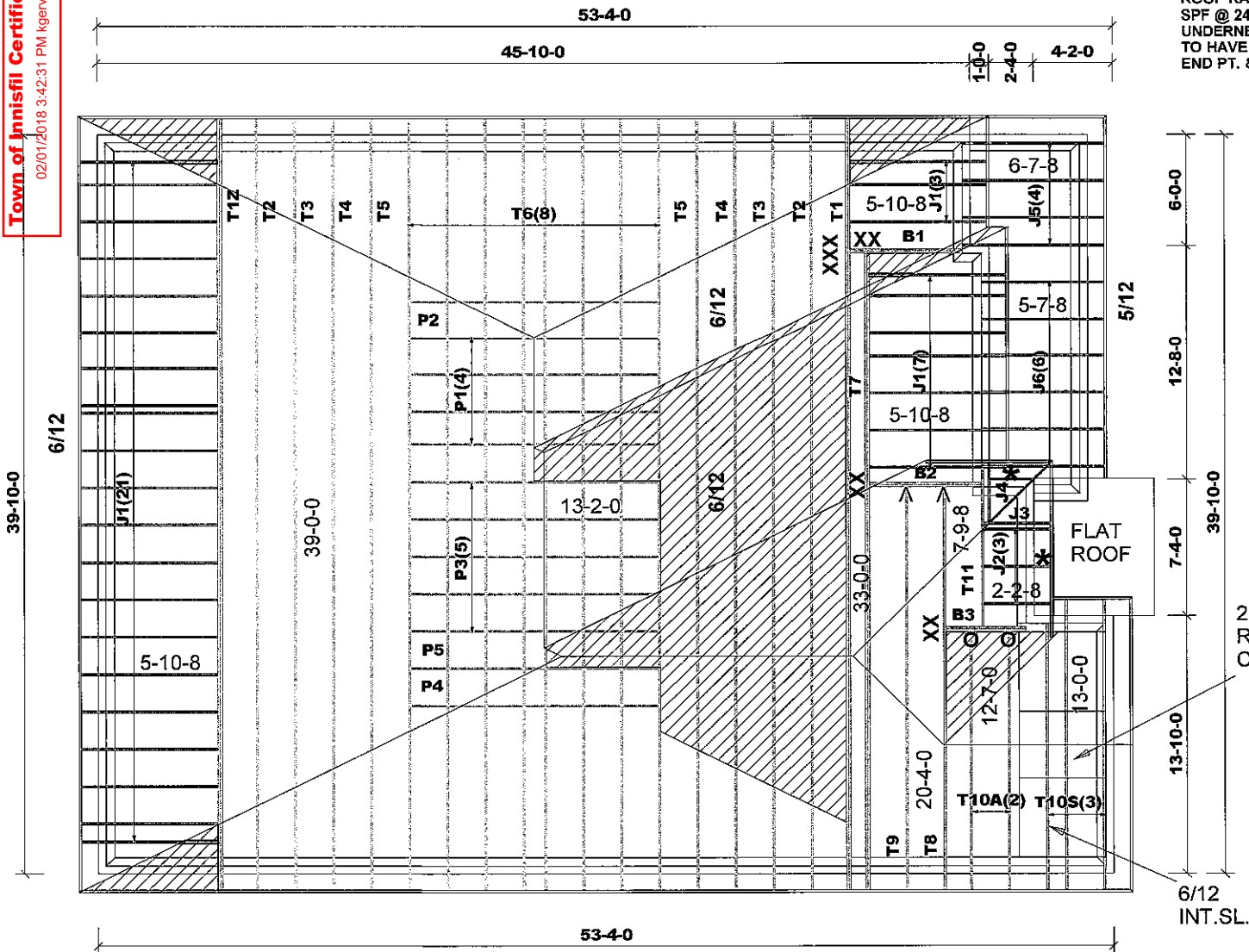




NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALL SLOPES ARE
12/12 UNLESS NOTED

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS

GRD SNOW LD 54.34 PSF

TC DEAD 3 PSF

BC LIVE 10.5 PSF

BC DEAD 7 PSF

ASPHALT SHINGLES

12" FINISHED OH.

R.T.M.C.

2X6 EXTERIOR

WALLS

2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)

HGUS26-2 - (XX)

HGUS26-3 - (XXX)

LUS24 - (O)

CONV.FRAMING
BY OTHERS

DENOTES:

2'-2" RAISED PLATE
AND CEILING *

B1 DENOTES
3-2X10 SPF#2
(FLUSH)

B2 DENOTES
2-2X10 SPF#2
(FLUSH)

B3 DENOTES
2-2X8 SPF#2
(FLUSH)

T-170681

mil, 043



Job Track: **42067**

Layout ID: **288315**

Plan Log: **94324**

Builder / Location:

BAYVIEW WELLINGTON / INNISFIL

Project: **ALCONA SHORES**

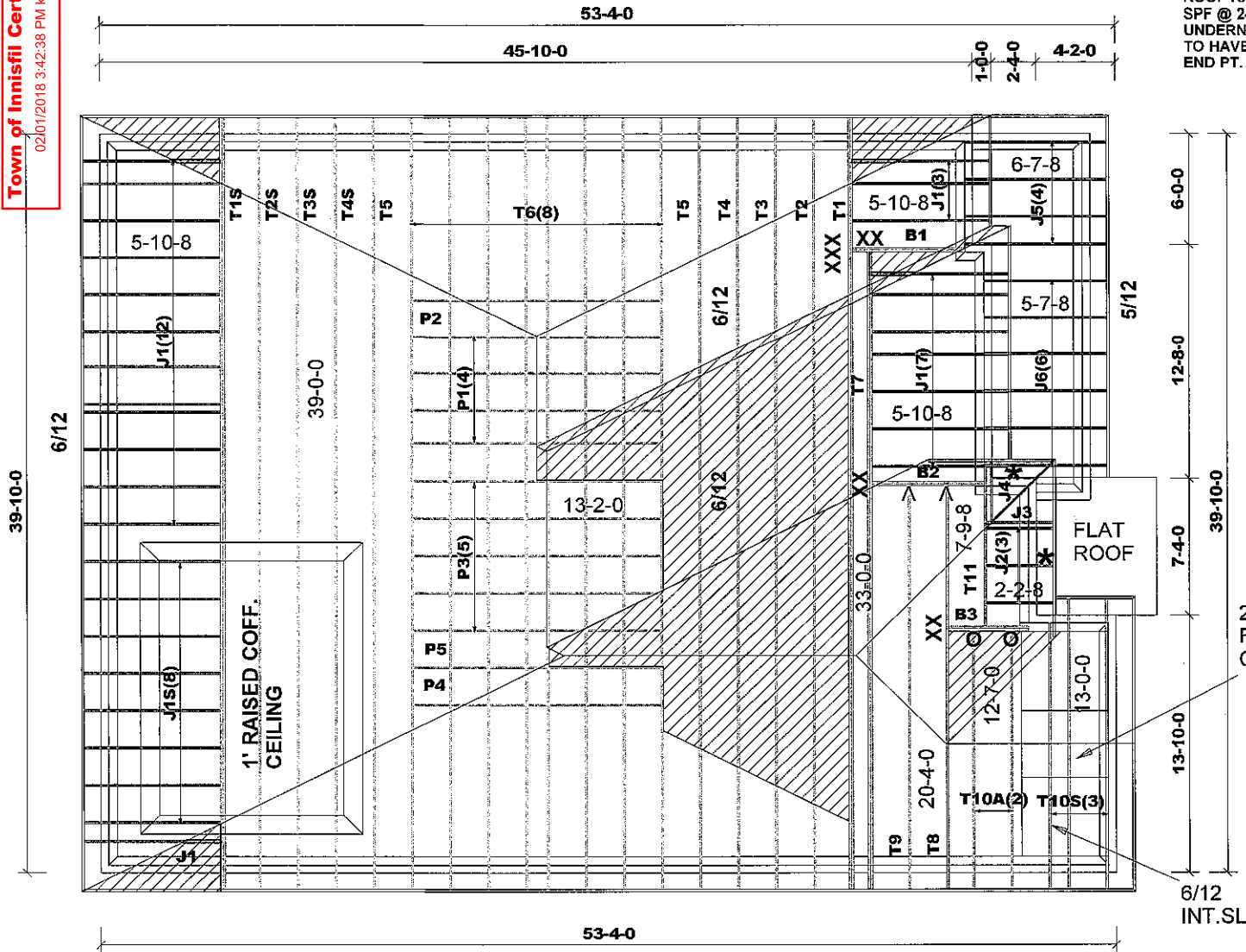
Date: **9/20/2017** Designer: **ella**

Model / Elevation:

S48-3 GOLDENEYE 3 EL:A

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0



NOTES:

TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

**ALL SLOPES ARE
12/12 UNLESS NOTED**

**DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS**
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

**ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD
HARDWARE:**

LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 -(XXX)
LUS24 - (O)

**CONV.FRAMING
BY OTHERS**

**DENOTES:
2'-2" RAISED PLATE
AND CEILING**

B1 DENOTES
3-2X10 SPF#2
(FLUSH)

B2 DENOTES
2-2X10 SPF#2
(FLUSH)

B3 DENOTES
2-2X8 SPF#2
(FLUSH)

T-170681

m11,043



Job Track: **42067**

Layout ID: **287973**

Plan Log: **94324**

Builder / Location:

BAYVIEW WELLINGTON / INNISFIL

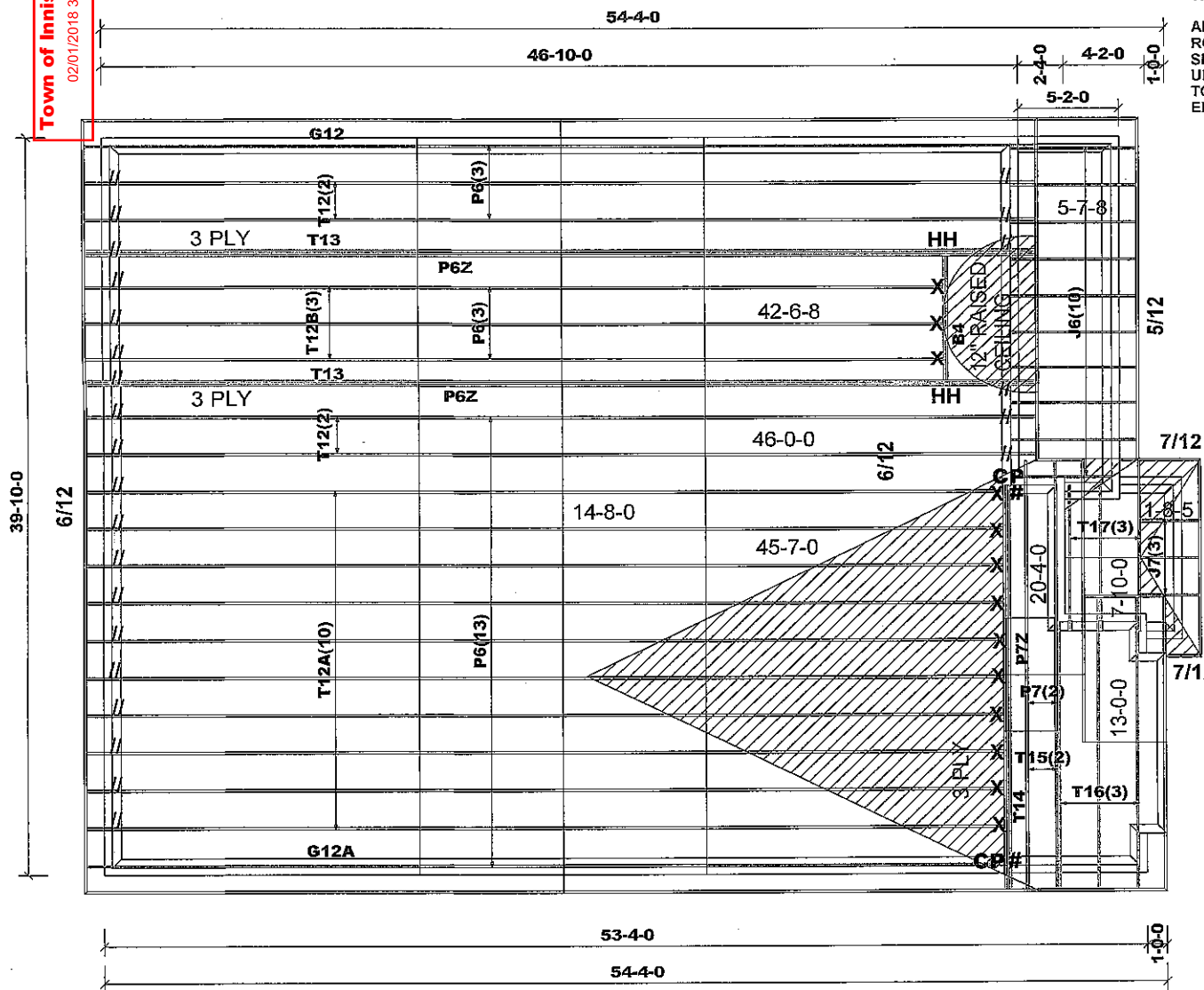
Project: **ALCONA SHORES**

Date: 9/20/2017 Designer: ella

Model / Elevation:

S48-3 GOLDENEYE 3 EL:A +OPT. COFF.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALL SLOPES ARE
12/12 UNLESS NOTED

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26 -(X)
HGUS48 - (HH)
DBL H2.5A -(//)
TRIPLE TS18 -(#)
(CRUSH PL)CP3-6- (CP)

CONV.FRAMING
BY OTHERS



B4 DENOTES
2-1 3/4"x9 1/2" LVL BEAM
(VERSA-LAM 2.0 2800 DF) FLUSH

T-170681

M11,034



Job Track: **42067**
Layout ID: **288316**
Plan Log: **94324**

Builder / Location:
BAYVIEW WELLINGTON / INNISFIL

Project: **ALCONA SHORES**

Date: 9/20/2017 Designer: ella

Model / Elevation:
S48-3 GOLDENEYE 3 EL:B

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED,
OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE.

Mitek ver 7.5.0



Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288315 LOCATION:
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
 MODEL: S48-3 GOLDENEYE 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	12.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08	01-10-08	458.92		
	2 Ply		0.00							283.34		
	1	T1Z HIP GIRDER	12.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08	01-10-08	458.92		
	2 Ply		0.00							283.34		
	2	T2 HIP	12.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08	01-10-08	409.88		
			0.00							252.66		
	2	T3 HIP	12.00	39-00-00	06-01-04	2 X 4	2 X 6	01-03-08	01-10-08	410.82		
			0.00							256.00		
	2	T4 HIP	12.00	39-00-00	07-01-04	2 X 6	2 X 6	01-03-08	01-10-08	446.98		
			0.00							273.00		
	2	T5 HIP	12.00	39-00-00	08-01-04	2 X 6	2 X 6	01-03-08	01-10-08	463.60		
			0.00							284.00		
	8	T6 HIP	12.00	39-00-00	09-01-04	2 X 6	2 X 6	01-03-08	01-10-08	2028.96		
			0.00							1246.64		
	1	T7 HIP GIRDER	12.00	33-00-00	04-01-04	2 X 4	2 X 6	01-03-08	01-10-08	353.02		
	2 Ply		0.00							220.00		
	1	T8 HIP GIRDER	12.00	20-04-00	08-03-00	2 X 4	2 X 6	01-03-08	01-10-08	123.47		
			0.00							78.33		
	1	T9 HIP	12.00	20-04-00	10-03-00	2 X 4	2 X 4	01-03-08	01-10-08	111.33		
			0.00							69.50		
	2	T10A COMMON	12.00	12-07-00	08-04-08	2 X 4	2 X 4	01-03-08	01-10-08	129.84		
			0.00							82.66		
	3	T10S ROOF	12.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08	01-10-08	215.76		
			6.00							138.99		
	1	T11 HALF HIP	12.00	07-09-08	04-01-00	2 X 4	2 X 4	01-03-08	01-10-08	38.74		
			0.00							25.83		
	4	P1 COMMON	6.00	13-02-00	03-03-12	2 X 4	2 X 4	00-00-00	00-00-04	149.96		
			0.00							92.68		
	1	P2 HIP	6.00	13-02-00	01-03-12	2 X 4	2 X 4	00-00-00	00-00-04	37.24		
			0.00							24.17		
	5	P3 COMMON	6.00	13-02-00	03-06-12	2 X 4	2 X 4	00-00-00	00-00-04	195.05		
			0.00							124.15		
	1	P4 HALF HIP	6.00	13-02-00	01-03-12	2 X 4	2 X 4	00-00-00	00-00-04	39.82		
			0.00							25.33		
	1	P5 HALF HIP	6.00	13-02-00	03-03-12	2 X 4	2 X 4	00-00-00	00-00-04	44.73		
			0.00							29.17		



Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288315 LOCATION:
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: S48-3 GOLDENEYE 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	31	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	520.49 330.77		
	3	J2 JACK-OPEN	12.00 0.00	02-02-08	04-01-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-01-00	30.48 21.99		
	1	J3 JACK-OPEN	12.00 0.00	02-02-08	03-07-15	2 X 4	2 X 4	01-03-08 -00-05-01	01-10-08 00-03-08	9.48 6.67		
	1	J4 JACK-OPEN	12.00 0.00	01-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-10-08 00-03-08	9.10 6.00		
	4	J5 JACK-OPEN	5.00 0.00	06-07-08	03-09-11	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 03-09-11	72.44 45.32		
	6	J6 JACK-OPEN	5.00 0.00	05-07-08	03-04-11	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 03-04-11	94.44 60.00		

TOTAL # TRUSS= 88

TOTAL BFT OF ALL TRUSSES=

4260.54 BFT. TOTAL WEIGHT OF ALL TRUSSES= 6853.47 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-3	
3	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= 8



Delivery Shiplist

DATE	09/15/17
SALES REP	Mario

JOB TRACK:42067	LAYOUT ID: 287973	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS	SUB-BUILDER:	
MODEL: S48-3 GOLDENEYE 3	ELEVATION: A + OPT. COFF.	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	12.00	39-00-00	04-01-04	2 X 4	2 X 6	01-03-08	01-10-08	407.74		
	2 Ply		0.00							253.66		
	1	T1S HIP	12.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08	01-10-08	460.64		
	2 Ply		12.00							290.00		
	1	T2 HIP	12.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08	01-10-08	204.94		
			0.00							126.33		
	1	T2S HIP	12.00	39-00-00	05-01-04	2 X 6	2 X 6	01-03-08	01-10-08	227.50		
			12.00							139.17		
	1	T3 HIP	12.00	39-00-00	06-01-04	2 X 4	2 X 6	01-03-08	01-10-08	205.41		
			0.00							128.00		
	1	T3S HIP	12.00	39-00-00	06-01-04	2 X 4	2 X 6	01-03-08	01-10-08	230.48		
			12.00							144.33		
	1	T4 HIP	12.00	39-00-00	07-01-04	2 X 6	2 X 6	01-03-08	01-10-08	223.49		
			0.00							136.50		
	1	T4S HIP	12.00	39-00-00	07-01-04	2 X 4	2 X 6	01-03-08	01-10-08	238.04		
			12.00							147.33		
	2	T5 HIP	12.00	39-00-00	08-01-04	2 X 6	2 X 6	01-03-08	01-10-08	463.60		
			0.00							284.00		
	8	T6 HIP	12.00	39-00-00	09-01-04	2 X 6	2 X 6	01-03-08	01-10-08	2028.96		
			0.00							1246.64		
	1	T7 HIP GIRDER	12.00	33-00-00	04-01-04	2 X 4	2 X 6	01-03-08	01-10-08	353.02		
	2 Ply		0.00							220.00		
	1	T8 HIP GIRDER	12.00	20-04-00	08-03-00	2 X 4	2 X 6	01-03-08	01-10-08	123.47		
			0.00							78.33		
	1	T9 HIP	12.00	20-04-00	10-03-00	2 X 4	2 X 4	01-03-08	01-10-08	111.33		
			0.00							69.50		
	2	T10A COMMON	12.00	12-07-00	08-04-08	2 X 4	2 X 4	01-03-08	01-10-08	129.84		
			0.00							82.66		
	3	T10S ROOF	12.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08	01-10-08	215.76		
			6.00							138.99		
	1	T11 HALF HIP	12.00	07-09-08	04-01-00	2 X 4	2 X 4	01-03-08	01-10-08	38.74		
			0.00							25.83		
	4	P1 COMMON	6.00	13-02-00	03-03-12	2 X 4	2 X 4	00-00-00	00-00-04	149.96		
			0.00							92.68		
	1	P2 HIP	6.00	13-02-00	01-03-12	2 X 4	2 X 4	00-00-00	00-00-04	37.24		
			0.00							24.17		



Delivery Shiplist

DATE	09/15/17
SALES REP	Mario

JOB TRACK:42067	LAYOUT ID: 287973	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES	SUB-BUILDER:	
MODEL: S48-3 GOLDENEYE 3	ELEVATION: A	+ OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	5	P3 COMMON	6.00 0.00	13-02-00	03-06-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-04 00-06-04	195.05 124.15		
	1	P4 HALF HIP	6.00 0.00	13-02-00	01-03-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-04 01-03-12	39.82 25.33		
	1	P5 HALF HIP	6.00 0.00	13-02-00	03-03-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-04 03-03-12	44.73 29.17		
	23	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	386.17 245.41		
	8	J1S JACK-OPEN	6.00 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	181.36 132.00		
	3	J2 JACK-OPEN	12.00 0.00	02-02-08	04-01-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-01-00	30.48 21.99		
	1	J3 JACK-OPEN	12.00 0.00	02-02-08	03-07-15	2 X 4	2 X 4	01-03-08 -00-05-01	01-10-08 00-03-08	9.48 6.67		
	1	J4 JACK-OPEN	12.00 0.00	01-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-10-08 00-03-08	9.10 6.00		
	4	J5 JACK-OPEN	5.00 0.00	06-07-08	03-09-11	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 03-09-11	72.44 45.32		
	6	J6 JACK-OPEN	5.00 0.00	05-07-08	03-04-11	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 03-04-11	94.44 60.00		

TOTAL # TRUSS= 88

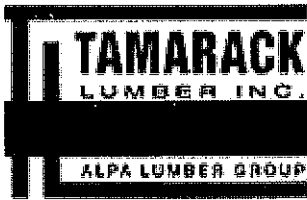
TOTAL BFT OF ALL TRUSSES=

4324.16 BFT. TOTAL WEIGHT OF ALL TRUSSES= 6913.23 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	LJS26DS	
2	Hangers	LUS24	
3	Hangers	HGUS26-2	
1	Hangers	HGUS26-3	

TOTAL # ITEMS= 8



Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288316 LOCATION:
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHOCK SUB-BUILDER:
 MODEL: S48-3 GOLDENEYE 3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	T12 PIGGYBACK	6.00 0.00	46-00-00	09-00-00	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	1087.52 652.68		
	1	G12 GABLE	6.00 0.00	46-00-00	09-00-00	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	286.72 182.50		
	10	T12A PIGGYBACK	6.00 0.00	45-07-00	09-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	2704.10 1621.70		
	1	G12A GABLE	6.00 0.00	45-07-00	09-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	282.93 180.50		
	3	T12B PIGGYBACK	6.00 0.00	42-06-08	09-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 02-10-12	755.01 456.51		
	2 3 Ply	T13 PIGGYBACK	6.00 0.00	46-00-00	09-00-00	2 X 6	2 X 8	01-03-08 01-03-08	01-02-00 01-02-00	1797.36 1102.02		
	1 3 Ply	T14 PIGGYBACK	12.00 0.00	20-04-00	09-00-00	2 X 6	2 X	01-03-08 01-03-08	01-10-08 01-10-08	536.79 347.01		
	2	T15 PIGGYBACK	12.00 0.00	20-04-00	09-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	218.46 138.66		
	3	T16 COMMON	12.00 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	203.58 129.99		
	3	T17 COMMON	7.00 0.00	07-10-00	03-06-13	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	99.72 64.50		
	19	P6 PIGGYBACK	6.00 0.00	12-09-06	03-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	677.35 421.23		
	2 3 Ply	P6Z PIGGYBACK	6.00 0.00	12-09-06	03-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	213.90 133.02		
	2	P7 PIGGYBACK	12.00 0.00	04-11-10	02-11-00	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	30.96 20.00		
	1 3 Ply	P7Z PIGGYBACK	12.00 0.00	04-11-10	02-11-00	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	46.44 30.00		
	10	J6 JCA2	5.00 0.00	05-07-08	03-04-11	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 03-04-11	157.40 100.00		
	3	J7 JACK-OPEN	12.00 0.00	01-08-05	03-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-06-07	26.22 18.00		

TOTAL # TRUSS= 79.00

TOTAL BFT OF ALL TRUSSES=

5598.32 BFT. TOTAL WEIGHT OF ALL TRUSSES= 9124.46 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	09/20/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288316 LOCATION:
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS SUB-BUILDER:
 MODEL: S48-3 GOLDENEYE 3 ELEVATION: B

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
13	Hangers	HGUS26	
2	Hangers	HGUS48	
50	Hangers	H2.5A	
6	Hangers	TS18	
2	Crush Plates	CP3-6	
2	Beam	1 3/4" X 9 1/2" LVL	08-00-00

TOTAL # ITEMS= 75.00



Delivery Shiplist

DATE	09/15/17
SALES REP	Mario

JOB TRACK: 42067	LAYOUT ID: 287974	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS	SUB-BUILDER:	
MODEL: S48-3 GOLDENEYE 3	ELEVATION: B	+ OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	T12 PIGGYBACK	6.00 0.00	46-00-00	09-00-00	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	1087.52 652.68		
	1	G12 GABLE	6.00 0.00	46-00-00	09-00-00	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	286.72 182.50		
	2	T12A PIGGYBACK	6.00 0.00	45-07-00	09-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	540.82 324.34		
	1	G12A GABLE	6.00 0.00	45-07-00	09-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	282.93 180.50		
	3	T12B PIGGYBACK	6.00 0.00	42-06-08	09-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 02-10-12	755.01 456.51		
	8	T12SA PIGGYBACK	6.00 10.00	45-07-00	09-00-00	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	2363.36 1457.36		
	2	T13 PIGGYBACK	6.00 0.00	46-00-00	09-00-00	2 X 6	2 X 8	01-03-08 01-03-08	01-02-00 01-02-00	1797.36 1102.02		
	3 Ply											
	1	T14 PIGGYBACK	12.00 0.00	20-04-00	09-00-00	2 X 6	2 X	01-03-08 01-03-08	01-10-08 01-10-08	536.79 347.01		
	2	T15 PIGGYBACK	12.00 0.00	20-04-00	09-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	218.46 138.66		
	3	T16 COMMON	12.00 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	203.58 129.99		
	3	T17 COMMON	7.00 0.00	07-10-00	03-06-13	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	99.72 64.50		
	19	P6 PIGGYBACK	6.00 0.00	12-09-06	03-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	677.35 421.23		
	2	P6Z PIGGYBACK	6.00 0.00	12-09-06	03-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	213.90 133.02		
	3 Ply											
	2	P7 PIGGYBACK	12.00 0.00	04-11-10	02-11-00	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	30.96 20.00		
	1	P7Z PIGGYBACK	12.00 0.00	04-11-10	02-11-00	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	46.44 30.00		
	3 Ply											
	10	J6 JCA2	5.00 0.00	05-07-08	03-04-11	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 03-04-11	157.40 100.00		
	3	J7 JACK-OPEN	12.00 0.00	01-08-05	03-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-06-07	26.22 18.00		

TOTAL # TRUSS= 79

TOTAL BFT OF ALL TRUSSES=

5758.32 BFT. TOTAL WEIGHT OF ALL TRUSSES= 9324.54 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

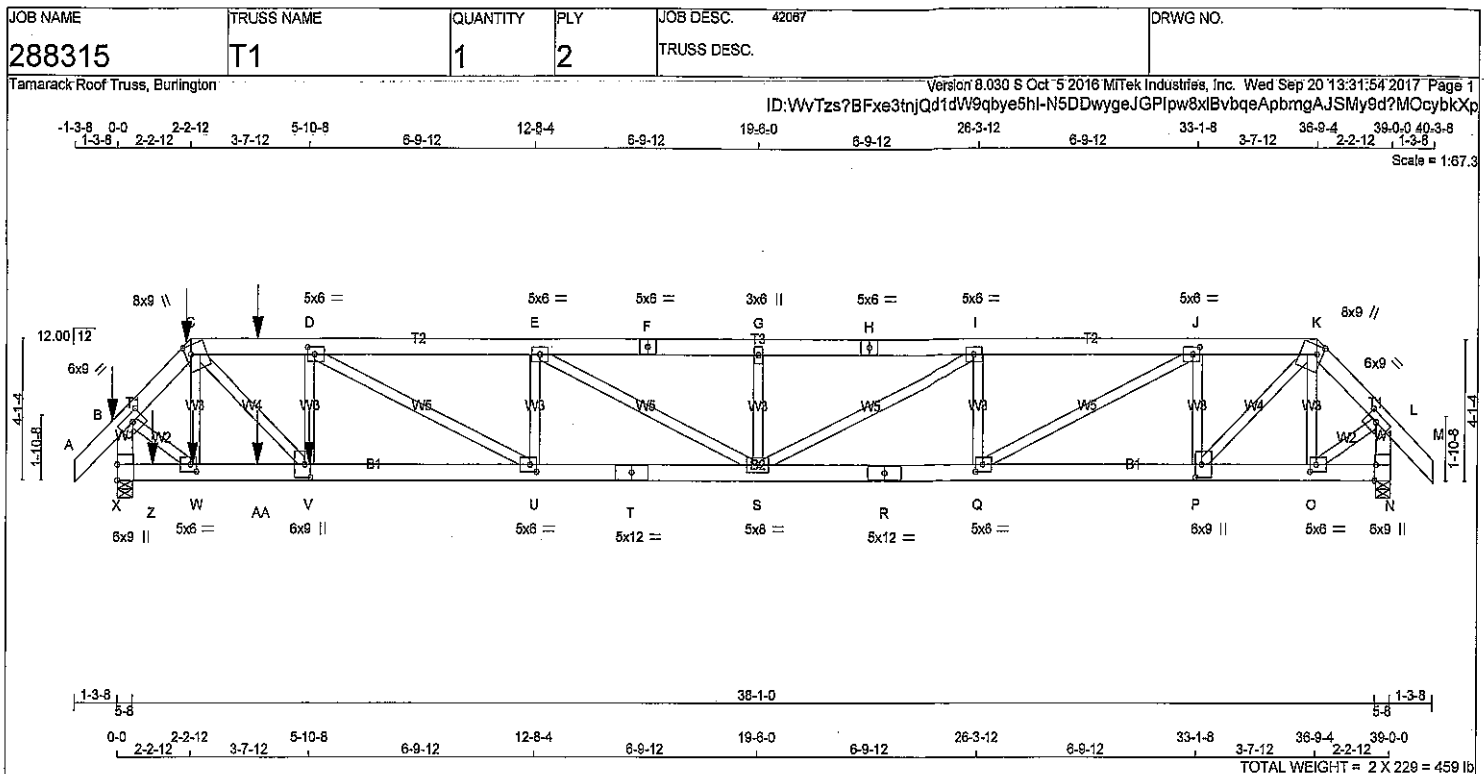
DATE	09/15/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 287974 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
 MODEL: S48-3 GOLDENEYE 3 ELEVATION: B + OPT. CORR.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
13	Hangers	HGUS26	
2	Hangers	HGUS48	
50	Hangers	H2.5A	
6	Hangers	TS18	
2	Crush Plates	CP3-6	
2	Beam	1 3/4" X 9 1/2" LVL	08-00-00

TOTAL # ITEMS= 75



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - F	2x6	DRY 1650F 1.5E	SPF
F - H	2x6	DRY 1650F 1.5E	SPF
H - K	2x6	DRY 1650F 1.5E	SPF
K - M	2x6	DRY No.2	SPF
X - B	2x6	DRY No.2	SPF
N - L	2x6	DRY No.2	SPF
X - T	2x6	DRY 1650F 1.5E	SPF
T - R	2x6	DRY 1650F 1.5E	SPF
R - N	2x6	DRY 1650F 1.5E	SPF
ALL WEBS	2x4	DRY No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-K	2	12
K-M	2	12
X-B	2	12
N-L	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-T	2	12
T-R	2	12
R-N	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	
D-V	2	3
J-P	2	3

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTVW+tm	MT20	8.0	9.0	3.00	2.00
D	TMVW-t	MT20	5.0	6.0	2.50	2.50
E	TMVW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW+tm	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMVW-t	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.50	2.50
K	TTVW+tm	MT20	8.0	9.0	3.00	2.00
L	TMVW-L	MT20	6.0	9.0	2.75	4.00
N	BMV1+t	MT20	6.0	9.0	Edge	0.50
O	BMVW-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
X	7813	0	7813	0	5-8	5-8
N	3880	0	3880	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	6058	4019 / 0	1019 / 0	0 / 0	0 / 0	1020 / 0	0 / 0
N	3009	2003 / 0	509 / 0	0 / 0	0 / 0	497 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. PERM. LIVE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. PERM. LIVE (LBS)	
FR-TO		FROM	TO	CSI (MAX)		FR-TO		CSI (MAX)	
A-B	0 / 63	-122.2	-122.2	0.05 (1)	10.00	W-C	-2269 / 0	0.19 (1)	
B-C	-6811 / 0	-122.2	-122.2	0.09 (1)	4.54	C-V	0 / 8921	0.79 (1)	
C-Y	-11023 / 0	-122.2	-122.2	0.15 (1)	4.17	V-D	-1417 / 0	0.12 (1)	
Y-D	-11023 / 0	-122.2	-122.2	0.15 (1)	4.17	D-U	0 / 1746	0.15 (1)	
D-E	-12592 / 0	-122.2	-122.2	0.26 (1)	3.98	E-U	-593 / 25	0.05 (1)	
E-F	-12040 / 0	-122.2	-122.2	0.23 (1)	3.97	S-I	-591 / 0	0.25 (1)	
F-G	-12040 / 0	-122.2	-122.2	0.23 (1)	3.97	S-G	-816 / 0	0.07 (1)	
G-H	-12040 / 0	-122.2	-122.2	0.23 (1)	3.97	S-I	0 / 2896	0.24 (1)	
H-I	-12040 / 0	-122.2	-122.2	0.23 (1)	3.97	Q-I	-2115 / 0	0.18 (1)	
I-J	-9684 / 0	-122.2	-122.2	0.18 (1)	4.39	Q-J	0 / 4873	0.43 (1)	
J-K	-5366 / 0	-122.2	-122.2	0.11 (1)	5.57	P-J	-2961 / 0	0.25 (1)	
K-L	-3230 / 0	-122.2	-122.2	0.04 (1)	6.13	P-K	0 / 4430	0.39 (1)	
L-M	0 / 63	-122.2	-122.2	0.05 (1)	10.00	O-K	-1139 / 0	0.10 (1)	
X-B	-7857 / 0	0.0	0.0	0.29 (1)	5.36	B-W	0 / 5453	0.48 (1)	
N-L	-3880 / 0	0.0	0.0	0.14 (1)	7.19	O-L	0 / 2586	0.23 (1)	

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
B	0-0	-25	-28	FRONT	VERT	DEAD
C	2-2-12	-13	-14	FRONT	VERT	DEAD
C	2-2-12	-151	-151	BACK	VERT	TOTAL
C	2-2-12	-183	-183	FRONT	VERT	TOTAL
V	5-10-8	-4841	-4841	BACK	VERT	TOTAL
W	2-3-8	-40	-70	BACK	VERT	TOTAL
Y	4-3-8	-147	-147	BACK	VERT	TOTAL
Z	1-0-12	-40	-70	BACK	VERT	TOTAL
AA	4-3-8	-40	-70	BACK	VERT	TOTAL

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
B	0-0	-25	-28	FRONT	VERT	DEAD
C	2-2-12	-13	-14	FRONT	VERT	DEAD
C	2-2-12	-151	-151	BACK	VERT	TOTAL
C	2-2-12	-183	-183	FRONT	VERT	TOTAL
V	5-10-8	-4841	-4841	BACK	VERT	TOTAL
W	2-3-8	-40	-70	BACK	VERT	TOTAL
Y	4-3-8	-147	-147	BACK	VERT	TOTAL
Z	1-0-12	-40	-70	BACK	VERT	TOTAL
AA	4-3-8	-40	-70	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIAL LOADS:

TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL)= L/999 (0.39")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL)= L/787 (0.60")

CSI: TC=0.29 (B-X:1), BC=0.56 (S-U:1), WB=0.79 (C-V:1), SS=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	FLI
MAX	MIN	MAX	MIN
MT20	618	354	1567
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 1.00 (T) (INPUT = 1.00)



DWG NO. TAM 47120-17

STRUCTURAL
COMPONENT ONLY

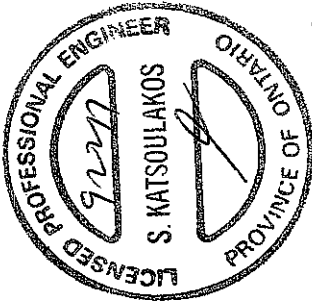
CONTINUED ON PAGE 2

PLATES (table is in inches)

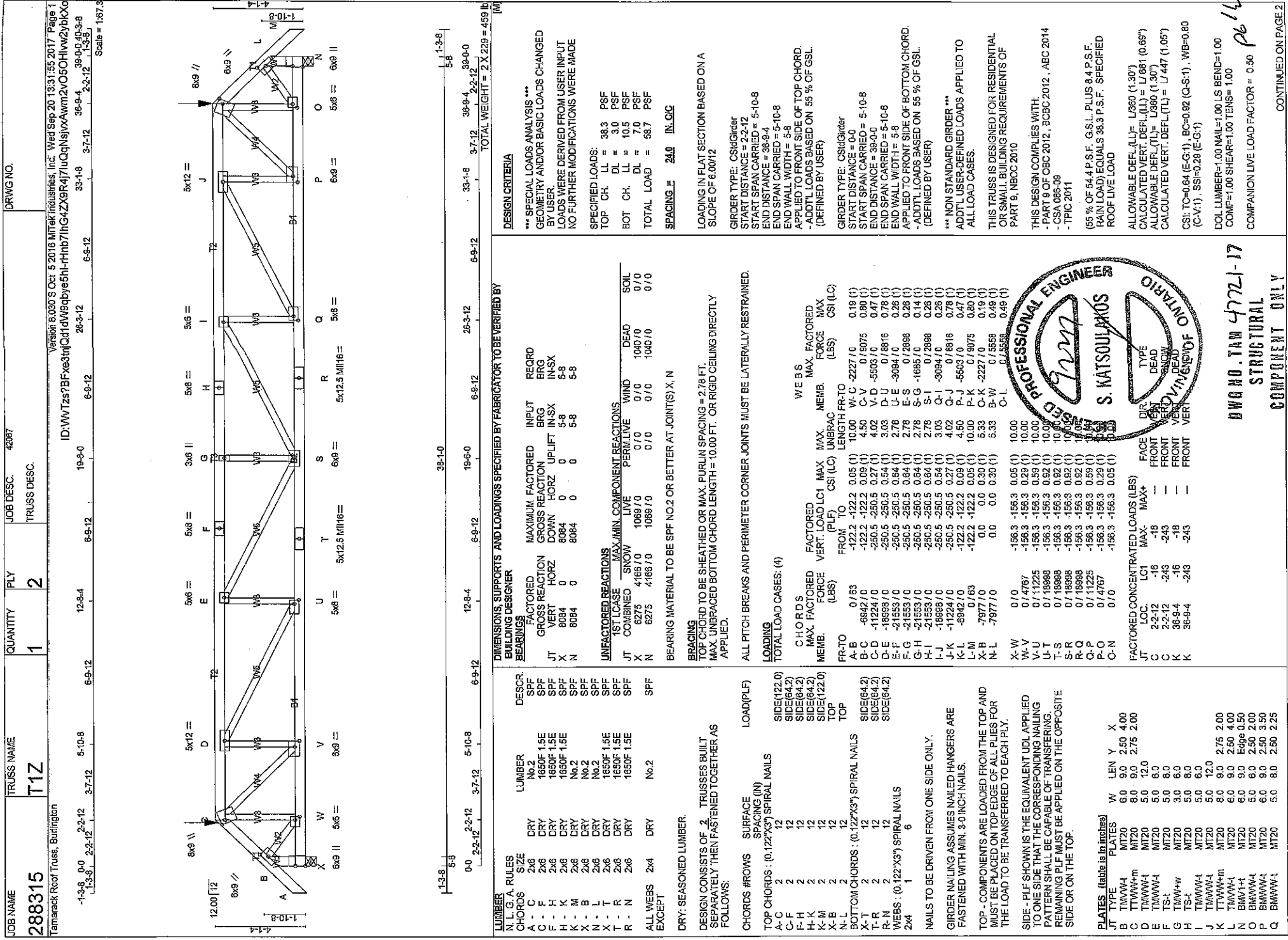
PLATE TYPE	W	LEN	Y	X
P BMWWW-1	MT20	6.0	9.0	4.50 2.25
Q BMWWW-1	MT20	6.0	6.0	2.50 2.50
R ES-1	MT20	5.0	12.0	
S BMWWW-1	MT20	5.0	6.0	
T SS-1	MT20	5.0	12.0	
U BMWWW-1	MT20	5.0	6.0	2.50 2.50
V BMWWW-1	MT20	6.0	9.0	4.50 2.25
W BMWWW-1	MT20	5.0	6.0	2.50 2.25
X BMV1-1	MT20	6.0	9.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGERS(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 28.0 lbs FACTORED DOWN AT 0-0, 166.6 lbs FACTORED DOWN AT 2-2-12, AND 150.8 lbs FACTORED DOWN AT 2-2-12, AND 147.1 lbs FACTORED DOWN AT 4-3-8 ON TOP CHORD, AND 66.9 lbs FACTORED DOWN AT 1-0-12, 66.9 lbs FACTORED DOWN AT 2-3-8, AND 66.9 lbs FACTORED DOWN AT 4-3-8, AND 4841.5 lbs FACTORED DOWN AT 5-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



pub
DWG NO. TAM-4772-D-17
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288315	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Wed Sep 20 13:31:55 2017 Page 2

ID:WV7zs7BFxe3inQd1dW9qbye5Hl-Hnb7lInG4ZX9R4i7luQqN5vxAwm2vO5OIhw2YbXG

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
R BS-1	MI16	5.0	12.5		
S BMMWW-1	MT20	6.0	9.0		
T BS-1	MI16	5.0	12.5		
U BMMWW-1	MT20	5.0	8.0	2.50	2.25
V BMMWW-1	MT20	6.0	9.0	2.50	3.50
W BMMWW-1	MT20	5.0	8.0	2.50	2.00
X BMMWW-1	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGERS OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 259.3 lbs FACTORED DOWN AT 2-2-12, AND 259.3 lbs FACTORED DOWN AT 36-9-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

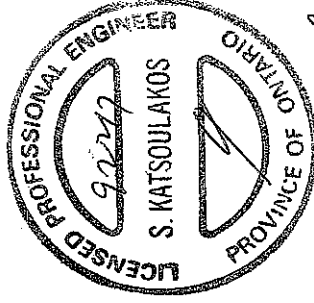
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656
MI16 473 276 2341 1245 4454 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

.JSI GRIP= 0.90 (V) (INPUT = 0.90)

.JSI METAL= 0.84 (T) (INPUT = 1.00)



DWG NO. TAM 47721-17
STRUCTURAL
COMPONENT ONLY

[illegible]

PLATES (table is in inches)

JT TYPE	W	LEN	Y	X
F TS-1	MT20	5.0	6.0	
G TMMW-1	MT20	5.0	8.0	2.50 3.25
H TMMW-1	MT20	4.0	6.0	
I TS-1	MT20	5.0	8.0	
J TMMW-1	MT20	2.0	4.0	2.50 1.00
K TMMW-1	MT20	5.0	6.0	2.50 2.50
L TMMW-1	MT20	5.0	6.0	2.50 2.75
M TMMW-1	MT20	5.0	6.0	
N TMMW-1	MT20	6.0	9.0	
O TMMW-1	MT20	5.0	6.0	1.75 3.00
P BMMW-1	MT20	3.0	6.0	
Q BMMW-1	MT20	3.0	6.0	3.50 1.25
R BMMW-1	MT20	5.0	8.0	
S BMMW-1	MT20	8.0	9.0	2.50 2.75
T BMMW-1	MT20	5.0	8.0	2.50 2.00
U BMMW-1	MT20	5.0	8.0	2.50 2.00
V BMMW-1	MT20	8.0	16.0	2.25 7.75
W				
X BMMW-1	MT20	6.0	9.0	Edge
Y BS-1	MT20	8.0	9.0	3.00 2.50
Z BMMW-1	MT20	5.0	8.0	2.50 2.50
AA BMMW-1	MT20	6.0	9.0	3.00 4.25
AB BMMW-1	MT20	5.0	6.0	
AC BMMW-1	MT20	3.0	6.0	
AD NPH	MT20	3.0	4.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

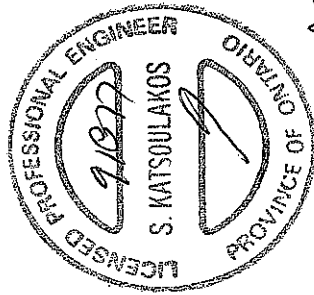
1) SPECIAL HANGERS(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 343.1 lbs FACTORED DOWN AT 2-2-12,
285.2 lbs FACTORED DOWN AT 38-9-4, 147.1 lbs
FACTORED DOWN AT 4-3-8, 147.1 lbs
FACTORED DOWN AT 6-3-8, 147.1 lbs
FACTORED DOWN AT 8-3-8, 147.1 lbs
FACTORED DOWN AT 10-3-8, 147.1 lbs
FACTORED DOWN AT 12-3-8, 147.1 lbs
FACTORED DOWN AT 14-3-8, 147.1 lbs
FACTORED DOWN AT 16-3-8, 147.1 lbs
FACTORED DOWN AT 18-3-8, 147.1 lbs
FACTORED DOWN AT 20-3-8, 89.2 lbs
FACTORED DOWN AT 22-3-8, 89.2 lbs
FACTORED DOWN AT 24-3-8, 89.2 lbs
FACTORED DOWN AT 26-3-8, 89.2 lbs
FACTORED DOWN AT 28-3-8, 89.2 lbs
FACTORED DOWN AT 30-3-8, AND 89.2 lbs
FACTORED DOWN AT 32-3-8, AND 89.2 lbs
FACTORED DOWN AT 34-3-8 ON TOP CHORD,
AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9
lbs
FACTORED DOWN AT 2-3-8, 69.9 lbs
FACTORED DOWN AT 4-3-8, 69.9 lbs
FACTORED DOWN AT 6-3-8, 69.9 lbs
FACTORED DOWN AT 8-3-8, 69.9 lbs
FACTORED DOWN AT 10-3-8, 69.9 lbs
FACTORED DOWN AT 12-3-8, 69.9 lbs
FACTORED DOWN AT 14-3-8, 69.9 lbs
FACTORED DOWN AT 16-3-8, 69.9 lbs
FACTORED DOWN AT 18-3-8, 74.5 lbs
FACTORED DOWN AT 20-3-8, 182.7 lbs
FACTORED DOWN AT 22-3-8, 182.7 lbs
FACTORED DOWN AT 24-3-8, 182.7 lbs
FACTORED DOWN AT 26-3-8, 182.7 lbs
FACTORED DOWN AT 28-3-8, 182.7 lbs
FACTORED DOWN AT 30-3-8, 182.7 lbs
FACTORED DOWN AT 32-3-8, 182.7 lbs
FACTORED DOWN AT 34-3-8, AND 166.7 lbs
FACTORED DOWN AT 36-3-8, AND 65.2 lbs
FACTORED DOWN AT 37-3-8 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

LOADING TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD L1 (PLF)	MAX. CS (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS	
					MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO	0/8274	-28.0	-28.0	0.38 (1)	10.00	
AA-AU	0/8274	-28.0	-28.0	0.38 (1)	10.00	
ALL-AV	0/8274	-28.0	-28.0	0.38 (1)	10.00	
AV-Z	0/8274	-28.0	-28.0	0.53 (1)	10.00	
ZAW	0/11771	-28.0	-28.0	0.53 (1)	10.00	
AW-Y	0/11771	-28.0	-28.0	0.53 (1)	10.00	
Y-AX	0/11771	-28.0	-28.0	0.53 (1)	10.00	
AX-AY	0/11771	-28.0	-28.0	0.53 (1)	10.00	
AY-X	0/11771	-28.0	-28.0	0.53 (1)	10.00	
X-AZ	0/506	-28.0	-28.0	0.06 (2)	10.00	
AZ-BA	0/506	-28.0	-28.0	0.06 (2)	10.00	
BA-W	0/506	-28.0	-28.0	0.06 (2)	10.00	
V-BB	0/19237	-28.0	-28.0	0.90 (1)	10.00	
BB-BC	0/19237	-28.0	-28.0	0.90 (1)	10.00	
BC-U	0/19237	-28.0	-28.0	0.90 (1)	10.00	
U-BD	0/12609	-28.0	-28.0	0.81 (1)	10.00	
BD-BE	0/12609	-28.0	-28.0	0.81 (1)	10.00	
BE-T	0/12609	-28.0	-28.0	0.81 (1)	10.00	
T-BE	0/5390	-28.0	-28.0	0.31 (1)	10.00	
BE-BG	0/5390	-28.0	-28.0	0.31 (1)	10.00	
BG-S	0/5390	-28.0	-28.0	0.31 (1)	10.00	
S-BH	0/3684	-28.0	-28.0	0.16 (1)	10.00	
BH-R	0/0	-28.0	-28.0	0.01 (2)	10.00	
R-Q	0/0	-28.0	-28.0	0.01 (2)	10.00	
Q	0/0	-28.0	-28.0	0.01 (2)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	L1	MAX.	FACE	DIR.	TYPE
C	2-2-12	-13	-14	FRONT	VERT	DEAD
C	2-2-12	-151	-151	FRONT	VERT	TOTAL
C	2-2-12	-180	-180	FRONT	VERT	SNOW
F	14-3-8	-147	-147	FRONT	VERT	TOTAL
G	16-8-8	-147	-147	FRONT	VERT	TOTAL
I	24-8-8	-89	-89	FRONT	VERT	TOTAL
J	26-8-8	-89	-89	FRONT	VERT	TOTAL
M	36-9-4	-13	-14	FRONT	VERT	DEAD
M	36-9-4	-83	-93	FRONT	VERT	TOTAL
M	36-9-4	-180	-180	FRONT	VERT	SNOW
U	26-8-8	-183	-183	FRONT	VERT	TOTAL
X	16-8-8	-40	-70	FRONT	VERT	TOTAL
AB	2-3-8	-40	-70	FRONT	VERT	TOTAL
AE	4-3-8	-147	-147	FRONT	VERT	TOTAL
AF	6-3-8	-147	-147	FRONT	VERT	TOTAL
AG	8-3-8	-147	-147	FRONT	VERT	TOTAL
AH	10-3-8	-147	-147	FRONT	VERT	TOTAL
AI	12-3-8	-147	-147	FRONT	VERT	TOTAL
AJ	14-8-8	-147	-147	FRONT	VERT	TOTAL
AK	16-8-8	-147	-147	FRONT	VERT	TOTAL
AL	20-8-8	-89	-89	FRONT	VERT	TOTAL
AM	22-8-8	-89	-89	FRONT	VERT	TOTAL
AN	26-8-8	-89	-89	FRONT	VERT	TOTAL
AO	30-8-8	-89	-89	FRONT	VERT	TOTAL
AP	32-8-8	-89	-89	FRONT	VERT	TOTAL
AQ	34-8-8	-89	-89	FRONT	VERT	TOTAL
AR	1-0-12	-40	-70	FRONT	VERT	TOTAL
AS	4-3-8	-40	-70	FRONT	VERT	TOTAL
AT	6-3-8	-40	-70	FRONT	VERT	TOTAL
AU	8-3-8	-40	-70	FRONT	VERT	TOTAL
AV	10-3-8	-40	-70	FRONT	VERT	TOTAL
AW	12-3-8	-40	-70	FRONT	VERT	TOTAL
AX	14-3-8	-40	-70	FRONT	VERT	TOTAL
AY	16-8-8	-40	-70	FRONT	VERT	TOTAL
AZ	18-8-8	-40	-70	FRONT	VERT	TOTAL
BA	20-8-8	-43	-75	FRONT	VERT	TOTAL
BB	22-8-8	-183	-183	FRONT	VERT	TOTAL
BC	24-8-8	-183	-183	FRONT	VERT	TOTAL
BD	26-8-8	-183	-183	FRONT	VERT	TOTAL
BE	30-8-8	-183	-183	FRONT	VERT	TOTAL
BF	32-8-8	-183	-183	FRONT	VERT	TOTAL
BG	34-8-8	-183	-183	FRONT	VERT	TOTAL
BH	36-8-8	-197	-197	FRONT	VERT	TOTAL
BI	37-8-8	-49	-85	FRONT	VERT	TOTAL



DWG NO. TAM 46916-17
STRUCTURAL
COMPONENT ONLY

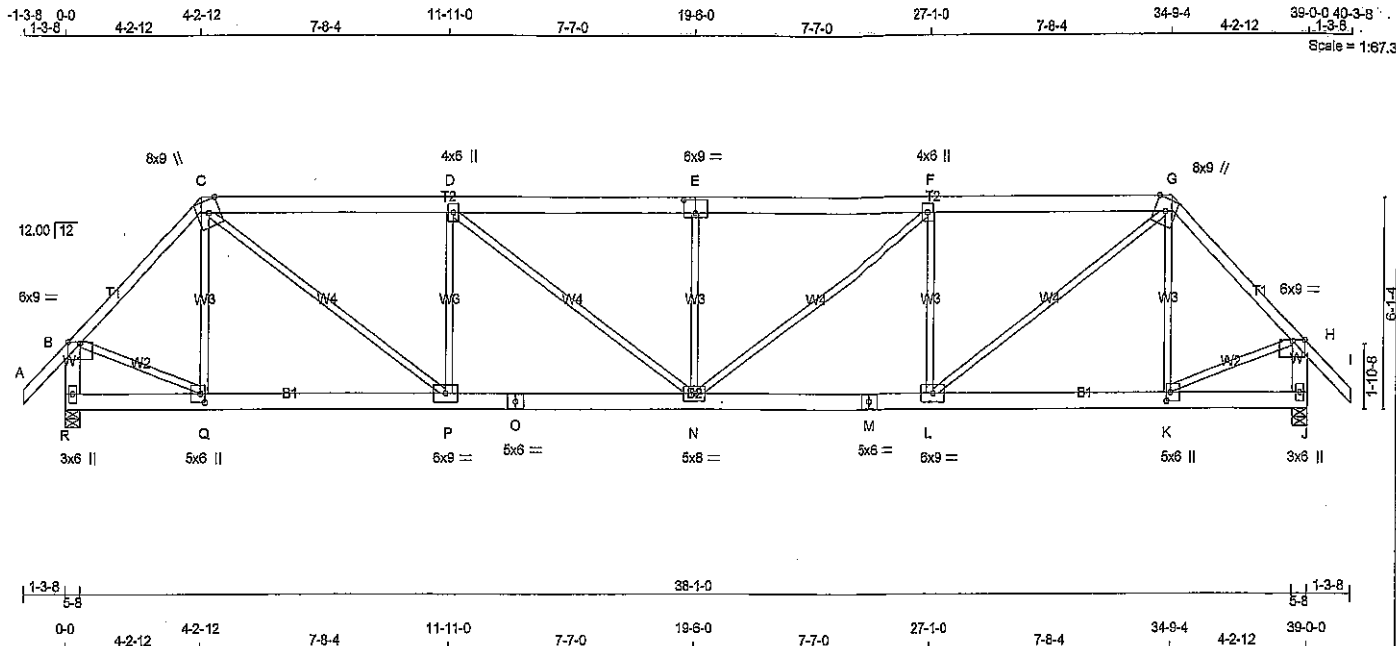
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	287900	DRWG NO.
287900	T3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Tue Sep 12 16:06:19 2017 Page 1
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Scale = 1:87.3

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
E - G	2x6	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	
D	TMVW+H	MT20	4.0	6.0		
E	TSW-I	MT20	6.0	9.0	4.50	4.50
F	TMVW+H	MT20	4.0	6.0		
G	TTWW+m	MT20	8.0	9.0	Edge	
H	TMVW-p	MT20	6.0	9.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMVW+H	MT20	5.0	6.0	3.00	1.50
L	BMVW+H	MT20	6.0	9.0		
M	BS-I	MT20	5.0	6.0		
N	BMVW+H	MT20	5.0	8.0		
O	BS-I	MT20	5.0	6.0		
P	BMVW+H	MT20	6.0	9.0		
Q	BMVW+H	MT20	5.0	6.0	3.00	1.50
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
R	3100	0	3100	0
J	3100	0	3100	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0	0 / 0
J	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO						FR-TO			
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	Q-C	-455 / 76	0.23 (1)	
B-C	-2806 / 0	-122.2	-122.2	0.59 (1)	3.61	C-P	0 / 3047	0.69 (1)	
C-D	-4432 / 0	-122.2	-122.2	0.68 (1)	3.48	P-D	-1599 / 0	0.81 (1)	
D-E	-5148 / 0	-122.2	-122.2	0.74 (1)	3.19	D-N	0 / 893	0.20 (1)	
E-F	-5148 / 0	-122.2	-122.2	0.74 (1)	3.19	N-E	-858 / 0	0.44 (1)	
F-G	-4432 / 0	-122.2	-122.2	0.68 (1)	3.48	N-F	0 / 893	0.20 (1)	
G-H	-2806 / 0	-122.2	-122.2	0.59 (1)	3.61	L-F	-1599 / 0	0.81 (1)	
H-I	0 / 60	-122.2	-122.2	0.17 (1)	10.00	L-G	0 / 3047	0.69 (1)	
R-B	-3070 / 0	0.0	0.0	0.22 (1)	6.03	K-G	-455 / 76	0.23 (1)	
J-H	-3070 / 0	0.0	0.0	0.22 (1)	6.03	B-Q	0 / 2093	0.47 (1)	
R-Q	0 / 0	-28.0	-28.0	0.13 (2)	10.00	K-H	0 / 2093	0.47 (1)	
Q-P	0 / 1978	-28.0	-28.0	0.32 (1)	10.00				
P-O	0 / 4432	-28.0	-28.0	0.61 (1)	10.00				
O-N	0 / 4432	-28.0	-28.0	0.61 (1)	10.00				
N-M	0 / 4432	-28.0	-28.0	0.61 (1)	10.00				
M-L	0 / 4432	-28.0	-28.0	0.61 (1)	10.00				
L-K	0 / 1978	-28.0	-28.0	0.32 (1)	10.00				
K-J	0 / 0	-28.0	-28.0	0.13 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	38.3	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.31")
ALLOWABLE DEFL.(TL) = L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.47")

CSI: TC=0.74 (D-E:1), BC=0.61 (N-P:1), WB=0.81 (F-L:1), SSI=0.34 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

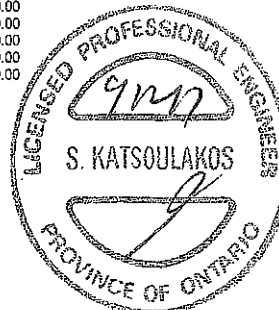
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

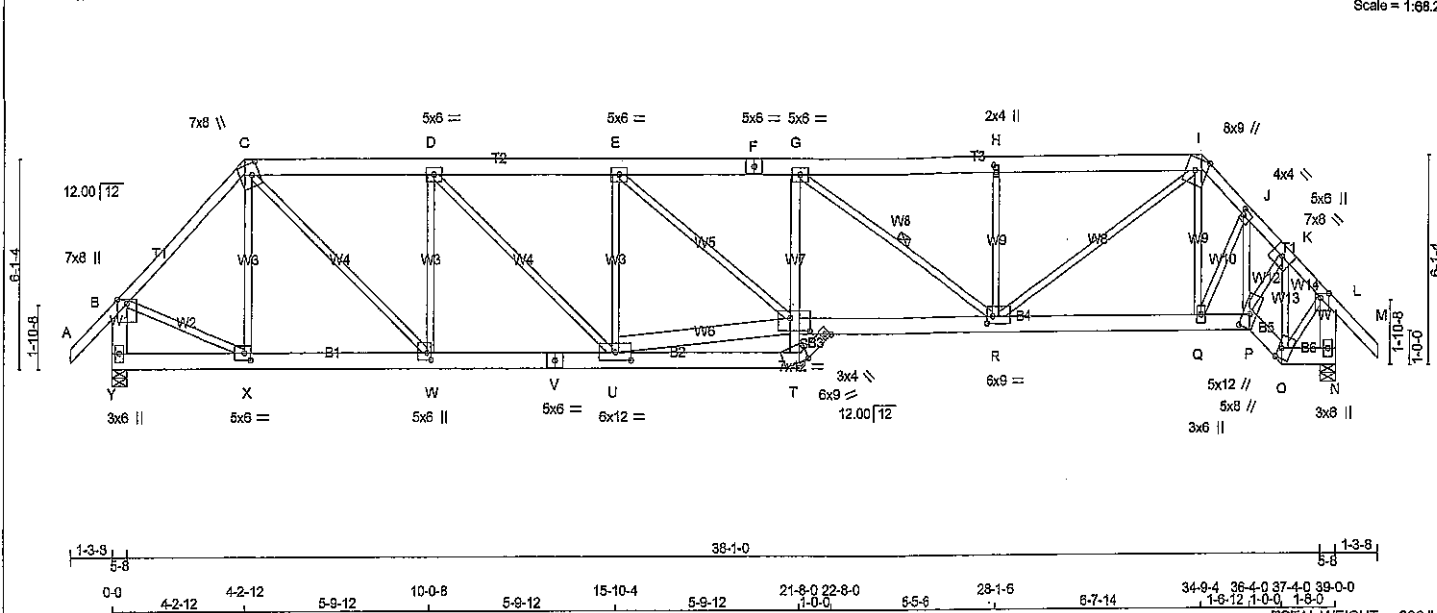
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.72 (O) (INPUT = 1.00)



DWG NO. TAM 46280-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



Scale = 1:68.2

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

T - G 2x4 DRY No.2 SPF

U - S 2x6 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	7.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	4.00	2.75
D, E, G						
D	TMVW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TMVW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	8.0	9.0	Edge	4.50
J	TMVW-t	MT20	4.0	4.0	2.00	1.00
K	TMVW-t	MT20	7.0	8.0		
L	TMVW+p	MT20	5.0	6.0	1.50	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BBVW+m	MT20	5.0	8.0	3.50	1.25
P	BBVW+m	MT20	5.0	12.0	5.00	2.50
Q	BMVW+t	MT20	3.0	6.0		
R	BMVW+t	MT20	6.0	9.0	2.50	2.25
S	BWVW-t	MT20	7.0	12.0	4.50	7.50
T						
U	BBW-m	MT20	6.0	9.0	Edge	
T	BMVW+t	MT20	6.0	12.0	2.75	8.00
V	BS-t	MT20	5.0	6.0		
W	BMVW+t	MT20	5.0	6.0	2.50	1.50
X	BMVW-t	MT20	5.0	6.0	2.50	2.25
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
Y	3097	0	3097	0	5-8	5-8
N	3103	0	3103	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM LIVE			
Y	2404	1596 / 0	410 / 0	0 / 0	0 / 0	395 / 0	0 / 0
N	2408	1600 / 0	409 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 80	-122.2 -122.2	0.17 (1)	X-C	-482 / 28	0.25 (1)	
B-C	-2792 / 0	-122.2 -122.2	0.59 (1)	C-W	0 / 2751	0.62 (1)	
C-D	-3948 / 0	-122.2 -122.2	0.37 (1)	W-D	-1767 / 0	0.90 (1)	
D-E	-4918 / 0	-122.2 -122.2	0.43 (1)	D-U	0 / 1367	0.31 (1)	
E-F	-6122 / 0	-122.2 -122.2	0.43 (1)	U-E	-1643 / 0	0.84 (1)	
F-G	-6122 / 0	-122.2 -122.2	0.43 (1)	T-S	0 / 116	0.16 (1)	
G-H	-5041 / 0	-122.2 -122.2	0.54 (1)	S-G	0 / 234	0.16 (1)	
H-I	-5041 / 0	-122.2 -122.2	0.54 (1)	Q-I	0 / 195	0.04 (3)	
I-J	-3356 / 0	-122.2 -122.2	0.16 (1)	Q-J	0 / 83	0.02 (2)	
J-K	-3279 / 0	-122.2 -122.2	0.19 (1)	P-J	-299 / 0	0.06 (1)	
K-L	-2161 / 0	-122.2 -122.2	0.21 (1)	P-K	0 / 1942	0.44 (1)	
L-M	0 / 80	-122.2 -122.2	0.17 (1)	O-K	-2578 / 0	0.49 (1)	
Y-B	-3057 / 0	0.0 0.0	0.22 (1)	B-X	0 / 2083	0.47 (1)	
N-L	-3079 / 0	0.0 0.0	0.22 (1)	O-L	0 / 1920	0.43 (1)	
Y-X	0 / 0	-28.0 -28.0	0.09 (2)	U-S	0 / 4907	0.58 (1)	
X-W	0 / 1988	-28.0 -28.0	0.29 (1)	E-S	0 / 1625	0.34 (1)	
W-V	0 / 3948	-28.0 -28.0	0.52 (1)	R-H	-229 / 0	0.34 (1)	
V-U	0 / 3948	-28.0 -28.0	0.52 (1)	G-H	-2351 / 0	0.56 (1)	
U-T	0 / 108	-28.0 -28.0	0.11 (3)	R-I	0 / 624	0.73 (1)	
S-R	0 / 8146	-28.0 -28.0	0.62 (1)				
R-Q	0 / 2385	-28.0 -28.0	0.39 (1)				
Q-P	0 / 2346	-28.0 -28.0	0.36 (1)				
P-O	0 / 1956	-28.0 -28.0	0.23 (1)				
O-N	0 / 0	-28.0 -28.0	0.01 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.37")
ALLOWABLE DEFL.(TL) = L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/836 (0.56")

CS1: TC=0.59 (B-C:1), BC=0.62 (R-S:1), WB=0.90 (D-W:1), SS1=0.29 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

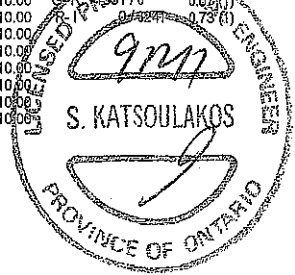
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

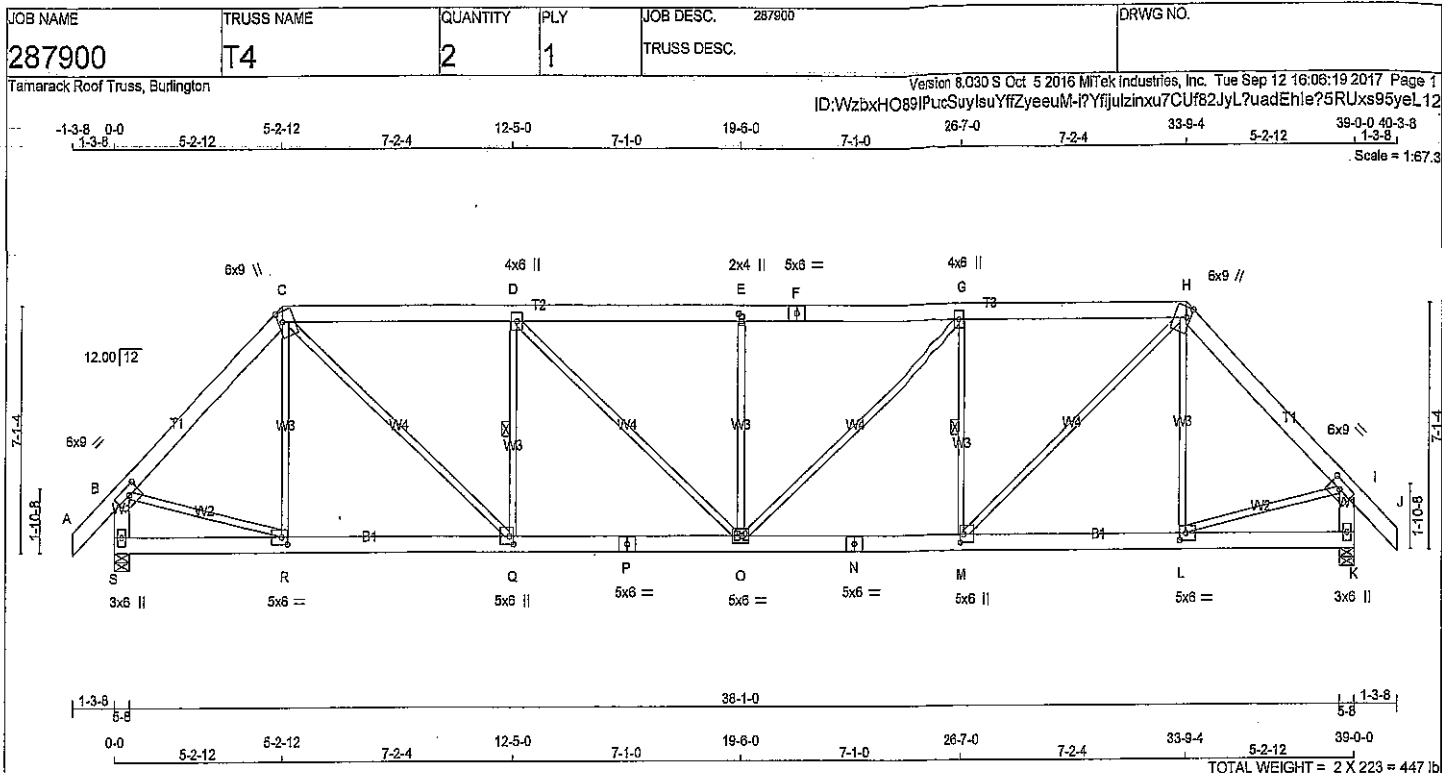
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.75 (S) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - J	2x6	DRY No.2	SPF
S - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - N	2x6	DRY No.2	SPF
N - K	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	6.0	9.0	3.50	1.50
D	TMVW-t	MT20	4.0	6.0		
E	TMVW-w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	3.50	1.50
I	TMVW-t	MT20	6.0	9.0	2.75	4.00
K	BMV-t+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.25
M	BMVW-t	MT20	5.0	6.0	2.75	1.50
N	BS-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.75	1.50
R	BMVW-t	MT20	5.0	6.0	2.50	2.25
S	BMV-t+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG UPLIFT IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
S	3107	0	3107	0	5-8	5-8
K	3107	0	3107	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
S	2411	1602 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0
K	2411	1602 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-Q, G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED (PLF)	MAX. FACTORED (LBS)			MAX. FACTORED (LBS)	MAX. FACTORED (LBS)
FR-TO						FR-TO		
A-B	0 / 63	-122.2	-122.2	0.09 (1)	10.00	R-C	-301 / 144	0.23 (1)
B-C	-2891 / 0	-122.2	-122.2	0.35 (1)	4.60	C-Q	0 / 2451	0.55 (1)
C-D	-3835 / 0	-122.2	-122.2	0.56 (1)	3.85	Q-D	-1461 / 0	0.43 (1)
D-E	-4364 / 0	-122.2	-122.2	0.59 (1)	3.61	D-O	0 / 724	0.16 (1)
E-F	-4364 / 0	-122.2	-122.2	0.59 (1)	3.61	O-E	-800 / 0	0.61 (1)
F-G	-4364 / 0	-122.2	-122.2	0.59 (1)	3.61	O-G	0 / 724	0.16 (1)
G-H	-3835 / 0	-122.2	-122.2	0.56 (1)	3.85	M-G	-1461 / 0	0.43 (1)
H-I	-2891 / 0	-122.2	-122.2	0.35 (1)	4.60	M-H	0 / 2451	0.55 (1)
I-J	0 / 63	-122.2	-122.2	0.09 (1)	10.00	L-H	-301 / 144	0.23 (1)
S-B	-3055 / 0	0.0	0.0	0.21 (1)	6.03	B-R	0 / 2097	0.47 (1)
K-I	-3055 / 0	0.0	0.0	0.21 (1)	6.03	L-I	0 / 2097	0.47 (1)
S-R	0 / 0	-28.0	-28.0	0.13 (2)	10.00			
R-Q	0 / 2030	-28.0	-28.0	0.32 (1)	10.00			
Q-P	0 / 3636	-28.0	-28.0	0.53 (1)	10.00			
P-O	0 / 3636	-28.0	-28.0	0.53 (1)	10.00			
O-N	0 / 3636	-28.0	-28.0	0.53 (1)	10.00			
N-M	0 / 3636	-28.0	-28.0	0.53 (1)	10.00			
M-L	0 / 2030	-28.0	-28.0	0.32 (1)	10.00			
L-K	0 / 0	-28.0	-28.0	0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 36.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 56.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 36.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.59 (E-G:1), BC=0.53 (O-Q:1),
WB=0.61 (E-O:1), SSI=0.32 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

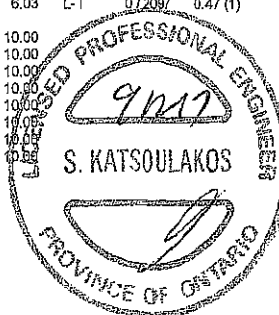
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (R) (INPUT = 0.90)
JSI METAL= 0.65 (N) (INPUT = 1.00)

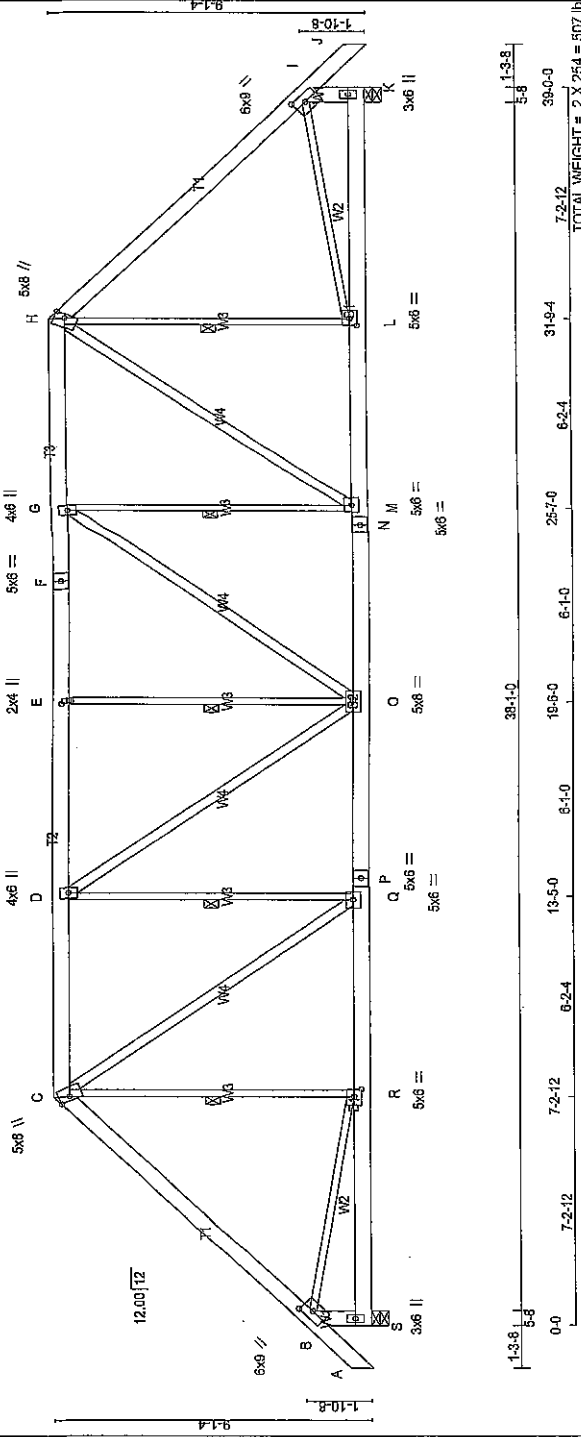


DRWG NO. YAH46217
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 46917 - 177
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287900	T6	2	1	TRUSS DESC.	

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Tue Sep 12 16:03:20 2017 Page 11
 ID: WzbxH089gPucSjyUuYfZjeuM-AB61w3vNkOvNWHngDsZyVYX2d1cf15AeghPhYel,11
 38-0-0 40-3-6
 1-3-8
 Scale = 1/8" = 1'-0"



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
DESCR.		LUMBER		FACTORED		MAXIMUM		REQUIRED	
N.L.G.A. RULES		SIZE		GROSS REACTION		GROSS REACTION		BRG	
CHORDS		No.2		VERT		HORIZ		IN-SX	
A - C	2x6	DRY	No.2	3107	0	3107	0	5-8	5-8
F - H	2x6	DRY	No.2	3107	0	3107	0	5-8	5-8
S - B	2x6	DRY	No.2	3107	0	3107	0	5-8	5-8
K - I	2x6	DRY	No.2	3107	0	3107	0	5-8	5-8
P - N	2x6	DRY	No.2	3107	0	3107	0	5-8	5-8
S - N	2x6	DRY	No.2	3107	0	3107	0	5-8	5-8
N - K	2x6	DRY	No.2	3107	0	3107	0	5-8	5-8
UNFACTORED REACTIONS									
ST. CASE		MAX. MIN.		COMPONENT REACTIONS		WIND		DEAD	
COMBINED		LIVE		PERM. LIVE		WIND		DEAD	
JT	2411	1802/0	410/0	0/0	0/0	0/0	399/0	0/0	0/0
S	2411	1802/0	410/0	0/0	0/0	0/0	399/0	0/0	0/0
K	2411	1802/0	410/0	0/0	0/0	0/0	399/0	0/0	0/0
BEARING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATEROALLY RESTRAINED.									
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, D-Q, E-O, G-M, H-L.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB. MAX. FACTORED									
FORCE (LBS)									
FR-TO									
A-B									
B-C									
C-D									
D-E									
E-F									
F-G									
G-H									
H-I									
I-J									
J-K									
K-L									
S-R									
R-Q									
Q-P									
P-O									
O-N									
N-M									
M-L									
L-K									

DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH.	LL =	38.3	PSF						
BOT CH.	LL =	3.0	PSF						
DL	=	10.5	PSF						
DL	=	7.0	PSF						
TOTAL LOAD	=	58.7	PSF						
SPACING = 24.0 IN. OC									
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF OBC 2012, SBC 2012, ABC 2014									
- CSA 086-09									
- TFC 2011									
(65% OF 64.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEFL.(LL) = L/360 (1.30")									
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")									
ALLOWABLE DEFL.(LL) = L/360 (1.30")									
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")									
CSI: TC=0.70 (B-C-1), BC=0.41 (O-Q-1), WB=0.82 (G-M-1), SSF=0.27 (C-D-1)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
COMP=1.10 SHEAR=1.10 TENS=1.10									
COMPANION LIVE LOAD FACTOR = 0.50									
AUTOSOLVE HEELS OFF									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									
PLATE GRIP(OR) SHEAR SECTION									
(PSI) (PL)									
MAX MIN MAX MIN MAX MIN									
MT20 618 354 1687 822 2284 1656									
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.88 (R) (INPUT = 0.90)									
JSI METAL= 0.55 (B) (INPUT = 1.00)									

7047

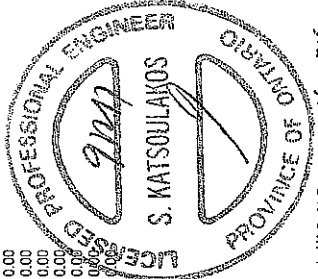
REGISTERED PROFESSIONAL ENGINEER

S. KATSOUKAKOS

PROVINCE OF ONTARIO

DWOND, TAN 46284-17

STRUCTURAL



DWG NO. TAN 46284-17
 STRUCTURAL
 COMPONENT ONLY

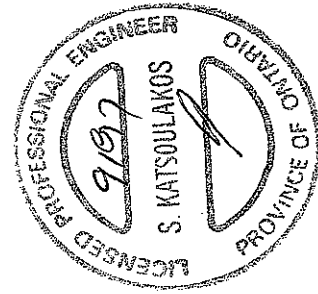
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PLATES (table is in inches)
JT TYPE PLATES W L E N Y X
R BMWWW-1 MT20 5.0 6.0
S BS-1 MT20 5.0 6.0 4.25 2.00
T BMWW-1 MT20 5.0 6.0
U BMWW-1 MT20 6.0 9.0
V BMWW-1 MT20 5.0 6.0
W BMV1+p MT20 3.0 6.0

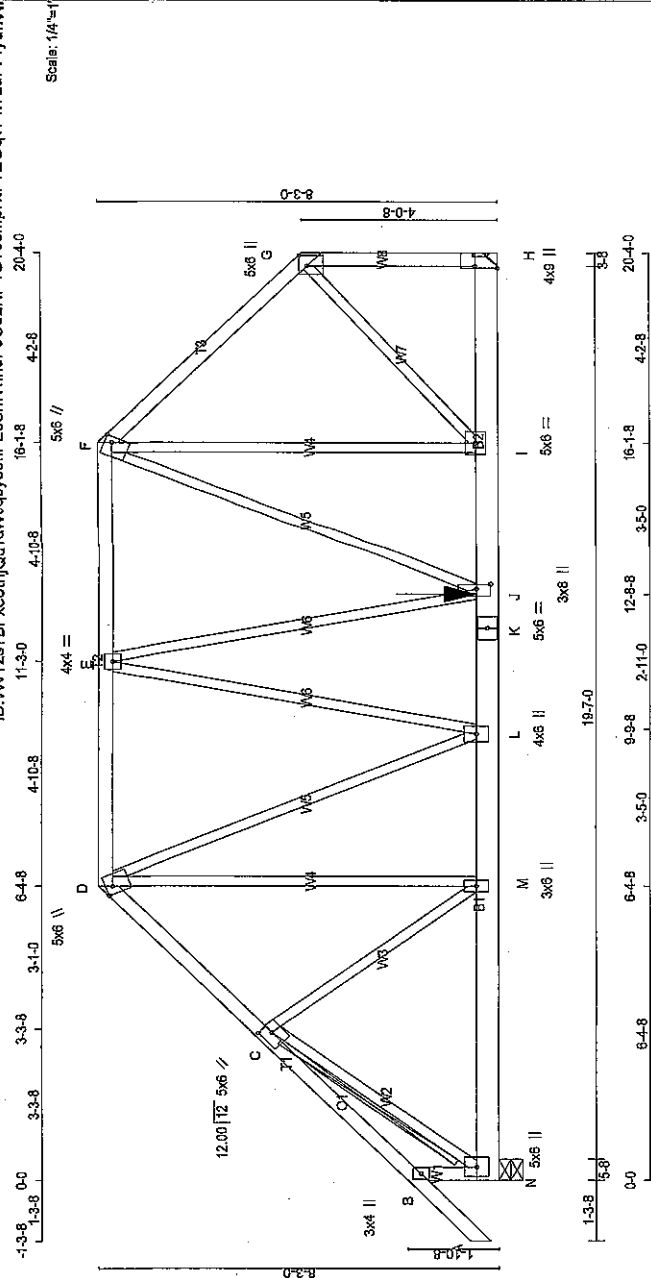
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGERS(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 343.1 lbs FACTORED DOWN AT 30-9-4,
147.1 lbs FACTORED DOWN AT 21-4-12, 147.1
lbs FACTORED DOWN AT 23-4-12, 147.1 lbs
FACTORED DOWN AT 25-4-12, AND 147.1 lbs
FACTORED DOWN AT 27-4-12, AND 147.1 lbs
FACTORED DOWN AT 28-8-8 ON TOP CHORD,
AND 2196.0 lbs FACTORED DOWN AT 20-5-8,
69.9 lbs FACTORED DOWN AT 21-4-12, 69.9 lbs
FACTORED DOWN AT 23-4-12, 69.9 lbs
FACTORED DOWN AT 25-4-12, 69.9 lbs
FACTORED DOWN AT 27-4-12, 69.9 lbs
FACTORED DOWN AT 28-8-8, AND 69.9 lbs
FACTORED DOWN AT 30-9-8, AND 69.9 lbs
FACTORED DOWN AT 31-10-0 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)					FACE	DIR.	TYPE
JT	LOC.	LC1	MAX-	MAX+			
J	30-9-4	-180	-180	—	FRONT	VERT	SNOW
N	30-9-8	-40	-70	—	FRONT	VERT	TOTAL
O	25-4-12	-40	-70	—	FRONT	VERT	TOTAL
P	20-5-8	-2196	-2196	—	FRONT	VERT	TOTAL
X	21-4-12	-147	-147	—	FRONT	VERT	TOTAL
Y	23-4-12	-147	-147	—	FRONT	VERT	TOTAL
Z	27-4-12	-147	-147	—	FRONT	VERT	TOTAL
AA	28-8-8	-147	-147	—	FRONT	VERT	TOTAL
AB	21-4-12	-40	-70	—	FRONT	VERT	TOTAL
AC	23-4-12	-40	-70	—	FRONT	VERT	TOTAL
AD	27-4-12	-40	-70	—	FRONT	VERT	TOTAL
AE	28-8-8	-40	-70	—	FRONT	VERT	TOTAL
AF	31-10-0	-40	-70	—	FRONT	VERT	TOTAL



per
DWG NO. T7 46 91B-17
STRUCTURAL
COMPONENT ONLY



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
BOT CH. LL = 10.5 PSF
TOTAL LOAD = 58.7 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55% OF 54.4 P.S.F. G.S.I. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFLECTION (LL) = L/360 (0.06")
CALCULATED VERT. DEFLECTION (LL) = L/360 (0.06")
ALLOWABLE DEFLECTION (TL) = L/360 (0.06")
CALCULATED VERT. DEFLECTION (TL) = L/360 (0.06")

CSI: TC=0.02 (G-H-I), BC=0.34 (J-L-I), WB=0.93
(C-N-I), SS=0.32 (D-E-I)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

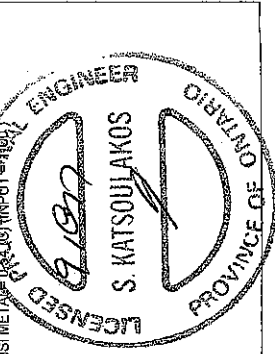
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PLI) (PLI)
MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (MIN) (MAX)
JSI METAL GRIP= 0.89 (MIN) (MAX)



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	UPLIFT	BRG
N	2209	0	0	5-8
H	2387	0	0	5-8

UNFACTORED REACTIONS	MAX. LIVE	MIN. PERM. LIVE	WIND	DEAD	SOIL
JT	1703	1152/0	0/0	0/0	0/0
H	1855	1227/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-N
FASTEN T AND L BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER
80% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB	MAX	FACTORED	WEBS	MAX	FORCE	MAX
FR-TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO
A-B	0/60	-122.2	-122.2	0.19	(1)	10.00	C-M	0/115	0.03	(3)				
B-C	0/30	-122.2	-122.2	0.20	(1)	10.00	M-D	0/219	0.05	(3)				
C-D	-1844/0	-122.2	-122.2	0.27	(1)	4.84	D-L	0/846	0.21	(1)				
D-E	-1613/0	-122.2	-122.2	0.59	(1)	4.35	L-E	-655/0	0.68	(1)				
E-F	-1722/0	-122.2	-122.2	0.60	(1)	4.22	E-J	-72/83	0.10	(1)				
F-G	-1616/0	-122.2	-122.2	0.31	(1)	4.57	J-F	0/1502	0.37	(1)				
G-H	-320/0	0.0	0.0	0.04	(1)	7.81	I-F	-729/0	0.93	(1)				
H-G	-2356/0	0.0	0.0	0.62	(1)	5.51	N-C	-2199/0	0.93	(1)				
N-M	0/1242	-28.0	-28.0	0.24	(1)	10.00	I-G	0/1484	0.37	(1)				
M-L	0/1278	-28.0	-28.0	0.23	(2)	10.00								
L-K	0/1736	-28.0	-28.0	0.34	(1)	10.00								
K-J	0/1736	-28.0	-28.0	0.34	(1)	10.00								
J-I	0/1129	-28.0	-28.0	0.27	(1)	10.00								
I-H	0/0	-28.0	-28.0	0.06	(1)	10.00								

FACTORED CONCENTRATED LOADS (LBS)

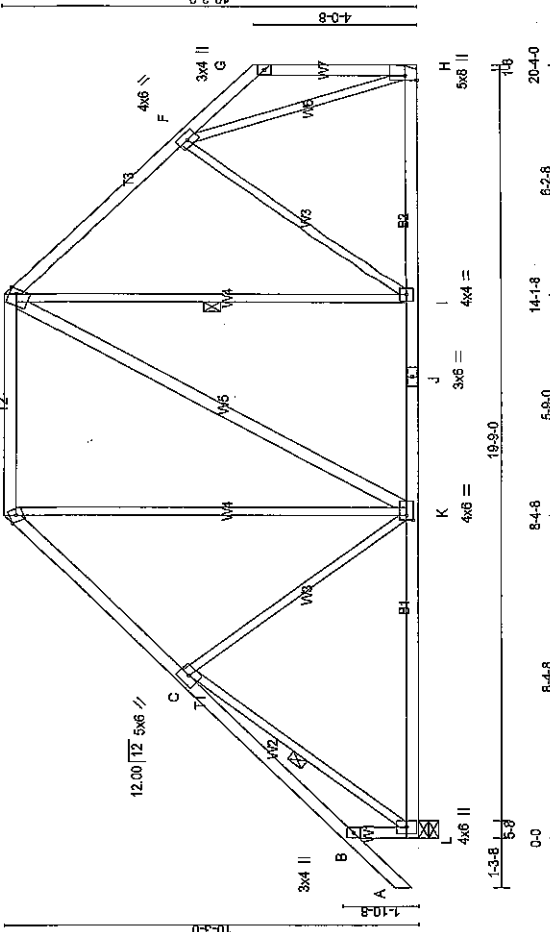
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	TOTAL
J	12-8.8	-1371	-1371	-	FRONT	VERT		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1371.3 lbs FACTORED DOWN AT
12-8.8 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

DWG NO. TAM4699-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



LUMBER		N. L. G. A. RULES		CHORDS		SIZE		DRY		LUMBER		DESCR	
A	- D	2x4	2x4	DRY	No.2								SPF
D	- E	2x4	2x4	DRY	No.2								SPF
L	- B	2x4	2x4	DRY	No.2								SPF
L	- G	2x4	2x4	DRY	No.2								SPF
H	- G	2x4	2x4	DRY	No.2								SPF
L	- J	2x4	2x4	DRY	No.2								SPF
J	- H	2x4	2x4	DRY	No.2								SPF
ALL WEBS				2x3	DRY	No.2							SPF
EXCEPT													
K - E				2x4	DRY	No.2							SPF
DRY: SEASONED LUMBER.													

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

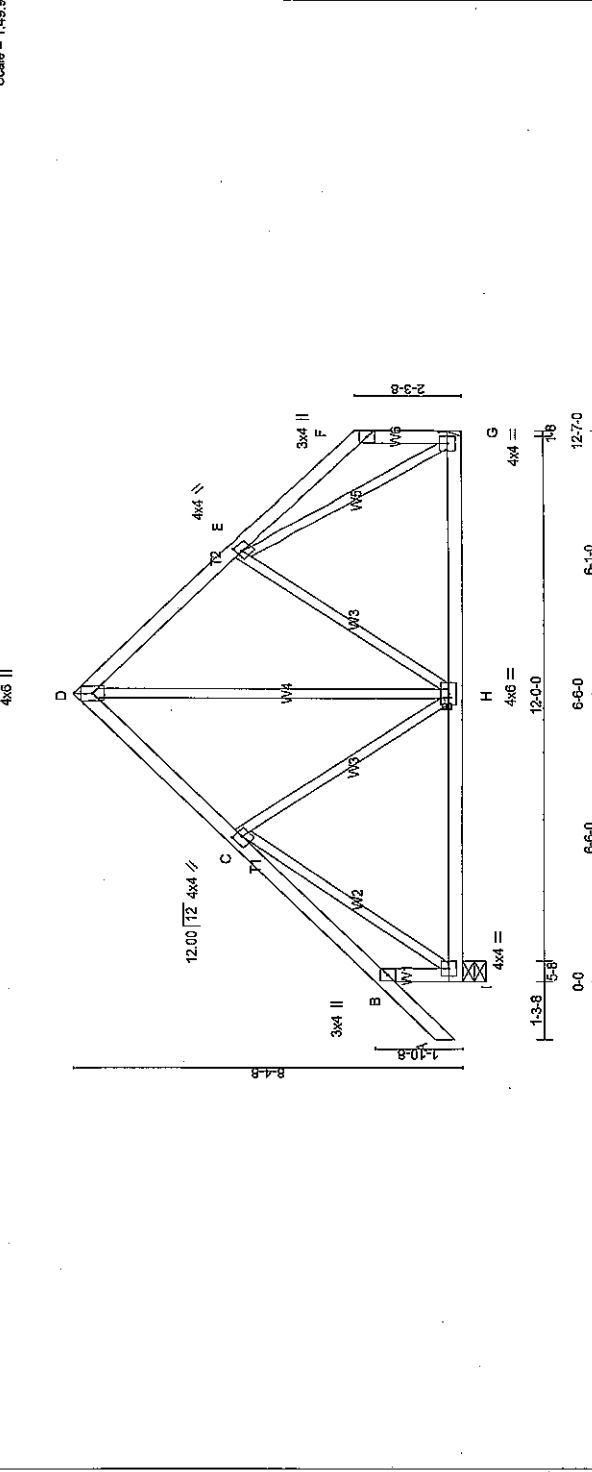
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	287973	DRWG NO.
287973	T10A	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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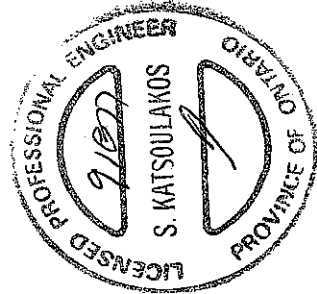
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Scale = 1/4" = 1'-0"



TOTAL WEIGHT = 2 X 65 = 130 LB

LUMBER				DESIGN CRITERIA			
N.L.G.A. RULES	CHORDS	SIZE	DRY	SPECIFIED LOADS:	TOP CH.	LL = 38.3 PSF	
					DL = 3.0 PSF		
					BOT CH.	LL = 10.5 PSF	
					DL = 7.0 PSF		
					TOTAL LOAD	= 58.7 PSF	
					SPACING	= 24.0 IN/C	
					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010		
					THIS DESIGN COMPLIES WITH:		
					- PART 8 OF CBC 2012, BOBC 2012, ABC 2014		
					- CSA 086-09		
					- TPC 2011		
					(65% OF 84.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD		
					ALLOWABLE DEF. (LL) = L/360 (0.42")		
					ALLOWABLE DEF. (LL) = L/969 (0.08")		
					ALLOWABLE DEF. (LL) = L/360 (0.42")		
					CALCULATED VERT. DEF. (LL) = L/988 (0.10")		
					CSI: TC=0.21 (B.C.1) BC=0.37 (H.42), WB=0.48 (C.1), SS=0.15 (H.13)		
					DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10		
					COMPANION LIVE LOAD FACTOR = 0.90		
					TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.		
					NAIL VALUES		
					PLATE GRIP (PS) SHEAR SECTION (PS)		
					MAX MIN MAX MIN MAX MIN		
					MT20 618 554 1387 822 2284 1656		
					PLATE PLACEMENT TOL. = 0.250 inches		
					PLATE ROTATION TOL. = 5.0 Deg.		
					JSI GRIP= 0.83 (C) (INPUT = 0.90)		
					JSI METAL= 0.33 (C) (INPUT = 1.00)		

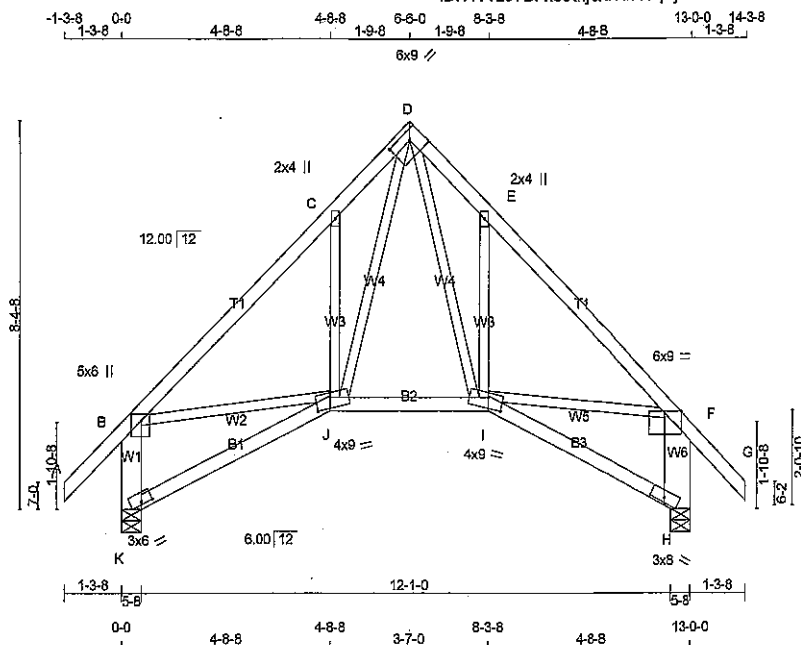


DWG NO. TAN 4692-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	287973	DRWG NO.
287973	T10S	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 72 = 216 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
K - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS	EXCEPT	2x3	DRY	No.2	SPF
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DRY, SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	5.0	6.0	1.75 2.75
C	TMW+w	MT20	2.0	4.0	
D	TTWW-h	MT20	6.0	9.0	2.25 5.50
E	TMW+w	MT20	2.0	4.0	
F	TMW-p	MT20	6.0	9.0	Edge
H	BVM1-I	MT20	3.0	8.0	1.25 4.00
I	BBWW-m	MT20	4.0	9.0	2.75 4.00
J	BBWW-m	MT20	4.0	9.0	2.75 4.00
K	BVM1-I	MT20	3.0	6.0	0.75 3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
K	1147	0	1147	0	0	5-8	5-8	5-8	5-8
H	1147	0	1147	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
K	879	604 / 0	137 / 0	0 / 0	0 / 0
H	879	604 / 0	137 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 60	-122.2 -122.2	0.17 (1)	D-I	0 / 825
B-C	-1056 / 0	-122.2 -122.2	0.34 (1)	I-E	-573 / 0
C-D	-1181 / 0	-122.2 -122.2	0.25 (1)	J-D	0 / 825
D-E	-1181 / 0	-122.2 -122.2	0.25 (1)	J-C	-573 / 0
E-F	-1056 / 0	-122.2 -122.2	0.34 (1)	B-J	0 / 778
F-G	0 / 60	-122.2 -122.2	0.17 (1)	I-F	0 / 778
K-B	-1081 / 0	0.0 0.0	0.09 (1)		
H-F	-1081 / 0	0.0 0.0	0.07 (1)		
K-J	0 / 0	-28.0 -28.0	0.18 (2)		
J-I	0 / 531	-28.0 -28.0	0.16 (2)		
I-H	0 / 0	-28.0 -28.0	0.18 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")
CSI: TC=0.34 (B-C:1), BC=0.18 (H-I:2), WB=0.19 (D-J:1), SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

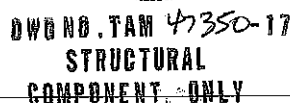


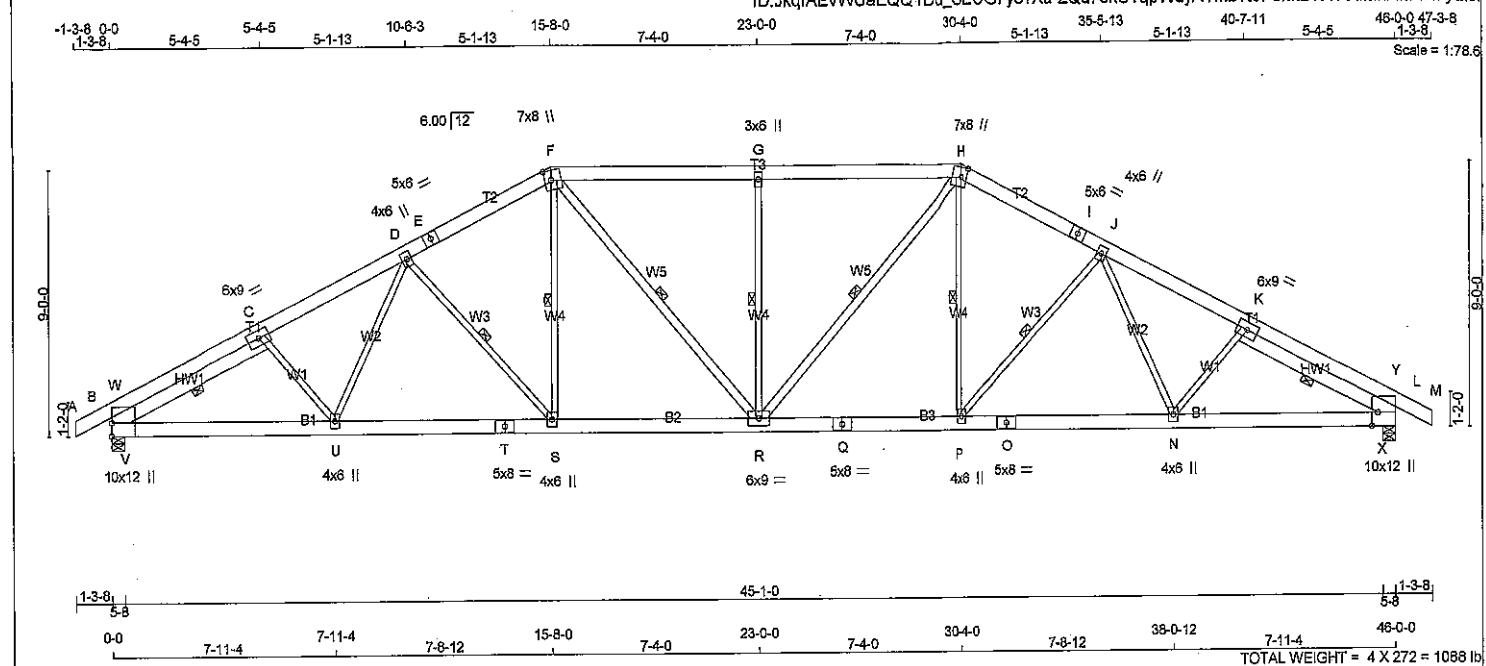
DWG NO. TAM 46922-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 39 lb

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x6	DRY	1650F 1.5E	SPF
F - H	2x6	DRY	1650F 1.5E	SPF
H - I	2x6	DRY	1650F 1.5E	SPF
I - M	2x6	DRY	1650F 1.5E	SPF
B - T	2x6	DRY	1650F 1.5E	SPF
T - Q	2x6	DRY	1650F 1.5E	SPF
Q - O	2x6	DRY	1650F 1.5E	SPF
O - L	2x6	DRY	1650F 1.5E	SPF

REINFORCING MEMBERS

MEMBER	SIZE	TYPE	NO.	DESCR.
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT

MEMBER	SIZE	TYPE	NO.	DESCR.
F - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW+1	MT20	10.0	12.0	5.50	
C	TMBMW+1	MT20	6.0	9.0		
D	TMBMW+1	MT20	4.0	6.0		
E	TS-1	MT20	5.0	6.0		
F	TTWW+M	MT20	7.0	8.0	4.00	2.75
G	TMW+W	MT20	3.0	6.0		
H	TTWW+M	MT20	7.0	8.0	4.00	2.75
I	TS-1	MT20	5.0	6.0		
J	TMW+W	MT20	4.0	6.0		
K	TMBMW+1	MT20	6.0	9.0		
L	TMBMW+1	MT20	10.0	12.0	Edge	2.50
N, P, S, U						
N	BMWW+1	MT20	4.0	6.0		
O, Q, T						
O	BS-1	MT20	5.0	8.0		
R	BMWW+1	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
B	4741 0	5035 239	-1146 5-8	5-8
L	4741 0	5035 0	-1146 5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1146 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1146 LBS. FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 239 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
B	3577	2724 / 0 483 / 0 0 / 0 187 / -1182 566 / 0 0 / 0
L	3577	2724 / 0 483 / 0 0 / 0 78 / -1182 566 / 0 0 / 0

HORIZONTAL REACTIONS

JT	REACT	0 / 0	0 / 0	171 / -171	0 / 0	0 / 0
B						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-S, F-S, F-R, G-R, H-R, H-P, J-P, C-V, K-X.

LOADING
TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH (LC)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 2	-168.0 -168.0	0.09 (2)	10.00	C-U	-221 / 443	0.10 (3)
B-W	-5888 / 1129	-168.0 -168.0	0.30 (2)	3.97	U-D	-218 / 383	0.15 (10)
W-C	-3428 / 879	-168.0 -168.0	0.27 (2)	4.91	D-S	-1370 / 498	0.57 (2)
C-D	-7252 / 1807	-168.0 -168.0	0.38 (2)	3.53	S-F	-284 / 1263	0.28 (2)
D-E	-8153 / 1645	-168.0 -168.0	0.34 (2)	3.83	F-R	-441 / 1957	0.31 (3)
E-F	-8153 / 1645	-168.0 -168.0	0.34 (2)	3.83	R-G	-1829 / 579	0.88 (2)
F-G	-6197 / 1873	-168.0 -168.0	0.74 (2)	3.36	H-R	-441 / 1957	0.31 (2)
G-H	-6197 / 1873	-168.0 -168.0	0.74 (2)	3.36	P-H	-285 / 1263	0.28 (3)
H-I	-6153 / 1645	-168.0 -168.0	0.34 (3)	3.83	P-J	-1370 / 498	0.57 (3)
I-J	-6153 / 1645	-168.0 -168.0	0.34 (3)	3.83	J-N	-210 / 383	0.15 (9)
J-K	-7252 / 1808	-168.0 -168.0	0.38 (3)	3.53	N-K	-212 / 443	0.10 (2)
K-Y	-3428 / 881	-168.0 -168.0	0.27 (3)	4.91	V-W	-358 / 2710	0.00 (1)
Y-L	-5888 / 1131	-168.0 -168.0	0.30 (3)	3.97	V-C	-4353 / 958	0.54 (2)
L-M	0 / 2	-168.0 -168.0	0.09 (3)	10.00	K-X	-4353 / 957	0.54 (3)
					X-Y	-360 / 2710	0.00 (1)
B-V	-865 / 3043	-28.0 -28.0	0.25 (2)	6.25			
V-U	-1654 / 6629	-28.0 -28.0	0.56 (2)	6.25			
U-T	-1495 / 6357	-28.0 -28.0	0.53 (2)	6.25			
T-S	-1495 / 6357	-28.0 -28.0	0.53 (2)	6.25			
S-R	-1189 / 5478	-28.0 -28.0	0.47 (1)	6.25			
R-Q	-1002 / 5478	-28.0 -28.0	0.47 (1)	6.25			
Q-P	-1002 / 5478	-28.0 -28.0	0.47 (1)	6.25			
P-O	-1258 / 6357	-28.0 -28.0	0.53 (3)	6.25			
O-N	-1258 / 6357	-28.0 -28.0	0.53 (3)	6.25			
N-X	-1418 / 6629	-28.0 -28.0	0.58 (3)	6.25			
X-L	-628 / 3043	-28.0 -28.0	0.25 (3)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 51.8 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 74.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(80 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.30")
ALLOWABLE DEFL.(TL) = L/180 (3.07")
CALCULATED VERT. DEFL.(TL) = L/999 (0.37")

CSI: TC=0.74 (G-H-2), BC=0.56 (N-X-3), WB=0.86 (G-R-2), SSI=0.55 (F-G-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.97 (O) (INPUT = 1.00)

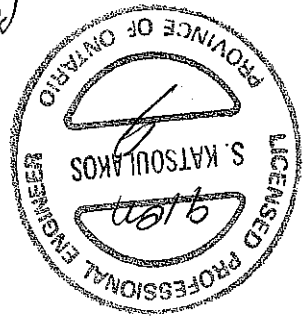
DWG NO. TAM 46923-17
STRUCTURAL
COMPONENT ONLY
CONTINUED ON PAGE 2

JOB NAME	287974	TRUSS NAME	T12	QUANTITY	4	PLY	1	JOB DESC.	287974	TRUSS DESC.		DRWG NO.	
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Tamarack Roof Truss, Burlington

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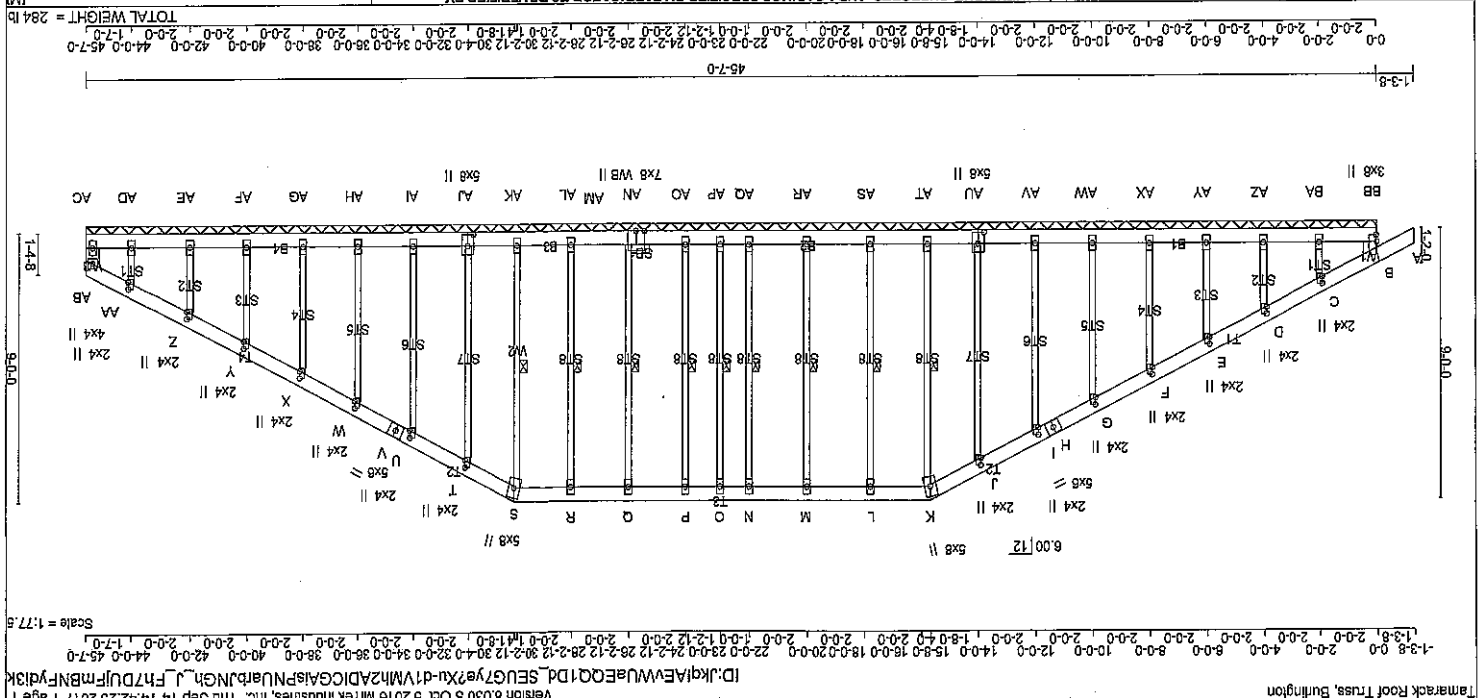
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT
(40.0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS. CpG, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM, INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0.0) FT-IN-SX AWAY
FROM EAVE.



OWG NO. TAM 46923-17
STRUCTURAL
COMPONENT ONLY

[illegible]

	TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8) WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (10.0) FT-IN-SX AWAY FROM EAVE.	
	PROVINCE OF ONTARIO S. KATSOULAKOS LICENSED PROFESSIONAL ENGINEER 9107 OWNED, JAN 469217 STRUCTURAL COMPONENT ONLY	

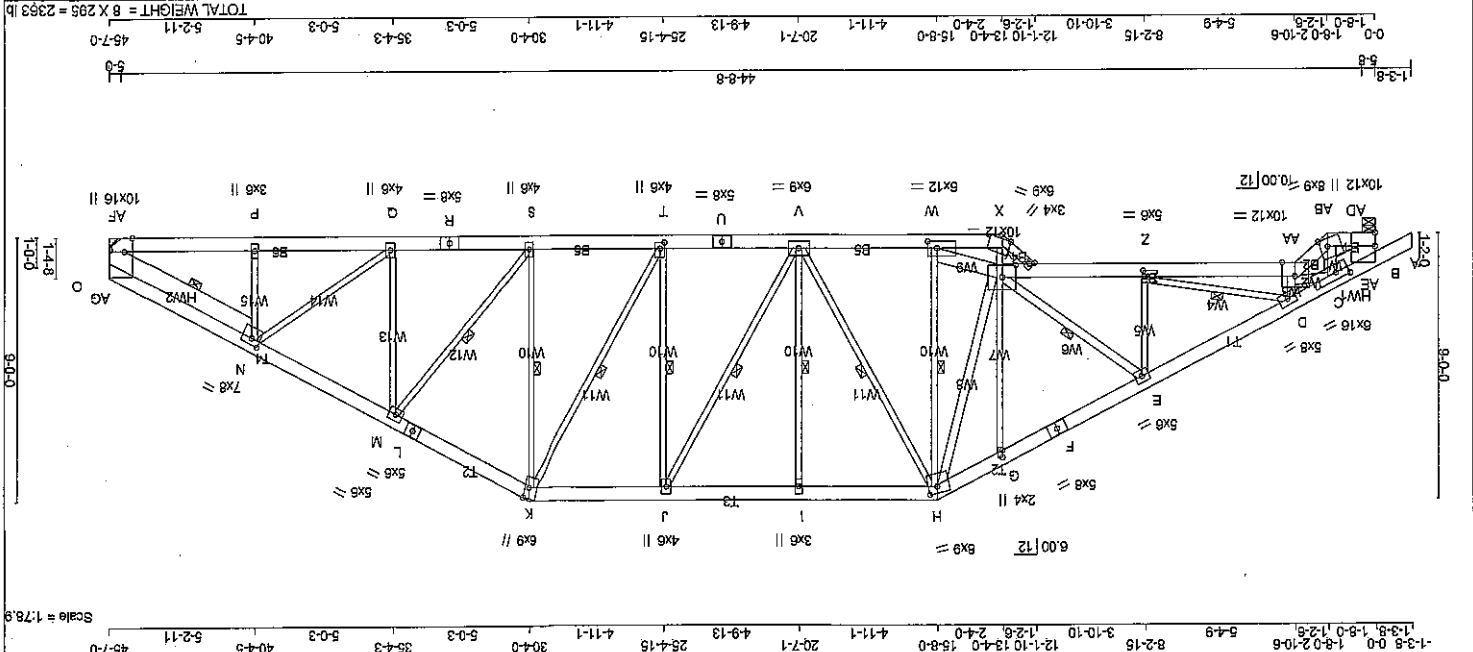


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LUMBER		CHORDS		BEARINGS		BRACING		LOADING		DESIGN CRITERIA	
BB	2x6	BB	2x6	BB	2x6	BB	2x6	BB	2x6	BB	2x6
BA	2x6	BA	2x6	BA	2x6	BA	2x6	BA	2x6	BA	2x6
AZ	2x6	AZ	2x6	AZ	2x6	AZ	2x6	AZ	2x6	AZ	2x6
AY	2x6	AY	2x6	AY	2x6	AY	2x6	AY	2x6	AY	2x6
AX	2x6	AX	2x6	AX	2x6	AX	2x6	AX	2x6	AX	2x6
AV	2x6	AV	2x6	AV	2x6	AV	2x6	AV	2x6	AV	2x6
AU	2x6	AU	2x6	AU	2x6	AU	2x6	AU	2x6	AU	2x6
AT	2x6	AT	2x6	AT	2x6	AT	2x6	AT	2x6	AT	2x6
AS	2x6	AS	2x6	AS	2x6	AS	2x6	AS	2x6	AS	2x6
AR	2x6	AR	2x6	AR	2x6	AR	2x6	AR	2x6	AR	2x6
AQ	2x6	AQ	2x6	AQ	2x6	AQ	2x6	AQ	2x6	AQ	2x6
AP	2x6	AP	2x6	AP	2x6	AP	2x6	AP	2x6	AP	2x6
AO	2x6	AO	2x6	AO	2x6	AO	2x6	AO	2x6	AO	2x6
AN	2x6	AN	2x6	AN	2x6	AN	2x6	AN	2x6	AN	2x6
AM	2x6	AM	2x6	AM	2x6	AM	2x6	AM	2x6	AM	2x6
AL	2x6	AL	2x6	AL	2x6	AL	2x6	AL	2x6	AL	2x6
AK	2x6	AK	2x6	AK	2x6	AK	2x6	AK	2x6	AK	2x6
AJ	2x6	AJ	2x6	AJ	2x6	AJ	2x6	AJ	2x6	AJ	2x6
AI	2x6	AI	2x6	AI	2x6	AI	2x6	AI	2x6	AI	2x6
AH	2x6	AH	2x6	AH	2x6	AH	2x6	AH	2x6	AH	2x6
AG	2x6	AG	2x6	AG	2x6	AG	2x6	AG	2x6	AG	2x6
AF	2x6	AF	2x6	AF	2x6	AF	2x6	AF	2x6	AF	2x6
AE	2x6	AE	2x6	AE	2x6	AE	2x6	AE	2x6	AE	2x6
AD	2x6	AD	2x6	AD	2x6	AD	2x6	AD	2x6	AD	2x6
AC	2x6	AC	2x6	AC	2x6	AC	2x6	AC	2x6	AC	2x6

LUMBER		CHORDS		BEARINGS		BRACING		LOADING		DESIGN CRITERIA	
BB	2x6	BB	2x6	BB	2x6	BB	2x6	BB	2x6	BB	2x6
BA	2x6	BA	2x6	BA	2x6	BA	2x6	BA	2x6	BA	2x6
AZ	2x6	AZ	2x6	AZ	2x6	AZ	2x6	AZ	2x6	AZ	2x6
AY	2x6	AY	2x6	AY	2x6	AY	2x6	AY	2x6	AY	2x6
AX	2x6	AX	2x6	AX	2x6	AX	2x6	AX	2x6	AX	2x6
AV	2x6	AV	2x6	AV	2x6	AV	2x6	AV	2x6	AV	2x6
AU	2x6	AU	2x6	AU	2x6	AU	2x6	AU	2x6	AU	2x6
AT	2x6	AT	2x6	AT	2x6	AT	2x6	AT	2x6	AT	2x6
AS	2x6	AS	2x6	AS	2x6	AS	2x6	AS	2x6	AS	2x6
AR	2x6	AR	2x6	AR	2x6	AR	2x6	AR	2x6	AR	2x6
AQ	2x6	AQ	2x6	AQ	2x6	AQ	2x6	AQ	2x6	AQ	2x6
AP	2x6	AP	2x6	AP	2x6	AP	2x6	AP	2x6	AP	2x6
AO	2x6	AO	2x6	AO	2x6	AO	2x6	AO	2x6	AO	2x6
AN	2x6	AN	2x6	AN	2x6	AN	2x6	AN	2x6	AN	2x6
AM	2x6	AM	2x6	AM	2x6	AM	2x6	AM	2x6	AM	2x6
AL	2x6	AL	2x6	AL	2x6	AL	2x6	AL	2x6	AL	2x6
AK	2x6	AK	2x6	AK	2x6	AK	2x6	AK	2x6	AK	2x6
AJ	2x6	AJ	2x6	AJ	2x6	AJ	2x6	AJ	2x6	AJ	2x6
AI	2x6	AI	2x6	AI	2x6	AI	2x6	AI	2x6	AI	2x6
AH	2x6	AH	2x6	AH	2x6	AH	2x6	AH	2x6	AH	2x6
AG	2x6	AG	2x6	AG	2x6	AG	2x6	AG	2x6	AG	2x6
AF	2x6	AF	2x6	AF	2x6	AF	2x6	AF	2x6	AF	2x6
AE	2x6	AE	2x6	AE	2x6	AE	2x6	AE	2x6	AE	2x6
AD	2x6	AD	2x6	AD	2x6	AD	2x6	AD	2x6	AD	2x6
AC	2x6	AC	2x6	AC	2x6	AC	2x6	AC	2x6	AC	2x6

	TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(b) WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2) BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (10) FT-IN-SX AWAY FROM EAVE	
	287974 2nd DWG NO. T12B 17 STRUCTURAL COMPONENT ONLY 	



DESIGN CRITERIA

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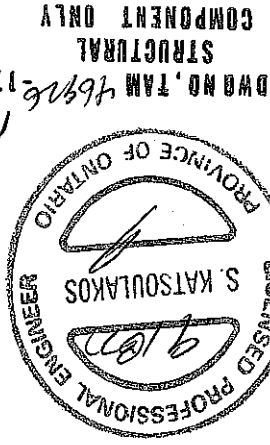
JOB NAME	287974	TRUSS NAME	T12SA	QUANTITY	8	PLY	1	JOB DESC.	287974	TRUSS DESC.		DRWG NO.	
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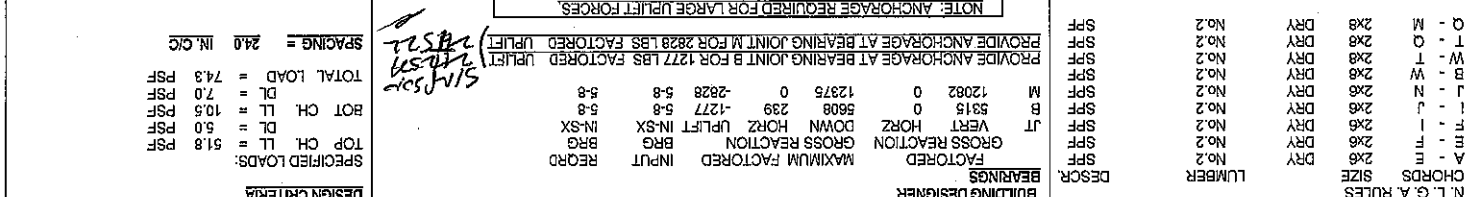
LOADING
TOTAL LOAD CASES: (8)

CHORDS				WEBS			
MAX. FACTORED	FORCE	VERT. LOAD	CS1 (LC)	MAX. MEMB.	FORCE	MAX. FACTORED	CS1 (LC)
FR-TO	(LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH FR-TO	N-AF	4950 / 1024
W-V	-1169 / 5440	-28.0	-28.0	0.81 (2)	6.25	N-AF	4950 / 1024
U-U	-1196 / 5922	-28.0	-28.0	0.52 (1)	6.25	AF-AG	454 / 2907
T-S	-978 / 5316	-28.0	-28.0	0.43 (1)	6.25		
S-R	-1180 / 6097	-28.0	-28.0	0.48 (3)	6.25		
R-O	-1180 / 6097	-28.0	-28.0	0.48 (3)	6.25		
Q-P	-1290 / 6179	-28.0	-28.0	0.51 (3)	6.25		
P-AF	-1289 / 6180	-28.0	-28.0	0.51 (3)	6.25		
A-F-O	-460 / 2172	-28.0	-28.0	0.17 (3)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(b)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT
(40-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CP, Cq, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY
FROM EAVE.



[illegible]

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

9) STRENGTH MEMBERS ON CONCENTRATED LOADS) 4575.0 lbs FACTORED DOWN AND UP AT 2-0-12, 4675.0 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN
UP AT 4-0-12, 4675.0 lbs FACTORED DOWN
UP AT 6-0-12, 4675.0 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN
UP AT 8-0-12, 4675.0 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN
UP AT 10-0-12, 4675.0 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN
UP AT 12-0-12, 4675.0 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN
UP AT 14-0-12, 4675.0 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN
UP AT 16-0-12, 4675.0 lbs FACTORED DOWN AND 1080.7 lbs FACTORED DOWN
UP AT 18-0-12 ON BOTTOM CHORD) DESIGN FOR UNSPECIFIED CONNECTIONS IS DELEGATED TO THE BUILDING DESIGNER.

LOADING					
(8) CHORDS					
MEMB.					
MAX FACTORED	FORCED	VERT LOAD	LCL MAX	MEMB.	FORCE MAX
(LBS)	(LBS)	(KIP)	CSI (LC)	LENGTH	FR-TO
L-V	-3484 / 15375	-28.0	-28.0	0.75 (3)	6.25
L-W	-3484 / 15375	-28.0	-28.0	0.75 (3)	6.25
W-K	-3484 / 15375	-28.0	-28.0	0.75 (3)	6.25
K-X	-11 / 24	-28.0	-28.0	0.93 (3)	6.25
X-J	-11 / 24	-28.0	-28.0	0.93 (3)	6.25
FACTORED CONCENTRATED LOADS (LBS)					
JT	LCL	MAX+	FACE	DIR	TYPE
12.0-12	-4675	1081	BACK	VERT	TOTAL
N	-4675	1081	BACK	VERT	TOTAL
R	-4675	1081	BACK	VERT	TOTAL
S	-4675	1081	BACK	VERT	TOTAL
T	-4675	1081	BACK	VERT	TOTAL
U	-4675	1081	BACK	VERT	TOTAL
V	-4675	1081	BACK	VERT	TOTAL
W	-4675	1081	BACK	VERT	TOTAL
X	-4684	1081	BACK	VERT	TOTAL

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

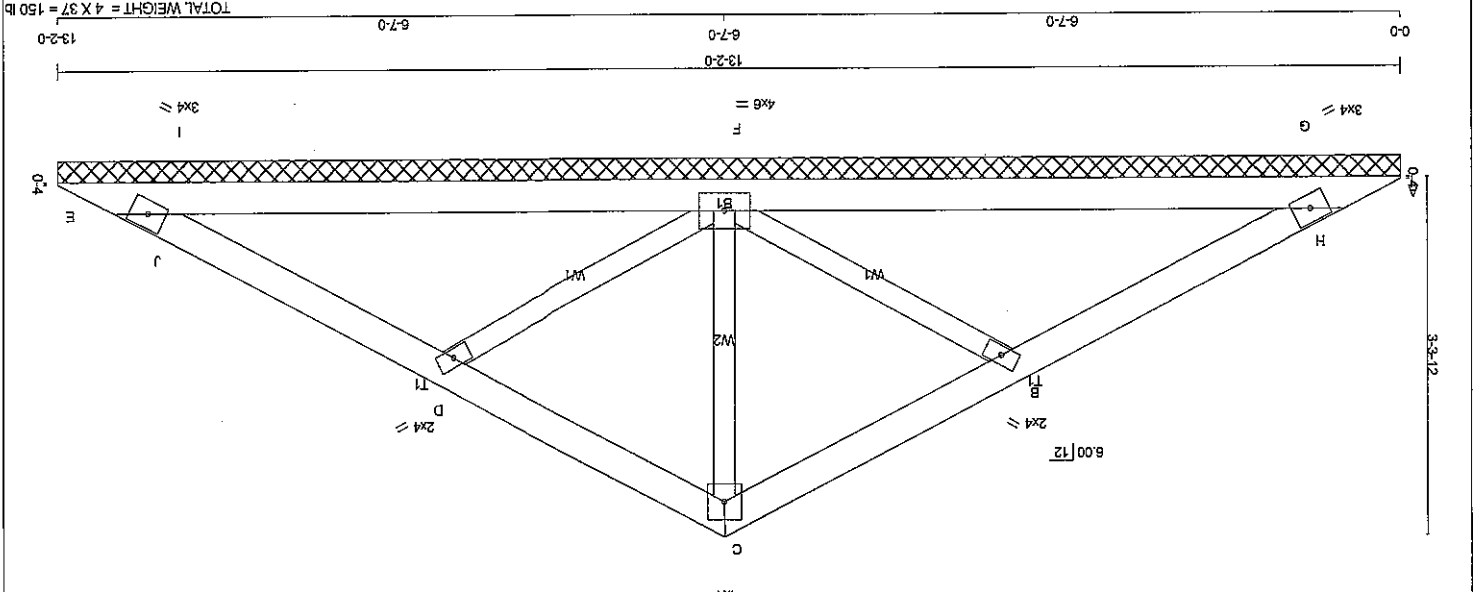
WIND LOAD APPLIED IS DERIVED FROM REFERENCE WIND VELOCITY PRESSURE OF (1.6) PSF
(40.0)-FT H-N-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENT, CPQC, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM, INTERNAL,
WIND TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (4.0)-FT H-N-SX AY
FROM EAVE.
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (4.0)-FT H-N-SX AY

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF 19.6 PSF AT 40-0.0 FT MIN-8X REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE MAIN WIND FORCE RESISTING SYSTEM, INTERNAL WIND PRESSURE IS BASED ON DESIGN CATEGORY 2. BUILDING MAY BE LOCATED ON OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST 10.0 FT MIN-5X AWAY FROM EAVE.



OWANO, TAN 492-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.
287973	P1	4	1	287973	

[illegible]

UNFACTORED REACTIONS									
1ST FLOOR		MAX. MIN. COMPONENT REACTIONS		DEAD		LIVE		PERMALIVE WIND	
UT	TYPE	PLATES	WT	LEN	Y	X	A	E	F
B	BM1-1	MT20	3.0	4.0			138	107.0	14.0
C	TM1-1	MT20	2.0	4.0			139	107.0	14.0
D	TM1-1	MT20	2.0	4.0			1268	782.0	248.0
E	BM1-1	MT20	3.0	4.0					0.0
F	BM1-1	MT20	4.0	6.0					0.0

BEARING MATERIAL TO BE SPFF NO.2 OR BETTER AT JOINT(S) A, E, F									
1ST FLOOR		MAX. MIN. COMPONENT REACTIONS		DEAD		LIVE		PERMALIVE WIND	
UT	TYPE	PLATES	WT	LEN	Y	X	A	E	F
B	BM1-1	MT20	3.0	4.0			138	107.0	14.0
C	TM1-1	MT20	2.0	4.0			139	107.0	14.0
D	TM1-1	MT20	2.0	4.0			1268	782.0	248.0
E	BM1-1	MT20	3.0	4.0					0.0
F	BM1-1	MT20	4.0	6.0					0.0

BRACING									
1ST FLOOR		MAX. MIN. COMPONENT REACTIONS		DEAD		LIVE		PERMALIVE WIND	
UT	TYPE	PLATES	WT	LEN	Y	X	A	E	F
B	BM1-1	MT20	3.0	4.0			138	107.0	14.0
C	TM1-1	MT20	2.0	4.0			139	107.0	14.0
D	TM1-1	MT20	2.0	4.0			1268	782.0	248.0
E	BM1-1	MT20	3.0	4.0					0.0
F	BM1-1	MT20	4.0	6.0					0.0

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.									
1ST FLOOR		MAX. MIN. COMPONENT REACTIONS		DEAD		LIVE		PERMALIVE WIND	
UT	TYPE	PLATES	WT	LEN	Y	X	A	E	F
B	BM1-1	MT20	3.0	4.0			138	107.0	14.0
C	TM1-1	MT20	2.0	4.0			139	107.0	14.0
D	TM1-1	MT20	2.0	4.0			1268	782.0	248.0
E	BM1-1	MT20	3.0	4.0					0.0
F	BM1-1	MT20	4.0	6.0					0.0

65% OF 64.4 P.S.F. G.S.L. PLUS 8.4 P.S.F.									
1ST FLOOR		MAX. MIN. COMPONENT REACTIONS		DEAD		LIVE		PERMALIVE WIND	
UT	TYPE	PLATES	WT	LEN	Y	X	A	E	F
B	BM1-1	MT20	3.0	4.0			138	107.0	14.0
C	TM1-1	MT20	2.0	4.0			139	107.0	14.0
D	TM1-1	MT20	2.0	4.0			1268	782.0	248.0
E	BM1-1	MT20	3.0	4.0					0.0
F	BM1-1	MT20	4.0	6.0					0.0

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCCC 2010									
1ST FLOOR		MAX. MIN. COMPONENT REACTIONS		DEAD		LIVE		PERMALIVE WIND	
UT	TYPE	PLATES	WT	LEN	Y	X	A	E	F
B	BM1-1	MT20	3.0	4.0			138	107.0	14.0
C	TM1-1	MT20	2.0	4.0			139	107.0	14.0
D	TM1-1	MT20	2.0	4.0			1268	782.0	248.0
E	BM1-1	MT20	3.0	4.0					0.0
F	BM1-1	MT20	4.0	6.0					0.0

THIS DESIGN COMPARES WITH:									
1ST FLOOR		MAX. MIN. COMPONENT REACTIONS		DEAD		LIVE		PERMALIVE WIND	
UT	TYPE	PLATES	WT	LEN	Y	X	A	E	F
B	BM1-1	MT20	3.0	4.0			138	107.0	

[illegible][illegible]

020000, TAMIL NADU
STRUCTURAL
COMPONENT ONLY



PLATE ROTATION TOL = 5.0 Deg.
JSI GRIP = 0.54 (C) (INPUT = 0.90)
JSI METAL = 0.25 (B) (INPUT = 1.00)

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE RUSS MANUFACTURING PLANT.

NAIL VALUES

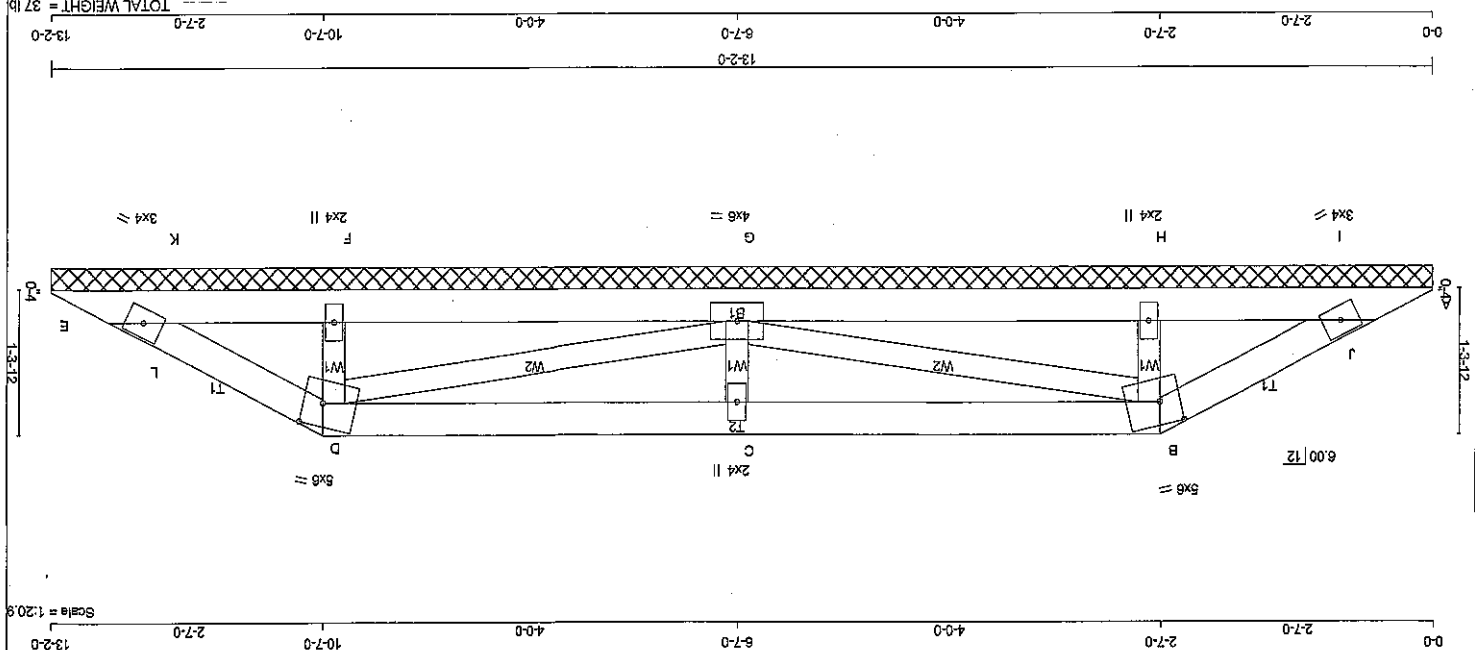
SECTION	SHEAR	MIN MAX (PL)	MIN MAX (PS)	MIN MAX (PL)	MIN MAX (PS)
MT20	618	354	1687	822	2284
					1656

PLATE PLACEMENT TOL = 0.250 inches

THIS DESIGN IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010
THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCSC 2012, CSA 2012
- CSA 086-09
- TPC 2011
(55% OF 54 P.S.F. G.S.L. PLUS 8 P.S.F.
RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED
ROOF LIVE LOAD
CSF: TC=0.32 (B-C-1), BC=0.27 (F-G-2), WB=0
(C-F-1), SS=0.18 (B-H-1)
DOCLUMBER=1.00 VAL=1.00LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH. LT =	38.3	PSF
	BOT CH. LT =	10.5	PSF
	DL =	7.0	PSF
	TOTAL LOAD =	55.7	PSF

[illegible]

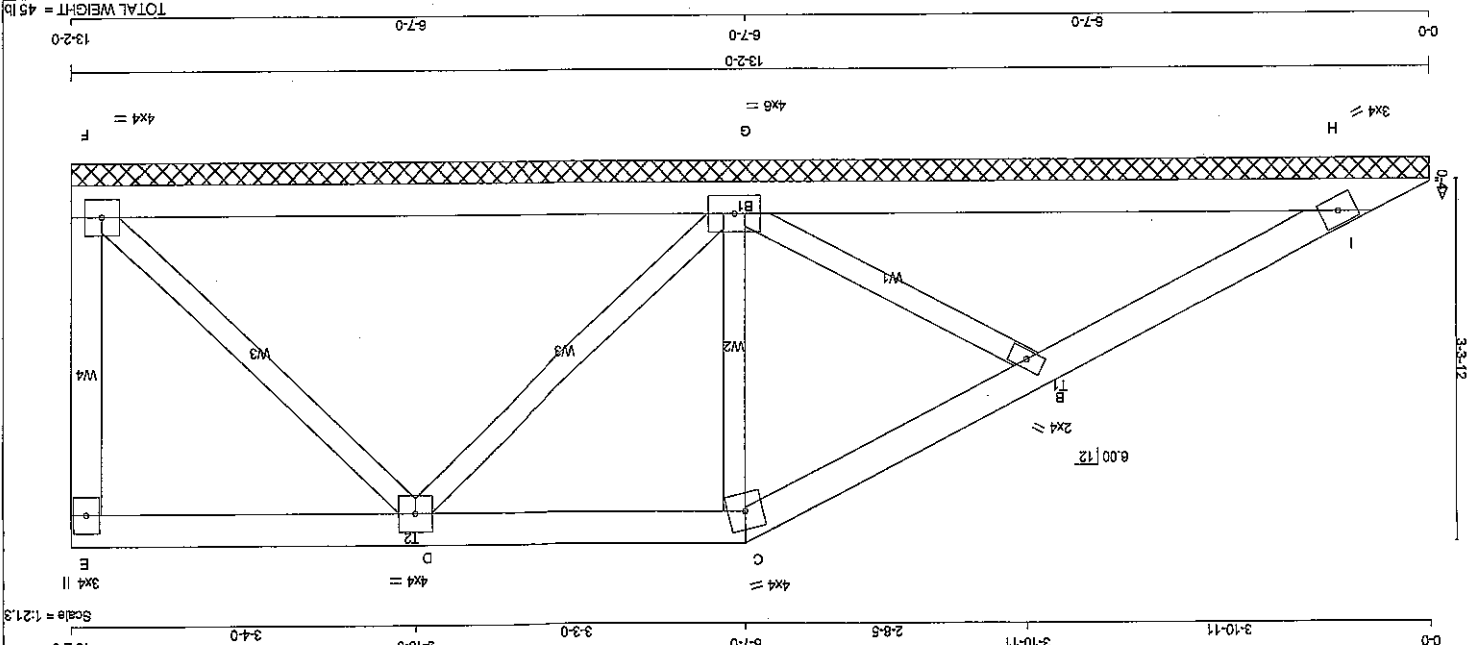
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					
BEARINGS					
	FACTORED	GROSS REACTION	GROSS REACTION	MAXIMUM FACTORED	INPUT
JT	VERT	HORIZ	DOWN	UP/LIFT	IN-SX
A	153	0	153	0	13-2-0
E	153	0	153	0	13-2-0
H	0	0	0	0	13-2-0
G	695	0	695	0	13-2-0
F	488	0	488	0	13-2-0
UNFAIRED REACTIONS					
	1ST LCASE	MAX/MIN COMPONENT REACTIONS			
JT	COMBINED	**SNOW	LIVE	PERMITIVE	WIND
A	117	81/0	18/0	0/0	0/0
E	117	81/0	18/0	0/0	0/0
H	117	81/0	18/0	0/0	0/0
G	384	364/0	85/0	0/0	0/0
F	389	240/0	78/0	0/0	0/0
BRACING					
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.					
MAX. UNBRAECED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.					
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATEROALLY RESTRAINED.					
LOADING TOTAL LOAD CASES: (4)					

CHORDS		MEMB.		FACTORED		FROM TO		LENGTH		MEMB.		FACTORED		WEBS	
FR-TO	(LBS)	FORCE	VERT. LOAD	CS1 (LC)	MAX	CS1 (LC)	MAX	FR-TO	LENGTH	MEMB.	FORCE	MAX	FACTORED	WEBS	
A-J	0/56	-122.2	-122.2	0.05 (1)	10.00	-122.2	-122.2	H-B	-93/0	0.26	0.05 (1)	0.05 (1)			
J-B	0/76	-122.2	-122.2	0.08 (1)	10.00	-122.2	-122.2	B-G	0/26	0.26	0.01 (1)	0.01 (1)			
B-C	0/72	-122.2	-122.2	0.34 (4)	10.00	-122.2	-122.2	G-C	-611/0	0.26	0.09 (1)	0.09 (1)			
C-D	0/72	-122.2	-122.2	0.34 (4)	10.00	-122.2	-122.2	G-D	0/26	0.26	0.01 (1)	0.01 (1)			
L-E	0/58	-122.2	-122.2	0.05 (1)	10.00	-122.2	-122.2	F-D	-93/4	0.26	0.00 (1)	0.00 (1)			
A-1	-73/0	-28.0	-28.0	0.09 (1)	6.25	-28.0	-28.0	K-L	-107/9	0.26	0.00 (1)	0.00 (1)			



46911-17
STRUCTURAL
COMPONENT ONLY

COMPANION LIVE LOAD FACTOR = 0.50
COL LUMBER=100 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS)
(PL)
MAX MIN MAX MIN
WT20 818 354 1667 822 2284 1656
PLATE PLACEMENT TOL = 0.250 inches
CLATE ROTATION TOL = 5.0 Deg.
ISI GRIP=0.32 (C) (NPUR=1.00)
ISI METAL=0.11 (C) (NPUR=1.00)

[illegible]

PLATES (table is in inches)									
JT	TYPE	LEN	Y	X	A	B	C	D	E
198	141/0	28/0	0/0	0/0	29/0	0/0	0/0	0/0	0/0
179	129/0	24/0	0/0	0/0	26/0	0/0	0/0	0/0	0/0
1170	737/0	225/0	0/0	0/0	208/0	0/0	0/0	0/0	0/0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, G BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally Restrained. 65% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, CBCO 2012, ABC 2014 - CSA 086-09 - TPIC 2011									
1ST LOOSE MAX MIN COMPONENT REACTIONS SOIL DEAD PERM.LIVE WIND SLOPE OF 6.00/12 LOADING IN FLAT SECTION BASED ON A									

[illegible]

MT20 018 364 1667 822 2284 1659
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.57 (D) (IN PUT = 0.80)
JSI METAL = 0.28 (B) (IN PUT = 1.00)



Dwg No. TAM 4614-17
 STRUCTURAL
 COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
BOT CH.	LL	=	7.0	PSF
	DL	=	58.7	PSF

TOTAL LOAD = 103.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010

THIS DESIGN COMPLIES WITH:

-PART 9 OF CBC 2012, CBCO 2012, ABC 2014

-CSA 086-09

-TPIC 2011

65 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD/EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.30 (B-C1), BC=0.32 (G-H2), WB=0.22 (D-G-1), SS=0.20 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPACTION LIVE LOAD FACTOR = 0.50

RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

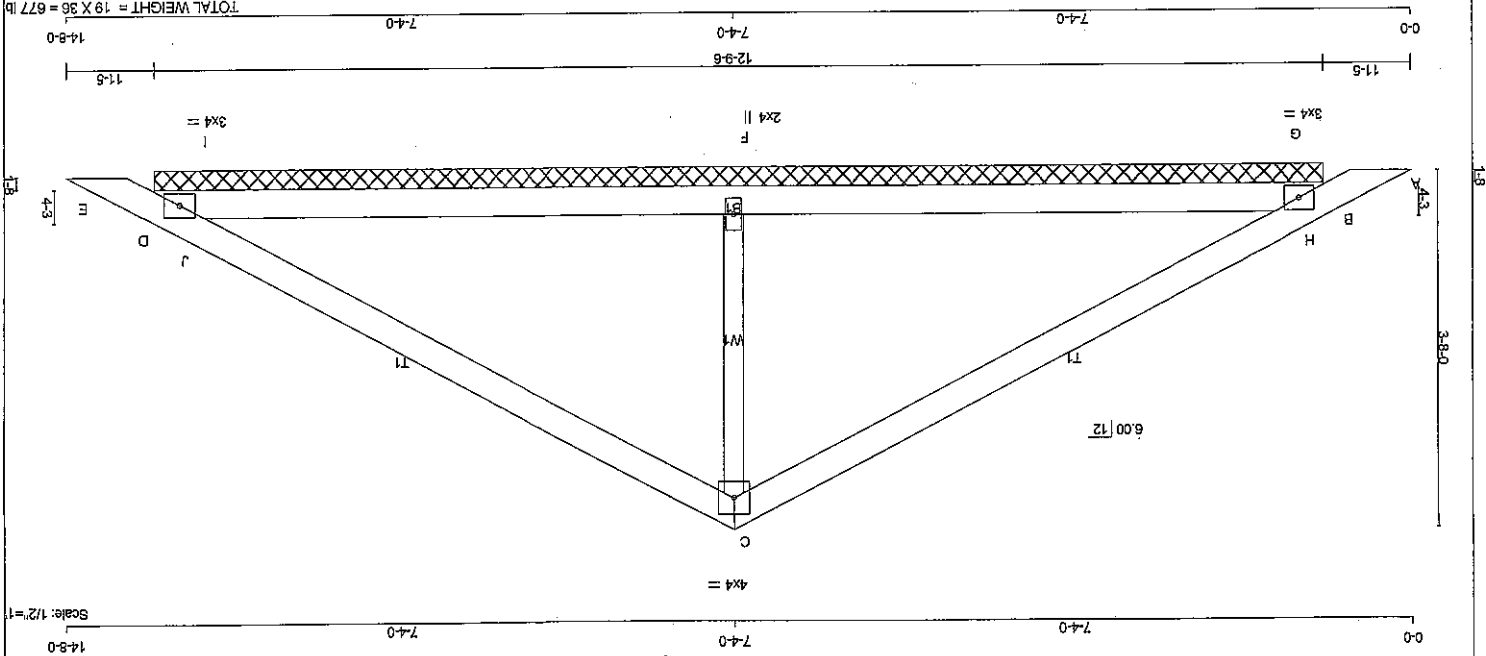
NAIL VALUES

SECTION	SHEAR	(PL)	MAX MIN	(PL)	MAX MIN	MT20
PLATE GRIP(DR)	822	1667	818	354	918	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

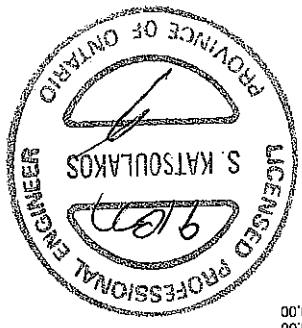
JSI GRIP= 0.57 (D) (INPUT = 0.80)
JSI METAL= 0.26 (B) (INPUT = 1.00)



LUMBER									
N. L. G. A. RULES									
CHORDS									
SIZE									
LUMBER									
DESCR.									
SPF									
SPF									
SPF									
FACTORED									
MAXIMUM FACTORED									
GROSS REACTION									
GROSS REACTION									
DOWN VERT									
UPVERT									
BRG									
RECORD									
INPUT									
RECORD									
UNFACTORED REACTIONS									
1ST LOOSE									
MAX./MIN. COMPONENT REACTIONS									
JT									
B									
D									
F									
773									
0									
661									
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BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
LOADING
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
TOTAL LOAD CASES: (4)
DO NOT EXCEED 1.00 NAIL/1.00 LBS BEND=1.10

CHORDS		FACTORED		MEMB.		WEBS		COMP	
FR-TO	(LBS)	PLF	CS (LC)	UNBRAC	FR-TO	CS (LC)	(LBS)	CS (LC)	MAX FACTORED
FR-TO	VERT. LOAD	MAX	MEMB.	FORCE	MAX	MEMB.	FORCE	MAX	COMPANION FIVE-TO-ONE FACTOR = 0.50-
B-G	0/287	-28.0	-28.0	0.55 (1)	10.00	-28.0	-28.0	0.55 (1)	10.00
G-F	0/287	-28.0	-28.0	0.55 (1)	10.00	-28.0	-28.0	0.55 (1)	10.00
F-I	0/287	-28.0	-28.0	0.55 (1)	10.00	-28.0	-28.0	0.55 (1)	10.00
L-D	0/287	-28.0	-28.0	0.55 (1)	10.00	-28.0	-28.0	0.55 (1)	10.00
D-E	0/22	-122.2	-122.2	0.07 (1)	10.00	-122.2	-122.2	0.07 (1)	10.00
J-D	-147/0	-122.2	-122.2	0.29 (1)	6.25	-122.2	-122.2	0.29 (1)	6.25
C-J	-337/0	-122.2	-122.2	0.62 (1)	6.25	-122.2	-122.2	0.62 (1)	6.25
H-C	-337/0	-122.2	-122.2	0.62 (1)	6.25	-122.2	-122.2	0.62 (1)	6.25
A-B	-147/0	-122.2	-122.2	0.29 (1)	6.25	-122.2	-122.2	0.29 (1)	6.25
B-H	0/22	-122.2	-122.2	0.07 (1)	10.00	-122.2	-122.2	0.07 (1)	10.00
A-B	0.08 (1)	-372/3	F-C	-372/3	0.08 (1)	-372/3	F-C	-372/3	0.08 (1)
B-H	0.00 (1)	-931/127	G-H	-931/127	0.00 (1)	-931/127	G-H	-931/127	0.00 (1)
C-J	0.00 (1)	-931/127	I-J	-931/127	0.00 (1)	-931/127	I-J	-931/127	0.00 (1)
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP (OR) SHEAR SECTION (PL) (PSI) MAX MIN MAX MIN MAX MIN M720 018 354 1667 822 2294 1656 PLATE PLACEMENT TOL = 0.250 inches COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION FIVE-TO-ONE FACTOR = 0.50-									



OWN NO. TAM 46934-17
STRUCTURAL
COMPONENT ONLY

COMPACTION FIVE-TON FACTOR = 0.50 —

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

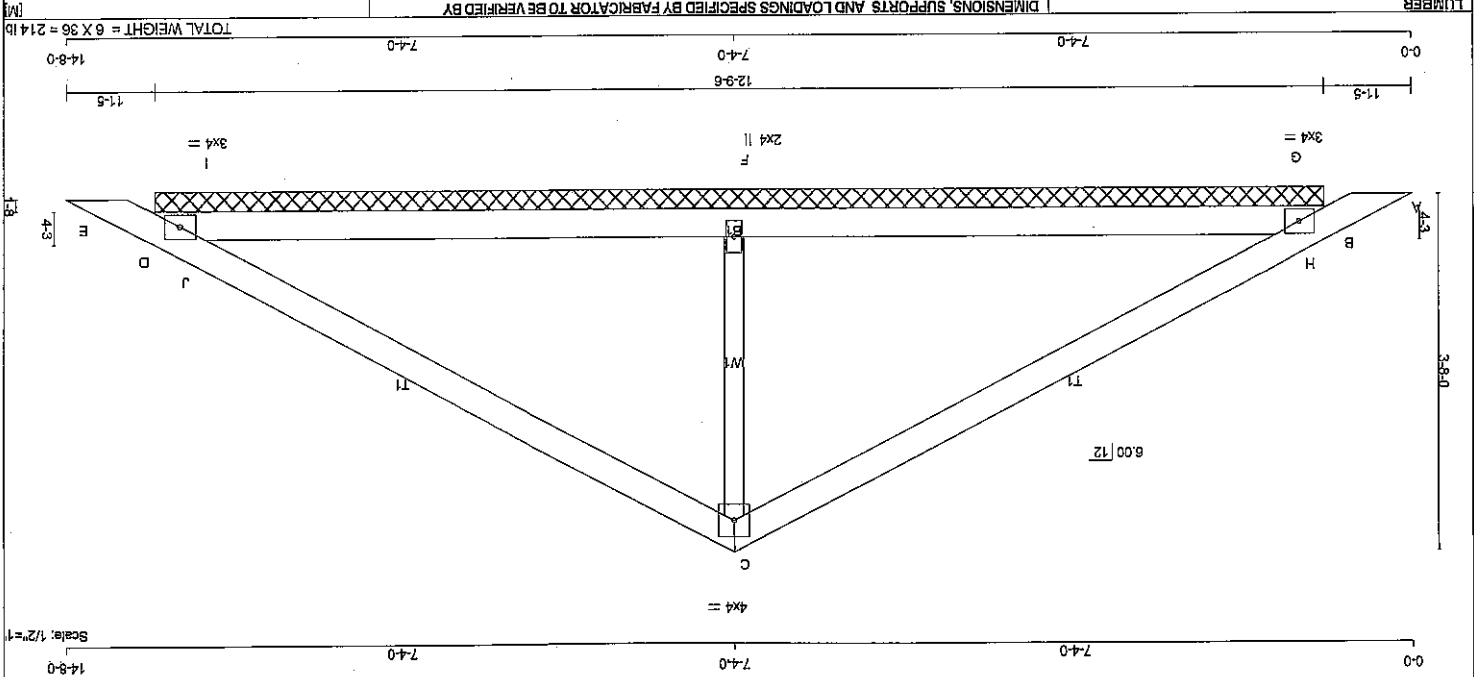
NAIL VALUES

PLATE GRIP (GR)	SHEAR	SECTION
(psi)	(PL)	
MAX MIN	MAX MIN	
MT20	818 354	1687 822
	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.58 (B) (INPUT = 0.90)
JSI METAL = 0.16 (D) (INPUT = 1.00)



N L G A SIZE	CHORDS	A - C	B - D	ALL WEBS	2x3	DRY	No.2	SPP
		A - C	2x4	DRY	No.2	SPP		
		C - E	2x4	DRY	No.2	SPP		
		B - D	2x4	DRY	No.2	SPP		
				ALL WEBS	2x3	DRY	No.2	SPP

DESIGN CONSISTS OF 1 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS	SPACING (IN)	
A-C	12	TOP
C-E	12	TOP
B-D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

PLATES	(tablets in bottles)	JT	TYPE	PLATES	W	LEN	Y	X
C	BTMB-1	MT20	3.0	4.0				
B	TTW-p	MT20	4.0	4.0				
D	BTMB-1	MT20	3.0	4.0				
F	BMV1+w	MT20	2.0	4.0				

MEMB	MAX FACTORED	FORCE	VERT-LOAD CT	MAX	CS1 (LC)	UNBRAC	LENGTH FR TO	F-C	G-H	I-J	-925/126	0.00 (1)
FR-TO	A-B	0.22	-122.2	-122.2	0.02 (1)	10.00	F-C	-371/3				0.03 (1)
B-H	-146/0	-122.2	-122.2	0.10 (1)	6.25	G-H	-925/126					0.00 (1)
C-H	-339/0	-122.2	-122.2	0.20 (1)	6.25	I-J	-925/126					0.00 (1)
C-J	-336/0	-122.2	-122.2	0.20 (1)	6.25							
D-D	-146/0	-122.2	-122.2	0.10 (1)	6.25							
D-E	0.22	-122.2	-122.2	0.02 (1)	10.00							
B-G	0/288	-28.0	-28.0	0.18 (1)	10.00							
G-F	0/288	-28.0	-28.0	0.18 (1)	10.00							
F-I	0/288	-28.0	-28.0	0.18 (1)	10.00							
I-D	0/288	-28.0	-28.0	0.18 (1)	10.00							

[illegible]

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH, LL	=	38.3	PSF
DL	=	3.0	PSF
BOT CH, LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010
THIS DESIGN COMPLIES WITH:
-PART 9 OF OBC 2012, BCBBC 2012, ABC 2014
-CSA 086-09
-TPIC 2011

(55 % OF 54.4 P.S.F. @ S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

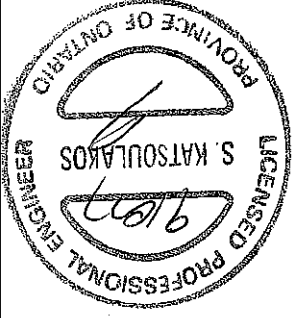
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN

THE TRUSS MANUFACTURING PLANT:

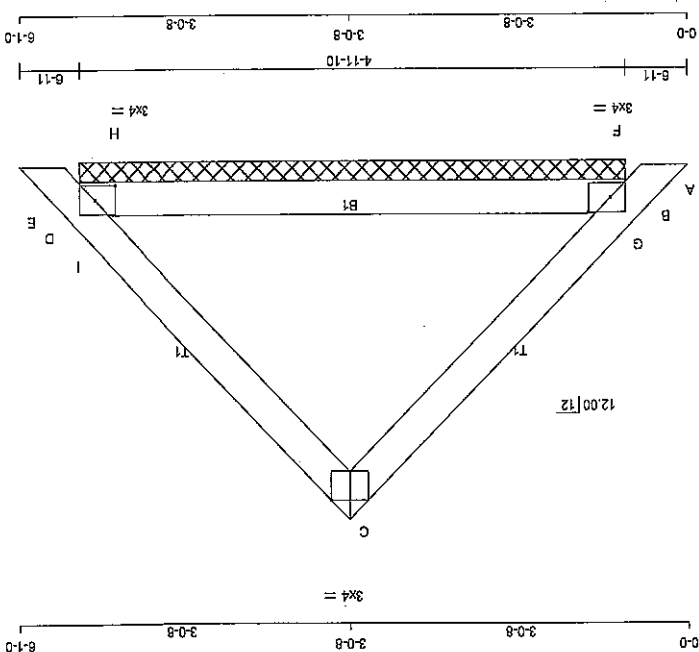
NAIL VALUES	
SECTION	GRIP(DRY)
SHEAR	(PLI)
(PLI)	(PSI)
MAX MIN	MAX MIN
MT20	018 354 1667 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg
JSGRIP = 0.19 (B) (INPUT = 0.90)
JSMETAL = 0.05 (D) (INPUT = 1.00)



OWONG, TAM 46935 17
STRUCTURAL
COMPONENT ONLY

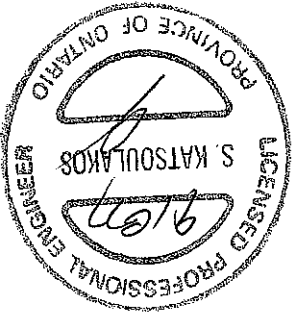


TOTAL WEIGHT = $2 \times 15 = 31 \text{ lb}$

N. L. & A. RULES		CHORDS		DRY: SEASONED LUMBER.	
LUMBER	SIZE	DRY	DRY	DRY	LUMBER
A - C	2x4	DRY	No.2		
B - E	2x4	DRY	No.2		
C - F	2x4	DRY	No.2		
D - G	2x4	DRY	No.2		

BUILDING DESIGNER									
UNIFORMITY, STABILITY AND LOADS IN LINE WITH DESIGNER'S RECOMMENDATION									
BRACING									
FACTORED									
MAXIMUM FACTORED									
RECORD	INPUT	FACTORED	DOWN	GROSS REACTION	VERT	HORIZ	UP/LIFT	BR-SX	IN-SX
4-11-10	4-11-10	4-11-10	4-11-10	4-11-10	4-11-10	4-11-10	4-11-10	4-11-10	4-11-10
D	429	0	429	0	0	0	0	0	0
B	429	0	429	0	0	0	0	0	0
UT	VERT	HORIZ	DOWN	HORIZ	UP/LIFT	BR-SX	IN-SX		
FACTORED									
MAXIMUM FACTORED									
GROSS REACTION									
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BR-SX									
IN-SX									
RECORD									

OWG NO. TAM 4695
STRUCTURAL
COMPONENT ONLY



DESIGN CRITERIA

TOP CHORD LOADS:

SPACING =	24	IN C/C
TOTAL LOAD =	56.7	PSF
BOT CH. DL =	7.0	PSF
BOT CH. LL =	10.5	PSF
DL =	3.0	PSF
TOP CH. LL =	38.3	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLES WITH:

-PART 9 OF CBC 2012, CBC 2012, ABC 2014

- CSA 086-09

- TPC 2011

(5% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUIVALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSF TO=0.13 (C-G-1), BC=0.19 (D-H-3)

(F-G-1), SS=0.19 (D-H-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

--0.50

60M PANNON LIVE LOAD FACTOR=--0.50--

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING SECTION.

NAIL VALUES

SECTION	SHEAR	(PL)	(PS)	MAX MIN	MAX MIN
MIT20	618	354	1687	B22	2284
	1656				

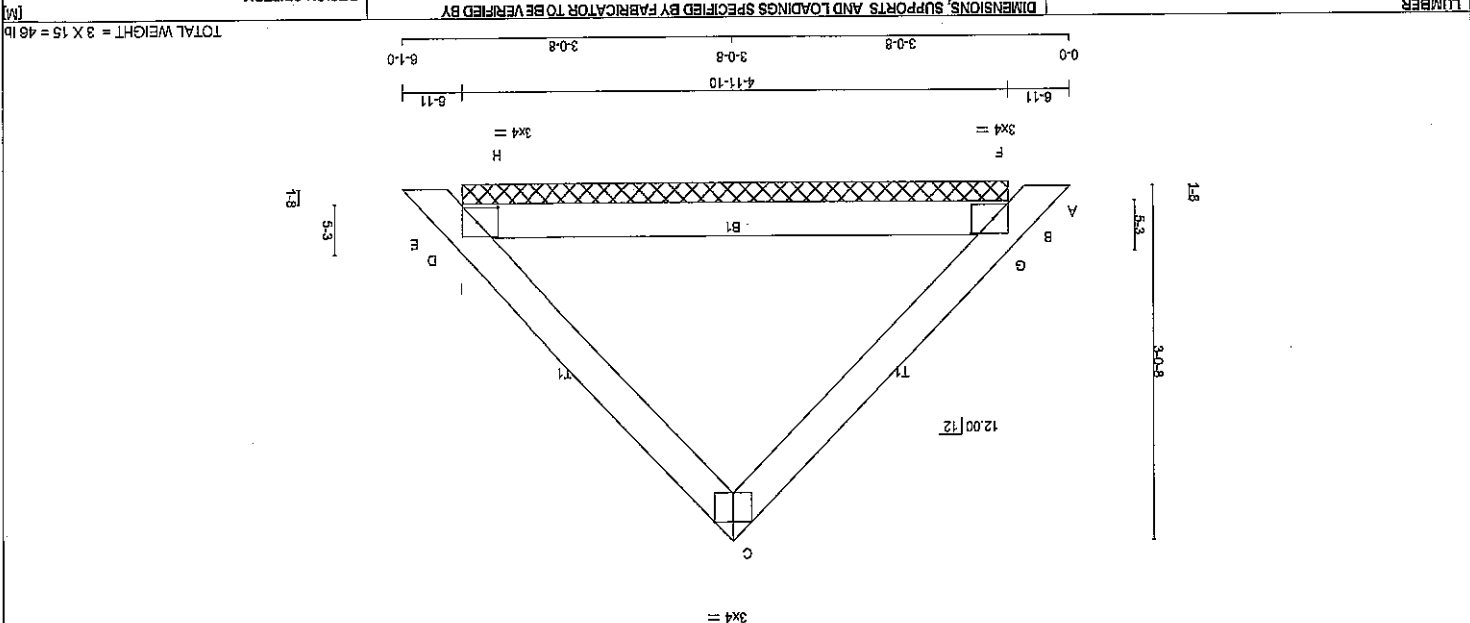
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.36 (B) (INPUT = 1.00)

JSI METAL=0.09 (D) (INPUT = 1.00)

Scale = 1:20.0



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

FACTORED MAXIMUM FACTORED

GROSS REACTION BRG

DOWN HORZ UPLIFT IN-SX

429 0 0 429 0 0 429 0 0 429 0 0

4-11-10 4-11-10 4-11-10 4-11-10 4-11-10 4-11-10

RECORD BRG IN-SX

WIND PERM/LIVE

DEAD SOIL

0/0 52/0 0/0 52/0 0/0 52/0 0/0 52/0 0/0 52/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

MEMB. FORCE

FROM-TO

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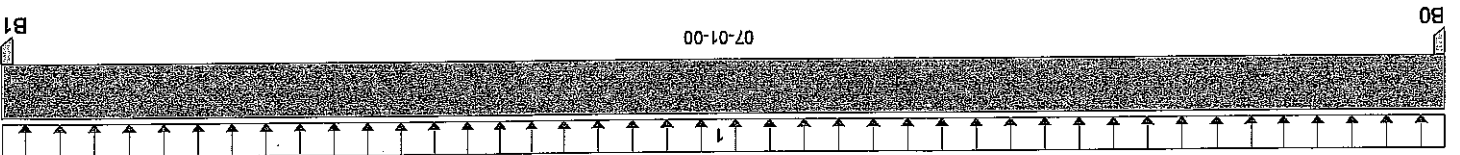


DRWG NO. JAN 4693217
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 38.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.6 PSF
DL = 7.0 PSF
TOTAL LOAD = 58.7 PSF
SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010
THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPOC 2011
(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED
CSF TC=0.04 (C-G-1), BC=0.05 (F-H-2), WB=0.00
(F-G-1), SSF=0.06 (D-H-3)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
-COMPANION LIVE LOAD FACTOR=-0.50
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
MAX MIN MAX MIN
MT20 619 354 1667 822 2284 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg
JSI GRIP= 0.12 (B) (INPUT = 0.80)
JSI METAL= 0.03 (D) (INPUT = 1.00)



Build 6080
Job Name: BAYVIEW WELLINGTON/INNISFIL
Address: S48-2 EL:B
City, Province, Postal Code:
Customer:
Code reports: CCMC 12472-R
Misc:
Company:
Designer: ELLA
Specifier:
Description: Designs\FB01
File Name: BC CALC Project



Total Horizontal Product Length = 07-01-00

Reaction Summary (Down / Uplift) (lbs)			
Bearing	Live	Dead	Snow
B0, 3-1/2"	855 / 0	1,008 / 0	4,220 / 0
B1, 3-1/2"	855 / 0	1,008 / 0	4,220 / 0

Load Summary			
Tag Description	Load Type	Ref. Start	End
1	Upl. Area (lb/ft²)	L 00-00-00	07-01-00
			11
			12
			52
			23-00-00

Controls Summary			
Pos. / Moment	End Shear	Total Load Defl.	Live Load Defl.
12,418 ft-lbs	5,565 lbs	L/575 (0.138")	L/999 (0.114")
20,970 ft-lbs	11,571 lbs	0.331"	n/a
59.2%	48.1%	41.8%	n/a
5	5	13	17
03-06-08	01-01-00	03-06-08	03-06-08
			03-06-08
			03-06-08
			00-00-00

Bearing Supports			
Dim. (L x W)	Demand	Support	Member
3-1/2" x 3-1/2"	8,017 lbs	n/a	53.6%
3-1/2" x 3-1/2"	8,017 lbs	n/a	53.6%
			Unspecified
			Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBC 2010 and CSA O86.
Design based on Dry Service Condition.
Importance Factor : Normal Part code : Part 9



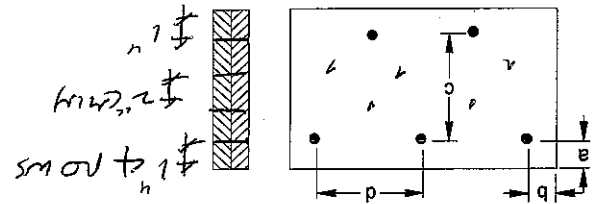
STRUCTURAL
COMPONENT ONLY



Build 6080
Job Name: BAYVIEW WELLINGTON/INNISFIL
Address: S48-2 EL:B
City, Province, Postal Code:
Customer:
Code reports: CCMC 12472-R
Misc:

File Name: BC CALC Project
Description: Designs\FB01
Designer: ELLA
Company:
Misc:

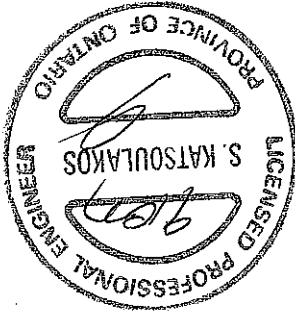
Connection Diagram



a minimum = 4" c = 3-1/2" d = 4" b minimum = 3" Nails
Member has no side loads.
Connectors are: 16d

3/8" ARBOX SPIRAL

Disclosure
Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call 1-800-964-6999 before installation.
BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCIO®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



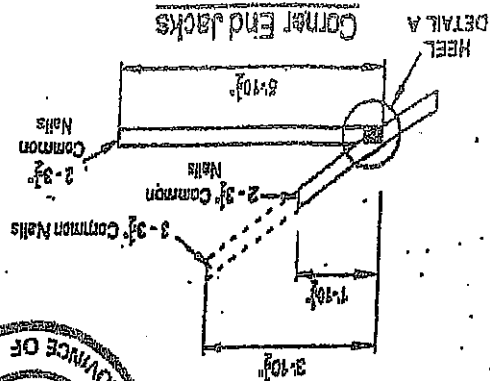
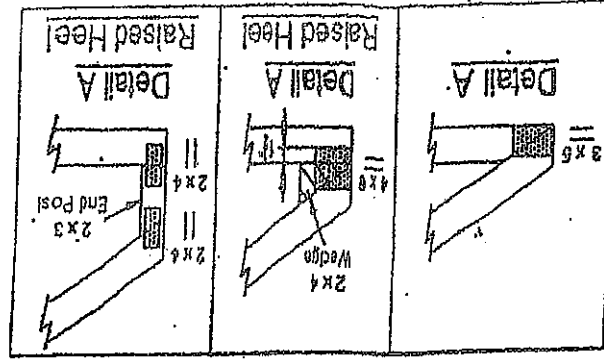
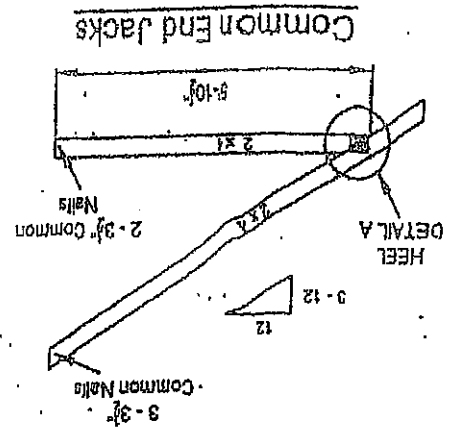
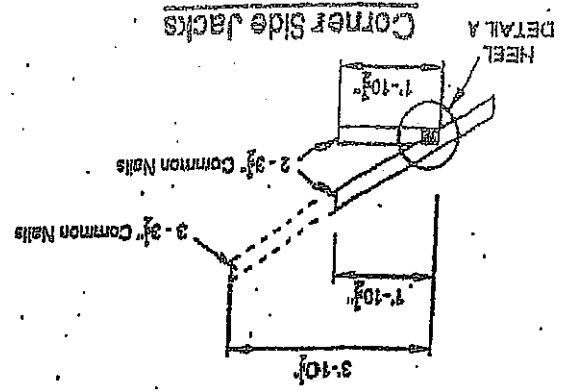
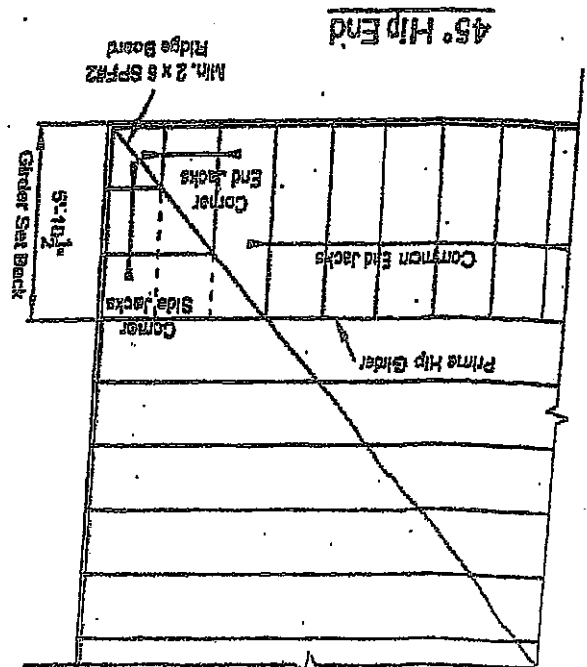
OWNED, TAM 46938-17
STRUCTURAL
COMPONENT ONLY

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
 BOTTOM CHORD : 2x4 SPF#2
 WEBS : 2x3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOADS:
 TOP CHORD LIVE LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 44.8 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATE DESIGN)
 (TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (518) 287-2242

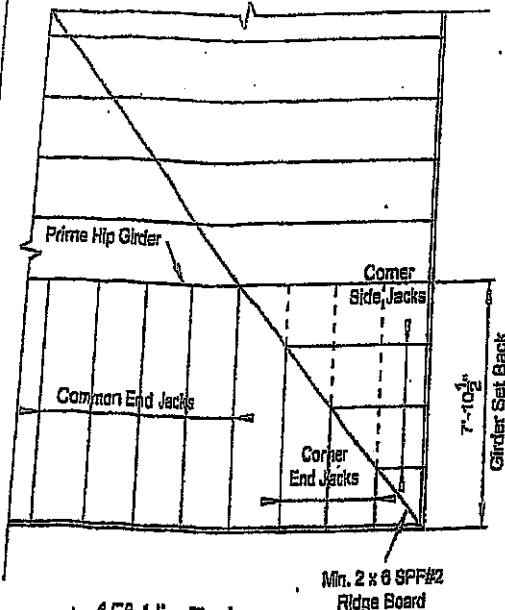
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

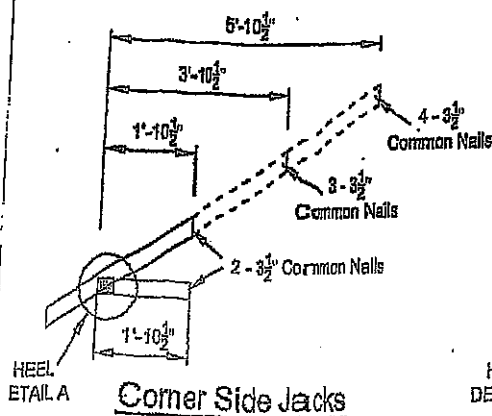
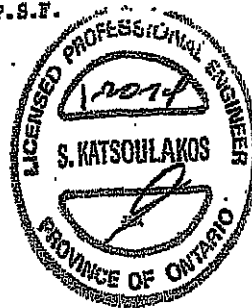
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 44.8 P.S.F.

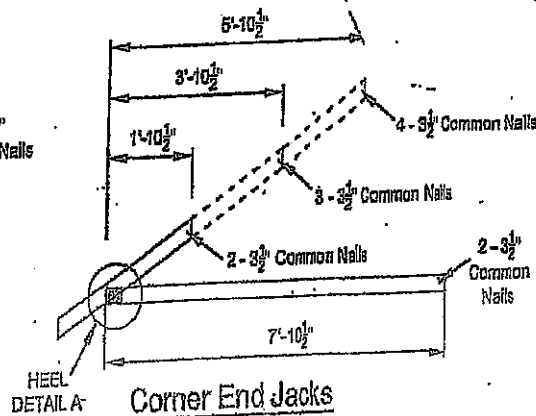


45° Hip End

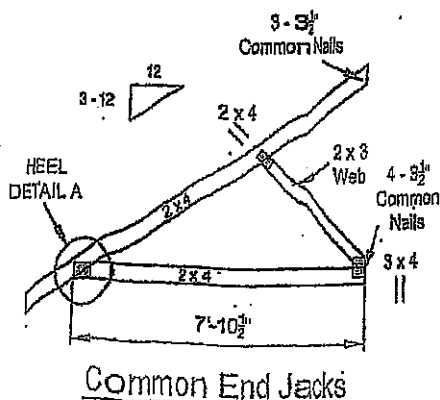
DWG NO TAN 3503.14
STRUCTURAL
COMPONENT ONLY



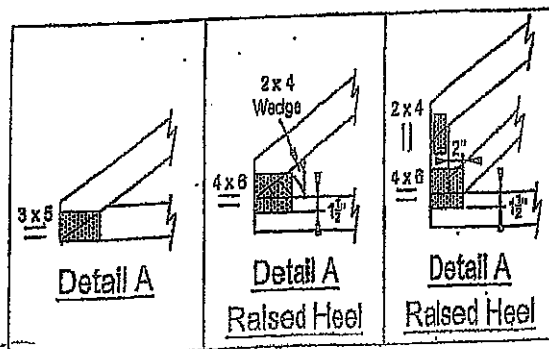
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A
Raised Heel

Detail A
Raised Heel

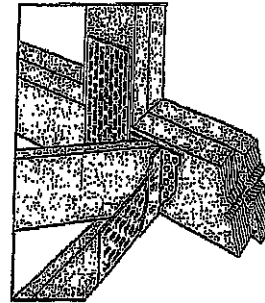
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

FACE MOUNT HANGERS

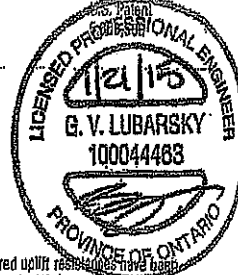
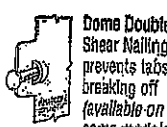
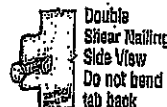


These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
 These products are approved for installation with the Strong-Drive® SD Connector series. See page 24 for more information.

Model No.	Ga	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g	Header	Joist	U.P.F.		S.P.F.	
								Uplift	Normal	Uplift	Normal
								(R _u =1.55) (R _p =1.00) (R _u =1.55) (R _p =1.00)	(R _u =1.55) (R _p =1.00) (R _u =1.55) (R _p =1.00)	(R _u =1.55) (R _p =1.00) (R _u =1.55) (R _p =1.00)	(R _u =1.55) (R _p =1.00) (R _u =1.55) (R _p =1.00)
								lbs	lbs	lbs	lbs
								kN	kN	kN	kN
DOUBLE 2x SIZES											
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	4-16d	2-16d	835	2020	580	1435
								3.71	8.99	2.62	6.38
LUS26-2	18	3 1/8	4 1/8	2	4	4-16d	4-16d	1720	2595	1545	1920
								765	1154	687	854
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	14-16d	6-16d	2850	7335	2085	5205
								1265	3263	920	2315
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	20-16d	8-16d	4385	8950	3110	6355
								1951	3981	1383	2827
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2675
								765	1478	687	1145
HHUS28-2	14	3 1/8	7 1/8	3	5 1/8	22-16d	8-16d	3765	8940	2675	6345
								1675	3977	1190	2822
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
								2700	5774	1917	4099
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4600	2320	3195
								1148	2032	1032	1421
HHUS210-2	14	3 1/8	9 1/8	3	8	30-16d	10-16d	4745	9660	4310	7000
								2121	4207	1917	3124
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
								3043	6514	2160	4626
TRIPLE 2x SIZES											
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	20-16d	8-16d	4385	8950	3110	6355
								1951	3981	1383	2827
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
								2700	5774	1917	4099
HHUS210-3	14	4 1/8	9	3	7 1/8	30-16d	10-16d	4745	10545	4310	7485
								2121	4631	1917	3329
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
								3043	6514	2160	4626
QUADRUPLE 2x SIZES											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	20-16d	8-16d	4385	8950	3110	6355
								1951	3981	1383	2827
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
								2700	5774	1917	4099
HHUS210-4	14	6 1/8	9 1/8	3	7 1/8	30-16d	10-16d	4745	10545	4310	7485
								2121	4631	1917	3329
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
								3043	6514	2160	4626
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	58-16d	20-16d	7640	14995	5425	10845
								3388	6670	2413	4735
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	66-16d	22-16d	10130	16400	7185	11645
								4506	7295	3200	5180
4x SIZES											
LUS46	18	3 1/8	4 1/8	2	3 1/8	4-16d	4-16d	1720	2595	1545	1920
								765	1154	687	854
HHUS46	14	3 1/8	5 1/8	3	3 1/8	14-16d	6-16d	2850	7335	2085	5205
								1190	3268	920	2315
HGUS46	12	3 1/8	5 1/8	4	4 1/8	20-16d	8-16d	4385	8950	3110	6355
								1951	3981	1383	2827
LUS48	18	3 1/8	6 1/8	2	3 1/8	6-16d	4-16d	1720	3325	1545	2675
								765	1478	687	1145
HHUS48	14	3 1/8	7 1/8	3	6 1/8	22-16d	8-16d	3765	8940	2675	6345
								1675	3977	1190	2822
HGUS48	12	3 1/8	7 1/8	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
								2700	5774	1917	4099
LUS410	18	3 1/8	8 1/8	2	6 1/8	8-16d	6-16d	2580	4600	2320	3195
								1148	2032	1032	1421
HGUS410	12	3 1/8	9	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
								3043	6514	2160	4626
HGUS412	12	3 1/8	10 1/8	4	10 1/8	58-16d	20-16d	7640	14995	5425	10845
								3388	6670	2413	4735
HGUS414	12	3 1/8	12 1/8	4	11 1/8	66-16d	22-16d	10130	16400	7185	11645
								4506	7295	3200	5180



Typical HUS26 Installation
 (Truss Designer to provide fastener quantity for connecting multiple members together)



1. Factored uplift resistance has been increased 15% for wind or seismic loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply glider truss. For fastening to single-ply truss request technical bulletin T-410/FORTRUSS and/or see installation notes.
5. NAILS: 15d = 0.162" dia. x 3 1/2" long. See pages 22-23 for other nail sizes and information.

Plated Truss Connectors



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS Standard & Double Shear Joist Hangers

SIMPSON
Strong-Tie



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed cost,
or a combination of these features.

Most hangers in this series have double shear nailing - an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double shear hangers range from the light capacity LUS hangers to the highest capacity HHUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

MATERIAL: See tables below and on page 155

FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

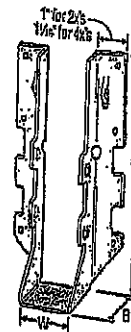
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With 3x carrying members, use 16d x 2 1/2" nails into the header and 16d commons into the joist with no reduction in resistance. With 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value.

OPTIONS: • LUS and LUL hangers cannot be modified.

- HUS hangers available with the header flanges turned in for 2-2x (3/8") and 4x only, with no load reduction. See HUSC Concealed Flange Illustration.
- Concealed flanges are not available for HGUS.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See hanger options on page 230.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

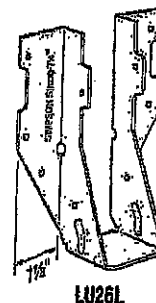
These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.



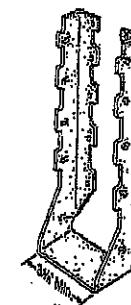
LUS28



HUS210
(HUS26, HUS28,
and HHUS similar)



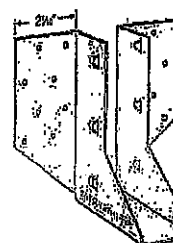
LU26L



HUSC
Concealed Flanges
(not available for HHUS,
HGUS and HUS2x)



HUS412



LJS26DS

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance			
		W	H	B	D ^a	Header	Joist	D-Fit-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _u = 1.18) lbs	(K _n = 1.00) lbs	(K _u = 1.18) lbs	(K _n = 1.00) lbs
SINGLE 2x SIZES											
LUS24	18	1 1/4	3 1/4	1 3/4	2 1/4	4-10d	2-10d	710	1825	645	1155
LU24L	22	1 1/4	3	1 1/4	2 1/4	4-10d	2-10dx1 1/2	316	723	287	534
LU26L	22	1 1/4	5	1 1/4	4	6-10d	4-10dx1 1/2	360	1020	320	725
LUS26	18	1 1/4	4 1/4	1 3/4	3 3/4	4-10d	4-10d	1,600	4,540	1,420	3,220
HUS26	16	1 1/4	5 1/4	2	3 3/4	14-16d	6-16d	720	1605	645	1140
LJS26DS	18	1 1/4	5	1 3/4	4	16-16d	6-16d	8,200	7,340	2,870	5,070
HGUS26	12	1 1/4	5 1/4	2	4 1/4	20-16d	8-16d	1,420	2,170	1,250	1,630
LU26L	20	1 1/4	6 1/4	1 3/4	5 1/4	8-10d	6-10dx1 1/2	6,320	9,650	5,740	7,250
LUS28	18	1 1/4	6 1/4	1 3/4	3 3/4	6-10d	4-10d	2,705	4,940	2,065	3,875
HUS28	16	1 1/4	7 1/4	3	6 1/4	22-16d	8-16d	11,300	20,970	9,280	17,340
HGUS28	12	1 1/4	7 1/4	3	6 1/4	36-16d	12-16d	2,055	4,265	1,460	4,115
LU210L	20	1 1/4	8	1 3/4	7 1/4	10-10d	6-10dx1 1/2	9,140	18,070	6,490	18,310
LUS210	18	1 1/4	7 1/4	1 3/4	3 3/4	8-10d	4-10d	2,665	6,625	2,665	5,700
								11,860	29,510	11,930	26,350
								1,140	2,185	1,020	1,550
								5,070	9,720	4,540	6,890
								1,420	2,520	1,250	1,790
								6,320	11,210	6,740	7,860
								3,605	5,965	2,675	4,945
								16,040	23,860	11,900	19,330
								3,310	7,675	3,310	6,900
								14,740	34,190	14,740	30,730
								1,140	2,495	1,020	1,770
								5,070	11,100	4,540	7,870
								1,420	2,785	1,250	2,210
								6,320	12,390	6,740	9,830

See footnotes on page 155.

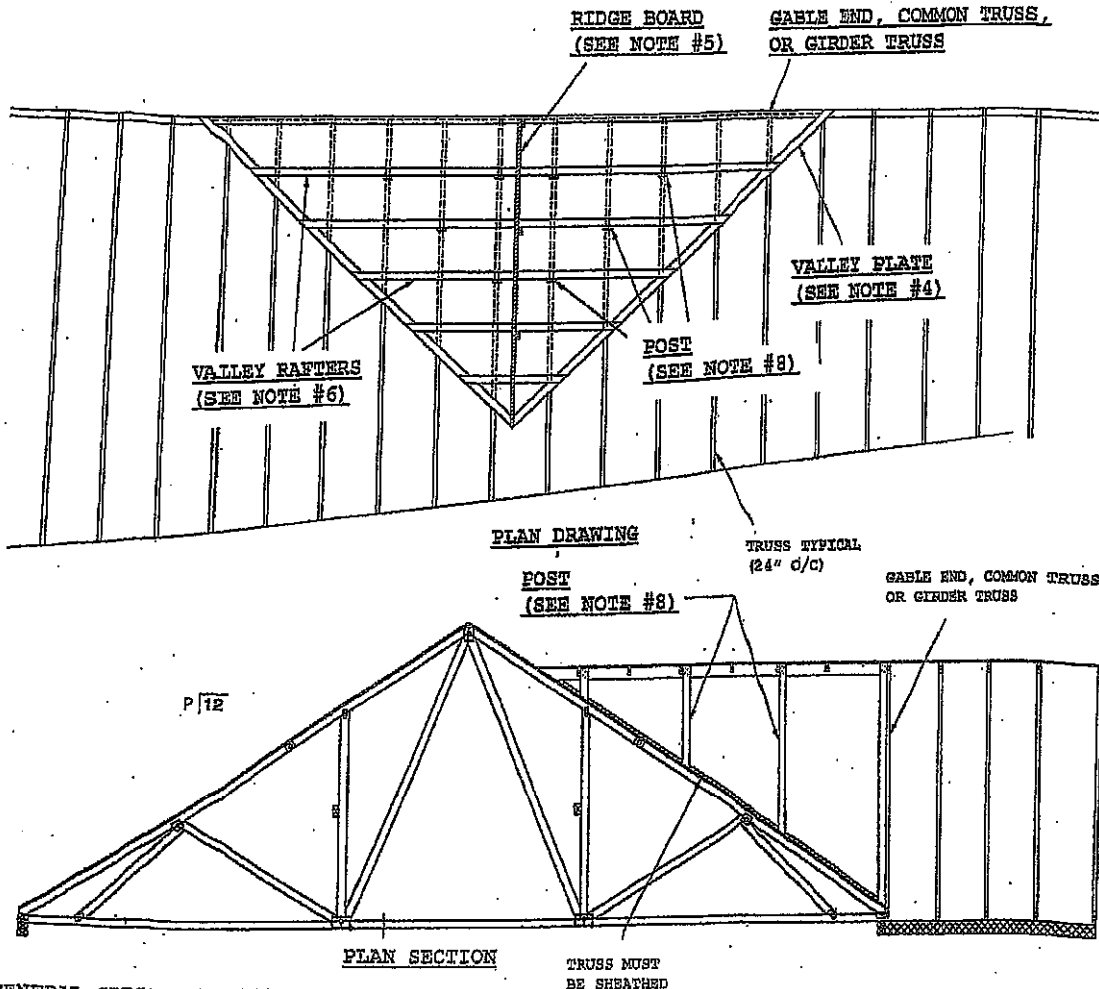
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MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON FLAT DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75' IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

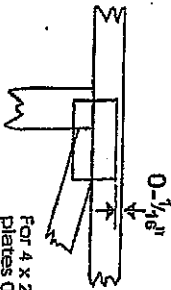
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAW 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



For 4 x 2 orientation, locate plates 0- $\frac{1}{4}$ \"/>

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

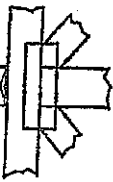
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

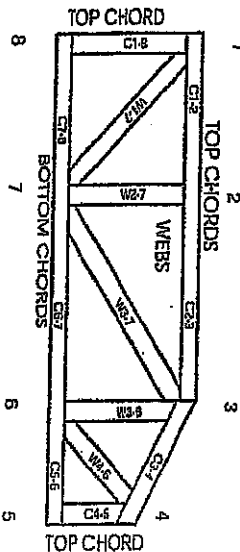


Indicates location where bearings (supports) occur. Icons vary but number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED A LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORMANCE™

Mitek Engineering Reference Sheet MH-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with the retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

TECHNICAL BULLETIN

H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

MATERIAL: 18 gauge **FINISH:** G90 galvanized

DESIGN: • Factored resistances are in accordance with CSA 086-14

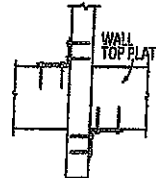
- Factored resistances have been increased 15%. No further increase is permitted.

INSTALLATION: • Use all specified fasteners

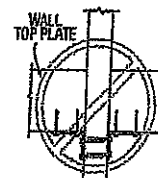
- Nails: 8d = 0.131" dia. x 2 1/2" long common wire, 8d x 1 1/2" = 0.131" x 1 1/2" long, 10d x 1 1/2" = 0.146" x 1 1/2" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0 .

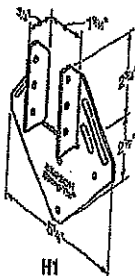
Hurricane Tie Installations to achieve Twice the Load (Top View)



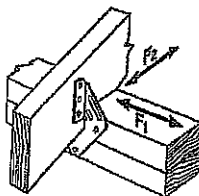
Install diagonally across from each other for minimum 2x truss.



Nailing into both sides of a single ply 2x truss may cause the wood to split.



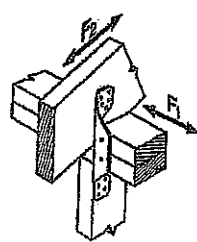
H1



H1 Installation



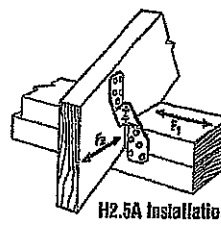
H2A



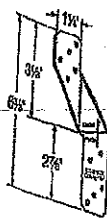
H2A Installation



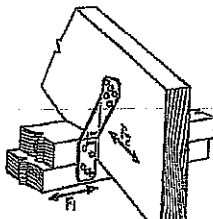
H2.5A



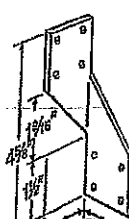
H2.5A Installation



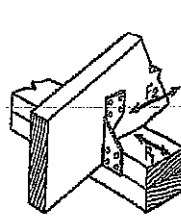
H2.5T



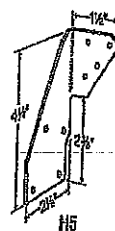
H2.5T Installation
(Nails into both top plates)



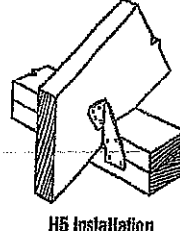
H3



H3 Installation



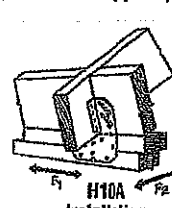
H5



H5 Installation
(Nails into both top plates)



H10A



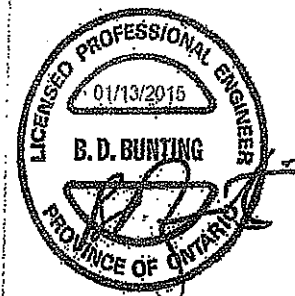
H10A Installation

Model No.	Ga	Fasteners		Factored Resistance (lbs)						
				D-Fir-L			S-P-F			
		To Waller	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
						(K ₀ = 1.15)			(K ₀ = 1.15)	
H1	18	6-8d x 1 1/2"	4-8d	-	740	685	380	680	485	215
H2A	18	5-8d x 1 1/2"	2-8d x 1 1/2"	5-8d x 1 1/2"	880	220	75	590	155	55
H2.5A	18	5-8d	5-8d	-	805	160	160	755	160	160
H2.5T	18	5-8d	5-8d	-	835	175	240	740	180	210
H3	18	4-8d	4-8d	-	740	180	265	615	125	190
H5	18	4-8d	4-8d	-	685	180	305	500	130	215
H10A	18	9-10d x 1 1/2"	9-10d x 1 1/2"	-	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot

be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



UNIT
STATES
DESIGN

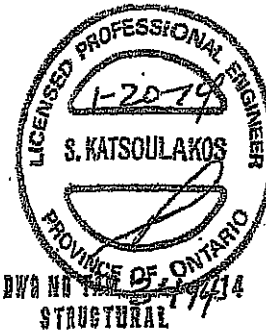
This technical bulletin is effective until December 31, 2014, and remains in effect until January 1, 2015. This information is updated periodically and should be re-evaluated each year after December 31, 2014. Contact Simpson Strong-Tie for current information and limited warranty or go to www.strongtie.com.
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800-999-5099
www.strongtie.com

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473C rev 10-08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.