1

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Scale = 1:42.8

TOTAL WEIGHT = 2 X 61 = 121 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - C	2x4	DRY	No.2
K - J	2x4	DRY	No.2
I - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
M - K	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.75 2.00
C	TMV+p	MT20	3.0	4.0	
D	TTVW+p	MT20	4.0	9.0	Edge
E	TMV+p	MT20	3.0	4.0	
F	TMVW+p	MT20	4.0	6.0	2.75 2.00
H	BMVW1-1	MT20	4.0	4.0	
I	BMV+p	MT20	3.0	4.0	
J	BVMWVW-1	MT20	5.0	8.0	3.00 2.50
K	BVMWVW-1	MT20	5.0	8.0	3.00 2.50
L	BMV+p	MT20	3.0	4.0	
M	BMVW1-1	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	947	0	947	0	0
H	947	0	947	0	0

UNFACTORED REACTIONS							
1ST CASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	722	502 / 0	108 / 0	0 / 0	0 / 0	112 / 0	0 / 0
H	722	502 / 0	108 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING  
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 60	-122.2	-122.2	0.17 (1)	D-J	0 / 341	0.08 (1)
B-C	-666 / 0	-122.2	-122.2	0.13 (1)	K-D	0 / 341	0.08 (1)
C-D	-691 / 0	-122.2	-122.2	0.18 (1)	M-K	-15 / 0	0.00 (1)
D-E	-691 / 0	-122.2	-122.2	0.18 (1)	B-K	0 / 502	0.11 (1)
E-F	-666 / 0	-122.2	-122.2	0.13 (1)	J-H	-15 / 0	0.00 (1)
F-G	0 / 60	-122.2	-122.2	0.17 (1)	J-F	0 / 502	0.11 (1)
M-B	-913 / 0	0.0	0.0	0.10 (1)			
H-F	-913 / 0	0.0	0.0	0.10 (1)			
M-L	0 / 13	-28.0	-28.0	0.03 (2)			
L-K	0 / 46	0.0	0.0	0.02 (1)			
K-C	-318 / 0	0.0	0.0	0.02 (1)			
K-J	0 / 305	-28.0	-28.0	0.08 (2)			
I-J	0 / 46	0.0	0.0	0.02 (1)			
J-E	-318 / 0	0.0	0.0	0.02 (1)			
I-H	0 / 13	-28.0	-28.0	0.03 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

DESIGN ASSUMPTIONS  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")  
CALCULATED VERT. DEFL.(LL) = L/ 994 (0.12")  
ALLOWABLE DEFL.(TL)= L/360 (0.34")  
CALCULATED VERT. DEFL.(TL) = L/ 599 (0.21")

CSI: TC=0.18 (C-D:1), BC=0.38 (J-K:2), WB=0.11 (B-K:1), SSI=0.13 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)  
JSI METAL= 0.18 (F) (INPUT = 1.00)



DRWG NO. TAM 25723-10  
STRUCTURAL  
COMPONENT ONLY



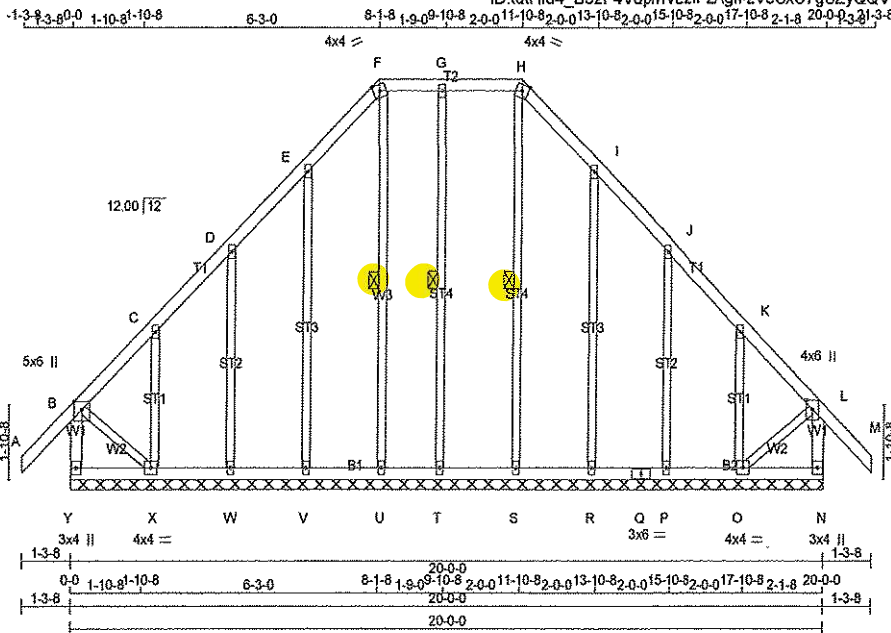


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
266137	G10	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Thu Jun 02 08:37:19 2016 Page 1

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Scale = 1:56.7

TOTAL WEIGHT = 115 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C, D, E, G, I, J, K						
C	TMVW+w	MT20	2.0	4.0		
F	TTW-m	MT20	4.0	4.0	Edge	
H	TTW-m	MT20	4.0	4.0	Edge	
L	TMVW+p	MT20	4.0	6.0	2.75	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		
P, R, S, T, U, V, W						
P	BMV1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	6.0		
X	BMVW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-S, G-T, F-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0/60	-122.2	-122.2 0.17 (1)	S-H	-177/0	0.12 (1)	10.00
B-C	-42/0	-122.2	-122.2 0.08 (1)	T-G	-278/0	0.19 (1)	10.00
C-D	-56/0	-122.2	-122.2 0.08 (1)	V-E	-271/0	0.34 (1)	10.00
D-E	-39/0	-122.2	-122.2 0.07 (1)	W-D	-225/0	0.13 (1)	10.00
E-F	-55/0	-122.2	-122.2 0.07 (1)	X-C	-280/0	0.07 (1)	10.00
F-G	-25/0	-122.2	-122.2 0.06 (1)	R-I	-272/0	0.34 (1)	10.00
G-H	-25/0	-122.2	-122.2 0.06 (1)	P-J	-225/0	0.13 (1)	10.00
H-I	-56/0	-122.2	-122.2 0.07 (1)	O-K	-280/0	0.07 (1)	10.00
I-J	-39/0	-122.2	-122.2 0.07 (1)	U-F	-144/0	0.10 (1)	10.00
J-K	-56/0	-122.2	-122.2 0.08 (1)	B-X	0/52	0.01 (1)	10.00
K-L	-42/0	-122.2	-122.2 0.08 (1)	O-L	0/52	0.01 (1)	10.00
L-M	0/60	-122.2	-122.2 0.17 (1)				
Y-B	-348/0	0.0	0.0 0.04 (1)				
N-L	-347/0	0.0	0.0 0.04 (1)				
Y-X	0/0	-28.0	-28.0 0.03 (2)				
X-W	0/35	-28.0	-28.0 0.04 (2)				
W-V	0/31	-28.0	-28.0 0.03 (2)				
V-U	0/27	-28.0	-28.0 0.03 (2)				
U-T	0/25	-28.0	-28.0 0.02 (2)				
T-S	0/25	-28.0	-28.0 0.02 (2)				
S-R	0/27	-28.0	-28.0 0.03 (2)				
R-Q	0/31	-28.0	-28.0 0.02 (2)				
Q-P	0/31	-28.0	-28.0 0.02 (2)				
P-O	0/35	-28.0	-28.0 0.04 (2)				
O-N	0/0	-28.0	-28.0 0.03 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 060-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1), BC=0.04 (W-X:2), WB=0.34 (I-R:1), SSI=0.12 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

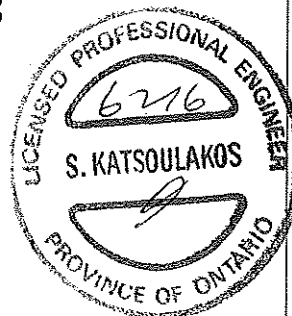
NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

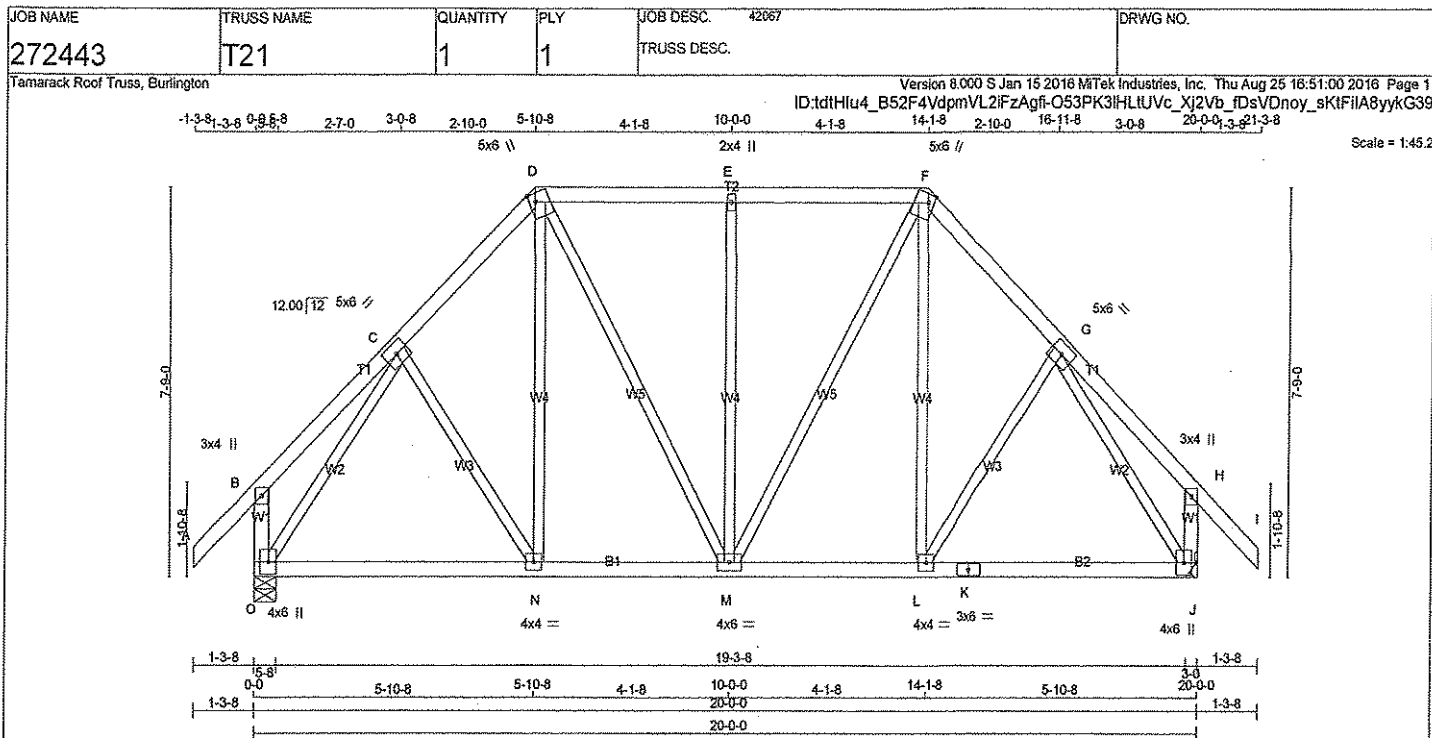
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.61 (F) (INPUT = 0.90)  
JSI METAL= 0.08 (K) (INPUT = 1.00)



DWG NO. TAM25725 -16  
STRUCTURAL  
COMPONENT ONLY





**LUMBER**  
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	5.0	6.0		
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW+t	MT20	4.0	4.0		
M	BMVW+t	MT20	4.0	6.0		
N	BMVW+t	MT20	4.0	4.0		
O	BMVW1+p	MT20	4.0	6.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UPLIFT		
O	1673	0	1673	0	5-8	1-13
J	1673	0	1673	0	5-8	1-13

HANGER BY OTHERS  
MIN. SEAT SIZE: 1-13

**UNFACTORED REACTIONS**

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1290	871/0	210/0	0/0	0/0	208/0	0/0
J	1290	871/0	210/0	0/0	0/0	208/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.64 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)
	FR-TO	FROM-TO	FR-TO	FROM-TO				FR-TO	FROM-TO	
A-B	0/60	-122.2	-122.2	0.17 (1)	10.00	C-N	0/89	0.02 (3)		
B-C	0/28	-122.2	-122.2	0.16 (1)	10.00	N-D	0/248	0.06 (2)		
C-D	-1235/0	-122.2	-122.2	0.14 (1)	5.64	D-M	0/387	0.09 (1)		
D-E	-1040/0	-122.2	-122.2	0.27 (1)	5.83	M-E	-608/0	0.68 (1)		
E-F	-1040/0	-122.2	-122.2	0.27 (1)	5.83	M-F	0/387	0.09 (1)		
F-G	-1235/0	-122.2	-122.2	0.14 (1)	5.64	L-F	0/248	0.06 (2)		
G-H	0/28	-122.2	-122.2	0.16 (1)	10.00	L-G	0/89	0.02 (3)		
H-I	0/60	-122.2	-122.2	0.17 (1)	10.00	O-C	-1549/0	0.71 (1)		
O-B	-308/0	0.0	0.0	0.03 (1)	7.81	G-J	-1549/0	0.71 (1)		
J-H	-308/0	0.0	0.0	0.03 (1)	7.81					
O-N	0/849	-28.0	-28.0	0.30 (2)	10.00					
N-M	0/852	-28.0	-28.0	0.31 (2)	10.00					
M-L	0/852	-28.0	-28.0	0.31 (2)	10.00					
L-K	0/849	-28.0	-28.0	0.30 (2)	10.00					
K-J	0/849	-28.0	-28.0	0.30 (2)	10.00					

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL	= 38.3	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =  $L/360$  (0.67")  
CALCULATED VERT. DEFL. (LL) =  $L/999$  (0.06")  
ALLOWABLE DEFL. (TL) =  $L/360$  (0.67")  
CALCULATED VERT. DEFL. (TL) =  $L/999$  (0.09")

CS1: TC=0.27 (D-E-1), BC=0.31 (M-N-2), WB=0.71 (C-O-1), SS1=0.24 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284

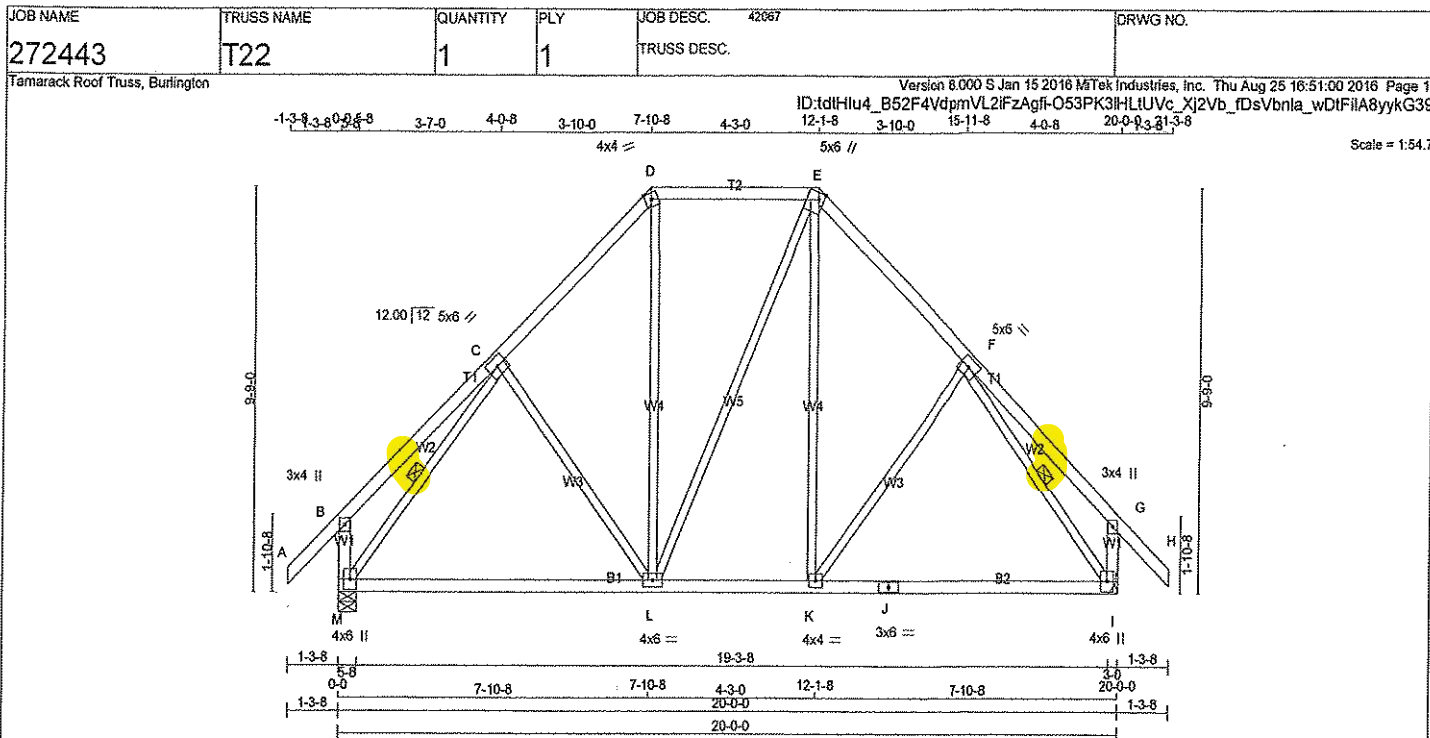
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)  
JSI METAL= 0.33 (G) (INPUT = 1.00)



DWG NO. YAM39292-10  
**STRUCTURAL COMPONENT ONLY**



TOTAL WEIGHT = 105 lb

(M/F)

# LUMBER

## N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
M - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

## PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	6.0	6.0		
D	TTVW-m	MT20	4.0	4.0	Edge	
E	TTVW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVWt+p	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVWt+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

## DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

### BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
M	1673	0	1673	0
I	1673	0	1673	0

### UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
M	1290	871 / 0	210 / 0
I	1290	871 / 0	210 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

### LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	LC1		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 60	-122.2 -122.2	C-L	-182 / 52
B-C	0 / 40	-122.2 -122.2	L-D	0 / 339
C-D	-1136 / 0	-122.2 -122.2	E-F	0 / 1
D-E	-776 / 0	-122.2 -122.2	F-G	0 / 337
E-F	-1136 / 0	-122.2 -122.2	G-H	-182 / 52
F-G	0 / 40	-122.2 -122.2	H-I	-1511 / 0
G-H	0 / 60	-122.2 -122.2	I-J	-1510 / 0
H-I	-352 / 0	0.0 0.0		
I-J	-352 / 0	0.0 0.0		

M-L	0 / 882	-28.0 -28.0	0.46 (2)	10.00
L-K	0 / 775	-28.0 -28.0	0.42 (2)	10.00
K-J	0 / 881	-28.0 -28.0	0.46 (2)	10.00
J-I	0 / 881	-28.0 -28.0	0.46 (2)	10.00

## DESIGN CRITERIA

### SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.67")  
CALCULATED VERT. DEFL.(LL) = 1/999 (0.18")  
ALLOWABLE DEFL.(TL) = L/360 (0.67")  
CALCULATED VERT. DEFL.(TL) = 1/822 (0.29")

CSI: TC=0.31 (F-G:1), BC=0.46 (H-K:2), WB=0.46 (C-M:1), SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

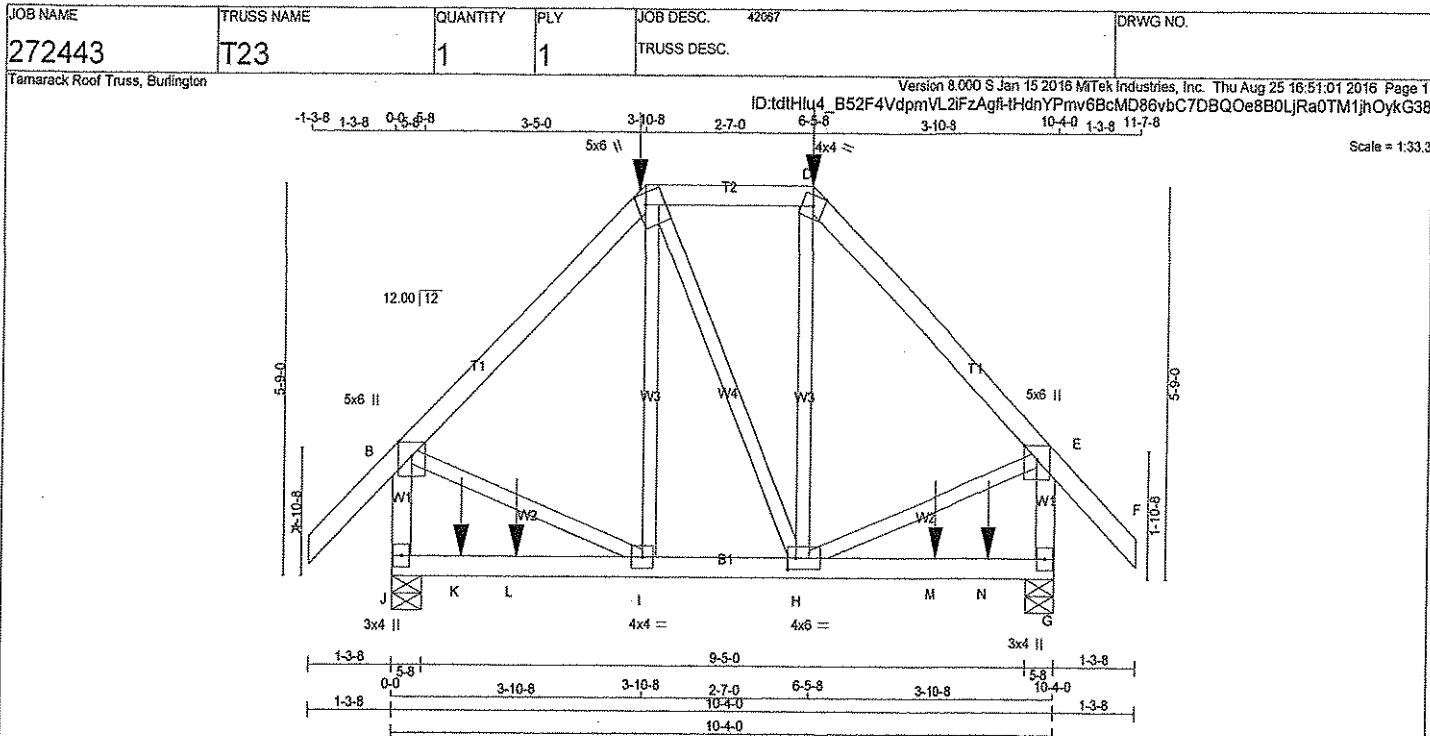
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)  
JSI METAL= 0.38 (C) (INPUT = 1.00)



DRWG NO. YAM 39293-10  
STRUCTURAL  
COMPONENT ONLY





TOTAL WEIGHT = 55 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	5.0	6.0	Edge
C	TTW+m	MT20	5.0	6.0	2.00 1.50
D	TTW-m	MT20	4.0	4.0	Edge
E	TMW+p	MT20	5.0	6.0	Edge
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-I	MT20	4.0	6.0	2.00 1.50
I	BMVWW-I	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 171.3 lbs FACTORED DOWN AT 3-10-8, AND 171.3 lbs FACTORED DOWN AT 6-5-8 ON TOP CHORD, AND 42.6 lbs FACTORED DOWN AT 1-0-12, 604.1 lbs FACTORED DOWN AT 1-11-0, AND 604.1 lbs FACTORED DOWN AT 8-5-0, AND 42.6 lbs FACTORED DOWN AT 9-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

##### BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
J	1746	0	1746	0	0	5-8	2-3	
G	1746	0	1746	0	0	5-8	2-3	

##### UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. SNOW		LIVE		PERM LIVE		WIND		DEAD		SOIL	
	VERT	HORZ	DOWN	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
J	1346	911/0	218/0	0/0	0/0	0/0	0/0	217/0	0/0		217/0	0/0	0/0	0/0
G	1346	911/0	218/0	0/0	0/0	0/0	0/0	217/0	0/0		217/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

##### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

##### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/60	-122.2 -122.2	0.19 (1)	10.00	I-C	0/207	0.05 (2)
B-C	-1040/0	-122.2 -122.2	0.38 (1)	5.61	C-H	0/2	0.00 (2)
C-D	-737/0	-122.2 -122.2	0.18 (1)	6.25	H-D	0/210	0.05 (2)
D-E	-1041/0	-122.2 -122.2	0.38 (1)	5.60	B-I	0/785	0.19 (1)
E-F	0/60	-122.2 -122.2	0.19 (1)	10.00	H-E	0/785	0.19 (1)
J-B	-1426/0	0.0 0.0	0.17 (1)	6.79			
G-E	-1427/0	0.0 0.0	0.17 (1)	6.79			
J-K	0/0	-28.0 -28.0	0.81 (1)	10.00			
K-L	0/0	-28.0 -28.0	0.81 (1)	10.00			
L-I	0/0	-28.0 -28.0	0.81 (1)	10.00			
I-H	0/736	-28.0 -28.0	0.53 (1)	10.00			
H-M	0/0	-28.0 -28.0	0.81 (1)	10.00			
M-N	0/0	-28.0 -28.0	0.81 (1)	10.00			
N-G	0/0	-28.0 -28.0	0.81 (1)	10.00			

##### FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-171	-171	---	FRONT	VERT	TOTAL
D	6-5-8	-171	-171	---	FRONT	VERT	TOTAL
K	1-0-12	-24	-43	---	FRONT	VERT	TOTAL
L	1-11-0	-604	-604	---	FRONT	VERT	TOTAL
M	8-5-0	-604	-604	---	FRONT	VERT	TOTAL
N	9-3-4	-24	-43	---	FRONT	VERT	TOTAL

#### DESIGN CRITERIA

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*  
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.  
LOADS WERE DERIVED FROM USER INPUT  
NO FURTHER MODIFICATIONS WERE MADE

##### SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	58.7	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

\*\*\* NON STANDARD GIRDER \*\*\*  
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")  
ALLOWABLE DEFL.(TL) = L/360 (0.34")  
CALCULATED VERT. DEFL.(TL) = L/863 (0.14")

CSI: TC=0.38 (D-E:1), BC=0.81 (I-J:1), WB=0.19 (E-H:1), SSI=0.38 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (H) (INPUT = 0.60)  
JSI METAL= 0.28 (I) (INPUT = 1.00)

DRWG NO. TAM39294-10  
STRUCTURAL  
COMPONENT ONLY





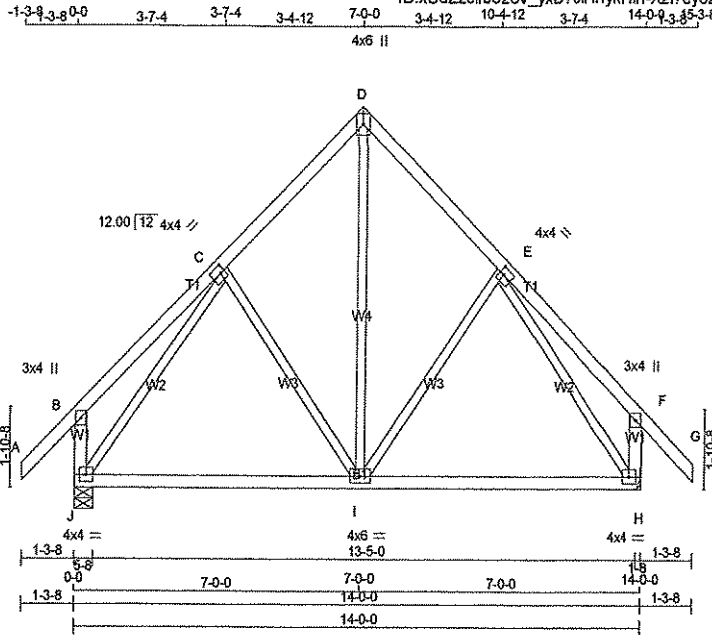
DWG NO. YAM 39295-18  
STRUCTURAL  
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272442	T100	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MITek Industries, Inc. Mon Aug 29 04:51:58 2016 Page 1

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Scale = 1:52.7

TOTAL WEIGHT = 2 X 72 = 144 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
J 1222 0	1222 0	5-8	1-8
H 1222 0	1222 0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD
J	937	642 / 0	147 / 0	0 / 0	0 / 0	148 / 0
H	937	642 / 0	147 / 0	0 / 0	0 / 0	148 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO					FR-TO		
A-B	0 / 60	-122.2	-122.2	0.17 (1)	I-D	0 / 566	0.13 (1)
B-C	0 / 35	-122.2	-122.2	0.24 (1)	I-E	-199 / 35	0.13 (1)
C-D	-672 / 0	-122.2	-122.2	0.19 (1)	C-I	-199 / 35	0.13 (1)
D-E	-672 / 0	-122.2	-122.2	0.19 (1)	J-C	-993 / 0	0.63 (1)
E-F	0 / 35	-122.2	-122.2	0.24 (1)	E-H	-993 / 0	0.63 (1)
F-G	0 / 60	-122.2	-122.2	0.17 (1)			
J-B	-332 / 0	0.0	0.0	0.04 (1)			
H-F	-332 / 0	0.0	0.0	0.04 (1)			
J-I	0 / 566	-28.0	-28.0	0.45 (2)			
I-H	0 / 566	-28.0	-28.0	0.45 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.47")  
CALCULATED VERT. DEFL.(LL) = 1/999 (0.07")  
ALLOWABLE DEFL.(TL) = 1/360 (0.47")  
CALCULATED VERT. DEFL.(TL) = 1/999 (0.12")

CSI: TC=0.24 (E-F:1), BC=0.45 (H-I:2), WB=0.63 (E-H:1), SSI=0.17 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)  
JSI METAL= 0.38 (E) (INPUT = 1.00)

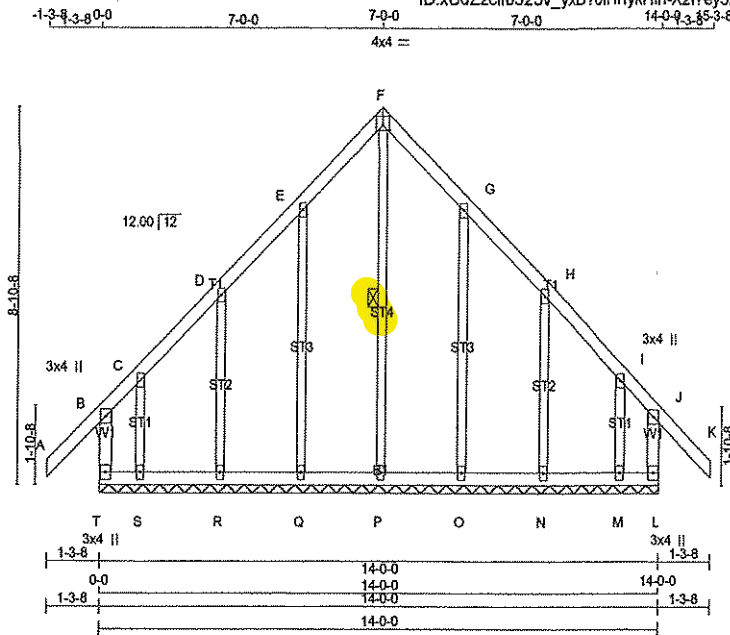


DWG NO. TAW39297-10  
STRUCTURAL  
COMPONENT ONLY

JOB NAME <b>272442</b>	TRUSS NAME <b>G100</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. 42687 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MITek Industries, Inc. Mon Aug 29 04:51:58 2016 Page 1  
ID:xUdZcclb323v\_yxB70iHhykHn-X2l7ey5Z260EolnjO4vV4VISHRLUOHv2MC6wu1yJ6DF



Scale = 1:53.4

TOTAL WEIGHT = 74 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C, D, E, G, H, I					
C	TMW+w	MT20	2.0	4.0	
F	TTW+p	MT20	4.0	4.0	1.50 2.00
J	TMV+p	MT20	3.0	4.0	
L	BMV1+p	MT20	3.0	4.0	
M, N, O, P, Q, R, S					
M	BMW1+w	MT20	2.0	4.0	
T	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.  
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.  
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
T-B	-287 / 0	0.0	0.0	0.04 (1)	P-F	-308 / 0	0.15 (1)
A-B	0 / 60	-122.2	-122.2	0.17 (1)	Q-E	-249 / 0	0.20 (1)
B-C	-34 / 0	-122.2	-122.2	0.12 (1)	R-D	-250 / 0	0.09 (1)
C-D	0 / 32	-122.2	-122.2	0.06 (1)	S-C	-87 / 0	0.01 (1)
D-E	0 / 32	-122.2	-122.2	0.07 (1)	O-G	-249 / 0	0.20 (1)
E-F	0 / 36	-122.2	-122.2	0.07 (1)	N-H	-250 / 0	0.09 (1)
F-G	0 / 36	-122.2	-122.2	0.07 (1)	M-I	-87 / 0	0.01 (1)
G-H	0 / 32	-122.2	-122.2	0.07 (1)			
H-I	0 / 32	-122.2	-122.2	0.06 (1)			
I-J	-34 / 0	-122.2	-122.2	0.12 (1)			
J-K	0 / 60	-122.2	-122.2	0.17 (1)			
L-J	-287 / 0	0.0	0.0	0.04 (1)			
T-S	-13 / 0	-28.0	-28.0	0.02 (3)			
S-R	-17 / 0	-28.0	-28.0	0.02 (2)			
R-Q	-23 / 0	-28.0	-28.0	0.02 (2)			
Q-P	-27 / 0	-28.0	-28.0	0.02 (2)			
P-O	-27 / 0	-28.0	-28.0	0.02 (2)			
O-N	-23 / 0	-28.0	-28.0	0.02 (2)			
N-M	-17 / 0	-28.0	-28.0	0.02 (2)			
M-L	-13 / 0	-28.0	-28.0	0.02 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

DESIGN ASSUMPTIONS  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1), BC=0.02 (Q-R:2), WB=0.20 (G-O:1), SSI=0.09 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (F) (INPUT = 0.90)  
JSI METAL= 0.07 (H) (INPUT = 1.00)



DWG NO. YAW 39298-10  
STRUCTURAL  
COMPONENT ONLY

STRUCTURAL  
COMPONENT ONLY

# HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

**MATERIAL:** 12 gauge

**FINISH:** G90 galvanized

**DESIGN:**

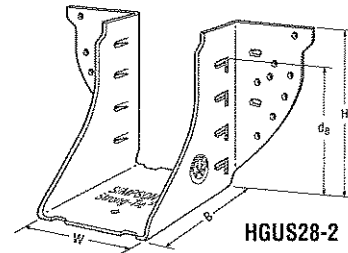
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%.  
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

**INSTALLATION:**

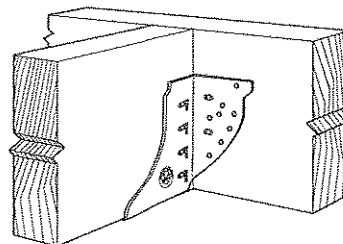
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

**OPTIONS:**

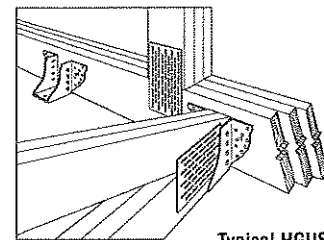
- See current catalogue for options



HGUS28-2



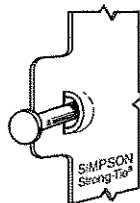
Typical HGUS Installation



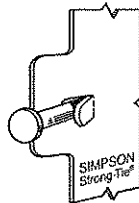
Typical HGUS Installation  
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d <sub>g</sub> <sup>1</sup>	Face	Joist	D. Fir-L		S-P-F	
								Uplift (K <sub>o</sub> =1.15)	Normal (K <sub>o</sub> =1.00)	Uplift (K <sub>o</sub> =1.15)	Normal (K <sub>o</sub> =1.00)
HGUS26	12	1½	5¾	5	4½ <sub>2</sub>	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3½ <sub>16</sub>	5¾ <sub>16</sub>	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4½ <sub>16</sub>	5¾ <sub>16</sub>	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6½ <sub>16</sub>	5¾ <sub>16</sub>	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3½ <sub>16</sub>	7¾ <sub>16</sub>	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4½ <sub>16</sub>	7¾ <sub>16</sub>	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6½ <sub>16</sub>	7¾ <sub>16</sub>	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3½ <sub>16</sub>	9¾ <sub>16</sub>	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4½ <sub>16</sub>	9¾ <sub>16</sub>	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6½ <sub>16</sub>	9¾ <sub>16</sub>	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6½ <sub>16</sub>	10¾	4	10½	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6½ <sub>16</sub>	12¾	4	11½	66-16d	22-16d	10130	16400	7195	11645

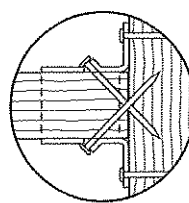
1. d<sub>g</sub> is the distance from the seat of the hanger to the highest joist nail.



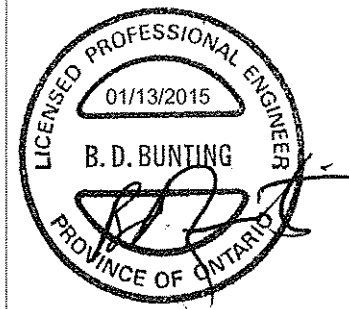
Dome Double Shear Nailing prevents tabs breaking off (available on some models).  
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see [www.strongtie.com](http://www.strongtie.com).

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099  
[www.strongtie.com](http://www.strongtie.com)

# HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

**MATERIAL:** See table

**FINISH:** G90 galvanized

**DESIGN:**

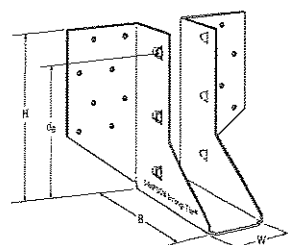
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%  
No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

**INSTALLATION:**

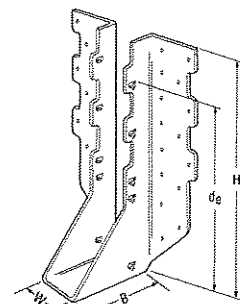
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

**OPTIONS:**

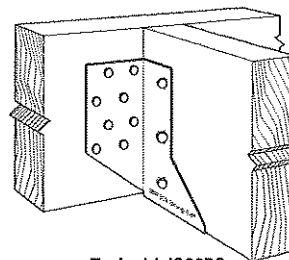
- See current catalogue for options



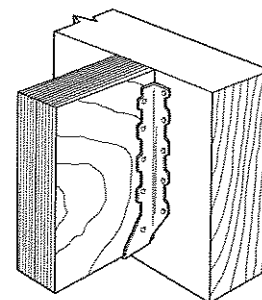
LJS26DS



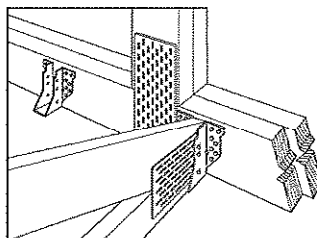
HUS210  
(HUS26, HUS28, similar)



Typical LJS26DS  
Installation



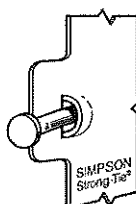
Typical HUS  
Installation



Typical HUS Installation  
(Truss Designer to provide fastener  
quantity for connecting multiple  
members together)

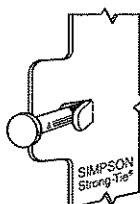
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d <sub>g</sub> <sup>1</sup>	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K <sub>0</sub> =1.15)	Normal (K <sub>0</sub> =1.00)	Uplift (K <sub>0</sub> =1.15)	Normal (K <sub>0</sub> =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 1/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 3/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d<sub>g</sub> is the distance from the seat of the hanger to the highest joist nail.

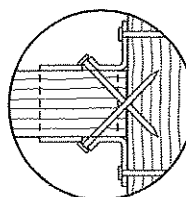


Dome Double  
Shear Nailing  
prevents tabs  
breaking off  
(available on  
some models).

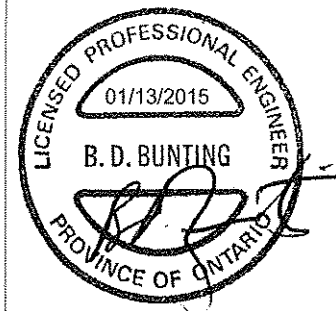
U.S. Patent  
5,603,580



Double  
Shear  
Nailing  
Side  
View. Do  
not bend  
tab back.



Double  
Shear  
Nailing  
Top View.



LIMIT  
STATES  
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see [www.strongtie.com](http://www.strongtie.com).

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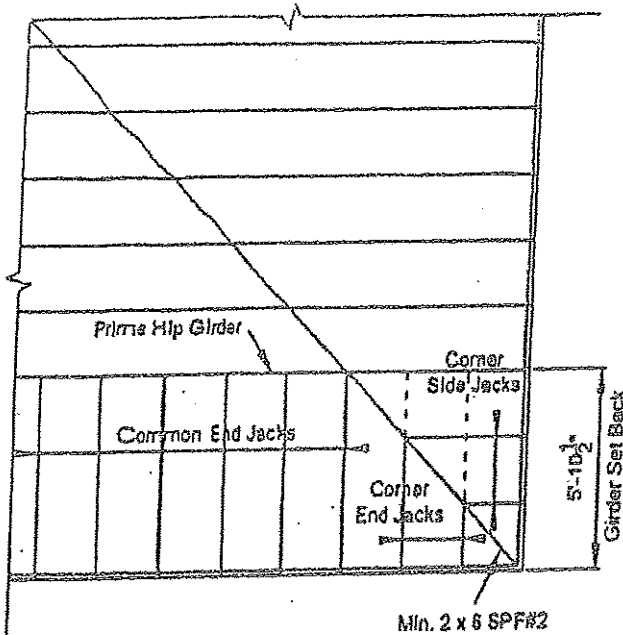
T-SPECHUS15 1/15 exp. 12/16

800-999-5099  
[www.strongtie.com](http://www.strongtie.com)



# MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

## LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

## DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

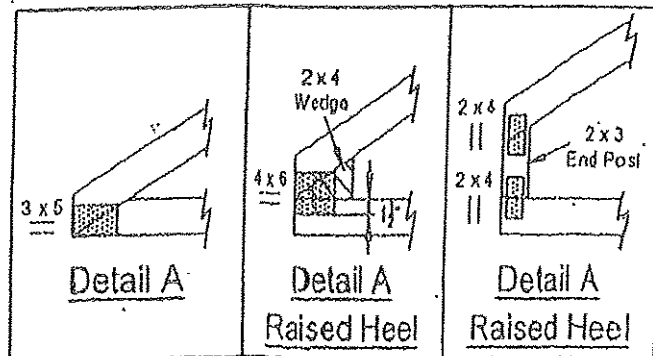
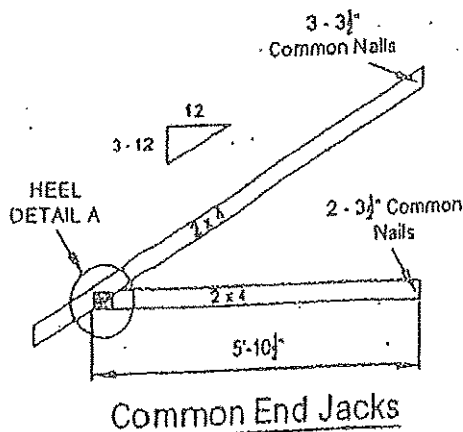
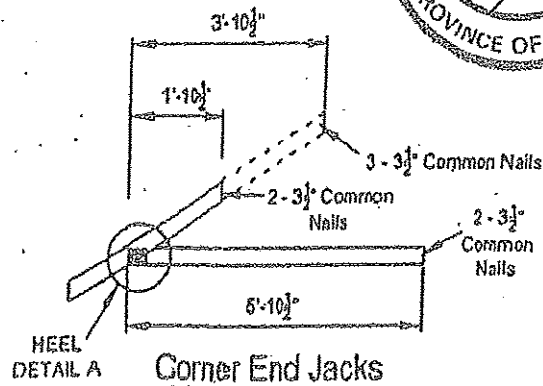
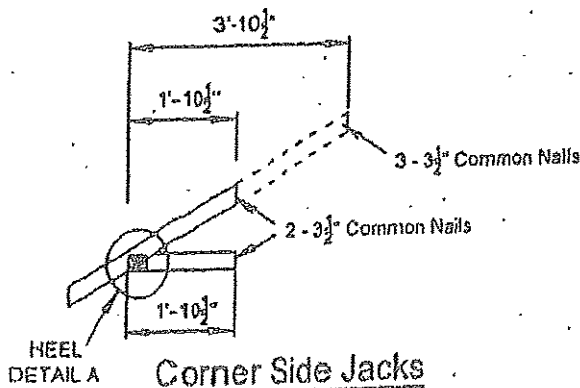
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

DWG NO T&M 3495.14  
STRUCTURAL  
COMPONENT ONLY



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)  
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

# MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

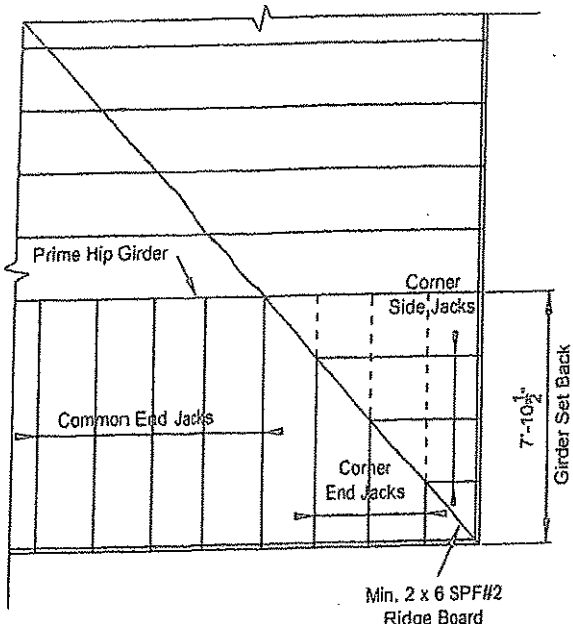
## LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2  
BOTTOM CHORD : 2 x 4 SPF#2  
WEBS : 2 x 3 SPF#2  
UNLESS OTHERWISE SHOWN

## DESIGN LOAD:

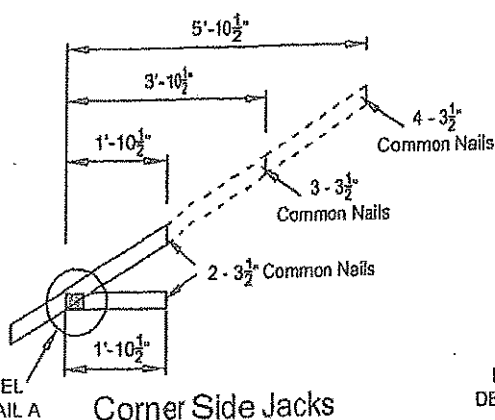
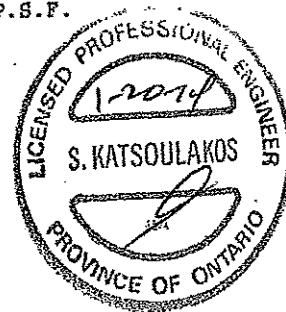
TOP CHORD LIVE LOAD : 34.8 P.S.F.  
TOP CHORD DEAD LOAD : 3.0 P.S.F.  
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.  
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

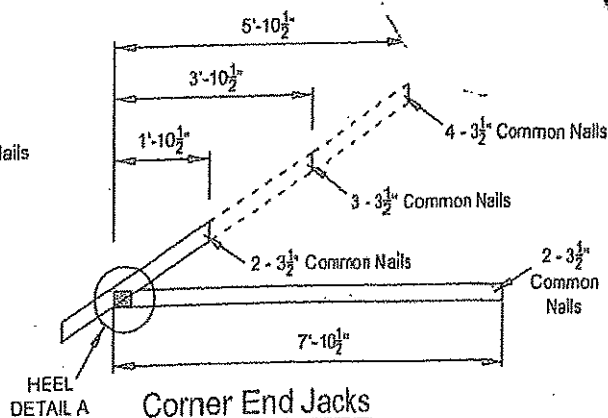


45° Hip End

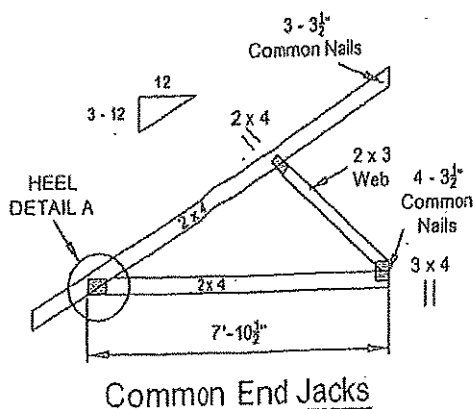
DWG NO TAM 3503.14  
STRUCTURAL  
COMPONENT ONLY



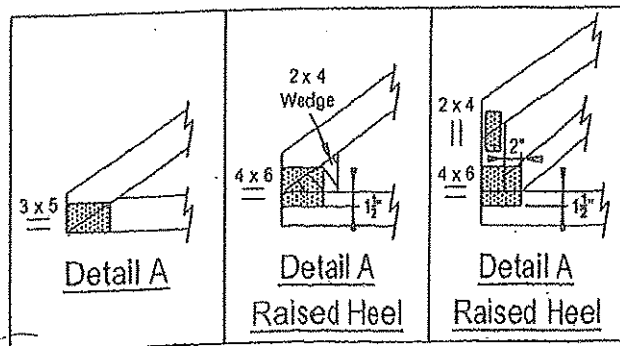
Corner Side Jacks



Corner End Jacks



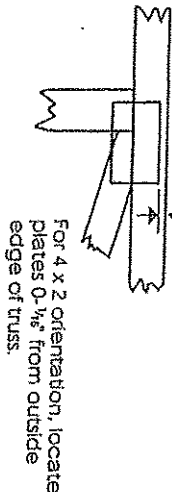
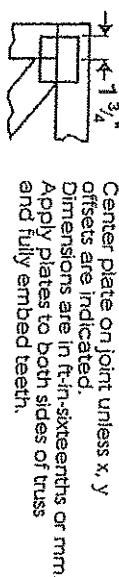
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)  
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

# Symbols

## PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in Mitek software or upon request.

## PLATE SIZE

4 X 4

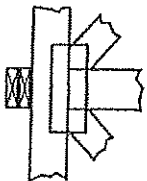
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use I, I or Eliminator bracing if indicated.

## BEARING

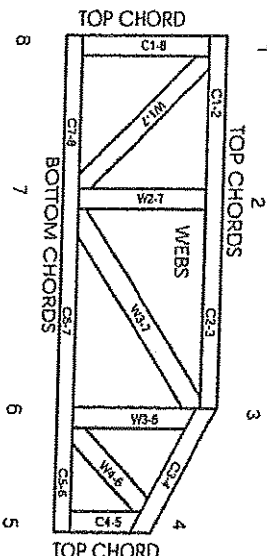


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:  
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses  
DSB-89: Design Standard for Bracing.  
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

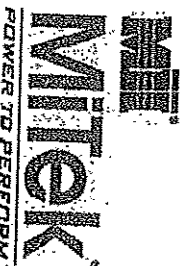
## PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet M11-7473C rev. 10-'08



# General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

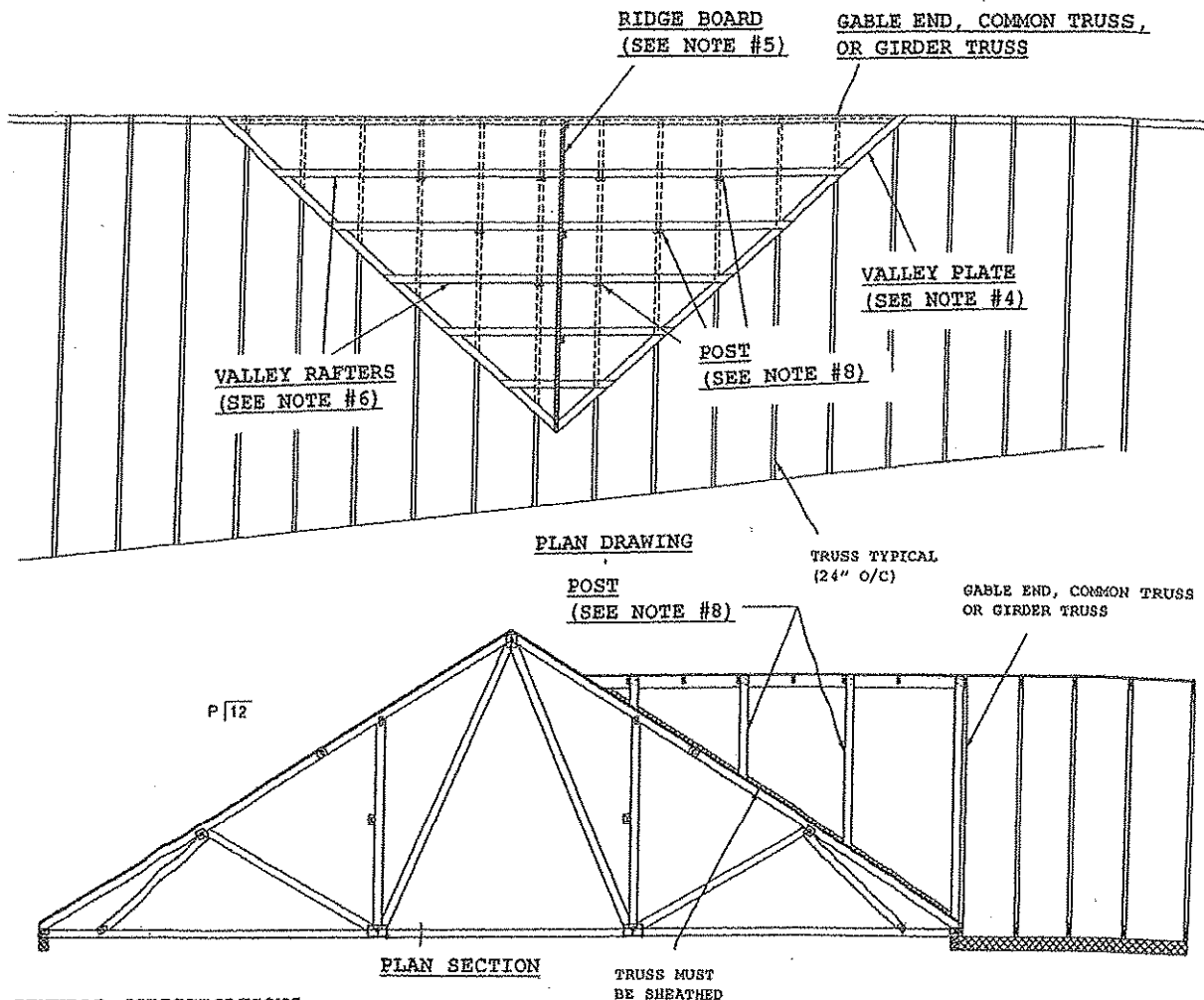
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative I, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pultrus provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

# MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

## CONVENTIONAL VALLEY FRAMING DETAIL



### GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

### NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.).
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.).
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

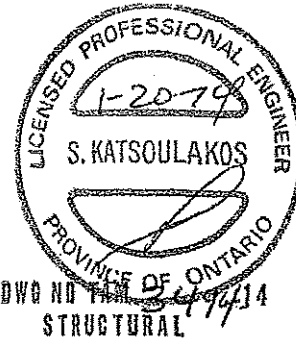


DWG NO TAM 6305.14  
STRUCTURAL  
COMPONENT ONLY

Micro City Engineering Services Inc.  
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61  
Glencoe, Ontario  
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



### **Responsibilities:**

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

### **SPECIFICATIONS:**

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

**All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:**

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from [www.tpic.ca](http://www.tpic.ca) and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.