



FROM PLAN DATED: NOV. 2015

BUILDER: BAYVIEW WELLINGTON

ALCONA SITE:

MODEL: S32-10C-15

ELEVATION: A

CITY: INNISFIL, ON

LOT:

SALESMAN: MARIO DESIGNER: CZ

REVISION: -

CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6. NOTES:

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER SQUASH BLOCKS

INTERIOR UNIFORM LOAD BEARING MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. CANTILEVERED JOISTS WALLS.

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE ENDS.

INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. REFER TO THE NORDIC

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

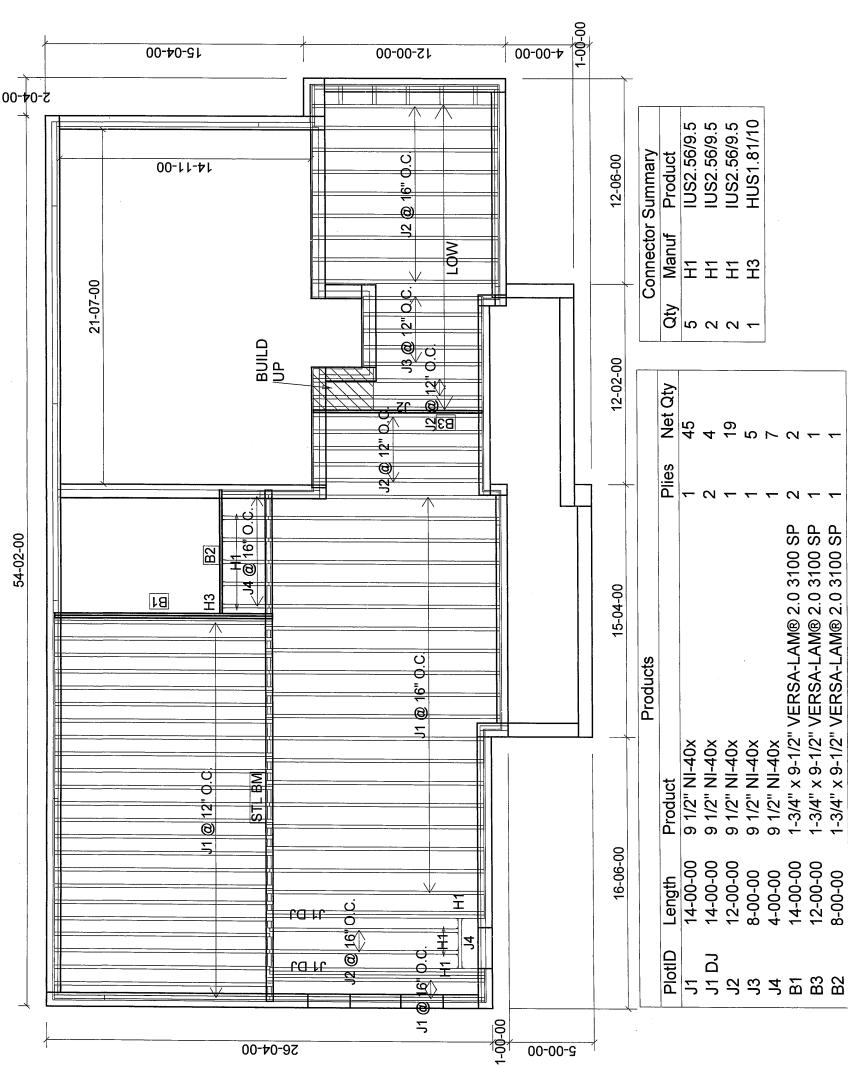
SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 11/09/2017

FLOOR **1**St

Fown of Innisfil Certified Model

15/02/2018 2:24:57 PM kgervais



00-00-9



FROM PLAN DATED: NOV. 2015

BUILDER: BAYVIEW WELLINGTON

ALCONA SITE:

MODEL: \$32-10C-15

ELEVATION: A LO1:

76-04-00

CITY: INNISFIL, ON

SALESMAN: MARIO DESIGNER: CZ **REVISION: -** NOTES:

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6. **SQUASH BLOCKS**

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. CANTILEVERED JOISTS WALLS.

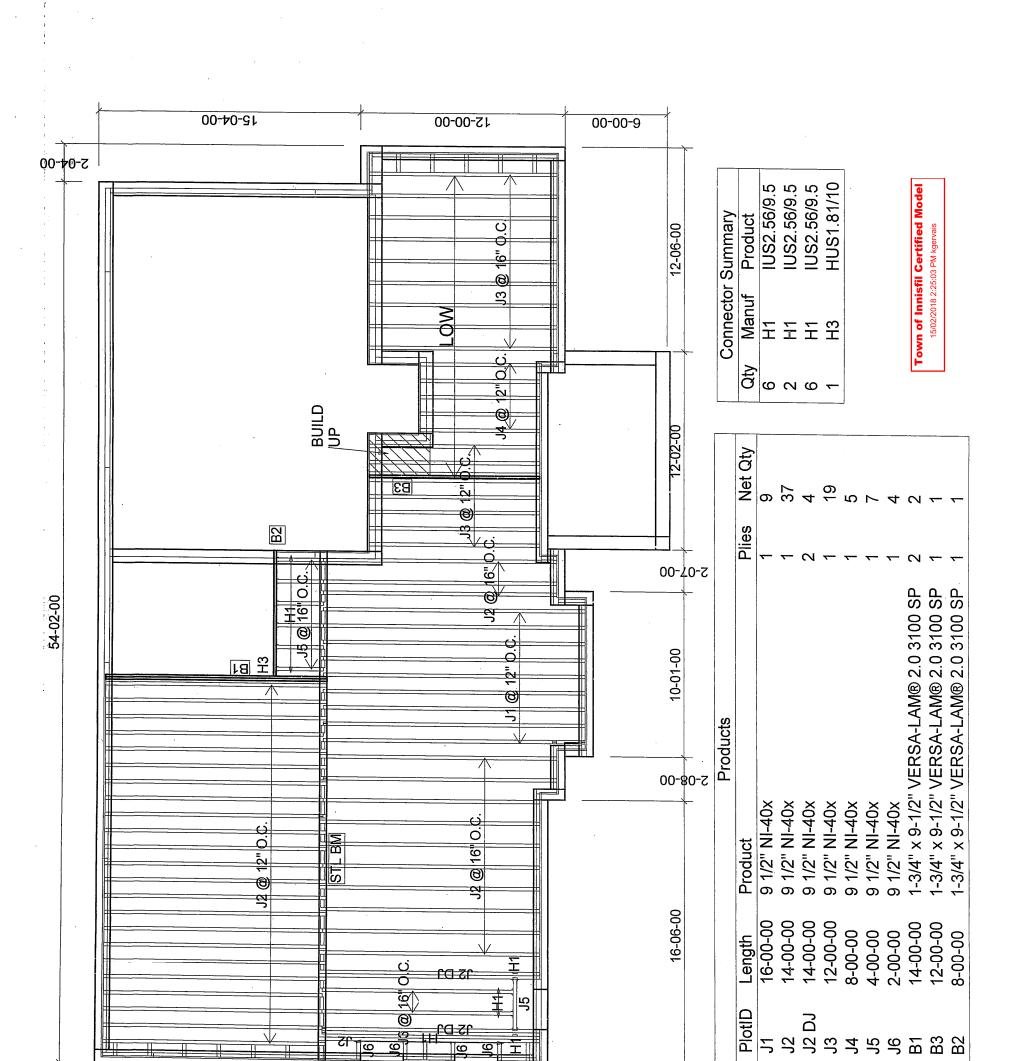
REQUIRE I-JOIST BLOCKING ALONG
BEARING AND RIMBOARD CLOSURE
AT ENDS.
REFER TO THE NORDIC
INSTALLATION GUIDE FOR PROPER
STORAGE AND INSTALLATION.

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft LOADING:

SUBFLOOR: 5/8" GLUED AND NAILED

1st FLOOR DATE: 11/09/2017

WOD



\$

26-04-00



FROM PLAN DATED: NOV. 2015

BUILDER: BAYVIEW WELLINGTON

SITE: ALCONA

MODEL: \$32-10C-15

LOT:

ELEVATION:

CITY: INNISFIL, ON

SALESMAN: MARIO **DESIGNER:** CZ **REVISION: -** NOTES:

CERAMIC TILE APPLICATION
AS PER O.B.C. 9.30.6.
SQUASH BLOCKS
2x4 OR 2x6 #2 S.P.F. REQ'D UNDER
INTERIOR UNIFORM LOAD BEARING

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. CANTILEVERED JOISTS

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE ENDS.

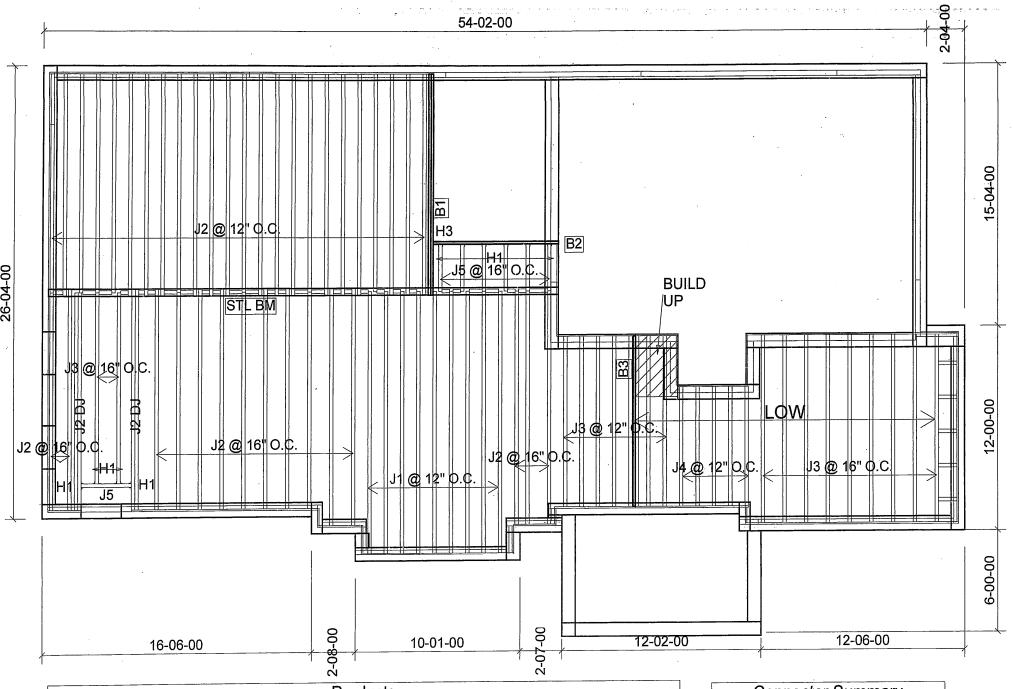
LOADING:

DESIGN LOADS: L/480.000 DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft LIVE LOAD: 40.0 lb/ft²

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 07/09/2017

FLOOR **1st**



Products							
PlotID	Length	Product	Plies	Net Qty			
J1	16-00-00	9 1/2" NI-40x	1	9			
J2	14-00-00	9 1/2" NI-40x	1	39			
J2 DJ	14-00-00	9 1/2" NI-40x	2	4			
J3	12-00-00	9 1/2" NI-40x	1	19			
J4	8-00-00	9 1/2" NI-40x	1	5			
J5	4-00-00	9 1/2" NI-40x	1	7			
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B3	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B2	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			

Connector Summary						
Qty Manuf Product						
6	H1	IUS2.56/9.5				
2	H1	IUS2.56/9.5				
2	H1	IUS2.56/9.5				
1	H3	HUS1.81/10				

Town of Innisfil Certified Model 15/02/2018 2:25:08 PM kgervais



FROM PLAN DATED: NOV. 2015

BUILDER:

BAYVIEW WELLINGTON

SITE: ALCONA

MODEL: S32-10C-15

ELEVATION: B

LOT:

CITY: INNISFIL, ON

SALESMAN: MARIO DESIGNER: CZ REVISION: -

NOTES:

CERAMIC TILE APPLICATION

AS PER O.B.C. 9.30.6. SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING

WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS.

CANTILEVERED JOISTS

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE

AT ENDS.

REFER TO THE NORDIC

INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION.

LOADING:

DESIGN LOADS: L/480.000

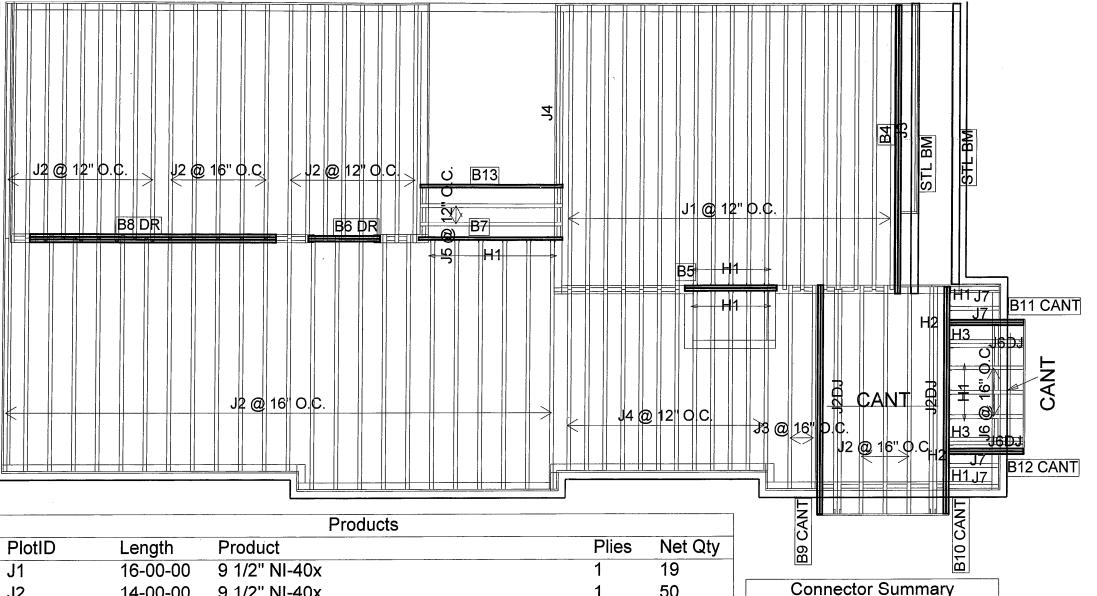
LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 07/09/2017

1st FLOOR

WOD



2

2

	•				•
J1	16-00-00	9 1/2" NI-40x	1	19	
J2	14-00-00	9 1/2" NI-40x	1	50	
J2DJ	14-00-00	9 1/2" NI-40x	2	4	
J3	12-00-00	9 1/2" NI-40x	1	3	
J4	10-00-00	9 1/2" NI-40x	1	13	
J5	8-00-00	9 1/2" NI-40x	1	2	
J6	6-00-00	9 1/2" NI-40x	1	3	
J6DJ	6-00-00	9 1/2" NI-40x	2	4	
J7	4-00-00	9 1/2" NI-40x	1	4	
B4	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	
B10 CANT	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	
B9 CANT	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	
B13	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	

1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

B7

B5

B6 DR

B8 DR

B11 CANT

B12 CANT

8-00-00 6-00-00

6-00-00

6-00-00

4-00-00

14-00-00

Connector Summary							
Qty Manuf Product							
6	H1	IUS2.56/9.5					
15	H1	IUS2.56/9.5					
2	H2	HGUS410					
2	H3	LF359					

Town of Innisfil Certified Model
15/02/2018 2:25:14 PM kgervais



FROM PLAN DATED: NOV. 2015

BUILDER:

BAYVIEW WELLINGTON

SITE: ALCONA

MODEL: S32-10C-15

ELEVATION: A

LOT:

CITY: INNISFIL, ON

SALESMAN: MARIO DESIGNER: CZ REVISION: -

NOTES:

CERAMIC TILE APPLICATION

AS PER O.B.C. 9.30.6. SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING

WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS.

CANTILEVERED JOISTS

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE

AT ENDS.

REFER TO THE NORDIC

INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION.

LOADING:

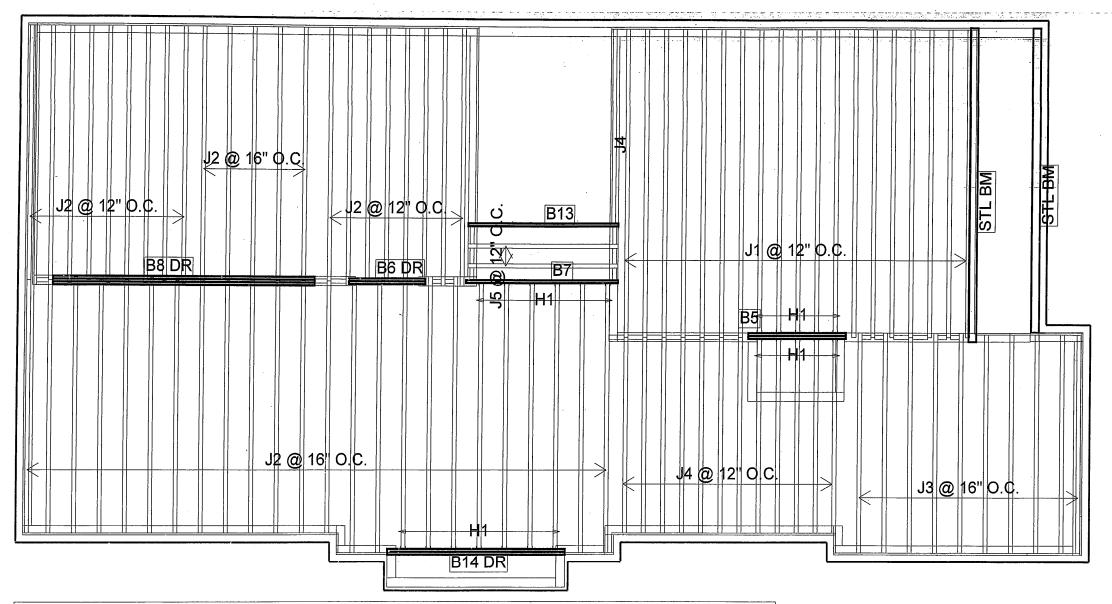
DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 11/09/2017

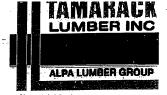
2nd FLOOR



Products						
PlotID	Length	Product	Plies	Net Qty		
J1	16-00-00	9 1/2" NI-40x	1	19		
J2	14-00-00	9 1/2" NI-40x	1	47		
J3	12-00-00	9 1/2" NI-40x	1	10		
J4	10-00-00	9 1/2" NI-40x	1	13		
J5	8-00-00	9 1/2" NI-40x	1	2		
B14 DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B13	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	. 1	1		
B7	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B6 DR	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B8 DR	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3		

Connector Summary						
Qty Manuf Product						
6	H1	IUS2.56/9.5				
17	H1	IUS2.56/9.5				

Town of Innisfil Certified Model 15/02/2018 2:25:18 PM kgervais



FROM PLAN DATED: NOV. 2015

BUILDER:

BAYVIEW WELLINGTON

SITE: ALCONA

MODEL: S32-10C-15

ELEVATION: B

LOT:

CITY: INNISFIL, ON

SALESMAN: MARIO DESIGNER: CZ REVISION: -

NOTES:

CERAMIC TILE APPLICATION

AS PER O.B.C. 9.30.6. SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING

WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS.

CANTILEVERED JOISTS

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE

AT ENDS.

REFER TO THE NORDIC

INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION.

LOADING:

DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 07/09/2017

2nd FLOOR



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i1516)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 10:52:01

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

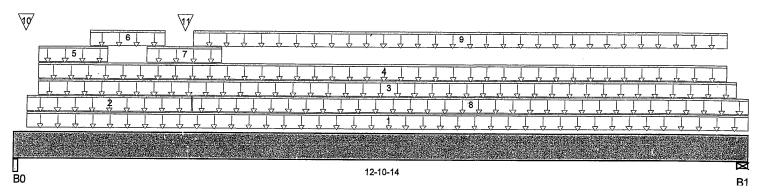
Description: Designs\Flush Beams\Basment\Flush Beams\B1(i1516

Specifier:

Designer: Company: Misc:

Town of Innisfil Certified Model

15/02/2018 2:25:28 PM kgervais



Total Horizontal Product Length = 12-10-14

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	De ad	Snow	Wind				
B0, 5"	2,233 / 0	2,151/0		٠				
B1, 2-3/8"	498/0	1.191/0						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Tag Description		Load Type	Ref	Ref. Start	End	1.00	0.65	1.00	1.15	
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-11	12-10-14	8	4			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-11	03-01-08	12				n/a
3	Us er Load	Unf. Lin. (lb/ft)	L	00-05-00	12-08-08		60			n/a
4	12(i409)	Unf. Lin. (lb/ft)	L	00-05-00	12-06-08		81			n/a
5	12(i409)	Unf. Lin. (lb/ft)	L	00-05-00	01-07-12	110	54			n/a
6	12(i409)	Unf. Lin. (lb/ft)	L	01-03-12	02-07-12	116	58			n/a
7	12(i409)	Unf. Lin. (lb/ft)	L	02-03-12	03-07-12	219	138			n/a
8	FC1 Floor Material	Unf. Lin. (lb/ft)	L	03-01-08	12-10-14	6				n/a
9	12(i409)	Unf. Lin. (lb/ft)	L	03-01-08	12-06-08	15	10			n/a
10	11(i398)	Conc. Pt. (lbs)	L	00-02-04	00-02-04	693	385			n/a
11	B2(i1543)	Conc. Pt. (lbs)	L	02-11-12	02-11-12	1.086	578			n/a

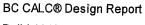
Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	10,180 ft-lbs	25,408 ft-lbs	40.1%	1	03-09-12
End Shear	4,135 lbs	11,571 lbs	35.7%	1	01-02-08
Total Load Defl.	L/369 (0.404")	0.621"	65.1%	4	06-04-02
Live Load Defl.	L/945 (0.158")	0.414"	38.1%	5	05-10-01
MaxDefl.	0.404"	n/a	n/a	4	06-04-02
Span / Depth	15.7	n/a	n/a		00-00-00

			Demand/ Resistance	Demand/ Resistance	
Bearing Supports	Dim . (L x W)	De m an d	_	Member	Material



1/6 2

DWG NO.TAM4536/-17 STRUCTURAL COMPONENT ONLY



Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i15

Specifier: Designer:

Company:

CONFORMS TO OBG 2012

Misc:

B0	Beam	5" x 3-1/2"	6,039 lbs	80.8%	28.3%	Unspecified
B1	Wall/Plate	2-3/8" x 3-1/2"	1,668 lbs	72.3%	25.3%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

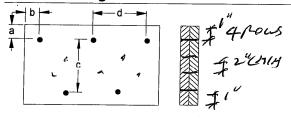
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

Connection Diagram



a minimum = 2" b minimum = 3"

Calculated Side Load = 188.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nails 🗈

Connectors are: . . .

312" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST® BC RIM BOARD™. BCk® . BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

Town of Innisfil Certified Model

15/02/2018 2:25:32 PM kgervais



DWB NO . TAM 453617 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i2405)

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 4, 2016 10:10:17

B1

Build 4340

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs \Flush Beams \Basment\Flush Beams \B2(i2405

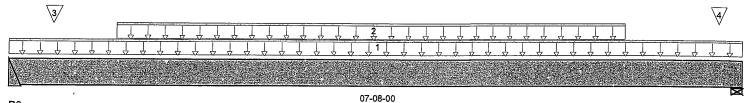
Specifier:

Designer: Company:

Misc:

Town of Innisfil Certified Model

15/02/2018 2:25:38 PM kgervais



B0

Total Horizontal Product Length = 07-08-00

Reaction Summary (Down / Uplift) (lbs)							
Be aring	Live	De ad	Snow	Wind			
B0	1,092 / 0	563/0					
B1.5-1/2"	1779/0	1739/0					

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description *	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
1	Us er Load	Unf. Lin. (lb/ft)	L	-00-00-00	07-08-00	240	120			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-01-08	06-05-08	58	29			n/a
3	J5(i2247)	Conc. Pt. (lbs)	L	00-05-08	00-05-08	57	28			n/a
4	-	Conc. Pt. (lbs)	L	07-05-01	07-05-01	662	1,161			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	4,088 ft-lbs	12,704 ft-lbs	32.2%	1	03-07-08
End Shear	1,771 lbs	5,785 lbs	30.6%	1	00-11-08
Total Load Defl.	L/999 (0.107")	n/a	n/a	4	03-08-08
Live Load Defl.	L/999 (0.071")	n/a	n/a	5	03-08-08
MaxDefl.	0.107"	n/a	n/a	4	03-08-08
Span / Depth	9.1	n/a	n/a		00-00-00

Bearii	ng Supports	Dim . (L x W)	De m an d	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Hanger	2" x 1-3/4"	2,342 lbs	n/a	54.8%	HUS1.81/10
B1	Wall/Plate	5-1/2" x 1-3/4"	4,842 lbs	94.2%	41.2%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSAO86.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

DWG NO. TAM 45362-17 STRUCTURAL COMPONENT ONLY

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALO®, BC FRAMER®, AJS™, ALLJOIST®, BCRIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®,

VERSA-STRAND®, VERSA-STUD® are trademarks of Boise-Cascade Wood





Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i1540)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 10:52:01

Build 4340

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i1540

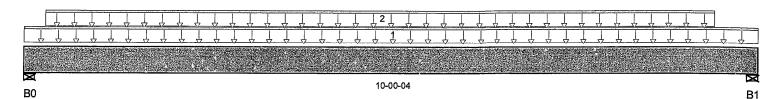
Specifier:

Designer: Company:

Misc:

Town of Innisfil Certified Model

15/02/2018 2:25:45 PM kgervais



Total Horizontal Product Length = 10-00-04

Reaction Summary	(Down / Uplift) (lbs)				
Be aring	Live	De ad	Snow	Wind	
B0, 2-3/8"	138/0	371/0			
B1, 4-3/8"	142/0	367/0			

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description *	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-00-04	28	14			n/a
2	Us er Load	Unf. Lin. (lb/ft)	L	00-03-07	09-05-06		60			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Mornent	1,264 ft-lbs	8,258 ft-lbs	15.3%	0	04-11-02
End Shear	493 lbs	3,761 lbs	13.1%	0	00-11-14
Total Load Defl.	L/999 (0.081")	n/a	n/a	4	04-11-02
Live Load Defl.	L/999 (0.021")	n/a	n/a	5	04-11-02
MaxDefl.	0.081"	n/a	n/a	4	04-11-02
Span / Depth	12.1	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	De m an d	De m an d/ Re s istance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	2-3/8" x 1-3/4"	519 lbs	45%	15.8%	Unspecified
B1	Wall/Plate	4-3/8" x 1-3/4"	514 lbs	24.2%	8.5%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

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Deflections less than 1/8" were ignored in the results.

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

Disclosure

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DWO NO.TAM 4536多17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B4(i1440)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 10:52:01

Build 4340

Job Name:

Address: City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

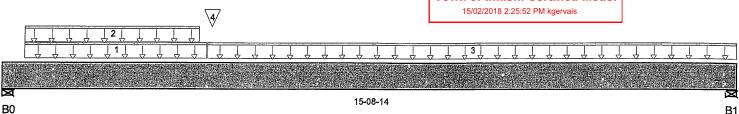
Description: Designs\Flush Beams\1st Floor\Flush Beams\84(i1440)

Specifier:

Designer: Company:

Misc:





Total Horizontal Product Length = 15-08-14

Reaction Summary (Down / Uplift) (lbs)										
Be aring	Live	De ad	Snow	Wind						
B0, 5-1/2"	431/0	821/0	997/0							
B1, 4-3/8"	283/0	327/0	259/0							

Lo	ad Summary					Live	De ad	Snow	Wind	Trib.
	g Description	Load Type	Ref	f. Start	End	1.00	0.65	1.00	1.15	
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-05-08	04-04-08	8	4			n/a
2	Us er Load	Unf. Lin. (lb/ft)	L	00-05-08	04-02-10	44	140	152		n/a
3	FC2 Floor Material	Unf. Lin. (lb/ft)	L	04-04-08	15-08-14	28	14			n/a
4	Us er Load	Conc. Pt. (lbs)	L	04-05-10	04-05-10	198	294	684		n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	7,538 ft-lbs	25,408 ft-lbs	29.7%	13	04-05-10
End Shear	2,375 lbs	11,571 lbs	20.5%	13	01-03-00
Total Load Defl.	L/471 (0.383")	0.752"	51%	45	07-01-14
Live Load Defl.	L/781 (0.231")	0.501"	46.1%	61	07-01-14
MaxDefl.	0.383"	n/a	n/a	45	07-01-14
Span / Depth	19	n/a	n/a		00-00-00

				Resistance	Resistance	
Bea	ring Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	2,737 lbs	33.3%	11.7%	Unspecified
B1	Wall/Plate	4-3/8" x 3-1/2"	962 lbs	14.7%	5.2%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

Deflections less than 1/8" were ignored in the results.



PWEND. FRW 4536417 STRUCTURAL COMPONERT ONLY BC CALC® Design Report

*

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

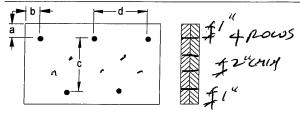
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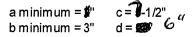
Description: Designs\Flush Beams\1st Floor\Flush Beams\B4(i144

Specifier: Designer: Company:

Misc:

Connection Diagram





Calculated Side Load = 0.1 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Nails
3½" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

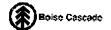
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Town of Innisfil Certified Model

15/02/2018 2:25:56 PM kgervais



COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B5(i1651)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 10:52:02

Build 4340

Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

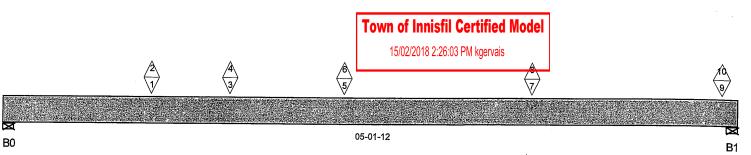
Description: Designs\Flush Beams\1st Floor\Flush Beams\B5(i1651)

Specifier:

Designer: Company:

Misc

Misc:



	÷	Total H	lorizontal Pro	duct Length =	05-01-12	2			
Reaction Summary	(Down / Uplift) ()	bs)	· · · · · · · · · · · · · · · · · · ·						
Be aring	Live	, De a	ıd	Snow		Wind			
B0, 5-1/2"	808/	125 367	7/0						
B1, 6-1/4"	1,135	/ 139 524	/0						
Load Summary					Live	Dead	Snow	Wind	Trib.
Tag Description	Load Ty	pe	Ref. Start	En d	1.00	0.65	1.00	1.15	
1 -	Conc. P	t. (lbs)	L 01-00-	01-00-0	7 466	204			n/a
2 -	Conc. P	t. (lbs)	L 01-00-	07 01-00-0	7 -51				n/a
3 J5(i1424)	Conc. P	t. (lbs)	L 01-07-0	00 01-07-0	0 60				n/a
4 J5(i1424)	Conc. P	t. (lbs)	L 01-07-0	00 01-07-0	0 -51				n/a
5 -	Conc. P	t. (lbs)	L 02-04-0	09 02-04-0	9 468	204			n/a
6 -	Conc. P	t. (lbs)	L 02-04-0	09 02-04-0	9 -51				n/a
7 -	Conc. P	t. (lbs)	L 03-08-0	08-08-0	8 468	204			n/a
8 -	Conc. P	t. (lbs)	L 03-08-0	03-08-0	8 -51				n/a
9 -	Conc. Pt	t. (lbs)	L 05-00-0	7 05-00-0	7 477	204			n/a
10 -	Conc. Pt	t. (lbs)	L 05-00-0	7 05-00-0	7 -60				n/a
	Factored	Factored	Demand /	Load	Loca	tion			
Controls Summary	Demand	Resistance	Resistand	e Case					
Pos. Moment	1,864 ft-lbs	25,408 ft-lbs	7.3%	1		02-04-08			
End Shear	1,370 lbs	11,571 lbs	11.8%	1		03-10-00			
Total Load Defl.	L/999 (0.008")	n/a	n/a	6		02-06-06			
Live Load Defl.	L/999 (0.006")	n/a	n/a	8		02-06-06			
MaxDefl.	0.008"	n/a	n/a	6		02-06-06			
Span / Depth	5.4	n/a	n/a			00-00-00			

Bear	ing Supports	Dim . (L x W)	De m an d	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	1,670 lbs	20.3%	7.1%	Unspecified
B1	Wall/Plate	6-1/4" x 3-1/2"	2,358 lbs	25.2%	8.8%	Unspecified

Notes



BWO NO. TAM 4536517 STRUCTURAL COMPONENT ONLY BC CALC® Design Report

*

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B5(i165

Specifier: Designer:

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

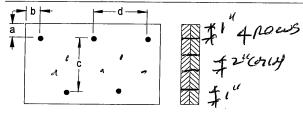
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9
Deflections less than 1/8" were ignored in the results.

Connection Diagram



a minimum = $\frac{1}{2}$ " c= $\frac{3}{2}$ -1/2" d = $\frac{3}{6}$

Calculated Side Load = 697.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Nails
3½ ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Town of Innisfil Certified Model

15/02/2018 2:28:38 PM kgervais



OWE NO. TAM 45-36517 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B6 DR(i1276)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 10:52:03

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B6

Specifier:

Designer: Company:

Misc:

<u> </u>	2/	3	4	5
B0		04-00-00		B1

Total Horizontal F	roduct Len	gth = 04-00-00
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Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0, 4"	1,035 / 0	535/0							
B1, 4"	1,035 / 0	535/0							

Lc	ad Summary					Live	Dead	Snow	Wind	Trib.
Tag Description	Load Type	Ref	ef. Start End 1	1.00	0.65	1.00	1.15			
1	J2(i1216)	Conc. Pt. (lbs)	L	00-03-08	00-03-08	357	178			n/a
2	J2(i1218)	Conc. Pt. (lbs)	L	00-11-08	00-11-08	249	124			n/a
3	-	Conc. Pt. (lbs)	L	01-09-02	01-09-02	606	302			n/a
4	-	Conc. Pt. (lbs)	L	02-11-08	02-11-08	609	304			n/a
5	J2(i1294)	Conc. Pt. (lbs)	L	03-11-08	03-11-08	249	124			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	1,681 ft-lbs	25,408 ft-lbs	6.6%	1	01-11-08
End Shear	1,544 lbs	11,571 lbs	13.3%	1	02-10-08
Total Load Defl.	L/999 (0.005")	n/a	n/a	4	02-00-03
Live Load Defl.	L/999 (0.003")	n/a	n/a	5	02-00-03
Max Defl.	0.005"	n/a	n/a	4	02-00-03
Span / Depth	4.4	n/a	n/a		00-00-00

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15/02/2018 2:28:42 PM kgervais

Bearing Supports		Dim . (L x W)	De man d	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	4" x 3-1/2"	2,221 lbs	24.4%	13%	Unspecified
B1	Wall/Plate	4" x 3-1/2"	2,222 lbs	24.4%	13%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

 $\label{lem:calculation} \textbf{Calculation assumes member is partially braced}. \textbf{See engineering report for the unbraced length} \; .$

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBG 2012



DWO NO.TAM 4536617 STRUCTURAL COMPONENT ONLY BC CALC® Design Report

*

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

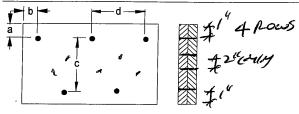
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B

Specifier:

Designer: Company:

Misc:

Connection Diagram



Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Nails

312" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B7(i1553)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 10:52:02

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B7(i1553)

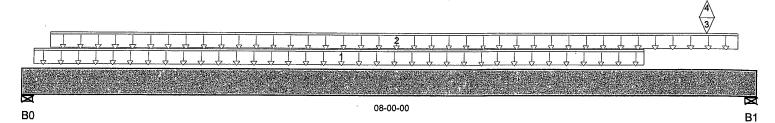
Specifier:

Designer: Company:

Misc:

Town of Innisfil Certified Model

15/02/2018 2:28:50 PM kgervais



Total Horizontal Product Length = 08-00-00

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	De ad	Snow	Wind				
B0, 6-1/2"	1,112/0	574/0						
B1, 5-1/2"	1,102/2	568/0						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-01-08	06-09-08	270	134			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-03-09	07-09-11	16	8			n/a
3	J2(i1529)	Conc. Pt. (lbs)	L	07-05-08	07-05-08	298	148			n/a
4	J2(i1529)	Conc. Pt. (lbs)	L	07-05-08	07-05-08	-2				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,843 ft-lbs	12,704 ft-lbs	30.2%	1	03-05-08
End Shear	1,818 lbs	5,785 lbs	31.4%	1	01-04-00
Total Load Defl.	L/999 (0.099")	n/a	n/a	6	04-00-08
Live Load Defl.	L/999 (0.066")	n/a	n/a	8	04-00-08
MaxDefl.	0.099"	n/a	n/a	6	04-00-08
Span / Depth	9	n/a	n/a		00-00-00

Bea	ring Supports	Dim . (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	6-1/2" x 1-3/4"	2,386 lbs	49.1%	17.2%	Unspecified
B1	Wall/Plate	5-1/2" x 1-3/4"	2,363 lbs	57.5%	20.1%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSAO86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

DWOND.TAM 45367-17 STRUCTURAL COMPONENT ONLY

CONFORMS TO OBG 2012

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8 DR(i1221)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 2, 2016 10:52:03

BC CALC® Design Report



Build 4340 Job Name:

Address:

City, Province, Postal Code:, Customer:

Code reports:

CCMC 12472-R

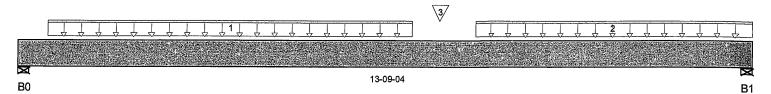
File Name: S32-10C-15.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B8

Specifier: Designer: Company: Misc:

Town of Innisfil Certified Model

15/02/2018 2:28:54 PM kgervais



Total Horizontal Product Length = 13-09-04

Reaction Summary (Down / Uplift) (Ibs)								
Be aring	Live	De ad	Snow	Wind				
B0, 4"	3,211 / 0	1,730 / 0						
B1, 4"	3,503 / 0	1,877 / 0						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-06-08	07-04-08	499	250			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	08-06-08	13-09-04	512	256			n/a
3	-	Conc. Pt. (lbs)	L	07-10-08	07-10-08	627	314			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	23,832 ft-lbs	60,415 ft-lbs	39.4%	1	06-10-08
End Shear	6,630 lbs	21,696 lbs	30.6%	1	01-03-14
Total Load Defl.	L/438 (0.362")	0.661"	54.7%	4	06-10-08
Live Load Defl.	L/674 (0.236'')	0.441"	53.4%	5	06-10-08
MaxDefl.	0.362"	n/a	n/a	4	06-10-08
Span / Depth	13.4	n/a	n/a		00-00-00

Beari	ng Supports	Dim . (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Wall/Plate	4" x 5-1/4"	6,980 lbs	51.2%	27.2%	Unspecified
B1	Wall/Plate	4" x 5-1/4"	7,601 lbs	55.7%	29.7%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012



DWO NO . TAM 4536917 STRUCTURAL COMPONENT ONLY

Dry | 1 span | No cantilevers | 0/12 slope (deg)

BC CALC® Design Report

Build 4340

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Job Name:

City, Province, Postal Code: Address:

Customer:

Code reports:

Connection Diagram

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B

Specifier:

Designer:

Company:

MSC.

c = 6.7/8" 6''þ 0 a minimum =**≵**" b minimum = 3" 100 0

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

e minimum = 3"

Member has no side loads.

Connectors are: 16d

ARDOX SPIRAL Nails 31/8

Completeness and accuracy of input must be verified by anyone who would rely on building codes. To obtain Installation Guide particular application. Output here based current Installation Guide and applicable Installation of BOISE engineered wood products must be in accordance with output as evidence of suitability for 1-800-964-6999 before installation. on building code-accepted design properties and analysis methods. or ask questions, please call Disclosure

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Town of Innisfil Certified Model

15/02/2018 2:28:59 PM kgervais

S. KATSOULAKOS DWB NO. TAM 4536817 STRUCTURAL COMPONERT



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B9 CANT(i1642)

Dry | 2 spans | Left cantilever | 0/12 slope (deg)

August 2, 2016 10:52:02

BC CALC® Design Report

Build 4340 Job Name:

Addres s: City, Province, Postal Code:,

Customer:

CCMC 12472-R

Code reports:

File Name: S32-10C-15.mmdl Description: Designs\Flush Beams\1st Floor\Flush Beams\B9 CANT_t

Specifier: Designer: Company:

Msc.

Town of Innisfil Certified Model 15/02/2018 2:29:04 PM kgervais

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Reaction Summary (Dow	Jown / Uplift) (lbs)	6		7	
8	LIVE	Dean	Mone	Mind	
B1, 5-1/2"	264/0	416/0	280/0		
B2, 5-1/2"	147/7	111/0	0 / 16		

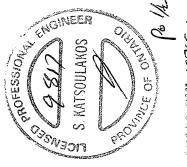
Total Horizontal Product Length = 12-05-14

•			• - -	•)					
ľ	oad Summary					Live	Dead	Snow	Snow Wind	Trib.
Tag	Description	Load Type	22	Ref. Start End	Ē	1.00	0.65	1.00 1.15	1.15	
~	FC2 Floor Material	Unf. Lin. (lb/ft)		L 00-00-00 12-03-04 20	12-03-04	20	10			n/a
7	UserLoad	Unf. Lin. (lb/ft)	_	00-00-00	01-08-14	44	140	152		n/a
က	FC2 Floor Material	Unf. Lin. (lb/ft)		00-00-00	01-03-06	9				n/a
4	FC2 Floor Material	Unf. Lin. (lb/ft)		01-03-06 12-03-04 7	12-03-04	7	4			n/a

	Factored	Factored	Demand /	Load	Location	
Controls Summary	Demand	Resistance	Resistance	Case		
Pos. Moment	853 ft-lbs	25,408 ft-lbs	3.4%	45	07-01-14	
Neg. Moment	-531 ft-lbs	-25,408 ft-lbs	2.1%	49	01-06-02	
Neg. Moment	-531 ft-lbs	-25,408 ft-lbs	2.1%	49	01-06-02	
End Shear	284 lbs	11,571 lbs	2.5%	45	11-02-14	
Cont. Shear	334 lbs	11,571 lbs	2.9%	13	02-06-06	
Total Load Deff.	1/999 (0.024")	n/a	n/a	108		
Live Load Defl.	1/999 (0.015")	n/a	n/a	160		
Total Neg. Deff.	2xL/1,998 (-0.009",)9") n/a	n/a	108		
MaxDefl.	0.024"	n/a	n/a	108		
Span / Depth	13.4	n/a	n/a		00-00-00	

	Material	Unspecified Unspecified
Demand/ Demand/ Resistance Resistance		4.6% 1.5%
Demand/ Demand/ Resistance Resistan	Support	13% 4.4%
	De m an d	1,073 lbs 359 lbs
	Dim. (L x W)	5-1/2" x 3-1/2" 1,073 lbs 5-1/2" x 3-1/2" 359 lbs
	Bearing Supports	Wall/Plate Wall/Plate
	Bear	B1 B2

Notes



DWOND, TAM 45365-17 Structural COMPONENT Description: Designs\Flush Beams\1st Floor\Flush Beams\B9 CAN

File Name: S32-10C-15.mmdl

BC CALC® Design Report

ŧ

Build 4340 Job Name: Address: City, Province, Postal Code: Customer:

Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

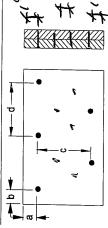
Unbalanced snow loads determined from building geometry were used in selected product's BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086 verification.

Design based on Dry Service Condition.

Part code: Part 9 Importance Factor: Normal

Deflections less than 1/8" were ignored in the results.

Connection Diagram



c=7-1/2" 0 = p b minimum = 3" a minimum = ["

Member has no side loads.

Connectors are: 16d

SPIRAL ARDOX Nails 3%" ...

Company: Designer: Specifier:

MSC.

Completeness and accuracy of input must building codes. To obtain Installation Guide be verified by anyone who would rely on particular application. Output here based current Installation Guide and applicable Installation of BOISE engineered wood products must be in accordance with output as evidence of suitability for 1-800-964-6999 before installation. on building code-accepted design properties and analysis methods. or ask questions, please call

CONFORMS TO DBC 2012

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Town of Innisfil Certified Model 15/02/2018 2:29:08 PM kgervais

WAR CHIEF DWG NO. TAM 45365-17 Oliver ONOFESSION A S. KATSOULAKOS OUNCE OF CASWADIA CAS

STRUCTURAL COMPONERT

7110



1st Floor\...\B10 CANT(i2441) Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

Dry | 2 spans | Left cantilever | 0/12 slope (deg)

September 12, 2017 15:42:53

BC CALC® Design Report

City, Province, Postal Code:, **Customer:** Address:

Job Name: Build 5033

CCMC 12472-R

Code reports:

File Name: S32-10C-15 ELAmmdl Description: Designs\Flush Beams\1st Floor\Flush Beams\B10 CANT\ Specifier:

Company. Designer:

Msc:

Town of Innisfil Certified Model 15/02/2018 2:29:11 PM kgervais

8 K (4) \$\lambda{\pi} E 2 **€ ®** <**€** (S) MW

Reaction Summary (Down / Uplift) (1bs.) Bearing B1, 5-1/2" 468/244		Total Horizontal Proc De ad 226 / 0	Total Horizontal Product Length = 12-04-12 De ad Sn ow \ 226 / 0 280 / 609	Vind
	2477	50 /0	0 70 70	

82

10-10-10

01-06-02

2	Load Summary			Live	Dead	Snow Wind	Trib.
Ta.	ag Description	Load Type	Ref. Start	End 1.00	99.0	1.00 1.15	
0	FC2 Floor Material	Unf. Lin. (Ib/ff)	00-00-00 T	12-04-12 15	7		n/a
_	UserLoad	Unf. Lin. (lb/ft)	O0-00-00 T	01-08-14 44	140	152	n/a
7	FC2 Floor Material	Unf. Lin. (Ib/ff)	00-00-00 T	01-05-12 6			n/a
က	J9(i2370)	Conc. Pt. (lbs)	L 02-07-06	02-07-06 53	26		n/a
4	B12 CANT(i2476)	Conc. Pt. (lbs)	L 03-04-10	03-04-10 39	4	-72	n/a
2	B12 CANT(i2476)	Conc. Pt. (lbs)	L 03-04-10	03-04-10 -27			n/a
9	J8 (i2462)	Conc. Pt. (lbs)	L 03-11-06	03-11-06 56	-175	-520	n/a
/	J8 (i2462)	Conc. Pt. (lbs)	L 03-11-06	03-11-06 -169			n/a
ω	J7 (i2447)	Conc. Pt. (lbs)	L 05-03-06	05-03-06 78	56		n/a
თ	J7 (i2447)	Conc. Pt. (lbs)	L 05-03-06	05-03-06 -26			n/a
10	J7 (i2439)	Conc. Pt. (lbs)	T 06-07-06	06-07-06 78	56		n/a
7	J7 (i2439)	Conc. Pt. (lbs)	T 08-07-06	06-07-06 -26			n/a
12	J7 (i2454)	Conc. Pt. (lbs)	L 07-11-06	07-11-06 78	56		n/a
13	J7 (i2454)	Conc. Pt. (lbs)	L 07-11-06	07-11-06 -26			n/a
4	J8(i2466)	Conc. Pt. (lbs)	90-c0-60 T	09-03-06 73	-170	-520	n/a
15	J8(i2466)	Conc. Pt. (lbs)	L 09-03-06	09-03-06 -175			n/a
16	B11 CANT(i2386)	Conc. Pt. (lbs)	L 10-05-02	10-05-02 56	6ç-	-75	n/a
17	B11 CANT(i2386)	Conc. Pt. (lbs)	L 10-05-02	10-05-02 -33		The state of the s	n/a
48	J9(i2419)	Conc. Pt. (lbs)	L 11-02-06	11-02-06 51	26	OFESSION	n/a

A CHIGINEER POLINICE OF OUTPERO S. KATSOULAKOS PICENSED

DWG NO. TAM 46472-17 A 120 STRUCTURAL COMPONENT



1st Floor\...\B10 CANT(i2441) Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

Dry | 2 spans | Left cantilever | 0/12 slope (deg)

September 12, 2017 15:42:53

File Name: S32-10C-15 ELAmmdl Description: Designs\Flush Beams\1st Floor\Flush Beams\B10 CA\

BC CALC® Design Report **Build** 5033 Job Name:

Company. Msc. CCMC 12472-R Code reports Customer:

City, Province, Postal Code.

Address

Designer: Specifier:

90-60-90 00-00-00 06-07-06 90-60-60 11-02-14 02-06-06 01-06-02 12-04-12 00-00-00 90-60-90 90-60-90 107 107 106 154 107 292 404 Load Demand / Resistance 11.7% 9.2% 5.4% 9.4% n/a ďa n/a 2 ď ď 25,408 ft-lbs -25,408 ft-lbs 11,571 lbs 11,571 lbs 29 ď ď ď ā Resistance Factored 2xL1,998 (0.046") L/999 (-0.082") L/999 (-0.092") -0.092" -2,962 ft-lbs ,373 ft-lbs 1,059 lbs 1,084 lbs 1,068 lbs Factored Demand 833 lbs Controls Summary Fotal Load Defl. Fotal Neg. Defl Live Load Defl. Pos. Moment Neg. Moment Span / Depth Cont. Shear End Shear Max Defl. Polifi Polit

Unspecified Un specified Material Resistance 4.8% 5.7% Member Resistance 10.9% Support 1,124 lbs 1,068 lbs De mand 5-1/2" x 3-1/2" 4-3/8" x 3-1/2" Dim. (L x W) Bearing Supports

B1 Wall/Plate Wall/Plate

De mand/

De man d/

Cautions

LASTER. ۲ 1 -05/W/S)-Uplift of 833 lbs found at span 1 - Right. — (Uplift of 1,068 lbs found at span 2 - Right. —

Notes

Design meets User specified (2xL/240) Total load deflection criteria

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

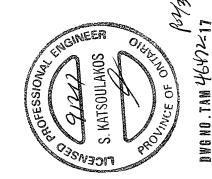
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086.

CONF OR TOUR OR TOUR

verification.

Design based on Dry Service Condition.
Importance Factor: Normal Part code: Part 9
Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

Town of Innisfil Certified Model 15/02/2018 2:29:15 PM kgervais



710 STRUCTURAL COMPENENT



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10 CANT(i2441)

Dry | 2 spans | Left cantilever | 0/12 slope (deg)

September 12, 2017 15:42:53

BC CALC® Design Report Build 5033

Job Name: Address:

City, Province, Postal Code:, Customer

Code reports:

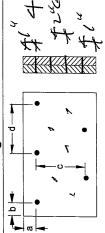
CCMC 12472-R

File Name: S32-10C-15 ELAmmdl Description: Designs\Flush Beams\1st Floor\Flush Beams\B10 CA\ Designer: Specifier:

Company

Misc

Connection Diagram



ζ c=1/2" **Q** = **p** a minimum = b minimum = 3"

Calculated Side Load = 131.1 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record Nails

3½" Å Connectors are:

ARDOX SPIRAL

Completeness and accuracy of input must Installation Guide or ask questions, please be verified by anyone w ho w ould rely on Installation of Boise Cascade engineered wood products must be in accordance with current installation Guide and particular application. Output here based call 1-800-964-6999 before installation. applicable building codes. To obtain output as evidence of suitability for on building code-accepted design properties and analysis methods

SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are ALLJOIST®, BC RIM BOARD™, BCI® BOISE GLULAM™, SIMPLE FRAMING trademarks of Boise Cascade Wood BC CALO®, BC FRAMER®, AJSTM

Products L.L.C.

Town of Innisfil Certified Model 15/02/2018 2:29:19 PM kgervais S. KATSOULAKOS 33 Olastro A POLINICE OF

DWG NO. TAM 4647217 COMPENENT UNLY STRUCTURAL



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11 CANT(i1619)

Dry | 2 spans | Right cantilever | 0/12 slope (deg)

August 2, 2016 10:52:02

BC CALC® Design Report **Build 4340**

Job Name: Address:

City, Province, Postal Code:

CCMC 12472-R Code reports: Customer:

File Name: S32-10C-15.mmdl Description: Designs\Flush Beams\1st Floor\Flush Beams\B11 CAN

Specifier:

Designer: Company: Msc.

Town of Innisfil Certified Model 15/02/2018 2:29:23 PM kgervais

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Total Horizontal Product Length = 04-01-14

Reaction Summary (Down / Uplift) (Ibs	/ Uplift) (Ibs.)								
Be aring	Live	De ad	Ø	now	Wind				
B0	53 / 33	0/33	0	29/0					
B1, 5-1/2"	203/0	390/0	κ	31/0					
Load Summary					Live	Dead	Snow	Snow Wind	Trib.
Tag Description	Load Type	Re f.	Ref. Start	Пd	1.00	0.65	1.00	1.15	
1 FC2 Floor Material	Unf. Lin. (lb/ft)	ر ا	00-00-0	04-01-14	23	11			n/a
2 FC2 Floor Material	Unf. Lin. (lb/ft)	L 0	00-00-0	00-00-00 02-10-08 15	15	œ			n/a
3 UserLoad	Unf. Lin. (lb/ft)	L 0	2-05-00	04-01-14	44	140	152		n/a
4 FC2 Floor Material	Unf. Lin. (lb/ft)	ں ب	2-10-08	02-10-08 04-01-14 6	9	က			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	10 ft-lbs	n/a	n/a	47	00-02-05
Neg. Moment	-536 ft-lbs	-25,408 ft-lbs	•	49	02-07-12
Neg. Moment	-536 ft-lbs	-25,408 ft-lbs	.,	49	02-07-12
End Shear	136 lbs	11,571 lbs	1.2%	99	00-11-08
Cont. Shear	230 lbs	11,571 lbs	2%	49	03-08-00
Uplift	158 lbs	n/a	n/a	99	
Total Load Deff.	2xL/1,998 (0.0	02") n/a	n/a	154	
Live Load Deff.	2xL/1,998 (0.001")	01") n/a	n/a	206	
Total Neg. Defl.	1/999 (-0.001")) n/a	n/a	154	01-07-00
MaxDeff.	-0.001"	n/a	n/a	154	
Span / Depth	3.2	n/a	n/a		00-00-00

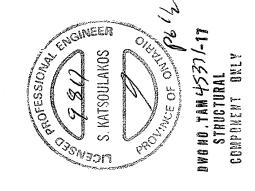
		aterial	1GUS410	HGUS410	Unspecified
De m an d/	Resistance	Support Member Material	1.9%	0.01	4.6%
Demand/ Demand/	Resistance Resistance	Support	n/a	n/a	13.2%
		Demand	sql 09	158 lbs	1,085 lbs
		$Dim.(L \times W)$	2" x3-1/2"	2" x3-1/2"	5-1/2" x 3-1/2"
		Bearing Supports	Hanger	Hanger Uplift	Wall/Plate
		Bearir	<u>8</u>	8	20

Cautions

(05.D) 0) 1605410 511250 Uplift of 158 lbs found at span 1 - Left. Hanger B0 cannot handle uplift of -158 lbs.

Notes

Page 1 of 2



BC CALC® Design Report

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Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11 CA

Specifier: Designer:

Company: Misc:

Design meets User specified (2xL/240) Total load deflection criteria. Design meets User specified (2xL/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

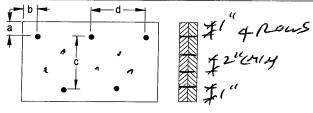
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086. Unbalanced snow loads determined from building geometry were used in selected product's verification.

CONFORMS TO BBC 2012

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9
Deflections less than 1/8" were ignored in the results.

Connection Diagram



Member has no side loads.

Connectors are: 16d Nails ... 4.

312" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Town of Innisfil Certified Model

15/02/2018 2:29:28 PM kgervais



OWO NO. TAM 45371-17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12 CANT(i1617)

BC CALC® Design Report



Dry | 2 spans | Right cantilever | 0/12 slope (deg)

August 2, 2016 10:52:02

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

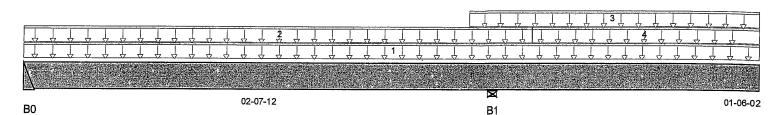
Description: Designs\Flush Beams\1st Floor\Flush Beams\B12 CAN

Specifier:

Designer: Company: Misc:

Town of Innisfil Certified Model

15/02/2018 2:29:32 PM kgervais



Total Horizontal Product Length = 04-01-14

Reaction Summary	(Down / Uplift) (lbs)			
Be aring	Live	De ad	Snow	Wind
B0	37 / 28	0 / 39	0 / 68	
B1, 5-1/2"	162/0	359/0	319/0	

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description [*]	Load Type	Ref	f. Start	En d	1.00	0.65	1.00	1.15	
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	04-01-14	11	6			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-10-08	15	8			n/a
3	Us er Load	Unf. Lin. (lb/ft)	L	02-06-01	04-01-14	44	140	152		n/a
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L	02-10-08	04-01-14	6	3		_	n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	2 ft-lbs	n/a	n/a	47	00-04-01
Neg. Moment	-521 ft-lbs	-25,408 ft-lbs	2%	49	02-07-12
Neg. Moment	-521 ft-lbs	-25,408 ft-lbs	2%	49	02-07-12
End Shear	146 lbs	11,571 lbs	1.3%	51	00-11-08
Cont. Shear	223 lbs	11,571 lbs	1.9%	49	03-08-00
Uplift	164 lbs	n/a	n/a	66	00-00-00
Total Load Defl.	2xL/1,998 (0.002	2'') n/a	n/a	154	4 04-01-14
Live Load Defl.	2xL/1,998 (0.001	l'') n/a	n/a	206	04-01-14
Total Neg. Defl.	L/999 (-0.001")	n/a	n/a	154	1 01-07-00
Max Defl.	-0.001"	n/a	n/a	154	1 01-07-00
Span / Depth	3.2	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Bear	ing Supports	Dim . (L x W)	De m an d	Support	Member	Material
B0	Hanger	2" x 3-1/2"	20 lbs	rı/a	1.9%	HGUS410
B0	Hanger Uplift	2" x 3-1/2"	164 lbs	n/a	0.02	HGUS410
B1	Wall/Plate	5-1/2" x 3-1/2"	1,008 lbs	12.3%	4.3%	Unspecified

Cautions

Uplift of 164 lbs found at span 1 - Left.

Hanger B0 cannot handle uplift of -164 lbs.) - (SIMSON 1-H605410 CO. BD)



UWUNU. TAM 45372479 STRUCTURAL COMPONENT ONLY

POVINCE OF OUTES

BC CALC® Design Report

*

Build 4340

Job Name:

Address:

W

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B12 CA

Specifier: Designer:

Company:

Misc:

Design meets User specified (2xL/240) Total load deflection criteria. Design meets User specified (2xL/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86. Unbalanced snow loads determined from building geometry were used in selected product's

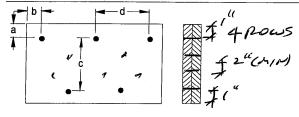
verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9
Deflections less than 1/8" were ignored in the results.

CONFORMS TO DBC 2012

Connection Diagram



Member has no side loads.

Connectors are: 16d 👊 🧩 Nails 🥕

312" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Town of Innisfil Certified Model

15/02/2018 2:29:36 PM kgervais





Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B13(i2368)

BC CALC® Design Report

*

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 4, 2016 10:09:55

Build 4340

Job Name: Address:

City, Province, Postal Code:,

Customer:

B₀

Code reports:

CCMC 12472-R

File Name: S32-10C-15.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B13(i236)

Specifier:

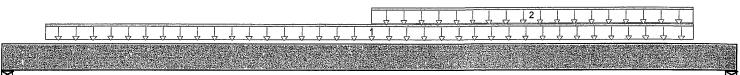
Designer: Company:

Misc:

Town of Innisfil Certified Model

15/02/2018 2:29:40 PM kgervais

B1



Total Horizontal Product Length = 07-11-00

07-11-00

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0, 5-1/2"	295/0	167/0							
B1, 5-1/2"	707/0	373/0		÷					

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Ret	f. Start	En d	1.00	0.65	1.00	1.15	
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-05-08	07-05-08	23	11			n/a
2	Us er Load	Unf. Lin. (lb/ft)	L	03-11-07	07-05-07	240	120			n/a

CONFORMS TO OBG 2012

Cantrala Summanı	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	2,149 ft-lbs	12,704 ft-lbs	16.9%	1	04-09-10
End Shear	1,074 lbs	5,785 lbs	18.6%	1	06-08-00
Total Load Defl.	L/999 (0.051")	n/a	n/a	4	04-02-08
Live Load Defl.	L/999 (0.033")	n/a	n/a	5	04-02-08
MaxDefl.	0.051"	n/a	n/a	4	04-02-08
Span / Depth	9	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Bear	ing Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 1-3/4"	651 lbs	12.7%	5.5%	Unspecified
B1	Wall/Plate	5-1/2" x 1-3/4"	1,526 lbs	29.7%	13%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO.TAM 45373.17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i2732)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 7, 2017 16:16:59

BC CALC® Design Report

*

ort 📑

Build 5033 Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15 EL B-WOD.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i2732)

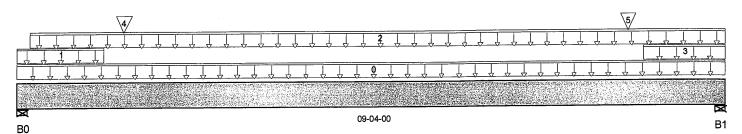
Specifier:

Designer: Company:

Misc:

Town of Innisfil Certified Model

15/02/2018 2:29:44 PM kgervais



Total Horizontal Product Length = 09-04-00

Reaction Summary (Down / Uplift) (lbs)										
Bearing	Live	De ad	Snow	Wind						
B0, 5-1/2"	1,836 / 0	1,413/0	1,976 / 0							
B1 5-1/2"	1.936 / 0	1,460/0	1,973 / 0							

	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	LOWROOF	Unf. Lin. (lb/ft)	L	00-00-00	09-04-00	33	30	111	-	n/a
1	Us er Load	Unf. Lin. (lb/ft)	L	00-00-00	01-01-12	146	232	489		n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	00-02-00	09-04-00	285	143			n/a
3	Us er Load	Unf. Lin. (lb/ft)	L	08-03-00	09-04-00	146	232	489		n/a
4	Us er Load	Conc. Pt. (lbs)	L	01-04-12	01-04-12	264	340	912		n/a
5	UserLoad	Conc. Pt. (lbs)	L	08-00-10	08-00-10	264	340	912		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	8,282 ft-lbs	25,408 ft-lbs	32.6%	1	04-10-00
End Shear	4,634 lbs	11,571 lbs	40%	13	01-03-00
Total Load Defl.	L/614 (0.167")	0.427"	39.1%	35	04-08-00
Live Load Defl.	L/999 (0.112")	n/a	n/a	51	04-08-00
Max Defl.	0.167"	n/a	n/a	35	04-08-00
Span / Depth	10.8	n/a	n/a		00-00-00

				Resistance	Resistance	
Bear	ring Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	5,648 lbs	54.9%	24%	Unspecified
B1	Wall/Plate	5-1/2" x 3-1/2"	5,753 lbs	56%	24.5%	Unspecified

Notes



DWO NO.TAM 4537417 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i2732)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 7, 2017 16:16:59

BC CALC® Design Report

Build 5033

Job Name: Address:

City, Province, Postal Code:, Customer:

Code reports:

CCMC 12472-R

File Name: S32-10C-15 EL B-WOD.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i27

Specifier: Designer:

Company: Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

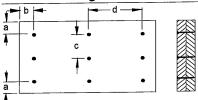
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Connection Diagram



a minimum = 2"

c = 2-3/4" b minimum = 3"

Calculated Side Load = 594.5 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Nails
3½ ARDDX SPI

ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Town of Innisfil Certified Model

15/02/2018 2:29:48 PM kgervais

DWO NO . TAM 45

STRUCTURAL

COMPONENT ONLY



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			E	Bare			1/2" Gyp	sum Ceiling	
Depth	Series		On Cen	tre Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15' - 9"	15'-3 "	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-//6	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21' - 7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	Span Blocking a	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18' - 5"	17'-8"	N/A
	Ni-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
11-//6	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22' - 9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
	NI-90x	23'-4"	21' - 8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
14"	NI-70	25'-3"	23'-4"	22' - 3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27 '- 5"	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			В	are		Į	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	tre Spacing	
		12"	16"	19.2"	24"	12"	16"	/ 19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
11 7/0"	N!-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18' - 9"	17'-11"	17'-2"
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19' - 2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22 '- 9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24' - 2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	18'-7"	17' - 9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17' - 8"	22'-5"	20'-6"	19'-4"	17'-8"
11 7/0"	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11-7/8"	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22' - 6"	21'-5"	20'-0"
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20' - 9"
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23' - 8"	22'-4"	20'-10"
14"	NI-70	26'-1"	24 '- 3"	23'-2"	21'-10"	26'-8"	24'-11"	23 '- 9"	22'-4"
	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25' - 3"	24'-1"	22'-9"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"
	NI-60	27'-3"	25' - 5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
1.0"	NI-70	28' - 8"	26' - 8"	25 '- 4"	23'-11"	29'-3"	27'-4"	26'-1"	24' - 8"
16"	NI-80	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			E	are		<u> </u>	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11 7/09	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/8"	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18' - 9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23' - 6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing		1	On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
···	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	N!-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17' - 9"	N/A
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
11-7/8	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22' - 5"	21'-0"	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	NI-70	27' - 9"	25'-8"	24'-6"	N/A	28'-5"	26' - 5"	25'-2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25 '- 6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			В	are			1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
11 7/0"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19' - 9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20' - 11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23' - 2"	22'-0"	20'-10"	25'-9"	23'-10"	22 '- 9"	21 '- 6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23 '- 8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	Ni-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"
11 7/09	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/8"	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	NI-90x	24'-3"	22' - 6"	21'-3"	19'-7"	24'-8"	22' - 7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24' - 3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25 '- 10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
10"	NI-70	28' - 8"	26'-8"	25'-3"	23'-4"	29'-3"	26'-11"	25' - 3"	23'-4"
16"	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

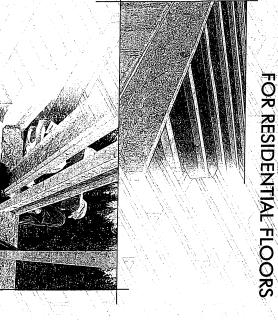
^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

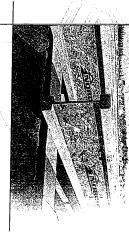
^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



NSTALLATION GUIDE **₹00**







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SAFETY AND CONSTRUCTION PRECAUTIONS

WARNING



braced, or serious injuuntil fully fastened and Do not walk on I-joists ries can result.



concentrated loads from building materials. Once sheathed, do not over-stress I-joist with materials over unsheathed 1-joists.

Avoid Accidents by Following these Important Guidelines:

Fjoists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, Brace and nail each Lipist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When Lipists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.



- Never stack building

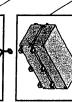
- to prevent I-joist rollover or buckling. temporary bracing, often called struts, or temporary sheathing must be applied
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay. bracing over at least two 1-joists.
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully Never install a damaged I-joist.

STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- 2. Store, stack, and handle I-joists vertically and level only.
- 3. Always stack and handle I-joists in the upright position only.
- 4. Do not store I-joists in direct contact with the ground and/or flatwise
- Protect I-joists from weather, and use spacers to separate bundles.

6. Bundled units should be kept intact until time of installation

- 7. When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury
- Pick I-joists in bundles as shipped by the supplier
- Orient the bundles so that the webs of the I-joists are vertical.
- Pick the bundles at the 5th points, using a spreader bar if necessary.
- 8. Do not handle Ljoists in a horizontal orientation
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST





MAXIMUM FLOOR SPANS

- 1. Maximum clear spans applicable to simple-span or For multiple-span applications, the end spans shall be 40% or more of the adjacent span. for floor vibration and a live load deflection limit of L/480 live load of 40 psf and dead load of 15 psf. The ultimate multiple-span residential floor construction with a design limit states are based on the factored loads of 1.50L + .25D. The serviceability limit states include the consideration
- 2. Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum of gypsum and/or a row of blocking at mid-span. assumed. Increased spans may be achieved with the used Standard. No concrete topping or bridging element was thickness of 5/8 inch for a joist spacing of 19.2 inches or shall meet the requirements given in CGBS-71.26 less, or 3/4 inch for joist spacing of 24 inches. Adhesive
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings
- Bearing stiffeners are not required when I-joists are used required for hangers. with the spans and spacings given in this table, except as
- 5. This span chart is based on uniform loads. For applications be required based on the use of the design properties. with other than uniform loads, an engineering analysis may
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- SI units conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

MAXIMUM FLOOR SPANS FOR NORDIC 1-JOISTS SIMPLE AND MULTIPLE SPANS

Joist Jo																
Joist									1,245							
Simpl On cent			,						187							
le spans re spacing	19.2	13.9° 14.8°	14-10	15-6	2015-6	16.5	1847	17-6"	17:10	17-10	0	1924	19-9"	19:91	2019* 2111*	21'-5"
	24"	3-5 4-0	14:11	1517	15.9	16.6	16:9	17.7	17711	177-JUN	1812	19:5	19:10	19-10	20-10	21-6
	12"	75.5	77.7	18:7	8-10	20-0	20:3	21.0	22:3°	22'2"	2017	24.3	24.9	24.27	26-0°	26-11"
Multiple On centre	16°	15'4"	16:5	17.4	17.6	18-6	18:9	20:2	2017 2017	20-6	20-11	22.5 -	22410	22:9"	2410	24-10
e spans e spacina	19.2"	14:10	15:10*	16:-9*	16-11	17-9	18-0	19:3	19-8°	19:-8"	20-0	21.5	21:-10	21:9"	22-11	231-9
	24"	7447	15:5	16-10	17.0	16-/ 17-7	18-1	19-1	19:9"	19:4"	20:1:	21.6	211-10	21-10	23-0	23-9*

I-JOIST HANGERS

Hangers shown illustrate the three

most commonly used metal hangers

Hangers should be selected based

All nailing must meet the hanger

to support 1-joists.

manutacturer's recommendations

4. Web stiffeners are required when the

brace the top flange of the I-joist. sides of the hangers do not laterally maximum spans.

and load capacity based on the on the joist depth, flange width

NI-40x			ð	RATED LOAD	CONCENTRATED		
						·	MILS
SIOF-I	NORDIC	z					
T 13032-R	CCMC EVALUATION REPORT 13032-R	MC EVALUAI	CC.				
21:10° 23:0° 23:4° 23:4° 23:9° 24:1°	21.9 22.11 23.3 23.9 24.0	22.9 24.0 24.5 24.10 25.2	24.7° 26.5° 26.5° 26.11' 27.3°	20-10- 20-10- 21-2" 21-6" 21-10	20:9: 21:1: 21:5: 21:9:	21(9) 22-11 22-6 22-6	23(6) 22(1) 24-5 24-8
21.6 21.6 21.10 22.2	21.5 21.5 21.10 22.0	22:5" 22:10" 23:11"	24'3" 24'9" 25'0"	1915 1910 20-0	1994 1991 1911	2013	22.2
20:1:	19:8" 20:0"	20.6	22:22 22:27	18/24	17-10	18.7	2015
19:9:	19-8: 19-8:	228 228 247 247	3252 749 749	17:74 17:11 18:01 18:01	77.8 77.8 72.10	18:3	20°2
11 <i>6:7"</i> 11717' 18:11'	16-8"	17:3* 18:6* 18:71	2004 2004 2004	16:30 16:30	1876.		
15:51 16:11 16:10 17:0"	15:10* 16:9* 16:11*	1645* 1647* 174*	7:5* 7:7* 8:7* 8:10*	14.9 15.7 15.9	14-8* 14-10* 15-6*	6514 641	
24"	spans spacing 19.2:	Multiple spans On centre spacing 16" 19.2"	12°	24"	<u>,, ,, ,,, ,,, ,,,,,,,,,,,,,,,,,,,,,,,</u>	Simple spans On centre spaci 16" 19.2	12 "

Top Moun

race Mount

WEB STIFFENERS

RECOMMENDATIONS:

- A bearing stiffener is required in all the stiffener and the flange is at the top. Construction Guide (C101).The gap between engineered applications with factored i-joist properties table found of the I-joist reactions greater than shown in the
- sides of the hanger do not extend up to, and the I-joist is supported in a hanger and the stiffener and flange is at the top. support, the top flange. The gap between the A bearing stiffener is required when
- by the code. The gap between the stiffener and the flange is at the bottom. than 2,370 lbs is applied to the top flange adjusted for other load durations as permitted standard term load duration, and may be tip and the support. These values are for cantilever, anywhere between the cantilever between supports, or in the case of a where a factored concentrated load greater ■ A load stiffener is required at locations

SI units conversion: 1 inch = 25.4 mm

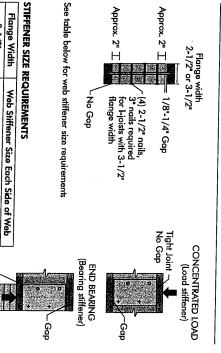
3-1/2" 2-1/2

1-1/2" x 2-5/16" minimum width 1" x 2-5/16" minimum width

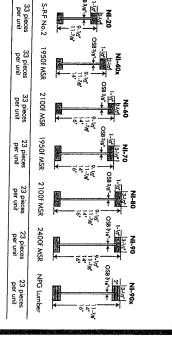
Tight Join

FIGURE 2

WEB STIFFENER INSTALLATION DET



SERIES



Chantiers Chibougamau Ltd. harvests its own trees, which enables Nortic products to adhere to strict quality control procedures throughout the finished product, reflects our commitment to quality. manufacturing process. Every phase of the operation, from forest to the ...

lumber in their flanges, ensuring consistent quality, superior strength cirro longer span carrying capacity. Nordic Engineered Wood I-joists use only finger-jointed back spruce

2015-04-1 ത

INSTALLING NORDIC I-JOISTS

- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, ஜார்ச் அன்
- 2. Except for cutting to length, 1-joist flanges should never be cut, drilled, or notched
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment

Matter Tax

- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple அளில்க்கு nust
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings 2045-024-16
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- 8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the Ljoist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the Lioist. Or, attach the load to blocking that has been securely fastened to the
- 9. Never install Lioists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- 11. For Hoists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below
- 12. Due to shrinkage, common framing lumber set on edge **may never** be used as blocking or rim boards. I-joist blocking panels or other engineered wood products – such as rim board – must be cut to fit between the Lipists, and an i-joist-compatible depth selected
- 13. Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used
- 14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

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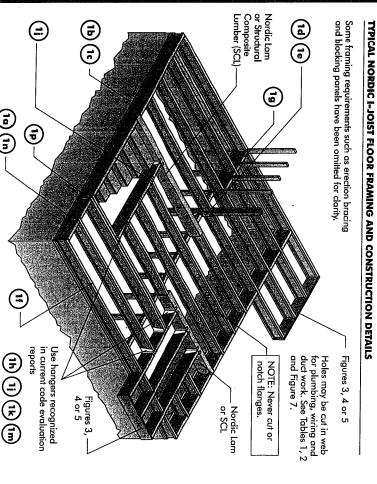
NI blocking panel

€

wire or spiral nail at top and One 2-1/2"

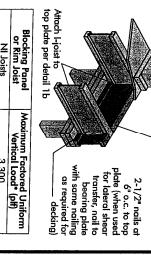
start nails at least 1-1

FIGURE 1



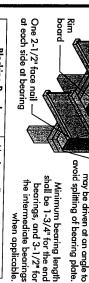
All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.

(1a) (1n)



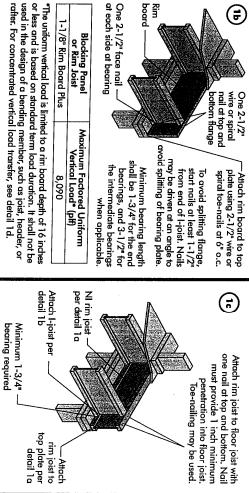
load transfer, see detail 1d

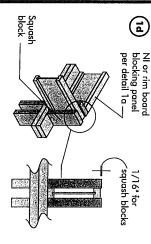
such as joist, header, or rafter. For concentrated vertical It shall not be used in the design of a bending member,



8.090	1-1/8" Rim Board Plus
Maximum Factored Uniform Vertical Load* (plf)	Blocking Panel or Rim Joist

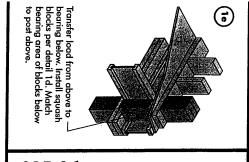
rafter. For concentrated vertical load transfer, see detail 1d.

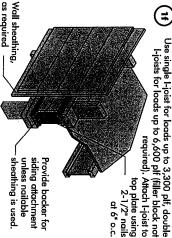




actored \ vash Blo e 5-	1-1/8" Rim Board Phie	2x Lumber		Pair of Squash Blocks
5-1/2" wide 8,500	7 300	5,500	3-1/2" wide	Maximum Factor Pair of Squas
	007.7	8,500	5-1/2" wide	ned Vertical per h Blocks (lbs)

Provide lateral bracing per detail 1a, 1b,





required when rim board is used. Bracing per code shall be Rim board may be used in lieu of I-joists. Backer is not carried to the foundation.

ŧ

beam. 1/8" overhang allowed past inside inside face of wall or 2x plate flush with

filler block shown. Nordic Lam or SCL Multiple I-joist header with full depth headers may also be used. Verify double I-joist capacity to support

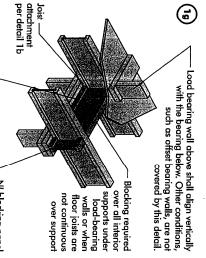
concentrated loads

Do not bevel-cut

face of wall or beam.

detail 1p

Filler block per



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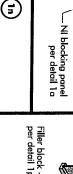
Backer block (use if hanger load exceeds 360 lbs)

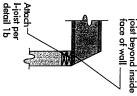
backer block will fit. Clinch. Install backer tight to top flange. Before installing a backer block to a double I-joist, drive three additional 3" nails through the webs and filler block where the

Use twelve 3" nails, clinched when possible. Maximum factored

resistance for hanger for this detail = 1,620 lbs.

6" a.c. to top plate 2-1/2" nails at -Double I-joist header





at bearing for lateral Note: Blocking required support, not shown

per detail 1p hanger Top- or face-mount

> laterally support the top flatiggy high ring stiffeners, shall the view of Note: Unless hanger sides

20~90416

For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

(both sides for face-mount

Backer block required

hangers)

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

3-1/2"	2-1/2"	Flange Width Mater
1-1/2"	7"	Material Thickness Required*
7-1/4"	5-1/2"	Minimum Depth**

- to CAN/CSA-O325 or CAN/CSA-O437 Standard better for solid sawn lumber and wood structural panels conforming Minimum grade for backer block material shall be S-P-F No. 2 or
- minus 4-1/4". joists with 1-1/2" thick flanges. For 2" thick flanges use net depth

(1_S

-One 2-1/2" nails at top and bottom flange

board ŝ

-2x4 min. (1/8" gap minimum)

lumber piece

Two 2-1/2" nails from each web to

For face-mount hangers use net joist depth minus 3-1/4" for



beams, see the manufacturer's For nailing schedules for multiple recommendations

recommendations.

Note: Unless hanger sides laterally support the top flange, bearing

stiffeners shall be used. support the top flange, bearing Note: Unless hanger sides laterally manufacturer's recommendations Top-mount hanger installed per –

(

- 1. Support back of I-joist web during nailing to prevent damage to web/flange connecti
- Leave a 1/8 to 1/4-inch gap between to of filler block and bottom of top I-joist
- Filler block is required between joists for
- 4. Nail joists together with two rows of 3" are required. can be clinched, only two nails per foot possible) on each side of the double I-ic Total of four nails per foot required. If n nails at 12 inches o.c. (clinched when
- The maximum factored load that may be using this detail is 860 lbf/ft. Verify double applied to one side of the double joist

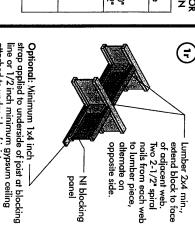
FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION

Maximum support capacity = 1,620 lbs.

clinch when possible.

detail 1h. Nail with twelve 3" nails, Backer block attached per recommendations manufacturer's install hanger per

Ξ.	0.00	0.01	EF 1-30131 COLVAINOCHOIN
မွ	Flange Size	Joist Depth	Filler Block Size
ĭ	2-1/2"× 1-1/2"	9-1/2" 11-7/8" 14" 16"	2-1/8" × 6" 2-1/8" × 8" 2-1/8" × 10" 2-1/8" × 12"
oist.	3-1/2"× 1-1/2"	9-1/2" 11-7/8" 14" 16"	일 × 8 일 × 10 기 기 기 기 기 기 기 기 기 기 기 기 기 기 기 기 기 기 기
6	3-1/2"× 2"	11-7/8" 14" 16"	3" x 7" 3" x 9" 3" x 11"

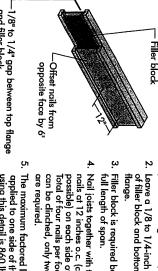


In some local codes, blocking is prescriptively required in the first joist space (or first and second joist space) next to the starter joist. Where required, see local code requirements for spacing of the blocking

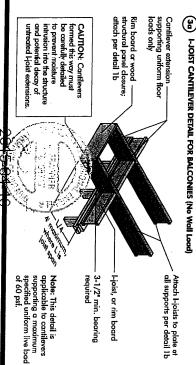
One 2-1/2" nails one side only 2-1/2" nails at 6" o.c.

1-joist blocking panel

attached to underside of joists.



CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)



(y) LUMBER CANTILEVER DETAIL FOR BALCONIES (No Wall Load)

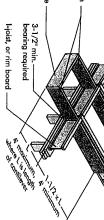
Full depth backer block with 1/8" gap between block and top flange of I-joist. See detail 1h. Nail with 2 rows of 3" nails at 6" o.c. and clinch.

plate at all supports per detail 1b Attach I-joists to

2x8 min. Nail to backer block and joist with 2 rows of 3" nails at 6" o.c. and clinch. (Cantilever nails may be used to attach backer block if length of nail is sufficient to allow clinching.) floor loads only Cantilever extension supporting uniform

Lumber or wood structural panel closure

Note: This detail is applicable to cantilevers supporting a maximum specified uniform live load of 60 psf.



CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

below for NI

Roof truss span

<u>ک</u> 0 cantilever

SSULL

Girder A

— 13'-0" maximum

Roof trussspan

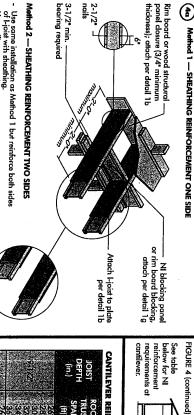
2'-0" Jack trusses

maximum cantilever

requirements for a span of 26 ft. shall be permitted to be used. the cantilevered floor joists, the L-joist reinforcement For hip roofs with the jack trusses running parallel to

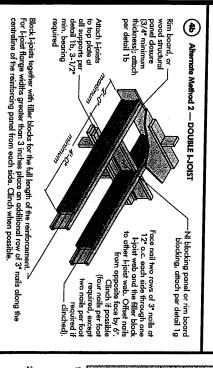
See table

Method 1 — SHEATHING REINFORCEMENT ONE SIDE



- Use nailing pattern shown for Method 1 with opposite face nailing offset by 3".

Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of jost. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports detail 1b. Verify reinforced I-joist capacity



	JOIST DEPTH	ROOF	LL = 30 psf, DL = 15 psf	ROOF LOADING (UNFACTORED) LL = 40 psf, DL = 15 psf	LL = 50 psf, DL = 15 ps	
ZZZZZZZ ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	(ii	SPAN	JOIST SPACING (in.)	JOIST SPACING (in.) 16 19.2	5	24
ZZZZZZZ ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ		98	- Z	1 2 1 2	2 2	٠××
ZZZZZZZ ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ		6 4 K	1 2 1 2	300 (XX)	i vitalis i vita	×××
ZZZZZZZ ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ		286	ZZ	ZZ	Z	× N >
ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	11.7/8	828 828	77.	1 Z Z	2	×××
ZZZZZZZ ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ		38	ZZZ	1 2	2	××
ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ		881	ZZZ	zzz	ZZ	2.5
ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	T.	222	ZZ ZZ	zzź	ZZZ	i N N
ZZZZZZZ ZZZZZZZ ZZZZZZZ ZZZZZZZ ZZZZ		686	ZZZ			(XN)
Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z		90 k	ZZZZ	ZZZ	ZZZ	اد د د ا
	5 6 5 6	4 % %	ZZZ	ZZZZ	ZZZZ	000 ±

- . N = No reinforcement required.
 1 = NI reinforced with 3/4" wood structural
- panel on one side only.

 2 = NI reinforced with 3/4" wood structural
 panel on both sides, or double I-joist.
 X = Try a deeper joist or closer spacing.
 2. Maximum design load shall be 1.5 per roof
 dead load, 5/5 per floor broth load, and 80
 pit wall load. Wall load is based on 3:0"
- For larger openings, or multiple 3'-0' width openings spaced less than 6'-0' o.c., additional loss beneath the opening's cripple stude may be required.

 3. Table applies to joist 15' to 24' o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a five load deflection limit of L480, Use 12' o.c. requirements for lesser spacing.
 - 4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a
- Cantilevered joists supporting girder trusses or roof beams may require additional

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS

- The distance between the inside edge of the support and the centreline of any Table 1 or 2, respectively. nole or duct chase opening shall be in compliance with the requirements of
- 'n 1-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. between the top or bottom of the hole or opening and the adjacent L-joist flange. the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of
- Ċ The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- ٥ Where more than one hole is necessary, the distance between adjacent hole opening shall be sized and located in compliance with the requirements of size of the largest square hole (or twice the length of the langest side of the langest rectangular hole or duct chase opening) and each hole and duct chase edges shall exceed twice the diameter of the largest round hole or twice the Tables 1 and 2, respectively
- .7 A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to
- % A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- . 0 All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf LOCATION OF CIRCULAR HOLES IN JOIST WEBS

. Above tal	7				Joist Depth
ole may be us	ekinjeje	evereteyete	ereleteletele	elelejeje	Joist Series
ed for I-joist	100				
spacina of 2					5
4 inches on	11.6 3.6 1.9 1.9 2.00		N		n distan
centre or le					ຼ ໘
n	514664 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 4 2 4 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	eo aa yuu Gae kege	11111	ide face und hole 7
- 1	11000		8.0 B.4 8.10 III.0 100 III.2 100 III.2 103 III.14 8.91 III.2		FIEL: 1
	10000 10000	6.6 6.6 6.6 6.6	111144	ii di	port to c (in.)
- 1	100	100			. 0
	1122 1224 1234	111111	141111	11-11	ole (ff-in
The state of the state of the state of	2010 2010	18 22 18 22 18 23 18 25	11,10,400	10.6 14.0 14.11	Span adjustment Factor

Above have may be used on lots spacing or 4s inches on centre or less.
 Hole location distance is measured from inside face of supports to centre of hole.
 Distances in this chart are based on uniformly loaded joists.

OPTIONAL:

The above table is based on the Hoists used at their maximum span. If the Hoists are placed at less than their full maximum span (see Maximum Floir Spans), the minimum distance from the centreline of the tools to the face of any support (D) as given above may be reduced as follows:

Dreduced = Lactual x D

Where: Dreduced Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications (th. The reduced distance shall not be less than 6 inches from the face of the support to edge of the hole.

Lactual The actual measured span distance between the inside faces of supports (fi

D ₹ Span Adjustment Factor given in this table.

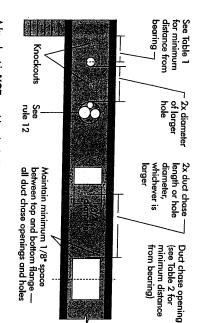
II The minimum distance from the inside face of any support to centre of hole from this table

If Lactual is greater than 1, use 1 in the above calculation for Lactual

0/5-04-1 ത

FIGURE 7

FIELD-CUT HOLE LOCATOR



and may be ignored for purposes of calculating minimum distances A knockout is NOT considered a hole, may be utilized wherever it occurs between holes

> spaced 15 inches on centre along the length of the I-joist. Where possible, it is electrical or small plumbing lines. They for the contractor's convenience to instal field-cut holes. preferable to use knockouts instead of are [-] Knockouts are prescored holes provided /2 inches in diameter, and are



should be cut with sharp saw. Holes in webs

Never drill, cut or notch the flange, or

over-cut the web.

the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between stress concentrations. Slightly rounding the holes is another good method to the corners, as this can cause unnecessary For rectangular holes, avoid over-cutting he corners is recommended. Starting

TABLE 2

DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only

### 100	Joist Depth	Joist Series	Minimu 8	ım distan 10	ice from i 12	nside fac Duct c 14	e of any hase len 16	support t gth (in.) 18	o centre 20	of open 22
Series Se			T. L.	4.5	Works W	******************	8115	641	V. V.	
5.00 5.00 5.00 7.00 7.00 7.00 7.00 7.00			544	5-9	6-0 6-2	645 675	6-10 7-10	7.3	e / 1	
6.8 7.2 7.6 8.1 8.6 9.1 7.2 7.6 9.1 7.2 7.6 9.1 7.2 7.6 9.1 7.2 7.6 9.1 7.2 7.6 9.1 7.2 7.6 9.1 7.2 7.6 9.1 7.2 7.6 9.1 7.2 7.4 8.0 9.2 7.2 7.4 8.0 9.2 8.0 9.2 7.4 7.4 8.0 9.2 8.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9			551. 541.	5.5° 5.8°	5-10° 6-0°	6.3	867	72	7.6	
7.4 7.8 8.0 9.0 9.3 7.4 9.1 9.5 9.0 9.3 7.4 9.5 9.5 9.5 9.5 9.5 9.5 9.5 9.5 9.5 9.5		a) t	6.0	652 71.5	9.6	77.1	7.5		83	
7.22 7.77 8.17 8.17 8.17 9.17 9.17 9.17 9.17 9.17 9.17 9.17 9					6.0	8.6	90	9.3	9-9 9-6	
8 1			7.2	27	18 18	180 010 010	8-10 0-10	94 194	9,	
9.9 9.3 9.5 9.1 10.6 10.4 10.8 9.5 9.10 10.4 10.8 10.4 10.4 10.4 10.5 10.4 10.5 10.4 10.5 10.5 10.5 10.5 10.5 10.5 10.5 10.5				811	985 285	Bije	9.4	9.8	Đ¢.	
10-4 10-5 10-4 10-5			989	990	0.99 Y 200 C	10.1	00.	107		
10.4 10.5 11.5 11.5 12.5 12.5 10.5 10.5 10.5 10.5 10.5 10.5 10.5 10			9.0		9.9	15 14 14	100 171			6
			94	9:9	0.3	10.7	19-11	11.5	151	9
			100	0 5		L Lo	12.1.	12161 12161	3	82
			100	10.9	113-3 8-3	31.9	12:1			

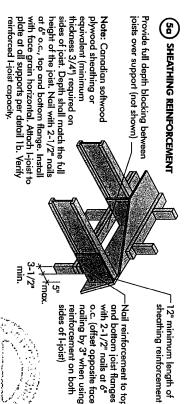
Above table may be used for I-joist spacing of 24 inches on centre or less.

Duct drase opening location distance is measured from inside face of supports to centre of opening.

The above table is based on simple-span joists only. For other applications, contact your local distributor.

Distances are based on uniformly loaded floor joists that meet the span requirements for a destign live load of 40 psf and dead load of 15 psf, and a live load deflection limit of 1/480. For other applications, contact your local distributor.

BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



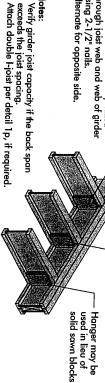
o.c. (offset opposite face nailing by 3" when using and bottom joist flanges with 2-1/2" nails at 6" Nail reinforcement to top

(3/4" minimum thickness), structural panel closure attach per detail 1b. Kim board or wood supports per detail 1b. 3-1/2" minimum I-joist Attach I-joist to plate at all (not shown for clarity) Provide full depth blocking SET-BACK DETAIL ∄ox. ហ្ន Bearing walls girder joist per detail 5c. Attach joists to

(F

(F) SET-BACK CONNECTION

through joist web and web of girder Alternate for opposite side. using 2-1/2" nails. (2x6 S-P-F No. 2 or better) nailed Vertical solid sawn blocks



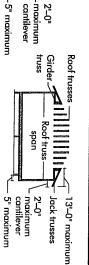
exceeds the joist spacing.

bottom flanges. Nail joist end using 3" nails, toe-nail at top and Hanger may be used in lieu of

panel on both sides, or double I-joist.

dead load, 55 psf floor total load, and 80 plf wall load. Wall load is based on 3'-0"

below for NI reinforcement FIGURE 5 (continued) requirements at cantilever. See table Roof truss span Lmaximum 2'-0" cantilever



trusses running parallel to the cantilevered floor joists, For hip roofs with the jack requirements for a span of the I-joist reinforcement be used 26 ft. shall be permitted to

BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

TOICT	ROOF					ROOF L	OADING	(UNFAC	OREU)		The second second		
DEPTH	TRUSS SPAN	or = 11	30 psf, NST SPA	DL = 15 ₁ CING (in.)	osf)	_ ==	= 40 psf, DIST SPA	DL = 15 CING (in	psf .)	ے آ	= 50 psf, OIST SPA	DL = 15 CING (in.	psf
The second secon	(H)	12	16	19.2	24	12	16	19.2	24	12	16	19.2	24
	2 /2 /2 2 /8 /2 3 /8 /2		××	××	××	22	××	××	××	X2	××	××	××
9-1721	2 W C	32	«××	(××	××	N N	××	××	××	××	××	××	××
	36	2	××	×	××	X	××	××	××	××	××	××	××
	48k	722	3 NO K	‹ ××	<××		××	××	××	2	×	××	××
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	4 38	zz	52i	×××	* *>	ـ ود	<××	٧××	×××) N –	××	××	××
	* 142		2	X	×		X;	×	×	2	×	××	××

- 1 = NI reinforced with 3/4" wood structural N = No reinforcement required.
- panel on one side only.
 2 = NI reinforced with 3/4" wood structural
- X = Try a deeper joist or closer spacing.2. Maximum design load shall be: 15 psf roof
- For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple studs may be required.
- Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of 1/480. Use 12" o.c. requirements for lesser spacing.
 - For conventional roof construction using a the supporting wall and the ridge beam.
 When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a above is equivalent to the distance between ridge beam, the Roof Truss Span column
- truss is used.

 Cantilevered joist supporting girder trusses or roof beams may require additional reinforcing.

INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- 2. Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading give.
- 3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel trom damage when tapped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time a thinner line (1/8 inch) than used on I-joist flanges. before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying
- 8. Tap the second row of parels into place, using a block to protect groove edges.
- Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common nail to assure accurate and consistent spacing.)
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels finished deck can be walked on right away and will carry construction loads without damage to the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The

FASTENERS FOR SHEATHING AND SUBFLOORING(1)

24	20	jo	Maximum N Joist Spacing T (in.)
3/4	5/8	5/8	Minimum Panel Thickness (in.)
2"	. 2"	2"	Na Common Wire or Spiral Nails
1-3/4"	1-3/4"	1-3/4"	iil Size and Ty Ring Thread Nails or Screws
2"	2"	2"	pe Staples
6"	6"	6"	Maximun of Fast Edges
12"	12"	12"	1 Spacing leners Interm. Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- 2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- Flooring screws shall not be less than 1/8-inch in diameter.
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manutacturer.

Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5.

IMPORTANT NOTE:

Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with your local distributor.

RIM BOARD INSTALLATION DETAILS

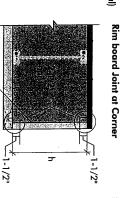
8 ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

Rim board Joint Between Floor Joists



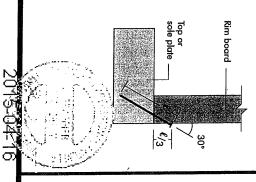


(typical) top and bottom (1) 2-1/2" nail



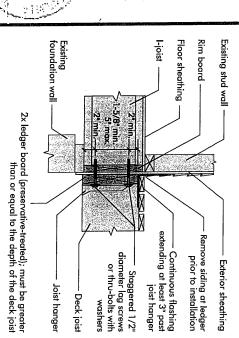
(F TOE-NAIL CONNECTION AT RIM BOARD

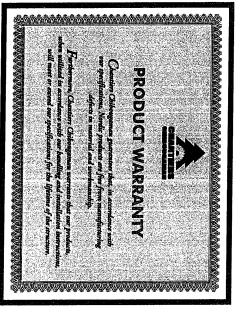
2-1/2" toe-nails at 6" o.c. (typical) —

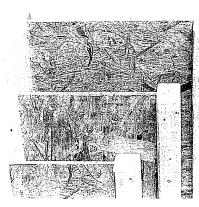


F 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL

Rim board joint —







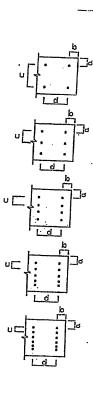
MICRO CITY

engineering services inc.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

LVLHEA		SED AND GO	
		DER AND COL BER NAILING	
	DETAIL NUMBER	NUMBER OF ROWS	SPACING
	. A	2:	12
	В	2	. 8
	С	2	6
	D	2	4
- Samuel	1A	3	12
	1B	3	8
	1C	3	. 6
-	1D	. 3:	4
	2A	4	. 12 .
-	2B	4	8
-	2C	4	6
Ŀ	2D	4	4
╟	. 3A	5	12
-	3B	5	. 8
1	3C	5	6
L	3D	5	4
1	4A	6	12
⊩	4B	6	8
ŀ	4C	6	6
Ŀ	4D	6	4



NOTES

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
 - (2) MINIMUM LUMBER END DISTANCE "b" = 2"
 - (3) MINIMUM NAIL ROW SPACING "c" = 2"
 - (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
 - (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
- (6) DO NOT USE AIR-DRIVEN NAILS



DNO NO TANNICOI. 14
STRUCTURAL
COMPONENT ONLY
TO BE USED ONLY
WITH BEAM CALCS
PEARING THE
STAMP BELOWS

PROVICE NAILING
DETAIL № × SEE
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