

T-170678

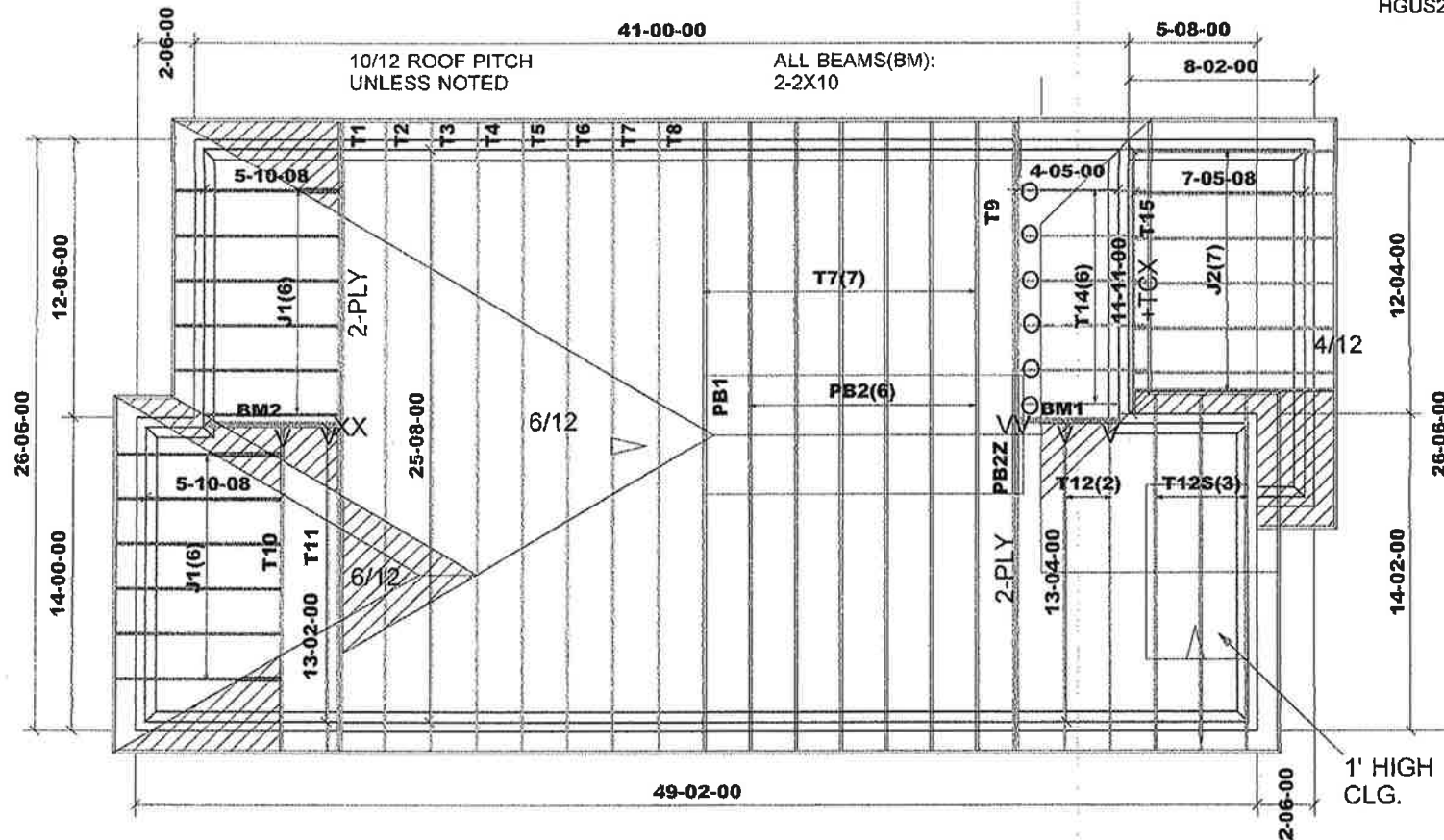
ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR
WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
S_s= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)



Town of Innisfil Certified Model
15/02/2019 10:27:14 AM kgervais

mill, 017



Job Track: **42067**
Layout ID: **272426**
Plan Log: **87565**

Builder / Location:

BAYVIEW WELLINGTON / INNISFIL

Project:

ALCONA SHORES

Date: 9/11/2017

Designer: JG

Model / Elevation:

S32-8-15G / A

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE

Mitek ver 7.5.0

T-170678

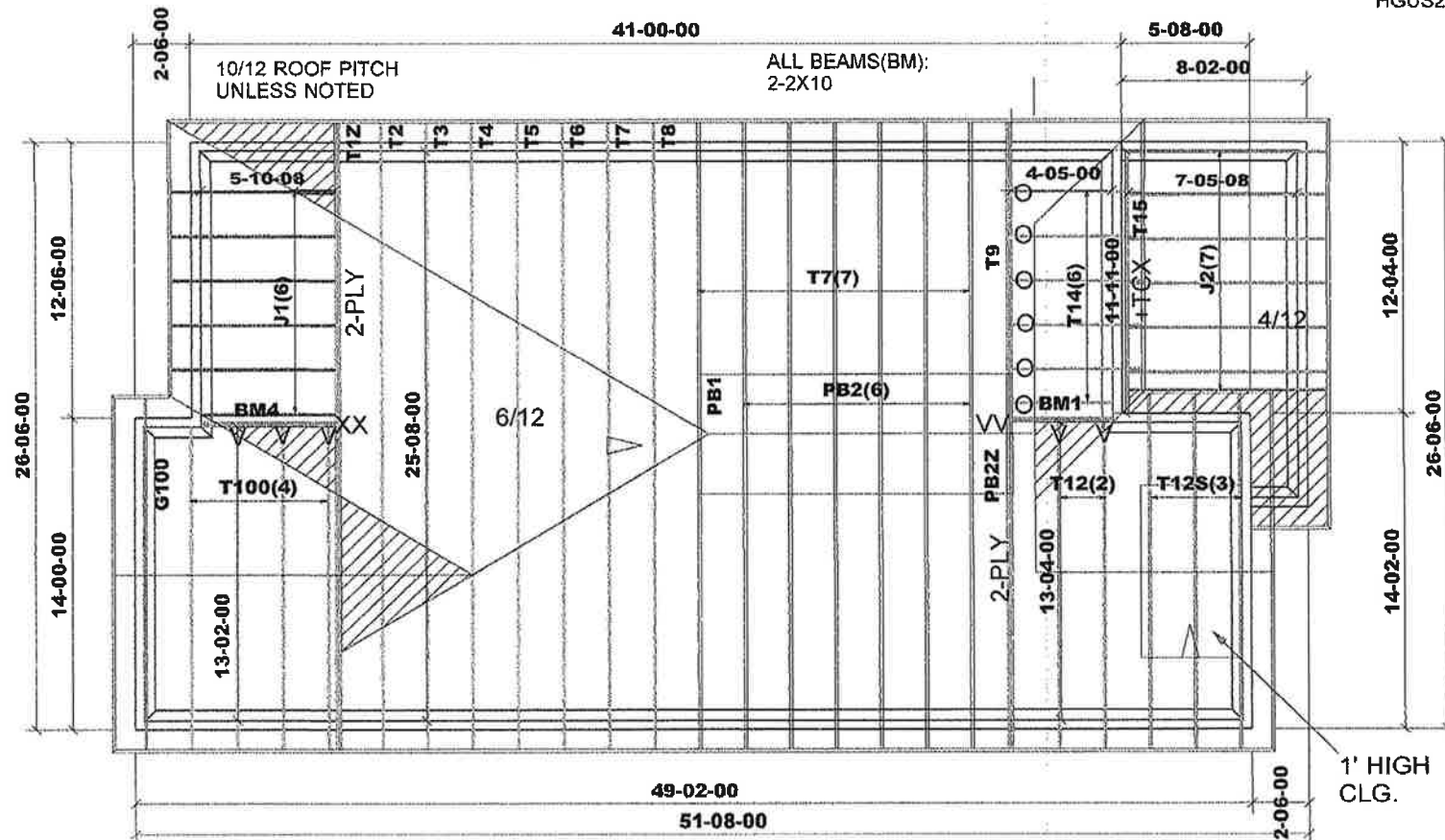
ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR
WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL 
FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)



Town of Innisfil Certified Model
15/02/2018 10:27:16 AM kgervais

m11,017



Job Track: 42067

Layout ID: 272427

Plan Log: 87565

Builder / Location:

BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 9/11/2017

Designer: JG

Model / Elevation:

S32-8-15G / A-REAR UPGRADE

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
Mitok ver.7.5.0

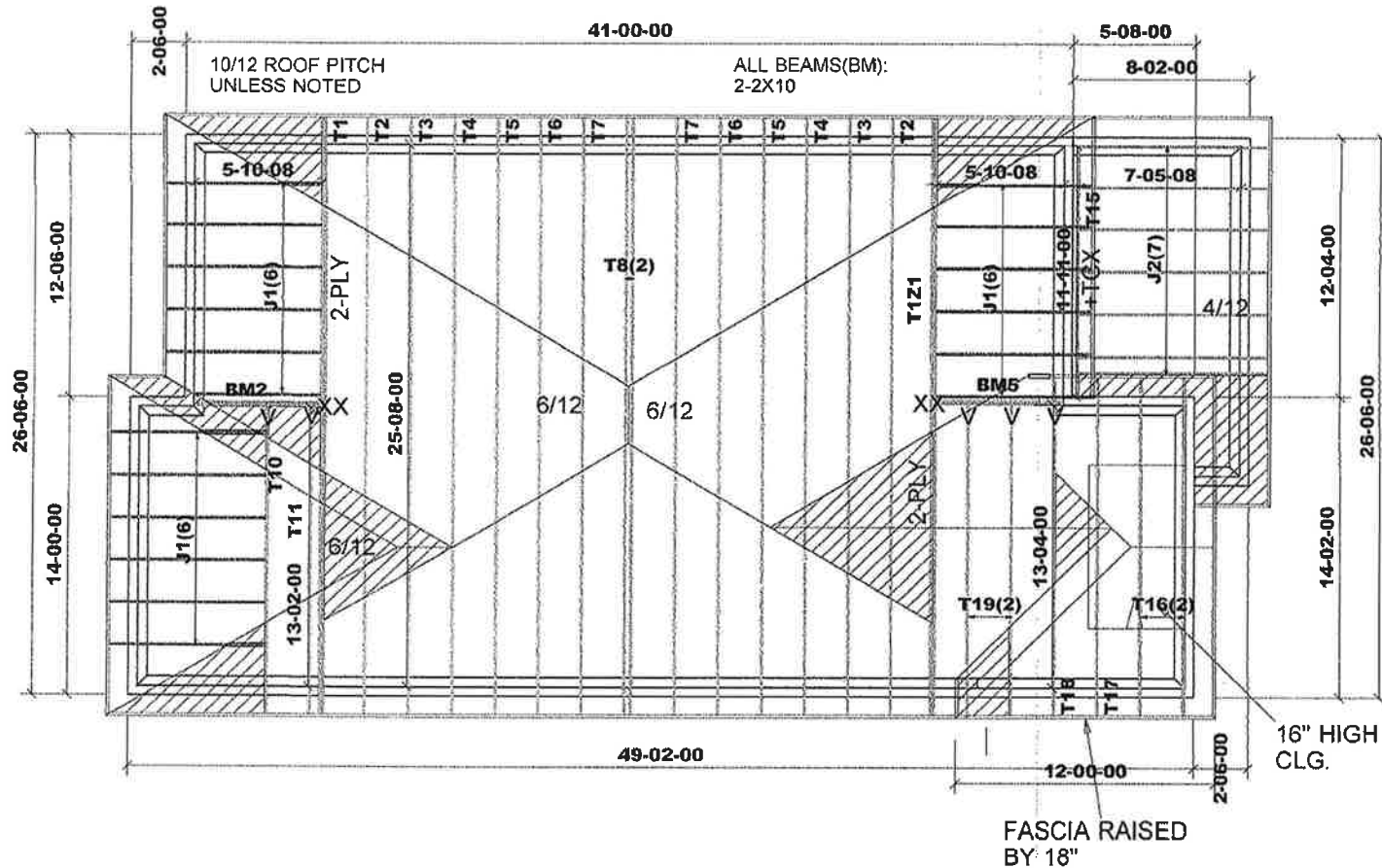
T-170678

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance between end points and between rows of bracing does not exceed 6'.
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR
WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
S_s = 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL FRAMING 
HARDWARE
LJS26DS(V)
HGUS26-2(XX)



Town of Innisfil Certified Model
15/02/2018 10:27:24 AM kgervais

11/1/17



Job Track: **42067**
Layout ID: **272428**
Plan Log: **87565**

Builder / Location:

BAYVIEW WELLINGTON / INNISFIL

Model / Elevation:

S32-8-15G / B

Project: **ALCONA SHORES**

Date: 9/11/2017 Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milek ver 7.5.0

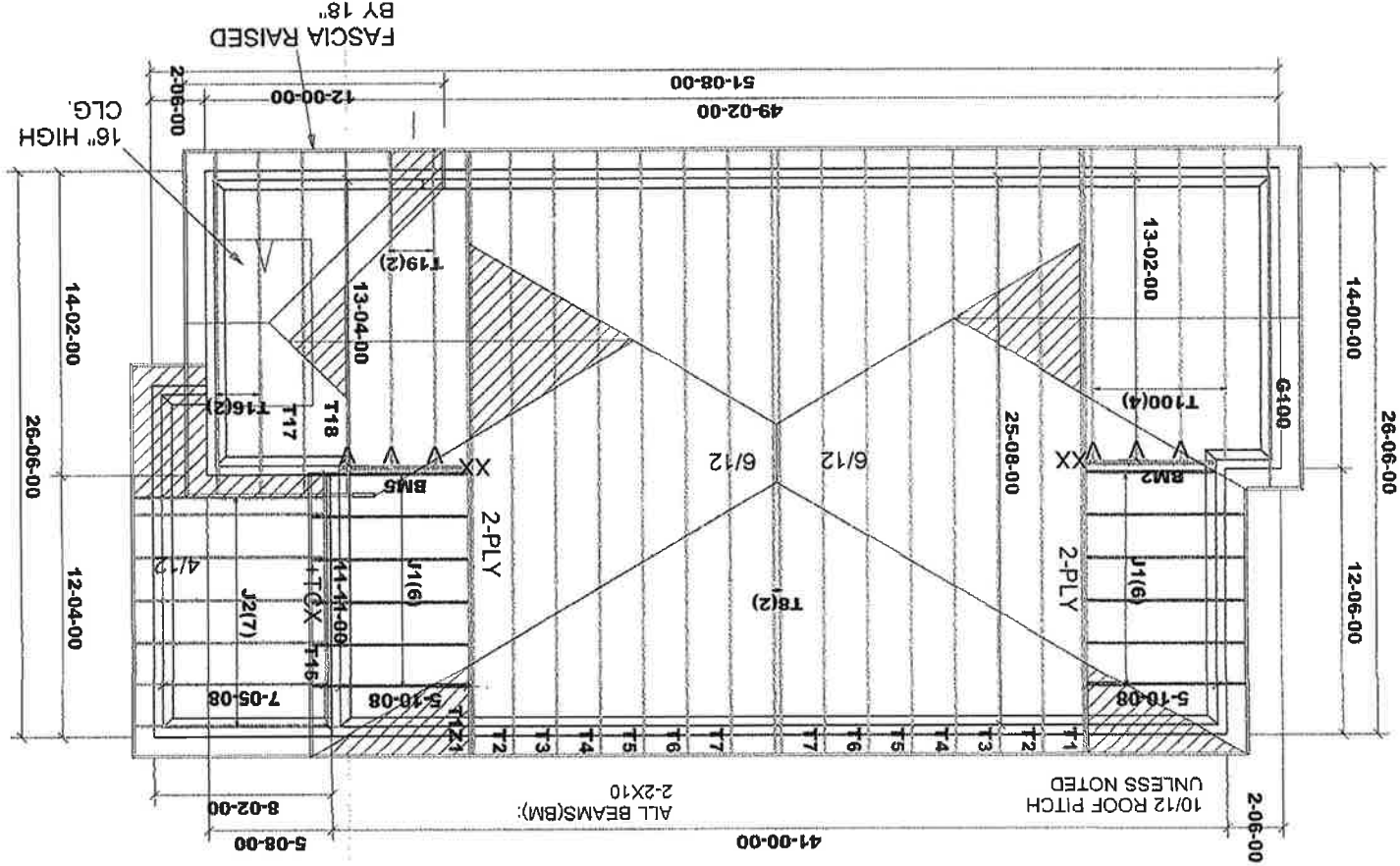


ml, 017

Job Track: 42067
Layout ID: 272429
Plan Log: 87566

Builder / Location:
Project: ALCONA SHORES
Date: 9/11/2017
Designer: JG

Model / Elevation:
S32-8-15G / B-REAR UPGRADE
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
Mike ver. 7.5.0



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC, LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE CROSS POINT. POSTS LONGER THAN 6' TO BE laterally braced so that the distance between end points and between rows of bracing does not exceed 6'.
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART 9

- 12" FINISH O.H
- R.T.M.C
- 2X6 EXTERIOR WALLS
- ASPHALT SHINGLES
- 2X6 FASCIA BOARD
- DESIGN LOADS:
 - GROUND SNOW LOAD
 - S_s = 2.6 kPa
 - BC DEAD 10.5 PSF
 - BC LIVE 3 PSF
 - HARDWARE LUS26DS(V)
 - HGUS26-2(XX)
- CONVENTIONAL FRAMING
- DENOTES

Town of Innisfil Certified Model
15/02/2018 10:27:31 AM kgervais

T-170678



Delivery Shiplist

JOB TRACK: 42067 LAYOUT ID: 272426 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-8-15G ELEVATION: A

DATE	09/11/17
SALES REP	Maño

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG		HEEL HEIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT	LEFT	RIGHT	LEFT	RIGHT			
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08	01-03-08	01-07-11	01-07-11	255.20		
								01-03-08		01-07-11		158.00		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	107.29		
								01-03-08		01-07-11		69.67		
	1	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	113.57		
								01-03-08		01-07-11		72.00		
	1	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	116.38		
								01-03-08		01-07-11		72.83		
	1	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	122.26		
								01-03-08		01-07-11		77.67		
	1	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	126.45		
								01-03-08		01-07-11		80.00		
	8	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	1032.32		
								01-03-08		01-07-11		653.36		
	1	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	141.43		
								01-03-08		01-07-11		87.67		
	1 2 Ply	T9 PIGGYBACK	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 6	01-03-08	01-03-08	01-07-11	01-07-11	299.46		
								01-03-08		01-07-11		188.66		
	1	T10 HIP GIRDER	10.00 0.00	13-02-00	04-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	60.46		
								01-03-08		01-07-11		39.33		
	1	T11 HIP	10.00 0.00	13-02-00	05-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	59.16		
								01-03-08		01-07-11		39.50		
	2	T12 COMMON	10.00 0.00	13-04-00	07-02-06	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	126.88		
								01-03-08		01-07-11		80.34		
	3	T12S ROOF	10.00 0.00	13-04-00	07-02-06	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	211.56		
								01-03-08		01-07-11		141.00		
	6	T14 MONOPITCH	10.00 0.00	04-05-00	03-02-07	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	142.50		
								00-00-00		03-02-07		99.00		
	1	T15 FLAT GIRDER	0.00 0.00	11-11-00	02-09-12	2 X 6	2 X 6	00-00-00	00-00-00	02-09-12	02-09-12	66.17		
								00-00-00		02-09-12		39.33		
	1	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00	00-00-00	00-04-13	00-04-13	12.35		
								00-00-00		00-04-13		9.33		
	6	PB2 PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00	00-00-00	00-04-13	00-04-13	75.24		
								00-00-00		00-04-13		52.02		
	1 2 Ply	PB2Z PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00	00-00-00	00-04-13	00-04-13	25.08		
								00-00-00		00-04-13		17.34		



Delivery Shiplist

DATE	09/11/17
SALES REP	Marlo

JOB TRACK: 42067 LAYOUT ID: 272426 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-8-15G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT		LEFT RIGHT	LEFT RIGHT			
	12	J1	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04			
	7	J2	4.00 0.00	07-05-08	02-09-12	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-09-12	154.07 98.00			

TOTAL # TRUSS= 60.00

TOTAL BFT OF ALL TRUSSES=

2203.09 BFT.

TOTAL WEIGHT OF ALL TRUSSES= 3449.31 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
6	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 12.00



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272427 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-8-15G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TO BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG		HEEL HEIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT	LEFT	RIGHT	LEFT	RIGHT			
	1 2 Ply	T12 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08	01-03-08	01-07-11	01-07-11	251.84		
								01-03-08		01-07-11		155.34		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	107.29		
								01-03-08		01-07-11		69.67		
	1	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	113.57		
								01-03-08		01-07-11		72.00		
	1	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	116.38		
								01-03-08		01-07-11		72.83		
	1	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	122.26		
								01-03-08		01-07-11		77.67		
	1	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	126.45		
								01-03-08		01-07-11		80.00		
	8	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	1032.32		
								01-03-08		01-07-11		653.36		
	1	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	141.43		
								01-03-08		01-07-11		87.67		
	1 2 Ply	T9 PIGGYBACK	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 6	01-03-08	01-03-08	01-07-11	01-07-11	299.46		
								01-03-08		01-07-11		188.66		
	2	T12 COMMON	10.00 0.00	13-04-00	07-02-06	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	126.88		
								01-03-08		01-07-11		80.34		
	3	T12S ROOF	10.00 0.00	13-04-00	07-02-06	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	211.56		
								01-03-08		01-07-11		141.00		
	6	T14 MONOPITCH	10.00 0.00	04-05-00	03-02-07	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	03-02-07	142.50		
								00-00-00		01-07-11		99.00		
	1	T15 FLAT GIRDER	0.00 0.00	11-11-00	02-09-12	2 X 6	2 X 6	00-00-00	00-00-00	02-09-12	02-09-12	66.17		
								00-00-00		02-09-12		39.33		
	4	T100 COMMON	10.00 0.00	13-02-00	07-01-08	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	251.08		
								01-03-08		01-07-11		160.68		
	1	G100 COMMON	10.00 0.00	13-02-00	07-01-08	2 X 4	2 X 4	01-03-08	01-03-08	01-07-11	01-07-11	64.21		
								01-03-08		01-07-11		42.17		
	1	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00	00-00-00	00-04-13	00-04-13	12.42		
								00-00-00		00-04-13		9.33		
	6	PB2 PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00	00-00-00	00-04-13	00-04-13	75.66		
								00-00-00		00-04-13		52.02		
	1 2 Ply	PB2Z PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00	00-00-00	00-04-13	00-04-13	25.08		
								00-00-00		00-04-13		17.34		



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272427 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-8-15G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TO BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG		HEEL HEIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT	LEFT	RIGHT	LEFT	RIGHT			
	6	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00		01-02-00 04-01-04		100.74 64.02		
	7	J2 JACK-PARTIAL	4.00 0.00	07-05-08	02-09-12	2 X 4	2 X 4	01-03-08 00-00-00		00-03-15 02-09-12		154.07 98.00		

TOTAL # TRUSSES= 57.00

TOTAL BFT OF ALL TRUSSES=

2260.43 BFT.

TOTAL WEIGHT OF ALL TRUSSES= 3541.37 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
5	Hangers	LUS26DS	
6	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 13.00



Delivery Shiplist

DATE	09/11/17
SALES REP	Maño

JOB TRACK: 42067	LAYOUT ID: 272428	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-8-15G	ELEVATION: B	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG		HEEL HEIGHT		LBS. BFT.	BUNDLE #	LOAD BY: REMARKS
	PLY					TOP	BOT	LEFT	RIGHT	LEFT	RIGHT			
	1	T1	10.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08		01-07-11		255.20		
	2 Ply	HIP GIRDER	0.00					01-03-08		01-07-11		158.00		
	1	T1Z1	10.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08		01-07-11		255.20		
	2 Ply	HIP GIRDER	0.00					01-03-08		01-07-11		158.00		
	2	T2	10.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08		01-07-11		214.58		
		HIP	0.00					01-03-08		01-07-11		139.34		
	2	T3	10.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08		01-07-11		227.14		
		HIP	0.00					01-03-08		01-07-11		144.00		
	2	T4	10.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08		01-07-11		232.76		
		HIP	0.00					01-03-08		01-07-11		145.66		
	2	T5	10.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08		01-07-11		244.52		
		HIP	0.00					01-03-08		01-07-11		155.34		
	2	T6	10.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08		01-07-11		252.90		
		HIP	0.00					01-03-08		01-07-11		160.00		
	2	T7	10.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08		01-07-11		258.08		
		HIP	0.00					01-03-08		01-07-11		163.34		
	2	T8	10.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08		01-07-11		282.86		
		HIP	0.00					01-03-08		01-07-11		175.34		
	1	T10	10.00	13-02-00	04-01-04	2 X 4	2 X 4	01-03-08		01-07-11		60.46		
		HIP GIRDER	0.00					01-03-08		01-07-11		39.33		
	1	T11	10.00	13-02-00	05-01-04	2 X 4	2 X 4	01-03-08		01-07-11		59.16		
		HIP	0.00					01-03-08		01-07-11		39.50		
	1	T15	0.00	11-11-00	02-09-12	2 X 6	2 X 6	00-00-00		02-09-12		66.17		
		FLAT GIRDER	0.00					00-00-00		02-09-12		39.33		
	2	T16	10.00	13-04-00	08-08-06	2 X 4	2 X 4	01-03-08		03-01-11		160.88		
		ROOF	0.00					01-03-08		03-01-11		102.34		
	1	T17	10.00	13-04-00	07-04-02	2 X 4	2 X 4	01-03-08		03-01-11		86.11		
		HIP	0.00					01-03-08		03-01-11		56.17		
	1	T18	10.00	13-04-00	05-08-02	2 X 4	2 X 4	01-03-08		03-01-11		67.66		
		HIP	0.00					01-03-08		03-01-11		43.67		
	2	T19	10.00	13-04-00	07-11-06	2 X 4	2 X 4	01-05-00		01-07-11		130.26		
		COMMON	0.00					00-00-00		03-01-11		83.00		
	18	J1	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08		01-02-00		302.22		
		JACK-OPEN	0.00					00-00-00		04-01-04		192.06		
	7	J2	4.00	07-05-08	02-09-12	2 X 4	2 X 4	01-03-08		00-03-15		154.07		
		JACK-PARTIAL	0.00					00-00-00		02-09-12		98.00		

TOTAL # TRUSS= 52.00

TOTAL BFT OF ALL TRUSSES=

2092.42 BFT.

TOTAL WEIGHT OF ALL TRUSSES= 3310.23 LBS.



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

Page 2 of 2

JOB TRACK: 42067	LAYOUT ID: 272428	LOCATION: INNSFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-8-15G	ELEVATION: B	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272429 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/MALCONA SHO SUB-BUILDER:
 MODEL: S32-8-15G ELEVATION: B-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG		HEEL HEIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT	LEFT	RIGHT	LEFT	RIGHT			
	1	T1	10.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08	01-07-11	01-07-11		251.84		
	2 Ply	HIP GIRDER	0.00					01-03-08	01-07-11	01-07-11		155.34		
	1	T1Z1	10.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08	01-07-11	01-07-11		251.84		
	2 Ply	HIP GIRDER	0.00					01-03-08	01-07-11	01-07-11		155.34		
	2	T2	10.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		214.58		
		HIP	0.00					01-03-08	01-07-11	01-07-11		139.34		
	2	T3	10.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		227.14		
		HIP	0.00					01-03-08	01-07-11	01-07-11		144.00		
	2	T4	10.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		232.76		
		HIP	0.00					01-03-08	01-07-11	01-07-11		145.66		
	2	T5	10.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		244.52		
		HIP	0.00					01-03-08	01-07-11	01-07-11		155.34		
	2	T6	10.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		252.90		
		HIP	0.00					01-03-08	01-07-11	01-07-11		160.00		
	2	T7	10.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		258.08		
		HIP	0.00					01-03-08	01-07-11	01-07-11		163.34		
	2	T8	10.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		282.86		
		HIP	0.00					01-03-08	01-07-11	01-07-11		175.34		
	1	T15	0.00	11-11-00	02-09-12	2 X 6	2 X 6	00-00-00	02-09-12	02-09-12		66.17		
		FLAT GIRDER	0.00					00-00-00	02-09-12	02-09-12		39.33		
	2	T16	10.00	13-04-00	08-08-06	2 X 4	2 X 4	01-03-08	03-01-11	03-01-11		160.88		
		ROOF	0.00					01-03-08	03-01-11	03-01-11		102.34		
	1	T17	10.00	13-04-00	07-04-02	2 X 4	2 X 4	01-03-08	03-01-11	03-01-11		86.11		
		HIP	0.00					01-03-08	03-01-11	03-01-11		56.17		
	1	T18	10.00	13-04-00	05-08-02	2 X 4	2 X 4	01-03-08	03-01-11	03-01-11		67.66		
		HIP	0.00					01-03-08	03-01-11	03-01-11		43.67		
	2	T19	10.00	13-04-00	07-11-06	2 X 4	2 X 4	01-05-00	01-07-11	01-07-11		130.26		
		COMMON	0.00					00-00-00	03-01-11	03-01-11		83.00		
	4	T100	10.00	13-02-00	07-01-08	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		251.08		
		COMMON	0.00					01-03-08	01-07-11	01-07-11		160.68		
	1	G100	10.00	13-02-00	07-01-08	2 X 4	2 X 4	01-03-08	01-07-11	01-07-11		64.21		
		COMMON	0.00					01-03-08	01-07-11	01-07-11		42.17		
	12	J1	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08	01-02-00	01-02-00		201.48		
		JACK-OPEN	0.00					00-00-00	04-01-04	04-01-04		128.04		
	7	J2	4.00	07-05-08	02-09-12	2 X 4	2 X 4	01-03-08	00-03-15	00-03-15		154.07		
		JACK-PARTIAL	0.00					00-00-00	02-09-12	02-09-12		98.00		

TOTAL # TRUSS= 49.00

TOTAL BFT OF ALL TRUSSES=

2147.10 BFT.

TOTAL WEIGHT OF ALL TRUSSES= 3398.44 LBS.



DATE	09/11/17
SALES REP	Mario

Delivery Shiplist

JOB TRACK:42067	LAYOUT ID: 272429	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-8-15G	ELEVATION: B-REAR	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	

TOTAL # ITEMS= 8.00

PLATES (tabe in inches)
 JT TYPE W LEN Y X
 R BMV1+P MT20 3.0 8.0
 Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC	LC1	MAX-	MAX+	FACE
X	3-11-4	-40	-70	-	FRONT
Y	5-11-4	-40	-70	-	FRONT
Z	9-11-4	-40	-70	-	FRONT
AA	11-11-4	-40	-70	-	FRONT
AB	12-4-8	-2539	-2539	-	FRONT
					VERT
					TOTAL

HANGERS NOTES

1) -SPECIAL HANGERS) OR CONNECTIONS)
 REQUIRED TO SUPPORT CONCENTRATED
 LOAD(S) 227.8 lbs FACTORED DOWN AT 2-11-8,
 148.2 lbs FACTORED DOWN AT 3-11-4, 147.1 lbs
 FACTORED DOWN AT 5-11-4, 147.1 lbs
 FACTORED DOWN AT 7-11-4, AND 147.1 lbs
 FACTORED DOWN AT 9-11-4, AND 147.1 lbs
 FACTORED DOWN AT 11-11-4 ON TOP CHORD,
 AND 69.9 lbs FACTORED DOWN AT 1-11-4, 88.9
 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs
 FACTORED DOWN AT 5-11-4, 69.9 lbs
 FACTORED DOWN AT 7-11-4, 69.9 lbs
 FACTORED DOWN AT 9-11-4, AND 2538.7 lbs
 FACTORED DOWN AT 11-11-4, AND 2538.7 lbs
 FACTORED DOWN AT 12-4-8 ON BOTTOM
 CHORD. DESIGN FOR UNSPECIFIED
 CONNECTION(S) IS DELEGATED TO THE
 BUILDING DESIGNER.



DWG NO. TAN 4602-17
 STRUCTURAL
 COMPONENT ONLY
 PKC

PLATES (table is in inches)
 JT TYPE PLATES W LEN Y X
 R BMV1+P MT20 3.0 6.0
 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)										
JT	LOC.	LCI	MAX-	MAX+	FACE		DIR.		TYPE	
Y	5-11-4	-40	-70	—	BACK	VERT	BACK	VERT	TOTAL	TOTAL
Z	8-11-4	-40	-70	—	BACK	VERT	BACK	VERT	TOTAL	TOTAL
AA	11-11-4	-40	-70	—	BACK	VERT	BACK	VERT	TOTAL	TOTAL
AA	12-2-8	-1539	-1539	—	BACK	VERT	BACK	VERT	TOTAL	TOTAL

HANGERS NOTES

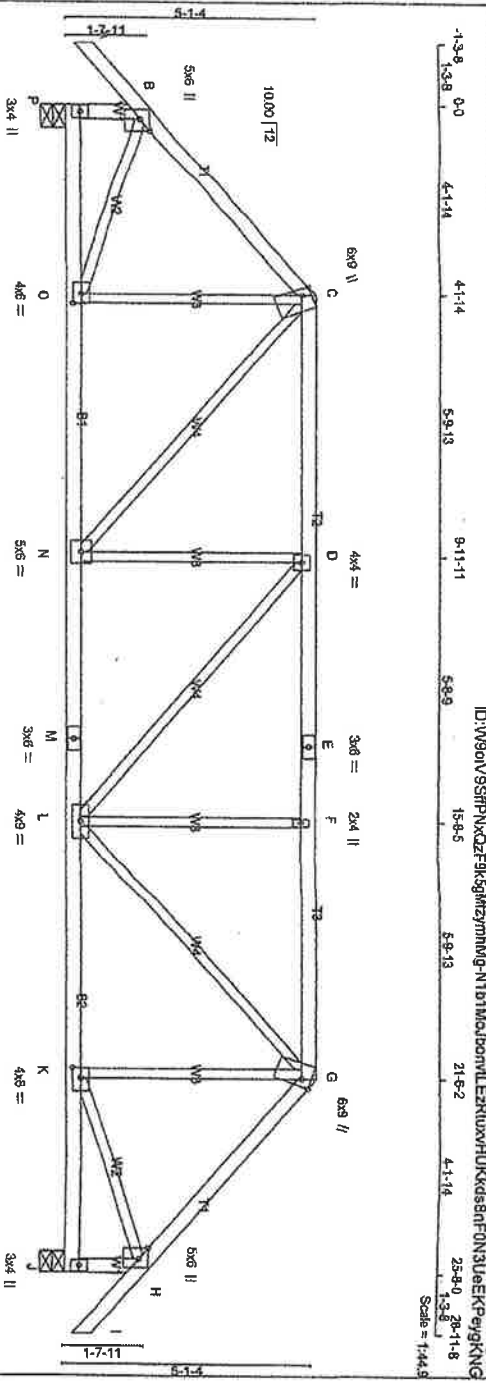
1) SPECIAL HANGERS (S) OR CONNECTIONS) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.8 lbs FACTORED DOWN AT 2-11-8, 148.2 lbs FACTORED DOWN AT 3-11-4, 147.1 lbs FACTORED DOWN AT 5-11-4, 147.1 lbs FACTORED DOWN AT 7-11-4, AND 147.1 lbs FACTORED DOWN AT 9-11-4, AND 147.1 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 89.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, AND 69.9 lbs FACTORED DOWN AT 11-11-4, AND 1539.5 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTIONS) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 46012-17
 STRUCTURAL
 COMPONENT ONLY

post

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42667	DRAWG NO.
272179	T2	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						
Version 8.030 S Oct 5 2016 Mark Industries, Inc. Wed Sep 08 15:12:45 2017 Page 1						
ID:\9801\SRP\N\GZF\SGM\m\g-N\1b1\mch\onv\1\ERL\W\H\U\K\B\N\ON\3\U\B\K\P\g\K\G						
Scale = 1/4" = 1'-0"						



1'-3.8	5'-8	4'-1.14	4'-1.14	5'-9.13	6'-11-11	5'-8.9	15'-8.5	5'-9.13	21'-6.2	4'-1.14	5'-8	1'-3.8
0.0	4'-1.14	4'-1.14	5'-9.13	6'-11-11	5'-8.9	15'-8.5	5'-9.13	21'-6.2	4'-1.14	5'-8	1'-3.8	0.0
24'-9.0												
TOTAL WEIGHT = 2 X 107 = 214 LB												

ALL WEBS 2x3 DRY No.2 SPF

LUMBER	N.L.G.A. RULES	LUMBER	DESCR	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
CHORDS	2x4	No.2	SPF	GROSS REACTION	DOWN	HORIZ	BRG
A - C	2x4	No.2	SPF	2097	0	0	5-8
E - G	2x4	No.2	SPF	2097	0	0	5-8
G - I	2x4	No.2	SPF	2097	0	0	5-8
I - K	2x4	No.2	SPF	2097	0	0	5-8
K - M	2x4	No.2	SPF	2097	0	0	5-8
M - J	2x4	No.2	SPF	2097	0	0	5-8
J - L	2x4	No.2	SPF	2097	0	0	5-8
L - N	2x4	No.2	SPF	2097	0	0	5-8
N - O	2x4	No.2	SPF	2097	0	0	5-8
O - P	2x4	No.2	SPF	2097	0	0	5-8
P - Q	2x4	No.2	SPF	2097	0	0	5-8
Q - R	2x4	No.2	SPF	2097	0	0	5-8
R - S	2x4	No.2	SPF	2097	0	0	5-8
S - T	2x4	No.2	SPF	2097	0	0	5-8
T - U	2x4	No.2	SPF	2097	0	0	5-8
U - V	2x4	No.2	SPF	2097	0	0	5-8
V - W	2x4	No.2	SPF	2097	0	0	5-8
W - X	2x4	No.2	SPF	2097	0	0	5-8
X - Y	2x4	No.2	SPF	2097	0	0	5-8
Y - Z	2x4	No.2	SPF	2097	0	0	5-8
Z - A	2x4	No.2	SPF	2097	0	0	5-8

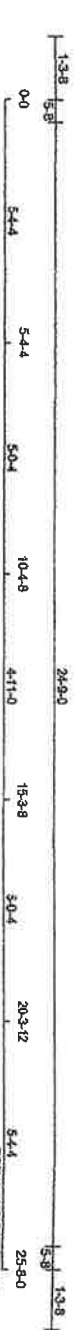
ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.													- CSA 086-09 - TPC 2011	
Edge- INDICATES TOUCHES EDGE OF CHORD.	EDGE OF CHORD.	REFERENCE CORNER OF PLATE	LOADING TOTAL LOAD CASES (4)	CHORDS				WEB S				ALLOWABLE DEF. (L) = U/960 (0.067) CALCULATED VERT. DEF. (L) = U/960 (0.137) ALLOWABLE DEF. (TL) = U/960 (0.057) CALCULATED VERT. DEF. (TL) = U/993 (0.207)		
				MAX. FACTORED FORCE (lbs)	FACTORED VERT. LOAD (L) (PL)	MAX. CSI (L)	MAX. UMBRAC (R-C)	MEMB. FORCE (lbs)	MAX. FACTORED FORCE (lbs)	MAX. CSI (L/C)				
B	THWV+P	M/20	5.0	8.0	Edge 2.75								(55 % OF 54.4 P.S.F. G.S.L. PLUS 84 P.S.F. RAIN LOAD) EQUALS 98.3 P.S.F. SPECIFIED ROOF LIVE LOAD	
C	THWV+M	M/20	5.0	8.0	Edge 1.75									
D	THWV+M	M/20	4.0	4.0										
E	TS-1	M/20	3.0	6.0										
F	THWV+W	M/20	2.0	4.0	Edge 1.75									
G	THWV+M	M/20	6.0	9.0	Edge 2.75									
H	THWV+P	M/20	3.0	4.0										
J	BAV+P	M/20	3.0	4.0										
K	BAWV+M	M/20	4.0	9.0	2.00 2.75									
L	BAWV+M	M/20	4.0	9.0										
M	BS	M/20	3.0	6.0										
N	BSWV+M	M/20	4.0	9.0										
O	BSWV+M	M/20	4.0	9.0										
P	BSWV+P	M/20	3.0	4.0										

COMPACTION LIVE LOAD FACTOR = 0.50									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									
PLATE GRIP/									

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ZF9k5gWfZymhMg-N1b1MolbonvL EZRuxvHUKJus9HF0d3UeEKPygKNG



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL =	39.3	PSF
DL =	3.0	PSF
BOT CH. LL =	10.5	PSF
DL =	7.0	PSF
TOTAL LOAD =	56.7	PSF

SPACING = 24.0 IN. OC

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.0%12

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, RBCB 2012, ABC 2014
- CSA 086-09
- TPC 2011

65 % OF 64.4 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN/LAND EQUALS 38.3 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEF. (I/L) = U₈₅₀ (0.867)
CALCULATED VERT. DEF. (I/L) = U₅₉₅ (0.097)
ALLOWABLE DEF. (I/L) = U₈₅₀ (0.867)
CALCULATED VERT. DEF. (I/L) = U₅₉₅ (0.157)

CS1: TC=0.90 (F-G1), BC=0.40 (K-M1.1), WB=0.38
(P-M1.1), SS=0.28 (C-D.1)



OWING. TAM 52102-17
STRUCTURAL
COMPONENT ONLY

[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
272179	T5	2	1			

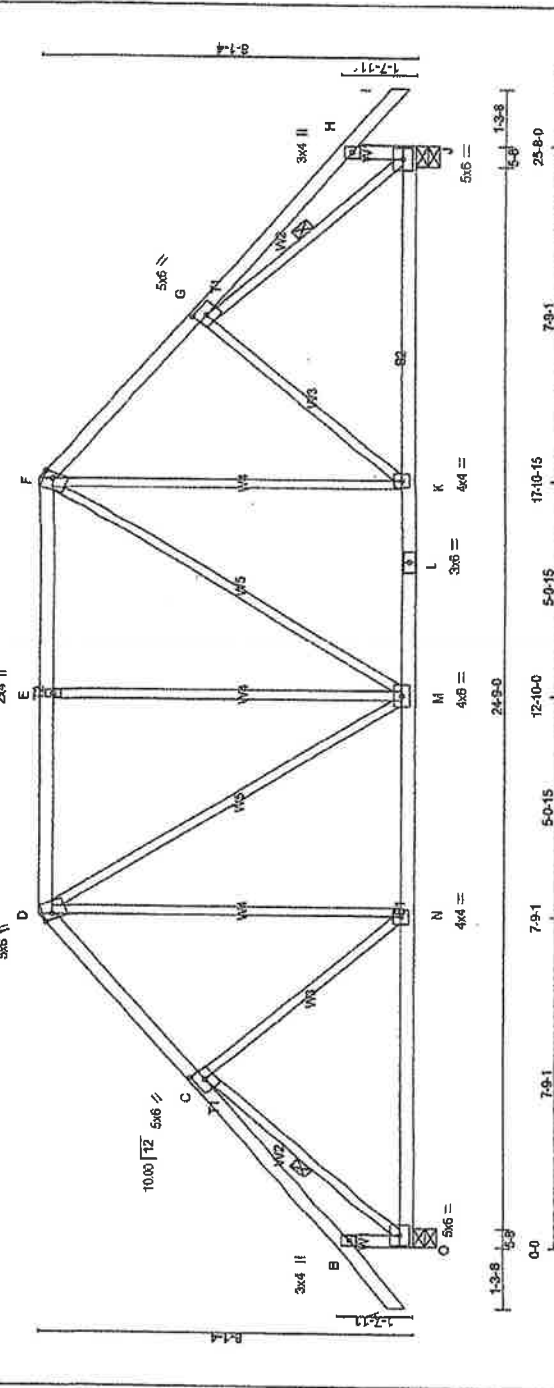
Tamarack Roof Truss, Burlington

Version: 8/20/15 Oct 5 2016 Mitek Industries, Inc. Wed Sep 08 15:12:45 2017 Page 1

ID: V50nV9SIPfNnQZfYg6fzmyHmg-N Tb lMcbDnvtL E5RtUoas/KP-th3UeKFPyG/KNG

1-3-8 0-0 3-11-13 3-9-5 7-9-1 5-0-15 12-10-0 11-10-15 3-9-5 3-11-13 25-8-0 26-11-8 1-3-8

Scale: 1/4"=1'



LUMBER										DESIGN CRITERIA									
N.L.G.A. RULES										TOTAL WEIGHT = 2 X 122 = 245 LB									
CHORDS										SLOPE OF 6.00/12									
LUMBER										LOADING IN FLAT SECTION BASED ON A									
SIZE										SLOPE OF 6.00/12									
DRY										THIS TRUSS IS DESIGNED FOR RESIDENTIAL									
DRY										OR SMALL BUILDING REQUIREMENTS OF									
DRY										PART 9, NBC 2010									
DRY										THIS DESIGN COMPLIES WITH:									
DRY										- PART 9 OF OBC 2012, CBC 2012, ABC 2014									
DRY										- CSA 088-09									
DRY										- TPC 2011									
DRY										[55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F.									
DRY										RAIN LOAD] EQUALS 38.3 P.S.F. SPECIFIED									
DRY										ROOF LIVE LOAD									
DRY										ALLOWABLE DEF. (LL) = U/360 (0.86")									
DRY										CALCULATED VERT. DEF. (LL) = U/966 (0.16")									
DRY										ALLOWABLE DEF. (TL) = U/360 (0.86")									
DRY										CALCULATED VERT. DEF. (TL) = U/668 (0.26")									
ALL WEBS 2x3										SPACING = 24.0 IN. C/C									
EXCEPT										LOADING IN FLAT SECTION BASED ON A									
DRY: SEASONED LUMBER										SLOPE OF 6.00/12									
DRY										THIS TRUSS IS DESIGNED FOR RESIDENTIAL									
DRY										OR SMALL BUILDING REQUIREMENTS OF									
DRY										PART 9, NBC 2010									
DRY										THIS DESIGN COMPLIES WITH:									
DRY										- PART 9 OF OBC 2012, CBC 2012, ABC 2014									
DRY										- CSA 088-09									
DRY										- TPC 2011									
DRY										[55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F.									
DRY										RAIN LOAD] EQUALS 38.3 P.S.F. SPECIFIED									
DRY										ROOF LIVE LOAD									
DRY										ALLOWABLE DEF. (LL) = U/360 (0.86")									
DRY										CALCULATED VERT. DEF. (LL) = U/966 (0.16")									
DRY										ALLOWABLE DEF. (TL) = U/360 (0.86")									
DRY										CALCULATED VERT. DEF. (TL) = U/668 (0.26")									

CSI: TC=0.43 (DE-1), RC=0.52 (M&Z), WE=0.86 (E&M), SSI=0.30 (E-1)

DOL LUMBER=1.00 KALI=1.00 LS BE=0.1, 1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES	SHEAR (PS)	SECTION (PLI)
MAX MIN MAX MIN MAX MIN		
MT20	618 354 1657 822 2384 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRP= 0.90 (G) (INPUT = 0.80)
JSI METAL= 0.94 (G) (INPUT = 1.00)



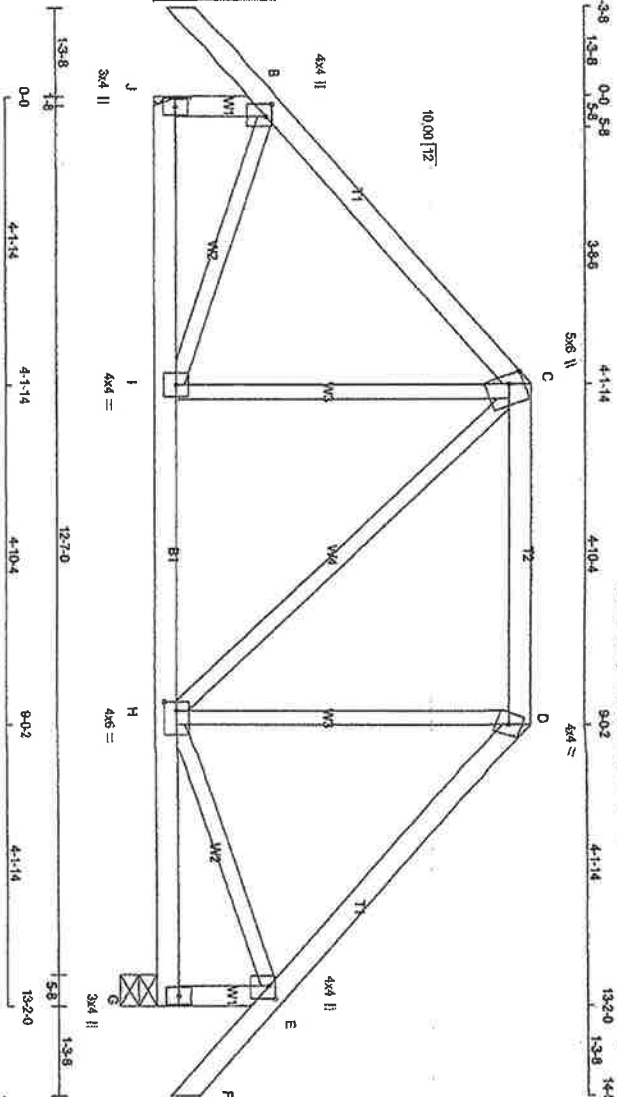
DWG NO. TAM 5010417
STRUCTURAL
COMPONENT ONLY

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JOB NAME 272426 Tamarack Roof Truss, Burlington	TRUSS NAME T9	QUANTITY 1	PLY 2	JOB DESC. 42687	DRWG NO.																																																		
Version 8.000 S Oct 5 2016 MJtek Industries, Inc. Mon Sep 11 13:37:53 2017 Page 2 ID:xs1UD4jP8rQnSDmCytZ5yKky-Dhm23HvefDboeHRWBBI6Ep8CSkmDdsDh4AvefC																																																							
HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 277.2 lbs FACTORED DOWN AT 1-11-4, 277.2 lbs FACTORED DOWN AT 3-11-4, 277.2 lbs FACTORED DOWN AT 5-11-4, 277.2 lbs FACTORED DOWN AT 7-11-4, 277.2 lbs FACTORED DOWN AT 9-11-4, AND 277.2 lbs FACTORED DOWN AT 11-5-4, AND 1048.3 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.		<table border="1"> <thead> <tr> <th colspan="4">FACTORED CONCENTRATED LOADS (LBS)</th> <th colspan="2">FACE</th> <th colspan="2">DIR.</th> <th colspan="2">TYPE</th> </tr> <tr> <th>JT</th> <th>LOC.</th> <th>LC1</th> <th>MAX-</th> <th>MAX+</th> <th>BACK</th> <th>VERT</th> <th>VERT</th> <th>TOTAL</th> <th>TOTAL</th> </tr> </thead> <tbody> <tr> <td>U</td> <td>9-11-4</td> <td>-277</td> <td>-277</td> <td>-</td> <td>BACK <td>VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td></td></td></tr> <tr> <td>V</td> <td>11-5-4</td> <td>-277</td> <td>-277</td> <td>-</td> <td>BACK <td>VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td></td></td></tr> <tr> <td>W</td> <td>12-2-8</td> <td>-1048</td> <td>-1048</td> <td>-</td> <td>BACK <td>VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td></td></td></tr> </tbody> </table>				FACTORED CONCENTRATED LOADS (LBS)				FACE		DIR.		TYPE		JT	LOC.	LC1	MAX-	MAX+	BACK	VERT	VERT	TOTAL	TOTAL	U	9-11-4	-277	-277	-	BACK <td>VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td></td>	VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td>	VERT <td>TOTAL</td> <td>TOTAL</td>	TOTAL	TOTAL	V	11-5-4	-277	-277	-	BACK <td>VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td></td>	VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td>	VERT <td>TOTAL</td> <td>TOTAL</td>	TOTAL	TOTAL	W	12-2-8	-1048	-1048	-	BACK <td>VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td></td>	VERT <td>VERT <td>TOTAL</td> <td>TOTAL</td> </td>	VERT <td>TOTAL</td> <td>TOTAL</td>	TOTAL	TOTAL
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		LICENSED PROFESSIONAL ENGINEER 9/24/16 S. KATSOULAKOS PROVINCE OF ONTARIO <i>pur</i> DWG NO. TAM 4603-17 STRUCTURAL COMPONENT ONLY																																																					



DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL =	38.3 PSF
DL =	3.0 PSF
BOT CH. LL =	10.5 PSF
DL =	7.0 PSF
TOTAL LOAD =	59.7 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

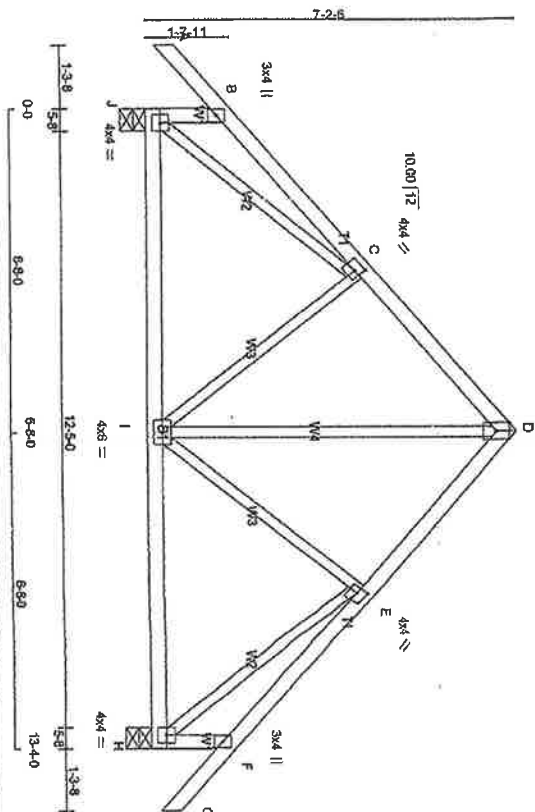
THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA B85-09
- TPIC 2011

(55% OF S4 P.S.F. G.S.T. PLUS 84 P.S.F.
RAIN LOAD) EQUALS 383 P.S.F. SPECIFIED
ROOF LINE LOAD



OWEND, TAM
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 63 = 127 lb (kN)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

LUMBER N.L.G.A. RULES	CHORDS SIZE	LUMBER No.2	DESCR.
A - D	2x4	DRY	SPF
E - G	2x4	DRY	SPF
H - I	2x4	DRY	SPF
J - K	2x4	DRY	SPF
L - M	2x4	DRY	SPF
N - O	2x4	DRY	SPF
P - Q	2x4	DRY	SPF
R - S	2x4	DRY	SPF
T - U	2x4	DRY	SPF
V - W	2x4	DRY	SPF
X - Y	2x4	DRY	SPF
Z	2x4	DRY	SPF

UNFACTORED REACTIONS

1ST LOSE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERMATIVE WIND DEAD SOIL
JT	867 616/0 140/0 0/0 0/0 142/0 0/0
H	897 616/0 140/0 0/0 0/0 142/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED MOMENT (LBS-FT)
FR-TO	0/54	-122.2	0.17 (1)	10.00	1-D	0.12 (1)
A-B	0/30	-122.2	0.21 (1)	10.00	1-E	0.09 (1)
B-C	0/30	-122.2	0.17 (1)	10.00	1-F	0.09 (1)
C-D	-690/0	-122.2	0.17 (1)	6.25	J-C	0.44 (1)
D-E	-690/0	-122.2	0.17 (1)	10.00	E-H	0.44 (1)
E-F	0/30	-122.2	0.21 (1)	10.00		
F-G	0/54	-122.2	0.17 (1)	10.00		
G-H	-325/0	0.0	0.03 (1)	7.81		
H-F	-325/0	0.0	0.03 (1)	7.81		
J-I	0/631	-28.0	0.42 (2)	10.00		
I-H	0/631	-28.0	0.42 (2)	10.00		

WEBS

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)

MAX. FACTORED MOMENT (LBS-FT)

MAX. UNBRACED LENGTH (FT)

MAX. MEMB. FORCE (LBS)

MAX. FACTORED FORCE (LBS)



DWBD, TAM 4605617
 STRUCTURAL
 COMPONENT ONLY

272426

T12S

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TRUSS DESC.

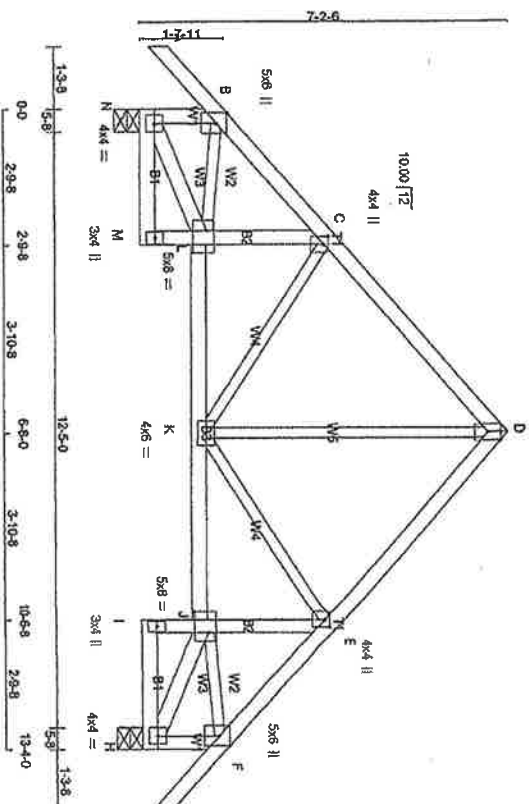
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MATEX industries, Inc. Mon Sep 11 13:37:55 2017 Page 1
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10-88 240 13-08 13-4-0 14-7-8
28 438

13.8 0.58 2.40 2.98 3.10.8 6.8.0 3.10.8 10.6.8 2.4.0 13.0.8 13.4.0 14.7.8
1.3.8 0.58 2.40 2.98 3.10.8 6.8.0 3.10.8 10.6.8 2.4.0 13.0.8 13.4.0 14.7.8
4.6.11

Scale = 1:43.2



TOTAL WEIGHT = $3 \times 71 = 212$ lb

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

N.L.G.A. RULES		LUMBER		DESCR.	
CHORDS	SIZE				
A - D	2x4	DRY	No.2	SF	SF
D - G	2x4	DRY	No.2	SF	SF
G - B	2x4	DRY	No.2	SF	SF
B - F	2x4	DRY	No.2	SF	SF
F - A	2x4	DRY	No.2	SF	SF
M - C	2x4	DRY	No.2	SF	SF
C - E	2x4	DRY	No.2	SF	SF
E - H	2x4	DRY	No.2	SF	SF
ALL WEBS		DRY	No.2	SF	SF
EXCEPT	2x4	DRY	No.2	SF	SF
N - L	2x4	DRY	No.2	SF	SF
J - H	2x4	DRY	No.2	SF	SF

BUILDING DESIGNER									
BEARINGS									
FACTORED					MAXIMUM FACTORED				
GROSS REACTION					GROSS REACTION				
JT	VERT	HORIZ			DOWN	HORIZ	UPLIFT	IN-SX	
N	1171	0	0		1171	0	0	5.8	5.8
H	1171	0			1171	0		5.8	5.8

UNFACTORED REACTIONS									
1ST LAYER					MAXIMUM COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE		PERLATIVE	WIND		DEAD	SOIL
N	897	616/0	140/0		0/0	0/0		142/0	0/0
H	897	616/0	140/0		0/0	0/0		142/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

DRY: SEASONED LUMBER

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

PLATE TYPE	PLATES	W	LEN	Y	X
B. TIANW-P	MT20	5.0	6.0	Edge	
C. TIANW-P	MT20	4.0	4.0	100	2.00
D. TIANW-P	MT20	4.0	6.0	Edge	
E. TIANW-P	MT20	4.0	4.0	100	2.00
F. TIANW-P	MT20	5.0	6.0	Edge	
G. TIANW-P	MT20	4.0	4.0		
H. TIANW-P	MT20	3.0	4.0		
I. TIANW-P	MT20	3.0	8.0	3.25	5.50
J. TIANW-P	MT20	4.0	6.0		
K. TIANW-P	MT20	5.0	8.0	3.25	5.50
L. TIANW-P	MT20	3.0	4.0		
M. TIANW-P	MT20	4.0	4.0		
N. TIANW-P	MT20	4.0	4.0		

EDGE - INDICATES REFERENCE CORNER OF PLAIN TOUCHES EDGE OF CHORD.

TOTAL LOAD CASES: (4)									
LOADING					WEBS				
CHORDS					MEMB.				
MEMB.	MAX FACTORED FORCE (KBS)	VERT LOAD/LC (PL)	MAX CS1 (LC)	MAX UNBRAC FR-TO	MEMB. FORCE (KBS)	MAX CS1 (LC)	MAX FACTORED FORCE (KBS)	MAX CS1 (LC)	MAX FACTORED FORCE (KBS)
FR-TO	FROM TO	FR-TO	FROM TO	FR-TO	FR-TO	FROM TO	FR-TO	FROM TO	FR-TO
A-B	0.154	-122.2	-122.2	0.17 (1)	10.00	K-D	0.559	0.13 (1)	0.13 (1)
B-C	-1370.10	-122.2	0.20 (1)	5.89	K-E	-427.10	0.16 (1)	0.16 (1)	0.16 (1)
C-D	-780.10	-122.2	0.28 (1)	6.25	C-K	-427.10	0.18 (1)	0.18 (1)	0.18 (1)
D-E	-1780.10	-122.2	0.23 (1)	6.25	N-L	-25.10	0.09 (1)	0.09 (1)	0.09 (1)
E-F	-1770.10	-122.2	0.20 (1)	5.68	B-L	0.1935	0.21 (1)	0.21 (1)	0.21 (1)
F-G	0.684	-122.2	0.17 (1)	10.00	J-H	-25.10	0.00 (1)	0.00 (1)	0.00 (1)
G-H	-1124.10	0.0	0.0	0.12 (1)	J-F	0.1835	0.21 (1)	0.21 (1)	0.21 (1)
H-F	-124.10	0.0	0.0	0.12 (1)					
				7.48					
N-M	0.723	-28.0	-28.0	0.06 (1)	10.00				
M-L	0.655	0.0	0.0	0.04 (1)	10.00				
L-C	0.1112	0.0	0.0	0.04 (1)	10.00				
L-K	0.8941	-28.0	-28.0	0.22 (1)	10.00				
K-J	0.9941	-28.0	-28.0	0.22 (1)	10.00				
I-J	0.655	0.0	0.0	0.04 (1)	10.00				
J-E	0.1112	0.0	0.0	0.04 (1)	10.00				
I-H	0.7233	-28.0	-28.0	0.06 (1)	10.00				

ROCK LIFE LOAD

ALLOWABLE DEF. (L) = $U/990$ (0.447)
CALCULATED VERT. DEF. (L) = $U/999$ (0.033)
ALLOWABLE DEF. (T) = $U/950$ (0.47)
CALCULATED VERT. DEF. (T) = $U/990$ (0.047)

CSI: T=0.023 (D-E), RC=0.22 (K-L-2), WB=0.21 (B-L), SS=0.17 (D-E-1)

DO: LUMBER=1.00 NAIL=1.00 S. BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPARISON LIVE LOAD FACTOR = 0.50

AUTOSLOVE HELPS OFF

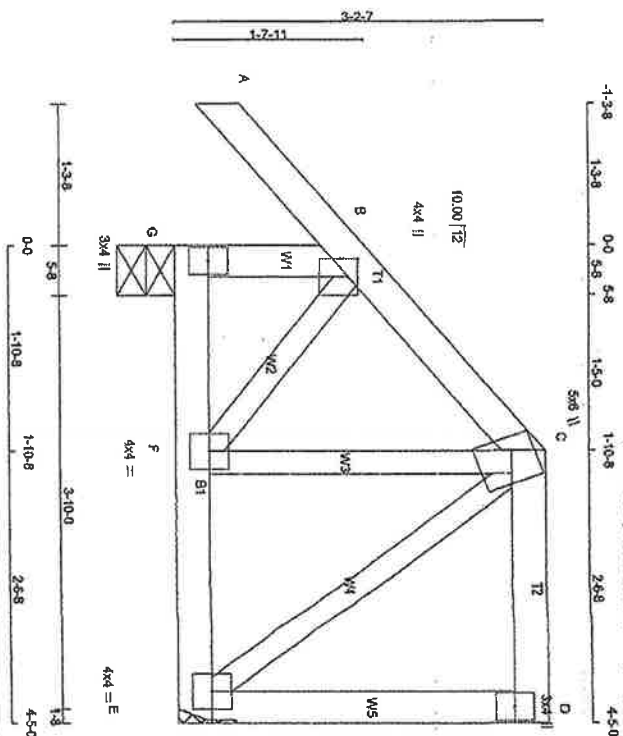
TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

WALL VALUES

PLATE (GAP/GR)	SHEAR	SECTION
(PS)	(PL)	
(PS)	(PL)	
MAX MIN	MAX MIN	MAX MIN
48720	618	354 1657 622 2284 1655

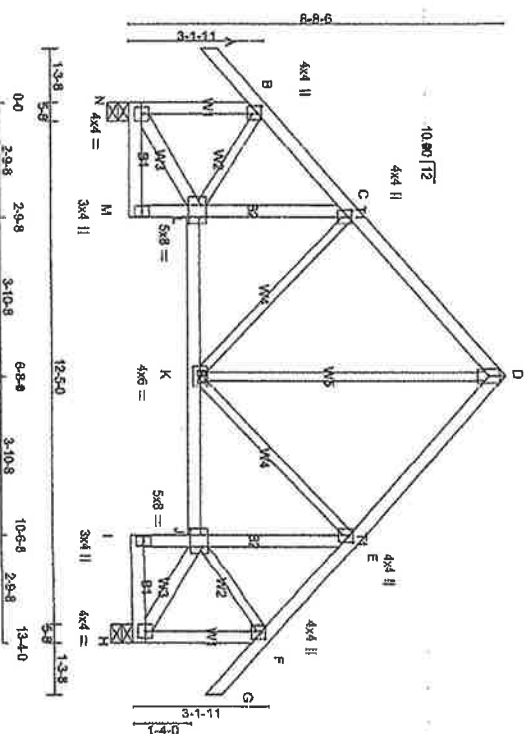


DWANE, TAM 4607-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 8 X 24 = 142 lb

BUILDINGS, SUPPORTS, AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
LUMBER		N.L.G.A. RULES		DESCR.		BEARINGS			
CHORDS	SIZE	LUMBER	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQUIRED BRG		
A - C	2x4	DRY	No.2	VERT	DOWN	UP/LIFT	IN-SX		
C - D	2x4	DRY	No.2	VERT	HORIZ		IN-SX		
E - D	2x4	DRY	No.2	SPF					
G - B	2x4	DRY	No.2	SPF					
G - E	2x4	DRY	No.2	SPF					
ALL WEBS		2x3	DRY	No.2	SPF				
EXCEPT									
UNFACTORED REACTIONS									
1ST LOOSE		MAX MIN. COMPONENT REACTIONS		DEAD		SOIL			
JT	COMBINED	SNOW	LIVE	PER LIVE	WIND				
DRY: SEASONED LUMBER									



TOTAL WEIGHT = 2X 80 = 161 LB

LUMBER N.L.G.A. RULES		LUMBER		DESCR.	
CHORDS	SIZE				
A - D	2x4	DRY	No.2	SPE	
D - G	2x4	DRY	No.2	SPE	
N - B	2x4	DRY	No.2	SPE	
N - F	2x4	DRY	No.2	SPE	
N - M	2x4	DRY	No.2	SPE	
M - C	2x4	DRY	No.2	SPE	
L - J	2x4	DRY	No.2	SPE	
I - E	2x4	DRY	No.2	SPE	
H	2x4	DRY	No.2	SPE	
ALL WEBS	2x3	DRY	No.2	SPE	
EXCEPT	2x4	DRY	No.2	SPE	
J - H	2x4	DRY	No.2	SPE	
DRY SEASONED LUMBER.					

BUILDING DESIGNER		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
BEARINGS		GROSS REACTION		DOWN		HORIZ		BRG	
VERT		HORIZ		UP/LEFT		IN/SLX		IN/SLX	
JT	1171	0	1171	0	0	5.8	5.8	5.8	5.8
H	1171	0	1171	0	0	5.8	5.8	5.8	5.8

UNFACTORED REACTIONS		MAX MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LOOSE		SNOW		PERM LIVE		0/0		0/0	
JT	897	816/0	140/0	8/0	0/0	142/0	0/0	0/0	0/0
H	897	816/0	140/0	8/0	0/0	142/0	0/0	0/0	0/0

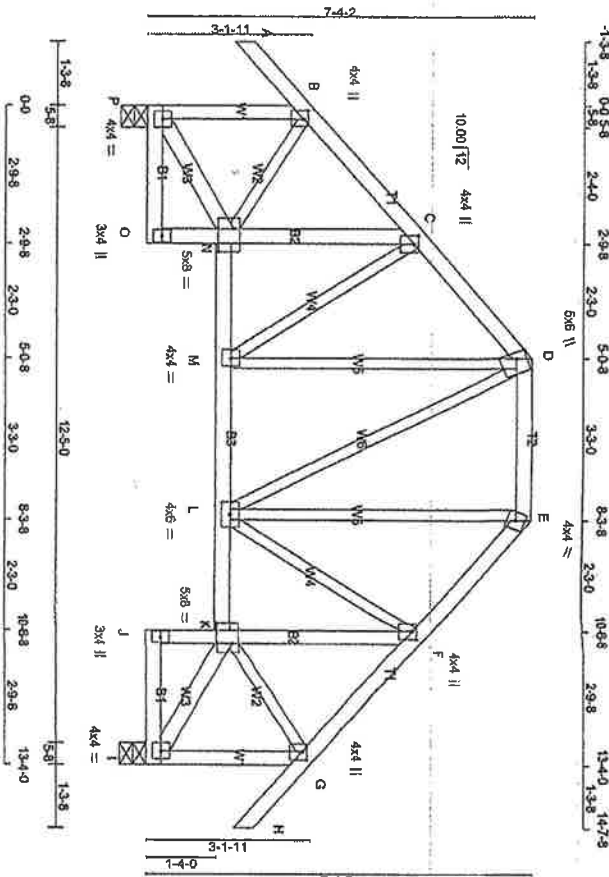
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.		WEBS		MAX. FACTORED	
LOADING		MEMB.		FORCE	
CHORDS		FROM TO		CS1 (LC)	
A-B		-122.2 -122.2 0.17 (1)		0.09 (1)	
B-C		-122.2 -122.2 0.20 (1)		0.11 (1)	
C-D		-122.2 -122.2 0.22 (1)		0.11 (1)	
D-E		-122.2 -122.2 0.22 (1)		0.00 (1)	
E-F		-122.2 -122.2 0.20 (1)		0.16 (1)	
F-G		-122.2 -122.2 0.17 (1)		0.00 (1)	
G-H		-122.2 -122.2 0.17 (1)		0.17 (2)	
H-F		0.0 0.0 0.20 (1)		0.16 (1)	



DWG NO. TAW 46013-17
 STRUCTURAL
 COMPONENT ONLY

Tamarack Roof Truss, Burlington

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Scale = 1/4"=1'-0"

TOTAL WEIGHT = 861 lb (min)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

ALL G. A. RULES	CHORDS	SIZE	DESCR	NUMBER
A - D	2x4	DRY	SPF	No.2
E - H	2x4	DRY	SPF	No.2
I - J	2x4	DRY	SPF	No.2
K - L	2x4	DRY	SPF	No.2
M - N	2x4	DRY	SPF	No.2
O - P	2x4	DRY	SPF	No.2
Q - R	2x4	DRY	SPF	No.2
S - T	2x4	DRY	SPF	No.2
U - V	2x4	DRY	SPF	No.2
W - X	2x4	DRY	SPF	No.2
Y - Z	2x4	DRY	SPF	No.2

FACTORED BEARINGS	FACTORED BEARINGS	MAXIMUM FACTORED BEARINGS	INPUT RECORD
VERT	GROSS REACTION	DOWN	BRG
HORIZ	GROSS REACTION	UP/LIFT	BRG
VERT	GROSS REACTION	DOWN	BRG
HORIZ	GROSS REACTION	UP/LIFT	BRG

1ST CASE	MAXIMUM COMPONENT REACTIONS	WIND	DEAD	SOL
JT	897	616/0	140/0	0/0
I	897	616/0	140/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (1)	MAX CS (1)	MEMB.	MAX FACTORED FORCE (LBS)	MAX CS (1)	MAX CS (1)
FR-TO	FROM TO	CS (1)	LENGTH	FR-TO	FROM TO	CS (1)	LENGTH
A-B	-779/0	-1222	0.17 (1)	0.00	C-M	-118/0	0.04 (1)
B-C	-745/0	-1222	0.17 (1)	6.25	M-D	0.17/8	0.04 (2)
C-D	-565/0	-1222	0.10 (1)	6.25	D-L	0.12	0.04 (2)
D-E	-746/0	-1222	0.17 (1)	6.25	L-E	0.19/0	0.04 (2)
E-F	-779/0	-1222	0.10 (1)	6.25	F-F	-116/0	0.04 (1)
F-G	-779/0	-1222	0.10 (1)	6.25	P-N	-10/0	0.09 (1)
G-H	-1129/0	-1222	0.17 (1)	10.00	K-I	-78/3	0.16 (1)
H-I	-1129/0	0.0	0.20 (1)	7.47	B-N	-10/0	0.02 (1)
I-G	0.0	0.0	0.20 (1)	7.47	K-G	0.1/93	0.16 (1)
P-O	0.1/8	-28.0	0.06 (2)	10.00			
N-C	-293/4	0.0	0.02 (1)	10.00			
M-H	0.1/618	0.0	0.02 (1)	7.81			
L-L	0.1/555	-28.0	0.12 (1)	10.00			
L-K	0.1/618	-28.0	0.10 (1)	10.00			
J-K	0.1/65	0.0	0.02 (1)	10.00			
K-F	-293/3	0.0	0.02 (1)	7.81			
	0.1/8	-28.0	0.06 (2)	10.00			

ENSED PROFESSIONAL

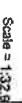
9/14/7



DWG NO. TAM 46014-17
STRUCTURAL
COMPONENT ONLY

TRUSS DESC.

Version 8.030 S Oct 5 2018 iATek Industries, Inc. Mon Sep 11 13:32:52 2017 Page 1
ID: 0qcbEKARi9Yz0j3Gcdv8yKlIG-Xn5GUmSqS1S1SKOmAO7H7ZLKpQsmpwHtHaeQpF77ysIMw

LOADING IN FLAT SECTION BASED ON A SCORE OF 6.0043

THIS DESIGN COMPLIES WITH:
-PART 9 OF OBC 2012, BCBC 2012, ABC 2014
-CSA 086-09
TMC 2014

ALLOWABLE DEF. (L)₁ = U 260 (0.44°)
CALCULATED VERT. DEF. (L) = U 999 (0.05°)
ALLOWABLE DEF. (L)₂ = U 560 (0.44°)
CALCULATED VERT. DEF. (L) = U 999 (0.10°)

$$\text{COMPANION LIVE LOAD FACTOR} = 0.50$$

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE VALUES	SHEAR		SECTION
PLATE GRIPOIDRY	(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

```
JSI GRIP= 0.84 (P) (INPUT = 0.90 )
JSI METAL= 0.22 (B) (INPUT = 1.00 )
```



DIYANO, TAN 460617
STRUCTURAL
COMPONENT ONLY



DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 38.3 PSF
DL	= 3.0 PSF
BOI CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 58.7 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 008-09
- TFC 2011

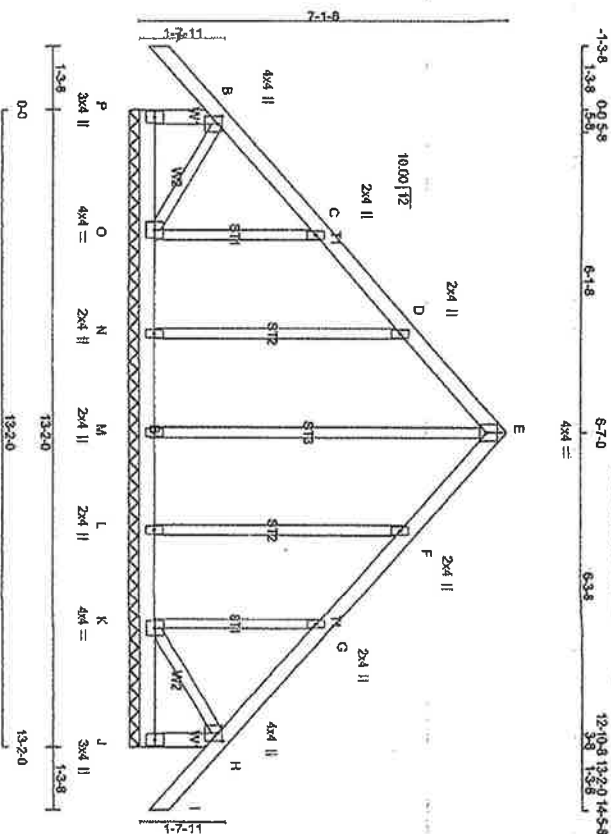
65% OF 54 P.S.F. G.S.L. PLUS 8 P.S.F. RAIN LOAD EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.44")
CALCULATED VERT. DEF. (LL) = L/999 (0.12")

CS1: TO=0.29 (B/C:1), EO=0.44 (H/L2), WB=0.55
 CALCULATED VERT. DEF. (U)= 1/827 (0.19)
 (C+1), SS=0.17 (H+1.3)
 DO. LUMBER=1.00 NAL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TEN=1.10
 COMPANION LIVE LOAD FACTOR = 0.35
 TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL
 IN THE TRUSS MANUFACTURING PLANT
 NAL VALUES



DWING, TAM 460167
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 64 lb

LUMBER		N.L.G.A. RULES		DESORT	
CHORDS	SIZE	DRY	LUMBER	SPF	SPF
C - B	2x4	DRY	No.2	SPF	SPF
A - E	2x4	DRY	No.2	SPF	SPF
E - I	2x4	DRY	No.2	SPF	SPF
J - H	2x4	DRY	No.2	SPF	SPF
P - J	2x4	DRY	No.2	SPF	SPF
ALL WEBS		2x3	DRY	No.2	SPF
ALL GABLE WEBS		2x3	DRY	No.2	SPF
GABLE STUDS SPACED AT 2'-0" O.C.					

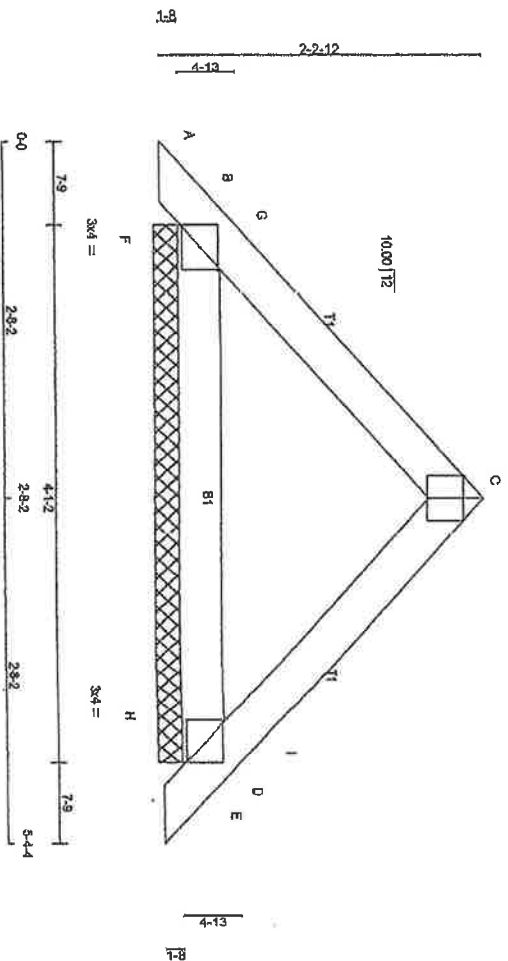
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER												
BEARINGS												
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.												
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.												
BEARING MATERIAL TO BE SIF NO.2 OR BETTER AT JOINT(S)												
BRACING												
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.												
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.												
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATEROALLY RESTRAINED												
LOADING												
TOTAL LOAD CASES: (4)												
CHORDS												
MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED HORZ. LOAD (CL)	MAX CSI (L/C)	MAX UNBRACED LENGTH (FT.)	WEAR MEAB.	W E B S	MAX FACTORED FORCE (LBS)	MAX CSI (L/C)			
PR-TO	FROM	TO										
P-B	-344 / 0	0.0	0.0	0.04 (1)	7.81	M-E	-171 / 0	0.15 (1)	0.15 (1)			
A-B	0 / 5.4	-122.2	-122.2	0.17 (1)	10.00	N-D	-257 / 0	0.11 (1)	0.11 (1)			
B-C	-32 / 0	-122.2	-122.2	0.09 (1)	6.25	O-C	-306 / 0	0.07 (1)	0.07 (1)			
C-D	-45 / 0	-122.2	-122.2	0.09 (1)	6.25	L-F	-257 / 0	0.11 (1)	0.11 (1)			
D-E	-46 / 0	-122.2	-122.2	0.06 (1)	6.25	K-G	-306 / 0	0.07 (1)	0.07 (1)			
E-F	-45 / 0	-122.2	-122.2	0.06 (1)	6.25	B-D	0 / 42	0.01 (1)	0.01 (1)			
F-G	-32 / 0	-122.2	-122.2	0.09 (1)	6.25	K-H	0 / 42	0.01 (1)	0.01 (1)			
G-H	0 / 5.4	-122.2	-122.2	0.17 (1)	10.00							
H-I	0 / 5.4	0.0	0.0	0.04 (1)	7.81							
J-H	-344 / 0	0.0	0.0	0.04 (1)	7.81							
P-O	0 / 0	-28.0	-28.0	0.04 (1)	10.00							
C-N	0 / 29	-28.0	-28.0	0.04 (1)	10.00							
N-M	0 / 25	-28.0	-28.0	0.03 (1)	10.00							
M-L	0 / 25	-28.0	-28.0	0.03 (1)	10.00							
L-K	0 / 29	-28.0	-28.0	0.04 (1)	10.00							
K-J	0 / 0	-28.0	-28.0	0.04 (1)	10.00							

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH	LL = 36.3 PSF
BOT CH	DL = 3.0 PSF
DL	LL = 10.5 PSF
DL	LL = 7.0 PSF
TOTAL LOAD	= 56.7 PSF
SPACING	= 24.0 IN. OC
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC/C 2010	
THIS DESIGN COVERS WITH:	
- PART 9 OF NBC 2012, CBC/C 2012, ABC 2014	
- CSA S308-08	
- TRC 2011	
65% OF 44.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 83.3 P.S.F. SPECIFIED ROOF LIVE LOAD	
CSI: TO=0.17 (A-B), BC=0.04 (K-L2), WB=0.15 (E-M1), SSM=0.11 (G+H1)	
DOL LUMBER=1.00 NAIL=L00 L5 BEND=1.10	
COMP=1.10 SHEAR=1.10 TENS=1.10	
COMPANION LIVE LOAD FACTOR = 0.50	
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.	
NAIL VALUES	
PLATE GRIP/DUR	SHEAR SECTION
(P-SI)	(P-L)
(P-L)	(P-L)
MAX MIN MAX MIN MAX MIN	
M120 618 354 1667 822 2294 1556	
PLATE PLACEMENT TOL. = 2/50 inches	
PLATE ROTATION TOL. = 5.0 Deg.	
ISI GRP= 0.28 (E), (INPUT = 0.80)	
ISI METAL= 0.08 (C) (INPUT = 1.00)	



DWNO. TAM 4601-17
STRUCTURAL
COMPONENT ONLY

00 2-9-2 3-4 = 2-9-2 2-9-2 5-4-4 Scale = 1:15.4



7-9 2-9-2 4-1-2 2-9-2 7-9 5-4-4 TOTAL WEIGHT = 2 X 13 = 25.0 (lb)

LUMBER N.L.G.A. RULES			
CHORDS	SIZE	LAMBER	DESCR
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
DRY: SEASONED LUMBER.			
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:			
BUILDING DESIGNER			
FACTORED			
1ST CASE	2ND CASE	3RD CASE	4TH CASE
1	2	3	4
5	6	7	8
9	10	11	12
13	14	15	16
17	18	19	20
21	22	23	24
25	26	27	28
29	30	31	32
33	34	35	36
37	38	39	40
41	42	43	44
45	46	47	48
49	50	51	52
53	54	55	56
57	58	59	60
61	62	63	64
65	66	67	68
69	70	71	72
73	74	75	76
77	78	79	80
81	82	83	84
85	86	87	88
89	90	91	92
93	94	95	96
97	98	99	100

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY			
UNFACTORED REACTIONS			
1ST CASE	2ND CASE	3RD CASE	4TH CASE
1	2	3	4
5	6	7	8
9	10	11	12
13	14	15	16
17	18	19	20
21	22	23	24
25	26	27	28
29	30	31	32
33	34	35	36
37	38	39	40
41	42	43	44
45	46	47	48
49	50	51	52
53	54	55	56
57	58	59	60
61	62	63	64
65	66	67	68
69	70	71	72
73	74	75	76
77	78	79	80
81	82	83	84
85	86	87	88
89	90	91	92
93	94	95	96
97	98	99	100

FACTORED			
1ST CASE	2ND CASE	3RD CASE	4TH CASE
1	2	3	4
5	6	7	8
9	10	11	12
13	14	15	16
17	18	19	20
21	22	23	24
25	26	27	28
29	30	31	32
33	34	35	36
37	38	39	40
41	42	43	44
45	46	47	48
49	50	51	52
53	54	55	56
57	58	59	60
61	62	63	64
65	66	67	68
69	70	71	72
73	74	75	76
77	78	79	80
81	82	83	84
85	86	87	88
89	90	91	92
93	94	95	96
97	98	99	100

FACTORED			
1ST CASE	2ND CASE	3RD CASE	4TH CASE
1	2	3	4
5	6	7	8
9	10	11	12
13	14	15	16
17	18	19	20
21	22	23	24
25	26	27	28
29	30	31	32
33	34	35	36
37	38	39	40
41	42	43	44
45	46	47	48
49	50	51	52
53	54	55	56
57	58	59	60
61	62	63	64
65	66	67	68
69	70	71	72
73	74	75	76
77	78	79	80
81	82	83	84
85	86	87	88
89	90	91	92
93	94	95	96
97	98	99	100



INTERTRAC
 STRUCTURAL
 COMPONENT ONLY

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: 690 galvanized

DESIGN:

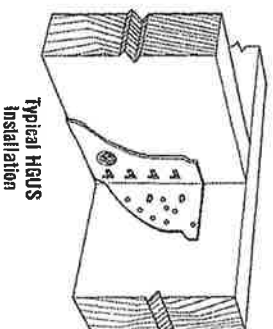
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%.
- No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

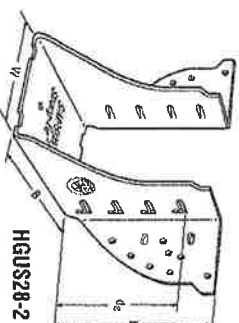
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

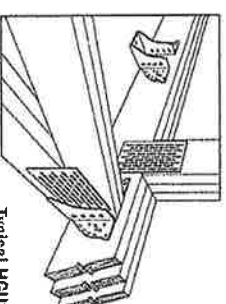
- See current catalogue for options



Typical HGUS Installation



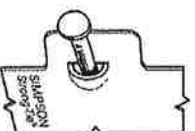
HGUS28-2



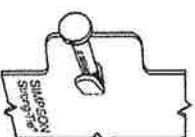
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners				Factored Resistance (lbs)			
		W	H	B	d ₁	Face	Joist	Uplift	Normal	D-F ₁ -1	S-P-F	Uplift	Normal
HGUS26	12	1½	5¾	5	4¾	20-16d	8-16d	2685	6625	2585	5700		
HGUS26-2	12	3¾	5¾	4	4¾	20-16d	8-16d	4385	8950	3100	6355		
HGUS26-3	12	4¾	5½	4	4¾	20-16d	8-16d	4385	8950	3100	6355		
HGUS26-4	12	6¾	5¾	4	4¾	20-16d	8-16d	4385	8950	3100	6355		
HGUS28	12	1¾	7¼	5	6¼	36-16d	12-16d	3310	7675	3100	6900		
HGUS28-2	12	3¾	7¾	4	6¼	36-16d	12-16d	6070	12980	4310	9215		
HGUS28-3	12	4¾	7¼	4	6¼	36-16d	12-16d	6070	12980	4310	9215		
HGUS28-4	12	6¾	7¾	4	6¼	36-16d	12-16d	6070	12980	4310	9215		
HGUS210-2	12	3¾	9¾	4	8¼	46-16d	16-16d	6840	14645	4855	10400		
HGUS210-3	12	4¾	9¼	4	8¼	46-16d	16-16d	6840	14645	4855	10400		
HGUS210-4	12	6¾	9¾	4	8¼	46-16d	16-16d	6840	14645	4855	10400		
HGUS212-4	12	6¾	10¾	4	10¼	56-16d	20-16d	7640	14995	5425	10645		
HGUS214-4	12	6¾	12¾	4	11¼	66-16d	22-16d	10130	16400	7195	11645		

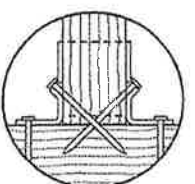
1. d₂ is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPEC-HGUS15 1/15 rev. 12/16

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

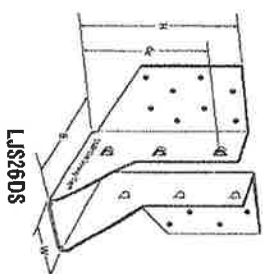
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

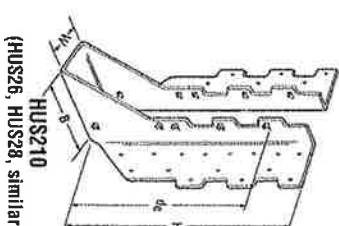
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

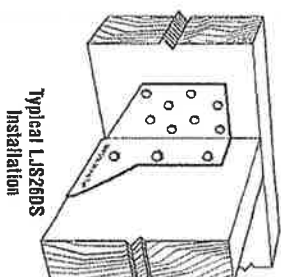
- See current catalogue for options



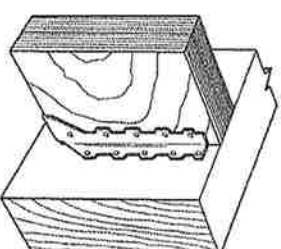
LJS26DS



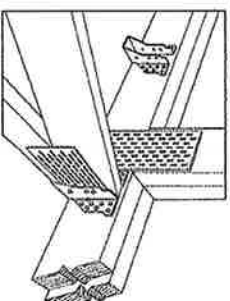
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



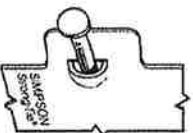
Typical HUS
Installation



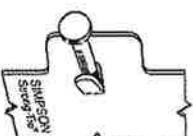
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga	Dimensions (in)				Fasteners				Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist			D, Ft-L	S-P-F		
LJS26DS	18	1⅞	5	3½	4⅜	16-16d	6-16d	2055	4265	1460	4115		
HUS26	16	1⅞	5⅜	3	3⅞	14-16d	6-16d	2705	4940	2065	3875		
HUS28	16	1⅞	7⅞	3	6⅞	22-16d	8-16d	3605	5385	2675	4345		
HUS210	16	1⅞	9⅞	3	7⅞	30-16d	10-16d	4505	5795	4010	4740		
HUS18/10	16	1⅞	9	3	8	30-16d	10-16d	4505	6450	4010	5200		

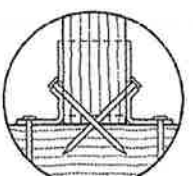
1. d_g is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.simpsonstrongtie.com.
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1-SPECHUS15 1/15 exp. 12/16

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www.simpsonstrongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

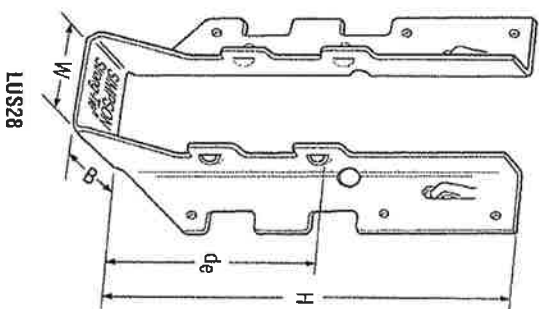
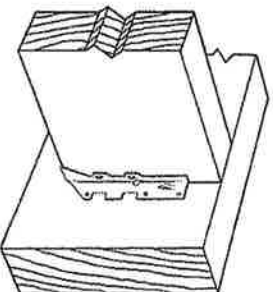
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

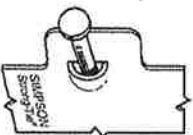
- These hangers cannot be modified.

Typical LUS Installation

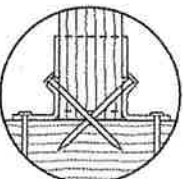


Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	de ¹	Face	Joist	D, Fl-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1⅞	3⅞	1¾	1⅞	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅞	4-16d	2-16d	835	2020	590	1435
LUS26	18	1⅞	4¼	1¾	3⅞	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4⅞	2	3¼	4-16d	4-16d	1720	2695	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¼	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1⅞	7¾	1¾	3¾	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8¾	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. de is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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1-SPEC LUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

TECH-NOTES

TN 15-001

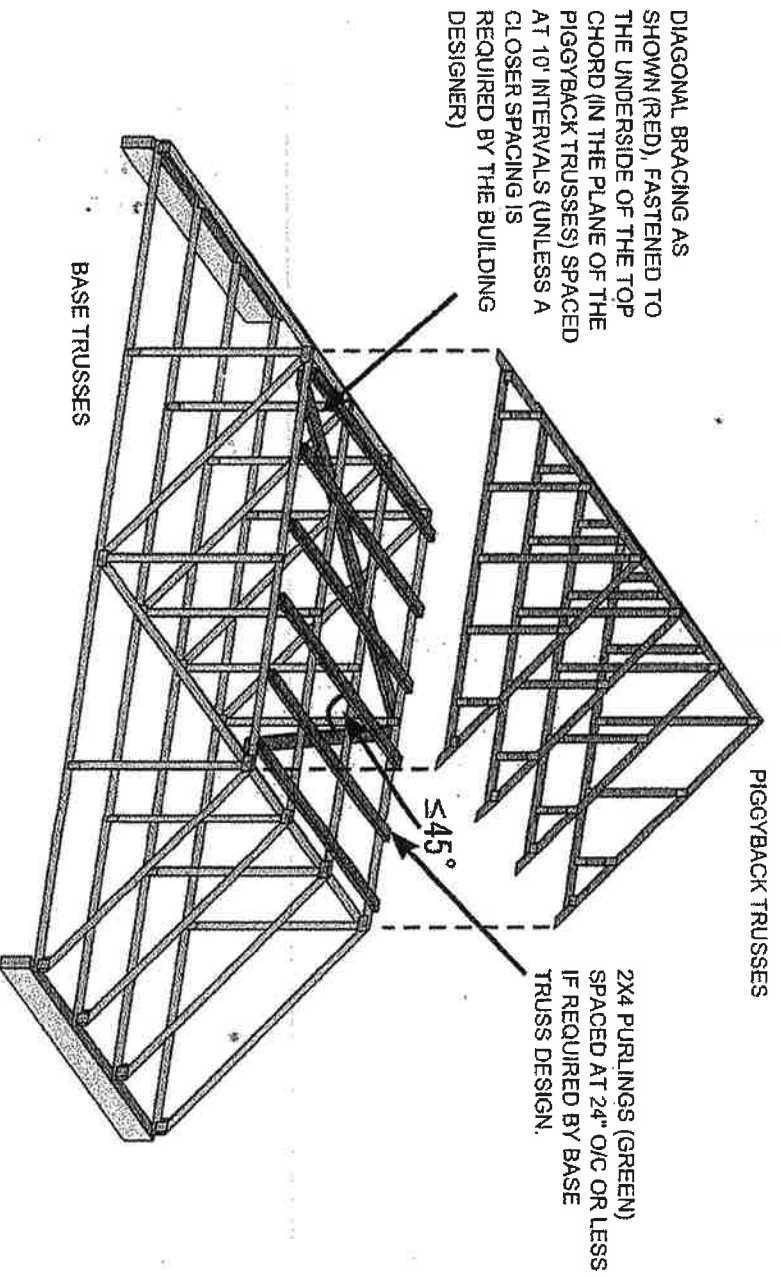
Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



SKETCH FROM BCSC-CANADA 2013

Disclaimer:

OwTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OwTFA technical committee and although there may be professional engineers involved in development, the information contained in the Tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OwTFA takes no responsibility with respect to the information provided but has developed this Tech-note to offer guidance where it is not currently readily available.

MICRO CITY ENGINEERING SERVICES INC.

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LUMBER SPECIFICATION

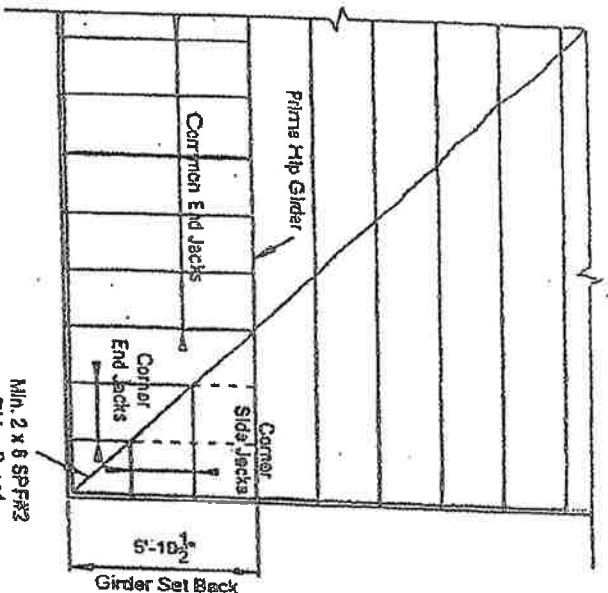
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD

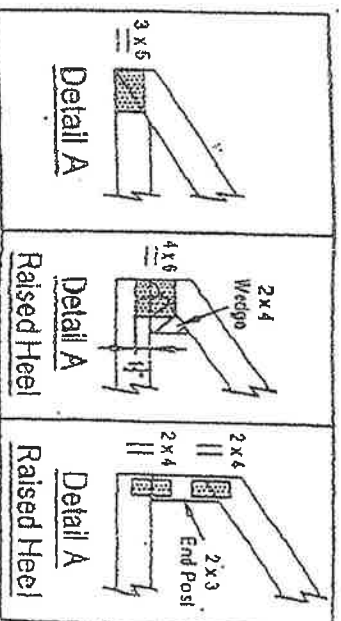
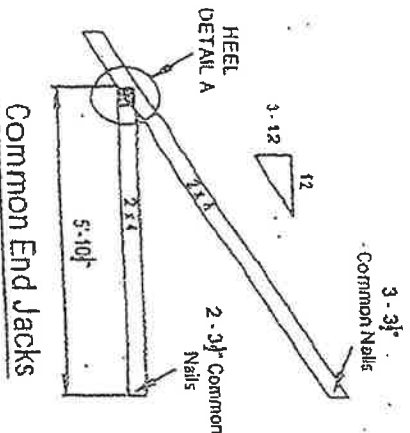
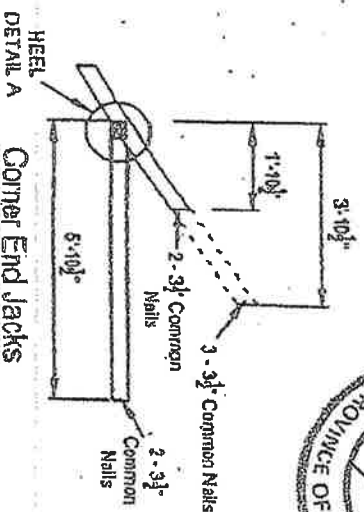
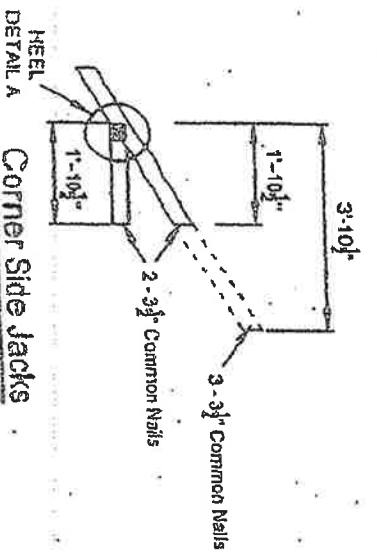
: 44.8 P.S.F.



45° Hip End

Min. 2 x 6 SPF#3
Ridge Board

DRS NO TAM 3-495.14
STRUCTURAL
COMPONENT ONLY



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012. (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICROCITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

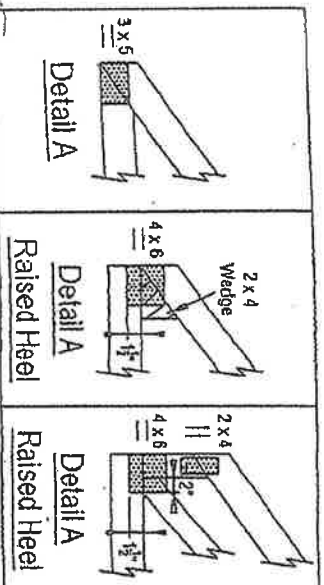
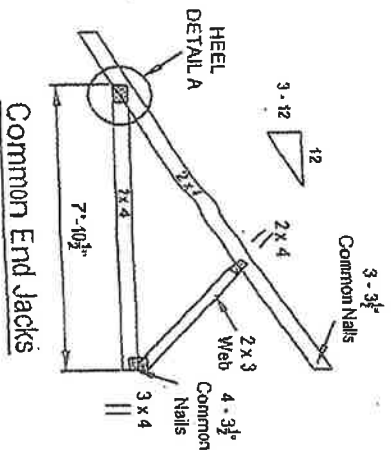
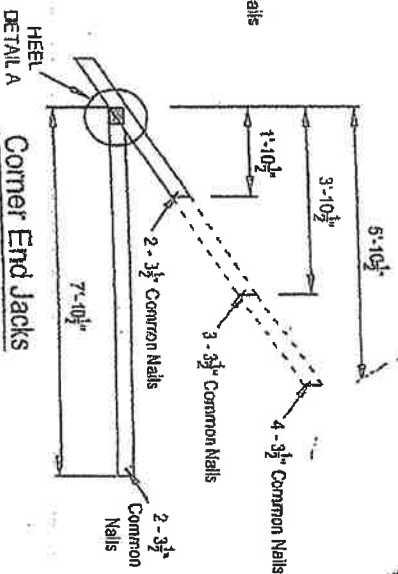
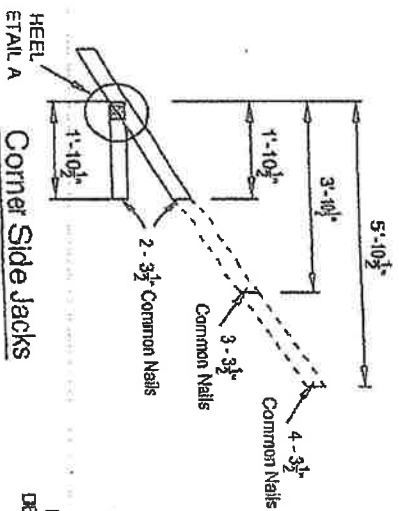
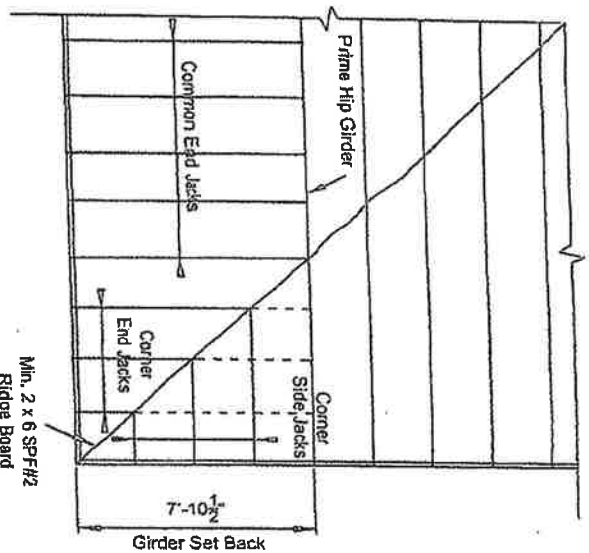
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

DRG NO TAM 3503.14
STRUCTURAL
COMPONENT ONLY



45° Hip End



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

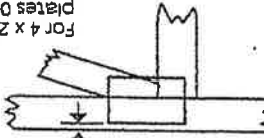
Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.



* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

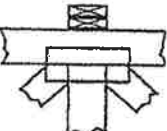
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

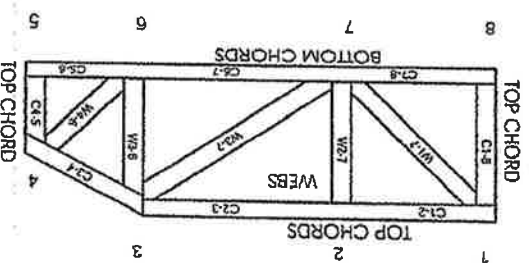


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
 TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
 DSB-89: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
 dimensions shown in ft-in-sixteenths or mm
 (Drawings shown in ft-in-sixteenths or mm)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:
 11996-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet MIT-7473C rev. 10-08

General Safety Notes
 Failure to Follow Could Cause Property Damage or Personal Injury

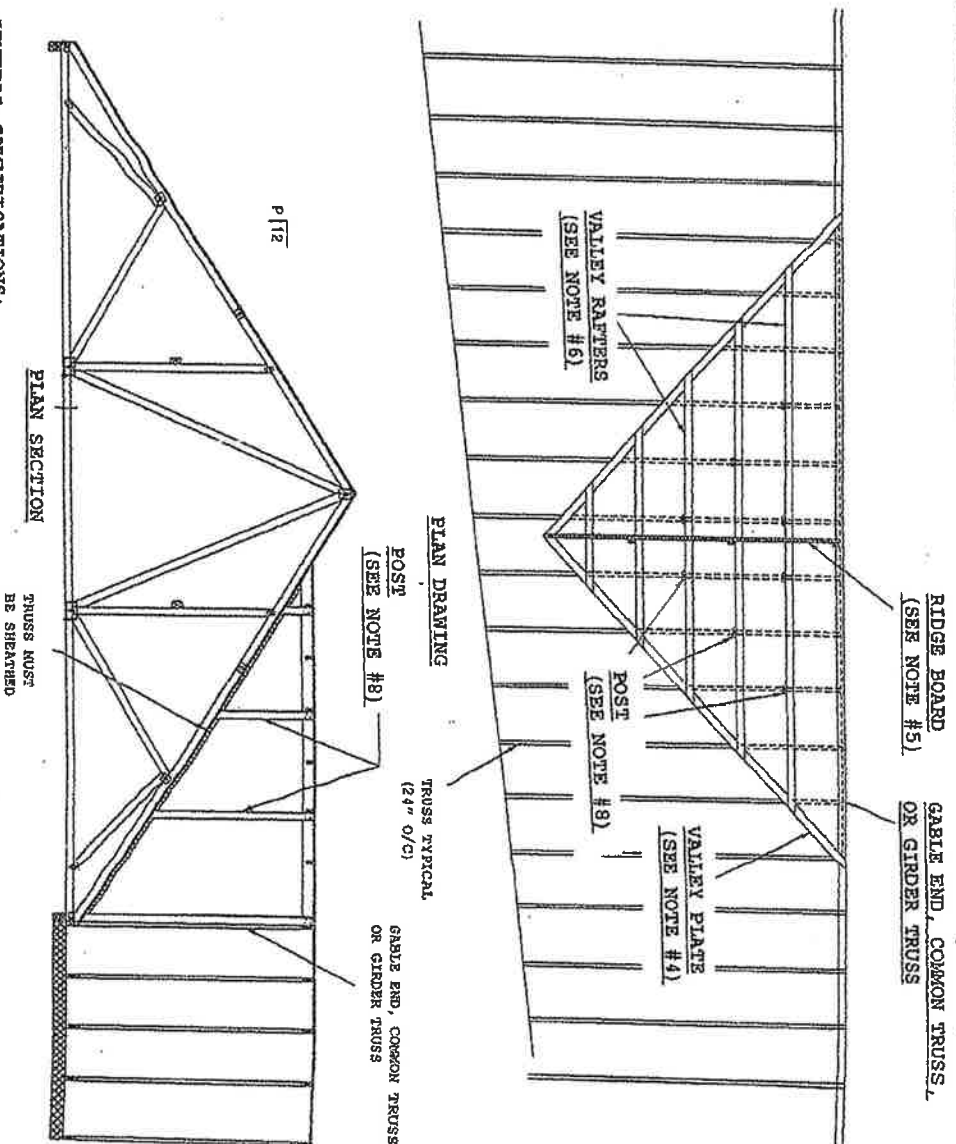
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing members may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BAIS TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 1.6d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 R2 RIDGE BOARD (MAX. 10'-0" RIDGES) OR 2 X 8 R2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 1.6d (3" X 0.131") NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 1.6d (3.5" X 0.131") ROE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 1.6d (3.5" X 0.131") ROE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 1.6d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 1.6d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 R2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 R2 SPF, OR BETTER, OR BE PRE-ASSIGNED TWO (2) PLY 2 X 4 R2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROES OF 1.6d (3" X 0.11") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (15) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (16) BASE TRUSS SPACING (24" O/C MAX.)
- (17) ALL PRE-ENGINEERED BASE TRUSS CONFORMERS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME.
- (18) WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (19) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (20) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSP Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.



