

52-02-00

FROM PLAN DATED: FEB 2016

BUILDER: BAYVIEW WELLINGTON

ALCONA SITE:

MODEL: S32-9-15

ELEVATION: A,B

CITY: INNISFIL, ON

LOT:

SALESMAN: MARIO DESIGNER: CZ **REVISION: -** NOTES:

CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6. SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. CANTILEVERED JOISTS REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. AT ENDS.

LOADING:

IUS2.56/9.5 HUS1.81/10 HUS1.81/10

IUS2.56/9.5 IUS2.56/9.5

Product

Manuf

nnector Summary

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 11/09/2017

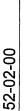
1st FLOOR

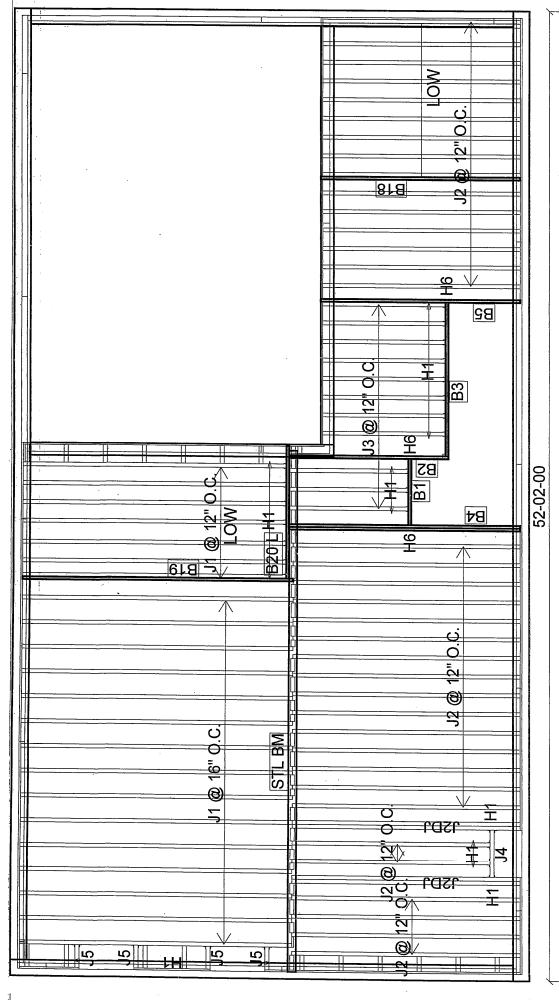
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26-06-00

Co	ty		- L-		-					,		_	
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	Net Qty	22	37	4	_	_	4	_	_	2	_	_	_
	Plies	_	_	0	_	~	_	_	_	7	-		7
Products	Product	9 1/2" NI-40x	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP										
	Length	14-00-00	12-00-00	12-00-00	8-00-00	4-00-00	2-00-00	12-00-00	12-00-00	12-00-00	10-00-00	10-00-00	
	PlotID	11	72	J2DJ	J3	کل	J5	B18	B5	B4	B2	B3	Š





	J	connector	Connector Summary
Г	Qty	Manuf	Product
T	9	도	IUS2.56/9.5
	7	Ŧ	IUS2.56/9.5
	9	Ŧ	IUS2.56/9.5
	2	9H	HUS1.81/10
	_	9H	HUS1.81/10

Net Qty

Plies

Products

Colliferior Carrillary
Manuf Product
H1 IUS2.56/9.5
H1 IUS2.56/9.5
H1 IUS2.56/9.5
H6 HUS1.81/10
H6 HUS1.81/10
Manuf 11 11 16 16

VERSA-LAM® 2.0 3100 SP

VERSA-LAM® 2.0 3100 SP VERSA-LAM® 2.0 3100 SP

1-3/4" x 9-1/2 1-3/4" x 9-1/2 1-3/4" x 9-1/2 1-3/4" x 9-1/2

14-00-00 12-00-00 12-00-00 8-00-00 2-00-00 14-00-00 12-00-00 12-00-00 10-00-00

J1 J2D J3 J4 J5 B19 B18 B20 B3

9 1/2" NI-40x 9 1/2" NI-40x 9 1/2" NI-40x 9 1/2" NI-40x

9 1/2" NI-40x

PlotID

VERSA-LAM® 2.0 3100 SP

-3/4" x 9-1/2' -3/4" x 9-1/2'

VERSA-LAM® 2.0 3100 SP VERSA-LAM® 2.0 3100 SP VERSA-LAM® 2.0 3100 SP

-3/4" x 9-1/2"

8-00-00 4-00-00

VERSA-LAM® 2.0 3100 SP

FROM PLAN DATED: FEB 2016

ALPA LUMBER GROUP

TAMARACK LUMBER INC

BUILDER: BAYVIEW WELLINGTON

SITE: ALCONA

MODEL: \$32-9-15

ELEVATION: A,B

LOT:

26-06-00

CITY: INNISFIL, ON

SALESMAN: MARIO **DESIGNER:** CZ REVISION: -

NOTES:

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING **CERAMIC TILE APPLICATION** AS PER O.B.C. 9.30.6 SQUASH BLOCKS

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. CANTILEVERED JOISTS

REFER TO THE NORDIC

INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION

DESIGN LOADS: L/480.000 LOADING:

DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft LIVE LOAD: 40.0 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 07/09/2017

1st FLOOR

SUNKEN

26-06-00

52-02-00

FROM PLAN DATED: FEB 2016

BUILDER: BAYVIEW WELLINGTON

SITE: ALCONA

MODEL: S32-9-15

ELEVATION: A,B

LOT:

CITY: INNISFIL, ON

SALESMAN: MARIO DESIGNER: CZ

NOTES:

REVISION: -

CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6. SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. CANTILEVERED JOISTS REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS.

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION.

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 07/09/2017

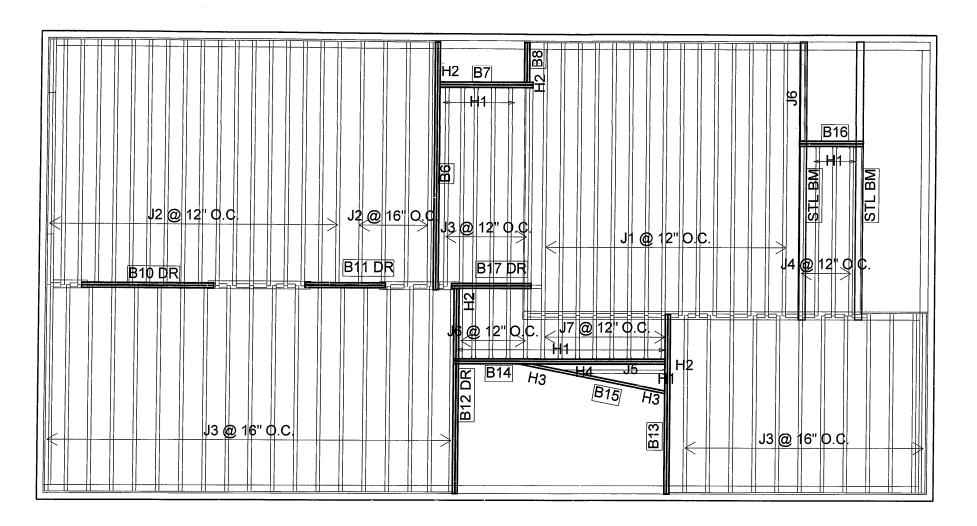
1st FLOOR

75-06-00 25-06-00	He B12" O.C. B3 H6 H6 B3 M5 H6 H6 H6 H6 H6 H6 H6 H6 H6 H	52-02-00
0 0	B¢ P	52-02-00

26-06-00

		Products			ے 	
PlotID	PlotID Length	Product	Plies	Net Qty	Qty	Σ
17	14-00-00	9 1/2" NI-40x	_	23	_	I
72	12-00-00	9 1/2" NI-40x	_	38	7	I
J2DJ	12-00-00	9 1/2" NI-40x	7	4	2	I
J3	8-00-00	9 1/2" NI-40x	~		7	Ĭ
40	4-00-00	9 1/2" NI-40x	_	_	_	Ĭ
B18	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	_	_		
B5	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	_	_		
B4	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	7	2		
B2	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	~			
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	_	_		
B 1	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	~			

	Products				Connector	Connector Summary	
Length	Product	Plies	Net Qty	Qty	Manuf	Product	
14-00-00	9 1/2" NI-40x	_	23	7	Ī	IUS2.56/9.5	
12-00-00	9 1/2" NI-40x	_	38	7	Ξ	IUS2.56/9.5	
12-00-00	9 1/2" NI-40x	7	4	7	Ξ	IUS2.56/9.5	
8-00-00	9 1/2" NI-40x	~		7	9 9 1	HUS1.81/10	
4-00-00	9 1/2" NI-40x	_	_	_	9H	HUS1.81/10	
12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	_	~				
12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP		_				
12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	7	7				
10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	_	_				
10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	~	_		09/03/2018 1:51	09/03/2018 1:51:45 PM kgervais	
4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	_	_				



PlotID Length Product Plies Net Qty J1 16-00-00 9 1/2" NI-40x 1 15 J2 14-00-00 9 1/2" NI-40x 1 22 J3 12-00-00 9 1/2" NI-40x 1 37 J4 10-00-00 9 1/2" NI-40x 1 4 J5 8-00-00 9 1/2" NI-40x 1 1
J2 14-00-00 9 1/2" NI-40x 1 22 J3 12-00-00 9 1/2" NI-40x 1 37 J4 10-00-00 9 1/2" NI-40x 1 4
J3 12-00-00 9 1/2" NI-40x 1 37 J4 10-00-00 9 1/2" NI-40x 1 4
J4 10-00-00 9 1/2" NI-40x 1 4
J5 8-00-00 9 1/2" NI-40x 1 1
J6 6-00-00 9 1/2" NI-40x 1 6
J7 4-00-00 9 1/2" NI-40x 1 8
B6 16-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B14 14-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B12 DR 12-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B13 12-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B15 10-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1 1
B10 DR 8-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B11 DR 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B17 DR 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B7 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B16 4-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2
B8 4-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2 2

С	onnector	Summary
Qty	Manuf	Product
22	H1	IUS2.56/9.5
4	H2	HGUS410
1	H3	LS 90
1	H3	LS 90
1	H4	LSSUH310

Town of Inniefii Certified Model
09/03/2018 1:51:47 PM kgervais



FROM PLAN DATED: FEB 2016

BUILDER:

BAYVIEW WELLINGTON

SITE: ALCONA

MODEL: S32-9-15

ELEVATION: A,B

LOT:

CITY: INNISFIL, ON

SALESMAN: MARIO DESIGNER: CZ REVISION: -

NOTES:

CERAMIC TILE APPLICATION

AS PER O.B.C. 9.30.6. SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING

WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS.

CANTILEVERED JOISTS

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE

AT ENDS.

REFER TO THE NORDIC

INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION.

LOADING:

DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 11/09/2017

2nd FLOOR

Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i11655) August 2, 2016 13:40:34 Boise Cascade

Dry | 1 span | No cantilevers | 0/12 slope (deg)

BC CALC® Design Report

Build 4340 Job Name: Address:

City, Province, Postal Code: Customer:

CCMC 12472-R

Code reports:

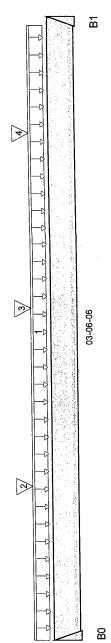
File Name: S32-9.mmdl Description: Designs∖Flush Beams\Basment∖Flush Beams∖B1(i1165

Specifier:

Company. Designer:

Msc.

09/03/2018 1:51:50 PM kgervais



Total Horizontal Product Length = 03-06-06

				,		
Reaction Summary (Down / Uplift) (lbs) Bearing	own / Uplift) (Ib	s) Dead		Snow	Wind	
80	0/969		0			
B1	596/0	306/0	၁			
					Live Dead	1 Snow Wind Trib.
Load Summary Tag Description	Load Type		Ref. Start	End	1.00 0.65	1.00 1.15
1 UserLoad	Unf. Lin. (lb/ft)	(lb/ft)	00-00-00	03-05-14	240 120	n/a
2 (13(11721)	Conc. Pt	(lbs)	_ 00-10-08	00-10-08	126	n/a
3 (111731)	Conc. Pt. (lbs)	l (sql)	- 01-10-08	01-10-08	125	n/a
4 J3(i11643)	Conc. Pt. (lbs)	l (sql)	- 02-10-08	02-10-08	104 52	n/a
	Factored	Factored	Demand /	Load	Location	Disclosure
Controls Summary	Demand	Resistance	Resistance	Case		Completeness and accuracy of input must
Doe Momont	1 089 ff-lhs	12 704 ft-lbs	8.6%	_	01-10-08	be verified by anyone w ho would rely on
To a Shoor	75.4 lbs	5.785 lbs	13%	•	00-11-08	output as evidence of suitability for
Total ond Def	1,999 (0,006")	5,1 55 1,5 1/a), D/a	4	01-09-02	particular application. Output here based
live Load Defi	1/999 (0.004")	n/a	n/a	5	01-09-02	on building code-accepted design
Max Defl.	0.006"	n/a	n/a	4	01-09-02	properties and analysis memous.
Span / Depth	4.2	n/a	n/a		00-00-00	products must be in accordance with
			1	7		current Installation Guide and applicable
			Demand/ Posistance	De mano/ Rosistance		building codes. To obtain Installation Guide
Boaring Supports	Dim. (L x W)	Demand	Support	Member	Material	or ask questions, please call
Bo Hanger	2" × 1-3/4"	1,277 lbs	n/a	29.9%	HUS1.81/10	
	2" × 1-3/4"	1,278 lbs	n/a	29.9%	HUS1.81/10	BC CALO®, BC FRAMER® , AJS™,
						ALLJOIST®, BC RIM BOARD TM , BCI®,
Notes						BOISE GLULAIVI", SIIVIPLE FRANKIIVG
Design meets Code minimum (L/240) Total load deflection criteria.	imum (L/240) Tota	I load deflection	n criteria.			SYSTEMS, VERSA-LAIMS, VERSA-ININI

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood

Products L.L.C.

Calculations assume Member is Fully Braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Part code: Part 9 Importance Factor: Normal

Deflections less than 1/8" were ignored in the results.

CONFORMS TO 08C 2012

DWOND.TAM 4525617 STRUCTURAL COMPONENT ONLY

S. KATSOULAKOS WAS OIHAINO A POLINCE OF

Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i11673) August 2, 2016 13:40:33 Roise Cascado

BC CALC® Design Report

Build 4340 Job Name:

Dry | 1 span | No cantilevers | 0/12 slope (deg)

S32-9.mmdl File Name:

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i1167

Specifier:

Designer: Company:

Misc.

CCMC 12472-R

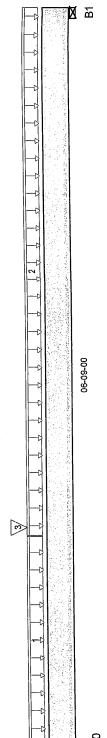
Code reports:

Customer:

City, Province, Postal Code:

Address:

09/03/2018 1:51:51 PM kgervais



Total Horizontal Product Length = 06-09-00

Reaction Summary (Down / Uplift) (Ibs.)	n/Uplift)(Ibs)							
Bearing	Live		Snow	Wind				
B0. 3-1/2"	525/0	284/0						
B1, 8"	317/0	178/0						
				Live	Dead	Snow	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00 1.15	1.15	
1 FC1 Floor Material	Unf. Lin. (lb/ft)	T 00-00-00	01-11-00) 27	13			n/a
2 FC1 Floor Material	Unf. Lin. (lb/ft)	L 01-11-00 06-09-00 40)0-60-90	40	20			n/a

n/a n/a n/a

13 20 307

40 597

01-11-00 01-11-14 00-00-00

Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Conc. Pt. (lbs)

FC1 Floor Material FC1 Floor Material

B1 (i11655)

01-11-14

Controle Summary	Factored	Factored Resistance	Demand / Resistance	Load	Location	D O
Doe Momont	1 887 ff-lhs	12 704 ft-lbs	14.9%	-	01-11-14	ڡٞ
FOST INCIDENT	1,023 lbs	5.785 lbs		~-	01-01-00	ō
Total one Defi	("800 () 606/ 1	e/u		4	02-11-10	ä
Total Load Dell.	1 /999 (0.018")	<i>a</i> / <i>a</i>	, re/c	2	02-11-10	ō
Live Load Dell.	0.000 (0.000)	<i>e</i> /c	e/u	4	02-11-10	<u>a</u>
Max Dell. On on / Douth	7.5	g /c	e/u		00-00-00	으
Spail Depui	?	5	1			ܩ

		Material	Unspecified	Unspecified
De m an d/	Resistance	Support Member N		4.1%
Demand/ Demand/	Resistance Resistance	Support	23%	3.5%
		Demand	1,142 lbs	e98 lbs
		Dim.(LxW) Demand	3-1/2" x 1-3/4" 1,142 lbs	8" × 1-3/4"
		na Supports	B0 Post	Wall/Plate
		Beari	80	B1

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced

Resistance Factor phi has been applied to all presented results per CSA 086. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

Completeness and accuracy of input must be verified by anyone who would rely on building codes. To obtain Installation Guide varticular application. Output here based current Installation Guide and applicable nstallation of BOISE engineered wood products must be in accordance with or ask questions, please call 1-800-964-6999 before installation. output as evidence of suitability for on building code-accepted design properties and analysis methods

ALLJOIST®, BC RIM BOARD™, BC®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood BC CALO®, BC FRAMER®, AJS™, PLUS®, VERSA-RIM®, Products L.L.C.

S. KATSOULAKOS BAR OIHEINO POVINCE OF

> OWBHO. FAM 45257=19 70 STRUCTURAL COMPONENT

August 2, 2016 13:40:33 Basment/Flush Beams/B3(i11262) Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Boise Cascade

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

Build 4340 Job Name Address:

City, Province, Postal Code: Code reports: Customer:

CCMC 12472-R

S32-9.mmdl File Name:

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i1126

Designer: Specifier:

Company. Msc. 09/03/2018 1:51:52 PM kgervais

Town of in

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65

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-03-14	Wind
Total Horizontal Product Length = 08-03-1	Snow
Total Horizontal F	De ad
	mary (Down / Uplift) (15s) Live
	Reaction Sumi Bearing

08-03-14

├8

Bearing	Live	Dead	Snow	Wind		
B0. 1-11/16"	363/0	226/0				
B1	515/0	371/0				
			1	Live Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	End 1	1.00 0.65	1.00 1.15	
1 (3(11246)	Conc. Pt. (lbs)	L 01-02-06	01-02-06	85 43		n/a
2 (3(11303)	Conc. Pt. (lbs)	L 02-05-06	02-02-06	65 33		n/a
3 (111337)	Conc. Pt. (lbs)	L 03-05-06	03-05-06	130 65		n/a
4 (3/11/378)	Conc. Pt. (lbs)	L 04-02-06				n/a
5 .13(11327)	Conc. Pt. (lbs)	L 05-02-06	05-02-06 130	30 92		n/a
6 .13(i11339)	Conc. Pt. (lbs)	L 06-02-06				n/a
7 .I3(i11274)	Conc. Pt. (lbs)	L 07-02-06				n/a
8 J3(i11334)	Conc. Pt. (lbs)	L 08-02-06	08-02-06	78 59		n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos Moment	sq	12,704 ft-lbs	1		04-02-06
Fnd Shear		5,785 lbs	18%	_	07-04-06
Total Load Deff	("2400) (666/1	n/a	n/a	4	04-02-06
live Load Defi	1/999 (0.046")	n/a	n/a	2	04-02-06
Max Defl.	0.075"	n/a	n/a	4	04-02-06
Span / Depth	10.3	n/a	n/a		00-00-00

		Material		Hanger
Demand/	Resistance	Member	23.2%	79%
Demand/ Demand/	Resistance Resistance	Support Member Material	34.9%	n/a
		Demand	t" 826 lbs	1,237 lbs
		Dim.(L x W) Demand	1-11/16"×1-3/4" 826 lbs	2" ×1-3/4"
		Bearing Supports	Post	B1 Hanger
		Bear	8	<u></u>

Notes

Design meets Code minimum (L/240) Total load deflection criteria

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor : Normal Part code : Part 9 Deflections less than 1/8" were ignored in the results.

S. KATSOULAKOS 33 Oldalino A DUNCE OF

DWONO.TAM 45256-17 Structural COMPONENT

BC CALC® Design Report

Build 4340

Job Name:

Address:

Customer:

City, Province, Postal Code:,

Code reports:

CCMC 12472-R

File Name: S32-9.mmdl Description: Designs\Flush Beams\Basment\Flush Beams\B3(i11;

Designer:

Specifier:

Company:

Msc.

Disclosure

Completeness and accuracy of input must current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6899 before installation. be verified by anyone who would rely on particular application. Output here based Installation of BOISE engineered wood products must be in accordance with output as evidence of suitability for on building code-accepted design properties and analysis methods.

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S. KATSOULAKOS S. OINNE POVINCE OF

DWO NO. TAN 4525817 STRUCTURAL COMPONENT

Boise Cascado

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:33

BC CALC® Design Report **Build 4340**

Code reports: **Customer:**

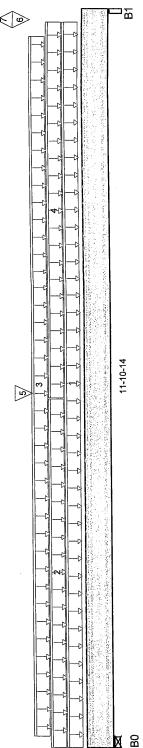
Address: City, Province, Postal Code:,

Job Name:

CCMC 12472-R

File Name: S32-9.mmdl
Description: Designs\Flush Beams\Basment\Flush Beams\B4(i1163)
Specifier:
Designer:
Company:

Town of Innisfii Certified 09/03/2018 1:51:55 PM kgervais



Total Horizontal Product Length = 11-10-14

				,					
Reaction Summary (Dow Be aring	own / Uplift) (Ibs Live	s) Dead		Snow	Wind				
B0, 2-3/8"	479/0		0/						
B1, 5"	2,614 / 807		1,439/0 0	0/1					
in committee in					Live De	Dead S	Snow Wind	рı	Trib.
Load Summary Tag Description	Load Type	Ф	Ref. Start	E G	1.00 0.1	0.65	1.00 1.15	5	
1 FC1 Floor Material	Unf. Lin. (lb/ft)	(lb/ft)	L 00-00-00	11-08-06	20 10	0			n/a
2 FC1 Floor Material	Unf. Lin.	(lb/ft)	L 00-00-00	05-07-06	9				n/a
3 UserLoad	Unf. Lin. (lb/ft)	(lb/ft)	L 00-02-02	11-05-10	09	0			n/a
4 FC1 Floor Material	Unf. Lin. (lb/ft)	(lb/ft)	L 05-07-06	11-08-06	20 10	0			n/a
5 B1(11655)	Conc. Pt. (lbs)	(sql)	L 05-08-04	05-08-04	596	306			n/a
6 13(16946)	Conc. Pt. (lbs)	(lbs)	L 11-08-10	11-08-10 2,108		786 -1	_		n/a
7 13(16946)	Conc. Pt. (lbs)	(lbs)	L 11-08-10	11-08-10	-807				n/a
	Factored	Factored	Demand /	Load	Location				
Controls Summary	Demand	Resistance	Resistance	Case					
Pos Moment	6.216 ft-lbs	25,408 ft-lbs	24.5%	-	05-08-03	~			
End Shear	1.476 lbs	11,571 lbs	12.8%	_	10-08-06	"			
Total Load Defl.	L/728 (0.188")	0.571"		56	05-10-01				
live Load Defi.	("680.0) (666/")	n/a	n/a	83	05-10-01				
MaxDeff	0.188"	n/a	n/a	56	05-10-01				
Span / Depth	14.4	n/a	n/a		00-00-00	_			
			Demand/ D	De man d/				or included by	
			9	Resistance		ą	SCHAN	1510	
Bearing Supports	Dim. (L x W)	Demand	Support N	Member	Material	dental	O. C. C.		
Bo Wall/Plate	2-3/8" × 3-1/2"	1,517 lbs	34.2%	15%	Unspecified	03	G C (2)		
	5" x3-1/2"	5,720 lbs	61.2%	26.8%	Unspecified	SA:	The second second	THE STREET	
						S KATSOULANDS	S KATSOULANDS	ULANOS WE	

Olyphy S. KATSOULAKOS ADVINCE OF TICE

DWOND.TAM4526917 STRUCTURAL COMPONENT ONLY

Notes

File Name: S32-9.mmdl Description: Designs\Flush Beams\Basment\Flush Beams\B4(i11t

S32-9.mmdl

Designer: Company:

Msc.

Specifier:

BC CALC® Design Report

Build 4340 Job Name:

Addres s:

City, Province, Postal Code:,

Customer

Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA 086. Calculations assume Member is Fully Braced.

CONFORMS TO OBC 2012 Unbalanced snow loads determined from building geometry were used in selected product's BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSAO86.

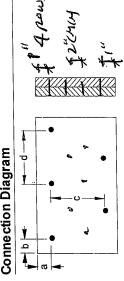
verification.

Design based on Dry Service Condition.

Part code: Part 9 Importance Factor: Normal

Deflections less than 1/8" were ignored in the results.

Connection Diagram



c=**3**-1/2" a minimum = 2" b minimum = 3" Calculated Side Load = 111.1 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: "

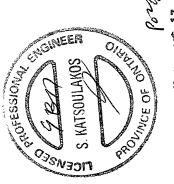
Nails

312" ARDOX SPIRAL

Disclosure

building codes. To obtain Installation Guide Completeness and accuracy of input must be verified by anyone who would rely on particular application. Output here based current Installation Guide and applicable Installation of BOISE engineered wood products must be in accordance with output as evidence of suitability for or ask questions, please call 1-800-964-6999 before installation. on building code-accepted design properties and analysis methods

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DWG NO. TAM 45259-17 COMPONENT ONLY STRUCTURAL

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 11, 2017 10:49:01

Build 5033 Job Name:

Addres s: City, Province, Postal Code:, Customer

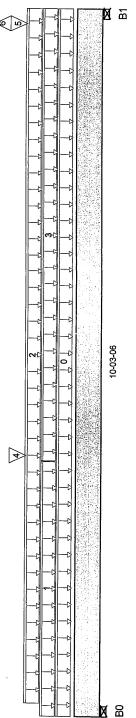
CCMC 12472-R Code reports:

File Name: S32-9-15G.mmdl
Description: Designs\Flush Beams\Basment\Flush Beams\B5(i13033)
Specifier:

Company. Designer:

Msc.

09/03/2018 1:51:59 PM kgervais



10-03-06
gth =
t Len
Produc
Horizontal
Total

									ı
Reaction Summary (Down / Uplift) (1bs Bearing Live	Down / Uplift) (II	bs) Dead		Snow	Wind	*			
B0, 2-3/8"	428/0		0/1						
511.0	43012	930/0	0/1				**		
Load Summary					Live	Dead	Snow Wind	d Trib.	<u>ن</u>
Tag Description	Load Type	be	Ref. Start	폡	1.00	0.65	1.00 1.15		
0 FC1 Floor Material	Unf. Lin. (lb/ft)	. (Ib/ft)	00-00-00 T	10-03-06	16	8		2	<u> </u>
1 FC1 Floor Material	Unf. Lin. (lb/ft)	(lb/ft)	O0-00-00 T	03-08-06	က			2	n/a
2 UserLoad	Unf. Lin. (lb/ft)	. (Ib/ff)	L 00-02-03	10-03-06		99		ב	n/a
3 FC1 Floor Material	Unf. Lin. (lb/ff)	(lp/ff)	L 03-08-06	10-03-06	4			ď	n/a
4 B3(i12947)	Conc. Pt. (lbs)	t (lbs)	L 03-09-04	03-09-04	527	75		2	ص
5 6(i144)	Conc. Pt. (lbs)	t. (lbs)	L 10-00-10	10-00-10	129	95		. 70	[A
6 6(i144)	Conc. Pt. (lbs)	t. (lbs)	L 10-00-10	10-00-10	7			u /u	□⁄a
	Factored	Factored	Demand /	Load	Location				
Controls Summary	Demand	Resistance	Resistance	Case					
Pos. Moment	4,239 ft-lbs	12,704 ft-lbs	33.4%	_	03-09-04	4			
End Shear	1,283 lbs	5,785 lbs	22.2%	_	00-11-14	4			
Total Load Deff.	L/625 (0.187")	0.488"		9	04-09-11	. —			
Live Load Defl.	("80:0) 666/1	n/a		80	04-08-10				
Max Defl.	0.187"	n/a	n/a	9	04-09-11	_			
Span / Depth	12.3	n/a	n/a		00-00-00				
			Demand/ D	De m an d/			and the same of th	SSION TO STANK	
			Resistance R	Resistance			AC	1	
Bearing Supports	Dim . (L x W)	De mand	Support N	Member	Material		0.		*************
B0 Wall/Plate	2-3/8" x 1-3/4"	1,389 lbs	62.6%	27.4%	Unspecified		PSI	1041	ملط - تعلاد محرج والمال
B1 Wall/Plate	5-1/2" x 1-3/4"	1,433 lbs	27.9%	12.2%	Unspecified		Vac	SOX MULTONIA AKOS	F
Notes)N	ALSOO E	diameter.

POLINCE OF OTHER

DWO ND, TAM 4646.17 Structural

Y INO

COMPONENT



Boise Cascade Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i13033)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 11, 2017 10:49:01

Build 5033

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: \$32-9-15G.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5(i130:

Specifier:

Designer: Company.

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B6(i11294)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:34

BC CALC® Design Report



Build 4340 Job Name: Address:

City, Province, Postal Code:, Customer:

Code reports:

CCMC 12472-R

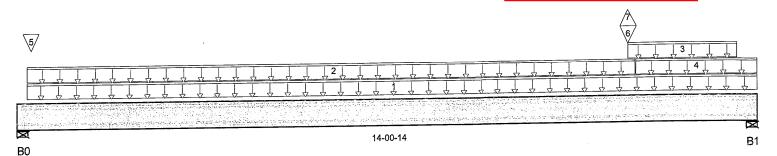
File Name: \$32-9.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B6(i11294

Specifier: Designer: Company:

Misc:

09/03/2018 1:52:09 PM kgervais



Total Horizontal Product Length = 14-00-14

Reaction Summary (Do	wn / Uplift) (lbs) Live	De ad	Snow	Wind	_
B0, 5-1/2"	218/0	253/0	16 / 0		
B1, 4-3/8"	600/3	727/0	182/0		

						Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type	Ref. St	tart	End	1.00	0.65	1.00	1.15	
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-	02-04	14-00-14	8	4			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-	02-04	11-09-08	12	6			n/a
2	UserLoad	Unf. Lin. (lb/ft)	L 11-	07-12	13-08-08	33	130	96		n/a
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L 11-	09-08	14-00-14	6				n/a
5	FC2 Floor Material	Conc. Pt. (lbs)	L 00-	03-02	00-03-02		29	,		n/a
6	B7 (i11318)	Conc. Pt. (lbs)	L 11-	07-12	11-07-12	491	418			n/a
7	B7 (i11318)	Conc. Pt. (lbs)	L 11-	07-12	11-07-12	-3				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,364 ft-lbs	25,408 ft-lbs	13.2%	1	11-07-12
End Shear	1,647 lbs	11,571 lbs	14.2%	1	12-11-00
Total Load Defl.	L/1,093 (0.147"	0.669"	22%	58	07 - 07-00
Live Load Defl.	L/999 (0.074")	n/a	n/a	85	07-07-00
Max Defl.	0.147"	n/a	n/a	58	07-07-00
Span / Depth	16.9	n/a	n/a		00-00-00

Poo	ring Sunnorte	Dim . (L × W)	De man d	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	ring Supports Wall/Plate	5-1/2" x 3-1/2"	651 lbs	6.3%	2.8%	Unspecified
B1	Wall/Plate	4-3/8" x 3-1/2"	1,900 lbs	23.2%	10.2%	Unspecified

Notes



DWG NO. TAM 4526117 STRUCTURAL COMPONENT ONLY

BC CALC® Design Report

Build 4340

Job Name:

Address: City, Province, Postal Code:,

Customer:

Code reports:

reports: CCMC

CCMC 12472-R

File Name: S32-9.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B6(i112

Specifier:

Designer: Company:

CONFORMS TO DBC 2012

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

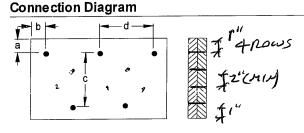
Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86. Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9
Deflections less than 1/8" were ignored in the results.



a minimum = $\mathbf{1}^{"}$ c = $\mathbf{3}$ -1/2" b minimum = 3" d = $\mathbf{6}^{"}$

Calculated Side Load = 93.6 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

Nails 3½" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWO NO . TAM45261 -17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7(i11318)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:34



BC CALC® Design Report

Build 4340 Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

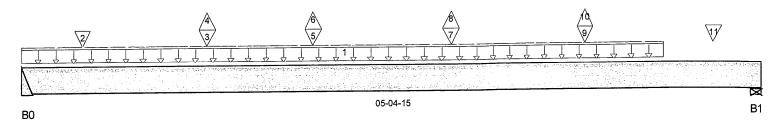
File Name: S32-9.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B7(i1131

Specifier: Designer: Company:

Misc:

09/03/2018 1:52:17 PM kgervais



Total Horizontal Product Length = 05-04-15

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
В0	499/3	425/0							
B1 5-1/2"	464/4	490/0	81 / 0						

	I O					Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type	Load Type Ref. Star		Start End 1		0.65	.65 1.00		
1	Us er Load	Unf. Lin. (lb/ft)	L	00-00-00	04-08-08		60			n/a
2	J4(i11256)	Conc. Pt. (lbs)	L	00-05-04	00-05-04	176	87			n/a
3	J2(i11271)	Conc. Pt. (lbs)	L	01-04-04	01-04-04	164	82			n/a
4	J2(i11271)	Conc. Pt. (lbs)	Ĺ	01-04-04	01-04-04	-1				n/a
5	J2(i9515)	Conc. Pt. (lbs)	L	02-01-08	02-01-08	173	86			n/a
6	J2(i9515)	Conc. Pt. (lbs)	L	02-01-08	02-01-08	-2				n/a
7	J2(i9684)	Conc. Pt. (lbs)	L	03-01-08	03-01-08	196	97			n/a
8	J2(i9684)	Conc. Pt. (lbs)	L	03-01-08	03-01-08	- 2				n/a
9	J2(i9481)	Conc. Pt. (lbs)	L	04-01-08	04-01-08	193	96			n/a
10	J2(i9481)	Conc. Pt. (lbs)	L	04-01-08	04-01-08	-2				n/a
11	B8 (i8634)	Conc. Pt. (lbs)	Ĺ	05-00-12	05-00-12	54	129	81		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,494 ft-lbs	25,408 ft-lbs	5.9%	1	02-02-04
End Shear	1,046 lbs	11,571 lbs	9%	1	04-01-15
Total Load Defl.	L/999 (0.009")	n/a	n/a	58	02-06-12
Live Load Defl.	L/999 (0.005")	n/a	n/a	85	02-06-12
MaxDefl.	0.009"	n/a	n/a	58	02-06-12
Span / Depth	6.2	n/a	n/a		00-00-00

Bearing Supports		Dim . (L × W)	De man d	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B0	Hanger	2" x3-1/2"	1,280 lbs	n/a	15%	HGUS410	
B1	Wall/Plate	5-1/2" x 3-1/2"	1,349 lbs	13.1%	5.7%	Unspecified	

Notes



BC CALC® Design Report

Build 4340

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-9.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\87(i113

Specifier:

Designer: Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

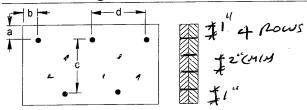
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86. Unbalanced snow loads determined from building geometry were used in selected product's verification.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9
Deflections less than 1/8" were ignored in the results.

Connection Diagram



Calculated Side Load = 364.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Nails 3½ ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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ALLJOIST®, BC RIM BOARD™, BCI®,
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PLUS®, VERSA-RIM®,
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STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B8(i8634)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:34

Build 4340

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-9.mmdl

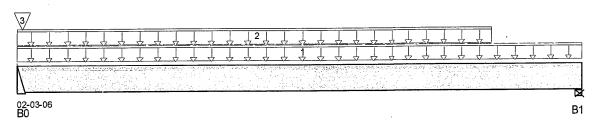
Description: Designs\Flush Beams\1st Floor\Flush Beams\B8(i8634)

Specifier:

Designer: Company: Misc:

Town of innisfii Certified Model

09/03/2018 1:52:22 PM kgervais



Total Horizontal Product Length = 02-03-06

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0	58 / 0	139/0	88 / 0						
B1. 4-3/8"	55 / 0	123/0	74 / 0						

	oad Summary					Live	Dead	Snow	Wind	Trib.
	ng Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-03-06	24	9			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	01-11-00	29	114	84		n/a
3	FC2 Floor Material	Conc. Pt. (lbs)	L	00-00-04	00-00-04	2				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	140 ft-lbs	25,408 ft-lbs	0.6%	13	01-00-08
End Shear	26 lbs	11,571 lbs	0.2%	13	00-11-08
Total Load Defl.	L/999 (0")	n/a	n/a	45	01-00-08
Live Load Defl.	L/999 (0")	n/a	n/a	61	01-00-08
Max Defl.	0"	n/a	n/a	45	01-00-08
Span / Depth	2.4	n/a	n/a		00-00-00

				De m an d/	De man d/	
				Resistance	Resistance	
Bea	ring Supports	Dim . (L x W)	De m an d	Support	Member	Material
B0	Hanger	2" x 3-1/2"	334 lbs	n/a	3.9%	HGUS410
B1	Wall/Plate	4-3/8" x 3-1/2"	292 lbs	3.6%	1.6%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's

CONFORMS TO OBC 2012

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9 Deflections less than 1/8" were ignored in the results.



DWG NO. TAM 45263-17 STRUCTURAL COMPONENT ONLY

BC CALC® Design Report

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-9.mmdl

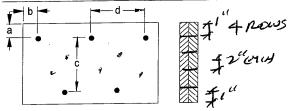
Description: Designs\Flush Beams\1st Floor\Flush Beams\88(i863

Specifier:

Designer: Company:

Misc:

Connection Diagram



Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d 5 A Nails

312" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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BWB NO. FAM 45263-17 STRUCTURAL COMPONERT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10 DR(i11299)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:35

BC CALC® Design Report



Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: \$32-9.mmdl

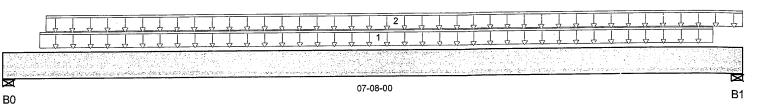
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1(

Specifier:
Designer:
Company:

Misc:

Town of Innisfii Certified Model

09/03/2018 1:52:24 PM kgervais



Total Horizontal Product Length = 07-08-00

Reaction Summary (Down / Uplift) (lbs)							
Be aring	Live	De ad	Snow	Wind			
B0,4"	1,849 / 0	1,168 / 0					
B1, 4"	1,893/0	1,216/0					

ما	ad Summan				Live	Dead	Snow Wind	Trib.
	ad Summary g Description	Load Type	Ref. Start	End	1.00	0.65	1.00 1.15	
1	Smoothed Load	Unf. Lin. (lb/ft)	L 00-04-08	07-04-08	534	268		n/a
2	Us er Load	Unf. Lin. (lb/ft)	L 00-05-04	07-08-00		60		n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	7,347 ft-lbs	25,408 ft-lbs	28.9%	1	03-10-08
End Shear	3,558 lbs	11,571 lbs	30.7%	1	01-01-08
Total Load Defl.	L/999 (0.096")	n/a	n/a	4	03-10-08
Live Load Defl.	L/999 (0.058")	n/a	n/a	5	03-10-08
Max Defl.	0.096" [`]	n/a	n/a	. 4	03-10-08
Span / Depth	9	n/a	n/a		00-00-00

Bearin	ng Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Wall/Plate	4" ×3-1/2"	4,233 lbs	37.2%	24.8%	Unspecified
B1	Wall/Plate	4" ×3-1/2"	4,359 lbs	38.3%	25.5%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced length .

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBG 2012



DWO NO . TAM 45267-17 STRUCTURAL COMPONENT ONLY

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

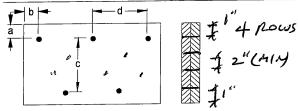
File Name: S32-9.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B

Specifier: Designer:

Company: Misc:

Connection Diagram



a minimum = 2" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d ∧ Nails

312" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWO NO . TAM 45264-17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11 DR(i12307)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 7, 2017 10:49:02

BC CALC® Design Report



Build 5033 Job Name:

Address: City, Province, Postal Code:,

Customer: Code reports:

CCMC 12472-R

File Name: S32-9-15G.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B11

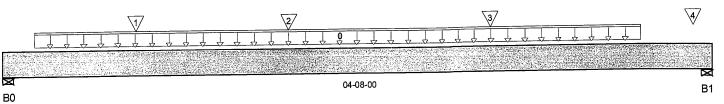
Specifier:

Designer: Company:

Misc:

Town of innisfii Certified Model

09/03/2018 1:52:26 PM kgervais



Total Horizontal Product Length = 04-08-00

Reaction Summary (Down	n / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 4"	1,018/0	650/0	•		
B1, 4"	1,220/0	807/0			

			Live	Dead	Snow Wind	i rib.
Load Summary Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0 Smoothed Load	Unf. Lin. (lb/ft)	L 00-02-08	04-02-08 228	114		n/a
1 J3(i12227)	Conc. Pt. (lbs)	L 00-10-08	00-10-08 274	197		n/a
	Conc. Pt. (lbs)	1 01-10-08	01-10-08 320	231	,	n/a
2 J3(i12223)	Conc. Pt. (lbs)	1 03-02-08	03-02-08 366	264		n/a
3 J3(i12230)	• •	_ 00 02 03	04-06-08 366	264		n/a
4 J3(i12233)	Conc. Pt. (lbs)	L 04-00-00	0.0000			

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	2,510 ft-lbs	25,408 ft-lbs	9.9%	1	02-02-08
End Shear	1.946 lbs	11.571 lbs	16.8%	1	03-06-08
Total Load Defl.	L/999 (0.011")	n/a	n/a	4	02-04-00
Live Load Defl.	L/999 (0.007")	n/a	n/a	5	02-04-00
Max Defl.	0.011"	n/a	n/a	4	02-04-00
Span / Depth	5.2	n/a	n/a		00-00-00

_		Dim . (L x W)	De man d	De mand/ Re sistance Support	Demand/ Resistance Member	Material
Bearing Supports				40.70/	Unanasified	
B0	Wall/Plate	4" x 3-1/2"	2,340 lbs	20.6%	13.7%	Unspecified
B1	Wall/Plate	4" x 3-1/2"	2,8381bs	25%	16.6%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-03-06, Bottom: 00-03-06.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012



DWO NO . TAM 45265.. 17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11 DR(i12307)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 7, 2017 10:49:02

BC CALC® Design Report

Build 5033 Job Name:

City, Province, Postal Code:,

Customer:

Address:

Code reports:

CCMC 12472-R

File Name: S32-9-15G.mmdl

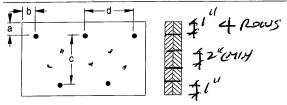
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1

Specifier: Designer:

Company:

Misc:

Connection Diagram



a minimum = 1" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Nails

312" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWO NO . TAM 45265-17 STRUCTURAL RAMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12 DR(i13097)



CCMC 12472-R

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 11, 2017 10:48:29

В1

BC CALC® Design Report Build 5033

Job Name: Address:

City, Province, Postal Code:,

Customer:

B0

Code reports:

H-1031 1 1033

File Name: S32-9-15G.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B12

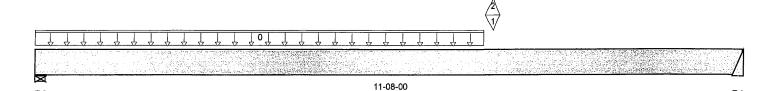
Specifier: Designer:

Company:

Misc:

Town of innisfii Certified Mode

09/03/2018 1:52:28 PM kgervais



Total Horizontal Product Length = 11-08-00

 Reaction Summary (Down / Uplift) (Ibs)

 Bearing
 Live
 Dead
 Snow
 Wind

 B0, 5-1/2"
 509/0
 351/0

 B1
 842/1
 528/0

Lc	ad Summary					Live	Dead	Snow Wind	Trib.	
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	R1 (i12898)	Unf. Lin. (lb/ft)	L	00-00-00	07-04-08	8	6			n/a
1	B14(i13150)	Conc. Pt. (lbs)	L	07-06-04	07-06-04	1,293	720			n/a
2	B14(i13150)	Conc. Pt. (lbs)	L	07-06-04	07-06-04	-1				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	7,669 ft-lbs	24,074 ft-lbs	31.9%	1	07-06-04
End Shear	1,912 lbs	11,571 lbs	16.5%	1	10-08-08
Total Load Defl.	L/685 (0.196")	0.558"	35%	6	06-05-00
Live Load Defl.	L/999 (0.121")	n/a	n/a	8	06-05-00
Max Defl.	0.196"	n/a	n/a	6	06-05-00
Span / Depth	14.1	n/a	n/a		00-00-00

Beari	ng Supports	Dim.(L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	1,202 lbs	7.7%	5.1%	Unspecified
B1	Hanger	2" x 3-1/2"	1,924 lbs	n/a	22.5%	HGUS410

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 03-11-00, Bottom: 03-11-00

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBG 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWO NO. TAM 46469-17
STRUCTURAL
GOMPONENT ONLY

PROFESSION



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12 DR(i13097)

September 11, 2017 10:48:29

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

File Name: S32-9-15G.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1

Specifier:

Designer: Company:

City, Province, Postal Code:, Customer:

Build 5033

Job Name:

Address:

Code reports:

CCMC 12472-R

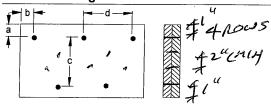
Misc:

Disclosure

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a minimum = 1" b minimum = 3" d = 920

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Nails ARDOX SPIRAL

ON THE OF ON THE

DWO NO . TAM 46469-17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13(i13168)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 11, 2017 10:48:28

Build 5033 Job Name:

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

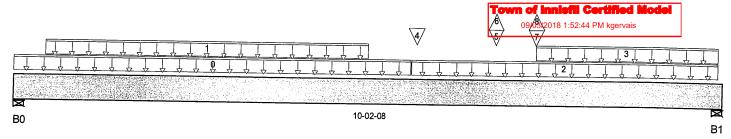
CCMC 12472-R

File Name: S32-9-15G.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B13(i1316i

Specifier:
Designer:
Company:

Misc:



Total Horizontal Product Length = 10-02-08

Reaction Summary (Dow	n / Uplift) (lbs)				
Be aring	Live	De ad	Snow	Wind	
B0, 5-1/2"	340/2	454/0			
B1, 3-1/2"	656/4	503/0			

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Та	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	05-08-08	24	12			
1	Us er Load	Unf. Lin. (lb/ft)	Ĺ	00-05-06	05-01-01		60			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	ī	05-08-08	10-01-12	21	11			n/a
3	FC2 Floor Material	Unf. Lin. (lb/ft)	ī	07-06-04	10-01-12		11			n/a
4	B15(i13149)	Conc. Pt. (lbs)	ı	05-09-04	05-09-04	-	00			n/a
5	J5(i13064)	Conc. Pt. (lbs)	L.				68			n/a
	` '	` '	L	06-11-04	06-11-04	68	32			n/a
6	J5(i13064)	Conc. Pt. (lbs)	L	06-11-04	06-11-04	-5				
7	B14(i13150)	Conc. Pt. (lbs)	- 1	07-06-04	07-06-04	573	353			n/a
8	B14(i13150)	` '				•.•	303			n/a
•	2. ((10100)	Conc. Pt. (lbs)	L	07-06-04	07-06-04	-1				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,792 ft-lbs	25,408 ft-lbs	14.9%	1	06-11-03
End Shear	1,542 lbs	11,571 lbs	13.3%	1	09-01-08
Total Load Defl.	L/999 (0.086")	n/a	n/a	6	05-04-12
Live Load Defl.	L/999 (0.045")	n/a	n/a	8	05-06-10
Max Defl.	0.086"	n/a	n/a	6	05-04-12
Span / Depth	12.1	n/a	n/a	•	00-00-00

	ing Supports	Dim. (L x W)	De man d	De man d/ Re s istance Support	Demand/ Resistance Member	Material
B0 B1	Wall/Plate Wall/Plate	5-1/2" x 3-1/2" 3-1/2" x 3-1/2"	1,076 lbs 1,612 lbs	10.5% 24.6%	4.6% 10.8%	Unspecified Unspecified
Notes	6		•		10.070	on opedited



DWO NO. TAM 46472₁₇ STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13(i13168)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 11, 2017 10:48:28

Build 5033

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: \$32-9-15G.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B13(i13

Specifier:

Designer: Company:

CONFORMS TO DBC 2012

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

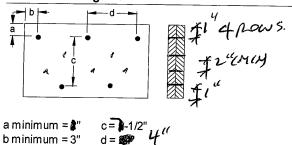
Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Connection Diagram



Calculated Side Load = 163.5 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

∕\ ½" ARDOX SPIRAL

S. KATSOULAKOS S. S. WATSOULAKOS S. WATSOULAKOS

DWO NO .TAM 46470-17 STRUCTURAL COMPENENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i13150)



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 11, 2017 10:48:28

BC CALC® Design Report Build 5033 Job Name:

Address: City, Province, Postal Code:,

Customer:

Code reports:

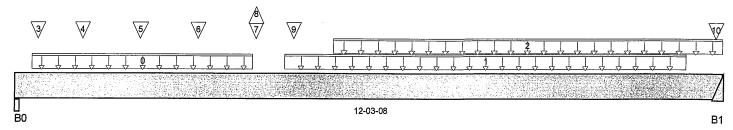
CCMC 12472-R

File Name: S32-9-15G.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i1315)

Specifier: Designer: Company: Misc:

09/03/2018 1:52:48 PM kgervais



Total Horizontal Product Length = 12-03-08

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0, 3-1/2"	1,288 / 1	717/0							
B1	576 / 1	355/0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	Us er Load	Unf. Lin. (lb/ft)	L	00-03-08	04-01-08	240	120			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	04-08-00	11-08-00	52	26			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	05-06-01	12-03-08	10	5			n/a
3	J6(i12972)	Conc. Pt. (lbs)	L	00-04-12	00-04-12	58	29			n/a
4	J6(i12988)	Conc. Pt. (lbs)	L	01-02-00	01-02-00	83	41			n/a
5	J6(i13047)	Conc. Pt. (lbs)	L	02-02-00	02-02-00	93	47			n/a
6	J6(i12980)	Conc. Pt. (lbs)	L	03-02-00	03-02-00	90	45			n/a
7	J6(i13059)	Conc. Pt. (lbs)	L	04-02-00	04-02-00	46	22			n/a
8	J6(i13059)	Conc. Pt. (lbs)	L	04-02-00	04-02-00	-2				n/a
9	B15(i13149)	Conc. Pt. (lbs)	L	04-09-13	04-09-13	96	70			n/a
10	J7(i13018)	Conc. Pt. (lbs)	L	12-02-00	12-02-00	33	17			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	5,614 ft-lbs	25,408 ft-lbs	22.1%	1	04-09-13
End Shear	2,301 lbs	11,571 lbs	19.9%	1	01-01-00
Total Load Defl.	L/727 (0.198")	0.598"	33%	6	05-10-00
Live Load Defl.	L/999 (0.124")	n/a	n/a	8	05-10-00
Max Defl.	0.198"	n/a	n/a	6	05-10-00
Span / Depth	15.1	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	Demand	De man d/ Re s istance Support	Demand/ Resistance Member	Material
B0	Beam	3-1/2" x 3-1/2"	2,829 lbs	21.2%	18.9%	Unspecified
B1	Hanger	2" x 3-1/2"	1,307 lbs	n/a	15.3%	HGUS410

Notes

POVINCE OF ON THE DWO NO . TAM 46471-17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i13150)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 11, 2017 10:48:28

Build 5033

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: \$32-9-15G.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i13*

Specifier:

Designer: Company:

CONFORMS TO OBC 2012

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

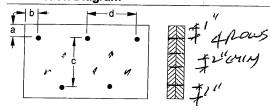
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Connection Diagram



a minimum = $\mathbf{1}$ " $c = \mathbf{1} - 1/2$ " $d = \mathbf{2}$

Calculated Side Load = 132.3 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

ur Nails

Connectors are: **

3%" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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S. KATSOULAKOS S. S. VACE OF ONTERS

Page 2 of 2

DWO NO . TAM 4642 (-17 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B15(i11685)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:35

BC CALC® Design Report



Build 4340 Job Name:

Address: City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-9.mmdl

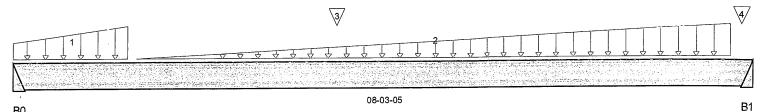
Description: Designs\Flush Beams\1st Floor\Flush Beams\B15(i116)

Specifier: Designer: Company:

Misc:

n of innisfii Certified Mo

12/03/2018 8:44:38 AM kgervais



B0

Total Horizontal Product Length = 08-03-05

Reaction Summary (D	own / Uplift) (lbs)				
Be aring	Live	De ad	Snow	Wind	
B0	85 / 0	62 / 0			
B1	108/0	74 / 0			

	ad Cummanı					Live 1.00	Dead 0.65	Snow	/ Wind 1.15	Trib.
	ad Summary Description	Load Type	Ref. Start I					1.00		
1	FC2 Floor Material	Trapezoidal (lb/ft)	L	00-00-00		3	1			n/a
•		,			01-03-06	9	5			n/a
2	FC2 Floor Material	Trapezoidal (lb/ft)	L	01-04-07		0	0			n/a
_	1 OZ 1 TOOI Matoriai				08-00-07	26	13			n/a
3	J6(i11621)	Conc. Pt. (lbs)	L	03-07-07	03-07-07	91	45			n/a
4	FC2 Floor Material	Conc. Pt. (lbs)	L	08-01-14	08-01-14	6	3			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	612 ft-lbs	12,704 ft-lbs	4.8%	1	03-07-07
End Shear	242 lbs	5,785 lbs	4.2%	1	07-03-13
Total Load Defl.	L/999 (0.018")	n/a	n/a	4	04-01-00
Live Load Defl.	L/999 (0.011")	n/a	n/a	5	04-01-00
Max Defl.	0.018" [^]	n/a	n/a	4	04-01-00
Span / Depth	10.2	n/a	n/a		00-00-00

Bea	ring Supports	Dim . (L x W)	De man d	De mand/ Re sistance Support	Resistance Member	Material
B0	Hanger	2" x 1-3/4"	205 lbs	n/a	4.8%	LS 90
B1	Hanger	2" x 1-3/4"	255 lbs	n/a	6%	LS 90

Notes



DWG NO . TAM45269-17 STRUCTURAL COMPONENT ONLY

BC CALC® Design Report

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-9.mmdl

Description: Designs \Flush Beams\1st Floor\Flush Beams\B15(i11

Specifier: Designer:

Company:

CONFORMS TO OBG 2012

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced

length.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

Deflections less than 1/8" were ignored in the results.

Disclosure

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DWO NO. TAM 45 26217 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B16(i11575)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:35

BC CALC® Design Report



Build 4340 Job Name:

Address: City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

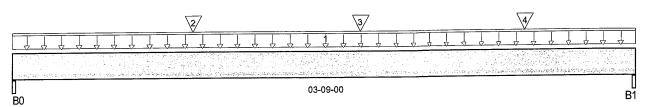
File Name: S32-9.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B16(i115`

Specifier: Designer: Company:

Misc:

12/03/2018 8:44:45 AM kgervais



Total Horizontal Product Length = 03-09-00

Reaction Summary (Dow	n / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5"	520/0	763/0	780/0		
B1, 5"	577/0	791/0	780/0		

Load Summon				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
1 User Load	Unf. Lin. (lb/ft)	L 00-00-00	03-09-00	143	330	416	n/a
2 J5(i11612)	Conc. Pt. (lbs)	L 01-01-00	01-01-00	189	95		n/a
3 J5(i11603)	Conc. Pt. (lbs)	L 02-01-00	02-01-00	200	100		n/a
4 J5(i11596)	Conc. Pt. (lbs)	L 03-01-00	03-01-00	167	83		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,552 ft-lbs	25,408 ft-lbs	6.1%	13	01-11-00
End Shear	1,034 lbs	11,571 lbs	8.9%	1	01-02-08
Total Load Defl.	L/999 (0.004")	n/a	n/a	45	01-10-06
Live Load Defl.	L/999 (0.002")	n/a	n/a	61	01-10-06
Max Defl.	0.004"	n/a	n/a	45	01-10-06
Span / Depth	3.8	n/a	n/a		00-00-00

				De m an d/	Demand/	
				Resistance	Resistance	
Bear	ing Supports	Dim . (L x W)	Demand	Support	Member	Material
B0	Beam	5" x 3-1/2"	2,384 lbs	25.5%	11.2%	Unspecified
B1	Beam	5" x 3-1/2"	2,448 lbs	26.2%	11.5%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086 Unbalanced snow loads determined from building geometry were used in selected product \$ 0NF ORMS TO 0BC 2012 verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9 Deflections less than 1/8" were ignored in the results.



STRUCTURAL

COMPONENT OKLY

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

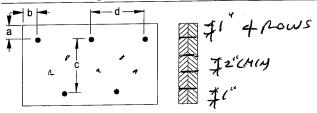
File Name: S32-9.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\816(i11

Specifier: Designer:

Company: Misc:

Connection Diagram



a minimum = 2" c=3-1/2" b minimum = 3" d = 3 6

Calculated Side Load = 326.2 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

312" ARDOX SPIRAL

Disclosure

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DWG NO. TAM 45270-17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B17 DR(i11570)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:35

BC CALC® Design Report



Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-9.mmdl

Description: Designs\Dropped Beams\1stFloor\Dropped Beams\B17

Specifier: Designer: Company:

Misc:

Town of innisfii Certified Model

12/03/2018 8:44:47 AM kgervais

\(\frac{\beta}{1} \)
\(\frac{\delta}{3} \)
\(\frac{\delta}{4} \)
\(\frac{\delta}{5} \)
\(\frac{\delta}{3} \)

Total Horizontal Product Length = 04-07-03

Reaction Summary (Down / Uplift) (Ibs)						
Be aring	Live	De ad	Snow	Wind		
B0, 5-1/4"	2,027 / 807	711/0	0/1			
B1 5-1/2"	716/0	380/0				

Loade	umma m/					Live	Dead	Snow	w Wind	i rib.
Load Summary Tag Description		Load Type	Ref. Start		End	1.00	0.65	1.00 1	1.15	
1 -		Conc. Pt. (lbs)	L	00-04-08	00-04-08	1,387	369	-1		n/a
2 -		Conc. Pt. (lbs)	L	00-04-08	00-04-08	-807				n/a
3 J2(i	9515)	Conc. Pt. (lbs)	L	01-03-12	01-03-12	416	208			n/a
,	9684)	Conc. Pt. (lbs)	L	02-03-12	02-03-12	470	235			n/a
5 J2(i	9481)	Conc. Pt. (lbs)	L	03-03-12	03-03-12	470	235			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	1,901 ft-lbs	25,408 ft-lbs	7.5%	1	02-03-12
End Shear	1,535 lbs	11,571 lbs	13.3%	1	03-04-03
Uplift	571 lbs	n/a	n/a	12	00-00-00
Total Load Defl.	L/999 (0.007")	n/a	n/a	56	02-03-06
Live Load Defl.	L/999 (0.004")	n/a	n/a	83	02-03-06
MaxDefl.	0.007"	n/a	n/a	56	02-03-06
Span / Depth	4.8	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Bear	ing Supports	Dim . (L x W)	De m an d	Support	Member	Material
B0	Wall/Plate	5-1/4" x 3-1/2"	3,929 lbs	26.3%	17.5%	Unspecified
B1	Wall/Plate	5-1/2" x 3-1/2"	1,550 lbs	9.9%	6.6%	Unspecified

Cautions

Uplift of 571 lbs found at span 1 - Left. (SIMPSON 1-42-57 @ O. Bo)

Notes

S. KATSOULAKOS STORY OF ONLINE OF ON

DWG NO. TAM45201-17 STRUCTURAL COMPONENT ONLY

Build 4340

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-9.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B

Specifier:

Designer: Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced length .

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

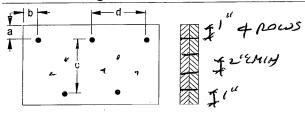
Unbalanced snow loads determined from building geometry were used in selected product's verification.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9
Deflections less than 1/8" were ignored in the results.

Connection Diagram



a minimum = 2" c = 3-1/2" b minimum = 3" d = 200 6"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d 🔥 Nails

3½" ARDOX SPIRAL

Disclosure

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STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B18(i11690)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:40:34

BC CALC® Design Report



Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

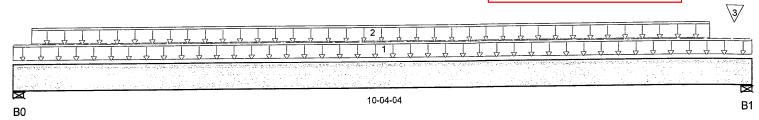
File Name: S32-9.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B18(i116

Specifier: Designer: Company: Misc:

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12/03/2018 8:44:48 AM kgervais



Total Horizontal Product Length = 10-04-04

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0, 2-3/8"	102/0	365/0							
B1. 4-3/8"	328/0	507/0							

Load Cummons				Live	Dead	Snow Wind	irib.
Load Summary Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00 1.15	
1 FC1 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	10-04-04	20	10		n/a
2 UserLoad	Unf. Lin. (lb/ft)	L 00-03-01	09-09-06		60		n/a
3 6(1144)	Conc. Pt. (lbs)	L 10-01-06	10-01-06	223	148		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,286 ft-lbs	8,258 ft-lbs	15.6%	0	05-01-02
End Shear	491 lbs	3,761 lbs	13.1%	0	00-11-14
Total Load Defl.	L/999 (0.082")	n/a	n/a	4	05-01-02
Live Load Defl.	L/999 (0.017")	n/a	n/a	5	05-01-02
Max Defl.	0.082"	n/a	n/a	4	05-01-02
Span / Depth	12.5	n/a	n/a		00-00-00

D. oui	u a Cump outo	Dim . (L x W)	De man d	De mand/ Re sistance Support	Demand/ Resistance Member	Material
Be ari	ng Supports Wall/Plate	2-3/8" x 1-3/4"		35.5%	15.5%	Unspecified
B1	Wall/Plate	4-3/8" x 1-3/4"	1,126 lbs	27.6%	12.1%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9 Deflections less than 1/8" were ignored in the results. CONFORMS TO OBC 2012

 $\mathsf{ALLJOIST} \mathsf{@} \ , \ \mathsf{BC} \ \mathsf{RIM} \ \mathsf{BOARD}^\mathsf{TM}, \ \mathsf{BC} \mathsf{l} \mathsf{@} \ .$ BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®,

or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™,

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DWO NO. TAM 4520217 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B19(i11281)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:44:44

BC CALC® Design Report

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

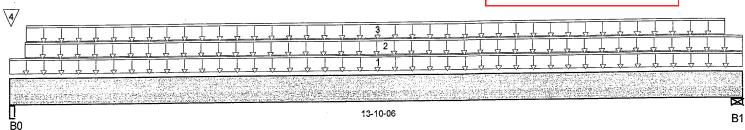
CCMC 12472-R

File Name: S32-9 sunken.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B19(i112

Specifier: Designer: Company: Misc:

12/03/2018 8:44:51 AM kgervais



Total Horizontal Product Length = 13-10-06

Reaction Summary	(Down / Uplift) (lbs)				
Be aring	Live	De ad	Snow	Wind	
B0, 2-1/2"	239/0	572/0			
B1 2-3/8"	169/0	515/0			

	1.0					Live	Dead	Snow	Wind	Trib.
	oad Summary g Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	13-10-06	21	10			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-03-06	13-10-06	4	2			n/a
<u>ر</u> غ	Us er Load	Unf. Lin. (lb/ft)	L	00-03-10	13-06-10		60			n/a
4	12(i6943)	Conc. Pt. (lbs)	L	00-00-04	00-00-04	71	56			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,485 ft-lbs	8,258 ft-lbs	30.1%	0	06-11-04
End Shear	701 lbs	3,761 lbs	18.6%	0	01-00-00
Total Load Defl.	L/525 (0.31")	0.679"	45.7%	4	06~11-04
Live Load Defl.	L/999 (0.075")	n/a	n/a	5	06-11-04
Max Defl.	0.31"	n/a	n/a	4	06-11-04
Span / Depth	17.2	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	De m an d	De mand/ Re sistance Support	Demand/ Resistance Member	Material
B0	Beam	2-1/2" x 1-3/4"	800 lbs	52.7%	23.1%	Unspecified
B1	Wall/Plate	2-3/8" x 1-3/4"	720 lbs	49.9%	21.9%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

CONFORMS TO OBG 2012

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

DWO NO. TAM 45223-17 STRUCTURAL COMPONENT ONLY

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are





Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B20 L(i11282)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 13:44:44

В1

BC CALC® Design Report



Build 4340 Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

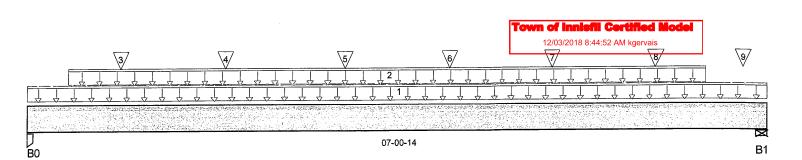
CCMC 12472-R

File Name: S32-9 sunken.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B20 L(i11

Specifier: Designer: Company:

Misc:



Total Horizontal Product Length = 07-00-14

07-00-14

Reaction Summary (Down / Uplift) (lbs)											
Be aring	Live	De ad	Snow	Wind							
B0, 3-1/2"	837/0	623/0									
B1. 4-3/8"	873/0	697/0									

10	oad Summary					Live	Dead	Snow	vvina	ı rıp.
	g Description	Load Type	Re	f. Start	End	1.00	0.65	- 1.00	1.15	
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	07-00-14	6	3			n/a
2	Us er Load	Unf. Lin. (lb/ft)	L	00-04-11	06-06-00		60			n/a
3	J1(i11050)	Conc. Pt. (lbs)	L.	00-10-10	00-10-10	241	121			n/a
4	J1 (i11095)	Conc. Pt. (lbs)	L	01-10-10	01-10-10	289	145			n/a
5	J1 (i11095)	Conc. Pt. (lbs)	L	03-00-04	03-00-04	289	145			n/a
6	J1(i11024)	Conc. Pt. (lbs)	L	04-00-04	04-00-04	271	135			n/a
7	J1 (i11024)	Conc. Pt. (lbs)	L	05-00-04	05-00-04	271	135			n/a
8	J1 (i11072)	Conc. Pt. (lbs)	L	06-00-04	06-00-04	284	142			n/a
9	1(i 127)	Conc. Pt. (lbs)	L	06-10-03	06-10-03	25	76			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	3,552 ft-lbs	12,704 ft-lbs	28%	1.	03-04-14
End Shear	1,948 lbs	5,785 lbs	33.7%	1	05-11-00
Total Load Defl.	L/999 (0.079")	n/a	n/a	4	03-06-00
Live Load Defl.	L/999 (0.046")	n/a	n/a	5	03-06-00
MaxDefl.	0.079"	n/a	n/a	4	03-06-00
Span / Depth	8.3	n/a	n/a		00-00-00

Beari	ng Supports	Dim . (L x W)	De m an d	De mand/ Re sistance Support	Demand/ Resistance Member	Material
B0	Post	3-1/2" x 1-3/4"	2,034 lbs	40.9%	27.2%	Unspecified
B1	Wall/Plate	4-3/8" x 1-3/4"	2,180 lbs	53.3%	23.3%	Unspecified

Notes



DWO NO. TAM 4527417 STRUCTURAL COMPONENT ONLY

BC CALC® Design Report

Build 4340

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-9 sunken.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B20 L(i

Specifier: Designer: Company:

CONFORMS TO OBG 2012

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALO®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



DWO NO. TAM 45204.17 STRUCTURAL COMPONENT ONLY



COMPANY
TAMARACK LUMBER
3269 NORTH SERVICE ROAD
BURLINGTON, ON
CAROL ZHANG
Aug. 2, 2016 14:48

PROJECT
J1-2ND FL.wwb

Design Check Calculation Sheet

Nordic Sizer - Canada 6.3.1

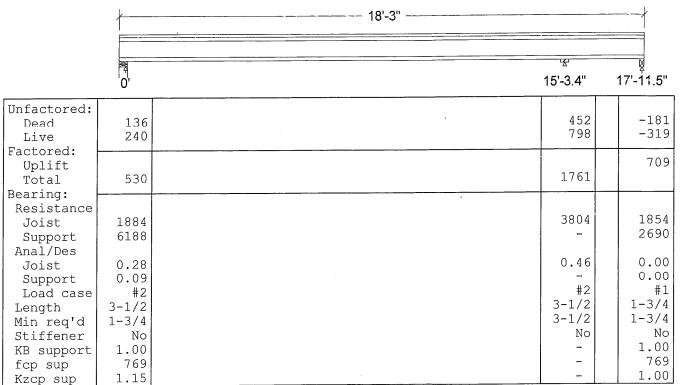
Town of innisfii Certified Mode

12/03/2018 8:45:10 AM kgervais

Loads:

	Load	Type	Distribution	Pat-	Location	[ft]	Magnitud	е	Unit
		11		tern	Start	End	Start	End	
l	Load1	Dead	Full Area	No			20.00		psf
l	Load3	Live	Full Area	No			40.00		psf
ĺ	Self-weight	Dead	Full UDL	No			2.7		plf

Maximum Reactions (lbs), Bearing Resistances (lbs) and Bearing Lengths (in):



Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

Nordic 9-1/2" NI-40x Floor joist @ 12" o.c.

Supports: 1 - Lumber Wall, No.1/No.2; 2 - Non-wood; 3 - Lumber Beam, No.1/No.2; Total length: 18'-3.0"; 5/8" nailed and glued OSB sheathing

This section PASSES the design code check.

S. KATSOULAKOS ST. PS 14

DWO NO. TAM 45275-17 STRUCTURAL COMPONENT ORLY J1-2ND FL.wwb

Nordic Sizer - Canada 6.3.1

Page 2

Limit States Design using CSA-O86-09 and Vibration Criterion:

Criterion	Analysis Value	Design	Value	Unit	Analysis/Design
Shear	Vf = 942	Vr =	1895	lbs	Vf/Vr = 0.50
Moment(+)	Mf = 1593	Mr =	4824	lbs-ft	Mf/Mr = 0.33
Moment (-)	Mf = 2206	Mr =	4824	lbs-ft	Mf/Mr = 0.46
Perm. Defl'n	$0.06 = \langle L/999$	0.51 =	L/360	in	0.12
Live Defl'n	$0.11 = \langle L/999$	0.38 =	L/480	in	0.28
Total Defl'n	$0.17 = \langle L/999$	0.76 =	L/240	in	0.22
Bare Defl'n	$0.12 = \langle L/999$	0.51 =	L/360	in	0.24
Vibration	Lmax = 15'-3	Lv =	17'-8	ft	·
Defl'n	= 0.027	=	0.043	in	0.64

Additional Data:

FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#
Vr	1895	1.00	1.00	_	-	-	_	-	#2
Mr+	4824	1.00	1.00	-	1.000		-	-	#2
Mr-	4824	1.00	1.00	-	1.000	_	-	-	#2
ΕI	218.1 n	nillion	_	_	_	-	-	-	#2

CRITICAL LOAD COMBINATIONS:

Shear :	LC #2 = 1.25D + 1.5L
Moment(+):	LC #2 = 1.25D + 1.5L
Moment(-):	LC #2 = 1.25D + 1.5L
Deflection:	LC $#1 = 1.0D$ (permanent)
	LC #2 = 1.0D + 1.0L (live)
	LC #2 = 1.0D + 1.0L (total)
	LC #2 = 1.0D + 1.0L (bare joist)
Bearing :	Support $1 - LC \#2 = 1.25D + 1.5L$
_	Support $2 - LC \# 2 = 1.25D + 1.5L$

Support 3 - LC #1 = 1.4D Load Types: D=dead W=wind S=snow H=earth,groundwater E=earthquake L=live(use,occupancy) Ls=live(storage,equipment) f=fire

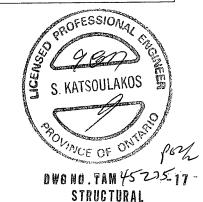
All Load Combinations (LCs) are listed in the Analysis output

CALCULATIONS:

Deflection: Elcomp = 258e06 lb-in2 K= 4.94e06 lbs
"Live" deflection = Deflection from all non-dead loads (live, wind, snow...)

Design Notes:

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC Part 4) and the CSA O86-09 Engineering Design in Wood standard, which includes Update No.1. CONFORMS TO OBC 2012
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



COMPONENT ONLY



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			<u> </u>	Bare			1/2" Gyp	sum Ceiling	•
Depth	Series		On Cen	tre Spacing				re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17' - 3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11 7/6	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9 "	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	Ni-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling		
Depth	Series		On Cent	re Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A		
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A		
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A		
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A		
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A		
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A		
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A		
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A		
11-7/6	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A		
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A		
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A		
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A		
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A		
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A		
	NI-80	25' -7"	23 '- 8"	22'-7"	N/A	26'-2"	24'-4"	23' - 2"	N/A		
	NI-90x	26' - 4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A		
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A		
16"	NI-70	2 7'- 9"	25' - 8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A		
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A		
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27' - 5"	26'-2"	N/A		

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			8	Bare		İ	1/2" Gyr	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cen	tre Spacing	
		12"	16"	19.2"	24"	12"	16"	/ 19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17' - 9"	17'-0"
11 7/01	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21' - 8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22' - 3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25' - 9"	23'-10"	22' - 9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling	
Depth	Series		On Cent	re Spacing		On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"	
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"	
	NI-80	20' - 3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20' - 6"	19'-4"	17'-8"	
11-7/8"	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"	
11-7/0	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"	
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22' - 6"	21'-5"	20'-0"	
	NI-90x	24'-3"	22' - 6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"	
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"	
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"	
14"	NI-70	26'-1"	24'-3"	23 '- 2"	21'-10"	26'-8"	24'-11"	23' - 9"	22'-4"	
	NI-80	26' - 6"	24'-7"	23' - 5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"	
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9".	25'-11"	24'-8"	23'-4"	
	NI-60	27' - 3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24' - 9"	23'-1"	
16"	NI-70	28'-8"	26'-8"	25 '- 4"	23'-11"	29'-3"	27'-4"	26'-1"	24' - 8"	
10	NI-80 .	29'-1"	27'-0"	25'-9"	24' - 4"	29'-8"	27'-9"	26' - 5"	25'-0"	
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25' - 8"	

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







				Bare			1/2" Gyp:	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-//0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
•		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17 '- 9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19' - 5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
11-7/8"	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22' - 5"	21'-0"	N/A
14"	NI-70	25 '- 3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22' - 9"	N/A
14"	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23' - 3"	N/A	26'-10"	24'-11"	23' - 9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25' - 6"	N/A
	NI-90x	29' - 0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26' - 2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.
4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			. В	are		1	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
Береп		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15' - 5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
11-7/8"	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
14"	NI-60	21'-10"	20'-2"	19' - 3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19' - 9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25' - 9"	23'-10"	22'-9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Centi	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19' - 3"	17'-9"	15'-10"
44 7/0"	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/8"	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22' - 5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24' - 3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27' - 1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
16"	NI-70	28'-8"	26 '- 8"	25'-3"	23'-4"	29'-3"	26'-11"	25 '- 3"	23'-4"
16"	NI-80	29'-1"	27'-0"	25' - 9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

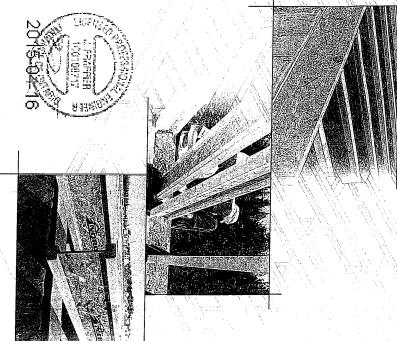
^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

ENGINEERED WOOD

NSTALLATION GUIDE

FOR RESIDENTIAL FLOORS



Distributed by:



SAFETY AND CONSTRUCTION PRECAUTIONS



braced, or serious injuuntil fully fastened and Do not walk on 1-joists ries can result.



concentrated loads from building materials. Once sheathed, do not over-stress I-joist with unsheathed I-joists. materials over



Never stack building bracing over at least two I-joists.

WARNING

braced and sheathed. l-joists are not stable until completely installed, and will not carry any load until fully

Avoid Accidents by Following these Important Guidelines:

- Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- 2. When the building is completed, the floor sheathing will provide lateral to prevent l-joist rollover or buckling. temporary bracing, often called struts, or temporary sheathing must be applied support for the top flanges of the I-joists. Until this sheathing is applied,
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail and spaced no more than 8 feet on centre, and must be secured with a
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- 3. For cantilevered 1-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each 1-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joits, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully

STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped bundles
- 2. Store, stack, and handle Lioists vertically and level only.
- 3. Always stack and handle I-joists in the upright position only.
- Protect I-joists from weather, and use spacers to separate bundles.

4. Do not store I-joists in direct contact with the ground and/or flatwise

- 7. When handling I-joists with a crane on the job site, take a few 6. Bundled units should be kept intact until time of installation

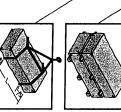
simple precautions to prevent damage to the I-joists and injury

■ Pick I-joists in bundles as shipped by the supplier.

- Pick the bundles at the 5th points, using a spreader bar if necessary ■ Orient the bundles so that the webs of the I-joists are vertical.
- Do not handle I-joists in a horizontal orientation.

œ

9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.



MAXIMUM FLOOR SPANS

- 1. Maximum clear spans applicable to simple-span or or more of the adjacent span. For multiple-span applications, the end spans shall be 40% 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. limit states are based on the factored loads of 1.50L + live load of 40 psf and dead load of 15 psf. The ultimate multiple-span residential floor construction with a design
- of gypsum and/or a row of blocking at mid-span. Standard. No concrete topping or bridging element was shall meet the requirements given in CGBS-71.26 assumed. Increased spans may be achieved with the used less, or 3/4 inch for joist spacing of 24 inches. Adhesive
- bearings, and 3-1/2 inches for the intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used
- 5. This span chart is based on uniform loads. For applications be required based on the use of the design properties. with other than uniform loads, an engineering analysis may
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 1 foot = 0.305 m

SIMPLE AND MULTIPLE SPANS MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

I-JOIST HANGERS

Hangers shown illustrate the three

most commonly used metal hangers

Joist Joist Depth Serie										
s† les						20,42			900g	2415 2418
Simple On centre	16"	15121	152	17:00	17:30	1867* 1819*	2040	20-3 20-8 20-11	20-8 20-9	22.6 22.9
spans spacing										
						APPEN		19:5		
o		-						24.3		
e s		1.00						3888 -6-1		
pans pacing								21-10 21-10 21-10 21-10		546 4365
	24ª	4.7	6-10	6-7	9.1		9.4	22 - 1 22 - 1 23 - 1 24 - 1 25 - 1 25 - 1 25 - 1 26	317	314

4. Web stiffeners are required when the

sides of the hangers do not laterally

brace the top flange of the I-joist.

Hangers should be selected based

and load capacity based on the

on the joist depth, tlange width

maximum spans.

All nailing must meet the hanger

manutacturer's recommendations.

to support 1-joists.

CCMC EVALUATION REPORT 13032-R

Top Mou

Face Mount

- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or
- 3. Minimum bearing length shall be 1-3/4 inches for the end
- with the spans and spacings given in this table, except as required for hangers.
- 7. SI units conversion: 1 inch = 25.4 mm

WEB STIFFENERS

RECOMMENDATIONS:

- the stiffener and the flange is at the top. engineered applications with factored A bearing stiffener is required in all Construction Guide (C101).The gap between -joist properties table found of the I-joist reactions greater than shown in the
- stiffener and flange is at the top. support, the top flange. The gap between the sides of the hanger do not extend up to, and A bearing stiffener is required when the I-joist is supported in a hanger and the
- and the flange is at the bottom. by the code. The gap between the stiffener adjusted for other load durations as permitted standard term load duration, and may be tip and the support. These values are for than 2,370 lbs is applied to the top flange A load stiffener is required at locations cantilever, anywhere between the cantilever where a factored concentrated load greater between supports, or in the case of a

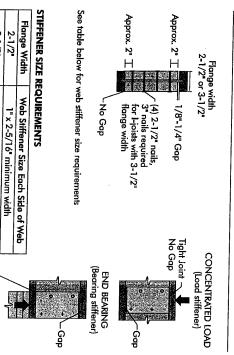
SI units conversion: 1 inch = 25.4 mm

3-1/2"

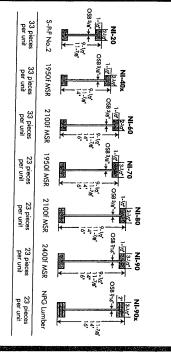
1-1/2" x 2-5/16" minimum width

Tight Join:

WEB STIFFENER INSTALLATION DETAILS



NORDIC I-JOIST SERIES



finished product, reflects our commitment to quality. products to adhere to strict quality control procedures throughout the Chantiers Chibougamau Ltd. harvests its own trees, which enables. Nortic manufacturing process. Every phase of the operation, from forest to the ex-

longer span carrying capacity. lumber in their flanges, ensuring consistent quality, superior strength sino. Nordic Engineered Wood I-joists use only finger-jointed back spruce The second second

2015-04-16

INSTALLING NORDIC I-JOISTS

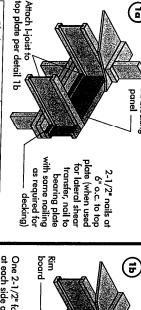
- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, தூர்த்தத்த
- 2. Except for cutting to length, Lioist flanges should never be cut, drilled, or notched
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment

- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple அளில்கத்துமை
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings 0.45.04.16
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- 8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely tastened to the I-loist webs.
- 9. Never install Lioists where they will be permanently exposed to weather, or where they will remain in direct contact with
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or 1-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge **may never** be used as blocking or rim boards. Lipist blocking l-joist-compatible depth selected. anels or other engineered wood products – such as rim board – must be cut to fit between the Ljoists, and an
- 13. Provide permanent lateral support of the bottom flange of all L-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all camilevered L-joists at the end support next to the camilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
- 14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to underlayment layer is installed minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

(

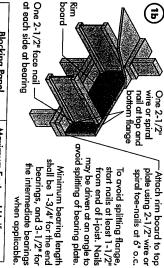
NI blocking

One 2-1/2"



1	NI Joists	or Rim Joist
	3,300	Maximum Factored Uniform Vertical Load* (plf)

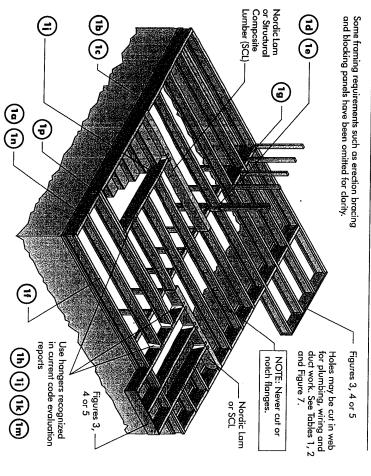
such as joist, header, or rafter. For concentrated vertical load transter, see detail 1d. It shall not be used in the design of a bending member, inches or less and is based on standard term load duration *The uniform vertical load is limited to a joist depth of 16



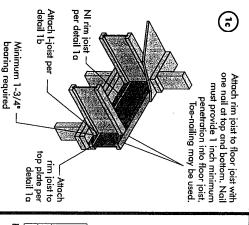
1-1/8" Rim Board Plus	Blocking Panel or Rim Joist
8,090	Maximum Factored Uniform Vertical Load* (plf)

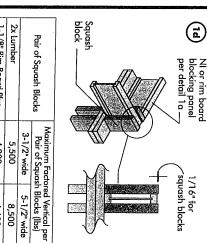
or less and is based on standard term load duration. It shall not be rafter. For concentrated vertical load transfer, see detail 1d. used in the design of a bending member, such as joist, header, or *The uniform vertical load is limited to a rim board depth of 16 inches

TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



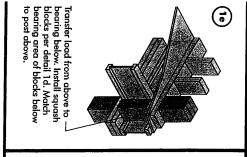
All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity

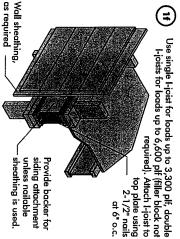




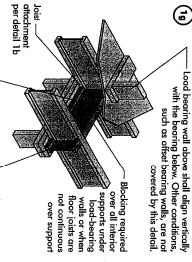
	3-1/2" wide	5-1/2" wide
2x Lumber	5,500	8,500
1-1/8" Rim Board Plus	4,300	6,600

Provide lateral bracing per detail 1a, 1b, or 1c

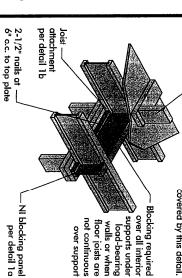




Rim board may be used in lieu of Lioists. Backer is not required when rim board is used. Bracing per code shall be



carried to the foundation.



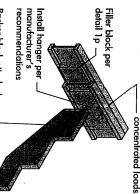
beam. 1/8" overhang allowed past inside inside face of wall or 2x plate flush with (T) Multiple 1-joist header with full depth filler block shown. Nordic Lam or SCL double 1-joist capacity to support headers may also be used. Verify

 \equiv

Nordic Lam or SCL

Ē

tace of wall or beam



Top- or face-mount hanger

recommendations installed per manufacturer's

clinch when possible. detail 1h. Nail with twelve 3" nails, Backer block attached per

Maximum support capacity = 1,620 lbs.

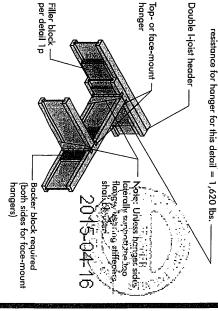
(<u>T</u> face of wall joist beyond inside Do not bevel-cut

l-joist per detail 1b Note: Blocking required

support, not shown at bearing for lateral

for clarity

(Use twelve 3" nails, clinched when possible. Maximum factored backer block will fit. Clinch, Install backer tight to top tlange. additional 3" nails through the webs and filler block where the Before installing a backer block to a double I-joist, drive three Backer block (use if hanger load exceeds 360 lbs)



For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

nailing without splitting) BACKER BLOCKS (Blocks must be long enough to permit required

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	1"	5-1/2"
3-1/2"	1-1/2"	7-1/4"

- better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard. Minimum grade for backer block material shall be S-P-F No. 2 or
- ** For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".
- Optional: Minimum 1x4 inch nails from each web Two 2-1/2" spiral of adjacent web. extend block to face opposite side alternate on to lumber piece, NI blocking

line or 1/2 inch minimum gypsum ceiling strap applied to underside of joist at blocking attached to underside of joists

(Is In some local codes, blocking is prescriptively required in the first joist space (or first and second joist space) next to board —One 2-1/2" nails at top and bottom flange 쟑 - One 2-1/2" nails one side only 2-1/2" nails at 6" o.c. iumber piece iwo 2-1/2" nails from each web to I-joist blocking panel ·2x4 min. (1/8" gap minimum)

- the starter joist. Where required, see local code requirements tor spacing of the blocking
- All nails are common spiral in this detail



stiffeners shall be used

recommendations.

beams, see the manufacturer's

Notes:

(

Filler block

- 1. Support back of I-joist web during nailing to prevent damage to web/flange connection
- 2. Leave a 1/8 to 1/4-inch gap between to of filler block and bottom of top I-joist
- Filler block is required between joists to full length ot span.
- Nail joists together with two rows of 3" are required. can be clinched, only two nails per foot possible) on each side of the double I-jc Total of four nails per foot required. If n nails at 12 inches o.c. (clinched when

 Offset nails from opposite face by 6"

The maximum factored load that may be using this detail is 860 lbf/ft. Verify double applied to one side of the double joist

1/8" to 1/4" gap between top flange

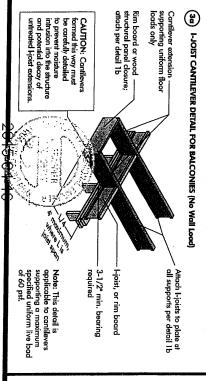
FILLER BLOCK REQUIREMENTS FOR DOUBLE 1-JOIST CONSTRUCTION

(

Lumber 2x4 min.,

•			
8	Flange Size	Joist Depth	Filler Block Size
-	2-1/2"× 1-1/2"	9-1/2" 11-7/8" 14" 16"	2-1/8" x 6" 2-1/8" x 8" 2-1/8" x 10" 2-1/8" x 12"
oist.	3-1/2"× 1-1/2"	9-1/2" 11-7/8" 14" 16"	3" × 6" 3" × 8" 3" × 10"
ĕ	3-1/2" × 2"	11-7/8" 14" 16"	3"×7" 3"×9" 11"

CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)



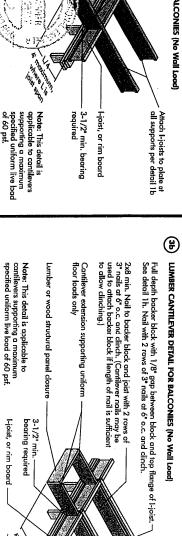
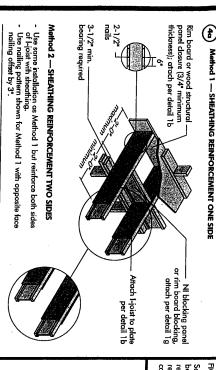
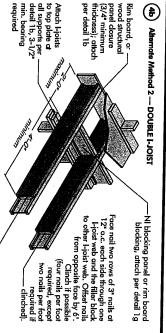


plate at all supports per detail 1b Attach I-joists to

CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)





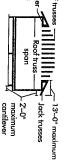
Note: Canadian softwood phywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Natl with 2-1/2" nails at 6" o.c., top and bottom flange. Install with tace grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

- N = No reinforcement required.
 N reinforced with 3/4 wood shuckural penale for one side only wood shuckural penale for one side only wood shuckural penale for one bids sides, or double Hoist.
 N reinforced with 3/4 wood shuckural penale for one bids sides, or double Hoist.
 N reinforced with 1/4 reinforced wit width window or door openings

Block Hoists together with filler blocks for the full length of the reinforcement. >
For Hoist florge widths greater than 3 inches place an additional row of 3" nails along the
centreline of the reinforcing panel from each side. Clinch when possible.

FIGURE 4 (continued) below for NI See table requirements at Roof truss _ span cantilever 2'-0"





For hip roofs with the jack trusses running parallel to the cantilevered floor joists, the Lioist reinforcement requirements for a span of 26 ft. shall be permitted to be used.

CANTILEVER REINFORCEMENT METHODS ALLOWED

ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	JOIST DEPTH (in.)	ROOF TRUSS SPAN (fi)	IL = 30 psf, DL = 15 psi JOIST SPACING (in.) ! 16 19.2 ! N I	ROOF LOADIN LL = 40 psi JOIST SP 12 16	ACTO 15 psf 2 (in.)	N 51 LL
ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	18	28 28 34 34 86	1 2 2		uu	×××××
ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	11778	26 28 32 34 36 37	ZZZZZZ ZZZZZZ 		36 <u>-</u> -	(××0000
Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z	÷.	20 22 30 28 20 24 36 20 34 36	ZZZZZZ			, ברביבים
Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z	0	88.48988	ZZZZZZŻ			z <mark>N</mark>

- For larger openings, or multiple 3'.0" width openings spaced less than 6'.0" o.c., additional joists beneath the opening's cripple
- studs may be required.
 3. Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12* o.c. requirements for lesser spacing.
 - 4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a
- cantilevered joists supporting girder trusses or roof beams may require additional

WEB HOLES

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- 'n I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
- ω Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. between the top or bottom of the hole or opening and the adjacent I-joist tlange. the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of
- Ċı The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- ٥. Where more than one hole is necessary, the distance between adjacent hole opening shall be sized and located in compliance with the requirements of size of the largest square hole (or twice the length of the longest side of the lables 1 and 2, respectively. longest rectangular hole or duct chase opening) and each hole and duct chase edges shall exceed twice the diameter of the largest round hole or twice the
- 7. A knockout is not considered a hole, may be utilized anywhere it occurs, and and/or duct chase openings. may be ignored for purposes of calculating minimum distances between holes
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a verification. cantilevered section of a joist. Holes of greater size may be permitted subject to
- % A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

Above table					Joist Depth
may be used		i i i lepele (e			Joist Series
for Lini					2
et engrin	988 48458			erke.	3
20, 20	204 244 044		97,144.4		4
inches of					
	100			30.00	9 و
			\$4.6 \$4.6	100	Roun
ı		distance of	222256	100	e race d hole 7
ı				140	ot any diame 8 8.
			10.10 8 4 4 10.4 4 4 10.4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		suppo iter (in 5/8
	(2009)		1411111	312.2	9) 10 (10 (10 (10 (10 (10 (10 (10 (10 (10 (
- 13	1000				0
3			1111111		ot hole 0-3/4]
-			141111		(ff-in,)
			10000) 12 12-3/
1 1 2 1 1 1 1 1					. 8
21210	1 00 00	8 22 9 5 9 5 9 5			Span ushment actor

- Above table may be used for I-joist spacing of 24 inches on centre or less.
 I-de location distance is measured from inside face of supports to centre of hole.
 Distances in this chart are based on uniformly loaded joists.

OPTIONAL:

The above table is based on the I-joists used at their maximum span. If the I-joists are placed at less than their full maximum span (see Maximum Prox Spairs).

- Spairs are placed at less than their full maximum span (see Maximum Prox Spairs).

Dreduced = Lactual x D

Where:

Dreduced Lactual

¥ II Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications (fit. The reduced distance shall not be less than 6 inches from the tace of the support to edge of the hole.

The actual measured span distance between the inside faces of supports (fit). Span Adjustment Factor given in this table.

U

The minimum distance from the inside face of any support to centre of hole from this table

If <u>ractual</u> is greater than 1, use 1 in the above calculation for <u>ractual</u>

0.504

ത

over-cut the web. notch the flange, or Never drill, cut or bearing for minimum

distance from See Table 1

2x diameter of larger hole

diameter, length or hole 2x duct chase

Duct chase opening (see Table 2 for

whichever is

from bearing) minimum distance

spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of

electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are for the contractor's convenience to instal

Knockouts are prescored holes provided

field-cut holes

FIELD-CUT HOLE LOCATOR

FIGURE 7

sharp saw. should be cut with a Holes in webs

and then making the cuts between the corners, as this can cause unnecessary stress concentrations. Slightly rounding the holes is another good method to diameter hole in each of the four corners the rectangular hole by drilling a 1-inch the corners is recommended. Starting For rectangular holes, avoid over-cutting ninimize damage to the I-joist

A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances

Knockouts

See rule 12

all duct chase openings and holes between top and bottom flange Maintain minimum 1/8" space Θ

2

TABLE 2

DUCT CHASE OPENING SIZES AND LOCATIONS - Simple Span Only

Joist Depth	Joist Series	Minimu	m distan	ce from i	nside fac Duct ct	of any s	th Cin	o centre c	of openiin	g (ff-in.)
	To a second	8	10	12	14	16	18	20	22	24
		1001 40	ກປາປາ 4 ກູ້ ໝໍໜູ້ນ	1864 1200 1200 1200	76.F	5.8° 6-10°	6'-1". 7'-3" 7'-5"	6-6* 7-8* 8-6*	7.1 8.2 8.3	96.7 887
		5.0	X 50		6.5	6:10:	73	7.6	8-1* 8-2*	8-4 8-6
		76.6 7.8	77 82	8 7 0 0	88 6	98 561	9-1	0 9 0 0 0 0	10.1	10.7
	i Sisj.	22			ទីស្នាស់ ទីស្នាស់	8-7 8-10	14-1 14-1		000	10.4
		8.77	8:77	9.0	9-6	10.1	9.8	100 100 100	10.8	1 1 C
T.		900	9 9 9 1 - 4	109 158	9-10-1- 1-1-0-1-	44	100 81	 56,1	13:3 13:3	13-0 12-0
		9.2	9:01 9:02 9:08	10.0	10-6	10:11		51196 2196		12.13. 12.13.
		200	000 968	46K	04.6	31.10°	12.6 12.3	13:2	13.3	14-10
	NEGO.	1111	1112	111.8	72.0°	12.6	10 10 10	် ရှင် ရင်	14.20	

14'4' 15:2"

- . Above table may be used for I-joist spacing of 24 inches on centre or less.

 Duct thatse opening location distance is measured from inside face of supports to centre of opening.

 The above table is based on simple-span joists only. For other applications, contact your local distributor.

 Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of I/480. For other applications, contact your local distributor.

BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

FIGURE 5 (continued)

below for NI requirements at reinforcement See table

> Roof truss span

L-maximum 21-0

> Girder. truss

Roof trusses

Roof trussspan

✓ Jack trusses

13'-0" maximum

For hip roofs with the jack trusses running parallel to the cantilevered floor joists,

2'-0" maximum

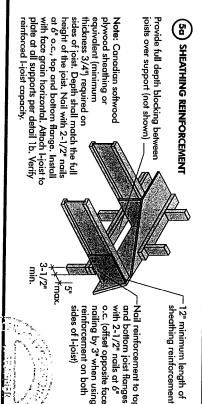
cantilever - 5" maximum

26 ft. shall be permitted to the I-joist reinforcement

requirements for a span of

be used.

cantilever



nailing by 3" when using o.c. (offset opposite face with 2-1/2" nails at 6" Nail reinforcement to top

JOIST DEPTH (in.)

19.2

12

192

12

LL = 50 psf, DL = 15 psfJOIST SPACING (in.

SET-BACK DETAIL

Bearing walls

F

(3/4" minimum thickness), structural panel closure attach per detail 1b. Rim board or wood

between joists over support (not shown for clarity)

Provide full depth blocking

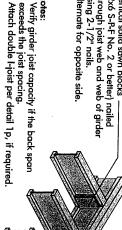
Attach I-joist to plate at all supports per detail 1b.

3-1/2" minimum I-joist bearing required.

ΞQX. ςī Attach joists to girder joist per detail 5c.

চূ SET-BACK CONNECTION

through joist web and web of girder using 2-1/2" nails. Alternate for opposite side. 2x6 S-P-F No. 2 or better) nailed ertical solid sawn blocks



bottom flanges. Nail joist end using 3" nails, toe-nail at top and

solid sawn blocks used in lieu of Hanger may be

N = No reinforcement required.
 N = NI reinforced with 3/4" wood structural

panel on one side only. 2 = NI reinforced with 3/4"

2 = NI reinforced with 3/4" wood strudural panel on both sides, or double I-joist.
X = Try a deeper joist or closer spacing.

2. Maximum design load shall be: 15 psf roof dead load, 55 psf floor total load, and wall load. Wall load is based on 3'-0" and 80 plf

BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED 12 LL = 30 psf, DL = 15 psf JOIST SPACING (in.) -5" maximum ROOF LOADING (UNFACTORED) 12 LL = 40 psf, DL = 15 psfJOIST SPACING (in.

additional joists beneath the opening's cripple For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" o.c., studs may be required.

ω the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use Table applies to joists 12" to 24" o.c. that meet

For conventional roof construction using a distance between the supporting walls as if a the Roof Truss Span is equivalent to the the supporting wall and the ridge beam. When the roof is framed using a ridge board, above is equivalent to the distance between idge beam, the Roof Truss Span column

truss is used.

Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- 2. Snap a chalk line across the Hoists four feet in from the wall for panel edge alignment and as a boundary for spreading give.
- 3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manufacturer.
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single Lioist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time a thinner line (1/8 inch) than used on I-joist flanges. before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying
- 8. Tap the second row of panels into place, using a block to protect groove edges.
- 9. Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and nail to assure accurate and consistent spacing.) /8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the finished deck can be walked on right away and will carry construction loads without damage to the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The tor cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels

FASTENERS FOR SHEATHING AND SUBFLOORING(1)

3/4	20 5/8	16 1 5/8	Maximum Minimum Joist Panel Spacing Thickness (in.) (in.)
2"	. 2"	2"	Common Wire or Spiral Nails
1-3/4"	1-3/4*	1-3/4"	iil Size and Ty Ring Thread Nails or Screws
2	2"	2"	pe Staples
6"	6"	6"	Maximun of Fas Edges
12"	12"	12"	n Spacing teners Interm, Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- 2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood 1 Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manutacturer.

Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5.

IMPORTANT NOTE:

Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with your local distributor.

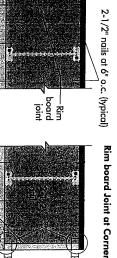
RIM BOARD INSTALLATION DETAILS

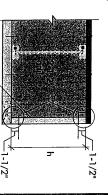
(8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

Rim board Joint Between Floor Joists



(typical)

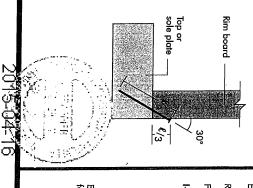




(F) TOE-NAIL CONNECTION AT RIM BOARD

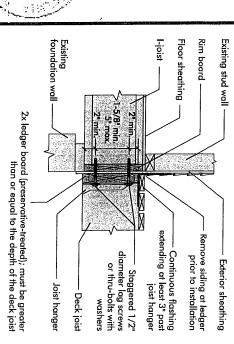
6" o.c. (typical)

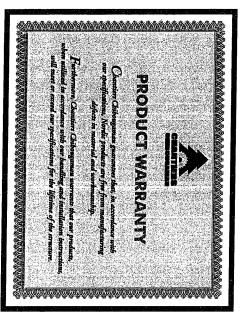
2-1/2" toe-nails at

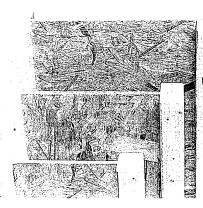


(%) 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL

Rim board joint







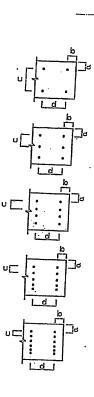
· MICRO CITY

Engineering services inc.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1MO

	LVL HEADER AND CONVENTIONAL				
	LUME	DETAILS			
	DETAIL	NUMBER	,		
	NUMBER	OF ROWS	,		
			d"		
	. A	2	12		
	В	2	. 8		
	C	2	6		
	D	2	4		
	1A	3	12		
	1B	3	8		
i	1C	3	. 6		
	1D	3	4		
۱	2A	4	. 12		
	2B	4	8 .		
1	2C	4	6		
L	2D	4	4		
	ЗА	5	12		
3B 3C 3D 4A		5	8		
		5	6		
		5	4		
		6	12		
L	4B	6	8		
4C		6	6		
4D		6	4		



NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
 - (2) MINIMUM LUMBER END DISTANCE "b" = 2"
 - (3) MINIMUM NAIL ROW SPACING "c" = 2"
 - (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
 - (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
 - (6) DO NOT USE AIR-DRIVEN NAILS



DNG NO TANNICO1. 14
STRUCTURAL
COMPONENT ONLY
TO BE USED ONLY
WITH BEAM CALCS
BEARING THE
STAMP BELOWS

PROVICE NAILING
DETAIL № × SEE
ONG #TAMN1001-14