ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

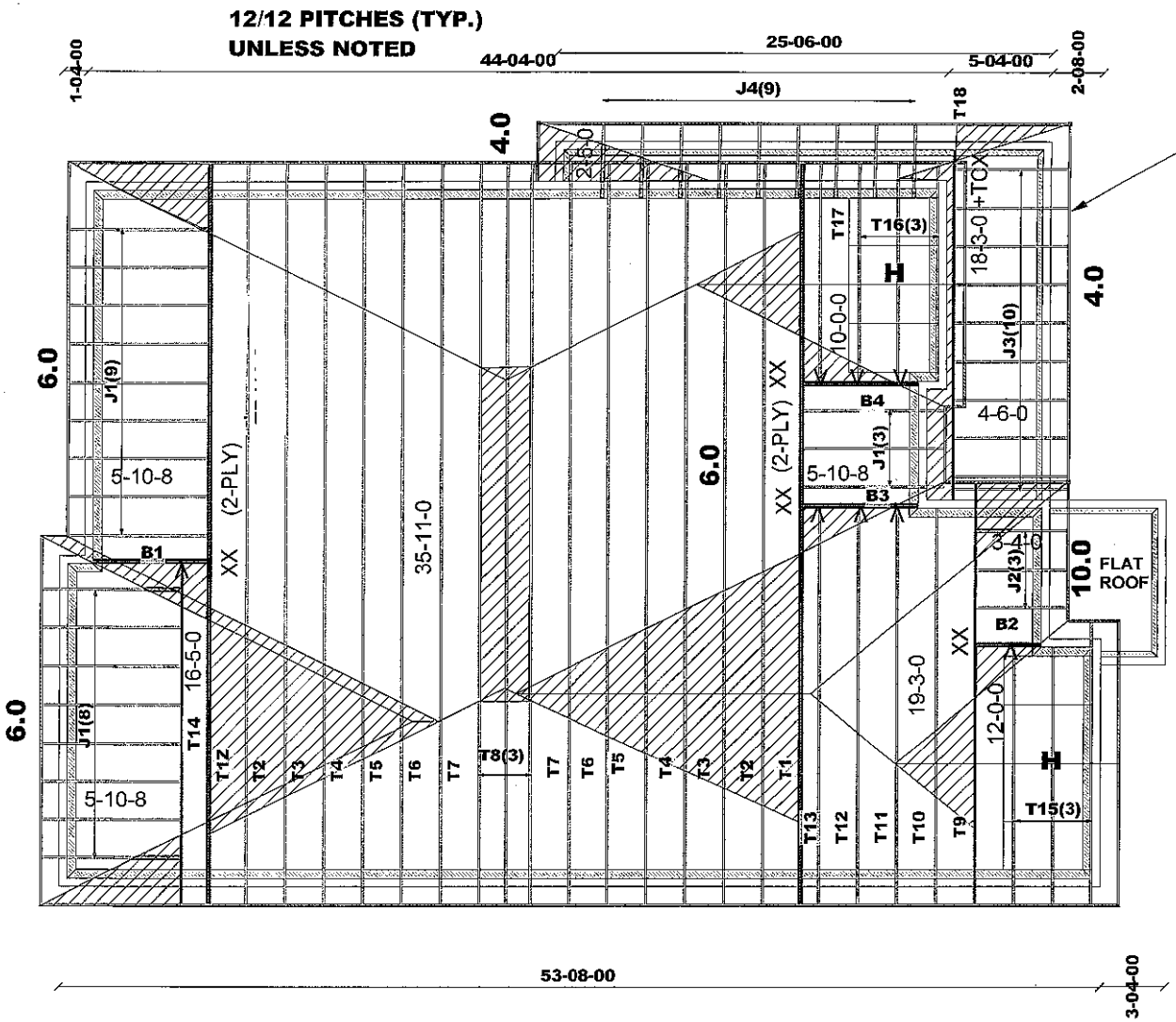
DENOTE:

H -1'-3"-0 HIGH VAULT CEILING (A.P.P)

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC,LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

Town of Innisfil Certified Model
04/01/2018 8:48:53 AM kgervais

M11,040

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track:	42067
Layout ID:	288369
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Model / Elevation: S45-3 HUMMINGBIRD 3/ A

Project: ALCONA SHORES

Date: 9/21/2017 Designer: sonny

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

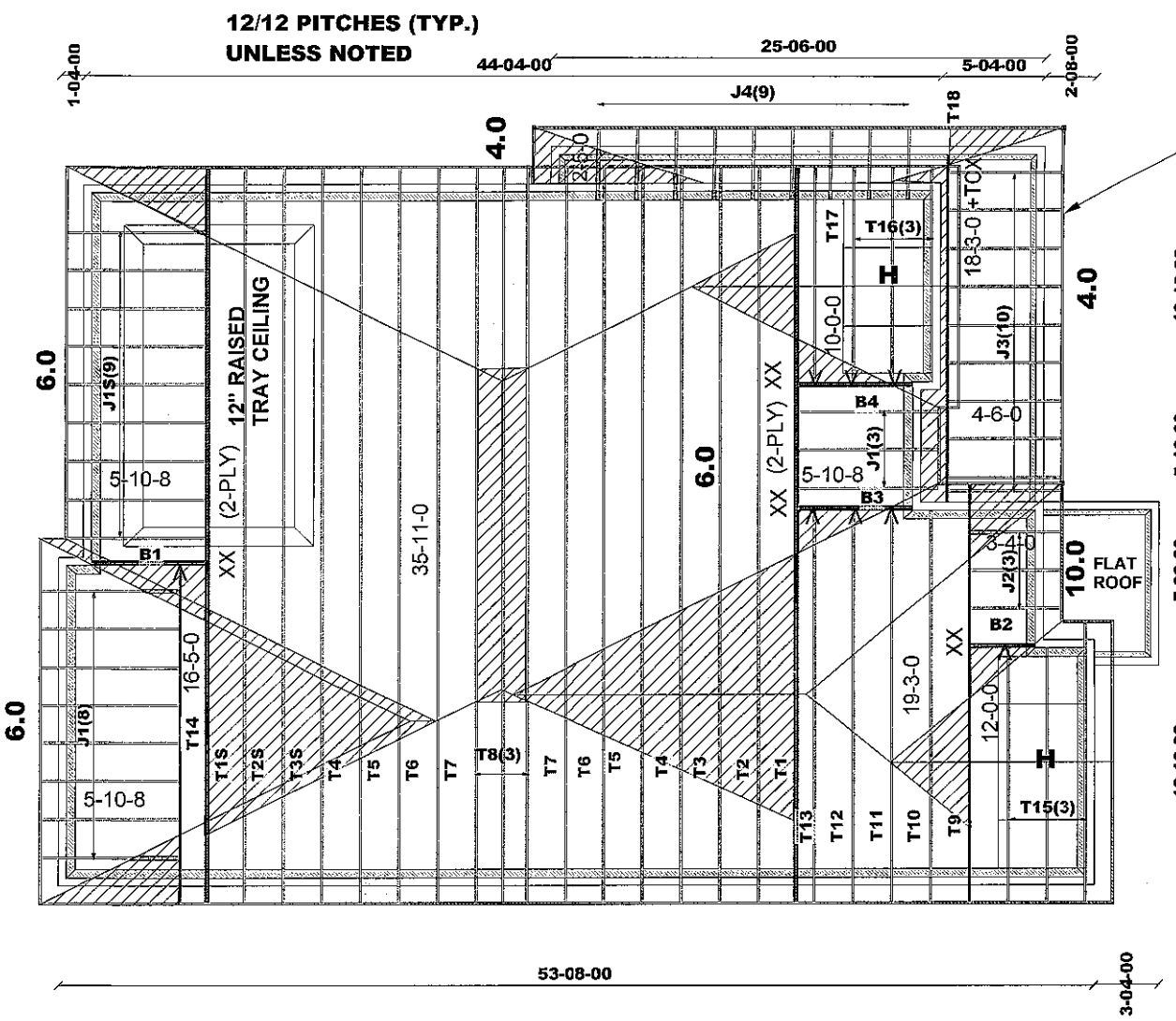
DENOTE:

H -1'-3"-0 HIGH VAULT CEILING (A.P.P)

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES: CONVENTIONAL FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
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BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD	38.3 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

Town of Innisfil Certified Model
04/01/2018 8:49:14 AM kgervais

m11,040

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	288368
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Model / Elevation: S45-3 HUMMINGBIRD 3/A+ OPT. COFF.

Project: ALCONA SHORES

Date: 9/21/2017 Designer: sonny

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

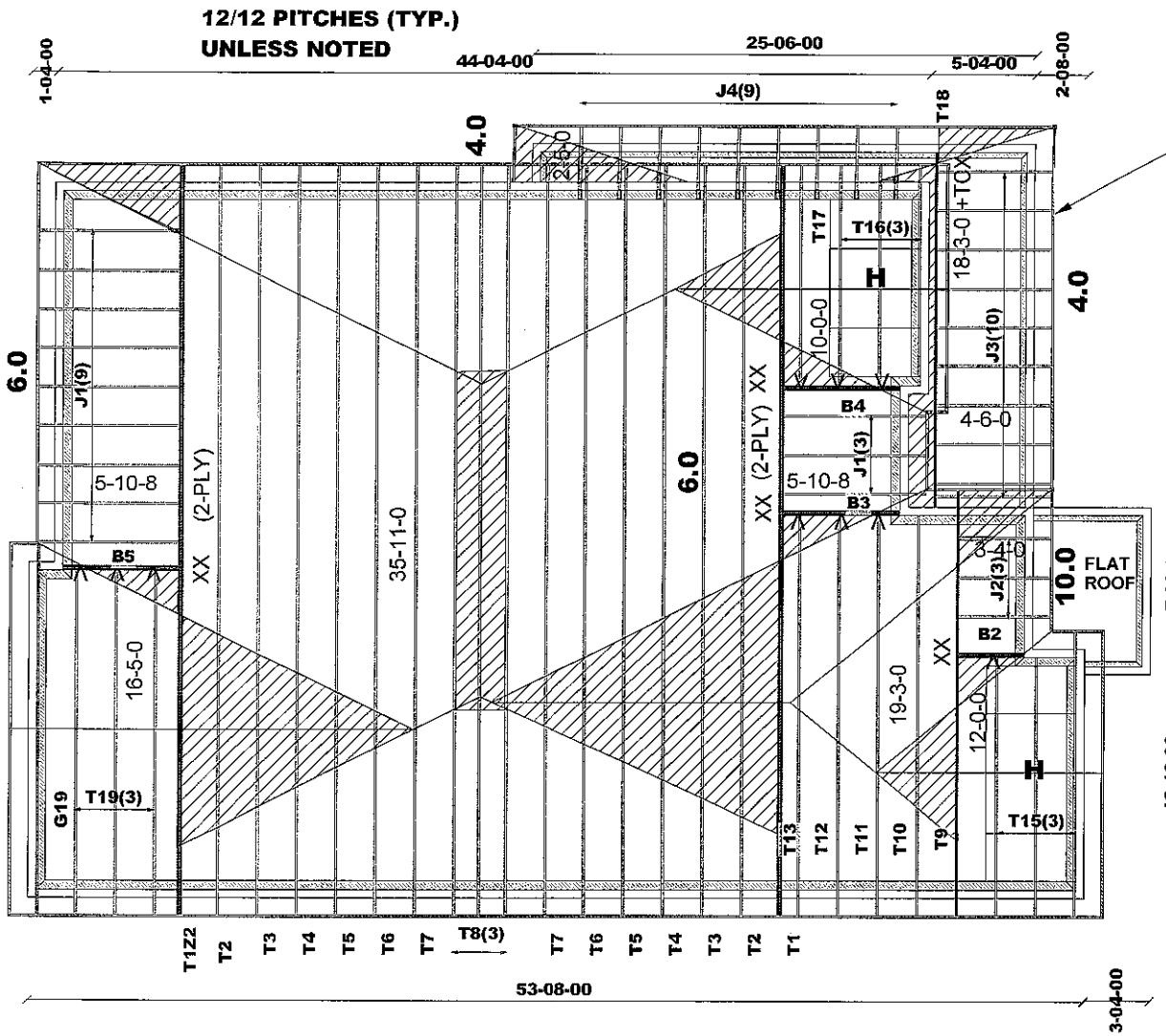
DENOTE:

H -1'-3"-0 HIGH VAULT CEILING (A.P.P)

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
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BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

Town of Innisfil Certified Model
04/01/2018 8:49:15 AM kgervais

M11,046

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	288371
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Model / Elevation: S45-3 HUMMINGBIRD 3/ A (REAR UPGRADE)

Project: ALCONA SHORES

Date: 9/21/2017 Designer: sonny

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

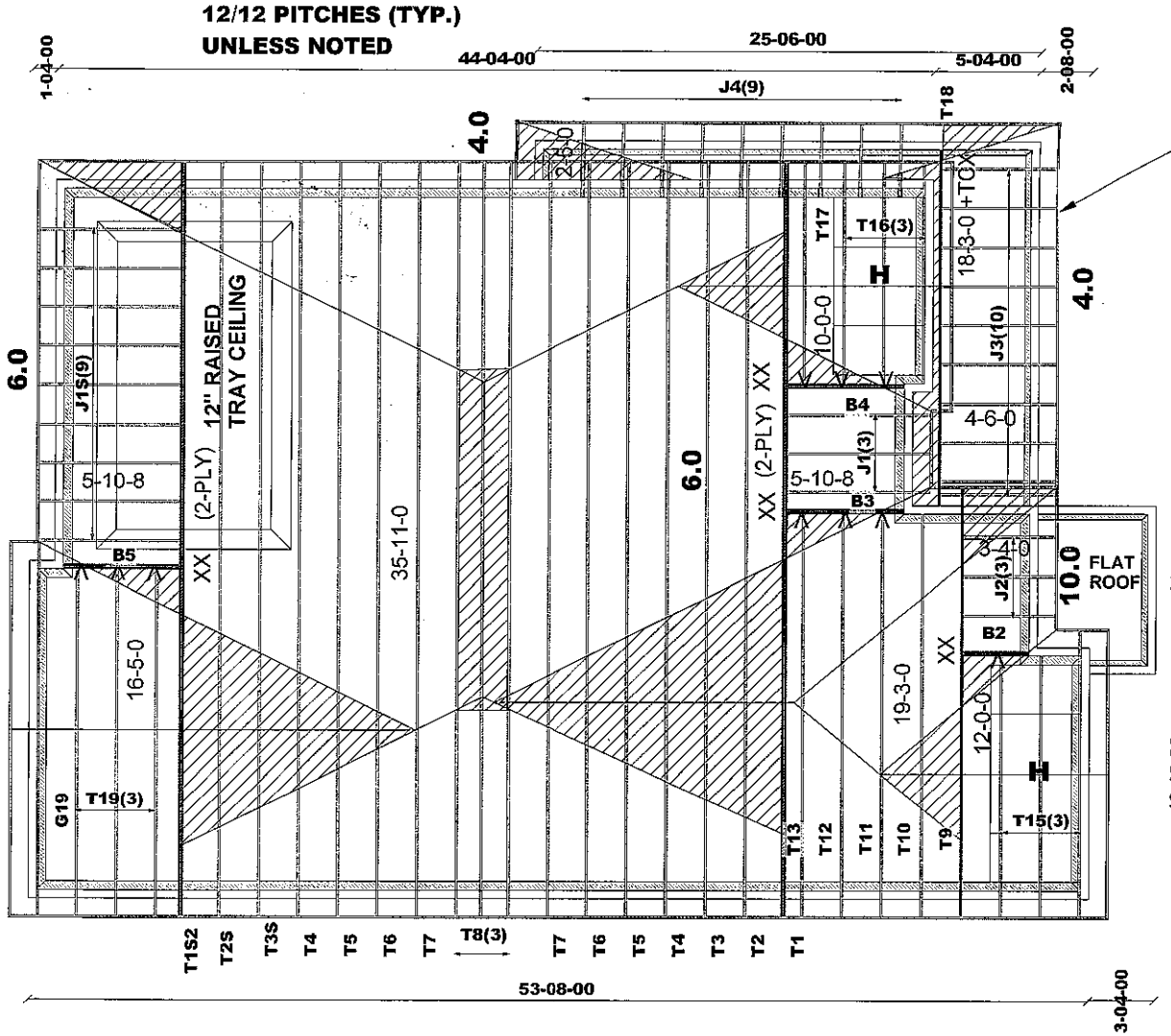
DENOTE:

H -1'-3"-0 HIGH VAULT CEILING (A.P.P)

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
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BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD	38.3 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

Town of Innisfil Certified Model
04/01/2018 8:49:17 AM kgervais

M11,040

TAMARACK LUMBER INC.

Job Track: 42067
Layout ID: 288370
Plan Log: 94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 9/21/2017 Designer: sonny

Model / Elevation: S45-3 HUMMINGBIRD 3/ A (REAR UPGRADE)+ OPT. COFF.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

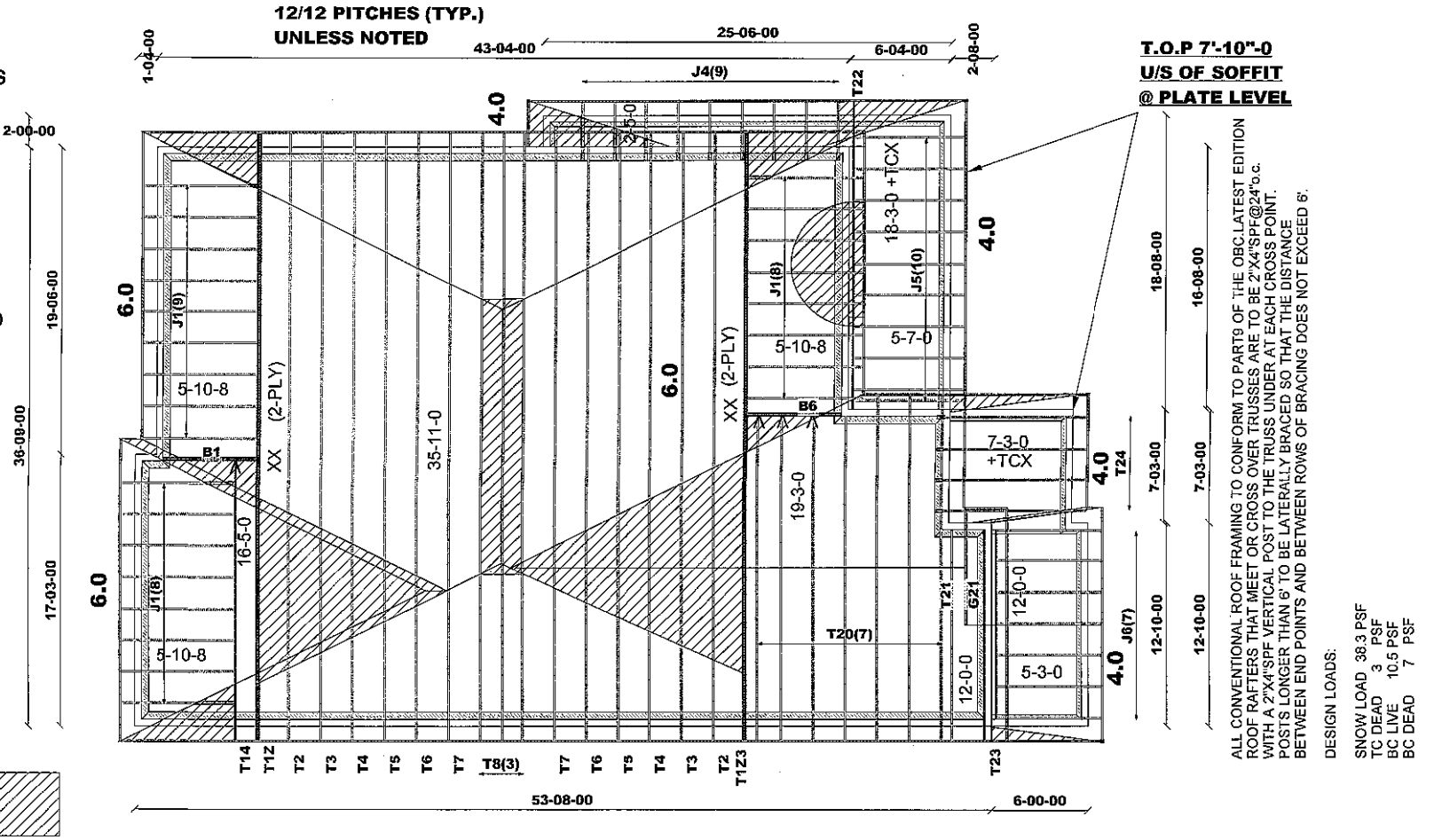
HGUS26-2 -(XX)
LJS26DS -(V)

DENOTE:

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL
FRAMING



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BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD	38.3 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

Town of Innisfil Certified Model
04/01/2018 8:49:19 AM kgervais

m11,040

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	288373
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Model / Elevation: S45-3 HUMMINGBIRD 3/B

Project: ALCONA SHORES

Date: 9/22/2017 Designer: sonny

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

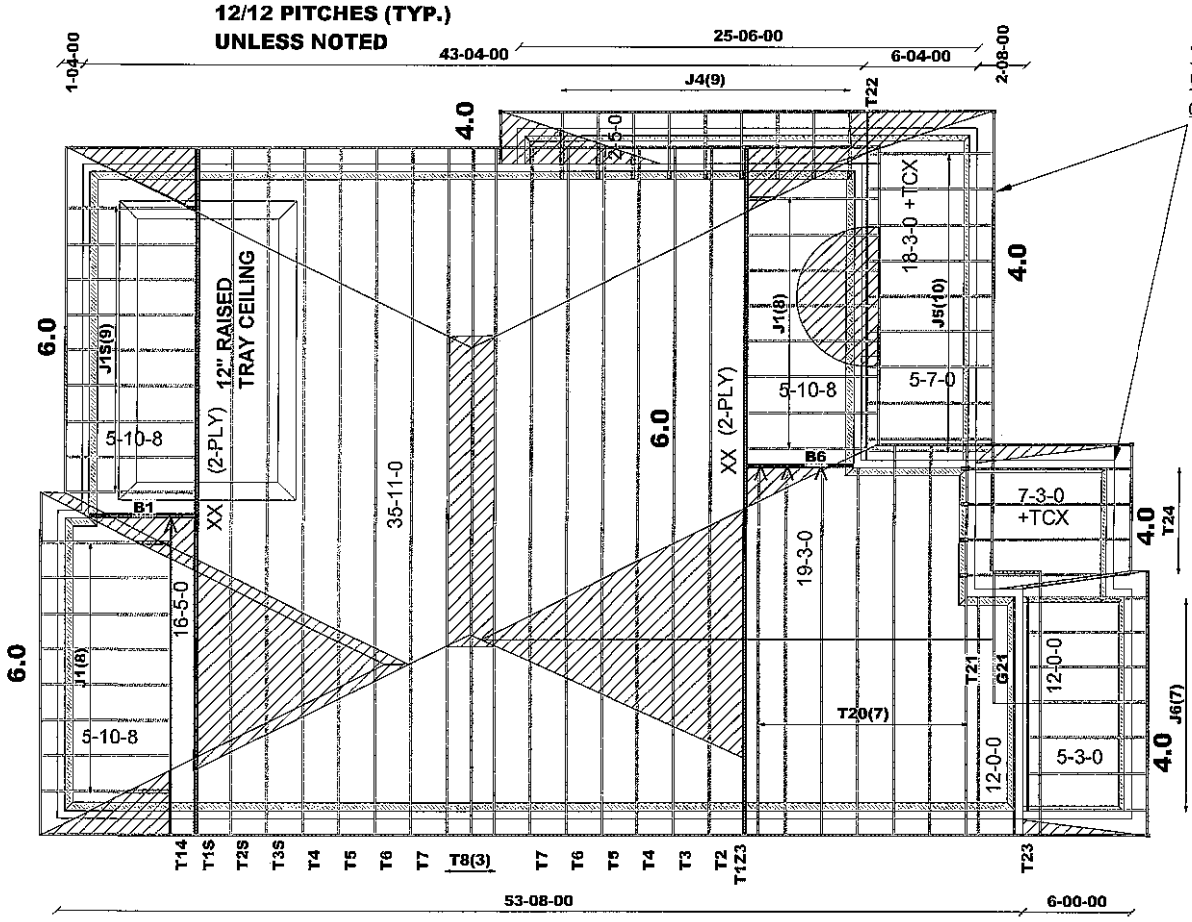
HGUS26-2 -(XX)
LJS26DS -(V)

DENOTE:

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
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DESIGN LOADS:

SNOW LOAD	38.3 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

Town of Innisfil Certified Model
04/01/2018 8:49:22 AM kgervais

M11,044.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track:	42067
Layout ID:	288372
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Model / Elevation: S45-3 HUMMINGBIRD 3/B+ OPT. COFF.

Project: ALCONA SHORES

Date: 9/22/2017 Designer: sonny

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

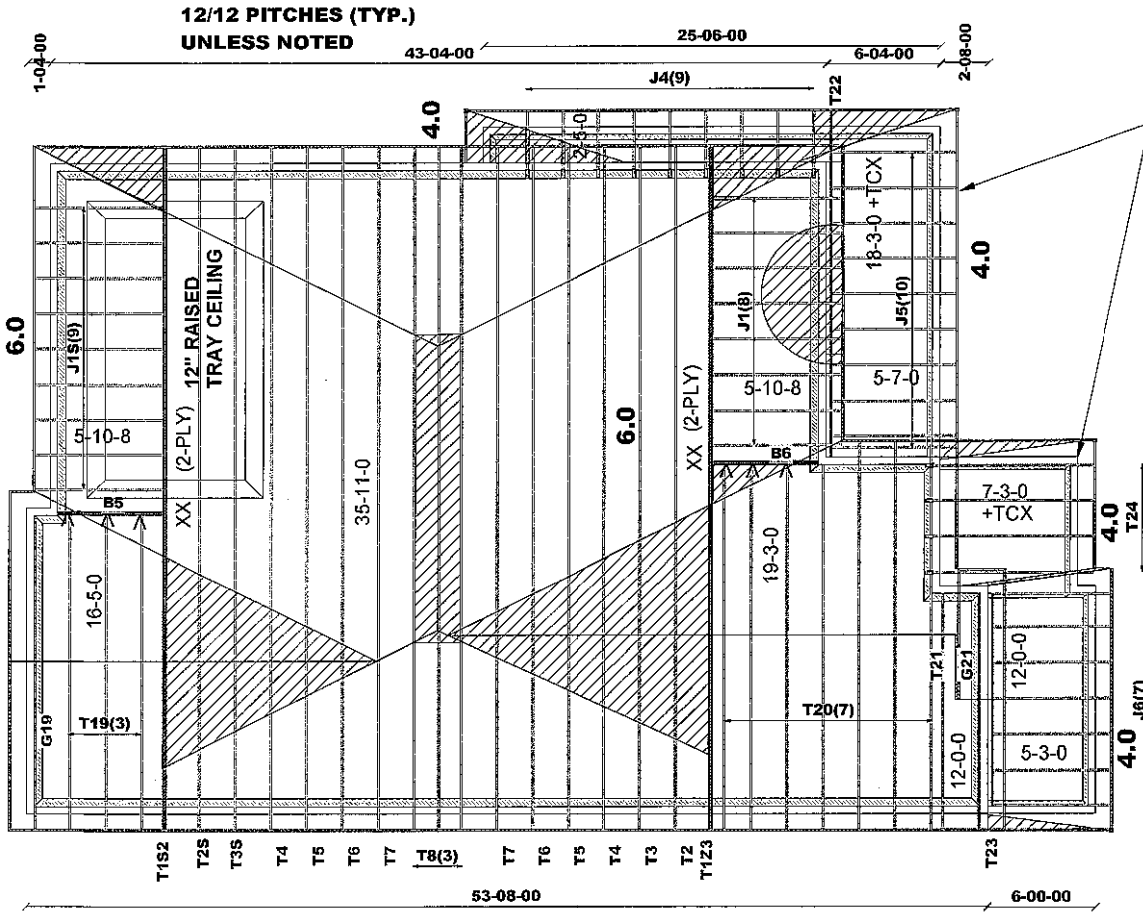
HGUS26-2 -(XX)
LJS26DS -(V)

DENOTE:

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL
FRAMING



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DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

Town of Innisfil Certified Model
04/01/2018 8:49:24 AM kgervais

MU.040

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	42067
Layout ID:	288374
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 9/22/2017 Designer: sonny

Model / Elevation: S45-3 HUMMINGBIRD 3/ B(REAR UPGRADE)+ OPT. COFF.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

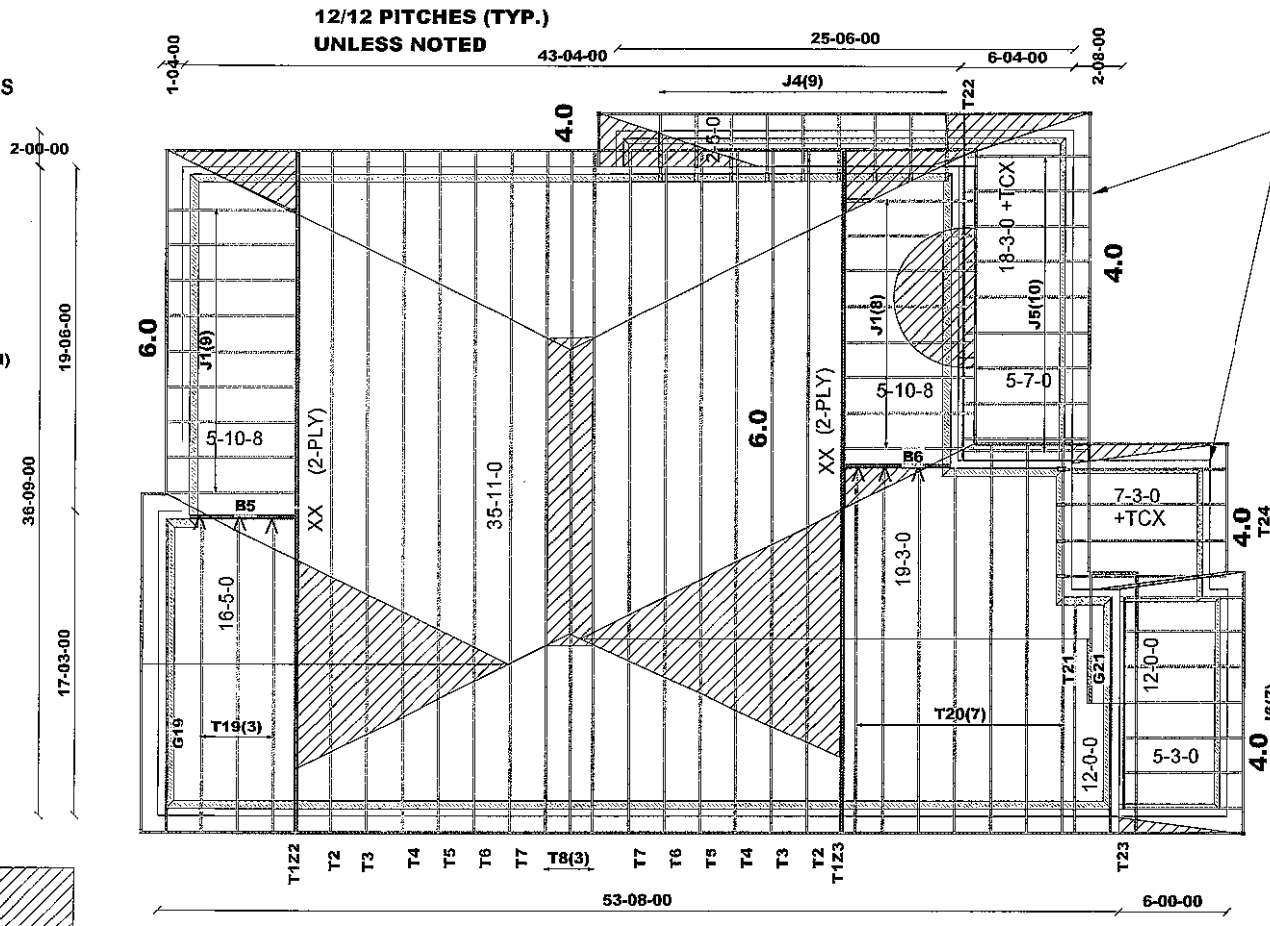
HGUS26-2 -(XX)
LJS26DS -(V)

DENOTE:

ALL B- 2-2X10 (FLUSH)

T-170681

DENOTES:
CONVENTIONAL
FRAMING



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DESIGN LOADS:

SNOW LOAD	38.3 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

Town of Innisfil Certified Model
04/01/2018 8:49:27 AM kgervais

m11,046o

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track:	42067
Layout ID:	288375
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 9/22/2017 Designer: sonny

Model / Elevation: S45-3 HUMMINGBIRD 3/ B(REAR UPGRADE)

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

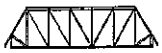
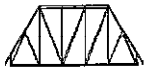
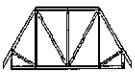
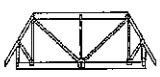
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288369 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORE SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	1 2 Ply	T1Z HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	2	T2 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	316.78 199.66		
	2	T3 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	322.52 203.34		
	2	T4 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	354.40 224.34		
	2	T5 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	353.28 221.34		
	2	T6 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	371.42 231.00		
	2	T7 HIP	12.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	383.52 236.00		
	3	T8 HIP	12.00 0.00	35-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	657.36 404.01		
	1	T9 HIP GIRDER	12.00 0.00	19-03-00	04-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	99.18 62.67		
	1	T10 HIP	12.00 0.00	19-03-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	88.53 57.33		
	1	T11 HIP	12.00 0.00	19-03-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	98.65 63.00		
	1	T12 HIP	12.00 0.00	19-03-00	09-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	101.50 64.67		
	1	T13 HIP	12.00 0.00	19-03-00	11-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	120.43 76.33		
	1	T14 HIP GIRDER	12.00 0.00	16-05-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	90.35 58.00		
	3	T15 ROOF	12.00 6.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	193.65 123.99		
	3	T16 ROOF	12.00 6.00	10-00-00	06-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	170.70 113.01		
	1	T17 COMMON	12.00 0.00	10-00-00	06-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	48.59 31.17		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

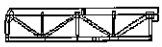
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288369 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T18 HALF HIP	0.00 0.00	18-03-00	02-05-03	2 X 6	2 X 4	01-03-08 00-00-00	00-03-08 02-05-03	81.64 50.83		
	20	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	335.80 213.40		
	3	J2 JACK-OPEN	10.00 0.00	03-04-00	04-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-05-00	36.99 26.01		
	10	J3 JACK-OPEN	4.00 0.00	04-06-00	02-05-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-05-03	127.00 86.70		
	9	J4 JACK-OPEN	12.00 0.00	02-05-00	04-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-10-07	96.66 65.97		

TOTAL # TRUSS= **75.00** TOTAL BFT OF ALL TRUSSES= **3367.45** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5344.11** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

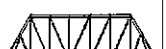
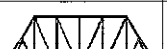
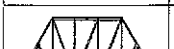
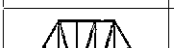
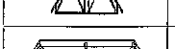
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288368 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: A + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	1 2 Ply	T1S HIP GIRDER	12.00 12.00	35-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	442.04 276.66		
	1	T2 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	158.39 99.83		
	1	T2S HIP	12.00 12.00	35-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	209.15 129.17		
	1	T3 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	161.26 101.67		
	1	T3S HIP	12.00 12.00	35-11-00	06-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	210.53 130.50		
	2	T4 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	354.40 224.34		
	2	T5 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	353.28 221.34		
	2	T6 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	371.42 231.00		
	2	T7 HIP	12.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	383.52 236.00		
	3	T8 HIP	12.00 0.00	35-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	657.36 404.01		
	1	T9 HIP GIRDER	12.00 0.00	19-03-00	04-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	99.18 62.67		
	1	T10 HIP	12.00 0.00	19-03-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	88.53 57.33		
	1	T11 HIP	12.00 0.00	19-03-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	98.65 63.00		
	1	T12 HIP	12.00 0.00	19-03-00	09-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	101.50 64.67		
	1	T13 HIP	12.00 0.00	19-03-00	11-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	120.43 76.33		
	1	T14 HIP GIRDER	12.00 0.00	16-05-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	90.35 58.00		
	3	T15 ROOF	12.00 6.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	193.65 123.99		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

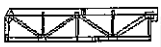
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288368 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES **SUB-BUILDER:**
MODEL: S45-3 HUMMINGBIRD 3 **ELEVATION:** A + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	T16 ROOF	12.00 6.00	10-00-00	06-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	170.70 113.01		
	1	T17 COMMON	12.00 0.00	10-00-00	06-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	48.59 31.17		
	1	T18 HALF HIP	0.00 0.00	18-03-00	02-05-03	2 X 6	2 X 4	01-03-08 00-00-00	00-03-08 02-05-03	81.64 50.83		
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	9	J1S JACK-OPEN	6.00 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	208.17 148.50		
	3	J2 JACK-OPEN	10.00 0.00	03-04-00	04-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-05-00	36.99 26.01		
	10	J3 JACK-OPEN	4.00 0.00	04-06-00	02-05-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-05-03	127.00 86.70		
	9	J4 JACK-OPEN	12.00 0.00	02-05-00	04-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-10-07	96.66 65.97		

TOTAL # TRUSS= **75** TOTAL BFT OF ALL TRUSSES= **3477.41** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5495.66** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **12**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

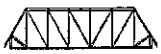
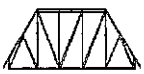
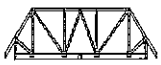
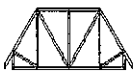
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288371 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: A (REAR UPGRADE)

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	1 2 Ply	T1Z2 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	2	T2 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	316.78 199.66		
	2	T3 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	322.52 203.34		
	2	T4 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	354.40 224.34		
	2	T5 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	353.28 221.34		
	2	T6 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	371.42 231.00		
	2	T7 HIP	12.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	383.52 236.00		
	3	T8 HIP	12.00 0.00	35-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	657.36 404.01		
	1	T9 HIP GIRDER	12.00 0.00	19-03-00	04-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	99.18 62.67		
	1	T10 HIP	12.00 0.00	19-03-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	88.53 57.33		
	1	T11 HIP	12.00 0.00	19-03-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	98.65 63.00		
	1	T12 HIP	12.00 0.00	19-03-00	09-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	101.50 64.67		
	1	T13 HIP	12.00 0.00	19-03-00	11-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	120.43 76.33		
	3	T19 COMMON	12.00 0.00	16-05-00	10-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	247.56 155.01		
	1	G19 GABLE	12.00 0.00	16-05-00	10-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	91.60 58.67		
	3	T15 ROOF	12.00 6.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	193.65 123.99		
	3	T16 ROOF	12.00 6.00	10-00-00	06-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	170.70 113.01		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288371 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES **SUB-BUILDER:**
MODEL: S45-3 HUMMINGBIRD 3 **ELEVATION:** A (REAR UPGRADE)

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T17 COMMON	12.00 0.00	10-00-00	06-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	48.59 31.17		
	1	T18 HALF HIP	0.00 0.00	18-03-00	02-05-03	2 X 6	2 X 4	01-03-08 00-00-00	00-03-08 02-05-03	81.64 50.83		
	12	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04		
	3	J2 JACK-OPEN	10.00 0.00	03-04-00	04-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-05-00	36.99 26.01		
	10	J3 JACK-OPEN	4.00 0.00	04-06-00	02-05-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-05-03	127.00 86.70		
	9	J4 JACK-OPEN	12.00 0.00	02-05-00	04-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-10-07	96.66 65.97		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **3437.77** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5458.60** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
10	Hangers	LJS26DS	

TOTAL # ITEMS= **14.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

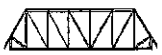
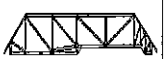
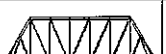
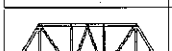
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288370 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES **SUB-BUILDER:**
MODEL: S45-3 HUMMINGBIRD 3 **ELEVATION:** A (REAR UPGRADE) +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	1 2 Ply	T1S2 HIP GIRDER	12.00 12.00	35-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	442.04 276.66		
	1	T2 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	158.39 99.83		
	1	T2S HIP	12.00 12.00	35-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	209.15 129.17		
	1	T3 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	161.26 101.67		
	1	T3S HIP	12.00 12.00	35-11-00	06-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	212.17 131.83		
	2	T4 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	354.40 224.34		
	2	T5 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	353.28 221.34		
	2	T6 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	371.42 231.00		
	2	T7 HIP	12.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	383.52 236.00		
	3	T8 HIP	12.00 0.00	35-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	657.36 404.01		
	1	T9 HIP GIRDER	12.00 0.00	19-03-00	04-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	99.18 62.67		
	1	T10 HIP	12.00 0.00	19-03-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	88.53 57.33		
	1	T11 HIP	12.00 0.00	19-03-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	98.65 63.00		
	1	T12 HIP	12.00 0.00	19-03-00	09-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	101.50 64.67		
	1	T13 HIP	12.00 0.00	19-03-00	11-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	120.43 76.33		
	3	T19 COMMON	12.00 0.00	16-05-00	10-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	247.56 155.01		
	1	G19 GABLE	12.00 0.00	16-05-00	10-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	91.60 58.67		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

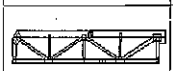
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288370 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOWS SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: A (REAR UPGRADE) +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	T15 ROOF	12.00 / 6.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	193.65 / 123.99		
	3	T16 ROOF	12.00 / 6.00	10-00-00	06-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	170.70 / 113.01		
	1	T17 COMMON	12.00 / 0.00	10-00-00	06-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	48.59 / 31.17		
	1	T18 HALF HIP	0.00 / 0.00	18-03-00	02-05-03	2 X 6	2 X 4	01-03-08 / 00-00-00	00-03-08 / 02-05-03	81.64 / 50.83		
	3	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	50.37 / 32.01		
	9	J1S JACK-OPEN	6.00 / 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	208.17 / 148.50		
	3	J2 JACK-OPEN	10.00 / 0.00	03-04-00	04-05-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-05-00	36.99 / 26.01		
	10	J3 JACK-OPEN	4.00 / 0.00	04-06-00	02-05-03	2 X 4	2 X 4	01-03-08 / 00-00-00	00-11-03 / 02-05-03	127.00 / 86.70		
	9	J4 JACK-OPEN	12.00 / 0.00	02-05-00	04-03-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 00-10-07	96.66 / 65.97		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **3549.06** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5611.79** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
10	Hangers	LJS26DS	

TOTAL # ITEMS= **14.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

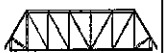
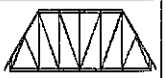
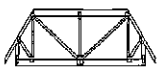
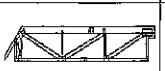
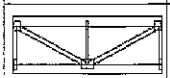
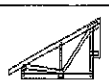
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288373 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 / 2 Ply	T1Z HIP GIRDER	12.00 / 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	447.58 / 277.34		
	1 / 2 Ply	T1Z3 HIP GIRDER	12.00 / 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	447.58 / 277.34		
	2	T2 HIP	12.00 / 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	316.78 / 199.66		
	2	T3 HIP	12.00 / 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	322.52 / 203.34		
	2	T4 HIP	12.00 / 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	354.40 / 224.34		
	2	T5 HIP	12.00 / 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	353.28 / 221.34		
	2	T6 HIP	12.00 / 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	371.42 / 231.00		
	2	T7 HIP	12.00 / 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	383.52 / 236.00		
	3	T8 HIP	12.00 / 0.00	35-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	657.36 / 404.01		
	7	T20 GABLE	12.00 / 0.00	19-03-00	11-03-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	723.45 / 452.69		
	1	T14 HIP GIRDER	12.00 / 0.00	16-05-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	90.35 / 58.00		
	1	T21 COMMON	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	63.57 / 41.33		
	1	G21 GABLE	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	60.98 / 39.33		
	1	T22 HALF HIP	12.00 / 0.00	18-03-00	02-09-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 02-09-08	89.33 / 57.17		
	1	T23 FLAT GIRDER	0.00 / 0.00	12-00-00	02-08-03	2 X 6	2 X 6	00-00-00 / 00-00-00	02-08-03 / 02-08-03	58.04 / 33.67		
	4	T24 MONOPITCH	4.00 / 0.00	08-00-04	03-07-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-11-03 / 03-07-04	147.56 / 98.68		
	25	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	419.75 / 266.75		
	9	J4 JACK-OPEN	12.00 / 0.00	02-05-00	04-03-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 00-10-07	96.66 / 65.97		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288373 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	10	J5 JACK-OPEN	4.00 0.00	05-07-00	02-09-08	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-09-08	152.30 100.00		
	7	J6 JACK-OPEN	4.00 0.00	05-03-00	02-08-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-08-03	101.22 65.31		

TOTAL # TRUSS= **86.00** TOTAL BFT OF ALL TRUSSES= **3553.27** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5657.65** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **6.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

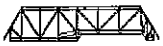
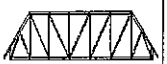
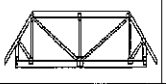
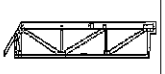
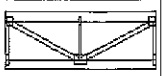
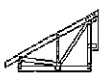
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288372 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: B +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1Z3 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	1 2 Ply	T1S HIP GIRDER	12.00 12.00	35-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	442.04 276.66		
	1	T2 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	158.39 99.83		
	1	T2S HIP	12.00 12.00	35-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	209.15 129.17		
	1	T3 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	161.26 101.67		
	1	T3S HIP	12.00 12.00	35-11-00	06-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	212.17 131.83		
	2	T4 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	354.40 224.34		
	2	T5 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	353.28 221.34		
	2	T6 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	371.42 231.00		
	2	T7 HIP	12.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	383.52 236.00		
	3	T8 HIP	12.00 0.00	35-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	657.36 404.01		
	7	T20 GABLE	12.00 0.00	19-03-00	11-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	723.45 452.69		
	1	T14 HIP GIRDER	12.00 0.00	16-05-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	90.35 58.00		
	1	T21 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	63.57 41.33		
	1	G21 GABLE	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.98 39.33		
	1	T22 HALF HIP	12.00 0.00	18-03-00	02-09-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-09-08	89.33 57.17		
	1	T23 FLAT GIRDER	0.00 0.00	12-00-00	02-08-03	2 X 6	2 X 6	00-00-00 00-00-00	02-08-03 02-08-03	58.04 33.67		
	4	T24 MONOPITCH	4.00 0.00	08-00-04	03-07-04	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 03-07-04	147.56 98.68		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288372 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: B +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	9	J1S JACK-OPEN	6.00 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	208.17 148.50		
	9	J4 JACK-OPEN	12.00 0.00	02-05-00	04-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-10-07	96.66 65.97		
	10	J5 JACK-OPEN	4.00 0.00	05-07-00	02-09-08	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-09-08	152.30 100.00		
	7	J6 JACK-OPEN	4.00 0.00	05-03-00	02-08-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-08-03	101.22 65.31		

TOTAL # TRUSS= **86.00** TOTAL BFT OF ALL TRUSSES= **3664.56** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5810.84** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **6.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

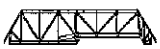
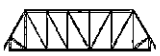
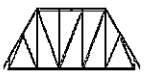
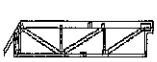
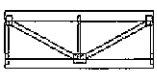
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288374 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORE SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: B (REAR UPGRADE)+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1Z3 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	1 2 Ply	T1S2 HIP GIRDER	12.00 12.00	35-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	442.04 276.66		
	1	T2 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	158.39 99.83		
	1	T2S HIP	12.00 12.00	35-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	209.15 129.17		
	1	T3 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	161.26 101.67		
	1	T3S HIP	12.00 12.00	35-11-00	06-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	212.17 131.83		
	2	T4 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	354.40 224.34		
	2	T5 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	353.28 221.34		
	2	T6 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	371.42 231.00		
	2	T7 HIP	12.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	383.52 236.00		
	3	T8 HIP	12.00 0.00	35-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	657.36 404.01		
	7	T20 GABLE	12.00 0.00	19-03-00	11-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	723.45 452.69		
	3	T19 COMMON	12.00 0.00	16-05-00	10-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	247.56 155.01		
	1	G19 GABLE	12.00 0.00	16-05-00	10-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	91.60 58.67		
	1	T21 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	63.57 41.33		
	1	G21 GABLE	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.98 39.33		
	1	T22 HALF HIP	12.00 0.00	18-03-00	02-09-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-09-08	89.33 57.17		
	1	T23 FLAT GIRDER	0.00 0.00	12-00-00	02-08-03	2 X 6	2 X 6	00-00-00 00-00-00	02-08-03 02-08-03	58.04 33.67		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

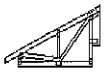
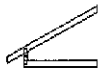
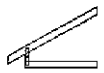
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288374 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES **SUB-BUILDER:**
MODEL: S45-3 HUMMINGBIRD 3 **ELEVATION:** B (REAR UPGRADE)+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	T24 MONOPITCH	4.00 0.00	08-00-04	03-07-04	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 03-07-04	147.56 98.68		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	9	J1S JACK-OPEN	6.00 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	208.17 148.50		
	9	J4 JACK-OPEN	12.00 0.00	02-05-00	04-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-10-07	96.66 65.97		
	10	J5 JACK-OPEN	4.00 0.00	05-07-00	02-09-08	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-09-08	152.30 100.00		
	7	J6 JACK-OPEN	4.00 0.00	05-03-00	02-08-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-08-03	101.22 65.31		

TOTAL # TRUSS= **81.00** TOTAL BFT OF ALL TRUSSES= **3734.88** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5925.33** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	

TOTAL # ITEMS= **8.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

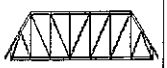
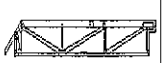
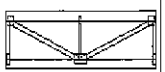
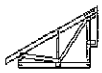
Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288375 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: B (REAR UPGRADE)

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1Z2 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	1 2 Ply	T1Z3 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	447.58 277.34		
	2	T2 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	316.78 199.66		
	2	T3 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	322.52 203.34		
	2	T4 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	354.40 224.34		
	2	T5 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	353.28 221.34		
	2	T6 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	371.42 231.00		
	2	T7 HIP	12.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	383.52 236.00		
	3	T8 HIP	12.00 0.00	35-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	657.36 404.01		
	7	T20 GABLE	12.00 0.00	19-03-00	11-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	723.45 452.69		
	3	T19 COMMON	12.00 0.00	16-05-00	10-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	247.56 155.01		
	1	G19 GABLE	12.00 0.00	16-05-00	10-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	91.60 58.67		
	1	T21 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	63.57 41.33		
	1	G21 GABLE	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.98 39.33		
	1	T22 HALF HIP	12.00 0.00	18-03-00	02-09-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-09-08	89.33 57.17		
	1	T23 FLAT GIRDER	0.00 0.00	12-00-00	02-08-03	2 X 6	2 X 6	00-00-00 00-00-00	02-08-03 02-08-03	58.04 33.67		
	4	T24 MONOPITCH	4.00 0.00	08-00-04	03-07-04	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 03-07-04	147.56 98.68		
	17	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	285.43 181.39		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/22/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288375 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-3 HUMMINGBIRD 3 ELEVATION: B (REAR UPGRADE)

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	9	J4 JACK-OPEN	12.00 0.00	02-05-00	04-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-10-07	96.66 65.97		
	10	J5 JACK-OPEN	4.00 0.00	05-07-00	02-09-08	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-09-08	152.30 100.00		
	7	J6 JACK-OPEN	4.00 0.00	05-03-00	02-08-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-08-03	101.22 65.31		

TOTAL # TRUSS= **81.00** TOTAL BFT OF ALL TRUSSES= **3623.59** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5772.14** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	

TOTAL # ITEMS= **8.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Fri Sep 22 13:04:02 2017 Page 1
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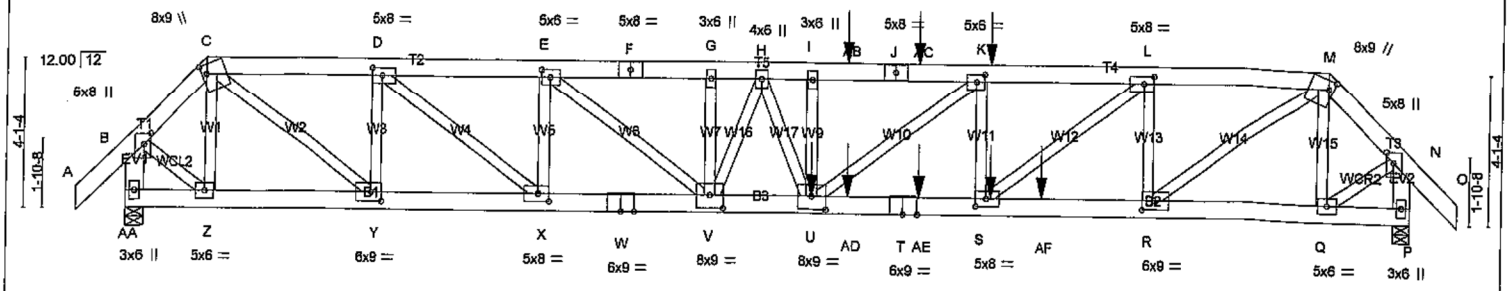

TOTAL WEIGHT = 2 X 224 = 448 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
M - O	2x6	DRY	No.2	SPF
AA- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AA- W	2x6	DRY	1650F 1.5E	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- J	2	12	SIDE(0.0)
J- M	2	12	SIDE(183.1)
M- O	2	12	TOP
AA- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AA- W	2	12	TOP
W- T	2	12	SIDE(183.1)
T- P	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
I- U	1	3	SIDE(573.4)
2x4	1	6	
G- V	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	3.50	2.25
C	TTWW+m	MT20	8.0	9.0	3.00	1.50
D	TMWW-t	MT20	5.0	8.0	2.50	3.50
E	TMWW-t	MT20	5.0	6.0	2.50	2.75
F	TS-t	MT20	5.0	8.0		
G	TMW+w	MT20	3.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMW+w	MT20	3.0	6.0		
J	TS-t	MT20	5.0	8.0		
K	TMWW-t	MT20	5.0	6.0	2.50	2.75
L	TMWW-t	MT20	5.0	8.0	2.50	3.50
M	TTWW+m	MT20	8.0	9.0	3.00	1.50
N	TMVW+p	MT20	5.0	8.0	3.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AA	4935	0	4935	0	0	5-8	5-8
P	6089	0	6089	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	3798	2579 / 0	610 / 0	0 / 0	0 / 0	609 / 0	0 / 0
P	4672	3198 / 0	733 / 0	0 / 0	0 / 0	740 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM	TO	MAX CSI (LC)
A- B	0 / 63	-122.2	-122.2	0.05 (1)
B- C	-4221 / 0	-122.2	-122.2	0.05 (1)
C- D	-8282 / 0	-122.2	-122.2	0.23 (1)
D- E	-12556 / 0	-122.2	-122.2	0.36 (1)
E- F	-15903 / 0	-122.2	-122.2	0.58 (1)
F- G	-15903 / 0	-122.2	-122.2	0.58 (1)
G- H	-15903 / 0	-122.2	-122.2	0.47 (1)
H- I	-17411 / 0	-122.2	-122.2	0.56 (1)
I-AB	-17411 / 0	-122.2	-122.2	0.75 (1)
AB- J	-17411 / 0	-122.2	-122.2	0.75 (1)
J-AC	-17411 / 0	-122.2	-122.2	0.75 (1)
AC- K	-17411 / 0	-122.2	-122.2	0.75 (1)
K- L	-15649 / 0	-122.2	-122.2	0.53 (1)
L- M	-10647 / 0	-122.2	-122.2	0.33 (1)
M- N	-5223 / 0	-122.2	-122.2	0.06 (1)
N- O	0 / 63	-122.2	-122.2	0.05 (1)
AA- B	-4974 / 0	0.0	0.0	0.18 (1)
P- N	-6079 / 0	0.0	0.0	0.23 (1)

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	MAX CSI (LC)
Z- C	-1421 / 0	10.00	0.12 (1)
C- Y	0 / 6685	5.54	0.59 (1)
Y- D	-3678 / 0	4.08	0.33 (1)
D- X	0 / 5405	3.29	0.48 (1)
X- E	-3170 / 0	2.74	0.27 (1)
E- V	0 / 4233	2.74	0.37 (1)
V- G	-402 / 0	2.83	0.03 (1)
U- I	-579 / 4	2.63	0.05 (1)
U- K	0 / 2227	2.44	0.20 (1)
S- K	-2236 / 0	2.44	0.19 (1)
S- L	0 / 6327	2.44	0.56 (1)
R- L	-4365 / 0	2.44	0.38 (1)
R- M	0 / 8778	2.82	0.78 (1)
Q- M	-1962 / 0	3.57	0.17 (1)
B- Z	0 / 3380	5.08	0.30 (1)
Q- N	0 / 4182	10.00	0.37 (1)
V- H	-2111 / 0	6.52	0.20 (1)
H- U	0 / 2050	6.00	0.18 (1)

AA- Z	0 / 0	-28.0	-28.0	0.04 (1)	10.00
Z- Y	0 / 2886	-28.0	-28.0	0.16 (1)	10.00
Y- X	0 / 8282	-28.0	-28.0	0.38 (1)	10.00
X- W	0 / 12556	-28.0	-28.0	0.56 (1)	10.00
W- V	0 / 12556	-28.0	-28.0	0.58 (1)	10.00
V- U	0 / 16668	-28.0	-28.0	0.80 (1)	10.00
U-AD	0 / 15649	-28.0	-28.0	0.79 (1)	10.00
AD- T	0 / 15649	-28.0	-28.0	0.79 (1)	10.00
T-AE	0 / 15649	-28.0	-28.0	0.79 (1)	10.00
AE- S	0 / 15649	-28.0	-28.0	0.79 (1)	10.00
S-AF	0 / 16846	-28.0	-28.0	0.82 (1)	10.00
AF- R	0 / 16846	-28.0	-28.0	0.82 (1)	10.00
R- Q	0 / 3574	-28.0	-28.0	0.24 (1)	10.00
Q- P	0 / 0	-28.0	-28.0	0.02 (2)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	24-4-4	-147	-147	—	FRONT	VERT	TOTAL
S	24-4-4	-40	-70	---	FRONT	VERT	TOTAL
U	19-4-8	-2900	-2900	—	FRONT	VERT	TOTAL
AB	20-4-4	-147	-147	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 921 (0.47")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 611 (0.71")

CSI: TC=0.75 (I-K:1) , BC=0.82 (R-S:1) , WB=0.78 (M-R:1) , SSI=0.42 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.96 (W) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47851-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMV1+p	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	6.0	9.0	3.00	3.75
S	BMWW-t	MT20	5.0	8.0	2.50	3.50
T	BS-t	MT20	6.0	9.0		
U	BMWWW-t	MT20	8.0	9.0	4.25	4.50
V	BMWWW-t	MT20	8.0	9.0	4.25	4.50
W	BS-t	MT20	6.0	9.0		
X	BMWW-t	MT20	5.0	8.0	2.50	3.50
Y	BMWW-t	MT20	6.0	9.0	3.00	3.75
Z	BMWW-t	MT20	5.0	6.0		
AA	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 147.1 lbs FACTORED DOWN AT 20-4-4, AND 147.1 lbs FACTORED DOWN AT 22-4-4, AND 147.1 lbs FACTORED DOWN AT 24-4-4 ON TOP CHORD, AND 2899.6 lbs FACTORED DOWN AT 19-4-8, 69.9 lbs FACTORED DOWN AT 20-4-4, 69.9 lbs FACTORED DOWN AT 22-4-4, AND 69.9 lbs FACTORED DOWN AT 24-4-4, AND 1813.0 lbs FACTORED DOWN AT 25-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
AC	22-4-4	-147	-147	—	FRONT	VERT	TOTAL
AD	20-4-4	-40	-70	—	FRONT	VERT	TOTAL
AE	22-4-4	-40	-70	—	FRONT	VERT	TOTAL
AF	25-9-8	-1813	-1813	—	FRONT	VERT	TOTAL

DWG NO. TAM47851-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288369	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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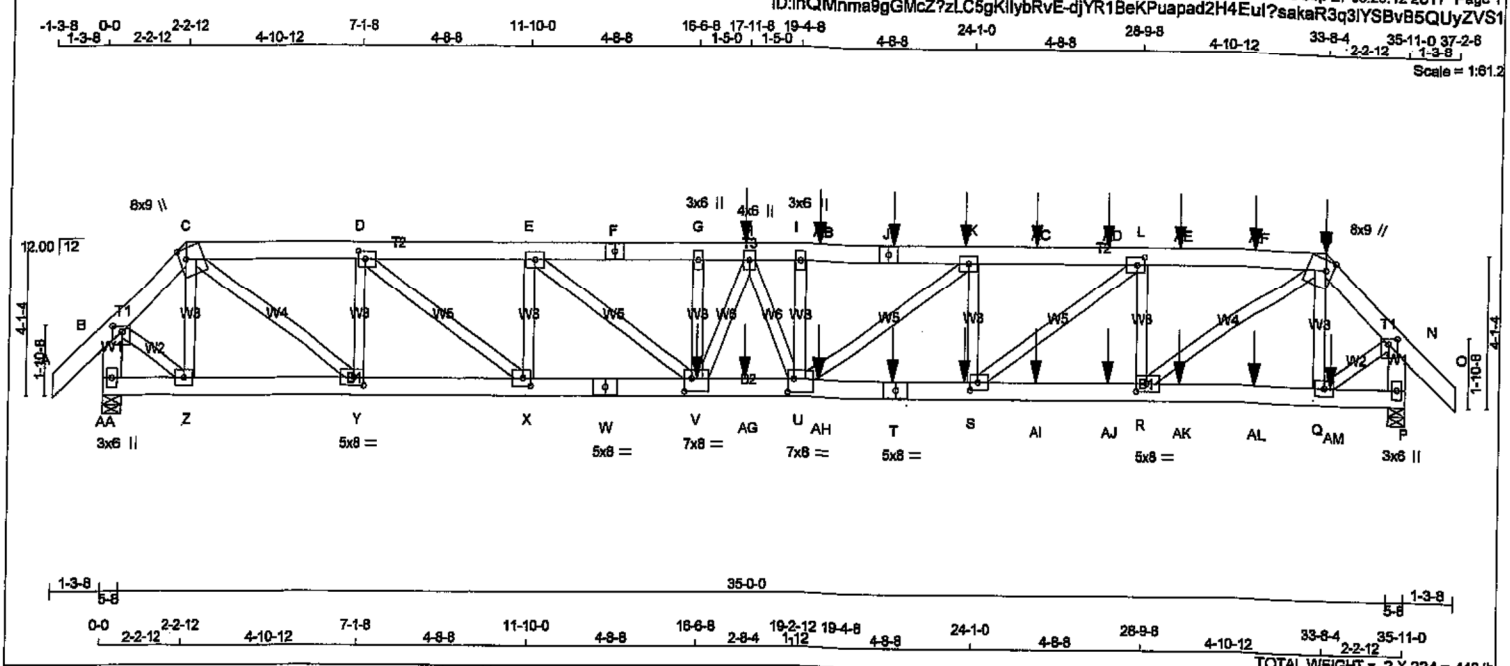

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
M - O	2x6	DRY	No.2	SPF
AA- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AA- W	2x6	DRY	1650F 1.5E	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- J	2	12	SIDE(0.0)
J- M	2	12	SIDE(183.1)
M- O	2	12	SIDE(122.0)
AA- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AA- W	2	12	TOP
W- T	2	12	SIDE(183.1)
T- P	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
G- V	1	4	SIDE(368.1)
I- U	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMW+w	MT20	3.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWW-t	MT20	5.0	6.0		
L	TMWW-t	MT20	5.0	6.0	2.50	2.50
M	TTWW+m	MT20	6.0	9.0	Edge	2.25
N	TMVW+p	MT20	5.0	6.0	1.75	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AA	4499	0	4499	0	0	5-8	5-8
P	5247	0	5247	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	3488	2323 / 0	589 / 0	0 / 0	0 / 0	576 / 0	0 / 0
P	4069	2706 / 0	689 / 0	0 / 0	0 / 0	674 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00
B- C	-3821 / 0	-122.2	-122.2	0.05 (1)	5.75
C- D	-7431 / 0	-122.2	-122.2	0.21 (1)	4.29
D- E	-11161 / 0	-122.2	-122.2	0.30 (1)	3.53
E- F	-13876 / 0	-122.2	-122.2	0.48 (1)	3.03
F- G	-13876 / 0	-122.2	-122.2	0.48 (1)	3.03
G- H	-13876 / 0	-122.2	-122.2	0.37 (1)	3.12
H- I	-13505 / 0	-122.2	-122.2	0.34 (1)	3.19
I-AB	-13505 / 0	-122.2	-122.2	0.49 (1)	3.05
AB- J	-13505 / 0	-122.2	-122.2	0.49 (1)	3.05
J- K	-13505 / 0	-122.2	-122.2	0.49 (1)	3.05
K-AC	-11613 / 0	-122.2	-122.2	0.39 (1)	3.38
AC-AD	-11613 / 0	-122.2	-122.2	0.39 (1)	3.38
AD- L	-11613 / 0	-122.2	-122.2	0.39 (1)	3.38
L-AE	-8215 / 0	-122.2	-122.2	0.29 (1)	4.02
AE-AF	-8215 / 0	-122.2	-122.2	0.29 (1)	4.02
AF- M	-8215 / 0	-122.2	-122.2	0.29 (1)	4.02
M- N	-4504 / 0	-122.2	-122.2	0.05 (1)	5.40
N- O	0 / 63	-122.2	-122.2	0.05 (1)	10.00
AA- B	-4532 / 0	0.0	0.0	0.17 (1)	6.77
P- N	-5286 / 0	0.0	0.0	0.20 (1)	6.36
AA- Z	0 / 0	-28.0	-28.0	0.04 (1)	10.00
Z- Y	0 / 2622	-28.0	-28.0	0.15 (1)	10.00
Y- X	0 / 7431	-28.0	-28.0	0.34 (1)	10.00
X- W	0 / 11161	-28.0	-28.0	0.52 (1)	10.00
W- V	0 / 11161	-28.0	-28.0	0.52 (1)	10.00
V-AG	0 / 13729	-28.0	-28.0	0.62 (1)	10.00
AG- U	0 / 13729	-28.0	-28.0	0.62 (1)	10.00
U-AH	0 / 11613	-28.0	-28.0	0.53 (1)	10.00
AH- T	0 / 11613	-28.0	-28.0	0.53 (1)	10.00
T- S	0 / 11613	-28.0	-28.0	0.53 (1)	10.00
S-AI	0 / 8216	-28.0	-28.0	0.38 (1)	10.00
AI-AJ	0 / 8216	-28.0	-28.0	0.38 (1)	10.00
AJ- R	0 / 8216	-28.0	-28.0	0.38 (1)	10.00
R-AK	0 / 3092	-28.0	-28.0	0.17 (1)	10.00
AK-AL	0 / 3092	-28.0	-28.0	0.17 (1)	10.00
AL- Q	0 / 3092	-28.0	-28.0	0.17 (1)	10.00
Q-AM	0 / 0	-28.0	-28.0	0.05 (2)	10.00
AM- P	0 / 0	-28.0	-28.0	0.05 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- C	-1273 / 0	0.11 (1)
C- Y	0 / 5968	0.53 (1)
Y- D	-3459 / 0	0.30 (1)
D- X	0 / 4717	0.42 (1)
X- E	-2707 / 0	0.23 (1)
E- V	0 / 3434	0.30 (1)
V- G	-327 / 19	0.03 (1)
U- I	-676 / 0	0.06 (1)
U- K	0 / 2393	0.21 (1)
S- K	-2388 / 0	0.20 (1)
S- L	0 / 4296	0.38 (1)
R- L	-3590 / 0	0.31 (1)
R- M	0 / 6358	0.56 (1)
Q- M	-1459 / 0	0.12 (1)
B- Z	0 / 3060	0.27 (1)
Q- N	0 / 3807	0.32 (1)
V- H	0 / 405	0.04 (1)
H- U	-621 / 0	0.06 (1)

DESIGN CRITERIA

[M]

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.38")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 750 (0.57")

CSI: TC=0.49 (I-K:1) , BC=0.62 (U-V:1) , WB=0.56 (M-R:1) , SSI=0.18 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (Y) (INPUT = 0.90)
JSI METAL= 0.95 (W) (INPUT = 1.00)

DWG NO. TAM 48110-17

STRUCTURAL COMPONENT ONLY

P6/6

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288369	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMV1+p	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	3.50
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BS-t	MT20	5.0	8.0		
U	BMWWW-t	MT20	7.0	8.0	4.25	2.00
V	BMWWW-t	MT20	7.0	8.0	4.25	2.50
W	BS-t	MT20	5.0	8.0		
X	BMWW-t	MT20	5.0	6.0	2.50	2.50
Y	BMWW-t	MT20	5.0	8.0	2.50	3.50
Z	BMWW-t	MT20	5.0	6.0		
AA	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 33-8-4, 147.1 lbs FACTORED DOWN AT 17-10-4, 147.1 lbs FACTORED DOWN AT 19-10-4, 147.1 lbs FACTORED DOWN AT 21-10-4, 147.1 lbs FACTORED DOWN AT 23-10-4, 147.1 lbs FACTORED DOWN AT 25-10-4, 147.1 lbs FACTORED DOWN AT 27-10-4, 147.1 lbs FACTORED DOWN AT 29-10-4, AND 147.1 lbs FACTORED DOWN AT 31-10-4, AND 150.2 lbs FACTORED DOWN AT 33-8-4 ON TOP CHORD, AND 2112.9 lbs FACTORED DOWN AT 16-6-8, 69.9 lbs FACTORED DOWN AT 17-10-4, 69.9 lbs FACTORED DOWN AT 19-10-4, 69.9 lbs FACTORED DOWN AT 21-10-4, 69.9 lbs FACTORED DOWN AT 23-10-4, 69.9 lbs FACTORED DOWN AT 25-10-4, 69.9 lbs FACTORED DOWN AT 27-10-4, 69.9 lbs FACTORED DOWN AT 29-10-4, AND 69.9 lbs FACTORED DOWN AT 31-10-4, AND 69.9 lbs FACTORED DOWN AT 33-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	17-10-4	-147	-147	—	BACK	VERT	TOTAL
J	21-10-4	-147	-147	—	BACK	VERT	TOTAL
K	23-10-4	-147	-147	—	BACK	VERT	TOTAL
M	33-8-4	-13	-14	—	FRONT	VERT	DEAD
M	33-8-4	-150	-150	—	BACK	VERT	TOTAL
M	33-8-4	-183	-183	—	FRONT	VERT	SNOW
S	23-10-4	-40	-70	—	BACK	VERT	TOTAL
T	21-10-4	-40	-70	—	BACK	VERT	TOTAL
V	16-6-8	-2113	-2113	—	BACK	VERT	TOTAL
AB	19-10-4	-147	-147	—	BACK	VERT	TOTAL
AC	25-10-4	-147	-147	—	BACK	VERT	TOTAL
AD	27-10-4	-147	-147	—	BACK	VERT	TOTAL
AE	29-10-4	-147	-147	—	BACK	VERT	TOTAL
AF	31-10-4	-147	-147	—	BACK	VERT	TOTAL
AG	17-10-4	-40	-70	—	BACK	VERT	TOTAL
AH	19-10-4	-40	-70	—	BACK	VERT	TOTAL
AI	25-10-4	-40	-70	—	BACK	VERT	TOTAL
AJ	27-10-4	-40	-70	—	BACK	VERT	TOTAL
AK	29-10-4	-40	-70	—	BACK	VERT	TOTAL
AL	31-10-4	-40	-70	—	BACK	VERT	TOTAL
AM	33-10-4	-40	-70	—	BACK	VERT	TOTAL

DWG NO. TAM4811a-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288371	T1Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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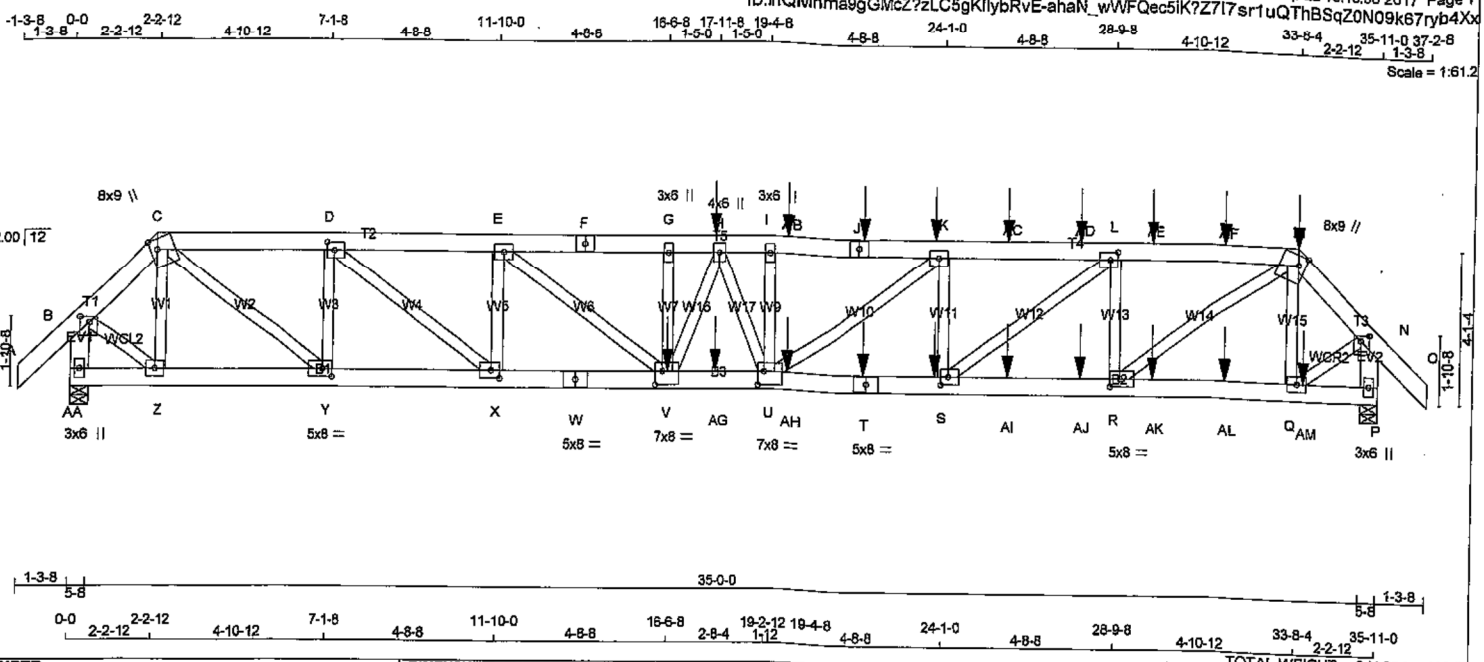

TOTAL WEIGHT = 2 X 224 = 448 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
M - O	2x6	DRY	No.2	SPF
AA- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AA- W	2x6	DRY	1650F 1.5E	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- J	2	12	SIDE(0.0)
J- M	2	12	SIDE(183.1)
M- O	2	12	SIDE(122.0)
AA- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AA- W	2	12	TOP
W- T	2	12	SIDE(183.1)
T- P	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
G- V	1	4	SIDE(424.9)
I- U	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMW+w	MT20	3.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWW-t	MT20	5.0	6.0		
L	TMWW-t	MT20	5.0	6.0	2.50	2.50
M	TTWW+m	MT20	8.0	9.0	Edge	2.25
N	TMVW+p	MT20	5.0	6.0	1.75	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED CROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
AA	4616	0	4616	0	0	5-8	5-8
P	5347	0	5347	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	3566	2397 / 0	588 / 0	0 / 0	0 / 0	581 / 0	0 / 0
P	4136	2770 / 0	688 / 0	0 / 0	0 / 0	678 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00
B- C	-3929 / 0	-122.2	-122.2	0.05 (1)	5.69
C- D	-7659 / 0	-122.2	-122.2	0.21 (1)	4.23
D- E	-11541 / 0	-122.2	-122.2	0.32 (1)	3.46
E- F	-14400 / 0	-122.2	-122.2	0.51 (1)	2.94
F- G	-14400 / 0	-122.2	-122.2	0.51 (1)	2.94
G- H	-14400 / 0	-122.2	-122.2	0.40 (1)	3.03
H- I	-13956 / 0	-122.2	-122.2	0.37 (1)	3.12
I-AB	-13956 / 0	-122.2	-122.2	0.52 (1)	2.98
AB- J	-13956 / 0	-122.2	-122.2	0.52 (1)	2.98
J- K	-13956 / 0	-122.2	-122.2	0.52 (1)	2.98
K-AC	-11936 / 0	-122.2	-122.2	0.40 (1)	3.32
AC-AD	-11936 / 0	-122.2	-122.2	0.40 (1)	3.32
AD- L	-11936 / 0	-122.2	-122.2	0.40 (1)	3.32
L-AE	-8410 / 0	-122.2	-122.2	0.30 (1)	3.98
AE-AF	-8410 / 0	-122.2	-122.2	0.30 (1)	3.98
AF- M	-8410 / 0	-122.2	-122.2	0.30 (1)	3.98
M- N	-4596 / 0	-122.2	-122.2	0.05 (1)	5.35
N- O	0 / 63	-122.2	-122.2	0.05 (1)	10.00
AA- B	-4651 / 0	0.0	0.0	0.17 (1)	6.70
P- N	-5388 / 0	0.0	0.0	0.20 (1)	6.31
AA- Z	0 / 0	-28.0	-28.0	0.04 (1)	10.00
Z- Y	0 / 2896	-28.0	-28.0	0.15 (1)	10.00
Y- X	0 / 7659	-28.0	-28.0	0.35 (1)	10.00
X- W	0 / 11541	-28.0	-28.0	0.54 (1)	10.00
W- V	0 / 11541	-28.0	-28.0	0.54 (1)	10.00
V-AG	0 / 14219	-28.0	-28.0	0.64 (1)	10.00
AG- U	0 / 14219	-28.0	-28.0	0.64 (1)	10.00
U-AH	0 / 11936	-28.0	-28.0	0.54 (1)	10.00
AH- T	0 / 11936	-28.0	-28.0	0.54 (1)	10.00
T- S	0 / 11936	-28.0	-28.0	0.54 (1)	10.00
S-AI	0 / 8410	-28.0	-28.0	0.39 (1)	10.00
AI-AJ	0 / 8410	-28.0	-28.0	0.39 (1)	10.00
AJ- R	0 / 8410	-28.0	-28.0	0.39 (1)	10.00
R-AK	0 / 3155	-28.0	-28.0	0.18 (1)	10.00
AK-AL	0 / 3155	-28.0	-28.0	0.18 (1)	10.00
AL- Q	0 / 3155	-28.0	-28.0	0.18 (1)	10.00
Q-AM	0 / 0	-28.0	-28.0	0.05 (1)	10.00
AM- P	0 / 0	-28.0	-28.0	0.05 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
Z- C	-1312 / 0	0.11 (1)
C- Y	0 / 6159	0.54 (1)
Y- D	-3574 / 0	0.31 (1)
D- X	0 / 4910	0.43 (1)
X- E	-2822 / 0	0.24 (1)
E- V	0 / 3616	0.32 (1)
V- G	-316 / 19	0.03 (1)
U- I	-674 / 0	0.06 (1)
U- K	0 / 2556	0.23 (1)
S- K	-2487 / 0	0.21 (1)
S- L	0 / 4459	0.39 (1)
R- L	-3688 / 0	0.32 (1)
R- M	0 / 6522	0.58 (1)
Q- M	-1492 / 0	0.13 (1)
B- Z	0 / 3146	0.28 (1)
Q- N	0 / 3680	0.33 (1)
V- H	0 / 500	0.04 (1)
H- U	-725 / 0	0.07 (1)

LICENSED PROFESSIONAL ENGINEER
9257
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 726 (0.59")

CSI: TC=0.52 (I-K:1) , BC=0.64 (U-V:1) , WB=0.58 (M-R:1) , SSI=0.18 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.97 (W) (INPUT = 1.00)

DWG NO. TAM 47875-17
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288371	T1Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMV1+p	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	8.0	2.50	3.25
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BS-t	MT20	5.0	8.0		
U	BMWWW-t	MT20	7.0	8.0	4.25	2.00
V	BMWWW-t	MT20	7.0	8.0	4.25	2.50
W	BS-t	MT20	5.0	8.0		
X	BMWW-t	MT20	5.0	6.0	2.50	2.50
Y	BMWW-t	MT20	5.0	8.0	2.50	3.25
Z	BMWW-t	MT20	5.0	6.0		
AA	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 33-8-4, 147.1 lbs FACTORED DOWN AT 17-10-4, 147.1 lbs FACTORED DOWN AT 19-10-4, 147.1 lbs FACTORED DOWN AT 21-10-4, 147.1 lbs FACTORED DOWN AT 23-10-4, 147.1 lbs FACTORED DOWN AT 25-10-4, 147.1 lbs FACTORED DOWN AT 27-10-4, 147.1 lbs FACTORED DOWN AT 29-10-4, AND 147.1 lbs FACTORED DOWN AT 31-10-4, AND 150.2 lbs FACTORED DOWN AT 33-8-4 ON TOP CHORD, AND 2330.4 lbs FACTORED DOWN AT 16-6-8, 69.9 lbs FACTORED DOWN AT 17-10-4, 69.9 lbs FACTORED DOWN AT 19-10-4, 69.9 lbs FACTORED DOWN AT 21-10-4, 69.9 lbs FACTORED DOWN AT 23-10-4, 69.9 lbs FACTORED DOWN AT 25-10-4, 69.9 lbs FACTORED DOWN AT 27-10-4, 69.9 lbs FACTORED DOWN AT 29-10-4, AND 69.9 lbs FACTORED DOWN AT 31-10-4, AND 69.9 lbs FACTORED DOWN AT 33-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	17-10-4	-147	-147	---	BACK	VERT	TOTAL
J	21-10-4	-147	-147	---	BACK	VERT	TOTAL
K	23-10-4	-147	-147	---	BACK	VERT	TOTAL
M	33-8-4	-13	-14	---	FRONT	VERT	DEAD
M	33-8-4	-150	-150	---	BACK	VERT	TOTAL
M	33-8-4	-183	-183	---	FRONT	VERT	SNOW
S	23-10-4	-40	-70	---	BACK	VERT	TOTAL
T	21-10-4	-40	-70	---	BACK	VERT	TOTAL
V	16-6-8	-2330	-2330	---	BACK	VERT	TOTAL
AB	19-10-4	-147	-147	---	BACK	VERT	TOTAL
AC	25-10-4	-147	-147	---	BACK	VERT	TOTAL
AD	27-10-4	-147	-147	---	BACK	VERT	TOTAL
AE	29-10-4	-147	-147	---	BACK	VERT	TOTAL
AF	31-10-4	-147	-147	---	BACK	VERT	TOTAL
AG	17-10-4	-40	-70	---	BACK	VERT	TOTAL
AH	19-10-4	-40	-70	---	BACK	VERT	TOTAL
AI	25-10-4	-40	-70	---	BACK	VERT	TOTAL
AJ	27-10-4	-40	-70	---	BACK	VERT	TOTAL
AK	29-10-4	-40	-70	---	BACK	VERT	TOTAL
AL	31-10-4	-40	-70	---	BACK	VERT	TOTAL
AM	33-10-4	-40	-70	---	BACK	VERT	TOTAL

DWG NO. TAM47875-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	T1Z3	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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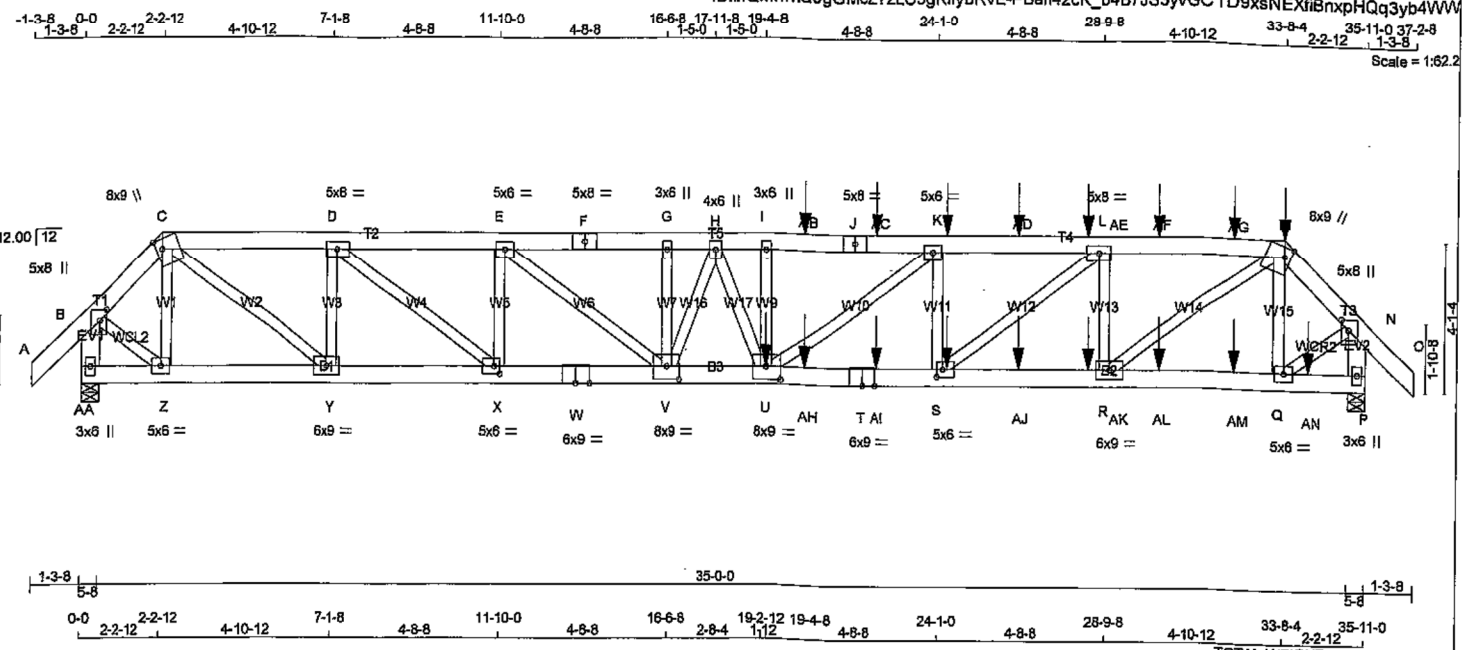


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
M - O	2x6	DRY	No.2	SPF
AA- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AA- W	2x6	DRY	1650F 1.5E	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- J	2	12	SIDE(0.0)
J- M	2	12	SIDE(183.1)
M- O	2	12	SIDE(122.0)
AA- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AA- W	2	12	TOP
W- T	2	12	SIDE(183.1)
T- P	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
I- U	1	2	SIDE(679.6)
G- V	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	3.50	2.25
C	TTWW+m	MT20	8.0	9.0	3.25	2.00
D	TMWW-t	MT20	5.0	8.0		
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	8.0		
G	TMW+w	MT20	3.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMW+w	MT20	3.0	6.0		
J	TS-t	MT20	5.0	8.0		
K	TMWW-t	MT20	5.0	8.0		
L	TMWW-t	MT20	5.0	8.0		
M	TTWW+m	MT20	8.0	9.0	3.25	2.00
N	TMVW+p	MT20	5.0	8.0	3.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AA	4762	0	4762	0	0	5-8	5-8
P	5840	0	5840	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	3679	2474 / 0	607 / 0	0 / 0	0 / 0	598 / 0	0 / 0
P	4525	3019 / 0	761 / 0	0 / 0	0 / 0	745 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

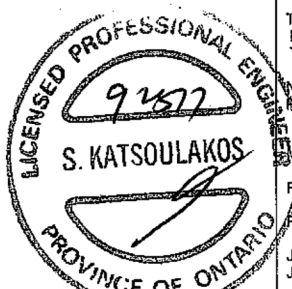
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.05 (1)		10.00
B- C	-4063 / 0	-122.2	-122.2	0.05 (1)		5.62
C- D	-7946 / 0	-122.2	-122.2	0.22 (1)		4.16
D- E	-12000 / 0	-122.2	-122.2	0.34 (1)		3.38
E- F	-15116 / 0	-122.2	-122.2	0.54 (1)		2.86
F- G	-15116 / 0	-122.2	-122.2	0.54 (1)		2.86
G- H	-15116 / 0	-122.2	-122.2	0.43 (1)		2.95
H- I	-16529 / 0	-122.2	-122.2	0.51 (1)		2.75
I-AB	-16529 / 0	-122.2	-122.2	0.68 (1)		2.58
AB- J	-16529 / 0	-122.2	-122.2	0.68 (1)		2.58
J-AC	-16529 / 0	-122.2	-122.2	0.68 (1)		2.58
AC- K	-16529 / 0	-122.2	-122.2	0.68 (1)		2.58
K-AD	-13832 / 0	-122.2	-122.2	0.48 (1)		3.03
AD-AE	-13832 / 0	-122.2	-122.2	0.48 (1)		3.03
AE- L	-13832 / 0	-122.2	-122.2	0.48 (1)		3.03
L-AF	-9502 / 0	-122.2	-122.2	0.33 (1)		3.74
AF-AG	-9502 / 0	-122.2	-122.2	0.33 (1)		3.74
AG- M	-9502 / 0	-122.2	-122.2	0.33 (1)		3.74
M- N	-5047 / 0	-122.2	-122.2	0.06 (1)		5.15
N- O	0 / 63	-122.2	-122.2	0.05 (1)		10.00
AA- B	-4799 / 0	0.0	0.0	0.18 (1)		6.61
P- N	-5885 / 0	0.0	0.0	0.22 (1)		6.08
AA- Z	0 / 0	-28.0	-28.0	0.04 (1)		10.00
Z- Y	0 / 2787	-28.0	-28.0	0.16 (1)		10.00
Y- X	0 / 7946	-28.0	-28.0	0.36 (1)		10.00
X- W	0 / 12000	-28.0	-28.0	0.54 (1)		10.00
W- V	0 / 12000	-28.0	-28.0	0.54 (1)		10.00
V- U	0 / 15842	-28.0	-28.0	0.72 (1)		10.00
U-AH	0 / 13832	-28.0	-28.0	0.65 (1)		10.00
AH- T	0 / 13832	-28.0	-28.0	0.65 (1)		10.00
T-AI	0 / 13832	-28.0	-28.0	0.65 (1)		10.00
AI- S	0 / 13832	-28.0	-28.0	0.65 (1)		10.00
S-AJ	0 / 9502	-28.0	-28.0	0.44 (1)		10.00
AJ-AK	0 / 9502	-28.0	-28.0	0.44 (1)		10.00
AK- R	0 / 9502	-28.0	-28.0	0.44 (1)		10.00
R-AL	0 / 3465	-28.0	-28.0	0.20 (1)		10.00
AL-AM	0 / 3465	-28.0	-28.0	0.20 (1)		10.00
AM- Q	0 / 3465	-28.0	-28.0	0.20 (1)		10.00
Q-AN	0 / 0	-28.0	-28.0	0.06 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- C	-1362 / 0	0.12 (1)
C- Y	0 / 6402	0.57 (1)
Y- D	-3711 / 0	0.32 (1)
D- X	0 / 5127	0.45 (1)
X- E	-2990 / 0	0.26 (1)
E- V	0 / 3941	0.35 (1)
V- G	-404 / 0	0.03 (1)
U- I	-505 / 21	0.04 (1)
U- K	0 / 3411	0.30 (1)
S- K	-3057 / 0	0.26 (1)
S- L	0 / 5477	0.48 (1)
R- L	-4286 / 0	0.37 (1)
R- M	0 / 7492	0.65 (1)
Q- M	-1642 / 0	0.14 (1)
B- Z	0 / 3253	0.29 (1)
Q- N	0 / 4041	0.36 (1)
V- H	-2004 / 0	0.19 (1)
H- U	0 / 1897	0.17 (1)



DESIGN CRITERIA

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LOADS WERE DERIVED FROM USER INPUT
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	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

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THIS DESIGN COMPLIES WITH:
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- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 987 (0.44")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 652 (0.66")

CSI: TC=0.68 (I-K:1) , BC=0.72 (U-V:1) , WB=0.66 (M-R:1) , SSI=0.17 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.93 (W) (INPUT = 1.00)

pg 1 of 2

DWG NO. TAM 47876 17

STRUCTURAL

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	T1Z3	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMV1+p	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	6.0	9.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BS-t	MT20	6.0	9.0		
U	BMWWW-t	MT20	8.0	9.0	4.25	4.50
V	BMWWW-t	MT20	8.0	9.0	4.25	4.50
W	BS-t	MT20	6.0	9.0		
X	BMWW-t	MT20	5.0	6.0	2.50	2.00
Y	BMWW-t	MT20	6.0	9.0		
Z	BMWW-t	MT20	5.0	6.0		
AA	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 33-8-4, 147.1 lbs FACTORED DOWN AT 20-4-4, 147.1 lbs FACTORED DOWN AT 22-4-4, 147.1 lbs FACTORED DOWN AT 24-4-4, 147.1 lbs FACTORED DOWN AT 26-4-4, 147.1 lbs FACTORED DOWN AT 28-4-4, AND 147.1 lbs FACTORED DOWN AT 30-4-4, AND 147.1 lbs FACTORED DOWN AT 32-4-4 ON TOP CHORD, AND 3306.9 lbs FACTORED DOWN AT 19-4-8, 69.9 lbs FACTORED DOWN AT 20-4-4, 69.9 lbs FACTORED DOWN AT 22-4-4, 69.9 lbs FACTORED DOWN AT 24-4-4, 69.9 lbs FACTORED DOWN AT 26-4-4, 69.9 lbs FACTORED DOWN AT 28-4-4, 69.9 lbs FACTORED DOWN AT 30-4-4, AND 69.9 lbs FACTORED DOWN AT 32-4-4, AND 69.9 lbs FACTORED DOWN AT 34-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AN- P	0 / 0	-28.0	-28.0	0.06 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	24-4-4	-147	-147	—	FRONT	VERT	TOTAL
M	33-8-4	-13	-14	—	FRONT	VERT	DEAD
M	33-8-4	-183	-183	—	FRONT	VERT	SNOW
S	24-4-4	-40	-70	—	FRONT	VERT	TOTAL
U	19-4-8	-3307	-3307	—	FRONT	VERT	TOTAL
AB	20-4-4	-147	-147	—	FRONT	VERT	TOTAL
AC	22-4-4	-147	-147	—	FRONT	VERT	TOTAL
AD	26-4-4	-147	-147	—	FRONT	VERT	TOTAL
AE	28-4-4	-147	-147	—	FRONT	VERT	TOTAL
AF	30-4-4	-147	-147	—	FRONT	VERT	TOTAL
AG	32-4-4	-147	-147	—	FRONT	VERT	TOTAL
AH	20-4-4	-40	-70	—	FRONT	VERT	TOTAL
AI	22-4-4	-40	-70	—	FRONT	VERT	TOTAL
AJ	26-4-4	-40	-70	—	FRONT	VERT	TOTAL
AK	28-4-4	-40	-70	—	FRONT	VERT	TOTAL
AL	30-4-4	-40	-70	—	FRONT	VERT	TOTAL
AM	32-4-4	-40	-70	—	FRONT	VERT	TOTAL
AN	34-4-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9157
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47876-17
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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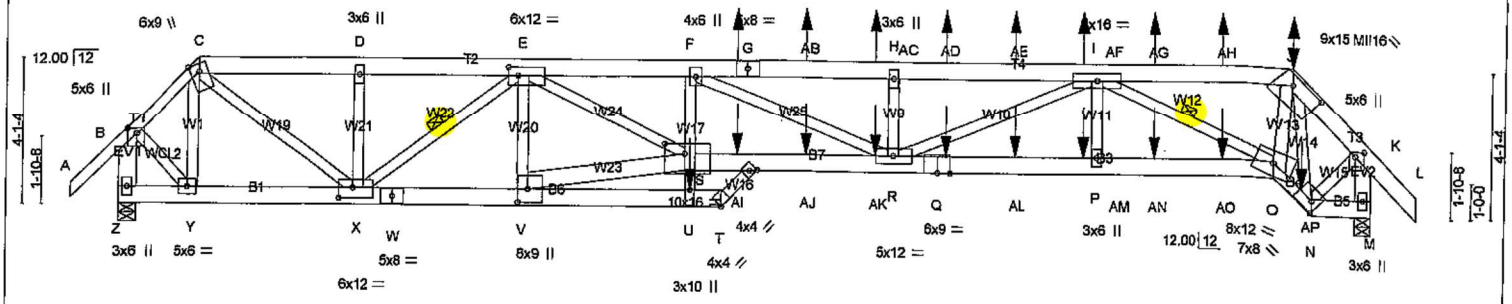
Scale: 3/16"=1'



TOTAL WEIGHT = 2 X 221 = 442 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	1650F 1.5E	SPF
G - J	2x6	DRY	1650F 1.5E	SPF
J - L	2x6	DRY	No.2	SPF
Z - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
Z - W	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF
Q - O	2x6	DRY	2100F 1.8E	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	2	12	SIDE(0.0)
G- J	2	12	SIDE(183.1)
J- L	2	12	SIDE(122.0)
Z- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- W	2	12	TOP
W- T	2	12	SIDE(183.1)
Q- S	2	12	SIDE(0.0)
Q- O	2	12	SIDE(197.8)
N- M	2	12	TOP
O- N	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
F- U	1	4	SIDE(368.1)
2x4	1	6	
N- J	1	6	SIDE(30.9)
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	6.0	12.0	2.75	3.25
F	TMWW+t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	8.0		
H	TMW+w	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	4858	0	4858	0	0	5-8	5-8
M	6319	0	6319	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD DL

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	3757	2522 / 0	626 / 0	0 / 0	0 / 0	609 / 0	0 / 0
M	4854	3321 / 0	771 / 0	0 / 0	0 / 0	762 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-O, E-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.09 (1)	10.00
B- C	-3841 / 0	-122.2	-122.2	0.11 (1)	4.75
C- D	-7905 / 0	-122.2	-122.2	0.12 (1)	4.80
D- E	-7904 / 0	-122.2	-122.2	0.11 (1)	4.80
E- F	-21400 / 0	-122.2	-122.2	0.46 (1)	2.90
F- G	-19874 / 0	-122.2	-122.2	0.49 (1)	2.98
G-AB	-19874 / 0	-122.2	-122.2	0.49 (1)	2.98
AB-AC	-19874 / 0	-122.2	-122.2	0.49 (1)	2.98
AC- H	-19874 / 0	-122.2	-122.2	0.49 (1)	2.98
H-AD	-19874 / 0	-122.2	-122.2	0.43 (1)	3.05
AD-AE	-19874 / 0	-122.2	-122.2	0.43 (1)	3.05
AE-AF	-19874 / 0	-122.2	-122.2	0.43 (1)	3.05
AF- I	-19874 / 0	-122.2	-122.2	0.43 (1)	3.05
I-AG	-5873 / 0	-122.2	-122.2	0.12 (1)	5.37
AG-AH	-5873 / 0	-122.2	-122.2	0.12 (1)	5.37
AH- J	-5873 / 0	-122.2	-122.2	0.12 (1)	5.37
J- K	-4695 / 0	-122.2	-122.2	0.06 (1)	5.30
K- L	0 / 63	-122.2	-122.2	0.05 (1)	10.00
Z- B	-4897 / 0	0.0	0.0	0.19 (1)	6.56
M- K	-6296 / 0	0.0	0.0	0.23 (1)	5.91
Z- Y	0 / 0	-28.0	-28.0	0.05 (1)	10.00
Y- X	0 / 2789	-28.0	-28.0	0.23 (1)	10.00
X- W	0 / 12312	-28.0	-28.0	0.85 (1)	10.00
W- V	0 / 12312	-28.0	-28.0	0.85 (1)	10.00
V- U	0 / 492	-28.0	-28.0	0.06 (1)	10.00
U- T	0 / 0	-28.0	-28.0	0.01 (7)	10.00
S-AI	0 / 21591	-28.0	-28.0	0.72 (1)	10.00
AI-AJ	0 / 21591	-28.0	-28.0	0.72 (1)	10.00
AJ-AK	0 / 21591	-28.0	-28.0	0.72 (1)	10.00
AK- R	0 / 21591	-28.0	-28.0	0.72 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-1668 / 0	0.14 (1)
R- H	-691 / 37	0.04 (1)
R- I	0 / 6230	0.55 (1)
P- I	0 / 1182	0.10 (1)
I- O	-9414 / 0	0.85 (1)
O- J	0 / 9336	0.83 (1)
J- N	-6788 / 0	0.56 (1)
B- Y	0 / 3310	0.29 (1)
N- K	0 / 4206	0.37 (1)
U- S	0 / 2203	0.57 (1)
S- F	-230 / 362	0.40 (1)
C- X	0 / 8495	0.57 (1)
V- E	-2339 / 0	0.20 (1)
X- D	-626 / 0	0.05 (5)
X- E	-5527 / 0	0.52 (1)
V- S	0 / 12082	0.78 (1)
E- S	0 / 10350	0.91 (1)
F- R	-1885 / 0	0.49 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 629 (0.69")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 416 (1.04")

CSI: TC=0.49 (F-H:1) , BC=0.85 (V-X:1) , WB=0.91 (E-S:1) , SSI=0.23 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.99 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4785217

STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMWWW-t	MT20	5.0	16.0		
J	TTWW-h	MI16	9.0	15.0	3.00	10.25
K	TMVW+p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BBWW-h	MT20	7.0	8.0	3.25	2.50
O	BBWW-m	MT20	8.0	12.0		
P	BMW+w	MT20	3.0	6.0		
Q	BS-t	MT20	6.0	9.0		
R	BMWWW-t	MT20	5.0	12.0		
S	BWMWW*-t	MT20	10.0	16.0	3.25	7.25
T	NP+w	MT20	4.0	4.0		
U	BMW+w	MT20	3.0	10.0		
V	BMWW+t	MT20	8.0	9.0	4.50	3.25
W	BS-t	MT20	5.0	8.0		
X	BMWWW-t	MT20	6.0	12.0	3.50	4.75
Y	BMWW-t	MT20	5.0	6.0		
Z	BMV1+p	MT20	3.0	6.0		
AA	NP+w	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 391.0 lbs FACTORED DOWN AT 33-8-4, 28.3 lbs FACTORED UP AT 17-10-4, 28.3 lbs FACTORED UP AT 19-10-4, 28.3 lbs FACTORED UP AT 21-10-4, 28.3 lbs FACTORED UP AT 23-10-4, 28.3 lbs FACTORED UP AT 25-10-4, 28.3 lbs FACTORED UP AT 27-10-4, 28.3 lbs FACTORED UP AT 29-10-4, AND 28.3 lbs FACTORED UP AT 31-10-4, AND 25.2 lbs FACTORED UP AT 33-8-4 ON TOP CHORD, AND 2112.9 lbs FACTORED DOWN AT 16-6-8, 338.4 lbs FACTORED DOWN AT 17-10-4, 338.4 lbs FACTORED DOWN AT 19-10-4, 338.4 lbs FACTORED DOWN AT 21-10-4, 338.4 lbs FACTORED DOWN AT 23-10-4, 338.4 lbs FACTORED DOWN AT 25-10-4, 338.4 lbs FACTORED DOWN AT 27-10-4, 338.4 lbs FACTORED DOWN AT 29-10-4, AND 338.4 lbs FACTORED DOWN AT 31-10-4, AND 352.4 lbs FACTORED DOWN AT 33-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
R- Q	0 / 14199	-28.0	-28.0	0.49 (1)	10.00
Q-AL	0 / 14199	-28.0	-28.0	0.49 (1)	10.00
AL-AM	0 / 14199	-28.0	-28.0	0.49 (1)	10.00
AM- P	0 / 14199	-28.0	-28.0	0.49 (1)	10.00
P-AN	0 / 14200	-28.0	-28.0	0.47 (1)	10.00
AN-AO	0 / 14200	-28.0	-28.0	0.47 (1)	10.00
AO- O	0 / 14200	-28.0	-28.0	0.47 (1)	10.00
O-AP	0 / 6097	-28.0	-28.0	0.60 (1)	10.00
AP- N	0 / 6097	-28.0	-28.0	0.60 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.01 (7)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	17-10-4	17	—	28	BACK	VERT	TOTAL
J	33-8-4	-25	-28	—	FRONT	VERT	DEAD
J	33-8-4	14	—	25	BACK	VERT	TOTAL
J	33-8-4	-366	-366	—	FRONT	VERT	SNOW
Q	23-10-4	-338	-338	—	BACK	VERT	TOTAL
U	16-6-8	-2113	-2113	—	BACK	VERT	TOTAL
AB	19-10-4	17	—	28	BACK	VERT	TOTAL
AC	21-10-4	17	—	28	BACK	VERT	TOTAL
AD	23-10-4	17	—	28	BACK	VERT	TOTAL
AE	25-10-4	17	—	28	BACK	VERT	TOTAL
AF	27-10-4	17	—	28	BACK	VERT	TOTAL
AG	29-10-4	17	—	28	BACK	VERT	TOTAL
AH	31-10-4	17	—	28	BACK	VERT	TOTAL
AI	17-10-4	-338	-338	—	BACK	VERT	TOTAL
AJ	19-10-4	-338	-338	—	BACK	VERT	TOTAL
AK	21-10-4	-338	-338	—	BACK	VERT	TOTAL
AL	25-10-4	-338	-338	—	BACK	VERT	TOTAL
AM	27-10-4	-338	-338	—	BACK	VERT	TOTAL
AN	29-10-4	-338	-338	—	BACK	VERT	TOTAL
AO	31-10-4	-338	-338	—	BACK	VERT	TOTAL
AP	33-10-4	-352	-352	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9287
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4705217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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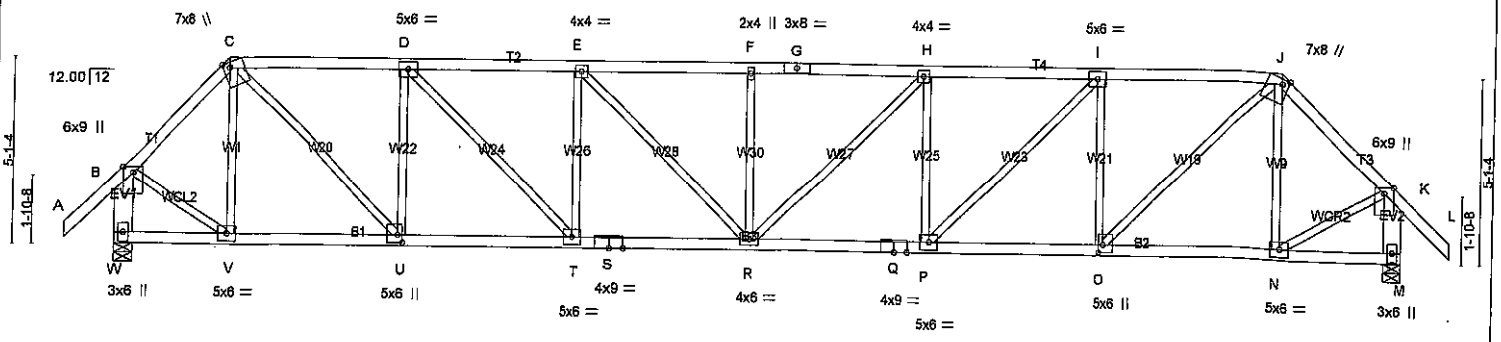


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	6.0	9.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N, P, T, V						
N	BMWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	5.0	6.0	2.50	1.50
Q	BS-t	MT20	4.0	9.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BS-t	MT20	4.0	9.0		
U	BMWW+t	MT20	5.0	6.0	2.50	1.50
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

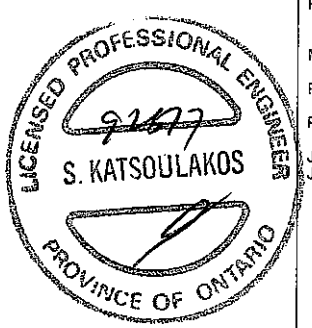
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2394 / 0	-122.2	-122.2	0.31 (1)	4.15
C- D	-3619 / 0	-122.2	-122.2	0.70 (1)	3.07
D- E	-4712 / 0	-122.2	-122.2	0.87 (1)	2.53
E- F	-5087 / 0	-122.2	-122.2	0.82 (1)	2.54
F- G	-5087 / 0	-122.2	-122.2	0.82 (1)	2.54
G- H	-5087 / 0	-122.2	-122.2	0.82 (1)	2.54
H- I	-4712 / 0	-122.2	-122.2	0.87 (1)	2.53
I- J	-3619 / 0	-122.2	-122.2	0.70 (1)	3.07
J- K	-2394 / 0	-122.2	-122.2	0.31 (1)	4.15
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00
W- B	-2839 / 0	0.0	0.0	0.21 (1)	6.23
M- K	-2839 / 0	0.0	0.0	0.21 (1)	6.23
W- V	0 / 0	-28.0	-28.0	0.12 (2)	10.00
V- U	0 / 1671	-28.0	-28.0	0.35 (1)	10.00
U- T	0 / 3619	-28.0	-28.0	0.64 (1)	10.00
T- S	0 / 4712	-28.0	-28.0	0.83 (1)	10.00
S- R	0 / 4712	-28.0	-28.0	0.83 (1)	10.00
R- Q	0 / 4712	-28.0	-28.0	0.83 (1)	10.00
Q- P	0 / 4712	-28.0	-28.0	0.83 (1)	10.00
P- O	0 / 3619	-28.0	-28.0	0.64 (1)	10.00
O- N	0 / 1671	-28.0	-28.0	0.35 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.12 (2)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
V- C	-634 / 0	0.24 (1)
N- J	-634 / 0	0.24 (1)
B- V	0 / 1853	0.42 (1)
N- K	0 / 1853	0.42 (1)
O- J	0 / 2701	0.61 (1)
C- U	0 / 2701	0.61 (1)
O- I	-1737 / 0	0.67 (1)
U- D	-1737 / 0	0.67 (1)
P- I	0 / 1536	0.35 (1)
D- T	0 / 1536	0.35 (1)
P- H	-942 / 0	0.36 (1)
T- E	-942 / 0	0.36 (1)
R- H	0 / 527	0.12 (1)
E- R	0 / 527	0.12 (1)
R- F	-606 / 0	0.23 (1)



DWG NO. TAM 47853 17
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 158 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.37")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 756 (0.57")

CSI: TC=0.87 (D-E:1) , BC=0.83 (R-T:1) , WB=0.67 (I-O:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (V) (INPUT = 0.90)
JSI METAL= 0.88 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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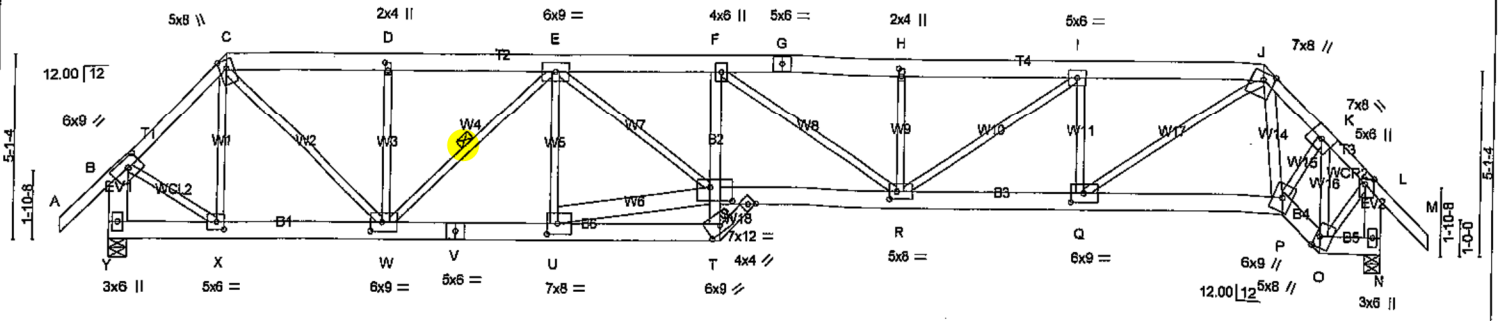

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - F	2x4	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
J - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMWWW-t	MT20	6.0	9.0		
F	TMVW+p	MT20	4.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	
K	TMWW-t	MT20	7.0	8.0		
L	TMVW+p	MT20	5.0	6.0	1.75	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	5.0	8.0	3.50	1.25
P	BBWW+m	MT20	6.0	9.0		
Q	BMWW-t	MT20	6.0	9.0	3.00	4.25
R	BMWWW-t	MT20	5.0	8.0	2.50	2.50
S	BVMWW-I	MT20	7.0	12.0	4.50	7.75
T						
T	BVW*-m	MT20	6.0	9.0	2.25	4.50
U	BMWW-t	MT20	7.0	8.0	3.50	3.50
V	BS-t	MT20	5.0	6.0		
W	BMWWW-t	MT20	6.0	9.0	3.00	4.00
X	BMWW-t	MT20	5.0	6.0	2.50	2.75
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+w	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Y	2865	0	2865	0	0	IN-SX	IN-SX
N	2871	0	2871	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2222	1478 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
N	2227	1482 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2404 / 0	-122.2	-122.2	0.31 (1)	4.14
C- D	-3611 / 0	-122.2	-122.2	0.24 (1)	4.32
D- E	-3611 / 0	-122.2	-122.2	0.23 (1)	4.35
E- F	-6642 / 0	-122.2	-122.2	0.41 (1)	3.18
F- G	-6248 / 0	-122.2	-122.2	0.43 (1)	3.26
G- H	-6248 / 0	-122.2	-122.2	0.43 (1)	3.26
H- I	-6248 / 0	-122.2	-122.2	0.41 (1)	3.28
I- J	-4784 / 0	-122.2	-122.2	0.34 (1)	3.74
J- K	-2985 / 0	-122.2	-122.2	0.18 (1)	3.90
K- L	-1983 / 0	-122.2	-122.2	0.21 (1)	4.59
L- M	0 / 60	-122.2	-122.2	0.17 (1)	10.00
Y- B	-2849 / 0	0.0	0.0	0.20 (1)	6.21
N- L	-2848 / 0	0.0	0.0	0.20 (1)	6.21
Y- X	0 / 0	-28.0	-28.0	0.07 (2)	10.00
X- W	0 / 1717	-28.0	-28.0	0.26 (1)	10.00
W- V	0 / 4813	-28.0	-28.0	0.62 (1)	10.00
V- U	0 / 4813	-28.0	-28.0	0.62 (1)	10.00
U- T	0 / 123	-28.0	-28.0	0.08 (2)	10.00
T- S	0 / 91	0.0	0.0	0.18 (1)	10.00
S- F	-253 / 74	0.0	0.0	0.18 (1)	7.81
S- R	0 / 6677	-28.0	-28.0	0.67 (1)	10.00
R- Q	0 / 4794	-28.0	-28.0	0.64 (1)	10.00
Q- P	0 / 2153	-28.0	-28.0	0.32 (1)	10.00
P- O	0 / 1794	-28.0	-28.0	0.21 (1)	10.00
O- N	0 / 0	-28.0	-28.0	0.01 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-619 / 0	0.21 (1)
C- W	0 / 2673	0.60 (1)
W- D	-630 / 0	0.21 (1)
W- E	-1676 / 0	0.49 (1)
U- E	-878 / 0	0.29 (1)
U- S	0 / 4804	0.57 (1)
E- S	0 / 2333	0.53 (1)
F- R	-525 / 0	0.35 (1)
R- H	-635 / 0	0.14 (1)
R- I	0 / 1784	0.40 (1)
Q- I	-1728 / 0	0.39 (1)
J- P	-152 / 56	0.03 (1)
P- K	0 / 1751	0.39 (1)
O- K	-2362 / 0	0.45 (1)
B- X	0 / 1875	0.42 (1)
O- L	0 / 1762	0.40 (1)
Q- J	0 / 3245	0.73 (1)

TOTAL WEIGHT = 209 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 723 (0.60")

CSI: TC=0.43 (F-H:1) , BC=0.87 (R-S:1) , WB=0.73 (J-Q:1) , SSI=0.22 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (W) (INPUT = 0.90)
JSI METAL= 0.88 (U) (INPUT = 1.00)

DWG NO. TAM47054 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T3	1	1	TRUSS DESC.	

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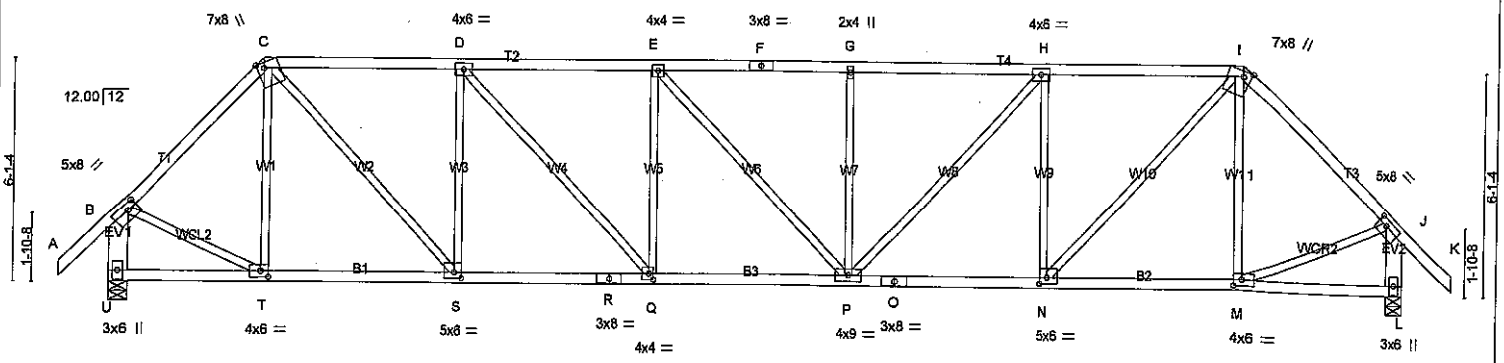

TOTAL WEIGHT = 161 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW-t	MT20	5.0	8.0	2.00	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMWW-t	MT20	5.0	6.0	2.00	2.50
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BS-t	MT20	3.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.00	2.50
T	BMWW-t	MT20	4.0	6.0	2.00	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2868	0	2868	0	0	5-8	5-8
L	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
L	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B-C	-2502 / 0	-122.2	-122.2	0.56 (1)	3.83
C-D	-3377 / 0	-122.2	-122.2	0.84 (1)	2.99
D-E	-4127 / 0	-122.2	-122.2	0.96 (1)	2.58
E-F	-4126 / 0	-122.2	-122.2	0.78 (1)	2.87
F-G	-4126 / 0	-122.2	-122.2	0.78 (1)	2.87
G-H	-4126 / 0	-122.2	-122.2	0.96 (1)	2.58
H-I	-3378 / 0	-122.2	-122.2	0.84 (1)	2.98
I-J	-2502 / 0	-122.2	-122.2	0.56 (1)	3.83
J-K	0 / 60	-122.2	-122.2	0.17 (1)	10.00
U-B	-2824 / 0	0.0	0.0	0.21 (1)	6.23
L-J	-2824 / 0	0.0	0.0	0.21 (1)	6.23
U-T	0 / 0	-28.0	-28.0	0.16 (2)	10.00
T-S	0 / 1754	-28.0	-28.0	0.38 (1)	10.00
S-R	0 / 3378	-28.0	-28.0	0.62 (1)	10.00
R-Q	0 / 3378	-28.0	-28.0	0.62 (1)	10.00
Q-P	0 / 4127	-28.0	-28.0	0.74 (1)	10.00
P-O	0 / 3378	-28.0	-28.0	0.62 (1)	10.00
O-N	0 / 3378	-28.0	-28.0	0.62 (1)	10.00
N-M	0 / 1754	-28.0	-28.0	0.38 (1)	10.00
M-L	0 / 0	-28.0	-28.0	0.16 (2)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
T-C	-464 / 29	0.27 (1)
C-S	0 / 2343	0.53 (1)
S-D	-1540 / 0	0.90 (1)
D-Q	0 / 1096	0.25 (1)
Q-E	-646 / 0	0.38 (1)
E-P	-3 / 0	0.00 (1)
P-G	-645 / 0	0.38 (1)
P-H	0 / 1093	0.25 (1)
N-H	-1538 / 0	0.90 (1)
N-I	0 / 2344	0.53 (1)
M-I	-464 / 29	0.27 (1)
B-T	0 / 1668	0.42 (1)
M-J	0 / 1668	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 982 (0.44")

CSI: TC=0.96 (G-H:1) , BC=0.74 (P-Q:1) , WB=0.90 (D-S:1) , SSI=0.32 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (T) (INPUT = 0.90)
SI METAL= 0.88 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
92627
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47855-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T3S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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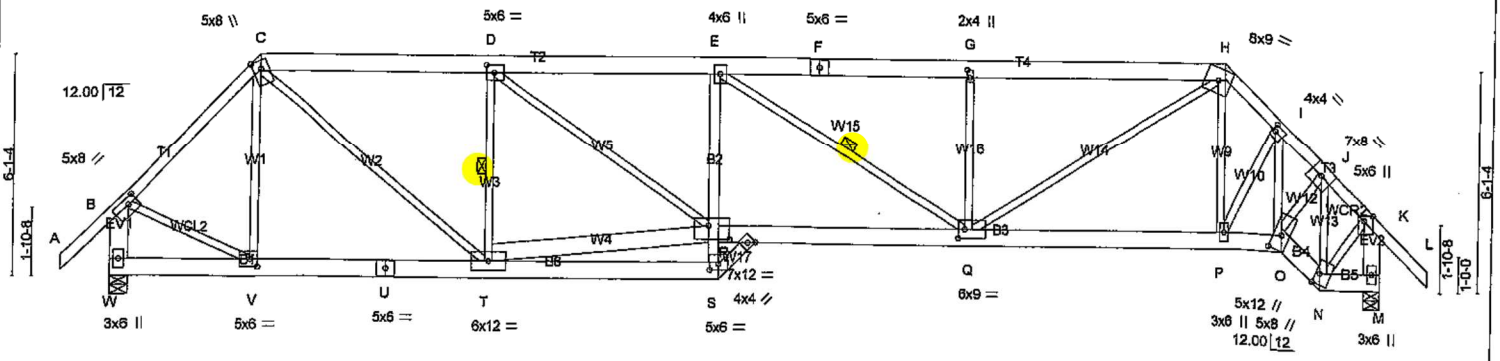


TOTAL WEIGHT = 212 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - E	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	8.0	2.50	2.75
E	TMVW+p	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TTWW-m	MT20	8.0	9.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMWW-t	MT20	7.0	8.0		
K	TMVW+p	MT20	5.0	6.0	1.75	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BBWW+m	MT20	5.0	8.0	3.50	1.25
O	BBWW+m	MT20	5.0	12.0	5.00	2.50
P	BMWW+t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	6.0	9.0	3.00	2.25
R	BVMWW-I	MT20	7.0	12.0	4.50	7.00
S						
S	BVW*-I	MT20	5.0	6.0	2.00	3.00
T	BMWWW-t	MT20	6.0	12.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1+p	MT20	3.0	6.0		
X	NP+w	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2865	0	2865	0	0	5-8	5-8
M	2871	0	2871	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2222	1478 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
M	2227	1462 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

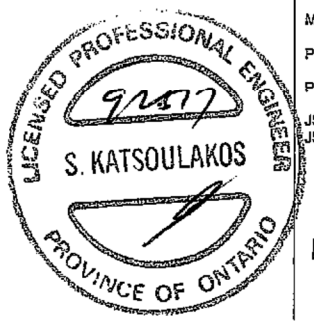
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-T, E-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2518 / 0	-122.2	-122.2	0.56 (1)	3.81
C- D	-3888 / 0	-122.2	-122.2	0.45 (1)	4.05
D- E	-5256 / 0	-122.2	-122.2	0.55 (1)	3.38
E- F	-4656 / 0	-122.2	-122.2	0.61 (1)	3.49
F- G	-4656 / 0	-122.2	-122.2	0.61 (1)	3.49
G- H	-4656 / 0	-122.2	-122.2	0.61 (1)	3.49
H- I	-3062 / 0	-122.2	-122.2	0.14 (1)	3.90
I- J	-2996 / 0	-122.2	-122.2	0.16 (1)	3.91
J- K	-1983 / 0	-122.2	-122.2	0.21 (1)	4.59
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00
W- B	-2829 / 0	0.0	0.0	0.20 (1)	6.23
M- K	-2848 / 0	0.0	0.0	0.20 (1)	6.21
W- V	0 / 0	-28.0	-28.0	0.09 (2)	10.00
V- U	0 / 1791	-28.0	-28.0	0.27 (1)	10.00
U- T	0 / 1791	-28.0	-28.0	0.27 (1)	10.00
T- S	0 / 103	-28.0	-28.0	0.15 (3)	10.00
S- R	0 / 129	0.0	0.0	0.15 (1)	10.00
R- E	-365 / 86	0.0	0.0	0.17 (1)	7.81
R- Q	0 / 5279	-28.0	-28.0	0.73 (1)	10.00
Q- P	0 / 2177	-28.0	-28.0	0.37 (1)	10.00
P- O	0 / 2144	-28.0	-28.0	0.36 (1)	10.00
O- N	0 / 1791	-28.0	-28.0	0.21 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.01 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-449 / 42	0.23 (1)
C- T	0 / 2513	0.57 (1)
T- D	-2012 / 0	0.46 (1)
T- R	0 / 3658	0.43 (1)
D- R	0 / 1895	0.43 (1)
P- H	0 / 219	0.05 (3)
P- I	0 / 79	0.02 (2)
O- I	-277 / 0	0.08 (1)
O- J	0 / 1771	0.40 (1)
N- J	-2357 / 0	0.45 (1)
B- V	0 / 1886	0.42 (1)
N- K	0 / 1758	0.40 (1)
Q- H	0 / 2948	0.66 (1)
E- Q	-742 / 0	0.36 (1)
Q- G	-1011 / 0	0.34 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 916 (0.47")

CSI: TC=0.61 (G-H:1) , BC=0.73 (Q-R:1) , WB=0.66 (H-Q:1) , SSI=0.32 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.68 (H) (INPUT = 1.00)

DWG NO. TAM47856-17
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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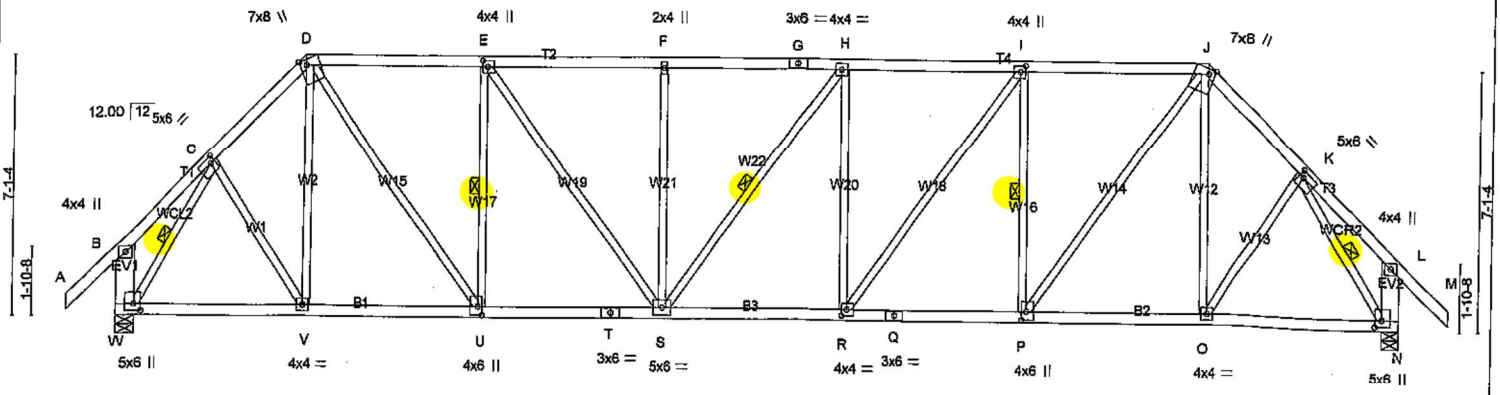

TOTAL WEIGHT = 2 X 177 = 354 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW+t	MT20	4.0	4.0	2.00	1.75
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	4.0	4.0	2.00	1.75
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMWW-t	MT20	5.0	6.0	2.00	1.50
L	TMV+p	MT20	4.0	4.0		
N	BMVW1+p	MT20	5.0	6.0	2.25	2.25
O	BMWW-t	MT20	4.0	4.0		
P	BMWW+t	MT20	4.0	6.0	2.75	1.50
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.00	1.75
S	BMWWW-t	MT20	5.0	6.0		
T	BS-t	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	6.0	2.75	1.50
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1+p	MT20	5.0	6.0	2.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2868	0	2868	0	0	5-8	5-8
N	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
N	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-W, K-N, I-P, E-U, H-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B- C	-2 / 19	-122.2	-122.2	0.10 (1)		10.00
C- D	-2567 / 0	-122.2	-122.2	0.19 (1)		4.17
D- E	-2977 / 0	-122.2	-122.2	0.66 (1)		3.38
E- F	-3525 / 0	-122.2	-122.2	0.73 (1)		3.05
F- G	-3525 / 0	-122.2	-122.2	0.59 (1)		3.27
G- H	-3525 / 0	-122.2	-122.2	0.59 (1)		3.27
H- I	-3527 / 0	-122.2	-122.2	0.73 (1)		3.06
I- J	-2977 / 0	-122.2	-122.2	0.66 (1)		3.38
J- K	-2568 / 0	-122.2	-122.2	0.19 (1)		4.17
K- L	-2 / 19	-122.2	-122.2	0.10 (1)		10.00
L- M	0 / 60	-122.2	-122.2	0.17 (1)		10.00
W- B	-300 / 0	0.0	0.0	0.02 (1)		7.81
N- L	-300 / 0	0.0	0.0	0.02 (1)		7.81
W- V	0 / 1574	-28.0	-28.0	0.36 (1)		10.00
V- U	0 / 1790	-28.0	-28.0	0.39 (1)		10.00
U- T	0 / 2977	-28.0	-28.0	0.55 (1)		10.00
T- S	0 / 2977	-28.0	-28.0	0.55 (1)		10.00
S- R	0 / 3527	-28.0	-28.0	0.63 (1)		10.00
R- Q	0 / 2977	-28.0	-28.0	0.54 (1)		10.00
Q- P	0 / 2977	-28.0	-28.0	0.54 (1)		10.00
P- O	0 / 1790	-28.0	-28.0	0.39 (1)		10.00
O- N	0 / 1574	-28.0	-28.0	0.36 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 389	0.09 (1)
V- D	-157 / 104	0.14 (1)
O- J	-156 / 104	0.14 (1)
O- K	0 / 389	0.09 (1)
W- C	-2961 / 0	0.58 (1)
K- N	-2961 / 0	0.58 (1)
P- J	0 / 1962	0.44 (1)
D- U	0 / 1963	0.44 (1)
P- I	-1427 / 0	0.46 (1)
U- E	-1426 / 0	0.45 (1)
R- I	0 / 923	0.21 (1)
E- S	0 / 919	0.21 (1)
R- H	-599 / 0	0.52 (1)
S- F	-597 / 0	0.52 (1)
S- H	-3 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.73 (E-F:1) , BC=0.63 (R-S:1) , WB=0.58 (K-N:1) , SSI=0.29 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.80 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47857-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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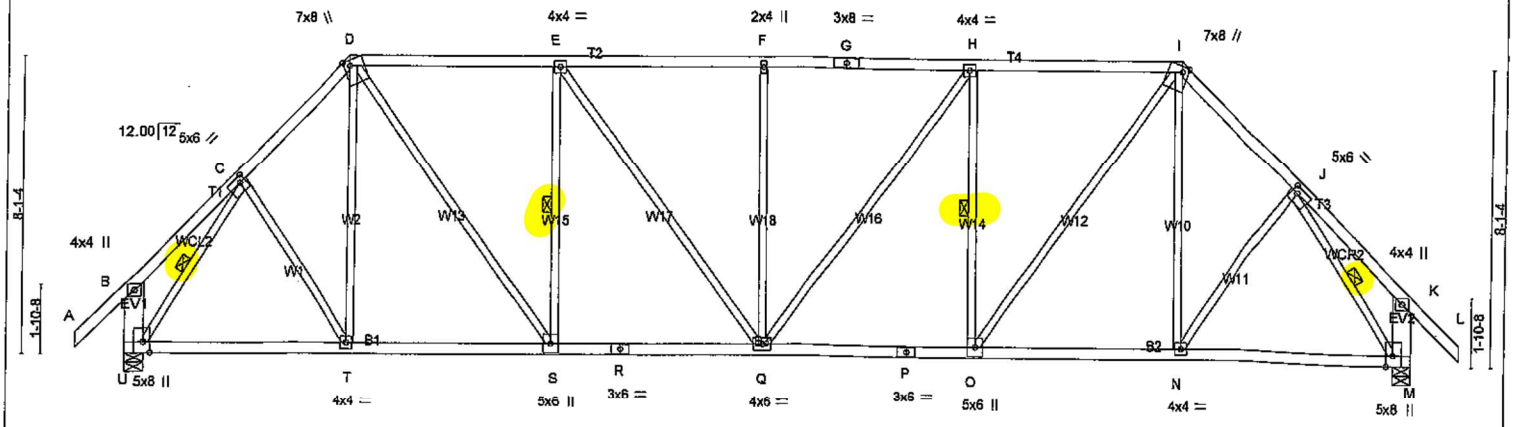
Scale = 1:62.2



TOTAL WEIGHT = 2 X 177 = 353 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.00	1.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	5.0	8.0	Edge	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	5.0	8.0	Edge	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-U, J-M, H-O, E-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	C- T	0 / 250	0.08 (1)
B- C	0 / 27	-122.2	-122.2	0.16 (1)	10.00	T- D	-9 / 188	0.04 (3)
C- D	-2584 / 0	-122.2	-122.2	0.26 (1)	4.08	N- I	-9 / 188	0.04 (3)
D- E	-2840 / 0	-122.2	-122.2	0.89 (1)	3.10	N- J	0 / 250	0.06 (1)
E- F	-3144 / 0	-122.2	-122.2	0.93 (1)	2.91	U- C	-2979 / 0	0.67 (1)
F- G	-3144 / 0	-122.2	-122.2	0.93 (1)	2.91	J- M	-2979 / 0	0.67 (1)
G- H	-3144 / 0	-122.2	-122.2	0.93 (1)	2.91	O- I	0 / 1720	0.39 (1)
H- I	-2840 / 0	-122.2	-122.2	0.89 (1)	3.10	D- S	0 / 1720	0.39 (1)
I- J	-2584 / 0	-122.2	-122.2	0.26 (1)	4.08	O- H	-1213 / 0	0.50 (1)
J- K	0 / 27	-122.2	-122.2	0.16 (1)	10.00	S- E	-1213 / 0	0.50 (1)
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00	Q- H	0 / 508	0.11 (1)
U- B	-320 / 0	0.0	0.0	0.02 (1)	7.81	E- Q	0 / 508	0.11 (1)
M- K	-320 / 0	0.0	0.0	0.02 (1)	7.81	Q- F	-654 / 0	0.83 (1)
U- T	0 / 1655	-28.0	-28.0	0.43 (2)	10.00			
T- S	0 / 1800	-28.0	-28.0	0.45 (2)	10.00			
S- R	0 / 2840	-28.0	-28.0	0.55 (1)	10.00			
R- Q	0 / 2840	-28.0	-28.0	0.55 (1)	10.00			
Q- P	0 / 2840	-28.0	-28.0	0.55 (1)	10.00			
P- O	0 / 2840	-28.0	-28.0	0.55 (1)	10.00			
O- N	0 / 1800	-28.0	-28.0	0.45 (2)	10.00			
N- M	0 / 1655	-28.0	-28.0	0.43 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.93 (E-F:1) , BC=0.55 (Q-S:1) , WB=0.83 (F-Q:1) , SSI=0.34 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.79 (R) (INPUT = 1.00)

DWG NO. TAM 47858 17
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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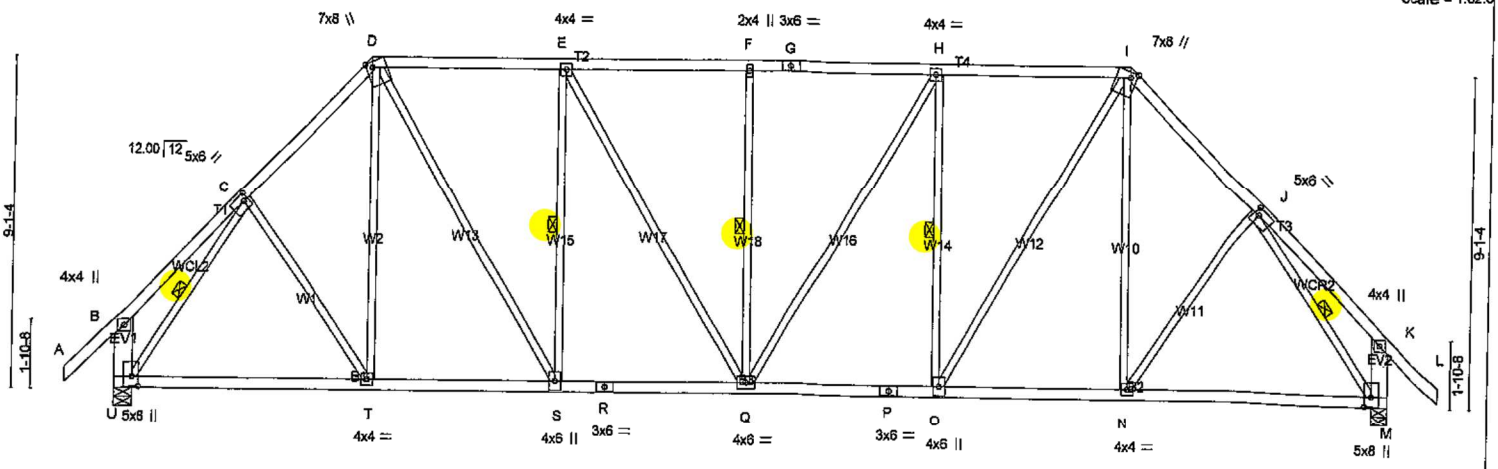

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMV+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.50
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	5.0	8.0	3.25	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	5.0	8.0	3.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-U, J-M, H-O, E-S, F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 34	-122.2	-122.2	0.24 (1)	10.00
C- D	-2573 / 0	-122.2	-122.2	0.36 (1)	3.99
D- E	-2555 / 0	-122.2	-122.2	0.69 (1)	3.54
E- F	-2781 / 0	-122.2	-122.2	0.71 (1)	3.38
F- G	-2781 / 0	-122.2	-122.2	0.71 (1)	3.38
G- H	-2781 / 0	-122.2	-122.2	0.71 (1)	3.38
H- I	-2555 / 0	-122.2	-122.2	0.69 (1)	3.54
I- J	-2573 / 0	-122.2	-122.2	0.36 (1)	3.99
J- K	0 / 34	-122.2	-122.2	0.24 (1)	10.00
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00
U- B	-341 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-341 / 0	0.0	0.0	0.03 (1)	7.81
U- T	0 / 1712	-28.0	-28.0	0.50 (2)	10.00
T- S	0 / 1790	-28.0	-28.0	0.51 (2)	10.00
S- R	0 / 2555	-28.0	-28.0	0.48 (1)	10.00
R- Q	0 / 2555	-28.0	-28.0	0.48 (1)	10.00
Q- P	0 / 2555	-28.0	-28.0	0.48 (1)	10.00
P- O	0 / 2555	-28.0	-28.0	0.48 (1)	10.00
O- N	0 / 1790	-28.0	-28.0	0.51 (2)	10.00
N- M	0 / 1712	-28.0	-28.0	0.50 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 171	0.04 (2)
T- D	0 / 246	0.06 (3)
N- I	0 / 246	0.06 (3)
N- J	0 / 171	0.04 (2)
U- C	-2983 / 0	0.79 (1)
J- M	-2983 / 0	0.79 (1)
O- I	0 / 1457	0.33 (1)
D- S	0 / 1457	0.33 (1)
O- H	-1110 / 0	0.60 (1)
S- E	-1110 / 0	0.60 (1)
Q- H	0 / 437	0.10 (1)
E- Q	0 / 437	0.10 (1)
Q- F	-597 / 0	0.32 (1)

TOTAL WEIGHT = 2 X 186 = 371 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.71 (E-F:1) , BC=0.51 (N-O:2) , WB=0.79 (J-M:1) , SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.73 (J) (INPUT = 1.00)

DWG NO. TAM47859-17
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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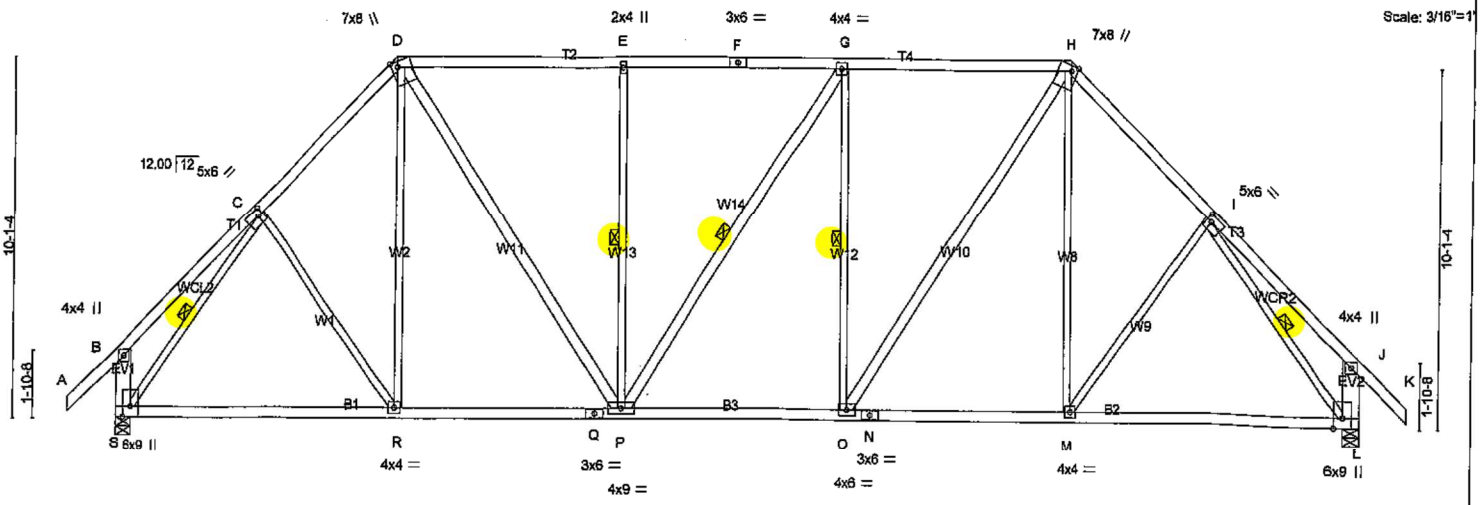

TOTAL WEIGHT = 2 X 192 = 384 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0	2.25	1.75
J	TMV+p	MT20	4.0	4.0		
L	BMVW1+p	MT20	6.0	9.0	Edge	
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2868	0	2868	0	0	5-8	5-8
L	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
L	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L, G-O, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B- C	0 / 40	-122.2	-122.2	0.32 (1)		10.00
C- D	-2546 / 0	-122.2	-122.2	0.49 (1)		3.89
D- E	-2447 / 0	-122.2	-122.2	0.97 (1)		3.15
E- F	-2447 / 0	-122.2	-122.2	0.97 (1)		3.11
F- G	-2447 / 0	-122.2	-122.2	0.97 (1)		3.11
G- H	-2449 / 0	-122.2	-122.2	0.98 (1)		3.11
H- I	-2546 / 0	-122.2	-122.2	0.49 (1)		3.89
I- J	0 / 40	-122.2	-122.2	0.32 (1)		10.00
J- K	0 / 60	-122.2	-122.2	0.17 (1)		10.00
S- B	-362 / 0	0.0	0.0	0.03 (1)		7.81
L- J	-362 / 0	0.0	0.0	0.03 (1)		7.81
S- R	0 / 1754	-28.0	-28.0	0.60 (2)		10.00
R- Q	0 / 1768	-28.0	-28.0	0.61 (2)		10.00
Q- P	0 / 1768	-28.0	-28.0	0.61 (2)		10.00
P- O	0 / 2449	-28.0	-28.0	0.50 (1)		10.00
O- N	0 / 1768	-28.0	-28.0	0.61 (2)		10.00
N- M	0 / 1768	-28.0	-28.0	0.61 (2)		10.00
M- L	0 / 1754	-28.0	-28.0	0.60 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- R	0 / 134	0.03 (3)
R- D	0 / 335	0.08 (2)
M- H	0 / 334	0.08 (2)
M- I	0 / 134	0.03 (3)
S- C	-2981 / 0	0.94 (1)
I- L	-2981 / 0	0.94 (1)
O- H	0 / 1228	0.20 (1)
D- P	0 / 1224	0.20 (1)
O- G	-855 / 0	0.60 (1)
P- E	-854 / 0	0.60 (1)
P- G	-3 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.98 (G-H:1) , BC=0.61 (P-R:2) , WB=0.94 (C-S:1) , SSI=0.37 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.74 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9107
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 42860 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T8	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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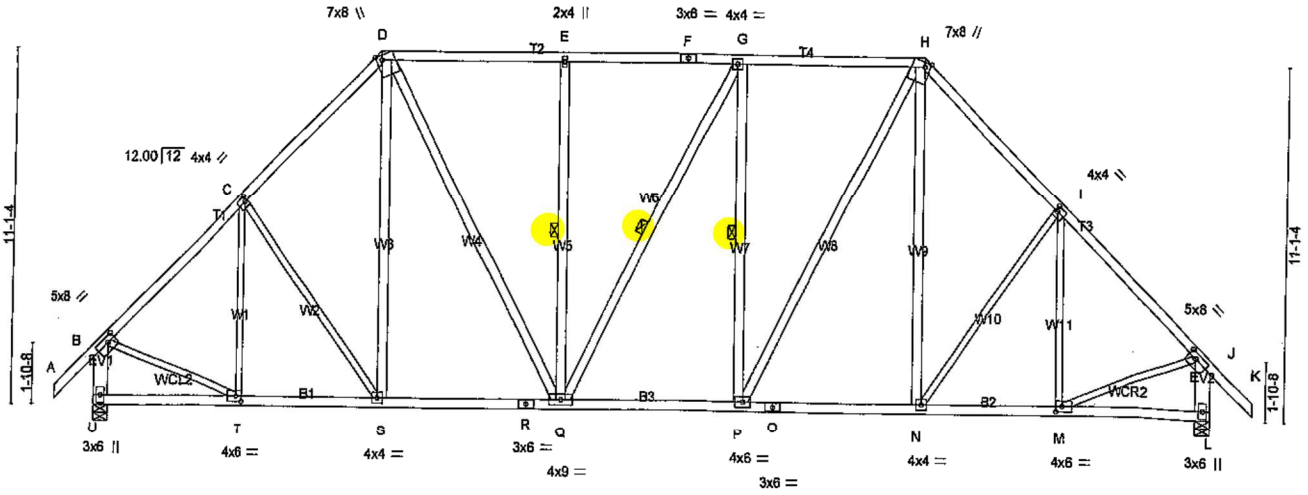

TOTAL WEIGHT = 3 X 219 = 657 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - C	2x3	DRY	No.2	SPF
C - S	2x3	DRY	No.2	SPF
N - I	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
B - T	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMVW-t	MT20	5.0	8.0	2.00	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	4.0	6.0	2.00	2.25
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2868	0	2868	0	0	5-8	5-8
L	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
L	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, G-Q, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	-2541 / 0	-122.2	-122.2	0.47 (1)	3.92	
C- D	-2468 / 0	-122.2	-122.2	0.44 (1)	3.97	
D- E	-2226 / 0	-122.2	-122.2	0.56 (1)	3.96	
E- F	-2227 / 0	-122.2	-122.2	0.51 (1)	4.03	
F- G	-2227 / 0	-122.2	-122.2	0.51 (1)	4.03	
G- H	-2228 / 0	-122.2	-122.2	0.56 (1)	3.96	
H- I	-2488 / 0	-122.2	-122.2	0.44 (1)	3.97	
I- J	-2541 / 0	-122.2	-122.2	0.47 (1)	3.92	
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
U- B	-2811 / 0	0.0	0.0	0.21 (1)	6.24	
L- J	-2811 / 0	0.0	0.0	0.21 (1)	6.24	
U- T	0 / 0	-28.0	-28.0	0.14 (3)	10.00	
T- S	0 / 1830	-28.0	-28.0	0.38 (1)	10.00	
S- R	0 / 1723	-28.0	-28.0	0.37 (1)	10.00	
R- Q	0 / 1723	-28.0	-28.0	0.37 (1)	10.00	
Q- P	0 / 2228	-28.0	-28.0	0.46 (1)	10.00	
P- O	0 / 1722	-28.0	-28.0	0.38 (1)	10.00	
O- N	0 / 1722	-28.0	-28.0	0.38 (1)	10.00	
N- M	0 / 1830	-28.0	-28.0	0.38 (1)	10.00	
M- L	0 / 0	-28.0	-28.0	0.14 (3)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
T- C	-449 / 30	0.32 (1)
C- S	-188 / 0	0.23 (1)
S- D	0 / 336	0.05 (2)
D- Q	0 / 1054	0.17 (1)
Q- E	-764 / 0	0.49 (1)
Q- G	-3 / 0	0.00 (1)
P- G	-765 / 0	0.49 (1)
P- H	0 / 1057	0.17 (1)
N- H	0 / 335	0.05 (2)
N- I	-188 / 0	0.23 (1)
M- I	-448 / 30	0.32 (1)
B- T	0 / 1921	0.43 (1)
M- J	0 / 1921	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.56 (G-H:1) , BC=0.46 (P-Q:1) , WB=0.49 (G-P:1) , SSI=0.33 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.54 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO . TAM 47864-17
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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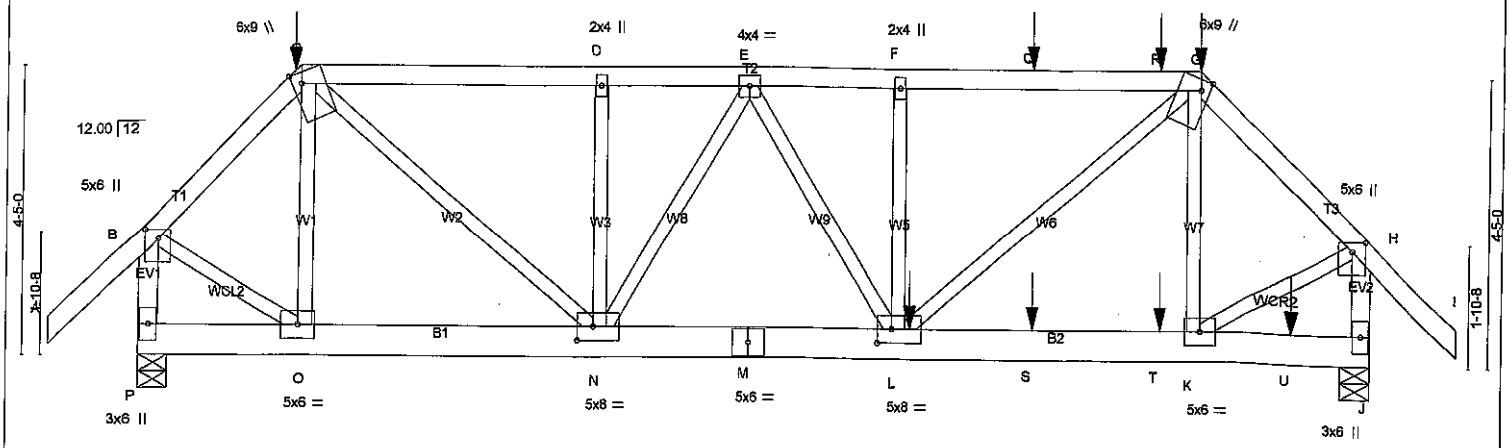

TOTAL WEIGHT = 99 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	1.75	2.50
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0	2.50	2.50
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0	2.50	3.00
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 222.3 lbs FACTORED DOWN AT 2-6-8, 30.8 lbs FACTORED DOWN AT 14-0-12, AND 52.6 lbs FACTORED DOWN AT 16-0-12, AND 208.5 lbs FACTORED DOWN AT 16-8-8 ON TOP CHORD, AND 647.7 lbs FACTORED DOWN AT 12-1-8, 23.3 lbs FACTORED DOWN AT 14-0-12, AND 23.3 lbs FACTORED DOWN AT 16-0-12, AND 23.3 lbs FACTORED DOWN AT 18-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2106	0	2106	0	0	5-8	5-8
J	2328	0	2328	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1594	1131 / 0	226 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	1767	1249 / 0	258 / 0	0 / 0	0 / 0	260 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.19 (1)	10.00	
B- C	-1615 / 0	-122.2	-122.2	0.19 (1)	4.99	
C- D	-2299 / 0	-122.2	-122.2	0.48 (1)	3.96	
D- E	-2299 / 0	-122.2	-122.2	0.43 (1)	3.96	
E- F	-2603 / 0	-122.2	-122.2	0.48 (1)	3.67	
F- Q	-2603 / 0	-122.2	-122.2	0.57 (1)	3.62	
Q- R	-2603 / 0	-122.2	-122.2	0.57 (1)	3.62	
R- G	-2603 / 0	-122.2	-122.2	0.57 (1)	3.62	
G- H	-1821 / 0	-122.2	-122.2	0.20 (1)	4.74	
H- I	0 / 60	-122.2	-122.2	0.19 (1)	10.00	
P- B	-2092 / 0	0.0	0.0	0.25 (1)	5.80	
J- H	-2317 / 0	0.0	0.0	0.27 (1)	5.55	
P- O	0 / 0	-28.0	-28.0	0.06 (2)	10.00	
O- N	0 / 1123	-28.0	-28.0	0.19 (1)	10.00	
N- M	0 / 2463	-28.0	-28.0	0.40 (1)	10.00	
M- L	0 / 2463	-28.0	-28.0	0.40 (1)	10.00	
L- S	0 / 1267	-28.0	-28.0	0.23 (1)	10.00	
S- T	0 / 1267	-28.0	-28.0	0.23 (1)	10.00	
T- K	0 / 1267	-28.0	-28.0	0.23 (1)	10.00	
K- U	0 / 0	-28.0	-28.0	0.06 (2)	10.00	
U- J	0 / 0	-28.0	-28.0	0.06 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-477 / 5	0.14 (1)
C- N	0 / 1539	0.38 (1)
N- D	-585 / 0	0.17 (1)
L- F	-621 / 0	0.18 (1)
L- G	0 / 1749	0.43 (1)
K- G	-509 / 34	0.14 (1)
B- O	0 / 1292	0.32 (1)
K- H	0 / 1457	0.36 (1)
N- E	-328 / 0	0.12 (1)
E- L	0 / 282	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-6-8	-14	-15	---	FRONT	VERT	DEAD
C	2-6-8	-209	-209	---	FRONT	VERT	SNOW
G	16-8-8	-209	-209	---	FRONT	VERT	SNOW
L	12-1-8	-648	-648	---	FRONT	VERT	TOTAL
Q	14-0-12	-31	-31	---	FRONT	VERT	TOTAL
R	16-0-12	-53	-53	---	FRONT	VERT	TOTAL
S	14-0-12	-13	-23	---	FRONT	VERT	TOTAL
T	16-0-12	-13	-23	---	FRONT	VERT	TOTAL
U	18-0-12	-13	-23	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9487
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4796417
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.57 (F-G:1) , BC=0.40 (L-N:1) , WB=0.43 (G-L:1) , SSI=0.32 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (N) (INPUT = 0.90)
JSI METAL= 0.45 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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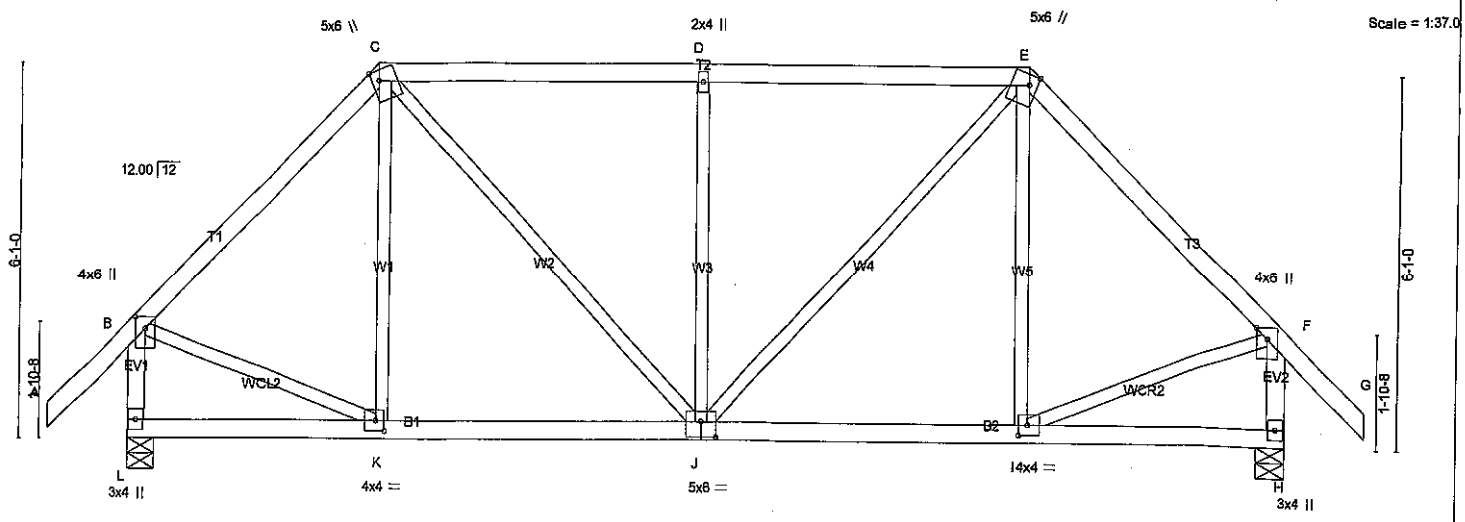

TOTAL WEIGHT = 89 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table ts in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	4.0	6.0	2.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BSWWW-I	MT20	5.0	6.0	3.00	3.00
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1616	0	1616	0	0	IN-SX	IN-SX
H	1616	0	1616	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0
H	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-1193 / 0	-122.2	-122.2	0.42 (1)	5.30
C- D	-1284 / 0	-122.2	-122.2	0.66 (1)	4.66
D- E	-1284 / 0	-122.2	-122.2	0.66 (1)	4.66
E- F	-1193 / 0	-122.2	-122.2	0.42 (1)	5.30
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00
L- B	-1571 / 0	0.0	0.0	0.17 (1)	6.58
H- F	-1571 / 0	0.0	0.0	0.17 (1)	6.58
L- K	0 / 0	-28.0	-28.0	0.15 (2)	10.00
K- J	0 / 838	-28.0	-28.0	0.24 (2)	10.00
J- I	0 / 838	-28.0	-28.0	0.24 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.15 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-145 / 145	0.08 (1)
C- J	0 / 652	0.15 (1)
J- D	-807 / 0	0.47 (1)
J- E	0 / 652	0.15 (1)
I- E	-145 / 145	0.08 (1)
B- K	0 / 891	0.20 (1)
I- F	0 / 891	0.20 (1)

LICENSED PROFESSIONAL ENGINEER
9287
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T4M 4706317
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.66 (D-E:1) , BC=0.24 (J-K:2) , WB=0.47 (D-J:1) , SSI=0.32 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42057	DRWG NO.
288368	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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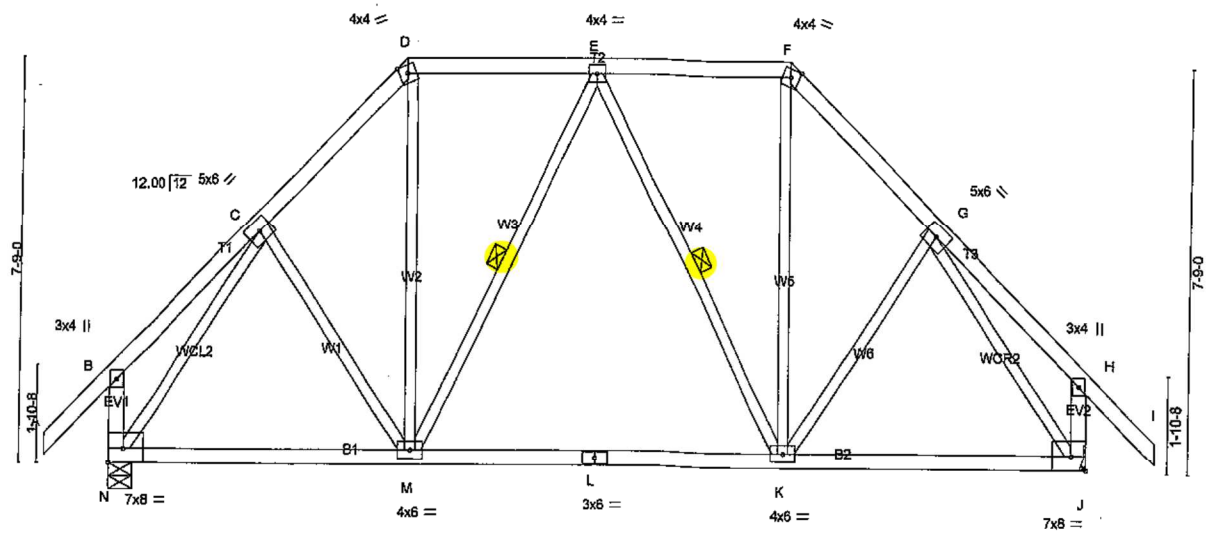

TOTAL WEIGHT = 99 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	Edge	
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	7.0	8.0		Edge
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	7.0	8.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1616	0	1616	0	0	IN-SX	IN-SX
J	1616	0	1616	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0
J	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 27	-122.2	-122.2	0.15 (1)	10.00	
C- D	-1176 / 0	-122.2	-122.2	0.13 (1)	5.75	
D- E	-815 / 0	-122.2	-122.2	0.22 (1)	6.25	
E- F	-815 / 0	-122.2	-122.2	0.22 (1)	6.25	
F- G	-1176 / 0	-122.2	-122.2	0.13 (1)	5.75	
G- H	0 / 27	-122.2	-122.2	0.15 (1)	10.00	
H- I	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
N- B	-309 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-309 / 0	0.0	0.0	0.03 (1)	7.81	
N- M	0 / 814	-28.0	-28.0	0.37 (2)	10.00	
M- L	0 / 950	-28.0	-28.0	0.38 (2)	10.00	
L- K	0 / 950	-28.0	-28.0	0.38 (2)	10.00	
K- J	0 / 814	-28.0	-28.0	0.37 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-10 / 88	0.02 (3)
M- D	0 / 489	0.11 (1)
M- E	-306 / 0	0.14 (1)
E- K	-306 / 0	0.14 (1)
K- F	0 / 489	0.11 (1)
K- G	-10 / 86	0.02 (3)
N- C	-1486 / 0	0.68 (1)
G- J	-1486 / 0	0.68 (1)

LICENSED PROFESSIONAL ENGINEER
92577
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47864 17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.22 (D-E:1) , BC=0.38 (K-M:2) , WB=0.68 (C-N:1) , SSI=0.22 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.82 (C) (INPUT = 0.90)
SI METAL= 0.40 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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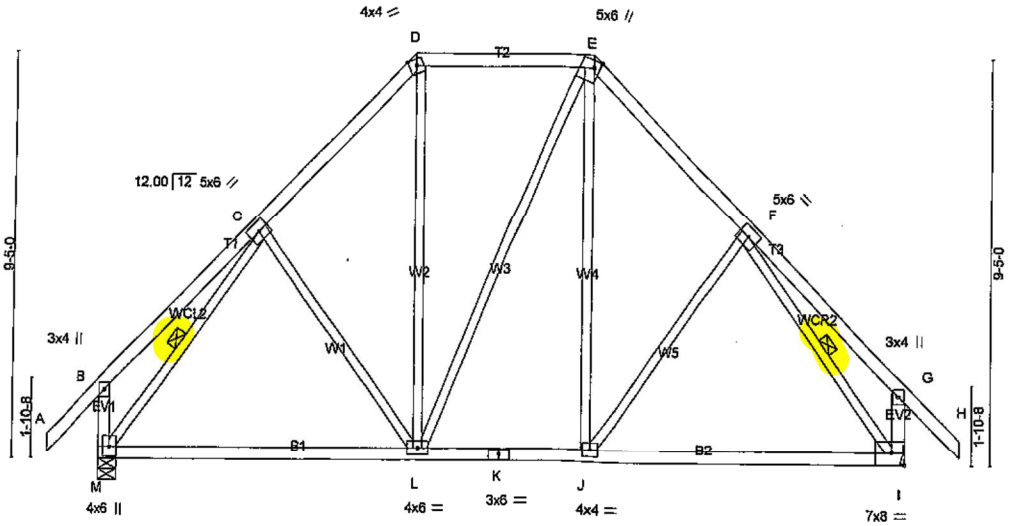

TOTAL WEIGHT = 102 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	7.0	8.0		Edge
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
M	1616	0	1616	0	0	IN-SX	IN-SX
I	1616	0	1616	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0
I	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 38	-122.2	-122.2	0.28 (1)	10.00	
C- D	-1088 / 0	-122.2	-122.2	0.23 (1)	5.80	
D- E	-744 / 0	-122.2	-122.2	0.28 (1)	6.25	
E- F	-1088 / 0	-122.2	-122.2	0.23 (1)	5.80	
F- G	0 / 38	-122.2	-122.2	0.28 (1)	10.00	
G- H	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
M- B	-344 / 0	0.0	0.0	0.04 (1)	7.81	
I- G	-344 / 0	0.0	0.0	0.04 (1)	7.81	
M- L	0 / 839	-28.0	-28.0	0.42 (2)	10.00	
L- K	0 / 743	-28.0	-28.0	0.39 (2)	10.00	
K- J	0 / 743	-28.0	-28.0	0.39 (2)	10.00	
J- I	0 / 838	-28.0	-28.0	0.43 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-165 / 53	0.13 (1)
L- D	0 / 322	0.07 (2)
L- E	0 / 1	0.00 (2)
J- E	0 / 321	0.07 (2)
J- F	-165 / 53	0.13 (1)
M- C	-1449 / 0	0.41 (1)
F- I	-1448 / 0	0.41 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47865 17
STRUCTURAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 930 (0.25")

CSI: TC=0.28 (F-G:1) , BC=0.43 (I-J:2) , WB=0.41 (C-M:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.42 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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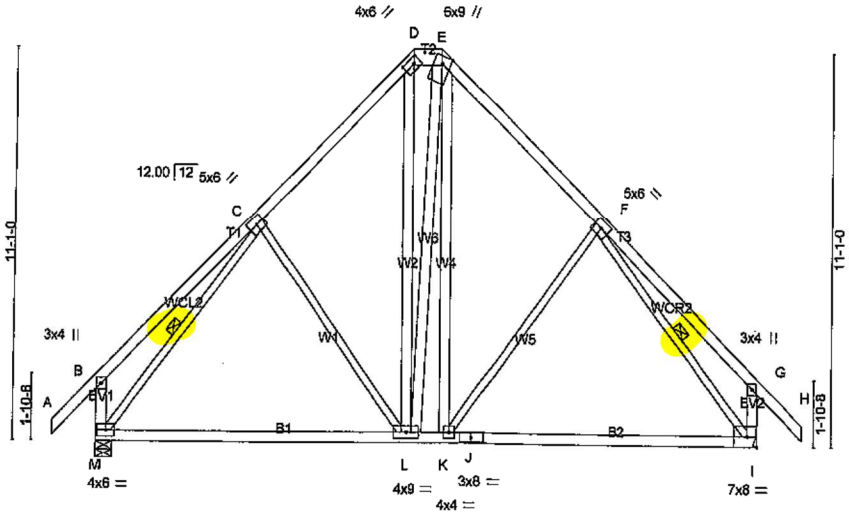

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - L	2x3	DRY	No.2	SPF
K - F	2x3	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
F - I	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-h	MT20	4.0	6.0	2.00	3.75
E	TTWW+m	MT20	6.0	9.0	Edge	
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	7.0	8.0		Edge
J	BS-t	MT20	3.0	0.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1616	0	1616	0	0	IN-SX	IN-SX
I	1616	0	1616	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0
I	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 48	-122.2	-122.2	0.43 (1)	10.00
C- D	-1002 / 0	-122.2	-122.2	0.35 (1)	5.81
D- E	-670 / 0	-122.2	-122.2	0.01 (1)	6.25
E- F	-1000 / 0	-122.2	-122.2	0.35 (1)	5.82
F- G	0 / 48	-122.2	-122.2	0.43 (1)	10.00
G- H	0 / 60	-122.2	-122.2	0.17 (1)	10.00
M- B	-381 / 0	0.0	0.0	0.04 (1)	7.81
I- G	-381 / 0	0.0	0.0	0.04 (1)	7.81
M- L	0 / 851	-28.0	-28.0	0.66 (2)	10.00
L- K	0 / 669	-28.0	-28.0	0.65 (2)	10.00
K- J	0 / 850	-28.0	-28.0	0.64 (2)	10.00
J- I	0 / 850	-28.0	-28.0	0.64 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-295 / 35	0.34 (1)
L- D	0 / 406	0.07 (1)
K- E	0 / 391	0.06 (1)
K- F	-295 / 34	0.34 (1)
M- C	-1419 / 0	0.55 (1)
F- I	-1418 / 0	0.55 (1)
L- E	0 / 14	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4706617
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 120 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 977 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 589 (0.39")

CSI: TC=0.43 (F-G:1) , BC=0.66 (L-M:2) , WB=0.55 (C-M:1) , SSI=0.21 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.88 (M) (INPUT = 0.90)
USI METAL= 0.83 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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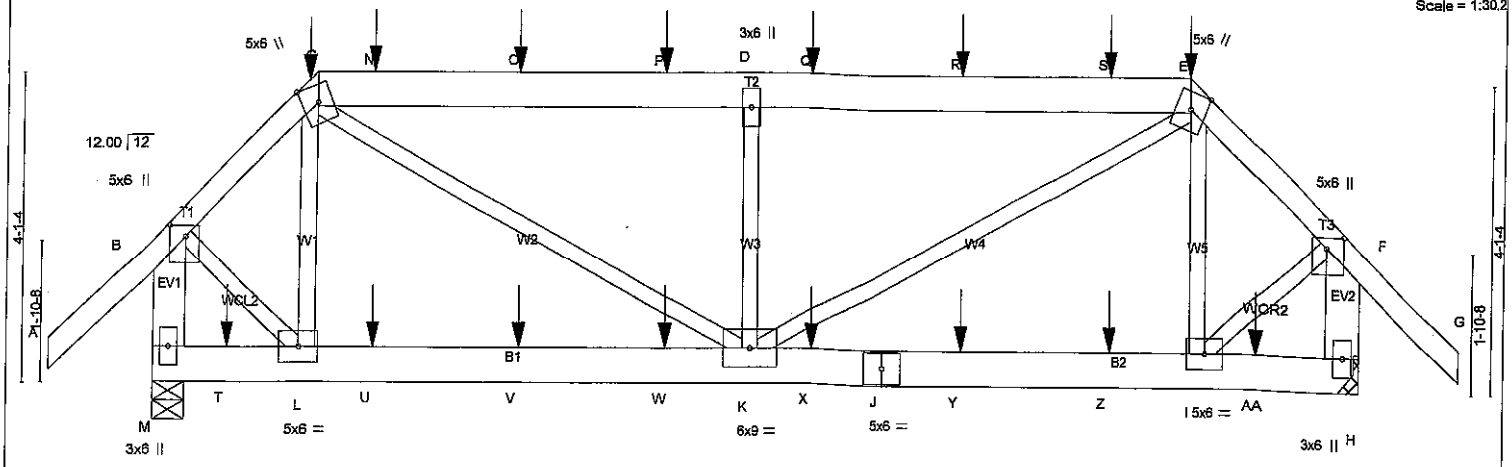

TOTAL WEIGHT = 90 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	5.0	6.0	Edge	
D	TMW+w	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	Edge	
F	TMVW+p	MT20	5.0	6.0	1.75	2.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 2-2-12, 195.5 lbs FACTORED DOWN AT 14-2-4, 157.3 lbs FACTORED DOWN AT 3-0-12, 147.1 lbs FACTORED DOWN AT 5-0-12, 147.1 lbs FACTORED DOWN AT 7-0-12, 147.1 lbs FACTORED DOWN AT 9-0-12, AND 147.1 lbs FACTORED DOWN AT 11-0-12, AND 147.1 lbs FACTORED DOWN AT 13-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, 69.9 lbs FACTORED DOWN AT 7-0-12, 69.9 lbs FACTORED DOWN AT 9-0-12, 69.9 lbs FACTORED DOWN AT 11-0-12, AND 69.9 lbs FACTORED DOWN AT 13-0-12, AND 69.9 lbs FACTORED DOWN AT 15-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
M	2219	0	2219	0	0	IN-SX	IN-SX
H	2191	0	2191	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1723	1143 / 0	294 / 0	0 / 0	0 / 0	286 / 0	0 / 0
H	1701	1129 / 0	290 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.19 (1)	10.00
B- C	-1637 / 0	-122.2	-122.2	0.15 (1)	5.00
C- N	-2547 / 0	-122.2	-122.2	0.63 (1)	4.28
N- O	-2547 / 0	-122.2	-122.2	0.63 (1)	4.28
O- P	-2547 / 0	-122.2	-122.2	0.63 (1)	4.28
P- D	-2547 / 0	-122.2	-122.2	0.63 (1)	4.28
D- Q	-2547 / 0	-122.2	-122.2	0.63 (1)	4.28
Q- R	-2547 / 0	-122.2	-122.2	0.63 (1)	4.28
R- S	-2547 / 0	-122.2	-122.2	0.63 (1)	4.28
S- E	-2547 / 0	-122.2	-122.2	0.63 (1)	4.28
E- F	-1618 / 0	-122.2	-122.2	0.15 (1)	5.03
F- G	0 / 60	-122.2	-122.2	0.19 (1)	10.00
M- B	-2247 / 0	0.0	0.0	0.17 (1)	6.79
H- F	-2225 / 0	0.0	0.0	0.17 (1)	6.81
M- T	0 / 0	-28.0	-28.0	0.16 (2)	10.00
T- L	0 / 0	-28.0	-28.0	0.16 (2)	10.00
L- U	0 / 1174	-28.0	-28.0	0.27 (1)	10.00
U- V	0 / 1174	-28.0	-28.0	0.27 (1)	10.00
V- W	0 / 1174	-28.0	-28.0	0.27 (1)	10.00
W- K	0 / 1174	-28.0	-28.0	0.27 (1)	10.00
K- X	0 / 1161	-28.0	-28.0	0.27 (1)	10.00
X- J	0 / 1161	-28.0	-28.0	0.27 (1)	10.00
J- Y	0 / 1161	-28.0	-28.0	0.27 (1)	10.00
Y- Z	0 / 1161	-28.0	-28.0	0.27 (1)	10.00
Z- I	0 / 1161	-28.0	-28.0	0.27 (1)	10.00
I-AA	0 / 0	-28.0	-28.0	0.16 (2)	10.00
AA- H	0 / 0	-28.0	-28.0	0.16 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-490 / 112	0.12 (1)
C- K	0 / 1609	0.40 (1)
K- D	-1439 / 0	0.34 (1)
K- E	0 / 1625	0.40 (1)
I- E	-481 / 112	0.12 (1)
B- L	0 / 1401	0.35 (1)
I- F	0 / 1386	0.34 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-13	-14	—	FRONT	VERT	DEAD
C	2-2-12	-183	-183	—	FRONT	VERT	SNOW
E	14-2-4	-13	-14	—	FRONT	VERT	DEAD
E	14-2-4	-183	-183	—	FRONT	VERT	SNOW
N	3-0-12	-157	-157	—	BACK	VERT	TOTAL
O	5-0-12	-147	-147	—	BACK	VERT	TOTAL
P	7-0-12	-147	-147	—	BACK	VERT	TOTAL
Q	9-0-12	-147	-147	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.63 (D-E:1) , BC=0.27 (K-L:1) , WB=0.40 (E-K:1) , SSI=0.48 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
92877
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO . TAM47867-17
STRUCTURAL

p6/6

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	11-0-12	-147	-147	—	BACK	VERT	TOTAL
S	13-0-12	-147	-147	—	BACK	VERT	TOTAL
T	1-0-12	-40	-70	—	BACK	VERT	TOTAL
U	3-0-12	-40	-70	—	BACK	VERT	TOTAL
V	5-0-12	-40	-70	—	BACK	VERT	TOTAL
W	7-0-12	-40	-70	—	BACK	VERT	TOTAL
X	9-0-12	-40	-70	—	BACK	VERT	TOTAL
Y	11-0-12	-40	-70	—	BACK	VERT	TOTAL
Z	13-0-12	-40	-70	—	BACK	VERT	TOTAL
AA	15-0-12	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
92577
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47867-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T15	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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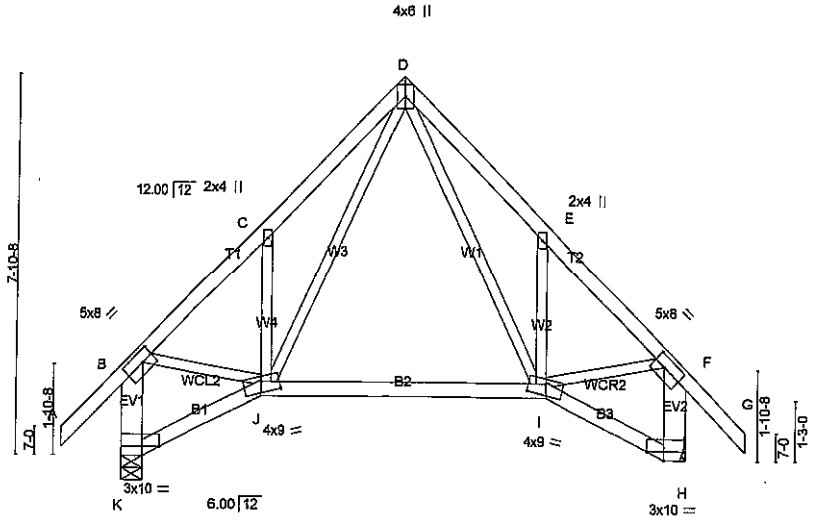


TOTAL WEIGHT = 3 X 65 = 194 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	8.0	2.50	3.00
H	BVM1-p	MT20	3.0	10.0	Edge	4.50
I	BBWWW-m	MT20	4.0	9.0	2.75	4.50
J	BBWWW-m	MT20	4.0	9.0	2.75	4.50
K	BVM1-p	MT20	3.0	10.0	Edge	4.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1072	0	1072	0	0	IN-SX	IN-SX
H	1072	0	1072	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

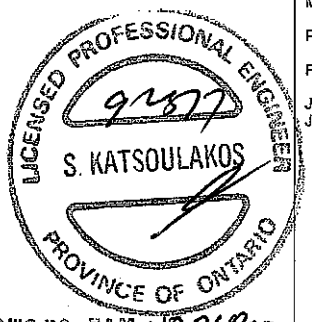
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B-C	-904 / 0	-122.2	-122.2	0.13 (1)	6.25	
C-D	-958 / 0	-122.2	-122.2	0.13 (1)	6.21	
D-E	-958 / 0	-122.2	-122.2	0.13 (1)	6.21	
E-F	-904 / 0	-122.2	-122.2	0.13 (1)	6.25	
F-G	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
K-B	-1030 / 0	0.0	0.0	0.08 (1)	7.81	
H-F	-1030 / 0	0.0	0.0	0.08 (1)	7.81	
K-J	0 / 0	-28.0	-28.0	0.07 (2)	10.00	
J-I	0 / 411	-28.0	-28.0	0.35 (2)	10.00	
I-H	0 / 0	-28.0	-28.0	0.07 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 / 567	0.13 (1)
I-E	-442 / 0	0.10 (1)
J-D	0 / 567	0.13 (1)
J-C	-442 / 0	0.10 (1)
B-J	0 / 661	0.15 (1)
I-F	0 / 661	0.15 (1)



DWG NO. TAM 47866 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 865 (0.17")

CSI: TC=0.17 (F-G:1) , BC=0.35 (I-J:2) , WB=0.15 (B-J:1) , SSI=0.12 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (I) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T16	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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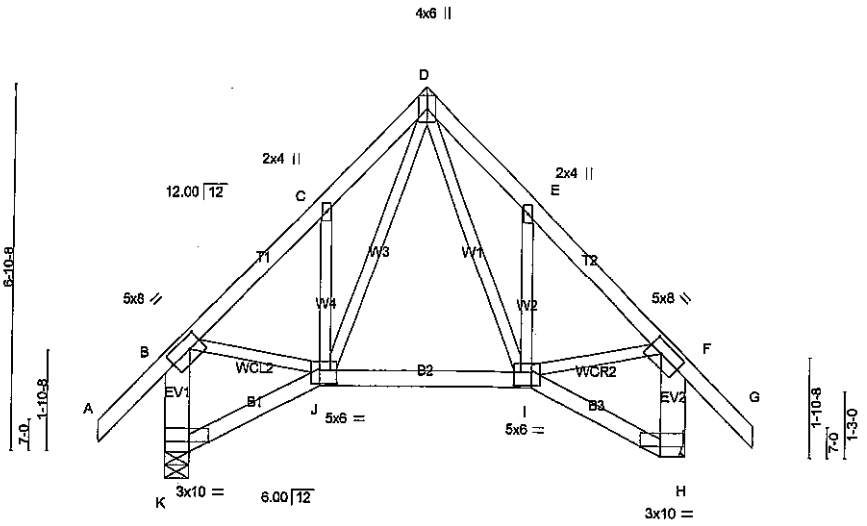

TOTAL WEIGHT = 3 X 57 = 171 lb

[MII[F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	8.0	2.50	3.00
H	BVM1-p	MT20	3.0	10.0	Edge	4.50
I	BBWWW-p	MT20	5.0	6.0	3.00	4.00
J	BBWWW-p	MT20	5.0	6.0	3.00	4.00
K	BVM1-p	MT20	3.0	10.0	Edge	4.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	922	0	922	0	0	IN-SX	IN-SX
H	922	0	922	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	702	489 / 0	105 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	702	489 / 0	105 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	D- I	0 / 458		0.10 (1)
B- C	-703 / 0	-122.2	-122.2	0.13 (1)	6.25	I- E	-374 / 0		0.08 (1)
C- D	-752 / 0	-122.2	-122.2	0.09 (1)	6.25	J- D	0 / 458		0.10 (1)
D- E	-752 / 0	-122.2	-122.2	0.09 (1)	6.25	J- C	-374 / 0		0.08 (1)
E- F	-703 / 0	-122.2	-122.2	0.13 (1)	6.25	B- J	0 / 513		0.12 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I- F	0 / 513		0.12 (1)
K- B	-880 / 0	0.0	0.0	0.07 (1)	7.81				
H- F	-880 / 0	0.0	0.0	0.07 (1)	7.81				
K- J	0 / 0	-28.0	-28.0	0.07 (3)	10.00				
J- I	0 / 346	-28.0	-28.0	0.17 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.07 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.17 (F-G:1) , BC=0.17 (I-J:2) , WB=0.12 (B-J:1) , SSI=0.12 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (J) (INPUT = 0.90)
JSI METAL= 0.18 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9 2577
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47869-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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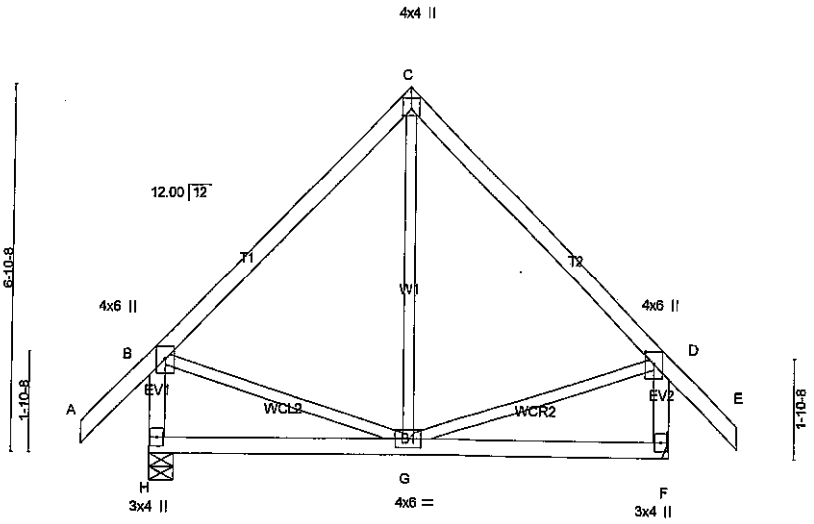


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	922	0	922	0	0	IN-SX	IN-SX
						5-8	5-8
F	922	0	922	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	702	489 / 0	105 / 0	0 / 0	0 / 0	108 / 0	0 / 0
F	702	489 / 0	105 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	G- C	-10 / 225	0.05 (3)
B- C	-425 / 0	-122.2	-122.2	0.39 (1)	6.25	B- G	0 / 314	0.07 (1)
C- D	-425 / 0	-122.2	-122.2	0.39 (1)	6.25	G- D	0 / 314	0.07 (1)
D- E	0 / 60	-122.2	-122.2	0.17 (1)	10.00			
H- B	-868 / 0	0.0	0.0	0.10 (1)	7.81			
F- D	-868 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.21 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.21 (3)	10.00			

TOTAL WEIGHT = 49 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.39 (B-C:1) , BC=0.21 (F-G:3) , WB=0.07 (B-G:1) , SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

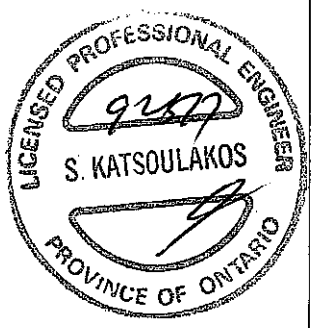
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)



DWG NO. TAM 47870-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288368	T18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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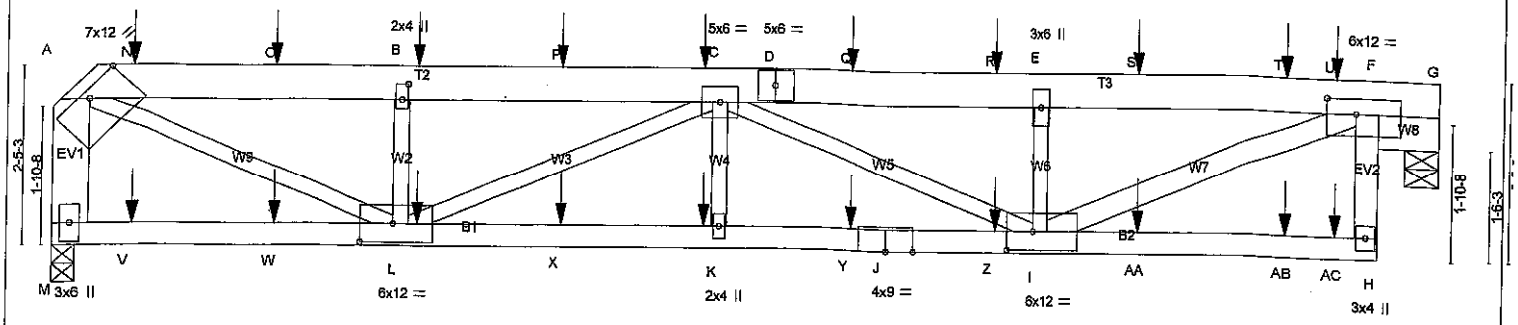


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	1650F 1.5E	SPF
D - G	2x6	DRY	1650F 1.5E	SPF
F - G	2x6	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVWm	MT20	7.0	12.0	1.25	6.25
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMWWW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TMVWW-t	MT20	6.0	12.0	2.50	4.75
H	BMV+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	6.0	12.0	3.00	4.50
J	BS-t	MT20	4.0	9.0		
K	BMW+w	MT20	2.0	4.0		
L	BMWWW-t	MT20	6.0	12.0	3.00	5.50
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 84.1 lbs FACTORED DOWN AT 1-0-4, 84.1 lbs FACTORED DOWN AT 3-0-4, 84.1 lbs FACTORED DOWN AT 5-0-4, 84.1 lbs FACTORED DOWN AT 7-0-4, 84.1 lbs FACTORED DOWN AT 9-0-4, 84.1 lbs FACTORED DOWN AT 11-0-4, 84.1 lbs FACTORED DOWN AT 13-0-4, 84.1 lbs FACTORED DOWN AT 15-0-4, AND 84.1 lbs FACTORED DOWN AT 17-0-4, AND 84.1 lbs FACTORED DOWN AT 17-8-4 ON TOP CHORD, AND 41.9 lbs FACTORED DOWN AT 1-0-4, 41.9 lbs FACTORED DOWN AT 3-0-4, 41.9 lbs FACTORED DOWN AT 5-0-4, 41.9 lbs FACTORED DOWN AT 7-0-4, 41.9 lbs FACTORED DOWN AT 9-0-4, 41.9 lbs FACTORED DOWN AT 11-0-4, 41.9 lbs FACTORED DOWN AT 13-0-4, 41.9 lbs FACTORED DOWN AT 15-0-4, AND 41.9 lbs FACTORED DOWN AT 17-0-4, AND 52.6 lbs FACTORED DOWN AT 17-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1954	0	1954	0	0	3-8	3-8
G	1976	0	1976	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): G

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1538	983 / 0	287 / 0	0 / 0	0 / 0	268 / 0	0 / 0
G	1548	1002 / 0	280 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

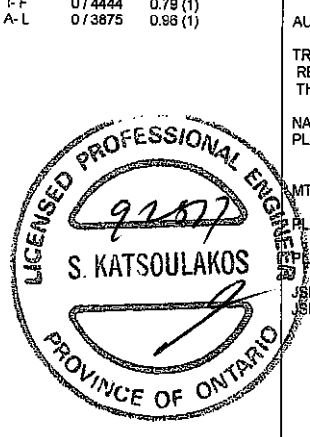
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- N	-3561 / 0	-122.2	-122.2	0.22 (1)		4.87
N- O	-3561 / 0	-122.2	-122.2	0.22 (1)		4.87
O- B	-3561 / 0	-122.2	-122.2	0.22 (1)		4.87
B- P	-3561 / 0	-122.2	-122.2	0.20 (1)		4.90
P- C	-3561 / 0	-122.2	-122.2	0.20 (1)		4.90
C- D	-4054 / 0	-122.2	-122.2	0.38 (1)		4.43
D- Q	-4054 / 0	-122.2	-122.2	0.38 (1)		4.43
Q- R	-4054 / 0	-122.2	-122.2	0.38 (1)		4.43
R- E	-4054 / 0	-122.2	-122.2	0.38 (1)		4.43
E- S	-4054 / 0	-122.2	-122.2	0.88 (1)		3.17
S- T	-4054 / 0	-122.2	-122.2	0.88 (1)		3.17
T- U	-4054 / 0	-122.2	-122.2	0.88 (1)		3.17
U- F	-4054 / 0	-122.2	-122.2	0.88 (1)		3.17
F- G	0 / 0	-122.2	-122.2	0.49 (1)		10.00
H- F	0 / 170	0.0	0.0	0.03 (2)		10.00
M- A	-1870 / 0	0.0	0.0	0.15 (1)		7.29
M- V	0 / 0	-28.0	-28.0	0.22 (3)		10.00
V- W	0 / 0	-28.0	-28.0	0.22 (3)		10.00
W- L	0 / 0	-28.0	-28.0	0.22 (3)		10.00
L- X	0 / 4517	-28.0	-28.0	0.90 (1)		10.00
X- K	0 / 4517	-28.0	-28.0	0.90 (1)		10.00
K- Y	0 / 4517	-28.0	-28.0	0.91 (1)		10.00
Y- J	0 / 4517	-28.0	-28.0	0.91 (1)		10.00
J- Z	0 / 4517	-28.0	-28.0	0.91 (1)		10.00
Z- I	0 / 4517	-28.0	-28.0	0.91 (1)		10.00
I-AA	0 / 0	-28.0	-28.0	0.23 (3)		10.00
AA-AB	0 / 0	-28.0	-28.0	0.23 (3)		10.00
AB-AC	0 / 0	-28.0	-28.0	0.23 (3)		10.00
AC- H	0 / 0	-28.0	-28.0	0.23 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-877 / 0	0.15 (1)
L- C	-1054 / 0	0.43 (1)
K- C	0 / 273	0.07 (3)
C- I	-511 / 0	0.21 (1)
I- E	-1398 / 0	0.23 (1)
I- F	0 / 4444	0.79 (1)
A- L	0 / 3875	0.96 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-0-4	-84	-84	—	BACK	VERT	TOTAL
C	9-0-4	-84	-84	—	BACK	VERT	TOTAL
K	9-0-4	-24	-42	—	BACK	VERT	TOTAL
L	5-0-4	-24	-42	—	BACK	VERT	TOTAL



TOTAL WEIGHT = 82 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 892 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 582 (0.39")

CSI: TC=0.88 (E-F:1) , BC=0.91 (I-K:1) , WB=0.96 (A-L:1) , SSI=0.66 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (F) (INPUT = 0.90)
SSI METAL= 0.92 (J) (INPUT = 1.00)

DWG NO. TAM47871-17
STRUCTURAL
COMPONENT

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	1-0-4	-84	-84	--	BACK	VERT	TOTAL
O	3-0-4	-84	-84	--	BACK	VERT	TOTAL
P	7-0-4	-84	-84	--	BACK	VERT	TOTAL
Q	11-0-4	-84	-84	--	BACK	VERT	TOTAL
R	13-0-4	-84	-84	--	BACK	VERT	TOTAL
S	15-0-4	-84	-84	--	BACK	VERT	TOTAL
T	17-0-4	-84	-84	--	BACK	VERT	TOTAL
U	17-8-4	-84	-84	--	BACK	VERT	TOTAL
V	1-0-4	-24	-42	--	BACK	VERT	TOTAL
W	3-0-4	-24	-42	--	BACK	VERT	TOTAL
X	7-0-4	-24	-42	--	BACK	VERT	TOTAL
Y	11-0-4	-24	-42	--	BACK	VERT	TOTAL
Z	13-0-4	-24	-42	--	BACK	VERT	TOTAL
AA	15-0-4	-24	-42	--	BACK	VERT	TOTAL
AB	17-0-4	-24	-42	--	BACK	VERT	TOTAL
AC	17-8-4	-30	-53	--	BACK	VERT	TOTAL

DWG NO. TAM 47871-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288370	T19	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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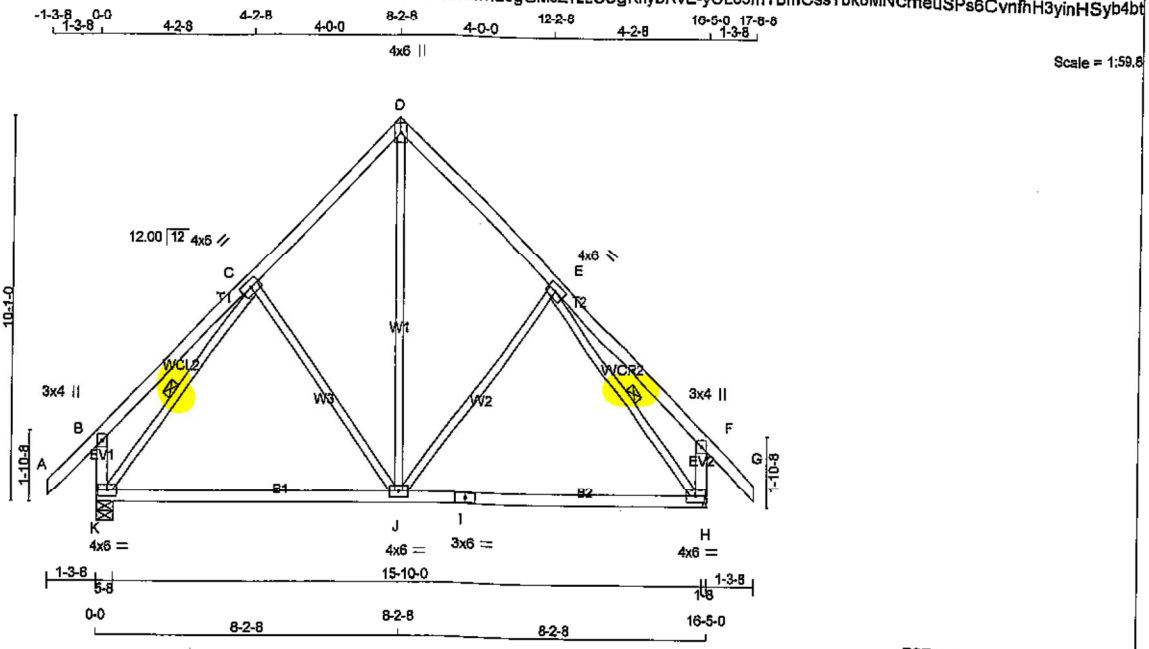


TOTAL WEIGHT = 3 X 83 = 248 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	6.0	
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BS-t	MT20	3.0	6.0	
J	BMWWW-t	MT20	4.0	6.0	
K	BMVW1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1403	0	1403	0	0	IN-SX	IN-SX
H	1403	0	1403	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1079	734 / 0	172 / 0	0 / 0	0 / 0	173 / 0	0 / 0
H	1079	734 / 0	172 / 0	0 / 0	0 / 0	173 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

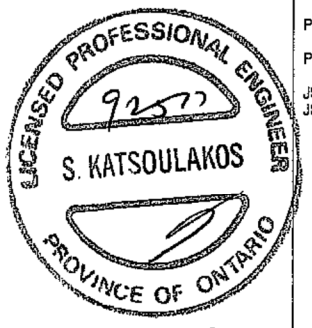
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 42	-122.2	-122.2	0.34 (1)	10.00
C- D	-810 / 0	-122.2	-122.2	0.26 (1)	6.25
D- E	-810 / 0	-122.2	-122.2	0.26 (1)	6.25
E- F	0 / 42	-122.2	-122.2	0.34 (1)	10.00
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00
K- B	-359 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-359 / 0	0.0	0.0	0.04 (1)	7.81
K- J	0 / 696	-28.0	-28.0	0.62 (2)	10.00
J- I	0 / 696	-28.0	-28.0	0.62 (2)	10.00
I- H	0 / 696	-28.0	-28.0	0.62 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 696	0.16 (1)
J- E	-256 / 33	0.24 (1)
C- J	-256 / 33	0.24 (1)
K- C	-1183 / 0	0.38 (1)
E- H	-1183 / 0	0.38 (1)



DWG NO. TAM47873-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/ 890 (0.22")

CSI: TC=0.34 (E-F:1) , BC=0.62 (H-J:2) , WB=0.38 (C-K:1) , SSI=0.20 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288370	G19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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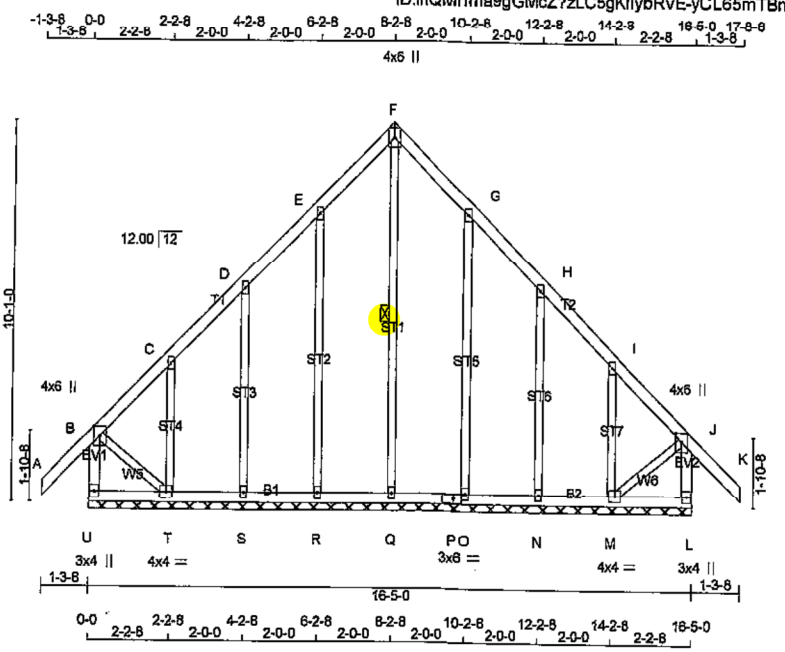

TOTAL WEIGHT = 92 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0		
J	TMVW+p	MT20	4.0	6.0	2.75	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, Q, R, S						
N	BMW1+w	MT20	2.0	4.0		
P	BS-t	MT20	3.0	6.0	1.50	2.50
T	BMWW1-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-42 / 0	-122.2	-122.2	0.07 (1)	6.25
C- D	-56 / 0	-122.2	-122.2	0.07 (1)	6.25
D- E	-38 / 0	-122.2	-122.2	0.07 (1)	6.25
E- F	-56 / 0	-122.2	-122.2	0.07 (1)	6.25
F- G	-56 / 0	-122.2	-122.2	0.07 (1)	6.25
G- H	-38 / 0	-122.2	-122.2	0.07 (1)	6.25
H- I	-56 / 0	-122.2	-122.2	0.07 (1)	6.25
I- J	-42 / 0	-122.2	-122.2	0.07 (1)	6.25
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00
U- B	-345 / 0	0.0	0.0	0.04 (1)	7.81
L- J	-345 / 0	0.0	0.0	0.04 (1)	7.81
U- T	0 / 0	-28.0	-28.0	0.03 (2)	10.00
T- S	0 / 35	-28.0	-28.0	0.03 (2)	10.00
S- R	0 / 31	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 27	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 27	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 27	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 31	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 35	-28.0	-28.0	0.03 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- F	-154 / 0	0.10 (1)
R- E	-279 / 0	0.35 (1)
S- D	-223 / 0	0.13 (1)
T- C	-278 / 0	0.07 (1)
O- G	-279 / 0	0.35 (1)
N- H	-223 / 0	0.13 (1)
M- I	-278 / 0	0.07 (1)
B- T	0 / 52	0.01 (1)
M- J	0 / 52	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.03 (M-N:2) , WB=0.35 (G-O:1) , SSI=0.09 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (T) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

DWG NO. TAM 47874-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	T20	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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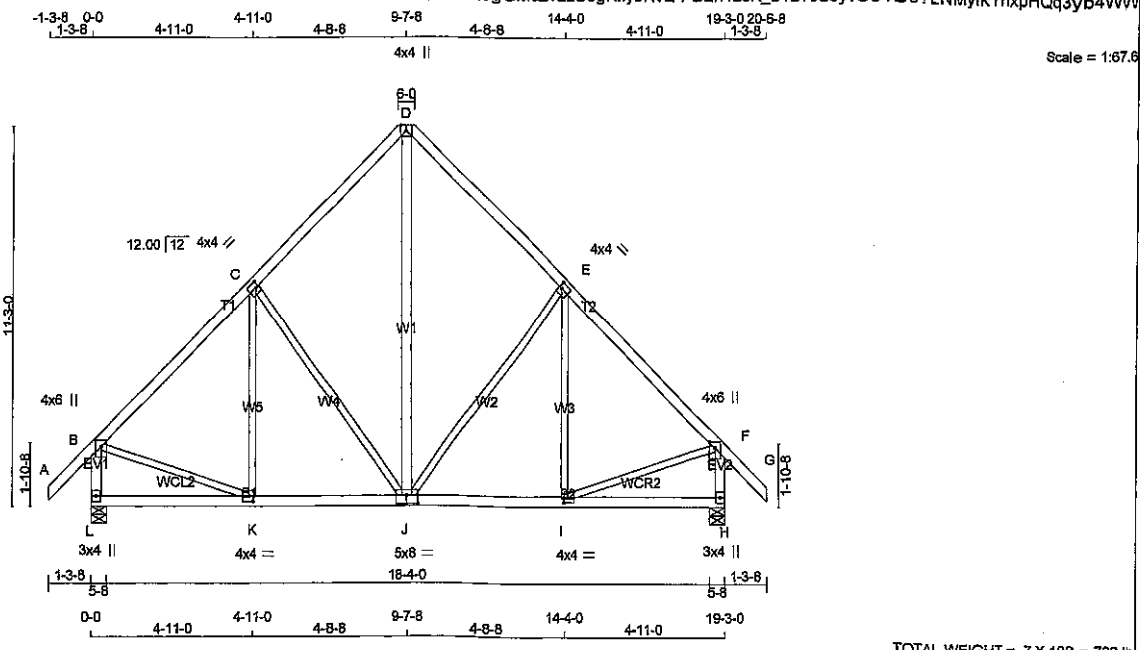

TOTAL WEIGHT = 7 X 103 = 723 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMTMW+p	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BSWWW-I	MT20	5.0	8.0	3.00	4.00
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1616	0	1616	0	0	5-8	5-8
H	1616	0	1616	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0
H	1246	843 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	J- D	0 / 827	0.13 (1)
B- C	-1202 / 0	-122.2	-122.2	0.40 (1)	5.35	J- E	-435 / 0	0.58 (1)
C- D	-940 / 0	-122.2	-122.2	0.38 (1)	5.90	I- E	-123 / 166	0.09 (1)
D- E	-940 / 0	-122.2	-122.2	0.38 (1)	5.90	C- J	-435 / 0	0.58 (1)
E- F	-1202 / 0	-122.2	-122.2	0.40 (1)	5.35	K- C	-123 / 166	0.09 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00	B- K	0 / 926	0.21 (1)
L- B	-1560 / 0	0.0	0.0	0.17 (1)	6.60	I- F	0 / 926	0.21 (1)
H- F	-1560 / 0	0.0	0.0	0.17 (1)	6.60			
L- K	0 / 0	-28.0	-28.0	0.16 (3)	10.00			
K- J	0 / 885	-28.0	-28.0	0.25 (2)	10.00			
J- I	0 / 885	-28.0	-28.0	0.25 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.16 (3)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47877-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.40 (E-F:1) , BC=0.25 (I-J:2) , WB=0.58 (E-J:1) , SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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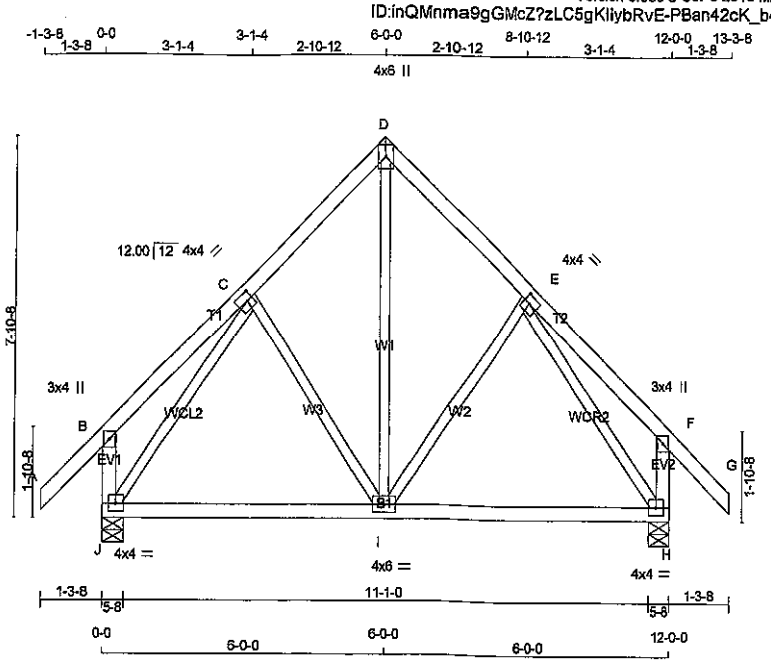

TOTAL WEIGHT = 64 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1072	0	1072	0	0	5-8	5-8
H	1072	0	1072	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 459	0.10 (1)
B- C	0 / 30	-122.2	-122.2	0.17 (1)	10.00	I- E	-152 / 36	0.08 (1)
C- D	-559 / 0	-122.2	-122.2	0.13 (1)	6.25	C- I	-152 / 36	0.08 (1)
D- E	-559 / 0	-122.2	-122.2	0.13 (1)	6.25	J- C	-836 / 0	0.40 (1)
E- F	0 / 30	-122.2	-122.2	0.17 (1)	10.00	E- H	-836 / 0	0.40 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00			
J- B	-309 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-309 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 461	-28.0	-28.0	0.33 (2)	10.00			
I- H	0 / 461	-28.0	-28.0	0.33 (2)	10.00			

DWG NO. TAM 47878 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17 (B-C:1) , BC=0.33 (I-J:2) , WB=0.40 (E-H:1) , SSI=0.14 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10.

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	G21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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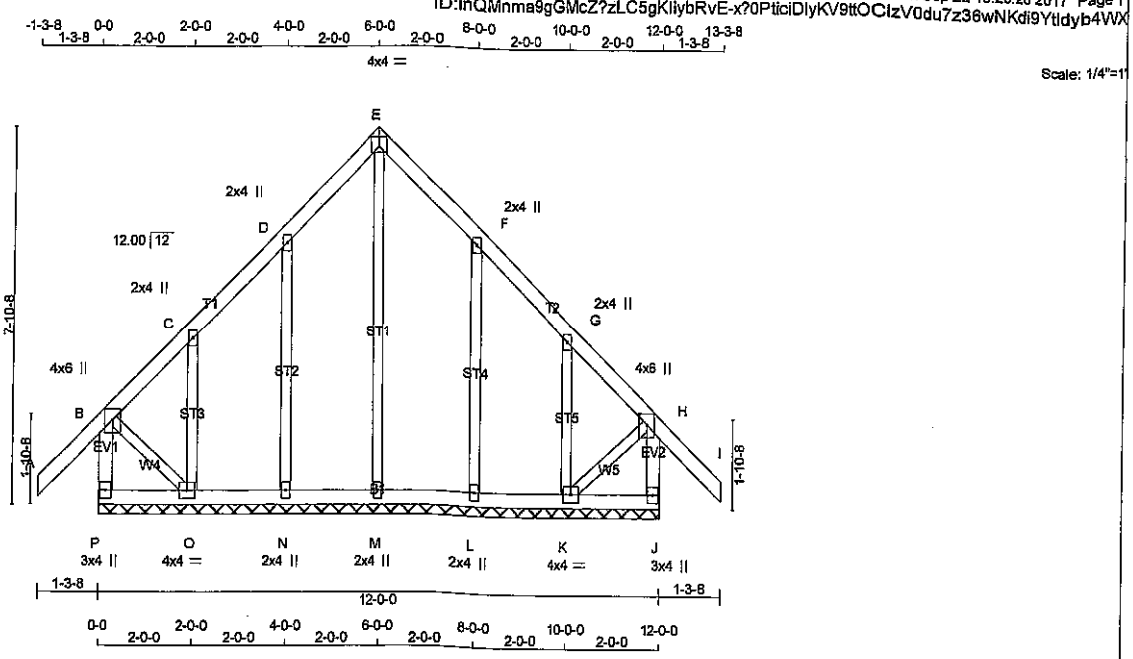

TOTAL WEIGHT = 65 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	6.0	2.75	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 60	-122.2	-122.2	1	0.17 (1)	10.00
B-C	-76 / 0	-122.2	-122.2	1	0.16 (1)	6.25
C-D	-17 / 0	-122.2	-122.2	1	0.08 (1)	6.25
D-E	-42 / 0	-122.2	-122.2	1	0.08 (1)	6.25
E-F	-42 / 0	-122.2	-122.2	1	0.08 (1)	6.25
F-G	-17 / 0	-122.2	-122.2	1	0.08 (1)	6.25
G-H	-76 / 0	-122.2	-122.2	1	0.16 (1)	6.25
H-I	0 / 60	-122.2	-122.2	1	0.17 (1)	10.00
P-B	-387 / 0	0.0	0.0	1	0.04 (1)	7.81
J-H	-387 / 0	0.0	0.0	1	0.04 (1)	7.81
P-O	0 / 0	-28.0	-28.0	2	0.03 (2)	10.00
O-N	0 / 22	-28.0	-28.0	2	0.03 (2)	10.00
N-M	0 / 16	-28.0	-28.0	2	0.02 (2)	10.00
M-L	0 / 16	-28.0	-28.0	2	0.02 (2)	10.00
L-K	0 / 22	-28.0	-28.0	2	0.03 (2)	10.00
K-J	0 / 0	-28.0	-28.0	2	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-170 / 0	0.19 (1)
N-D	-292 / 0	0.15 (1)
O-C	-160 / 0	0.04 (1)
L-F	-292 / 0	0.15 (1)
K-G	-160 / 0	0.04 (1)
B-O	0 / 34	0.01 (1)
K-H	0 / 34	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (H-I:1) , BC=0.03 (K-L:2) , WB=0.19 (E-M:1) , SSI=0.09 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (E) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

DWG NO. TAM 47882-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	T22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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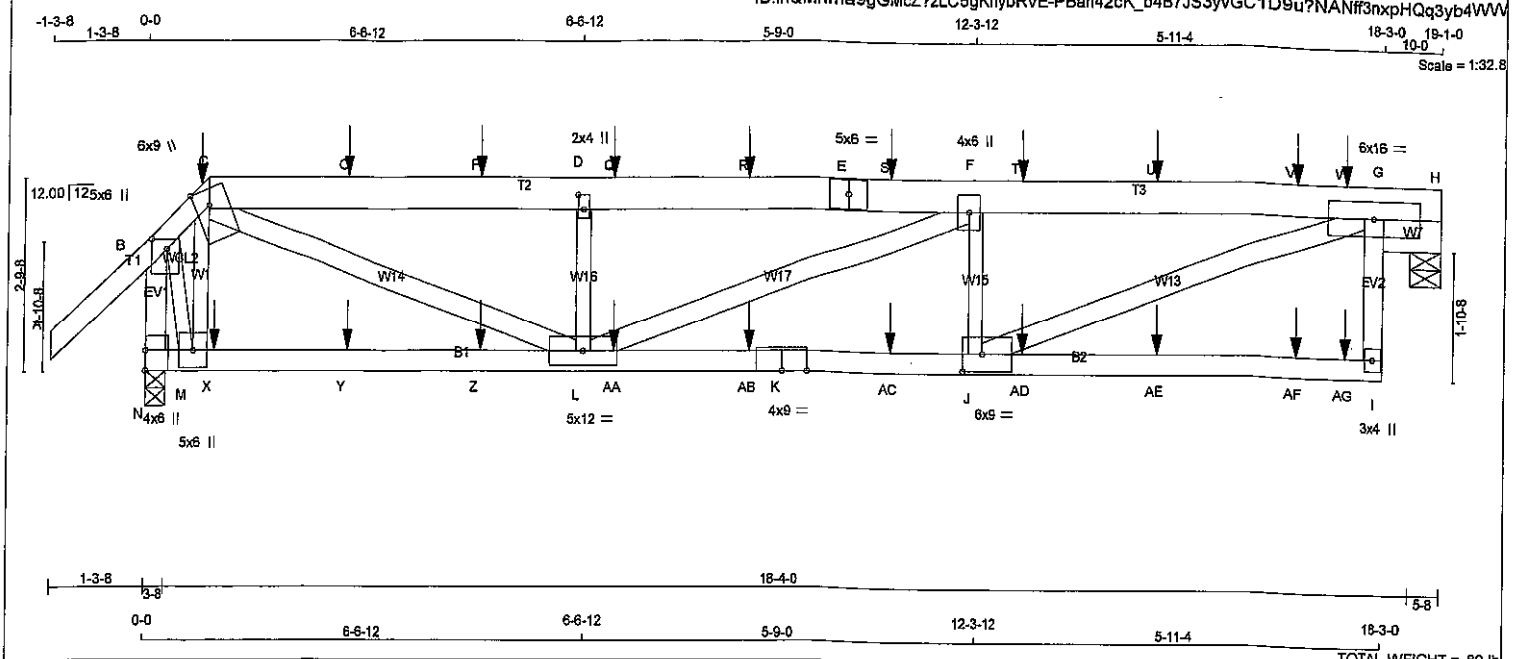


TOTAL WEIGHT = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	2100F 1.8E	SPF
E - H	2x6	DRY	2100F 1.8E	SPF
G - H	2x6	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
C - L	2x4	DRY	No.2	SPF
L - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	2.50
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMVWW-t	MT20	6.0	16.0		
I	BMV+p	MT20	3.0	4.0		
J	BMWW-t	MT20	6.0	9.0	3.00	3.50
K	BS-t	MT20	4.0	9.0		
L	BMWWW-t	MT20	5.0	12.0		
M	BMWW+t	MT20	5.0	6.0		
N	BMV1+t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 137.1 lbs FACTORED DOWN AT 11-0, 133.7 lbs FACTORED DOWN AT 3-0-4, 133.7 lbs FACTORED DOWN AT 5-0-4, 133.7 lbs FACTORED DOWN AT 7-0-4, 133.7 lbs FACTORED DOWN AT 9-0-4, 133.7 lbs FACTORED DOWN AT 11-0-4, 133.7 lbs FACTORED DOWN AT 13-0-4, 133.7 lbs FACTORED DOWN AT 15-0-4, AND 133.7 lbs FACTORED DOWN AT 17-0-4, AND 133.7 lbs FACTORED DOWN AT 17-8-4 ON TOP CHORD, AND 62.3 lbs FACTORED DOWN AT 1-0-4, 62.3 lbs FACTORED DOWN AT 3-0-4, 62.3 lbs FACTORED DOWN AT 5-0-4, 62.3 lbs FACTORED DOWN AT 7-0-4, 62.3 lbs FACTORED DOWN AT 9-0-4, 62.3 lbs FACTORED DOWN AT 11-0-4, 62.3 lbs FACTORED DOWN AT 13-0-4, 62.3 lbs FACTORED DOWN AT 15-0-4, AND 62.3 lbs FACTORED DOWN AT 17-0-4, AND 73.0 lbs FACTORED DOWN AT 17-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2430	0	2430	0	0	3-8	3-8
H	2287	0	2287	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): H

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1892	1244 / 0	329 / 0	0 / 0	0 / 0	319 / 0	0 / 0
H	1793	1159 / 0	326 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

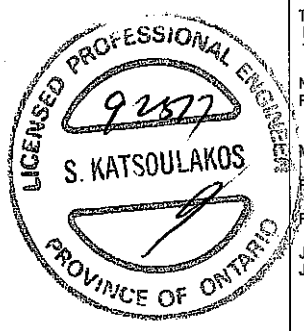
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60		-122.2	-122.2	0.19 (1)	10.00
B- C	-1027 / 0		-122.2	-122.2	0.19 (1)	5.92
C- O	-4167 / 0		-122.2	-122.2	0.28 (1)	4.83
O- P	-4167 / 0		-122.2	-122.2	0.28 (1)	4.83
P- D	-4167 / 0		-122.2	-122.2	0.28 (1)	4.83
D- Q	-4167 / 0		-122.2	-122.2	0.43 (1)	4.61
Q- R	-4167 / 0		-122.2	-122.2	0.43 (1)	4.61
R- E	-4167 / 0		-122.2	-122.2	0.43 (1)	4.61
E- S	-4167 / 0		-122.2	-122.2	0.43 (1)	4.61
S- F	-4167 / 0		-122.2	-122.2	0.43 (1)	4.61
F- T	-4520 / 0		-122.2	-122.2	0.87 (1)	3.42
T- U	-4520 / 0		-122.2	-122.2	0.87 (1)	3.42
U- V	-4520 / 0		-122.2	-122.2	0.87 (1)	3.42
V- W	-4520 / 0		-122.2	-122.2	0.87 (1)	3.42
W- G	-4520 / 0		-122.2	-122.2	0.87 (1)	3.42
G- H	0 / 0		-122.2	-122.2	0.57 (1)	10.00
I- G	0 / 251		0.0	0.0	0.04 (2)	10.00
N- B	-2635 / 0		0.0	0.0	0.32 (1)	5.23
N- M	0 / 0		-28.0	-28.0	0.33 (2)	10.00
M- X	0 / 816		-28.0	-28.0	0.43 (2)	10.00
X- Y	0 / 816		-28.0	-28.0	0.43 (2)	10.00
Y- Z	0 / 816		-28.0	-28.0	0.43 (2)	10.00
Z- L	0 / 816		-28.0	-28.0	0.43 (2)	10.00
L-AA	0 / 4520		-28.0	-28.0	0.99 (1)	10.00
AA-AB	0 / 4520		-28.0	-28.0	0.99 (1)	10.00
AB- K	0 / 4520		-28.0	-28.0	0.99 (1)	10.00
K-AC	0 / 4520		-28.0	-28.0	0.99 (1)	10.00
AC- J	0 / 4520		-28.0	-28.0	0.99 (1)	10.00
J-AD	0 / 0		-28.0	-28.0	0.43 (3)	10.00
AD-AE	0 / 0		-28.0	-28.0	0.43 (3)	10.00
AE-AF	0 / 0		-28.0	-28.0	0.43 (3)	10.00
AF-AG	0 / 0		-28.0	-28.0	0.43 (3)	10.00
AG- I	0 / 0		-28.0	-28.0	0.43 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-1254 / 0	0.22 (1)
B- M	0 / 1753	0.43 (1)
J- G	0 / 4881	0.86 (1)
C- L	0 / 3648	0.64 (1)
J- F	-1543 / 0	0.27 (1)
L- D	-1036 / 0	0.18 (1)
L- F	-363 / 0	0.19 (1)



DWG NO. TAM 47879-17
STRUCTURAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 939 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 584 (0.39")

CSI: TC=0.87 (F-G:1) , BC=0.99 (J-L:1) , WB=0.86 (G-J:1) , SSI=0.76 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.94 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	T22	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	11-0	-137	-137	---	BACK	VERT	TOTAL
O	3-0-4	-134	-134	---	BACK	VERT	TOTAL
P	5-0-4	-134	-134	---	BACK	VERT	TOTAL
Q	7-0-4	-134	-134	---	BACK	VERT	TOTAL
R	9-0-4	-134	-134	---	BACK	VERT	TOTAL
S	11-0-4	-134	-134	---	BACK	VERT	TOTAL
T	13-0-4	-134	-134	---	BACK	VERT	TOTAL
U	15-0-4	-134	-134	---	BACK	VERT	TOTAL
V	17-0-4	-134	-134	---	BACK	VERT	TOTAL
W	17-8-4	-134	-134	---	BACK	VERT	TOTAL
X	1-0-4	-36	-62	---	BACK	VERT	TOTAL
Y	3-0-4	-36	-62	---	BACK	VERT	TOTAL
Z	5-0-4	-36	-62	---	BACK	VERT	TOTAL
AA	7-0-4	-36	-62	---	BACK	VERT	TOTAL
AB	9-0-4	-36	-62	---	BACK	VERT	TOTAL
AC	11-0-4	-36	-62	---	BACK	VERT	TOTAL
AD	13-0-4	-36	-62	---	BACK	VERT	TOTAL
AE	15-0-4	-36	-62	---	BACK	VERT	TOTAL
AF	17-0-4	-36	-62	---	BACK	VERT	TOTAL
AG	17-8-4	-42	-73	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9287
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47879-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Fri Sep 22 13:20:30 2017 Page 1
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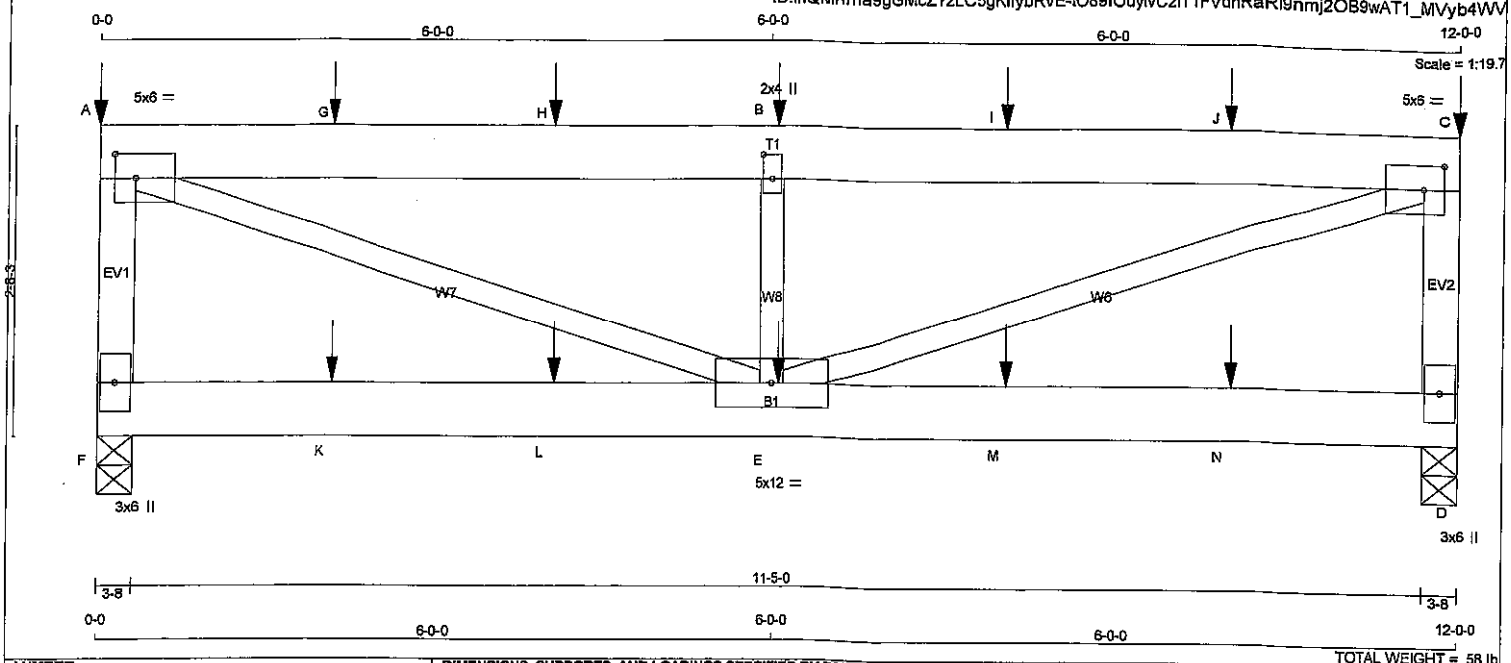

TOTAL WEIGHT = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.00
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMVW-t	MT20	5.0	6.0	2.50	2.00
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	12.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 175.7 lbs FACTORED DOWN AT 0-0, 118.4 lbs FACTORED DOWN AT 2-0-12, 118.4 lbs FACTORED DOWN AT 4-0-12, 118.4 lbs FACTORED DOWN AT 6-0-12, 118.4 lbs FACTORED DOWN AT 8-0-12, AND 118.4 lbs FACTORED DOWN AT 10-0-12, AND 175.7 lbs FACTORED DOWN AT 12-0-0 ON TOP CHORD, AND 56.0 lbs FACTORED DOWN AT 2-0-12, 56.0 lbs FACTORED DOWN AT 4-0-12, 56.0 lbs FACTORED DOWN AT 6-0-12, AND 56.0 lbs FACTORED DOWN AT 8-0-12, AND 56.0 lbs FACTORED DOWN AT 10-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1450	0	1450	0	0	3-8	3-8
D	1458	0	1456	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1121	753 / 0	186 / 0	0 / 0	0 / 0	183 / 0	0 / 0
D	1126	756 / 0	187 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-1335 / 0	0.0	0.0	0.17 (1)	6.97
A- G	-2094 / 0	-122.2	-122.2	0.48 (1)	4.94
G- H	-2094 / 0	-122.2	-122.2	0.48 (1)	4.94
H- B	-2094 / 0	-122.2	-122.2	0.48 (1)	4.94
B- I	-2094 / 0	-122.2	-122.2	0.48 (1)	4.94
I- J	-2094 / 0	-122.2	-122.2	0.48 (1)	4.94
J- C	-2094 / 0	-122.2	-122.2	0.48 (1)	4.94
D- C	-1340 / 0	0.0	0.0	0.17 (1)	6.96
F- K	0 / 0	-28.0	-28.0	0.19 (3)	10.00
K- L	0 / 0	-28.0	-28.0	0.19 (3)	10.00
L- E	0 / 0	-28.0	-28.0	0.19 (3)	10.00
E- M	0 / 0	-28.0	-28.0	0.19 (3)	10.00
M- N	0 / 0	-28.0	-28.0	0.19 (3)	10.00
N- D	0 / 0	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- C	0 / 2233	0.55 (1)
A- E	0 / 2233	0.55 (1)
E- B	-1287 / 0	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-176	-176	—	FRONT	VERT	TOTAL
B	6-0-12	-118	-118	—	FRONT	VERT	TOTAL
C	12-0-0	-176	-176	—	FRONT	VERT	TOTAL
E	6-0-12	-32	-56	—	FRONT	VERT	TOTAL
G	2-0-12	-118	-118	—	FRONT	VERT	TOTAL
H	4-0-12	-118	-118	—	FRONT	VERT	TOTAL
I	8-0-12	-118	-118	—	FRONT	VERT	TOTAL
J	10-0-12	-118	-118	—	FRONT	VERT	TOTAL
K	2-0-12	-32	-56	—	FRONT	VERT	TOTAL
L	4-0-12	-32	-56	—	FRONT	VERT	TOTAL
M	8-0-12	-32	-56	—	FRONT	VERT	TOTAL
N	10-0-12	-32	-56	—	FRONT	VERT	TOTAL

DWG NO. T-AM 4) 09-22-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.48 (A-B:1) , BC=0.19 (E-F:3) , WB=0.55 (C-E:1) , SSI=0.39 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.52 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROFESSIONAL ENGINEER PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288372	T24	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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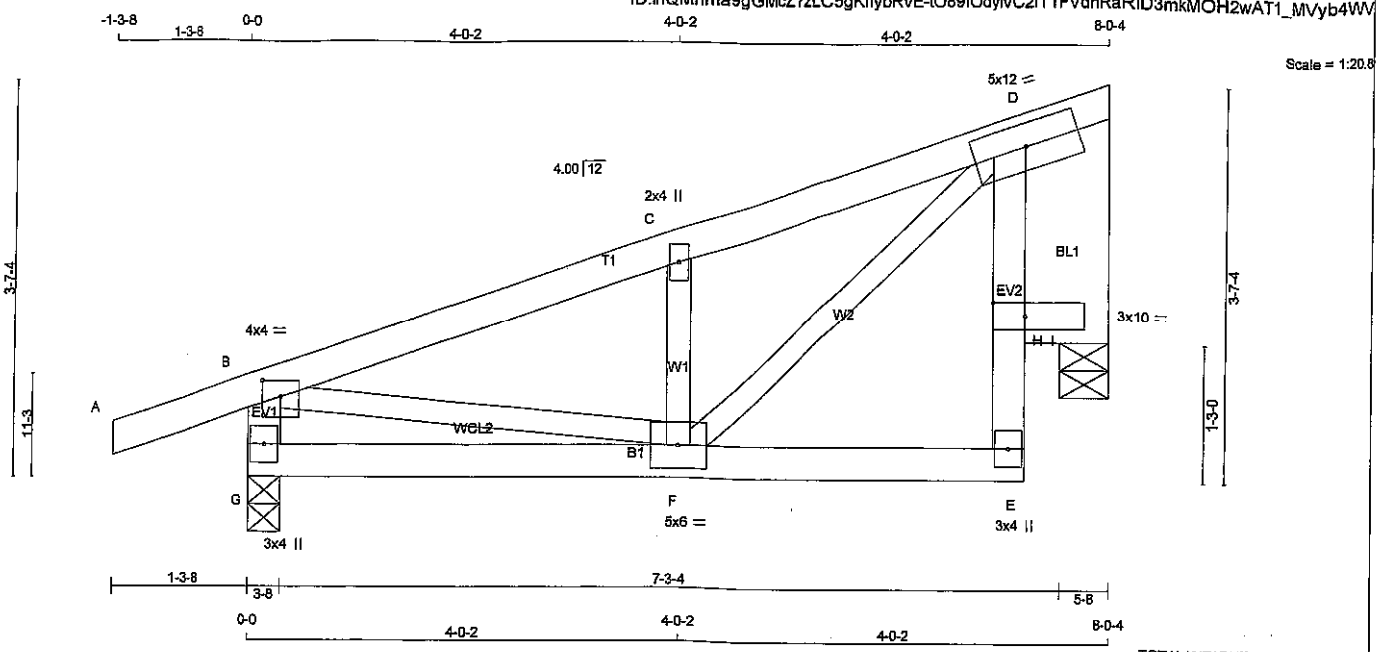


TOTAL WEIGHT = 4 X 37 = 148 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x10	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TMVWK1-t	MT20	5.0	12.0		
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	KP-p	MT20	3.0	10.0	1.50	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I(D)	507	0	507	0	0	5-8	5-8 (** SEE "BEARING NOTE" **)
G	746	0	746	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I(D)	396	258 / 0	71 / 0	0 / 0	0 / 0	67 / 0	0 / 0
G	566	399 / 0	81 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I(D), G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-122.2	-122.2	0.15 (1)	10.00
B- C	-708 / 0	-122.2	-122.2	0.21 (1)	6.25
C- D	-704 / 0	-122.2	-122.2	0.20 (1)	6.25
E- H	0 / 56	0.0	0.0	0.14 (1)	10.00
H- D	0 / 56	0.0	0.0	0.14 (1)	10.00
G- B	-694 / 0	0.0	0.0	0.07 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.10 (3)	10.00
F- E	0 / 90	-28.0	-28.0	0.11 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-534 / 0	0.08 (1)
F- D	0 / 780	0.18 (1)
B- F	0 / 693	0.16 (1)
D- I	-526 / 0	0.03 (1)
H- I	0 / 141	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.21 (B-C:1) , BC=0.11 (E-F:3) , WB=0.18 (D-F:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

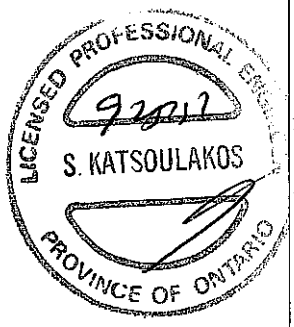
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)



DWG NO. TAM 47881-17
STRUCTURAL
COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

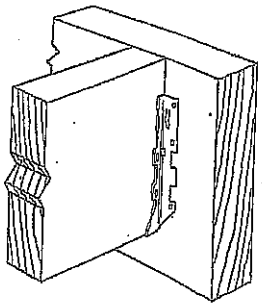
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

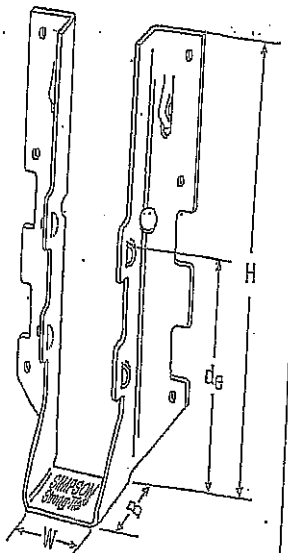
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



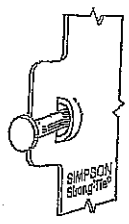
Typical LUS Installation



LUS28

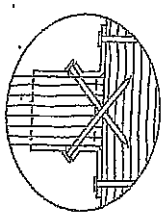
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_g[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⅜	3⅛	1¾	1⁵⁄₁₆	4-10d	2-10d	710	1630	645	1165
LUS24-2	18	3⅛	3⅛	2	1⁵⁄₁₆	4-16d	2-16d	835	2020	590	1435
LUS26	18	1⅜	4¾	1¾	3⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1⅜	6⅝	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2376
LUS210	18	1⅜	7⁹⁄₁₆	1¾	3⅞	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2580	4500	2320	3196
LUS210-3	18	4⅝	8⅝	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

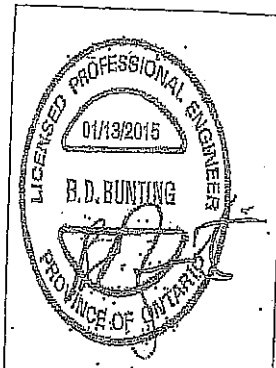


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



This technical bulletin is effective until December 31, 2016, and contains information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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HUS/LJS - Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

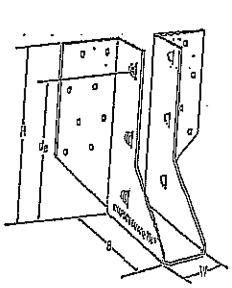
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

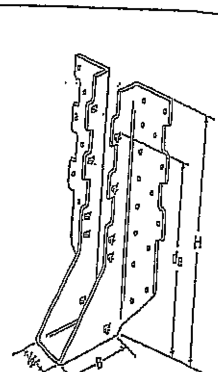
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

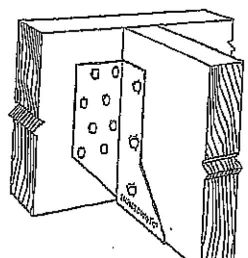
- See current catalogue for options



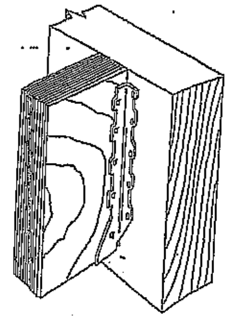
LJS26DS



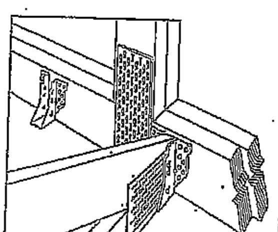
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



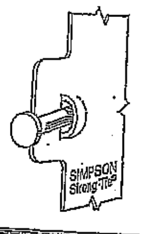
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

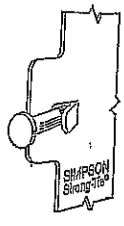
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1 9/16	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	22-16d	8-16d	3605	5365	2875	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

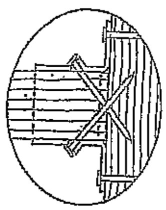


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

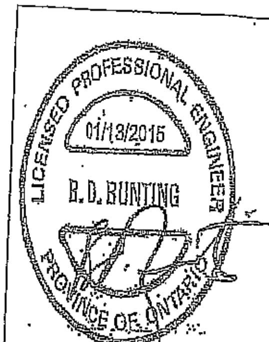
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

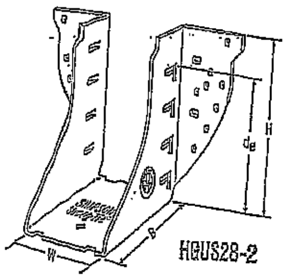
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

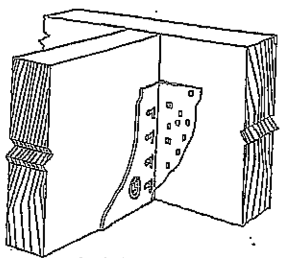
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

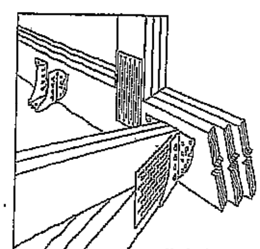
OPTIONS:

- See current catalogue for options.



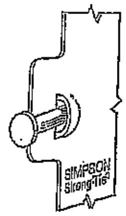
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

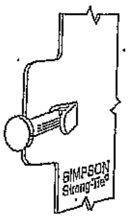
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⁹⁄₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6356
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7⁹⁄₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⁹⁄₁₆	7⁹⁄₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⁵⁄₁₆	9⁹⁄₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⁹⁄₁₆	9⁹⁄₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

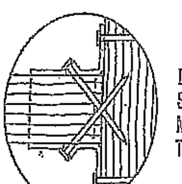


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
01/13/2015
B. D. BUNTING
PROVINCE OF ONTARIO

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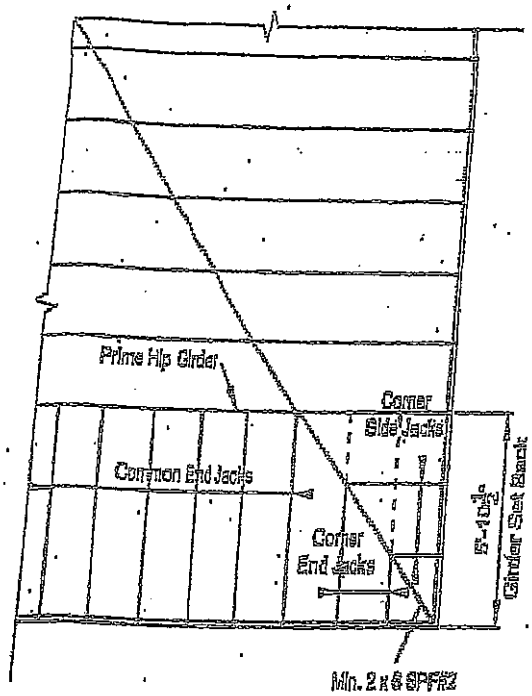
MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

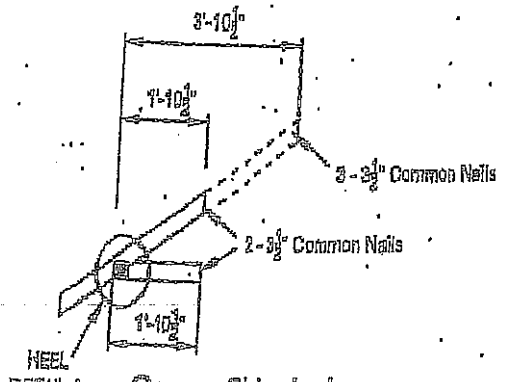
TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

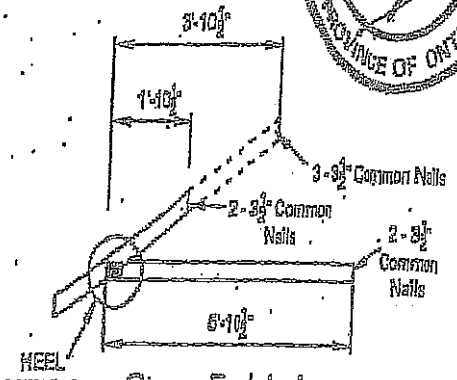
TOP CHORD LIVE LOAD	: 39.4 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F
TOTAL LOAD	59.9 P.S.F.

DWG NO TIM2495.14
STRUCTURAL COMPONENT ONLY

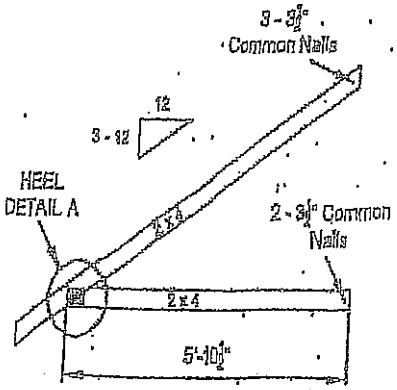
LICENSED PROFESSIONAL ENGINEER
S. KOTSOULAKOS
PROVINCE OF ONTARIO
1-29-14



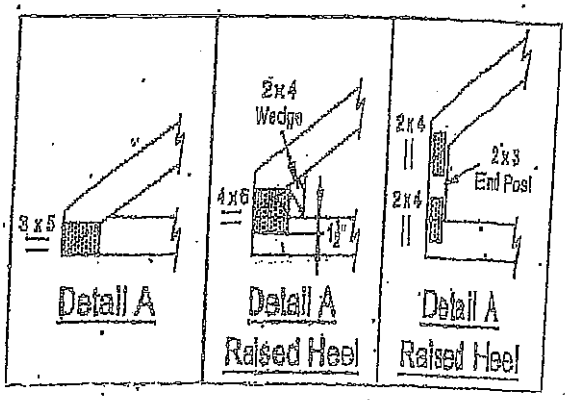
Corner Side Jacks



Corner End Jacks



Common End Jacks

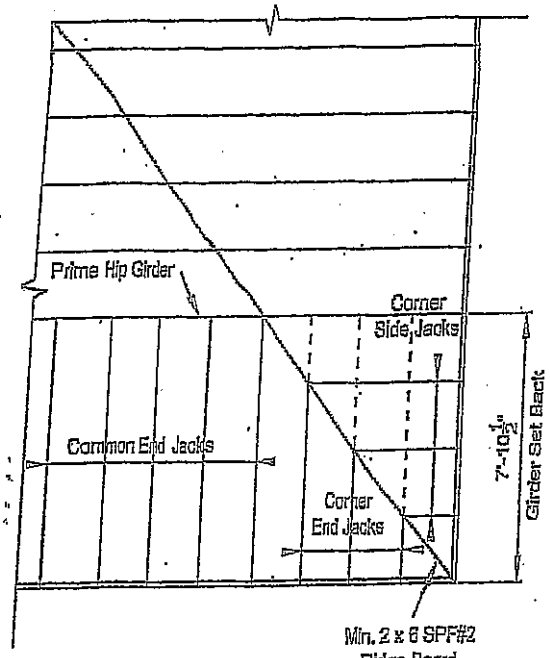


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO UNIT
ENGINEERING SERVICES INC. TEL: (519) 287-2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



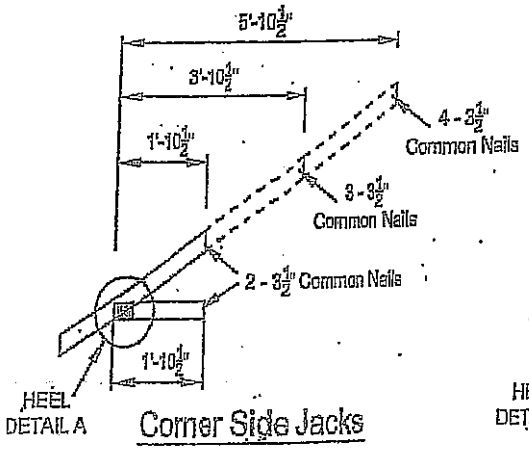
45° Hip End

DESIGN LOADS:

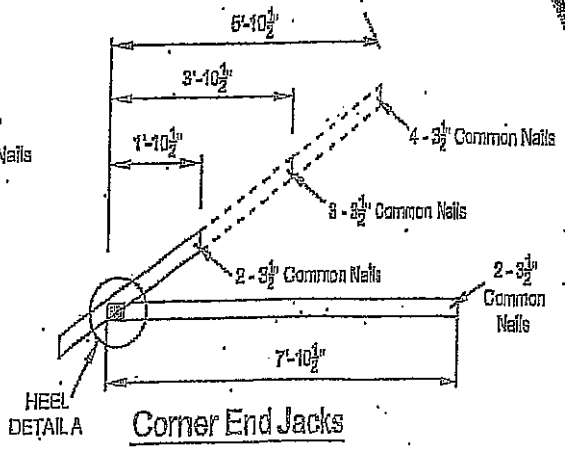
TOP CHORD LIVE LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F.

DVG NO TRN 350314
STRUCTURAL COMPONENT ONLY

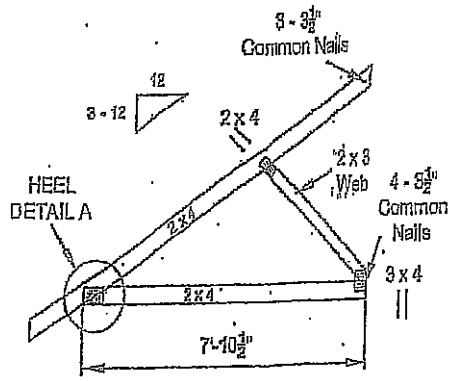
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO



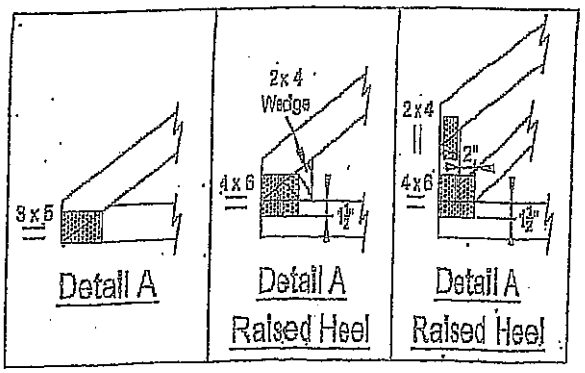
Corner Side Jacks



Corner End Jacks



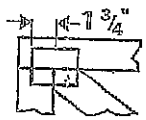
Common End Jacks



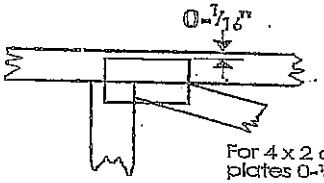
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

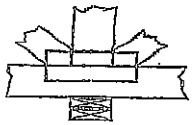
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

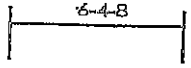
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

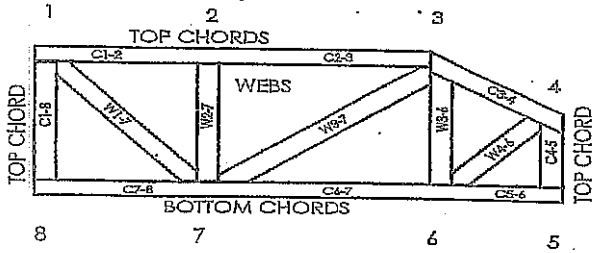
DSB-89: *Design Standard for Bracing.*

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek®

POWER TO PERFORM.™

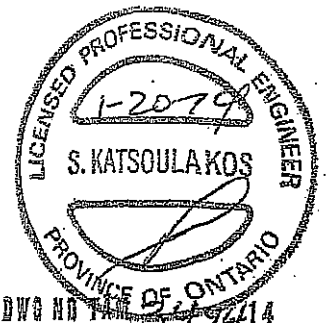
MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



DWG NO T[illegible]-[illegible]-14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.