ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

CONV. FRAMING FOR REAR UPGRADE ONLY

HARDWARE:

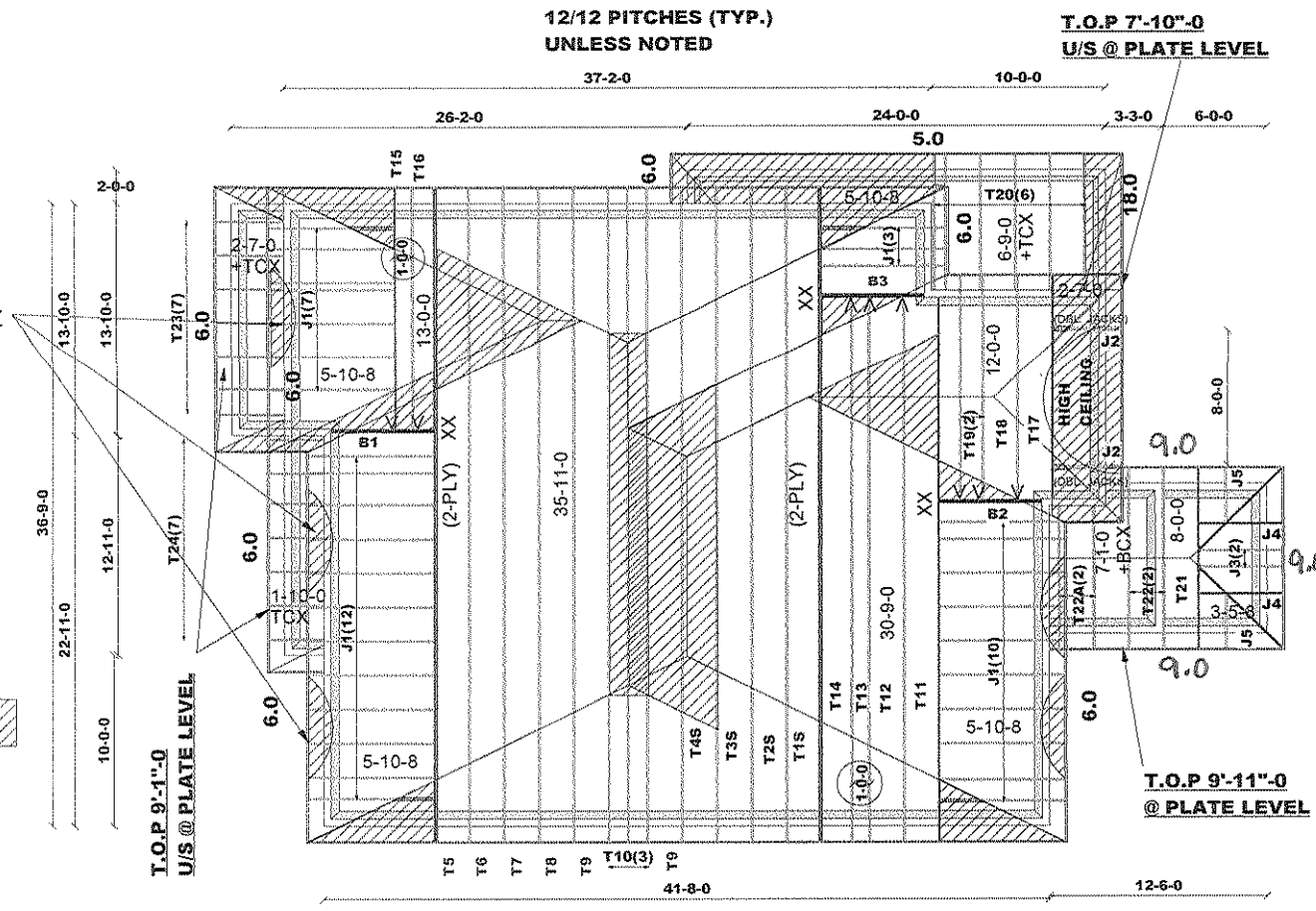
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES:

ALL B- 2-2X10

CONV.FRAMING BY OTHERS

T-170681



Town of Innisfil Certified Model
03/01/2018 8:46:42 AM kgervais

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

M11,023

TAMARACK

Job Track: 42067
Layout ID: 288117
Plan Log: 94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 9/19/2017 Designer: ella

Model / Elevation: S45-1- HUMMINGBIRD 1/ A OR A REAR UPGRADE

Mitek ver 7.6.0

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

CONV. FRAMING FOR REAR UPGRADE ONLY

HARDWARE:

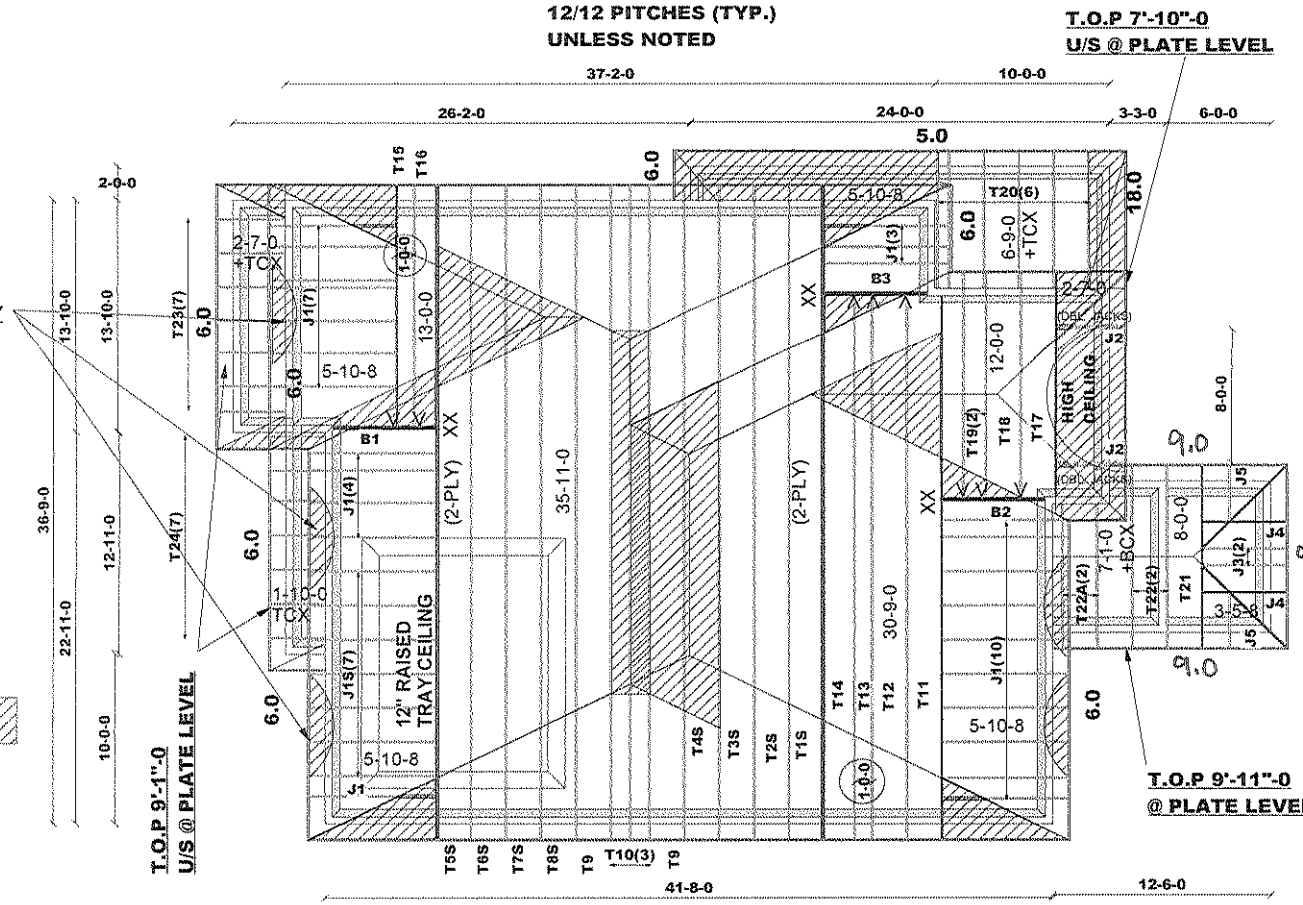
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES:

ALL B- 2-2X10

CONV.FRAMING BY OTHERS

T-170681



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN LOADS:

SNOW LOAD	38.3 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

Town of Innisfil Certified Model
03/01/2018 8:46:48 AM kgervais

M11,033

TAMARACK LUMBER INC

Job Track: 42067
Layout ID: 288116
Plan Log: 94324

Builder / Location:
BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 9/19/2017 Designer: ella

Model / Elevation: S45-1- HUMMINGBIRD 1/
A OR A REAR UPGRADE +OPT. COFF.

Mitek ver 7.5.0

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES:

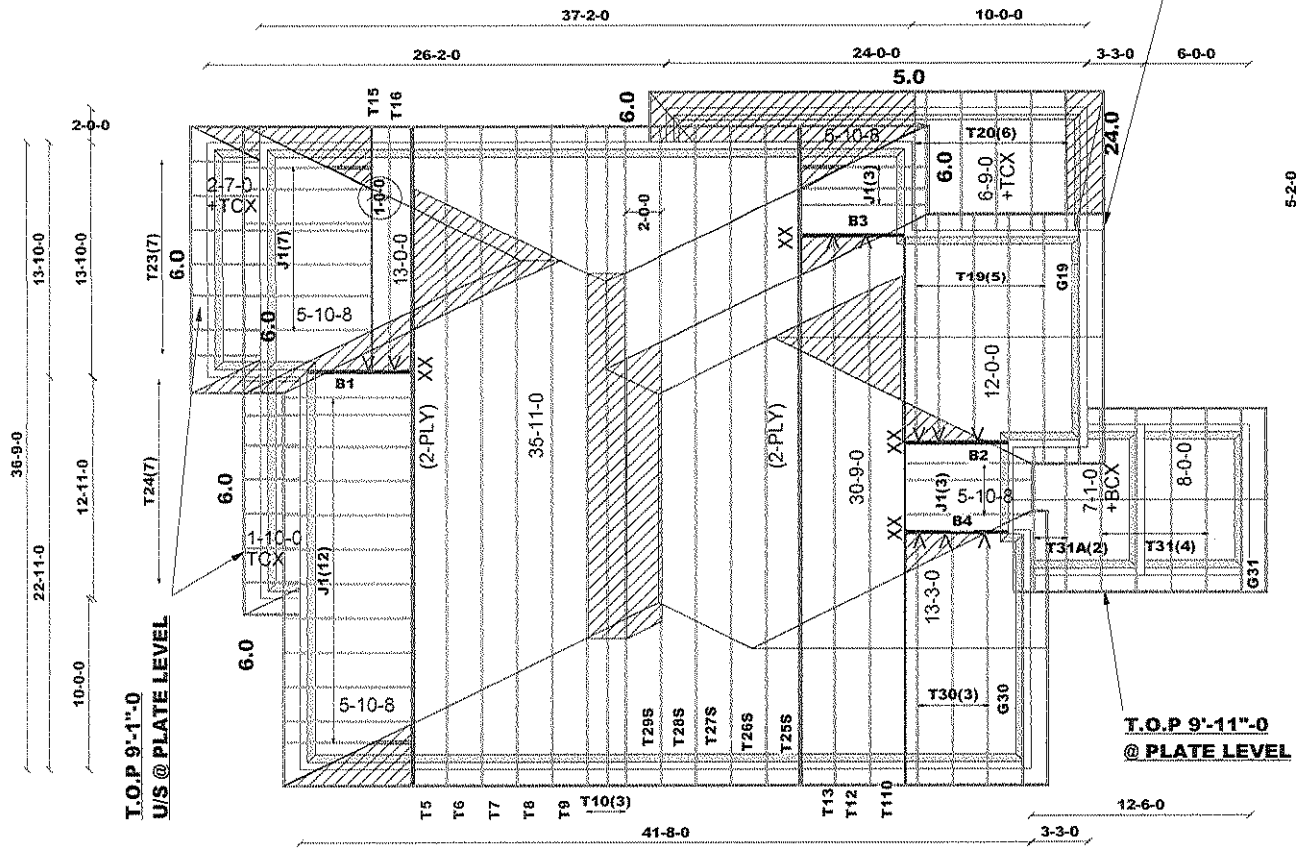
ALL B- 2-2X10

CONV.FRAMING
BY OTHERS

T-170681

12/12 PITCHES (TYP.)
UNLESS NOTED

T.O.P 7'-10"-0
U/S @ PLATE LEVEL



T.O.P 9'-11"-0
U/S @ PLATE LEVEL

T.O.P 9'-11"-0
@ PLATE LEVEL

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

Town of Innisfil Certified Model
03/01/2018 8:46:53 AM kgervais

ML-0033

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	288250
Plan Log:	94324

Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**

Project: **ALCONA SHORES**

Date: 9/19/2017 Designer: sonny

Model / Elevation: **S45-1- HUMMINGBIRD 1/ B**

Mitek ver 7.5.0

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

HARDWARE:

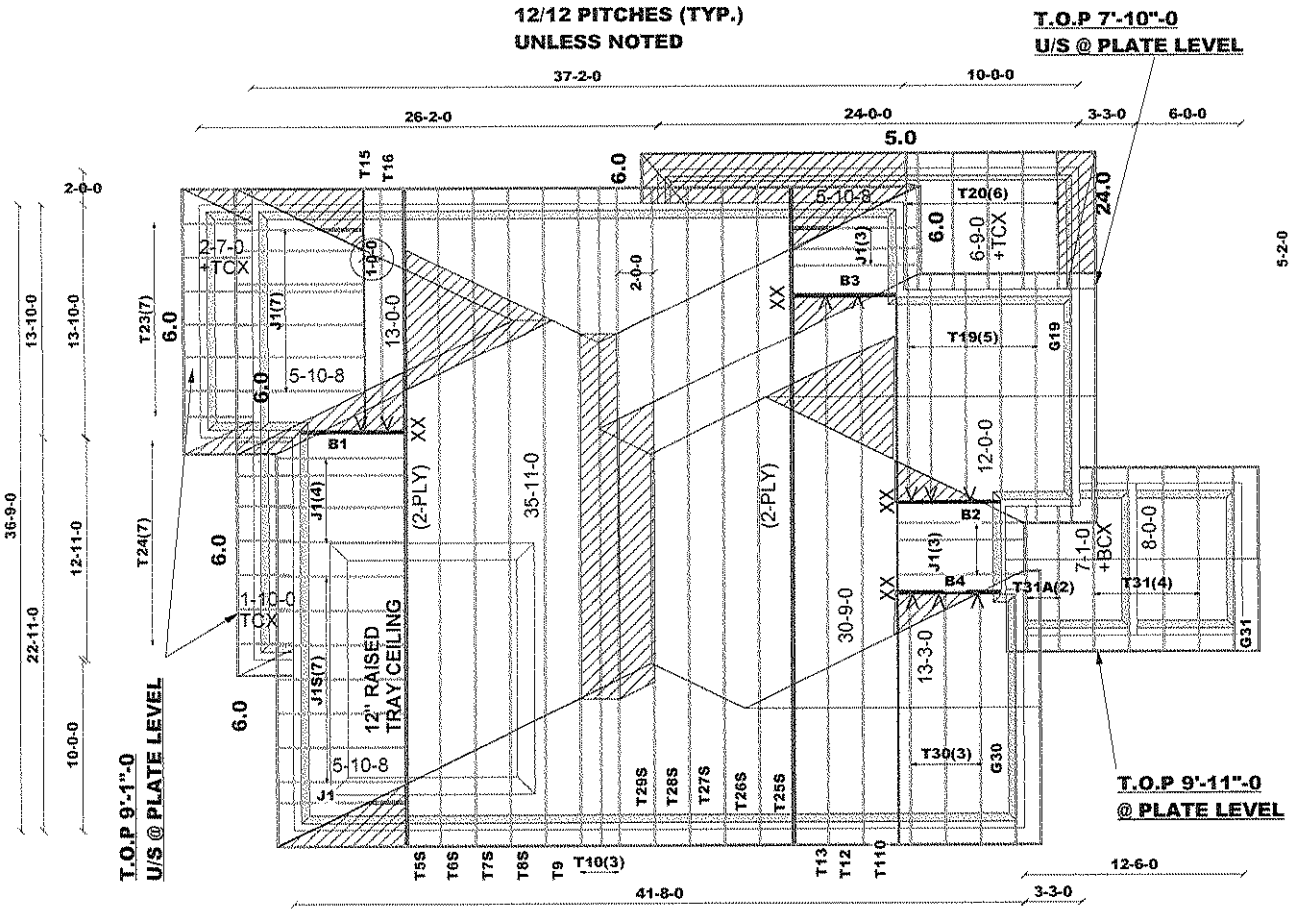
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES:

ALL B- 2-2X10

CONV.FRAMING
BY OTHERS

T-170681



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC,LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD	38.3 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

Town of Innisfil Certified Model
03/01/2018 8:46:59 AM kgervais

mil.033

TAMARACK TRUSS

Job Track: 42067
Layout ID: 288118
Plan Log: 94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 9/19/2017 Designer: ella

Model / Elevation: S45-1- HUMMINGBIRD 1/ B+OPT. COFF.

Mitek ver 7.5.0

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

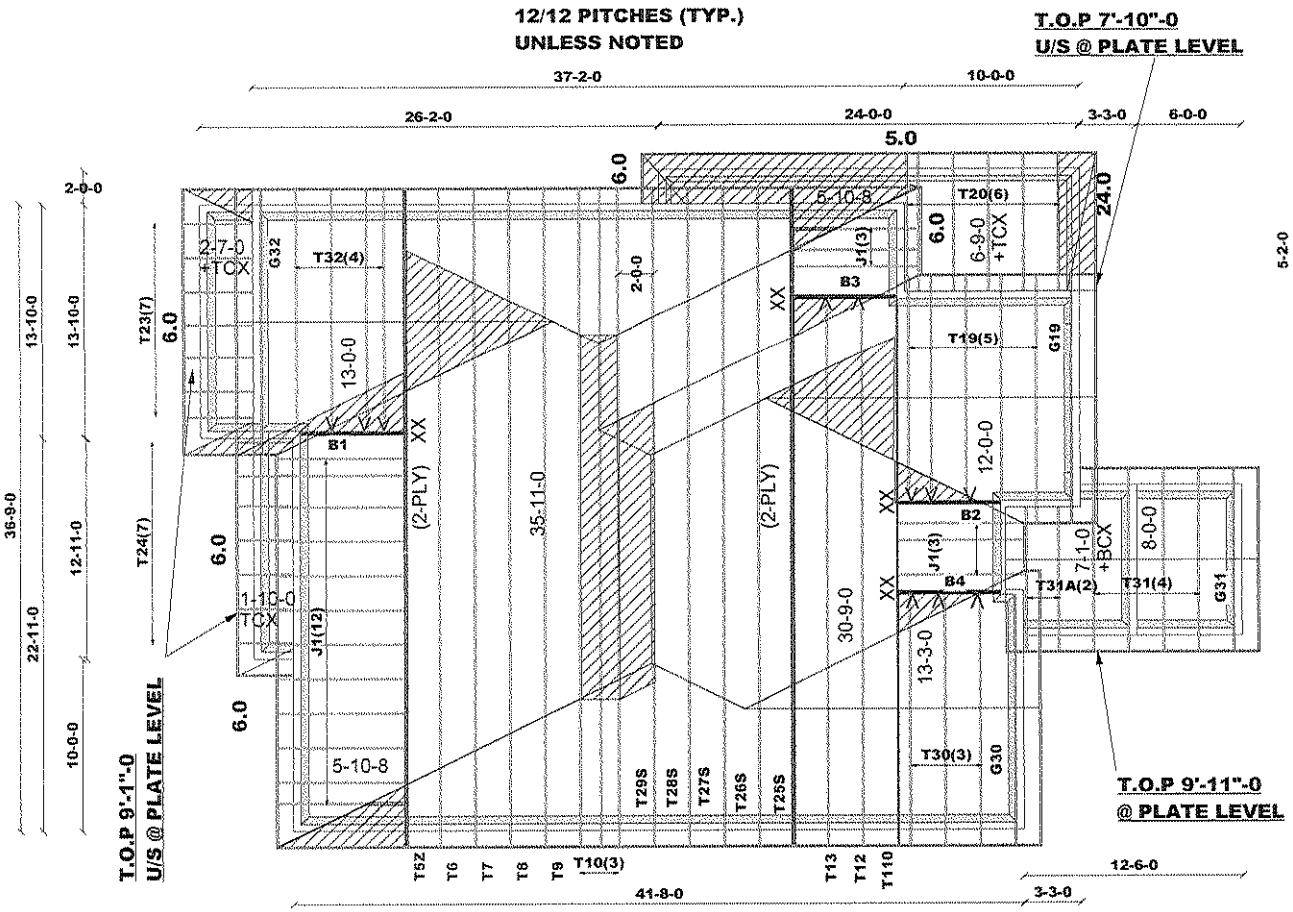
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES:

ALL B- 2-2X10

CONV.FRAMING BY OTHERS

T-170681



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

Town of Innisfil Certified Model
03/01/2018 8:47:05 AM kgervais

mil.023

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	288252
Plan Log:	94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL

Project: ALCONA SHORES

Date: 9/19/2017 Designer: sonny

Model / Elevation: S45-1- HUMMINGBIRD 1/ B(REAR UPGRADE)

Mitek ver 7.5.0

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

HARDWARE:

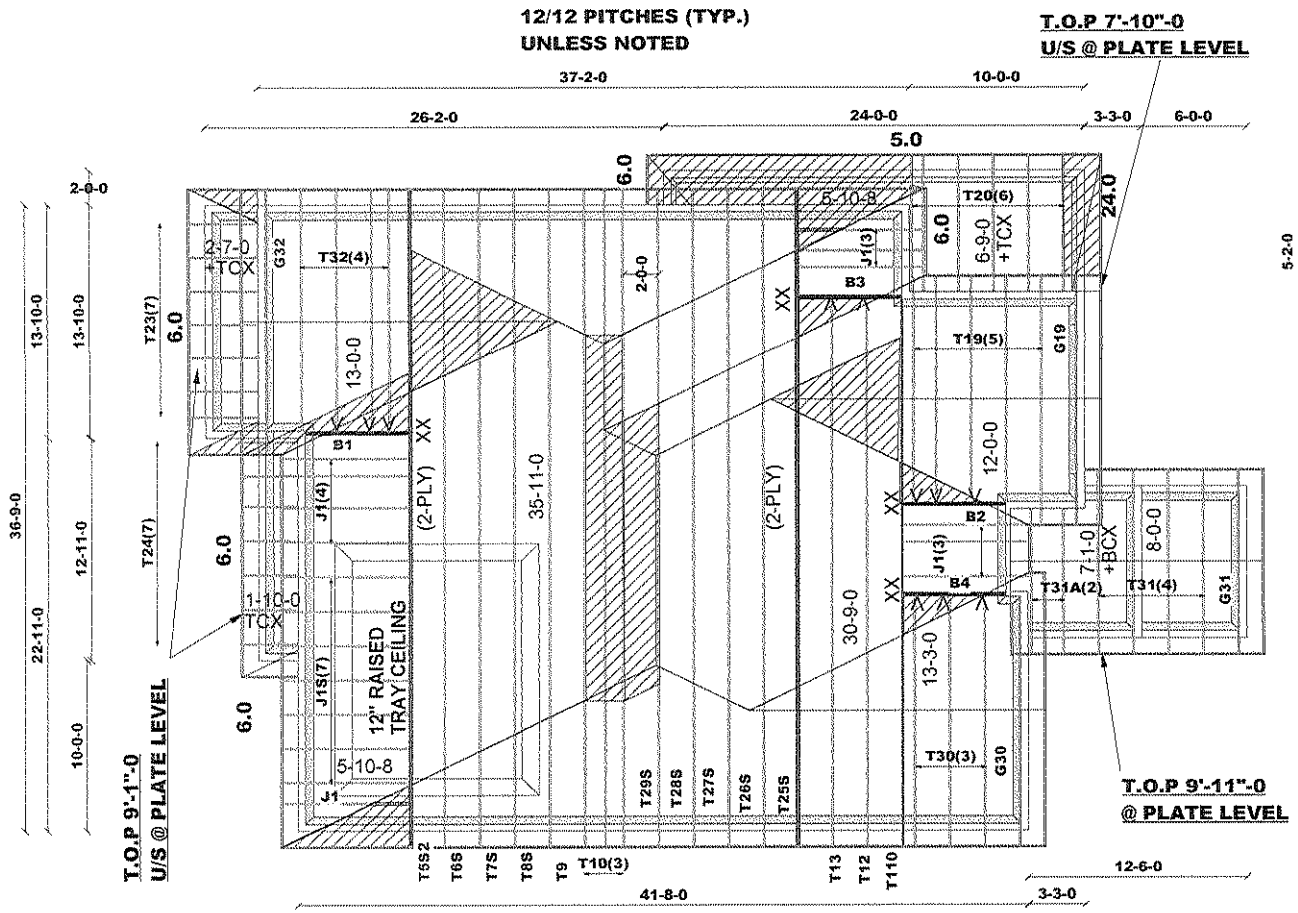
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES:

ALL B- 2-2X10

CONV.FRAMING
BY OTHERS

T-170681



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 38.3 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

Town of Innisfil Certified Model
03/01/2018 8:47:09 AM kgervais

M11,032

TAMARACK TRUSS INC.

Job Track: 42067
Layout ID: 288251
Plan Log: 94324

Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 9/19/2017 Designer: elia

Model / Elevation: S45-1- HUMMINGBIRD 1/ B(REAR UPGRADE)+OPT. COFF.

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

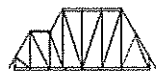
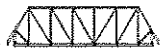
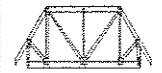
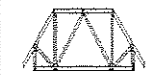
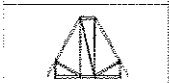
Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288117 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S45-1 HAMMINGBIRD 1 ELEVATION: A OR A (REAR UPGRADE)

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 / 2 Ply	T1S ROOF	12.00 / 0.00	35-11-00	07-05-12	2 X 4	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	434.90 / 271.34		
	1	T2S ROOF	12.00 / 0.00	35-11-00	08-05-12	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	177.25 / 111.83		
	1	T3S ROOF	12.00 / 0.00	35-11-00	09-05-12	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	197.34 / 125.00		
	1	T4S ROOF	12.00 / 0.00	35-11-00	10-05-12	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	211.77 / 132.17		
	1 / 2 Ply	T5 HIP GIRDER	12.00 / 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	430.00 / 266.00		
	1	T6 HIP	12.00 / 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	158.39 / 100.50		
	1	T7 HIP	12.00 / 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	161.26 / 102.33		
	1	T8 HIP	12.00 / 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	177.20 / 112.83		
	2	T9 HIP	12.00 / 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	353.28 / 222.66		
	3	T10 HIP	12.00 / 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	557.13 / 348.51		
	1	T11 HIP GIRDER	12.00 / 0.00	30-09-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	188.43 / 117.67		
	1	T12 HIP	12.00 / 0.00	30-09-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	137.47 / 87.17		
	1	T13 HIP	12.00 / 0.00	30-09-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	138.94 / 88.00		
	1	T14 HIP	12.00 / 0.00	30-09-00	06-07-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	142.86 / 89.83		
	1	T15 HIP GIRDER	12.00 / 0.00	13-00-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	68.46 / 45.67		
	1	T16 HIP	12.00 / 0.00	13-00-00	04-07-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	60.33 / 38.67		
	1	T17 HIP GIRDER	12.00 / 0.00	12-00-00	04-05-08	2 X 4	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	66.64 / 41.33		
	1	T18 HIP	12.00 / 0.00	12-00-00	06-05-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	62.00 / 39.17		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

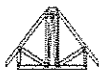
Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288117 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-1 HAMMINGBIRD 1 ELEVATION: A OR A (REAR UPGRADE)

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T19 COMMON	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	127.14 / 82.66		
	6	T20 MONOPITCH	5.00 / 0.00	06-09-00	04-02-03	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-09 / 04-02-03	196.20 / 131.04		
	1	T21 HIP GIRDER	9.00 / 0.00	08-00-00	04-01-06	2 X 4	2 X 4	01-03-08 / 01-03-08	01-06-04 / 01-06-04	43.88 / 31.33		
	2	T22 COMMON	9.00 / 0.00	08-00-00	04-06-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-06-04 / 01-06-04	72.92 / 46.66		
	2	T22A COMMON	9.00 / 0.00	07-01-00	04-06-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-06-04 / 00-03-08	69.02 / 45.34		
	7	T23 MONOPITCH	6.00 / 0.00	02-07-00	02-10-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-10-02	112.77 / 78.19		
	7	T24 MONOPITCH	6.00 / 0.00	01-10-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-05-10	91.63 / 65.31		
	32	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	537.28 / 341.44		
	2 / 2 Ply	J2 JACK	12.00 / 0.00	02-07-00	04-05-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 04-05-08	44.80 / 29.32		
	2	J3 JACK-OPEN	9.00 / 0.00	03-05-08	04-01-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-06-04 / 04-01-06	24.40 / 17.34		
	2	J4 JACK-OPEN	9.00 / 0.00	03-05-08	02-10-05	2 X 4	2 X 4	01-03-08 / -01-08-01	01-06-04 / 00-03-08	19.64 / 14.66		
	2	J5 JACK-OPEN	9.00 / 0.00	01-10-08	02-10-05	2 X 4	2 X 4	01-03-08 / -00-01-01	01-06-04 / 00-03-08	16.02 / 12.00		

TOTAL # TRUSS= **92.00** TOTAL BFT OF ALL TRUSSES= **3235.97** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5079.35** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **11.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

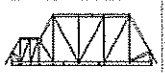
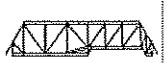
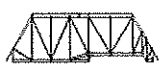
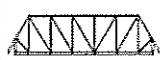
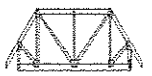
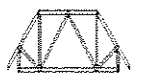
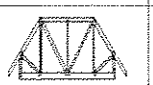
Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288116 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-1 HAMMINGBIRD 1 ELEVATION: A OR A (REAR UPGRADE) + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1S ROOF	12.00 0.00	35-11-00	07-05-12	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	434.90 271.34		
	1	T2S ROOF	12.00 0.00	35-11-00	08-05-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	177.25 111.83		
	1	T3S ROOF	12.00 0.00	35-11-00	09-05-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	197.34 125.00		
	1	T4S ROOF	12.00 0.00	35-11-00	10-05-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	211.77 132.17		
	1 2 Ply	T5S HIP GIRDER	12.00 12.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	454.04 280.66		
	1	T6S HIP	12.00 12.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	184.36 116.33		
	1	T7S HIP	12.00 12.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	191.79 121.50		
	1	T8S HIP	12.00 12.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	199.15 124.50		
	2	T9 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	353.28 222.66		
	3	T10 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	557.13 348.51		
	1	T11 HIP GIRDER	12.00 0.00	30-09-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	188.43 117.67		
	1	T12 HIP	12.00 0.00	30-09-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	137.47 87.17		
	1	T13 HIP	12.00 0.00	30-09-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	138.94 88.00		
	1	T14 HIP	12.00 0.00	30-09-00	06-07-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	142.86 89.83		
	1	T15 HIP GIRDER	12.00 0.00	13-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	68.46 45.67		
	1	T16 HIP	12.00 0.00	13-00-00	04-07-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.33 38.67		
	1	T17 HIP GIRDER	12.00 0.00	12-00-00	04-05-08	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	66.64 41.33		
	1	T18 HIP	12.00 0.00	12-00-00	06-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	62.00 39.17		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

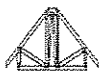
Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288116 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-1 HAMMINGBIRD 1 ELEVATION: A OR A (REAR UPGRADE) + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T19 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	127.14 82.66		
	6	T20 MONOPITCH	5.00 0.00	06-09-00	04-02-03	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 04-02-03	196.20 131.04		
	1	T21 HIP GIRDER	9.00 0.00	08-00-00	04-01-06	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	43.88 31.33		
	2	T22 COMMON	9.00 0.00	08-00-00	04-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	72.92 46.66		
	2	T22A COMMON	9.00 0.00	07-01-00	04-06-04	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 00-03-08	69.02 45.34		
	7	T23 MONOPITCH	6.00 0.00	02-07-00	02-10-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-10-02	112.77 78.19		
	7	T24 MONOPITCH	6.00 0.00	01-10-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-05-10	91.63 65.31		
	25	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	419.75 266.75		
	7	J1S JACK-OPEN	6.00 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	171.29 122.50		
	2 2 Ply	J2 JACK	12.00 0.00	02-07-00	04-05-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-05-08	44.80 29.32		
	2	J3 JACK-OPEN	9.00 0.00	03-05-08	04-01-06	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 04-01-06	24.40 17.34		
	2	J4 JACK-OPEN	9.00 0.00	03-05-08	02-10-05	2 X 4	2 X 4	01-03-08 -01-08-01	01-06-04 00-03-08	19.64 14.66		
	2	J5 JACK-OPEN	9.00 0.00	01-10-08	02-10-05	2 X 4	2 X 4	01-03-08 -00-01-01	01-06-04 00-03-08	16.02 12.00		

TOTAL # TRUSS= **92.00** TOTAL BFT OF ALL TRUSSES= **3345.11** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5235.60** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **11.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

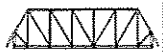
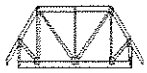
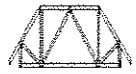
Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288250 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOW SUB-BUILDER:
MODEL: S45-1 HUMMINGBIRD 1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T25S ROOF	12.00 0.00	35-11-00	07-00-12	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	481.46 297.34		
	1	T26S ROOF	12.00 0.00	35-11-00	08-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	174.43 111.17		
	1	T27S ROOF	12.00 0.00	35-11-00	09-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	184.49 117.50		
	1	T28S ROOF	12.00 0.00	35-11-00	10-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	208.50 131.50		
	1	T29S ROOF	12.00 0.00	35-11-00	11-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	213.38 133.00		
	1 2 Ply	T5 HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	430.00 266.00		
	1	T6 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	158.39 100.50		
	1	T7 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	161.26 102.33		
	1	T8 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	177.20 112.83		
	1	T9 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	176.64 111.33		
	3	T10 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	557.13 348.51		
	1	T110 HIP GIRDER	12.00 0.00	30-09-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	205.01 129.67		
	1	T12 HIP	12.00 0.00	30-09-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	137.47 87.17		
	1	T13 HIP	12.00 0.00	30-09-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	138.94 88.00		
	1	T15 HIP GIRDER	12.00 0.00	13-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	68.46 45.67		
	1	T16 HIP	12.00 0.00	13-00-00	04-07-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.33 38.67		
	3	T30 COMMON	12.00 0.00	13-03-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	206.79 132.00		
	1	G30 GABLE	12.00 0.00	13-03-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	70.06 44.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288250 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S45-1 HUMMINGBIRD 1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T19 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	317.85 206.65		
	1	G19 GABLE	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.98 39.33		
	4	T31 COMMON	12.00 0.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	162.96 102.68		
	1	G31 GABLE	12.00 0.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	39.74 25.67		
	2	T31A COMMON	12.00 0.00	07-11-00	05-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-03-08	77.68 50.00		
	6	T20 MONOPITCH	5.00 0.00	06-09-00	04-02-03	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 04-02-03	196.20 131.04		
	7	T23 MONOPITCH	6.00 0.00	02-07-00	02-10-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-10-02	112.77 78.19		
	7	T24 MONOPITCH	6.00 0.00	01-10-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-05-10	91.63 65.31		
	25	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	419.75 266.75		

TOTAL # TRUSS= **82.00** TOTAL BFT OF ALL TRUSSES= **3362.81** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5289.50** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
10	Hangers	LJS26DS	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

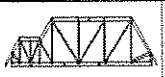
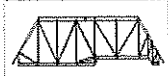
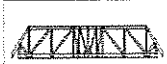
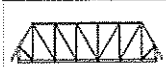
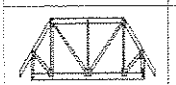
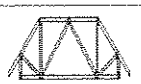
Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288118 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S45-1 HAMMINGBIRD 1 ELEVATION: B + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T25S ROOF	12.00 0.00	35-11-00	07-00-12	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	481.46 297.34		
	1	T26S ROOF	12.00 0.00	35-11-00	08-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	174.43 111.17		
	1	T27S ROOF	12.00 0.00	35-11-00	09-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	184.49 117.50		
	1	T28S ROOF	12.00 0.00	35-11-00	10-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	208.50 131.50		
	1	T29S ROOF	12.00 0.00	35-11-00	11-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	213.38 133.00		
	1 2 Ply	T5S HIP GIRDER	12.00 12.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	454.04 280.66		
	1	T6S HIP	12.00 12.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	184.36 116.33		
	1	T7S HIP	12.00 12.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	191.79 121.50		
	1	T8S HIP	12.00 12.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	199.15 124.50		
	1	T9 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	176.64 111.33		
	3	T10 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	557.13 348.51		
	1	T110 HIP GIRDER	12.00 0.00	30-09-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	205.01 129.67		
	1	T12 HIP	12.00 0.00	30-09-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	137.47 87.17		
	1	T13 HIP	12.00 0.00	30-09-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	138.94 88.00		
	1	T15 HIP GIRDER	12.00 0.00	13-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	68.46 45.67		
	1	T16 HIP	12.00 0.00	13-00-00	04-07-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.33 38.67		
	3	T30 COMMON	12.00 0.00	13-03-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	206.79 132.00		
	1	G30 GABLE	12.00 0.00	13-03-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	70.06 44.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288118 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOW SUB-BUILDER:
MODEL: S45-1 HAMMINGBIRD 1 ELEVATION: B + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	5	T19 COMMON	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	317.85 / 206.65		
	1	G19 GABLE	12.00 / 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	60.98 / 39.33		
	4	T31 COMMON	12.00 / 0.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	162.96 / 102.68		
	1	G31 GABLE	12.00 / 0.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	39.74 / 25.67		
	2	T31A COMMON	12.00 / 0.00	07-11-00	05-10-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 00-03-08	77.68 / 50.00		
	6	T20 MONOPITCH	5.00 / 0.00	06-09-00	04-02-03	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-09 / 04-02-03	196.20 / 131.04		
	7	T23 MONOPITCH	6.00 / 0.00	02-07-00	02-10-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-10-02	112.77 / 78.19		
	7	T24 MONOPITCH	6.00 / 0.00	01-10-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-05-10	91.63 / 65.31		
	18	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	302.22 / 192.06		
	7	J1S JACK-OPEN	6.00 / 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	171.29 / 122.50		

TOTAL # TRUSS= **82.00** TOTAL BFT OF ALL TRUSSES= **3471.95** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5445.75** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
10	Hangers	LJS26DS	

TOTAL # ITEMS= **14.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

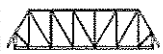
Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288252 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS SUB-BUILDER:
MODEL: S45-1 HUMMINGBIRD 1 ELEVATION: B (REAR UPGRADE)

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T25S ROOF	12.00 0.00	35-11-00	07-00-12	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	481.46 297.34		
	1	T26S ROOF	12.00 0.00	35-11-00	08-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	174.43 111.17		
	1	T27S ROOF	12.00 0.00	35-11-00	09-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	184.49 117.50		
	1	T28S ROOF	12.00 0.00	35-11-00	10-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	208.50 131.50		
	1	T29S ROOF	12.00 0.00	35-11-00	11-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	213.38 133.00		
	1 2 Ply	T5Z HIP GIRDER	12.00 0.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	430.00 266.00		
	1	T6 HIP	12.00 0.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	158.39 100.50		
	1	T7 HIP	12.00 0.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	161.26 102.33		
	1	T8 HIP	12.00 0.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	177.20 112.83		
	1	T9 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	176.64 111.33		
	3	T10 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	557.13 348.51		
	1	T110 HIP GIRDER	12.00 0.00	30-09-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	205.01 129.67		
	1	T12 HIP	12.00 0.00	30-09-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	137.47 87.17		
	1	T13 HIP	12.00 0.00	30-09-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	138.94 88.00		
	3	T30 COMMON	12.00 0.00	13-03-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	206.79 132.00		
	1	G30 GABLE	12.00 0.00	13-03-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	70.06 44.00		
	5	T19 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	317.85 206.65		
	1	G19 GABLE	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.98 39.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288252 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOWES SUB-BUILDER:
MODEL: S45-1 HUMMINGBIRD 1 ELEVATION: B (REAR UPGRADE)

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	T32 COMMON	12.00 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	271.44 173.32		
	1	G32 GABLE	12.00 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	68.66 43.33		
	4	T31 COMMON	12.00 0.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	162.96 102.68		
	1	G31 GABLE	12.00 0.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	39.74 25.67		
	2	T31A COMMON	12.00 0.00	07-11-00	05-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-03-08	77.68 50.00		
	6	T20 MONOPITCH	5.00 0.00	06-09-00	04-02-03	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 04-02-03	196.20 131.04		
	7	T23 MONOPITCH	6.00 0.00	02-07-00	02-10-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-10-02	112.77 78.19		
	7	T24 MONOPITCH	6.00 0.00	01-10-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-05-10	91.63 65.31		
	18	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	302.22 192.06		

TOTAL # TRUSS= **78.00** TOTAL BFT OF ALL TRUSSES= **3420.43** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5383.28** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
11	Hangers	LJS26DS	

TOTAL # ITEMS= **15.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

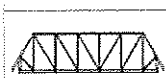
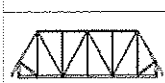
Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288251 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S45-1 HUMMINGBIRD 1 ELEVATION: B (REAR UPGRADE) +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T25S ROOF	12.00 0.00	35-11-00	07-00-12	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	481.46 297.34		
	1	T26S ROOF	12.00 0.00	35-11-00	08-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	174.43 111.17		
	1	T27S ROOF	12.00 0.00	35-11-00	09-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	184.49 117.50		
	1	T28S ROOF	12.00 0.00	35-11-00	10-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	208.50 131.50		
	1	T29S ROOF	12.00 0.00	35-11-00	11-00-12	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	213.38 133.00		
	1 2 Ply	T5S2 HIP GIRDER	12.00 12.00	35-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	454.04 280.66		
	1	T6S HIP	12.00 12.00	35-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	184.36 116.33		
	1	T7S HIP	12.00 12.00	35-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	191.79 121.50		
	1	T8S HIP	12.00 12.00	35-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	199.15 124.50		
	1	T9 HIP	12.00 0.00	35-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	176.64 111.33		
	3	T10 HIP	12.00 0.00	35-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	557.13 348.51		
	1	T110 HIP GIRDER	12.00 0.00	30-09-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	205.01 129.67		
	1	T12 HIP	12.00 0.00	30-09-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	137.47 87.17		
	1	T13 HIP	12.00 0.00	30-09-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	138.94 88.00		
	5	T19 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	317.85 206.65		
	1	G19 GABLE	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	60.98 39.33		
	3	T30 COMMON	12.00 0.00	13-03-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	206.79 132.00		
	1	G30 GABLE	12.00 0.00	13-03-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	70.06 44.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/19/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288251 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S45-1 HUMMINGBIRD 1 ELEVATION: B (REAR UPGRADE) +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	4	T32 COMMON	12.00 / 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	271.44 / 173.32		
	1	G32 GABLE	12.00 / 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	68.66 / 43.33		
	4	T31 COMMON	12.00 / 0.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	162.96 / 102.68		
	1	G31 GABLE	12.00 / 0.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	39.74 / 25.67		
	2	T31A COMMON	12.00 / 0.00	07-11-00	05-10-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 00-03-08	77.68 / 50.00		
	6	T20 MONOPITCH	5.00 / 0.00	06-09-00	04-02-03	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-09 / 04-02-03	196.20 / 131.04		
	7	T23 MONOPITCH	6.00 / 0.00	02-07-00	02-10-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-10-02	112.77 / 78.19		
	7	T24 MONOPITCH	6.00 / 0.00	01-10-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-05-10	91.63 / 65.31		
	11	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	184.69 / 117.37		
	7	J1S JACK-OPEN	6.00 / 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	171.29 / 122.50		

TOTAL # TRUSS= **78.00** TOTAL BFT OF ALL TRUSSES= **3529.57** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5539.53** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
11	Hangers	LJS26DS	

TOTAL # ITEMS= **15.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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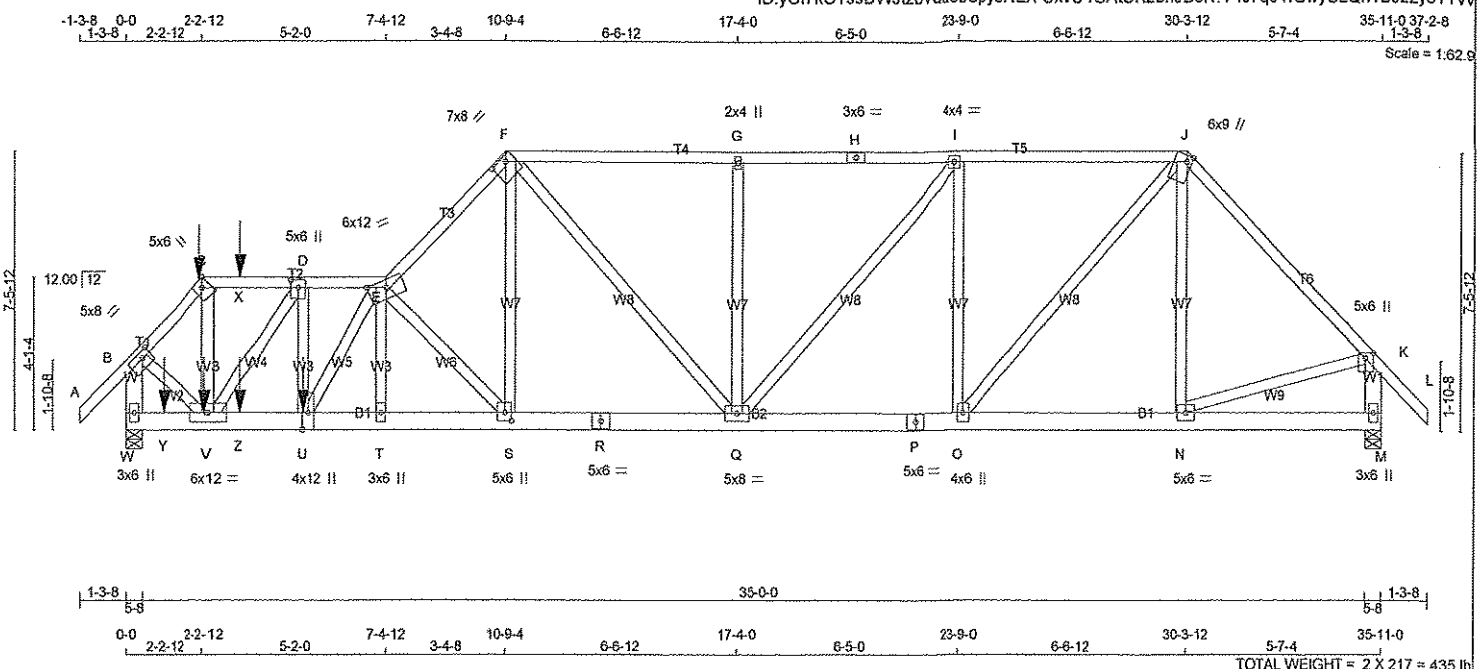

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	SIDE(61.0)
C- E 1	12	SIDE(61.0)
E- F 1	12	TOP
F- H 1	12	TOP
H- J 1	12	TOP
J- L 1	12	TOP
W- B 2	12	TOP
M- K 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
W- R 2	12	SIDE(183.1)
R- P 2	12	TOP
P- M 2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 1	6	
D- U 1	2	SIDE(829.7)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTW+h	MT20	5.0	6.0	Edge	
D	TMWW+t	MT20	5.0	6.0	2.50	2.50
E	TTWWW-m	MT20	6.0	12.0	Edge	
F	TTWW-h	MT20	7.0	8.0	1.75	5.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TTWW+m	MT20	6.0	9.0	Edge	1.75
K	TMVW+p	MT20	5.0	6.0	1.75	2.75
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	6773	0	6773	0	0	5-8	5-8
M	3455	0	3455	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	5234	3515 / 0	864 / 0	0 / 0	0 / 0	855 / 0	0 / 0
M	2676	1788 / 0	448 / 0	0 / 0	0 / 0	439 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM TO	MAX (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 60	-122.2 -122.2	0.09 (1)		10.00	V- C	0 / 3309	0.29 (1)
B- C	-5697 / 0	-122.2 -122.2	0.17 (1)		3.97	V- D	-6502 / 0	0.78 (1)
C- X	-4131 / 0	-122.2 -122.2	0.19 (1)		4.50	U- D	0 / 5001	0.44 (1)
X- D	-4131 / 0	-122.2 -122.2	0.19 (1)		4.50	U- E	-1565 / 0	0.17 (1)
D- E	-7914 / 0	-122.2 -122.2	0.26 (1)		3.32	T- E	0 / 118	0.01 (2)
E- F	-6902 / 0	-122.2 -122.2	0.34 (1)		3.50	E- S	-5613 / 0	0.85 (1)
F- G	-5020 / 0	-122.2 -122.2	0.62 (1)		3.77	S- F	0 / 4270	0.38 (1)
G- H	-5021 / 0	-122.2 -122.2	0.62 (1)		3.75	N- J	-380 / 119	0.13 (1)
H- I	-5021 / 0	-122.2 -122.2	0.62 (1)		3.75	B- V	0 / 4679	0.41 (1)
I- J	-4139 / 0	-122.2 -122.2	0.56 (1)		4.13	N- K	0 / 2363	0.21 (1)
J- K	-3251 / 0	-122.2 -122.2	0.54 (1)		4.64	O- J	0 / 2727	0.24 (1)
K- L	0 / 60	-122.2 -122.2	0.09 (1)		10.00	F- Q	0 / 77	0.01 (1)
W- B	-6788 / 0	0.0 0.0	0.26 (1)		5.72	O- I	-1828 / 0	0.63 (1)
M- K	-3392 / 0	0.0 0.0	0.13 (1)		7.57	Q- G	-844 / 0	0.29 (1)
						Q- I	0 / 1315	0.42 (1)
W- Y	0 / 0	-28.0 -28.0	0.06 (1)		10.00			
Y- V	0 / 0	-28.0 -28.0	0.06 (1)		10.00			
V- Z	0 / 7913	-28.0 -28.0	0.66 (1)		10.00			
Z- U	0 / 7913	-28.0 -28.0	0.66 (1)		10.00			
U- T	0 / 8753	-28.0 -28.0	0.71 (1)		10.00			
T- S	0 / 8750	-28.0 -28.0	0.65 (1)		10.00			
S- R	0 / 4968	-28.0 -28.0	0.36 (1)		10.00			
R- Q	0 / 4968	-28.0 -28.0	0.36 (1)		10.00			
Q- P	0 / 4139	-28.0 -28.0	0.30 (1)		10.00			
P- O	0 / 4139	-28.0 -28.0	0.30 (1)		10.00			
O- N	0 / 2285	-28.0 -28.0	0.18 (1)		10.00			
N- M	0 / 0	-28.0 -28.0	0.07 (3)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-13	-14	---	FRONT	VERT	DEAD
C	2-2-12	-151	-151	---	BACK	VERT	TOTAL
C	2-2-12	-180	-180	---	FRONT	VERT	SNOW
U	5-0-8	-3882	-3882	---	BACK	VERT	TOTAL
V	2-3-8	-40	-70	---	BACK	VERT	TOTAL
X	3-3-8	-147	-147	---	BACK	VERT	TOTAL
Y	1-0-12	-40	-70	---	BACK	VERT	TOTAL
Z	3-3-8	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

[M]

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.62 (F-G:1) , BC=0.71 (T-U:1) , WB=0.85 (E-S:1) , SSI=0.21 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.72 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 42067 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 47468-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T1S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	5.0	6.0	2.75	2.25
T	BMW+w	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	12.0	Edge	
V	BMWWW-t	MT20	6.0	12.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 192.5 lbs FACTORED DOWN AT 2-2-12, AND 150.6 lbs FACTORED DOWN AT 2-2-12, AND 147.1 lbs FACTORED DOWN AT 3-3-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-3-8, AND 69.9 lbs FACTORED DOWN AT 3-3-8, AND 3882.5 lbs FACTORED DOWN AT 5-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47468 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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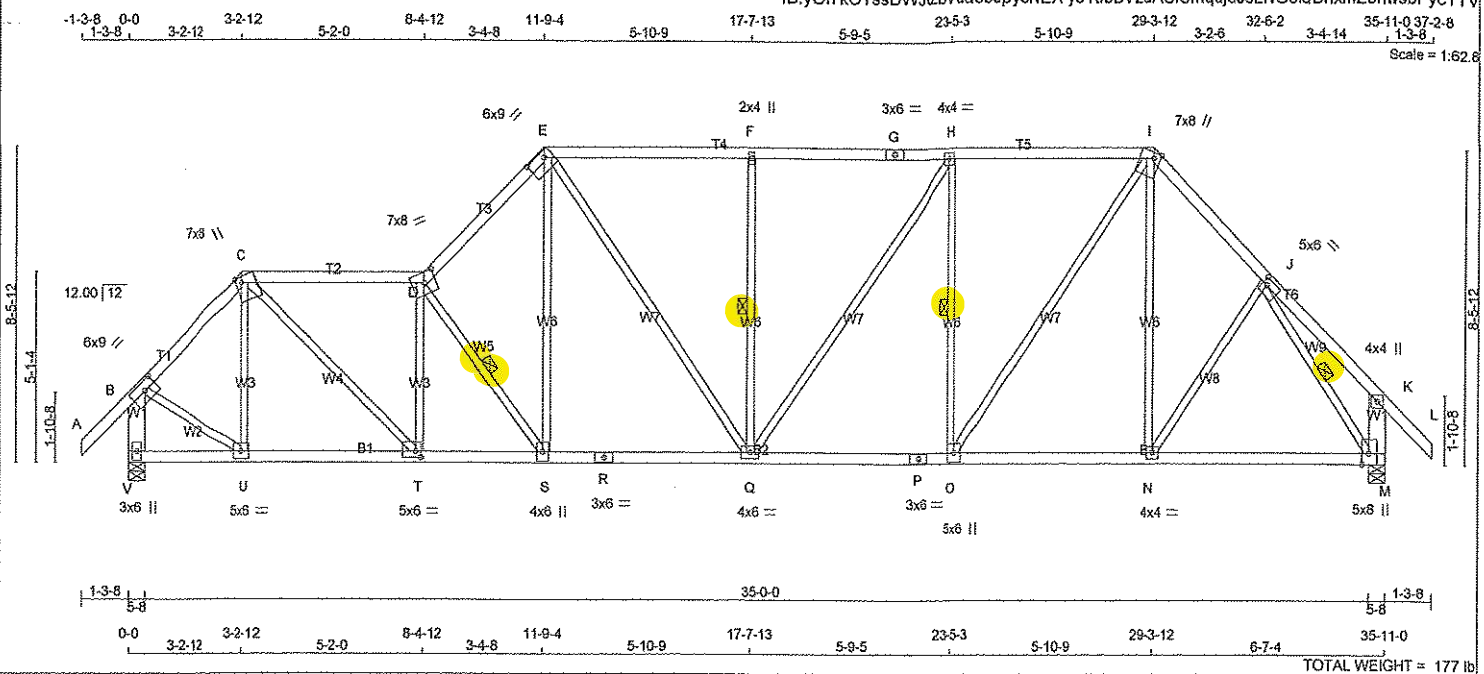

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TTWW-m	MT20	7.0	8.0	3.25	4.00
E	TTWW-h	MT20	6.0	9.0	2.00	6.25
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.50
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	5.0	8.0	Edge	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.00	1.75
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
V	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-S, J-M, H-O, F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B- C	-2398 / 0	-122.2	-122.2	0.31 (1)		4.15
C- D	-3590 / 0	-122.2	-122.2	0.86 (1)		2.76
D- E	-3682 / 0	-122.2	-122.2	0.46 (1)		3.31
E- F	-3015 / 0	-122.2	-122.2	0.83 (1)		3.13
F- G	-3016 / 0	-122.2	-122.2	0.84 (1)		3.11
G- H	-3016 / 0	-122.2	-122.2	0.84 (1)		3.11
H- I	-2740 / 0	-122.2	-122.2	0.80 (1)		3.30
I- J	-2582 / 0	-122.2	-122.2	0.30 (1)		4.05
J- K	0 / 30	-122.2	-122.2	0.19 (1)		10.00
K- L	0 / 60	-122.2	-122.2	0.17 (1)		10.00
V- B	-2843 / 0	0.0	0.0	0.21 (1)		6.22
M- K	-328 / 0	0.0	0.0	0.02 (1)		7.81
V- U	0 / 0	-28.0	-28.0	0.14 (2)		10.00
U- T	0 / 1674	-28.0	-28.0	0.37 (1)		10.00
T- S	0 / 3627	-28.0	-28.0	0.65 (1)		10.00
S- R	0 / 2624	-28.0	-28.0	0.53 (1)		10.00
R- Q	0 / 2624	-28.0	-28.0	0.53 (1)		10.00
Q- P	0 / 2740	-28.0	-28.0	0.55 (1)		10.00
P- O	0 / 2740	-28.0	-28.0	0.55 (1)		10.00
O- N	0 / 1796	-28.0	-28.0	0.47 (2)		10.00
N- M	0 / 1679	-28.0	-28.0	0.45 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-619 / 0	0.24 (1)
C- T	0 / 2635	0.59 (1)
T- D	-1719 / 0	0.66 (1)
D- S	-1740 / 0	0.44 (1)
S- E	0 / 1547	0.35 (1)
N- I	0 / 212	0.05 (3)
N- J	0 / 210	0.05 (2)
B- U	0 / 1856	0.42 (1)
J- M	-2982 / 0	0.71 (1)
O- I	0 / 1611	0.36 (1)
E- Q	0 / 671	0.15 (1)
O- H	-1159 / 0	0.53 (1)
Q- F	-764 / 0	0.35 (1)
Q- H	0 / 478	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.86 (C-D:1) , BC=0.65 (S-T:1) , WB=0.71 (J-M:1) , SSI=0.33 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.74 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47469-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
288116	T3S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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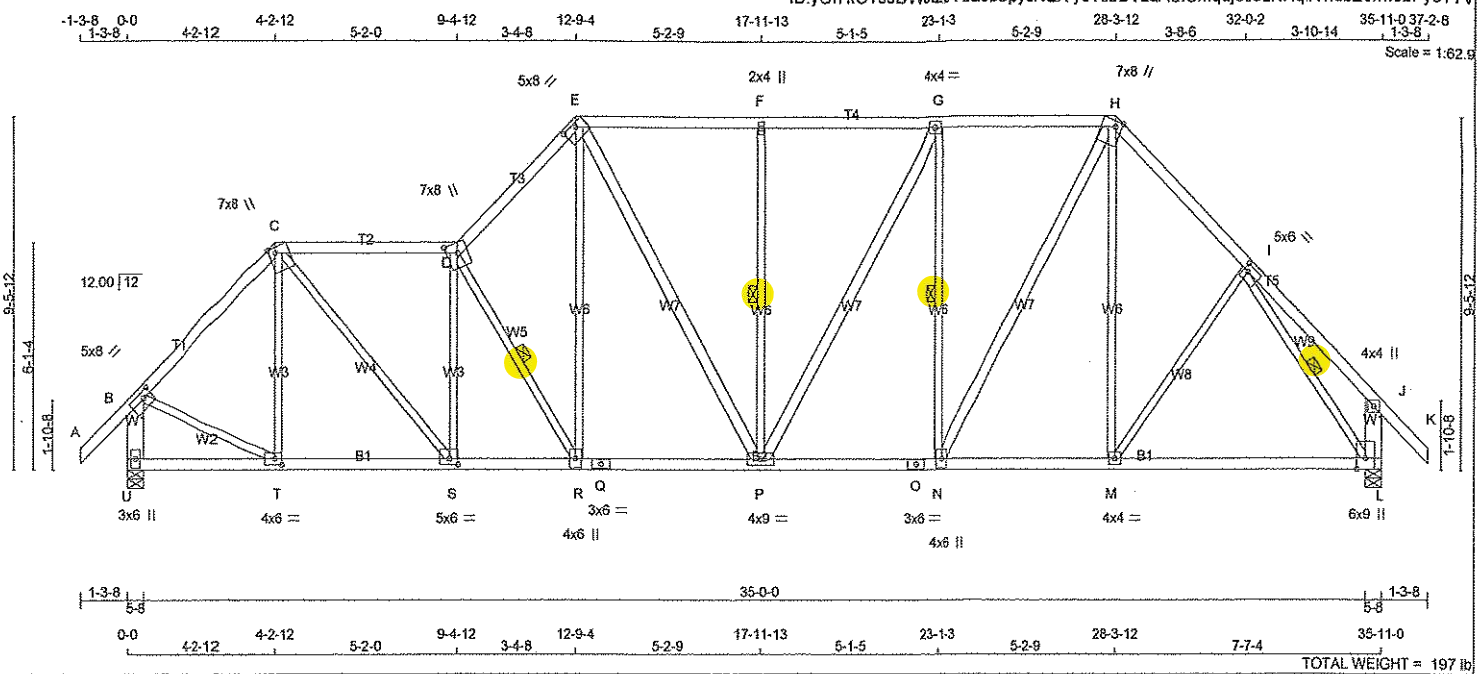

TOTAL WEIGHT = 197 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TTWW+m	MT20	7.0	8.0	3.25	3.50
E	TTWW-h	MT20	5.0	8.0	1.25	4.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0	2.25	1.50
J	TMV+p	MT20	4.0	4.0		
L	BMVW1+p	MT20	6.0	9.0	Edge	
M	BMWW-t	MT20	4.0	4.0		
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW+t	MT20	4.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.00	2.75
T	BMWW-t	MT20	4.0	6.0	2.00	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2868	0	2868	0	0	5-8	5-8
L	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
L	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-R, I-L, G-N, F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	1	0.17 (1)	10.00
B- C	-2503 / 0	-122.2	-122.2	1	0.56 (1)	3.83
C- D	-3209 / 0	-122.2	-122.2	1	0.81 (1)	2.98
D- E	-3410 / 0	-122.2	-122.2	1	0.43 (1)	3.45
E- F	-2677 / 0	-122.2	-122.2	1	0.60 (1)	3.61
F- G	-2677 / 0	-122.2	-122.2	1	0.60 (1)	3.59
G- H	-2459 / 0	-122.2	-122.2	1	0.59 (1)	3.75
H- I	-2564 / 0	-122.2	-122.2	1	0.41 (1)	3.96
I- J	0 / 36	-122.2	-122.2	1	0.27 (1)	10.00
J- K	0 / 60	-122.2	-122.2	1	0.17 (1)	10.00
U- B	-2824 / 0	0.0	0.0	1	0.21 (1)	6.23
L- J	-349 / 0	0.0	0.0	1	0.03 (1)	7.81
U- T	0 / 0	-28.0	-28.0	1	0.16 (2)	10.00
T- S	0 / 1755	-28.0	-28.0	1	0.38 (1)	10.00
S- R	0 / 3237	-28.0	-28.0	1	0.56 (1)	10.00
R- Q	0 / 2428	-28.0	-28.0	1	0.48 (1)	10.00
Q- P	0 / 2428	-28.0	-28.0	1	0.48 (1)	10.00
P- O	0 / 2459	-28.0	-28.0	1	0.49 (1)	10.00
O- N	0 / 2459	-28.0	-28.0	1	0.49 (1)	10.00
N- M	0 / 1783	-28.0	-28.0	1	0.54 (2)	10.00
M- L	0 / 1729	-28.0	-28.0	1	0.53 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
T- C	-459 / 28	0.27 (1)
C- S	0 / 2268	0.50 (1)
S- D	-1561 / 0	0.91 (1)
D- R	-1602 / 0	0.49 (1)
R- E	0 / 1493	0.34 (1)
M- H	0 / 269	0.06 (3)
M- I	0 / 149	0.03 (2)
B- T	0 / 1869	0.42 (1)
I- L	-2983 / 0	0.84 (1)
N- H	0 / 1367	0.22 (1)
E- P	0 / 504	0.08 (1)
N- G	-1072 / 0	0.64 (1)
P- F	-674 / 0	0.40 (1)
P- G	0 / 450	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.81 (C-D:1) , BC=0.56 (R-S:1) , WB=0.91 (D-S:1) , SSI=0.30 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (T) (INPUT = 0.90)
SI METAL= 0.75 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM41490-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T4S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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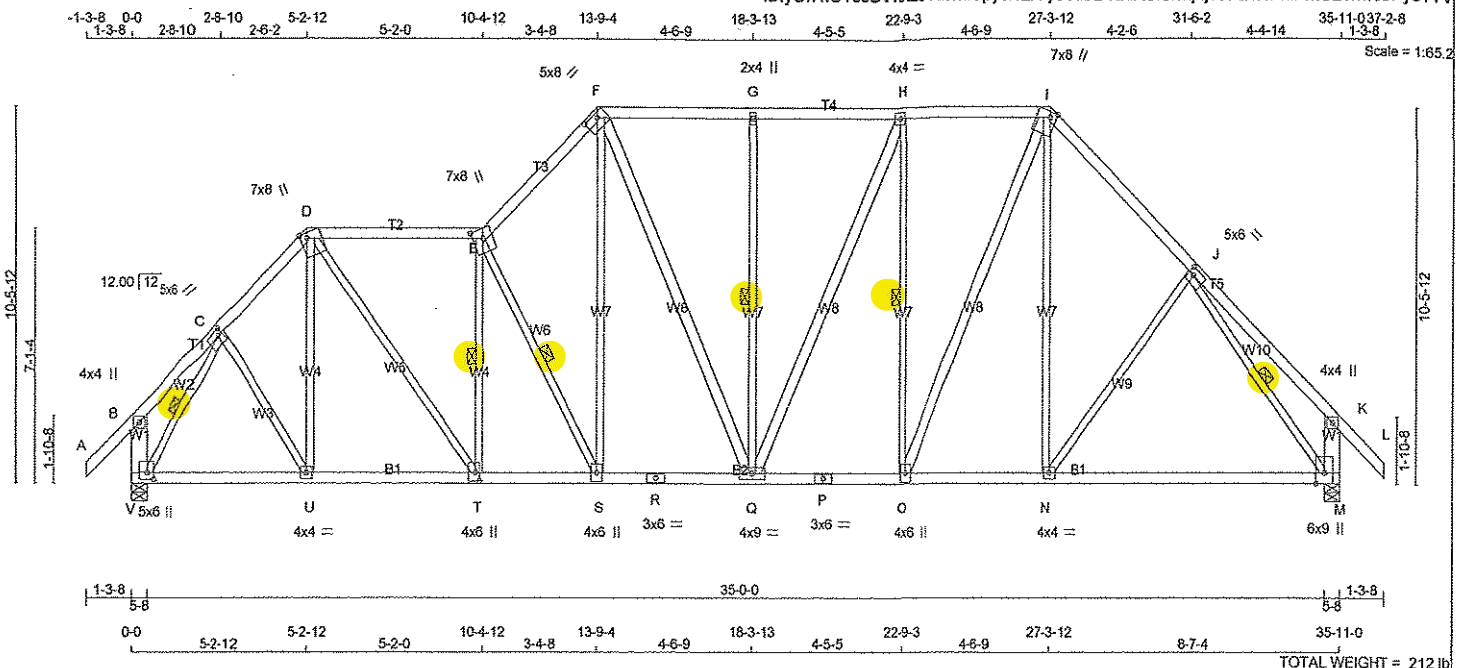

TOTAL WEIGHT = 212 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
F - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TTWW+m	MT20	7.0	8.0	3.25	3.50
F	TTWW-h	MT20	5.0	8.0	1.50	4.75
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	6.0	9.0	Edge	
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW+t	MT20	4.0	6.0	2.75	1.50
U	BMWW-t	MT20	4.0	4.0		
V	BMVW1+p	MT20	5.0	6.0	2.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-T, E-S, C-V, J-M, H-O, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B- C	-2 / 19	-122.2	-122.2	0.10 (1)		10.00
C- D	-2569 / 0	-122.2	-122.2	0.19 (1)		4.16
D- E	-2918 / 0	-122.2	-122.2	0.77 (1)		3.16
E- F	-3171 / 0	-122.2	-122.2	0.41 (1)		3.59
F- G	-2405 / 0	-122.2	-122.2	0.42 (1)		4.00
G- H	-2405 / 0	-122.2	-122.2	0.43 (1)		3.98
H- I	-2231 / 0	-122.2	-122.2	0.42 (1)		4.12
I- J	-2532 / 0	-122.2	-122.2	0.54 (1)		3.85
J- K	0 / 43	-122.2	-122.2	0.36 (1)		10.00
K- L	0 / 60	-122.2	-122.2	0.17 (1)		10.00
V- B	-300 / 0	0.0	0.0	0.02 (1)		7.81
M- K	-370 / 0	0.0	0.0	0.03 (1)		7.81
V- U	0 / 1575	-28.0	-28.0	0.37 (1)		10.00
U- T	0 / 1791	-28.0	-28.0	0.40 (1)		10.00
T- S	0 / 2939	-28.0	-28.0	0.51 (1)		10.00
S- R	0 / 2257	-28.0	-28.0	0.44 (1)		10.00
R- Q	0 / 2257	-28.0	-28.0	0.44 (1)		10.00
Q- P	0 / 2232	-28.0	-28.0	0.44 (1)		10.00
P- O	0 / 2232	-28.0	-28.0	0.44 (1)		10.00
O- N	0 / 1757	-28.0	-28.0	0.64 (2)		10.00
N- M	0 / 1766	-28.0	-28.0	0.63 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	0 / 389	0.09 (1)
U- D	-149 / 111	0.13 (1)
D- T	0 / 1884	0.42 (1)
T- E	-1405 / 0	0.45 (1)
E- S	-1528 / 0	0.59 (1)
S- F	0 / 1463	0.33 (1)
N- I	0 / 358	0.08 (2)
N- J	-19 / 126	0.03 (3)
V- C	-2962 / 0	0.58 (1)
J- M	-2977 / 0	1.00 (1)
O- I	0 / 1161	0.19 (1)
F- Q	0 / 362	0.06 (1)
O- H	-986 / 0	0.76 (1)
Q- G	-582 / 0	0.45 (1)
Q- H	0 / 433	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.77 (D-E:1) , BC=0.64 (N-O:2) , WB=1.00 (J-M:1) , SSI=0.26 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 0.74 (J) (INPUT = 1.00)

DWG NO. TAM 4747 / w 17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T5S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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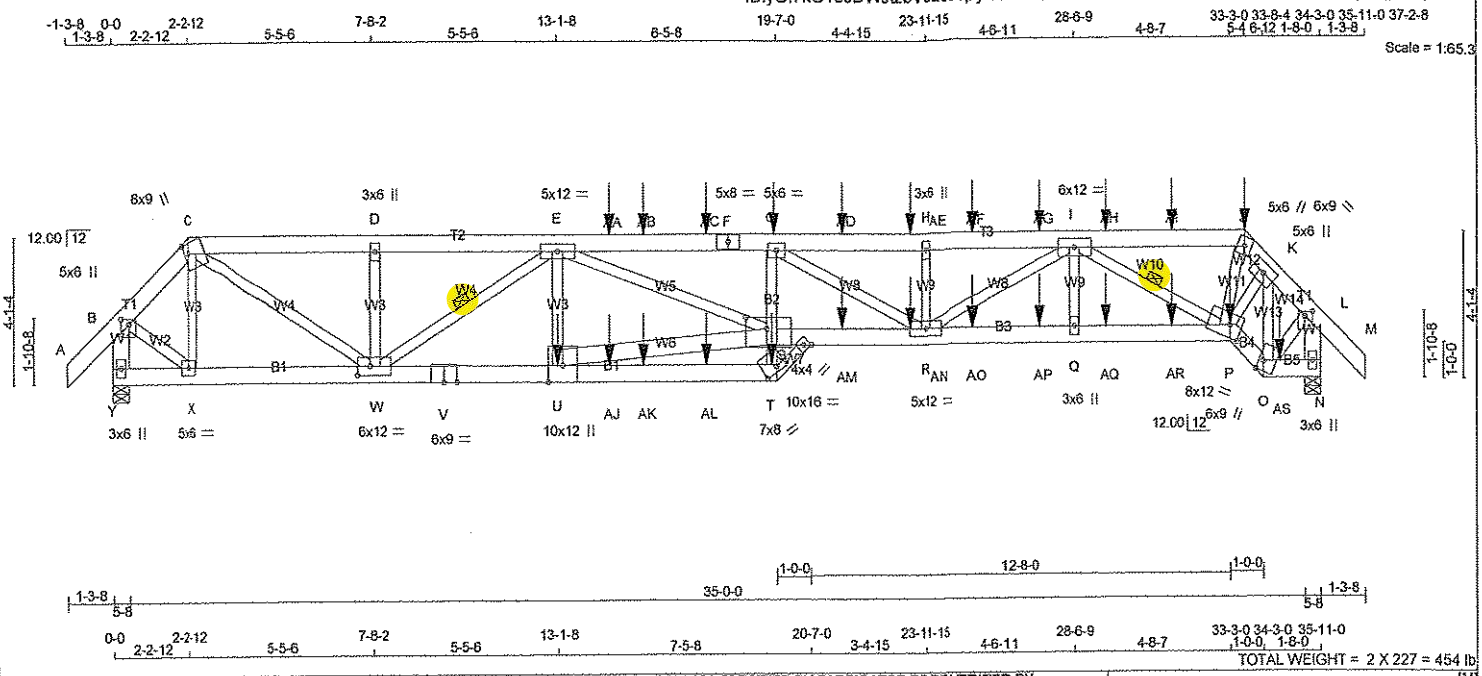

TOTAL WEIGHT = 2 X 227 = 454 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	1650F 1.5E	SPF
F - J	2x6	DRY	1650F 1.5E	SPF
J - M	2x6	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	1650F 1.5E	SPF
V - T	2x6	DRY	1650F 1.5E	SPF
T - G	2x4	DRY	No.2	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	SIDE(0.0)
F- J	2	12	SIDE(183.1)
J- M	2	12	SIDE(0.0)
Y- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Y- V	2	12	TOP
V- T	2	12	SIDE(183.1)
S- P	2	12	SIDE(0.0)
P- O	2	12	SIDE(0.0)
O- N	2	12	SIDE(122.0)
T- G	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
E- U	1	4	SIDE(446.2)
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTVW+m	MT20	8.0	9.0	3.25	1.75
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	12.0		
F	TS-t	MT20	5.0	8.0		
G	TMVW-t	MT20	5.0	6.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWWW-t	MT20	6.0	12.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	5268	0	5268	0	0	5-8	5-8
N	5819	0	5819	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	4073	2732 / 0	676 / 0	0 / 0	0 / 0	666 / 0	0 / 0
N	4536	2977 / 0	793 / 0	0 / 0	0 / 0	766 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W, I-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00
B- C	-4541 / 0	-122.2	-122.2	0.05 (1)	5.38
C- D	-9596 / 0	-122.2	-122.2	0.17 (1)	4.39
D- E	-9596 / 0	-122.2	-122.2	0.20 (1)	4.37
E-AA	-20064 / 0	-122.2	-122.2	0.56 (1)	2.91
AA-AB	-20064 / 0	-122.2	-122.2	0.56 (1)	2.91
AB-AC	-20064 / 0	-122.2	-122.2	0.56 (1)	2.91
AC- F	-20064 / 0	-122.2	-122.2	0.56 (1)	2.91
F- G	-20064 / 0	-122.2	-122.2	0.56 (1)	2.91
G-AD	-17496 / 0	-122.2	-122.2	0.35 (1)	3.27
AD-AE	-17496 / 0	-122.2	-122.2	0.35 (1)	3.27
AE- H	-17496 / 0	-122.2	-122.2	0.35 (1)	3.27
H-AF	-17496 / 0	-122.2	-122.2	0.33 (1)	3.29
AF-AG	-17496 / 0	-122.2	-122.2	0.33 (1)	3.29
AG- I	-17496 / 0	-122.2	-122.2	0.33 (1)	3.29
I-AH	-5489 / 0	-122.2	-122.2	0.15 (1)	5.47
AH-AI	-5489 / 0	-122.2	-122.2	0.15 (1)	5.47
AI- J	-5489 / 0	-122.2	-122.2	0.15 (1)	5.47
J- K	-6562 / 0	-122.2	-122.2	0.25 (1)	4.42
K- L	-4376 / 0	-122.2	-122.2	0.22 (1)	5.25
L- M	0 / 63	-122.2	-122.2	0.05 (1)	10.00
Y- B	-5326 / 0	0.0	0.0	0.20 (1)	6.34
N- L	-5782 / 0	0.0	0.0	0.21 (1)	6.13
Y- X	0 / 0	-28.0	-28.0	0.06 (1)	10.00
X- W	0 / 3116	-28.0	-28.0	0.18 (1)	10.00
W- V	0 / 14900	-28.0	-28.0	0.65 (1)	10.00
V- U	0 / 14900	-28.0	-28.0	0.65 (1)	10.00
U-AJ	0 / 580	-28.0	-28.0	0.11 (2)	10.00
AJ-AK	0 / 580	-28.0	-28.0	0.11 (2)	10.00
AK-AL	0 / 580	-28.0	-28.0	0.11 (2)	10.00
AL- T	0 / 580	-28.0	-28.0	0.11 (2)	10.00
T- S	0 / 311	0.0	0.0	0.46 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-1506 / 0	0.13 (1)
C- W	0 / 7736	0.68 (1)
W- D	-555 / 2	0.05 (1)
W- E	-6409 / 0	0.67 (1)
U- E	0 / 427	0.04 (2)
U- S	0 / 14503	0.94 (1)
E- S	0 / 5595	0.49 (1)
B- X	0 / 3639	0.32 (1)
G- R	-3227 / 0	0.51 (1)
Q- I	0 / 474	0.04 (2)
R- H	-752 / 0	0.05 (1)
R- I	0 / 5450	0.48 (1)
P- J	0 / 3723	0.33 (1)
I- P	-8334 / 0	0.67 (1)
O- K	-4765 / 0	0.33 (1)
O- L	0 / 3623	0.32 (1)
P- K	0 / 4146	0.37 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 688 (0.63")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 453 (0.95")

CSI: TC=0.56 (E-G:1) , BC=0.91 (R-S:1) , WB=0.94 (S-U:1) , SSI=0.25 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

TAM No. 47472-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T5S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
J	TTW+m	MT20	5.0	6.0		
K	TMWW-t	MT20	6.0	9.0		
L	TMWW+p	MT20	5.0	6.0	1.50	2.75
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	6.0	9.0	3.50	1.50
P	BBWWW-m	MT20	8.0	12.0	4.25	4.25
Q	BMW+w	MT20	3.0	6.0		
R	BMWWW-t	MT20	5.0	12.0		
S	BVMWW-t	MT20	10.0	16.0	3.75	7.50
T						
T						
T	BVW*-m	MT20	7.0	8.0	1.75	5.00
U	BMWW+t	MT20	10.0	12.0	Edge	
V	BS-t	MT20	6.0	9.0		
W	BMWWW-t	MT20	6.0	12.0	3.00	4.50
X	BMWW-t	MT20	5.0	6.0		
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+w	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 317.2 lbs FACTORED DOWN AT 33-8-4, 147.1 lbs FACTORED DOWN AT 14-7-8, 147.1 lbs FACTORED DOWN AT 15-7-8, 147.1 lbs FACTORED DOWN AT 17-6-0, 147.1 lbs FACTORED DOWN AT 19-6-0, 118.7 lbs FACTORED DOWN AT 21-6-0, 118.7 lbs FACTORED DOWN AT 23-6-0, 118.7 lbs FACTORED DOWN AT 25-6-0, 118.7 lbs FACTORED DOWN AT 27-6-0, AND 118.7 lbs FACTORED DOWN AT 29-6-0, AND 118.7 lbs FACTORED DOWN AT 31-6-0 ON TOP CHORD, AND 2412.2 lbs FACTORED DOWN AT 13-1-8, 69.9 lbs FACTORED DOWN AT 14-7-8, 69.9 lbs FACTORED DOWN AT 15-7-8, 69.9 lbs FACTORED DOWN AT 17-6-0, 92.9 lbs FACTORED DOWN AT 19-5-4, 153.2 lbs FACTORED DOWN AT 21-6-0, 153.2 lbs FACTORED DOWN AT 23-6-0, 153.2 lbs FACTORED DOWN AT 25-6-0, 153.2 lbs FACTORED DOWN AT 27-6-0, 153.2 lbs FACTORED DOWN AT 29-6-0, 153.2 lbs FACTORED DOWN AT 31-6-0, AND 167.2 lbs FACTORED DOWN AT 33-3-0, AND 82.7 lbs FACTORED DOWN AT 34-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)
S- G	0 / 586	0.0	0.0	0.49 (1)	10.00			
S-AM	0 / 20287	-28.0	-28.0	0.91 (1)	10.00			
AM-AN	0 / 20287	-28.0	-28.0	0.91 (1)	10.00			
AN- R	0 / 20287	-28.0	-28.0	0.91 (1)	10.00			
R-AO	0 / 12784	-28.0	-28.0	0.61 (1)	10.00			
AO-AP	0 / 12784	-28.0	-28.0	0.61 (1)	10.00			
AP- Q	0 / 12784	-28.0	-28.0	0.61 (1)	10.00			
Q-AQ	0 / 12784	-28.0	-28.0	0.60 (1)	10.00			
AQ-AR	0 / 12784	-28.0	-28.0	0.60 (1)	10.00			
AR- P	0 / 12784	-28.0	-28.0	0.60 (1)	10.00			
P- O	0 / 3719	-28.0	-28.0	0.24 (1)	10.00			
O-AS	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
AS- N	0 / 0	-28.0	-28.0	0.02 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	19-6-0	-147	-147	—	FRONT	VERT	TOTAL
J	33-8-4	-13	-14	—	FRONT	VERT	DEAD
J	33-8-4	-122	-122	—	FRONT	VERT	TOTAL
J	33-8-4	-183	-183	—	FRONT	VERT	SNOW
P	33-3-0	-167	-167	—	FRONT	VERT	TOTAL
T	19-5-4	-53	-93	—	FRONT	VERT	TOTAL
U	13-1-8	-2412	-2412	—	FRONT	VERT	TOTAL
AA	14-7-8	-147	-147	—	FRONT	VERT	TOTAL
AB	15-7-8	-147	-147	—	FRONT	VERT	TOTAL
AC	17-6-0	-147	-147	—	FRONT	VERT	TOTAL
AD	21-6-0	-119	-119	—	FRONT	VERT	TOTAL
AE	23-6-0	-119	-119	—	FRONT	VERT	TOTAL
AF	25-6-0	-119	-119	—	FRONT	VERT	TOTAL
AG	27-6-0	-119	-119	—	FRONT	VERT	TOTAL
AH	29-6-0	-119	-119	—	FRONT	VERT	TOTAL
AI	31-6-0	-119	-119	—	FRONT	VERT	TOTAL
AJ	14-7-8	-40	-70	—	FRONT	VERT	TOTAL
AK	15-7-8	-40	-70	—	FRONT	VERT	TOTAL
AL	17-6-0	-40	-70	—	FRONT	VERT	TOTAL
AM	21-6-0	-153	-153	—	FRONT	VERT	TOTAL
AN	23-6-0	-153	-153	—	FRONT	VERT	TOTAL
AO	25-6-0	-153	-153	—	FRONT	VERT	TOTAL
AP	27-6-0	-153	-153	—	FRONT	VERT	TOTAL
AQ	29-6-0	-153	-153	—	FRONT	VERT	TOTAL
AR	31-6-0	-153	-153	—	FRONT	VERT	TOTAL
AS	34-8-12	-47	-83	—	FRONT	VERT	TOTAL

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.97 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-47492-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288251	T5S2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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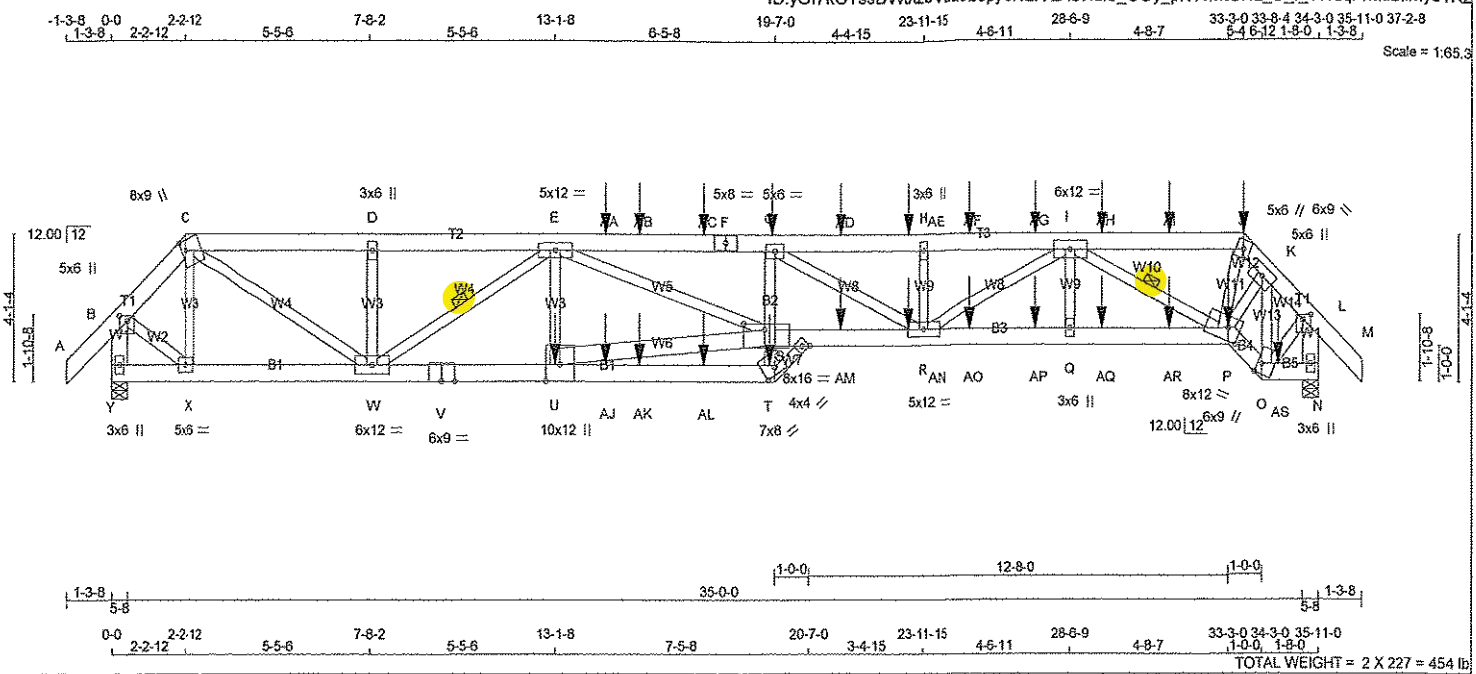


TOTAL WEIGHT = 2 X 227 = 454 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	1650F 1.5E	SPF
F - J	2x6	DRY	1650F 1.5E	SPF
J - M	2x6	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	1650F 1.5E	SPF
V - T	2x6	DRY	1650F 1.5E	SPF
T - G	2x4	DRY	No.2	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	SIDE(0.0)
F- J	2	12	SIDE(183.1)
J- M	2	12	SIDE(0.0)
Y- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Y- V	2	12	TOP
V- T	2	12	SIDE(183.1)
S- P	2	12	SIDE(0.0)
P- O	2	12	SIDE(0.0)
O- N	2	12	SIDE(122.0)
T- G	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
E- U	1	4	SIDE(437.0)
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-6 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	3.25	1.75
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	12.0		
F	TS-t	MT20	5.0	8.0		
G	TMVW-t	MT20	5.0	6.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWWW-t	MT20	6.0	12.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	5246	0	5246	0	0	5-8	5-8
N	5806	0	5806	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD 0.19

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	4053	2724 / 0	669 / 0	0 / 0	0 / 0	660 / 0	0 / 0
N	4524	2973 / 0	789 / 0	0 / 0	0 / 0	762 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-W, I-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00		X- C	-1499 / 0	0.13 (1)
B- C	-4520 / 0	-122.2	-122.2	0.05 (1)	5.38		C- W	0 / 7656	0.68 (1)
C- D	-9549 / 0	-122.2	-122.2	0.17 (1)	4.41		W- D	-556 / 2	0.05 (1)
D- E	-9549 / 0	-122.2	-122.2	0.20 (1)	4.38		W- E	-6370 / 0	0.67 (1)
E-AA	-19985 / 0	-122.2	-122.2	0.56 (1)	2.92		U- E	0 / 405	0.04 (2)
AA-AB	-19985 / 0	-122.2	-122.2	0.56 (1)	2.92		U- S	0 / 14425	0.94 (1)
AB-AC	-19985 / 0	-122.2	-122.2	0.56 (1)	2.92		E- S	0 / 5596	0.49 (1)
AC- F	-19985 / 0	-122.2	-122.2	0.56 (1)	2.92		B- X	0 / 3619	0.32 (1)
F- G	-19985 / 0	-122.2	-122.2	0.56 (1)	2.92		G- R	-3201 / 0	0.51 (1)
G-AD	-17439 / 0	-122.2	-122.2	0.34 (1)	3.28		Q- I	0 / 475	0.04 (2)
AD-AE	-17439 / 0	-122.2	-122.2	0.34 (1)	3.28		R- H	-752 / 0	0.05 (1)
AE- H	-17439 / 0	-122.2	-122.2	0.34 (1)	3.28		R- I	0 / 5424	0.48 (1)
H-AF	-17439 / 0	-122.2	-122.2	0.33 (1)	3.30		P- J	0 / 3712	0.33 (1)
AF-AG	-17439 / 0	-122.2	-122.2	0.33 (1)	3.30		I- P	-8309 / 0	0.67 (1)
AG- I	-17439 / 0	-122.2	-122.2	0.33 (1)	3.30		O- K	-4754 / 0	0.33 (1)
I-AH	-5476 / 0	-122.2	-122.2	0.15 (1)	5.47		O- L	0 / 3614	0.32 (1)
AH-AI	-5476 / 0	-122.2	-122.2	0.15 (1)	5.47		P- K	0 / 4136	0.37 (1)
AI- J	-5476 / 0	-122.2	-122.2	0.15 (1)	5.47				
J- K	-6547 / 0	-122.2	-122.2	0.25 (1)	4.43				
K- L	-4366 / 0	-122.2	-122.2	0.22 (1)	5.26				
L- M	0 / 63	-122.2	-122.2	0.05 (1)	10.00				
Y- B	-5304 / 0	0.0	0.0	0.20 (1)	6.35				
N- L	-5769 / 0	0.0	0.0	0.21 (1)	6.13				
Y- X	0 / 0	-28.0	-28.0	0.06 (1)	10.00				
X- W	0 / 3102	-28.0	-28.0	0.18 (1)	10.00				
W- V	0 / 14821	-28.0	-28.0	0.65 (1)	10.00				
V- U	0 / 14821	-28.0	-28.0	0.65 (1)	10.00				
U-AJ	0 / 578	-28.0	-28.0	0.11 (2)	10.00				
AJ-AK	0 / 578	-28.0	-28.0	0.11 (2)	10.00				
AK-AL	0 / 578	-28.0	-28.0	0.11 (2)	10.00				
AL- T	0 / 578	-28.0	-28.0	0.11 (2)	10.00				
T- S	0 / 311	0.0	0.0	0.46 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 691 (0.62")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 455 (0.95")

CSI: TC=0.56 (E-G:1) , BC=0.91 (R-S:1) , WB=0.94 (S-U:1) , SSI=0.25 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LICENSED PROFESSIONAL ENGINEER 92077 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 47509 -17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288251	T5S2	1	2	TRUSS DESC.	

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
J	TTW+m	MT20	5.0	6.0		
K	TMWW-t	MT20	6.0	9.0		
L	TMVW+p	MT20	5.0	6.0	1.50	2.75
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	6.0	9.0	3.50	1.50
P	BBWWW-m	MT20	8.0	12.0	4.25	4.25
Q	BMW+w	MT20	3.0	6.0		
R	BMWWW-t	MT20	5.0	12.0		
S	BVMWW-I	MT20	8.0	16.0	2.00	7.50
T						
T						
T	BVW*-m	MT20	7.0	8.0	2.25	4.25
U	BMWW+t	MT20	10.0	12.0	Edge	
V	BS-t	MT20	6.0	9.0		
W	BMWWW-t	MT20	6.0	12.0		
X	BMWW-t	MT20	5.0	6.0		
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+w	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 317.2 lbs FACTORED DOWN AT 33-8-4, 147.1 lbs FACTORED DOWN AT 14-7-8, 147.1 lbs FACTORED DOWN AT 15-7-8, 147.1 lbs FACTORED DOWN AT 17-6-0, 147.1 lbs FACTORED DOWN AT 19-6-0, 118.7 lbs FACTORED DOWN AT 21-6-0, 118.7 lbs FACTORED DOWN AT 23-6-0, 118.7 lbs FACTORED DOWN AT 25-6-0, 118.7 lbs FACTORED DOWN AT 27-6-0, AND 118.7 lbs FACTORED DOWN AT 29-6-0, AND 118.7 lbs FACTORED DOWN AT 31-6-0 ON TOP CHORD, AND 2377.1 lbs FACTORED DOWN AT 13-1-8, 69.9 lbs FACTORED DOWN AT 14-7-8, 69.9 lbs FACTORED DOWN AT 15-7-8, 69.9 lbs FACTORED DOWN AT 17-6-0, 92.9 lbs FACTORED DOWN AT 19-5-4, 153.2 lbs FACTORED DOWN AT 21-6-0, 153.2 lbs FACTORED DOWN AT 23-6-0, 153.2 lbs FACTORED DOWN AT 25-6-0, 153.2 lbs FACTORED DOWN AT 27-6-0, 153.2 lbs FACTORED DOWN AT 29-6-0, 153.2 lbs FACTORED DOWN AT 31-6-0, AND 167.2 lbs FACTORED DOWN AT 33-3-0, AND 82.7 lbs FACTORED DOWN AT 34-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
S- G	0 / 572	0.0	0.0	0.49 (1)	10.00
S-AM	0 / 20207	-28.0	-28.0	0.91 (1)	10.00
AM-AN	0 / 20207	-28.0	-28.0	0.91 (1)	10.00
AN- R	0 / 20207	-28.0	-28.0	0.91 (1)	10.00
R-AO	0 / 12748	-28.0	-28.0	0.61 (1)	10.00
AO-AP	0 / 12748	-28.0	-28.0	0.61 (1)	10.00
AP- Q	0 / 12748	-28.0	-28.0	0.61 (1)	10.00
Q-AQ	0 / 12749	-28.0	-28.0	0.59 (1)	10.00
AQ-AR	0 / 12749	-28.0	-28.0	0.59 (1)	10.00
AR- P	0 / 12749	-28.0	-28.0	0.59 (1)	10.00
P- O	0 / 3710	-28.0	-28.0	0.24 (1)	10.00
O-AS	0 / 0	-28.0	-28.0	0.02 (2)	10.00
AS- N	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	19-6-0	-147	-147	---	FRONT	VERT	TOTAL
J	33-8-4	-13	-14	---	FRONT	VERT	DEAD
J	33-8-4	-122	-122	---	FRONT	VERT	TOTAL
J	33-8-4	-183	-183	---	FRONT	VERT	SNOW
P	33-3-0	-167	-167	---	FRONT	VERT	TOTAL
T	19-5-4	-53	-93	---	FRONT	VERT	TOTAL
U	13-1-8	-2377	-2377	---	FRONT	VERT	TOTAL
AA	14-7-8	-147	-147	---	FRONT	VERT	TOTAL
AB	15-7-8	-147	-147	---	FRONT	VERT	TOTAL
AC	17-6-0	-147	-147	---	FRONT	VERT	TOTAL
AD	21-6-0	-119	-119	---	FRONT	VERT	TOTAL
AE	23-6-0	-119	-119	---	FRONT	VERT	TOTAL
AF	25-6-0	-119	-119	---	FRONT	VERT	TOTAL
AG	27-6-0	-119	-119	---	FRONT	VERT	TOTAL
AH	29-6-0	-119	-119	---	FRONT	VERT	TOTAL
AI	31-6-0	-119	-119	---	FRONT	VERT	TOTAL
AJ	14-7-8	-40	-70	---	FRONT	VERT	TOTAL
AK	15-7-8	-40	-70	---	FRONT	VERT	TOTAL
AL	17-6-0	-40	-70	---	FRONT	VERT	TOTAL
AM	21-6-0	-153	-153	---	FRONT	VERT	TOTAL
AN	23-6-0	-153	-153	---	FRONT	VERT	TOTAL
AO	25-6-0	-153	-153	---	FRONT	VERT	TOTAL
AP	27-6-0	-153	-153	---	FRONT	VERT	TOTAL
AQ	29-6-0	-153	-153	---	FRONT	VERT	TOTAL
AR	31-6-0	-153	-153	---	FRONT	VERT	TOTAL
AS	34-8-12	-47	-83	---	FRONT	VERT	TOTAL

JSI GRIP= 0.90 (W) (INPUT = 0.90)
JSI METAL= 1.00 (S) (INPUT = 1.00)

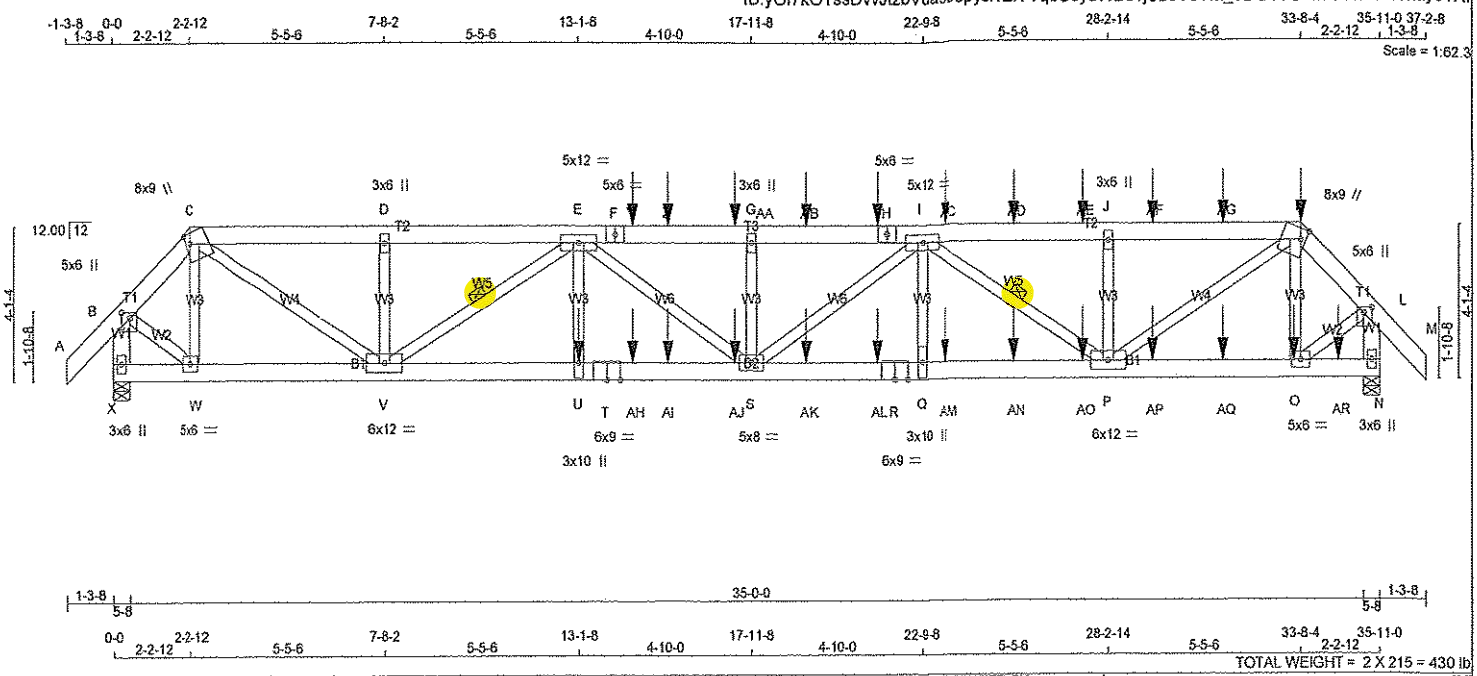
LICENSED PROFESSIONAL ENGINEER
92077
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47509-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288117	T5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:yOi7kOTssDWJtzbVua5b3pycNEX-VqbU3ydWBu1jSboVoVm_eDGVvGuhLJmx91W6kayc1Rf



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - T	2x6	DRY	1650F 1.5E	SPF
T - R	2x6	DRY	1650F 1.5E	SPF
R - N	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- H	2	12	SIDE(0.0)
H- K	2	12	SIDE(61.0)
K- M	2	12	SIDE(122.0)
X- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- T	2	12	SIDE(183.1)
T- R	2	12	SIDE(0.1)
R- N	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
E- U	1	4	SIDE(446.2)
I- Q	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	3.25	1.75
D, G, J						
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	12.0		
F	TS-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWWW-t	MT20	5.0	12.0		
K	TTWW+m	MT20	8.0	9.0	3.25	1.75
L	TMW+p	MT20	5.0	6.0	1.75	2.75
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	12.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	5122	0	5122	0	0	5-8	5-8
N	5336	0	5336	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	3957	2660 / 0	652 / 0	0 / 0	0 / 0	645 / 0	0 / 0
N	4151	2739 / 0	716 / 0	0 / 0	0 / 0	695 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-V, I-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 63	-122.2	-122.2	0.05 (1)		10.00
B- C	-4400 / 0	-122.2	-122.2	0.05 (1)		5.44
C- D	-9250 / 0	-122.2	-122.2	0.29 (1)		3.83
D- E	-9250 / 0	-122.2	-122.2	0.26 (1)		3.86
E- F	-14259 / 0	-122.2	-122.2	0.56 (1)		2.90
F- Y	-14259 / 0	-122.2	-122.2	0.56 (1)		2.90
Y- Z	-14259 / 0	-122.2	-122.2	0.56 (1)		2.90
Z-AA	-14259 / 0	-122.2	-122.2	0.56 (1)		2.90
AA- G	-14259 / 0	-122.2	-122.2	0.56 (1)		2.90
G-AB	-14259 / 0	-122.2	-122.2	0.48 (1)		2.99
AB- H	-14259 / 0	-122.2	-122.2	0.48 (1)		2.99
H- I	-14259 / 0	-122.2	-122.2	0.48 (1)		2.99
I-AC	-8886 / 0	-122.2	-122.2	0.34 (1)		3.85
AC-AD	-8886 / 0	-122.2	-122.2	0.34 (1)		3.85
AD-AE	-8886 / 0	-122.2	-122.2	0.34 (1)		3.85
AE- J	-8886 / 0	-122.2	-122.2	0.34 (1)		3.85
J-AF	-8885 / 0	-122.2	-122.2	0.35 (1)		3.83
AF-AG	-8885 / 0	-122.2	-122.2	0.35 (1)		3.83
AG- K	-8885 / 0	-122.2	-122.2	0.35 (1)		3.83
K- L	-4586 / 0	-122.2	-122.2	0.05 (1)		5.35
L- M	0 / 63	-122.2	-122.2	0.05 (1)		10.00
X- B	-5171 / 0	0.0	0.0	0.19 (1)		6.42
N- L	-5376 / 0	0.0	0.0	0.20 (1)		6.32
X- W	0 / 0	-28.0	-28.0	0.05 (1)		10.00
W- V	0 / 3018	-28.0	-28.0	0.17 (1)		10.00
V- U	0 / 14237	-28.0	-28.0	0.64 (1)		10.00
U- T	0 / 14237	-28.0	-28.0	0.65 (1)		10.00
T-AH	0 / 14237	-28.0	-28.0	0.65 (1)		10.00
AH-AI	0 / 14237	-28.0	-28.0	0.65 (1)		10.00
AI-AJ	0 / 14237	-28.0	-28.0	0.65 (1)		10.00
AJ- S	0 / 14237	-28.0	-28.0	0.65 (1)		10.00
S-AK	0 / 12596	-28.0	-28.0	0.56 (1)		10.00
AK-AL	0 / 12596	-28.0	-28.0	0.56 (1)		10.00
AL- R	0 / 12596	-28.0	-28.0	0.56 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
W- C	-1476 / 0	0.13 (1)
C- V	0 / 7468	0.66 (1)
V- D	-654 / 0	0.06 (1)
V- E	-6001 / 0	0.64 (1)
U- E	0 / 2551	0.23 (1)
E- S	0 / 155	0.01 (3)
S- G	-993 / 0	0.08 (1)
S- I	0 / 2082	0.18 (1)
Q- I	0 / 423	0.04 (2)
I- P	-4464 / 0	0.47 (1)
P- J	-1106 / 0	0.09 (1)
P- K	0 / 6875	0.61 (1)
O- K	-1439 / 0	0.12 (1)
B- W	0 / 3523	0.31 (1)
O- L	0 / 3671	0.32 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 711 (0.61")

CSI: TC=0.56 (E-G:1) , BC=0.65 (S-U:1) , WB=0.66 (C-V:1) , SSI=0.20 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.95 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DRWG NO. TAM 47493-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288117	T5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:yOl7kOTssDWJtzbVua5b3pycNEX-VqbU3ydWBu1jSboVoVm_eDGVvGuhLJmx91W6kayc1Rf

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMW+w	MT20	3.0	10.0		
R	BS-t	MT20	6.0	9.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	6.0	9.0		
U	BMW+w	MT20	3.0	10.0		
V	BMWWW-t	MT20	6.0	12.0		
W	BMWW-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 345.6 lbs FACTORED DOWN AT 33-8-4, 147.1 lbs FACTORED DOWN AT 14-7-8, 147.1 lbs FACTORED DOWN AT 15-7-8, 147.1 lbs FACTORED DOWN AT 17-6-0, 147.1 lbs FACTORED DOWN AT 19-6-0, 147.1 lbs FACTORED DOWN AT 21-6-0, 147.1 lbs FACTORED DOWN AT 23-6-0, 147.1 lbs FACTORED DOWN AT 25-6-0, 147.1 lbs FACTORED DOWN AT 27-6-0, AND 147.1 lbs FACTORED DOWN AT 29-6-0, AND 147.1 lbs FACTORED DOWN AT 31-6-0 ON TOP CHORD, AND 2412.2 lbs FACTORED DOWN AT 13-1-8, 69.9 lbs FACTORED DOWN AT 14-7-8, 69.9 lbs FACTORED DOWN AT 15-7-8, 69.9 lbs FACTORED DOWN AT 17-6-0, 69.9 lbs FACTORED DOWN AT 19-6-0, 69.9 lbs FACTORED DOWN AT 21-6-0, 69.9 lbs FACTORED DOWN AT 23-6-0, 69.9 lbs FACTORED DOWN AT 25-6-0, 69.9 lbs FACTORED DOWN AT 27-6-0, 69.9 lbs FACTORED DOWN AT 29-6-0, 69.9 lbs FACTORED DOWN AT 31-6-0, AND 69.9 lbs FACTORED DOWN AT 33-6-0, AND 69.9 lbs FACTORED DOWN AT 34-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- Q	0 / 12596	-28.0	-28.0	0.56 (1)	10.00			
Q-AM	0 / 12596	-28.0	-28.0	0.57 (1)	10.00			
AM-AN	0 / 12596	-28.0	-28.0	0.57 (1)	10.00			
AN-AO	0 / 12596	-28.0	-28.0	0.57 (1)	10.00			
AO- P	0 / 12596	-28.0	-28.0	0.57 (1)	10.00			
P-AP	0 / 3150	-28.0	-28.0	0.18 (1)	10.00			
AP-AQ	0 / 3150	-28.0	-28.0	0.18 (1)	10.00			
AQ- O	0 / 3150	-28.0	-28.0	0.18 (1)	10.00			
O-AR	0 / 0	-28.0	-28.0	0.06 (1)	10.00			
AR- N	0 / 0	-28.0	-28.0	0.06 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	21-6-0	-147	-147	---	FRONT	VERT	TOTAL
K	33-8-4	-13	-14	---	FRONT	VERT	DEAD
K	33-8-4	-150	-150	---	FRONT	VERT	TOTAL
K	33-8-4	-183	-183	---	FRONT	VERT	SNOW
O	33-6-0	-40	-70	---	FRONT	VERT	TOTAL
U	13-1-8	-2412	-2412	---	FRONT	VERT	TOTAL
Y	14-7-8	-147	-147	---	FRONT	VERT	TOTAL
Z	15-7-8	-147	-147	---	FRONT	VERT	TOTAL
AA	17-6-0	-147	-147	---	FRONT	VERT	TOTAL
AB	19-6-0	-147	-147	---	FRONT	VERT	TOTAL
AC	23-6-0	-147	-147	---	FRONT	VERT	TOTAL
AD	25-6-0	-147	-147	---	FRONT	VERT	TOTAL
AE	27-6-0	-147	-147	---	FRONT	VERT	TOTAL
AF	29-6-0	-147	-147	---	FRONT	VERT	TOTAL
AG	31-6-0	-147	-147	---	FRONT	VERT	TOTAL
AH	14-7-8	-40	-70	---	FRONT	VERT	TOTAL
AI	15-7-8	-40	-70	---	FRONT	VERT	TOTAL
AJ	17-6-0	-40	-70	---	FRONT	VERT	TOTAL
AK	19-6-0	-40	-70	---	FRONT	VERT	TOTAL
AL	21-6-0	-40	-70	---	FRONT	VERT	TOTAL
AM	23-6-0	-40	-70	---	FRONT	VERT	TOTAL
AN	25-6-0	-40	-70	---	FRONT	VERT	TOTAL
AO	27-6-0	-40	-70	---	FRONT	VERT	TOTAL
AP	29-6-0	-40	-70	---	FRONT	VERT	TOTAL
AQ	31-6-0	-40	-70	---	FRONT	VERT	TOTAL
AR	34-8-12	-40	-70	---	FRONT	VERT	TOTAL

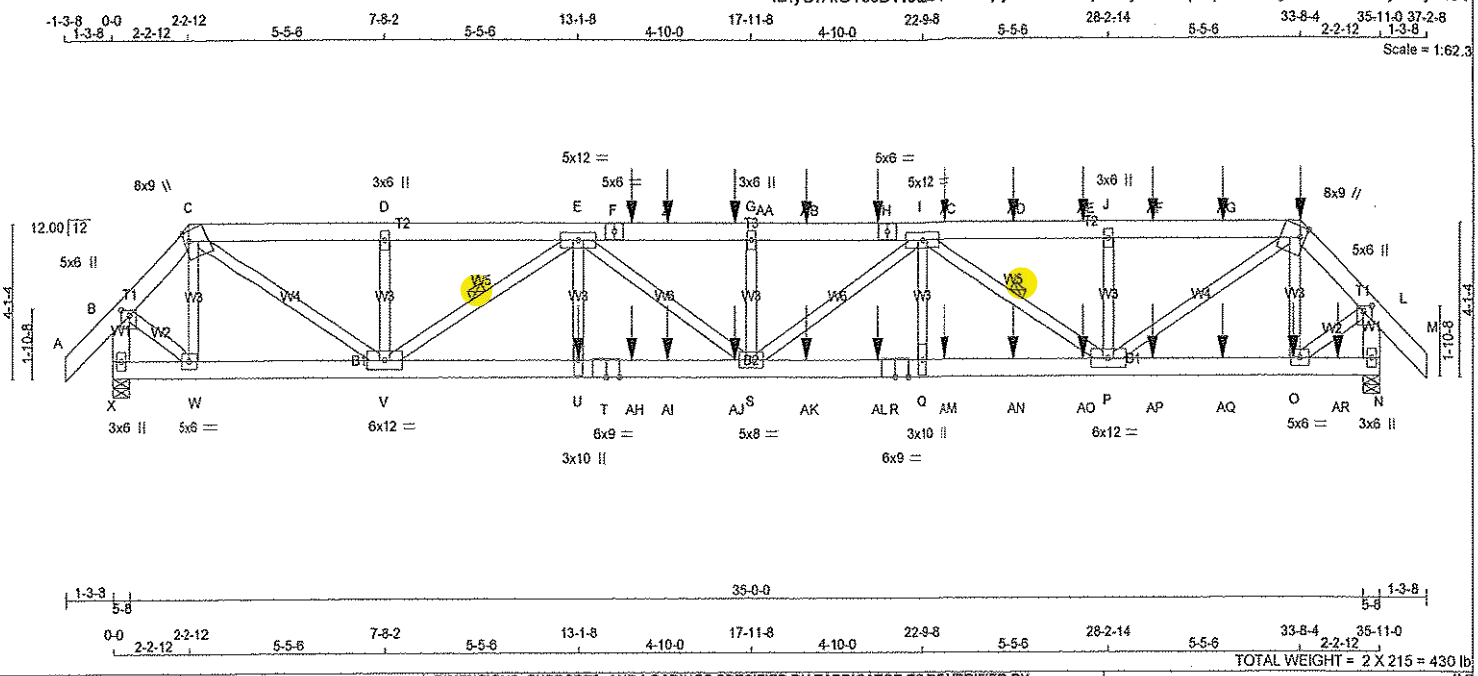
DWG NO. TAM47493-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288252	T5Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:yOi7kOTssDWJtzbVua5b3pycNEX-DG8pcRxyFGoe4phqN77OoltjDh3teP2mBFyk2Jyc1JV



[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - T	2x6	DRY	1650F 1.5E	SPF
T - R	2x6	DRY	1650F 1.5E	SPF
R - N	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- H	2	12	SIDE(0.0)
H- K	2	12	SIDE(61.0)
K- M	2	12	SIDE(122.0)
X- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- T	2	12	SIDE(183.1)
T- R	2	12	SIDE(0.1)
R- N	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
E- U	1	4	SIDE(437.1)
I- Q	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	3.25	1.75
D, G, J						
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	12.0		
F	TS-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWWW-t	MT20	5.0	12.0		
K	TTWW+m	MT20	8.0	9.0	3.25	1.75
L	TMWW+p	MT20	5.0	6.0	1.75	2.75
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	12.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
X	5100	0	5100	0	0	5-8	5-8
N	5323	0	5323	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	3937	2653 / 0	645 / 0	0 / 0	0 / 0	639 / 0	0 / 0
N	4139	2735 / 0	712 / 0	0 / 0	0 / 0	692 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-V, I-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS							WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC	MAX CSI	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI
FR-TO		FROM	TO				FR-TO		
A- B	0 / 63	-122.2	-122.2	0.05 (1)		10.00	W- C	-1468 / 0	0.13 (1)
B- C	-4379 / 0	-122.2	-122.2	0.05 (1)		5.46	C- V	0 / 7429	0.66 (1)
C- D	-9203 / 0	-122.2	-122.2	0.29 (1)		3.85	V- D	-654 / 0	0.06 (1)
D- E	-9203 / 0	-122.2	-122.2	0.26 (1)		3.88	V- E	-5962 / 0	0.53 (1)
E- F	-14196 / 0	-122.2	-122.2	0.56 (1)		2.91	U- E	0 / 2517	0.22 (1)
F- Y	-14196 / 0	-122.2	-122.2	0.56 (1)		2.91	E- S	0 / 172	0.02 (3)
Y- Z	-14196 / 0	-122.2	-122.2	0.55 (1)		2.91	S- G	-993 / 0	0.08 (1)
Z-AA	-14196 / 0	-122.2	-122.2	0.56 (1)		2.91	S- I	0 / 2081	0.18 (1)
AA- G	-14196 / 0	-122.2	-122.2	0.56 (1)		2.91	Q- I	0 / 424	0.04 (2)
G-AB	-14196 / 0	-122.2	-122.2	0.48 (1)		3.00	I- P	-4441 / 0	0.47 (1)
AB- H	-14196 / 0	-122.2	-122.2	0.48 (1)		3.00	P- J	-1107 / 0	0.09 (1)
H- I	-14196 / 0	-122.2	-122.2	0.48 (1)		3.00	P- K	0 / 6852	0.61 (1)
I-AC	-8859 / 0	-122.2	-122.2	0.34 (1)		3.85	O- K	-1435 / 0	0.12 (1)
AC-AD	-8859 / 0	-122.2	-122.2	0.34 (1)		3.85	B- W	0 / 3506	0.31 (1)
AD-AE	-8859 / 0	-122.2	-122.2	0.34 (1)		3.85	O- L	0 / 3662	0.32 (1)
AE- J	-8859 / 0	-122.2	-122.2	0.34 (1)		3.85			
J-AF	-8859 / 0	-122.2	-122.2	0.35 (1)		3.84			
AF-AG	-8859 / 0	-122.2	-122.2	0.35 (1)		3.84			
AG- K	-8859 / 0	-122.2	-122.2	0.35 (1)		3.84			
K- L	-4574 / 0	-122.2	-122.2	0.05 (1)		5.36			
L- M	0 / 63	-122.2	-122.2	0.05 (1)		10.00			
X- B	-5148 / 0	0.0	0.0	0.19 (1)		6.42			
N- L	-5363 / 0	0.0	0.0	0.20 (1)		6.32			
X- W	0 / 0	-28.0	-28.0	0.05 (1)		10.00			
W- V	0 / 3004	-28.0	-28.0	0.17 (1)		10.00			
V- U	0 / 14158	-28.0	-28.0	0.64 (1)		10.00			
U- T	0 / 14158	-28.0	-28.0	0.65 (1)		10.00			
T-AH	0 / 14158	-28.0	-28.0	0.65 (1)		10.00			
AH-AI	0 / 14158	-28.0	-28.0	0.65 (1)		10.00			
AI-AJ	0 / 14158	-28.0	-28.0	0.65 (1)		10.00			
AJ- S	0 / 14158	-28.0	-28.0	0.65 (1)		10.00			
S-AK	0 / 12550	-28.0	-28.0	0.56 (1)		10.00			
AK-AL	0 / 12550	-28.0	-28.0	0.56 (1)		10.00			
AL- R	0 / 12550	-28.0	-28.0	0.56 (1)		10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	38.3 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	10.5 PSF
		DL	=	7.0 PSF
TOTAL	LOAD		=	58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 714 (0.60")

CSI: TC=0.56 (E-G:1) , BC=0.65 (S-U:1) , WB=0.66 (C-V:1) , SSI=0.20 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.94 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9 2017 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 47512-17 R0/L

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288252	T5Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMW+w	MT20	3.0	10.0		
R	BS-t	MT20	6.0	9.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	6.0	9.0		
U	BMW+w	MT20	3.0	10.0		
V	BMWWW-t	MT20	6.0	12.0		
W	BMWW-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 345.6 lbs FACTORED DOWN AT 33-8-4, 147.1 lbs FACTORED DOWN AT 14-7-8, 147.1 lbs FACTORED DOWN AT 15-7-8, 147.1 lbs FACTORED DOWN AT 17-6-0, 147.1 lbs FACTORED DOWN AT 19-6-0, 147.1 lbs FACTORED DOWN AT 21-6-0, 147.1 lbs FACTORED DOWN AT 23-6-0, 147.1 lbs FACTORED DOWN AT 25-6-0, 147.1 lbs FACTORED DOWN AT 27-6-0, AND 147.1 lbs FACTORED DOWN AT 29-6-0, AND 147.1 lbs FACTORED DOWN AT 31-6-0 ON TOP CHORD, AND 2377.1 lbs FACTORED DOWN AT 13-1-8, 69.9 lbs FACTORED DOWN AT 14-7-8, 69.9 lbs FACTORED DOWN AT 15-7-8, 69.9 lbs FACTORED DOWN AT 17-6-0, 69.9 lbs FACTORED DOWN AT 19-6-0, 69.9 lbs FACTORED DOWN AT 21-6-0, 69.9 lbs FACTORED DOWN AT 23-6-0, 69.9 lbs FACTORED DOWN AT 25-6-0, 69.9 lbs FACTORED DOWN AT 27-6-0, 69.9 lbs FACTORED DOWN AT 29-6-0, 69.9 lbs FACTORED DOWN AT 31-6-0, AND 69.9 lbs FACTORED DOWN AT 33-6-0, AND 69.9 lbs FACTORED DOWN AT 34-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- Q	0 / 12550	-28.0	-28.0	0.56 (1)	10.00			
Q-AM	0 / 12550	-28.0	-28.0	0.57 (1)	10.00			
AM-AN	0 / 12550	-28.0	-28.0	0.57 (1)	10.00			
AN-AO	0 / 12550	-28.0	-28.0	0.57 (1)	10.00			
AO- P	0 / 12550	-28.0	-28.0	0.57 (1)	10.00			
P-AP	0 / 3142	-28.0	-28.0	0.18 (1)	10.00			
AP-AQ	0 / 3142	-28.0	-28.0	0.18 (1)	10.00			
AQ- O	0 / 3142	-28.0	-28.0	0.18 (1)	10.00			
O-AR	0 / 0	-28.0	-28.0	0.06 (1)	10.00			
AR- N	0 / 0	-28.0	-28.0	0.06 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	21-6-0	-147	-147	--	FRONT	VERT	TOTAL
K	33-8-4	-13	-14	--	FRONT	VERT	DEAD
K	33-8-4	-150	-150	--	FRONT	VERT	TOTAL
K	33-8-4	-183	-183	--	FRONT	VERT	SNOW
O	33-6-0	-40	-70	--	FRONT	VERT	TOTAL
U	13-1-8	-2377	-2377	--	FRONT	VERT	TOTAL
Y	14-7-8	-147	-147	--	FRONT	VERT	TOTAL
Z	15-7-8	-147	-147	--	FRONT	VERT	TOTAL
AA	17-6-0	-147	-147	--	FRONT	VERT	TOTAL
AB	19-6-0	-147	-147	--	FRONT	VERT	TOTAL
AC	23-6-0	-147	-147	--	FRONT	VERT	TOTAL
AD	25-6-0	-147	-147	--	FRONT	VERT	TOTAL
AE	27-6-0	-147	-147	--	FRONT	VERT	TOTAL
AF	29-6-0	-147	-147	--	FRONT	VERT	TOTAL
AG	31-6-0	-147	-147	--	FRONT	VERT	TOTAL
AH	14-7-8	-40	-70	--	FRONT	VERT	TOTAL
AI	15-7-8	-40	-70	--	FRONT	VERT	TOTAL
AJ	17-6-0	-40	-70	--	FRONT	VERT	TOTAL
AK	19-6-0	-40	-70	--	FRONT	VERT	TOTAL
AL	21-6-0	-40	-70	--	FRONT	VERT	TOTAL
AM	23-6-0	-40	-70	--	FRONT	VERT	TOTAL
AN	25-6-0	-40	-70	--	FRONT	VERT	TOTAL
AO	27-6-0	-40	-70	--	FRONT	VERT	TOTAL
AP	29-6-0	-40	-70	--	FRONT	VERT	TOTAL
AQ	31-6-0	-40	-70	--	FRONT	VERT	TOTAL
AR	34-8-12	-40	-70	--	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM49512-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T6S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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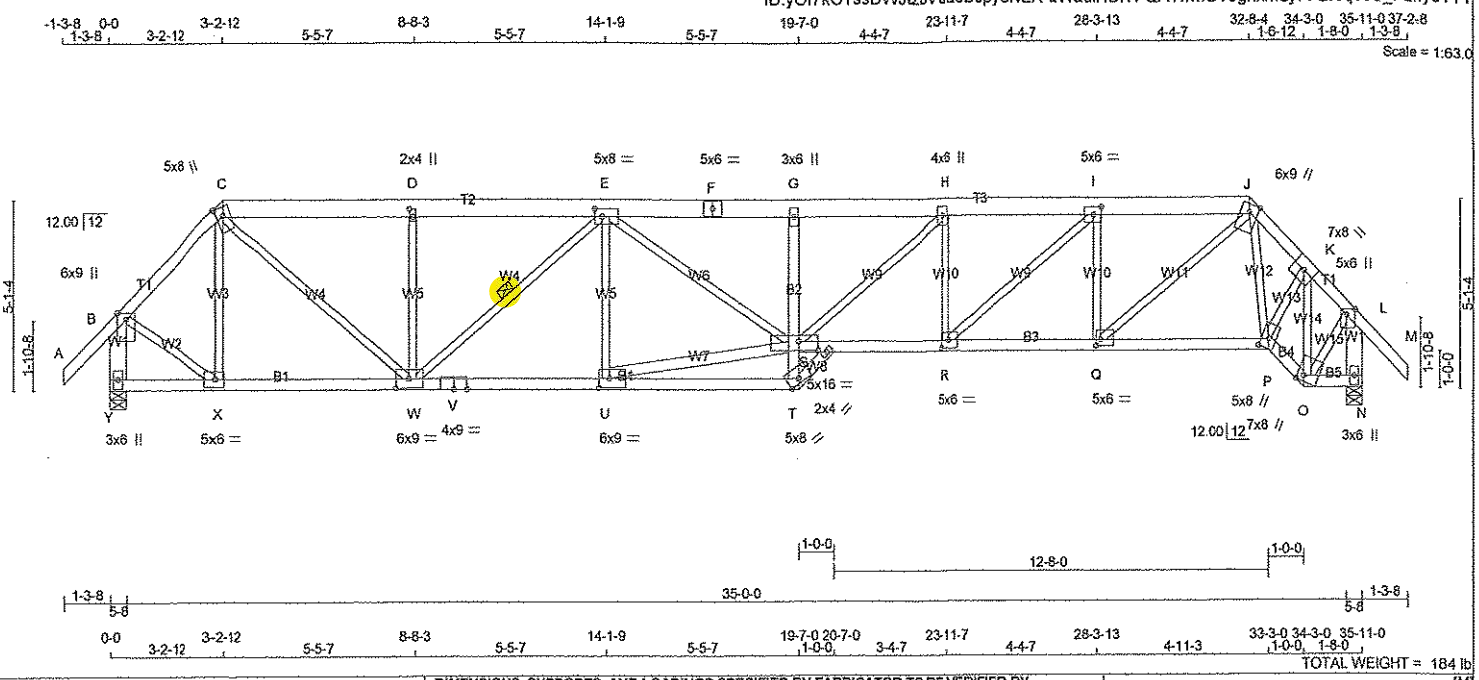

TOTAL WEIGHT = 184 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - G	2x4	DRY	No.2	SPF
S - P	2x4	DRY	1650F 1.5E	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	5.0	8.0	2.75	2.25
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMWWW-t	MT20	5.0	8.0	2.50	2.50
F	TS-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	6.0	9.0	Edge	
K	TMWW-t	MT20	7.0	8.0		
L	TMVW+p	MT20	5.0	6.0	1.75	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	7.0	8.0	Edge	2.25
P	BBWW+m	MT20	5.0	8.0	3.25	2.50
Q	BMWW-t	MT20	5.0	6.0	2.00	1.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BVMWWW-t	MT20	5.0	16.0	3.00	6.50
T	BVW*-m	MT20	5.0	8.0	1.50	3.75
U	BMWW-t	MT20	6.0	9.0	3.00	3.50
V	BS-t	MT20	4.0	9.0		
W	BMWWW-t	MT20	6.0	9.0	3.00	4.25
X	BMWW-t	MT20	5.0	6.0		
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Y	2865	0	2865	0	0	5-8	5-8
N	2871	0	2871	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD 0.4

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2222	1478 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
N	2227	1482 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2354 / 0	-122.2	-122.2	0.31 (1)	4.18
C- D	-3808 / 0	-122.2	-122.2	0.31 (1)	4.17
D- E	-3808 / 0	-122.2	-122.2	0.30 (1)	4.17
E- F	-6419 / 0	-122.2	-122.2	0.44 (1)	3.21
F- G	-6419 / 0	-122.2	-122.2	0.44 (1)	3.21
G- H	-6465 / 0	-122.2	-122.2	0.39 (1)	3.25
H- I	-5795 / 0	-122.2	-122.2	0.32 (1)	3.48
I- J	-4364 / 0	-122.2	-122.2	0.26 (1)	3.98
J- K	-2890 / 0	-122.2	-122.2	0.18 (1)	3.96
K- L	-1922 / 0	-122.2	-122.2	0.20 (1)	4.65
L- M	0 / 60	-122.2	-122.2	0.17 (1)	10.00
Y- B	-2842 / 0	0.0	0.0	0.21 (1)	6.22
N- L	-2848 / 0	0.0	0.0	0.21 (1)	6.21
Y- X	0 / 0	-28.0	-28.0	0.14 (2)	10.00
X- W	0 / 1682	-28.0	-28.0	0.36 (1)	10.00
W- V	0 / 4950	-28.0	-28.0	0.89 (1)	10.00
V- U	0 / 4950	-28.0	-28.0	0.89 (1)	10.00
U- T	0 / 170	-28.0	-28.0	0.20 (2)	10.00
T- S	0 / 104	0.0	0.0	0.24 (1)	10.00
S- G	-572 / 0	0.0	0.0	0.26 (1)	7.81
S- R	0 / 5795	-28.0	-28.0	0.74 (1)	10.00
R- Q	0 / 4364	-28.0	-28.0	0.57 (1)	10.00
Q- P	0 / 2098	-28.0	-28.0	0.31 (1)	10.00
P- O	0 / 1738	-28.0	-28.0	0.28 (1)	10.00
O- N	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-668 / 0	0.24 (1)
C- W	0 / 2829	0.64 (1)
W- D	-738 / 0	0.26 (1)
W- E	-1505 / 0	0.52 (1)
U- E	-733 / 0	0.26 (1)
U- S	0 / 4863	0.78 (1)
E- S	0 / 1796	0.40 (1)
S- H	0 / 873	0.20 (1)
R- H	-1086 / 0	0.26 (1)
R- I	0 / 1872	0.42 (1)
Q- I	-1805 / 0	0.43 (1)
Q- J	0 / 2983	0.67 (1)
J- P	-239 / 11	0.06 (1)
P- K	0 / 1761	0.40 (1)
O- K	-2365 / 0	0.47 (1)
B- X	0 / 1855	0.42 (1)
O- L	0 / 1758	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.41")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 679 (0.63")

CSI: TC=0.44 (E-G:1) , BC=0.89 (U-W:1) , WB=0.78 (S-U:1) , SSI=0.23 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI: GRIP = 0.90 (Q) (INPUT = 0.90)
LSI METAL= 0.96 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 41493-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288117	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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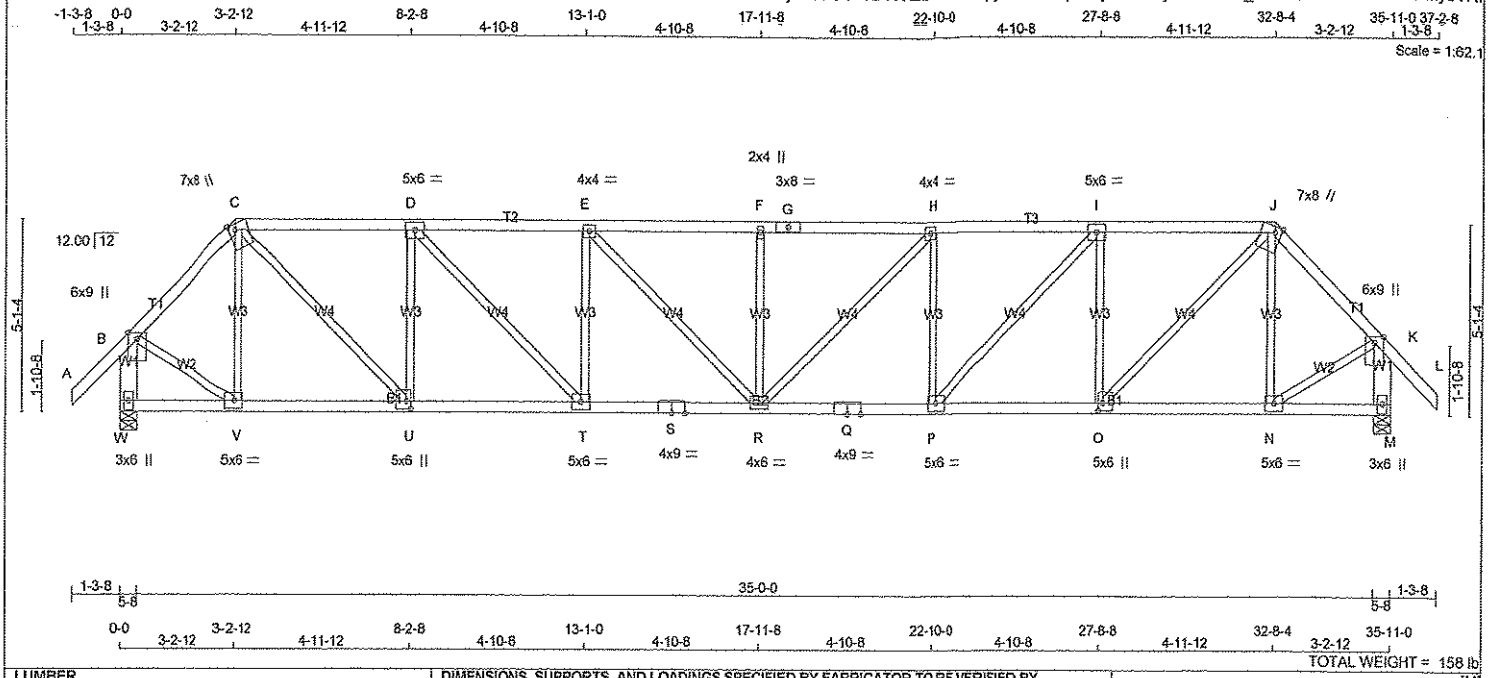

TOTAL WEIGHT = 158 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	6.0	9.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N, P, T, V						
N	BMWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	5.0	6.0	2.50	1.50
Q	BS-t	MT20	4.0	9.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BS-t	MT20	4.0	9.0		
U	BMWW+t	MT20	5.0	6.0	2.50	1.50
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	-2394 / 0	-122.2	-122.2	0.31 (1)	4.15	
C- D	-3619 / 0	-122.2	-122.2	0.70 (1)	3.07	
D- E	-4712 / 0	-122.2	-122.2	0.87 (1)	2.53	
E- F	-5087 / 0	-122.2	-122.2	0.82 (1)	2.54	
F- G	-5087 / 0	-122.2	-122.2	0.82 (1)	2.54	
G- H	-5087 / 0	-122.2	-122.2	0.82 (1)	2.54	
H- I	-4712 / 0	-122.2	-122.2	0.87 (1)	2.53	
I- J	-3619 / 0	-122.2	-122.2	0.70 (1)	3.07	
J- K	-2394 / 0	-122.2	-122.2	0.31 (1)	4.15	
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
W- B	-2839 / 0	0.0	0.0	0.21 (1)	6.23	
M- K	-2839 / 0	0.0	0.0	0.21 (1)	6.23	
W- V	0 / 0	-28.0	-28.0	0.12 (2)	10.00	
V- U	0 / 1671	-28.0	-28.0	0.35 (1)	10.00	
U- T	0 / 3619	-28.0	-28.0	0.64 (1)	10.00	
T- S	0 / 4712	-28.0	-28.0	0.83 (1)	10.00	
S- R	0 / 4712	-28.0	-28.0	0.83 (1)	10.00	
R- Q	0 / 4712	-28.0	-28.0	0.83 (1)	10.00	
Q- P	0 / 4712	-28.0	-28.0	0.83 (1)	10.00	
P- O	0 / 3619	-28.0	-28.0	0.64 (1)	10.00	
O- N	0 / 1671	-28.0	-28.0	0.35 (1)	10.00	
N- M	0 / 0	-28.0	-28.0	0.12 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-634 / 0	0.24 (1)
N- J	-634 / 0	0.24 (1)
B- V	0 / 1853	0.42 (1)
N- K	0 / 1853	0.42 (1)
O- J	0 / 2701	0.61 (1)
C- U	0 / 2701	0.61 (1)
O- I	-1737 / 0	0.67 (1)
U- D	-1737 / 0	0.67 (1)
P- I	0 / 1536	0.35 (1)
D- T	0 / 1536	0.35 (1)
P- H	-942 / 0	0.36 (1)
T- E	-942 / 0	0.36 (1)
R- H	0 / 527	0.12 (1)
E- R	0 / 527	0.12 (1)
R- F	-606 / 0	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.37")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 756 (0.57")

CSI: TC=0.87 (D-E:1) , BC=0.83 (R-T:1) , WB=0.67 (I-O:1) , SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(S) GRIP= 0.89 (N) (INPUT = 0.90)
(S) METAL= 0.93 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47444-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T7S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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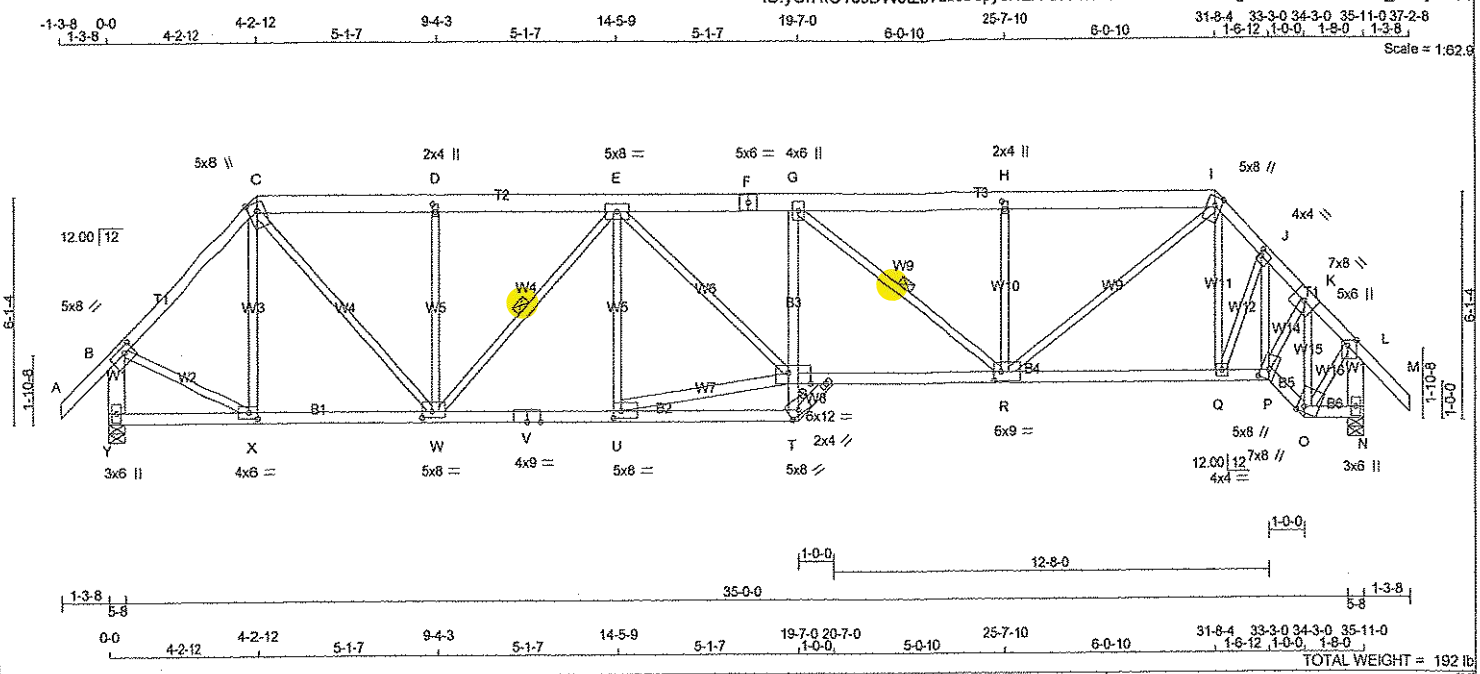

TOTAL WEIGHT = 192 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - G	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMWWW-t	MT20	5.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	5.0	8.0	2.75	2.25
J	TMWW-t	MT20	4.0	4.0	2.00	1.00
K	TMWW-t	MT20	7.0	8.0		
L	TMVW+p	MT20	5.0	6.0	1.75	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	7.0	8.0	Edge	2.25
P	BBWW+m	MT20	5.0	8.0	3.25	2.50
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	6.0	9.0	3.00	2.50
S	BVMWW-t	MT20	6.0	12.0	Edge	7.75
T	BVW*-m	MT20	5.0	8.0	1.50	3.50
U	BMWW-t	MT20	5.0	8.0	2.25	2.50
V	BS-t	MT20	4.0	9.0		
W	BMWWW-t	MT20	5.0	8.0	2.00	3.50
X	BMWW-t	MT20	4.0	6.0	2.00	2.50
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Y	2865	0	2865	0	0	5-8	5-8
N	2871	0	2871	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2222	1478 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
N	2227	1482 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W, G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B- C	-2474 / 0	-122.2	-122.2	0.55 (1)		3.85
C- D	-3294 / 0	-122.2	-122.2	0.25 (1)		4.47
D- E	-3293 / 0	-122.2	-122.2	0.25 (1)		4.47
E- F	-5116 / 0	-122.2	-122.2	0.31 (1)		3.67
F- G	-5116 / 0	-122.2	-122.2	0.31 (1)		3.67
G- H	-4278 / 0	-122.2	-122.2	0.43 (1)		3.84
H- I	-4279 / 0	-122.2	-122.2	0.42 (1)		3.84
I- J	-2979 / 0	-122.2	-122.2	0.14 (1)		3.95
J- K	-2900 / 0	-122.2	-122.2	0.16 (1)		3.97
K- L	-1922 / 0	-122.2	-122.2	0.20 (1)		4.65
L- M	0 / 60	-122.2	-122.2	0.17 (1)		10.00
Y- B	-2823 / 0	0.0	0.0	0.21 (1)		6.23
N- L	-2848 / 0	0.0	0.0	0.21 (1)		6.21
Y- X	0 / 0	-28.0	-28.0	0.15 (2)		10.00
X- W	0 / 1761	-28.0	-28.0	0.37 (1)		10.00
W- V	0 / 4109	-28.0	-28.0	0.75 (1)		10.00
V- U	0 / 4109	-28.0	-28.0	0.75 (1)		10.00
U- T	0 / 104	-28.0	-28.0	0.18 (2)		10.00
T- S	0 / 97	0.0	0.0	0.15 (1)		10.00
S- G	0 / 195	0.0	0.0	0.15 (1)		10.00
S- R	0 / 5139	-28.0	-28.0	0.95 (1)		10.00
R- Q	0 / 2115	-28.0	-28.0	0.48 (2)		10.00
Q- P	0 / 2078	-28.0	-28.0	0.44 (1)		10.00
P- O	0 / 1733	-28.0	-28.0	0.28 (1)		10.00
O- N	0 / 0	-28.0	-28.0	0.02 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-496 / 6	0.28 (1)
C- W	0 / 2316	0.52 (1)
W- D	-697 / 0	0.38 (1)
W- E	-1218 / 0	0.48 (1)
U- E	-661 / 0	0.36 (1)
U- S	0 / 4089	0.66 (1)
E- S	0 / 1367	0.31 (1)
G- R	-1082 / 0	0.43 (1)
R- H	-864 / 0	0.31 (1)
R- I	0 / 2748	0.62 (1)
Q- I	0 / 155	0.03 (3)
Q- J	0 / 98	0.02 (2)
P- J	-318 / 0	0.07 (1)
P- K	0 / 1781	0.40 (1)
O- K	-2360 / 0	0.47 (1)
B- X	0 / 1868	0.42 (1)
O- L	0 / 1753	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 882 (0.49")

CSI: TC=0.55 (B-C:1) , BC=0.95 (R-S:1) , WB=0.66 (S-U:1) , SSI=0.27 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.81 (V) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47474-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288117	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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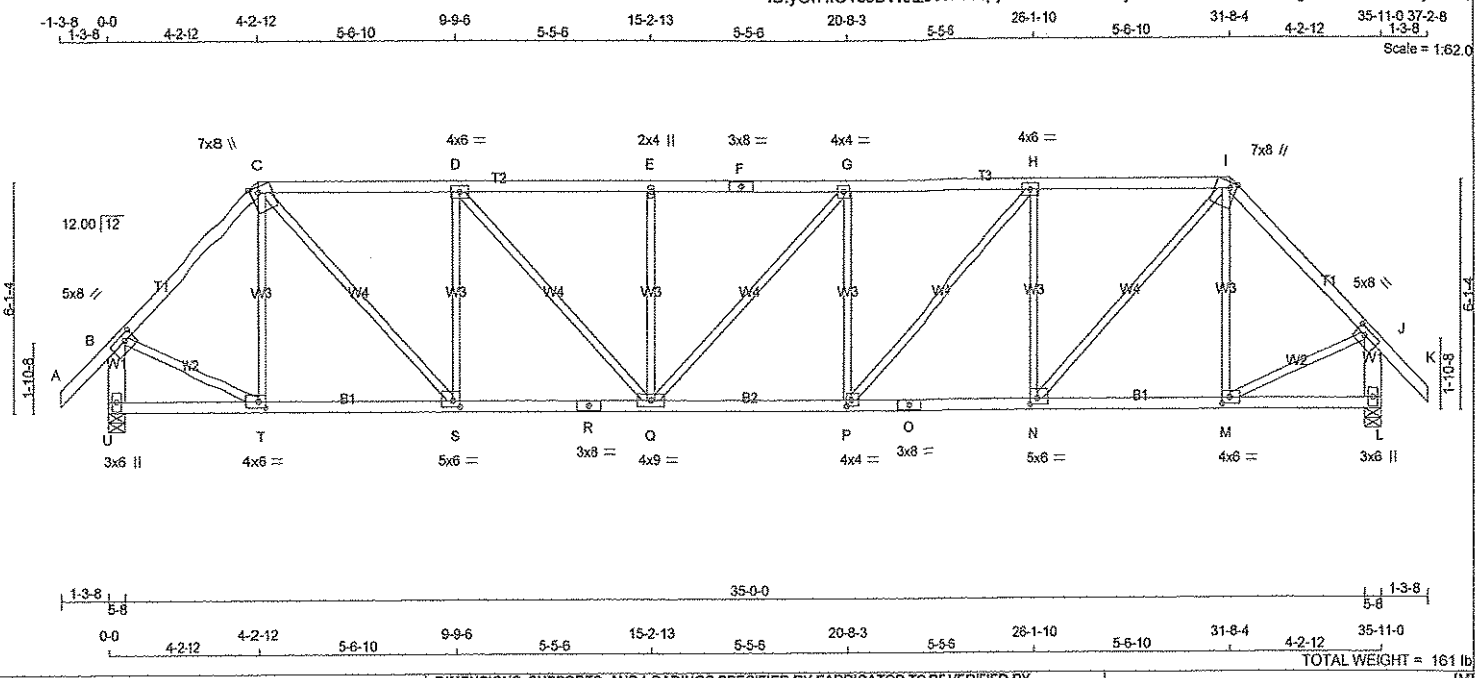


[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMW-t	MT20	5.0	8.0	2.00	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMWW-t	MT20	5.0	6.0	2.00	2.50
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.00	2.50
T	BMWW-t	MT20	4.0	6.0	2.00	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2868	0	2868	0	0	5-8	5-8
L	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
L	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B-C	-2502 / 0	-122.2	-122.2	0.56 (1)		3.83
C-D	-3378 / 0	-122.2	-122.2	0.84 (1)		2.98
D-E	-4126 / 0	-122.2	-122.2	0.96 (1)		2.58
E-F	-4126 / 0	-122.2	-122.2	0.78 (1)		2.87
F-G	-4126 / 0	-122.2	-122.2	0.78 (1)		2.87
G-H	-4127 / 0	-122.2	-122.2	0.96 (1)		2.58
H-I	-3377 / 0	-122.2	-122.2	0.84 (1)		2.99
I-J	-2502 / 0	-122.2	-122.2	0.56 (1)		3.83
J-K	0 / 60	-122.2	-122.2	0.17 (1)		10.00
U-B	-2824 / 0	0.0	0.0	0.21 (1)		6.23
L-J	-2824 / 0	0.0	0.0	0.21 (1)		6.23
U-T	0 / 0	-28.0	-28.0	0.16 (2)		10.00
T-S	0 / 1754	-28.0	-28.0	0.38 (1)		10.00
S-R	0 / 3378	-28.0	-28.0	0.62 (1)		10.00
R-Q	0 / 3378	-28.0	-28.0	0.62 (1)		10.00
Q-P	0 / 4127	-28.0	-28.0	0.74 (1)		10.00
P-O	0 / 3378	-28.0	-28.0	0.62 (1)		10.00
O-N	0 / 3378	-28.0	-28.0	0.62 (1)		10.00
N-M	0 / 1754	-28.0	-28.0	0.38 (1)		10.00
M-L	0 / 0	-28.0	-28.0	0.16 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
T-C	-464 / 29	0.27 (1)
M-I	-464 / 29	0.27 (1)
B-T	0 / 1868	0.42 (1)
M-J	0 / 1868	0.42 (1)
N-I	0 / 2343	0.53 (1)
C-S	0 / 2344	0.53 (1)
N-H	-1540 / 0	0.90 (1)
S-D	-1538 / 0	0.90 (1)
P-H	0 / 1096	0.25 (1)
D-Q	0 / 1093	0.25 (1)
P-G	-646 / 0	0.38 (1)
Q-E	-645 / 0	0.38 (1)
Q-G	-3 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 982 (0.44")

CSI: TC=0.96 (D-E:1) , BC=0.74 (P-Q:1) , WB=0.90 (H-N:1) , SSI=0.32 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

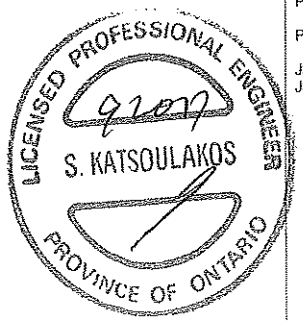
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.92 (R) (INPUT = 1.00)



DWG NO. TAM 474957
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T8S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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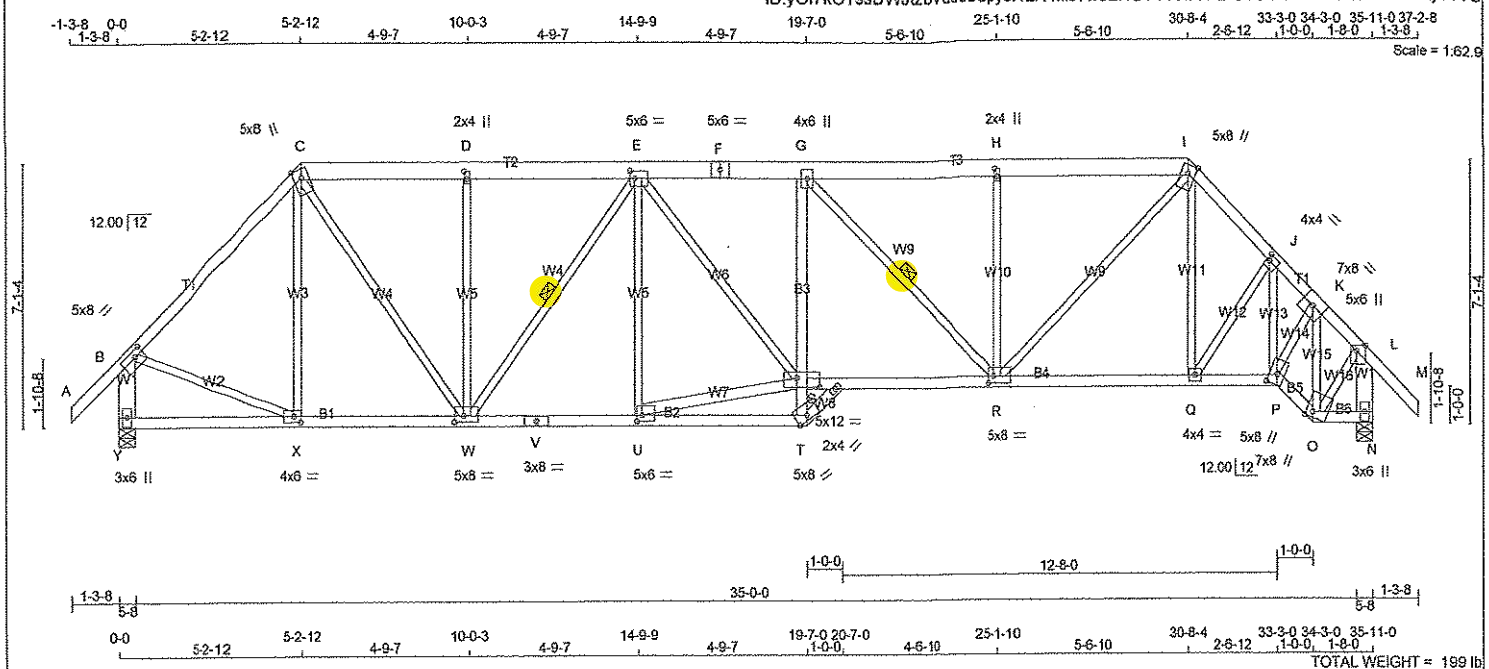


TOTAL WEIGHT = 199 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - G	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMWWW-t	MT20	5.0	6.0	2.50	1.50
F	TS-t	MT20	5.0	8.0		
G	TMVW+p	MT20	4.0	6.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	5.0	8.0	Edge	
J	TMWW-t	MT20	4.0	4.0	2.00	1.00
K	TMWW-t	MT20	7.0	8.0		
L	TMVW+p	MT20	5.0	6.0	1.75	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	7.0	8.0	Edge	2.25
P	BBWW+m	MT20	5.0	8.0	3.25	2.50
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.25	2.00
S	BVMWW-t	MT20	5.0	12.0	3.00	7.75
T	BVW*-m	MT20	5.0	8.0	1.50	3.75
U	BMWW-t	MT20	5.0	6.0	2.00	1.75
V	BS-t	MT20	3.0	8.0		
W	BMWWW-t	MT20	5.0	8.0	2.00	3.00
X	BMWW-t	MT20	4.0	6.0	2.00	2.25
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Y	2865	0	2865	0	0	IN-SX 5-8	5-8
N	2871	0	2871	0	0	IN-SX 5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2222	1478 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
N	2227	1482 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W, G-R.

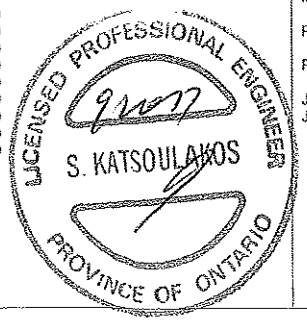
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B- C	-2529 / 0	-122.2	-122.2	0.93 (1)		3.41
C- D	-2920 / 0	-122.2	-122.2	0.21 (1)		4.74
D- E	-2920 / 0	-122.2	-122.2	0.21 (1)		4.74
E- F	-4224 / 0	-122.2	-122.2	0.24 (1)		4.05
F- G	-4224 / 0	-122.2	-122.2	0.24 (1)		4.05
G- H	-3615 / 0	-122.2	-122.2	0.33 (1)		4.22
H- I	-3615 / 0	-122.2	-122.2	0.33 (1)		4.22
I- J	-2964 / 0	-122.2	-122.2	0.20 (1)		3.90
J- K	-2886 / 0	-122.2	-122.2	0.16 (1)		3.97
K- L	-1922 / 0	-122.2	-122.2	0.20 (1)		4.65
L- M	0 / 60	-122.2	-122.2	0.17 (1)		10.00
Y- B	-2808 / 0	0.0	0.0	0.21 (1)		6.25
N- L	-2848 / 0	0.0	0.0	0.21 (1)		6.21
Y- X	0 / 0	-28.0	-28.0	0.18 (2)		10.00
X- W	0 / 1796	-28.0	-28.0	0.39 (1)		10.00
W- V	0 / 3520	-28.0	-28.0	0.65 (1)		10.00
V- U	0 / 3520	-28.0	-28.0	0.65 (1)		10.00
U- T	0 / 71	-28.0	-28.0	0.16 (2)		10.00
T- S	0 / 89	0.0	0.0	0.11 (1)		10.00
S- G	0 / 185	0.0	0.0	0.10 (1)		10.00
S- R	0 / 4237	-28.0	-28.0	0.80 (1)		10.00
R- Q	0 / 2090	-28.0	-28.0	0.45 (1)		10.00
Q- P	0 / 2092	-28.0	-28.0	0.41 (1)		10.00
P- O	0 / 1729	-28.0	-28.0	0.28 (1)		10.00
O- N	0 / 0	-28.0	-28.0	0.02 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-368 / 75	0.31 (1)
C- W	0 / 1955	0.44 (1)
W- D	-647 / 0	0.53 (1)
W- E	-1028 / 0	0.47 (1)
U- E	-606 / 0	0.50 (1)
U- S	0 / 3531	0.57 (1)
E- S	0 / 1116	0.25 (1)
G- R	-882 / 0	0.38 (1)
R- H	-789 / 0	0.43 (1)
R- I	0 / 2194	0.49 (1)
Q- I	0 / 189	0.04 (3)
Q- J	0 / 26	0.01 (3)
P- J	-325 / 0	0.07 (1)
P- K	0 / 1815	0.41 (1)
O- K	-2354 / 0	0.47 (1)
B- X	0 / 1867	0.42 (1)
O- L	0 / 1749	0.39 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.93 (B-C:1) , BC=0.80 (R-S:1) , WB=0.57 (S-U:1) , SSI=0.24 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.95 (V) (INPUT = 1.00)

DWG NO. TAM47475-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288117	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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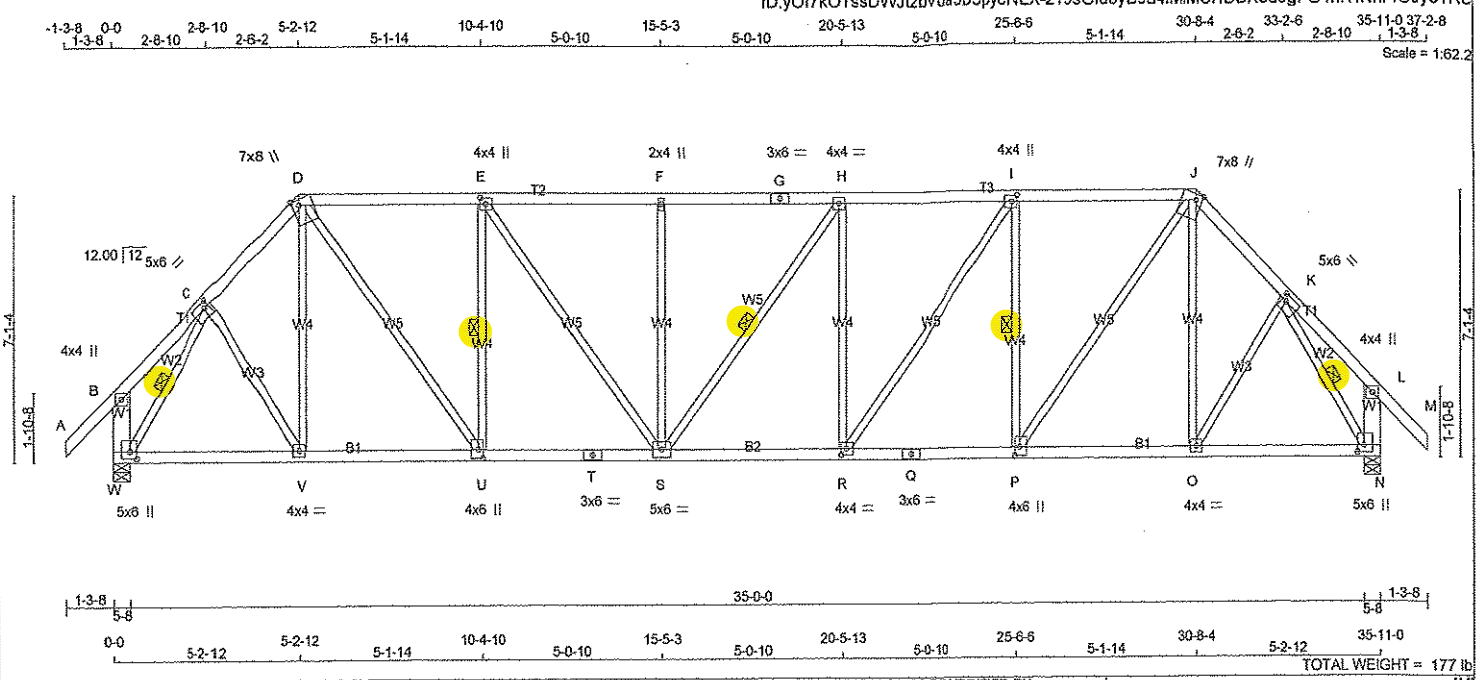

TOTAL WEIGHT = 177 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW+t	MT20	4.0	4.0	2.00	1.75
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	4.0	4.0	2.00	1.75
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMWW-t	MT20	5.0	6.0	2.00	1.50
L	TMV+p	MT20	4.0	4.0		
N	BMVW1+p	MT20	5.0	6.0	2.25	2.25
O	BMWW-t	MT20	4.0	4.0		
P	BMWW+t	MT20	4.0	6.0	2.75	1.50
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.00	1.75
S	BMWWW-t	MT20	5.0	6.0		
T	BS-t	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	6.0	2.75	1.50
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1+p	MT20	5.0	6.0	2.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2868	0	2868	0	0	5-8	5-8
N	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
N	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-W, K-N, I-P, E-U, H-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2 / 19	-122.2	-122.2	0.10 (1)	10.00
C- D	-2567 / 0	-122.2	-122.2	0.19 (1)	4.17
D- E	-2977 / 0	-122.2	-122.2	0.66 (1)	3.38
E- F	-3525 / 0	-122.2	-122.2	0.73 (1)	3.05
F- G	-3525 / 0	-122.2	-122.2	0.59 (1)	3.27
G- H	-3525 / 0	-122.2	-122.2	0.59 (1)	3.27
H- I	-3527 / 0	-122.2	-122.2	0.73 (1)	3.06
I- J	-2977 / 0	-122.2	-122.2	0.66 (1)	3.38
J- K	-2568 / 0	-122.2	-122.2	0.19 (1)	4.17
K- L	-2 / 19	-122.2	-122.2	0.10 (1)	10.00
L- M	0 / 60	-122.2	-122.2	0.17 (1)	10.00
W- B	-300 / 0	0.0	0.0	0.02 (1)	7.81
N- L	-300 / 0	0.0	0.0	0.02 (1)	7.81
W- V	0 / 1574	-28.0	-28.0	0.36 (1)	10.00
V- U	0 / 1790	-28.0	-28.0	0.39 (1)	10.00
U- T	0 / 2977	-28.0	-28.0	0.55 (1)	10.00
T- S	0 / 2977	-28.0	-28.0	0.55 (1)	10.00
S- R	0 / 3527	-28.0	-28.0	0.63 (1)	10.00
R- Q	0 / 2977	-28.0	-28.0	0.54 (1)	10.00
Q- P	0 / 2977	-28.0	-28.0	0.54 (1)	10.00
P- O	0 / 1790	-28.0	-28.0	0.39 (1)	10.00
O- N	0 / 1574	-28.0	-28.0	0.36 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 389	0.09 (1)
V- D	-157 / 104	0.14 (1)
O- J	-156 / 104	0.14 (1)
O- K	0 / 389	0.09 (1)
W- C	-2961 / 0	0.58 (1)
K- N	-2961 / 0	0.58 (1)
P- J	0 / 1962	0.44 (1)
D- U	0 / 1963	0.44 (1)
P- I	-1427 / 0	0.46 (1)
U- E	-1426 / 0	0.45 (1)
R- I	0 / 923	0.21 (1)
E- S	0 / 919	0.21 (1)
R- H	-599 / 0	0.52 (1)
S- F	-597 / 0	0.52 (1)
S- H	-3 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.73 (E-F:1) , BC=0.63 (R-S:1) , WB=0.58 (K-N:1) , SSI=0.29 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.83 (T) (INPUT = 1.00)

DWG NO. TAM 47496-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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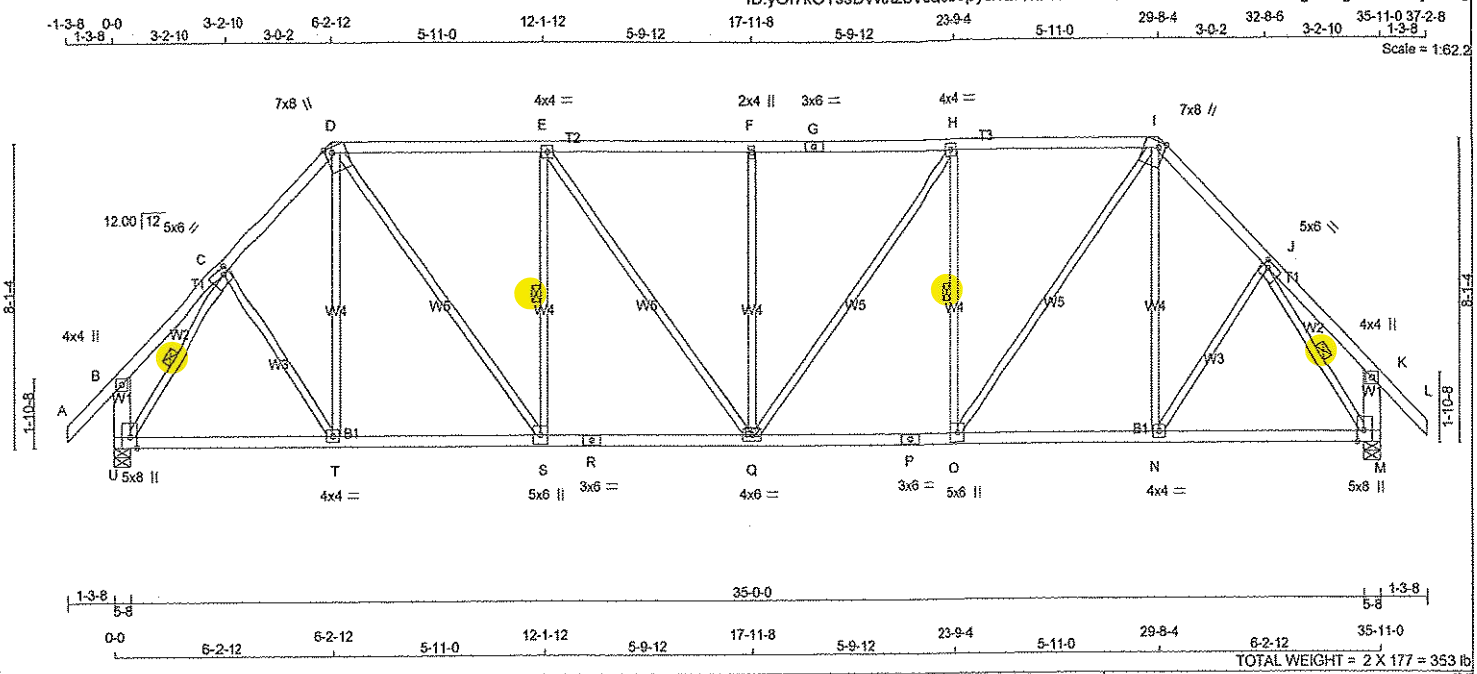

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.00	1.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	5.0	8.0	Edge	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	5.0	8.0	Edge	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	C- T	0 / 250	0.06 (1)
B- C	0 / 27	-122.2	-122.2	0.16 (1)	10.00	T- D	-9 / 188	0.04 (3)
C- D	-2584 / 0	-122.2	-122.2	0.26 (1)	4.08	D- S	0 / 1720	0.39 (1)
D- E	-2840 / 0	-122.2	-122.2	0.89 (1)	3.10	S- E	-1213 / 0	0.50 (1)
E- F	-3144 / 0	-122.2	-122.2	0.93 (1)	2.91	E- Q	0 / 508	0.11 (1)
F- G	-3144 / 0	-122.2	-122.2	0.93 (1)	2.91	Q- F	-654 / 0	0.83 (1)
G- H	-3144 / 0	-122.2	-122.2	0.93 (1)	2.91	Q- H	0 / 508	0.11 (1)
H- I	-2840 / 0	-122.2	-122.2	0.89 (1)	3.10	O- H	-1213 / 0	0.50 (1)
I- J	-2584 / 0	-122.2	-122.2	0.26 (1)	4.08	O- I	0 / 1720	0.39 (1)
J- K	0 / 27	-122.2	-122.2	0.16 (1)	10.00	N- I	-9 / 188	0.04 (3)
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00	N- J	0 / 250	0.06 (1)
U- B	-320 / 0	0.0	0.0	0.02 (1)	7.81	U- C	-2979 / 0	0.67 (1)
M- K	-320 / 0	0.0	0.0	0.02 (1)	7.81	J- M	-2979 / 0	0.67 (1)
U- T	0 / 1655	-28.0	-28.0	0.43 (2)	10.00			
T- S	0 / 1800	-28.0	-28.0	0.45 (2)	10.00			
S- R	0 / 2840	-28.0	-28.0	0.55 (1)	10.00			
R- Q	0 / 2840	-28.0	-28.0	0.55 (1)	10.00			
Q- P	0 / 2840	-28.0	-28.0	0.55 (1)	10.00			
P- O	0 / 2840	-28.0	-28.0	0.55 (1)	10.00			
O- N	0 / 1800	-28.0	-28.0	0.45 (2)	10.00			
N- M	0 / 1655	-28.0	-28.0	0.43 (2)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.93 (F-H:1) , BC=0.55 (Q-S:1) , WB=0.83 (F-Q:1) , SSI=0.34 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.74 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47476-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T10	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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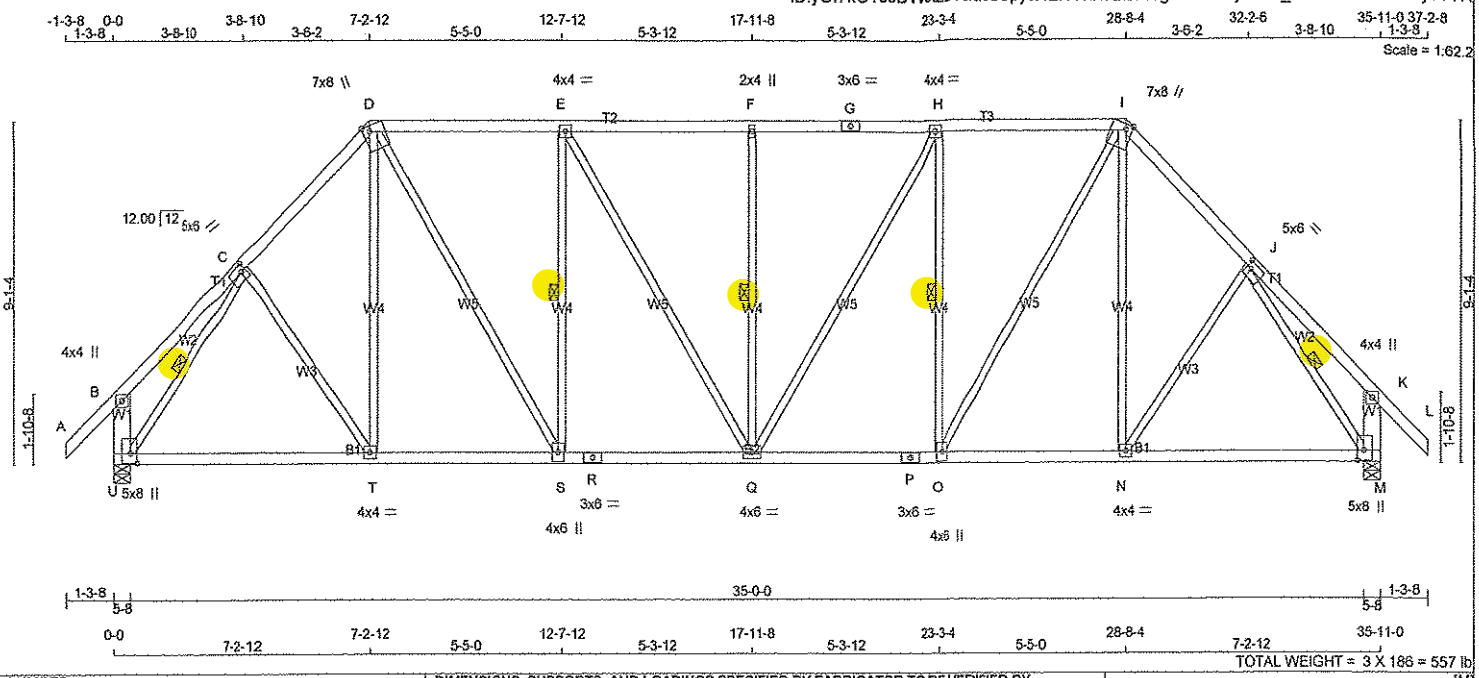


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.50
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	5.0	8.0	3.25	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	5.0	8.0	3.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-U, J-M, H-O, E-S, F-Q.

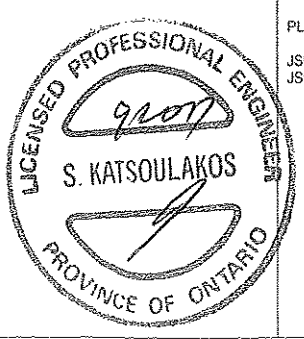
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	1	0.17	10.00
B- C	0 / 34	-122.2	-122.2	1	0.24	10.00
C- D	-2573 / 0	-122.2	-122.2	1	0.36	3.99
D- E	-2555 / 0	-122.2	-122.2	1	0.69	3.54
E- F	-2781 / 0	-122.2	-122.2	1	0.71	3.38
F- G	-2781 / 0	-122.2	-122.2	1	0.71	3.38
G- H	-2781 / 0	-122.2	-122.2	1	0.71	3.38
H- I	-2555 / 0	-122.2	-122.2	1	0.69	3.54
I- J	-2573 / 0	-122.2	-122.2	1	0.36	3.99
J- K	0 / 34	-122.2	-122.2	1	0.24	10.00
K- L	0 / 60	-122.2	-122.2	1	0.17	10.00
U- B	-341 / 0	0.0	0.0	1	0.03	7.81
M- K	-341 / 0	0.0	0.0	1	0.03	7.81
U- T	0 / 1712	-28.0	-28.0	2	0.50	10.00
T- S	0 / 1790	-28.0	-28.0	2	0.51	10.00
S- R	0 / 2555	-28.0	-28.0	1	0.48	10.00
R- Q	0 / 2555	-28.0	-28.0	1	0.48	10.00
Q- P	0 / 2555	-28.0	-28.0	1	0.48	10.00
P- O	0 / 2555	-28.0	-28.0	1	0.48	10.00
O- N	0 / 1790	-28.0	-28.0	2	0.51	10.00
N- M	0 / 1712	-28.0	-28.0	2	0.50	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- T	0 / 171	0.04 (2)
T- D	0 / 246	0.06 (3)
N- I	0 / 246	0.06 (3)
N- J	0 / 171	0.04 (2)
U- C	-2983 / 0	0.79 (1)
J- M	-2983 / 0	0.79 (1)
O- I	0 / 1457	0.33 (1)
D- S	0 / 1457	0.33 (1)
O- H	-1110 / 0	0.60 (1)
S- E	-1110 / 0	0.60 (1)
Q- H	0 / 437	0.10 (1)
E- Q	0 / 437	0.10 (1)
Q- F	-597 / 0	0.32 (1)



[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.71 (E-F:1) , BC=0.51 (S-T:2) , WB=0.79 (C-U:1) , SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.73 (J) (INPUT = 1.00)

DWG NO. TAM47477-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T11	1	1	TRUSS DESC.	

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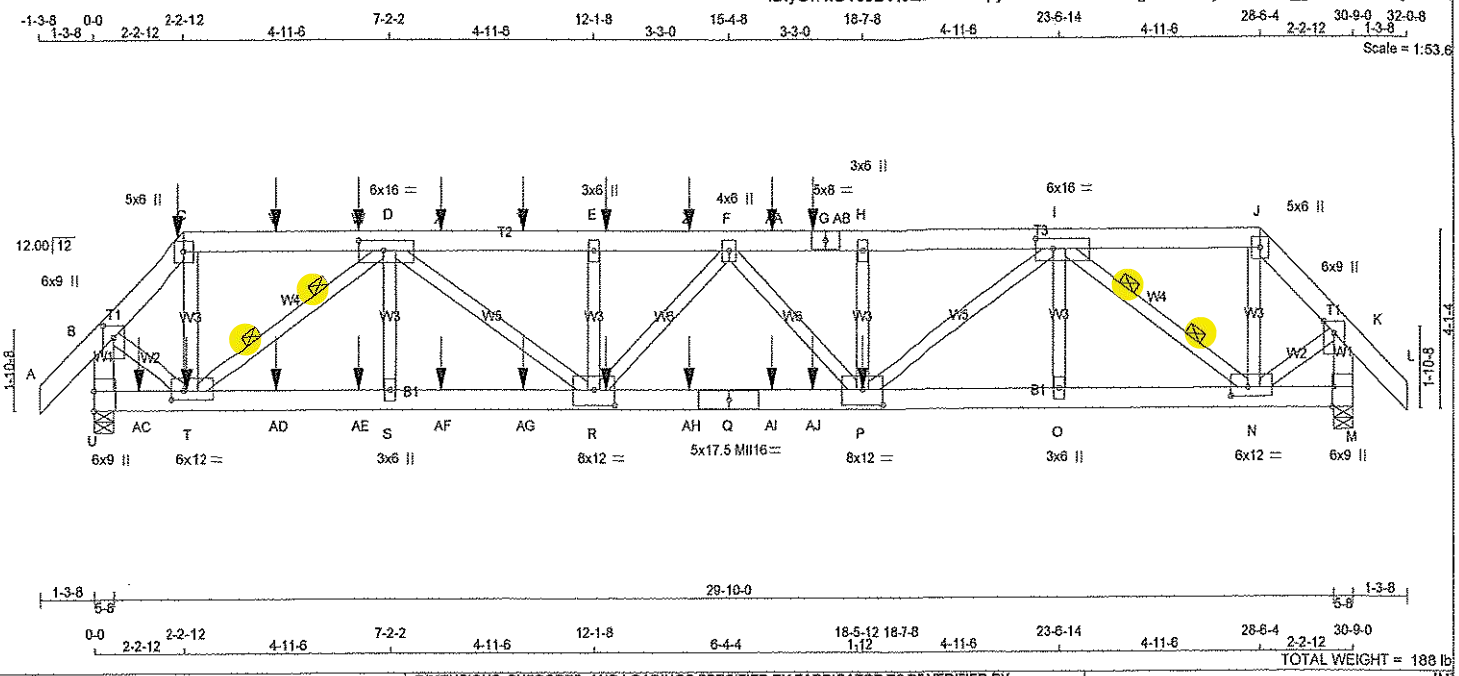


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	1650F 1.5E	SPF
G - J	2x6	DRY	1650F 1.5E	SPF
J - L	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0	3.25	3.00
C	TTW+p	MT20	5.0	6.0		
D	TMWWW-t	MT20	6.0	16.0	2.75	7.25
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	8.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWWW-t	MT20	6.0	16.0	2.75	5.25
J	TTW+p	MT20	5.0	6.0		
K	TMVW+p	MT20	6.0	9.0	3.25	3.00
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWWW-t	MT20	6.0	12.0	2.50	5.00
O	BMW+w	MT20	3.0	6.0		
P	BMWWW-t	MT20	8.0	12.0	4.25	6.00
Q	BS-t	MII16	5.0	17.5		
R	BMWWW-t	MT20	8.0	12.0	4.25	6.00
S	BMW+w	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	12.0	2.50	3.50
U	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 346.1 lbs FACTORED DOWN AT 2-2-12, 147.1 lbs FACTORED DOWN AT 4-5-0, 147.1 lbs FACTORED DOWN AT 6-5-0, 147.1 lbs FACTORED DOWN AT 8-5-0, 147.1 lbs FACTORED DOWN AT 10-5-0, 147.1 lbs FACTORED DOWN AT 12-5-0, 147.1 lbs FACTORED DOWN AT 14-5-0, AND 147.1 lbs FACTORED DOWN AT 16-5-0, AND 147.1 lbs FACTORED DOWN AT 17-5-0 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-3-8, 69.9 lbs FACTORED DOWN AT 4-5-0, 69.9 lbs FACTORED DOWN AT 6-5-0, 69.9 lbs FACTORED DOWN AT 8-5-0, 69.9 lbs FACTORED DOWN AT 10-5-0, 69.9 lbs FACTORED DOWN AT 12-5-0, 69.9 lbs FACTORED DOWN AT 14-5-0, 69.9 lbs FACTORED DOWN AT 16-5-0, AND 69.9 lbs FACTORED DOWN AT 17-5-0, AND 2097.4 lbs FACTORED DOWN AT 18-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	4659	0	4659	0	0	5-8	5-8
M	4335	0	4335	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3616	2401 / 0	614 / 0	0 / 0	0 / 0	601 / 0	0 / 0
M	3340	2261 / 0	541 / 0	0 / 0	0 / 0	538 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF D-T, I-N.

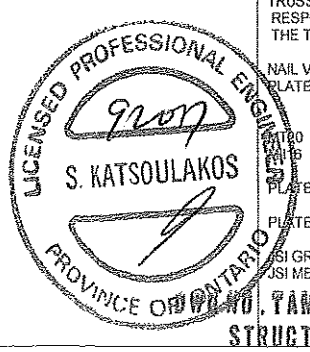
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.10 (1)	10.00	
B- C	-3930 / 0	-122.2	-122.2	0.13 (1)	4.23	
C- V	-2829 / 0	-122.2	-122.2	0.26 (1)	5.27	
V- W	-2829 / 0	-122.2	-122.2	0.26 (1)	5.27	
W- D	-2829 / 0	-122.2	-122.2	0.26 (1)	5.27	
D- X	-9773 / 0	-122.2	-122.2	0.52 (1)	2.92	
X- Y	-9773 / 0	-122.2	-122.2	0.52 (1)	2.92	
Y- E	-9773 / 0	-122.2	-122.2	0.52 (1)	2.92	
E- Z	-9773 / 0	-122.2	-122.2	0.33 (1)	3.11	
Z- F	-9773 / 0	-122.2	-122.2	0.33 (1)	3.11	
F-AA	-10765 / 0	-122.2	-122.2	0.46 (1)	2.84	
AA-AB	-10765 / 0	-122.2	-122.2	0.46 (1)	2.84	
AB- G	-10765 / 0	-122.2	-122.2	0.46 (1)	2.84	
G- H	-10765 / 0	-122.2	-122.2	0.46 (1)	2.84	
H- I	-10766 / 0	-122.2	-122.2	0.46 (1)	2.85	
I- J	-2641 / 0	-122.2	-122.2	0.16 (1)	5.54	
J- K	-3648 / 0	-122.2	-122.2	0.12 (1)	4.38	
K- L	0 / 63	-122.2	-122.2	0.10 (1)	10.00	
U- B	-4652 / 0	0.0	0.0	0.35 (1)	4.94	
M- K	-4341 / 0	0.0	0.0	0.32 (1)	5.11	
U-AC	0 / 0	-28.0	-28.0	0.06 (2)	10.00	
AC- T	0 / 0	-28.0	-28.0	0.06 (2)	10.00	
T-AD	0 / 7103	-28.0	-28.0	0.42 (1)	10.00	
AD-AE	0 / 7103	-28.0	-28.0	0.42 (1)	10.00	
AE- S	0 / 7103	-28.0	-28.0	0.42 (1)	10.00	
S-AF	0 / 7103	-28.0	-28.0	0.43 (1)	10.00	
AF-AG	0 / 7103	-28.0	-28.0	0.43 (1)	10.00	
AG- R	0 / 7103	-28.0	-28.0	0.43 (1)	10.00	
R-AH	0 / 10592	-28.0	-28.0	0.68 (1)	10.00	
AH- Q	0 / 10592	-28.0	-28.0	0.68 (1)	10.00	
Q-AI	0 / 10592	-28.0	-28.0	0.68 (1)	10.00	
AI-AJ	0 / 10592	-28.0	-28.0	0.68 (1)	10.00	
AJ- P	0 / 10592	-28.0	-28.0	0.68 (1)	10.00	
P- O	0 / 7181	-28.0	-28.0	0.44 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	0 / 1984	0.35 (1)
T- D	-5365 / 0	0.72 (1)
S- D	0 / 372	0.07 (3)
D- R	0 / 3317	0.59 (1)
R- E	-733 / 0	0.13 (1)
P- H	-530 / 0	0.09 (1)
P- I	0 / 4453	0.79 (1)
O- I	0 / 197	0.03 (2)
I- N	-5700 / 0	0.77 (1)
N- J	0 / 2159	0.39 (1)
B- T	0 / 3147	0.56 (1)
N- K	0 / 2921	0.52 (1)
R- F	-1232 / 0	0.34 (1)
F- P	0 / 262	0.05 (1)



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DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 864 (0.43")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 565 (0.65")

CSI: TC=0.52 (D-E:1) , BC=0.68 (P-R:1) , WB=0.79 (I-P:1) , SSI=0.39 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.90 (I) (INPUT = 0.90)
MSI METAL= 0.93 (Q) (INPUT = 1.00)

pg 1 of 2

DWG. NO. TAM4747B-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- N	0 / 7181	-28.0	-28.0	0.41 (1)	10.00			
N- M	0 / 0	-28.0	-28.0	0.04 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-13	-14	---	FRONT	VERT	DEAD
C	2-2-12	-151	-151	---	FRONT	VERT	TOTAL
C	2-2-12	-183	-183	---	FRONT	VERT	SNOW
E	12-5-0	-147	-147	---	FRONT	VERT	TOTAL
P	18-7-8	-2097	-2097	---	FRONT	VERT	TOTAL
R	12-5-0	-40	-70	---	FRONT	VERT	TOTAL
T	2-3-8	-40	-70	---	FRONT	VERT	TOTAL
V	4-5-0	-147	-147	---	FRONT	VERT	TOTAL
W	6-5-0	-147	-147	---	FRONT	VERT	TOTAL
X	8-5-0	-147	-147	---	FRONT	VERT	TOTAL
Y	10-5-0	-147	-147	---	FRONT	VERT	TOTAL
Z	14-5-0	-147	-147	---	FRONT	VERT	TOTAL
AA	16-5-0	-147	-147	---	FRONT	VERT	TOTAL
AB	17-5-0	-147	-147	---	FRONT	VERT	TOTAL
AC	1-0-12	-40	-70	---	FRONT	VERT	TOTAL
AD	4-5-0	-40	-70	---	FRONT	VERT	TOTAL
AE	6-5-0	-40	-70	---	FRONT	VERT	TOTAL
AF	8-5-0	-40	-70	---	FRONT	VERT	TOTAL
AG	10-5-0	-40	-70	---	FRONT	VERT	TOTAL
AH	14-5-0	-40	-70	---	FRONT	VERT	TOTAL
AI	16-5-0	-40	-70	---	FRONT	VERT	TOTAL
AJ	17-5-0	-40	-70	---	FRONT	VERT	TOTAL

DWG NO. TAM47478-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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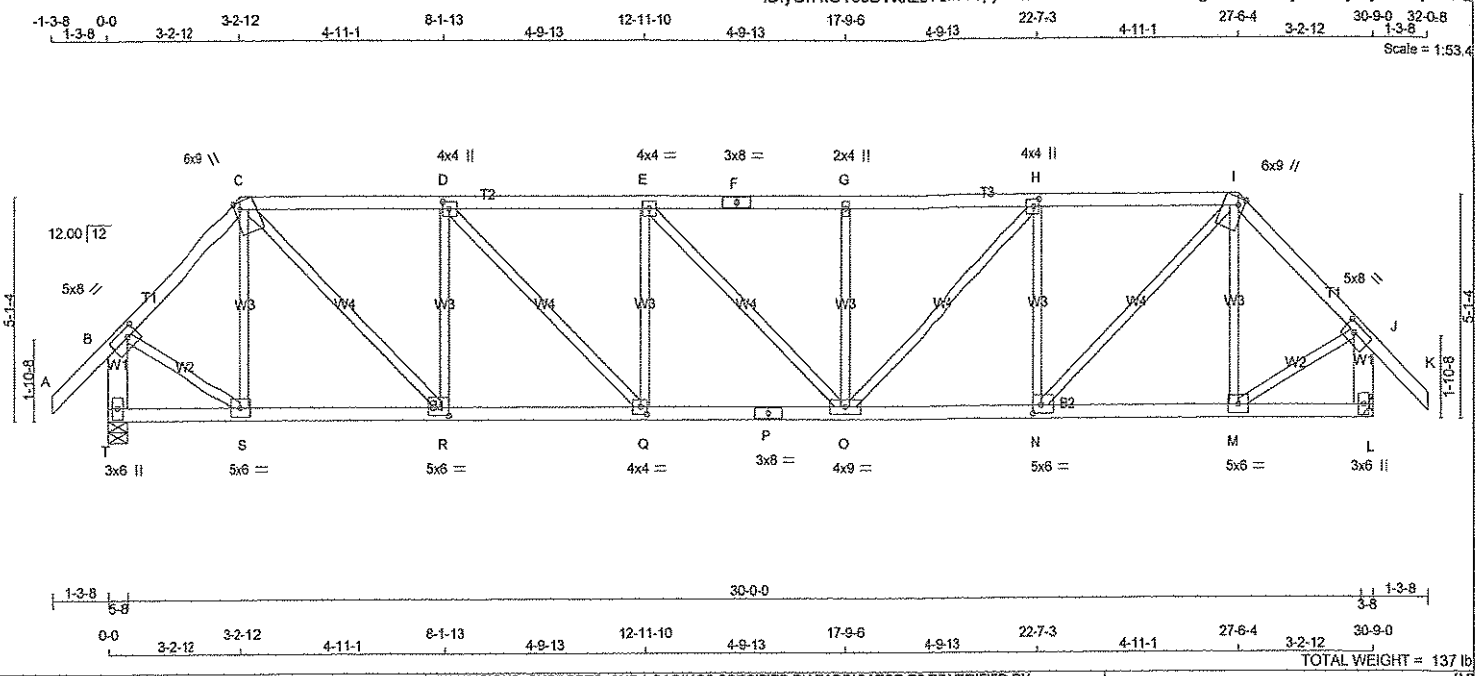

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW+t	MT20	4.0	4.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	4.0	2.00	1.75
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW-t	MT20	5.0	8.0	2.25	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.25	2.50
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.75
R	BMWW-t	MT20	5.0	6.0	2.25	2.50
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2480	0	2480	0	0	IN-SX	IN-SX
L	2480	0	2480	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1921	1282 / 0	323 / 0	0 / 0	0 / 0	316 / 0	0 / 0
L	1921	1282 / 0	323 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	-2017 / 0	-122.2	-122.2	0.29 (1)	4.48	
C- D	-2943 / 0	-122.2	-122.2	0.59 (1)	3.49	
D- E	-3650 / 0	-122.2	-122.2	0.67 (1)	3.08	
E- F	-3648 / 0	-122.2	-122.2	0.54 (1)	3.25	
F- G	-3648 / 0	-122.2	-122.2	0.54 (1)	3.25	
G- H	-3648 / 0	-122.2	-122.2	0.67 (1)	3.07	
H- I	-2943 / 0	-122.2	-122.2	0.59 (1)	3.49	
I- J	-2017 / 0	-122.2	-122.2	0.29 (1)	4.48	
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
T- B	-2450 / 0	0.0	0.0	0.18 (1)	6.60	
L- J	-2449 / 0	0.0	0.0	0.18 (1)	6.60	
T- S	0 / 0	-28.0	-28.0	0.12 (2)	10.00	
S- R	0 / 1409	-28.0	-28.0	0.30 (1)	10.00	
R- Q	0 / 2943	-28.0	-28.0	0.53 (1)	10.00	
Q- P	0 / 3650	-28.0	-28.0	0.65 (1)	10.00	
P- O	0 / 3650	-28.0	-28.0	0.65 (1)	10.00	
O- N	0 / 2943	-28.0	-28.0	0.53 (1)	10.00	
N- M	0 / 1409	-28.0	-28.0	0.30 (1)	10.00	
M- L	0 / 0	-28.0	-28.0	0.12 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-516 / 0	0.20 (1)
C- R	0 / 2139	0.48 (1)
R- D	-1359 / 0	0.52 (1)
D- Q	0 / 1060	0.22 (1)
Q- E	-571 / 0	0.22 (1)
E- O	-3 / 0	0.00 (1)
O- G	-570 / 0	0.22 (1)
O- H	0 / 996	0.22 (1)
N- H	-1358 / 0	0.52 (1)
N- I	0 / 2140	0.48 (1)
M- I	-516 / 0	0.20 (1)
B- S	0 / 1561	0.35 (1)
M- J	0 / 1561	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.67 (G-H:1) , BC=0.65 (O-Q:1) , WB=0.52 (D-R:1) , SSI=0.28 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(S) GRIP= 0.90 (B) (INPUT = 0.90)
(S) METAL= 1.00 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 42474-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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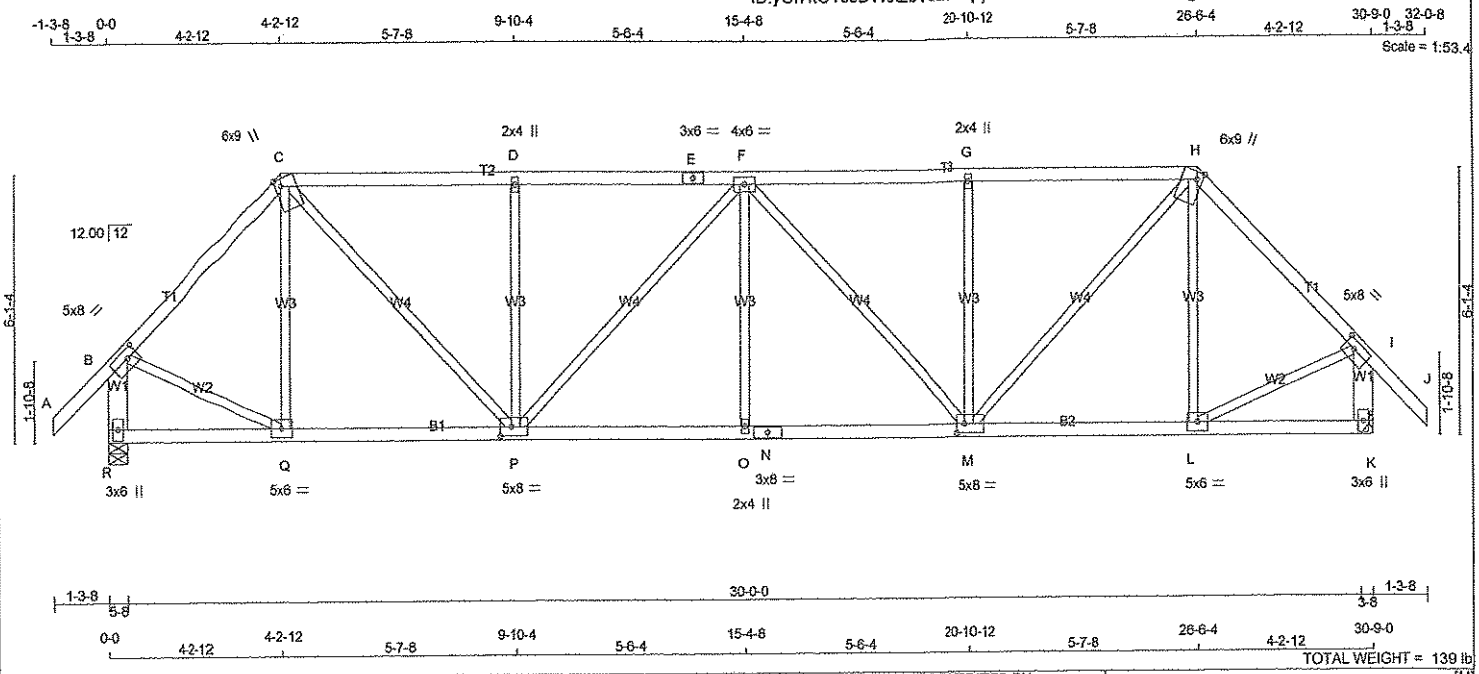


TOTAL WEIGHT = 139 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWWW-t	MT20	4.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-t	MT20	5.0	8.0	2.25	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0	2.50	2.25
N	BS-t	MT20	3.0	8.0		
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	3.25
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2480	0	2480	0	0	IN-SX	IN-SX
K	2480	0	2480	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1921	1282 / 0	323 / 0	0 / 0	0 / 0	316 / 0	0 / 0
K	1921	1282 / 0	323 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	Q- C	-362 / 68	0.21 (1)
B- C	-2097 / 0	-122.2	-122.2	0.51 (1)	4.16	C- P	0 / 1816	0.41 (1)
C- D	-2737 / 0	-122.2	-122.2	0.76 (1)	3.34	P- D	-760 / 0	0.44 (1)
D- E	-2737 / 0	-122.2	-122.2	0.76 (1)	3.34	P- F	-540 / 0	0.72 (1)
E- F	-2737 / 0	-122.2	-122.2	0.76 (1)	3.34	O- F	0 / 270	0.05 (2)
F- G	-2737 / 0	-122.2	-122.2	0.76 (1)	3.34	F- M	-540 / 0	0.72 (1)
G- H	-2737 / 0	-122.2	-122.2	0.76 (1)	3.34	M- G	-760 / 0	0.44 (1)
H- I	-2097 / 0	-122.2	-122.2	0.51 (1)	4.16	M- H	0 / 1816	0.41 (1)
I- J	0 / 60	-122.2	-122.2	0.17 (1)	10.00	L- H	-362 / 68	0.21 (1)
R- B	-2436 / 0	0.0	0.0	0.18 (1)	6.61	B- Q	0 / 1568	0.35 (1)
K- I	-2436 / 0	0.0	0.0	0.18 (1)	6.61	L- I	0 / 1568	0.35 (1)
R- Q	0 / 0	-28.0	-28.0	0.16 (2)	10.00			
Q- P	0 / 1471	-28.0	-28.0	0.33 (1)	10.00			
P- O	0 / 3109	-28.0	-28.0	0.58 (1)	10.00			
O- N	0 / 3109	-28.0	-28.0	0.58 (1)	10.00			
N- M	0 / 3109	-28.0	-28.0	0.58 (1)	10.00			
M- L	0 / 1471	-28.0	-28.0	0.33 (1)	10.00			
L- K	0 / 0	-28.0	-28.0	0.16 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.76 (C-D:1) , BC=0.58 (O-P:1) , WB=0.72 (F-M:1) , SSI=0.32 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

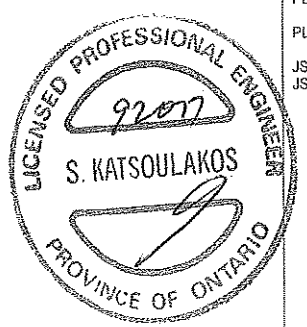
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.82 (N) (INPUT = 1.00)



DWG NO. TAM 4748017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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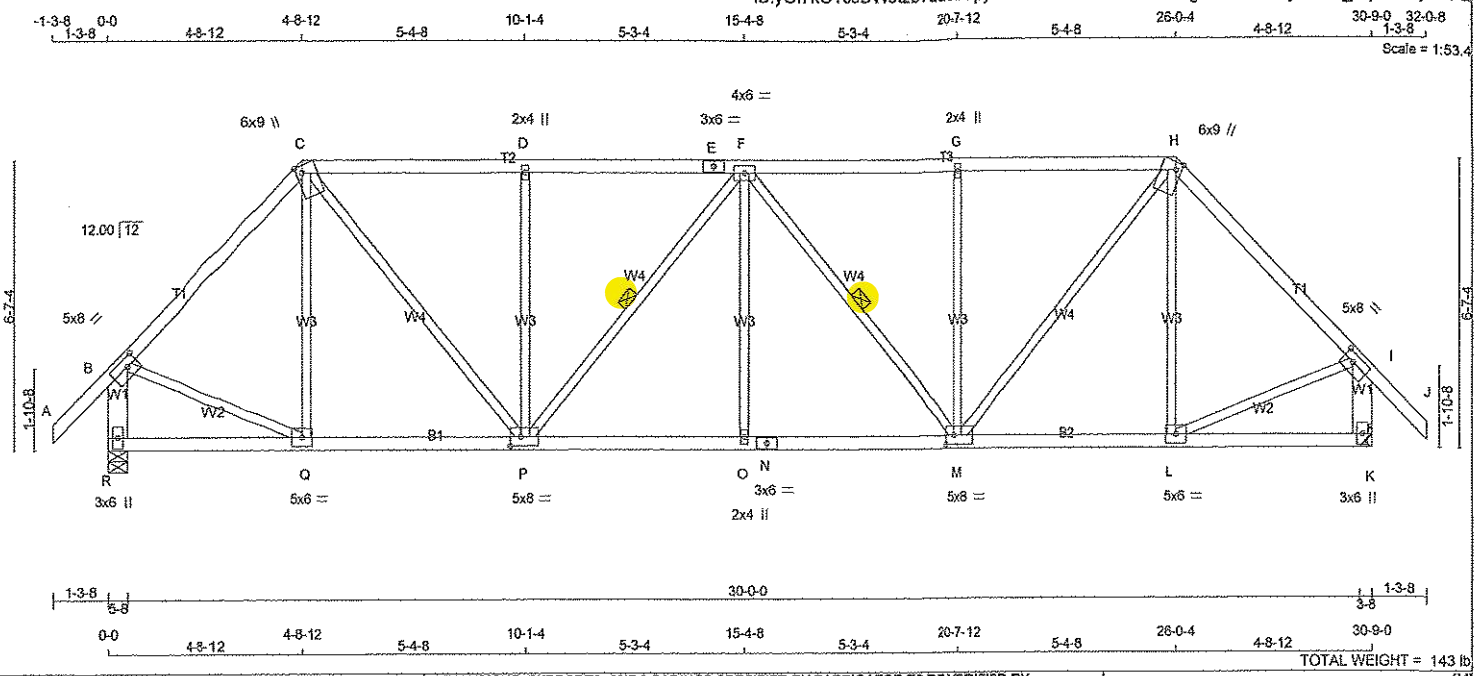

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWWW-t	MT20	4.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-t	MT20	5.0	8.0	2.25	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0	2.50	2.50
N	BS-t	MT20	3.0	6.0		
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	3.00
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2480	0	2480	0	0	IN-SX	IN-SX
K	2480	0	2480	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1921	1282 / 0	323 / 0	0 / 0	0 / 0	316 / 0	0 / 0
K	1921	1282 / 0	323 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P, F-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	Q- C	-307 / 97	0.22 (1)
B- C	-2113 / 0	-122.2	-122.2	0.66 (1)	3.98	C- P	0 / 1633	0.37 (1)
C- D	-2546 / 0	-122.2	-122.2	0.67 (1)	3.59	P- D	-725 / 0	0.52 (1)
D- E	-2546 / 0	-122.2	-122.2	0.66 (1)	3.59	P- F	-489 / 0	0.22 (1)
E- F	-2546 / 0	-122.2	-122.2	0.66 (1)	3.59	O- F	0 / 260	0.06 (2)
F- G	-2546 / 0	-122.2	-122.2	0.66 (1)	3.59	F- M	-489 / 0	0.22 (1)
G- H	-2546 / 0	-122.2	-122.2	0.67 (1)	3.59	M- G	-725 / 0	0.52 (1)
H- I	-2113 / 0	-122.2	-122.2	0.66 (1)	3.98	M- H	0 / 1633	0.37 (1)
I- J	0 / 60	-122.2	-122.2	0.17 (1)	10.00	L- H	-307 / 97	0.22 (1)
R- B	-2428 / 0	0.0	0.0	0.18 (1)	6.62	B- Q	0 / 1562	0.35 (1)
K- I	-2428 / 0	0.0	0.0	0.18 (1)	6.62	L- I	0 / 1562	0.35 (1)
R- Q	0 / 0	-28.0	-28.0	0.17 (2)	10.00			
Q- P	0 / 1484	-28.0	-28.0	0.34 (1)	10.00			
P- O	0 / 2859	-28.0	-28.0	0.54 (1)	10.00			
O- N	0 / 2859	-28.0	-28.0	0.54 (1)	10.00			
N- M	0 / 2859	-28.0	-28.0	0.54 (1)	10.00			
M- L	0 / 1484	-28.0	-28.0	0.34 (1)	10.00			
L- K	0 / 0	-28.0	-28.0	0.17 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.67 (C-D:1) , BC=0.54 (O-P:1) , WB=0.52 (D-P:1) , SSI=0.31 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.75 (N) (INPUT = 1.00)

DWG NO. TAM 47401-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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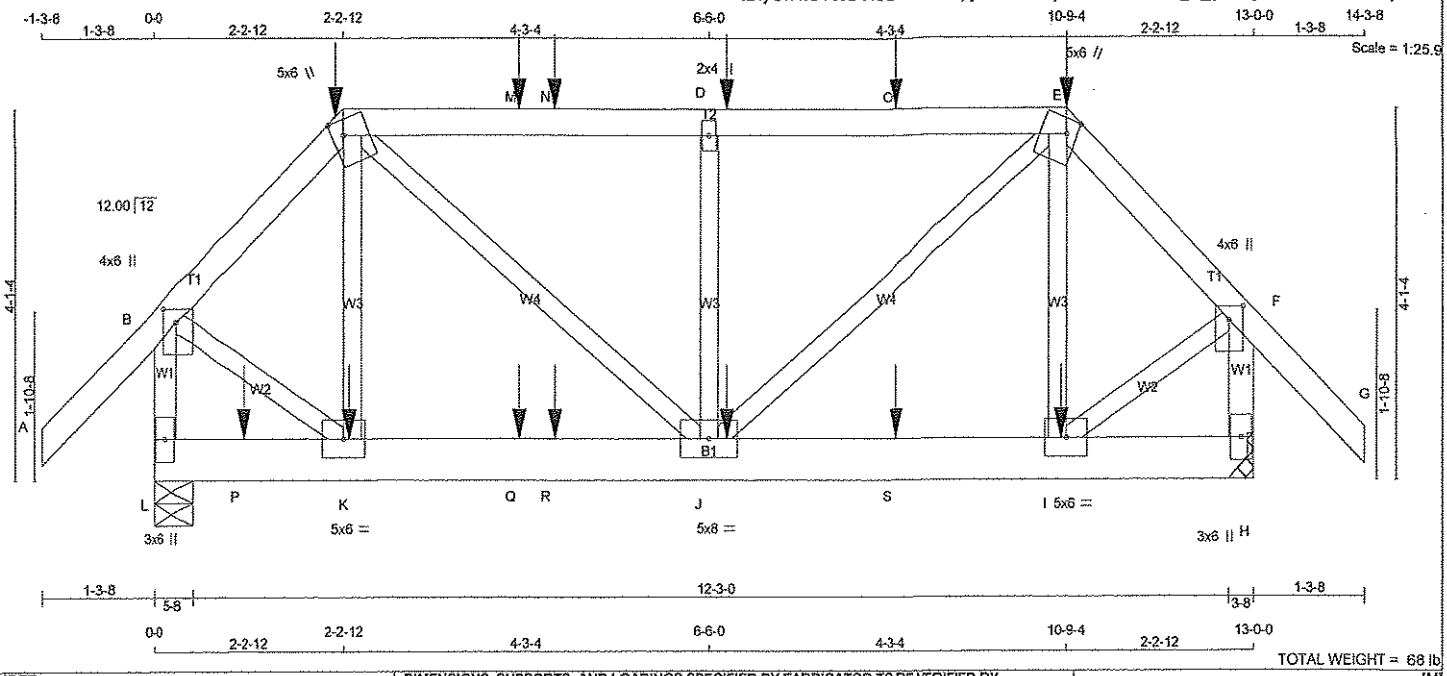

TOTAL WEIGHT = 68 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	1.75	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	4.0	6.0	1.75	2.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 346.1 lbs FACTORED DOWN AT 2-2-12, 346.1 lbs FACTORED DOWN AT 10-9-4, 147.1 lbs FACTORED DOWN AT 4-3-8, 147.1 lbs FACTORED DOWN AT 4-8-8, AND 147.1 lbs FACTORED DOWN AT 6-8-8, AND 147.1 lbs FACTORED DOWN AT 8-8-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-3-8, 69.9 lbs FACTORED DOWN AT 4-3-8, 69.9 lbs FACTORED DOWN AT 4-8-8, 69.9 lbs FACTORED DOWN AT 6-8-8, AND 69.9 lbs FACTORED DOWN AT 8-8-8, AND 69.9 lbs FACTORED DOWN AT 10-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1969	0	1969	0	0	5-8	5-8
H	1884	0	1884	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1527	1015 / 0	258 / 0	0 / 0	0 / 0	255 / 0	0 / 0
H	1445	989 / 0	225 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.19 (1)	10.00
B- C	-1435 / 0	-122.2	-122.2	0.14 (1)	5.28
C- M	-1690 / 0	-122.2	-122.2	0.72 (1)	3.93
M- N	-1690 / 0	-122.2	-122.2	0.72 (1)	3.93
N- D	-1690 / 0	-122.2	-122.2	0.72 (1)	3.93
D- O	-1690 / 0	-122.2	-122.2	0.72 (1)	3.93
O- E	-1690 / 0	-122.2	-122.2	0.72 (1)	3.93
E- F	-1373 / 0	-122.2	-122.2	0.14 (1)	5.37
F- G	0 / 60	-122.2	-122.2	0.19 (1)	10.00
L- B	-1950 / 0	0.0	0.0	0.23 (1)	5.98
H- F	-1879 / 0	0.0	0.0	0.22 (1)	6.07
L- P	0 / 0	-28.0	-28.0	0.08 (2)	10.00
P- K	0 / 0	-28.0	-28.0	0.08 (2)	10.00
K- Q	0 / 998	-28.0	-28.0	0.21 (1)	10.00
Q- R	0 / 998	-28.0	-28.0	0.21 (1)	10.00
R- J	0 / 998	-28.0	-28.0	0.21 (1)	10.00
J- S	0 / 953	-28.0	-28.0	0.19 (1)	10.00
S- I	0 / 953	-28.0	-28.0	0.19 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.06 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-372 / 131	0.09 (1)
C- J	0 / 917	0.23 (1)
J- D	-1072 / 0	0.27 (1)
J- E	0 / 976	0.24 (1)
I- E	-393 / 66	0.10 (1)
B- K	0 / 1165	0.29 (1)
I- F	0 / 1133	0.28 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	2-2-12	-13	-14	—	FRONT	VERT	DEAD
C	2-2-12	-151	-151	—	FRONT	VERT	TOTAL
C	2-2-12	-183	-183	—	FRONT	VERT	SNOW
D	6-8-8	-147	-147	—	FRONT	VERT	TOTAL
E	10-9-4	-13	-14	—	FRONT	VERT	DEAD
E	10-9-4	-151	-151	—	FRONT	VERT	TOTAL
E	10-9-4	-183	-183	—	FRONT	VERT	SNOW
I	10-8-8	-40	-70	—	FRONT	VERT	TOTAL
J	6-8-8	-40	-70	—	FRONT	VERT	TOTAL
K	2-3-8	-40	-70	—	FRONT	VERT	TOTAL
M	4-3-8	-147	-147	—	FRONT	VERT	TOTAL
N	4-8-8	-147	-147	—	FRONT	VERT	TOTAL
O	8-8-8	-147	-147	—	FRONT	VERT	TOTAL
P	1-0-12	-40	-70	—	FRONT	VERT	TOTAL
Q	4-3-8	-40	-70	—	FRONT	VERT	TOTAL
R	4-8-8	-40	-70	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.72 (D-E:1) , BC=0.21 (J-K:1) , WB=0.29 (B-K:1) , SSI=0.44 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM47482-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42057	DRWG NO.
288116	T15	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	8-8-8	-40	-70	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 42402-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 19 15:59:32 2017 Page 1

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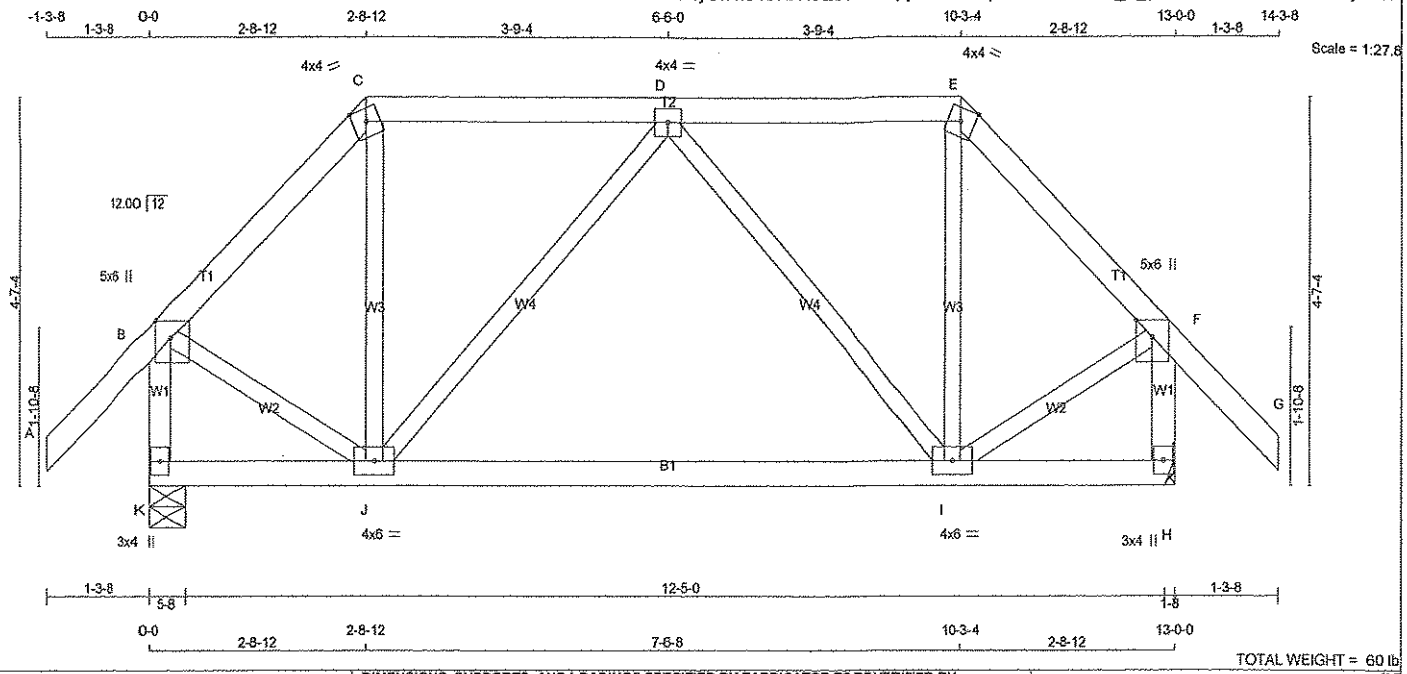

TOTAL WEIGHT = 60 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1147	0	1147	0	0	5-8	5-8
H	1147	0	1147	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	879	604 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	879	604 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	J- C	0 / 226	0.05 (2)	
B- C	-739 / 0	-122.2	-122.2	0.16 (1)	6.25	J- D	-369 / 0	0.20 (1)	
C- D	-524 / 0	-122.2	-122.2	0.29 (1)	6.25	D- I	-369 / 0	0.20 (1)	
D- E	-524 / 0	-122.2	-122.2	0.29 (1)	6.25	I- E	0 / 226	0.05 (2)	
E- F	-739 / 0	-122.2	-122.2	0.16 (1)	6.25	B- J	0 / 590	0.13 (1)	
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I- F	0 / 590	0.13 (1)	
K- B	-1142 / 0	0.0	0.0	0.13 (1)	7.44				
H- F	-1142 / 0	0.0	0.0	0.13 (1)	7.44				
K- J	0 / 0	-28.0	-28.0	0.25 (3)	10.00				
J- I	0 / 763	-28.0	-28.0	0.32 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.25 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.29 (D-E:1) , BC=0.32 (I-J:2) , WB=0.20 (D-J:1) , SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (J) (INPUT = 0.90)
JSI METAL= 0.19 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4740317
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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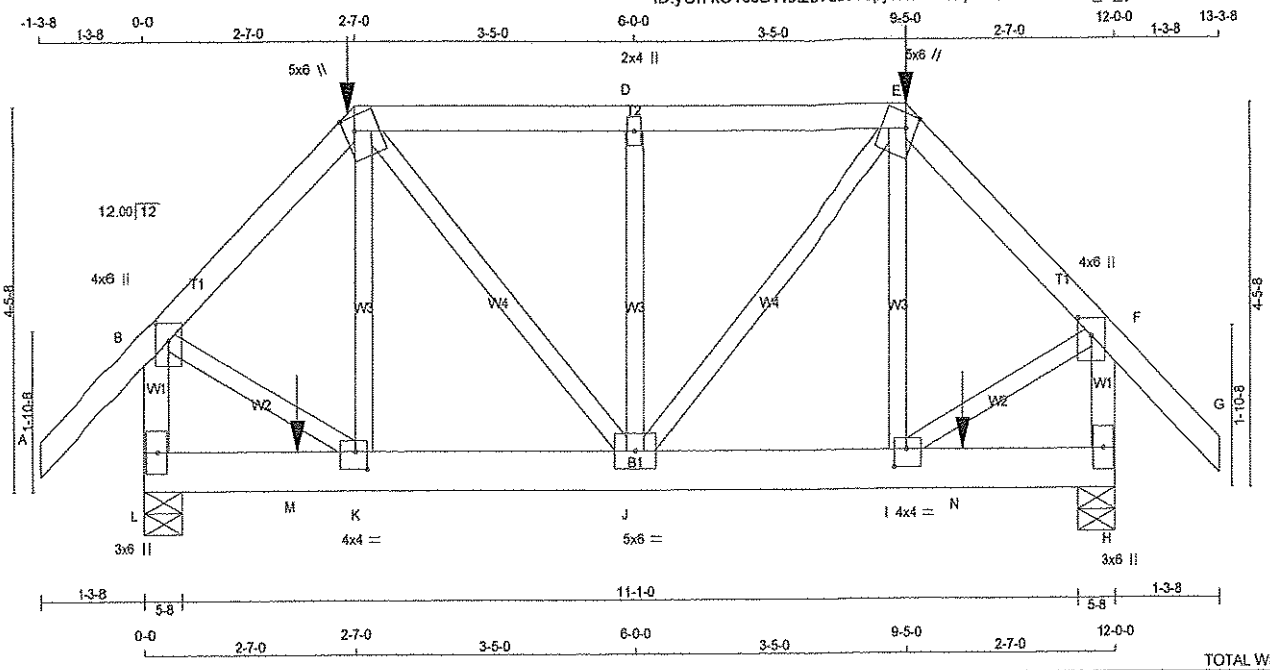

TOTAL WEIGHT = 67 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	4.0	6.0	2.50	2.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0	2.50	1.75
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.50	1.75
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 74.0 lbs FACTORED DOWN AT 2-7-0, AND 74.0 lbs FACTORED DOWN AT 9-5-0 ON TOP CHORD, AND 264.1 lbs FACTORED DOWN AT 1-10-8, AND 264.1 lbs FACTORED DOWN AT 10-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1410	0	1410	0	0	5-8	5-8
H	1410	0	1410	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1079	743 / 0	166 / 0	0 / 0	0 / 0	169 / 0	0 / 0
H	1079	743 / 0	166 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.19 (1)	10.00		K- C	-13 / 124	0.03 (3)
B- C	-911 / 0	-122.2	-122.2	0.17 (1)	6.23		C- J	0 / 341	0.08 (1)
C- D	-860 / 0	-122.2	-122.2	0.26 (1)	6.22		J- D	-498 / 0	0.14 (1)
D- E	-860 / 0	-122.2	-122.2	0.26 (1)	6.22		J- E	0 / 341	0.08 (1)
E- F	-911 / 0	-122.2	-122.2	0.17 (1)	6.23		I- E	-13 / 124	0.03 (3)
F- G	0 / 60	-122.2	-122.2	0.19 (1)	10.00		B- K	0 / 727	0.18 (1)
L- B	-1320 / 0	0.0	0.0	0.16 (1)	7.00		I- F	0 / 727	0.18 (1)
H- F	-1320 / 0	0.0	0.0	0.16 (1)	7.00				
L- M	0 / 0	-28.0	-28.0	0.09 (1)	10.00				
M- K	0 / 0	-28.0	-28.0	0.09 (1)	10.00				
K- J	0 / 640	-28.0	-28.0	0.14 (1)	10.00				
J- I	0 / 640	-28.0	-28.0	0.14 (1)	10.00				
I- N	0 / 0	-28.0	-28.0	0.09 (1)	10.00				
N- H	0 / 0	-28.0	-28.0	0.09 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-7-0	-5	-6	—	FRONT	VERT	DEAD
C	2-7-0	-69	-69	—	FRONT	VERT	SNOW
E	9-5-0	-5	-6	—	FRONT	VERT	DEAD
E	9-5-0	-69	-69	—	FRONT	VERT	SNOW
M	1-10-8	-264	-264	—	BACK	VERT	TOTAL
N	10-1-8	-264	-264	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.26 (C-D:1) , BC=0.14 (J-K:1) , WB=0.18 (F-I:1) , SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.24 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM4746417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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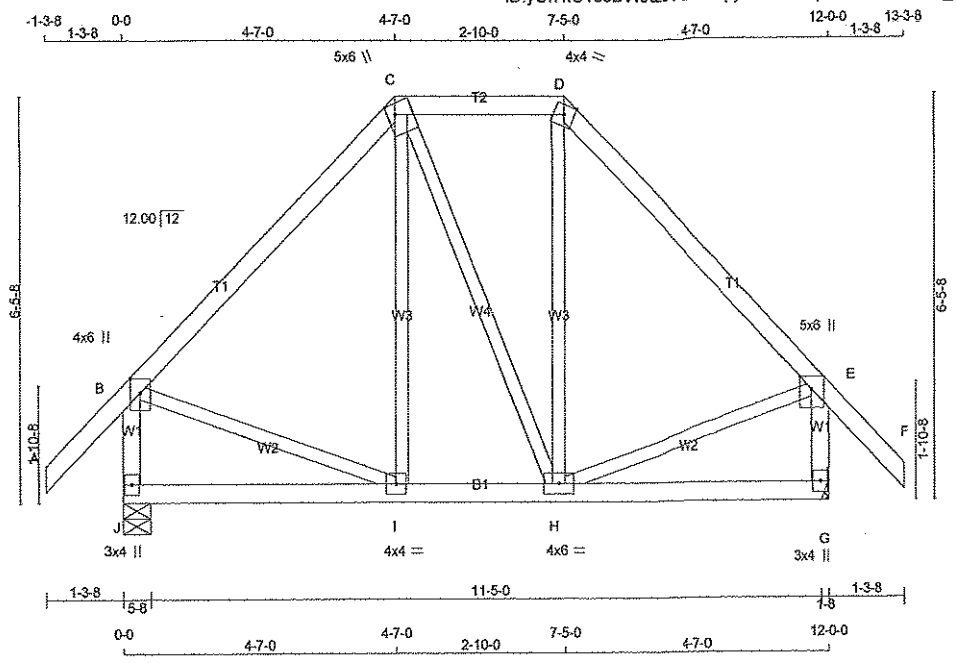


Scale = 1:37.2

TOTAL WEIGHT = 62 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1072	0	1072	0	0	IN-SX	IN-SX
G	1072	0	1072	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
G	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I- C	-26 / 141	0.03 (3)	
B- C	-602 / 0	-122.2	-122.2	0.33 (1)	6.25	C- H	0 / 1	0.00 (3)	
C- D	-424 / 0	-122.2	-122.2	0.13 (1)	6.25	H- D	-26 / 142	0.03 (3)	
D- E	-602 / 0	-122.2	-122.2	0.33 (1)	6.25	B- I	0 / 446	0.10 (1)	
E- F	0 / 60	-122.2	-122.2	0.17 (1)	10.00	H- E	0 / 448	0.10 (1)	
J- B	-1016 / 0	0.0	0.0	0.11 (1)	7.77				
G- E	-1016 / 0	0.0	0.0	0.11 (1)	7.77				
J- I	0 / 0	-28.0	-28.0	0.13 (2)	10.00				
I- H	0 / 424	-28.0	-28.0	0.16 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.13 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.33 (B-C:1) , BC=0.16 (H-I:2) , WB=0.10 (B-I:1) , SSI=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

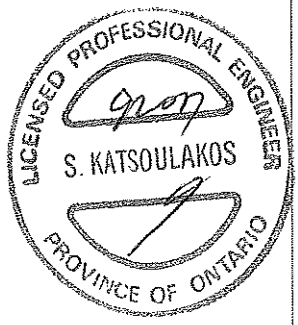
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (H) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



DWG NO. TAM 42405 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T19	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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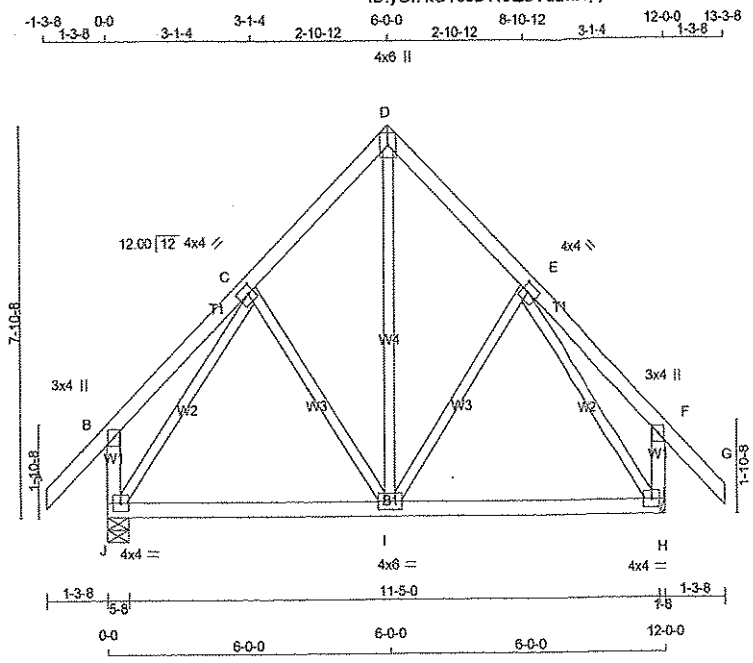


TOTAL WEIGHT = 2 X 64 = 127 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1072	0	1072	0	0	5-8	5-8
H	1072	0	1072	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	820	566 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 459	0.10 (1)
B- C	0 / 30	-122.2	-122.2	0.17 (1)	10.00	I- E	-152 / 36	0.08 (1)
C- D	-559 / 0	-122.2	-122.2	0.13 (1)	6.25	C- I	-152 / 36	0.08 (1)
D- E	-559 / 0	-122.2	-122.2	0.13 (1)	6.25	J- C	-836 / 0	0.40 (1)
E- F	0 / 30	-122.2	-122.2	0.17 (1)	10.00	E- H	-836 / 0	0.40 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00			
J- B	-309 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-309 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 461	-28.0	-28.0	0.33 (2)	10.00			
I- H	0 / 461	-28.0	-28.0	0.33 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17 (E-F:1) , BC=0.33 (H-I:2) , WB=0.40 (C-J:1) , SSI=0.14 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

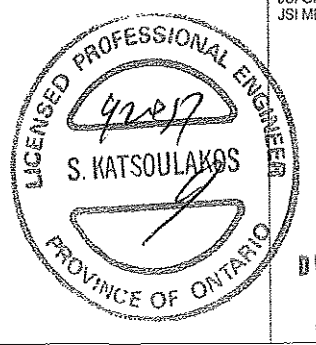
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)



DWG NO. TAM4T40617
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	G19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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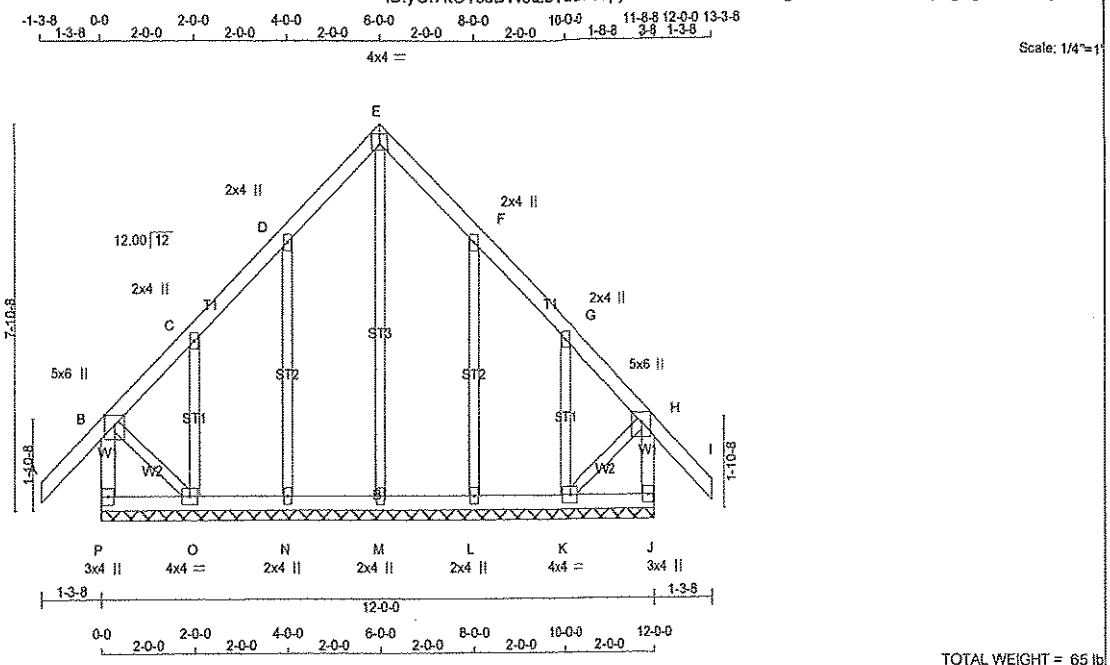

TOTAL WEIGHT = 65 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-76 / 0	-122.2	-122.2	0.16 (1)	6.25
C- D	-17 / 0	-122.2	-122.2	0.08 (1)	6.25
D- E	-42 / 0	-122.2	-122.2	0.08 (1)	6.25
E- F	-42 / 0	-122.2	-122.2	0.08 (1)	6.25
F- G	-17 / 0	-122.2	-122.2	0.08 (1)	6.25
G- H	-76 / 0	-122.2	-122.2	0.16 (1)	6.25
H- I	0 / 60	-122.2	-122.2	0.17 (1)	10.00
P- B	-387 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-387 / 0	0.0	0.0	0.04 (1)	7.81
P- O	0 / 0	-28.0	-28.0	0.03 (2)	10.00
O- N	0 / 22	-28.0	-28.0	0.03 (2)	10.00
N- M	0 / 16	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 16	-28.0	-28.0	0.02 (2)	10.00
L- K	0 / 22	-28.0	-28.0	0.03 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-170 / 0	0.19 (1)
N- D	-292 / 0	0.15 (1)
O- C	-160 / 0	0.04 (1)
L- F	-292 / 0	0.15 (1)
K- G	-160 / 0	0.04 (1)
B- O	0 / 34	0.01 (1)
K- H	0 / 34	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (H-I:1) , BC=0.03 (K-L:2) , WB=0.19 (E-M:1) , SSI=0.09 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (E) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

DWG NO. TAM 47506-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T20	6	1	TRUSS DESC.	

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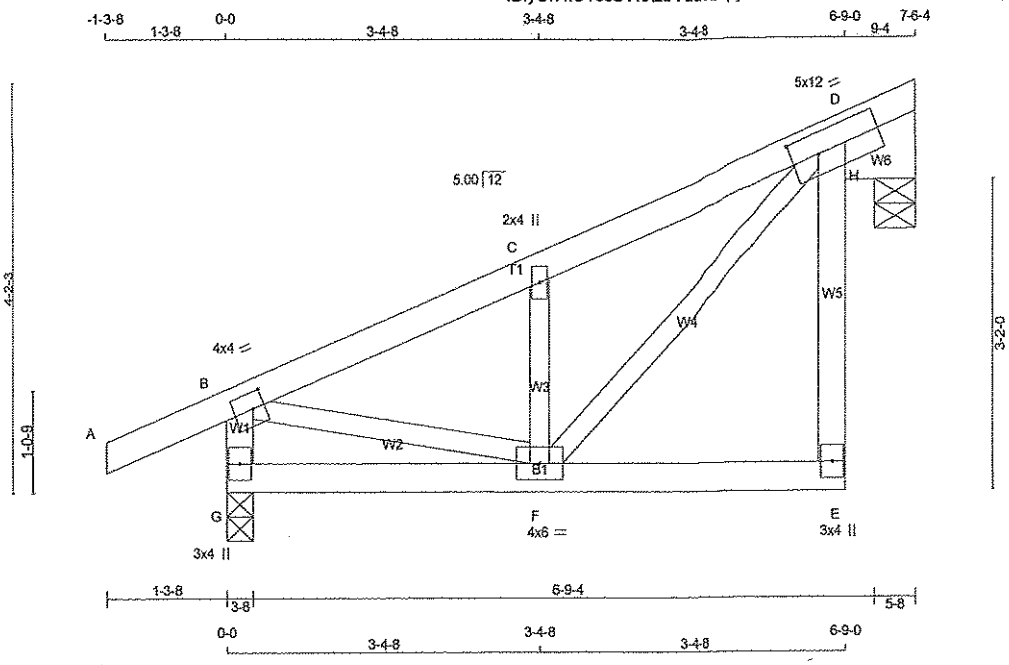

TOTAL WEIGHT = 6 X 33 = 196 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-t	MT20	5.0	12.0	2.50	3.50
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I(D)	469	0	469	0	0	5-8	5-8
G	709	0	709	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
I(D)	367	239 / 0	66 / 0	0 / 0	0 / 0	62 / 0	0 / 0
G	537	380 / 0	76 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I(D), G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 32	-122.2	-122.2	0.16 (1)	10.00		F- C	-492 / 0	0.08 (1)
B- C	-574 / 0	-122.2	-122.2	0.16 (1)	6.25		F- D	0 / 654	0.15 (1)
C- D	-577 / 0	-122.2	-122.2	0.16 (1)	6.25		B- F	0 / 559	0.13 (1)
E- H	0 / 64	0.0	0.0	0.30 (1)	10.00		D- I	-668 / 0	0.03 (1)
H- D	0 / 64	0.0	0.0	0.30 (1)	10.00		H- I	0 / 475	0.00 (1)
G- B	-667 / 0	0.0	0.0	0.07 (1)	7.81				
G- F	0 / 0	-28.0	-28.0	0.08 (3)	10.00				
F- E	0 / 71	-28.0	-28.0	0.09 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	58.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.30 (E-H:1) , BC=0.09 (E-F:3) , WB=0.15 (D-F:1) , SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

DWG NO. TAM42407-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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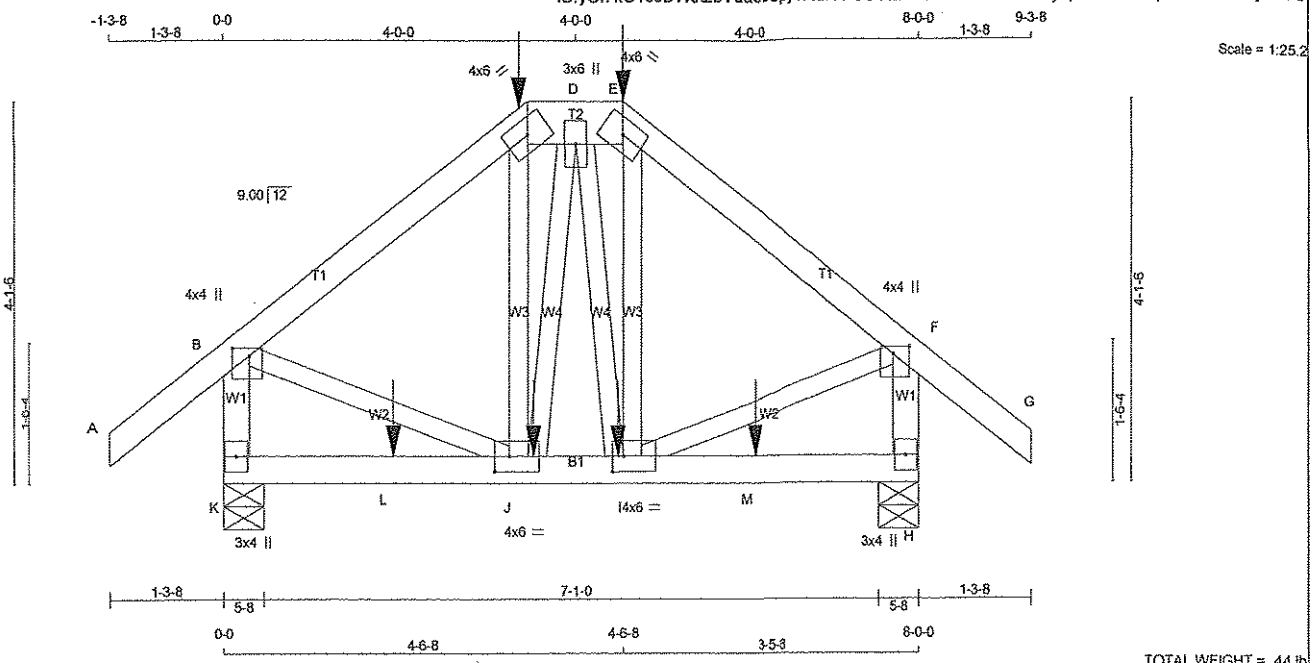


TOTAL WEIGHT = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTW-h	MT20	4.0	6.0		
D	TMWW+t	MT20	3.0	6.0		
E	TTW-h	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	6.0	2.00	1.50
J	BMWWW-t	MT20	4.0	6.0	2.00	2.00
K	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 292.3 lbs FACTORED DOWN AT 3-5-8, AND 292.3 lbs FACTORED DOWN AT 4-6-8 ON TOP CHORD, AND 27.5 lbs FACTORED DOWN AT 1-11-4, 25.3 lbs FACTORED DOWN AT 3-6-4, AND 25.3 lbs FACTORED DOWN AT 4-5-12, AND 27.5 lbs FACTORED DOWN AT 6-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1084	0	1084	0	0	5-8	5-8
H	1084	0	1084	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	817	586 / 0	111 / 0	0 / 0	0 / 0	120 / 0	0 / 0
H	817	586 / 0	111 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 51	-122.2	-122.2	0.18 (1)	10.00		J- C	-76 / 66	0.02 (1)
B- C	-748 / 0	-122.2	-122.2	0.29 (1)	6.25		I- E	-59 / 54	0.02 (1)
C- D	-603 / 0	-122.2	-122.2	0.02 (3)	6.25		B- J	0 / 637	0.16 (1)
D- E	-603 / 0	-122.2	-122.2	0.02 (3)	6.25		I- F	0 / 637	0.16 (1)
E- F	-748 / 0	-122.2	-122.2	0.29 (1)	6.25		J- D	-49 / 36	0.01 (1)
F- G	0 / 51	-122.2	-122.2	0.18 (1)	10.00		D- I	-65 / 48	0.02 (1)
K- B	-1040 / 0	0.0	0.0	0.12 (1)	7.68				
H- F	-1040 / 0	0.0	0.0	0.12 (1)	7.68				
K- L	0 / 0	-28.0	-28.0	0.09 (2)	10.00				
L- J	0 / 0	-28.0	-28.0	0.09 (2)	10.00				
J- I	0 / 612	-28.0	-28.0	0.14 (1)	10.00				
I- M	0 / 0	-28.0	-28.0	0.09 (3)	10.00				
M- H	0 / 0	-28.0	-28.0	0.09 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-5-8	-292	-292	---	FRONT	VERT	TOTAL
E	4-6-8	-292	-292	---	FRONT	VERT	TOTAL
I	4-5-12	-14	-25	---	FRONT	VERT	TOTAL
J	3-6-4	-14	-25	---	FRONT	VERT	TOTAL
L	1-11-4	-8	-28	---	FRONT	VERT	TOTAL
M	6-0-12	-8	-28	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.29 (E-F:1) , BC=0.14 (I-J:1) , WB=0.16 (F-I:1) , SSI=0.15 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

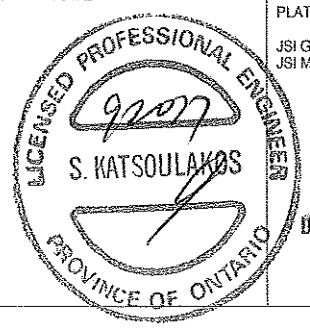
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (F) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)



DWG NO. TAM 4746017

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T22	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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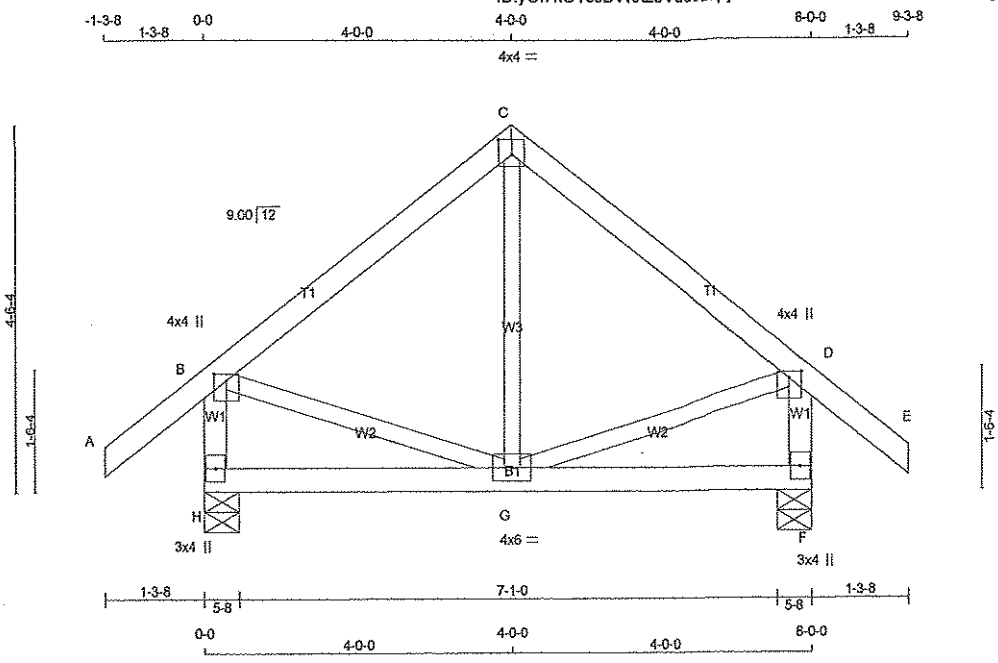

TOTAL WEIGHT = 2 X 36 = 73 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	769	0	769	0	0	5-8	5-8
F	769	0	769	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	584	411 / 0	84 / 0	0 / 0	0 / 0	88 / 0	0 / 0
F	584	411 / 0	84 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 51	-122.2	-122.2	0.17 (1)	10.00		G-C	-41 / 166	0.04 (3)
B-C	-373 / 0	-122.2	-122.2	0.25 (1)	6.25		B-G	0 / 311	0.07 (1)
C-D	-373 / 0	-122.2	-122.2	0.25 (1)	6.25		G-D	0 / 311	0.07 (1)
D-E	0 / 51	-122.2	-122.2	0.17 (1)	10.00				
H-B	-726 / 0	0.0	0.0	0.08 (1)	7.81				
F-D	-726 / 0	0.0	0.0	0.08 (1)	7.81				
H-G	0 / 0	-28.0	-28.0	0.13 (3)	10.00				
G-F	0 / 0	-28.0	-28.0	0.13 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.25 (C-D:1) , BC=0.13 (G-H:3) , WB=0.07 (D-G:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

DWG NO. TAM 47489-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
288116	T22A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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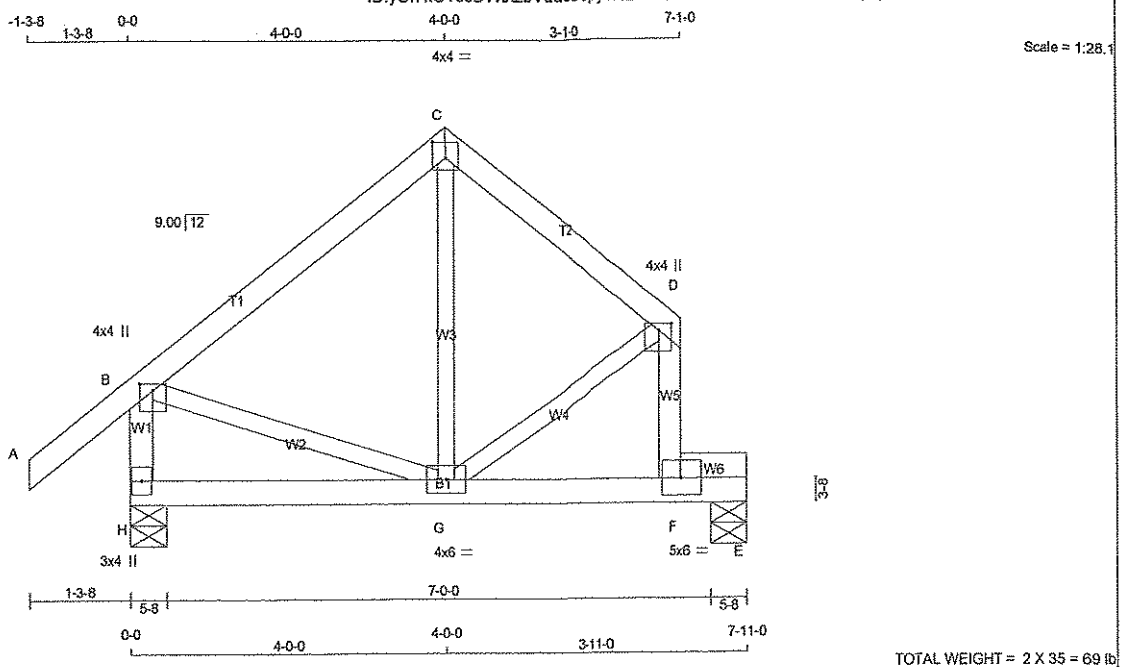


TOTAL WEIGHT = 2 X 35 = 69 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMVW-t	MT20	5.0	6.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	758	0	758	0	0	5-8	5-8
E	498	0	498	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	575	405 / 0	83 / 0	0 / 0	0 / 0	87 / 0	0 / 0
E	400	242 / 0	83 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 51	-122.2	-122.2	0.17 (1)	10.00	G- C	0 / 171	0.04 (3)
B- C	-382 / 0	-122.2	-122.2	0.34 (1)	6.25	B- G	0 / 319	0.07 (1)
C- D	-382 / 0	-122.2	-122.2	0.20 (1)	6.25	G- D	0 / 358	0.08 (1)
H- B	-733 / 0	0.0	0.0	0.08 (1)	7.81			
F- D	-604 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.18 (2)	10.00			
G- F	0 / 0	-28.0	-28.0	0.55 (1)	10.00			
F- E	0 / 0	-28.0	-28.0	0.23 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 948 (0.10")

CSI: TC=0.34 (B-C:1) , BC=0.55 (F-G:1) , WB=0.08 (D-G:1) , SSI=0.19 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

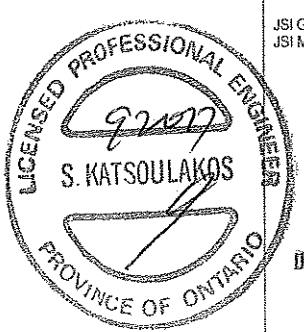
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (F) (INPUT = 0.90)
JSI METAL= 0.43 (F) (INPUT = 1.00)



DWG NO. TAM 4749017

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T23	7	1	TRUSS DESC.	

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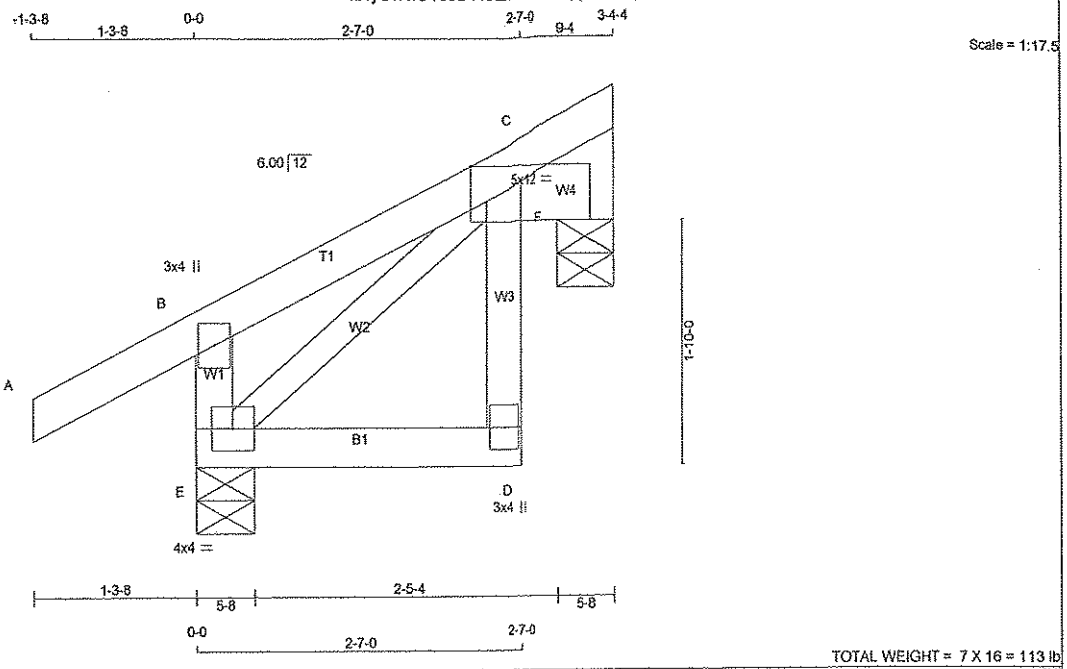


TOTAL WEIGHT = 7 X 16 = 113 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-p	MT20	5.0	12.0	Edge	1.50
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G(C)	160	0	160	0	0	5-8	5-8
E	393	0	393	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G(C)	125	82 / 0	22 / 0	0 / 0	0 / 0	21 / 0	0 / 0
E	290	220 / 0	32 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G(C), E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-122.2	-122.2	0.16 (1)	10.00	E- C	-53 / 0	0.01 (1)
B- C	0 / 0	-122.2	-122.2	0.14 (1)	10.00	C- G	-247 / 0	0.01 (1)
D- F	0 / 63	0.0	0.0	0.10 (1)	10.00	F- G	0 / 188	0.00 (1)
F- C	0 / 63	0.0	0.0	0.10 (1)	10.00			
E- B	-324 / 0	0.0	0.0	0.03 (1)	7.81			
E- D	0 / 40	-28.0	-28.0	0.06 (2)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	38.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16 (A-B:1) , BC=0.06 (D-E:2) , WB=0.01 (C-E:1) , SSI=0.12 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

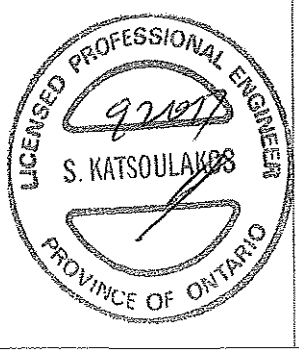
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



DWG NO. TAM47491-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288116	T24	7	1	TRUSS DESC.	

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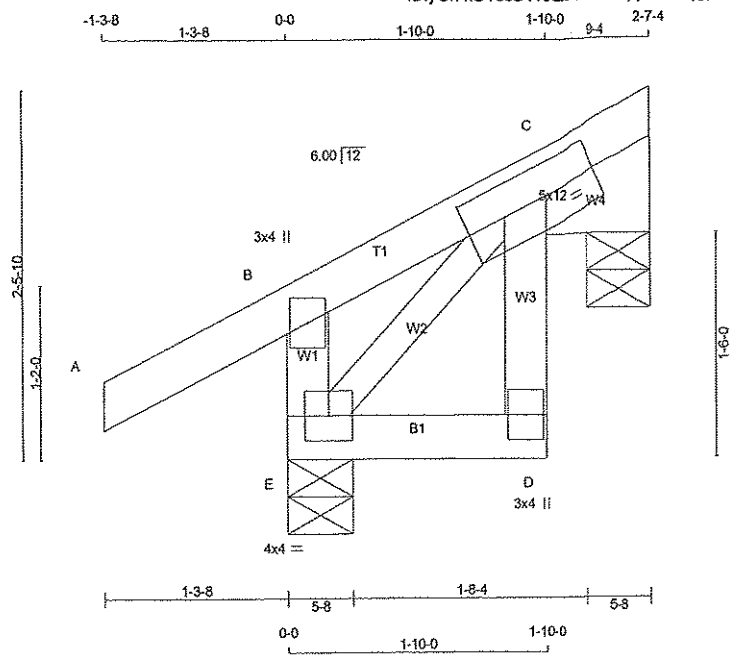

TOTAL WEIGHT = 7 X 13 = 92 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	5.0	12.0	2.50	3.25
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
C	76	0	76	0	-40	5-8	5-8
E	365	0	365	0	0	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	68	32 / -38	19 / 0	0 / 0	0 / 0	15 / 0	0 / 0
E	261	212 / 0	19 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 37	-122.2	-122.2	0.16 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	-27 / 0	-122.2	-122.2	0.15 (1)	6.25			
D- C	0 / 45	0.0	0.0	0.01 (3)	10.00			
E- B	-339 / 0	0.0	0.0	0.03 (1)	7.81			
E- D	0 / 0	-28.0	-28.0	0.03 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.16 (A-B:1) , BC=0.03 (D-E:3) , WB=0.00 (C-E:1) , SSI=0.12 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 42492-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T25S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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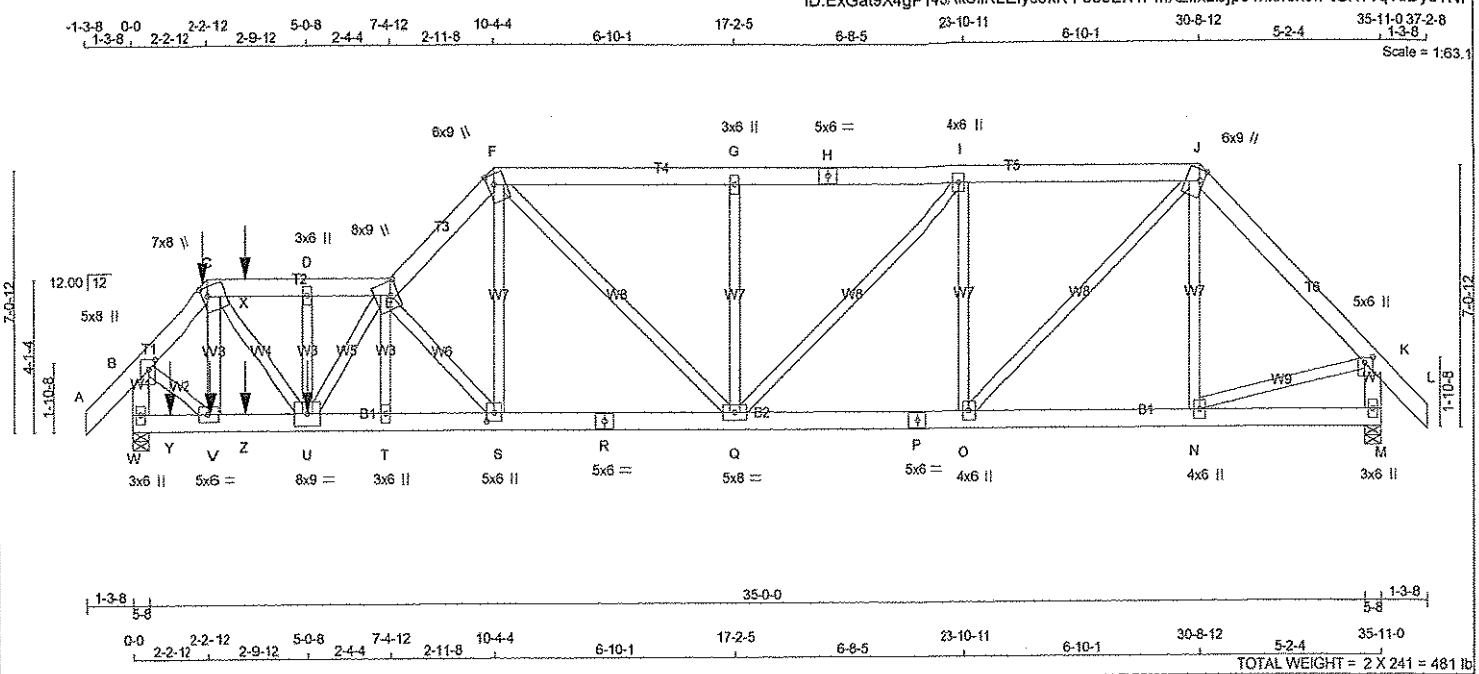

TOTAL WEIGHT = 2 X 241 = 481 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- E	2	12	SIDE(61.0)
E- F	2	12	TOP
F- H	2	12	TOP
H- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- R	2	12	SIDE(183.1)
R- P	2	12	TOP
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
D- U	1	3	SIDE(592.5)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3d INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	3.25	2.25
C	TTWW+m	MT20	7.0	8.0	3.00	1.75
D	TMW+w	MT20	3.0	6.0		
E	TTWWW+m	MT20	6.0	9.0	4.50	2.75
F	TTWW+m	MT20	6.0	9.0	3.25	2.00
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	9.0	3.50	1.50
K	TMVW+p	MT20	5.0	6.0	1.75	2.75
M	BMV1+p	MT20	3.0	6.0		
N	BMWW+t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	5969	0	5969	0	0	5-8	5-8
M	3330	0	3330	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4614	3096 / 0	763 / 0	0 / 0	0 / 0	755 / 0	0 / 0
M	2578	1723 / 0	432 / 0	0 / 0	0 / 0	423 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.05 (1)		10.00
B- C	-5141 / 0	-122.2	-122.2	0.06 (1)		5.11
C- X	-6971 / 0	-122.2	-122.2	0.15 (1)		4.43
X- D	-6971 / 0	-122.2	-122.2	0.15 (1)		4.43
D- E	-6971 / 0	-122.2	-122.2	0.11 (1)		4.47
E- F	-6684 / 0	-122.2	-122.2	0.11 (1)		4.56
F- G	-5037 / 0	-122.2	-122.2	0.24 (1)		4.97
G- H	-5038 / 0	-122.2	-122.2	0.25 (1)		4.96
H- I	-5038 / 0	-122.2	-122.2	0.25 (1)		4.96
I- J	-4183 / 0	-122.2	-122.2	0.23 (1)		5.35
J- K	-3143 / 0	-122.2	-122.2	0.18 (1)		6.04
K- L	0 / 63	-122.2	-122.2	0.05 (1)		10.00
W- B	-5969 / 0	0.0	0.0	0.22 (1)		6.04
M- K	-3275 / 0	0.0	0.0	0.12 (1)		7.67
W- Y	0 / 0	-28.0	-28.0	0.07 (1)		10.00
Y- V	0 / 0	-28.0	-28.0	0.07 (1)		10.00
V- Z	0 / 3531	-28.0	-28.0	0.32 (1)		10.00
Z- U	0 / 3531	-28.0	-28.0	0.32 (1)		10.00
U- T	0 / 7933	-28.0	-28.0	0.61 (1)		10.00
T- S	0 / 7934	-28.0	-28.0	0.61 (1)		10.00
S- R	0 / 4814	-28.0	-28.0	0.35 (1)		10.00
R- Q	0 / 4814	-28.0	-28.0	0.35 (1)		10.00
Q- P	0 / 4184	-28.0	-28.0	0.30 (1)		10.00
P- O	0 / 4184	-28.0	-28.0	0.30 (1)		10.00
O- N	0 / 2204	-28.0	-28.0	0.18 (1)		10.00
N- M	0 / 0	-28.0	-28.0	0.07 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1618 / 0	0.14 (1)
U- D	0 / 5583	0.49 (1)
U- D	-51 / 116	0.01 (3)
U- E	-1761 / 0	0.18 (1)
T- E	-32 / 40	0.00 (3)
E- S	-4912 / 0	0.62 (1)
S- F	0 / 3952	0.35 (1)
N- J	-353 / 123	0.10 (1)
B- V	0 / 4116	0.36 (1)
N- K	0 / 2279	0.20 (1)
O- J	0 / 2742	0.24 (1)
F- Q	0 / 310	0.03 (1)
O- I	-1719 / 0	0.47 (1)
Q- G	-863 / 0	0.24 (1)
Q- I	0 / 1209	0.11 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	2-2-12	-13	-14	---	FRONT	VERT	DEAD
C	2-2-12	-151	-151	---	BACK	VERT	DEAD
C	2-2-12	-183	-183	---	FRONT	VERT	SNOW
U	5-0-8	-2935	-2935	---	BACK	VERT	TOTAL
V	2-3-8	-40	-70	---	BACK	VERT	TOTAL
X	3-3-8	-147	-147	---	BACK	VERT	TOTAL
Y	1-0-12	-40	-70	---	BACK	VERT	TOTAL
Z	3-3-8	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.25 (G-I:1) , BC=0.61 (T-U:1) , WB=0.62 (E-S:1) , SSI=0.16 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.44 (V) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47497-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	5.0	6.0	2.75	2.50
T	BMW+w	MT20	3.0	6.0		
U	BMWWW-t	MT20	8.0	9.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 2-2-12, AND 150.6 lbs FACTORED DOWN AT 2-2-12, AND 147.1 lbs FACTORED DOWN AT 3-3-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-3-8, AND 69.9 lbs FACTORED DOWN AT 3-3-8, AND 2934.9 lbs FACTORED DOWN AT 5-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47497-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T26S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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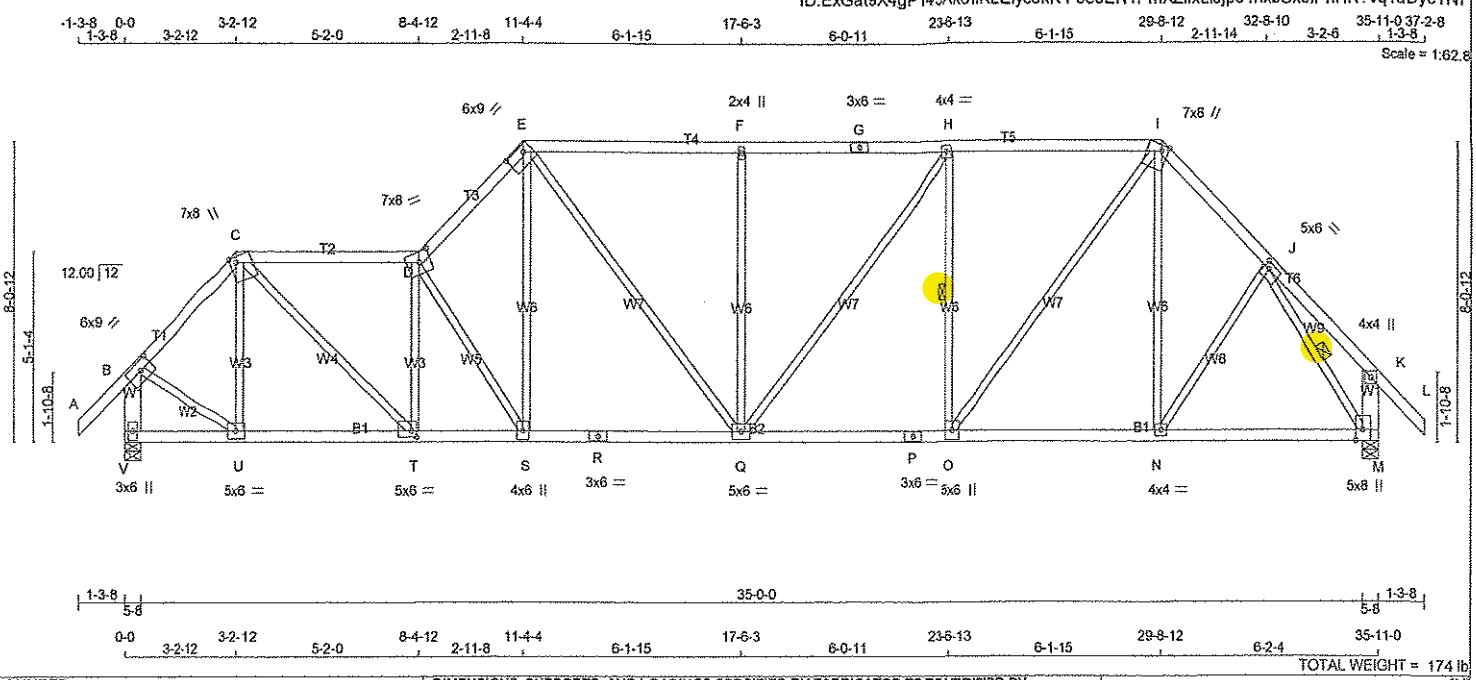

TOTAL WEIGHT = 174 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TTWW-m	MT20	7.0	8.0	3.25	4.00
E	TTWW-h	MT20	6.0	9.0	2.00	6.25
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.00	1.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	5.0	8.0	Edge	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.00	1.75
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
V	2868	0	2868	0	0	IN-SX 5-8	IN-SX 5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-M, H-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)		10.00
B- C	-2399 / 0	-122.2	-122.2	0.31 (1)		4.15
C- D	-3589 / 0	-122.2	-122.2	0.86 (1)		2.76
D- E	-3807 / 0	-122.2	-122.2	0.39 (1)		3.32
E- F	-3182 / 0	-122.2	-122.2	0.96 (1)		2.88
F- G	-3183 / 0	-122.2	-122.2	0.96 (1)		2.86
G- H	-3183 / 0	-122.2	-122.2	0.96 (1)		2.86
H- I	-2879 / 0	-122.2	-122.2	0.92 (1)		3.05
I- J	-2584 / 0	-122.2	-122.2	0.26 (1)		4.09
J- K	0 / 27	-122.2	-122.2	0.16 (1)		10.00
K- L	0 / 60	-122.2	-122.2	0.17 (1)		10.00
V- B	-2843 / 0	0.0	0.0	0.21 (1)		6.22
M- K	-319 / 0	0.0	0.0	0.02 (1)		7.81
V- U	0 / 0	-28.0	-28.0	0.15 (2)		10.00
U- T	0 / 1675	-28.0	-28.0	0.37 (1)		10.00
T- S	0 / 3627	-28.0	-28.0	0.65 (1)		10.00
S- R	0 / 2714	-28.0	-28.0	0.55 (1)		10.00
R- Q	0 / 2714	-28.0	-28.0	0.55 (1)		10.00
Q- P	0 / 2879	-28.0	-28.0	0.58 (1)		10.00
P- O	0 / 2879	-28.0	-28.0	0.58 (1)		10.00
O- N	0 / 1800	-28.0	-28.0	0.45 (2)		10.00
N- M	0 / 1653	-28.0	-28.0	0.43 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-617 / 0	0.24 (1)
C- T	0 / 2633	0.59 (1)
T- D	-1729 / 0	0.66 (1)
D- S	-1733 / 0	0.93 (1)
S- E	0 / 1601	0.36 (1)
N- I	-11 / 190	0.04 (3)
N- J	0 / 255	0.06 (1)
B- U	0 / 1656	0.42 (1)
J- M	-2979 / 0	0.66 (1)
O- I	0 / 1732	0.39 (1)
E- Q	0 / 753	0.17 (1)
O- H	-1195 / 0	0.49 (1)
Q- F	-801 / 0	1.00 (1)
Q- H	0 / 494	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.96 (E-F:1) , BC=0.65 (S-T:1) , WB=1.00 (F-Q:1) , SSI=0.35 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.79 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47498-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T27S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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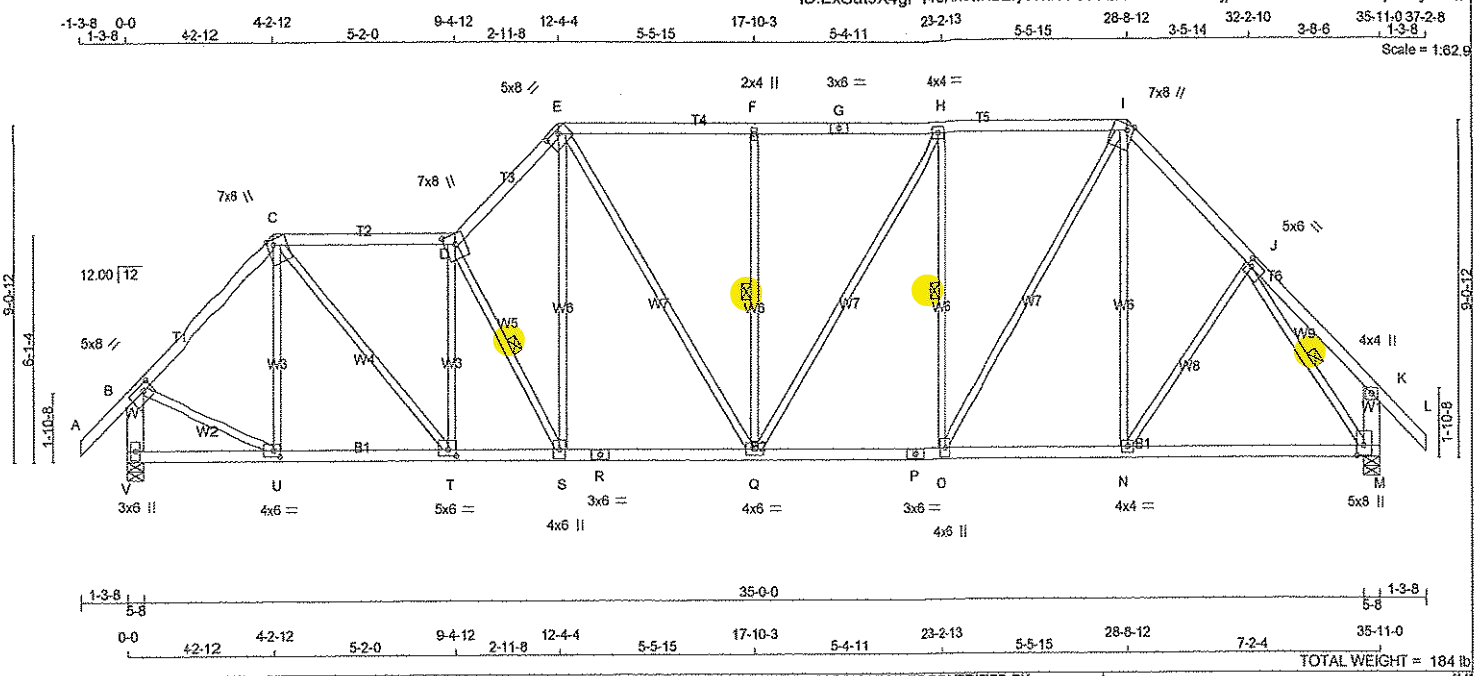

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TTWW+m	MT20	7.0	8.0	3.25	3.50
E	TTWW-h	MT20	5.0	8.0	1.25	4.50
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.50
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	5.0	8.0	3.25	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.00	2.75
U	BMWW-t	MT20	4.0	6.0	2.00	2.25
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-S, J-M, H-O, F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	1 (1)	0.17	10.00
B- C	-2503 / 0	-122.2	-122.2	1 (1)	0.56	3.83
C- D	-3208 / 0	-122.2	-122.2	1 (1)	0.81	2.98
D- E	-3518 / 0	-122.2	-122.2	1 (1)	0.36	3.47
E- F	-2809 / 0	-122.2	-122.2	1 (1)	0.68	3.43
F- G	-2809 / 0	-122.2	-122.2	1 (1)	0.69	3.41
G- H	-2809 / 0	-122.2	-122.2	1 (1)	0.69	3.41
H- I	-2568 / 0	-122.2	-122.2	1 (1)	0.67	3.58
I- J	-2574 / 0	-122.2	-122.2	1 (1)	0.36	4.00
J- K	0 / 34	-122.2	-122.2	1 (1)	0.23	10.00
K- L	0 / 60	-122.2	-122.2	1 (1)	0.17	10.00
V- B	-2825 / 0	0.0	0.0	1 (1)	0.21	6.23
M- K	-340 / 0	0.0	0.0	1 (1)	0.03	7.81
V- U	0 / 0	-28.0	-28.0	2 (2)	0.16	10.00
U- T	0 / 1755	-28.0	-28.0	1 (1)	0.38	10.00
T- S	0 / 3237	-28.0	-28.0	1 (1)	0.57	10.00
S- R	0 / 2506	-28.0	-28.0	1 (1)	0.50	10.00
R- Q	0 / 2506	-28.0	-28.0	1 (1)	0.50	10.00
Q- P	0 / 2568	-28.0	-28.0	1 (1)	0.51	10.00
P- O	0 / 2568	-28.0	-28.0	1 (1)	0.51	10.00
O- N	0 / 1791	-28.0	-28.0	1 (2)	0.51	10.00
N- M	0 / 1710	-28.0	-28.0	1 (2)	0.49	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
U- C	-458 / 30	0.27 (1)
C- T	0 / 2206	0.50 (1)
T- D	-1570 / 0	0.92 (1)
D- S	-1600 / 0	0.47 (1)
S- E	0 / 1538	0.35 (1)
N- I	0 / 245	0.06 (3)
N- J	0 / 173	0.04 (2)
B- U	0 / 1869	0.42 (1)
J- M	-2983 / 0	0.78 (1)
O- I	0 / 1451	0.33 (1)
E- Q	0 / 570	0.13 (1)
O- H	-1108 / 0	0.59 (1)
Q- F	-711 / 0	0.38 (1)
Q- H	0 / 460	0.10 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.81 (C-D:1) , BC=0.57 (S-T:1) , WB=0.92 (D-T:1) , SSI=0.31 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.89 (T) (INPUT = 0.90)
USI METAL= 0.78 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47499-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T28S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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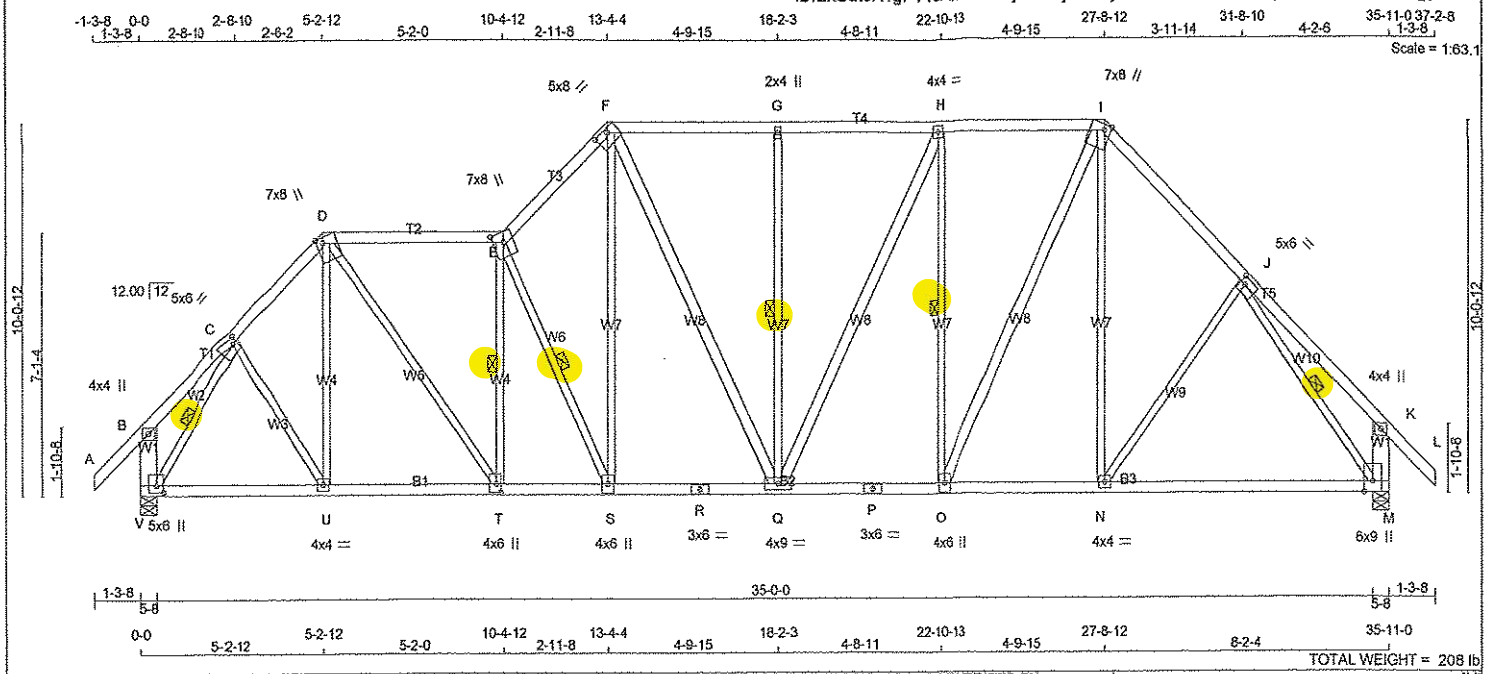

TOTAL WEIGHT = 208 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
F - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TTWW+m	MT20	7.0	8.0	3.25	3.50
F	TTWW-h	MT20	5.0	8.0	1.25	4.50
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1+p	MT20	6.0	9.0	Edge	
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW+t	MT20	4.0	6.0	2.75	1.50
U	BMWW-t	MT20	4.0	4.0		
V	BMVW1+p	MT20	5.0	6.0	2.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2868	0	2868	0	0	5-8	5-8
M	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
M	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T, E-S, C-V, J-M, H-O, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	-2 / 19	-122.2	-122.2	0.10 (1)	10.00	
C- D	-2569 / 0	-122.2	-122.2	0.19 (1)	4.16	
D- E	-2917 / 0	-122.2	-122.2	0.77 (1)	3.16	
E- F	-3266 / 0	-122.2	-122.2	0.33 (1)	3.61	
F- G	-2512 / 0	-122.2	-122.2	0.49 (1)	3.85	
G- H	-2512 / 0	-122.2	-122.2	0.50 (1)	3.83	
H- I	-2320 / 0	-122.2	-122.2	0.48 (1)	3.97	
I- J	-2547 / 0	-122.2	-122.2	0.48 (1)	3.69	
J- K	0 / 40	-122.2	-122.2	0.32 (1)	10.00	
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
V- B	-300 / 0	0.0	0.0	0.02 (1)	7.81	
M- K	-361 / 0	0.0	0.0	0.03 (1)	7.81	
V- U	0 / 1575	-28.0	-28.0	0.37 (1)	10.00	
U- T	0 / 1791	-28.0	-28.0	0.40 (1)	10.00	
T- S	0 / 2939	-28.0	-28.0	0.50 (1)	10.00	
S- R	0 / 2326	-28.0	-28.0	0.46 (1)	10.00	
R- Q	0 / 2326	-28.0	-28.0	0.46 (1)	10.00	
Q- P	0 / 2321	-28.0	-28.0	0.46 (1)	10.00	
P- O	0 / 2321	-28.0	-28.0	0.46 (1)	10.00	
O- N	0 / 1769	-28.0	-28.0	0.60 (2)	10.00	
N- M	0 / 1752	-28.0	-28.0	0.58 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- U	0 / 389	0.09 (1)
U- D	-147 / 113	0.13 (1)
D- T	0 / 1883	0.42 (1)
T- E	-1413 / 0	0.45 (1)
E- S	-1529 / 0	0.57 (1)
S- F	0 / 1497	0.34 (1)
N- I	0 / 319	0.07 (2)
N- J	0 / 135	0.03 (3)
V- C	-2963 / 0	0.58 (1)
J- M	-2980 / 0	0.93 (1)
O- I	0 / 1243	0.20 (1)
F- Q	0 / 420	0.07 (1)
O- H	-1022 / 0	0.71 (1)
Q- G	-620 / 0	0.43 (1)
Q- H	0 / 439	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.77 (D-E:1) , BC=0.60 (N-O:2) , WB=0.93 (J-M:1) , SSI=0.27 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 0.73 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47502 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T29S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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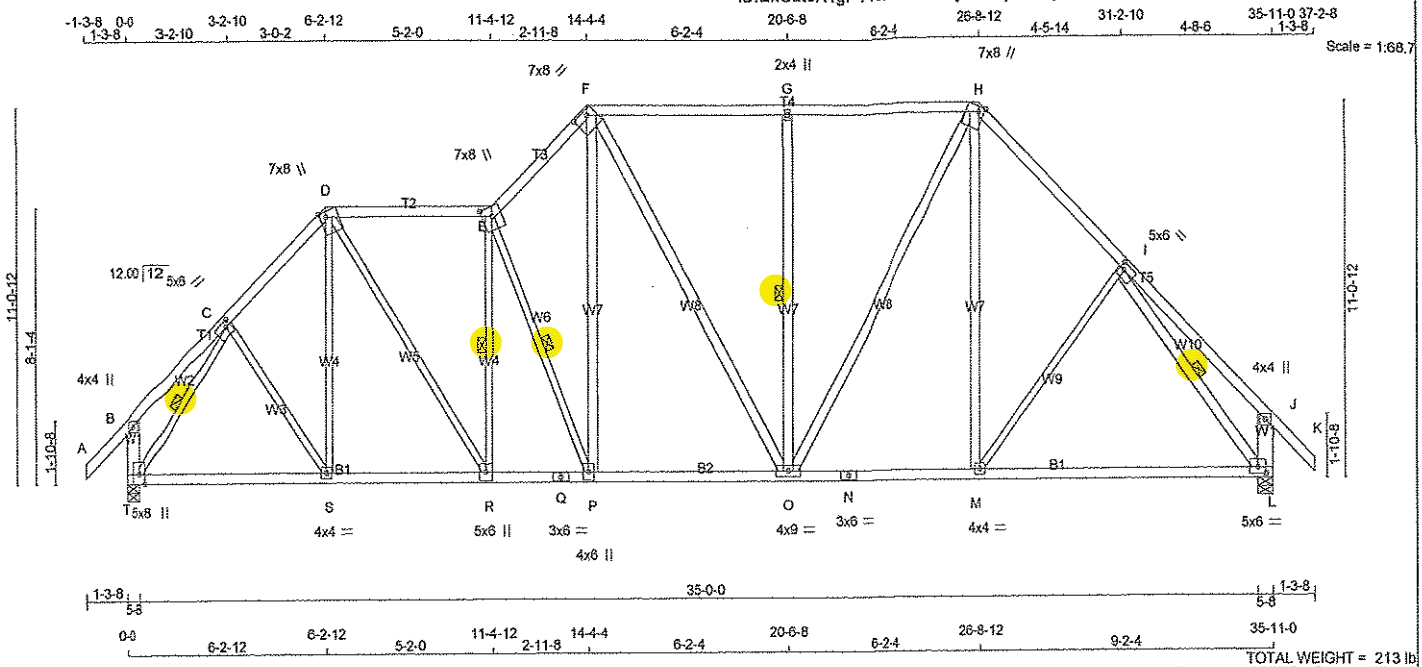

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	1.75
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TTWW+m	MT20	7.0	8.0	3.25	3.50
F	TTWW-h	MT20	7.0	8.0	1.75	5.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0	2.25	1.75
J	TMV+p	MT20	4.0	4.0		
L	BMWW1-t	MT20	5.0	6.0	2.25	3.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW+t	MT20	5.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMW1+p	MT20	5.0	8.0	Edge	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2868	0	2868	0	0	5-8	5-8
L	2868	0	2868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0
L	2225	1480 / 0	377 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, E-P, G-O, C-T, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 27	-122.2	-122.2	0.16 (1)	10.00	
C- D	-2584 / 0	-122.2	-122.2	0.22 (1)	4.14	
D- E	-2679 / 0	-122.2	-122.2	0.52 (1)	3.70	
E- F	-3042 / 0	-122.2	-122.2	0.25 (1)	3.83	
F- G	-2260 / 0	-122.2	-122.2	0.69 (1)	3.67	
G- H	-2260 / 0	-122.2	-122.2	0.69 (1)	3.67	
H- I	-2507 / 0	-122.2	-122.2	0.45 (1)	3.97	
I- J	0 / 47	-122.2	-122.2	0.42 (1)	10.00	
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
T- B	-320 / 0	0.0	0.0	0.02 (1)	7.81	
L- J	-382 / 0	0.0	0.0	0.03 (1)	7.81	
T- S	0 / 1655	-28.0	-28.0	0.43 (2)	10.00	
S- R	0 / 1800	-28.0	-28.0	0.45 (2)	10.00	
R- Q	0 / 2696	-28.0	-28.0	0.52 (1)	10.00	
Q- P	0 / 2696	-28.0	-28.0	0.52 (1)	10.00	
P- O	0 / 2171	-28.0	-28.0	0.44 (1)	10.00	
O- N	0 / 1739	-28.0	-28.0	0.71 (2)	10.00	
N- M	0 / 1739	-28.0	-28.0	0.71 (2)	10.00	
M- L	0 / 1782	-28.0	-28.0	0.70 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX FORCE CSI (LC)
C- S	0 / 250	0.05 (1)
S- D	-5 / 186	0.04 (3)
D- R	0 / 1611	0.36 (1)
R- E	-1280 / 0	0.53 (1)
E- P	-1472 / 0	0.70 (1)
P- F	0 / 1519	0.24 (1)
F- O	0 / 178	0.03 (1)
O- G	-918 / 0	0.58 (1)
O- H	0 / 1044	0.17 (1)
M- H	0 / 413	0.07 (2)
M- I	-76 / 114	0.09 (1)
T- C	-2979 / 0	0.67 (1)
I- L	-2972 / 0	0.80 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	58.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 833 (0.52")

CSI: TC=0.69 (F-G:1) , BC=0.71 (M-O:2) , WB=0.80 (I-L:1) , SSI=0.37 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.74 (I) (INPUT = 1.00)

[M][F]

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4750 1-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T30	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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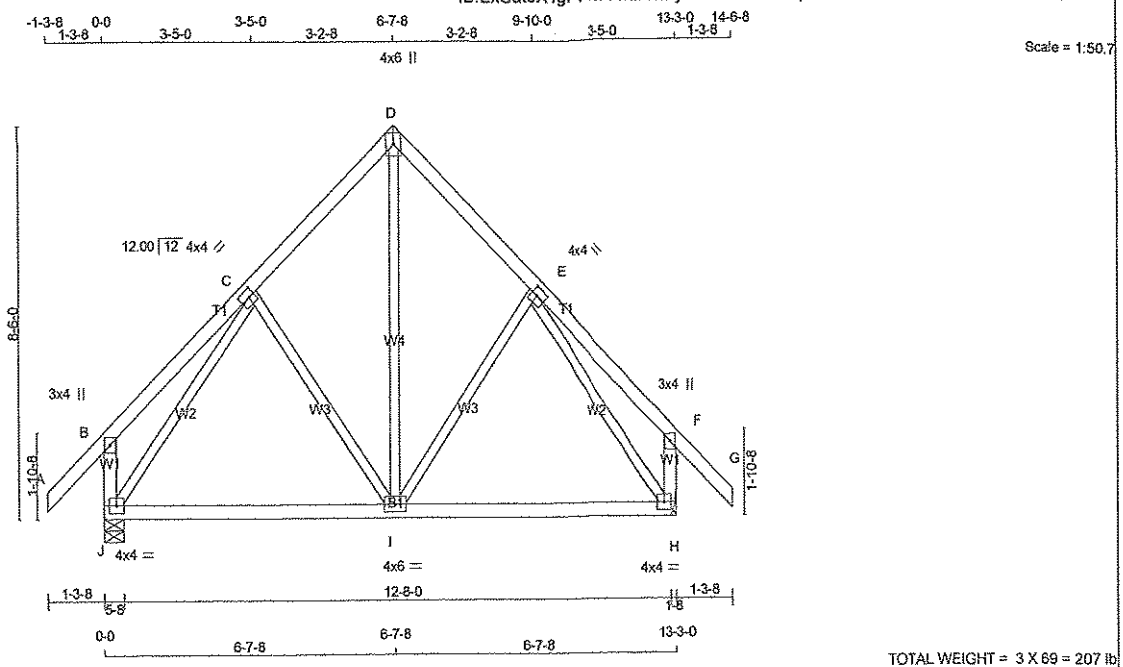

TOTAL WEIGHT = 3 X 69 = 207 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1166	0	1166	0	0	5-8	5-8
H	1166	0	1166	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	893	613 / 0	139 / 0	0 / 0	0 / 0	141 / 0	0 / 0
H	893	613 / 0	139 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 525	0.12 (1)
B- C	0 / 33	-122.2	-122.2	0.21 (1)	10.00	I- E	-181 / 35	0.11 (1)
C- D	-629 / 0	-122.2	-122.2	0.17 (1)	6.25	C- I	-181 / 35	0.11 (1)
D- E	-629 / 0	-122.2	-122.2	0.17 (1)	6.25	J- C	-934 / 0	0.53 (1)
E- F	0 / 33	-122.2	-122.2	0.21 (1)	10.00	E- H	-934 / 0	0.53 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00			
J- B	-323 / 0	0.0	0.0	0.04 (1)	7.81			
H- F	-323 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 526	-28.0	-28.0	0.41 (2)	10.00			
I- H	0 / 526	-28.0	-28.0	0.41 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.21 (B-C:1) , BC=0.41 (I-J:2) , WB=0.53 (E-H:1) , SSI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9107
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47502-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	G30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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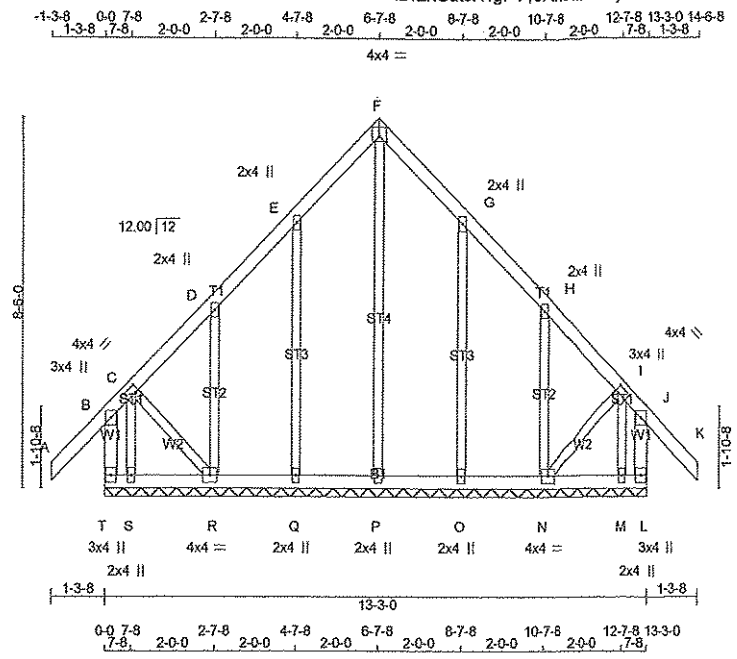

TOTAL WEIGHT = 74 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D, E, G, H						
D	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, O, P, Q, S						
M	BMW1+w	MT20	2.0	4.0		
N	BMWW1-t	MT20	4.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	-112 / 0	-122.2	-122.2	0.16 (1)	6.25	
C- D	-53 / 0	-122.2	-122.2	0.05 (1)	6.25	
D- E	-48 / 0	-122.2	-122.2	0.07 (1)	6.25	
E- F	-63 / 0	-122.2	-122.2	0.07 (1)	6.25	
F- G	-63 / 0	-122.2	-122.2	0.07 (1)	6.25	
G- H	-48 / 0	-122.2	-122.2	0.07 (1)	6.25	
H- I	-53 / 0	-122.2	-122.2	0.05 (1)	6.25	
I- J	-112 / 0	-122.2	-122.2	0.16 (1)	6.25	
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
T- B	-367 / 0	0.0	0.0	0.04 (1)	7.81	
L- J	-367 / 0	0.0	0.0	0.04 (1)	7.81	
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
S- R	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
R- Q	0 / 37	-28.0	-28.0	0.03 (2)	10.00	
Q- P	0 / 32	-28.0	-28.0	0.02 (2)	10.00	
P- O	0 / 32	-28.0	-28.0	0.02 (2)	10.00	
O- N	0 / 37	-28.0	-28.0	0.03 (2)	10.00	
N- M	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-144 / 0	0.20 (1)
Q- E	-277 / 0	0.19 (1)
R- D	-236 / 0	0.07 (1)
S- C	-75 / 0	0.01 (1)
O- G	-277 / 0	0.19 (1)
N- H	-236 / 0	0.07 (1)
M- I	-75 / 0	0.01 (1)
C- R	0 / 63	0.01 (1)
N- I	0 / 63	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (J-K:1) , BC=0.03 (Q-R:2) , WB=0.20 (F-P:1) , SSI=0.11 (I-J:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (F) (INPUT = 0.90)
JSI METAL= 0.08 (G) (INPUT = 1.00)

DWG NO. TAM 47507-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T31	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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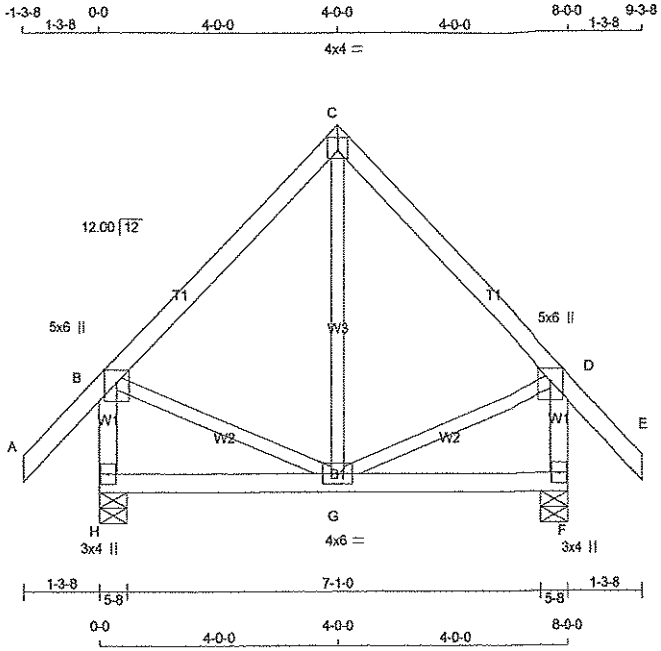


TOTAL WEIGHT = 4 X 41 = 163 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	771	0	771	0	0	5-8	5-8
F	771	0	771	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	585	413 / 0	84 / 0	0 / 0	0 / 0	88 / 0	0 / 0
F	585	413 / 0	84 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	G- C	-35 / 168	0.04 (3)	
B- C	-321 / 0	-122.2	-122.2	0.25 (1)	6.25	B- G	0 / 243	0.05 (1)	
C- D	-321 / 0	-122.2	-122.2	0.25 (1)	6.25	G- D	0 / 243	0.05 (1)	
D- E	0 / 60	-122.2	-122.2	0.17 (1)	10.00				
H- B	-728 / 0	0.0	0.0	0.08 (1)	7.81				
F- D	-728 / 0	0.0	0.0	0.08 (1)	7.81				
H- G	0 / 0	-28.0	-28.0	0.13 (3)	10.00				
G- F	0 / 0	-28.0	-28.0	0.13 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.25 (B-C:1) , BC=0.13 (F-G:3) , WB=0.05 (B-G:1) , SSI=0.13 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

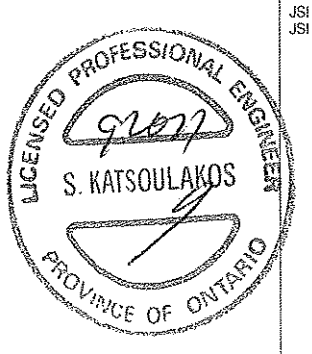
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (C) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



DWG NO. TAM 42503-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	G31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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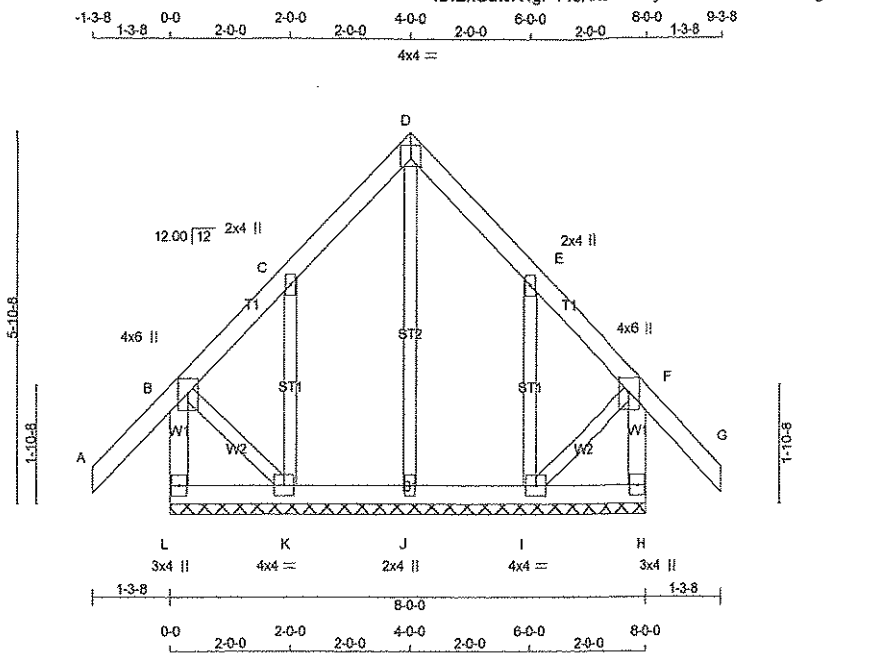

TOTAL WEIGHT = 43 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	J- D	-195 / 0	0.10 (1)
B- C	-62 / 0	-122.2	-122.2	0.16 (1)	6.25	K- C	-209 / 0	0.05 (1)
C- D	-32 / 0	-122.2	-122.2	0.07 (1)	6.25	I- E	-209 / 0	0.05 (1)
D- E	-32 / 0	-122.2	-122.2	0.07 (1)	6.25	B- K	0 / 27	0.01 (1)
E- F	-62 / 0	-122.2	-122.2	0.16 (1)	6.25	I- F	0 / 27	0.01 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00			
L- B	-369 / 0	0.0	0.0	0.04 (1)	7.81			
H- F	-369 / 0	0.0	0.0	0.04 (1)	7.81			
L- K	0 / 0	-28.0	-28.0	0.03 (3)	10.00			
K- J	0 / 15	-28.0	-28.0	0.03 (2)	10.00			
J- I	0 / 15	-28.0	-28.0	0.03 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.03 (J-K:2) , WB=0.10 (D-J:1) , SSI=0.09 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (D) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47508-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T31A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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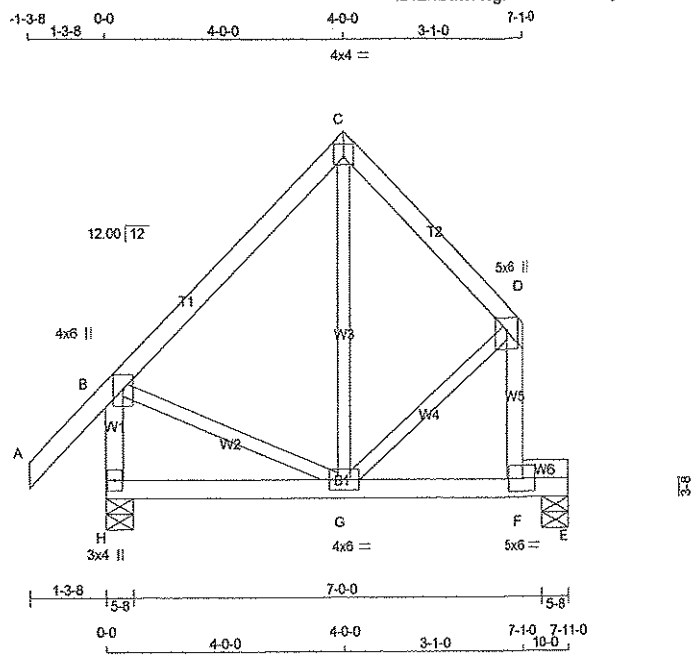

TOTAL WEIGHT = 2 X 39 = 78 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	5.0	6.0	Edge	
F	BMVW-t	MT20	5.0	6.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	760	0	760	0	0	5-8	5-8
E	498	0	498	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	576	406 / 0	83 / 0	0 / 0	0 / 0	87 / 0	0 / 0
E	400	242 / 0	83 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00		G- C	0 / 174	0.04 (3)
B- C	-328 / 0	-122.2	-122.2	0.34 (1)	6.25		B- G	0 / 248	0.06 (1)
C- D	-328 / 0	-122.2	-122.2	0.20 (1)	6.25		G- D	0 / 296	0.07 (1)
H- B	-736 / 0	0.0	0.0	0.08 (1)	7.81				
F- D	-604 / 0	0.0	0.0	0.09 (1)	7.81				
H- G	0 / 0	-28.0	-28.0	0.18 (2)	10.00				
G- F	0 / 0	-28.0	-28.0	0.55 (1)	10.00				
F- E	0 / 0	-28.0	-28.0	0.23 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 953 (0.10")

CSI: TC=0.34 (B-C:1) , BC=0.55 (F-G:1) , WB=0.07 (D-G:1) , SSI=0.19 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (F) (INPUT = 0.90)
JSI METAL= 0.43 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47504-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288251	T32	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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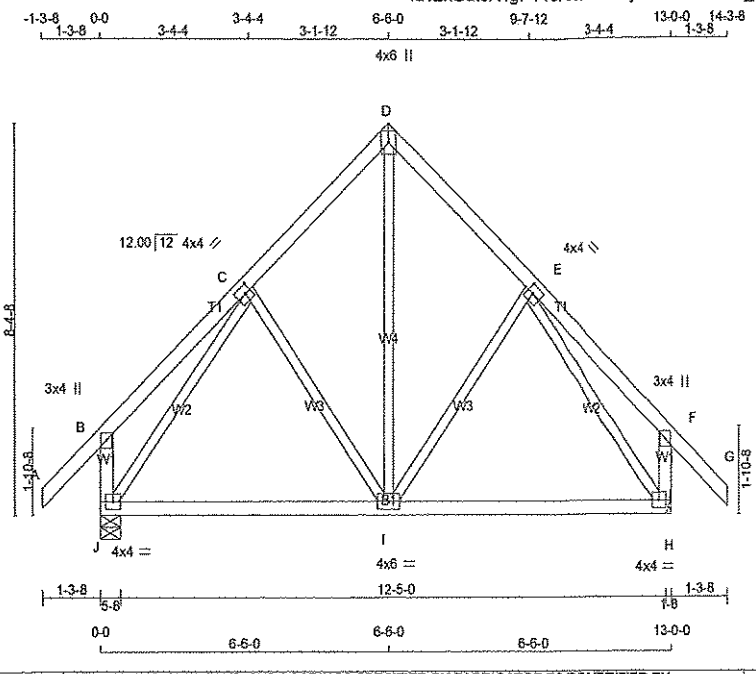

TOTAL WEIGHT = 4 X 68 = 271 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1147	0	1147	0	0	5-8	5-8
H	1147	0	1147	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	879	604 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	879	604 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 32	-122.2	-122.2	0.21 (1)	10.00	
C- D	-615 / 0	-122.2	-122.2	0.16 (1)	6.25	
D- E	-615 / 0	-122.2	-122.2	0.16 (1)	6.25	
E- F	0 / 32	-122.2	-122.2	0.21 (1)	10.00	
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
J- B	-320 / 0	0.0	0.0	0.04 (1)	7.81	
H- F	-320 / 0	0.0	0.0	0.04 (1)	7.81	
J- I	0 / 513	-28.0	-28.0	0.39 (2)	10.00	
I- H	0 / 513	-28.0	-28.0	0.39 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 512	0.12 (1)
I- E	-175 / 35	0.10 (1)
C- I	-175 / 35	0.10 (1)
J- C	-914 / 0	0.50 (1)
E- H	-914 / 0	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.21 (E-F:1) , BC=0.39 (H-I:2) , WB=0.50 (E-H:1) , SSI=0.15 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

DWG NO. TAM 475/2-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288251	G32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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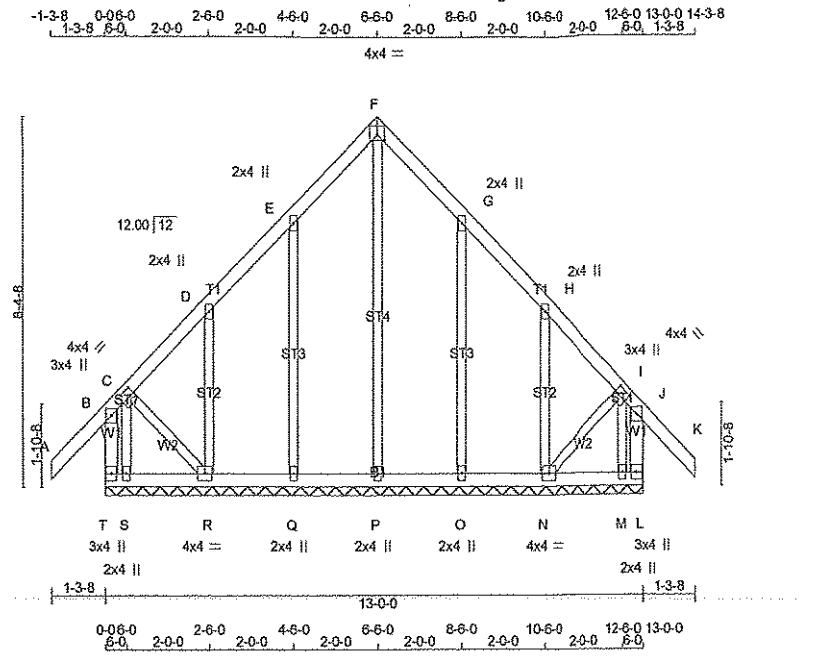


TOTAL WEIGHT = 73 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D, E, G, H						
D	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, O, P, Q, S						
M	BMW1+w	MT20	2.0	4.0		
N	BMWW1-t	MT20	4.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	P- F	-137 / 0	0.18 (1)
B- C	-131 / 0	-122.2	-122.2	0.16 (1)	6.25	Q- E	-278 / 0	0.18 (1)
C- D	-61 / 0	-122.2	-122.2	0.05 (1)	6.25	R- D	-231 / 0	0.07 (1)
D- E	-52 / 0	-122.2	-122.2	0.07 (1)	6.25	S- C	-49 / 0	0.01 (1)
E- F	-68 / 0	-122.2	-122.2	0.07 (1)	6.25	O- G	-278 / 0	0.18 (1)
F- G	-68 / 0	-122.2	-122.2	0.07 (1)	6.25	N- H	-231 / 0	0.07 (1)
G- H	-52 / 0	-122.2	-122.2	0.07 (1)	6.25	M- I	-49 / 0	0.01 (1)
H- I	-61 / 0	-122.2	-122.2	0.05 (1)	6.25	C- R	0 / 66	0.01 (1)
I- J	-131 / 0	-122.2	-122.2	0.16 (1)	6.25	N- I	0 / 66	0.01 (1)
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00			
T- B	-386 / 0	0.0	0.0	0.04 (1)	7.81			
L- J	-386 / 0	0.0	0.0	0.04 (1)	7.81			
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
S- R	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
R- Q	0 / 40	-28.0	-28.0	0.03 (2)	10.00			
Q- P	0 / 36	-28.0	-28.0	0.02 (2)	10.00			
P- O	0 / 36	-28.0	-28.0	0.02 (2)	10.00			
O- N	0 / 40	-28.0	-28.0	0.03 (2)	10.00			
N- M	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (J-K:1) , BC=0.03 (N-O:2) , WB=0.18 (F-P:1) , SSI=0.12 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

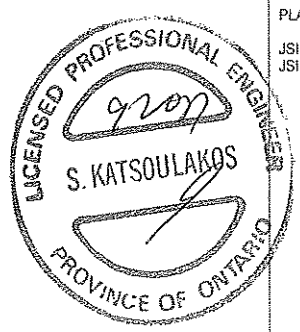
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (F) (INPUT = 0.90)
JSI METAL= 0.08 (G) (INPUT = 1.00)



DWG NO. TAM 475 11-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T110	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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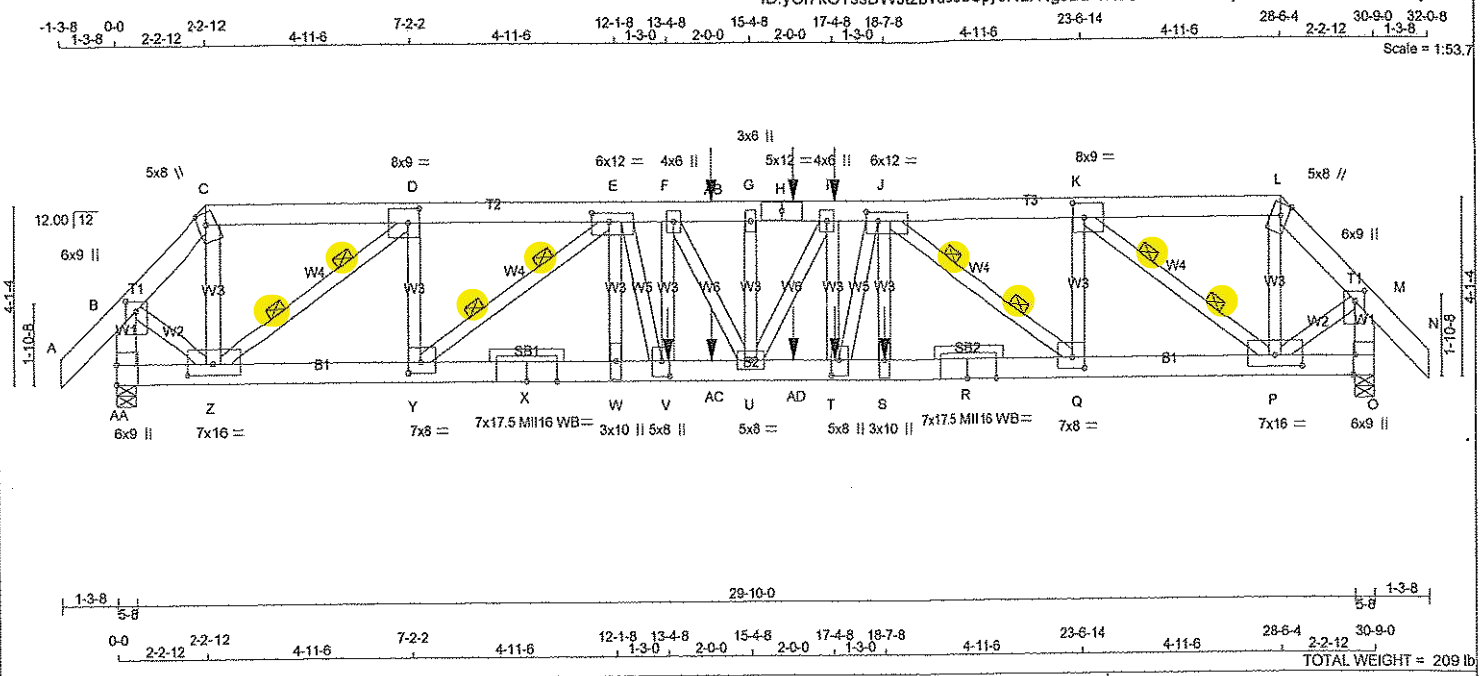


TOTAL WEIGHT = 209 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - H	2x6	DRY	1650F 1.5E	SPF
H - L	2x6	DRY	1650F 1.5E	SPF
L - N	2x6	DRY	No.2	SPF
AA- B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
AA- X	2x6	DRY	2100F 1.8E	SPF
X - R	2x6	DRY	2100F 1.8E	SPF
R - O	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0	2.75	2.75
C	TTW+m	MT20	5.0	8.0	Edge	2.25
D	TMWW-t	MT20	8.0	9.0	4.00	3.25
E	TMWWW-t	MT20	6.0	12.0	2.50	5.00
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	12.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMWWW-t	MT20	6.0	12.0	2.50	3.50
K	TMWW-t	MT20	8.0	9.0	4.00	3.25
L	TTW+m	MT20	5.0	8.0	Edge	2.25
M	TMVW+p	MT20	6.0	9.0	2.75	2.75
O	BMV1+t	MT20	6.0	9.0	Edge	0.50
P	BMWWW-t	MT20	7.0	16.0	3.00	6.00
Q	BMWW-t	MT20	7.0	8.0	3.00	3.50
R	BS-t	MII16	7.0	17.5		
S	BMW+w	MT20	3.0	10.0		
T	BMWW+t	MT20	5.0	8.0	4.25	2.25
U	BMWWW-t	MT20	5.0	8.0		
V	BMWW+t	MT20	5.0	8.0	4.25	2.25
W	BMW+w	MT20	3.0	10.0		
X	BS-t	MII16	7.0	17.5		
Y	BMWW-t	MT20	7.0	8.0	3.00	3.50
Z	BMWWW-t	MT20	7.0	16.0	3.00	8.00
AA	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 147.1 lbs FACTORED DOWN AT 14-5-0, AND 147.1 lbs FACTORED DOWN AT 16-5-0, AND 147.1 lbs FACTORED DOWN AT 17-5-0 ON TOP CHORD, AND 2252.0 lbs FACTORED DOWN AT 13-4-8, 69.9 lbs FACTORED DOWN AT 14-5-0, 69.9 lbs FACTORED DOWN AT 16-5-0, AND 69.9 lbs FACTORED DOWN AT 17-5-0, AND 2368.4 lbs FACTORED DOWN AT 18-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
AA	4963	0	4963	0	0	5-8	5-8
O	5193	0	5193	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	3803	2611 / 0	592 / 0	0 / 0	0 / 0	600 / 0	0 / 0
O	3977	2735 / 0	617 / 0	0 / 0	0 / 0	626 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF D-Z, E-Y, J-Q, K-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	10 (1)		10.00
B- C	-4242 / 0	-122.2	-122.2	15 (1)		4.08
C- D	-3073 / 0	-122.2	-122.2	17 (1)		5.22
D- E	-8468 / 0	-122.2	-122.2	34 (1)		3.31
E- F	-13709 / 0	-122.2	-122.2	64 (1)		2.34
F-AB	-13946 / 0	-122.2	-122.2	67 (1)		2.28
AB- G	-13946 / 0	-122.2	-122.2	67 (1)		2.28
G- H	-13946 / 0	-122.2	-122.2	63 (1)		2.34
H- I	-13946 / 0	-122.2	-122.2	63 (1)		2.34
I- J	-13876 / 0	-122.2	-122.2	59 (1)		2.39
J- K	-8952 / 0	-122.2	-122.2	40 (1)		3.18
K- L	-3220 / 0	-122.2	-122.2	17 (1)		5.13
L- M	-4443 / 0	-122.2	-122.2	16 (1)		3.99
M- N	0 / 63	-122.2	-122.2	10 (1)		10.00
AA- B	-4996 / 0	0.0	0.0	37 (1)		4.78
O- M	-5219 / 0	0.0	0.0	39 (1)		4.67
AA- Z	0 / 0	-28.0	-28.0	06 (1)		10.00
Z- Y	0 / 8468	-28.0	-28.0	51 (1)		10.00
Y- X	0 / 12858	-28.0	-28.0	75 (1)		10.00
X- W	0 / 12858	-28.0	-28.0	75 (1)		10.00
W- V	0 / 12858	-28.0	-28.0	79 (1)		10.00
V-AC	0 / 13709	-28.0	-28.0	84 (1)		10.00
AC- U	0 / 13709	-28.0	-28.0	84 (1)		10.00
U-AD	0 / 13876	-28.0	-28.0	82 (1)		10.00
AD- T	0 / 13876	-28.0	-28.0	82 (1)		10.00
T- S	0 / 13517	-28.0	-28.0	87 (1)		10.00
S- R	0 / 13517	-28.0	-28.0	87 (1)		10.00
R- Q	0 / 13517	-28.0	-28.0	87 (1)		10.00
Q- P	0 / 8952	-28.0	-28.0	53 (1)		10.00
P- O	0 / 0	-28.0	-28.0	06 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- C	0 / 2618	0.46 (1)
Z- D	-6736 / 0	0.91 (1)
Y- D	0 / 3360	0.59 (1)
Y- E	-5482 / 0	0.74 (1)
W- E	0 / 252	0.04 (2)
E- V	0 / 2624	0.46 (1)
V- F	-354 / 0	0.06 (1)
T- I	-426 / 0	0.07 (1)
T- J	0 / 1107	0.20 (1)
S- J	0 / 2017	0.36 (1)
J- Q	-5700 / 0	0.77 (1)
Q- K	0 / 3585	0.63 (1)
K- P	-7157 / 0	0.97 (1)
P- L	0 / 2756	0.49 (1)
B- Z	0 / 3396	0.60 (1)
P- M	0 / 3558	0.63 (1)
U- I	0 / 147	0.03 (1)
F- U	0 / 494	0.09 (1)
U- G	-581 / 0	0.10 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 680 (0.54")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 451 (0.82")

CSI: TC=0.67 (F-G:1) , BC=0.87 (S-T:1) , WB=0.97 (K-P:1) , SSI=0.25 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.96 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9207 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM4750S - 17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288118	T110	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	16-5-0	-147	-147	---	FRONT	VERT	TOTAL
I	17-5-0	-147	-147	---	FRONT	VERT	TOTAL
S	18-7-8	-2368	-2368	---	FRONT	VERT	TOTAL
T	17-5-0	-40	-70	---	FRONT	VERT	TOTAL
V	13-4-8	-2252	-2252	---	FRONT	VERT	TOTAL
AB	14-5-0	-147	-147	---	FRONT	VERT	TOTAL
AC	14-5-0	-40	-70	---	FRONT	VERT	TOTAL
AD	16-5-0	-40	-70	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9/20/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47505-17
STRUCTURAL
COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

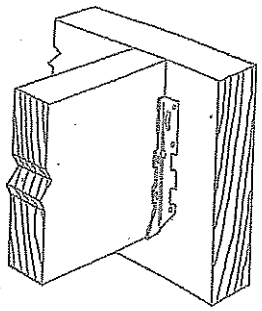
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

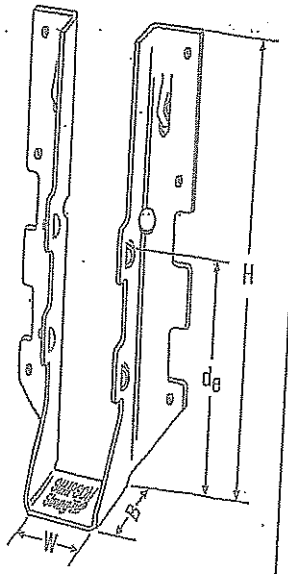
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



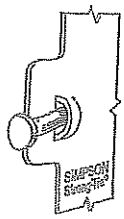
Typical LUS Installation



LUS26

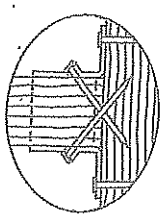
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_g[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⅜	3⅛	1¾	11⅝	4-10d	2-10d	710	1630	645	1165
LUS24-2	18	3⅛	3⅛	2	11⅝	4-16d	2-16d	835	2020	590	1435
LUS26	18	1⅜	4¾	1¾	8⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅜	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1⅜	6⅝	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅛	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1⅜	7⅞	1¾	3⅞	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅛	8⅞	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

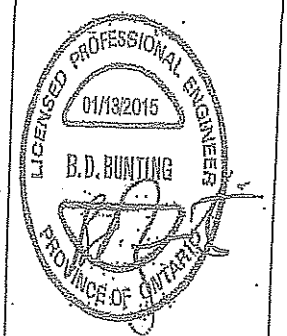


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

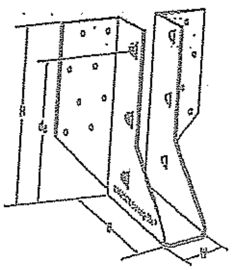
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

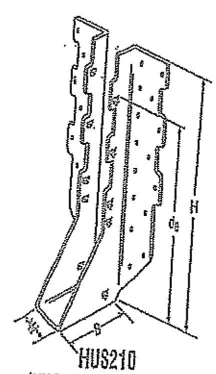
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

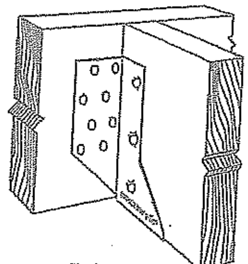
- See current catalogue for options

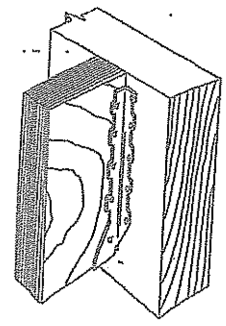


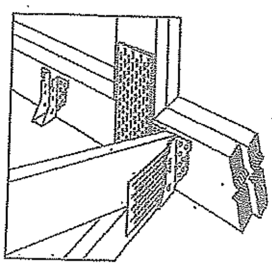
LJS26DS



HUS210
(HUS26, HUS28, similar)

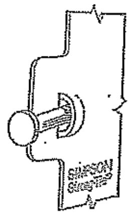

Typical LJS26DS Installation


Typical HUS Installation


Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

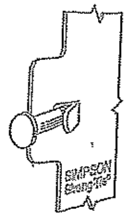
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_g[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1 9/16	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

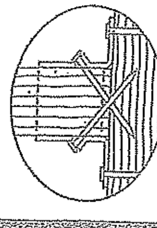


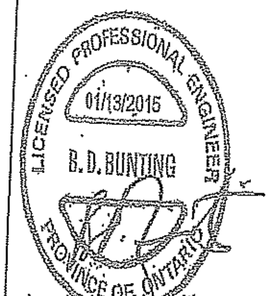
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.

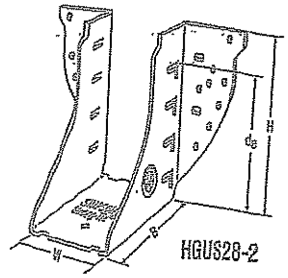

Double Shear Nailing Top View.



HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



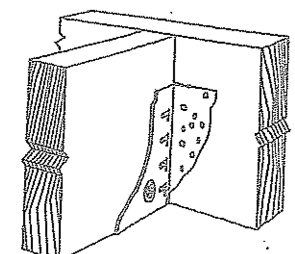
HGUS28-2

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



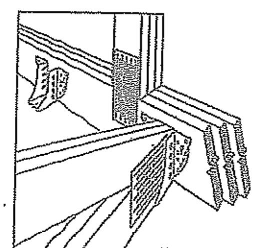
Typical HGUS Installation

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

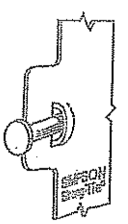
- See current catalogue for options.



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

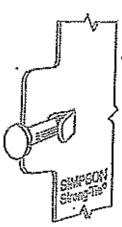
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 7/16	5 7/16	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 7/16	7 3/16	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3 7/16	9 3/16	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	46-16d	16-16d	6840	14646	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

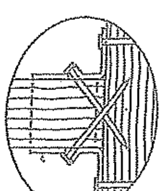


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

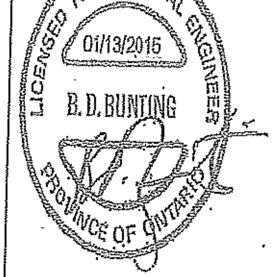
U.S. Patent 5,603,580



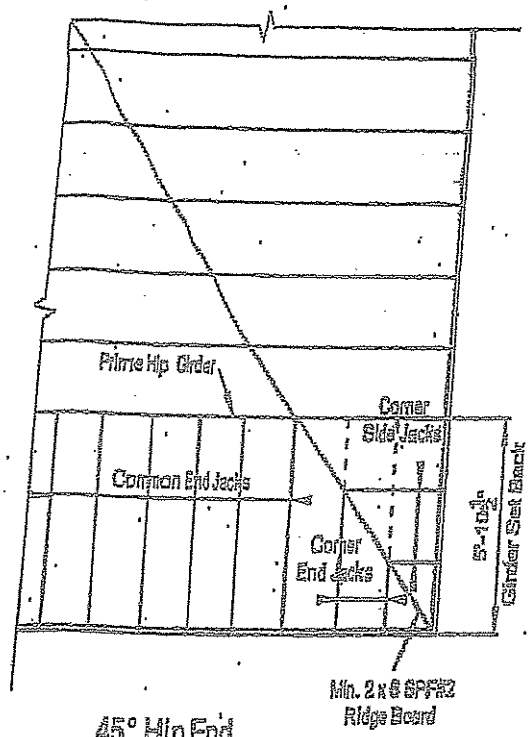
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



MICRO CITY
ENGINEERING SERVICES INC.
TEL: (619) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

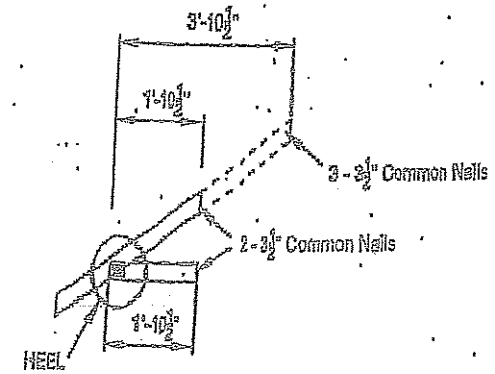
TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

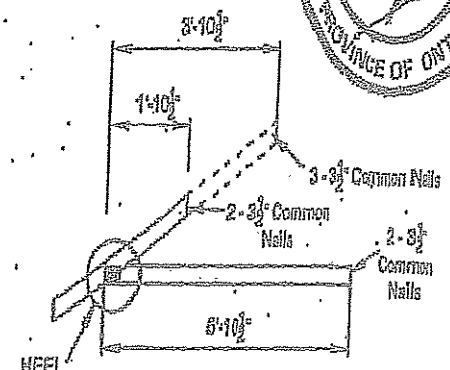
TOP CHORD LIVE LOAD	:	39.4 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F
TOTAL LOAD		59.9 P.S.F.

DWG NO TMM2485.1A
STRUCTURAL COMPONENT ONLY

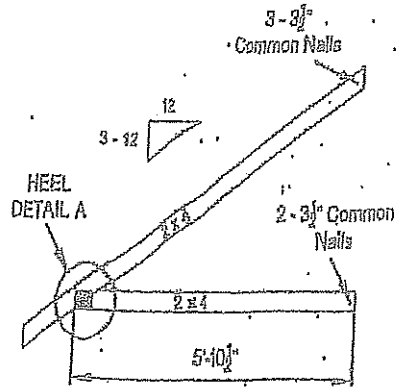
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



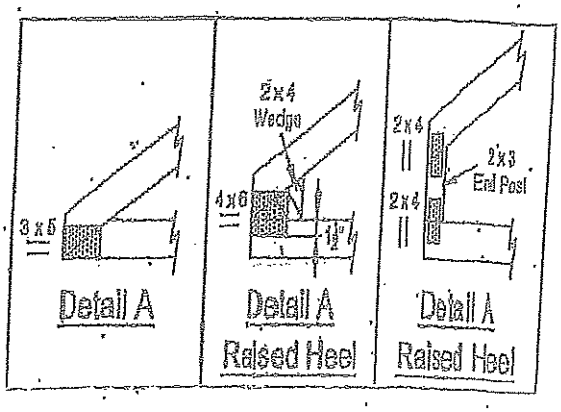
Corner Side Jacks



Corner End Jacks



Common End Jacks



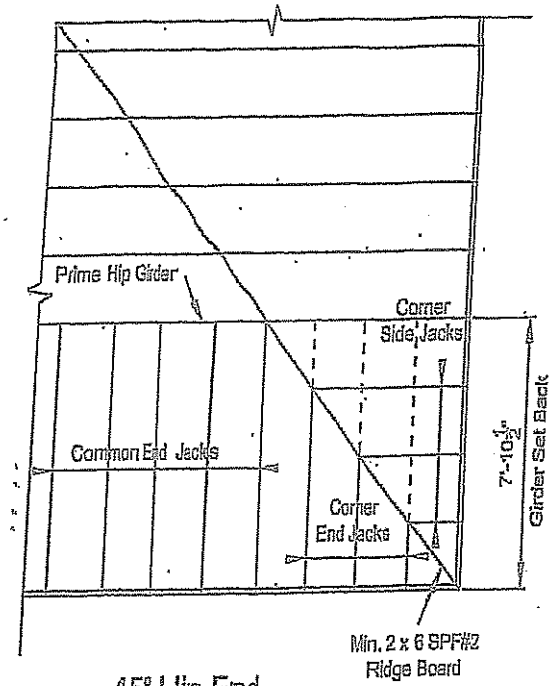
Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



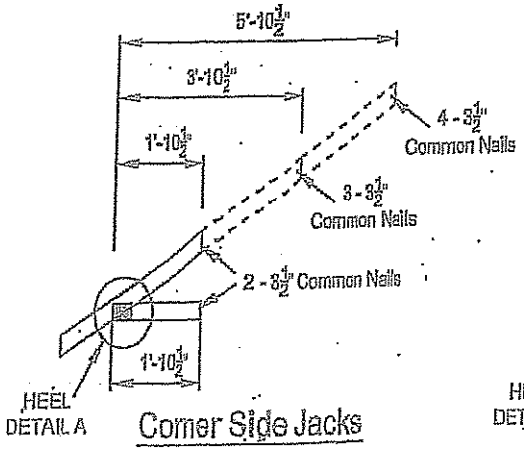
45° Hip End

DESIGN LOAD:

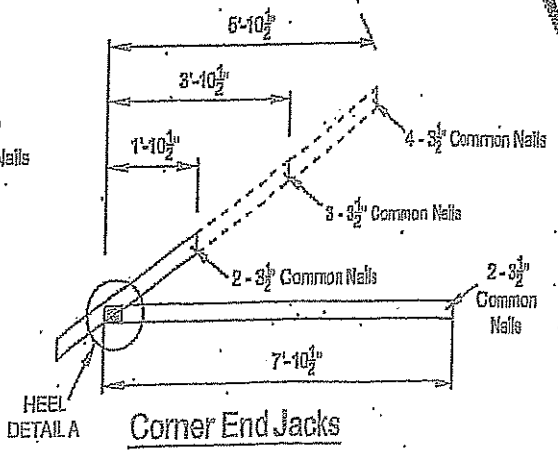
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TRM 3503 TA
STRUCTURAL COMPONENT ONLY

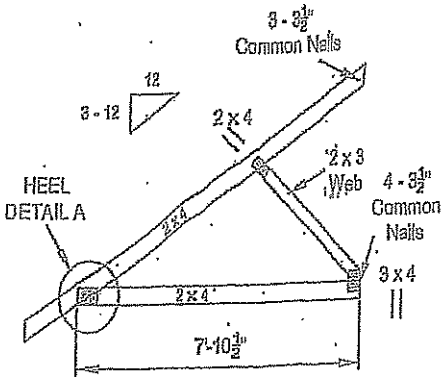
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



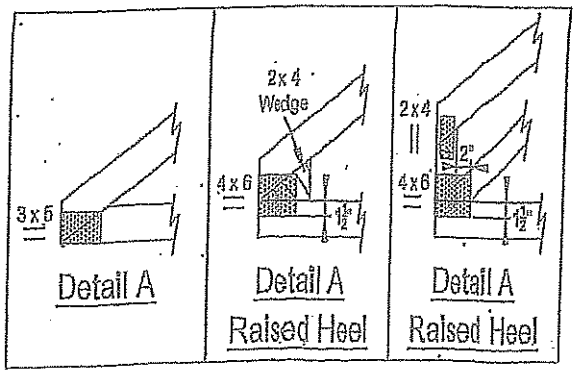
Corner Side Jacks



Corner End Jacks



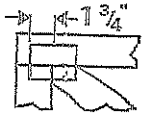
Common End Jacks



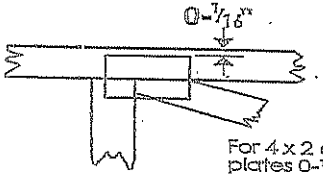
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

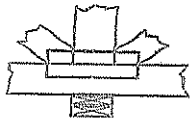
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

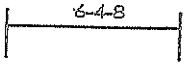
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

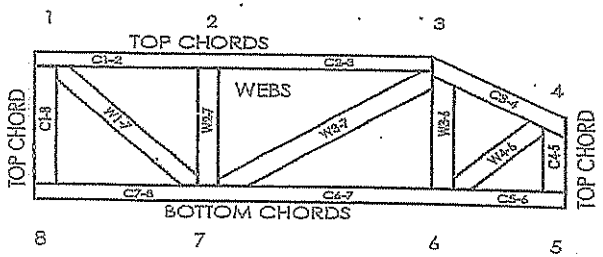
DSB-89: *Design Standard for Bracing.*

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS
CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

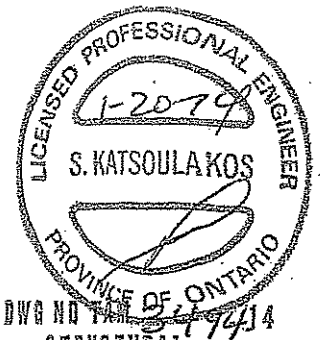
MiTek®
POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



DWG NO T[illegible] 2-14/14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.