ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

CP3-6 -(CP)
THGQ3-SDS4.5 (MAX)- (S)
HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(0)

DENOTES:

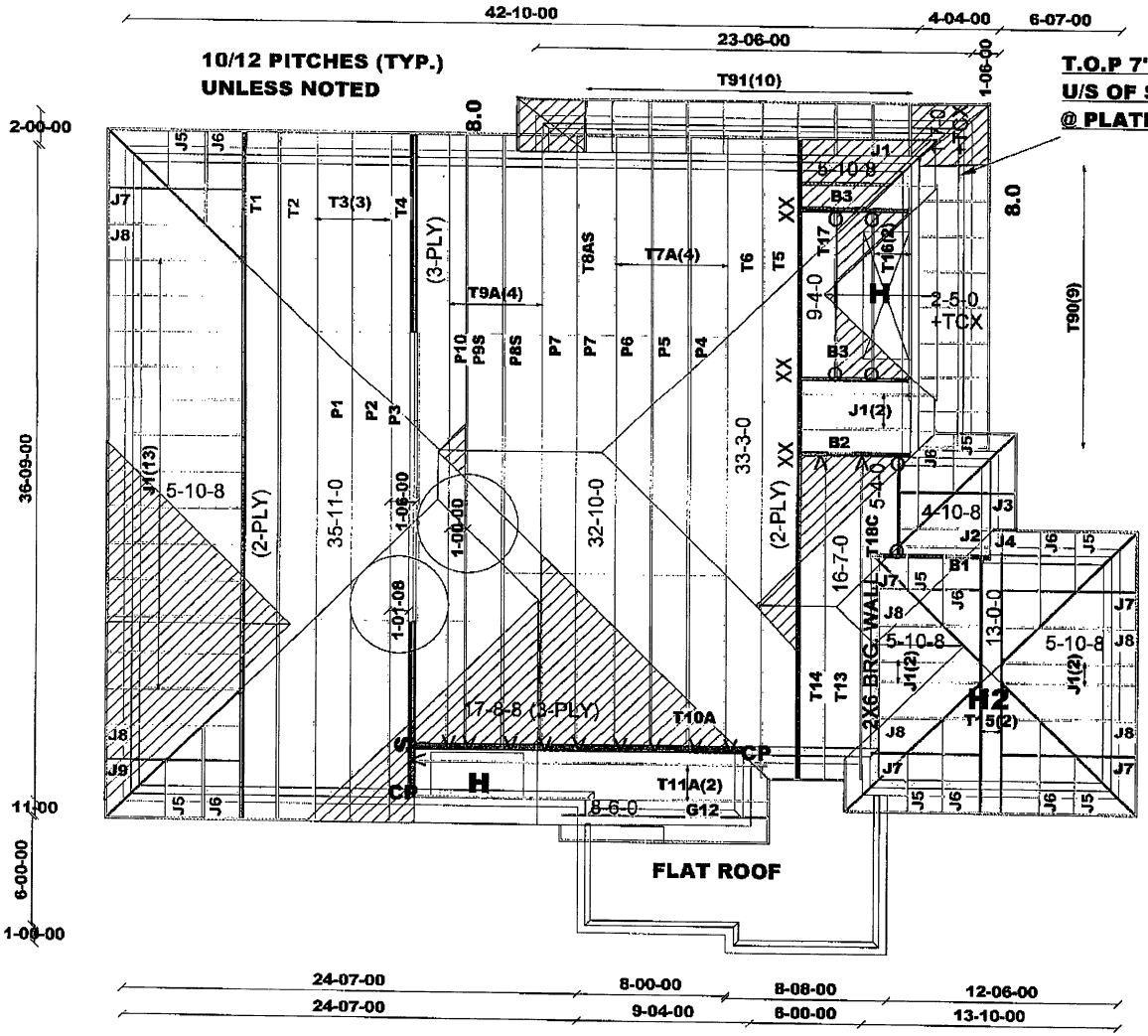
H2 -12" HIGHER PLATE

H -12" RAISED CEILING

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

MII, 047

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track:	42067
Layout ID:	288459
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 9/25/2017 Designer: sonny

Model / Elevation: S45-4C HUMMINGBIRD 4/ A

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

CP3-6 -(CP)
THGQ3-SDS4.5 (MAX)- (S)
HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(0)

DENOTES:

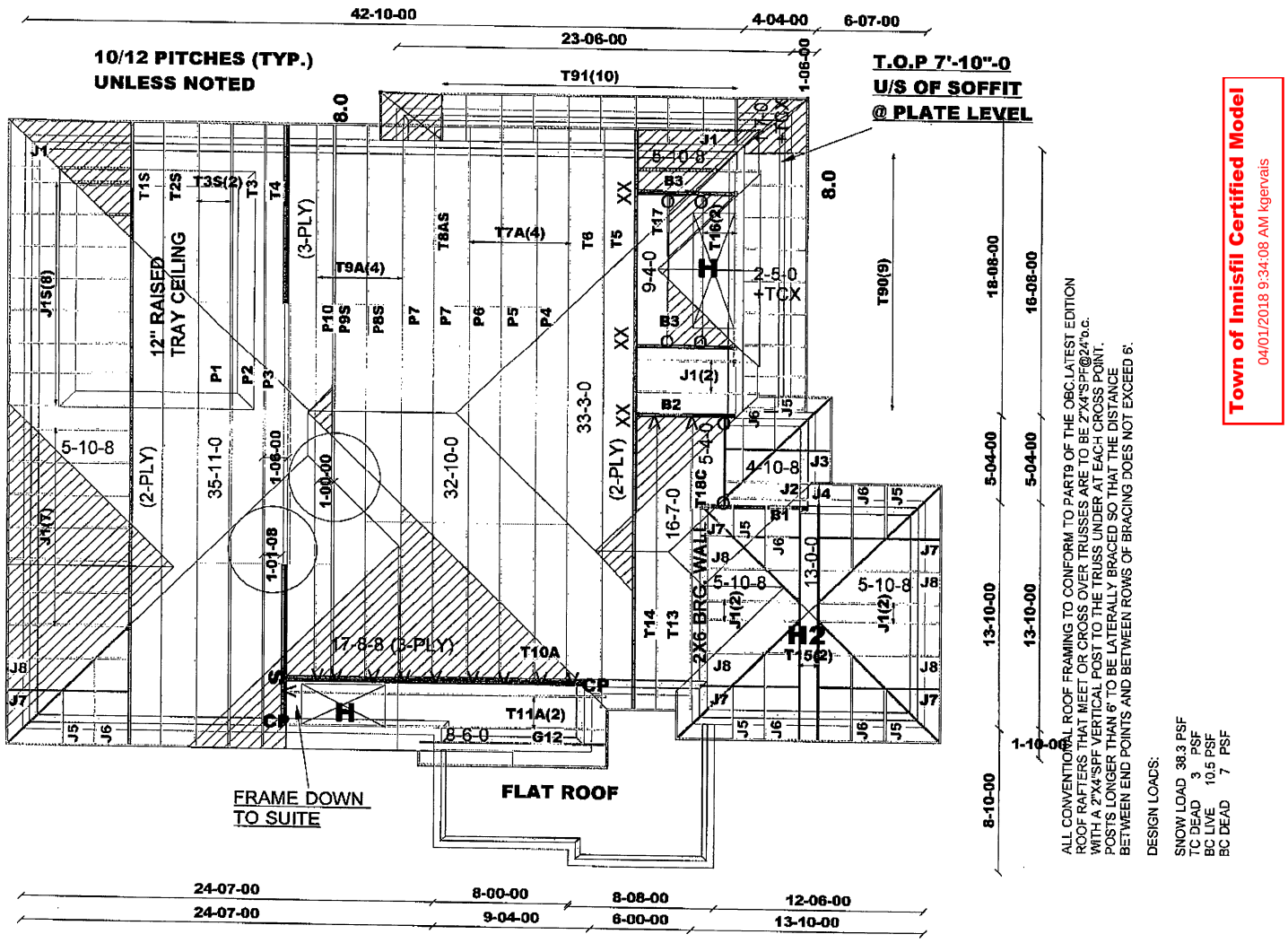
H2 -12" HIGHER PLATE

H -12" RAISED CEILING

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

M11,047

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track:	42067
Layout ID:	288458
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 9/25/2017 Designer: sonny

Model / Elevation: S45-4C HUMMINGBIRD 4/ A+OPT. COFF.

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(0)

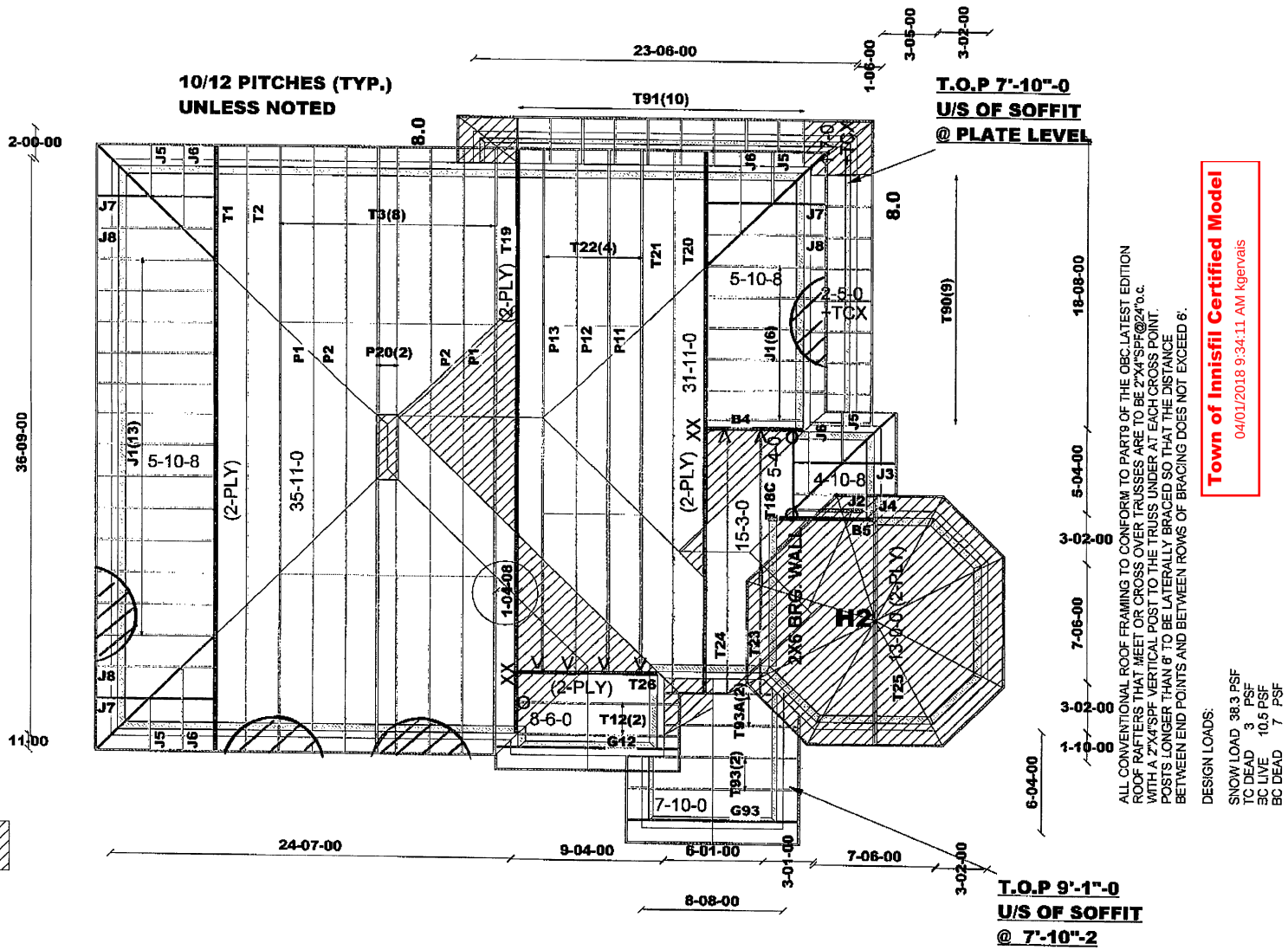
DENOTES:

H2 -12" HIGHER PLATE

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC,LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 8' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

m11,047

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S45-4C HUMMINGBIRD 4/ B
	Layout ID: 288461	Project: ALCONA SHORES	
	Plan Log: 94324	Date: 9/26/2017 Designer: sonny	

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

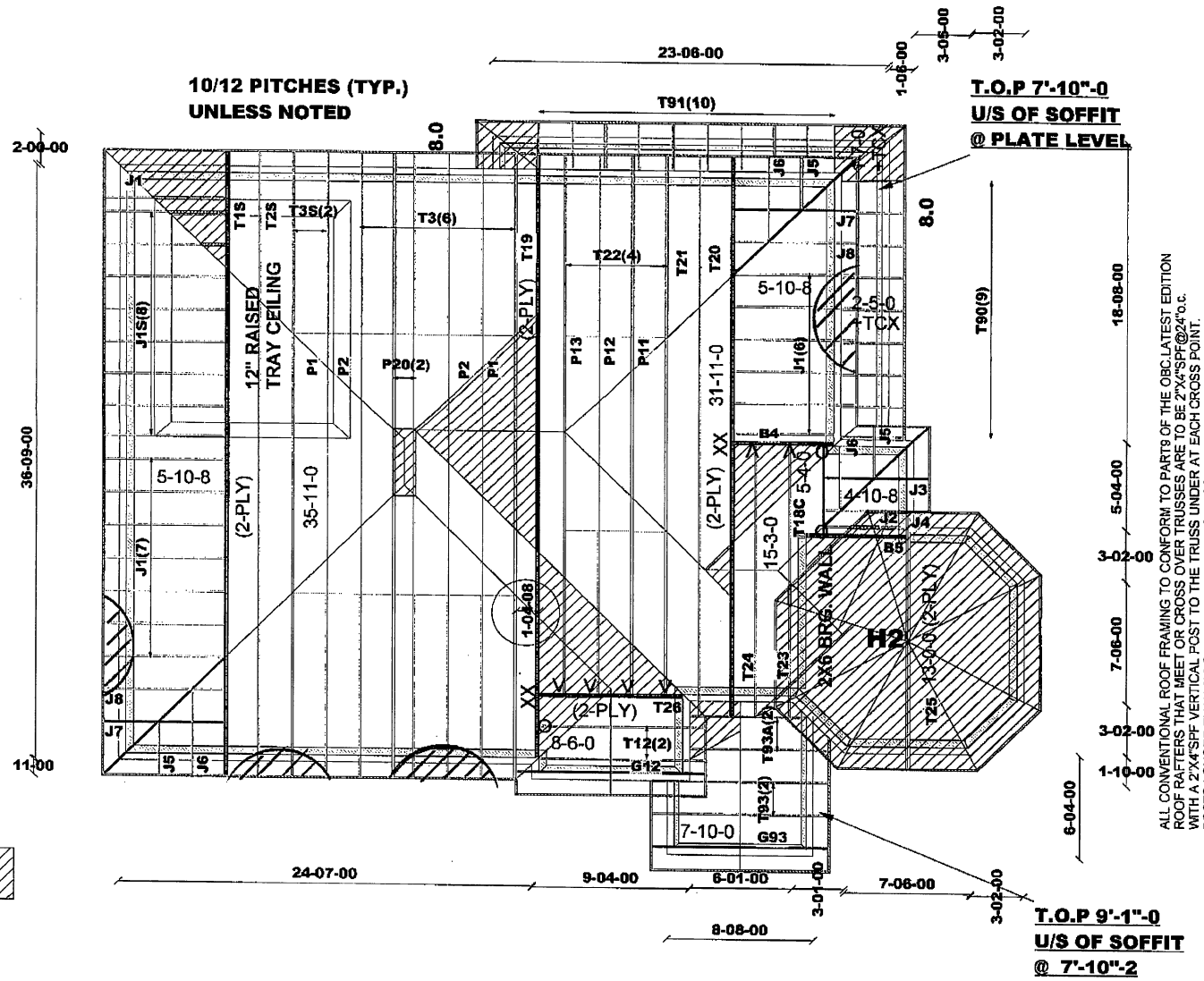
HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(0)

DENOTES:

H2 -12" HIGHER PLATE

ALL B- 2-2X10 (FLUSH)



T-170681

DENOTES:
CONVENTIONAL
FRAMING

M11,047

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 42067
Layout ID: 288460
Plan Log: 94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 9/26/2017 Designer: sonny

Model / Elevation: S45-4C HUMMINGBIRD 4/ B+OPT. COFF.

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/25/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288459 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	35-11-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	453.90 266.66		
	1	T2 HIP	10.00 0.00	35-11-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	173.43 108.00		
	3	T3 PIGGYBACK	10.00 0.00	35-11-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	576.45 360.99		
	1 3 Ply	T4 PIGGYBACK	10.00 0.00	35-11-00	09-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	769.89 459.00		
	1 2 Ply	T5 HIP GIRDER	10.00 0.00	33-03-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	419.18 246.66		
	1	T6 HIP	10.00 0.00	33-03-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	156.78 99.00		
	4	T7A HIP	10.00 0.00	32-10-00	09-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-11-14	661.64 416.00		
	1	T8AS PIGGYBACK	10.00 0.00	32-10-00	09-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-02-08	191.33 120.17		
	4	T9A PIGGYBACK	10.00 0.00	32-10-00	09-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-02-08	727.56 456.68		
	1 3 Ply	T10A COMMON	10.00 0.00	17-08-08	10-10-08	2 X 6	2 X 6	00-00-00 00-00-00	01-07-11 05-04-04	402.03 242.01		
	2	T11A ROOF	10.00 0.00	17-08-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-04-04	203.40 129.66		
	1	G12 COMMON	10.00 0.00	08-06-00	05-02-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	41.53 27.83		
	1	T13 HIP	10.00 0.00	16-07-00	07-04-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	83.14 52.67		
	1	T14 COMMON	10.00 0.00	16-07-00	08-06-10	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	76.46 47.33		
	2	T15 HIP GIRDER	10.00 0.00	13-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	149.84 97.34		
	2	T16 ROOF	10.00 0.00	09-04-00	05-06-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	95.50 66.00		
	1	T17 HALF HIP	10.00 0.00	09-04-00	04-11-11	2 X 4	2 X 4	00-00-00 00-00-00	03-11-03 04-11-11	47.07 30.50		
	1	T18C HALF HIP	10.00 0.00	05-04-00	05-08-07	2 X 6	2 X 4	00-00-00 00-00-00	01-07-11 05-05-14	29.78 19.17		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

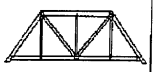
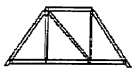
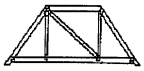
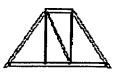
Delivery Shiplist

DATE	09/25/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288459 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	9	T90 MONOPITCH	8.00 0.00	02-05-00	03-06-05	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-05	159.84 106.47		
	10	T91 MONOPITCH	10.00 0.00	01-07-00	03-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-04	165.30 128.30		
	1	P1 PIGGYBACK	10.00 0.00	14-10-09	01-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	48.62 30.17		
	1	P2 PIGGYBACK	10.00 0.00	14-10-09	03-02-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	52.02 33.50		
	1 3 Ply	P3 PIGGYBACK	10.00 0.00	14-10-09	04-01-10	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	150.15 92.49		
	1	P4 PIGGYBACK	10.00 0.00	12-02-09	01-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	40.40 25.83		
	1	P5 PIGGYBACK	10.00 0.00	12-02-09	03-02-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	40.99 26.67		
	1	P6 PIGGYBACK	10.00 0.00	12-02-09	04-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	44.02 28.17		
	2	P7 PIGGYBACK	10.00 0.00	12-02-09	05-05-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	87.92 54.34		
	1	P8S PIGGYBACK	10.00 0.00	14-10-09	05-05-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	53.67 34.67		
	1	P9S PIGGYBACK	10.00 0.00	14-10-09	05-05-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	57.26 37.00		
	1	P10 PIGGYBACK	10.00 0.00	14-10-09	04-11-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	51.26 31.33		
	20	J1 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	380.00 240.00		
	1	J2 JACK-OPEN	10.00 0.00	04-10-08	05-08-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-05-13	16.37 10.67		
	1	J3 JACK-OPEN	10.00 0.00	04-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -03-01-01	01-07-11 00-03-08	15.65 10.50		
	1	J4 JACK-OPEN	10.00 0.00	04-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -01-01-01	01-07-11 00-03-08	18.62 11.83		
	7	J5 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	69.93 49.00		
	7	J6 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 01-10-15	01-07-11 00-05-13	90.72 58.31		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/25/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288459 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOR SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	J7 JACK-OPEN	10.00 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -04-01-01	01-07-11 00-03-08	64.65 43.35		
	6	J8 JACK-OPEN	10.00 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	123.30 78.00		
	1	J9 JACK-OPEN	10.00 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -04-01-01	01-07-11 00-03-08	17.58 11.67		

TOTAL # TRUSS= **116.00** TOTAL BFT OF ALL TRUSSES= **4387.94** BFT. TOTAL WEIGHT OF ALL TRUSSES= **7007.18** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Crush Plates	CP3-6	
1	Hangers	THGQ3 SDS4.5 MAX	
3	Hangers	HGUS26-2	
11	Hangers	LJS26DS	
6	Hangers	LUS24	

TOTAL # ITEMS= **23.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

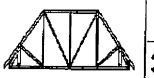
Delivery Shiplist

DATE	09/25/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288458 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: A +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S HIP GIRDER	10.00 12.00	35-11-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	484.62 292.66		
	1	T2S HIP	10.00 12.00	35-11-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	179.25 114.33		
	1	T3 PIGGYBACK	10.00 0.00	35-11-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	192.15 120.33		
	2	T3S HIP	10.00 12.00	35-11-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	399.64 255.32		
	1 3 Ply	T4 PIGGYBACK	10.00 0.00	35-11-00	09-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	769.89 459.00		
	1 2 Ply	T5 HIP GIRDER	10.00 0.00	33-03-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	419.18 246.66		
	1	T6 HIP	10.00 0.00	33-03-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	156.78 99.00		
	4	T7A HIP	10.00 0.00	32-10-00	09-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-11-14	661.64 416.00		
	1	T8AS PIGGYBACK	10.00 0.00	32-10-00	09-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-02-08	191.33 120.17		
	4	T9A PIGGYBACK	10.00 0.00	32-10-00	09-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-02-08	727.56 456.68		
	1 3 Ply	T10A COMMON	10.00 0.00	17-08-08	10-10-08	2 X 6	2 X 6	00-00-00 00-00-00	01-07-11 05-04-04	402.03 242.01		
	2	T11A ROOF	10.00 0.00	17-08-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-04-04	203.40 129.66		
	1	G12 COMMON	10.00 0.00	08-06-00	05-02-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	41.53 27.83		
	1	T13 HIP	10.00 0.00	16-07-00	07-04-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	83.14 52.67		
	1	T14 COMMON	10.00 0.00	16-07-00	08-06-10	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	76.46 48.00		
	2	T15 HIP GIRDER	10.00 0.00	13-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	149.84 97.34		
	2	T16 ROOF	10.00 0.00	09-04-00	05-06-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	95.50 66.00		
	1	T17 HALF HIP	10.00 0.00	09-04-00	04-11-11	2 X 4	2 X 4	00-00-00 00-00-00	03-11-03 04-11-11	47.07 30.50		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

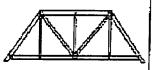
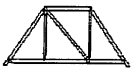
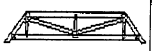
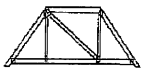
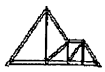
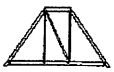
Delivery Shiplist

DATE	09/25/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288458 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: A +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T18C HALF HIP	10.00 0.00	05-04-00	05-08-07	2 X 6	2 X 4	00-00-00 00-00-00	01-07-11 05-05-14	29.78 19.17		
	9	T90 MONOPITCH	8.00 0.00	02-05-00	03-06-05	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-05	159.84 106.47		
	10	T91 MONOPITCH	10.00 0.00	01-07-00	03-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-04	165.30 128.30		
	1	P1 PIGGYBACK	10.00 0.00	14-10-09	01-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	48.62 30.17		
	1	P2 PIGGYBACK	10.00 0.00	14-10-09	03-02-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	52.02 33.50		
	1 3 Ply	P3 PIGGYBACK	10.00 0.00	14-10-09	04-01-10	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	150.15 92.49		
	1	P4 PIGGYBACK	10.00 0.00	12-02-09	01-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	40.40 25.83		
	1	P5 PIGGYBACK	10.00 0.00	12-02-09	03-02-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	40.99 26.67		
	1	P6 PIGGYBACK	10.00 0.00	12-02-09	04-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	44.02 28.17		
	2	P7 PIGGYBACK	10.00 0.00	12-02-09	05-05-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	87.92 54.34		
	1	P8S PIGGYBACK	10.00 0.00	14-10-09	05-05-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	53.67 34.67		
	1	P9S PIGGYBACK	10.00 0.00	14-10-09	05-05-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	57.26 37.00		
	1	P10 PIGGYBACK	10.00 0.00	14-10-09	04-11-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	51.26 31.33		
	15	J1 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	285.00 180.00		
	8	J1S JACK-OPEN	10.00 12.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-06-07	237.36 162.64		
	1	J2 JACK-OPEN	10.00 0.00	04-10-08	05-08-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-05-13	16.37 10.67		
	1	J3 JACK-OPEN	10.00 0.00	04-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -03-01-01	01-07-11 00-03-08	15.65 10.50		
	1	J4 JACK-OPEN	10.00 0.00	04-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -01-01-01	01-07-11 00-03-08	18.62 11.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/25/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288458 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: A +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	6	J5 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	59.94 42.00		
	6	J6 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 01-10-15	01-07-11 00-05-13	77.76 49.98		
	5	J7 JACK-OPEN	10.00 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -04-01-01	01-07-11 00-03-08	87.90 58.35		
	5	J8 JACK-OPEN	10.00 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	102.75 65.00		

TOTAL # TRUSS= **115** TOTAL BFT OF ALL TRUSSES= **4513.24** BFT. TOTAL WEIGHT OF ALL TRUSSES= **7163.59** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Crush Plates	CP3-6	
3	Hangers	HGUS26-2	
11	Hangers	LJS26DS	
6	Hangers	LUS24	
1	Hangers	THGQ3 SDS4.5 MAX	

TOTAL # ITEMS= **23**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/26/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288461 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	35-11-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	453.90 266.66		
	1	T2 HIP	10.00 0.00	35-11-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	173.43 108.00		
	8	T3 PIGGYBACK	10.00 0.00	35-11-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1537.20 962.64		
	1 2 Ply	T19 HIP GIRDER	10.00 0.00	35-11-00	08-08-11	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	512.26 304.66		
	1 2 Ply	T20 HIP GIRDER	10.00 0.00	31-11-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	419.06 250.66		
	1	T21 HIP	10.00 0.00	31-11-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	153.11 97.00		
	4	T22 HIP	10.00 0.00	31-11-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	656.56 413.32		
	1	T23 HIP	10.00 0.00	15-03-00	07-04-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	79.83 50.83		
	1	T24 COMMON	10.00 0.00	15-03-00	07-11-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	71.11 45.33		
	1 2 Ply	T25 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	117.26 75.34		
	1 2 Ply	T26 HALF HIP	10.00 0.00	08-06-00	04-09-03	2 X 6	2 X 8	00-00-00 00-00-00	01-07-11 04-09-03	116.34 74.66		
	1	T18C HALF HIP	10.00 0.00	05-04-00	05-08-07	2 X 6	2 X 4	00-00-00 00-00-00	01-07-11 05-05-14	29.78 19.17		
	2	T12 COMMON	10.00 0.00	08-06-00	05-02-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	79.46 53.66		
	1	G12 COMMON	10.00 0.00	08-06-00	05-02-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	41.53 27.83		
	9	T90 MONOPITCH	8.00 0.00	02-05-00	03-06-05	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-05	159.84 106.47		
	10	T91 MONOPITCH	10.00 0.00	01-07-00	03-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-04	165.30 128.30		
	2	T93 COMMON	10.00 0.00	07-10-00	03-08-00	2 X 4	2 X 4	01-03-08 01-03-08	00-04-13 00-04-13	54.76 35.00		
	2	T93A COMMON	10.00 0.00	06-11-00	03-08-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-13 00-03-08	56.00 36.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

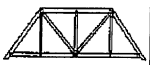
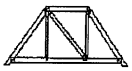
Delivery Shiplist

DATE	09/26/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288461 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G93 COMMON	10.00 / 0.00	07-10-00	03-08-00	2 X 4	2 X 4	01-03-08 / 01-03-08	00-04-13 / 00-04-13	29.68 / 19.50		
	2	P1 PIGGYBACK	10.00 / 0.00	14-10-09	01-06-06	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	97.24 / 60.34		
	2	P2 PIGGYBACK	10.00 / 0.00	14-10-09	03-02-06	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	104.04 / 67.00		
	2	P20 PIGGYBACK	10.00 / 0.00	14-10-09	04-10-06	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	102.30 / 64.00		
	1	P11 PIGGYBACK	10.00 / 0.00	10-10-09	01-06-06	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	36.30 / 22.83		
	1	P12 PIGGYBACK	10.00 / 0.00	10-10-09	03-02-06	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	37.02 / 24.17		
	1	P13 PIGGYBACK	10.00 / 0.00	10-10-09	04-11-03	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	33.93 / 20.50		
	19	J1 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	361.00 / 228.00		
	1	J2 JACK-OPEN	10.00 / 0.00	04-10-08	05-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-08-07	16.37 / 10.67		
	1	J3 JACK-OPEN	10.00 / 0.00	04-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -03-01-01	01-07-11 / 00-03-08	15.65 / 10.50		
	1	J4 JACK-OPEN	10.00 / 0.00	04-10-08	04-09-09	2 X 4	2 X 4	01-05-00 / -01-01-01	01-07-11 / 00-03-08	18.80 / 12.50		
	4	J5 JACK-OPEN	10.00 / 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -00-01-01	01-07-11 / 00-03-08	39.96 / 28.00		
	4	J6 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / 01-10-15	01-07-11 / 00-05-13	51.84 / 33.32		
	3	J7 JACK-OPEN	10.00 / 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -04-01-01	01-07-11 / 00-03-08	52.74 / 35.01		
	3	J8 JACK-OPEN	10.00 / 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / -02-01-01	01-07-11 / 00-03-08	61.65 / 39.00		

TOTAL # TRUSS= **99** TOTAL BFT OF ALL TRUSSES= **3731.53** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5935.25** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/26/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288461 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOWS **SUB-BUILDER:**
MODEL: S45-4C HUMMINGBIRD 4 **ELEVATION:** B

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	

TOTAL # ITEMS= **11**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

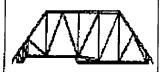
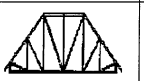
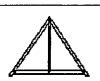
Delivery Shiplist

DATE	09/26/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288460 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: B +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S HIP GIRDER	10.00 12.00	35-11-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	484.62 292.66		
	1	T2S HIP	10.00 12.00	35-11-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	179.25 114.33		
	6	T3 PIGGYBACK	10.00 0.00	35-11-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1152.90 721.98		
	2	T3S HIP	10.00 12.00	35-11-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	399.64 255.32		
	1 2 Ply	T19 HIP GIRDER	10.00 0.00	35-11-00	08-08-11	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	512.26 304.66		
	1 2 Ply	T20 HIP GIRDER	10.00 0.00	31-11-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	419.06 250.66		
	1	T21 HIP	10.00 0.00	31-11-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	153.11 97.00		
	4	T22 HIP	10.00 0.00	31-11-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	656.56 413.32		
	1	T23 HIP	10.00 0.00	15-03-00	07-04-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	79.83 50.83		
	1	T24 COMMON	10.00 0.00	15-03-00	07-11-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	71.11 45.33		
	1 2 Ply	T25 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	117.26 75.34		
	1 2 Ply	T26 HALF HIP	10.00 0.00	08-06-00	04-09-03	2 X 6	2 X 8	00-00-00 00-00-00	01-07-11 04-09-03	116.34 74.66		
	1	P13 PIGGYBACK	10.00 0.00	10-10-09	04-11-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	33.93 20.50		
	1	G12 COMMON	10.00 0.00	08-06-00	05-02-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	41.53 27.83		
	2	T12 COMMON	10.00 0.00	08-06-00	05-02-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	79.46 53.66		
	1	T18C HALF HIP	10.00 0.00	05-04-00	05-08-07	2 X 6	2 X 4	00-00-00 00-00-00	01-07-11 05-05-14	29.78 19.17		
	9	T90 MONOPITCH	8.00 0.00	02-05-00	03-06-05	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-05	159.84 106.47		
	10	T91 MONOPITCH	10.00 0.00	01-07-00	03-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-04	165.30 128.30		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

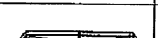
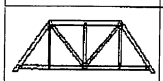
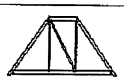
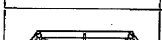
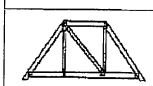
Delivery Shiplist

DATE	09/26/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288460 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-4C HUMMINGBIRD 4 ELEVATION: B +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T93 COMMON	10.00 0.00	07-10-00	03-08-00	2 X 4	2 X 4	01-03-08 01-03-08	00-04-13 00-04-13	54.76 35.00		
	2	T93A COMMON	10.00 0.00	06-11-00	03-08-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-13 00-03-08	56.00 36.66		
	1	G93 COMMON	10.00 0.00	07-10-00	03-08-00	2 X 4	2 X 4	01-03-08 01-03-08	00-04-13 00-04-13	29.68 19.50		
	14	J1 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	266.00 168.00		
	8	J1S JACK-OPEN	10.00 12.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-06-07	237.36 162.64		
	1	J2 JACK-OPEN	10.00 0.00	04-10-08	05-08-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-08-07	16.37 10.67		
	1	J3 JACK-OPEN	10.00 0.00	04-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -03-01-01	01-07-11 00-03-08	15.65 10.50		
	1	J4 JACK-OPEN	10.00 0.00	04-10-08	04-09-09	2 X 4	2 X 4	01-05-00 -01-01-01	01-07-11 00-03-08	18.80 12.50		
	3	J5 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	29.97 21.00		
	3	J6 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 01-10-15	01-07-11 00-05-13	38.88 24.99		
	2	J7 JACK-OPEN	10.00 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -04-01-01	01-07-11 00-03-08	35.16 23.34		
	2	J8 JACK-OPEN	10.00 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	41.10 26.00		
	2	P1 PIGGYBACK	10.00 0.00	14-10-09	01-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	97.24 60.34		
	2	P2 PIGGYBACK	10.00 0.00	14-10-09	03-02-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	104.04 67.00		
	2	P20 PIGGYBACK	10.00 0.00	14-10-09	04-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	102.30 64.00		
	1	P11 PIGGYBACK	10.00 0.00	10-10-09	01-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	36.30 22.83		
	1	P12 PIGGYBACK	10.00 0.00	10-10-09	03-02-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	37.02 24.17		

TOTAL # TRUSS= **98** TOTAL BFT OF ALL TRUSSES= **3841.16** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6068.41** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/26/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288460 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES **SUB-BUILDER:**
MODEL: S45-4C HUMMINGBIRD 4 **ELEVATION:** B +OPT. COFF

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	
3	Hangers	LUS24	

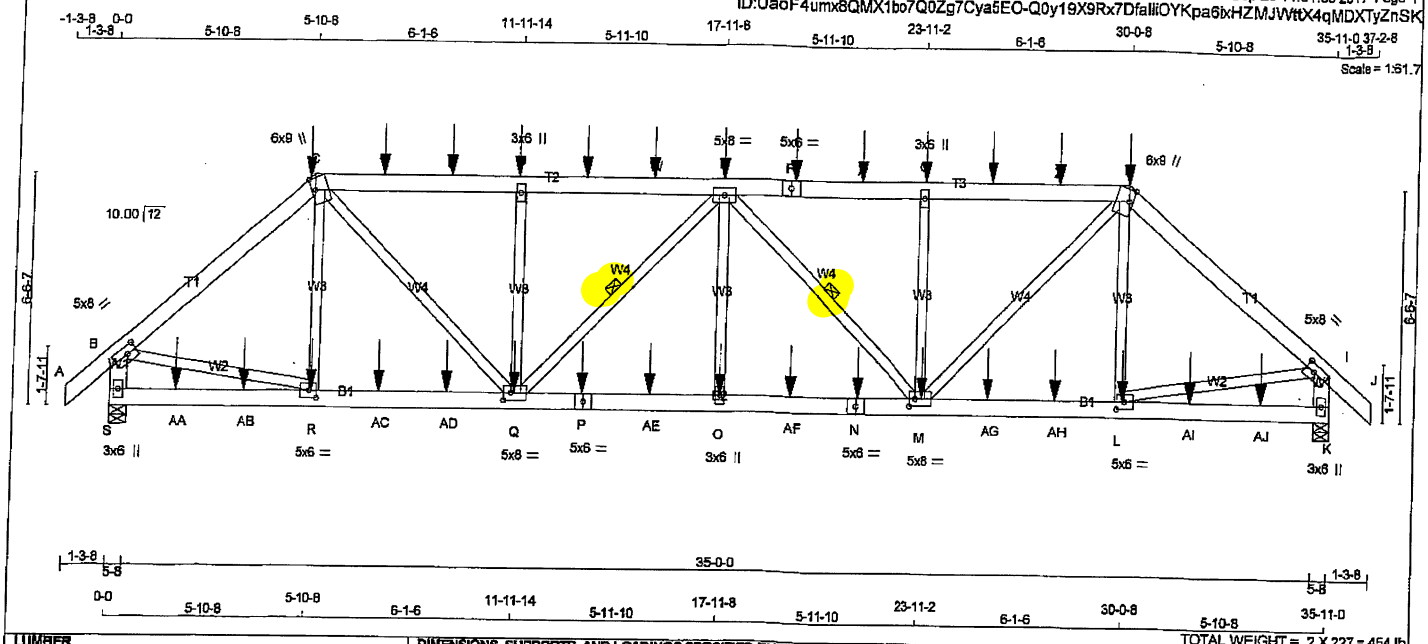
TOTAL # ITEMS= 11

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288459	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 11:51:05 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-Q0y19X9Rx7DfalliOYKpa6ixHZMJWttX4qMDXTyZnSK



TOTAL WEIGHT = 2 X 227 = 454 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	SIDE(122.0)
C-F	2	12	SIDE(183.1)
F-H	2	12	SIDE(183.1)
H-J	2	12	SIDE(122.0)
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(183.1)
P-N	2	12	SIDE(183.1)
N-K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.00	1.25
I	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWWW-t	MT20	5.0	8.0	2.50	2.00
N	BS-t	MT20	5.0	6.0		
O	BMW+w	MT20	3.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0	2.50	2.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	4748	0	4748	0	0	5-8	5-8
K	4755	0	4755	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	3713	2417 / 0	664 / 0	0 / 0	0 / 0	632 / 0	0 / 0
K	3721	2418 / 0	668 / 0	0 / 0	0 / 0	635 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.05 (1)	10.00
B- C	-5239 / 0	-122.2	-122.2	0.26 (1)	4.88
C- T	-6287 / 0	-122.2	-122.2	0.32 (1)	4.45
T- U	-6287 / 0	-122.2	-122.2	0.32 (1)	4.45
U- D	-6287 / 0	-122.2	-122.2	0.32 (1)	4.45
D- V	-6287 / 0	-122.2	-122.2	0.32 (1)	4.45
V- W	-6287 / 0	-122.2	-122.2	0.32 (1)	4.45
W- E	-6287 / 0	-122.2	-122.2	0.32 (1)	4.45
E- F	-6285 / 0	-122.2	-122.2	0.32 (1)	4.45
F- X	-6285 / 0	-122.2	-122.2	0.32 (1)	4.45
X- G	-6285 / 0	-122.2	-122.2	0.32 (1)	4.45
G- Y	-6285 / 0	-122.2	-122.2	0.32 (1)	4.45
Y- Z	-6285 / 0	-122.2	-122.2	0.32 (1)	4.45
Z- H	-6285 / 0	-122.2	-122.2	0.32 (1)	4.45
H- I	-5241 / 0	-122.2	-122.2	0.26 (1)	4.87
I- J	0 / 56	-122.2	-122.2	0.05 (1)	10.00
S- B	-4645 / 0	0.0	0.0	0.17 (1)	6.70
K- I	-4646 / 0	0.0	0.0	0.17 (1)	6.70
S-AA	0 / 0	-28.0	-28.0	0.11 (3)	10.00
AA-AB	0 / 0	-28.0	-28.0	0.11 (3)	10.00
AB- R	0 / 0	-28.0	-28.0	0.11 (3)	10.00
R-AC	0 / 4004	-28.0	-28.0	0.32 (1)	10.00
AC-AD	0 / 4004	-28.0	-28.0	0.32 (1)	10.00
AD- Q	0 / 4004	-28.0	-28.0	0.32 (1)	10.00
Q- P	0 / 6977	-28.0	-28.0	0.50 (1)	10.00
P-AE	0 / 6977	-28.0	-28.0	0.50 (1)	10.00
AE- O	0 / 6977	-28.0	-28.0	0.50 (1)	10.00
O-AF	0 / 6977	-28.0	-28.0	0.50 (1)	10.00
AF- N	0 / 6977	-28.0	-28.0	0.50 (1)	10.00
N- M	0 / 6977	-28.0	-28.0	0.50 (1)	10.00
M-AG	0 / 4005	-28.0	-28.0	0.32 (1)	10.00
AG-AH	0 / 4005	-28.0	-28.0	0.32 (1)	10.00
AH- L	0 / 4005	-28.0	-28.0	0.32 (1)	10.00
L-AI	0 / 0	-28.0	-28.0	0.11 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-398 / 309	0.09 (1)
C- Q	0 / 3211	0.28 (1)
Q- D	-1285 / 0	0.29 (1)
Q- E	-984 / 0	0.17 (1)
O- E	0 / 519	0.05 (2)
E- M	-986 / 0	0.17 (1)
M- G	-1281 / 0	0.29 (1)
M- H	0 / 3208	0.28 (1)
L- H	-394 / 314	0.09 (1)
B- R	0 / 4081	0.36 (1)
L- I	0 / 4083	0.36 (1)

LICENSED PROFESSIONAL ENGINEER 92617 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM47939-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.32 (C-D:1) , BC=0.50 (M-O:1) , WB=0.36 (I-L:1) , SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.89 (M) (INPUT = 0.90)
USI METAL= 0.57 (P) (INPUT = 1.00)

Po 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288459	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-Q0y19X9Rx7DfalliOYKpa6bxHZMJWtfX4qMDXTyZnSK

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 699.4 lbs FACTORED DOWN AT 5-10-8, 147.1 lbs FACTORED DOWN AT 7-11-4, 147.1 lbs FACTORED DOWN AT 9-11-4, 147.1 lbs FACTORED DOWN AT 11-11-4, 147.1 lbs FACTORED DOWN AT 13-11-4, 147.1 lbs FACTORED DOWN AT 15-11-4, 147.1 lbs FACTORED DOWN AT 17-11-8, 147.1 lbs FACTORED DOWN AT 19-11-12, 147.1 lbs FACTORED DOWN AT 21-11-12, 147.1 lbs FACTORED DOWN AT 23-11-12, 147.1 lbs FACTORED DOWN AT 25-11-12, AND 147.1 lbs FACTORED DOWN AT 27-11-12, AND 699.4 lbs FACTORED DOWN AT 30-0-8 ON TOP CHORD, AND 77.4 lbs FACTORED DOWN AT 1-11-4, 88.9 lbs FACTORED DOWN AT 3-11-4, 73.3 lbs FACTORED DOWN AT 5-11-4, 73.3 lbs FACTORED DOWN AT 7-11-4, 73.3 lbs FACTORED DOWN AT 9-11-4, 73.3 lbs FACTORED DOWN AT 11-11-4, 73.3 lbs FACTORED DOWN AT 13-11-4, 73.3 lbs FACTORED DOWN AT 15-11-4, 73.3 lbs FACTORED DOWN AT 17-11-8, 73.3 lbs FACTORED DOWN AT 19-11-12, 73.3 lbs FACTORED DOWN AT 21-11-12, 73.3 lbs FACTORED DOWN AT 23-11-12, 73.3 lbs FACTORED DOWN AT 25-11-12, 73.3 lbs FACTORED DOWN AT 27-11-12, 73.3 lbs FACTORED DOWN AT 29-11-12, AND 88.9 lbs FACTORED DOWN AT 31-11-12, AND 88.9 lbs FACTORED DOWN AT 33-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AI-AJ	0 / 0		-28.0	-28.0	0.11 (3)	10.00			
AJ- K	0 / 0		-28.0	-28.0	0.11 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-699	-699	—	FRONT	VERT	TOTAL
D	11-11-4	-147	-147	—	FRONT	VERT	TOTAL
E	17-11-8	-147	-147	—	FRONT	VERT	TOTAL
F	19-11-12	-147	-147	—	FRONT	VERT	TOTAL
G	23-11-12	-147	-147	—	FRONT	VERT	TOTAL
H	30-0-8	-699	-699	—	FRONT	VERT	TOTAL
L	29-11-12	-42	-73	—	FRONT	VERT	TOTAL
M	23-11-12	-42	-73	—	FRONT	VERT	TOTAL
N	21-11-12	-42	-73	—	FRONT	VERT	TOTAL
O	17-11-8	-42	-73	—	FRONT	VERT	TOTAL
P	13-11-4	-42	-73	—	FRONT	VERT	TOTAL
Q	11-11-4	-42	-73	—	FRONT	VERT	TOTAL
R	5-11-4	-42	-73	—	FRONT	VERT	TOTAL
T	7-11-4	-147	-147	—	FRONT	VERT	TOTAL
U	9-11-4	-147	-147	—	FRONT	VERT	TOTAL
V	13-11-4	-147	-147	—	FRONT	VERT	TOTAL
W	15-11-4	-147	-147	—	FRONT	VERT	TOTAL
X	21-11-12	-147	-147	—	FRONT	VERT	TOTAL
Y	25-11-12	-147	-147	—	FRONT	VERT	TOTAL
Z	27-11-12	-147	-147	—	FRONT	VERT	TOTAL
AA	1-11-4	-42	-77	—	FRONT	VERT	TOTAL
AB	3-11-4	-51	-89	—	FRONT	VERT	TOTAL
AC	7-11-4	-42	-73	—	FRONT	VERT	TOTAL
AD	9-11-4	-42	-73	—	FRONT	VERT	TOTAL
AE	15-11-4	-42	-73	—	FRONT	VERT	TOTAL
AF	19-11-12	-42	-73	—	FRONT	VERT	TOTAL
AG	25-11-12	-42	-73	—	FRONT	VERT	TOTAL
AH	27-11-12	-42	-73	—	FRONT	VERT	TOTAL
AI	31-11-12	-51	-89	—	FRONT	VERT	TOTAL
AJ	33-11-12	-51	-89	—	FRONT	VERT	TOTAL

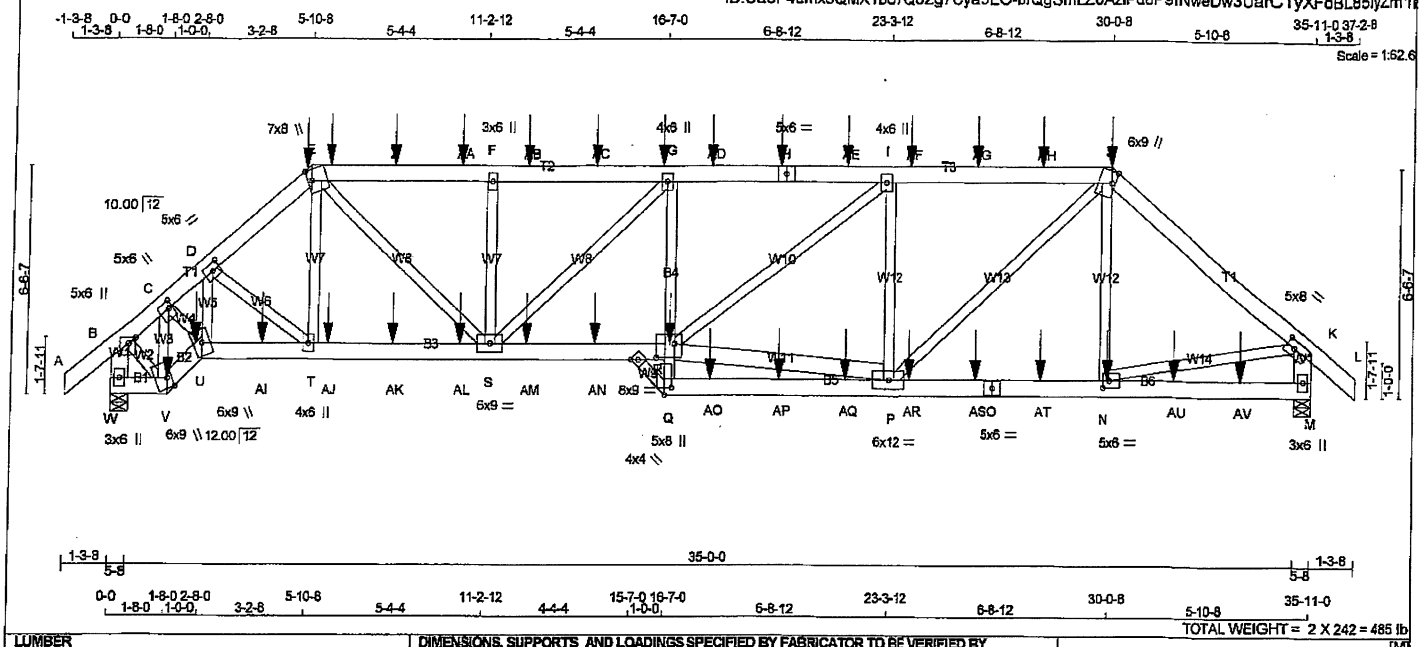
DWG NO. TAM47939-17 Rev 2

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:45 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-brQgSmLZ0AziPd8P9fNweDw3UarCTyXFoBL85lyZm1z



TOTAL WEIGHT = 2 X 242 = 485 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - V	2x6	DRY	No.2	SPF
V - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	2	12	SIDE(122.0)
E- H	2	12	SIDE(183.1)
H- J	2	12	SIDE(61.0)
J- L	2	12	SIDE(122.0)
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- V	2	12	SIDE(61.0)
V- U	2	12	SIDE(122.0)
U- R	2	12	SIDE(61.0)
Q- O	2	12	SIDE(0.0)
O- M	2	12	SIDE(183.1)
G- Q	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW+t	MT20	5.0	6.0	2.50	1.25
D	TMWW+t	MT20	5.0	6.0	2.50	2.75
E	TTWW+m	MT20	7.0	8.0	3.75	1.50
F	TMW+w	MT20	3.0	6.0		
G	TMWW+p	MT20	4.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	9.0	4.00	1.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	5347	0	5347	0	0	5-8	5-8
M	4955	0	4955	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4171	2734 / 0	733 / 0	0 / 0	0 / 0	704 / 0	0 / 0
M	3866	2530 / 0	681 / 0	0 / 0	0 / 0	655 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

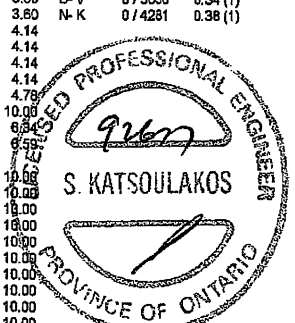
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
FR-TO					
A- B	0 / 56	-122.2	-122.2	0.05 (1)	10.00
B- C	-4394 / 0	-122.2	-122.2	0.15 (1)	5.33
C- D	-7245 / 0	-122.2	-122.2	0.19 (1)	4.32
D- E	-6937 / 0	-122.2	-122.2	0.13 (1)	4.47
E- Y	-8081 / 0	-122.2	-122.2	0.31 (1)	4.03
Y- Z	-8081 / 0	-122.2	-122.2	0.31 (1)	4.03
Z-AA	-8081 / 0	-122.2	-122.2	0.31 (1)	4.03
AA- F	-8081 / 0	-122.2	-122.2	0.31 (1)	4.03
F-AB	-8081 / 0	-122.2	-122.2	0.28 (1)	4.06
AB-AC	-8081 / 0	-122.2	-122.2	0.28 (1)	4.06
AC- G	-8081 / 0	-122.2	-122.2	0.28 (1)	4.06
G-AD	-9037 / 0	-122.2	-122.2	0.52 (1)	3.60
AD- H	-9037 / 0	-122.2	-122.2	0.52 (1)	3.60
H-AE	-9037 / 0	-122.2	-122.2	0.52 (1)	3.60
AE- I	-9037 / 0	-122.2	-122.2	0.52 (1)	3.60
I-AF	-6662 / 0	-122.2	-122.2	0.45 (1)	4.14
AF-AG	-6662 / 0	-122.2	-122.2	0.45 (1)	4.14
AG-AH	-6662 / 0	-122.2	-122.2	0.45 (1)	4.14
AH- J	-6662 / 0	-122.2	-122.2	0.45 (1)	4.14
J- K	-5495 / 0	-122.2	-122.2	0.27 (1)	4.78
K- L	0 / 56	-122.2	-122.2	0.05 (1)	10.00
W- B	-5324 / 0	0.0	0.0	0.19 (1)	6.34
M- K	-4845 / 0	0.0	0.0	0.17 (1)	6.59
W- V	0 / 0	-28.0	-28.0	0.01 (2)	10.00
V- U	0 / 4130	-28.0	-28.0	0.27 (1)	10.00
U-AI	0 / 5686	-28.0	-28.0	0.45 (1)	10.00
AI- T	0 / 5686	-28.0	-28.0	0.45 (1)	10.00
T-AJ	0 / 5331	-28.0	-28.0	0.43 (1)	10.00
AJ-AK	0 / 5331	-28.0	-28.0	0.43 (1)	10.00
AK-AL	0 / 5331	-28.0	-28.0	0.43 (1)	10.00
AL- S	0 / 5331	-28.0	-28.0	0.43 (1)	10.00
S-AM	0 / 9068	-28.0	-28.0	0.70 (1)	10.00
AM-AN	0 / 9068	-28.0	-28.0	0.70 (1)	10.00
AN- R	0 / 9068	-28.0	-28.0	0.70 (1)	10.00
Q- R	0 / 225	0.0	0.0	0.13 (1)	10.00
R- G	-110 / 290	0.0	0.0	0.12 (1)	7.81
Q-AO	0 / 154	-28.0	-28.0	0.14 (3)	10.00
AO-AP	0 / 154	-28.0	-28.0	0.14 (3)	10.00
AP-AQ	0 / 154	-28.0	-28.0	0.14 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
V- C	-4901 / 0	0.30 (1)
C- U	0 / 4216	0.37 (1)
U- D	0 / 222	0.02 (3)
D- T	-449 / 0	0.04 (1)
T- E	0 / 748	0.07 (1)
E- S	0 / 3731	0.33 (1)
S- F	-976 / 0	0.14 (1)
S- G	-1360 / 0	0.49 (1)
R- P	0 / 6841	0.44 (1)
R- I	0 / 2673	0.24 (1)
P- I	-3111 / 0	0.69 (1)
P- J	0 / 3608	0.32 (1)
N- J	-435 / 303	0.10 (1)
B- V	0 / 3830	0.34 (1)
N- K	0 / 4281	0.38 (1)



DWG NO. TAM47941-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.52 (G-I:1) , BC=0.70 (R-S:1) , WB=0.69 (I-P:1) , SSI=0.27 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-brQgSmLZ0AziPd8P9fNweDw3UarCTyXFoBL85lyZm1

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	TMVW-t	MT20	5.0	8.0	2.50	3.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	12.0		
Q						
Q						
Q	BVW*+m	MT20	5.0	6.0	2.50	2.50
R	BVMWW-l	MT20	8.0	9.0	4.75	6.50
S	BMWWW-t	MT20	6.0	9.0		
T	BMWW+t	MT20	4.0	6.0		
U	BBWW+m	MT20	6.0	9.0		
V	BBWW+m	MT20	6.0	9.0	3.50	1.50
W	BMV1+p	MT20	3.0	6.0		
X	NP+w	MT20	4.0	4.0	2.00	1.75

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 514.0 lbs FACTORED DOWN AT 5-10-8, 128.4 lbs FACTORED DOWN AT 6-5-12, 104.2 lbs FACTORED DOWN AT 8-5-12, 104.2 lbs FACTORED DOWN AT 10-5-12, 104.2 lbs FACTORED DOWN AT 12-5-12, 104.2 lbs FACTORED DOWN AT 14-5-12, 104.2 lbs FACTORED DOWN AT 16-5-12, 147.1 lbs FACTORED DOWN AT 17-11-8, 147.1 lbs FACTORED DOWN AT 19-11-12, 147.1 lbs FACTORED DOWN AT 21-11-12, 147.1 lbs FACTORED DOWN AT 23-11-12, 147.1 lbs FACTORED DOWN AT 25-11-12, AND 147.1 lbs FACTORED DOWN AT 27-11-12, AND 688.1 lbs FACTORED DOWN AT 30-0-8 ON TOP CHORD, AND 98.3 lbs FACTORED DOWN AT 1-8-0, 182.3 lbs FACTORED DOWN AT 2-8-0, 168.3 lbs FACTORED DOWN AT 4-5-12, 168.3 lbs FACTORED DOWN AT 6-5-12, 168.3 lbs FACTORED DOWN AT 8-5-12, 168.3 lbs FACTORED DOWN AT 10-5-12, 168.3 lbs FACTORED DOWN AT 12-5-12, 168.3 lbs FACTORED DOWN AT 14-5-12, 178.8 lbs FACTORED DOWN AT 16-8-12, 73.3 lbs FACTORED DOWN AT 17-11-8, 73.3 lbs FACTORED DOWN AT 19-11-12, 73.3 lbs FACTORED DOWN AT 21-11-12, 73.3 lbs FACTORED DOWN AT 23-11-12, 73.3 lbs FACTORED DOWN AT 25-11-12, 73.3 lbs FACTORED DOWN AT 27-11-12, 73.3 lbs FACTORED DOWN AT 29-11-12, AND 88.9 lbs FACTORED DOWN AT 31-11-12, AND 88.9 lbs FACTORED DOWN AT 33-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AQ- P	0 / 154	-28.0	-28.0	0.14 (3)	10.00			
P-AR	0 / 4199	-28.0	-28.0	0.33 (1)	10.00			
AR-AS	0 / 4199	-28.0	-28.0	0.33 (1)	10.00			
AS- O	0 / 4199	-28.0	-28.0	0.33 (1)	10.00			
O-AT	0 / 4199	-28.0	-28.0	0.33 (1)	10.00			
AT- N	0 / 4199	-28.0	-28.0	0.33 (1)	10.00			
N-AU	0 / 0	-28.0	-28.0	0.11 (3)	10.00			
AU-AV	0 / 0	-28.0	-28.0	0.11 (3)	10.00			
AV- M	0 / 0	-28.0	-28.0	0.11 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	5-10-8	-33	-36	—	FRONT	VERT	DEAD
E	5-10-8	-482	-482	—	FRONT	VERT	SNOW
G	16-5-12	-104	-104	—	FRONT	VERT	TOTAL
H	19-11-12	-147	-147	—	FRONT	VERT	TOTAL
J	30-0-8	-688	-688	—	FRONT	VERT	TOTAL
N	29-11-12	-42	-73	—	FRONT	VERT	TOTAL
R	16-8-12	-179	-179	—	FRONT	VERT	TOTAL
U	2-8-0	-182	-182	—	FRONT	VERT	TOTAL
V	1-8-0	-55	-96	—	FRONT	VERT	TOTAL
Y	6-5-12	-128	-128	—	FRONT	VERT	TOTAL
Z	8-5-12	-104	-104	—	FRONT	VERT	TOTAL
AA	10-5-12	-104	-104	—	FRONT	VERT	TOTAL
AB	12-5-12	-104	-104	—	FRONT	VERT	TOTAL
AC	14-5-12	-104	-104	—	FRONT	VERT	TOTAL
AD	17-11-8	-147	-147	—	FRONT	VERT	TOTAL
AE	21-11-12	-147	-147	—	FRONT	VERT	TOTAL
AF	23-11-12	-147	-147	—	FRONT	VERT	TOTAL
AG	25-11-12	-147	-147	—	FRONT	VERT	TOTAL
AH	27-11-12	-147	-147	—	FRONT	VERT	TOTAL
AI	4-5-12	-168	-168	—	FRONT	VERT	TOTAL
AJ	6-5-12	-168	-168	—	FRONT	VERT	TOTAL
AK	8-5-12	-168	-168	—	FRONT	VERT	TOTAL
AL	10-5-12	-168	-168	—	FRONT	VERT	TOTAL
AM	12-5-12	-168	-168	—	FRONT	VERT	TOTAL
AN	14-5-12	-168	-168	—	FRONT	VERT	TOTAL
AO	17-11-8	-42	-73	—	FRONT	VERT	TOTAL
AP	19-11-12	-42	-73	—	FRONT	VERT	TOTAL
AQ	21-11-12	-42	-73	—	FRONT	VERT	TOTAL
AR	23-11-12	-42	-73	—	FRONT	VERT	TOTAL
AS	25-11-12	-42	-73	—	FRONT	VERT	TOTAL
AT	27-11-12	-42	-73	—	FRONT	VERT	TOTAL
AU	31-11-12	-51	-89	—	FRONT	VERT	TOTAL
AV	33-11-12	-51	-89	—	FRONT	VERT	TOTAL

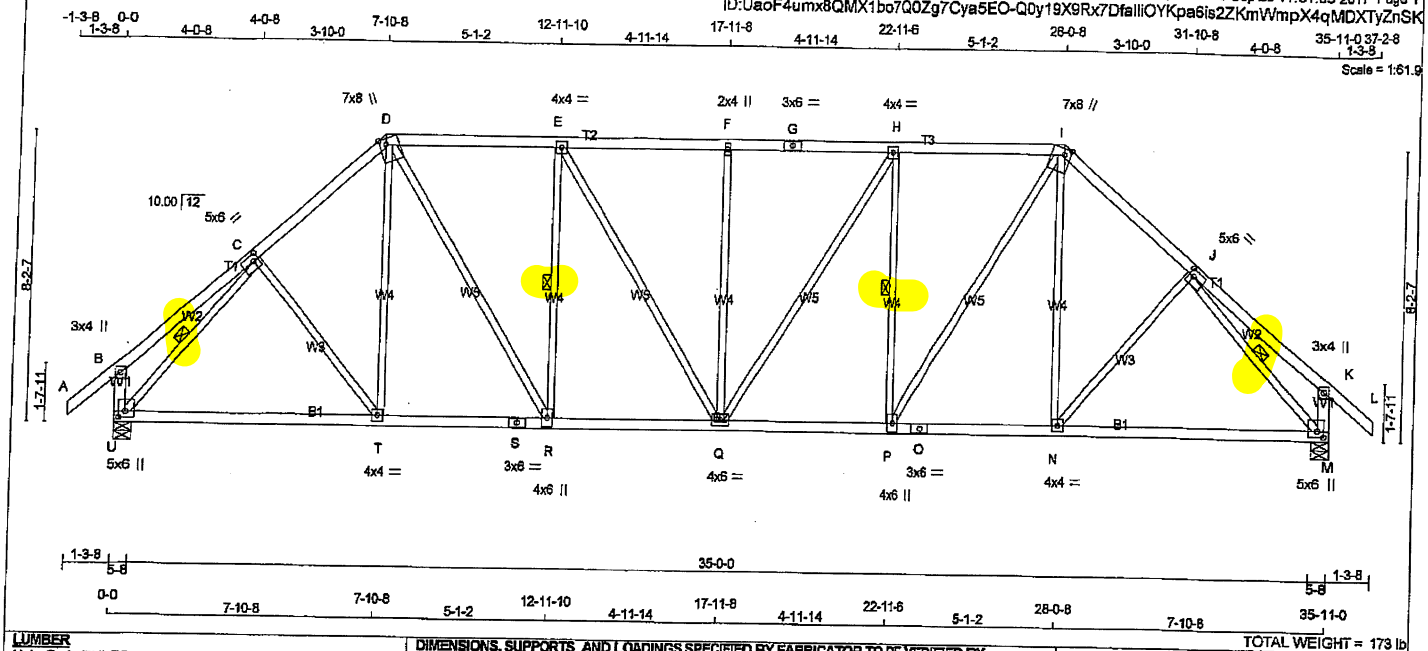
JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.55 (K) (INPUT = 1.00)

DWG NO. TAM 4294-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288459	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 11:51:05 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-Q0y19X9Rx7DfalliOYKpa6is2ZKmWmpX4qMDXTyZnSK



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	5.0	6.0	2.00	2.00
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	5.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2867	0	2867	0	0	5-8	5-8
M	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
M	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-U, J-M, H-P, E-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	0 / 34	-122.2	-122.2	0.29 (1)	10.00
C-D	-2802 / 0	-122.2	-122.2	0.44 (1)	3.76
D-E	-2869 / 0	-122.2	-122.2	0.64 (1)	3.45
E-F	-3093 / 0	-122.2	-122.2	0.66 (1)	3.30
F-G	-3093 / 0	-122.2	-122.2	0.66 (1)	3.30
G-H	-3093 / 0	-122.2	-122.2	0.66 (1)	3.30
H-I	-2889 / 0	-122.2	-122.2	0.64 (1)	3.45
I-J	-2802 / 0	-122.2	-122.2	0.44 (1)	3.76
J-K	0 / 34	-122.2	-122.2	0.29 (1)	10.00
K-L	0 / 54	-122.2	-122.2	0.17 (1)	10.00
U-B	-356 / 0	0.0	0.0	0.04 (1)	7.81
M-K	-356 / 0	0.0	0.0	0.04 (1)	7.81
U-T	0 / 2073	-28.0	-28.0	0.59 (2)	10.00
T-S	0 / 2123	-28.0	-28.0	0.60 (2)	10.00
S-R	0 / 2123	-28.0	-28.0	0.60 (2)	10.00
R-Q	0 / 2870	-28.0	-28.0	0.53 (1)	10.00
Q-P	0 / 2870	-28.0	-28.0	0.53 (1)	10.00
P-O	0 / 2123	-28.0	-28.0	0.60 (2)	10.00
O-N	0 / 2123	-28.0	-28.0	0.60 (2)	10.00
N-M	0 / 2073	-28.0	-28.0	0.59 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-T	0 / 157	0.04 (3)
T-D	0 / 286	0.06 (2)
N-I	0 / 286	0.06 (2)
N-J	0 / 157	0.04 (3)
U-C	-3185 / 0	0.81 (1)
J-M	-3185 / 0	0.81 (1)
P-I	0 / 1376	0.31 (1)
D-R	0 / 1376	0.31 (1)
P-H	-1046 / 0	0.45 (1)
R-E	-1046 / 0	0.45 (1)
Q-H	0 / 419	0.09 (1)
E-Q	0 / 419	0.09 (1)
Q-F	-560 / 0	0.73 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.66 (F-H:1) , BC=0.60 (R-T:2) , WB=0.81 (J-M:1) , SSI=0.29 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (MAXIMUM 0.90)
JSI METAL=0.79 (C) (INPUT 0.400)

LICENSED PROFESSIONAL ENGINEER
91677
S. KATSOULAKOS
PROVINCE OF ONTARIO

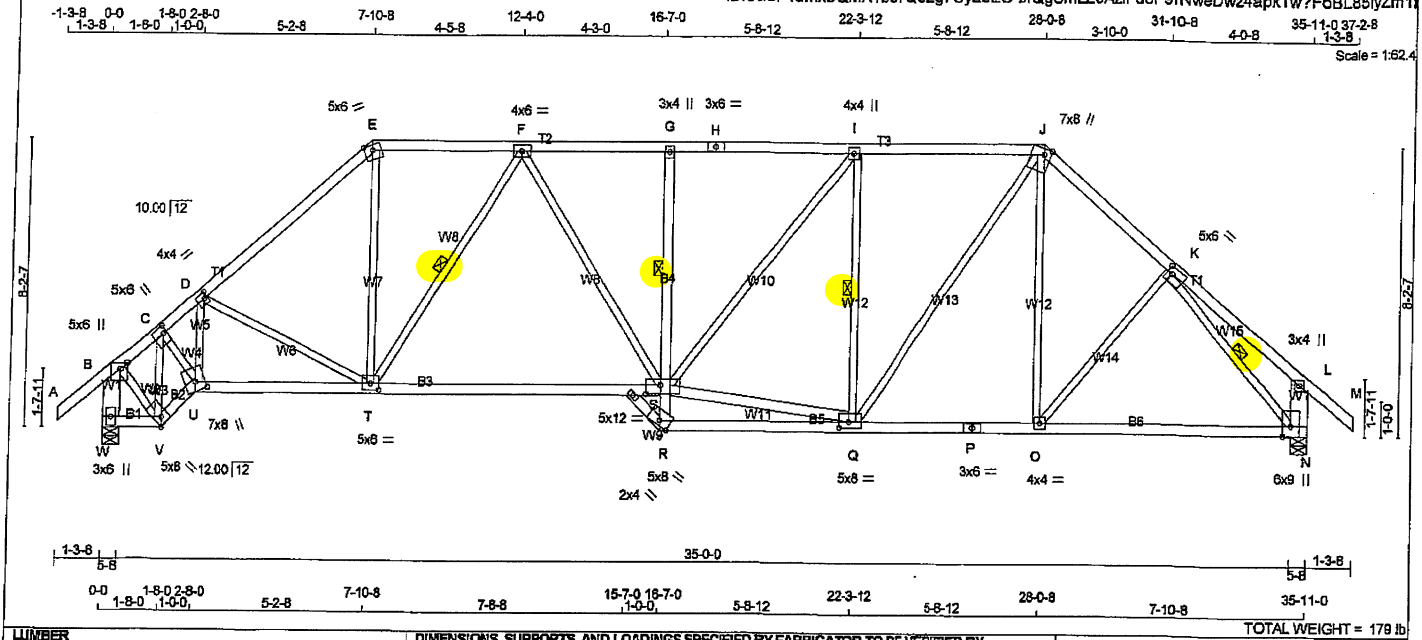
DWG NO. TAM 4794017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:45 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-brQgSmLZ0AziPd8P9fNweDwz4apkTw?FoBL85lyZm1i



LUMBER

N. L. G. A. RULES

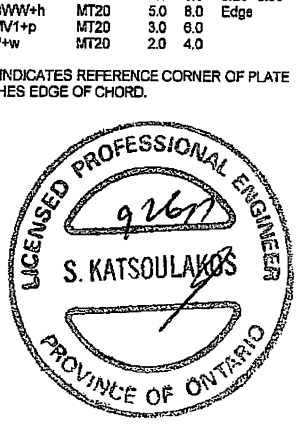
CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW+t	MT20	5.0	6.0	2.50	1.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW-m	MT20	5.0	6.0	Edge	
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	4.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMWW-t	MT20	5.0	6.0	2.25	1.75
L	TMV+p	MT20	3.0	4.0		
N	BMVW1+p	MT20	6.0	9.0	Edge	
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0	2.00	3.50
R	BVW*-m	MT20	5.0	8.0	1.50	3.75
S	BVWWWW-t	MT20	5.0	12.0	3.00	5.00
T	BMWWW-t	MT20	5.0	8.0	2.25	3.00
U	BBWW+m	MT20	7.0	8.0	3.25	3.50
V	BBWW+h	MT20	5.0	8.0	Edge	
W	BMV1+p	MT20	3.0	6.0		
X	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2870	0	2870	0	0	5-8	5-8
N	2864	0	2864	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	MAX./MIN. COMPONENT REACTIONS						
W	2226	1481 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
N	2222	1477 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-S, I-Q, K-N, F-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B-C	-2076 / 0	-122.2	-122.2	0.21 (1)	4.50	
C-D	-3326 / 0	-122.2	-122.2	0.52 (1)	3.23	
D-E	-3192 / 0	-122.2	-122.2	0.75 (1)	3.31	
E-F	-2442 / 0	-122.2	-122.2	0.42 (1)	3.97	
F-G	-3509 / 0	-122.2	-122.2	0.53 (1)	3.29	
G-H	-3499 / 0	-122.2	-122.2	0.93 (1)	2.76	
H-I	-3499 / 0	-122.2	-122.2	0.93 (1)	2.76	
I-J	-2929 / 0	-122.2	-122.2	0.85 (1)	3.11	
J-K	-2797 / 0	-122.2	-122.2	0.44 (1)	3.77	
K-L	0 / 34	-122.2	-122.2	0.29 (1)	10.00	
L-M	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
W-B	-2847 / 0	0.0	0.0	0.20 (1)	6.21	
N-L	-356 / 0	0.0	0.0	0.02 (1)	7.81	
W-V	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
V-U	0 / 2004	-28.0	-28.0	0.33 (1)	10.00	
U-T	0 / 2740	-28.0	-28.0	0.75 (2)	10.00	
T-S	0 / 3158	-28.0	-28.0	0.79 (2)	10.00	
R-S	0 / 117	0.0	0.0	0.10 (1)	10.00	
S-G	-565 / 0	0.0	0.0	0.12 (1)	6.25	
R-Q	0 / 67	-28.0	-28.0	0.20 (2)	10.00	
Q-P	0 / 2119	-28.0	-28.0	0.59 (2)	10.00	
P-O	0 / 2119	-28.0	-28.0	0.59 (2)	10.00	
O-N	0 / 2069	-28.0	-28.0	0.58 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-2545 / 0	0.44 (1)
C-U	0 / 2276	0.51 (1)
U-D	-382 / 9	0.07 (1)
D-T	-359 / 0	0.21 (1)
T-E	0 / 1539	0.35 (1)
S-Q	0 / 2919	0.47 (1)
S-I	0 / 888	0.20 (1)
Q-I	-1483 / 0	0.63 (1)
Q-J	0 / 1388	0.31 (1)
O-J	0 / 281	0.06 (2)
O-K	0 / 157	0.04 (3)
B-V	0 / 1914	0.43 (1)
K-N	-3181 / 0	0.79 (1)
T-F	-1344 / 0	0.61 (1)
F-S	0 / 853	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 772 (0.56")

CSI: TC=0.93 (G-I:1) , BC=0.79 (S-T:2) , WB=0.79 (K-N:1) , SSI=0.33 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.78 (K) (INPUT = 1.00)

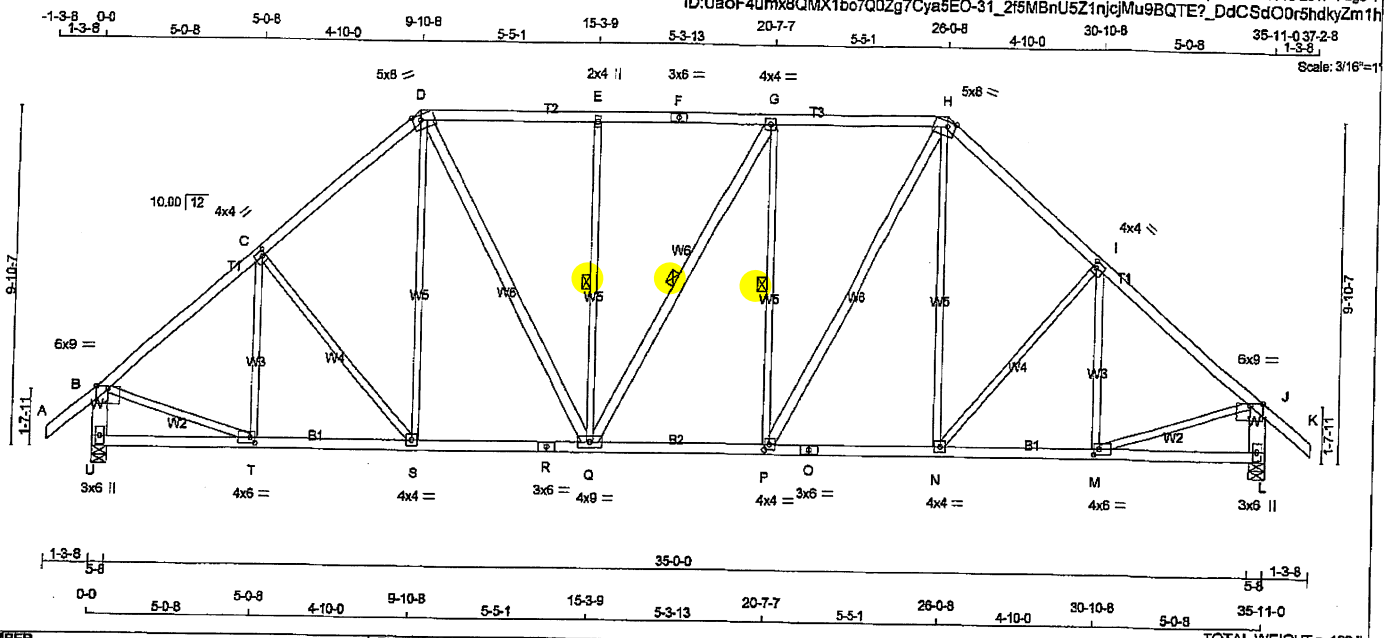
DWG NO. TAM 47942017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:46 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-31_2f5MBnU5Z1njcjMu9BQTE?_DdCSdO0r5hdkyZm1h



TOTAL WEIGHT = 192 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	6.0	9.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	1.75
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	4.0	6.0	2.00	1.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2867	0	2867	0	0	5-8	5-8
L	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
L	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-Q, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	-2815 / 0	-122.2	-122.2	0.53 (1)	3.67	
C- D	-2688 / 0	-122.2	-122.2	0.51 (1)	3.76	
D- E	-2514 / 0	-122.2	-122.2	0.49 (1)	3.85	
E- F	-2514 / 0	-122.2	-122.2	0.46 (1)	3.90	
F- G	-2514 / 0	-122.2	-122.2	0.46 (1)	3.90	
G- H	-2516 / 0	-122.2	-122.2	0.49 (1)	3.85	
H- I	-2688 / 0	-122.2	-122.2	0.51 (1)	3.76	
I- J	-2815 / 0	-122.2	-122.2	0.53 (1)	3.67	
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
U- B	-2607 / 0	0.0	0.0	0.19 (1)	6.25	
L- J	-2607 / 0	0.0	0.0	0.19 (1)	6.25	
U- T	0 / 0	-28.0	-28.0	0.16 (3)	10.00	
T- S	0 / 2197	-28.0	-28.0	0.43 (1)	10.00	
S- R	0 / 2028	-28.0	-28.0	0.41 (1)	10.00	
R- Q	0 / 2028	-28.0	-28.0	0.41 (1)	10.00	
Q- P	0 / 2516	-28.0	-28.0	0.49 (1)	10.00	
P- O	0 / 2028	-28.0	-28.0	0.42 (1)	10.00	
O- N	0 / 2028	-28.0	-28.0	0.42 (1)	10.00	
N- M	0 / 2197	-28.0	-28.0	0.43 (1)	10.00	
M- L	0 / 0	-28.0	-28.0	0.16 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-420 / 59	0.22 (1)
C- S	-258 / 0	0.28 (1)
S- D	0 / 358	0.08 (2)
D- Q	0 / 885	0.16 (1)
Q- E	-707 / 0	0.46 (1)
Q- G	-4 / 0	0.00 (1)
P- G	-708 / 0	0.47 (1)
P- H	0 / 990	0.16 (1)
N- H	0 / 357	0.08 (2)
N- I	-259 / 0	0.28 (1)
M- I	-419 / 59	0.22 (1)
B- T	0 / 2269	0.51 (1)
M- J	0 / 2270	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.53 (I-J:1) , BC=0.49 (P-Q:1) , WB=0.51 (J-M:1) , SSI=0.31 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (DRY/NORMAL) 0.90 (WET/NORMAL)
JSI METAL= 0.65 (DRY/INPUT = 1.00)

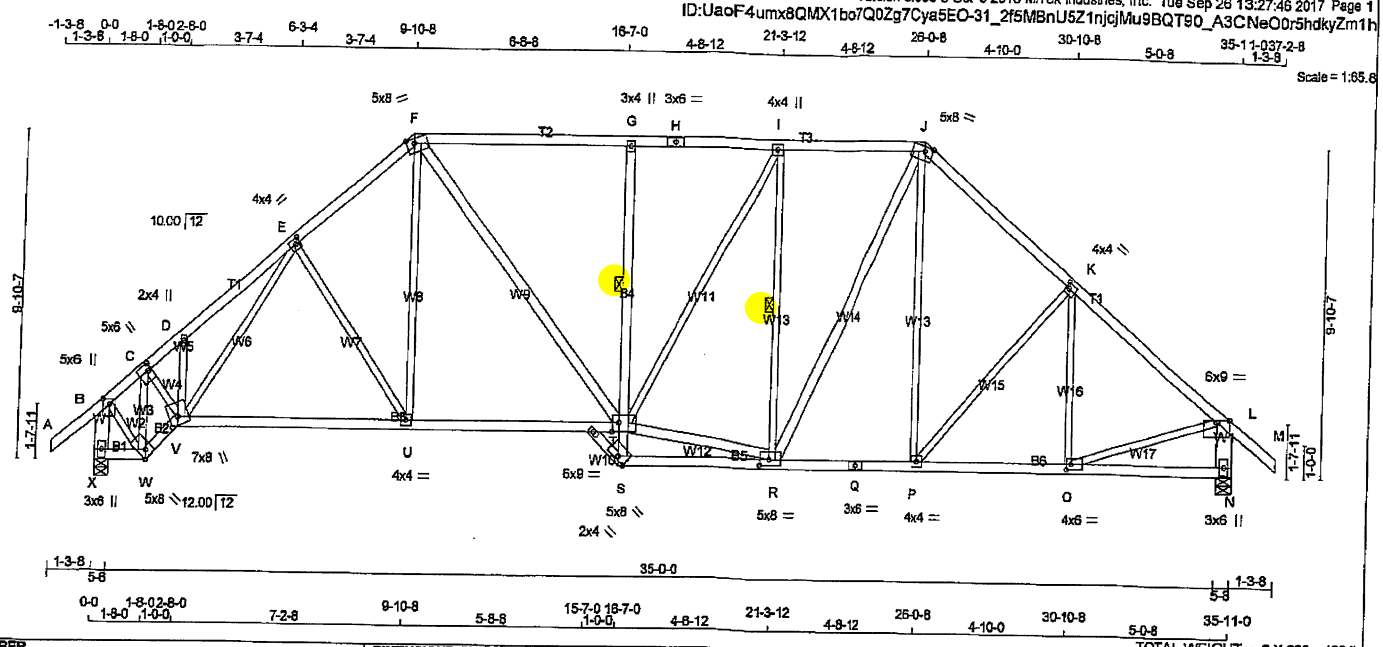
DWG NO. TAM 47943 17
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
9267
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42057	DRWG NO.
288458	T3S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:46 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-31_2f5MBnU5Z1njcjMu9BQT90_A3CNeO0r5hdkyZm1h



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
X - B	2x8	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - W	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TMWW+t	MT20	5.0	6.0	2.50	1.25
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TMV+p	MT20	3.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	4.0		
J	TTWW-m	MT20	5.0	8.0	Edge	3.00
K	TMWW-t	MT20	4.0	4.0	2.00	1.25
L	TMVW-p	MT20	6.0	9.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWWW-t	MT20	5.0	8.0	2.25	3.50
S	BVW*-m	MT20	5.0	8.0	1.25	3.75
T	BVMWWW-I	MT20	6.0	9.0	3.25	2.50
U	BMWW-t	MT20	4.0	4.0		
V	BBWWW+m	MT20	7.0	8.0	3.00	3.00
W	BBWW+h	MT20	5.0	8.0	Edge	
X	BMV1+p	MT20	3.0	6.0		
Y	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2870	0	2870	0	0	5-8	5-8
N	2864	0	2864	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2226	1481 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
N	2222	1477 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T, I-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B-C	-2082 / 0	-122.2	-122.2	0.17 (1)	4.57	
C-D	-3373 / 0	-122.2	-122.2	0.19 (1)	3.70	
D-E	-3409 / 0	-122.2	-122.2	0.34 (1)	3.56	
E-F	-2983 / 0	-122.2	-122.2	0.30 (1)	3.81	
F-G	-2861 / 0	-122.2	-122.2	0.85 (1)	3.03	
G-H	-2856 / 0	-122.2	-122.2	0.63 (1)	3.39	
H-I	-2856 / 0	-122.2	-122.2	0.63 (1)	3.39	
I-J	-2456 / 0	-122.2	-122.2	0.41 (1)	4.00	
J-K	-2662 / 0	-122.2	-122.2	0.51 (1)	3.77	
K-L	-2812 / 0	-122.2	-122.2	0.53 (1)	3.67	
L-M	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
X-B	-2847 / 0	0.0	0.0	0.20 (1)	6.21	
N-L	-2804 / 0	0.0	0.0	0.19 (1)	6.26	
X-W	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
W-V	0 / 2032	-28.0	-28.0	0.33 (1)	10.00	
V-U	0 / 2527	-28.0	-28.0	0.66 (2)	10.00	
U-T	0 / 2268	-28.0	-28.0	0.64 (2)	10.00	
S-T	0 / 90	0.0	0.0	0.07 (1)	10.00	
T-G	-828 / 0	0.0	0.0	0.16 (1)	6.25	
S-R	0 / 44	-28.0	-28.0	0.16 (2)	10.00	
R-Q	0 / 2023	-28.0	-28.0	0.42 (1)	10.00	
Q-P	0 / 2023	-28.0	-28.0	0.42 (1)	10.00	
P-O	0 / 2195	-28.0	-28.0	0.43 (1)	10.00	
O-N	0 / 0	-28.0	-28.0	0.17 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W-C	-2580 / 0	0.45 (1)
C-V	0 / 2092	0.47 (1)
V-D	-342 / 0	0.06 (1)
V-E	0 / 203	0.05 (1)
E-U	-478 / 0	0.39 (1)
U-F	0 / 631	0.14 (1)
F-T	0 / 948	0.15 (1)
T-R	0 / 2478	0.40 (1)
T-I	0 / 826	0.19 (1)
R-I	-1262 / 0	0.83 (1)
R-J	0 / 991	0.16 (1)
P-J	0 / 331	0.07 (2)
P-K	-263 / 0	0.26 (1)
O-K	-415 / 66	0.22 (1)
B-W	0 / 1940	0.44 (1)
O-L	0 / 2267	0.51 (1)

DWG NO. TAM47944-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.85 (F-G:1) , BC=0.66 (U-V:2) , WB=0.83 (I-R:1) , SSI=0.38 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

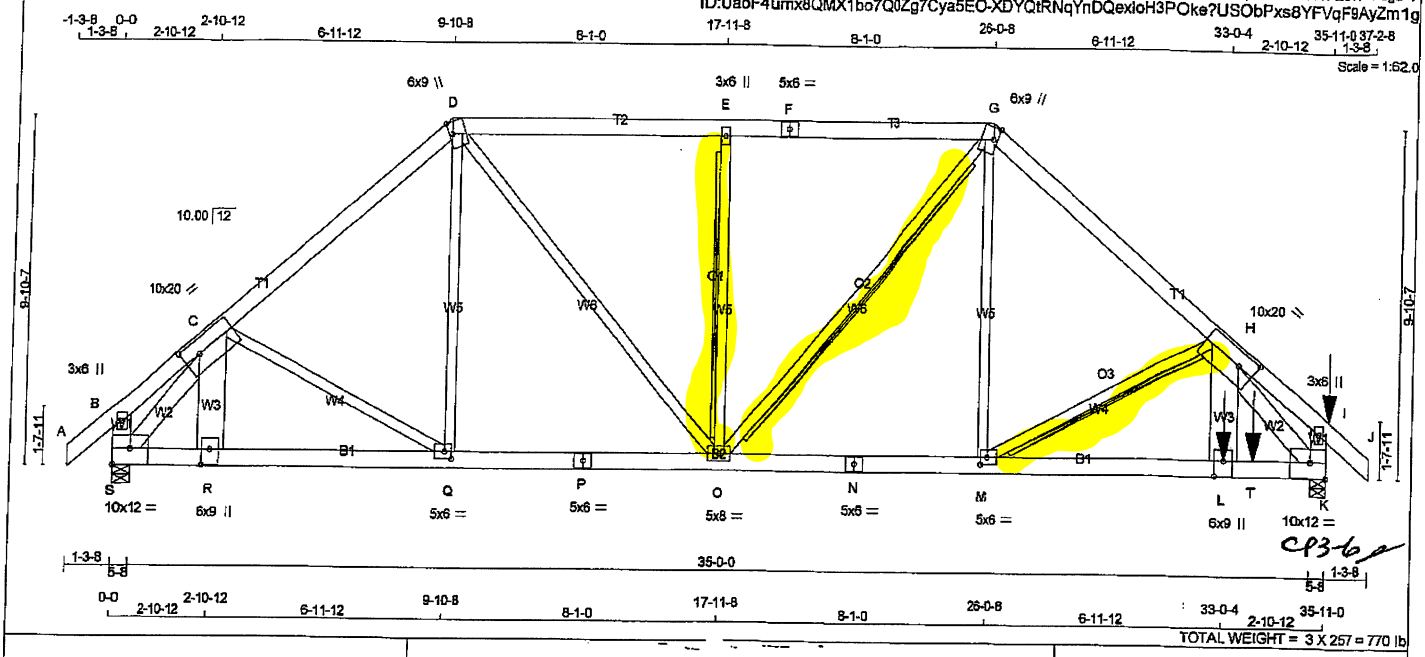
JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.55 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T4	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:47 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-XDYQtRNqYnDQexloH3POke?USObPxs8YFVqF9AyZm1g



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - N	2x6	DRY	2100F 1.8E	SPF
N - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
R - C	2x10	DRY	No.2	SPF
L - H	2x10	DRY	No.2	SPF
S - C	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	2	12	TOP
D-F	2	12	TOP
F-G	2	12	TOP
G-J	2	12	SIDE(274.6)
S-B	2	11	TOP
K-I	2	11	SIDE(59.4)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	3	4	TOP
P-N	2	12	TOP
N-K	3	4	SIDE(1681.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	
H-L 2x10	4	4	SIDE(2053.9)

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWWW-t	MT20	10.0	20.0	4.50	5.75
D	TTWW+m	MT20	6.0	9.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TTWW+m	MT20	6.0	9.0	4.00	1.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3945	0	3945	0	0	5-8	5-8
K	16693	0	16693	0	0	5-8	5-8

SIMPson SB / CP3-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	3060	2036 / 0	519 / 0	0 / 0	0 / 0	506 / 0	0 / 0
K	12985	8576 / 0	2242 / 0	0 / 0	0 / 0	2167 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-O, G-O, H-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.03 (1)		10.00
B- C	0 / 114	-122.2	-122.2	0.12 (1)		10.00
C- D	-4230 / 0	-122.2	-122.2	0.14 (1)		6.25
D- E	-4724 / 0	-122.2	-122.2	0.23 (1)		5.98
E- F	-4726 / 0	-122.2	-122.2	0.23 (1)		5.98
F- G	-4726 / 0	-122.2	-122.2	0.23 (1)		5.98
G- H	-6544 / 0	-122.2	-122.2	0.21 (1)		5.33
H- I	-29 / 30	-122.2	-122.2	0.03 (1)		6.25
I- J	0 / 56	-122.2	-122.2	0.03 (1)		10.00
S- B	-169 / 0	0.0	0.0	0.00 (1)		7.81
K- I	-1524 / 0	0.0	0.0	0.03 (1)		7.81
S- R	0 / 3027	-28.0	-28.0	0.05 (1)		10.00
R- Q	0 / 3030	-28.0	-28.0	0.07 (1)		10.00
Q- P	0 / 3210	-28.0	-28.0	0.07 (1)		10.00
P- O	0 / 3210	-28.0	-28.0	0.07 (1)		10.00
O- N	0 / 5065	-28.0	-28.0	0.11 (1)		10.00
N- M	0 / 5065	-28.0	-28.0	0.11 (1)		10.00
M- L	0 / 12176	-28.0	-28.0	0.26 (1)		10.00
L- T	0 / 11853	-28.0	-28.0	0.33 (1)		10.00
T- K	0 / 11853	-28.0	-28.0	0.33 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	0 / 262	0.01 (3)
Q- C	0 / 197	0.01 (1)
Q- D	0 / 345	0.02 (3)
O- D	0 / 2321	0.12 (1)
O- E	-1176 / 0	0.13 (1)
O- G	-521 / 0	0.10 (1)
M- G	0 / 3778	0.20 (1)
M- H	-7953 / 0	0.59 (1)
L- H	0 / 13447	0.36 (1)
S- C	-4758 / 0	0.47 (1)
H- K	-13447 / 0	0.69 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	35-10-4	-1223	-1223	—	BACK	VERT	TOTAL
L	33-0-4	-12350	-12350	—	BACK	VERT	TOTAL
T	33-10-4	-1318	-1318	—	BACK	VERT	TOTAL

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.23 (E-G:1) , BC=0.33 (K-L:1) , WB=0.69 (H-K:1) , SSI=0.16 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.99 (C) (INPUT = 1.00)

P6/6

DWG NO. TAM 42945-17
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T4	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-XDYQtRNqYnDQexloH3POke?USObPxs8YFVqF9AyZm1g

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TMWWW-t	MT20	10.0	20.0	4.50	5.75
I	TMV+p	MT20	3.0	6.0		
K	BMVW1-t	MT20	10.0	12.0	5.50	5.50
L	BMW+w	MT20	6.0	9.0	5.25	3.00
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMW+w	MT20	6.0	9.0	5.25	3.00
S	BMVW1-t	MT20	10.0	12.0	5.50	5.50

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1223.4 lbs FACTORED DOWN AT 35-10-4 ON TOP CHORD, AND 12350.5 lbs FACTORED DOWN AT 33-0-4, AND 1317.6 lbs FACTORED DOWN AT 33-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

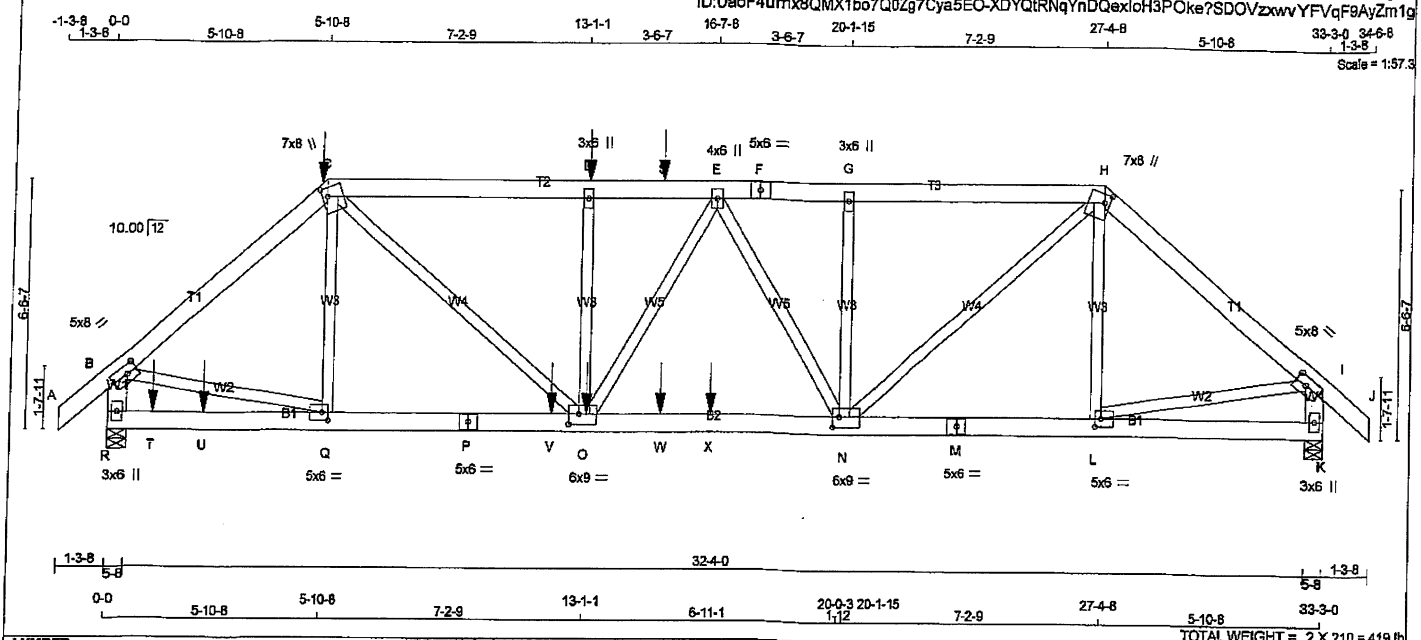
DWG NO. TAM 47945-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:47 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-XDYQtRNqYnDQexloH3POke?SDOVzxwvYFVqF9AyZm1g



TOTAL WEIGHT = 2 X 210 = 419 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
M - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	SIDE(122.0)
C-F	2	12	SIDE(183.1)
F-H	2	12	TOP
H-J	2	12	TOP
R-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R-P	2	12	SIDE(0.0)
P-M	2	12	SIDE(197.8)
M-K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	7.0	8.0	2.75	1.75
D	TMW+w	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	7.0	8.0	2.75	1.75
I	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	6.0	9.0	3.25	2.25
O	BMWWW-t	MT20	6.0	9.0	3.25	3.25
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	6302	0	6302	0	0	5-8	5-8
K	4516	0	4516	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	4809	3338 / 0	725 / 0	0 / 0	0 / 0	746 / 0	0 / 0
K	3466	2370 / 0	547 / 0	0 / 0	0 / 0	550 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 56	-122.2	-122.2	0.05 (1)	10.00
B-C	-8455 / 0	-122.2	-122.2	0.29 (1)	4.46
C-D	-8424 / 0	-122.2	-122.2	0.38 (1)	3.91
D-S	-8424 / 0	-122.2	-122.2	0.28 (1)	3.97
S-E	-8424 / 0	-122.2	-122.2	0.28 (1)	3.97
E-F	-7488 / 0	-122.2	-122.2	0.26 (1)	4.19
F-G	-7488 / 0	-122.2	-122.2	0.26 (1)	4.19
G-H	-7487 / 0	-122.2	-122.2	0.34 (1)	4.14
H-I	-4935 / 0	-122.2	-122.2	0.25 (1)	5.00
I-J	0 / 56	-122.2	-122.2	0.05 (1)	10.00
R-B	-5599 / 0	0.0	0.0	0.20 (1)	6.21
K-I	-4406 / 0	0.0	0.0	0.16 (1)	6.84
R-T	0 / 0	-28.0	-28.0	0.34 (1)	10.00
T-U	0 / 0	-28.0	-28.0	0.34 (1)	10.00
U-Q	0 / 0	-28.0	-28.0	0.34 (1)	10.00
Q-P	0 / 4943	-28.0	-28.0	0.42 (1)	10.00
P-V	0 / 4943	-28.0	-28.0	0.42 (1)	10.00
V-O	0 / 4943	-28.0	-28.0	0.42 (1)	10.00
O-W	0 / 8029	-28.0	-28.0	0.74 (1)	10.00
W-X	0 / 8029	-28.0	-28.0	0.74 (1)	10.00
X-N	0 / 8029	-28.0	-28.0	0.74 (1)	10.00
N-M	0 / 3763	-28.0	-28.0	0.32 (1)	10.00
M-L	0 / 3763	-28.0	-28.0	0.32 (1)	10.00
L-K	0 / 0	-28.0	-28.0	0.04 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-57 / 259	0.02 (3)
C-O	0 / 4540	0.40 (1)
O-D	-1019 / 0	0.23 (1)
N-G	-819 / 0	0.18 (1)
N-H	0 / 4858	0.43 (1)
L-H	-719 / 39	0.16 (1)
B-Q	0 / 5029	0.44 (1)
L-I	0 / 3845	0.34 (1)
O-E	0 / 785	0.07 (1)
E-N	-1078 / 0	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-33	-36	—	FRONT	VERT	DEAD
C	5-10-8	-482	-482	—	FRONT	VERT	SNOW
D	13-1-12	-147	-147	—	BACK	VERT	TOTAL
O	13-1-12	-42	-73	—	BACK	VERT	TOTAL
S	15-1-12	-147	-147	—	BACK	VERT	TOTAL
T	1-2-4	-42	-73	—	BACK	VERT	TOTAL
U	2-7-8	-1216	-1216	—	BACK	VERT	TOTAL
V	12-2-8	-1216	-1216	—	BACK	VERT	TOTAL
W	15-1-12	-42	-73	—	BACK	VERT	TOTAL
X	16-6-8	-2105	-2105	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.11")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.38 (C-D:1) , BC=0.74 (N-O:1) , WB=0.44 (B-Q:1) , SSI=0.42 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.64 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9261 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 47946-17

STRUCTURAL

PERMANENT BRACE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:UacF4umx8QMX1bo7Q0Zg7Cya5EO-XDYQtRNqYnDQexloH3POke?SDOVzxwvYFVqF9AyZm1g

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 514.0 lbs FACTORED DOWN AT 5-10-8, AND 147.1 lbs FACTORED DOWN AT 13-1-12, AND 147.1 lbs FACTORED DOWN AT 15-1-12 ON TOP CHORD, AND 73.3 lbs FACTORED DOWN AT 1-2-4, 1216.4 lbs FACTORED DOWN AT 2-7-8, 1216.4 lbs FACTORED DOWN AT 12-2-8, 73.3 lbs FACTORED DOWN AT 13-1-12, AND 73.3 lbs FACTORED DOWN AT 15-1-12, AND 2104.6 lbs FACTORED DOWN AT 16-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47946 17

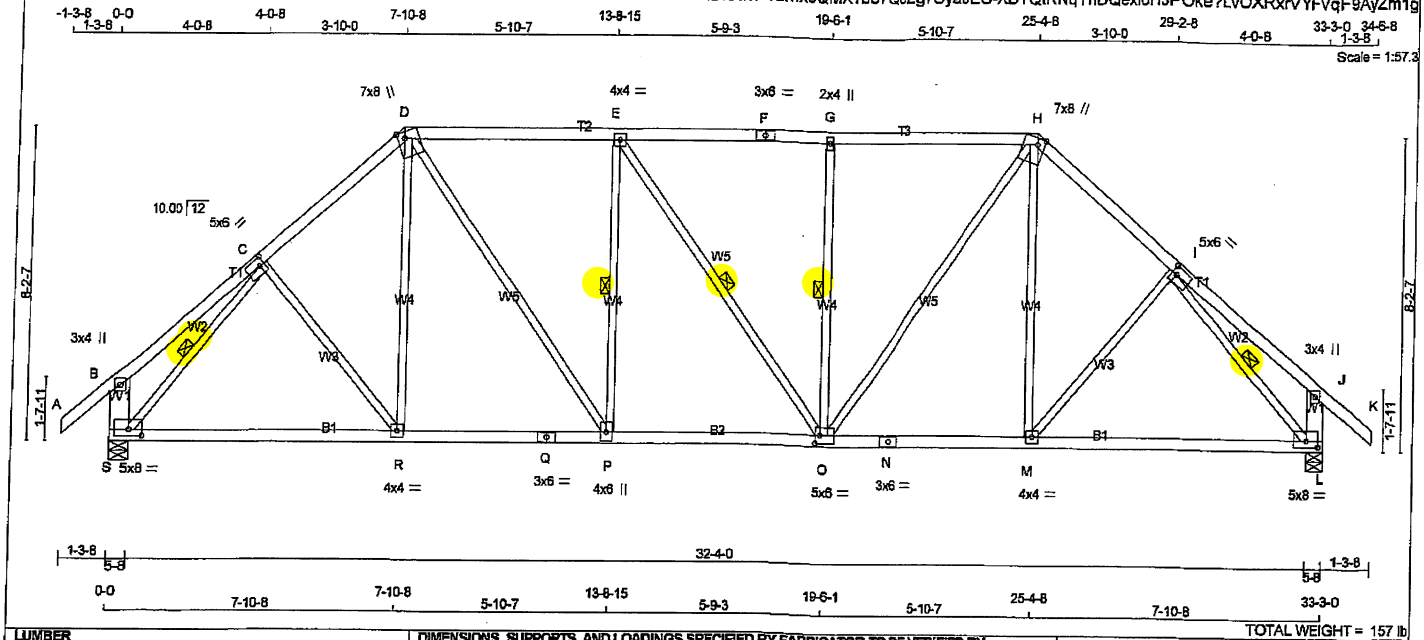
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:47 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-XDYQtRNqYnDQexloH3POke?LvOXRxrVYFVqF9AyZm1g



TOTAL WEIGHT = 157 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0	2.50	1.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	8.0	2.00	4.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0	2.50	1.50
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	8.0	2.00	4.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2667	0	2667	0	0	5-8	5-8
L	2667	0	2667	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2067	1377 / 0	349 / 0	0 / 0	0 / 0	341 / 0	0 / 0
L	2067	1377 / 0	349 / 0	0 / 0	0 / 0	341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, E-O, G-O, C-S, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B-C	0 / 34	-122.2	-122.2	0.29 (1)		10.00
C-D	-2541 / 0	-122.2	-122.2	0.42 (1)		3.94
D-E	-2603 / 0	-122.2	-122.2	0.78 (1)		3.39
E-F	-2601 / 0	-122.2	-122.2	0.77 (1)		3.39
F-G	-2601 / 0	-122.2	-122.2	0.77 (1)		3.39
G-H	-2601 / 0	-122.2	-122.2	0.77 (1)		3.42
H-I	-2541 / 0	-122.2	-122.2	0.42 (1)		3.94
I-J	0 / 34	-122.2	-122.2	0.29 (1)		10.00
J-K	0 / 54	-122.2	-122.2	0.17 (1)		10.00
S-B	-355 / 0	0.0	0.0	0.02 (1)		7.81
L-J	-355 / 0	0.0	0.0	0.02 (1)		7.81
S-R	0 / 1902	-28.0	-28.0	0.57 (2)		10.00
R-Q	0 / 1923	-28.0	-28.0	0.58 (2)		10.00
Q-P	0 / 1923	-28.0	-28.0	0.58 (2)		10.00
P-O	0 / 2603	-28.0	-28.0	0.51 (1)		10.00
O-N	0 / 1924	-28.0	-28.0	0.58 (2)		10.00
N-M	0 / 1924	-28.0	-28.0	0.58 (2)		10.00
M-L	0 / 1902	-28.0	-28.0	0.57 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C-R	0 / 141	0.03 (3)
R-D	0 / 311	0.07 (2)
D-P	0 / 1139	0.26 (1)
P-E	-769 / 0	0.33 (1)
E-O	-3 / 0	0.00 (1)
O-G	-768 / 0	0.33 (1)
O-H	0 / 1136	0.26 (1)
M-H	0 / 312	0.07 (2)
M-I	0 / 141	0.03 (3)
S-C	-2923 / 0	0.73 (1)
I-L	-2923 / 0	0.73 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.11")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.78 (D-E:1) , BC=0.58 (M-O:2) , WB=0.73 (I-L:1) , SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (LUMBER = 0.90)
JSI METAL= 0.70 (INPUT = 1.00)

DWG NO. TAM 47947-17
STRUCTURAL COMPONENT ONLY

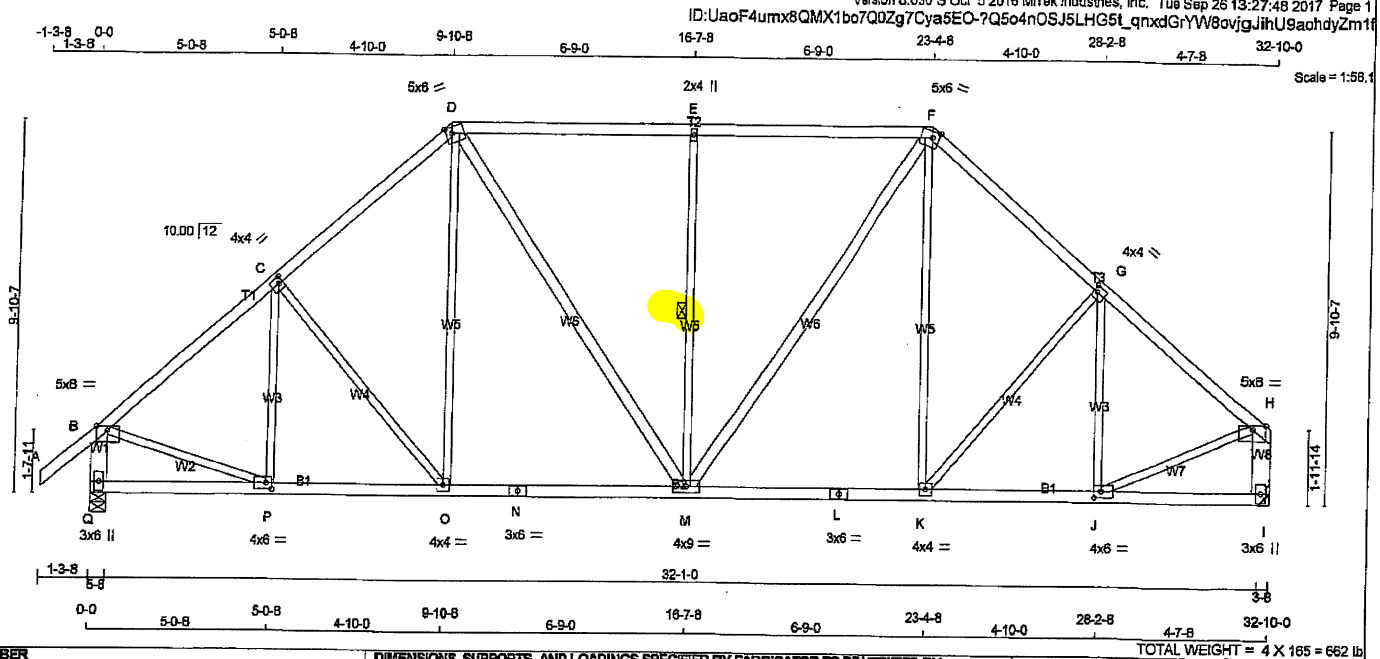
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T7A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:48 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-?Q5o4nOSJ5LHG5t_qnxdGrYW8ovjgJihU9aohdyZm1f



TOTAL WEIGHT = 4 X 165 = 662 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	Edge	
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.25
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.00
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2635	0	2635	0	0	5-8	5-8
I	2466	0	2466	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2043	1361 / 0	345 / 0	0 / 0	0 / 0	337 / 0	0 / 0
I	1929	1255 / 0	345 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	-2538 / 0	-122.2	-122.2	0.51 (1)	3.86
C-D	-2378 / 0	-122.2	-122.2	0.48 (1)	3.99
D-E	-2195 / 0	-122.2	-122.2	0.81 (1)	3.42
E-F	-2195 / 0	-122.2	-122.2	0.81 (1)	3.42
F-G	-2318 / 0	-122.2	-122.2	0.44 (1)	4.08
G-H	-2359 / 0	-122.2	-122.2	0.44 (1)	4.05
Q-B	-2574 / 0	0.0	0.0	0.18 (1)	6.47
I-H	-2411 / 0	0.0	0.0	0.18 (1)	6.64
Q-P	0 / 0	-28.0	-28.0	0.15 (3)	10.00
P-O	0 / 1985	-28.0	-28.0	0.43 (1)	10.00
O-N	0 / 1791	-28.0	-28.0	0.45 (2)	10.00
N-M	0 / 1791	-28.0	-28.0	0.45 (2)	10.00
M-L	0 / 1747	-28.0	-28.0	0.45 (2)	10.00
L-K	0 / 1747	-28.0	-28.0	0.45 (2)	10.00
K-J	0 / 1847	-28.0	-28.0	0.40 (1)	10.00
J-I	0 / 0	-28.0	-28.0	0.13 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-370 / 70	0.19 (1)
C-O	-296 / 0	0.32 (1)
O-D	0 / 406	0.09 (2)
D-M	0 / 701	0.11 (1)
M-E	-1012 / 0	0.67 (1)
M-F	0 / 777	0.12 (1)
K-F	0 / 342	0.08 (2)
K-G	-154 / 0	0.17 (1)
J-G	-524 / 1	0.27 (1)
B-P	0 / 2050	0.26 (1)
J-K	0 / 1961	0.44 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T.AM 47948 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.81 (D-E:1) , BC=0.45 (M-O:2) , WB=0.67 (E-M:1) , SSI=0.40 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

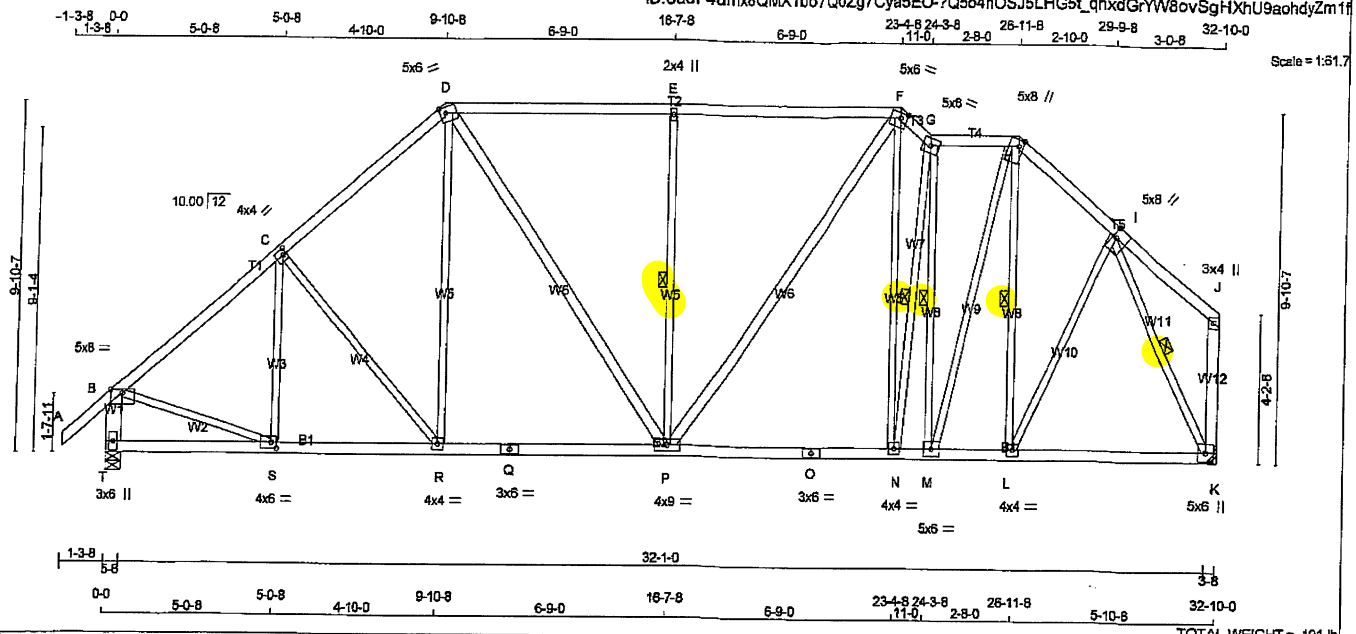
JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.55 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T8AS	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:48 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-?Q5o4nOSJ5LHG5t_qnxdGrYW8ovSgHXhU9aohdyZm1f



TOTAL WEIGHT = 191 lb

[M][F]

LUMBER

N. L. G. A. RULES

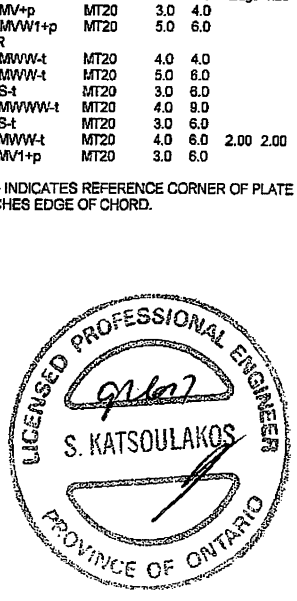
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	1.75	2.25
G	TTWW-m	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	8.0	2.25	1.50
I	TMWW+t	MT20	5.0	8.0	Edge	1.25
J	TMV+p	MT20	3.0	4.0		
K	BMWW1+p	MT20	5.0	6.0		
L, N, R						
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	6.0	2.00	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2635	0	2635	0	0	IN-SX	IN-SX
K	2466	0	2466	0	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2043	1361 / 0	345 / 0	0 / 0	0 / 0	337 / 0	0 / 0
K	1929	1255 / 0	345 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T .

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, G-N, G-M, H-L, I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	1	0.17 (1)	10.00
B- C	-2539 / 0	-122.2	-122.2	1	0.51 (1)	3.86
C- D	-2378 / 0	-122.2	-122.2	1	0.48 (1)	3.99
D- E	-2196 / 0	-122.2	-122.2	1	0.81 (1)	3.42
E- F	-2196 / 0	-122.2	-122.2	1	0.81 (1)	3.42
F- G	-2273 / 0	-122.2	-122.2	1	0.06 (1)	4.51
G- H	-1756 / 0	-122.2	-122.2	1	0.14 (1)	4.91
H- I	-1816 / 0	-122.2	-122.2	1	0.16 (1)	4.84
I- J	0 / 25	-122.2	-122.2	1	0.16 (1)	10.00
T- B	-2575 / 0	0.0	0.0	1	0.18 (1)	6.47
K- J	-137 / 0	0.0	0.0	1	0.04 (1)	7.81
T- S	0 / 0	-28.0	-28.0	3	0.15 (3)	10.00
S- R	0 / 1985	-28.0	-28.0	1	0.42 (1)	10.00
R- Q	0 / 1790	-28.0	-28.0	2	0.47 (2)	10.00
Q- P	0 / 1790	-28.0	-28.0	2	0.47 (2)	10.00
P- O	0 / 1742	-28.0	-28.0	2	0.47 (2)	10.00
O- N	0 / 1742	-28.0	-28.0	2	0.47 (2)	10.00
N- M	0 / 1773	-28.0	-28.0	2	0.35 (2)	10.00
M- L	0 / 1366	-28.0	-28.0	2	0.37 (2)	10.00
L- K	0 / 1040	-28.0	-28.0	2	0.34 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-368 / 71	0.19 (1)
C- R	-297 / 0	0.32 (1)
R- D	0 / 403	0.09 (2)
D- P	0 / 704	0.11 (1)
P- E	-1012 / 0	0.67 (1)
P- F	0 / 789	0.13 (1)
N- F	0 / 436	0.10 (1)
N- G	-285 / 31	0.16 (1)
M- G	-1390 / 0	0.75 (1)
M- H	0 / 1395	0.31 (1)
L- H	-494 / 0	0.27 (1)
L- I	0 / 750	0.17 (1)
B- S	0 / 2050	0.46 (1)
I- K	-2490 / 0	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.81 (D-E:1) , BC=0.47 (P-R:2) , WB=0.80 (I-K:1) , SSI=0.40 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

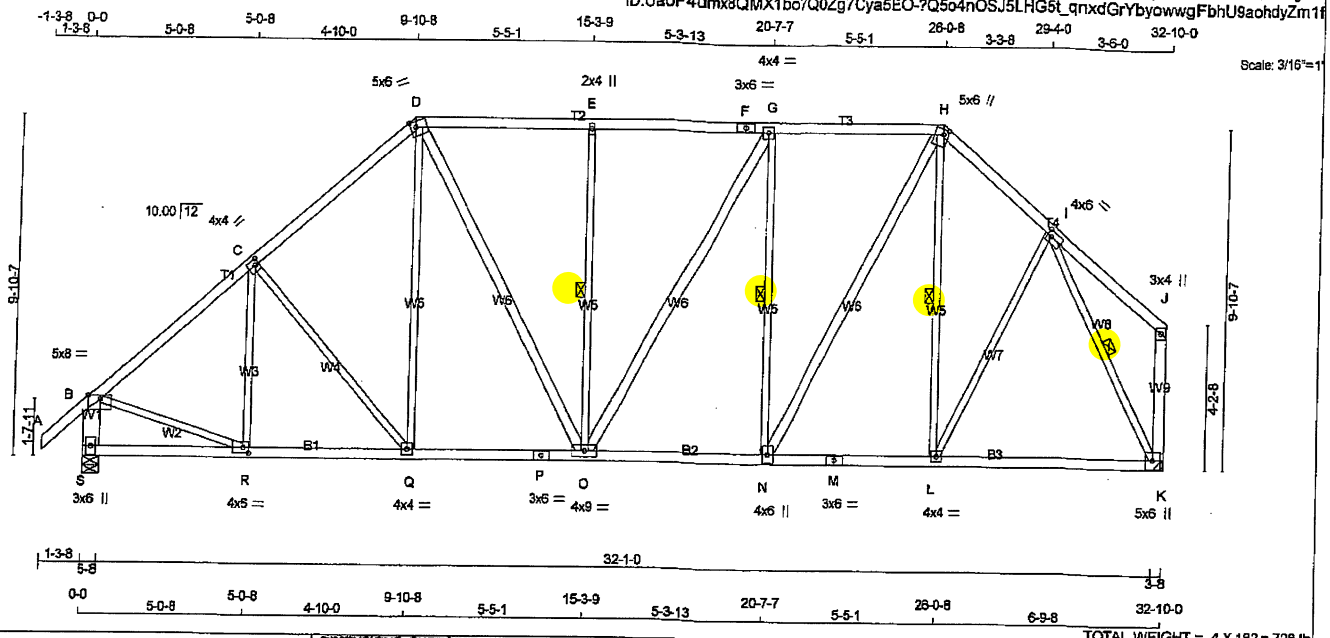
JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.81 (O) (INPUT = 1.00)

DWG NO. T4N 47949-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T9A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:48 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-?Q5o4nOSJ5LHG5t_qnxdGrYbyowwgFbhU9aohdyZm1f



TOTAL WEIGHT = 4 X 182 = 728 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	1.75	1.75
I	TMWW-t	MT20	4.0	6.0	2.00	1.50
J	TMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW+t	MT20	4.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	4.0	6.0	2.00	2.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	2635	0	2635	0	0	5-8	5-8
K	2466	0	2466	0	0	IN-SX	IN-SX

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2043	1361 / 0	345 / 0	0 / 0	0 / 0	337 / 0	0 / 0
K	1929	1255 / 0	345 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, G-N, H-L, I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	-2539 / 0	-122.2	-122.2	0.51 (1)	3.66
C-D	-2376 / 0	-122.2	-122.2	0.48 (1)	3.99
D-E	-2145 / 0	-122.2	-122.2	0.47 (1)	4.16
E-F	-2145 / 0	-122.2	-122.2	0.43 (1)	4.21
F-G	-2145 / 0	-122.2	-122.2	0.43 (1)	4.21
G-H	-2016 / 0	-122.2	-122.2	0.46 (1)	4.27
H-I	-1868 / 0	-122.2	-122.2	0.21 (1)	4.73
I-J	0 / 29	-122.2	-122.2	0.22 (1)	10.00
S-B	-2575 / 0	0.0	0.0	0.18 (1)	6.47
K-J	-164 / 0	0.0	0.0	0.05 (1)	7.81
S-R	0 / 0	-28.0	-28.0	0.16 (3)	10.00
R-Q	0 / 1986	-28.0	-28.0	0.40 (1)	10.00
Q-P	0 / 1789	-28.0	-28.0	0.37 (1)	10.00
P-O	0 / 1789	-28.0	-28.0	0.37 (1)	10.00
O-N	0 / 2016	-28.0	-28.0	0.41 (1)	10.00
N-M	0 / 1404	-28.0	-28.0	0.44 (2)	10.00
M-L	0 / 1404	-28.0	-28.0	0.44 (2)	10.00
L-K	0 / 1133	-28.0	-28.0	0.41 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-364 / 80	0.19 (1)
C-Q	-301 / 0	0.32 (1)
Q-D	0 / 375	0.08 (2)
D-O	0 / 722	0.12 (1)
O-E	-709 / 0	0.47 (1)
O-G	0 / 288	0.04 (1)
N-G	-946 / 0	0.62 (1)
N-H	0 / 1241	0.20 (1)
L-H	-331 / 64	0.22 (1)
L-I	0 / 600	0.13 (1)
B-R	0 / 2051	0.46 (1)
I-K	-2495 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 599 (0.18")

CSI: TC=0.51 (B-C:1) , BC=0.44 (L-N:2) , WB=0.93 (I-K:1) , SSI=0.31 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.54 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

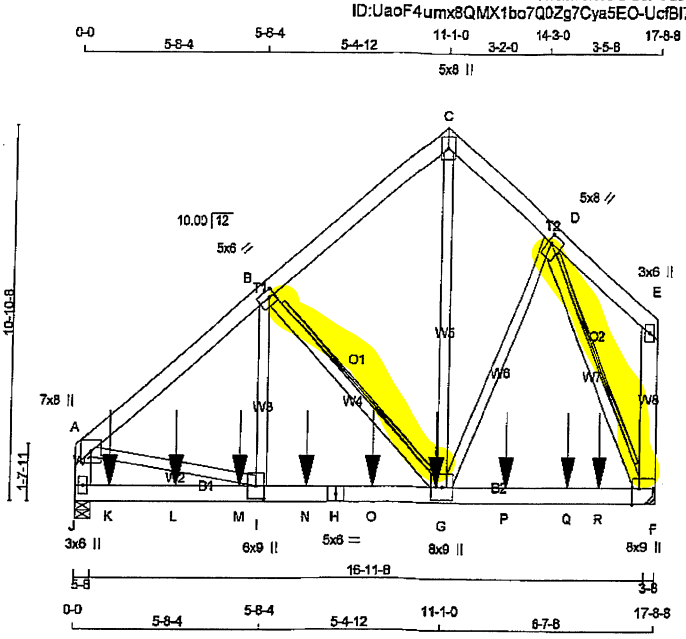
DWG NO. TAM 47950-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T10A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:49 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-UcfBI7O44PT8uESAOUSsp35qQBA8PlWqjpJLE3yZm1e



TOTAL WEIGHT = 3 X 134 = 402 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
H - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
J- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- H	2	6	SIDE(636.0)
H- F	2	5	SIDE(1272.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	7.0	8.0		
B	TMWW-t	MT20	5.0	6.0	2.50	2.00
C	TTW+p	MT20	5.0	8.0		
D	TMWW+t	MT20	5.0	8.0	3.00	1.25
E	TMV+p	MT20	3.0	6.0		
F	BMWW1+t	MT20	8.0	9.0	Edge	2.50
G	BMWWW+t	MT20	8.0	9.0		
H	BS-t	MT20	5.0	8.0		
I	BMWW+t	MT20	6.0	9.0	4.50	2.75
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	12223	0	12223	0	0	5-8	5-8
F	12378	0	12378	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	9501	6287 / 0	1633 / 0	0 / 0	0 / 0	1562 / 0	0 / 0
F	9621	6357 / 0	1653 / 0	0 / 0	0 / 0	1602 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT B-G, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-11312 / 0	-122.2	-122.2	0.19 (1)	4.32
B- C	-7254 / 0	-122.2	-122.2	0.12 (1)	5.22
C- D	-7178 / 0	-122.2	-122.2	0.10 (1)	5.24
D- E	-55 / 0	-122.2	-122.2	0.07 (1)	6.25
J- A	-9273 / 0	0.0	0.0	0.20 (1)	6.01
F- E	-283 / 0	0.0	0.0	0.03 (1)	7.81
J- K	0 / 0	-28.0	-28.0	0.47 (1)	10.00
K- L	0 / 0	-28.0	-28.0	0.47 (1)	10.00
L- M	0 / 0	-28.0	-28.0	0.47 (1)	10.00
M- I	0 / 0	-28.0	-28.0	0.47 (1)	10.00
I- N	0 / 8707	-28.0	-28.0	0.76 (1)	10.00
N- H	0 / 8707	-28.0	-28.0	0.76 (1)	10.00
H- O	0 / 8707	-28.0	-28.0	0.76 (1)	10.00
O- G	0 / 8707	-28.0	-28.0	0.76 (1)	10.00
G- P	0 / 3772	-28.0	-28.0	0.83 (1)	10.00
P- Q	0 / 3772	-28.0	-28.0	0.83 (1)	10.00
Q- R	0 / 3772	-28.0	-28.0	0.83 (1)	10.00
R- F	0 / 3772	-28.0	-28.0	0.83 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	0 / 5268	0.28 (1)
B- G	-4651 / 0	0.35 (1)
G- C	0 / 8675	0.46 (1)
G- D	0 / 4318	0.23 (1)
A- I	0 / 8573	0.48 (1)
D- F	0 / 5268	0.28 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	10-10-12	-2438	-2438	—	FRONT	VERT	TOTAL
K	1-0-4	-2438	-2438	—	FRONT	VERT	TOTAL
L	3-0-4	-2438	-2438	—	FRONT	VERT	TOTAL
M	5-0-4	-2438	-2438	—	FRONT	VERT	TOTAL
N	7-0-4	-2438	-2438	—	FRONT	VERT	TOTAL
O	9-0-4	-2438	-2438	—	FRONT	VERT	TOTAL
P	13-0-4	-2438	-2438	—	FRONT	VERT	TOTAL
Q	15-0-4	-2438	-2438	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 911 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 597 (0.36")

CSI: TC=0.20 (A-J:1) , BC=0.83 (F-G:1) , WB=0.76 (D-F:1) , SSI=0.91 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.69 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4795/-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T10A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-UcfBI7O44PT8uESAOUSsp35qQBA8PiWqjpJLE3yZm1e

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2438.0 lbs FACTORED DOWN AT 1-0-4, 2438.0 lbs FACTORED DOWN AT 3-0-4, 2438.0 lbs FACTORED DOWN AT 5-0-4, 2438.0 lbs FACTORED DOWN AT 7-0-4, 2438.0 lbs FACTORED DOWN AT 9-0-4, 2438.0 lbs FACTORED DOWN AT 10-10-12, 2438.0 lbs FACTORED DOWN AT 13-0-4, AND 2438.0 lbs FACTORED DOWN AT 15-0-4, AND 2438.0 lbs FACTORED DOWN AT 16-0-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	16-0-4	-2438	-2438	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9267
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47954-17 Rev

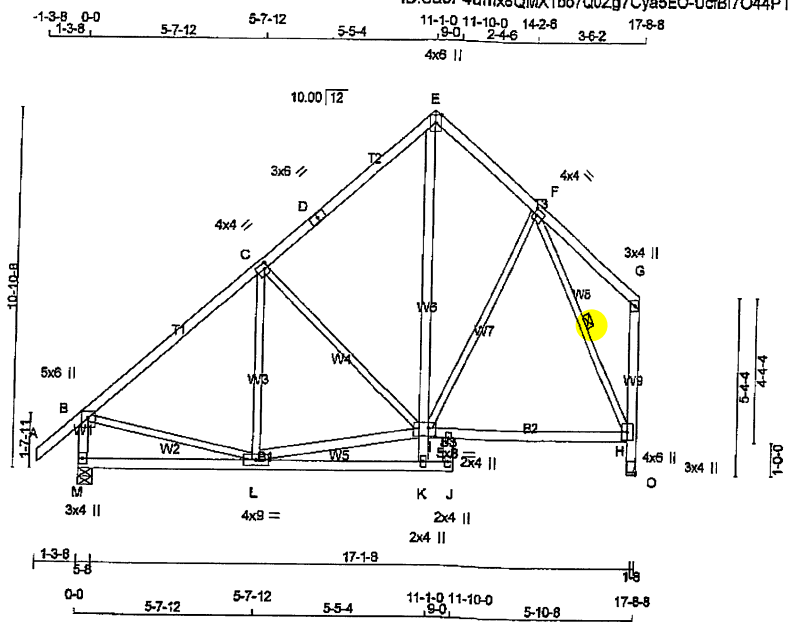
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T11A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2015 MiTek Industries, Inc. Tue Sep 26 13:27:49 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-UcfBI7O44PT8uESAOUSsp35iwBGFPpXqjpJLE3yZm1e



Scale = 1:66.3

TOTAL WEIGHT = 2 X 102 = 203 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF.
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMV+p	MT20	3.0	4.0		
H	BVMW+p	MT20	4.0	6.0		
I	BWMWWW"-I	MT20	5.0	8.0	3.00	2.50
J	NP+w	MT20	2.0	4.0		
K	BMW+w	MT20	2.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMV1+p	MT20	3.0	4.0		
N	NP+w	MT20	2.0	4.0		
O	EBSP-t	MT20	3.0	4.0		1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1509	0	1509	0	0	IN-SX	IN-SX
O	1346	0	1346	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1166	763 / 0	193 / 0	0 / 0	0 / 0	190 / 0	0 / 0
O	1060	677 / 0	198 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	L-C	-210 / 178	0.14 (1)
B-C	-1166 / 0	-122.2	-122.2	0.75 (1)	4.70	K-I	0 / 140	0.03 (2)
C-D	-832 / 0	-122.2	-122.2	0.70 (1)	5.43	I-E	0 / 595	0.11 (1)
D-E	-832 / 0	-122.2	-122.2	0.70 (1)	5.43	B-L	0 / 978	0.22 (1)
E-F	-797 / 0	-122.2	-122.2	0.22 (1)	6.25	I-F	0 / 149	0.03 (2)
F-G	0 / 28	-122.2	-122.2	0.21 (1)	10.00	F-H	-1212 / 0	0.47 (1)
M-B	-1447 / 0	0.0	0.0	0.15 (1)	6.79	L-I	0 / 958	0.15 (1)
O-H	-1346 / 0	0.0	0.0	0.14 (1)	6.99	C-I	-490 / 0	0.50 (1)
H-G	-167 / 0	0.0	0.0	0.06 (1)	7.81			
M-L	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
L-K	0 / 12	-28.0	-28.0	0.24 (3)	10.00			
K-J	0 / 0	-28.0	-28.0	0.03 (2)	10.00			
I-H	0 / 542	-28.0	-28.0	0.43 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 679 (0.24")

CSI: TC=0.75 (B-C:1) , BC=0.43 (H-I:2) , WB=0.50 (C-I:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

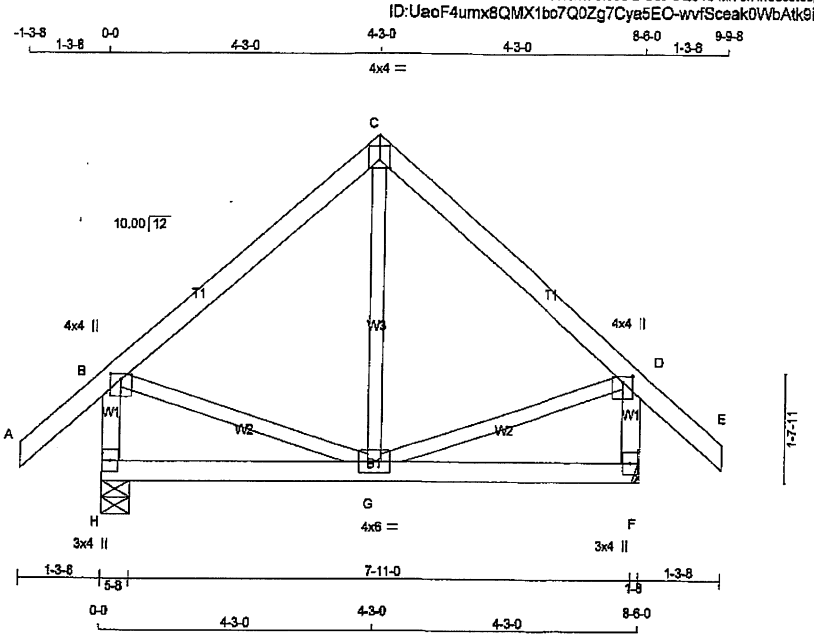
JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 1.00)

DWG NO. TAM 4795217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:47 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-wvfSceak0WbAtk9iUfz3XEofMrZZrQZk68gw9FyZmJU



TOTAL WEIGHT = 2 X 40 = 79 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	808	0	808	0	0	5-8	5-8
F	808	0	808	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	613	431 / 0	89 / 0	0 / 0	0 / 0	93 / 0	0 / 0
F	613	431 / 0	89 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	G-C	-33 / 180	0.04 (3)
B-C	-380 / 0	-122.2	-122.2	0.28 (1)	6.25	B-G	0 / 305	0.07 (1)
C-D	-380 / 0	-122.2	-122.2	0.28 (1)	6.25	G-D	0 / 305	0.07 (1)
D-E	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
H-B	-762 / 0	0.0	0.0	0.08 (1)	7.81			
F-D	-762 / 0	0.0	0.0	0.08 (1)	7.81			
H-G	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
G-F	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

DWG NO. TAM 97973 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.28 (B-C:1) , BC=0.15 (G-H:3) , WB=0.07 (D-G:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

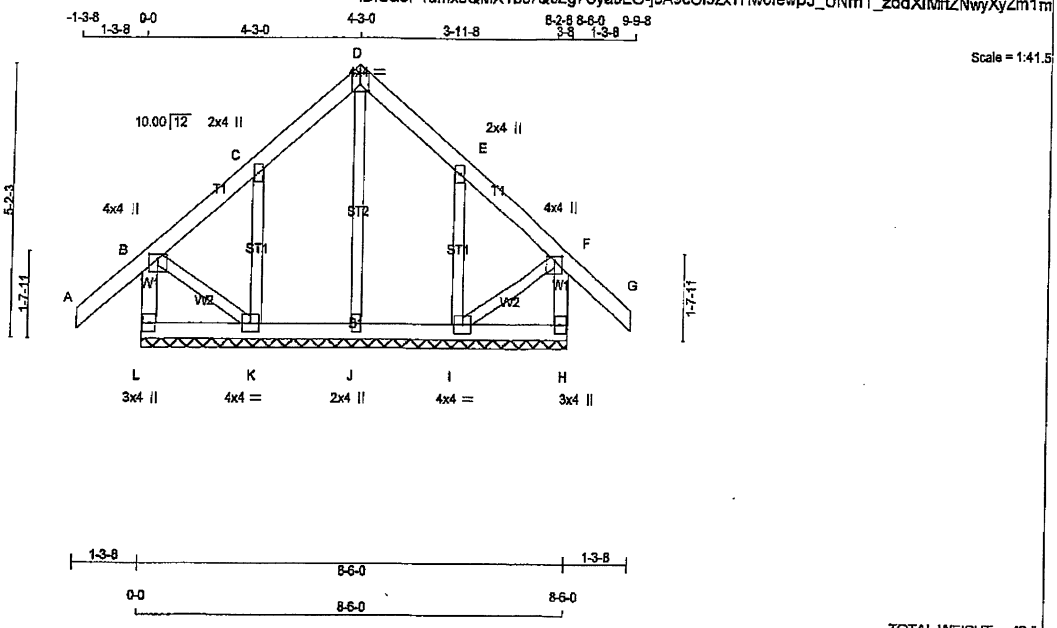
JSI GRIP= 0.64 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	G12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:41 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-j3A9cOl3zxTHw0rewpJ_UNmT_zddXIMftZNwyXyZm1m



TOTAL WEIGHT = 42 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
L- B	-307 / 0	0.0	0.0	0.03 (1)		7.81
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	-14 / 0	-122.2	-122.2	0.09 (1)		6.25
C- D	-42 / 0	-122.2	-122.2	0.09 (1)		6.25
D- E	-42 / 0	-122.2	-122.2	0.09 (1)		6.25
E- F	-14 / 0	-122.2	-122.2	0.09 (1)		6.25
F- G	0 / 54	-122.2	-122.2	0.17 (1)		10.00
H- F	-307 / 0	0.0	0.0	0.03 (1)		7.81
L- K	0 / 0	-28.0	-28.0	0.03 (3)		10.00
K- J	0 / 17	-28.0	-28.0	0.04 (2)		10.00
J- I	0 / 17	-28.0	-28.0	0.04 (2)		10.00
I- H	0 / 0	-28.0	-28.0	0.03 (3)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
J- D	-164 / 0	0.06 (1)
K- C	-315 / 0	0.06 (1)
I- E	-315 / 0	0.06 (1)
B- K	0 / 30	0.01 (1)
I- F	0 / 30	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (F-G:1) , BC=0.04 (J-K:2) , WB=0.06 (C-K:1) , SSI=0.10 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

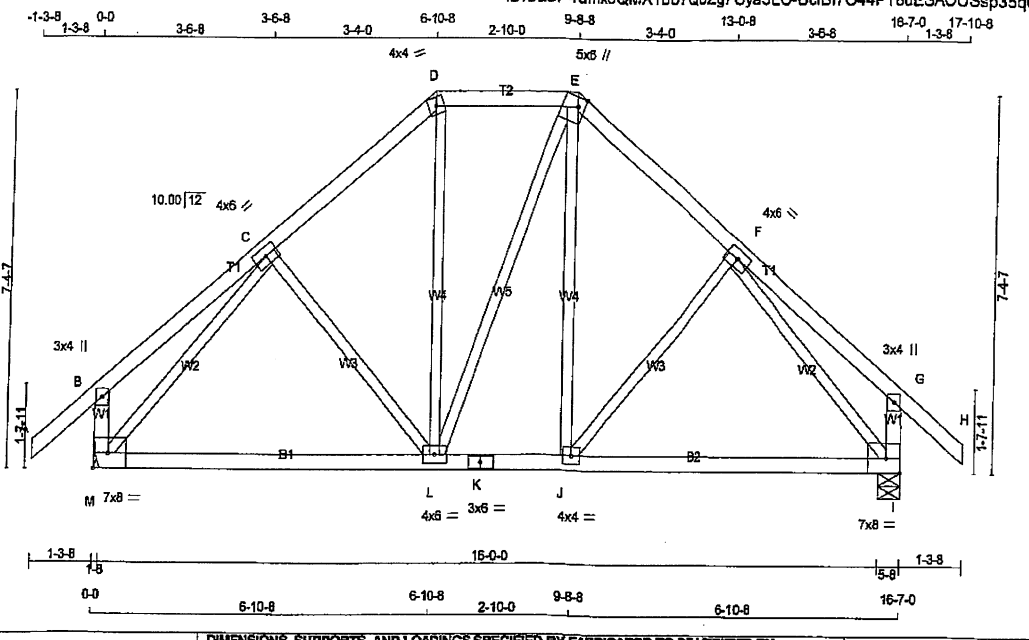
JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

DWG NO. TAM 42961-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:49 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-UcfBI7O44PT8uESAOUSsp35q0BHOPnrqjpJLE3yZm1e



TOTAL WEIGHT = 83 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	7.0	8.0	3.50	3.50
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	7.0	8.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1415	0	1415	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	1415	0	1415	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1088	740 / 0	174 / 0	0 / 0	0 / 0	174 / 0	0 / 0
I	1088	740 / 0	174 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

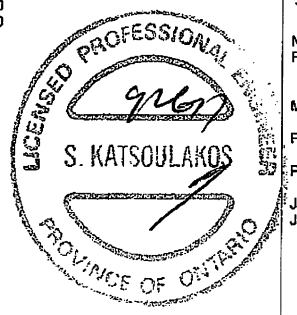
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	0 / 31	-122.2	-122.2	0.23 (1)	10.00
C-D	-974 / 0	-122.2	-122.2	0.18 (1)	6.10
D-E	-727 / 0	-122.2	-122.2	0.13 (1)	6.25
E-F	-973 / 0	-122.2	-122.2	0.18 (1)	6.10
F-G	0 / 31	-122.2	-122.2	0.23 (1)	10.00
G-H	0 / 54	-122.2	-122.2	0.17 (1)	10.00
M-B	-329 / 0	0.0	0.0	0.04 (1)	7.81
I-G	-329 / 0	0.0	0.0	0.04 (1)	7.81
M-L	0 / 825	-28.0	-28.0	0.36 (2)	10.00
L-K	0 / 726	-28.0	-28.0	0.35 (2)	10.00
K-J	0 / 726	-28.0	-28.0	0.35 (2)	10.00
J-I	0 / 824	-28.0	-28.0	0.36 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-157 / 53	0.08 (1)
L-D	0 / 270	0.06 (2)
L-E	0 / 2	0.00 (2)
J-E	0 / 267	0.06 (2)
J-F	-157 / 52	0.08 (1)
M-C	-1299 / 0	0.61 (1)
F-I	-1298 / 0	0.61 (1)



DWG NO. TAM4795317
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.23 (F-G:1) , BC=0.36 (I-J:2) , WB=0.61 (C-M:1) , SSI=0.16 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

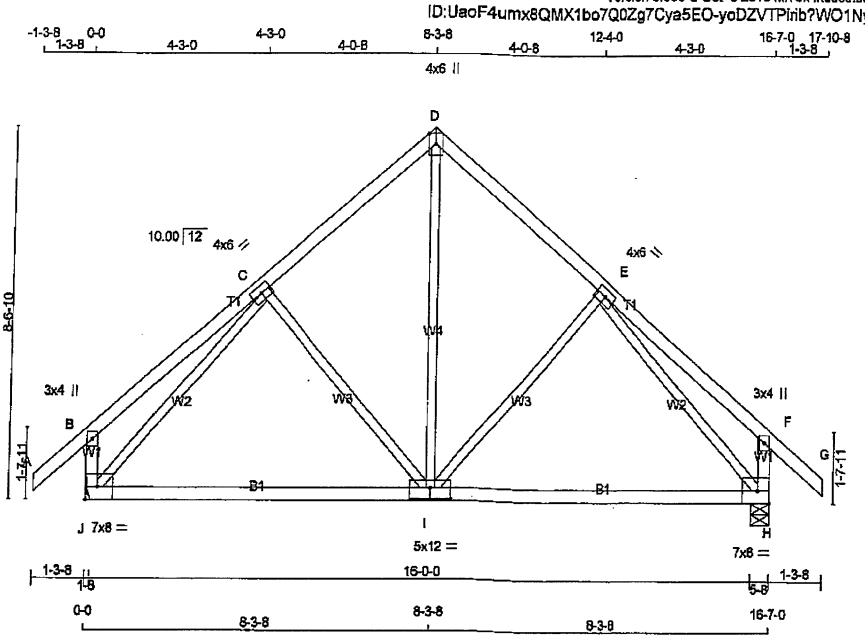
JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.49 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:50 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-yoDZVTPirib?WO1NyCz5LGdz0bYF8Ax_xT3vmVyZm1d



TOTAL WEIGHT = 76 lb [MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	7.0	8.0		Edge
I	BSWWW-t	MT20	5.0	12.0	3.00	6.00
J	BMVW1-t	MT20	7.0	8.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1415	0	1415	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
H	1415	0	1415	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1088	740 / 0	174 / 0	0 / 0	0 / 0	174 / 0	0 / 0
H	1088	740 / 0	174 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	1	0.17 (1)	10.00
B-C	0 / 38	-122.2	-122.2	1	0.34 (1)	10.00
C-D	-892 / 0	-122.2	-122.2	1	0.27 (1)	6.18
D-E	-892 / 0	-122.2	-122.2	1	0.27 (1)	6.18
E-F	0 / 38	-122.2	-122.2	1	0.34 (1)	10.00
F-G	0 / 54	-122.2	-122.2	1	0.17 (1)	10.00
J-B	-361 / 0	0.0	0.0	1	0.04 (1)	7.81
H-F	-361 / 0	0.0	0.0	1	0.04 (1)	7.81
J-I	0 / 839	-28.0	-28.0	2	0.64 (2)	10.00
I-H	0 / 839	-28.0	-28.0	2	0.64 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 702	0.16 (1)
I-E	-276 / 36	0.20 (1)
C-I	-276 / 36	0.20 (1)
J-C	-1279 / 0	0.87 (1)
E-H	-1279 / 0	0.87 (1)

DWG NO. TAM 47954-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/ 862 (0.23")

CSI: TC=0.34 (B-C:1) , BC=0.64 (I-J:2) , WB=0.87 (E-H:1) , SSI=0.20 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

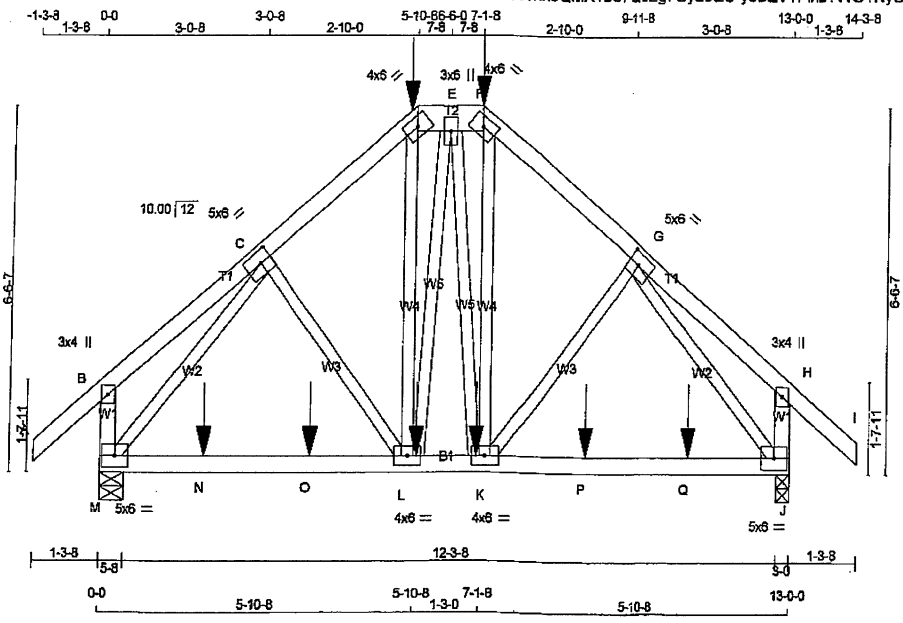
JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.54 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T15	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:50 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-yoDZVTPirib?WO1NyCz5LGd?4bbS8C?_xT3vmVyZm1d



TOTAL WEIGHT = 2 X 75 = 150 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTW-h	MT20	4.0	6.0		
E	TMWW+t	MT20	3.0	6.0		
F	TTW-h	MT20	4.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 705.7 lbs FACTORED DOWN AT 5-10-8, AND 716.7 lbs FACTORED DOWN AT 7-1-8 ON TOP CHORD, AND 77.4 lbs FACTORED DOWN AT 1-11-4, 73.3 lbs FACTORED DOWN AT 3-11-4, 73.3 lbs FACTORED DOWN AT 5-11-4, 73.3 lbs FACTORED DOWN AT 7-0-12, AND 73.3 lbs FACTORED DOWN AT 9-0-12, AND 77.4 lbs FACTORED DOWN AT 11-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION VERT	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1982	0	1982	0	0	5-8	5-8
J	1983	0	1983	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1522	1039 / 0	240 / 0	0 / 0	0 / 0	243 / 0	0 / 0
J	1522	1040 / 0	240 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.19 (1)	10.00
B- C	0 / 24	-122.2	-122.2	0.16 (1)	10.00
C- D	-1691 / 0	-122.2	-122.2	0.21 (1)	4.87
D- E	-1282 / 0	-122.2	-122.2	0.07 (2)	6.25
E- F	-1281 / 0	-122.2	-122.2	0.07 (2)	6.25
F- G	-1693 / 0	-122.2	-122.2	0.21 (1)	4.87
G- H	0 / 24	-122.2	-122.2	0.16 (1)	10.00
H- I	0 / 54	-122.2	-122.2	0.19 (1)	10.00
M- B	-310 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-310 / 0	0.0	0.0	0.04 (1)	7.81
M- N	0 / 1226	-28.0	-28.0	0.49 (2)	10.00
N- O	0 / 1226	-28.0	-28.0	0.49 (2)	10.00
O- L	0 / 1226	-28.0	-28.0	0.49 (2)	10.00
L- K	0 / 1280	-28.0	-28.0	0.50 (2)	10.00
K- P	0 / 1227	-28.0	-28.0	0.49 (2)	10.00
P- Q	0 / 1227	-28.0	-28.0	0.49 (2)	10.00
Q- J	0 / 1227	-28.0	-28.0	0.49 (2)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	0 / 123	0.03 (2)
L- D	0 / 181	0.04 (3)
L- E	0 / 102	0.03 (3)
E- K	0 / 134	0.03 (3)
K- F	0 / 147	0.04 (3)
K- G	0 / 122	0.03 (2)
M- C	-1994 / 0	0.74 (1)
G- J	-1995 / 0	0.74 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-706	-706	—	BACK	VERT	TOTAL
F	7-1-8	-717	-717	—	BACK	VERT	TOTAL
K	7-0-12	-42	-73	—	BACK	VERT	TOTAL
L	5-11-4	-42	-73	—	BACK	VERT	TOTAL
N	1-11-4	-42	-77	—	BACK	VERT	TOTAL
O	3-11-4	-42	-73	—	BACK	VERT	TOTAL
P	9-0-12	-42	-73	—	BACK	VERT	TOTAL
Q	11-0-12	-42	-77	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT. CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.21 (F-G:1) , BC=0.50 (K-L:2) , WB=0.74 (G-J:1) , SSI=0.22 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.49 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9267 S. KATSOULAKOS PROVINCE OF ONTARIO

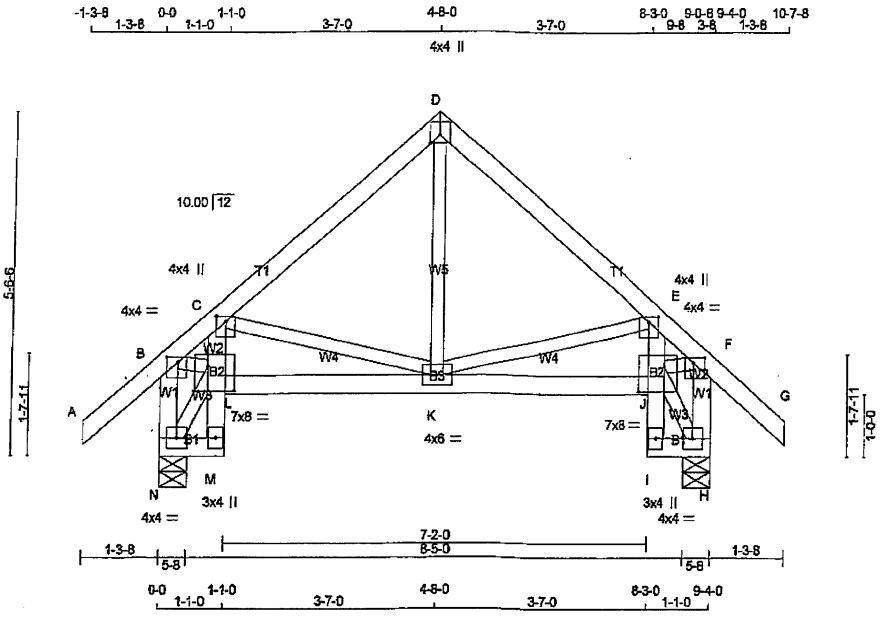
DWG NO. TAM 42955-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T16	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:50 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-yoDZVTPirib?WO1NyCz5LGd?9bgM8L5_xT3vmVyZm1d



TOTAL WEIGHT = 2 X 48 = 96 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.00	2.25
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW-p	MT20	4.0	4.0	1.00	2.25
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	7.0	8.0	4.75	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-l	MT20	7.0	8.0	4.75	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	870	0	870	0	0	5-8	5-8
H	870	0	870	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	662	463 / 0	98 / 0	0 / 0	0 / 0	102 / 0	0 / 0
H	682	463 / 0	98 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

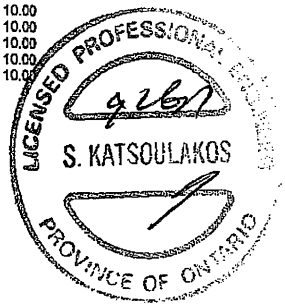
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	-913 / 0	-122.2	-122.2	0.15 (1)	6.25
C-D	-508 / 0	-122.2	-122.2	0.20 (1)	6.25
D-E	-508 / 0	-122.2	-122.2	0.20 (1)	6.25
E-F	-913 / 0	-122.2	-122.2	0.15 (1)	6.25
F-G	0 / 54	-122.2	-122.2	0.17 (1)	10.00
N-B	-814 / 0	0.0	0.0	0.09 (1)	7.81
H-F	-814 / 0	0.0	0.0	0.09 (1)	7.81
N-M	0 / 32	-28.0	-28.0	0.01 (2)	10.00
M-L	0 / 23	0.0	0.0	0.06 (1)	10.00
L-C	0 / 97	0.0	0.0	0.07 (1)	10.00
L-K	0 / 760	-28.0	-28.0	0.19 (2)	10.00
K-J	0 / 760	-28.0	-28.0	0.19 (2)	10.00
I-J	0 / 23	0.0	0.0	0.06 (1)	10.00
J-E	0 / 97	0.0	0.0	0.07 (1)	10.00
I-H	0 / 32	-28.0	-28.0	0.01 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-D	0 / 276	0.06 (2)
K-E	-401 / 0	0.11 (1)
C-K	-401 / 0	0.11 (1)
N-L	-54 / 0	0.01 (1)
B-L	0 / 707	0.16 (1)
J-H	-54 / 0	0.01 (1)
J-F	0 / 707	0.16 (1)



DWG NO. TAM #7956-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20 (D-E:1) , BC=0.19 (J-K:2) , WB=0.16 (F-J:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

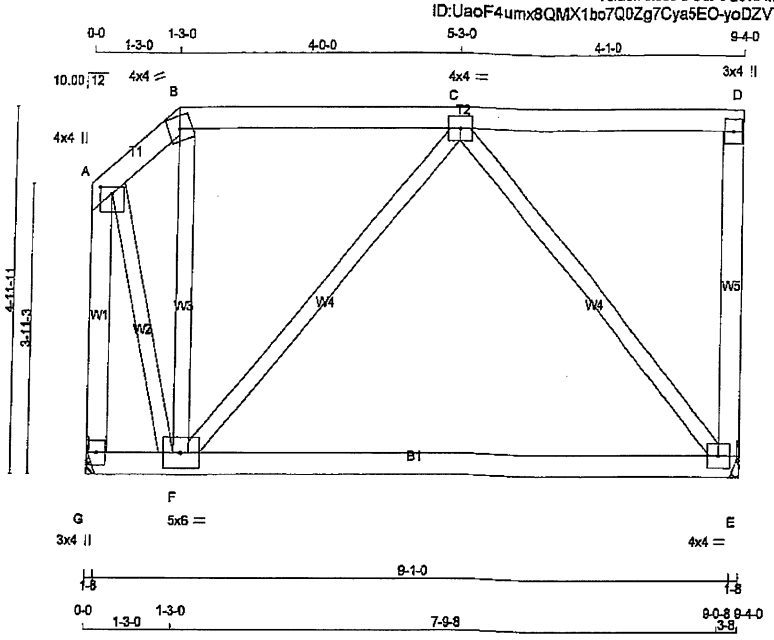
JSI GRIP= 0.66 (B) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:50 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-yoDZVTPirib?WO1NyCz5LGdz7bbt8Ir_xT3vmVyZm1d



Scale = 1:29.7

TOTAL WEIGHT = 47 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-m	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	701	0	701	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
G	701	0	701	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	548	357 / 0	98 / 0	0 / 0	0 / 0	93 / 0	0 / 0
G	548	357 / 0	98 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

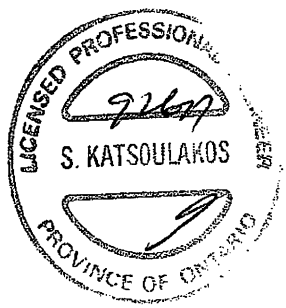
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-276 / 0	-122.2	-122.2	0.03 (1)		6.25
B-C	-208 / 0	-122.2	-122.2	0.33 (1)		6.25
C-D	0 / 0	-122.2	-122.2	0.33 (1)		10.00
E-D	-190 / 0	0.0	0.0	0.08 (1)		7.81
G-A	-815 / 0	0.0	0.0	0.21 (1)		7.81
G-F	0 / 0	-28.0	-28.0	0.45 (3)		10.00
F-E	0 / 368	-28.0	-28.0	0.47 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-62 / 97	0.03 (1)
A-F	0 / 598	0.13 (1)
F-C	-249 / 79	0.16 (1)
C-E	-560 / 0	0.37 (1)



DWG NO. TAM47952-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 829 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 499 (0.22")

CSI: TC=0.33 (B-C:1) , BC=0.47 (E-F:2) , WB=0.37 (C-E:1) , SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

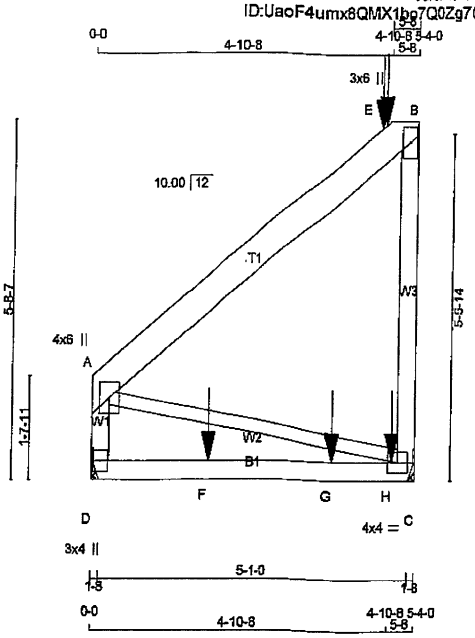
JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T18C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:51 2017 Page 1

ID:UaoF4umx8QMX1bg7Q0Zg7Cya5EO-Q?nxipQKc0js7YcZWvUKuUA4_?yXtqq7A7oSIyyZm1c



Scale = 1:34.8

TOTAL WEIGHT = 30 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMV+p	MT20	3.0	6.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 657.3 lbs FACTORED DOWN AT 4-10-8 ON TOP CHORD, AND 66.6 lbs FACTORED DOWN AT 1-11-4, AND 65.4 lbs FACTORED DOWN AT 3-11-4, AND 68.7 lbs FACTORED DOWN AT 4-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
C	1079	0	1079	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
D	494	0	494	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	616	580 / 0	114 / 0	0 / 0	0 / 0	122 / 0	0 / 0
D	398	239 / 0	84 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-E	0 / 174	-122.2	-122.2	0.44 (1)	6.25	A-C	0 / 0	0.00 (1)
E-B	0 / 174	-122.2	-122.2	0.44 (1)	6.25			
C-B	-927 / 0	0.0	0.0	0.58 (1)	7.81			
D-A	-382 / 0	0.0	0.0	0.04 (1)	7.81			
D-F	0 / 0	-28.0	-28.0	0.45 (2)	10.00			
F-G	0 / 0	-28.0	-28.0	0.45 (2)	10.00			
G-H	0 / 0	-28.0	-28.0	0.45 (2)	10.00			
H-C	0 / 0	-28.0	-28.0	0.45 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
E	4-10-8	-519	-519	—	BACK	VERT	TOTAL
E	4-11-4	-138	-138	—	BACK	VERT	TOTAL
F	1-11-4	-38	-67	—	BACK	VERT	TOTAL
G	3-11-4	-37	-65	—	BACK	VERT	TOTAL
H	4-11-4	-39	-69	—	BACK	VERT	TOTAL

DWG NO. T18H 47958 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 708 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 425 (0.15")

CSI: TC=0.58 (B-C:1) , BC=0.45 (C-D:2) , WB=0.00 (A-C:1) , SSI=0.47 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

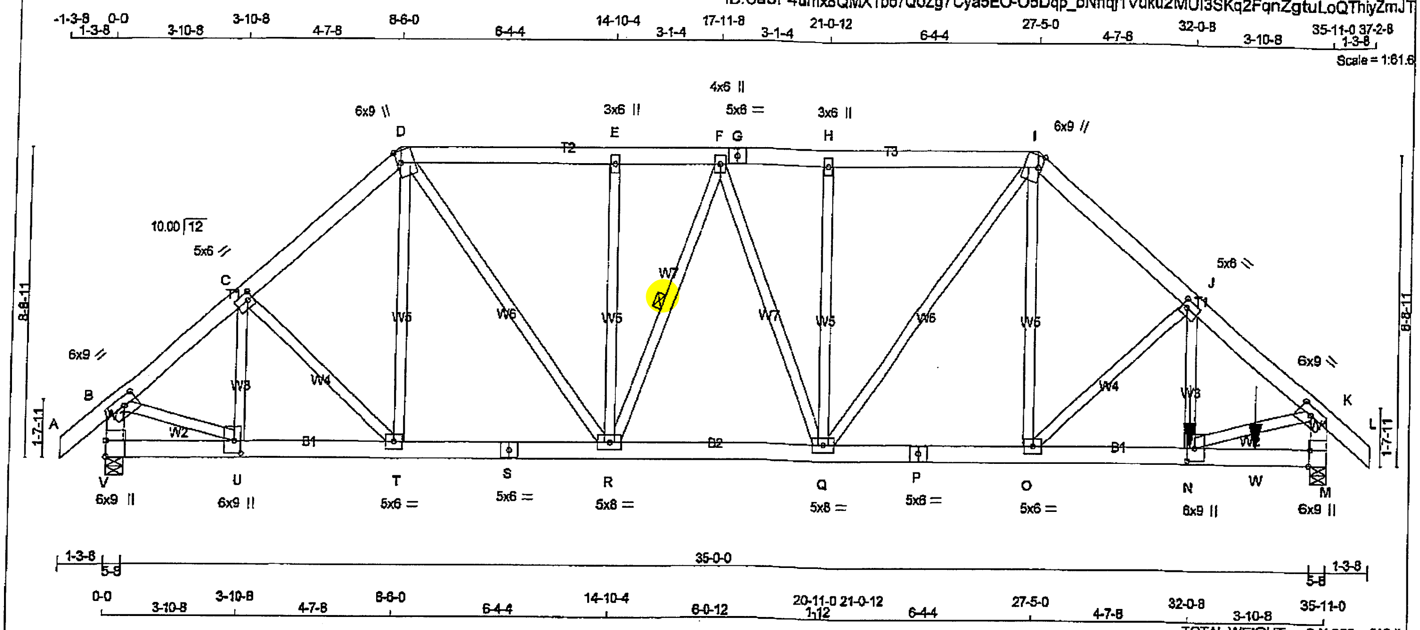
JSI GRIP= 0.55 (C) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T19	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:48 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-O5Dqp_bNnqj1Vuku2MUI3SKq2FqnZgtuLoQThiyZmJT



TOTAL WEIGHT = 2 X 256 = 512 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- G	2	12	TOP
G- I	2	12	TOP
I- L	2	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	TOP
S- P	2	12	TOP
P- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
J- N	2	3	SIDE(1229.1)
2x4	2	3	
T- C	1	6	
T- D	1	6	
R- D	1	6	
E- R	1	6	
H- Q	1	6	
Q- I	1	6	
O- I	1	6	
O- J	1	6	
U- B	1	6	
N- K	1	6	
R- F	1	6	
Q- F	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	6.0	9.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3499	0	3499	0	0	5-8	5-8
M	8440	0	8440	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
V	2708	1812 / 0	452 / 0	0 / 0	0 / 0	444 / 0	0 / 0
M	6488	4420 / 0	1033 / 0	0 / 0	0 / 0	1035 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.05 (1)	10.00
B- C	-3543 / 0	-122.2	-122.2	0.10 (1)	5.85
C- D	-3668 / 0	-122.2	-122.2	0.11 (1)	5.77
D- E	-4022 / 0	-122.2	-122.2	0.19 (1)	5.47
E- F	-4022 / 0	-122.2	-122.2	0.17 (1)	5.49
F- G	-4495 / 0	-122.2	-122.2	0.18 (1)	5.26
G- H	-4495 / 0	-122.2	-122.2	0.18 (1)	5.26
H- I	-4494 / 0	-122.2	-122.2	0.20 (1)	5.24
I- J	-5468 / 0	-122.2	-122.2	0.16 (1)	4.90
J- K	-8807 / 0	-122.2	-122.2	0.19 (1)	3.99
K- L	0 / 56	-122.2	-122.2	0.05 (1)	10.00
V- B	-3437 / 0	0.0	0.0	0.12 (1)	7.53
M- K	-7986 / 0	0.0	0.0	0.29 (1)	5.32
V- U	0 / 0	-28.0	-28.0	0.03 (2)	10.00
U- T	0 / 2750	-28.0	-28.0	0.22 (1)	10.00
T- S	0 / 2787	-28.0	-28.0	0.22 (1)	10.00
S- R	0 / 2787	-28.0	-28.0	0.22 (1)	10.00
R- Q	0 / 4279	-28.0	-28.0	0.31 (1)	10.00
Q- P	0 / 4223	-28.0	-28.0	0.30 (1)	10.00
P- O	0 / 4223	-28.0	-28.0	0.30 (1)	10.00
O- N	0 / 6775	-28.0	-28.0	0.47 (1)	10.00
N- W	0 / 0	-28.0	-28.0	0.30 (1)	10.00
W- M	0 / 0	-28.0	-28.0	0.30 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-704 / 0	0.08 (1)
C- T	0 / 47	0.00 (1)
T- D	0 / 264	0.02 (2)
D- R	0 / 2022	0.18 (1)
R- E	-756 / 0	0.38 (1)
Q- H	-749 / 0	0.38 (1)
Q- I	0 / 445	0.04 (1)
O- I	0 / 2542	0.22 (1)
O- J	-3511 / 0	0.90 (1)
N- J	0 / 3944	0.35 (1)
B- U	0 / 2861	0.25 (1)
N- K	0 / 7049	0.62 (1)
R- F	-729 / 0	0.14 (1)
F- Q	0 / 614	0.05 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	32-0-8	-5413	-5413	—	BACK	VERT	TOTAL
W	33-11-12	-780	-780	—	BACK	VERT	TOTAL

Po/C

DWG NO. TAM 47975-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.29 (K-M:1) , BC=0.47 (N-O:1) , WB=0.90 (J-O:1) , SSI=0.15 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.76 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T19	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-O5Dqp_bNnqj1Vuku2MUI3SKq2FqnZgtuLoQThiyZmJT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	TMWW+t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	3.0	6.0		
J	TTWW+m	MT20	6.0	9.0	4.00	1.25
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMVW-t	MT20	6.0	9.0	2.75	4.25
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWW+t	MT20	6.0	9.0	4.25	2.50
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW+t	MT20	6.0	9.0	4.25	2.50
V	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 5413.4 lbs FACTORED DOWN AT 32-0-8, AND 779.7 lbs FACTORED DOWN AT 33-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

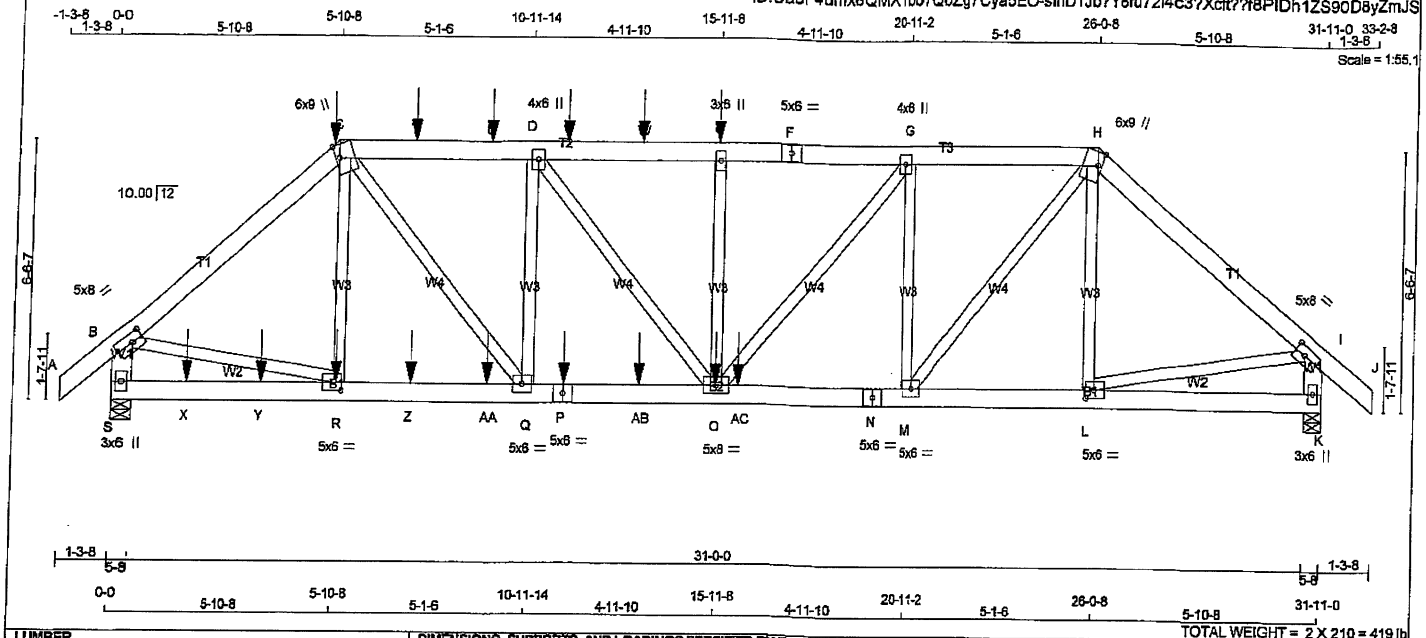
DWG NO. TAM 47975-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T20	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:49 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-slnD1Jb?Y8ru72l4c3?Xcft??f8PIDh1ZS90D8yZmJS



TOTAL WEIGHT = 2 X 210 = 419 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	SIDE(122.0)
C-F	2	12	SIDE(183.1)
F-H	2	12	TOP
H-J	2	12	TOP
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(183.1)
P-N	2	12	SIDE(183.1)
N-K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.00	1.25
I	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	5228	0	5228	0	0	5-8	5-8
K	4194	0	4194	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	4012	2743 / 0	631 / 0	0 / 0	0 / 0	639 / 0	0 / 0
K	3227	2193 / 0	517 / 0	0 / 0	0 / 0	517 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 56	-122.2	-122.2	0.05 (1)	10.00	
B-C	-5842 / 0	-122.2	-122.2	0.27 (1)	4.65	
C-T	-6408 / 0	-122.2	-122.2	0.25 (1)	4.49	
T-U	-6406 / 0	-122.2	-122.2	0.25 (1)	4.49	
U-D	-6406 / 0	-122.2	-122.2	0.25 (1)	4.49	
D-V	-7302 / 0	-122.2	-122.2	0.27 (1)	4.23	
V-W	-7302 / 0	-122.2	-122.2	0.27 (1)	4.23	
W-E	-7302 / 0	-122.2	-122.2	0.27 (1)	4.23	
E-F	-7302 / 0	-122.2	-122.2	0.19 (1)	4.33	
F-G	-7302 / 0	-122.2	-122.2	0.19 (1)	4.33	
G-H	-5851 / 0	-122.2	-122.2	0.16 (1)	4.76	
H-I	-4557 / 0	-122.2	-122.2	0.25 (1)	5.16	
I-J	0 / 56	-122.2	-122.2	0.05 (1)	10.00	
S-B	-5118 / 0	0.0	0.0	0.18 (1)	6.44	
K-I	-4118 / 0	0.0	0.0	0.15 (1)	7.02	
S-X	0 / 0	-28.0	-28.0	0.11 (3)	10.00	
X-Y	0 / 0	-28.0	-28.0	0.11 (3)	10.00	
Y-R	0 / 0	-28.0	-28.0	0.11 (3)	10.00	
R-Z	0 / 4463	-28.0	-28.0	0.35 (1)	10.00	
Z-AA	0 / 4463	-28.0	-28.0	0.35 (1)	10.00	
AA-Q	0 / 4463	-28.0	-28.0	0.35 (1)	10.00	
Q-P	0 / 6407	-28.0	-28.0	0.56 (1)	10.00	
P-AB	0 / 6407	-28.0	-28.0	0.56 (1)	10.00	
AB-O	0 / 6407	-28.0	-28.0	0.56 (1)	10.00	
O-AC	0 / 5851	-28.0	-28.0	0.64 (1)	10.00	
AC-N	0 / 5851	-28.0	-28.0	0.64 (1)	10.00	
N-M	0 / 5851	-28.0	-28.0	0.64 (1)	10.00	
M-L	0 / 3486	-28.0	-28.0	0.30 (1)	10.00	
L-K	0 / 0	-28.0	-28.0	0.06 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-481 / 299	0.11 (1)
C-Q	0 / 3008	0.27 (1)
Q-D	-2165 / 0	0.49 (1)
D-O	0 / 1414	0.12 (1)
O-E	-779 / 0	0.17 (1)
O-G	0 / 2292	0.20 (1)
M-G	-2425 / 0	0.54 (1)
M-H	0 / 3661	0.32 (1)
L-H	-518 / 71	0.12 (1)
B-R	0 / 4551	0.40 (1)
L-I	0 / 3558	0.31 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-33	-36	—	FRONT	VERT	DEAD
C	5-10-8	-688	-688	—	BACK	VERT	TOTAL
C	5-10-8	-482	-482	—	FRONT	VERT	SNOW
E	15-11-4	-147	-147	—	BACK	VERT	TOTAL
O	15-11-4	-42	-73	—	BACK	VERT	TOTAL
P	11-11-4	-42	-73	—	BACK	VERT	TOTAL
R	5-11-4	-42	-73	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.27 (D-E:1) , BC=0.64 (M-O:1) , WB=0.54 (G-M:1) , SSI=0.62 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

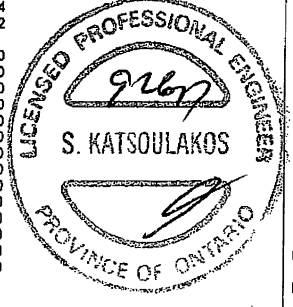
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (R) (INPUT = 0.90)
JSI METAL= 0.58 (B) (INPUT = 1.00)



DWG NO. TAM 47976-17 PG 1/2

STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T20	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 514.0 lbs FACTORED DOWN AT 5-10-8, 688.1 lbs FACTORED DOWN AT 5-10-8, 147.1 lbs FACTORED DOWN AT 7-11-4, 147.1 lbs FACTORED DOWN AT 9-11-4, 147.1 lbs FACTORED DOWN AT 11-11-4, AND 147.1 lbs FACTORED DOWN AT 13-11-4, AND 147.1 lbs FACTORED DOWN AT 15-11-4 ON TOP CHORD, AND 88.9 lbs FACTORED DOWN AT 1-11-4, 88.9 lbs FACTORED DOWN AT 3-11-4, 73.3 lbs FACTORED DOWN AT 5-11-4, 73.3 lbs FACTORED DOWN AT 7-11-4, 73.3 lbs FACTORED DOWN AT 9-11-4, 73.3 lbs FACTORED DOWN AT 11-11-4, 73.3 lbs FACTORED DOWN AT 13-11-4, AND 73.3 lbs FACTORED DOWN AT 15-11-4, AND 1986.0 lbs FACTORED DOWN AT 16-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	7-11-4	-147	-147	—	BACK	VERT	TOTAL
U	9-11-4	-147	-147	—	BACK	VERT	TOTAL
V	11-11-4	-147	-147	—	BACK	VERT	TOTAL
W	13-11-4	-147	-147	—	BACK	VERT	TOTAL
X	1-11-4	-51	-89	—	BACK	VERT	TOTAL
Y	3-11-4	-51	-89	—	BACK	VERT	TOTAL
Z	7-11-4	-42	-73	—	BACK	VERT	TOTAL
AA	9-11-4	-42	-73	—	BACK	VERT	TOTAL
AB	13-11-4	-42	-73	—	BACK	VERT	TOTAL
AC	16-6-8	-1986	-1986	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

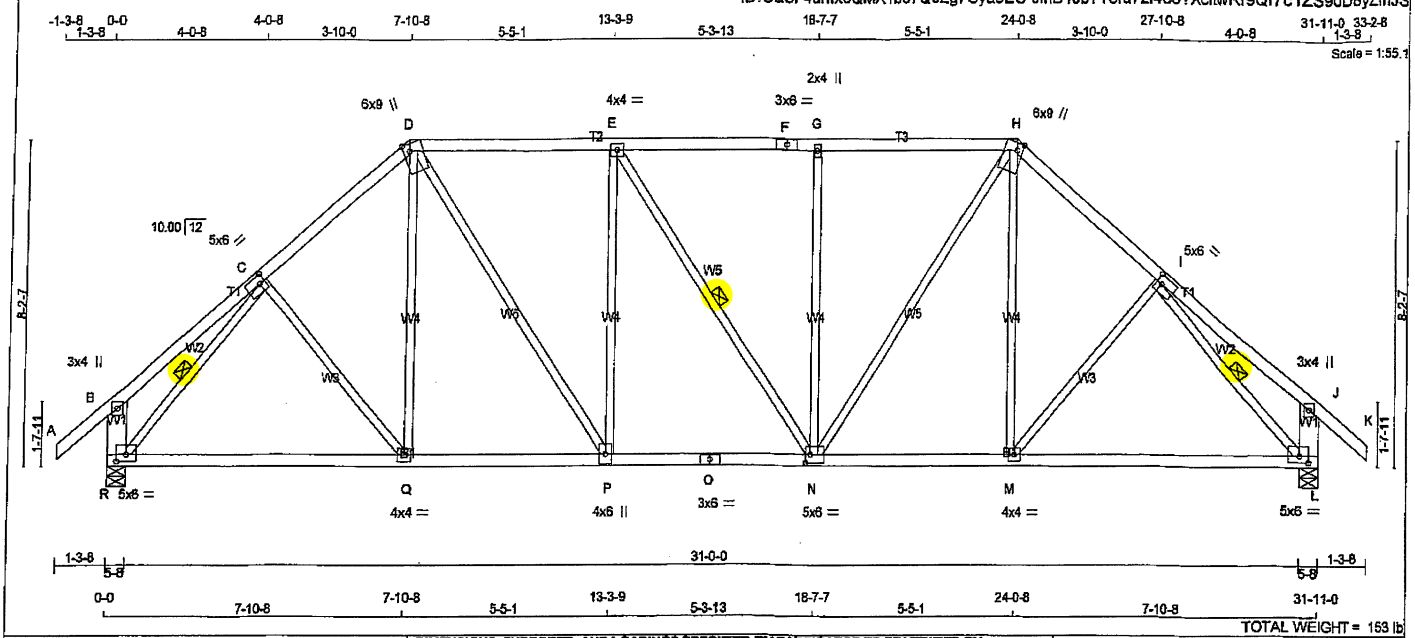
DWG NO. T4M 47926-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:49 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-sInD1Jb?Y8ru72l4c3?XcftwRf9QI7c1ZS90D8yZmJS



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	1.75
J	TMV+p	MT20	3.0	4.0		
L	BMW1-t	MT20	5.0	6.0	2.00	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	1.50
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMW1-t	MT20	5.0	6.0	2.00	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
R	2566	0	2566	0	0	IN-SX	IN-SX
L	2566	0	2566	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1989	1326 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
L	1989	1326 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C-Q	0 / 133	0.03 (3)
B-C	0 / 34	-122.2	-122.2	0.29 (1)	10.00	Q-D	0 / 316	0.07 (2)
C-D	-2410 / 0	-122.2	-122.2	0.41 (1)	4.04	D-P	0 / 1017	0.23 (1)
D-E	-2399 / 0	-122.2	-122.2	0.63 (1)	3.72	P-E	-709 / 0	0.93 (1)
E-F	-2397 / 0	-122.2	-122.2	0.63 (1)	3.73	E-N	-4 / 0	0.00 (1)
F-G	-2397 / 0	-122.2	-122.2	0.63 (1)	3.73	N-G	-707 / 0	0.93 (1)
G-H	-2397 / 0	-122.2	-122.2	0.62 (1)	3.75	N-H	0 / 1013	0.23 (1)
H-I	-2410 / 0	-122.2	-122.2	0.41 (1)	4.04	M-H	0 / 317	0.07 (2)
I-J	0 / 34	-122.2	-122.2	0.29 (1)	10.00	M-I	0 / 133	0.03 (3)
J-K	0 / 54	-122.2	-122.2	0.17 (1)	10.00	R-C	-2791 / 0	0.69 (1)
R-B	-355 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-2791 / 0	0.69 (1)
L-J	-355 / 0	0.0	0.0	0.02 (1)	7.81			
R-Q	0 / 1816	-28.0	-28.0	0.56 (2)	10.00			
Q-P	0 / 1823	-28.0	-28.0	0.57 (2)	10.00			
P-O	0 / 2400	-28.0	-28.0	0.47 (1)	10.00			
O-N	0 / 2400	-28.0	-28.0	0.47 (1)	10.00			
N-M	0 / 1823	-28.0	-28.0	0.57 (2)	10.00			
M-L	0 / 1816	-28.0	-28.0	0.56 (2)	10.00			

DWG NO. TAM 47972-17
STRUCTURAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.63 (D-E:1) , BC=0.57 (M-N:2) , WB=0.93 (E-P:1) , SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

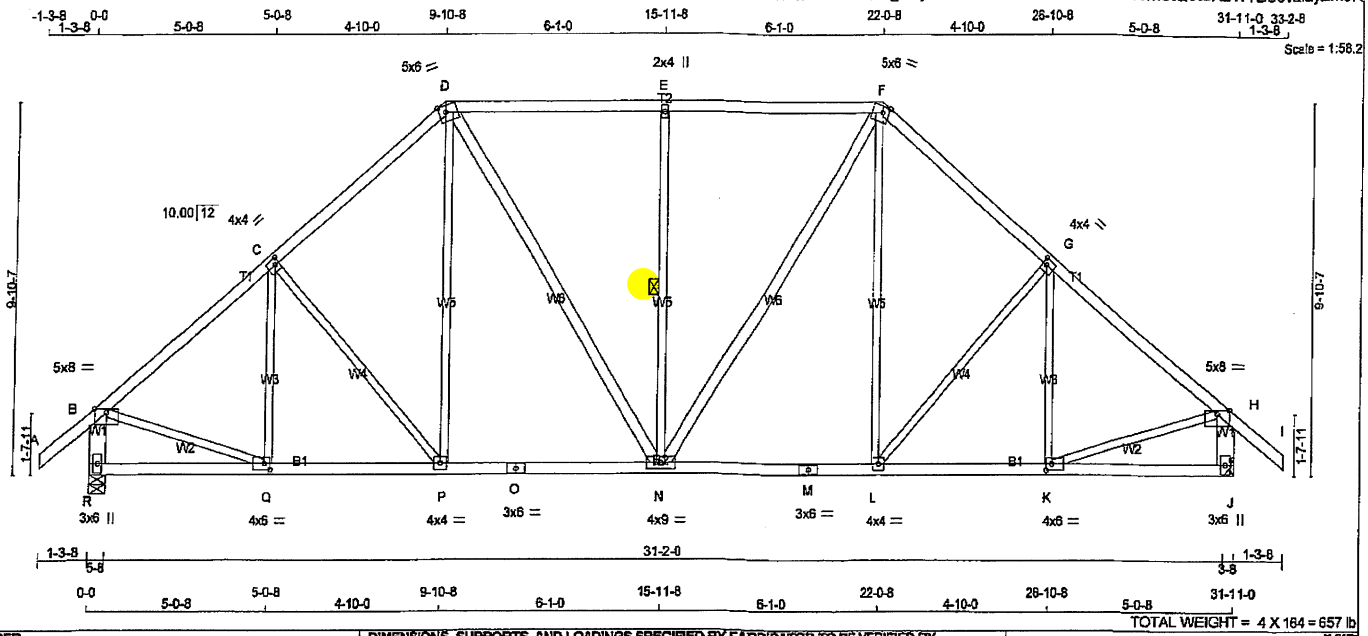
JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.73 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T22	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:50 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-KULbEfcdJRzlkCtGAnWm8tQ3t3XL1f1Bo6valayZmJR



TOTAL WEIGHT = 4 X 164 = 657 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	8.0	2.00	2.00
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2566	0	2566	0	0	5-8	5-8
J	2566	0	2566	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1989	1326 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	1989	1326 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

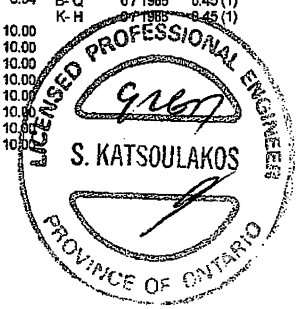
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	-2457 / 0	-122.2	-122.2	0.50 (1)	3.92
C-D	-2284 / 0	-122.2	-122.2	0.47 (1)	4.07
D-E	-2063 / 0	-122.2	-122.2	0.65 (1)	3.90
E-F	-2063 / 0	-122.2	-122.2	0.65 (1)	3.90
F-G	-2284 / 0	-122.2	-122.2	0.47 (1)	4.07
G-H	-2457 / 0	-122.2	-122.2	0.50 (1)	3.92
H-I	0 / 54	-122.2	-122.2	0.17 (1)	10.00
R-B	-2506 / 0	0.0	0.0	0.17 (1)	6.54
J-H	-2506 / 0	0.0	0.0	0.17 (1)	6.54
R-Q	0 / 0	-28.0	-28.0	0.16 (3)	10.00
Q-P	0 / 1922	-28.0	-28.0	0.40 (1)	10.00
P-O	0 / 1719	-28.0	-28.0	0.39 (1)	10.00
O-N	0 / 1719	-28.0	-28.0	0.39 (1)	10.00
N-M	0 / 1719	-28.0	-28.0	0.39 (1)	10.00
M-L	0 / 1719	-28.0	-28.0	0.39 (1)	10.00
L-K	0 / 1922	-28.0	-28.0	0.40 (1)	10.00
K-J	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
Q-C	-350 / 82	0.16 (1)
C-P	-312 / 0	0.34 (1)
P-D	0 / 396	0.09 (2)
D-N	0 / 642	0.10 (1)
N-E	-909 / 0	0.60 (1)
N-F	0 / 642	0.10 (1)
L-F	0 / 396	0.09 (2)
L-G	-312 / 0	0.34 (1)
K-G	-350 / 82	0.16 (1)
B-Q	0 / 1965	0.45 (1)
K-H	0 / 1965	0.45 (1)



DWG NO. TAM 4797817

STRUCTURAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.65 (D-E:1) , BC=0.40 (P-Q:1) , WB=0.60 (E-N:1) , SSI=0.36 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

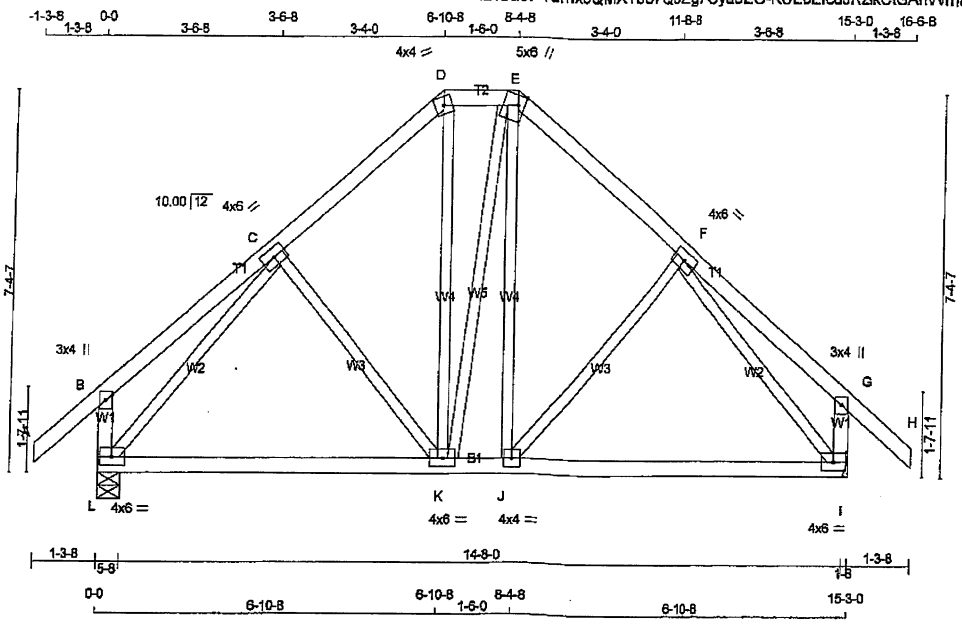
JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.51 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:50 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-KULbEfcdJRzlkCtGAnWm8tQAS3XV1gmBo6valayZmJR



Scale = 1:42.3

TOTAL WEIGHT = 80 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1315	0	1315	0	0	5-8	5-8
I	1315	0	1315	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1010	669 / 0	160 / 0	0 / 0	0 / 0	161 / 0	0 / 0
I	1010	669 / 0	160 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B-C	0 / 31	-122.2	-122.2	0.23 (1)	10.00	
C-D	-850 / 0	-122.2	-122.2	0.18 (1)	6.25	
D-E	-632 / 0	-122.2	-122.2	0.04 (1)	6.25	
E-F	-848 / 0	-122.2	-122.2	0.18 (1)	6.25	
F-G	0 / 31	-122.2	-122.2	0.23 (1)	10.00	
G-H	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
L-B	-329 / 0	0.0	0.0	0.04 (1)	7.81	
I-G	-329 / 0	0.0	0.0	0.04 (1)	7.81	
L-K	0 / 746	-28.0	-28.0	0.39 (2)	10.00	
K-J	0 / 630	-28.0	-28.0	0.39 (2)	10.00	
J-I	0 / 744	-28.0	-28.0	0.38 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-K	-180 / 45	0.09 (1)
K-D	0 / 271	0.05 (1)
K-E	0 / 8	0.00 (1)
J-E	0 / 262	0.06 (1)
J-F	-181 / 44	0.09 (1)
L-C	-1174 / 0	0.55 (1)
F-I	-1172 / 0	0.55 (1)

DWG NO. TAM 47979-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.23 (F-G:1) , BC=0.39 (K-L:2) , WB=0.55 (C-L:1) , SSI=0.16 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

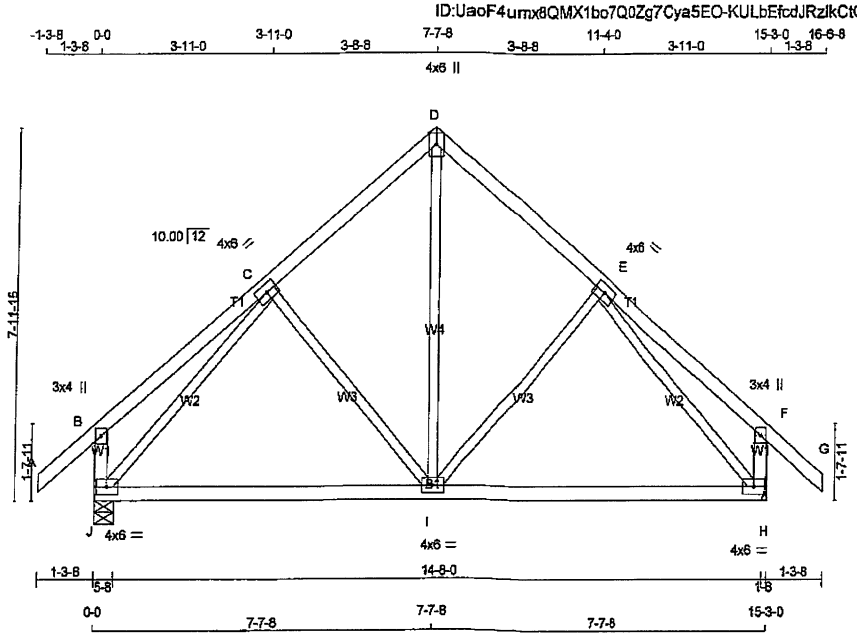
JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:50 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-KULbEfcdJRzlkCtGAnWm8tQ9a3V21eyBo6valayZmJR



TOTAL WEIGHT = 71 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
J	1315	0	1315	0	0	IN-SX	IN-SX
H	1315	0	1315	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1010	689 / 0	160 / 0	0 / 0	0 / 0	161 / 0	0 / 0
H	1010	689 / 0	160 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 630	0.14 (1)
B- C	0 / 35	-122.2	-122.2	0.29 (1)	10.00	I- E	-241 / 37	0.14 (1)
C- D	-809 / 0	-122.2	-122.2	0.22 (1)	6.25	C- I	-241 / 37	0.14 (1)
D- E	-809 / 0	-122.2	-122.2	0.22 (1)	6.25	J- C	-1164 / 0	0.67 (1)
E- F	0 / 35	-122.2	-122.2	0.29 (1)	10.00	E- H	-1164 / 0	0.67 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
J- B	-346 / 0	0.0	0.0	0.04 (1)	7.81			
H- F	-346 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 753	-28.0	-28.0	0.55 (2)	10.00			
I- H	0 / 753	-28.0	-28.0	0.55 (2)	10.00			

DWG NO. TAM 47988017

STRUCTURAL

COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.29 (E-F:1) , BC=0.55 (H-I:2) , WB=0.67 (C-J:1) , SSI=0.18 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

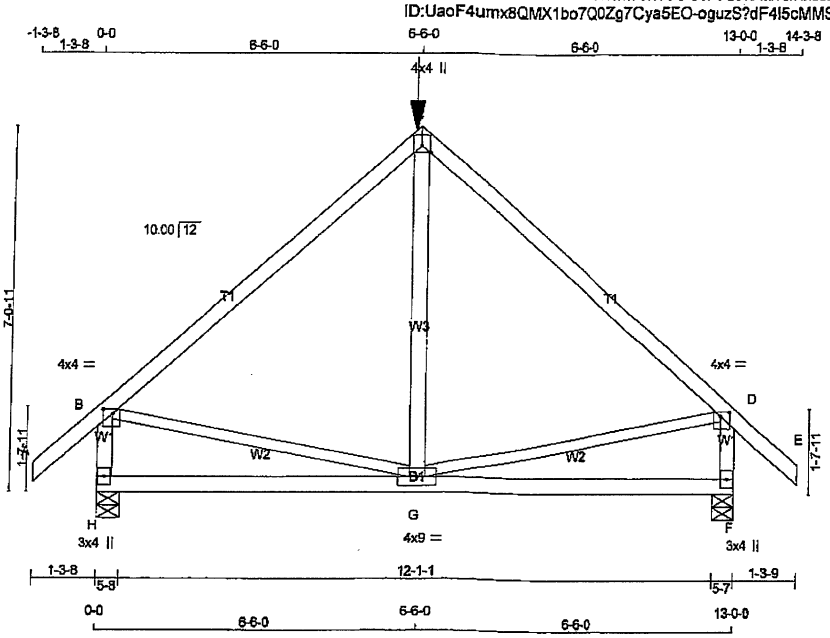
JSI GRIP= 0.75 (E) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T25	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:51 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-oguzS?dF4l5cMMSTjU1?h4yGwSupmDAK1me7l0yZmJQ



TOTAL WEIGHT = 2 X 59 = 117 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
H-B	1	12	TOP
F-D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H-F	1	12	SIDE(24.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
G-C	1	6	SIDE(223.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.00	2.25
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW-p	MT20	4.0	4.0	1.00	2.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1324.0 lbs FACTORED DOWN AT 6-6-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1992	0	1992	0	0	5-8	5-8
F	1992	0	1992	0	0	5-7	5-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1554	1017 / 0	274 / 0	0 / 0	0 / 0	263 / 0	0 / 0
F	1554	1017 / 0	274 / 0	0 / 0	0 / 0	263 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 54	-122.2	-122.2	0.09 (1)	10.00	G-C	-47 / 618	0.05 (3)
B-C	-1618 / 0	-122.2	-122.2	0.57 (1)	5.87	B-G	0 / 1268	0.16 (1)
C-D	-1618 / 0	-122.2	-122.2	0.57 (1)	5.87	G-D	0 / 1268	0.16 (1)
D-E	0 / 54	-122.2	-122.2	0.09 (1)	10.00			
H-B	-1652 / 0	0.0	0.0	0.11 (1)	7.81			
F-D	-1652 / 0	0.0	0.0	0.11 (1)	7.81			
H-G	0 / 0	-56.3	-56.3	0.38 (3)	10.00			
G-F	0 / 0	-56.3	-56.3	0.38 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-6-0	-1324	-1324	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9161
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47981-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-6-0
END SETBACK = 6-6-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.57 (C-D:1) , BC=0.38 (G-H:3) , WB=0.16 (B-G:1) , SSI=0.17 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

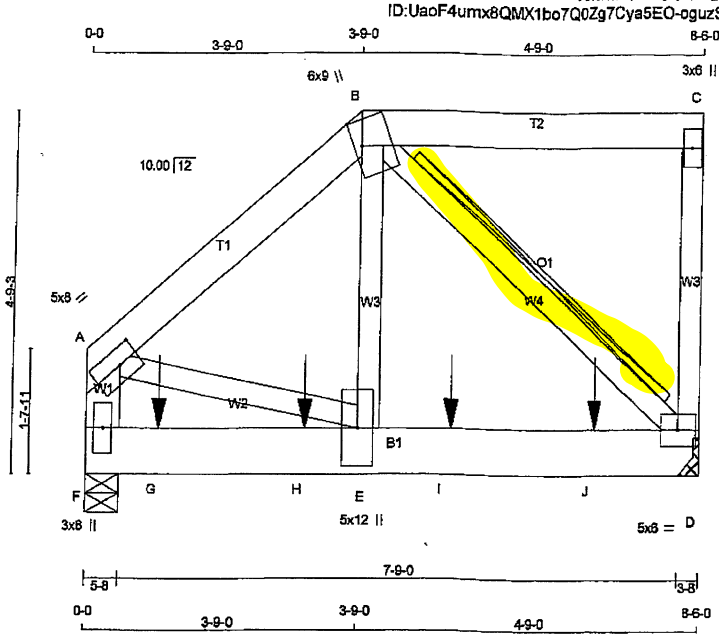
JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T26	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:51 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-oguzS?dF4l5cMMSTjU1?h4yMUSmFm5zK1me7l0yZmJQ



Scale = 1:28.9

TOTAL WEIGHT = 2 X 58 = 116 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	2	12	TOP
B- C	2	12	TOP
F- A	2	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-t	MT20	5.0	8.0	2.50	3.25
B	TTWW+m	MT20	6.0	9.0	4.00	1.25
C	TMV+p	MT20	3.0	6.0		
D	BMWW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	5.0	12.0		
F	BMV1+p	MT20	3.0	8.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2538.4 lbs FACTORED DOWN AT 1-0-4, 2538.4 lbs FACTORED DOWN AT 3-0-4, AND 2538.4 lbs FACTORED DOWN AT 5-0-4, AND 2538.4 lbs FACTORED DOWN AT 7-0-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	5442	0	5442	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
F	5989	0	5989	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	4197	2635 / 0	684 / 0	0 / 0	0 / 0	678 / 0	0 / 0
F	4618	3121 / 0	751 / 0	0 / 0	0 / 0	748 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4821 / 0	-122.2	-122.2	0.11 (1)	5.28	
B- C	0 / 0	-122.2	-122.2	0.13 (1)	10.00	
D- C	-290 / 0	0.0	0.0	0.04 (1)	7.81	
F- A	-4150 / 0	0.0	0.0	0.15 (1)	7.00	
F- G	0 / 0	-28.0	-28.0	0.63 (1)	10.00	
G- H	0 / 0	-28.0	-28.0	0.63 (1)	10.00	
H- E	0 / 0	-28.0	-28.0	0.63 (1)	10.00	
E- I	0 / 3734	-28.0	-28.0	0.87 (1)	10.00	
I- J	0 / 3734	-28.0	-28.0	0.87 (1)	10.00	
J- D	0 / 3734	-28.0	-28.0	0.87 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 5783	0.51 (1)
B- D	-4984 / 0	0.68 (1)
A- E	0 / 3857	0.32 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
G	1-0-4	-2538	-2538	—	FRONT	VERT	TOTAL
H	3-0-4	-2538	-2538	—	FRONT	VERT	TOTAL
I	5-0-4	-2538	-2538	—	FRONT	VERT	TOTAL
J	7-0-4	-2538	-2538	—	FRONT	VERT	TOTAL

LICENCED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.15 (A-F:1) , BC=0.87 (D-E:1) , WB=0.68 (B-D:1) , SSI=0.98 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

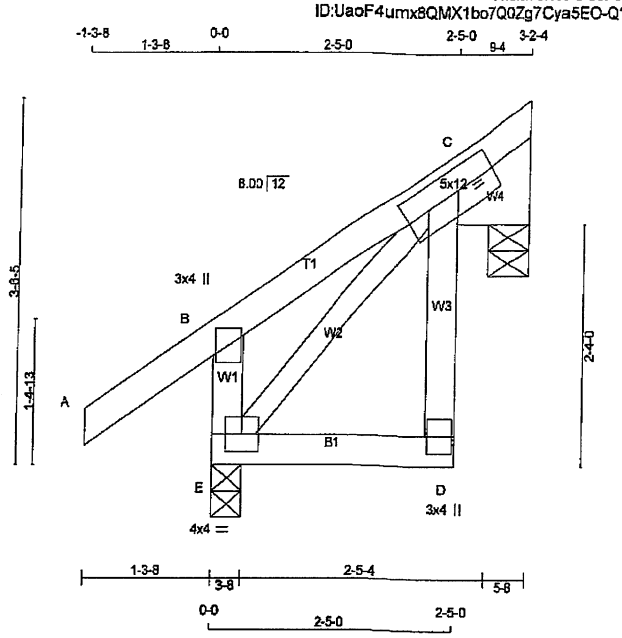
JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.49 (E) (INPUT = 1.00)

DWG NO. TAM 47982-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T90	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:51 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-Q?nxipQKc0Js7YcZWvUKuUABW?2ktqq7A7oSlyyZm1c



TOTAL WEIGHT = 9 X 18 = 160 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	5.0	12.0	2.50	3.00
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	182	0	182	0	0	5-8	5-8
E	349	0	349	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	142	92 / 0	25 / 0	0 / 0	0 / 0	24 / 0	0 / 0
E	255	197 / 0	25 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-122.2	-122.2	0.16 (1)	10.00
B- C	0 / 0	-122.2	-122.2	0.12 (1)	10.00
D- C	0 / 59	0.0	0.0	0.01 (3)	10.00
E- B	-315 / 0	0.0	0.0	0.03 (1)	7.81
E- D	0 / 0	-28.0	-28.0	0.05 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- C	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DWG NO. TAM 47959-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.16 (A-B:1) , BC=0.05 (D-E:3) , WB=0.00 (C-E:1) , SSI=0.11 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

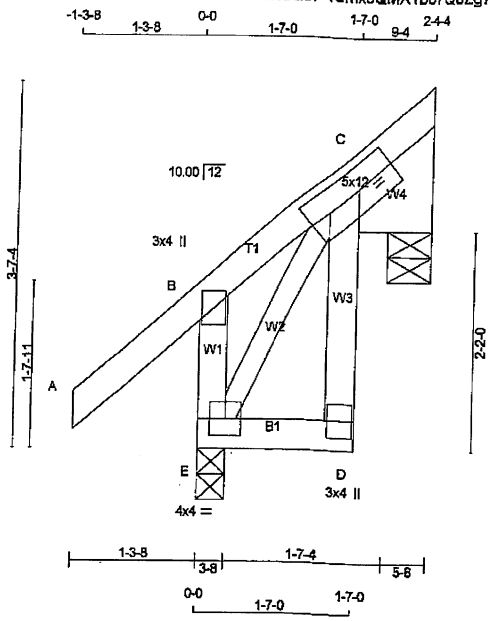
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	T91	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:51 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-Q?nxipQKc0js7YcZWvUKuUABS?2Atqq7A7oSlyyZm1c



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	5.0	12.0	2.50	2.50
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	45	0	47	0	-56	5-8	5-8
E	362	0	362	0	0	3-8	3-8

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	43	14 / -46	17 / 0	0 / 0	0 / 0	12 / 0	0 / 0
E	257	213 / 0	17 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	-47 / 0	-122.2	-122.2	0.16 (1)	6.25			
D- C	0 / 39	0.0	0.0	0.01 (3)	10.00			
E- B	-340 / 0	0.0	0.0	0.04 (1)	7.81			
E- D	0 / 0	-28.0	-28.0	0.02 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DWG NO. TAM42960-17
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 10 X 17 = 165 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.17 (A-B:1) , BC=0.02 (D-E:3) , WB=0.00 (C-E:1) , SSP=0.10 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

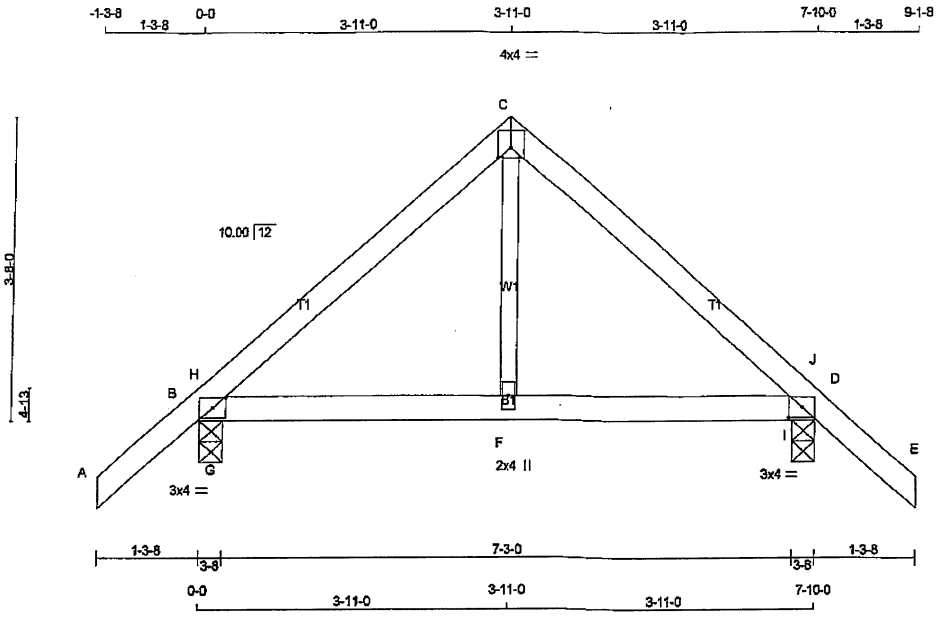
JSI GRIP= 0.20 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T93	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:51 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-oguzS?dF4l5cMMSTjU1?h4yLSSw3mFjK1me7l0yZmJQ



TOTAL WEIGHT = 2 X 27 = 55 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	758	0	758	0	0	3-8	3-8
D	758	0	758	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	574	405 / 0	82 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	574	405 / 0	82 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 52	-122.2	-122.2	0.17 (1)		10.00
B-H	-316 / 0	-122.2	-122.2	0.10 (1)		6.25
H-C	-511 / 0	-122.2	-122.2	0.21 (1)		6.25
C-J	-511 / 0	-122.2	-122.2	0.21 (1)		6.25
J-D	-316 / 0	-122.2	-122.2	0.10 (1)		6.25
D-E	0 / 52	-122.2	-122.2	0.17 (1)		10.00
B-G	0 / 380	-28.0	-28.0	0.24 (1)		10.00
G-F	0 / 380	-28.0	-28.0	0.24 (1)		10.00
F-I	0 / 380	-28.0	-28.0	0.24 (1)		10.00
I-D	0 / 380	-28.0	-28.0	0.24 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 256	0.06 (2)
G-H	-550 / 96	0.00 (1)
I-J	-550 / 96	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.21 (C-J:1) , BC=0.24 (F-I:1) , WB=0.06 (C-F:2) , SSI=0.42 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

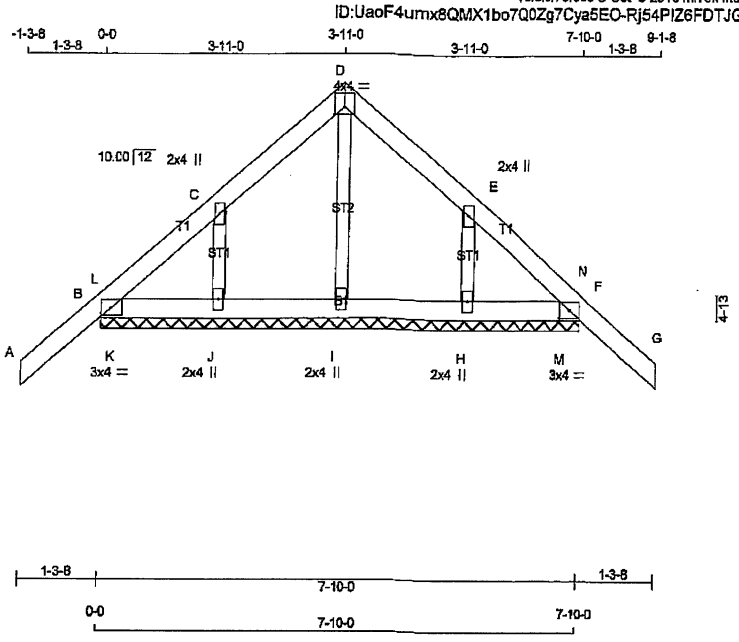
JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47985 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	G93	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 30 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 52	-122.2	-122.2	0.17 (1)		10.00
B- L	-291 / 0	-122.2	-122.2	0.16 (1)		6.25
L- C	-43 / 0	-122.2	-122.2	0.06 (1)		6.25
C- D	-40 / 0	-122.2	-122.2	0.06 (1)		6.25
D- E	-40 / 0	-122.2	-122.2	0.05 (1)		6.25
E- N	-43 / 0	-122.2	-122.2	0.05 (1)		6.25
N- F	-291 / 0	-122.2	-122.2	0.16 (1)		6.25
F- G	0 / 52	-122.2	-122.2	0.17 (1)		10.00
B- K	0 / 35	-28.0	-28.0	0.06 (1)		10.00
K- J	0 / 35	-28.0	-28.0	0.05 (1)		10.00
J- I	0 / 21	-28.0	-28.0	0.03 (2)		10.00
I- H	0 / 21	-28.0	-28.0	0.03 (2)		10.00
H- M	0 / 35	-28.0	-28.0	0.06 (1)		10.00
M- F	0 / 35	-28.0	-28.0	0.06 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
I- D	-160 / 0	0.04 (1)
J- C	-237 / 0	0.03 (1)
H- E	-237 / 0	0.03 (1)
K- L	0 / 225	0.00 (1)
M- N	0 / 225	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.06 (J-K:1) , WB=0.04 (D-I:1) , SSI=0.18 (B-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (D) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

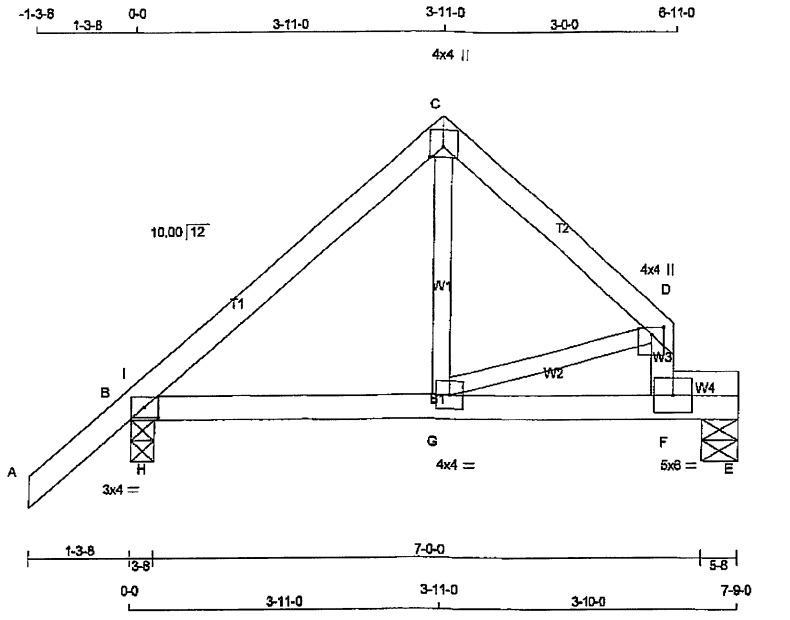
DWG NO. TAM 4798717
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	T93A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:52 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-GtSLfLetr3DT_W1fHCYEEIVW3sAeVhUUGQOhqTyZmJP



TOTAL WEIGHT = 2 X 28 = 56 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMVW-t	MT20	5.0	6.0		
G	BMWW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	746	0	748	0	0	3-8	3-8
E	486	0	486	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	586	399 / 0	81 / 0	0 / 0	0 / 0	86 / 0	0 / 0
E	390	236 / 0	81 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A-B	0 / 52	-122.2	-122.2	0.17 (1)	10.00	G-C	0 / 274	0.06 (2)
B-I	-297 / 0	-122.2	-122.2	0.14 (1)	6.25	G-D	0 / 395	0.09 (1)
I-C	-514 / 0	-122.2	-122.2	0.22 (1)	6.25	H-I	-597 / 73	0.00 (1)
C-D	-495 / 0	-122.2	-122.2	0.19 (1)	6.25			
F-D	-606 / 0	0.0	0.0	0.06 (1)	7.81			
B-H	0 / 381	-28.0	-28.0	0.25 (1)	10.00			
H-G	0 / 381	-28.0	-28.0	0.28 (1)	10.00			
G-F	0 / 0	-28.0	-28.0	0.54 (1)	10.00			
F-E	0 / 0	-28.0	-28.0	0.22 (1)	10.00			

DWG NO. TAM 47986 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.22 (C-I:1) , BC=0.54 (F-G:1) , WB=0.09 (D-G:1) , SSI=0.45 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

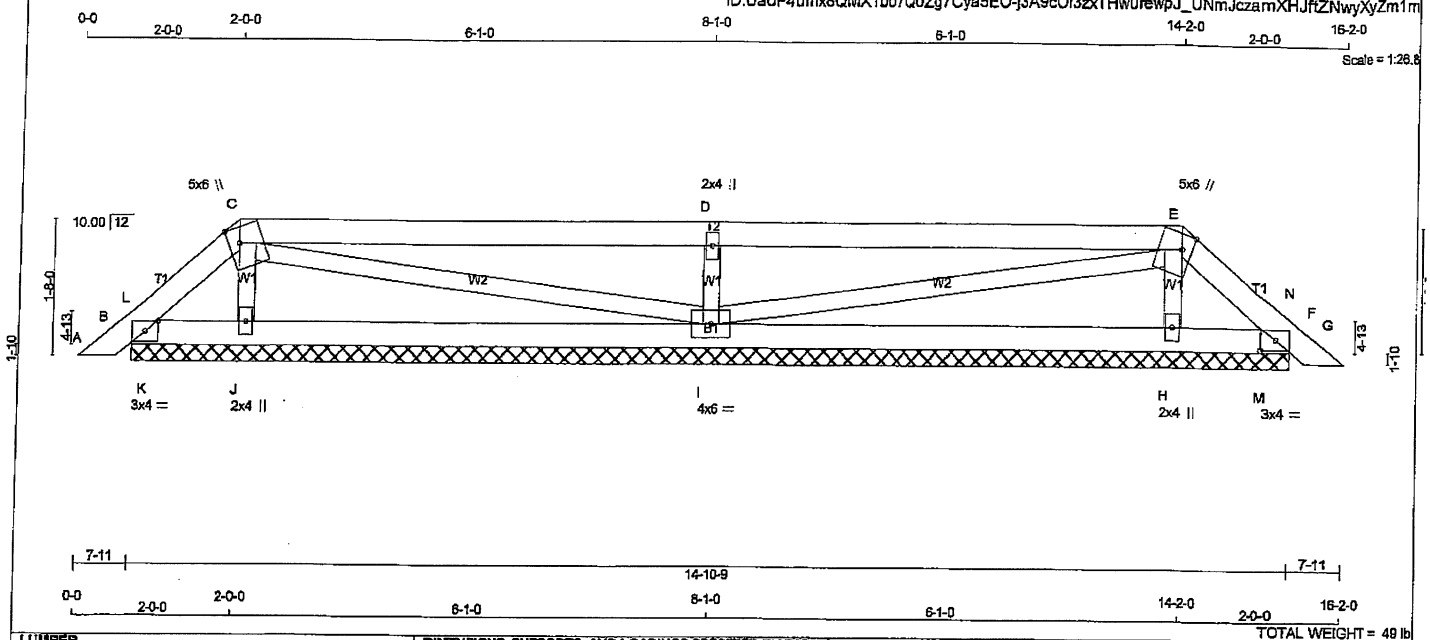
JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.42 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	P1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:41 2017 Page 1

ID:UacF4umx8QMX1bo7Q0Zg7Cya5EO-j3A9cOl3zxTHw0rewpJ_UNmJczamXHJftZNwyXyZm1m



TOTAL WEIGHT = 49 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	169	0	169	0	-37	14-10-9	14-10-9
F	169	0	169	0	-37	14-10-9	14-10-9
J	461	0	461	0	0	14-10-9	14-10-9
I	1110	0	1110	0	0	14-10-9	14-10-9
H	461	0	461	0	0	14-10-9	14-10-9

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	98	123 / 0	0 / -21	0 / 0	0 / 0	0 / -4	0 / 0
F	98	123 / 0	0 / -21	0 / 0	0 / 0	0 / -4	0 / 0
J	397	195 / 0	112 / 0	0 / 0	0 / 0	90 / 0	0 / 0
I	850	585 / 0	131 / 0	0 / 0	0 / 0	133 / 0	0 / 0
H	397	195 / 0	112 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

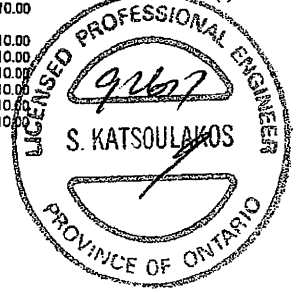
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- L	-20 / 74	-122.2	-122.2	0.04 (3)	6.25
L- C	-74 / 0	-122.2	-122.2	0.03 (2)	6.25
C- D	-8 / 3	-122.2	-122.2	0.77 (1)	10.00
D- E	-8 / 3	-122.2	-122.2	0.77 (1)	10.00
E- N	-74 / 0	-122.2	-122.2	0.03 (2)	6.25
N- F	-20 / 74	-122.2	-122.2	0.04 (3)	6.25
F- G	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- K	-4 / 49	-28.0	-28.0	0.04 (1)	10.00
K- J	-4 / 49	-28.0	-28.0	0.16 (2)	10.00
J- I	-5 / 24	-28.0	-28.0	0.22 (3)	10.00
I- H	-5 / 24	-28.0	-28.0	0.22 (3)	10.00
H- M	-4 / 49	-28.0	-28.0	0.16 (2)	10.00
M- F	-4 / 49	-28.0	-28.0	0.04 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-288 / 0	0.04 (1)
C- I	-16 / 2	0.01 (1)
I- D	-929 / 0	0.13 (1)
I- E	-16 / 2	0.01 (1)
H- E	-288 / 0	0.04 (1)
K- L	-158 / 0	0.00 (1)
M- N	-158 / 0	0.00 (1)



DWG NO. TAM 43963-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.77 (C-D:1) , BC=0.22 (I-J:3) , WB=0.13 (D-I:1) , SSI=0.36 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

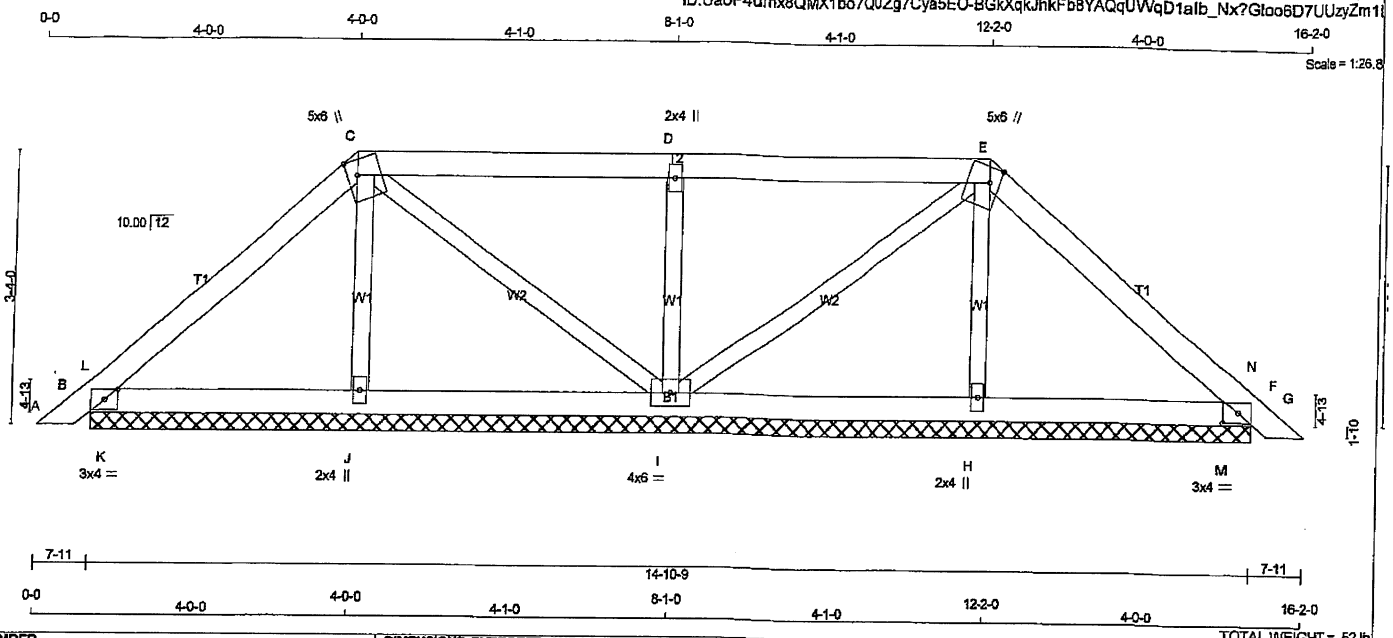
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	P2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:42 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-BGkXqkJhkFb8YAQqUWqD1aIb_Nx?Gtoo6D7UUzyZm1t



TOTAL WEIGHT = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	397	0	397	0	0	14-10-9	14-10-9
F	397	0	397	0	0	14-10-9	14-10-9
J	382	0	382	0	0	14-10-9	14-10-9
I	810	0	810	0	0	14-10-9	14-10-9
H	382	0	382	0	0	14-10-9	14-10-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	293	222 / 0	32 / 0	0 / 0	0 / 0	39 / 0	0 / 0
F	293	222 / 0	32 / 0	0 / 0	0 / 0	39 / 0	0 / 0
J	320	171 / 0	81 / 0	0 / 0	0 / 0	68 / 0	0 / 0
I	612	436 / 0	85 / 0	0 / 0	0 / 0	91 / 0	0 / 0
H	320	171 / 0	81 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-L	-70 / 0	-122.2	-122.2	0.07 (1)	6.25
L-C	-155 / 0	-122.2	-122.2	0.17 (1)	6.25
C-D	-32 / 0	-122.2	-122.2	0.34 (1)	6.25
D-E	-32 / 0	-122.2	-122.2	0.34 (1)	6.25
E-N	-155 / 0	-122.2	-122.2	0.17 (1)	6.25
N-F	-70 / 0	-122.2	-122.2	0.07 (1)	6.25
F-G	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-K	0 / 110	-28.0	-28.0	0.15 (1)	10.00
K-J	0 / 110	-28.0	-28.0	0.15 (1)	10.00
J-I	0 / 103	-28.0	-28.0	0.11 (2)	10.00
I-H	0 / 103	-28.0	-28.0	0.11 (2)	10.00
H-M	0 / 110	-28.0	-28.0	0.15 (1)	10.00
M-F	0 / 110	-28.0	-28.0	0.15 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	-219 / 0	0.04 (1)
C-I	-86 / 0	0.03 (1)
I-D	-622 / 0	0.12 (1)
I-E	-86 / 0	0.03 (1)
H-E	-219 / 0	0.04 (1)
K-L	-414 / 57	0.00 (1)
M-N	-414 / 57	0.00 (1)

DWG NO. TAM47964-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (D-E:1) , BC=0.15 (H-M:1) , WB=0.12 (D-I:1) , SSI=0.32 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

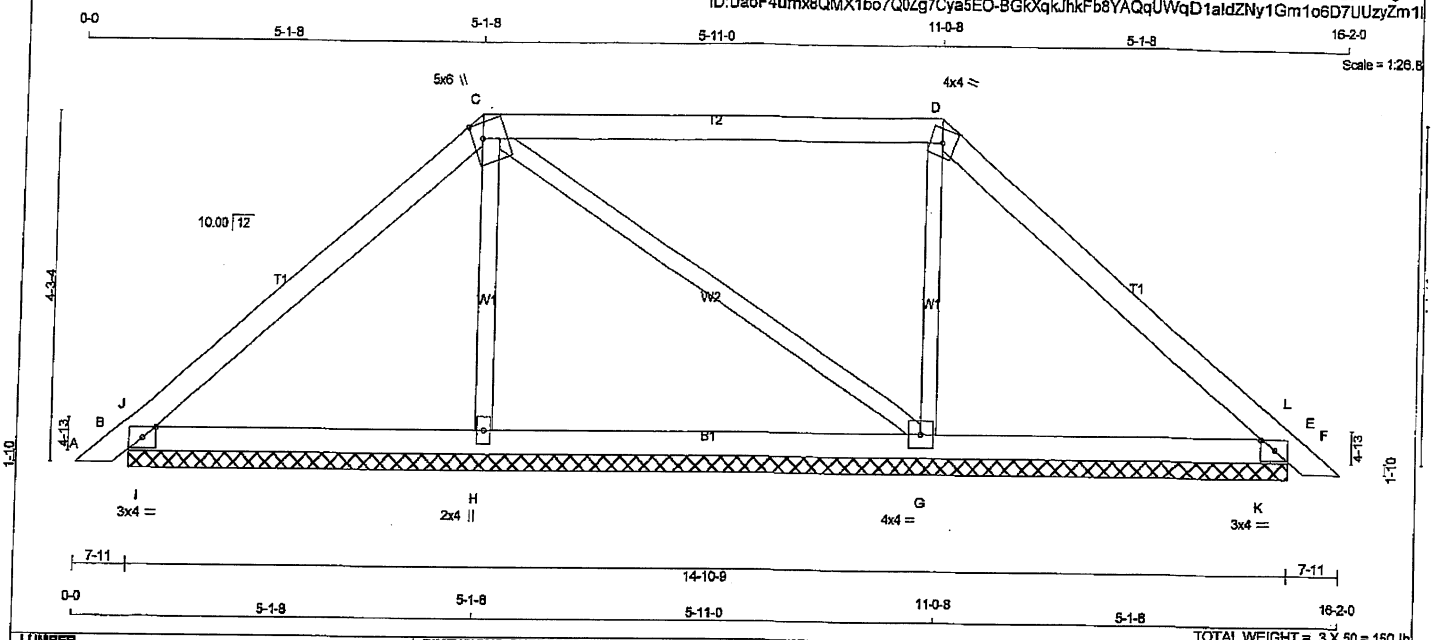
JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	P3	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:42 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-BGkXqkJhkFb8YAQqUWqD1aldZNy1Gm1o6D7UUzyZm1l



TOTAL WEIGHT = 3 X 50 = 150 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- E	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	585	0	585	0	0	14-10-9	14-10-9
E	546	0	546	0	0	14-10-9	14-10-9
H	569	0	569	0	0	14-10-9	14-10-9
G	671	0	671	0	0	14-10-9	14-10-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	428	330 / 0	43 / 0	0 / 0	0 / 0	55 / 0	0 / 0
E	402	305 / 0	44 / 0	0 / 0	0 / 0	53 / 0	0 / 0
H	471	262 / 0	113 / 0	0 / 0	0 / 0	96 / 0	0 / 0
G	538	326 / 0	112 / 0	0 / 0	0 / 0	100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-122.2	-122.2	0.01 (1)	10.00
B- J	-113 / 0	-122.2	-122.2	0.05 (1)	6.25
J- C	-325 / 0	-122.2	-122.2	0.10 (1)	6.25
C- D	-178 / 0	-122.2	-122.2	0.24 (1)	6.25
D- L	-265 / 0	-122.2	-122.2	0.10 (1)	6.25
L- E	-111 / 45	-122.2	-122.2	0.05 (1)	6.25
E- F	0 / 20	-122.2	-122.2	0.01 (1)	10.00
B- I	0 / 236	-28.0	-28.0	0.09 (1)	10.00
I- H	0 / 236	-28.0	-28.0	0.09 (1)	10.00
H- G	0 / 226	-28.0	-28.0	0.07 (2)	10.00
G- K	0 / 190	-28.0	-28.0	0.09 (1)	10.00
K- E	0 / 190	-28.0	-28.0	0.09 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-366 / 0	0.03 (1)
C- G	-57 / 0	0.02 (1)
G- D	-436 / 0	0.04 (1)
I- J	-685 / 82	0.00 (1)
K- L	-693 / 82	0.00 (1)

DWG NO. TAM 47965-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24 (C-D:1) , BC=0.09 (B-I:1) , WB=0.04 (D-G:1) , SSI=0.18 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

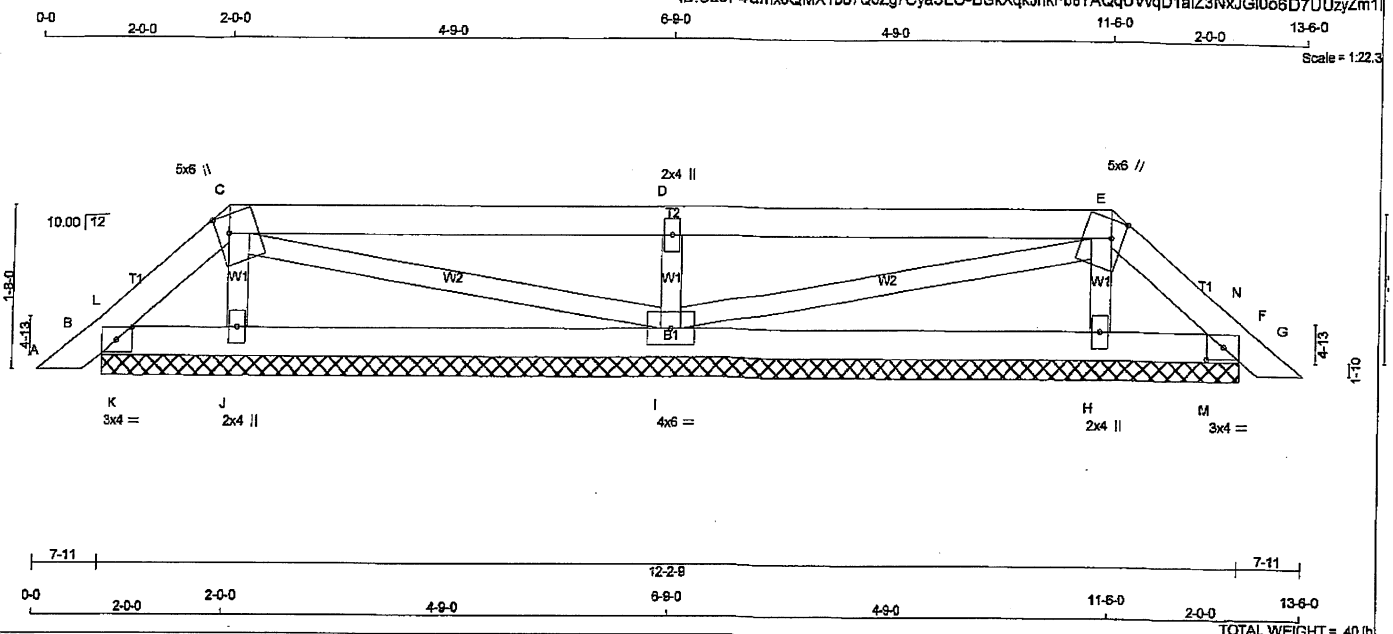
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	P4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:42 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-BGkXqkJhkFb8YAQqUWqD1alZ3NxJGI0o6D7UUzyZm1I



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	185	0	185	0	-2	12-2-9	12-2-9
F	185	0	185	0	-2	12-2-9	12-2-9
J	365	0	365	0	0	12-2-9	12-2-9
I	870	0	870	0	0	12-2-9	12-2-9
H	365	0	365	0	0	12-2-9	12-2-9

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	120	120 / 0	0 / -6	0 / 0	0 / 0	6 / 0	0 / 0
F	120	120 / 0	0 / -6	0 / 0	0 / 0	6 / 0	0 / 0
J	310	159 / 0	83 / 0	0 / 0	0 / 0	68 / 0	0 / 0
I	666	459 / 0	103 / 0	0 / 0	0 / 0	105 / 0	0 / 0
H	310	159 / 0	83 / 0	0 / 0	0 / 0	68 / 0	0 / 0

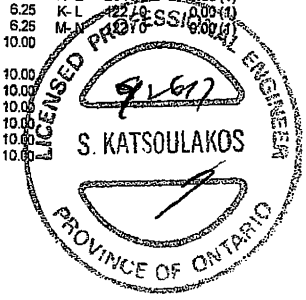
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-122.2	-122.2	0.03 (1)	10.00	J- C	-234 / 0	0.03 (1)
B- L	-38 / 33	-122.2	-122.2	0.02 (3)	6.25	C- I	-19 / 0	0.01 (1)
L- C	-66 / 0	-122.2	-122.2	0.02 (1)	6.25	I- D	-725 / 0	0.10 (1)
C- D	-8 / 1	-122.2	-122.2	0.47 (1)	10.00	I- E	-19 / 0	0.01 (1)
D- E	-8 / 1	-122.2	-122.2	0.47 (1)	10.00	H- E	-234 / 0	0.03 (1)
E- N	-66 / 0	-122.2	-122.2	0.02 (1)	6.25	K- L	-122 / 0	0.00 (1)
N- F	-38 / 33	-122.2	-122.2	0.02 (3)	6.25	M- N	-122 / 0	0.00 (1)
F- G	0 / 20	-122.2	-122.2	0.03 (1)	10.00			
B- K	-1 / 46	-28.0	-28.0	0.04 (1)	10.00			
K- J	-1 / 46	-28.0	-28.0	0.10 (2)	10.00			
J- I	-2 / 26	-28.0	-28.0	0.13 (2)	10.00			
I- H	-2 / 26	-28.0	-28.0	0.13 (2)	10.00			
H- M	-1 / 46	-28.0	-28.0	0.10 (2)	10.00			
M- F	-1 / 46	-28.0	-28.0	0.04 (1)	10.00			



DWG NO. TAM 47966-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47 (C-D:1) , BC=0.13 (H-I:2) , WB=0.10 (D-I:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

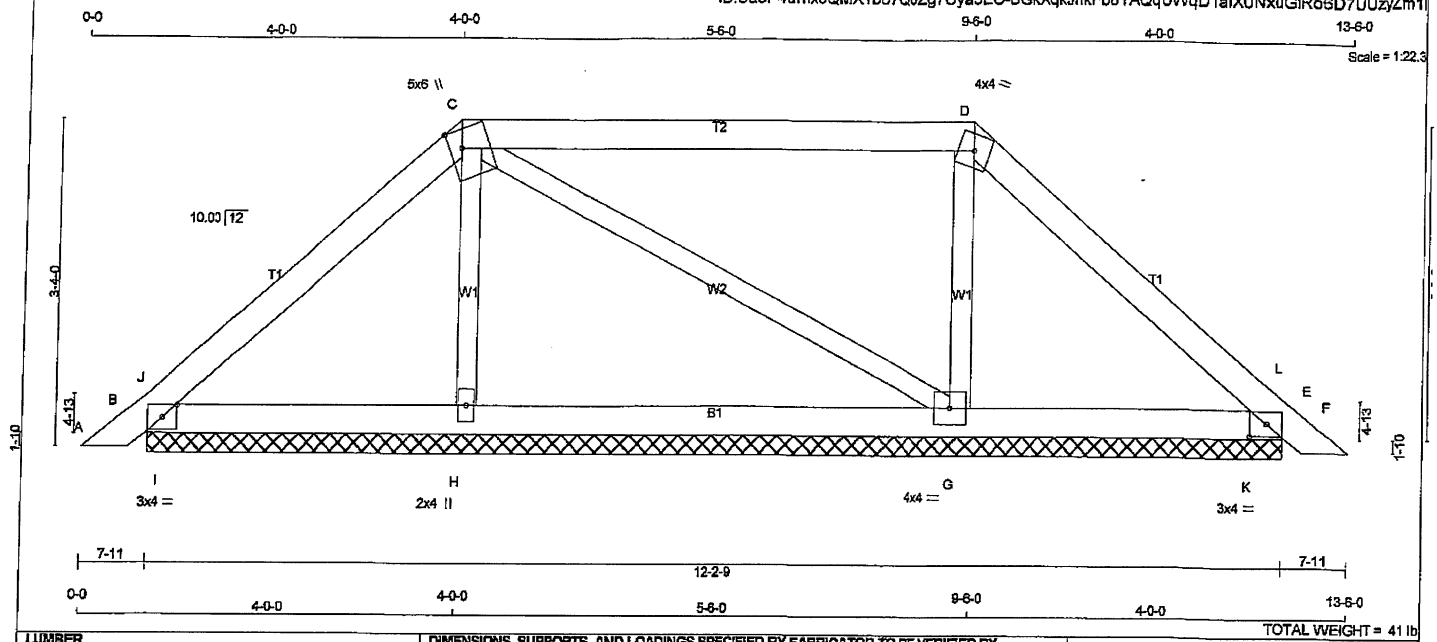
JSI GRIP= 0.38 (D) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	P5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:42 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-BGkXqkJhkFb8YAQqUWqD1aIXUNxuGlRo6D7UUzyZm1l



TOTAL WEIGHT = 41 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-l	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	437	0	437	0	0	12-2-9	12-2-9
E	408	0	408	0	0	12-2-9	12-2-9
H	528	0	528	0	0	12-2-9	12-2-9
G	596	0	596	0	0	12-2-9	12-2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	315	252 / 0	26 / 0	0 / 0	0 / 0	37 / 0	0 / 0
E	295	233 / 0	26 / 0	0 / 0	0 / 0	36 / 0	0 / 0
H	434	245 / 0	102 / 0	0 / 0	0 / 0	87 / 0	0 / 0
G	480	288 / 0	102 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- J	-64 / 0	-122.2	-122.2	0.08 (1)	6.25
J- C	-220 / 0	-122.2	-122.2	0.17 (1)	6.25
C- D	-110 / 0	-122.2	-122.2	0.63 (1)	6.25
D- L	-175 / 0	-122.2	-122.2	0.17 (1)	6.25
L- E	-41 / 0	-122.2	-122.2	0.08 (1)	6.25
E- F	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- I	0 / 160	-28.0	-28.0	0.16 (1)	10.00
I- H	0 / 160	-28.0	-28.0	0.16 (1)	10.00
H- G	0 / 147	-28.0	-28.0	0.15 (2)	10.00
G- K	0 / 125	-28.0	-28.0	0.16 (1)	10.00
K- E	0 / 125	-28.0	-28.0	0.16 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-355 / 0	0.07 (1)
C- G	-41 / 0	0.03 (1)
G- D	-403 / 0	0.08 (1)
I- J	-417 / 20	0.00 (1)
K- L	-422 / 20	0.00 (1)

DWG NO. TAM 4796717
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63 (C-D:1) , BC=0.16 (B-I:1) , WB=0.08 (D-G:1) , SSI=0.33 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

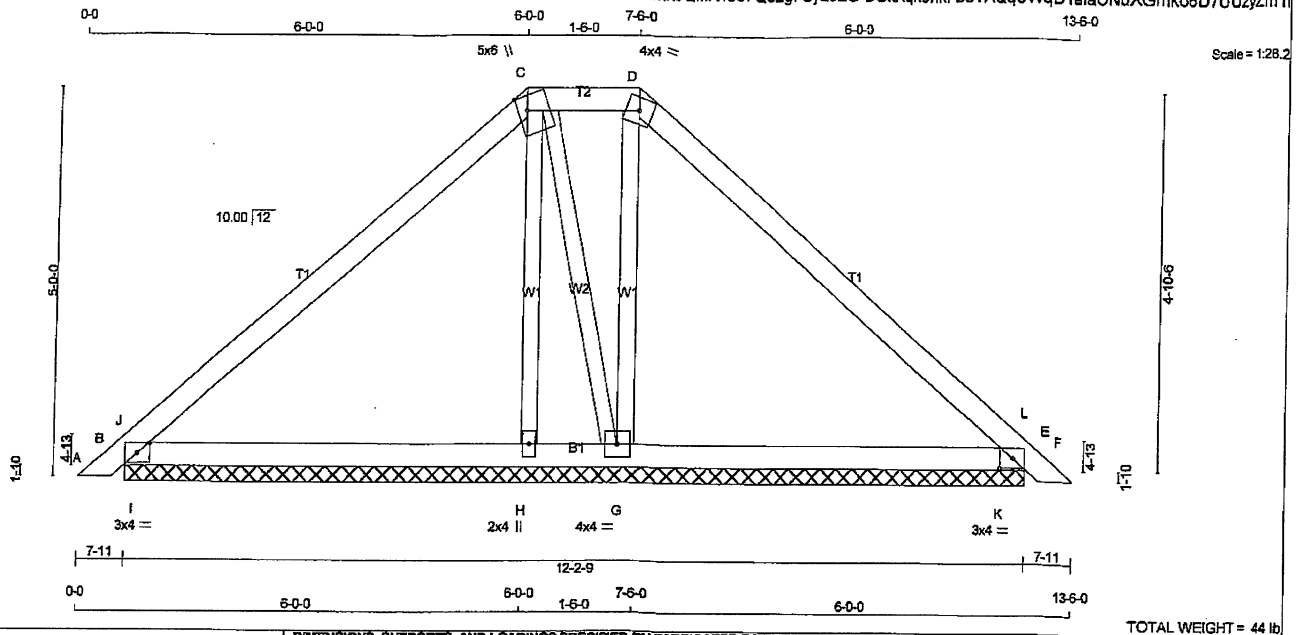
JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	P6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:42 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-BGkXqkJhkFb8YAQqUWqD1alaONuXGmko6D7UUzyZm1l



TOTAL WEIGHT = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	665	0	665	0	0	12-2-9	12-2-9
E	650	0	650	0	0	12-2-9	12-2-9
H	257	0	257	0	0	12-2-9	12-2-9
G	397	0	397	0	0	12-2-9	12-2-9

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	499	361 / 0	66 / 0	0 / 0	0 / 0	72 / 0	0 / 0
E	490	351 / 0	67 / 0	0 / 0	0 / 0	72 / 0	0 / 0
H	226	105 / 0	68 / 0	0 / 0	0 / 0	53 / 0	0 / 0
G	310	202 / 0	56 / 0	0 / 0	0 / 0	53 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

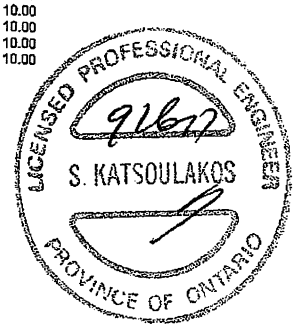
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-J	-222 / 140	-122.2	-122.2	0.26 (1)	6.25
J-C	-366 / 0	-122.2	-122.2	0.45 (1)	6.25
C-D	-243 / 0	-122.2	-122.2	0.05 (1)	6.25
D-L	-343 / 0	-122.2	-122.2	0.45 (1)	6.25
L-E	-225 / 165	-122.2	-122.2	0.26 (1)	6.25
E-F	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-I	0 / 264	-28.0	-28.0	0.38 (1)	10.00
I-H	0 / 264	-28.0	-28.0	0.38 (1)	10.00
H-G	0 / 262	-28.0	-28.0	0.27 (1)	10.00
G-K	0 / 246	-28.0	-28.0	0.38 (1)	10.00
K-E	0 / 246	-28.0	-28.0	0.38 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-87 / 11	0.03 (1)
C-G	-65 / 9	0.02 (1)
G-D	-164 / 22	0.06 (1)
I-J	-1011 / 179	0.00 (1)
K-L	-1015 / 179	0.00 (1)



DWG NO. TAM47968-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.45 (C-J:1) , BC=0.38 (B-I:1) , WB=0.06 (D-G:1) , SSI=0.79 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

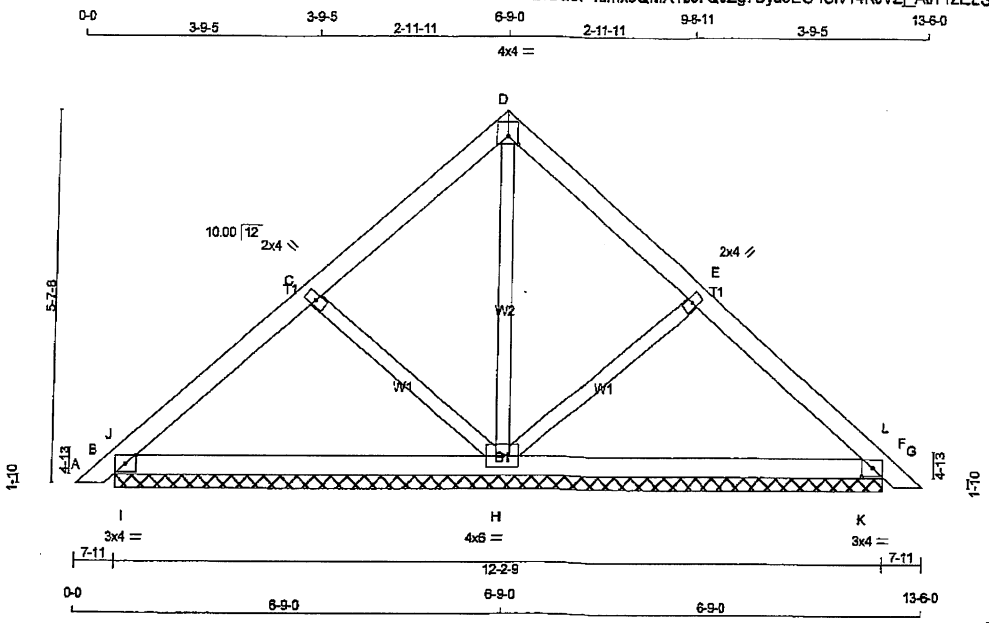
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	P7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:43 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-fSIv14KJVZI_AJ?12ELSZoro3nFN?BYyKts10PyZm1k



TOTAL WEIGHT = 2 X 44 = 88 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMWWW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	478	0	478	0	0	12-2-9	12-2-9
F	478	0	478	0	0	12-2-9	12-2-9
H	1013	0	1013	0	0	12-2-9	12-2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	365	252 / 0	56 / 0	0 / 0	0 / 0	57 / 0	0 / 0
F	365	252 / 0	56 / 0	0 / 0	0 / 0	57 / 0	0 / 0
H	795	513 / 0	145 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- J	-466 / 0	-122.2	-122.2	0.12 (3)	6.25
J- C	-276 / 0	-122.2	-122.2	0.19 (1)	6.25
C- D	-19 / 14	-122.2	-122.2	0.19 (1)	6.25
D- E	-19 / 14	-122.2	-122.2	0.19 (1)	6.25
E- L	-278 / 0	-122.2	-122.2	0.19 (1)	6.25
L- F	-466 / 0	-122.2	-122.2	0.12 (3)	6.25
F- G	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- I	0 / 244	-28.0	-28.0	0.10 (3)	10.00
I- H	0 / 244	-28.0	-28.0	0.27 (2)	10.00
H- K	0 / 244	-28.0	-28.0	0.27 (2)	10.00
K- F	0 / 244	-28.0	-28.0	0.10 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	-338 / 13	0.15 (1)
H- E	-364 / 0	0.10 (1)
C- H	-364 / 0	0.10 (1)
I- J	0 / 392	0.00 (1)
K- L	0 / 392	0.00 (1)

DWG NO. TAM 47969-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (D-E:1) , BC=0.27 (H-K:2) , WB=0.15 (D-H:1) , SSI=0.21 (F-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

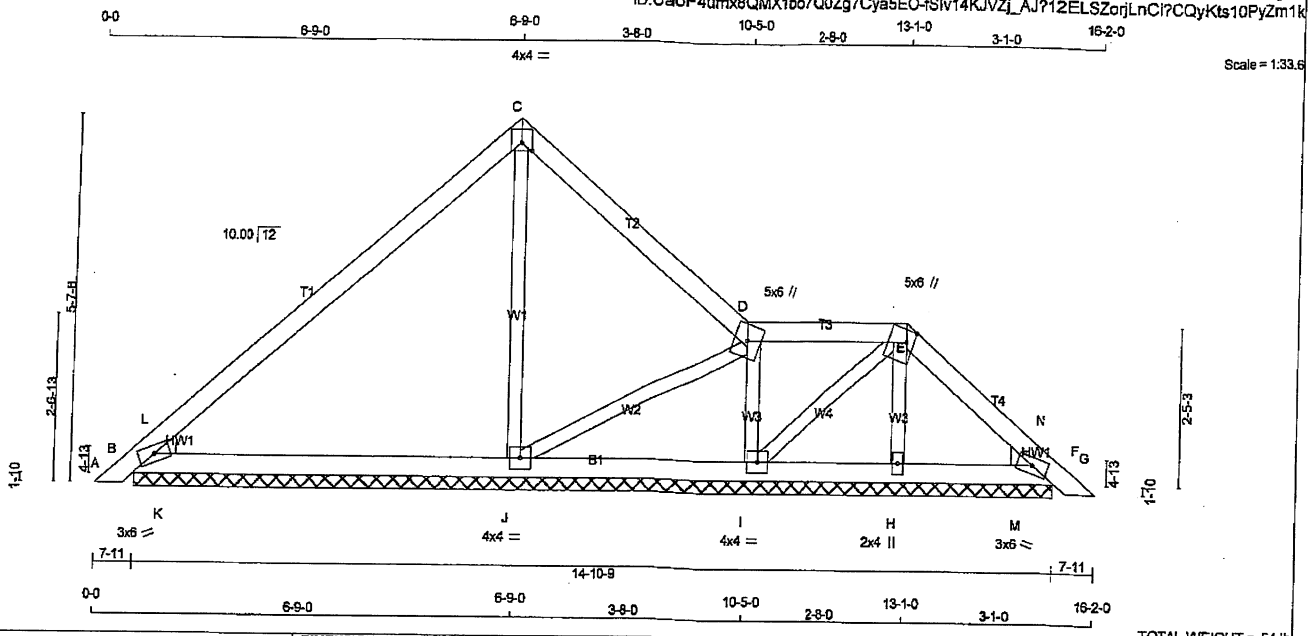
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (D) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42057	DRWG NO.
288458	P8S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:43 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-fSlv14KJVZj_AJ?12ELSZorjLnCl?CQyKts10PyZm1k



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	6.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TTWW+m	MT20	5.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMBH1-m	MT20	3.0	6.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	662	0	662	0	0	14-10-9	14-10-9	2x4 L
F	341	0	341	0	0	14-10-9	14-10-9	2x4 L
J	468	0	468	0	0	14-10-9	14-10-9	2x4 L
I	565	0	565	0	0	14-10-9	14-10-9	2x4 L
H	333	0	333	0	0	14-10-9	14-10-9	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	497	360 / 0	65 / 0	0 / 0	0 / 0	72 / 0	0 / 0
F	250	192 / 0	28 / 0	0 / 0	0 / 0	32 / 0	0 / 0
J	395	207 / 0	103 / 0	0 / 0	0 / 0	85 / 0	0 / 0
I	428	303 / 0	61 / 0	0 / 0	0 / 0	64 / 0	0 / 0
H	269	161 / 0	58 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-L	-139 / 0	-122.2	-122.2	0.23 (1)	6.25
L-C	-284 / 0	-122.2	-122.2	0.56 (1)	6.25
C-D	-264 / 0	-122.2	-122.2	0.28 (1)	6.25
D-E	-122 / 0	-122.2	-122.2	0.15 (1)	6.25
E-N	-162 / 0	-122.2	-122.2	0.08 (1)	6.25
N-F	-167 / 0	-122.2	-122.2	0.02 (3)	6.25
F-G	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-K	0 / 203	-28.0	-28.0	0.51 (1)	10.00
K-J	0 / 203	-28.0	-28.0	0.51 (1)	10.00
J-I	0 / 150	-28.0	-28.0	0.32 (1)	10.00
I-H	0 / 114	-28.0	-28.0	0.08 (1)	10.00
H-M	0 / 123	-28.0	-28.0	0.10 (1)	10.00
M-F	0 / 123	-28.0	-28.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	-199 / 26	0.09 (1)
J-D	0 / 61	0.01 (1)
I-D	-586 / 0	0.09 (1)
I-E	0 / 9	0.00 (1)
H-E	-182 / 0	0.03 (1)
K-L	-707 / 108	0.00 (1)
M-N	-142 / 21	0.00 (1)

DWG NO. TAM47970-17
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 54 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56 (C-L:1) , BC=0.51 (B-K:1) , WB=0.09 (D-I:1) , SSI=0.55 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

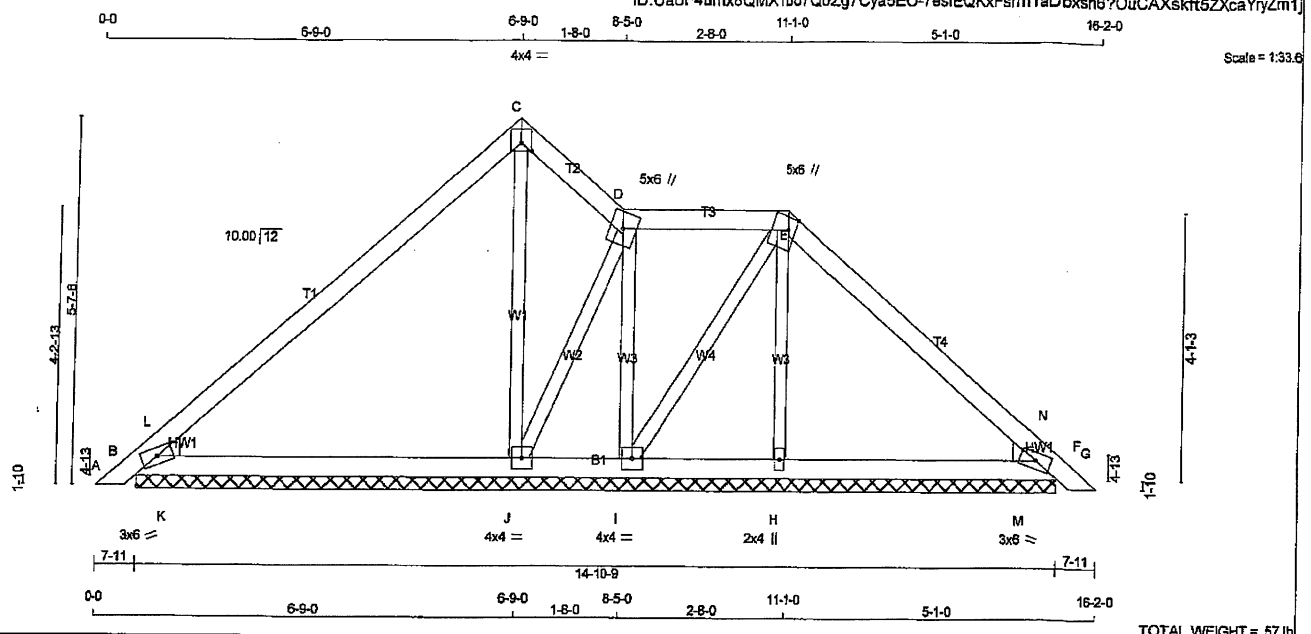
JSI GRIP= 0.36 (C) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	P9S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:44 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-7esIEQKxFsrmTaDbxsh6?OuCAXskft5ZXcaYryZm1j



TOTAL WEIGHT = 57 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	6.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TTWW+m	MT20	5.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMBH1-m	MT20	3.0	6.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	632	0	632	0	0	14-10-9	14-10-9	2x4 L
F	573	0	573	0	0	14-10-9	14-10-9	2x4 L
J	623	0	623	0	0	14-10-9	14-10-9	2x4 L
I	120	0	120	0	-21	14-10-9	14-10-9	2x4 L
H	422	0	422	0	0	14-10-9	14-10-9	2x4 L

PROVIDE ANCHORAGE AT BEARING JOINT I FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	477	341 / 0	65 / 0	0 / 0	0 / 0	70 / 0	0 / 0
F	425	316 / 0	51 / 0	0 / 0	0 / 0	59 / 0	0 / 0
J	510	293 / 0	117 / 0	0 / 0	0 / 0	101 / 0	0 / 0
I	71	86 / 0	0 / -13	0 / 0	0 / 0	0 / -2	0 / 0
H	356	187 / 0	92 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

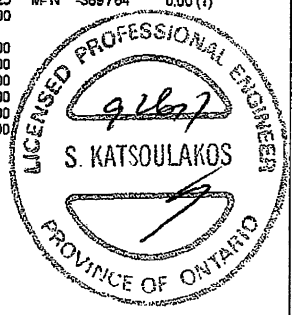
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- L	-136 / 0	-122.2	-122.2	0.24 (1)	6.25
L- C	-248 / 0	-122.2	-122.2	0.55 (1)	6.25
C- D	-227 / 0	-122.2	-122.2	0.05 (1)	6.25
D- E	-212 / 0	-122.2	-122.2	0.15 (1)	6.25
E- N	-314 / 0	-122.2	-122.2	0.28 (1)	6.25
N- F	-233 / 0	-122.2	-122.2	0.09 (1)	6.25
F- G	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- K	0 / 175	-28.0	-28.0	0.51 (1)	10.00
K- J	0 / 175	-28.0	-28.0	0.51 (1)	10.00
J- I	0 / 220	-28.0	-28.0	0.38 (1)	10.00
I- H	0 / 228	-28.0	-28.0	0.21 (1)	10.00
H- M	0 / 232	-28.0	-28.0	0.29 (1)	10.00
M- F	0 / 232	-28.0	-28.0	0.29 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-123 / 33	0.05 (1)
J- D	-112 / 0	0.03 (1)
I- D	-306 / 0	0.08 (1)
I- E	-28 / 5	0.01 (1)
H- E	-169 / 0	0.04 (1)
K- L	-727 / 103	0.00 (1)
M- N	-389 / 64	0.00 (1)



DWG NO. TAM4797-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.55 (C-L:1) , BC=0.51 (B-K:1) , WB=0.06 (D-I:1) , SSI=0.55 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) MAX	MIN	SHEAR (PSI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

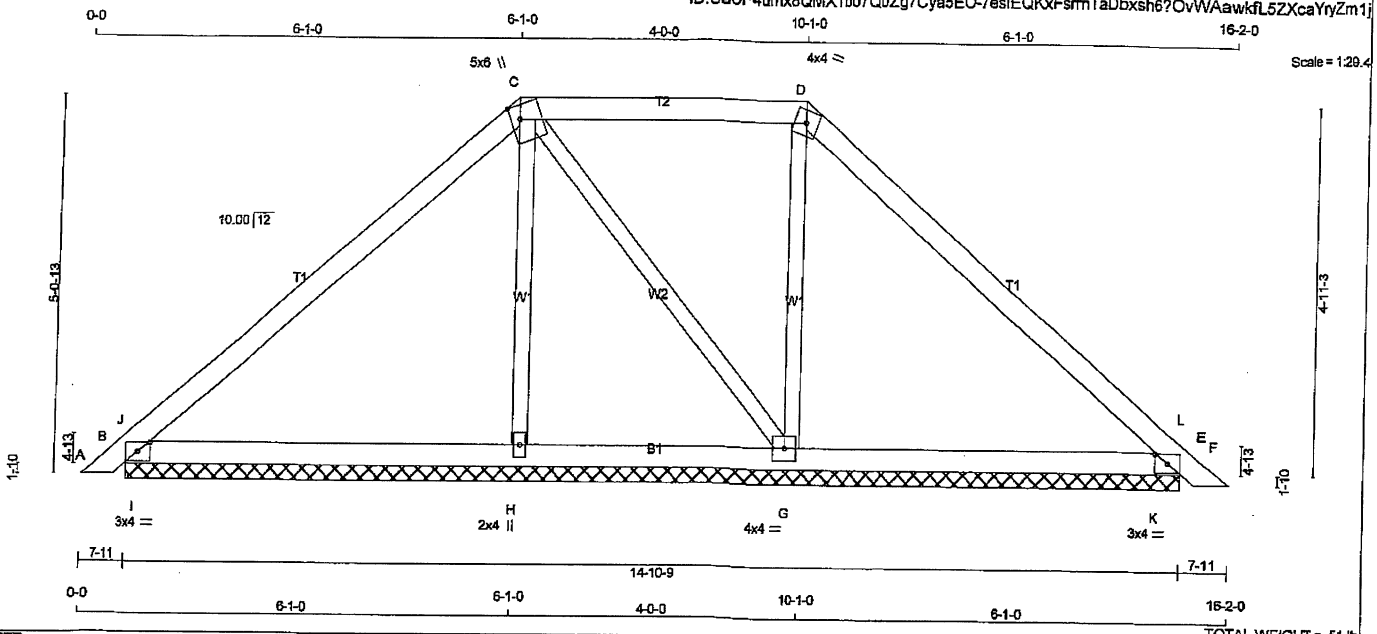
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42057	DRWG NO.
288458	P10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:44 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-7eslEQKxFsrmTaDbxsh6?OvWAawkfL5ZXcaYryZm1j



TOTAL WEIGHT = 51 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	709	0	709	0	0	14-10-9	14-10-9
E	671	0	671	0	0	14-10-9	14-10-9
H	419	0	419	0	0	14-10-9	14-10-9
G	571	0	571	0	0	14-10-9	14-10-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	530	388 / 0	67 / 0	0 / 0	0 / 0	75 / 0	0 / 0
E	505	363 / 0	68 / 0	0 / 0	0 / 0	74 / 0	0 / 0
H	353	185 / 0	92 / 0	0 / 0	0 / 0	76 / 0	0 / 0
G	451	265 / 0	86 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-J	-235 / 79	-122.2	-122.2	0.25 (1)	6.25
J-C	-412 / 0	-122.2	-122.2	0.47 (1)	6.25
C-D	-247 / 0	-122.2	-122.2	0.34 (1)	6.25
D-L	-353 / 0	-122.2	-122.2	0.47 (1)	6.25
L-E	-237 / 144	-122.2	-122.2	0.28 (1)	6.25
E-F	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-I	0 / 300	-28.0	-28.0	0.38 (1)	10.00
I-H	0 / 300	-28.0	-28.0	0.38 (1)	10.00
H-G	0 / 295	-28.0	-28.0	0.22 (1)	10.00
G-K	0 / 255	-28.0	-28.0	0.38 (1)	10.00
K-E	0 / 255	-28.0	-28.0	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-221 / 7	0.08 (1)
C-G	-74 / 2	0.05 (1)
G-D	-316 / 10	0.11 (1)
I-J	-996 / 196	0.00 (1)
K-L	-1004 / 197	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47972-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47 (C-J:1) , BC=0.38 (H-I:1) , WB=0.11 (D-G:1) , SSI=0.79 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

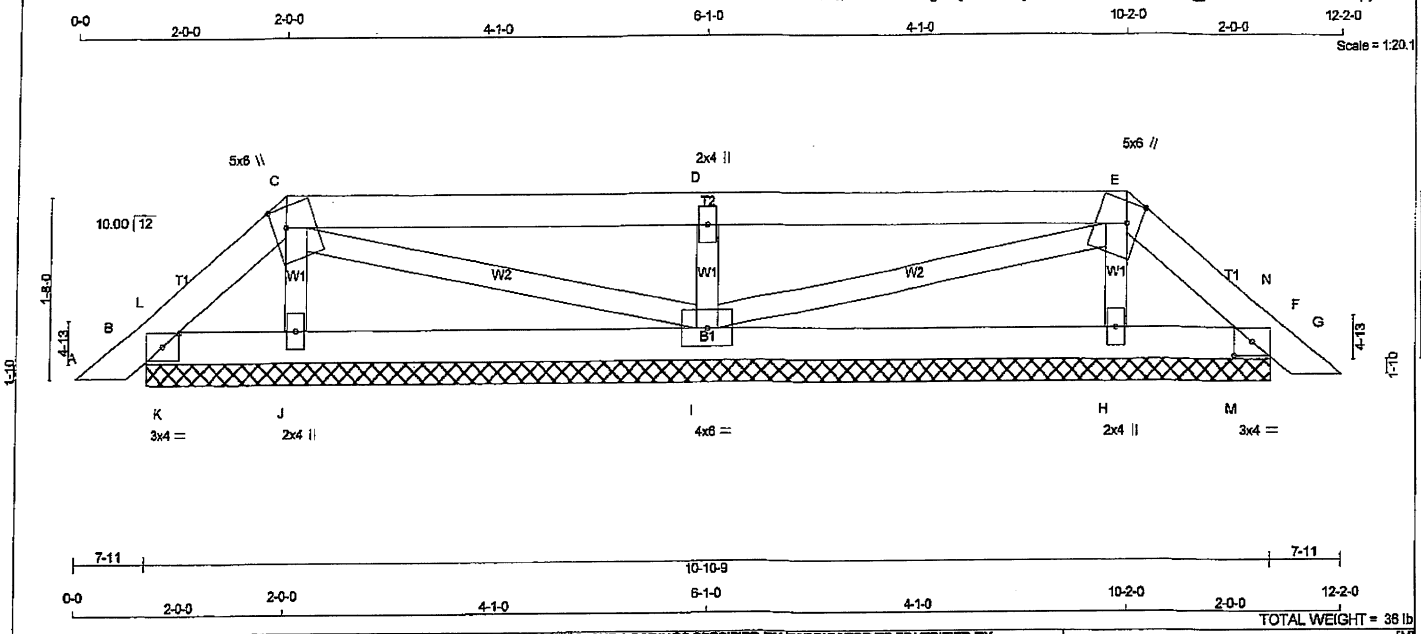
JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	P11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:45 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-Rj54PIZ6FDTJGbaVxxRq_1FTeRE46z?btUxMcpyZmJV



TOTAL WEIGHT = 36 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	190	0	190	0	0	10-10-9	10-10-9
F	190	0	190	0	0	10-10-9	10-10-9
J	319	0	319	0	0	10-10-9	10-10-9
I	750	0	750	0	0	10-10-9	10-10-9
H	319	0	319	0	0	10-10-9	10-10-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	129	119 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
F	129	119 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
J	269	141 / 0	70 / 0	0 / 0	0 / 0	58 / 0	0 / 0
I	574	396 / 0	88 / 0	0 / 0	0 / 0	90 / 0	0 / 0
H	269	141 / 0	70 / 0	0 / 0	0 / 0	58 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- L	-44 / 17	-122.2	-122.2	0.01 (3)	6.25
L- C	-63 / 0	-122.2	-122.2	0.02 (1)	6.25
C- D	-7 / 0	-122.2	-122.2	0.35 (1)	10.00
D- E	-7 / 0	-122.2	-122.2	0.35 (1)	10.00
E- N	-63 / 0	-122.2	-122.2	0.02 (1)	6.25
N- F	-44 / 17	-122.2	-122.2	0.01 (3)	6.25
F- G	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- K	0 / 44	-28.0	-28.0	0.04 (1)	10.00
K- J	0 / 44	-28.0	-28.0	0.07 (2)	10.00
J- I	0 / 27	-28.0	-28.0	0.10 (2)	10.00
I- H	0 / 27	-28.0	-28.0	0.10 (2)	10.00
H- M	0 / 44	-28.0	-28.0	0.07 (2)	10.00
M- F	0 / 44	-28.0	-28.0	0.04 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-208 / 0	0.03 (1)
C- I	-20 / 0	0.01 (1)
I- D	-623 / 0	0.09 (1)
I- E	-20 / 0	0.01 (1)
H- E	-208 / 0	0.03 (1)
K- L	-108 / 0	0.00 (1)
M- N	-108 / 0	0.00 (1)

FR-TO

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4179881 7
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35 (C-D:1) , BC=0.10 (I-J:2) , WB=0.09 (D-I:1) , SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

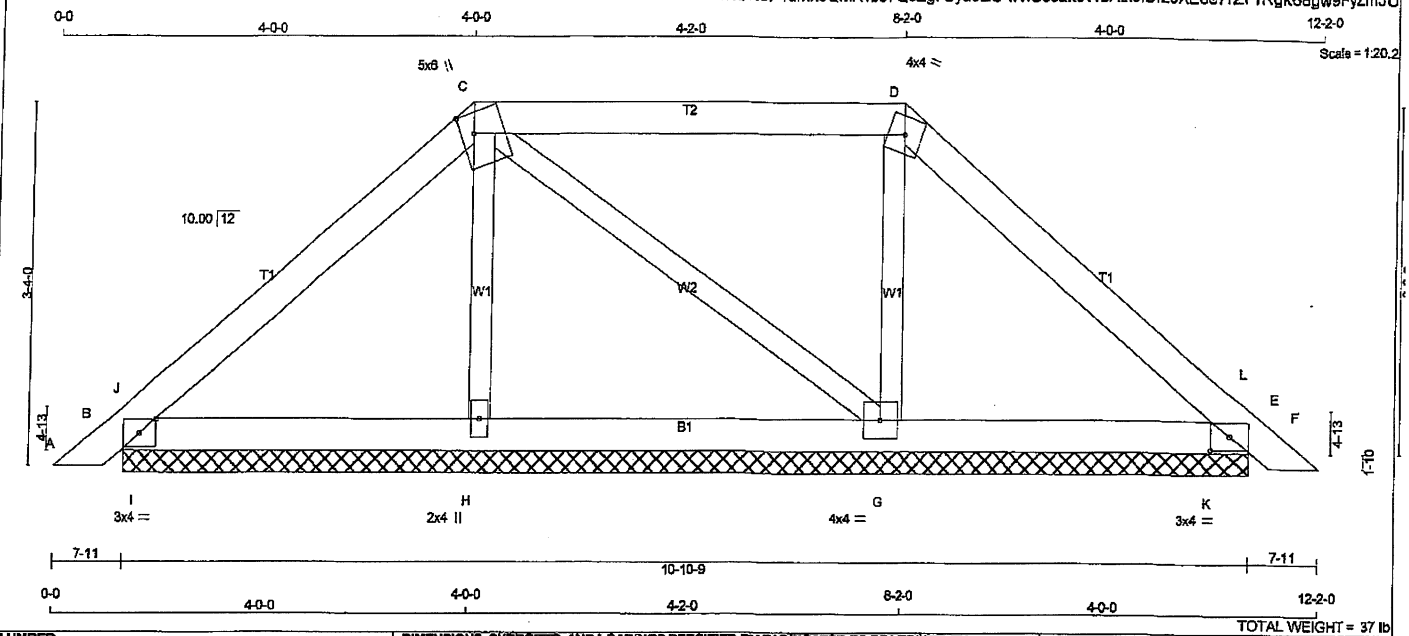
JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	P12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:47 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-wvfSceak0WbAtk9iUfz3XEoe7rZPrRgk68gw9FyZmJU



TOTAL WEIGHT = 37 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	440	0	440	0	0	10-10-9	10-10-9
E	408	0	408	0	0	10-10-9	10-10-9
H	417	0	417	0	0	10-10-9	10-10-9
G	504	0	504	0	0	10-10-9	10-10-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	322	247 / 0	33 / 0	0 / 0	0 / 0	42 / 0	0 / 0
E	301	228 / 0	33 / 0	0 / 0	0 / 0	40 / 0	0 / 0
H	344	193 / 0	81 / 0	0 / 0	0 / 0	69 / 0	0 / 0
G	402	246 / 0	81 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-J	-86 / 0	-122.2	-122.2	0.07 (1)	6.25
J-C	-215 / 0	-122.2	-122.2	0.17 (1)	6.25
C-D	-108 / 0	-122.2	-122.2	0.36 (1)	6.25
D-L	-167 / 0	-122.2	-122.2	0.17 (1)	6.25
L-E	-75 / 0	-122.2	-122.2	0.07 (1)	6.25
E-F	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-I	0 / 156	-28.0	-28.0	0.16 (1)	10.00
I-H	0 / 156	-28.0	-28.0	0.16 (1)	10.00
H-G	0 / 147	-28.0	-28.0	0.11 (2)	10.00
G-K	0 / 120	-28.0	-28.0	0.15 (1)	10.00
K-E	0 / 120	-28.0	-28.0	0.15 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-269 / 0	0.05 (1)
C-G	-47 / 0	0.02 (1)
G-D	-328 / 0	0.06 (1)
I-J	-398 / 58	0.00 (1)
K-L	-405 / 58	0.00 (1)

DWG NO. TAM4)989-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.36 (C-D:1) , BC=0.16 (B-I:1) , WB=0.06 (D-G:1) , SSI=0.32 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

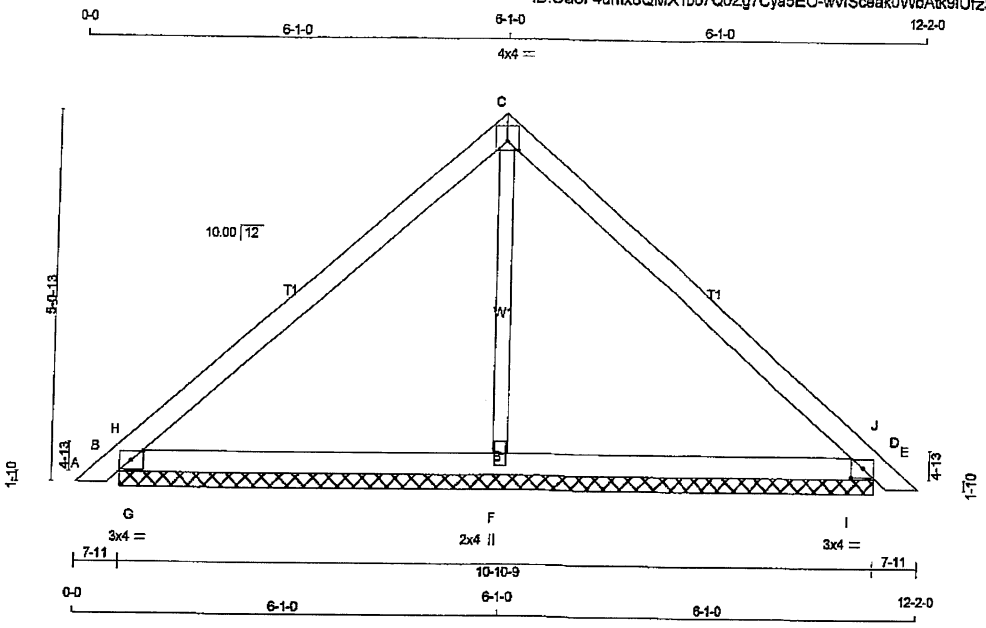
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	P13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:47 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-wvfSceak0WbAtk9iUfz3XEocIrVVrR7k68gw9FyZmJU



Scale = 1:30.0

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	655	0	655	0	0	10-10-9	10-10-9
D	655	0	655	0	0	10-10-9	10-10-9
F	458	0	458	0	0	10-10-9	10-10-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	487	361 / 0	58 / 0	0 / 0	0 / 0	67 / 0	0 / 0
D	487	361 / 0	58 / 0	0 / 0	0 / 0	67 / 0	0 / 0
F	395	194 / 0	112 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-H	-172 / 215	-122.2	-122.2	0.31 (1)	6.25
H-C	-369 / 0	-122.2	-122.2	0.45 (1)	6.25
C-J	-369 / 0	-122.2	-122.2	0.45 (1)	6.25
J-D	-172 / 215	-122.2	-122.2	0.31 (1)	6.25
D-E	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B-G	0 / 265	-28.0	-28.0	0.41 (1)	10.00
G-F	0 / 265	-28.0	-28.0	0.41 (1)	10.00
F-I	0 / 265	-28.0	-28.0	0.41 (1)	10.00
I-D	0 / 265	-28.0	-28.0	0.41 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-117 / 37	0.04 (1)
G-H	-1123 / 126	0.00 (1)
I-J	-1123 / 126	0.00 (1)

DWG NO. TAM 47990 17
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 34 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.45 (C-H:1) , BC=0.41 (B-G:1) , WB=0.04 (C-F:1) , SSI=0.86 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

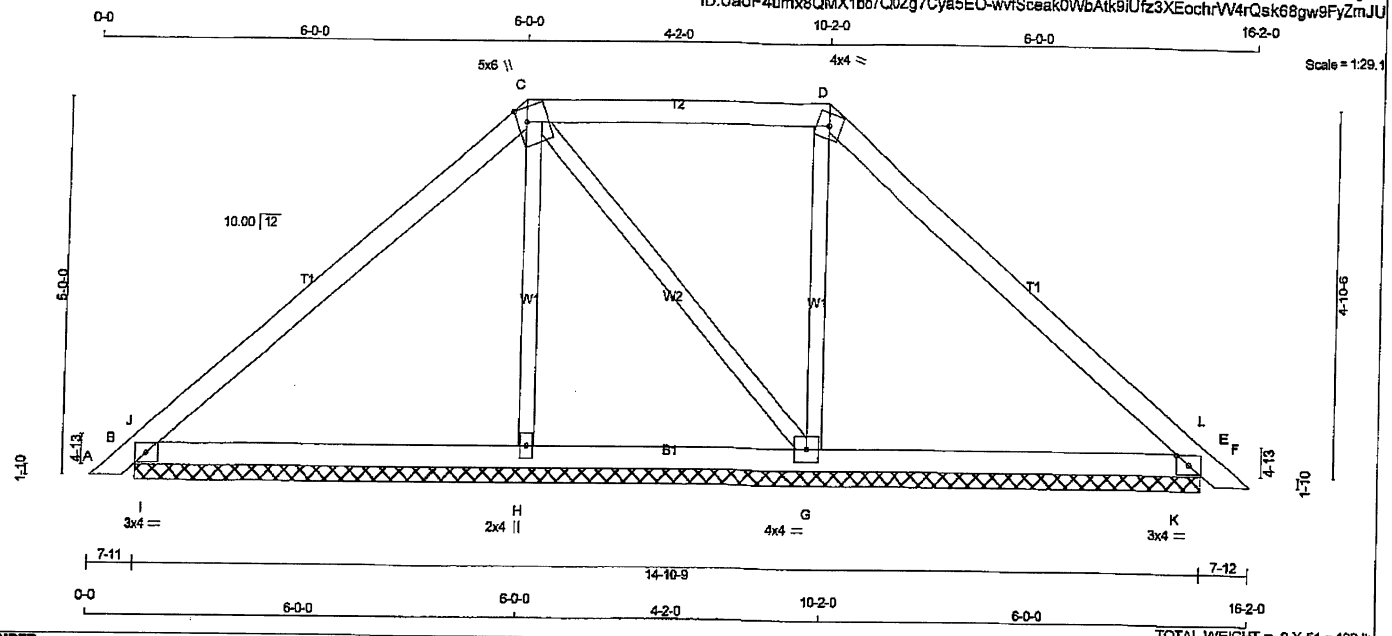
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288460	P20	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:08:47 2017 Page 1
ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-wvfSceak0WbAtk9iUfz3XEochrW4rQsk68gw9FyZmJU



TOTAL WEIGHT = 2 X 51 = 102 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	699	0	699	0	0	14-10-9	14-10-9
E	659	0	659	0	0	14-10-9	14-10-9
H	432	0	432	0	0	14-10-9	14-10-9
G	580	0	580	0	0	14-10-9	14-10-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	521	383 / 0	65 / 0	0 / 0	0 / 0	73 / 0	0 / 0
E	496	357 / 0	66 / 0	0 / 0	0 / 0	72 / 0	0 / 0
H	363	192 / 0	93 / 0	0 / 0	0 / 0	77 / 0	0 / 0
G	459	290 / 0	88 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

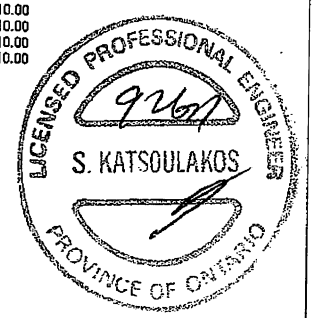
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- J	-226 / 68	-122.2	-122.2	0.24 (1)	6.25
J- C	-405 / 0	-122.2	-122.2	0.45 (1)	6.25
C- D	-240 / 0	-122.2	-122.2	0.36 (1)	6.25
D- L	-344 / 0	-122.2	-122.2	0.45 (1)	6.25
L- E	-227 / 134	-122.2	-122.2	0.25 (1)	6.25
E- F	0 / 20	-122.2	-122.2	0.03 (1)	10.00
B- I	0 / 294	-28.0	-28.0	0.37 (1)	10.00
I- H	0 / 294	-28.0	-28.0	0.37 (1)	10.00
H- G	0 / 269	-28.0	-28.0	0.22 (1)	10.00
G- K	0 / 248	-28.0	-28.0	0.37 (1)	10.00
K- E	0 / 248	-28.0	-28.0	0.37 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-234 / 6	0.08 (1)
C- G	-73 / 2	0.05 (1)
G- D	-327 / 8	0.11 (1)
I- J	-966 / 189	0.00 (1)
K- L	-974 / 189	0.00 (1)



DWG NO. TAM4799817
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.45 (C-J:1) , BC=0.37 (H-I:1) , WB=0.11 (D-G:1) , SSI=0.77 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

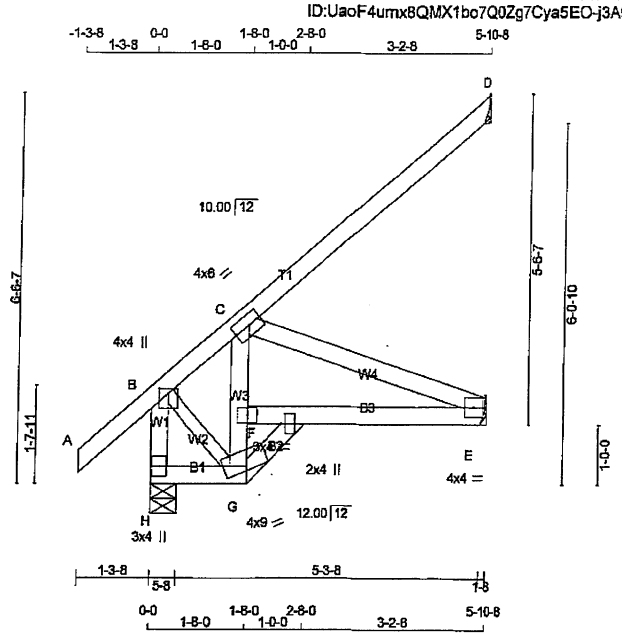
JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288458	J1S	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 26 13:27:41 2017 Page 1

ID:UaoF4umx8QMX1bo7Q0Zg7Cya5EO-j3A9cOl3zxTHw0rewpJ_UNmR9zbSXFeftZNwyXyZm1m



TOTAL WEIGHT = 8 X 30 = 237 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
E	BMW1-t	MT20	4.0	4.0		
F	BWM-I	MT20	3.0	4.0		
G						
G						
G	BBWW-m	MT20	4.0	9.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	NP+p	MT20	2.0	4.0	2.00	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	622	0	622	0	0	5-8	5-8
D	226	0	226	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	196	0	196	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	485	340 / 0	59 / 0	0 / 0	0 / 0	66 / 0	0 / 0
D	153	141 / 0	1 / 0	0 / 0	0 / 0	12 / 0	0 / 0
E	177	74 / 0	58 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
H-B	-601 / 0	0.0	0.0	0.06 (1)	7.81	
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B-C	-266 / 0	-122.2	-122.2	0.24 (1)	6.25	
C-D	-25 / 0	-122.2	-122.2	0.29 (1)	6.25	
H-G	0 / 0	-28.0	-28.0	0.02 (3)	10.00	
F-E	0 / 363	-28.0	-28.0	0.17 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-F	-169 / 0	0.30 (1)
F-C	-111 / 80	0.30 (1)
B-G	0 / 292	0.07 (1)
C-E	-389 / 0	0.10 (1)

FR-TO

LICENSED PROFESSIONAL ENGINEER
9267
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47962-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.29 (C-D:1) , BC=0.17 (E-F:2) , WB=0.30 (F-G:1) , SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

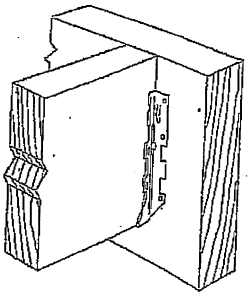
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

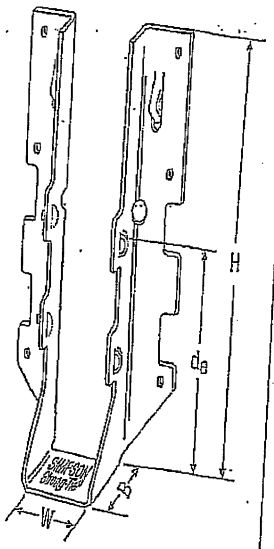
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



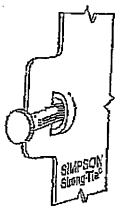
Typical LUS Installation



LUS28

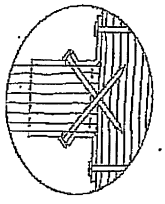
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistances (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_B[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.60)	Uplift (K_D=1.15)	Normal (K_D=1.60)
LUS24	18	1⅜	3⅛	1¾	1⅞	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1⅞	4-16d	2-16d	855	2020	590	1435
LUS26	18	1⅜	4¾	1¾	3⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2840
LUS28	18	1⅜	6⅝	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1⅜	7⅞	1¾	5⅞	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8⅞	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_B is the distance from the seat of the hanger to the highest joist nail.

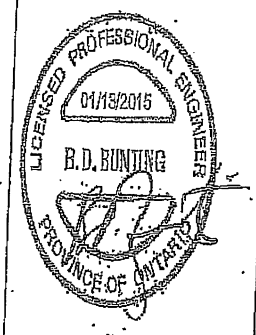


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



800.999.5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

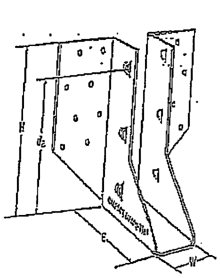
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

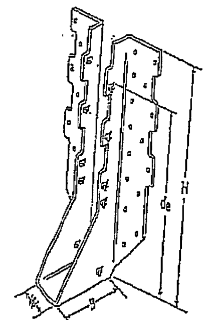
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

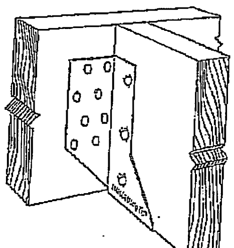
- See current catalogue for options



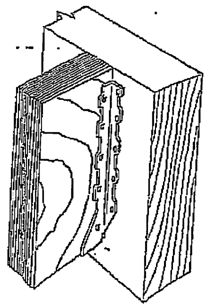
LJS26DS



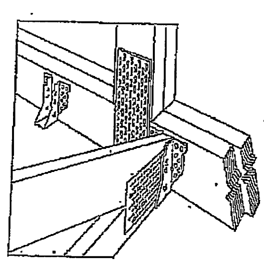
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



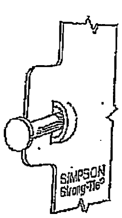
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

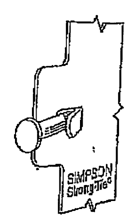
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7⁹⁄₃₂	3	6³⁄₃₂	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

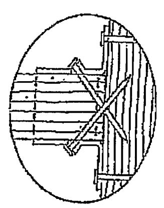


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

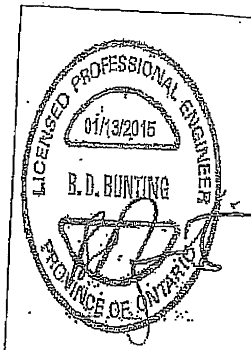
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

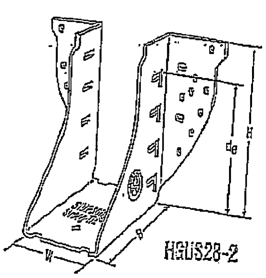


TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



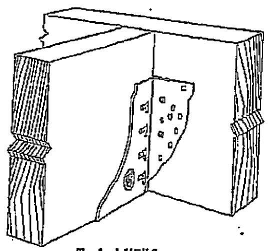
HGUS28-2

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

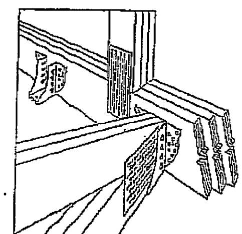

Typical HGUS Installation

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

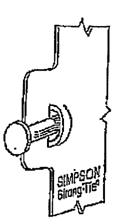
OPTIONS:

- See current catalogue for options.


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

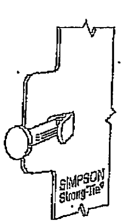
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⁵⁄₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7⁹⁄₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⁹⁄₁₆	7⁹⁄₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⁵⁄₁₆	9⁹⁄₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⁹⁄₁₆	9⁹⁄₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10845
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

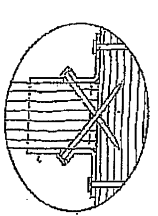


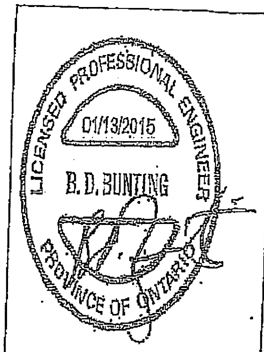
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



TECHNICAL BULLETIN

THGQ/THGQH – Truss Girder Hangers

SIMPSON Strong-Tie®

MATERIAL: THGQ—7 gauge, THGQH—3 gauge

FINISH: THGQ—G90 Galvanized,
THGQH—Simpson Strong-Tie® gray paint

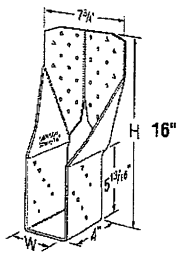
DESIGN:

- Factored resistances are in accordance with CSA O86-09
- Uplift resistances have been increased 15% for short term load duration. No further increase is allowed.
- Designer must ensure that vertical web member supporting a hanger is capable of resisting loads based on net cross section
- Girder truss must be a minimum of 2 plys
- Bearing assumes ϕFcp = 812 psi D.Fir-L and 615 psi S-P-F
- All multiple members must be fastened together to act as a single unit independent of the hanger fasteners
- Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses

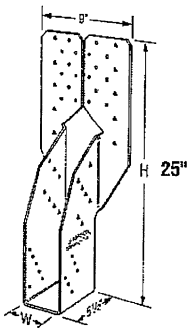
INSTALLATION:

- Use all specified fasteners
- Fill all round holes for min values and all round and triangle holes for max values
- Strong-Drive® SDS screws driven through truss plates must be approved by the truss designer. Pre-drilling, using a 5⁄32" bit, is required.
- Connector must be installed, centred on girder vertical web.

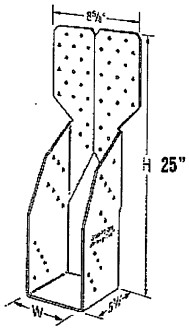
OPTIONS: • These hangers may be skewed 45°
• See current catalogue for options



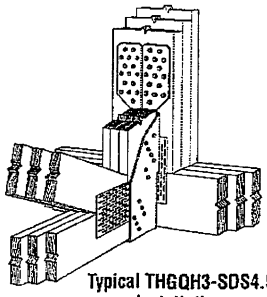
THGQ2-SDS3
(THGQ3-SDS3 Similar)



THGQH2-SDS3



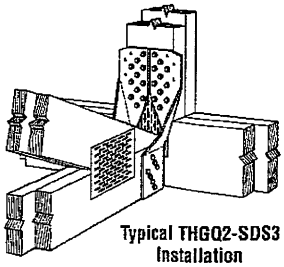
THGQH3-SDS4.5
(THGQH4-SDS6 Similar)



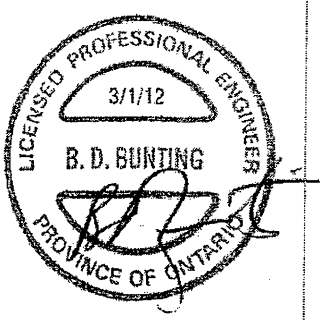
Typical THGQH3-SDS4.5 Installation

Model No.	Width W (in)	Max. B.C. Depth	Min Vert Web Size	Fasteners		Factored Resistance (lbs)			
						D.Fir-L		S-P-F	
				Header	Joist	Uplift	Normal	Uplift	Normal
						K_D=1.15	K_D=1.00	K_D=1.15	K_D=1.00
THGQ2-SDS3 (Min)	3 5⁄16	2x6	2x8	22-SDS¼"x3"	10-SDS¼"x3"	5205	11655	3750	8395
THGQ2-SDS3 (Max)	3 5⁄16	2x6	2x10	28-SDS¼"x3"	14-SDS¼"x3"	6555	18055	4720	13000
THGQH2-SDS3 (Min)	3 5⁄16	2x10	2x8	18-SDS¼"x3"	14-SDS¼"x3"	5790	12555	4170	9040
THGQH2-SDS3 (Max)	3 5⁄16	2x10	2x10	28-SDS¼"x3"	26-SDS¼"x3"	14190	18455	10215	15160
THGQ3-SDS4.5 (Min)	4 15⁄16	2x6	2x8	22-SDS¼"x4½"	10-SDS¼"x4½"	5205	11655	3750	8395
THGQ3-SDS4.5 (Max)	4 15⁄16	2x6	2x10	28-SDS¼"x4½"	14-SDS¼"x4½"	6555	17760	4720	12785
THGQH3-SDS4.5 (Min)	4 15⁄16	2x10	2x10	32-SDS¼"x4½"	14-SDS¼"x4½"	5790	17860	4170	12860
THGQH3-SDS4.5 (Max)	4 15⁄16	2x10	2x12	38-SDS¼"x4½"	26-SDS¼"x4½"	14190	21055	10215	15160
THGQH4-SDS6 (Min)	6 9⁄16	2x12	2x10	34-SDS¼"x6"	14-SDS¼"x6"	5790	17860	4170	12860
THGQH4-SDS6 (Max)	6 9⁄16	2x12	2x12	40-SDS¼"x6"	26-SDS¼"x6"	14190	24870	10215	17905

1. Factored resistances have been increased 15% for earthquake or wind load with no further increase allowed; reduce where other loads govern.
2. Minimum 2-ply girder required.
3. Connector must be installed centred on girder vertical webs.
4. The thickness of the supporting girder must be equal to or greater than the screw length. For applications where the length of the supplied screws exceeds the thickness of the supporting girder, 3" or 4½" screws may be substituted for the longer length screws with no load reduction, or a shim block may used as approved by the Designer.



Typical THGQ2-SDS3 Installation



LIMIT STATES DESIGN

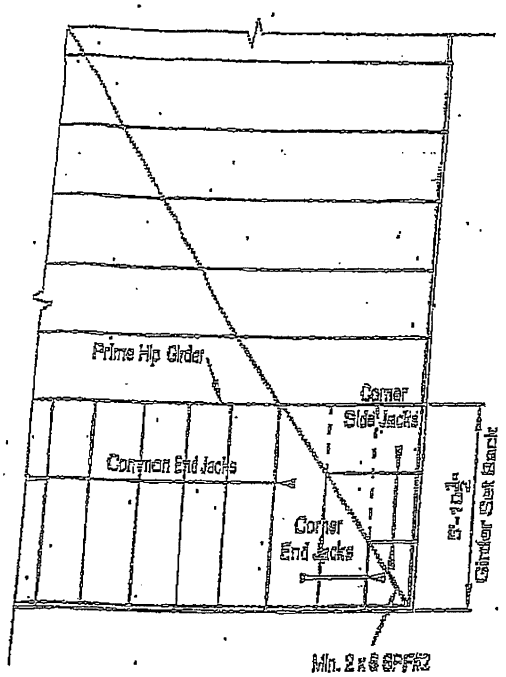
This technical bulletin is effective until December 31, 2013, and reflects information available as of February 1, 2012. This information is updated periodically and should not be relied upon after December 31, 2013; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

T-SPECTHGQ12 2/12 exp. 12/13

800-999-5099
www.strongtie.com

MICRO CITY

ENGINEERING SERVICES INC. TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

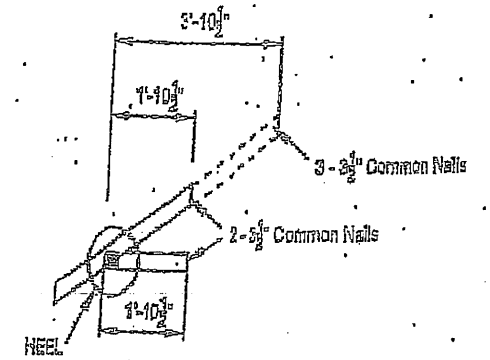
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

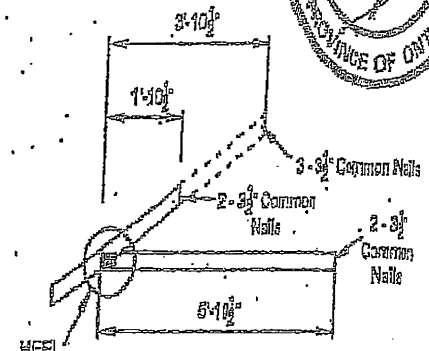
TOP CHORD LIVE LOAD	: 39.4 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F
TOTAL LOAD	59.9 P.S.F.

DWG NO TIN 2475.1A
STRUCTURAL COMPONENT ONLY

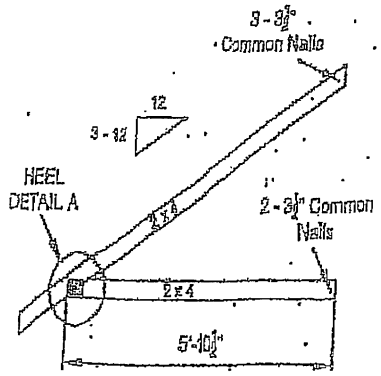
LICENSED PROFESSIONAL ENGINEER
1-20-17
S. KATSOULAKOS
PROVINCE OF ONTARIO



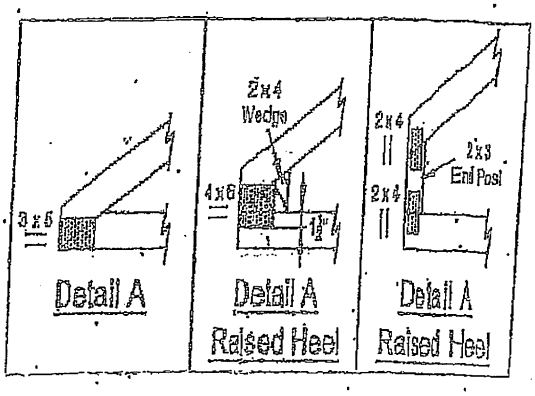
Corner Side Jacks



Corner End Jacks

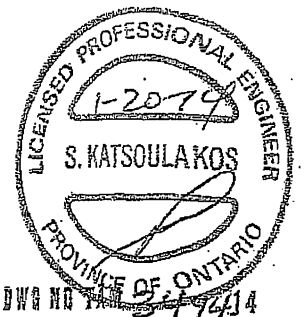


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



DWG NO [illegible] 14

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.