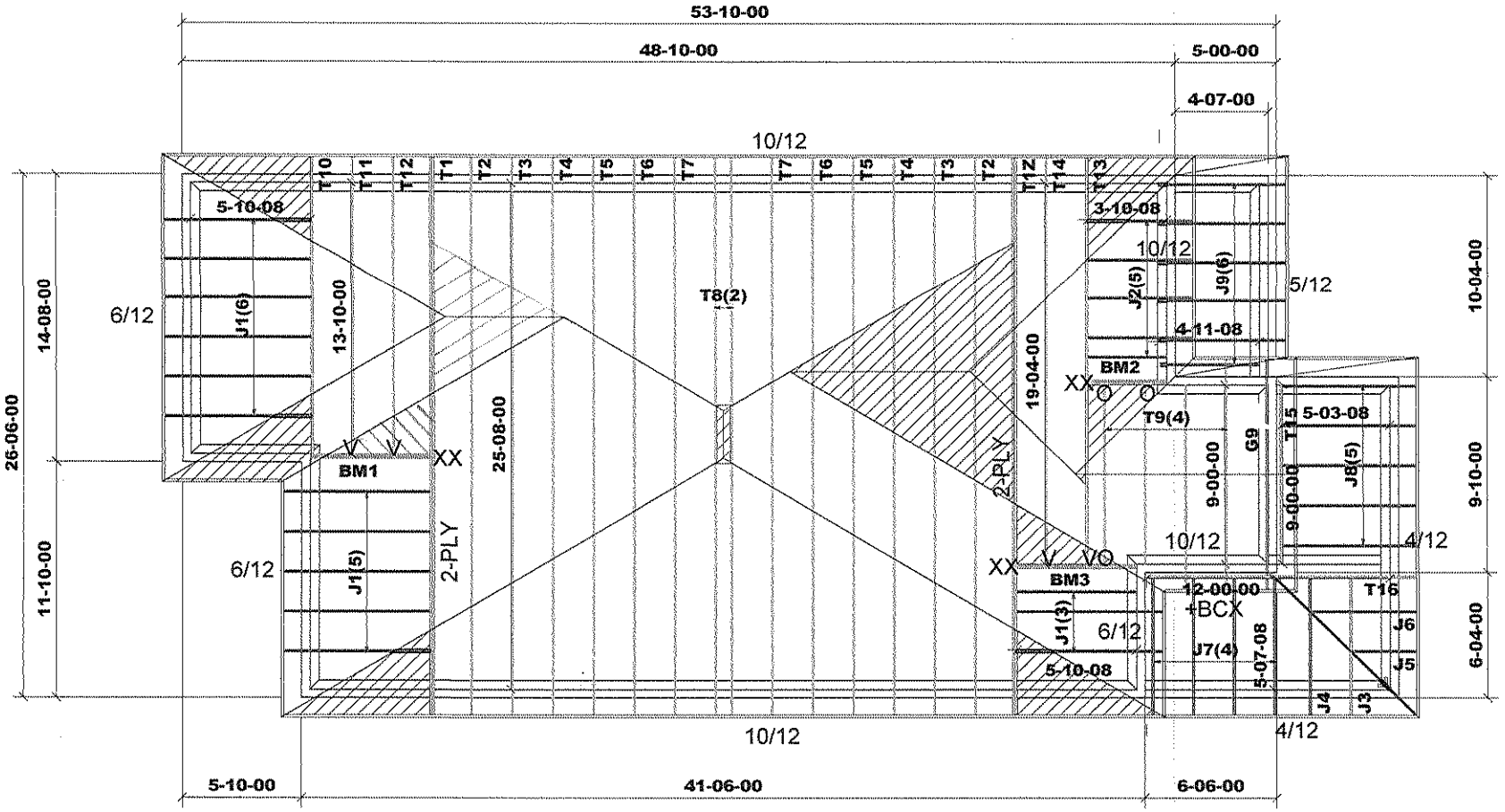
T-160325



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
LUS24(O)
HGUS26-2(XX)

BM1,2,3 : 2-2X10

Town of Innisfil Certified Model
05/01/2018 9:12:36 AM kgervais

MGG34

TAMARACK LUMBER INC.

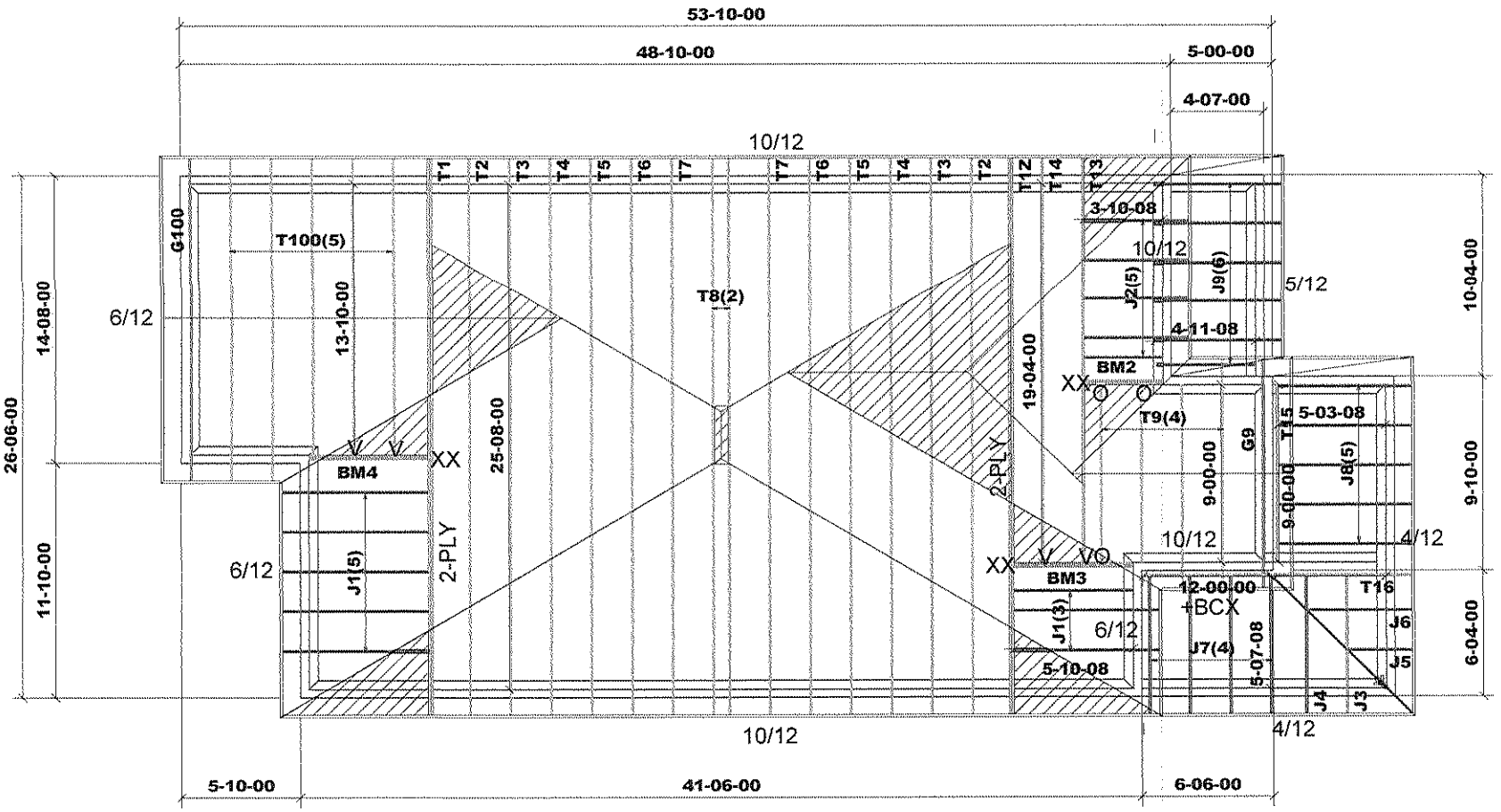
Job Track: **42067**
Layout ID: **267011**
Plan Log: **86121**

Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**
Project: **ALCONA SHORES**
Date: 4/21/2016 Designer: jane

Model / Elevation: S32-3-10G /A

Mitek ver 7.5.0

T-160596



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
LUS24(O)
HGUS26-2(XX)

BM2,3,4 : 2-2X10

Town of Innisfil Certified Model
05/01/2018 9:12:39 AM kgervais

M9634

TAMARACK LUMBER INC.

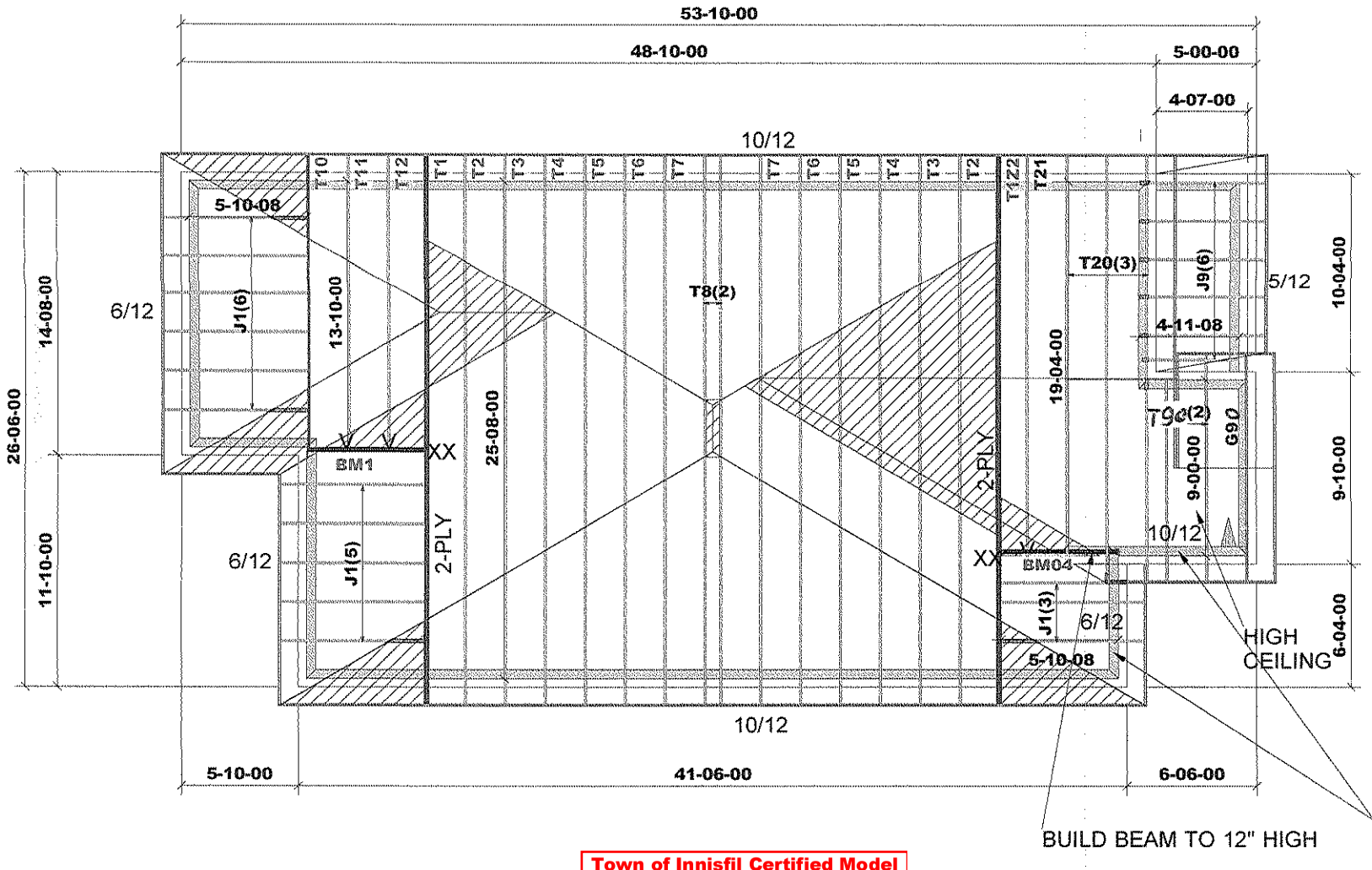
Job Track: **42067**
Layout ID: **272257**
Plan Log: **87565**

Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**
Project: **ALCONA SHORES**
Date: **8/22/2016** Designer: **JG**

Model / Elevation: **S32-3-10G / A-REAR UPG.**

Mitek ver 7.5.0

T-160325



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

BM1 : 2-2X10

BUILD BEAM TO 12" HIGH

1'-0" T.O.P SPLIT and 1'8" fascia split

Town of Innisfil Certified Model
05/01/2018 9:12:49 AM kgervais

M9634

TAMARACK TRUSS INC

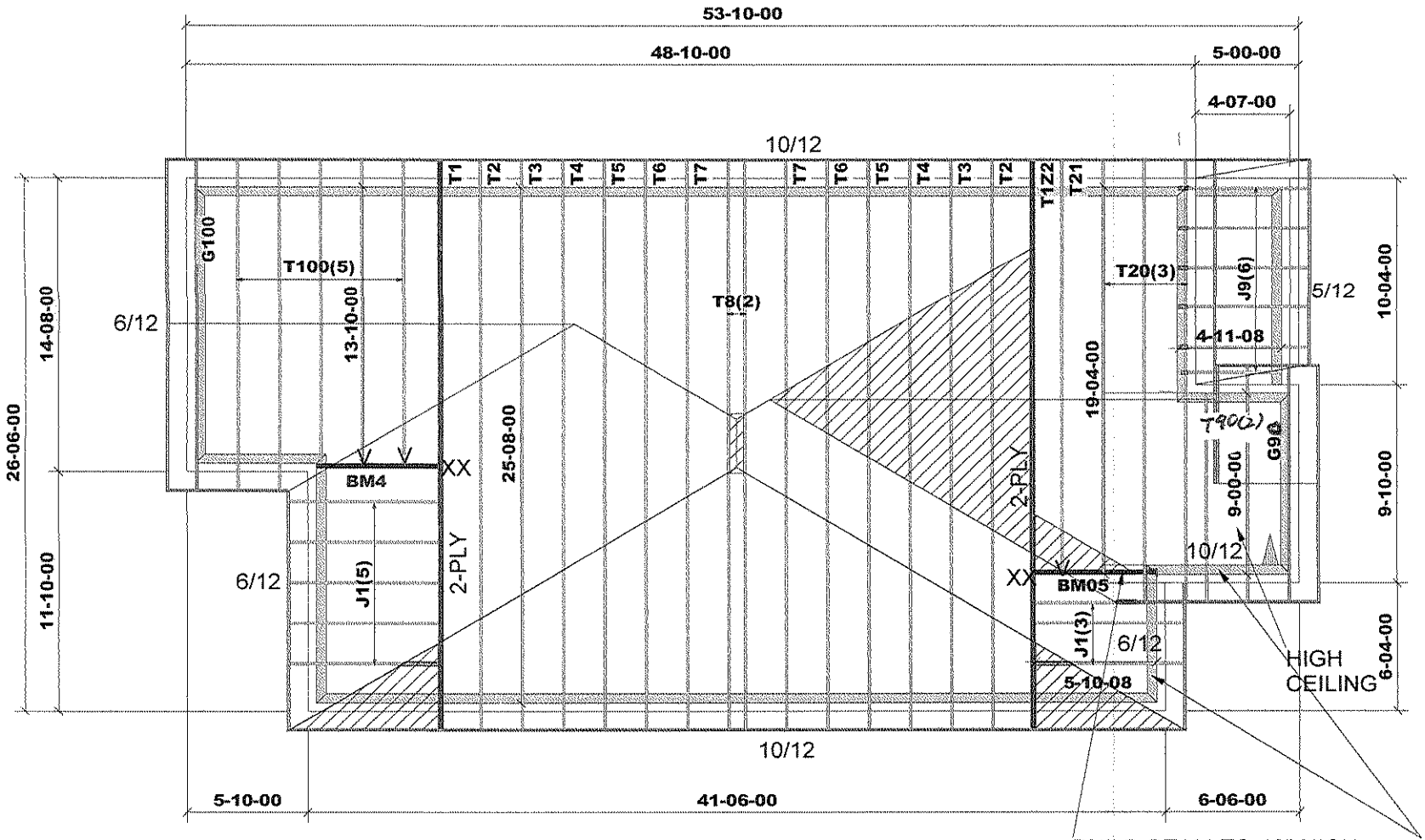
Job Track: **42067**
Layout ID: **267012**
Plan Log: **86121**

Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**
Project: **ALCONA SHORES**
Date: **11/9/2016** Designer: **JG**

Model / Elevation: **S32-3-10G/ B**

Mitek ver 7.5.0

T-160596



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

BM4 =BM05: 2-2X10

Town of Innisfil Certified Model
05/01/2018 9:12:51 AM kgervais

m9634

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	272258
Plan Log:	87565

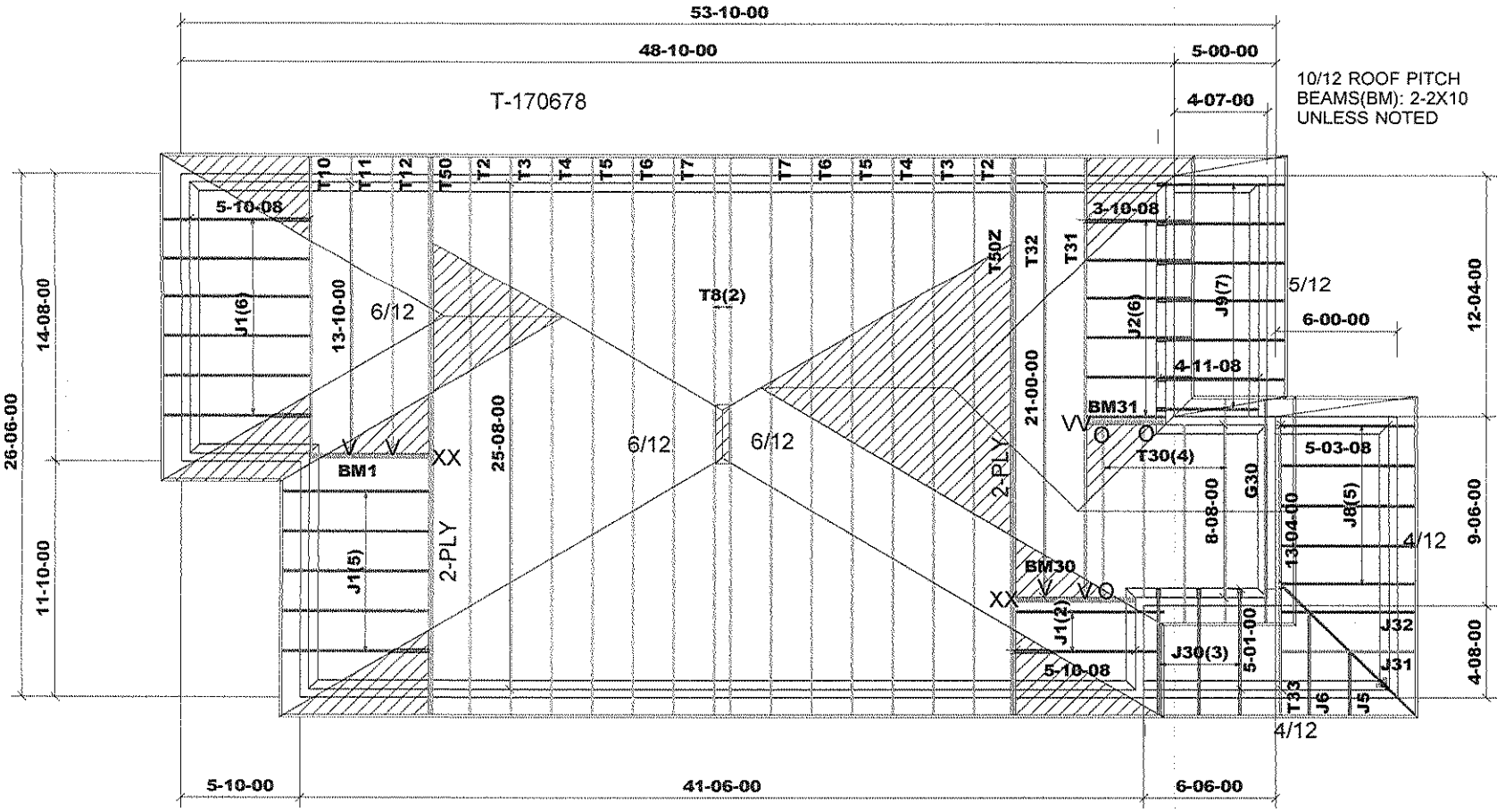
Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**

Model / Elevation: **S32-3-10G/ B- REAR UPGRADE**

Project: **ALCONA SHORES**

Date: 11/9/2016 Designer: JG

Mitek ver 7.5.0

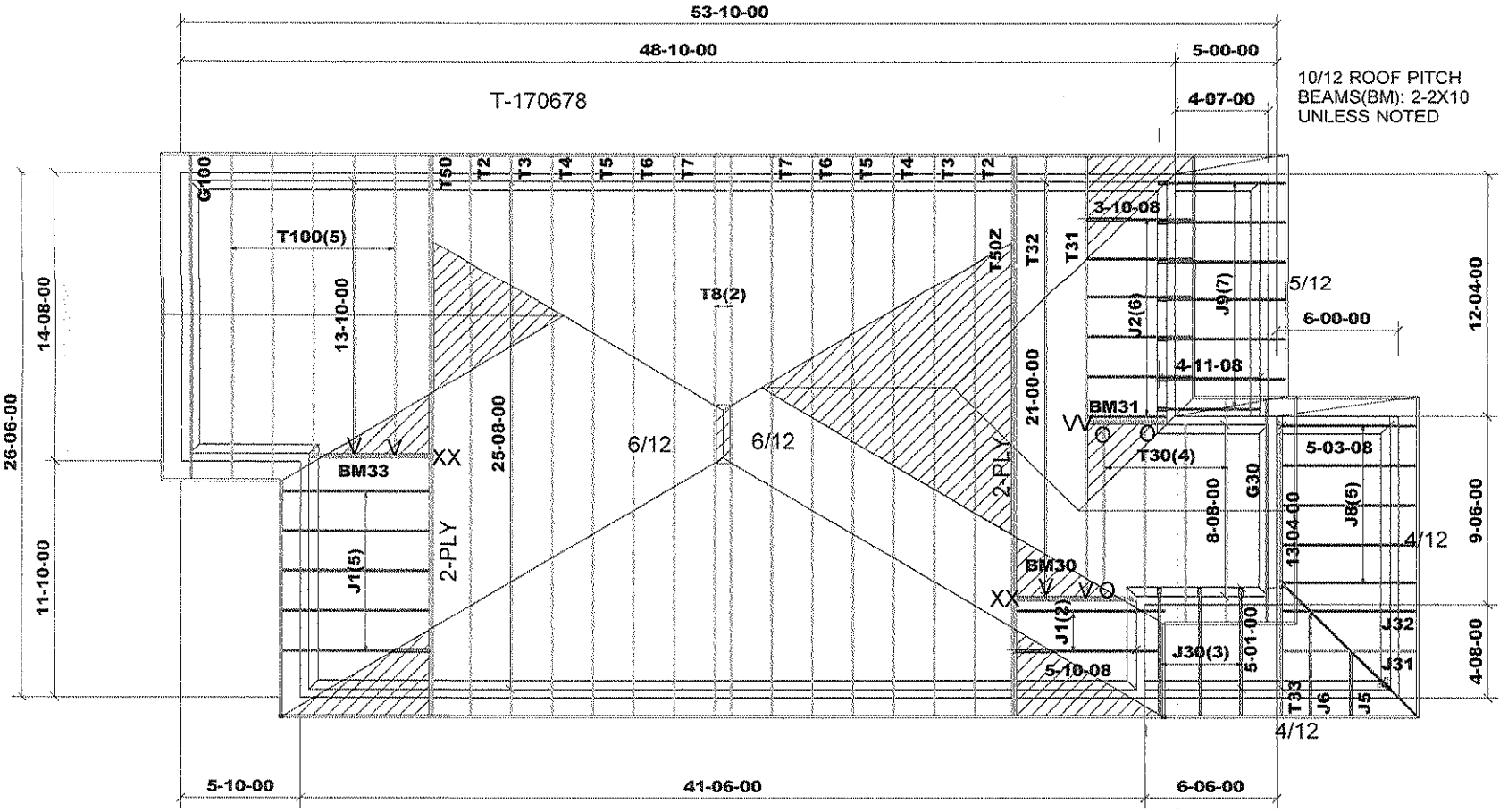


Town of Innisfil Certified Model
05/01/2018 9:12:53 AM kgervais

m91634

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-3-12 G / A
	Layout ID: 272270	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/7/2017 Designer: JG	

Mitek ver 7.5.0



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
LUS24(O)
HGUS26-2(XX)

Town of Innisfil Certified Model
05/01/2018 9:12:54 AM kgervais

m96634

TAMARACK LUMBER INC.

Job Track: 42067
Layout ID: 272271
Plan Log: 87565

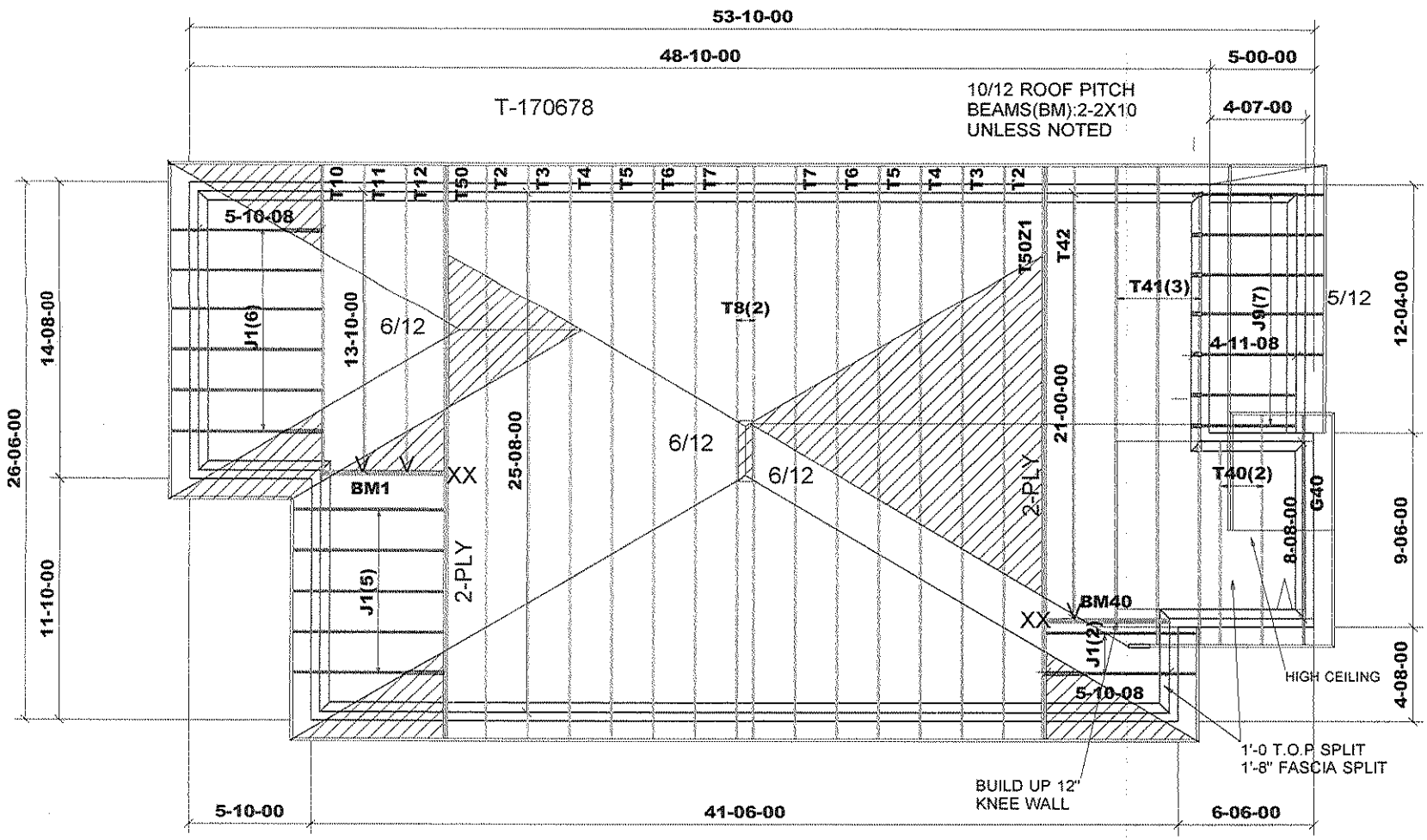
Builder / Location:
BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 9/7/2017 Designer: JG

Model / Elevation:
S32-3-12 G / A -REAR UPGRADE

Mitek ver 7.5.0



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa

TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

BM1 : 2-2X10

Town of Innisfil Certified Model
05/01/2018 9:12:58 AM kgervais

ma634

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	272272
Plan Log:	87565

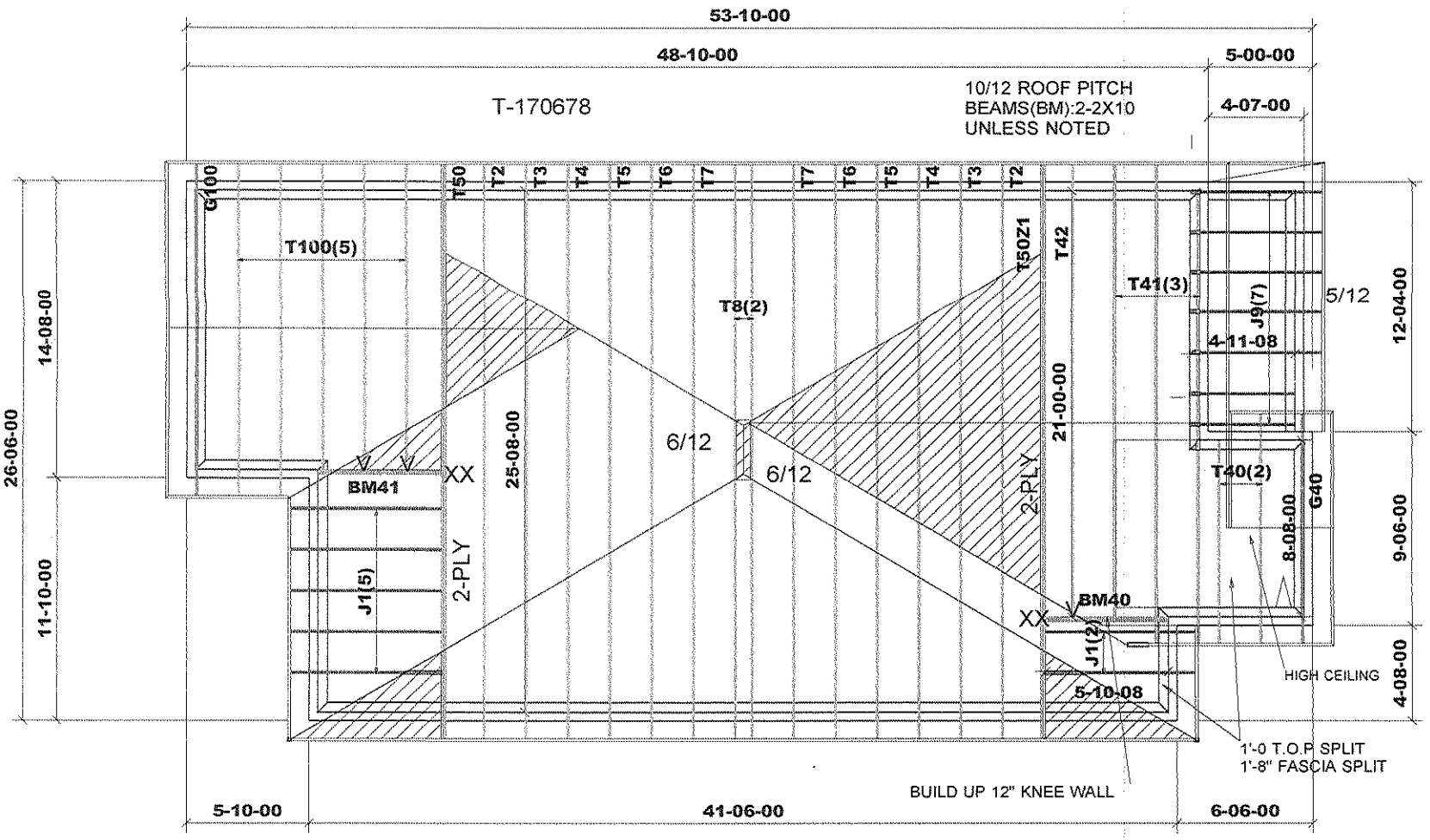
Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**

Model / Elevation: **S32-3-12G/ B**

Project: **ALCONA SHORES**

Date: 9/7/2017 Designer: JG

Mitek ver 7.5.0



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

BM1 : 2-2X10

Town of Innisfil Certified Model
05/01/2018 9:13:00 AM kgervais

M9603H

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	272273
Plan Log:	87565

Builder / Location:
BAYVIEW WELLINGTON / INNISFIL

Project: **ALCONA SHORES**

Date: 9/7/2017 Designer: JG

Model / Elevation:
S32-3-12G/ B - REAR UPGRADE

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

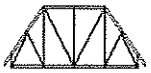
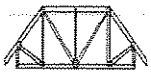
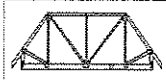
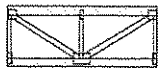
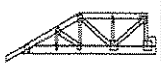
Delivery Shiplist

DATE	04/21/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 267011 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.96 154.66		
	1 2 Ply	T1Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.96 154.66		
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90 160.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	4	T9 COMMON	10.00 0.00	09-00-00	05-04-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	166.36 107.32		
	1	G9 COMMON	10.00 0.00	09-00-00	05-04-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	43.69 28.83		
	1	T10 HIP GIRDER	10.00 0.00	13-10-00	04-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	62.36 40.33		
	1	T11 HIP	10.00 0.00	13-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	61.08 40.17		
	1	T12 HIP	10.00 0.00	13-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	64.87 42.17		
	1	T13 HIP GIRDER	10.00 0.00	19-04-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	95.10 59.83		
	1	T14 HIP	10.00 0.00	19-04-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	95.50 60.67		
	1	T15 FLAT GIRDER	0.00 0.00	09-00-00	02-01-02	2 X 4	2 X 4	00-00-00 00-00-00	02-01-02 02-01-02	31.47 20.00		
	1	T16 HALF HIP	4.00 0.00	12-00-00	02-02-07	2 X 4	2 X 6	01-03-08 00-00-00	00-03-15 00-05-08	51.62 34.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

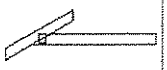
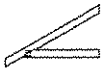
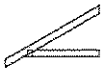
Delivery Shiplist

DATE	04/21/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 267011 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	5	J2 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	84.15 53.35		
	1	J3 JACK-OPEN	4.00 0.00	05-07-08	00-11-01	2 X 4	2 X 4	01-03-08 -03-10-01	00-03-15 00-03-08	10.23 6.67		
	1	J4 JACK-OPEN	4.00 0.00	05-07-08	01-07-01	2 X 4	2 X 4	01-03-08 -01-10-01	00-03-15 00-03-08	12.63 8.00		
	1	J5 JACK-OPEN	4.00 0.00	01-10-08	00-11-01	2 X 4	2 X 4	01-03-08 -00-01-01	00-03-15 00-03-08	5.95 4.00		
	1	J6 JACK-OPEN	4.00 0.00	01-10-08	01-07-01	2 X 4	2 X 4	01-03-08 01-10-15	00-03-15 00-04-03	8.35 5.33		
	4	J7 JACK-OPEN	4.00 0.00	05-07-08	02-02-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-07	59.36 37.32		
	5	J8 JACK-OPEN	4.00 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-02	70.30 46.65		
	6	J9 JACK-OPEN	5.00 0.00	04-11-08	02-06-02	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-06-02	85.86 52.02		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2188.71** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3450.70** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
3	Hangers	LUS24	

TOTAL # ITEMS= **10.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

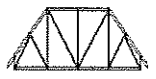
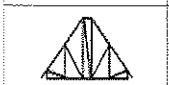
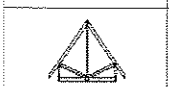
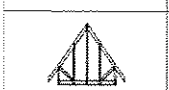
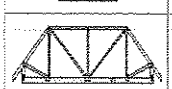
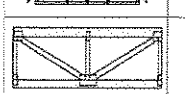
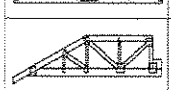
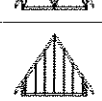
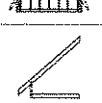
Delivery Shiplist

DATE	08/22/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272257 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3--10G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.96 154.66		
	1 2 Ply	T1Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.96 154.66		
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90 160.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	4	T9 COMMON	10.00 0.00	09-00-00	05-04-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	166.36 107.32		
	1	G9 COMMON	10.00 0.00	09-00-00	05-04-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	43.69 28.83		
	1	T13 HIP GIRDER	10.00 0.00	19-04-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	95.10 59.83		
	1	T14 HIP	10.00 0.00	19-04-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	95.50 60.67		
	1	T15 FLAT GIRDER	0.00 0.00	09-00-00	02-01-02	2 X 4	2 X 4	00-00-00 00-00-00	02-01-02 02-01-02	31.47 20.00		
	1	T16 HALF HIP	4.00 0.00	12-00-00	02-02-07	2 X 4	2 X 6	01-03-08 00-00-00	00-03-15 00-05-08	51.94 34.33		
	5	T100 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	327.15 210.85		
	1	G100 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	68.17 44.17		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

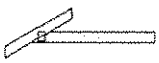
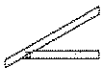
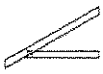
Delivery Shiplist

DATE	08/22/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272257 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3--10G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	5	J2 JACK-OPEN	10.00 / 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-10-07	84.15 / 53.35		
	1	J3 JACK-OPEN	4.00 / 0.00	05-07-08	00-11-01	2 X 4	2 X 4	01-03-08 / -03-10-01	00-03-15 / 00-03-08	10.23 / 6.67		
	1	J4 JACK-OPEN	4.00 / 0.00	05-07-08	01-07-01	2 X 4	2 X 4	01-03-08 / -01-10-01	00-03-15 / 00-03-08	12.63 / 8.00		
	1	J5 JACK-OPEN	4.00 / 0.00	01-10-08	00-11-01	2 X 4	2 X 4	01-03-08 / -00-01-01	00-03-15 / 00-03-08	5.95 / 4.00		
	1	J6 JACK-OPEN	4.00 / 0.00	01-10-08	01-07-01	2 X 4	2 X 4	01-03-08 / 01-10-15	00-03-15 / 00-04-03	8.35 / 5.33		
	4	J7 JACK-OPEN	4.00 / 0.00	05-07-08	02-02-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-02-07	59.36 / 37.32		
	5	J8 JACK-OPEN	4.00 / 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-02	70.30 / 46.65		
	6	J9 JACK-OPEN	5.00 / 0.00	04-11-08	02-06-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-05-06 / 02-06-02	85.86 / 52.02		

TOTAL # TRUSS= **65.00** TOTAL BFT OF ALL TRUSSES= **2257.04** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3557.29** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
3	Hangers	LUS24	

TOTAL # ITEMS= **10.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

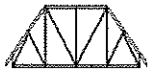
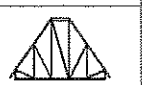
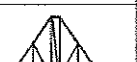
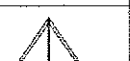
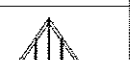
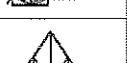
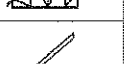
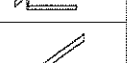
Delivery Shiplist

DATE	11/09/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 267012 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.96 154.66		
	1 2 Ply	T1Z2 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.96 154.66		
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90 160.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	2	T90 COMMON	10.00 0.00	09-00-00	6-0-11	2 X 4	2 X 4	01-03-08 01-03-08	2-3-11 2-3-11	83.18 53.66		
	1	G90 COMMON	10.00 0.00	09-00-00	6-0-11	2 X 4	2 X 4	01-03-08 01-03-08	2-3-11 2-3-11	43.69 28.83		
	1	T10 HIP GIRDER	10.00 0.00	13-10-00	04-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	62.36 40.33		
	1	T11 HIP	10.00 0.00	13-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	61.08 40.17		
	1	T12 HIP	10.00 0.00	13-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	64.87 42.17		
	3	T20 ROOF	10.00 0.00	19-04-00	10-6-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 2-3-11	293.58 185.49		
	1	T21 COMMON	10.00 0.00	19-00-00	10-6-06	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 3-7-0	92.40 59.33		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	6	J9 JACK-OPEN	5.00 0.00	04-11-08	02-06-02	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 00-04-07	85.86 52.02		

TOTAL # TRUSS= **48.00** TOTAL BFT OF ALL TRUSSES= **2043.72** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3228.84** LBS.

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	11/09/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 267012 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3 ELEVATION: B

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 5.00

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

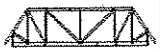
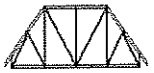
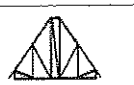
Delivery Shiplist

DATE	11/09/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272258 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3-10G ELEVATION: B-REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.96 154.66		
	1 2 Ply	T1Z2 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	246.96 154.66		
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90 160.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	2	T90 COMMON	10.00 0.00	09-00-00	05-04-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	83.18 53.66		
	1	G90 COMMON	10.00 0.00	09-00-00	05-04-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	43.69 28.83		
	3	T20 ROOF	10.00 0.00	19-04-00	10-02-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	293.58 185.49		
	1	T21 COMMON	10.00 0.00	19-00-00	10-02-06	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-11-00	92.40 59.33		
	5	T100 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	327.15 210.85		
	1	G100 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	68.17 44.17		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	6	J9 JACK-OPEN	5.00 0.00	04-11-08	02-06-02	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 00-04-07	85.86 52.02		

TOTAL # TRUSS= **45.00** TOTAL BFT OF ALL TRUSSES= **2112.05** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3335.11** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	11/09/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272258 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3-10G ELEVATION: B-REAR UPGRADE

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 5.00

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

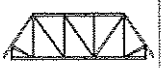
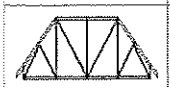
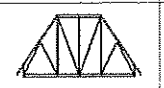
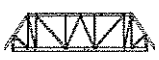
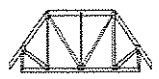
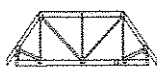
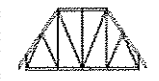
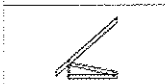
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272270 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S-32-3-12G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90 160.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1 2 Ply	T50 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	259.74 164.66		
	1 2 Ply	T50Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	259.74 164.66		
	1	T10 HIP GIRDER	10.00 0.00	13-10-00	04-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	62.36 40.33		
	1	T11 HIP	10.00 0.00	13-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	61.08 40.17		
	1	T12 HIP	10.00 0.00	13-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	64.87 42.17		
	4	T30 COMMON	10.00 0.00	08-08-00	05-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	161.40 107.32		
	1	G30 COMMON	10.00 0.00	08-08-00	05-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	42.25 27.83		
	1	T31 HIP GIRDER	10.00 0.00	21-00-00	04-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	117.90 72.00		
	1	T32 HIP	10.00 0.00	21-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	100.04 64.33		
	1	T33 HALF HIP	4.00 0.00	13-04-00	02-01-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-02	43.04 28.00		
	13	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	277.81 177.71		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

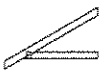
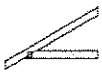
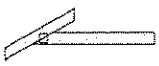
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272270 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S-32-3-12G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	6	J2 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	100.98 64.02		
	1	J5 JACK-OPEN	4.00 0.00	01-10-08	00-11-01	2 X 4	2 X 4	01-03-08 -00-01-01	00-03-15 00-03-08	5.95 4.00		
	1	J6 JACK-OPEN	4.00 0.00	01-10-08	01-07-01	2 X 4	2 X 4	01-03-08 01-10-15	00-03-15 00-04-03	8.35 5.33		
	5	J8 JACK-OPEN	4.00 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-02	70.30 46.65		
	7	J9 JACK-OPEN	5.00 0.00	04-11-08	02-06-02	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-06-02	100.17 60.69		
	3	J30 JACK-OPEN	4.00 0.00	05-01-00	02-00-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-00-04	40.71 26.01		
	1	J31 JACK-OPEN	4.00 0.00	05-03-08	00-11-01	2 X 4	2 X 4	01-03-08 -03-06-01	00-03-15 00-03-08	9.85 6.67		
	1	J32 JACK-OPEN	4.00 0.00	05-03-08	01-07-01	2 X 4	2 X 4	01-03-08 -01-06-01	00-03-15 00-03-08	12.25 8.00		

TOTAL # TRUSS= **67.00** TOTAL BFT OF ALL TRUSSES= **2233.57** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3511.63** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **10.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

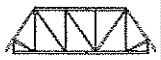
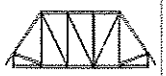
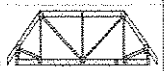
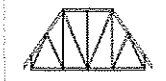
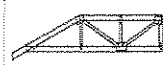
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272271 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3-12G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90 160.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1 / 2 Ply	T50 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	259.74 164.66		
	1 / 2 Ply	T50Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	259.74 164.66		
	4	T30 COMMON	10.00 0.00	08-08-00	05-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	161.40 107.32		
	1	G30 COMMON	10.00 0.00	08-08-00	05-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	42.25 27.83		
	1	T31 HIP GIRDER	10.00 0.00	21-00-00	04-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	117.90 72.00		
	1	T32 HIP	10.00 0.00	21-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	100.04 64.33		
	1	T33 HALF HIP	4.00 0.00	13-04-00	02-01-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-02	43.04 28.00		
	5	T100 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	327.15 210.85		
	1	G100 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	65.98 42.17		
	7	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	117.53 74.69		
	6	J2 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	100.98 64.02		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

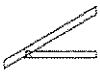
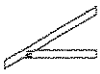
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272271 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3-12G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	J5 JACK-OPEN	4.00 0.00	01-10-08	00-11-01	2 X 4	2 X 4	01-03-08 -00-01-01	00-03-15 00-03-08	5.95 4.00		
	1	J6 JACK-OPEN	4.00 0.00	01-10-08	01-07-01	2 X 4	2 X 4	01-03-08 01-10-15	00-03-15 00-04-03	8.35 5.33		
	5	J8 JACK-OPEN	4.00 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-02	70.30 46.65		
	7	J9 JACK-OPEN	5.00 0.00	04-11-08	02-06-02	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-06-02	100.17 60.69		
	3	J30 JACK-OPEN	4.00 0.00	05-01-00	02-00-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-00-04	40.71 26.01		
	1	J31 JACK-OPEN	4.00 0.00	05-03-08	00-11-01	2 X 4	2 X 4	01-03-08 -03-06-01	00-03-15 00-03-08	9.85 6.67		
	1	J32 JACK-OPEN	4.00 0.00	05-03-08	01-07-01	2 X 4	2 X 4	01-03-08 -01-06-01	00-03-15 00-03-08	12.25 8.00		

TOTAL # TRUSS= 64.00 TOTAL BFT OF ALL TRUSSES= 2260.90 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3556.17 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 10.00

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

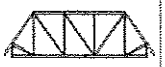
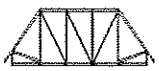
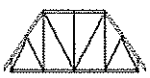
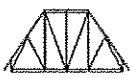
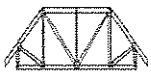
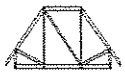
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272272 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3-12G ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90 160.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1 2 Ply	T50 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	261.10 166.00		
	1 2 Ply	T50Z1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	261.10 166.00		
	1	T10 HIP GIRDER	10.00 0.00	13-10-00	04-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	62.36 40.33		
	1	T11 HIP	10.00 0.00	13-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	61.08 40.17		
	1	T12 HIP	10.00 0.00	13-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	64.87 42.17		
	2	T40 COMMON	10.00 0.00	08-08-00	05-11-00	2 X 4	2 X 4	01-03-08 01-03-08	02-03-11 02-03-11	85.62 54.66		
	1	G40 COMMON	10.00 0.00	08-08-00	05-11-00	2 X 4	2 X 4	01-03-08 01-03-08	02-03-11 02-03-11	46.07 29.33		
	3	T41 ROOF	10.00 0.00	21-00-00	11-02-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 02-03-11	336.60 212.49		
	1	T42 COMMON	10.00 0.00	21-00-00	11-02-11	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-03-11	105.24 65.83		
	13	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	218.27 138.71		
	7	J9 JACK-OPEN	5.00 0.00	04-11-08	02-06-02	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-06-02	100.17 60.69		

TOTAL # TRUSS= 48.00 TOTAL BFT OF ALL TRUSSES= 2099.40 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3315.32 LBS.

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272272 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3-12G ELEVATION: B

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 5.00

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

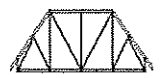
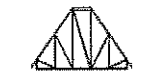
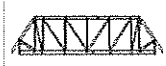
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272273 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3-12G ELEVATION: B-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	252.90 160.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1 / 2 Ply	T50 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	261.10 166.00		
	1 / 2 Ply	T50Z1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	261.10 166.00		
	2	T40 COMMON	10.00 0.00	08-08-00	05-11-00	2 X 4	2 X 4	01-03-08 01-03-08	02-03-11 02-03-11	85.62 54.66		
	1	G40 COMMON	10.00 0.00	08-08-00	05-11-00	2 X 4	2 X 4	01-03-08 01-03-08	02-03-11 02-03-11	46.07 29.33		
	3	T41 ROOF	10.00 0.00	21-00-00	11-02-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 02-03-11	336.60 212.49		
	1	T42 COMMON	10.00 0.00	21-00-00	11-02-11	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-03-11	105.24 65.83		
	5	T100 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	327.15 210.85		
	1	G100 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	65.98 42.17		
	7	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	117.53 74.69		
	7	J9 JACK-OPEN	5.00 0.00	04-11-08	02-06-02	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-06-02	100.17 60.69		

TOTAL # TRUSS= **45.00** TOTAL BFT OF ALL TRUSSES= **2165.73** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3419.40** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272273 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-3-12G ELEVATION: B-REAR

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= **5.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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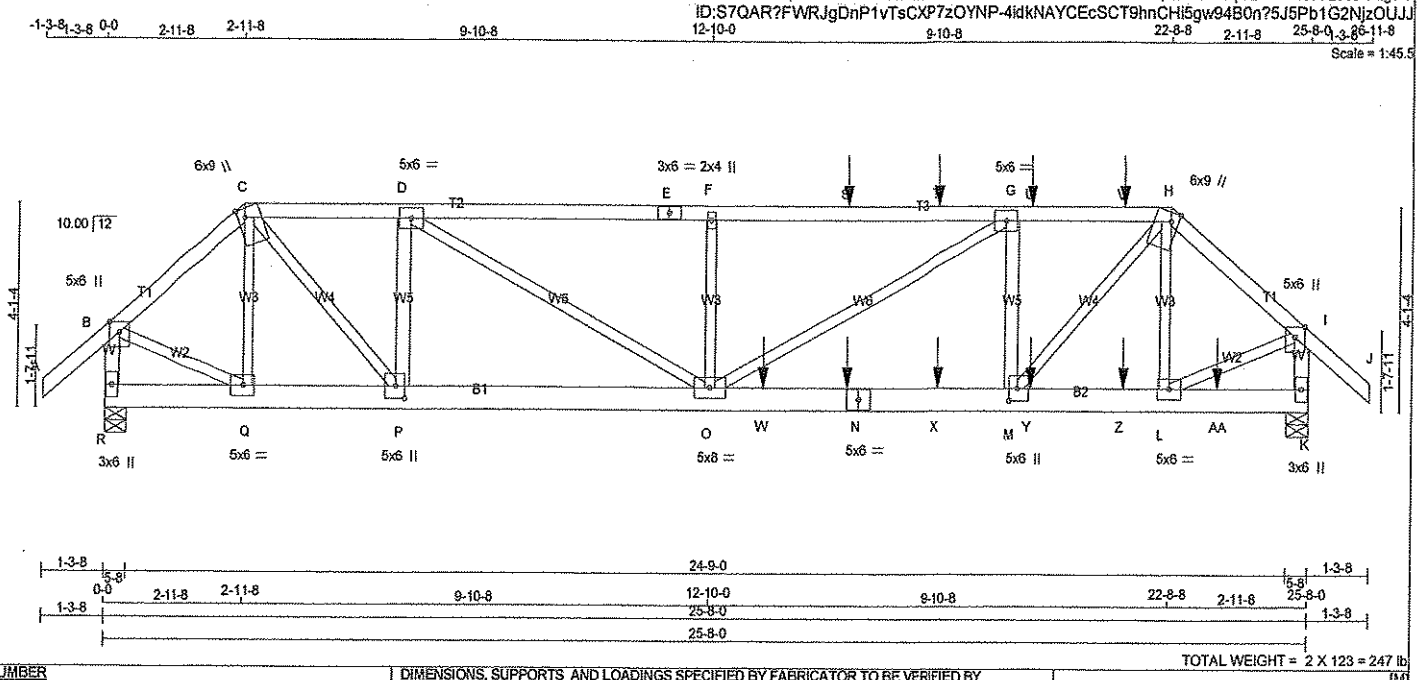

TOTAL WEIGHT = 2 X 123 = 247 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	TOP
C- E 1	12	TOP
E- H 1	12	SIDE(0.0)
H- J 1	12	TOP
R- B 1	12	TOP
K- I 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
R- N 2	12	SIDE(0.0)
N- K 2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	5.0	6.0	3.00	2.25
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	5.0	6.0	3.00	2.25
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3059	0	3059	0	0	5-8	1-11
K	3584	0	3584	0	0	5-8	2-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2358	1595 / 0	382 / 0	0 / 0	0 / 0	380 / 0	0 / 0
K	2786	1844 / 0	478 / 0	0 / 0	0 / 0	464 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00
B- C	-2639 / 0	-122.2	-122.2	0.13 (1)	5.32
C- D	-4155 / 0	-122.2	-122.2	0.30 (1)	4.37
D- E	-6907 / 0	-122.2	-122.2	0.81 (1)	3.11
E- F	-6907 / 0	-122.2	-122.2	0.81 (1)	3.11
F- S	-6907 / 0	-122.2	-122.2	0.83 (1)	3.08
S- T	-6907 / 0	-122.2	-122.2	0.83 (1)	3.08
T- G	-6907 / 0	-122.2	-122.2	0.83 (1)	3.08
G- U	-5088 / 0	-122.2	-122.2	0.52 (1)	3.69
U- V	-5088 / 0	-122.2	-122.2	0.52 (1)	3.69
V- H	-5088 / 0	-122.2	-122.2	0.52 (1)	3.69
H- I	-3338 / 0	-122.2	-122.2	0.14 (1)	4.98
I- J	0 / 54	-122.2	-122.2	0.09 (1)	10.00
R- B	-3044 / 0	0.0	0.0	0.17 (1)	6.62
K- I	-3518 / 0	0.0	0.0	0.20 (1)	6.24
R- Q	0 / 0	-28.0	-28.0	0.03 (1)	10.00
Q- P	0 / 2154	-28.0	-28.0	0.19 (1)	10.00
P- O	0 / 4156	-28.0	-28.0	0.46 (1)	10.00
O- W	0 / 5089	-28.0	-28.0	0.75 (1)	10.00
W- N	0 / 5089	-28.0	-28.0	0.75 (1)	10.00
N- X	0 / 5089	-28.0	-28.0	0.75 (1)	10.00
X- M	0 / 5089	-28.0	-28.0	0.75 (1)	10.00
M- Y	0 / 2526	-28.0	-28.0	0.34 (1)	10.00
Y- Z	0 / 2526	-28.0	-28.0	0.34 (1)	10.00
Z- L	0 / 2526	-28.0	-28.0	0.34 (1)	10.00
L-AA	0 / 0	-28.0	-28.0	0.03 (2)	10.00
AA- K	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-685 / 0	0.09 (1)
C- P	0 / 2963	0.37 (1)
P- D	-2215 / 0	0.20 (1)
M- G	-2126 / 0	0.19 (1)
M- H	0 / 3793	0.47 (1)
L- H	-1036 / 0	0.13 (1)
B- Q	0 / 2340	0.29 (1)
L- I	0 / 2751	0.34 (1)
O- G	0 / 2097	0.26 (1)
D- O	0 / 3174	0.39 (1)
O- F	-938 / 0	0.12 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	15-8-12	-40	-70	---	FRONT	VERT	TOTAL
S	15-8-12	-147	-147	---	FRONT	VERT	TOTAL
T	17-8-12	-147	-147	---	FRONT	VERT	TOTAL
U	19-8-12	-147	-147	---	FRONT	VERT	TOTAL
V	21-8-12	-148	-148	---	FRONT	VERT	TOTAL
W	13-11-8	-1660	-1660	---	FRONT	VERT	TOTAL
X	17-8-12	-40	-70	---	FRONT	VERT	TOTAL
Y	19-8-12	-40	-70	---	FRONT	VERT	TOTAL
Z	21-8-12	-40	-70	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.83 (F-G:1) , BC=0.75 (M-O:1) , WB=0.47 (H-M:1) , SSI=0.50 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.67 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 42116 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM17846-16
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 147.1 lbs FACTORED DOWN AT 15-8-12, 147.1 lbs FACTORED DOWN AT 17-8-12, AND 147.1 lbs FACTORED DOWN AT 19-8-12, AND 148.2 lbs FACTORED DOWN AT 21-8-12 ON TOP CHORD, AND 1660.4 lbs FACTORED DOWN AT 13-11-8, 69.9 lbs FACTORED DOWN AT 15-8-12, 69.9 lbs FACTORED DOWN AT 17-8-12, 69.9 lbs FACTORED DOWN AT 19-8-12, AND 69.9 lbs FACTORED DOWN AT 21-8-12, AND 69.9 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
AA	23-8-12	-40	-70	---	FRONT	VERT	TOTAL

DWG NO. TAM17846-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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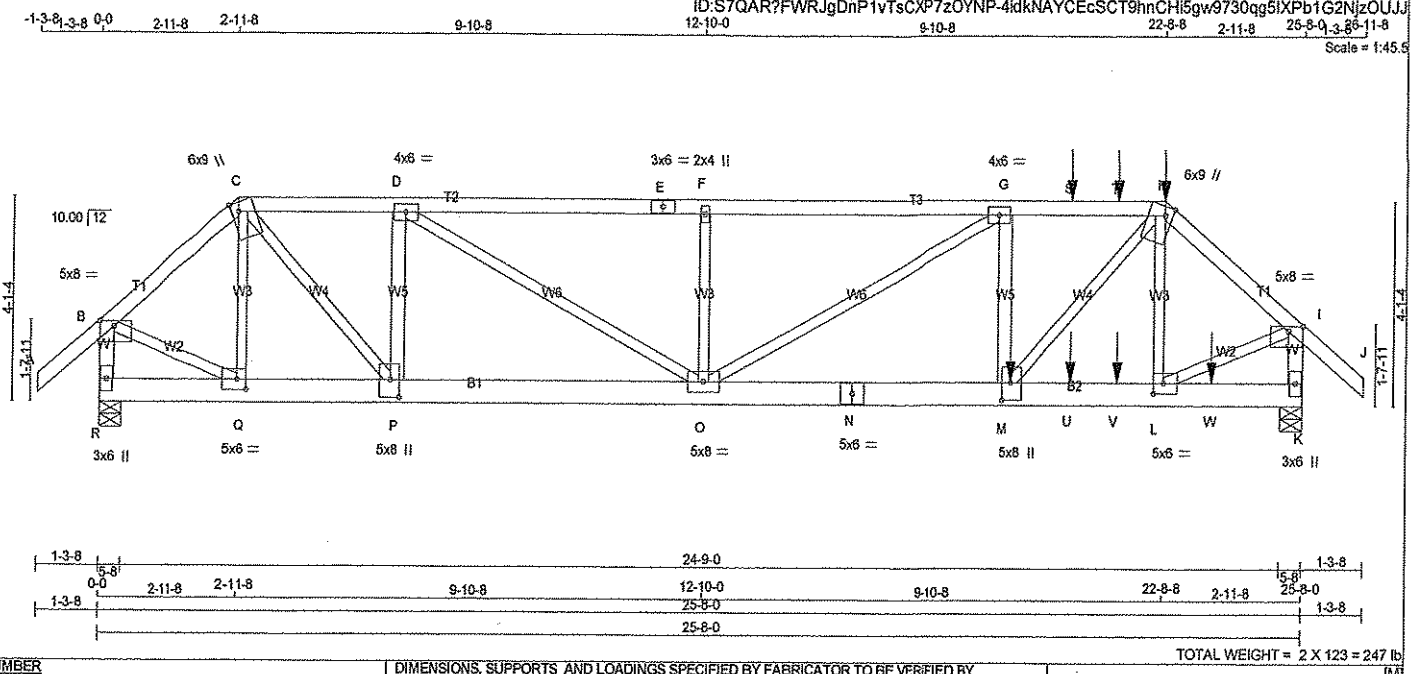

TOTAL WEIGHT = 2 X 123 = 247 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	1	12	TOP
K- I	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
G- M	1	3	SIDE(553.7)
2x4	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	1.25	3.50
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW+t	MT20	5.0	8.0	4.25	2.25
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	5.0	8.0	4.25	2.25
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2890	0	2890	0	0	5-8	1-9
K	4771	0	4771	0	0	5-8	4-4

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2228	1507 / 0	362 / 0	0 / 0	0 / 0	359 / 0	0 / 0
K	3884	2480 / 0	605 / 0	0 / 0	0 / 0	598 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00	
B- C	-2646 / 0	-122.2	-122.2	0.13 (1)	5.47	
C- D	-3921 / 0	-122.2	-122.2	0.31 (1)	4.46	
D- E	-6158 / 0	-122.2	-122.2	0.65 (1)	3.45	
E- F	-6158 / 0	-122.2	-122.2	0.65 (1)	3.45	
F- G	-6158 / 0	-122.2	-122.2	0.65 (1)	3.45	
G- S	-6631 / 0	-122.2	-122.2	0.43 (1)	3.41	
S- T	-6631 / 0	-122.2	-122.2	0.43 (1)	3.41	
T- H	-6631 / 0	-122.2	-122.2	0.43 (1)	3.41	
H- I	-4641 / 0	-122.2	-122.2	0.18 (1)	4.32	
I- J	0 / 54	-122.2	-122.2	0.09 (1)	10.00	
R- B	-2862 / 0	0.0	0.0	0.16 (1)	6.79	
K- I	-4754 / 0	0.0	0.0	0.27 (1)	5.49	
R- Q	0 / 0	-28.0	-28.0	0.02 (1)	10.00	
Q- P	0 / 2006	-28.0	-28.0	0.15 (1)	10.00	
P- O	0 / 3921	-28.0	-28.0	0.30 (1)	10.00	
O- N	0 / 6632	-28.0	-28.0	0.52 (1)	10.00	
N- M	0 / 6632	-28.0	-28.0	0.52 (1)	10.00	
M- U	0 / 3520	-28.0	-28.0	0.28 (1)	10.00	
U- V	0 / 3520	-28.0	-28.0	0.28 (1)	10.00	
V- L	0 / 3520	-28.0	-28.0	0.28 (1)	10.00	
L- W	0 / 0	-28.0	-28.0	0.05 (1)	10.00	
W- K	0 / 0	-28.0	-28.0	0.05 (1)	10.00	

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
Q- C	-706 / 0	0.09 (1)
C- P	0 / 2834	0.35 (1)
P- D	-1944 / 0	0.18 (1)
M- G	-502 / 29	0.05 (1)
M- H	0 / 4605	0.57 (1)
L- H	-1135 / 0	0.14 (1)
B- Q	0 / 2181	0.27 (1)
L- I	0 / 3824	0.47 (1)
O- G	-547 / 0	0.30 (1)
D- O	0 / 2581	0.32 (1)
O- F	-822 / 0	0.10 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
M	19-5-8	-2824	-2824	---	BACK	VERT	TOTAL
S	20-8-12	-147	-147	---	BACK	VERT	TOTAL
T	21-8-12	-148	-148	---	BACK	VERT	TOTAL
U	20-8-12	-40	-70	---	BACK	VERT	TOTAL
V	21-8-12	-40	-70	---	BACK	VERT	TOTAL
W	23-8-12	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.65 (F-G:1) , BC=0.52 (M-O:1) , WB=0.57 (H-M:1) , SSI=0.19 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.50)
JSI METAL= 0.58 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 42116 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17047-16
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T1Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 22-8-8, AND 147.1 lbs FACTORED DOWN AT 20-8-12, AND 148.2 lbs FACTORED DOWN AT 21-8-12 ON TOP CHORD, AND 2824.4 lbs FACTORED DOWN AT 19-5-8, 69.9 lbs FACTORED DOWN AT 20-8-12, AND 69.9 lbs FACTORED DOWN AT 21-8-12, AND 69.9 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
42176
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17847-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42667	DRWG NO.
267012	T1Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington — Version 8.000 S Jan 15 2016 MiTek Industries, Inc. Thu Apr 21 14:18:13 2016 Page 1

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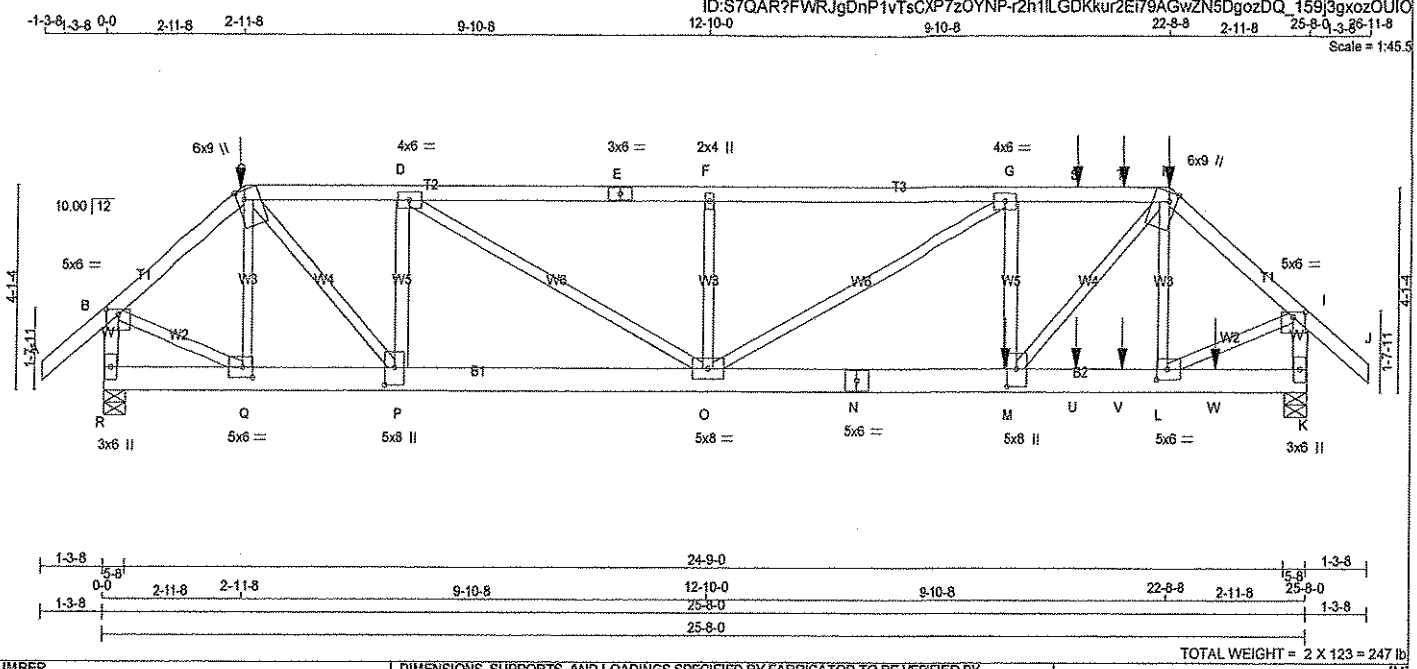

TOTAL WEIGHT = 2 X 123 = 247 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	1	12	TOP
K- I	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
G- M	1	4	SIDE(418.1)
2x4	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	6.0	1.25	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW+t	MT20	5.0	8.0	4.25	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	5.0	8.0	4.25	2.50
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2963	0	2963	0	0	5-8	1-10
K	4406	0	4406	0	0	5-8	3-11

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2295	1533 / 0	385 / 0	0 / 0	0 / 0	377 / 0	0 / 0
K	3430	2260 / 0	595 / 0	0 / 0	0 / 0	574 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00	Q- C	-734 / 0		0.09 (1)
B- C	-2720 / 0	-122.2	-122.2	0.13 (1)	5.41	C- P	0 / 2628		0.33 (1)
C- D	-3836 / 0	-122.2	-122.2	0.30 (1)	4.50	P- D	-1789 / 0		0.16 (1)
D- E	-5805 / 0	-122.2	-122.2	0.62 (1)	3.56	M- G	-657 / 1		0.06 (1)
E- F	-5805 / 0	-122.2	-122.2	0.62 (1)	3.56	M- H	0 / 4138		0.51 (1)
F- G	-5805 / 0	-122.2	-122.2	0.62 (1)	3.56	L- H	-1035 / 0		0.13 (1)
G- S	-6020 / 0	-122.2	-122.2	0.41 (1)	3.59	B- Q	0 / 2241		0.28 (1)
S- T	-6020 / 0	-122.2	-122.2	0.41 (1)	3.59	L- I	0 / 3504		0.43 (1)
T- H	-6020 / 0	-122.2	-122.2	0.41 (1)	3.59	O- G	-249 / 0		0.14 (1)
H- I	-4251 / 0	-122.2	-122.2	0.17 (1)	4.49	D- O	0 / 2271		0.28 (1)
I- J	0 / 54	-122.2	-122.2	0.09 (1)	10.00	O- F	-822 / 0		0.10 (1)
R- B	-2931 / 0	0.0	0.0	0.17 (1)	6.72				
K- I	-4385 / 0	0.0	0.0	0.25 (1)	5.68				
R- Q	0 / 0	-28.0	-28.0	0.02 (3)	10.00				
Q- P	0 / 2661	-28.0	-28.0	0.15 (1)	10.00				
P- O	0 / 3837	-28.0	-28.0	0.30 (1)	10.00				
O- N	0 / 6021	-28.0	-28.0	0.47 (1)	10.00				
N- M	0 / 6021	-28.0	-28.0	0.47 (1)	10.00				
M- U	0 / 3225	-28.0	-28.0	0.25 (1)	10.00				
U- V	0 / 3225	-28.0	-28.0	0.25 (1)	10.00				
V- L	0 / 3225	-28.0	-28.0	0.25 (1)	10.00				
L- W	0 / 0	-28.0	-28.0	0.04 (2)	10.00				
W- K	0 / 0	-28.0	-28.0	0.04 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	2-11-8	-228	-228	---	FRONT	VERT	TOTAL
H	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
M	19-2-8	-2305	-2305	---	BACK	VERT	TOTAL
S	20-8-12	-147	-147	---	BACK	VERT	TOTAL
T	21-8-12	-148	-148	---	BACK	VERT	TOTAL
U	20-8-12	-40	-70	---	BACK	VERT	TOTAL
V	21-8-12	-40	-70	---	BACK	VERT	TOTAL
W	23-8-12	-40	-70	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 42116 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM17864-16
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.62 (D-F:1) , BC=0.47 (M-O:1) , WB=0.51 (H-M:1) , SSI=0.19 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.53 (N) (INPUT = 1.00)

po 4/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267012	T1Z2	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 227.6 lbs FACTORED DOWN AT 22-8-8, AND 147.1 lbs FACTORED DOWN AT 20-8-12, AND 148.2 lbs FACTORED DOWN AT 21-8-12 ON TOP CHORD, AND 2304.5 lbs FACTORED DOWN AT 19-2-8, 69.9 lbs FACTORED DOWN AT 20-8-12, AND 69.9 lbs FACTORED DOWN AT 21-8-12, AND 69.9 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
42176
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17864-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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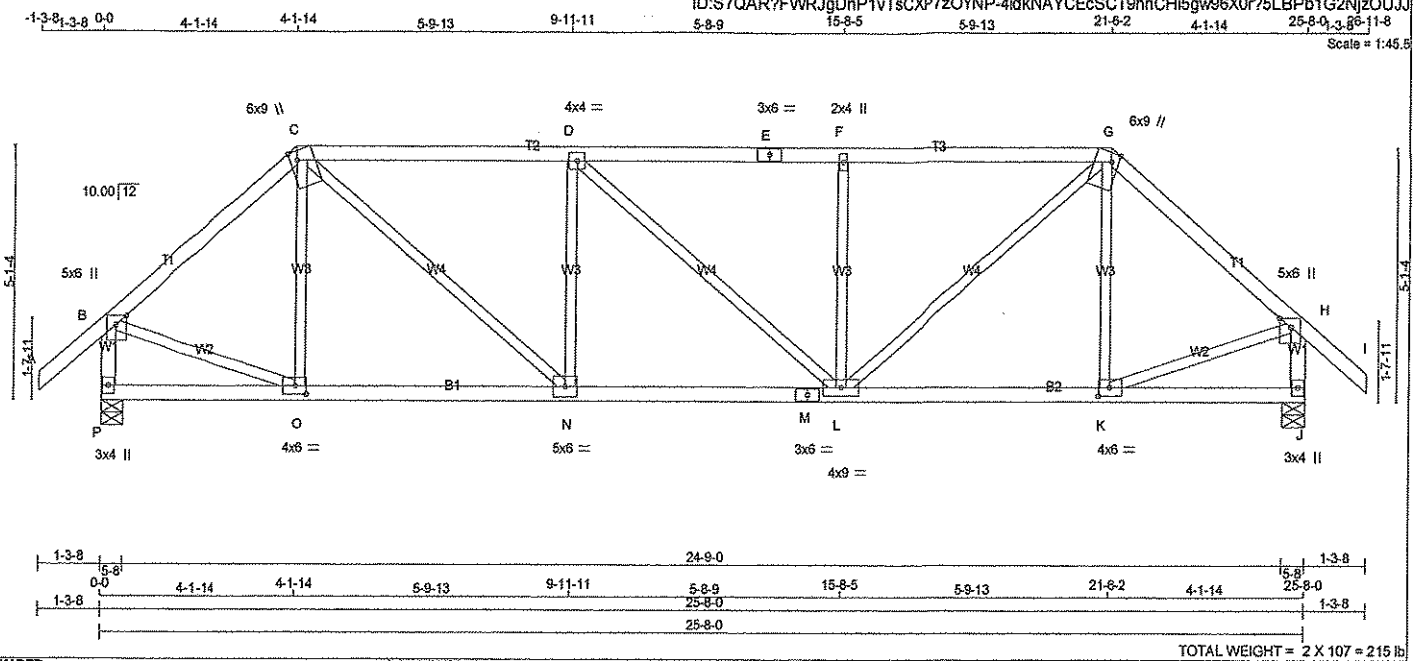

TOTAL WEIGHT = 2 X 107 = 215 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	2097	0	2097	0	0	5-8	3-5
J	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	
B- C	-1859 / 0	-122.2	-122.2	0.45 (1)	
C- D	-2538 / 0	-122.2	-122.2	0.75 (1)	
D- E	-2536 / 0	-122.2	-122.2	0.74 (1)	
E- F	-2536 / 0	-122.2	-122.2	0.74 (1)	
F- G	-2536 / 0	-122.2	-122.2	0.74 (1)	
G- H	-1859 / 0	-122.2	-122.2	0.45 (1)	
H- I	0 / 54	-122.2	-122.2	0.17 (1)	
P- B	-2054 / 0	0.0	0.0	0.22 (1)	
J- H	-2054 / 0	0.0	0.0	0.22 (1)	
P- O	0 / 0	-28.0	-28.0	0.17 (2)	
O- N	0 / 1418	-28.0	-28.0	0.32 (1)	
N- M	0 / 2538	-28.0	-28.0	0.50 (1)	
M- L	0 / 2538	-28.0	-28.0	0.50 (1)	
L- K	0 / 1418	-28.0	-28.0	0.32 (1)	
K- J	0 / 0	-28.0	-28.0	0.17 (2)	

MEMB. FR-TO	MAX. UNBRAC LENGTH
A- B	10.00
B- C	4.42
C- D	3.48
D- E	3.48
E- F	3.48
F- G	3.49
G- H	4.42
H- I	10.00
P- B	5.90
J- H	5.90
P- O	10.00
O- N	10.00
N- M	10.00
M- L	10.00
L- K	10.00
K- J	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-279 / 102	0.11 (1)
C- N	0 / 1450	0.33 (1)
N- D	-762 / 0	0.29 (1)
D- L	-2 / 0	0.00 (1)
L- F	-761 / 0	0.29 (1)
L- G	0 / 1448	0.33 (1)
K- G	-278 / 102	0.11 (1)
B- O	0 / 1489	0.34 (1)
K- H	0 / 1490	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.75 (C-D:1) , BC=0.50 (L-N:1) , WB=0.34 (H-K:1) , SSI=0.33 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 1.00)

DWG NO. TAM17642 16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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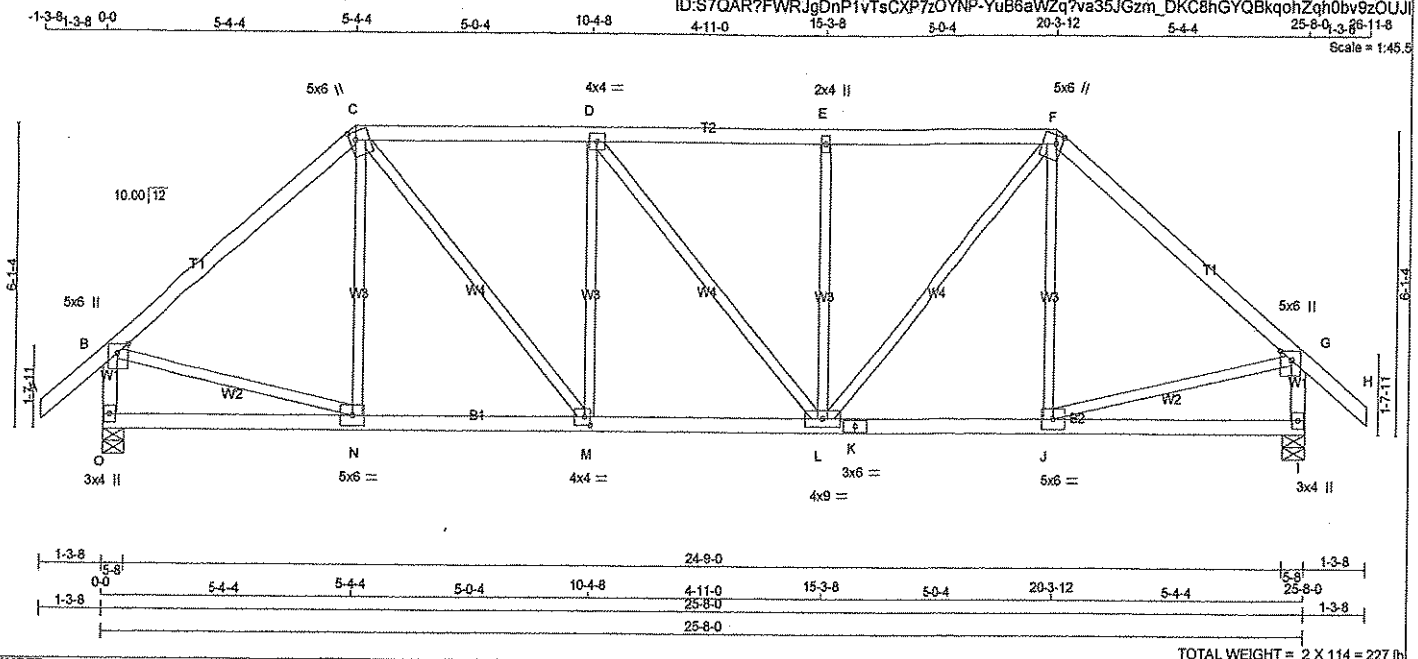

TOTAL WEIGHT = 2 X 114 = 227 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2097	0	2097	0	0	5-8	3-5
I	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
I	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)
B- C	-1864 / 0	-122.2	-122.2	0.80 (1)
C- D	-2108 / 0	-122.2	-122.2	0.50 (1)
D- E	-2106 / 0	-122.2	-122.2	0.50 (1)
E- F	-2105 / 0	-122.2	-122.2	0.50 (1)
F- G	-1864 / 0	-122.2	-122.2	0.80 (1)
G- H	0 / 54	-122.2	-122.2	0.17 (1)
O- B	-2035 / 0	0.0	0.0	0.22 (1)
I- G	-2035 / 0	0.0	0.0	0.22 (1)
O- N	0 / 0	-28.0	-28.0	0.19 (3)
N- M	0 / 1426	-28.0	-28.0	0.34 (2)
M- L	0 / 2108	-28.0	-28.0	0.40 (1)
L- K	0 / 1426	-28.0	-28.0	0.34 (2)
K- J	0 / 1426	-28.0	-28.0	0.34 (2)
J- I	0 / 0	-28.0	-28.0	0.19 (3)

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	MAX CSI (LC)
N- C	-175 / 159	10.00	0.10 (1)
C- M	0 / 1041	3.91	0.23 (1)
M- D	-655 / 0	4.11	0.38 (1)
D- L	-3 / 0	4.11	0.00 (1)
L- E	-654 / 0	4.13	0.38 (1)
L- F	0 / 1037	3.91	0.23 (1)
J- F	-173 / 159	10.00	0.10 (1)
B- N	0 / 1469	5.92	0.33 (1)
J- G	0 / 1470	5.92	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.80 (F-G:1) , BC=0.40 (L-M:1) , WB=0.38 (D-M:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.38 (G) (INPUT = 1.00)

DWG NO. TAM17849-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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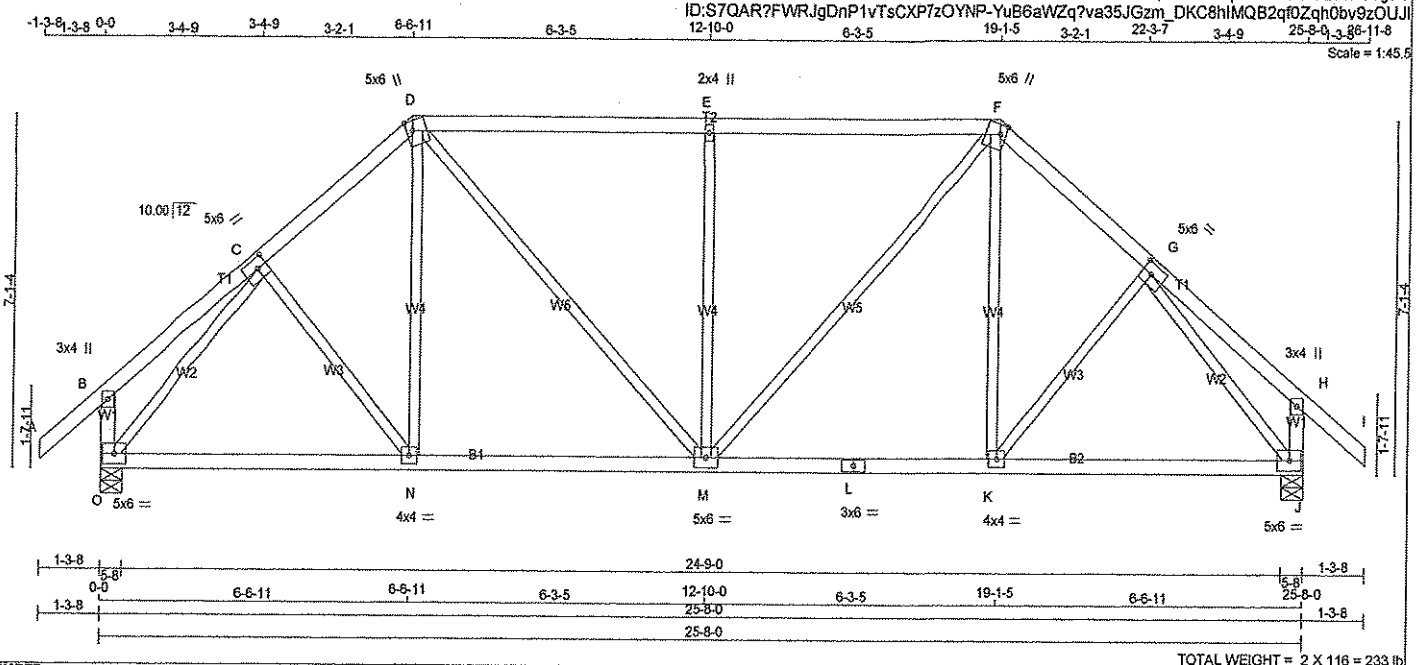

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2097	0	2097	0	0	5-8	2-4
J	2097	0	2097	0	0	5-8	2-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 27	-122.2	-122.2	0.19 (1)	10.00
C- D	-1859 / 0	-122.2	-122.2	0.19 (1)	4.77
D- E	-1910 / 0	-122.2	-122.2	0.68 (1)	3.96
E- F	-1910 / 0	-122.2	-122.2	0.68 (1)	3.96
F- G	-1859 / 0	-122.2	-122.2	0.19 (1)	4.77
G- H	0 / 27	-122.2	-122.2	0.19 (1)	10.00
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00
O- B	-325 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-325 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1374	-28.0	-28.0	0.43 (2)	10.00
N- M	0 / 1405	-28.0	-28.0	0.44 (2)	10.00
M- L	0 / 1405	-28.0	-28.0	0.44 (2)	10.00
L- K	0 / 1405	-28.0	-28.0	0.44 (2)	10.00
K- J	0 / 1374	-28.0	-28.0	0.43 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 124	0.03 (3)
N- D	0 / 276	0.06 (2)
D- M	0 / 744	0.17 (1)
M- E	-940 / 0	0.82 (1)
M- F	0 / 744	0.17 (1)
K- F	0 / 276	0.06 (2)
K- G	0 / 124	0.03 (3)
O- C	-2183 / 0	0.94 (1)
G- J	-2183 / 0	0.94 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68 (D-E:1) , BC=0.44 (M-N:2) , WB=0.94 (C-O:1) , SSI=0.37 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.54 (G) (INPUT = 1.00)

DWG NO. TAM17850-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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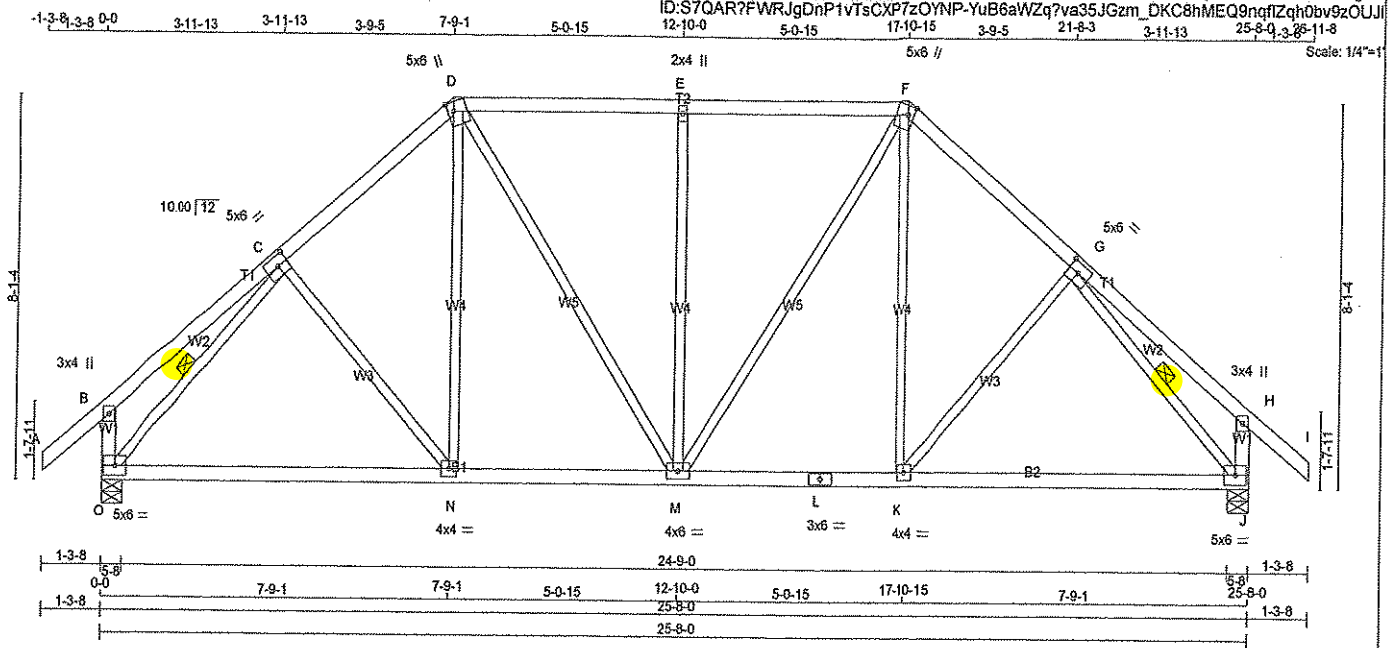

TOTAL WEIGHT = 2 X 122 = 245 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2097	0	2097	0	0	5-8	2-4
J	2097	0	2097	0	0	5-8	2-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 34	-122.2	-122.2	0.29 (1)	10.00
C- D	-1805 / 0	-122.2	-122.2	0.26 (1)	4.74
D- E	-1631 / 0	-122.2	-122.2	0.43 (1)	4.69
E- F	-1631 / 0	-122.2	-122.2	0.43 (1)	4.69
F- G	-1805 / 0	-122.2	-122.2	0.26 (1)	4.74
G- H	0 / 34	-122.2	-122.2	0.29 (1)	10.00
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00
O- B	-351 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-351 / 0	0.0	0.0	0.04 (1)	7.81
O- N	0 / 1413	-28.0	-28.0	0.52 (2)	10.00
N- M	0 / 1361	-28.0	-28.0	0.52 (2)	10.00
M- L	0 / 1361	-28.0	-28.0	0.52 (2)	10.00
L- K	0 / 1361	-28.0	-28.0	0.52 (2)	10.00
K- J	0 / 1413	-28.0	-28.0	0.52 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-84 / 97	0.05 (1)
N- D	0 / 367	0.08 (2)
D- M	0 / 496	0.11 (1)
M- E	-755 / 0	0.96 (1)
M- F	0 / 496	0.11 (1)
K- F	0 / 367	0.08 (2)
K- G	-84 / 97	0.05 (1)
O- C	-2176 / 0	0.55 (1)
G- J	-2176 / 0	0.55 (1)

LICENSED PROFESSIONAL ENGINEER
4/21/16
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA O86-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.43 (D-E:1) , BC=0.52 (M-N:2) , WB=0.96 (E-M:1) , SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.54 (G) (INPUT = 1.00)

DWG NO. TAM 17851-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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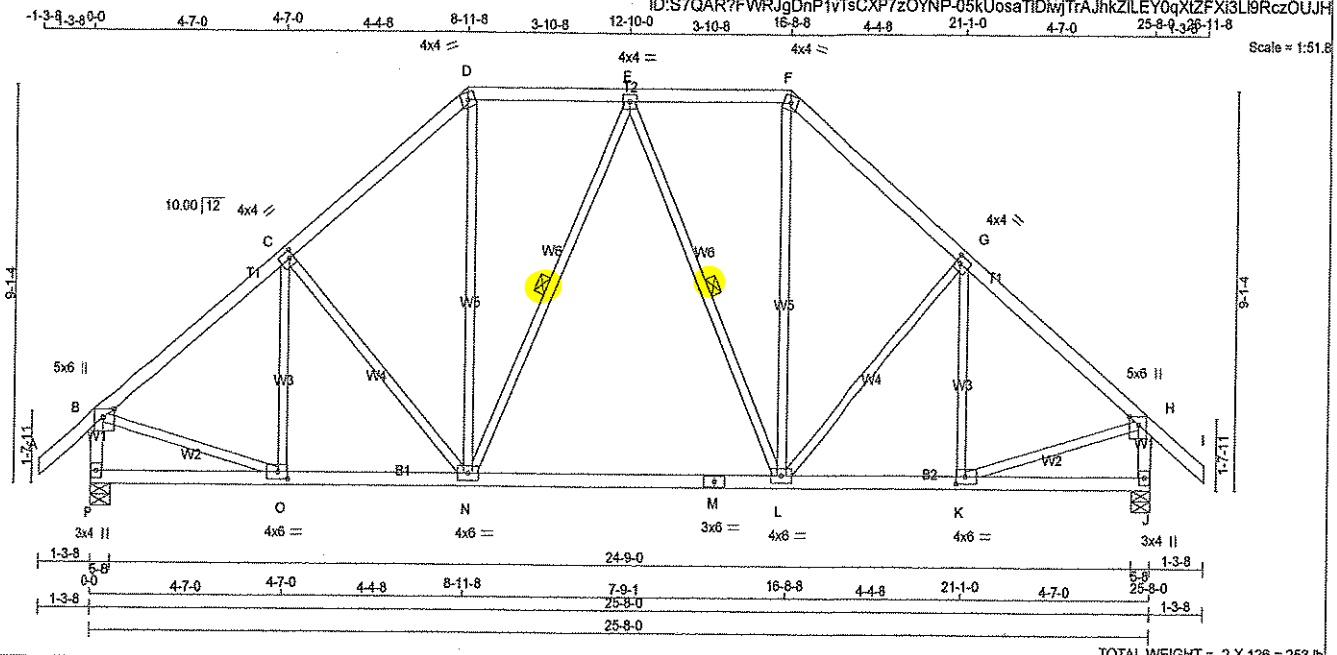

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	2097	0	2097	0	0	IN-SX	IN-SX
J	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	
B- C	-1884 / 0	-122.2	-122.2	0.37 (1)	
C- D	-1722 / 0	-122.2	-122.2	0.35 (1)	
D- E	-1295 / 0	-122.2	-122.2	0.24 (1)	
E- F	-1295 / 0	-122.2	-122.2	0.24 (1)	
F- G	-1722 / 0	-122.2	-122.2	0.35 (1)	
G- H	-1884 / 0	-122.2	-122.2	0.37 (1)	
H- I	0 / 54	-122.2	-122.2	0.17 (1)	
P- B	-2039 / 0	0.0	0.0	0.22 (1)	
J- H	-2039 / 0	0.0	0.0	0.22 (1)	
P- O	0 / 0	-28.0	-28.0	0.13 (2)	
O- N	0 / 1479	-28.0	-28.0	0.41 (2)	
N- M	0 / 1417	-28.0	-28.0	0.40 (2)	
M- L	0 / 1417	-28.0	-28.0	0.40 (2)	
L- K	0 / 1479	-28.0	-28.0	0.41 (2)	
K- J	0 / 0	-28.0	-28.0	0.13 (2)	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- C	-305 / 55	10.00	O- C	-305 / 55	0.14 (1)
C- N	-291 / 0	4.53	C- N	-291 / 0	0.25 (1)
N- D	0 / 688	4.71	N- D	0 / 688	0.15 (1)
N- E	-308 / 0	5.41	N- E	-308 / 0	0.20 (1)
E- L	-308 / 0	5.41	E- L	-308 / 0	0.20 (1)
L- F	0 / 688	4.71	L- F	0 / 688	0.15 (1)
L- G	-291 / 0	4.53	L- G	-291 / 0	0.25 (1)
K- G	-305 / 55	10.00	K- G	-305 / 55	0.14 (1)
B- O	0 / 1538	5.92	B- O	0 / 1538	0.35 (1)
K- H	0 / 1538	5.92	K- H	0 / 1538	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 64.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.37 (B-C:1) , BC=0.41 (N-O:2) , WB=0.35 (B-O:1) , SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)

DWG NO. TAM17852-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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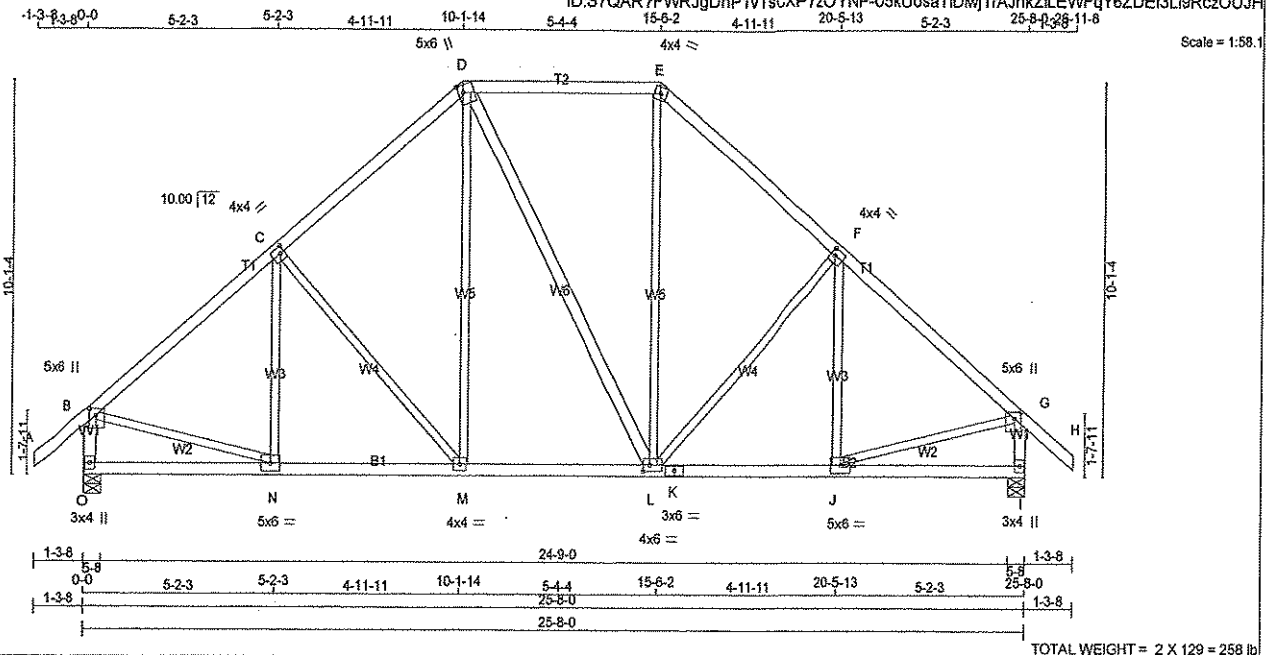

TOTAL WEIGHT = 2 X 129 = 258 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.00	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0	2.00	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2097	0	2097	0	0	5-8	3-5
I	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
I	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	N- C	-219 / 142	0.12 (1)
B- C	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38	C- M	-428 / 0	0.49 (1)
C- D	-1627 / 0	-122.2	-122.2	0.45 (1)	4.68	M- D	0 / 457	0.10 (1)
D- E	-1215 / 0	-122.2	-122.2	0.47 (1)	5.19	D- L	0 / 2	0.00 (1)
E- F	-1628 / 0	-122.2	-122.2	0.45 (1)	4.68	L- E	0 / 460	0.10 (1)
F- G	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38	L- F	-426 / 0	0.49 (1)
G- H	0 / 54	-122.2	-122.2	0.17 (1)	10.00	J- F	-221 / 140	0.12 (1)
O- B	-2037 / 0	0.0	0.0	0.22 (1)	5.92	B- N	0 / 1543	0.35 (1)
I- G	-2036 / 0	0.0	0.0	0.22 (1)	5.92	J- G	0 / 1543	0.35 (1)
O- N	0 / 0	-28.0	-28.0	0.18 (3)	10.00			
N- M	0 / 1497	-28.0	-28.0	0.33 (1)	10.00			
M- L	0 / 1215	-28.0	-28.0	0.28 (1)	10.00			
L- K	0 / 1496	-28.0	-28.0	0.32 (1)	10.00			
K- J	0 / 1496	-28.0	-28.0	0.32 (1)	10.00			
J- I	0 / 0	-28.0	-28.0	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.48 (B-C:1) , BC=0.33 (M-N:1) , WB=0.49 (C-M:1) , SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

DWG NO. TAW 17853 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T8	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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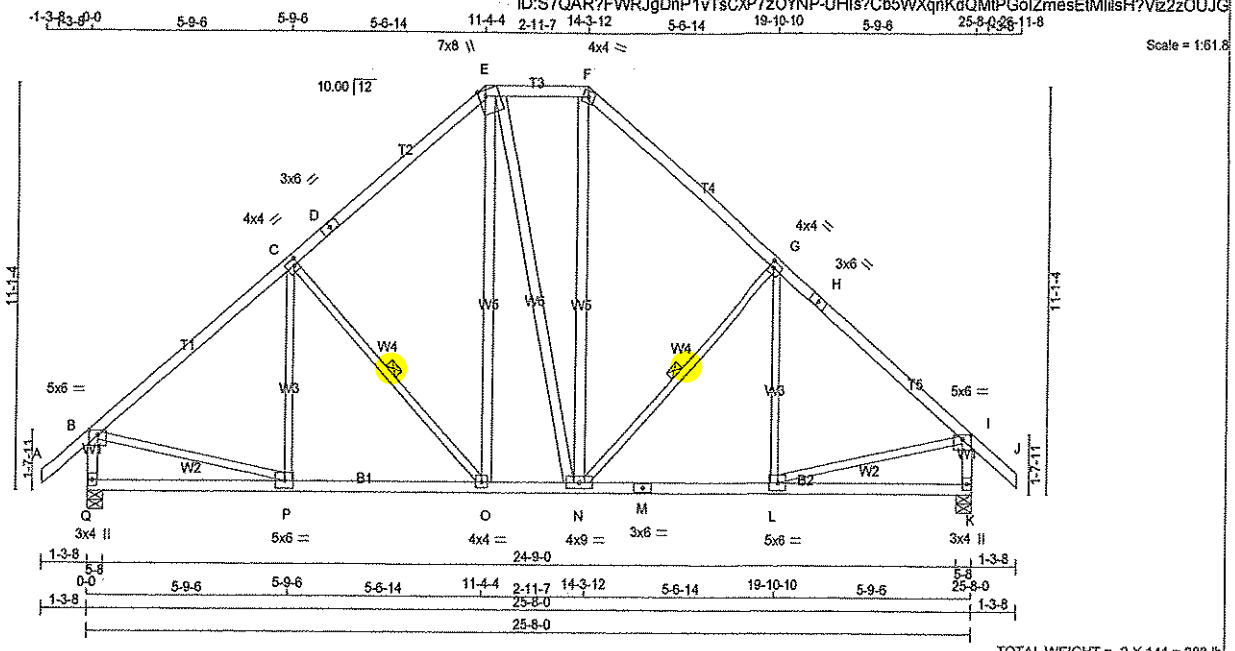

TOTAL WEIGHT = 2 X 141 = 283 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TS-t	MT20	3.0	6.0		
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2097	0	2097	0	0	5-8	3-5
K	2097	0	2097	0	0	5-8	3-5

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
K	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	-1904 / 0	-122.2	-122.2	0.61 (1)	4.17	
C- D	-1527 / 0	-122.2	-122.2	0.56 (1)	4.62	
D- E	-1527 / 0	-122.2	-122.2	0.56 (1)	4.62	
E- F	-1136 / 0	-122.2	-122.2	0.15 (1)	5.80	
F- G	-1529 / 0	-122.2	-122.2	0.56 (1)	4.62	
G- H	-1903 / 0	-122.2	-122.2	0.61 (1)	4.17	
H- I	-1903 / 0	-122.2	-122.2	0.61 (1)	4.17	
I- J	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
Q- B	-2032 / 0	0.0	0.0	0.22 (1)	5.93	
K- I	-2031 / 0	0.0	0.0	0.22 (1)	5.93	
Q- P	0 / 0	-28.0	-28.0	0.24 (3)	10.00	
P- O	0 / 1504	-28.0	-28.0	0.39 (2)	10.00	
O- N	0 / 1135	-28.0	-28.0	0.25 (1)	10.00	
N- M	0 / 1503	-28.0	-28.0	0.39 (2)	10.00	
M- L	0 / 1503	-28.0	-28.0	0.39 (2)	10.00	
L- K	0 / 0	-28.0	-28.0	0.24 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
P- C	-153 / 204	0.10 (1)
C- O	-550 / 0	0.26 (1)
O- E	0 / 505	0.08 (1)
E- N	0 / 5	0.00 (1)
N- F	0 / 511	0.08 (1)
N- G	-547 / 0	0.26 (1)
L- G	-157 / 201	0.11 (1)
B- P	0 / 1542	0.35 (1)
L- I	0 / 1541	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.61 (B-C:1) , BC=0.39 (O-P:2) , WB=0.35 (B-P:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.45 (M) (INPUT = 1.00)

DWG NO. TAM 17854-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T9	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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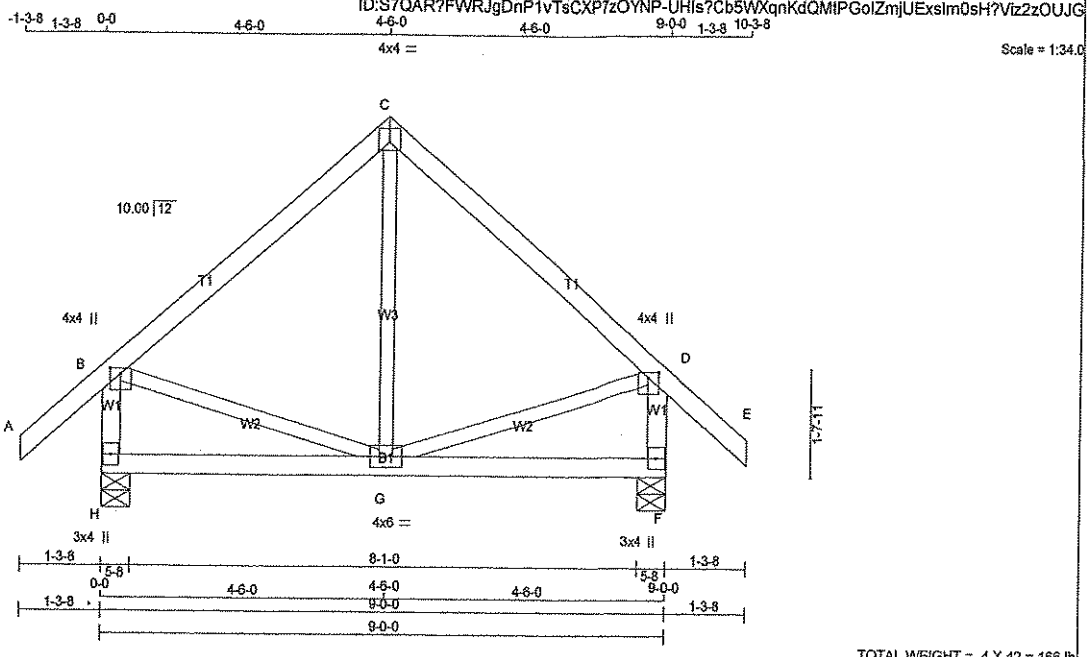

TOTAL WEIGHT = 4 X 42 = 166 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	845	0	845	0	0	5-8	1-8
F	845	0	845	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	643	450 / 0	95 / 0	0 / 0	0 / 0	98 / 0	0 / 0
F	643	450 / 0	95 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-408 / 0	-122.2	-122.2	0.32 (1)	6.25
C- D	-408 / 0	-122.2	-122.2	0.32 (1)	6.25
D- E	0 / 54	-122.2	-122.2	0.17 (1)	10.00
H- B	-797 / 0	0.0	0.0	0.08 (1)	7.81
F- D	-797 / 0	0.0	0.0	0.08 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-27 / 195	0.04 (3)
B- G	0 / 327	0.07 (1)
G- D	0 / 327	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.32 (C-D:1) , BC=0.17 (G-H:3) , WB=0.07 (B-G:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO . YAM 17855 - 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
267011	G9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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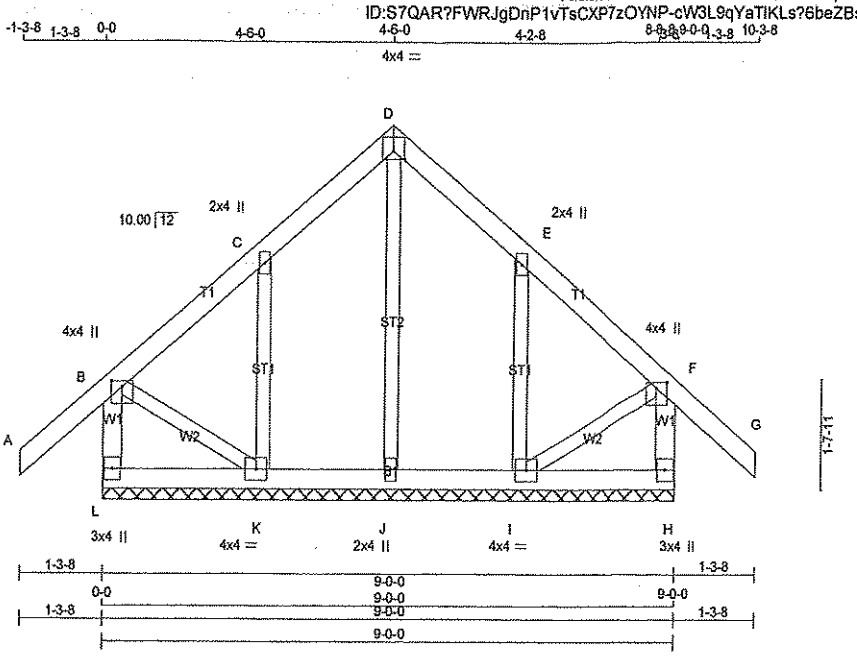

TOTAL WEIGHT = 44 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
L- B	-319 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-12 / 0	-122.2	-122.2	0.10 (1)	6.25
C- D	-44 / 0	-122.2	-122.2	0.10 (1)	6.25
D- E	-44 / 0	-122.2	-122.2	0.10 (1)	6.25
E- F	-12 / 0	-122.2	-122.2	0.10 (1)	6.25
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00
H- F	-319 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 0	-28.0	-28.0	0.04 (3)	10.00
K- J	0 / 16	-28.0	-28.0	0.04 (2)	10.00
J- I	0 / 16	-28.0	-28.0	0.04 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.04 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-157 / 0	0.07 (1)
K- C	-335 / 0	0.07 (1)
I- E	-335 / 0	0.07 (1)
B- K	0 / 28	0.01 (1)
I- F	0 / 28	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (F-G:1) , BC=0.04 (I-J:2) , WB=0.07 (E-I:1) , SSI=0.11 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
21-16
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/7856 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42687	DRWG NO.
267011	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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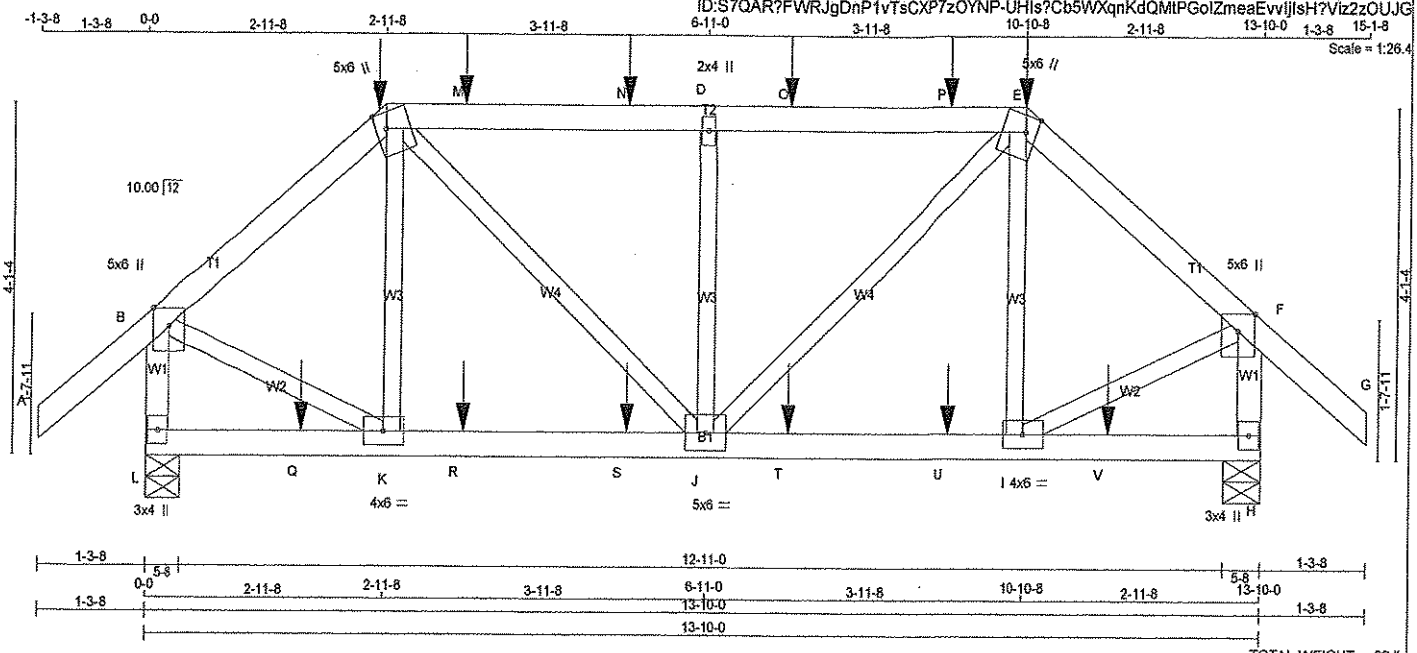

TOTAL WEIGHT = 62 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 227.6 lbs FACTORED DOWN AT 10-10-8, 148.2 lbs FACTORED DOWN AT 3-11-4, 147.1 lbs FACTORED DOWN AT 5-11-4, AND 147.1 lbs FACTORED DOWN AT 7-11-4, AND 150.8 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, AND 69.9 lbs FACTORED DOWN AT 9-11-4, AND 69.9 lbs FACTORED DOWN AT 11-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1851	0	1851	0	0	5-8	2-8
H	1854	0	1854	0	0	5-8	2-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1453	935 / 0	267 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1456	937 / 0	267 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.19 (1)	10.00
B- C	-1508 / 0	-122.2	-122.2	0.24 (1)	5.05
C- M	-1732 / 0	-122.2	-122.2	0.63 (1)	4.09
M- N	-1732 / 0	-122.2	-122.2	0.63 (1)	4.09
N- D	-1732 / 0	-122.2	-122.2	0.63 (1)	4.09
D- O	-1732 / 0	-122.2	-122.2	0.63 (1)	4.09
O- P	-1732 / 0	-122.2	-122.2	0.63 (1)	4.09
P- E	-1732 / 0	-122.2	-122.2	0.63 (1)	4.09
E- F	-1511 / 0	-122.2	-122.2	0.24 (1)	5.05
F- G	0 / 54	-122.2	-122.2	0.19 (1)	10.00
L- B	-1812 / 0	0.0	0.0	0.21 (1)	6.16
H- F	-1815 / 0	0.0	0.0	0.21 (1)	6.16
L- Q	0 / 0	-28.0	-28.0	0.15 (2)	10.00
Q- K	0 / 0	-28.0	-28.0	0.15 (2)	10.00
K- R	0 / 1146	-28.0	-28.0	0.29 (1)	10.00
R- S	0 / 1146	-28.0	-28.0	0.29 (1)	10.00
S- J	0 / 1146	-28.0	-28.0	0.29 (1)	10.00
J- T	0 / 1148	-28.0	-28.0	0.29 (1)	10.00
T- U	0 / 1148	-28.0	-28.0	0.29 (1)	10.00
U- I	0 / 1148	-28.0	-28.0	0.29 (1)	10.00
I- V	0 / 0	-28.0	-28.0	0.15 (2)	10.00
V- H	0 / 0	-28.0	-28.0	0.15 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-305 / 106	0.08 (1)
C- J	0 / 813	0.20 (1)
J- D	-946 / 0	0.25 (1)
J- E	0 / 809	0.20 (1)
I- E	-306 / 107	0.08 (1)
B- K	0 / 1254	0.31 (1)
I- F	0 / 1257	0.31 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-228	-228	---	FRONT	VERT	TOTAL
E	10-10-8	-228	-228	---	FRONT	VERT	TOTAL
M	3-11-4	-148	-148	---	FRONT	VERT	TOTAL
N	5-11-4	-147	-147	---	FRONT	VERT	TOTAL
O	7-11-4	-147	-147	---	FRONT	VERT	TOTAL
P	9-11-4	-151	-151	---	FRONT	VERT	TOTAL
Q	1-11-4	-40	-70	---	FRONT	VERT	TOTAL
R	3-11-4	-40	-70	---	FRONT	VERT	TOTAL
S	5-11-4	-40	-70	---	FRONT	VERT	TOTAL
T	7-11-4	-40	-70	---	FRONT	VERT	TOTAL
U	9-11-4	-40	-70	---	FRONT	VERT	TOTAL
V	11-11-4	-40	-70	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 42126 S. KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.63 (D-E:1) , BC=0.29 (I-J:1) , WB=0.31 (F-I:1) , SSI=0.42 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (E) (INPUT = 0.90)
JSI METAL= 0.29 (F) (INPUT = 1.00)

DWG NO. TAM 1705216

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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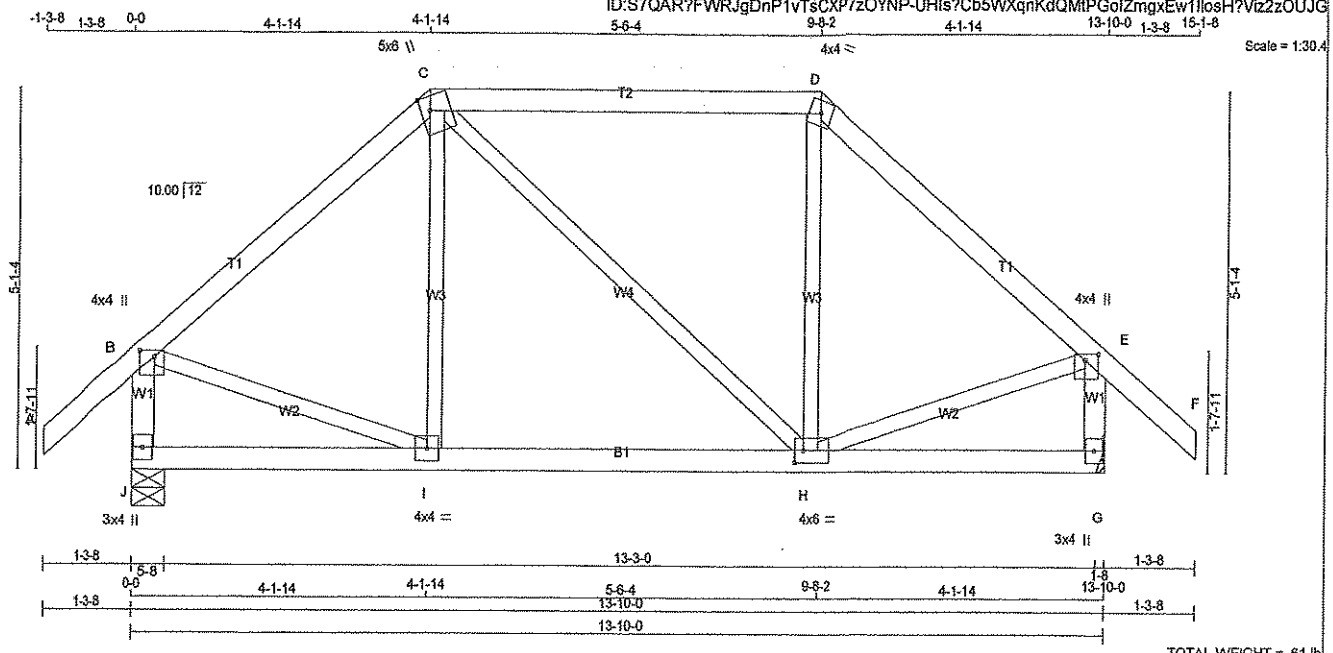

TOTAL WEIGHT = 61 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1208	0	1208	0	0	5-8	1-8
G	1208	0	1208	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	927	635 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0
G	927	635 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	I- C	-50 / 181	0.04 (3)
B- C	-843 / 0	-122.2	-122.2	0.28 (1)	6.25	C- H	0 / 0	0.00 (1)
C- D	-645 / 0	-122.2	-122.2	0.48 (1)	6.25	H- D	-50 / 181	0.04 (3)
D- E	-843 / 0	-122.2	-122.2	0.28 (1)	6.25	B- I	0 / 675	0.15 (1)
E- F	0 / 54	-122.2	-122.2	0.17 (1)	10.00	H- E	0 / 675	0.15 (1)
J- B	-1163 / 0	0.0	0.0	0.12 (1)	7.39			
G- E	-1163 / 0	0.0	0.0	0.12 (1)	7.39			
J- I	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
I- H	0 / 645	-28.0	-28.0	0.22 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.48 (C-D:1) , BC=0.22 (H-I:2) , WB=0.15 (B-I:1) , SSI=0.26 (C-D:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

DWG NO. TAM 17858-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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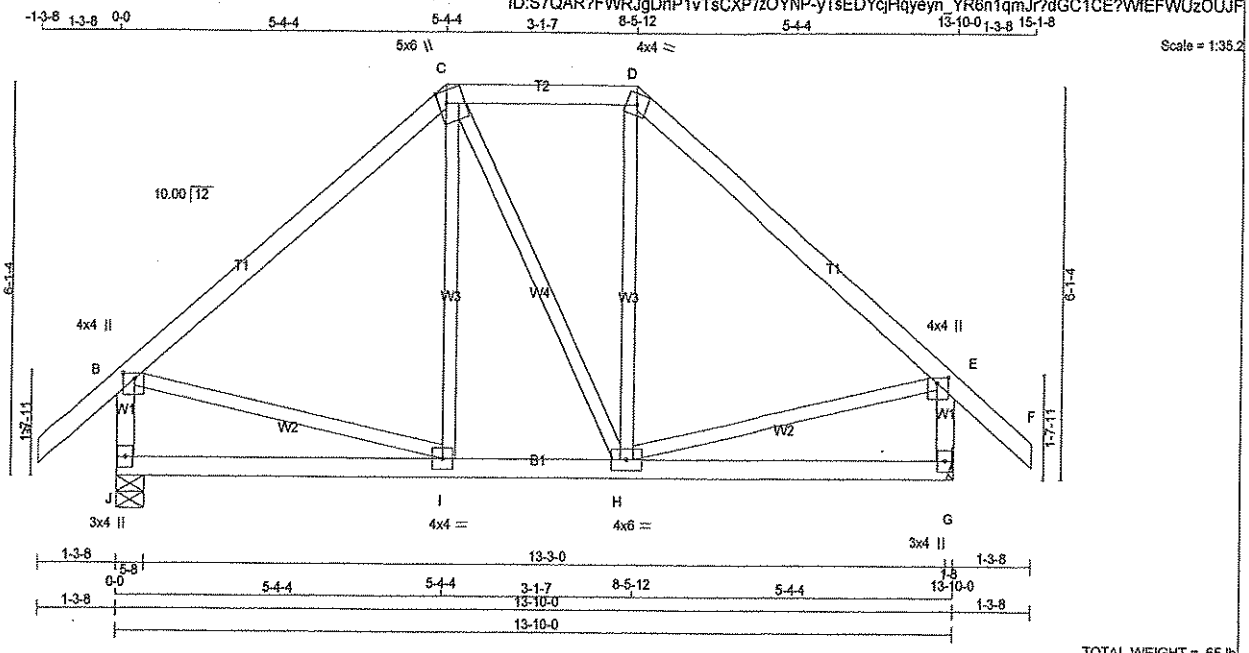

TOTAL WEIGHT = 65 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1208	0	1208	0	0	5-8	1-8
G	1208	0	1208	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	927	635 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0
G	927	635 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00		I- C	-17 / 185	0.04 (3)
B- C	-783 / 0	-122.2	-122.2	0.46 (1)	6.14		C- H	0 / 4	0.00 (3)
C- D	-601 / 0	-122.2	-122.2	0.16 (1)	6.25		H- D	-14 / 169	0.04 (3)
D- E	-784 / 0	-122.2	-122.2	0.46 (1)	6.14		B- I	0 / 617	0.14 (1)
E- F	0 / 54	-122.2	-122.2	0.17 (1)	10.00		H- E	0 / 618	0.14 (1)
J- B	-1143 / 0	0.0	0.0	0.12 (1)	7.43				
G- E	-1142 / 0	0.0	0.0	0.12 (1)	7.44				
J- I	0 / 0	-28.0	-28.0	0.18 (2)	10.00				
I- H	0 / 600	-28.0	-28.0	0.22 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.18 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.46 (D-E:1) , BC=0.22 (H-I:2) , WB=0.14 (E-H:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)

DWG NO. TAM 170591G
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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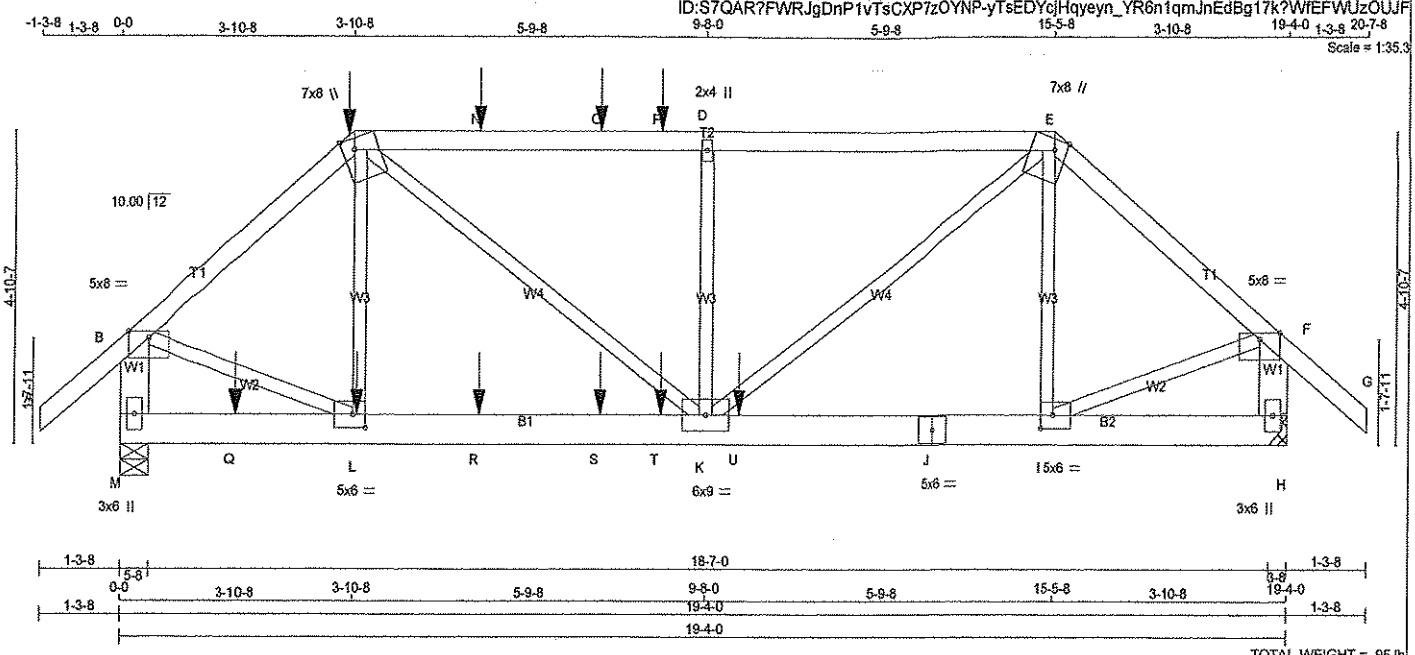


[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - G	2x4	DRY	2100F 1.8E	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 118.1 lbs FACTORED DOWN AT 3-10-8, 114.6 lbs FACTORED DOWN AT 5-11-4, AND 114.6 lbs FACTORED DOWN AT 7-11-4, AND 114.6 lbs FACTORED DOWN AT 8-11-4 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, 42.0 lbs FACTORED DOWN AT 3-11-4, 42.0 lbs FACTORED DOWN AT 5-11-4, 42.0 lbs FACTORED DOWN AT 7-11-4, AND 42.0 lbs FACTORED DOWN AT 8-11-4, AND 1135.0 lbs FACTORED DOWN AT 10-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	2544	0	2544	0	0	5-8	2-12
H	2415	0	2415	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-10	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1957	1329 / 0	313 / 0	0 / 0	0 / 0	315 / 0	0 / 0
H	1846	1275 / 0	282 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

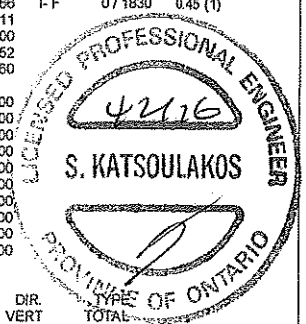
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.12 (1)	10.00		L- C	-372 / 169	0.13 (1)
B- C	-2375 / 0	-122.2	-122.2	0.28 (1)	5.04		C- K	0 / 1898	0.47 (1)
C- N	-3313 / 0	-122.2	-122.2	0.77 (1)	3.66		K- D	-1114 / 0	0.38 (1)
N- O	-3313 / 0	-122.2	-122.2	0.77 (1)	3.66		K- E	0 / 1984	0.49 (1)
O- P	-3313 / 0	-122.2	-122.2	0.77 (1)	3.66		I- E	-243 / 99	0.08 (1)
P- D	-3313 / 0	-122.2	-122.2	0.77 (1)	3.66		B- L	0 / 1904	0.47 (1)
D- E	-3313 / 0	-122.2	-122.2	0.77 (1)	3.66		I- F	0 / 1830	0.45 (1)
E- F	-2283 / 0	-122.2	-122.2	0.27 (1)	5.11				
F- G	0 / 54	-122.2	-122.2	0.12 (1)	10.00				
M- B	-2491 / 0	0.0	0.0	0.18 (1)	6.52				
H- F	-2410 / 0	0.0	0.0	0.18 (1)	6.60				
M- Q	0 / 0	-28.0	-28.0	0.11 (3)	10.00				
Q- L	0 / 0	-28.0	-28.0	0.11 (3)	10.00				
L- R	0 / 1811	-28.0	-28.0	0.39 (1)	10.00				
R- S	0 / 1811	-28.0	-28.0	0.39 (1)	10.00				
S- T	0 / 1811	-28.0	-28.0	0.39 (1)	10.00				
T- K	0 / 1811	-28.0	-28.0	0.39 (1)	10.00				
K- U	0 / 1743	-28.0	-28.0	0.51 (1)	10.00				
U- J	0 / 1743	-28.0	-28.0	0.51 (1)	10.00				
J- I	0 / 1743	-28.0	-28.0	0.51 (1)	10.00				
I- H	0 / 0	-28.0	-28.0	0.15 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-118	-118	---	BACK	VERT	TOTAL
L	3-11-4	-24	-42	---	BACK	VERT	TOTAL
N	5-11-4	-115	-115	---	BACK	VERT	TOTAL
O	7-11-4	-115	-115	---	BACK	VERT	TOTAL
P	8-11-4	-115	-115	---	BACK	VERT	TOTAL
Q	1-11-4	-24	-42	---	BACK	VERT	TOTAL
R	5-11-4	-24	-42	---	BACK	VERT	TOTAL
S	7-11-4	-24	-42	---	BACK	VERT	TOTAL
T	8-11-4	-24	-42	---	BACK	VERT	TOTAL
U	10-2-8	-1135	-1135	---	BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.77 (D-E:1) , BC=0.51 (I-K:1) , WB=0.49 (E-K:1) , SSI=0.73 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.45 (L) (INPUT = 1.00)

DWG NO. TAM17860-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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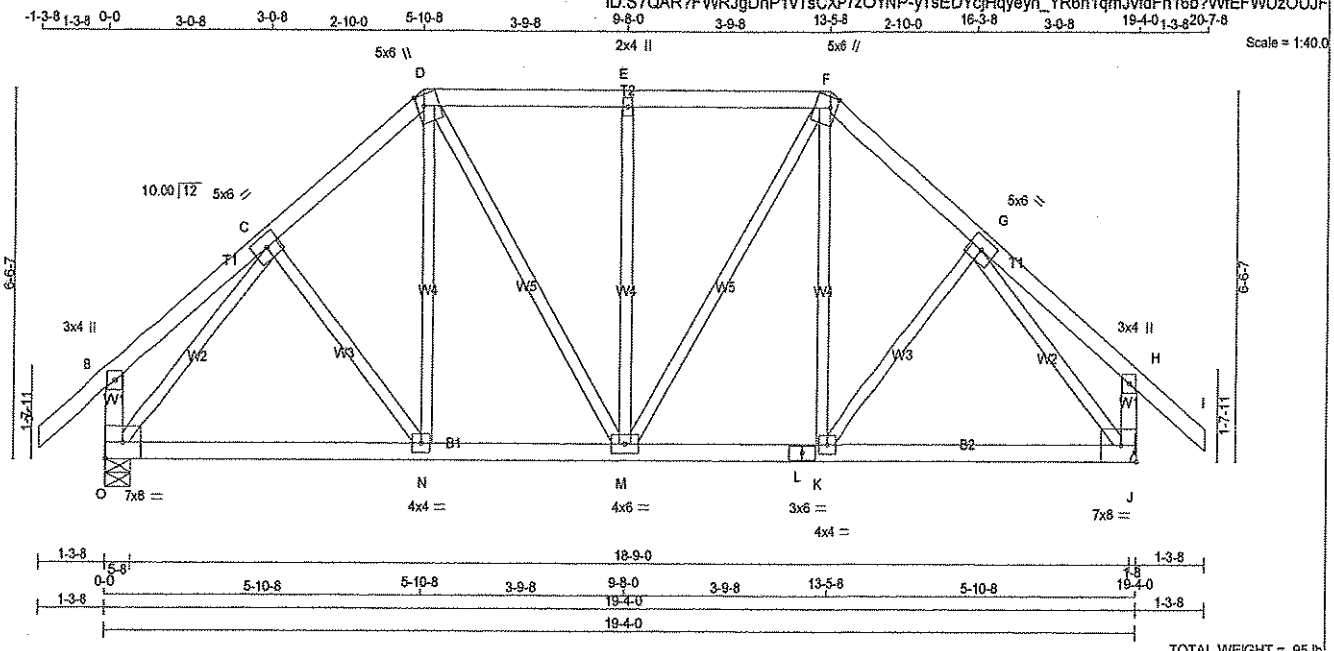

TOTAL WEIGHT = 95 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	7.0	8.0		Edge
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	7.0	8.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1621	0	1621	0	0	5-8	1-12
J	1621	0	1621	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1250	845 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0
J	1250	845 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00		C- N	-5 / 93	0.02 (3)
B- C	0 / 24	-122.2	-122.2	0.15 (1)	10.00		N- D	0 / 250	0.06 (2)
C- D	-1287 / 0	-122.2	-122.2	0.14 (1)	5.55		D- M	0 / 357	0.08 (1)
D- E	-1155 / 0	-122.2	-122.2	0.23 (1)	5.66		M- E	-557 / 0	0.39 (1)
E- F	-1155 / 0	-122.2	-122.2	0.23 (1)	5.66		M- F	0 / 357	0.08 (1)
F- G	-1287 / 0	-122.2	-122.2	0.14 (1)	5.55		K- F	0 / 250	0.06 (2)
G- H	0 / 24	-122.2	-122.2	0.15 (1)	10.00		K- G	-5 / 93	0.02 (3)
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00		O- C	-1577 / 0	0.56 (1)
O- B	-309 / 0	0.0	0.0	0.03 (1)	7.81		G- J	-1577 / 0	0.56 (1)
J- H	-309 / 0	0.0	0.0	0.03 (1)	7.81				
O- N	0 / 970	-28.0	-28.0	0.31 (2)	10.00				
N- M	0 / 970	-28.0	-28.0	0.32 (2)	10.00				
M- L	0 / 970	-28.0	-28.0	0.32 (2)	10.00				
L- K	0 / 970	-28.0	-28.0	0.32 (2)	10.00				
K- J	0 / 970	-28.0	-28.0	0.31 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.23 (D-E:1) , BC=0.32 (M-N:2) , WB=0.56 (G-J:1) , SSI=0.22 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.42 (L) (INPUT = 1.00)

DWG NO. YAM17861-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42057	DRWG NO.
267011	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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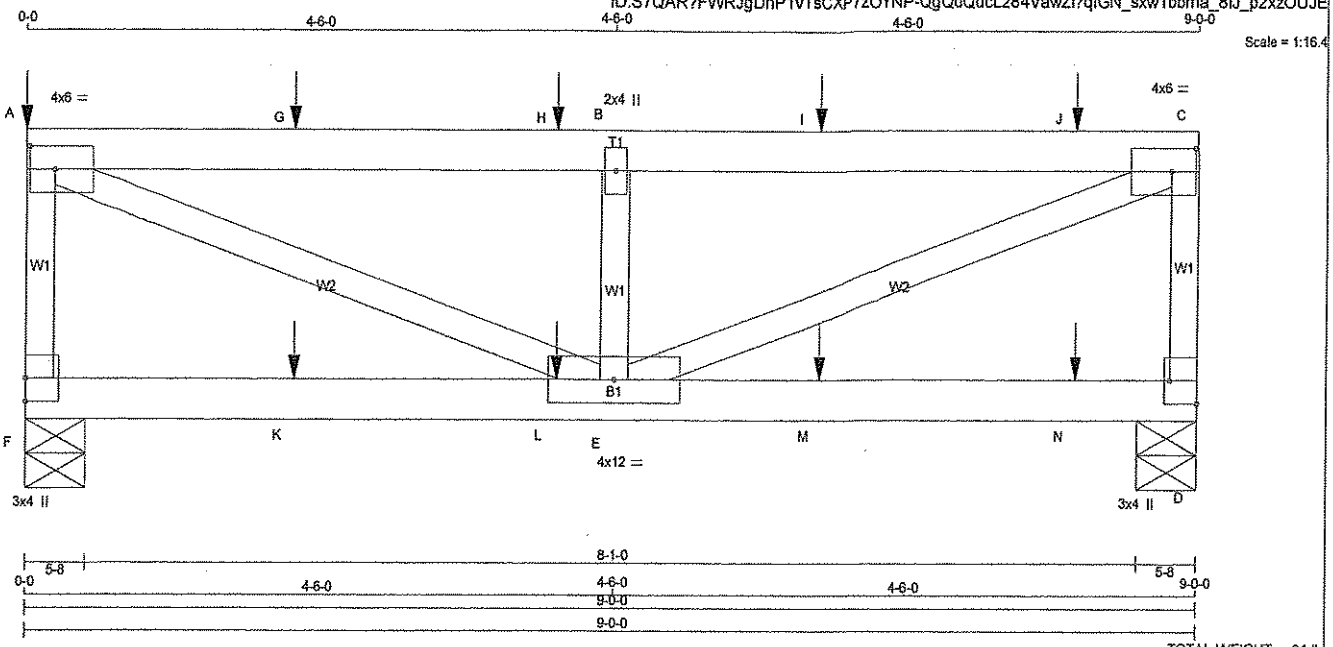

TOTAL WEIGHT = 31 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x3	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x3	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	2.00	2.25
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	2.25
D	BMV1+p	MT20	3.0	4.0	2.00	Edge
E	BMWWW-t	MT20	4.0	12.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 218.1 lbs FACTORED DOWN AT 0-0, 160.8 lbs FACTORED DOWN AT 2-0-12, 160.8 lbs FACTORED DOWN AT 4-0-12, AND 160.8 lbs FACTORED DOWN AT 6-0-12, AND 164.6 lbs FACTORED DOWN AT 8-0-12 ON TOP CHORD, AND 86.4 lbs FACTORED DOWN AT 2-0-12, 86.4 lbs FACTORED DOWN AT 4-0-12, AND 86.4 lbs FACTORED DOWN AT 6-0-12, AND 87.3 lbs FACTORED DOWN AT 8-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1327	0	1327	0	0	5-8	1-13
D	1236	0	1236	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1021	694 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0
D	967	629 / 0	173 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1234 / 0	0.0	0.0	0.21 (1)	6.29	A- E	0 / 1987	0.49 (1)
A- G	-1845 / 0	-122.2	-122.2	0.77 (1)	3.66	E- B	-1107 / 0	0.18 (1)
G- H	-1845 / 0	-122.2	-122.2	0.77 (1)	3.66	E- C	0 / 1987	0.49 (1)
H- B	-1845 / 0	-122.2	-122.2	0.77 (1)	3.66			
B- I	-1845 / 0	-122.2	-122.2	0.77 (1)	3.66			
I- J	-1845 / 0	-122.2	-122.2	0.77 (1)	3.66			
J- C	-1845 / 0	-122.2	-122.2	0.77 (1)	3.66			
D- C	-1100 / 0	0.0	0.0	0.19 (1)	6.59			
F- K	0 / 0	-28.0	-28.0	0.28 (2)	10.00			
K- L	0 / 0	-28.0	-28.0	0.28 (2)	10.00			
L- E	0 / 0	-28.0	-28.0	0.28 (2)	10.00			
E- M	0 / 0	-28.0	-28.0	0.28 (2)	10.00			
M- N	0 / 0	-28.0	-28.0	0.28 (2)	10.00			
N- D	0 / 0	-28.0	-28.0	0.28 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-218	-218	---	BACK	VERT	TOTAL
G	2-0-12	-161	-161	---	BACK	VERT	TOTAL
H	4-0-12	-161	-161	---	BACK	VERT	TOTAL
I	6-0-12	-161	-161	---	BACK	VERT	TOTAL
J	8-0-12	-165	-165	---	BACK	VERT	TOTAL
K	2-0-12	-86	-86	---	BACK	VERT	TOTAL
L	4-0-12	-86	-86	---	BACK	VERT	TOTAL
M	6-0-12	-86	-86	---	BACK	VERT	TOTAL
N	8-0-12	-87	-87	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.77 (A-B:1) , BC=0.28 (E-F:2) , WB=0.49 (A-E:1) , SSI=0.51 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.46 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17862-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
267011	T16	1	1	TRUSS DESC.	

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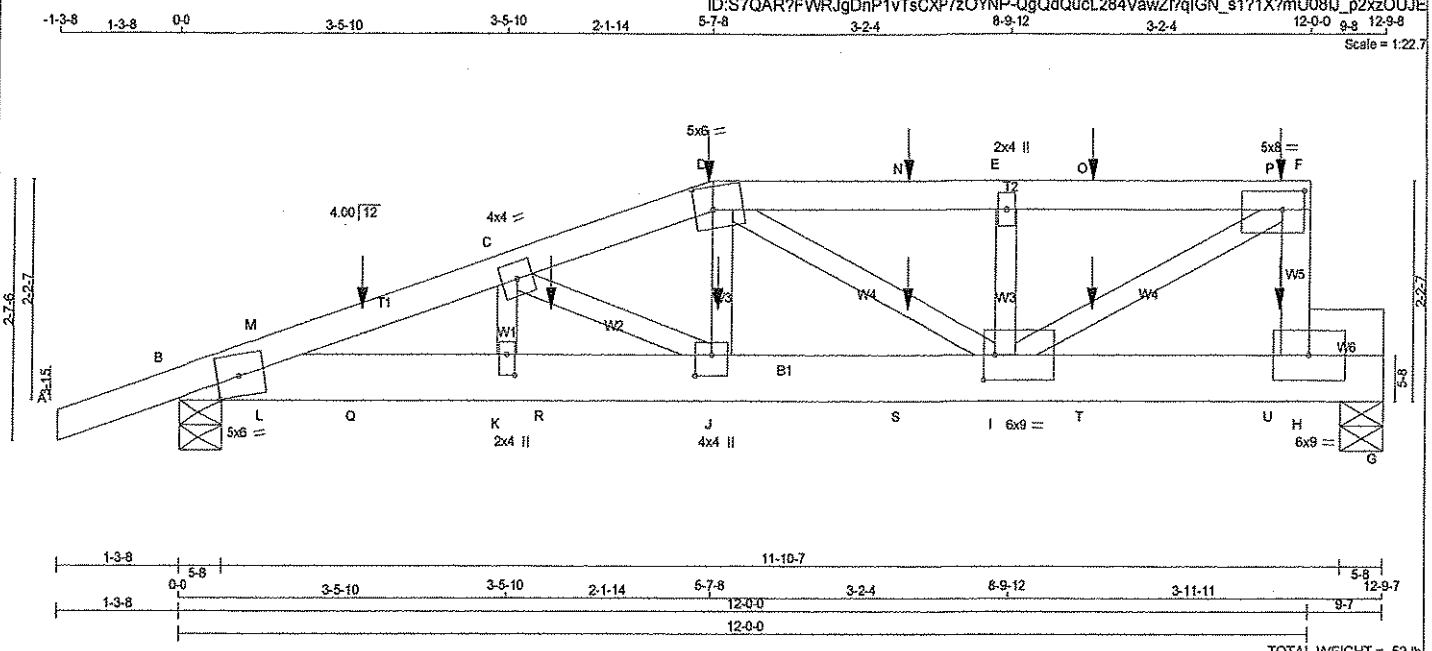

TOTAL WEIGHT = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
B - H	2x6	DRY	2100F 1.8E	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-m	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMW-t	MT20	5.0	8.0	2.25	2.75
H	BMW-t	MT20	6.0	9.0		
I	BMWWW-t	MT20	6.0	9.0	3.00	1.50
J	BMWW+t	MT20	4.0	4.0	2.50	2.00
K	BMW+w	MT20	2.0	4.0	2.50	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 520.8 lbs FACTORED DOWN AT 5-7-8, 179.1 lbs FACTORED DOWN AT 7-8-4, AND 179.1 lbs FACTORED DOWN AT 9-8-4, AND 221.1 lbs FACTORED DOWN AT 11-8-4 ON TOP CHORD, AND 93.0 lbs FACTORED DOWN AT 1-11-4, 70.5 lbs FACTORED DOWN AT 3-11-4, 93.1 lbs FACTORED DOWN AT 5-8-4, 93.1 lbs FACTORED DOWN AT 7-8-4, AND 93.1 lbs FACTORED DOWN AT 9-8-4, AND 93.1 lbs FACTORED DOWN AT 11-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1783	0	1783	0	0	5-8	1-15
G	1827	0	1827	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1385	919 / 0	237 / 0	0 / 0	0 / 0	229 / 0	0 / 0
G	1434	925 / 0	262 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-122.2	-122.2	0.17 (1)	10.00
B- M	-4092 / 0	-122.2	-122.2	0.26 (1)	3.25
M- C	-4075 / 0	-122.2	-122.2	0.39 (1)	3.11
C- D	-3573 / 0	-122.2	-122.2	0.27 (1)	3.46
D- N	-3072 / 0	-122.2	-122.2	0.44 (1)	3.49
N- E	-3072 / 0	-122.2	-122.2	0.44 (1)	3.49
E- O	-3072 / 0	-122.2	-122.2	0.45 (1)	3.49
O- P	-3072 / 0	-122.2	-122.2	0.45 (1)	3.49
P- F	-3072 / 0	-122.2	-122.2	0.45 (1)	3.49
H- F	-2125 / 0	0.0	0.0	0.26 (1)	5.77
B- L	0 / 3852	-28.0	-28.0	0.37 (1)	10.00
L- Q	0 / 3870	-28.0	-28.0	0.39 (1)	10.00
Q- K	0 / 3870	-28.0	-28.0	0.39 (1)	10.00
K- R	0 / 3870	-28.0	-28.0	0.33 (1)	10.00
R- J	0 / 3870	-28.0	-28.0	0.33 (1)	10.00
J- S	0 / 3416	-28.0	-28.0	0.22 (1)	10.00
S- I	0 / 3416	-28.0	-28.0	0.22 (1)	10.00
I- T	0 / 0	-28.0	-28.0	0.57 (1)	10.00
T- U	0 / 0	-28.0	-28.0	0.57 (1)	10.00
U- H	0 / 0	-28.0	-28.0	0.57 (1)	10.00
H- G	0 / 0	-28.0	-28.0	0.44 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 172	0.04 (3)
C- J	-536 / 0	0.10 (1)
J- D	0 / 490	0.12 (1)
D- I	-398 / 0	0.10 (1)
I- E	-779 / 0	0.13 (1)
I- F	0 / 3530	0.87 (1)
L- M	-159 / 22	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-7-8	-521	-521	---	BACK	VERT	TOTAL
J	5-8-4	-93	-93	---	BACK	VERT	TOTAL
N	7-8-4	-179	-179	---	BACK	VERT	TOTAL
O	9-8-4	-179	-179	---	BACK	VERT	TOTAL
P	11-8-4	-221	-221	---	BACK	VERT	TOTAL
Q	1-11-4	-93	-93	---	BACK	VERT	TOTAL
R	3-11-4	-56	-70	---	BACK	VERT	TOTAL
S	7-8-4	-93	-93	---	BACK	VERT	TOTAL
T	9-8-4	-93	-93	---	BACK	VERT	TOTAL
U	11-8-4	-93	-93	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 788 (0.19")

CSI: TC=0.45 (E-F:1) , BC=0.57 (H-I:1) , WB=0.87 (F-I:1) , SSI=0.61 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.93 (B) (INPUT = 1.00)

DWG NO. TAM17863 -16

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272258	T20	3	1	TRUSS DESC.	T20

Tamarack Roof Truss, Burlington

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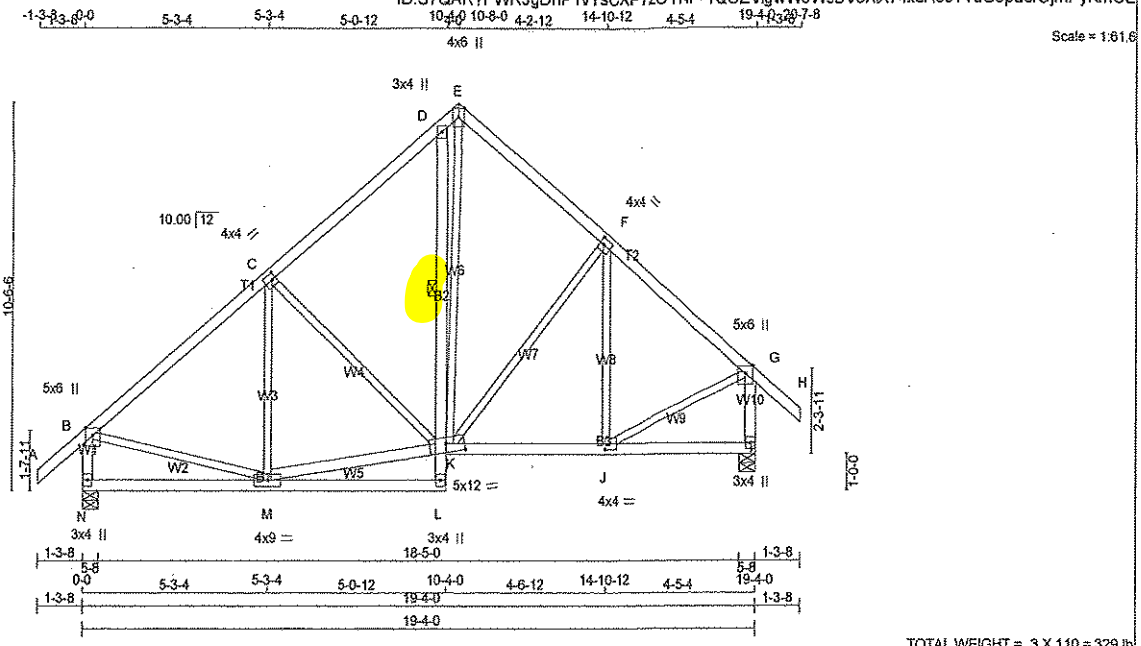


TOTAL WEIGHT = 3 X 110 = 329 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMV+p	MT20	3.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BVMWWWW+t	MT20	5.0	12.0	6.50	3.00
L	BMV+p	MT20	3.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1621	0	1621	0	0	5-8	1-12
I	1621	0	1621	0	0	5-8	1-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1250	845 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0
I	1250	845 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	-1330 / 0	-122.2	-122.2	0.44 (1)	5.08	
C- D	-1087 / 0	-122.2	-122.2	0.41 (1)	5.52	
D- E	-1098 / 0	-122.2	-122.2	0.07 (1)	5.98	
E- F	-1005 / 0	-122.2	-122.2	0.30 (1)	5.87	
F- G	-1178 / 0	-122.2	-122.2	0.31 (1)	5.52	
G- H	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
N- B	-1563 / 0	0.0	0.0	0.17 (1)	6.59	
I- G	-1573 / 0	0.0	0.0	0.20 (1)	6.58	
N- M	0 / 0	-28.0	-28.0	0.21 (3)	10.00	
M- L	0 / 13	-28.0	-28.0	0.21 (3)	10.00	
L- K	0 / 80	0.0	0.0	0.03 (1)	10.00	
K- D	-350 / 27	0.0	0.0	0.04 (1)	6.25	
K- J	0 / 935	-28.0	-28.0	0.26 (2)	10.00	
J- I	0 / 0	-28.0	-28.0	0.17 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-299 / 131	0.17 (1)
M- K	0 / 1065	0.17 (1)
C- K	-351 / 0	0.30 (1)
K- E	0 / 1145	0.26 (1)
J- F	-260 / 121	0.15 (1)
B- M	0 / 1068	0.24 (1)
J- G	0 / 1023	0.23 (1)
K- F	-304 / 0	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.44 (B-C:1) , BC=0.26 (J-K:2) , WB=0.31 (F-K:1) , SSI=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

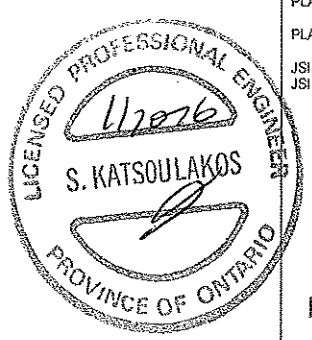
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.35 (J) (INPUT = 1.00)



DWG NO. TAM 49674-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272258	T21	1	1	TRUSS DESC.	**T21**

Tamarack Roof Truss, Burlington

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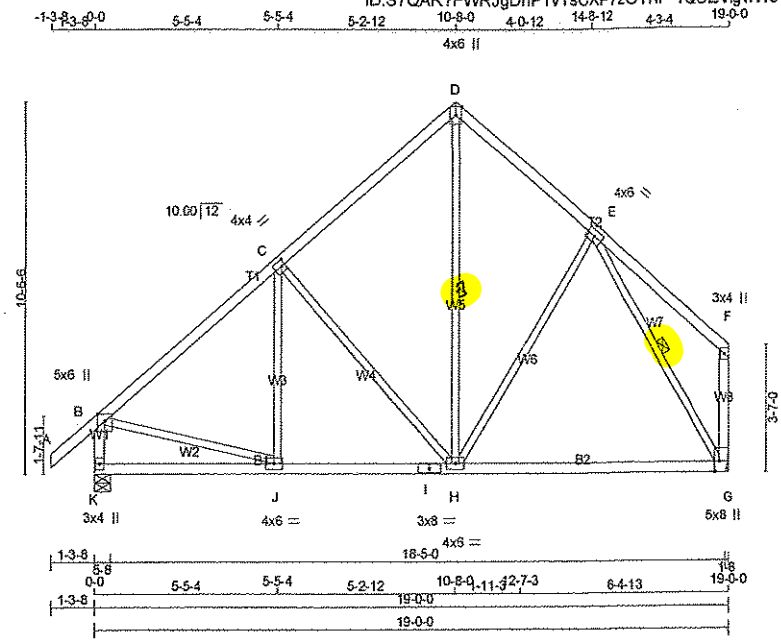


TOTAL WEIGHT = 93 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1+t	MT20	5.0	8.0	Edge	1.50
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1596	0	1596	0	0	5-8	1-12
G	1427	0	1427	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1230	832 / 0	199 / 0	0 / 0	0 / 0	198 / 0	0 / 0
G	1116	727 / 0	199 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-G. I) - H /

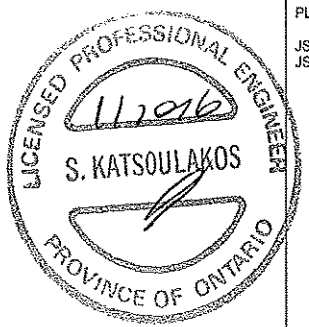
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-1290 / 0	-122.2	-122.2	0.49 (1)	5.06
C- D	-926 / 0	-122.2	-122.2	0.46 (1)	5.77
D- E	-913 / 0	-122.2	-122.2	0.27 (1)	6.12
E- F	0 / 38	-122.2	-122.2	0.34 (1)	10.00
K- B	-1529 / 0	0.0	0.0	0.16 (1)	6.65
G- F	-197 / 0	0.0	0.0	0.04 (1)	7.81
K- J	0 / 0	-28.0	-28.0	0.19 (2)	10.00
J- I	0 / 1029	-28.0	-28.0	0.52 (2)	10.00
I- H	0 / 1029	-28.0	-28.0	0.52 (2)	10.00
H- G	0 / 705	-28.0	-28.0	0.48 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-113 / 148	0.07 (1)
C- H	-536 / 0	0.68 (1)
H- D	0 / 688	0.15 (1)
H- E	-64 / 105	0.08 (1)
B- J	0 / 1059	0.24 (1)
E- G	-1335 / 0	0.55 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 721 (0.32")

CSI: TC=0.49 (B-C:1) , BC=0.52 (H-J:2) , WB=0.68 (C-H:1) , SSI=0.25 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.60 (I) (INPUT = 1.00)

DWG NO. TAM49675-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272270	T30	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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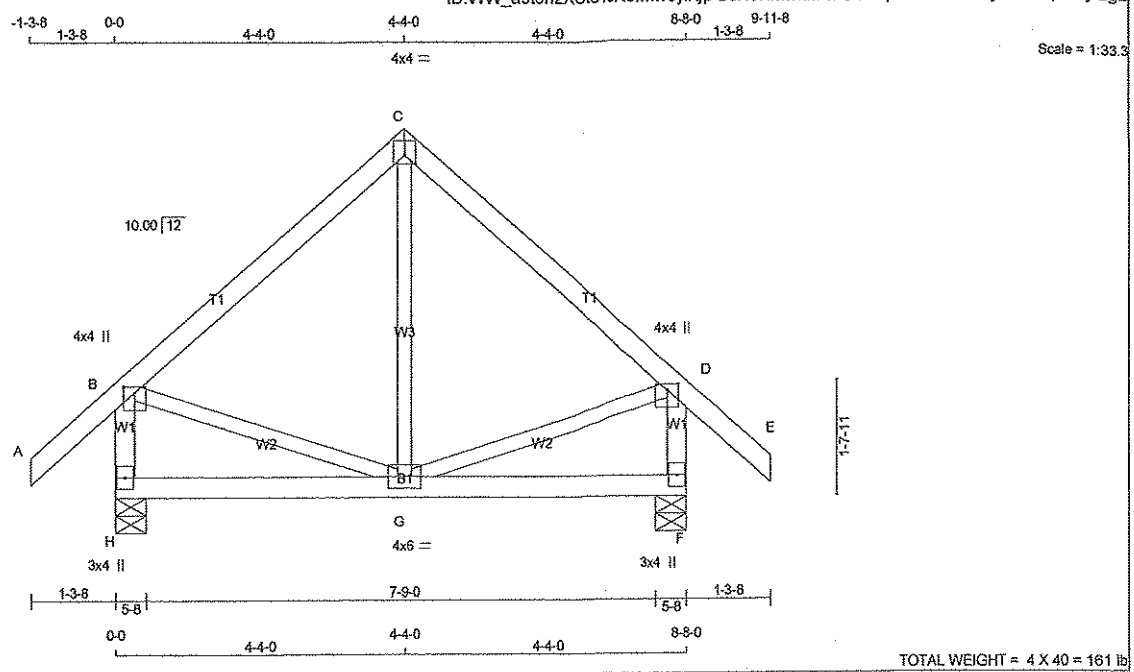

TOTAL WEIGHT = 4 X 40 = 161 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	820	0	820	0	0	5-8	5-8
F	820	0	820	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	623	437 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	623	437 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	G- C	-31 / 185	0.04 (3)
B- C	-389 / 0	-122.2	-122.2	0.30 (1)	6.25	B- G	0 / 312	0.07 (1)
C- D	-389 / 0	-122.2	-122.2	0.30 (1)	6.25	G- D	0 / 312	0.07 (1)
D- E	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
H- B	-773 / 0	0.0	0.0	0.08 (1)	7.81			
F- D	-773 / 0	0.0	0.0	0.08 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

DWG NO. TAM45216-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.30 (B-C:1) , BC=0.15 (G-H:3) , WB=0.07 (D-G:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272270	G30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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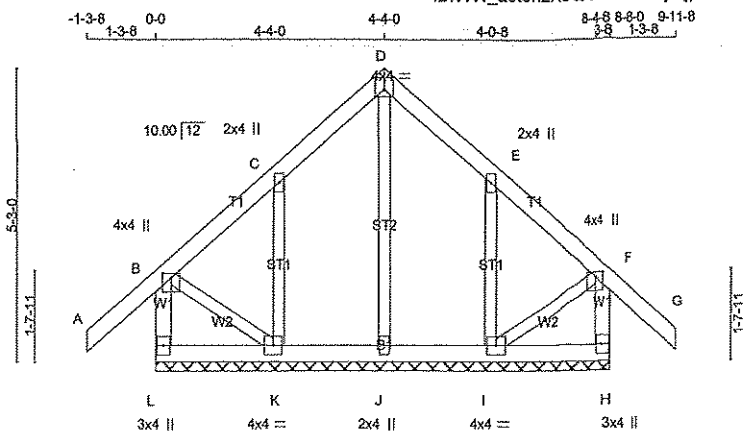

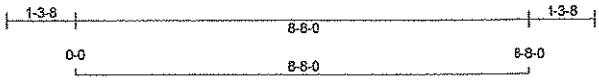

TOTAL WEIGHT = 42 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-311 / 0	0.0	0.0	0.03 (1)	7.81	J- D	-162 / 0	0.06 (1)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	K- C	-321 / 0	0.07 (1)
B- C	-14 / 0	-122.2	-122.2	0.09 (1)	6.25	I- E	-321 / 0	0.07 (1)
C- D	-43 / 0	-122.2	-122.2	0.09 (1)	6.25	B- K	0 / 30	0.01 (1)
D- E	-43 / 0	-122.2	-122.2	0.09 (1)	6.25	I- F	0 / 30	0.01 (1)
E- F	-14 / 0	-122.2	-122.2	0.09 (1)	6.25			
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
H- F	-311 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 0	-28.0	-28.0	0.04 (3)	10.00			
K- J	0 / 16	-28.0	-28.0	0.04 (2)	10.00			
J- I	0 / 16	-28.0	-28.0	0.04 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.04 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.04 (J-K:2) , WB=0.07 (C-K:1) , SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9-7-17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4521 7-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272270	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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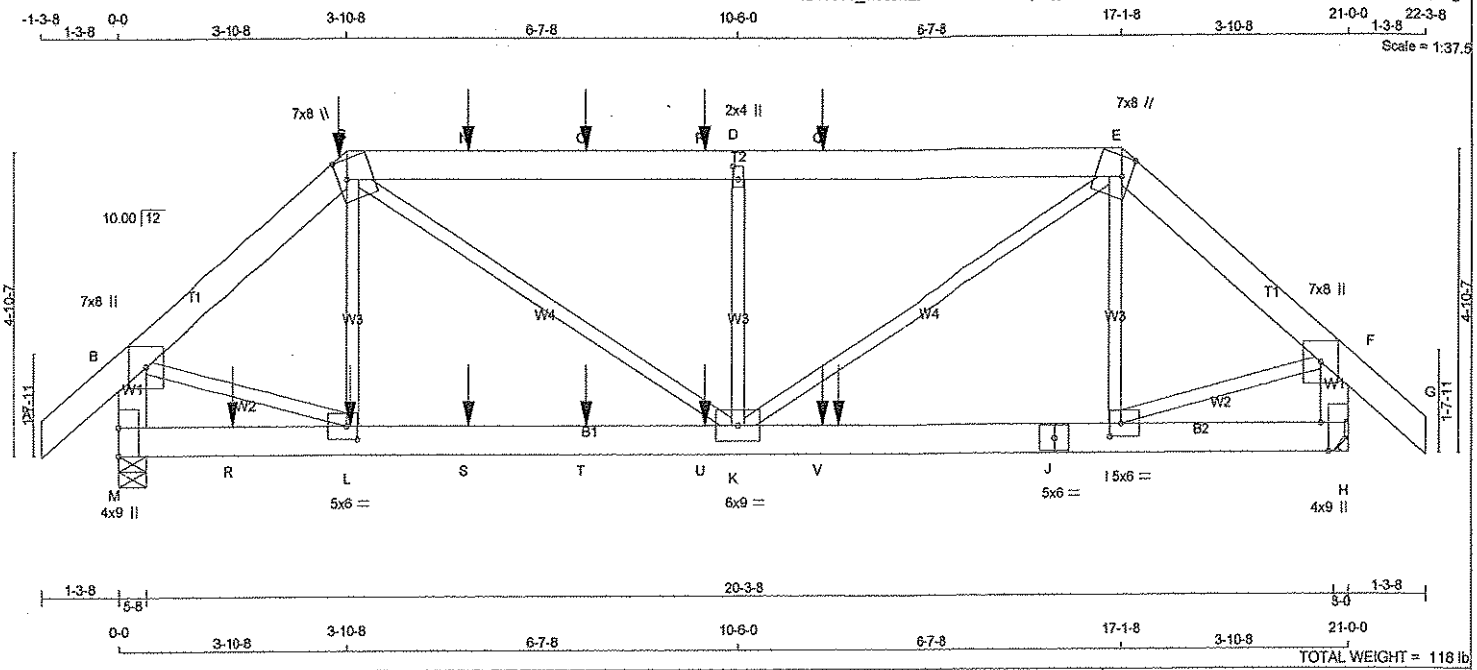

TOTAL WEIGHT = 118 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	7.0	8.0		
C	TTWW+m	MT20	7.0	8.0	3.75	1.75
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TTWW+m	MT20	7.0	8.0	3.75	1.75
F	TMVW+p	MT20	7.0	8.0		
H	BMV1+t	MT20	4.0	9.0	Edge	1.50
I	BMWW-t	MT20	5.0	6.0	2.50	2.25
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 161.1 lbs FACTORED DOWN AT 3-10-8, 118.1 lbs FACTORED DOWN AT 3-10-8, 114.6 lbs FACTORED DOWN AT 5-11-4, 114.6 lbs FACTORED DOWN AT 7-11-4, AND 114.6 lbs FACTORED DOWN AT 9-11-4, AND 114.6 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, 42.0 lbs FACTORED DOWN AT 3-11-4, 42.0 lbs FACTORED DOWN AT 5-11-4, 42.0 lbs FACTORED DOWN AT 7-11-4, 42.0 lbs FACTORED DOWN AT 9-11-4, AND 42.0 lbs FACTORED DOWN AT 11-11-4, AND 1055.9 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	2796	0	2796	0	0	5-8	5-8
H	2647	0	2647	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2156	1456 / 0	350 / 0	0 / 0	0 / 0	350 / 0	0 / 0
H	2026	1396 / 0	312 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 56		-122.2	-122.2	0.10 (1)	10.00		L- C	-437 / 169		0.14 (1)
B- C	-2700 / 0		-122.2	-122.2	0.21 (1)	4.85		I- E	-88 / 164		0.04 (3)
C- N	-3891 / 0		-122.2	-122.2	0.69 (1)	3.50		B- L	0 / 2145		0.53 (1)
N- O	-3891 / 0		-122.2	-122.2	0.69 (1)	3.50		I- F	0 / 2114		0.52 (1)
O- P	-3891 / 0		-122.2	-122.2	0.69 (1)	3.50		K- E	0 / 2227		0.55 (1)
P- D	-3891 / 0		-122.2	-122.2	0.69 (1)	3.50		C- K	0 / 2201		0.54 (1)
D- Q	-3891 / 0		-122.2	-122.2	0.69 (1)	3.50		K- D	-1292 / 0		0.41 (1)
Q- E	-3891 / 0		-122.2	-122.2	0.69 (1)	3.50					
E- F	-2662 / 0		-122.2	-122.2	0.21 (1)	4.88					
F- G	0 / 56		-122.2	-122.2	0.10 (1)	10.00					
M- B	-2730 / 0		0.0	0.0	0.20 (1)	6.27					
H- F	-2697 / 0		0.0	0.0	0.19 (1)	6.31					
M- R	0 / 0		-28.0	-28.0	0.12 (3)	10.00					
R- L	0 / 0		-28.0	-28.0	0.12 (3)	10.00					
L- S	0 / 2055		-28.0	-28.0	0.62 (1)	10.00					
S- T	0 / 2055		-28.0	-28.0	0.62 (1)	10.00					
T- U	0 / 2055		-28.0	-28.0	0.62 (1)	10.00					
U- K	0 / 2055		-28.0	-28.0	0.62 (1)	10.00					
K- V	0 / 2034		-28.0	-28.0	0.89 (1)	10.00					
V- J	0 / 2034		-28.0	-28.0	0.89 (1)	10.00					
J- I	0 / 2034		-28.0	-28.0	0.89 (1)	10.00					
I- H	0 / 0		-28.0	-28.0	0.31 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-161	-161	---	FRONT	VERT	TOTAL
C	3-10-8	-118	-118	---	BACK	VERT	TOTAL
L	3-11-4	-24	-42	---	BACK	VERT	TOTAL
N	5-11-4	-115	-115	---	BACK	VERT	TOTAL
O	7-11-4	-115	-115	---	BACK	VERT	TOTAL
P	9-11-4	-115	-115	---	BACK	VERT	TOTAL
Q	11-11-4	-115	-115	---	BACK	VERT	TOTAL
R	1-11-4	-24	-42	---	BACK	VERT	TOTAL
S	5-11-4	-24	-42	---	BACK	VERT	TOTAL
T	7-11-4	-24	-42	---	BACK	VERT	TOTAL
U	9-11-4	-24	-42	---	BACK	VERT	TOTAL
V	11-11-4	-24	-42	---	BACK	VERT	TOTAL
V	12-2-8	-1056	-1056	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.69 (D-E:1) , BC=0.89 (I-K:1) , WB=0.55 (E-K:1) , SSI=0.62 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.51 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9-7-17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T AM 45218-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272270	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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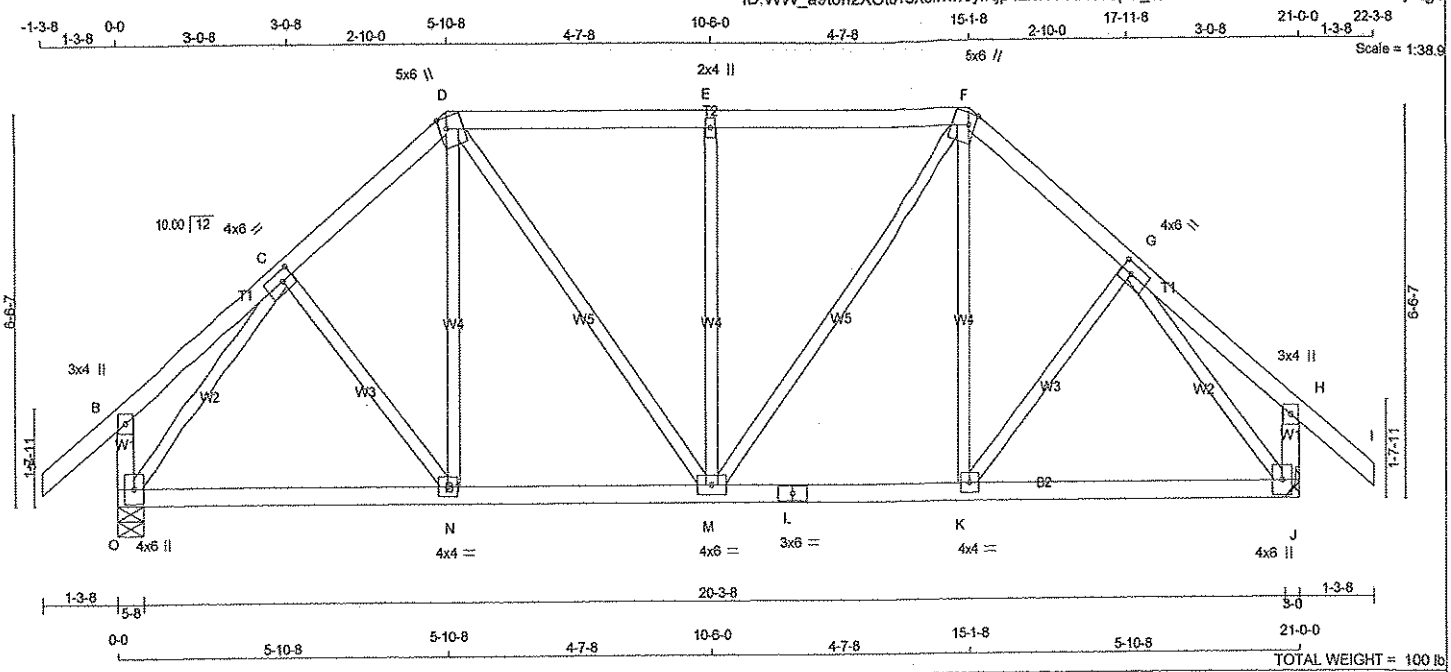

TOTAL WEIGHT = 100 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
O	1746	0	1746	0	0	IN-SX	IN-SX
J	1746	0	1746	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1348	909 / 0	221 / 0	0 / 0	0 / 0	218 / 0	0 / 0
J	1348	909 / 0	221 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 24	-122.2	-122.2	0.15 (1)	10.00	
C- D	-1442 / 0	-122.2	-122.2	0.14 (1)	5.32	
D- E	-1378 / 0	-122.2	-122.2	0.35 (1)	5.13	
E- F	-1378 / 0	-122.2	-122.2	0.35 (1)	5.13	
F- G	-1442 / 0	-122.2	-122.2	0.14 (1)	5.32	
G- H	0 / 24	-122.2	-122.2	0.15 (1)	10.00	
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
O- B	-313 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-313 / 0	0.0	0.0	0.03 (1)	7.81	
O- N	0 / 1070	-28.0	-28.0	0.33 (2)	10.00	
N- M	0 / 1088	-28.0	-28.0	0.34 (2)	10.00	
M- L	0 / 1088	-28.0	-28.0	0.34 (2)	10.00	
L- K	0 / 1088	-28.0	-28.0	0.34 (2)	10.00	
K- J	0 / 1070	-28.0	-28.0	0.33 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 104	0.02 (3)
N- D	0 / 240	0.05 (2)
D- M	0 / 486	0.11 (1)
M- E	-686 / 0	0.48 (1)
M- F	0 / 486	0.11 (1)
K- F	0 / 240	0.05 (2)
K- G	0 / 104	0.02 (3)
O- C	-1735 / 0	0.62 (1)
G- J	-1735 / 0	0.62 (1)

DWG NO. TAM45219-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.35 (D-E:1) , BC=0.34 (M-N:2) , WB=0.62 (C-O:1) , SSI=0.27 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.42 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
272270	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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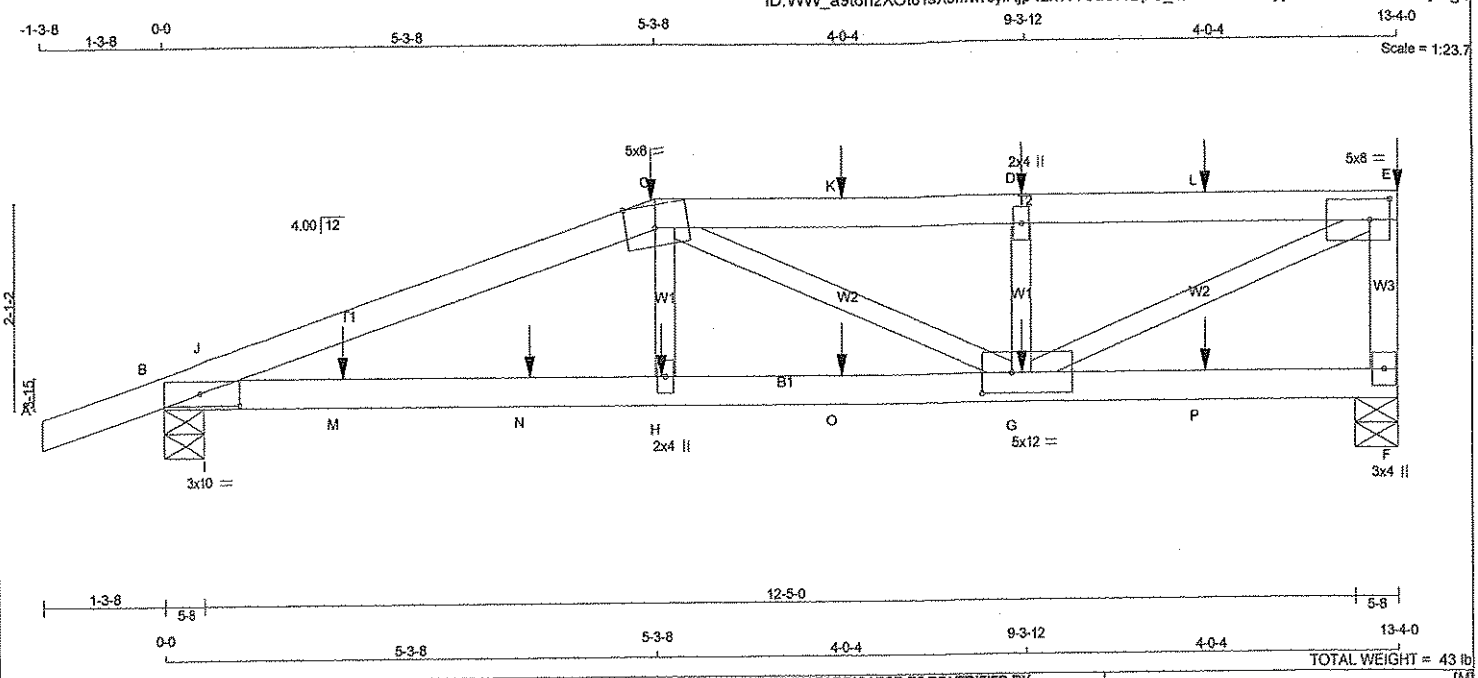

TOTAL WEIGHT = 43 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	10.0	1.50	5.00
C	TTWW-m	MT20	5.0	8.0	Edge	3.75
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	8.0	2.50	2.50
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	12.0	2.50	4.00
H	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 471.7 lbs FACTORED DOWN AT 5-3-8, 160.8 lbs FACTORED DOWN AT 7-3-4, 160.8 lbs FACTORED DOWN AT 9-3-4, AND 160.8 lbs FACTORED DOWN AT 11-3-4, AND 218.1 lbs FACTORED DOWN AT 13-4-0 ON TOP CHORD, AND 86.4 lbs FACTORED DOWN AT 1-11-4, 65.1 lbs FACTORED DOWN AT 3-11-4, 86.4 lbs FACTORED DOWN AT 5-4-4, 86.4 lbs FACTORED DOWN AT 7-3-4, AND 86.4 lbs FACTORED DOWN AT 9-3-4, AND 86.4 lbs FACTORED DOWN AT 11-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1988	0	1988	0	0	5-8	5-8
B	1848	0	1848	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1537	1031 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
B	1433	955 / 0	243 / 0	0 / 0	0 / 0	236 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 24	-122.2	-122.2	0.17 (1)	10.00	
B- J	-3873 / 0	-122.2	-122.2	0.23 (1)	3.38	
J- C	-3837 / 0	-122.2	-122.2	0.82 (1)	2.69	
C- K	-3200 / 0	-122.2	-122.2	0.64 (1)	3.14	
K- D	-3200 / 0	-122.2	-122.2	0.64 (1)	3.14	
D- L	-3200 / 0	-122.2	-122.2	0.64 (1)	3.14	
L- E	-3200 / 0	-122.2	-122.2	0.64 (1)	3.14	
F- E	-1890 / 0	0.0	0.0	0.23 (1)	6.06	
B- I	0 / 3637	-28.0	-28.0	0.70 (1)	10.00	
I- M	0 / 3637	-28.0	-28.0	0.81 (1)	10.00	
M- N	0 / 3637	-28.0	-28.0	0.81 (1)	10.00	
N- H	0 / 3637	-28.0	-28.0	0.81 (1)	10.00	
H- O	0 / 3662	-28.0	-28.0	0.65 (1)	10.00	
O- G	0 / 3662	-28.0	-28.0	0.65 (1)	10.00	
G- P	0 / 0	-28.0	-28.0	0.18 (2)	10.00	
P- F	0 / 0	-28.0	-28.0	0.18 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
H- C	0 / 463	0.11	(2)
C- G	-507 / 0	0.16	(1)
G- D	-959 / 0	0.16	(1)
G- E	0 / 3500	0.87	(1)
I- J	-214 / 199	0.00	(1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-3-8	-472	-472	---	FRONT	VERT	TOTAL
D	9-3-4	-161	-161	---	FRONT	VERT	TOTAL
E	13-4-0	-218	-218	---	FRONT	VERT	TOTAL
G	9-3-4	-86	-86	---	FRONT	VERT	TOTAL
H	5-4-4	-86	-86	---	FRONT	VERT	TOTAL
K	7-3-4	-161	-161	---	FRONT	VERT	TOTAL
L	11-3-4	-161	-161	---	FRONT	VERT	TOTAL
M	1-11-4	-95	-95	---	FRONT	VERT	TOTAL
N	3-11-4	-55	-65	---	FRONT	VERT	TOTAL
O	7-3-4	-86	-86	---	FRONT	VERT	TOTAL
P	11-3-4	-85	-85	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 653 (0.25")

CSI: TC=0.82 (C-J:1) , BC=0.81 (H-I:1) , WB=0.87 (E-G:1) , SSI=0.35 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL = 0.85 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9777
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45220-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272272	T40	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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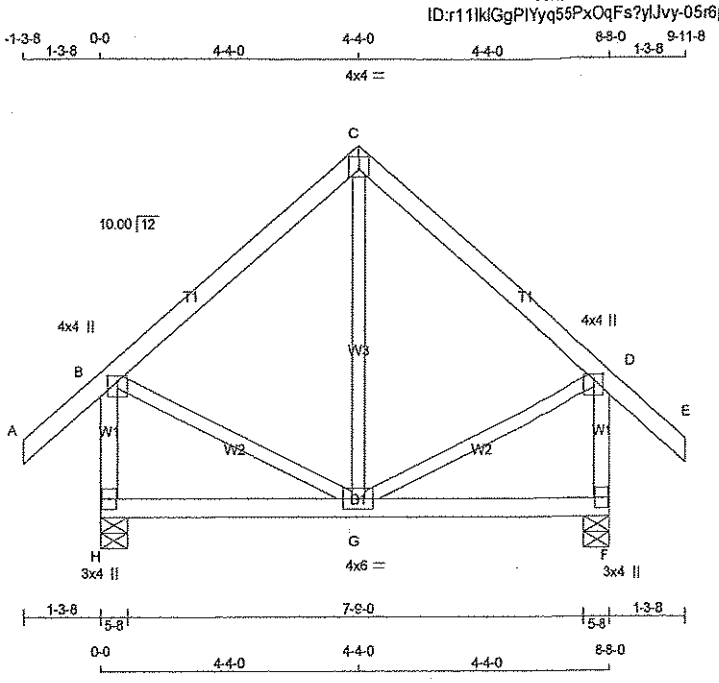

Scale = 1:37.2

TOTAL WEIGHT = 2 X 43 = 86 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	820	0	820	0	0	5-8	5-8
F	820	0	820	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	623	437 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	623	437 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	G- C	-91 / 159	0.05 (1)
B- C	-343 / 0	-122.2	-122.2	0.29 (1)	6.25	B- G	0 / 289	0.07 (1)
C- D	-343 / 0	-122.2	-122.2	0.29 (1)	6.25	G- D	0 / 289	0.07 (1)
D- E	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
H- B	-773 / 0	0.0	0.0	0.10 (1)	7.81			
F- D	-773 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/330 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.29 (B-C:1) , BC=0.15 (G-H:3) , WB=0.07 (B-G:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9.21.17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45222-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272272	G40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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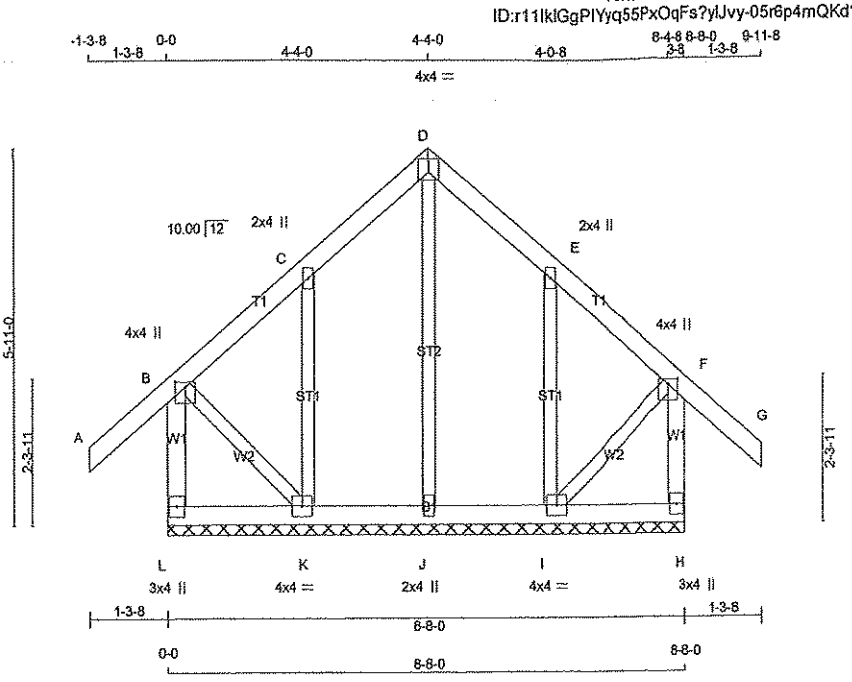

TOTAL WEIGHT = 46 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- B	-297 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 3	-122.2	-122.2	0.09 (1)	10.00	
C- D	-28 / 0	-122.2	-122.2	0.09 (1)	6.25	
D- E	-28 / 0	-122.2	-122.2	0.09 (1)	6.25	
E- F	0 / 3	-122.2	-122.2	0.09 (1)	10.00	
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
H- F	-297 / 0	0.0	0.0	0.04 (1)	7.81	
L- K	0 / 0	-28.0	-28.0	0.04 (3)	10.00	
K- J	0 / 5	-28.0	-28.0	0.04 (3)	10.00	
J- I	0 / 5	-28.0	-28.0	0.04 (3)	10.00	
I- H	0 / 0	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-181 / 0	0.09 (1)
K- C	-322 / 0	0.09 (1)
I- E	-322 / 0	0.09 (1)
B- K	0 / 17	0.00 (1)
I- F	0 / 17	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (F-G:1) , BC=0.04 (J-K:3) , WB=0.09 (D-J:1) , SSI=0.10 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (D) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9.21
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45223-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
272272	T41	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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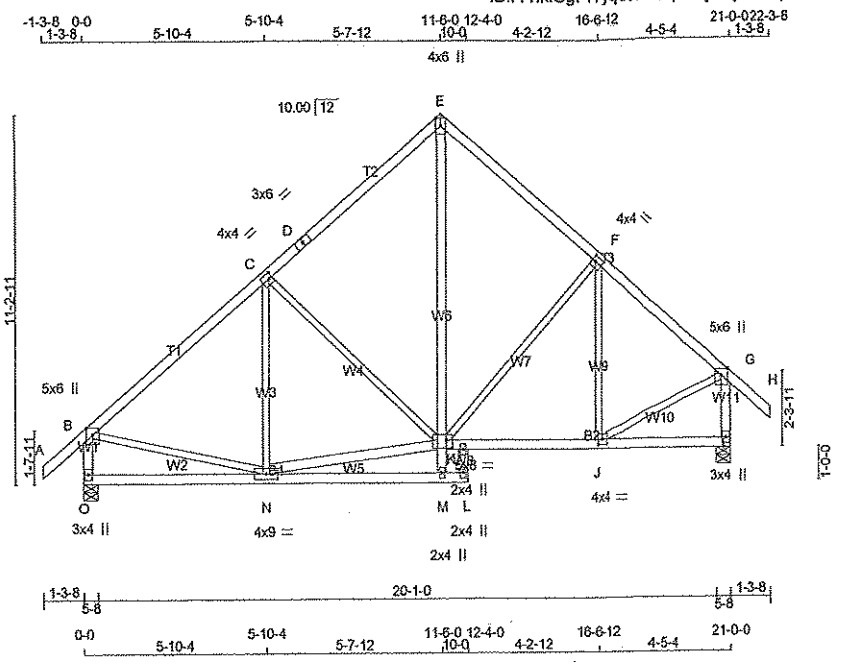

TOTAL WEIGHT = 3 X 112 = 337 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BWMWWW-t	MT20	5.0	8.0	3.00	2.50
L	NP+w	MT20	2.0	4.0		
M	BMW+w	MT20	2.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMV1+p	MT20	3.0	4.0		
P	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1757	0	1757	0	0	5-8	5-8
I	1759	0	1759	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1351	909 / 0	228 / 0	0 / 0	0 / 0	224 / 0	0 / 0
I	1364	909 / 0	230 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17	(1)	10.00	
B- C	-1487 / 0	-122.2	-122.2	0.59	(1)	4.62	
C- D	-1150 / 0	-122.2	-122.2	0.55	(1)	5.16	
D- E	-1150 / 0	-122.2	-122.2	0.55	(1)	5.16	
E- F	-1139 / 0	-122.2	-122.2	0.39	(1)	5.46	
F- G	-1325 / 0	-122.2	-122.2	0.36	(1)	5.17	
G- H	0 / 54	-122.2	-122.2	0.17	(1)	10.00	
O- B	-1693 / 0	0.0	0.0	0.18	(1)	6.39	
I- G	-1713 / 0	0.0	0.0	0.21	(1)	6.36	
O- N	0 / 0	-28.0	-28.0	0.27	(3)	10.00	
N- M	0 / 17	-28.0	-28.0	0.27	(3)	10.00	
M- L	0 / 0	-28.0	-28.0	0.02	(2)	10.00	
K- J	0 / 1055	-28.0	-28.0	0.29	(2)	10.00	
J- I	0 / 0	-28.0	-28.0	0.18	(3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-273 / 168	0.19 (1)
K- F	-320 / 0	0.37 (1)
J- F	-307 / 107	0.17 (1)
B- N	0 / 1213	0.27 (1)
J- G	0 / 1155	0.26 (1)
M- K	0 / 145	0.04 (2)
K- E	0 / 902	0.17 (1)
N- K	0 / 1185	0.19 (1)
C- K	-466 / 0	0.55 (1)

DWG NO. TAM 45224-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.59 (B-C:1) , BC=0.29 (J-K:2) , WB=0.55 (C-K:1) , SSI=0.27 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.40 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272272	T42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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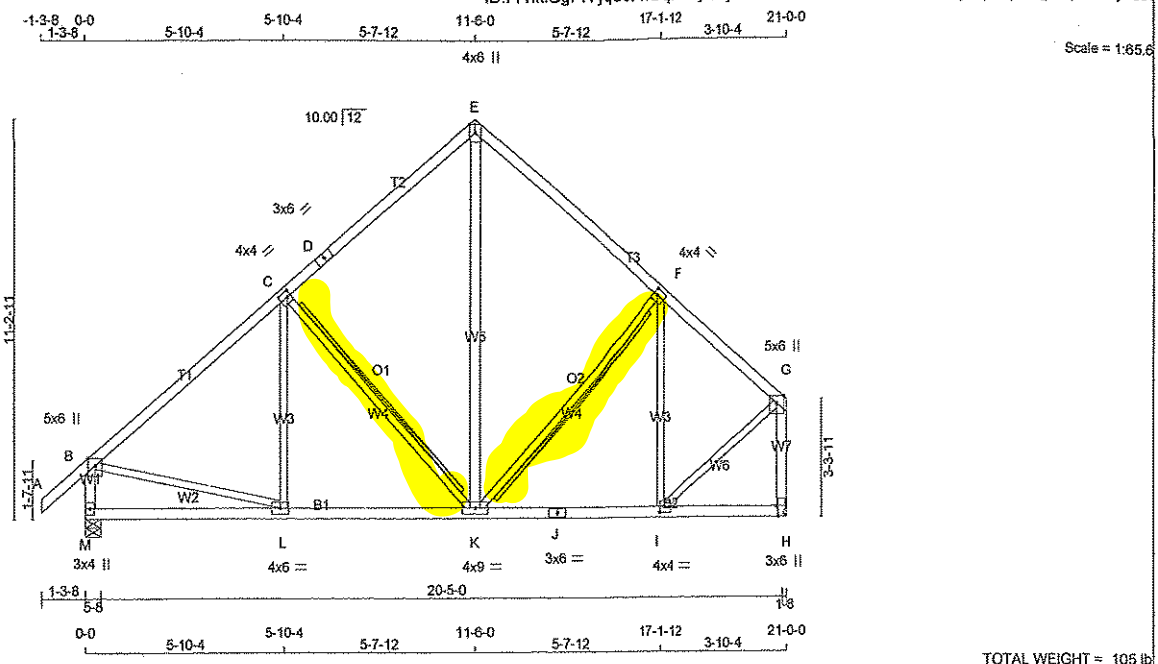

TOTAL WEIGHT = 105 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+t	MT20	3.0	6.0	Edge	0.50
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG	REQRD BRG
M	1746	0	1746	0	0	5-8	5-8
H	1577	0	1577	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1348	909 / 0	220 / 0	0 / 0	0 / 0	218 / 0	0 / 0
H	1233	803 / 0	220 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-K, F-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	-1470 / 0	-122.2	-122.2	0.59 (1)	4.64	
C- D	-1045 / 0	-122.2	-122.2	0.55 (1)	5.36	
D- E	-1045 / 0	-122.2	-122.2	0.55 (1)	5.36	
E- F	-1031 / 0	-122.2	-122.2	0.48 (1)	5.53	
F- G	-1074 / 0	-122.2	-122.2	0.40 (1)	5.55	
M- B	-1679 / 0	0.0	0.0	0.18 (1)	6.41	
H- G	-1538 / 0	0.0	0.0	0.29 (1)	6.64	
M- L	0 / 0	-28.0	-28.0	0.23 (3)	10.00	
L- K	0 / 1171	-28.0	-28.0	0.34 (2)	10.00	
K- J	0 / 872	-28.0	-28.0	0.26 (2)	10.00	
J- I	0 / 872	-28.0	-28.0	0.26 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.15 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-83 / 220	0.06 (1)
C- K	-609 / 0	0.71 (1)
K- E	0 / 730	0.12 (1)
K- F	-166 / 0	0.19 (1)
I- F	-526 / 10	0.36 (1)
B- L	0 / 1200	0.27 (1)
I- G	0 / 1104	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.59 (B-C:1) , BC=0.34 (K-L:2) , WB=0.71 (C-K:1) , SSI=0.27 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45225-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272270	T50	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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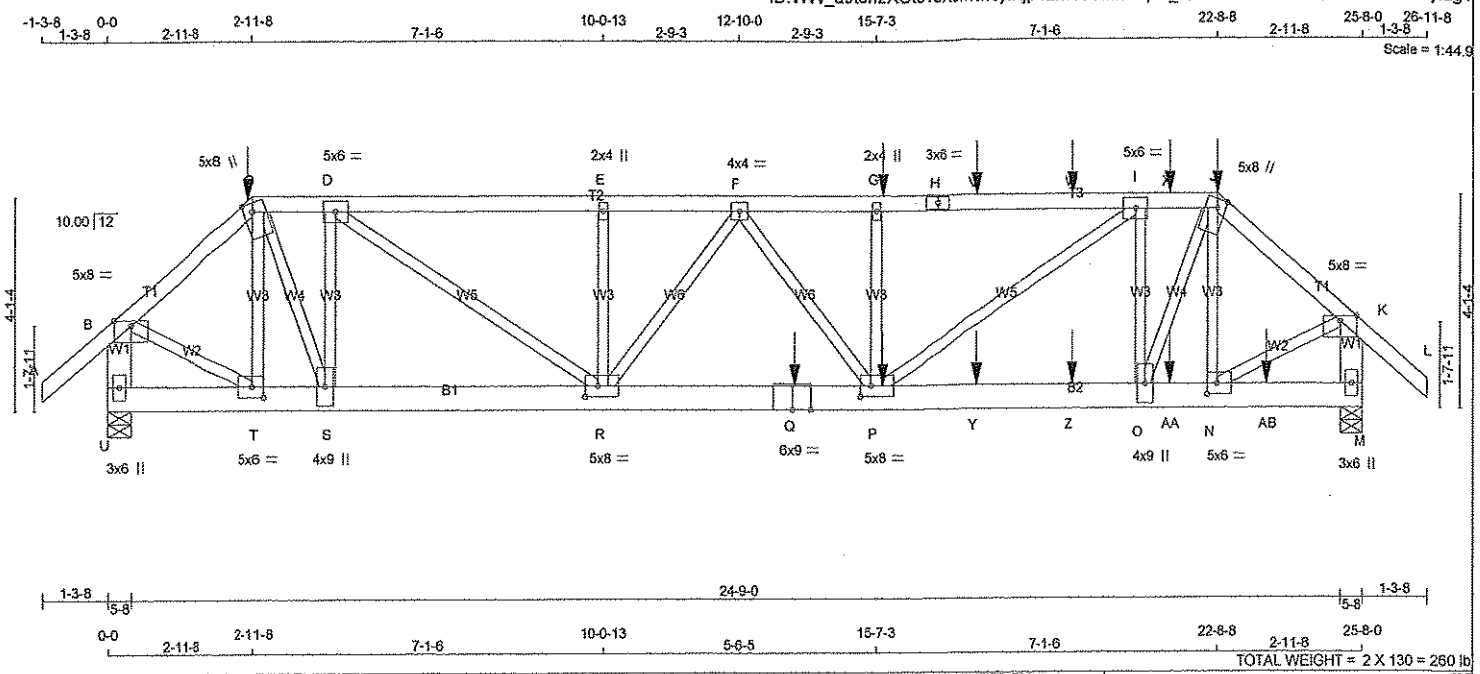

TOTAL WEIGHT = 2 X 130 = 260 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
J- L	1	12	SIDE(61.0)
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(0.0)
Q- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
P- G	1	6	SIDE(0.8)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	2.00	2.00
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	8.0	2.00	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW+t	MT20	4.0	9.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	2.50
Q	BS-t	MT20	6.0	9.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	3.00
S	BMWW+t	MT20	4.0	9.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3398	0	3398	0	0	5-8	5-8
M	4101	0	4101	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2617	1774 / 0	422 / 0	0 / 0	0 / 0	421 / 0	0 / 0
M	3177	2121 / 0	534 / 0	0 / 0	0 / 0	522 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.09 (1)		10.00
B- C	-3212 / 0	-122.2	-122.2	0.14 (1)		5.06
C- D	-3450 / 0	-122.2	-122.2	0.23 (1)		4.79
D- E	-6406 / 0	-122.2	-122.2	0.43 (1)		3.56
E- F	-6406 / 0	-122.2	-122.2	0.31 (1)		3.61
F- G	-7359 / 0	-122.2	-122.2	0.46 (1)		3.17
G- H	-7359 / 0	-122.2	-122.2	0.77 (1)		2.95
H- V	-7359 / 0	-122.2	-122.2	0.77 (1)		2.95
V- W	-7359 / 0	-122.2	-122.2	0.77 (1)		2.95
W- I	-7359 / 0	-122.2	-122.2	0.77 (1)		2.95
I- X	-4278 / 0	-122.2	-122.2	0.46 (1)		4.03
X- J	-4278 / 0	-122.2	-122.2	0.46 (1)		4.03
J- K	-3952 / 0	-122.2	-122.2	0.16 (1)		4.64
K- L	0 / 54	-122.2	-122.2	0.09 (1)		10.00
U- B	-3398 / 0	0.0	0.0	0.13 (1)		7.56
M- K	-4101 / 0	0.0	0.0	0.15 (1)		7.03

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-693 / 0	0.09 (1)
S- D	0 / 2575	0.32 (1)
S- D	-2549 / 0	0.32 (1)
O- I	-3230 / 0	0.41 (1)
O- J	0 / 3253	0.40 (1)
N- J	-768 / 0	0.10 (1)
B- T	0 / 2647	0.33 (1)
N- K	0 / 3257	0.40 (1)
P- I	0 / 3712	0.46 (1)
D- R	0 / 3564	0.44 (1)
P- G	-1067 / 0	0.14 (1)
R- E	-590 / 0	0.07 (1)
R- F	-862 / 0	0.16 (1)
F- P	0 / 736	0.09 (1)

U- T	0 / 0	-28.0	-28.0	0.05 (1)	10.00
T- S	0 / 2439	-28.0	-28.0	0.21 (1)	10.00
S- R	0 / 3452	-28.0	-28.0	0.37 (1)	10.00
R- Q	0 / 6919	-28.0	-28.0	0.99 (1)	10.00
Q- P	0 / 6919	-28.0	-28.0	0.99 (1)	10.00
P- Y	0 / 4283	-28.0	-28.0	0.52 (1)	10.00
Y- Z	0 / 4283	-28.0	-28.0	0.52 (1)	10.00
Z- O	0 / 4283	-28.0	-28.0	0.52 (1)	10.00
O-AA	0 / 3004	-28.0	-28.0	0.27 (1)	10.00
AA- N	0 / 3004	-28.0	-28.0	0.27 (1)	10.00
N-AB	0 / 0	-28.0	-28.0	0.07 (1)	10.00
AB- M	0 / 0	-28.0	-28.0	0.07 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-228	-228	---	FRONT	VERT	TOTAL
G	15-8-12	-237	-237	---	FRONT	VERT	TOTAL
J	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
P	15-8-12	-48	-85	---	FRONT	VERT	TOTAL
Q	13-11-8	-1660	-1660	---	FRONT	VERT	TOTAL
V	17-8-12	-237	-237	---	FRONT	VERT	TOTAL
W	19-8-12	-237	-237	---	FRONT	VERT	TOTAL
X	21-8-12	-238	-238	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 937 (0.33")

CSI: TC=0.77 (G-I:1) , BC=0.99 (P-R:1) , WB=0.46 (I-P:1) , SSI=0.42 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.88 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9-227
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4/5 2 4 17

STRUCTURAL COMPONENT ONLY

P6 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272270	T50	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 227.6 lbs FACTORED DOWN AT 22-8-8, 236.8 lbs FACTORED DOWN AT 15-8-12, 236.8 lbs FACTORED DOWN AT 17-8-12, AND 236.8 lbs FACTORED DOWN AT 19-8-12, AND 237.9 lbs FACTORED DOWN AT 21-8-12 ON TOP CHORD, AND 1660.4 lbs FACTORED DOWN AT 13-11-8, 84.6 lbs FACTORED DOWN AT 15-8-12, 84.6 lbs FACTORED DOWN AT 17-8-12, 84.6 lbs FACTORED DOWN AT 19-8-12, AND 84.6 lbs FACTORED DOWN AT 21-8-12, AND 84.6 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	17-8-12	-48	-85	—	FRONT	VERT	TOTAL
Z	19-8-12	-48	-85	—	FRONT	VERT	TOTAL
AA	21-8-12	-48	-85	—	FRONT	VERT	TOTAL
AB	23-8-12	-48	-85	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9-7-17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45214-17
STRUCTURAL
COMPONENT ONLY

P62/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272270	T50Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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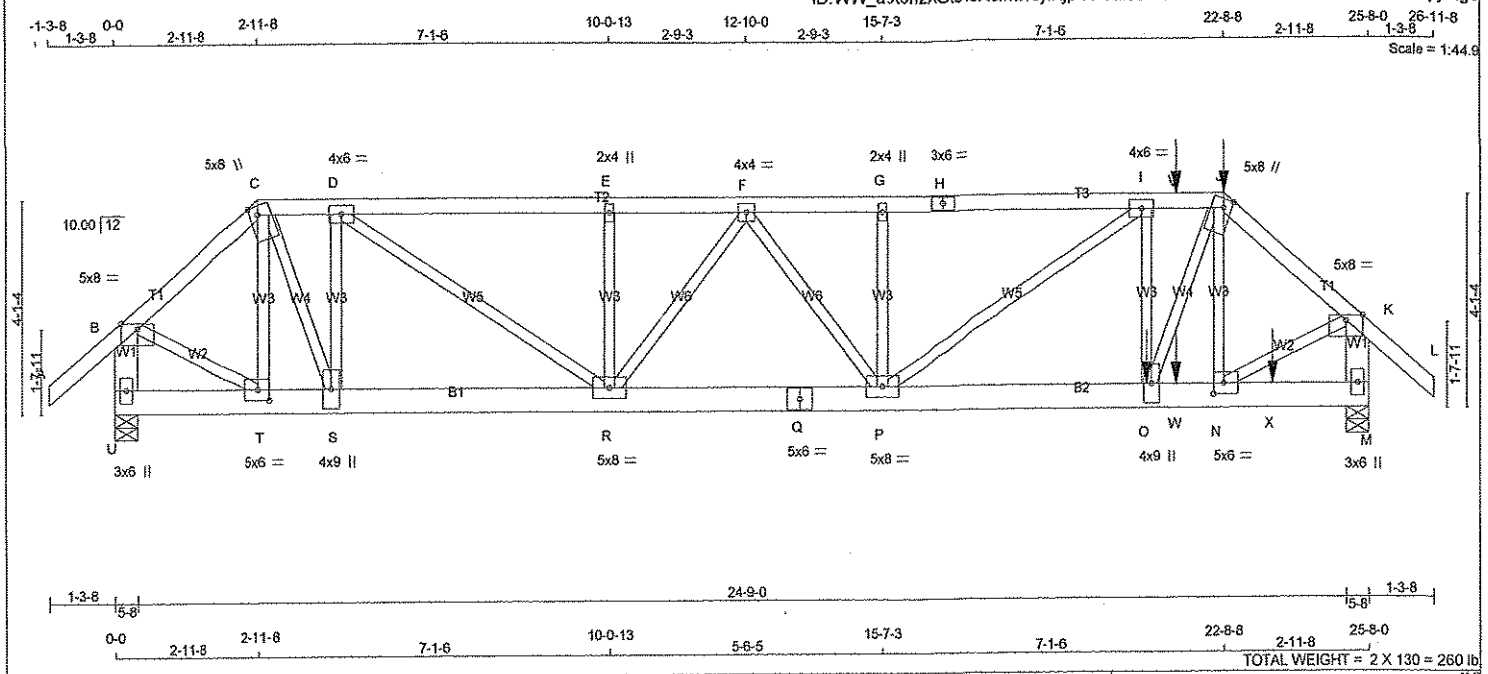

TOTAL WEIGHT = 2 X 130 = 260 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- H	1	12	TOP
H- J	1	12	SIDE(61.0)
J- L	1	12	SIDE(61.0)
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	TOP
Q- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
I- O	1	3	SIDE(601.8)
D- S	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	2.00	2.00
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	5.0	8.0	2.00	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW+t	MT20	4.0	9.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2703	0	2703	0	0	5-8	5-8
M	5062	0	5062	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2085	1408 / 0	340 / 0	0 / 0	0 / 0	337 / 0	0 / 0
M	3902	2638 / 0	634 / 0	0 / 0	0 / 0	630 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00		T- C	-608 / 0	0.08 (1)
B- C	-2471 / 0	-122.2	-122.2	0.12 (1)	5.62		C- S	0 / 2221	0.27 (1)
C- D	-2746 / 0	-122.2	-122.2	0.22 (1)	5.27		S- D	-1975 / 0	0.25 (1)
D- E	-4842 / 0	-122.2	-122.2	0.33 (1)	4.11		O- I	-925 / 0	0.12 (1)
E- F	-4842 / 0	-122.2	-122.2	0.25 (1)	4.14		O- J	0 / 4024	0.50 (1)
F- G	-5769 / 0	-122.2	-122.2	0.30 (1)	3.80		N- J	-1025 / 0	0.13 (1)
G- H	-5769 / 0	-122.2	-122.2	0.38 (1)	3.79		B- T	0 / 2037	0.25 (1)
H- I	-5769 / 0	-122.2	-122.2	0.38 (1)	3.79		N- K	0 / 4101	0.51 (1)
I- V	-5360 / 0	-122.2	-122.2	0.27 (1)	3.94		P- I	0 / 491	0.06 (1)
V- J	-5360 / 0	-122.2	-122.2	0.27 (1)	3.94		D- R	0 / 2527	0.31 (1)
J- K	-4976 / 0	-122.2	-122.2	0.19 (1)	4.18		P- G	-594 / 0	0.08 (1)
K- L	0 / 54	-122.2	-122.2	0.09 (1)	10.00		R- E	-585 / 0	0.07 (1)
U- B	-2695 / 0	0.0	0.0	0.10 (1)	7.81		R- F	-882 / 0	0.16 (1)
M- K	-5072 / 0	0.0	0.0	0.19 (1)	6.47		F- P	0 / 674	0.08 (1)
U- T	0 / 0	-28.0	-28.0	0.04 (1)	10.00				
T- S	0 / 1875	-28.0	-28.0	0.16 (1)	10.00				
S- R	0 / 2748	-28.0	-28.0	0.23 (1)	10.00				
R- Q	0 / 5368	-28.0	-28.0	0.40 (1)	10.00				
Q- P	0 / 5368	-28.0	-28.0	0.40 (1)	10.00				
P- O	0 / 5362	-28.0	-28.0	0.44 (1)	10.00				
O- W	0 / 3780	-28.0	-28.0	0.33 (1)	10.00				
W- N	0 / 3780	-28.0	-28.0	0.33 (1)	10.00				
N- X	0 / 0	-28.0	-28.0	0.08 (1)	10.00				
X- M	0 / 0	-28.0	-28.0	0.08 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
O	21-1-8	-3009	-3009	---	BACK	VERT	TOTAL
V	21-8-12	-238	-238	---	BACK	VERT	TOTAL
W	21-8-12	-48	-85	---	BACK	VERT	TOTAL
X	23-8-12	-48	-85	---	BACK	VERT	TOTAL

PROFESSIONAL ENGINEER PROVINCE OF ONTARIO 9-7-17 S. KATSOULAKOS

DWG NO. TAM 45225 -17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.38 (G-I:1) , BC=0.44 (O-P:1) , WB=0.51 (K-N:1) , SSI=0.17 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.47 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272270	T50Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 22-8-8, AND 237.9 lbs FACTORED DOWN AT 21-8-12 ON TOP CHORD, AND 3008.7 lbs FACTORED DOWN AT 21-1-8, AND 84.6 lbs FACTORED DOWN AT 21-8-12, AND 84.6 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9277
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45215-17 PGM
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42057	DRWG NO.
272272	T50Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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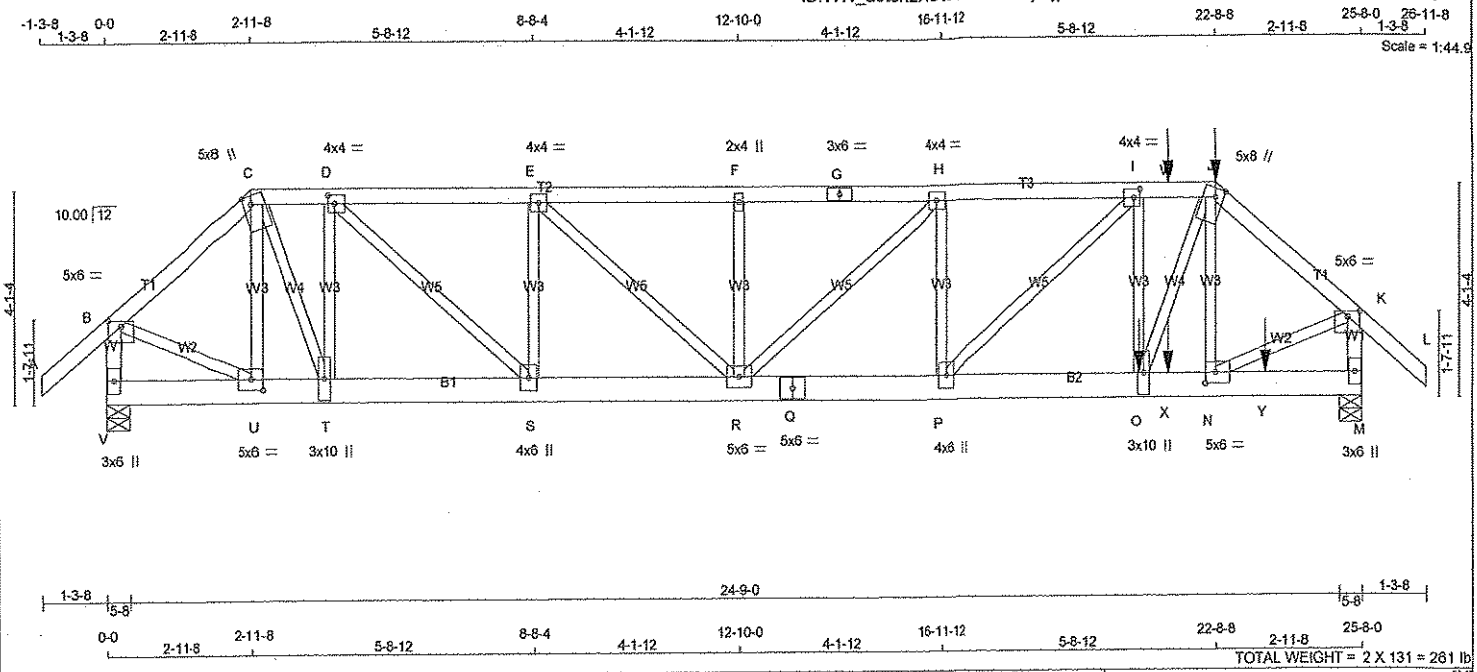

TOTAL WEIGHT = 2 X 131 = 261 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	1	12	TOP
G- J	1	12	SIDE(61.0)
J- L	1	12	SIDE(61.0)
V- B	1	12	TOP
M- K	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- Q	2	12	TOP
Q- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
I- O	1	4	SIDE(466.4)
D- T	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	5.0	8.0	2.00	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TTWW+m	MT20	5.0	8.0	2.00	2.00
K	TMVW-p	MT20	5.0	6.0	1.25	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW+t	MT20	3.0	10.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BMWW+t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2596	0	2596	0	0	5-8	5-8
M	4544	0	4544	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2010	1344 / 0	336 / 0	0 / 0	0 / 0	330 / 0	0 / 0
M	3536	2332 / 0	612 / 0	0 / 0	0 / 0	591 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00	
B- C	-2353 / 0	-122.2	-122.2	0.12 (1)	5.73	
C- D	-2561 / 0	-122.2	-122.2	0.10 (1)	5.57	
D- E	-4173 / 0	-122.2	-122.2	0.21 (1)	4.49	
E- F	-5045 / 0	-122.2	-122.2	0.24 (1)	4.11	
F- G	-5045 / 0	-122.2	-122.2	0.24 (1)	4.11	
G- H	-5045 / 0	-122.2	-122.2	0.24 (1)	4.11	
H- I	-5292 / 0	-122.2	-122.2	0.25 (1)	4.02	
I- W	-4708 / 0	-122.2	-122.2	0.13 (1)	4.34	
W- J	-4708 / 0	-122.2	-122.2	0.13 (1)	4.34	
J- K	-4419 / 0	-122.2	-122.2	0.17 (1)	4.41	
K- L	0 / 54	-122.2	-122.2	0.09 (1)	10.00	
V- B	-2583 / 0	0.0	0.0	0.15 (1)	7.06	
M- K	-4544 / 0	0.0	0.0	0.26 (1)	5.60	
V- U	0 / 0	-28.0	-28.0	0.03 (1)	10.00	
U- T	0 / 1786	-28.0	-28.0	0.15 (1)	10.00	
T- S	0 / 2561	-28.0	-28.0	0.20 (1)	10.00	
S- R	0 / 4173	-28.0	-28.0	0.32 (1)	10.00	
R- Q	0 / 5292	-28.0	-28.0	0.38 (1)	10.00	
Q- P	0 / 5292	-28.0	-28.0	0.38 (1)	10.00	
P- O	0 / 4708	-28.0	-28.0	0.40 (1)	10.00	
O- X	0 / 3358	-28.0	-28.0	0.31 (1)	10.00	
X- N	0 / 3358	-28.0	-28.0	0.31 (1)	10.00	
N- Y	0 / 0	-28.0	-28.0	0.07 (1)	10.00	
Y- M	0 / 0	-28.0	-28.0	0.07 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-546 / 0	0.07 (1)
C- T	0 / 1973	0.24 (1)
T- D	-1815 / 0	0.23 (1)
O- I	-928 / 0	0.12 (1)
O- J	0 / 3436	0.43 (1)
N- J	-892 / 0	0.11 (1)
B- U	0 / 1939	0.24 (1)
N- K	0 / 3642	0.45 (1)
P- I	0 / 785	0.10 (1)
D- S	0 / 2168	0.27 (1)
P- H	-317 / 37	0.04 (1)
S- E	-1315 / 0	0.17 (1)
R- H	-333 / 0	0.09 (1)
E- R	0 / 1172	0.15 (1)
R- F	-489 / 0	0.06 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
J	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
O	21-1-8	-2490	-2490	---	BACK	VERT	TOTAL
W	21-8-12	-148	-148	---	BACK	VERT	TOTAL
X	21-8-12	-40	-70	---	BACK	VERT	TOTAL
Y	23-8-12	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.26 (K-M:1) , BC=0.40 (O-P:1) , WB=0.45 (K-N:1) , SSI=0.12 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.45 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9-7-17 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM45221-17
STRUCTURAL COMPONENT ONLY

Pg 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272272	T50Z1	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW+t	MT20	3.0	10.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 22-8-8, AND 148.2 lbs FACTORED DOWN AT 21-8-12 ON TOP CHORD, AND 2489.7 lbs FACTORED DOWN AT 21-1-8, AND 69.9 lbs FACTORED DOWN AT 21-8-12, AND 69.9 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9-7-17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45221-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272258	T90	2	1	TRUSS DESC.	T9

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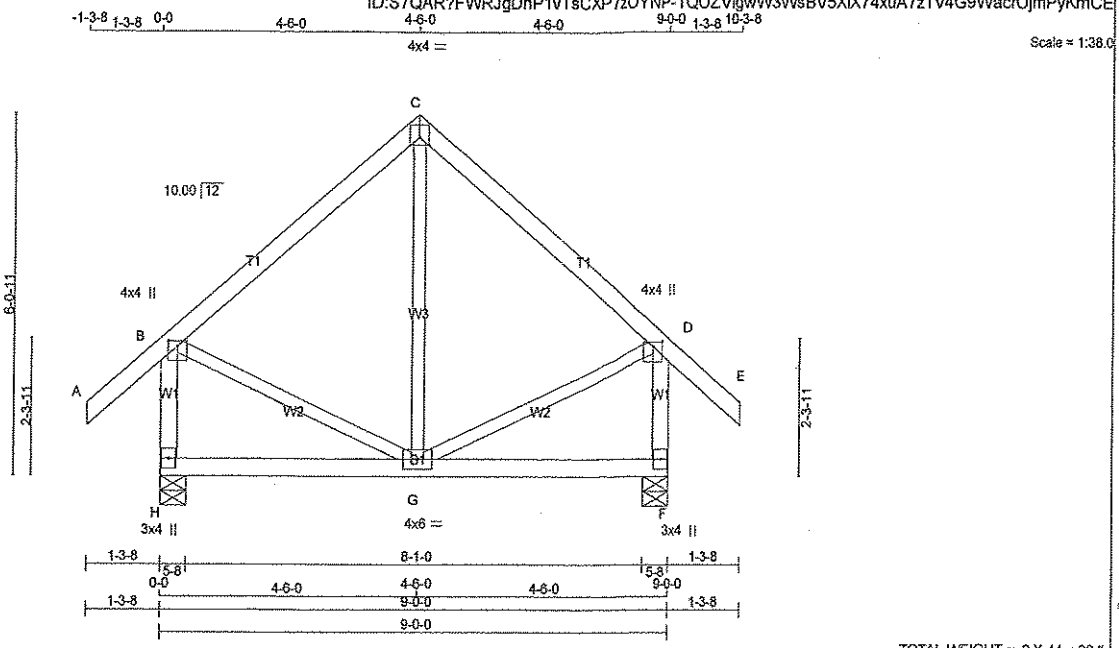


TOTAL WEIGHT = 2 X 44 = 88 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	845	0	845	0	0	5-8	1-8
F	845	0	845	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	643	450 / 0	95 / 0	0 / 0	0 / 0	98 / 0	0 / 0
F	643	450 / 0	95 / 0	0 / 0	0 / 0	98 / 0	0 / 0

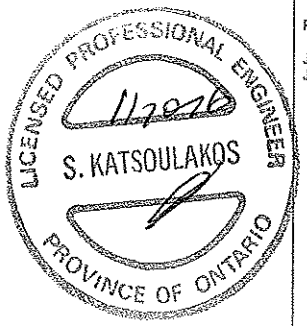
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	G- C	-88 / 168	0.05 (1)
B- C	-361 / 0	-122.2	-122.2	0.32 (1)	6.25	B- G	0 / 303	0.07 (1)
C- D	-361 / 0	-122.2	-122.2	0.32 (1)	6.25	G- D	0 / 303	0.07 (1)
D- E	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
H- B	-797 / 0	0.0	0.0	0.10 (1)	7.81			
F- D	-797 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.17 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.32 (C-D:1) , BC=0.17 (G-H:3) , WB=0.07 (B-G:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

DWG NO. TAM 49673 16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272258	G90	1	1	TRUSS DESC.	G9

Tamarack Roof Truss, Burlington

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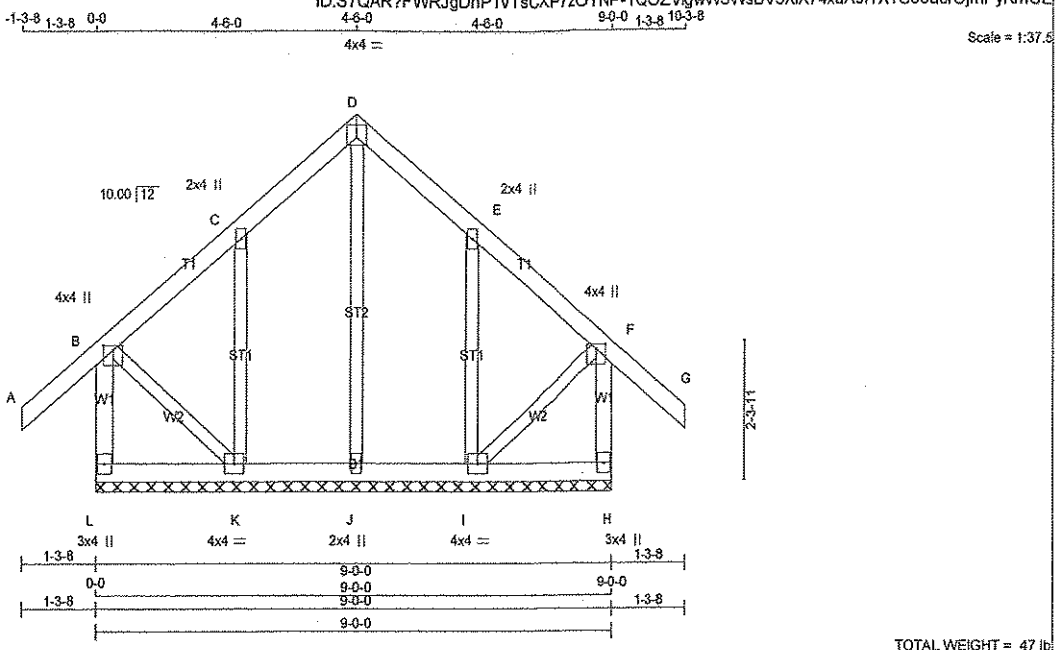


TOTAL WEIGHT = 47 lb

(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
L- B	-306 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 3	-122.2	-122.2	0.10 (1)	10.00
C- D	-30 / 0	-122.2	-122.2	0.10 (1)	6.25
D- E	-30 / 0	-122.2	-122.2	0.10 (1)	6.25
E- F	0 / 3	-122.2	-122.2	0.10 (1)	10.00
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00
H- F	-306 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 0	-28.0	-28.0	0.04 (3)	10.00
K- J	0 / 5	-28.0	-28.0	0.04 (3)	10.00
J- I	0 / 5	-28.0	-28.0	0.04 (3)	10.00
I- H	0 / 0	-28.0	-28.0	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-175 / 0	0.10 (1)
K- C	-336 / 0	0.10 (1)
I- E	-336 / 0	0.10 (1)
B- K	0 / 16	0.00 (1)
I- F	0 / 16	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.04 (H-I:3) , WB=0.10 (D-J:1) , SSI=0.11 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

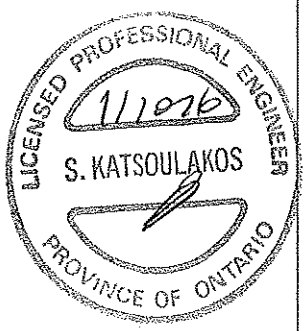
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (D) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)



DWG NO. TAM 4967616
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272257	T100	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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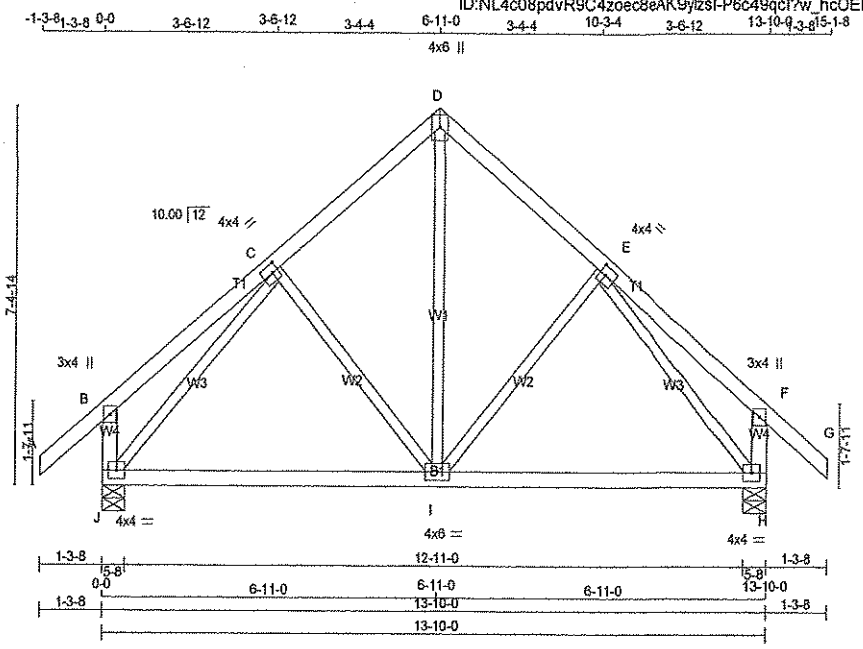

TOTAL WEIGHT = 5 X 65 = 327 lb

[M|F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1208	0	1208	0	0	5-8	1-8
H	1208	0	1208	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	927	635 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0
H	927	635 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 554	0.12 (1)
B- C	0 / 31	-122.2	-122.2	0.23 (1)	10.00	I- E	-205 / 38	0.10 (1)
C- D	-721 / 0	-122.2	-122.2	0.18 (1)	6.25	C- I	-205 / 38	0.10 (1)
D- E	-721 / 0	-122.2	-122.2	0.18 (1)	6.25	J- C	-1042 / 0	0.49 (1)
E- F	0 / 31	-122.2	-122.2	0.23 (1)	10.00	E- H	-1042 / 0	0.49 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
J- B	-330 / 0	0.0	0.0	0.04 (1)	7.81			
H- F	-330 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 662	-28.0	-28.0	0.45 (2)	10.00			
I- H	0 / 662	-28.0	-28.0	0.45 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.23 (B-C:1) , BC=0.45 (H-I:2) , WB=0.49 (C-J:1) , SSI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 1.00)

DWG NO. TAM38460216
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272257	G100	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MiTek Industries, Inc. Tue Aug 23 04:31:35 2016 Page 1

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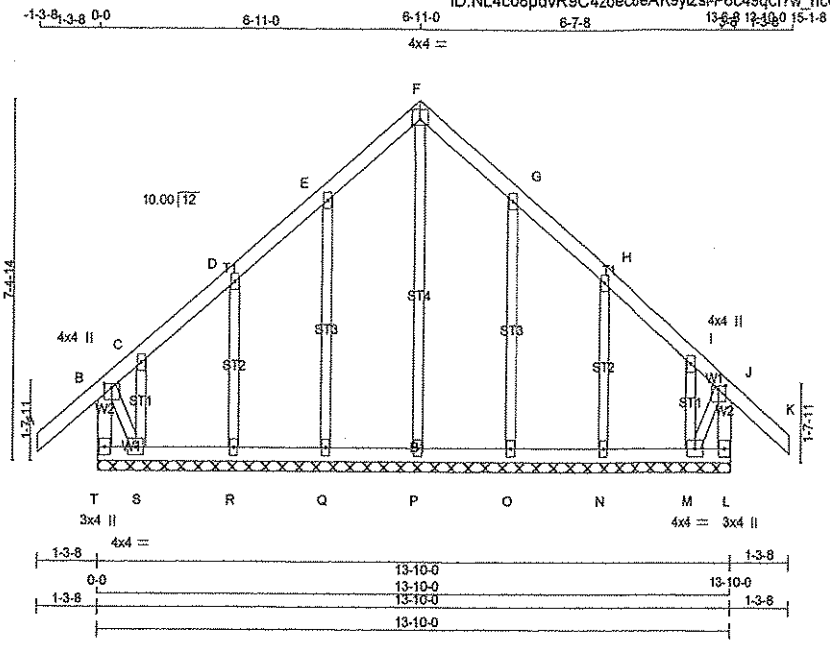


TOTAL WEIGHT = 68 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-380 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-100 / 0	-122.2	-122.2	0.16 (1)	6.25
C- D	-15 / 0	-122.2	-122.2	0.06 (1)	6.25
D- E	-11 / 0	-122.2	-122.2	0.07 (1)	6.25
E- F	-23 / 0	-122.2	-122.2	0.07 (1)	6.25
F- G	-23 / 0	-122.2	-122.2	0.07 (1)	6.25
G- H	-11 / 0	-122.2	-122.2	0.07 (1)	6.25
H- I	-15 / 0	-122.2	-122.2	0.06 (1)	6.25
I- J	-100 / 0	-122.2	-122.2	0.16 (1)	6.25
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00
L- J	-380 / 0	0.0	0.0	0.04 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 17	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 11	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 6	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 6	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 11	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 17	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-197 / 0	0.19 (1)
Q- E	-276 / 0	0.14 (1)
R- D	-246 / 0	0.06 (1)
S- C	-42 / 0	0.01 (1)
O- G	-276 / 0	0.14 (1)
N- H	-246 / 0	0.06 (1)
M- I	-42 / 0	0.01 (1)
B- S	0 / 33	0.01 (1)
M- J	0 / 33	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (J-K:1) , BC=0.02 (R-S:2) , WB=0.19 (F-P:1) , SSI=0.10 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (F) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)

DWG NO. TAM38461-16
STRUCTURAL
COMPONENT ONLY

Boise Cascade

Double 2 x 10 SPF #2

Beam\BM3

Dry | 1 span | No cantilevers | 0/12 slope (deg)

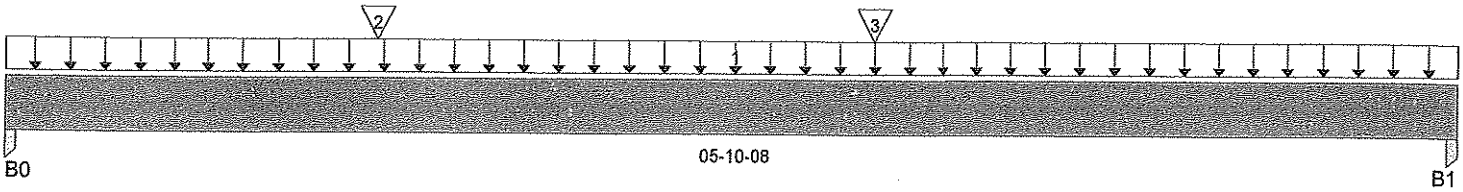
April 21, 2016 11:42:49

BC CALC® Design Report

Build 4516

Job Name:	42067/BAYVIEW WELLINGTON	File Name:	BC CALC Project
Address:	ALCONA SHORES/S32-3	Description:	Designs\BM3
City, Province, Postal Code:	INNISFIL, ON	Specifier:	
Customer:		Designer:	JG
Code reports:	NLGA	Company:	
		Misc:	

Town of Innisfil Certified Model
05/01/2018 9:19:46 AM kgervais



Total Horizontal Product Length = 05-10-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	448 / 0	450 / 0	1,782 / 0	
B1, 3-1/2"	400 / 0	402 / 0	1,598 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	Standard Load	Unf. Area (lb/ft^2)	L	00-00-00	05-10-08	11	10	39		05-06-00
2		Conc. Pt. (lbs)	L	01-06-00	01-06-00	203	203	845		n/a
3		Conc. Pt. (lbs)	L	03-06-00	03-06-00	289	289	1,275		n/a

Controls Summary

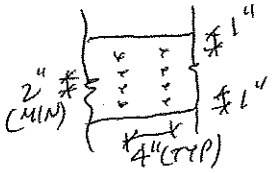
	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5,454 ft-lbs	6,040 ft-lbs	90.3%	5	03-06-00
End Shear	3,004 lbs	3,985 lbs	75.4%	5	01-00-12
Total Load Defl.	L/999 (0.066")	n/a	n/a	13	02-11-04
Live Load Defl.	L/999 (0.053")	n/a	n/a	17	02-11-04
Max Defl.	0.066"	n/a	n/a	13	02-11-04
Span / Depth	7	n/a	n/a		00-00-00

Bearing Supports

		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Post	3-1/2" x 3"	3,460 lbs	n/a	53.6%	Unspecified
B1	Post	3-1/2" x 3"	3,100 lbs	n/a	48%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Design meets User specified (1") Maximum total load deflection criteria.
Calculations assume Member is Fully Braced.
Resistance Factor phi has been applied to all presented results per CSA 086.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86.
Design based on Dry Service Condition.
The analysis of solid sawn wood members is in accordance with CSA 086 and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.
Importance Factor : Normal Part code : Part 4
Deflections less than 1/8" were ignored in the results.



PROVIDE 4 ROWS OF 3½" ARDOX SPIRAL NAILS @ 4 " O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 1 " LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

CONFORMS TO OBC 2012

DWG NO. TAM/7868-18
STRUCTURAL COMPONENT ONLY

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

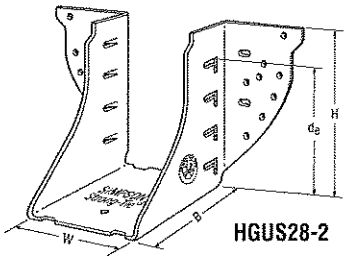
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

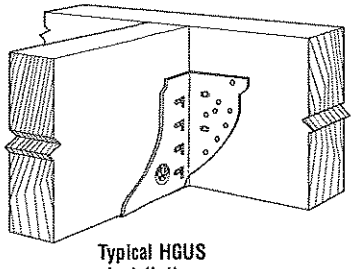
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

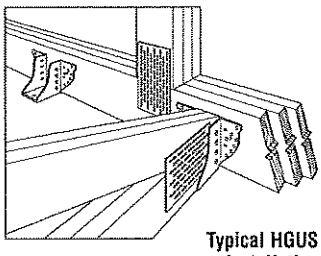
OPTIONS:

- See current catalogue for options



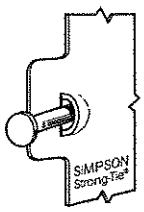
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

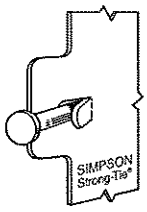
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5¾	5	4⅝₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅝₁₆	5⅞₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅜₁₆	5⅞₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅝₁₆	7⅜₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅜₁₆	7⅜₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅝₁₆	9⅜₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⅜₁₆	9⅜₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅜₁₆	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅜₁₆	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

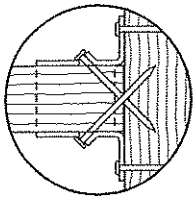


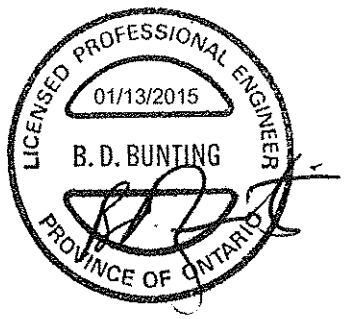
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

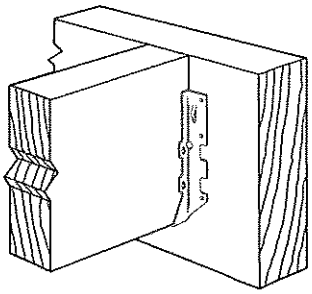
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

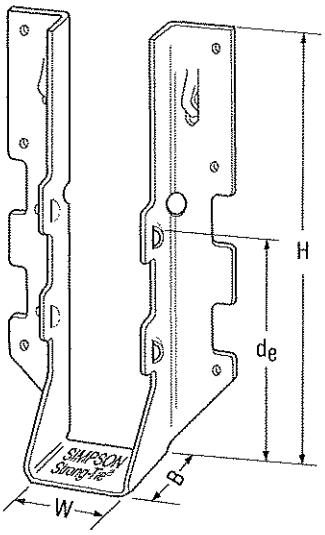
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



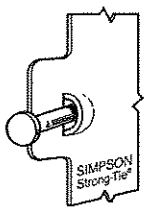
Typical LUS Installation



LUS28

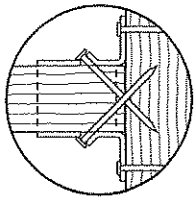
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

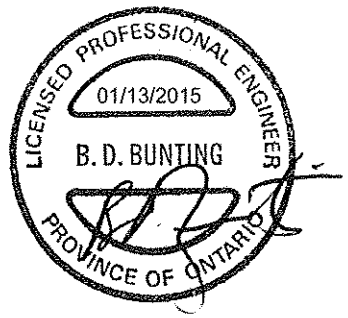


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



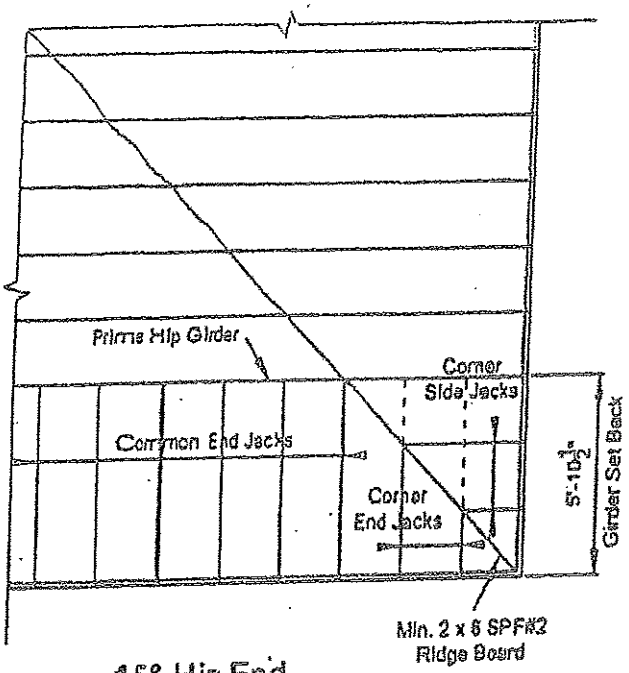
LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

T-SPECLUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

MICRO CITY
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

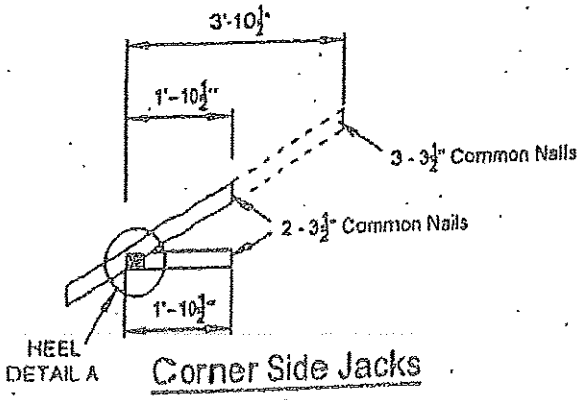
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

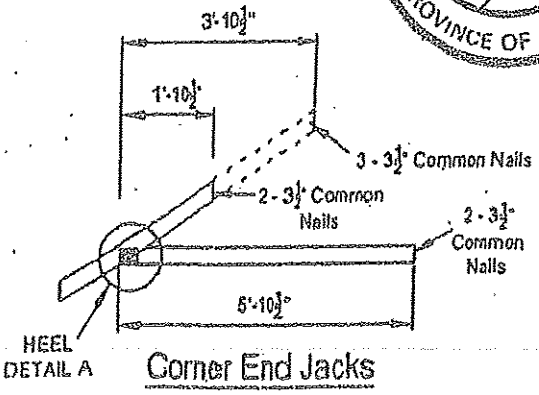
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14 R1
STRUCTURAL COMPONENT ONLY

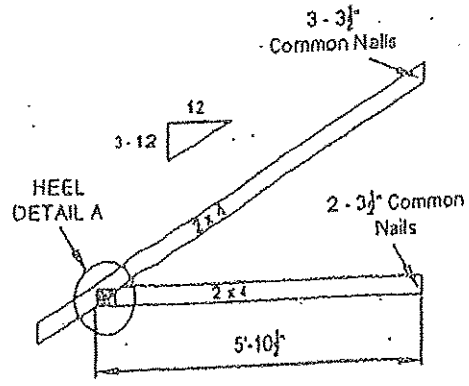
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



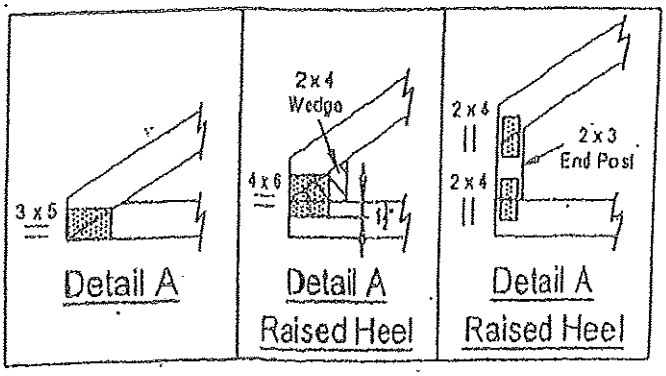
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

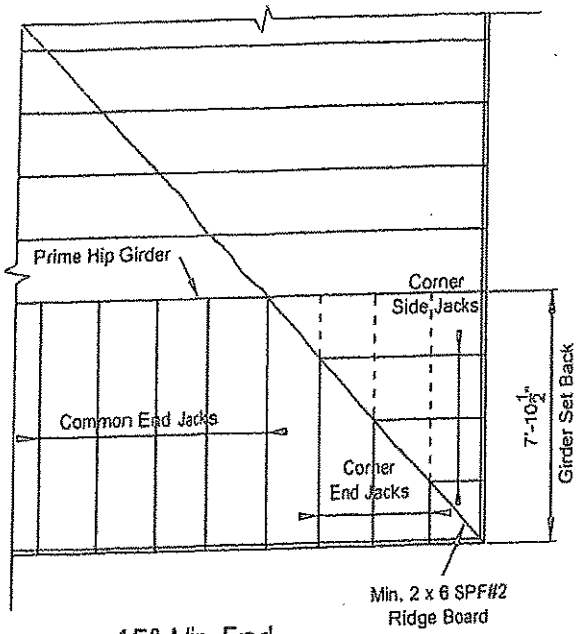
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



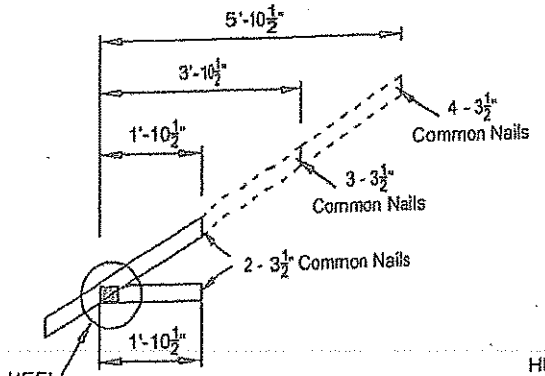
45° Hip End

DESIGN LOAD:

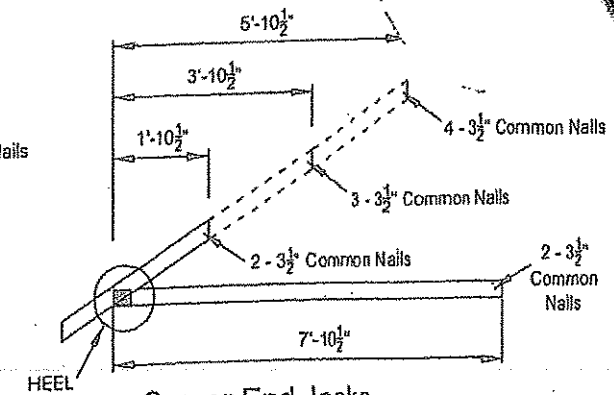
TOP CHORD LIVE LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F.

DWG NO TAM 3503.14
STRUCTURAL
COMPONENT ONLY

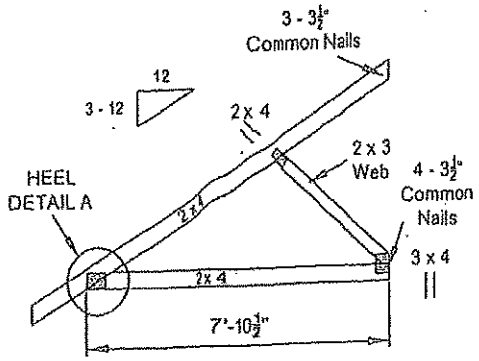
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



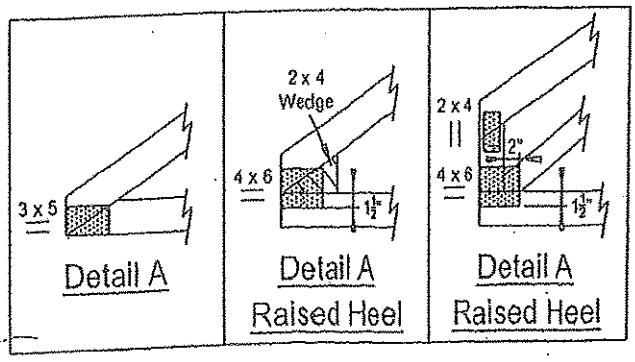
Corner Side Jacks



Corner End Jacks



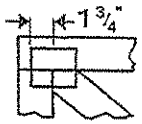
Common End Jacks



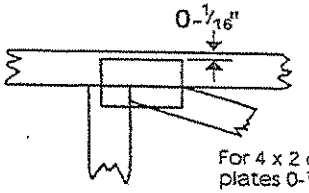
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

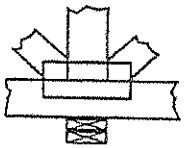
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

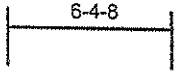
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

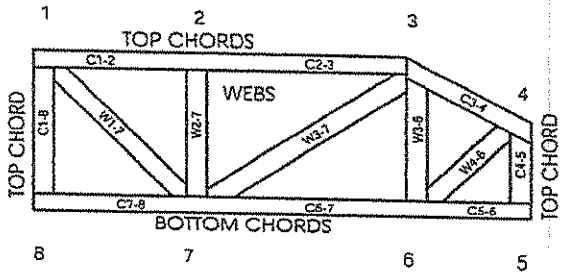
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

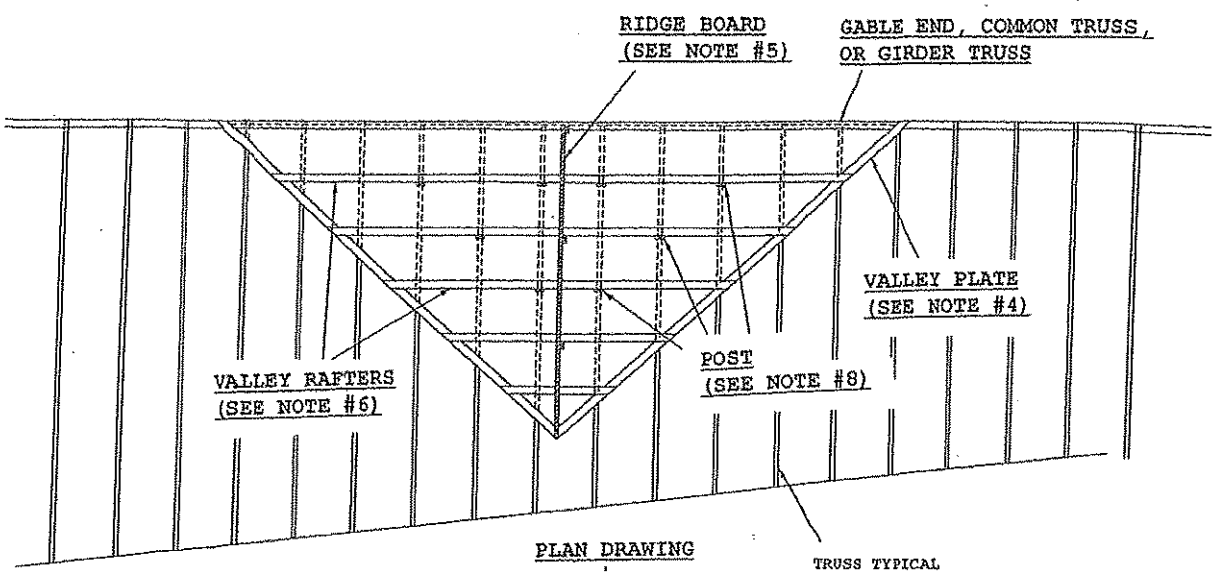
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY
ENGINEERING SERVICES INC.

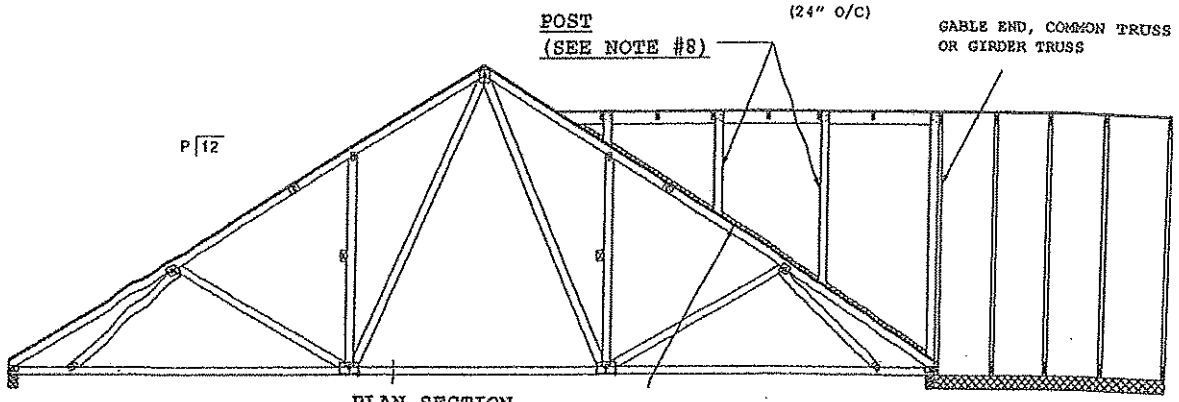
TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

TRUSS MUST
BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

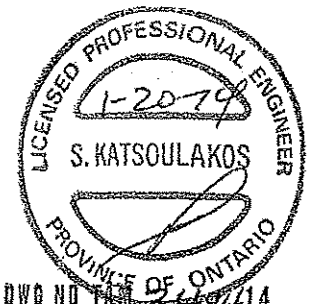
NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



DWG NO T[illegible]-[illegible]4
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.