ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

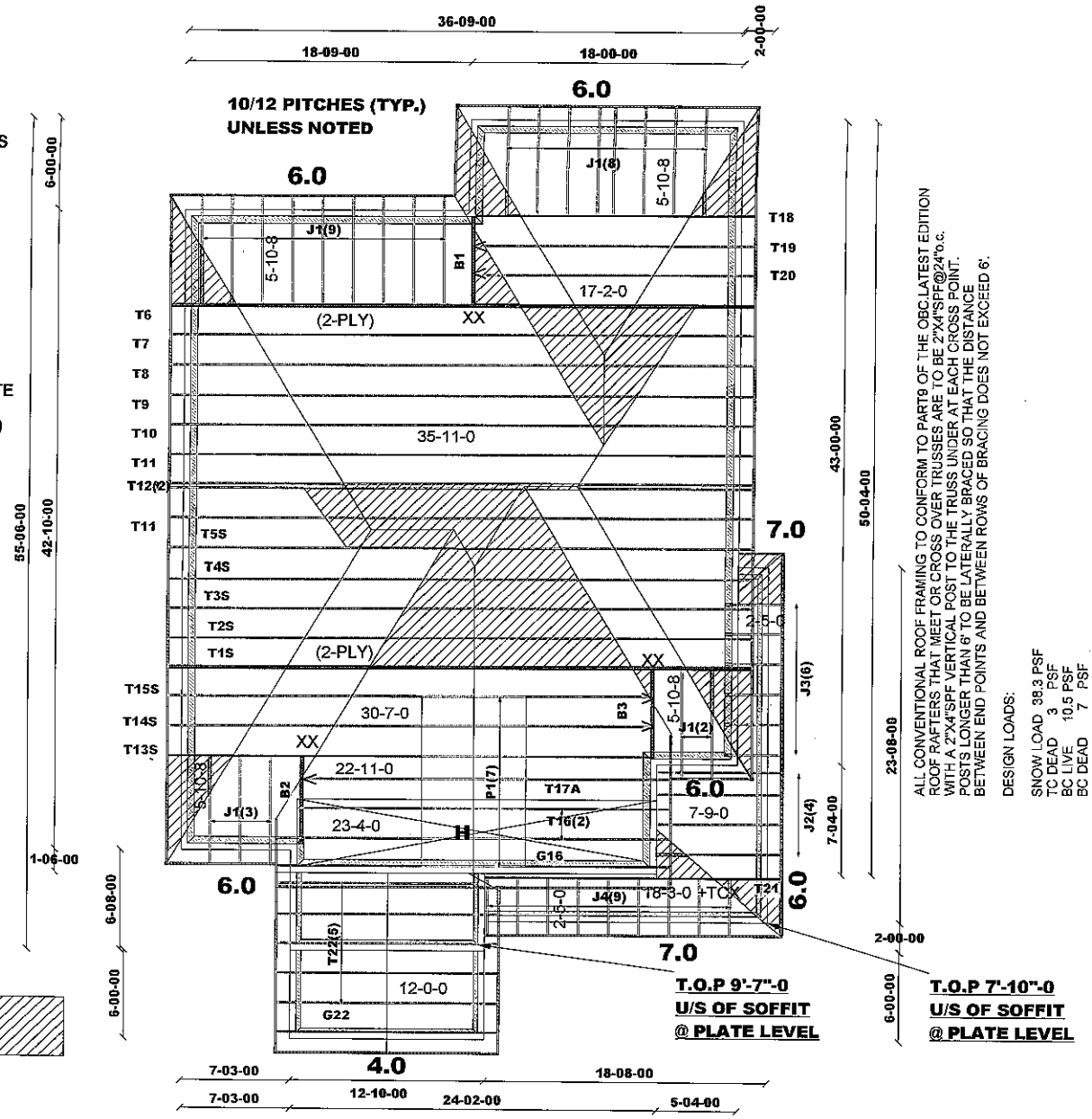
HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

DENOTE:

H - 1'-6"-0 HIGHER PLATE

ALL B-2-2X10 (FLUSH)



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC,LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T-170681

DENOTES:
CONVENTIONAL
FRAMING

Town of Innisfil Certified Model
03/01/2018 2:32:21 PM kgervais

M11,044

TAMARACK LUMBER INC.	Job Track: **42067**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **S45-2 HUMMINGBIRD 2/ A**
	Layout ID: **288285**	Project: **ALCONA SHORES**	
	Plan Log: **94324**	Date: **9/20/2017** Designer: **sonny**	

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

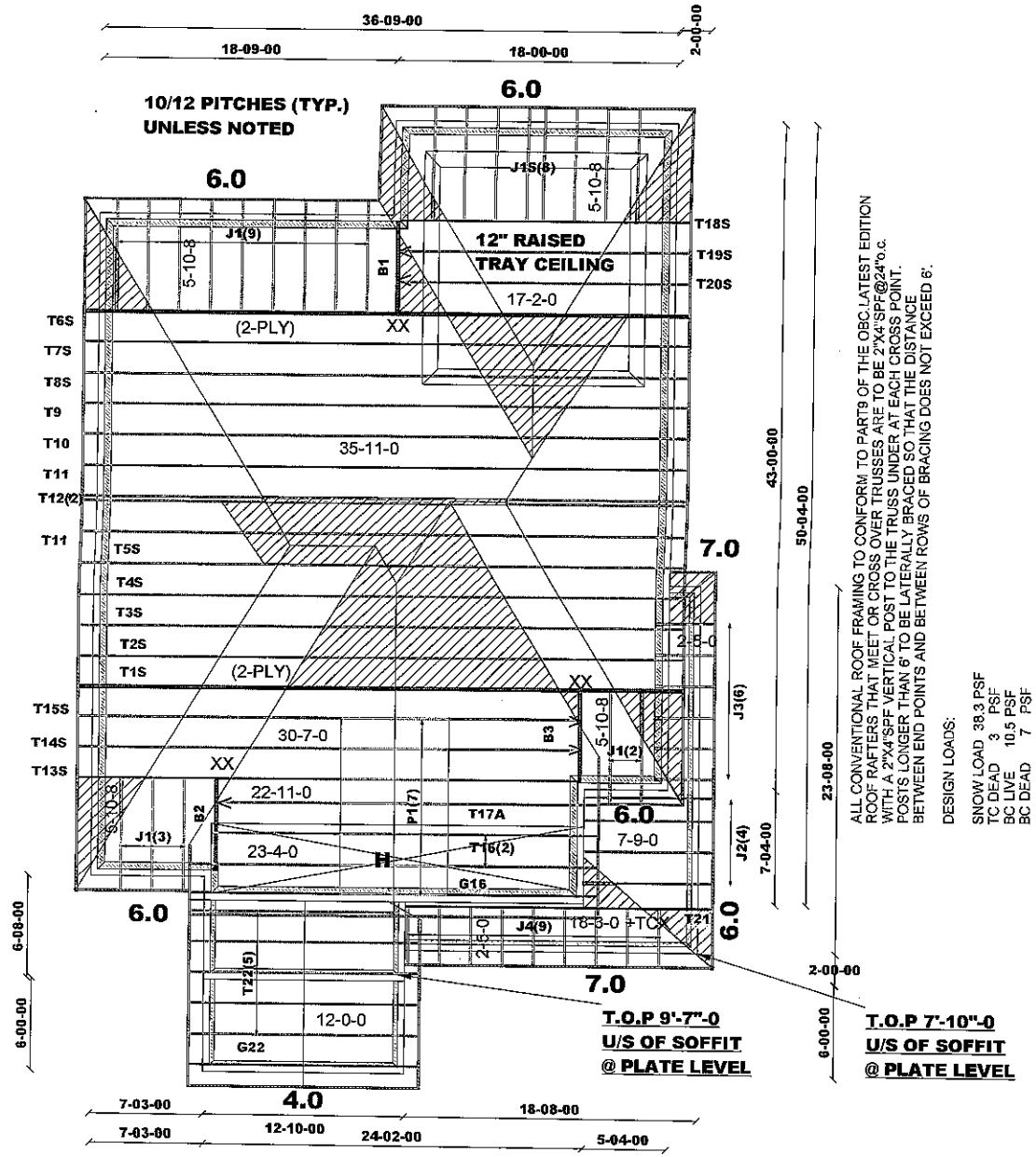
HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

DENOTE:

H - 1'-6"-0 HIGHER PLATE

ALL B-2-2X10 (FLUSH)



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T-170681

DENOTES:
CONVENTIONAL
FRAMING

mil,044

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	288283
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 9/20/2017 Designer: sonny

Model / Elevation: S45-2 HUMMINGBIRD 2/ A+OPT.COFF.

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

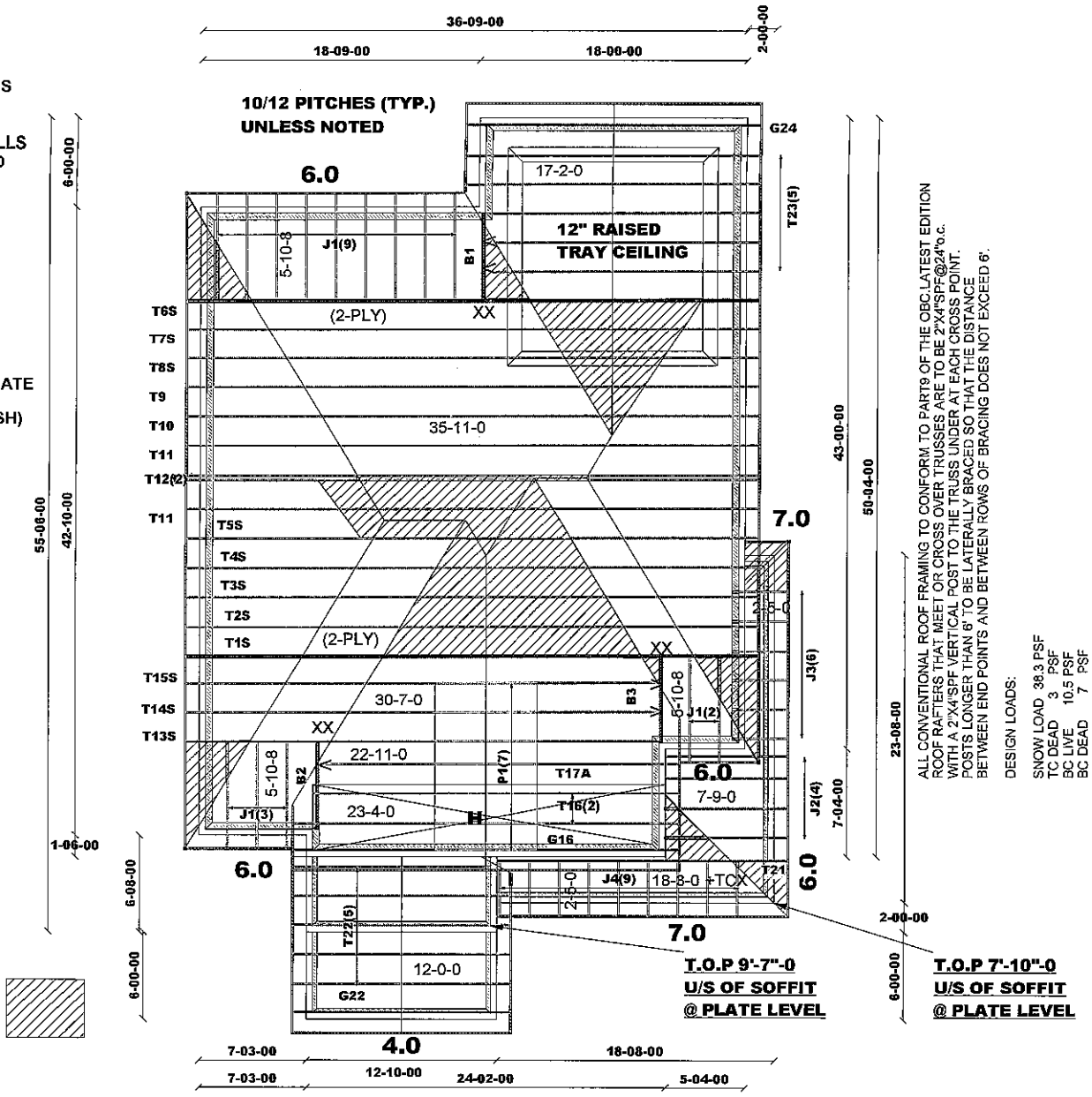
HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

DENOTE:

H - 1'-6"-0 HIGHER PLATE

ALL B-2-2X10 (FLUSH)



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T-170681

DENOTES:
CONVENTIONAL
FRAMING

mil.044

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S45-2 HUMMINGBIRD 2/ A (REAR UPGRADE) +OPT. COFF.
	Layout ID: 288286	Project: ALCONA SHORES	
	Plan Log: 94324	Date: 9/20/2017 Designer: sonny	

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

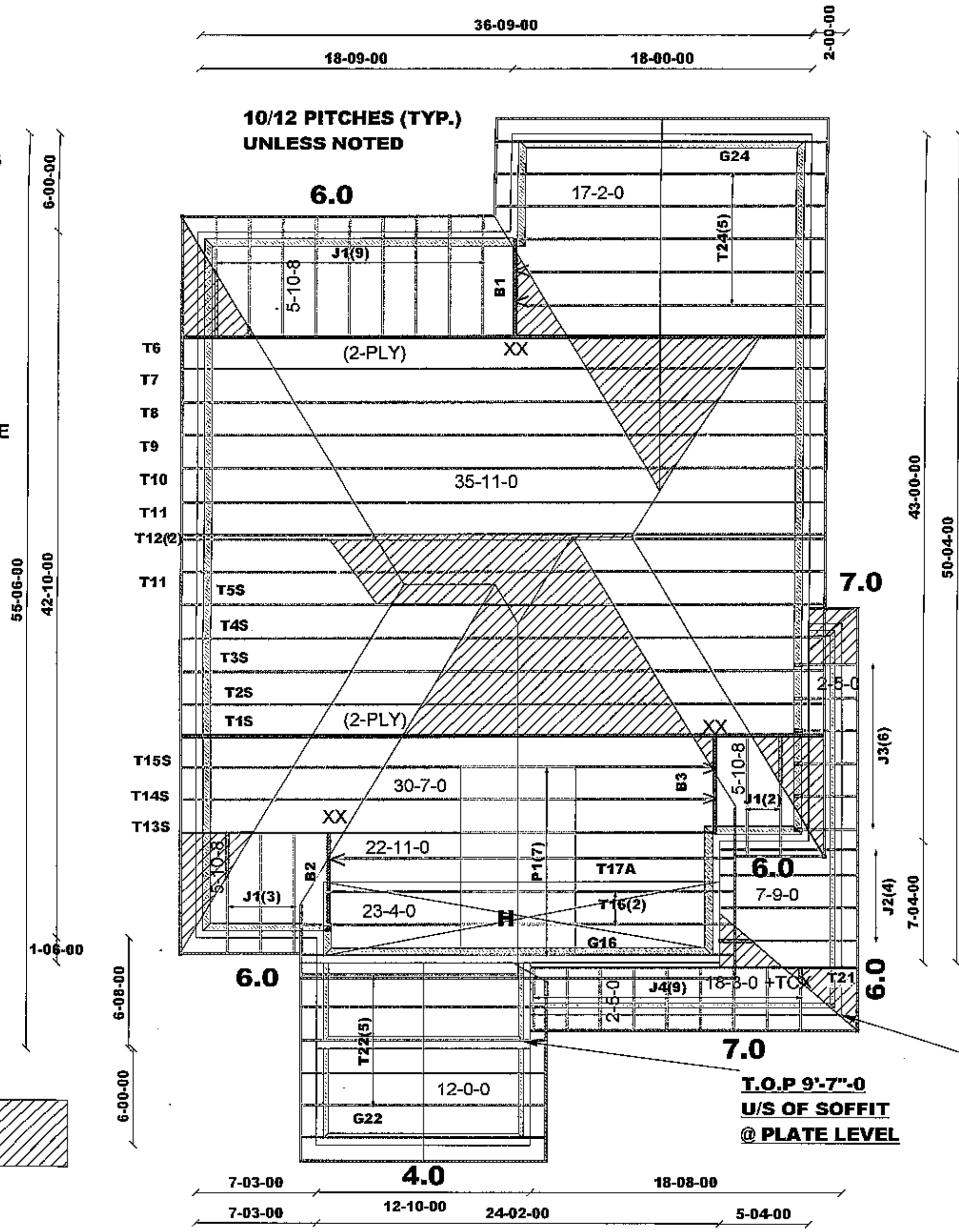
HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

DENOTE:

H - 1'-6"-0 HIGHER PLATE

ALL B-2-2X10 (FLUSH)



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T-170681

DENOTES:
CONVENTIONAL
FRAMING

MI.044

TAMARACK LUMBER INC.	Job Track: **42067**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **S45-2 HUMMINGBIRD 2/A(REAR UPGRADE)**
	Layout ID: **288288**	Project: **ALCONA SHORES**	
	Plan Log: **94324**	Date: 9/20/2017 Designer: sonny	

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

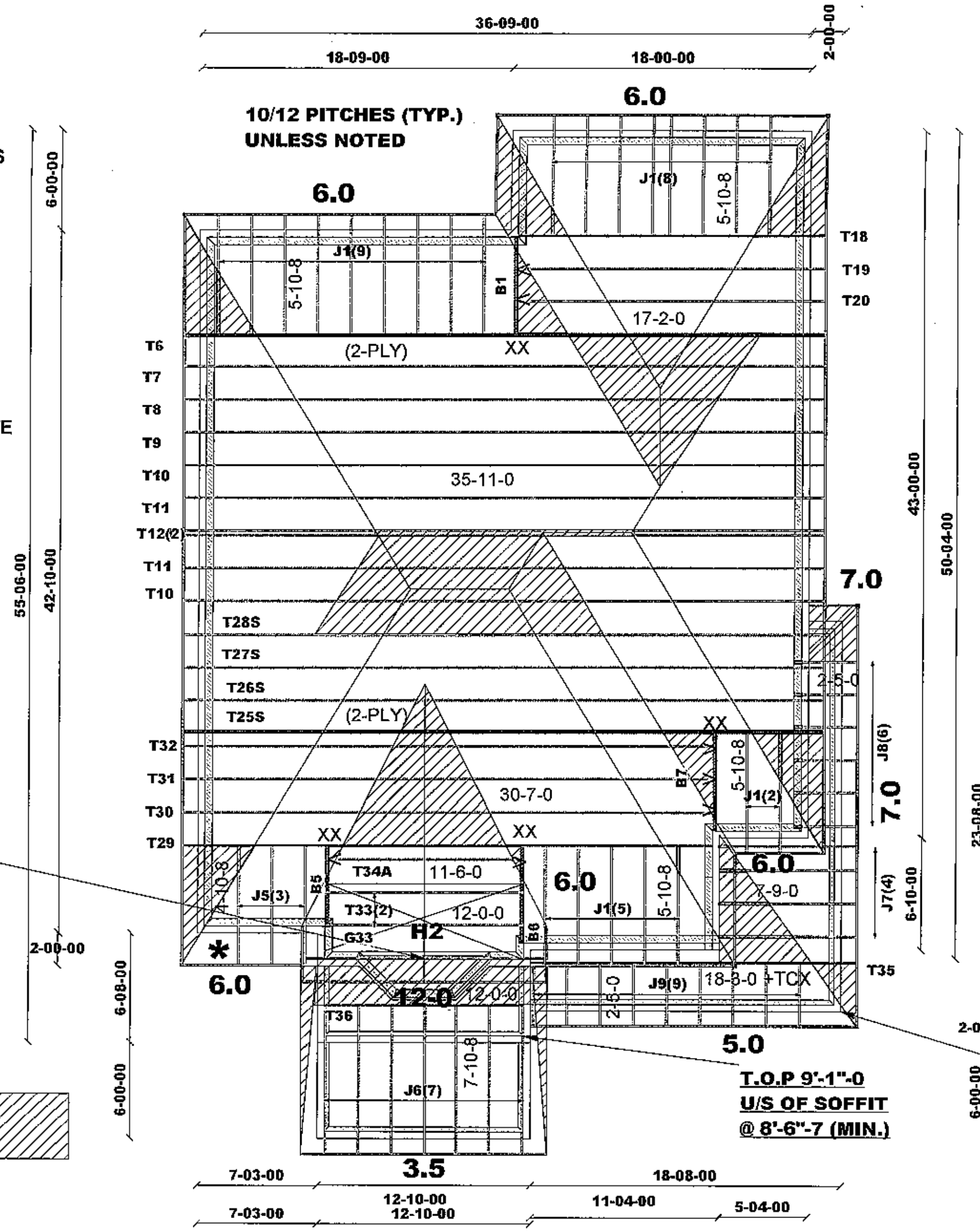
DENOTE:

H2 - 1'-2"-0 HIGHER PLATE

ALL B-2-2X10 (FLUSH)

* -24" FIN. O.H

NAIL 2-2X10
TO G33 FOR
CONTINUOS BRG.



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD	38.3	PSF
TC DEAD	3	PSF
BC LIVE	10.5	PSF
BC DEAD	7	PSF

T.O.P 7'-10"-0
U/S OF SOFFIT
@ PLATE LEVEL

T-170681

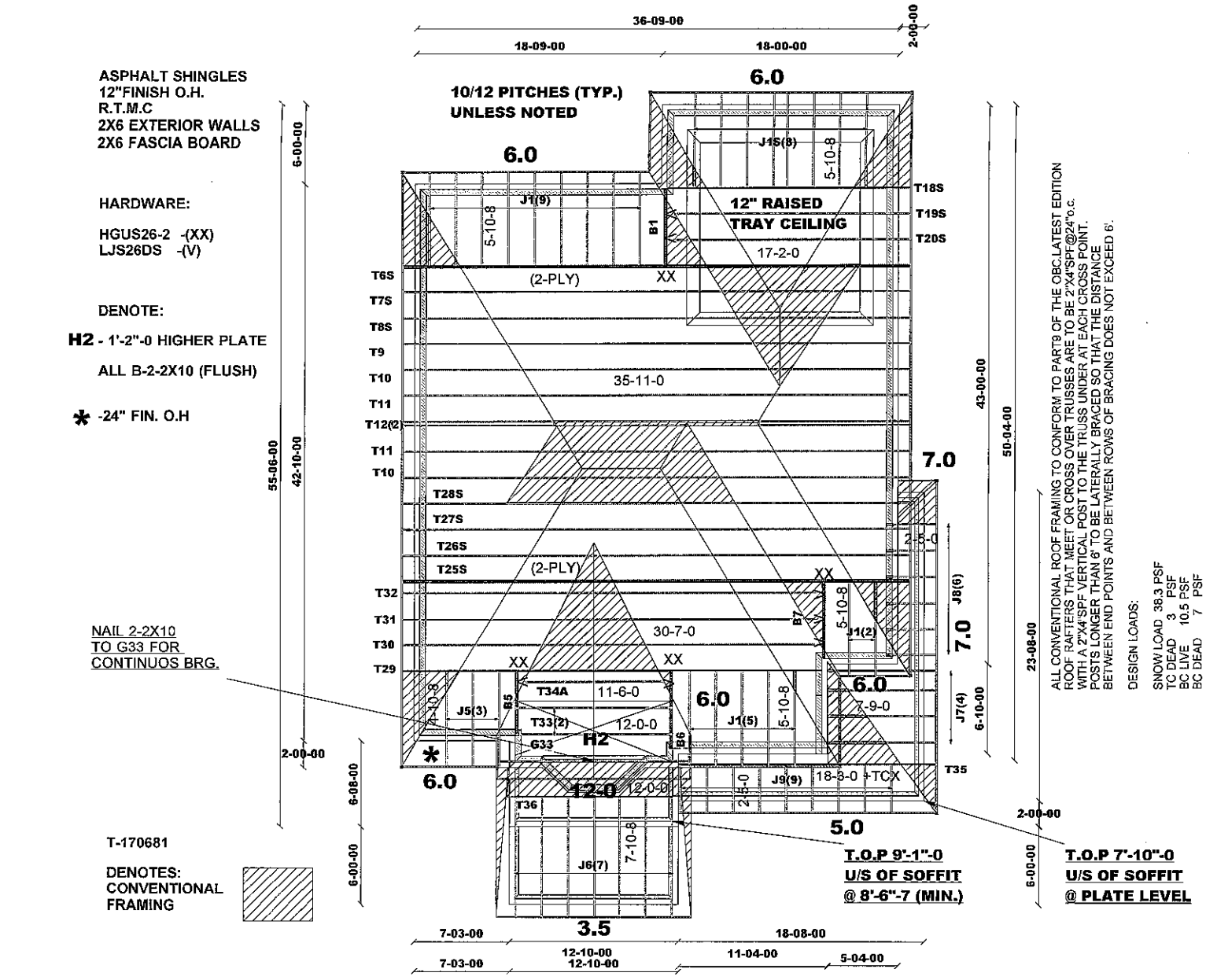
DENOTES:
CONVENTIONAL
FRAMING

mil:044

TAMARACK LUMBER INC.

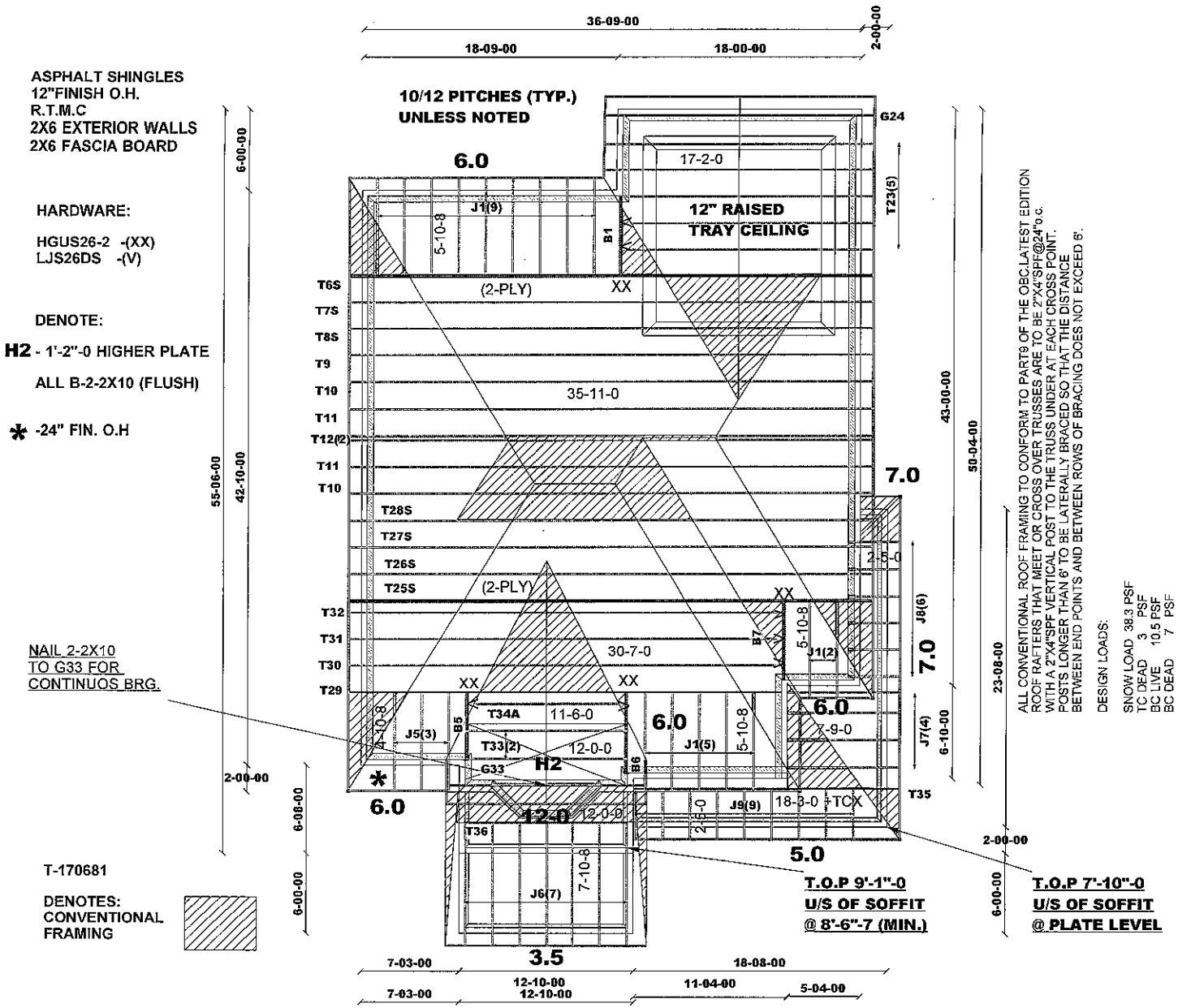
Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL		Model / Elevation: S45-2 HUMMINGBIRD 2/ B
Layout ID: 288292	Project: ALCONA SHORES		
Plan Log: 94324	Date: 9/21/2017	Designer: sonny	

Mitek ver 7.5.0



TAMARACK	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S45-2 HUMMINGBIRD 2/ B+OPT.COFF.
	Layout ID: 288291	Project: ALCONA SHORES	
	Plan Log: 94324	Date: 9/20/2017 Designer: sonny	

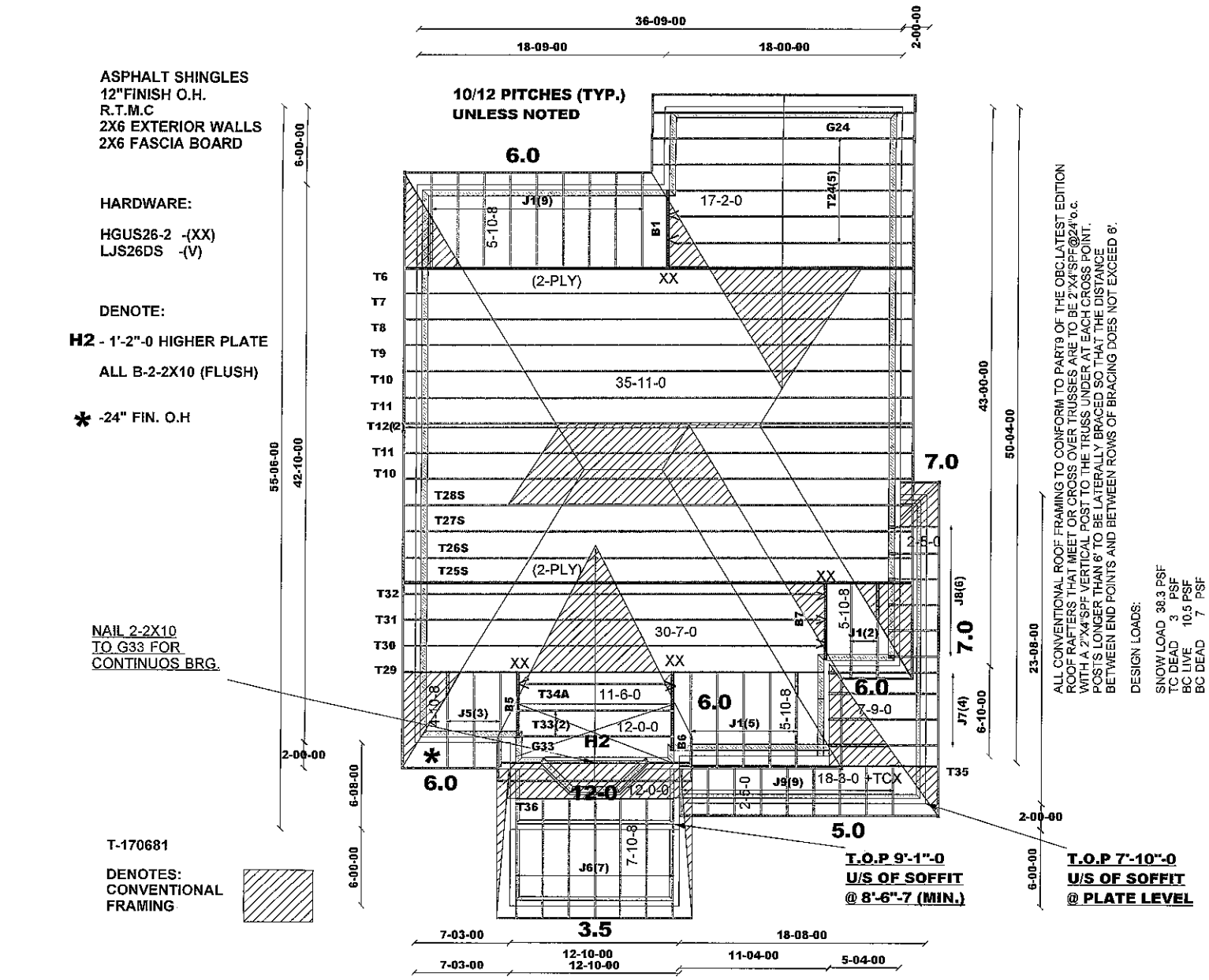
Mitek ver 7.5.0



M11,044

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S45-2 HUMMINGBIRD 2/ B (REAR UPGRADE) +OPT. COFF.
	Layout ID: 288293	Project: ALCONA SHORES	
	Plan Log: 94324	Date: 9/21/2017 Designer: sonny	

Mitek ver 7.5.0



mil,044

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S45-2 HUMMINGBIRD 2/ B (REAR UPGRADE)
	Layout ID: 288294	Project: ALCONA SHORES	
	Plan Log: 94324	Date: 9/21/2017 Designer: sonny	

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

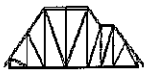
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288285 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD2 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1S ROOF	10.00 0.00	35-11-00	07-00-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	483.92 294.00		
	1	T2S ROOF	10.00 0.00	35-11-00	08-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	172.02 109.33		
	1	T3S ROOF	10.00 0.00	35-11-00	09-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	182.30 115.33		
	1	T4S ROOF	10.00 0.00	35-11-00	10-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	191.09 121.17		
	1	T5S ROOF	10.00 0.00	35-11-00	11-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	211.05 131.67		
	1 2 Ply	T6 HIP GIRDER	10.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	420.02 254.68		
	1	T7 HIP	10.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	156.88 99.83		
	1	T8 HIP	10.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	159.84 101.33		
	1	T9 HIP	10.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	165.55 103.50		
	1	T10 HIP	10.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.41 110.00		
	2	T11 HIP	10.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	361.74 226.66		
	2	T12 HIP	10.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	388.46 242.00		
	1	T13S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-07-11 03-01-11	233.96 143.34		
	1	T14S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-11	160.07 99.83		
	1	T15S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-11	152.52 96.33		
	2	T16 PIGGYBACK	10.00 0.00	23-04-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	224.42 141.34		
	1	G16 GABLE	10.00 0.00	23-04-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.81 78.67		
	1	T17A PIGGYBACK	10.00 0.00	22-11-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	03-01-11 03-05-14	121.47 76.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

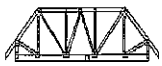
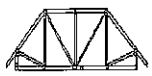
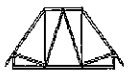
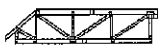
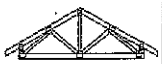
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288285 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD2 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T18 HIP GIRDER	10.00 0.00	17-02-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	88.75 56.67		
	1	T19 HIP	10.00 0.00	17-02-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	76.77 50.83		
	1	T20 HIP	10.00 0.00	17-02-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	77.72 50.33		
	1	T21 HALF HIP	6.00 0.00	18-03-00	02-08-06	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-08-06	79.44 50.67		
	5	T22 COMMON	4.00 0.00	12-00-00	02-11-03	2 X 4	2 X 4	01-03-08 01-03-08	00-11-03 00-11-03	225.90 145.85		
	1	G22 GABLE	4.00 0.00	12-00-00	02-11-03	2 X 4	2 X 4	01-03-08 01-03-08	00-11-03 00-11-03	39.53 26.17		
	7	P1 PIGGYBACK	10.00 0.00	05-07-05	02-08-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	127.96 85.19		
	22	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	369.38 234.74		
	4	J2 JACK TRUSS	6.00 0.00	06-11-00	05-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-00-08	116.68 77.32		
	6	J3 JACK	6.00 0.00	02-05-00	02-04-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-06-11	61.56 45.00		
	9	J4 JACK-OPEN	7.00 0.00	02-05-00	02-08-06	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 02-08-06	79.11 54.00		

TOTAL # TRUSS= **81.00** TOTAL BFT OF ALL TRUSSES= **3422.61** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5427.33** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **8.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

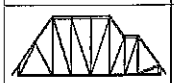
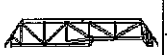
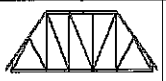
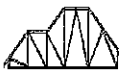
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288283 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: A+ OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S ROOF	10.00 0.00	35-11-00	07-00-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	483.92 294.00		
	1	T2S ROOF	10.00 0.00	35-11-00	08-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	172.02 109.33		
	1	T3S ROOF	10.00 0.00	35-11-00	09-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	182.30 115.33		
	1	T4S ROOF	10.00 0.00	35-11-00	10-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	191.09 121.17		
	1	T5S ROOF	10.00 0.00	35-11-00	11-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	211.05 131.67		
	1 2 Ply	T6S HIP GIRDER	10.00 12.00	35-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	425.24 268.68		
	1	T7S HIP	10.00 12.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	171.57 112.17		
	1	T8S HIP	10.00 12.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	170.78 110.50		
	1	T9 HIP	10.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	165.55 103.50		
	1	T10 HIP	10.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.41 110.00		
	2	T11 HIP	10.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	361.74 226.66		
	2	T12 HIP	10.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	388.46 242.00		
	1	T13S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-07-11 03-01-11	233.96 143.34		
	1	T14S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-11	160.07 99.83		
	1	T15S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-11	152.52 96.33		
	2	T16 PIGGYBACK	10.00 0.00	23-04-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	224.42 141.34		
	1	G16 GABLE	10.00 0.00	23-04-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.81 78.67		
	1	T17A PIGGYBACK	10.00 0.00	22-11-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	03-01-11 03-05-14	121.47 76.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

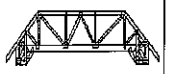
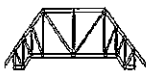
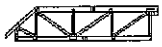
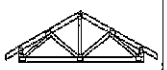
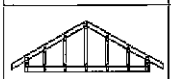
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288283 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: A+ OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T18S HIP GIRDER	10.00 0.00	17-02-00	04-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	94.53 65.00		
	1	T19S HIP	10.00 12.00	17-02-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	84.78 58.33		
	1	T20S HIP	10.00 12.00	17-02-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	85.69 58.33		
	1	T21 HALF HIP	6.00 0.00	18-03-00	02-08-06	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-08-06	79.44 50.67		
	5	T22 COMMON	4.00 0.00	12-00-00	02-11-03	2 X 4	2 X 4	01-03-08 01-03-08	00-11-03 00-11-03	225.90 145.85		
	1	G22 GABLE	4.00 0.00	12-00-00	02-11-03	2 X 4	2 X 4	01-03-08 01-03-08	00-11-03 00-11-03	39.53 26.17		
	7	P1 PIGGYBACK	10.00 0.00	05-07-05	02-08-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	127.96 85.19		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	8	J1S JACK-OPEN	6.00 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	184.48 132.00		
	4	J2 JACK TRUSS	6.00 0.00	06-11-00	05-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-00-08	116.68 77.32		
	6	J3 JACK	6.00 0.00	02-05-00	02-04-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-06-11	61.56 45.00		
	9	J4 JACK-OPEN	7.00 0.00	02-05-00	02-08-06	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 02-08-06	79.11 54.00		

TOTAL # TRUSS= **81.00** TOTAL BFT OF ALL TRUSSES= **3528.59** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5530.10** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **8.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

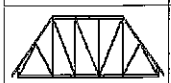
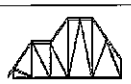
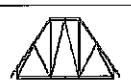
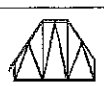
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288286 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: A REAR UPGRADE +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S ROOF	10.00 0.00	35-11-00	07-00-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	483.92 294.00		
	1	T2S ROOF	10.00 0.00	35-11-00	08-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	172.02 109.33		
	1	T3S ROOF	10.00 0.00	35-11-00	09-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	182.30 115.33		
	1	T4S ROOF	10.00 0.00	35-11-00	10-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	191.09 121.17		
	1	T5S ROOF	10.00 0.00	35-11-00	11-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	211.05 131.67		
	1 2 Ply	T6S HIP GIRDER	10.00 12.00	35-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	425.24 268.68		
	1	T7S HIP	10.00 12.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	171.57 112.17		
	1	T8S HIP	10.00 12.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	170.78 110.50		
	1	T9 HIP	10.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	165.55 103.50		
	1	T10 HIP	10.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.41 110.00		
	2	T11 HIP	10.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	361.74 226.66		
	2	T12 HIP	10.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	388.46 242.00		
	1	T13S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-07-11 03-01-11	233.96 143.34		
	1	T14S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-11	160.07 99.83		
	1	T15S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-11	152.52 96.33		
	2	T16 PIGGYBACK	10.00 0.00	23-04-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	224.42 141.34		
	1	G16 GABLE	10.00 0.00	23-04-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.81 78.67		
	1	T17A PIGGYBACK	10.00 0.00	22-11-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	03-01-11 03-05-14	121.47 76.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

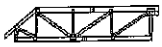
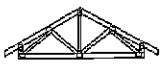
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288286 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: A REAR UPGRADE +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T21 HALF HIP	6.00 / 0.00	18-03-00	02-08-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-08-06	79.44 / 50.67		
	5	T22 COMMON	4.00 / 0.00	12-00-00	02-11-03	2 X 4	2 X 4	01-03-08 / 01-03-08	00-11-03 / 00-11-03	225.90 / 145.85		
	1	G22 GABLE	4.00 / 0.00	12-00-00	02-11-03	2 X 4	2 X 4	01-03-08 / 01-03-08	00-11-03 / 00-11-03	39.53 / 26.17		
	5	T23 ROOF	10.00 / 12.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	409.70 / 281.65		
	1	G24 COMMON	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	85.17 / 55.83		
	7	P1 PIGGYBACK	10.00 / 0.00	05-07-05	02-08-14	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	127.96 / 85.19		
	14	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	235.06 / 149.38		
	4	J2 JACK TRUSS	6.00 / 0.00	06-11-00	05-00-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-00-08	116.68 / 77.32		
	6	J3 JACK	6.00 / 0.00	02-05-00	02-04-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-06-11	61.56 / 45.00		
	9	J4 JACK-OPEN	7.00 / 0.00	02-05-00	02-08-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 02-08-06	79.11 / 54.00		

TOTAL # TRUSS= **76.00** TOTAL BFT OF ALL TRUSSES= **3552.41** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5575.49** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **8.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

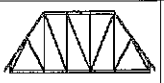
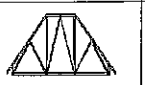
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288288 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: A REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S ROOF	10.00 0.00	35-11-00	07-00-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	483.92 294.00		
	1	T2S ROOF	10.00 0.00	35-11-00	08-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	172.02 109.33		
	1	T3S ROOF	10.00 0.00	35-11-00	09-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	182.30 115.33		
	1	T4S ROOF	10.00 0.00	35-11-00	10-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	191.09 121.17		
	1	T5S ROOF	10.00 0.00	35-11-00	11-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	211.05 131.67		
	1 2 Ply	T6 HIP GIRDER	10.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	420.02 254.68		
	1	T7 HIP	10.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	156.88 99.83		
	1	T8 HIP	10.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	159.84 101.33		
	1	T9 HIP	10.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	165.55 103.50		
	1	T10 HIP	10.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.41 110.00		
	2	T11 HIP	10.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	361.74 226.66		
	2	T12 HIP	10.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	388.46 242.00		
	1	T13S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-07-11 03-01-11	233.96 143.34		
	1	T14S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-11	160.07 99.83		
	1	T15S PIGGYBACK	10.00 0.00	30-07-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-11	152.52 96.33		
	2	T16 PIGGYBACK	10.00 0.00	23-04-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	224.42 141.34		
	1	G16 GABLE	10.00 0.00	23-04-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.81 78.67		
	1	T17A PIGGYBACK	10.00 0.00	22-11-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	03-01-11 03-05-14	121.47 76.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

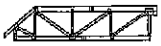
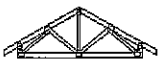
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288288 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: A REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T21 HALF HIP	6.00 0.00	18-03-00	02-08-06	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-08-06	79.44 50.67		
	5	T22 COMMON	4.00 0.00	12-00-00	02-11-03	2 X 4	2 X 4	01-03-08 01-03-08	00-11-03 00-11-03	225.90 145.85		
	1	G22 GABLE	4.00 0.00	12-00-00	02-11-03	2 X 4	2 X 4	01-03-08 01-03-08	00-11-03 00-11-03	39.53 26.17		
	5	T24 COMMON	10.00 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	394.00 259.15		
	1	G24 COMMON	10.00 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	85.17 55.83		
	7	P1 PIGGYBACK	10.00 0.00	05-07-05	02-08-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	127.96 85.19		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	4	J2 JACK TRUSS	6.00 0.00	06-11-00	05-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-00-08	116.68 77.32		
	6	J3 JACK	6.00 0.00	02-05-00	02-04-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-06-11	61.56 45.00		
	9	J4 JACK-OPEN	7.00 0.00	02-05-00	02-08-06	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 02-08-06	79.11 54.00		

TOTAL # TRUSS= **76.00** TOTAL BFT OF ALL TRUSSES= **3494.40** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5528.94** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **8.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

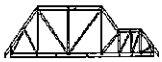
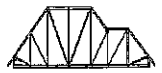
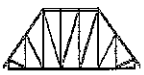
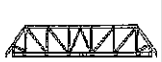
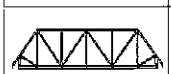
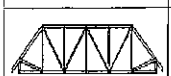
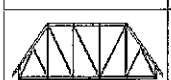
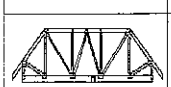
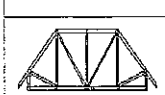
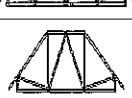
Delivery Shiplist

DATE	09/21/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288292 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T25S ROOF	10.00 0.00	35-11-00	07-06-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	467.08 280.66		
	1	T26S ROOF	10.00 0.00	35-11-00	08-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	175.69 111.50		
	1	T27S ROOF	10.00 0.00	35-11-00	09-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	181.75 113.67		
	1	T28S ROOF	10.00 0.00	35-11-00	10-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	185.91 115.83		
	1 2 Ply	T6 HIP GIRDER	10.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	420.02 254.68		
	1	T7 HIP	10.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	156.88 99.83		
	1	T8 HIP	10.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	159.83 101.33		
	1	T9 HIP	10.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	165.55 103.50		
	2	T10 HIP	10.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	348.82 220.00		
	2	T11 HIP	10.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	361.74 226.66		
	2	T12 HIP	10.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	388.46 242.00		
	1	T29 HIP GIRDER	10.00 0.00	30-07-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	186.01 112.34		
	1	T30 HIP	10.00 0.00	30-07-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	129.37 81.17		
	1	T31 HIP	10.00 0.00	30-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	137.06 87.00		
	1	T32 HIP	10.00 0.00	30-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.33 88.83		
	1	T18 HIP GIRDER	10.00 0.00	17-02-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	88.75 56.67		
	1	T19 HIP	10.00 0.00	17-02-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	76.77 50.83		
	1	T20 HIP	10.00 0.00	17-02-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	77.72 50.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

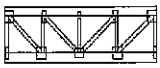
Delivery Shiplist

DATE	09/21/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288292 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T33 COMMON	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	127.14 / 82.66		
	1	G33 GABLE	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	60.98 / 39.33		
	1	T34A COMMON	12.00 / 0.00	11-06-00	09-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-08 / 03-03-08	65.70 / 42.50		
	1	T35 HALF HIP	7.00 / 0.00	18-03-00	02-00-10	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 02-00-10	78.68 / 51.00		
	1	T36 FLAT GIRDER	0.00 / 0.00	12-00-00	02-07-07	2 X 6	2 X 6	00-00-00 / 00-00-00	02-07-07 / 02-07-07	61.45 / 37.67		
	24	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	402.96 / 256.08		
	3	J5 JACK-OPEN	6.00 / 0.00	04-10-08	04-01-04	2 X 4	2 X 4	02-03-08 / 00-00-00	01-08-00 / 04-01-04	51.45 / 33.99		
	7	J6 JACK TRUSS	3.50 / 0.00	07-10-08	02-07-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-14 / 00-04-01	160.37 / 98.00		
	4	J7 JACK TRUSS	7.00 / 0.00	06-11-00	05-09-11	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 05-09-11	119.04 / 75.32		
	6	J8 JACK-OPEN	7.00 / 0.00	02-05-00	02-08-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 02-08-06	52.74 / 36.00		
	9	J9 JACK-OPEN	5.00 / 0.00	02-05-00	02-00-10	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-09 / 02-00-10	73.08 / 54.00		

TOTAL # TRUSS= **82.00** TOTAL BFT OF ALL TRUSSES= **3203.38** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5102.33** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
7	Hangers	LJS26DS	

TOTAL # ITEMS= **11.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

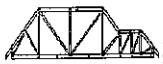
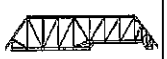
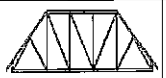
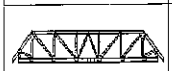
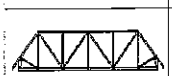
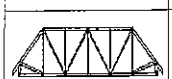
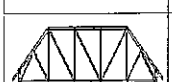
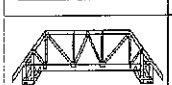
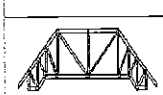
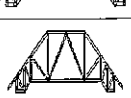
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288291 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: B +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T25S ROOF	10.00 0.00	35-11-00	07-06-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	467.08 280.66		
	1	T26S ROOF	10.00 0.00	35-11-00	08-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	175.69 111.50		
	1	T27S ROOF	10.00 0.00	35-11-00	09-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	181.75 113.67		
	1	T28S ROOF	10.00 0.00	35-11-00	10-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	185.91 115.83		
	1 2 Ply	T6S HIP GIRDER	10.00 12.00	35-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	425.24 268.68		
	1	T7S HIP	10.00 12.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	171.57 112.17		
	1	T8S HIP	10.00 12.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	170.78 110.50		
	1	T9 HIP	10.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	165.55 103.50		
	2	T10 HIP	10.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	348.82 220.00		
	2	T11 HIP	10.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	361.74 226.66		
	2	T12 HIP	10.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	388.46 242.00		
	1	T29 HIP GIRDER	10.00 0.00	30-07-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	186.01 112.34		
	1	T30 HIP	10.00 0.00	30-07-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	129.37 81.17		
	1	T31 HIP	10.00 0.00	30-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	137.06 87.00		
	1	T32 HIP	10.00 0.00	30-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.33 88.83		
	1	T18S HIP GIRDER	10.00 0.00	17-02-00	04-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	94.53 65.00		
	1	T19S HIP	10.00 12.00	17-02-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	84.78 58.33		
	1	T20S HIP	10.00 12.00	17-02-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	85.69 58.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

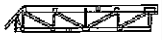
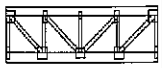
Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288291 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: B +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T33 COMMON	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	127.14 / 82.66		
	1	G33 GABLE	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	60.98 / 39.33		
	1	T34A COMMON	12.00 / 0.00	11-06-00	09-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-08 / 03-03-08	65.70 / 42.50		
	1	T35 HALF HIP	7.00 / 0.00	18-03-00	02-00-10	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 02-00-10	78.68 / 51.00		
	1	T36 FLAT GIRDER	0.00 / 0.00	12-00-00	02-07-07	2 X 6	2 X 6	00-00-00 / 00-00-00	02-07-07 / 02-07-07	61.45 / 37.67		
	16	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	268.64 / 170.72		
	8	J1S JACK-OPEN	6.00 / 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	184.48 / 132.00		
	3	J5 JACK-OPEN	6.00 / 0.00	04-10-08	04-01-04	2 X 4	2 X 4	02-03-08 / 00-00-00	01-08-00 / 04-01-04	51.45 / 33.99		
	7	J6 JACK TRUSS	3.50 / 0.00	07-10-08	02-07-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-14 / 00-04-01	160.37 / 98.00		
	4	J7 JACK TRUSS	7.00 / 0.00	06-11-00	05-09-11	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 05-09-11	119.04 / 75.32		
	6	J8 JACK-OPEN	7.00 / 0.00	02-05-00	02-08-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 02-08-06	52.74 / 36.00		
	9	J9 JACK-OPEN	5.00 / 0.00	02-05-00	02-00-10	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-09 / 02-00-10	73.08 / 54.00		

TOTAL # TRUSS= **82.00** TOTAL BFT OF ALL TRUSSES= **3309.36** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5205.11** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
7	Hangers	LJS26DS	

TOTAL # ITEMS= **11.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

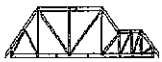
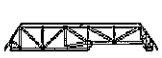
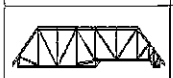
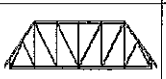
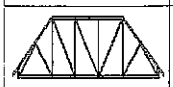
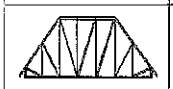
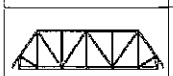
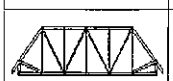
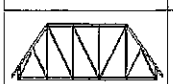
Delivery Shiplist

DATE	09/21/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288293 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES **SUB-BUILDER:**
MODEL: S45-2 HUMMINGBIRD 2 **ELEVATION:** B REAR UPGRADE +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T25S ROOF	10.00 0.00	35-11-00	07-06-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	467.08 280.66		
	1	T26S ROOF	10.00 0.00	35-11-00	08-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	175.69 111.50		
	1	T27S ROOF	10.00 0.00	35-11-00	09-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	181.75 113.67		
	1	T28S ROOF	10.00 0.00	35-11-00	10-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	185.91 115.83		
	1 2 Ply	T6S HIP GIRDER	10.00 12.00	35-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	425.24 268.68		
	1	T7S HIP	10.00 12.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	171.57 112.17		
	1	T8S HIP	10.00 12.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	170.78 110.50		
	1	T9 HIP	10.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	165.55 103.50		
	2	T10 HIP	10.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	348.82 220.00		
	2	T11 HIP	10.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	361.74 226.66		
	2	T12 HIP	10.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	388.46 242.00		
	1	T29 HIP GIRDER	10.00 0.00	30-07-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	186.01 112.34		
	1	T30 HIP	10.00 0.00	30-07-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	129.37 81.17		
	1	T31 HIP	10.00 0.00	30-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	137.06 87.00		
	1	T32 HIP	10.00 0.00	30-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.33 88.83		
	5	T23 ROOF	10.00 12.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	409.70 281.65		
	1	G24 COMMON	10.00 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	85.17 55.83		
	2	T33 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	127.14 82.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

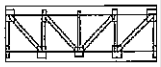
Delivery Shiplist

DATE	09/21/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288293 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: B REAR UPGRADE +OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G33 GABLE	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.98 39.33		
	1	T34A COMMON	12.00 0.00	11-06-00	09-00-08	2 X 4	2 X 4	00-00-00 00-00-00	03-03-08 03-03-08	65.70 42.50		
	1	T35 HALF HIP	7.00 0.00	18-03-00	02-00-10	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 02-00-10	78.68 51.00		
	1	T36 FLAT GIRDER	0.00 0.00	12-00-00	02-07-07	2 X 6	2 X 6	00-00-00 00-00-00	02-07-07 02-07-07	61.45 37.67		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	3	J5 JACK-OPEN	6.00 0.00	04-10-08	04-01-04	2 X 4	2 X 4	02-03-08 00-00-00	01-08-00 04-01-04	51.45 33.99		
	7	J6 JACK TRUSS	3.50 0.00	07-10-08	02-07-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-14 00-04-01	160.37 98.00		
	4	J7 JACK TRUSS	7.00 0.00	06-11-00	05-09-11	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 05-09-11	117.24 75.32		
	6	J8 JACK-OPEN	7.00 0.00	02-05-00	02-08-06	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 02-08-06	52.74 36.00		
	9	J9 JACK-OPEN	5.00 0.00	02-05-00	02-00-10	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-00-10	73.08 54.00		

TOTAL # TRUSS= **77.00** TOTAL BFT OF ALL TRUSSES= **3333.18** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5248.70** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
7	Hangers	LJS26DS	

TOTAL # ITEMS= **11.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

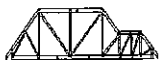
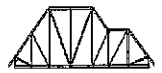
Delivery Shiplist

DATE	09/21/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288294 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-2 HUMMINGBIRD 2 ELEVATION: B REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T25S ROOF	10.00 0.00	35-11-00	07-06-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	467.08 280.66		
	1	T26S ROOF	10.00 0.00	35-11-00	08-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	175.69 111.50		
	1	T27S ROOF	10.00 0.00	35-11-00	09-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	181.75 113.67		
	1	T28S ROOF	10.00 0.00	35-11-00	10-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	185.91 115.83		
	1 2 Ply	T6 HIP GIRDER	10.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	420.02 254.68		
	1	T7 HIP	10.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	156.88 99.83		
	1	T8 HIP	10.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	159.83 101.33		
	1	T9 HIP	10.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	165.55 103.50		
	2	T10 HIP	10.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	348.82 220.00		
	2	T11 HIP	10.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	361.74 226.66		
	2	T12 HIP	10.00 0.00	35-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	388.46 242.00		
	1	T29 HIP GIRDER	10.00 0.00	30-07-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	186.01 112.34		
	1	T30 HIP	10.00 0.00	30-07-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	129.37 81.17		
	1	T31 HIP	10.00 0.00	30-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	137.06 87.00		
	1	T32 HIP	10.00 0.00	30-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.33 88.83		
	5	T24 COMMON	10.00 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	394.00 259.15		
	1	G24 COMMON	10.00 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	85.17 55.83		
	2	T33 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	127.14 82.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

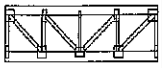
Delivery Shiplist

DATE	09/21/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288294 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORE **SUB-BUILDER:**
MODEL: S45-2 HUMMINGBIRD 2 **ELEVATION:** B REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G33 GABLE	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.98 39.33		
	1	T34A COMMON	12.00 0.00	11-06-00	09-00-08	2 X 4	2 X 4	00-00-00 00-00-00	03-03-08 03-03-08	65.70 42.50		
	1	T35 HALF HIP	7.00 0.00	18-03-00	02-00-10	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 02-00-10	78.68 51.00		
	1	T36 FLAT GIRDER	0.00 0.00	12-00-00	02-07-07	2 X 6	2 X 6	00-00-00 00-00-00	02-07-07 02-07-07	61.45 37.67		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	3	J5 JACK-OPEN	6.00 0.00	04-10-08	04-01-04	2 X 4	2 X 4	02-03-08 00-00-00	01-08-00 04-01-04	51.45 33.99		
	7	J6 JACK TRUSS	3.50 0.00	07-10-08	02-07-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-14 00-04-01	160.37 98.00		
	4	J7 JACK TRUSS	7.00 0.00	06-11-00	05-09-11	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 05-09-11	117.24 75.32		
	6	J8 JACK-OPEN	7.00 0.00	02-05-00	02-08-06	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 02-08-06	52.74 36.00		
	9	J9 JACK-OPEN	5.00 0.00	02-05-00	02-00-10	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-00-10	73.08 54.00		

TOTAL # TRUSS= **77.00** TOTAL BFT OF ALL TRUSSES= **3275.17** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5202.14** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
7	Hangers	LJS26DS	

TOTAL # ITEMS= **11.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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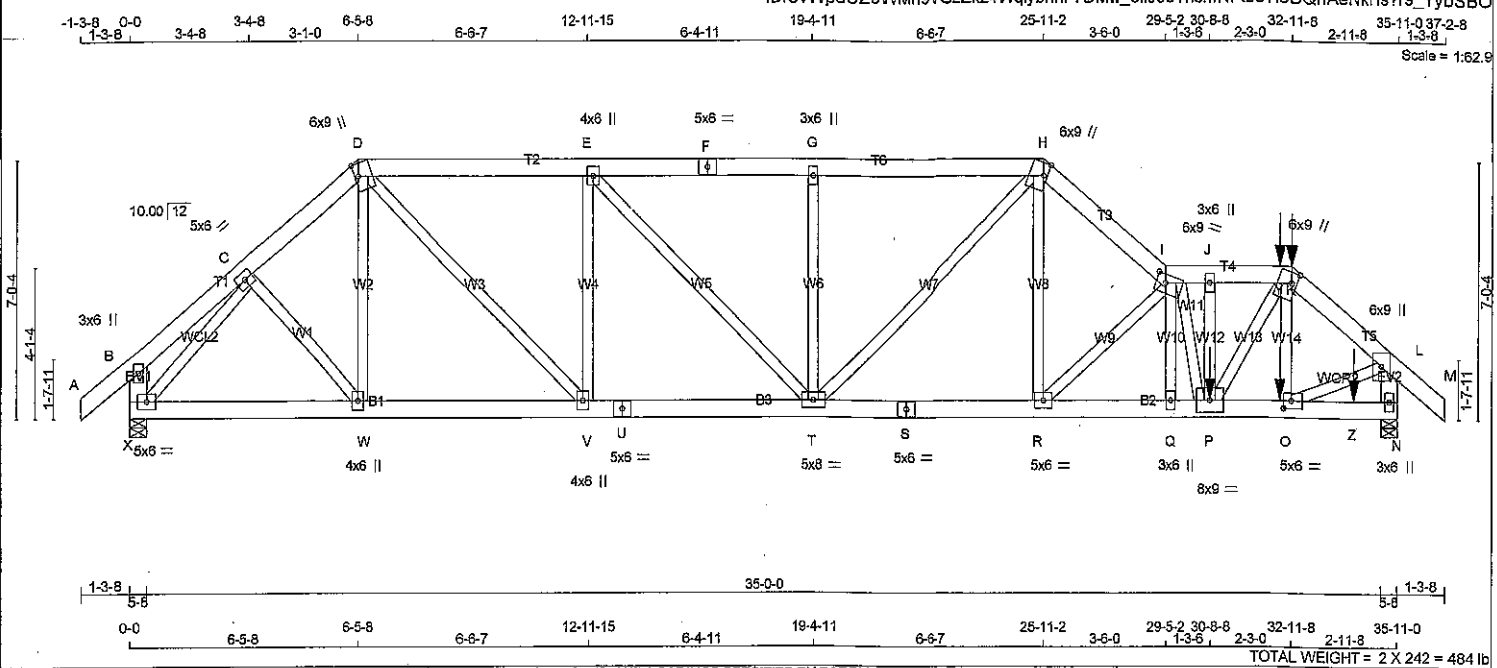

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- H	2	12	TOP
H- I	2	12	TOP
I- K	2	12	SIDE(122.0)
K- M	2	12	SIDE(122.0)
X- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- U	2	12	TOP
U- S	2	12	TOP
S- N	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
P- I	1	6	SIDE(272.7)
J- P	1	6	SIDE(273.1)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	6.0	9.0	4.00	1.25
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.00	1.25
I	TTWWW-m	MT20	6.0	9.0	2.75	3.25
J	TMW+w	MT20	3.0	6.0		
K	TTWW+m	MT20	6.0	9.0	3.50	1.50
L	TMVW+p	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
X	3322	0	3322	0	0	5-8	5-8
N	5747	0	5747	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2574	1718 / 0	433 / 0	0 / 0	0 / 0	423 / 0	0 / 0
N	4445	2979 / 0	739 / 0	0 / 0	0 / 0	727 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 56	-122.2	-122.2	0.05 (1)		10.00	C- W	0 / 311	0.03 (1)
B- C	-6 / 10	-122.2	-122.2	0.05 (1)		10.00	W- D	-23 / 200	0.02 (3)
C- D	-3479 / 0	-122.2	-122.2	0.07 (1)		5.93	D- V	0 / 2439	0.22 (1)
D- E	-4377 / 0	-122.2	-122.2	0.21 (1)		5.27	V- E	-1553 / 0	0.42 (1)
E- F	-5071 / 0	-122.2	-122.2	0.23 (1)		4.97	E- T	0 / 994	0.09 (1)
F- G	-5071 / 0	-122.2	-122.2	0.23 (1)		4.97	T- G	-830 / 0	0.22 (1)
G- H	-5071 / 0	-122.2	-122.2	0.22 (1)		4.97	T- H	0 / 416	0.04 (1)
H- I	-6129 / 0	-122.2	-122.2	0.12 (1)		4.71	R- H	0 / 3060	0.27 (1)
I- J	-6971 / 0	-122.2	-122.2	0.10 (1)		4.48	R- I	-3994 / 0	0.60 (1)
J- Y	-6968 / 0	-122.2	-122.2	0.12 (1)		4.46	Q- I	-57 / 42	0.00 (1)
Y- K	-6968 / 0	-122.2	-122.2	0.12 (1)		4.46	I- P	-1730 / 0	0.15 (1)
K- L	-5834 / 0	-122.2	-122.2	0.09 (1)		4.83	P- J	0 / 200	0.02 (1)
L- M	0 / 56	-122.2	-122.2	0.05 (1)		10.00	P- K	0 / 4837	0.43 (1)
X- B	-355 / 0	0.0	0.0	0.01 (1)		7.81	O- K	-1240 / 0	0.11 (1)
N- L	-5721 / 0	0.0	0.0	0.21 (1)		6.16	X- C	-3785 / 0	0.50 (1)
							O- L	0 / 4735	0.42 (1)
X- W	0 / 2442	-28.0	-28.0	0.20 (1)		10.00			
W- V	0 / 2651	-28.0	-28.0	0.21 (1)		10.00			
V- U	0 / 4376	-28.0	-28.0	0.32 (1)		10.00			
U- T	0 / 4376	-28.0	-28.0	0.32 (1)		10.00			
T- S	0 / 4777	-28.0	-28.0	0.34 (1)		10.00			
S- R	0 / 4777	-28.0	-28.0	0.34 (1)		10.00			
R- Q	0 / 7546	-28.0	-28.0	0.60 (1)		10.00			
Q- P	0 / 7544	-28.0	-28.0	0.60 (1)		10.00			
P- O	0 / 4406	-28.0	-28.0	0.39 (1)		10.00			
O- Z	0 / 0	-28.0	-28.0	0.05 (1)		10.00			
Z- N	0 / 0	-28.0	-28.0	0.05 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	32-11-8	-16	-16	---	FRONT	VERT	DEAD
K	32-11-8	-243	-243	---	FRONT	VERT	SNOW
O	32-7-12	-40	-70	---	FRONT	VERT	TOTAL
P	30-8-8	-2794	-2794	---	FRONT	VERT	TOTAL
Y	32-7-12	-189	-189	---	FRONT	VERT	TOTAL
Z	34-7-12	-40	-70	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47667-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.23 (E-G:1) , BC=0.60 (Q-R:1) , WB=0.60 (I-R:1) , SSI=0.16 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.96 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMWWW-t	MT20	8.0	9.0		
Q	BMW+w	MT20	3.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	5.0	8.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	4.0	6.0		
W	BMWW+t	MT20	4.0	6.0		
X	BMVW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 259.3 lbs FACTORED DOWN AT 32-11-8, AND 189.0 lbs FACTORED DOWN AT 32-7-12 ON TOP CHORD, AND 2793.9 lbs FACTORED DOWN AT 30-8-8, AND 69.9 lbs FACTORED DOWN AT 32-7-12, AND 69.9 lbs FACTORED DOWN AT 34-7-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM 47660-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T2S	1	1	TRUSS DESC.	

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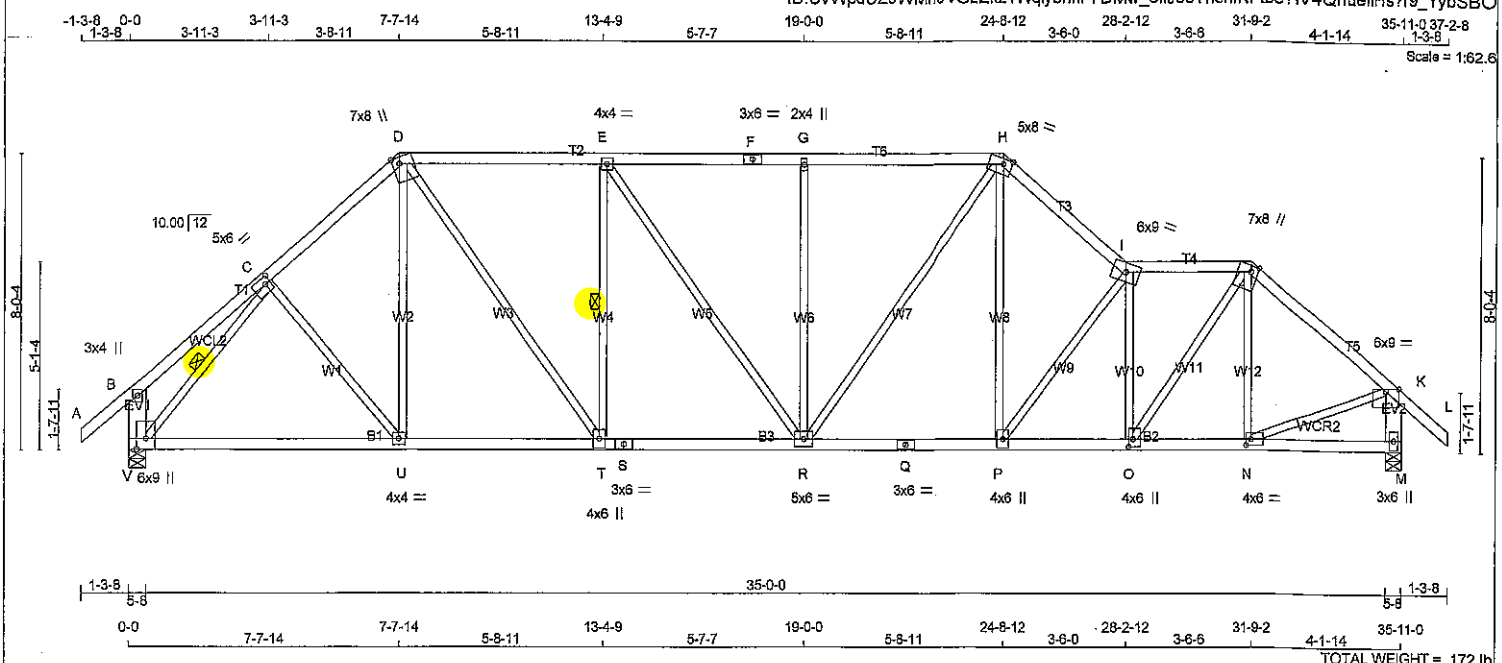

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TTWW-m	MT20	6.0	9.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW-p	MT20	6.0	9.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75
O	BMWW+t	MT20	4.0	6.0	2.50	1.50
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW+t	MT20	4.0	6.0		
U	BMWW-t	MT20	4.0	4.0		
V	BMVW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	2867	0	2867	0	0	5-8	5-8
M	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
M	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-T, C-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 32	-122.2	-122.2	0.27 (1)	10.00
C- D	-2809 / 0	-122.2	-122.2	0.41 (1)	3.78
D- E	-2986 / 0	-122.2	-122.2	0.79 (1)	3.20
E- F	-3181 / 0	-122.2	-122.2	0.81 (1)	3.08
F- G	-3181 / 0	-122.2	-122.2	0.81 (1)	3.08
G- H	-3181 / 0	-122.2	-122.2	0.81 (1)	3.10
H- I	-3501 / 0	-122.2	-122.2	0.45 (1)	3.38
I- J	-3360 / 0	-122.2	-122.2	0.41 (1)	3.45
J- K	-2733 / 0	-122.2	-122.2	0.54 (1)	3.69
K- L	0 / 54	-122.2	-122.2	0.17 (1)	10.00
V- B	-351 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-2820 / 0	0.0	0.0	0.20 (1)	6.24
V- U	0 / 2062	-28.0	-28.0	0.57 (2)	10.00
U- T	0 / 2129	-28.0	-28.0	0.59 (2)	10.00
T- S	0 / 2986	-28.0	-28.0	0.58 (1)	10.00
S- R	0 / 2986	-28.0	-28.0	0.58 (1)	10.00
R- Q	0 / 2704	-28.0	-28.0	0.53 (1)	10.00
Q- P	0 / 2704	-28.0	-28.0	0.53 (1)	10.00
P- O	0 / 3398	-28.0	-28.0	0.62 (1)	10.00
O- N	0 / 2083	-28.0	-28.0	0.41 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.12 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	0 / 165	0.04 (2)
U- D	0 / 274	0.06 (3)
D- T	0 / 1437	0.32 (1)
T- E	-1019 / 0	0.41 (1)
E- R	0 / 332	0.07 (1)
R- G	-746 / 0	0.92 (1)
R- H	0 / 800	0.18 (1)
P- H	0 / 1089	0.25 (1)
P- I	-1178 / 0	0.73 (1)
O- I	-1730 / 0	0.67 (1)
O- J	0 / 2195	0.49 (1)
N- J	-499 / 0	0.19 (1)
V- C	-3184 / 0	0.77 (1)
N- K	0 / 2190	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.81 (E-G:1) , BC=0.62 (O-P:1) , WB=0.92 (G-R:1) , SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.78 (C) (INPUT = 1.00)

DWG NO. TAM47668-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T3S	1	1	TRUSS DESC.	

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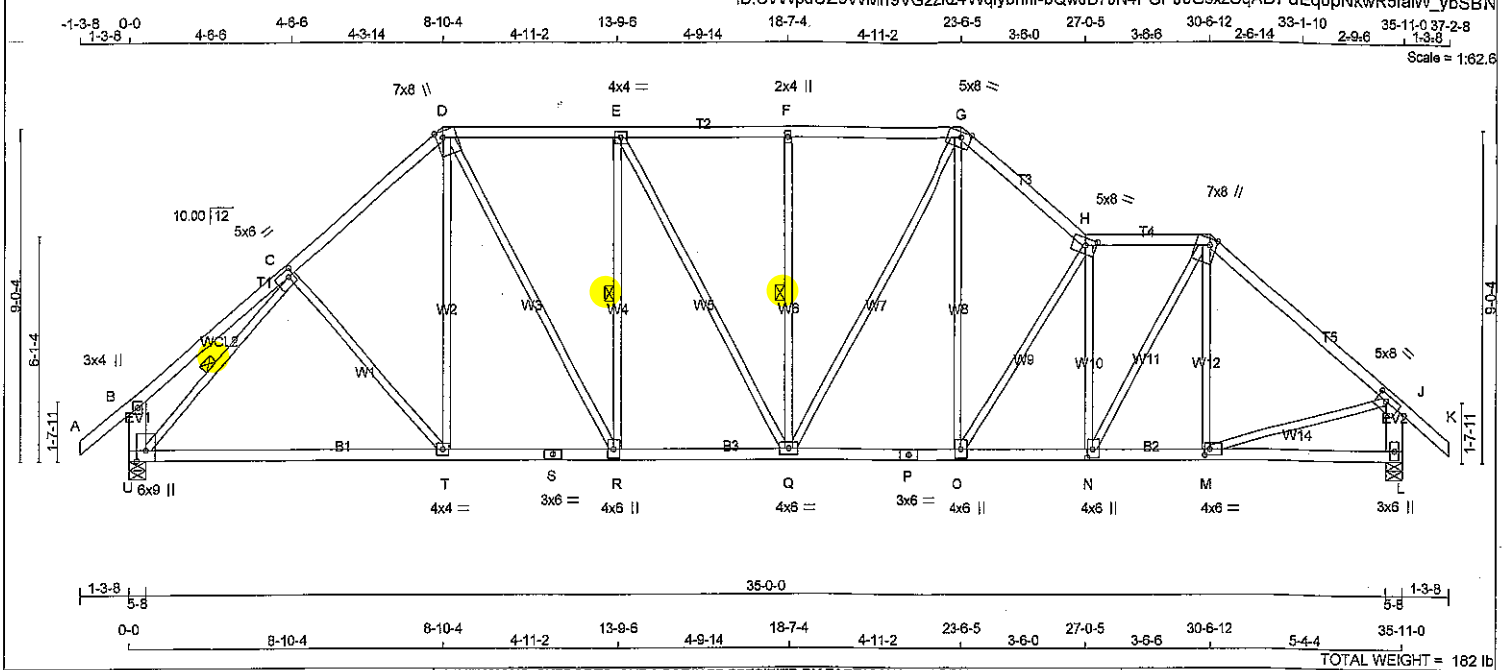


TOTAL WEIGHT = 162 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW-t	MT20	5.0	8.0	2.00	3.25
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	1.75
N	BMWW+t	MT20	4.0	6.0	2.75	1.75
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2867	0	2867	0	0	5-8	5-8
L	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
L	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

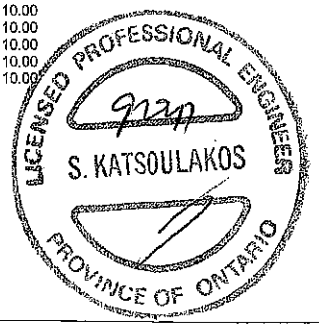
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, F-Q, C-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	0 / 39	-122.2	-122.2	0.37 (1)	10.00
C-D	-2764 / 0	-122.2	-122.2	0.57 (1)	3.65
D-E	-2658 / 0	-122.2	-122.2	0.53 (1)	3.69
E-F	-2809 / 0	-122.2	-122.2	0.54 (1)	3.59
F-G	-2809 / 0	-122.2	-122.2	0.54 (1)	3.61
G-H	-3263 / 0	-122.2	-122.2	0.43 (1)	3.51
H-I	-3085 / 0	-122.2	-122.2	0.39 (1)	3.61
I-J	-2797 / 0	-122.2	-122.2	0.98 (1)	3.14
J-K	0 / 54	-122.2	-122.2	0.17 (1)	10.00
U-B	-377 / 0	0.0	0.0	0.03 (1)	7.81
L-J	-2805 / 0	0.0	0.0	0.19 (1)	6.25
U-T	0 / 2113	-28.0	-28.0	0.69 (2)	10.00
T-S	0 / 2091	-28.0	-28.0	0.70 (2)	10.00
S-R	0 / 2091	-28.0	-28.0	0.70 (2)	10.00
R-Q	0 / 2658	-28.0	-28.0	0.50 (1)	10.00
Q-P	0 / 2520	-28.0	-28.0	0.48 (1)	10.00
P-O	0 / 2520	-28.0	-28.0	0.48 (1)	10.00
O-N	0 / 3113	-28.0	-28.0	0.56 (1)	10.00
N-M	0 / 2138	-28.0	-28.0	0.45 (1)	10.00
M-L	0 / 0	-28.0	-28.0	0.18 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-T	-36 / 134	0.03 (1)
T-D	0 / 375	0.08 (2)
D-R	0 / 1152	0.26 (1)
R-E	-916 / 0	0.48 (1)
E-Q	0 / 312	0.07 (1)
Q-F	-637 / 0	0.34 (1)
Q-G	0 / 588	0.13 (1)
O-G	0 / 1105	0.25 (1)
O-H	-1148 / 0	0.99 (1)
N-H	-1544 / 0	0.90 (1)
N-I	0 / 1858	0.42 (1)
M-I	-346 / 79	0.20 (1)
U-C	-3188 / 0	0.93 (1)
M-J	0 / 2205	0.50 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 939 (0.46")

CSI: TC=0.98 (I-J:1) , BC=0.70 (R-T:2) , WB=0.99 (H-O:1) , SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.79 (C) (INPUT = 1.00)

DWG NO. TAM49669 -17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T4S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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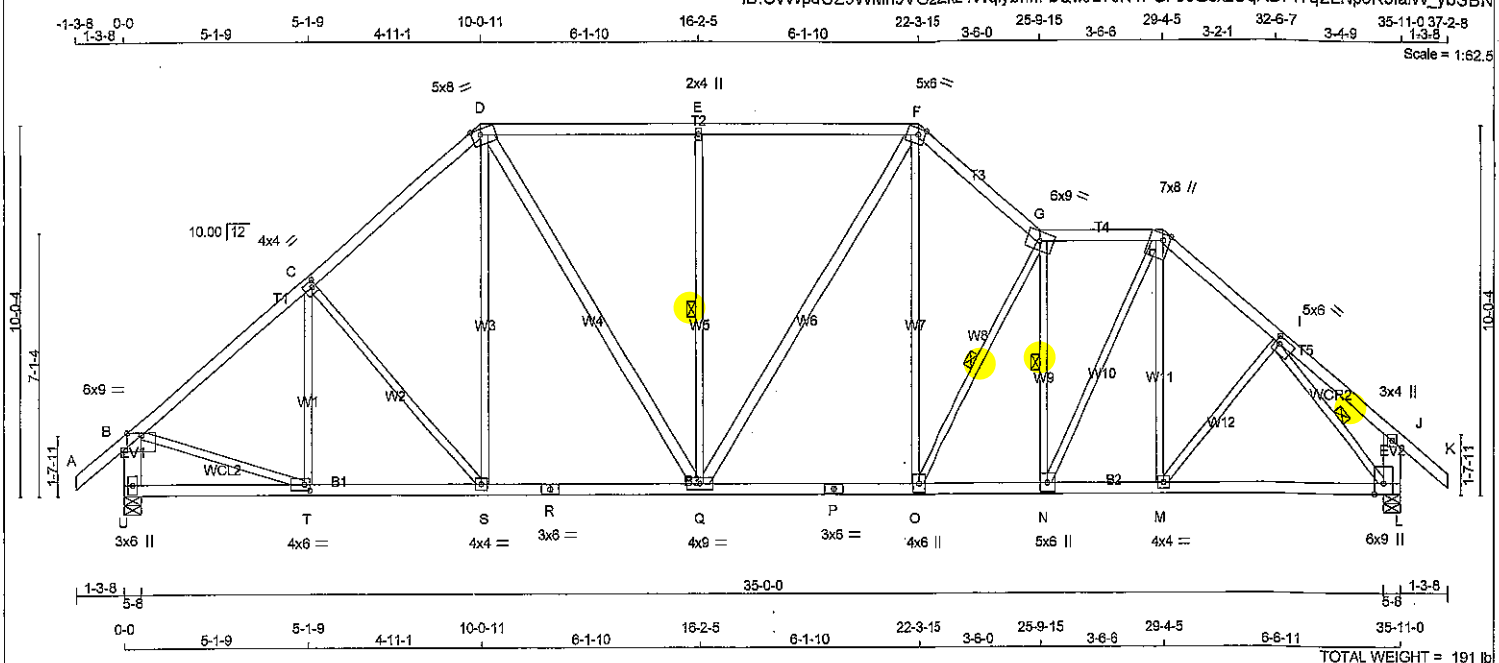

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TTWW-m	MT20	6.0	9.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0	2.25	1.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1+p	MT20	6.0	9.0	Edge	
M	BMWW-t	MT20	4.0	4.0		
N	BMWW+t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	4.0	6.0	2.00	1.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2867	0	2867	0	0	5-8	5-8
L	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
L	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-O, G-N, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B-C	-2820 / 0	-122.2	-122.2	0.56 (1)		3.64
C-D	-2678 / 0	-122.2	-122.2	0.52 (1)		3.75
D-E	-2532 / 0	-122.2	-122.2	0.70 (1)		3.49
E-F	-2532 / 0	-122.2	-122.2	0.70 (1)		3.49
F-G	-3049 / 0	-122.2	-122.2	0.31 (1)		3.76
G-H	-2857 / 0	-122.2	-122.2	0.29 (1)		3.86
H-I	-2829 / 0	-122.2	-122.2	0.24 (1)		3.96
I-J	0 / 28	-122.2	-122.2	0.18 (1)		10.00
J-K	0 / 54	-122.2	-122.2	0.17 (1)		10.00
U-B	-2806 / 0	0.0	0.0	0.19 (1)		6.25
L-J	-328 / 0	0.0	0.0	0.02 (1)		7.81
U-T	0 / 0	-28.0	-28.0	0.16 (3)		10.00
T-S	0 / 2202	-28.0	-28.0	0.45 (1)		10.00
S-R	0 / 2020	-28.0	-28.0	0.45 (1)		10.00
R-Q	0 / 2020	-28.0	-28.0	0.45 (1)		10.00
Q-P	0 / 2354	-28.0	-28.0	0.50 (1)		10.00
P-O	0 / 2354	-28.0	-28.0	0.50 (1)		10.00
O-N	0 / 2878	-28.0	-28.0	0.54 (1)		10.00
N-M	0 / 2147	-28.0	-28.0	0.50 (1)		10.00
M-L	0 / 1994	-28.0	-28.0	0.47 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-410 / 64	0.22 (1)
C-S	-277 / 0	0.31 (1)
S-D	0 / 382	0.09 (2)
D-Q	0 / 959	0.15 (1)
Q-E	-914 / 0	0.62 (1)
Q-F	0 / 333	0.05 (1)
O-F	0 / 1163	0.26 (1)
O-G	-1144 / 0	0.45 (1)
N-G	-1371 / 0	0.44 (1)
N-H	0 / 1575	0.35 (1)
M-H	0 / 188	0.04 (3)
M-I	0 / 236	0.05 (1)
B-T	0 / 2272	0.51 (1)
I-L	-3169 / 0	0.65 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.70 (D-E:1) , BC=0.54 (N-O:1) , WB=0.65 (I-L:1) , SSI=0.36 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.78 (I) (INPUT = 1.00)

DWG NO. TAM 47670-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
288283	T5S	1	1	TRUSS DESC.	

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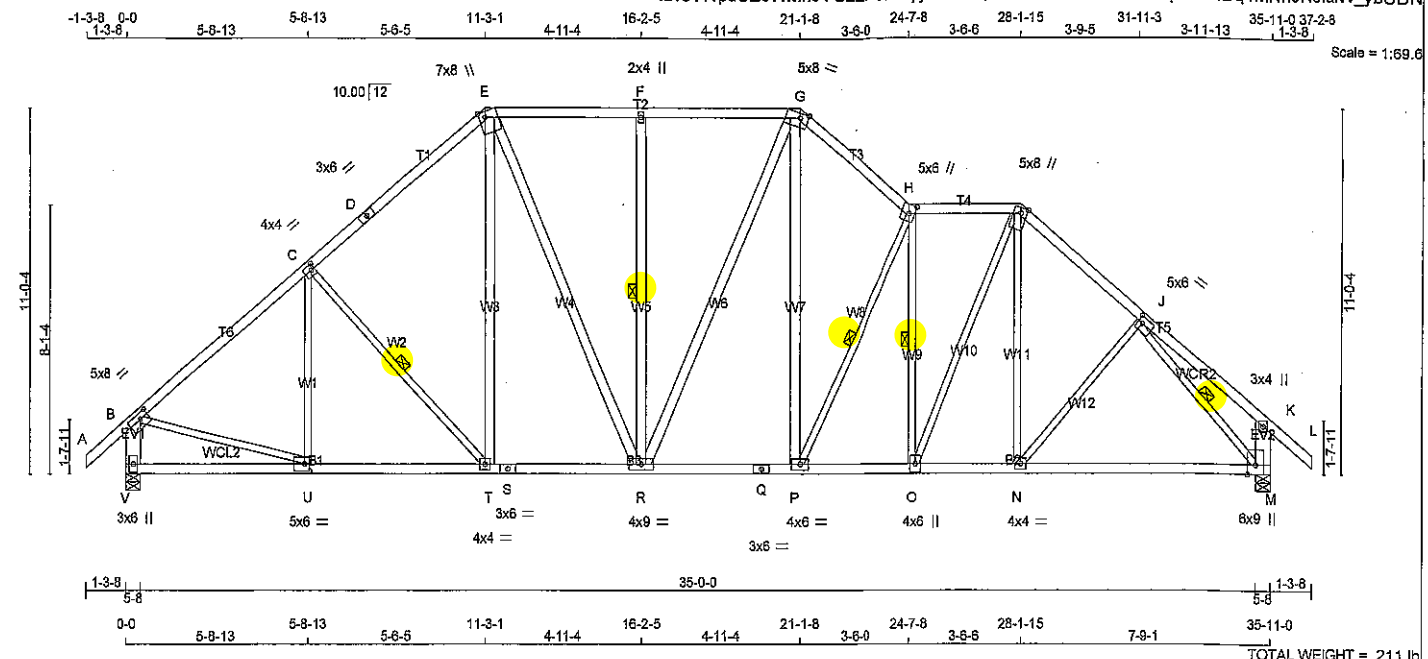


TOTAL WEIGHT = 211 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - E	2x4	DRY	No.2	SPF
E - R	2x4	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TTWW+m	MT20	5.0	6.0	3.25	2.00
I	TTWW+m	MT20	5.0	8.0	2.00	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	6.0	9.0	Edge	
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWWW-t	MT20	4.0	9.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2867	0	2867	0	0	5-8	5-8
M	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
M	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, F-R, H-P, H-O, J-M.

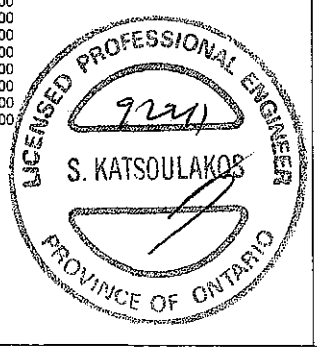
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	-2647 / 0	-122.2	-122.2	0.72 (1)	3.43
C-D	-2595 / 0	-122.2	-122.2	0.66 (1)	3.63
D-E	-2595 / 0	-122.2	-122.2	0.66 (1)	3.63
E-F	-2274 / 0	-122.2	-122.2	0.44 (1)	4.08
F-G	-2274 / 0	-122.2	-122.2	0.44 (1)	4.08
G-H	-2838 / 0	-122.2	-122.2	0.30 (1)	3.89
H-I	-2661 / 0	-122.2	-122.2	0.27 (1)	4.01
I-J	-2807 / 0	-122.2	-122.2	0.32 (1)	3.89
J-K	0 / 33	-122.2	-122.2	0.27 (1)	10.00
K-L	0 / 54	-122.2	-122.2	0.17 (1)	10.00
V-B	-2801 / 0	0.0	0.0	0.19 (1)	6.26
M-K	-353 / 0	0.0	0.0	0.02 (1)	7.81
V-U	0 / 0	-28.0	-28.0	0.22 (3)	10.00
U-T	0 / 2228	-28.0	-28.0	0.47 (1)	10.00
T-S	0 / 1954	-28.0	-28.0	0.39 (1)	10.00
S-R	0 / 1954	-28.0	-28.0	0.39 (1)	10.00
R-Q	0 / 2197	-28.0	-28.0	0.42 (1)	10.00
Q-P	0 / 2197	-28.0	-28.0	0.42 (1)	10.00
P-O	0 / 2677	-28.0	-28.0	0.50 (1)	10.00
O-N	0 / 2128	-28.0	-28.0	0.60 (2)	10.00
N-M	0 / 2068	-28.0	-28.0	0.59 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	-330 / 129	0.22 (1)
C-T	-411 / 0	0.19 (1)
T-E	0 / 445	0.07 (1)
E-R	0 / 765	0.12 (1)
R-F	-727 / 0	0.45 (1)
R-G	0 / 184	0.03 (1)
P-G	0 / 1196	0.19 (1)
P-H	-1171 / 0	0.59 (1)
O-H	-1167 / 0	0.49 (1)
O-I	0 / 1324	0.30 (1)
N-I	0 / 283	0.06 (2)
N-J	0 / 161	0.04 (3)
B-U	0 / 2285	0.51 (1)
J-M	-3186 / 0	0.78 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.72 (B-C:1) , BC=0.60 (N-O:2) , WB=0.78 (J-M:1) , SSI=0.29 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.79 (J) (INPUT = 1.00)

DWG NO. TAM 42671 -17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288285	T6	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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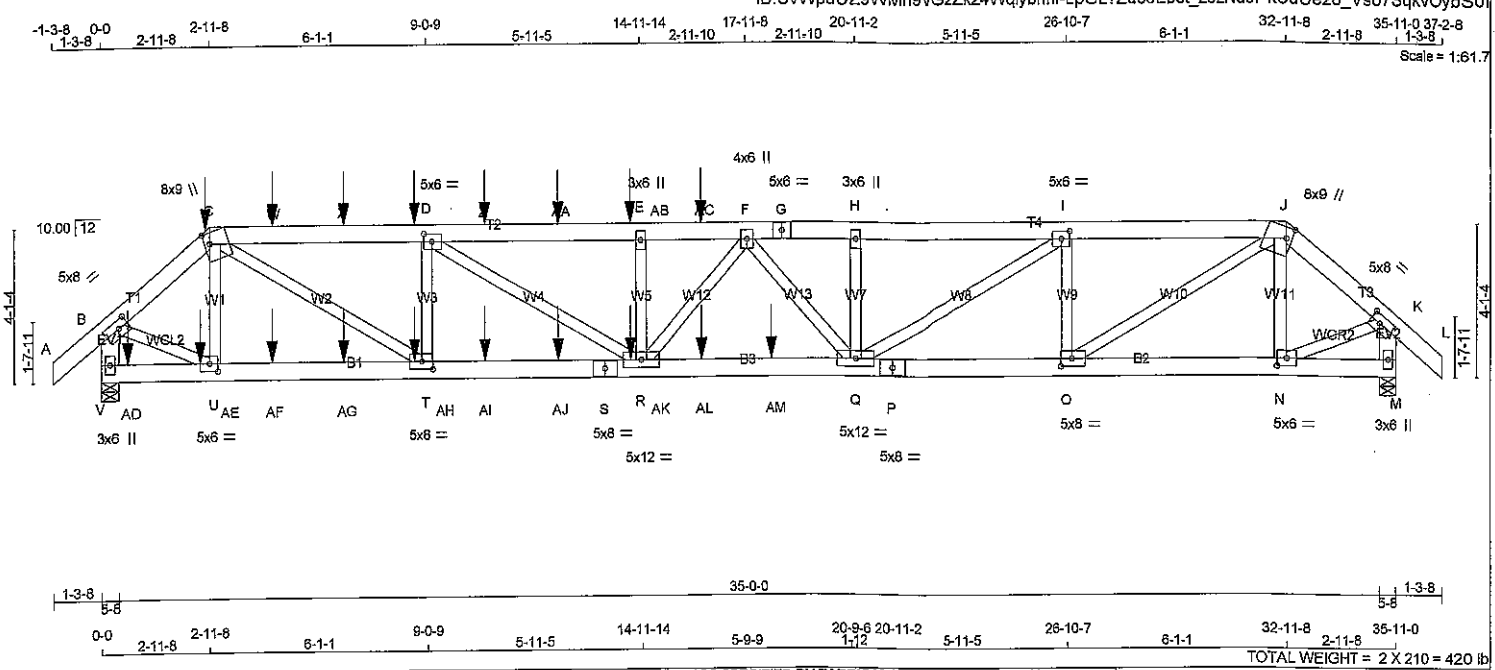


TOTAL WEIGHT = 2 X 210 = 420 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	2100F 1.8E	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(183.1)
G- J	2	12	TOP
J- L	2	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(197.8)
S- P	2	12	SIDE(197.8)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	3.50	1.75
D	TMWW-t	MT20	5.0	6.0	2.50	2.75
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	8.0	9.0	3.50	1.75
K	TMWW-t	MT20	5.0	8.0	2.50	3.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMWW-t	MT20	5.0	8.0	2.50	3.75
P	BS-t	MT20	5.0	8.0		
Q	BMWWW-t	MT20	5.0	12.0		
R	BMWWW-t	MT20	5.0	12.0		
S	BS-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	5137	0	5137	0	0	5-8	5-8
M	4257	0	4257	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3989	2644 / 0	681 / 0	0 / 0	0 / 0	664 / 0	0 / 0
M	3292	2208 / 0	546 / 0	0 / 0	0 / 0	537 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

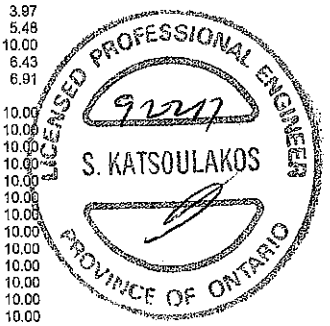
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 56	-122.2	-122.2	0.05 (1)	10.00	U- C	-1035 / 0	0.09 (1)
B- C	-5206 / 0	-122.2	-122.2	0.08 (1)	5.07	C- T	0 / 6502	0.57 (1)
C- W	-9526 / 0	-122.2	-122.2	0.44 (1)	3.62	T- D	-3260 / 0	0.28 (1)
W- X	-9526 / 0	-122.2	-122.2	0.44 (1)	3.62	D- R	0 / 3709	0.33 (1)
X- Y	-9526 / 0	-122.2	-122.2	0.44 (1)	3.62	R- E	-835 / 0	0.07 (1)
Y- D	-9526 / 0	-122.2	-122.2	0.44 (1)	3.62	Q- H	-470 / 0	0.04 (1)
D- Z	-12689 / 0	-122.2	-122.2	0.58 (1)	3.05	Q- I	0 / 4565	0.40 (1)
Z-AA	-12689 / 0	-122.2	-122.2	0.58 (1)	3.05	O- I	-3212 / 0	0.27 (1)
AA-AB	-12689 / 0	-122.2	-122.2	0.58 (1)	3.05	O- J	0 / 6087	0.54 (1)
AB- E	-12689 / 0	-122.2	-122.2	0.58 (1)	3.05	N- J	-876 / 0	0.07 (1)
E-AC	-12689 / 0	-122.2	-122.2	0.34 (1)	3.28	B- U	0 / 4225	0.37 (1)
AC- F	-12689 / 0	-122.2	-122.2	0.34 (1)	3.28	N- K	0 / 3474	0.31 (1)
F- G	-12361 / 0	-122.2	-122.2	0.31 (1)	3.34	R- F	-3 / 92	0.01 (3)
G- H	-12361 / 0	-122.2	-122.2	0.31 (1)	3.34	F- Q	-522 / 0	0.07 (1)
H- I	-12361 / 0	-122.2	-122.2	0.46 (1)	3.23			
I- J	-8470 / 0	-122.2	-122.2	0.30 (1)	3.97			
J- K	-4282 / 0	-122.2	-122.2	0.07 (1)	5.48			
K- L	0 / 56	-122.2	-122.2	0.05 (1)	10.00			
V- B	-5143 / 0	0.0	0.0	0.19 (1)	6.43			
M- K	-4293 / 0	0.0	0.0	0.15 (1)	6.91			
V-AD	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
AD-AE	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
AE- U	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
U-AF	0 / 3935	-28.0	-28.0	0.16 (1)	10.00			
AF-AG	0 / 3935	-28.0	-28.0	0.16 (1)	10.00			
AG-AH	0 / 3935	-28.0	-28.0	0.16 (1)	10.00			
AH- T	0 / 3935	-28.0	-28.0	0.16 (1)	10.00			
T-AI	0 / 9527	-28.0	-28.0	0.38 (1)	10.00			
AI-AJ	0 / 9527	-28.0	-28.0	0.38 (1)	10.00			
AJ- S	0 / 9527	-28.0	-28.0	0.38 (1)	10.00			
S-AK	0 / 9527	-28.0	-28.0	0.38 (1)	10.00			
AK- R	0 / 9527	-28.0	-28.0	0.38 (1)	10.00			
R-AL	0 / 12691	-28.0	-28.0	0.74 (1)	10.00			
AL-AM	0 / 12691	-28.0	-28.0	0.74 (1)	10.00			
AM- Q	0 / 12691	-28.0	-28.0	0.74 (1)	10.00			
Q- P	0 / 8471	-28.0	-28.0	0.38 (1)	10.00			
P- O	0 / 8471	-28.0	-28.0	0.38 (1)	10.00			
O- N	0 / 3235	-28.0	-28.0	0.14 (1)	10.00			
N- M	0 / 0	-28.0	-28.0	0.05 (1)	10.00			



DWG NO. TAM47694-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.41")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 695 (0.62")

CSI: TC=0.58 (D-E:1) , BC=0.74 (Q-R:1) , WB=0.57 (C-T:1) , SSI=0.41 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.86 (S) (INPUT = 1.00)

P61/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288285	T6	1	2	TRUSS DESC.	

PLATES (table is in inches)

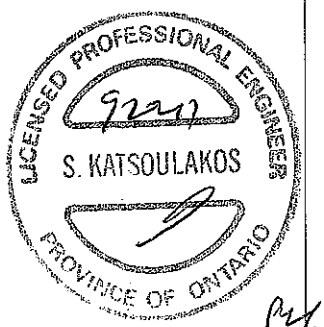
JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW-t	MT20	5.0	8.0	2.50	3.75
U	BMWW-t	MT20	5.0	6.0	2.50	2.75
V	BMV1+p	MT20	3.0	8.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 259.3 lbs FACTORED DOWN AT 2-11-8, 149.9 lbs FACTORED DOWN AT 2-11-8, 147.1 lbs FACTORED DOWN AT 4-8-4, 147.1 lbs FACTORED DOWN AT 6-8-4, 147.1 lbs FACTORED DOWN AT 8-8-4, 147.1 lbs FACTORED DOWN AT 10-8-4, 147.1 lbs FACTORED DOWN AT 12-8-4, AND 147.1 lbs FACTORED DOWN AT 14-8-4, AND 147.1 lbs FACTORED DOWN AT 16-8-4 ON TOP CHORD, AND 77.6 lbs FACTORED DOWN AT 8-4, 69.9 lbs FACTORED DOWN AT 2-8-4, 69.9 lbs FACTORED DOWN AT 4-8-4, 69.9 lbs FACTORED DOWN AT 6-8-4, 69.9 lbs FACTORED DOWN AT 8-8-4, 69.9 lbs FACTORED DOWN AT 10-8-4, 69.9 lbs FACTORED DOWN AT 12-8-4, 69.9 lbs FACTORED DOWN AT 14-8-4, AND 69.9 lbs FACTORED DOWN AT 16-8-4, AND 1844.2 lbs FACTORED DOWN AT 18-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-16	-16	—	FRONT	VERT	DEAD
C	2-11-8	-150	-150	—	BACK	VERT	TOTAL
C	2-11-8	-243	-243	—	FRONT	VERT	SNOW
W	4-8-4	-147	-147	—	BACK	VERT	TOTAL
X	6-8-4	-147	-147	—	BACK	VERT	TOTAL
Y	8-8-4	-147	-147	—	BACK	VERT	TOTAL
Z	10-8-4	-147	-147	—	BACK	VERT	TOTAL
AA	12-8-4	-147	-147	—	BACK	VERT	TOTAL
AB	14-8-4	-147	-147	—	BACK	VERT	TOTAL
AC	16-8-4	-147	-147	—	BACK	VERT	TOTAL
AD	8-4	-44	-78	—	BACK	VERT	TOTAL
AE	2-8-4	-40	-70	—	BACK	VERT	TOTAL
AF	4-8-4	-40	-70	—	BACK	VERT	TOTAL
AG	6-8-4	-40	-70	—	BACK	VERT	TOTAL
AH	8-8-4	-40	-70	—	BACK	VERT	TOTAL
AI	10-8-4	-40	-70	—	BACK	VERT	TOTAL
AJ	12-8-4	-40	-70	—	BACK	VERT	TOTAL
AK	14-8-4	-40	-70	—	BACK	VERT	TOTAL
AL	16-8-4	-40	-70	—	BACK	VERT	TOTAL
AM	18-7-8	-1844	-1844	—	BACK	VERT	TOTAL



DWG NO. TAM 47694-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T6S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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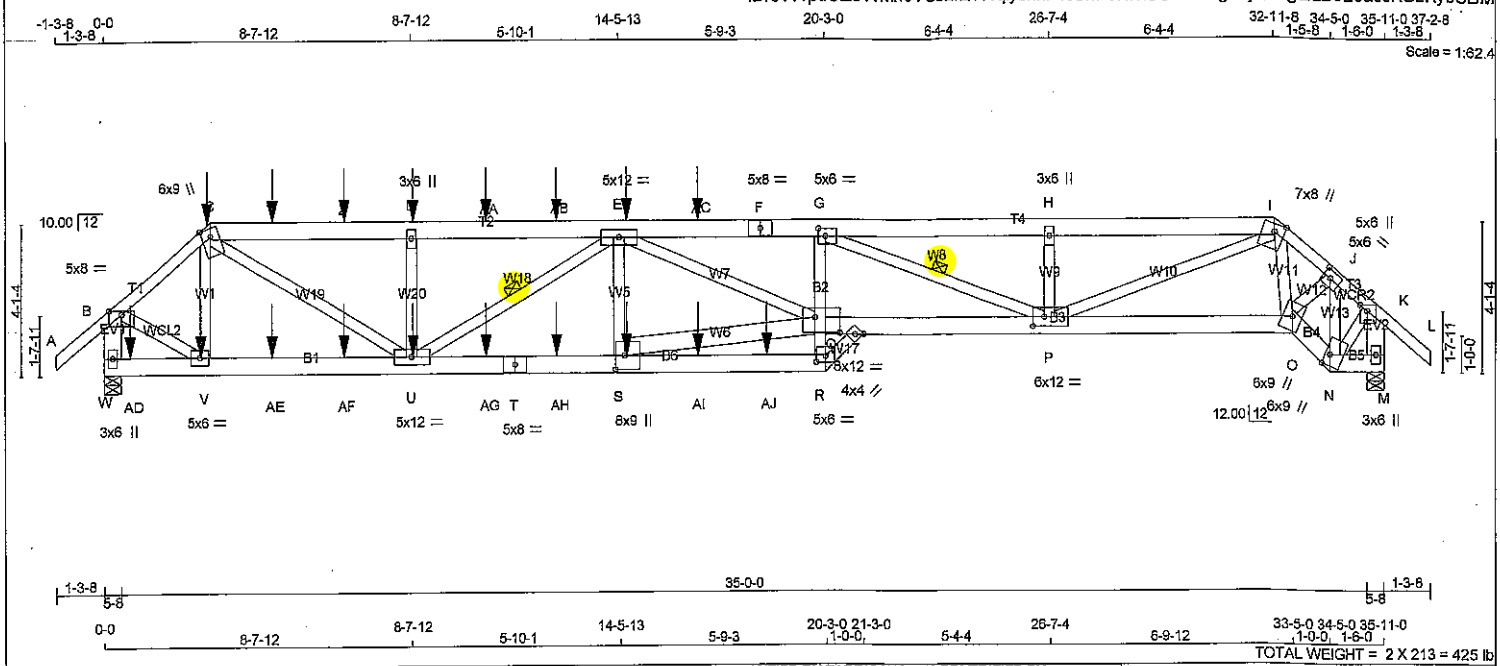


TOTAL WEIGHT = 2 X 213 = 425 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - R	2x6	DRY	1650F 1.5E	SPF
R - G	2x4	DRY	No.2	SPF
Q - O	2x6	DRY	1650F 1.5E	SPF
O - N	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
I- L	1	12	TOP
C- F	2	12	SIDE(183.1)
F- I	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(183.1)
T- R	2	12	SIDE(183.1)
Q- O	2	12	TOP
O- N	2	12	TOP
N- M	2	12	TOP
R- G	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	12.0		
F	TS-t	MT20	5.0	8.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.25
H	TMW+w	MT20	3.0	6.0		
I	TTWW+m	MT20	7.0	8.0	2.50	3.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	4957	0	4957	0	0	5-8	5-8
M	4237	0	4237	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3868	2535 / 0	681 / 0	0 / 0	0 / 0	651 / 0	0 / 0
M	3278	2196 / 0	546 / 0	0 / 0	0 / 0	536 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-P, E-U.

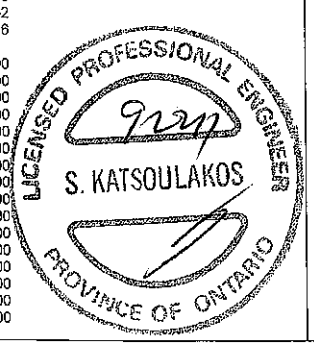
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00
B- C	-4746 / 0	-122.2	-122.2	0.19 (1)	4.27
C- Y	-9080 / 0	-122.2	-122.2	0.37 (1)	3.78
Y- Z	-9080 / 0	-122.2	-122.2	0.37 (1)	3.78
Z- D	-9080 / 0	-122.2	-122.2	0.37 (1)	3.78
D-AA	-9079 / 0	-122.2	-122.2	0.35 (1)	3.80
AA-AB	-9079 / 0	-122.2	-122.2	0.35 (1)	3.80
AB- E	-9079 / 0	-122.2	-122.2	0.35 (1)	3.80
E-AC	-17046 / 0	-122.2	-122.2	0.74 (1)	2.50
AC- F	-17046 / 0	-122.2	-122.2	0.74 (1)	2.50
F- G	-17046 / 0	-122.2	-122.2	0.74 (1)	2.50
G- H	-11902 / 0	-122.2	-122.2	0.50 (1)	3.25
H- I	-11903 / 0	-122.2	-122.2	0.46 (1)	3.29
I- J	-5463 / 0	-122.2	-122.2	0.16 (1)	4.03
J- K	-3169 / 0	-122.2	-122.2	0.12 (1)	5.10
K- L	0 / 54	-122.2	-122.2	0.09 (1)	10.00
W- B	-4964 / 0	0.0	0.0	0.18 (1)	6.52
M- K	-4216 / 0	0.0	0.0	0.16 (1)	6.96
W-AD	0 / 0	-28.0	-28.0	0.06 (2)	10.00
AD- V	0 / 0	-28.0	-28.0	0.06 (2)	10.00
V-AE	0 / 3707	-28.0	-28.0	0.22 (1)	10.00
AE-AF	0 / 3707	-28.0	-28.0	0.22 (1)	10.00
AF- U	0 / 3707	-28.0	-28.0	0.22 (1)	10.00
U-AG	0 / 12521	-28.0	-28.0	0.68 (1)	10.00
AG- T	0 / 12521	-28.0	-28.0	0.68 (1)	10.00
T-AH	0 / 12521	-28.0	-28.0	0.68 (1)	10.00
AH- S	0 / 12521	-28.0	-28.0	0.68 (1)	10.00
S-AI	0 / 461	-28.0	-28.0	0.51 (1)	10.00
AI-AJ	0 / 461	-28.0	-28.0	0.51 (1)	10.00
AJ- R	0 / 461	-28.0	-28.0	0.51 (1)	10.00
R- Q	0 / 1332	0.0	0.0	0.47 (1)	10.00
Q- G	0 / 1534	0.0	0.0	0.49 (1)	10.00
Q- P	0 / 17224	-28.0	-28.0	0.76 (1)	10.00
P- O	0 / 4274	-28.0	-28.0	0.23 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1223 / 0	0.11 (1)
S- E	-1149 / 0	0.10 (1)
S- Q	0 / 12255	0.60 (1)
E- Q	0 / 4997	0.44 (1)
G- P	-5745 / 0	0.69 (1)
P- H	-951 / 0	0.06 (1)
P- I	0 / 8272	0.73 (1)
I- O	-158 / 75	0.01 (1)
O- J	0 / 3231	0.29 (1)
N- J	-3935 / 0	0.25 (1)
B- V	0 / 3989	0.35 (1)
N- K	0 / 2999	0.27 (1)
U- E	-4058 / 0	0.47 (1)
C- U	0 / 6390	0.56 (1)
U- D	-1259 / 0	0.11 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 723 (0.60")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 477 (0.90")

CSI: TC=0.74 (E-G:1) , BC=0.76 (P-Q:1) , WB=0.80 (Q-S:1) , SSI=0.44 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.98 (S) (INPUT = 1.00)

p6/4

DWG NO. TAM47672-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N	BBWW+m	MT20	6.0	9.0	3.50	1.50
O	BBWW+m	MT20	6.0	9.0		
P	BMWWW-t	MT20	6.0	12.0	3.00	3.75
Q	BVMWW-I	MT20	6.0	12.0	5.00	8.25
R						
R						
R	BVW*-I	MT20	5.0	6.0	2.00	3.00
S	BMWW+t	MT20	8.0	9.0	4.50	3.00
T	BS-t	MT20	5.0	8.0		
U	BMWWW-t	MT20	5.0	12.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		
X	NP+w	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 222.6 lbs FACTORED DOWN AT 2-11-8, 147.1 lbs FACTORED DOWN AT 4-8-4, 147.1 lbs FACTORED DOWN AT 6-8-4, 147.1 lbs FACTORED DOWN AT 8-8-4, 147.1 lbs FACTORED DOWN AT 10-8-4, 147.1 lbs FACTORED DOWN AT 12-8-4, AND 147.1 lbs FACTORED DOWN AT 14-8-4, AND 147.1 lbs FACTORED DOWN AT 16-8-4 ON TOP CHORD, AND 77.6 lbs FACTORED DOWN AT 8-4, 69.9 lbs FACTORED DOWN AT 2-8-4, 69.9 lbs FACTORED DOWN AT 4-8-4, 69.9 lbs FACTORED DOWN AT 6-8-4, 69.9 lbs FACTORED DOWN AT 8-8-4, 69.9 lbs FACTORED DOWN AT 10-8-4, 69.9 lbs FACTORED DOWN AT 12-8-4, 69.9 lbs FACTORED DOWN AT 14-8-4, AND 69.9 lbs FACTORED DOWN AT 16-8-4, AND 1844.2 lbs FACTORED DOWN AT 18-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)		MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- N	0 / 2988	-28.0	-28.0	0.20 (1)		10.00			
N- M	0 / 0	-28.0	-28.0	0.01 (2)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-223	-223	—	BACK	VERT	TOTAL
D	8-8-4	-147	-147	—	BACK	VERT	TOTAL
E	14-8-4	-147	-147	—	BACK	VERT	TOTAL
S	14-8-4	-40	-70	—	BACK	VERT	TOTAL
U	8-8-4	-40	-70	—	BACK	VERT	TOTAL
V	2-8-4	-40	-70	—	BACK	VERT	TOTAL
Y	4-8-4	-147	-147	—	BACK	VERT	TOTAL
Z	6-8-4	-147	-147	—	BACK	VERT	TOTAL
AA	10-8-4	-147	-147	—	BACK	VERT	TOTAL
AB	12-8-4	-147	-147	—	BACK	VERT	TOTAL
AC	16-8-4	-147	-147	—	BACK	VERT	TOTAL
AD	8-4	-44	-78	—	BACK	VERT	TOTAL
AE	4-8-4	-40	-70	—	BACK	VERT	TOTAL
AF	6-8-4	-40	-70	—	BACK	VERT	TOTAL
AG	10-8-4	-40	-70	—	BACK	VERT	TOTAL
AH	12-8-4	-40	-70	—	BACK	VERT	TOTAL
AI	16-8-4	-40	-70	—	BACK	VERT	TOTAL
AJ	18-7-8	-1844	-1844	—	BACK	VERT	TOTAL

DWG NO. TAM 47602-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288285	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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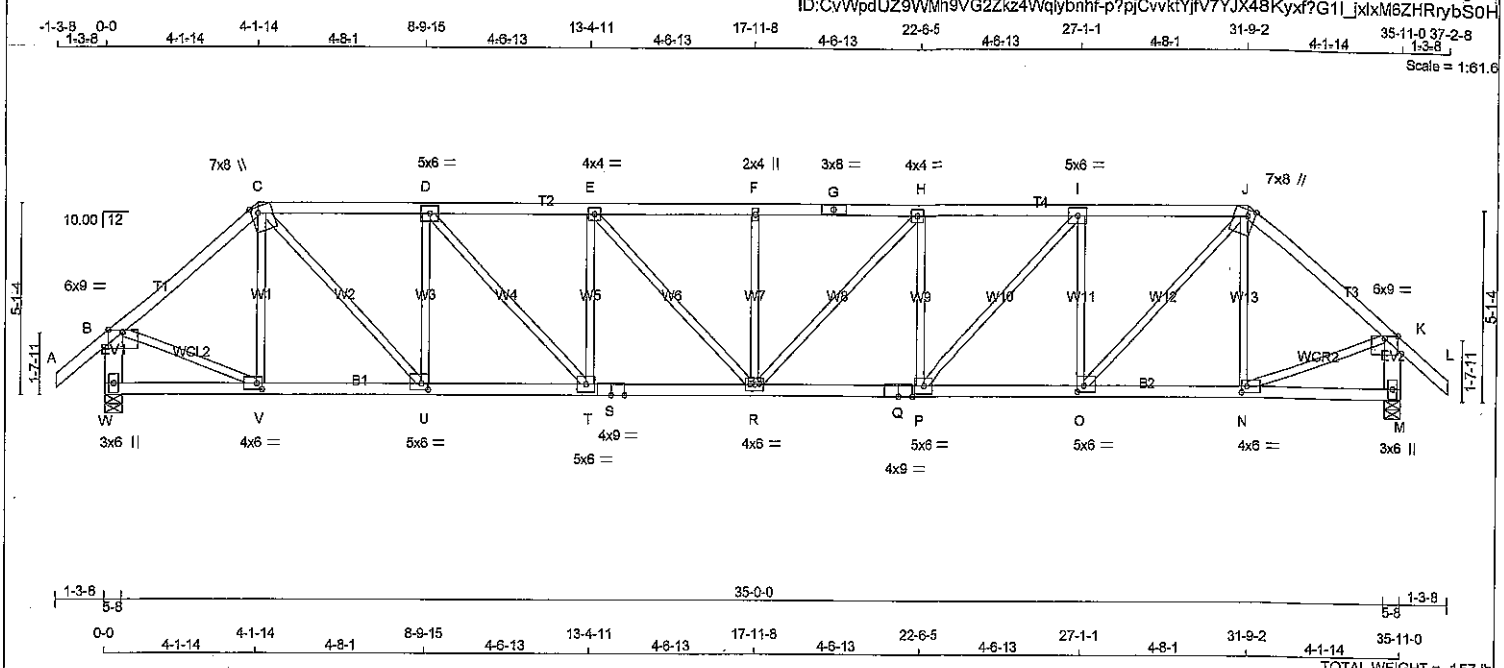

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW-p	MT20	6.0	9.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75
O	BMWW-t	MT20	5.0	6.0	2.00	2.25
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	4.0	9.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BS-t	MT20	4.0	9.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.00	2.25
V	BMWW-t	MT20	4.0	6.0	2.00	1.75
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2867	0	2867	0	0	5-8	5-8
M	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
M	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	-2733 / 0	-122.2	-122.2	0.54 (1)		3.69
C- D	-3791 / 0	-122.2	-122.2	0.63 (1)		3.08
D- E	-4751 / 0	-122.2	-122.2	0.77 (1)		2.62
E- F	-5079 / 0	-122.2	-122.2	0.73 (1)		2.61
F- G	-5079 / 0	-122.2	-122.2	0.73 (1)		2.61
G- H	-5079 / 0	-122.2	-122.2	0.73 (1)		2.61
H- I	-4751 / 0	-122.2	-122.2	0.77 (1)		2.62
I- J	-3791 / 0	-122.2	-122.2	0.63 (1)		3.08
J- K	-2733 / 0	-122.2	-122.2	0.54 (1)		3.69
K- L	0 / 54	-122.2	-122.2	0.17 (1)		10.00
W- B	-2821 / 0	0.0	0.0	0.20 (1)		6.24
M- K	-2821 / 0	0.0	0.0	0.20 (1)		6.24
W- V	0 / 0	-28.0	-28.0	0.13 (2)		10.00
V- U	0 / 2083	-28.0	-28.0	0.41 (1)		10.00
U- T	0 / 3791	-28.0	-28.0	0.67 (1)		10.00
T- S	0 / 4751	-28.0	-28.0	0.83 (1)		10.00
S- R	0 / 4751	-28.0	-28.0	0.83 (1)		10.00
R- Q	0 / 4751	-28.0	-28.0	0.83 (1)		10.00
Q- P	0 / 4751	-28.0	-28.0	0.83 (1)		10.00
P- O	0 / 3791	-28.0	-28.0	0.67 (1)		10.00
O- N	0 / 2083	-28.0	-28.0	0.41 (1)		10.00
N- M	0 / 0	-28.0	-28.0	0.13 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-499 / 0	0.19 (1)
C- U	0 / 2447	0.55 (1)
U- D	-1630 / 0	0.63 (1)
D- T	0 / 1394	0.31 (1)
T- E	-882 / 0	0.34 (1)
E- R	0 / 477	0.11 (1)
R- F	-567 / 0	0.22 (1)
R- H	0 / 477	0.11 (1)
P- H	-882 / 0	0.34 (1)
P- I	0 / 1394	0.31 (1)
O- I	-1630 / 0	0.63 (1)
O- J	0 / 2447	0.55 (1)
N- J	-499 / 0	0.19 (1)
B- V	0 / 2190	0.49 (1)
N- K	0 / 2190	0.49 (1)

LICENSED PROFESSIONAL ENGINEER
9221
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47695-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.37")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 765 (0.56")

CSI: TC=0.77 (H-I:1) , BC=0.83 (P-R:1) , WB=0.63 (I-O:1) , SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.87 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T7S	1	1	TRUSS DESC.	

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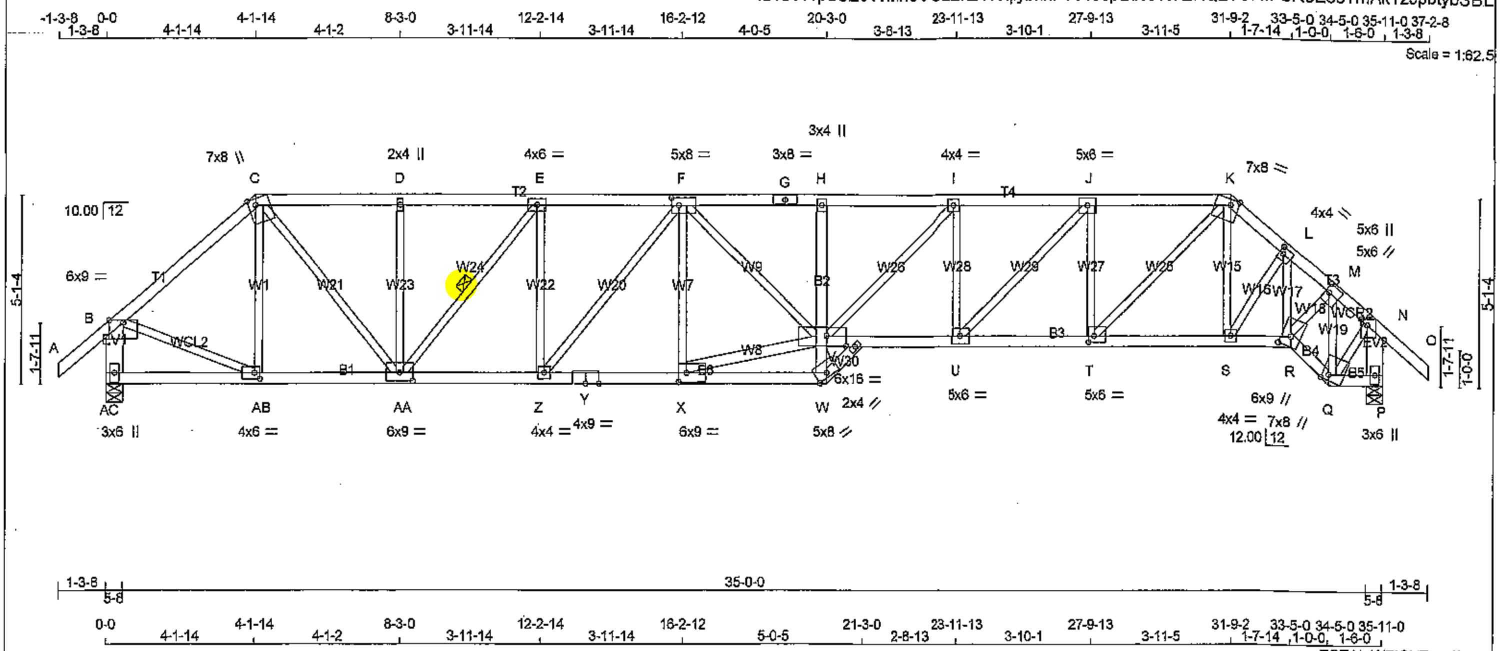

TOTAL WEIGHT = 172 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - O	2x4	DRY	No.2	SPF
AC- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AC- Y	2x4	DRY	No.2	SPF
Y - W	2x4	DRY	No.2	SPF
W - H	2x4	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
X - V	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMWWW-t	MT20	5.0	8.0	2.50	2.50
G	TS-t	MT20	3.0	8.0		
H	TMV+p	MT20	3.0	4.0		
I	TMWW-t	MT20	4.0	4.0		
J	TMWW-t	MT20	5.0	6.0		
K	TTWW-m	MT20	7.0	8.0	Edge	2.75
L	TMWW-t	MT20	4.0	4.0	2.00	1.25
M	TMWW+t	MT20	5.0	6.0	2.75	1.25
N	TMVW+p	MT20	5.0	6.0	2.00	2.00
P	BMV1+p	MT20	3.0	6.0		
Q	BBWW+m	MT20	7.0	8.0	Edge	2.25
R	BBWW+m	MT20	6.0	9.0	3.25	3.00
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	5.0	6.0	2.00	2.00
U	BMWW-t	MT20	5.0	6.0		
V	BVMWWW-l	MT20	6.0	16.0	3.25	6.50
W	BVW*-m	MT20	5.0	8.0	1.50	3.75
X	BMWW-t	MT20	6.0	9.0	3.00	2.50
Y	BS-t	MT20	4.0	9.0		
Z	BMWW-t	MT20	4.0	4.0		
AA	BMWWW-t	MT20	6.0	9.0	2.75	4.50
AB	BMWW-t	MT20	4.0	6.0	2.00	1.75
AC	BMV1+p	MT20	3.0	6.0		
AD	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AC	2864	0	2864	0	0	5-8	5-8
P	2870	0	2870	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
AC	2222	1477 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
P	2226	1481 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AC, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-AA.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	AB- C	-504 / 0	0.19 (1)
B- C	-2728 / 0	-122.2	-122.2	0.54 (1)	3.69	X- F	-1125 / 0	0.43 (1)
C- D	-3600 / 0	-122.2	-122.2	0.47 (1)	3.32	X- V	0 / 5029	0.81 (1)
D- E	-3600 / 0	-122.2	-122.2	0.47 (1)	3.32	F- V	0 / 1708	0.38 (1)
E- F	-4544 / 0	-122.2	-122.2	0.53 (1)	2.93	S- K	0 / 149	0.03 (2)
F- G	-6235 / 0	-122.2	-122.2	0.79 (1)	2.24	S- L	-33 / 2	0.01 (1)
G- H	-6235 / 0	-122.2	-122.2	0.79 (1)	2.24	R- L	-223 / 0	0.04 (1)
H- I	-6273 / 0	-122.2	-122.2	0.74 (1)	2.31	R- M	0 / 2061	0.46 (1)
I- J	-5652 / 0	-122.2	-122.2	0.71 (1)	2.44	Q- M	-2561 / 0	0.43 (1)
J- K	-4462 / 0	-122.2	-122.2	0.54 (1)	2.94	B-AB	0 / 2186	0.49 (1)
K- L	-3404 / 0	-122.2	-122.2	0.17 (1)	3.69	Q- N	0 / 1918	0.43 (1)
L- M	-3359 / 0	-122.2	-122.2	0.21 (1)	3.66	Z- F	-741 / 0	0.50 (1)
M- N	-1989 / 0	-122.2	-122.2	0.20 (1)	4.59	C-AA	0 / 2341	0.53 (1)
N- O	0 / 54	-122.2	-122.2	0.17 (1)	10.00	Z- E	0 / 670	0.15 (1)
AC- B	-2816 / 0	0.0	0.0	0.20 (1)	6.24	AA- D	-534 / 0	0.21 (1)
P- N	-2849 / 0	0.0	0.0	0.20 (1)	6.21	AA- E	-1480 / 0	0.40 (1)
						T- K	0 / 2562	0.58 (1)
AC-AB	0 / 0	-28.0	-28.0	0.12 (2)	10.00	V- I	0 / [illegible]	0.20 (1)
AB-AA	0 / 2079	-28.0	-28.0	0.41 (1)	10.00	T- J	-1686 / 0	0.49 (1)
AA- Z	0 / 4544	-28.0	-28.0	0.79 (1)	10.00	U- I	-1073 / 0	0.27 (1)
Z- Y	0 / 5017	-28.0	-28.0	0.87 (1)	10.00	U- J	0 / 1677	0.38 (1)
Y- X	0 / 5017	-28.0	-28.0	0.87 (1)	10.00			
X- W	0 / 147	-28.0	-28.0	0.12 (2)	10.00			
W- V	0 / 75	0.0	0.0	0.21 (1)	10.00			
V- H	-457 / 0	0.0	0.0	0.22 (1)	7.81			
V- U	0 / 5652	-28.0	-28.0	0.97 (1)	10.00			
U- T	0 / 4462	-28.0	-28.0	0.77 (1)	10.00			
T- S	0 / 2601	-28.0	-28.0	0.47 (1)	10.00			
S- R	0 / 2617	-28.0	-28.0	0.48 (1)	10.00			
R- Q	0 / 1912	-28.0	-28.0	0.31 (1)	10.00			
Q- P	0 / 0	-28.0	-28.0	0.02 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 922 (0.47")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 606 (0.71")

CSI: TC=0.79 (F-H:1) , BC=0.97 (U-V:1) , WB=0.81 (V-X:1) , SSI=0.23 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.96 (Y) (INPUT = 1.00)

DWG NO. TAM47673-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288285	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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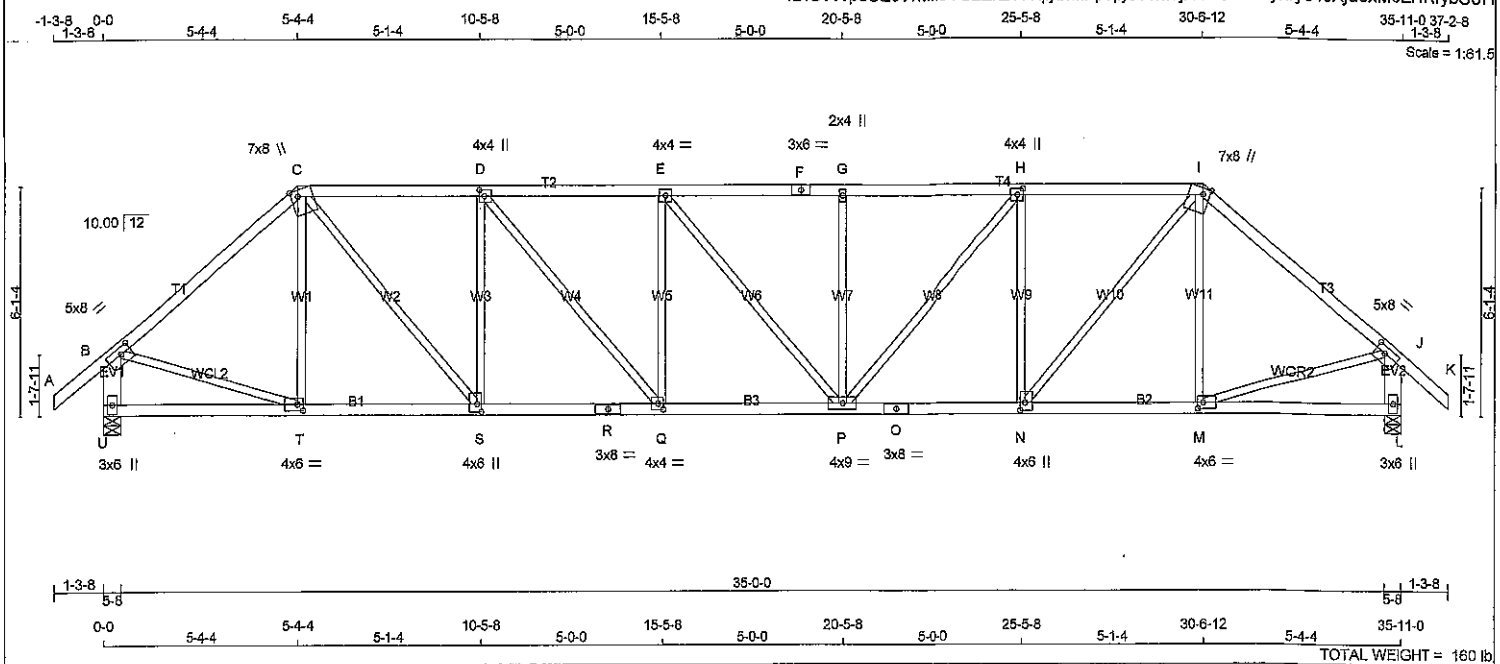

TOTAL WEIGHT = 160 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW+t	MT20	4.0	4.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	4.0	2.00	1.75
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW-t	MT20	5.0	8.0	2.00	3.25
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	1.75
N	BMWW+t	MT20	4.0	6.0	2.50	1.50
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.75
R	BS-t	MT20	3.0	8.0		
S	BMWW+t	MT20	4.0	6.0	2.50	1.50
T	BMWW-t	MT20	4.0	6.0	2.00	1.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2867	0	2867	0	0	5-8	5-8
L	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
L	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B-C	-2797 / 0	-122.2	-122.2	0.98 (1)	3.14
C-D	-3501 / 0	-122.2	-122.2	0.71 (1)	3.09
D-E	-4133 / 0	-122.2	-122.2	0.80 (1)	2.76
E-F	-4131 / 0	-122.2	-122.2	0.66 (1)	2.97
F-G	-4131 / 0	-122.2	-122.2	0.66 (1)	2.97
G-H	-4131 / 0	-122.2	-122.2	0.80 (1)	2.76
H-I	-3501 / 0	-122.2	-122.2	0.71 (1)	3.09
I-J	-2797 / 0	-122.2	-122.2	0.98 (1)	3.14
J-K	0 / 54	-122.2	-122.2	0.17 (1)	10.00
U-B	-2805 / 0	0.0	0.0	0.19 (1)	6.25
L-J	-2805 / 0	0.0	0.0	0.19 (1)	6.25
U-T	0 / 0	-28.0	-28.0	0.20 (3)	10.00
T-S	0 / 2139	-28.0	-28.0	0.45 (1)	10.00
S-R	0 / 3501	-28.0	-28.0	0.62 (1)	10.00
R-Q	0 / 3501	-28.0	-28.0	0.62 (1)	10.00
Q-P	0 / 4133	-28.0	-28.0	0.73 (1)	10.00
P-O	0 / 3502	-28.0	-28.0	0.63 (1)	10.00
O-N	0 / 3502	-28.0	-28.0	0.63 (1)	10.00
N-M	0 / 2138	-28.0	-28.0	0.45 (1)	10.00
M-L	0 / 0	-28.0	-28.0	0.20 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-343 / 97	0.20 (1)
C-S	0 / 2061	0.46 (1)
S-D	-1415 / 0	0.83 (1)
D-Q	0 / 969	0.22 (1)
Q-E	-593 / 0	0.35 (1)
E-P	-3 / 0	0.00 (1)
P-G	-591 / 0	0.35 (1)
P-H	0 / 965	0.22 (1)
N-H	-1414 / 0	0.83 (1)
N-I	0 / 2062	0.46 (1)
M-I	-344 / 97	0.20 (1)
B-T	0 / 2205	0.50 (1)
M-J	0 / 2205	0.50 (1)

DWG NO. TAM 47696 17
STRUCTURAL
COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 998 (0.43")

CSI: TC=0.98 (B-C:1) , BC=0.73 (P-Q:1) , WB=0.83 (D-S:1) , SSI=0.29 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.94 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T8S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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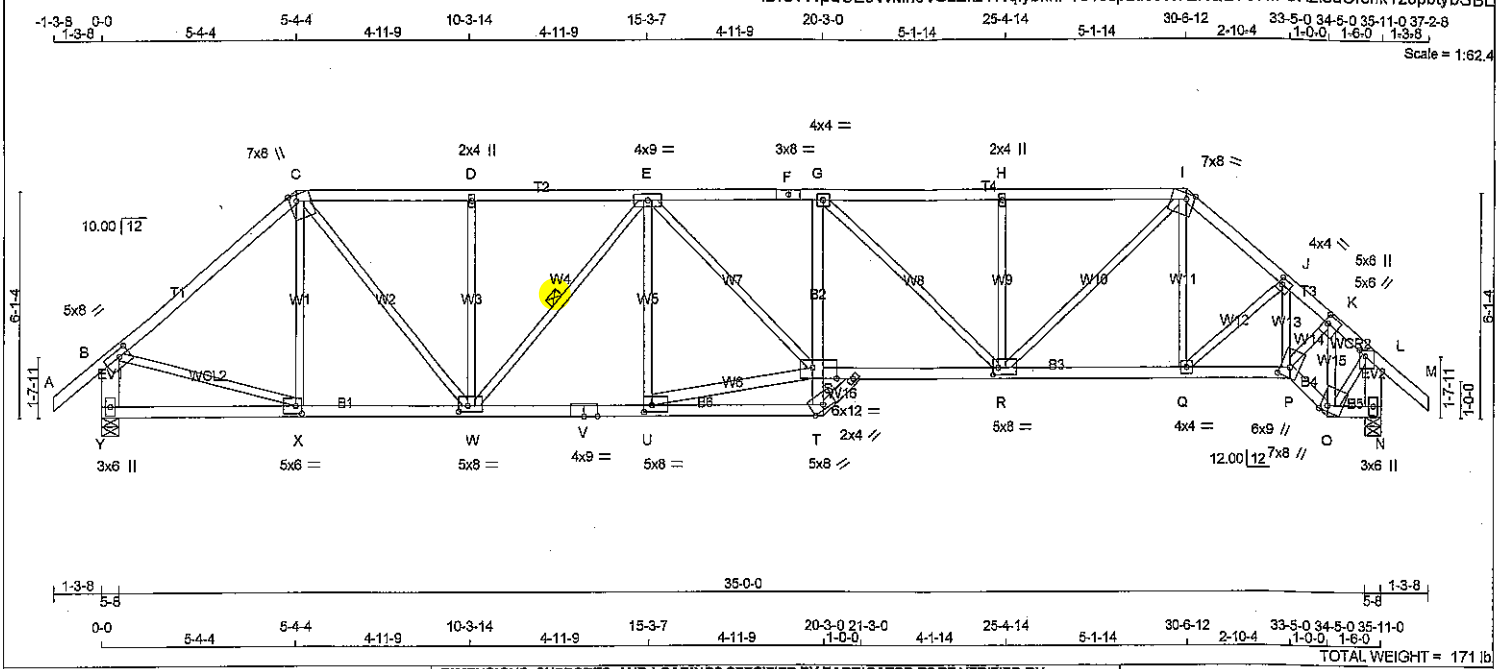


TOTAL WEIGHT = 171 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - G	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	4.0	9.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW-t	MT20	4.0	4.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	7.0	8.0	Edge	2.75
J	TMWW-t	MT20	4.0	4.0	2.00	1.25
K	TMWW+t	MT20	5.0	6.0	2.75	1.25
L	TMVW+p	MT20	5.0	6.0	2.00	2.00
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	7.0	8.0	Edge	2.25
P	BBWW+m	MT20	6.0	9.0	3.25	3.00
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.25	1.75
S	BVMWW-t	MT20	6.0	12.0	Edge	7.75
T	BVW+-m	MT20	5.0	8.0	Edge	4.00
U	BMWW-t	MT20	5.0	8.0	2.25	2.50
V	BS-t	MT20	4.0	9.0		
W	BMWWW-t	MT20	5.0	8.0	2.00	3.00
X	BMWW-t	MT20	5.0	6.0	2.50	2.00
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	2864	0	2864	0	0	5-8	5-8
N	2870	0	2870	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2222	1477 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
N	2226	1481 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

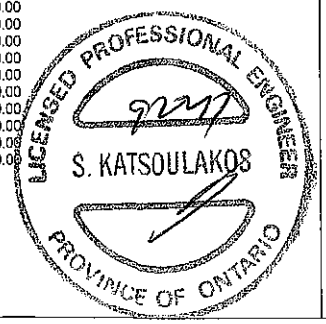
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-2794 / 0	-122.2	-122.2	0.97 (1)	3.14
C- D	-3473 / 0	-122.2	-122.2	0.68 (1)	3.14
D- E	-3474 / 0	-122.2	-122.2	0.67 (1)	3.14
E- F	-4991 / 0	-122.2	-122.2	0.77 (1)	2.60
F- G	-4991 / 0	-122.2	-122.2	0.77 (1)	2.60
G- H	-4256 / 0	-122.2	-122.2	0.88 (1)	2.64
H- I	-4256 / 0	-122.2	-122.2	0.89 (1)	2.64
I- J	-3352 / 0	-122.2	-122.2	0.26 (1)	3.64
J- K	-3350 / 0	-122.2	-122.2	0.23 (1)	3.65
K- L	-1987 / 0	-122.2	-122.2	0.20 (1)	4.59
L- M	0 / 54	-122.2	-122.2	0.17 (1)	10.00
Y- B	-2803 / 0	0.0	0.0	0.19 (1)	6.26
N- L	-2849 / 0	0.0	0.0	0.20 (1)	6.21
Y- X	0 / 0	-28.0	-28.0	0.20 (2)	10.00
X- W	0 / 2136	-28.0	-28.0	0.46 (1)	10.00
W- V	0 / 4116	-28.0	-28.0	0.75 (1)	10.00
V- U	0 / 4116	-28.0	-28.0	0.75 (1)	10.00
U- T	0 / 105	-28.0	-28.0	0.17 (2)	10.00
T- S	0 / 93	0.0	0.0	0.15 (1)	10.00
S- G	0 / 214	0.0	0.0	0.16 (1)	10.00
S- R	0 / 5013	-28.0	-28.0	0.90 (1)	10.00
R- Q	0 / 2558	-28.0	-28.0	0.49 (1)	10.00
Q- P	0 / 2642	-28.0	-28.0	0.50 (1)	10.00
P- O	0 / 1905	-28.0	-28.0	0.31 (1)	10.00
O- N	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-339 / 98	0.20 (1)
C- W	0 / 2043	0.46 (1)
W- D	-667 / 0	0.39 (1)
W- E	-995 / 0	0.38 (1)
U- E	-691 / 0	0.40 (1)
U- S	0 / 4100	0.66 (1)
E- S	0 / 1234	0.28 (1)
G- R	-1032 / 0	0.97 (1)
R- H	-709 / 0	0.27 (1)
R- I	0 / 2296	0.52 (1)
Q- I	0 / 236	0.05 (2)
Q- J	-116 / 0	0.03 (1)
P- J	-261 / 0	0.04 (1)
P- K	0 / 2108	0.47 (1)
O- K	-2551 / 0	0.43 (1)
B- X	0 / 2203	0.50 (1)
O- L	0 / 1910	0.43 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 843 (0.51")

CSI: TC=0.97 (B-C:1) , BC=0.90 (R-S:1) , WB=0.97 (G-R:1) , SSI=0.30 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.79 (V) (INPUT = 1.00)

DWG NO. TAM 42674-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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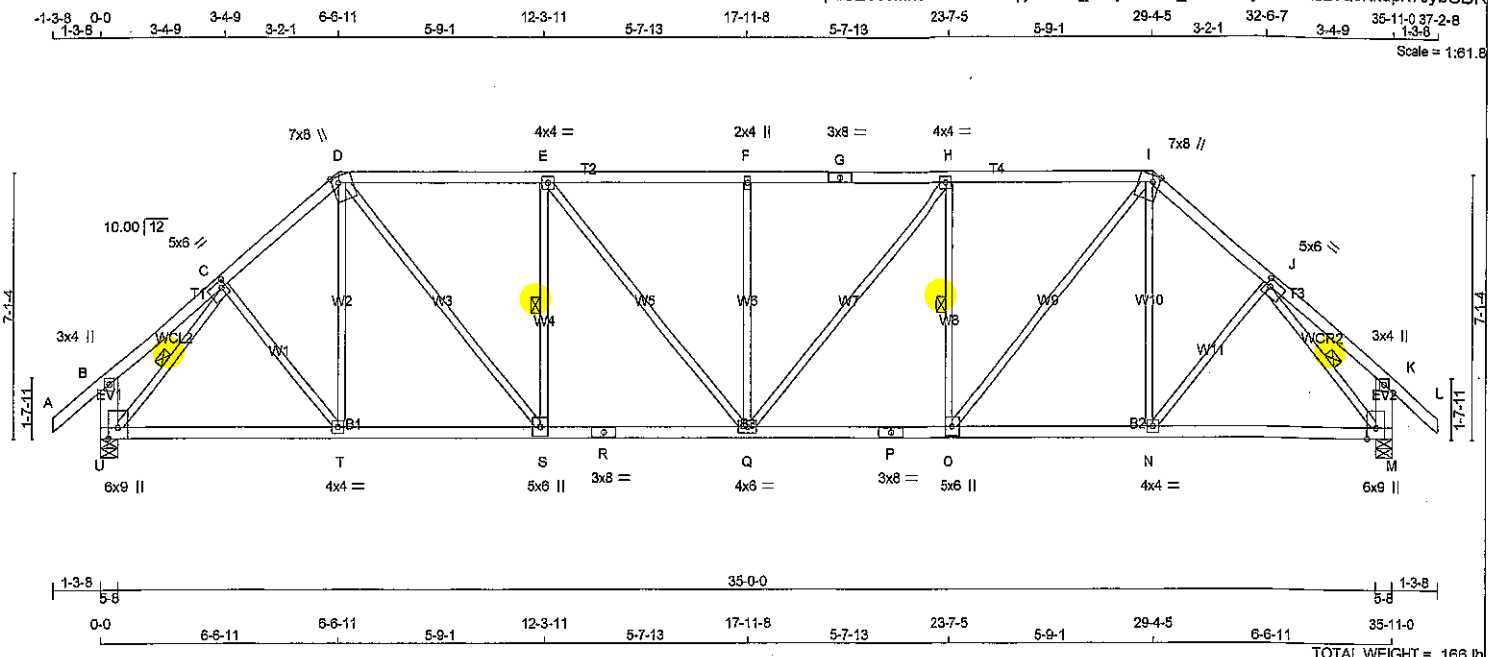

TOTAL WEIGHT = 166 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.50
K	TMV+p	MT20	3.0	4.0		
M	BMWW1+p	MT20	6.0	9.0	Edge	
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	8.0		
S	BMWW+t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMWW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2867	0	2867	0	0	5-8	5-8
M	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
M	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 25	-122.2	-122.2	0.18 (1)	10.00
C- D	-2629 / 0	-122.2	-122.2	0.30 (1)	3.89
D- E	-3273 / 0	-122.2	-122.2	0.90 (1)	2.92
E- F	-3603 / 0	-122.2	-122.2	0.95 (1)	2.72
F- G	-3603 / 0	-122.2	-122.2	0.95 (1)	2.72
G- H	-3603 / 0	-122.2	-122.2	0.95 (1)	2.72
H- I	-3273 / 0	-122.2	-122.2	0.90 (1)	2.92
I- J	-2629 / 0	-122.2	-122.2	0.30 (1)	3.89
J- K	0 / 25	-122.2	-122.2	0.18 (1)	10.00
K- L	0 / 54	-122.2	-122.2	0.17 (1)	10.00
U- B	-328 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-328 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 1994	-28.0	-28.0	0.48 (2)	10.00
T- S	0 / 2147	-28.0	-28.0	0.50 (2)	10.00
S- R	0 / 3273	-28.0	-28.0	0.61 (1)	10.00
R- Q	0 / 3273	-28.0	-28.0	0.61 (1)	10.00
Q- P	0 / 3273	-28.0	-28.0	0.61 (1)	10.00
P- O	0 / 3273	-28.0	-28.0	0.61 (1)	10.00
O- N	0 / 2147	-28.0	-28.0	0.50 (2)	10.00
N- M	0 / 1994	-28.0	-28.0	0.48 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 236	0.05 (1)
T- D	0 / 202	0.05 (3)
D- S	0 / 1741	0.39 (1)
S- E	-1181 / 0	0.38 (1)
E- Q	0 / 517	0.12 (1)
Q- F	-636 / 0	0.56 (1)
Q- H	0 / 517	0.12 (1)
O- H	-1181 / 0	0.38 (1)
O- I	0 / 1741	0.39 (1)
N- I	0 / 202	0.05 (3)
N- J	0 / 236	0.05 (1)
U- C	-3169 / 0	0.65 (1)
J- M	-3169 / 0	0.65 (1)

LICENSED PROFESSIONAL ENGINEER
97217
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.95 (F-H:1) , BC=0.61 (O-Q:1) , WB=0.65 (J-M:1) , SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.89 (P) (INPUT = 1.00)

DWG NO. TAM 47605-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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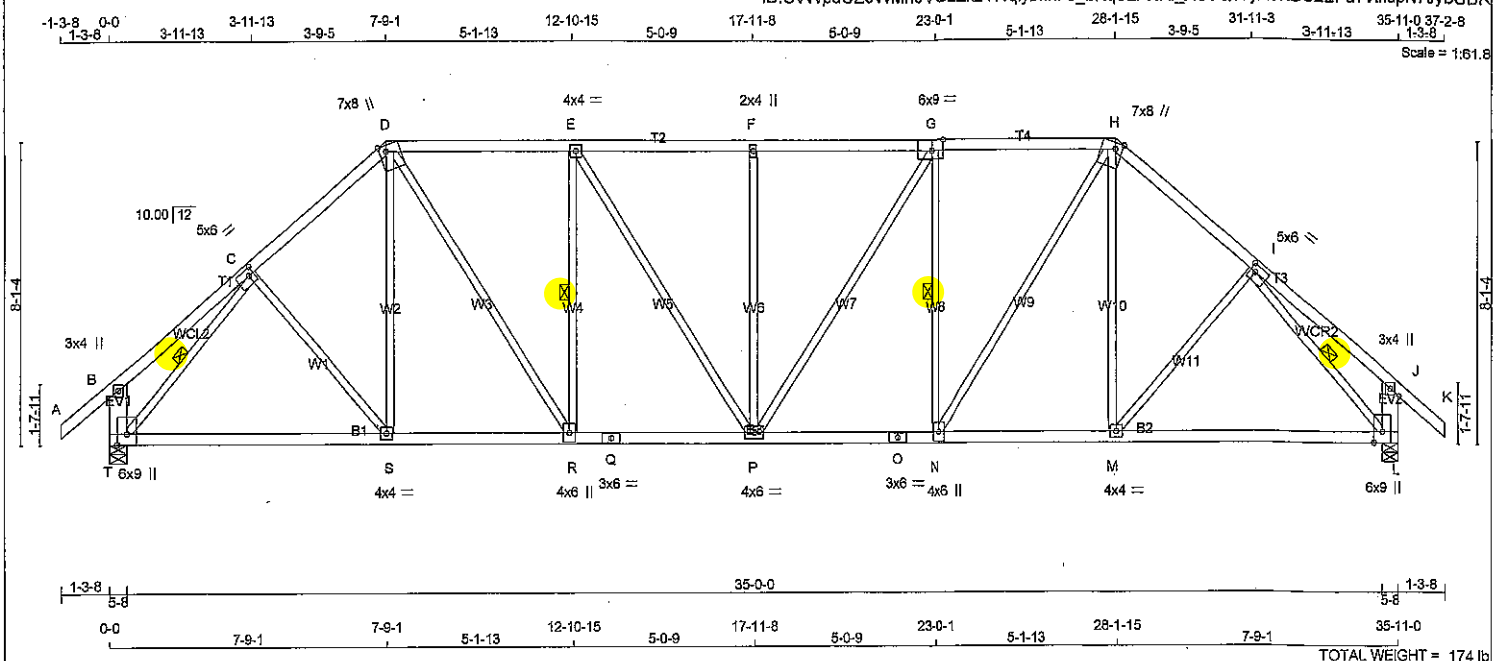

TOTAL WEIGHT = 174 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TSWW-l	MT20	6.0	9.0	3.25	4.25
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0	2.25	1.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1+p	MT20	6.0	9.0	Edge	
M	BMWW-t	MT20	4.0	4.0		
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW+t	MT20	4.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMVW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2867	0	2867	0	0	5-8	5-8
L	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
L	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, G-N, C-T, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 33	-122.2	-122.2	0.27 (1)	10.00
C- D	-2606 / 0	-122.2	-122.2	0.42 (1)	3.78
D- E	-2902 / 0	-122.2	-122.2	0.66 (1)	3.40
E- F	-3132 / 0	-122.2	-122.2	0.68 (1)	3.25
F- G	-3132 / 0	-122.2	-122.2	0.68 (1)	3.25
G- H	-2897 / 0	-122.2	-122.2	0.67 (1)	3.39
H- I	-2808 / 0	-122.2	-122.2	0.42 (1)	3.78
I- J	0 / 33	-122.2	-122.2	0.27 (1)	10.00
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00
T- B	-353 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-353 / 0	0.0	0.0	0.02 (1)	7.81
T- S	0 / 2067	-28.0	-28.0	0.57 (2)	10.00
S- R	0 / 2126	-28.0	-28.0	0.59 (2)	10.00
R- Q	0 / 2902	-28.0	-28.0	0.54 (1)	10.00
Q- P	0 / 2902	-28.0	-28.0	0.54 (1)	10.00
P- O	0 / 2911	-28.0	-28.0	0.54 (1)	10.00
O- N	0 / 2911	-28.0	-28.0	0.54 (1)	10.00
N- M	0 / 2126	-28.0	-28.0	0.59 (2)	10.00
M- L	0 / 2067	-28.0	-28.0	0.57 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	0 / 161	0.04 (3)
S- D	0 / 276	0.06 (2)
D- R	0 / 1406	0.32 (1)
R- E	-1059 / 0	0.44 (1)
E- P	0 / 424	0.10 (1)
P- F	-555 / 0	0.70 (1)
P- G	0 / 413	0.09 (1)
N- G	-1052 / 0	0.44 (1)
N- H	0 / 1397	0.31 (1)
M- H	0 / 276	0.06 (2)
M- I	0 / 161	0.04 (3)
T- C	-3185 / 0	0.78 (1)
I- L	-3185 / 0	0.78 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.68 (E-F:1) , BC=0.59 (R-S:2) , WB=0.78 (I-L:1) , SSI=0.30 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.79 (C) (INPUT = 1.00)

DWG NO. TAM42676-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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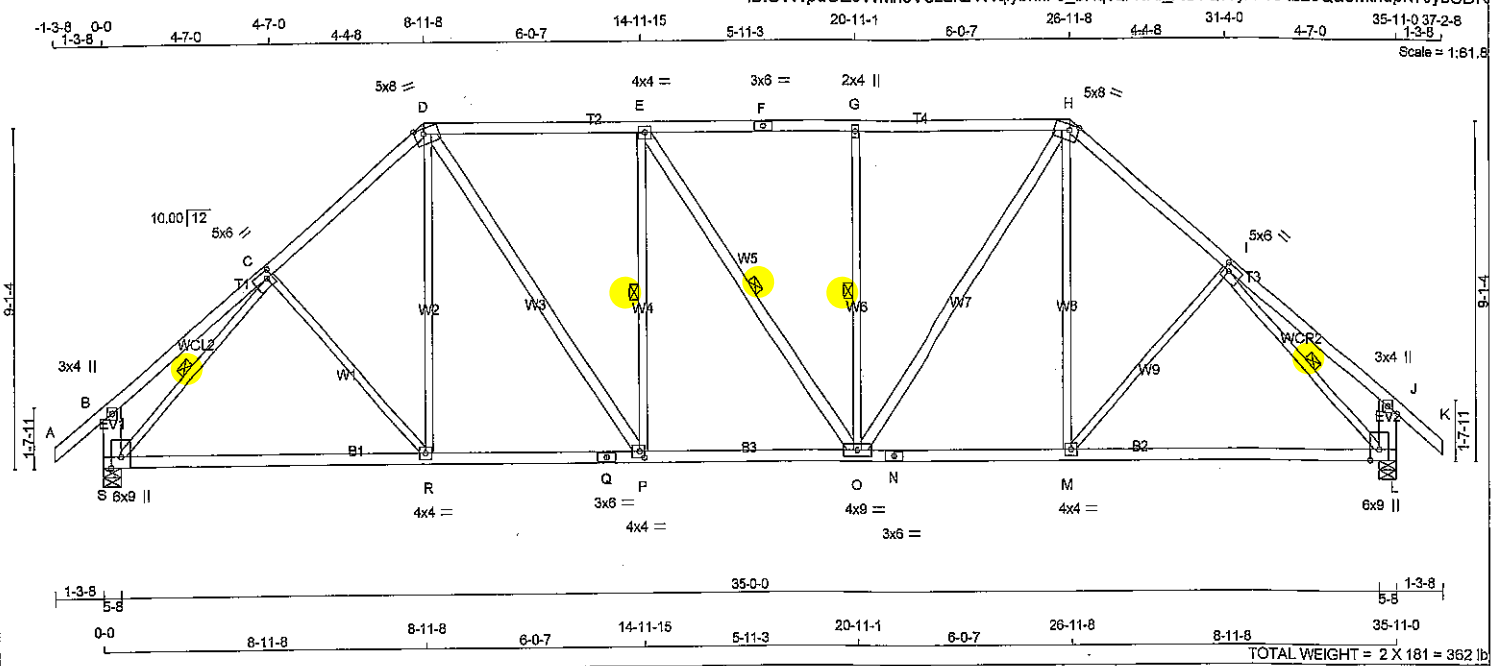


TOTAL WEIGHT = 2 X 181 = 362 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
E - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMWW-t	MT20	5.0	6.0	2.25	1.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1+p	MT20	6.0	9.0	Edge	
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2867	0	2867	0	0	5-8	5-8
L	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
L	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, E-O, G-O, C-S, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- R	-47 / 132	0.04 (1)
B- C	0 / 40	-122.2	-122.2	0.38 (1)	10.00	R- D	0 / 384	0.09 (2)
C- D	-2759 / 0	-122.2	-122.2	0.59 (1)	3.64	D- P	0 / 1134	0.18 (1)
D- E	-2729 / 0	-122.2	-122.2	0.86 (1)	3.21	P- E	-791 / 0	0.43 (1)
E- F	-2727 / 0	-122.2	-122.2	0.85 (1)	3.21	E- O	-3 / 0	0.00 (1)
F- G	-2727 / 0	-122.2	-122.2	0.85 (1)	3.21	O- G	-790 / 0	0.43 (1)
G- H	-2727 / 0	-122.2	-122.2	0.84 (1)	3.24	O- H	0 / 1130	0.18 (1)
H- I	-2759 / 0	-122.2	-122.2	0.59 (1)	3.64	M- H	0 / 384	0.09 (2)
I- J	0 / 40	-122.2	-122.2	0.38 (1)	10.00	M- I	-47 / 132	0.04 (1)
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00	S- C	-3187 / 0	0.94 (1)
S- B	-379 / 0	0.0	0.0	0.03 (1)	7.81	I- L	-3187 / 0	0.94 (1)
L- J	-379 / 0	0.0	0.0	0.03 (1)	7.81			
S- R	0 / 2116	-28.0	-28.0	0.70 (2)	10.00			
R- Q	0 / 2087	-28.0	-28.0	0.71 (2)	10.00			
Q- P	0 / 2087	-28.0	-28.0	0.71 (2)	10.00			
P- O	0 / 2729	-28.0	-28.0	0.54 (1)	10.00			
O- N	0 / 2087	-28.0	-28.0	0.71 (2)	10.00			
N- M	0 / 2087	-28.0	-28.0	0.71 (2)	10.00			
M- L	0 / 2116	-28.0	-28.0	0.70 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 903 (0.48")

CSI: TC=0.86 (D-E:1) , BC=0.71 (M-O:2) , WB=0.94 (I-L:1) , SSI=0.34 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

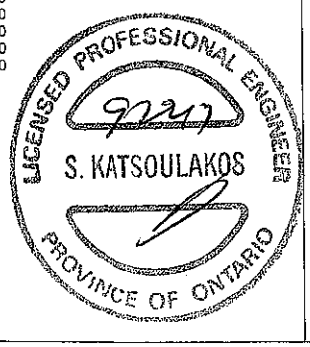
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.79 (I) (INPUT = 1.00)



DWG NO. TAM 42677-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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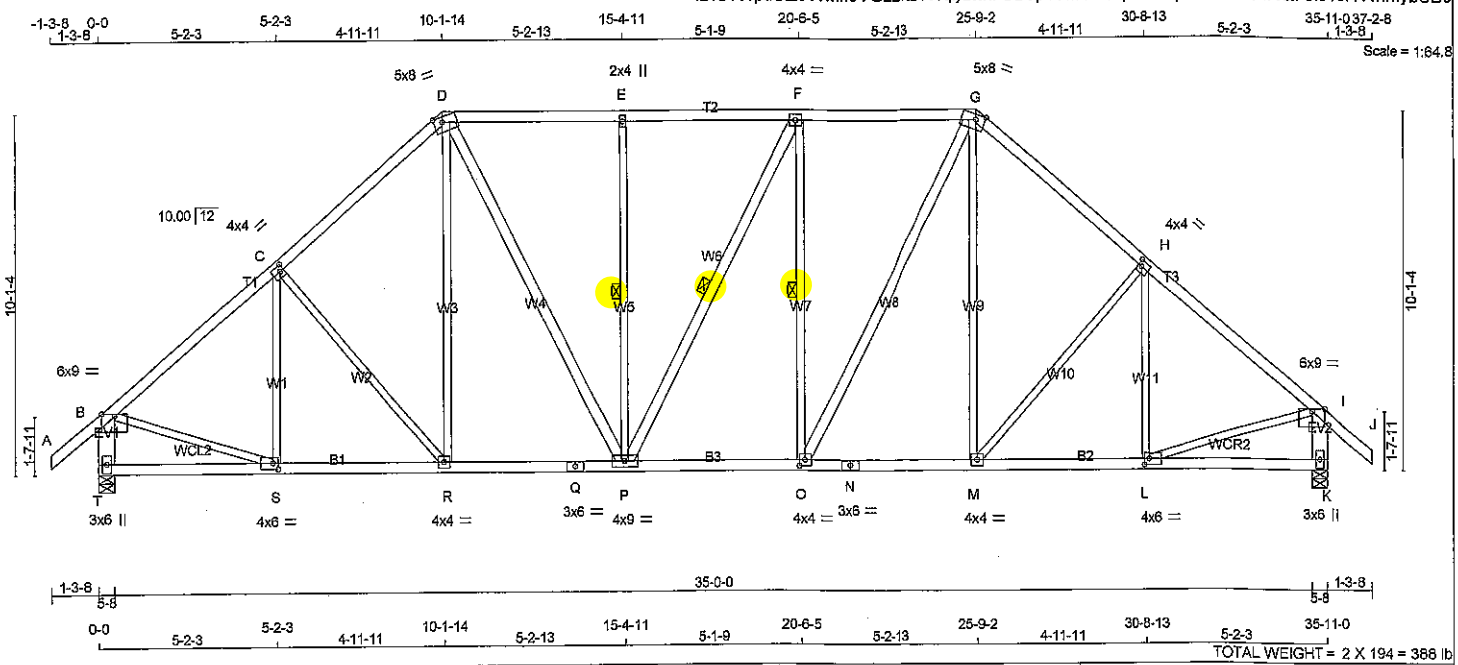


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	1.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	4.0	6.0	2.00	1.75
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
T	2867	0	2867	0	0	IN-SX 5-8	IN-SX 5-8
K	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
K	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, F-P, F-O.

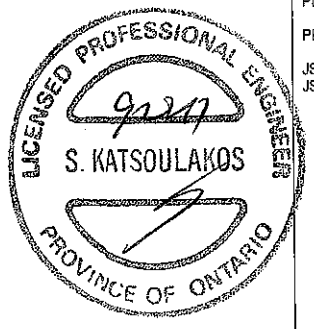
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B-C	-2823 / 0	-122.2	-122.2	0.57 (1)		3.63
C-D	-2671 / 0	-122.2	-122.2	0.53 (1)		3.74
D-E	-2455 / 0	-122.2	-122.2	0.46 (1)		3.94
E-F	-2455 / 0	-122.2	-122.2	0.43 (1)		3.98
F-G	-2457 / 0	-122.2	-122.2	0.46 (1)		3.94
G-H	-2671 / 0	-122.2	-122.2	0.53 (1)		3.74
H-I	-2823 / 0	-122.2	-122.2	0.57 (1)		3.63
I-J	0 / 54	-122.2	-122.2	0.17 (1)		10.00
T-B	-2806 / 0	0.0	0.0	0.19 (1)		6.25
K-I	-2806 / 0	0.0	0.0	0.19 (1)		6.25
T-S	0 / 0	-28.0	-28.0	0.17 (3)		10.00
S-R	0 / 2205	-28.0	-28.0	0.43 (1)		10.00
R-Q	0 / 2014	-28.0	-28.0	0.40 (1)		10.00
Q-P	0 / 2014	-28.0	-28.0	0.40 (1)		10.00
P-O	0 / 2457	-28.0	-28.0	0.48 (1)		10.00
O-N	0 / 2014	-28.0	-28.0	0.41 (1)		10.00
N-M	0 / 2014	-28.0	-28.0	0.41 (1)		10.00
M-L	0 / 2205	-28.0	-28.0	0.43 (1)		10.00
L-K	0 / 0	-28.0	-28.0	0.17 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-C	-401 / 74	0.22 (1)
C-R	-290 / 0	0.33 (1)
R-D	0 / 371	0.08 (2)
D-P	0 / 936	0.15 (1)
P-E	-681 / 0	0.48 (1)
P-F	-5 / 0	0.00 (1)
O-F	-683 / 0	0.48 (1)
O-G	0 / 941	0.15 (1)
M-G	0 / 370	0.08 (2)
M-H	-291 / 0	0.33 (1)
L-H	-400 / 74	0.22 (1)
B-S	0 / 2273	0.51 (1)
L-I	0 / 2273	0.51 (1)



[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	36.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.57 (H-I:1) , BC=0.48 (O-P:1) , WB=0.51 (I-L:1) , SSI=0.30 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.55 (L) (INPUT = 1.00)

DWG NO. TAM 4767B-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T13S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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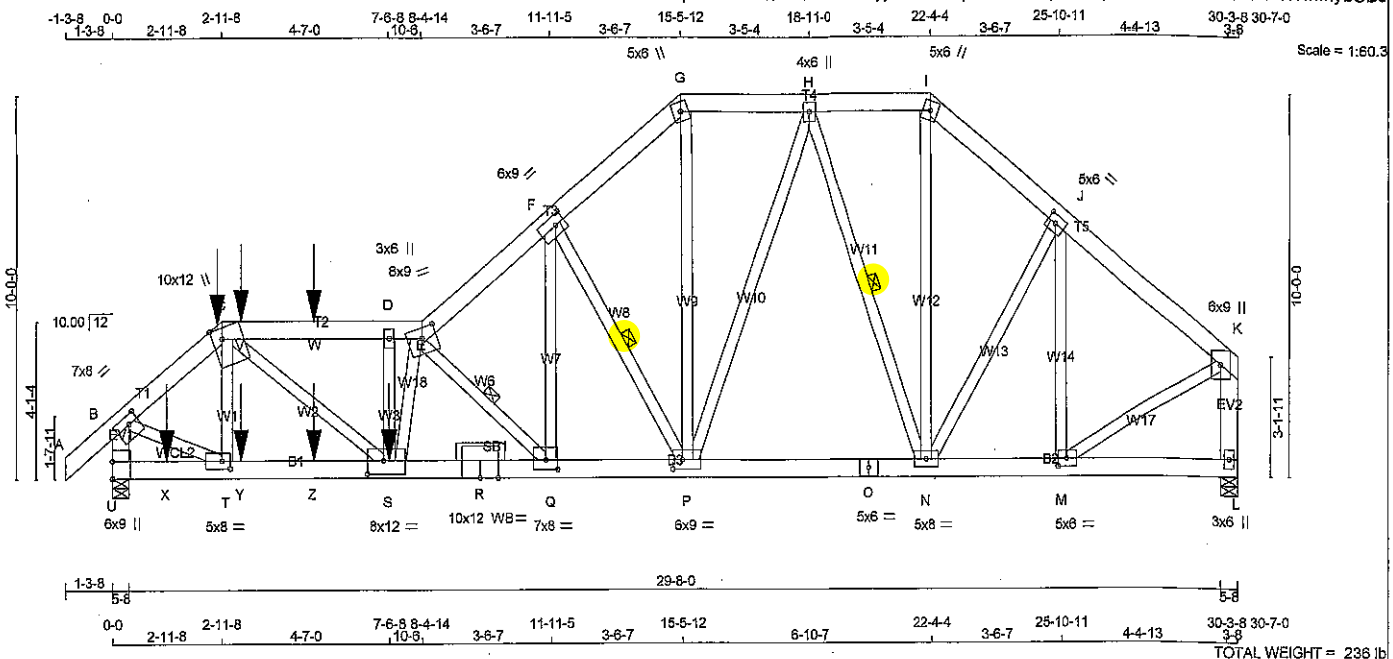


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	1650F 1.5E	SPF
R - O	2x6	DRY	1650F 1.5E	SPF
O - L	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	7.0	8.0	2.75	3.25
C	TTWW+m	MT20	10.0	12.0	Edge	3.25
D	TMW+w	MT20	3.0	6.0		
E	TTWW-m	MT20	8.0	9.0	3.25	4.50
F	TMWW-t	MT20	6.0	9.0	3.00	3.25
G	TTW+m	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTW+m	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMVW+p	MT20	6.0	9.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	9.0	3.00	3.00
Q	BMWW-t	MT20	7.0	8.0	3.00	4.00
R	BS-t	MT20	10.0	12.0		
S	BMWWW-t	MT20	8.0	12.0	4.25	5.25
T	BMWW-t	MT20	5.0	8.0	2.50	2.75
U	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 259.3 lbs FACTORED DOWN AT 2-11-8, AND 176.3 lbs FACTORED DOWN AT 3-5-12, AND 147.1 lbs FACTORED DOWN AT 5-5-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-5-12, 69.9 lbs FACTORED DOWN AT 3-5-12, AND 69.9 lbs FACTORED DOWN AT 5-5-12, AND 2538.1 lbs FACTORED DOWN AT 7-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	5003	0	5003	0	0	5-8	5-8
L	3008	0	3008	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3869	2594 / 0	643 / 0	0 / 0	0 / 0	632 / 0	0 / 0
L	2345	1540 / 0	411 / 0	0 / 0	0 / 0	395 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, F-P, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 56	-122.2	-122.2	0.10 (1)	10.00		T- C	-1052 / 0	0.18 (1)
B- C	-5044 / 0	-122.2	-122.2	0.23 (1)	3.69		C- S	0 / 5394	0.95 (1)
C- V	-8048 / 0	-122.2	-122.2	0.70 (1)	2.38		S- D	-265 / 171	0.05 (1)
V- W	-8048 / 0	-122.2	-122.2	0.70 (1)	2.38		E- Q	-5112 / 0	0.79 (1)
W- D	-8048 / 0	-122.2	-122.2	0.70 (1)	2.38		Q- F	0 / 3715	0.66 (1)
D- E	-8047 / 0	-122.2	-122.2	0.43 (1)	2.72		F- P	-3350 / 0	0.90 (1)
E- F	-5927 / 0	-122.2	-122.2	0.29 (1)	3.38		P- G	0 / 2072	0.37 (1)
F- G	-3874 / 0	-122.2	-122.2	0.18 (1)	4.21		P- H	0 / 1141	0.20 (1)
G- H	-2977 / 0	-122.2	-122.2	0.17 (1)	4.71		H- N	-1554 / 0	0.81 (1)
H- I	-2099 / 0	-122.2	-122.2	0.14 (1)	5.43		N- I	0 / 1375	0.24 (1)
I- J	-2756 / 0	-122.2	-122.2	0.20 (1)	4.82		N- J	0 / 267	0.05 (1)
J- K	-2505 / 0	-122.2	-122.2	0.22 (1)	4.99		M- J	-969 / 0	0.53 (1)
U- B	-4995 / 0	0.0	0.0	0.36 (1)	4.78		B- T	0 / 4093	0.72 (1)
L- K	-2948 / 0	0.0	0.0	0.31 (1)	6.08		M- K	0 / 2228	0.39 (1)
							S- E	-522 / 0	0.09 (1)
U- X	0 / 0	-28.0	-28.0	0.09 (2)	10.00				
X- T	0 / 0	-28.0	-28.0	0.09 (2)	10.00				
T- Y	0 / 3810	-28.0	-28.0	0.44 (1)	10.00				
Y- Z	0 / 3810	-28.0	-28.0	0.44 (1)	10.00				
Z- S	0 / 3810	-28.0	-28.0	0.44 (1)	10.00				
S- R	0 / 8169	-28.0	-28.0	0.80 (1)	10.00				
R- Q	0 / 8169	-28.0	-28.0	0.80 (1)	10.00				
Q- P	0 / 4592	-28.0	-28.0	0.45 (1)	10.00				
P- O	0 / 2605	-28.0	-28.0	0.29 (1)	10.00				
O- N	0 / 2605	-28.0	-28.0	0.29 (1)	10.00				
N- M	0 / 1946	-28.0	-28.0	0.19 (1)	10.00				
M- L	0 / 0	-28.0	-28.0	0.05 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	2-11-8	-16	-18	---	FRONT	VERT	DEAD
C	2-11-8	-243	-243	---	FRONT	VERT	SNOW
S	7-6-8	-2538	-2538	---	FRONT	VERT	TOTAL
V	3-5-12	-176	-176	---	FRONT	VERT	TOTAL
W	5-5-12	-147	-147	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 844 (0.43")

CSI: TC=0.70 (C-D:1) , BC=0.80 (Q-S:1) , WB=0.95 (C-S:1) , SSI=0.31 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.99 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47629-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T13S	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

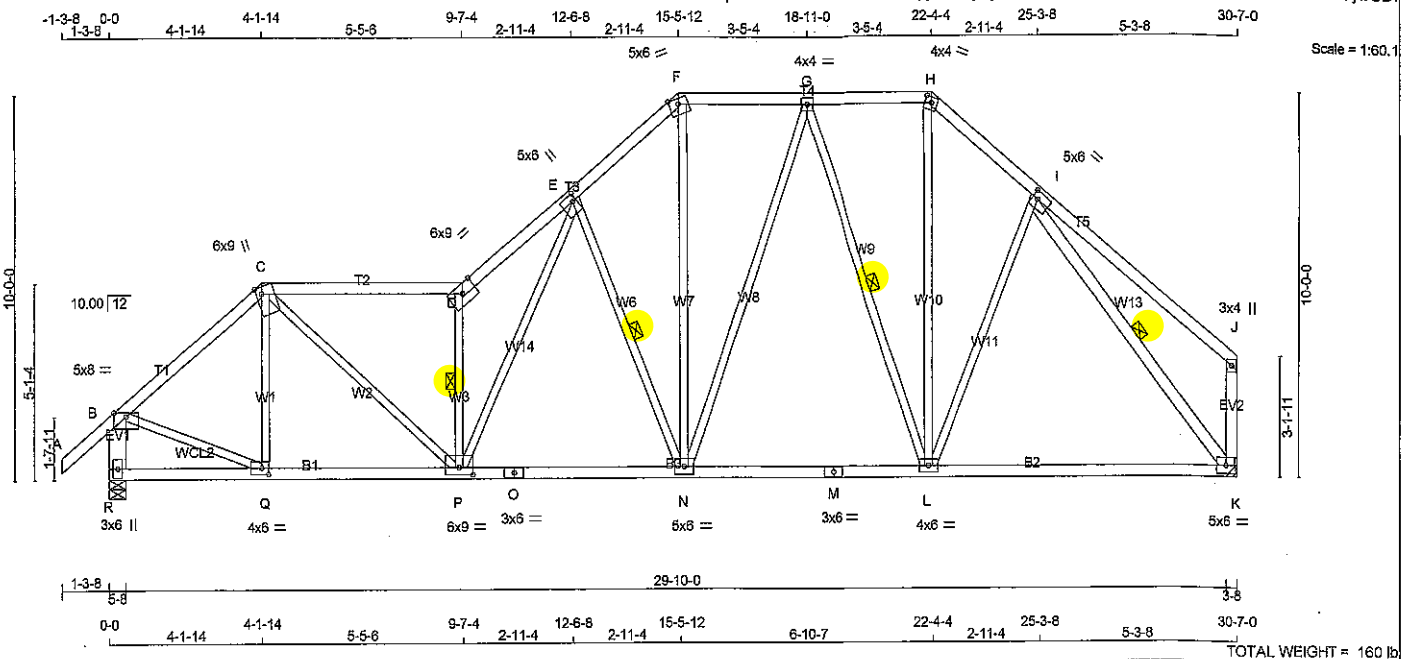
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	1-5-12	-40	-70	---	FRONT	VERT	TOTAL
Y	3-5-12	-40	-70	---	FRONT	VERT	TOTAL
Z	5-5-12	-40	-70	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9211
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4767F17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T14S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TTW-h	MT20	6.0	9.0	Edge	
E	TMWW+t	MT20	5.0	6.0	2.25	1.25
F	TTW-m	MT20	5.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	4.0	1.75	2.00
I	TMWW-t	MT20	5.0	6.0	2.25	1.75
J	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	6.0	9.0	2.25	4.50
Q	BMWW-t	MT20	4.0	6.0	2.00	2.25
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2466	0	2466	0	0	5-8	5-8
K	2297	0	2297	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1911	1275 / 0	321 / 0	0 / 0	0 / 0	314 / 0	0 / 0
K	1798	1170 / 0	321 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-P, E-N, G-L, I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

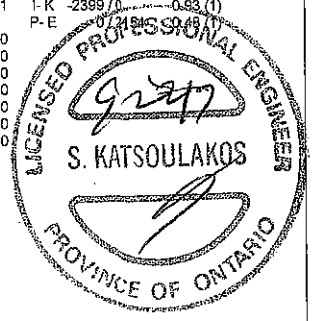
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	-2280 / 0	-122.2	-122.2	0.35 (1)	4.22	
C- D	-3137 / 0	-122.2	-122.2	0.61 (1)	3.32	
D- E	-4124 / 0	-122.2	-122.2	0.35 (1)	3.23	
E- F	-2392 / 0	-122.2	-122.2	0.22 (1)	4.27	
F- G	-1820 / 0	-122.2	-122.2	0.19 (1)	4.60	
G- H	-1454 / 0	-122.2	-122.2	0.17 (1)	5.26	
H- I	-1949 / 0	-122.2	-122.2	0.35 (1)	4.47	
I- J	0 / 36	-122.2	-122.2	0.40 (1)	10.00	
R- B	-2424 / 0	0.0	0.0	0.17 (1)	6.62	
K- J	-264 / 0	0.0	0.0	0.05 (1)	7.81	
R- Q	0 / 0	-28.0	-28.0	0.15 (2)	10.00	
Q- P	0 / 1739	-28.0	-28.0	0.38 (1)	10.00	
P- O	0 / 2377	-28.0	-28.0	0.50 (1)	10.00	
O- N	0 / 2377	-28.0	-28.0	0.50 (1)	10.00	
N- M	0 / 1720	-28.0	-28.0	0.61 (2)	10.00	
M- L	0 / 1720	-28.0	-28.0	0.61 (2)	10.00	
L- K	0 / 1415	-28.0	-28.0	0.58 (2)	10.00	

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 1877	0.42 (1)
P- D	-3106 / 0	0.62 (1)
E- N	-1437 / 0	0.60 (1)
N- F	0 / 1205	0.27 (1)
N- G	0 / 308	0.07 (1)
G- L	-818 / 0	0.64 (1)
L- H	0 / 950	0.21 (1)
L- I	0 / 181	0.04 (3)
B- Q	0 / 1827	0.41 (1)
I- K	-2399 / 0	0.83 (1)
P- E	0 / 1495	0.48 (1)



DWG NO. TAM47660-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.61 (C-D:1) , BC=0.61 (L-N:2) , WB=0.93 (I-K:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.78 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T15S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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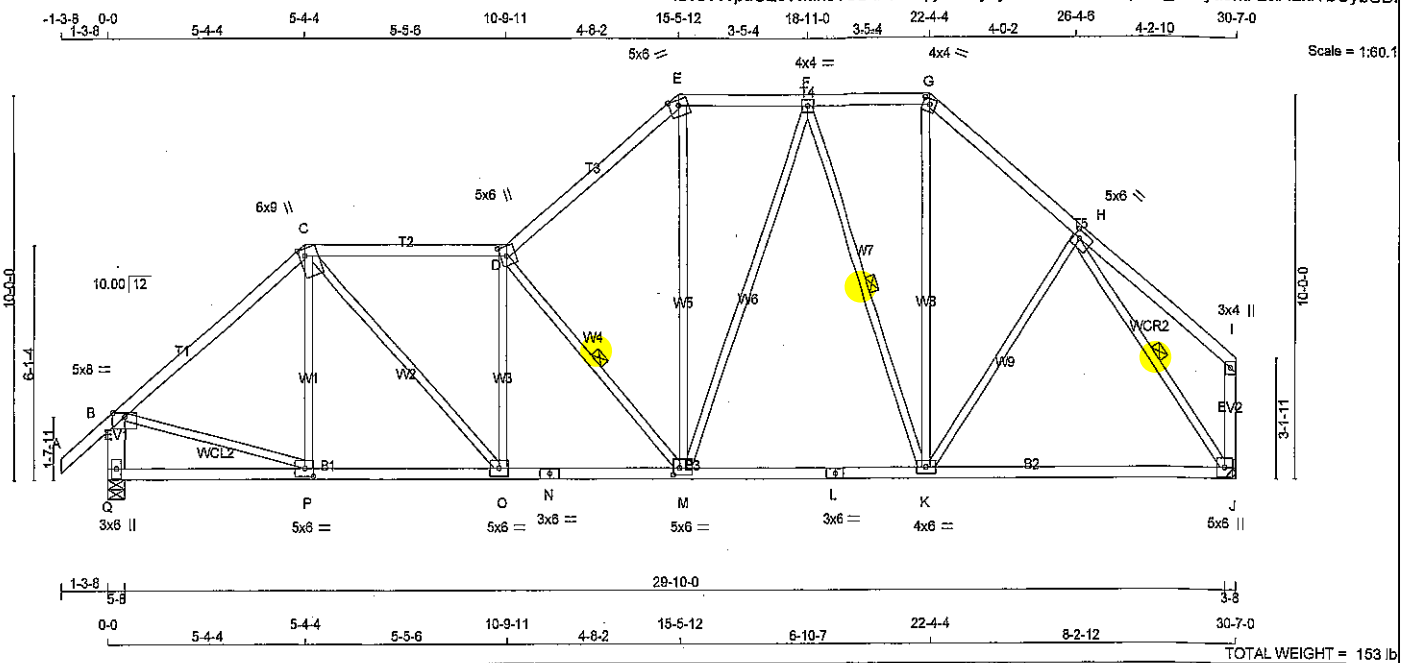


TOTAL WEIGHT = 153 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TTWW+m	MT20	5.0	6.0	3.00	2.00
E	TTW-m	MT20	5.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0	1.75	2.00
H	TMWW-t	MT20	5.0	6.0	2.25	2.00
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0	2.25	2.00
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2466	0	2466	0	0	IN-SX	IN-SX
J	2297	0	2297	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1911	1275 / 0	321 / 0	0 / 0	0 / 0	314 / 0	0 / 0
J	1796	1170 / 0	321 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-M, F-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

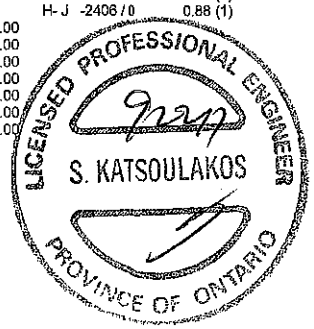
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-2314 / 0	-122.2	-122.2	0.57 (1)	3.92
C- D	-2759 / 0	-122.2	-122.2	0.58 (1)	3.58
D- E	-2350 / 0	-122.2	-122.2	0.44 (1)	4.06
E- F	-1816 / 0	-122.2	-122.2	0.19 (1)	4.80
F- G	-1455 / 0	-122.2	-122.2	0.18 (1)	5.25
G- H	-1924 / 0	-122.2	-122.2	0.30 (1)	4.58
H- I	0 / 37	-122.2	-122.2	0.32 (1)	10.00
Q- B	-2406 / 0	0.0	0.0	0.17 (1)	6.65
J- I	-192 / 0	0.0	0.0	0.03 (1)	7.81
Q- P	0 / 0	-28.0	-28.0	0.21 (2)	10.00
P- O	0 / 1769	-28.0	-28.0	0.40 (1)	10.00
O- N	0 / 2776	-28.0	-28.0	0.52 (1)	10.00
N- M	0 / 2776	-28.0	-28.0	0.52 (1)	10.00
M- L	0 / 1719	-28.0	-28.0	0.62 (2)	10.00
L- K	0 / 1719	-28.0	-28.0	0.62 (2)	10.00
K- J	0 / 1322	-28.0	-28.0	0.58 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-247 / 139	0.14 (1)
C- O	0 / 1459	0.33 (1)
O- D	-947 / 0	0.55 (1)
D- M	-1525 / 0	0.58 (1)
M- E	0 / 1040	0.23 (1)
M- F	0 / 300	0.07 (1)
F- K	-812 / 0	0.63 (1)
K- G	0 / 839	0.19 (1)
K- H	0 / 243	0.05 (2)
B- P	0 / 1824	0.41 (1)
H- J	-2406 / 0	0.88 (1)



DWG NO. TAM4268/-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.58 (C-D:1) , BC=0.62 (K-M:2) , WB=0.88 (H-J:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.75 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T16	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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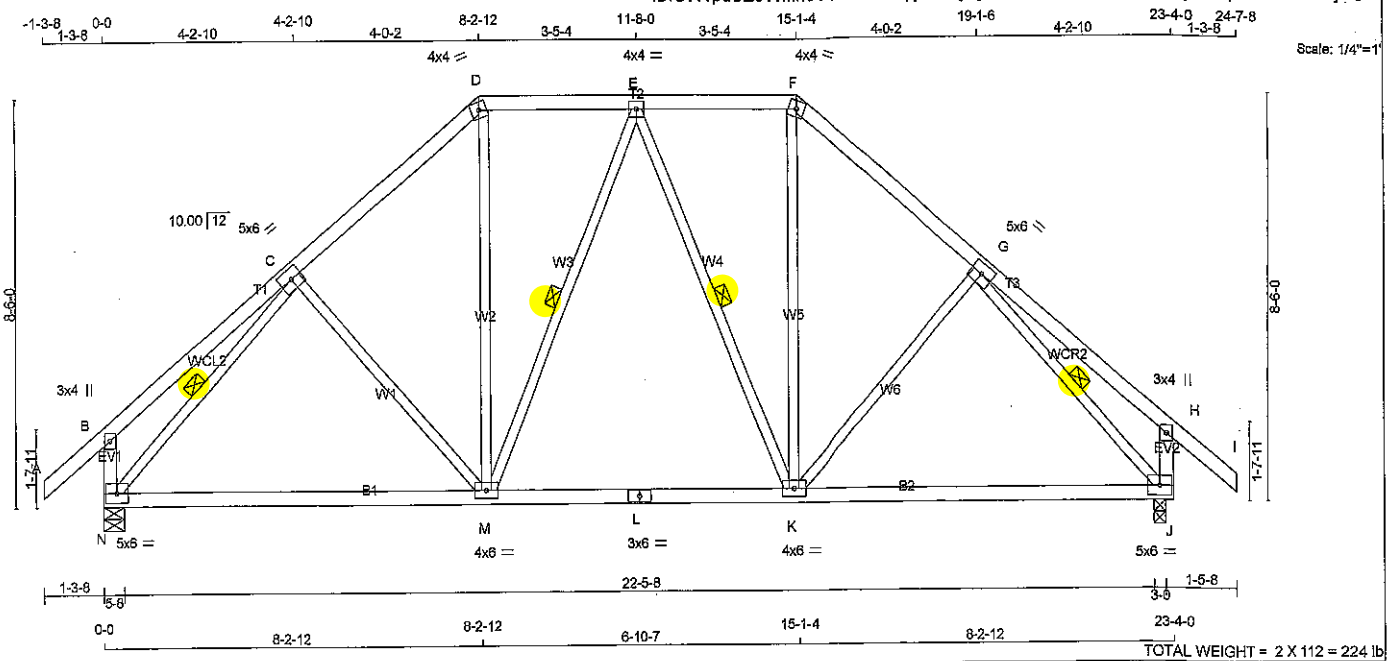

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1922	0	1922	0	0	5-8	5-8
J	1922	0	1922	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1485	998 / 0	245 / 0	0 / 0	0 / 0	242 / 0	0 / 0
J	1485	998 / 0	245 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, E-K, C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 37	-122.2	-122.2	0.33 (1)	10.00
C- D	-1546 / 0	-122.2	-122.2	0.28 (1)	5.00
D- E	-1164 / 0	-122.2	-122.2	0.18 (1)	5.71
E- F	-1164 / 0	-122.2	-122.2	0.18 (1)	5.71
F- G	-1546 / 0	-122.2	-122.2	0.28 (1)	5.00
G- H	0 / 37	-122.2	-122.2	0.33 (1)	10.00
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00
N- B	-361 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-361 / 0	0.0	0.0	0.04 (1)	7.81
N- M	0 / 1269	-28.0	-28.0	0.52 (2)	10.00
M- L	0 / 1265	-28.0	-28.0	0.52 (2)	10.00
L- K	0 / 1265	-28.0	-28.0	0.52 (2)	10.00
K- J	0 / 1269	-28.0	-28.0	0.52 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-168 / 73	0.12 (1)
M- D	0 / 603	0.14 (1)
M- E	-268 / 0	0.15 (1)
E- K	-268 / 0	0.15 (1)
K- F	0 / 603	0.14 (1)
K- G	-168 / 73	0.12 (1)
N- C	-1937 / 0	0.52 (1)
G- J	-1937 / 0	0.52 (1)

DWG NO. TAM 47602-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 830 (0.34")

CSI: TC=0.33 (G-H:1) , BC=0.52 (J-K:2) , WB=0.52 (G-J:1) , SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.48 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	G16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington — Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Sep 21 10:24:36 2017 Page 1

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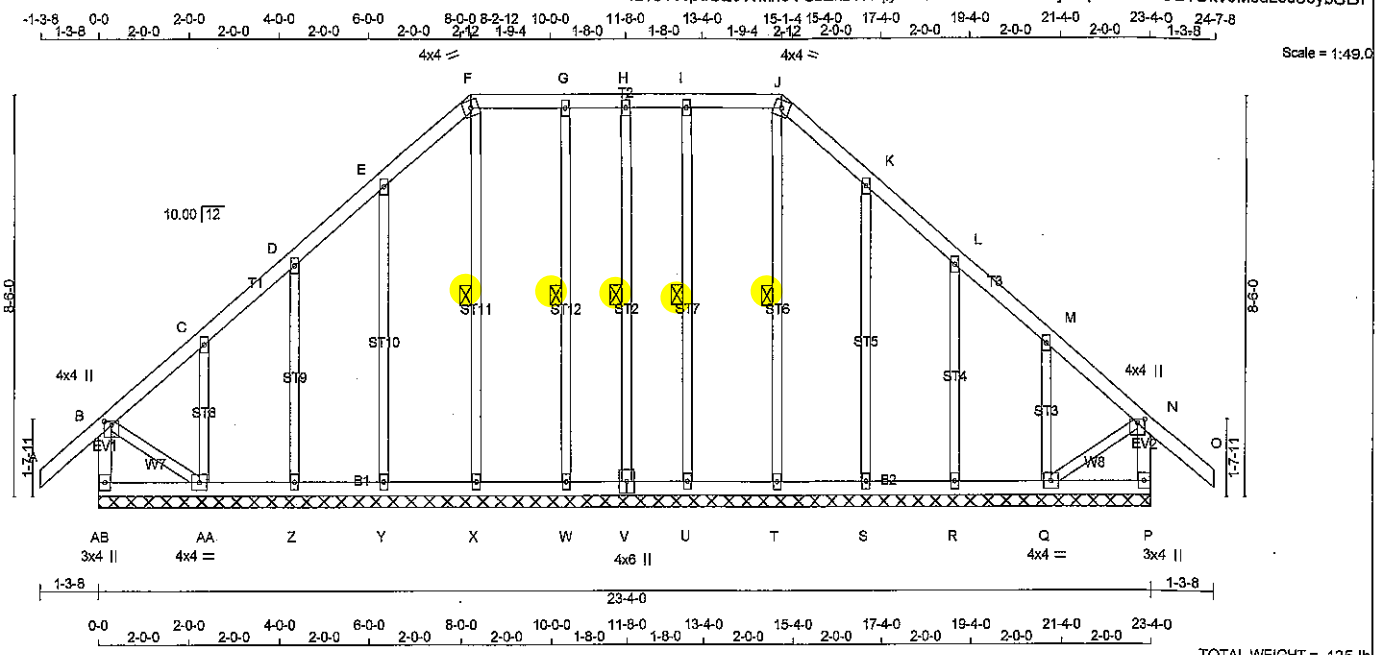


TOTAL WEIGHT = 125 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
AB- B	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
AB- V	2x4	DRY	No.2	SPF
V - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I, K, L, M						
C	TMW+w	MT20	2.0	4.0		
F	TTW-m	MT20	4.0	4.0		
J	TTW-m	MT20	4.0	4.0		
N	TMVW+p	MT20	4.0	4.0	1.00	2.00
P	BMV1+p	MT20	3.0	4.0		
Q	BMWW1-t	MT20	4.0	4.0		
R, S, T, U, W, X, Y, Z						
R	BMW1+w	MT20	2.0	4.0		
V	BSW1+I	MT20	4.0	6.0		
AA	BMWW1-t	MT20	4.0	4.0		
AB	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-V, J-T, I-U, F-X, G-W.

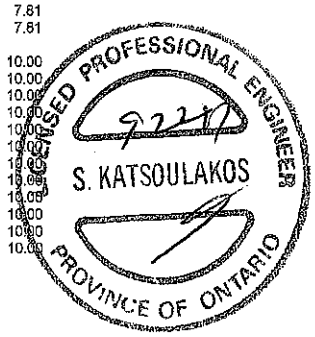
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-38 / 0	-122.2	-122.2	0.08 (1)	6.25
C- D	-52 / 0	-122.2	-122.2	0.08 (1)	6.25
D- E	-34 / 0	-122.2	-122.2	0.07 (1)	6.25
E- F	-50 / 0	-122.2	-122.2	0.07 (1)	6.25
F- G	-23 / 0	-122.2	-122.2	0.07 (1)	6.25
G- H	-23 / 0	-122.2	-122.2	0.05 (1)	6.25
H- I	-23 / 0	-122.2	-122.2	0.05 (1)	6.25
I- J	-23 / 0	-122.2	-122.2	0.07 (1)	6.25
J- K	-50 / 0	-122.2	-122.2	0.07 (1)	6.25
K- L	-34 / 0	-122.2	-122.2	0.07 (1)	6.25
L- M	-52 / 0	-122.2	-122.2	0.08 (1)	6.25
M- N	-38 / 0	-122.2	-122.2	0.08 (1)	6.25
N- O	0 / 54	-122.2	-122.2	0.17 (1)	10.00
AB- B	-339 / 0	0.0	0.0	0.04 (1)	7.81
P- N	-339 / 0	0.0	0.0	0.04 (1)	7.81
AB-AA	0 / 0	-28.0	-28.0	0.04 (2)	10.00
AA- Z	0 / 34	-28.0	-28.0	0.04 (2)	10.00
Z- Y	0 / 30	-28.0	-28.0	0.02 (2)	10.00
Y- X	0 / 26	-28.0	-28.0	0.02 (2)	10.00
X- W	0 / 23	-28.0	-28.0	0.02 (2)	10.00
W- V	0 / 23	-28.0	-28.0	0.02 (2)	10.00
V- U	0 / 23	-28.0	-28.0	0.02 (2)	10.00
U- T	0 / 23	-28.0	-28.0	0.02 (2)	10.00
T- S	0 / 26	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 30	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 34	-28.0	-28.0	0.04 (2)	10.00
Q- P	0 / 0	-28.0	-28.0	0.04 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- H	-135 / 0	0.06 (1)
Q- M	-291 / 0	0.06 (1)
R- L	-222 / 0	0.09 (1)
S- K	-272 / 0	0.22 (1)
T- J	-185 / 0	0.09 (1)
U- I	-242 / 0	0.11 (1)
AA- C	-291 / 0	0.06 (1)
Z- D	-222 / 0	0.09 (1)
Y- E	-272 / 0	0.22 (1)
X- F	-185 / 0	0.09 (1)
W- G	-242 / 0	0.11 (1)
B-AA	0 / 49	0.01 (1)
Q- N	0 / 49	0.01 (1)



DWG NO. TAM 47689 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (N-O:1) , BC=0.04 (Q-R:2) , WB=0.22 (K-S:1) , SSI=0.12 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (F) (INPUT = 0.90)
JSI METAL= 0.08 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T17A	1	1	TRUSS DESC.	

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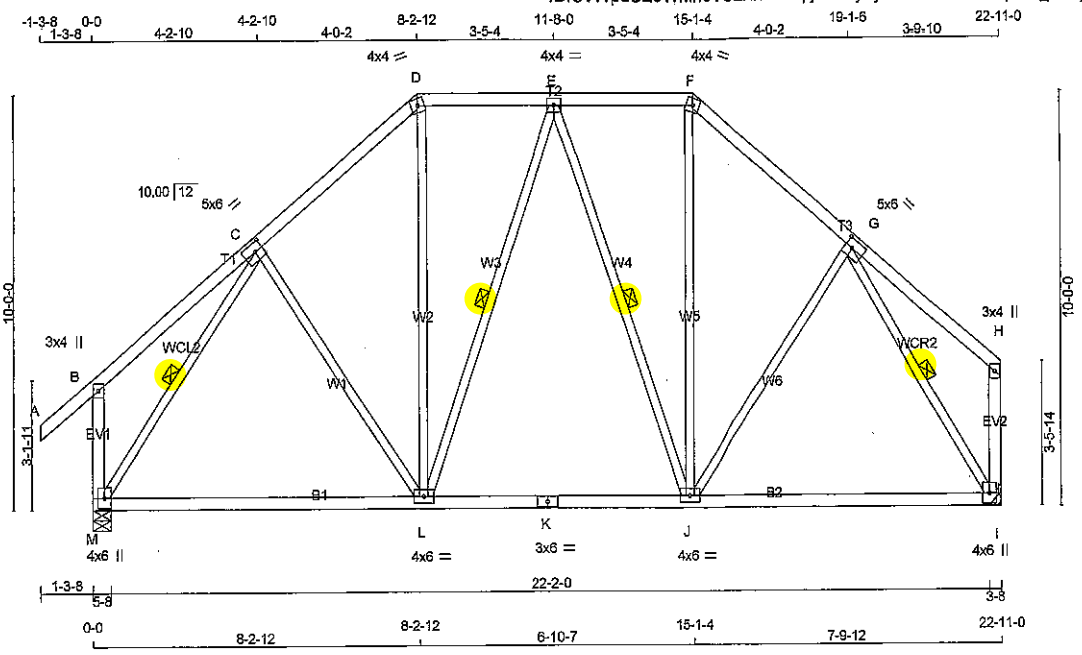


TOTAL WEIGHT = 121 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1691	0	1691	0	0	5-8	5-8
I	1721	0	1721	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1460	982 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0
I	1346	876 / 0	241 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, E-J, C-M, G-I.

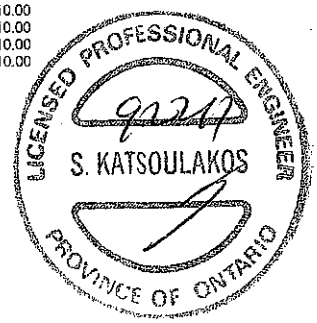
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B-C	0 / 38	-122.2	-122.2	0.33 (1)	10.00	
C-D	-1281 / 0	-122.2	-122.2	0.28 (1)	5.39	
D-E	-957 / 0	-122.2	-122.2	0.18 (1)	6.15	
E-F	-934 / 0	-122.2	-122.2	0.18 (1)	6.20	
F-G	-1247 / 0	-122.2	-122.2	0.26 (1)	5.47	
G-H	0 / 38	-122.2	-122.2	0.30 (1)	10.00	
M-B	-359 / 0	0.0	0.0	0.06 (1)	7.81	
I-H	-164 / 0	0.0	0.0	0.03 (1)	7.81	
M-L	0 / 943	-28.0	-28.0	0.49 (2)	10.00	
L-K	0 / 1032	-28.0	-28.0	0.50 (2)	10.00	
K-J	0 / 1032	-28.0	-28.0	0.50 (2)	10.00	
J-I	0 / 865	-28.0	-28.0	0.44 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	0 / 133	0.03 (3)
L-D	0 / 434	0.10 (1)
L-E	-229 / 6	0.18 (1)
E-J	-301 / 0	0.23 (1)
J-F	0 / 409	0.09 (1)
J-G	0 / 167	0.04 (2)
M-C	-1716 / 0	0.63 (1)
G-I	-1701 / 0	0.59 (1)



DWG NO. TAM 47683-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 844 (0.33")

CSI: TC=0.33 (B-C:1) , BC=0.50 (J-L:2) , WB=0.63 (C-M:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288285	T18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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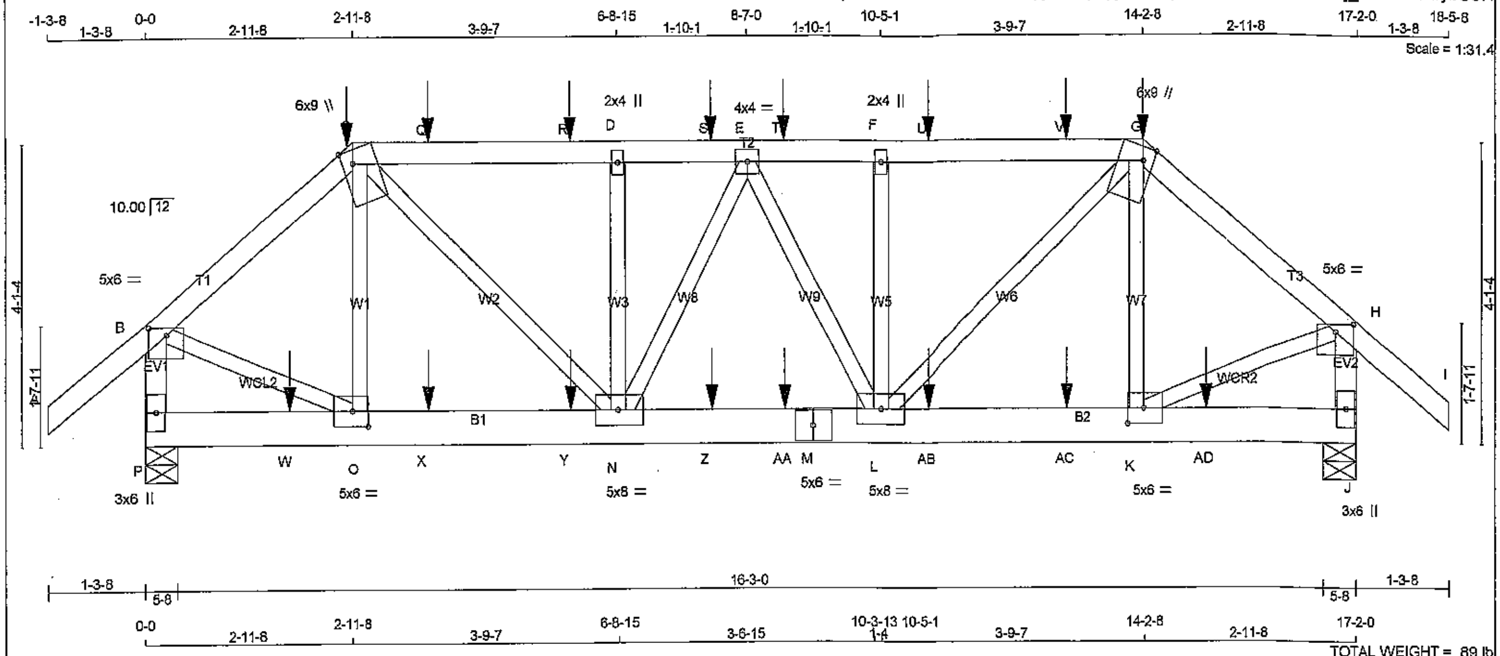

TOTAL WEIGHT = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW-p	MT20	5.0	6.0	1.25	3.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 259.3 lbs FACTORED DOWN AT 2-11-8, 259.3 lbs FACTORED DOWN AT 14-2-8, 147.1 lbs FACTORED DOWN AT 4-0-12, 147.1 lbs FACTORED DOWN AT 6-0-12, 147.1 lbs FACTORED DOWN AT 8-0-12, 147.1 lbs FACTORED DOWN AT 9-1-4, AND 147.1 lbs FACTORED DOWN AT 11-1-4, AND 147.1 lbs FACTORED DOWN AT 13-1-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-0-12, 69.9 lbs FACTORED DOWN AT 4-0-12, 69.9 lbs FACTORED DOWN AT 6-0-12, 69.9 lbs FACTORED DOWN AT 8-0-12, 69.9 lbs FACTORED DOWN AT 9-1-4, 69.9 lbs FACTORED DOWN AT 11-1-4, AND 69.9 lbs FACTORED DOWN AT 13-1-4, AND 69.9 lbs FACTORED DOWN AT 15-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2319	0	2319	0	0	5-8	5-8
J	2319	0	2319	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1795	1201 / 0	300 / 0	0 / 0	0 / 0	295 / 0	0 / 0
J	1795	1201 / 0	300 / 0	0 / 0	0 / 0	295 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.19 (1)	10.00
B- C	-2035 / 0	-122.2	-122.2	0.27 (1)	4.44
C- Q	-2547 / 0	-122.2	-122.2	0.50 (1)	3.71
Q- R	-2547 / 0	-122.2	-122.2	0.50 (1)	3.71
R- D	-2547 / 0	-122.2	-122.2	0.50 (1)	3.71
D- S	-2548 / 0	-122.2	-122.2	0.44 (1)	3.76
S- E	-2548 / 0	-122.2	-122.2	0.44 (1)	3.76
E- T	-2548 / 0	-122.2	-122.2	0.44 (1)	3.76
T- F	-2548 / 0	-122.2	-122.2	0.44 (1)	3.76
F- U	-2547 / 0	-122.2	-122.2	0.50 (1)	3.71
U- V	-2547 / 0	-122.2	-122.2	0.50 (1)	3.71
V- G	-2547 / 0	-122.2	-122.2	0.50 (1)	3.71
G- H	-2035 / 0	-122.2	-122.2	0.27 (1)	4.44
H- I	0 / 54	-122.2	-122.2	0.19 (1)	10.00
P- B	-2282 / 0	0.0	0.0	0.26 (1)	5.58
J- H	-2282 / 0	0.0	0.0	0.26 (1)	5.58
P- W	0 / 0	-28.0	-28.0	0.07 (2)	10.00
W- O	0 / 0	-28.0	-28.0	0.07 (2)	10.00
O- X	0 / 1546	-28.0	-28.0	0.25 (1)	10.00
X- Y	0 / 1546	-28.0	-28.0	0.25 (1)	10.00
Y- N	0 / 1546	-28.0	-28.0	0.25 (1)	10.00
N- Z	0 / 2589	-28.0	-28.0	0.43 (1)	10.00
Z-AA	0 / 2589	-28.0	-28.0	0.43 (1)	10.00
AA- M	0 / 2589	-28.0	-28.0	0.43 (1)	10.00
M- L	0 / 2589	-28.0	-28.0	0.43 (1)	10.00
L-AB	0 / 1546	-28.0	-28.0	0.25 (1)	10.00
AB-AC	0 / 1546	-28.0	-28.0	0.25 (1)	10.00
AC- K	0 / 1546	-28.0	-28.0	0.25 (1)	10.00
K-AD	0 / 0	-28.0	-28.0	0.07 (2)	10.00
AD- J	0 / 0	-28.0	-28.0	0.07 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-425 / 72	0.11 (1)
C- N	0 / 1402	0.35 (1)
N- D	-732 / 0	0.19 (1)
L- F	-732 / 0	0.19 (1)
L- G	0 / 1402	0.35 (1)
K- G	-425 / 72	0.11 (1)
B- O	0 / 1677	0.42 (1)
K- H	0 / 1677	0.42 (1)
N- E	-93 / 0	0.03 (1)
E- L	-93 / 0	0.03 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-16	-18	—	FRONT	VERT	DEAD
C	2-11-8	-243	-243	—	FRONT	VERT	SNOW
G	14-2-8	-16	-18	—	FRONT	VERT	DEAD
G	14-2-8	-243	-243	—	FRONT	VERT	SNOW
Q	4-0-12	-147	-147	—	FRONT	VERT	TOTAL
R	6-0-12	-147	-147	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9/22/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.50 (C-D:1) , BC=0.43 (L-N:1) , WB=0.42 (H-K:1) , SSI=0.39 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)

DWG NO. TAM 47697-17
STRUCTURAL
COMPONENT ONLY

[illegible]

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288285	T18	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	8-0-12	-147	-147	—	FRONT	VERT	TOTAL
T	9-1-4	-147	-147	—	FRONT	VERT	TOTAL
U	11-1-4	-147	-147	—	FRONT	VERT	TOTAL
V	13-1-4	-147	-147	—	FRONT	VERT	TOTAL
W	2-0-12	-40	-70	—	FRONT	VERT	TOTAL
X	4-0-12	-40	-70	—	FRONT	VERT	TOTAL
Y	6-0-12	-40	-70	—	FRONT	VERT	TOTAL
Z	8-0-12	-40	-70	—	FRONT	VERT	TOTAL
AA	9-1-4	-40	-70	—	FRONT	VERT	TOTAL
AB	11-1-4	-40	-70	—	FRONT	VERT	TOTAL
AC	13-1-4	-40	-70	—	FRONT	VERT	TOTAL
AD	15-1-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9221
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47697 - 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T18S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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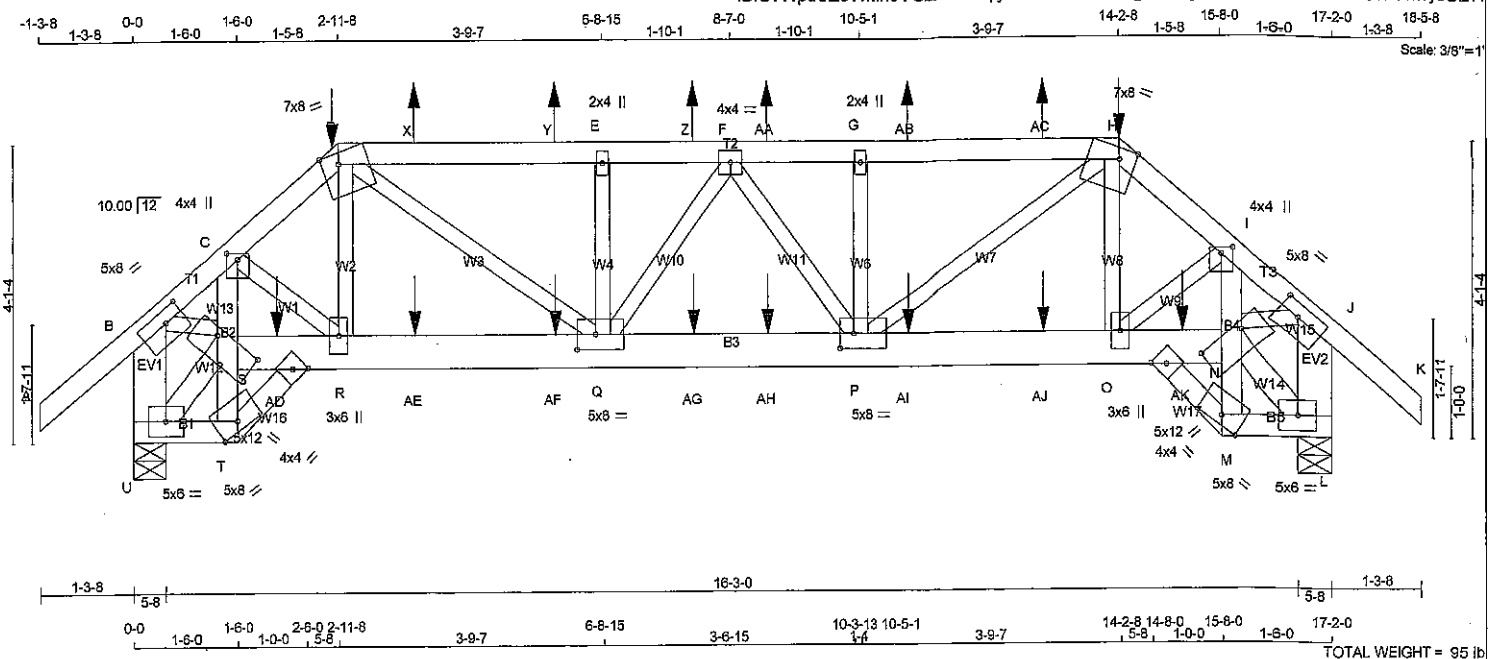

TOTAL WEIGHT = 95 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.25
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW-m	MT20	7.0	8.0	Edge	2.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	7.0	8.0	Edge	2.75
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-t	MT20	5.0	8.0	2.00	3.25
L	BMVW1-t	MT20	5.0	6.0		
M	BVW*-m	MT20	5.0	8.0	1.50	3.75
N	BVMWW-w	MT20	5.0	12.0	1.25	8.00
O	BMWW+t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	2.25
Q	BMWWW-t	MT20	5.0	8.0	2.50	3.25
R	BMWW+t	MT20	3.0	6.0		
S	BVMWW-w	MT20	5.0	12.0	1.25	8.00
T	BVW*-m	MT20	5.0	8.0	1.50	3.75
U	BMVW1-t	MT20	5.0	6.0		
V	NP+w	MT20	4.0	4.0	2.00	1.75
W	NP+w	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 259.3 lbs FACTORED DOWN AT 2-11-8, 259.3 lbs FACTORED DOWN AT 14-2-8, 20.0 lbs FACTORED UP AT 4-0-12, 20.0 lbs FACTORED UP AT 6-0-12, 20.0 lbs FACTORED UP AT 8-0-12, 20.0 lbs FACTORED UP AT 9-1-4, AND 20.0 lbs FACTORED UP AT 11-1-4, AND 20.0 lbs FACTORED UP AT 13-1-4 ON TOP CHORD, AND 310.7 lbs FACTORED DOWN AT 2-0-12, 306.6 lbs FACTORED DOWN AT 4-0-12, 306.6 lbs FACTORED DOWN AT 6-0-12, 306.6 lbs FACTORED DOWN AT 8-0-12, 306.6 lbs FACTORED DOWN AT 9-1-4, 306.6 lbs FACTORED DOWN AT 11-1-4, AND 306.6 lbs FACTORED DOWN AT 13-1-4, AND 310.7 lbs FACTORED DOWN AT 15-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2913	0	2913	0	0	5-8	5-8
L	2913	0	2913	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2234	1533 / 0	350 / 0	0 / 0	0 / 0	351 / 0	0 / 0
L	2234	1533 / 0	350 / 0	0 / 0	0 / 0	351 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.19 (1)	10.00	
B- C	-3277 / 0	-122.2	-122.2	0.31 (1)	3.55	
C- D	-3483 / 0	-122.2	-122.2	0.26 (1)	3.51	
D- X	-4311 / 0	-122.2	-122.2	0.53 (1)	2.89	
X- Y	-4311 / 0	-122.2	-122.2	0.53 (1)	2.89	
Y- E	-4311 / 0	-122.2	-122.2	0.53 (1)	2.89	
E- Z	-4311 / 0	-122.2	-122.2	0.38 (1)	3.03	
Z- F	-4311 / 0	-122.2	-122.2	0.38 (1)	3.03	
F-AA	-4311 / 0	-122.2	-122.2	0.38 (1)	3.03	
AA- G	-4311 / 0	-122.2	-122.2	0.38 (1)	3.03	
G-AB	-4311 / 0	-122.2	-122.2	0.53 (1)	2.89	
AB-AC	-4311 / 0	-122.2	-122.2	0.53 (1)	2.89	
AC- H	-4311 / 0	-122.2	-122.2	0.53 (1)	2.89	
H- I	-3483 / 0	-122.2	-122.2	0.26 (1)	3.51	
I- J	-3277 / 0	-122.2	-122.2	0.31 (1)	3.55	
J- K	0 / 54	-122.2	-122.2	0.19 (1)	10.00	
U- B	-2827 / 0	0.0	0.0	0.21 (1)	6.19	
L- J	-2827 / 0	0.0	0.0	0.21 (1)	6.19	
U- T	0 / 83	-28.0	-28.0	0.03 (2)	10.00	
T- S	0 / 33	0.0	0.0	0.14 (1)	10.00	
S- C	-360 / 0	0.0	0.0	0.14 (1)	7.81	
S-AD	0 / 2520	-28.0	-28.0	0.47 (1)	10.00	
AD- R	0 / 2520	-28.0	-28.0	0.47 (1)	10.00	
R-AE	0 / 2695	-28.0	-28.0	0.50 (1)	10.00	
AE-AF	0 / 2695	-28.0	-28.0	0.50 (1)	10.00	
AF- Q	0 / 2695	-28.0	-28.0	0.50 (1)	10.00	
Q-AG	0 / 4339	-28.0	-28.0	0.80 (1)	10.00	
AG-AH	0 / 4339	-28.0	-28.0	0.80 (1)	10.00	
AH- P	0 / 4339	-28.0	-28.0	0.80 (1)	10.00	
P-AI	0 / 2695	-28.0	-28.0	0.50 (1)	10.00	
AI-AJ	0 / 2695	-28.0	-28.0	0.50 (1)	10.00	
AJ- O	0 / 2695	-28.0	-28.0	0.50 (1)	10.00	
O-AK	0 / 2520	-28.0	-28.0	0.47 (1)	10.00	
AK- N	0 / 2520	-28.0	-28.0	0.47 (1)	10.00	
M- N	0 / 33	0.0	0.0	0.14 (1)	10.00	
N- I	-360 / 0	0.0	0.0	0.14 (1)	7.81	
M- L	0 / 83	-28.0	-28.0	0.03 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 198	0.05 (1)
R- D	0 / 498	0.12 (1)
D- Q	0 / 1987	0.49 (1)
Q- E	-420 / 36	0.08 (5)
P- G	-420 / 36	0.08 (5)
P- H	0 / 1987	0.49 (1)
O- H	0 / 498	0.12 (1)
O- I	0 / 198	0.05 (1)
Q- F	-51 / 0	0.01 (1)
F- P	-51 / 0	0.01 (1)
U- S	-106 / 0	0.01 (1)
B- S	0 / 2486	0.62 (1)
N- L	-106 / 0	0.01 (1)
N- J	0 / 2486	0.62 (1)

DWG NO. TAM 4764-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 974 (0.21")

CSI: TC=0.53 (G-H:1) , BC=0.80 (P-Q:1) , WB=0.62 (J-N:1) , SSI=0.24 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T18S	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	2-11-8	-16	-16	—	FRONT	VERT	DEAD
D	2-11-8	-243	-243	—	FRONT	VERT	SNOW
H	14-2-8	-16	-16	—	FRONT	VERT	DEAD
H	14-2-8	-243	-243	—	FRONT	VERT	SNOW
X	4-0-12	12	—	20	FRONT	VERT	TOTAL
Y	6-0-12	12	—	20	FRONT	VERT	TOTAL
Z	8-0-12	12	—	20	FRONT	VERT	TOTAL
AA	9-1-4	12	—	20	FRONT	VERT	TOTAL
AB	11-1-4	12	—	20	FRONT	VERT	TOTAL
AC	13-1-4	12	—	20	FRONT	VERT	TOTAL
AD	2-0-12	-311	-311	—	FRONT	VERT	TOTAL
AE	4-0-12	-307	-307	—	FRONT	VERT	TOTAL
AF	6-0-12	-307	-307	—	FRONT	VERT	TOTAL
AG	8-0-12	-307	-307	—	FRONT	VERT	TOTAL
AH	9-1-4	-307	-307	—	FRONT	VERT	TOTAL
AI	11-1-4	-307	-307	—	FRONT	VERT	TOTAL
AJ	13-1-4	-307	-307	—	FRONT	VERT	TOTAL
AK	15-1-4	-311	-311	—	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.65 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47664-17
STRUCTURAL
COMPONENT ONLY

PGM

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288285	T19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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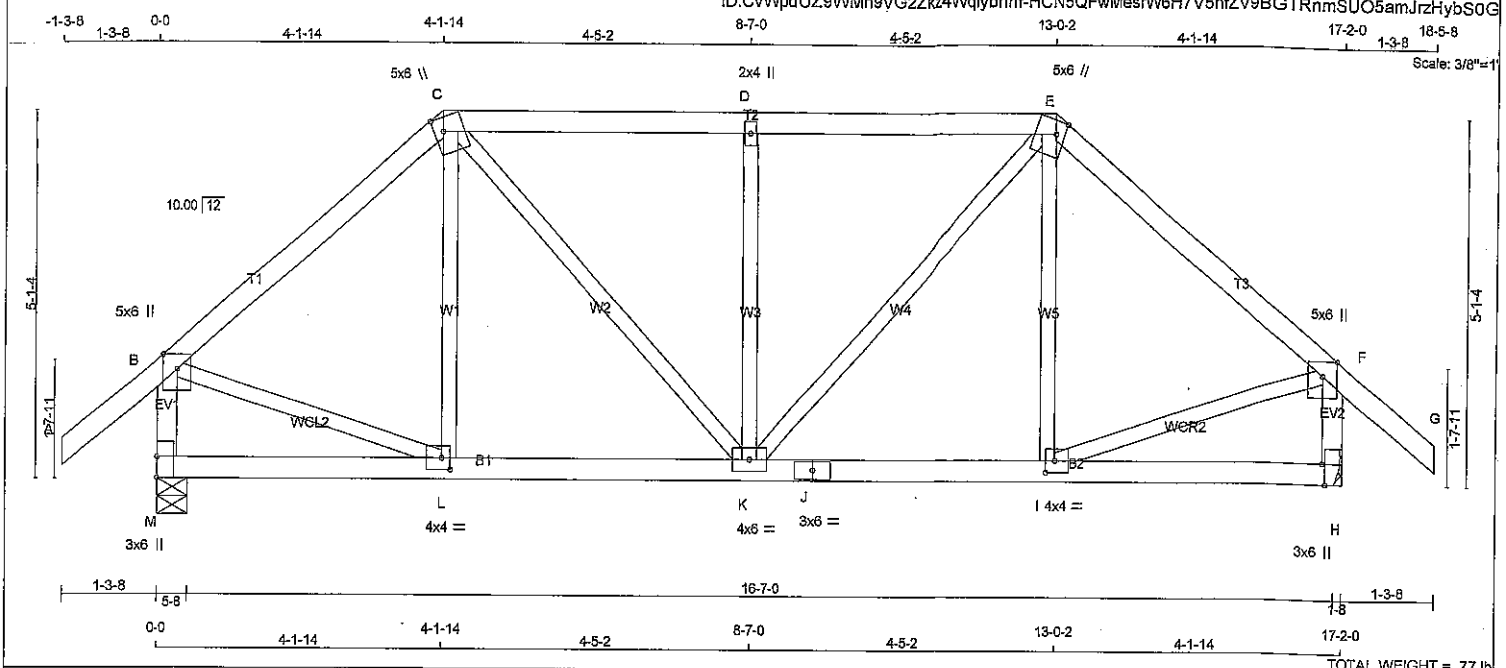

TOTAL WEIGHT = 77 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+t	MT20	3.0	6.0	Edge	0.50
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BMV1+t	MT20	3.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
M	1459	0	1459	0	0	IN-SX	IN-SX
H	1459	0	1459	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
H	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	L- C	-126 / 135		0.05 (1)
B- C	-1126 / 0	-122.2	-122.2	0.40 (1)	5.44	C- K	0 / 521		0.12 (1)
C- D	-1213 / 0	-122.2	-122.2	0.42 (1)	5.23	K- D	-657 / 0		0.25 (1)
D- E	-1213 / 0	-122.2	-122.2	0.42 (1)	5.23	K- E	0 / 521		0.12 (1)
E- F	-1126 / 0	-122.2	-122.2	0.40 (1)	5.44	I- E	-126 / 135		0.05 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00	B- L	0 / 903		0.20 (1)
M- B	-1411 / 0	0.0	0.0	0.15 (1)	6.86	I- F	0 / 903		0.20 (1)
H- F	-1411 / 0	0.0	0.0	0.15 (1)	6.86				
M- L	0 / 0	-28.0	-28.0	0.13 (3)	10.00				
L- K	0 / 860	-28.0	-28.0	0.22 (2)	10.00				
K- J	0 / 860	-28.0	-28.0	0.22 (2)	10.00				
J- I	0 / 860	-28.0	-28.0	0.22 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.13 (3)	10.00				

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4769B17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.42 (D-E:1) , BC=0.22 (K-L:2) , WB=0.25 (D-K:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (L) (INPUT = 0.90)
JSI METAL= 0.32 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T19S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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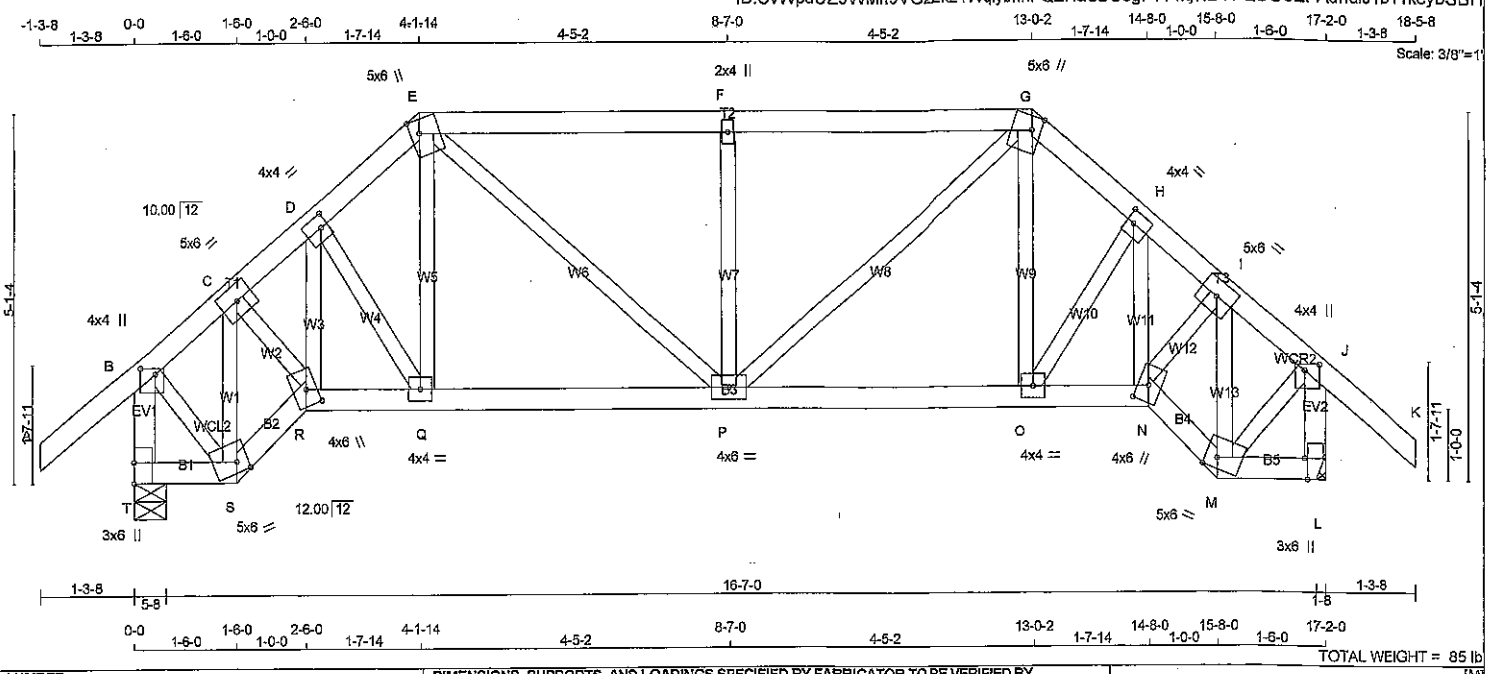

TOTAL WEIGHT = 85 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.50
C	TMWW-t	MT20	5.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMWW-t	MT20	5.0	6.0		
J	TMVW+p	MT20	4.0	4.0	1.00	2.50
L	BMV1+t	MT20	3.0	6.0	Edge	0.50
M	BBWW-m	MT20	5.0	6.0	1.75	2.00
N	BBWW+m	MT20	4.0	6.0	2.75	1.75
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BBWW+m	MT20	4.0	6.0	2.75	1.75
S	BBWW-m	MT20	5.0	6.0	1.75	2.00
T	BMV1+t	MT20	3.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	1459	0	1459	0	0	5-8	5-8
L	1459	0	1459	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
L	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-900 / 0	-122.2	-122.2	0.17 (1)	6.25
C- D	-1430 / 0	-122.2	-122.2	0.08 (1)	5.39
D- E	-1377 / 0	-122.2	-122.2	0.06 (1)	5.49
E- F	-1502 / 0	-122.2	-122.2	0.43 (1)	4.80
F- G	-1502 / 0	-122.2	-122.2	0.43 (1)	4.80
G- H	-1377 / 0	-122.2	-122.2	0.06 (1)	5.49
H- I	-1430 / 0	-122.2	-122.2	0.08 (1)	5.39
I- J	-900 / 0	-122.2	-122.2	0.17 (1)	6.25
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00
T- B	-1438 / 0	0.0	0.0	0.15 (1)	6.81
L- J	-1438 / 0	0.0	0.0	0.15 (1)	6.81
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 834	-28.0	-28.0	0.14 (1)	10.00
R- Q	0 / 1121	-28.0	-28.0	0.23 (1)	10.00
Q- P	0 / 1052	-28.0	-28.0	0.22 (1)	10.00
P- O	0 / 1052	-28.0	-28.0	0.22 (1)	10.00
O- N	0 / 1121	-28.0	-28.0	0.23 (1)	10.00
N- M	0 / 834	-28.0	-28.0	0.14 (1)	10.00
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-1097 / 0	0.19 (1)
R- C	0 / 868	0.20 (1)
R- D	-61 / 29	0.01 (1)
R- Q	-135 / 0	0.03 (1)
Q- E	0 / 223	0.05 (2)
E- P	0 / 593	0.13 (1)
P- F	-652 / 0	0.17 (1)
P- G	0 / 593	0.13 (1)
O- G	0 / 223	0.05 (2)
O- H	-135 / 0	0.03 (1)
N- H	-61 / 29	0.01 (1)
N- I	0 / 868	0.20 (1)
M- I	-1097 / 0	0.19 (1)
B- S	0 / 836	0.19 (1)
M- J	0 / 836	0.19 (1)

DWG NO. TAM 47685 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.43 (E-F:1) , BC=0.23 (Q-R:1) , WB=0.20 (C-R:1) , SSI=0.26 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288285	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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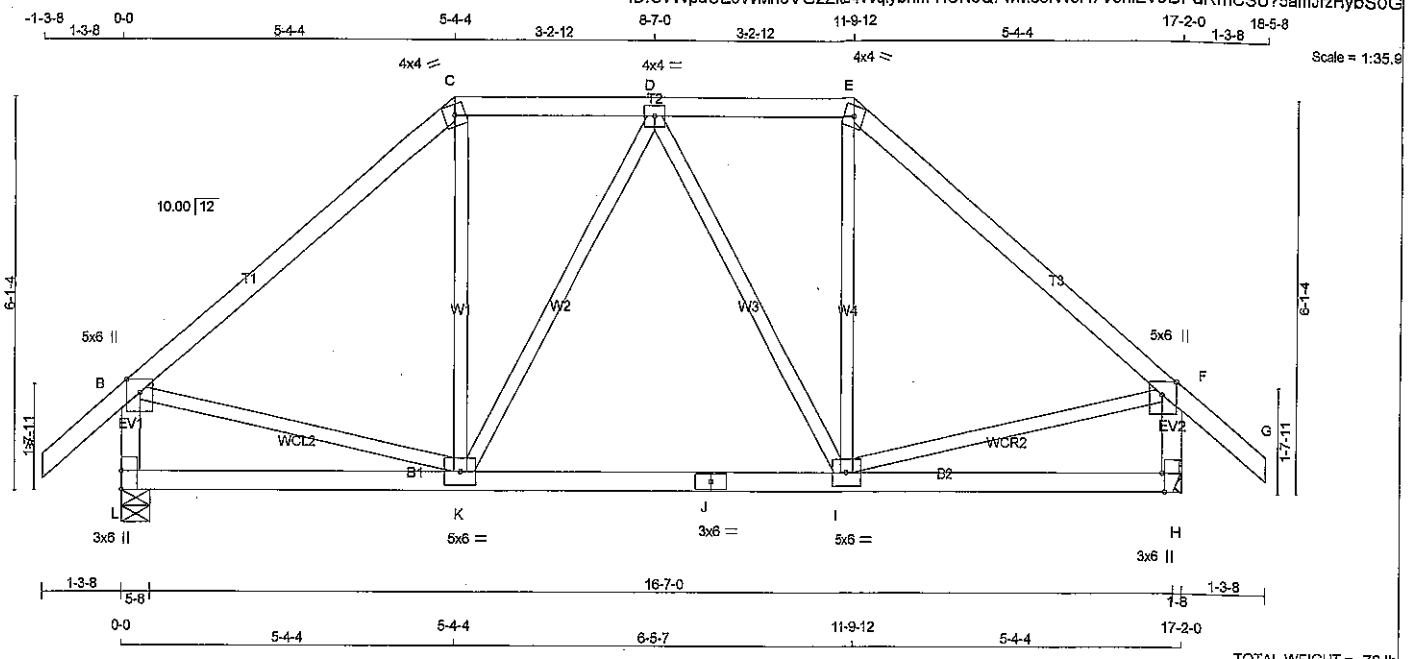

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+t	MT20	3.0	6.0	Edge	0.50
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMV1+t	MT20	3.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
L	1459	0	1459	0	0	IN-SX	IN-SX
H	1459	0	1459	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
H	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00		K- C	0 / 272	0.06 (2)
B- C	-1094 / 0	-122.2	-122.2	0.47 (1)	5.41		K- D	-268 / 0	0.21 (1)
C- D	-845 / 0	-122.2	-122.2	0.16 (1)	6.25		D- I	-268 / 0	0.21 (1)
D- E	-845 / 0	-122.2	-122.2	0.16 (1)	6.25		I- E	0 / 272	0.06 (2)
E- F	-1094 / 0	-122.2	-122.2	0.47 (1)	5.41		B- K	0 / 862	0.19 (1)
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00		I- F	0 / 862	0.19 (1)
L- B	-1397 / 0	0.0	0.0	0.15 (1)	6.89				
H- F	-1397 / 0	0.0	0.0	0.15 (1)	6.89				
L- K	0 / 0	-28.0	-28.0	0.22 (3)	10.00				
K- J	0 / 970	-28.0	-28.0	0.32 (2)	10.00				
J- I	0 / 970	-28.0	-28.0	0.32 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.22 (3)	10.00				

DWG NO. TAM 47689-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.47 (B-C:1) , BC=0.32 (I-K:2) , WB=0.21 (D-I:1) , SSI=0.20 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.31 (J) (INPUT = 1.00)

[M][F]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
288283	T20S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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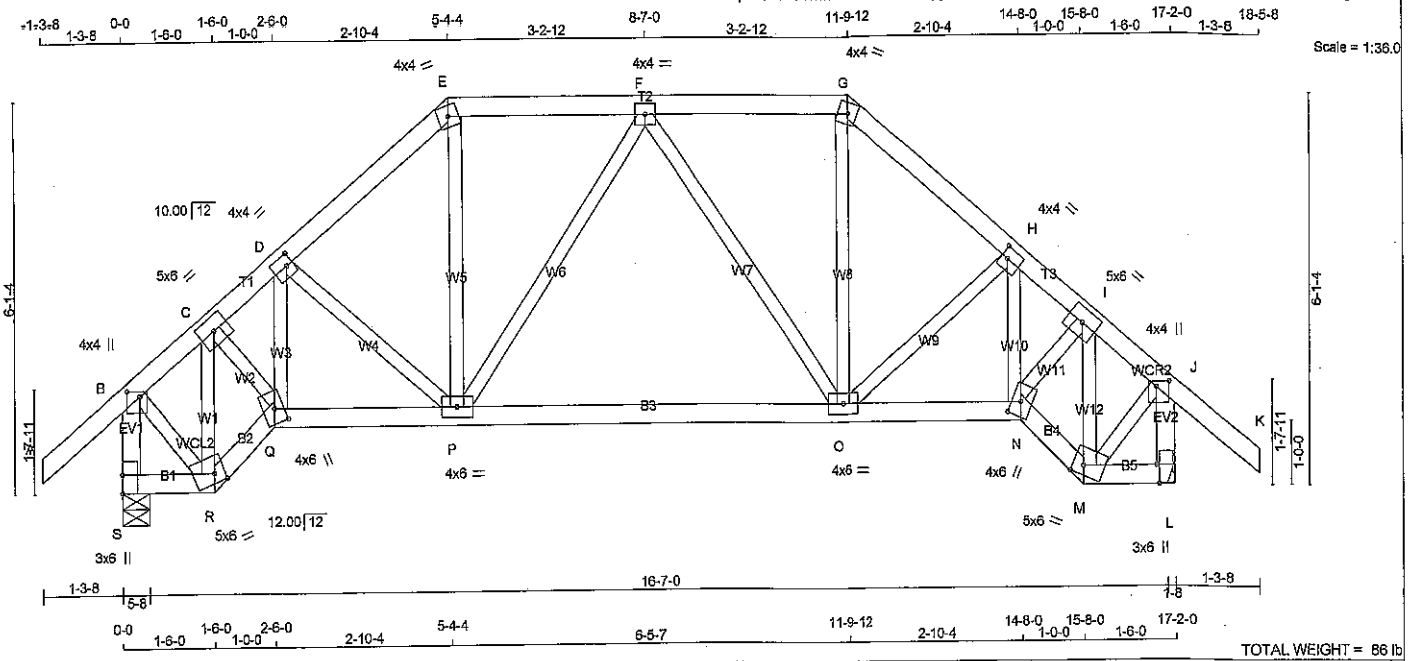

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.50
C	TMWW-t	MT20	5.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMWW-t	MT20	5.0	6.0		
J	TMVW+p	MT20	4.0	4.0	1.00	2.50
L	BMV1+t	MT20	3.0	6.0	Edge	0.50
M	BBWW-m	MT20	5.0	6.0	1.75	2.00
N	BBWW+m	MT20	4.0	6.0	2.75	1.75
O	BMWWW-t	MT20	4.0	6.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BBWW+m	MT20	4.0	6.0	2.75	1.75
R	BBWW-m	MT20	5.0	6.0	1.75	2.00
S	BMV1+t	MT20	3.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1459	0	1459	0	0	5-8	5-8
L	1459	0	1459	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
L	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B-C	-899 / 0	-122.2	-122.2	0.13 (1)		6.25
C-D	-1421 / 0	-122.2	-122.2	0.09 (1)		5.40
D-E	-1295 / 0	-122.2	-122.2	0.12 (1)		5.56
E-F	-989 / 0	-122.2	-122.2	0.16 (1)		6.10
F-G	-989 / 0	-122.2	-122.2	0.16 (1)		6.10
G-H	-1295 / 0	-122.2	-122.2	0.12 (1)		5.56
H-I	-1421 / 0	-122.2	-122.2	0.09 (1)		5.40
I-J	-899 / 0	-122.2	-122.2	0.13 (1)		6.25
J-K	0 / 54	-122.2	-122.2	0.17 (1)		10.00
S-B	-1438 / 0	0.0	0.0	0.15 (1)		6.81
L-J	-1438 / 0	0.0	0.0	0.15 (1)		6.81
S-R	0 / 0	-28.0	-28.0	0.02 (2)		10.00
R-Q	0 / 828	-28.0	-28.0	0.14 (1)		10.00
Q-P	0 / 1145	-28.0	-28.0	0.29 (2)		10.00
P-O	0 / 1141	-28.0	-28.0	0.29 (2)		10.00
O-N	0 / 1145	-28.0	-28.0	0.29 (2)		10.00
N-M	0 / 828	-28.0	-28.0	0.14 (1)		10.00
M-L	0 / 0	-28.0	-28.0	0.02 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-1088 / 0	0.18 (1)
C-Q	0 / 912	0.21 (1)
Q-D	-95 / 39	0.02 (1)
D-P	-221 / 0	0.06 (1)
P-E	0 / 520	0.12 (1)
P-F	-279 / 0	0.16 (1)
F-O	-279 / 0	0.16 (1)
O-G	0 / 520	0.12 (1)
O-H	-221 / 0	0.06 (1)
N-H	-95 / 39	0.02 (1)
N-I	0 / 912	0.21 (1)
M-I	-1088 / 0	0.18 (1)
B-R	0 / 829	0.19 (1)
M-J	0 / 829	0.19 (1)

DWG NO. TAM 47686-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.17 (A-B:1) , BC=0.29 (P-Q:2) , WB=0.21 (C-Q:1) , SSI=0.19 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.27 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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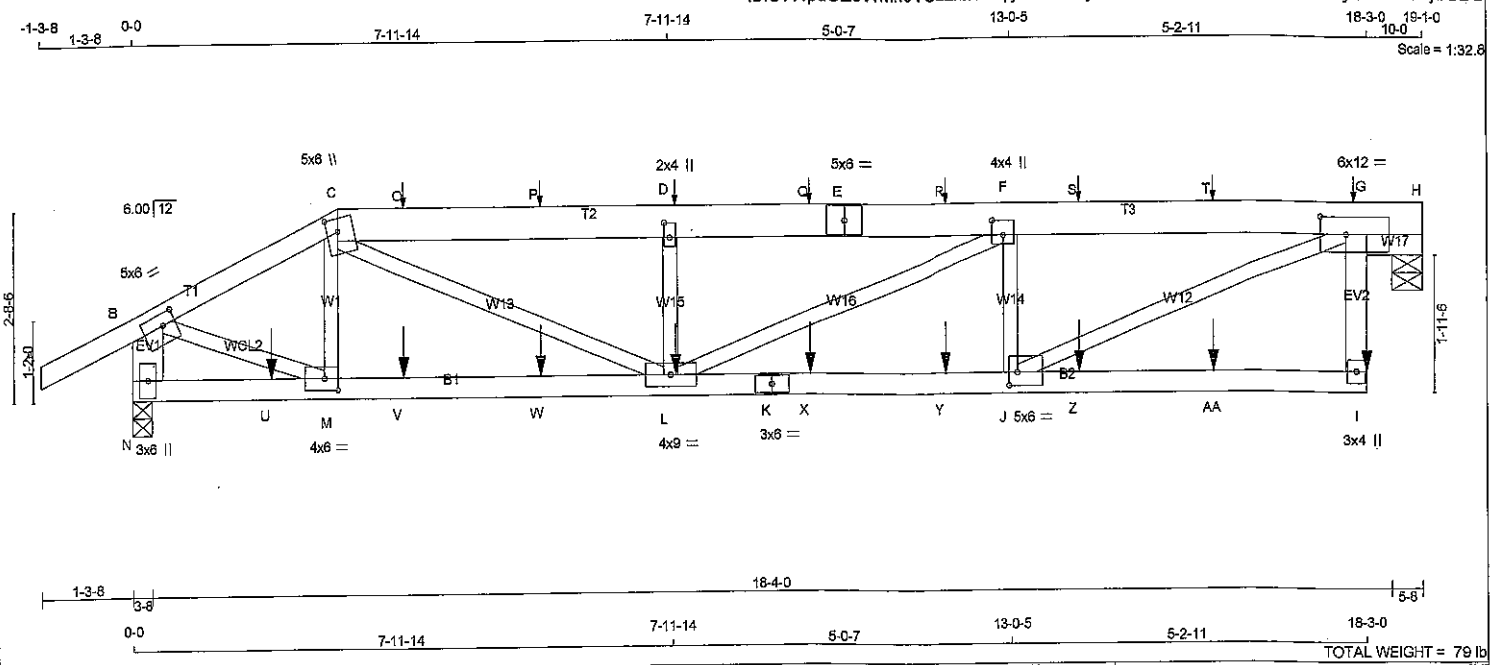


TOTAL WEIGHT = 79 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	4.0	2.50	2.00
G	TMVWW-t	MT20	6.0	12.0	3.00	4.50
I	BMV+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.25	1.50
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) AT 4-0-12, AT 6-0-12, AT 8-0-12, AT 10-0-12, AT 12-0-12, AT 14-0-12, AND AT 16-0-12, AND AT 18-0-12 ON TOP CHORD, AND 3.5 lbs FACTORED DOWN AT 2-0-12, 3.5 lbs FACTORED DOWN AT 4-0-12, 3.5 lbs FACTORED DOWN AT 6-0-12, 3.5 lbs FACTORED DOWN AT 8-0-12, 3.5 lbs FACTORED DOWN AT 10-0-12, 3.5 lbs FACTORED DOWN AT 12-0-12, 3.5 lbs FACTORED DOWN AT 14-0-12, AND 3.5 lbs FACTORED DOWN AT 16-0-12, AND 23.4 lbs FACTORED DOWN AT 18-3-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1608	0	1608	0	0	3-8	3-8
H	1431	0	1431	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): H

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1243	834 / 0	207 / 0	0 / 0	0 / 0	203 / 0	0 / 0
H	1118	730 / 0	199 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 37	-122.2	-122.2	0.17 (1)	10.00
B- C	-1754 / 0	-122.2	-122.2	0.26 (1)	4.72
C- O	-2960 / 0	-122.2	-122.2	0.29 (1)	4.58
O- P	-2960 / 0	-122.2	-122.2	0.29 (1)	4.58
P- D	-2960 / 0	-122.2	-122.2	0.29 (1)	4.58
D- Q	-2959 / 0	-122.2	-122.2	0.43 (1)	4.38
Q- E	-2959 / 0	-122.2	-122.2	0.43 (1)	4.38
E- R	-2959 / 0	-122.2	-122.2	0.43 (1)	4.38
R- F	-2959 / 0	-122.2	-122.2	0.43 (1)	4.38
F- S	-2801 / 0	-122.2	-122.2	0.93 (1)	3.03
S- T	-2801 / 0	-122.2	-122.2	0.93 (1)	3.03
T- G	-2801 / 0	-122.2	-122.2	0.93 (1)	3.03
G- H	0 / 0	-122.2	-122.2	0.53 (1)	10.00
I- G	0 / 132	0.0	0.0	0.02 (2)	10.00
N- B	-1582 / 0	0.0	0.0	0.11 (1)	7.77
N- U	0 / 0	-28.0	-28.0	0.13 (3)	10.00
U- M	0 / 0	-28.0	-28.0	0.13 (3)	10.00
M- V	0 / 1590	-28.0	-28.0	0.36 (1)	10.00
V- W	0 / 1590	-28.0	-28.0	0.36 (1)	10.00
W- L	0 / 1590	-28.0	-28.0	0.36 (1)	10.00
L- K	0 / 2801	-28.0	-28.0	0.57 (1)	10.00
K- X	0 / 2801	-28.0	-28.0	0.57 (1)	10.00
X- Y	0 / 2801	-28.0	-28.0	0.57 (1)	10.00
Y- J	0 / 2801	-28.0	-28.0	0.57 (1)	10.00
J- Z	0 / 0	-28.0	-28.0	0.19 (3)	10.00
Z-AA	0 / 0	-28.0	-28.0	0.19 (3)	10.00
AA- I	0 / 0	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-325 / 44	0.06 (1)
B- M	0 / 1641	0.41 (1)
J- G	0 / 3065	0.76 (1)
C- L	0 / 1518	0.38 (1)
J- F	-1086 / 0	0.19 (1)
L- D	-584 / 0	0.10 (1)
L- F	0 / 174	0.04 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	8-0-12	---	---	---	BACK	VERT	TOTAL
G	18-0-12	---	---	---	BACK	VERT	TOTAL
I	18-3-0	-13	-23	---	BACK	VERT	TOTAL
L	8-0-12	-2	-4	---	BACK	VERT	TOTAL
O	4-0-12	---	---	---	BACK	VERT	TOTAL
P	6-0-12	---	---	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 847 (0.27")

CSI: TC=0.93 (F-G:1) , BC=0.57 (J-L:1) , WB=0.76 (G-J:1) , SSI=0.56 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

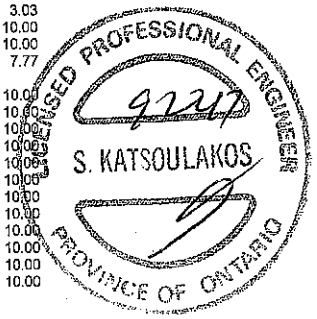
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.76 (K) (INPUT = 1.00)



DWG NO. TAM42067-17
STRUCTURAL COMPONENT ONLY

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	10-0-12	—	—	—	BACK	VERT	TOTAL
R	12-0-12	—	—	—	BACK	VERT	TOTAL
S	14-0-12	—	—	—	BACK	VERT	TOTAL
T	16-0-12	—	—	—	BACK	VERT	TOTAL
U	2-0-12	-2	-4	—	BACK	VERT	TOTAL
V	4-0-12	-2	-4	—	BACK	VERT	TOTAL
W	6-0-12	-2	-4	—	BACK	VERT	TOTAL
X	10-0-12	-2	-4	—	BACK	VERT	TOTAL
Y	12-0-12	-2	-4	—	BACK	VERT	TOTAL
Z	14-0-12	-2	-4	—	BACK	VERT	TOTAL
AA	16-0-12	-2	-4	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47682-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	T22	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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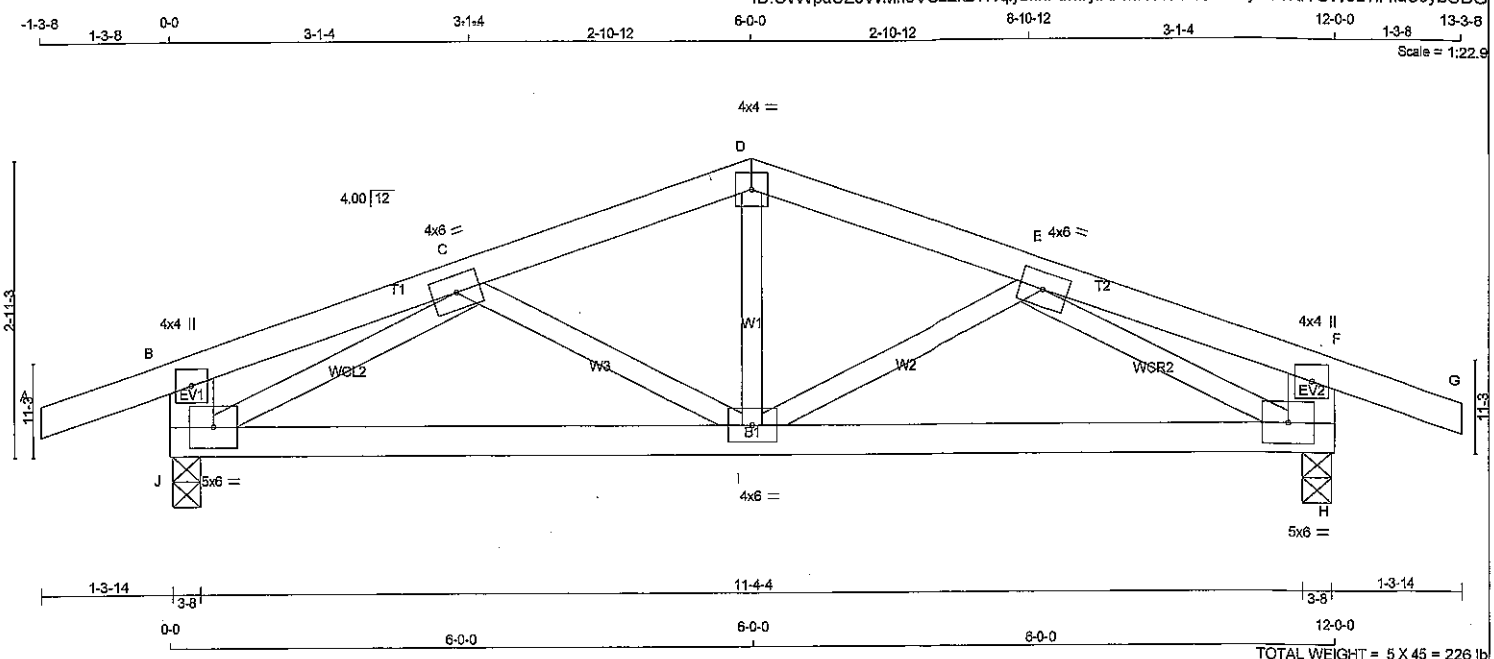


TOTAL WEIGHT = 5 X 45 = 226 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	4.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW-p	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	4.0	4.0	
H	BMVW1-t	MT20	5.0	6.0	
I	BMWWW-t	MT20	4.0	6.0	
J	BMVW1-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1065	0	1065	0	0	3-8	3-8
H	1065	0	1065	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	815	561 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	815	561 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

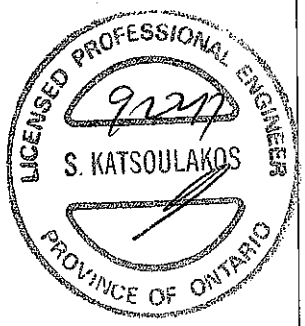
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-122.2	-122.2	0.15 (1)	10.00	I- D	0 / 417	0.09 (1)
B- C	0 / 10	-122.2	-122.2	0.14 (1)	10.00	I- E	-221 / 79	0.05 (1)
C- D	-1135 / 0	-122.2	-122.2	0.12 (1)	5.84	C- I	-221 / 79	0.05 (1)
D- E	-1135 / 0	-122.2	-122.2	0.12 (1)	5.84	J- C	-1432 / 0	0.30 (1)
E- F	0 / 10	-122.2	-122.2	0.14 (1)	10.00	E- H	-1432 / 0	0.30 (1)
F- G	0 / 26	-122.2	-122.2	0.15 (1)	10.00			
J- B	-316 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-316 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 1259	-28.0	-28.0	0.40 (2)	10.00			
I- H	0 / 1259	-28.0	-28.0	0.40 (2)	10.00			



DWG NO. TAM 4760817
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.15 (A-B:1) , BC=0.40 (H-I:2) , WB=0.30 (C-J:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (J) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288283	G22	1	1	TRUSS DESC.	

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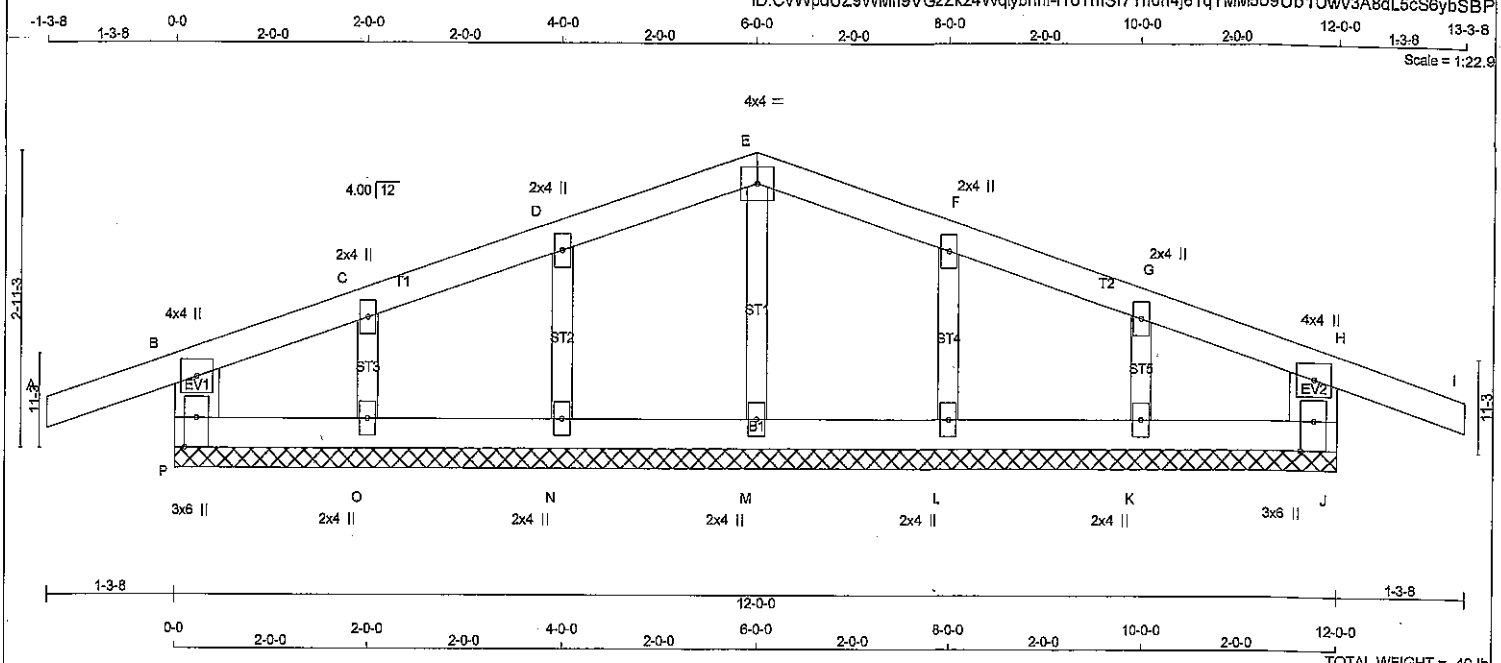

TOTAL WEIGHT = 40 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0		
H	TMV+p	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	6.0	Edge	1.50
K, L, M, N, O						
K	BMW1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	3.0	6.0	Edge	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-122.2	-122.2	1	0.15 (1)	10.00
B- C	-28 / 0	-122.2	-122.2	1	0.05 (1)	6.25
C- D	-18 / 0	-122.2	-122.2	1	0.06 (1)	6.25
D- E	-13 / 0	-122.2	-122.2	1	0.06 (1)	6.25
E- F	-13 / 0	-122.2	-122.2	1	0.06 (1)	6.25
F- G	-18 / 0	-122.2	-122.2	1	0.06 (1)	6.25
G- H	-28 / 0	-122.2	-122.2	1	0.05 (1)	6.25
H- I	0 / 26	-122.2	-122.2	1	0.15 (1)	10.00
P- B	-292 / 0	0.0	0.0	1	0.01 (2)	7.81
J- H	-292 / 0	0.0	0.0	1	0.01 (2)	7.81
P- O	0 / 26	-28.0	-28.0	1	0.03 (2)	10.00
O- N	0 / 18	-28.0	-28.0	1	0.02 (2)	10.00
N- M	0 / 12	-28.0	-28.0	1	0.02 (2)	10.00
M- L	0 / 12	-28.0	-28.0	1	0.02 (2)	10.00
L- K	0 / 18	-28.0	-28.0	1	0.02 (2)	10.00
K- J	0 / 26	-28.0	-28.0	1	0.03 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-242 / 0	0.04 (1)
N- D	-247 / 0	0.04 (1)
O- C	-237 / 0	0.03 (1)
L- F	-247 / 0	0.04 (1)
K- G	-237 / 0	0.03 (1)

DWG NO. TAM 4769017
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15 (A-B:1) , BC=0.03 (O-P:2) , WB=0.04 (E-M:1) , SSI=0.12 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (D) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288286	T23	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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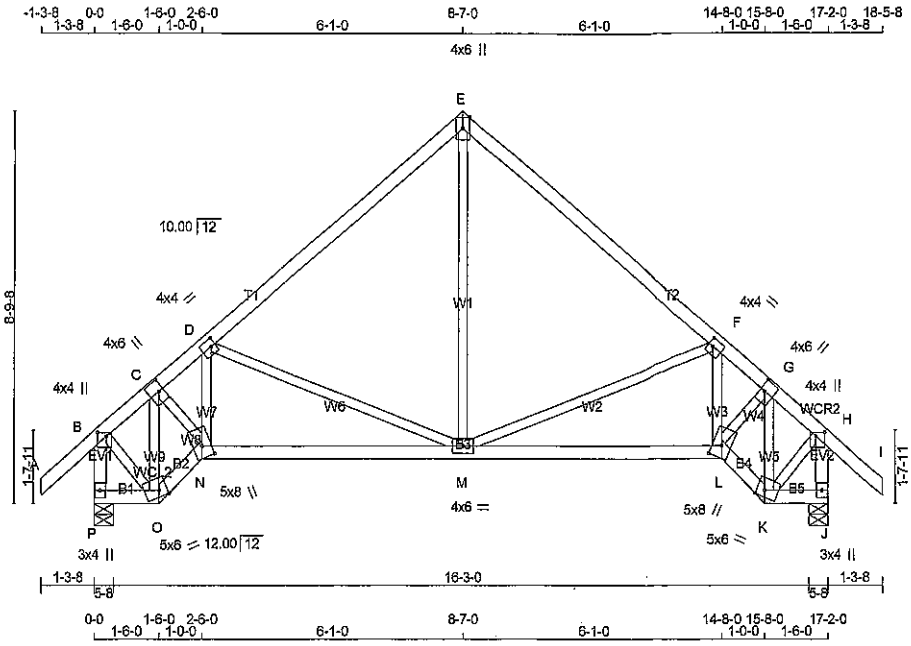


TOTAL WEIGHT = 5 X 82 = 410 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.50
C	TMWW+t	MT20	4.0	6.0	3.00	1.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMWW+t	MT20	4.0	6.0	3.00	1.25
H	TMVW+p	MT20	4.0	4.0	1.00	2.50
J	BMV1+p	MT20	3.0	4.0		
K	BBWW-m	MT20	5.0	6.0	Edge	2.25
L	BBWW+m	MT20	5.0	8.0	3.25	2.50
M	BMWWW-t	MT20	4.0	6.0		
N	BBWW+m	MT20	5.0	8.0	3.25	2.50
O	BBWW-m	MT20	5.0	6.0	Edge	2.25
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1459	0	1459	0	0	5-8	5-8
J	1459	0	1459	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
J	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

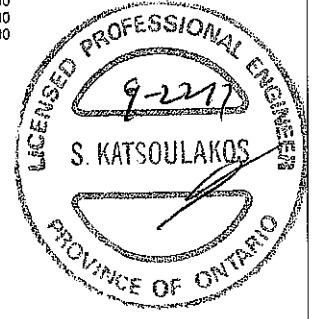
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	-891 / 0	-122.2	-122.2	0.14 (1)		6.25
C- D	-1366 / 0	-122.2	-122.2	0.49 (1)		4.91
D- E	-1015 / 0	-122.2	-122.2	0.55 (1)		5.43
E- F	-1015 / 0	-122.2	-122.2	0.55 (1)		5.43
F- G	-1366 / 0	-122.2	-122.2	0.49 (1)		4.91
G- H	-891 / 0	-122.2	-122.2	0.14 (1)		6.25
H- I	0 / 54	-122.2	-122.2	0.17 (1)		10.00
P- B	-1438 / 0	0.0	0.0	0.15 (1)		6.81
J- H	-1438 / 0	0.0	0.0	0.15 (1)		6.81
P- O	0 / 0	-28.0	-28.0	0.02 (2)		10.00
O- N	0 / 784	-28.0	-28.0	0.13 (1)		10.00
N- M	0 / 1296	-28.0	-28.0	0.43 (2)		10.00
M- L	0 / 1296	-28.0	-28.0	0.43 (2)		10.00
L- K	0 / 784	-28.0	-28.0	0.13 (1)		10.00
K- J	0 / 0	-28.0	-28.0	0.02 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	0 / 657	0.15 (1)
M- F	-599 / 0	0.46 (1)
L- F	-310 / 103	0.05 (1)
L- G	0 / 1197	0.27 (1)
K- G	-1029 / 0	0.17 (1)
D- M	-599 / 0	0.46 (1)
N- D	-310 / 103	0.05 (1)
C- N	0 / 1197	0.27 (1)
O- C	-1029 / 0	0.17 (1)
B- O	0 / 785	0.18 (1)
K- H	0 / 785	0.18 (1)



DWG NO. TAM 47692-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.55 (D-E:1) , BC=0.43 (M-N:2) , WB=0.46 (F-M:1) , SSI=0.33 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288286	G24	1	1	TRUSS DESC.	

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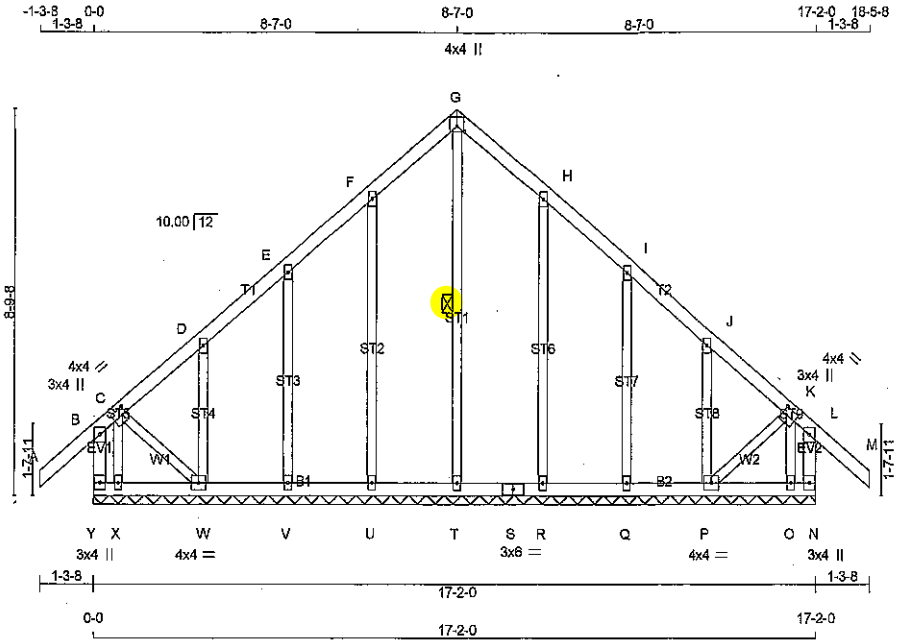


TOTAL WEIGHT = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D, E, F, H, I, J						
D	TMW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	1.50	2.00
K	TMWW-t	MT20	4.0	4.0	2.00	1.25
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, Q, R, T, U, V, X						
O	BMW1+w	MT20	2.0	4.0		
P	BMWW1-t	MT20	4.0	4.0		
S	BS-t	MT20	3.0	6.0		
W	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-T.

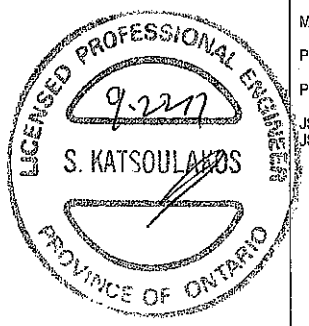
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-369 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-105 / 0	-122.2	-122.2	0.16 (1)	6.25
C- D	-55 / 0	-122.2	-122.2	0.06 (1)	6.25
D- E	-54 / 0	-122.2	-122.2	0.06 (1)	6.25
E- F	-43 / 0	-122.2	-122.2	0.07 (1)	6.25
F- G	-58 / 0	-122.2	-122.2	0.07 (1)	6.25
G- H	-58 / 0	-122.2	-122.2	0.07 (1)	6.25
H- I	-43 / 0	-122.2	-122.2	0.07 (1)	6.25
I- J	-54 / 0	-122.2	-122.2	0.06 (1)	6.25
J- K	-55 / 0	-122.2	-122.2	0.06 (1)	6.25
K- L	-105 / 0	-122.2	-122.2	0.16 (1)	6.25
L- M	0 / 54	-122.2	-122.2	0.17 (1)	10.00
N- L	-369 / 0	0.0	0.0	0.04 (1)	7.81
Y- X	0 / 0	-28.0	-28.0	0.02 (2)	10.00
X- W	0 / 0	-28.0	-28.0	0.02 (2)	10.00
W- V	0 / 41	-28.0	-28.0	0.03 (2)	10.00
V- U	0 / 36	-28.0	-28.0	0.02 (2)	10.00
U- T	0 / 32	-28.0	-28.0	0.02 (2)	10.00
T- S	0 / 32	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 32	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 36	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 41	-28.0	-28.0	0.03 (2)	10.00
P- O	0 / 0	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-153 / 0	0.07 (1)
U- F	-278 / 0	0.24 (1)
V- E	-233 / 0	0.10 (1)
W- D	-246 / 0	0.06 (1)
X- C	-59 / 0	0.01 (1)
R- H	-278 / 0	0.24 (1)
Q- I	-233 / 0	0.10 (1)
P- J	-246 / 0	0.06 (1)
O- K	-59 / 0	0.01 (1)
C- W	0 / 64	0.01 (1)
P- K	0 / 64	0.01 (1)



DWG NO. TAM 47693 - 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.03 (V-W:2) , WB=0.24 (H-R:1) , SSI=0.12 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288288	T24	5	1	TRUSS DESC.	

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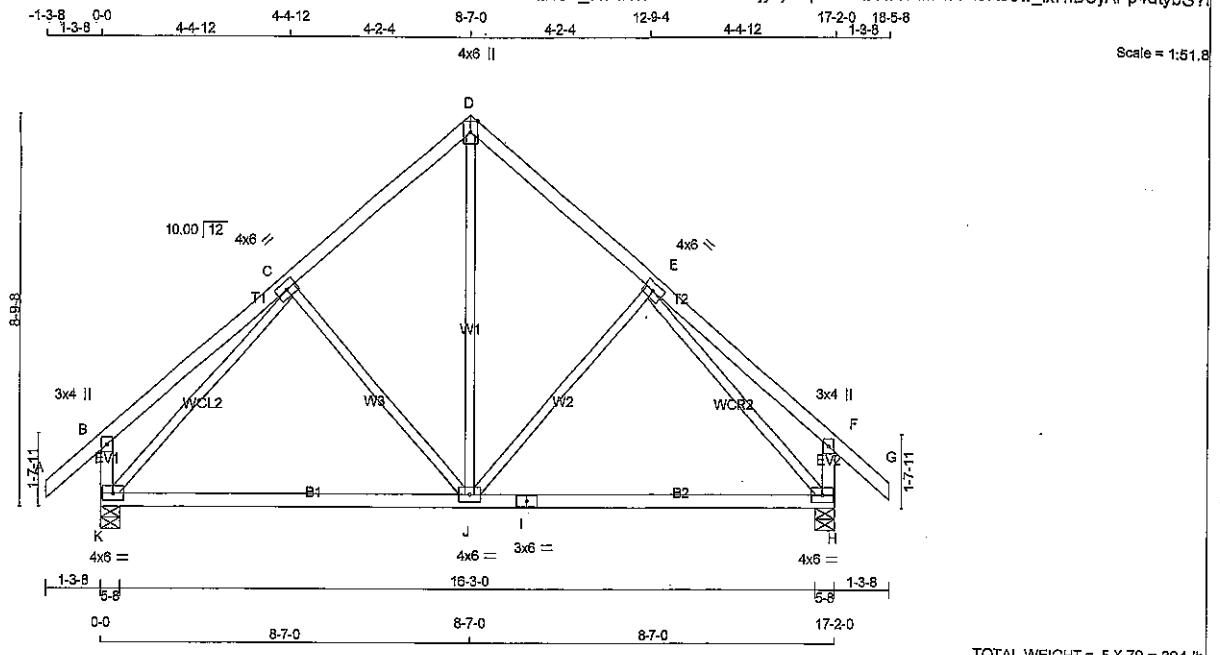

TOTAL WEIGHT = 5 X 79 = 394 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1459	0	1459	0	0	5-8	5-8
H	1459	0	1459	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
H	1123	762 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 40	-122.2	-122.2	0.37 (1)	10.00	
C- D	-929 / 0	-122.2	-122.2	0.29 (1)	6.05	
D- E	-929 / 0	-122.2	-122.2	0.29 (1)	6.05	
E- F	0 / 40	-122.2	-122.2	0.37 (1)	10.00	
F- G	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
K- B	-368 / 0	0.0	0.0	0.04 (1)	7.81	
H- F	-368 / 0	0.0	0.0	0.04 (1)	7.81	
K- J	0 / 877	-28.0	-28.0	0.69 (2)	10.00	
J- I	0 / 877	-28.0	-28.0	0.69 (2)	10.00	
I- H	0 / 877	-28.0	-28.0	0.69 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 734	0.17 (1)
J- E	-292 / 36	0.22 (1)
C- J	-292 / 36	0.22 (1)
K- C	-1330 / 0	0.98 (1)
E- H	-1330 / 0	0.98 (1)

DWG NO. TAM 47722-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 782 (0.26")

CSI: TC=0.37 (E-F:1) , BC=0.69 (J-K:2) , WB=0.98 (E-H:1) , SSI=0.20 (H-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.45 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T25S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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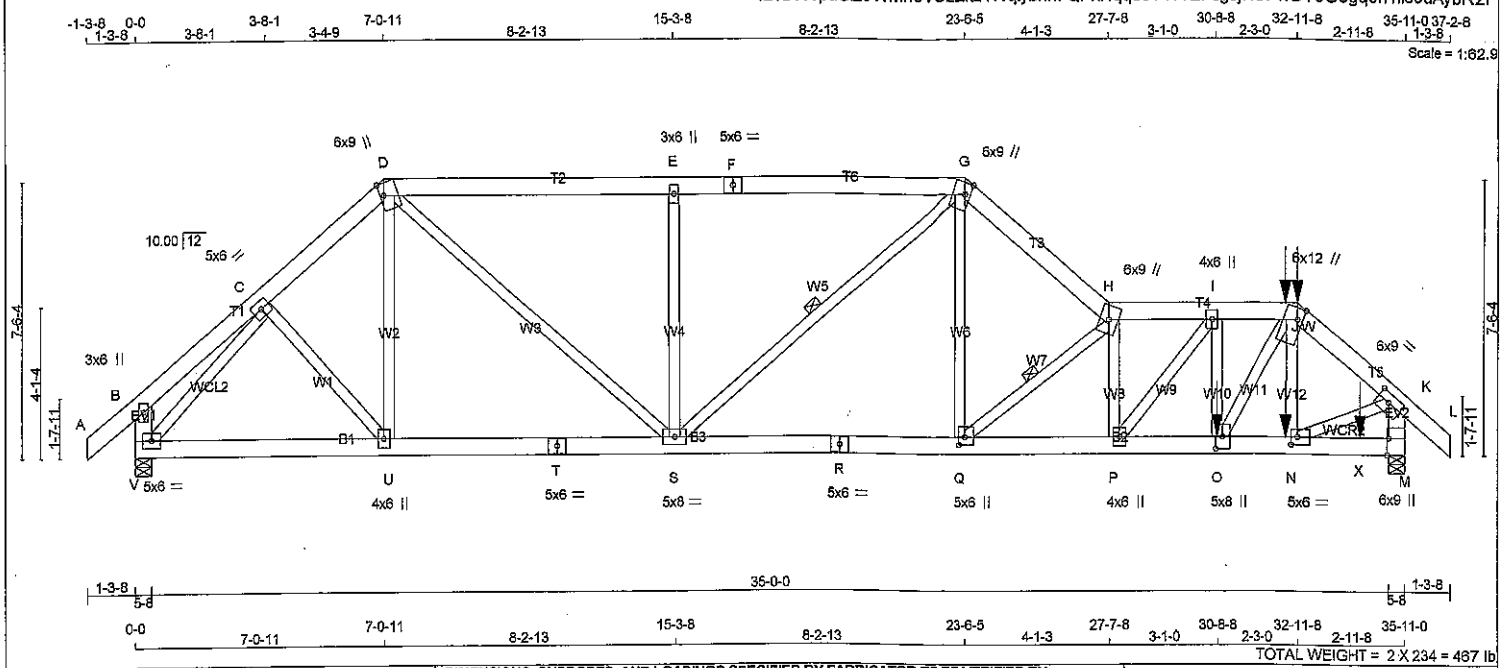

TOTAL WEIGHT = 2 X 234 = 467 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- G	2	12	TOP
G- H	2	12	TOP
H- J	2	12	SIDE(122.0)
J- L	2	12	SIDE(122.0)
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- T	2	12	TOP
T- R	2	12	TOP
R- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
I- O	1	2	SIDE(933.6)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	6.0	9.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TTWW+m	MT20	6.0	9.0	3.75	1.75
H	TTWW+m	MT20	6.0	9.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	12.0	3.75	1.75
K	TMVW-t	MT20	6.0	9.0	2.75	4.25
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWW-t	MT20	5.0	6.0	2.50	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3537	0	3537	0	0	5-8	5-8
M	7018	0	7018	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2737	1833 / 0	456 / 0	0 / 0	0 / 0	448 / 0	0 / 0
M	5408	3660 / 0	877 / 0	0 / 0	0 / 0	872 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-S, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 56	-122.2	-122.2	0.05 (1)	10.00	C- U	0 / 274	0.02 (1)
B- C	-6 / 14	-122.2	-122.2	0.06 (1)	10.00	U- D	0 / 258	0.02 (3)
C- D	-3763 / 0	-122.2	-122.2	0.08 (1)	5.75	D- S	0 / 2731	0.24 (1)
D- E	-4944 / 0	-122.2	-122.2	0.43 (1)	4.76	S- E	-1196 / 0	0.39 (1)
E- F	-4948 / 0	-122.2	-122.2	0.43 (1)	4.76	S- G	-536 / 0	0.16 (1)
F- G	-4946 / 0	-122.2	-122.2	0.43 (1)	4.76	Q- G	0 / 4083	0.36 (1)
G- H	-6859 / 0	-122.2	-122.2	0.17 (1)	4.46	Q- H	-5921 / 0	0.51 (1)
H- I	-9734 / 0	-122.2	-122.2	0.21 (1)	3.77	P- H	-1160 / 0	0.10 (1)
I- W	-8769 / 0	-122.2	-122.2	0.17 (1)	3.99	P- I	0 / 1542	0.14 (1)
W- J	-8769 / 0	-122.2	-122.2	0.17 (1)	3.99	O- I	-1178 / 0	0.10 (1)
J- K	-7224 / 0	-122.2	-122.2	0.12 (1)	4.40	O- J	0 / 6260	0.55 (1)
K- L	0 / 58	-122.2	-122.2	0.05 (1)	10.00	N- J	-1601 / 0	0.14 (1)
V- B	-366 / 0	0.0	0.0	0.01 (1)	7.81	V- C	-4091 / 0	0.63 (1)
M- K	-6999 / 0	0.0	0.0	0.25 (1)	5.65	N- K	0 / 5863	0.52 (1)
V- U	0 / 2680	-28.0	-28.0	0.23 (1)	10.00			
U- T	0 / 2867	-28.0	-28.0	0.24 (1)	10.00			
T- S	0 / 2867	-28.0	-28.0	0.24 (1)	10.00			
S- R	0 / 5354	-28.0	-28.0	0.39 (1)	10.00			
R- Q	0 / 5354	-28.0	-28.0	0.39 (1)	10.00			
Q- P	0 / 9780	-28.0	-28.0	0.72 (1)	10.00			
P- O	0 / 8766	-28.0	-28.0	0.68 (1)	10.00			
O- N	0 / 5453	-28.0	-28.0	0.46 (1)	10.00			
N- X	0 / 0	-28.0	-28.0	0.05 (1)	10.00			
X- M	0 / 0	-28.0	-28.0	0.06 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
J	32-11-8	-16	-16	---	FRONT	VERT	DEAD
J	32-11-8	-243	-243	---	FRONT	VERT	SNOW
N	32-7-12	-40	-70	---	FRONT	VERT	TOTAL
O	30-8-8	-4281	-4281	---	FRONT	VERT	TOTAL
W	32-7-12	-189	-189	---	FRONT	VERT	TOTAL
X	34-7-12	-40	-70	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.43 (E-G:1) , BC=0.72 (P-Q:1) , WB=0.63 (C-V:1) , SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.87 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-47706-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWW+t	MT20	5.0	8.0	4.00	2.25
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW+t	MT20	5.0	6.0	2.50	2.00
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 259.3 lbs FACTORED DOWN AT 32-11-8, AND 189.0 lbs FACTORED DOWN AT 32-7-12 ON TOP CHORD, AND 4280.7 lbs FACTORED DOWN AT 30-8-8, AND 69.9 lbs FACTORED DOWN AT 32-7-12, AND 69.9 lbs FACTORED DOWN AT 34-7-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47706-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T26S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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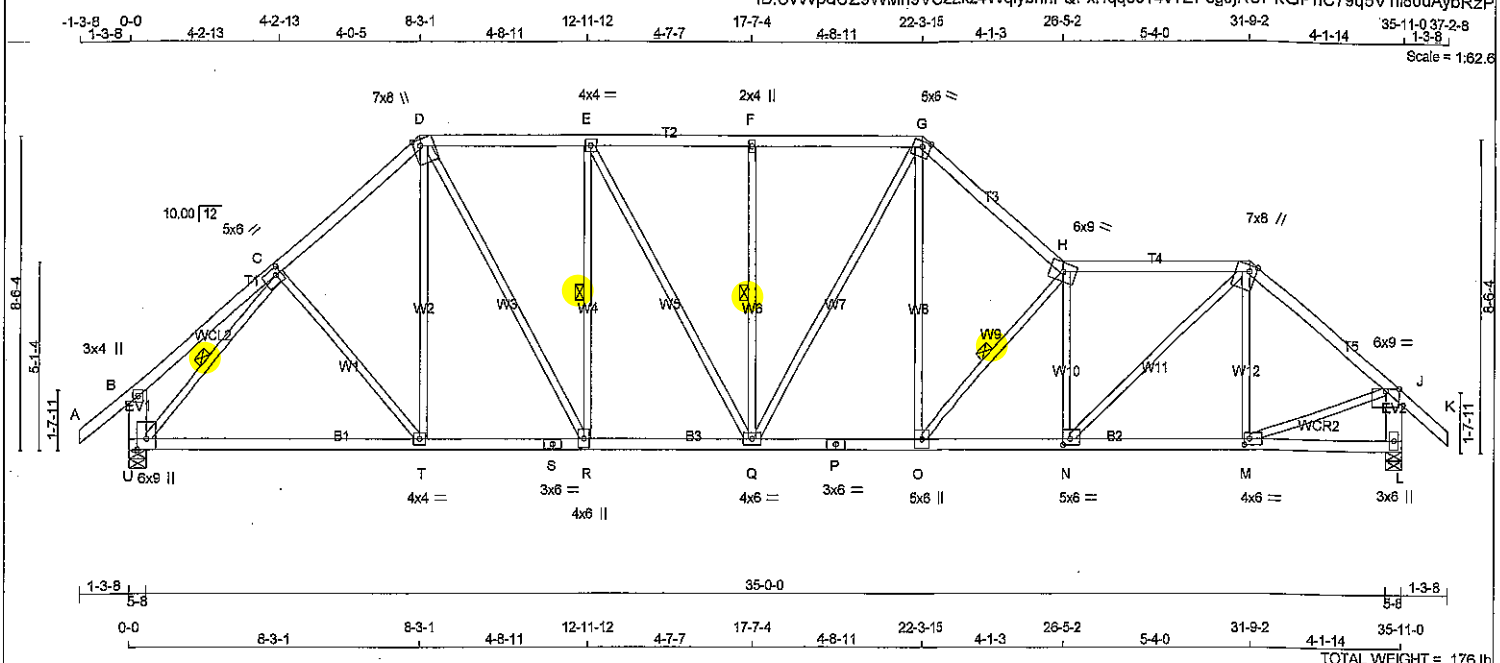

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.75
H	TTWW-m	MT20	6.0	9.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW-p	MT20	6.0	9.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	1.75
N	BMWW-t	MT20	5.0	6.0	2.00	2.50
O	BMWW+t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2867	0	2867	0	0	5-8	5-8
L	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
L	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, F-Q, H-O, C-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 36	-122.2	-122.2	0.32 (1)	10.00
C- D	-2788 / 0	-122.2	-122.2	0.49 (1)	3.72
D- E	-2750 / 0	-122.2	-122.2	0.50 (1)	3.69
E- F	-2979 / 0	-122.2	-122.2	0.51 (1)	3.54
F- G	-2979 / 0	-122.2	-122.2	0.51 (1)	3.55
G- H	-3597 / 0	-122.2	-122.2	0.62 (1)	3.18
H- I	-3899 / 0	-122.2	-122.2	0.97 (1)	2.48
I- J	-2737 / 0	-122.2	-122.2	0.54 (1)	3.68
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00
U- B	-364 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-2824 / 0	0.0	0.0	0.20 (1)	6.23
U- T	0 / 2090	-28.0	-28.0	0.63 (2)	10.00
T- S	0 / 2112	-28.0	-28.0	0.64 (2)	10.00
S- R	0 / 2112	-28.0	-28.0	0.64 (2)	10.00
R- Q	0 / 2750	-28.0	-28.0	0.51 (1)	10.00
Q- P	0 / 2782	-28.0	-28.0	0.52 (1)	10.00
P- O	0 / 2782	-28.0	-28.0	0.52 (1)	10.00
O- N	0 / 3933	-28.0	-28.0	0.89 (1)	10.00
N- M	0 / 2087	-28.0	-28.0	0.43 (1)	10.00
M- L	0 / 0	-28.0	-28.0	0.16 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 146	0.03 (3)
T- D	0 / 320	0.07 (2)
D- R	0 / 1279	0.29 (1)
R- E	-1022 / 0	0.47 (1)
E- Q	0 / 467	0.11 (1)
Q- F	-608 / 0	0.28 (1)
Q- G	0 / 395	0.09 (1)
O- G	0 / 1460	0.33 (1)
O- H	-1774 / 0	0.50 (1)
N- H	-1546 / 0	0.59 (1)
N- I	0 / 2459	0.55 (1)
M- I	-482 / 18	0.19 (1)
U- C	-3187 / 0	0.84 (1)
M- J	0 / 2193	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.97 (H-I:1) , BC=0.89 (N-O:1) , WB=0.84 (C-U:1) , SSI=0.27 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.79 (C) (INPUT = 1.00)

DWG NO. TAM 47707-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T27S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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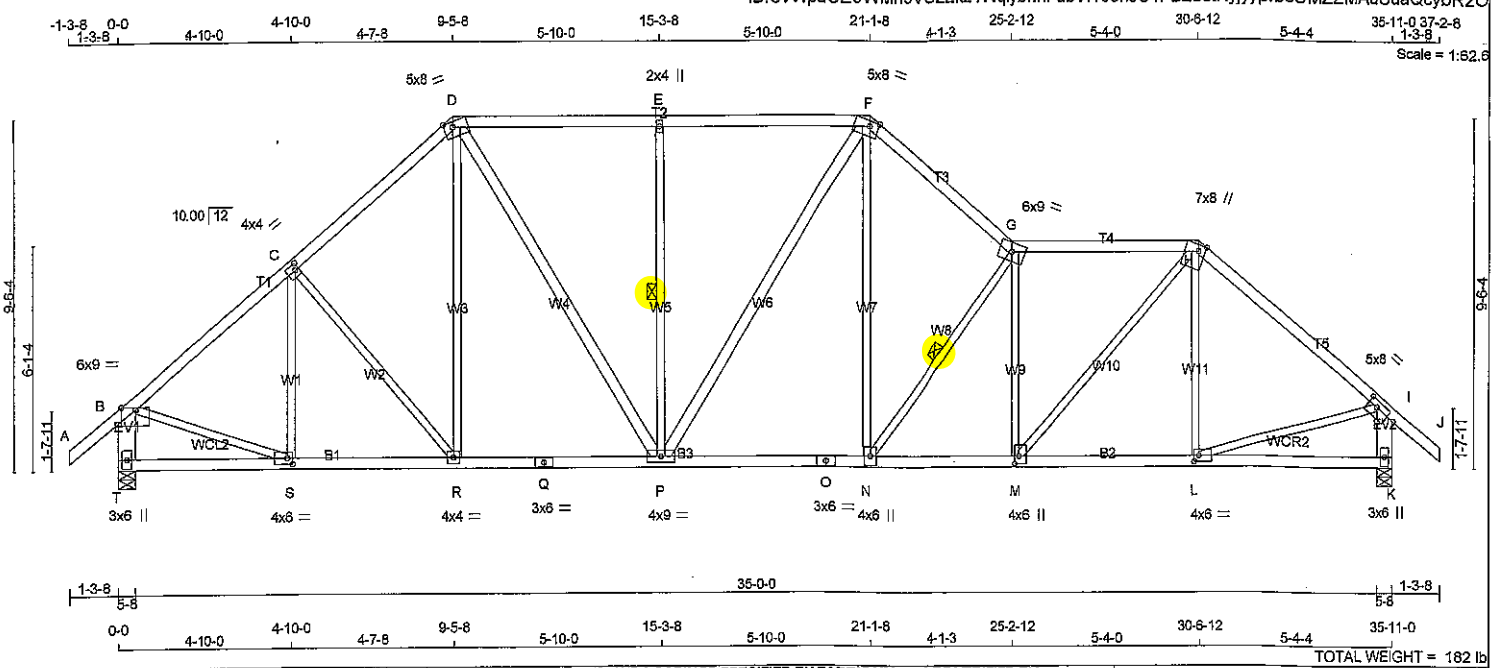


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TTWW-m	MT20	6.0	9.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW-t	MT20	5.0	8.0	2.00	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	1.75
M	BMWW+t	MT20	4.0	6.0	2.50	1.50
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	4.0	6.0	2.00	1.75
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2867	0	2867	0	0	5-8	5-8
K	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
K	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, G-N.

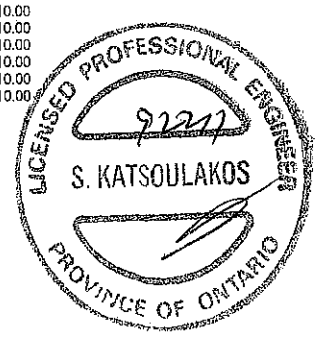
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00
B- C	-2801 / 0	-122.2	-122.2	0.49 (1)	3.73
C- D	-2713 / 0	-122.2	-122.2	0.47 (1)	3.79
D- E	-2630 / 0	-122.2	-122.2	0.64 (1)	3.54
E- F	-2630 / 0	-122.2	-122.2	0.64 (1)	3.54
F- G	-3304 / 0	-122.2	-122.2	0.43 (1)	3.52
G- H	-3474 / 0	-122.2	-122.2	0.62 (1)	3.16
H- I	-2799 / 0	-122.2	-122.2	0.63 (1)	3.56
I- J	0 / 54	-122.2	-122.2	0.17 (1)	10.00
T- B	-2808 / 0	0.0	0.0	0.19 (1)	6.25
K- I	-2807 / 0	0.0	0.0	0.19 (1)	6.25
T- S	0 / 0	-28.0	-28.0	0.14 (3)	10.00
S- R	0 / 2185	-28.0	-28.0	0.43 (1)	10.00
R- Q	0 / 2048	-28.0	-28.0	0.44 (1)	10.00
Q- P	0 / 2048	-28.0	-28.0	0.44 (1)	10.00
P- O	0 / 2555	-28.0	-28.0	0.52 (1)	10.00
O- N	0 / 2555	-28.0	-28.0	0.52 (1)	10.00
N- M	0 / 3499	-28.0	-28.0	0.63 (1)	10.00
M- L	0 / 2140	-28.0	-28.0	0.47 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.21 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-450 / 35	0.22 (1)
C- R	-211 / 0	0.21 (1)
R- D	0 / 338	0.08 (2)
D- P	0 / 1087	0.17 (1)
P- E	-865 / 0	0.52 (1)
P- F	0 / 140	0.02 (1)
N- F	0 / 1466	0.33 (1)
N- G	-1635 / 0	0.56 (1)
M- G	-1369 / 0	0.80 (1)
M- H	0 / 1991	0.45 (1)
L- H	-334 / 108	0.20 (1)
B- S	0 / 2263	0.51 (1)
L- I	0 / 2207	0.50 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.64 (D-E:1) , BC=0.63 (M-N:1) , WB=0.80 (G-M:1) , SSI=0.35 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (S) (INPUT = 0.90)
JSI METAL= 0.66 (O) (INPUT = 1.00)

DWG NO. TAM 47708-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T28S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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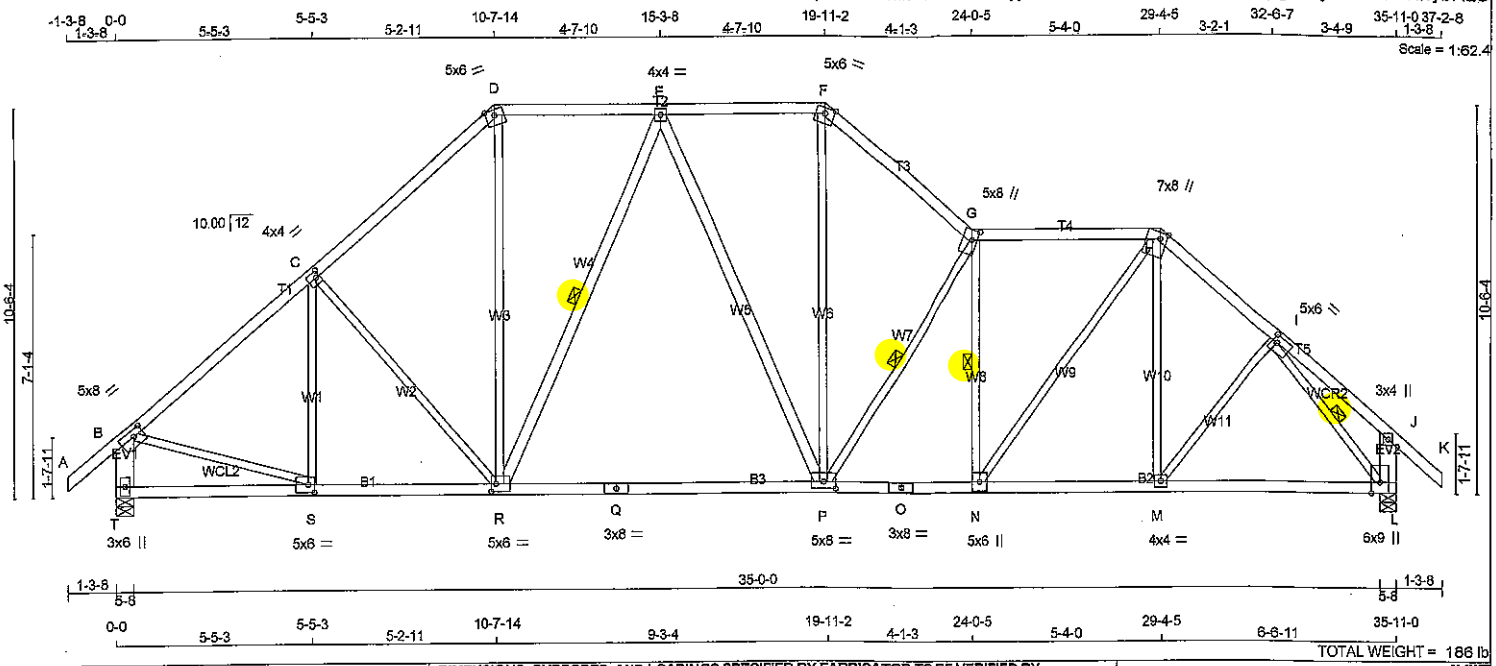


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	5.0	6.0	Edge	
G	TTWW+m	MT20	5.0	8.0	3.25	1.75
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0	2.25	1.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1+p	MT20	6.0	9.0	Edge	
M	BMWW-t	MT20	4.0	4.0		
N	BMWW+t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	5.0	8.0	2.25	4.00
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	5.0	6.0	2.50	1.50
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2867	0	2867	0	0	5-8	5-8
L	2867	0	2867	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
L	2224	1479 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

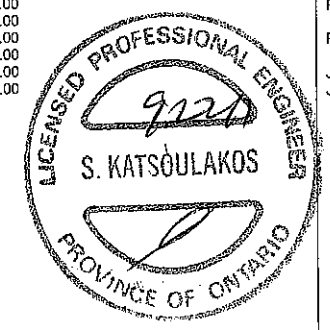
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-P, G-N, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	1	0.17 (1)	10.00
B- C	-2829 / 0	-122.2	-122.2	1	0.63 (1)	3.55
C- D	-2650 / 0	-122.2	-122.2	1	0.58 (1)	3.69
D- E	-2004 / 0	-122.2	-122.2	1	0.37 (1)	4.40
E- F	-2360 / 0	-122.2	-122.2	1	0.39 (1)	4.09
F- G	-3053 / 0	-122.2	-122.2	1	0.40 (1)	3.67
G- H	-3139 / 0	-122.2	-122.2	1	0.59 (1)	3.36
H- I	-2830 / 0	-122.2	-122.2	1	0.24 (1)	3.96
I- J	0 / 25	-122.2	-122.2	1	0.18 (1)	10.00
J- K	0 / 54	-122.2	-122.2	1	0.17 (1)	10.00
T- B	-2799 / 0	0.0	0.0	1	0.19 (1)	6.28
L- J	-328 / 0	0.0	0.0	1	0.02 (1)	7.81
T- S	0 / 0	-28.0	-28.0	2	0.18 (2)	10.00
S- R	0 / 2211	-28.0	-28.0	2	0.59 (2)	10.00
R- Q	0 / 2332	-28.0	-28.0	2	0.61 (2)	10.00
Q- P	0 / 2332	-28.0	-28.0	2	0.61 (2)	10.00
P- O	0 / 3157	-28.0	-28.0	2	0.72 (1)	10.00
O- N	0 / 3157	-28.0	-28.0	2	0.72 (1)	10.00
N- M	0 / 2148	-28.0	-28.0	2	0.52 (2)	10.00
M- L	0 / 1995	-28.0	-28.0	2	0.49 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-393 / 53	0.24 (1)
C- R	-324 / 0	0.42 (1)
R- D	0 / 1201	0.27 (1)
R- E	-808 / 0	0.56 (1)
E- P	0 / 149	0.02 (3)
P- F	0 / 1474	0.33 (1)
P- G	-1543 / 0	0.66 (1)
N- G	-1218 / 0	0.39 (1)
N- H	0 / 1623	0.37 (1)
M- H	0 / 213	0.05 (3)
M- I	0 / 236	0.05 (1)
B- S	0 / 2274	0.51 (1)
I- L	-3170 / 0	0.65 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 987 (0.44")

CSI: TC=0.63 (B-C:1) , BC=0.72 (N-P:1) , WB=0.66 (G-P:1) , SSI=0.27 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.83 (Q) (INPUT = 1.00)

DWG NO. TAM 47709-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T29	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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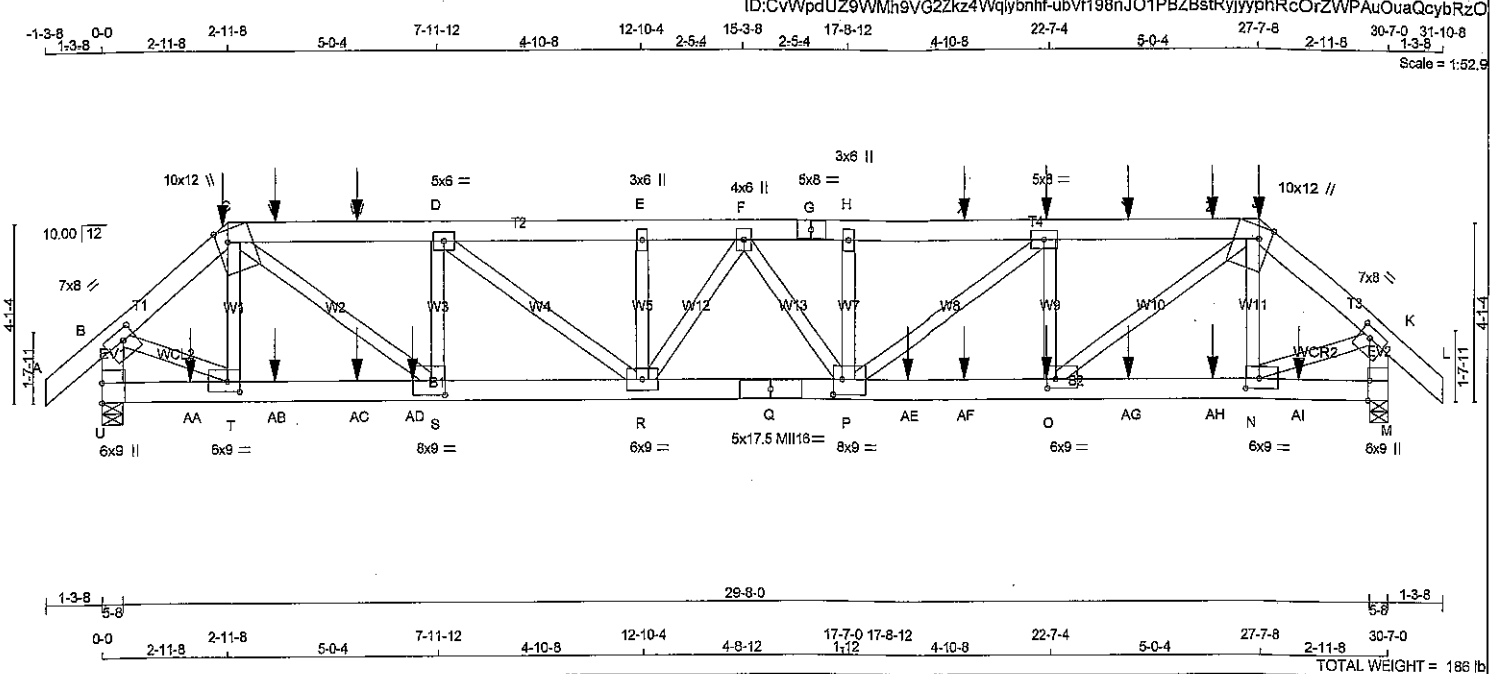


TOTAL WEIGHT = 186 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	1650F 1.5E	SPF
G - J	2x6	DRY	1650F 1.5E	SPF
J - L	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
O - J	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	7.0	8.0	2.75	3.25
C	TTWW+m	MT20	10.0	12.0	Edge	3.25
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	8.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	8.0		
J	TTWW+m	MT20	10.0	12.0	Edge	3.25
K	TMVW-t	MT20	7.0	8.0	2.75	3.25
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWW-t	MT20	6.0	9.0	2.75	3.75
O	BMWW-t	MT20	6.0	9.0	2.50	2.50
P	BMWWW-t	MT20	8.0	9.0	4.25	2.50
Q	BS-t	MII16	5.0	17.5		
R	BMWWW-t	MT20	6.0	9.0		
S	BMWW-t	MT20	8.0	9.0	4.00	3.75
T	BMWW-t	MT20	6.0	9.0	2.75	3.50
U	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 259.3 lbs FACTORED DOWN AT 2-11-8, 259.3 lbs FACTORED DOWN AT 27-7-8, 101.3 lbs FACTORED DOWN AT 4-0-12, 101.3 lbs FACTORED DOWN AT 6-0-12, 147.1 lbs FACTORED DOWN AT 20-6-4, 147.1 lbs FACTORED DOWN AT 22-6-4, AND 147.1 lbs FACTORED DOWN AT 24-6-4, AND 147.1 lbs FACTORED DOWN AT 26-6-4 ON TOP CHORD, AND 45.9 lbs FACTORED DOWN AT 2-0-12, 45.9 lbs FACTORED DOWN AT 4-0-12, 45.9 lbs FACTORED DOWN AT 6-0-12, 1438.3 lbs FACTORED DOWN AT 7-4-8, 1782.3 lbs FACTORED DOWN AT 19-1-8, 69.9 lbs FACTORED DOWN AT 20-6-4, 69.9 lbs FACTORED DOWN AT 22-6-4, 69.9 lbs FACTORED DOWN AT 24-6-4, AND 69.9 lbs FACTORED DOWN AT 26-6-4, AND 69.9 lbs FACTORED DOWN AT 28-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	4903	0	4903	0	0	5-8	5-8
M	4851	0	4851	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3777	2559 / 0	610 / 0	0 / 0	0 / 0	608 / 0	0 / 0
M	3752	2515 / 0	624 / 0	0 / 0	0 / 0	613 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

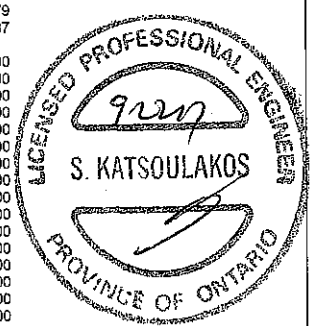
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.10 (1)	10.00
B- C	-4995 / 0	-122.2	-122.2	0.23 (1)	3.71
C- V	-8328 / 0	-122.2	-122.2	0.43 (1)	3.25
V- W	-8328 / 0	-122.2	-122.2	0.43 (1)	3.25
W- D	-8328 / 0	-122.2	-122.2	0.43 (1)	3.25
D- E	-9695 / 0	-122.2	-122.2	0.40 (1)	3.06
E- F	-9695 / 0	-122.2	-122.2	0.29 (1)	3.15
F- G	-10535 / 0	-122.2	-122.2	0.35 (1)	2.98
G- H	-10535 / 0	-122.2	-122.2	0.35 (1)	2.98
H- X	-10535 / 0	-122.2	-122.2	0.52 (1)	2.82
X- I	-10535 / 0	-122.2	-122.2	0.52 (1)	2.82
I- Y	-8400 / 0	-122.2	-122.2	0.44 (1)	3.22
Y- Z	-8400 / 0	-122.2	-122.2	0.44 (1)	3.22
Z- J	-8400 / 0	-122.2	-122.2	0.44 (1)	3.22
J- K	-4836 / 0	-122.2	-122.2	0.22 (1)	3.78
K- L	0 / 56	-122.2	-122.2	0.10 (1)	10.00
U- B	-4950 / 0	0.0	0.0	0.36 (1)	4.79
M- K	-4803 / 0	0.0	0.0	0.35 (1)	4.87
U-AA	0 / 0	-28.0	-28.0	0.12 (1)	10.00
AA- T	0 / 0	-28.0	-28.0	0.12 (1)	10.00
T-AB	0 / 3778	-28.0	-28.0	0.42 (1)	10.00
AB-AC	0 / 3778	-28.0	-28.0	0.42 (1)	10.00
AC-AD	0 / 3778	-28.0	-28.0	0.42 (1)	10.00
AD- S	0 / 3778	-28.0	-28.0	0.42 (1)	10.00
S- R	0 / 8328	-28.0	-28.0	0.56 (1)	10.00
R- Q	0 / 10207	-28.0	-28.0	0.77 (1)	10.00
Q- P	0 / 10207	-28.0	-28.0	0.77 (1)	10.00
P-AE	0 / 8400	-28.0	-28.0	0.98 (1)	10.00
AE-AF	0 / 8400	-28.0	-28.0	0.98 (1)	10.00
AF- O	0 / 8400	-28.0	-28.0	0.98 (1)	10.00
O-AG	0 / 3645	-28.0	-28.0	0.34 (1)	10.00
AG-AH	0 / 3645	-28.0	-28.0	0.34 (1)	10.00
AH- N	0 / 3645	-28.0	-28.0	0.34 (1)	10.00
N-AI	0 / 0	-28.0	-28.0	0.05 (3)	10.00
AI- M	0 / 0	-28.0	-28.0	0.05 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-920 / 0	0.16 (1)
C- S	0 / 5604	0.99 (1)
S- D	-1752 / 0	0.30 (1)
D- R	0 / 1706	0.30 (1)
R- E	-418 / 0	0.07 (1)
P- H	-433 / 0	0.07 (1)
P- I	0 / 2665	0.47 (1)
O- I	-2654 / 0	0.45 (1)
O- J	0 / 5857	0.75 (1)
N- J	-1199 / 0	0.21 (1)
B- T	0 / 4054	0.72 (1)
N- K	0 / 3924	0.69 (1)
R- F	-922 / 0	0.21 (1)
F- P	0 / 590	0.10 (1)



DWG NO. TAM 47710-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 855 (0.43")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 564 (0.65")

CSI: TC=0.52 (H-I:1) , BC=0.98 (O-P:1) , WB=0.99 (C-S:1) , SSI=0.93 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.98 (B) (INPUT = 1.00)

pg 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T29	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-16	-18	—	FRONT	VERT	DEAD
C	2-11-8	-243	-243	—	FRONT	VERT	SNOW
I	22-6-4	-147	-147	—	FRONT	VERT	TOTAL
J	27-7-8	-16	-18	—	FRONT	VERT	DEAD
J	27-7-8	-243	-243	—	FRONT	VERT	SNOW
O	22-6-4	-40	-70	—	FRONT	VERT	TOTAL
V	4-0-12	-101	-101	—	FRONT	VERT	TOTAL
W	6-0-12	-101	-101	—	FRONT	VERT	TOTAL
X	20-6-4	-147	-147	—	FRONT	VERT	TOTAL
Y	24-6-4	-147	-147	—	FRONT	VERT	TOTAL
Z	26-6-4	-147	-147	—	FRONT	VERT	TOTAL
AA	2-0-12	-26	-46	—	FRONT	VERT	TOTAL
AB	4-0-12	-26	-46	—	FRONT	VERT	TOTAL
AC	6-0-12	-26	-46	—	FRONT	VERT	TOTAL
AD	7-4-8	-1438	-1438	—	FRONT	VERT	TOTAL
AE	19-1-8	-1782	-1782	—	FRONT	VERT	TOTAL
AF	20-6-4	-40	-70	—	FRONT	VERT	TOTAL
AG	24-6-4	-40	-70	—	FRONT	VERT	TOTAL
AH	26-6-4	-40	-70	—	FRONT	VERT	TOTAL
AI	28-6-4	-40	-70	—	FRONT	VERT	TOTAL

DWG NO. TAM 47710-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T30	1	1	TRUSS DESC.	

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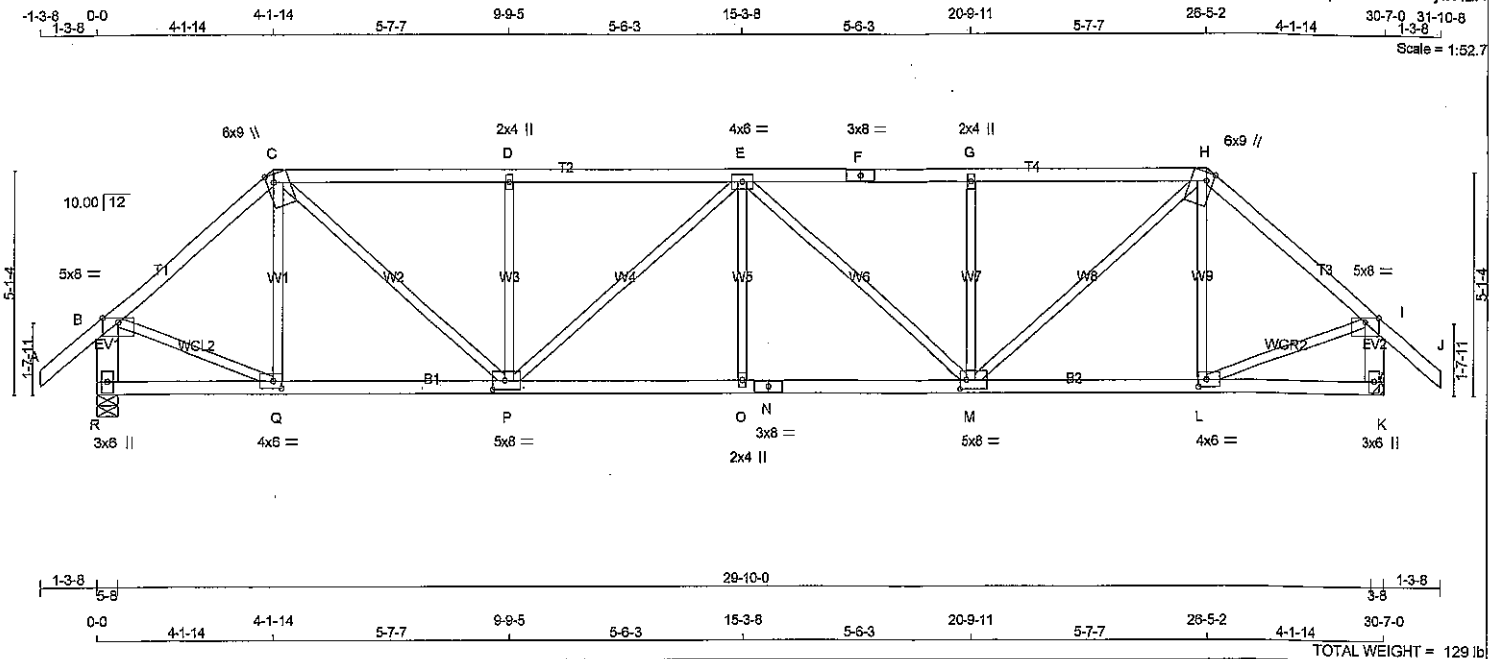


TOTAL WEIGHT = 129 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.25
M	BMWWW-t	MT20	5.0	8.0	2.50	2.00
N	BS-t	MT20	3.0	8.0		
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	3.50
Q	BMWW-t	MT20	4.0	6.0	2.00	2.25
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2466	0	2466	0	0	IN-SX	IN-SX
K	2466	0	2466	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1911	1275 / 0	321 / 0	0 / 0	0 / 0	314 / 0	0 / 0
K	1911	1275 / 0	321 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

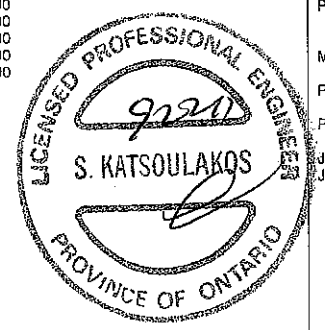
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B-C	-2279 / 0	-122.2	-122.2	0.49 (1)		4.03
C-D	-3265 / 0	-122.2	-122.2	0.84 (1)		3.02
D-E	-3265 / 0	-122.2	-122.2	0.83 (1)		3.02
E-F	-3265 / 0	-122.2	-122.2	0.83 (1)		3.02
F-G	-3265 / 0	-122.2	-122.2	0.83 (1)		3.02
G-H	-3265 / 0	-122.2	-122.2	0.84 (1)		3.02
H-I	-2279 / 0	-122.2	-122.2	0.49 (1)		4.03
I-J	0 / 54	-122.2	-122.2	0.17 (1)		10.00
R-B	-2423 / 0	0.0	0.0	0.17 (1)		6.62
K-I	-2423 / 0	0.0	0.0	0.17 (1)		6.62
R-Q	0 / 0	-28.0	-28.0	0.16 (2)		10.00
Q-P	0 / 1738	-28.0	-28.0	0.37 (1)		10.00
P-O	0 / 3714	-28.0	-28.0	0.67 (1)		10.00
O-N	0 / 3714	-28.0	-28.0	0.67 (1)		10.00
N-M	0 / 3714	-28.0	-28.0	0.67 (1)		10.00
M-L	0 / 1738	-28.0	-28.0	0.37 (1)		10.00
L-K	0 / 0	-28.0	-28.0	0.16 (2)		10.00

WEBS MEMB.	MAX. FORCE (LBS)	CSI (LC)
Q-C	-379 / 60	0.15 (1)
C-P	0 / 2007	0.45 (1)
P-D	-760 / 0	0.29 (1)
P-E	-595 / 0	0.62 (1)
O-E	0 / 269	0.06 (2)
E-M	-595 / 0	0.62 (1)
M-G	-760 / 0	0.29 (1)
M-H	0 / 2007	0.45 (1)
L-H	-379 / 60	0.15 (1)
B-Q	0 / 1827	0.41 (1)
L-I	0 / 1827	0.41 (1)



DWG NO. TAM47711 -17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.84 (C-D:1) , BC=0.67 (O-P:1) , WB=0.62 (E-P:1) , SSI=0.32 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.94 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington — Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Sep 21 10:39:34 2017 Page 1

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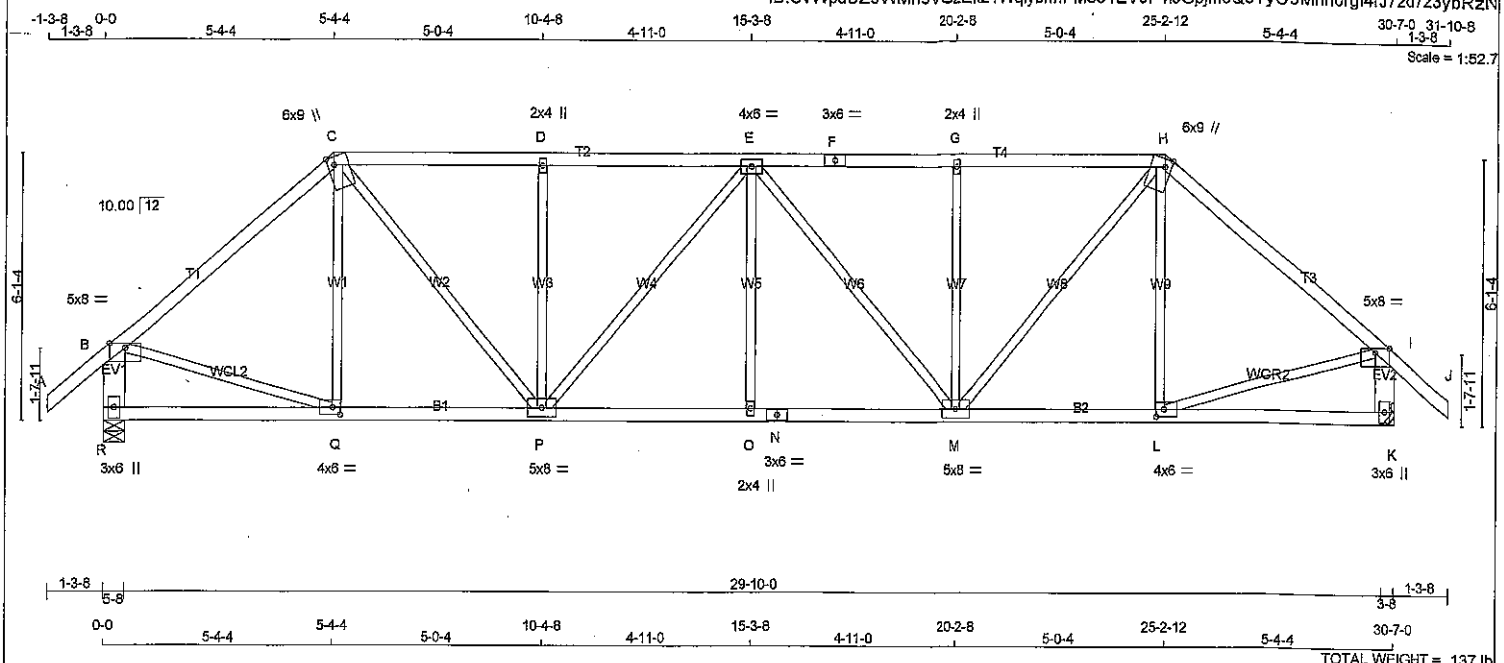

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.25
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	2.25
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2466	0	2466	0	0	IN-SX	IN-SX
K	2466	0	2466	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1911	1275 / 0	321 / 0	0 / 0	0 / 0	314 / 0	0 / 0
K	1911	1275 / 0	321 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	Q- C	-254 / 129	0.15 (1)
B- C	-2312 / 0	-122.2	-122.2	0.87 (1)	3.50	C- P	0 / 1526	0.34 (1)
C- D	-2768 / 0	-122.2	-122.2	0.59 (1)	3.56	P- D	-676 / 0	0.40 (1)
D- E	-2768 / 0	-122.2	-122.2	0.59 (1)	3.56	P- E	-460 / 0	0.53 (1)
E- F	-2768 / 0	-122.2	-122.2	0.59 (1)	3.56	O- E	0 / 248	0.06 (2)
F- G	-2768 / 0	-122.2	-122.2	0.59 (1)	3.56	E- M	-460 / 0	0.53 (1)
G- H	-2768 / 0	-122.2	-122.2	0.59 (1)	3.56	M- G	-676 / 0	0.40 (1)
H- I	-2312 / 0	-122.2	-122.2	0.87 (1)	3.50	M- H	0 / 1526	0.34 (1)
I- J	0 / 54	-122.2	-122.2	0.17 (1)	10.00	L- H	-254 / 129	0.15 (1)
R- B	-2405 / 0	0.0	0.0	0.17 (1)	6.65	B- Q	0 / 1822	0.41 (1)
K- I	-2405 / 0	0.0	0.0	0.17 (1)	6.65	L- I	0 / 1822	0.41 (1)
R- Q	0 / 0	-28.0	-28.0	0.19 (3)	10.00			
Q- P	0 / 1768	-28.0	-28.0	0.39 (1)	10.00			
P- O	0 / 3065	-28.0	-28.0	0.56 (1)	10.00			
O- N	0 / 3065	-28.0	-28.0	0.56 (1)	10.00			
N- M	0 / 3065	-28.0	-28.0	0.56 (1)	10.00			
M- L	0 / 1768	-28.0	-28.0	0.39 (1)	10.00			
L- K	0 / 0	-28.0	-28.0	0.19 (3)	10.00			

DWG NO. TAM47712-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.87 (B-C:1) , BC=0.56 (O-P:1) , WB=0.53 (E-M:1) , SSI=0.29 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.78 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington — Version 8.030.S Oct 5 2016 MiTek Industries, Inc. Thu Sep 21 10:39:35 2017 Page 1

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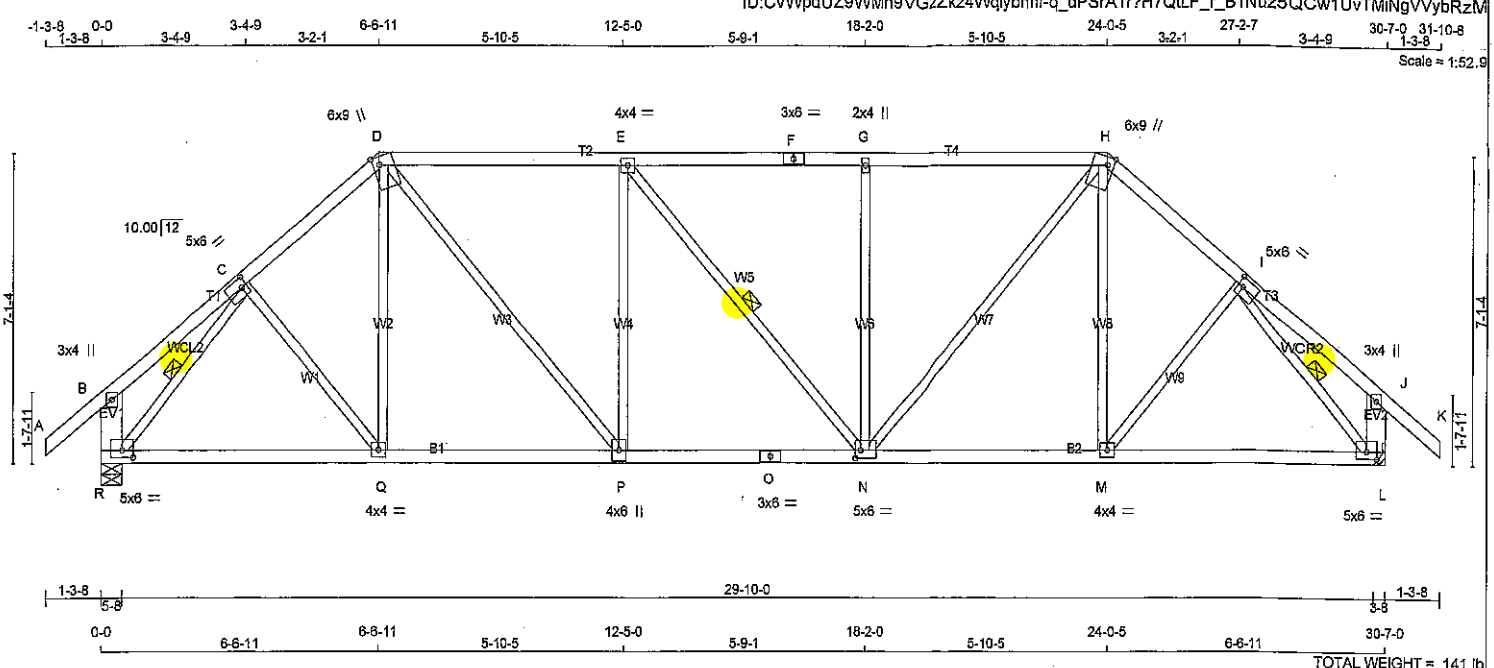

TOTAL WEIGHT = 141 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.50
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	1.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.00	3.00
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	6.0	2.25	1.50
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2466	0	2466	0	0	IN-SX	IN-SX
L	2466	0	2466	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1911	1275 / 0	321 / 0	0 / 0	0 / 0	314 / 0	0 / 0
L	1911	1275 / 0	321 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- Q	0 / 173	0.04 (2)
B- C	0 / 26	-122.2	-122.2	0.19 (1)	10.00	Q- D	0 / 231	0.05 (3)
C- D	-2323 / 0	-122.2	-122.2	0.27 (1)	4.25	D- P	0 / 1216	0.27 (1)
D- E	-2555 / 0	-122.2	-122.2	0.77 (1)	3.44	P- E	-768 / 0	0.67 (1)
E- F	-2553 / 0	-122.2	-122.2	0.76 (1)	3.44	E- N	-3 / 0	0.00 (1)
F- G	-2553 / 0	-122.2	-122.2	0.76 (1)	3.44	N- G	-767 / 0	0.67 (1)
G- H	-2553 / 0	-122.2	-122.2	0.76 (1)	3.45	N- H	0 / 1213	0.27 (1)
H- I	-2324 / 0	-122.2	-122.2	0.27 (1)	4.25	M- H	0 / 231	0.05 (3)
I- J	0 / 26	-122.2	-122.2	0.19 (1)	10.00	M- I	0 / 173	0.04 (2)
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00	R- C	-2655 / 0	0.55 (1)
R- B	-327 / 0	0.0	0.0	0.02 (1)	7.81	I- L	-2656 / 0	0.55 (1)
L- J	-327 / 0	0.0	0.0	0.02 (1)	7.81			
R- Q	0 / 1671	-28.0	-28.0	0.44 (2)	10.00			
Q- P	0 / 1760	-28.0	-28.0	0.46 (2)	10.00			
P- O	0 / 2555	-28.0	-28.0	0.49 (1)	10.00			
O- N	0 / 2555	-28.0	-28.0	0.49 (1)	10.00			
N- M	0 / 1761	-28.0	-28.0	0.46 (2)	10.00			
M- L	0 / 1671	-28.0	-28.0	0.45 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47713-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.77 (D-E:1) , BC=0.49 (N-P:1) , WB=0.67 (E-P:1) , SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.75 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T33	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington — Version 8.030 S Oct. 5 2016 MiTek Industries, Inc. Thu Sep 21 10:39:35 2017 Page 1

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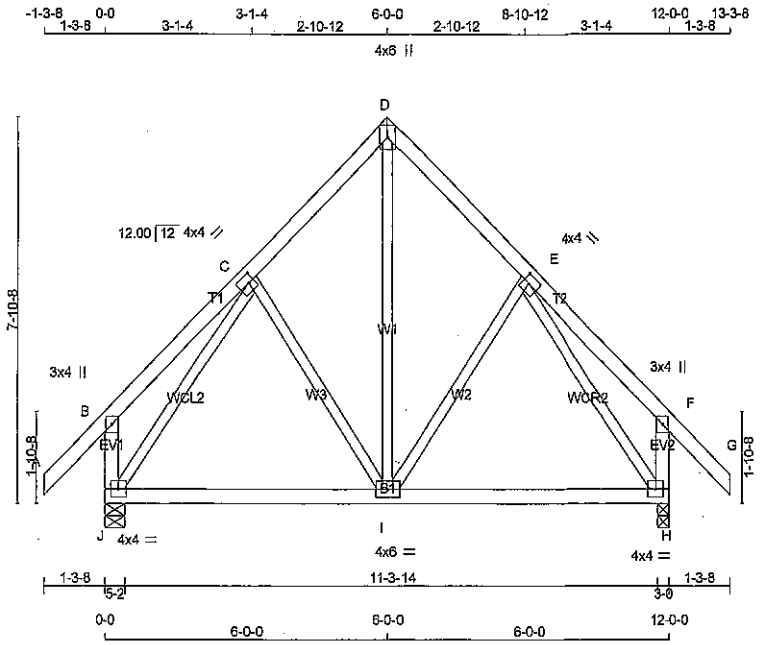


TOTAL WEIGHT = 2 X 84 = 127 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1072	0	1072	0	0	5-2	5-2
H	1072	0	1072	0	0	3-0	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00		I- D	0 / 459	0.10 (1)
B- C	0 / 30	-122.2	-122.2	0.17 (1)	10.00		I- E	-152 / 36	0.08 (1)
C- D	-559 / 0	-122.2	-122.2	0.13 (1)	6.25		C- I	-152 / 36	0.08 (1)
D- E	-559 / 0	-122.2	-122.2	0.13 (1)	6.25		J- C	-836 / 0	0.40 (1)
E- F	0 / 30	-122.2	-122.2	0.17 (1)	10.00		E- H	-836 / 0	0.40 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00				
J- B	-309 / 0	0.0	0.0	0.03 (1)	7.81				
H- F	-309 / 0	0.0	0.0	0.03 (1)	7.81				
J- I	0 / 461	-28.0	-28.0	0.33 (2)	10.00				
I- H	0 / 461	-28.0	-28.0	0.33 (2)	10.00				

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47714-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17 (B-C:1) , BC=0.33 (I-J:2) , WB=0.40 (C-J:1) , SSI=0.14 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

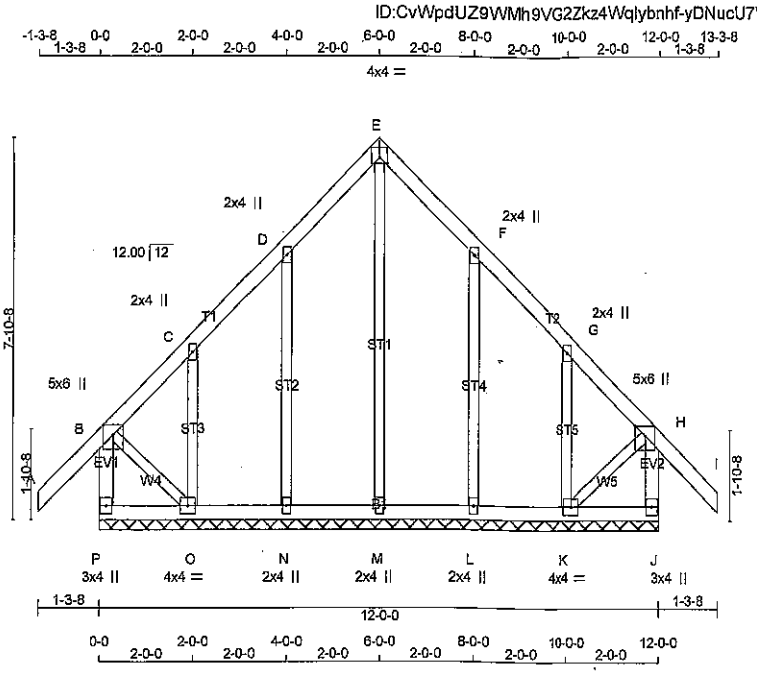
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	G33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 65 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B-C	-76 / 0	-122.2	-122.2	0.16 (1)		6.25
C-D	-17 / 0	-122.2	-122.2	0.08 (1)		6.25
D-E	-42 / 0	-122.2	-122.2	0.08 (1)		6.25
E-F	-42 / 0	-122.2	-122.2	0.08 (1)		6.25
F-G	-17 / 0	-122.2	-122.2	0.08 (1)		6.25
G-H	-76 / 0	-122.2	-122.2	0.16 (1)		6.25
H-I	0 / 60	-122.2	-122.2	0.17 (1)		10.00
P-B	-357 / 0	0.0	0.0	0.04 (1)		7.81
J-H	-357 / 0	0.0	0.0	0.04 (1)		7.81
P-O	0 / 0	-28.0	-28.0	0.03 (2)		10.00
O-N	0 / 22	-28.0	-28.0	0.03 (2)		10.00
N-M	0 / 16	-28.0	-28.0	0.02 (2)		10.00
M-L	0 / 16	-28.0	-28.0	0.02 (2)		10.00
L-K	0 / 22	-28.0	-28.0	0.03 (2)		10.00
K-J	0 / 0	-28.0	-28.0	0.03 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-170 / 0	0.19 (1)
N-D	-292 / 0	0.15 (1)
O-C	-160 / 0	0.04 (1)
L-F	-292 / 0	0.15 (1)
K-G	-160 / 0	0.04 (1)
B-O	0 / 34	0.01 (1)
K-H	0 / 34	0.01 (1)

DWG NO. TAM 47718-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (H-I:1) , BC=0.03 (N-O:2) , WB=0.19 (E-M:1) , SSI=0.09 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (E) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T34A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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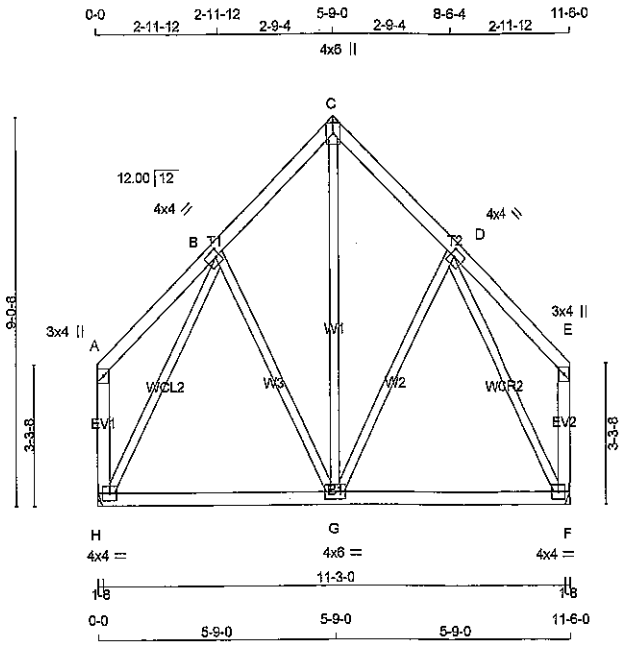

TOTAL WEIGHT = 66 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	864	0	864	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
F	864	0	864	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	675	440 / 0	121 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	675	440 / 0	121 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 29	-122.2	-122.2	0.16 (1)	10.00	G-C	0 / 315	0.07 (1)
B-C	-449 / 0	-122.2	-122.2	0.12 (1)	6.25	G-D	-66 / 62	0.05 (1)
C-D	-449 / 0	-122.2	-122.2	0.12 (1)	6.25	B-G	-66 / 62	0.05 (1)
D-E	0 / 29	-122.2	-122.2	0.16 (1)	10.00	H-B	-746 / 0	0.55 (1)
H-A	-132 / 0	0.0	0.0	0.02 (1)	7.81	D-F	-746 / 0	0.55 (1)
F-E	-132 / 0	0.0	0.0	0.02 (1)	7.81			
H-G	0 / 328	-28.0	-28.0	0.30 (2)	10.00			
G-F	0 / 328	-28.0	-28.0	0.30 (2)	10.00			

DWG NO. TAM-77715-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16 (A-B:1) , BC=0.30 (G-H:2) , WB=0.55 (D-F:1) , SSI=0.14 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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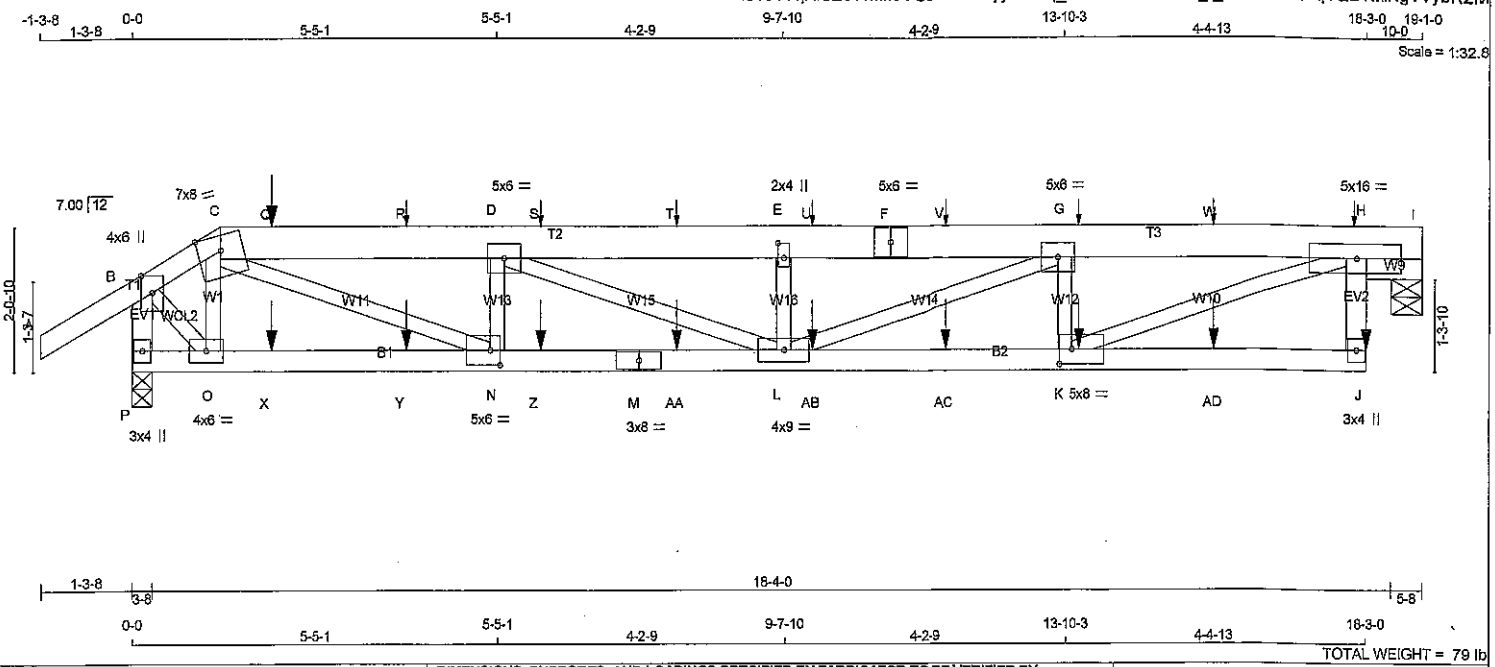


TOTAL WEIGHT = 79 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMVWW-t	MT20	5.0	16.0		
J	BMV+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	8.0	2.50	2.25
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	5.0	6.0	2.50	1.75
O	BMWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4.4 lbs FACTORED DOWN AT 2-0-12, AT 4-0-12, AT 6-0-12, AT 8-0-12, AT 10-0-12, AT 12-0-12, AT 14-0-12, AND AT 16-0-12, AND AT 18-0-12 ON TOP CHORD, AND 2.5 lbs FACTORED DOWN AT 2-0-12, 2.5 lbs FACTORED DOWN AT 4-0-12, 2.5 lbs FACTORED DOWN AT 6-0-12, 2.5 lbs FACTORED DOWN AT 8-0-12, 2.5 lbs FACTORED DOWN AT 10-0-12, 2.5 lbs FACTORED DOWN AT 12-0-12, 2.5 lbs FACTORED DOWN AT 14-0-12, AND 2.5 lbs FACTORED DOWN AT 16-0-12, AND 22.4 lbs FACTORED DOWN AT 18-3-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1616	0	1616	0	0	3-8	3-8
I	1423	0	1423	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): I

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1248	840 / 0	205 / 0	0 / 0	0 / 0	203 / 0	0 / 0
I	1111	726 / 0	197 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 42	-122.2	-122.2	0.18 (1)	10.00	O- C	-628 / 0	0.10 (1)
B- C	-1153 / 0	-122.2	-122.2	0.19 (1)	5.67	B- O	0 / 1243	0.31 (1)
C- Q	-3320 / 0	-122.2	-122.2	0.24 (1)	4.43	K- H	0 / 3670	0.91 (1)
Q- R	-3320 / 0	-122.2	-122.2	0.24 (1)	4.43	C- N	0 / 2504	0.62 (1)
R- D	-3320 / 0	-122.2	-122.2	0.24 (1)	4.43	K- G	-1189 / 0	0.19 (1)
D- S	-4061 / 0	-122.2	-122.2	0.29 (1)	4.02	N- D	-864 / 0	0.14 (1)
S- T	-4061 / 0	-122.2	-122.2	0.29 (1)	4.02	L- G	0 / 681	0.17 (1)
T- E	-4061 / 0	-122.2	-122.2	0.29 (1)	4.02	D- L	0 / 798	0.20 (1)
E- U	-4061 / 0	-122.2	-122.2	0.34 (1)	3.95	L- E	-417 / 0	0.07 (1)
U- F	-4061 / 0	-122.2	-122.2	0.34 (1)	3.95			
F- V	-4061 / 0	-122.2	-122.2	0.34 (1)	3.95			
V- G	-4061 / 0	-122.2	-122.2	0.34 (1)	3.95			
G- W	-3429 / 0	-122.2	-122.2	0.94 (1)	2.64			
W- H	-3429 / 0	-122.2	-122.2	0.94 (1)	2.64			
H- I	0 / 0	-122.2	-122.2	0.52 (1)	10.00			
J- H	0 / 117	0.0	0.0	0.02 (2)	10.00			
P- B	-1655 / 0	0.0	0.0	0.19 (1)	6.40			
P- O	0 / 0	-28.0	-28.0	0.12 (2)	10.00			
O- X	0 / 1007	-28.0	-28.0	0.28 (1)	10.00			
X- Y	0 / 1007	-28.0	-28.0	0.28 (1)	10.00			
Y- N	0 / 1007	-28.0	-28.0	0.28 (1)	10.00			
N- Z	0 / 3320	-28.0	-28.0	0.68 (1)	10.00			
Z- M	0 / 3320	-28.0	-28.0	0.68 (1)	10.00			
M-AA	0 / 3320	-28.0	-28.0	0.68 (1)	10.00			
AA- L	0 / 3320	-28.0	-28.0	0.68 (1)	10.00			
L-AB	0 / 3429	-28.0	-28.0	0.69 (1)	10.00			
AB-AC	0 / 3429	-28.0	-28.0	0.69 (1)	10.00			
AC- K	0 / 3429	-28.0	-28.0	0.69 (1)	10.00			
K-AD	0 / 0	-28.0	-28.0	0.14 (2)	10.00			
AD- J	0 / 0	-28.0	-28.0	0.14 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	14-0-12	—	—	—	BACK	VERT	TOTAL
H	18-0-12	—	—	—	BACK	VERT	TOTAL
J	18-3-0	-13	-22	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9221
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 944 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 618 (0.37")

CSI: TC=0.94 (G-H:1) , BC=0.69 (K-L:1) , WB=0.91 (H-K:1) , SSI=0.58 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.96 (M) (INPUT = 1.00)

PGL

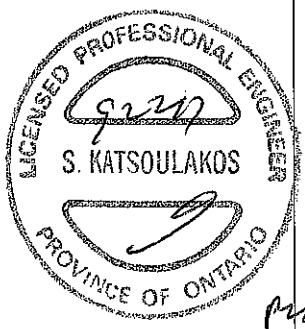
DWG NO. TAM 477617
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T35	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	14-0-12	-1	-3	—	BACK	VERT	TOTAL
Q	2-0-12	-4	-4	—	BACK	VERT	TOTAL
R	4-0-12	—	—	—	BACK	VERT	TOTAL
S	6-0-12	—	—	—	BACK	VERT	TOTAL
T	8-0-12	—	—	—	BACK	VERT	TOTAL
U	10-0-12	—	—	—	BACK	VERT	TOTAL
V	12-0-12	—	—	—	BACK	VERT	TOTAL
W	16-0-12	—	—	—	BACK	VERT	TOTAL
X	2-0-12	-1	-3	—	BACK	VERT	TOTAL
Y	4-0-12	-1	-3	—	BACK	VERT	TOTAL
Z	6-0-12	-1	-3	—	BACK	VERT	TOTAL
AA	8-0-12	-1	-3	—	BACK	VERT	TOTAL
AB	10-0-12	-1	-3	—	BACK	VERT	TOTAL
AC	12-0-12	-1	-3	—	BACK	VERT	TOTAL
AD	16-0-12	-1	-3	—	BACK	VERT	TOTAL



DWG NO. YAM 4716-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Sep 21 10:39:36 2017 Page 1

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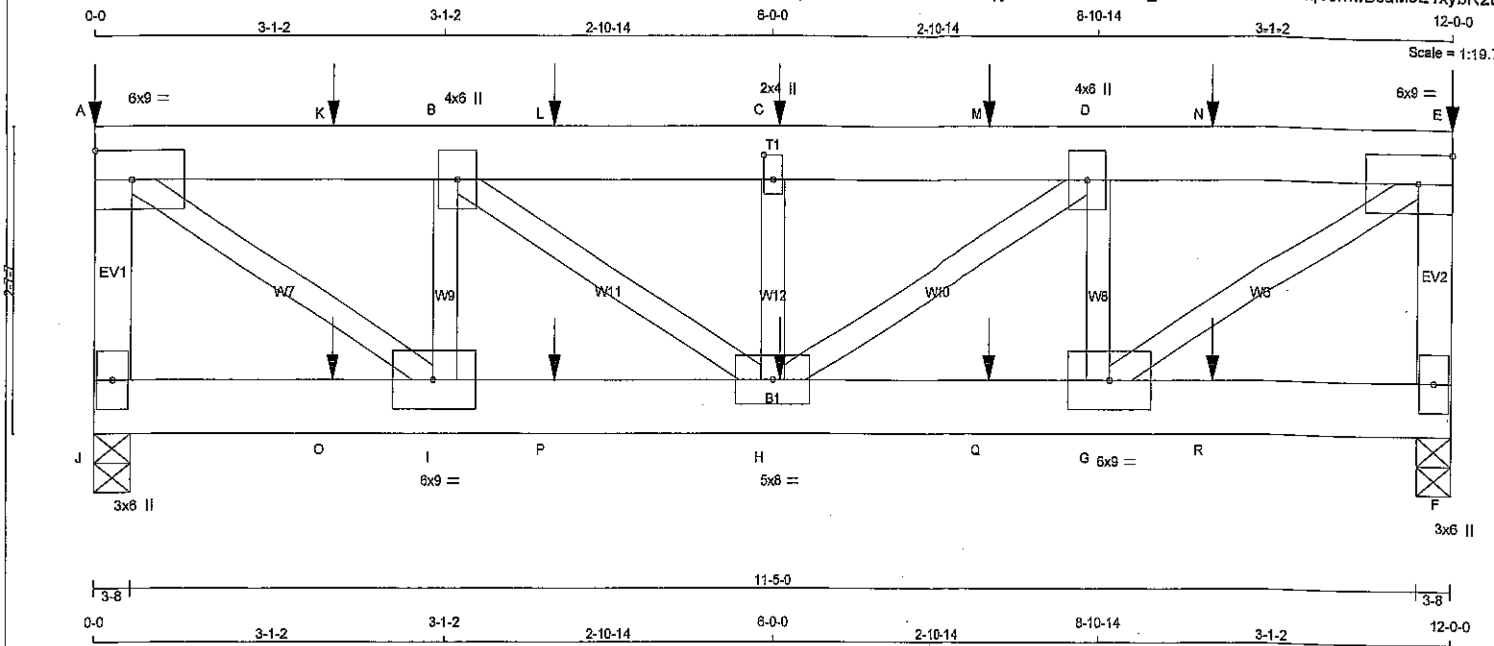

TOTAL WEIGHT = 61 lb
(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0		Edge
B	TMWW+t	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TMWW+t	MT20	4.0	6.0		
E	TMVW-t	MT20	6.0	9.0		Edge
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	6.0	9.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	6.0	9.0		
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 70.3 lbs FACTORED DOWN AND 15.6 lbs FACTORED UP AT 0-0, 16.5 lbs FACTORED DOWN AND 15.6 lbs FACTORED UP AT 2-0-12, 16.5 lbs FACTORED DOWN AND 15.6 lbs FACTORED UP AT 4-0-12, 16.5 lbs FACTORED DOWN AND 15.6 lbs FACTORED UP AT 6-0-12, 16.5 lbs FACTORED DOWN AND 15.6 lbs FACTORED UP AT 7-11-4, AND 16.5 lbs FACTORED DOWN AND 15.6 lbs FACTORED UP AT 9-11-4, AND 70.3 lbs FACTORED DOWN AND 15.6 lbs FACTORED UP AT 12-0-0 ON TOP CHORD, AND 437.3 lbs FACTORED DOWN AT 2-0-12, 437.3 lbs FACTORED DOWN AT 4-0-12, 437.3 lbs FACTORED DOWN AT 6-0-12, AND 437.3 lbs FACTORED DOWN AT 7-11-4, AND 437.3 lbs FACTORED DOWN AT 9-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	2106	0	2106	0	0	3-8	3-8
F	2106	0	2106	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1628	1082 / 0	266 / 0	0 / 0	0 / 0	280 / 0	0 / 0
F	1628	1082 / 0	266 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- A	-1959 / 0	0.0	0.0	0.25 (1)	5.97	G- E	0 / 2960	0.73 (1)
A- K	-2426 / 0	-122.2	-122.2	0.15 (1)	5.13	A- I	0 / 2960	0.73 (1)
K- B	-2426 / 0	-122.2	-122.2	0.15 (1)	5.13	G- D	-932 / 0	0.16 (1)
B- L	-3143 / 0	-122.2	-122.2	0.16 (1)	4.61	I- B	-932 / 0	0.16 (1)
L- C	-3143 / 0	-122.2	-122.2	0.16 (1)	4.61	H- D	0 / 893	0.22 (1)
C- M	-3143 / 0	-122.2	-122.2	0.16 (1)	4.61	B- H	0 / 893	0.22 (1)
M- D	-3143 / 0	-122.2	-122.2	0.16 (1)	4.61	H- C	-364 / 0	0.06 (1)
D- N	-2426 / 0	-122.2	-122.2	0.15 (1)	5.13			
N- E	-2426 / 0	-122.2	-122.2	0.15 (1)	5.13			
F- E	-1959 / 0	0.0	0.0	0.25 (1)	5.97			
J- O	0 / 0	-28.0	-28.0	0.18 (1)	10.00			
O- I	0 / 0	-28.0	-28.0	0.18 (1)	10.00			
I- P	0 / 2426	-28.0	-28.0	0.48 (1)	10.00			
P- H	0 / 2426	-28.0	-28.0	0.48 (1)	10.00			
H- Q	0 / 2426	-28.0	-28.0	0.48 (1)	10.00			
Q- G	0 / 2426	-28.0	-28.0	0.48 (1)	10.00			
G- R	0 / 0	-28.0	-28.0	0.18 (1)	10.00			
R- F	0 / 0	-28.0	-28.0	0.18 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-70	-70	16	FRONT	VERT	TOTAL
C	6-0-12	-16	-16	16	FRONT	VERT	TOTAL
E	12-0-0	-70	-70	16	FRONT	VERT	TOTAL
H	6-0-12	-437	-437	—	FRONT	VERT	TOTAL
K	2-0-12	-16	-16	16	FRONT	VERT	TOTAL
L	4-0-12	-16	-16	16	FRONT	VERT	TOTAL
M	7-11-4	-16	-16	16	FRONT	VERT	TOTAL
N	9-11-4	-16	-16	16	FRONT	VERT	TOTAL
O	2-0-12	-437	-437	—	FRONT	VERT	TOTAL
P	4-0-12	-437	-437	—	FRONT	VERT	TOTAL
Q	7-11-4	-437	-437	—	FRONT	VERT	TOTAL
R	9-11-4	-437	-437	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.25 (E-F:1) , BC=0.48 (H-I:1) , WB=0.73 (E-G:1) , SSI=0.26 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.42 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47717-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
288283	P1	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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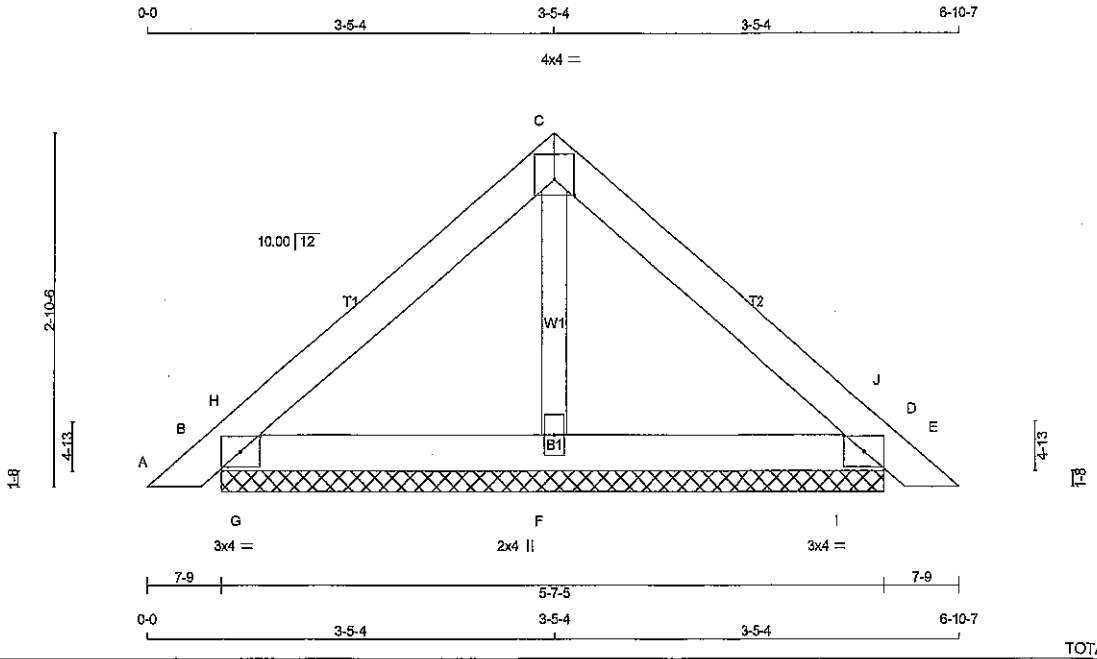

TOTAL WEIGHT = 7 X 18 = 128 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	335	0	335	0	0	5-7-5	5-7-5
D	335	0	335	0	0	5-7-5	5-7-5
F	303	0	303	0	0	5-7-5	5-7-5

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	247	167 / 0	27 / 0	0 / 0	0 / 0	33 / 0	0 / 0
D	247	167 / 0	27 / 0	0 / 0	0 / 0	33 / 0	0 / 0
F	254	137 / 0	64 / 0	0 / 0	0 / 0	53 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 19	-122.2	-122.2	0.03 (1)	10.00	F- C	-121 / 0	0.02 (1)	
B- H	-52 / 0	-122.2	-122.2	0.05 (1)	6.25	G- H	-322 / 36	0.00 (1)	
H- C	-136 / 0	-122.2	-122.2	0.11 (1)	6.25	I- J	-322 / 36	0.00 (1)	
C- J	-136 / 0	-122.2	-122.2	0.11 (1)	6.25				
J- D	-52 / 0	-122.2	-122.2	0.05 (1)	6.25				
D- E	0 / 19	-122.2	-122.2	0.03 (1)	10.00				
B- G	0 / 97	-28.0	-28.0	0.12 (1)	10.00				
G- F	0 / 97	-28.0	-28.0	0.12 (1)	10.00				
F- I	0 / 97	-28.0	-28.0	0.12 (1)	10.00				
I- D	0 / 97	-28.0	-28.0	0.12 (1)	10.00				

DWG NO. TAM 47691-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (C-J:1) , BC=0.12 (F-I:1) , WB=0.02 (C-F:1) , SSI=0.24 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288291	J7	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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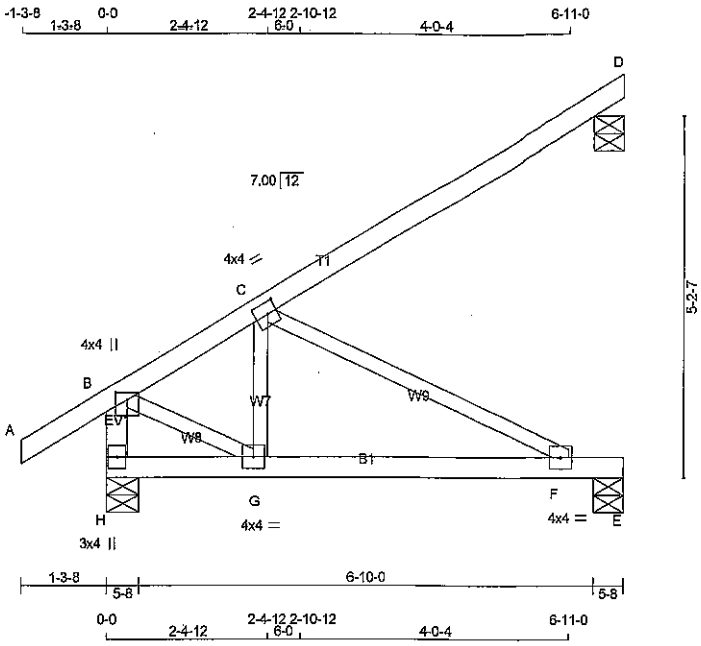


TOTAL WEIGHT = 4 X 29 = 117 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	BMW-t	MT20	4.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	749	0	749	0	0	5-8	5-8
E	304	0	304	0	0	5-8	5-8
D	278	0	278	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	568	401 / 0	81 / 0	0 / 0	0 / 0	86 / 0	0 / 0
E	266	124 / 0	79 / 0	0 / 0	0 / 0	63 / 0	0 / 0
D	189	172 / 0	2 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-122.2	-122.2	1	0.16 (1)	10.00	G-C	-10 / 206	0.05 (3)
B-C	-620 / 0	-122.2	-122.2	1	0.32 (1)	6.25	B-G	0 / 644	0.14 (1)
C-D	-28 / 0	-122.2	-122.2	1	0.43 (1)	6.25	C-F	-667 / 0	0.31 (1)
H-B	-773 / 0	0.0	0.0	1	0.08 (1)	7.81			
H-G	0 / 0	-28.0	-28.0	2	0.21 (2)	10.00			
G-F	0 / 593	-28.0	-28.0	1	0.47 (1)	10.00			
F-E	0 / 0	-28.0	-28.0	1	0.38 (1)	10.00			

DWG NO. TAM 47719-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 628 (0.15")

CSI: TC=0.43 (C-D:1) , BC=0.47 (F-G:1) , WB=0.31 (C-F:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.23 (F) (INPUT = 1.00)

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

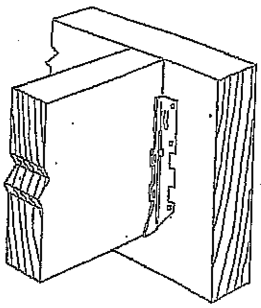
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

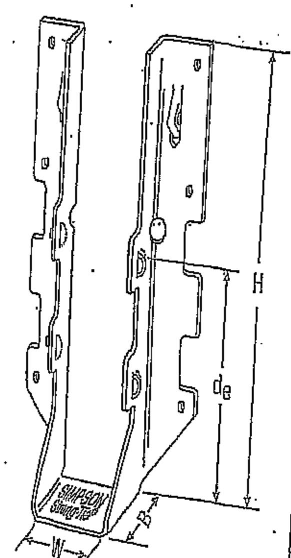
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



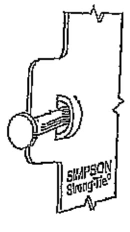
Typical LUS Installation



LUS28

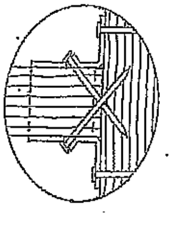
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3⅛	1¾	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4¾	1¾	3⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4 3/16	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6⅝	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1¾	3⅞	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8 3/16	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

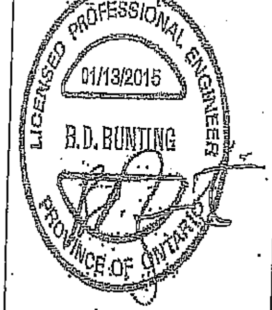


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

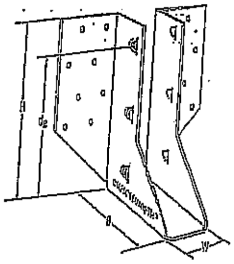
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

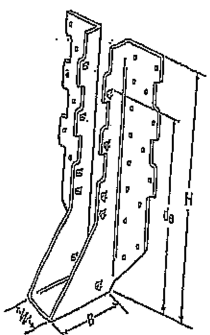
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

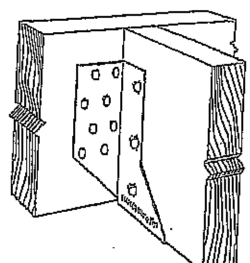
- See current catalogue for options

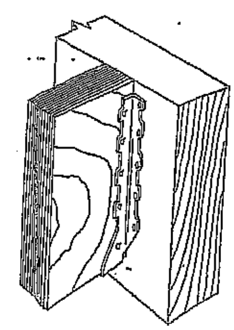


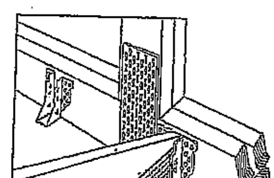
LJS26DS



HUS210
(HUS26, HUS28, similar)


Typical LJS26DS Installation


Typical HUS Installation


Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

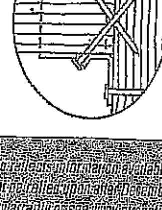
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1 9/16	6	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 9/32	22-16d	8-16d	3605	5965	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

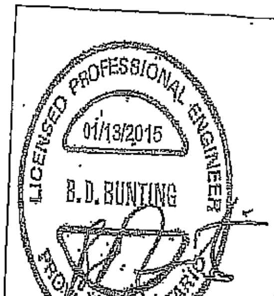
1. d_e is the distance from the seat of the hanger to the highest joist nail.

Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580

Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

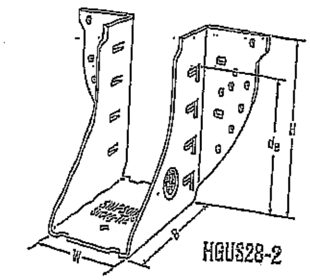
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

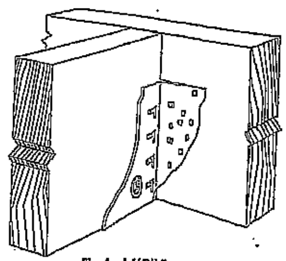
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

INSTALLATION:

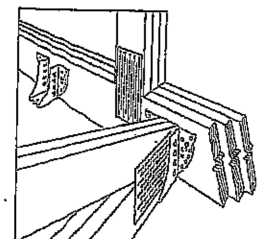
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



Typical HGUS Installation

OPTIONS:

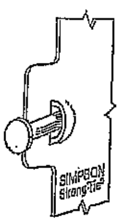
- See current catalogue for options.



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

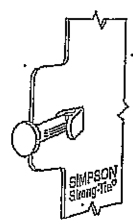
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1 5/8	5 3/8	5	4 5/32	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 5/8	7 1/8	5	6 1/8	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 9/16	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 1/4	4	6 3/8	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 9/16	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3 5/16	9 3/16	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9 1/4	4	8 3/8	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10 5/8	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12 5/8	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

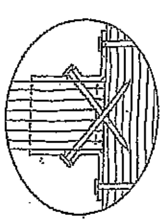


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.

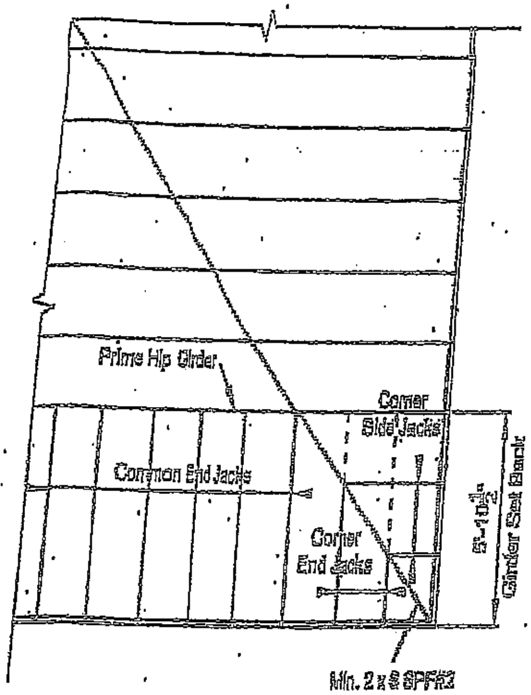


Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
01/13/2015
B. D. BUNTING
PROVINCE OF ONTARIO

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

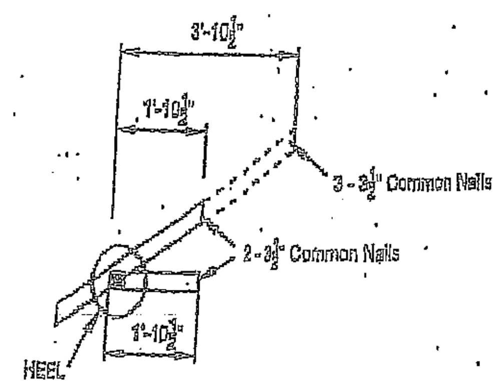
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

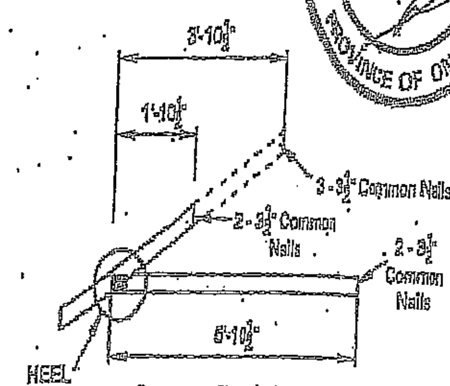
TOP CHORD LIVE LOAD	: 39.4 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F
TOTAL LOAD	: 59.9 P.S.F.

DWG NO TAM 2475.14
STRUCTURAL
COMPONENT ONLY

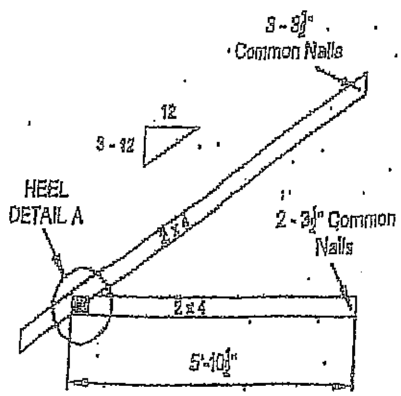
LICENSED PROFESSIONAL ENGINEER
1-20-17
S. KATSOULAKOS
PROVINCE OF ONTARIO



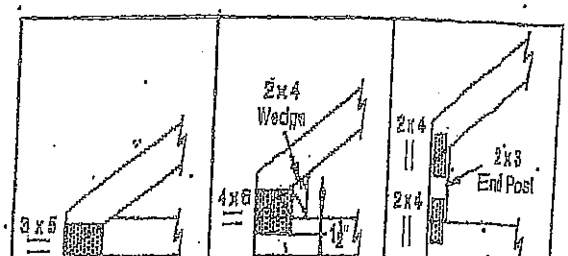
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

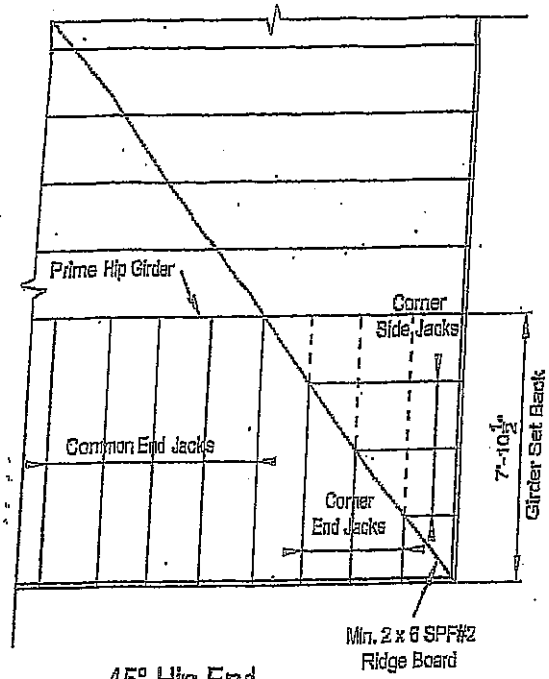
Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO UNIT
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



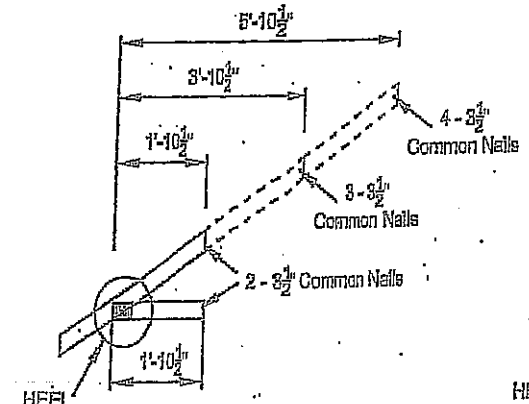
45° Hip End

DESIGN LOAD:

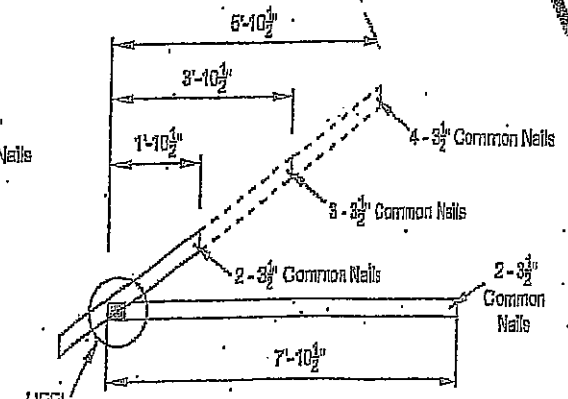
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TRM 3503 14
STRUCTURAL COMPONENT ONLY

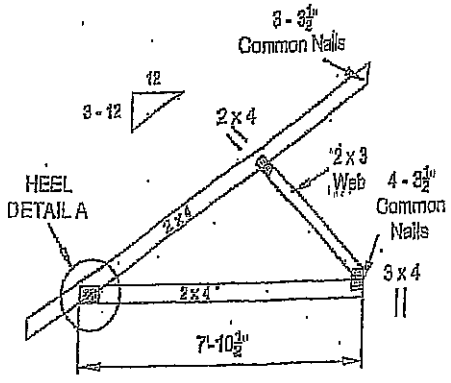
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



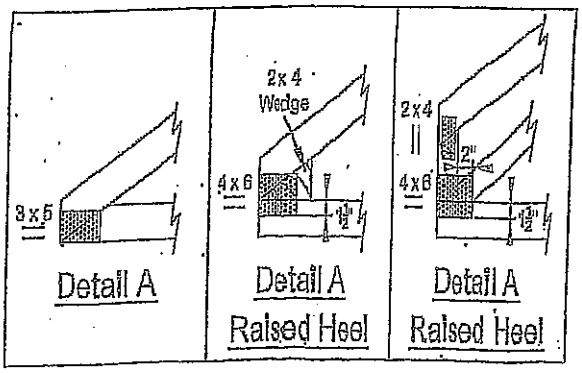
Corner Side Jacks



Corner End Jacks



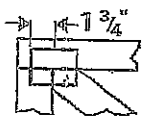
Common End Jacks



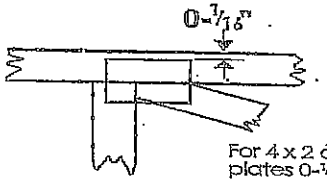
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

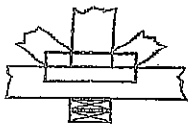
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

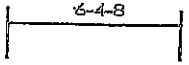
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

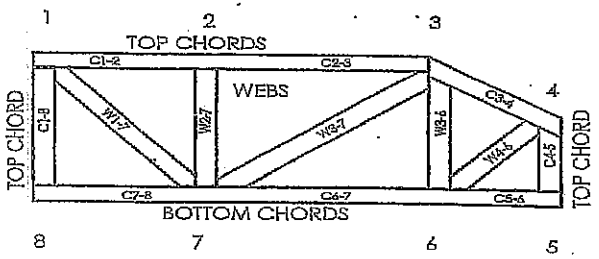
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS
CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER 1-2079 S. KATSOULAKOS PROVINCE OF ONTARIO
DWG NO TAM 2[illegible]4
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.