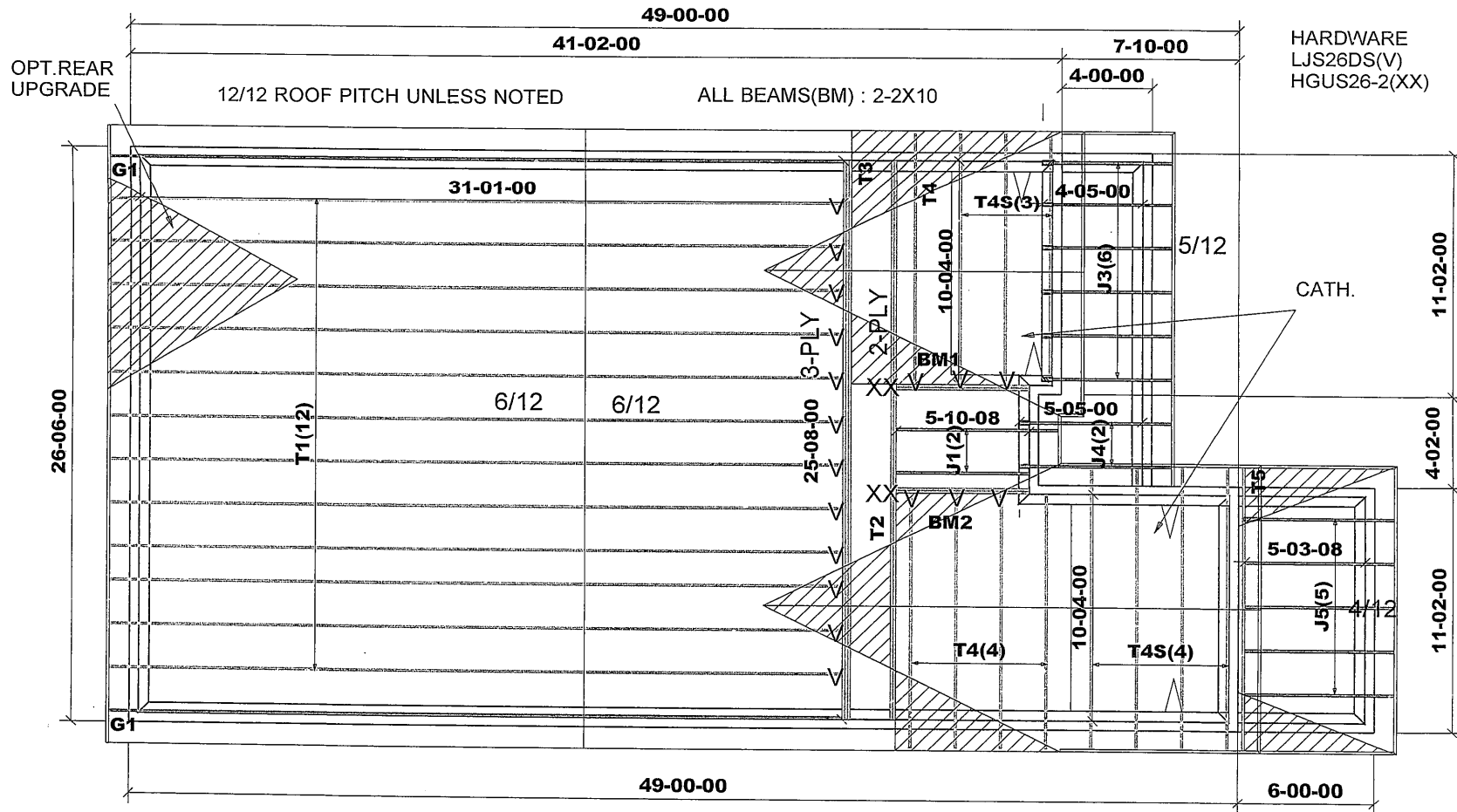


ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
S_s= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL FRAMING 



Town of Innisfil Certified Model
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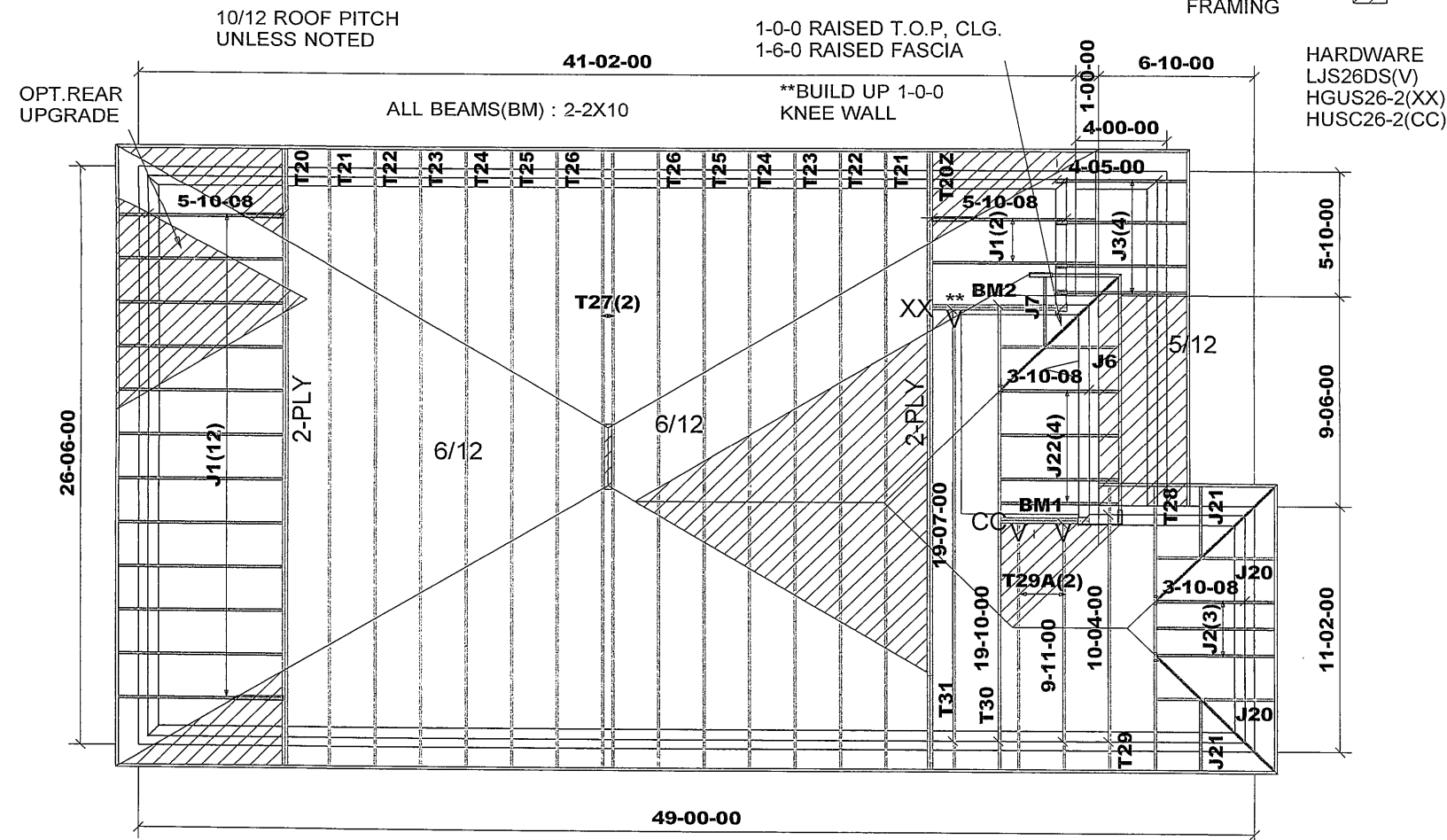
	Job Track: 42067 Layout ID: 272404 Plan Log: 87565	Builder / Location: BAYVIEW WELLINGTON / INNISFIL Project: ALCONA SHORES Date: 9/11/2017 Designer: JG	Model / Elevation: S32-7-15G / A THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Mitek ver 7.5.0
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ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.
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DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

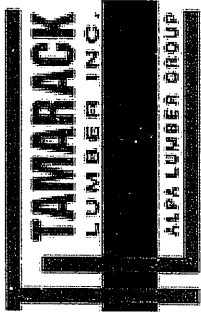
DENOTES
CONVENTIONAL
FRAMING



Town of Innisfil Certified Model
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Job Track: 42067 Layout ID: 272405 Plan Log: 87565	Builder / Location: BAYVIEW WELLINGTON / INNISFIL Project: ALCONA SHORES Date: 9/11/2017 Designer: JG	Model / Elevation: S32-7-15G / B THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
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Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK: 42067	LAYOUT ID: 272404	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S32-7-15G	ELEVATION: A	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	12	T1 COMMON	6.00 0.00	31-01-00	11-00-00	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 05-03-08	1706.52 1059.96		
	2	G1 COMMON	6.00 0.00	31-01-00	11-00-00	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 05-03-08	330.52 208.66		
	1 2 Ply	T2 FLAT GIRDER	0.00 0.00	25-08-00	04-01-04	2 X 6 2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	278.04 168.00		
	1 3 Ply	T3 FLAT GIRDER	0.00 0.00	25-08-00	05-01-04	2 X 6 2 X 6	00-00-00 00-00-00	05-01-04 05-01-04	451.77 275.49		
	5	T4 COMMON	12.00 0.00	10-04-00	07-00-08	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	249.55 159.15		
	7	T4S SCISSOR	12.00 6.00	10-04-00	07-00-08	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	345.45 217.00		
	1	T5 HIP GIRDER	10.00 0.00	10-04-00	02-01-02	2 X 4 2 X 4	01-03-08 01-03-08	01-00-07 01-00-07	42.12 28.33		
	2	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	33.58 21.34		
	6	J3 JACK-OPEN	5.00 0.00	04-05-00	02-03-07	2 X 4 2 X 4	01-03-08 00-00-00	00-05-06 02-03-07	78.12 52.02		
	2	J4 JACK-OPEN	5.00 0.00	05-05-00	02-08-07	2 X 4 2 X 4	01-03-08 00-00-00	00-05-06 02-08-07	30.80 20.00		
	5	J5 JACK-OPEN	4.00 0.00	05-03-08	02-01-02	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 02-01-02	70.30 46.65		

TOTAL # TRUSS= 47.00

TOTAL BFT OF ALL TRUSSES=

2256.60 BFT.

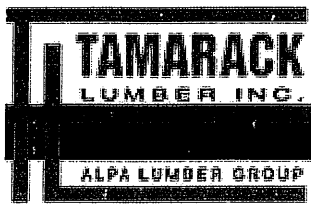
TOTAL WEIGHT OF ALL TRUSSES=

3616.77 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
18	Hangers	LJS26DS	

TOTAL # ITEMS= 20.00



Delivery Shiplist

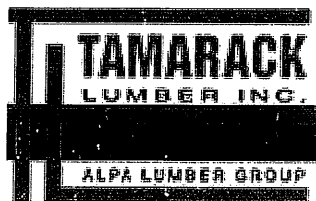
DATE	09/11/17
SALES REP	Mario

JOB TRACK:42067 LAYOUT ID: 272405 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-7-15G ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T20 HIP GIRDER	10.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.48		
	2 Ply		0.00							154.66		
	1	T20Z HIP GIRDER	10.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.48		
	2 Ply		0.00							154.66		
	2	T21 HIP	10.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58		
			0.00							139.34		
	2	T22 HIP	10.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14		
			0.00							144.00		
	2	T23 HIP	10.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76		
			0.00							145.66		
	2	T24 HIP	10.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52		
			0.00							155.34		
	2	T25 HIP	10.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90		
			0.00							160.00		
	2	T26 HIP	10.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08		
			0.00							163.34		
	2	T27 HIP	10.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86		
			0.00							175.34		
	1	T28 HIP GIRDER	10.00	10-04-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	50.44		
			0.00							32.83		
	1	T29 COMMON	10.00	10-04-00	05-11-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	46.60		
			0.00							30.00		
	2	T29A COMMON	10.00	09-11-00	05-11-06	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-11-14	87.46		
			0.00							56.00		
	1	T30 HIP GIRDER	10.00	19-10-00	06-04-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 02-01-11	109.73		
			0.00							71.00		
	1	T31 HIP	10.00	19-07-00	08-00-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-04-03	98.95		
			0.00							63.17		
	14	J1 JACK-OPEN	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06		
			0.00							149.38		
	3	J2 JACK-OPEN	10.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	50.49		
			0.00							32.01		
	4	J3 JACK-OPEN	5.00	04-05-00	02-03-07	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-03-07	52.08		
			0.00							34.68		
	1	J6 JACK-OPEN	10.00	03-10-08	03-07-09	2 X 4	2 X 4	01-03-08 -02-01-01	02-01-11 00-03-08	11.22		
			0.00							7.33		



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK:42067 LAYOUT ID: 272405 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-7-15G ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	J7 JACK-OPEN	10.00 0.00	01-10-08	03-07-09	2 X 4	2 X 4	01-03-08 -00-01-01	02-01-11 00-03-08	8.94 6.00		
	2	J20 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	27.46 18.66		
	2	J21 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	19.98 14.00		
	4	J22 JACK-OPEN	10.00 0.00	03-10-08	05-04-07	2 X 4	2 X 4	01-03-08 00-00-00	02-01-11 05-04-07	68.88 42.68		

TOTAL # TRUSS= 55.00

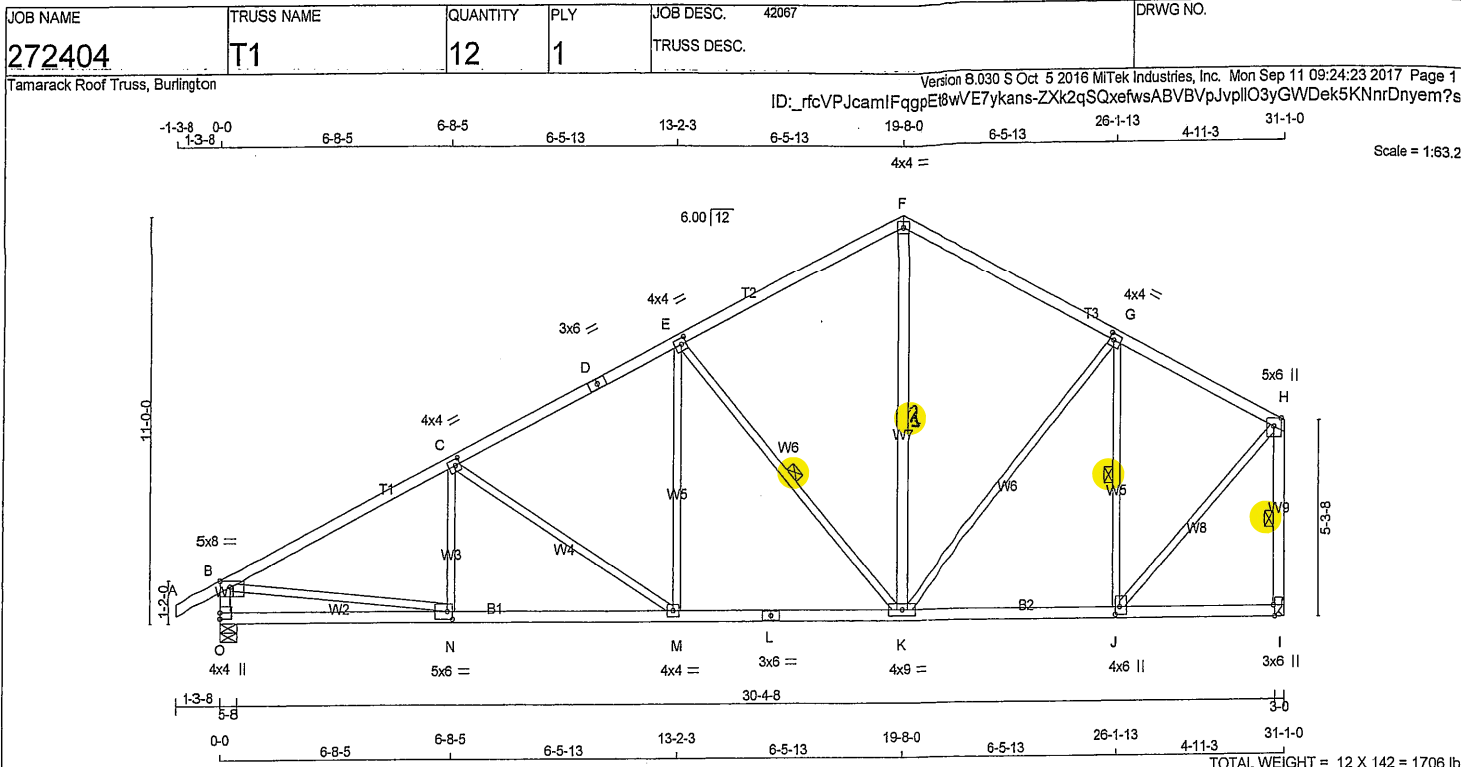
TOTAL BFT OF ALL TRUSSES=

1950.08 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3073.09 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
1	Hangers	HUSC26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 5.00



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
ALL WEBS EXCEPT K - F	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	TTW-p	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+t	MT20	3.0	6.0	Edge	0.50
J	BMVW-t	MT20	4.0	6.0	2.50	1.50
K	BMVWVW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	6.0	2.50	1.75
O	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT RRG	REQRD RRG
JT VERT 2500	DOWN 2500	5-8	5-8
O 2335	HORIZ 0	5-8	5-8
I 2335	UPLIFT 0	HANGER BY OTHERS	

MIN. SEAT SIZE: 3-0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
O 1938	1292 / 0	326 / 0
I 1826	1189 / 0	326 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K, G-J, H-I

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 37	-122.2	-122.2	0.16 (1)	10.00	N-C	-190 / 217	0.06 (1)	
B-C	-3388 / 0	-122.2	-122.2	0.96 (1)	2.79	C-M	-718 / 0	0.88 (1)	
C-D	-2749 / 0	-122.2	-122.2	0.73 (1)	3.44	M-E	0 / 567	0.13 (1)	
D-E	-2749 / 0	-122.2	-122.2	0.73 (1)	3.44	E-K	-1327 / 0	0.93 (1)	
E-F	-1809 / 0	-122.2	-122.2	0.68 (1)	4.10	K-F	0 / 951	0.15 (1)	
F-G	-1809 / 0	-122.2	-122.2	0.68 (1)	4.10	K-G	0 / 315	0.07 (1)	
G-H	-1497 / 0	-122.2	-122.2	0.57 (1)	4.60	J-G	-1221 / 0	0.46 (1)	
O-B	-2421 / 0	0.0	0.0	0.24 (1)	5.50	B-N	0 / 3086	0.69 (1)	
I-H	-2282 / 0	0.0	0.0	0.35 (1)	4.52	J-H	0 / 1982	0.44 (1)	
O-N	0 / 0	-28.0	-28.0	0.29 (3)	10.00				
N-M	0 / 3061	-28.0	-28.0	0.63 (1)	10.00				
M-L	0 / 2458	-28.0	-28.0	0.53 (1)	10.00				
L-K	0 / 2458	-28.0	-28.0	0.53 (1)	10.00				
K-J	0 / 1380	-28.0	-28.0	0.38 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/999 (0.28")

CSI: TC=0.96 (B-C:1), BC=0.63 (M-N:1), WB=0.93 (E-K:1), SSI=0.34 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

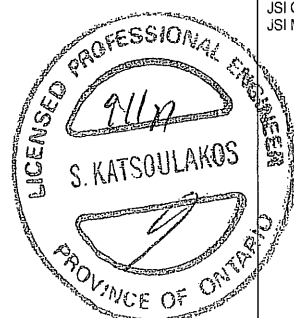
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.76 (N) (INPUT = 1.00)



DRWG NO. TAM 45849-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272404	G1	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. MEMB. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
U-T	0/2	-28.0 -28.0	0.02 (3)	10.00			

91112

S. KATSOULAKOS

PROVINCE OF ONTARIO

prb

DWG NO. TAN 45848 17

STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272404	T2	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 09:24:24 2017 Page 2

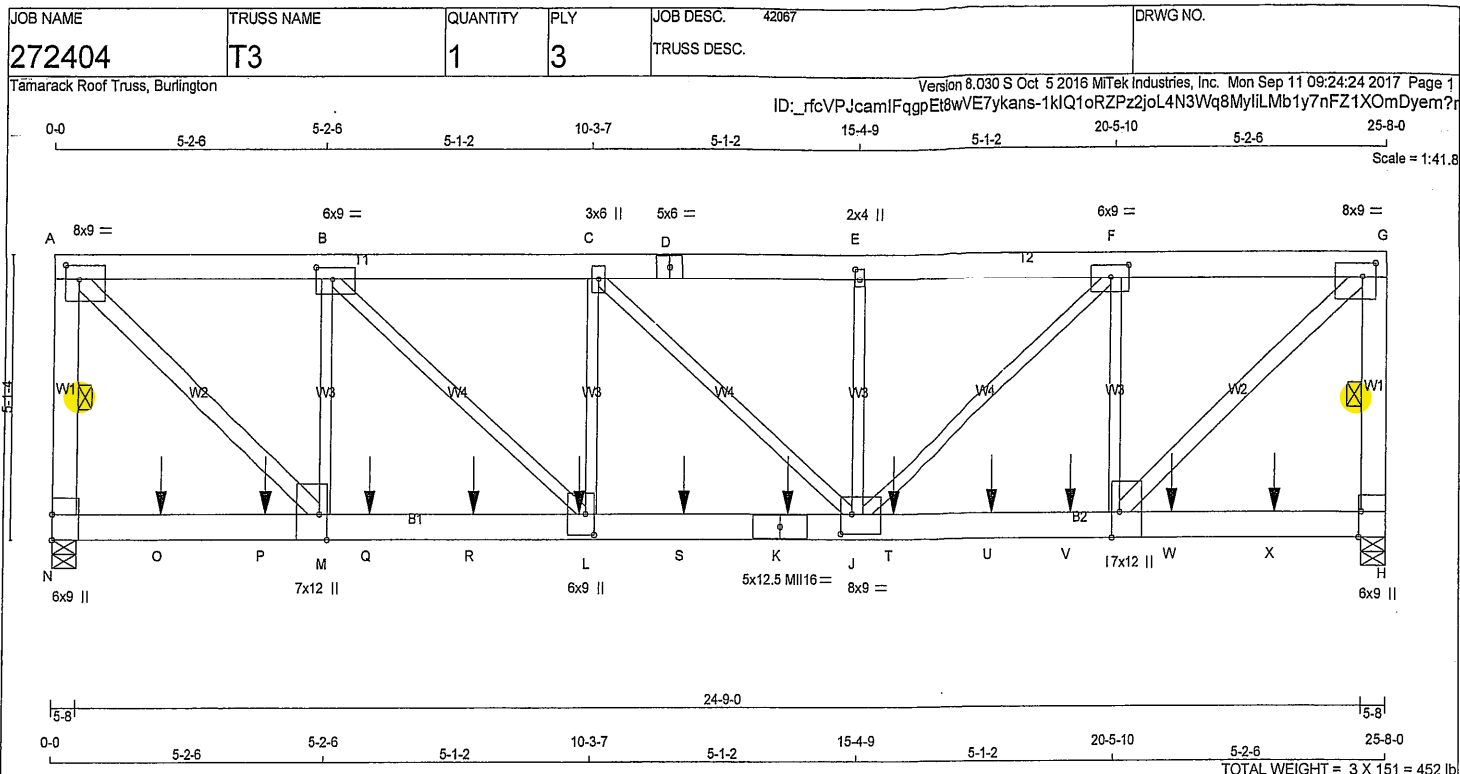
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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 147.1 lbs FACTORED DOWN AT 12-4-12, AND 147.1 lbs FACTORED DOWN AT 14-4-12 ON TOP CHORD, AND 1832.4 lbs FACTORED DOWN AT 10-5-8, 69.9 lbs FACTORED DOWN AT 12-4-12, AND 69.9 lbs FACTORED DOWN AT 14-4-12, AND 1886.7 lbs FACTORED DOWN AT 15-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAN 45849-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
N - A	2x6	DRY	No.2
A - D	2x6	DRY	2100F 1.8E
D - G	2x6	DRY	2100F 1.8E
H - G	2x6	DRY	No.2
N - K	2x6	DRY	2100F 1.8E
K - H	2x6	DRY	2100F 1.8E

LUMBER			
CHORDS	SIZE	DESCR.	SPF
N - A	2x6	DRY	No.2
A - D	2x6	DRY	2100F 1.8E
D - G	2x6	DRY	2100F 1.8E
H - G	2x6	DRY	No.2
N - K	2x6	DRY	2100F 1.8E
K - H	2x6	DRY	2100F 1.8E

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT
A - M 2x4 DRY 2100F 1.8E SPF
I - G 2x4 DRY 2100F 1.8E SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
N-A 2	12	TOP
A-D 2	12	TOP
D-G 2	12	TOP
G-H 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N-K 2	5	SIDE(1166.2)
K-H 2	5	SIDE(1203.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
A TMVW-t	MT20	8.0	9.0 3.25 3.00
B TMVW-t	MT20	6.0	9.0 2.75 3.75
C TMVW-t	MT20	3.0	6.0
D TS-t	MT20	5.0	6.0
E TMVW-t	MT20	2.0	4.0 2.50 1.00
F TMVW-t	MT20	6.0	9.0 2.75 4.25
G TMVW-t	MT20	8.0	9.0 3.00 3.00
H BMV1-t	MT20	6.0	9.0 Edge 0.50
I BMVW-t	MT20	7.0	12.0 Edge 1.75
J BMVW-t	MT20	8.0	9.0 4.25 2.50
K BS-t	MT16	5.0	12.5
L BMVW-t	MT20	6.0	9.0 4.50 2.00
M BMVW-t	MT20	7.0	12.0 Edge 1.75
N BMV1-t	MT20	6.0	9.0 5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REGRD
N	15637 0	15637 0	5-8
H	15897 0	15897 0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	12151	8045 / 0	2084 / 0	0 / 0	0 / 0	2021 / 0	0 / 0
H	12353	8180 / 0	2119 / 0	0 / 0	0 / 0	2054 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) N, H (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-N, G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
N-A	-14265 / 0	0.0 0.0 0.41 (1)	3.97
A-B	-15647 / 0	-122.2 -122.2 0.10 (1)	4.65
B-C	-22846 / 0	-122.2 -122.2 0.17 (1)	3.93
C-D	-23010 / 0	-122.2 -122.2 0.19 (1)	3.91
D-E	-23010 / 0	-122.2 -122.2 0.19 (1)	3.91
E-F	-23010 / 0	-122.2 -122.2 0.17 (1)	3.92
F-G	-15992 / 0	-122.2 -122.2 0.10 (1)	4.61
G-H	-14595 / 0	0.0 0.0 0.41 (1)	3.93
WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-M	0 / 20990	0.52 (1)	
M-B	-7272 / 0	0.81 (1)	
B-L	0 / 9745	0.73 (1)	
L-C	-726 / 0	0.08 (1)	
C-J	0 / 223	0.02 (1)	
J-E	-564 / 0	0.06 (1)	
E-F	0 / 9501	0.71 (1)	
F-I	-7107 / 0	0.79 (1)	
I-G	0 / 21453	0.54 (1)	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	14-0-12	-2307	-2307	---	FRONT	VERT	TOTAL
L	10-0-12	-2307	-2307	---	FRONT	VERT	TOTAL
O	2-0-12	-2307	-2307	---	FRONT	VERT	TOTAL
P	4-0-12	-2307	-2307	---	FRONT	VERT	TOTAL
Q	6-0-12	-2307	-2307	---	FRONT	VERT	TOTAL
R	8-0-12	-2307	-2307	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/734 (0.42")

CSI: TC=0.41*(G-H:1), BC=0.67 (I-J:1), WB=0.81 (B-M:1), SS=0.75 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656
MT16 473 276 2341 1245 4454 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (A) (INPUT = 0.90)
SI METAL= 0.99 (I) (INPUT = 1.00)



DWG NO. TAW 45890-17
STRUCTURAL
COMPONENT ONLY
CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2306.5 lbs FACTORED DOWN AT 2-0-12, 2306.5 lbs FACTORED DOWN AT 4-0-12, 2306.5 lbs FACTORED DOWN AT 6-0-12, 2306.5 lbs FACTORED DOWN AT 8-0-12, 2306.5 lbs FACTORED DOWN AT 10-0-12, 2306.5 lbs FACTORED DOWN AT 12-0-12, 2306.5 lbs FACTORED DOWN AT 14-0-12, 2306.5 lbs FACTORED DOWN AT 16-0-12, 2306.5 lbs FACTORED DOWN AT 18-0-12, 2306.5 lbs FACTORED DOWN AT 19-7-4, AND 2306.5 lbs FACTORED DOWN AT 21-7-4, AND 2306.5 lbs FACTORED DOWN AT 23-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
S	12-0-12	-2307	-2307	—	FRONT	VERT
T	16-0-12	-2307	-2307	—	FRONT	VERT
U	18-0-12	-2307	-2307	—	FRONT	VERT
V	19-7-4	-2307	-2307	—	FRONT	VERT
W	21-7-4	-2307	-2307	—	FRONT	VERT
X	23-7-4	-2307	-2307	—	FRONT	VERT
						TYPE
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL

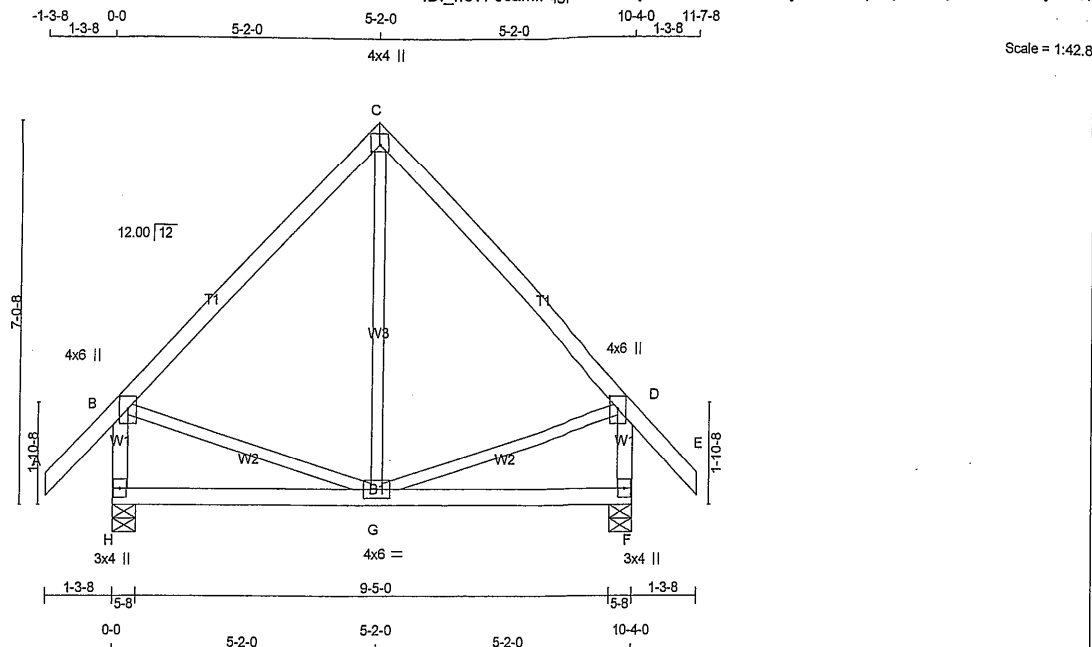


DWG NO. TAM 45850-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 272404	TRUSS NAME T4	QUANTITY 5	PLY 1	JOB DESC. 42067 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 5 X 50 = 250 lb
[M][F]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ
H	947	0	947	0	0
F	947	0	947	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
H	722	502 / 0	109 / 0	0 / 0	0 / 0	112 / 0	0 / 0	
F	722	502 / 0	109 / 0	0 / 0	0 / 0	112 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 60	G-C	-6 / 235
B-C	-442 / 0	B-G	0 / 326
C-D	-442 / 0	G-D	0 / 326
D-E	0 / 60		
H-B	-891 / 0		
F-D	-891 / 0		
H-G	0 / 0		
G-F	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.42 (B-C:1), BC=0.22 (F-G:3), WB=0.07 (B-G:1), SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

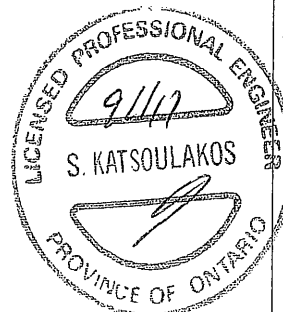
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (C) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

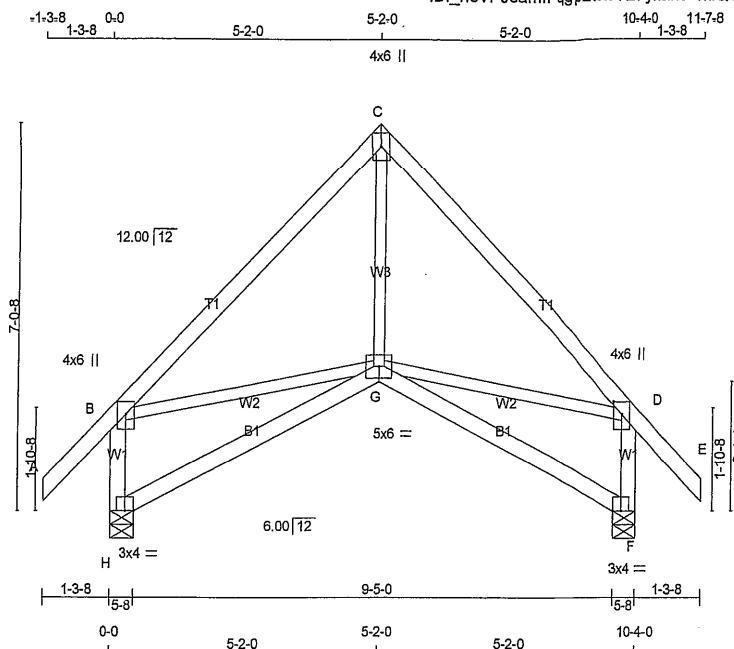


DWG NO. T4H 45858-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 272404	TRUSS NAME T4S	QUANTITY 7	PLY 1	JOB DESC. 42087	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:42.4

TOTAL WEIGHT = 7 X 49 = 345 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT.	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	4.0	6.0		
D	IMVW+p	MT20	4.0	6.0	2.75	2.00
F	BVM1-p	MT20	3.0	4.0		
G	BBWWW-p	MT20	5.0	6.0		
H	BVM1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	947	0	947	0
F	947	0	947	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	722	502 / 0	109 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	722	502 / 0	109 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LCI)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LCI)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	G-C	0 / 349
B-C	-657 / 0	-122.2	-122.2	0.43 (1)	6.25	B-G	0 / 471
C-D	-657 / 0	-122.2	-122.2	0.43 (1)	6.25	G-D	0 / 471
D-E	0 / 60	-122.2	-122.2	0.17 (1)	10.00		
H-B	-874 / 0	0.0	0.0	0.10 (1)	7.81		
F-D	-874 / 0	0.0	0.0	0.10 (1)	7.81		
H-G	0 / 0	-28.0	-28.0	0.22 (3)	10.00		
G-F	0 / 0	-28.0	-28.0	0.22 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.43 (B-C:1), BC=0.22 (G-H:3), WB=0.11 (D-G:1), SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

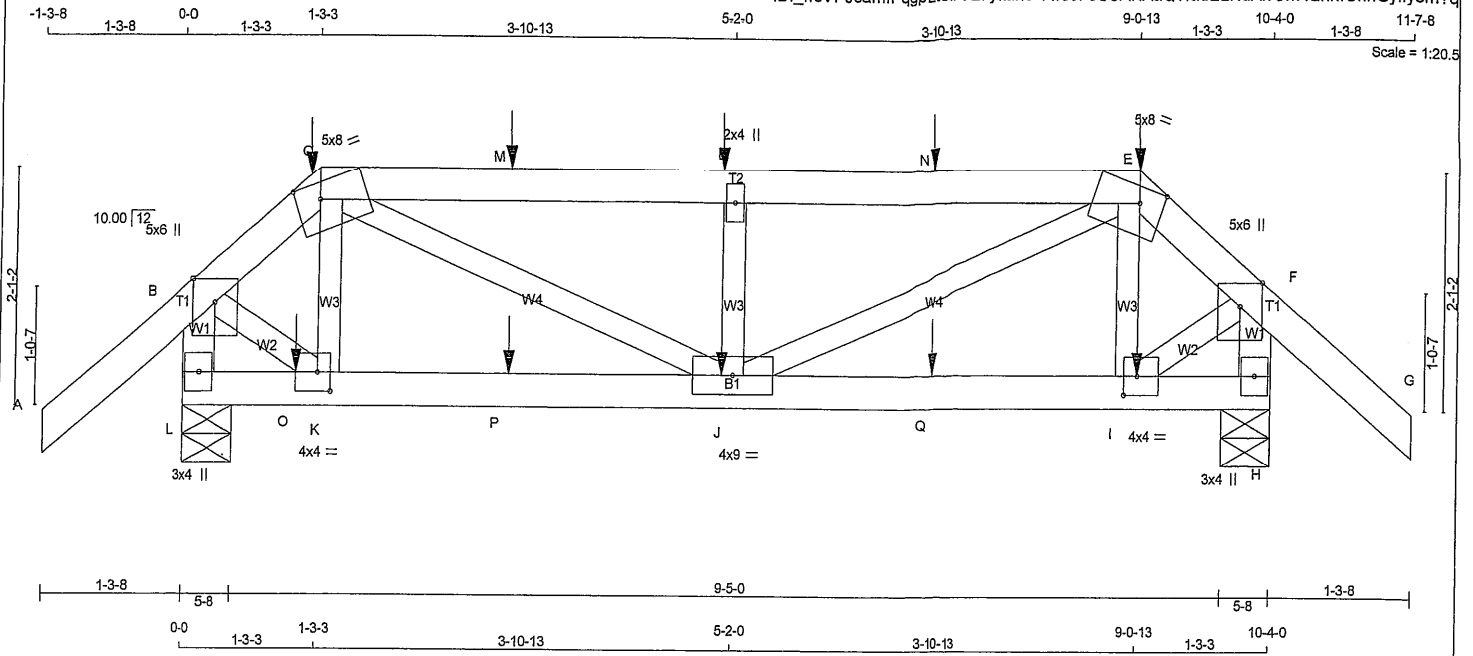
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (G) (INPUT = 0.90)
JSI METAL= 0.38 (H) (INPUT = 1.00)



DRWG NO. TAW45852-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 42 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	1.75	2.75
D	TMVW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	1.75	2.75
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0	2.00	1.50
J	BMVW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 84.4 lbs FACTORED DOWN AT 1-3-3, 84.4 lbs FACTORED DOWN AT 9-0-13, 163.8 lbs FACTORED DOWN AT 1-3-3, 160.8 lbs FACTORED DOWN AT 3-0-12, 160.8 lbs FACTORED DOWN AT 5-0-12, AND 160.8 lbs FACTORED DOWN AT 7-0-12, AND 164.5 lbs FACTORED DOWN AT 9-0-13 ON TOP CHORD, AND 86.4 lbs FACTORED DOWN AT 1-0-12, 86.4 lbs FACTORED DOWN AT 3-0-12, 86.4 lbs FACTORED DOWN AT 5-0-12, AND 86.4 lbs FACTORED DOWN AT 7-0-12, AND 86.4 lbs FACTORED DOWN AT 9-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
L	1658	0	1658	0
H	1644	0	1644	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
L	1279	862 / 0 208 / 0 0 / 0 0 / 0 209 / 0 0 / 0
H	1267	855 / 0 205 / 0 0 / 0 0 / 0 207 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

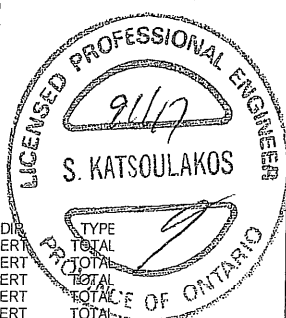
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)	CS1 (LC)
FR-TO				FR-TO			
A-B	0 / 54	-122.2	-122.2 0.19 (1)	K-C	-237 / 56	0.04 (1)	
B-C	-1353 / 0	-122.2	-122.2 0.20 (1)	C-J	0 / 1394	0.35 (1)	
C-M	-2240 / 0	-122.2	-122.2 0.54 (1)	J-D	-918 / 0	0.15 (1)	
M-D	-2240 / 0	-122.2	-122.2 0.54 (1)	J-E	0 / 1393	0.34 (1)	
D-N	-2240 / 0	-122.2	-122.2 0.54 (1)	I-E	-228 / 61	0.04 (1)	
N-E	-2240 / 0	-122.2	-122.2 0.54 (1)	B-K	0 / 1107	0.27 (1)	
E-F	-1354 / 0	-122.2	-122.2 0.20 (1)	I-F	0 / 1107	0.27 (1)	
F-G	0 / 54	-122.2	-122.2 0.19 (1)				
L-B	-1681 / 0	0.0	0.0 0.19 (1)				
H-F	-1682 / 0	0.0	0.0 0.19 (1)				
L-O	0 / 0	-28.0	-28.0 0.13 (2)				
O-K	0 / 0	-28.0	-28.0 0.13 (2)				
K-P	0 / 971	-28.0	-28.0 0.30 (1)				
P-J	0 / 971	-28.0	-28.0 0.30 (1)				
J-Q	0 / 972	-28.0	-28.0 0.30 (1)				
Q-I	0 / 972	-28.0	-28.0 0.30 (1)				
I-H	0 / 0	-28.0	-28.0 0.12 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	1-3-3	-84	-84		FRONT	VERT	TOTAL
C	1-3-3	-164	-164		BACK	VERT	TOTAL
D	5-0-12	-161	-161		BACK	VERT	TOTAL
E	9-0-13	-84	-84		FRONT	VERT	TOTAL
E	9-0-13	-165	-165		BACK	VERT	TOTAL
I	9-0-12	-86	-86		BACK	VERT	TOTAL
J	5-0-12	-86	-86		BACK	VERT	TOTAL
M	3-0-12	-161	-161		BACK	VERT	TOTAL
N	7-0-12	-161	-161		BACK	VERT	TOTAL
O	1-0-12	-86	-86		BACK	VERT	TOTAL
P	3-0-12	-86	-86		BACK	VERT	TOTAL
Q	7-0-12	-86	-86		BACK	VERT	TOTAL



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CS1: TC=0.54 (C-D:1), BC=0.30 (I-J:1), WB=0.35 (C-J:1), SS=0.34 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

DRWG NO. TAM 45863-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272404	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.36 (K) (INPUT = 1.00)



DWG NO. TAM 45869-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272405	T20	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington						
Version 8.030 S Oct 5 2016 MITek Industries, Inc. Mon Sep 11 09:28:04 2017 Page 2						
ID:CbcXd1J AWrxHhsIjTMM00yKMTA-b2bbC16ZbZmVB9vgynknGMPDc2mVVkMd8Gp1UTyelyP						

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 227.6 lbs FACTORED DOWN AT 22-8-8, 148.2 lbs FACTORED DOWN AT 3-11-4, 147.1 lbs FACTORED DOWN AT 5-11-4, 147.1 lbs FACTORED DOWN AT 7-11-4, 147.1 lbs FACTORED DOWN AT 9-11-4, 147.1 lbs FACTORED DOWN AT 11-11-4, 147.1 lbs FACTORED DOWN AT 13-11-4, 147.1 lbs FACTORED DOWN AT 15-11-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-228	-228	—	FRONT	VERT	TOTAL
D	5-11-4	-147	-147	—	FRONT	VERT	TOTAL
G	19-11-4	-147	-147	—	FRONT	VERT	TOTAL
H	22-8-8	-228	-228	—	FRONT	VERT	TOTAL
M	19-11-4	-40	-70	—	FRONT	VERT	TOTAL
N	15-11-4	-40	-70	—	FRONT	VERT	TOTAL
P	5-11-4	-40	-70	—	FRONT	VERT	TOTAL
S	3-11-4	-148	-148	—	FRONT	VERT	TOTAL
T	7-11-4	-147	-147	—	FRONT	VERT	TOTAL
U	9-11-4	-147	-147	—	FRONT	VERT	TOTAL
V	11-11-4	-147	-147	—	FRONT	VERT	TOTAL
W	13-11-4	-147	-147	—	FRONT	VERT	TOTAL
X	15-11-4	-147	-147	—	FRONT	VERT	TOTAL
Y	17-11-4	-147	-147	—	FRONT	VERT	TOTAL
Z	21-11-4	-161	-161	—	FRONT	VERT	TOTAL
AA	1-11-4	-40	-70	—	FRONT	VERT	TOTAL
AB	3-11-4	-40	-70	—	FRONT	VERT	TOTAL
AC	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
AD	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
AE	11-11-4	-40	-70	—	FRONT	VERT	TOTAL
AF	13-11-4	-40	-70	—	FRONT	VERT	TOTAL
AG	17-11-4	-40	-70	—	FRONT	VERT	TOTAL
AH	21-11-4	-40	-70	—	FRONT	VERT	TOTAL
AI	23-11-4	-40	-70	—	FRONT	VERT	TOTAL



DWG NO. TAM 45852-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272405	T20Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

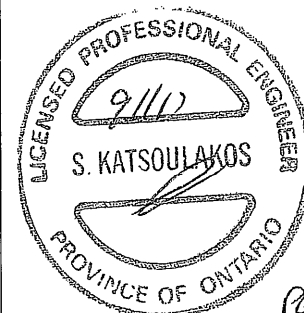
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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8,
AND 148.2 lbs FACTORED DOWN AT 3-11-4 ON
TOP CHORD, AND 69.9 lbs FACTORED DOWN
AT 1-11-4, AND 69.9 lbs FACTORED DOWN AT
3-11-4, AND 2928.2 lbs FACTORED DOWN AT
5-11-8 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.



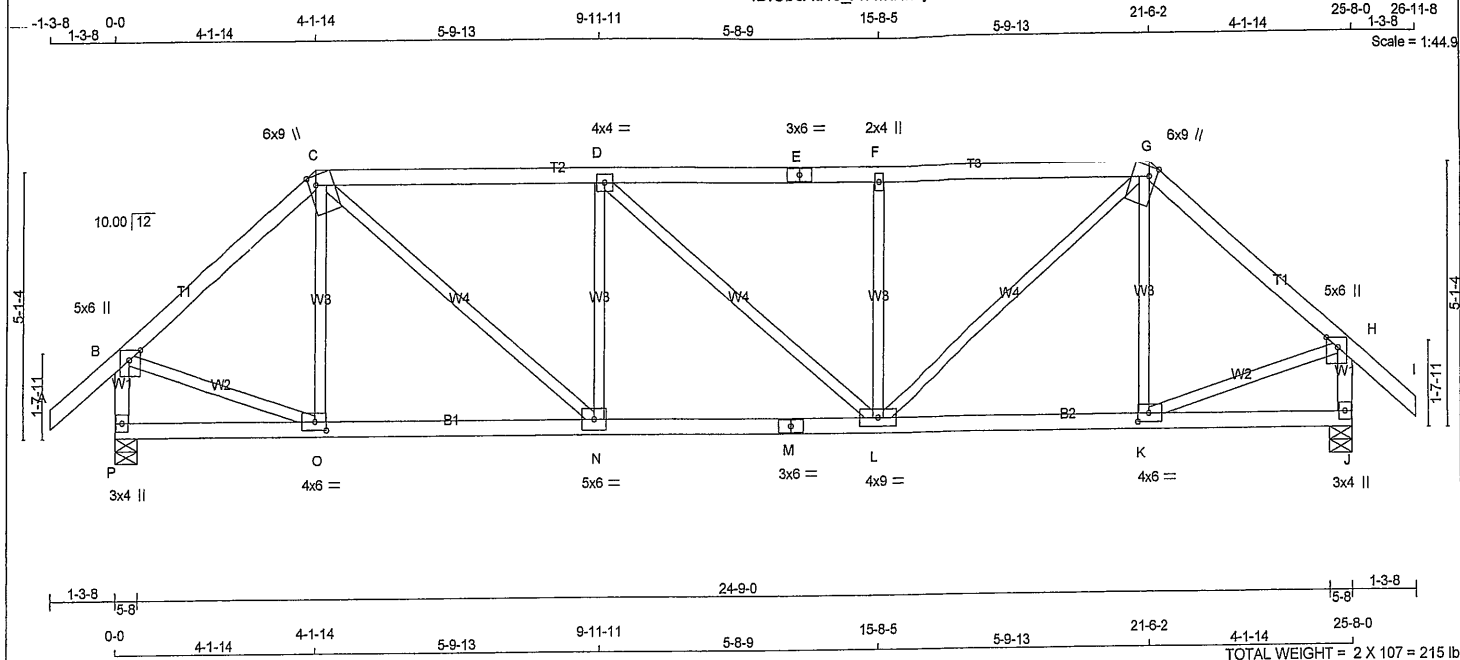
DWG NO. YAW 45865.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272405	T21	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
P - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
P	2097	0	2097	0	0	5-8	5-8	5-8	5-8
J	2097	0	2097	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
P	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0				
J	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)		CHORDS		MEMB.		FACTORED		W E B S		MAX. FACTORED	
FR-TO	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX.	MEMB.	FORCE (LBS)	MAX	MAX.	MEMB.	FORCE (LBS)	MAX
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	0-C	-279 / 102	0.11 (1)					
B-C	-1859 / 0	-122.2	-122.2	0.45 (1)	4.42	C-N	0 / 1450	0.33 (1)					
C-D	-2538 / 0	-122.2	-122.2	0.75 (1)	3.48	N-D	-762 / 0	0.29 (1)					
D-E	-2536 / 0	-122.2	-122.2	0.74 (1)	3.48	D-L	-2 / 0	0.00 (1)					
E-F	-2536 / 0	-122.2	-122.2	0.74 (1)	3.48	L-F	-761 / 0	0.29 (1)					
F-G	-2536 / 0	-122.2	-122.2	0.74 (1)	3.48	L-G	0 / 1448	0.33 (1)					
G-H	-1859 / 0	-122.2	-122.2	0.45 (1)	4.42	K-G	-278 / 102	0.11 (1)					
H-I	0 / 54	-122.2	-122.2	0.17 (1)	10.00	B-O	0 / 1489	0.34 (1)					
P-B	-2054 / 0	0.0	0.0	0.22 (1)	5.90	K-H	0 / 1490	0.34 (1)					
J-H	-2054 / 0	0.0	0.0	0.22 (1)	5.90								
P-O	0 / 0	-28.0	-28.0	0.17 (2)	10.00								
O-N	0 / 1418	-28.0	-28.0	0.32 (1)	10.00								
N-M	0 / 2538	-28.0	-28.0	0.50 (1)	10.00								
M-L	0 / 2538	-28.0	-28.0	0.50 (1)	10.00								
L-K	0 / 1418	-28.0	-28.0	0.32 (1)	10.00								
K-J	0 / 0	-28.0	-28.0	0.17 (2)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(5% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL)= L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL)= L/999 (0.20")

CSI: TC=0.75 (C-D:1), BC=0.50 (L-N:1), WB=0.34 (H-K:1), SSI=0.33 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.71 (M) (INPUT = 1.00)

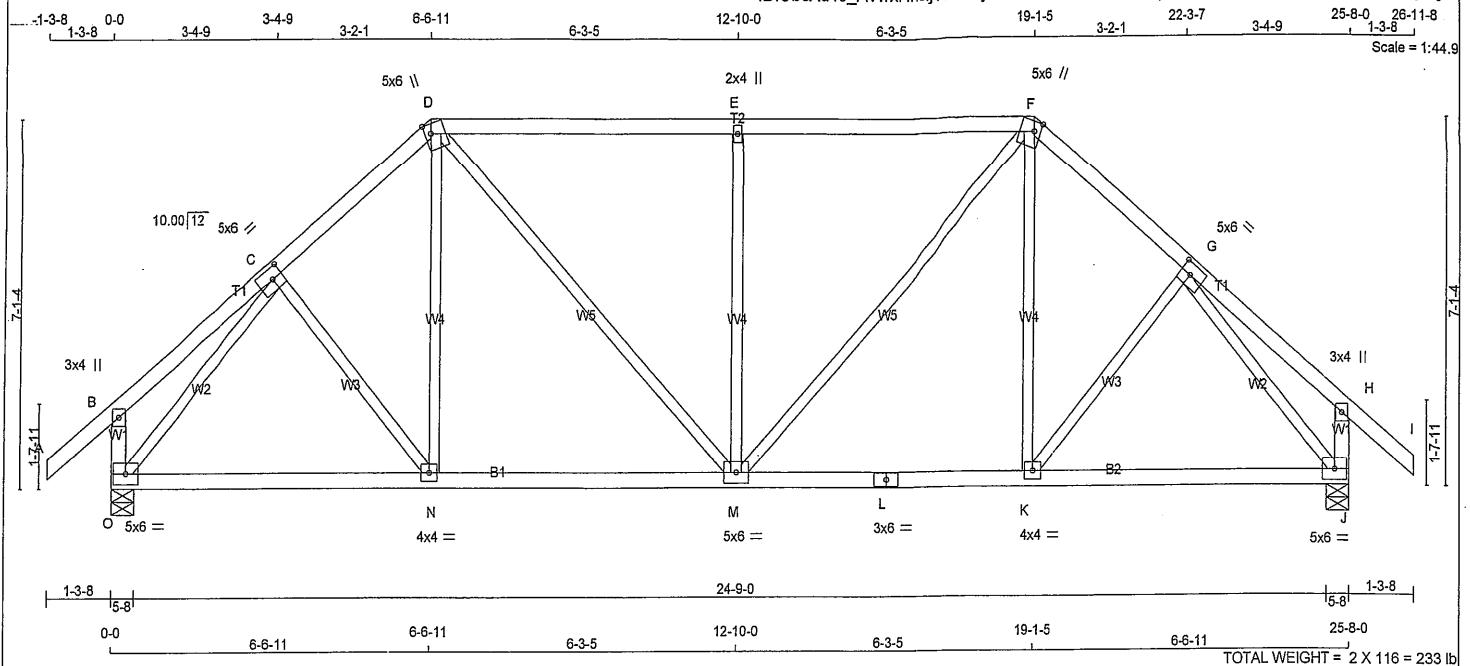


DWG NO. YAM 4586-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
272405	T23	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	BMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
O	2097	0	2097	0	5-8
J	2097	0	2097	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX /MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE		
O	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0
J	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 54	-122.2 -122.2	0.17 (1) 10.00
B-C	0 / 27	-122.2 -122.2	0.19 (1) 10.00
C-D	-1859 / 0	-122.2 -122.2	0.19 (1) 4.77
D-E	-1910 / 0	-122.2 -122.2	0.68 (1) 3.96
E-F	-1910 / 0	-122.2 -122.2	0.68 (1) 3.96
F-G	-1859 / 0	-122.2 -122.2	0.19 (1) 4.77
G-H	0 / 27	-122.2 -122.2	0.19 (1) 10.00
H-I	0 / 54	-122.2 -122.2	0.17 (1) 10.00
O-B	-325 / 0	0.0 0.0	0.03 (1) 7.81
J-H	-325 / 0	0.0 0.0	0.03 (1) 7.81
O-N	0 / 1374	-28.0 -28.0	0.43 (2) 10.00
N-M	0 / 1405	-28.0 -28.0	0.44 (2) 10.00
M-L	0 / 1405	-28.0 -28.0	0.44 (2) 10.00
L-K	0 / 1405	-28.0 -28.0	0.44 (2) 10.00
K-J	0 / 1374	-28.0 -28.0	0.43 (2) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.68 (D-E:1), BC=0.44 (M-N:2), WB=0.94 (G-J:1), SSI=0.37 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

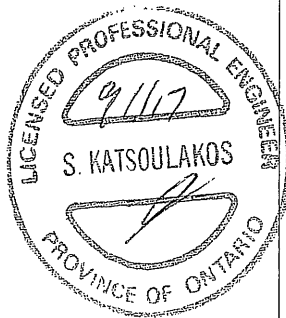
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN
MT20 618 354	1667 822
	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

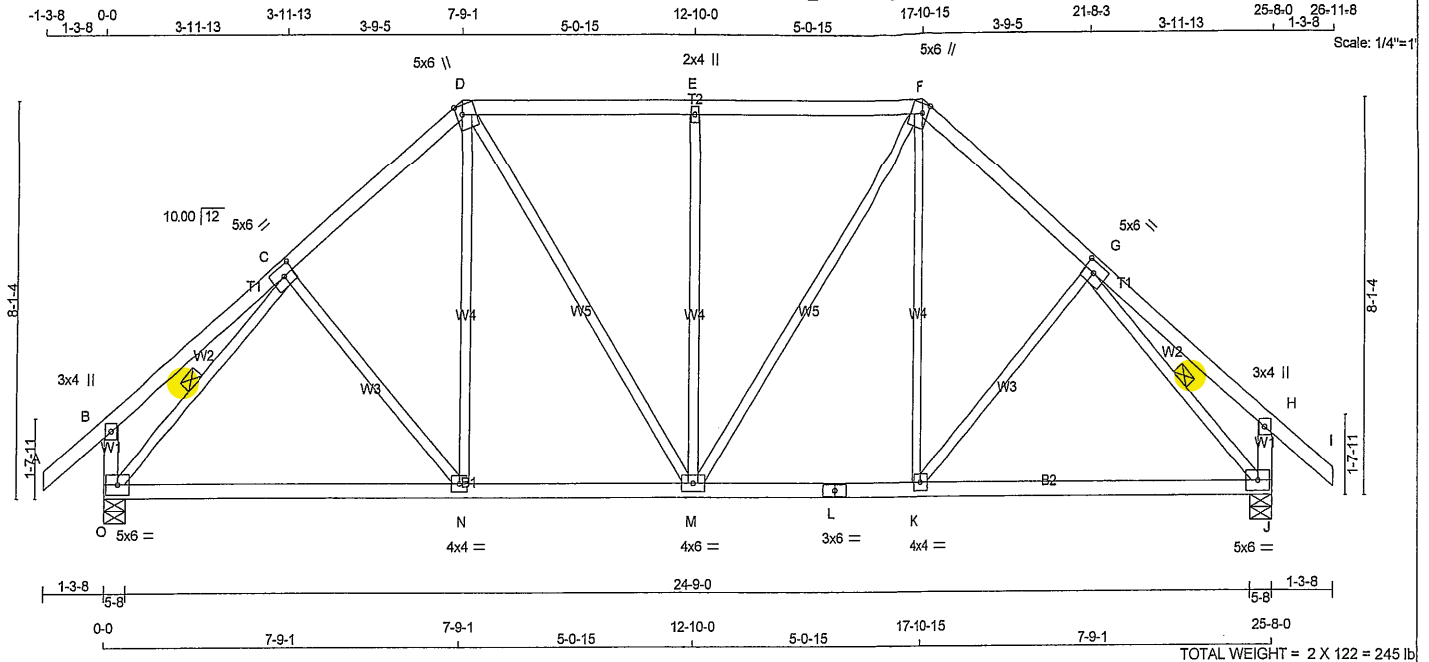


DWG NO. TAM 45858-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 272405	TRUSS NAME T24	QUANTITY 2	PLY 1	JOB DESC. 42087	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MITek Industries, Inc. Mon Sep 11 09:28:05 2017 Page 1
ID:CbcXd1J_AWrxHhsljTTM00ykMTA-3F9zQN6BMtuMpJTsvRlzpZyTJR32E2EnNwZb1vyelyO



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	2097	0	2097	0
J	2097	0	2097	0

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0 / 54	-122.2	-122.2	0.17 (1)	C-N	-84 / 97	0.05 (1)
B-C	0 / 34	-122.2	-122.2	0.29 (1)	N-D	0 / 367	0.08 (2)
C-D	-1805 / 0	-122.2	-122.2	0.26 (1)	D-M	0 / 496	0.11 (1)
D-E	-1631 / 0	-122.2	-122.2	0.43 (1)	M-E	-755 / 0	0.98 (1)
E-F	-1631 / 0	-122.2	-122.2	0.43 (1)	M-F	0 / 496	0.11 (1)
F-G	-1805 / 0	-122.2	-122.2	0.26 (1)	K-F	0 / 367	0.08 (2)
G-H	0 / 34	-122.2	-122.2	0.29 (1)	K-G	-84 / 97	0.05 (1)
H-I	0 / 54	-122.2	-122.2	0.17 (1)	O-C	-2176 / 0	0.55 (1)
O-B	-351 / 0	0.0	0.0	0.04 (1)	G-J	-2176 / 0	0.55 (1)
J-H	-351 / 0	0.0	0.0	0.04 (1)			
O-N	0 / 1413	-28.0	-28.0	0.52 (2)			
N-M	0 / 1361	-28.0	-28.0	0.52 (2)			
M-L	0 / 1361	-28.0	-28.0	0.52 (2)			
L-K	0 / 1361	-28.0	-28.0	0.52 (2)			
K-J	0 / 1413	-28.0	-28.0	0.52 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
 CALCULATED VERT. DEFL.(LL)= L/999 (0.16")
 ALLOWABLE DEFL.(TL)= L/360 (0.86")
 CALCULATED VERT. DEFL.(TL)= L/999 (0.26")

CSI: TC=0.43 (D-E:1), BC=0.52 (M-N:2), WB=0.96 (E-M:1), SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

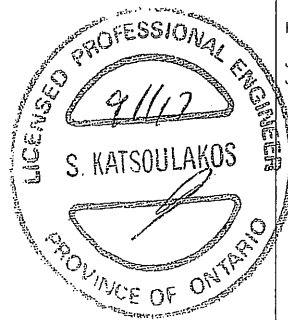
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

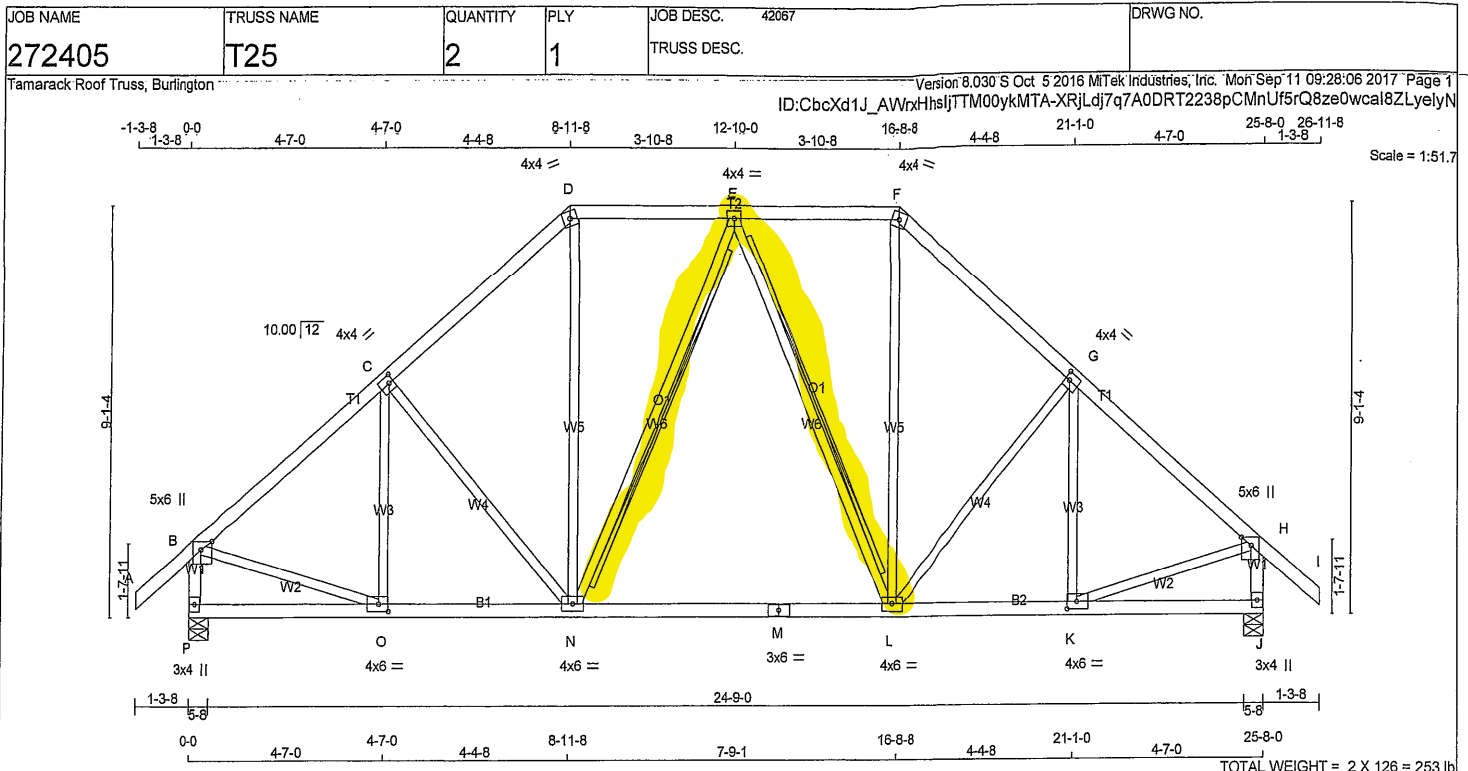
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
 JSI METAL= 0.54 (C) (INPUT = 1.00)



DRWG NO. TAW 4589-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	2097	0	2097	0	5-8	5-8
J	2097	0	2097	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
P	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 54	-122.2 -122.2	0.17 (1)	10.00	O-C	-305 / 55	0.14 (1)
B-C	-1884 / 0	-122.2 -122.2	0.37 (1)	4.53	C-N	-291 / 0	0.25 (1)
C-D	-1722 / 0	-122.2 -122.2	0.35 (1)	4.71	N-D	0 / 688	0.15 (1)
D-E	-1295 / 0	-122.2 -122.2	0.24 (1)	5.41	N-E	-308 / 0	0.23 (1)
E-F	-1295 / 0	-122.2 -122.2	0.24 (1)	5.41	E-L	-308 / 0	0.23 (1)
F-G	-1722 / 0	-122.2 -122.2	0.35 (1)	4.71	L-F	0 / 688	0.15 (1)
G-H	-1884 / 0	-122.2 -122.2	0.37 (1)	4.53	L-G	-291 / 0	0.25 (1)
H-I	0 / 54	-122.2 -122.2	0.17 (1)	10.00	K-G	-305 / 55	0.14 (1)
P-B	-2039 / 0	0.0	0.22 (1)	5.92	B-O	0 / 1538	0.35 (1)
J-H	-2039 / 0	0.0	0.22 (1)	5.92	K-H	0 / 1538	0.35 (1)
P-O	0 / 0	-28.0	-28.0	0.13 (2)	10.00		
O-N	0 / 1479	-28.0	-28.0	0.41 (2)	10.00		
N-M	0 / 1417	-28.0	-28.0	0.40 (2)	10.00		
M-L	0 / 1417	-28.0	-28.0	0.40 (2)	10.00		
L-K	0 / 1479	-28.0	-28.0	0.41 (2)	10.00		
K-J	0 / 0	-28.0	-28.0	0.13 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.37 (B-C:1), BC=0.41 (N-O:2), WB=0.35 (B-O:1), SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

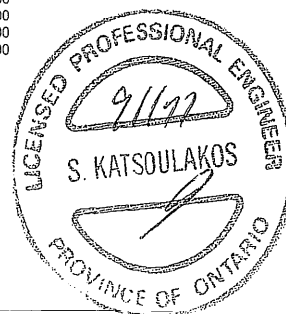
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

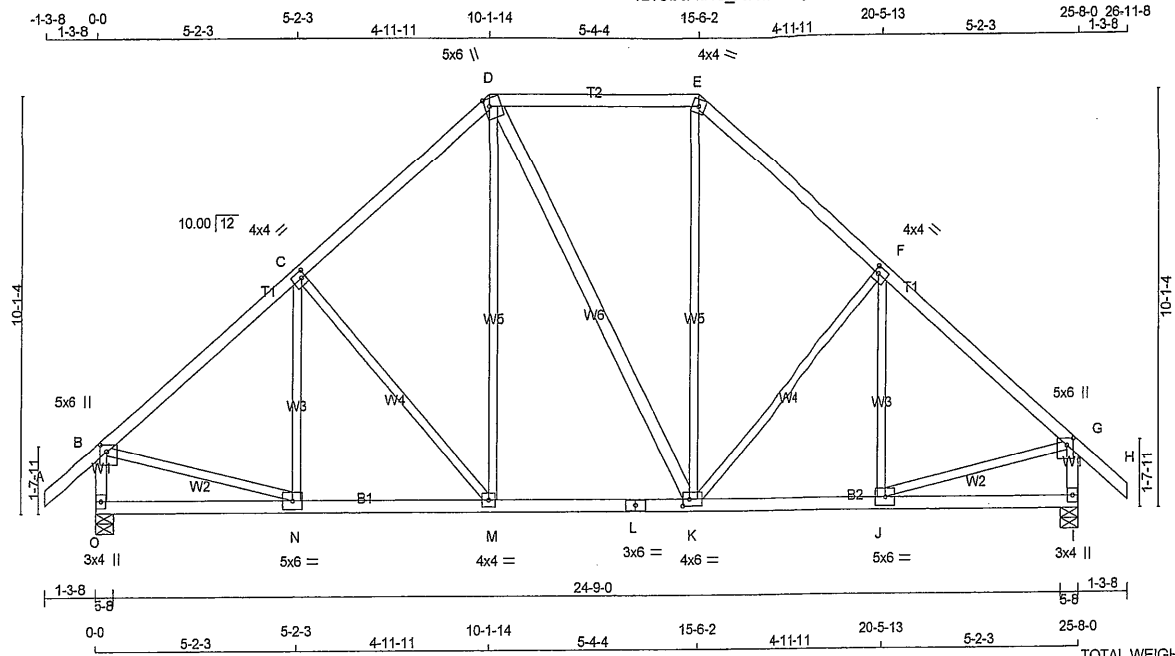
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.53 (M) (INPUT = 1.00)



DWG NO. TAW 4586019
STRUCTURAL COMPONENT ONLY

JOB NAME 272405	TRUSS NAME T26	QUANTITY 2	PLY 1	JOB DESC. 42067	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 09:28:06 2017 Page 1	ID:CbCXd1J_AWrxHsljTTM00ykMTA-XRjLdj7q7A0DRT2238pCMnUdKrRNzcjwcal8ZLyelyN



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0	2.00 2.00
C	TMWW-t	MT20	4.0	4.0	2.00 1.25
D	TTWW+m	MT20	5.0	6.0	2.25 1.50
E	TTW-m	MT20	4.0	4.0	
F	TMWW-t	MT20	4.0	4.0	2.00 1.25
G	TMVW+p	MT20	5.0	6.0	2.00 2.00
I	BMV1+p	MT20	3.0	4.0	
J	BMWW-t	MT20	5.0	6.0	
K	BMWW-t	MT20	4.0	6.0	2.00 2.00
L	BS-t	MT20	3.0	6.0	
M	BMWW-t	MT20	4.0	4.0	
N	BMWW-t	MT20	5.0	6.0	
O	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
O		2097	0	2097	0	0	5-8	5-8	
I		2097	0	2097	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
I	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)
FR-TO		FROM	TO		FR-TO			
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	N-C	-219 / 142	0.12 (1)
B-C	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38	C-M	-428 / 0	0.49 (1)
C-D	-1627 / 0	-122.2	-122.2	0.45 (1)	4.68	M-D	0 / 457	0.10 (1)
D-E	-1215 / 0	-122.2	-122.2	0.47 (1)	5.19	D-K	0 / 2	0.00 (1)
E-F	-1628 / 0	-122.2	-122.2	0.45 (1)	4.68	K-E	0 / 460	0.10 (1)
F-G	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38	K-F	-426 / 0	0.49 (1)
G-H	0 / 54	-122.2	-122.2	0.17 (1)	10.00	J-F	-221 / 140	0.12 (1)
O-B	-2037 / 0	0.0	0.0	0.22 (1)	5.92	B-N	0 / 1543	0.35 (1)
I-G	-2036 / 0	0.0	0.0	0.22 (1)	5.92	J-G	0 / 1543	0.35 (1)
O-N	0 / 0	-28.0	-28.0	0.18 (3)	10.00			
N-M	0 / 1497	-28.0	-28.0	0.33 (1)	10.00			
M-L	0 / 1215	-28.0	-28.0	0.28 (1)	10.00			
L-K	0 / 1215	-28.0	-28.0	0.28 (1)	10.00			
K-J	0 / 1496	-28.0	-28.0	0.32 (1)	10.00			
J-I	0 / 0	-28.0	-28.0	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.48 (B-C:1), BC=0.33 (M-N:1), WB=0.49 (C-M:1), SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.43 (B) (INPUT = 1.00)



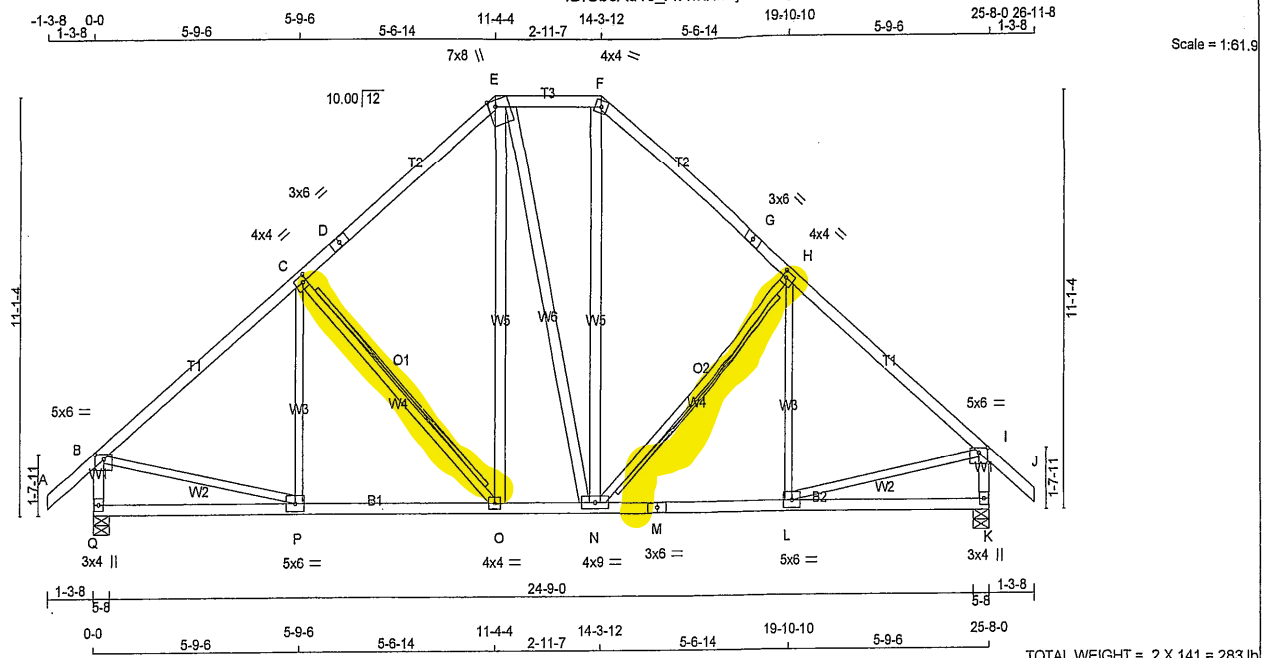
UWG NO. TAN 45868-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272405	T27	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:CbcXd1J_AWrxHhsjTTM00ykMTA-XRjLdj7qA0DRT2238pCMnUbCrQOzZMwcal8ZLyelyN



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
Q - M	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
O - E	2x4	DRY	No.2	SPF	
E - N	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWV+m	MT20	7.0	8.0	Edge	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVWV-t	MT20	4.0	9.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
Q	2097	0	2097	0
K	2097	0	2097	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
Q	1622	1087 / 0	269 / 0	0 / 0	265 / 0
K	1622	1087 / 0	269 / 0	0 / 0	265 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-O, H-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO						FR-TO			
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	P-C	-153 / 204	0.10 (1)	
B-C	-1904 / 0	-122.2	-122.2	0.81 (1)	4.17	C-O	-550 / 0	0.64 (1)	
C-D	-1527 / 0	-122.2	-122.2	0.56 (1)	4.62	O-E	0 / 505	0.08 (1)	
D-E	-1527 / 0	-122.2	-122.2	0.56 (1)	4.62	E-N	0 / 5	0.00 (1)	
E-F	-1136 / 0	-122.2	-122.2	0.15 (1)	5.80	N-F	0 / 511	0.08 (1)	
F-G	-1528 / 0	-122.2	-122.2	0.56 (1)	4.61	N-H	-548 / 0	0.64 (1)	
G-H	-1528 / 0	-122.2	-122.2	0.56 (1)	4.61	L-H	-157 / 201	0.11 (1)	
H-I	-1904 / 0	-122.2	-122.2	0.81 (1)	4.17	B-P	0 / 1542	0.35 (1)	
I-J	0 / 54	-122.2	-122.2	0.17 (1)	10.00	L-I	0 / 1541	0.35 (1)	
Q-B	-2032 / 0	0.0	0.0	0.22 (1)	5.93				
K-I	-2031 / 0	0.0	0.0	0.22 (1)	5.93				
Q-P	0 / 0	-28.0	-28.0	0.24 (3)	10.00				
P-O	0 / 1504	-28.0	-28.0	0.39 (2)	10.00				
O-N	0 / 1135	-28.0	-28.0	0.25 (1)	10.00				
N-M	0 / 1503	-28.0	-28.0	0.39 (2)	10.00				
M-L	0 / 1503	-28.0	-28.0	0.39 (2)	10.00				
L-K	0 / 0	-28.0	-28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.61 (B-C:1), BC=0.39 (O-P:2), WB=0.64 (C-O:1), SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

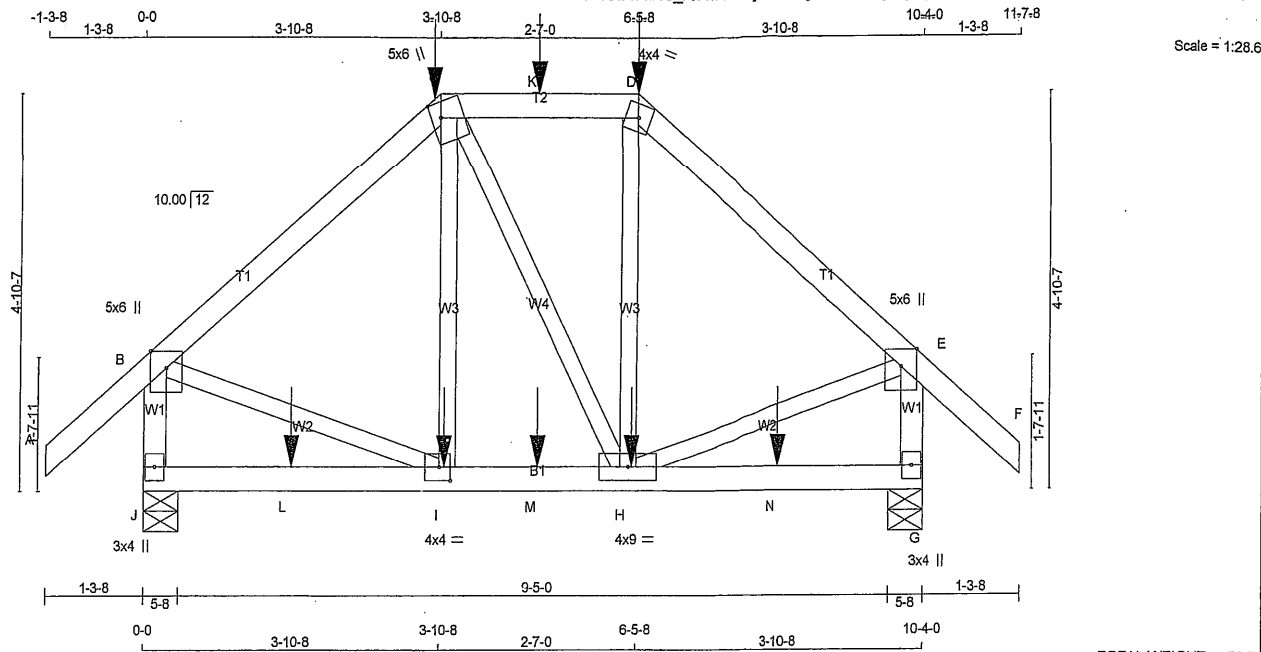
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.45 (M) (INPUT = 1.00)



DRWG NO. TAM 45862-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 384.1 lbs FACTORED DOWN AT 3-10-8, AND 114.6 lbs FACTORED DOWN AT 5-2-0, AND 384.1 lbs FACTORED DOWN AT 6-5-8 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, 42.0 lbs FACTORED DOWN AT 3-11-4, 42.0 lbs FACTORED DOWN AT 5-2-0, AND 42.0 lbs FACTORED DOWN AT 6-4-12, AND 42.0 lbs FACTORED DOWN AT 8-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED			MAXIMUM FACTORED			INPUT	REQRD
JT	GROSS	REACTION	GROSS	REACTION	BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1447	0	1447	0	0	5-8	5-8
G	1447	0	1447	0	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1094	775 / 0	153 / 0	0 / 0	0 / 0	166 / 0	0 / 0
G	1094	775 / 0	153 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH FR-TO	MAX. FACTORED LC1 MAX CSI (LC)
FR-TO				FR-TO			
A-B	0 / 54	-122.2	-122.2 0.19 (1)	10.00	I-C	-128 / 169	0.05 (1)
B-C	-1099 / 0	-122.2	-122.2 0.38 (1)	5.49	C-H	-1 / 2	0.00 (1)
C-K	-838 / 0	-122.2	-122.2 0.27 (1)	6.24	H-D	-130 / 171	0.05 (1)
K-D	-838 / 0	-122.2	-122.2 0.27 (1)	6.24	B-I	0 / 886	0.22 (1)
D-E	-1098 / 0	-122.2	-122.2 0.38 (1)	5.49	H-E	0 / 886	0.22 (1)
E-F	0 / 54	-122.2	-122.2 0.19 (1)	10.00			
J-B	-1389 / 0	0.0	0.0 0.16 (1)	6.87			
G-E	-1388 / 0	0.0	0.0 0.16 (1)	6.87			
J-L	0 / 0	-28.0	-28.0 0.15 (3)	10.00			
L-I	0 / 0	-28.0	-28.0 0.15 (3)	10.00			
I-M	0 / 839	-28.0	-28.0 0.21 (1)	10.00			
M-H	0 / 839	-28.0	-28.0 0.21 (1)	10.00			
H-N	0 / 0	-28.0	-28.0 0.15 (2)	10.00			
N-G	0 / 0	-28.0	-28.0 0.15 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-384	-384	---	BACK	VERT	TOTAL
D	6-5-8	-384	-384	---	BACK	VERT	TOTAL
H	6-4-12	-24	-42	---	BACK	VERT	TOTAL
I	3-11-4	-24	-42	---	BACK	VERT	TOTAL
K	5-2-0	-115	-115	---	BACK	VERT	TOTAL
L	1-11-4	-24	-42	---	BACK	VERT	TOTAL
M	5-2-0	-24	-42	---	BACK	VERT	TOTAL
N	8-4-12	-24	-42	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.38 (B-C:1), BC=0.21 (H-I:1), WB=0.22 (B-I:1), SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

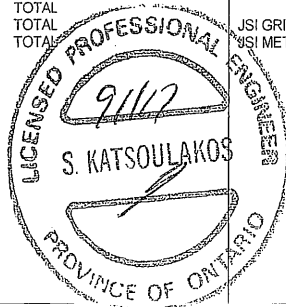
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.31 (I) (INPUT = 1.00)



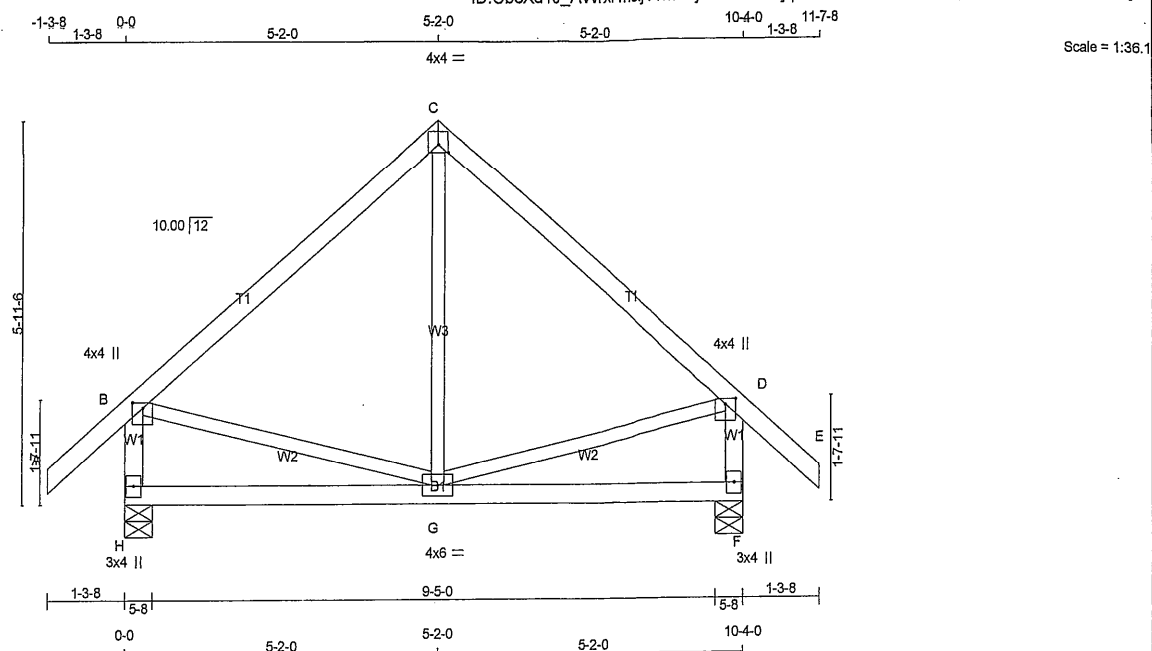
DWG NO. TAM45863-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272405	T29	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:36.1

TOTAL WEIGHT = 47 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
H - B	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	945	0	945	0
F	945	0	945	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
H	721	501 / 0	109 / 0	0 / 0	112 / 0	0 / 0
F	721	501 / 0	108 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 54	-122.2 -122.2	0.17 (1)	10.00	G-C	-10 / 233	0.05 (3)
B-C	-485 / 0	-122.2 -122.2	0.42 (1)	6.25	B-G	0 / 385	0.09 (1)
C-D	-485 / 0	-122.2 -122.2	0.42 (1)	6.25	G-D	0 / 385	0.09 (1)
D-E	0 / 54	-122.2 -122.2	0.17 (1)	10.00			
H-B	-890 / 0	0.0 0.0	0.09 (1)	7.81			
F-D	-890 / 0	0.0 0.0	0.09 (1)	7.81			
H-G	0 / 0	-28.0 -28.0	0.22 (3)	10.00			
G-F	0 / 0	-28.0 -28.0	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.42 (B-C:1), BC=0.22 (G-H:3), WB=0.09 (D-G:1), SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

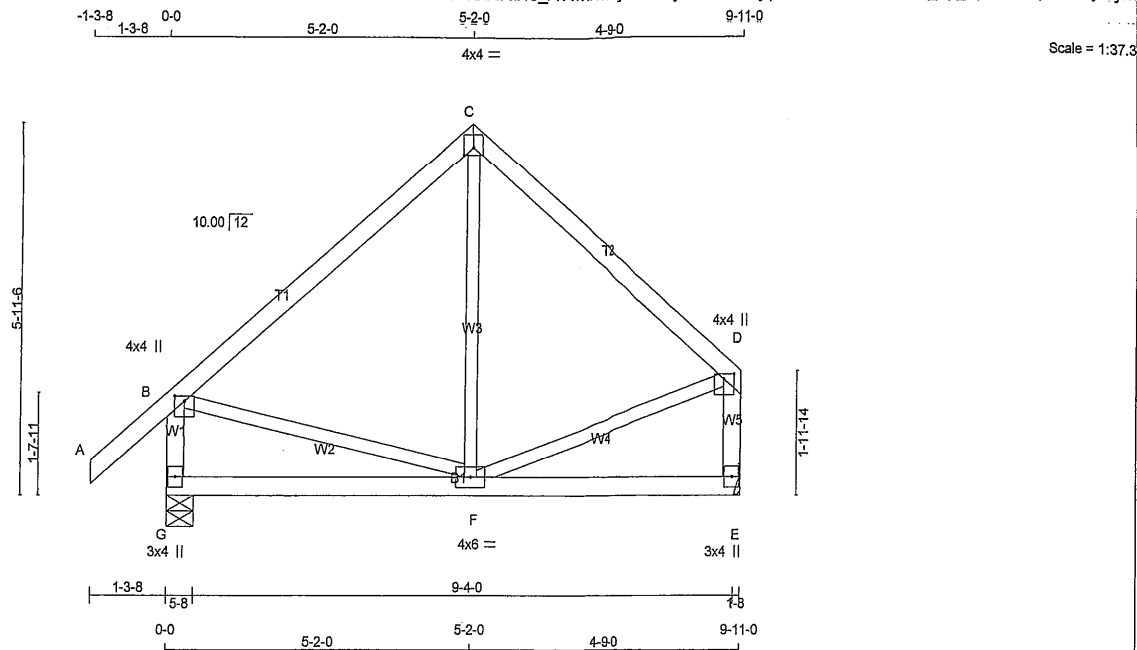
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)



DWNO. YAM 2586717
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV+p	MT20	4.0	4.0	1.00 2.00
C	TTWV-p	MT20	4.0	4.0	1.50 2.00
D	TMWV+p	MT20	4.0	4.0	1.00 2.00
E	BMV1+p	MT20	3.0	4.0	
F	BMWWV-t	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG					
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
G	914	0	914	0	0	5-8	5-8		
E	745	0	745	0	0	HANGER BY OTHERS			
						MIN. SEAT SIZE: 1-8			

UNFACTORED REACTIONS							
JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	697	485 / 0	104 / 0	0 / 0	0 / 0	107 / 0	0 / 0
E	582	379 / 0	104 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD LC1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD LC1 (LC)	MAX. UNBRAC LENGTH FR-TO	
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	F-C	-35 / 213	0.05 (3)	
B-C	-446 / 0	-122.2	-122.2	0.42 (1)	6.25	B-F	0 / 353	0.08 (1)	
C-D	-445 / 0	-122.2	-122.2	0.36 (1)	6.25	F-D	0 / 363	0.08 (1)	
G-B	-855 / 0	0.0	0.0	0.09 (1)	7.81				
E-D	-698 / 0	0.0	0.0	0.08 (1)	7.81				
G-F	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
F-E	0 / 0	-28.0	-28.0	0.20 (3)	10.00				

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL =	38.3 PSF
DL =	3.0 PSF
BOT CH. LL =	10.5 PSF
DL =	7.0 PSF
TOTAL LOAD =	58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
 ALLOWABLE DEFL.(TL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.42 (B-C:1), BC=0.20 (E-F:3), WB=0.08 (D-F:1), SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

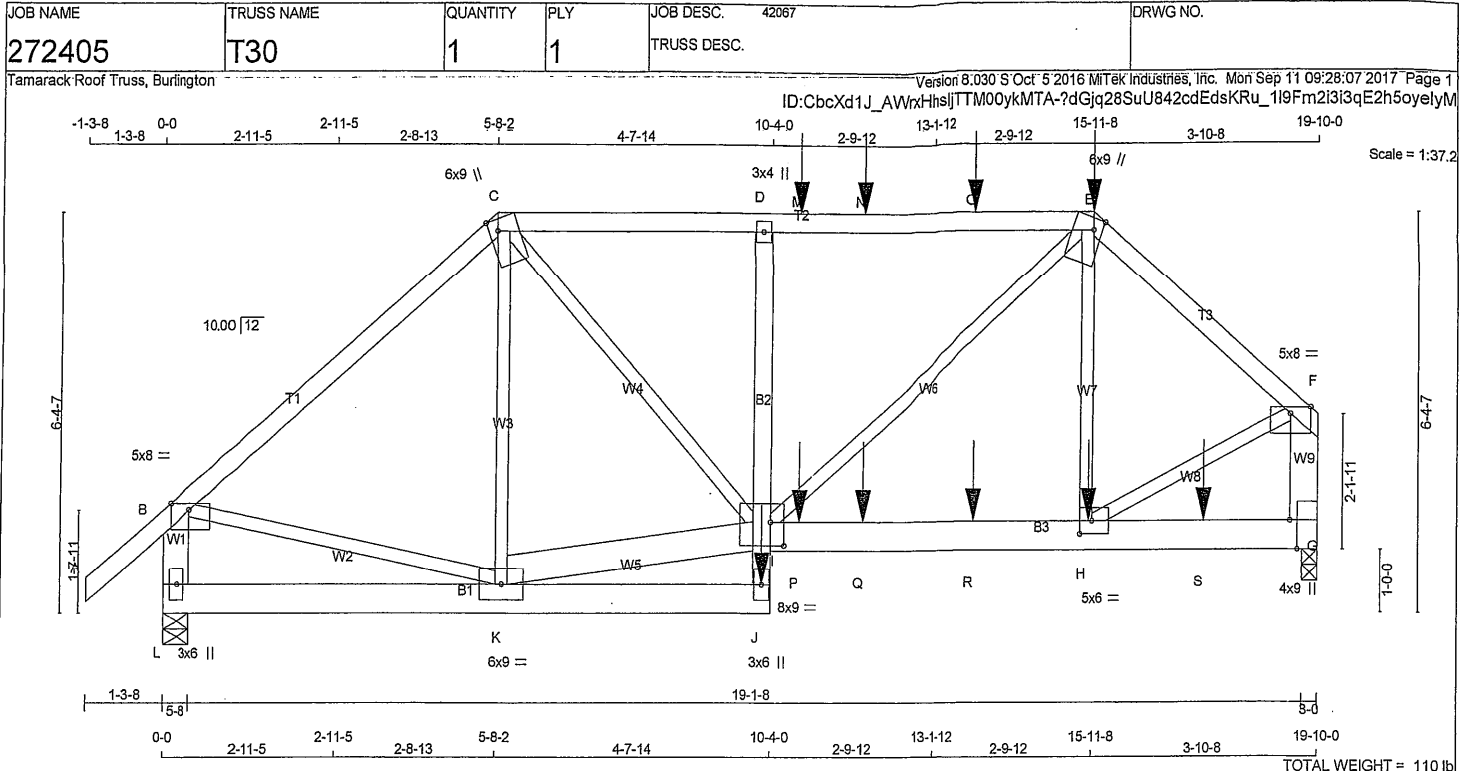
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
 JSI METAL= 0.20 (B) (INPUT = 1.00)



DWG NO. TAM 4586 17
STRUCTURAL COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - F	2x4	DRY	2100F 1.8E	SPF
L - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - I	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	8.0 Edge
C	TTVW+m	MT20	6.0	9.0 Edge 1.75
D	TMV+p	MT20	3.0	4.0
E	TTVW+m	MT20	6.0	9.0 Edge 1.75
F	TMVW-p	MT20	5.0	8.0 Edge
G	BMV1+t	MT20	4.0	9.0 Edge 1.50
H	BMVW-t	MT20	5.0	6.0 2.50 2.50
I	BMVW-t	MT20	8.0	9.0 4.50 2.75
J	BMV+p	MT20	3.0	6.0
K	BMVW-t	MT20	6.0	9.0
L	BMV1+p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.6 lbs FACTORED DOWN AT 10-9-13, 114.6 lbs FACTORED DOWN AT 11-10-12, AND 114.6 lbs FACTORED DOWN AT 13-10-12, AND 384.1 lbs FACTORED DOWN AT 15-11-8 ON TOP CHORD, AND 1056.0 lbs FACTORED DOWN AT 10-2-4, 55.0 lbs FACTORED DOWN AT 10-9-13, 45.9 lbs FACTORED DOWN AT 11-10-12, 45.9 lbs FACTORED DOWN AT 13-10-12, AND 45.9 lbs FACTORED DOWN AT 15-10-12, AND 37.8 lbs FACTORED DOWN AT 17-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
L	2419	0	2419	0	0
G	2639	0	2639	0	0

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS			
L	1860	1266 / 0	297 / 0	0 / 0	0 / 0	298 / 0
G	2041	1366 / 0	339 / 0	0 / 0	0 / 0	336 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.57 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. (PLF)	MAX. FACTORED	MEMB.	FORCE (LBS)	MAX. FACTORED	
FR-TO				FR-TO			
A-B	0 / 54	-122.2	-122.2 0.12 (1)	K-C	-573 / 36	0.36 (1)	
B-C	-2287 / 0	-122.2	-122.2 0.63 (1)	K-I	0 / 1737	0.23 (1)	
C-D	-3124 / 0	-122.2	-122.2 0.62 (1)	C-I	0 / 2055	0.51 (1)	
D-M	-3135 / 0	-122.2	-122.2 0.67 (1)	I-E	0 / 1695	0.42 (1)	
M-N	-3135 / 0	-122.2	-122.2 0.67 (1)	H-E	-557 / 153	0.23 (1)	
N-O	-3135 / 0	-122.2	-122.2 0.67 (1)	B-K	0 / 1793	0.44 (1)	
O-E	-3135 / 0	-122.2	-122.2 0.67 (1)	H-F	0 / 2028	0.50 (1)	
E-F	-2431 / 0	-122.2	-122.2 0.28 (1)				
L-B	-2355 / 0	0.0	0.0 0.17 (1)				
G-F	-2605 / 0	0.0	0.0 0.21 (1)				
L-K	0 / 0	-28.0	-28.0 0.12 (2)				
K-J	0 / 50	-28.0	-28.0 0.12 (2)				
J-I	0 / 1096	0.0	0.0 0.27 (1)				
I-D	-1035 / 0	0.0	0.0 0.43 (1)				
I-P	0 / 1851	-28.0	-28.0 0.33 (1)				
P-Q	0 / 1851	-28.0	-28.0 0.33 (1)				
Q-R	0 / 1851	-28.0	-28.0 0.33 (1)				
R-H	0 / 1851	-28.0	-28.0 0.33 (1)				
H-S	0 / 0	-28.0	-28.0 0.16 (3)				
S-G	0 / 0	-28.0	-28.0 0.16 (3)				

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX-	FACE	DIR.
E	15-11-8	-384	-384	---	FRONT VERT
H	15-10-12	-26	-46	---	FRONT VERT
J	10-2-4	-1056	-1056	---	FRONT VERT
M	10-9-13	-115	-115	---	FRONT VERT
N	11-10-12	-115	-115	---	FRONT VERT
O	13-10-12	-115	-115	---	FRONT VERT
P	10-9-13	-31	-55	---	FRONT VERT
Q	11-10-12	-26	-46	---	FRONT VERT
R	13-10-12	-26	-46	---	FRONT VERT
S	17-10-12	-16	-38	---	FRONT VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.67 (D-E:1), BC=0.43 (D-I:1), WB=0.51 (C-I:1), SSI=0.57 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

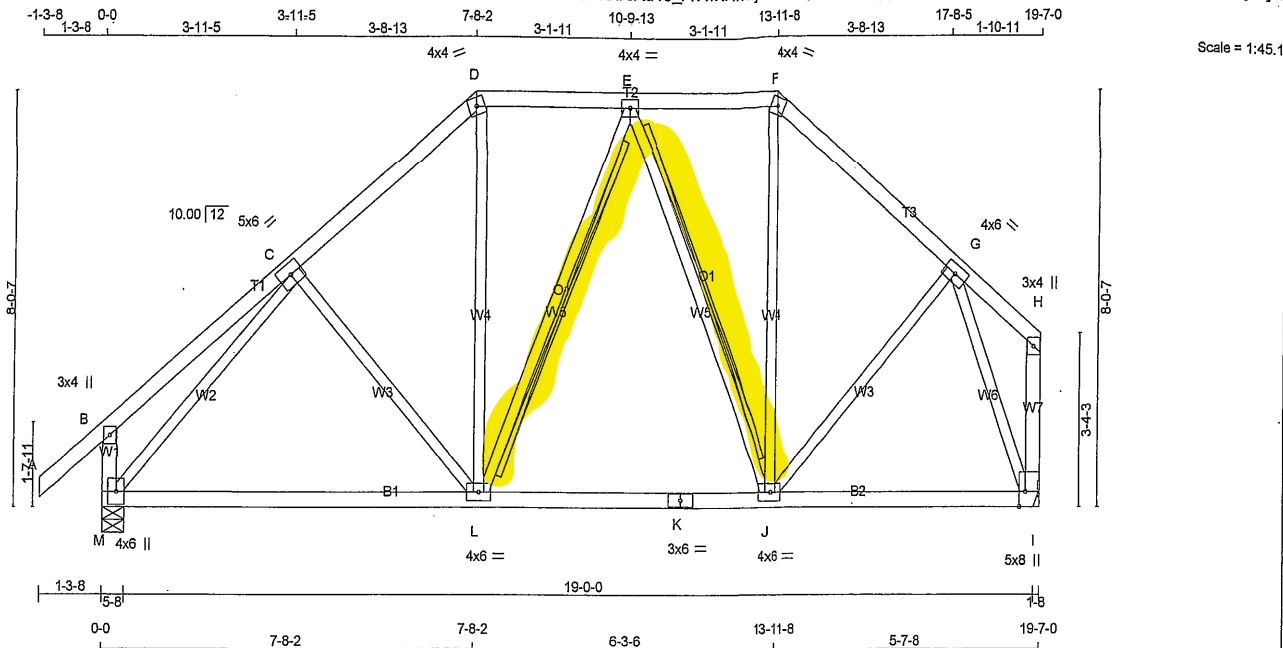
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.46 (H) (INPUT = 1.00)

DWG NO. TAM 45866-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
M - B	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	5.0	6.0		
D TTW-m	MT20	4.0	4.0		
E TMWW-t	MT20	4.0	4.0		
F TTW-m	MT20	4.0	4.0		
G TMWW-t	MT20	4.0	6.0		
H TMV+p	MT20	3.0	4.0		
I BMVW1+t	MT20	5.0	8.0	Edge 1.50	
J BMVWW-t	MT20	4.0	6.0		
K BS-t	MT20	3.0	6.0		
L BMVWW-t	MT20	4.0	6.0		
M BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1640	0	5-8	5-8
I	1471	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1284	855 / 0	206 / 0	0 / 0	0 / 0	204 / 0	0 / 0
I	1150	749 / 0	206 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-L, E-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0 / 54	-122.2 -122.2	0.17 (1)	C-L	-178 / 60	0.11 (1)	
B-C	0 / 35	-122.2 -122.2	0.29 (1)	L-D	0 / 420	0.09 (1)	
C-D	-1215 / 0	-122.2 -122.2	0.24 (1)	E-J	-67 / 75	0.08 (1)	
D-E	-910 / 0	-122.2 -122.2	0.15 (1)	J-K	-424 / 0	0.49 (1)	
E-F	-780 / 0	-122.2 -122.2	0.15 (1)	K-L	0 / 300	0.07 (1)	
F-G	-1037 / 0	-122.2 -122.2	0.22 (1)	L-M	0 / 347	0.08 (1)	
G-H	0 / 52	-122.2 -122.2	0.21 (1)	M-N	-1578 / 0	0.92 (1)	
H-I	-348 / 0	0.0 0.0	0.04 (1)	N-O	-1481 / 0	0.55 (1)	
I-J	-28 / 0	0.0 0.0	0.01 (1)				
M-L	0 / 1023	-28.0 -28.0	0.46 (2)				
L-K	0 / 935	-28.0 -28.0	0.46 (2)				
K-J	0 / 935	-28.0 -28.0	0.46 (2)				
J-I	0 / 548	-28.0 -28.0	0.24 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/977 (0.24")

CSI: TC=0.29 (B-C:1), BC=0.46 (L-M:2), WB=0.92 (C-M:1), SSI=0.18 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)



DRWG NO. TAM 45809-17
STRUCTURAL
COMPONENT ONLY

HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

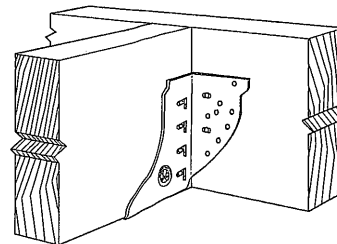
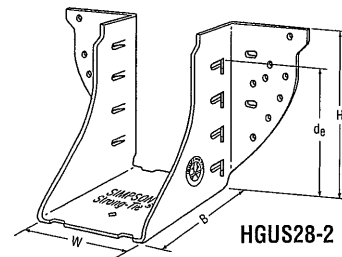
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

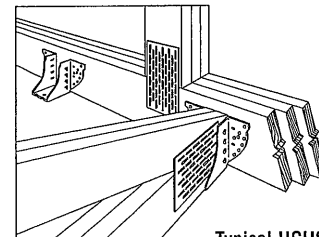
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



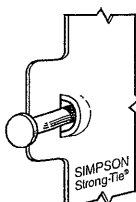
Typical HGUS Installation



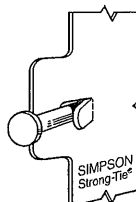
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
HGUS26	12	1⅞	5⅞	5	4⅞ ₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅞	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅞	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅞	66-16d	22-16d	10130	16400	7195	11645

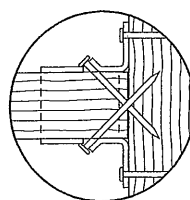
1. d_g is the distance from the seat of the hanger to the highest joist nail.



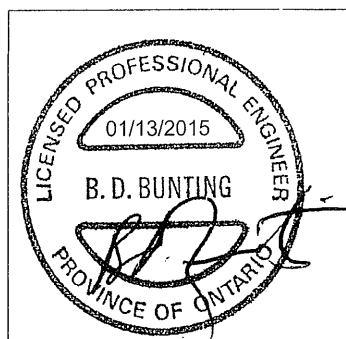
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

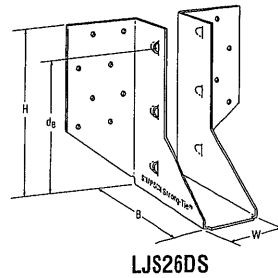
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

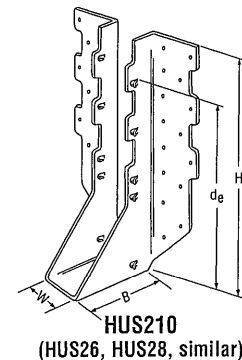
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

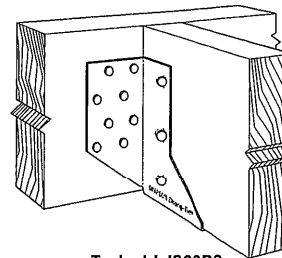
- See current catalogue for options



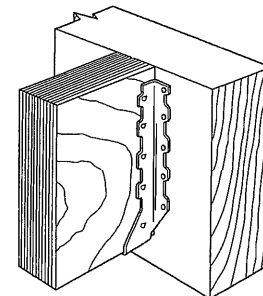
LJS26DS



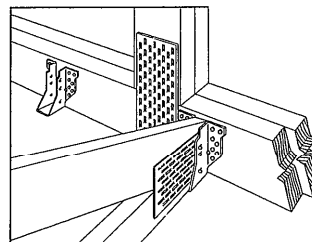
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



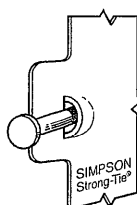
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

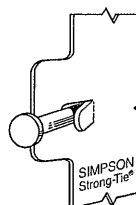
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LJS26DS	18	1½	5	3½	4½	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1½	5½	3	3½	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6½	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1½	9½	3	7½	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

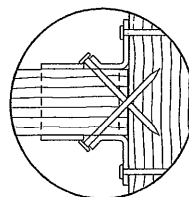


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

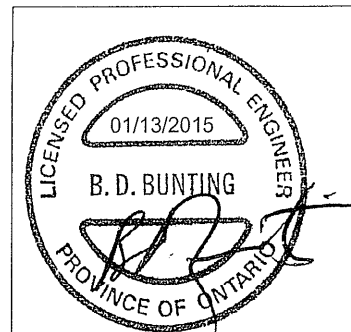
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

FACE MOUNT HANGERS LUS/HUS Double Shear Joist Hangers

SIMPSON
Strong-Tie


This product is preferable to similar connectors because of
 a) easier installation, b) higher capacities, c) lower installed cost,
 or a combination of these features.

See Hanger tables on pages 79-84. See Hanger Options on page 230 for hanger modifications, which may result in reduced resistances.

All hangers in this series have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation, and the use of standard nails for all connections. (Do not bend or remove tabs.)

MATERIAL: See tables, pages 79-84.

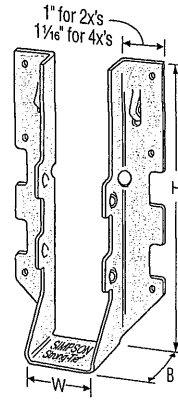
FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

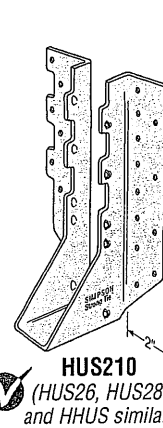
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances.
- Not designed for welded or nailer applications.
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated resistance.
- With 3x carrying members, use 16d x 2½" nails into the header and 16d commons into the joist with no reduction in resistances. With a single 2x carrying member, use 10d x 1½" nails into the header and 10d commons into the joist, reduce the resistance to 0.64 of the table value.

OPTIONS: • LUS hangers cannot be modified.

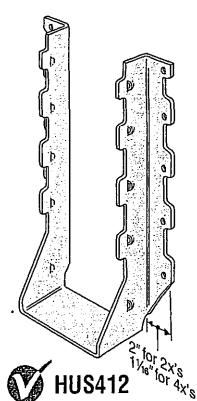
- HUS hangers available with the header flanges turned in for 2-2x (3½") and 4x only, with no reduction in resistances. See the HUSC Concealed Flange illustration.
- See Hanger Options, page 230.



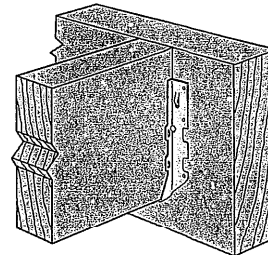
✓ **LUS28**



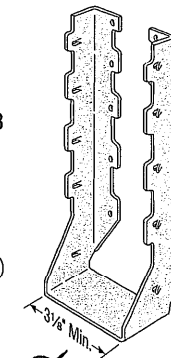
✓ **HUS210**
(HUS26, HUS28, and HHUS similar)



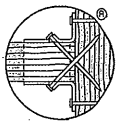
✓ **HUS412**



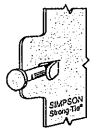
Typical LUS28 Installation
Standard LUS28 Installation use .148x3" (10d common)



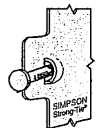
✓ **HUSC Concealed Flanges**
(not available for HHUS, HGUS and HUS2x)



Double Shear Nailing Top View



Double Shear Nailing Side View
Do not bend tab back



Dome Double Shear Nailing prevents tabs from breaking off (available on some models)
U.S. Patent 5,603,580

HUCQ Heavy Duty Joist Hangers

The HUCQ series are heavy duty joist hangers that incorporate Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws. Designed and tested for installation at the end of a beam or on a post, they provide a strong connection with fewer fasteners than nailed hangers.

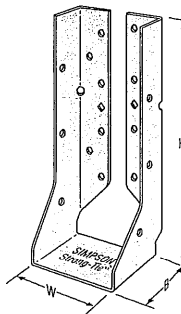
MATERIAL: 14 gauge

FINISH: Galvanized. Most models available in stainless steel or ZMAX® coating.

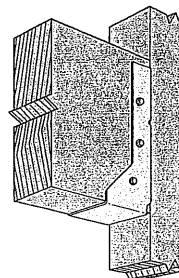
INSTALLATION: • Use all specified fasteners. See General Notes.

- Strong-Drive® SDS Heavy-Duty Connector screws supplied.
- For use on solid sawn or engineered wood products.

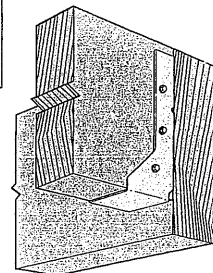
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.



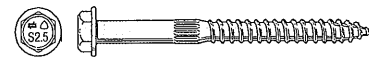
HUCQ410



Typical HUCQ Installation on a Post



Typical HUCQ Installation on a Beam



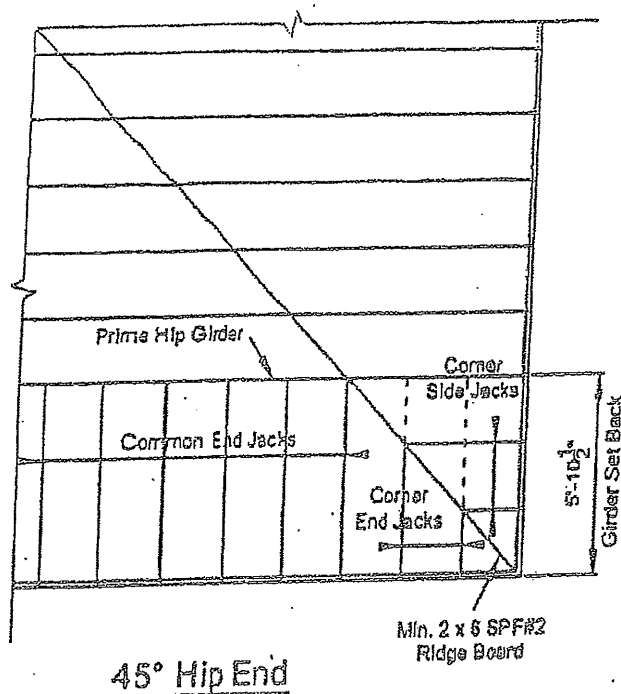
Strong-Drive® ¼"x2½" SDS HEAVY-DUTY CONNECTOR Screw
(See page 25 for more information)

Model No.	Dimensions (in)			Fasteners		Factored Resistance			
	W	H	B	Face	Joist	D. Fir-L		S-P-F	
						Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
						lbs	lbs	lbs	lbs
						kN	kN	kN	kN
HUCQ310-SDS	2⅞	9	3	8-¼"x2½" SDS	4-¼"x2½" SDS	2140	5235	1930	3770
						9.52	23.29	8.59	16.77
HUCQ210-2-SDS	3¼	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	3210	7270	2900	6825
						14.28	32.34	12.90	30.36
HUCQ410-SDS	3⅞	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	3210	7270	2900	6825
						14.28	32.34	12.90	30.36
HUCQ412-SDS	3⅞	11	3	14-¼"x2½" SDS	6-¼"x2½" SDS	3210	9090	2900	7645
						14.28	40.43	12.90	34.01
HUCQ210-3-SDS	4⅞	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	3210	7270	2900	6825
						14.28	32.34	12.90	30.36
HUCQ610-SDS	5⅞	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	3210	7270	2900	6825
						14.28	32.34	12.90	30.36
HUCQ612-SDS	5⅞	11	3	14-¼"x2½" SDS	6-¼"x2½" SDS	3210	9090	2900	7645
						14.28	40.43	12.90	34.01

- Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading such as cantilever construction.
- When using structural composite lumber columns, Strong-Drive® SDS Heavy-Duty Connector screws must be applied to the wide face of the column.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

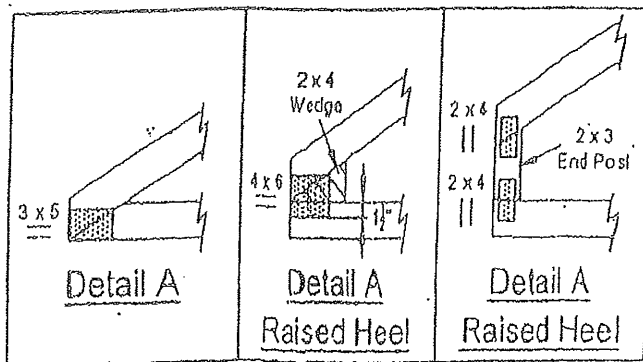
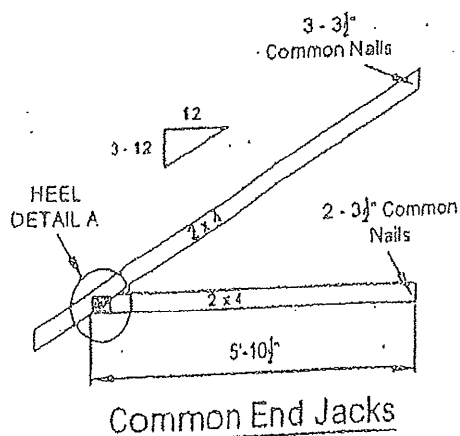
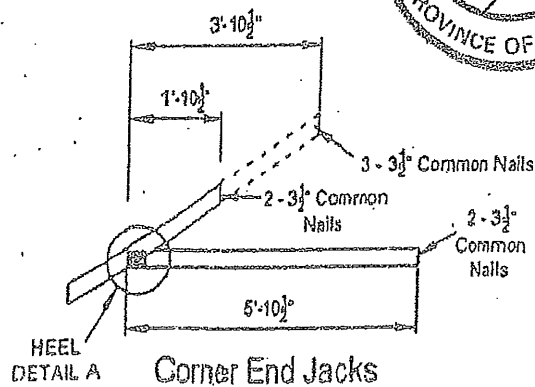
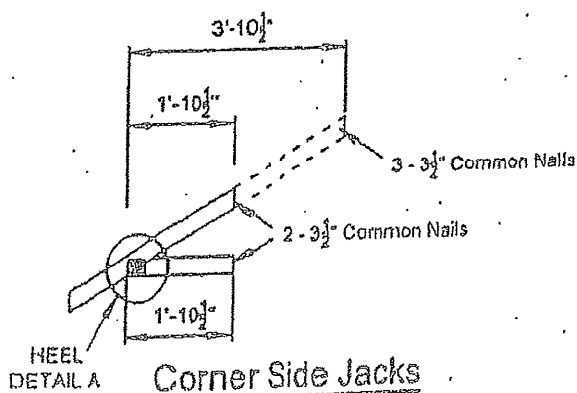
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

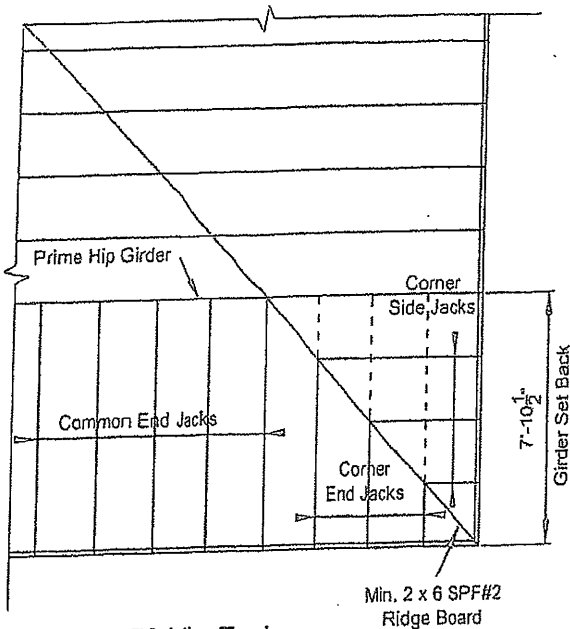
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

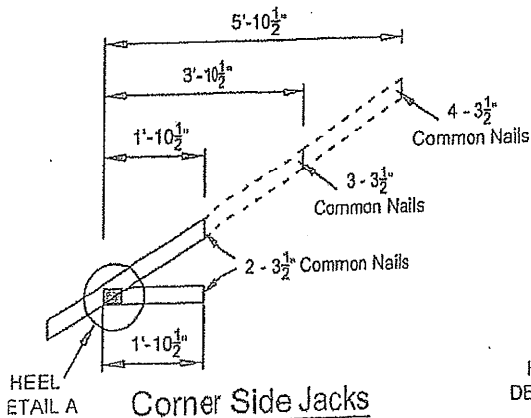
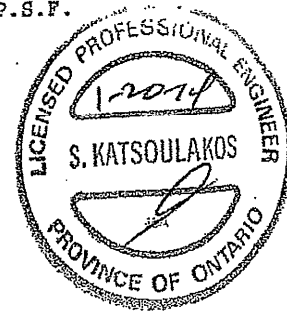
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

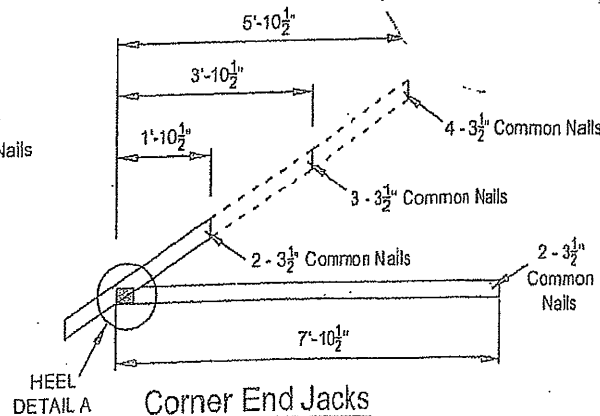


45° Hip End

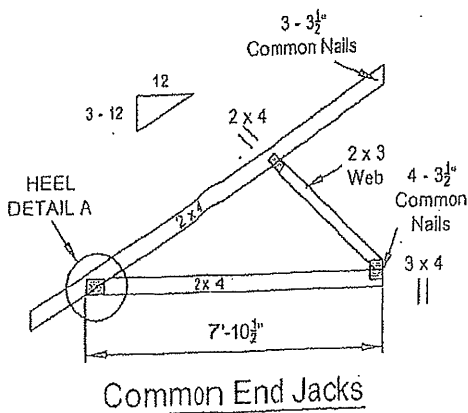
DWG NO TAM 3503.14
STRUCTURAL
COMPONENT ONLY



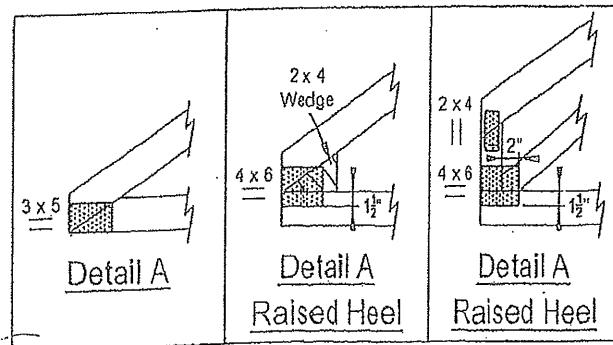
Corner Side Jacks



Corner End Jacks



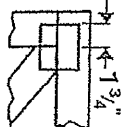
Common End Jacks



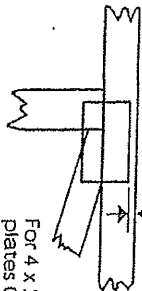
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

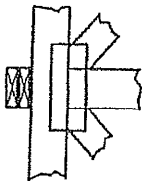
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



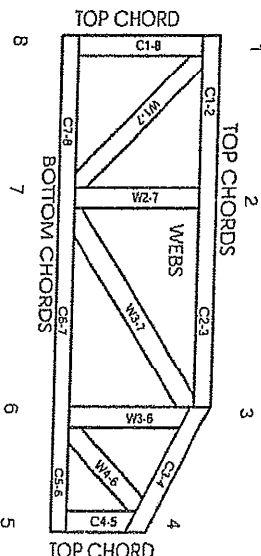
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications
DSF-89: Design Standard for Bracing.

Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings shown to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM™
Mitek Engineering Reference Sheet MIL-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

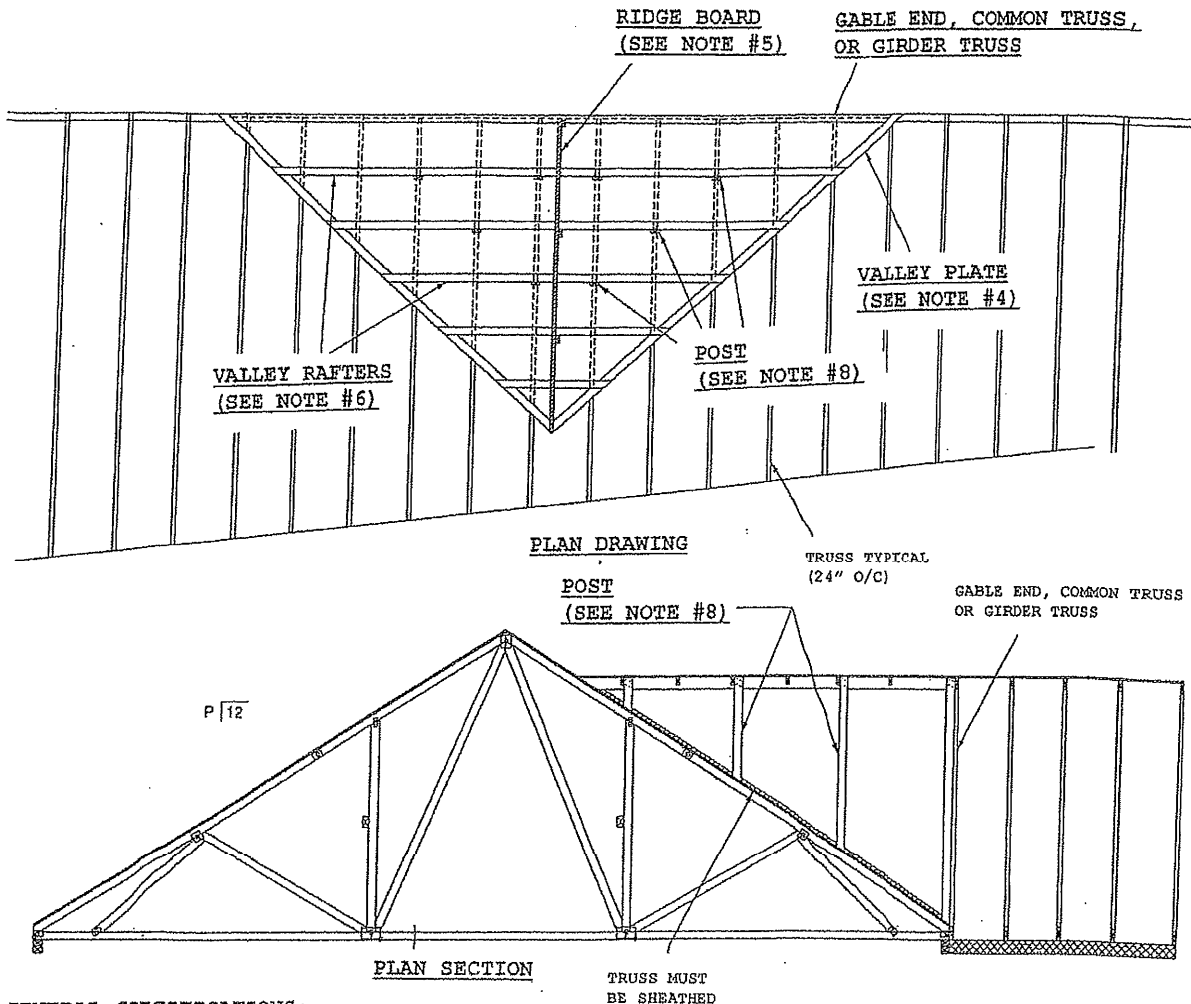
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD, MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.).
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.).
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.).
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWC NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.