10/12 ROOF PITCH UNLESS NOTED

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DENOTES CONVENTIONAL FRAMING

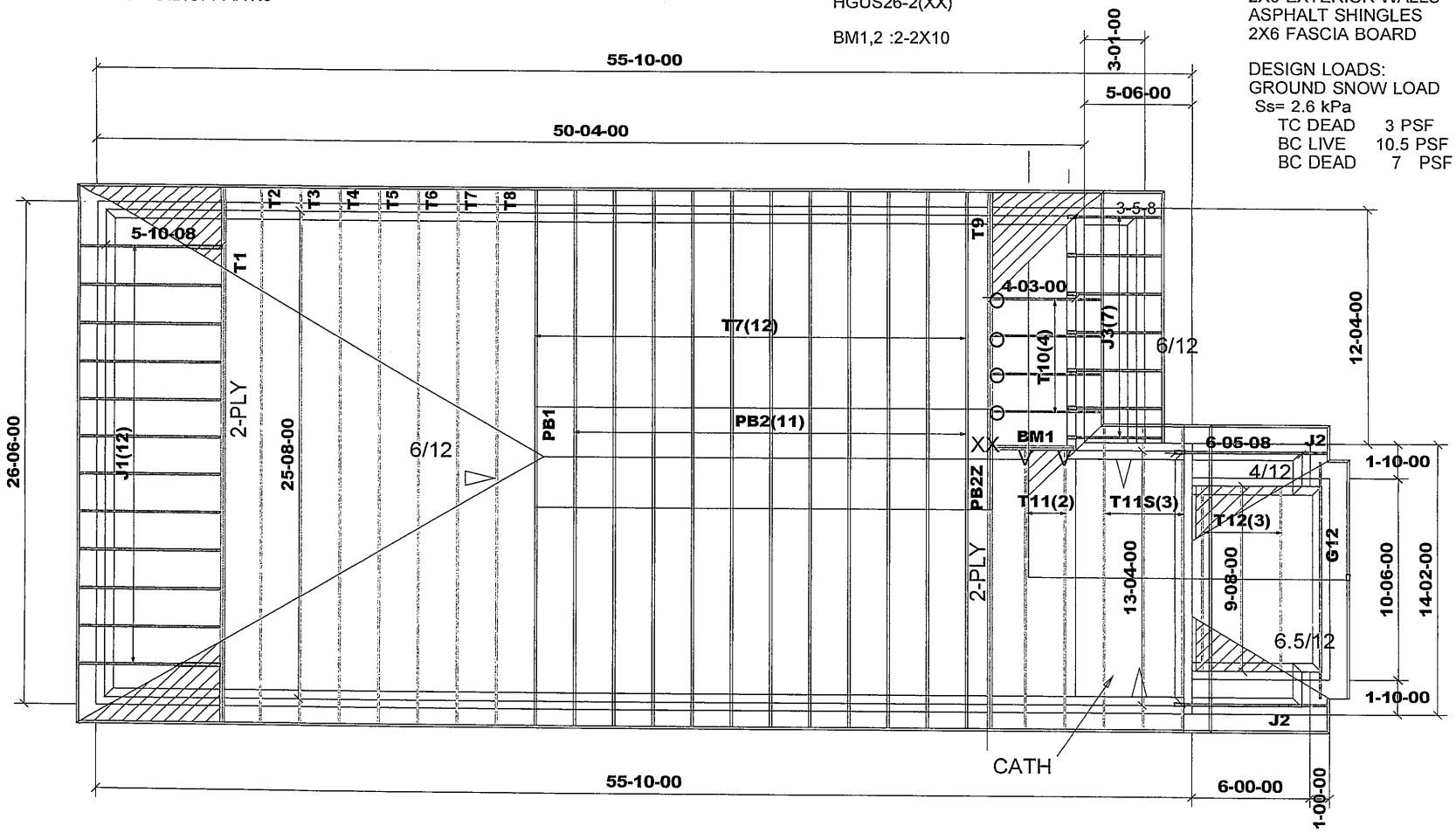
HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)

BM1,2 :2-2X10

T-170678

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



Town of Innisfil Certified Model
14/02/2018 10:05:33 AM kgervais

M9783

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-5-12G / A
	Layout ID: 266141	Project: ALCONA SHORES	
	Plan Log: 85812	Date: 9/8/2017 Designer: JG	

Mitek ver 7.5.0

T-170678

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

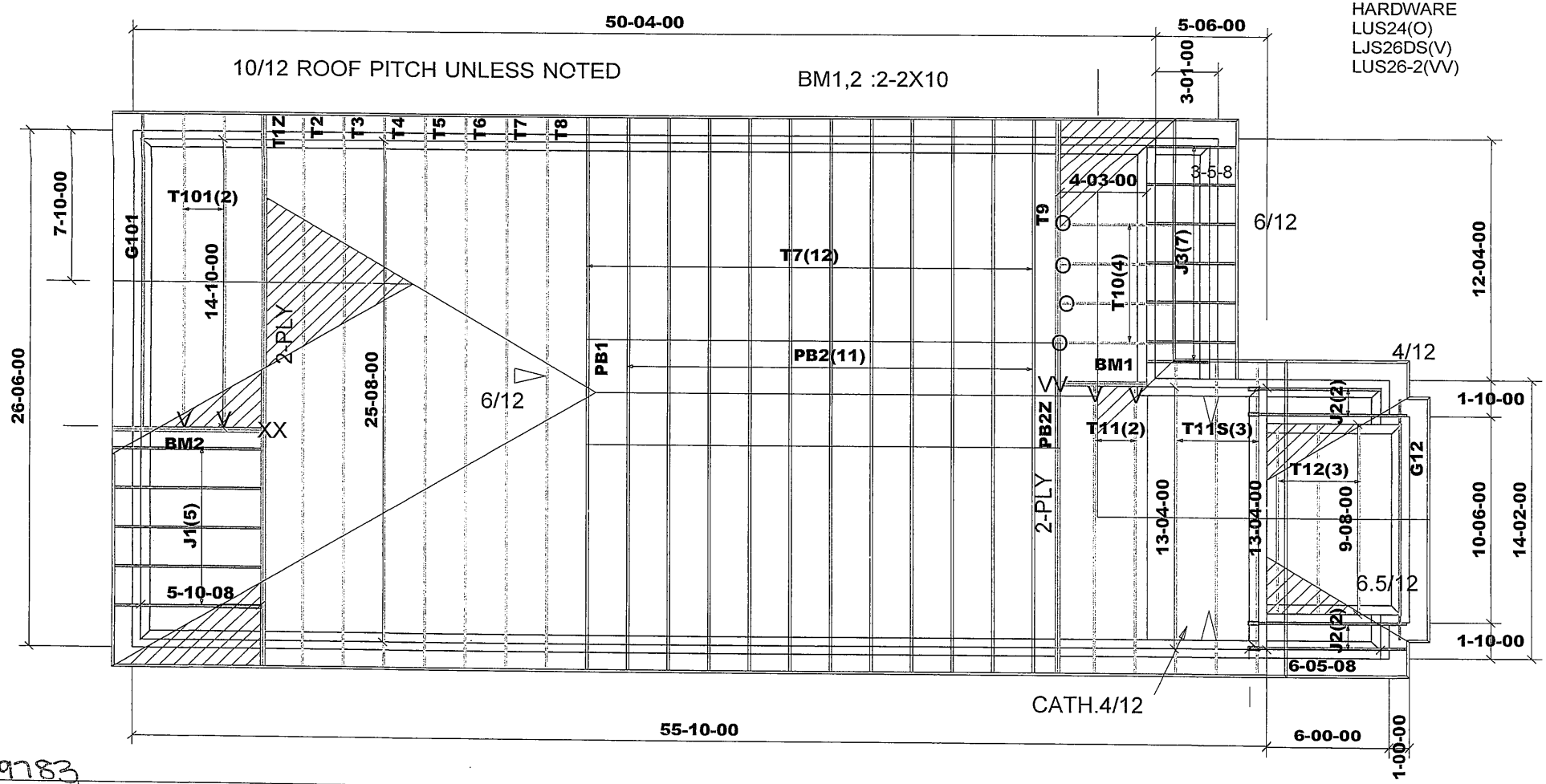
ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(VV)



M9783

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-5-12G / A-REAR UPGRADE
	Layout ID: 272357	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/8/2017 Designer: JG	

Mitek ver 7.5.0

12/12 ROOF PITCH
UNLESS NOTED

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DENOTES CONVENTIONAL FRAMING

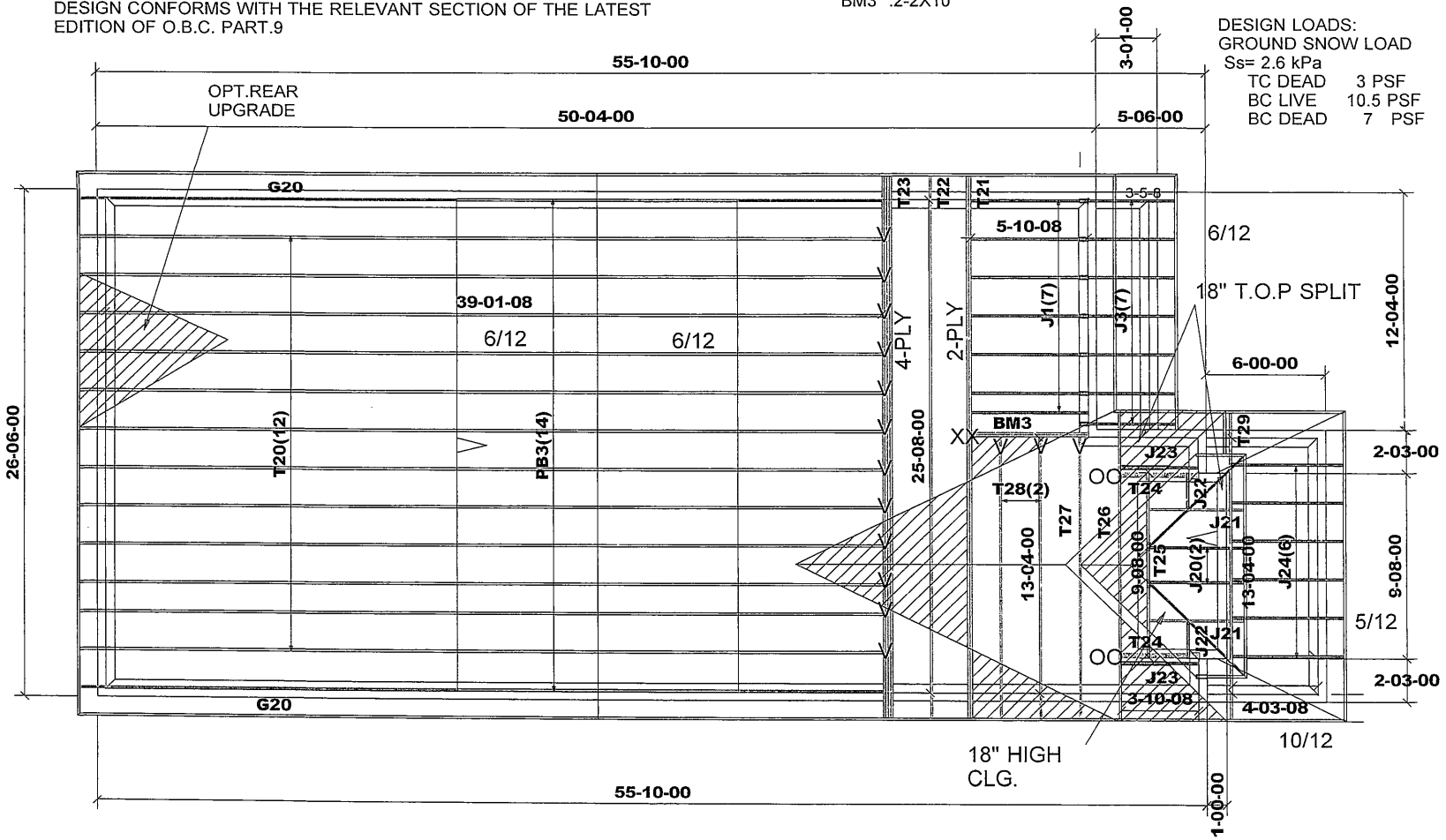
HARDWARE
LJS26DS(V)
HGUS26-2(XX)
LUS24-2(OO)

BM3 :2-2X10

T-170678

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



M9783

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-5-12 / B
	Layout ID: 266142	Project: ALCONA SHORES	
	Plan Log: 85812	Date: 9/8/2017 Designer: JG	

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

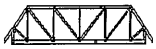
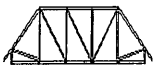
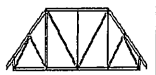
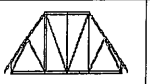
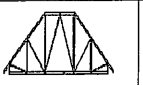
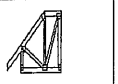
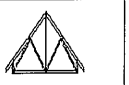
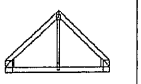
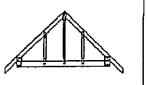
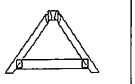
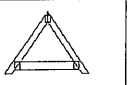
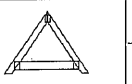
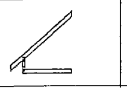
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 266141 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-5-12G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	245.66 152.66		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	107.29 69.67		
	1	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	113.57 72.00		
	1	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.38 72.83		
	1	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.26 77.67		
	1	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	126.45 80.00		
	13	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1677.52 1061.71		
	1	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.43 87.67		
	1 2 Ply	T9 PIGGYBACK	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	299.46 188.66		
	4	T10 JACK-CLOSED	10.00 0.00	04-03-00	03-07-14	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-14	99.32 72.68		
	2	T11 COMMON	10.00 0.00	13-04-00	07-02-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	126.88 80.34		
	3	T11S SCISSOR	10.00 4.00	13-04-00	07-02-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	187.53 118.50		
	3	T12 COMMON	6.50 0.00	09-08-00	03-02-14	2 X 4	2 X 4	00-00-00 00-00-00	00-07-08 00-07-08	87.24 54.51		
	1	G12 COMMON	6.50 0.00	09-08-00	03-02-14	2 X 4	2 X 4	01-03-08 01-05-00	00-07-08 00-07-08	33.39 22.17		
	1	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	12.42 9.33		
	11	PB2 PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	138.71 95.37		
	1 2 Ply	PB2Z PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	25.22 17.34		
	12	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

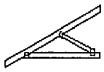
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 266141 **LOCATION:** INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO **SUB-BUILDER:**
MODEL: S32-5-12G **ELEVATION:** A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	J2 JACK-OPEN	4.00 0.00	06-05-08	02-05-12	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-12	38.52 25.34		
	7	J3 JACK-OPEN	6.00 0.00	03-05-08	02-03-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-03-08	75.81 51.31		

TOTAL # TRUSS= **71.00** TOTAL BFT OF ALL TRUSSES= **2537.80** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3976.54** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
4	Hangers	LUS24	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

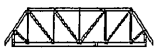
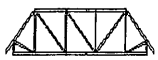
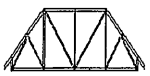
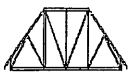
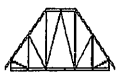
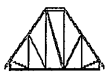
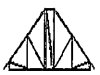
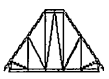
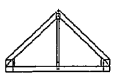
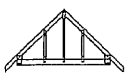
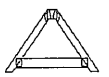
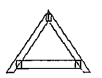
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272357 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-5-12G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	245.66 152.66		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	107.29 69.67		
	1	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	113.57 72.00		
	1	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.38 72.83		
	1	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.26 77.67		
	1	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	126.45 80.00		
	13	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1677.52 1061.71		
	1	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.43 87.67		
	1 2 Ply	T9 PIGGYBACK	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	299.46 188.66		
	4	T10 JACK-CLOSED	10.00 0.00	04-03-00	03-07-14	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-14	99.32 72.68		
	2	T11 COMMON	10.00 0.00	13-04-00	07-02-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	126.88 80.34		
	3	T11S SCISSOR	10.00 4.00	13-04-00	07-02-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	187.53 118.50		
	3	T12 COMMON	6.50 0.00	09-08-00	03-02-14	2 X 4	2 X 4	00-00-00 00-00-00	00-07-08 00-07-08	87.24 54.51		
	1	G12 COMMON	6.50 0.00	09-08-00	03-02-14	2 X 4	2 X 4	01-03-08 01-05-00	00-07-08 00-07-08	33.39 22.17		
	2	T101 COMMON	10.00 0.00	14-10-00	07-09-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	138.88 89.34		
	1	G101 COMMON	10.00 0.00	14-10-00	07-09-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	73.56 47.67		
	1	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	12.42 9.33		
	11	PB2 PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	138.71 95.37		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

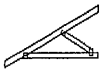
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272357 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-5-12G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	PB2Z PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	25.22 17.34		
	5	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	83.95 53.35		
	4	J2 JACK-OPEN	4.00 0.00	06-05-08	02-05-12	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-12	76.16 48.68		
	7	J3 JACK-OPEN	6.00 0.00	03-05-08	02-03-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-03-08	75.81 51.31		

TOTAL # TRUSS= **69.00** TOTAL BFT OF ALL TRUSSES= **2623.46** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4109.09** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **10.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

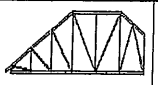
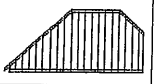
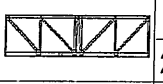
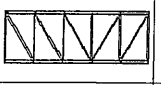
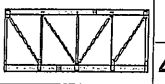
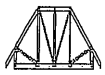
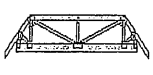
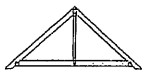
Delivery Shiplist

DATE	06/02/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 266142 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-5-12 Gr ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	12	T20 PIGGYBACK	6.00 0.00	39-01-08	10-00-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 06-04-04	2606.28 1604.04		
	2	G20 PIGGYBACK	6.00 0.00	39-01-08	10-00-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 06-04-04	473.18 302.66		
	1 2 Ply	T21 FLAT GIRDER	0.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	242.38 149.34		
	1	T22 FLAT	0.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	05-01-04 05-01-04	110.58 68.83		
	1 4 Ply	T23 FLAT GIRDER	0.00 0.00	25-08-00	06-01-04	2 X 6	2 X 8	00-00-00 00-00-00	00-00-00 06-01-04	670.92 401.32		
	2 2 Ply	T24 FLAT	0.00 0.00	03-10-08	01-06-00	2 X 4	2 X 4	00-00-00 00-00-00	01-06-00 01-06-00	54.84 34.68		
	1	T25 HIP GIRDER	12.00 0.00	09-08-00	05-04-00	2 X 4	2 X 4	00-00-00 00-00-00	01-05-08 01-05-08	46.63 30.17		
	1	T26 HIP GIRDER	12.00 0.00	13-04-00	05-09-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	80.20 50.17		
	1	T27 HIP	12.00 0.00	13-04-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	78.05 51.00		
	2	T28 COMMON	12.00 0.00	13-04-00	08-06-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	138.56 88.00		
	1	T29 HIP GIRDER	10.00 0.00	13-04-00	02-02-13	2 X 4	2 X 6	01-03-08 01-03-08	01-00-07 01-00-07	60.40 39.67		
	14	PB3 PIGGYBACK	6.00 0.00	12-03-06	03-05-00	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	480.90 291.62		
	7	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	117.53 74.69		
	7	J3 JACK-OPEN	6.00 0.00	03-05-08	02-03-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-03-08	75.81 51.31		
	2	J20 JACK-OPEN	12.00 0.00	03-10-08	05-04-00	2 X 4	2 X 4	00-10-08 00-00-00	01-05-08 05-04-00	33.30 22.66		
	2	J21 JACK-OPEN	12.00 0.00	03-10-08	03-02-15	2 X 4	2 X 4	00-10-08 -02-01-01	01-05-08 00-03-08	26.56 18.66		
	2	J22 JACK-OPEN	12.00 0.00	01-10-08	03-02-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-05-08 00-03-08	20.34 14.00		
	2	J23 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	35.80 22.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	06/02/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 266142 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SH0 SUB-BUILDER:
MODEL: S32-5-12 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	6	J24 JACK-OPEN	5.00 0.00	04-03-08	02-02-13	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-02-13	76.38 52.02		

TOTAL # TRUSS= **73.00** TOTAL BFT OF ALL TRUSSES= **3367.50** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5428.64** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
15	Hangers	LJS26DS	

TOTAL # ITEMS= **6.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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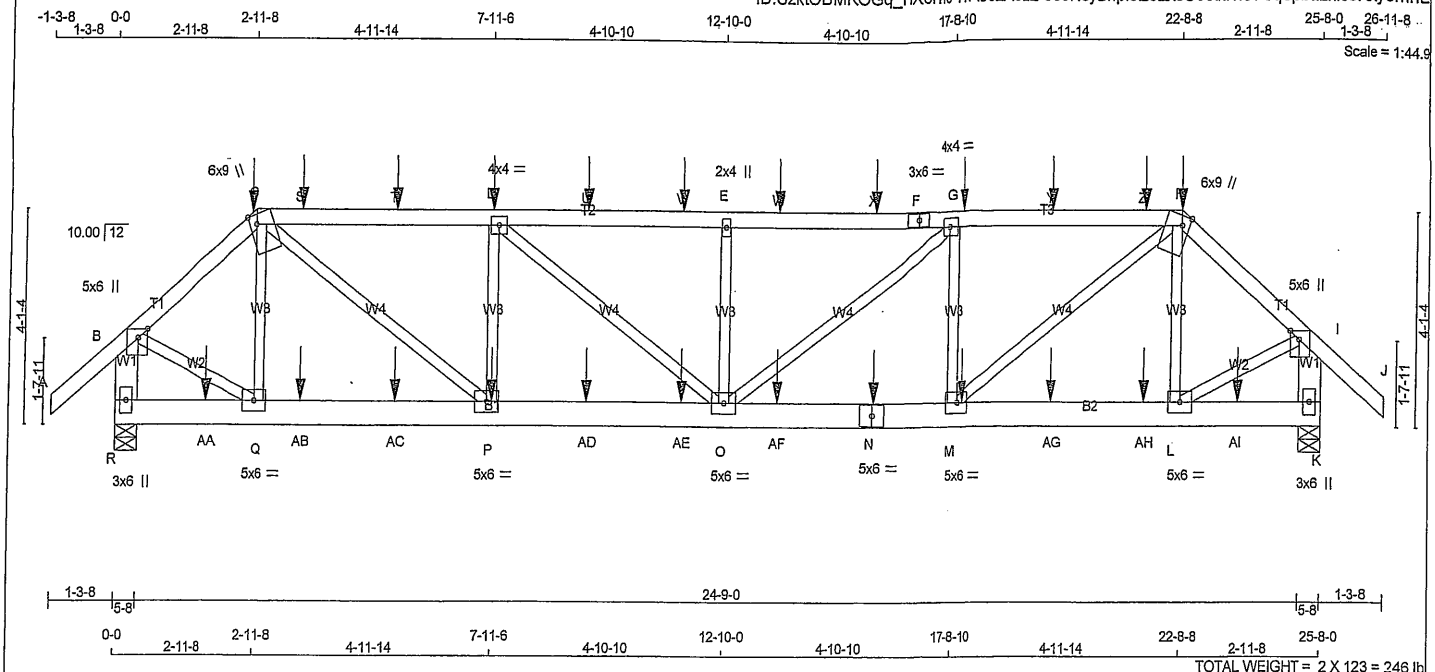


TOTAL WEIGHT = 2 X 123 = 246 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3240	0	3240	0	0	5-8	5-8
K	3262	0	3262	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2549	1634 / 0	472 / 0	0 / 0	0 / 0	443 / 0	0 / 0
K	2565	1645 / 0	474 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

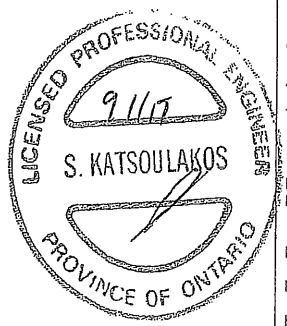
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B		0 / 54	-122.2	-122.2	0.09 (1)		10.00
B- C		-3027 / 0	-122.2	-122.2	0.14 (1)		5.18
C- S		-4796 / 0	-122.2	-122.2	0.49 (1)		3.89
S- T		-4796 / 0	-122.2	-122.2	0.49 (1)		3.89
T- D		-4796 / 0	-122.2	-122.2	0.49 (1)		3.89
D- U		-5521 / 0	-122.2	-122.2	0.53 (1)		3.62
U- V		-5521 / 0	-122.2	-122.2	0.53 (1)		3.62
V- E		-5521 / 0	-122.2	-122.2	0.53 (1)		3.62
E- W		-5521 / 0	-122.2	-122.2	0.53 (1)		3.62
W- X		-5521 / 0	-122.2	-122.2	0.53 (1)		3.62
X- F		-5521 / 0	-122.2	-122.2	0.53 (1)		3.62
F- G		-5521 / 0	-122.2	-122.2	0.53 (1)		3.62
G- Y		-4805 / 0	-122.2	-122.2	0.50 (1)		3.89
Y- Z		-4805 / 0	-122.2	-122.2	0.50 (1)		3.89
Z- H		-4805 / 0	-122.2	-122.2	0.50 (1)		3.89
H- I		-3046 / 0	-122.2	-122.2	0.14 (1)		5.17
I- J		0 / 54	-122.2	-122.2	0.09 (1)		10.00
R- B		-3222 / 0	0.0	0.0	0.12 (1)		7.71
K- I		-3241 / 0	0.0	0.0	0.12 (1)		7.70
R-AA		0 / 0	-28.0	-28.0	0.06 (2)		10.00
AA- Q		0 / 0	-28.0	-28.0	0.06 (2)		10.00
Q-AB		0 / 2298	-28.0	-28.0	0.19 (1)		10.00
AB-AC		0 / 2298	-28.0	-28.0	0.19 (1)		10.00
AC- P		0 / 2298	-28.0	-28.0	0.19 (1)		10.00
P-AD		0 / 4796	-28.0	-28.0	0.37 (1)		10.00
AD-AE		0 / 4796	-28.0	-28.0	0.37 (1)		10.00
AE- O		0 / 4796	-28.0	-28.0	0.37 (1)		10.00
O-AF		0 / 4805	-28.0	-28.0	0.38 (1)		10.00
AF- N		0 / 4805	-28.0	-28.0	0.38 (1)		10.00
N- M		0 / 4805	-28.0	-28.0	0.38 (1)		10.00
M-AG		0 / 2312	-28.0	-28.0	0.19 (1)		10.00
AG-AH		0 / 2312	-28.0	-28.0	0.19 (1)		10.00
AH- L		0 / 2312	-28.0	-28.0	0.19 (1)		10.00
L-AI		0 / 0	-28.0	-28.0	0.06 (2)		10.00
AI- K		0 / 0	-28.0	-28.0	0.06 (2)		10.00

WEBS MEMB.	FR-TO	MAX. FORCE (LBS)	MAX CSI
Q- C		-671 / 0	0.09 (1)
C- P		0 / 3112	0.39 (1)
P- D		-1639 / 0	0.21 (1)
D- O		0 / 912	0.11 (1)
O- E		-880 / 0	0.11 (1)
O- G		0 / 901	0.11 (1)
M- G		-1635 / 0	0.21 (1)
M- H		0 / 3105	0.38 (1)
L- H		-676 / 0	0.09 (1)
B- Q		0 / 2494	0.31 (1)
L- I		0 / 2510	0.31 (1)



DWG NO. TAM45829-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.53 (E-G:1) , BC=0.38 (M-O:1) , WB=0.39 (C-P:1) , SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.42 (N) (INPUT = 1.00)

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CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 171.3 lbs FACTORED DOWN AT 2-11-8, 171.3 lbs FACTORED DOWN AT 22-8-8, 148.2 lbs FACTORED DOWN AT 3-11-4, 147.1 lbs FACTORED DOWN AT 5-11-4, 147.1 lbs FACTORED DOWN AT 7-11-4, 147.1 lbs FACTORED DOWN AT 9-11-4, 147.1 lbs FACTORED DOWN AT 11-11-4, 147.1 lbs FACTORED DOWN AT 13-11-4, 147.1 lbs FACTORED DOWN AT 15-11-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-171	-171	—	FRONT	VERT	TOTAL
D	7-11-4	-147	-147	—	FRONT	VERT	TOTAL
G	17-11-4	-147	-147	—	FRONT	VERT	TOTAL
H	22-8-8	-171	-171	—	FRONT	VERT	TOTAL
M	17-11-4	-40	-70	—	FRONT	VERT	TOTAL
N	15-11-4	-40	-70	—	FRONT	VERT	TOTAL
P	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
S	3-11-4	-148	-148	—	FRONT	VERT	TOTAL
T	5-11-4	-147	-147	—	FRONT	VERT	TOTAL
U	9-11-4	-147	-147	—	FRONT	VERT	TOTAL
V	11-11-4	-147	-147	—	FRONT	VERT	TOTAL
W	13-11-4	-147	-147	—	FRONT	VERT	TOTAL
X	15-11-4	-147	-147	—	FRONT	VERT	TOTAL
Y	19-11-4	-147	-147	—	FRONT	VERT	TOTAL
Z	21-11-4	-161	-161	—	FRONT	VERT	TOTAL
AA	1-11-4	-40	-70	—	FRONT	VERT	TOTAL
AB	3-11-4	-40	-70	—	FRONT	VERT	TOTAL
AC	5-11-4	-40	-70	—	FRONT	VERT	TOTAL
AD	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
AE	11-11-4	-40	-70	—	FRONT	VERT	TOTAL
AF	13-11-4	-40	-70	—	FRONT	VERT	TOTAL
AG	19-11-4	-40	-70	—	FRONT	VERT	TOTAL
AH	21-11-4	-40	-70	—	FRONT	VERT	TOTAL
AI	23-11-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45829-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272357	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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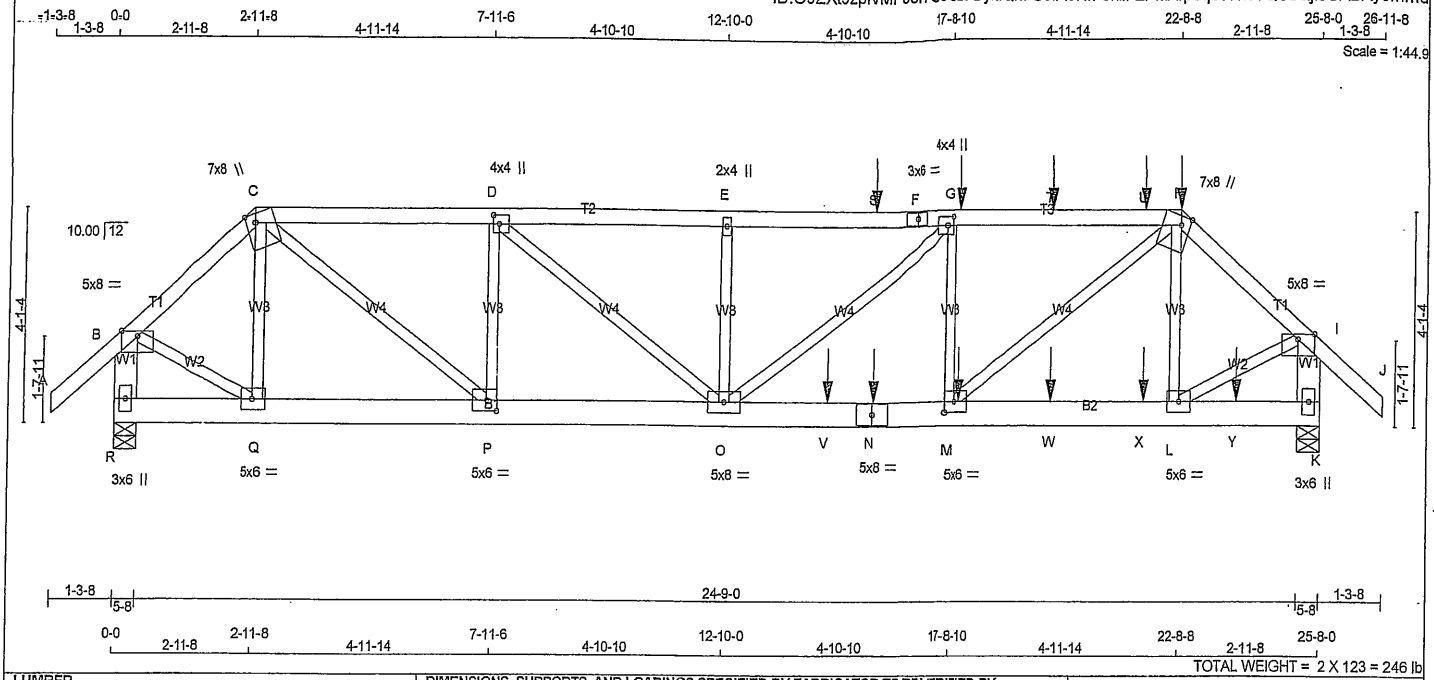

TOTAL WEIGHT = 2 X 123 = 246 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(0.1)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW+t	MT20	4.0	4.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW+t	MT20	4.0	4.0	2.00	1.50
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	5.0	8.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2980	0	2980	0	0	5-8	5-8
K	3812	0	3812	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2303	1546 / 0	381 / 0	0 / 0	0 / 0	376 / 0	0 / 0
K	2969	1953 / 0	516 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00
B- C	-2774 / 0	-122.2	-122.2	0.13 (1)	5.37
C- D	-4748 / 0	-122.2	-122.2	0.35 (1)	4.11
D- E	-6540 / 0	-122.2	-122.2	0.45 (1)	3.49
E- S	-6540 / 0	-122.2	-122.2	0.58 (1)	3.31
S- F	-6540 / 0	-122.2	-122.2	0.58 (1)	3.31
F- G	-6540 / 0	-122.2	-122.2	0.58 (1)	3.31
G- T	-6029 / 0	-122.2	-122.2	0.55 (1)	3.47
T- U	-6029 / 0	-122.2	-122.2	0.55 (1)	3.47
U- H	-6029 / 0	-122.2	-122.2	0.55 (1)	3.47
H- I	-3580 / 0	-122.2	-122.2	0.15 (1)	4.83
I- J	0 / 54	-122.2	-122.2	0.09 (1)	10.00
R- B	-2983 / 0	0.0	0.0	0.11 (1)	7.81
K- I	-3747 / 0	0.0	0.0	0.14 (1)	7.29
R- Q	0 / 0	-28.0	-28.0	0.05 (1)	10.00
Q- P	0 / 2106	-28.0	-28.0	0.20 (1)	10.00
P- O	0 / 4748	-28.0	-28.0	0.50 (1)	10.00
O- V	0 / 6029	-28.0	-28.0	0.94 (1)	10.00
V- N	0 / 6029	-28.0	-28.0	0.94 (1)	10.00
N- M	0 / 6029	-28.0	-28.0	0.94 (1)	10.00
M- W	0 / 2711	-28.0	-28.0	0.35 (1)	10.00
W- X	0 / 2711	-28.0	-28.0	0.35 (1)	10.00
X- L	0 / 2711	-28.0	-28.0	0.35 (1)	10.00
L- Y	0 / 0	-28.0	-28.0	0.04 (3)	10.00
Y- K	0 / 0	-28.0	-28.0	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-625 / 0	0.08 (1)
C- P	0 / 3292	0.41 (1)
P- D	-2034 / 0	0.26 (1)
D- O	0 / 2254	0.28 (1)
O- E	-571 / 0	0.07 (1)
O- G	0 / 642	0.08 (1)
M- G	-1458 / 0	0.18 (1)
M- H	0 / 4133	0.51 (1)
L- H	-1013 / 0	0.13 (1)
B- Q	0 / 2286	0.28 (1)
L- I	0 / 2950	0.37 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	17-11-4	-147	-147	---	FRONT	VERT	TOTAL
H	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
M	17-11-4	-40	-70	---	FRONT	VERT	TOTAL
N	15-11-4	-40	-70	---	FRONT	VERT	TOTAL
S	15-11-4	-147	-147	---	FRONT	VERT	TOTAL
T	19-11-4	-147	-147	---	FRONT	VERT	TOTAL
U	21-11-4	-161	-161	---	FRONT	VERT	TOTAL
V	14-11-8	-1568	-1568	---	FRONT	VERT	TOTAL
W	19-11-4	-40	-70	---	FRONT	VERT	TOTAL
X	21-11-4	-40	-70	---	FRONT	VERT	TOTAL
Y	23-11-4	-40	-70	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.58 (E-G:1) , BC=0.94 (M-O:1) , WB=0.51 (H-M:1) , SSI=0.32 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.81 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9167 S. KATSOULAKOS PROVINCE OF ONTARIO

DVG NO. TAM-05836-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272357	T1Z	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 22-8-8, 147.1 lbs FACTORED DOWN AT 15-11-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 1567.7 lbs FACTORED DOWN AT 14-11-8, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4583617
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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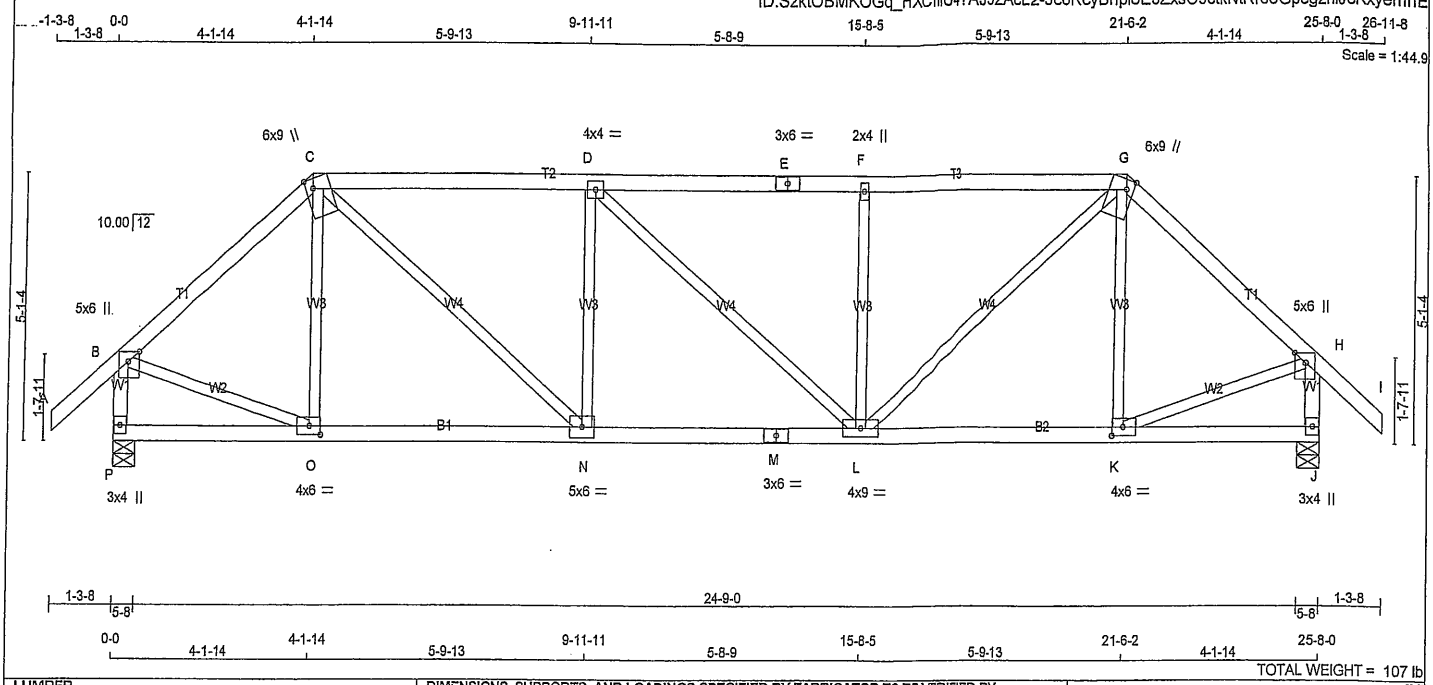


TOTAL WEIGHT = 107 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2097	0	2097	0	0	5-8	5-8
J	2097	0	2097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

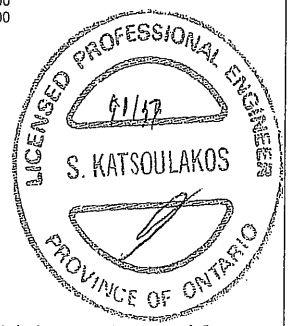
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	-1859 / 0	-122.2	-122.2	0.45 (1)		4.42
C- D	-2538 / 0	-122.2	-122.2	0.75 (1)		3.48
D- E	-2536 / 0	-122.2	-122.2	0.74 (1)		3.48
E- F	-2536 / 0	-122.2	-122.2	0.74 (1)		3.48
F- G	-2536 / 0	-122.2	-122.2	0.74 (1)		3.49
G- H	-1859 / 0	-122.2	-122.2	0.45 (1)		4.42
H- I	0 / 54	-122.2	-122.2	0.17 (1)		10.00
P- B	-2054 / 0	0.0	0.0	0.22 (1)		5.90
J- H	-2054 / 0	0.0	0.0	0.22 (1)		5.90
P- O	0 / 0	-28.0	-28.0	0.17 (2)		10.00
O- N	0 / 1418	-28.0	-28.0	0.32 (1)		10.00
N- M	0 / 2538	-28.0	-28.0	0.50 (1)		10.00
M- L	0 / 2538	-28.0	-28.0	0.50 (1)		10.00
L- K	0 / 1418	-28.0	-28.0	0.32 (1)		10.00
K- J	0 / 0	-28.0	-28.0	0.17 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-279 / 102	0.11 (1)
C- N	0 / 1450	0.33 (1)
N- D	-762 / 0	0.29 (1)
D- L	-2 / 0	0.00 (1)
L- F	-761 / 0	0.29 (1)
L- G	0 / 1448	0.33 (1)
K- G	-278 / 102	0.11 (1)
B- O	0 / 1489	0.34 (1)
K- H	0 / 1490	0.34 (1)



DWG NO. TAM4583017
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.75 (C-D:1) , BC=0.50 (L-N:1) , WB=0.34 (H-K:1) , SSI=0.33 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.71 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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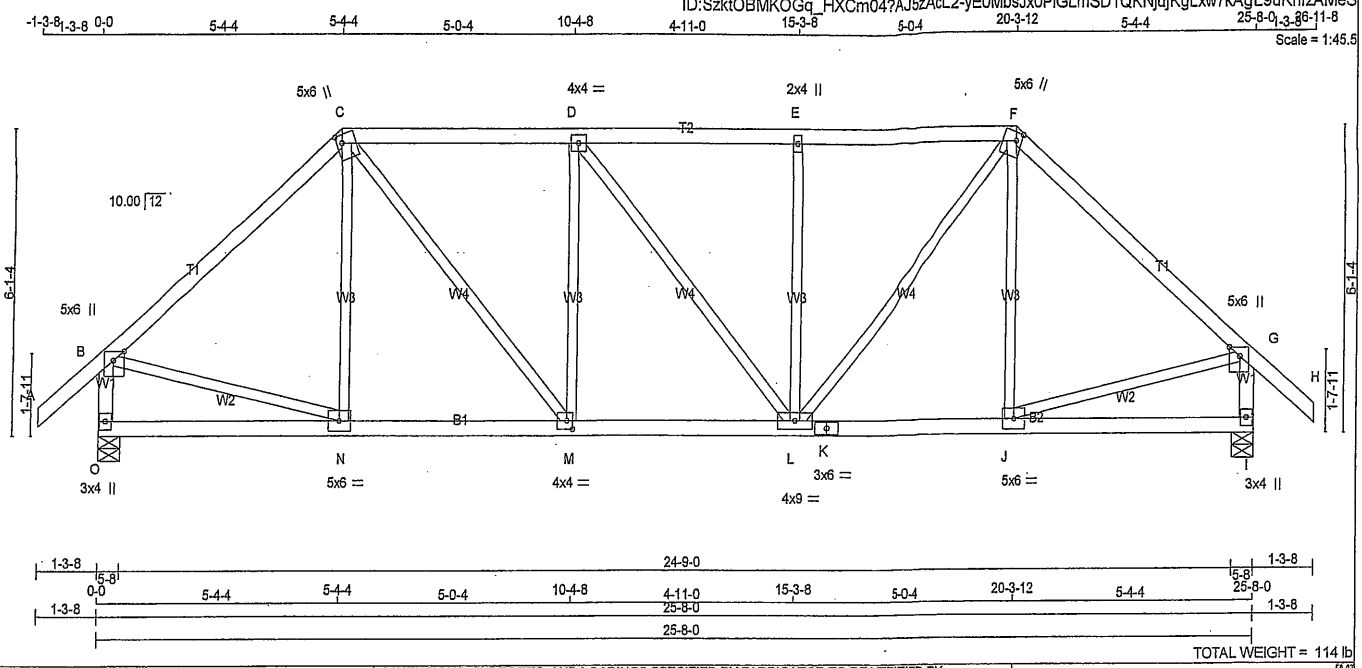

TOTAL WEIGHT = 114 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2097	0	2097	0	0	5-8	3-5
I	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
I	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM TO		
A- B	0 / 54	-122.2 -122.2	0.17 (1)	10.00
B- C	-1864 / 0	-122.2 -122.2	0.80 (1)	3.91
C- D	-2108 / 0	-122.2 -122.2	0.50 (1)	4.11
D- E	-2106 / 0	-122.2 -122.2	0.50 (1)	4.11
E- F	-2105 / 0	-122.2 -122.2	0.50 (1)	4.13
F- G	-1864 / 0	-122.2 -122.2	0.80 (1)	3.91
G- H	0 / 54	-122.2 -122.2	0.17 (1)	10.00
O- B	-2035 / 0	0.0 0.0	0.22 (1)	5.92
I- G	-2035 / 0	0.0 0.0	0.22 (1)	5.92
O- N	0 / 0	-28.0 -28.0	0.19 (3)	10.00
N- M	0 / 1426	-28.0 -28.0	0.34 (2)	10.00
M- L	0 / 2108	-28.0 -28.0	0.40 (1)	10.00
L- K	0 / 1426	-28.0 -28.0	0.34 (2)	10.00
K- J	0 / 1426	-28.0 -28.0	0.34 (2)	10.00
J- I	0 / 0	-28.0 -28.0	0.19 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
N- C	-175 / 159	0.10 (1)
C- M	0 / 1041	0.23 (1)
M- D	-655 / 0	0.38 (1)
D- L	-3 / 0	0.00 (1)
L- E	-654 / 0	0.38 (1)
L- F	0 / 1037	0.23 (1)
J- F	-173 / 159	0.10 (1)
B- N	0 / 1469	0.33 (1)
J- G	0 / 1470	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.80 (F-G:1) , BC=0.40 (L-M:1) , WB=0.38 (D-M:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.38 (G) (INPUT = 1.00)

DWG NO. TAM25783316
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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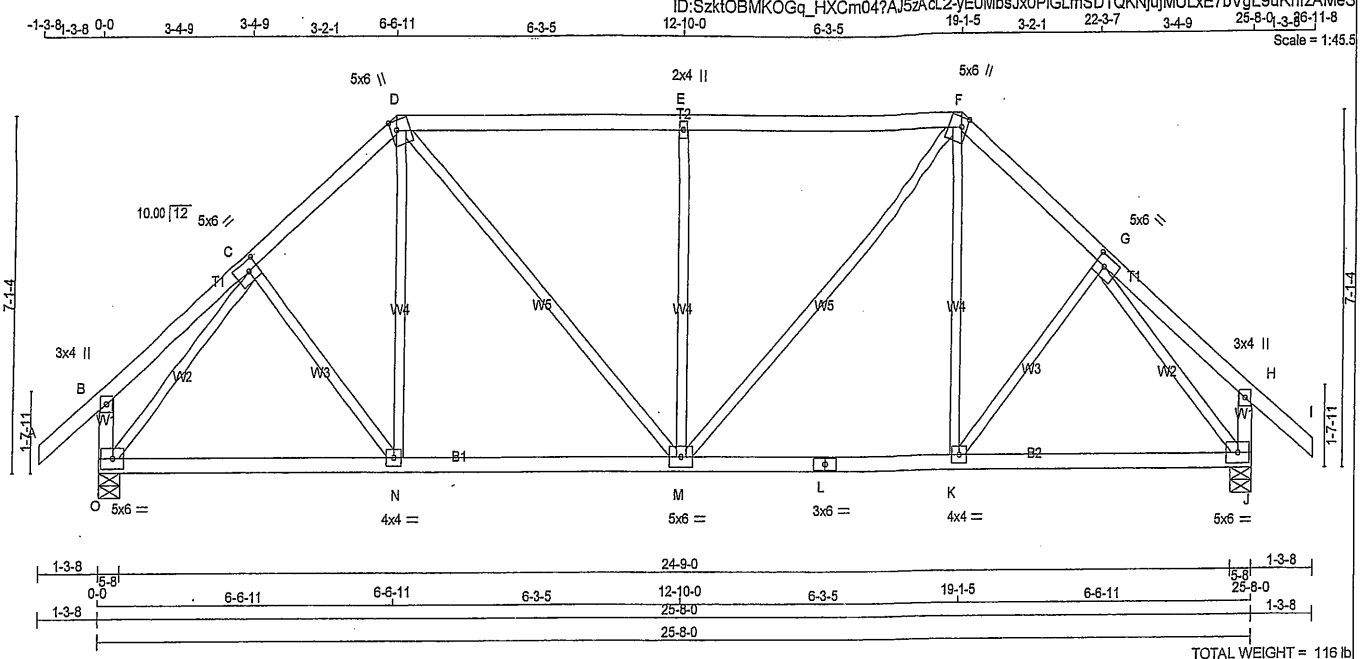


TOTAL WEIGHT = 116 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2097	0	2097	0	0	5-8	2-4
J	2097	0	2097	0	0	5-8	2-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	0 / 27	-122.2	-122.2	0.19 (1)		10.00
C- D	-1859 / 0	-122.2	-122.2	0.19 (1)		4.77
D- E	-1910 / 0	-122.2	-122.2	0.68 (1)		3.96
E- F	-1910 / 0	-122.2	-122.2	0.68 (1)		3.96
F- G	-1859 / 0	-122.2	-122.2	0.19 (1)		4.77
G- H	0 / 27	-122.2	-122.2	0.19 (1)		10.00
H- I	0 / 54	-122.2	-122.2	0.17 (1)		10.00
O- B	-325 / 0	0.0	0.0	0.03 (1)		7.81
J- H	-325 / 0	0.0	0.0	0.03 (1)		7.81
O- N	0 / 1374	-28.0	-28.0	0.43 (2)		10.00
N- M	*0 / 1405*	*-28.0*	*-28.0*	*0.44 (2)*		*10.00*
M- L	0 / 1405	-28.0	-28.0	0.44 (2)		10.00
L- K	0 / 1405	-28.0	-28.0	0.44 (2)		10.00
K- J	0 / 1374	-28.0	-28.0	0.43 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 124	0.03 (3)
N- D	0 / 276	0.06 (2)
D- M	0 / 744	0.17 (1)
M- E	-940 / 0	0.82 (1)
M- F	0 / 744	0.17 (1)
F- K	0 / 276	0.06 (2)
K- G	0 / 124	0.03 (3)
O- C	-2183 / 0	0.94 (1)
G- J	-2183 / 0	0.94 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68 (D-E:1) , BC=0.44 (M-N:2) , WB=0.94 (C-O:1) , SSI=0.37 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

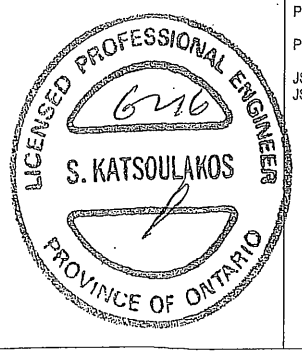
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.54 (G) (INPUT = 1.00)



DWG NO. TAM2578#16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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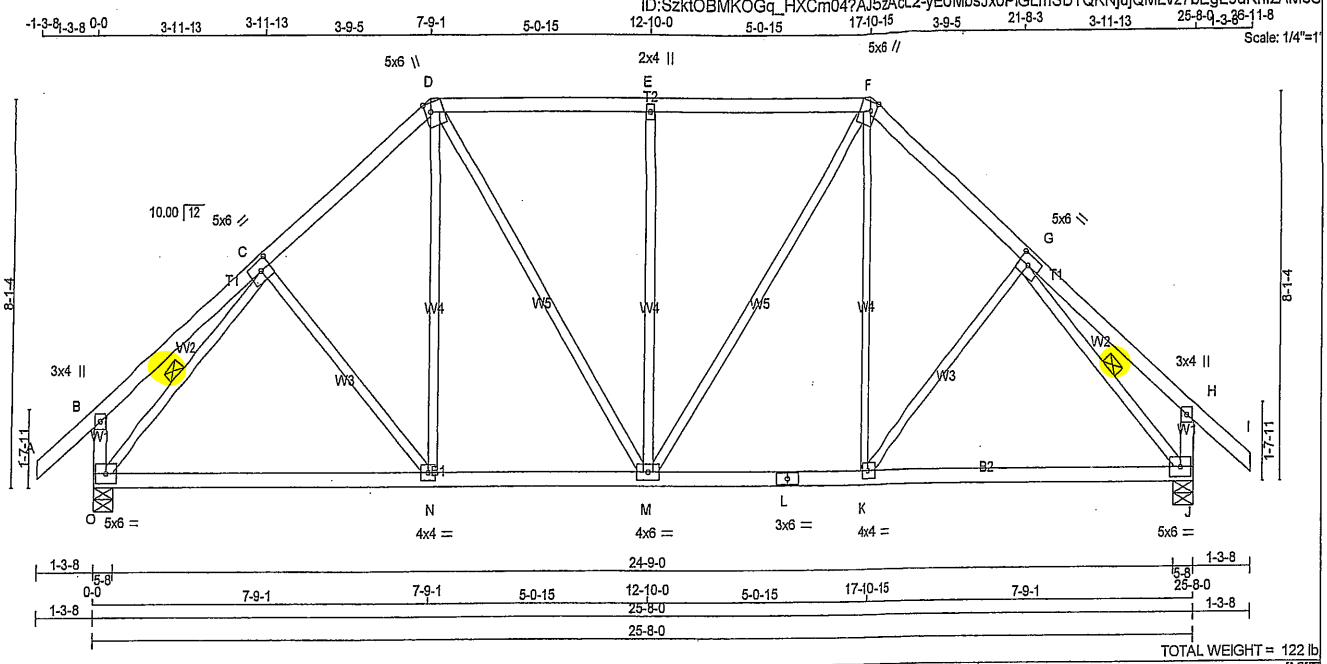

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2097	0	2097	0	0	5-8	2-4
J	2097	0	2097	0	0	5-8	2-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 34	-122.2	-122.2	0.29 (1)	10.00
C- D	-1805 / 0	-122.2	-122.2	0.26 (1)	4.74
D- E	-1631 / 0	-122.2	-122.2	0.43 (1)	4.69
E- F	-1631 / 0	-122.2	-122.2	0.43 (1)	4.69
F- G	-1805 / 0	-122.2	-122.2	0.26 (1)	4.74
G- H	0 / 34	-122.2	-122.2	0.29 (1)	10.00
H- I	*0 / 54*	-122.2	-122.2	*0.17 (1)*	*10.00*
O- B	-351 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-351 / 0	0.0	0.0	0.04 (1)	7.81
O- N	0 / 1413	-28.0	-28.0	0.52 (2)	10.00
N- M	0 / 1361	-28.0	-28.0	0.52 (2)	10.00
M- L	0 / 1361	-28.0	-28.0	0.52 (2)	10.00
L- K	0 / 1361	-28.0	-28.0	0.52 (2)	10.00
K- J	0 / 1413	-28.0	-28.0	0.52 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-84 / 97	0.05 (1)
N- D	0 / 367	0.08 (2)
D- M	0 / 496	0.11 (1)
M- E	-755 / 0	0.96 (1)
M- F	0 / 496	0.11 (1)
K- F	0 / 367	0.08 (2)
K- G	-84 / 97	0.05 (1)
O- C	*-2176 / 0*	*0.55 (1)*
G- J	-2176 / 0	0.55 (1)

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.43 (D-E:1) , BC=0.52 (M-N:2) , WB=0.96 (E-M:1) , SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT*.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

DWG NO. TAM 25785-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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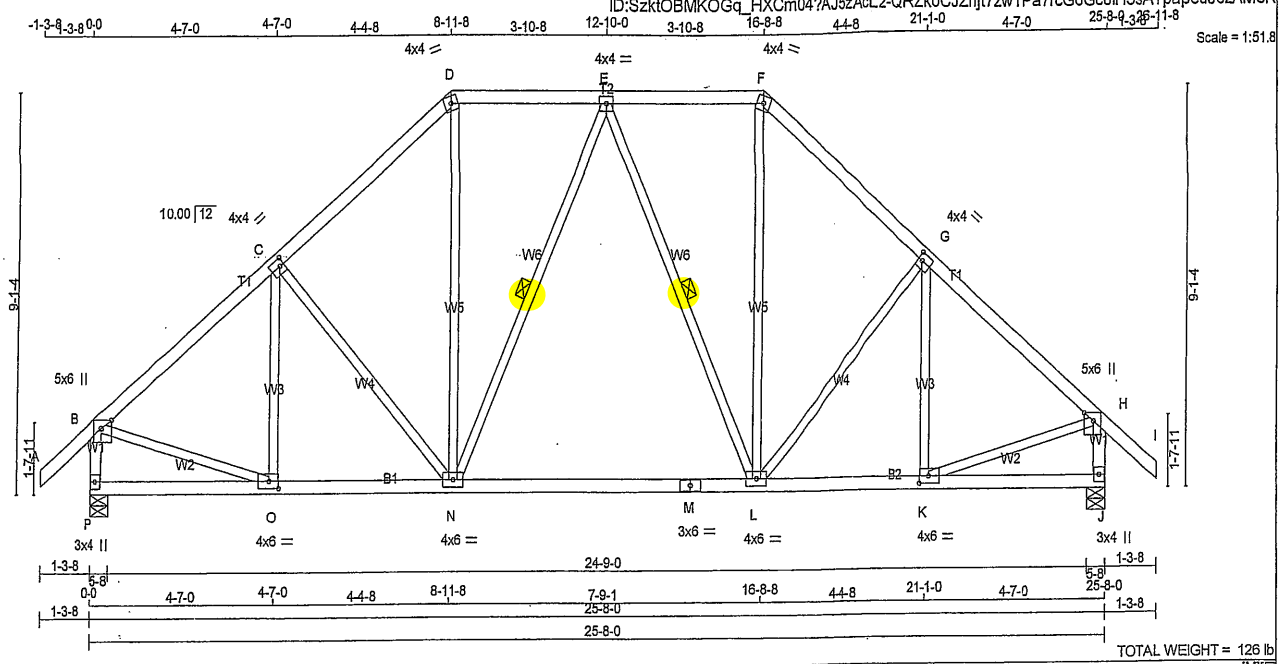


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2097	0	2097	0	0	5-8	3-5
J	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

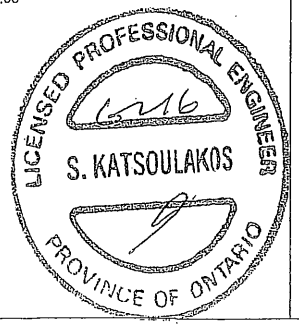
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-1884 / 0	-122.2	-122.2	0.37 (1)	4.53
C- D	-1722 / 0	-122.2	-122.2	0.35 (1)	4.71
D- E	-1295 / 0	-122.2	-122.2	0.24 (1)	5.41
E- F	-1295 / 0	-122.2	-122.2	0.24 (1)	5.41
F- G	-1722 / 0	-122.2	-122.2	0.35 (1)	4.71
G- H	-1884 / 0	-122.2	-122.2	0.37 (1)	4.53
H- I	*0 / 54*	-122.2	-122.2	*0.17 (1)*	*10.00*
P- B	-2039 / 0	0.0	0.0	0.22 (1)	5.92
J- H	-2039 / 0	0.0	0.0	0.22 (1)	5.92
P- O	0 / 0	-28.0	-28.0	0.13 (2)	10.00
O- N	0 / 1479	-28.0	-28.0	0.41 (2)	10.00
N- M	0 / 1417	-28.0	-28.0	0.40 (2)	10.00
M- L	0 / 1417	-28.0	-28.0	0.40 (2)	10.00
L- K	0 / 1479	-28.0	-28.0	0.41 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.13 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-305 / 55	0.14 (1)
C- N	-291 / 0	0.25 (1)
N- D	0 / 688	0.15 (1)
N- E	-308 / 0	0.20 (1)
E- L	-308 / 0	0.20 (1)
L- F	0 / 688	0.15 (1)
L- G	-291 / 0	0.25 (1)
K- G	*-305 / 55*	*0.14 (1)*
B- O	0 / 1538	0.35 (1)
K- H	0 / 1538	0.35 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.37 (B-C:1) , BC=0.41 (N-O:2) , WB=0.35 (H-K:1) , SSI=0.23 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)

DWG NO. TAM 25786-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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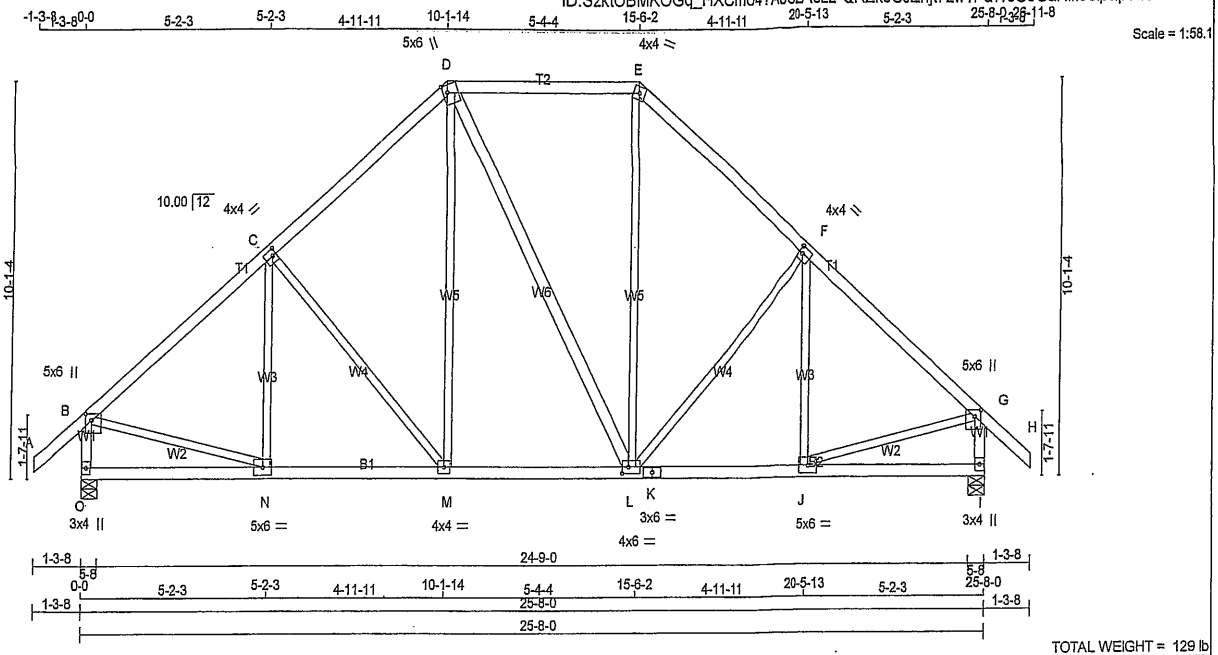


TOTAL WEIGHT = 129 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.00	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0	2.00	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2097	0	2097	0	0	5-8	3-5
I	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
I	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38
C- D	-1627 / 0	-122.2	-122.2	0.45 (1)	4.68
D- E	-1215 / 0	-122.2	-122.2	0.47 (1)	5.19
E- F	-1628 / 0	-122.2	-122.2	0.45 (1)	4.68
F- G	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38
G- H	0 / 54	-122.2	-122.2	0.17 (1)	10.00
O- B	-2037 / 0	0.0	0.0	0.22 (1)	5.92
I- G	-2036 / 0	0.0	0.0	0.22 (1)	5.92
O- N	0 / 0	-28.0	-28.0	0.18 (3)	10.00
N- M	0 / 1497	-28.0	-28.0	0.33 (1)	10.00
M- L	0 / 1215	-28.0	-28.0	0.28 (1)	10.00
L- K	0 / 1496	-28.0	-28.0	0.32 (1)	10.00
K- J	0 / 1496	-28.0	-28.0	0.32 (1)	10.00
J- I	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-219 / 142	0.12 (1)
C- M	-428 / 0	0.49 (1)
M- D	0 / 457	0.10 (1)
D- L	0 / 2	0.00 (1)
L- E	0 / 460	0.10 (1)
L- F	-426 / 0	0.49 (1)
J- F	-221 / 140	0.12 (1)
B- N	0 / 1543	0.35 (1)
J- G	0 / 1543	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.48 (B-C:1) , BC=0.33 (M-N:1) , WB=0.49 (C-M:1) , SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT*.

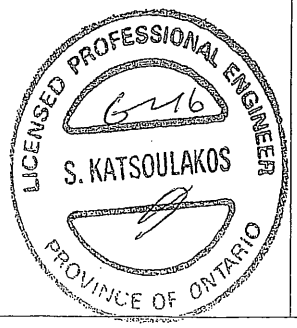
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)



DWG NO. TAM 25787 16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T8	13	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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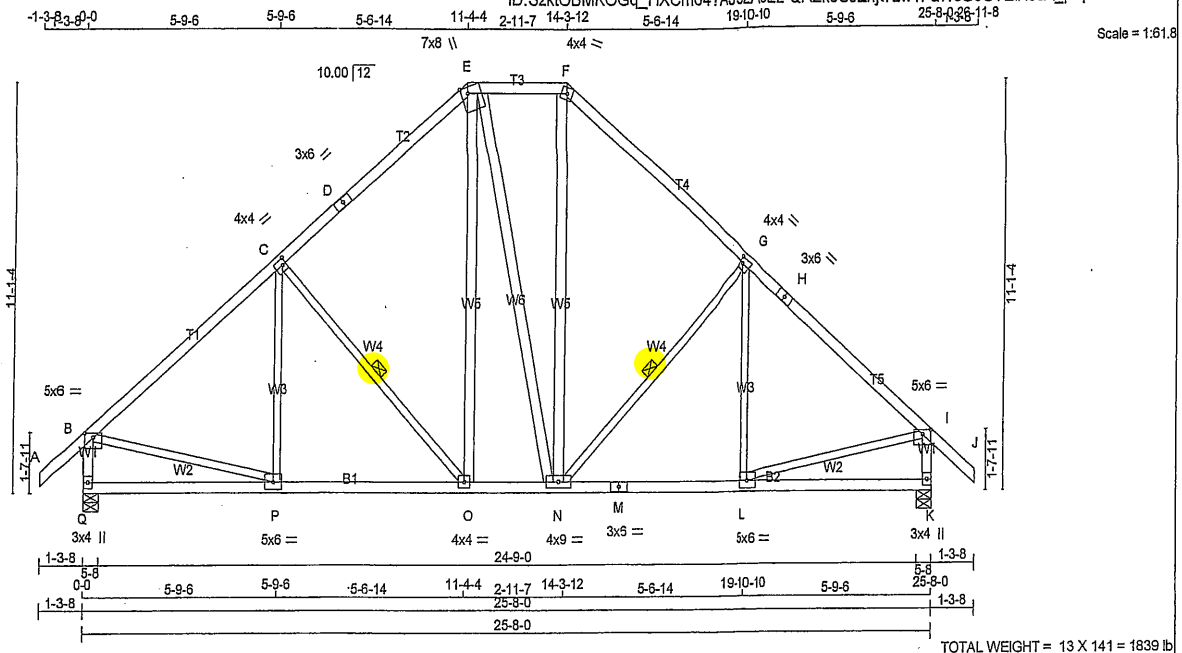


TOTAL WEIGHT = 13 X 141 = 1839 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TS-t	MT20	3.0	6.0		
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2097	0	2097	0	0	5-8	3-5
K	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
K	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	P- C	-153 / 204	0.10 (1)
B- C	-1904 / 0	-122.2	-122.2	0.62 (1)	4.17	C- O	-551 / 0	0.26 (1)
C- D	-1527 / 0	-122.2	-122.2	0.56 (1)	4.61	O- E	0 / 505	0.08 (1)
D- E	-1527 / 0	-122.2	-122.2	0.56 (1)	4.61	E- N	0 / 5	0.00 (1)
E- F	-1136 / 0	-122.2	-122.2	0.15 (1)	5.80	N- F	0 / 511	0.08 (1)
F- G	-1529 / 0	-122.2	-122.2	0.56 (1)	4.62	N- G	-547 / 0	0.26 (1)
G- H	-1903 / 0	-122.2	-122.2	0.61 (1)	4.17	L- G	-157 / 201	0.11 (1)
H- I	*-1903 / 0*	*-122.2*	*-122.2*	*0.61 (1)*	*4.17*	*B- P*	*0 / 1542*	*0.35 (1)*
I- J	0 / 54	-122.2	-122.2	0.17 (1)	10.00	L- I	0 / 1541	0.35 (1)
Q- B	-2032 / 0	0.0	0.0	0.22 (1)	5.93			
K- I	-2031 / 0	0.0	0.0	0.22 (1)	5.93			
Q- P	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
P- O	0 / 1504	-28.0	-28.0	0.39 (2)	10.00			
O- N	0 / 1135	-28.0	-28.0	0.25 (1)	10.00			
N- M	0 / 1503	-28.0	-28.0	0.39 (2)	10.00			
M- L	0 / 1503	-28.0	-28.0	0.39 (2)	10.00			
L- K	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.62 (B-C:1) , BC=0.39 (O-P:2) , WB=0.35 (B-P:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

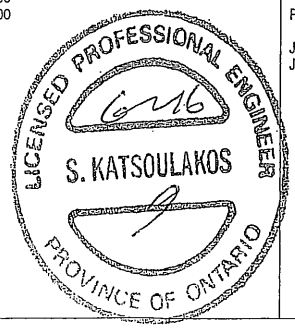
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.45 (M) (INPUT = 1.00)



DWG NO. TAM257BB-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T9	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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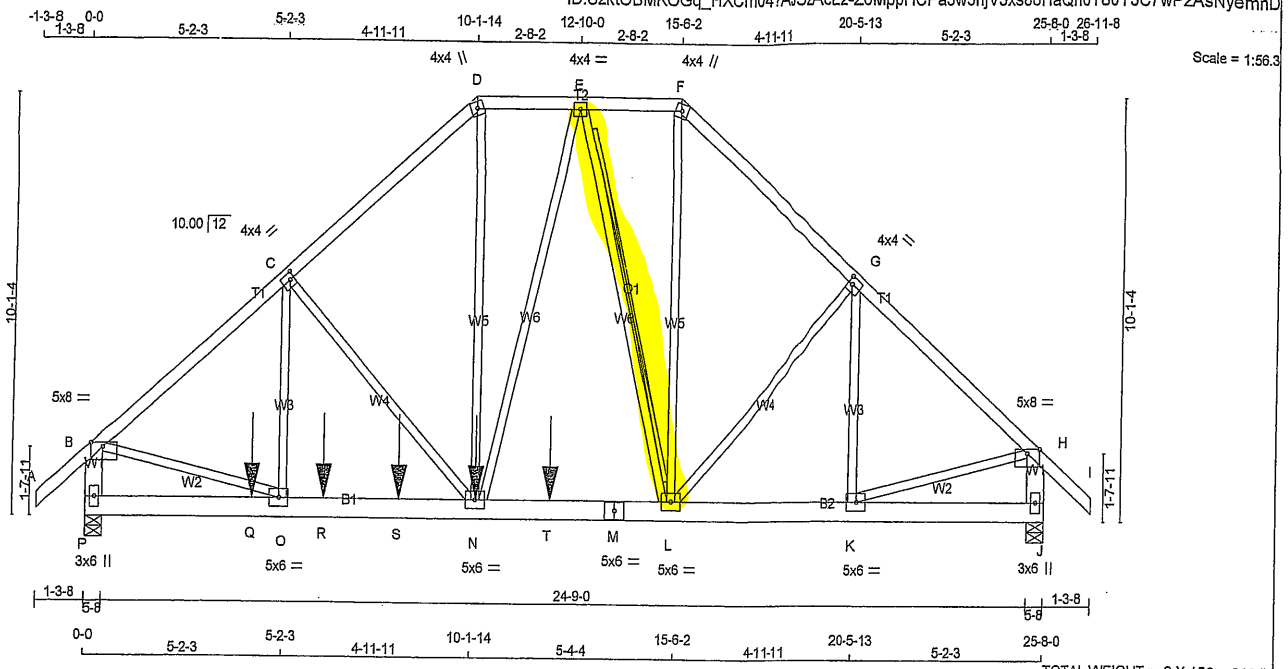

TOTAL WEIGHT = 2 X 150 = 299 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
P- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- M	2	12	SIDE(183.1)
M- J	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	3497	0	3497	0	0	5-8	5-8
J	2941	0	2941	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	2661	1861 / 0	393 / 0	0 / 0	0 / 0	408 / 0	0 / 0
J	2251	1551 / 0	347 / 0	0 / 0	0 / 0	353 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00	O- C	0 / 185	0.02 (3)
B- C	-3597 / 0	-122.2	-122.2	0.40 (1)	4.58	C- N	-598 / 0	0.33 (1)
C- D	-3170 / 0	-122.2	-122.2	0.37 (1)	4.83	N- D	0 / 1606	0.20 (1)
D- E	-2411 / 0	-122.2	-122.2	0.09 (1)	5.72	L- F	0 / 1395	0.17 (1)
E- F	-2152 / 0	-122.2	-122.2	0.08 (1)	5.97	L- G	-215 / 0	0.12 (1)
F- G	-2837 / 0	-122.2	-122.2	0.36 (1)	5.06	K- G	-498 / 64	0.13 (1)
G- H	-2925 / 0	-122.2	-122.2	0.37 (1)	4.99	B- O	0 / 2875	0.36 (1)
H- I	0 / 54	-122.2	-122.2	0.09 (1)	10.00	K- H	0 / 2345	0.29 (1)
P- B	-3406 / 0	0.0	0.0	0.13 (1)	7.55	N- E	0 / 341	0.04 (1)
J- H	-2854 / 0	0.0	0.0	0.11 (1)	7.81	E- L	-670 / 0	0.38 (1)
P- Q	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
Q- O	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
O- R	0 / 2798	-28.0	-28.0	0.42 (1)	10.00			
R- S	0 / 2798	-28.0	-28.0	0.42 (1)	10.00			
S- N	0 / 2798	-28.0	-28.0	0.42 (1)	10.00			
N- T	0 / 2324	-28.0	-28.0	0.46 (1)	10.00			
T- M	0 / 2324	-28.0	-28.0	0.46 (1)	10.00			
M- L	0 / 2324	-28.0	-28.0	0.46 (1)	10.00			
L- K	0 / 2283	-28.0	-28.0	0.29 (1)	10.00			
K- J	0 / 0	-28.0	-28.0	0.05 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	10-3-4	-291	-291	---	BACK	VERT	TOTAL
Q	4-4-4	-291	-291	---	BACK	VERT	TOTAL
R	6-3-4	-291	-291	---	BACK	VERT	TOTAL
S	8-3-4	-291	-291	---	BACK	VERT	TOTAL
T	12-2-8	-1079	-1079	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.40 (B-C:1) , BC=0.46 (L-N:1) , WB=0.38 (E-L:1) , SSI=0.26 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.86 (D) (INPUT = 0.90)
LSI METAL= 0.39 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-75831-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T9	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 291.2 lbs FACTORED DOWN AT 4-4-4, 291.2 lbs FACTORED DOWN AT 6-3-4, 291.2 lbs FACTORED DOWN AT 8-3-4, AND 291.2 lbs FACTORED DOWN AT 10-3-4, AND 1079.1 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9147
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45831-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272340	T9Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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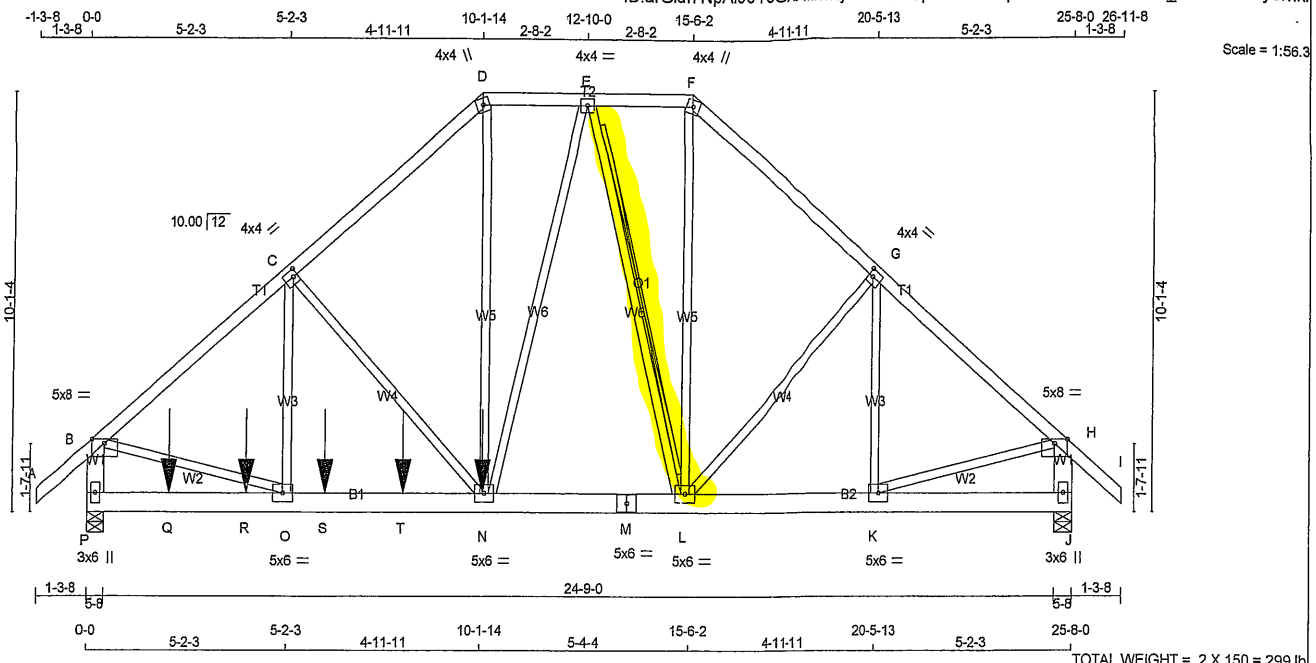

TOTAL WEIGHT = 2 X 150 = 299 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
P- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- M	2	12	SIDE(183.1)
M- J	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
N- D	1	6	SIDE(141.1)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	3774	0	3774	0	0	5-8	5-8
J	2827	0	2827	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	2870	2010 / 0	421 / 0	0 / 0	0 / 0	439 / 0	0 / 0
J	2168	1486 / 0	339 / 0	0 / 0	0 / 0	343 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00		O- C	0 / 308	0.04 (2)
B- C	-3773 / 0	-122.2	-122.2	0.41 (1)	4.49		C- N	-825 / 0	0.46 (1)
C- D	-3146 / 0	-122.2	-122.2	0.37 (1)	4.85		N- D	0 / 1592	0.20 (1)
D- E	-2393 / 0	-122.2	-122.2	0.09 (1)	5.73		L- F	0 / 1270	0.16 (1)
E- F	-1999 / 0	-122.2	-122.2	0.08 (1)	6.14		L- G	-303 / 0	0.17 (1)
F- G	-2638 / 0	-122.2	-122.2	0.35 (1)	5.21		K- G	-382 / 95	0.10 (1)
G- H	-2805 / 0	-122.2	-122.2	0.36 (1)	5.08		B- O	0 / 3014	0.37 (1)
H- I	0 / 54	-122.2	-122.2	0.09 (1)	10.00		K- H	0 / 2251	0.28 (1)
P- B	-3551 / 0	0.0	0.0	0.13 (1)	7.43		N- E	0 / 603	0.07 (1)
J- H	-2756 / 0	0.0	0.0	0.10 (1)	7.81		E- L	-936 / 0	0.53 (1)
P- Q	0 / 0	-28.0	-28.0	0.15 (1)	10.00				
Q- R	0 / 0	-28.0	-28.0	0.15 (1)	10.00				
R- O	0 / 0	-28.0	-28.0	0.15 (1)	10.00				
O- S	0 / 2933	-28.0	-28.0	0.34 (1)	10.00				
S- T	0 / 2933	-28.0	-28.0	0.34 (1)	10.00				
T- N	0 / 2933	-28.0	-28.0	0.34 (1)	10.00				
N- M	0 / 2238	-28.0	-28.0	0.19 (1)	10.00				
M- L	0 / 2238	-28.0	-28.0	0.19 (1)	10.00				
L- K	0 / 2191	-28.0	-28.0	0.16 (1)	10.00				
K- J	0 / 0	-28.0	-28.0	0.05 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	10-2-8	-1243	-1243	---	BACK	VERT	TOTAL
Q	2-1-12	-291	-291	---	BACK	VERT	TOTAL
R	4-1-12	-291	-291	---	BACK	VERT	TOTAL
S	6-1-12	-291	-291	---	BACK	VERT	TOTAL
T	8-1-12	-291	-291	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.41 (B-C:1) , BC=0.34 (N-O:1) , WB=0.53 (E-L:1) , SSI=0.17 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(GRIP= 0.85 (D) (INPUT = 0.90)
(METAL= 0.37 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9/11/17 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM45839-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272340	T9Z	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 291.2 lbs FACTORED DOWN AT 2-1-12, 291.2 lbs FACTORED DOWN AT 4-1-12, 291.2 lbs FACTORED DOWN AT 6-1-12, AND 291.2 lbs FACTORED DOWN AT 8-1-12, AND 1242.7 lbs FACTORED DOWN AT 10-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45839-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T10	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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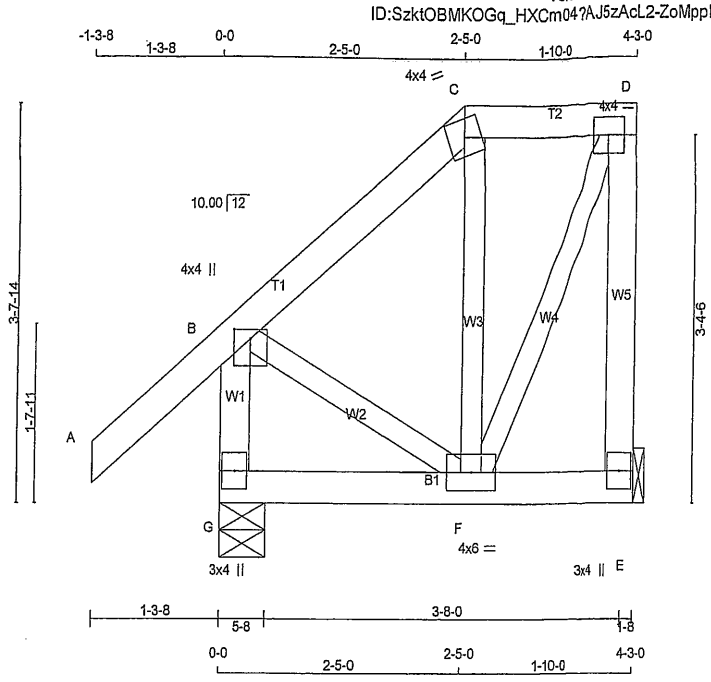

TOTAL WEIGHT = 4 X 25 = 99 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	488	0	488	0	0	5-8	5-8
E	319	0	319	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	364	268 / 0	45 / 0	0 / 0	0 / 0	51 / 0	0 / 0
E	250	163 / 0	45 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-457 / 0	0.0	0.0	0.05	(1)	7.81	F- C	-174 / 21	0.04 (1)
A- B	0 / 54	-122.2	-122.2	0.17	(1)	10.00	B- F	0 / 116	0.03 (1)
B- C	-134 / 0	-122.2	-122.2	0.12	(1)	6.25	F- D	0 / 213	0.05 (1)
E- D	-300 / 0	0.0	0.0	0.06	(1)	7.81			
C- D	-97 / 0	-122.2	-122.2	0.07	(1)	6.25			
G- F	0 / 0	-28.0	-28.0	0.04	(2)	10.00			
F- E	0 / 0	-28.0	-28.0	0.04	(3)	10.00			

DWG NO. TAM45832-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.17 (A-B:1) , BC=0.04 (F-G:2) , WB=0.05 (D-F:1) , SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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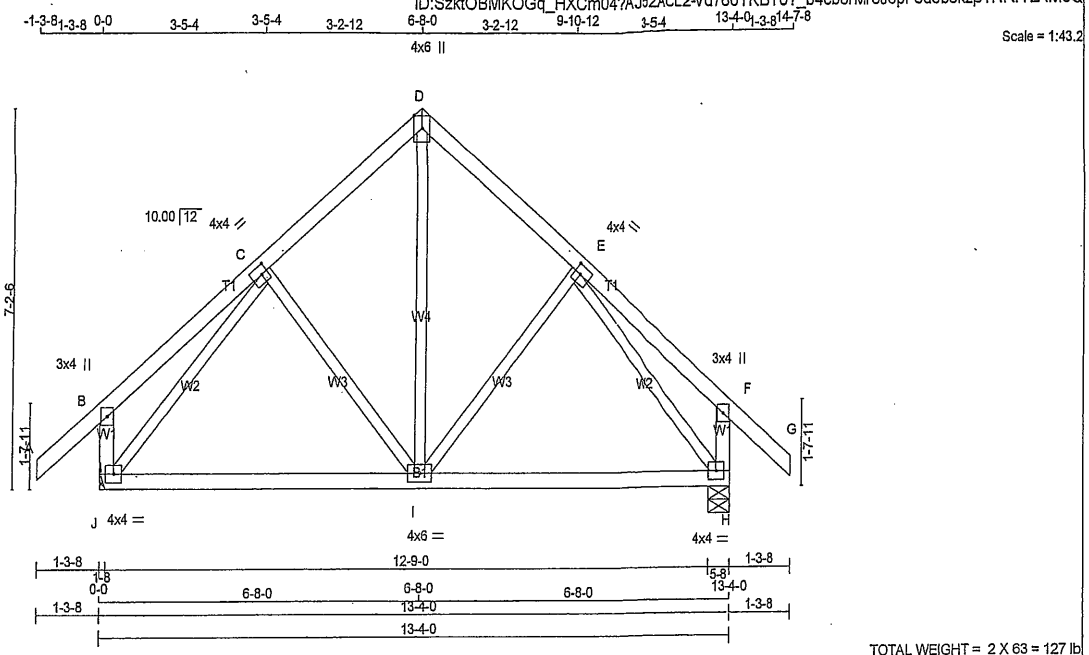

TOTAL WEIGHT = 2 X 63 = 127 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1171	0	1171	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	1171	0	1171	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	897	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0
H	897	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 30	-122.2	-122.2	0.21 (1)	10.00
C- D	-690 / 0	-122.2	-122.2	0.17 (1)	6.25
D- E	-690 / 0	-122.2	-122.2	0.17 (1)	6.25
E- F	0 / 30	-122.2	-122.2	0.21 (1)	10.00
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00
J- B	-325 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-325 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 631	-28.0	-28.0	0.42 (2)	10.00
I- H	0 / 631	-28.0	-28.0	0.42 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 527	0.12 (1)
I- E	-193 / 39	0.09 (1)
C- I	-193 / 39	0.09 (1)
J- C	-999 / 0	0.44 (1)
E- H	-999 / 0	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.21 (B-C:1) , BC=0.42 (H-I:2) , WB=0.44 (C-J:1) , SSI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 1.00)

DWG NO. TAM 257946
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T11S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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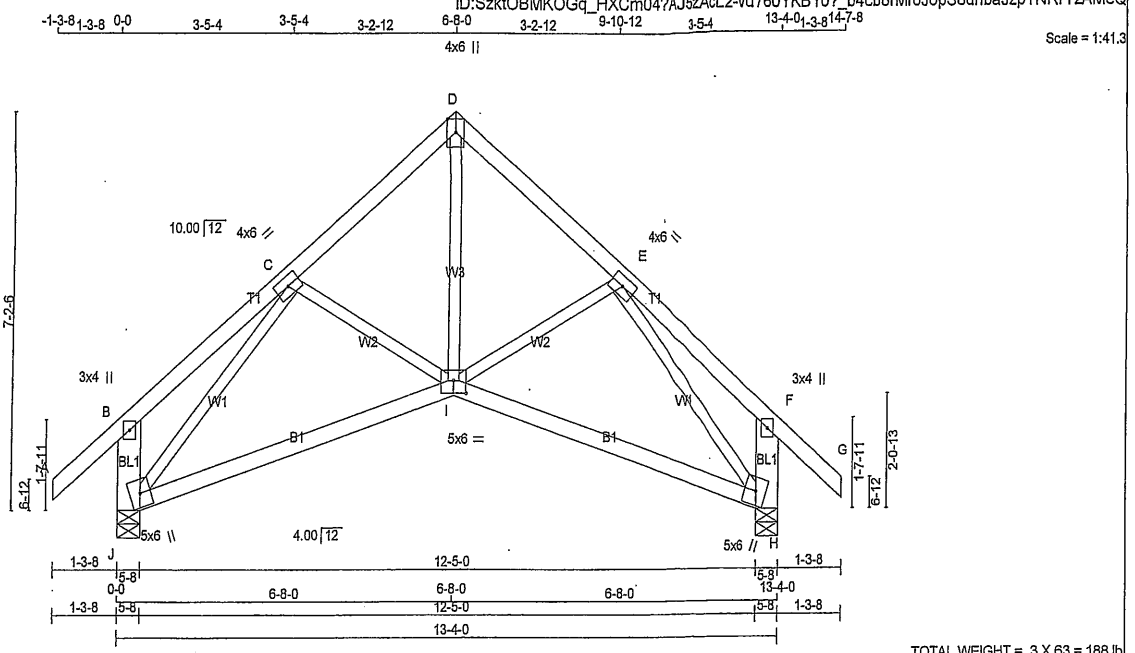

TOTAL WEIGHT = 3 X 63 = 188 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMK+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMK+p	MT20	3.0	4.0		
H	BWKM1*+m	MT20	5.0	6.0		
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BWKM1*+m	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1171	0	1171	0	0	5-8	1-8
H	1171	0	1171	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	897	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0
H	897	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 59	-122.2	-122.2	0.20 (1)	10.00	I- D	0 / 845	0.19 (1)
B- C	-13 / 0	-122.2	-122.2	0.14 (1)	6.25	I- E	-119 / 55	0.03 (1)
C- D	-941 / 0	-122.2	-122.2	0.16 (1)	6.21	C- I	-119 / 55	0.03 (1)
D- E	-941 / 0	-122.2	-122.2	0.16 (1)	6.21	J- C	-1250 / 0	0.53 (1)
E- F	-13 / 0	-122.2	-122.2	0.14 (1)	6.25	E- H	-1250 / 0	0.53 (1)
F- G	0 / 59	-122.2	-122.2	0.20 (1)	10.00			
J- B	-365 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-365 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 849	-28.0	-28.0	0.44 (2)	10.00			
I- H	0 / 849	-28.0	-28.0	0.44 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 665 (0.24")

CSI: TC=0.20 (F-G:1) , BC=0.44 (I-J:2) , WB=0.53 (C-J:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

DWG NO. TAM25792-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T12	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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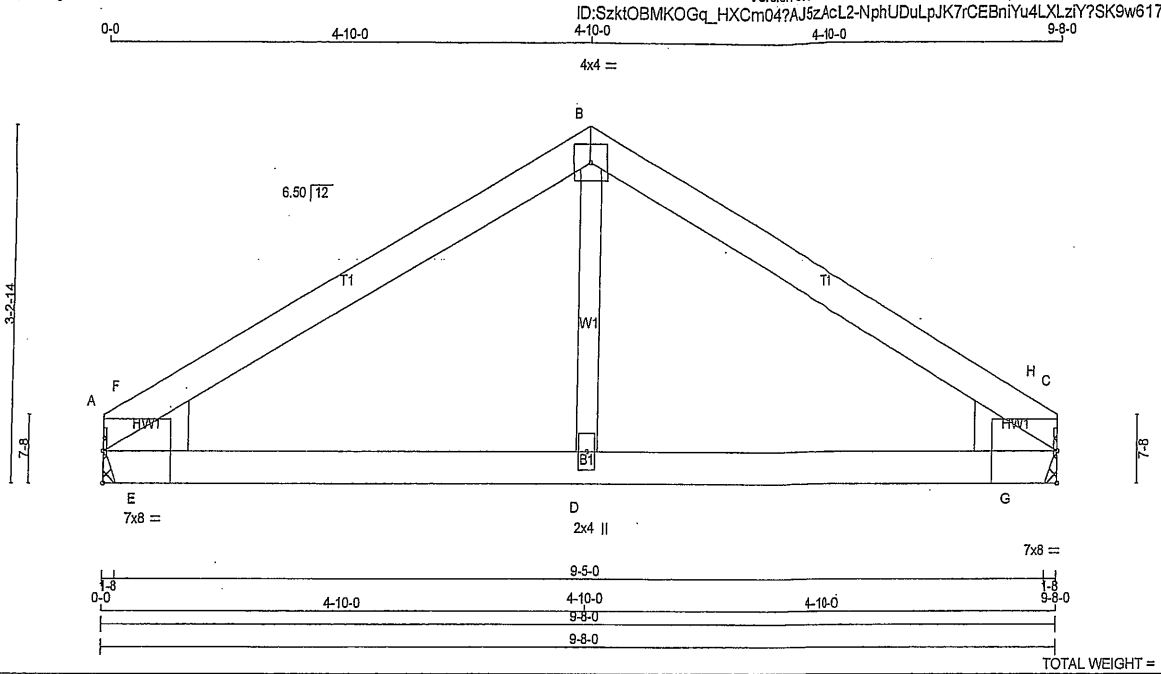

TOTAL WEIGHT = 3 X 29 = 87 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMH1-I	MT20	7.0	8.0		
B	TTW-p	MT20	4.0	4.0		
C	TMBMH1-I	MT20	7.0	8.0		
D	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
A	726	0	726	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8		2x6 L
C	726	0	726	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8		2x6 R

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	568	370 / 0	101 / 0	0 / 0	0 / 0	97 / 0	0 / 0
C	568	370 / 0	101 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- F	-728 / 0	-122.2	-122.2	0.18 (1)		6.25
F- B	-750 / 0	-122.2	-122.2	0.30 (1)		6.25
B- H	-750 / 0	-122.2	-122.2	0.30 (1)		6.25
H- C	-728 / 0	-122.2	-122.2	0.18 (1)		6.25
A- E	0 / 644	-28.0	-28.0	0.25 (1)		10.00
E- D	0 / 644	-28.0	-28.0	0.28 (1)		10.00
D- G	0 / 644	-28.0	-28.0	0.28 (1)		10.00
G- C	0 / 644	-28.0	-28.0	0.25 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- B	0 / 276	0.06 (2)
E- F	-257 / 196	0.00 (1)
G- H	-257 / 196	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.30 (B-F:1) , BC=0.28 (D-E:1) , WB=0.06 (B-D:2) , SSI=0.22 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.15 (C) (INPUT = 1.00)

DWG NO. TAM 25793-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	G12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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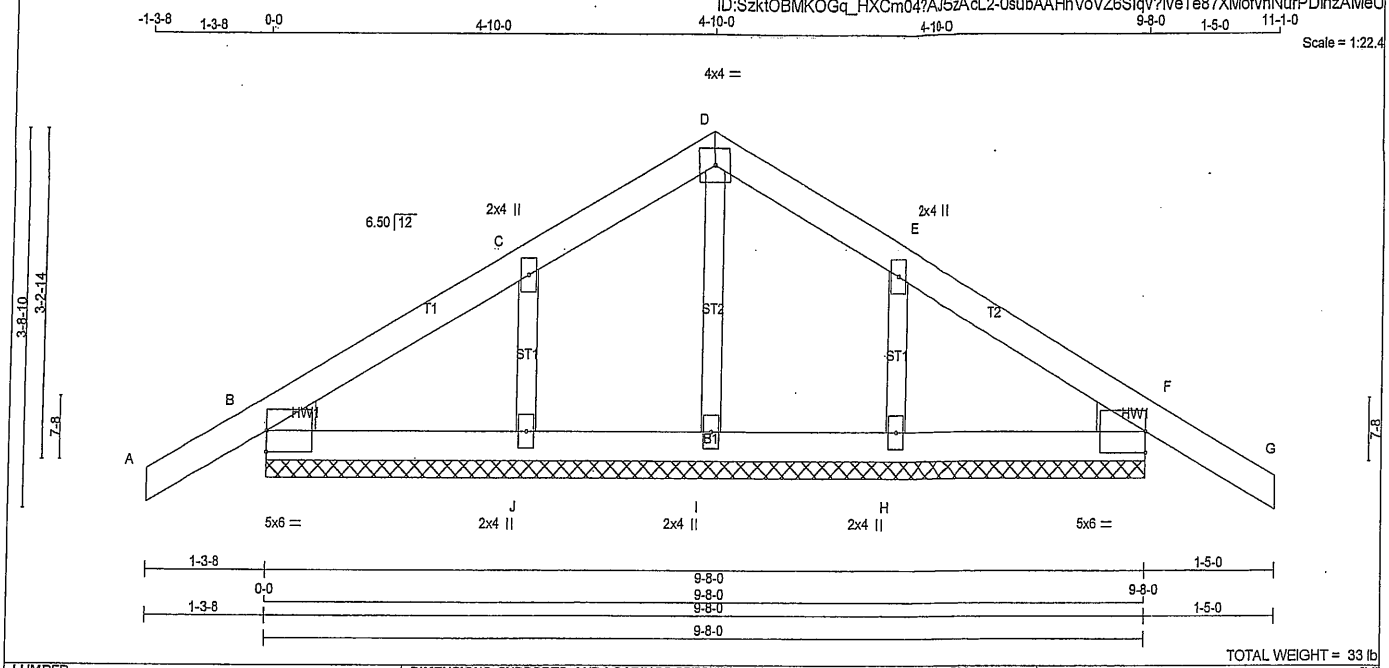

TOTAL WEIGHT = 33 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBM1-I	MT20	5.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBM1-I	MT20	5.0	6.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
2x4 R
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 36	-122.2	-122.2	0.13 (1)	10.00	I- D	-138 / 0	0.03 (1)
B- C	-19 / 0	-122.2	-122.2	0.14 (1)	6.25	J- C	-379 / 0	0.06 (1)
C- D	-42 / 0	-122.2	-122.2	0.14 (1)	6.25	H- E	-379 / 0	0.06 (1)
D- E	-42 / 0	-122.2	-122.2	0.14 (1)	6.25			
E- F	-19 / 0	-122.2	-122.2	0.14 (1)	6.25			
F- G	0 / 39	-122.2	-122.2	0.16 (1)	10.00			
B- J	0 / 31	-28.0	-28.0	0.06 (2)	10.00			
J- I	0 / 17	-28.0	-28.0	0.06 (2)	10.00			
I- H	0 / 17	-28.0	-28.0	0.06 (2)	10.00			
H- F	0 / 31	-28.0	-28.0	0.06 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16 (F-G:1) , BC=0.06 (F-H:2) , WB=0.06 (E-H:1) , SSI=0.15 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR =, 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

DWG NO. TAM-25794-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T20	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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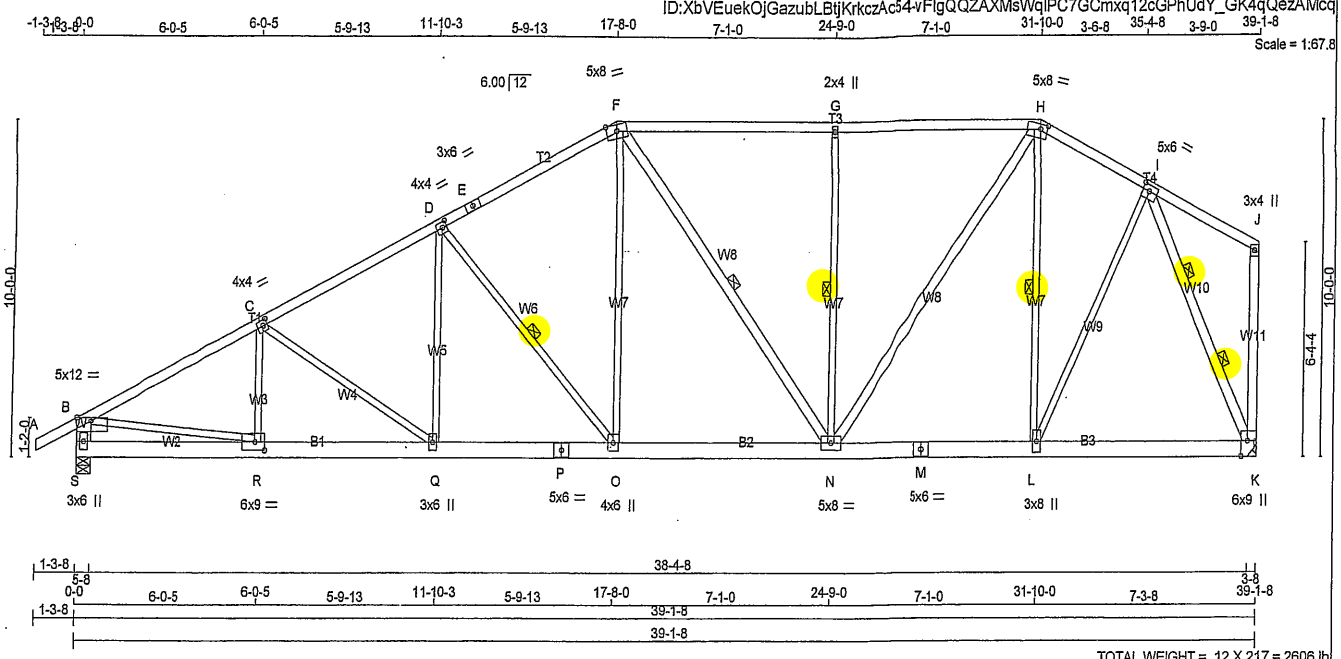


TOTAL WEIGHT = 12 X 217 = 2606 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	12.0	Edge	5.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	2.75
I	TMWW-t	MT20	5.0	6.0	2.25	2.75
J	TMV+p	MT20	3.0	4.0		
K	BMVW1+t	MT20	6.0	9.0	Edge	2.50
L	BMWW+t	MT20	3.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	3.0	6.0		
R	BMWW-t	MT20	6.0	9.0	3.00	4.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	3104	0	3104	0	0	5-8	4-9
K	2939	0	2939	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2410	1600 / 0	411 / 0	0 / 0	0 / 0	399 / 0	0 / 0
K	2298	1496 / 0	411 / 0	0 / 0	0 / 0	391 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-O, F-N, G-N, H-L.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

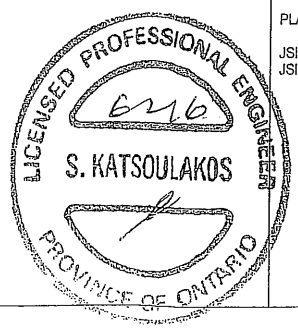
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 37	-122.2	-122.2	0.16 (1)	10.00
B- C	-4496 / 0	-122.2	-122.2	0.96 (1)	2.49
C- D	-4124 / 0	-122.2	-122.2	0.78 (1)	2.85
D- E	-3343 / 0	-122.2	-122.2	0.71 (1)	3.19
E- F	-3343 / 0	-122.2	-122.2	0.71 (1)	3.19
F- G	-2861 / 0	-122.2	-122.2	0.98 (1)	2.60
G- H	-2861 / 0	-122.2	-122.2	0.98 (1)	2.60
H- I	-2085 / 0	-122.2	-122.2	0.26 (1)	4.46
I- J	0 / 24	-122.2	-122.2	0.28 (1)	10.00
S- B	-3012 / 0	0.0	0.0	0.19 (1)	6.07
K- J	-171 / 0	0.0	0.0	0.13 (1)	7.81
S- R	0 / 0	-28.0	-28.0	0.12 (2)	10.00
R- Q	0 / 4048	-28.0	-28.0	0.56 (1)	10.00
Q- P	0 / 3689	-28.0	-28.0	0.50 (1)	10.00
P- O	0 / 3689	-28.0	-28.0	0.50 (1)	10.00
O- N	0 / 2972	-28.0	-28.0	0.42 (1)	10.00
N- M	0 / 1831	-28.0	-28.0	0.32 (1)	10.00
M- L	0 / 1831	-28.0	-28.0	0.32 (1)	10.00
L- K	0 / 1269	-28.0	-28.0	0.27 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-376 / 118	0.09 (1)
C- Q	-428 / 0	0.38 (1)
Q- D	0 / 428	0.10 (2)
D- O	-1099 / 0	0.60 (1)
O- F	0 / 1019	0.23 (1)
F- N	-188 / 0	0.15 (1)
N- G	-1070 / 0	0.70 (1)
N- H	0 / 1737	0.28 (1)
L- H	-968 / 0	0.63 (1)
L- I	0 / 1334	0.30 (1)
B- R	0 / 4081	0.92 (1)
I- K	-2970 / 0	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.98 (F-G:1) , BC=0.56 (Q-R:1) , WB=0.92 (B-R:1) , SSI=0.42 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT *RESPONSIBLE FOR QUALITY CONTROL IN* THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.67 (R) (INPUT = 1.00)

DWG NO. TAM25798-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	G20	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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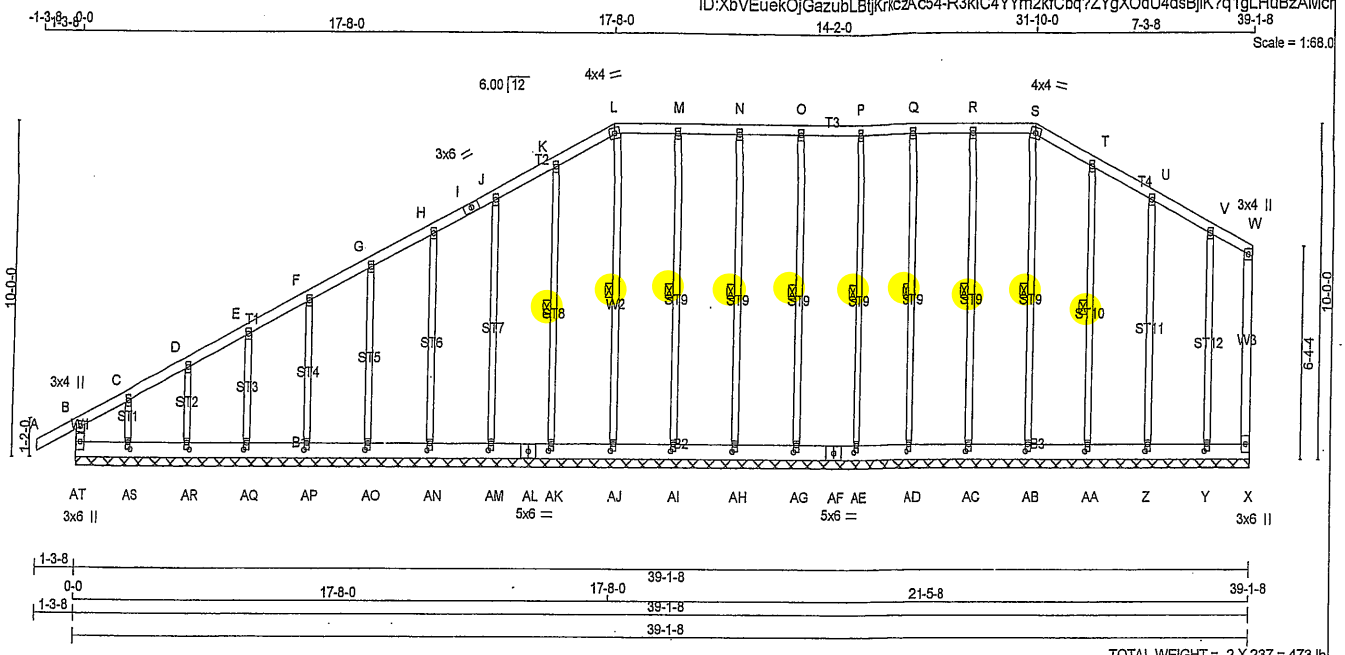

TOTAL WEIGHT = 2 X 237 = 473 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AT- B	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - S	2x4	DRY	No.2	SPF
S - W	2x4	DRY	No.2	SPF
X - W	2x4	DRY	No.2	SPF
AT- AL	2x6	DRY	No.2	SPF
AL- AF	2x6	DRY	No.2	SPF
AF- X	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, H, J, K, M, N, O, P, Q, R, T, U, V						
C	TMW+w	MT20	2.0	4.0		
I	TS-t	MT20	3.0	6.0		
L	TTW-m	MT20	4.0	4.0		
S	TTW-m	MT20	4.0	4.0		
W	TMV+p	MT20	3.0	4.0		
X	BMV1+p	MT20	3.0	6.0		
Y, Z, AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AM, AN, AO, AP, AQ, AR, AS						
Y	BMW1+w	MT20	2.0	4.0	2.50	1.00
AF	BS-t	MT20	5.0	6.0		
AL	BS-t	MT20	5.0	6.0		
AT	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF S-AB, R-AC, Q-AD, P-AE, O-AG, N-AH, M-AI, K-AK, T-AA, L-AJ.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AT- B	-320 / 0	0.0	0.0	0.06 (1)	7.81	AB- S	-250 / 0	0.16 (1)
A- B	0 / 37	-122.2	-122.2	0.16 (1)	10.00	AC- R	-253 / 0	0.17 (1)
B- C	-47 / 0	-122.2	-122.2	0.12 (1)	6.25	AD- Q	-243 / 0	0.16 (1)
C- D	-16 / 0	-122.2	-122.2	0.06 (1)	6.25	AE- P	-245 / 0	0.16 (1)
D- E	-16 / 0	-122.2	-122.2	0.06 (1)	6.25	AG- O	-245 / 0	0.16 (1)
E- F	-9 / 0	-122.2	-122.2	0.06 (1)	10.00	AH- N	-243 / 0	0.16 (1)
F- G	-6 / 0	-122.2	-122.2	0.06 (1)	10.00	AI- M	-250 / 0	0.16 (1)
G- H	-3 / 0	-122.2	-122.2	0.06 (1)	10.00	AK- K	-244 / 0	0.12 (1)
H- I	0 / 0	-122.2	-122.2	0.06 (1)	10.00	AM- J	-243 / 0	0.28 (1)
I- J	0 / 0	-122.2	-122.2	0.06 (1)	10.00	AN- H	-243 / 0	0.20 (1)
J- K	0 / 2	-122.2	-122.2	0.06 (1)	10.00	AO- G	-243 / 0	0.13 (1)
K- L	0 / 4	-122.2	-122.2	0.06 (1)	10.00	AP- F	-243 / 0	0.09 (1)
L- M	0 / 7	-122.2	-122.2	0.06 (1)	10.00	AQ- E	-239 / 0	0.06 (1)
M- N	0 / 7	-122.2	-122.2	0.06 (1)	10.00	AR- D	-256 / 0	0.04 (1)
N- O	0 / 7	-122.2	-122.2	0.06 (1)	10.00	AS- C	-169 / 0	0.02 (1)
O- P	0 / 7	-122.2	-122.2	0.06 (1)	10.00	AA- T	-239 / 0	0.12 (1)
P- Q	0 / 7	-122.2	-122.2	0.06 (1)	10.00	Z- U	-252 / 0	0.30 (1)
Q- R	0 / 7	-122.2	-122.2	0.06 (1)	10.00	Y- V	-211 / 0	0.17 (1)
R- S	0 / 7	-122.2	-122.2	0.06 (1)	10.00	AJ- L	-250 / 0	0.16 (1)
S- T	0 / 5	-122.2	-122.2	0.06 (1)	10.00			
T- U	0 / 2	-122.2	-122.2	0.06 (1)	10.00			
U- V	0 / 3	-122.2	-122.2	0.06 (1)	10.00			
V- W	0 / 5	-122.2	-122.2	0.04 (1)	10.00			
X- W	-67 / 0	0.0	0.0	0.01 (1)	7.81			
AT-AS	0 / 26	-28.0	-28.0	0.03 (1)	10.00			
AS-AR	0 / 19	-28.0	-28.0	0.01 (2)	10.00			
AR-AQ	0 / 13	-28.0	-28.0	0.01 (2)	10.00			
AQ-AP	0 / 9	-28.0	-28.0	0.01 (2)	10.00			
AP-AO	0 / 5	-28.0	-28.0	0.01 (2)	10.00			
AO-AN	0 / 2	-28.0	-28.0	0.01 (2)	10.00			
AN-AM	0 / 0	-28.0	-28.0	0.01 (2)	10.00			
AM-AL	-2 / 0	-28.0	-28.0	0.01 (2)	10.00			
AL-AK	-2 / 0	-28.0	-28.0	0.01 (2)	10.00			
AK-AJ	-4 / 0	-28.0	-28.0	0.01 (2)	10.00			
AJ-AI	-7 / 0	-28.0	-28.0	0.01 (2)	10.00			
AI-AH	-7 / 0	-28.0	-28.0	0.01 (2)	10.00			
AH-AG	-7 / 0	-28.0	-28.0	0.01 (2)	10.00			
AG-AF	-7 / 0	-28.0	-28.0	0.01 (2)	10.00			
AF-AE	-7 / 0	-28.0	-28.0	0.01 (2)	10.00			
AE-AD	-7 / 0	-28.0	-28.0	0.01 (2)	10.00			
AD-AC	-7 / 0	-28.0	-28.0	0.01 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16 (A-B:1) , BC=0.03 (AS-AT:1) , WB=0.30 (U-Z:1) , SSI=0.12 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (L) (INPUT = 0.90)
JSI METAL= 0.07 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 6216 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM25799-16
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AC-AB	-7 / 0	-28.0	-28.0	0.01 (2)	10.00			
AB-AA	-4 / 0	-28.0	-28.0	0.01 (2)	10.00			
AA- Z	-2 / 0	-28.0	-28.0	0.01 (2)	10.00			
Z- Y	0 / 0	-28.0	-28.0	0.01 (2)	10.00			
Y- X	0 / 2	-28.0	-28.0	0.01 (3)	10.00			

P62/2

DWG NO. TAM25799-16

STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T21	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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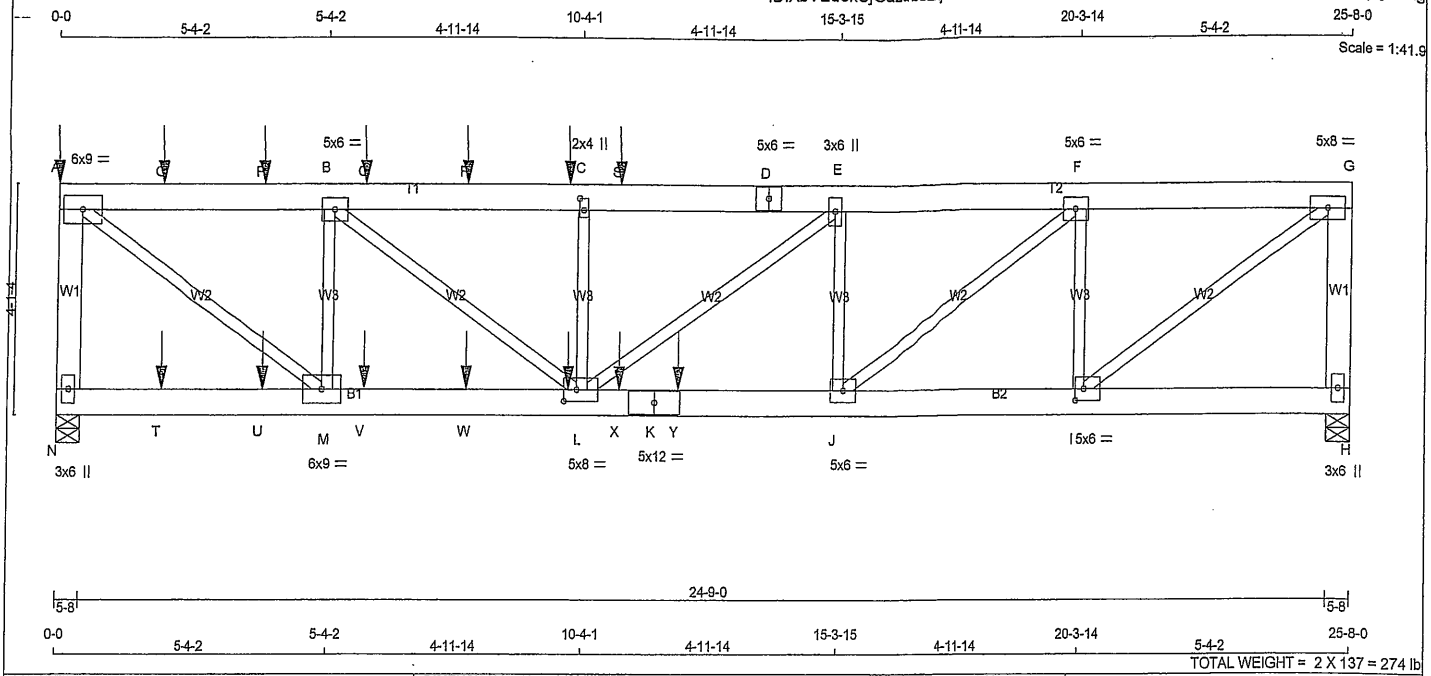

TOTAL WEIGHT = 2 X 137 = 274 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	1650F 1.5E	SPF
D - G	2x6	DRY	1650F 1.5E	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
K - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
N- A	2	12	TOP
A- D	2	12	SIDE(183.1)
D- G	2	12	TOP
G- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	SIDE(183.1)
K- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0		
B	TMWW-t	MT20	5.0	6.0		
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	3.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.00
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	12.0		
L	BMWWW-t	MT20	5.0	8.0	2.50	3.00
M	BMWW-t	MT20	6.0	9.0		
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	3914	0	3914	0	0	5-8	5-8
H	3105	0	3105	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3025	2032 / 0	500 / 0	0 / 0	0 / 0	493 / 0	0 / 0
H	2409	1603 / 0	408 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	TO	LC1 MAX (LC)	MAX. UNBRAC LENGTH
N- A	-3784 / 0	0.0	0.0	0.28 (1)	7.26
A- O	-4714 / 0	-122.2	-122.2	0.14 (1)	5.80
O- P	-4714 / 0	-122.2	-122.2	0.14 (1)	5.80
P- B	-4714 / 0	-122.2	-122.2	0.14 (1)	5.80
B- Q	-7447 / 0	-122.2	-122.2	0.16 (1)	4.86
Q- R	-7447 / 0	-122.2	-122.2	0.16 (1)	4.86
R- C	-7447 / 0	-122.2	-122.2	0.16 (1)	4.86
C- S	-7447 / 0	-122.2	-122.2	0.12 (1)	4.91
S- D	-7447 / 0	-122.2	-122.2	0.12 (1)	4.91
D- E	-7447 / 0	-122.2	-122.2	0.12 (1)	4.91
E- F	-6784 / 0	-122.2	-122.2	0.10 (1)	5.10
F- G	-4006 / 0	-122.2	-122.2	0.09 (1)	6.22
H- G	-3009 / 0	0.0	0.0	0.22 (1)	7.81
N- T	0 / 0	-28.0	-28.0	0.06 (2)	10.00
T- U	0 / 0	-23.0	-28.0	0.06 (2)	10.00
U- M	0 / 0	-28.0	-28.0	0.06 (2)	10.00
M- V	0 / 4714	-28.0	-28.0	0.35 (1)	10.00
V- W	0 / 4714	-28.0	-28.0	0.35 (1)	10.00
W- L	0 / 4714	-28.0	-28.0	0.35 (1)	10.00
L- X	0 / 6784	-28.0	-28.0	0.70 (1)	10.00
X- K	0 / 6784	-28.0	-28.0	0.70 (1)	10.00
K- Y	0 / 6784	-28.0	-28.0	0.70 (1)	10.00
Y- J	0 / 6784	-28.0	-28.0	0.70 (1)	10.00
J- I	0 / 4006	-28.0	-28.0	0.30 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.04 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX (LC)
I- G	0 / 4849	0.60 (1)
A- M	0 / 5706	0.71 (1)
I- F	-2736 / 0	0.33 (1)
M- B	-3144 / 0	0.38 (1)
J- F	0 / 3440	0.43 (1)
B- L	0 / 3385	0.42 (1)
J- E	-1091 / 0	0.13 (1)
L- C	-916 / 0	0.11 (1)
L- E	0 / 822	0.10 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-204	-204	---	BACK	VERT	TOTAL
C	10-0-12	-147	-147	---	BACK	VERT	TOTAL
L	10-0-12	-40	-70	---	BACK	VERT	TOTAL
O	2-0-12	-147	-147	---	BACK	VERT	TOTAL
P	4-0-12	-147	-147	---	BACK	VERT	TOTAL
Q	6-0-12	-147	-147	---	BACK	VERT	TOTAL
R	8-0-12	-147	-147	---	BACK	VERT	TOTAL
S	11-0-12	-147	-147	---	BACK	VERT	TOTAL
T	2-0-12	-40	-70	---	BACK	VERT	TOTAL
U	4-0-12	-40	-70	---	BACK	VERT	TOTAL
V	6-0-12	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.28 (A-N:1) , BC=0.70 (J-L:1) , WB=0.71 (A-M:1) , SSI=0.42 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.97 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9/11/17 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM45837217
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T21	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 204.4 lbs FACTORED DOWN AT 0-0, 147.1 lbs FACTORED DOWN AT 2-0-12, 147.1 lbs FACTORED DOWN AT 4-0-12, 147.1 lbs FACTORED DOWN AT 6-0-12, 147.1 lbs FACTORED DOWN AT 8-0-12, AND 147.1 lbs FACTORED DOWN AT 10-0-12, AND 147.1 lbs FACTORED DOWN AT 11-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-0-12, 69.9 lbs FACTORED DOWN AT 4-0-12, 69.9 lbs FACTORED DOWN AT 6-0-12, 69.9 lbs FACTORED DOWN AT 8-0-12, 69.9 lbs FACTORED DOWN AT 10-0-12, AND 69.9 lbs FACTORED DOWN AT 11-0-12, AND 1837.6 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	8-0-12	-40	-70	—	BACK	VERT	TOTAL
X	11-0-12	-40	-70	—	BACK	VERT	TOTAL
Y	12-2-8	-1838	-1838	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9/11
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45837-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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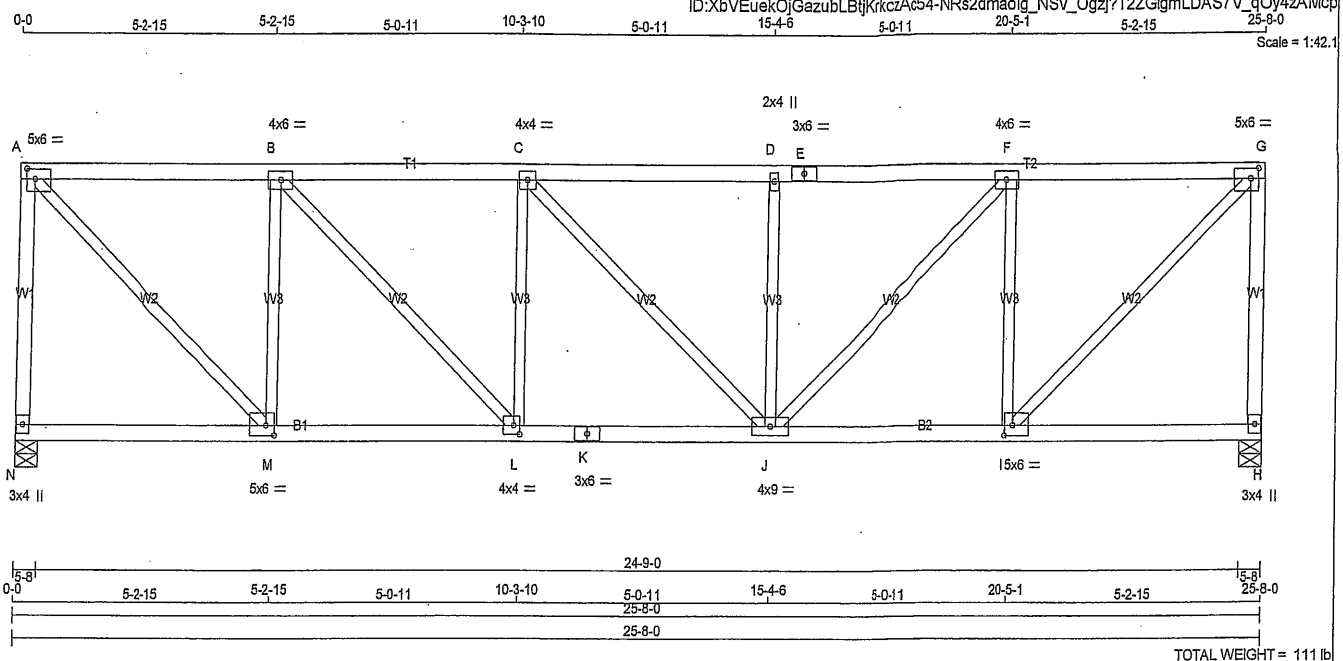


TOTAL WEIGHT = 111 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	2.00
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0	2.25	2.00
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BMWW-t	MT20	5.0	6.0	2.25	2.00
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1928	0	1928	0	0	5-8	2-12
H	1928	0	1928	0	0	5-8	2-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1508	981 / 0	270 / 0	0 / 0	0 / 0	257 / 0	0 / 0
H	1508	981 / 0	270 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
N- A	-1867 / 0	0.0	0.0	0.82 (1)	6.13
A- B	-1755 / 0	-122.2	-122.2	0.55 (1)	4.36
B- C	-2528 / 0	-122.2	-122.2	0.62 (1)	3.67
C- D	-2526 / 0	-122.2	-122.2	0.46 (1)	3.90
D- E	-2526 / 0	-122.2	-122.2	0.62 (1)	3.66
E- F	-2526 / 0	-122.2	-122.2	0.62 (1)	3.66
F- G	-1755 / 0	-122.2	-122.2	0.55 (1)	4.36
H- G	-1867 / 0	0.0	0.0	0.82 (1)	6.13
N- M	0 / 0	-28.0	-28.0	0.18 (3)	10.00
M- L	0 / 1755	-28.0	-28.0	0.37 (1)	10.00
L- K	0 / 2528	-28.0	-28.0	0.47 (1)	10.00
K- J	0 / 2528	-28.0	-28.0	0.47 (1)	10.00
J- I	0 / 1755	-28.0	-28.0	0.38 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	0 / 2382	0.54 (1)
A- M	0 / 2381	0.54 (1)
I- F	-1447 / 0	0.56 (1)
M- B	-1448 / 0	0.56 (1)
J- F	0 / 1064	0.24 (1)
B- L	0 / 1067	0.24 (1)
J- D	-597 / 0	0.23 (1)
L- C	-598 / 0	0.23 (1)
C- J	-3 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.82 (G-H:1) , BC=0.47 (J-L:1) , WB=0.56 (B-M:1) , SSI=0.30 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

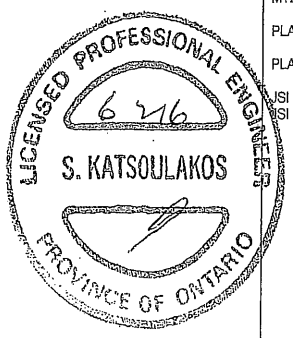
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (I) (INPUT = 0.90)
SI METAL= 0.69 (K) (INPUT = 1.00)



DWG NO. TAM25801-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T23	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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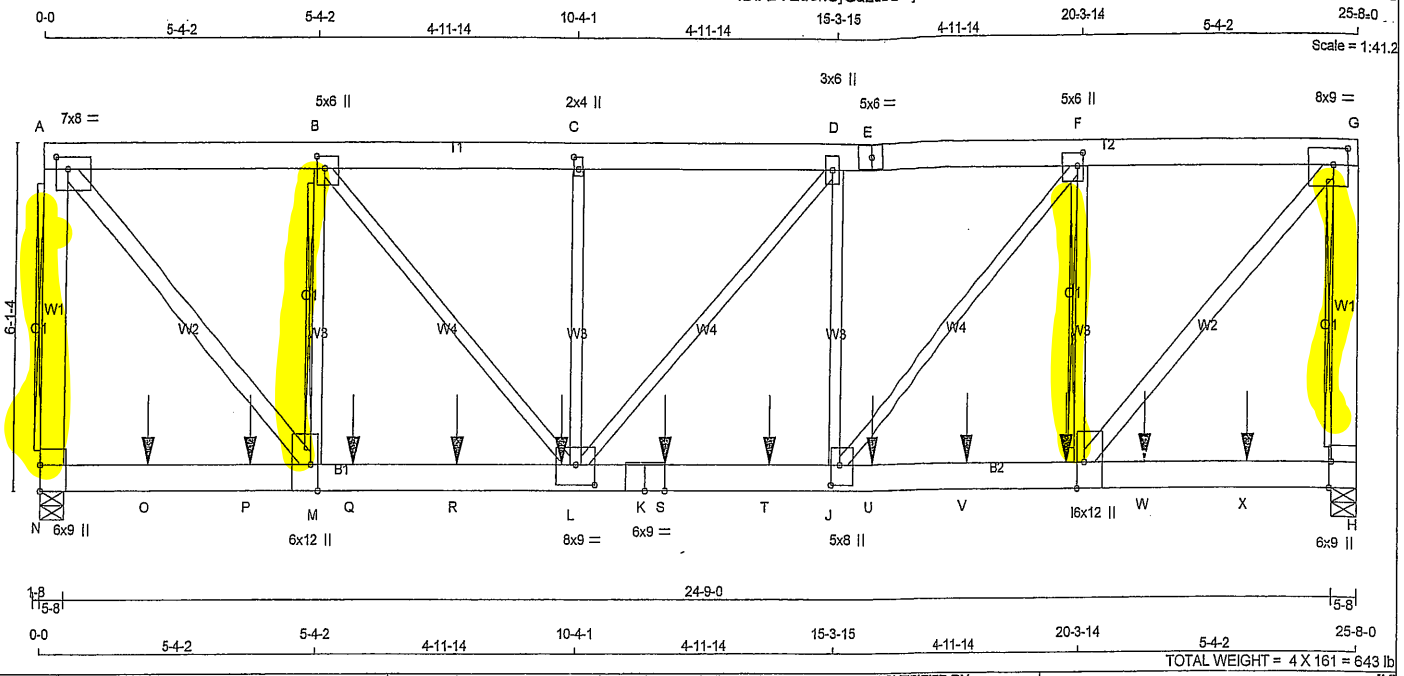

TOTAL WEIGHT = 4 X 161 = 643 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x6	DRY	No.2	SPF
A - E	2x6	DRY	2100F 1.8E	SPF
E - G	2x6	DRY	2100F 1.8E	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	2100F 1.8E	SPF
K - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
N- A	2	12	TOP
A- E	2	12	TOP
E- G	2	12	TOP
G- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	3	5	SIDE(1142.9)
K- H	3	5	SIDE(1397.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
L- C	1	4	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	7.0	8.0	2.50	2.75
B	TMWW+t	MT20	5.0	6.0	2.50	1.75
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TMWW+t	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	5.0	6.0	2.75	1.50
G	TMVW-t	MT20	8.0	9.0	3.50	3.25
H	BMV1+t	MT20	6.0	9.0	Edge	0.50
I	BMWW+t	MT20	6.0	12.0	Edge	1.75
J	BMWW+t	MT20	5.0	8.0	4.25	2.00
K	BS-t	MT20	6.0	9.0		
L	BMWWW-t	MT20	8.0	9.0	4.25	4.50
M	BMWW+t	MT20	6.0	12.0	Edge	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	19122	0	19122	0	0	5-8	5-8
H	19659	0	19659	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	14877	9819 / 0	2572 / 0	0 / 0	0 / 0	2485 / 0	0 / 0
H	15295	10096 / 0	2644 / 0	0 / 0	0 / 0	2555 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT A-N, G-H, F-I, B-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
N- A	-17310 / 0	0.0	0.0	0.39 (1)	7.81
A- B	-16080 / 0	-122.2	-122.2	0.09 (1)	4.94
B- C	-23025 / 0	-122.2	-122.2	0.14 (1)	4.25
C- D	-23025 / 0	-122.2	-122.2	0.14 (1)	4.24
D- E	-23141 / 0	-122.2	-122.2	0.14 (1)	4.24
E- F	-23141 / 0	-122.2	-122.2	0.14 (1)	4.24
F- G	-16510 / 0	-122.2	-122.2	0.10 (1)	4.89
H- G	-17772 / 0	0.0	0.0	0.40 (1)	7.81
N- O	0 / 0	-28.0	-28.0	0.45 (1)	10.00
O- P	0 / 0	-28.0	-28.0	0.45 (1)	10.00
P- M	0 / 0	-28.0	-28.0	0.45 (1)	10.00
M- Q	0 / 16080	-28.0	-28.0	0.67 (1)	10.00
Q- R	0 / 16080	-28.0	-28.0	0.67 (1)	10.00
R- L	0 / 16080	-28.0	-28.0	0.67 (1)	10.00
L- K	0 / 23141	-28.0	-28.0	0.59 (1)	10.00
K- S	0 / 23141	-28.0	-28.0	0.59 (1)	10.00
S- T	0 / 23141	-28.0	-28.0	0.59 (1)	10.00
T- J	0 / 23141	-28.0	-28.0	0.59 (1)	10.00
J- U	0 / 16510	-28.0	-28.0	0.63 (1)	10.00
U- V	0 / 16510	-28.0	-28.0	0.63 (1)	10.00
V- I	0 / 16510	-28.0	-28.0	0.63 (1)	10.00
I- W	0 / 0	-28.0	-28.0	0.43 (1)	10.00
W- X	0 / 0	-28.0	-28.0	0.43 (1)	10.00
X- H	0 / 0	-28.0	-28.0	0.43 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- F	-8134 / 0	0.45 (1)
M- B	-8513 / 0	0.47 (1)
I- G	0 / 24013	0.96 (1)
A- M	0 / 23387	0.94 (1)
J- F	0 / 10012	0.56 (1)
B- L	0 / 10486	0.59 (1)
J- D	-477 / 2	0.06 (1)
L- C	-574 / 0	0.07 (1)
L- D	-175 / 0	0.05 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	20-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
L	10-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
O	2-0-12	-2911	-2911	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 882 (0.35")

CSI: TC=0.40 (G-H:1) , BC=0.67 (L-M:1) , WB=0.96 (G-I:1) , SSI=0.72 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.95 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45838-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2910.5 lbs FACTORED DOWN AT 2-0-12, 2910.5 lbs FACTORED DOWN AT 4-0-12, 2910.5 lbs FACTORED DOWN AT 6-0-12, 2910.5 lbs FACTORED DOWN AT 8-0-12, 2910.5 lbs FACTORED DOWN AT 10-0-12, 2910.5 lbs FACTORED DOWN AT 12-0-12, 2910.5 lbs FACTORED DOWN AT 14-0-12, 2910.5 lbs FACTORED DOWN AT 16-0-12, 2910.5 lbs FACTORED DOWN AT 18-0-12, 2910.5 lbs FACTORED DOWN AT 20-0-12, AND 2910.5 lbs FACTORED DOWN AT 21-7-4, AND 2910.5 lbs FACTORED DOWN AT 23-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	4-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
Q	6-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
R	8-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
S	12-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
T	14-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
U	16-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
V	18-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
W	21-7-4	-2911	-2911	—	FRONT	VERT	TOTAL
X	23-7-4	-2911	-2911	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9147
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45838-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T24	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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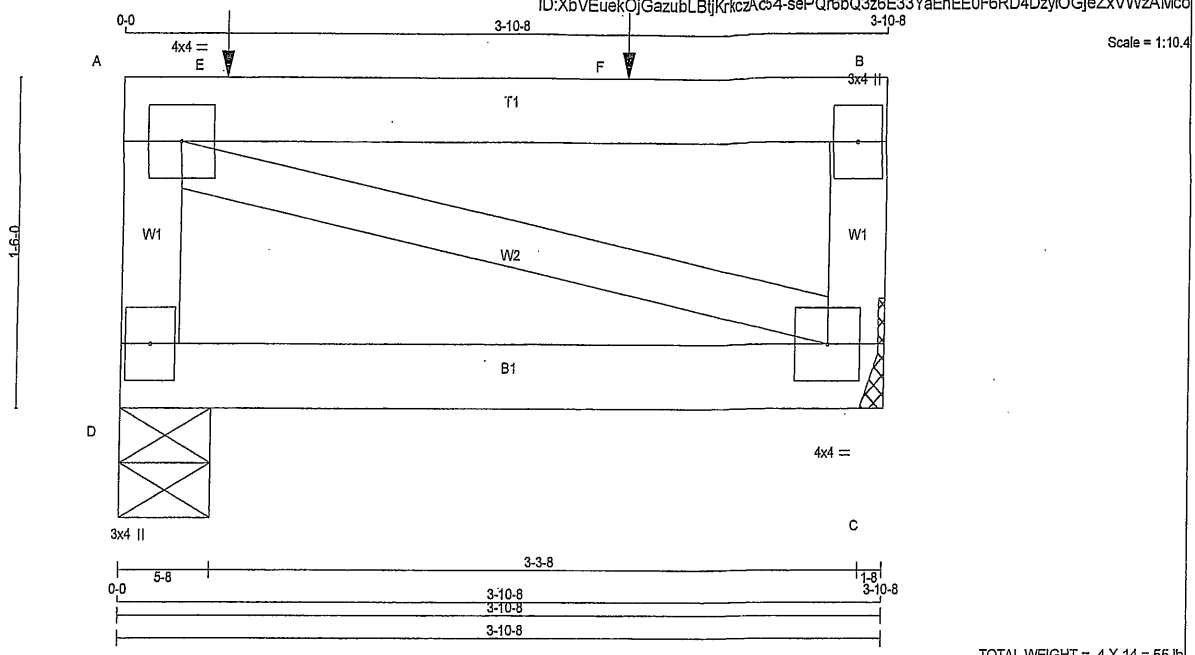


Scale = 1:10.4

TOTAL WEIGHT = 4 X 14 = 55 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	9	SIDE(15.4)
A- B	1	12	SIDE(61.0)
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	4.0	4.0	
B	*TMV+p*	*MT20*	*3.0*	*4.0*	
C	BMVW1-t	MT20	4.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 292.7 lbs FACTORED DOWN AT 6-4, AND 999.2 lbs FACTORED DOWN AT 2-6-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	894	0	894	0	0	5-8	1-8
C	980	0	980	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	683	471 / 0	105 / 0	0 / 0	0 / 0	108 / 0	0 / 0
C	763	502 / 0	132 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO				FR-TO		
D- A	-839 / 0	0.0 0.0	0.04 (1)		7.81	A- C	0 / 0	0.00 (1)
A- E	0 / 0	-122.2 -122.2	0.78 (1)		10.00			
E- F	0 / 0	-122.2 -122.2	0.78 (1)		10.00			
F- B	0 / 0	-122.2 -122.2	0.78 (1)		10.00			
C- B	-926 / 0	0.0 0.0	0.05 (1)		7.81			
D- C	0 / 0	-28.0 -28.0	0.06 (3)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	6-4	-293	-293	---	BACK	VERT	TOTAL
F	*2-6-4*	*-999*	*-999*	*---*	*TOP*	*VERT*	*TOTAL*

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.78 (A-B:1) , BC=0.06 (C-D:3) , WB=0.00 (A-C:1) , SSI=0.36 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

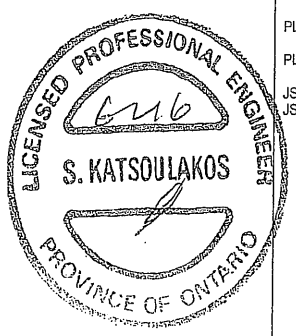
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (A) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)



DWG NO. TAM25603.16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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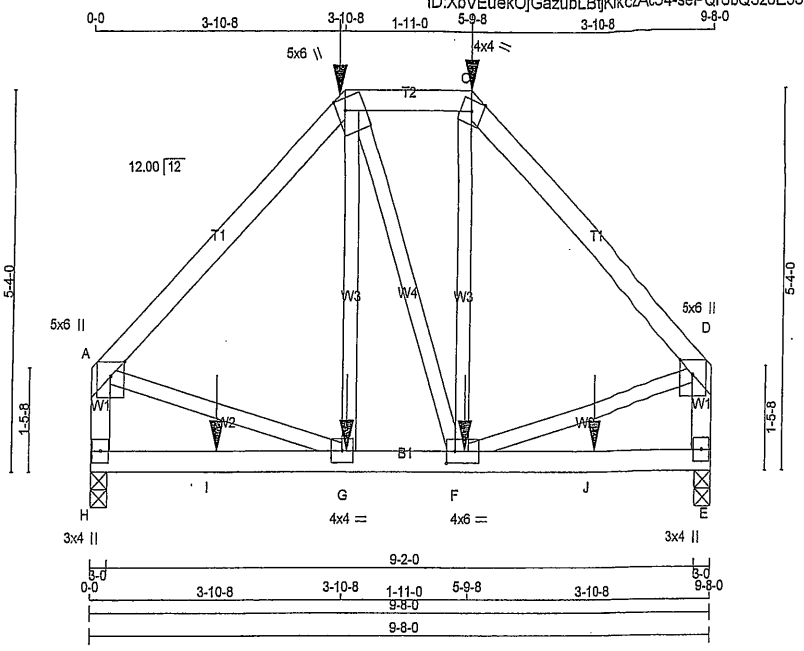
Scale: 3/8"=1'



TOTAL WEIGHT = 47 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMVW+p	MT20	5.0	6.0	Edge	
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0	2.00	1.50
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 3-10-8, AND 348.1 lbs FACTORED DOWN AT 5-9-8 ON TOP CHORD, AND 41.4 lbs FACTORED DOWN AT 1-11-4, 41.4 lbs FACTORED DOWN AT 3-11-4, AND 41.4 lbs FACTORED DOWN AT 5-8-12, AND 41.4 lbs FACTORED DOWN AT 7-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1121	0	1121	0	0	3-0	1-8
E	1121	0	1121	0	0	3-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	862	585 / 0	137 / 0	0 / 0	0 / 0	140 / 0	0 / 0
E	862	585 / 0	137 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-913 / 0	-122.2	-122.2	0.38 (1)	5.90		G- B	-53 / 159	0.04 (3)
B- C	-641 / 0	-122.2	-122.2	0.09 (1)	6.25		B- F	-1 / 3	0.00 (3)
C- D	-912 / 0	-122.2	-122.2	0.38 (1)	5.90		F- C	-54 / 164	0.04 (3)
H- A	-1063 / 0	0.0	0.0	0.12 (1)	7.61		A- G	0 / 667	0.17 (1)
E- D	-1063 / 0	0.0	0.0	0.12 (1)	7.61		F- D	0 / 667	0.17 (1)
H- I	0 / 0	-28.0	-28.0	0.15 (2)	10.00				
I- G	0 / 0	-28.0	-28.0	0.15 (2)	10.00				
G- F	0 / 642	-28.0	-28.0	0.18 (2)	10.00				
F- J	0 / 0	-28.0	-28.0	0.15 (2)	10.00				
J- E	0 / 0	-28.0	-28.0	0.15 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-10-8	-348	-348	---	BACK	VERT	TOTAL
C	5-9-8	-348	-348	---	BACK	VERT	TOTAL
F	5-8-12	-24	-41	---	BACK	VERT	TOTAL
G	3-11-4	-24	-41	---	BACK	VERT	TOTAL
I	1-11-4	-24	-41	---	BACK	VERT	TOTAL
J	7-8-12	-24	-41	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.38 (A-B:1) , BC=0.18 (F-G:2) , WB=0.17 (A-G:1) , SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.24 (G) (INPUT = 1.00)

DWG NO. TAM25604-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T26	1	1	TRUSS DESC.	

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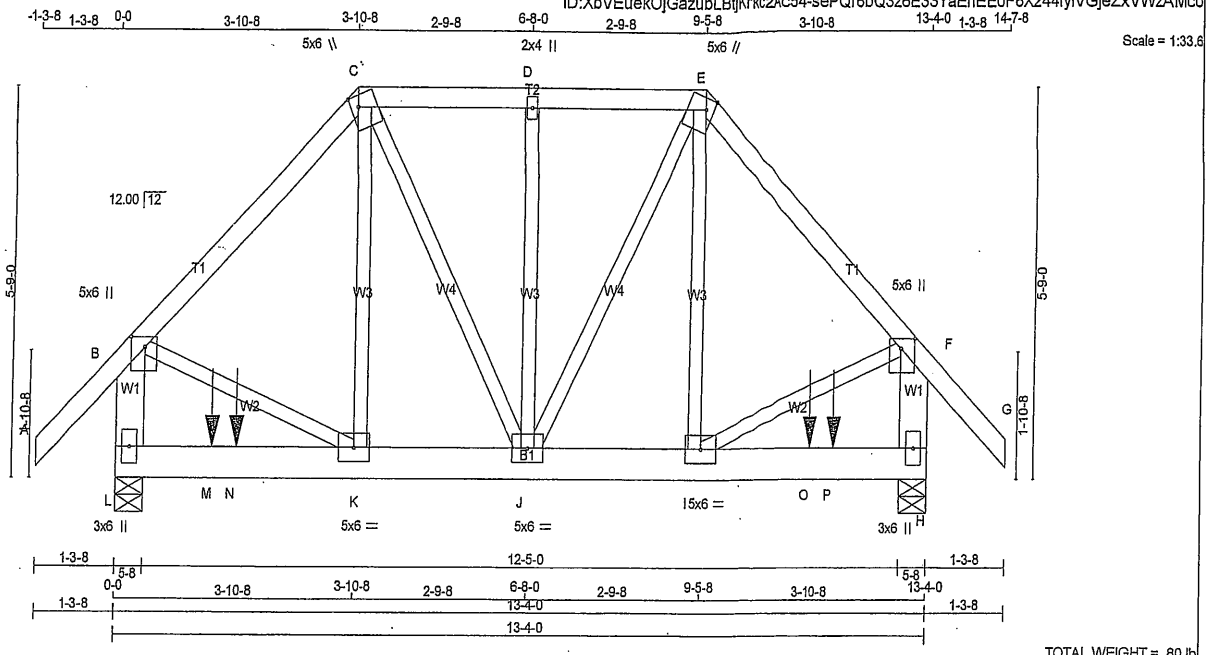

TOTAL WEIGHT = 80 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	1.75	2.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 42.6 lbs FACTORED DOWN AT 1-6-12, 952.4 lbs FACTORED DOWN AT 1-11-8, AND 952.4 lbs FACTORED DOWN AT 11-4-8, AND 42.6 lbs FACTORED DOWN AT 11-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	2149	0	2149	0	0	5-8	2-5
H	2149	0	2149	0	0	5-8	2-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1657	1118 / 0	270 / 0	0 / 0	0 / 0	269 / 0	0 / 0
H	1657	1118 / 0	270 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.19 (1)	10.00	K- C	0 / 558	0.14 (1)
B- C	-1343 / 0	-122.2	-122.2	0.40 (1)	5.06	C- J	-13 / 18	0.01 (3)
C- D	-965 / 0	-122.2	-122.2	0.17 (1)	6.09	J- D	-397 / 0	0.19 (1)
D- E	-965 / 0	-122.2	-122.2	0.17 (1)	6.09	J- E	-13 / 18	0.01 (3)
E- F	-1343 / 0	-122.2	-122.2	0.40 (1)	5.06	I- E	0 / 558	0.14 (1)
F- G	0 / 60	-122.2	-122.2	0.19 (1)	10.00	B- K	0 / 1006	0.25 (1)
L- B	-1703 / 0	0.0	0.0	0.13 (1)	7.55	I- F	0 / 1006	0.25 (1)
H- F	-1703 / 0	0.0	0.0	0.13 (1)	7.55			
L- M	0 / 0	-28.0	-28.0	0.60 (1)	10.00			
M- N	0 / 0	-28.0	-28.0	0.60 (1)	10.00			
N- K	0 / 0	-28.0	-28.0	0.60 (1)	10.00			
K- J	0 / 957	-28.0	-28.0	0.44 (1)	10.00			
J- I	0 / 957	-28.0	-28.0	0.44 (1)	10.00			
I- O	0 / 0	-28.0	-28.0	0.60 (1)	10.00			
O- P	0 / 0	-28.0	-28.0	0.60 (1)	10.00			
P- H	0 / 0	-28.0	-28.0	0.60 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	1-6-12	-24	-43	---	BACK	VERT	TOTAL
N	1-11-8	-952	-952	---	BACK	VERT	TOTAL
O	11-4-8	-952	-952	---	BACK	VERT	TOTAL
P	11-9-4	-24	-43	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.40 (E-F:1) , BC=0.60 (K-L:1) , WB=0.25 (F-I:1) , SSI=0.43 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT *RESPONSIBLE FOR QUALITY CONTROL IN* THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.29 (F) (INPUT = 1.00)

DWG NO. TAM2580516
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T27	1	1	TRUSS DESC.	

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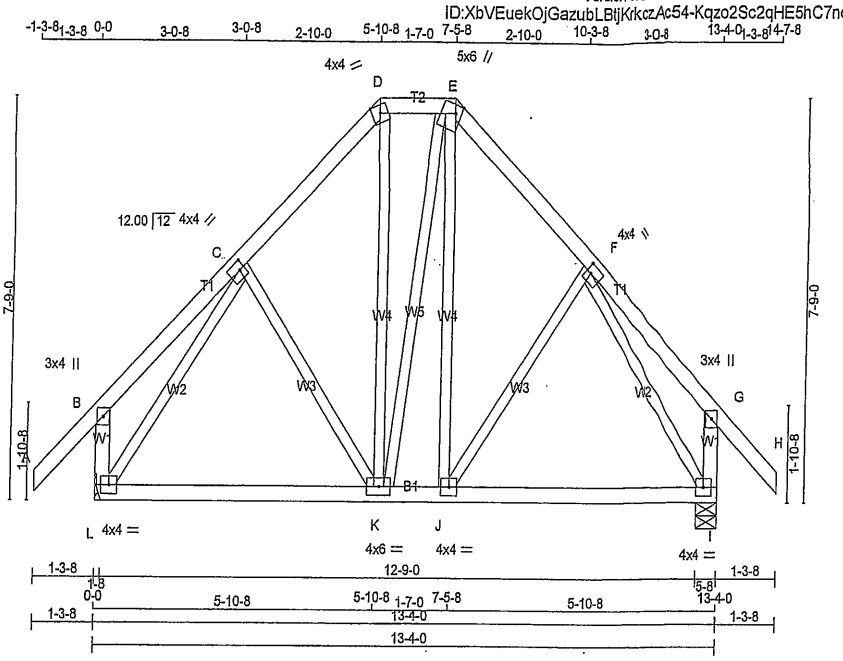


TOTAL WEIGHT = 78 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1172	0	1172	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	1172	0	1172	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	898	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0
I	898	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 29	-122.2	-122.2	0.17 (1)	10.00
C- D	-672 / 0	-122.2	-122.2	0.13 (1)	6.25
D- E	-456 / 0	-122.2	-122.2	0.04 (1)	6.25
E- F	-670 / 0	-122.2	-122.2	0.13 (1)	6.25
F- G	0 / 29	-122.2	-122.2	0.17 (1)	10.00
G- H	0 / 60	-122.2	-122.2	0.17 (1)	10.00
L- B	-306 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-306 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 522	-28.0	-28.0	0.27 (2)	10.00
K- J	0 / 454	-28.0	-28.0	0.27 (2)	10.00
J- I	0 / 521	-28.0	-28.0	0.26 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- K	-122 / 43	0.06 (1)
K- D	0 / 218	0.05 (1)
K- E	0 / 7	0.00 (1)
J- E	0 / 210	0.05 (1)
J- F	-122 / 42	0.06 (1)
L- C	-953 / 0	0.43 (1)
F- I	-951 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.17 (G-H:1) , BC=0.27 (K-L:2) , WB=0.43 (C-L:1) , SSI=0.14 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

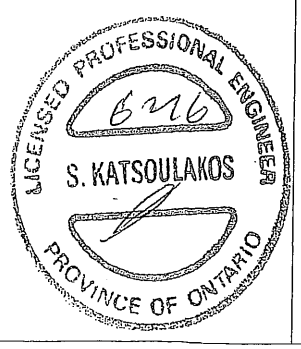
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)



DWG NO. TAM 25806 16

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T28	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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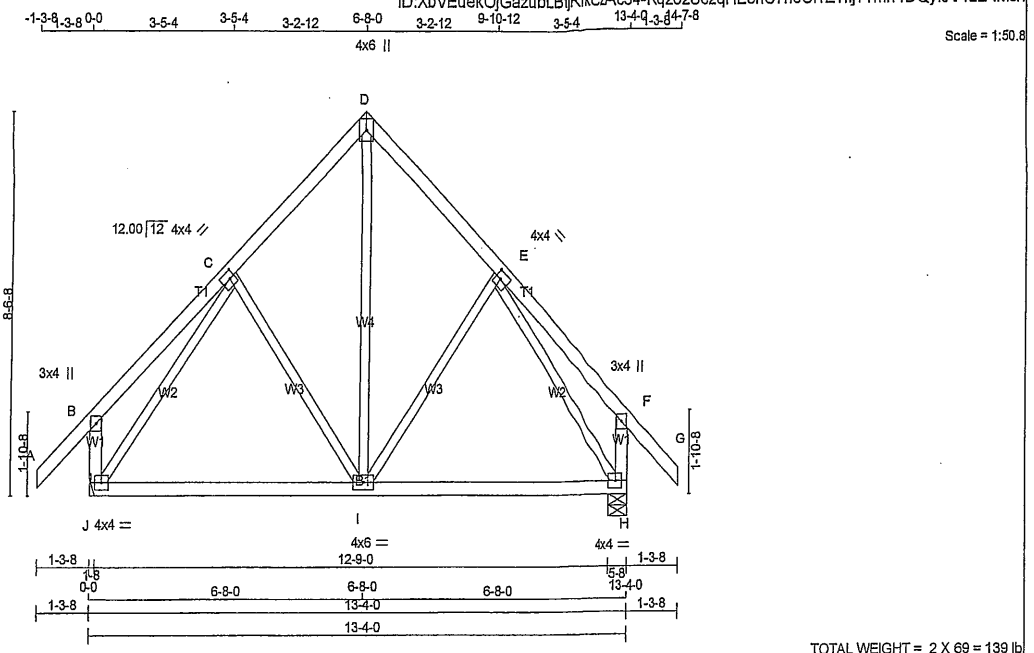

TOTAL WEIGHT = 2 X 69 = 139 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1172	0	1172	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	1172	0	1172	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	898	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0
H	898	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 33	-122.2	-122.2	0.22 (1)	10.00	
C- D	-634 / 0	-122.2	-122.2	0.17 (1)	6.25	
D- E	-634 / 0	-122.2	-122.2	0.17 (1)	6.25	
E- F	0 / 33	-122.2	-122.2	0.22 (1)	10.00	
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
J- B	-324 / 0	0.0	0.0	0.04 (1)	7.81	
H- F	-324 / 0	0.0	0.0	0.04 (1)	7.81	
J- I	0 / 531	-28.0	-28.0	0.41 (2)	10.00	
I- H	0 / 531	-28.0	-28.0	0.41 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 530	0.12 (1)
I- E	-183 / 35	0.11 (1)
C- I	-183 / 35	0.11 (1)
J- C	-940 / 0	0.54 (1)
E- H	-940 / 0	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.22 (B-C:1) , BC=0.41 (I-J:2) , WB=0.54 (E-H:1) , SSI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM2580216
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	T29	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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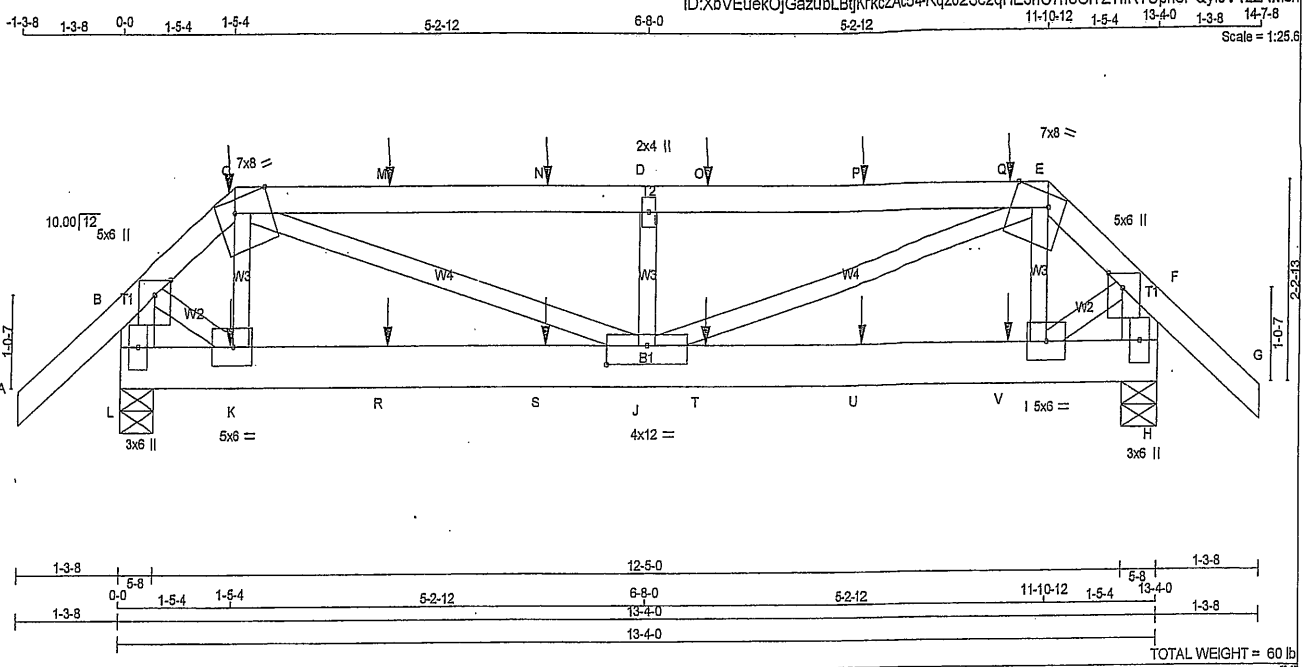

TOTAL WEIGHT = 60 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - G	2x4	DRY	2100F 1.8E	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	7.0	8.0	Edge	5.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	7.0	8.0	Edge	5.50
F	TMVW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	12.0	2.50	6.00
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 105.3 lbs FACTORED DOWN AT 1-5-4, 101.7 lbs FACTORED DOWN AT 3-4-12, 101.7 lbs FACTORED DOWN AT 5-4-12, 101.7 lbs FACTORED DOWN AT 7-4-12, AND 101.7 lbs FACTORED DOWN AT 9-4-12, AND 132.3 lbs FACTORED DOWN AT 11-4-12 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 1-4-12, 70.4 lbs FACTORED DOWN AT 3-4-12, 70.4 lbs FACTORED DOWN AT 5-4-12, 70.4 lbs FACTORED DOWN AT 7-4-12, AND 70.4 lbs FACTORED DOWN AT 9-4-12, AND 70.4 lbs FACTORED DOWN AT 11-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1715	0	1715	0	0	5-8	1-14
H	1694	0	1694	0	0	5-8	1-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1322	892 / 0	215 / 0	0 / 0	0 / 0	216 / 0	0 / 0
H	1304	885 / 0	209 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: 1

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	-154 / 0	-122.2	-122.2	0.12 (1)	10.00
B- C	-1644 / 0	-122.2	-122.2	0.13 (1)	5.99
C- M	-2967 / 0	-122.2	-122.2	0.62 (1)	4.09
M- N	-2967 / 0	-122.2	-122.2	0.62 (1)	4.09
N- D	-2967 / 0	-122.2	-122.2	0.62 (1)	4.09
D- O	-2967 / 0	-122.2	-122.2	0.62 (1)	4.09
O- P	-2967 / 0	-122.2	-122.2	0.62 (1)	4.09
P- Q	-2967 / 0	-122.2	-122.2	0.62 (1)	4.09
Q- E	-2967 / 0	-122.2	-122.2	0.62 (1)	4.09
E- F	-1627 / 0	-122.2	-122.2	0.13 (1)	6.01
F- G	0 / 54	-122.2	-122.2	0.12 (1)	10.00
L- B	-1854 / 0	0.0	0.0	0.13 (1)	7.32
H- F	-1838 / 0	0.0	0.0	0.13 (1)	7.34
L- K	0 / 0	-28.0	-28.0	0.18 (1)	10.00
K- R	0 / 1212	-28.0	-28.0	0.34 (1)	10.00
R- S	0 / 1212	-28.0	-28.0	0.34 (1)	10.00
S- J	0 / 1212	-28.0	-28.0	0.34 (1)	10.00
J- T	0 / 1199	-28.0	-28.0	0.34 (1)	10.00
T- U	0 / 1199	-28.0	-28.0	0.34 (1)	10.00
U- V	0 / 1199	-28.0	-28.0	0.34 (1)	10.00
V- I	0 / 1199	-28.0	-28.0	0.34 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-20 / 154	0.04 (3)
C- J	0 / 1861	0.46 (1)
J- D	-1076 / 0	0.17 (1)
J- E	0 / 1874	0.46 (1)
I- E	-29 / 151	0.04 (3)
B- K	0 / 1307	0.32 (1)
I- F	0 / 1293	0.32 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-5-4	-105	-105	---	BACK	VERT	TOTAL
K	1-4-12	-70	-70	---	BACK	VERT	TOTAL
M	3-4-12	-102	-102	---	BACK	VERT	TOTAL
N	5-4-12	-102	-102	---	BACK	VERT	TOTAL
O	7-4-12	-102	-102	---	BACK	VERT	TOTAL
P	9-4-12	-102	-102	---	BACK	VERT	TOTAL
Q	11-4-12	-132	-132	---	BACK	VERT	TOTAL
R	3-4-12	-70	-70	---	BACK	VERT	TOTAL
S	5-4-12	-70	-70	---	BACK	VERT	TOTAL
T	7-4-12	-70	-70	---	BACK	VERT	TOTAL
U	9-4-12	-70	-70	---	BACK	VERT	TOTAL
V	11-4-12	-70	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.62 (D-E:1) , BC=0.34 (I-J:1) , WB=0.46 (E-J:1) , SSI=0.49 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
6216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM2580816
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272340	T40	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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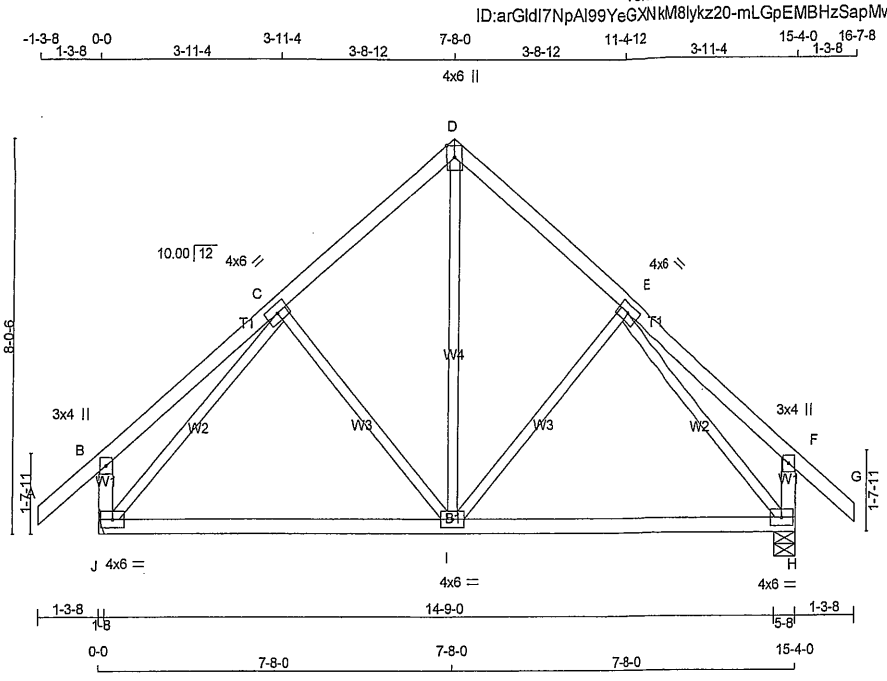

TOTAL WEIGHT = 2 X 71 = 143 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1321	0	1321	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	1321	0	1321	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1015	692 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0
H	1015	692 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 35	-122.2	-122.2	0.29 (1)	10.00
C- D	-814 / 0	-122.2	-122.2	0.23 (1)	6.25
D- E	-814 / 0	-122.2	-122.2	0.23 (1)	6.25
E- F	0 / 35	-122.2	-122.2	0.29 (1)	10.00
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00
J- B	-347 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-347 / 0	0.0	0.0	0.04 (1)	7.81
J- I	0 / 758	-26.0	-28.0	0.55 (2)	10.00
I- H	0 / 758	-28.0	-28.0	0.55 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 634	0.14 (1)
I- E	-244 / 37	0.15 (1)
C- I	-244 / 37	0.15 (1)
J- C	-1171 / 0	0.68 (1)
E- H	-1171 / 0	0.68 (1)

DWG NO. TAM 45840-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.29 (E-F:1) , BC=0.55 (I-J:2) , WB=0.68 (E-H:1) , SSI=0.18 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272340	T40S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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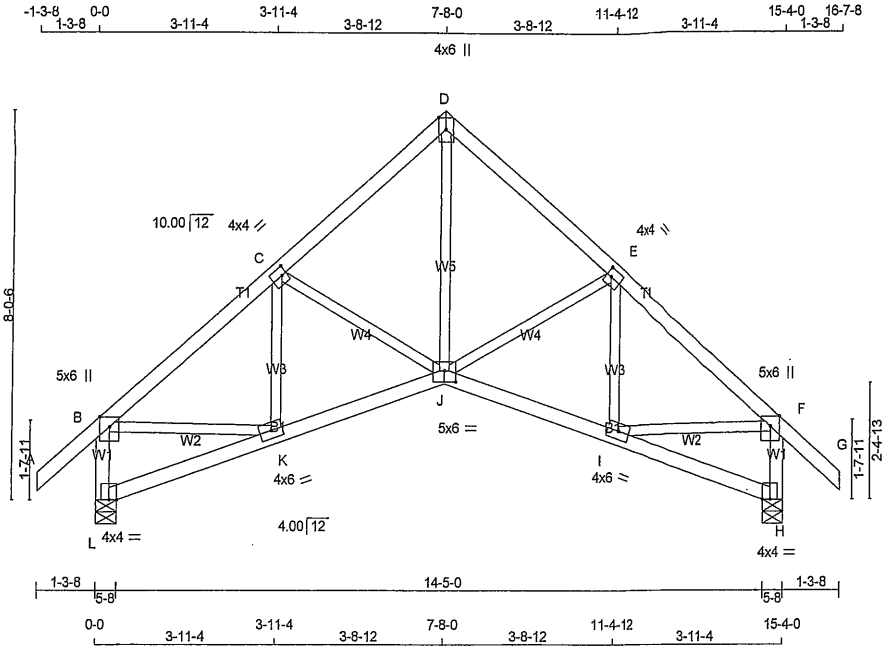

TOTAL WEIGHT = 3 X 70 = 210 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BVM1-p	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BBWWW-p	MT20	5.0	6.0	2.75	3.00
K	BMWW-t	MT20	4.0	6.0		
L	BVM1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1321	0	1321	0	0	5-8	5-8
H	1321	0	1321	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1015	692 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0
H	1015	692 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	J- D	0 / 1048	0.24 (1)
B- C	-1342 / 0	-122.2	-122.2	0.24 (1)	5.34	J- E	-256 / 0	0.08 (1)
C- D	-1123 / 0	-122.2	-122.2	0.23 (1)	5.72	I- E	-263 / 85	0.06 (1)
D- E	-1123 / 0	-122.2	-122.2	0.23 (1)	5.72	C- J	-256 / 0	0.08 (1)
E- F	-1342 / 0	-122.2	-122.2	0.24 (1)	5.34	K- C	-263 / 85	0.06 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00	B- K	0 / 1058	0.24 (1)
L- B	-1275 / 0	0.0	0.0	0.14 (1)	7.13	I- F	0 / 1058	0.24 (1)
H- F	-1275 / 0	0.0	0.0	0.14 (1)	7.13			
L- K	0 / 7	-28.0	-28.0	0.11 (3)	10.00			
K- J	0 / 1111	-28.0	-28.0	0.23 (1)	10.00			
J- I	0 / 1111	-28.0	-28.0	0.23 (1)	10.00			
I- H	0 / 7	-28.0	-28.0	0.11 (3)	10.00			

DWG NO. TAM 45841-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.24 (B-C:1) , BC=0.23 (J-K:1) , WB=0.24 (B-K:1) , SSI=0.18 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.42 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272342	T45	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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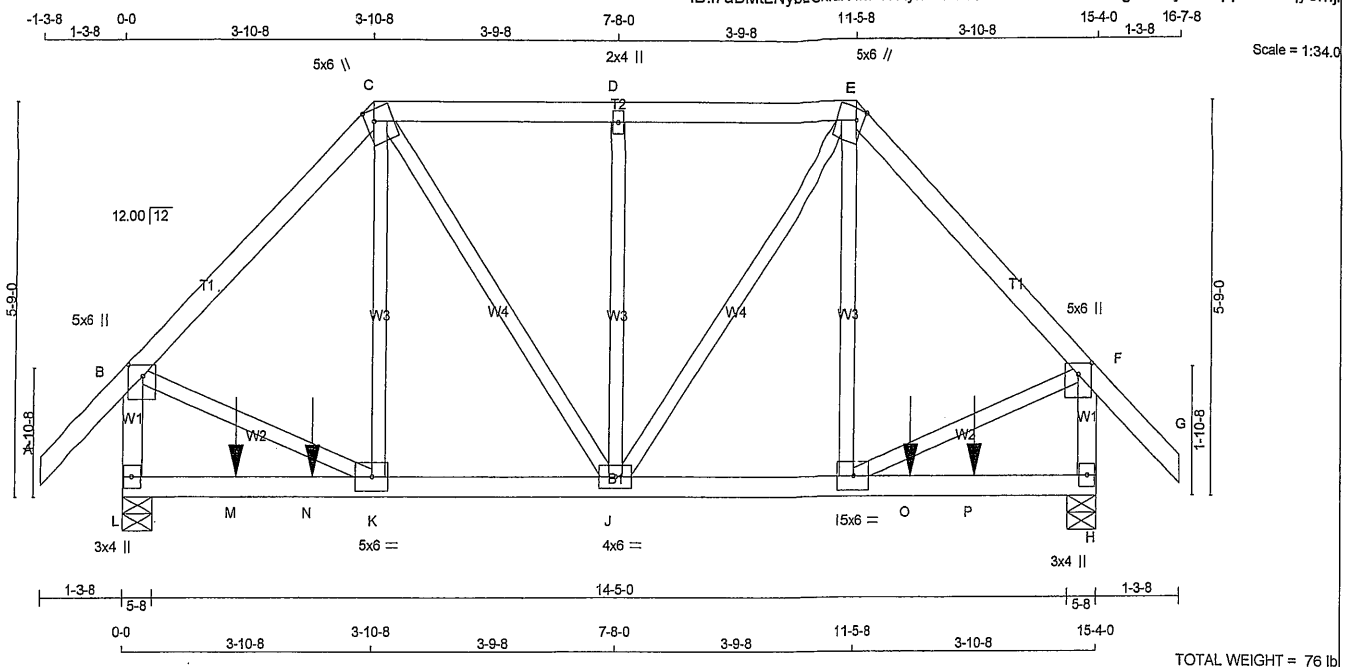

TOTAL WEIGHT = 76 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	2.00	2.50
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 42.6 lbs FACTORED DOWN AT 1-9-4, 966.3 lbs FACTORED DOWN AT 2-11-8, AND 966.3 lbs FACTORED DOWN AT 12-4-8, AND 42.6 lbs FACTORED DOWN AT 13-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2313	0	2313	0	0	5-8	5-8
H	2312	0	2312	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1785	1203 / 0	292 / 0	0 / 0	0 / 0	290 / 0	0 / 0
H	1784	1203 / 0	291 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	0 / 60	-122.2	-122.2	0.19 (1)	10.00	K- C	0 / 629	0.16 (1)	
B- C	-1727 / 0	-122.2	-122.2	0.43 (1)	4.55	C- J	0 / 210	0.05 (1)	
C- D	-1349 / 0	-122.2	-122.2	0.34 (1)	5.11	J- D	-550 / 0	0.29 (1)	
D- E	-1349 / 0	-122.2	-122.2	0.34 (1)	5.11	J- E	0 / 209	0.05 (1)	
E- F	-1728 / 0	-122.2	-122.2	0.43 (1)	4.55	I- E	0 / 630	0.16 (1)	
F- G	0 / 60	-122.2	-122.2	0.19 (1)	10.00	B- K	0 / 1303	0.32 (1)	
L- B	-2099 / 0	0.0	0.0	0.25 (1)	5.80	I- F	0 / 1303	0.32 (1)	
H- F	-2100 / 0	0.0	0.0	0.25 (1)	5.80				
L- M	0 / 0	-28.0	-28.0	0.72 (1)	10.00				
M- N	0 / 0	-28.0	-28.0	0.72 (1)	10.00				
N- K	0 / 0	-28.0	-28.0	0.72 (1)	10.00				
K- J	0 / 1229	-28.0	-28.0	0.83 (1)	10.00				
J- I	0 / 1229	-28.0	-28.0	0.83 (1)	10.00				
I- O	0 / 0	-28.0	-28.0	0.72 (1)	10.00				
O- P	0 / 0	-28.0	-28.0	0.72 (1)	10.00				
P- H	0 / 0	-28.0	-28.0	0.72 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	1-9-4	-24	-43	—	FRONT	VERT	TOTAL
N	2-11-8	-966	-966	—	FRONT	VERT	TOTAL
O	12-4-8	-966	-966	—	FRONT	VERT	TOTAL
P	13-4-12	-24	-43	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.43 (E-F:1) , BC=0.83 (I-J:1) , WB=0.32 (F-I:1) , SSI=0.76 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.36 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45842-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272342	T46	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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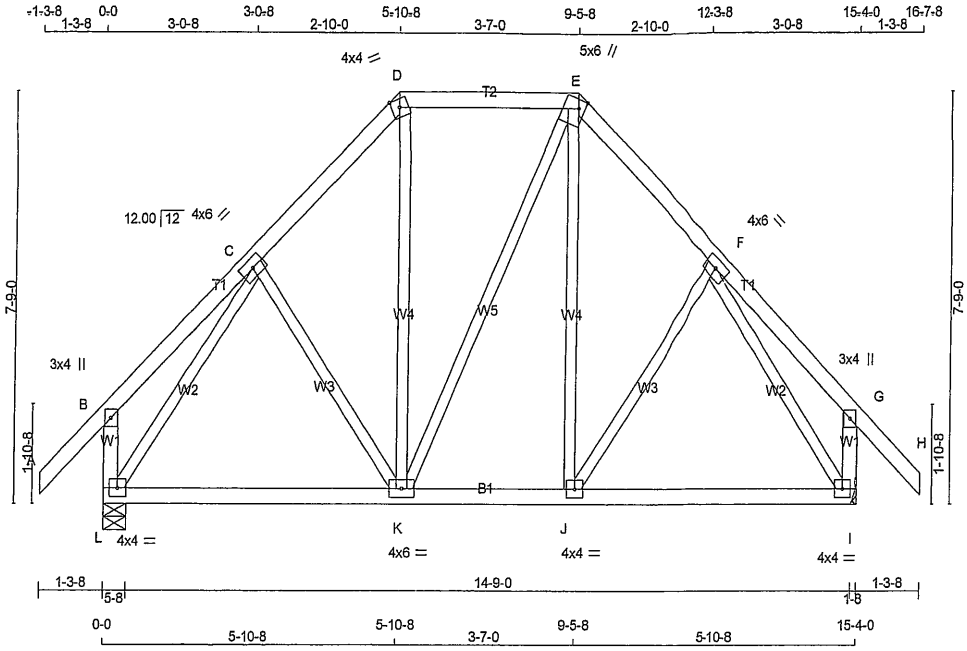

Scale = 1:43.9

TOTAL WEIGHT = 83 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1322	0	1322	0	0	5-8	5-8
I	1322	0	1322	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1016	693 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0
I	1016	693 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 29	-122.2	-122.2	0.16 (1)	10.00
C- D	-838 / 0	-122.2	-122.2	0.13 (1)	6.25
D- E	-573 / 0	-122.2	-122.2	0.20 (1)	6.25
E- F	-837 / 0	-122.2	-122.2	0.13 (1)	6.25
F- G	0 / 29	-122.2	-122.2	0.16 (1)	10.00
G- H	0 / 60	-122.2	-122.2	0.17 (1)	10.00
L- B	-307 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-307 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 618	-28.0	-28.0	0.27 (2)	10.00
K- J	0 / 572	-28.0	-28.0	0.25 (2)	10.00
J- I	0 / 618	-28.0	-28.0	0.27 (2)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-85 / 56	0.04 (1)
K- D	0 / 238	0.05 (2)
K- E	0 / 1	0.00 (2)
J- E	0 / 236	0.05 (2)
J- F	-85 / 55	0.04 (1)
L- C	-1129 / 0	0.51 (1)
F- I	-1128 / 0	0.51 (1)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45843-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.20 (D-E:1) , BC=0.27 (I-J:2) , WB=0.51 (C-L:1) , SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272342	T47	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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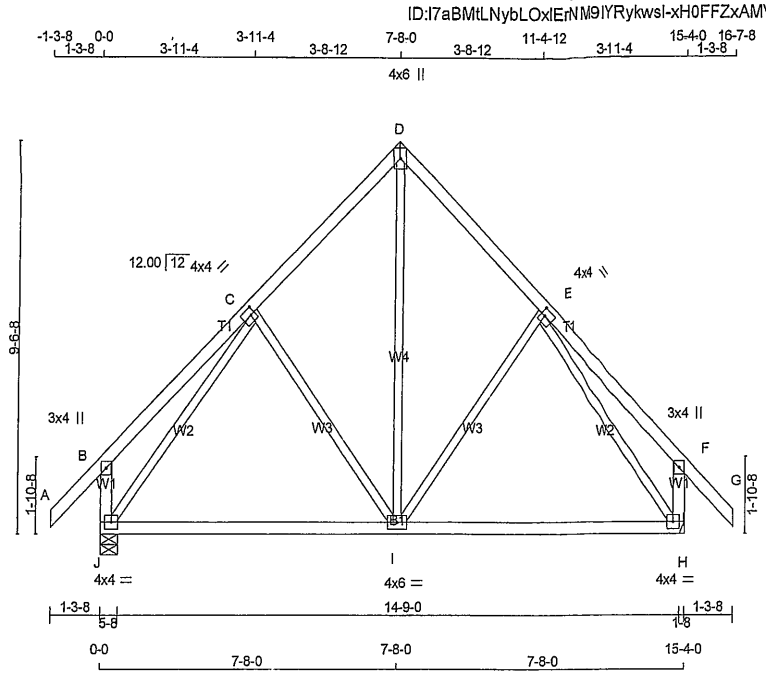

TOTAL WEIGHT = 2 X 78 = 156 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1322	0	1322	0	0	5-8	5-8
H	1322	0	1322	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1016	693 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0
H	1016	693 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00		I- D	0 / 638	0.14 (1)
B- C	0 / 39	-122.2	-122.2	0.29 (1)	10.00		I- E	-230 / 34	0.18 (1)
C- D	-748 / 0	-122.2	-122.2	0.23 (1)	6.25		C- I	-230 / 34	0.18 (1)
D- E	-748 / 0	-122.2	-122.2	0.23 (1)	6.25		J- C	-1098 / 0	0.84 (1)
E- F	0 / 39	-122.2	-122.2	0.29 (1)	10.00		E- H	-1098 / 0	0.84 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00				
J- B	-347 / 0	0.0	0.0	0.04 (1)	7.81				
H- F	-347 / 0	0.0	0.0	0.04 (1)	7.81				
J- I	0 / 637	-28.0	-28.0	0.54 (2)	10.00				
I- H	0 / 637	-28.0	-28.0	0.54 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.29 (E-F:1) , BC=0.54 (I-J:2) , WB=0.84 (C-J:1) , SSI=0.18 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.42 (E) (INPUT = 1.00)

DWG NO. TAM 458494-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272342	T48	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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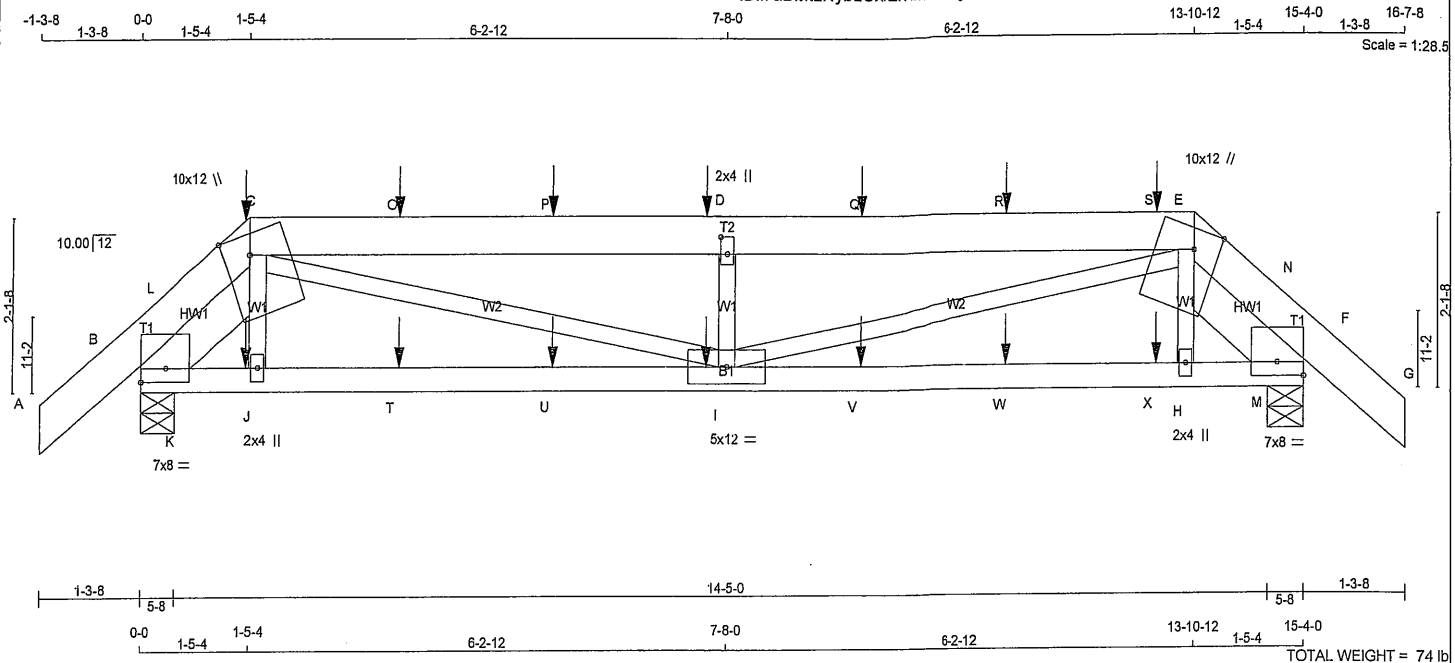

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	7.0	8.0	2.00	Edge
C	TTWWW+m	MT20	10.0	12.0	3.00	4.00
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TTWWW+m	MT20	10.0	12.0	3.00	4.00
F	TMBMW1-I	MT20	7.0	8.0	2.00	Edge
H	BMW+w	MT20	2.0	4.0		
I	BMWWW-t	MT20	5.0	12.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 110.0 lbs FACTORED DOWN AT 1-6-8, 106.8 lbs FACTORED DOWN AT 3-4-12, 106.8 lbs FACTORED DOWN AT 5-4-12, 106.8 lbs FACTORED DOWN AT 7-4-12, 106.8 lbs FACTORED DOWN AT 9-4-12, AND 106.8 lbs FACTORED DOWN AT 11-4-12, AND 143.7 lbs FACTORED DOWN AT 13-4-12 ON TOP CHORD, AND 65.4 lbs FACTORED DOWN AT 1-4-12, 65.4 lbs FACTORED DOWN AT 3-4-12, 65.4 lbs FACTORED DOWN AT 5-4-12, 65.4 lbs FACTORED DOWN AT 7-4-12, 65.4 lbs FACTORED DOWN AT 9-4-12, AND 65.4 lbs FACTORED DOWN AT 11-4-12, AND 65.4 lbs FACTORED DOWN AT 13-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1954	0	1954	0	0	5-8	5-8
F	1946	0	1946	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1509	1014 / 0	248 / 0	0 / 0	0 / 0	247 / 0	0 / 0
F	1499	1015 / 0	243 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-122.2	-122.2	0.10 (1)	10.00	J- C	0 / 476	0.12 (2)
B- L	-2343 / 0	-122.2	-122.2	0.43 (1)	4.77	H- E	0 / 479	0.12 (2)
L- C	-2275 / 0	-122.2	-122.2	0.18 (1)	6.25	I- E	0 / 2863	0.71 (1)
C- O	-4255 / 0	-122.2	-122.2	0.57 (1)	3.56	C- I	0 / 2863	0.71 (1)
O- P	-4255 / 0	-122.2	-122.2	0.57 (1)	3.56	I- D	-1235 / 0	0.20 (1)
P- D	-4255 / 0	-122.2	-122.2	0.57 (1)	3.56	K- L	-116 / 0	0.00 (1)
D- Q	-4255 / 0	-122.2	-122.2	0.61 (1)	3.50	M- N	-130 / 0	0.00 (1)
Q- R	-4255 / 0	-122.2	-122.2	0.61 (1)	3.50			
R- S	-4255 / 0	-122.2	-122.2	0.61 (1)	3.50			
S- E	-4255 / 0	-122.2	-122.2	0.61 (1)	3.50			
E- N	-2280 / 0	-122.2	-122.2	0.18 (1)	6.25			
N- F	-2333 / 0	-122.2	-122.2	0.44 (1)	4.77			
F- G	0 / 39	-122.2	-122.2	0.10 (1)	10.00			
B- K	0 / 1502	-28.0	-28.0	0.34 (1)	10.00			
K- J	0 / 1502	-28.0	-28.0	0.55 (1)	10.00			
J- T	0 / 1502	-28.0	-28.0	0.55 (1)	10.00			
T- U	0 / 1502	-28.0	-28.0	0.55 (1)	10.00			
U- I	0 / 1502	-28.0	-28.0	0.55 (1)	10.00			
I- V	0 / 1502	-28.0	-28.0	0.57 (1)	10.00			
V- W	0 / 1502	-28.0	-28.0	0.57 (1)	10.00			
W- X	0 / 1502	-28.0	-28.0	0.57 (1)	10.00			
X- H	0 / 1502	-28.0	-28.0	0.57 (1)	10.00			
H- M	0 / 1502	-28.0	-28.0	0.57 (1)	10.00			
M- F	0 / 1502	-28.0	-28.0	0.34 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-6-8	-110	-110	---	BACK	VERT	TOTAL
D	7-4-12	-107	-107	---	BACK	VERT	TOTAL
I	7-4-12	-65	-65	---	BACK	VERT	TOTAL
J	1-4-12	-65	-65	---	BACK	VERT	TOTAL
O	3-4-12	-107	-107	---	BACK	VERT	TOTAL
P	5-4-12	-107	-107	---	BACK	VERT	TOTAL
Q	9-4-12	-107	-107	---	BACK	VERT	TOTAL
R	11-4-12	-107	-107	---	BACK	VERT	TOTAL
S	13-4-12	-144	-144	---	BACK	VERT	TOTAL
T	3-4-12	-65	-65	---	BACK	VERT	TOTAL
U	5-4-12	-65	-65	---	BACK	VERT	TOTAL
V	9-4-12	-65	-65	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 9117 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 45845-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 671 (0.27")

CSI: TC=0.61 (D-E:1) , BC=0.57 (H-I:1) , WB=0.71 (C-I:1) , SSI=0.38 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.48 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272342	T48	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	11-4-12	-65	-65	—	BACK	VERT	TOTAL
X	13-4-12	-65	-65	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45845-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272357	T101	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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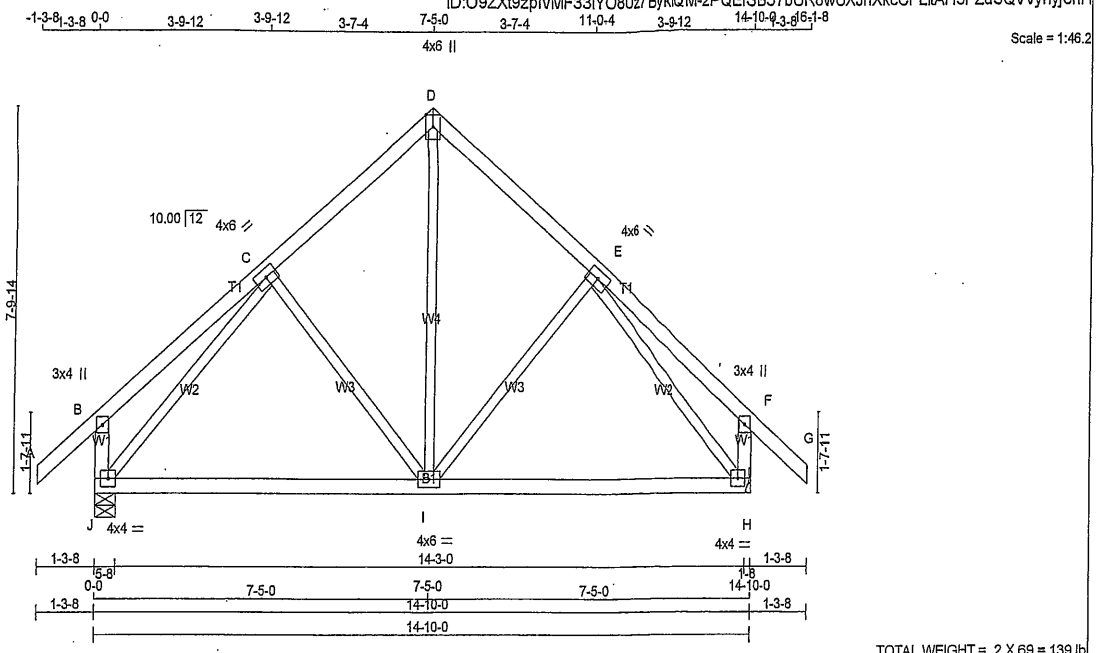


TOTAL WEIGHT = 2 X 69 = 139 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1283	0	1283	0	0	IN-SX 5-8	IN-SX 1-8
H	1283	0	1283	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	985	673 / 0	156 / 0	0 / 0	0 / 0	157 / 0	0 / 0
H	985	673 / 0	156 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

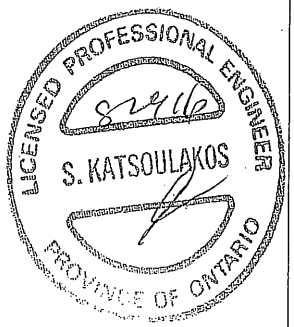
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 608	0.14 (1)
B- C	0 / 34	-122.2	-122.2	0.27 (1)	10.00	I- E	-231 / 38	0.13 (1)
C- D	-783 / 0	-122.2	-122.2	0.21 (1)	6.25	C- I	-231 / 38	0.13 (1)
D- E	-783 / 0	-122.2	-122.2	0.21 (1)	6.25	J- C	-1128 / 0	0.61 (1)
E- F	0 / 34	-122.2	-122.2	0.27 (1)	10.00	E- H	-1128 / 0	0.61 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
J- B	-341 / 0	0.0	0.0	0.04 (1)	7.81			
H- F	-341 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 726	-28.0	-28.0	0.52 (2)	10.00			
I- H	0 / 726	-28.0	-28.0	0.52 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.27 (B-C:1) , BC=0.52 (I-J:2) , WB=0.61 (C-J:1) , SSI=0.18 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)

DWG NO. TAM39265 -16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272357	G101	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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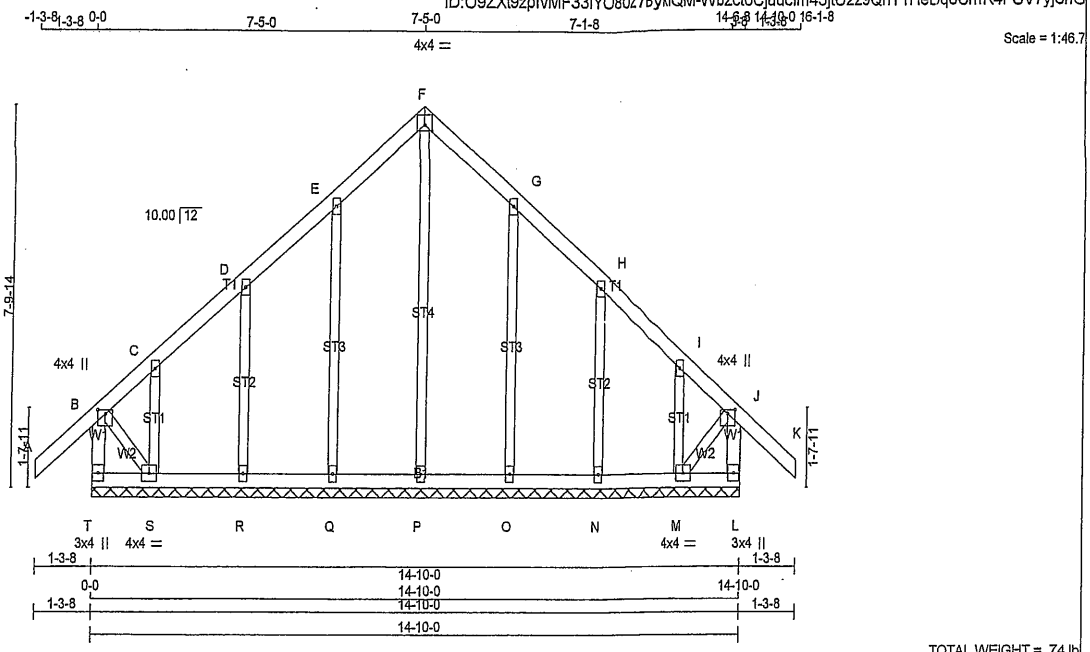

TOTAL WEIGHT = 74 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-379 / 0	0.0	0.0	0.04 (1)		7.81
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	-87 / 0	-122.2	-122.2	0.16 (1)		6.25
C- D	-22 / 0	-122.2	-122.2	0.06 (1)		6.25
D- E	-22 / 0	-122.2	-122.2	0.07 (1)		6.25
E- F	-34 / 0	-122.2	-122.2	0.07 (1)		6.25
F- G	-34 / 0	-122.2	-122.2	0.07 (1)		6.25
G- H	-22 / 0	-122.2	-122.2	0.07 (1)		6.25
H- I	-22 / 0	-122.2	-122.2	0.06 (1)		6.25
I- J	-87 / 0	-122.2	-122.2	0.16 (1)		6.25
J- K	0 / 54	-122.2	-122.2	0.17 (1)		10.00
L- J	-379 / 0	0.0	0.0	0.04 (1)		7.81
T- S	0 / 0	-28.0	-28.0	0.02 (2)		10.00
S- R	0 / 25	-28.0	-28.0	0.02 (2)		10.00
R- Q	0 / 19	-28.0	-28.0	0.02 (2)		10.00
Q- P	0 / 14	-28.0	-28.0	0.02 (2)		10.00
P- O	0 / 14	-28.0	-28.0	0.02 (2)		10.00
O- N	0 / 19	-28.0	-28.0	0.02 (2)		10.00
N- M	0 / 25	-28.0	-28.0	0.02 (2)		10.00
M- L	0 / 0	-28.0	-28.0	0.02 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-183 / 0	0.20 (1)
Q- E	-274 / 0	0.16 (1)
R- D	-251 / 0	0.07 (1)
S- C	-107 / 0	0.02 (1)
O- G	-274 / 0	0.16 (1)
N- H	-251 / 0	0.07 (1)
M- I	-107 / 0	0.02 (1)
B- S	0 / 39	0.01 (1)
M- J	0 / 39	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.02 (M-N:2) , WB=0.20 (F-P:1) , SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (F) (INPUT = 0.90)
JSI METAL= 0.07 (G) (INPUT = 1.00)

DWG NO. TAM39266-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272342	T210	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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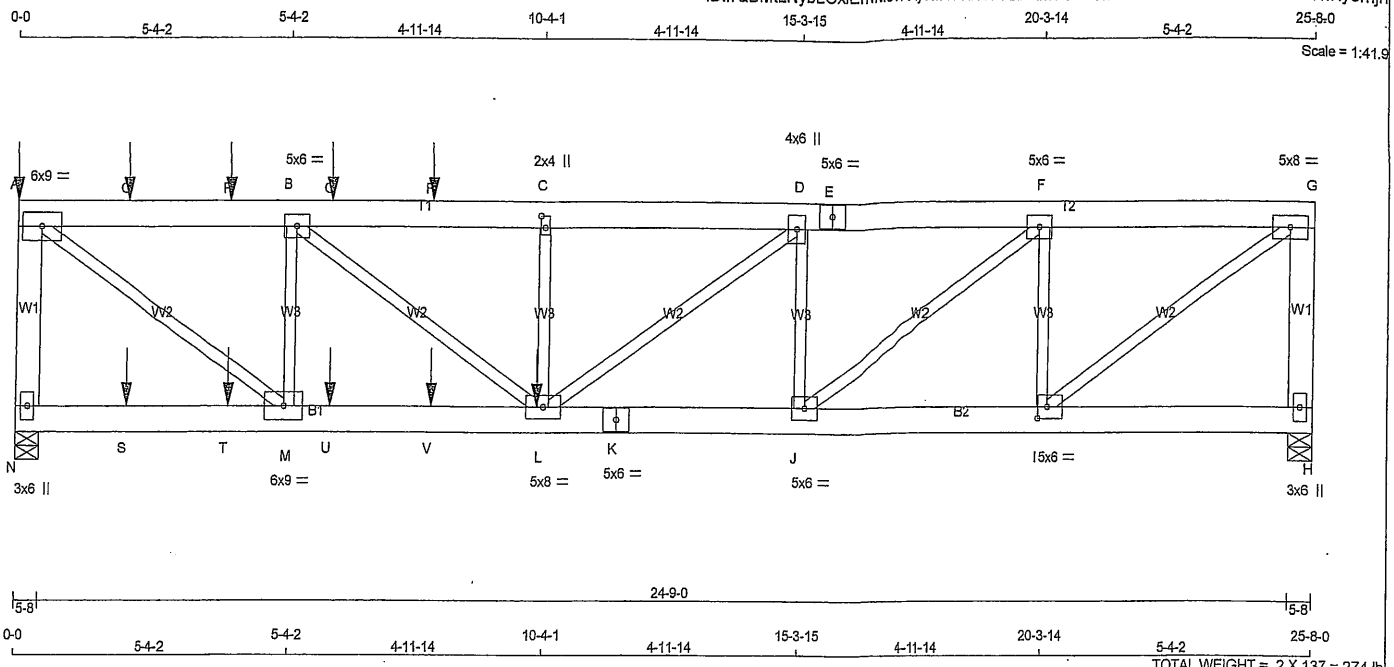


[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x6	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
N- A	2	12	TOP
A- E	2	12	SIDE(61.0)
E- G	2	12	TOP
G- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	SIDE(183.1)
K- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
L- C	1	5	SIDE(345.1)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0		
B	TMWW-t	MT20	5.0	6.0		
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.25
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	6.0	9.0		
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	3935	0	3935	0	0	5-8	5-8
H	2893	0	2893	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3034	2050 / 0	494 / 0	0 / 0	0 / 0	490 / 0	0 / 0
H	2244	1493 / 0	380 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
N- A	-3840 / 0	0.0	0.0	0.28 (1)	7.22
A- O	-4811 / 0	-122.2	-122.2	0.23 (1)	5.07
O- P	-4811 / 0	-122.2	-122.2	0.23 (1)	5.07
P- B	-4811 / 0	-122.2	-122.2	0.23 (1)	5.07
B- Q	-7306 / 0	-122.2	-122.2	0.27 (1)	4.24
Q- R	-7306 / 0	-122.2	-122.2	0.27 (1)	4.24
R- C	-7306 / 0	-122.2	-122.2	0.27 (1)	4.24
C- D	-7306 / 0	-122.2	-122.2	0.19 (1)	4.33
D- E	-6047 / 0	-122.2	-122.2	0.17 (1)	4.70
E- F	-6047 / 0	-122.2	-122.2	0.17 (1)	4.70
F- G	-3730 / 0	-122.2	-122.2	0.14 (1)	5.70
H- G	-2820 / 0	0.0	0.0	0.21 (1)	7.81
N- S	0 / 0	-28.0	-28.0	0.08 (3)	10.00
S- T	0 / 0	-28.0	-28.0	0.08 (3)	10.00
T- M	0 / 0	-28.0	-28.0	0.08 (3)	10.00
M- U	0 / 4811	-28.0	-28.0	0.39 (1)	10.00
U- V	0 / 4811	-28.0	-28.0	0.39 (1)	10.00
V- L	0 / 4811	-28.0	-28.0	0.39 (1)	10.00
L- K	0 / 6047	-28.0	-28.0	0.46 (1)	10.00
K- J	0 / 6047	-28.0	-28.0	0.46 (1)	10.00
J- I	0 / 3730	-28.0	-28.0	0.27 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.05 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	0 / 4515	0.56 (1)
A- M	0 / 5824	0.72 (1)
I- F	-2395 / 0	0.29 (1)
M- B	-2979 / 0	0.36 (1)
J- F	0 / 2869	0.36 (1)
B- L	0 / 3089	0.38 (1)
J- D	-1523 / 0	0.18 (1)
L- C	-626 / 0	0.07 (1)
L- D	0 / 1559	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-199	-199	—	BACK	VERT	TOTAL
L	10-2-8	-2025	-2025	—	BACK	VERT	TOTAL
O	2-1-12	-147	-147	—	BACK	VERT	TOTAL
P	4-1-12	-147	-147	—	BACK	VERT	TOTAL
Q	6-1-12	-147	-147	—	BACK	VERT	TOTAL
R	8-1-12	-147	-147	—	BACK	VERT	TOTAL
S	2-1-12	-40	-70	—	BACK	VERT	TOTAL
T	4-1-12	-40	-70	—	BACK	VERT	TOTAL
U	6-1-12	-40	-70	—	BACK	VERT	TOTAL
V	8-1-12	-40	-70	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.28 (A-N:1) , BC=0.46 (J-L:1) , WB=0.72 (A-M:1) , SSI=0.19 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.52 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45846-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272342	T210	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 199.3 lbs FACTORED DOWN AT 0-0, 147.1 lbs FACTORED DOWN AT 2-1-12, 147.1 lbs FACTORED DOWN AT 4-1-12, AND 147.1 lbs FACTORED DOWN AT 6-1-12, AND 147.1 lbs FACTORED DOWN AT 8-1-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-12, 69.9 lbs FACTORED DOWN AT 4-1-12, 69.9 lbs FACTORED DOWN AT 6-1-12, AND 69.9 lbs FACTORED DOWN AT 8-1-12, AND 2024.9 lbs FACTORED DOWN AT 10-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

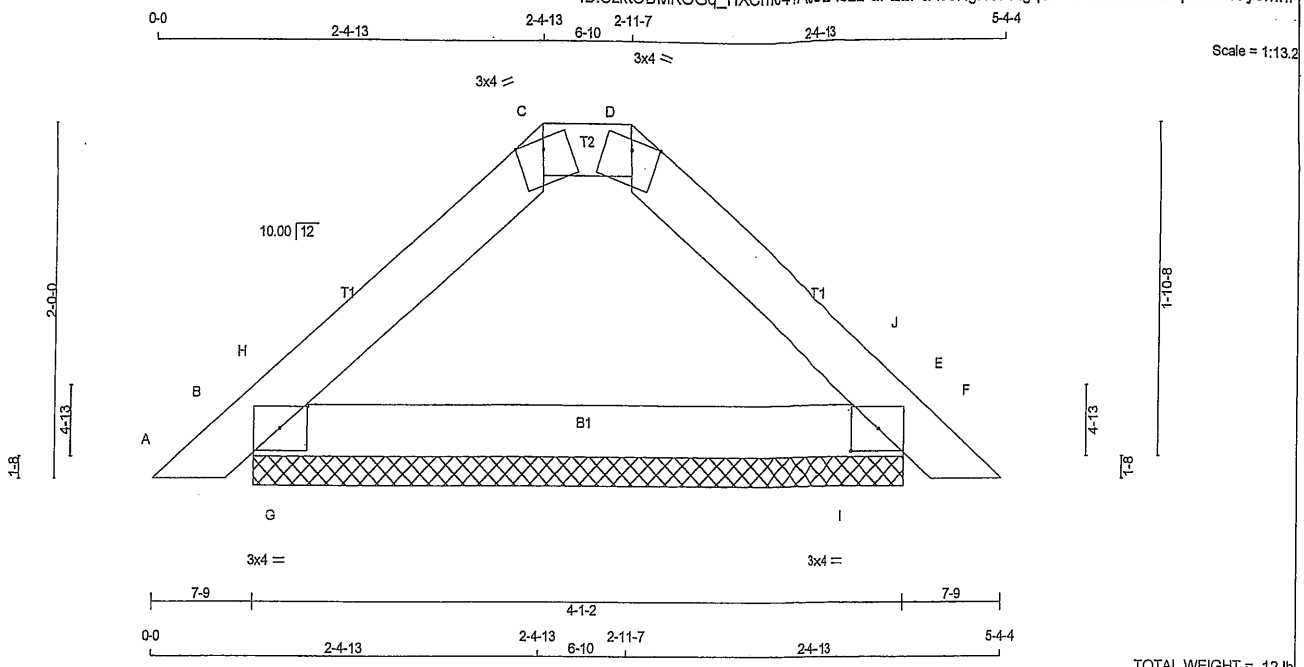
LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45846-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	PB1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:SzktOBMKOGq_HXCm04?AJ5zAcL2-dPE2PcA93RgNSPMgqS5eC9LRwkYa4EfqS5Z3nUyemnF



Scale = 1:13.2

TOTAL WEIGHT = 12 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TT-m	MT20	3.0	4.0	Edge	
D	TT-m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	373	0	373	0	0	4-1-2	4-1-2
E	373	0	373	0	0	4-1-2	4-1-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	285	198 / 0	43 / 0	0 / 0	0 / 0	44 / 0	0 / 0
E	285	198 / 0	43 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-122.2	-122.2	0.03 (1)	10.00	G- H	-122 / 52	0.00 (1)
B- H	-42 / 30	-122.2	-122.2	0.06 (2)	6.25	I- J	-122 / 52	0.00 (1)
H- C	-192 / 0	-122.2	-122.2	0.03 (1)	6.25			
C- D	-144 / 0	-122.2	-122.2	0.02 (1)	6.25			
D- J	-192 / 0	-122.2	-122.2	0.03 (1)	6.25			
J- E	-42 / 30	-122.2	-122.2	0.06 (2)	6.25			
E- F	0 / 19	-122.2	-122.2	0.03 (1)	10.00			
B- G	-117 / 0	-28.0	-28.0	0.06 (1)	6.25			
G- I	0 / 144	-28.0	-28.0	0.08 (2)	10.00			
I- E	-117 / 0	-28.0	-28.0	0.06 (1)	6.25			

DWG NO. TAM45833-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06 (B-H:2) , BC=0.08 (G-I:2) , WB=0.00 (G-H:1) , SSI=0.14 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

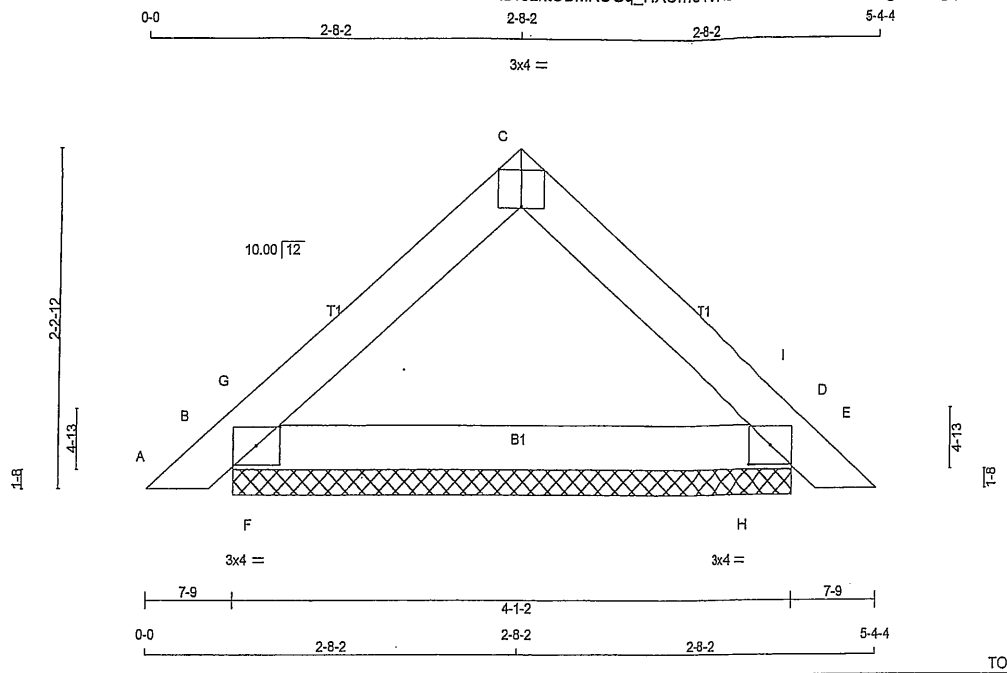
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	PB2	11	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:SzktOBMKOGq_HXCm04?AJ5zAcL2-dPE2PcA93RgNSPMgqS5eC9LQQkYM4EfqS5Z3nUyemnF



Scale = 1:15.4

TOTAL WEIGHT = 11 X 13 = 139 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	373	0	373	0	0	4-1-2	4-1-2
D	373	0	373	0	0	4-1-2	4-1-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	285	198 / 0	43 / 0	0 / 0	0 / 0	44 / 0	0 / 0
D	285	198 / 0	43 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-122.2	-122.2	0.03 (1)	10.00	F- G	0 / 214	0.00 (1)
B- G	-307 / 0	-122.2	-122.2	0.06 (2)	6.25	H- I	0 / 214	0.00 (1)
G- C	-186 / 0	-122.2	-122.2	0.09 (1)	6.25			
C- I	-186 / 0	-122.2	-122.2	0.09 (1)	6.25			
I- D	-307 / 0	-122.2	-122.2	0.06 (2)	6.25			
D- E	0 / 19	-122.2	-122.2	0.03 (1)	10.00			
B- F	0 / 150	-28.0	-28.0	0.05 (3)	10.00			
F- H	0 / 150	-28.0	-28.0	0.09 (2)	10.00			
H- D	0 / 150	-28.0	-28.0	0.05 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09 (C-I:1) , BC=0.09 (F-H:2) , WB=0.00 (F-G:1) , SSI=0.10 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

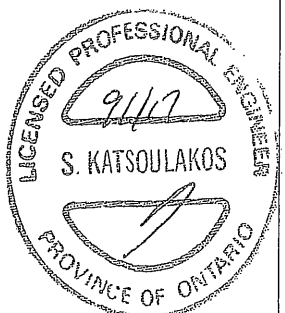
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

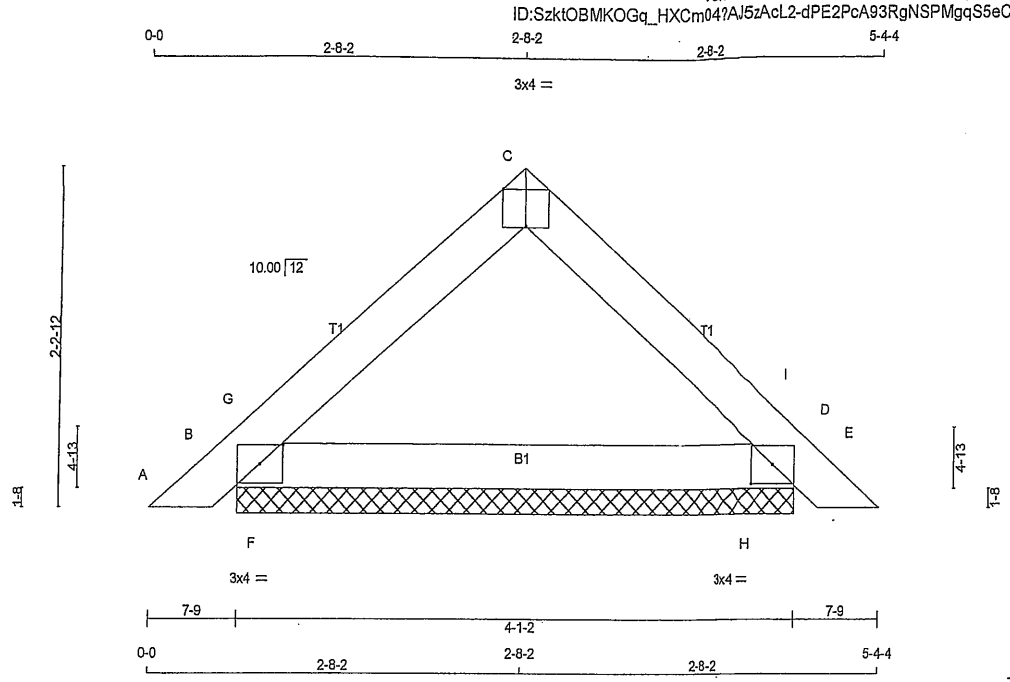


DWG NO. TAM 45834-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	PB2Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:SzktOBMKOGq_HXCm04?AJ5zAcL2-dPE2PcA93RgNSPMgqS5eC9LR7kZ34EfqS5Z3nUyemnF



Scale = 1:15.4

TOTAL WEIGHT = 2 X 13 = 25 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	373	0	373	0	0	4-1-2	4-1-2
D	373	0	373	0	0	4-1-2	4-1-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	285	198 / 0	43 / 0	0 / 0	0 / 0	44 / 0	0 / 0
D	285	198 / 0	43 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-122.2	-122.2	0.01 (1)	10.00	F- G	0 / 211	0.00 (1)
B- G	-307 / 0	-122.2	-122.2	0.03 (2)	6.25	H- I	0 / 211	0.00 (1)
G- C	-186 / 0	-122.2	-122.2	0.04 (1)	6.25			
C- I	-186 / 0	-122.2	-122.2	0.04 (1)	6.25			
I- D	-307 / 0	-122.2	-122.2	0.03 (2)	6.25			
D- E	0 / 19	-122.2	-122.2	0.01 (1)	10.00			
B- F	0 / 150	-28.0	-28.0	0.02 (3)	10.00			
F- H	0 / 150	-28.0	-28.0	0.05 (2)	10.00			
H- D	0 / 150	-28.0	-28.0	0.02 (3)	10.00			

DWG NO. TAM45835-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (C-I:1) , BC=0.05 (F-H:2) , WB=0.00 (F-G:1) , SSI=0.05 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (D) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266142	PB3	14	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MiTek Industries, Inc. Thu Jun 02 10:29:29 2016 Page 1

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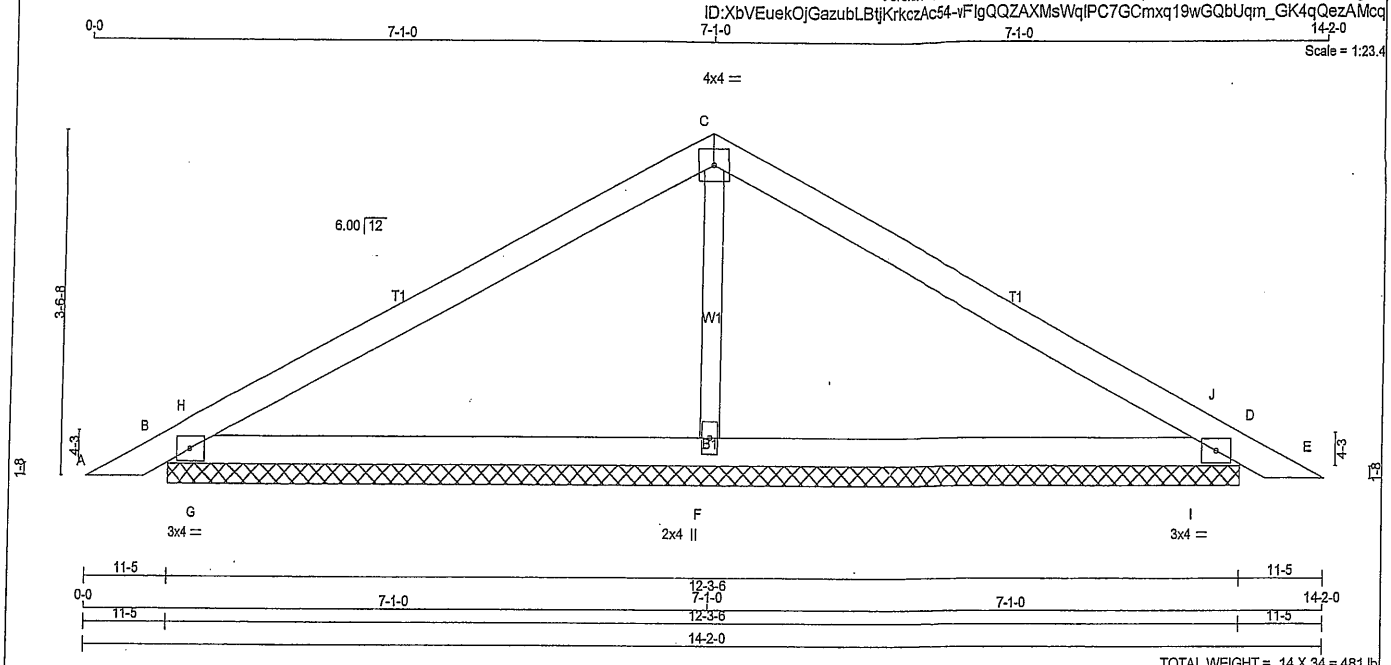


TOTAL WEIGHT = 14 X 34 = 481 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	654	0	654	0	0	12-3-6	1-8
D	654	0	654	0	0	12-3-6	1-8
F	751	0	751	0	0	12-3-6	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	488	358 / 0	61 / 0	0 / 0	0 / 0	69 / 0	0 / 0
D	488	358 / 0	61 / 0	0 / 0	0 / 0	69 / 0	0 / 0
F	611	357 / 0	136 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-122.2	-122.2	0.07 (1)	10.00
B- H	-136 / 0	-122.2	-122.2	0.27 (1)	6.25
H- C	-315 / 0	-122.2	-122.2	0.57 (1)	6.25
C- J	-315 / 0	-122.2	-122.2	0.57 (1)	6.25
J- D	-136 / 0	-122.2	-122.2	0.27 (1)	6.25
D- E	0 / 22	-122.2	-122.2	0.07 (1)	10.00
B- G	0 / 268	-28.0	-28.0	0.51 (1)	10.00
G- F	0 / 268	-28.0	-28.0	0.51 (1)	10.00
F- I	0 / 268	-28.0	-28.0	0.51 (1)	10.00
I- D	0 / 268	-28.0	-28.0	0.51 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-365 / 0	0.07 (1)
G- H	-864 / 118	0.00 (1)
I- J	-864 / 118	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.57 (C-H:1) , BC=0.51 (B-G:1) , WB=0.07 (C-F:1) , SSI=0.66 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

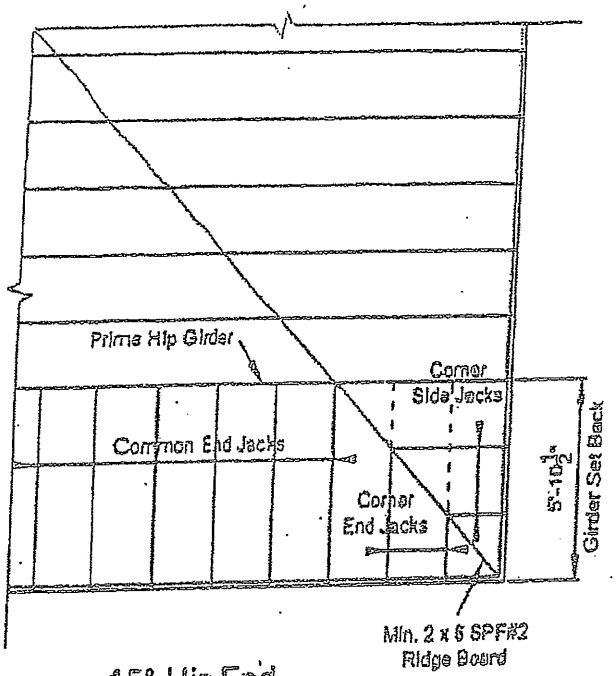
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (B) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

DWG NO. TAM2509-16
STRUCTURAL
COMPONENT ONLY

MICRO CITY
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

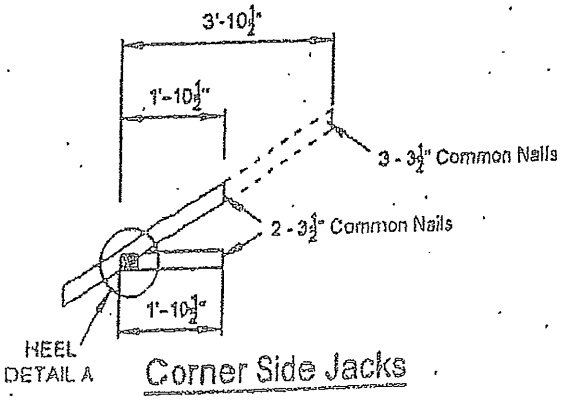
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

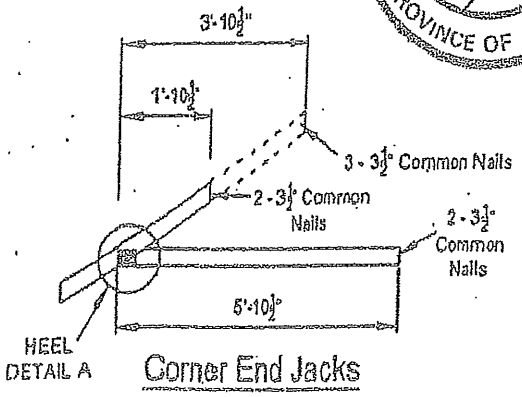
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14 R1
STRUCTURAL COMPONENT ONLY

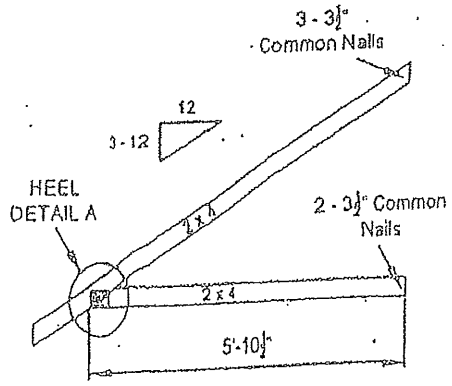
LICENSED PROFESSIONAL ENGINEER
1-2-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



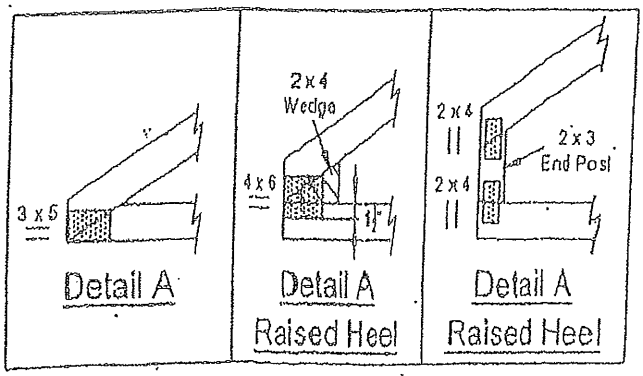
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

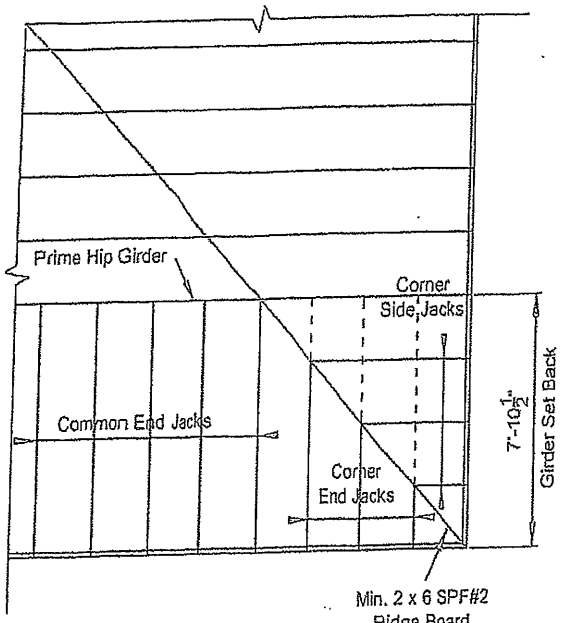
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



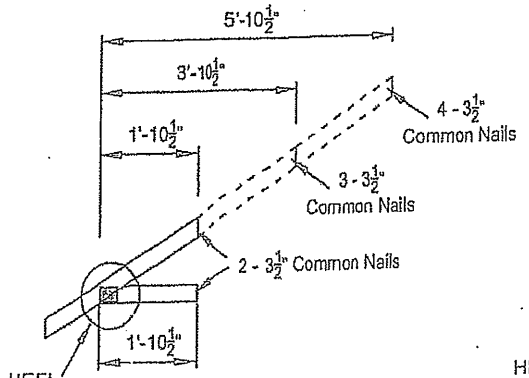
45° Hip End

DESIGN LOAD:

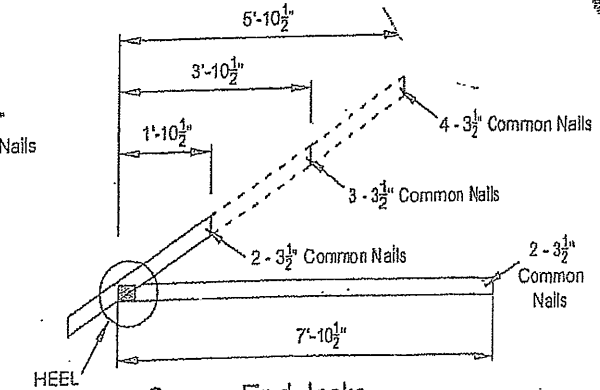
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3503.14
STRUCTURAL COMPONENT ONLY

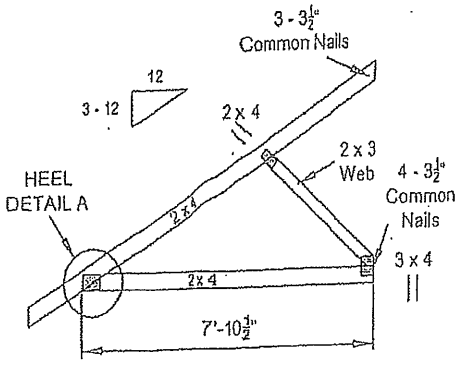
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



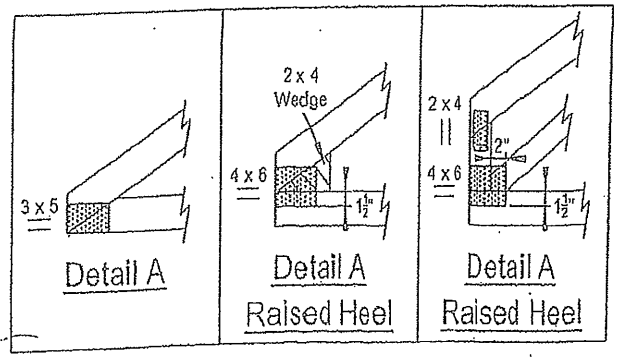
Corner Side Jacks



Corner End Jacks



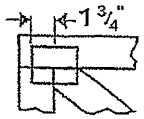
Common End Jacks



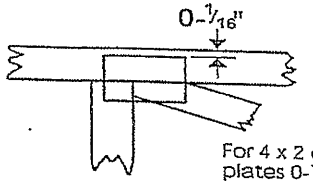
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

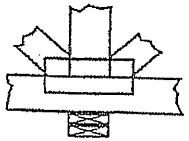
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

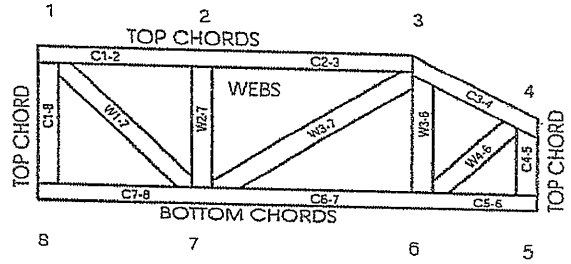
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

· CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

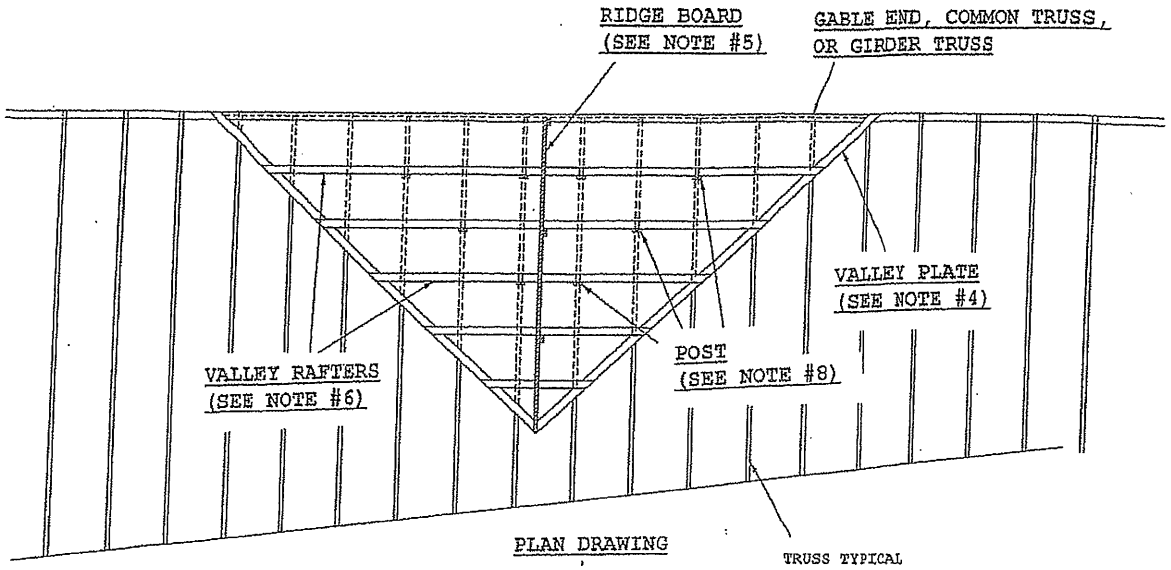
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY

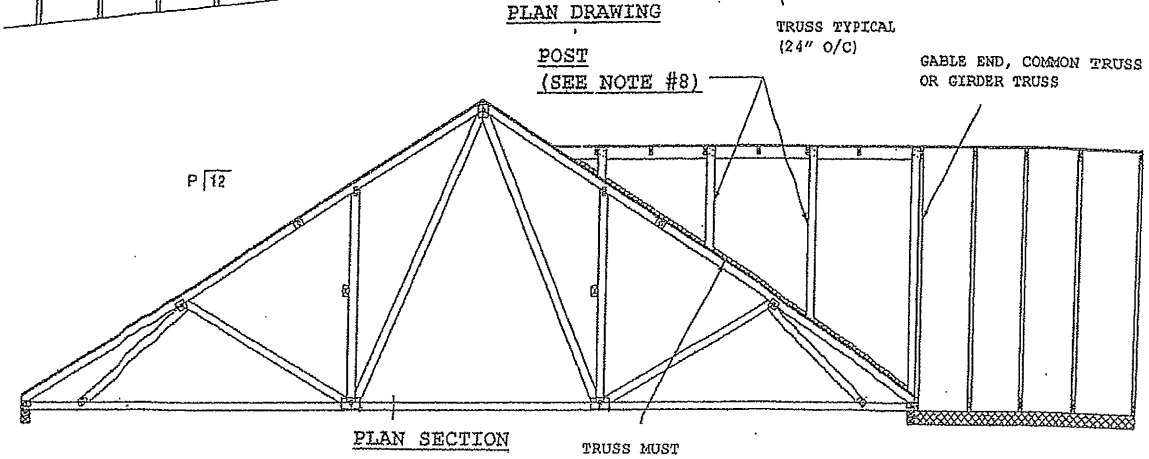
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

TRUSS MUST
BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-74
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2019
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO [illegible] 7/24/14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

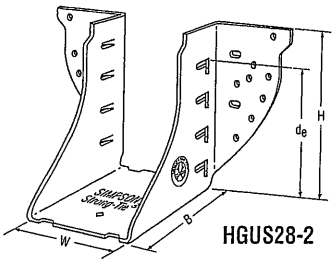
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

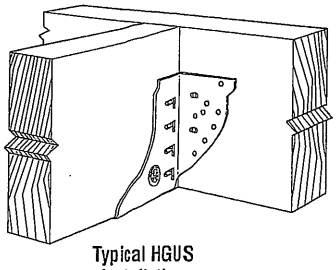
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

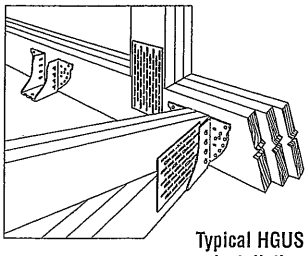
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications


Typical HGUS Installation

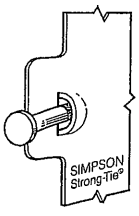
OPTIONS:

- See current catalogue for options


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

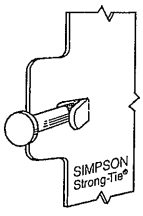
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝32	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅝16	5⅞16	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4⅝16	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅝16	5⅞16	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅝16	7⅜16	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅝16	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅝16	7⅜16	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅝16	9⅜16	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4⅝16	9¼	4	8⅜	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⅝16	9⅜16	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅝16	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅝16	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

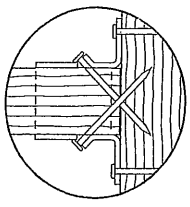


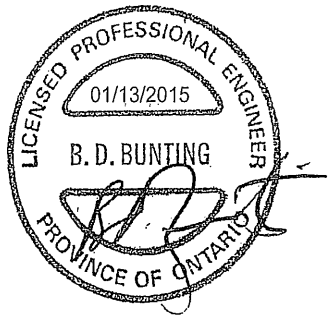
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

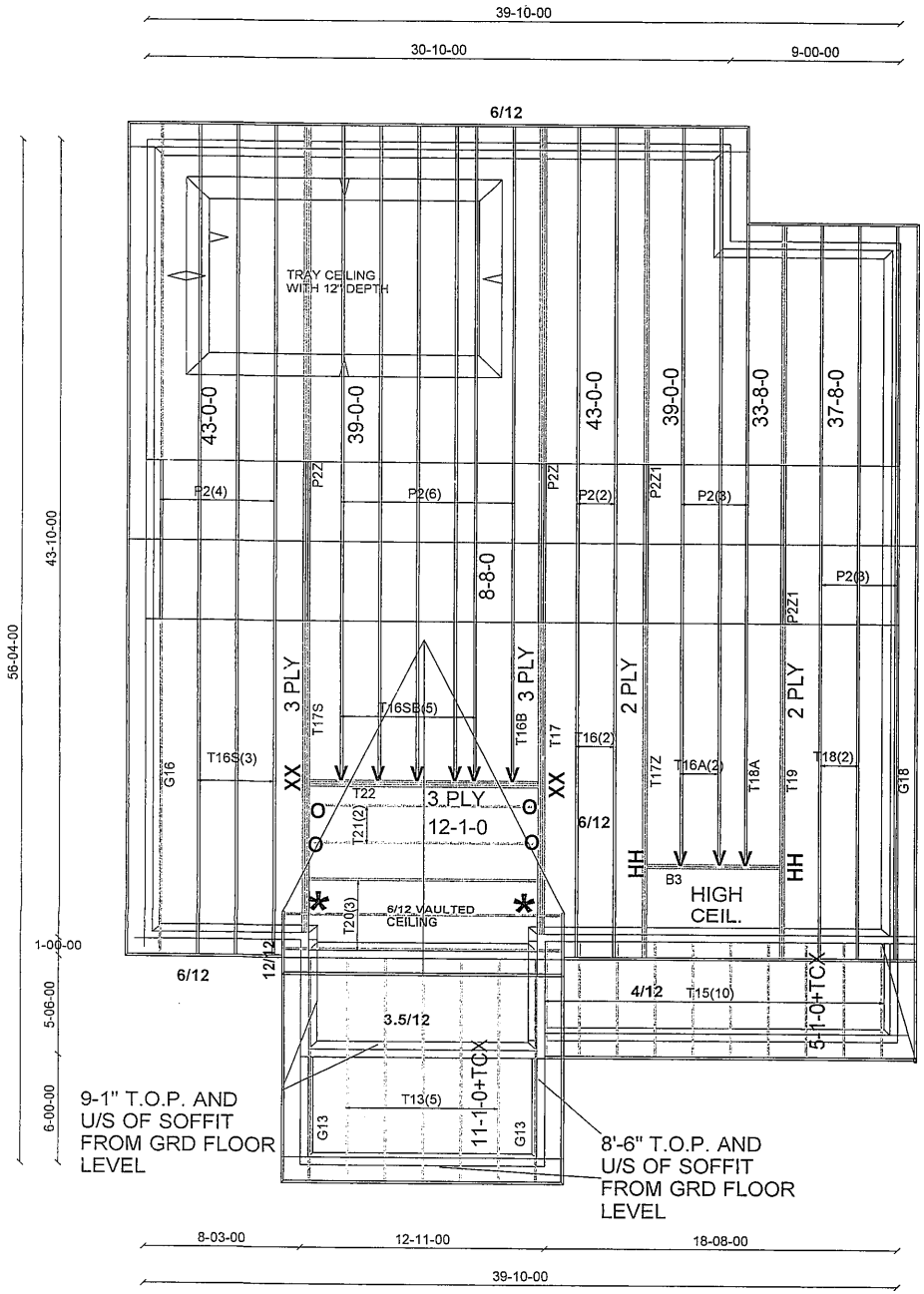


LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

800-999-5099
www.strongtie.com

Town of Innisfil Certified Model
14/02/2018 10:13:23 AM kgervais



NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

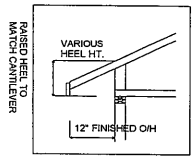
HARDWARE:

LJS26DS - (V)
LUS24 -(O)
HGUS28-3 - (XXX)
HGUS46 -(HH)

CONV.
FRAMING

B3-DENOTES
2-1 3/4X9 1/2"LVLBM
(FLUSH)

DENOTES:
1'-4" KNEE WALL
NAILED TO B.C. OF
GIRDER *



T-170681

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **42067**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **S48-1 GOLDENEYE 1 B OR B W.O.D.** Quote Folder
	Layout ID: **273416**	Project: ALCONA SHORES	
	Plan Log: **88256**	Date: **9/30/2013** Designer: **PETER/ELLA**	

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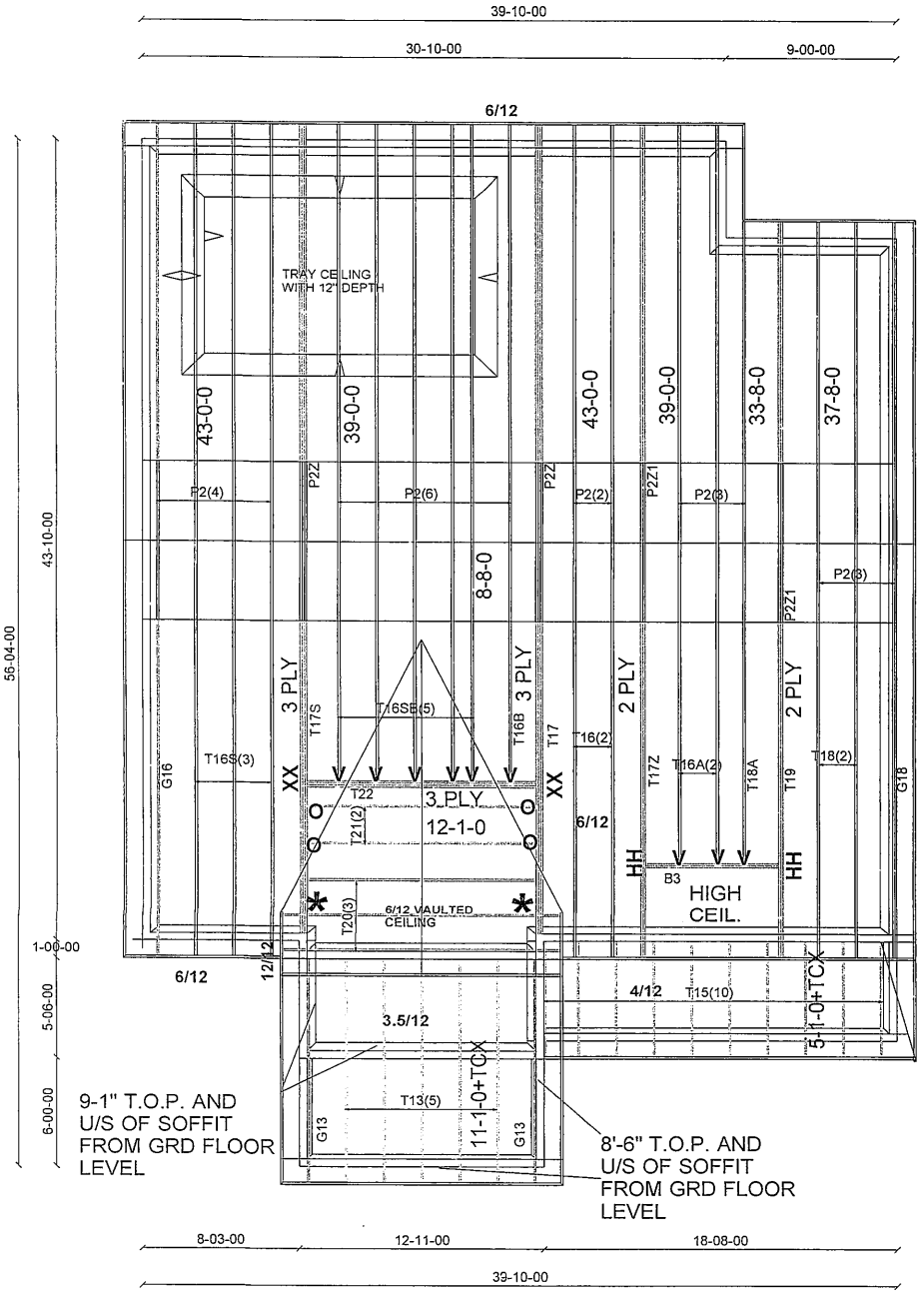

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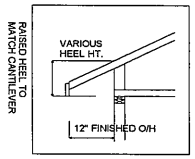
HARDWARE:

LJS26DS - (V)
LUS24 -(O)
HGUS28-3 - (XXX)
HGUS46 -(HH)

CONV. FRAMING

B3-DENOTES
2-1 3/4X9 1/2"LVLBM
(FLUSH)

DENOTES:
1'-4" KNEE WALL
NAILED TO B.C. OF
GIRDER *



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