

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4'SP@24"o.c. WITH A 2"X4'SP VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

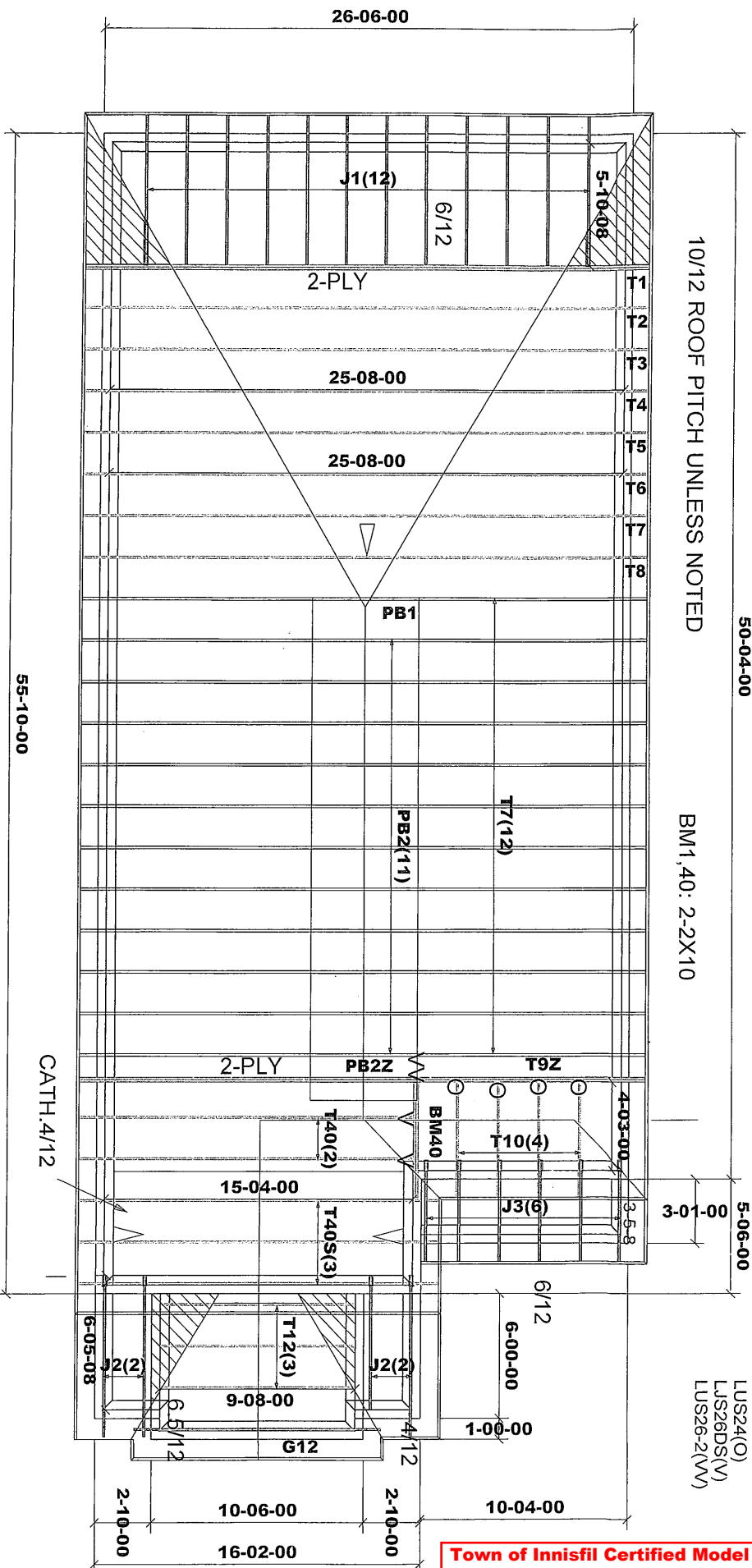
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa


TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

 DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(V)



Town of Innisfil Certified Model
05/01/2018 2:10:47 PM kgervais

 TAMARACK LUMBER CO. INC. ALUMINUM ROOF GROUP		Mill 004	
Job Track:	42067	Builder / Location:	
Layout ID:	272340	BAYVIEW WELLINGTON / INNISFIL	
Plan Log:	87565	Project:	ALCONA SHORES
Date:	9/9/2017	Designer:	JG
		Model / Elevation:	
		S32-5-10G / A	
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC. AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
Mtek ver 7.5.0			


12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART 9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING



10/12 ROOF PITCH UNLESS NOTED

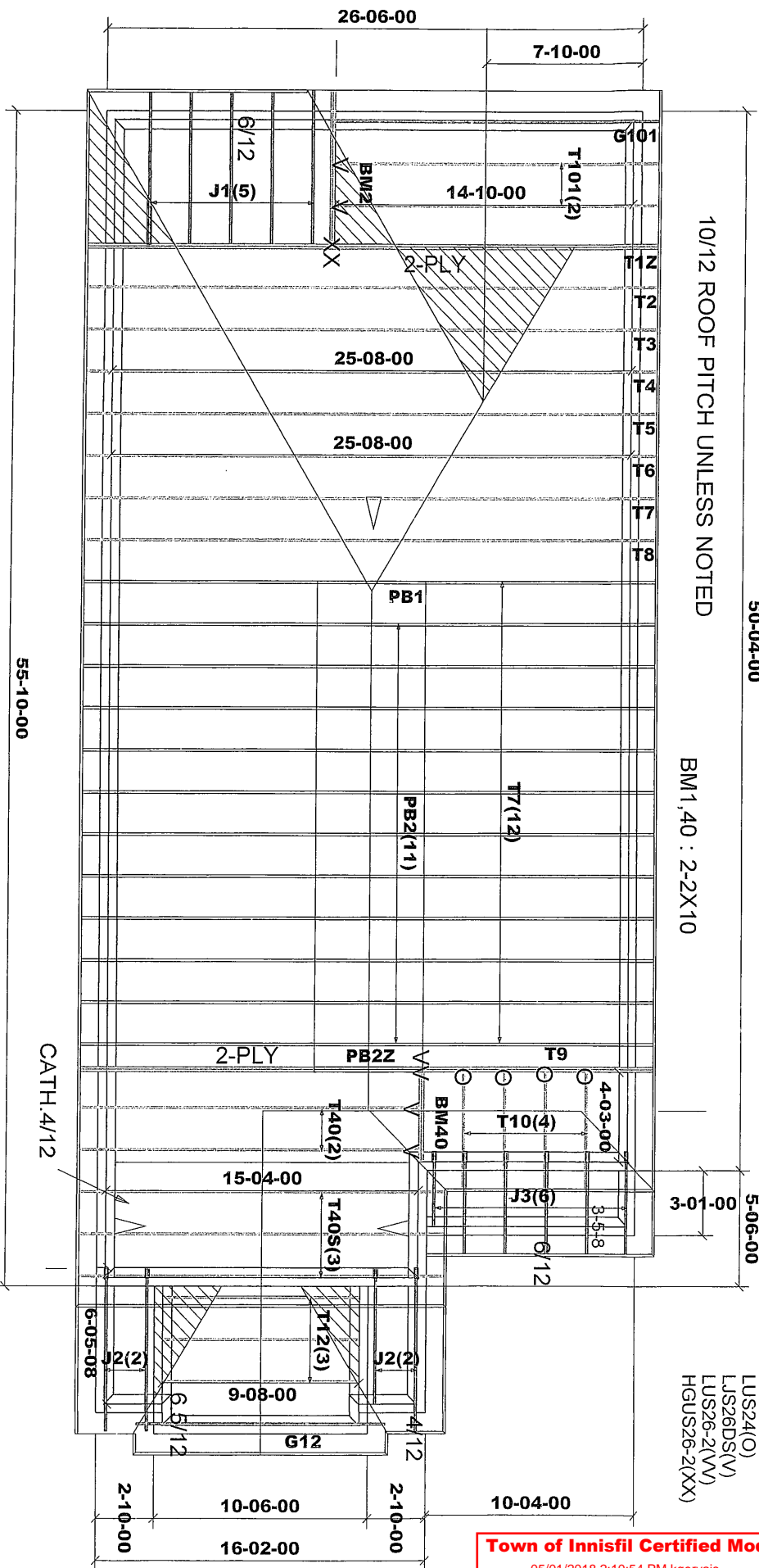
BM1,40 : 2-2X10

50-04-00

5-06-00

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(V)
HGUS26-2(XX)

Town of Innisfil Certified Model
05/01/2018 2:10:54 PM kgervais



mi 004

TAMARACK
LUMBER INC.
ALF. LUMBER GROUP

Job Track:	42067
Layout ID:	272341
Plan Log:	875665

Builder / Location:

BAYVIEW WELLINGTON / INNISFILL	THESE
ALCONA SHORES	OR RE

Model / Elevation:

S32-5-10G / A-REAR UPGRADE

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Milex, ver 7.5.0

Mitek ver 7.5.0

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss = 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

 DENOTES CONVENTIONAL FRAMING

HARDWARE
HGUS26-2(XX)
LJS26DS(V)
LUS24-2(OO)

OPT.REAR
UPGRADE

12/12 ROOF PITCH UNLESS NOTED

BM21: 2-2X10

50-04-00

5-06-00

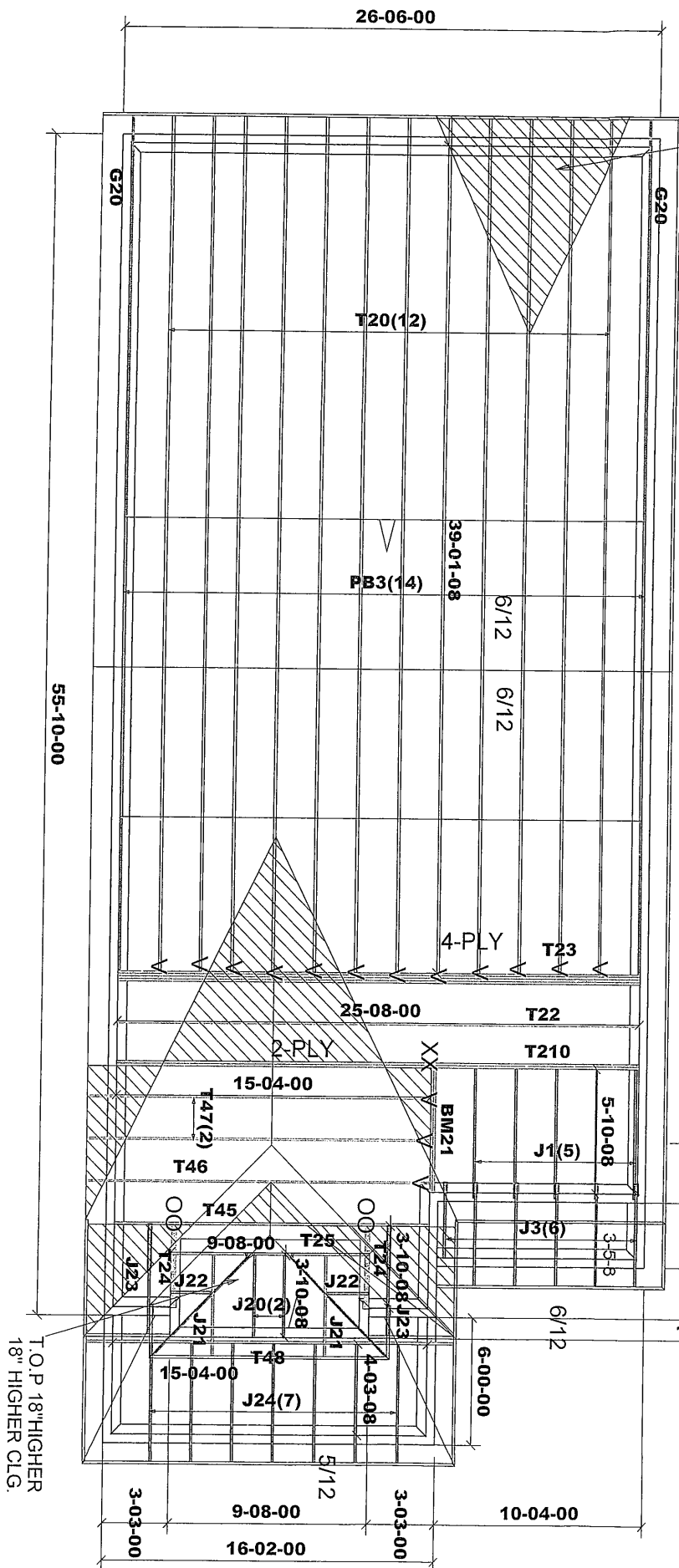
3-01-00

1-00-00

HARDWARE
HGUS26-2(XX)
LJS26DS(V)
LUS24-2(OO)

Town of Innisfil Certified Model

05/01/2018 2:10:59 PM kgervais



millions

TAMARACK
LUMBER INC.
ALFA LUMBER GROUP

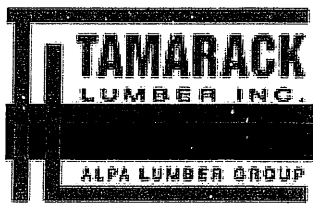
Job Track:	42067
Layout ID:	272342
Plan Log:	87565

Builder / Location:		BAYVIEW WELLINGTON / INNISFILL	
Project:	ALCONA SHORES	THESE OR RE TAMAR	
Date:	9/8/2017	NO	
Postcodes:			

Model / Elevation:
S32-5-10G / B

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Mitek ver 7.5.0



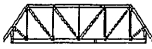











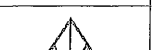
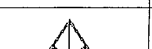


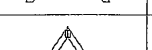

Delivery Shiplist

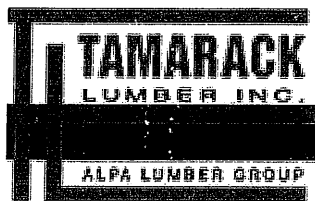
DATE	09/09/17
SALES REP	Mario

JOB TRACK:42067 LAYOUT ID: 272340 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-5-10G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	10.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	245.66		
	2 Ply		0.00							152.66		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	107.29 69.67		
	1	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	113.57 72.00		
	1	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.38 72.83		
	1	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.26 77.67		
	1	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	126.45 80.00		
	13	T7 PIGGYBACK	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1677.52 1061.71		
	1	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.43 87.67		
	1	T9Z PIGGYBACK	10.00	25-08-00	10-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	299.46		
	2 Ply		0.00							188.66		
	4	T10 JACK-CLOSED	10.00 0.00	04-03-00	03-07-14	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-14	99.32 72.68		
	3	T12 COMMON	6.50 0.00	09-08-00	03-02-14	2 X 4	2 X 4	00-00-00 00-00-00	00-07-08 00-07-08	87.24 54.51		
	1	G12 COMMON	6.50 0.00	09-08-00	03-02-14	2 X 4	2 X 4	01-03-08 01-03-08	00-07-08 00-07-08	33.23 22.17		
	2	T40 COMMON	10.00 0.00	15-04-00	08-00-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	142.90 90.66		
	3	T40S SCISSOR	10.00 4.00	15-04-00	08-00-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	210.18 134.49		
	1	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	12.42 9.33		
	11	PB2 PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	138.71 95.37		
	1	PB2Z PIGGYBACK	10.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	25.22		
	2 Ply		0.00							17.34		
	12	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04		



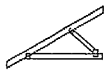

Delivery Shiplist

DATE	09/09/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272340 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-5-10G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	J2 JACK-OPEN	4.00 0.00	06-05-08	02-05-12	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-12	75.12 48.68		
	6	J3 JACK-OPEN	6.00 0.00	03-05-08	02-03-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-03-08	61.02 40.02		

TOTAL # TRUSS= 72.00

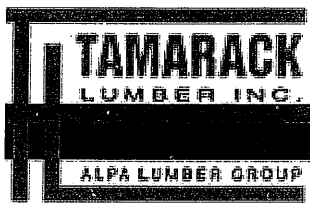
TOTAL BFT OF ALL TRUSSES=

2576.16 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4036.86 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	09/09/17
SALES REP	Mario

JOB TRACK:42067 LAYOUT ID: 272341 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-5-10G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1Z HIP GIRDER	10.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08	01-07-11	245.66		
	2 Ply		0.00					01-03-08	01-07-11	152.66		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	107.29 69.67		
	1	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	113.57 72.00		
	1	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.38 72.83		
	1	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.26 77.67		
	1	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	126.45 80.00		
	13	T7 PIGGYBACK	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1677.52 1061.71		
	1	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.43 87.67		
	1	T9 PIGGYBACK	10.00	25-08-00	10-01-04	2 X 4	2 X 6	01-03-08	01-07-11	299.46		
	2 Ply		0.00					01-03-08	01-07-11	188.66		
	4	T10 JACK-CLOSED	10.00 0.00	04-03-00	03-07-14	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-14	99.32 72.68		
	3	T12 COMMON	6.50 0.00	09-08-00	03-02-14	2 X 4	2 X 4	00-00-00 00-00-00	00-07-08 00-07-08	87.24 54.51		
	1	G12 COMMON	6.50 0.00	09-08-00	03-02-14	2 X 4	2 X 4	01-03-08 01-03-08	00-07-08 00-07-08	33.23 22.17		
	2	T40 COMMON	10.00 0.00	15-04-00	08-00-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	142.90 90.66		
	3	T40S SCISSOR	10.00 4.00	15-04-00	08-00-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	210.18 134.49		
	2	T101 COMMON	10.00 0.00	14-10-00	07-09-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	138.88 89.34		
	1	G101 COMMON	10.00 0.00	14-10-00	07-09-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	73.56 47.67		
	1	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	12.42 9.33		
	11	PB2 PIGGYBACK	10.00 0.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	138.71 95.37		







Delivery Shiplist

DATE	09/09/17
SALES REP	Mario

JOB TRACK:42067 LAYOUT ID: 272341 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-5-10G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

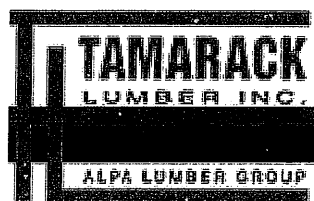
PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	PB2Z	10.00	04-01-02	02-01-04	2 X 4	2 X 4	00-00-00	00-04-13	25.22		
	2 Ply	PIGGYBACK	0.00							17.34		
	5	J1	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00	01-02-00	84.75		
		JACK-OPEN	0.00					00-00-00	04-01-04	53.35		
	4	J2	4.00	06-05-08	02-05-12	2 X 4	2 X 4	01-03-08	00-03-15	76.28		
		JACK-OPEN	0.00					00-00-00	02-05-12	48.68		
	6	J3	6.00	03-05-08	02-03-08	2 X 4	2 X 4	01-03-08	00-06-12	64.98		
		JACK-OPEN	0.00					00-00-00	02-03-08	43.98		

TOTAL # TRUSS= 68.00 TOTAL BFT OF ALL TRUSSES= 2642.44 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4137.69 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 10.00



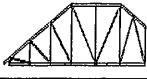
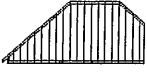
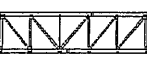
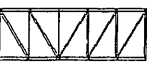
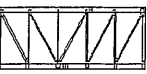
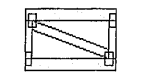



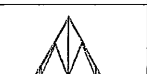
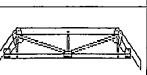
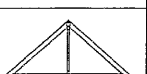

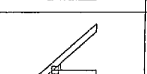

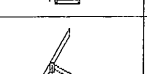
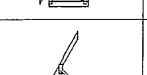
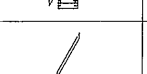
Delivery Shiplist

DATE	09/09/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272342 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S32-5-10G ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	12	T20 PIGGYBACK	6.00 0.00	39-01-08	10-00-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 06-04-04	2606.28 1604.04		
	2	G20 PIGGYBACK	6.00 0.00	39-01-08	10-00-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 06-04-04	474.16 303.34		
	1 2 Ply	T210 FLAT GIRDER	0.00 0.00	25-08-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	274.42 166.00		
	1	T22 FLAT	0.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	05-01-04 05-01-04	110.58 68.83		
	1 4 Ply	T23 FLAT GIRDER	0.00 0.00	25-08-00	06-01-04	2 X 6	2 X 6	00-00-00 00-00-00	06-01-04 06-01-04	643.28 394.68		
	2 2 Ply	T24 FLAT	0.00 0.00	03-10-08	01-06-00	2 X 4	2 X 4	00-00-00 00-00-00	01-06-00 01-06-00	54.84 34.68		
	1	T25 HIP GIRDER	12.00 0.00	09-08-00	05-04-00	2 X 4	2 X 4	00-00-00 00-00-00	01-05-08 01-05-08	46.63 30.17		
	1	T45 HIP GIRDER	12.00 0.00	15-04-00	05-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	75.88 49.33		
	1	T46 HIP	12.00 0.00	15-04-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	83.08 53.67		
	2	T47 COMMON	12.00 0.00	15-04-00	09-06-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	155.72 98.34		
	1	T48 HIP GIRDER	10.00 0.00	15-04-00	02-01-08	2 X 6	2 X 4	01-03-08 01-03-08	00-11-02 00-11-02	73.95 46.67		
	14	PB3 PIGGYBACK	6.00 0.00	12-03-06	03-05-00	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	479.36 291.62		
	5	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	83.95 53.35		
	6	J3 JACK-OPEN	6.00 0.00	03-05-08	02-03-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-03-08	64.98 43.98		
	2	J20 JACK-OPEN	12.00 0.00	03-10-08	05-04-00	2 X 4	2 X 4	00-10-08 00-00-00	01-05-08 05-04-00	33.30 22.66		
	2	J21 JACK-OPEN	12.00 0.00	03-10-08	03-04-15	2 X 4	2 X 4	00-10-08 -01-11-01	01-05-08 00-03-08	27.10 18.66		
	2	J22 JACK-OPEN	12.00 0.00	02-00-08	03-04-15	2 X 4	2 X 4	00-10-08 -00-01-01	01-05-08 03-01-01	20.14 16.34		
	2	J23 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	35.80 22.66		




Delivery Shiplist

DATE	09/09/17
SALES REP	Mario

JOB TRACK:42067 LAYOUT ID: 272342 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SH0 SUB-BUILDER:
 MODEL: S32-5-10G ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	7	J24 JACK-OPEN	5.00 0.00	04-03-08	02-01-08	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-01-08	83.51 56.00		

TOTAL # TRUSS= 71.00

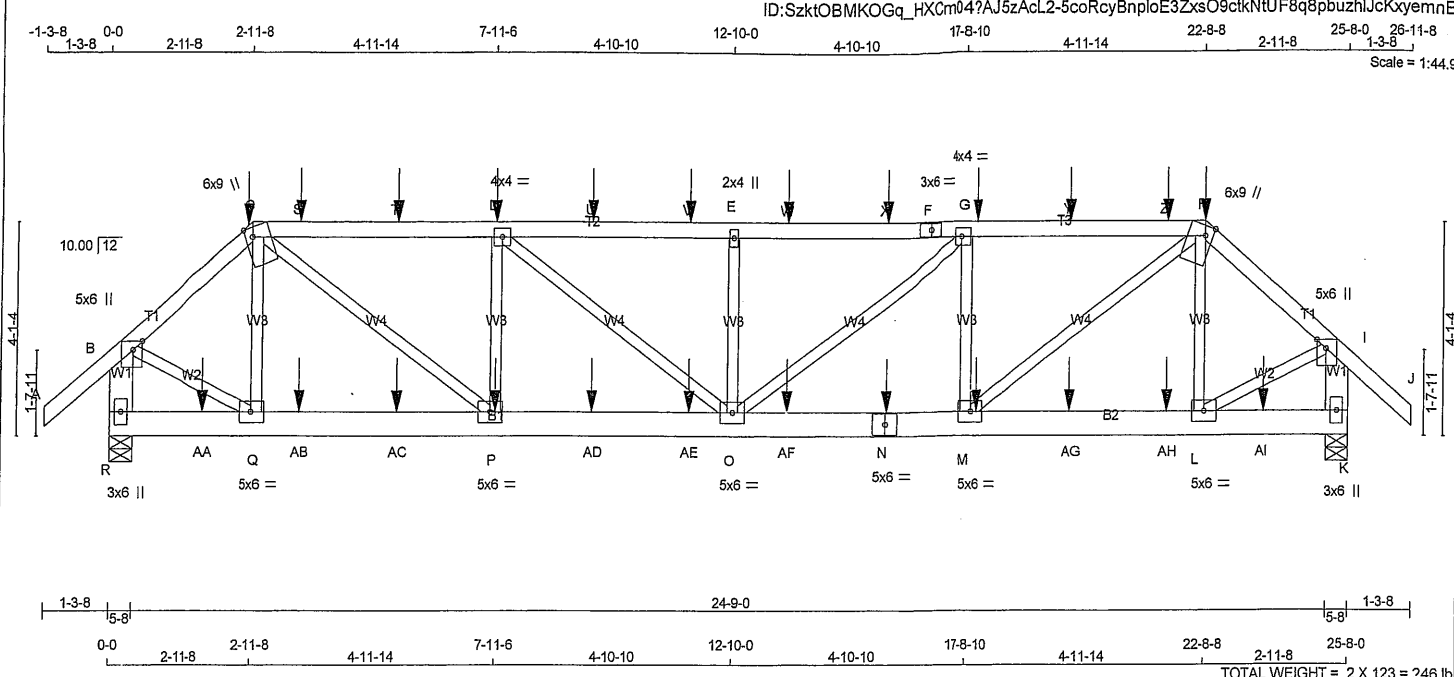
TOTAL BFT OF ALL TRUSSES=

3375.02 BFT. TOTAL WEIGHT OF ALL TRUSSES= 5426.96 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
15	Hangers	LJS26DS	
2	Hangers	LUS24-2	

TOTAL # ITEMS= 18.00



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
R - N	2x6	DRY No.2	SPF
N - K	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-F	12	SIDE(61.0)
F-H	12	SIDE(61.0)
H-J	12	SIDE(61.0)
R-B	2	TOP
K-I	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-N	2	SIDE(183.1)
N-K	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW+p	MT20	5.0	6.0	2.00 2.25
C TTWW+m	MT20	6.0	9.0	Edge 1.75
D TMWW-t	MT20	4.0	4.0	
E TMW+w	MT20	2.0	4.0	
F TS-t	MT20	3.0	6.0	
G TMWW-t	MT20	4.0	4.0	
H TTWW+m	MT20	6.0	9.0	Edge 1.75
I TMVW+p	MT20	5.0	6.0	2.00 2.25
K BMV+p	MT20	3.0	6.0	
L, M, P, Q				
L BMWW-t	MT20	5.0	6.0	
N BS-t	MT20	5.0	6.0	
O BMWW-t	MT20	5.0	6.0	
R BMV+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
R	3240	0	3240	0	0	5-8	5-8	5-8	5-8
K	3262	0	3262	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LOASE		MAX /MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
R	2549	1634 / 0	472 / 0	0 / 0	0 / 0
K	2565	1645 / 0	474 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 54	Q-C	-671 / 0
B-C	-3027 / 0	C-P	0 / 3112
C-S	-4796 / 0	P-D	-1639 / 0
S-T	-4796 / 0	D-O	0 / 912
T-D	-4796 / 0	O-E	-880 / 0
D-U	-5521 / 0	U-V	0 / 901
U-V	-5521 / 0	V-E	-1635 / 0
V-E	-5521 / 0	E-W	0 / 3105
E-W	-5521 / 0	W-X	-678 / 0
W-X	-5521 / 0	X-F	0 / 2494
X-F	-5521 / 0	F-G	0 / 2510
F-G	-5521 / 0	G-Y	
G-Y	-4805 / 0	Y-Z	
Y-Z	-4805 / 0	Z-H	
Z-H	-4805 / 0	H-I	
H-I	-3046 / 0	I-J	
I-J	0 / 54	J-K	
J-K	-3222 / 0	K-L	
K-L	-3241 / 0		

PLATES		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
RA-AA	0 / 0	AA-Q	-28.0 -28.0 0.06 (2)
AA-Q	0 / 0	Q-AB	-28.0 -28.0 0.06 (2)
Q-AB	0 / 2298	AB-AC	-28.0 -28.0 0.19 (1)
AB-AC	0 / 2298	AC-P	-28.0 -28.0 0.19 (1)
AC-P	0 / 2298	P-AD	-28.0 -28.0 0.37 (1)
P-AD	0 / 4796	AD-AE	-28.0 -28.0 0.37 (1)
AD-AE	0 / 4796	AE-O	-28.0 -28.0 0.37 (1)
AE-O	0 / 4805	O-AF	-28.0 -28.0 0.38 (1)
O-AF	0 / 4805	AF-N	-28.0 -28.0 0.38 (1)
AF-N	0 / 4805	N-M	-28.0 -28.0 0.38 (1)
N-M	0 / 2312	M-AG	-28.0 -28.0 0.19 (1)
M-AG	0 / 2312	AG-AH	-28.0 -28.0 0.19 (1)
AG-AH	0 / 2312	AH-L	-28.0 -28.0 0.19 (1)
AH-L	0 / 0	L-AI	-28.0 -28.0 0.06 (2)
L-AI	0 / 0	AI-K	-28.0 -28.0 0.06 (2)



DRWG NO. TAM 45829-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	
TOP CH. LL	= 38.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.53 (E-G:1), BC=0.38 (M-O:1), WB=0.39 (C-P:1), SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

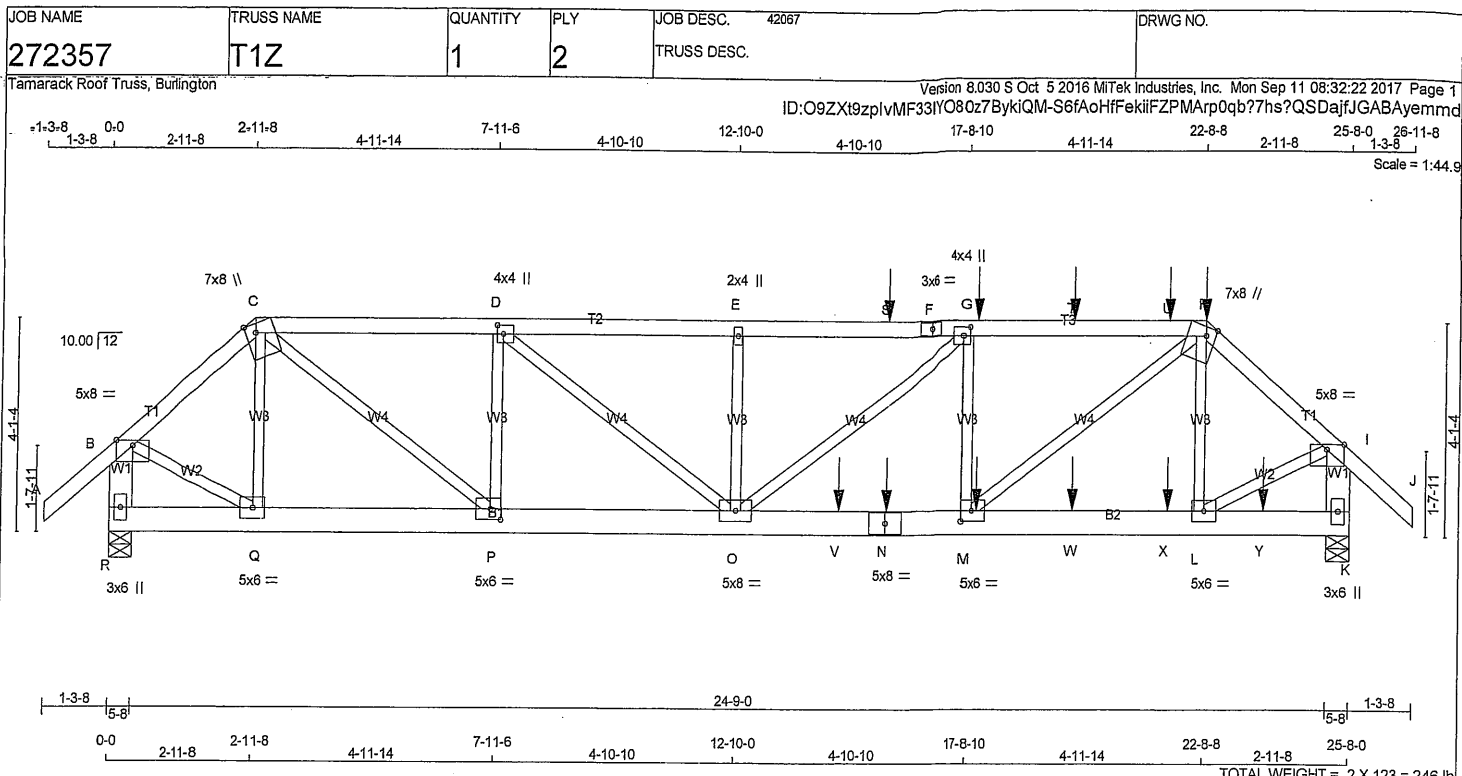
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE	GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MT20	618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.42 (N) (INPUT = 1.00)



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
R - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
R - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-F	12	SIDE(0.1)
F-H	12	SIDE(61.0)
H-J	12	SIDE(61.0)
R-B	2	TOP
K-I	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-N	2	SIDE(0.0)
N-K	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	X
B TMWV-p	MT20	5.0	8.0	Edge
C TTWW+m	MT20	7.0	8.0	Edge 2.25
D TMWV-t	MT20	4.0	4.0	2.00 1.50
E TMW+w	MT20	2.0	4.0	
F TS-t	MT20	3.0	6.0	
G TMWV+t	MT20	4.0	4.0	2.00 1.50
H TTWW+m	MT20	7.0	8.0	Edge 2.25
I TMWV-p	MT20	5.0	8.0	Edge
K BMV1+p	MT20	3.0	6.0	
L BMWV-t	MT20	5.0	6.0	
M BMWV-t	MT20	5.0	6.0	2.50 2.50
N BS-t	MT20	5.0	8.0	
O BMWV-t	MT20	5.0	8.0	
P BMWV-t	MT20	5.0	6.0	2.50 2.50
Q BMWV-t	MT20	5.0	6.0	
R BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG
JT	VERT	DOWN	UP
R	2980	0	0
K	3812	0	0

UNFACTORED REACTIONS			
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
R	2303	1546 / 0	381 / 0
K	2969	1953 / 0	518 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0 / 54	-122.2 -122.2	0.09 (1)
B-C	-2774 / 0	-122.2 -122.2	0.13 (1)
C-D	-4748 / 0	-122.2 -122.2	0.35 (1)
D-E	-6540 / 0	-122.2 -122.2	0.45 (1)
E-S	-6540 / 0	-122.2 -122.2	0.58 (1)
S-F	-6540 / 0	-122.2 -122.2	0.58 (1)
F-G	-6540 / 0	-122.2 -122.2	0.58 (1)
G-T	-6029 / 0	-122.2 -122.2	0.55 (1)
T-U	-6029 / 0	-122.2 -122.2	0.55 (1)
U-H	-6029 / 0	-122.2 -122.2	0.55 (1)
H-I	-3580 / 0	-122.2 -122.2	0.15 (1)
I-J	0 / 54	-122.2 -122.2	0.09 (1)
R-B	-2983 / 0	0.0	0.11 (1)
K-I	-3747 / 0	0.0	0.14 (1)
WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH
Q-C	-625 / 0	0.08 (1)	10.00
C-P	0 / 3292	0.41 (1)	5.37
P-D	-2034 / 0	0.26 (1)	10.00
D-O	0 / 2254	0.28 (1)	3.49
O-E	-571 / 0	0.07 (1)	3.31
E-G	0 / 642	0.08 (1)	3.31
G-H	-1458 / 0	0.18 (1)	3.31
H-I	0 / 4133	0.51 (1)	3.47
I-L	-1013 / 0	0.13 (1)	3.47
L-I	0 / 2286	0.28 (1)	4.83
I-J	0 / 2950	0.37 (1)	10.00
R-Q	0 / 0	-28.0 -28.0	0.05 (1)
Q-P	0 / 2106	-28.0 -28.0	0.20 (1)
P-O	0 / 4748	-28.0 -28.0	0.50 (1)
O-V	0 / 6029	-28.0 -28.0	0.94 (1)
V-N	0 / 6029	-28.0 -28.0	0.94 (1)
N-M	0 / 6029	-28.0 -28.0	0.94 (1)
M-W	0 / 2711	-28.0 -28.0	0.35 (1)
W-X	0 / 2711	-28.0 -28.0	0.35 (1)
X-L	0 / 2711	-28.0 -28.0	0.35 (1)
L-Y	0 / 0	-28.0 -28.0	0.04 (3)
Y-K	0 / 0	-28.0 -28.0	0.04 (3)

FACTORED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1	MAX. MAX+
G	17-11-4	-147	-147
H	22-8-8	-228	-228
M	17-11-4	-40	-70
N	15-11-4	-40	-70
S	15-11-4	-147	-147
T	19-11-4	-147	-147
U	21-11-4	-161	-161
V	14-11-8	-1568	-1568
W	19-11-4	-40	-70
X	21-11-4	-40	-70
Y	23-11-4	-40	-70
FACE DIR.			
G	FRONT	VERT	TOTAL
H	FRONT	VERT	TOTAL
M	FRONT	VERT	TOTAL
N	FRONT	VERT	TOTAL
S	FRONT	VERT	TOTAL
T	FRONT	VERT	TOTAL
U	FRONT	VERT	TOTAL
V	FRONT	VERT	TOTAL
W	FRONT	VERT	TOTAL
X	FRONT	VERT	TOTAL
Y	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL)= L/999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL)= L/999 (0.28")

CSI: TC=0.58 (E-G:1), BC=0.94 (M-O:1),
WB=0.51 (H-M:1), SSI=0.32 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

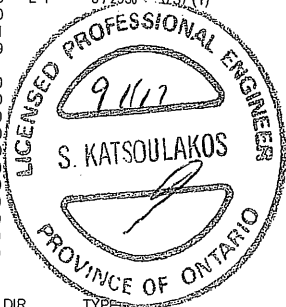
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.81 (N) (INPUT = 1.00)



DWG NO. TAM 4583617
STRUCTURAL

CONTINUED ON PAGE 2

COMPONENT ONLY

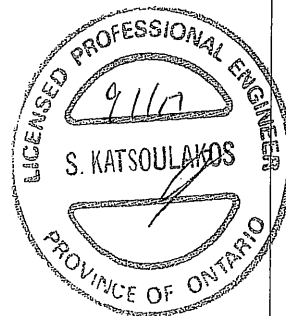
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272357	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 08:32:22 2017 Page 2
ID:O9Zxt9zp|vMF33|YO80z7BykiQM-S6fAoHfFekilFZPMArp0qb?7hs?QSDaifJGABAyemmd

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 22-8-8, 147.1 lbs FACTORED DOWN AT 15-11-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 1567.7 lbs FACTORED DOWN AT 14-11-8, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



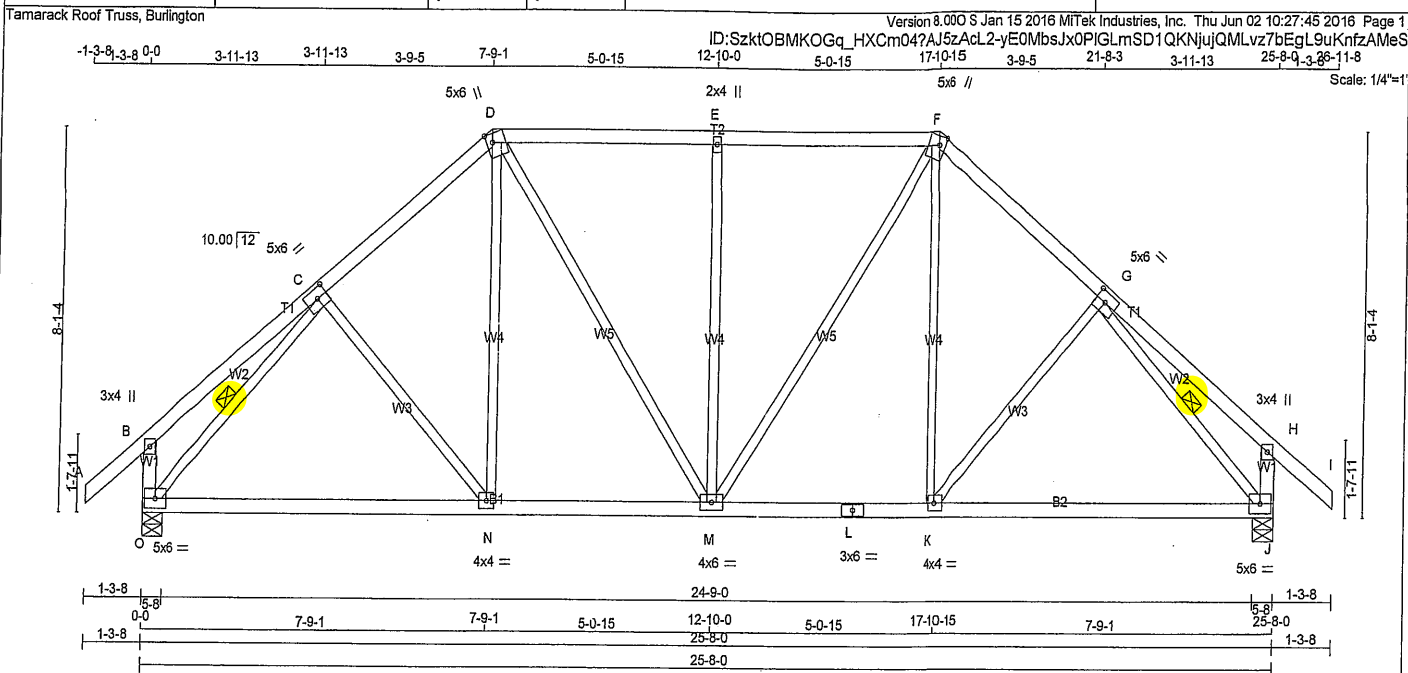
DWG NO. TAM4583617
STRUCTURAL
COMPONENT ONLY

PG 1



DWG NO. TAM 25783 16
STRUCTURAL
COMPONENT ONLY

A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The outer ring contains the text "LICENSED PROFESSIONAL ENGINEER" at the top and "PROVINCE OF ONTARIO" at the bottom. In the center, the license number "6216" is handwritten in the upper half, and the name "S. KATSOULAKOS" is printed in the lower half, with a stylized signature over it.



LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 122 lb									
N. L. G. A. RULES										BEARINGS										[M/F]									
CHORDS SIZE LUMBER DESCR.																				DESIGN CRITERIA									
A - D 2x4 DRY No.2 SPF										FACTORED MAXIMUM FACTORED INPUT REQ'D										SPECIFIED LOADS:									
D - F 2x4 DRY No.2 SPF										GROSS REACTION GROSS REACTION BRG BRG										TOP CH. LL = 38.3 PSF									
F - I 2x4 DRY No.2 SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										DL = 3.0 PSF									
O - B 2x4 DRY No.2 SPF										O 2097 0 2097 0 0 5-8 2-4										BOT CH. LL = 10.5 PSF									
J - H 2x4 DRY No.2 SPF										J 2097 0 2097 0 0 5-8 2-4										DL = 7.0 PSF									
O - L 2x4 DRY No.2 SPF																				TOTAL LOAD = 58.7 PSF									
L - J 2x4 DRY No.2 SPF																													
ALL WEBS 2x3 DRY No.2 SPF										UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C									
EXCEPT										1ST LCASE MAX/MIN COMPONENT REACTIONS																			
DRY: SEASONED LUMBER:										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12									
										O 1622 1087 / 0 270 / 0 0 / 0 0 / 0 265 / 0 0 / 0										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
										J 1622 1087 / 0 270 / 0 0 / 0 0 / 0 265 / 0 0 / 0										THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011									
										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J										(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD									
										BRACING										ALLOWABLE DEFL.(LL)= L/360 (0.86")									
										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.										CALCULATED VERT. DEFL.(LL) = L/999 (0.16")									
										MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.										ALLOWABLE DEFL.(TL)= L/360 (0.86")									
										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.										CALCULATED VERT. DEFL.(TL) = L/999 (0.26")									
										1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-J.										CSI: TC=0.43 (D-E:1), BC=0.52 (M-N:2), WB=0.96 (E-M:1), SSI=0.30 (D-E:1)									
										END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10									
										LOADING										COMPANION LIVE LOAD FACTOR = 0.50									
										TOTAL LOAD CASES: (4)										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
										CHORDS										NAIL VALUES									
										MEMB. MAX. FACTORED FORCE (LBS)										PLATE GRIP(DRY) SHEAR SECTION									
										VERT. LOAD LC1 MAX (PLF) CSI (LC)										(PSI) (PLI) (PLI)									
										FR-TO FROM TO LENGTH FR-TO										MAX MIN MAX MIN MAX MIN									
										A-B 0 / 54 -122.2 -122.2 0.17 (1) 10.00										MT20 618 354 1667 822 2284 1656									
										B-C 0 / 34 -122.2 -122.2 0.29 (1) 10.00										PLATE PLACEMENT TOL. = 0.250 inches									
										C-D -1805 / 0 -122.2 -122.2 0.26 (1) 4.74										PLATE ROTATION TOL. = 5.0 Deg.									
										D-E -1631 / 0 -122.2 -122.2 0.43 (1) 4.69										JSI GRIP= 0.90 (G) (INPUT = 0.90)									
										E-F -1631 / 0 -122.2 -122.2 0.43 (1) 4.69										JSI METAL= 0.54 (C) (INPUT = 1.00)									
										F-G -1805 / 0 -122.2 -122.2 0.26 (1) 4.74																			
										G-H 0 / 34 -122.2 -122.2 0.29 (1) 10.00																			
										H-I 0 / 54 -122.2 -122.2 0.17 (1) 10.00																			
										O-B -351 / 0 0.0 0.0 0.04 (1) 7.81																			
										J-H -351 / 0 0.0 0.0 0.04 (1) 7.81																			
										O-N 0 / 1413 -28.0 -28.0 0.52 (2) 10.00																			
										N-M 0 / 1361 -28.0 -28.0 0.52 (2) 10.00																			
										M-L 0 / 1361 -28.0 -28.0 0.52 (2) 10.00																			
										L-K 0 / 1361 -28.0 -28.0 0.52 (2) 10.00																			
										K-J 0 / 1413 -28.0 -28.0 0.52 (2) 10.00																			

LICENSED PROFESSIONAL ENGINEER

6-216

S. KATSOULAKOS

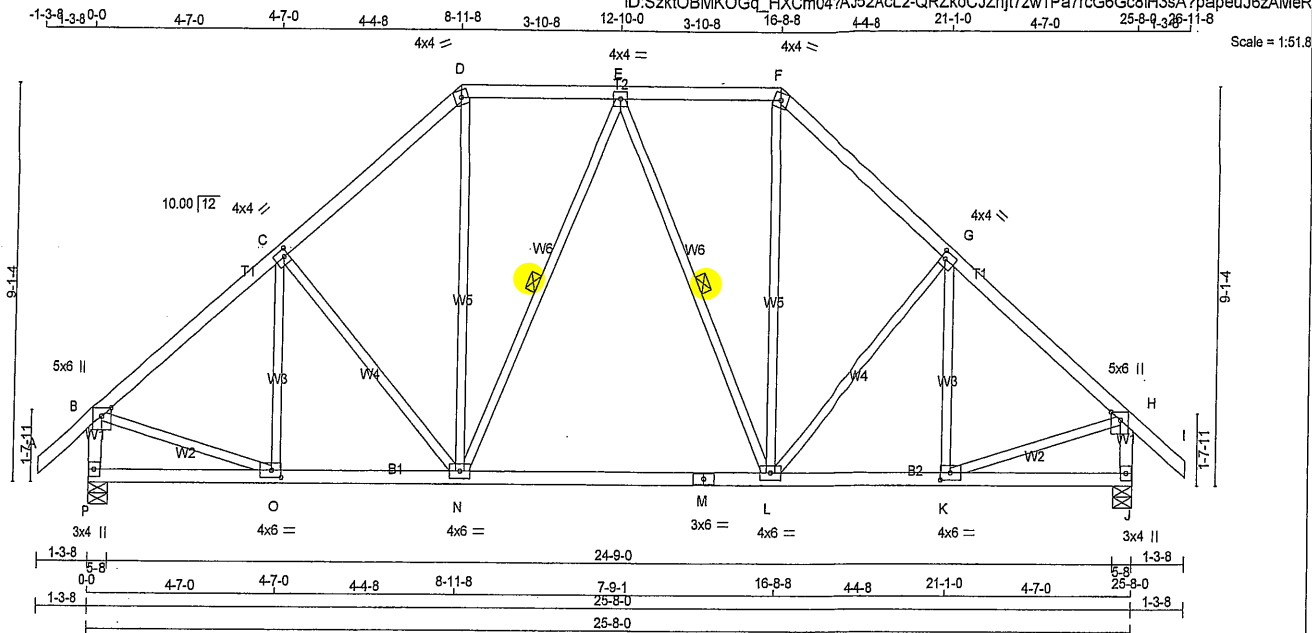
PROVINCE OF ONTARIO

DWG NO. TAM 25785-16

STRUCTURAL COMPONENT ONLY



DWG NO. TAM 25785-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 126 lb
 [M/F]

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - I	2x4	DRY	No.2 SPF
P - B	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
P - M	2x4	DRY	No.2 SPF
M - J	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	4.0	6.0	2.00	2.75
L	BMVW-wt	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-wt	MT20	4.0	6.0		
O	BMVW-t	MT20	4.0	6.0	2.00	2.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
	DOWN	UP	IN-SX	IN-SX
JT	2097	0	5-8	3-5
P	2097	0	5-8	3-5
J	2097	0	5-8	3-5

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE PERM. LIVE WIND DEAD SOIL
JT	1622	1087 / 0	270 / 0 0 / 0 0 / 0 265 / 0 0 / 0
P	1622	1087 / 0	270 / 0 0 / 0 0 / 0 265 / 0 0 / 0
J	1622	1087 / 0	270 / 0 0 / 0 0 / 0 265 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 54	-122.2	-122.2 0.17 (1)	10.00	O-C	-305 / 55	0.14 (1)
B-C	-1884 / 0	-122.2	-122.2 0.37 (1)	4.53	C-N	-291 / 0	0.25 (1)
C-D	-1722 / 0	-122.2	-122.2 0.35 (1)	4.71	N-D	0 / 688	0.15 (1)
D-E	-1295 / 0	-122.2	-122.2 0.24 (1)	5.41	N-E	-308 / 0	0.20 (1)
E-F	-1295 / 0	-122.2	-122.2 0.24 (1)	5.41	E-L	-308 / 0	0.20 (1)
F-G	-1722 / 0	-122.2	-122.2 0.35 (1)	4.71	L-F	0 / 688	0.15 (1)
G-H	-1884 / 0	-122.2	-122.2 0.37 (1)	4.53	L-G	-291 / 0	0.25 (1)
H-I	0 / 54	-122.2	-122.2 0.17 (1)	10.00	K-G	-305 / 55	0.14 (1)
P-B	-2039 / 0	0.0	0.0 0.22 (1)	5.92	B-O	0 / 1538	0.35 (1)
J-H	-2039 / 0	0.0	0.0 0.22 (1)	5.92	K-H	0 / 1538	0.35 (1)
P-O	0 / 0	-28.0	-28.0 0.13 (2)	10.00			
O-N	0 / 1479	-28.0	-28.0 0.41 (2)	10.00			
N-M	0 / 1417	-28.0	-28.0 0.40 (2)	10.00			
M-L	0 / 1417	-28.0	-28.0 0.40 (2)	10.00			
L-K	0 / 1479	-28.0	-28.0 0.41 (2)	10.00			
K-J	0 / 0	-28.0	-28.0 0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 38.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.86")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.86")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.19")

CSI: TC=0.37 (B-C:1), BC=0.41 (N-O:2), WB=0.35 (H-K:1), SSI=0.23 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1687
	822	2284	1656

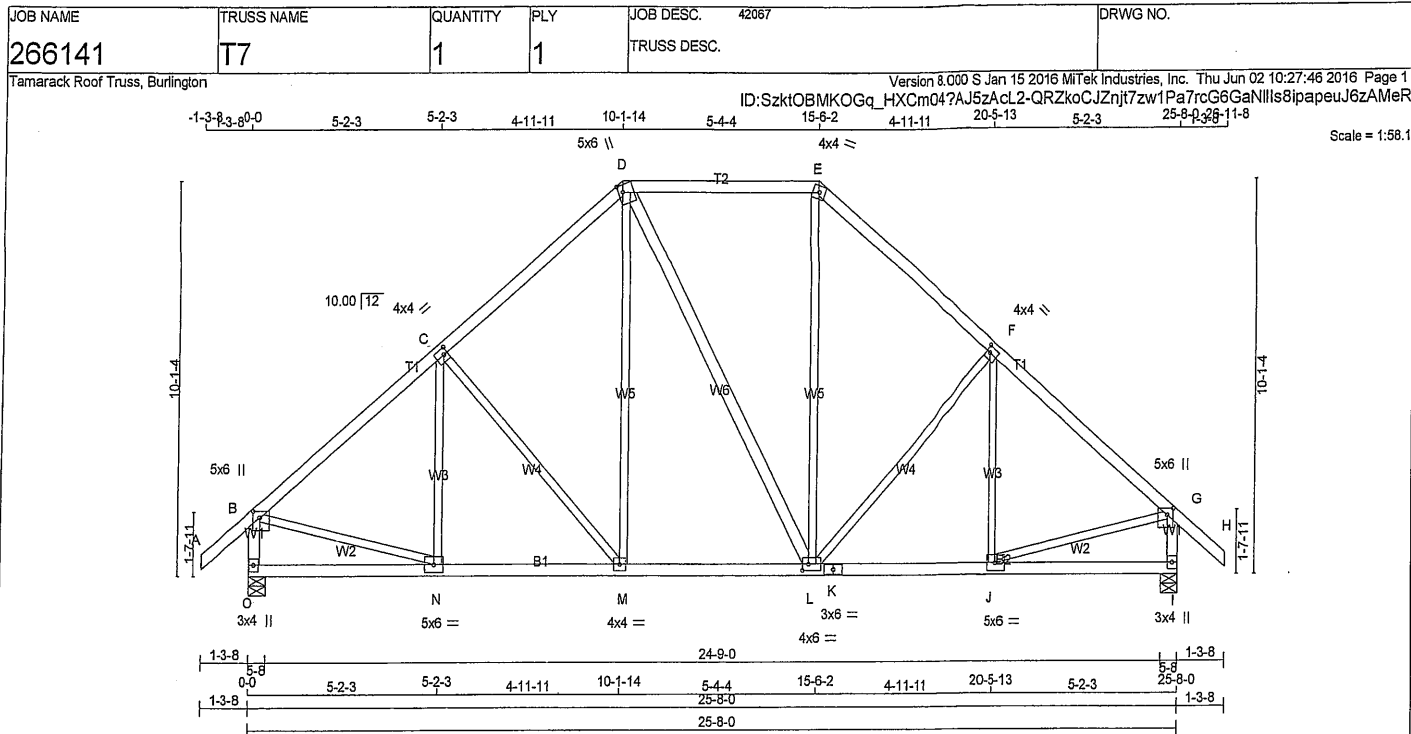
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
 JSI METAL= 0.42 (M) (INPUT = 1.00)



DRWG NO. TAM 2578616
 STRUCTURAL
 COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - E	2x4	DRY	No.2 SPF
E - H	2x4	DRY	No.2 SPF
O - B	2x4	DRY	No.2 SPF
I - G	2x4	DRY	No.2 SPF
O - K	2x4	DRY	No.2 SPF
K - I	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT D - L	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMVW+t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMVW+t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.00	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMVW+t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW+t	MT20	4.0	6.0	2.00	2.00
M	BMVW+t	MT20	4.0	4.0		
N	BMVW+t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2097	0	2097	0	0	5-8	3-5
I	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN	COMPONENT REACTIONS
O	1622	1087 / 0	270 / 0
I	1622	1087 / 0	270 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	N-C	-219 / 142
B-C	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38	C-M	-428 / 0
C-D	-1627 / 0	-122.2	-122.2	0.45 (1)	4.68	M-D	0 / 457
D-E	-1215 / 0	-122.2	-122.2	0.47 (1)	5.19	D-L	0 / 2
E-F	-1628 / 0	-122.2	-122.2	0.45 (1)	4.68	L-E	0 / 450
F-G	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38	L-F	-426 / 0
G-H	0 / 54	-122.2	-122.2	0.17 (1)	10.00	J-F	-221 / 140
O-B	-2037 / 0	0.0	0.0	0.22 (1)	5.92	B-N	0 / 1543
I-G	-2036 / 0	0.0	0.0	0.22 (1)	5.92	J-G	0 / 1543
O-N	0 / 0	-28.0	-28.0	0.18 (3)	10.00		
N-M	0 / 1497	-28.0	-28.0	0.33 (1)	10.00		
M-L	0 / 1215	-28.0	-28.0	0.28 (1)	10.00		
L-K	0 / 1496	-28.0	-28.0	0.32 (1)	10.00		
K-J	0 / 1496	-28.0	-28.0	0.32 (1)	10.00		
J-I	0 / 0	-28.0	-28.0	0.17 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.48 (B-C:1), BC=0.33 (M-N:1), WB=0.49 (C-M:1), SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

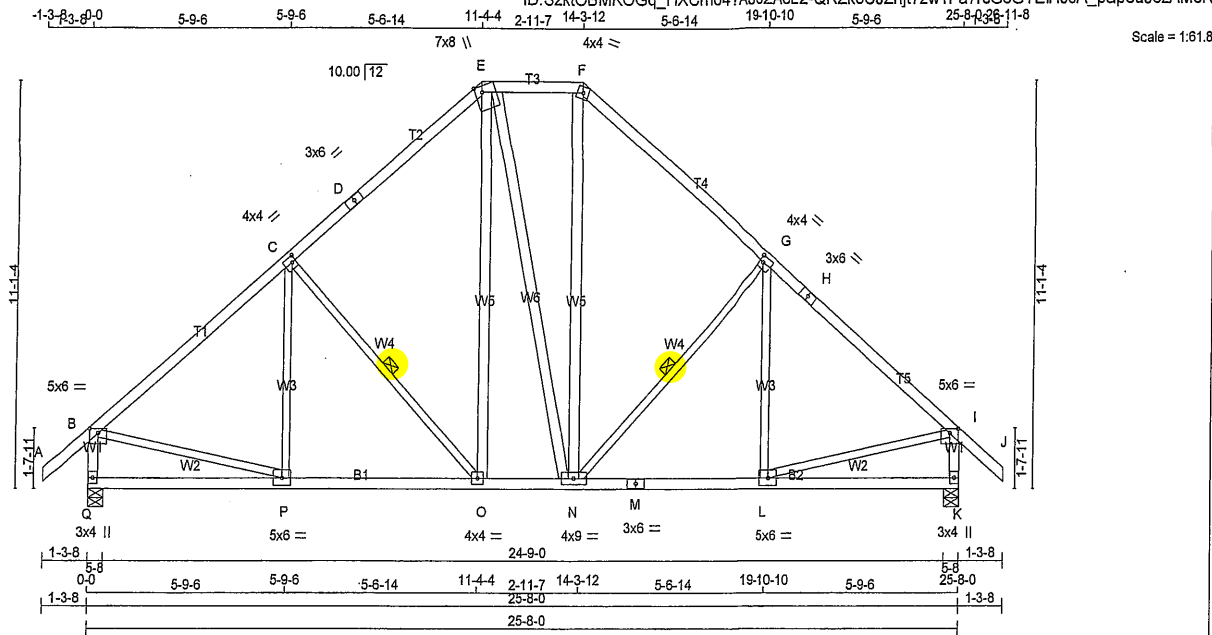
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)



DRWG NO. TAM 2578716
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
O - E	2x4	DRY	No.2
E - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 1.50 3.00
C	TMWW-t	MT20	4.0	4.0 2.00 1.25
D	TS-t	MT20	3.0	6.0
E	TTWW+m	MT20	7.0	8.0 Edge 2.25
F	TTW-m	MT20	4.0	4.0
G	TMWW-t	MT20	4.0	4.0 2.00 1.25
H	TS-t	MT20	3.0	6.0
I	TMVW-p	MT20	5.0	6.0 1.50 3.00
K	BMV1+p	MT20	3.0	4.0
L	BMWW-t	MT20	5.0	6.0
M	BS-t	MT20	3.0	6.0
N	BMWW-t	MT20	4.0	9.0
O	BMWW-t	MT20	4.0	4.0
P	BMWW-t	MT20	5.0	6.0
Q	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
Q	2097	0	2097	0	5-8
K	2097	0	2097	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
Q	1622	1087 / 0	270 / 0
K	1622	1087 / 0	270 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO
A-B	0 / 54	-122.2 -122.2 0.17 (1)	10.00 P-C -153 / 204 0.10 (1)
B-C	-1904 / 0	-122.2 -122.2 0.62 (1)	4.17 C-O -551 / 0 0.26 (1)
C-D	-1527 / 0	-122.2 -122.2 0.56 (1)	4.61 O-E 0 / 505 0.08 (1)
D-E	-1527 / 0	-122.2 -122.2 0.56 (1)	4.61 E-N 0 / 5 0.00 (1)
E-F	-1136 / 0	-122.2 -122.2 0.15 (1)	5.80 N-F 0 / 511 0.08 (1)
F-G	-1529 / 0	-122.2 -122.2 0.56 (1)	4.62 N-G -547 / 0 0.26 (1)
G-H	-1903 / 0	-122.2 -122.2 0.61 (1)	4.17 L-G -157 / 201 0.11 (1)
H-I	-1903 / 0	-122.2 -122.2 0.61 (1)	4.17 B-P 0 / 1542 0.35 (1)
I-J	0 / 54	-122.2 -122.2 0.17 (1)	10.00 L-I 0 / 1541 0.35 (1)
Q-B	-2032 / 0	0.0 0.0 0.22 (1)	5.93
K-I	-2031 / 0	0.0 0.0 0.22 (1)	5.93
Q-P	0 / 0	-28.0 -28.0 0.24 (3)	10.00
P-O	0 / 1504	-28.0 -28.0 0.39 (2)	10.00
O-N	0 / 1135	-28.0 -28.0 0.25 (1)	10.00
N-M	0 / 1503	-28.0 -28.0 0.39 (2)	10.00
M-L	0 / 1503	-28.0 -28.0 0.39 (2)	10.00
L-K	0 / 0	-28.0 -28.0 0.24 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.62 (B-C:1), BC=0.39 (O-P:2), WB=0.35 (B-P:1), SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

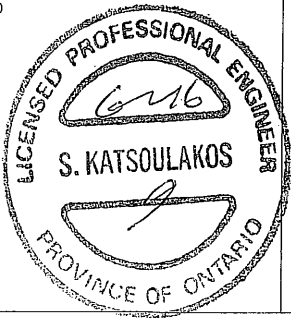
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

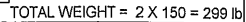
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.45 (M) (INPUT = 1.00)



DWG NO. TAM25788-16
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
266141	T9	1	2	TRUSS DESC.	

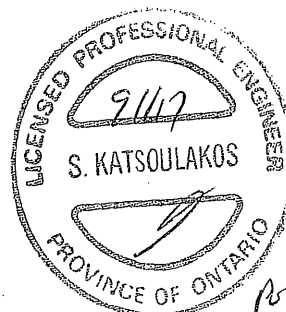
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 08:31:44 2017 Page 2

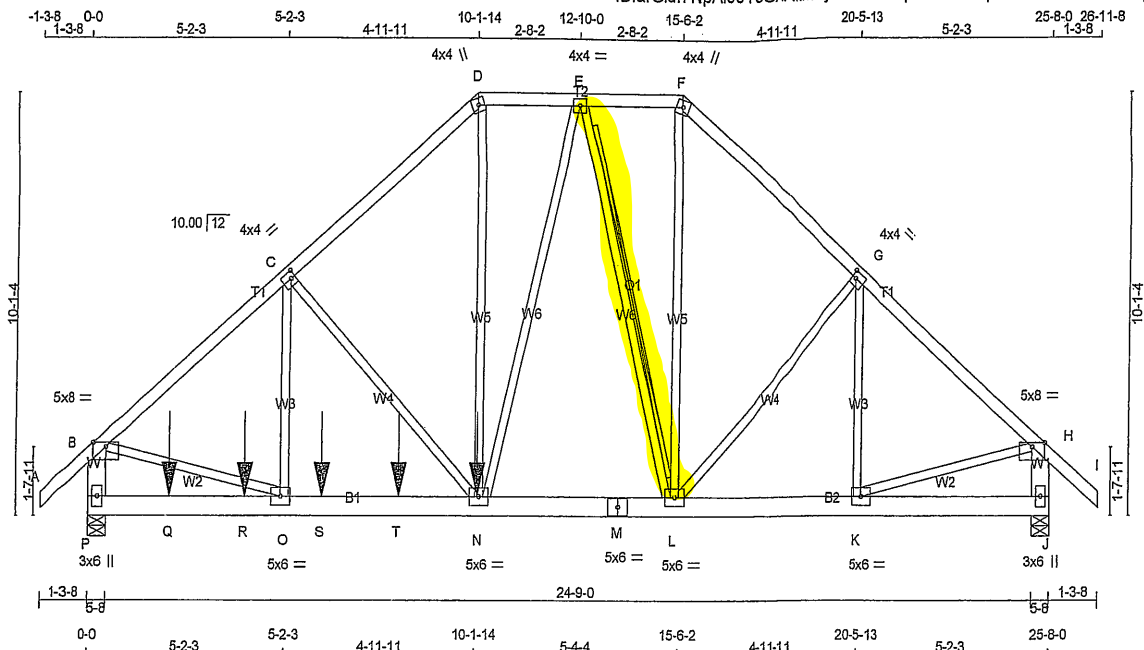
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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 291.2 lbs FACTORED DOWN AT 4-4-4, 291.2 lbs FACTORED DOWN AT 6-3-4, 291.2 lbs FACTORED DOWN AT 8-3-4, AND 291.2 lbs FACTORED DOWN AT 10-3-4, AND 1079.1 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 45831-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-F 1	12	TOP
F-I 1	12	TOP
P-B 2	12	TOP
J-H 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-M 2	12	SIDE(183.1)
M-J 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
N-D 1	6	SIDE(141.1)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge
C	TMVW-t	MT20	4.0	4.0	2.00 1.25
D	TTW+m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	TTW+m	MT20	4.0	4.0	
G	TMVW-t	MT20	4.0	4.0	2.00 1.25
H	TMVW-p	MT20	5.0	8.0	Edge
J	BMV1+p	MT20	3.0	6.0	
K	BMVW-t	MT20	5.0	6.0	
L	BMVW-t	MT20	5.0	6.0	
M	BS-t	MT20	5.0	6.0	
N	BMVW-t	MT20	5.0	6.0	
O	BMVW-t	MT20	5.0	6.0	
P	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
P	3774	0	3774	0
J	2827	0	2827	0

UNFACTORED REACTIONS

	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	2870	2010 / 0	421 / 0	0 / 0	0 / 0	439 / 0	0 / 0
P	2168	1486 / 0	339 / 0	0 / 0	0 / 0	343 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 54	-122.2 -122.2	0.09 (1)	10.00	O-C	0 / 308	0.04 (2)
B-C	-3773 / 0	-122.2 -122.2	0.41 (1)	4.49	C-N	-825 / 0	0.46 (1)
C-D	-3146 / 0	-122.2 -122.2	0.37 (1)	4.85	N-D	0 / 1592	0.20 (1)
D-E	-2393 / 0	-122.2 -122.2	0.09 (1)	5.73	L-F	0 / 1270	0.16 (1)
E-F	-1999 / 0	-122.2 -122.2	0.08 (1)	6.14	L-G	-303 / 0	0.17 (1)
F-G	-2638 / 0	-122.2 -122.2	0.35 (1)	5.21	K-G	-382 / 95	0.10 (1)
G-H	-2805 / 0	-122.2 -122.2	0.36 (1)	5.08	B-O	0 / 3014	0.37 (1)
H-I	0 / 54	-122.2 -122.2	0.09 (1)	10.00	K-H	0 / 2251	0.28 (1)
P-B	-3551 / 0	0.0 0.0	0.13 (1)	7.43	N-E	0 / 603	0.07 (1)
J-H	-2756 / 0	0.0 0.0	0.10 (1)	7.81	E-L	-936 / 0	0.53 (1)
P-Q	0 / 0	-28.0 -28.0	0.15 (1)	10.00			
Q-R	0 / 0	-28.0 -28.0	0.15 (1)	10.00			
R-O	0 / 0	-28.0 -28.0	0.15 (1)	10.00			
O-S	0 / 2933	-28.0 -28.0	0.34 (1)	10.00			
S-T	0 / 2933	-28.0 -28.0	0.34 (1)	10.00			
T-N	0 / 2933	-28.0 -28.0	0.34 (1)	10.00			
N-M	0 / 2238	-28.0 -28.0	0.19 (1)	10.00			
M-L	0 / 2238	-28.0 -28.0	0.19 (1)	10.00			
L-K	0 / 2191	-28.0 -28.0	0.16 (1)	10.00			
K-J	0 / 0	-28.0 -28.0	0.05 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
N	10-2-8	-1243	-1243	—	BACK	VERT
Q	2-1-12	-291	-291	—	BACK	VERT
R	4-1-12	-291	-291	—	BACK	VERT
S	6-1-12	-291	-291	—	BACK	VERT
T	8-1-12	-291	-291	—	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.41 (B-C-1), BC=0.34 (N-O-1), WB=0.53 (E-L-1), SSI=0.17 (O-P-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

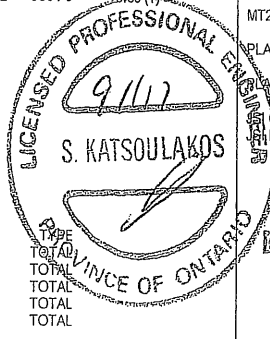
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.85 (D) (INPUT = 0.90)
METAL = 0.37 (O) (INPUT = 1.00)



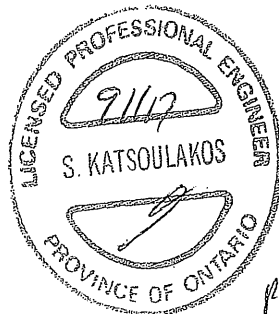
DRWG NO. TAM 45839-17
STRUCTURAL COMPONENT ONLY

JOB NAME 272340	TRUSS NAME T9Z	QUANTITY 1	PLY 2	JOB DESC. 42067 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 291.2 lbs FACTORED DOWN AT 2-1-12, 291.2 lbs FACTORED DOWN AT 4-1-12, 291.2 lbs FACTORED DOWN AT 6-1-12, AND 291.2 lbs FACTORED DOWN AT 8-1-12, AND 1242.7 lbs FACTORED DOWN AT 10-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

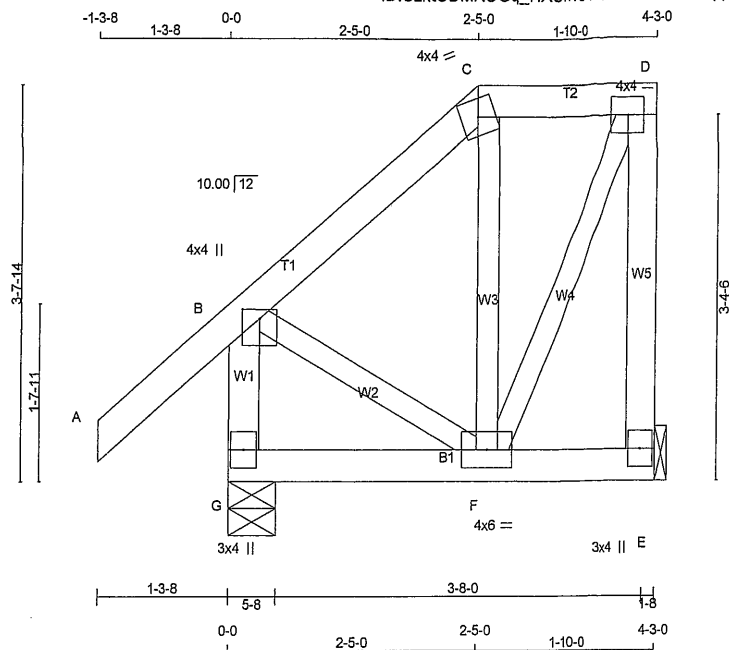


DWG NO. TAM 45839-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42057	DRWG NO.
266141	T10	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:21.5

TOTAL WEIGHT = 4 X 25 = 99 lb

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS					
G - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
G	488	0	488	0
E	319	0	319	0

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) E

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	364	268 / 0	45 / 0	0 / 0	0 / 0	51 / 0	0 / 0
E	250	163 / 0	45 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	FR-TO			
G-B	-457 / 0	0.0	0.0	0.05 (1)	7.81	F-C	-174 / 21
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	B-F	0 / 116
B-C	-134 / 0	-122.2	-122.2	0.12 (1)	6.25	F-D	0 / 213
E-D	-300 / 0	0.0	0.0	0.06 (1)	7.81		
C-D	-97 / 0	-122.2	-122.2	0.07 (1)	6.25		
G-F	0 / 0	-28.0	-28.0	0.04 (2)	10.00		
F-E	0 / 0	-28.0	-28.0	0.04 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.17 (A-B:1), BC=0.04 (F-G:2), WB=0.05 (D-F:1), SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

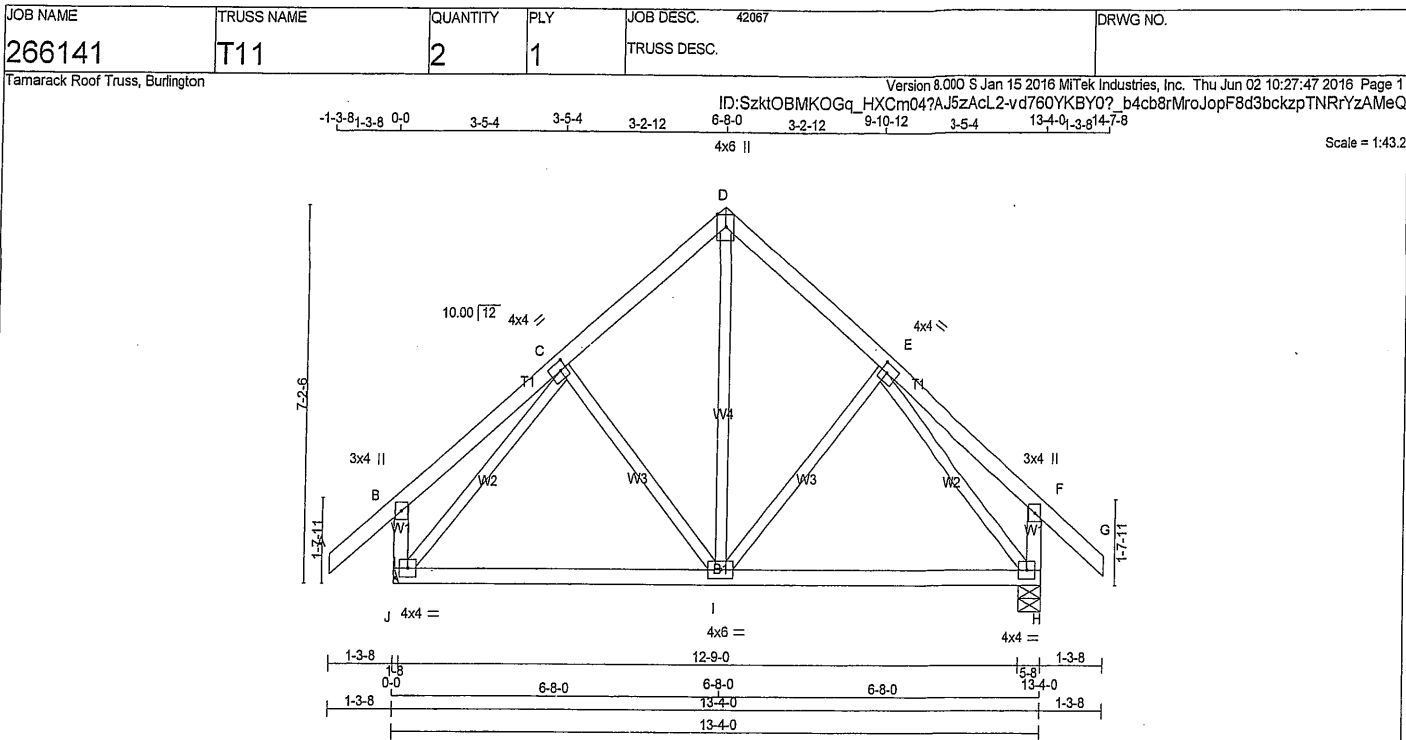
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DWG NO. TAM45832-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	4.0	6.0	Edge	
E	TMVW+t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVWV1-t	MT20	4.0	4.0		
I	BMVWVW-t	MT20	4.0	6.0		
J	BMVWV1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
J	1171	0	1171	0
H	1171	0	1171	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
J	897	616 / 0	140 / 0 0 / 0 0 / 0 142 / 0 0 / 0
H	897	616 / 0	140 / 0 0 / 0 0 / 0 142 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 54	-122.2	-122.2 0.17 (1)	10.00	I-D	0 / 527	0.12 (1)
B-C	0 / 30	-122.2	-122.2 0.21 (1)	10.00	I-E	-193 / 39	0.09 (1)
C-D	-690 / 0	-122.2	-122.2 0.17 (1)	6.25	C-I	-193 / 39	0.09 (1)
D-E	-690 / 0	-122.2	-122.2 0.17 (1)	6.25	J-C	-999 / 0	0.44 (1)
E-F	0 / 30	-122.2	-122.2 0.21 (1)	10.00	E-H	-999 / 0	0.44 (1)
F-G	0 / 54	-122.2	-122.2 0.17 (1)	10.00			
J-B	-325 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-325 / 0	0.0	0.0 0.03 (1)	7.81			
J-I	0 / 631	-28.0	-28.0 0.42 (2)	10.00			
I-H	0 / 631	-28.0	-28.0 0.42 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.21 (B-C:1), BC=0.42 (H-I:2), WB=0.44 (C-J:1), SI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 1.00)

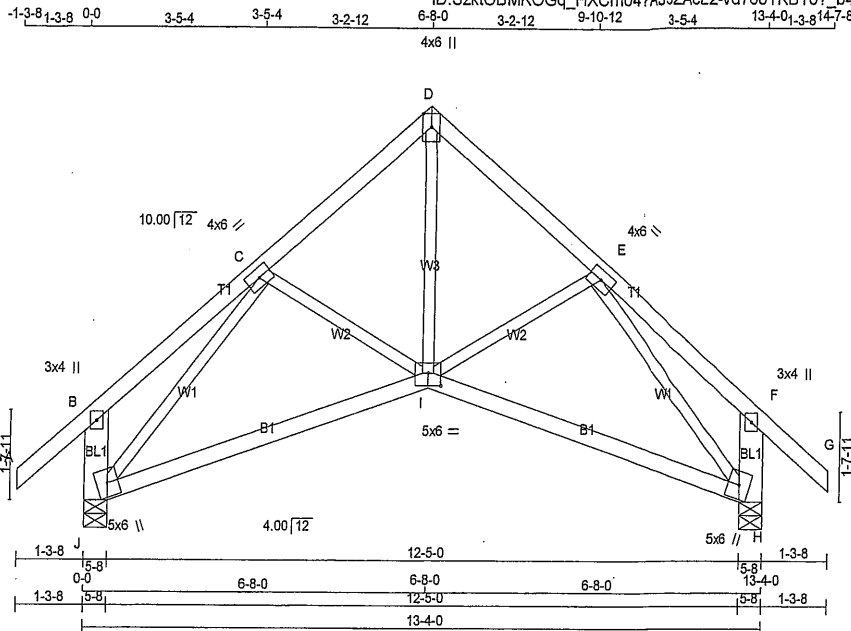


DRWG NO. TAM 2579016
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Tamarack Roof Truss, Burlington

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Scale = 1:41.3

TOTAL WEIGHT = 3 X 63 = 188 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

BEARING BLOCKS			
BL1	2x6	DRY	No.2
BL1	2x6	DRY	No.2

ALL WEBS EXCEPT			
2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMK+p	MT20	3.0	4.0		
C	TMWV-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWV-t	MT20	4.0	6.0		
F	TMK+p	MT20	3.0	4.0		
H	BWK1+m	MT20	5.0	6.0		
I	BBWVW-p	MT20	5.0	6.0	2.75	3.00
J	BWK1+m	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED GROSS REACTION				MAXIMUM FACTORED GROSS REACTION			
JT	VERT	HORZ	DOWN	UP	BRG	IN-SX	REORD
J	1171	0	1171	0	0	5-8	1-8
H	1171	0	1171	0	0	5-8	1-8

UNFACTORED REACTIONS							
1ST CASE							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	897	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0
H	897	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 59	-122.2	-122.2 0.20 (1)	10.00	I-D	0 / 845	0.19 (1)
B-C	-13 / 0	-122.2	-122.2 0.14 (1)	6.25	I-E	-119 / 55	0.03 (1)
C-D	-941 / 0	-122.2	-122.2 0.16 (1)	6.21	C-I	-119 / 55	0.03 (1)
D-E	-941 / 0	-122.2	-122.2 0.16 (1)	6.21	J-C	-1250 / 0	0.53 (1)
E-F	-13 / 0	-122.2	-122.2 0.14 (1)	6.25	E-H	-1250 / 0	0.53 (1)
F-G	0 / 59	-122.2	-122.2 0.20 (1)	10.00			
J-B	-365 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-365 / 0	0.0	0.0 0.03 (1)	7.81			
J-I	0 / 849	-28.0	-28.0 0.44 (2)	10.00			
I-H	0 / 849	-28.0	-28.0 0.44 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 38.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
 ALLOWABLE DEFL.(TL)= L/360 (0.44")
 CALCULATED VERT. DEFL.(TL) = L/665 (0.24")

CSI: TC=0.20 (F-G:1), BC=0.44 (I-J:2), WB=0.53 (C-D:1), SS=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

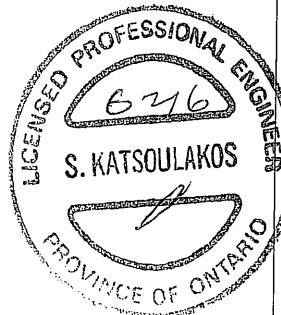
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

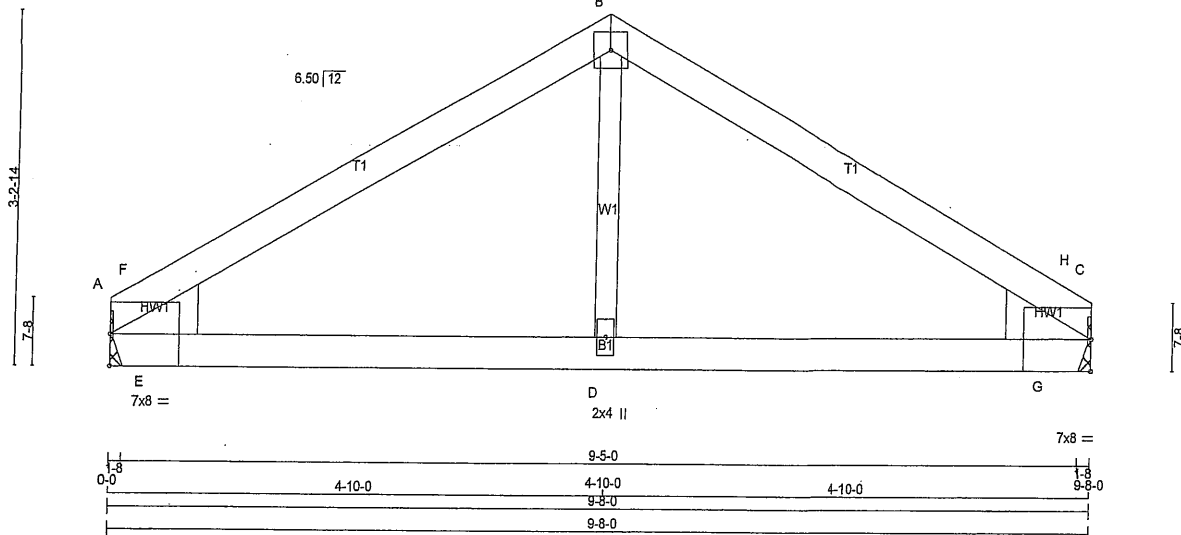
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
 JSI METAL= 0.30 (E) (INPUT = 1.00)



DWG NO. TAM 25792-16
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 COMPONENT ONLY



TOTAL WEIGHT = 3 X 29 = 87 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
A - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMBMH1-I	MT20	7.0	8.0	
B	TTW-p	MT20	4.0	4.0	
C	TMBMH1-I	MT20	7.0	8.0	
D	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
FACTORED		MAXIMUM FACTORED		INPUT		REQRD			
GROSS REACTION		GROSS REACTION		BRG		BRG		HEEL	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE	
A	726	0	726	0	0	HANGER BY OTHERS	2x6 L		
C	726	0	726	0	0	HANGER BY OTHERS	2x6 R		
MIN. SEAT SIZE: 1-8									

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	568	370 / 0	101 / 0	0 / 0	0 / 0	97 / 0	0 / 0
C	568	370 / 0	101 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. LC1 (LC)
A-F	-728 / 0	-122.2	-122.2 0.18 (1)	D-B	0 / 276	0.06 (2)	
F-B	-750 / 0	-122.2	-122.2 0.30 (1)	E-F	-257 / 196	0.00 (1)	
B-H	-750 / 0	-122.2	-122.2 0.30 (1)	G-H	-257 / 196	0.00 (1)	
H-C	-728 / 0	-122.2	-122.2 0.18 (1)				
A-E	0 / 644	-28.0	-28.0 0.25 (1)				
E-D	0 / 644	-28.0	-28.0 0.28 (1)				
D-G	0 / 644	-28.0	-28.0 0.28 (1)				
G-C	0 / 644	-28.0	-28.0 0.25 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 38.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL)= L/360 (0.32")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.30 (B-F:1), BC=0.28 (D-E:1), WB=0.06 (B-D:2), SSI=0.22 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

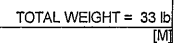
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
 JSI METAL= 0.15 (C) (INPUT = 1.00)



DWG NO. TAM 25793-16
 STRUCTURAL
 COMPONENT ONLY



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
2x4 R
BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S)

TOTAL WEIGHT = 33 LB

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A- B	0 / 36	-122.2	-122.2	0.13 (1)	10.00	I- D	-138 / 0	0.03 (1)	
B- C	-19 / 0	-122.2	-122.2	0.14 (1)	6.25	J- C	-379 / 0	0.06 (1)	
C- D	-42 / 0	-122.2	-122.2	0.14 (1)	6.25	H- E	-379 / 0	0.06 (1)	
D- E	-42 / 0	-122.2	-122.2	0.14 (1)	6.25				
E- F	-19 / 0	-122.2	-122.2	0.14 (1)	6.25				
F- G	0 / 39	-122.2	-122.2	0.16 (1)	10.00				
B- J	0 / 31	-28.0	-28.0	0.06 (2)	10.00				
J- I	0 / 17	-28.0	-28.0	0.06 (2)	10.00				
I- H	0 / 17	-28.0	-28.0	0.06 (2)	10.00				
H- F	0 / 31	-28.0	-28.0	0.06 (2)	10.00				

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16 (F-G:1), BC=0.06 (F-H:2), WB=0.06 (E-H:1), SSI=0.15 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0,250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



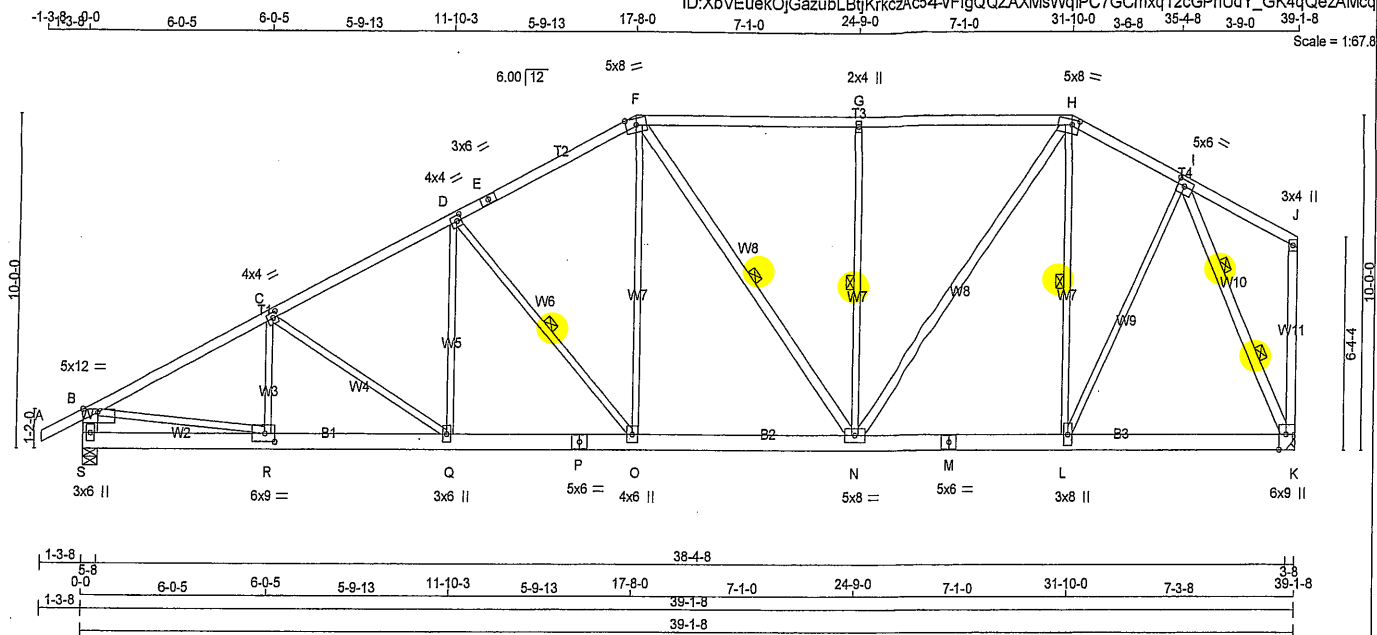
DWG NO. TAM 25794-16
STRUCTURAL
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Tamarack Roof Truss, Burlington

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Scale = 1:67.8



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - J	2x4	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
M - K	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
F - N	2x4	DRY	No.2
N - H	2x4	DRY	No.2
I - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	12.0 Edge 5.50
C	TMWW-t	MT20	4.0	4.0 2.00 1.75
D	TMWW-t	MT20	4.0	4.0 2.00 1.75
E	TS-t	MT20	3.0	6.0
F	TTWW-m	MT20	5.0	8.0 Edge
G	TMW-w	MT20	2.0	4.0
H	TTWW-m	MT20	5.0	8.0 1.75 2.75
I	TMWW-t	MT20	5.0	6.0 2.25 2.75
J	TMV-p	MT20	3.0	4.0
K	BMVW1-t	MT20	6.0	9.0 Edge 2.50
L	BMWW-t	MT20	3.0	8.0
M	BS-t	MT20	5.0	6.0
N	BMWWW-t	MT20	5.0	8.0
O	BMWW-t	MT20	4.0	6.0
P	BS-t	MT20	5.0	6.0
Q	BMWW-t	MT20	3.0	6.0
R	BMWW-t	MT20	6.0	9.0 3.00 4.00
S	BMV1-p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
	GROSS REACTION		GROSS REACTION		BRG		BRG	
S	3104	0	3104	0	5-8		4-9	
K	2939	0	2939	0			HANGER BY OTHERS	
							MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS					DEAD	SOIL
	COMBINED		SNOW	LIVE	PERM.LIVE	WIND			
S	2410	1800 / 0	411 / 0	0 / 0	0 / 0	0 / 0	399 / 0	0 / 0	0 / 0
K	2298	1496 / 0	411 / 0	0 / 0	0 / 0	0 / 0	391 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

- 1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O, F-N, G-N, H-L.
- 2 LATERAL BRACE(S) AT 1/3 LENGTH OF I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 37	-122.2 -122.2	0.16 (1)	10.00	R-C	-376 / 118	0.09 (1)
B-C	-4496 / 0	-122.2 -122.2	0.96 (1)	2.49	C-Q	-428 / 0	0.38 (1)
C-D	-4124 / 0	-122.2 -122.2	0.78 (1)	2.85	Q-D	0 / 428	0.10 (2)
D-E	-3343 / 0	-122.2 -122.2	0.71 (1)	3.19	D-O	-1099 / 0	0.60 (1)
E-F	-3343 / 0	-122.2 -122.2	0.71 (1)	3.19	O-F	0 / 1019	0.23 (1)
F-G	-2861 / 0	-122.2 -122.2	0.98 (1)	2.80	F-N	-188 / 0	0.15 (1)
G-H	-2861 / 0	-122.2 -122.2	0.98 (1)	2.80	N-G	-1070 / 0	0.70 (1)
H-I	-2085 / 0	-122.2 -122.2	0.26 (1)	4.46	N-H	0 / 1737	0.28 (1)
I-J	0 / 24	-122.2 -122.2	0.28 (1)	10.00	L-H	-968 / 0	0.63 (1)
S-B	-3012 / 0	0.0 0.0	0.19 (1)	6.07	L-I	0 / 1334	0.30 (1)
K-J	-171 / 0	0.0 0.0	0.13 (1)	7.81	B-R	0 / 4081	0.92 (1)
					I-K	-2970 / 0	0.54 (1)
S-R	0 / 0	-28.0 -28.0	0.12 (2)	10.00			
R-Q	0 / 4048	-28.0 -28.0	0.56 (1)	10.00			
Q-P	0 / 3689	-28.0 -28.0	0.50 (1)	10.00			
P-O	0 / 3689	-28.0 -28.0	0.50 (1)	10.00			
O-N	0 / 2972	-28.0 -28.0	0.42 (1)	10.00			
N-M	0 / 1831	-28.0 -28.0	0.32 (1)	10.00			
M-L	0 / 1831	-28.0 -28.0	0.32 (1)	10.00			
L-K	0 / 1269	-28.0 -28.0	0.27 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 38.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL)= L/999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL)= L/999 (0.36")

CSI: TC=0.98 (F-G:1), BC=0.56 (Q-R:1), WB=0.82 (B-R:1), SSI=0.42 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

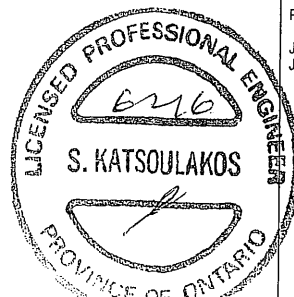
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

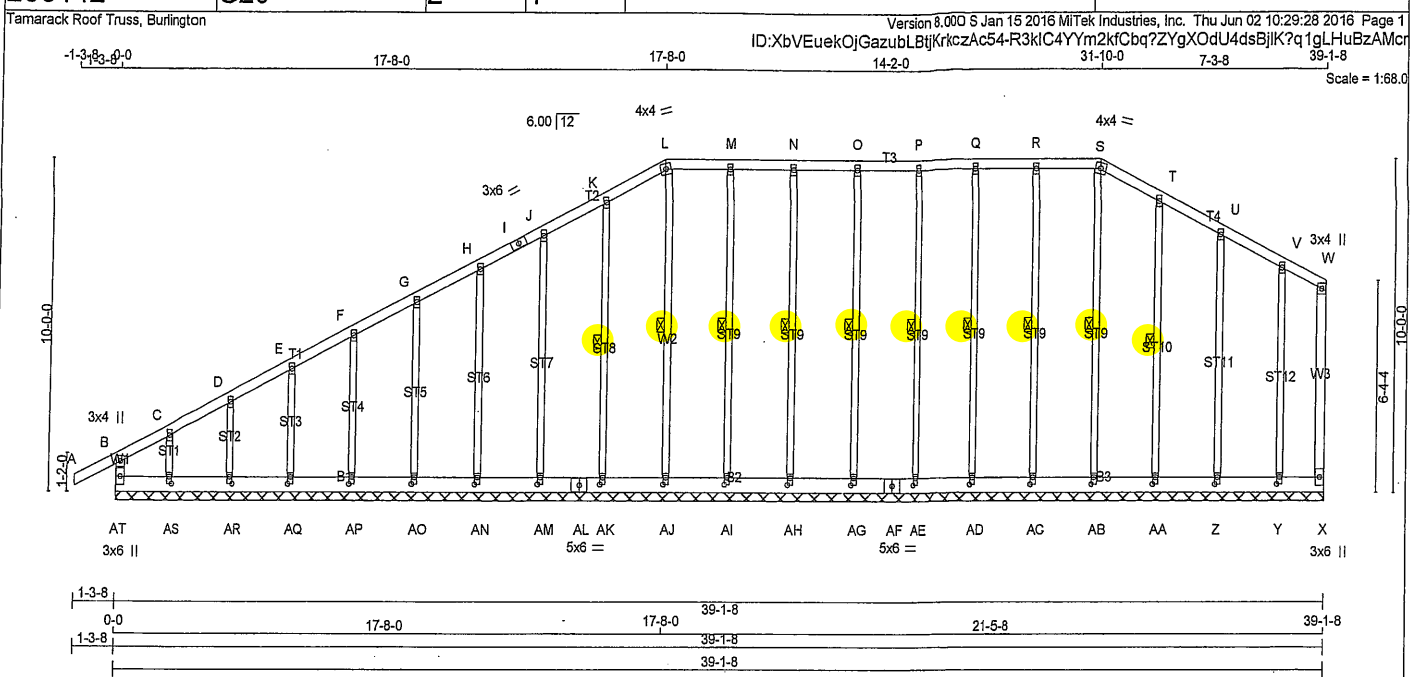
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.67 (R) (INPUT = 1.00)



DRWG NO. TAM25798-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 237 = 473 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
AT- B	2x4	DRY	No.2
A- I	2x4	DRY	No.2
I- L	2x4	DRY	No.2
L- S	2x4	DRY	No.2
S- W	2x4	DRY	No.2
X- W	2x4	DRY	No.2
AT- AL	2x6	DRY	No.2
AL- AF	2x6	DRY	No.2
AF- X	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C, D, E, F, G, H, J, K, M, N, O, P, Q, R, T, U, V					
C TMW+w	MT20	2.0	4.0		
I TS-t	MT20	3.0	6.0		
L TTW-m	MT20	4.0	4.0		
S TTW-m	MT20	4.0	4.0		
W TMV+p	MT20	3.0	4.0		
X BMV1+p	MT20	3.0	6.0		
Y, Z, AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AM, AN, AO, AP, AQ, AR, AS					
Y BMW1+w	MT20	2.0	4.0	2.50	1.00
AF BS-t	MT20	5.0	6.0		
AL BS-t	MT20	5.0	6.0		
AT BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF S-AB, R-AC, Q-AD, P-AE, O-AG, N-AH, M-AI, K-AK, T-AA, L-AJ.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO (LC)	
FR-TO				FR-TO			
AT-B	-320 / 0	0.0	0.0 0.06 (1)	AB-S	-250 / 0	0.16 (1)	
A-B	0 / 37	-122.2	-122.2 0.16 (1)	AC-R	-253 / 0	0.17 (1)	
B-C	-47 / 0	-122.2	-122.2 0.12 (1)	AD-Q	-243 / 0	0.16 (1)	
C-D	-16 / 0	-122.2	-122.2 0.06 (1)	AE-P	-245 / 0	0.16 (1)	
D-E	-16 / 0	-122.2	-122.2 0.06 (1)	AG-O	-245 / 0	0.16 (1)	
E-F	-9 / 0	-122.2	-122.2 0.06 (1)	AH-N	-243 / 0	0.16 (1)	
F-G	-6 / 0	-122.2	-122.2 0.06 (1)	AI-M	-250 / 0	0.16 (1)	
G-H	-3 / 0	-122.2	-122.2 0.06 (1)	AK-K	-244 / 0	0.12 (1)	
H-I	0 / 0	-122.2	-122.2 0.06 (1)	AM-J	-243 / 0	0.28 (1)	
I-J	0 / 0	-122.2	-122.2 0.06 (1)	AN-H	-243 / 0	0.20 (1)	
J-K	0 / 2	-122.2	-122.2 0.06 (1)	AO-G	-243 / 0	0.13 (1)	
K-L	0 / 4	-122.2	-122.2 0.06 (1)	AP-F	-243 / 0	0.08 (1)	
L-M	0 / 7	-122.2	-122.2 0.06 (1)	AQ-E	-239 / 0	0.08 (1)	
M-N	0 / 7	-122.2	-122.2 0.06 (1)	AR-D	-258 / 0	0.04 (1)	
N-O	0 / 7	-122.2	-122.2 0.06 (1)	AS-C	-169 / 0	0.02 (1)	
O-P	0 / 7	-122.2	-122.2 0.06 (1)	AA-T	-239 / 0	0.12 (1)	
P-Q	0 / 7	-122.2	-122.2 0.06 (1)	Z-U	-252 / 0	0.30 (1)	
Q-R	0 / 7	-122.2	-122.2 0.06 (1)	Y-V	-211 / 0	0.17 (1)	
R-S	0 / 7	-122.2	-122.2 0.06 (1)	AJ-L	-250 / 0	0.16 (1)	
S-T	0 / 5	-122.2	-122.2 0.06 (1)				
T-U	0 / 2	-122.2	-122.2 0.06 (1)				
U-V	0 / 3	-122.2	-122.2 0.06 (1)				
V-W	0 / 5	-122.2	-122.2 0.04 (1)				
X-W	-67 / 0	0.0	0.0 0.01 (1)				
AT-AS	0 / 26	-28.0	-28.0 0.03 (1)				
AS-AR	0 / 19	-28.0	-28.0 0.01 (2)				
AR-AQ	0 / 13	-28.0	-28.0 0.01 (2)				
AQ-AP	0 / 9	-28.0	-28.0 0.01 (2)				
AP-AO	0 / 5	-28.0	-28.0 0.01 (2)				
AO-AN	0 / 2	-28.0	-28.0 0.01 (2)				
AN-AM	0 / 0	-28.0	-28.0 0.01 (2)				
AM-AL	-2 / 0	-28.0	-28.0 0.01 (2)				
AL-AK	-2 / 0	-28.0	-28.0 0.01 (2)				
AK-AJ	-4 / 0	-28.0	-28.0 0.01 (2)				
AJ-AI	-7 / 0	-28.0	-28.0 0.01 (2)				
AI-AH	-7 / 0	-28.0	-28.0 0.01 (2)				
AH-AG	-7 / 0	-28.0	-28.0 0.01 (2)				
AG-AF	-7 / 0	-28.0	-28.0 0.01 (2)				
AF-AE	-7 / 0	-28.0	-28.0 0.01 (2)				
AE-AD	-7 / 0	-28.0	-28.0 0.01 (2)				
AD-AC	-7 / 0	-28.0	-28.0 0.01 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16 (A-B:1), BC=0.03 (AS-AT:1), WB=0.30 (U-Z:1), SSI=0.12 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (L) (INPUT = 0.90)

JSI METAL= 0.07 (S) (INPUT = 1.00)

6216

S. KATSOUAKOS

PROVINCE OF ONTARIO

DWG NO. TAM 25799-16

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 266142	TRUSS NAME G20	QUANTITY 2	PLY 1	JOB DESC. 42067 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MiTek Industries, Inc. Thu Jun 02 10:29:29 2016 Page 2

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LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO		LENGTH	FR-TO			
AC-AB	-7 / 0	-28.0	-28.0	0.01 (2)	10.00				
AB-AA	-4 / 0	-28.0	-28.0	0.01 (2)	10.00				
AA-Z	-2 / 0	-28.0	-28.0	0.01 (2)	10.00				
Z-Y	0 / 0	-28.0	-28.0	0.01 (2)	10.00				
Y-X	0 / 2	-28.0	-28.0	0.01 (3)	10.00				



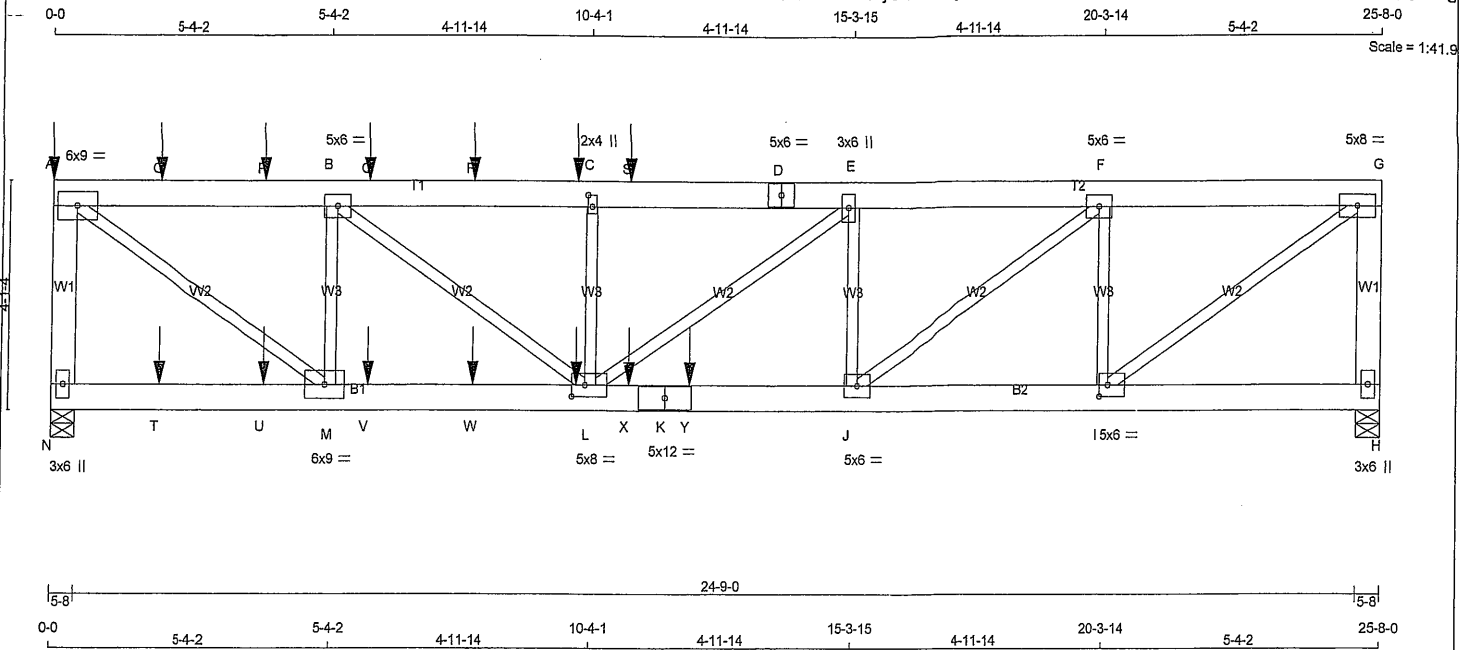
DWG NO. TAM 25799-16

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
266142	T21	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 08:33:23 2017 Page 1
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HANGERS NOTES

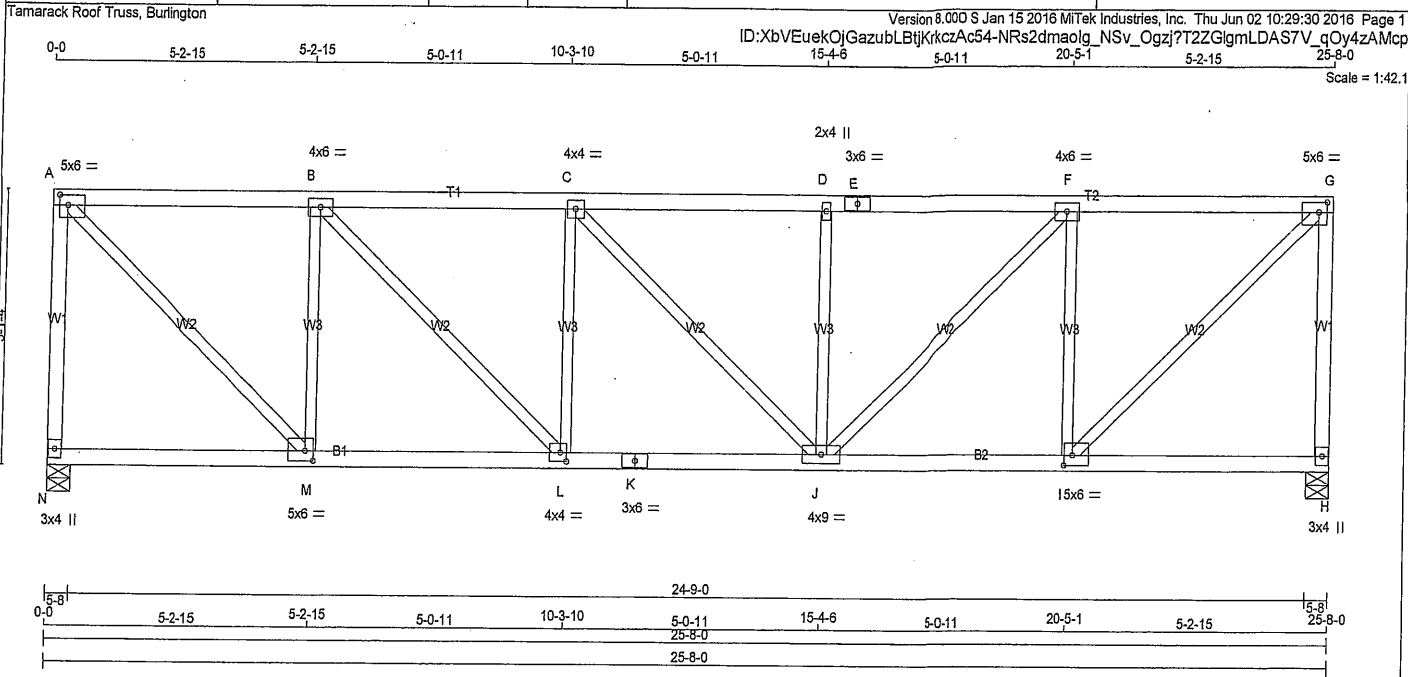
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 204.4 lbs FACTORED DOWN AT 0-0, 147.1 lbs FACTORED DOWN AT 2-0-12, 147.1 lbs FACTORED DOWN AT 4-0-12, 147.1 lbs FACTORED DOWN AT 6-0-12, 147.1 lbs FACTORED DOWN AT 8-0-12, AND 147.1 lbs FACTORED DOWN AT 10-0-12, AND 147.1 lbs FACTORED DOWN AT 11-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-0-12, 69.9 lbs FACTORED DOWN AT 4-0-12, 69.9 lbs FACTORED DOWN AT 6-0-12, 69.9 lbs FACTORED DOWN AT 8-0-12, 69.9 lbs FACTORED DOWN AT 10-0-12, AND 69.9 lbs FACTORED DOWN AT 11-0-12, AND 1837.6 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	8-0-12	-40	-70	—	BACK	VERT	TOTAL
X	11-0-12	-40	-70	—	BACK	VERT	TOTAL
Y	12-2-8	-1838	-1838	—	BACK	VERT	TOTAL



DWONG.TAM 45837-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
N - A	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	2.00
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0		
D	TMVW-t	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0	2.25	2.00
J	BMVW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	4.0	2.00	1.50
M	BMVW-t	MT20	5.0	6.0	2.25	2.00
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG
N	1928 0	1928 0	5-8
H	1928 0	1928 0	5-8

UNFACTORED REACTIONS							
JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1508	981/0	270/0	0/0	0/0	257/0	0/0
H	1508	981/0	270/0	0/0	0/0	257/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
N-A	-1867/0	0.0	0.0 0.82 (1)	I-G	0/2382	0.54 (1)	
A-B	-1755/0	-122.2	-122.2 0.55 (1)	A-M	0/2381	0.54 (1)	
B-C	-2528/0	-122.2	-122.2 0.62 (1)	I-F	-1447/0	0.56 (1)	
C-D	-2526/0	-122.2	-122.2 0.46 (1)	M-B	-1448/0	0.56 (1)	
D-E	-2526/0	-122.2	-122.2 0.62 (1)	J-F	0/1064	0.24 (1)	
E-F	-2526/0	-122.2	-122.2 0.62 (1)	B-L	0/1067	0.24 (1)	
F-G	-1755/0	-122.2	-122.2 0.55 (1)	J-D	-597/0	0.23 (1)	
H-G	-1867/0	0.0	0.0 0.82 (1)	L-C	-598/0	0.23 (1)	
				C-J	-3/0	0.00 (1)	
N-M	0/0	-28.0	-28.0 0.18 (3)				
M-L	0/1755	-28.0	-28.0 0.37 (1)				
L-K	0/2528	-28.0	-28.0 0.47 (1)				
K-J	0/2528	-28.0	-28.0 0.47 (1)				
J-I	0/1755	-28.0	-28.0 0.38 (1)				
I-H	0/0	-28.0	-28.0 0.18 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 38.3 PSF
 DL = 3.0 PSF
 ROT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
 ALLOWABLE DEFL.(TL) = L/360 (0.86")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.82 (G-H:1), BC=0.47 (J-L:1), WB=0.56 (B-M:1), SSI=0.30 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

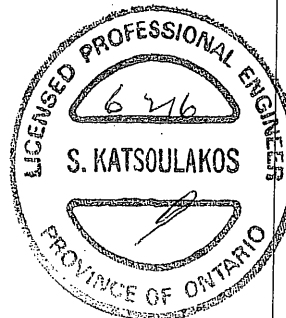
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

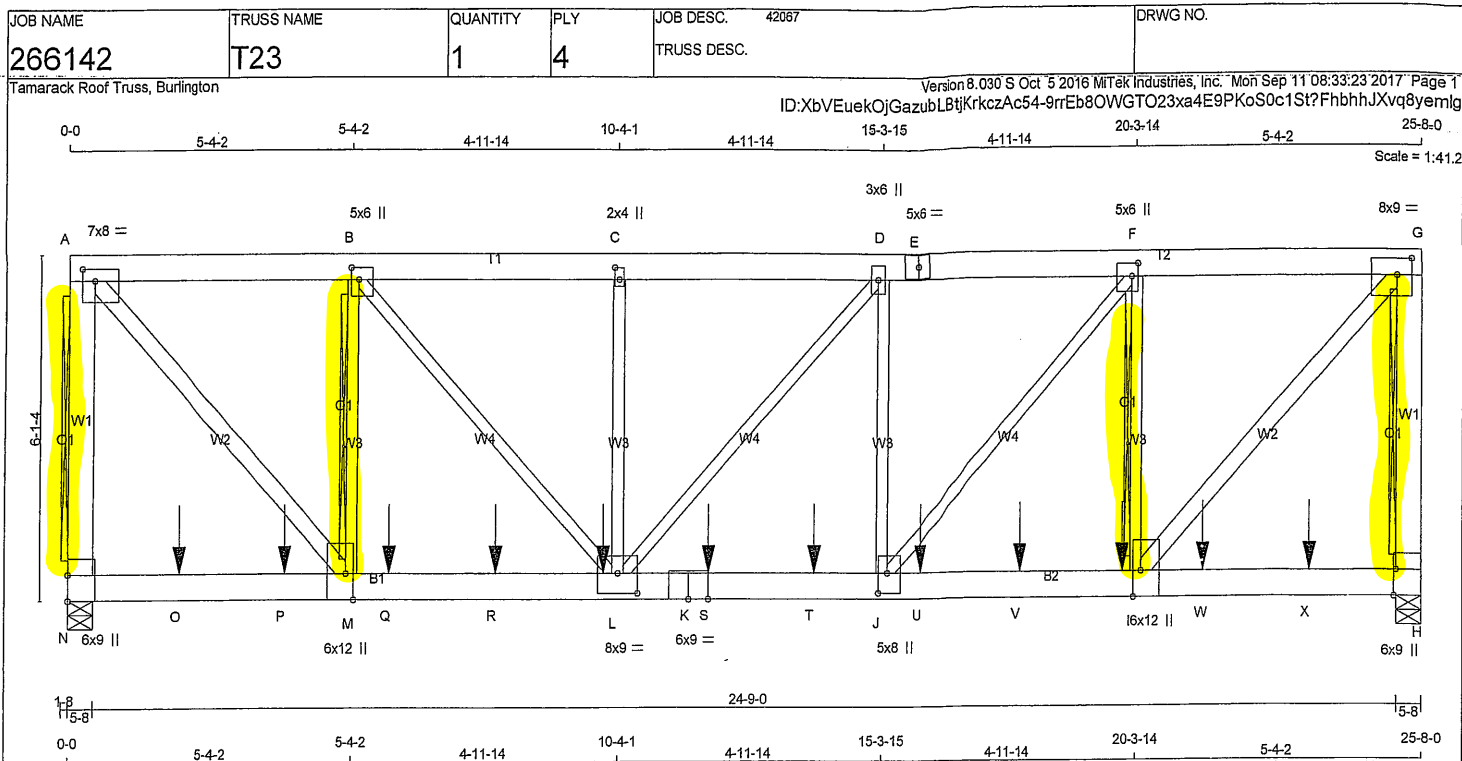
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
 JSI METAL= 0.69 (K) (INPUT = 1.00)



DRWG NO. TAM25801-16
 STRUCTURAL
 COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
N - A	2x6	DRY No.2	SPF
A - E	2x6	DRY 2100F 1.8E	SPF
E - G	2x6	DRY 2100F 1.8E	SPF
H - G	2x6	DRY No.2	SPF
N - K	2x6	DRY 2100F 1.8E	SPF
K - H	2x6	DRY 2100F 1.8E	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - G	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
N-A	2 12	TOP
A-E	2 12	TOP
E-G	2 12	TOP
G-H	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N-K	3 5	SIDE(1142.9)
K-H	3 5	SIDE(1397.2)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1 6	
L-C	1 6	
2x4	1 6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSAO86 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	7.0	8.0 2.50 2.75
B	TMVW-t	MT20	5.0	6.0 2.50 1.75
C	TMW-w	MT20	2.0	4.0 2.50 1.00
D	TMVW-t	MT20	3.0	8.0
E	TS-t	MT20	5.0	6.0
F	TMVW-t	MT20	5.0	6.0 2.75 1.50
G	TMVW-t	MT20	8.0	9.0 3.50 3.25
H	BMV1-t	MT20	6.0	9.0 Edge 0.50
I	BMVW-t	MT20	6.0	12.0 Edge 1.75
J	BMVW-t	MT20	5.0	8.0 4.25 2.00
K	BS-t	MT20	6.0	9.0
L	BMVW-t	MT20	8.0	9.0 4.25 4.50
M	BMVW-t	MT20	6.0	12.0 Edge 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
N	19122	0	19122	0	5-8
H	19659	0	19659	0	5-8

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS			
N	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
N	14877	9819 / 0	2572 / 0	0 / 0	0 / 0	2485 / 0
H	15295	10096 / 0	2644 / 0	0 / 0	0 / 0	2555 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT A-N, G-H, F-I, B-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	
TOTAL LOAD CASES: (4)	

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
N-A	-17310 / 0	I-F	-8134 / 0
A-B	-16080 / 0	M-B	-8513 / 0
B-C	-23025 / 0	I-G	0 / 24013
C-D	-23025 / 0	A-M	0 / 23387
D-E	-23141 / 0	J-F	0 / 10012
E-F	-23141 / 0	B-L	0 / 10486
F-G	-16510 / 0	J-D	-477 / 2
G-H	-17772 / 0	L-C	-574 / 0
		L-D	-175 / 0
N-O	0 / 0		
O-P	0 / 0		
P-M	0 / 0		
M-Q	0 / 16080		
Q-R	0 / 16080		
R-L	0 / 16080		
L-K	0 / 23141		
K-S	0 / 23141		
S-T	0 / 23141		
T-J	0 / 23141		
J-U	0 / 16510		
U-V	0 / 16510		
V-I	0 / 16510		
I-W	0 / 0		
W-X	0 / 0		
X-H	0 / 0		

FACTORED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1	MAX. MAX+
I	20-0-12	-2911	-2911
L	10-0-12	-2911	-2911
O	2-0-12	-2911	-2911

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/998 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/882 (0.35")

CSI: TC=0.40 (G-H-1), BC=0.67 (L-M-1), WB=0.96 (G-H-1), SSI=0.72 (L-M-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.95 (K) (INPUT = 1.00)



DWG NO. TAM 4583B 17
STRUCTURAL
COMPONENT ONLY

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMV1+t	MT20	6.0	9.0	5.50	

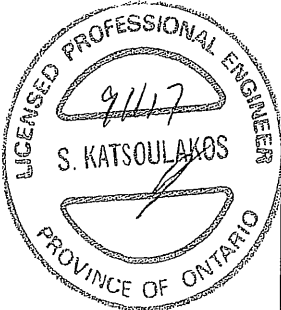
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2910.5 lbs FACTORED DOWN AT 2-0-12, 2910.5 lbs FACTORED DOWN AT 4-0-12, 2910.5 lbs FACTORED DOWN AT 6-0-12, 2910.5 lbs FACTORED DOWN AT 8-0-12, 2910.5 lbs FACTORED DOWN AT 10-0-12, 2910.5 lbs FACTORED DOWN AT 12-0-12, 2910.5 lbs FACTORED DOWN AT 14-0-12, 2910.5 lbs FACTORED DOWN AT 16-0-12, 2910.5 lbs FACTORED DOWN AT 18-0-12, 2910.5 lbs FACTORED DOWN AT 20-0-12, AND 2910.5 lbs FACTORED DOWN AT 21-7-4, AND 2910.5 lbs FACTORED DOWN AT 23-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	4-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
Q	6-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
R	8-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
S	12-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
T	14-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
U	16-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
V	18-0-12	-2911	-2911	—	FRONT	VERT	TOTAL
W	21-7-4	-2911	-2911	—	FRONT	VERT	TOTAL
X	23-7-4	-2911	-2911	—	FRONT	VERT	TOTAL



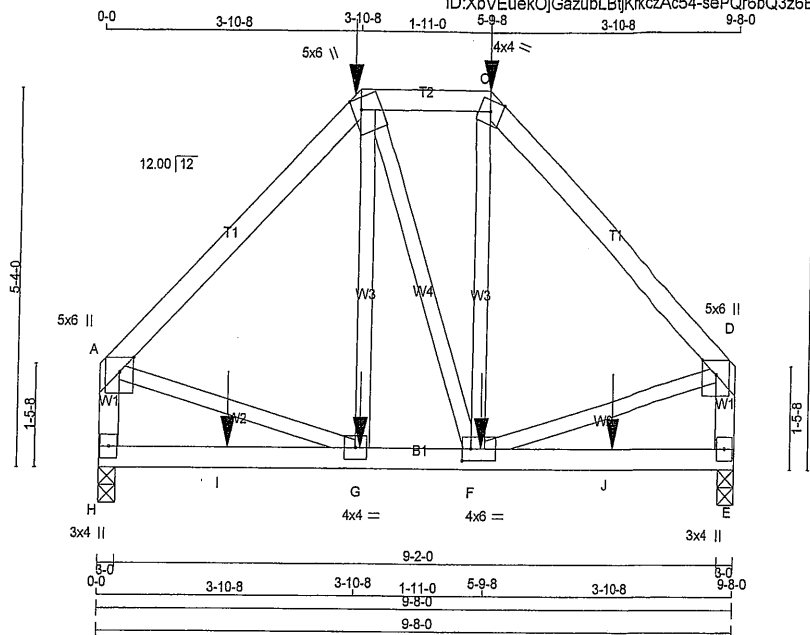
DWG NO. TAM4583B-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

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Scale: 3/8"=1'



TOTAL WEIGHT = 47 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
H - A	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
H	1121	0	1121	0
E	1121	0	1121	0

UNFACTORED REACTIONS

	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	585 / 0	137 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	862	585 / 0	137 / 0	0 / 0	0 / 0	140 / 0	0 / 0
E	862	585 / 0	137 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.90 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-913 / 0	-122.2	-122.2	0.38 (1)	5.90	G-B	-53 / 159
B-C	-641 / 0	-122.2	-122.2	0.09 (1)	6.25	B-F	-1 / 3
C-D	-912 / 0	-122.2	-122.2	0.38 (1)	5.90	F-C	-54 / 164
H-A	-1063 / 0	0.0	0.0	0.12 (1)	7.61	A-G	0 / 667
E-D	-1063 / 0	0.0	0.0	0.12 (1)	7.61	F-D	0 / 667
H-I	0 / 0	-28.0	-28.0	0.15 (2)	10.00		
I-G	0 / 0	-28.0	-28.0	0.15 (2)	10.00		
G-F	0 / 642	-28.0	-28.0	0.18 (2)	10.00		
F-J	0 / 0	-28.0	-28.0	0.15 (2)	10.00		
J-E	0 / 0	-28.0	-28.0	0.15 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
B	3-10-8	-348	-348	---	BACK	VERT	TOTAL
C	5-9-8	-348	-348	---	BACK	VERT	TOTAL
F	5-9-12	-24	-41	---	BACK	VERT	TOTAL
G	3-11-4	-24	-41	---	BACK	VERT	TOTAL
I	1-11-4	-24	-41	---	BACK	VERT	TOTAL
J	7-8-12	-24	-41	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 38.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL)= L/360 (0.32")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.38 (A-B:1), BC=0.18 (F-G:2), WB=0.17 (A-G:1), SS=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
 JSI METAL= 0.24 (G) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMVW+p	MT20	5.0	6.0	Edge	
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	6.0	2.00	1.50
G	BMVWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 3-10-8, AND 348.1 lbs FACTORED DOWN AT 5-9-8 ON TOP CHORD, AND 41.4 lbs FACTORED DOWN AT 1-11-4, 41.4 lbs FACTORED DOWN AT 3-11-4, AND 41.4 lbs FACTORED DOWN AT 5-8-12, AND 41.4 lbs FACTORED DOWN AT 7-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 2580416
 STRUCTURAL
 COMPONENT ONLY

266142

T26

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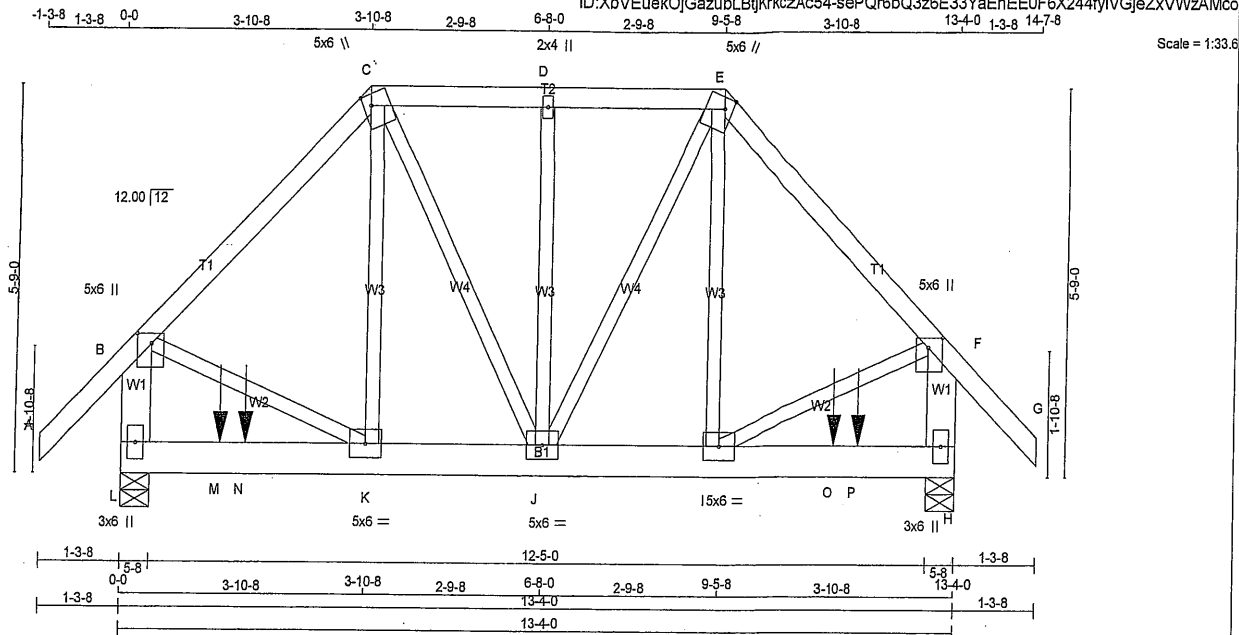
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TRUSS DESC.

Tamarack Roof Truss, Burlington

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Scale = 1:33.6

TOTAL WEIGHT = 80 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
L - H	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
L	2149	0	0	5-8
H	2149	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED						
L	1657	1118 / 0	270 / 0	0 / 0	0 / 0	289 / 0	0 / 0
H	1657	1118 / 0	270 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 60	-122.2 -122.2	0.19 (1)	10.00	K-C	0 / 558	0.14 (1)
B-C	-1343 / 0	-122.2 -122.2	0.40 (1)	5.06	C-J	-13 / 18	0.01 (3)
C-D	-965 / 0	-122.2 -122.2	0.17 (1)	6.09	J-D	-397 / 0	0.19 (1)
D-E	-965 / 0	-122.2 -122.2	0.17 (1)	6.09	J-E	-13 / 18	0.01 (3)
E-F	-1343 / 0	-122.2 -122.2	0.40 (1)	5.06	I-E	0 / 558	0.14 (1)
F-G	0 / 60	-122.2 -122.2	0.19 (1)	10.00	B-K	0 / 1006	0.25 (1)
L-B	-1703 / 0	0.0	0.0	0.13 (1)	I-F	0 / 1006	0.25 (1)
H-F	-1703 / 0	0.0	0.0	0.13 (1)			
L-M	0 / 0	-28.0	-28.0	0.60 (1)	10.00		
M-N	0 / 0	-28.0	-28.0	0.60 (1)	10.00		
N-K	0 / 0	-28.0	-28.0	0.60 (1)	10.00		
K-J	0 / 957	-28.0	-28.0	0.44 (1)	10.00		
J-I	0 / 957	-28.0	-28.0	0.44 (1)	10.00		
I-O	0 / 0	-28.0	-28.0	0.60 (1)	10.00		
O-P	0 / 0	-28.0	-28.0	0.60 (1)	10.00		
P-H	0 / 0	-28.0	-28.0	0.60 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	1-6-12	-24	-43	---	BACK	VERT	TOTAL
N	1-11-8	-952	-952	---	BACK	VERT	TOTAL
O	11-4-8	-952	-952	---	BACK	VERT	TOTAL
P	11-9-4	-24	-43	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.40 (E-F:1), BC=0.60 (K-L:1), WB=0.25 (F-I:1), SSI=0.43 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

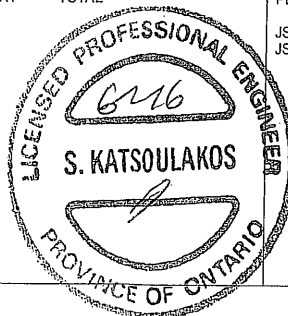
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

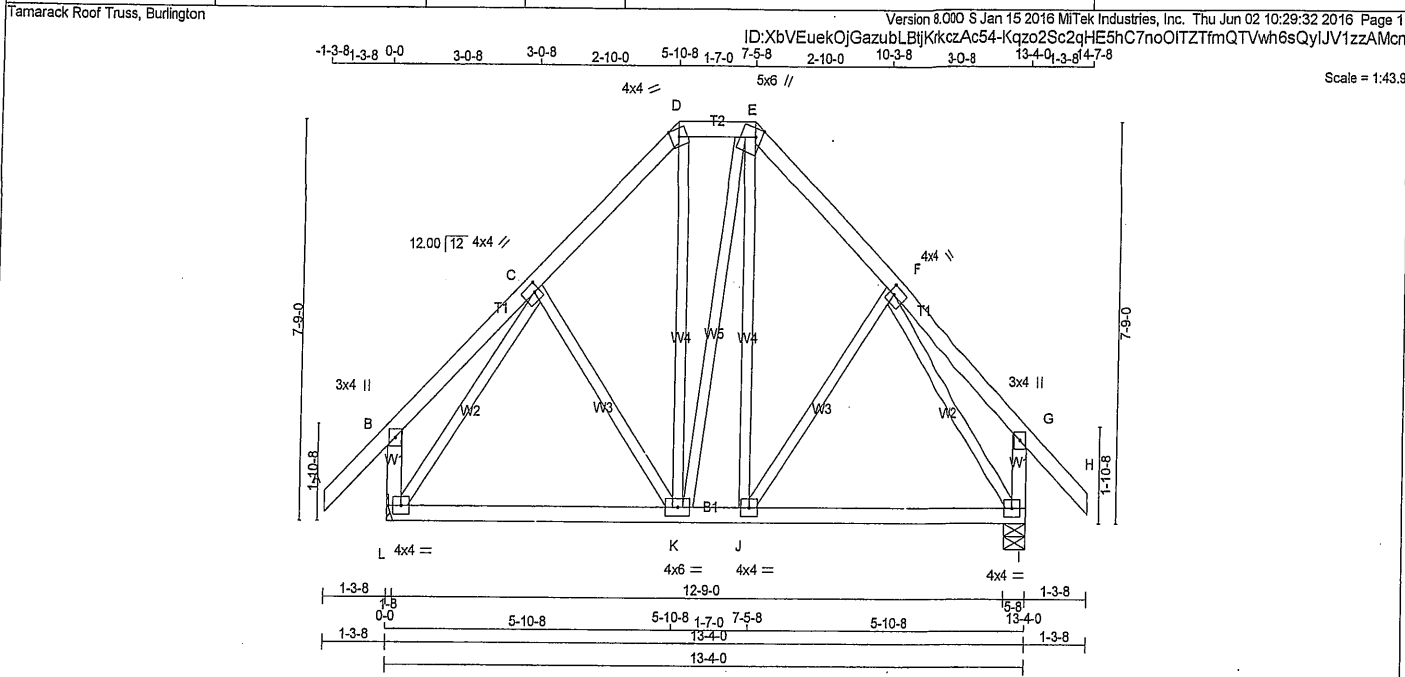
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.29 (F) (INPUT = 1.00)DWG NO. TAM 2560516
STRUCTURAL
COMPONENT ONLY



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+t	MT20	4.0	4.0	2.00	1.25
D	TTW+m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW+t	MT20	4.0	4.0	2.00	1.25
G	TMV+p	MT20	3.0	4.0		
I	BMVVW+t	MT20	4.0	4.0		
J	BMWW+t	MT20	4.0	4.0		
K	BMWWW+t	MT20	4.0	6.0		
L	BMVVW+t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	VERT HORZ	DOWN HORZ	UPLIFT	IN-SX
I	1172 0	1172 0	0	HANGER BY OTHERS
			5-8	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
L	898	616 / 0 140 / 0 0 / 0 0 / 0 142 / 0 0 / 0
I	898	616 / 0 140 / 0 0 / 0 0 / 0 142 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		
		VERT. (PLF)	LOAD LC1	MAX CSI (LC)	UNBRAC LENGTH			MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	C-K	-122 / 43	0.06 (1)	
B-C	0 / 29	-122.2	-122.2	0.17 (1)	10.00	K-D	0 / 218	0.05 (1)	
C-D	-672 / 0	-122.2	-122.2	0.13 (1)	6.25	K-E	0 / 7	0.00 (1)	
D-E	-456 / 0	-122.2	-122.2	0.04 (1)	6.25	J-E	0 / 210	0.05 (1)	
E-F	-670 / 0	-122.2	-122.2	0.13 (1)	6.25	J-F	-122 / 42	0.06 (1)	
F-G	0 / 29	-122.2	-122.2	0.17 (1)	10.00	L-C	-953 / 0	0.43 (1)	
G-H	0 / 60	-122.2	-122.2	0.17 (1)	10.00	F-I	-951 / 0	0.43 (1)	
L-B	-306 / 0	0.0	0.0	0.03 (1)	7.81				
I-G	-306 / 0	0.0	0.0	0.03 (1)	7.81				
L-K	0 / 522	-28.0	-28.0	0.27 (2)	10.00				
K-J	0 / 454	-28.0	-28.0	0.27 (2)	10.00				
J-I	0 / 521	-28.0	-28.0	0.26 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS: TOP CH. LL = 38.3 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, CBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44") CALCULATED VERT. DEFL.(LL) = L/999 (0.05") ALLOWABLE DEFL.(TL)= L/360 (0.44") CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.17 (G-H:1), BC=0.27 (K-L:2), WB=0.43 (C-L:1), SSI=0.14 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

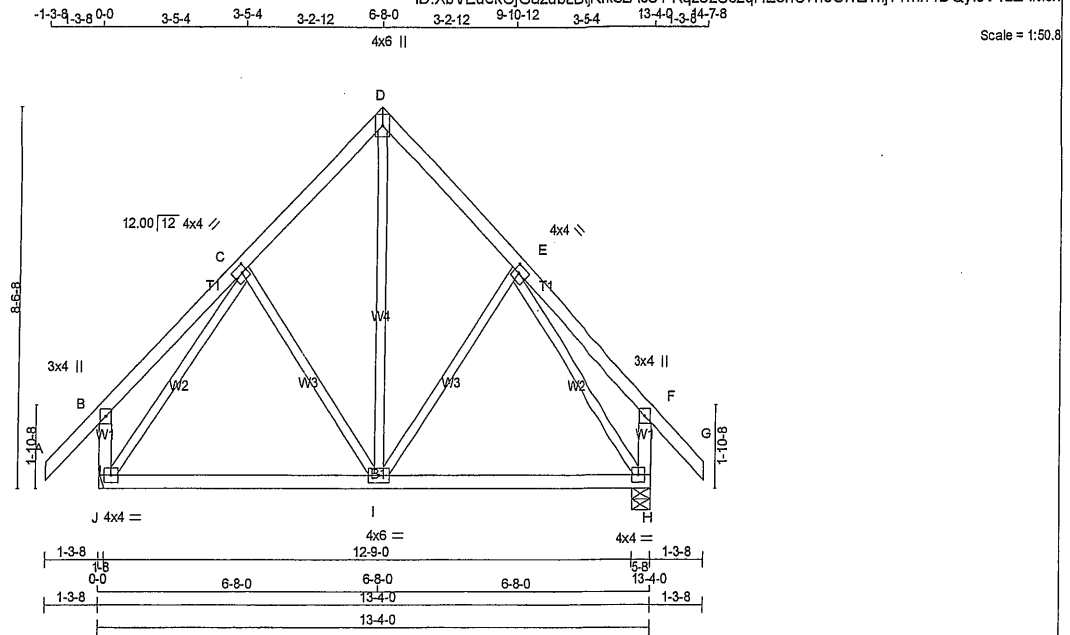
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90) JSI METAL= 0.36 (C) (INPUT = 1.00)



DRWG NO. YAM 2580616 STRUCTURAL COMPONENT ONLY



TOTAL WEIGHT = 2 X 69 = 139 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG			
J	1172 0	1172 0	0	0	IN-SX	IN-SX	
H	1172 0	1172 0	0	5-8	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	898	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0	
H	898	616 / 0	140 / 0	0 / 0	0 / 0	142 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. LC1 CSI (LC)
FR-TO				FR-TO			
A-B	0 / 60	-122.2	-122.2 0.17 (1)	I-D	0 / 530	10.00	0.12 (1)
B-C	0 / 33	-122.2	-122.2 0.22 (1)	I-E	-183 / 35	10.00	0.11 (1)
C-D	-634 / 0	-122.2	-122.2 0.17 (1)	C-I	-183 / 35	6.25	0.11 (1)
D-E	-634 / 0	-122.2	-122.2 0.17 (1)	J-C	-940 / 0	6.25	0.54 (1)
E-F	0 / 33	-122.2	-122.2 0.22 (1)	E-H	-940 / 0	10.00	0.54 (1)
F-G	0 / 60	-122.2	-122.2 0.17 (1)				
J-B	-324 / 0	0.0	0.0 0.04 (1)				
H-F	-324 / 0	0.0	0.0 0.04 (1)				
J-I	0 / 531	-28.0	-28.0 0.41 (2)				
I-H	0 / 531	-28.0	-28.0 0.41 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 38.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.44")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.22 (B-C:1), BC=0.41 (I-J:2), WB=0.54 (E-H:1), SSI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

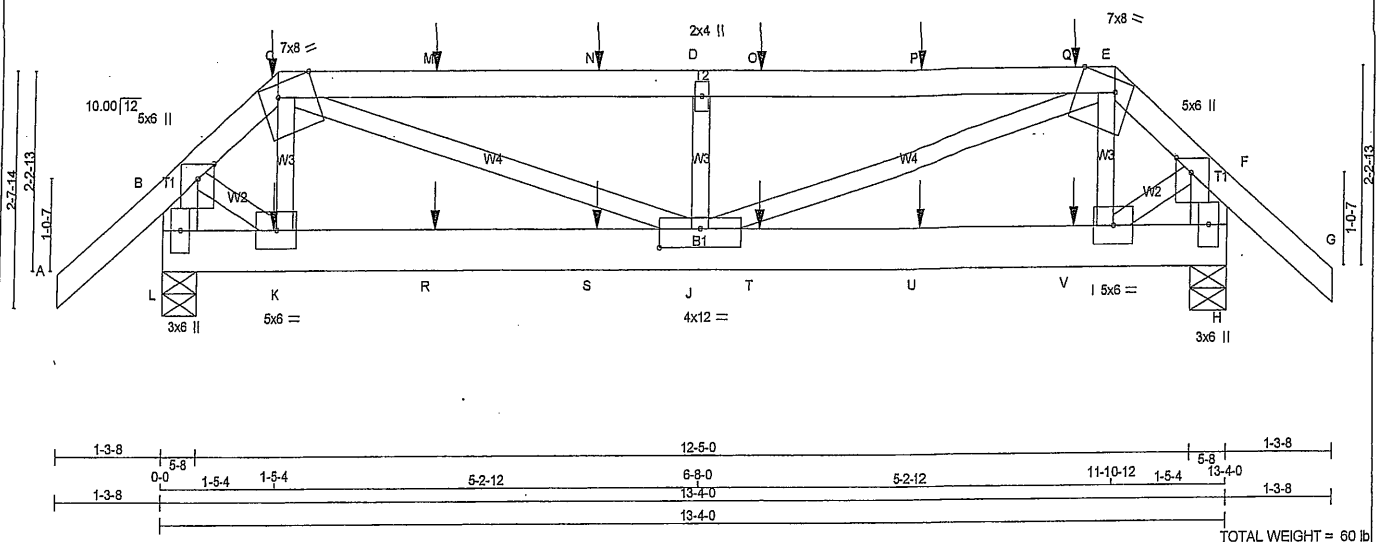
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
 JSI METAL= 0.36 (C) (INPUT = 1.00)



DRWG NO. TAN 2500716
 STRUCTURAL
 COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY 2100F 1.8E SPF
C - E 2x4 DRY 2100F 1.8E SPF
E - G 2x4 DRY 2100F 1.8E SPF
L - B 2x6 DRY No.2 SPF
H - F 2x6 DRY No.2 SPF
L - H 2x6 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	7.0	8.0	Edge	5.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	7.0	8.0	Edge	5.50
F	TMW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	6.0		
I	BMWV-t	MT20	5.0	6.0		
J	BMWVW-t	MT20	4.0	12.0	2.50	6.00
K	BMWV-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 105.3 lbs FACTORED DOWN AT 1-5-4, 101.7 lbs FACTORED DOWN AT 3-4-12, 101.7 lbs FACTORED DOWN AT 5-4-12, 101.7 lbs FACTORED DOWN AT 7-4-12, AND 101.7 lbs FACTORED DOWN AT 9-4-12, AND 132.3 lbs FACTORED DOWN AT 11-4-12 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 1-4-12, 70.4 lbs FACTORED DOWN AT 3-4-12, 70.4 lbs FACTORED DOWN AT 5-4-12, 70.4 lbs FACTORED DOWN AT 7-4-12, AND 70.4 lbs FACTORED DOWN AT 9-4-12, AND 70.4 lbs FACTORED DOWN AT 11-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1715	0	1715	0
H	1694	0	1694	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIV	WIND	DEAD	SOIL
L	1322	892 / 0	215 / 0	0 / 0	0 / 0	216 / 0	0 / 0
H	1304	885 / 0	209 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING
TOTAL LOAD CASES: 1

MEMB.	CHORDS MAX. FACTORED FORCE (BS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	CHORDS MAX. FACTORED FORCE (BS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH (FT)
FR-TO									
A-B	-1644 / 0	-122.2	-122.2	0.12 (1)	10.9	C-C	-20 / 154	0.04 (3)	
B-C	-1644 / 0	-122.2	-122.2	0.13 (1)	5.99	C-J	0 / 1861	0.46 (1)	
C-M	-1967 / 0	-122.2	-122.2	0.62 (1)	4.09	J-D	-1078 / 0	0.17 (1)	
M-N	-1967 / 0	-122.2	-122.2	0.62 (1)	4.09	J-E	0 / 1874	0.46 (1)	
N-G	-1967 / 0	-122.2	-122.2	0.62 (1)	4.09	I-E	-29 / 151	0.04 (3)	
D-L	-1967 / 0	-122.2	-122.2	0.62 (1)	4.09	B-K	0 / 1307	0.32 (1)	
Q-P	-1967 / 0	-122.2	-122.2	0.62 (1)	4.09	I-F	0 / 1293	0.32 (1)	
P-Q	-1967 / 0	-122.2	-122.2	0.62 (1)	4.09				
Q-E	-1967 / 0	-122.2	-122.2	0.62 (1)	4.09				
E-F	-1627 / 0	-122.2	-122.2	0.13 (1)	6.01				
F-G	0 / 54	-122.2	-122.2	0.12 (1)	10.00				
L-B	-1854 / 0	0.0	0.0	0.13 (1)	7.32				
H-F	-1838 / 0	0.0	0.0	0.13 (1)	7.34				
L-K	0 / 0	-28.0	-28.0	0.18 (1)	10.00				
K-R	0 / 1212	-28.0	-28.0	0.34 (1)	10.00				
R-S	0 / 1212	-28.0	-28.0	0.34 (1)	10.00				
S-J	0 / 1212	-28.0	-28.0	0.34 (1)	10.00				
J-T	0 / 1199	-28.0	-28.0	0.34 (1)	10.00				
T-U	0 / 1199	-28.0	-28.0	0.34 (1)	10.00				
U-V	0 / 1199	-28.0	-28.0	0.34 (1)	10.00				
V-I	0 / 1199	-28.0	-28.0	0.34 (1)	10.00				
I-H	0 / 0	-28.0	-28.0	0.19 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE
C	1-5-4	-105	-105	BACK	VERT	TOTAL
K	1-4-12	-70	-70	BACK	VERT	TOTAL
M	3-4-12	-102	-102	BACK	VERT	TOTAL
N	5-4-12	-102	-102	BACK	VERT	TOTAL
O	7-4-12	-102	-102	BACK	VERT	TOTAL
P	9-4-12	-102	-102	BACK	VERT	TOTAL
Q	11-4-12	-132	-132	BACK	VERT	TOTAL
R	3-4-12	-70	-70	BACK	VERT	TOTAL
S	5-4-12	-70	-70	BACK	VERT	TOTAL
T	7-4-12	-70	-70	BACK	VERT	TOTAL
U	9-4-12	-70	-70	BACK	VERT	TOTAL
V	11-4-12	-70	-70	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.62 (D-E-1), BC=0.34 (I-J-1), WB=0.46 (E-J-1), SSI=0.49 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

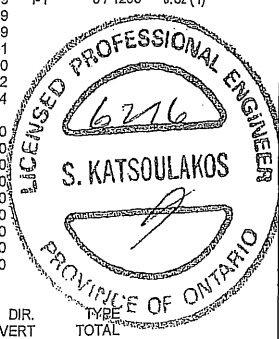
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1666

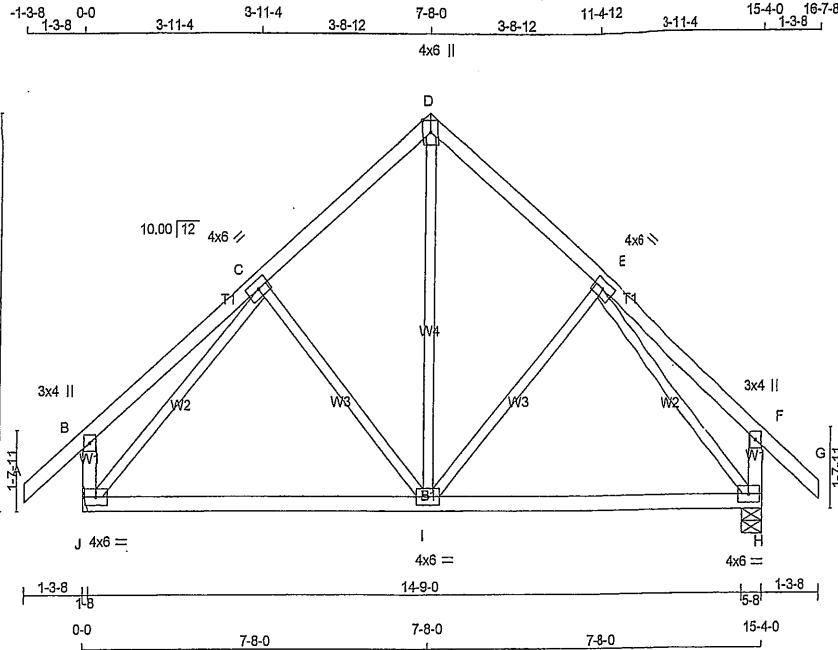
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM 2580816
STRUCTURAL
COMPONENT ONLY





Scale = 1:47.4

TOTAL WEIGHT = 2 X 71 = 143 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMVWV-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG
JT	VERT	HORZ	HORZ	IN-SX	IN-SX
J	1321	0	1321	0	0
H	1321	0	1321	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1015	692 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0
H	1015	692 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 54	I-D	0 / 634
B-C	0 / 35	I-E	-244 / 37
C-D	-814 / 0	C-I	-244 / 37
D-E	-814 / 0	J-C	-1171 / 0
E-F	0 / 35	E-H	-1171 / 0
F-G	0 / 54		
J-B	-347 / 0		
H-F	-347 / 0		
J-I	0 / 758		
I-H	0 / 758		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.29 (E-F:1), BC=0.55 (I-J:2), WB=0.68 (E-H:1), SSI=0.18 (I-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

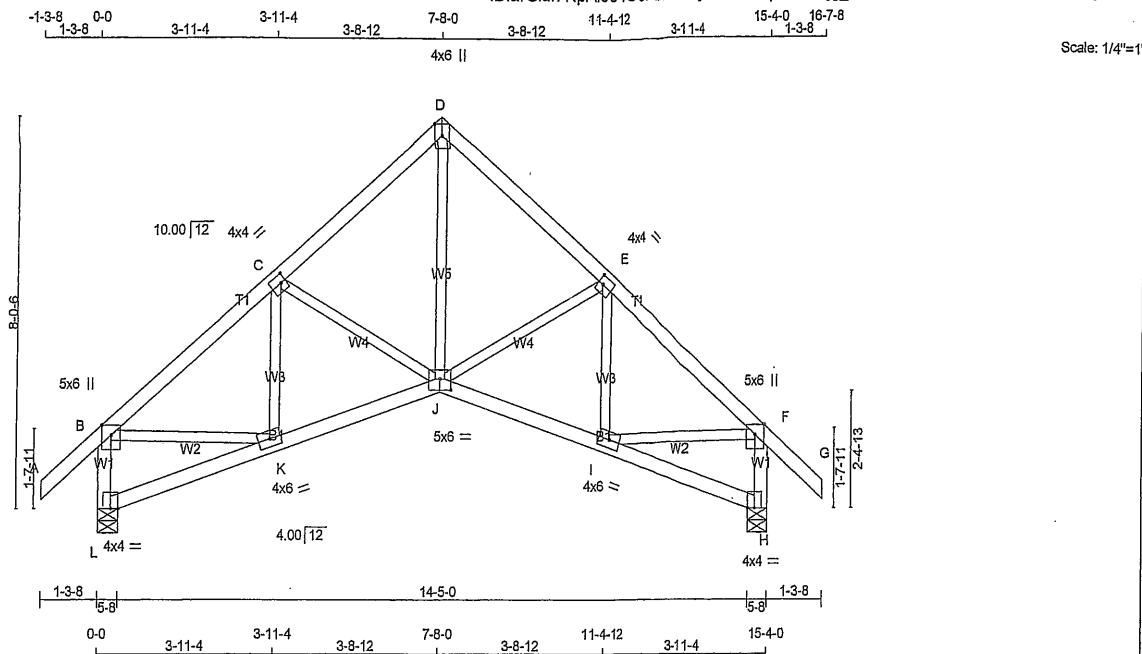


DWG NO. TAM 45840-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272340	T40S	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:arGldI7NpAl99YeGXNkM8lykz20-EYqBSiCvkjg_4dm60boTNIws2VGjjHPsaWP8Jyemke



TOTAL WEIGHT = 3 X 70 = 210 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25	
D	TTW+p	MT20	4.0	6.0	Edge		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25	
F	TMVW+p	MT20	5.0	6.0	Edge		
H	BVM1-p	MT20	4.0	4.0			
I	BMVW-t	MT20	4.0	6.0			
J	BBVW-t	MT20	5.0	6.0	2.75	3.00	
K	BMVW-t	MT20	4.0	6.0			
L	BVM1-p	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1321	0	1321	0	0	5-8	5-8
H	1321	0	1321	0	0	5-8	5-8
UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1015	692 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0
H	1015	692 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. (LC) UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. (LC) UNBRAC	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 54	-122.2	-122.2 0.17 (1)	10.00	J-D	0 / 1048	0.24 (1)
B-C	-1342 / 0	-122.2	-122.2 0.24 (1)	5.34	J-E	-256 / 0	0.08 (1)
C-D	-1123 / 0	-122.2	-122.2 0.23 (1)	5.72	I-E	-263 / 85	0.06 (1)
D-E	-1123 / 0	-122.2	-122.2 0.23 (1)	5.72	C-J	-256 / 0	0.08 (1)
E-F	-1342 / 0	-122.2	-122.2 0.24 (1)	5.34	K-C	-263 / 85	0.06 (1)
F-G	0 / 54	-122.2	-122.2 0.17 (1)	10.00	B-K	0 / 1058	0.24 (1)
L-B	-1275 / 0	0.0	0.0 0.14 (1)	7.13	I-F	0 / 1058	0.24 (1)
H-F	-1275 / 0	0.0	0.0 0.14 (1)	7.13			
L-K	0 / 7	-28.0	-28.0 0.11 (3)	10.00			
K-J	0 / 1111	-28.0	-28.0 0.23 (1)	10.00			
J-I	0 / 1111	-28.0	-28.0 0.23 (1)	10.00			
I-H	0 / 7	-28.0	-28.0 0.11 (3)	10.00			

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 38.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.24 (B-C:1), BC=0.23 (J-K:1), WB=0.24 (B-K:1), SSI=0.18 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

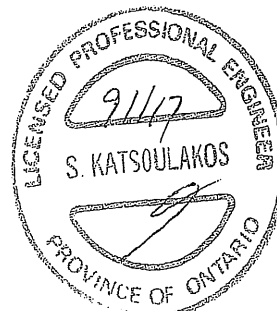
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PLI)	(PLI)			
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

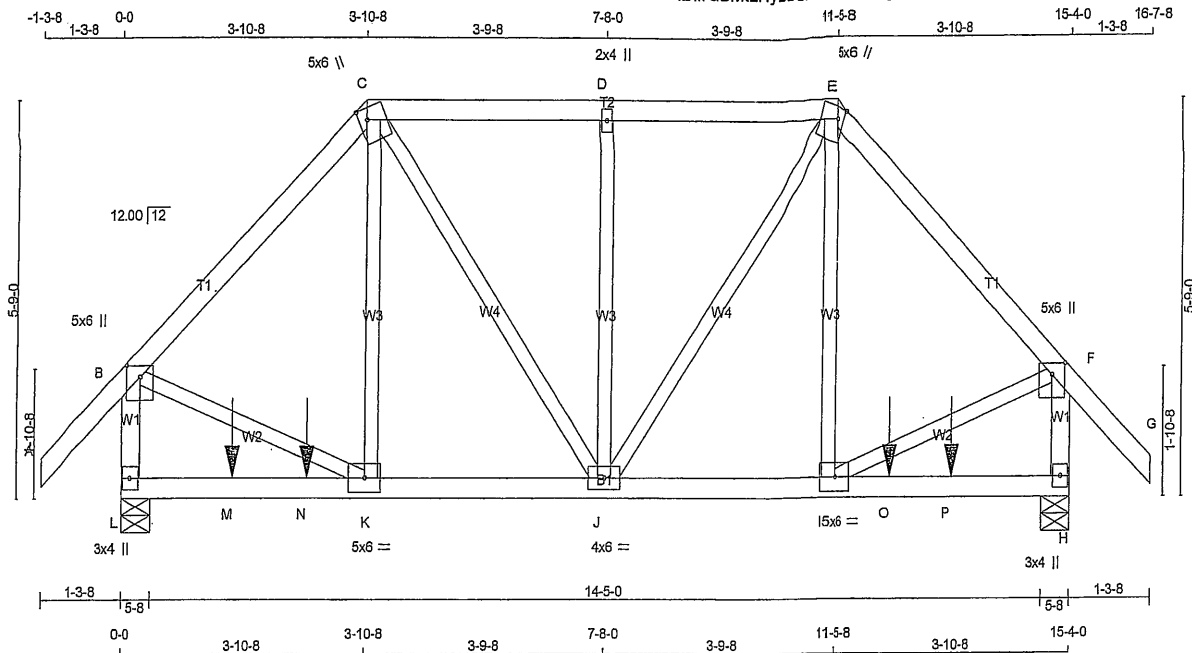
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.42 (L) (INPUT = 1.00)



DWG NO. TAM 45841-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



Scale = 1:34.0

TOTAL WEIGHT = 76 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	2.00	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMW+p	MT20	5.0	6.0	2.00	2.50
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 42.6 lbs FACTORED DOWN AT 1-9-4, 966.3 lbs FACTORED DOWN AT 2-11-8, AND 966.3 lbs FACTORED DOWN AT 12-4-8, AND 42.6 lbs FACTORED DOWN AT 13-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT		REQD	
	GROSS REACTION		GROSS REACTION		BRG		BRG	
L	2313	0	2313	0	5-8		5-8	
H	2312	0	2312	0	5-8		5-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
L	1785	1203 / 0	292 / 0	0 / 0	0 / 0	290 / 0	0 / 0
H	1784	1203 / 0	291 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 60	-122.2	-122.2 0.19 (1)	10.00	K-C	0 / 629	0.16 (1)
B-C	-1727 / 0	-122.2	-122.2 0.43 (1)	4.55	C-J	0 / 210	0.05 (1)
C-D	-1349 / 0	-122.2	-122.2 0.34 (1)	5.11	J-D	-550 / 0	0.29 (1)
D-E	-1349 / 0	-122.2	-122.2 0.34 (1)	5.11	J-E	0 / 209	0.05 (1)
E-F	-1728 / 0	-122.2	-122.2 0.43 (1)	4.55	I-E	0 / 630	0.16 (1)
F-G	0 / 60	-122.2	-122.2 0.19 (1)	10.00	B-K	0 / 1303	0.32 (1)
L-B	-2099 / 0	0.0	0.0 0.25 (1)	5.80	I-F	0 / 1303	0.32 (1)
H-F	-2100 / 0	0.0	0.0 0.25 (1)	5.80			
L-M	0 / 0	-28.0	-28.0 0.72 (1)	10.00			
M-N	0 / 0	-28.0	-28.0 0.72 (1)	10.00			
N-K	0 / 0	-28.0	-28.0 0.72 (1)	10.00			
K-J	0 / 1229	-28.0	-28.0 0.83 (1)	10.00			
J-I	0 / 1229	-28.0	-28.0 0.83 (1)	10.00			
I-O	0 / 0	-28.0	-28.0 0.72 (1)	10.00			
O-P	0 / 0	-28.0	-28.0 0.72 (1)	10.00			
P-H	0 / 0	-28.0	-28.0 0.72 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
M	1-9-4	-24	-43	---	FRONT	VERT	TOTAL
N	2-11-8	-966	-966	---	FRONT	VERT	TOTAL
O	12-4-8	-966	-966	---	FRONT	VERT	TOTAL
P	13-4-12	-24	-43	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.15")

CSI: TC=0.43 (E-F:1), BC=0.83 (J-I:1), WB=0.32 (F-I:1), SSI=0.73 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

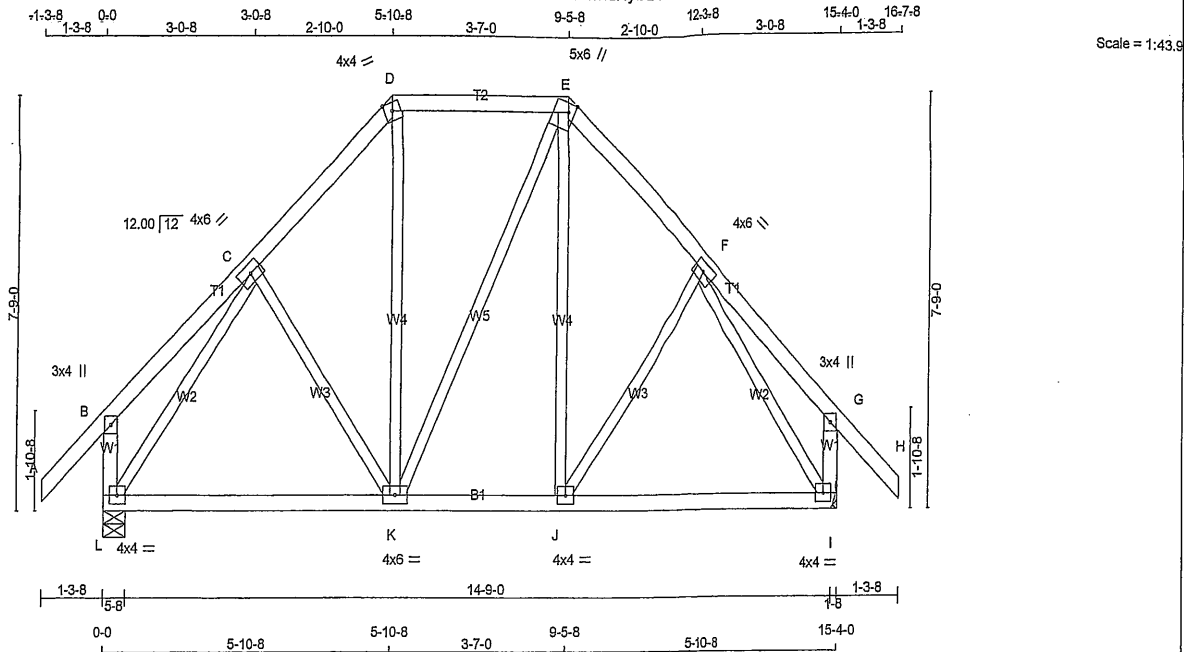
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)

JSI METAL= 0.36 (B) (INPUT = 1.00)



DRW NO. TAM 45842-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 83 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
L - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
L - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
L	1322	0	1322	0	5-8	5-8
I	1322	0	1322	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
L	1016	693 / 0	161 / 0		0 / 0	0 / 0	162 / 0	0 / 0
I	1016	693 / 0	161 / 0		0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 80	-122.2 -122.2	0.17 (1)	10.00	C-K	-85 / 56	0.04 (1)
B-C	0 / 29	-122.2 -122.2	0.16 (1)	10.00	K-D	0 / 238	0.05 (2)
C-D	-838 / 0	-122.2 -122.2	0.13 (1)	6.25	K-E	0 / 1	0.00 (2)
D-E	-573 / 0	-122.2 -122.2	0.20 (1)	6.25	J-E	0 / 236	0.05 (2)
E-F	-837 / 0	-122.2 -122.2	0.13 (1)	6.25	J-F	-85 / 55	0.04 (1)
F-G	0 / 29	-122.2 -122.2	0.16 (1)	10.00	L-C	-1129 / 0	0.51 (1)
G-H	0 / 60	-122.2 -122.2	0.17 (1)	10.00	F-I	-1128 / 0	0.51 (1)
L-B	-307 / 0	0.0 0.0	0.03 (1)	7.81			
I-G	-307 / 0	0.0 0.0	0.03 (1)	7.81			
L-K	0 / 618	-28.0 -28.0	0.27 (2)	10.00			
K-J	0 / 572	-28.0 -28.0	0.25 (2)	10.00			
J-I	0 / 618	-28.0 -28.0	0.27 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 38.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
 ALLOWABLE DEFL.(TL)= L/360 (0.51")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.20 (D-E:1), BC=0.27 (I-J:2), WB=0.51 (C-L:1), SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
 JSI METAL= 0.28 (C) (INPUT = 1.00)

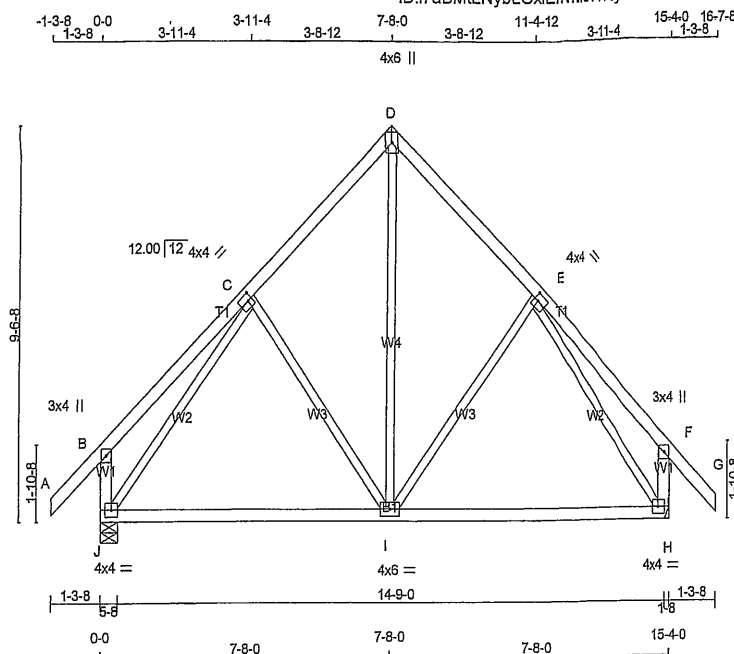


DWG NO. TAM 45843-17
 STRUCTURAL
 COMPONENT ONLY

Tamarack Roof Truss, Burlington

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ID:17aBMtLNyblOXiEnM9IYRkwsI-xH0FFZxAMVOOcSnUBKB6REnLieGgV91OuZo7nHyemjh



Scale = 1:56.7

TOTAL WEIGHT = 2 X 78 = 156 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
J		1322	0	1322	0	0	5-8	5-8	
H		1322	0	1322	0	0			

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1016	693 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0
H	1016	693 / 0	161 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	FORCE (LBS)	MAX. LC1 (LC)	MAX. LC1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I-D	0 / 638
B-C	0 / 39	-122.2	-122.2	0.29 (1)	10.00	I-E	-230 / 34
C-D	-748 / 0	-122.2	-122.2	0.23 (1)	6.25	C-I	-230 / 34
D-E	-748 / 0	-122.2	-122.2	0.23 (1)	6.25	J-C	-1098 / 0
E-F	0 / 39	-122.2	-122.2	0.29 (1)	10.00	E-H	-1098 / 0
F-G	0 / 60	-122.2	-122.2	0.17 (1)	10.00		
J-B	-347 / 0	0.0	0.0	0.04 (1)	7.81		
H-F	-347 / 0	0.0	0.0	0.04 (1)	7.81		
J-I	0 / 637	-28.0	-28.0	0.54 (2)	10.00		
I-H	0 / 637	-28.0	-28.0	0.54 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.51")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.10")
ALLOWABLE DEFL.(TL) = 1/360 (0.51")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.17")

CSI: TC=0.29 (E-F:1), BC=0.54 (I-J:2), WB=0.84 (C-J:1), SSI=0.18 (H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2264 1656

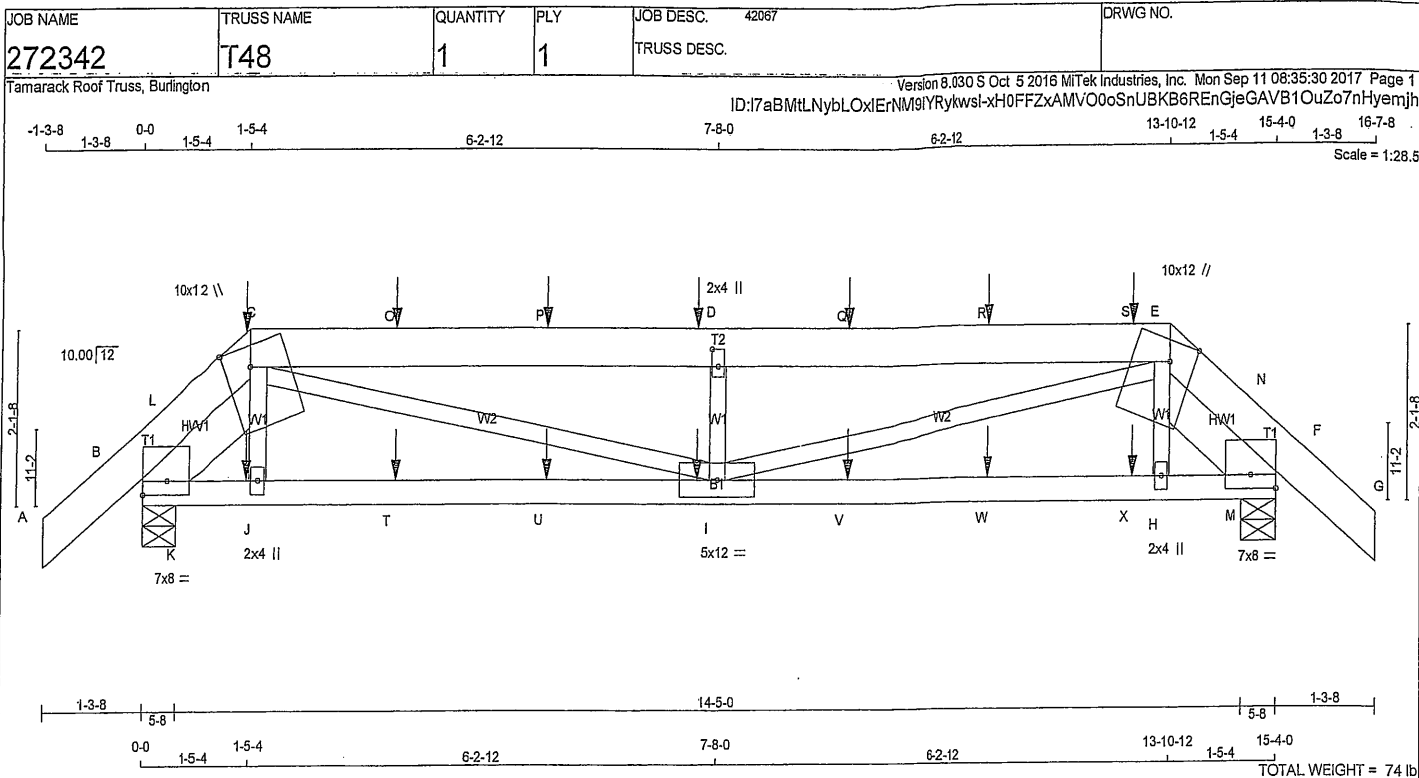
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.42 (E) (INPUT = 1.00)



DWG NO. TAM 4584/17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF
C - E	2x6 DRY	No.2	SPF
E - G	2x6 DRY	No.2	SPF
B - F	2x4 DRY	No.2	SPF

REINFORCING MEMBERS

HW1	2x6 DRY	No.2	SPF
HW2	2x6 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	7.0	8.0	2.00	Edge
C	TTWWW+m	MT20	10.0	12.0	3.00	4.00
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TTWWW+m	MT20	10.0	12.0	3.00	4.00
F	TMBMW1-I	MT20	7.0	8.0	2.00	Edge
H	BMW+w	MT20	2.0	4.0		
I	BMW+w	MT20	5.0	12.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 110.0 lbs FACTORED DOWN AT 1-6-8, 106.8 lbs FACTORED DOWN AT 3-4-12, 106.8 lbs FACTORED DOWN AT 5-4-12, 106.8 lbs FACTORED DOWN AT 7-4-12, 106.8 lbs FACTORED DOWN AT 9-4-12, AND 106.8 lbs FACTORED DOWN AT 11-4-12, AND 143.7 lbs FACTORED DOWN AT 13-4-12 ON TOP CHORD, AND 65.4 lbs FACTORED DOWN AT 1-4-12, 65.4 lbs FACTORED DOWN AT 3-4-12, 65.4 lbs FACTORED DOWN AT 5-4-12, 65.4 lbs FACTORED DOWN AT 7-4-12, 65.4 lbs FACTORED DOWN AT 9-4-12, AND 65.4 lbs FACTORED DOWN AT 11-4-12, AND 65.4 lbs FACTORED DOWN AT 13-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
B	1954	0	1954	0
F	1946	0	1946	0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	1509	1014 / 0	248 / 0	0 / 0	0 / 0	247 / 0	0 / 0
F	1499	1015 / 0	243 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 39	-122.2 -122.2	0.10 (1)	10.00	J-C	0 / 476	0.12 (2)
B-L	-2343 / 0	-122.2 -122.2	0.43 (1)	4.77	H-E	0 / 479	0.12 (2)
L-C	-2275 / 0	-122.2 -122.2	0.18 (1)	6.25	I-E	0 / 2863	0.71 (1)
C-O	-4255 / 0	-122.2 -122.2	0.57 (1)	3.56	C-I	0 / 2863	0.71 (1)
O-P	-4255 / 0	-122.2 -122.2	0.57 (1)	3.56	I-D	-1235 / 0	0.20 (1)
P-D	-4255 / 0	-122.2 -122.2	0.57 (1)	3.56	K-L	-116 / 0	0.00 (1)
D-Q	-4255 / 0	-122.2 -122.2	0.61 (1)	3.50	M-N	-130 / 0	0.00 (1)
Q-R	-4255 / 0	-122.2 -122.2	0.61 (1)	3.50			
R-S	-4255 / 0	-122.2 -122.2	0.61 (1)	3.50			
S-E	-4255 / 0	-122.2 -122.2	0.61 (1)	3.50			
E-N	-2280 / 0	-122.2 -122.2	0.18 (1)	6.25			
N-F	-2333 / 0	-122.2 -122.2	0.44 (1)	4.77			
F-G	0 / 39	-122.2 -122.2	0.10 (1)	10.00			
B-K	0 / 1502	-28.0 -28.0	0.34 (1)	10.00			
K-J	0 / 1502	-28.0 -28.0	0.55 (1)	10.00			
J-T	0 / 1502	-28.0 -28.0	0.55 (1)	10.00			
T-U	0 / 1502	-28.0 -28.0	0.55 (1)	10.00			
U-I	0 / 1502	-28.0 -28.0	0.55 (1)	10.00			
I-V	0 / 1502	-28.0 -28.0	0.57 (1)	10.00			
V-W	0 / 1502	-28.0 -28.0	0.57 (1)	10.00			
W-X	0 / 1502	-28.0 -28.0	0.57 (1)	10.00			
X-H	0 / 1502	-28.0 -28.0	0.57 (1)	10.00			
H-M	0 / 1502	-28.0 -28.0	0.57 (1)	10.00			
M-F	0 / 1502	-28.0 -28.0	0.34 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-6-8	-110	-110		BACK	VERT	TOTAL
D	7-4-12	-107	-107		BACK	VERT	TOTAL
I	7-4-12	-65	-65		BACK	VERT	TOTAL
J	1-4-12	-65	-65		BACK	VERT	TOTAL
O	3-4-12	-107	-107		BACK	VERT	TOTAL
P	5-4-12	-107	-107		BACK	VERT	TOTAL
Q	9-4-12	-107	-107		BACK	VERT	TOTAL
R	11-4-12	-107	-107		BACK	VERT	TOTAL
S	13-4-12	-144	-144		BACK	VERT	TOTAL
T	3-4-12	-65	-65		BACK	VERT	TOTAL
U	5-4-12	-65	-65		BACK	VERT	TOTAL
V	9-4-12	-65	-65		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.51")
CALCULATED VERT. DEFL. (LL) = L/999 (0.18")
ALLOWABLE DEFL. (TL) = L/360 (0.51")
CALCULATED VERT. DEFL. (TL) = L/671 (0.27")

CSI: TC=0.61 (D-E:1), BC=0.57 (H-I:1), WB=0.71 (C-I:1), SSI=0.38 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

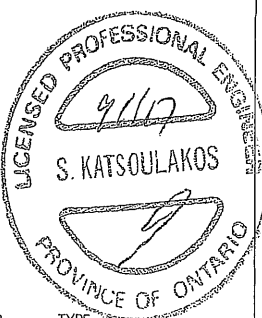
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.48 (I) (INPUT = 1.00)



DWG NO. TAM 45845-17
STRUCTURAL
COMPONENT ONLY

file

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272342	T48	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 08:35:30 2017 Page 2
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FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
W	11-4-12	-65	-65	—	BACK	VERT
X	13-4-12	-65	-65	—	BACK	VERT
						TOTAL



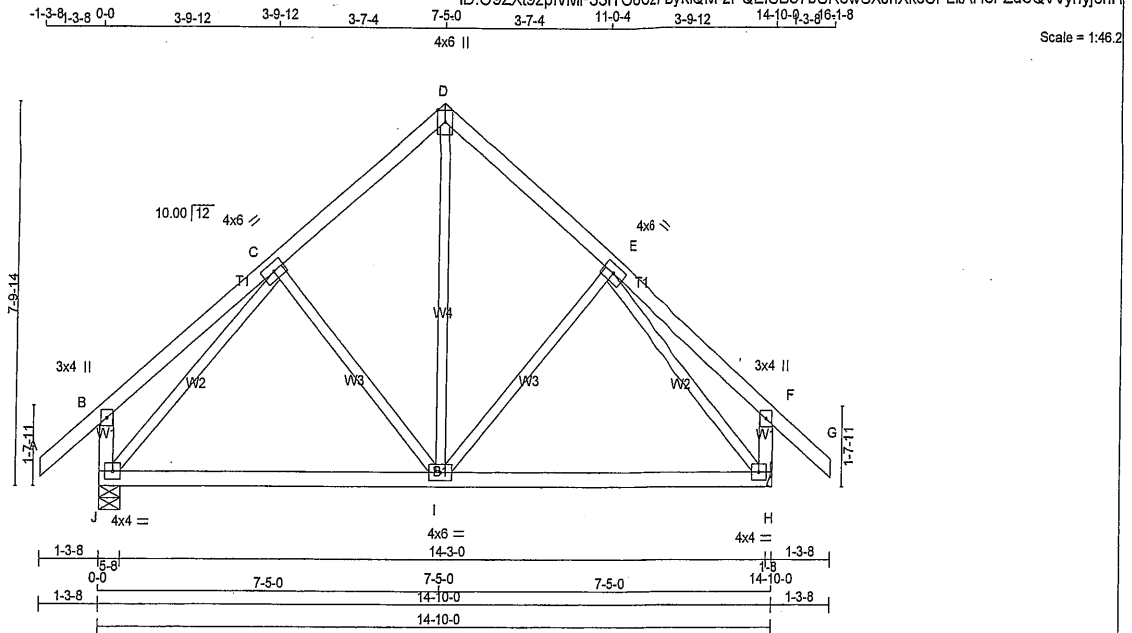
DWNG. TAM 45845-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 272357	TRUSS NAME T101	QUANTITY 2	PLY 1	JOB DESC. 42067 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MiTek Industries, Inc. Mon Aug 29 04:13:32 2016 Page 1

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TOTAL WEIGHT = 2 X 69 = 139 lb

[M/F]

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	4.0	
I	BMVWW-t	MT20	4.0	6.0	
J	BMVW1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED				MAXIMUM FACTORED		INPUT	
GROSS REACTION				GROSS REACTION		BRG	
JT	VERT	HORZ		DOWN	HORZ	IN-SX	IN-SX
J	1283	0		1283	0	5-8	1-8
H	1283	0		1283	0	0	
HANGER BY OTHERS							
MIN. SEAT SIZE: 1-8							

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	985	673 / 0	156 / 0	0 / 0	0 / 0	157 / 0	0 / 0
H	985	673 / 0	156 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)		FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 54	-122.2 -122.2	0.17 (1)	10.00	I-D	0 / 608	0.14 (1)
B-C	0 / 34	-122.2 -122.2	0.27 (1)	10.00	I-E	-231 / 38	0.13 (1)
C-D	-783 / 0	-122.2 -122.2	0.21 (1)	6.25	C-I	-231 / 38	0.13 (1)
D-E	-783 / 0	-122.2 -122.2	0.21 (1)	6.25	J-C	-1128 / 0	0.61 (1)
E-F	0 / 34	-122.2 -122.2	0.27 (1)	10.00	E-H	-1128 / 0	0.61 (1)
F-G	0 / 54	-122.2 -122.2	0.17 (1)	10.00			
J-B	-341 / 0	0.0	0.0	0.04 (1)	7.81		
H-F	-341 / 0	0.0	0.0	0.04 (1)	7.81		
J-I	0 / 726	-28.0	-28.0	0.52 (2)	10.00		
I-H	0 / 726	-28.0	-28.0	0.52 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.27 (B-C:1), BC=0.52 (I-J:2), WB=0.61 (C-J:1), SSI=0.18 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

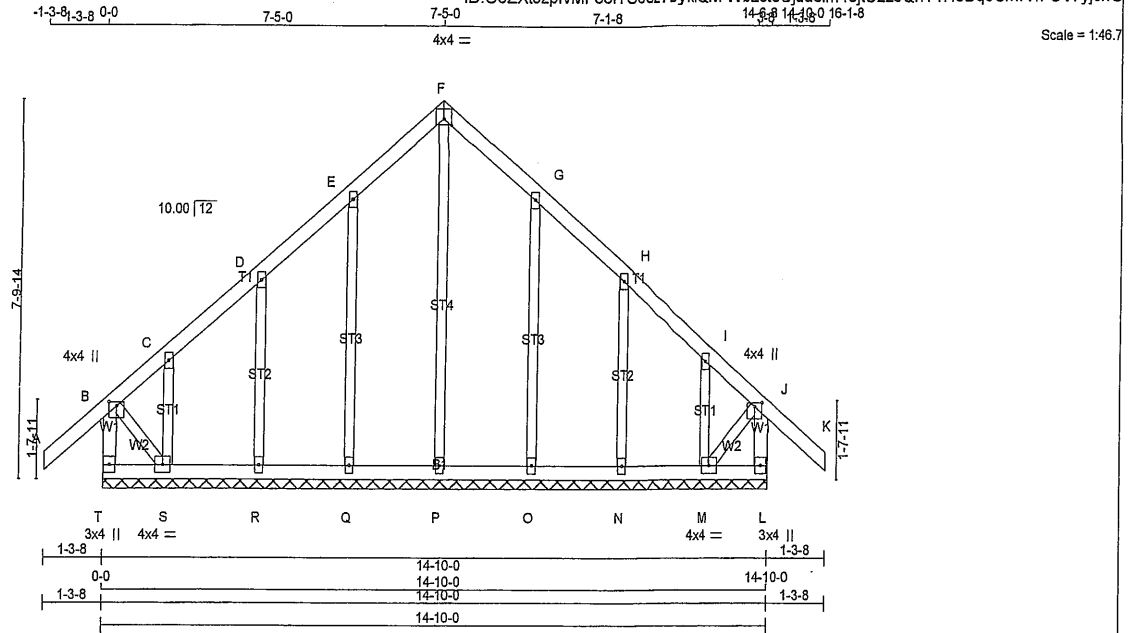
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)



DWG NO. TAM39265 -16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 74 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
T - B 2x4 DRY No.2 SPF
A - F 2x4 DRY No.2 SPF
F - K 2x4 DRY No.2 SPF
L - J 2x4 DRY No.2 SPF
T - L 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I					
C TMVW+w	MT20	2.0	4.0		
F TTW+p	MT20	4.0	4.0	1.50	2.00
J TMVW+p	MT20	4.0	4.0	1.00	2.00
L BMV1+p	MT20	3.0	4.0		
M BMVW1-t	MT20	4.0	4.0		
N, O, P, Q, R					
N BMV1+w	MT20	2.0	4.0		
S BMVW1-t	MT20	4.0	4.0		
T BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (LC1)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM	TO		FR-TO		
T-B	-379 / 0	0.0	0.0	0.04 (1)	7.81	P-F	-183 / 0
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	Q-E	-274 / 0
B-C	-87 / 0	-122.2	-122.2	0.16 (1)	6.25	R-D	-251 / 0
C-D	-22 / 0	-122.2	-122.2	0.06 (1)	6.25	S-C	-107 / 0
D-E	-22 / 0	-122.2	-122.2	0.07 (1)	6.25	O-G	-274 / 0
E-F	-34 / 0	-122.2	-122.2	0.07 (1)	6.25	N-H	-251 / 0
F-G	-34 / 0	-122.2	-122.2	0.07 (1)	6.25	M-I	-107 / 0
G-H	-22 / 0	-122.2	-122.2	0.07 (1)	6.25	B-S	0 / 39
H-I	-22 / 0	-122.2	-122.2	0.06 (1)	6.25	M-J	0 / 39
I-J	-87 / 0	-122.2	-122.2	0.16 (1)	6.25		
J-K	0 / 54	-122.2	-122.2	0.17 (1)	10.00		
L-J	-379 / 0	0.0	0.0	0.04 (1)	7.81		
T-S	0 / 0	-28.0	-28.0	0.02 (2)	10.00		
S-R	0 / 25	-28.0	-28.0	0.02 (2)	10.00		
R-Q	0 / 19	-28.0	-28.0	0.02 (2)	10.00		
Q-P	0 / 14	-28.0	-28.0	0.02 (2)	10.00		
P-O	0 / 14	-28.0	-28.0	0.02 (2)	10.00		
O-N	0 / 19	-28.0	-28.0	0.02 (2)	10.00		
N-M	0 / 25	-28.0	-28.0	0.02 (2)	10.00		
M-L	0 / 0	-28.0	-28.0	0.02 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1), BC=0.02 (M-N:2), WB=0.20 (F-P:1), SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

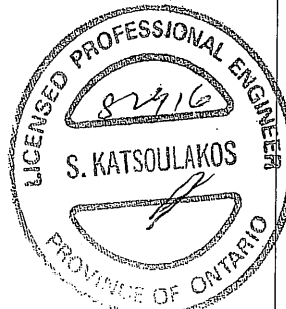
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

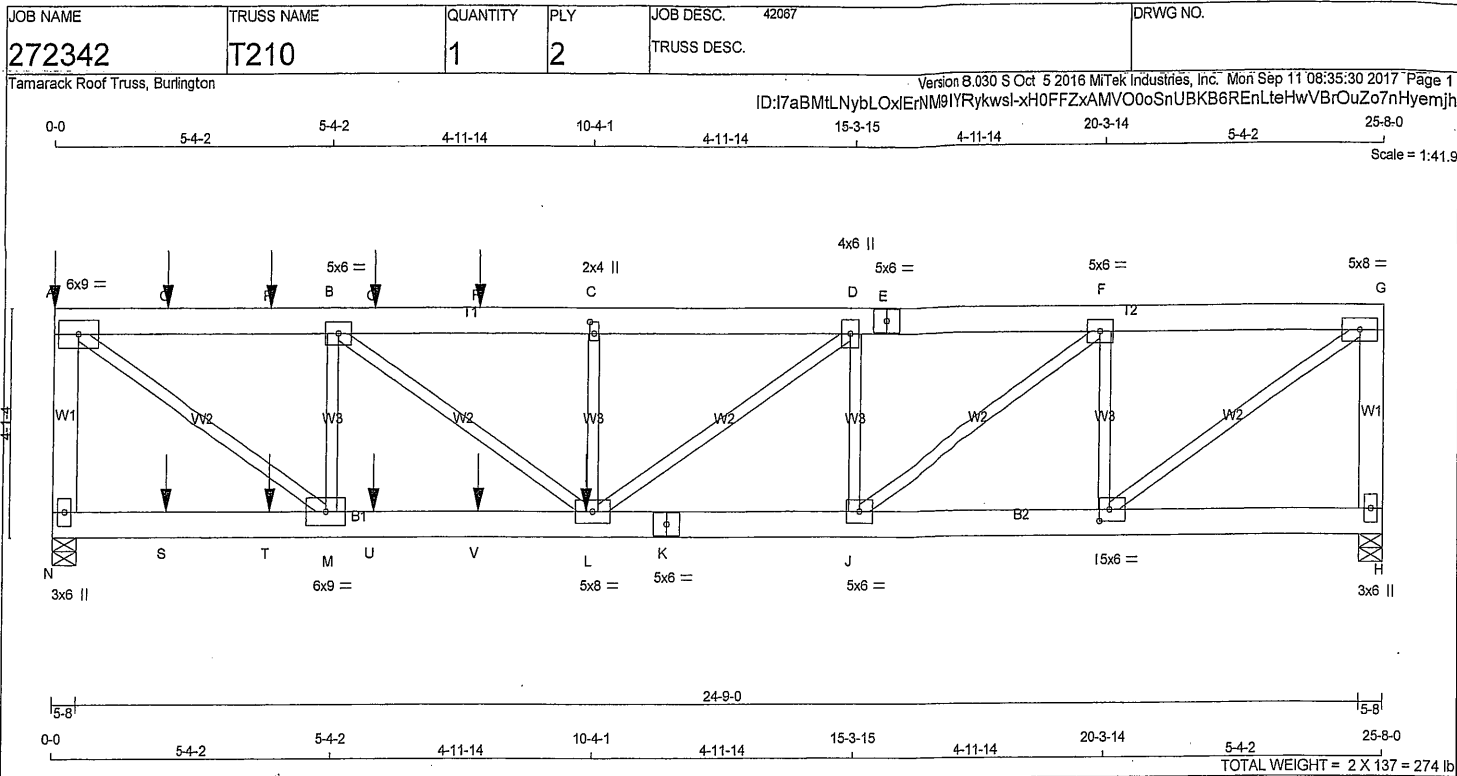
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (F) (INPUT = 0.90)
JSI METAL= 0.07 (G) (INPUT = 1.00)



DWG NO. TAM 39266-10
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.

N - A	2x6	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

TOP CHORDS: (0.122"X3") SPIRAL NAILS		
N - A	2	12
A - E	2	12
E - G	2	12
G - H	2	12
BOTTOM CHORDS: (0.122"X3") SPIRAL NAILS		
N - K	2	12
K - H	2	12
WEBS: (0.122"X3") SPIRAL NAILS		
L - C	1	5
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-t	MT20	6.0	9.0		
B	TMWV-t	MT20	5.0	6.0		
C	TMWV-t	MT20	2.0	4.0	2.50	1.00
D	TMWV-t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWV-t	MT20	5.0	6.0		
G	TMWV-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWV-t	MT20	5.0	6.0	2.50	2.25
J	BMWV-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWV-t	MT20	5.0	8.0		
M	BMWV-t	MT20	6.0	9.0		
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
N	3935	0	3935	0
H	2893	0	2893	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3034	2050 / 0	494 / 0	0 / 0	0 / 0	490 / 0	0 / 0
H	2244	1493 / 0	380 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MAX. MEMB. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. MEMB. UNBRACED LENGTH
FR-TO		FROM	TO		FR-TO		
N-A	-3840 / 0	0.0	0.0	0.28 (1)	7.22	I-G	0 / 4515
A-O	-4811 / 0	-122.2	-122.2	0.23 (1)	5.07	A-M	0 / 5824
O-P	-4811 / 0	-122.2	-122.2	0.23 (1)	5.07	I-F	-2395 / 0
P-B	-4811 / 0	-122.2	-122.2	0.23 (1)	5.07	M-B	-2979 / 0
B-Q	-7306 / 0	-122.2	-122.2	0.27 (1)	4.24	J-F	0 / 2869
Q-R	-7306 / 0	-122.2	-122.2	0.27 (1)	4.24	B-L	0 / 3089
R-C	-7306 / 0	-122.2	-122.2	0.27 (1)	4.24	J-D	-1523 / 0
C-D	-7306 / 0	-122.2	-122.2	0.19 (1)	4.33	L-C	-626 / 0
D-E	-6047 / 0	-122.2	-122.2	0.17 (1)	4.70	L-D	0 / 1559
E-F	-6047 / 0	-122.2	-122.2	0.17 (1)	4.70		
F-G	-3730 / 0	-122.2	-122.2	0.14 (1)	5.70		
H-G	-2820 / 0	0.0	0.0	0.21 (1)	7.81		

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MAX. MEMB. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. MEMB. UNBRACED LENGTH
FR-TO		FROM	TO		FR-TO		
N-S	0 / 0	-28.0	-28.0	0.08 (3)	10.00		
S-T	0 / 0	-28.0	-28.0	0.08 (3)	10.00		
T-M	0 / 0	-28.0	-28.0	0.08 (3)	10.00		
M-U	0 / 4811	-28.0	-28.0	0.39 (1)	10.00		
U-V	0 / 4811	-28.0	-28.0	0.39 (1)	10.00		
V-L	0 / 4811	-28.0	-28.0	0.39 (1)	10.00		
L-K	0 / 6047	-28.0	-28.0	0.46 (1)	10.00		
K-J	0 / 6047	-28.0	-28.0	0.46 (1)	10.00		
J-I	0 / 3730	-28.0	-28.0	0.27 (1)	10.00		
I-H	0 / 0	-28.0	-28.0	0.05 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TOTAL
A	0-0	-199	-199	---	BACK	VERT
L	10-2-8	-2025	-2025	---	BACK	VERT
O	2-1-12	-147	-147	---	BACK	VERT
P	4-1-12	-147	-147	---	BACK	VERT
Q	6-1-12	-147	-147	---	BACK	VERT
R	8-1-12	-147	-147	---	BACK	VERT
S	2-1-12	-40	-40	---	BACK	VERT
T	4-1-12	-40	-40	---	BACK	VERT
U	6-1-12	-40	-40	---	BACK	VERT
V	8-1-12	-40	-40	---	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL)= L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL)= L/999 (0.24")

CSI: TC=0.28 (A-N:1), BC=0.46 (J-L:1), WB=0.72 (A-M:1), SSI=0.19 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.52 (K) (INPUT = 1.00)

DRG NO. TAM 45846-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272342	T210	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

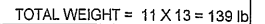
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 199.3 lbs FACTORED DOWN AT 0-0, 147.1 lbs FACTORED DOWN AT 2-1-12, 147.1 lbs FACTORED DOWN AT 4-1-12, AND 147.1 lbs FACTORED DOWN AT 6-1-12, AND 147.1 lbs FACTORED DOWN AT 8-1-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-12, 69.9 lbs FACTORED DOWN AT 4-1-12, 69.9 lbs FACTORED DOWN AT 6-1-12, AND 69.9 lbs FACTORED DOWN AT 8-1-12, AND 2024.9 lbs FACTORED DOWN AT 10-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

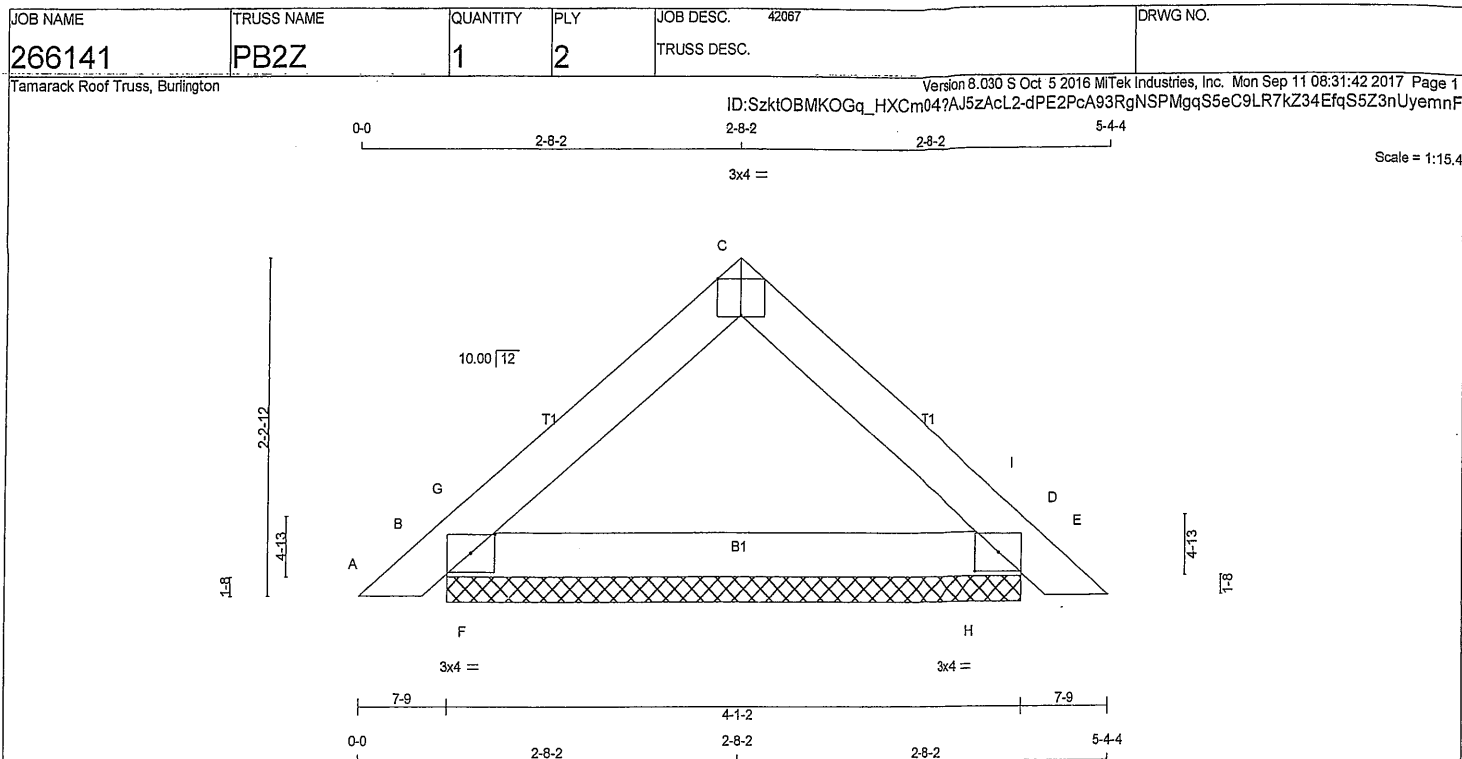


DWG NO. TAM45846-17
 STRUCTURAL
 COMPONENT ONLY

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DWG NO. TAM 45834/17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A- C	12	TOP
C- E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B- D	12	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
	VERT	DOWN	BRG	BRG
B	373	0	4-1-2	4-1-2
D	373	0	4-1-2	4-1-2

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	285	198 / 0	43 / 0	0 / 0	0 / 0	44 / 0	0 / 0
D	285	198 / 0	43 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 19	-122.2	-122.2	0.01 (1)	10.00	F-G	0 / 211
B-G	-307 / 0	-122.2	-122.2	0.03 (2)	6.25	H-I	0 / 211
G-C	-186 / 0	-122.2	-122.2	0.04 (1)	6.25		
C-I	-186 / 0	-122.2	-122.2	0.04 (1)	6.25		
I-D	-307 / 0	-122.2	-122.2	0.03 (2)	6.25		
D-E	0 / 19	-122.2	-122.2	0.01 (1)	10.00		
B-F	0 / 150	-28.0	-28.0	0.02 (3)	10.00		
F-H	0 / 150	-28.0	-28.0	0.05 (2)	10.00		
H-D	0 / 150	-28.0	-28.0	0.02 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (C-I:1), BC=0.05 (F-H:2), WB=0.00 (F-G:1), SSI=0.05 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

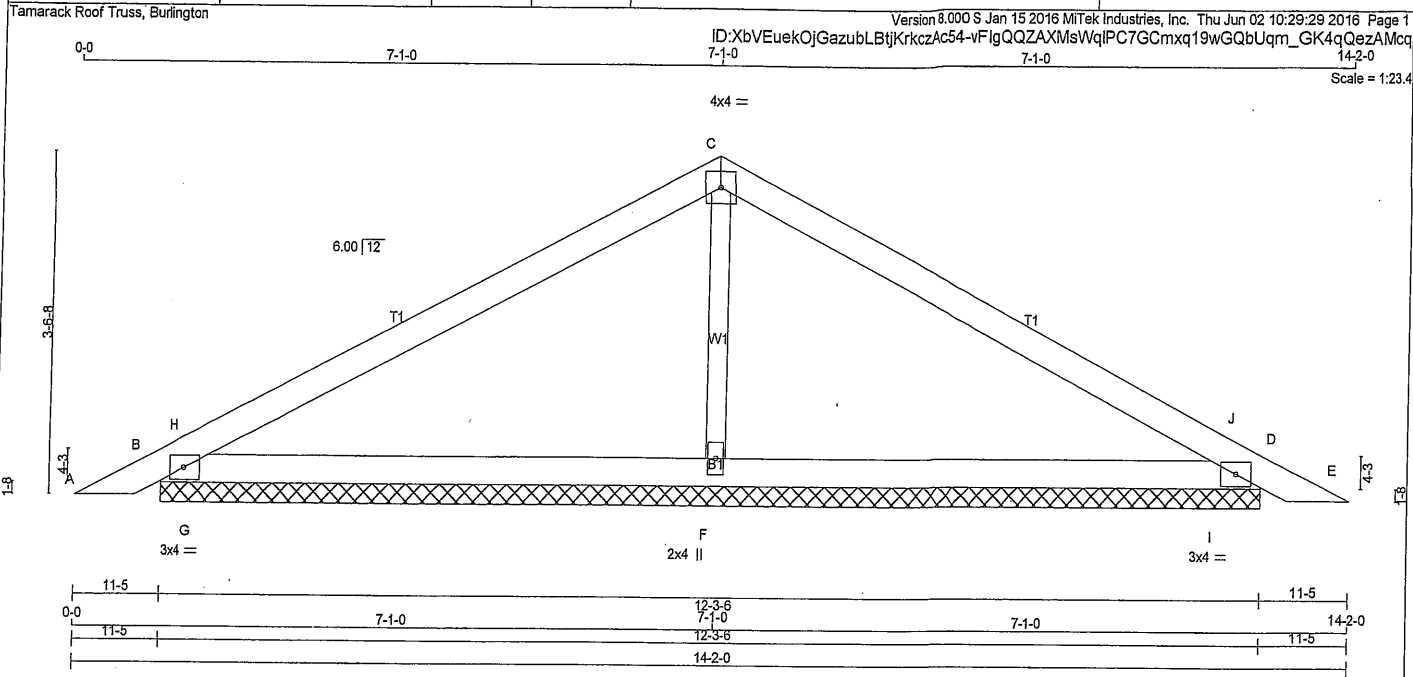
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (D) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DWG NO. TAM 45835-17
STRUCTURAL
COMPONENT ONLY



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:	
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL = 38.3 PSF	
C - E	2x4	DRY	No.2	SPF	JT VERT	DOWN	HORZ	UPLIFT	IN-SX	DL = 3.0 PSF	
B - D	2x4	DRY	No.2	SPF	B 654	0	654	0	0	BOT CH. LL = 10.5 PSF	
					D 654	0	654	0	0	DL = 7.0 PSF	
					F 751	0	751	0	0	TOTAL LOAD = 58.7 PSF	
ALL WEBS	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS			SPACING = 24.0 IN. C/C	
DRY: SEASONED LUMBER.					JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
					B 488	358 / 0	61 / 0	0 / 0	0 / 0	69 / 0	0 / 0
					D 488	358 / 0	61 / 0	0 / 0	0 / 0	69 / 0	0 / 0
					F 611	357 / 0	136 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 22	-122.2 -122.2	0.07 (1)	10.00	F-C	-365 / 0	0.07 (1)
B-H	-136 / 0	-122.2 -122.2	0.27 (1)	6.25	G-H	-864 / 118	0.00 (1)
H-C	-315 / 0	-122.2 -122.2	0.57 (1)	6.25	I-J	-864 / 118	0.00 (1)
C-J	-315 / 0	-122.2 -122.2	0.57 (1)	6.25			
J-D	-136 / 0	-122.2 -122.2	0.27 (1)	6.25			
D-E	0 / 22	-122.2 -122.2	0.07 (1)	10.00			
B-G	0 / 268	-28.0 -28.0	0.51 (1)	10.00			
G-F	0 / 268	-28.0 -28.0	0.51 (1)	10.00			
F-I	0 / 268	-28.0 -28.0	0.51 (1)	10.00			
I-D	0 / 268	-28.0 -28.0	0.51 (1)	10.00			

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (B) (INPUT = 0.90)

JSI METAL= 0.15 (D) (INPUT = 1.00)



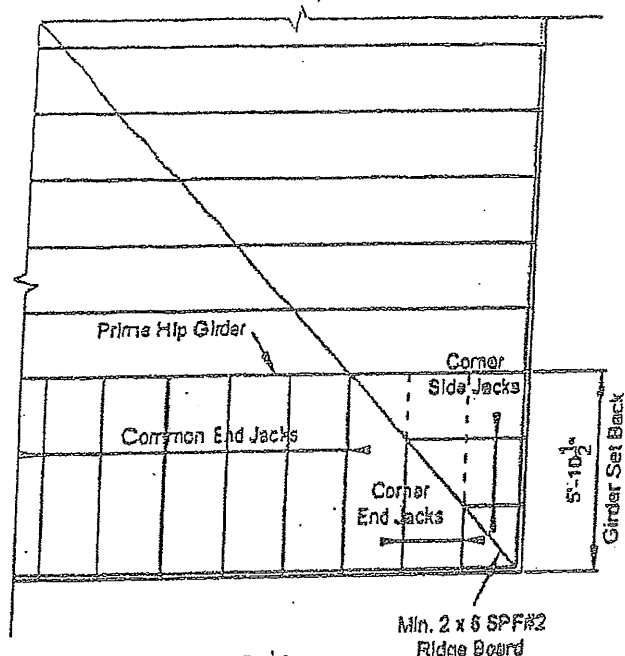
DRWG NO. TAM 25 809-16

STRUCTURAL

COMPONENT ONLY

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

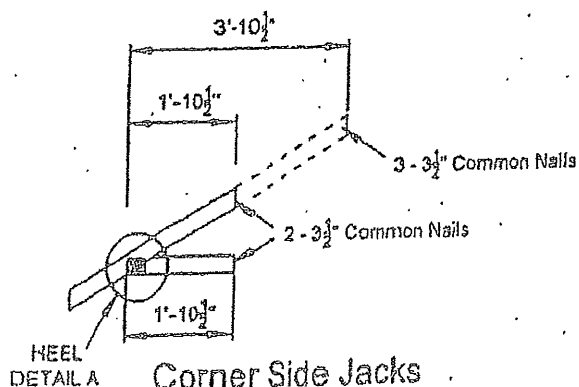
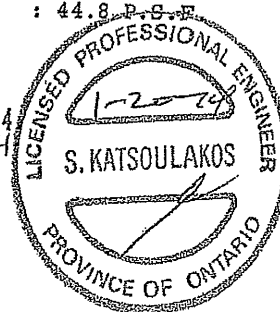
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

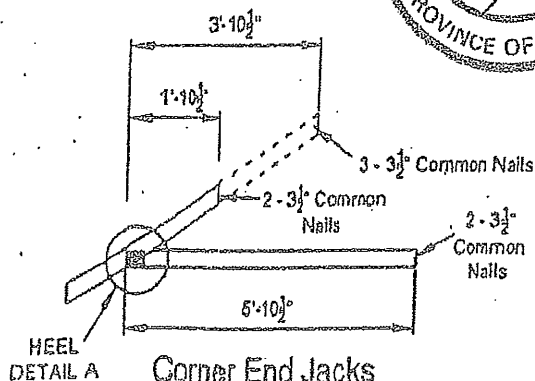
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

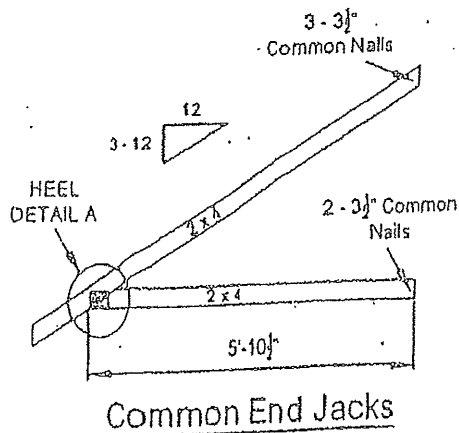
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



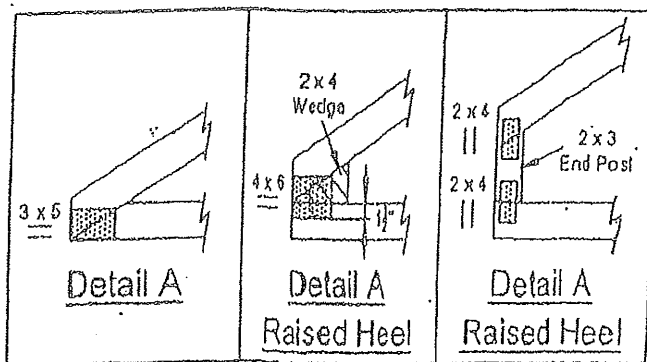
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

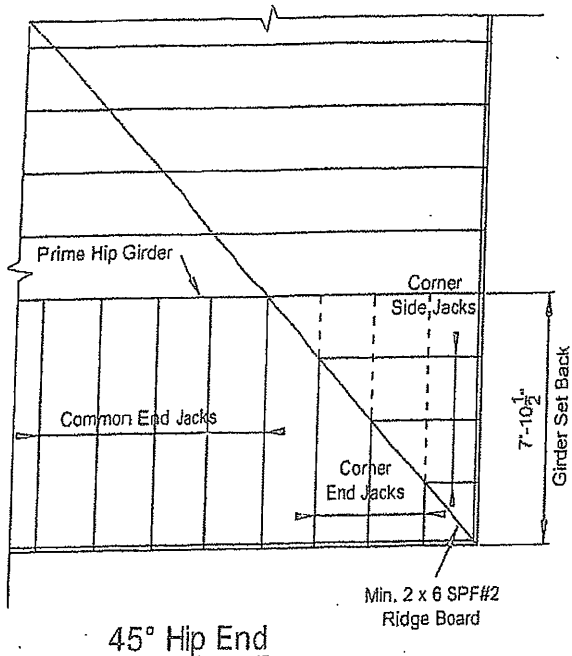
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

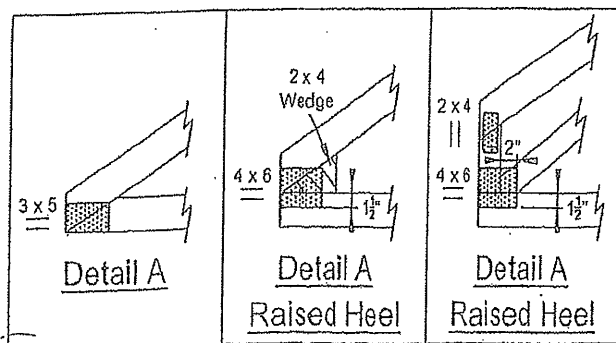
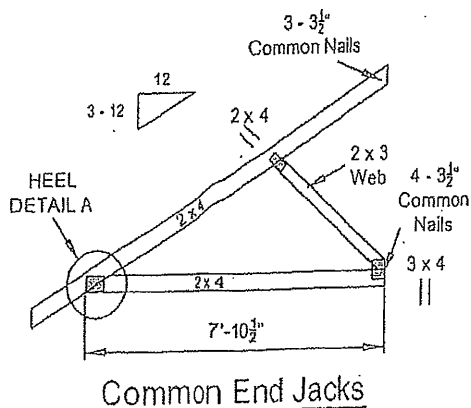
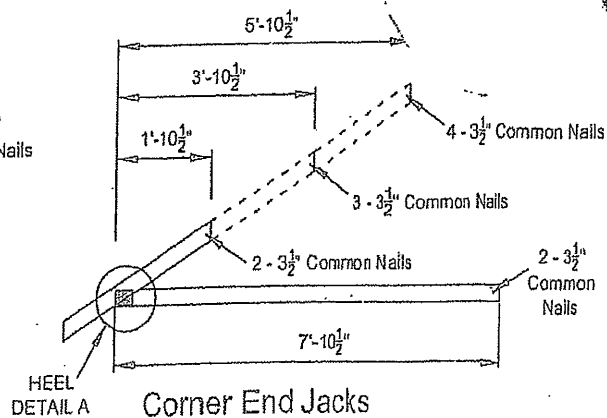
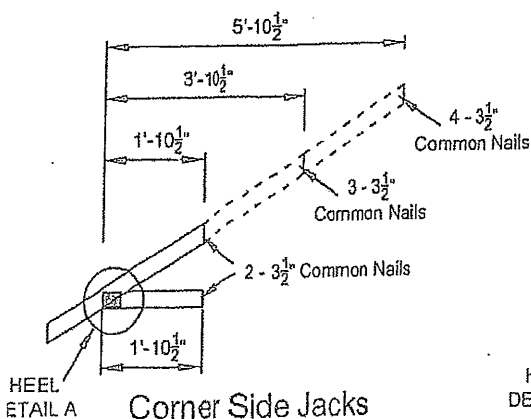
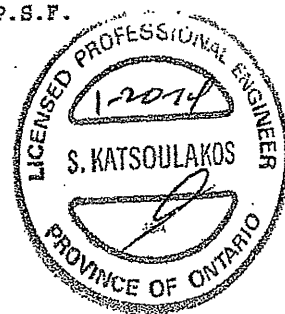
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.



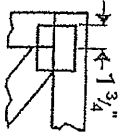
DWG NO YAM 3503.14
STRUCTURAL
COMPONENT ONLY



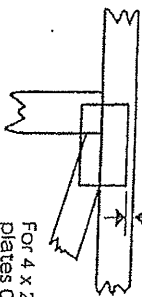
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mittek software or upon request.

PLATE SIZE

4 X 4

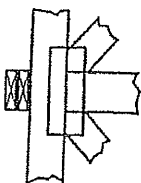
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



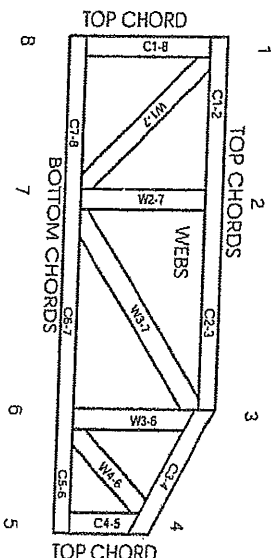
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8

dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

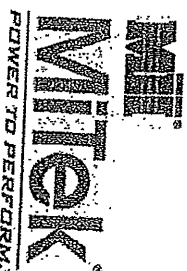
PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mittek Engineering Reference Sheet: MIT-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

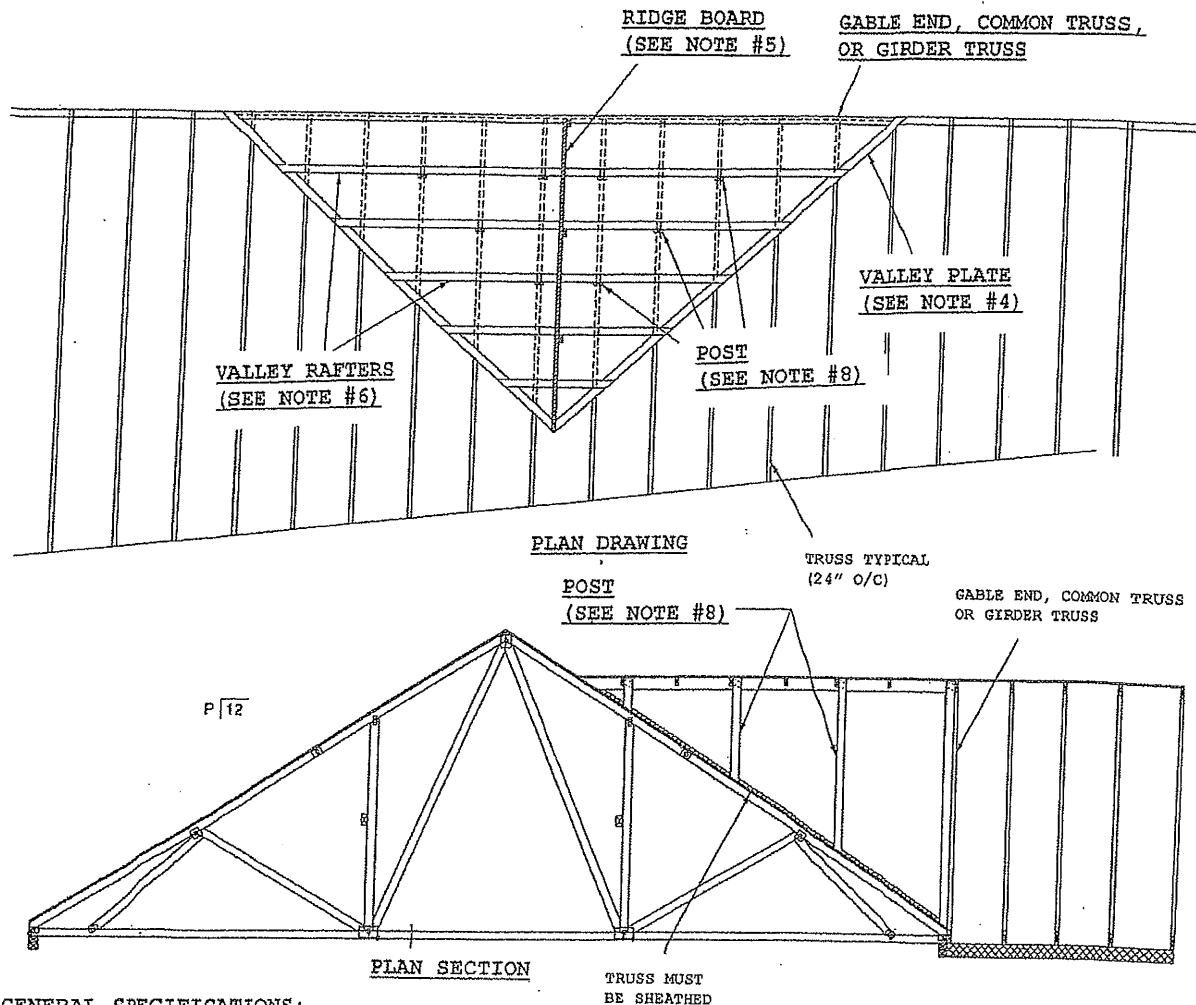
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

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CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

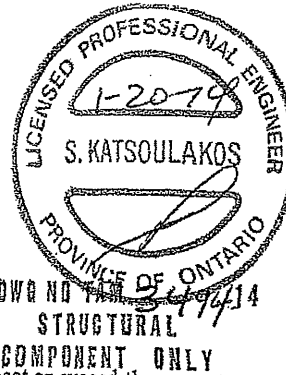


DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpjc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

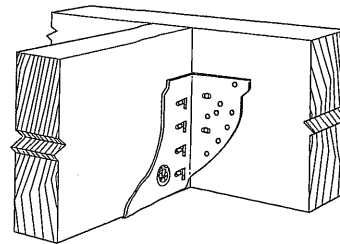
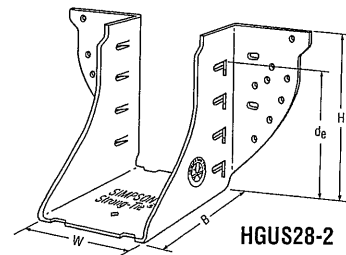
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

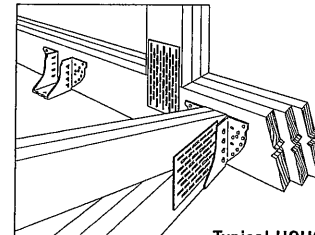
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



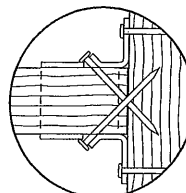
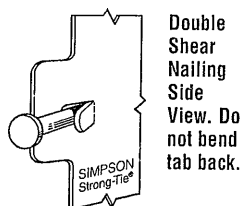
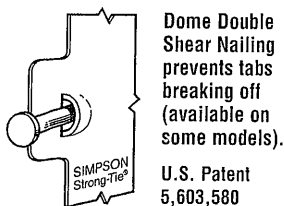
Typical HGUS Installation



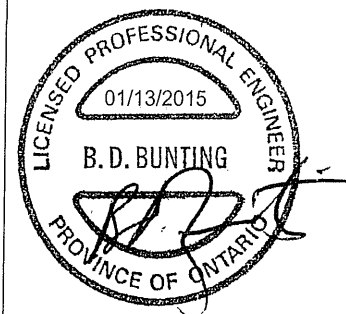
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1⅞	5⅜	5	4⅝ ₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅞	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅞	36-16d	12-15d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅞	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅞	66-16d	22-16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

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LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

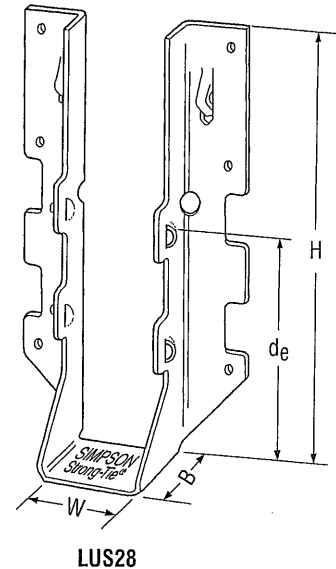
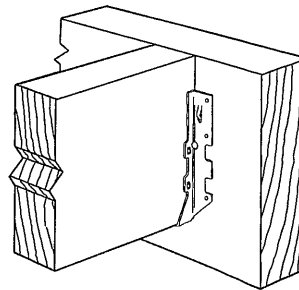
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

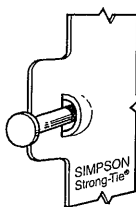
- These hangers cannot be modified.

Typical LUS Installation



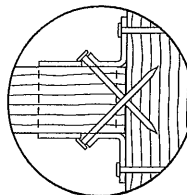
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1⅞	3⅞	1¾	1⅞	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅞	4-16d	2-16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¾	3⅞	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¾	3⅞	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8⅞	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

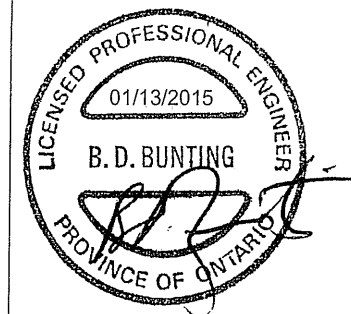


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECLUS15-1/15 exp: 12/16

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