ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

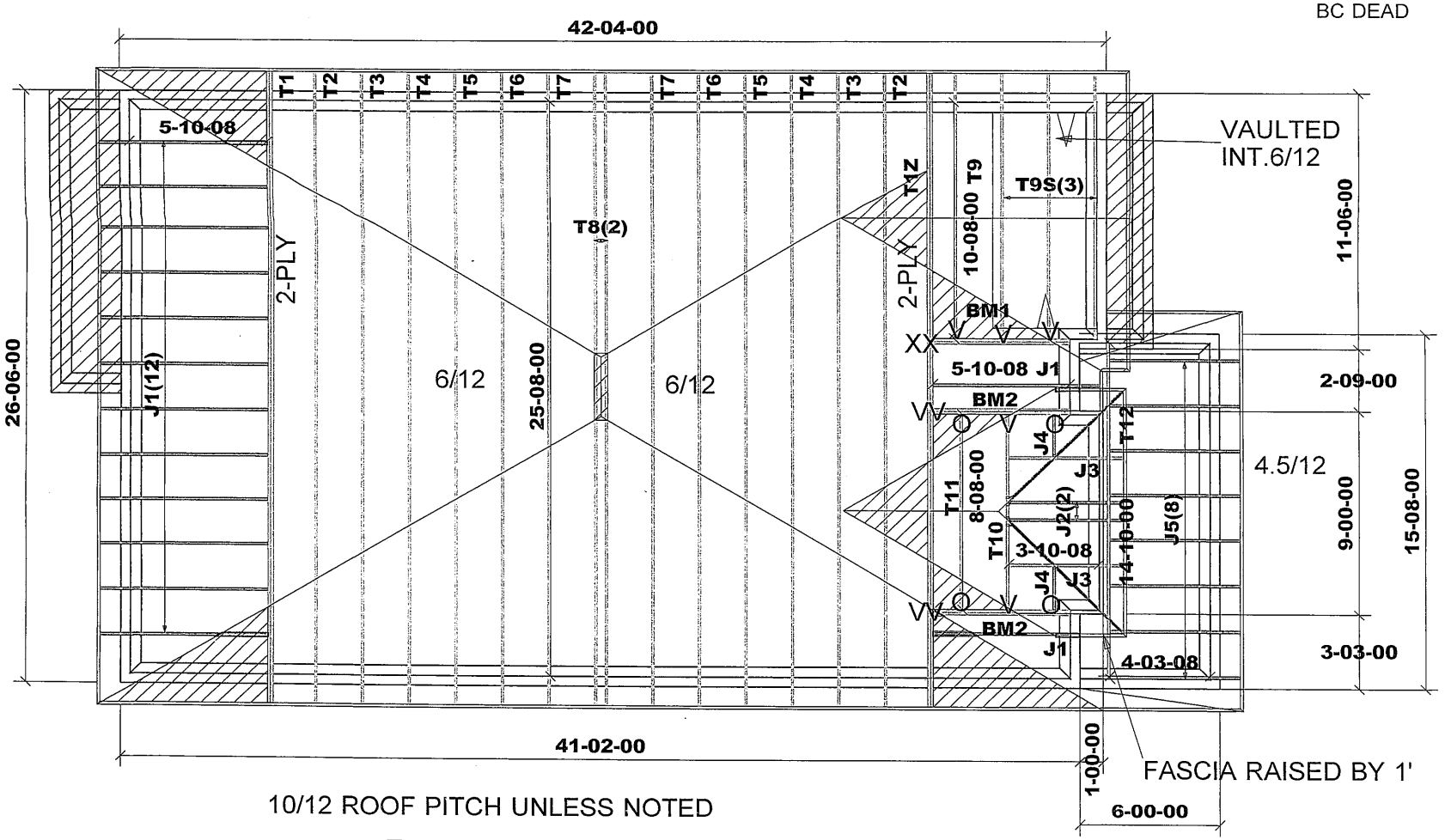
DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)

BM1,2 : 2-2X10

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



10/12 ROOF PITCH UNLESS NOTED

T-170678

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-1-10G / A
	Layout ID: 272179	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/6/2017 Designer: JG	

Mitek ver 7.5.0

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

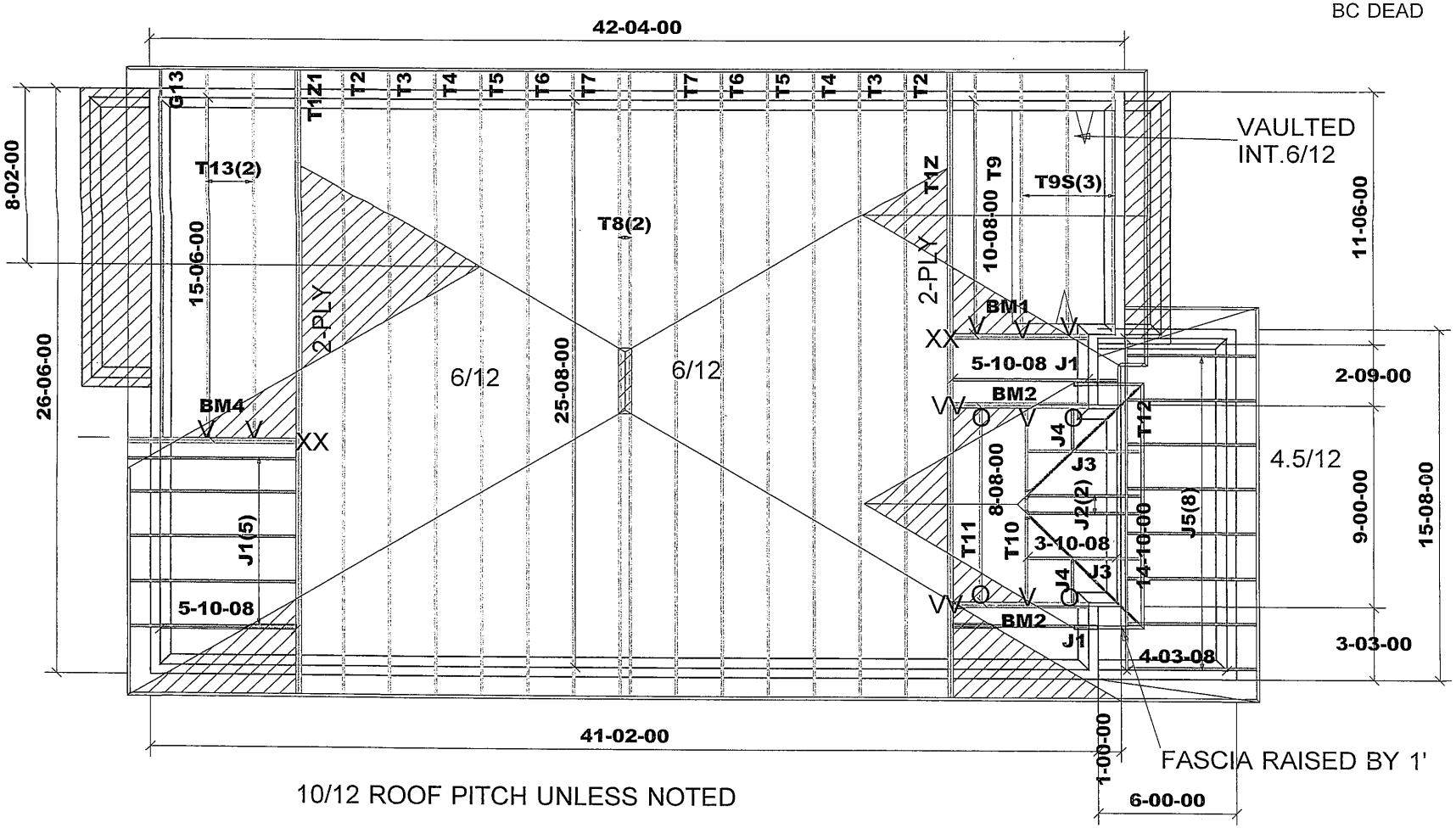
DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)

BM1,2,4 : 2-2X10

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



10/12 ROOF PITCH UNLESS NOTED

T-170678

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-1-10G / A-REAR UPGRADE
	Layout ID: 272205	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/6/2017 Designer: JG	

Mitek ver 7.5.0

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

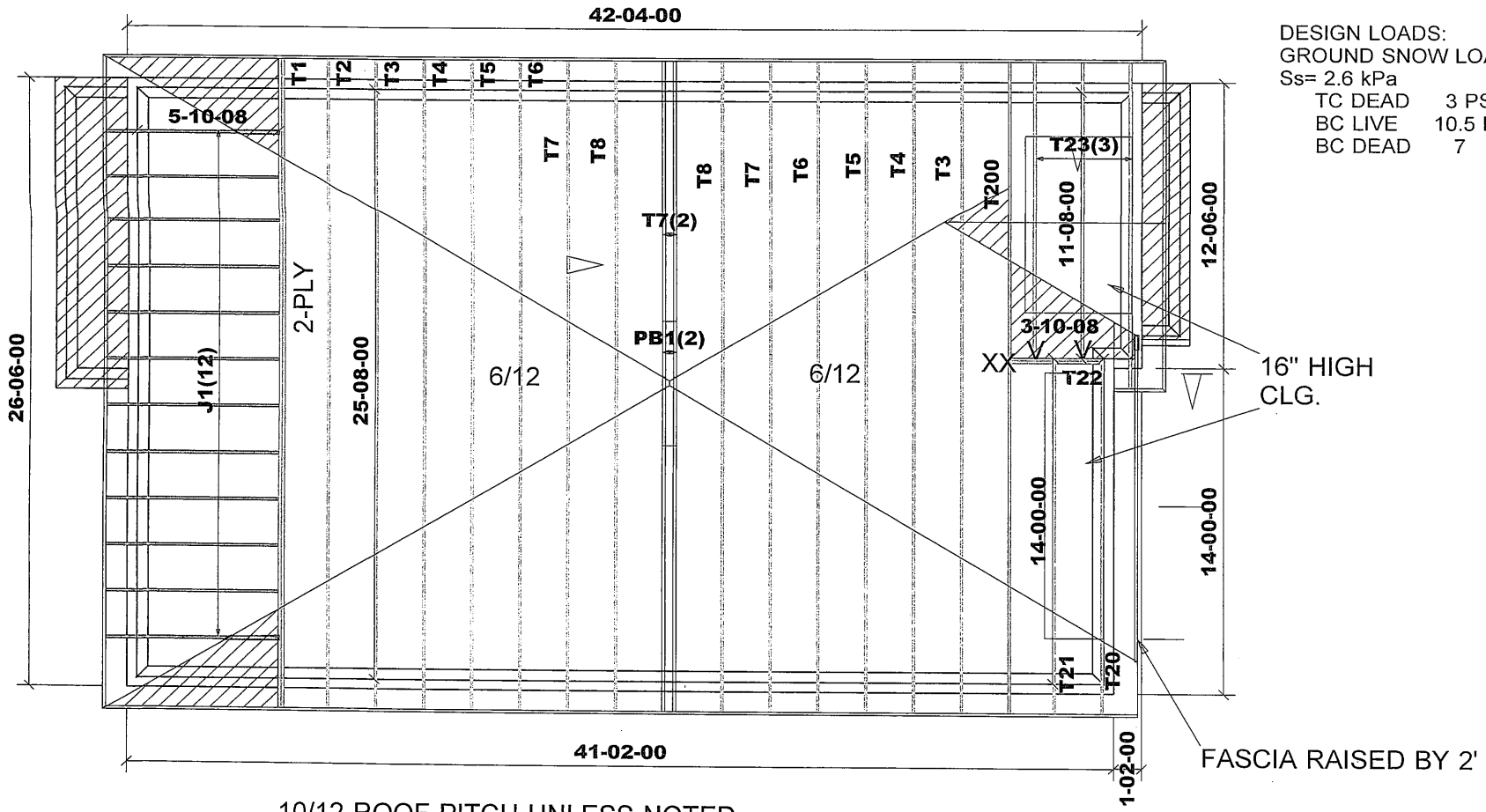
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



10/12 ROOF PITCH UNLESS NOTED

T-170678

Town of Innisfil Certified Model
15/03/2018 8:43:32 AM kgervais

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-1-10G / B
	Layout ID: 272180	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/6/2017 Designer: JG	

Mitek ver 7.5.0

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

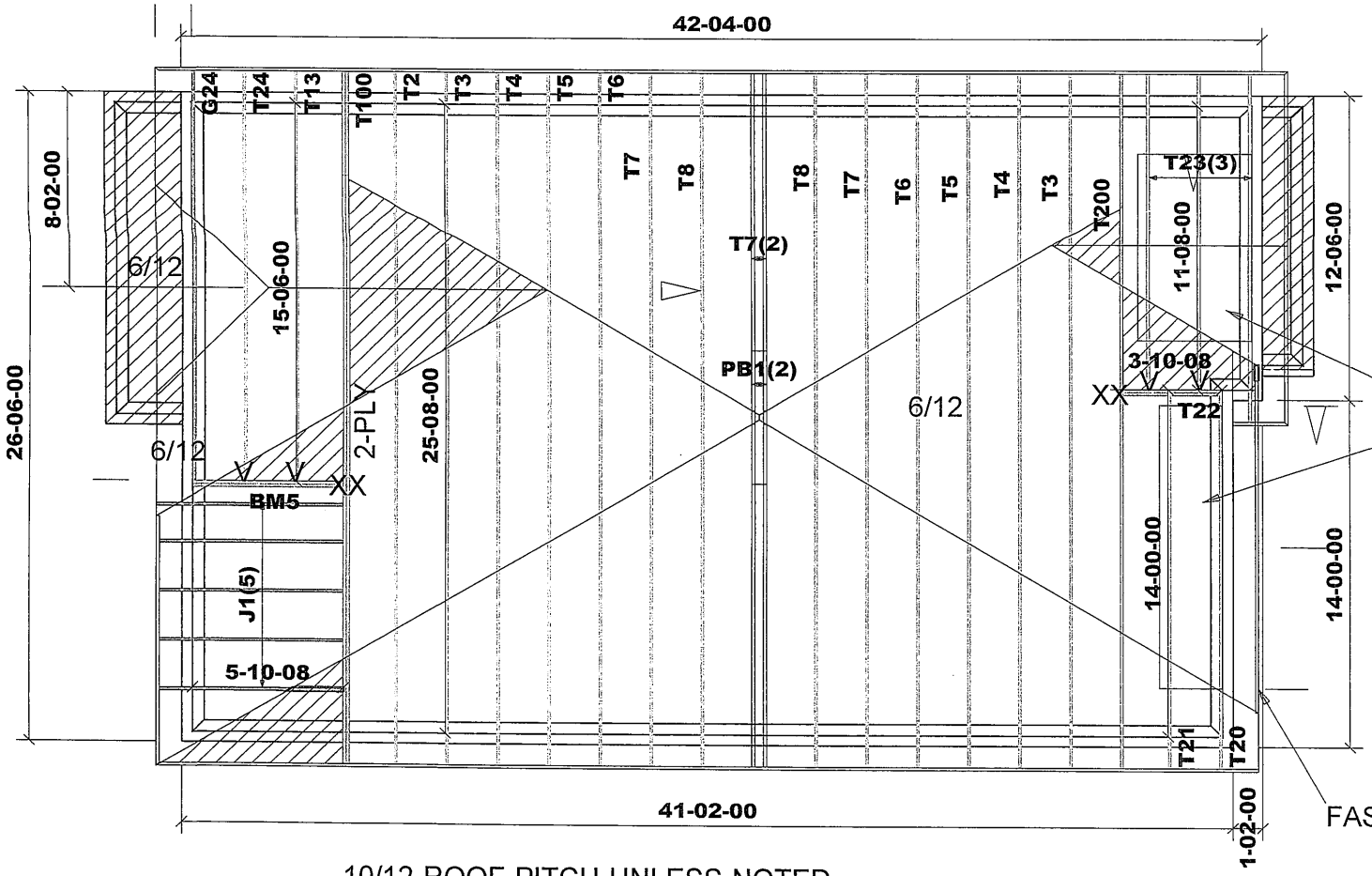
DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

BM5:2-2X10

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



10/12 ROOF PITCH UNLESS NOTED

T-170678

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-1-10G / B-REAR UPGRADE
	Layout ID: 272206	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/6/2017 Designer: JG	

Mitek ver 7.5.0

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

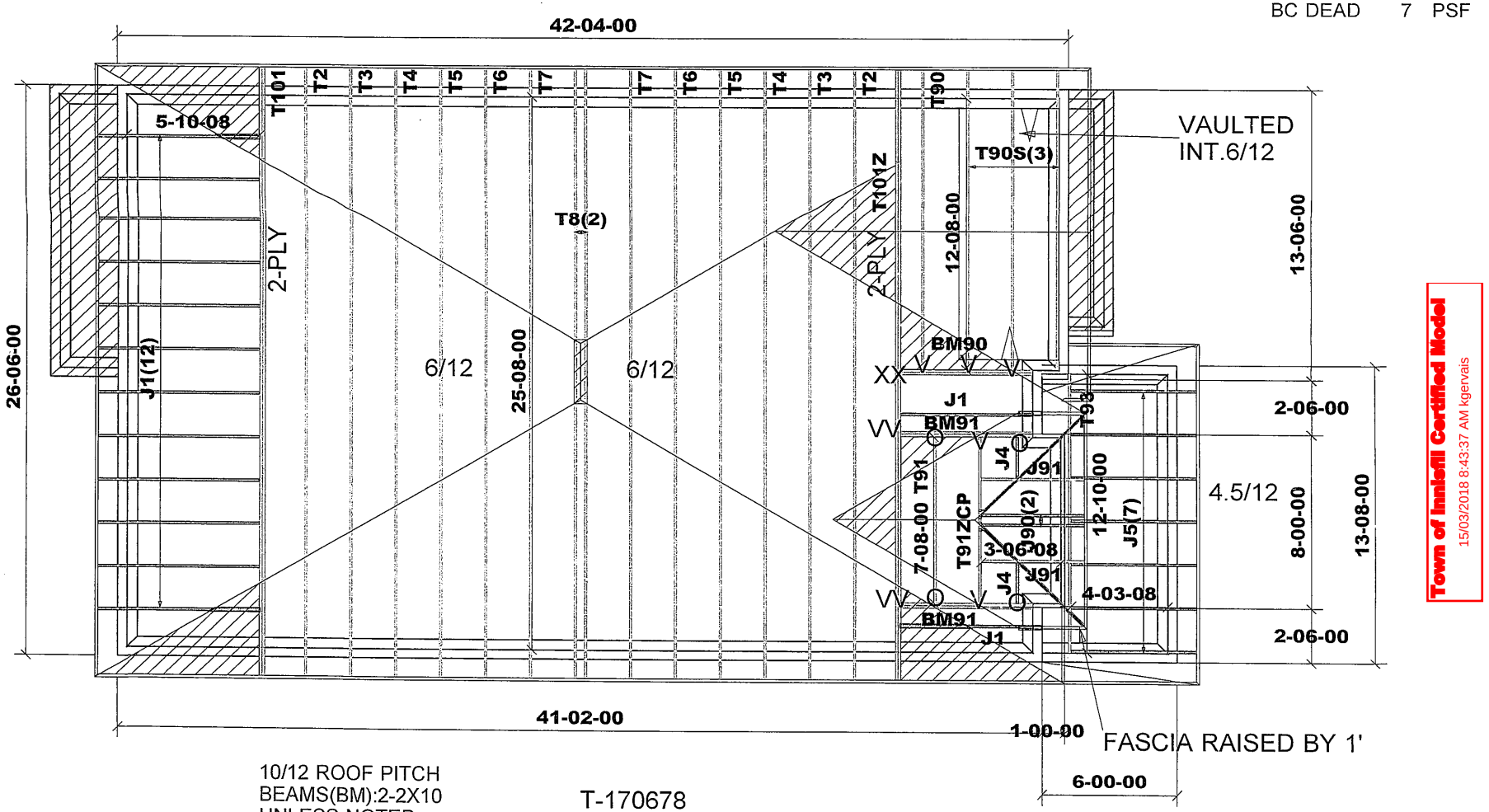
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-170678

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track: 42067
Layout ID: 272218
Plan Log: 87565

Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 9/6/2017 Designer: JG

Model / Elevation: S32-1-12G / A

Mitek ver 7.5.0

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

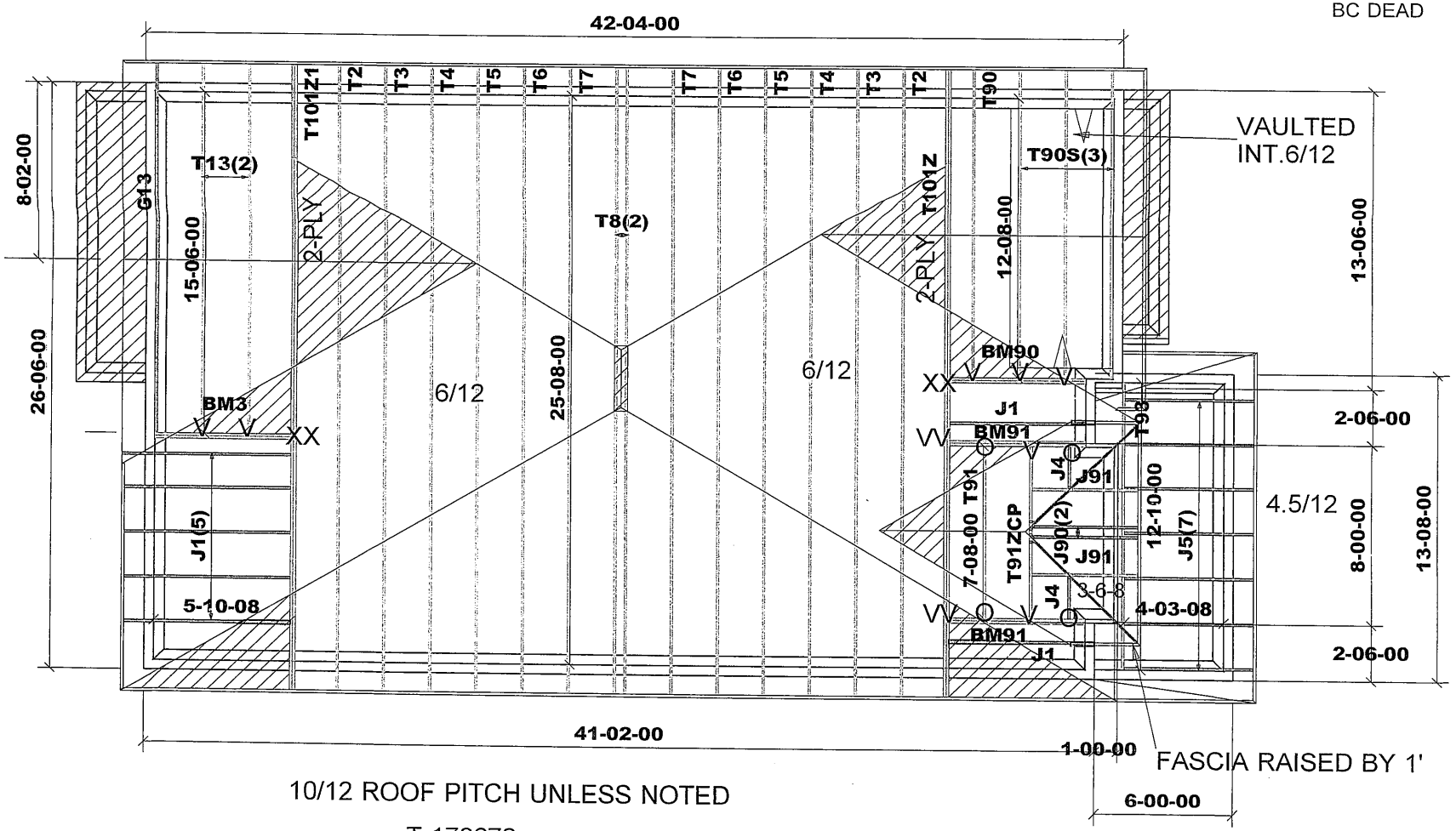
DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)

BM3,90,91 : 2-2X10

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



10/12 ROOF PITCH UNLESS NOTED

T-170678

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-1-12G / A-REAR UPG.
	Layout ID: 272220	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/6/2017 Designer: JG	

Mitek ver 7.5.0

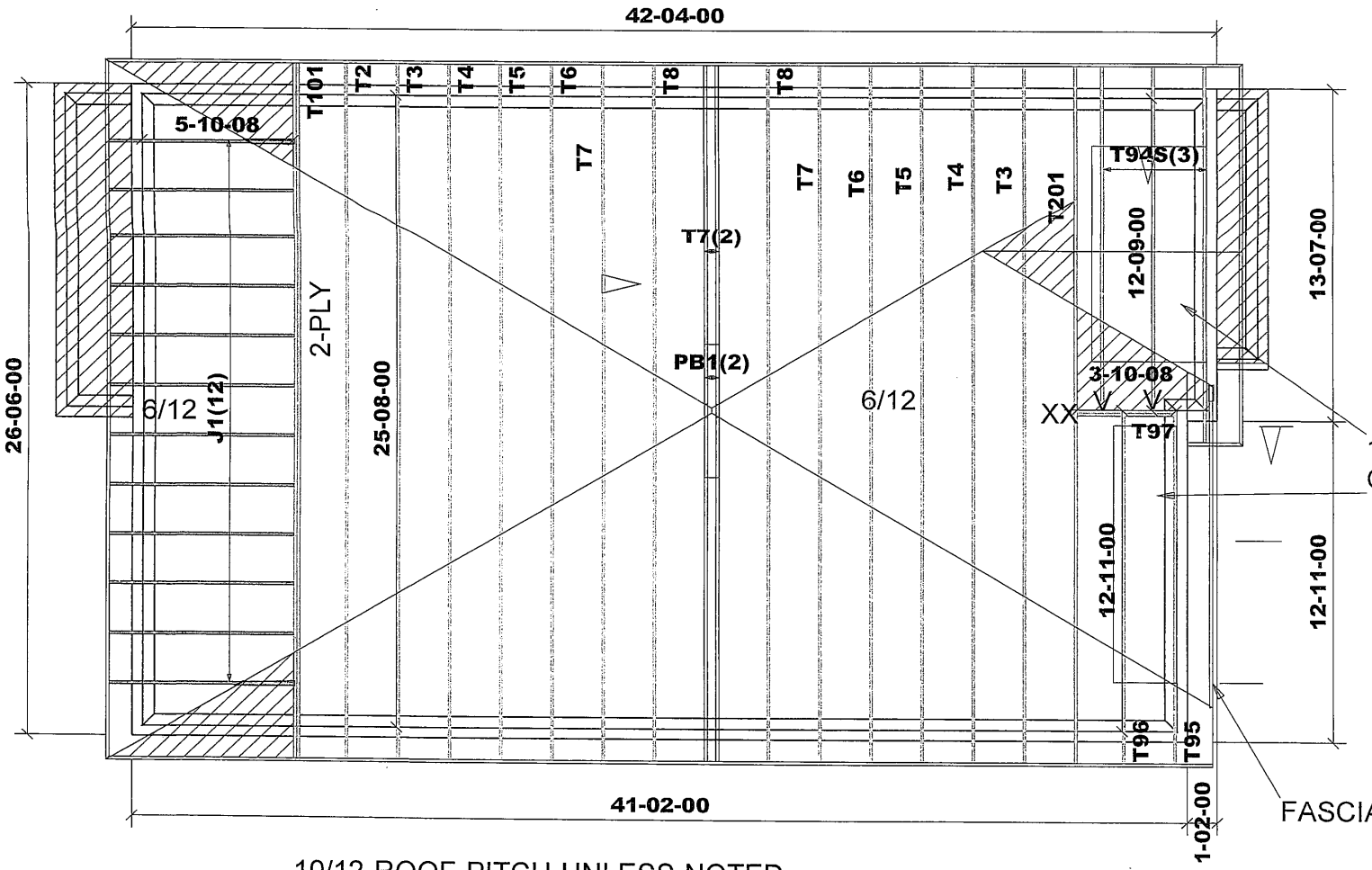
ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.
DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



10/12 ROOF PITCH UNLESS NOTED

T-170678

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-1-12G / B
	Layout ID: 272219	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/6/2017 Designer: JG	

Mitek ver 7.5.0

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

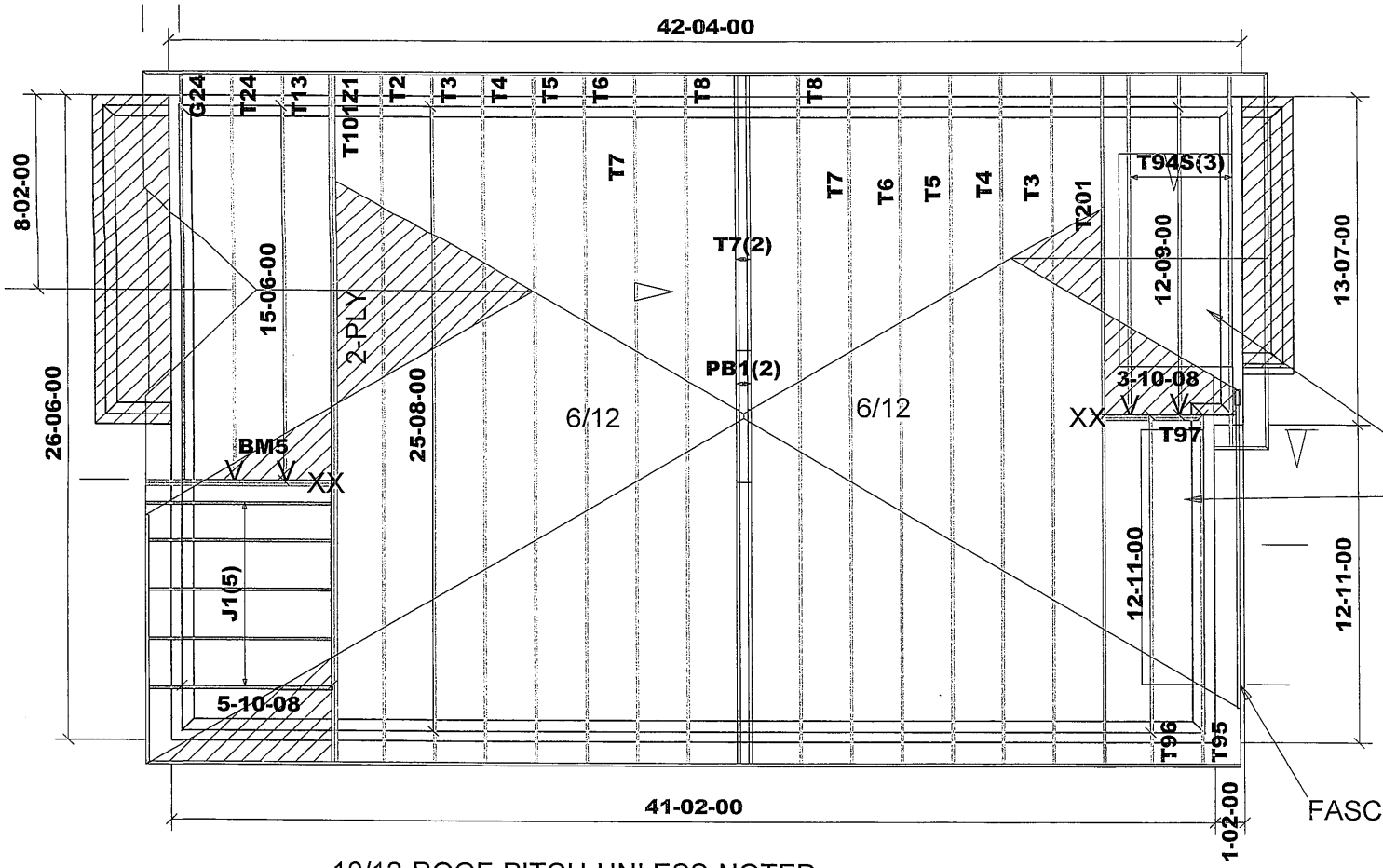
DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

BM5:2-2X10

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.6 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



10/12 ROOF PITCH UNLESS NOTED

T-170678

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S32-1-12G / B-REAR UPG.
	Layout ID: 272221	Project: ALCONA SHORES	
	Plan Log: 87565	Date: 9/6/2017 Designer: JG	

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

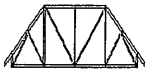
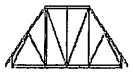
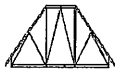
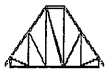
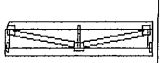
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272179 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-1-10G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	280.36 168.00		
	1 2 Ply	T1Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	280.36 168.00		
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	243.86 154.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1	T9 COMMON	10.00 0.00	10-08-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	47.84 31.00		
	3	T9S SCISSORS	10.00 6.00	10-08-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	142.68 90.51		
	1	T10 HIP GIRDER	10.00 0.00	08-08-00	05-07-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-03 02-05-03	42.90 29.67		
	1	T11 COMMON	10.00 0.00	08-08-00	06-00-08	2 X 4	2 X 4	00-00-00 00-00-00	02-05-03 02-05-03	39.45 26.00		
	1	T12 HALF HIP	0.00 0.00	14-10-00	01-11-05	2 X 6	2 X 6	00-00-00 00-00-00	01-05-07 01-11-05	73.41 46.33		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	2	J2 JACK-OPEN	10.00 0.00	03-10-08	05-07-15	2 X 4	2 X 4	01-00-08 00-00-00	02-05-03 05-07-15	34.86 22.66		
	2	J3 JACK-OPEN	10.00 0.00	03-10-08	03-11-01	2 X 4	2 X 4	01-00-08 -02-01-01	02-05-03 00-03-08	26.52 17.66		
	2	J4 JACK-OPEN	10.00 0.00	01-10-08	03-11-01	2 X 4	2 X 4	00-00-00 -00-01-01	02-05-03 00-03-08	14.70 10.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272179 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-1-10G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	8	J5 JACK-OPEN	4.50 0.00	04-03-08	01-11-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-00 01-11-05	94.56 64.00		

TOTAL # TRUSS= **53.00** TOTAL BFT OF ALL TRUSSES= **1900.89** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3016.50** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
4	Hangers	LUS24	
2	Hangers	LUS26-2	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

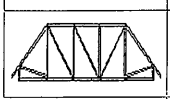
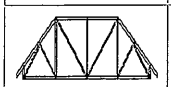
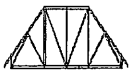
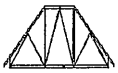
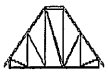
Delivery Shiplist

DATE	09/06/17
SALES REP	Rick

JOB TRACK: 42067 LAYOUT ID: 272205 LOCATION: INNISFIL
BUILDER: TREASURE HILL HOMES/121 CARMIC SUB-BUILDER:
MODEL: S32-1-10G ELEVATION: A-REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	280.36 168.00		
	1 2 Ply	T1Z1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	280.36 168.00		
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	243.86 154.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1	T9 COMMON	10.00 0.00	10-08-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	47.84 31.00		
	3	T9S SCISSORS	10.00 6.00	10-08-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	142.68 90.51		
	1	T10 HIP GIRDER	10.00 0.00	08-08-00	05-07-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-03 02-05-03	42.90 29.67		
	1	T11 COMMON	10.00 0.00	08-08-00	06-00-08	2 X 4	2 X 4	00-00-00 00-00-00	02-05-03 02-05-03	39.45 26.00		
	1	T12 HALF HIP	0.00 0.00	14-10-00	01-11-05	2 X 6	2 X 6	00-00-00 00-00-00	01-05-07 01-11-05	73.41 46.33		
	2	T13 COMMON	10.00 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	144.24 93.34		
	1	G13 COMMON	10.00 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	77.28 50.67		
	7	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00 00-00-00	01-02-00 04-01-04	118.65 74.69		
	2	J2 JACK-OPEN	10.00 0.00	03-10-08	05-07-15	2 X 4	2 X 4	01-00-08 00-00-00	02-05-03 05-07-15	34.86 22.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/06/17
SALES REP	Rick

JOB TRACK: 42067 **LAYOUT ID:** 272205 **LOCATION:** INNISFIL
BUILDER: TREASURE HILL HOMES/121 CARMIC **SUB-BUILDER:**
MODEL: S32-1-10G **ELEVATION:** A-REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	J3 JACK-OPEN	10.00 0.00	03-10-08	03-11-01	2 X 4	2 X 4	01-00-08 -02-01-01	02-05-03 00-03-08	26.52 17.66		
	2	J4 JACK-OPEN	10.00 0.00	01-10-08	03-11-01	2 X 4	2 X 4	00-00-00 -00-01-01	02-05-03 00-03-08	14.70 10.66		
	8	J5 JACK-OPEN	4.50 0.00	04-03-08	01-11-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-00 01-11-05	94.56 64.00		

TOTAL # TRUSS= **49.00** TOTAL BFT OF ALL TRUSSES= **1970.21** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3121.61** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
7	Hangers	LJS26DS	
4	Hangers	LUS24	
2	Hangers	LUS26-2	

TOTAL # ITEMS= **15.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

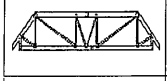
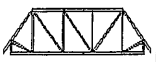
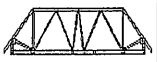
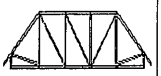
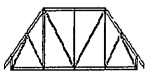
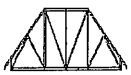
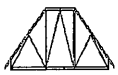
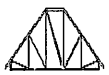
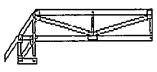
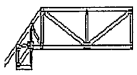
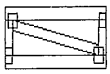
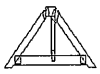
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272180 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-1-10G ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	280.36 168.00		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	107.29 69.67		
	1	T200 HIP GIRDER	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	126.06 79.67		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	243.86 154.00		
	4	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	516.16 326.68		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1	T20 HALF HIP	10.00 0.00	14-00-00	03-02-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-10-00	54.29 36.00		
	1	T21 HALF HIP	10.00 0.00	14-00-00	04-02-00	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	58.92 39.17		
	1 2 Ply	T22 FLAT GIRDER	0.00 0.00	03-10-08	01-04-00	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 01-04-00	26.58 17.34		
	3	T23 ROOF	10.00 0.00	11-08-00	06-06-00	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	182.04 117.51		
	2	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	26.82 20.66		
	12	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04		

TOTAL # TRUSS= **39.00** TOTAL BFT OF ALL TRUSSES= **1777.08** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2811.14** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
2	Hangers	LJS26DS	

TOTAL # ITEMS= **3.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

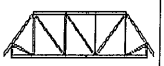
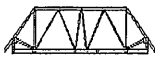
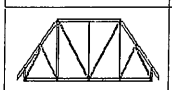
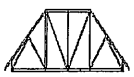
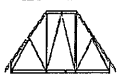
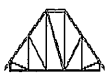
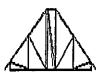
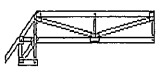
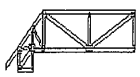
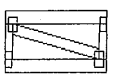
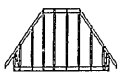
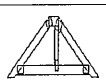
Delivery Shiplist

DATE	09/06/17
SALES REP	Rick

JOB TRACK: 42067 LAYOUT ID: 272206 LOCATION: INNISFIL
BUILDER: TREASURE HILL HOMES/121 CARMIC SUB-BUILDER:
MODEL: S32-1-10G ELEVATION: B-REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T100 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	241.32 154.66		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	107.29 69.67		
	1	T200 HIP GIRDER	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	126.06 79.67		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	243.86 154.00		
	4	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	516.16 326.68		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1	T13 COMMON	10.00 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	72.12 46.67		
	1	T20 HALF HIP	10.00 0.00	14-00-00	03-02-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-10-00	54.29 36.00		
	1	T21 HALF HIP	10.00 0.00	14-00-00	04-02-00	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	58.92 39.17		
	1 2 Ply	T22 FLAT GIRDER	0.00 0.00	03-10-08	01-04-00	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 01-04-00	26.58 17.34		
	3	T23 ROOF	10.00 0.00	11-08-00	06-06-00	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	182.04 117.51		
	1	T24 HIP	10.00 0.00	15-06-00	07-03-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	80.10 50.83		
	1	G24 HIP	10.00 0.00	15-06-00	05-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	73.44 49.33		
	2	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	26.82 20.66		
	5	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00 00-00-00	01-02-00 04-01-04	84.75 53.35		

TOTAL # TRUSS= **35.00** TOTAL BFT OF ALL TRUSSES= **1835.88** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2881.03** LBS.

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/06/17
SALES REP	Rick

JOB TRACK: 42067 **LAYOUT ID:** 272206 **LOCATION:** INNISFIL
BUILDER: TREASURE HILL HOMES/121 CARMIC **SUB-BUILDER:**
MODEL: S32-1-10G **ELEVATION:** B-REAR UPGRADE

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **6.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

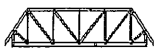
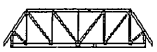
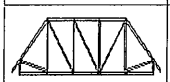
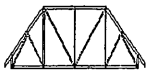
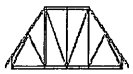
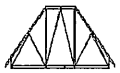
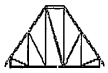
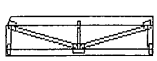
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272218 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-1-12G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T101 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	245.66 152.66		
	1 2 Ply	T101Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	245.66 152.66		
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	243.86 154.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1	T90 COMMON	10.00 0.00	12-08-00	06-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	60.76 39.50		
	3	T90S SCISSORS	10.00 6.00	12-08-00	06-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	173.31 113.01		
	1	T91 COMMON	10.00 0.00	07-08-00	05-07-08	2 X 4	2 X 4	00-00-00 00-00-00	02-05-03 02-05-03	35.77 23.83		
	1	T91ZCP HIP GIRDER	10.00 0.00	07-08-00	05-04-10	2 X 4	2 X 4	00-00-00 00-00-00	02-05-03 02-05-03	35.41 23.83		
	1	T93 HALF HIP	0.00 0.00	12-10-00	01-11-05	2 X 6	2 X 4	00-00-00 00-00-00	00-03-08 01-11-05	52.55 33.67		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	2	J4 JACK-OPEN	10.00 0.00	01-10-08	03-11-01	2 X 4	2 X 4	00-00-00 -00-01-01	02-05-03 00-03-08	14.70 10.66		
	7	J5 JACK-OPEN	4.50 0.00	04-03-08	01-11-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-00 01-11-05	82.74 56.00		
	2	J90 JACK-OPEN	10.00 0.00	03-06-08	05-04-10	2 X 4	2 X 4	01-00-08 00-00-00	02-05-03 05-04-10	32.40 22.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272218 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SH0 SUB-BUILDER:
MODEL: S32-1-12G ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	J91 JACK-OPEN	10.00 0.00	03-06-08	03-11-01	2 X 4	2 X 4	01-00-08 -01-09-01	02-05-03 00-03-08	25.76 17.66		

TOTAL # TRUSS= **52.00** TOTAL BFT OF ALL TRUSSES= **1872.54** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2943.58** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
2	Hangers	LUS24	
2	Hangers	LUS26-2	

TOTAL # ITEMS= **10.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

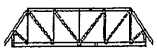
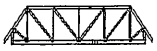
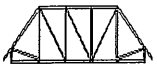
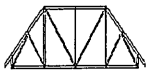
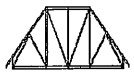
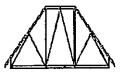
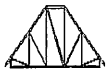
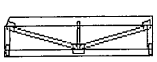
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272220 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SH0 SUB-BUILDER:
MODEL: S32-1-12G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T101Z HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	245.66 152.66		
	1 2 Ply	T101Z1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	245.66 152.66		
	2	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	214.58 139.34		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	243.86 154.00		
	2	T7 HIP	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	258.08 163.34		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	2	T13 COMMON	10.00 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	144.24 93.34		
	1	G13 COMMON	10.00 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	77.28 50.67		
	1	T90 COMMON	10.00 0.00	12-08-00	06-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	60.76 39.50		
	3	T90S SCISSORS	10.00 6.00	12-08-00	06-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	173.31 113.01		
	1	T91 COMMON	10.00 0.00	07-08-00	05-07-08	2 X 4	2 X 4	00-00-00 00-00-00	02-05-03 02-05-03	35.77 23.83		
	1	T91ZCP HIP GIRDER	10.00 0.00	07-08-00	05-04-10	2 X 4	2 X 4	00-00-00 00-00-00	02-05-03 02-05-03	35.41 23.83		
	1	T93 HALF HIP	0.00 0.00	12-10-00	01-11-05	2 X 6	2 X 4	00-00-00 00-00-00	00-03-08 01-11-05	52.55 33.67		
	7	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00 00-00-00	01-02-00 04-01-04	118.65 74.69		
	2	J4 JACK-OPEN	10.00 0.00	01-10-08	03-11-01	2 X 4	2 X 4	00-00-00 -00-01-01	02-05-03 00-03-08	14.70 10.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272220 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-1-12G ELEVATION: A-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	7	J5 JACK-OPEN	4.50 0.00	04-03-08	01-11-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-00 01-11-05	82.74 56.00		
	2	J90 JACK-OPEN	10.00 0.00	03-06-08	05-04-10	2 X 4	2 X 4	01-00-08 00-00-00	02-05-03 05-04-10	32.64 22.66		
	2	J91 JACK-OPEN	10.00 0.00	03-06-08	03-11-01	2 X 4	2 X 4	01-00-08 -01-09-01	02-05-03 00-03-08	25.76 17.66		

TOTAL # TRUSS= **48.00** TOTAL BFT OF ALL TRUSSES= **1941.86** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3048.93** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
7	Hangers	LJS26DS	
4	Hangers	LUS24	
2	Hangers	LUS26-2	

TOTAL # ITEMS= **15.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

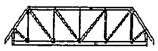
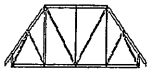
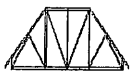
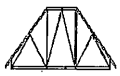
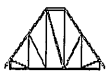
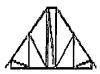
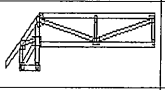
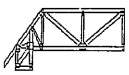
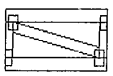
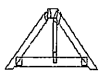
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272219 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S32-1-12G ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 / 2 Ply	T101 HIP GIRDER	10.00 / 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	245.66 / 152.66		
	1	T2 HIP	10.00 / 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	107.29 / 69.67		
	1	T201 HIP GIRDER	10.00 / 0.00	25-08-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	130.22 / 81.17		
	2	T3 HIP	10.00 / 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	227.14 / 144.00		
	2	T4 HIP	10.00 / 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	232.76 / 145.66		
	2	T5 HIP	10.00 / 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	244.52 / 155.34		
	2	T6 HIP	10.00 / 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	243.86 / 154.00		
	4	T7 PIGGYBACK	10.00 / 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	516.16 / 326.68		
	2	T8 HIP	10.00 / 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	282.86 / 175.34		
	3	T94S ROOF	10.00 / 0.00	12-09-00	06-11-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	194.22 / 126.48		
	1	T95 HALF HIP	10.00 / 0.00	12-11-00	03-02-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-10-00	51.19 / 34.67		
	1	T96 HALF HIP	10.00 / 0.00	12-11-00	04-02-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-10-00	56.37 / 37.00		
	1 / 2 Ply	T97 FLAT GIRDER	0.00 / 0.00	03-10-08	01-04-00	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-00 / 01-04-00	26.58 / 17.34		
	2	PB1 PIGGYBACK	10.00 / 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	26.82 / 20.66		
	12	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	201.48 / 128.04		

TOTAL # TRUSS= 39.00 TOTAL BFT OF ALL TRUSSES= 1768.71 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2787.13 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
2	Hangers	LJS26DS	

TOTAL # ITEMS= 3.00

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

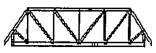
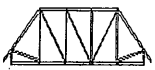
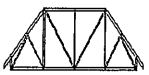
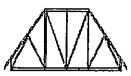
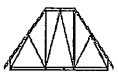
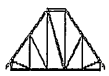
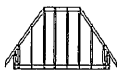
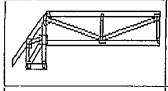
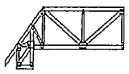
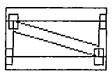
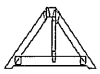
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 272221 **LOCATION:** INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO **SUB-BUILDER:**
MODEL: S32-1-12G **ELEVATION:** B-REAR

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T101Z1 HIP GIRDER	10.00 0.00	25-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	245.66 152.66		
	1	T2 HIP	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	107.29 69.67		
	1	T201 HIP GIRDER	10.00 0.00	25-08-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	130.22 81.17		
	2	T3 HIP	10.00 0.00	25-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	227.14 144.00		
	2	T4 HIP	10.00 0.00	25-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	232.76 145.66		
	2	T5 HIP	10.00 0.00	25-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.52 155.34		
	2	T6 HIP	10.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	243.86 154.00		
	4	T7 PIGGYBACK	10.00 0.00	25-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	516.16 326.68		
	2	T8 HIP	10.00 0.00	25-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	282.86 175.34		
	1	T13 COMMON	10.00 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	72.12 46.67		
	1	T24 HIP	10.00 0.00	15-06-00	07-03-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	80.10 50.83		
	1	G24 HIP	10.00 0.00	15-06-00	05-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	73.09 49.33		
	3	T94S ROOF	10.00 0.00	12-09-00	06-11-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	194.22 126.48		
	1	T95 HALF HIP	10.00 0.00	12-11-00	03-02-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-10-00	51.19 34.67		
	1	T96 HALF HIP	10.00 0.00	12-11-00	04-02-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-10-00	56.37 37.00		
	1 2 Ply	T97 FLAT GIRDER	0.00 0.00	03-10-08	01-04-00	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 01-04-00	26.58 17.34		
	2	PB1 PIGGYBACK	10.00 0.00	04-01-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	26.82 20.66		
	5	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00 00-00-00	01-02-00 04-01-04	84.75 53.35		

TOTAL # TRUSS= **35.00** TOTAL BFT OF ALL TRUSSES= **1840.85** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2895.71** LBS.

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 272221 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SH0 SUB-BUILDER:
MODEL: S32-1-12G ELEVATION: B-REAR

HARDWARE

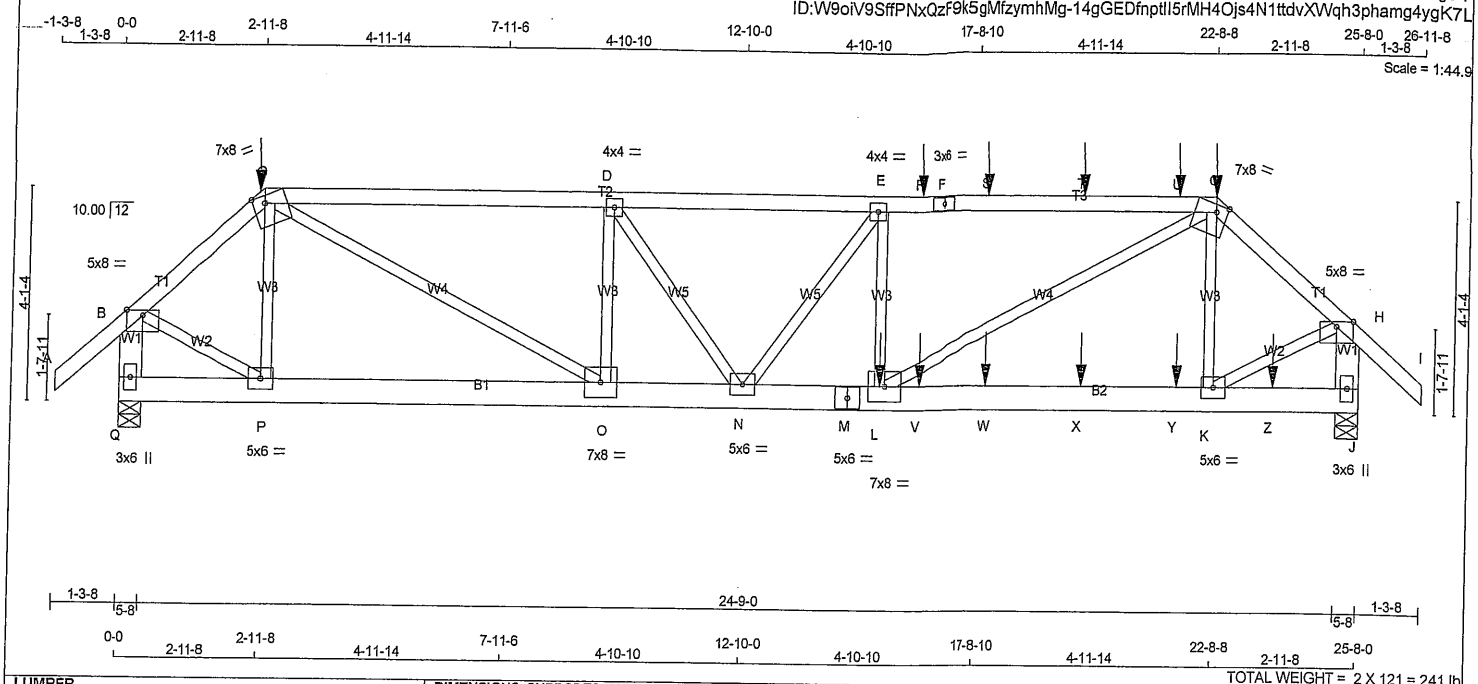
QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **6.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272206	T100	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Sep 06 15:29:44 2017 Page 1
ID:W9oiV9SffPNxQzF9k5gMfzymhMg-14gGEDfnptI5rMH4Ojs4N1ttdvXWqh3phamg4ygK7L



TOTAL WEIGHT = 2 X 121 = 241 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - G	2x4	DRY	2100F 1.8E	SPF
G - I	2x4	DRY	2100F 1.8E	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(65.9)
C- F	1	12	SIDE(65.9)
F- G	1	12	SIDE(65.9)
G- I	1	12	SIDE(65.9)
Q- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- M	2	12	SIDE(0.1)
M- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- L	1	6	SIDE(279.8)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	7.0	8.0	1.75	2.75
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TTWW-m	MT20	7.0	8.0	1.75	2.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K, N, P						
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	7.0	8.0		
M	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	7.0	8.0		
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3242	0	3242	0	0	5-8	5-8
J	4211	0	4211	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2504	1685 / 0	412 / 0	0 / 0	0 / 0	407 / 0	0 / 0
J	3272	2167 / 0	561 / 0	0 / 0	0 / 0	544 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.06 (1)	10.00
B- C	-3072 / 0	-122.2	-122.2	0.08 (1)	6.21
C- D	-5899 / 0	-122.2	-122.2	0.43 (1)	4.52
D- E	-6659 / 0	-122.2	-122.2	0.49 (1)	4.15
E- R	-7343 / 0	-122.2	-122.2	0.72 (1)	3.79
R- F	-7343 / 0	-122.2	-122.2	0.72 (1)	3.79
F- S	-7343 / 0	-122.2	-122.2	0.72 (1)	3.79
S- T	-7343 / 0	-122.2	-122.2	0.72 (1)	3.79
T- U	-7343 / 0	-122.2	-122.2	0.72 (1)	3.79
U- G	-7343 / 0	-122.2	-122.2	0.72 (1)	3.79
G- H	-4115 / 0	-122.2	-122.2	0.09 (1)	5.56
H- I	0 / 54	-122.2	-122.2	0.06 (1)	10.00
Q- B	-3265 / 0	0.0	0.0	0.12 (1)	7.68
J- H	-4255 / 0	0.0	0.0	0.16 (1)	6.94
Q- P	0 / 0	-28.0	-28.0	0.08 (2)	10.00
P- O	0 / 2332	-28.0	-28.0	0.23 (1)	10.00
O- N	0 / 5899	-28.0	-28.0	0.43 (1)	10.00
N- M	0 / 7342	-28.0	-28.0	0.54 (1)	10.00
M- L	0 / 7342	-28.0	-28.0	0.54 (1)	10.00
L- V	0 / 3126	-28.0	-28.0	0.32 (1)	10.00
V- W	0 / 3126	-28.0	-28.0	0.32 (1)	10.00
W- X	0 / 3126	-28.0	-28.0	0.32 (1)	10.00
X- Y	0 / 3126	-28.0	-28.0	0.32 (1)	10.00
Y- K	0 / 3126	-28.0	-28.0	0.32 (1)	10.00
K- Z	0 / 0	-28.0	-28.0	0.14 (2)	10.00
Z- J	0 / 0	-28.0	-28.0	0.14 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-691 / 0	0.09 (1)
C- O	0 / 4027	0.50 (1)
O- D	-1771 / 0	0.22 (1)
D- E	-308 / 266	0.04 (1)
L- E	0 / 4758	0.59 (1)
K- G	-842 / 69	0.11 (1)
B- P	0 / 2531	0.31 (1)
K- H	0 / 3391	0.42 (1)
D- N	0 / 1268	0.16 (1)
N- E	-1139 / 0	0.21 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-228	-228	---	FRONT	VERT	TOTAL
G	22-8-8	-455	-455	---	FRONT	VERT	TOTAL
L	15-7-8	-1774	-1774	---	FRONT	VERT	TOTAL
R	16-5-4	-147	-147	---	FRONT	VERT	TOTAL
S	17-11-4	-147	-147	---	FRONT	VERT	TOTAL
T	19-11-4	-147	-147	---	FRONT	VERT	TOTAL
U	21-11-4	-161	-161	---	FRONT	VERT	TOTAL
V	16-5-4	-40	-70	---	FRONT	VERT	TOTAL
W	17-11-4	-40	-70	---	FRONT	VERT	TOTAL
X	19-11-4	-40	-70	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.72 (E-G:1) , BC=0.54 (L-N:1) , WB=0.59 (G-L:1) , SSI=0.35 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.61 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9607 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 50122-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272206	T100	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 455.2 lbs FACTORED DOWN AT 22-8-8, 147.1 lbs FACTORED DOWN AT 16-5-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 1774.2 lbs FACTORED DOWN AT 15-7-8, 69.9 lbs FACTORED DOWN AT 16-5-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	21-11-4	-40	-70	—	FRONT	VERT	TOTAL
Z	23-11-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50122-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272218	T101	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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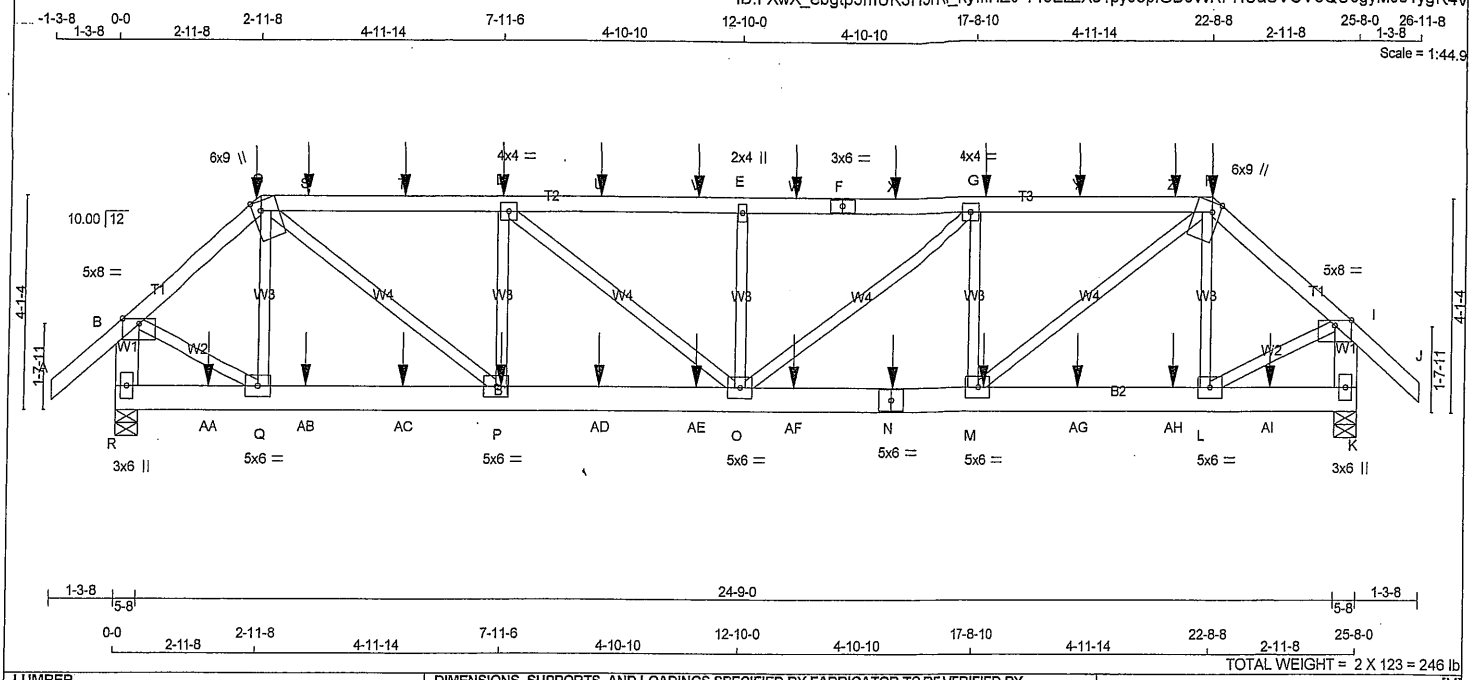

TOTAL WEIGHT = 2 X 123 = 246 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	3296	0	3296	0	0	5-8	5-8
K	3318	0	3318	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2593	1662 / 0	480 / 0	0 / 0	0 / 0	450 / 0	0 / 0
K	2609	1674 / 0	482 / 0	0 / 0	0 / 0	453 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. FROM	TO	LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.09 (1)		10.00
B- C	-3085 / 0	-122.2	-122.2	0.14 (1)		5.15
C- S	-4840 / 0	-122.2	-122.2	0.50 (1)		3.88
S- T	-4840 / 0	-122.2	-122.2	0.50 (1)		3.88
T- D	-4840 / 0	-122.2	-122.2	0.50 (1)		3.88
D- U	-5565 / 0	-122.2	-122.2	0.53 (1)		3.60
U- V	-5565 / 0	-122.2	-122.2	0.53 (1)		3.60
V- E	-5565 / 0	-122.2	-122.2	0.53 (1)		3.60
E- W	-5565 / 0	-122.2	-122.2	0.53 (1)		3.60
W- F	-5565 / 0	-122.2	-122.2	0.53 (1)		3.60
F- X	-5565 / 0	-122.2	-122.2	0.53 (1)		3.60
X- G	-5565 / 0	-122.2	-122.2	0.53 (1)		3.60
G- Y	-4849 / 0	-122.2	-122.2	0.50 (1)		3.87
Y- Z	-4849 / 0	-122.2	-122.2	0.50 (1)		3.87
Z- H	-4849 / 0	-122.2	-122.2	0.50 (1)		3.87
H- I	-3105 / 0	-122.2	-122.2	0.14 (1)		5.13
I- J	0 / 54	-122.2	-122.2	0.09 (1)		10.00
R- B	-3278 / 0	0.0	0.0	0.12 (1)		7.66
K- I	-3296 / 0	0.0	0.0	0.12 (1)		7.64
R-AA	0 / 0	-28.0	-28.0	0.06 (2)		10.00
AA- Q	0 / 0	-28.0	-28.0	0.06 (2)		10.00
Q-AB	0 / 2342	-28.0	-28.0	0.20 (1)		10.00
AB-AC	0 / 2342	-28.0	-28.0	0.20 (1)		10.00
AC- P	0 / 2342	-28.0	-28.0	0.20 (1)		10.00
P-AD	0 / 4840	-28.0	-28.0	0.38 (1)		10.00
AD-AE	0 / 4840	-28.0	-28.0	0.38 (1)		10.00
AE- O	0 / 4840	-28.0	-28.0	0.38 (1)		10.00
O-AF	0 / 4849	-28.0	-28.0	0.38 (1)		10.00
AF- N	0 / 4849	-28.0	-28.0	0.38 (1)		10.00
N- M	0 / 4849	-28.0	-28.0	0.38 (1)		10.00
M-AG	0 / 2357	-28.0	-28.0	0.20 (1)		10.00
AG-AH	0 / 2357	-28.0	-28.0	0.20 (1)		10.00
AH- L	0 / 2357	-28.0	-28.0	0.20 (1)		10.00
L-AI	0 / 0	-28.0	-28.0	0.06 (2)		10.00
AI- K	0 / 0	-28.0	-28.0	0.06 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-690 / 0	0.09 (1)
C- P	0 / 3112	0.39 (1)
P- D	-1639 / 0	0.21 (1)
D- O	0 / 912	0.11 (1)
O- E	-880 / 0	0.11 (1)
O- G	0 / 900	0.11 (1)
M- G	-1635 / 0	0.21 (1)
M- H	0 / 3105	0.38 (1)
L- H	-696 / 0	0.09 (1)
B- Q	0 / 2543	0.31 (1)
L- I	0 / 2559	0.32 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 500861

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.53 (E-G:1) , BC=0.38 (M-O:1) , WB=0.39 (C-P:1) , SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.43 (N) (INPUT = 1.00)

Pg 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272218	T101	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:FXwX_ebgtp5mUK3H5rRi_KymHZ9-T45EzZXe4py9oplGD9WRPHUaSVCV3QU0gyM081ygK4v

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 227.6 lbs FACTORED DOWN AT 22-8-8, 148.2 lbs FACTORED DOWN AT 3-11-4, 147.1 lbs FACTORED DOWN AT 5-11-4, 147.1 lbs FACTORED DOWN AT 7-11-4, 147.1 lbs FACTORED DOWN AT 9-11-4, 147.1 lbs FACTORED DOWN AT 11-11-4, 147.1 lbs FACTORED DOWN AT 13-11-4, 147.1 lbs FACTORED DOWN AT 15-11-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-228	-228	—	FRONT	VERT	TOTAL
D	7-11-4	-147	-147	—	FRONT	VERT	TOTAL
G	17-11-4	-147	-147	—	FRONT	VERT	TOTAL
H	22-8-8	-228	-228	—	FRONT	VERT	TOTAL
M	17-11-4	-40	-70	—	FRONT	VERT	TOTAL
N	15-11-4	-40	-70	—	FRONT	VERT	TOTAL
P	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
S	3-11-4	-148	-148	—	FRONT	VERT	TOTAL
T	5-11-4	-147	-147	—	FRONT	VERT	TOTAL
U	9-11-4	-147	-147	—	FRONT	VERT	TOTAL
V	11-11-4	-147	-147	—	FRONT	VERT	TOTAL
W	13-11-4	-147	-147	—	FRONT	VERT	TOTAL
X	15-11-4	-147	-147	—	FRONT	VERT	TOTAL
Y	19-11-4	-147	-147	—	FRONT	VERT	TOTAL
Z	21-11-4	-161	-161	—	FRONT	VERT	TOTAL
AA	1-11-4	-40	-70	—	FRONT	VERT	TOTAL
AB	3-11-4	-40	-70	—	FRONT	VERT	TOTAL
AC	5-11-4	-40	-70	—	FRONT	VERT	TOTAL
AD	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
AE	11-11-4	-40	-70	—	FRONT	VERT	TOTAL
AF	13-11-4	-40	-70	—	FRONT	VERT	TOTAL
AG	19-11-4	-40	-70	—	FRONT	VERT	TOTAL
AH	21-11-4	-40	-70	—	FRONT	VERT	TOTAL
AI	23-11-4	-40	-70	—	FRONT	VERT	TOTAL

DWG NO. TAM5008217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272218	T101Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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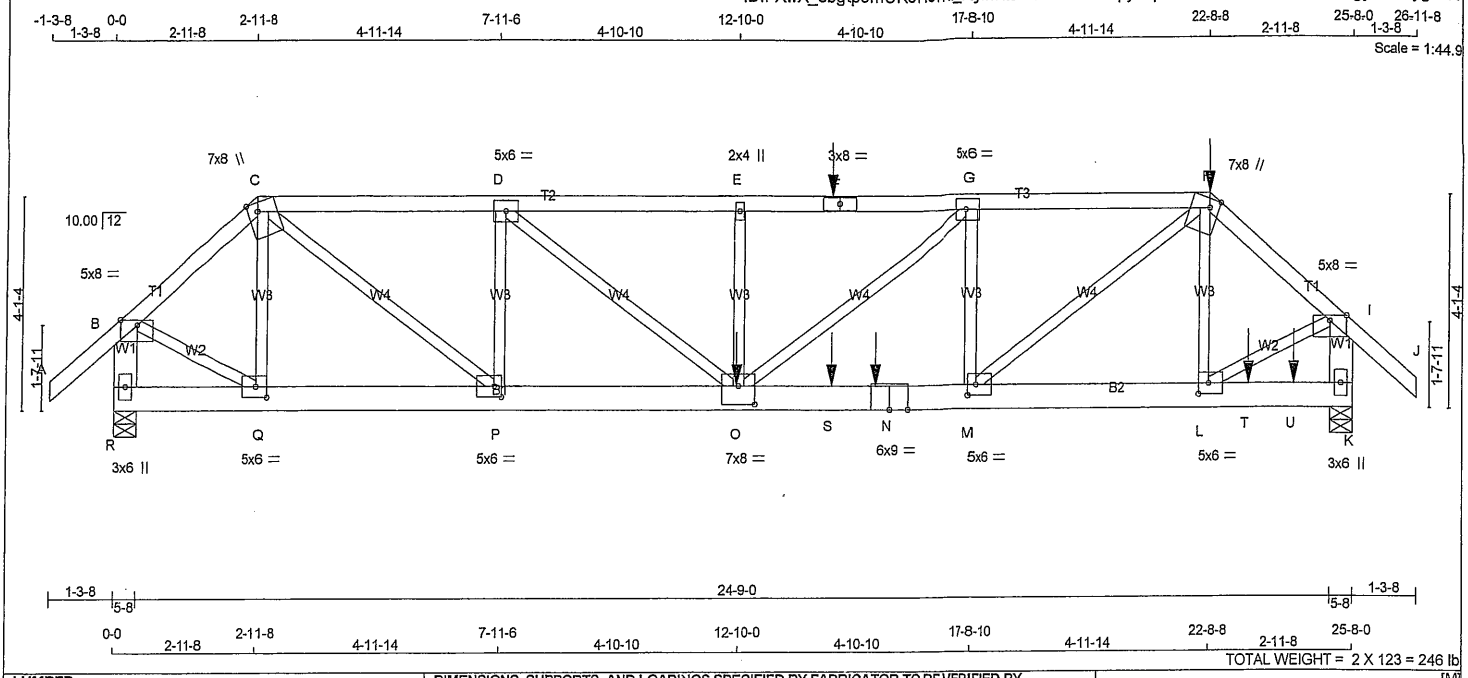


TOTAL WEIGHT = 2 X 123 = 246 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(0.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- O	1	5	SIDE(348.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0	2.50	1.75
N	BS-t	MT20	6.0	9.0		
O	BMWWW-t	MT20	7.0	8.0	4.25	4.00
P	BMWW-t	MT20	5.0	6.0	2.50	1.75
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3778	0	3778	0	0	5-8	5-8
K	5308	0	5308	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2893	1990 / 0	448 / 0	0 / 0	0 / 0	456 / 0	0 / 0
K	4070	2789 / 0	635 / 0	0 / 0	0 / 0	646 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00		Q- C	-895 / 0	0.11 (1)
B- C	-3615 / 0	-122.2	-122.2	0.15 (1)	4.82		C- P	0 / 4635	0.57 (1)
C- D	-6462 / 0	-122.2	-122.2	0.43 (1)	3.52		P- D	-2730 / 0	0.35 (1)
D- E	-9187 / 0	-122.2	-122.2	0.64 (1)	2.80		D- O	0 / 3427	0.42 (1)
E- F	-9187 / 0	-122.2	-122.2	0.68 (1)	2.75		O- E	-636 / 0	0.08 (1)
F- G	-9187 / 0	-122.2	-122.2	0.68 (1)	2.75		O- G	0 / 1975	0.24 (1)
G- H	-7617 / 0	-122.2	-122.2	0.56 (1)	3.14		M- G	-1924 / 0	0.24 (1)
H- I	-4838 / 0	-122.2	-122.2	0.19 (1)	4.24		M- H	0 / 4892	0.61 (1)
I- J	0 / 54	-122.2	-122.2	0.09 (1)	10.00		L- H	-484 / 0	0.06 (1)
R- B	-3781 / 0	0.0	0.0	0.14 (1)	7.26		B- Q	0 / 2979	0.37 (1)
K- I	-4942 / 0	0.0	0.0	0.18 (1)	6.53		L- I	0 / 3988	0.49 (1)
R- Q	0 / 0	-28.0	-28.0	0.05 (1)	10.00				
Q- P	0 / 2742	-28.0	-28.0	0.23 (1)	10.00				
P- O	0 / 6462	-28.0	-28.0	0.47 (1)	10.00				
O- S	0 / 7617	-28.0	-28.0	0.93 (1)	10.00				
S- N	0 / 7617	-28.0	-28.0	0.93 (1)	10.00				
N- M	0 / 7617	-28.0	-28.0	0.93 (1)	10.00				
M- L	0 / 3690	-28.0	-28.0	0.38 (1)	10.00				
L- T	0 / 0	-28.0	-28.0	0.25 (1)	10.00				
T- U	0 / 0	-28.0	-28.0	0.25 (1)	10.00				
U- K	0 / 0	-28.0	-28.0	0.25 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
F	14-8-12	-147	-147	---	BACK	VERT	TOTAL
H	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
N	15-7-8	-1200	-1200	---	BACK	VERT	TOTAL
O	12-9-8	-2037	-2037	---	BACK	VERT	TOTAL
S	14-8-12	-40	-70	---	BACK	VERT	TOTAL
T	23-6-8	-1200	-1200	---	BACK	VERT	TOTAL
U	24-5-12	-40	-70	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 9607 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM50087-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD.		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 888 (0.35")

CSI: TC=0.68 (E-G:1) , BC=0.93 (M-O:1) , WB=0.61 (H-M:1) , SSI=0.32 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.84 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 22-8-8, AND 147.1 lbs FACTORED DOWN AT 14-8-12 ON TOP CHORD, AND 2037.2 lbs FACTORED DOWN AT 12-9-8, 69.9 lbs FACTORED DOWN AT 14-8-12, 1200.2 lbs FACTORED DOWN AT 15-7-8, AND 1200.2 lbs FACTORED DOWN AT 23-6-8, AND 69.9 lbs FACTORED DOWN AT 24-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM5008Z-17
STRUCTURAL
COMPONENT ONLY

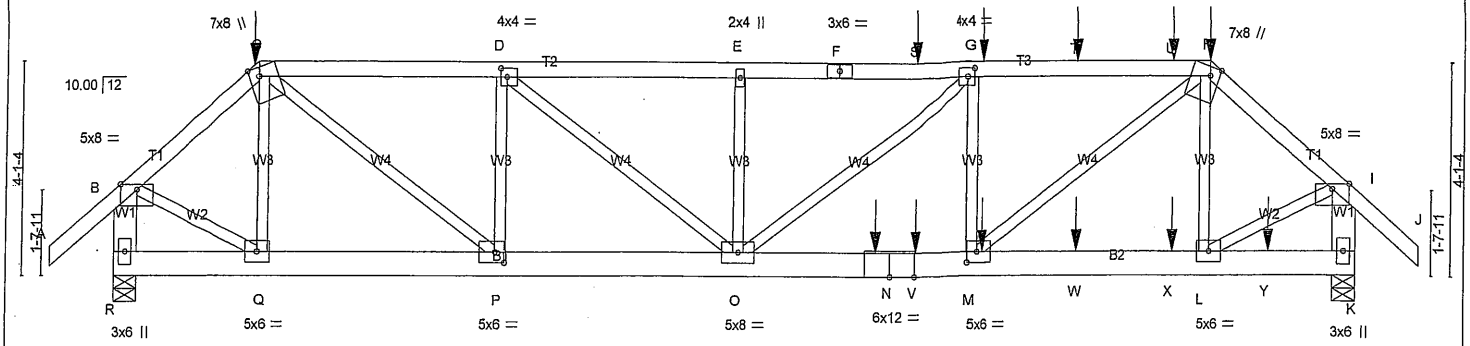
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272220	T101Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:FXwX_ebgtp5mUK3H5rRi_KymHZ9-v5WCjvQULkb1WnDFMwJ1kAyESOHEcxK_YD9Gd_ygK2T

1-3-8 0-0 2-11-8 2-11-8 4-11-14 7-11-6 4-10-10 12-10-0 4-10-10 17-8-10 4-11-14 22-8-8 2-11-8 25-8-0 1-3-8 26-11-8

Scale = 1:44.9



1-3-8 5-8 24-9-0 1-3-8 5-8

0-0 2-11-8 2-11-8 4-11-14 7-11-6 4-10-10 12-10-0 4-10-10 17-8-10 4-11-14 22-8-8 2-11-8 25-8-0

TOTAL WEIGHT = 2 X 123 = 246 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	6.0	12.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	3197	0	3197	0	0	5-8	5-8
K	4028	0	4028	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2469	1662 / 0	406 / 0	0 / 0	0 / 0	401 / 0	0 / 0
K	3129	2074 / 0	535 / 0	0 / 0	0 / 0	519 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00		Q- C	-703 / 0	0.09 (1)
B- C	-2999 / 0	-122.2	-122.2	0.14 (1)	5.20		C- P	0 / 3284	0.41 (1)
C- D	-4912 / 0	-122.2	-122.2	0.36 (1)	4.04		P- D	-2004 / 0	0.25 (1)
D- E	-6664 / 0	-122.2	-122.2	0.46 (1)	3.44		D- O	0 / 2203	0.27 (1)
E- F	-6664 / 0	-122.2	-122.2	0.57 (1)	3.29		O- E	-556 / 0	0.07 (1)
F- S	-6664 / 0	-122.2	-122.2	0.57 (1)	3.29		O- G	0 / 175	0.02 (1)
S- G	-6664 / 0	-122.2	-122.2	0.57 (1)	3.29		M- G	-1184 / 0	0.15 (1)
G- T	-6524 / 0	-122.2	-122.2	0.57 (1)	3.33		M- H	0 / 4548	0.56 (1)
T- U	-6524 / 0	-122.2	-122.2	0.57 (1)	3.33		L- H	-1126 / 0	0.14 (1)
U- H	-6524 / 0	-122.2	-122.2	0.57 (1)	3.33		B- Q	0 / 2472	0.31 (1)
H- I	-3796 / 0	-122.2	-122.2	0.16 (1)	4.72		L- I	0 / 3128	0.39 (1)
I- J	0 / 54	-122.2	-122.2	0.09 (1)	10.00				
R- B	-3197 / 0	0.0	0.0	0.12 (1)	7.74				
K- I	-3952 / 0	0.0	0.0	0.15 (1)	7.14				
R- Q	0 / 0	-28.0	-28.0	0.05 (1)	10.00				
Q- P	0 / 2276	-28.0	-28.0	0.21 (1)	10.00				
P- O	0 / 4912	-28.0	-28.0	0.48 (1)	10.00				
O- N	0 / 6524	-28.0	-28.0	1.00 (1)	10.00				
N- V	0 / 6524	-28.0	-28.0	1.00 (1)	10.00				
V- M	0 / 6524	-28.0	-28.0	1.00 (1)	10.00				
M- W	0 / 2874	-28.0	-28.0	0.42 (1)	10.00				
W- X	0 / 2874	-28.0	-28.0	0.42 (1)	10.00				
X- L	0 / 2874	-28.0	-28.0	0.42 (1)	10.00				
L- Y	0 / 0	-28.0	-28.0	0.03 (3)	10.00				
Y- K	0 / 0	-28.0	-28.0	0.03 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-228	-228	---	FRONT	VERT	TOTAL
G	17-11-4	-147	-147	---	FRONT	VERT	TOTAL
H	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
M	17-11-4	-40	-70	---	FRONT	VERT	TOTAL
N	15-7-8	-1774	-1774	---	FRONT	VERT	TOTAL
S	16-5-4	-147	-147	---	FRONT	VERT	TOTAL
T	19-11-4	-147	-147	---	FRONT	VERT	TOTAL
U	21-11-4	-161	-161	---	FRONT	VERT	TOTAL
V	16-5-4	-40	-70	---	FRONT	VERT	TOTAL
W	19-11-4	-40	-70	---	FRONT	VERT	TOTAL
X	21-11-4	-40	-70	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.57 (E-G:1) , BC=1.00 (M-O:1) , WB=0.56 (H-M:1) , SSI=0.42 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.89 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50093-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272220	T101Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:FXwX_ebgtp5mUK3H5rRi_KyrmHZ9-v5WCjvQULkb1WnDFMwJ1kAyESOHEcxK_YD9Gd_ygK2T

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 227.6 lbs FACTORED DOWN AT 22-8-8, 147.1 lbs FACTORED DOWN AT 16-5-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 1774.2 lbs FACTORED DOWN AT 15-7-8, 69.9 lbs FACTORED DOWN AT 16-5-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	23-11-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50093-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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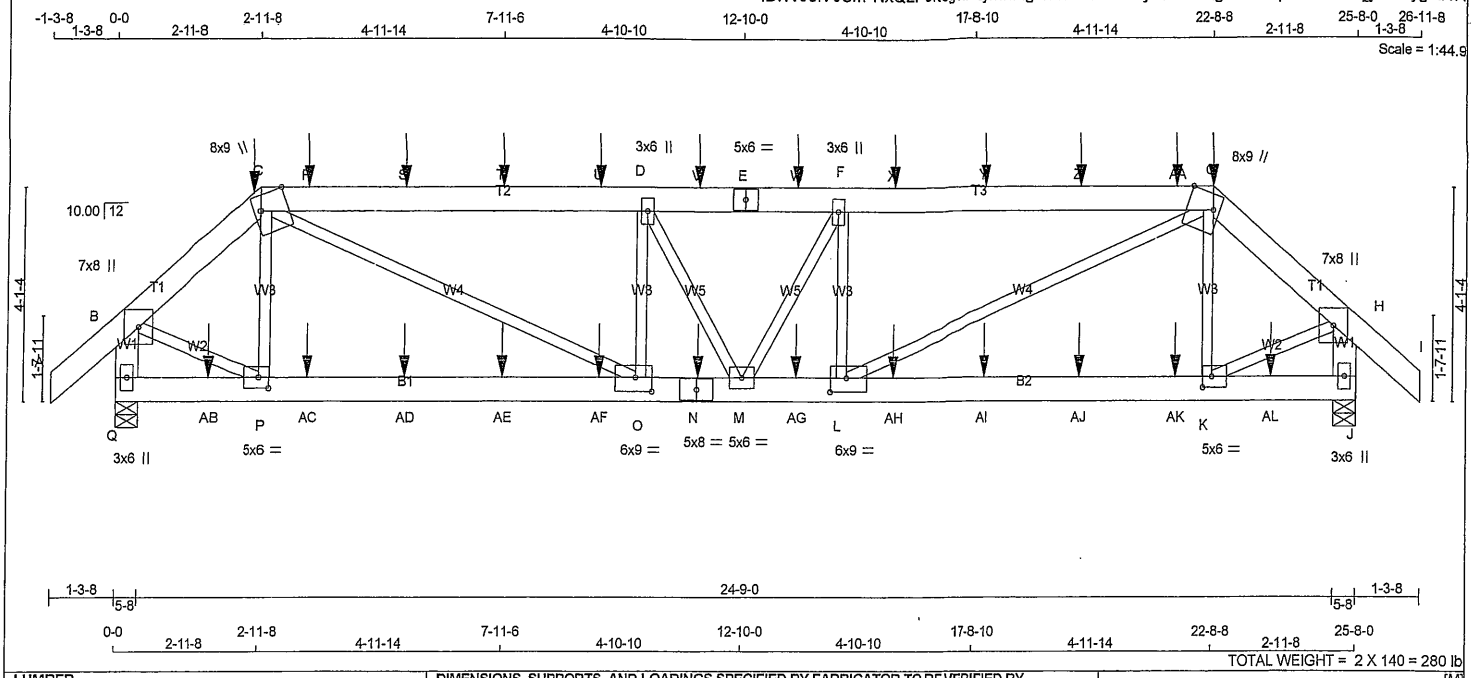


TOTAL WEIGHT = 2 X 140 = 280 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- E	2	12	SIDE(61.0)
E- G	2	12	SIDE(61.0)
G- I	2	12	SIDE(122.0)
Q- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	SIDE(0.0)
N- J	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	7.0	8.0		
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW+t	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	3.0	6.0		
G	TTWW+m	MT20	8.0	9.0	Edge	6.50
H	TMVW+p	MT20	7.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	6.0	9.0	3.25	4.00
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	8.0		
O	BMWW-t	MT20	6.0	9.0	3.25	4.00
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	3301	0	3301	0	0	5-8	5-8
J	3326	0	3326	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2596	1666 / 0	480 / 0	0 / 0	0 / 0	450 / 0	0 / 0
J	2615	1679 / 0	483 / 0	0 / 0	0 / 0	453 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

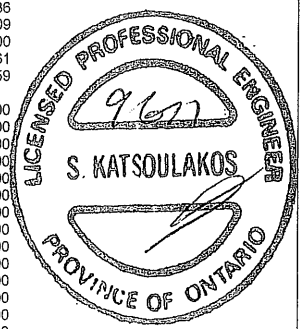
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.05 (1)	10.00
B- C	-3233 / 0	-122.2	-122.2	0.06 (1)	6.10
C- R	-5705 / 0	-122.2	-122.2	0.52 (1)	4.36
R- S	-5705 / 0	-122.2	-122.2	0.52 (1)	4.36
S- T	-5705 / 0	-122.2	-122.2	0.52 (1)	4.36
T- U	-5705 / 0	-122.2	-122.2	0.52 (1)	4.36
U- D	-5705 / 0	-122.2	-122.2	0.52 (1)	4.36
D- V	-5681 / 0	-122.2	-122.2	0.36 (1)	4.55
V- E	-5681 / 0	-122.2	-122.2	0.36 (1)	4.55
E- W	-5681 / 0	-122.2	-122.2	0.36 (1)	4.55
W- F	-5681 / 0	-122.2	-122.2	0.36 (1)	4.55
F- X	-5710 / 0	-122.2	-122.2	0.52 (1)	4.36
X- Y	-5710 / 0	-122.2	-122.2	0.52 (1)	4.36
Y- Z	-5710 / 0	-122.2	-122.2	0.52 (1)	4.36
Z-AA	-5710 / 0	-122.2	-122.2	0.52 (1)	4.36
AA- G	-5710 / 0	-122.2	-122.2	0.52 (1)	4.36
G- H	-3256 / 0	-122.2	-122.2	0.06 (1)	6.09
H- I	0 / 56	-122.2	-122.2	0.05 (1)	10.00
Q- B	-3341 / 0	0.0	0.0	0.12 (1)	7.61
J- H	-3363 / 0	0.0	0.0	0.12 (1)	7.59
Q-AB	0 / 0	-28.0	-28.0	0.15 (2)	10.00
AB- P	0 / 0	-28.0	-28.0	0.15 (2)	10.00
P-AC	0 / 2455	-28.0	-28.0	0.27 (1)	10.00
AC-AD	0 / 2455	-28.0	-28.0	0.27 (1)	10.00
AD-AE	0 / 2455	-28.0	-28.0	0.27 (1)	10.00
AE-AF	0 / 2455	-28.0	-28.0	0.27 (1)	10.00
AF- O	0 / 2455	-28.0	-28.0	0.27 (1)	10.00
O- N	0 / 5706	-28.0	-28.0	0.44 (1)	10.00
N- M	0 / 5706	-28.0	-28.0	0.44 (1)	10.00
M-AG	0 / 5710	-28.0	-28.0	0.44 (1)	10.00
AG- L	0 / 5710	-28.0	-28.0	0.44 (1)	10.00
L-AH	0 / 2472	-28.0	-28.0	0.27 (1)	10.00
AH-AI	0 / 2472	-28.0	-28.0	0.27 (1)	10.00
AI-AJ	0 / 2472	-28.0	-28.0	0.27 (1)	10.00
AJ-AK	0 / 2472	-28.0	-28.0	0.27 (1)	10.00
AK- K	0 / 2472	-28.0	-28.0	0.27 (1)	10.00
K-AL	0 / 0	-28.0	-28.0	0.15 (2)	10.00
AL- J	0 / 0	-28.0	-28.0	0.15 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-509 / 195	0.06 (1)
C- O	0 / 3580	0.44 (1)
O- D	-1203 / 51	0.14 (1)
L- F	-1197 / 51	0.14 (1)
L- G	0 / 3565	0.44 (1)
K- G	-515 / 197	0.06 (1)
B- P	0 / 2628	0.33 (1)
K- H	0 / 2647	0.33 (1)
D- M	-129 / 0	0.02 (3)
M- F	-130 / 0	0.02 (3)



DWG NO. TAM 50099-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.52 (C-D:1) , BC=0.44 (L-M:1) , WB=0.44 (C-O:1) , SSI=0.29 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (K) (INPUT = 0.90)
JSI METAL= 0.47 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T1	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 227.6 lbs FACTORED DOWN AT 22-8-8, 148.2 lbs FACTORED DOWN AT 3-11-4, 147.1 lbs FACTORED DOWN AT 5-11-4, 147.1 lbs FACTORED DOWN AT 7-11-4, 147.1 lbs FACTORED DOWN AT 9-11-4, 147.1 lbs FACTORED DOWN AT 11-11-4, 147.1 lbs FACTORED DOWN AT 13-11-4, 147.1 lbs FACTORED DOWN AT 15-11-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-228	-228	---	FRONT	VERT	TOTAL
G	22-8-8	-228	-228	---	FRONT	VERT	TOTAL
N	11-11-4	-40	-70	---	FRONT	VERT	TOTAL
R	3-11-4	-148	-148	---	FRONT	VERT	TOTAL
S	5-11-4	-147	-147	---	FRONT	VERT	TOTAL
T	7-11-4	-147	-147	---	FRONT	VERT	TOTAL
U	9-11-4	-147	-147	---	FRONT	VERT	TOTAL
V	11-11-4	-147	-147	---	FRONT	VERT	TOTAL
W	13-11-4	-147	-147	---	FRONT	VERT	TOTAL
X	15-11-4	-147	-147	---	FRONT	VERT	TOTAL
Y	17-11-4	-147	-147	---	FRONT	VERT	TOTAL
Z	19-11-4	-147	-147	---	FRONT	VERT	TOTAL
AA	21-11-4	-161	-161	---	FRONT	VERT	TOTAL
AB	1-11-4	-40	-70	---	FRONT	VERT	TOTAL
AC	3-11-4	-40	-70	---	FRONT	VERT	TOTAL
AD	5-11-4	-40	-70	---	FRONT	VERT	TOTAL
AE	7-11-4	-40	-70	---	FRONT	VERT	TOTAL
AF	9-11-4	-40	-70	---	FRONT	VERT	TOTAL
AG	13-11-4	-40	-70	---	FRONT	VERT	TOTAL
AH	15-11-4	-40	-70	---	FRONT	VERT	TOTAL
AI	17-11-4	-40	-70	---	FRONT	VERT	TOTAL
AJ	19-11-4	-40	-70	---	FRONT	VERT	TOTAL
AK	21-11-4	-40	-70	---	FRONT	VERT	TOTAL
AL	23-11-4	-40	-70	---	FRONT	VERT	TOTAL

DWG NO. TAM50099-17 for

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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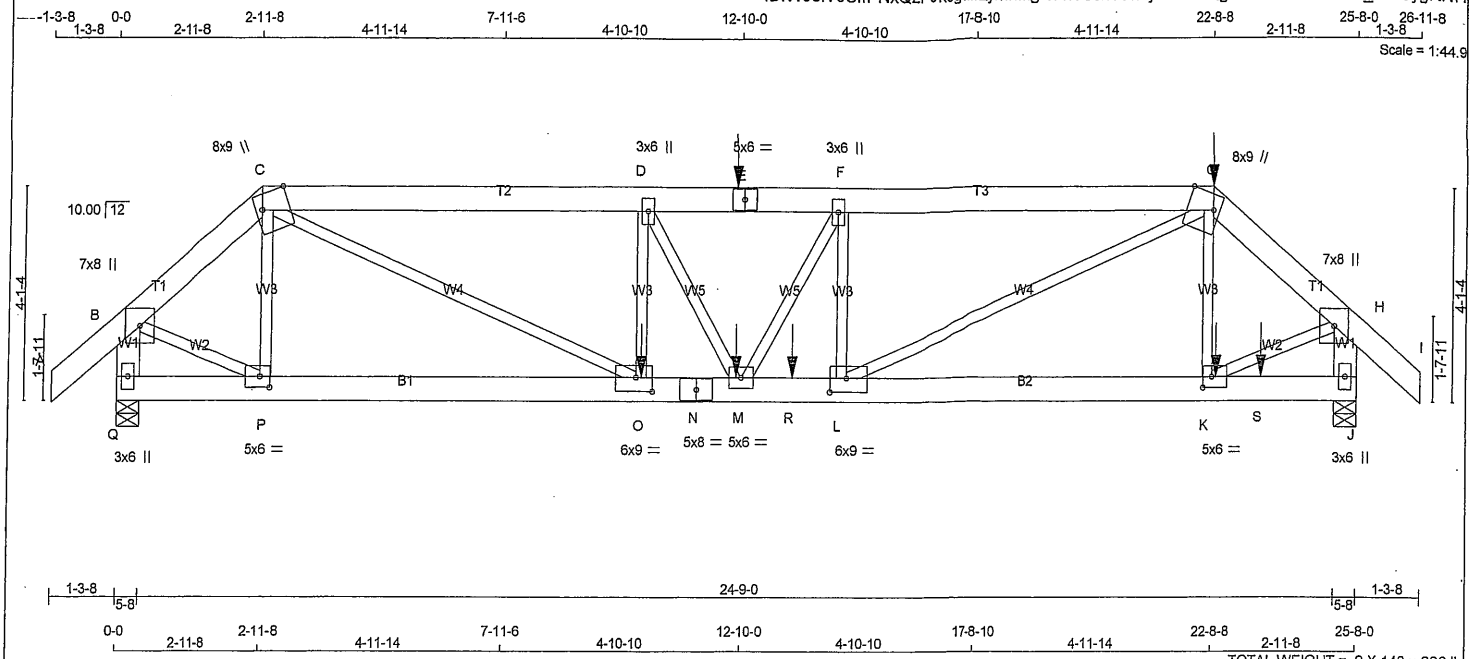

TOTAL WEIGHT = 2 X 140 = 280 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	SIDE(0.0)
E- G	2	12	SIDE(61.0)
G- I	2	12	SIDE(122.0)
Q- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	SIDE(183.1)
N- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
K- G	1	6	SIDE(198.7)
D- O	1	6	SIDE(290.1)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	7.0	8.0		
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW+t	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	3.0	6.0		
G	TTWW+m	MT20	8.0	9.0	Edge	6.50
H	TMVW+p	MT20	7.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	6.0	9.0	3.25	4.00
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	8.0		
O	BMWW-t	MT20	6.0	9.0	3.25	4.00
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	4124	0	4124	0	0	5-8	5-8
J	5280	0	5280	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	3152	2180 / 0	480 / 0	0 / 0	0 / 0	492 / 0	0 / 0
J	4047	2777 / 0	630 / 0	0 / 0	0 / 0	640 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 56	-122.2	-122.2	0.05 (1)	10.00	P- C	-801 / 0		0.10 (1)
B- C	-4165 / 0	-122.2	-122.2	0.07 (1)	5.54	C- O	0 / 6769		0.84 (1)
C- D	-9305 / 0	-122.2	-122.2	0.50 (1)	3.62	O- D	-1129 / 0		0.14 (1)
D- E	-9494 / 0	-122.2	-122.2	0.30 (1)	3.75	L- F	-1505 / 0		0.18 (1)
E- F	-9494 / 0	-122.2	-122.2	0.30 (1)	3.75	L- G	0 / 5533		0.68 (1)
F- G	-9114 / 0	-122.2	-122.2	0.48 (1)	3.67	K- G	0 / 341		0.04 (2)
G- H	-5344 / 0	-122.2	-122.2	0.09 (1)	5.01	B- P	0 / 3385		0.42 (1)
H- I	0 / 56	-122.2	-122.2	0.05 (1)	10.00	K- H	0 / 4343		0.54 (1)
Q- B	-4201 / 0	0.0	0.0	0.15 (1)	6.97	D- M	0 / 385		0.05 (1)
J- H	-5290 / 0	0.0	0.0	0.19 (1)	6.36	M- F	0 / 77		0.01 (1)
Q- P	0 / 0	-28.0	-28.0	0.14 (1)	10.00				
P- O	0 / 3158	-28.0	-28.0	0.34 (1)	10.00				
O- N	0 / 9306	-28.0	-28.0	0.71 (1)	10.00				
N- M	0 / 9306	-28.0	-28.0	0.71 (1)	10.00				
M- R	0 / 9114	-28.0	-28.0	0.88 (1)	10.00				
R- L	0 / 9114	-28.0	-28.0	0.88 (1)	10.00				
L- K	0 / 4089	-28.0	-28.0	0.34 (1)	10.00				
K- S	0 / 0	-28.0	-28.0	0.10 (2)	10.00				
S- J	0 / 0	-28.0	-28.0	0.10 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
E	12-8-12	-147	-147	---	BACK	VERT	TOTAL
G	22-8-8	-230	-230	---	FRONT	VERT	TOTAL
K	22-9-8	-1463	-1463	---	BACK	VERT	TOTAL
M	12-8-12	-40	-70	---	BACK	VERT	TOTAL
O	10-9-8	-1814	-1814	---	BACK	VERT	TOTAL
R	13-10-8	-1463	-1463	---	BACK	VERT	TOTAL
S	23-8-12	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 939 (0.33")

CSI: TC=0.50 (C-D:1) , BC=0.88 (L-M:1) , WB=0.84 (C-O:1) , SSI=0.26 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.96 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9627 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 50100-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 230.2 lbs FACTORED DOWN AT 22-8-8, AND 147.1 lbs FACTORED DOWN AT 12-8-12 ON TOP CHORD, AND 1814.0 lbs FACTORED DOWN AT 10-9-8, 69.9 lbs FACTORED DOWN AT 12-8-12, 1463.3 lbs FACTORED DOWN AT 13-10-8, AND 1463.3 lbs FACTORED DOWN AT 22-9-8, AND 69.9 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9607
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM5010 0-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272205	T1Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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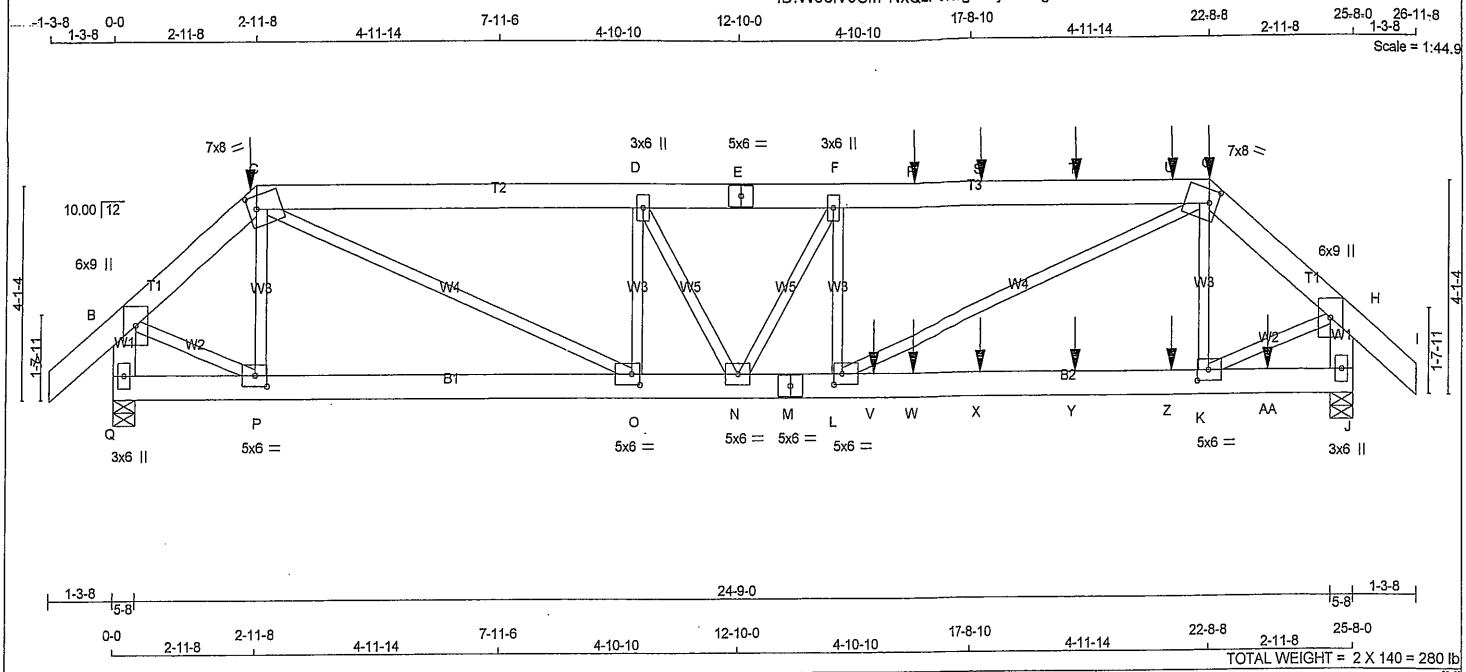

TOTAL WEIGHT = 2 X 140 = 280 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- E	2	12	SIDE(61.0)
E- G	2	12	SIDE(61.0)
G- I	2	12	SIDE(122.0)
Q- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- M	2	12	TOP
M- J	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0		
C	TTWW-m	MT20	7.0	8.0	3.00	2.00
D	TMWW+t	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	3.0	6.0		
G	TTWW-m	MT20	7.0	8.0	3.00	2.00
H	TMVW+p	MT20	6.0	9.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3249	0	3249	0	0	5-8	5-8
J	4217	0	4217	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2508	1689 / 0	412 / 0	0 / 0	0 / 0	407 / 0	0 / 0
J	3276	2171 / 0	561 / 0	0 / 0	0 / 0	544 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 56	-122.2	-122.2	0.05 (1)	10.00	P- C	-615 / 20	0.07 (1)
B- C		-3169 / 0	-122.2	-122.2	0.06 (1)	6.15	C- O	0 / 4290	0.53 (1)
C- D		-6298 / 0	-122.2	-122.2	0.38 (1)	4.39	O- D	-1594 / 0	0.19 (1)
D- E		-6825 / 0	-122.2	-122.2	0.37 (1)	4.21	L- F	0 / 523	0.06 (3)
E- F		-6825 / 0	-122.2	-122.2	0.37 (1)	4.21	L- G	0 / 4811	0.60 (1)
F- R		-7643 / 0	-122.2	-122.2	0.60 (1)	3.76	K- G	-607 / 183	0.07 (1)
R- S		-7643 / 0	-122.2	-122.2	0.60 (1)	3.76	B- P	0 / 2576	0.32 (1)
S- T		-7643 / 0	-122.2	-122.2	0.60 (1)	3.76	K- H	0 / 3502	0.43 (1)
T- U		-7643 / 0	-122.2	-122.2	0.60 (1)	3.76	D- N	0 / 1078	0.13 (1)
U- G		-7643 / 0	-122.2	-122.2	0.60 (1)	3.76	N- F	-1674 / 0	0.24 (1)
G- H		-4309 / 0	-122.2	-122.2	0.07 (1)	5.46			
H- I		0 / 56	-122.2	-122.2	0.05 (1)	10.00			
Q- B		-3282 / 0	0.0	0.0	0.12 (1)	7.66			
J- H		-4335 / 0	0.0	0.0	0.16 (1)	6.88			
Q- P		0 / 0	-28.0	-28.0	0.10 (2)	10.00			
P- O		0 / 2403	-28.0	-28.0	0.24 (1)	10.00			
O- N		0 / 6298	-28.0	-28.0	0.51 (1)	10.00			
N- M		0 / 7643	-28.0	-28.0	0.75 (1)	10.00			
M- L		0 / 7643	-28.0	-28.0	0.75 (1)	10.00			
L- V		0 / 3274	-28.0	-28.0	0.47 (1)	10.00			
V- W		0 / 3274	-28.0	-28.0	0.47 (1)	10.00			
W- X		0 / 3274	-28.0	-28.0	0.47 (1)	10.00			
X- Y		0 / 3274	-28.0	-28.0	0.47 (1)	10.00			
Y- Z		0 / 3274	-28.0	-28.0	0.47 (1)	10.00			
Z- K		0 / 3274	-28.0	-28.0	0.47 (1)	10.00			
K-AA		0 / 0	-28.0	-28.0	0.20 (2)	10.00			
AA- J		0 / 0	-28.0	-28.0	0.20 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-228	-228	---	FRONT	VERT	TOTAL
G	22-8-8	-455	-455	---	FRONT	VERT	TOTAL
R	16-5-4	-147	-147	---	FRONT	VERT	TOTAL
S	17-11-4	-147	-147	---	FRONT	VERT	TOTAL
T	19-11-4	-147	-147	---	FRONT	VERT	TOTAL
U	21-11-4	-161	-161	---	FRONT	VERT	TOTAL
V	15-7-8	-1774	-1774	---	FRONT	VERT	TOTAL
W	16-5-4	-40	-70	---	FRONT	VERT	TOTAL
X	17-11-4	-40	-70	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.60 (F-G:1) , BC=0.75 (L-N:1) , WB=0.60 (G-L:1) , SSI=0.60 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.66 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 7677 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM50113-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272205	T1Z1	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.6 lbs FACTORED DOWN AT 2-11-8, 455.2 lbs FACTORED DOWN AT 22-8-8, 147.1 lbs FACTORED DOWN AT 16-5-4, 147.1 lbs FACTORED DOWN AT 17-11-4, AND 147.1 lbs FACTORED DOWN AT 19-11-4, AND 161.0 lbs FACTORED DOWN AT 21-11-4 ON TOP CHORD, AND 1774.2 lbs FACTORED DOWN AT 15-7-8, 69.9 lbs FACTORED DOWN AT 16-5-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4, AND 69.9 lbs FACTORED DOWN AT 23-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	19-11-4	-40	-70	—	FRONT	VERT	TOTAL
Z	21-11-4	-40	-70	—	FRONT	VERT	TOTAL
AA	23-11-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9607
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50113 -17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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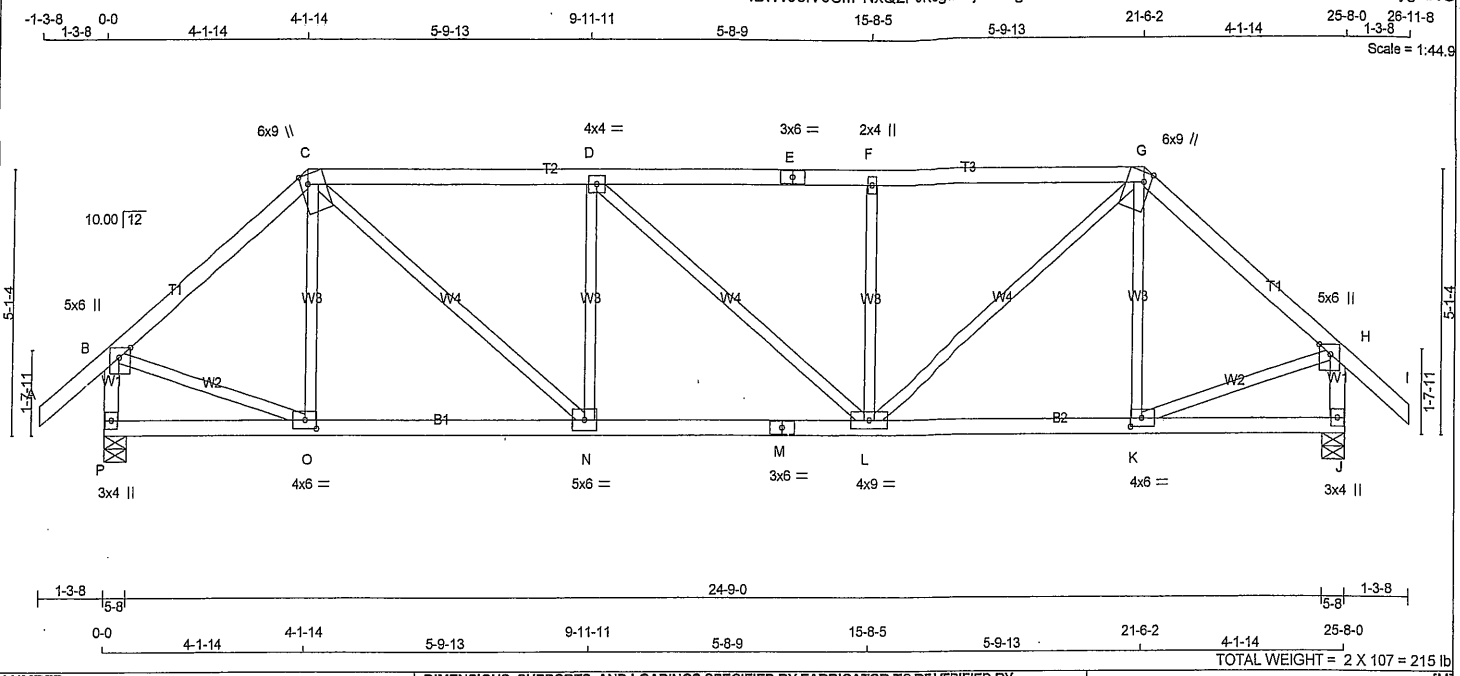

TOTAL WEIGHT = 2 X 107 = 215 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	2097	0	2097	0	0	5-8	5-8
J	2097	0	2097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	270 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	O- C	-279 / 102	0.11 (1)
B- C	-1859 / 0	-122.2	-122.2	0.45 (1)	4.42	C- N	0 / 1450	0.33 (1)
C- D	-2538 / 0	-122.2	-122.2	0.75 (1)	3.48	N- D	-762 / 0	0.29 (1)
D- E	-2536 / 0	-122.2	-122.2	0.74 (1)	3.48	D- L	-2 / 0	0.00 (1)
E- F	-2536 / 0	-122.2	-122.2	0.74 (1)	3.48	L- F	-761 / 0	0.29 (1)
F- G	-2536 / 0	-122.2	-122.2	0.74 (1)	3.49	L- G	0 / 1448	0.33 (1)
G- H	-1859 / 0	-122.2	-122.2	0.45 (1)	4.42	K- G	-278 / 102	0.11 (1)
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00	B- O	0 / 1489	0.34 (1)
P- B	-2054 / 0	0.0	0.0	0.22 (1)	5.90	K- H	0 / 1490	0.34 (1)
J- H	-2054 / 0	0.0	0.0	0.22 (1)	5.90			
P- O	0 / 0	-28.0	-28.0	0.17 (2)	10.00			
O- N	0 / 1418	-28.0	-28.0	0.32 (1)	10.00			
N- M	0 / 2538	-28.0	-28.0	0.50 (1)	10.00			
M- L	0 / 2538	-28.0	-28.0	0.50 (1)	10.00			
L- K	0 / 1418	-28.0	-28.0	0.32 (1)	10.00			
K- J	0 / 0	-28.0	-28.0	0.17 (2)	10.00			

DWG NO. TAM 5010 1-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.75 (C-D:1) , BC=0.50 (L-N:1) , WB=0.34 (H-K:1) , SSI=0.33 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.71 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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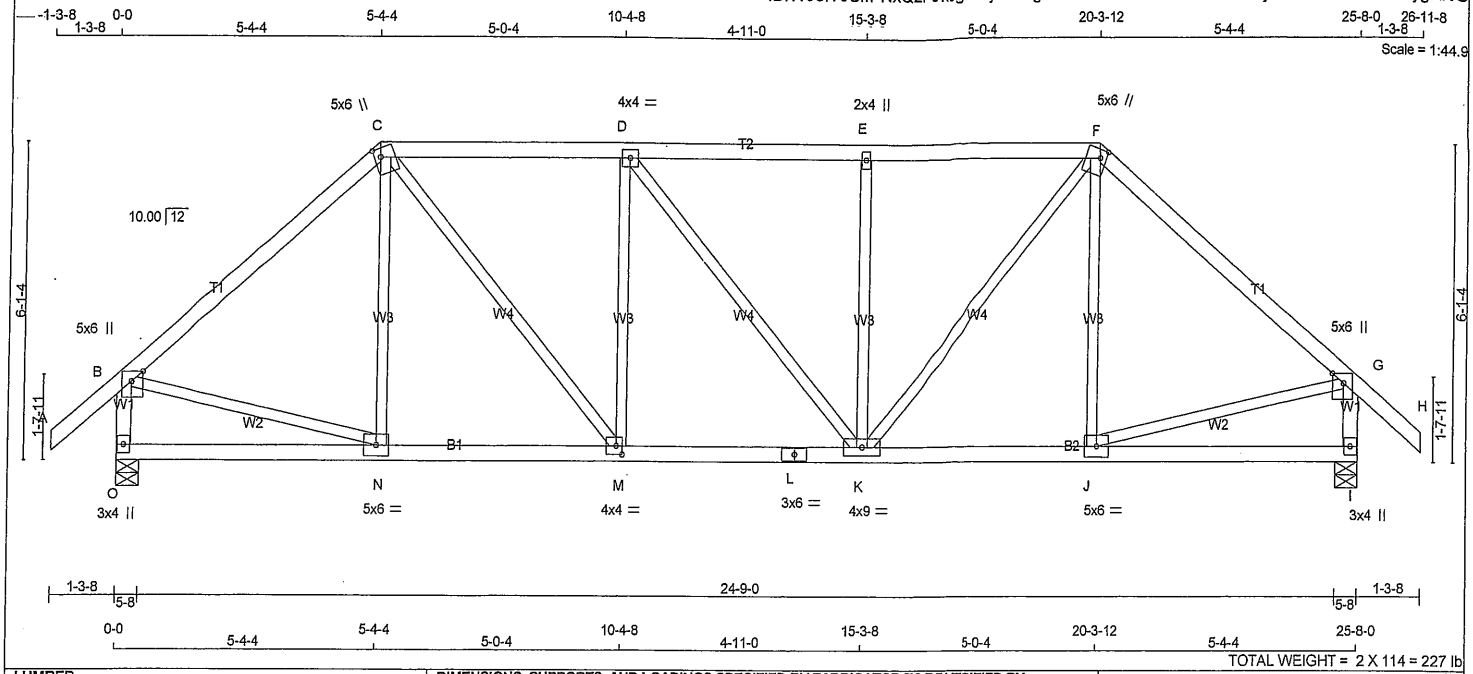

TOTAL WEIGHT = 2 X 114 = 227 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2097	0	2097	0	0	5-8	5-8
I	2097	0	2097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
I	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00		N- C	-175 / 159	0.10 (1)
B- C	-1864 / 0	-122.2	-122.2	0.80 (1)	3.91		C- M	0 / 1041	0.23 (1)
C- D	-2108 / 0	-122.2	-122.2	0.50 (1)	4.11		M- D	-655 / 0	0.38 (1)
D- E	-2106 / 0	-122.2	-122.2	0.50 (1)	4.11		D- K	-3 / 0	0.00 (1)
E- F	-2105 / 0	-122.2	-122.2	0.50 (1)	4.13		K- E	-654 / 0	0.38 (1)
F- G	-1864 / 0	-122.2	-122.2	0.80 (1)	3.91		K- F	0 / 1037	0.23 (1)
G- H	0 / 54	-122.2	-122.2	0.17 (1)	10.00		J- F	-173 / 159	0.10 (1)
O- B	-2035 / 0	0.0	0.0	0.22 (1)	5.92		B- N	0 / 1469	0.33 (1)
I- G	-2035 / 0	0.0	0.0	0.22 (1)	5.92		J- G	0 / 1470	0.33 (1)
O- N	0 / 0	-28.0	-28.0	0.19 (3)	10.00				
N- M	0 / 1426	-28.0	-28.0	0.34 (2)	10.00				
M- L	0 / 2108	-28.0	-28.0	0.40 (1)	10.00				
L- K	0 / 2108	-28.0	-28.0	0.40 (1)	10.00				
K- J	0 / 1426	-28.0	-28.0	0.34 (2)	10.00				
J- I	0 / 0	-28.0	-28.0	0.19 (3)	10.00				

DWG NO. TAM5D102-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.80 (F-G:1) , BC=0.40 (K-M:1) , WB=0.38 (D-M:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.59 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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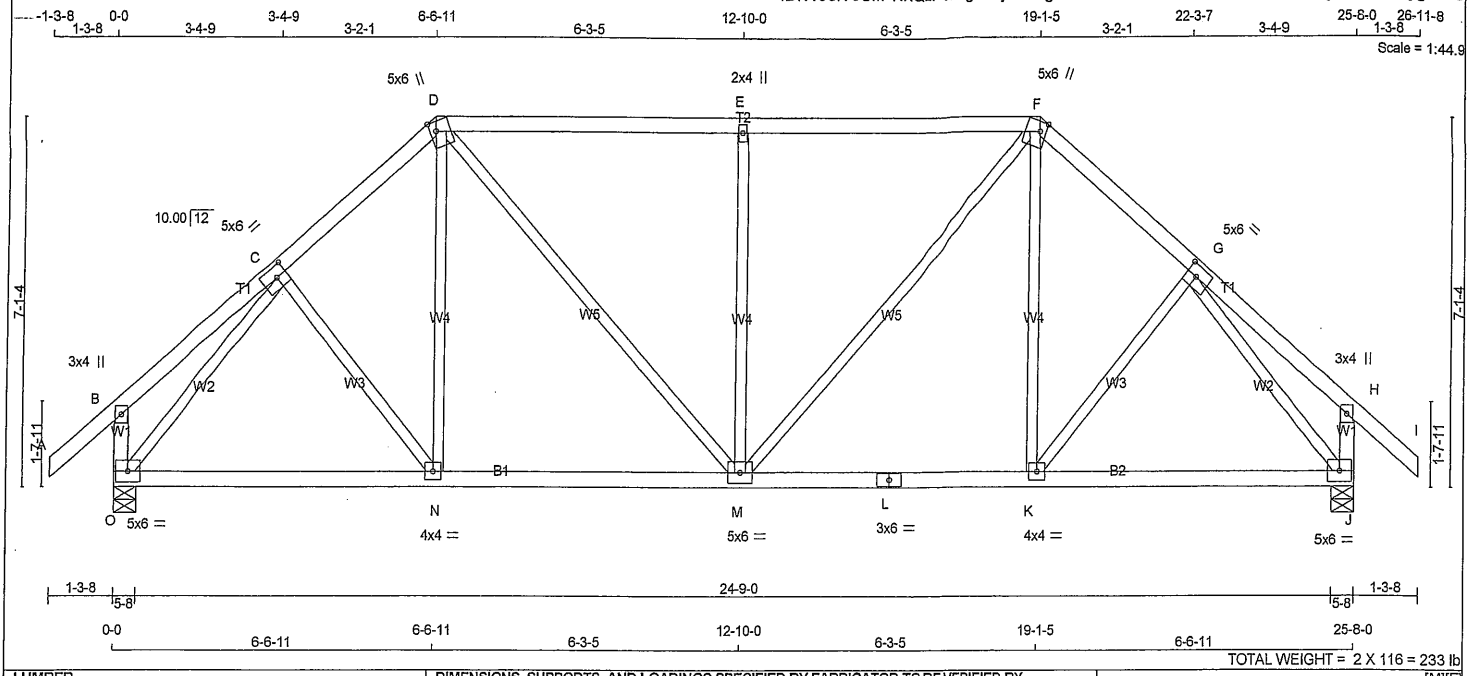

TOTAL WEIGHT = 2 X 116 = 233 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2097	0	2097	0	0	5-8	5-8
J	2097	0	2097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- N	0 / 124	0.03 (3)	
B- C	0 / 27	-122.2	-122.2	0.19 (1)	10.00	N- D	0 / 276	0.06 (2)	
C- D	-1859 / 0	-122.2	-122.2	0.19 (1)	4.77	D- M	0 / 744	0.17 (1)	
D- E	-1910 / 0	-122.2	-122.2	0.68 (1)	3.96	M- E	-940 / 0	0.82 (1)	
E- F	-1910 / 0	-122.2	-122.2	0.68 (1)	3.96	M- F	0 / 744	0.17 (1)	
F- G	-1859 / 0	-122.2	-122.2	0.19 (1)	4.77	K- F	0 / 276	0.06 (2)	
G- H	0 / 27	-122.2	-122.2	0.19 (1)	10.00	K- G	0 / 124	0.03 (3)	
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00	O- C	-2183 / 0	0.94 (1)	
O- B	-325 / 0	0.0	0.0	0.03 (1)	7.81	G- J	-2183 / 0	0.94 (1)	
J- H	-325 / 0	0.0	0.0	0.03 (1)	7.81				
O- N	0 / 1374	-28.0	-28.0	0.43 (2)	10.00				
N- M	0 / 1405	-28.0	-28.0	0.44 (2)	10.00				
M- L	0 / 1405	-28.0	-28.0	0.44 (2)	10.00				
L- K	0 / 1405	-28.0	-28.0	0.44 (2)	10.00				
K- J	0 / 1374	-28.0	-28.0	0.43 (2)	10.00				

DWG NO. TAM 50103-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68 (D-E:1) , BC=0.44 (M-N:2) , WB=0.94 (C-O:1) , SSI=0.37 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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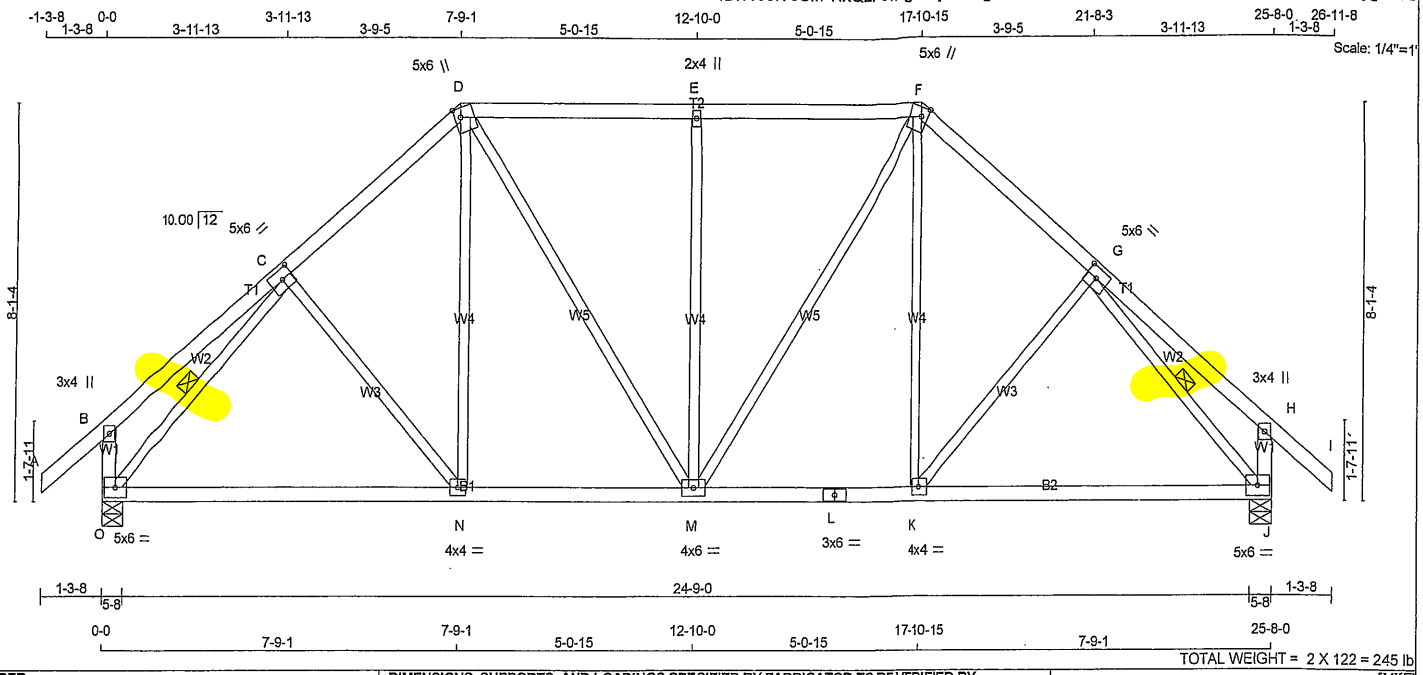

TOTAL WEIGHT = 2 X 122 = 245 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2097	0	2097	0	0	5-8	5-8
J	2097	0	2097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- N	-84 / 97	0.05 (1)
B- C	0 / 34	-122.2	-122.2	0.29 (1)	10.00	N- D	0 / 367	0.08 (2)
C- D	-1805 / 0	-122.2	-122.2	0.26 (1)	4.74	D- M	0 / 496	0.11 (1)
D- E	-1631 / 0	-122.2	-122.2	0.43 (1)	4.69	M- E	-755 / 0	0.96 (1)
E- F	-1631 / 0	-122.2	-122.2	0.43 (1)	4.69	M- F	0 / 496	0.11 (1)
F- G	-1805 / 0	-122.2	-122.2	0.26 (1)	4.74	K- F	0 / 367	0.08 (2)
G- H	0 / 34	-122.2	-122.2	0.29 (1)	10.00	K- G	-84 / 97	0.05 (1)
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00	O- C	-2176 / 0	0.55 (1)
O- B	-351 / 0	0.0	0.0	0.04 (1)	7.81	G- J	-2176 / 0	0.55 (1)
J- H	-351 / 0	0.0	0.0	0.04 (1)	7.81			
O- N	0 / 1413	-28.0	-28.0	0.52 (2)	10.00			
N- M	0 / 1361	-28.0	-28.0	0.52 (2)	10.00			
M- L	0 / 1361	-28.0	-28.0	0.52 (2)	10.00			
L- K	0 / 1361	-28.0	-28.0	0.52 (2)	10.00			
K- J	0 / 1413	-28.0	-28.0	0.52 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
9607
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM5010417
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.43 (D-E:1) , BC=0.52 (M-N:2) , WB=0.96 (E-M:1) , SSI=0.30 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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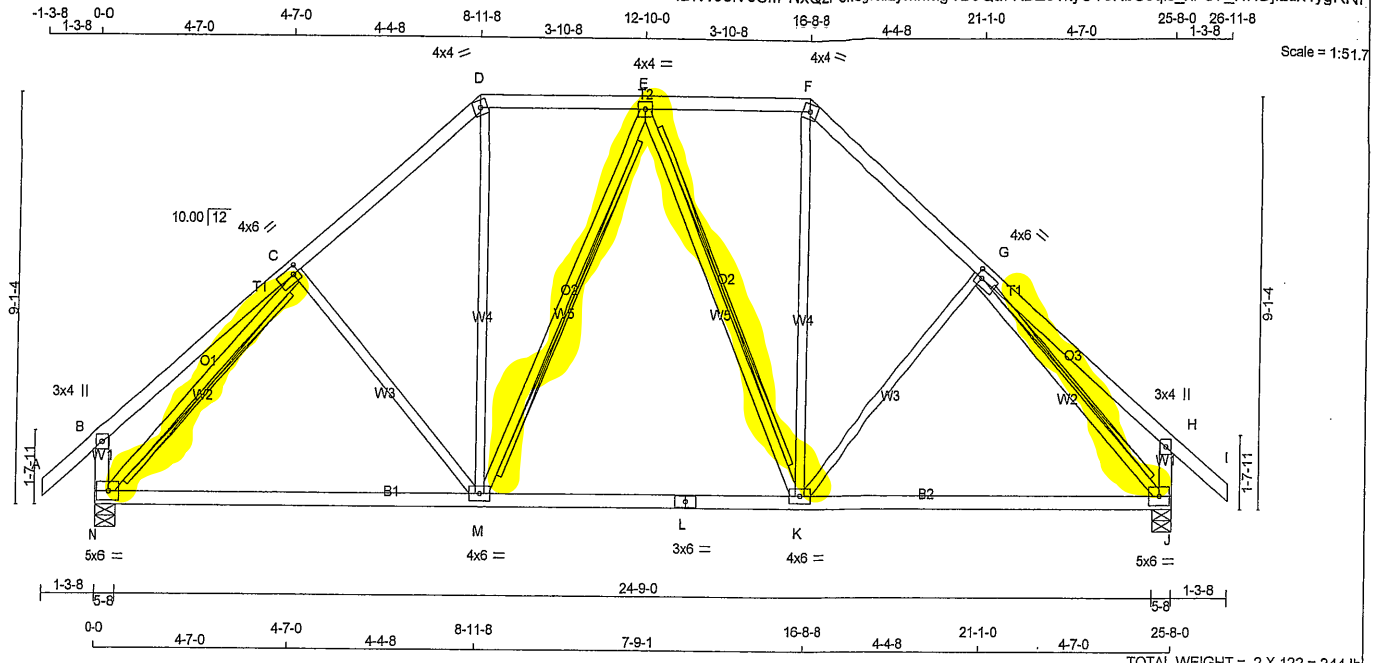

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0	2.00	1.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	2097	0	2097	0	0	5-8	5-8
N	2097	0	2097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
N	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-M, E-K, G-J, C-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	0 / 40	-122.2	-122.2	0.39 (1)		10.00
C- D	-1734 / 0	-122.2	-122.2	0.35 (1)		4.71
D- E	-1305 / 0	-122.2	-122.2	0.24 (1)		5.40
E- F	-1305 / 0	-122.2	-122.2	0.24 (1)		5.40
F- G	-1734 / 0	-122.2	-122.2	0.35 (1)		4.71
G- H	0 / 40	-122.2	-122.2	0.39 (1)		10.00
H- I	0 / 54	-122.2	-122.2	0.17 (1)		10.00
N- B	-385 / 0	0.0	0.0	0.04 (1)		7.81
J- H	-385 / 0	0.0	0.0	0.04 (1)		7.81
N- M	0 / 1434	-28.0	-28.0	0.61 (2)		10.00
M- L	0 / 1427	-28.0	-28.0	0.62 (2)		10.00
L- K	0 / 1427	-28.0	-28.0	0.62 (2)		10.00
K- J	0 / 1434	-28.0	-28.0	0.61 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-207 / 72	0.18 (1)
M- D	0 / 692	0.16 (1)
M- E	-308 / 0	0.23 (1)
E- K	-308 / 0	0.23 (1)
K- F	0 / 692	0.16 (1)
K- G	-207 / 72	0.18 (1)
G- J	-2153 / 0	0.74 (1)
N- C	-2153 / 0	0.74 (1)

LICENSED PROFESSIONAL ENGINEER 9677 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 50105-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 670 (0.46")

CSI: TC=0.39 (B-C:1) , BC=0.62 (K-M:2) , WB=0.74 (C-N:1) , SSI=0.23 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.53 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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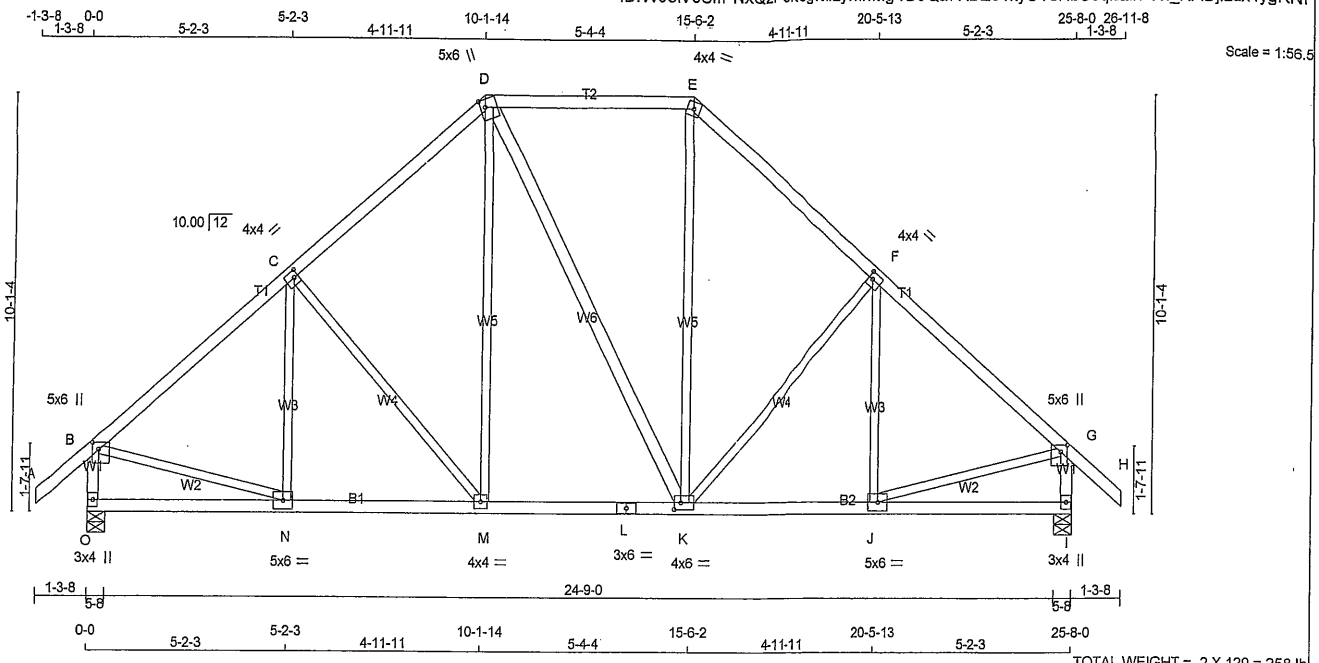


TOTAL WEIGHT = 2 X 129 = 258 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.00	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2097	0	2097	0	0	5-8	5-8
I	2097	0	2097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
I	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	N- C	-219 / 142	0.12 (1)
B- C	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38	C- M	-428 / 0	0.49 (1)
C- D	-1627 / 0	-122.2	-122.2	0.45 (1)	4.68	M- D	0 / 457	0.10 (1)
D- E	-1215 / 0	-122.2	-122.2	0.47 (1)	5.19	D- K	0 / 2	0.00 (1)
E- F	-1628 / 0	-122.2	-122.2	0.45 (1)	4.68	K- E	0 / 460	0.10 (1)
F- G	-1901 / 0	-122.2	-122.2	0.48 (1)	4.38	K- F	-426 / 0	0.49 (1)
G- H	0 / 54	-122.2	-122.2	0.17 (1)	10.00	J- F	-221 / 140	0.12 (1)
O- B	-2037 / 0	0.0	0.0	0.22 (1)	5.92	B- N	0 / 1543	0.35 (1)
I- G	-2036 / 0	0.0	0.0	0.22 (1)	5.92	J- G	0 / 1543	0.35 (1)
O- N	0 / 0	-28.0	-28.0	0.18 (3)	10.00			
N- M	0 / 1497	-28.0	-28.0	0.33 (1)	10.00			
M- L	0 / 1215	-28.0	-28.0	0.28 (1)	10.00			
L- K	0 / 1215	-28.0	-28.0	0.28 (1)	10.00			
K- J	0 / 1496	-28.0	-28.0	0.32 (1)	10.00			
J- I	0 / 0	-28.0	-28.0	0.17 (3)	10.00			

DWG NO. TAM50106-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.48 (B-C:1) , BC=0.33 (M-N:1) , WB=0.49 (C-M:1) , SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

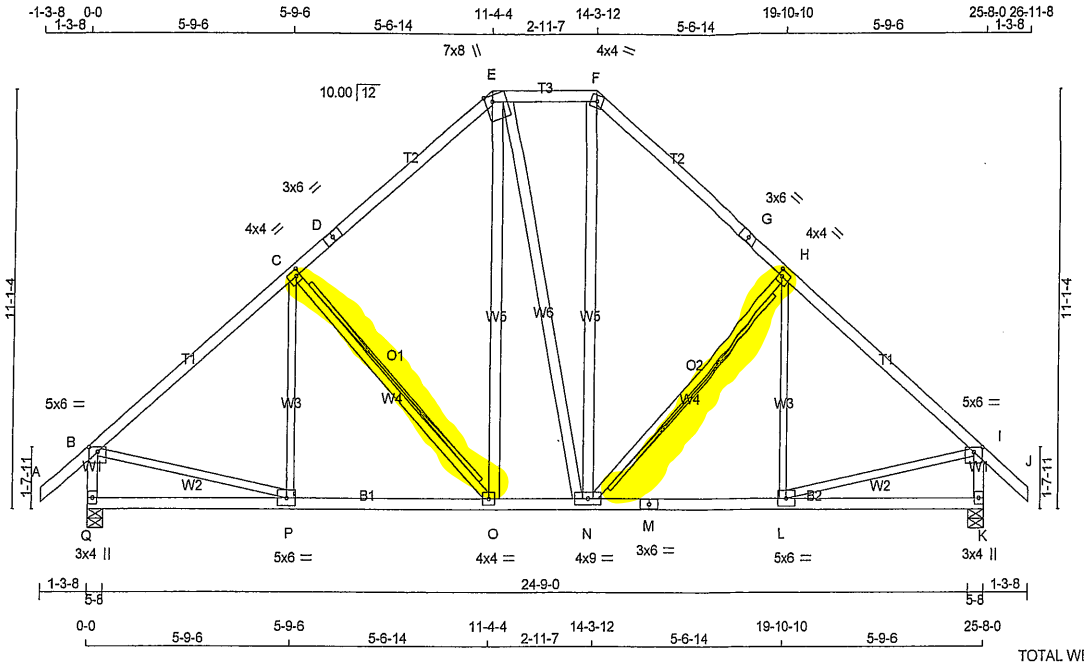
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T8	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:W9oiV9SffPNxQzF9k5gMfzymhMg-JQjonTLsKP9baY7q?JzNMvP6Dfrvjr3MyyjRUWygKNE



TOTAL WEIGHT = 2 X 141 = 283 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2097	0	2097	0	0	5-8	5-8
K	2097	0	2097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0
K	1622	1087 / 0	269 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-O, H-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	P- C	-153 / 204	0.10 (1)
B- C	-1904 / 0	-122.2	-122.2	0.61 (1)	4.17	C- O	-550 / 0	0.64 (1)
C- D	-1527 / 0	-122.2	-122.2	0.56 (1)	4.62	O- E	0 / 505	0.08 (1)
D- E	-1527 / 0	-122.2	-122.2	0.56 (1)	4.62	E- N	0 / 5	0.00 (1)
E- F	-1136 / 0	-122.2	-122.2	0.15 (1)	5.80	N- F	0 / 511	0.08 (1)
F- G	-1528 / 0	-122.2	-122.2	0.56 (1)	4.61	N- H	-548 / 0	0.64 (1)
G- H	-1528 / 0	-122.2	-122.2	0.56 (1)	4.61	L- H	-157 / 201	0.11 (1)
H- I	-1904 / 0	-122.2	-122.2	0.61 (1)	4.17	B- P	0 / 1542	0.35 (1)
I- J	0 / 54	-122.2	-122.2	0.17 (1)	10.00	L- I	0 / 1541	0.35 (1)
Q- B	-2032 / 0	0.0	0.0	0.22 (1)	5.93			
K- I	-2031 / 0	0.0	0.0	0.22 (1)	5.93			
Q- P	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
P- O	0 / 1504	-28.0	-28.0	0.39 (2)	10.00			
O- N	0 / 1135	-28.0	-28.0	0.25 (1)	10.00			
N- M	0 / 1503	-28.0	-28.0	0.39 (2)	10.00			
M- L	0 / 1503	-28.0	-28.0	0.39 (2)	10.00			
L- K	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.61 (B-C:1) , BC=0.39 (O-P:2) , WB=0.64 (C-O:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.45 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9627
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50107 -17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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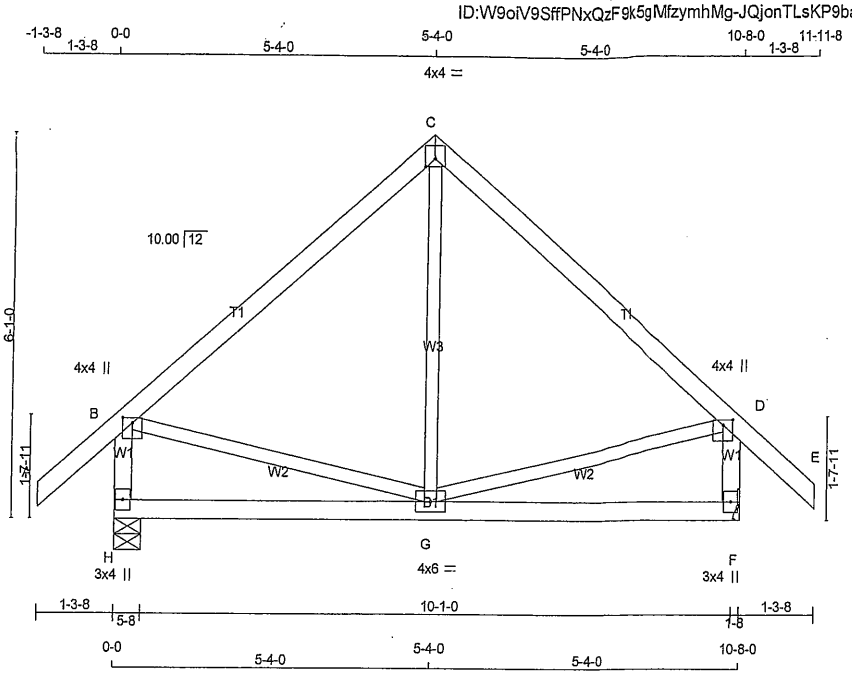

TOTAL WEIGHT = 48 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	970	0	970	0	0	IN-SX 5-8	IN-SX 5-8
F	970	0	970	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	741	514 / 0	112 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	741	514 / 0	112 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	G- C	-5 / 242	0.06 (3)
B- C	-505 / 0	-122.2	-122.2	0.45 (1)	6.25	B- G	0 / 399	0.09 (1)
C- D	-505 / 0	-122.2	-122.2	0.45 (1)	6.25	G- D	0 / 399	0.09 (1)
D- E	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
H- B	-913 / 0	0.0	0.0	0.10 (1)	7.81			
F- D	-913 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.23 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.45 (C-D:1) , BC=0.23 (G-H:3) , WB=0.09 (B-G:1) , SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)

DWG NO. TAM50108-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T9S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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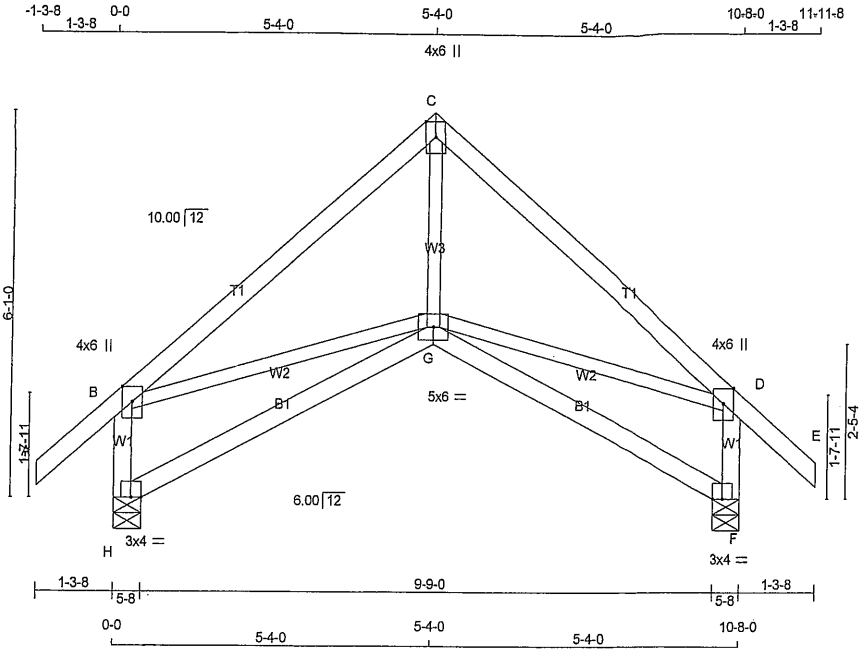

TOTAL WEIGHT = 3 X 48 = 143 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BVM1-p	MT20	3.0	4.0		
G	BBWWW-p	MT20	5.0	6.0		
H	BVM1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	970	0	970	0	0	5-8	5-8
F	970	0	970	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	741	514 / 0	112 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	741	514 / 0	112 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO					
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	G- C	0 / 438	0.10 (2)
B- C	-844 / 0	-122.2	-122.2	0.46 (1)	5.98	B- G	0 / 663	0.15 (1)
C- D	-844 / 0	-122.2	-122.2	0.46 (1)	5.98	G- D	0 / 663	0.15 (1)
D- E	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
H- B	-896 / 0	0.0	0.0	0.10 (1)	7.81			
F- D	-896 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.46 (C-D:1) , BC=0.24 (F-G:3) , WB=0.15 (D-G:1) , SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.39 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM5D109-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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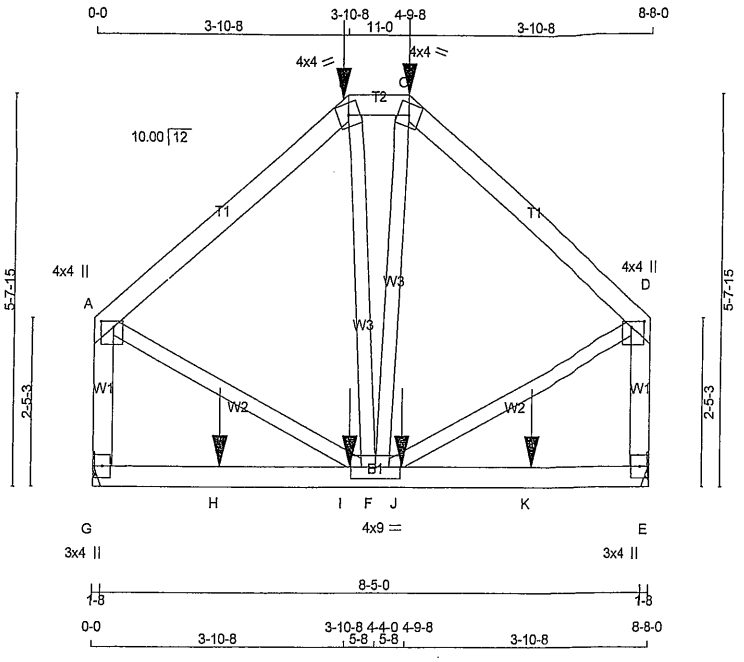

TOTAL WEIGHT = 43 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.25
B	TTW-m	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.00	2.25
E	BMV1+p	MT20	3.0	4.0		
F	BMWWWW*-I	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 402.1 lbs FACTORED DOWN AT 3-10-8, AND 402.1 lbs FACTORED DOWN AT 4-9-8 ON TOP CHORD, AND 46.7 lbs FACTORED DOWN AT 1-11-4, 45.9 lbs FACTORED DOWN AT 3-11-4, AND 45.9 lbs FACTORED DOWN AT 4-8-12, AND 46.7 lbs FACTORED DOWN AT 6-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1108	0	1108	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	1108	0	1108	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	848	582 / 0	131 / 0	0 / 0	0 / 0	135 / 0	0 / 0
E	848	582 / 0	131 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)		CSI (LC)	WEBS MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A- B	-801 / 0	-122.2	-122.2	0.37 (1)	6.20	A- F	0 / 682	0.17 (1)
B- C	-598 / 0	-122.2	-122.2	0.02 (1)	6.25	F- D	0 / 682	0.17 (1)
C- D	-801 / 0	-122.2	-122.2	0.37 (1)	6.20	B- F	-180 / 114	0.09 (1)
G- A	-1049 / 0	0.0	0.0	0.15 (1)	7.64	F- C	-180 / 114	0.09 (1)
E- D	-1049 / 0	0.0	0.0	0.15 (1)	7.64			
G- H	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
H- I	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
I- F	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
F- J	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
J- K	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
K- E	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-10-8	-402	-402	---	FRONT	VERT	TOTAL
C	4-9-8	-402	-402	---	FRONT	VERT	TOTAL
H	1-11-4	-29	-47	---	FRONT	VERT	TOTAL
I	3-11-4	-26	-46	---	FRONT	VERT	TOTAL
J	4-8-12	-26	-46	---	FRONT	VERT	TOTAL
K	6-8-12	-29	-47	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.37 (A-B:1) , BC=0.24 (F-G:3) , WB=0.17 (D-F:1) , SSI=0.18 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (A) (INPUT = 0.90)
JSI METAL= 0.23 (D) (INPUT = 1.00)

DWG NO. TAM 50110-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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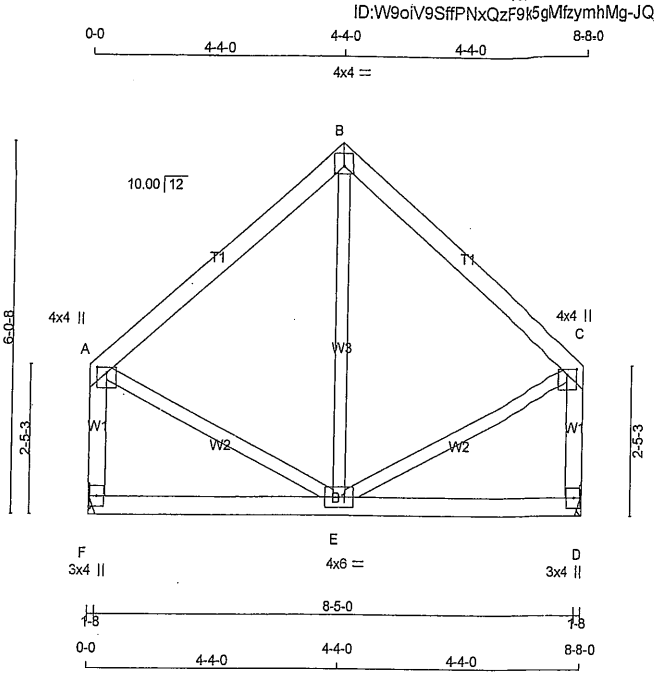

TOTAL WEIGHT = 39 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG REQRD IN-SX	REQRD BRG REQRD IN-SX
F	651	0	651	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
D	651	0	651	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	509	331 / 0	91 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	509	331 / 0	91 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-336 / 0	-122.2	-122.2		0.29 (1)	6.25	E- B	-100 / 155	0.06 (1)
B- C	-336 / 0	-122.2	-122.2		0.29 (1)	6.25	A- E	0 / 286	0.06 (1)
F- A	-604 / 0	0.0	0.0		0.08 (1)	7.81	E- C	0 / 286	0.06 (1)
D- C	-604 / 0	0.0	0.0		0.08 (1)	7.81			
F- E	0 / 0	-28.0	-28.0		0.15 (3)	10.00			
E- D	0 / 0	-28.0	-28.0		0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.29 (B-C:1) , BC=0.15 (E-F:3) , WB=0.06 (A-E:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

DWG NO. TAM 5011-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272179	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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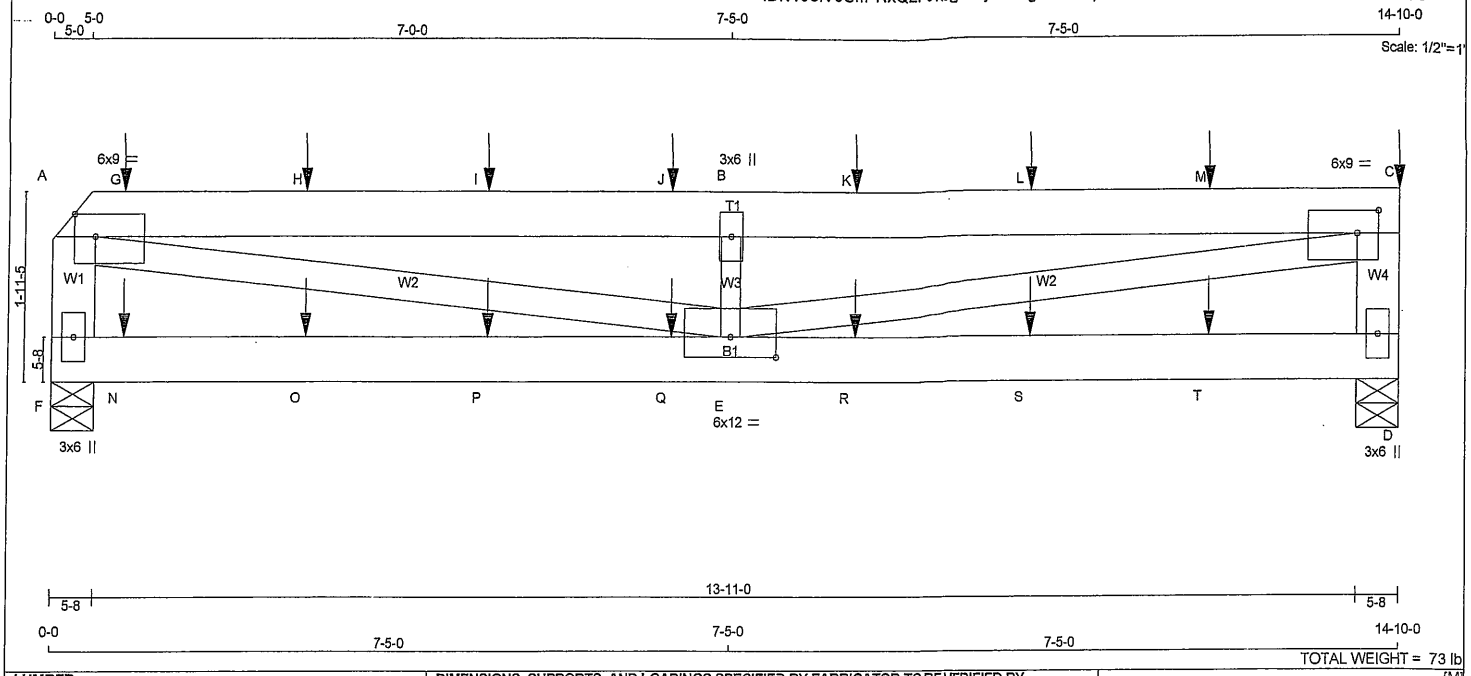

TOTAL WEIGHT = 73 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
E - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	2.75	2.75
B	TMW+w	MT20	3.0	6.0		
C	TMVW-t	MT20	6.0	9.0	2.75	2.75
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	6.0	12.0	2.50	6.00
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 120.3 lbs FACTORED DOWN AT 9-4, 106.3 lbs FACTORED DOWN AT 2-9-4, 106.3 lbs FACTORED DOWN AT 4-9-4, 106.3 lbs FACTORED DOWN AT 6-9-4, 106.3 lbs FACTORED DOWN AT 8-9-4, 106.3 lbs FACTORED DOWN AT 10-9-4, AND 106.3 lbs FACTORED DOWN AT 12-9-4, AND 163.6 lbs FACTORED DOWN AT 14-10-0 ON TOP CHORD, AND 69.0 lbs FACTORED DOWN AT 9-4, 65.8 lbs FACTORED DOWN AT 2-9-4, 65.8 lbs FACTORED DOWN AT 4-9-4, 65.8 lbs FACTORED DOWN AT 6-9-4, 65.8 lbs FACTORED DOWN AT 8-9-4, AND 65.8 lbs FACTORED DOWN AT 10-9-4, AND 65.8 lbs FACTORED DOWN AT 12-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM 50112-17

STRUCTURAL

COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1829	0	1829	0	0	5-8	5-8
F	1785	0	1785	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1418	944 / 0	240 / 0	0 / 0	0 / 0	234 / 0	0 / 0
F	1396	909 / 0	250 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- G	-4849 / 0	-122.2	-122.2	0.91 (1)	2.78	E- C	0 / 4945	0.87 (1)
G- H	-4849 / 0	-122.2	-122.2	0.91 (1)	2.78	A- E	0 / 4945	0.87 (1)
H- I	-4849 / 0	-122.2	-122.2	0.91 (1)	2.78	E- B	-1501 / 0	0.23 (1)
I- J	-4849 / 0	-122.2	-122.2	0.91 (1)	2.78			
J- B	-4849 / 0	-122.2	-122.2	0.91 (1)	2.78			
B- K	-4849 / 0	-122.2	-122.2	0.88 (1)	2.87			
K- L	-4849 / 0	-122.2	-122.2	0.88 (1)	2.87			
L- M	-4849 / 0	-122.2	-122.2	0.88 (1)	2.87			
M- C	-4849 / 0	-122.2	-122.2	0.88 (1)	2.87			
D- C	-1630 / 0	0.0	0.0	0.12 (1)	7.68			
F- A	-1544 / 0	0.0	0.0	0.11 (1)	7.81			
F- N	0 / 0	-28.0	-28.0	0.32 (1)	10.00			
N- O	0 / 0	-28.0	-28.0	0.32 (1)	10.00			
O- P	0 / 0	-28.0	-28.0	0.32 (1)	10.00			
P- Q	0 / 0	-28.0	-28.0	0.32 (1)	10.00			
Q- E	0 / 0	-28.0	-28.0	0.32 (1)	10.00			
E- R	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
R- S	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
S- T	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
T- D	0 / 0	-28.0	-28.0	0.33 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	14-10-0	-164	-164	---	BACK	VERT	TOTAL
G	9-4	-120	-120	---	BACK	VERT	TOTAL
H	2-9-4	-106	-106	---	BACK	VERT	TOTAL
I	4-9-4	-106	-106	---	BACK	VERT	TOTAL
J	6-9-4	-106	-106	---	BACK	VERT	TOTAL
K	8-9-4	-106	-106	---	BACK	VERT	TOTAL
L	10-9-4	-106	-106	---	BACK	VERT	TOTAL
M	12-9-4	-106	-106	---	BACK	VERT	TOTAL
N	9-4	-69	-69	---	BACK	VERT	TOTAL
O	2-9-4	-66	-66	---	BACK	VERT	TOTAL
P	4-9-4	-66	-66	---	BACK	VERT	TOTAL
Q	6-9-4	-66	-66	---	BACK	VERT	TOTAL
R	8-9-4	-66	-66	---	BACK	VERT	TOTAL
S	10-9-4	-66	-66	---	BACK	VERT	TOTAL
T	12-9-4	-66	-66	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 604 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 397 (0.45")

CSI: TC=0.91 (A-B:1) , BC=0.33 (D-E:1) , WB=0.87 (C-E:1) , SSI=0.51 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.81 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272205	T13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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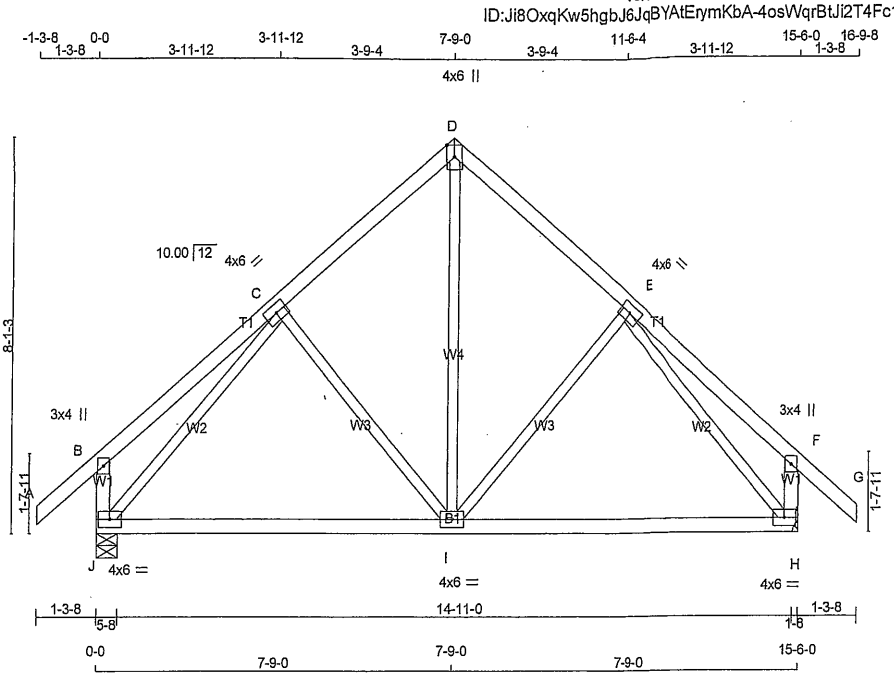

TOTAL WEIGHT = 2 X 72 = 144 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1333	0	1333	0	0	5-8	5-8
H	1333	0	1333	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1025	699 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	1025	699 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 643	0.14 (1)
B- C	0 / 35	-122.2	-122.2	0.30 (1)	10.00	I- E	-248 / 37	0.15 (1)
C- D	-825 / 0	-122.2	-122.2	0.23 (1)	6.25	C- I	-248 / 37	0.15 (1)
D- E	-825 / 0	-122.2	-122.2	0.23 (1)	6.25	J- C	-1185 / 0	0.70 (1)
E- F	0 / 35	-122.2	-122.2	0.30 (1)	10.00	E- H	-1185 / 0	0.70 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
J- B	-349 / 0	0.0	0.0	0.04 (1)	7.81			
H- F	-349 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 769	-28.0	-28.0	0.56 (2)	10.00			
I- H	0 / 769	-28.0	-28.0	0.56 (2)	10.00			

DWG NO. TAM50114-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.30 (B-C:1) , BC=0.56 (I-J:2) , WB=0.70 (C-J:1) , SSI=0.18 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272205	G13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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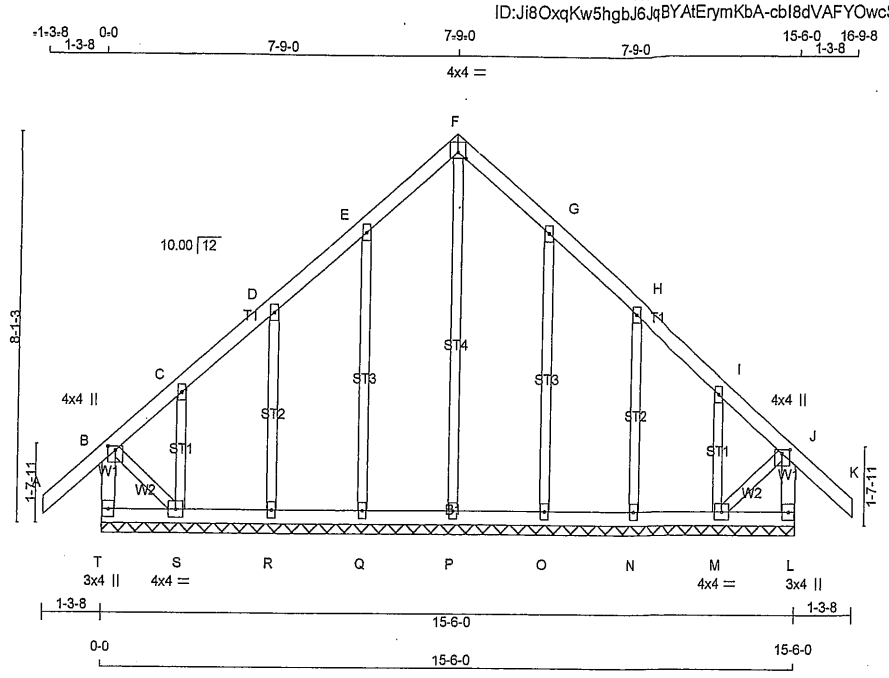

TOTAL WEIGHT = 77 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-382 / 0	0.0	0.0	0.04 (1)	7.81	P- F	-178 / 0		0.22 (1)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	Q- E	-275 / 0		0.18 (1)
B- C	-81 / 0	-122.2	-122.2	0.16 (1)	6.25	R- D	-250 / 0		0.08 (1)
C- D	-26 / 0	-122.2	-122.2	0.06 (1)	6.25	S- C	-144 / 0		0.03 (1)
D- E	-26 / 0	-122.2	-122.2	0.07 (1)	6.25	O- G	-275 / 0		0.18 (1)
E- F	-38 / 0	-122.2	-122.2	0.07 (1)	6.25	N- H	-250 / 0		0.08 (1)
F- G	-38 / 0	-122.2	-122.2	0.07 (1)	6.25	M- I	-144 / 0		0.03 (1)
G- H	-26 / 0	-122.2	-122.2	0.07 (1)	6.25	B- S	0 / 40		0.01 (1)
H- I	-26 / 0	-122.2	-122.2	0.06 (1)	6.25	M- J	0 / 40		0.01 (1)
I- J	-81 / 0	-122.2	-122.2	0.16 (1)	6.25				
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00				
L- J	-382 / 0	0.0	0.0	0.04 (1)	7.81				
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00				
S- R	0 / 27	-28.0	-28.0	0.03 (2)	10.00				
R- Q	0 / 22	-28.0	-28.0	0.02 (2)	10.00				
Q- P	0 / 17	-28.0	-28.0	0.02 (2)	10.00				
P- O	0 / 17	-28.0	-28.0	0.02 (2)	10.00				
O- N	0 / 22	-28.0	-28.0	0.02 (2)	10.00				
N- M	0 / 27	-28.0	-28.0	0.03 (2)	10.00				
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00				

DWG NO. TAM 5015-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.03 (R-S:2) , WB=0.22 (F-P:1) , SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (F) (INPUT = 0.90)
JSI METAL= 0.07 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272180	T200	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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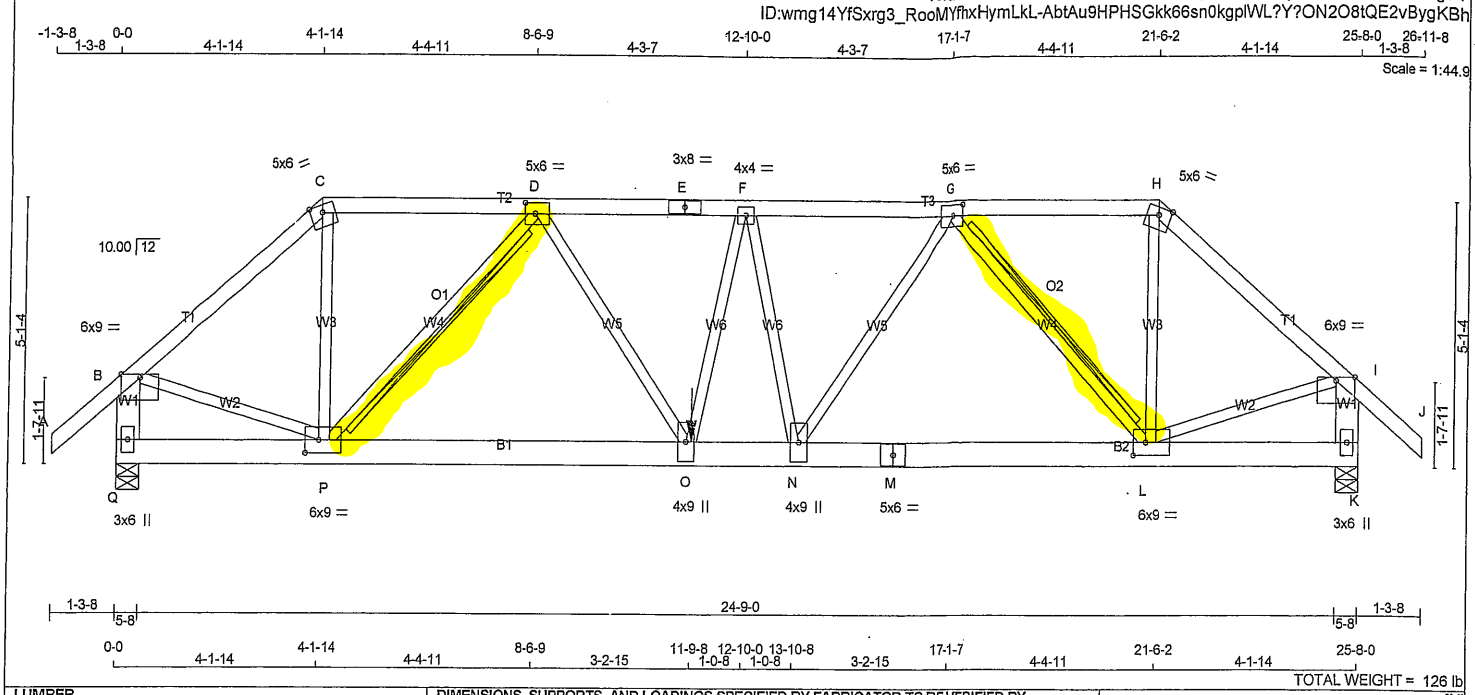


TOTAL WEIGHT = 126 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

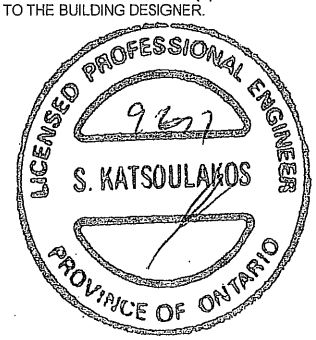
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTW-m	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWWW-t	MT20	6.0	9.0	3.00	3.00
M	BS-t	MT20	5.0	6.0		
N	BMWW+t	MT20	4.0	9.0		
O	BMWW+t	MT20	4.0	9.0		
P	BMWWW-t	MT20	6.0	9.0	3.00	3.50
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1749.4 lbs FACTORED DOWN AT 11-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 5016-17
STRUCTURAL
COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	3046	0	3046	0	0	5-8	5-8
K	2897	0	2897	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2344	1592 / 0	377 / 0	0 / 0	0 / 0	376 / 0	0 / 0
K	2231	1513 / 0	360 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-P, G-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.19 (1)	10.00		P- C	0 / 1455	0.36 (1)
B- C	-3013 / 0	-122.2	-122.2	0.63 (1)	3.36		D- O	0 / 1567	0.39 (1)
C- D	-2363 / 0	-122.2	-122.2	0.46 (1)	3.93		N- G	0 / 1411	0.35 (1)
D- E	-4928 / 0	-122.2	-122.2	0.79 (1)	2.41		L- H	0 / 1340	0.33 (1)
E- F	-4928 / 0	-122.2	-122.2	0.79 (1)	2.41		B- P	0 / 2399	0.59 (1)
F- G	-4585 / 0	-122.2	-122.2	0.75 (1)	2.56		L- I	0 / 2254	0.56 (1)
G- H	-2219 / 0	-122.2	-122.2	0.46 (1)	4.03		F- N	-1008 / 0	0.40 (1)
H- I	-2831 / 0	-122.2	-122.2	0.61 (1)	3.49		O- F	0 / 521	0.13 (1)
I- J	0 / 54	-122.2	-122.2	0.19 (1)	10.00		P- D	-2536 / 0	0.81 (1)
Q- B	-3011 / 0	0.0	0.0	0.22 (1)	6.03		G- L	-2368 / 0	0.76 (1)
K- I	-2854 / 0	0.0	0.0	0.21 (1)	6.16				
Q- P	0 / 0	-28.0	-28.0	0.14 (3)	10.00				
P- O	0 / 4051	-28.0	-28.0	0.65 (1)	10.00				
O- N	0 / 4811	-28.0	-28.0	0.67 (1)	10.00				
N- M	0 / 3795	-28.0	-28.0	0.58 (1)	10.00				
M- L	0 / 3795	-28.0	-28.0	0.58 (1)	10.00				
L- K	0 / 0	-28.0	-28.0	0.14 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	11-9-8	-1749	-1749	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012', BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.79 (D-F:1) , BC=0.67 (N-O:1) , WB=0.81 (D-P:1) , SSI=0.28 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.73 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272219	T201	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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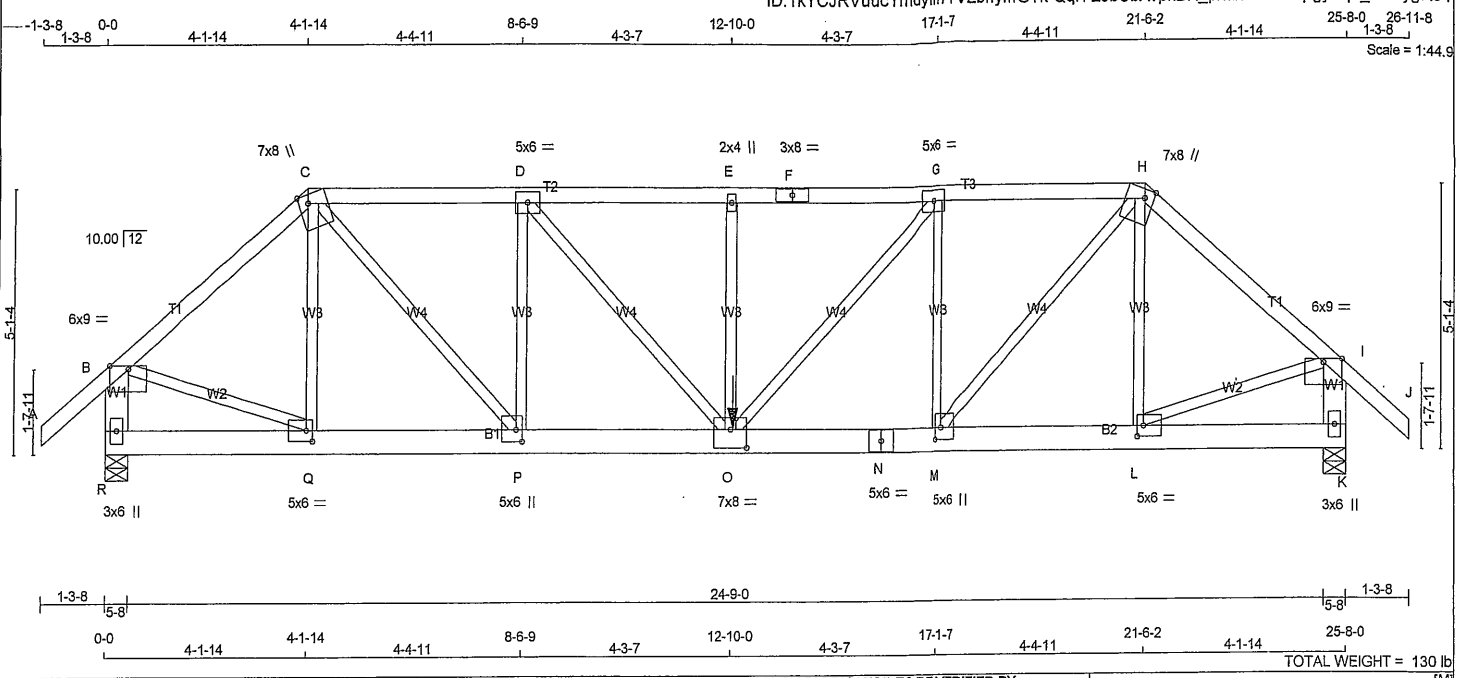

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	1.50
M	BMWW+t	MT20	5.0	6.0	2.75	1.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	7.0	8.0	4.25	4.00
P	BMWW+t	MT20	5.0	6.0	2.75	1.50
Q	BMWW-t	MT20	5.0	6.0	2.50	1.50
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1785.3 lbs FACTORED DOWN AT 12-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2990	0	2990	0	0	5-8	5-8
K	2990	0	2990	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2302	1561 / 0	371 / 0	0 / 0	0 / 0	370 / 0	0 / 0
K	2302	1561 / 0	371 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A- B	0 / 54	-122.2 -122.2	0.19 (1)	10.00	Q- C	-504 / 4	0.19 (1)
B- C	-2921 / 0	-122.2 -122.2	0.62 (1)	3.42	C- P	0 / 2586	0.64 (1)
C- D	-3991 / 0	-122.2 -122.2	0.66 (1)	2.87	P- D	-1732 / 0	0.64 (1)
D- E	-5026 / 0	-122.2 -122.2	0.81 (1)	2.35	D- O	0 / 1541	0.38 (1)
E- F	-5026 / 0	-122.2 -122.2	0.81 (1)	2.35	O- E	-470 / 0	0.17 (1)
F- G	-5026 / 0	-122.2 -122.2	0.81 (1)	2.35	O- G	0 / 1541	0.38 (1)
G- H	-3991 / 0	-122.2 -122.2	0.66 (1)	2.87	M- G	-1732 / 0	0.64 (1)
H- I	-2921 / 0	-122.2 -122.2	0.62 (1)	3.42	M- H	0 / 2586	0.64 (1)
I- J	0 / 54	-122.2 -122.2	0.19 (1)	10.00	L- H	-504 / 4	0.19 (1)
R- B	-2942 / 0	0.0 0.0	0.22 (1)	6.09	B- Q	0 / 2329	0.58 (1)
K- I	-2942 / 0	0.0 0.0	0.22 (1)	6.09	L- I	0 / 2329	0.58 (1)
R- Q	0 / 0	-28.0 -28.0	0.06 (2)	10.00			
Q- P	0 / 2227	-28.0 -28.0	0.33 (1)	10.00			
P- O	0 / 3992	-28.0 -28.0	0.65 (1)	10.00			
O- N	0 / 3992	-28.0 -28.0	0.65 (1)	10.00			
N- M	0 / 3992	-28.0 -28.0	0.65 (1)	10.00			
M- L	0 / 2227	-28.0 -28.0	0.33 (1)	10.00			
L- K	0 / 0	-28.0 -28.0	0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	12-10-8	-1785	-1785	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 995 (0.31")

CSI: TC=0.81 (D-E:1) , BC=0.65 (M-O:1) , WB=0.64 (G-M:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.66 (N) (INPUT = 1.00)

DWG NO. TAM50094-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272180	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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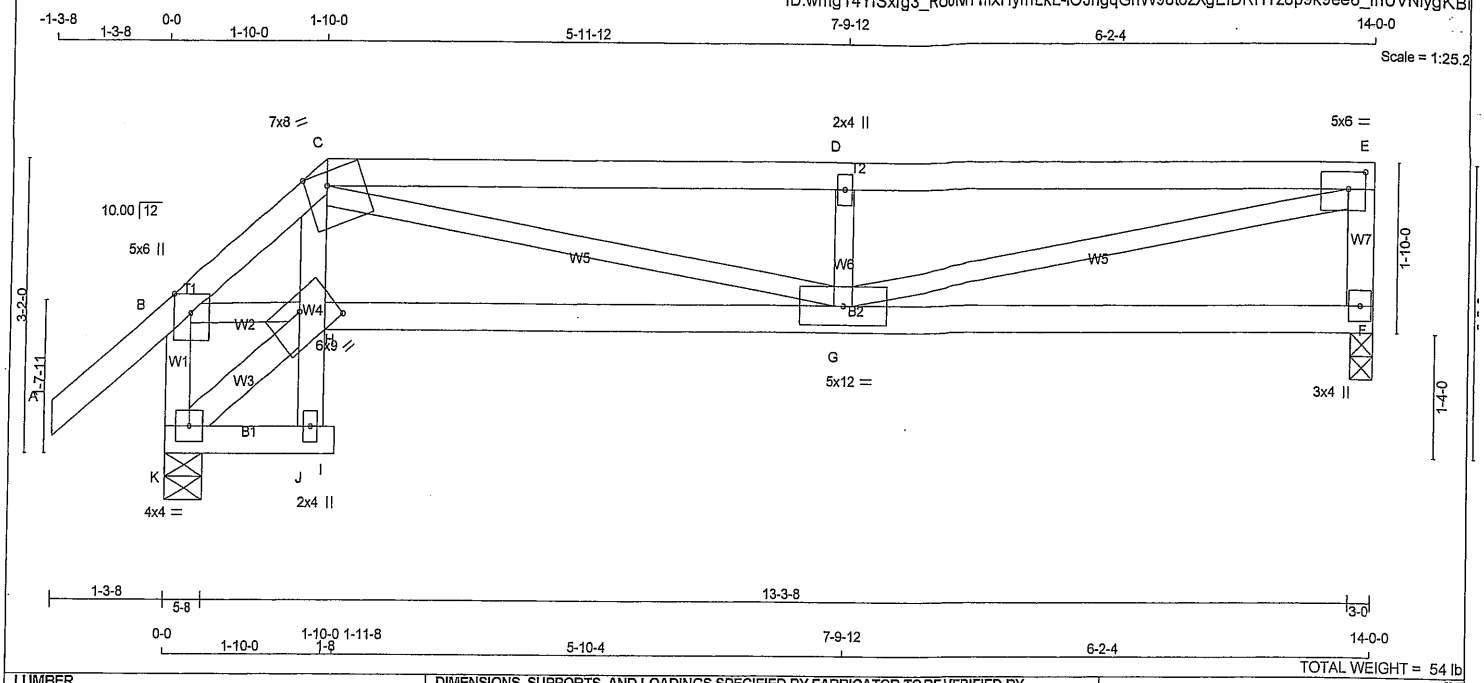

TOTAL WEIGHT = 54 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0	2.25	2.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	12.0		
H	BWMWW*+w	MT20	6.0	9.0	4.50	4.00
J	BMW+w	MT20	2.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1044	0	1044	0	0	3-0	3-0
K	1236	0	1236	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	818	530 / 0	148 / 0	0 / 0	0 / 0	140 / 0	0 / 0
K	951	646 / 0	152 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	-1351 / 0	-122.2	-122.2	0.18 (1)		5.37
C- D	-2674 / 0	-122.2	-122.2	0.92 (1)		3.05
D- E	-2674 / 0	-122.2	-122.2	0.94 (1)		3.05
F- E	-965 / 0	0.0	0.0	0.10 (1)		7.81
K- B	-1188 / 0	0.0	0.0	0.13 (1)		7.32
K- J	0 / 31	-28.0	-28.0	0.03 (2)		10.00
J- I	0 / 0	-28.0	-28.0	0.00 (2)		10.00
H- G	0 / 1019	-28.0	-28.0	0.35 (2)		10.00
G- F	0 / 0	-28.0	-28.0	0.27 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- D	-892 / 0	0.13 (1)
G- E	0 / 2755	0.62 (1)
J- H	0 / 55	0.06 (1)
H- C	0 / 160	0.07 (1)
K- H	-40 / 0	0.00 (1)
B- H	0 / 1000	0.22 (1)
C- G	0 / 1709	0.38 (1)

DWG NO. TAM5017 -17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 684 (0.25")

CSI: TC=0.94 (D-E:1) , BC=0.35 (G-H:2) , WB=0.62 (E-G:1) , SSI=0.36 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.67 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272180	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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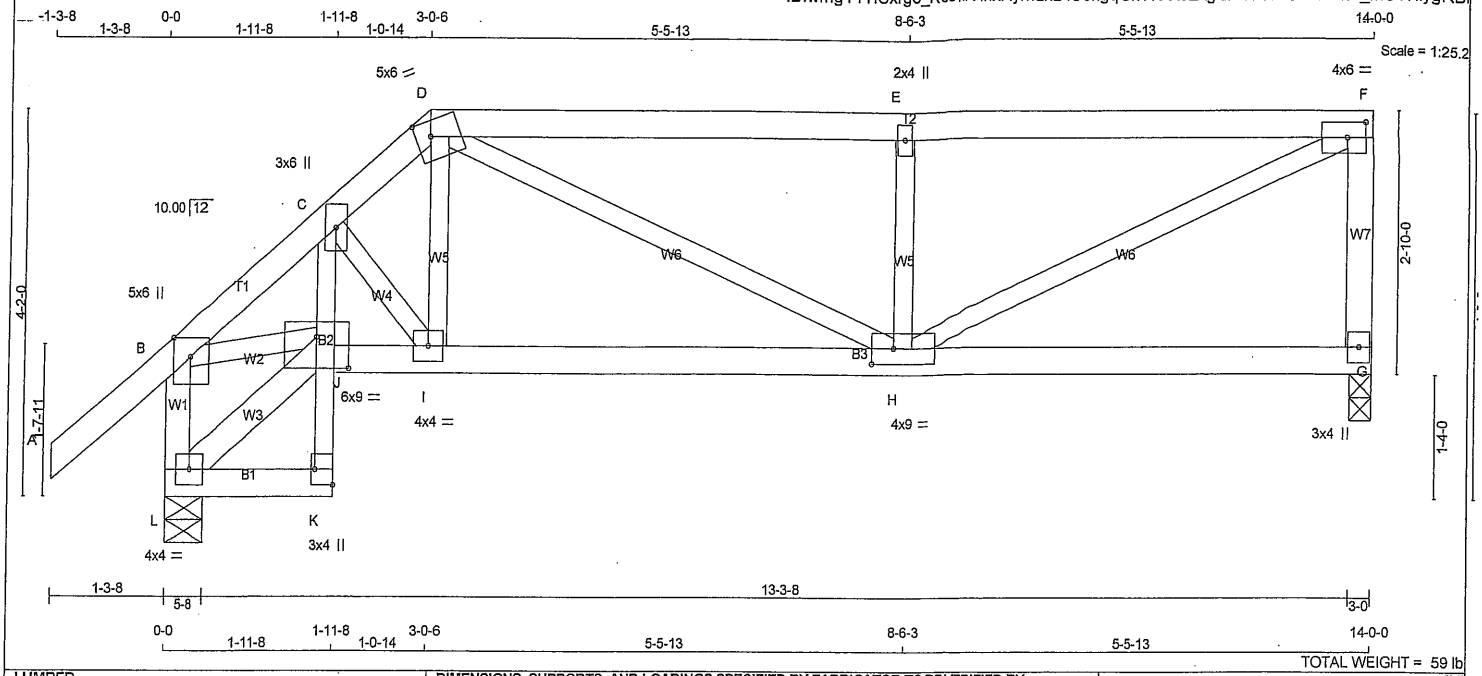

TOTAL WEIGHT = 59 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - C	2x3	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	3.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0	2.00	2.50
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0	2.00	3.00
I	BMWW-t	MT20	4.0	4.0		
J	BVMWW-I	MT20	6.0	9.0	4.00	4.50
K	BMV+p	MT20	3.0	4.0	2.00	Edge
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1043	0	1043	0	0	3-0	3-0
L	1229	0	1229	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	817	530 / 0	147 / 0	0 / 0	0 / 0	140 / 0	0 / 0
L	942	646 / 0	147 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- I	-233 / 0	0.04 (1)
B- C	-1498 / 0	-122.2	-122.2	0.19 (1)	5.16	I- D	0 / 242	0.05 (2)
C- D	-1257 / 0	-122.2	-122.2	0.08 (1)	5.66	D- H	0 / 635	0.14 (1)
D- E	-1557 / 0	-122.2	-122.2	0.67 (1)	4.30	H- E	-820 / 0	0.14 (1)
E- F	-1557 / 0	-122.2	-122.2	0.68 (1)	4.29	H- F	0 / 1714	0.39 (1)
G- F	-976 / 0	0.0	0.0	0.14 (1)	7.81	L- J	-15 / 0	0.00 (1)
L- B	-1194 / 0	0.0	0.0	0.13 (1)	7.32	B- J	0 / 1114	0.25 (1)
L- K	0 / 12	-28.0	-28.0	0.03 (2)	10.00			
K- J	0 / 45	0.0	0.0	0.05 (1)	10.00			
J- C	0 / 203	0.0	0.0	0.09 (1)	10.00			
J- I	0 / 1124	-28.0	-28.0	0.26 (1)	10.00			
I- H	0 / 980	-28.0	-28.0	0.30 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.20 (3)	10.00			

DWG NO. TAM5016-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.68 (E-F:1) , BC=0.30 (H-I:2) , WB=0.39 (F-H:1) , SSI=0.32 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.39 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272180	T22	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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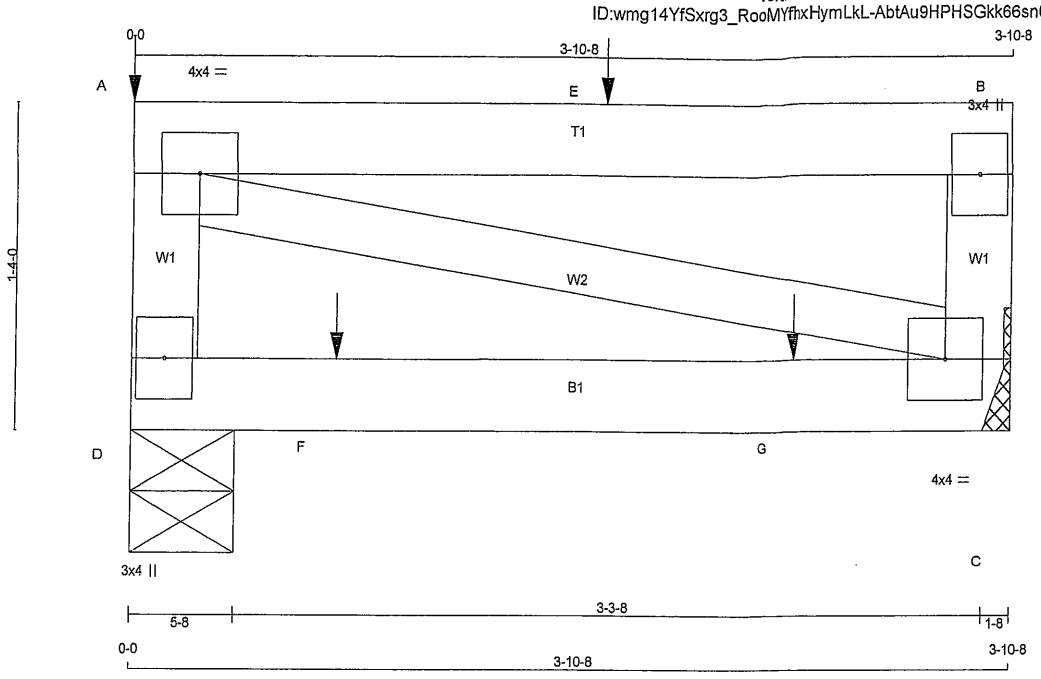

Scale = 1:9.5

TOTAL WEIGHT = 2 X 13 = 27 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 979.1 lbs FACTORED DOWN AT 0-0, AND 920.9 lbs FACTORED DOWN AT 2-0-12 ON TOP CHORD, AND 1019.0 lbs FACTORED DOWN AT 10-12, AND 1017.8 lbs FACTORED DOWN AT 2-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	2741	0	2741	0	0	5-8	5-8
C	1777	0	1777	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2127	1414 / 0	362 / 0	0 / 0	0 / 0	352 / 0	0 / 0
C	1367	930 / 0	219 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-1647 / 0	0.0	0.0	0.09 (1)	7.81	A- C	0 / 0	0.00 (1)
A- E	0 / 0	-122.2	-122.2	0.84 (1)	10.00			
E- B	0 / 0	-122.2	-122.2	0.84 (1)	10.00			
C- B	-727 / 0	0.0	0.0	0.04 (1)	7.81			
D- F	0 / 0	-28.0	-28.0	0.76 (1)	10.00			
F- G	0 / 0	-28.0	-28.0	0.76 (1)	10.00			
G- C	0 / 0	-28.0	-28.0	0.76 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-979	-979	—	TOP	VERT	TOTAL
E	2-0-12	-921	-921	—	TOP	VERT	TOTAL
F	10-12	-1019	-1019	—	FRONT	VERT	TOTAL
G	2-10-12	-1018	-1018	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM5011 9-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 394 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 360 (0.18")

CSI: TC=0.84 (A-B:1) , BC=0.76 (C-D:1) , WB=0.00 (A-C:1) , SSI=0.47 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (A) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272180	T23	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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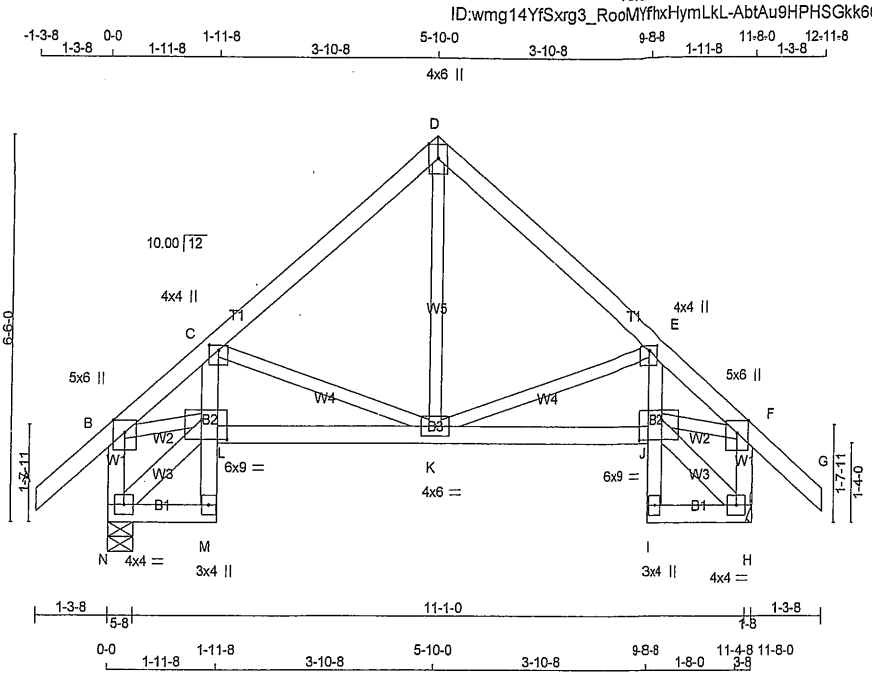


TOTAL WEIGHT = 3 X 61 = 182 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	6.0	9.0	4.00	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-l	MT20	6.0	9.0	4.00	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1045	0	1045	0	0	5-8	5-8
H	1045	0	1045	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	799	552 / 0	123 / 0	0 / 0	0 / 0	125 / 0	0 / 0
H	799	552 / 0	123 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

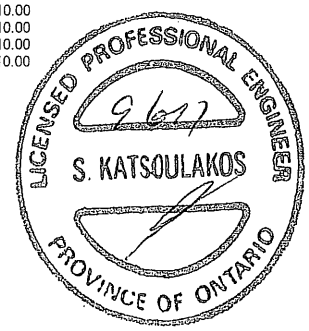
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	K- D	0 / 435	0.10 (1)
B- C	-1310 / 0	-122.2	-122.2	0.17 (1)	5.48	K- E	-565 / 0	0.17 (1)
C- D	-693 / 0	-122.2	-122.2	0.25 (1)	6.25	C- K	-565 / 0	0.17 (1)
D- E	-693 / 0	-122.2	-122.2	0.25 (1)	6.25	N- L	-38 / 0	0.00 (1)
E- F	-1310 / 0	-122.2	-122.2	0.17 (1)	5.48	B- L	0 / 1018	0.23 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00	J- H	-38 / 0	0.00 (1)
N- B	-996 / 0	0.0	0.0	0.11 (1)	7.81	J- F	0 / 1018	0.23 (1)
H- F	-996 / 0	0.0	0.0	0.11 (1)	7.81			
N- M	0 / 29	-28.0	-28.0	0.03 (2)	10.00			
M- L	0 / 44	0.0	0.0	0.06 (1)	10.00			
L- C	0 / 208	0.0	0.0	0.09 (1)	10.00			
L- K	0 / 1046	-28.0	-28.0	0.24 (1)	10.00			
K- J	0 / 1046	-28.0	-28.0	0.24 (1)	10.00			
I- J	0 / 44	0.0	0.0	0.06 (1)	10.00			
J- E	0 / 208	0.0	0.0	0.09 (1)	10.00			
I- H	0 / 29	-28.0	-28.0	0.03 (2)	10.00			



DWG NO. TAN5012O-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.25 (C-D:1) , BC=0.24 (K-L:1) , WB=0.23 (B-L:1) , SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.25 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272206	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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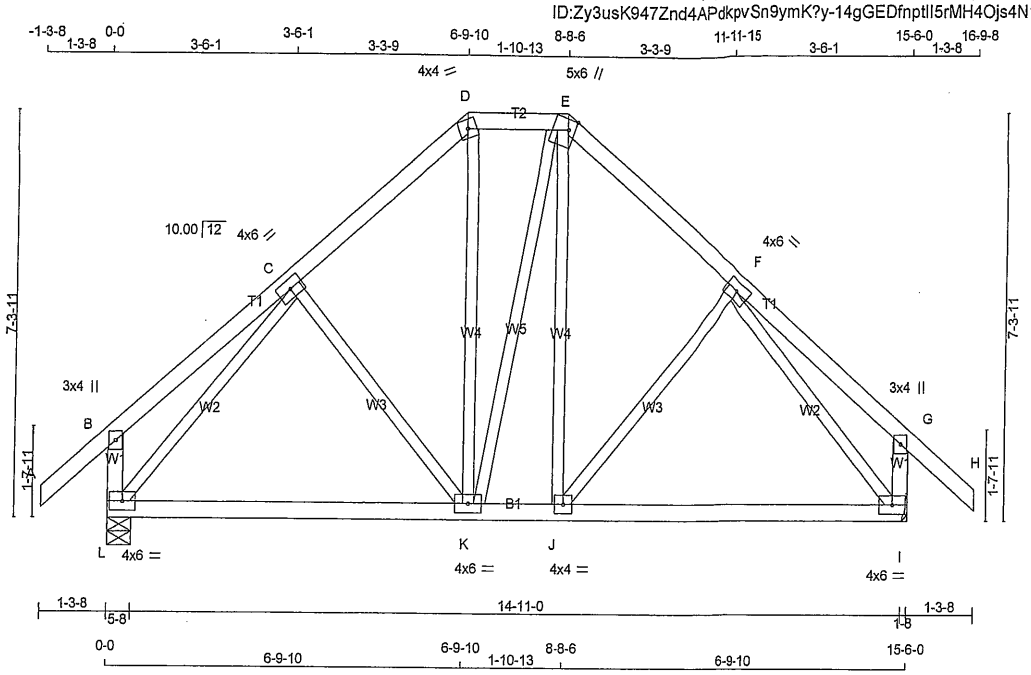

Scale = 1:42.0

TOTAL WEIGHT = 80 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1333	0	1333	0	0	5-8	5-8
I	1333	0	1333	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1025	699 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0
I	1025	699 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 31	-122.2	-122.2	0.22 (1)	10.00	
C- D	-878 / 0	-122.2	-122.2	0.18 (1)	6.25	
D- E	-653 / 0	-122.2	-122.2	0.06 (1)	6.25	
E- F	-876 / 0	-122.2	-122.2	0.18 (1)	6.25	
F- G	0 / 31	-122.2	-122.2	0.22 (1)	10.00	
G- H	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
L- B	-328 / 0	0.0	0.0	0.03 (1)	7.81	
I- G	-328 / 0	0.0	0.0	0.03 (1)	7.81	
L- K	0 / 759	-28.0	-28.0	0.36 (2)	10.00	
K- J	0 / 652	-28.0	-28.0	0.36 (2)	10.00	
J- I	0 / 758	-28.0	-28.0	0.35 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-169 / 47	0.08 (1)
K- D	0 / 266	0.06 (1)
K- E	0 / 5	0.00 (1)
J- E	0 / 261	0.06 (1)
J- F	-170 / 47	0.08 (1)
L- C	-1198 / 0	0.55 (1)
F- I	-1196 / 0	0.55 (1)

LICENSED PROFESSIONAL ENGINEER
9647
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50123-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.22 (F-G:1) , BC=0.36 (K-L:2) , WB=0.55 (C-L:1) , SSI=0.16 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272206	G24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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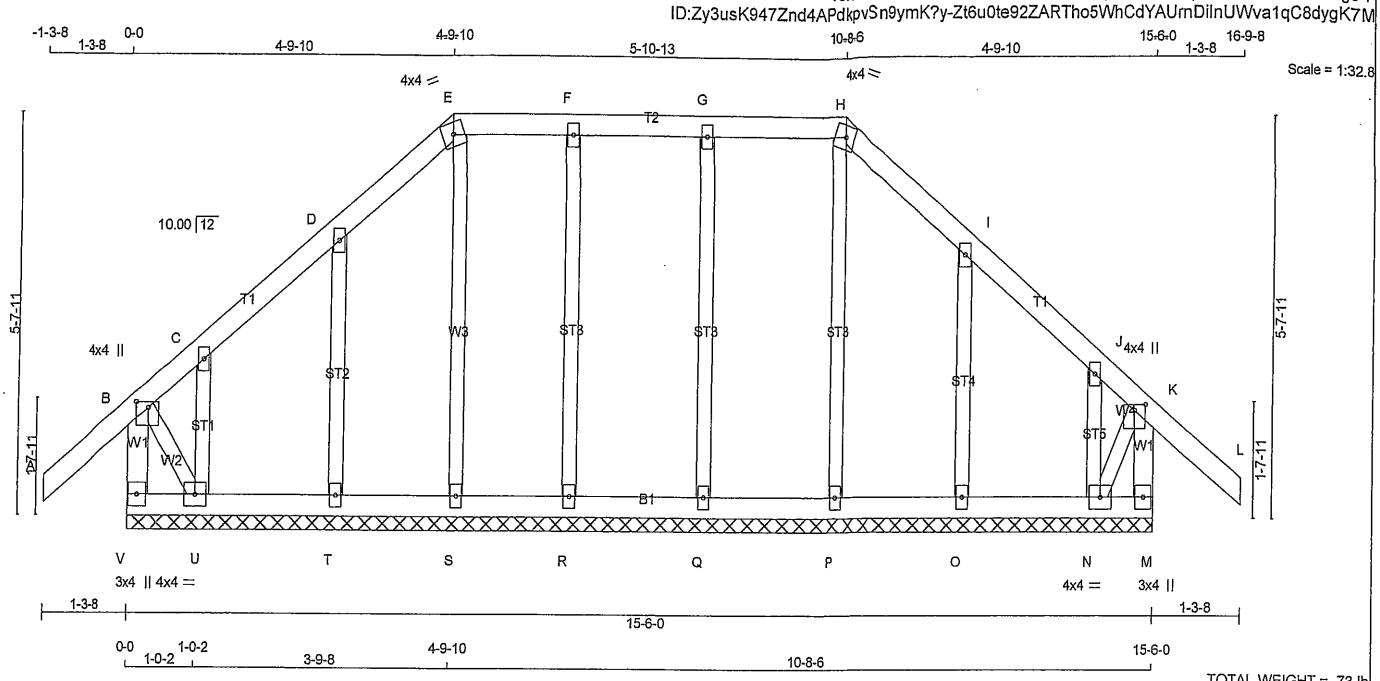


TOTAL WEIGHT = 73 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
V - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G, I, J						
C	TMW+w	MT20	2.0	4.0		
E	TTW-m	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	4.0		
K	TMVW+p	MT20	4.0	4.0	1.00	2.00
M	BMV1+p	MT20	3.0	4.0		
N	BMWW1-t	MT20	4.0	4.0		
O, P, Q, R, S, T						
O	BMW1+w	MT20	2.0	4.0		
U	BMWW1-t	MT20	4.0	4.0		
V	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

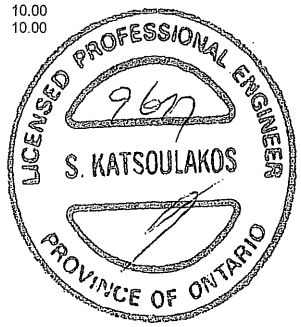
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
V- B	-346 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-74 / 0	-122.2	-122.2	0.16 (1)	6.25
C- D	0 / 5	-122.2	-122.2	0.07 (1)	10.00
D- E	-14 / 0	-122.2	-122.2	0.07 (1)	6.25
E- F	0 / 6	-122.2	-122.2	0.06 (1)	10.00
F- G	0 / 6	-122.2	-122.2	0.07 (1)	10.00
G- H	0 / 6	-122.2	-122.2	0.07 (1)	10.00
H- I	-15 / 0	-122.2	-122.2	0.07 (1)	6.25
I- J	0 / 5	-122.2	-122.2	0.07 (1)	10.00
J- K	-88 / 0	-122.2	-122.2	0.16 (1)	6.25
K- L	0 / 54	-122.2	-122.2	0.17 (1)	10.00
M- K	-358 / 0	0.0	0.0	0.04 (1)	7.81
V- U	0 / 0	-28.0	-28.0	0.02 (2)	10.00
U- T	0 / 4	-28.0	-28.0	0.02 (2)	10.00
T- S	-2 / 0	-28.0	-28.0	0.02 (2)	10.00
S- R	-6 / 0	-28.0	-28.0	0.02 (2)	10.00
R- Q	-6 / 0	-28.0	-28.0	0.02 (2)	10.00
Q- P	-6 / 0	-28.0	-28.0	0.02 (2)	10.00
P- O	-2 / 0	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 6	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- H	-200 / 0	0.10 (1)
Q- G	-282 / 0	0.14 (1)
R- F	-249 / 0	0.12 (1)
T- D	-269 / 0	0.07 (1)
U- C	-65 / 0	0.01 (1)
O- I	-283 / 0	0.07 (1)
N- J	-29 / 0	0.00 (1)
S- E	-171 / 0	0.08 (1)
B- U	0 / 11	0.00 (1)
N- K	0 / 12	0.00 (1)



DWG NO. TAM 5012417
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.02 (N-O:2) , WB=0.14 (G-Q:1) , SSI=0.12 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

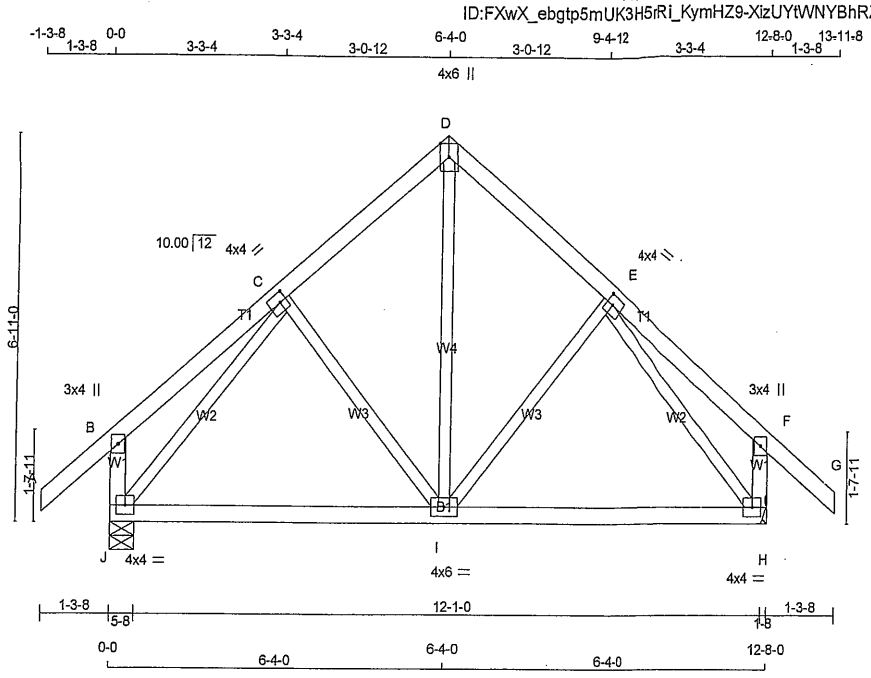
JSI GRIP= 0.27 (K) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272218	T90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:FXwX_ebgtp5mUK3H5rRi_KymHZ9-XizUYtWNYBhRZV8u5kUzKsPKJiW?bX2jDetv39ygK4x

Scale = 1:41.7



TOTAL WEIGHT = 61 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1121	0	1121	0	0	5-8	5-8
H	1121	0	1121	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	858	590 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	858	590 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 491	0.11 (1)
B- C	0 / 28	-122.2	-122.2	0.19 (1)	10.00	I- E	-176 / 39	0.07 (1)
C- D	-649 / 0	-122.2	-122.2	0.15 (1)	6.25	C- I	-176 / 39	0.07 (1)
D- E	-649 / 0	-122.2	-122.2	0.15 (1)	6.25	J- C	-942 / 0	0.38 (1)
E- F	0 / 28	-122.2	-122.2	0.19 (1)	10.00	E- H	-942 / 0	0.38 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
J- B	-317 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-317 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 589	-28.0	-28.0	0.38 (2)	10.00			
I- H	0 / 589	-28.0	-28.0	0.38 (2)	10.00			

DWG NO. TAM5006E17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.19 (E-F:1) , BC=0.38 (I-J:2) , WB=0.38 (E-H:1) , SSI=0.15 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

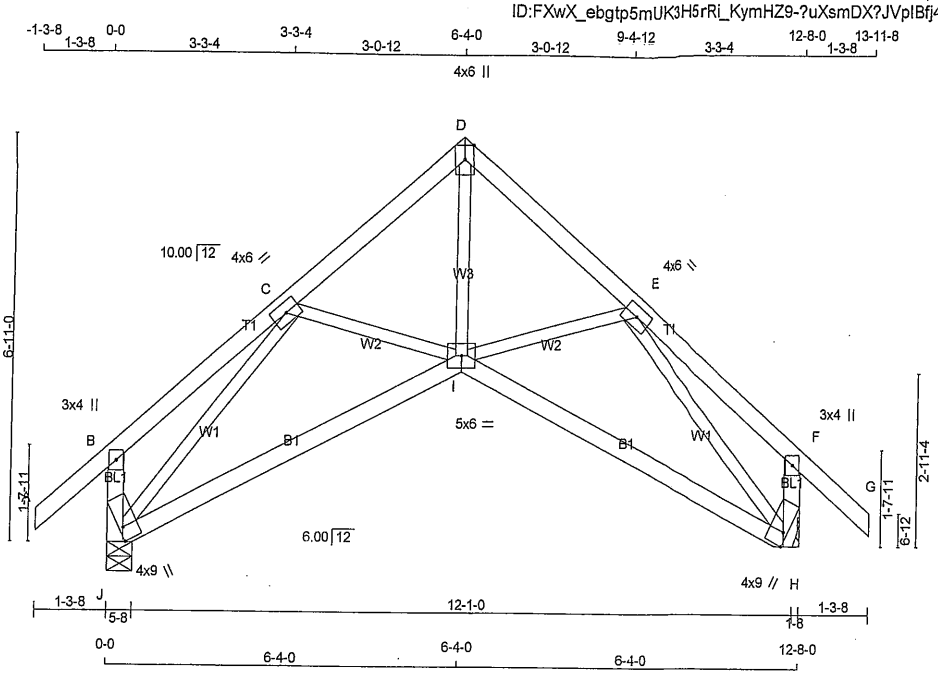
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.35 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272218	T90S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 3 X 58 = 173 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2 - 2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMK+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMK+p	MT20	3.0	4.0		
H	BWKM1*+m	MT20	4.0	9.0	Edge	0.75
I	BBWWW-p	MT20	5.0	6.0		
J	BWKM1*+m	MT20	4.0	9.0	Edge	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1121	0	1121	0	0	5-8	5-8
H	1121	0	1121	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	858	590 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	858	590 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.18 (1)	10.00
B- C	-1 / 10	-122.2	-122.2	0.16 (1)	10.00
C- D	-1108 / 0	-122.2	-122.2	0.14 (1)	5.87
D- E	-1108 / 0	-122.2	-122.2	0.14 (1)	5.87
E- F	-1 / 10	-122.2	-122.2	0.16 (1)	10.00
F- G	0 / 56	-122.2	-122.2	0.18 (1)	10.00
J- B	-338 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-338 / 0	0.0	0.0	0.04 (1)	7.81
J- I	0 / 977	-28.0	-28.0	0.42 (2)	10.00
I- H	0 / 977	-28.0	-28.0	0.42 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 1076	0.24 (1)
I- E	-43 / 81	0.02 (3)
C- I	-43 / 81	0.02 (3)
J- C	-1422 / 0	0.60 (1)
E- H	-1422 / 0	0.60 (1)

LICENSED PROFESSIONAL ENGINEER 9617 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM5008 9-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 702 (0.22")

CSI: TC=0.18 (F-G:1) , BC=0.42 (H-I:2) , WB=0.60 (E-H:1) , SSI=0.14 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

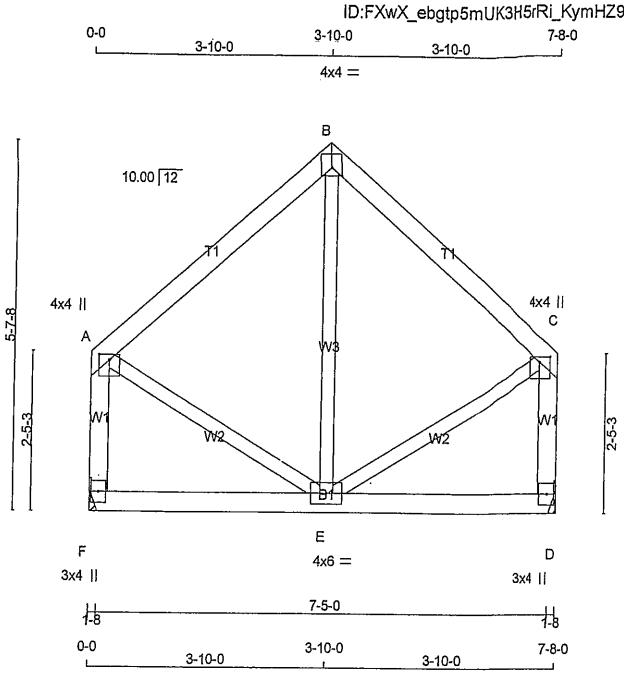
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.43 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272218	T91	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:FXwX_ebgtp5mUK3H5rRi_KymHZ9-?uXsmDX?JVpIBfj4fS?Cs3xUS5wKK3OtSIdTbbygK4w



Scale = 1:35.6

TOTAL WEIGHT = 36 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	576	0	576	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
D	576	0	576	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	450	293 / 0	81 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	450	293 / 0	81 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-283 / 0	-122.2	-122.2	0.23 (1)	6.25		E- B	-106 / 129	0.05 (1)
B- C	-283 / 0	-122.2	-122.2	0.23 (1)	6.25		A- E	0 / 248	0.06 (1)
F- A	-534 / 0	0.0	0.0	0.07 (1)	7.81		E- C	0 / 248	0.06 (1)
D- C	-534 / 0	0.0	0.0	0.07 (1)	7.81				
F- E	0 / 0	-28.0	-28.0	0.12 (3)	10.00				
E- D	0 / 0	-28.0	-28.0	0.12 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23 (B-C:1) , BC=0.12 (D-E:3) , WB=0.06 (A-E:1) , SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)

DWG NO. TAM50090-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272218	T91ZCP	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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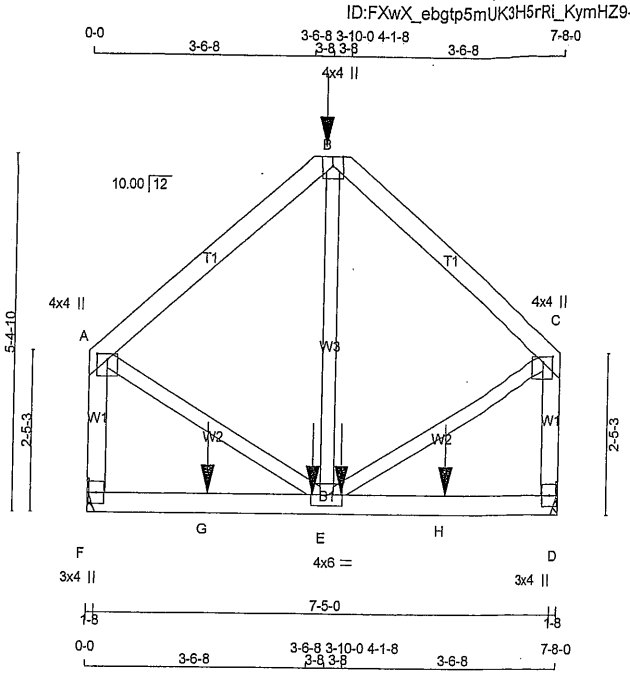
Scale = 1:35.2



TOTAL WEIGHT = 35 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMTMW+p	MT20	4.0	4.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 634.4 lbs FACTORED DOWN AT 3-10-0 ON TOP CHORD, AND 38.6 lbs FACTORED DOWN AT 1-11-4, 37.8 lbs FACTORED DOWN AT 3-7-4, AND 37.8 lbs FACTORED DOWN AT 4-0-12, AND 38.6 lbs FACTORED DOWN AT 5-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	939	0	939	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
D	939	0	939	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	721	489 / 0	113 / 0	0 / 0	0 / 0	119 / 0	0 / 0
D	721	489 / 0	113 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-617 / 0	-122.2	-122.2	0.35 (1)	6.25	A- E	0 / 541	0.13 (1)
B- C	-617 / 0	-122.2	-122.2	0.35 (1)	6.25	E- C	0 / 541	0.13 (1)
F- A	-889 / 0	0.0	0.0	0.12 (1)	7.81	E- B	-313 / 185	0.15 (1)
D- C	-889 / 0	0.0	0.0	0.12 (1)	7.81			
F- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
G- E	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
E- H	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
H- D	0 / 0	-28.0	-28.0	0.17 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-10-0	-634	-634	—	BACK	VERT	TOTAL
E	3-7-4	-22	-38	—	BACK	VERT	TOTAL
E	4-0-12	-22	-38	—	BACK	VERT	TOTAL
G	1-11-4	-24	-39	—	BACK	VERT	TOTAL
H	5-8-12	-24	-39	—	BACK	VERT	TOTAL

DWG NO. TAM5009I-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.35 (B-C:1) , BC=0.17 (E-F:3) , WB=0.15 (B-E:1) , SSI=0.15 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

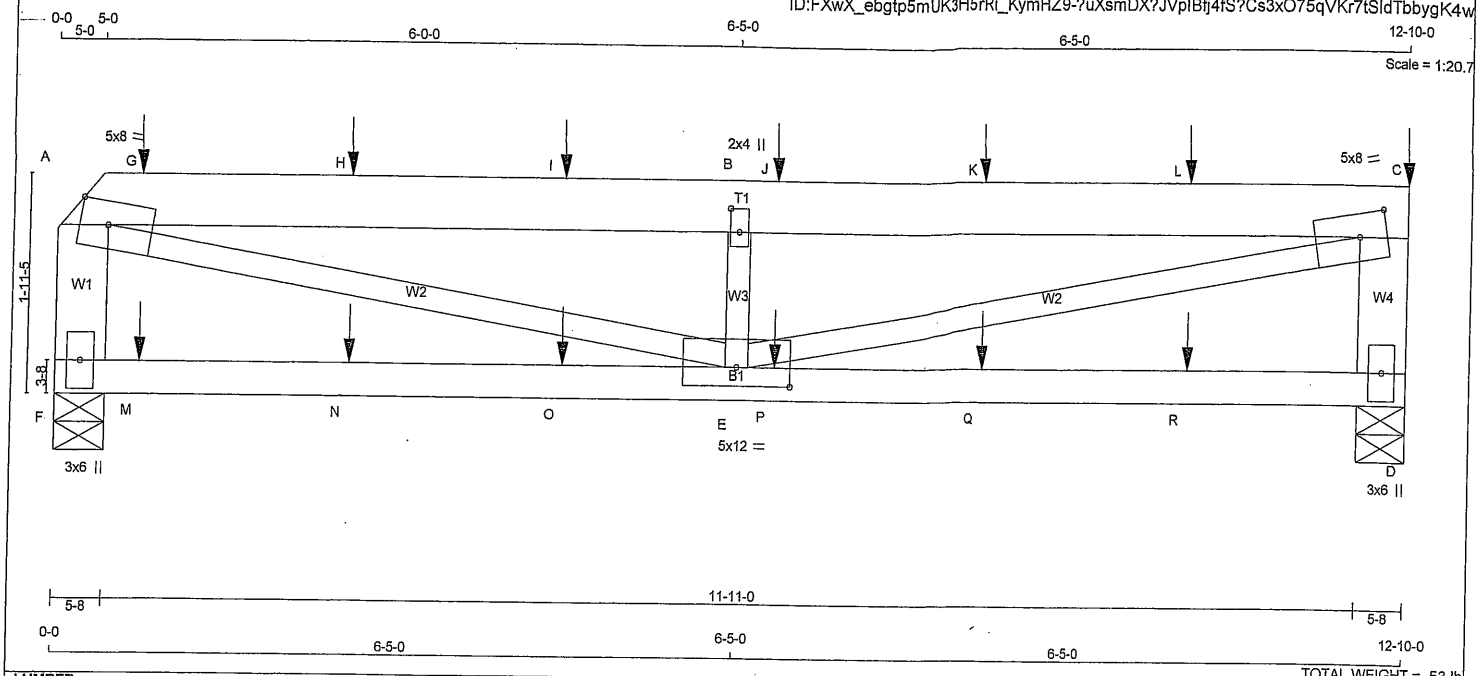
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (A) (INPUT = 0.90)
JSI METAL= 0.20 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272218	T93	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 53 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+w	MT20	5.0	8.0	Edge	
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMVW+w	MT20	5.0	8.0	3.00	2.50
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	12.0	2.00	6.00
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 120.3 lbs FACTORED DOWN AT 9-4, 106.3 lbs FACTORED DOWN AT 2-9-4, 106.3 lbs FACTORED DOWN AT 4-9-4, 106.3 lbs FACTORED DOWN AT 6-9-4, 106.3 lbs FACTORED DOWN AT 8-9-4, AND 106.3 lbs FACTORED DOWN AT 10-9-4, AND 163.6 lbs FACTORED DOWN AT 12-10-0 ON TOP CHORD, AND 69.0 lbs FACTORED DOWN AT 9-4, 65.8 lbs FACTORED DOWN AT 2-9-4, 65.8 lbs FACTORED DOWN AT 4-9-4, 65.8 lbs FACTORED DOWN AT 6-9-4, AND 65.8 lbs FACTORED DOWN AT 8-9-4, AND 65.8 lbs FACTORED DOWN AT 10-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1593	0	1593	0	0	5-8	5-8
F	1548	0	1548	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1233	824 / 0	207 / 0	0 / 0	0 / 0	202 / 0	0 / 0
F	1211	788 / 0	216 / 0	0 / 0	0 / 0	206 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- G	-3548 / 0	-122.2	-122.2	0.64 (1)	3.75	
G- H	-3548 / 0	-122.2	-122.2	0.64 (1)	3.75	
H- I	-3548 / 0	-122.2	-122.2	0.64 (1)	3.75	
I- B	-3548 / 0	-122.2	-122.2	0.64 (1)	3.75	
B- J	-3548 / 0	-122.2	-122.2	0.60 (1)	3.81	
J- K	-3548 / 0	-122.2	-122.2	0.60 (1)	3.81	
K- L	-3548 / 0	-122.2	-122.2	0.60 (1)	3.81	
L- C	-3548 / 0	-122.2	-122.2	0.60 (1)	3.81	
D- C	-1463 / 0	0.0	0.0	0.11 (1)	7.81	
F- A	-1376 / 0	0.0	0.0	0.10 (1)	7.81	
F- M	0 / 0	-28.0	-28.0	0.49 (2)	10.00	
M- N	0 / 0	-28.0	-28.0	0.49 (2)	10.00	
N- O	0 / 0	-28.0	-28.0	0.49 (2)	10.00	
O- E	0 / 0	-28.0	-28.0	0.49 (2)	10.00	
E- P	0 / 0	-28.0	-28.0	0.49 (2)	10.00	
P- Q	0 / 0	-28.0	-28.0	0.49 (2)	10.00	
Q- R	0 / 0	-28.0	-28.0	0.49 (2)	10.00	
R- D	0 / 0	-28.0	-28.0	0.49 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- C	0 / 3652	0.90 (1)
A- E	0 / 3652	0.90 (1)
E- B	-1278 / 0	0.20 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	12-10-0	-164	-164	---	BACK	VERT	TOTAL
G	9-4	-120	-120	---	BACK	VERT	TOTAL
H	2-9-4	-106	-106	---	BACK	VERT	TOTAL
I	4-9-4	-106	-106	---	BACK	VERT	TOTAL
J	6-9-4	-106	-106	---	BACK	VERT	TOTAL
K	8-9-4	-106	-106	---	BACK	VERT	TOTAL
L	10-9-4	-106	-106	---	BACK	VERT	TOTAL
M	9-4	-69	-69	---	BACK	VERT	TOTAL
N	2-9-4	-66	-66	---	BACK	VERT	TOTAL
O	4-9-4	-66	-66	---	BACK	VERT	TOTAL
P	6-9-4	-66	-66	---	BACK	VERT	TOTAL
Q	8-9-4	-66	-66	---	BACK	VERT	TOTAL
R	10-9-4	-66	-66	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 9627 S. KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 738 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 462 (0.33")

CSI: TC=0.64 (A-B:1) , BC=0.49 (E-F:2) , WB=0.90 (C-E:1) , SSI=0.44 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.63 (C) (INPUT = 1.00)

DWG NO. TAM50092-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272219	T94S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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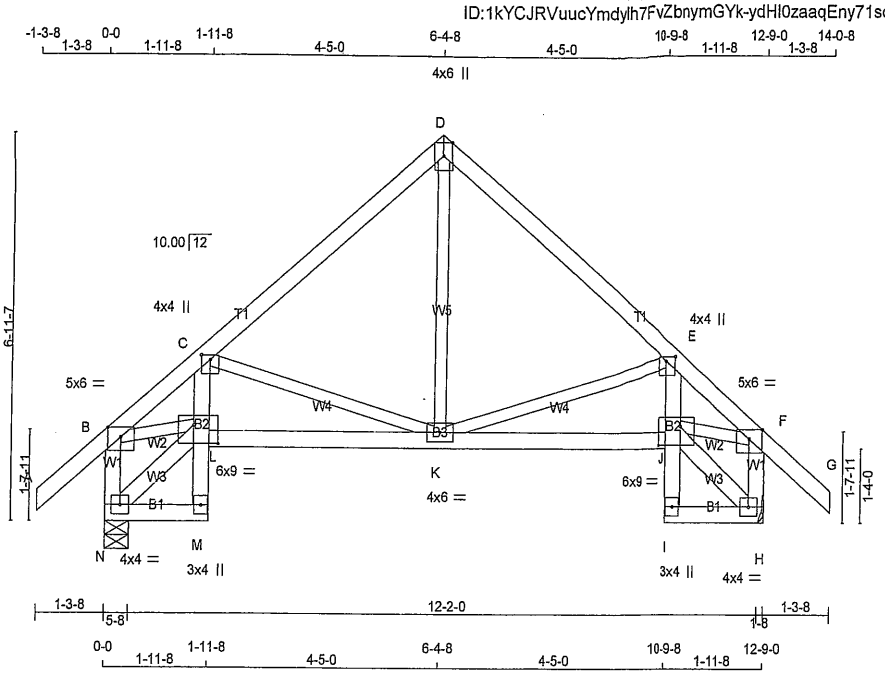


TOTAL WEIGHT = 3 X 65 = 194 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	6.0	9.0	4.00	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-l	MT20	6.0	9.0	4.00	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1127	0	1127	0	0	5-8	5-8
H	1127	0	1127	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	863	593 / 0	134 / 0	0 / 0	0 / 0	136 / 0	0 / 0
H	863	593 / 0	134 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

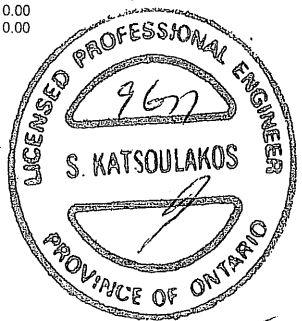
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54		-122.2	-122.2	0.17 (1)		10.00
B- C	-1487 / 0		-122.2	-122.2	0.22 (1)		5.15
C- D	-767 / 0		-122.2	-122.2	0.32 (1)		6.25
D- E	-767 / 0		-122.2	-122.2	0.32 (1)		6.25
E- F	-1487 / 0		-122.2	-122.2	0.22 (1)		5.15
F- G	0 / 54		-122.2	-122.2	0.17 (1)		10.00
N- B	-1074 / 0		0.0	0.0	0.11 (1)		7.61
H- F	-1074 / 0		0.0	0.0	0.11 (1)		7.61
N- M	0 / 33		-28.0	-28.0	0.03 (2)		10.00
M- L	0 / 44		0.0	0.0	0.07 (1)		10.00
L- C	0 / 229		0.0	0.0	0.11 (1)		10.00
L- K	0 / 1201		-28.0	-28.0	0.29 (1)		10.00
K- J	0 / 1201		-28.0	-28.0	0.29 (1)		10.00
I- J	0 / 44		0.0	0.0	0.07 (1)		10.00
J- E	0 / 229		0.0	0.0	0.11 (1)		10.00
I- H	0 / 33		-28.0	-28.0	0.03 (2)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX (LC)
K- D	0 / 473		0.11 (1)
K- E	-665 / 0		0.25 (1)
C- K	-665 / 0		0.25 (1)
N- L	-43 / 0		0.00 (1)
B- L	0 / 1170		0.26 (1)
J- H	-43 / 0		0.00 (1)
J- F	0 / 1170		0.26 (1)



DWG NO. TAM50095 17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.32 (C-D:1) , BC=0.29 (K-L:1) , WB=0.26 (B-L:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (L) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272219	T95	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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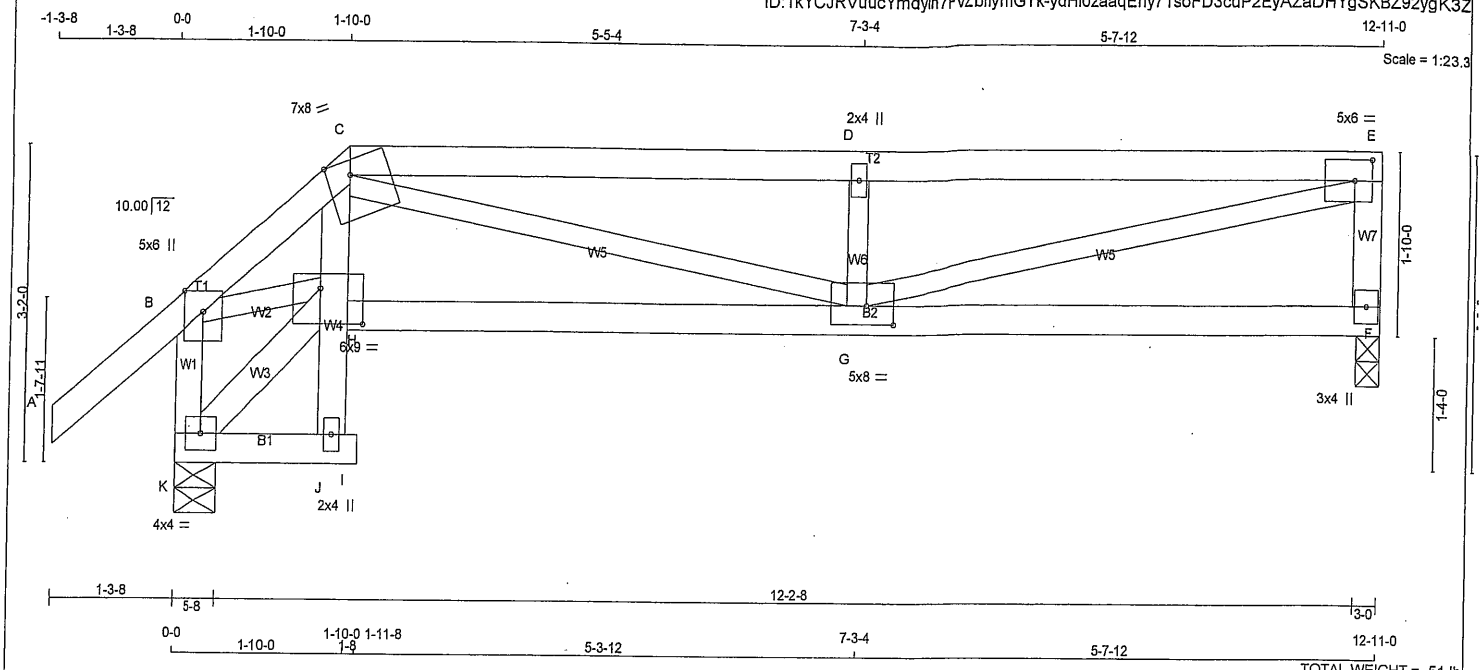

TOTAL WEIGHT = 51 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0	2.50	2.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0	2.25	3.50
H	BWMWW*-l	MT20	6.0	9.0	4.25	5.50
J	BMW+w	MT20	2.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	962	0	962	0	0	3-0	3-0
K	1155	0	1155	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	754	488 / 0	136 / 0	0 / 0	0 / 0	129 / 0	0 / 0
K	887	605 / 0	141 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00		C- G	0 / 1226	0.28 (1)
B- C	-1442 / 0	-122.2	-122.2	0.18 (1)	5.24		G- D	-813 / 0	0.12 (1)
C- D	-2261 / 0	-122.2	-122.2	0.70 (1)	3.67		G- E	0 / 2342	0.53 (1)
D- E	-2261 / 0	-122.2	-122.2	0.71 (1)	3.67		J- H	0 / 55	0.08 (1)
F- E	-890 / 0	0.0	0.0	0.10 (1)	7.81		H- C	0 / 266	0.11 (1)
K- B	-1101 / 0	0.0	0.0	0.12 (1)	7.54		K- H	-46 / 0	0.01 (1)
							B- H	0 / 1080	0.24 (1)
K- J	0 / 34	-28.0	-28.0	0.03 (2)	10.00				
J- I	0 / 0	-28.0	-28.0	0.00 (2)	10.00				
H- G	0 / 1081	-28.0	-28.0	0.32 (2)	10.00				
G- F	0 / 0	-28.0	-28.0	0.23 (3)	10.00				

DWG NO. TAM50096-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 880 (0.18")

CSI: TC=0.71 (D-E:1) , BC=0.32 (G-H:2) , WB=0.53 (E-G:1) , SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.56 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272219	T96	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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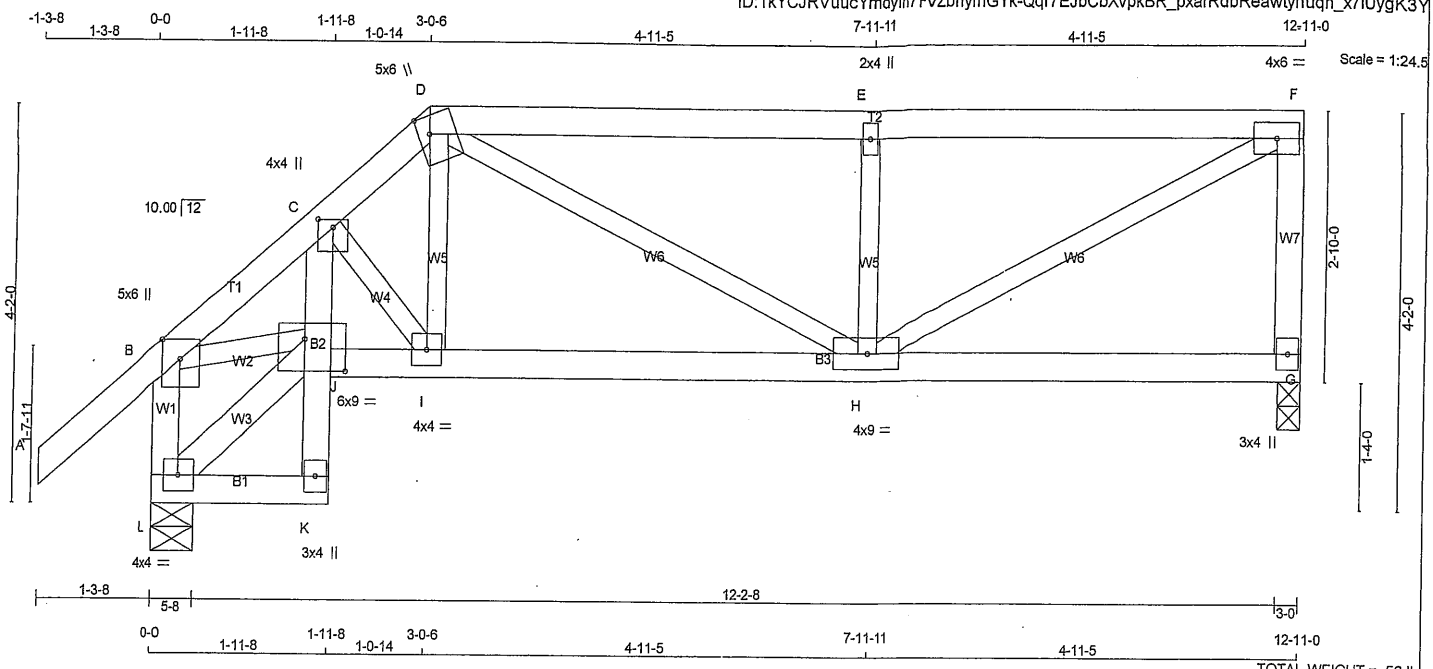


TOTAL WEIGHT = 56 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BVMWW-I	MT20	6.0	9.0	4.00	5.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	961	0	961	0	0	3-0	3-0
L	1148	0	1148	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	753	488 / 0	136 / 0	0 / 0	0 / 0	129 / 0	0 / 0
L	879	605 / 0	136 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

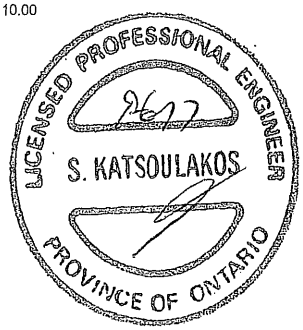
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-1383 / 0	-122.2	-122.2	0.18 (1)	5.32
C- D	-1124 / 0	-122.2	-122.2	0.07 (1)	5.91
D- E	-1305 / 0	-122.2	-122.2	0.52 (1)	4.91
E- F	-1305 / 0	-122.2	-122.2	0.52 (1)	4.91
G- F	-901 / 0	0.0	0.0	0.13 (1)	7.81
L- B	-1097 / 0	0.0	0.0	0.12 (1)	7.55
L- K	0 / 31	-28.0	-28.0	0.03 (2)	10.00
K- J	0 / 44	0.0	0.0	0.07 (1)	10.00
J- C	0 / 248	0.0	0.0	0.10 (1)	10.00
J- I	0 / 1062	-28.0	-28.0	0.23 (1)	10.00
I- H	0 / 878	-28.0	-28.0	0.25 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- I	-279 / 0	0.04 (1)
I- D	0 / 222	0.05 (2)
D- H	0 / 480	0.11 (1)
H- E	-738 / 0	0.13 (1)
H- F	0 / 1465	0.33 (1)
L- J	-40 / 0	0.00 (1)
B- J	0 / 1028	0.23 (1)



DWG NO. TAM5009717
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.52 (E-F:1) , BC=0.25 (H-I:2) , WB=0.33 (F-H:1) , SSI=0.29 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272219	T97	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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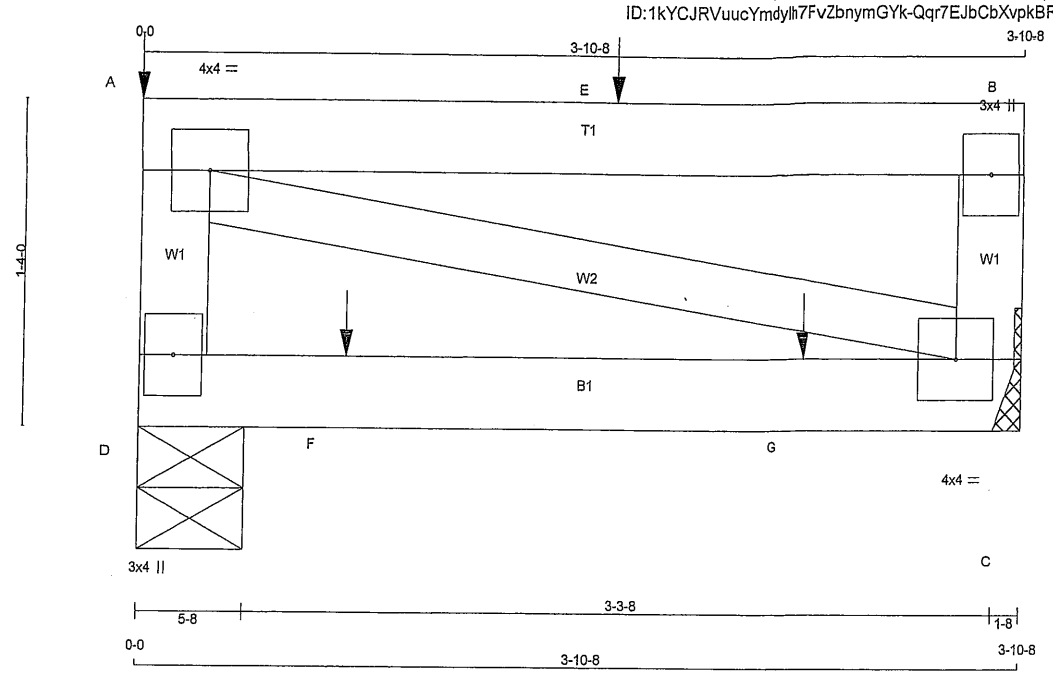

Scale = 1:9.5

TOTAL WEIGHT = 2 X 13 = 27 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	4.0	4.0	
B	TMV+p	MT20	3.0	4.0	
C	BMVW1-t	MT20	4.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 897.1 lbs FACTORED DOWN AT 0-0, AND 838.8 lbs FACTORED DOWN AT 2-0-12 ON TOP CHORD, AND 1100.3 lbs FACTORED DOWN AT 10-12, AND 1099.2 lbs FACTORED DOWN AT 2-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	2704	0	2704	0	0	5-8	5-8
C	1813	0	1813	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2098	1395 / 0	357 / 0	0 / 0	0 / 0	347 / 0	0 / 0
C	1395	948 / 0	224 / 0	0 / 0	0 / 0	224 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-1526 / 0	0.0	0.0	0.08 (1)	7.81	A- C	0 / 0	0.00 (1)
A- E	0 / 0	-122.2	-122.2	0.78 (1)	10.00			
E- B	0 / 0	-122.2	-122.2	0.78 (1)	10.00			
C- B	-683 / 0	0.0	0.0	0.04 (1)	7.81			
D- F	0 / 0	-28.0	-28.0	0.69 (1)	10.00			
F- G	0 / 0	-28.0	-28.0	0.69 (1)	10.00			
G- C	0 / 0	-28.0	-28.0	0.69 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-897	-897	---	TOP	VERT	TOTAL
E	2-0-12	-839	-839	---	TOP	VERT	TOTAL
F	10-12	-1100	-1100	---	FRONT	VERT	TOTAL
G	2-10-12	-1099	-1099	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T AM 50098 17.
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 397 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 360 (0.18")

CSI: TC=0.78 (A-B:1) , BC=0.69 (C-D:1) , WB=0.00 (A-C:1) , SSI=0.51 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (A) (INPUT = 0.90)
JSI METAL= 0.11 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
272180	PB1	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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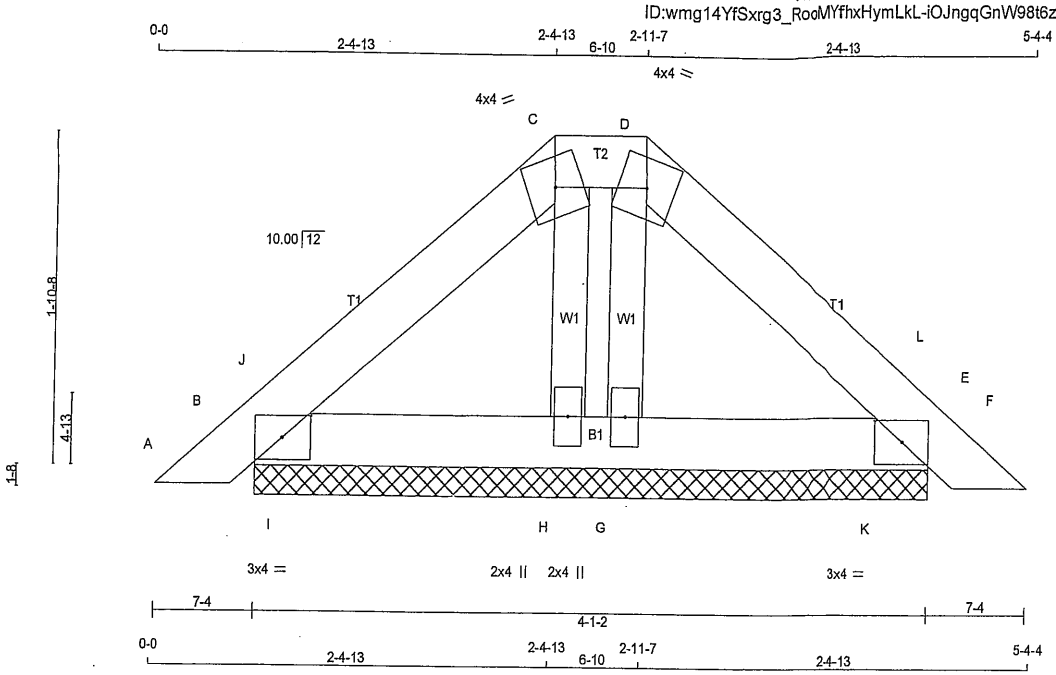


TOTAL WEIGHT = 2 X 14 = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	227	0	227	0	0	4-1-2	4-1-2
E	227	0	227	0	0	4-1-2	4-1-2
G	143	0	143	0	0	4-1-2	4-1-2
H	143	0	143	0	0	4-1-2	4-1-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	168	127 / 0	18 / 0	0 / 0	0 / 0	22 / 0	0 / 0
E	168	127 / 0	18 / 0	0 / 0	0 / 0	22 / 0	0 / 0
G	115	69 / 0	24 / 0	0 / 0	0 / 0	22 / 0	0 / 0
H	115	69 / 0	24 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 18	-122.2	-122.2	0.02 (1)	10.00	G- D	-83 / 0	0.01 (1)
B- J	-48 / 0	-122.2	-122.2	0.01 (1)	6.25	H- C	-83 / 0	0.01 (1)
J- C	-64 / 0	-122.2	-122.2	0.04 (1)	6.25	I- J	-129 / 24	0.00 (1)
C- D	-41 / 0	-122.2	-122.2	0.01 (1)	6.25	K- L	-129 / 24	0.00 (1)
D- L	-64 / 0	-122.2	-122.2	0.04 (1)	6.25			
L- E	-48 / 0	-122.2	-122.2	0.01 (1)	6.25			
E- F	0 / 18	-122.2	-122.2	0.02 (1)	10.00			
B- I	0 / 46	-28.0	-28.0	0.05 (1)	10.00			
I- H	0 / 46	-28.0	-28.0	0.05 (1)	10.00			
H- G	0 / 41	-28.0	-28.0	0.04 (1)	10.00			
G- K	0 / 46	-28.0	-28.0	0.05 (1)	10.00			
K- E	0 / 46	-28.0	-28.0	0.05 (1)	10.00			

DWG NO. TAM5012I-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (D-L:1) , BC=0.05 (G-K:1) , WB=0.01 (C-H:1) , SSI=0.10 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.04 (E) (INPUT = 1.00)

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

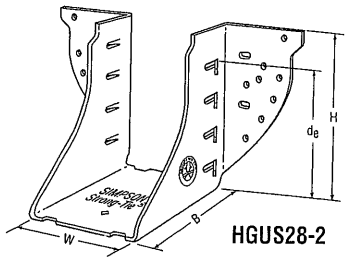
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

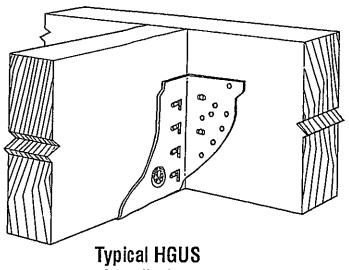
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

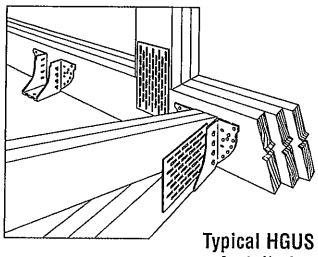
OPTIONS:

- See current catalogue for options



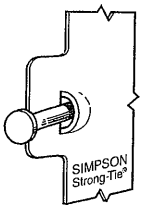
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

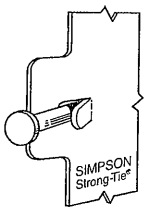
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅝₁₆	5⅞₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅜₁₆	5⅞₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅝₁₆	7⅜₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅜₁₆	7⅜₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅝₁₆	9⅜₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⅜₁₆	9⅜₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅜₁₆	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅜₁₆	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

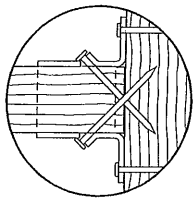


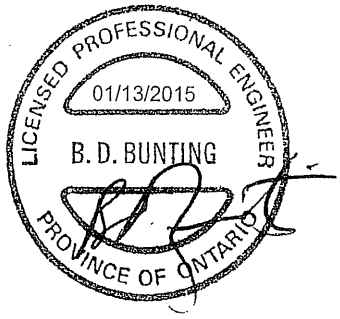
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

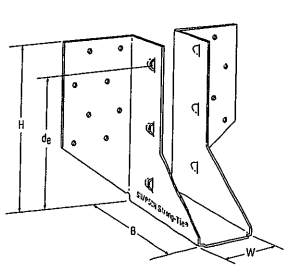
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

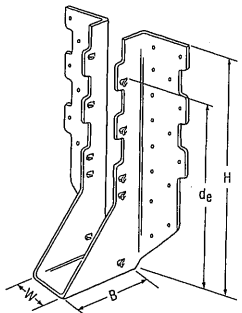
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

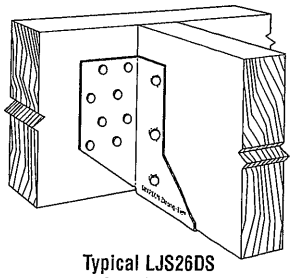
- See current catalogue for options



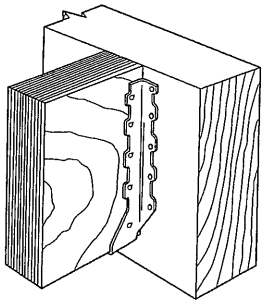
LJS26DS



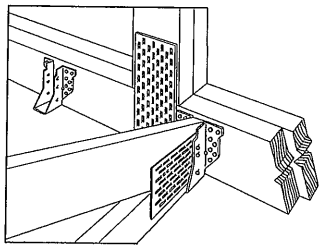
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



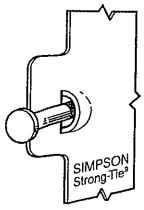
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

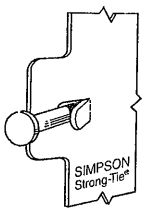
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1 9/16	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

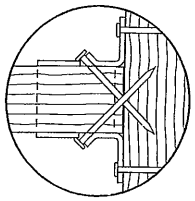


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

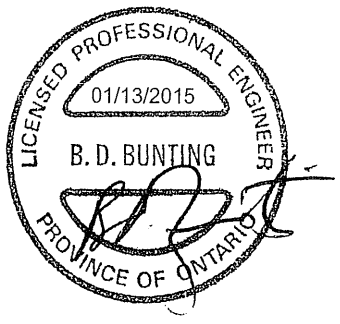
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

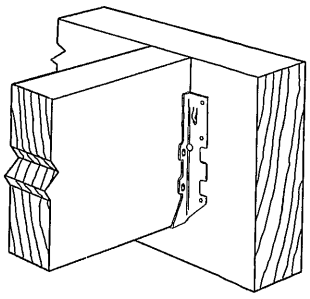
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

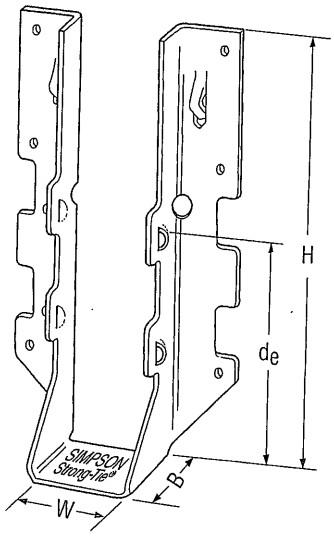
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



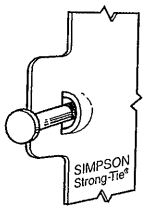
Typical LUS Installation



LUS28

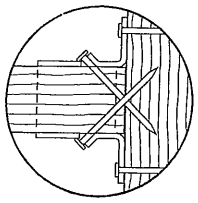
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	11 5/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	11 3/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

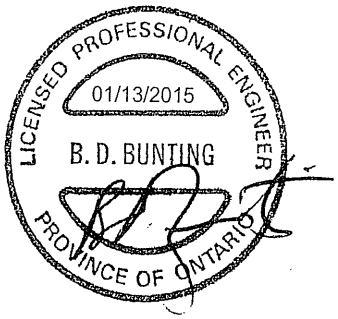


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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800-999-5099
www.strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

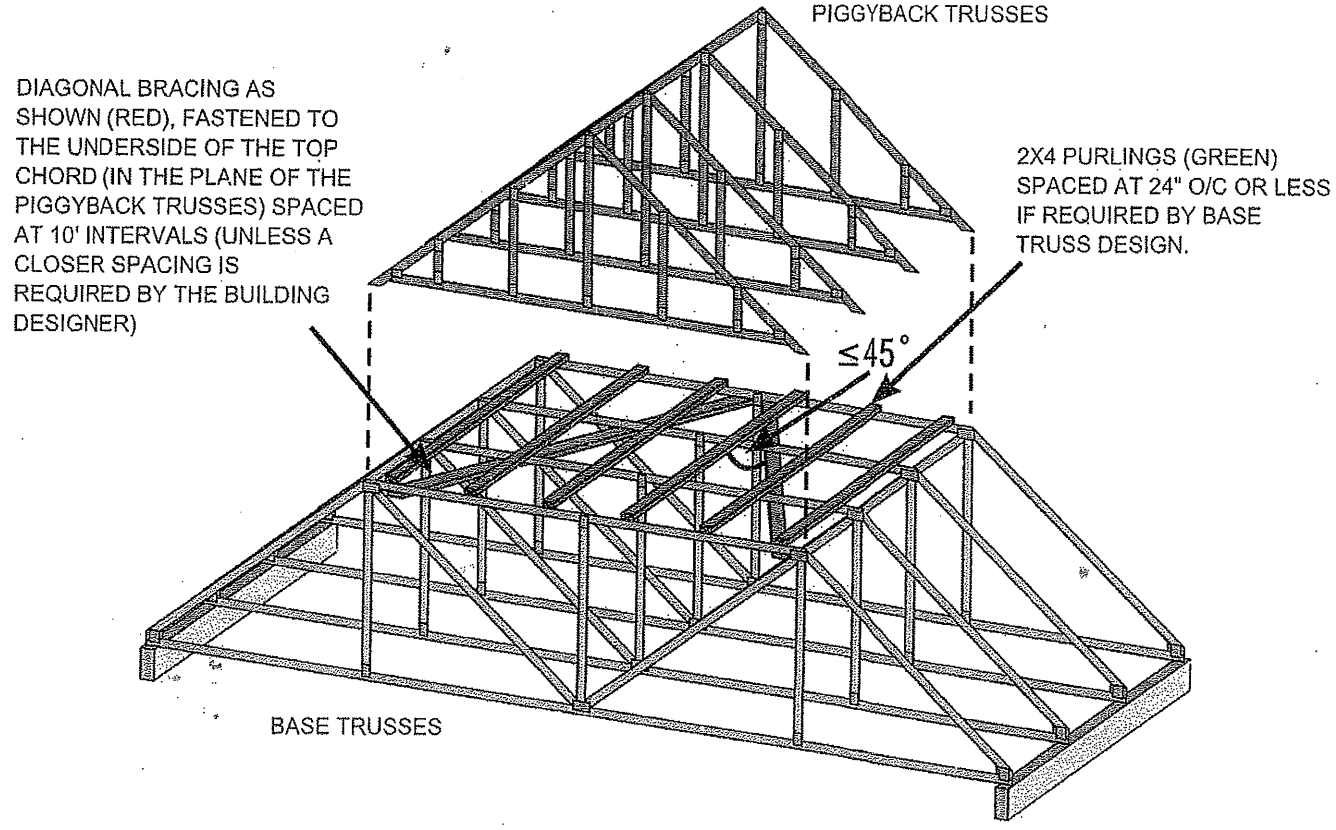
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

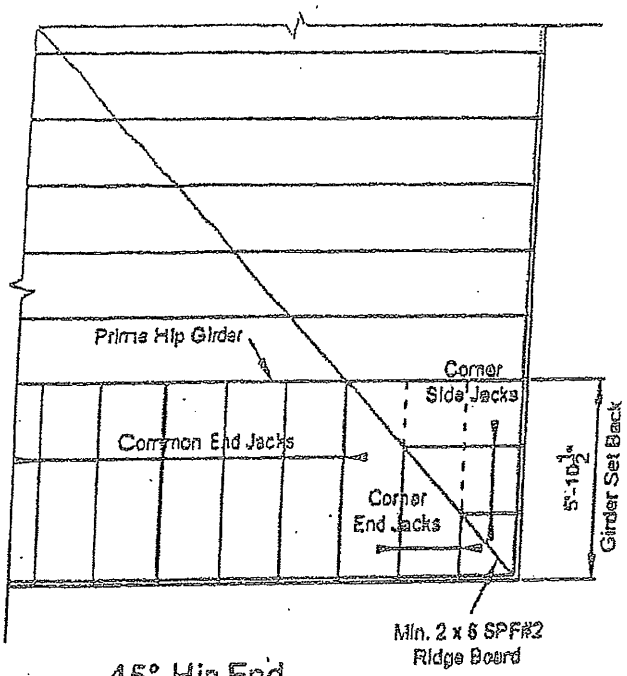
Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

MICRO CITY
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

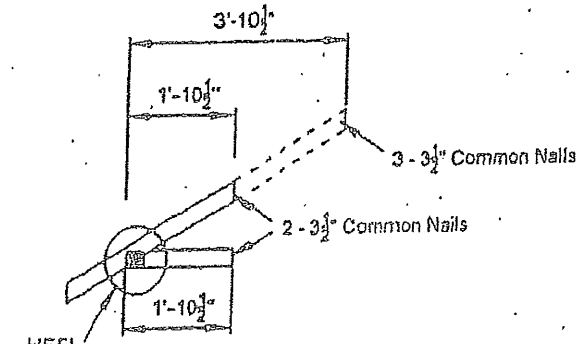
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

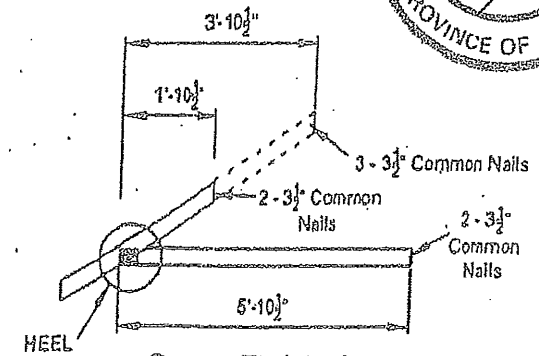
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14
STRUCTURAL COMPONENT ONLY

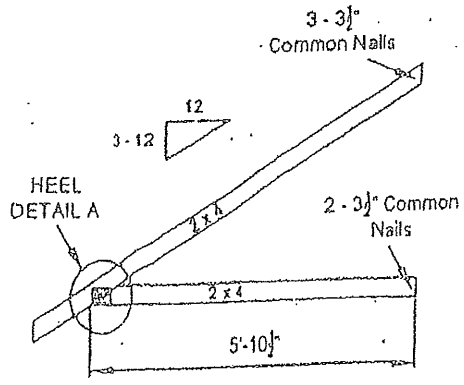
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
1-2-14
PROVINCE OF ONTARIO



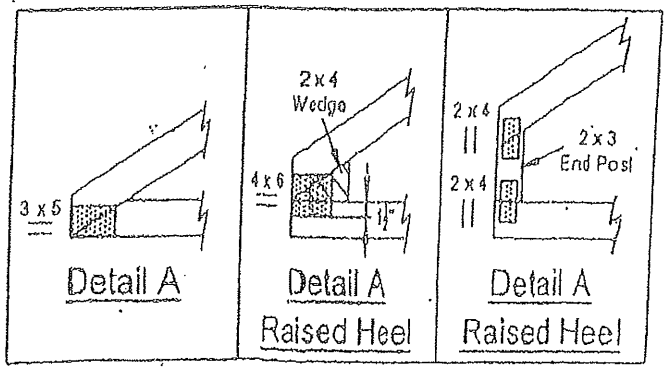
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

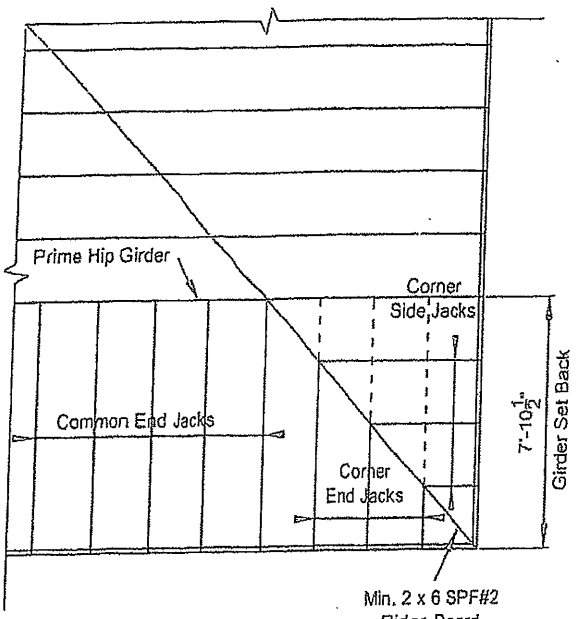
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



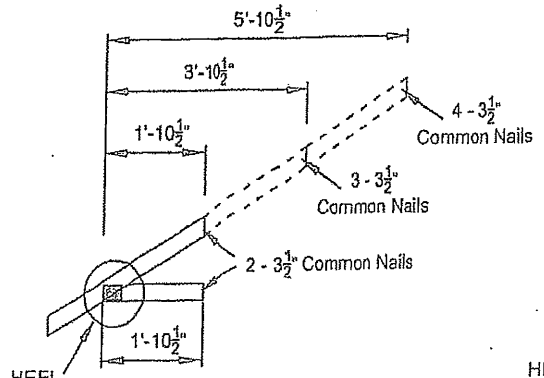
45° Hip End

DESIGN LOAD:

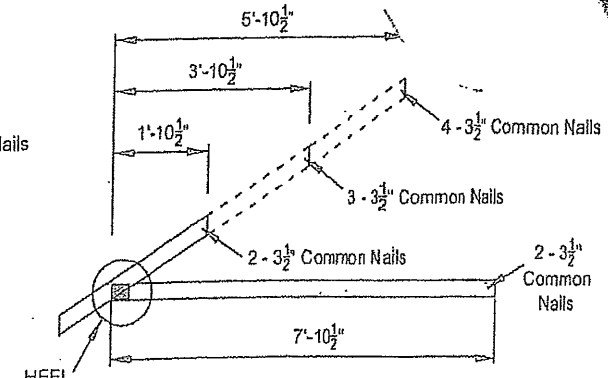
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3503.14
STRUCTURAL COMPONENT ONLY

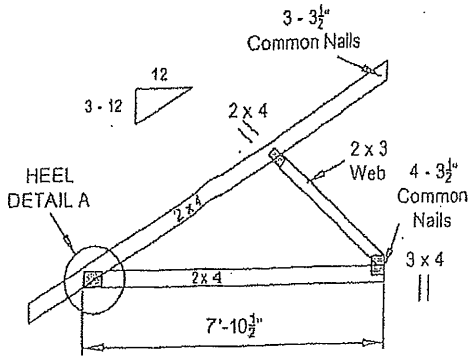
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



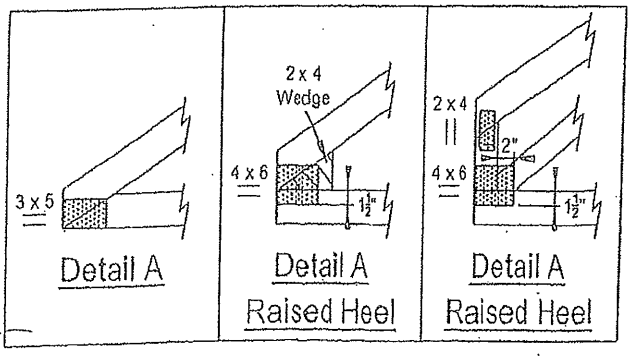
Corner Side Jacks



Corner End Jacks



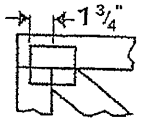
Common End Jacks



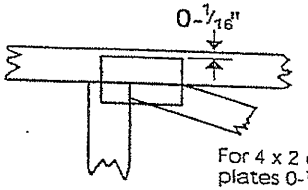
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

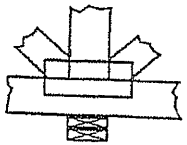
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

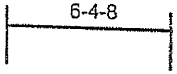


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

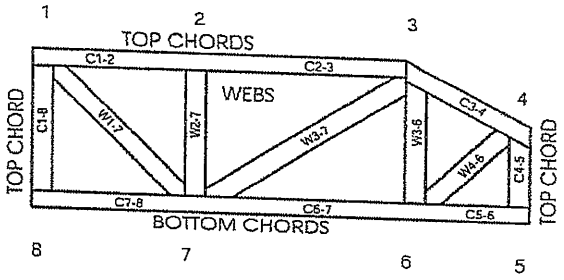
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS
· CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®
POWER TO PERFORM.™

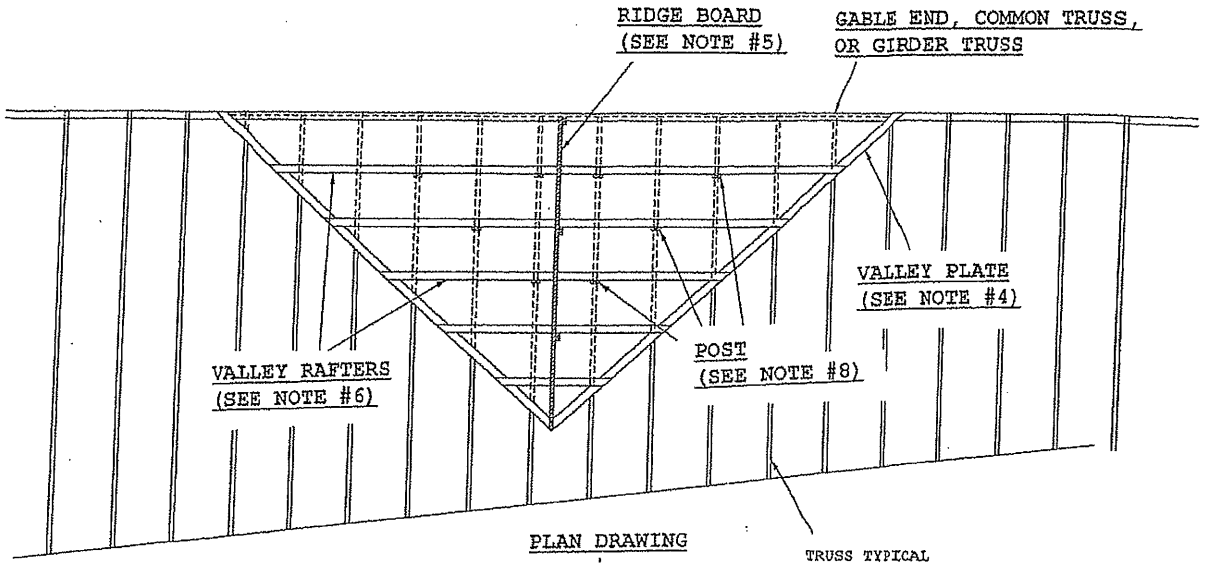
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

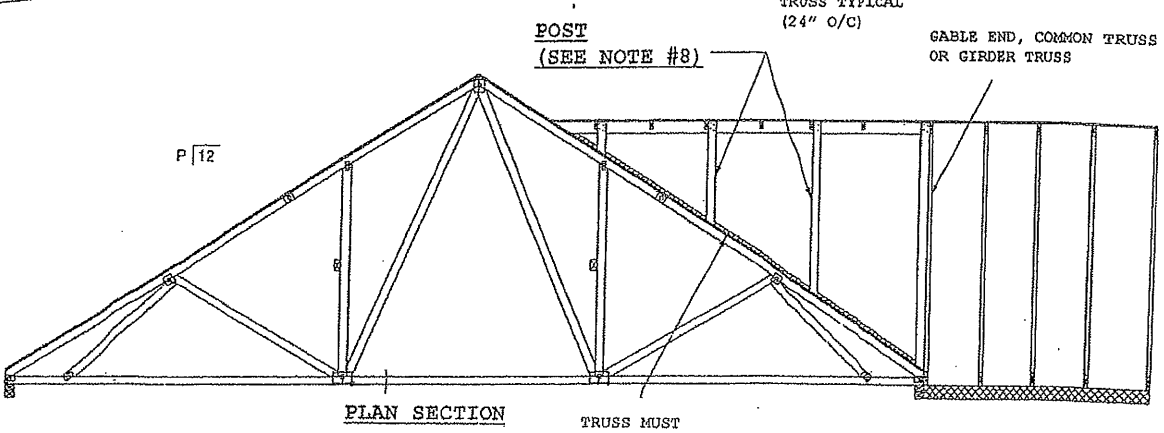
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

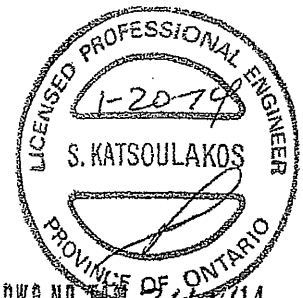
(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



DWG NO [illegible] 14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.