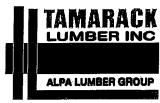


PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	9 1/2" NI-40x	1	2
J2	18-00-00	9 1/2" NI-40x	2	4
J3	16-00-00	9 1/2" NI-40x	1	4
J4	14-00-00	9 1/2" NI-40x	1	26
J5	14-00-00	9 1/2" NI-40x	2	16
J6	12-00-00	9 1/2" NI-40x	1	21
J7	8-00-00	9 1/2" NI-40x	1	23
J8	6-00-00	9 1/2" NI-40x	1	3
J9	4-00-00	9 1/2" NI-40x	1	9
J10	18-00-00	9 1/2" NI-80	1	18
B6	18-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B1	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary							
Qty Manuf Product							
6	H1	IUS2.56/9.5					
8	H1	IUS2.56/9.5					
10	H1	IUS2.56/9.5					
9	H1	IUS2.56/9.5					
2	H2	HUS1.81/9.5					
1	H2	HUS1.81/9.5					

Town of Innisfil Certified Model

03/01/2018 2:19:41 PM kgervais



FROM PLAN DATED: NOV 2015

BUILDER:

**BAYVIEW WELLINGTON** 

SITE:

**ALCONA SHORES** 

MODEL: S45-2

**ELEVATION: A&B** 

LOT:

CITY: INNISFILL

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

**CERAMIC TILE APPLICATION** 

AS PER O.B.C. 9.30.6. SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING

WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS.

**CANTILEVERED JOISTS** 

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE

AT ENDS.

REFER TO THE NORDIC

INSTALLATION GUIDE FOR PROPER

STORAGE AND INSTALLATION.

LOADING:

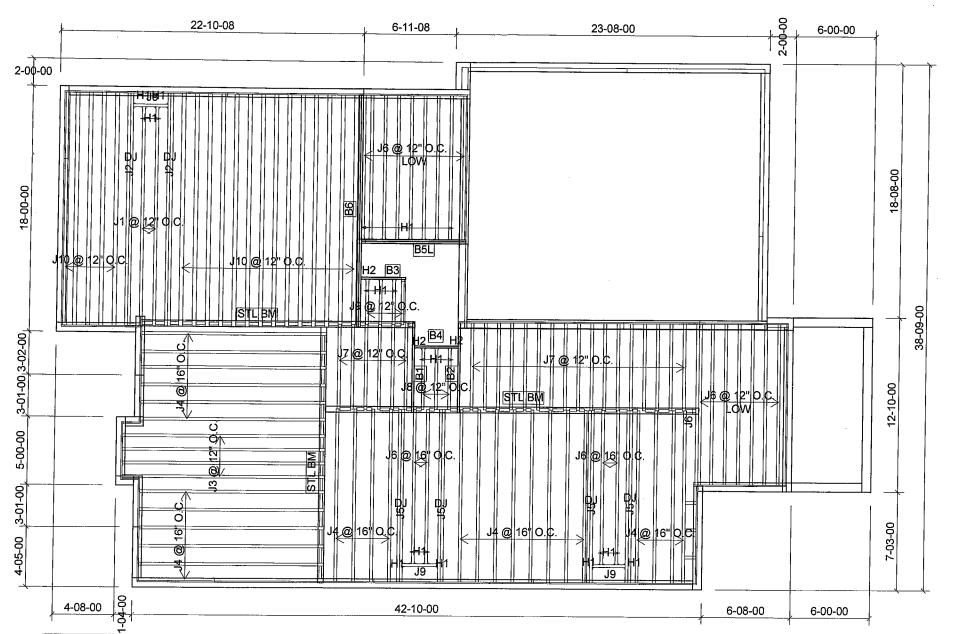
DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 9/1/2017

# 1st FLOOR



	Products					
PlotID	Length	Product	Plies	Net Qty		
J1	18-00-00	9 1/2" NI-40x	1	2		
J2	18-00-00	9 1/2" NI-40x	2	4		
J3	16-00-00	9 1/2" NI-40x	1	4		
J4	14-00-00	9 1/2" NI-40x	1	28		
J5	14-00-00	9 1/2" NI-40x	2	8		
J6	12-00-00	9 1/2" NI-40x	1	21		
J7	8-00-00	9 1/2" NI-40x	1	23		
J8	6-00-00	9 1/2" NI-40x	1	3		
J9	4-00-00	9 1/2" NI-40x	1	7		
J10	18-00-00	9 1/2" NI-80	1	19		
B6	18-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B1	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B2	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B5L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B3	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		

Connector Summary							
Qty	Manuf	Product					
6	H1	IUS2.56/9.5					
8	H1	IUS2.56/9.5					
6	H1	IUS2.56/9.5					
6	H1	IUS2.56/9.5					
2	H2	HUS1.81/9.5					
1	H2	HUS1.81/9.5					

Town of Innisfil Certified Model 03/01/2018 2:19:47 PM kgervais



FROM PLAN DATED: NOV 2015

BUILDER:

**BAYVIEW WELLINGTON** 

SITE:

**ALCONA SHORES** 

MODEL: S45-2

**ELEVATION: A&B** 

LOT:

CITY: INNISFILL

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

**CERAMIC TILE APPLICATION** 

AS PER O.B.C. 9.30.6. SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING

WALLS

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS.

**CANTILEVERED JOISTS** 

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE

AT ENDS.

REFER TO THE NORDIC

**INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION.** 

LOADING:

DESIGN LOADS: L/480.000

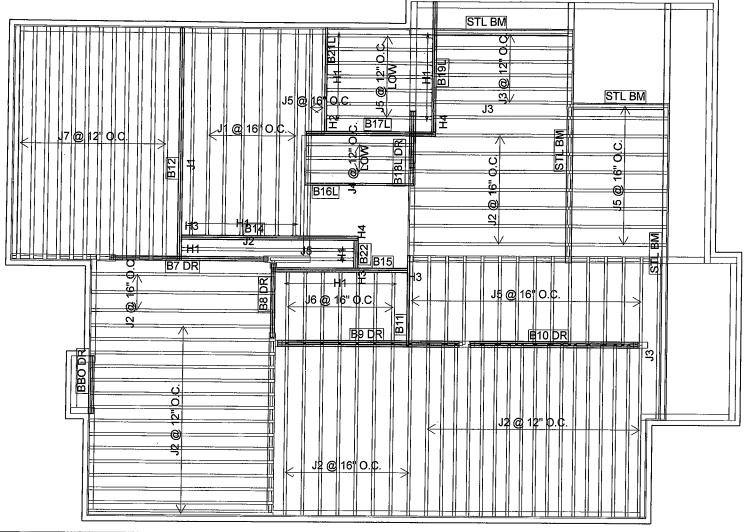
LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 9/1/2017

# 1st FLOOR

**W.O.B & W.O.D** 



PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	7
J2	14-00-00	9 1/2" NI-40x	1	51
J3	12-00-00	9 1/2" NI-40x	1	8
J4	10-00-00	9 1/2" NI-40x	1	3
J5	8-00-00	9 1/2" NI-40x	1	33
J6	6-00-00	9 1/2" NI-40x	1	7
J7	18-00-00	9 1/2" NI-80	1	12
B18L DR	6-00-00	1-3/4" x 7-1/4" VERSA-LAM® 2.0 3100 SP	3	3
B12	18-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B14	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B10 DR	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B9 DR	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B15	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7 DR	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B16L	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B17L	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B19L	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B21L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B8 DR	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B22	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

Connector Summary							
Qty	Product						
7	H1	IUS2.56/9.5					
24	H1	IUS2.56/9.5					
1	H2	HUS1.81/9.5					
1	H3	HGUS410					
2	H3	HGUS410					
2	H4	HUC410					

Town of Innisfil Certified Model
03/01/2018 2:19:51 PM kgervais



FROM PLAN DATED: NOV 2015

**BUILDER:** 

**BAYVIEW WELLINGTON** 

SITE:

**ALCONA SHORES** 

MODEL: S45-2

**ELEVATION: A** 

LOT:

**CITY: INNISFILL** 

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

**CERAMIC TILE APPLICATION** 

AS PER O.B.C. 9.30.6.

**SQUASH BLOCKS** 

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING

WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS.

**CANTILEVERED JOISTS** 

REQUIRE I-JOIST BLOCKING ALONG

BEARING AND RIMBOARD CLOSURE

AT ENDS.

REFER TO THE NORDIC

INSTALLATION GUIDE FOR PROPER

STORAGE AND INSTALLATION.

LOADING:

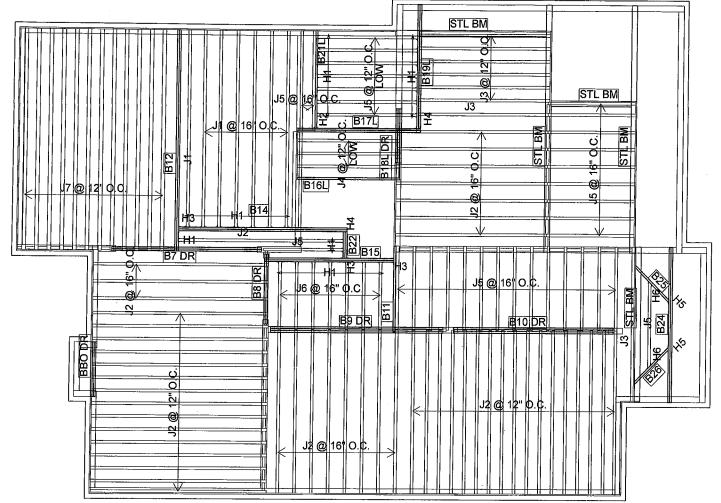
DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 fb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 9/1/2017

# 2nd FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	7
J2	14-00-00	9 1/2" NI-40x	1	51
J3	12-00-00	9 1/2" NI-40x	1	8
J4	10-00-00	9 1/2" NI-40x	1	3
J5	8-00-00	9 1/2" NI-40x	1	34
J6	6-00-00	9 1/2" NI-40x	1	7
J7	18-00-00	9 1/2" NI-80	1	12
B18L DR	6-00-00	1-3/4" x 7-1/4" VERSA-LAM® 2.0 3100 SP	3	3
B12	18-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B14	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B24	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B10 DR	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B9 DR	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B15	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7 DR	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B16L	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B17L	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B19L	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B21L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B8 DR	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B25	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B26	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B22	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

	Connector	Summary
Qty	Manuf	Product
7	H1	IUS2.56/9.5
24	H1	IUS2.56/9.5
1	H2	HUS1.81/9.5
1	H3	HGUS410
2	H3	HGUS410
2	H4	HUC410
1	H5	LSSUI25
1	H5	LSSUI25
1	H6	LSSUH310
1	H6	LSSUH310

Town of Innisfil Certified Model
03/01/2018 2:22:06 PM kgervais



FROM PLAN DATED: NOV 2015

BUILDER:

BAYVIEW WELLINGTON

SITE:

**ALCONA SHORES** 

**MODEL:** S45-2

**ELEVATION: B** 

LOT:

**CITY: INNISFILL** 

SALESMAN: M D
DESIGNER: AJ
REVISION:

NOTES:

**CERAMIC TILE APPLICATION** 

AS PER O.B.C. 9.30.6.

SQUASH BLOCKS

2x4 OR 2x6 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING

WALLS.

MULTIPLE SQUASH BLOCKS REQ'D

UNDER CONCENTRATED LOADS.

**CANTILEVERED JOISTS** 

REQUIRE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE

AT ENDS.

REFER TO THE NORDIC

INSTALLATION GUIDE FOR PROPER

STORAGE AND INSTALLATION.

LOADING:

DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 9/1/2017

# 2nd FLOOR

# NORDIC **STRUCTURES**

**COMPANY** TAMARACK LUMBER 3269 NORTH SERVICE ROAD BURLINGTON ON Sep. 22, 2016 12:33

PROJECT **BAYVIEW WELLINGTON ALCONA SHORES** 45-2 J7 2ND FLOOR Beam1

### **Design Check Calculation Sheet**

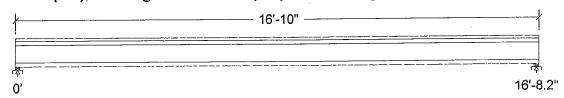
Nordic Sizer - Canada 6.3.1

**Town of Innisfil Certified Model** 03/01/2018 2:22:21 PM kgervais

### Loads:

Load	Туре	Distribution	Pat-	Location [ft] Magnitude		е	Unit	
	- 11 -	i	tern	Start	End	Start	End	
Load1	Dead	Full Area				20.00		psf
Load2	Live	Full Area				40.00		psf

## Maximum Reactions (lbs), Bearing Resistances (lbs) and Bearing Lengths (in):



Unfactored: Dead Live	167 334		167 334
Factored:	700		709
Total	709		, 0 5
Bearing:   Resistance			
Joist	1893		1893
Support	3813	'	3813
Anal/Des			
Joist	0.37		0.37
Support	0.19		0.19
Load case	#2		#2
Length	1-3/4		1-3/4
Min req'd	1-3/4		1-3/4
Stiffener	No		No
KB support	1.00		1.00
fcp sup	769	•	769
Kzcp sup	1.00		1.00

### Nordic Joist 9-1/2" NI-80 Floor joist @ 12" o.c.

Supports: All - Lumber Sill plate, No.1/No.2

Total length: 16'-10.0"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling

This section PASSES the design code check.

### Limit States Design using CSA-086-09 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 709	Vr = 1895	lbs	Vf/Vr = 0.37
Moment(+)	Mf = 2958	Mr = 8958	lbs-ft	Mf/Mr = 0.33
Perm. Defl'n	$0.11 = \langle L/999$	0.56 = L/360	in	FESSION 0.20
Live Defl'n	0.22 = L/922	0.42 = L/480	in QR	0.52
Total Defl'n	0.33 = L/615	0.83 = L/240	in /	572 0.39
Bare Defl'n	0.24 = L/826	0.56 = L/360	in/&	A STATE OF THE PERSON NAMED IN STATE OF THE P
Vibration	Lmax = 16'-8	Lv = 17'-10	ft	TSOULAKOS \$\bigg\ 0.82
Defl'n	= 0.031	= 0.038	ir S. K	MISUULAKUS \$ 0.82
L	<u> </u>		ă.	

STRUCTURAL STRUCTURAL

COMPONENT ONLY

### WoodWorks® Sizer

### for NORDIC STRUCTURES

Beam1

Additional Data:

#### Nordic Sizer - Canada 6.3.1

Page 2

1	Additional	Data:									
	FACTORS:	f/E	KD	KH	KZ	KL	KT	KS		LC#	
	Vr	1895	1.00	1.00	-	-	_	-	-		
i	Mr+				-	1.000	_	-	-	—	
	EI	324.1 m	illion	_	=	-	-	-	-	#2	
Į	CRITICAL LO	AD COMB	INATIONS	<b>S</b> :							
ĺ	Shear										
ı	Moment(+)										
Į	Deflection										
l				+ 1.0L							
l				+ 1.0L							
l				+ 1.0L							
I	Bearing	: Suppo	rt 1 - I	LC #2 = 1	1.25D +	1.5L					
i		Suppo	rt 2 - I	LC #2 = 1	1.25D +	1.5L		_			
l	Load Types	s: D=dea	d W=wir	nd S=sno	ow H=ea	irth,grou	ındwate	r E=ear	thquake		
ļ						Lve(stora			r=r1re		
١	Load Patte	erns: s=	S/2 L=I	J+Ls _=1	no patte	ern load	in this	s span			
l	All Load C		ions (LO	Cs) are .	listed i	in the An	alysis	output			
l	CALCULATIO										
ļ	Deflection	: EIco	mp =	367e06	lb-in2	K = 4.94	e06 lbs	3	. ,		
l	"Live" def	lection	= Defle	ection fi	rom all	non-dead	Loads	(live,	wind, si	now)	
ı											

### **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC Part 4) and the CSA O86-09 Engineering Design in Wood standard, which includes Update No.1. CONFORMS TO DBC 2012
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWG NO.TAM 447 ∞ 17
STRUCTURAL
COMPONENT ONLY

# NORDIC STRUCTURES

COMPANY TAMARACK LUMBER 3269 NORTH SERVICE ROAD BURLINGTON ON Sep. 22, 2016 12:35 PROJECT
BAYVIEW WELLINGTON
ALCONA SHORES
45-2
J3 1ST FLOOR
Beam1

### **Design Check Calculation Sheet**

Nordic Sizer – Canada 6.3.1

**Town of Innisfil Certified Model** 

03/01/2018 2:22:33 PM kgervais

#### Loads:

Load	Type	Distribution	Pat-	Location	[ft]	Magnitud	de	Unit
			tern	Start	End	Start	End	
Load1	Dead	Full Area			,	20.00		psf
Load2	Live	Full Area				40.00		psf

## Maximum Reactions (lbs), Bearing Resistances (lbs) and Bearing Lengths (in):



Unfactored: Dead Live	151 302		151 302
Factored: Total	642		642
Bearing: Resistance			
Joist	1855		1855
Support	2724		2724
Anal/Des		,	
Joist	0.35		0.35
Support	0.24		0.24
Load case	#2	'	#2
Length	1-3/4		1-3/4
Min req'd	1-3/4	·	1-3/4
Stiffener	No		No
KB support	1.00		1.00
fcp sup	769	·	769
Kzcp sup	1.00		1.00

### Nordic Joist 9-1/2" NI-40x Floor joist @ 12" o.c.

Supports: All - Lumber Sill plate, No.1/No.2
Total length: 15'-3.0"; 5/8" nailed and glued OSB sheathing
This section PASSES the design code check.

### Limit States Design using CSA-086-09 and Vibration Criterion:

			·	.,
Criterion	Analysis Value	Design Value	Unit Analysis/Des	
Shear	Vf = 642	Vr = 1895	lbs Vf/Vr =	0.34
Moment(+)	Mf = 2423	Mr = 4824	lbs-ft Mr =	0.50
Perm. Defl'n	$0.10 = \langle L/999 \rangle$	0.50 = L/360	in SGFESSIONA	0.20
Live Defl'n	0.20 = L/890	0.38 = L/480	in	0.54
Total Defl'n	0.31 = L/593	0.76 = L/240	in (3/ 9/1)	0.40
Bare Defl'n	0.24 = L/765	0.50 = L/360	in 5	0.47
Vibration	Lmax = 15'-1	Lv = 16'-3	ft 3 s. KATSOULAKOS 2	
Defl'n	= 0.034	= 0.043	in 3 3 Mil 300 Ellis	0.79
		<del></del>	A AND THE PROPERTY OF THE PROP	

POLINCE OF ONTREE DWG NO. TAM 44701-17
STRUCTURAL
COMPUNENT ONLY

### WoodWorks® Sizer

### for NORDIC STRUCTURES

#### Beam1

#### Nordic Sizer - Canada 6.3.1

Page 2

Additional Data:	
1110101101 2/2 114 114	I LC#
1 1 1000 1:00	+2
1111. 1021 1.00 1.00	#2
EI 218.1 million	+2
CRITICAL LOAD COMBINATIONS:	
Shear : LC $\#2 = 1.25D + 1.5L$	
Moment(+): LC $\#2 = 1.25D + 1.5L$	
Deflection: LC #1 = 1.0D (permanent)	
LC #2 = 1.0D + 1.0L (live)	
LC #2 = 1.0D + 1.0L  (total)	
LC #2 = 1.0D + 1.0L (bare joist)	
Bearing : Support 1 - LC #2 = 1.25D + 1.5L	
Support 2 - LC $\#2 = 1.25D + 1.5L$	1
Load Types: D=dead W=wind S=snow H=earth, groundwater E=earthqua	
L=live(use,occupancy) Ls=live(storage,equipment) f=fi	re
Load Patterns: s=S/2 L=L+Ls =no pattern load in this span	
All Load Combinations (LCs) are listed in the Analysis output	
CALCULATIONS:	
Deflection: EIcomp = 258e06 lb-in2 K= 4.94e06 lbs	,
"Live" deflection = Deflection from all non-dead loads (live, wind,	snow)

### **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC Part 4) and the CSA O86-09 Engineering Design in Wood standard, which includes Update No.1.

  2. Black weight that the default deflection limits are appreciate for your application.
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWO NO. TAM 44701-17
STRUCTURAL
COMPONENT ONLY



## Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i2748)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:45

Build 4340 Job Name:

Address:
City, Province, Postal Code:INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i2748)

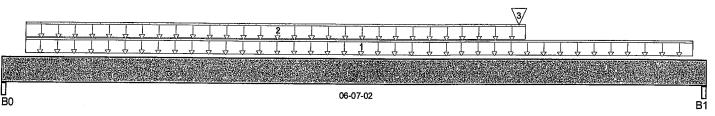
Specifier:

Designer: Company:

Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:22:42 PM kgervais



Total Horizontal Product Length = 06-07-02

Reaction Summary	(Down / Uplift) (lbs)				
Be aring	Live	Dead	Snow	Wind	
B0, 5-1/4"	210/0	109/0			
B1, 4-1/8"	460/0	231/0			

	ead Summary g Description	Load Type	Re	f. Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	06-05-10	11	4			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	04-10-14	15	6			n/a
3	B4 (i2746)	Conc. Pt. (lbs)	L	04-10-00	04-10-00	527	255			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,402 ft-lbs	12,704 ft-lbs	11%	1	04-10-00
End Shear	945 lbs	5,785 lbs	16.3%	1	05-05-08
Total Load Defl.	L/999 (0.021")	n/a	n/a	4	03-07-02
Live Load Defl.	L/999 (0.014")	n/a	n/a	5	03-07-02
Max Defl.	0.021"	n/a	n/a	4	03-07-02
Span / Depth	7.5	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Beam	5-1/4" x 1-3/4"	452 lbs	11.5%	4%	Unspecified
B1	Beam	4-1/8" x 1-3/4"	980 lbs	31.8%	11.1%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

#### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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PROFESSION OF



DWO NO . TAM 44702-17 STRUCTURAL COMPONENT ONLY

CONFORMS TO OBC 2012



## Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i3155)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:45

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i3155)

Specifier:

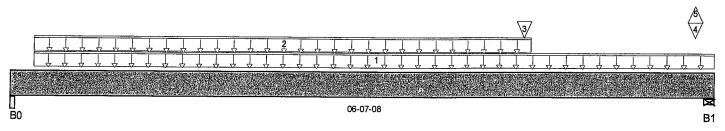
Designer: A

Company:

Misc:

Town of Innisfil Certified Model

03/01/2018 2:22:48 PM kgervais



Total Horizontal Product Length = 06-07-08

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	De ad	Snow	Wind				
B0, 5-1/4"	237/0	119/0				<del></del> -		
B1. 4-3/8"	1.175 / 21	58870						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	Tag Description	Load Type	ad Type Ref. Start En		En d	1.00	.00 0.65	1.00	1.15	
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	06-07-08	20	7			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	04-10-14	14	5			n/a
3	B4 (i2746)	Conc. Pt. (lbs)	L	04-10-00	04-10-00	529	256			n/a
4	2(i 1449)	Conc. Pt. (lbs)	L	06-05-05	06-05-05	685	345			n/a
5	2(i1449)	Conc. Pt. (lbs)	L	06-05-05	06-05-05	-21				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,469 ft-lbs	12,704 ft-lbs	11.6%	1	04-10-00
End Shear	981 lbs	5,785 lbs	16.9%	1	05-05-10
Total Load Defl.	L/999 (0.022")	n/a	n/a	6	03-07-02
Live Load Defl.	L/999 (0.015")	n/a	n/a	8	03-07-02
Max Defl.	0.022"	n/a	n/a	6	03-07-02
Span / Depth	7.5	n/a	n/a		00-00-00

Beari	ing Supports	Dim . (L x W)	Demand	De mand/ Re sistance Support	Demand/ Resistance Member	Material
B0	Beam	5-1/4" x 1-3/4"	505 lbs	12.9%	4.5%	Unspecified
B1	Wall/Plate	4-3/8" x 1-3/4"	2,498 lbs	76.4%	26.7%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012



DWO NO.TAM 44703-17 STRUCTURAL COMPONENT ONLY



## Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i3155)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:45

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i315)

Specifier:

Designer: AJ

Company.

Misc:

#### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO.TAM 44703-17 STRUGTURAL COMPONENT ONLY



## Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i2749)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:46

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i2749)

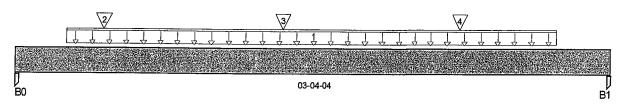
Specifier:

Designer: A. Company.

Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:22:57 PM kgervais



Total Horizontal Product Length = 03-04-04

Reaction Summary	(Down / Uplift) (lbs)					
Be aring	Live	De ad	Snow	Wind		
B0, 3-1/2"	448/0	217/0		1 444	***************************************	<u> </u>
B1, 3-1/2"	412/0	204/0				

Load Summary				Live	Live	Live Dead	Snow Wind	Trib.	
Tag Description	Load Type	F	ef. Start	En d	1.00	0.65	1.00	1.15	
1 User Load	Unf. Lin. (lb/ft)	L	00-03-08	03-00-10	240	120			n/a
2 J9(i1549)	Conc. Pt. (lbs)	L	00-06-00	00-06-00	70	26			n/a
3 J9(i1549)	Conc. Pt. (lbs)	L	01-06-00	01-06-00	70	26			n/a
4 J9(i1484)	Conc. Pt. (lbs)	L	02-06-00	02-06-00	58	22			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	684 ft-lbs	12,704 ft-lbs	5.4%	1	01-07-03
End Shear	834 lbs	5,785 lbs	14.4%	1	02-03-04
Total Load Defl.	L/999 (0.003")	n/a	n/a	4	01-08-05
Live Load Defl.	L/999 (0.002")	n/a	n/a	5	01-08-05
Max Defl.	0.003"	n/a	n/a	4	01-08-05
Span / Depth	3.7	n/a	n/a		00-00-00

Beari	ng Supports	Dim.(L x W)	Demand	De mand/ Re s istance Support	Demand/ Resistance Member	Material
B0	Post	3-1/2" x 1-3/4"	943 lbs	23.7%	12.6%	Unspecified
B1	Post	3-1/2" x 1-3/4"	874 lbs	22%	11.7%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9
Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBG 2012

DWG NO , TAM 44704-17 STRUCTURAL COMPONENT AND V

#### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call 1-800-964-6999 before installation.

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# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i2746)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:46

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i2746)

Specifier:

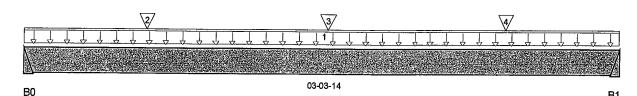
Designer: Company.

Misc:

**Town of Innisfil Certified Model** 

B1

03/01/2018 2:23:01 PM kgervais



Total Horizontal Product Length = 03-03-14

Reaction Summary (Down / Uplift) ( lbs )									
Be aring	Live	De ad	Snow	Wind					
B0	526/0	255/0							
B1	529/0	256/0							

Load Summary			Li	ve Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	End 1.0	0.65	1.00 1.15	
1 User Load	Unf. Lin. (lb/ft)	L 00-00-00	03-03-14 24	10 120	·	n/a
2 J8(i1564)	Conc. Pt. (lbs)	L 00-08-04	00-08-04 83	3 31		n/a
3 J8(i1496)	Conc. Pt. (lbs)	L 01-08-04	01-08-04 94	l 35		n/a
4 J8(i1524)	Conc. Pt. (lbs)	L 02-08-04	02-08-04 81	30		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	859 ft-1bs	12,704 ft-lbs	6.8%	1	01-08-04
End Shear	557 lbs	5,785 lbs	9.6%	1	00-11-08
Total Load Defl.	L/999 (0.004")	n/a	n/a	4	01-07-11
Live Load Defl.	L/999 (0.003")	n/a	n/a	5	01-07-11
Max Defl.	0.004"	n/a	n/a	4	01-07-11
Span / Depth	3.9	n/a	n/a		00-00-00

Beari	ng Supports	Dim. (L x W)	Demand	De mand/ Re sistance Support	Demand/ Resistance Member	Material
B0	Hanger	2" x 1-3/4"	1,108 lbs	n/a	26%	Hanger
B1	Hanger	2" x 1-3/4"	1,113 lbs	n/a	26.1%	Hanger

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

DWG NO . TAM 44705-17 STRUCTURAL COMPONENT ONLY

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B5L(i2989)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:46

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer: Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5L(i2989

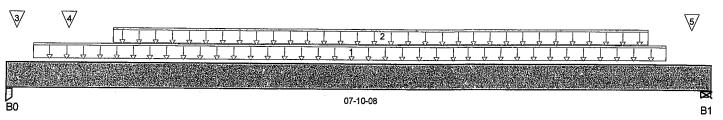
Specifier:

Designer: A Company:

Misc:

Town of Innisfil Certified Model

03/01/2018 2:23:05 PM kgervais



Total Horizontal Product Length = 07-10-08

			•					
Reaction Summary	Reaction Summary (Down / Uplift) (lbs)							
Be aring	Live	Dead	Snow	Wind				
B0, 3-1/2"	1,708 / 0	785/0			W			
B1, 5-1/2"	2.910/0	1.467 / 0						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	Us er Load	Unf. Lin. (lb/ft)	L	00-03-08	07-04-08	240	120			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-02-02	07-02-02	221	83			n/a
- 3	J6(i2745)	Conc. Pt. (lbs)	L	00-01-04	00-01-04	75	28			n/a
4	J6(i2741)	Conc. Pt. (lbs)	L	00-08-02	00-08-02	186	70			n/a
5	12 (i1663)	Conc. Pt. (lbs)	L	07-07-12	07-07-12	1,331	730			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,301 ft-lbs	25,408 ft-lbs	24.8%	1	03-08-02
End Shear	3,245 lbs	11,571 lbs	28%	1	06-07-08
Total Load Defl.	L/999 (0.084")	n/a	n/a	4	03-09-10
Live Load Defl.	L/999 (0.057")	n/a	n/a	5	03-09-10
Max Defl.	0.084"	n/a	n/a	4	03-09-10
Span / Depth	9.2	n/a	n/a		00-00-00

Beari	ng Supports	Dim. (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Post	3-1/2" x 3-1/2"	3,544 lbs	44.5%	23.7%	Unspecified
B1	Wall/Plate	5-1/2" x 3-1/2"	6,198 lbs	75.4%	26.4%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012



DWO NO. TAM 44706.17
STRUCTURAL
COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B5L(i2989)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:46

BC CALC® Design Report

**Build 4340** Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports: CCMC 12472-R File Name: S45-2.mmdl

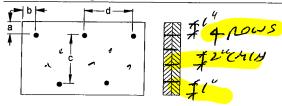
Description: Designs\Flush Beams\Basment\Flush Beams\B5L(i29)

Specifier:

Designer: Company:

Misc:

#### Connection Diagram





Calculated Side Load = 408.2 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d

Λ Nails

3-1/2 in.

ARDOX SPIRAL

#### Disclosure

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COMPONENT ONLY



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6(i3394)

Dry | 2 spans | No cantilevers | 0/12 slope (deg)

September 1, 2017 08:43:24

BC CALC® Design Report Build 5033

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

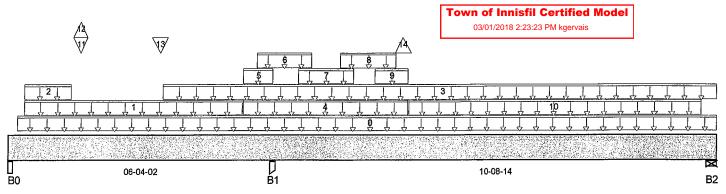
File Name: S45-2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B6(i3394)

Specifier:

Designer: Company:

Misc:



Total Horizontal Product Length = 17-01-00

Reaction Summary (Down / Uplift) (Ibs)								
Be aring	Live	De ad	Snow	Wind				
B0, 5-1/4"	2,707 / 286	1,234 / 0						
B1, 3-1/2"	3,408 / 330	2,402/0						
B2 2-3/8"	346/144	462/0						

١o	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	17-01-00	7	3			n/a
1	11 (i1662)	Unf. Lin. (lb/ft)	L	00-04-10	05-07-06		81			n/a
2	11 (i1662)	Unf. Lin. (lb/ft)	L	00-04-10	01-06-02	395	148			n/a
3	FC2 Floor Material	Unf. Lin. (lb/ft)	L	03-08-06	17-01-00	6				n/a
4	13 (i1665)	Unf. Lin. (lb/ft)	L	05-07-06	09-06-14		61			n/a
5	13 (i1665)	Unf. Lin. (lb/ft)	L	05-07-06	06-04-04	1,316	671			n/a
6	13 (i1665)	Unf. Lin. (lb/ft)	L	05-11-06	07-03-06	125	47			n/a
7	13 (i1665)	Unf. Lin. (lb/ft)	L	06-11-06	08-03-06	125	47			n/a
8	13 (i1665)	Unf. Lin. (lb/ft)	L	07-11-06	09-03-06	113	43			n/a
9	13 (i1665)	Unf. Lin. (lb/ft)	L	08-09-02	09-06-14	651	217			n/a
10	5(i 1481)	Unf. Lin. (lb/ft)	L	09-06-14	16-08-10	43	100			n/a
11	11 (i1662)	Conc. Pt. (lbs)	L	01-08-10	01-08-10	2,688	1,211			n/a
12	11 (ì1662)	Conc. Pt. (lbs)	L	01-08-10	01-08-10	-92		e algebra desail	COLLABORATION CONTRACTOR	n/a
13	B3 (i3438)	Conc. Pt. (lbs)	L	03-07-08	03-07-08	395	194	PROFE	SSIONA	n/a
14	13 (ì1665)	Conc. Pt. (lbs)	L	09-05-02	09-05-02	-297	l	S STATE OF		n/a

DWG NO. YAH 44702.17 STRUCTURAL COMPONENT ONLY

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## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6(i3394)

\*

CCMC 12472-R

Dry | 2 spans | No cantilevers | 0/12 slope (deg)

September 1, 2017 08:43:24

BC CALC® Design Report

Build 5033 Job Name:

Address: City, Province, Postal Code:INNISFILL,

Customer:

Code reports:

File Name: S45-2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B6(i339-

Specifier:

Designer: AJ Company:

Misc:

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,586 ft-lbs	25,408 ft-lbs	25.9%	3	01-08-10
Neg. Moment	-5,690 ft-lbs	-25,408 ft-lbs	22.4%	1	06-04-02
End Shear	4,717 lbs	11,571 lbs	40.8%	3	01-02-12
Cont. Shear	4,678 lbs	11,571 lbs	40.4%	1	05-04-14
Total Load Defl.	L/999 (0.075")	n/a	n/a	13	11-11-00
Live Load Defl.	L/999 (0.043")	n/a	n/a	17	11-07-05
Total Neg. Defl.	L/999 (-0.012")	n/a	n/a	12	08-06-04
Max Defl.	0.075"	n/a	n/a	13	11-11-00
Span / Depth	13.4	n/a	n/a		00-00-00

B0	Beam	5-1/4" x 3-1/2"	5,603 lbs		57.1%	25%	Unspecified
	ng Supports	Dim.(L x W)	De man d		De mand/ Resistance Support	De mand/ Resistance Member	Material
Span/	Depth	13.4		n/a	n/a		00-00-0
MaxDe	eff.	0.075"		n/a	n/a	13	11-11-0
Total N	eg. Defl.	L/999 (-0.012")		n/a	n/a	12	08-06-0
Live Lo	ad Defi.	L/999 (0.043")		n/a	n/a	17	11-07-0

8,114 lbs

1,096 lbs

81.6%

24.7%

Disclosure
Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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trademarks of Boise Cascade Wood
Products L.L.C.

#### Notes

**B1** 

**B2** 

Design meets Code minimum (L/240) Total load deflection criteria.

3-1/2" x 3-1/2"

2-3/8" x 3-1/2"

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO DBC 2012

54.3%

10.8%

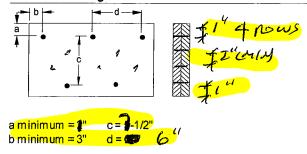
Unspecified

Unspecified

#### Connection Diagram

Post

Wall/Plate



Calculated Side Load = 48.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: Nails

3½" ARDOX SPIRAL



STAUCTURAL COMPONENT ONLY

Page 2 of 2



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7 DR(i3184)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:46

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B7 D

Specifier:

Designer: A. Company.

Misc:

#### **Town of Innisfil Certified Model**

03/01/2018 2:23:49 PM kgervais



B0

11-10-00

B1

Total Horizontal Product Length = 11-10-00

Reaction Summary (Down / Uplift) (Ibs)									
Be aring	Live	De ad	won	Wind					
B0, 4"	2,509 / 118	1,010/0							
B1,4"	1,098 / 90	478/0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-00-00	03-06-04	378	142		· · ·	n/a
2	Bk1(i3119)	Unf. Lin. (lb/ft)	L	04-02-00	05-00-04	114	43			n/a
3	J7(i2792)	Conc. Pt. (lbs)	L	04-00-04	04-00-04	357	134			n/a
4	B12(i3152)	Conc. Pt. (lbs)	L	05-02-00	05-02-00	1,787	692			n/a
5	B12(i3152)	Conc. Pt. (lbs)	L	05-02-00	05-02-00	-208				n/a

CONFORMS TO OBC 2012

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	14,094 ft-lbs	22,080 ft-lbs	63.8%	1	05-02-00
End Shear	4,271 lbs	11,571 lbs	36.9%	1	01-01-08
Total Load Defl.	L/355 (0.382")	0.565"	67.7%	6	05-06-03
Live Load Defl.	L/499 (0.271")	0.376"	72.1%	8	05-06-03
Max Defl.	0.382"	n/a	n/a	6	05-06-03
Span / Depth	14.3	n/a	n/a		00-00-00

Bea	ring Supports	Dim . (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Wall/Plate	4" x 3-1/2"	5,026 lbs	55.3%	29.4%	Unspecified
B1	Wall/Plate	4" x 3-1/2"	2,245 lbs	24.7%	13.1%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced length.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

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DWBRD.TAM 4470317 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7 DR(i3184)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:46

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: \$45-2.mmdl

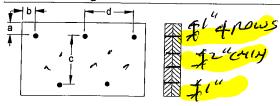
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B7

Specifier:

Designer: AJ Company:

Misc:

#### **Connection Diagram**





Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

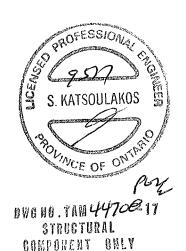
Connectors are: 16d

Nails Mails SPIRAL

#### Disclosure

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## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8 DR(i3188)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

B0

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B8 D

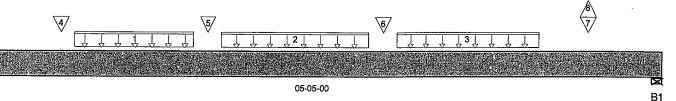
Specifier:

Designer: A. Company:

Msc:

### Town of Innisfil Certified Model

03/01/2018 2:26:05 PM kgervais



Total Horizontal Product Length = 05-05-00

Reaction Summary (Down / Uplift) (Ibs)									
Be aring	Live	De ad	Snow	Wind					
B0, 4"	722/0	299/0							
B1, 4"	1,218 / 13	531/0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	Start End 1.0	1.00	0.65	1.00	1.15	
1	Bk1(i2924)	Unf. Lin. (lb/ft)	L	00-10-12	01-09-12	22				n/a
2	Bk1(i2956)	Unf. Lin. (lb/ft)	L	02-00-04	03-01-12	22	8			n/a
3	Bk1(i2956)	Unf. Lin. (lb/ft)	L	03-04-04	04-05-12	22	8			n/a
4	J2(i2960)	Conc. Pt. (lbs)	L	00-09-08	00-09-08	284	107			n/a
5	J2(i2974)	Conc. Pt. (lbs)	Ł	01-11-00	01-11-00	329	123			n/a
6	J2(i3027)	Conc. Pt. (lbs)	L	03-03-00	03-03-00	357	134			n/a
7	-	Conc. Pt. (lbs)	L	04-10-03	04-10-03	871	376			n/a
8	-	Conc. Pt. (lbs)	L	04-10-03	04-10-03	-13				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,815 ft-lbs	25,408 ft-lbs	7.1%	1	03-03-00
End Shear	1,222 lbs	11,571 lbs	10.6%	1	04-03-08
Total Load Defl.	L/999 (0.011")	n/a	n/a	6	02-08-11
Live Load Defl.	L/999 (0.008")	n/a	n/a	8	02-08-11
Max Defl.	0.011"	n/a	n/a	6	02-08-11
Span / Depth	6.2	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Beari	ng Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	4" x 3-1/2"	1,457 lbs	16%	8.5%	Unspecified
B1	Wall/Plate	4" x 3-1/2"	2,491 lbs	27.4%	14.6%	Unspecified

Notes

S. KATSOULAKOS S

DWG NO. TAN 4470917 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8 DR(i3188)

\*

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

BC CALC® Design Report

Build 4340
Job Name:

File Name: S45-2.mmdl

CONFORMS TO OBC 2012

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B8

Specifier:

Designer: A Company.

Misc:

City, Province, Postal Code: INNISFILL,

Customer:

Address:

Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced length.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

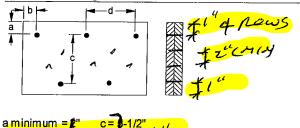
Deflections less than 1/8" were ignored in the results.

#### Disclosure

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Connection Diagram



Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Member has no side loads.

Connectors are: 16d

b minimum = 3"

Od Nails

3½" ARDOX SPIRAL

S. KATSOULAKOS FROM NICE OF ONTHER PARTITION OF THE PARTI

DVB NO. TAM 4470917 STRUCTURAL CAMPONENT ONLY



## Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B9 DR(i3171)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: \$45-2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B9 D

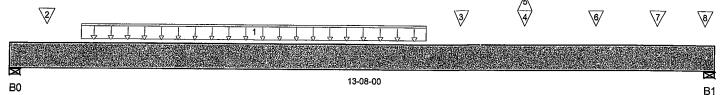
Specifier:

Designer: A. Company:

Misc:

### **Town of Innisfil Certified Model**

03/01/2018 2:26:26 PM kgervais



Total Horizontal Product Length = 13-08-00

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	Dead	Snow	Wind				
B0, 4"	2,423 / 1	1,012/0						
B1, 4"	2,602 / 1	1,089 / 0						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	Smoothed Load	Unf. Lin. (lb/ft)	L	01-04-06	08-00-06	358	134			n/a
2	-	Conc. Pt. (lbs)	L	00-08-06	00-08-06	437	164			n/a
3	-	Conc. Pt. (lbs)	L	08-08-06	08-08-06	466	175			n/a
4	-	Conc. Pt. (lbs)	L	09-11-11	09-11-11	569	234			n/a
5	-	Conc. Pt. (lbs)	L	09-11-11	09-11-11	-2				n/a
6	-	Conc. Pt. (lbs)	L	11-04-06	11-04-06	477	179			n/a
7	-	Conc. Pt. (lbs)	L	12-06-11	12-06-11	435	163			n/a
8	J2(i3058)	Conc. Pt. (lbs)	L	13-05-10	13-05-10	251	94			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	15,992 ft-lbs	39,636 ft-lbs	40.3%	1	07-04-06
End Shear	4,683 lbs	17,356 lbs	27%	1	12-06-08
Total Load Defl.	L/340 (0.464")	0.656"	70.6%	6	06-10-06
Live Load Defl.	L/482 (0.327")	0.438"	74.7%	8	06-10-06
Max Defi.	0.464"	n/a	n/a	6	06-10-06
Span / Depth	16.6	n/a	n/a		00-00-00

Bear	ring Supports	Dim. (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Wall/Plate	4" x 5-1/4"	4,900 lbs	35.9%	19.1%	Unspecified
B1	Wall/Plate	4" x 5-1/4"	5,264 lbs	38.6%	20.5%	Unspecified

Notes



DWO NO. TAM 4471217 STRUCTURAL COMPONENT ONLY



### Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B9 DR(i3171)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL.

File Name: S45-2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B\$

Specifier:

Designer: Company:

Customer: Code reports:

CCMC 12472-R

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced lenath.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

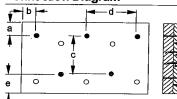
Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

#### Connection Diagram



4 nows

a minimum = 2"

b minimum = 3"d = 20 e minimum = 2"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Member has no side loads.

Connectors are: 16d

Nails 📑

ARDOX SPIRAL

#### Disclosure

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DWO NO . TAN 44710-17 STRUCTURAL COMPONENT ONLY



## Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10 DR(i3055)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer: Code reports:

e reports: CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B10

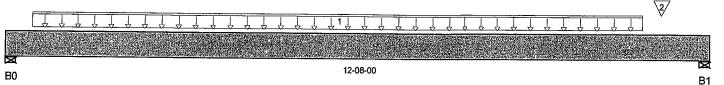
Specifier:

Designer: Company:

Misc:

Town of Innisfil Certified Model

03/01/2018 2:26:49 PM kgervais



Total Horizontal Product Length = 12-08-00

Reaction Summary (Down / Uplift) (Ibs)									
Be aring `	Live	De ad	Snow	Wind					
B0, 4"	2,259 / 0	937/0							
B1, 4"	2,279/0	945/0							

	ad Summary	1		<b>.</b> .		Live	Dead		Wind	Trib.
ıa	g Description	Load Type	Re f.	Start	En d	1.00	0.65	1.00	1.15	
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-05-10	11-05-10	375	141	,		n/a
2	-	Conc. Pt. (lbs)	L	11-09-07	11-09-07	409	153			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	14,101 ft-lbs	39,636 ft-lbs	35.6%	1	06-02-06
End Shear	4,331 lbs	17,356 lbs	25%	1	01-01-08
Total Load Defl.	L/418 (0.348")	0.606"	57.4%	4	06-04-11
Live Load Defl.	L/591 (0.246")	0.404"	61%	5	06-04-11
Max Defl.	0.348"	n/a	n/a	4	06-04-11
Span / Depth	15.3	n/a	n/a		00-00-00

Bearing Supports		Dim.(LxW)	Demand	De mand/ Resistance Support	De mand/ Resistance Member	Material	
B0	Wall/Plate	4" x 5-1/4"	4,559 lbs	33.4%	17.8%	Unspecified	
B1	Wall/Plate	4" x 5-1/4"	4,600 lbs	33.7%	18%	Unspecified	

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced length .

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO UBC 2012



DWO NO. TAM 44711-17 STRUCTURAL COMPONENT ONLY



## Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10 DR(i3055)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer: Code reports:

CCMC 12472-R

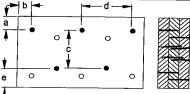
File Name: \$45-2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1

Specifier: Designer: AJ

Company: Misc:

#### **Connection Diagram**



e minimum = 2"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

4 pours

Member has no side loads.

Connectors are: 16d

Nails '

312" ARDOX SPIRAL

#### Disclosure

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SEROFESSION OF STANDARDS TO STRUCTURAL COMPONENT ONLY



## Triple 1-3/4" x 7-1/4" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B18L DR(i3007)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

В1

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

B0

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\818I

Specifier:

Designer: A. Company:

Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:26:58 PM kgervais

<u>√</u> <u>2</u>/ <u>3</u>3 <u>√</u>5/ 04-02-00

Total Horizontal Product Length = 04-02-00

Reaction Summary (Down / Uplift) ( lbs )									
Be aring	Live	De ad	Snow	Wind					
B0, 4"	1,295 / 224	499/0							
B1, 4"	1,645 / 357	610/0							

Lo	oad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	J4(i3043)	Conc. Pt. (lbs)	L	00-08-04	00-08-04	410	163			n/a
2	J4(i3016)	Conc. Pt. (lbs)	L	01-08-04	01-08-04	309	121			n/a
3	B17L(i3044)	Conc. Pt. (lbs)	L	02-06-00	02-06-00	1,880	651			n/a
4	B17L(i3044)	Conc. Pt. (lbs)	L	02-06-00	02-06-00	-581				n/a
5	J5(i1708)	Conc. Pt. (lbs)	L	03-08-04	03-08-04	341	128			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	3,693 ft-lbs	23,788 ft-lbs	15.5%	1	02-06-00
Neg. Moment	-132 ft-lbs	-23,788 ft-lbs	0.6%	4	02-06-00
Neg. Moment	-132 ft-lbs	-23,788 ft-lbs	0.6%	4	02-06-00
End Shear	2,707 lbs	13,246 lbs	20.4%	1	03-02-12
Total Load Defl.	L/999 (0.016")	n/a	n/a	6	02-01-12
Live Load Defl.	L/999 (0.011")	n/a	n/a	8	02-01-12
Max Defl.	0.016"	n/a	n/a	6	02-01-12
Span / Depth	6	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Beari	ng Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	4" x 5-1/4"	2,567 lbs	18.8%	10%	Unspecified
B1	Wall/Plate	4" x 5-1/4"	3,229 lbs	23.7%	12.6%	Unspecified

Notes



DVB NO. TAM 447/2-17
SYRUCTURAL
COMPONENT ONLY



## Triple 1-3/4" x 7-1/4" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B18L DR(i3007)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1

Specifier:

Designer: Company:

CONFORMS TO OBC 2012

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

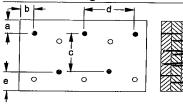
Deflections less than 1/8" were ignored in the results.

## Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALO®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®. BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

**Connection Diagram** 



4 rows

a minimum = 12" c = 24/4" b minimum = 3"

e minimum = 2"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Member has no side loads.

Connectors are: 16d

^ Nails

3%" ARDOX SPIRAL

PROFESSION VILLE OVINCE OF ONTHE DYD NO. TAM 44712 STRUCTURAL COMPONENT ONLY



# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B11(i3181)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:47

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i3181'

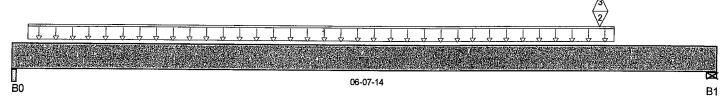
Specifier:

Designer: A Company:

Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:27:04 PM kgervais



Total Horizontal Product Length = 06-07-14

Reaction Summary (Down / Uplift) (lbs)								
Be aring	Live	De ad	Snow	Wind				
B0, 4-3/8"	151/3	78 / 0						
B1, 5-1/2"	573/20	271/0						

	ad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
1	FC5 Floor Material	Unf. Lin. (lb/ft)	L	00-01-12	05-08-06	27	10			n/a
2	B15(i3132)	Conc. Pt. (lbs)	L	05-06-10	05-06-10	572	260	•		n/a
3	B15(i3132)	Conc. Pt. (lbs)	L	05-06-10	05-06-10	-23				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	842 ft-lbs	12,704 ft-lbs	6.6%	1	05-06-10
End Shear	971 lbs	5,785 lbs	16.8%	1	05-04-14
Total Load Defl.	L/999 (0.014")	n/a	n/a	6	03-06-09
Live Load Defl.	L/999 (0.009")	n/a	n/a	8	03-06-09
Max Defl.	0.014"	n/a	n/a	6	03-06-09
Span / Depth	7.5	n/a	n/a		00-00-00

Beari	ng Supports	Dim.(L x W)	Demand	De mand/ Resistance Support	Demand/ Resistance Member	Material
B0	Beam	4-3/8" x 1-3/4"	324 lbs	4.9%	3.5%	Unspecified
B1 -	Wall/Plate	5-1/2" x 1-3/4"	1,1991bs	29.2%	10.2%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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OVINCE OF ON



CONFORMS TO DBC 2012



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12(i3152)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:48

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i3152)

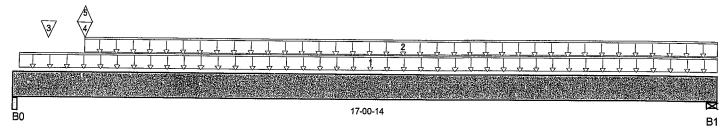
Specifier:

Designer: A Company:

Misc:

#### **Town of Innisfil Certified Model**

03/01/2018 2:27:10 PM kgervais



#### Total Horizontal Product Length = 17-00-14

Reaction Summary (Down / Uplift) (Ibs)									
Be aring	Live	De ad	Snow	Wind					
B0, 3-1/2"	1,789 / 209	693/0							
B1, 4-3/8"	<del>4</del> 77/20	256/0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Та	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	FC5 Floor Material	Unf. Lin. (lb/ft)	L	00-01-12	17-00-14	23	9			n/a
2	FC5 Floor Material	Unf. Lin. (lb/ft)	L	01-08-08	17-00-14	17	6			n/a
3	J2(i3157)	Conc. Pt. (lbs)	L	00-10-00	00-10-00	178	66			n/a
4	B14(i3185)	Conc. Pt. (lbs)	L	01-08-08	01-08-08	1.437	474			n/a
5	B14(i3185)	Conc. Pt. (lbs)	L	01-08-08	01-08-08	-227	., .			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5,587 ft-lbs	25,408 ft-lbs	22%	1	05-08-07
End Shear	3,383 lbs	11,571 lbs	29.2%	1	01-01-00
Total Load Defl.	L/506 (0.392")	0.827"	47.4%	6	07-11-02
Live Load Defl.	L/747 (0.266")	0.551"	48.2%	8	07-11-02
Max Defl.	0.392"	n/a	n/a	6	07-11-02
Span / Depth	20.9	n/a	n/a		00-00-00

Bearing Supports		Dim.(L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Beam	3-1/2" x 3-1/2"	3,549 lbs	33.2%	23.7%	Unspecified
B1	Wall/Plate	4-3/8" x 3-1/2"	1,035 lbs	15.8%	5.5%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012



DWG NO.TAM 4471417 STRUGTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12(i3152)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:48

BC CALC® Design Report

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

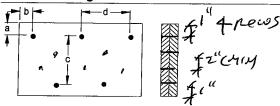
Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i31!

Specifier:

Designer: AJ Company.

Misc:

Connection Diagram



a minimum = 2" b minimum = 3"

Calculated Side Load = 166.1 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

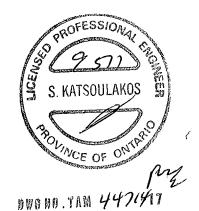
Connectors are:

Nails · `ARDOX SPIRAL

#### Disclosure

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STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i3185)

BC CALC® Design Report



Dry | 2 spans | Right cantilever | 0/12 slope (deg)

September 20, 2016 10:40:48

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i3185)

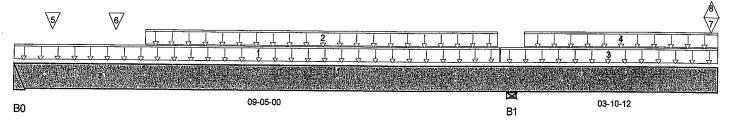
Specifier:

Designer: AJ Company:

Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:27:16 PM kgervais



Total Horizontal Product Length = 13-03-12

	(Down / Uplift) (lbs)				
Bearing	Live	De ad	Snow	Wind	
B0	1,477 / 240	473/0			
B1, 5-1/2"	2.683 / 91	1210/0			

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	FC5 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-02-04	18	7			n/a
2	Smoothed Load	Unf. Lin. (ib/ft)	L	02-05-12	09-01-12	302	113			n/a
3	FC5 Floor Material	Unf. Lin. (lb/ft)	L	09-02-04	13-03-12	20	8			n/a
4	Us er Load	Unf. Lin. (lb/ft)	L	09-07-12	13-03-12	240	120			n/a
5	J1(i2872)	Conc. Pt. (lbs)	L	80-80-00	00-08-08	314	118			n/a
6	J1(i3130)	Conc. Pt. (lbs)	L	01-11-00	01-11-00	371	139			n/a
7	B22(i3143)	Conc. Pt. (lbs)	L	13-02-00	13-02-00	70				n/a
8	B22(i3143)	Conc. Pt. (lbs)	L	13-02-00	13-02-00	-65				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,584 ft-lbs	25,408 ft-lbs	25.9%	3	04-05-12
Neg. Moment	-4,703 ft-lbs	-25,408 ft-lbs	18.5%	1	09-05-00
Neg. Moment	-4,703 ft-lbs	-25,408 ft-lbs	18.5%	1	09-05-00
End Shear	2,566 lbs	11,571 lbs	22.2%	3	00-11-08
Cont. Shear	3,213 lbs	11,571 lbs	27.8%	1	08-04-12
Total Load Defl.	L/815 (0.137")	0.466"	29.4%	12	04-07-12
Live Load Defl.	2xL/584 (-0.16")	-0.26"	61.7%	16	13-03-12
Total Neg. Defl.	2xL/587 (-0.159	") -0.39"	40.9%	12	13-03-12
Max Defl.	0.137"	n/a	n/a	12	04-07-12
Span / Depth	11.8	n/a	n/a		00-00-00

Bea	ring Supports	Dim . (L x W)	Demand	De mand/ Re sistance Support	Demand/ Resistance Member	Material
B0	Hanger	2" x 3-1/2"	2,807 lbs	n/a	32.9%	Hanger
B1	Wall/Plate	5-1/2" x 3-1/2"	5,538 lbs	67.3%	23.6%	Unspecified

Notes

S. KATSOULAKOS S.

OWEND. TAM 447/5-17
STRUCTURAL
OURSPEKENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i3185)

\*

Dry | 2 spans | Right cantilever | 0/12 slope (deg)

September 20, 2016 10:40:48

BC CALC® Design Report

Build 4340 Job Name:

Address: City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

e:INNISFILL, CCMC 12472-R File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i318

Specifier:

Designer: AJ Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets User specified (2xL/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

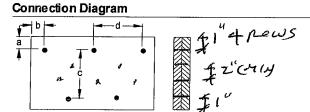
Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9
Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBG 2012



Calculated Side Load = 409.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d

Nails . . .

3-1/2 i

312" ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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COMPONENT ONLY



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B15(i3132)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:48

Build 4340 Job Name:

Address:
City, Province, Postal Code: INNISFILL,

Customer:

Code reports: CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B15(i3132)

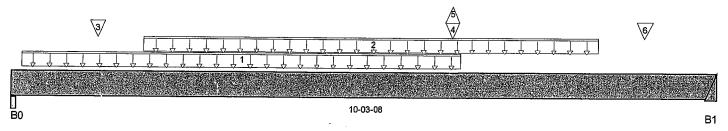
Specifier:

Designer: A Company:

Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:27:24 PM kgervais



Total Horizontal Product Length = 10-03-08

Reaction Summary	(Down / Uplift) (lbs)				
Bearing	Live	De ad	Snow	Wind	
B0, 3-1/2"	574/14	264/0	**-		 
B1	575/22	262 / 0			

Lo	ad Summary				Live	Dead	Snow	Wind	Trib.	
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	FC5 Floor Material	Unf. Lin. (lb/ft)	L	00-01-12	06-06-08	16	6			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-11-00	08-07-00	110	42			n/a
3	J6(i3047)	Conc. Pt. (lbs)	L	01-03-00	01-03-00	124	46			n/a
4	B22(i3143)	Conc. Pt. (lbs)	L	06-04-12	06-04-12	52	16			n/a
5	B22(i3143)	Conc. Pt. (lbs)	L	06-04-12	06-04-12	-36				n/a
6	J6(i3031)	Conc. Pt. (lbs)	L	09-03-00	09-03-00	140	52			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,300 ft-lbs	25,408 ft-lbs	13%	1	05-03-00
End Shear	1,179 lbs	11,571 lbs	10.2%	1	09-04-00
Total Load Defl.	L/999 (0.082")	n/a	n/a	6	05-03-00
Live Load Defl.	L/999 (0.056")	n/a	n/a	8	05-03-00
Max Defl.	0.082"	n/a	n/a	6	05-03-00
Span / Depth	12.6	n/a	n/a		00-00-00

Bearii	ng Supports	Dim.(L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Beam	3-1/2" x 3-1/2"	1,191 lbs	11.1%	8%	Unspecified
B1	Hanger	2" x 3-1/2"	1,191 lbs	n/a	13.9%	Hanger

Notes



STRUCTURAL COMPOSENT BNLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B15(i3132)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:48

BC CALC® Design Report

File Name: S45-2.mmdl Description: Designs\Flush Beams\1st Floor\Flush Beams\B15(i31;

Specifier:

Misc:

Designer: AJ Company.

City, Province, Postal Code: INNISFILL, Customer:

Code reports:

**Build 4340** 

Job Name: Address:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

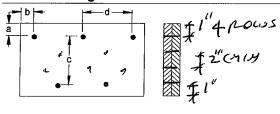
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012 Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

#### **Connection Diagram**



c = 3 - 1/2" a minimum = 🛊 " b minimum = 3"

Calculated Side Load = 195.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nails

Connectors are:

ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B16L(i2656)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:48

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B16L(i265)

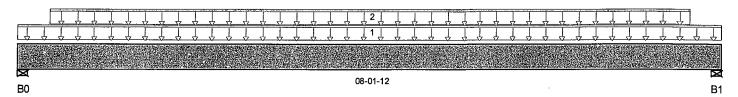
Specifier:

Designer: Company.

Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:27:33 PM kgervais



Total Horizontal Product Length = 08-01-12

Reaction Summary (Down / Uplift) ( lbs )									
Be aring	Live	De ad	Snow	Wind					
B0, 4-3/8"	971/0	495/0							
B1.4-3/8"	972/0	495/0							

	ad Summary Description	Load Type	Rei	f. Start	En d	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	FC4 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-01-12	20	7			n/a
2	UserLoad	Unf. Lin. (lb/ft)	L	00-04-06	07-09-06	240	120			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	nand Resistance		Case	
Pos. Moment	3,948 ft-lbs	12,704 ft-lbs	31.1%	1	04-00-14
End Shear	1,619 lbs	5,785 lbs	28%	1	01-01-14
Total Load Defl.	L/999 (0.114")	n/a	n/a	4	04-00-14
Live Load Defl.	L/999 (0.076")	n/a	n/a	5	04-00-14
Max Defl.	0.114"	n/a	n/a	4	04-00-14
Span / Depth	9.5	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Bear	ing Supports	Dim. (L x W)	Demand	Support	Member	Material
B0	Wall/Plate	4-3/8" x 1-3/4"	2,076 lbs	63.5%	22.2%	Unspecified
B1	Wall/Plate	4-3/8" x 1-3/4"	2,077 lbs	63.5%	22.2%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012

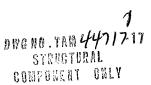
#### Disclosure

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PROFESSION

POVINCE OF





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B17L(i3044)

BC CALC® Design Report



Dry | 2 spans | Right cantilever | 0/12 slope (deg)

September 20, 2016 10:40:48

Build 4340

File Name: S45-2.mmdl Description: Designs\Flush Beams\1st Floor\Flush Beams\B17L(i304,

Job Name: Address:

Specifier:

City, Province, Postal Code: INNISFILL,

Designer: Company.

Customer:

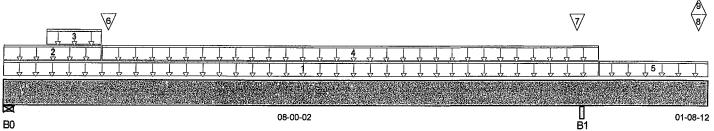
Misc:

Code reports:

CCMC 12472-R

**Town of Innisfil Certified Model** 

03/01/2018 2:27:49 PM kgervais



Total Horizontal Product Length = 09-08-14

Reaction Summary (Down / Uplift) (lbs)								
Be aring	Live	De ad	Snow	Wind	i			
B0, 4-3/8"	622/274	179/0					- 107	
B1, 5-1/4"	1,863 / 573	647/0						
				Livo	Dood	Snow Wind	Trib	

Lo	ad Summary				Live		Dead	Snow Wind		Trib.
	g Description	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
1	FC4 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-02-14	16	6			n/a
2	16 (i1675)	Unf. Lin. (lb/ft)	L	-00-00-00	01-04-06		10			n/a
3	16 (i1675)	Unf. Lin. (lb/ft)	L	00-07-02	01-04-06	109	42			n/a
4	FC4 Floor Material	Unf. Lin. (lb/ft)	L	01-04-06	08-02-14	24	9			n/a
5	FC4 Floor Material	Unf. Lin. (lb/ft)	L	08-02-14	09-08-14	27	10			n/a
6	B21L(i1671)	Conc. Pt. (lbs)	L	01-05-04	01-05-04	344	146			n/a
7	19(i2108)	Conc. Pt. (lbs)	L	07-11-02	07-11-02	23				n/a
8	-	Conc. Pt. (lbs)	L	09-07-02	09-07-02	1,306	400			n/a
9	-	Conc. Pt. (lbs)	L	09-07-02	09-07-02	<del>-4</del> 75				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,307 ft-lbs	25,408 ft-lbs	5.1%	3	03-04-03
Neg. Moment	-3,994 ft-lbs	-25,408 ft-lbs	15.7%	1	08-00-02
Neg. Moment	-3,994 ft-lbs	-25,408 ft-lbs	15.7%	1	08-00-02
End Shear	1,069 lbs	11,571 lbs	9.2%	3	01-01-14
Cont. Shear	2,547 lbs	11,571 lbs	22%	1	09-00-04
Uplift	277 lbs	n/a	n/a	6	08-00-02
Uplift	277 lbs	n/a	n/a	6	08-00-02
Total Load Defl.	2xL/1,998 (0.04	7") n/a	n/a	13	09-08-14
Live Load Defl.	2xL/1,998 (0.03	9") n/a	n/a	17	09-08-14
Total Neg. Defl.	L/999 (-0.031")	n/a	n/a	13	04-10-15
Max Defl.	-0.031"	n/a	n/a	13	04-10-15
Span / Depth	9.7	n/a	n/a		00-00-00

Demand

Dim. (LxW)

Support	Member	Material	
Resistance	Resistance		
De m an d/	De mand/		

DWO NO. TAM 447/817 STRUCTURAL COMPONENT ONLY

Page 1 of 2

**Bearing Supports** 

TO VINCE OF ON THE



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B17L(i3044)

BC CALC® Design Report



Dry | 2 spans | Right cantilever | 0/12 slope (deg)

September 20, 2016 10:40:48

Build 4340

Job Name:

Address: City, Province, Postal Code:INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B17L(i3(

Specifier:

Designer: AJ

Company:

Misc:

B0 Wall/Plate 4-3/8" x 3-1/2" 1,156 lbs 17.7% 6.29	% Unspecified
B1 Beam 5-1/4" x 3-1/2" 3,603 lbs 22.5% 16.19	% Unspecified

### Cautions

Uplift of 277 lbs found at span 1 - Right. Uplift of 277 lbs found at span 2 - Left. - (SIMPSON 1-H2-5A-@0.31)

### Notes

Design meets User specified (2xL/240) Total load deflection criteria.

Design meets User specified (2xL/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

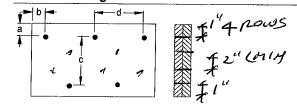
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9
Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012

Connection Diagram



Calculated Side Load = 230.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

Nails
3½" ARDOX SPIRAL

Disclosure

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DWG NO.TAM 4471817 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B19L(i3114)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:49

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B19L(i311.

Specifier:

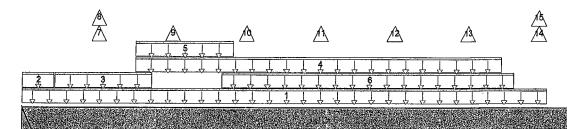
Designer: AJ Company:

Misc:

**Town of Innisfil Certified Model** 

В1

03/01/2018 2:27:59 PM kgervais



B0

09-07-04

Total Horizontal Product Length = 09-07-04

Reaction Summary (Down / Uplift) (Ibs)										
Bearing	Live	De ad	Snow	Wind						
B0	1,173 / 481	347/0								
B1. 5-1/2"	619/344	178/0								

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	
1	17(i1699)	Unf. Lin. (lb/ft)	L	-00-00-00	07-02-00		10			n/a
2	17 (i1699)	Unf. Lin. (lb/ft)	L	-00-00-00	00-05-04	458	171			n/a
3	17 (i1699)	Unf. Lin. (lb/ft)	L	00-05-04	01-09-04	193	72			n/a
4	Smoothed Load	Unf. Lin. (lb/ft)	L	01-06-08	06-06-08	39				n/a
5	17 (i1699)	Unf. Lin. (lb/ft)	L	01-06-08	02-10-08	166	62			n/a
6	17 (i1699)	Unf. Lin. (lb/ft)	L	02-08-08	06-08-08	211	79			n/a
7	J5(i1708)	Conc. Pt. (lbs)	L	01-00-08	01-00-08	43	-34			n/a
8	J5(i1708)	Conc. Pt. (lbs)	L	01-00-08	01-00-08	-132				n/a
9	J5(i1703)	Conc. Pt. (lbs)	L	02-00-08	02-00-08	-120	-30			n/a
10	J5(i1703)	Conc. Pt. (lbs)	L	03-00-08	03-00-08	-120	-30			n/a
11	J5(i1703)	Conc. Pt. (lbs)	L	04-00-08	04-00-08	-120	-30			n/a
12	J5(i1703)	Conc. Pt. (lbs)	L	05-00-08	05-00-08	-120	-30			n/a
13	J5(i1703)	Conc. Pt. (lbs)	L	06-00-08	06-00-08	-120	-30			n/a
14	J5(i1705)	Conc. Pt. (lbs)	L	07-00-08	07-00-08	24	-25			n/a
15	J5(i1705)	Conc. Pt. (lbs)	L	07-00-08	07-00-08	-93				n/a



DWG WO. TAM 4471717 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B19L(i3114)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:49

Build 4340

Job Name:

Address: City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B19L(i3\*

Specifier:

Designer: AJ Company:

Misc:

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	4,144 ft-lbs	25,408 ft-lbs	16.3%	1	04-03-08
Neg. Moment	-1,162 ft-lbs	-25,408 ft-lbs	4.6%	4	04-00-08
Neg. Moment	-1,162 ft-lbs	-25,408 ft-lbs	4.6%	4	04-00-08
End Shear	2,018 lbs	11,571 lbs	17.4%	1	00-11-08
Uplift	409 lbs	n/a	n/a	4	00-00-00
Total Load Defl.	L/999 (0.083")	n/a	n/a	6	04-06-08
Live Load Defl.	L/999 (0.065")	n/a	n/a	8	04-06-08
Total Neg. Defl.	L/999 (-0.016")	n/a	n/a	7	04-08-00
MaxDefl.	0.083"	n/a	n/a	6	04-06-08
Span / Depth	11.5	n/a	n/a		00-00-00

Disclosure Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

Bear	ing Supports	Dim . (L x W)	De man d	De mand/ Re sistance Su pport	De mand/ Resistance Member	Material
B0	Hanger	2" x 3-1/2"	2,193 lbs	n/a	25.7%	Hanger
B0 B1	Hanger Uplift Wall/Plate	2" x3-1/2" 5-1/2" x 3-1/2"	410 lbs 1,151 lbs	n/a 9.2%	0.04 4.9%	Hanger Unspecified

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### Ca utions

Uplift of 409 lbs found at span 1 - Left.

(5149500 WC410@0-BD)

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced

Resistance Factor phi has been applied to all presented results per CSA 086.

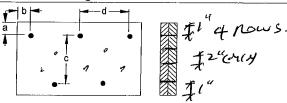
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9 Deflections less than 1/8" were ignored in the results.

Connection Diagram



a minimum = 2" b minimum = 3" d= 3 6

Calculated Side Load = 118.4 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

Nails ARDOX SPIRAL

S. KATSOULAKOS BOLINCE OF OMIGE

STRUCTURAL COMPORENT ONLY

DWO NO. YAM 4471917

Page 2 of 2

Town of Innisfil Certified Model 03/01/2018 2:28:03 PM kgervais



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B21L(i1671)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:49

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B21L(i167)

Specifier:

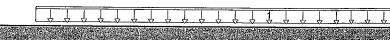
Designer: A. Company:

Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:28:08 PM kgervais

2/



3

В0

07-07-04

В1

Total Horizontal Product Length = 07-07-04

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0	350/1	149/0		· · · · · · · · · · · · · · · · · · ·					
B1, 5-1/2"	400/1	170/0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
1	Smoothed Load	Unf. Lin. (lb/ft)	L	01-06-08	06-06-08	109	41			n/a
2	J6(i1708)	Conc. Pt. (lbs)	L	01-00-08	01-00-08	121	45			n/a
3	J6(i1705)	Conc. Pt. (lbs)	L	07-00-08	07-00-08	84	32			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,392 ft-lbs	12,704 ft-lbs	11%	1	04-00-08
End Shear	705 lbs	5,785 lbs	12.2%	1	00-11-08
Total Load Defl.	L/999 (0.035")	n/a	n/a	6	03-08-00
Live Load Defl.	L/999 (0.025")	n/a	n/a	8	03-08-00
Max Defl.	0.035"	n/a	n/a	6	03-08-00
Span / Depth	9	n/a	n/a		00-00-00

Bearir	ng Supports	Dim. (L x W)	Demand	De mand/ Re sistance Support	De mand/ Resistance Member	Material
B0	Hanger	2" x 1-3/4"	711 lbs	n/a	16.6%	Hanger
B1	Wall/Plate	5-1/2" x 1-3/4"	812 lbs	19.8%	6.9%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012

DWG NG. TAM 4472017 STREGTURAL COMPONENT ONLY

### Disclosure

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### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B22(i3143)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:49

Build 4340

Job Name: Address:

City, Province, Postal Code:INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B22(i3143)

Specifier:

Designer: A Company:

Msc:

**Town of Innisfil Certified Model** 

03/01/2018 2:28:17 PM kgervais



Total Horizontal Product Length = 02-00-12

Reaction Summary (Down / Uplift) (Ibs)									
Bearing	Live	De ad	Snow	Wind					
B0	51 / 35	17 / 0							
B1	71 / 66	12 / 0							

Load Summary			Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	oad Type Ref. Start End 1.		0.65	1.00 1.15	
1 J5(i3196)	Conc. Pt. (lbs)	L 00-07-12	2 00-07-12 28	10		n/a
2 J5(i3196)	Conc. Pt. (lbs)	L 00-07-12	2 00-07-12 -2			n/a
3 J2(i3157)	Conc. Pt. (lbs)	L 01-04-00	01-04-00 94	-1		n/a
4 J2(i3157)	Conc. Pt. (lbs)	L 01-04-00	01-04-00 -99			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	72 ft-lbs	25,408 ft-lbs	0.3%	1	01-04-00
Neg. Moment	-57 ft-lbs	-25,408 ft-lbs	0.2%	4	01-04-00
Neg. Moment	-57 ft-lbs	-25,408 ft-lbs	0.2%	4	01-04-00
End Shear	83 lbs	11,571 lbs	0.7%	4	01-01-04
Uplift	88 lbs	n/a	n/a	4	02-00-12
Total Load Defl.	L/999 (0")	n/a	n/a	6	01-00-13
Live Load Defl.	L/999 (0")	n/a	n/a	8	01-01-00
Max Defl.	0"	n/a	n/a	6	01-00-13
Span / Depth	2.3	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Hanger	2" x 3-1/2"	98 lbs	n/a	1.1%	Hanger
B0	Hanger Uplift	2" x 3-1/2"	37 lbs	n/a	0.00	Hanger
B1	Hanger	2" x 3-1/2"	121 lbs	n/a	1.4%	Hanger
B1	Hanger Uplift	2" x 3-1/2"	88 lbs	n/a	0.01	Hanger

Cautions

Uplift of 88 lbs found at span 1 - Right. (SIMSON 1-146US41200. B)

Notes





### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B22(i3143)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 10:40:49

BC CALC® Design Report

**Build 4340** Job Name:

Address: City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\822(i314

Specifier:

Designer: AJ

Company.

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume Member is Fully Braced.

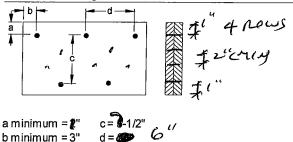
Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9 Deflections less than 1/8" were ignored in the results. CONFORMS TO OBC 2012

Connection Diagram



Calculated Side Load = 21.4 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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STRUCTURAL COMPOSED ONLY



### Boiso Coscodo Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B25(i3739)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 11:08:36

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2 EL-B.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B25(i3739)

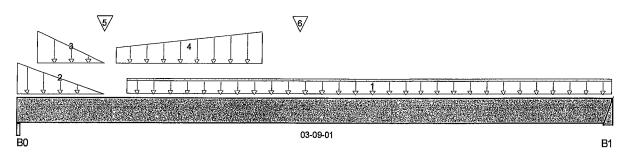
Specifier:

Designer: Company.

Misc:

Town of Innisfil Certified Model

03/01/2018 2:30:59 PM kgervais



Total Horizontal Product Length = 03-09-01

Reaction Summary (Do	wn / Uplift) (lbs)				
Bearing	Live	De ad	Snow	Wind	
B0, 7-7/16"	166/0	106/0	221/0		<u> </u>
B1	111/0	89 / 0	257/0		

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Ref. Start		En d	1.00	0.65	1.00	1.15	
1	Us er Load	Unf. Lin. (lb/ft)	L	00-08-05	03-09-01	44	40	156		n/a
2	FC5 Floor Material	Trapezoidal (lb/ft)	L	00-00-00		22	8			n/a
					00-06-09	0	0			n/a
3	FC5 Floor Material	Trapezoidal (lb/ft)	. L	00-01-10		17				n/a
					00-06-09	0				n/a
4	FC5 Floor Material	Trapezoidal (lb/ft)	L	00-07-07		10	4			n/a
					01-06-06	19	7			n/a
5	FC5 Floor Material	Conc. Pt. (lbs)	L	00-06-09	00-06-09	24	9			n/a
6	J5(i3742)	Conc. Pt. (lbs)	L	01-09-03	01-09-03	92	35			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	433 ft-1bs	12,704 ft-lbs	3.4%	13	01-11-15
End Shear	503 lbs	5,785 lbs	8.7%	13	01-04-15
Total Load Defl.	L/999 (0.002")	n/a	n/a	45	02-01-01
Live Load Defl.	L/999 (0.002")	n/a	n/a	61	02-01-01
Max Defl.	0.002"	n/a	n/a	45	02-01-01
Span / Depth	3.9	n/a	n/a		00-00-00

				De mand/	Demand/		
Bearing Supports				Resistance	Resistance		
		Dim . (L x W)	Demand	Support	Member	Material	
B0	Beam	7-7/16" x 1-3/4"	547 lbs	7.9%	3.4%	Unspecified	
B1	Hanger	2" x 1-3/4"	552 lbs	n/a	12.9%	LSSUI25	



Page 1 of 2



STRUCTURAL **COMPONENT CHLY** 



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B25(i3739)

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 11:08:36

Build 4340

Job Name:

Address: City, Province, Postal Code:INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2 EL-B.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B25(i37;

Specifier:

Designer: AJ

CONFORMS TO OBC 2012

Company.

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced

length.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086. Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B24(i3738)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 11:08:37

Build 4340

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2 EL-B.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\824(i3738)

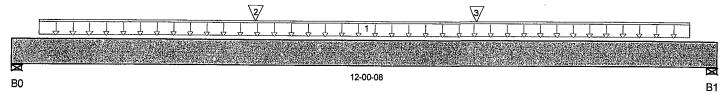
Specifier:

Designer: AJ

Company: Misc:

**Town of Innisfil Certified Model** 

03/01/2018 2:31:13 PM kgervais



Total Horizontal Product Length = 12-00-08

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	De ad	Snow	Wind				
B0, 5-1/2"	356/0	369/0	1,129/0					
B1, 5-1/2"	356/0	369/0	1,129/0					

	ad Summary g Description	Load Type	Re	f. Start	En d	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	Us er Load	Unf. Lin. (lb/ft)	Ĺ	00-05-08	11-07-00	44	40	156		n/a
2	B26(i3740)	Conc. Pt. (lbs)	L	04-01-10	04-01-10	112	89	261		n/a
3	B25(i3739)	Conc. Pt. (lbs)	L	07-10-14	07-10-14	111	89	261		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	7,118 ft-lbs	24,271 ft-lbs	29.3%	13	06-00-04
End Shear	2,076 lbs	11,571 lbs	17.9%	13	01-03-00
Total Load Defl.	L/575 (0.235")	0.563"	41.7%	45	06-00-04
Live Load Defl.	L/736 (0.183")	0.375"	48.9%	61	06-00-04
Max Defl.	0.235"	n/a	n/a	45	06-00-04
Span / Depth	14.2	n/a	n/a		00-00-00

Beari	ng Supports	Dim.(LxW)	Demand	De man d/ Re sistance Support	De mand/ Resistance Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	2,333 lbs	22.7%	9.9%	Unspecified
B1	Wall/Plate	5-1/2" x 3-1/2"	2,333 lbs	22.7%	9.9%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced length.

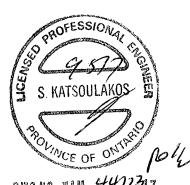
Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086. Unbalanced snow loads determined from building geometry were used in selected product's verification. CONFORMS TO UBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.



DWG HO. TAN 4472317 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B24(i3738)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 11:08:37

BC CALC® Design Report

**Build 4340** 

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2 EL-B.mmdl

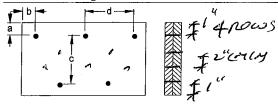
Description: Designs\Flush Beams\1st Floor\Flush Beams\B24(i37;

Specifier:

Designer: AJ Company:

Misc:

### Connection Diagram



c= \$-1/2" a minimum = 2" b minimum = 3"

Calculated Side Load = 113.5 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: "The Nails April 1946 - 3½" ARDOX SPIRAL

### Disclosure

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DVano. TAN 44725-17 STRUCTURAL COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B26(i3740)

BC CALC® Design Report



CCMC 12472-R

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 11:08:37

**Build 4340** Job Name: File Name: S45-2 EL-B.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B26(i3740)

Address:

Specifier:

City, Province, Postal Code: INNISFILL, Customer:

Designer: AJ

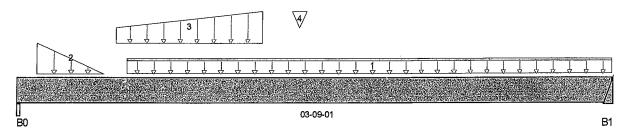
Company:

Code reports:

Misc:

Town of Innisfil Certified Model

03/01/2018 2:31:23 PM kgervais



Total Horizontal Product Length = 03-09-01

Reaction Summary (	Down / Uplift) (lbs)				
Bearing	Live	De ad	Snow	Wind	
B0, 7-7/16"	138/0	95 / 0	221/0		
B1	111/0	89 / 0	257/0		

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Ref. Start		En d	1.00	0.65	1.00	1.15	
1	Us er Load	Unf. Lin. (lb/ft)	L	00-08-05	03-09-01	44	40	156		n/a
2	FC5 Floor Material	Trapezoidal (lb/ft)	L	00-01-10		17				n/a
					00-06-09	0				n/a
3	FC5 Floor Material	Trapezoidal (lb/ft)	L	00-07-07		10	4			n/a
					01-06-06	19	7			n/a
4	J5(i3742)	Conc. Pt. (lbs)	L	01-09-03	01-09-03	93	35			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	433 ft-lbs	12,704 ft-lbs	3.4%	13	01-11-15
End Shear	503 lbs	5,785 lbs	8.7%	13	01-04-15
Total Load Defl.	L/999 (0.002")	n/a	n/a	45	02-01-01
Live Load Defl.	L/999 (0.002")	n/a	n/a	61	02-01-01
Max Defl.	0.002"	n/a	n/a	45	02-01-01
Span / Depth	3.9	n/a	n/a		00-00-00

Beari	ng Supports	Dim.(L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Beam	7-7/16" x 1-3/4"	519 lbs	7.5%	3.3%	Unspecified
B1	Hanger	2" x 1-3/4"	552 lbs	n/a	12.9%	LSSUI25

Notes

NINCE OF ONLES DWG NO. TAM 4472

STRUCTURAL COMPONENT ONLY



### Boiso Cascado Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B26(i3740)

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2016 11:08:37

**Build 4340** 

Job Name: Address:

City, Province, Postal Code: INNISFILL,

Customer:

Code reports:

CCMC 12472-R

File Name: S45-2 EL-B.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B26(i374

Specifier:

Designer: AJ

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculation assumes member is partially braced. See engineering report for the unbraced length.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086. Unbalanced snow loads determined from building geometry were used in selected product's verification. CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Deflections less than 1/8" were ignored in the results.

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			В	are			1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cen	tre Spacing	
		12"	16"	19.2"	24"	12"	16"	<b>/</b> 19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
11-7/0	NI-70	20'-9"	19'-2"	18'-3"	<b>17'-</b> 5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22 <b>'-</b> 9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
44.7/01	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11-7/8"	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19' <b>-</b> 9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
·	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
14"	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"
	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25' <b>-</b> 3"	24'-1"	22'-9"
	NI-90x	27' <b>-</b> 3"	25'-4"	24'-1"	22' <b>-</b> 9"	27'-9"	25'-11"	24'-8"	23'-4"
	NI-60	27'-3"	25 <b>'-</b> 5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-70	28'-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"
10	N!-80 .	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27' <b>-</b> 9"	26'-5"	25' <b>-</b> 0"
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







				are		1	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	Ni-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	N1-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11 7/0"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/8"	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
•	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19 <b>'-1"</b>	N/A	22'-3"	20'-7"	19' <b>-</b> 8"	N/A
	N!-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21' <del>-9</del> "	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	ipan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
11 7/0"	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26 <b>'-</b> 5"	25'-2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26' <del>-</del> 9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26' <b>-</b> 2"	N/A

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			E	Bare		1	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-1/0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17' <b>-</b> 9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	N!-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	Nł-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	N!-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17' <b>-</b> 9"	N/A	21'-3"	19'-3"	17'-9"	N/A
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
11-//0	N!-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	N!-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
16"	NI-80	28' <b>-</b> 2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27' <b>-</b> 5"	26' <b>-</b> 2"	N/A

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			E	are		·	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15' <b>-</b> 7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	Ni-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
11-770	NI-70	20' <b>-</b> 9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	N!-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23' <b>-</b> 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22' <b>-</b> 9"	21'-6"
10	NI-80	25' <b>-</b> 6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"_	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking a	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"
11-7/8"	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/6	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
	Ni-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22' <b>-</b> 5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	Ni-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
16"	NI-70	28'-8"	26'-8"	25'-3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
10	N!-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28' <b>-</b> 5"	26'-11"	24'-10"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

Avoid Accidents by Following these Important Guidelines:

Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous

over interior supports and a load-bearing wall is planned at that location,

blocking will be required at the interior support.

Lipoists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

ENGINEERED WOOD

WARNING

until fully fastened and braced, or serious injuries can result. Do not walk on I-joists

2. When the building is completed, the floor sheathing will provide lateral

support for the top flanges of the I-joists. Until this sheathing is applied

temporary bracing, often called struts, or temporary sheathing must be applied



Once sheathed, do not unsheathed I-joists. materials over

NSTALLATION GUIDE

FOR RESIDENTIAL FLOORS

- over-stress 1-joist with concentrated loads from Never stack building 3. For cantilevered 1-joists, brace top and bottom flanges, and brace ends with

- building materials.

■ Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long to prevent I-joist rollover or buckling. the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each Ljoist. Nail

Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay. bracing over at least two I-joists.

- 4. Install and fully nail permanent sheathing to each 1-joist before placing loads closure panels, rim board, or cross-bridging.
- on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

can result in serious accidents. Follow these installation guidelines carefully. Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic i-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required

# STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- Store, stack, and handle I-joists vertically and level only.

- 3. Always stack and handle I-joists in the upright position only.
- 4. Do not store Ljoists in direct contact with the ground and/or flatwise
- Protect I-joists from weather, and use spacers to separate bundles.
- Bundled units should be kept intact until time of installation.
- 7. When handling I-joists with a crane on the job site, take a few to your work crew. simple precautions to prevent damage to the I-joists and injury
- Pick 1-joists in bundles as shipped by the supplier

Distributed by:

- Orient the bundles so that the webs of the I-joists are vertical.
- Pick the bundles at the 5th points, using a spreader bar if necessary
- 8. Do not handle I-joists in a horizontal orientation.
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.







FSC correct

### MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to simple-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate or more of the adjacent span. For multiple-span applications, the end spans shall be 40% for floor vibration and a live load deflection limit of L/480. limit states are based on the factored loads of 1.50L + .25D. The serviceability limit states include the consideration
- 2. Spans are based on a composite floor with glued-nailed of gypsum and/or a row of blocking at mid-span. Standard. No concrete topping or bridging element was less, or 3/4 inch for joist spacing of 24 inches. Adhesive oriented strand board (OSB) sheathing with a minimum assumed. Increased spans may be achieved with the used shall meet the requirements given in CGBS-71.26 thickness of 5/8 inch for a joist spacing of 19.2 inches or
- 3. Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when L-joists are used with the spans and spacings given in this table, except as required for hangers.
- 5. This span chart is based on uniform loads. For applications be required based on the use of the design properties. with other than uniform loads, an engineering analysis may
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. St units conversion: 1 inch = 25.4 mm 1 toot =  $0.305 \, \text{m}$

### SIMPLE AND MULTIPLE SPANS MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

Joist	Joist		Simple	spans			Multip	e spans
Depth	Series	12"	On centr	e spacing	20"	19:	On centr	e spacini
		151	14/2	13:59	1315	Kera		Official Actions
		1651	152	14:8	14:9	3175	16.5	15:10
		16:35	1514	14'10"	14:117	77.7	16:7	16-0
			1611	15-6	15:71	10.7	17.7	1
		<b>1</b> 217/3	16/3	15-8°	159	18510	17.7	
		16121 S	16:0	# 1515°	1524	and Red C	17170	10-14
			17.00	16.5	IX.	500	100.4	100
			725	7.27	100	300		
		19-6	1810	177	17.6	212	1017	20-0
		9.90	18.3	177.		200	5	77-0
	1	20:25	18.7	17.10	17.17	20.00	7007	7-3
		2014	18.9	17.11	18-0	22.5	20-9	19:10:
		1 20-1	7.2.8	17.10	ablazio.	32120 W	1.Y.DC	10 p.
	Ē	20-5	1182111	18-11	18121	22.7	20-11	20.0
		217	20:0	19:1	19:2	23510	22:1	21.
	The state of		20-3	194	19:5	24.3	22'.5'	6
		3,6	20.8	9.9	19-10	24.9	22'30"	21-10
		( ) ( ) ( ) ( ) ( ) ( )	20-41	12-11-X	20-0	25-0	23-1	22-0
			20-8	19.9	19-10	24-7	22-9	21:9
	<u>4</u>		20.27	20.9	20-10	26-0	24-0	22:11
		300-1	3.4	L	X 1 X	26'5	24-5	23-3
		24.8	22.0	2110	1	26-1	24-10	23-9

### CCMC EVALUATION REPORT 13032-R

### **I-JOIST HANGERS**

- Hangers shown illustrate the three to support I-joists. most commonly used metal hangers
- 2. All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based and load capacity based on the on the joist depth, flange width
- 4. Web stiffeners are required when the brace the top flange of the I-joist. sides of the hangers do not laterally









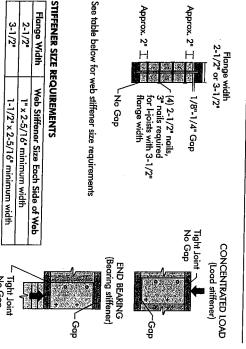
### WEB STIFFENERS

### RECOMMENDATIONS:

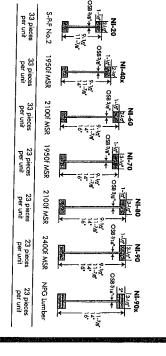
- A bearing stiffener is required in all the stiffener and the flange is at the top. engineered applications with factored I-joist properties table found of the I-joist reactions greater than shown in the Construction Guide (C101).The gap between
- sides of the hanger do not extend up to, and support, the top flange. The gap between the A bearing stiffener is required when stittener and tlange is at the top. the I-joist is supported in a hanger and the
- and the flange is at the bottom. by the code. The gap between the stiffener adjusted for other load durations as permitted standard term load duration, and may be tip and the support. These values are for cantilever, anywhere between the cantilever than 2,370 lbs is applied to the top flange where a factored concentrated load greater A load stiffener is required at locations between supports, or in the case of a
- SI units conversion: 1 inch = 25.4 mm

### FIGURE 2

### WEB STIFFENER INSTALLATION DETAILS



### **NORDIC I-JOIST SERIES**



finished product, reflects our commitment to quality. manufacturing process. Every phase of the operation, from forest to the ... products to adhere to strict quality control procedures throughout the and Chantiers Chibougamau Ltd. harvests its own trees, which enables. Nextic 

longer span carrying capacity. lumber in their flanges, ensuring consistent quality, superior strength signs. Nordic Engineered Wood I-joists use only finger-jointed black spruce 11.



### INSTALLING NORDIC I-JOISTS

- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, contact your supplier.
- 2. Except for cutting to length, 1-joist flanges should never be cut, drilled, or notched.
- 3. Install Lioists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
- 4. I-foists must be anchored securely to supports before floor sheathing is attached, and supports for multiple அள்ளேல் நாய்த்து மத்திரும் அள்ளேல் நடிக்கும்.
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings
- 6. When using hangers, seat Lipists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the 1-joist end and a header.
- 8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the I-joist webs.
- Never install Lioists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I-joist blocking panels or other engineered wood products such as rim board must be cut to fit between the I-joists, and an I-joist-compatible depth selected.
- 13. Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
- 14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed.
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

(F)

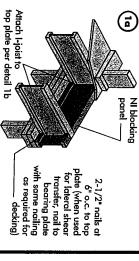
One 2-1/2" wire or spiral nail at top and

-Attach rim board to top plate using 2-1/2" wire or

spiral toe-nails at 6" o.c.

### Id only be applied to ippment and security suspend all astened to the direct contact with direct contact w

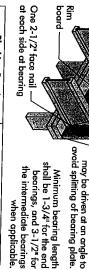
All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fit No. 2 or better. Individual components not shown to scale for clarity.



To avoid splitting flange, start nails at least 1-1/2" from end of I-joist. Nails

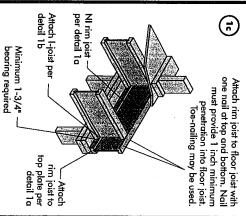
*The iniform matical is a	NI Joists	Blocking Panel or Rim Joist
*The minimum water I have a second	3,300	Maximum Factored Uniform Vertical Load* (pH)

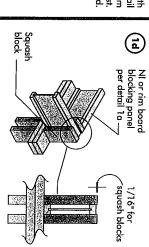
Ine unitorm vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



8,090	1-1/8" Rim Board Plus
Maximum Factored Uniform Vertical Load* (plf)	Blocking Panel or Rim Joist

\*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



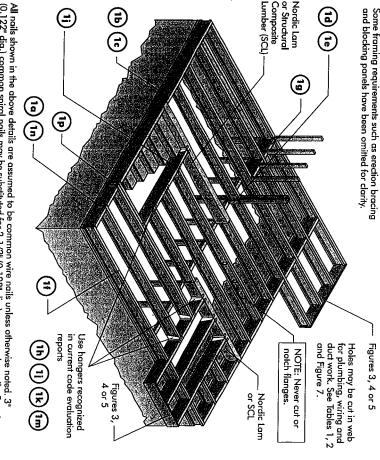


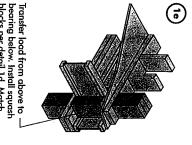
8,500	5,500	2x Lumber
5-1/2" wide	3-1/2" wide	
h Blocks (lbs)	Pair of Squash Blocks (lbs)	Pair of Squash Blocks
red Vertical per	Maximum Factored Vertical per	

Provide lateral bracing per detail 1a, 1b, or 1c

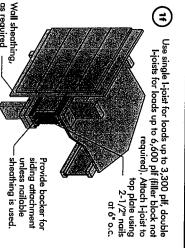
### ₽ F 1

# TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



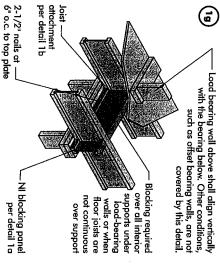


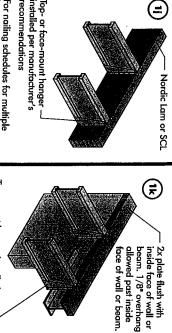
bearing below. Install squash blocks per detail 1d. Match bearing area of blocks below



Rim board may be used in lieu of I-joists. Backer is not carried to the foundation. required when rim board is used. Bracing per code shall be

**(1)** 





iller block per

manufacturer's recommendations Top-mount hanger installed per \_\_\_

manufacturer's

support the top flange, bearing Note: Unless hanger sides laterally stiffeners shall be used

Maximum support capacity = 1,620 lbs

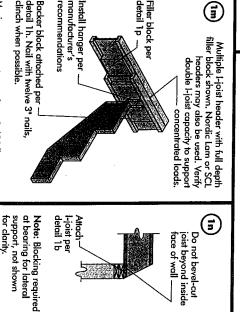
support the top flange, bearing stiffeners shall be used.

Note: Unless hanger sides laterally

beams, see the manufacturer's

recommendations

recommendations installed per manutacturer's Top- or face-mount hanger



**(** Top- or face-mount hanger per detail 1p Double I-joist header Filler block backer block will fit. Clinch. Install backer tight to top flange. additional 3" nails through the webs and filler block where the Before installing a backer block to a double 1-joist, drive three Backer block (use if hanger load exceeds 360 lbs) Use twelve 3" nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs. laterally surport the tao flangry his tring attiffection shair (1874) sections of Backer block required Note: Unless hanger sides (both sides for face-mount 2019-04-16

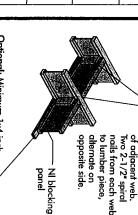
For hanger capacity see hanger manufacturer's recommendations. Verify double 1-joist capacity to support concentrated loads.

nangers)

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	1"	5-1/2"
3-1/2"	1-1/2"	7-1/4"

- better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard. Minimum grade for backer block material shall be S-P-F No. 2 or
- For face-mount hangers use net joist depth minus 3-1/4" for minus 4-1/4". joists with 1-1/2" thick flanges. For 2" thick flanges use net depth

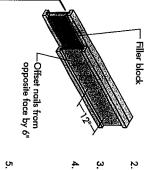


strap applied to underside of joist at blocking line or 1/2 inch minimum gypsum ceiling attached to underside of joists

Optional: Minimum 1x4 inch

### € −One 2-1/2" nails at top and bottom flange board One 2-1/2" nails one side only 2-1/2" nails at 6" o.c. lumber piece lwo 2-1/2" nails from each web to —2x4 min. (1/8" gap minimum) i-joist blocking panel

- the starter joist. Where required, see local code requirements the first joist space (or first and second joist space) next to In some local codes, blocking is prescriptively required in for spacing of the blocking



—1/8" to 1/4" gap between top flange and filler block

### Zotes:

**(** 

Support back of I-joist web during nailing to prevent damage to web/flange connection.

DOUBLE I-JOIST CONSTRUCTION

**(** 

extend block to face

Lumber 2x4 min.,

FILLER BLOCK REQUIREMENTS FOR

Flange Size

oist

- 2. Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist
- 3. Filler block is required between joists for full length of span.

2-1/2"× 1-1/2"

2-1/8" x 12" 2-1/8" × 10" 2-1/8" × 8" 2-1/8" x 6" Block Size

9-1/2"

4. Nail joists together with two rows of 3" are requirea can be clinched, only two nails per foot possible) on each side of the double I-joist. Total of four nails per foot required. If nails nails at 12 inches o.c. (clinched when

3-1/2"× 1-1/2"

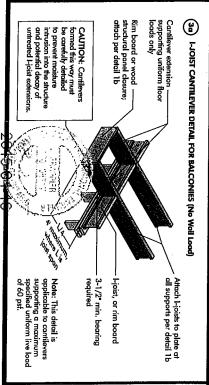
3" × 10" 3" × 12" သူ × စူ သူ x ၀-

The maximum factored load that may be applied to one side of the double joist using this detail is 860 lbf/ft. Verify double

3-1/2"× 2"

3"×9" 3"×11"

# CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)





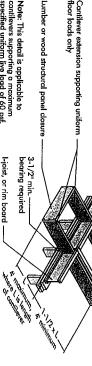
Full depth backer block with  $1/8^{\circ}$  gap between block and top flange of I-joist. See detail 1h. Nail with 2 rows of  $3^{\circ}$  nails at  $6^{\circ}$  o.c. and clinch.

Attach I-joists to plate at all supports per detail 1b

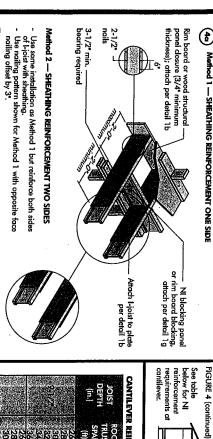
2x8 min. Nail to backer block and joist with 2 rows of 3" nails at 6" o.c. and clinch. (Cantilever nails may be used to attach backer block if length of nail is sufficient to allow clinching.)

floor loads only Cantilever extension supporting uniform

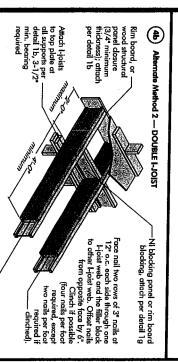
specified uniform live load of 60 psf. Note: This detail is applicable to



# CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



Note: Caractian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and botham flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.



Block I-joists together with filler blocks for the full length of the reinforcement. \rightarrow For I-joist flange widths greater than 3 inches place an additional row of 3" nails along the centreline of the reinforcing panel from each side. Clinch when possible.

Roof truss span

<u>ئ</u> 2 cantilever

> Girder Roof trusses

For hip roofs with the jack trusses running parallel to the cantilevered floor joists, the I-joist reinforcement requirements for a span of 26 ft. shall be permitted to

Roof truss. span

maximum cantilever -<u>2</u>-0 · Jack trusses 13'-0" maximum

CANTILEVI	R REINFO	RCEMENT METHODS ALLOWED			
JOIST DEPTH (in.)	ROOF TRUSS SPAN (#)	⊒ ≡ŏ	FLOADING (UNFACTO L = 40 psf, Dt = 15 ps JOIST SPACING (in.) 16 19.2		f 24
		X	N 1 2 2 2 X 2 2 X 2 X 2 X X 2 X X X X X X		****
11.778:	28882 68882	2222 2222 	ZZZZ)		(×××)
	38° 26	N N	Z -	Z Z Z	- ××
2	2888	ZZZZ			NN⊐:
	36 138 40	ZZZZ		Z	ממא
	30 20 32 00 32 00 30 30 30 30 30 30 30 30 30 30 30 30 3	ZZZZ			, , , , , ,
160	4488 4686 4	ZZZZZ	ZZZZZ	NZZZZ	เพพพพ-
			100	がではア Mark では	×

- 1. N = No reinforcement required.
  1 = NI reinforced with 3/4\* wood structural panel on one side only.
  2 = NI reinforced with 3/4\* wood structural panel on both sides, or double I-joist.
- X = Iry a deeper joist or closer spacing.

  Avaimum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 psf wall load. Wall load is based on 3:0"
- For larger openings, or multiple 3'-0" width openings spaced less than 6'-0' c.c., additional joist beneath the opening's cripple studs may be required.

  3. Table applies to joist 12' to 24' o.c. that meet the floor span requirements for a design live load of 40 per and dead load of 15 per and a live load ad deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.
- 4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the ord is formed using a ridge board, in the Roof Truss Span is equivalent to the
- truss is used.

  5. Cantilevered joists supporting girder trusses or roof beams may require additional distance between the supporting walls as if a

### **WEB HOLES**

# **RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS**

- The distance between the inside edge of the support and the centreline of any Table 1 or 2, respectively. hole or duct chase opening shall be in compliance with the requirements of
- ы l-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- ω Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. the L-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent L-joist flange. The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of
- Ģ The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- ٥. Where more than one hole is necessary, the distance between adjacent hole size of the largest square hole (or twice the length of the longest side of the opening shall be sized and located in compliance with the requirements of longest rectangular hole or duct chase opening) and each hole and duct chase edges shall exceed twice the diameter of the largest round hole or twice the Tables 1 and 2, respectively.
- .7 A knockout is **not** considered a hole, may be utilized anywhere it occurs, and and/or duct chase openings. may be ignored for purposes of calculating minimum distances between holes
- œ cantilevered section of a joist. Holes of greater size may be permitted subject to Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a
- 9 A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- ō, All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

bearing -

 $\odot$ 

distance from or minimum See Table 1

2x diameter of larger hole

diameter, length or hole 2x duct chase

(see Table 2 for minimum distance Duct chase opening

from bearing)

spaced 15 inches on centre along the length of the Lioist. Where possible, it is preterable to use knockouts instead of

field-cut holes

electrical or small plumbing lines. They

/2 inches in diameter, and are

whichever is

FIELD-CUT HOLE LOCATOR

FIGURE 7

### Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Above table			(C)	(e)	Joist Depth
may he lise	ajolojula	.;;-{-1-1-1-	stateratorists	exedeiele	Joist Series
for Liniet en					
ring of 34 in	00-8 00-8 00-8		anta da d		nimum 4
when on contr	1				<b>- 18</b>
,			100 141 5 5 721 84 728 86 748 69		
	5.6 7.8 8.0 6.5	330000 360000	3000 03 3000 03	1111	ce of any ole diam 8 8
			79" = 8.4" = 10.0" = 111.4" =		90
	27-77-77-78-58-58-58-58-58-58-58-58-58-58-58-58-58	2000年中代15.C000年前25.6		生物 化工作工作 化二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十	
	12.2	[[[]]	111111	11111	de (ff-in
			Talahal Barusa		
	21.21	100 CE 10	19 19 19 19 19 19 19 19 19 19 19 19 19 1		Span adjustment Factor

- races unue may be used for I-joist spacing or 24 inches on centre of less.

  Hole location distance is measured from inside face of supports to centre of hole

  Distances in this chart are based on uniformly loaded joists.

### OPTIONAL:

The above table is based on the I-joists used at their maximum span. If the I-joists are placed at less than their full maximum span (see Maximum Fic.) Spans the minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

Dreduced = Lactual × D SAF

Where:

Lactual Dreduced = Distance from the inside face of any support to centre of hole, reduced for less-than-maximum spon applications (fit. The reduced distance shall not be less than 6 inches from the face of the support to edge of the hole.

The actual measured span distance between the inside faces of supports (fit).

ף אֱ

Span Adjustment Factor given in this table.

H<u>adual</u> is greater than 1, use 1 in the above calculation for <u>batual</u>
SAF The minimum distance from the inside face of any support to centre of hole from this table

2019947

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TABLE 2

for the contractor's convenience to instal Knockouts are prescored holes provided DUCT CHASE OPENING SIZES AND LOCATIONS - Simple Span Only

	7.			Joist Depth
	e e e e e			Joist Series
-1		255550	ono ono a	M
222	king yan		e e koja	nimu 8
		B 33 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	ង្គាល់ សង្គសុំ ភពលលក់ <b>ខ</b>	n dish 10
	d-la cara	888Y8Y	.0001	ince fi
120	62 å å å å å	### ## ## ## ## ## #### ##############	Pakea	om in
1116 846	900 900 900 900 900	8888888 366463	00005 10000	side fo Duct
				F B
	46	20 B 8 B 8 B 1 B 1 B 1 B 1 B 1 B 1 B 1 B 1	10.	any su lengt 6
123 233	9.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1		27625	pport h (in.) 18
		99998 9969 9969	77876	to cen
			3.7	2.
	355 = 35 44 - 44 44 - 44		8887 3-32-1	openi 22
	and the		100	ਰ
e e	<b>့</b> _ ဇမ္မင်စ	1664¢≟¢	0.00 4.4	¥ <u>5</u>

the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the corners, as this can cause unnecessary the corners is recommended. Starting stress concentrations. Slightly rounding For redangular holes, avoid over-cutting 19.00

sharp saw. should be cut with a Holes in webs over-cut the web. **Never** drill, cut or notch the flange, or

the holes is another good method to

and may be ignored for purposes of calculating minimum distances A knockout is **NOT** considered a hale, may be utilized wherever it occurs

See rule 12

between top and bottom flange — all duct chase openings and holes Maintain minimum 1/8" space

between holes

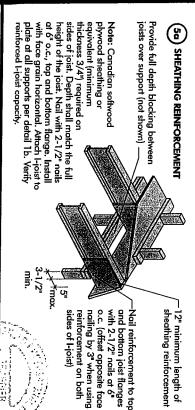
Above table may be used for I-joist spacing of 24 inches on centre or less.

Duct chase opening location distance is measured from inside face of supports to centre of opening.

The above table is based on simple-span joists only for other applications, contact your local distributor,

Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of I/480. For other applications, contact your local distributor.

# BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



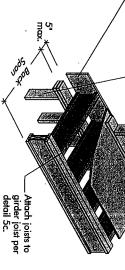
nailing by 3" when using o.c. (offset opposite face reinforcement on both and bottom joist flanges with 2-1/2" nails at 6"

F SET-BACK DETAIL

Bearing walls

structural panel closure (3/4" minimum thickness), attach per detail 1b. Rim board or wood

- between joists over support Provide full depth blocking (not shown for clarity)
- supports per detail 1b. Attach 1-joist to plate at all
- bearing required. 3-1/2" minimum I-joist

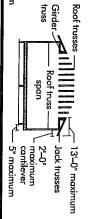


### (5c) SET-BACK CONNECTION

using 2-1/2" nails. through joist web and web of girder Alternate for opposite side.

- Verify girder joist capacity if the back span exceeds the joist spacing.
- Attach double I-joist per detail 1p, if required
- bottom flanges. nails, toe-nail at top and Nail joist end using 3" used in lieu of solid sawn blocks Hanger may be

### FIGURE 5 (continued) cantilever. requirements at See table reinforcement below for NI Roof truss span 7 2 0 ∟maximum -5" maximum cantilever



requirements for a span of the I-joist reinforcement the cantilevered floor joists, trusses running parallel to For hip roofs with the jack 26 ft. shall be permitted to

# BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

<b>2</b>			16 B	JOIST DEPTH (in.)
28 30 32 34 38 38 40	688413386 86814886	8663338	8 2 3 B B B	ROOF TRUSS SPAN (ft)
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**************************************	*****	××××××	×××× <b>×</b>	psf .) 24
	6z	222777	××××××	ROOF LL J
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×××××××	×××××××		×××××	5 psf 1.) 2,

- N = No reinforcement required.
  1 = NI reinforced with 3/4" wood structural
- panel on one side only.

  2 = NI reinforced with 3/4" wood structural
- X = Try a deeper joist or closer spacing. panel on both sides, or double I-joist.
- Maximum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 plf wall load. Wall load is based on 3'-0" width window or door openings
- studs may be required additional joists beneath the opening's cripple openings spaced less than 6'-0" o.c., For larger openings, or multiple 3'-0" width
- μ the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use Table applies to joists 12" to 24" o.c. that meet
- For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between truss is used. the Roof Truss Span is equivalent to the the supporting wall and the ridge beam. distance between the supporting walls as if a When the roof is framed using a ridge board,
- Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

# INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- 2. Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- 3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manufacturer.
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single 1-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying a thinner line (1/8 inch) than used on I-joist flanges.

2-1/2" toe-nails at 6" o.c. (turical)

o.c. (typical)

Rim board joint

- 8. Tap the second row of panels into place, using a block to protect groove edges.
- 9. Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and nail to assure accurate and consistent spacing.) 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations finished deck can be walked on right away and will carry construction loads without damage to the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the

## FASTENERS FOR SHEATHING AND SUBFLOORING(1)

		-	
24	20	16	Maximum Joist Spacing (in.)
3/4	5/8	5/8	Minimum Panel Thickness (in.)
2"	. 22	2"	No Common Wire or Spiral Nails
1-3/4"	1-3/4*	1-3/4"	iil Size and Ty Ring Thread Nails or Screws
2"	2"	2"	pe Staples
6"	6	6,	Maximum of Fast Edges
12"	12"	12"	n Spacing teners Interm, Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter.
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manutacturer.
- Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5

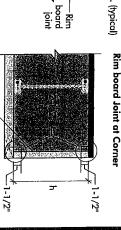
### IMPORTANT NOTE:

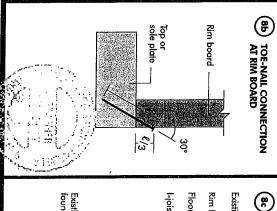
Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with your local distributor.

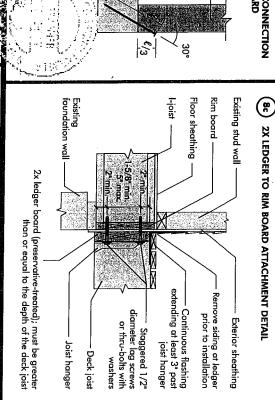
# **RIM BOARD INSTALLATION DETAILS**

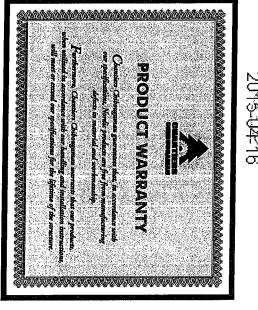
# (8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

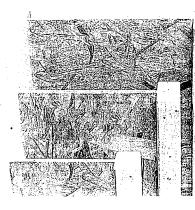












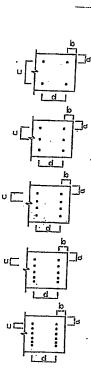
### MICRO CITY

### ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

	LVL HEADER AND CONVENTIONA				
		BER NAILING I			
			TAILO		
	DETAIL	NUMBER	SPACING		
	NUMBER	OF ROWS	(INCHES o/c		
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	В	2	8		
	С	2	6		
	D	2	4		
	1A	3	12		
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L	4B	6	8		
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L.	4D	6	4		



### NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
- (2) MINIMUM LUMBER END DISTANCE "b" = 2"
- (3) MINIMUM NAIL ROW SPACING "c" = 2"
- (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
- (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
- (6) DO NOT USE AIR-DRIVEN NAILS



DVG NO TÄNNIOOI. 14

STRUGTURAL

COMPONENT ONLY

TO BE USED ONLY

WITH BEAM CALCS

PSEARING THE

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