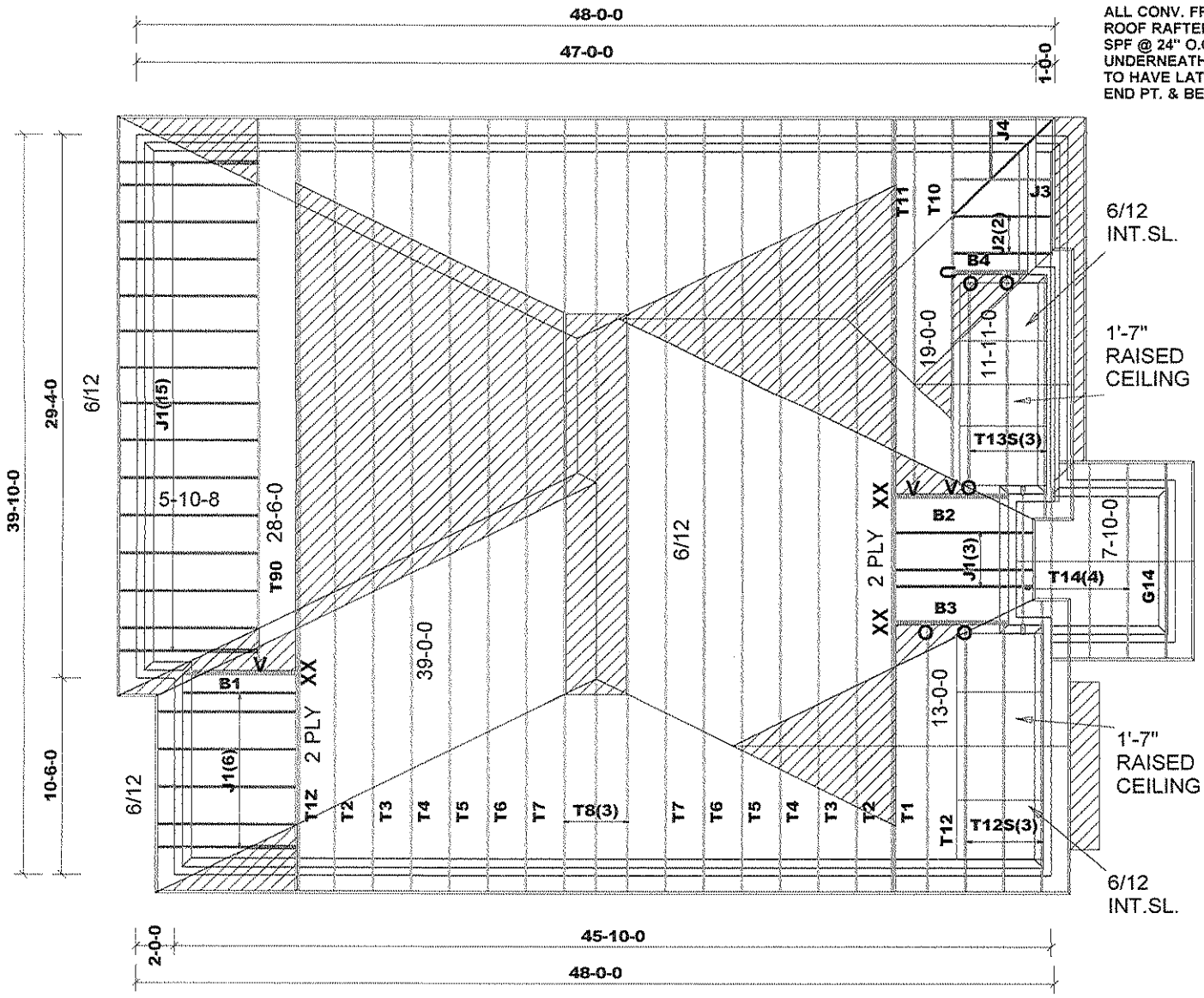
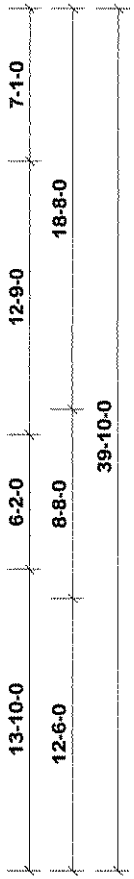
NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'





ALL SLOPES ARE 12/12 UNLESS NOTED

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS

GRD SNOW LD	54.34 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
HGUS26-2 - (XX)
LUS24 - (O)
LUS26-2 - (U)

CONV.FRAMING BY OTHERS

B1,B2,B3 DENOTES
2-2X10 SPF#2
(FLUSH)

B4 DENOTES
2-2X8 SPF#2
(FLUSH)

T-170681

Town of Innisfil Certified Model
02/12/2017 9:42:43 AM kgervais

MIL.032

TAMARACK LUMBER INC.

Job Track:	42067
Layout ID:	288236
Plan Log:	94324

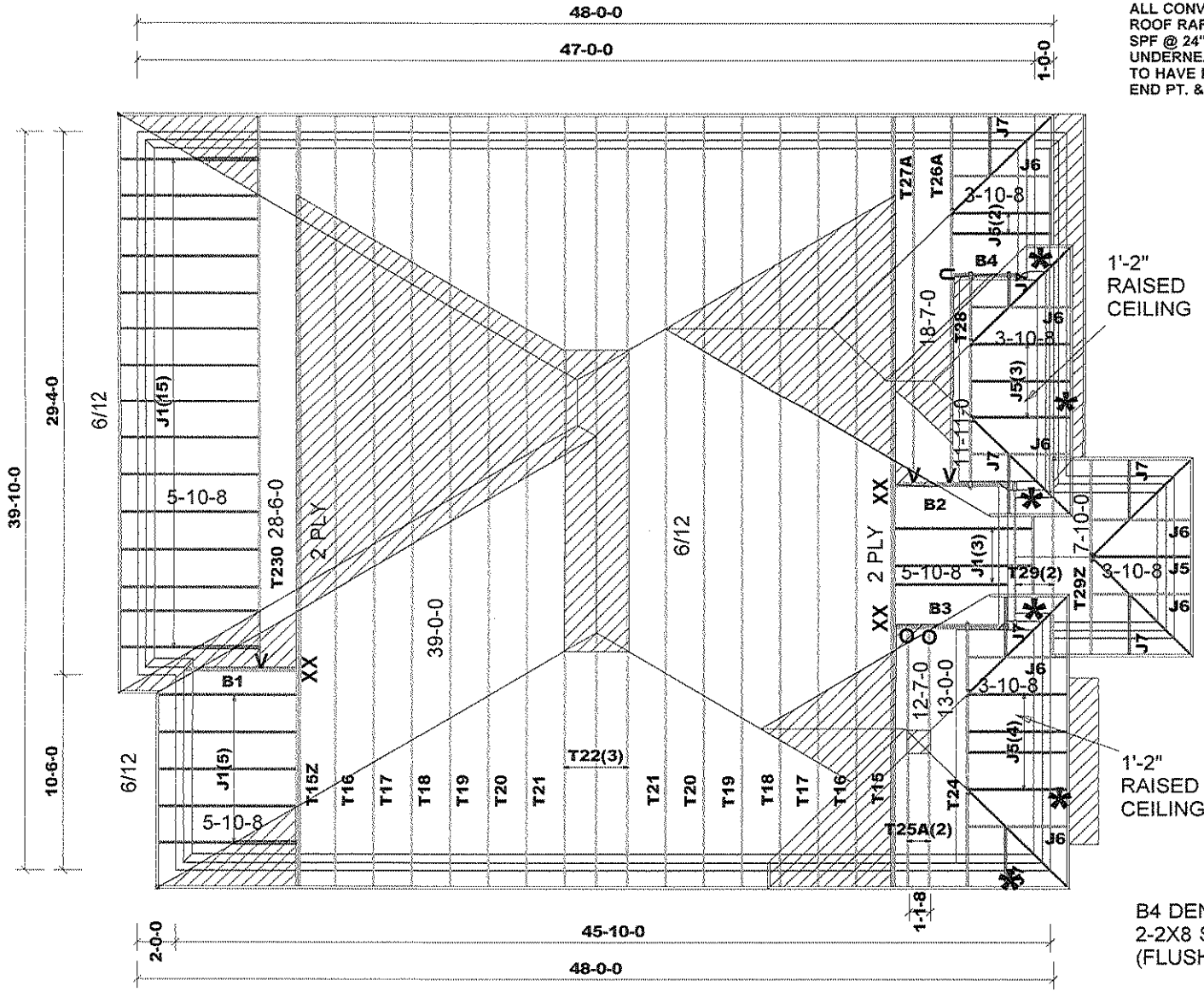
Builder / Location: BAYVIEW WELLINGTON / INNISFIL
Project: ALCONA SHORES
Date: 9/18/2017 Designer: ella

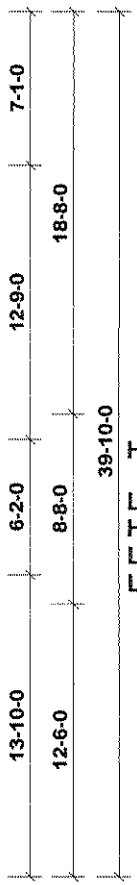
Model / Elevation: S48-2 GOLDEN EYE 2 A

Mitek ver 7.5.0

NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'





ALL SLOPES ARE 10/12 UNLESS NOTED

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
HGUS26-2 - (XX)
LUS24 - (O)
LUS26-2 - (U)

CONV.FRAMING BY OTHERS

DENOTES:1'-2" RAISED PLATE *

B4 DENOTES 2-2X8 SPF#2 (FLUSH)+KNEE WALL

B1,B2,B3 DENOTES 2-2X10 SPF#2 (FLUSH)+KNEE WALL IF RQ

T-170681

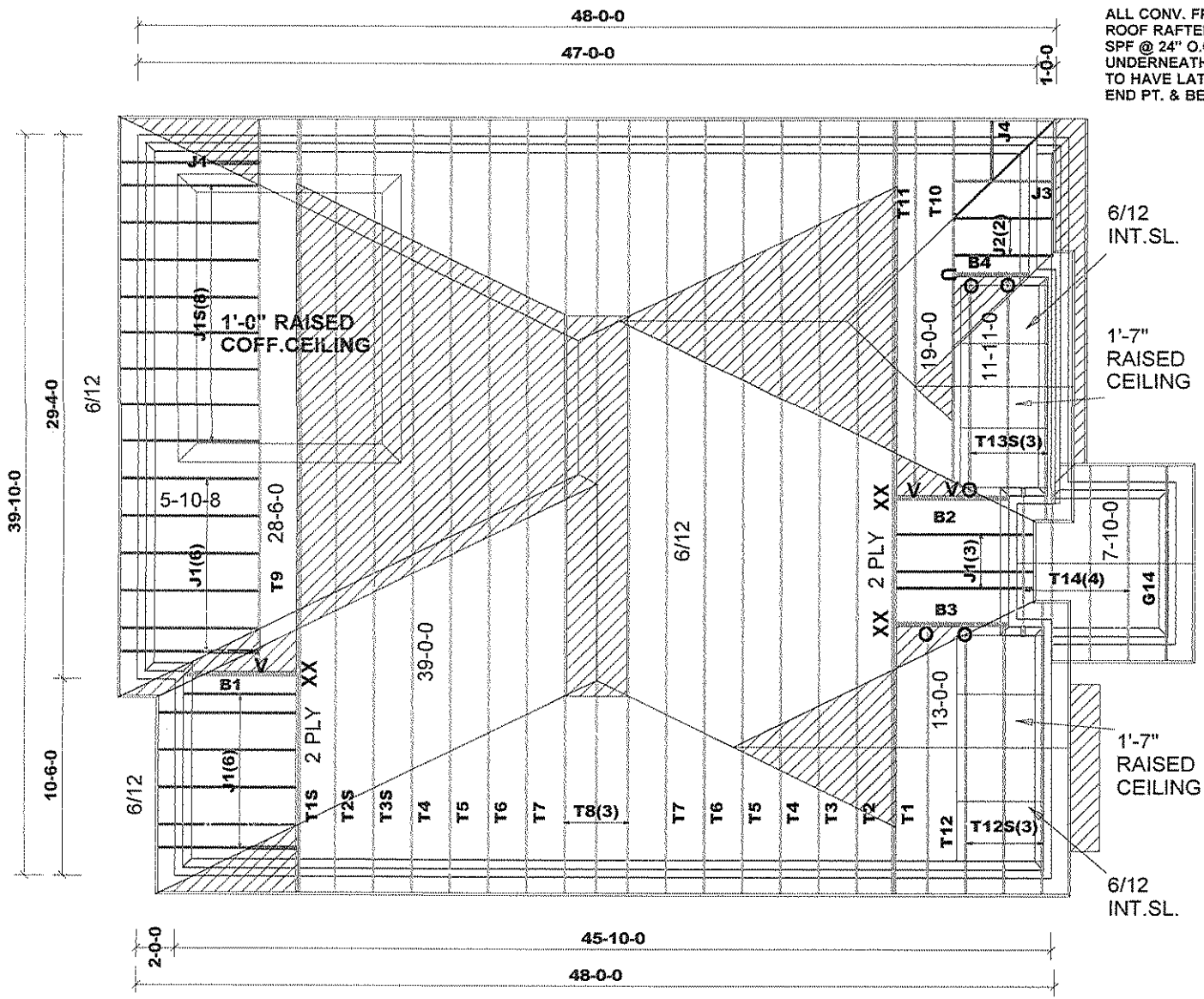
mil,032

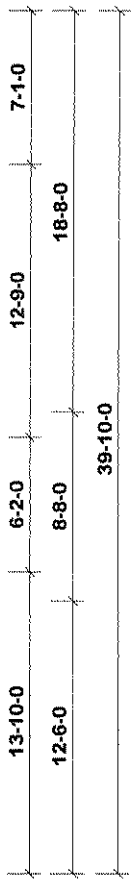
TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S48-2 GOLDEN EYE 2 B
	Layout ID: 288238	Project: ALCONA SHORES	
	Plan Log: 94324	Date: 9/18/2017 Designer: ella	

Mitek ver 7.5.0

NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'





ALL SLOPES ARE 12/12 UNLESS NOTED

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
HGUS26-2 - (XX)
LUS24 - (O)
LUS26-2 - (U)

CONV.FRAMING BY OTHERS

B1,B2,B3 DENOTES
2-2X10 SPF#2
(FLUSH)

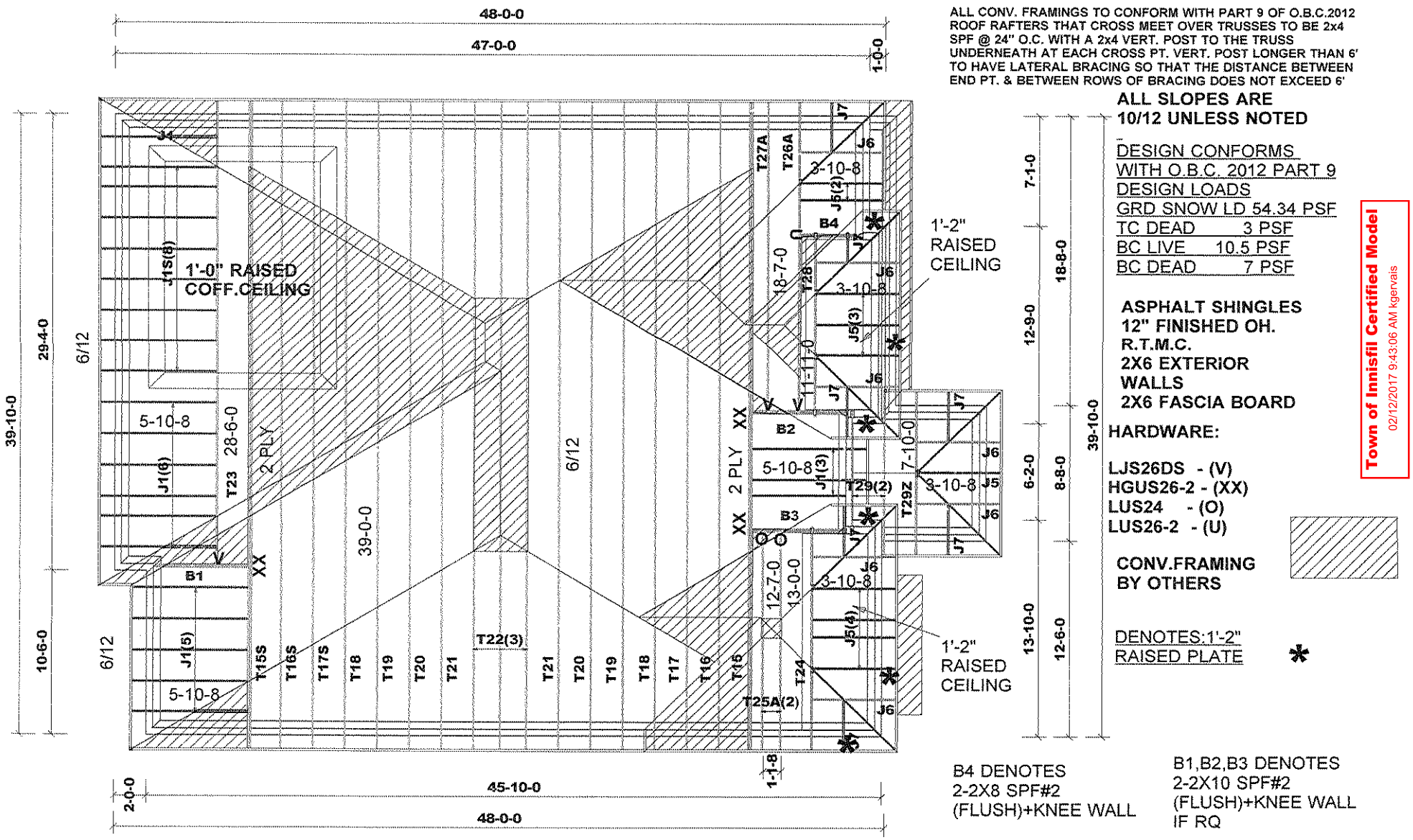
B4 DENOTES
2-2X8 SPF#2
(FLUSH)

T-170681

mil.032

TAMARACK	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL
	Layout ID: 287900	Model / Elevation: S48-2 GOLDEN EYE 2 A + OPT. COFF.
	Plan Log: 94324	Project: ALCONA SHORES
		Date: 9/12/2017 Designer: ella

Mitek ver 7.5.0



NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALL SLOPES ARE 10/12 UNLESS NOTED

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
HGUS26-2 - (XX)
LUS24 - (O)
LUS26-2 - (U)

CONV.FRAMING BY OTHERS

DENOTES:1'-2" RAISED PLATE *

B4 DENOTES 2-2X8 SPF#2 (FLUSH)+KNEE WALL

B1,B2,B3 DENOTES 2-2X10 SPF#2 (FLUSH)+KNEE WALL IF RQ

T-170681

mil.030

TAMARACK LUMBER INC	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S48-2 GOLDEN EYE 2 B + OPT. COFF.
	Layout ID: 287902	Project: ALCONA SHORES	
	Plan Log: 94324	Date: 9/12/2017 Designer: ella	

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

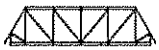
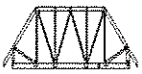
Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288236 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S48-2 GOLDENEYE 2 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	12.00 0.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	483.70 301.34		
	1 2 Ply	T1Z HIP GIRDER	12.00 0.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	483.70 301.34		
	2	T2 HIP	12.00 0.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	409.88 252.66		
	2	T3 HIP	12.00 0.00	39-00-00	06-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	410.82 256.00		
	2	T4 HIP	12.00 0.00	39-00-00	07-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	446.98 273.00		
	2	T5 HIP	12.00 0.00	39-00-00	08-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	463.60 284.00		
	2	T6 HIP	12.00 0.00	39-00-00	09-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	507.24 311.66		
	2	T7 HIP	12.00 0.00	39-00-00	10-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	475.78 287.34		
	3	T8 HIP	12.00 0.00	39-00-00	11-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	786.60 480.99		
	1	T90 HIP GIRDER	12.00 0.00	28-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	170.71 107.00		
	1	T10 HIP GIRDER	12.00 0.00	19-00-00	05-09-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	110.17 69.67		
	1	T11 HIP	12.00 0.00	19-00-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	97.99 63.00		
	1	T12 COMMON	12.00 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	67.86 43.33		
	3	T12S ROOF	12.00 6.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	207.63 132.99		
	3	T13S ROOF	12.00 6.00	11-11-00	07-10-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	195.24 123.99		
	4	T14 COMMON	12.00 0.00	07-10-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	160.36 102.68		
	1	G14 COMMON	12.00 0.00	07-10-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	42.51 28.67		
	24	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	402.96 256.08		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288236 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S48-2 GOLDENEYE 2 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	J2 JACK-OPEN	12.00 / 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 05-09-00	29.50 / 18.66		
	1	J3 JACK-OPEN	12.00 / 0.00	03-10-08	03-07-15	2 X 4	2 X 4	01-03-08 / -02-01-01	01-10-08 / 00-03-08	11.39 / 7.33		
	1	J4 JACK-OPEN	12.00 / 0.00	01-10-08	03-07-15	2 X 4	2 X 4	01-03-08 / -00-01-01	01-10-08 / 00-03-08	9.10 / 6.00		

TOTAL # TRUSS= **62.00** TOTAL BFT OF ALL TRUSSES= **3707.73** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5973.72** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
1	Hangers	LUS26-2	
3	Hangers	LJS26DS	
5	Hangers	LUS24	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

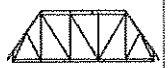
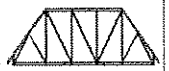
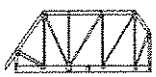
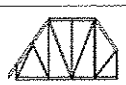
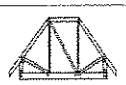
Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288238 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S48-2 GOLDENEYE2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T15 HIP GIRDER	10.00 0.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	496.58 306.00		
	1 2 Ply	T15Z HIP GIRDER	10.00 0.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	496.58 306.00		
	2	T16 HIP	10.00 0.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	393.88 242.34		
	2	T17 HIP	10.00 0.00	39-00-00	06-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	427.16 260.00		
	2	T18 HIP	10.00 0.00	39-00-00	07-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	443.08 269.00		
	2	T19 HIP	10.00 0.00	39-00-00	08-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	440.98 270.66		
	2	T20 HIP	10.00 0.00	39-00-00	09-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	452.34 277.34		
	2	T21 HIP	10.00 0.00	39-00-00	10-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	452.48 282.00		
	3	T22 HIP	10.00 0.00	39-00-00	11-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	744.90 453.99		
	1	T230 HIP GIRDER	10.00 0.00	28-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	168.63 102.00		
	1	T24 HIP GIRDER	10.00 0.00	13-00-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	59.76 38.50		
	2	T25A HIP	10.00 0.00	12-07-00	07-08-07	2 X 4	2 X 4	01-03-08 00-00-00	02-09-11 03-01-14	151.52 100.00		
	1	T26A HIP GIRDER	10.00 0.00	18-07-00	04-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 03-01-14	96.91 61.33		
	1	T27A HIP	10.00 0.00	18-07-00	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-14	90.00 57.17		
	1	T28 HIP GIRDER	10.00 0.00	11-11-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	63.37 40.67		
	2	T29 COMMON	10.00 0.00	07-10-00	04-10-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	74.46 50.34		
	1	T29Z COMMON	10.00 0.00	07-10-00	04-10-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	37.23 25.17		
	23	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	386.17 245.41		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288238 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOP SUB-BUILDER:
MODEL: S48-2 GOLDENEYE2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	10	J5 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	137.50 86.70		
	7	J6 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	74.55 51.31		
	7	J7 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	58.59 42.00		

TOTAL # TRUSS= **76.00** TOTAL BFT OF ALL TRUSSES= **3567.93** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5746.67** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
1	Hangers	LUS26-2	
3	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **9.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

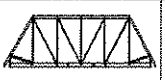
Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 287900 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S48-2 GOLDEN EYE 2 ELEVATION: A + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	12.00 0.00	39-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	433.00 274.66		
	1 2 Ply	T1S HIP	12.00 12.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	453.44 285.00		
	1	T2 HIP	12.00 0.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	204.94 126.33		
	1	T2S HIP	12.00 12.00	39-00-00	05-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	227.50 139.17		
	1	T3 HIP	12.00 0.00	39-00-00	06-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	205.41 128.00		
	1	T3S HIP	12.00 12.00	39-00-00	06-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	230.48 144.33		
	2	T4 HIP	12.00 0.00	39-00-00	07-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	446.98 273.00		
	2	T5 HIP	12.00 0.00	39-00-00	08-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	463.60 284.00		
	2	T6 HIP	12.00 0.00	39-00-00	09-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	507.24 311.66		
	2	T7 HIP	12.00 0.00	39-00-00	10-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	475.78 287.34		
	3	T8 HIP	12.00 0.00	39-00-00	11-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	786.60 480.99		
	1	T9 HIP GIRDER	12.00 12.00	28-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	176.59 116.33		
	1	T10 HIP GIRDER	12.00 0.00	19-00-00	05-09-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	110.17 69.67		
	1	T11 HIP	12.00 0.00	19-00-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	97.99 63.00		
	1	T12 COMMON	12.00 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	67.86 43.33		
	3	T12S ROOF	12.00 6.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	207.63 132.99		
	3	T13S ROOF	12.00 6.00	11-11-00	07-10-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	195.24 123.99		
	4	T14 COMMON	12.00 0.00	07-10-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	160.36 102.68		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 287900 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S48-2 GOLDEN EYE 2 ELEVATION: A + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G14 COMMON	12.00 0.00	07-10-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	39.08 25.67		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	8	J1S JACK-OPEN	6.00 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	181.36 132.00		
	2	J2 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	29.50 18.66		
	1	J3 JACK-OPEN	12.00 0.00	03-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-10-08 00-03-08	11.39 7.33		
	1	J4 JACK-OPEN	12.00 0.00	01-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-10-08 00-03-08	9.10 6.00		

TOTAL # TRUSS= **62.00** TOTAL BFT OF ALL TRUSSES= **3746.85** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5989.88** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LJS26DS	
5	Hangers	LUS24	
1	Hangers	LUS26-2	
3	Hangers	HGUS26-2	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

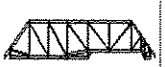
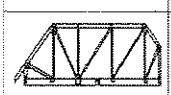
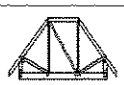
Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 287902 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S48-2 GOLDENEYE 2 ELEVATION: B + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T15 HIP GIRDER	10.00 0.00	39-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	416.80 256.00		
	1 2 Ply	T15S HIP	10.00 12.00	39-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	436.88 272.66		
	1	T16 HIP	10.00 0.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	196.94 121.17		
	1	T16S HIP	10.00 12.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	218.46 136.83		
	1	T17 HIP	10.00 0.00	39-00-00	06-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	213.58 130.00		
	1	T17S HIP	10.00 12.00	39-00-00	06-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	229.42 144.33		
	2	T18 HIP	10.00 0.00	39-00-00	07-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	443.08 269.00		
	2	T19 HIP	10.00 0.00	39-00-00	08-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	440.98 270.66		
	2	T20 HIP	10.00 0.00	39-00-00	09-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	452.34 277.34		
	2	T21 HIP	10.00 0.00	39-00-00	10-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	452.48 282.00		
	3	T22 HIP	10.00 0.00	39-00-00	11-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	744.90 453.99		
	1	T23 HIP GIRDER	10.00 12.00	28-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	173.75 110.33		
	1	T24 HIP GIRDER	10.00 0.00	13-00-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	59.76 38.50		
	2	T25A HIP	10.00 0.00	12-07-00	07-08-07	2 X 4	2 X 4	01-03-08 00-00-00	02-09-11 03-01-14	151.52 100.00		
	1	T26A HIP GIRDER	10.00 0.00	18-07-00	04-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 03-01-14	96.91 61.33		
	1	T27A HIP	10.00 0.00	18-07-00	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-14	90.00 57.17		
	1	T28 HIP GIRDER	10.00 0.00	11-11-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	63.37 40.67		
	2	T29 COMMON	10.00 0.00	07-10-00	04-10-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	74.46 50.34		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 287902 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S48-2 GOLDENEYE 2 ELEVATION: B + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T29Z COMMON	10.00 / 0.00	07-10-00	04-10-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	37.23 / 25.17		
	15	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	251.85 / 160.05		
	8	J1S JACK-OPEN	6.00 / 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	181.36 / 132.00		
	10	J5 JACK-OPEN	10.00 / 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-10-07	137.50 / 86.70		
	7	J6 JACK-OPEN	10.00 / 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -02-01-01	01-07-11 / 00-03-08	74.55 / 51.31		
	7	J7 JACK-OPEN	10.00 / 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -00-01-01	01-07-11 / 00-03-08	58.59 / 42.00		

TOTAL # TRUSS= **76.00** TOTAL BFT OF ALL TRUSSES= **3569.55** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5696.71** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS26-2	
3	Hangers	HGUS26-2	

TOTAL # ITEMS= **9.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288236	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 18 15:08:13 2017 Page 1

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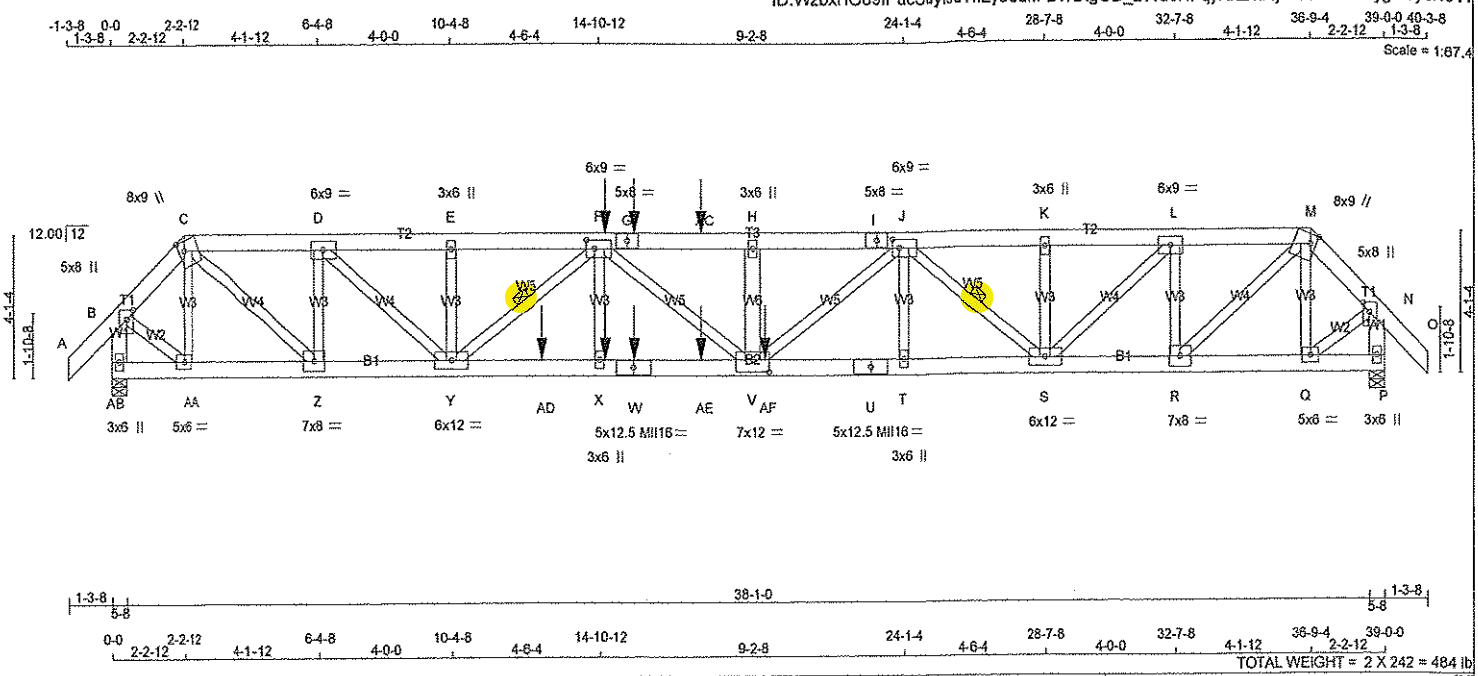

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - M	2x6	DRY	No.2	SPF
M - O	2x6	DRY	No.2	SPF
AB- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AB- W	2x6	DRY	2100F 1.8E	SPF
W - U	2x6	DRY	2100F 1.8E	SPF
U - P	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - H	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- G	2	12	SIDE(183.1)
G- I	2	12	SIDE(0.1)
I- M	2	12	TOP
M- O	2	12	TOP
AB- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AB- W	2	12	SIDE(197.8)
W- U	2	12	SIDE(197.8)
U- P	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
H- V	2	5	SIDE(629.3)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	3.25	2.25
C	TTWW+m	MT20	8.0	9.0	3.25	2.00
D	TMWW-t	MT20	6.0	9.0		
E, H, K						
E	TMW+w	MT20	3.0	6.0		
F	TMWWW-t	MT20	6.0	9.0	3.00	3.00
G	TS-t	MT20	5.0	8.0		
I	TS-t	MT20	5.0	8.0		
J	TMWWW-t	MT20	6.0	9.0	3.00	2.50
L	TMWW-t	MT20	6.0	9.0		
M	TTWW+m	MT20	8.0	9.0	3.25	2.00
N	TMVW+p	MT20	5.0	8.0	3.25	2.25
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AB	6080	0	6080	0	0	5-8	5-8
P	5509	0	5509	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	4673	3184 / 0	743 / 0	0 / 0	0 / 0	745 / 0	0 / 0
P	4239	2880 / 0	680 / 0	0 / 0	0 / 0	679 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-Y, J-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.05 (1)		10.00
B- C	-5285 / 0	-122.2	-122.2	0.06 (1)		5.06
C- D	-9446 / 0	-122.2	-122.2	0.25 (1)		3.84
D- E	-14656 / 0	-122.2	-122.2	0.47 (1)		2.96
E- F	-14656 / 0	-122.2	-122.2	0.49 (1)		2.95
F- G	-20338 / 0	-122.2	-122.2	0.95 (1)		2.03
G-AC	-20338 / 0	-122.2	-122.2	0.95 (1)		2.03
AC- H	-20338 / 0	-122.2	-122.2	0.95 (1)		2.03
H- I	-20338 / 0	-122.2	-122.2	0.89 (1)		2.14
I- J	-20338 / 0	-122.2	-122.2	0.89 (1)		2.14
J- K	-12894 / 0	-122.2	-122.2	0.40 (1)		3.22
K- L	-12894 / 0	-122.2	-122.2	0.37 (1)		3.24
L- M	-8490 / 0	-122.2	-122.2	0.22 (1)		4.05
M- N	-4745 / 0	-122.2	-122.2	0.08 (1)		5.29
N- O	0 / 63	-122.2	-122.2	0.05 (1)		10.00
AB- B	-6148 / 0	0.0	0.0	0.23 (1)		5.97
P- N	-5551 / 0	0.0	0.0	0.21 (1)		6.23
AB-AA	0 / 0	-28.0	-28.0	0.05 (1)		10.00
AA- Z	0 / 3627	-28.0	-28.0	0.14 (1)		10.00
Z- Y	0 / 9446	-28.0	-28.0	0.31 (1)		10.00
Y-AD	0 / 18886	-28.0	-28.0	0.80 (1)		10.00
AD- X	0 / 18886	-28.0	-28.0	0.80 (1)		10.00
X- W	0 / 18886	-28.0	-28.0	0.56 (1)		10.00
W-AE	0 / 18886	-28.0	-28.0	0.56 (1)		10.00
AE- V	0 / 18886	-28.0	-28.0	0.56 (1)		10.00
V-AF	0 / 17276	-28.0	-28.0	0.67 (1)		10.00
AF- U	0 / 17276	-28.0	-28.0	0.67 (1)		10.00
U- T	0 / 17276	-28.0	-28.0	0.67 (1)		10.00
T- S	0 / 17276	-28.0	-28.0	0.50 (1)		10.00
S- R	0 / 8489	-28.0	-28.0	0.27 (1)		10.00
R- Q	0 / 3254	-28.0	-28.0	0.12 (1)		10.00
Q- P	0 / 0	-28.0	-28.0	0.04 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- C	-1752 / 0	0.15 (1)
Y- E	-445 / 0	0.04 (1)
V- H	-601 / 0	0.03 (1)
S- K	-491 / 0	0.04 (1)
Q- M	-1622 / 0	0.14 (1)
B-AA	0 / 4232	0.37 (1)
Q- N	0 / 3799	0.34 (1)
D- Y	0 / 7050	0.62 (1)
C- Z	0 / 7765	0.68 (1)
Z- D	-5190 / 0	0.44 (1)
R- M	0 / 6933	0.61 (1)
S- L	0 / 5959	0.53 (1)
R- L	-4448 / 0	0.38 (1)
F- V	0 / 1852	0.16 (1)
Y- F	-5434 / 0	0.48 (1)
X- F	0 / 1292	0.11 (1)
J- S	-5630 / 0	0.50 (1)
V- J	0 / 3906	0.35 (1)
T- J	0 / 499	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 819 (0.57")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 543 (0.85")

CSI: TC=0.95 (F-H:1) , BC=0.80 (X-Y:1) , WB=0.68 (C-Z:1) , SSI=0.98 (T-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.76 (W) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 90077 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 47520 -17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288236	T1	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	7.0	8.0		
S	BMWWW-t	MT20	6.0	12.0		
T	BMW+w	MT20	3.0	6.0		
U	BS-t	MII16	5.0	12.5		
V	BMWWW-t	MT20	7.0	12.0	4.25	6.00
W	BS-t	MII16	5.0	12.5		
X	BMW+w	MT20	3.0	6.0		
Y	BMWWW-t	MT20	6.0	12.0		
Z	BMWW-t	MT20	7.0	8.0		
AA	BMWW-t	MT20	5.0	6.0		
AB	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 147.1 lbs FACTORED DOWN AT 15-0-12, AND 147.1 lbs FACTORED DOWN AT 15-11-4, AND 147.1 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 1643.1 lbs FACTORED DOWN AT 13-1-8, 69.9 lbs FACTORED DOWN AT 15-0-12, 69.9 lbs FACTORED DOWN AT 15-11-4, AND 69.9 lbs FACTORED DOWN AT 17-11-4, AND 3170.6 lbs FACTORED DOWN AT 19-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	15-0-12	-147	-147	---	FRONT	VERT	TOTAL
G	15-11-4	-147	-147	---	FRONT	VERT	TOTAL
W	15-11-4	-40	-70	---	FRONT	VERT	TOTAL
X	15-0-12	-40	-70	---	FRONT	VERT	TOTAL
AC	17-11-4	-147	-147	---	FRONT	VERT	TOTAL
AD	13-1-8	-1643	-1643	---	FRONT	VERT	TOTAL
AE	17-11-4	-40	-70	---	FRONT	VERT	TOTAL
AF	19-10-8	-3171	-3171	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4 2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

P6 of 6

DWG NO. TAM 47520-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
288236	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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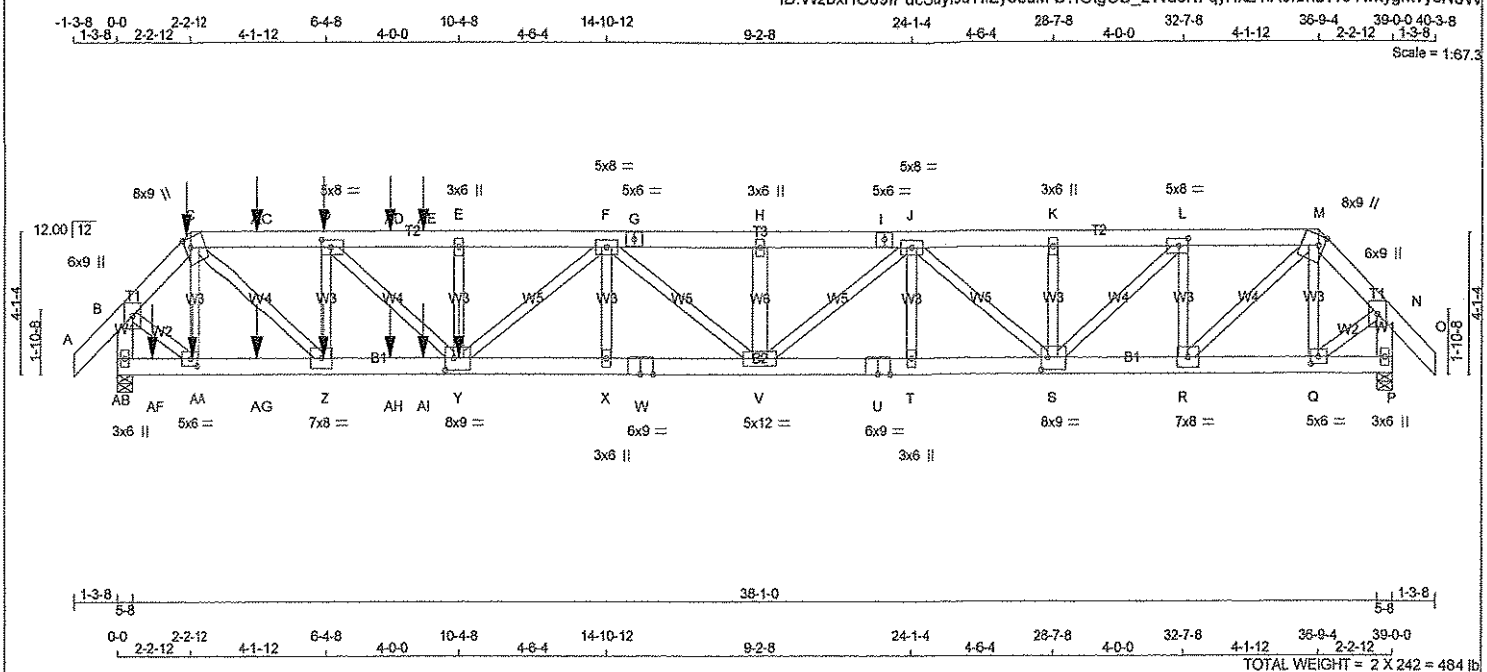

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - M	2x6	DRY	No.2	SPF
M - O	2x6	DRY	No.2	SPF
AB- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AB- W	2x6	DRY	2100F 1.8E	SPF
W - U	2x6	DRY	2100F 1.8E	SPF
U - P	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - H	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(183.1)
G- I	2	12	TOP
I- M	2	12	TOP
M- O	2	12	TOP
AB- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AB- W	2	12	SIDE(197.8)
W- U	2	12	TOP
U- P	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
E- Y	1	3	SIDE(602.9)
K- S	1	3	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	9.0	3.25	2.00
D	TMWW-t	MT20	5.0	8.0	2.50	3.50
E, H, K						
E	TMW+w	MT20	3.0	6.0		
F	TMWWW-t	MT20	5.0	8.0		
G	TS-t	MT20	5.0	6.0		
I	TS-t	MT20	5.0	6.0		
J	TMWWW-t	MT20	5.0	8.0		
L	TMWW-t	MT20	5.0	8.0	2.50	3.50
M	TTWW+m	MT20	8.0	9.0	3.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AB	6375	0	6375	0	0	5-8	5-8
P	4082	0	4082	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	4960	3270 / 0	857 / 0	0 / 0	0 / 0	833 / 0	0 / 0
P	3169	2103 / 0	540 / 0	0 / 0	0 / 0	526 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00	AA- C	-1837 / 0	0.16 (1)
B- C	-5516 / 0	-122.2	-122.2	0.07 (1)	4.97	Y- E	-601 / 0	0.05 (1)
C-AC	-9444 / 0	-122.2	-122.2	0.28 (1)	3.81	V- H	-517 / 0	0.03 (1)
AC- D	-9444 / 0	-122.2	-122.2	0.28 (1)	3.81	S- K	-488 / 0	0.04 (1)
D-AD	-13677 / 0	-122.2	-122.2	0.48 (1)	3.04	Q- M	-1145 / 0	0.10 (1)
AD-AE	-13677 / 0	-122.2	-122.2	0.48 (1)	3.04	B-AA	0 / 4416	0.39 (1)
AE- E	-13677 / 0	-122.2	-122.2	0.48 (1)	3.04	Q- N	0 / 2751	0.24 (1)
E- F	-13677 / 0	-122.2	-122.2	0.45 (1)	3.09	D- Y	0 / 5727	0.51 (1)
F- G	-13015 / 0	-122.2	-122.2	0.40 (1)	3.20	C- Z	0 / 7494	0.66 (1)
G- H	-13015 / 0	-122.2	-122.2	0.40 (1)	3.20	Z- D	-4698 / 0	0.40 (1)
H- I	-13015 / 0	-122.2	-122.2	0.40 (1)	3.20	R- M	0 / 4835	0.43 (1)
I- J	-13015 / 0	-122.2	-122.2	0.40 (1)	3.20	S- L	0 / 3862	0.34 (1)
J- K	-8877 / 0	-122.2	-122.2	0.22 (1)	3.97	R- L	-3086 / 0	0.26 (1)
K- L	-8877 / 0	-122.2	-122.2	0.20 (1)	3.99	F- V	-1095 / 0	0.21 (1)
L- M	-6008 / 0	-122.2	-122.2	0.14 (1)	4.74	Y- F	-252 / 0	0.05 (1)
M- N	-3435 / 0	-122.2	-122.2	0.04 (1)	5.99	X- F	0 / 272	0.02 (2)
N- O	0 / 63	-122.2	-122.2	0.05 (1)	10.00	J- S	-3252 / 0	0.64 (1)
AB- B	-6403 / 0	0.0	0.0	0.24 (1)	5.87	V- J	0 / 2049	0.18 (1)
P- N	-4106 / 0	0.0	0.0	0.15 (1)	7.03	T- J	0 / 248	0.02 (2)
AB-AF	0 / 0	-28.0	-28.0	0.04 (1)	10.00			
AF-AA	0 / 0	-28.0	-28.0	0.04 (1)	10.00			
AA-AG	0 / 3785	-28.0	-28.0	0.14 (1)	10.00			
AG- Z	0 / 3785	-28.0	-28.0	0.14 (1)	10.00			
Z-AH	0 / 9444	-28.0	-28.0	0.32 (1)	10.00			
AH-AI	0 / 9444	-28.0	-28.0	0.32 (1)	10.00			
AI- Y	0 / 9444	-28.0	-28.0	0.32 (1)	10.00			
Y- X	0 / 13873	-28.0	-28.0	0.43 (1)	10.00			
X- W	0 / 13873	-28.0	-28.0	0.40 (1)	10.00			
W- V	0 / 13873	-28.0	-28.0	0.40 (1)	10.00			
V- U	0 / 11409	-28.0	-28.0	0.34 (1)	10.00			
U- T	0 / 11409	-28.0	-28.0	0.34 (1)	10.00			
T- S	0 / 11409	-28.0	-28.0	0.33 (1)	10.00			
S- R	0 / 6008	-28.0	-28.0	0.19 (1)	10.00			
R- Q	0 / 2357	-28.0	-28.0	0.09 (1)	10.00			
Q- P	0 / 0	-28.0	-28.0	0.03 (1)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 777 (0.60")

CSI: TC=0.48 (D-E:1) , BC=0.43 (X-Y:1) , WB=0.66 (C-Z:1) , SSI=0.14 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.88 (W) (INPUT = 1.00)

DWG NO. TAM 42524 - T7
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288236	T1Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	TMVW+p	MT20	6.0	9.0		
P	BMV1+p	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMWW-t	MT20	7.0	8.0		
S	BMWWW-t	MT20	8.0	9.0	4.25	2.50
T	BMW+w	MT20	3.0	6.0		
U	BS-t	MT20	6.0	9.0		
V	BMWWW-t	MT20	5.0	12.0		
W	BS-t	MT20	6.0	9.0		
X	BMW+w	MT20	3.0	6.0		
Y	BMWWW-t	MT20	8.0	9.0	4.25	3.00
Z	BMWW-t	MT20	7.0	8.0		
AA	BMWW-t	MT20	5.0	6.0	2.50	2.75
AB	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 2-2-12, 150.6 lbs FACTORED DOWN AT 2-2-12, 147.1 lbs FACTORED DOWN AT 4-3-8, 147.1 lbs FACTORED DOWN AT 6-3-8, AND 147.1 lbs FACTORED DOWN AT 8-3-8, AND 147.1 lbs FACTORED DOWN AT 9-3-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-3-8, 69.9 lbs FACTORED DOWN AT 4-3-8, 69.9 lbs FACTORED DOWN AT 6-3-8, 69.9 lbs FACTORED DOWN AT 8-3-8, AND 69.9 lbs FACTORED DOWN AT 9-3-8, AND 3069.3 lbs FACTORED DOWN AT 10-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-13	-14	---	FRONT	VERT	DEAD
C	2-2-12	-151	-151	---	BACK	VERT	TOTAL
C	2-2-12	-183	-183	---	FRONT	VERT	SNOW
D	6-3-8	-147	-147	---	BACK	VERT	TOTAL
Y	10-4-8	-3069	-3069	---	BACK	VERT	TOTAL
Z	6-3-8	-40	-70	---	BACK	VERT	TOTAL
AA	2-3-8	-40	-70	---	BACK	VERT	TOTAL
AC	4-3-8	-147	-147	---	BACK	VERT	TOTAL
AD	8-3-8	-147	-147	---	BACK	VERT	TOTAL
AE	9-3-8	-147	-147	---	BACK	VERT	TOTAL
AF	1-0-12	-40	-70	---	BACK	VERT	TOTAL
AG	4-3-8	-40	-70	---	BACK	VERT	TOTAL
AH	8-3-8	-40	-70	---	BACK	VERT	TOTAL
AI	9-3-8	-40	-70	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47521-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:17 2017 Page 1
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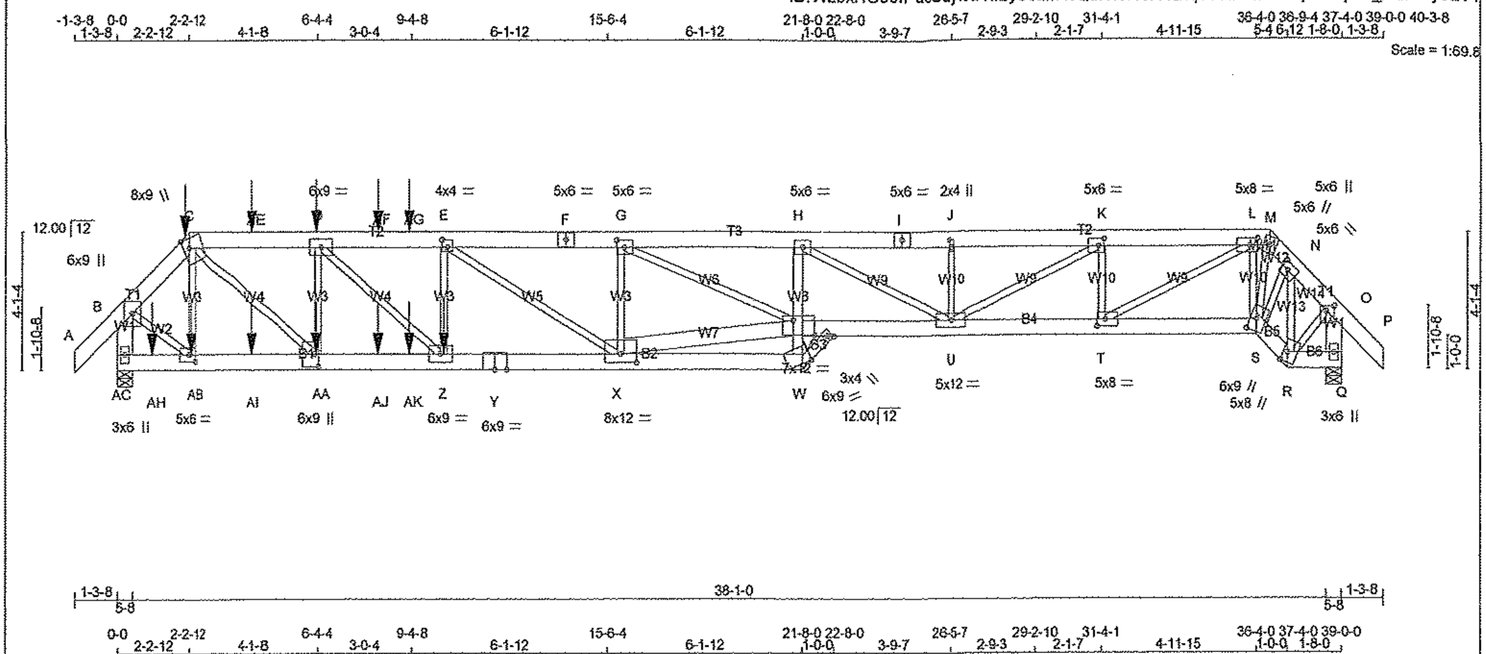

TOTAL WEIGHT = 2 X 227 = 453 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - M	2x6	DRY	No.2	SPF
M - P	2x6	DRY	No.2	SPF
AC- B	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
AC- Y	2x6	DRY	1650F 1.5E	SPF
Y - W	2x6	DRY	1650F 1.5E	SPF
V - S	2x6	DRY	1650F 1.5E	SPF
S - R	2x6	DRY	1650F 1.5E	SPF
R - Q	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
W - H	2x4	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(183.1)
F- I	2	12	TOP
I- M	2	12	TOP
M- P	2	12	TOP
AC- B	2	12	TOP
Q- O	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AC- Y	2	12	SIDE(183.1)
Y- W	2	12	TOP
V- S	2	12	TOP
S- R	2	12	TOP
R- Q	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- Z	1	2	SIDE(617.6)
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	9.0	Edge	2.25
D	TMWW-t	MT20	6.0	9.0		
E	TMWW-t	MT20	4.0	4.0	2.50	2.00
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
AC	6187	0	6187	0	0	5-8	5-8
Q	4074	0	4074	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AC	4833	3153 / 0	857 / 0	0 / 0	0 / 0	823 / 0	0 / 0
Q	3163	2099 / 0	540 / 0	0 / 0	0 / 0	525 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AC, Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00	AB- C	-1762 / 0	0.21 (1)
B- C	-5338 / 0	-122.2	-122.2	0.06 (1)	5.04	Z- E	-769 / 0	0.09 (1)
C-AE	-9300 / 0	-122.2	-122.2	0.28 (1)	3.83	E- X	0 / 198	0.02 (1)
AE- D	-9300 / 0	-122.2	-122.2	0.28 (1)	3.83	X- G	-2268 / 0	0.27 (1)
D-AF	-13600 / 0	-122.2	-122.2	0.48 (1)	3.05	W- V	0 / 118	0.38 (1)
AF-AG	-13600 / 0	-122.2	-122.2	0.48 (1)	3.05	V- H	0 / 1201	0.48 (1)
AG- E	-13600 / 0	-122.2	-122.2	0.48 (1)	3.05	R- N	-3250 / 0	0.31 (1)
E- F	-13764 / 0	-122.2	-122.2	0.51 (1)	3.03	B-AB	0 / 4292	0.53 (1)
F- G	-13764 / 0	-122.2	-122.2	0.51 (1)	3.03	R- O	0 / 2428	0.30 (1)
G- H	-16861 / 0	-122.2	-122.2	0.73 (1)	2.55	X- V	0 / 13548	0.88 (1)
H- I	-13894 / 0	-122.2	-122.2	0.54 (1)	2.97	G- V	0 / 3351	0.41 (1)
I- J	-13894 / 0	-122.2	-122.2	0.54 (1)	2.97	S- L	-3088 / 0	0.28 (1)
J- K	-13894 / 0	-122.2	-122.2	0.47 (1)	3.05	U- J	-646 / 0	0.06 (1)
K- L	-9570 / 0	-122.2	-122.2	0.29 (1)	3.78	T- K	-2963 / 0	0.27 (1)
L- M	-3772 / 0	-122.2	-122.2	0.03 (1)	5.81	H- U	-3581 / 0	0.93 (1)
M- N	-4381 / 0	-122.2	-122.2	0.19 (1)	5.28	U- K	0 / 4917	0.61 (1)
N- O	-2992 / 0	-122.2	-122.2	0.18 (1)	6.12	T- L	0 / 6466	0.80 (1)
O- P	0 / 63	-122.2	-122.2	0.05 (1)	10.00	AA- D	-4711 / 0	0.56 (1)
AC- B	-6243 / 0	0.0	0.0	0.23 (1)	5.94	C-AA	0 / 7453	0.92 (1)
Q- O	-4051 / 0	0.0	0.0	0.15 (1)	7.06	D- Z	0 / [illegible]	[illegible] (1)
						S- M	0 / 2543	0.31 (1)
AC-AH	0 / 0	-28.0	-28.0	0.07 (1)	10.00	S- N	0 / [illegible]	[illegible] (1)
AH-AB	0 / 0	-28.0	-28.0	0.07 (1)	10.00			
AB-AI	0 / 3684	-28.0	-28.0	0.22 (1)	10.00			
AI-AA	0 / 3684	-28.0	-28.0	0.22 (1)	10.00			
AA-AJ	0 / 9300	-28.0	-28.0	0.46 (1)	10.00			
AJ-AK	0 / 9300	-28.0	-28.0	0.46 (1)	10.00			
AK- Z	0 / 9300	-28.0	-28.0	0.46 (1)	10.00			
Z- Y	0 / 13600	-28.0	-28.0	0.63 (1)	10.00			
Y- X	0 / 13600	-28.0	-28.0	0.63 (1)	10.00			
X- W	0 / 490	-28.0	-28.0	0.05 (2)	10.00			
V- U	0 / 17050	-28.0	-28.0	0.76 (1)	10.00			
U- T	0 / 9570	-28.0	-28.0	0.46 (1)	10.00			
T- S	0 / 3884	-28.0	-28.0	0.19 (1)	10.00			
S- R	0 / 2560	-28.0	-28.0	0.11 (1)	10.00			
R- Q	0 / 0	-28.0	-28.0	0.00 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO
9/12/17

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 699 (0.67")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 460 (1.02")

CSI: TC=0.73 (G-H:1) , BC=0.76 (U-V:1) , WB=0.93 (H-U:1) , SSI=0.21 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.92 (V) (INPUT = 1.00)

PG 1/2

DWG NO. TAM 46277-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T1S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TMWW-t	MT20	5.0	6.0		
I	TS-t	MT20	5.0	6.0		
J	TMW+w	MT20	2.0	4.0	2.50	1.00
K	TMWW-t	MT20	5.0	6.0	2.50	2.00
L	TMWW-t	MT20	5.0	8.0	2.50	3.00
M	TTW+m	MT20	5.0	6.0		
N	TMWW-t	MT20	5.0	6.0	2.50	2.50
O	TMVW+p	MT20	5.0	6.0	1.75	2.75
Q	BMV1+p	MT20	3.0	6.0		
R	BBWW+m	MT20	5.0	8.0	3.50	1.50
S	BBWWW+m	MT20	6.0	9.0	4.50	2.25
T	BMWW-t	MT20	5.0	8.0	2.50	3.00
U	BMWWW-t	MT20	5.0	12.0		
V	BWMWW*-I	MT20	7.0	12.0	5.25	8.00
W						
W						
W	BBW-m	MT20	6.0	9.0	Edge	
X	BMWWW-t	MT20	8.0	12.0	3.00	6.00
Y	BS-t	MT20	6.0	9.0		
Z	BMWW-t	MT20	6.0	9.0		
AA	BMWW+t	MT20	6.0	9.0	4.50	1.50
AB	BMWW-t	MT20	5.0	6.0	2.50	2.50
AC	BMV1+p	MT20	3.0	6.0		
AD	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 150.6 lbs FACTORED DOWN AT 2-2-12, 147.1 lbs FACTORED DOWN AT 4-3-8, 147.1 lbs FACTORED DOWN AT 6-3-8, AND 147.1 lbs FACTORED DOWN AT 8-3-8, AND 147.1 lbs FACTORED DOWN AT 9-3-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-3-8, 69.9 lbs FACTORED DOWN AT 4-3-8, 69.9 lbs FACTORED DOWN AT 6-3-8, 69.9 lbs FACTORED DOWN AT 8-3-8, AND 69.9 lbs FACTORED DOWN AT 9-3-8, AND 3069.3 lbs FACTORED DOWN AT 10-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-151	-151	---	BACK	VERT	TOTAL
D	6-3-8	-147	-147	---	BACK	VERT	TOTAL
Z	10-4-8	-3069	-3069	---	BACK	VERT	TOTAL
AA	6-3-8	-40	-70	---	BACK	VERT	TOTAL
AB	2-3-8	-40	-70	---	BACK	VERT	TOTAL
AE	4-3-8	-147	-147	---	BACK	VERT	TOTAL
AF	8-3-8	-147	-147	---	BACK	VERT	TOTAL
AG	9-3-8	-147	-147	---	BACK	VERT	TOTAL
AH	1-0-12	-40	-70	---	BACK	VERT	TOTAL
AI	4-3-8	-40	-70	---	BACK	VERT	TOTAL
AJ	8-3-8	-40	-70	---	BACK	VERT	TOTAL
AK	9-3-8	-40	-70	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM46277-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:18 2017 Page 1

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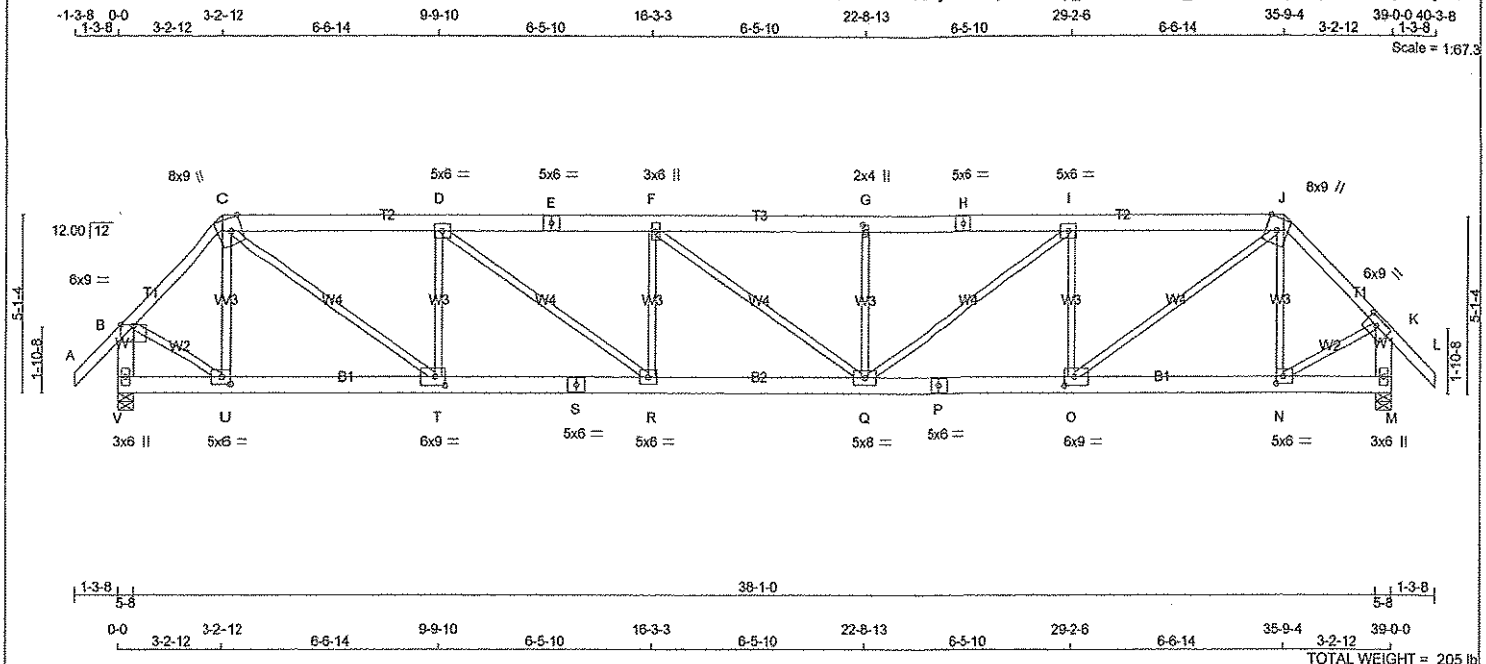


TOTAL WEIGHT = 205 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	Edge	
K	TMVW-t	MT20	6.0	9.0	2.50	4.00
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	6.0	9.0	3.00	3.75
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	6.0	9.0	3.00	3.75
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	3100	0	3100	0	0	5-8	5-8
M	3100	0	3100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
M	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
MAX UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

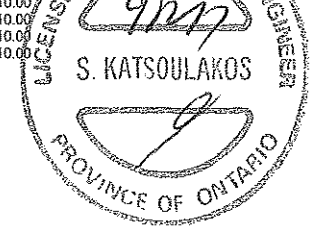
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2693 / 0	-122.2	-122.2	0.34 (1)	3.93
C- D	-4732 / 0	-122.2	-122.2	0.50 (1)	3.61
D- E	-6058 / 0	-122.2	-122.2	0.60 (1)	3.14
E- F	-6058 / 0	-122.2	-122.2	0.60 (1)	3.14
F- G	-6052 / 0	-122.2	-122.2	0.50 (1)	3.26
G- H	-6052 / 0	-122.2	-122.2	0.61 (1)	3.13
H- I	-6052 / 0	-122.2	-122.2	0.61 (1)	3.13
I- J	-4734 / 0	-122.2	-122.2	0.50 (1)	3.61
J- K	-2692 / 0	-122.2	-122.2	0.34 (1)	3.93
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00
V- B	-3092 / 0	0.0	0.0	0.22 (1)	6.00
M- K	-3092 / 0	0.0	0.0	0.22 (1)	6.00
V- U	0 / 0	-28.0	-28.0	0.11 (2)	10.00
U- T	0 / 1893	-28.0	-28.0	0.31 (1)	10.00
T- S	0 / 4732	-28.0	-28.0	0.64 (1)	10.00
S- R	0 / 4732	-28.0	-28.0	0.64 (1)	10.00
R- Q	0 / 6058	-28.0	-28.0	0.60 (1)	10.00
Q- P	0 / 4734	-28.0	-28.0	0.64 (1)	10.00
P- O	0 / 4734	-28.0	-28.0	0.64 (1)	10.00
O- N	0 / 1893	-28.0	-28.0	0.31 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.11 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-631 / 0	0.21 (1)
C- T	0 / 3480	0.78 (1)
T- D	-1849 / 0	0.62 (1)
D- R	0 / 1632	0.37 (1)
R- F	-768 / 0	0.26 (1)
F- Q	-7 / 0	0.01 (1)
Q- G	-766 / 0	0.26 (1)
Q- I	0 / 1623	0.37 (1)
O- I	-1847 / 0	0.62 (1)
O- J	0 / 3482	0.78 (1)
N- J	-632 / 0	0.21 (1)
B- U	0 / 2078	0.47 (1)
N- K	0 / 2077	0.47 (1)



DWG NO. TAM46228-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 767 (0.61")

CSI: TC=0.61 (G-I:1) , BC=0.80 (Q-R:1) , WB=0.78 (J-O:1) , SSI=0.29 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.79 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:18 2017 Page 1

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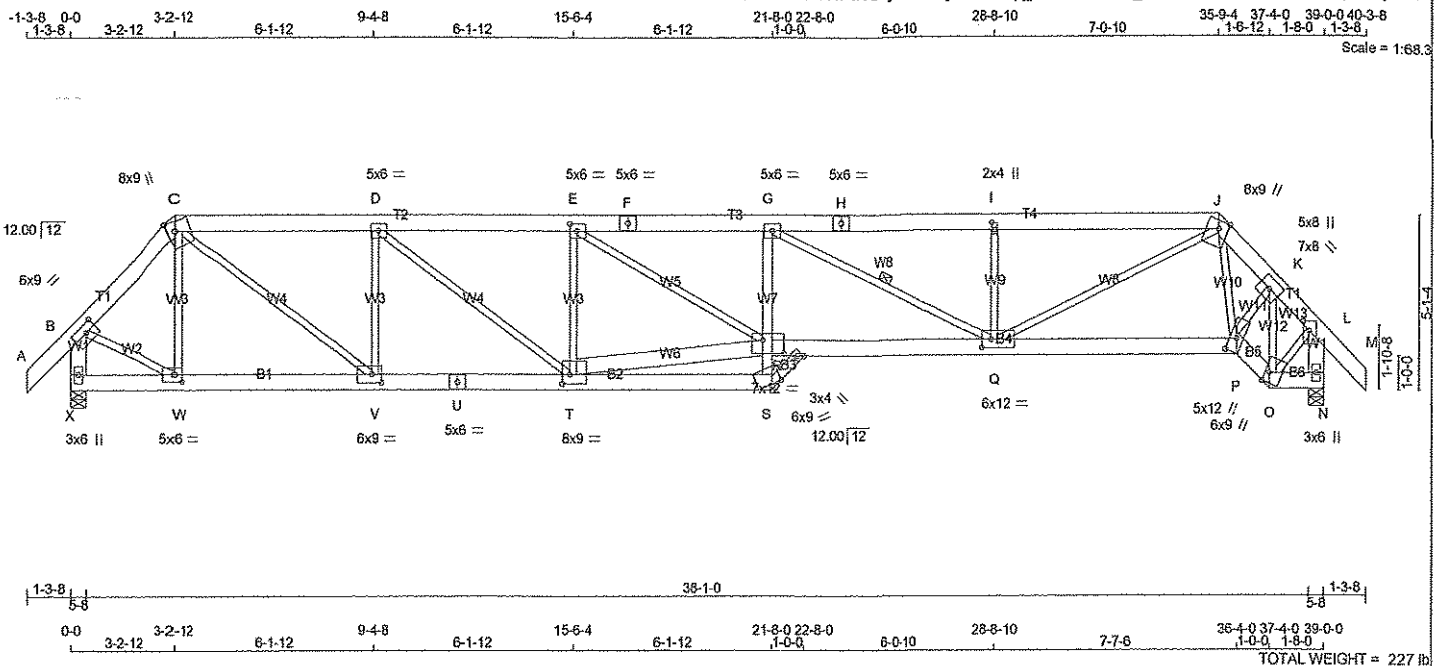


TOTAL WEIGHT = 227 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	1650F 1.5E	SPF
U - S	2x6	DRY	1650F 1.5E	SPF
R - P	2x6	DRY	1650F 1.5E	SPF
P - O	2x6	DRY	1650F 1.5E	SPF
O - N	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	8.0	9.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0	2.50	2.50
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMW+w	MT20	2.0	4.0	2.50	1.00
J	TTWW+m	MT20	8.0	9.0	Edge	3.50
K	TMWW-t	MT20	7.0	8.0		
L	TMW+p	MT20	5.0	8.0	3.25	2.25
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	6.0	9.0	3.50	1.50
P	BBWW+m	MT20	5.0	12.0	5.00	2.50
Q	BMWWW-t	MT20	6.0	12.0	3.00	3.50
R	BWMWW*-I	MT20	7.0	12.0	4.75	8.00
S						
S	BBW-m	MT20	6.0	9.0	Edge	
T	BMWWW-t	MT20	8.0	9.0	3.25	2.75
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	6.0	9.0	3.00	3.75
W	BMWW-t	MT20	5.0	6.0	2.50	2.50
X	BMV1+p	MT20	3.0	6.0		
Y	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	3103	0	3103	0	0	5-8	5-8
N	3110	0	3110	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2408	1600 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0
N	2413	1604 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

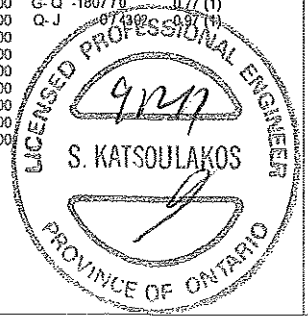
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.09 (1)	10.00
B- C	-2749 / 0	-122.2	-122.2	0.14 (1)	4.93
C- D	-4590 / 0	-122.2	-122.2	0.45 (1)	3.71
D- E	-5938 / 0	-122.2	-122.2	0.54 (1)	3.22
E- F	-7787 / 0	-122.2	-122.2	0.63 (1)	2.79
F- G	-7787 / 0	-122.2	-122.2	0.63 (1)	2.79
G- H	-6225 / 0	-122.2	-122.2	0.72 (1)	2.98
H- I	-6225 / 0	-122.2	-122.2	0.72 (1)	2.98
I- J	-6224 / 0	-122.2	-122.2	0.71 (1)	2.98
J- K	-3337 / 0	-122.2	-122.2	0.16 (1)	4.53
K- L	-2237 / 0	-122.2	-122.2	0.13 (1)	5.34
L- M	0 / 63	-122.2	-122.2	0.09 (1)	10.00
X- B	-3098 / 0	0.0	0.0	0.21 (1)	6.00
N- L	-3087 / 0	0.0	0.0	0.21 (1)	6.01
X- W	0 / 0	-28.0	-28.0	0.07 (2)	10.00
W- V	0 / 1917	-28.0	-28.0	0.21 (1)	10.00
V- U	0 / 4591	-28.0	-28.0	0.39 (1)	10.00
U- T	0 / 4591	-28.0	-28.0	0.39 (1)	10.00
T- S	0 / 177	-28.0	-28.0	0.08 (3)	10.00
R- Q	0 / 7837	-28.0	-28.0	0.65 (1)	10.00
Q- P	0 / 2393	-28.0	-28.0	0.26 (1)	10.00
P- O	0 / 1974	-28.0	-28.0	0.15 (1)	10.00
O- N	0 / 0	-28.0	-28.0	0.01 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-570 / 0	0.19 (1)
C- V	0 / 3336	0.75 (1)
V- D	-1848 / 0	0.62 (1)
D- T	0 / 1704	0.38 (1)
T- E	-1822 / 0	0.61 (1)
S- R	0 / 124	0.26 (1)
R- G	0 / 241	0.25 (1)
J- P	-107 / 84	0.02 (1)
P- K	0 / 1902	0.43 (1)
O- K	-2498 / 0	0.44 (1)
B- W	0 / 2076	0.47 (1)
O- L	0 / 1873	0.42 (1)
T- R	0 / 5876	0.69 (1)
E- R	0 / 2120	0.48 (1)
Q- I	-948 / 0	0.21 (1)
G- Q	-1807 / 0	0.77 (1)
Q- J	67 / 4302	0.97 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 882 (0.53")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 580 (0.81")

CSI: TC=0.72 (G-I:1) , BC=0.65 (Q-R:1) , WB=0.97 (J-Q:1) , SSI=0.31 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.92 (R) (INPUT = 1.00)

DWG NO. YAM46279-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:19 2017 Page 1

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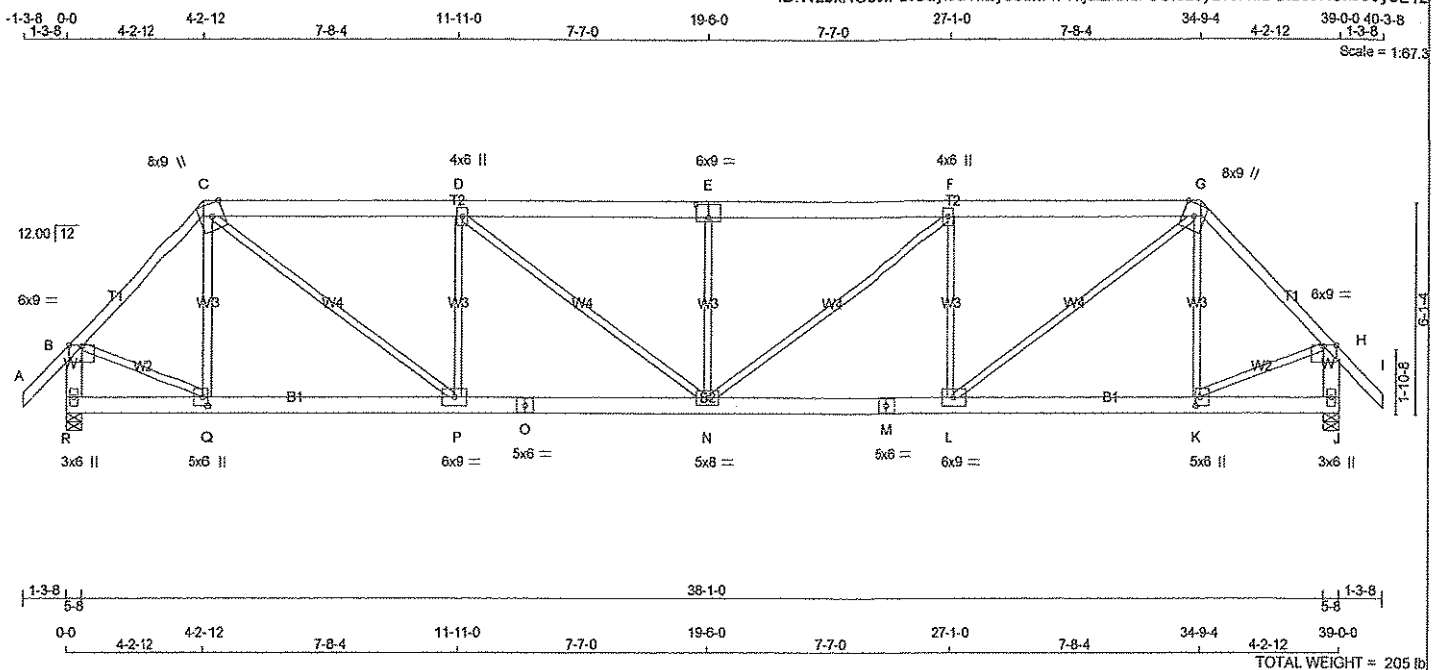

TOTAL WEIGHT = 205 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	
D	TMWW+t	MT20	4.0	6.0		
E	TSW-t	MT20	6.0	9.0	4.50	4.50
F	TMWW+t	MT20	4.0	6.0		
G	TTWW+m	MT20	8.0	9.0	Edge	
H	TMVW-p	MT20	6.0	9.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW+t	MT20	5.0	6.0	3.00	1.50
L	BMWW-t	MT20	6.0	9.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	6.0	9.0		
Q	BMWW+t	MT20	5.0	6.0	3.00	1.50
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3100	0	3100	0	0	5-8	5-8
J	3100	0	3100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
J	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2806 / 0	-122.2	-122.2	0.59 (1)	3.61
C- D	-4432 / 0	-122.2	-122.2	0.68 (1)	3.48
D- E	-5148 / 0	-122.2	-122.2	0.74 (1)	3.19
E- F	-5148 / 0	-122.2	-122.2	0.74 (1)	3.19
F- G	-4432 / 0	-122.2	-122.2	0.68 (1)	3.48
G- H	-2806 / 0	-122.2	-122.2	0.59 (1)	3.61
H- I	0 / 60	-122.2	-122.2	0.17 (1)	10.00
R- B	-3070 / 0	0.0	0.0	0.22 (1)	6.03
J- H	-3070 / 0	0.0	0.0	0.22 (1)	6.03
R- Q	0 / 0	-28.0	-28.0	0.13 (2)	10.00
Q- P	0 / 1978	-28.0	-28.0	0.32 (1)	10.00
P- O	0 / 4432	-28.0	-28.0	0.61 (1)	10.00
O- N	0 / 4432	-28.0	-28.0	0.61 (1)	10.00
N- M	0 / 4432	-28.0	-28.0	0.61 (1)	10.00
M- L	0 / 4432	-28.0	-28.0	0.61 (1)	10.00
L- K	0 / 1978	-28.0	-28.0	0.32 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.13 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-455 / 76	0.23 (1)
C- P	0 / 3047	0.69 (1)
P- D	-1599 / 0	0.81 (1)
D- N	0 / 893	0.20 (1)
N- E	-858 / 0	0.44 (1)
N- F	0 / 693	0.20 (1)
L- F	-1599 / 0	0.81 (1)
L- G	0 / 3047	0.69 (1)
K- G	-455 / 76	0.23 (1)
B- Q	0 / 2093	0.47 (1)
K- H	0 / 2093	0.47 (1)

DWG NO. TAM46280017
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.47")

CSI: TC=0.74 (D-E:1) , BC=0.61 (N-P:1) , WB=0.81 (F-L:1) , SSI=0.34 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.72 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T3S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:19 2017 Page 1

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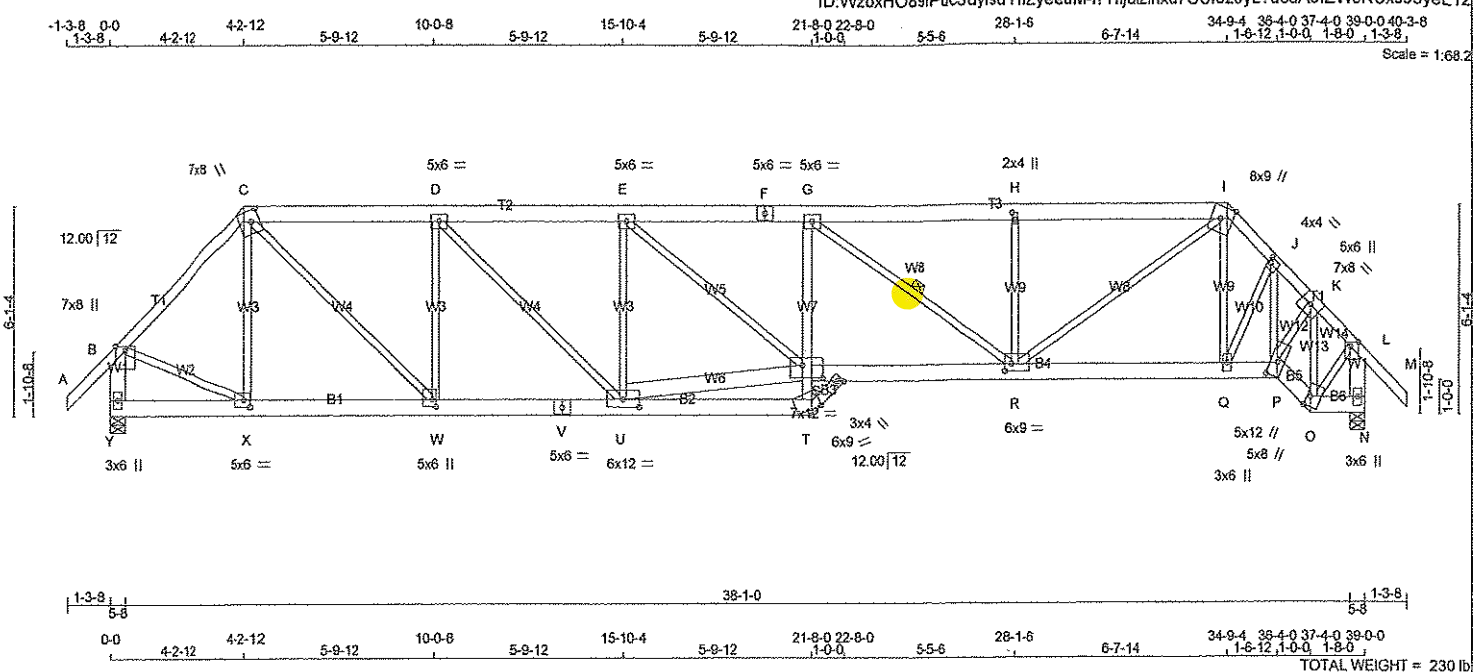


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - G	2x4	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	7.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	4.00	2.75
D, E, G						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	8.0	9.0	Edge	4.50
J	TMWW-t	MT20	4.0	4.0	2.00	1.00
K	TMWW-t	MT20	7.0	8.0		
L	TMVW+p	MT20	5.0	6.0	1.50	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	5.0	8.0	3.50	1.25
P	BBWW+m	MT20	5.0	12.0	5.00	2.50
Q	BMWW+t	MT20	3.0	6.0		
R	BMWWW-t	MT20	6.0	9.0	2.50	2.25
S	BWMWW*-l	MT20	7.0	12.0	4.50	7.50
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWW-t	MT20	6.0	12.0	2.75	6.00
V	BS-t	MT20	5.0	6.0		
W	BMWW+t	MT20	5.0	6.0	2.50	1.50
X	BMWW-t	MT20	5.0	6.0	2.50	2.25
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	3097	0	3097	0	0	5-8	5-8
N	3103	0	3103	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2404	1596 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
N	2408	1600 / 0	409 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

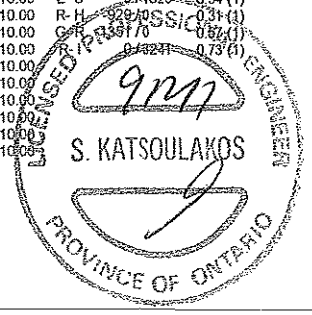
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2792 / 0	-122.2	-122.2	0.59 (1)	3.62
C- D	-3948 / 0	-122.2	-122.2	0.37 (1)	4.03
D- E	-4918 / 0	-122.2	-122.2	0.43 (1)	3.61
E- F	-6122 / 0	-122.2	-122.2	0.43 (1)	3.30
F- G	-6122 / 0	-122.2	-122.2	0.43 (1)	3.30
G- H	-5041 / 0	-122.2	-122.2	0.54 (1)	3.46
H- I	-5041 / 0	-122.2	-122.2	0.54 (1)	3.46
I- J	-3356 / 0	-122.2	-122.2	0.16 (1)	3.72
J- K	-3279 / 0	-122.2	-122.2	0.19 (1)	3.74
K- L	-2161 / 0	-122.2	-122.2	0.21 (1)	4.43
L- M	0 / 60	-122.2	-122.2	0.17 (1)	10.00
Y- B	-3057 / 0	0.0	0.0	0.22 (1)	6.03
N- L	-3079 / 0	0.0	0.0	0.22 (1)	6.01
Y- X	0 / 0	-28.0	-28.0	0.09 (2)	10.00
X- W	0 / 1968	-28.0	-28.0	0.29 (1)	10.00
W- V	0 / 3948	-28.0	-28.0	0.52 (1)	10.00
V- U	0 / 3948	-28.0	-28.0	0.52 (1)	10.00
U- T	0 / 108	-28.0	-28.0	0.11 (3)	10.00
S- R	0 / 6146	-28.0	-28.0	0.82 (1)	10.00
R- Q	0 / 2385	-28.0	-28.0	0.39 (1)	10.00
Q- P	0 / 2346	-28.0	-28.0	0.38 (1)	10.00
P- O	0 / 1956	-28.0	-28.0	0.23 (1)	10.00
O- N	0 / 0	-28.0	-28.0	0.01 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-482 / 28	0.25 (1)
C- W	0 / 2751	0.62 (1)
W- D	-1767 / 0	0.99 (1)
D- U	0 / 1367	0.31 (1)
U- E	-1643 / 0	0.84 (1)
T- S	0 / 116	0.16 (1)
S- G	0 / 234	0.16 (1)
Q- I	0 / 195	0.04 (3)
Q- J	0 / 83	0.02 (2)
P- J	-299 / 0	0.06 (1)
P- K	0 / 1942	0.44 (1)
O- K	-2578 / 0	0.49 (1)
B- X	0 / 2083	0.47 (1)
O- L	0 / 1920	0.43 (1)
U- S	0 / 4907	0.58 (1)
E- S	0 / 4525	0.34 (1)
R- H	-929 / 0	0.31 (1)
G- R	-4351 / 0	0.52 (1)
R- I	0 / 4321	0.73 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.37")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 836 (0.56")

CSI: TC=0.59 (B-C:1) , BC=0.82 (R-S:1) , WB=0.90 (D-W:1) , SSI=0.29 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.75 (S) (INPUT = 1.00)

DWG NO. TAM 46281-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:19 2017 Page 1

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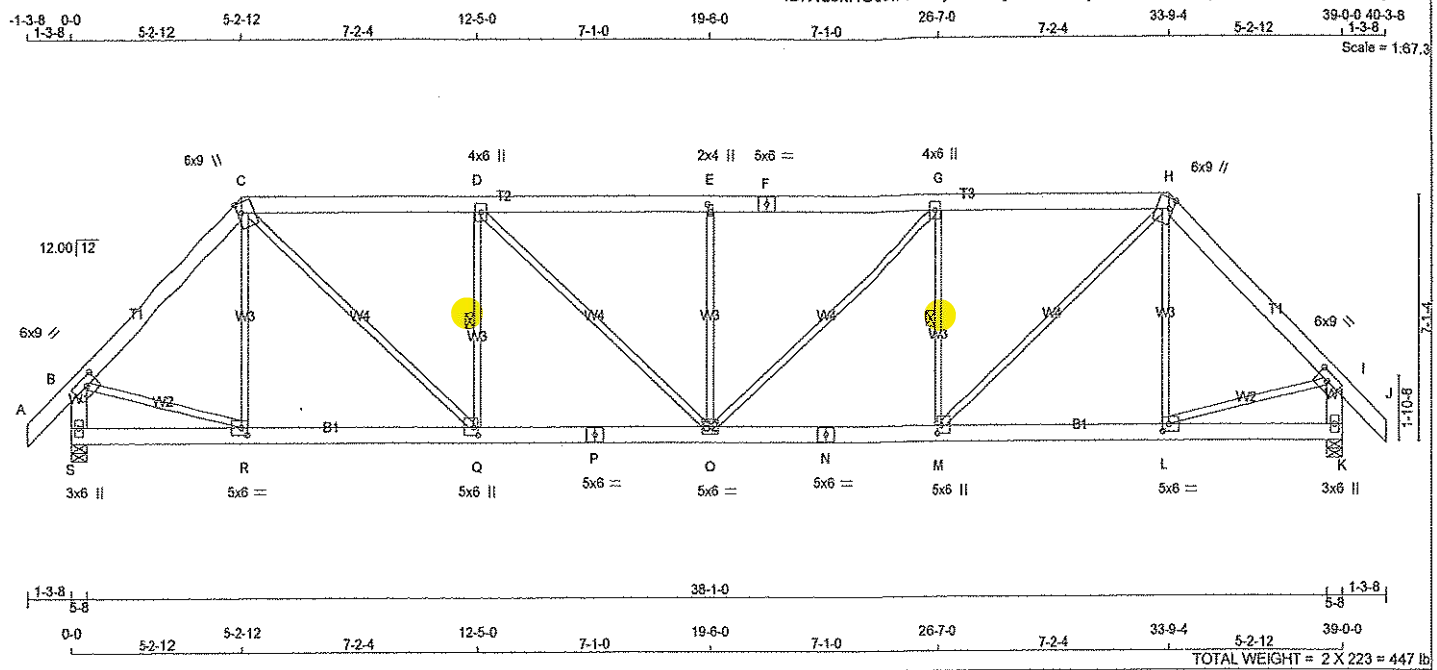

TOTAL WEIGHT = 2 X 223 = 447 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	6.0	9.0	3.50	1.50
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	3.50	1.50
I	TMVW-t	MT20	6.0	9.0	2.75	4.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMWW+t	MT20	5.0	6.0	2.75	1.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	6.0	2.75	1.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3107	0	3107	0	0	5-8	5-8
K	3107	0	3107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2411	1602 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0
K	2411	1602 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-Q, G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 63	-122.2	-122.2	0.09	(1)	10.00	R-C	-301 / 144	0.23 (1)
B-C	-2891 / 0	-122.2	-122.2	0.35	(1)	4.60	C-Q	0 / 2451	0.55 (1)
C-D	-3835 / 0	-122.2	-122.2	0.56	(1)	3.85	Q-D	-1461 / 0	0.43 (1)
D-E	-4364 / 0	-122.2	-122.2	0.59	(1)	3.61	D-O	0 / 724	0.16 (1)
E-F	-4364 / 0	-122.2	-122.2	0.59	(1)	3.61	O-E	-800 / 0	0.61 (1)
F-G	-4364 / 0	-122.2	-122.2	0.59	(1)	3.61	O-G	0 / 724	0.16 (1)
G-H	-3835 / 0	-122.2	-122.2	0.56	(1)	3.85	M-G	-1461 / 0	0.43 (1)
H-I	-2891 / 0	-122.2	-122.2	0.35	(1)	4.60	M-H	0 / 2451	0.55 (1)
I-J	0 / 63	-122.2	-122.2	0.09	(1)	10.00	L-H	-301 / 144	0.23 (1)
S-B	-3055 / 0	0.0	0.0	0.21	(1)	6.03	B-R	0 / 2097	0.47 (1)
K-I	-3055 / 0	0.0	0.0	0.21	(1)	6.03	L-I	0 / 2097	0.47 (1)
S-R	0 / 0	-28.0	-28.0	0.13	(2)	10.00			
R-Q	0 / 2030	-28.0	-28.0	0.32	(1)	10.00			
Q-P	0 / 3836	-28.0	-28.0	0.53	(1)	10.00			
P-O	0 / 3836	-28.0	-28.0	0.53	(1)	10.00			
O-N	0 / 3836	-28.0	-28.0	0.53	(1)	10.00			
N-M	0 / 3836	-28.0	-28.0	0.53	(1)	10.00			
M-L	0 / 2030	-28.0	-28.0	0.32	(1)	10.00			
L-K	0 / 0	-28.0	-28.0	0.13	(2)	10.00			

DWG NO. TAM46282-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.59 (E-G:1) , BC=0.53 (O-Q:1) , WB=0.61 (E-O:1) , SSI=0.32 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (R) (INPUT = 0.90)
JSI METAL= 0.66 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:20 2017 Page 1

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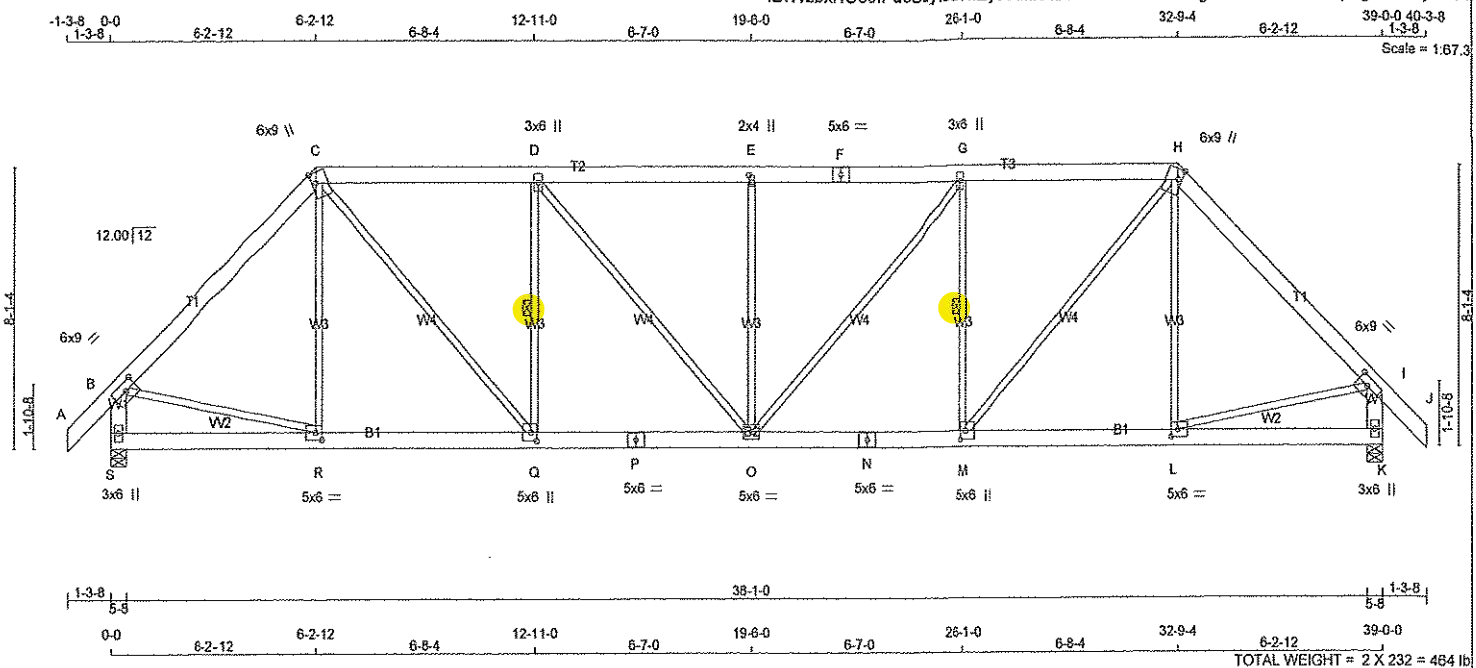


TOTAL WEIGHT = 2 X 232 = 464 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	6.0	9.0	3.50	1.50
D	TMWW+t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	3.50	1.50
I	TMVW-t	MT20	6.0	9.0	2.75	4.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW+t	MT20	5.0	6.0	3.00	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	6.0	3.00	2.00
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3107	0	3107	0	0	5-8	5-8
K	3107	0	3107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2411	1602 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0
K	2411	1602 / 0	409 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-Q, G-M.

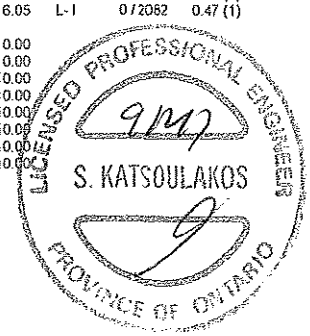
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.09 (1)		10.00
B- C	-2894 / 0	-122.2	-122.2	0.51 (1)		4.42
C- D	-3389 / 0	-122.2	-122.2	0.46 (1)		4.17
D- E	-3784 / 0	-122.2	-122.2	0.48 (1)		3.97
E- F	-3784 / 0	-122.2	-122.2	0.48 (1)		3.97
F- G	-3784 / 0	-122.2	-122.2	0.48 (1)		3.97
G- H	-3389 / 0	-122.2	-122.2	0.46 (1)		4.17
H- I	-2894 / 0	-122.2	-122.2	0.51 (1)		4.42
I- J	0 / 63	-122.2	-122.2	0.09 (1)		10.00
S- B	-3038 / 0	0.0	0.0	0.21 (1)		6.05
K- I	-3038 / 0	0.0	0.0	0.21 (1)		6.05
S- R	0 / 0	-28.0	-28.0	0.14 (2)		10.00
R- Q	0 / 2035	-28.0	-28.0	0.32 (1)		10.00
Q- P	0 / 3389	-28.0	-28.0	0.46 (1)		10.00
P- O	0 / 3389	-28.0	-28.0	0.46 (1)		10.00
O- N	0 / 3389	-28.0	-28.0	0.46 (1)		10.00
N- M	0 / 3389	-28.0	-28.0	0.46 (1)		10.00
M- L	0 / 2035	-28.0	-28.0	0.32 (1)		10.00
L- K	0 / 0	-28.0	-28.0	0.14 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-217 / 192	0.24 (1)
C- Q	0 / 2049	0.46 (1)
Q- D	-1356 / 0	0.52 (1)
D- O	0 / 605	0.14 (1)
O- E	-742 / 0	0.83 (1)
O- G	0 / 605	0.14 (1)
M- G	-1356 / 0	0.52 (1)
M- H	0 / 2049	0.46 (1)
L- H	-217 / 192	0.24 (1)
B- R	0 / 2062	0.47 (1)
L- I	0 / 2062	0.47 (1)



DWG NO. TAM46283-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.51 (B-C:1) , BC=0.46 (O-Q:1) , WB=0.83 (E-O:1) , SSI=0.29 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.58 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:20 2017 Page 1

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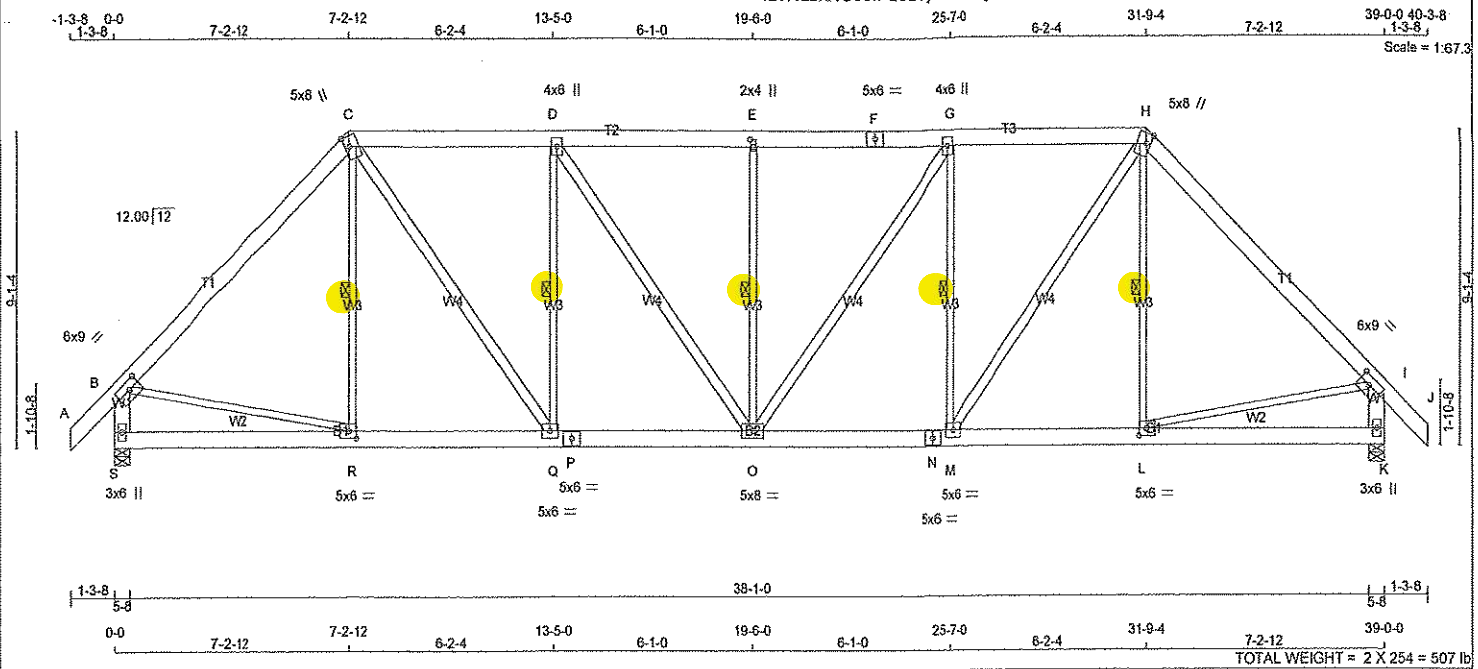

TOTAL WEIGHT = 2 X 254 = 507 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - Q	2x4	DRY	No.2	SPF
D - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	5.0	8.0	3.50	1.50
D	TMWW+(	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	5.0	8.0	3.50	1.50
I	TMVW-t	MT20	6.0	9.0	2.75	4.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3107	0	3107	0	0	5-8	5-8
K	3107	0	3107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2411	1602 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0
K	2411	1602 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-R, D-Q, E-O, G-M, H-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.09 (1)	10.00
B- C	-2872 / 0	-122.2	-122.2	0.70 (1)	4.17
C- D	-3043 / 0	-122.2	-122.2	0.38 (1)	4.46
D- E	-3339 / 0	-122.2	-122.2	0.40 (1)	4.28
E- F	-3339 / 0	-122.2	-122.2	0.40 (1)	4.28
F- G	-3339 / 0	-122.2	-122.2	0.40 (1)	4.28
G- H	-3043 / 0	-122.2	-122.2	0.38 (1)	4.46
H- I	-2872 / 0	-122.2	-122.2	0.70 (1)	4.17
I- J	0 / 63	-122.2	-122.2	0.09 (1)	10.00
S- B	-3022 / 0	0.0	0.0	0.21 (1)	6.07
K- I	-3022 / 0	0.0	0.0	0.21 (1)	6.07
S- R	0 / 0	-28.0	-28.0	0.16 (3)	10.00
R- Q	0 / 2022	-28.0	-28.0	0.33 (1)	10.00
Q- P	0 / 3043	-28.0	-28.0	0.41 (1)	10.00
P- O	0 / 3043	-28.0	-28.0	0.41 (1)	10.00
O- N	0 / 3043	-28.0	-28.0	0.41 (1)	10.00
N- M	0 / 3043	-28.0	-28.0	0.41 (1)	10.00
M- L	0 / 2022	-28.0	-28.0	0.33 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-149 / 240	0.07 (1)
C- Q	0 / 1748	0.28 (1)
Q- D	-1251 / 0	0.62 (1)
D- O	0 / 513	0.08 (1)
O- E	-684 / 0	0.34 (1)
O- G	0 / 513	0.08 (1)
M- G	-1251 / 0	0.62 (1)
M- H	0 / 1748	0.28 (1)
L- H	-149 / 240	0.07 (1)
B- R	0 / 2057	0.46 (1)
L- I	0 / 2057	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.70 (B-C:1) , BC=0.41 (O-Q:1) , WB=0.62 (G-M:1) , SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (R) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/12/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO , TAM 46284-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:21 2017 Page 1

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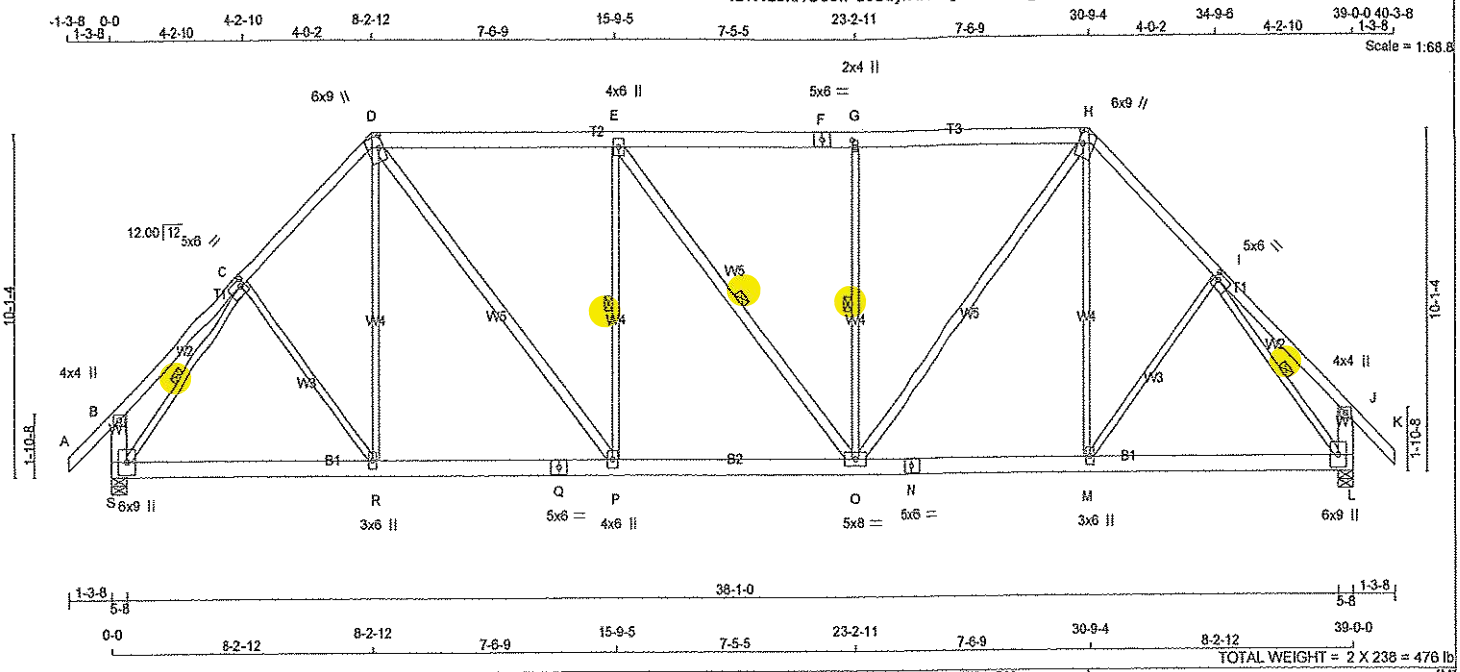


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
E - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	6.0	9.0	4.25	1.75
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TTWW+m	MT20	6.0	9.0	4.25	1.75
I	TMWW-t	MT20	5.0	6.0	2.25	1.50
J	TMV+p	MT20	4.0	4.0		
L	BMVW1+p	MT20	6.0	9.0		
M	BMWW+t	MT20	3.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	3.0	6.0		
S	BMVW1+p	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3100	0	3100	0	0	5-8	5-8
L	3100	0	3100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
L	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, E-O, G-O, C-S, I-L

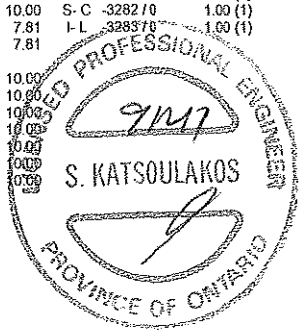
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 40	-122.2	-122.2	0.32 (1)	10.00	
C- D	-2843 / 0	-122.2	-122.2	0.52 (1)	3.68	
D- E	-2934 / 0	-122.2	-122.2	0.54 (1)	4.30	
E- F	-2928 / 0	-122.2	-122.2	0.54 (1)	4.31	
F- G	-2928 / 0	-122.2	-122.2	0.54 (1)	4.31	
G- H	-2928 / 0	-122.2	-122.2	0.52 (1)	4.34	
H- I	-2843 / 0	-122.2	-122.2	0.52 (1)	3.68	
I- J	0 / 40	-122.2	-122.2	0.32 (1)	10.00	
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
S- B	-362 / 0	0.0	0.0	0.03 (1)	7.81	
L- J	-362 / 0	0.0	0.0	0.03 (1)	7.81	
S- R	0 / 1950	-28.0	-28.0	0.35 (2)	10.00	
R- Q	0 / 1987	-28.0	-28.0	0.36 (2)	10.00	
Q- P	0 / 1987	-28.0	-28.0	0.36 (2)	10.00	
P- O	0 / 2933	-28.0	-28.0	0.42 (1)	10.00	
O- N	0 / 1988	-28.0	-28.0	0.36 (2)	10.00	
N- M	0 / 1988	-28.0	-28.0	0.36 (2)	10.00	
M- L	0 / 1951	-28.0	-28.0	0.35 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 149	0.03 (3)
R- D	0 / 328	0.07 (2)
D- P	0 / 1537	0.25 (1)
P- E	-1005 / 0	0.64 (1)
E- O	-9 / 0	0.01 (1)
O- G	-1002 / 0	0.64 (1)
O- H	0 / 1526	0.25 (1)
M- H	0 / 331	0.07 (2)
M- I	0 / 149	0.03 (3)
S- C	-3282 / 0	1.00 (1)
I- L	-3283 / 0	1.00 (1)



DWG NO. TAM 46285-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.54 (D-E:1) , BC=0.42 (O-P:1) , WB=1.00 (I-L:1) , SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.81 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T8	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:21 2017 Page 1

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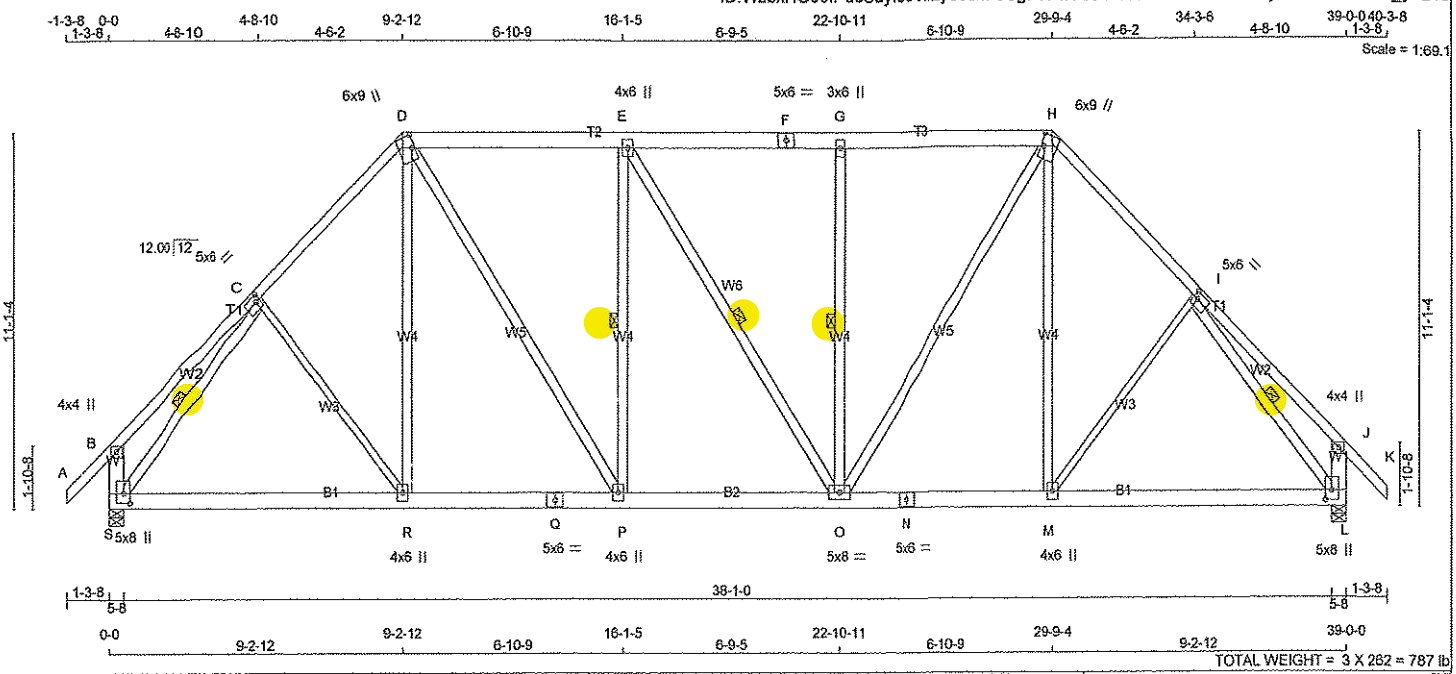


TOTAL WEIGHT = 3 X 262 = 787 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	6.0	9.0	4.50	1.00
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.50	1.00
I	TMWW-t	MT20	5.0	6.0	2.25	1.50
J	TMV+p	MT20	4.0	4.0		
L	BMVW1+p	MT20	5.0	8.0	3.50	2.25
M, P, R						
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
S	BMVW1+p	MT20	5.0	8.0	3.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	3100	0	3100	0	0	5-8	5-8
L	3100	0	3100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
L	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, E-O, G-O, C-S, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

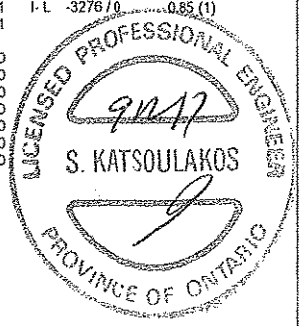
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 47	-122.2	-122.2	0.42 (1)	10.00
C- D	-2804 / 0	-122.2	-122.2	0.70 (1)	3.53
D- E	-2661 / 0	-122.2	-122.2	0.44 (1)	4.63
E- F	-2656 / 0	-122.2	-122.2	0.43 (1)	4.63
F- G	-2656 / 0	-122.2	-122.2	0.43 (1)	4.63
G- H	-2656 / 0	-122.2	-122.2	0.42 (1)	4.66
H- I	-2805 / 0	-122.2	-122.2	0.70 (1)	3.53
I- J	0 / 47	-122.2	-122.2	0.42 (1)	10.00
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00
S- B	-383 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-383 / 0	0.0	0.0	0.03 (1)	7.81
S- R	0 / 1982	-28.0	-28.0	0.40 (2)	10.00
R- Q	0 / 1957	-28.0	-28.0	0.40 (2)	10.00
Q- P	0 / 1957	-28.0	-28.0	0.40 (2)	10.00
P- O	0 / 2661	-28.0	-28.0	0.38 (1)	10.00
O- N	0 / 1957	-28.0	-28.0	0.40 (2)	10.00
N- M	0 / 1957	-28.0	-28.0	0.40 (2)	10.00
M- L	0 / 1983	-28.0	-28.0	0.40 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	-47 / 126	0.06 (1)
R- D	0 / 403	0.06 (2)
D- P	0 / 1297	0.21 (1)
P- E	-916 / 0	0.54 (1)
E- O	-8 / 0	0.01 (1)
O- G	-913 / 0	0.54 (1)
O- H	0 / 1288	0.21 (1)
M- H	0 / 404	0.06 (2)
M- I	-47 / 126	0.06 (1)
S- C	-3275 / 0	0.85 (1)
I- L	-3276 / 0	0.85 (1)



DWG NO. TAM 46286-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.70 (H-I:1) , BC=0.40 (M-O:2) , WB=0.85 (I-L:1) , SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.81 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:22 2017 Page 1

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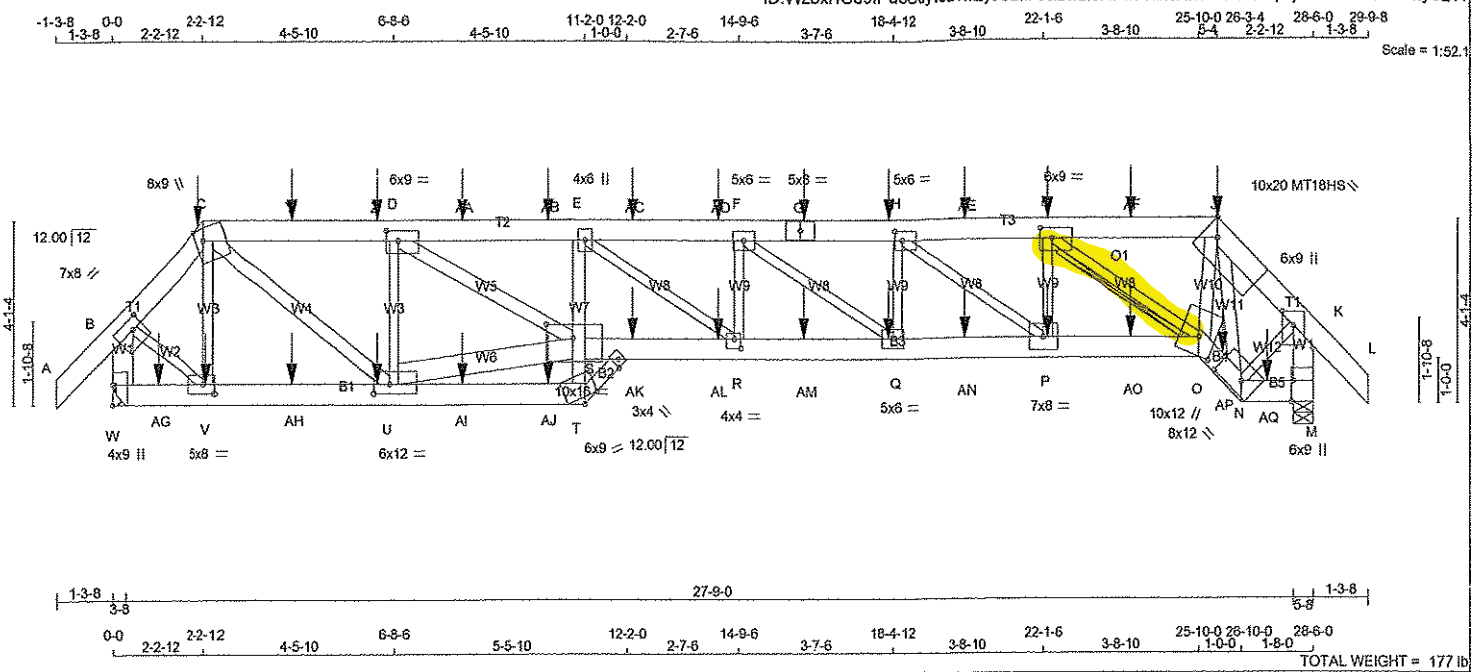


TOTAL WEIGHT = 177 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
O - N	2x6	DRY	2100F 1.8E	SPF
N - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - U	2x4	DRY	No.2	SPF
T - E	2x4	DRY	No.2	SPF
J - N	2x4	DRY	No.2	SPF
U - S	2x6	DRY	2100F 1.8E	SPF
D - S	2x4	DRY	No.2	SPF
O - J	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

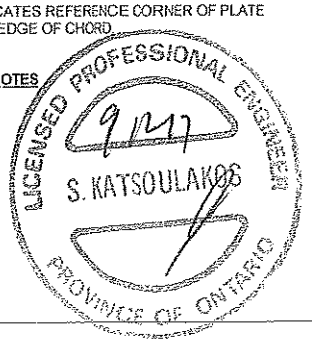
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	7.0	8.0	2.75	3.00
C	TTWW+m	MT20	8.0	9.0	Edge	2.25
D	TMWW-t	MT20	6.0	9.0	2.75	3.50
E	TMWW+t	MT20	4.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	5.0	8.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.00
I	TMWW-t	MT20	6.0	9.0	2.75	3.25
J	TTWW-h	MT18HS	10.0	20.0	Edge	4.00
K	TMVW+p	MT20	6.0	9.0	3.75	3.00
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BBWW-h	MT20	8.0	12.0	3.25	7.50
O	BBWW+m	MT20	10.0	12.0	5.00	4.75
P	BMWW-t	MT20	7.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.50	1.75
S	BWMWW'-t	MT20	10.0	16.0	3.75	7.50
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWW-t	MT20	6.0	12.0	2.50	4.50
V	BMWW-t	MT20	5.0	8.0	2.50	3.00
W	BMV1+t	MT20	4.0	9.0	5.50	
X	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

HANGERS NOTES

1)



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3990	0	3990	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
M	4251	0	4251	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3118	2032 / 0	554 / 0	0 / 0	0 / 0	532 / 0	0 / 0
M	3328	2159 / 0	597 / 0	0 / 0	0 / 0	571 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 1.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 6.99 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.10 (1)	10.00
B- C	-3306 / 0	-122.2	-122.2	0.11 (1)	4.57
C- Y	-5425 / 0	-122.2	-122.2	0.47 (1)	3.34
Y- Z	-5425 / 0	-122.2	-122.2	0.47 (1)	3.34
Z- D	-5425 / 0	-122.2	-122.2	0.47 (1)	3.34
D-AA	-9960 / 0	-122.2	-122.2	0.95 (1)	1.91
AA-AB	-9960 / 0	-122.2	-122.2	0.95 (1)	1.91
AB- E	-9960 / 0	-122.2	-122.2	0.95 (1)	1.91
E-AC	-10626 / 0	-122.2	-122.2	0.96 (1)	1.82
AC-AD	-10626 / 0	-122.2	-122.2	0.96 (1)	1.82
AD- F	-10626 / 0	-122.2	-122.2	0.96 (1)	1.82
F- G	-9807 / 0	-122.2	-122.2	0.84 (1)	2.10
G- H	-9807 / 0	-122.2	-122.2	0.84 (1)	2.10
H-AE	-7544 / 0	-122.2	-122.2	0.56 (1)	2.77
AE- I	-7544 / 0	-122.2	-122.2	0.56 (1)	2.77
I-AF	-3895 / 0	-122.2	-122.2	0.31 (1)	4.06
AF- J	-3895 / 0	-122.2	-122.2	0.31 (1)	4.06
J- K	-3120 / 0	-122.2	-122.2	0.11 (1)	4.69
K- L	0 / 63	-122.2	-122.2	0.10 (1)	10.00
W- B	-3987 / 0	0.0	0.0	0.30 (1)	5.33
M- K	-4212 / 0	0.0	0.0	0.31 (1)	5.19
W-AG	0 / 0	-28.0	-28.0	0.05 (1)	10.00
AG- V	0 / 0	-28.0	-28.0	0.05 (1)	10.00
V-AH	0 / 2283	-28.0	-28.0	0.17 (1)	10.00
AH- U	0 / 2283	-28.0	-28.0	0.17 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1062 / 0	0.25 (1)
C- U	0 / 4089	0.72 (1)
U- D	-3642 / 0	0.87 (1)
T- S	0 / 175	0.35 (1)
S- E	-1061 / 0	0.36 (1)
J- N	-4465 / 0	0.69 (1)
B- V	0 / 2659	0.66 (1)
U- S	0 / 5468	0.29 (1)
D- S	0 / 5165	0.91 (1)
O- J	0 / 5995	0.77 (1)
N- K	0 / 2747	0.68 (1)
Q- H	0 / 1029	0.25 (1)
P- I	0 / 1957	0.48 (1)
R- F	-56 / 141	0.03 (3)
E- R	0 / 711	0.18 (1)
F- Q	-1005 / 0	0.34 (1)
H- P	-2777 / 0	0.95 (1)
I- O	-4495 / 0	0.93 (1)

DWG NO. TAM 46687-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 682 (0.50")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 446 (0.77")

CSI: TC=0.96 (E-F:1) , BC=0.68 (Q-R:1) , WB=0.95 (H-P:1) , SSI=0.37 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MT18HS	580	354	2729	1405	4191	2010

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.97 (S) (INPUT = 1.00)

p6/6

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 192.5 lbs FACTORED DOWN AT 2-2-12, 192.5 lbs FACTORED DOWN AT 26-3-4, 150.6 lbs FACTORED DOWN AT 2-2-12, 147.1 lbs FACTORED DOWN AT 4-3-8, 147.1 lbs FACTORED DOWN AT 6-3-8, 147.1 lbs FACTORED DOWN AT 8-3-8, 147.1 lbs FACTORED DOWN AT 10-3-8, 89.2 lbs FACTORED DOWN AT 12-3-8, 89.2 lbs FACTORED DOWN AT 14-3-8, 89.2 lbs FACTORED DOWN AT 16-3-8, 89.2 lbs FACTORED DOWN AT 18-3-8, 89.2 lbs FACTORED DOWN AT 20-2-8, 89.2 lbs FACTORED DOWN AT 22-2-8, AND 89.2 lbs FACTORED DOWN AT 24-2-8, AND 92.7 lbs FACTORED DOWN AT 26-3-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-3-8, 69.9 lbs FACTORED DOWN AT 4-3-8, 69.9 lbs FACTORED DOWN AT 6-3-8, 69.9 lbs FACTORED DOWN AT 8-3-8, 76.5 lbs FACTORED DOWN AT 10-3-8, 182.7 lbs FACTORED DOWN AT 12-3-8, 182.7 lbs FACTORED DOWN AT 14-3-8, 182.7 lbs FACTORED DOWN AT 16-3-8, 182.7 lbs FACTORED DOWN AT 18-3-8, 182.7 lbs FACTORED DOWN AT 20-2-8, 182.7 lbs FACTORED DOWN AT 22-2-8, 182.7 lbs FACTORED DOWN AT 24-2-8, AND 196.7 lbs FACTORED DOWN AT 26-2-8, AND 79.6 lbs FACTORED DOWN AT 27-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	WEBS MEMB. FR-TO	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-AI	0 / 217	-28.0	-28.0	0.07 (2)		10.00		
AI-AJ	0 / 217	-28.0	-28.0	0.07 (2)		10.00		
AJ- T	0 / 217	-28.0	-28.0	0.07 (2)		10.00		
S-AK	0 / 10044	-28.0	-28.0	0.63 (1)		10.00		
AK-AL	0 / 10044	-28.0	-28.0	0.63 (1)		10.00		
AL- R	0 / 10044	-28.0	-28.0	0.63 (1)		10.00		
R-AM	0 / 10626	-28.0	-28.0	0.68 (1)		10.00		
AM- Q	0 / 10626	-28.0	-28.0	0.68 (1)		10.00		
Q-AN	0 / 9807	-28.0	-28.0	0.64 (1)		10.00		
AN- P	0 / 9807	-28.0	-28.0	0.64 (1)		10.00		
P-AO	0 / 7545	-28.0	-28.0	0.49 (1)		10.00		
AO- O	0 / 7545	-28.0	-28.0	0.49 (1)		10.00		
O-AP	0 / 4092	-28.0	-28.0	0.23 (1)		10.00		
AP- N	0 / 4092	-28.0	-28.0	0.23 (1)		10.00		
N-AQ	0 / 0	-28.0	-28.0	0.02 (2)		10.00		
AQ- M	0 / 0	-28.0	-28.0	0.02 (2)		10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-13	-14	---	FRONT	VERT	DEAD
C	2-2-12	-151	-151	---	BACK	VERT	TOTAL
C	2-2-12	-180	-180	---	FRONT	VERT	SNOW
G	16-3-8	-89	-89	---	BACK	VERT	TOTAL
H	18-3-8	-89	-89	---	BACK	VERT	TOTAL
I	22-2-8	-89	-89	---	BACK	VERT	TOTAL
J	26-3-4	-13	-14	---	FRONT	VERT	DEAD
J	26-3-4	-93	-93	---	BACK	VERT	TOTAL
J	26-3-4	-180	-180	---	FRONT	VERT	SNOW
P	22-2-8	-183	-183	---	BACK	VERT	TOTAL
Q	18-3-8	-183	-183	---	BACK	VERT	TOTAL
U	6-3-8	-40	-70	---	BACK	VERT	TOTAL
V	2-3-8	-40	-70	---	BACK	VERT	TOTAL
Y	4-3-8	-147	-147	---	BACK	VERT	TOTAL
Z	6-3-8	-147	-147	---	BACK	VERT	TOTAL
AA	8-3-8	-147	-147	---	BACK	VERT	TOTAL
AB	10-3-8	-147	-147	---	BACK	VERT	TOTAL
AC	12-3-8	-89	-89	---	BACK	VERT	TOTAL
AD	14-3-8	-89	-89	---	BACK	VERT	TOTAL
AE	20-2-8	-89	-89	---	BACK	VERT	TOTAL
AF	24-2-8	-89	-89	---	BACK	VERT	TOTAL
AG	1-0-12	-40	-70	---	BACK	VERT	TOTAL
AH	4-3-8	-40	-70	---	BACK	VERT	TOTAL
AI	8-3-8	-40	-70	---	BACK	VERT	TOTAL
AJ	10-3-8	-44	-77	---	BACK	VERT	TOTAL
AK	12-3-8	-183	-183	---	BACK	VERT	TOTAL
AL	14-3-8	-183	-183	---	BACK	VERT	TOTAL
AM	16-3-8	-183	-183	---	BACK	VERT	TOTAL
AN	20-2-8	-183	-183	---	BACK	VERT	TOTAL
AO	24-2-8	-183	-183	---	BACK	VERT	TOTAL
AP	26-2-8	-197	-197	---	BACK	VERT	TOTAL
AQ	27-5-4	-45	-80	---	BACK	VERT	TOTAL

DWG NO. TAM46287-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:22 2017 Page 1

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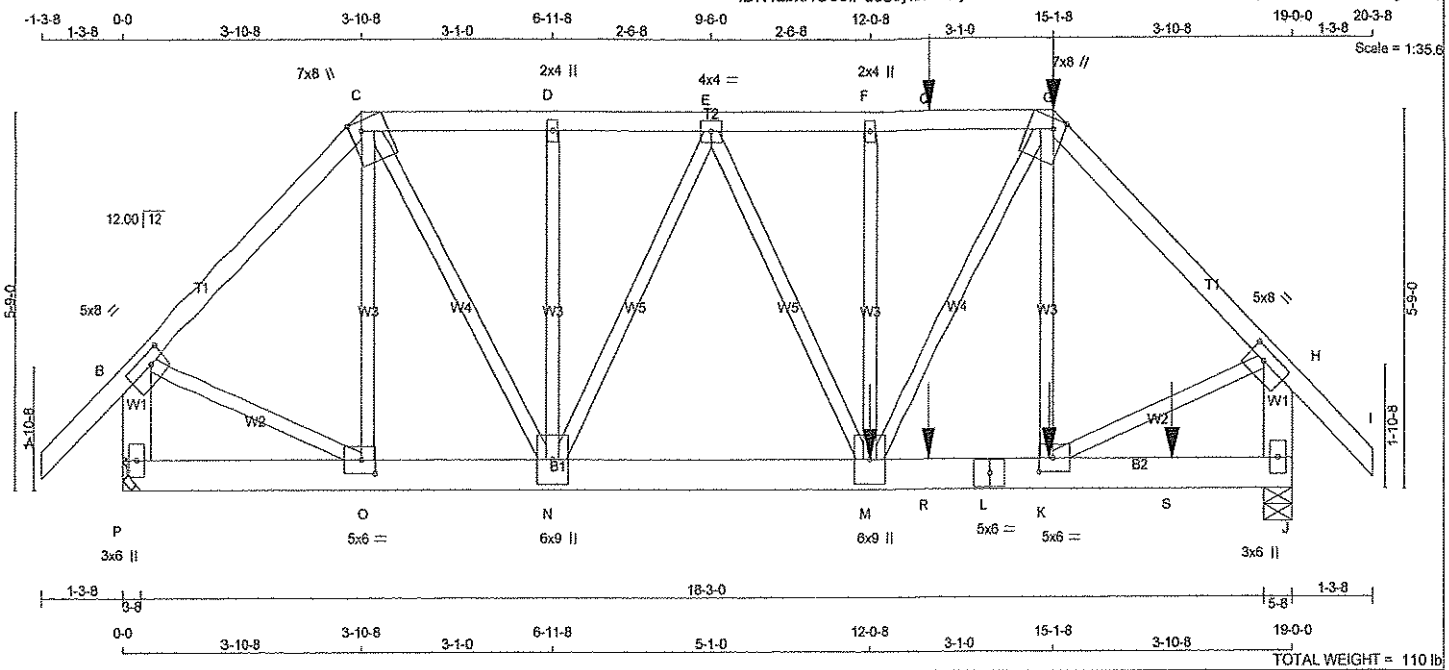

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	7.0	8.0	Edge	2.25
H	TMW-t	MT20	5.0	8.0	2.00	3.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BS-t	MT20	5.0	6.0		
M	BMWWW+t	MT20	6.0	9.0		
N	BMWWW+t	MT20	6.0	9.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 55.6 lbs FACTORED DOWN AT 13-0-12, AND 227.5 lbs FACTORED DOWN AT 15-1-8 ON TOP CHORD, AND 1369.2 lbs FACTORED DOWN AT 12-0-8, 35.0 lbs FACTORED DOWN AT 13-0-12, AND 35.0 lbs FACTORED DOWN AT 15-0-12, AND 37.3 lbs FACTORED DOWN AT 17-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2175	0	2175	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	2727	0	2727	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1666	1145 / 0	258 / 0	0 / 0	0 / 0	262 / 0	0 / 0
J	2088	1437 / 0	323 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.19 (1)	10.00	
B- C	-1788 / 0	-122.2	-122.2	0.44 (1)	4.48	
C- D	-1939 / 0	-122.2	-122.2	0.21 (1)	4.60	
D- E	-1939 / 0	-122.2	-122.2	0.19 (1)	4.63	
E- F	-2467 / 0	-122.2	-122.2	0.21 (1)	4.16	
F- Q	-2467 / 0	-122.2	-122.2	0.28 (1)	4.07	
Q- G	-2467 / 0	-122.2	-122.2	0.28 (1)	4.07	
G- H	-2359 / 0	-122.2	-122.2	0.50 (1)	3.93	
H- I	0 / 60	-122.2	-122.2	0.19 (1)	10.00	
P- B	-2132 / 0	0.0	0.0	0.16 (1)	6.93	
J- H	-2683 / 0	0.0	0.0	0.21 (1)	6.32	
P- O	0 / 0	-28.0	-28.0	0.05 (3)	10.00	
O- N	0 / 1253	-28.0	-28.0	0.20 (1)	10.00	
N- M	0 / 2270	-28.0	-28.0	0.38 (1)	10.00	
M- R	0 / 1654	-28.0	-28.0	0.27 (1)	10.00	
R- L	0 / 1654	-28.0	-28.0	0.27 (1)	10.00	
L- K	0 / 1654	-28.0	-28.0	0.27 (1)	10.00	
K- S	0 / 0	-28.0	-28.0	0.07 (3)	10.00	
S- J	0 / 0	-28.0	-28.0	0.07 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	LC	MAX CSI (LC)
O- C	-336 / 20		0.16 (1)
C- N	0 / 1373		0.34 (1)
N- D	-369 / 0		0.18 (1)
N- E	-774 / 0		0.47 (1)
E- M	0 / 461		0.11 (1)
M- F	-397 / 0		0.19 (1)
M- G	0 / 1627		0.40 (1)
K- G	-403 / 60		0.20 (1)
B- O	0 / 1339		0.33 (1)
K- H	0 / 1767		0.44 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	15-1-8	-227	-227	—	FRONT	VERT	TOTAL
K	15-0-12	-20	-35	—	FRONT	VERT	TOTAL
M	12-0-8	-1369	-1369	—	FRONT	VERT	TOTAL
Q	13-0-12	-56	-56	—	FRONT	VERT	TOTAL
R	13-0-12	-20	-35	—	FRONT	VERT	TOTAL
S	17-0-12	-14	-37	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.50 (G-H:1) , BC=0.38 (M-N:1) , WB=0.47 (E-N:1) , SSI=0.22 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (K) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 46268 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:22 2017 Page 1

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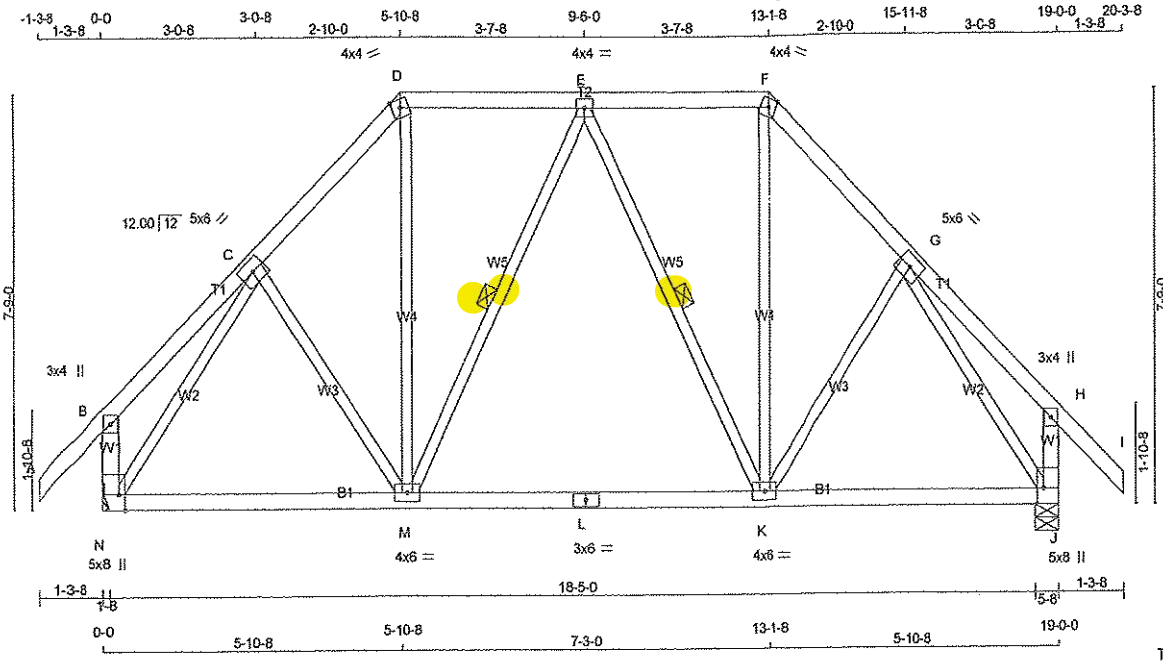


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	Edge	
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1+t	MT20	5.0	8.0	Edge	1.50
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1+t	MT20	5.0	8.0	Edge	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1597	0	1597	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
J	1597	0	1597	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1231	833 / 0	200 / 0	0 / 0	0 / 0	198 / 0	0 / 0
J	1231	833 / 0	200 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

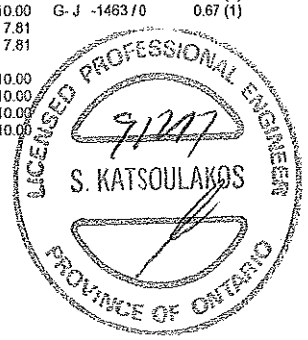
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00		C- M	-15 / 84	0.02 (3)
B- C	0 / 27	-122.2	-122.2	0.16 (1)	10.00		M- D	0 / 479	0.11 (1)
C- D	-1154 / 0	-122.2	-122.2	0.13 (1)	5.79		M- E	-294 / 0	0.14 (1)
D- E	-800 / 0	-122.2	-122.2	0.20 (1)	6.25		E- K	-294 / 0	0.14 (1)
E- F	-800 / 0	-122.2	-122.2	0.20 (1)	6.25		K- F	0 / 479	0.11 (1)
F- G	-1154 / 0	-122.2	-122.2	0.13 (1)	5.79		K- G	-15 / 84	0.02 (3)
G- H	0 / 27	-122.2	-122.2	0.16 (1)	10.00		N- C	-1463 / 0	0.67 (1)
H- I	0 / 60	-122.2	-122.2	0.17 (1)	10.00		G- J	-1463 / 0	0.67 (1)
N- B	-309 / 0	0.0	0.0	0.03 (1)	7.81				
J- H	-309 / 0	0.0	0.0	0.03 (1)	7.81				
N- M	0 / 802	-28.0	-28.0	0.36 (2)	10.00				
M- L	0 / 925	-28.0	-28.0	0.37 (2)	10.00				
L- K	0 / 925	-28.0	-28.0	0.37 (2)	10.00				
K- J	0 / 802	-28.0	-28.0	0.36 (2)	10.00				



DWG NO. TAM 46289-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.20 (D-E:1) , BC=0.37 (K-M:2) , WB=0.67 (G-J:1) , SSI=0.21 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.37 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:23 2017 Page 1

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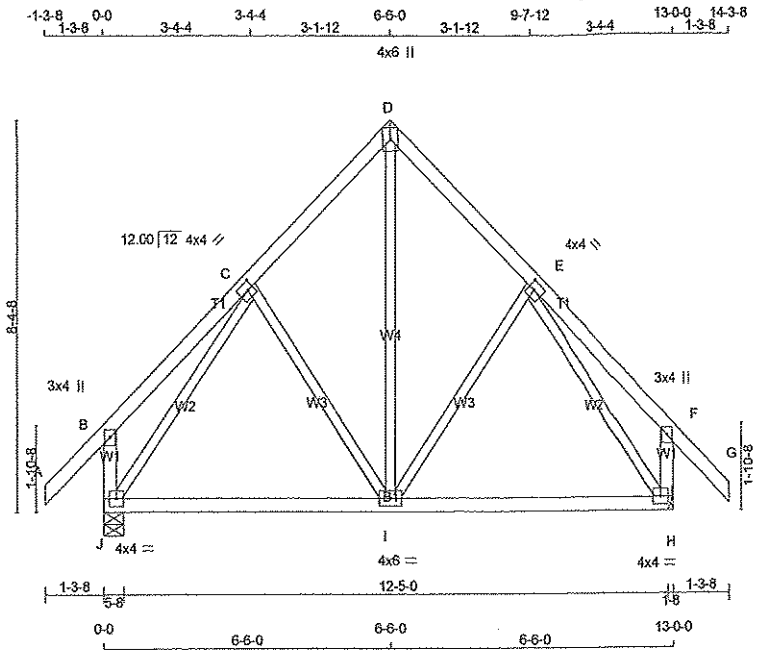

TOTAL WEIGHT = 68 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1147	0	1147	0	0	5-8	5-8
H	1147	0	1147	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	879	604 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	879	604 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I- D	0 / 512	0.12 (1)
B- C	0 / 32	-122.2	-122.2	0.21 (1)	10.00	I- E	-175 / 35	0.10 (1)
C- D	-615 / 0	-122.2	-122.2	0.16 (1)	6.25	C- I	-175 / 35	0.10 (1)
D- E	-615 / 0	-122.2	-122.2	0.16 (1)	6.25	J- C	-914 / 0	0.50 (1)
E- F	0 / 32	-122.2	-122.2	0.21 (1)	10.00	E- H	-914 / 0	0.50 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00			
J- B	-320 / 0	0.0	0.0	0.04 (1)	7.81			
H- F	-320 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 513	-28.0	-28.0	0.39 (2)	10.00			
I- H	0 / 513	-28.0	-28.0	0.39 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.21 (E-F:1) , BC=0.39 (H-I:2) , WB=0.50 (E-H:1) , SSI=0.15 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.35 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4629017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T12S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:23 2017 Page 1

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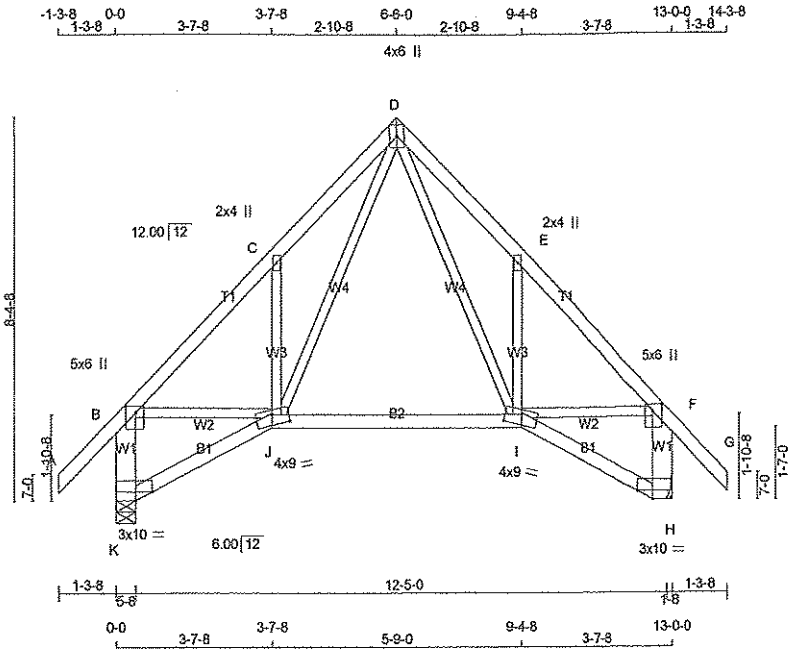

TOTAL WEIGHT = 3 X 69 = 208 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	5.0	6.0	1.75	2.75
H	BVM1-p	MT20	3.0	10.0	Edge	4.50
I	BBWWW-m	MT20	4.0	9.0	2.75	4.50
J	BBWWW-m	MT20	4.0	9.0	2.75	4.50
K	BVM1-p	MT20	3.0	10.0	Edge	4.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1147	0	1147	0	0	5-8	5-8
H	1147	0	1147	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	879	604 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	879	604 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00		D- I	0 / 692	0.16 (1)
B- C	-1044 / 0	-122.2	-122.2	0.19 (1)	5.94		I- E	-486 / 0	0.12 (1)
C- D	-1105 / 0	-122.2	-122.2	0.16 (1)	5.85		J- D	0 / 692	0.16 (1)
D- E	-1105 / 0	-122.2	-122.2	0.16 (1)	5.85		J- C	-486 / 0	0.12 (1)
E- F	-1044 / 0	-122.2	-122.2	0.19 (1)	5.94		B- J	0 / 758	0.17 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00		I- F	0 / 758	0.17 (1)
K- B	-1096 / 0	0.0	0.0	0.09 (1)	7.81				
H- F	-1096 / 0	0.0	0.0	0.09 (1)	7.81				
K- J	0 / 0	-28.0	-28.0	0.11 (2)	10.00				
J- I	0 / 475	-28.0	-28.0	0.32 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.11 (2)	10.00				

DWG NO. TAM4629/-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.19 (B-C:1) , BC=0.32 (I-J:2) , WB=0.17 (F-I:1) , SSI=0.14 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (J) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T13S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:23 2017 Page 1

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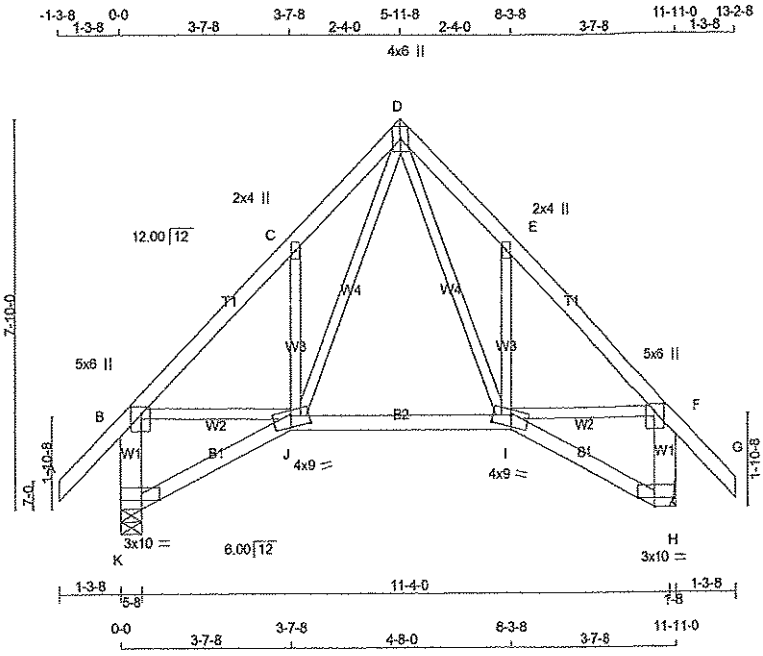

TOTAL WEIGHT = 3 X 65 = 195 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	5.0	6.0	1.75	2.75
H	BVM1-p	MT20	3.0	10.0	Edge	4.50
I	BBWWW-m	MT20	4.0	9.0	2.75	4.50
J	BBWWW-m	MT20	4.0	9.0	2.75	4.50
K	BVM1-p	MT20	3.0	10.0	Edge	4.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1065	0	1065	0	0	5-8	5-8
H	1065	0	1065	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	815	562 / 0	125 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	815	562 / 0	125 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	D- I	0 / 628	0.14 (1)
B- C	-924 / 0	-122.2	-122.2	0.20 (1)	6.20	I- E	-455 / 0	0.11 (1)
C- D	-989 / 0	-122.2	-122.2	0.15 (1)	6.12	J- D	0 / 628	0.14 (1)
D- E	-989 / 0	-122.2	-122.2	0.15 (1)	6.12	J- C	-455 / 0	0.11 (1)
E- F	-924 / 0	-122.2	-122.2	0.20 (1)	6.20	B- J	0 / 671	0.15 (1)
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00	I- F	0 / 671	0.15 (1)
K- B	-1015 / 0	0.0	0.0	0.08 (1)	7.81			
H- F	-1015 / 0	0.0	0.0	0.08 (1)	7.81			
K- J	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
J- I	0 / 440	-28.0	-28.0	0.22 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.11 (2)	10.00			

DWG NO. TAM46292-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.20 (E-F:1) , BC=0.22 (I-J:2) , WB=0.15 (B-J:1) , SSI=0.14 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	T14	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:23 2017 Page 1

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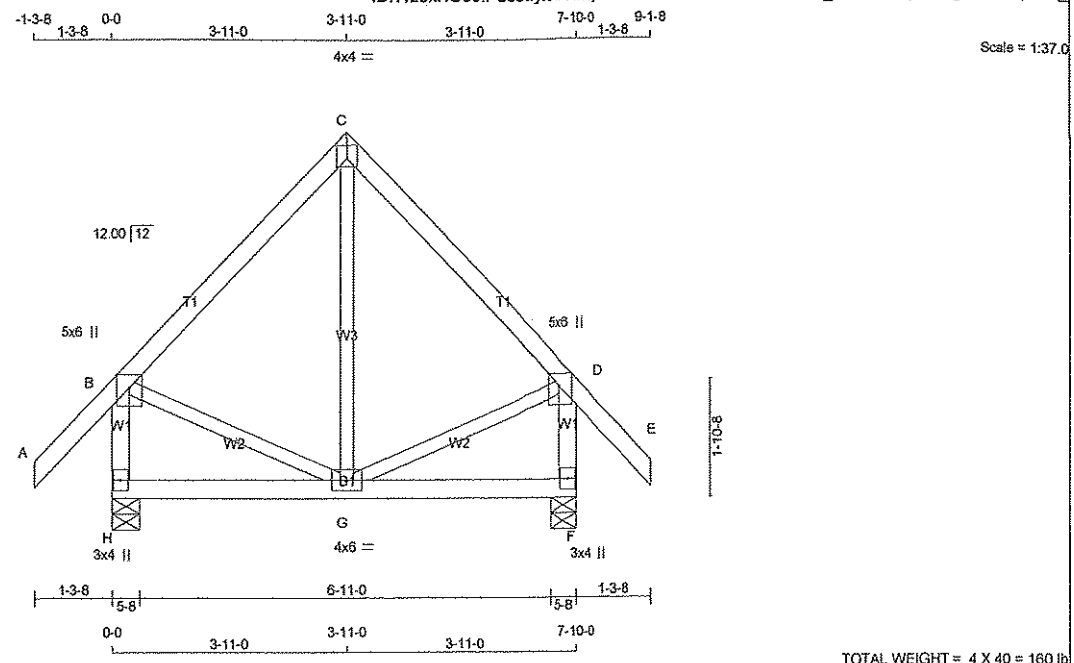

TOTAL WEIGHT = 4 X 40 = 160 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	759	0	759	0	0	5-8	5-8
F	759	0	759	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	575	406 / 0	82 / 0	0 / 0	0 / 0	87 / 0	0 / 0
F	575	406 / 0	82 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00		G- C	-37 / 163	0.04 (3)
B- C	-312 / 0	-122.2	-122.2	0.24 (1)	6.25		B- G	0 / 237	0.05 (1)
C- D	-312 / 0	-122.2	-122.2	0.24 (1)	6.25		G- D	0 / 237	0.05 (1)
D- E	0 / 60	-122.2	-122.2	0.17 (1)	10.00				
H- B	-716 / 0	0.0	0.0	0.08 (1)	7.81				
F- D	-716 / 0	0.0	0.0	0.08 (1)	7.81				
H- G	0 / 0	-28.0	-28.0	0.12 (3)	10.00				
G- F	0 / 0	-28.0	-28.0	0.12 (3)	10.00				

DWG NO. TAM 46293-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24 (C-D:1) , BC=0.12 (F-G:3) , WB=0.05 (B-G:1) , SSI=0.13 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287900	DRWG NO.
287900	G14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:06:16 2017 Page 1

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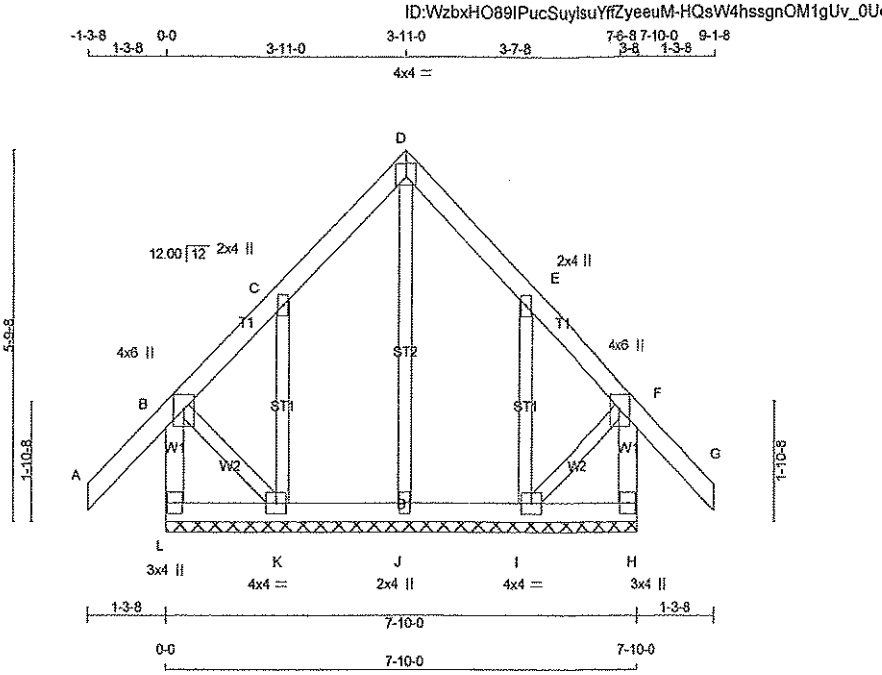

TOTAL WEIGHT = 43 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
L- B	-366 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-63 / 0	-122.2	-122.2	0.16 (1)	6.25
C- D	-31 / 0	-122.2	-122.2	0.07 (1)	6.25
D- E	-31 / 0	-122.2	-122.2	0.07 (1)	6.25
E- F	-63 / 0	-122.2	-122.2	0.16 (1)	6.25
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00
H- F	-366 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 0	-28.0	-28.0	0.03 (3)	10.00
K- J	0 / 14	-28.0	-28.0	0.03 (2)	10.00
J- I	0 / 14	-28.0	-28.0	0.03 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-197 / 0	0.10 (1)
K- C	-201 / 0	0.05 (1)
I- E	-201 / 0	0.05 (1)
B- K	0 / 26	0.01 (1)
I- F	0 / 26	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1) , BC=0.03 (J-K:2) , WB=0.10 (D-J:1) , SSI=0.09 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (D) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 46284-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288238	T15	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 18 15:09:16 2017 Page 1

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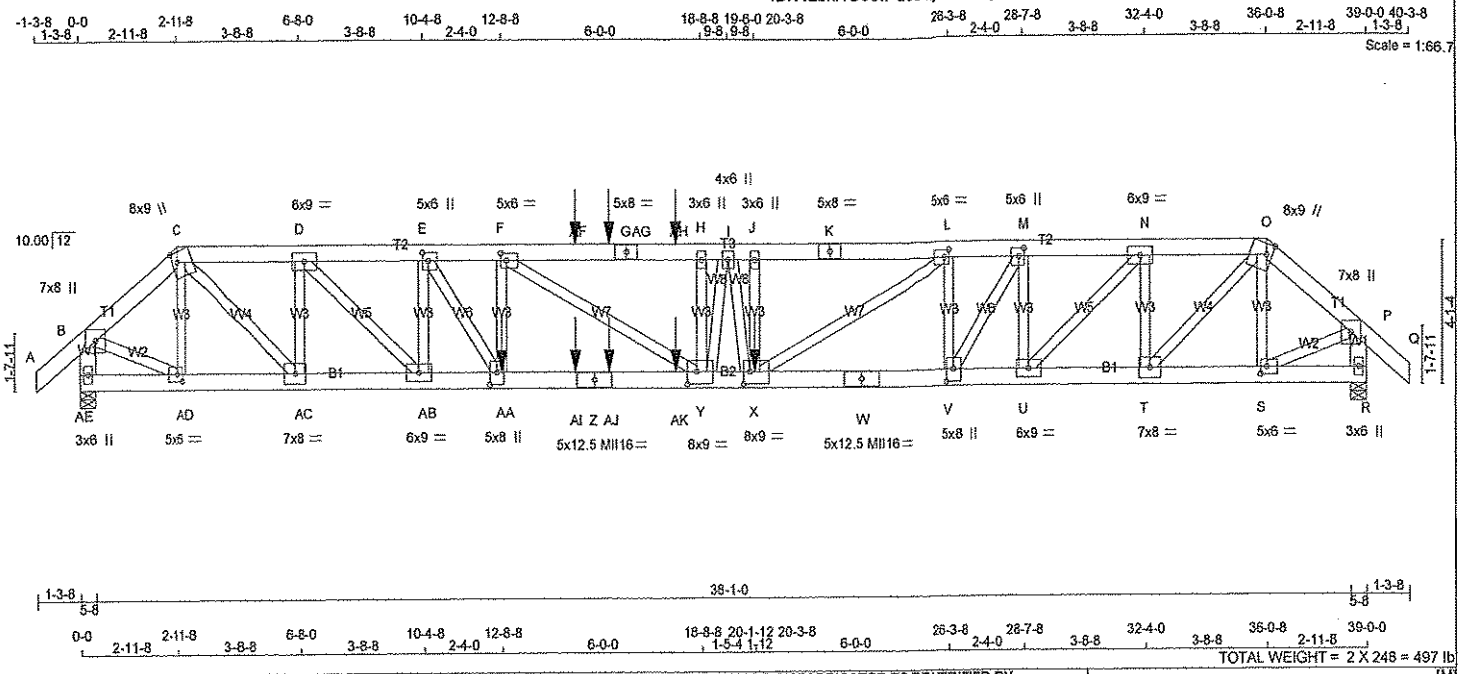

TOTAL WEIGHT = 2 X 248 = 497 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	1650F 1.5E	SPF
G - K	2x6	DRY	1650F 1.5E	SPF
K - O	2x6	DRY	1650F 1.5E	SPF
O - Q	2x6	DRY	No.2	SPF
AE- B	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
AE- Z	2x6	DRY	1650F 1.5E	SPF
Z - W	2x6	DRY	1650F 1.5E	SPF
W - R	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- G	2	12	SIDE(0.0)
G- K	2	12	SIDE(0.0)
K- O	2	12	TOP
O- Q	2	12	TOP
AE- B	2	12	TOP
R- P	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AE- Z	2	12	SIDE(183.1)
Z- W	2	12	SIDE(183.1)
W- R	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
F- AA	1	4	SIDE(433.7)
J- X	1	6	SIDE(295.8)
X- I	1	6	SIDE(294.4)
2x4	1	6	
L- V	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	7.0	8.0		
C	TTWW+m	MT20	8.0	9.0	3.50	2.00
D	TMWW-t	MT20	6.0	9.0		
E	TMWW+t	MT20	5.0	6.0	2.75	2.00
F	TMWW-t	MT20	5.0	6.0	2.50	2.00
G	TS-t	MT20	5.0	8.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMW+w	MT20	3.0	6.0		
K	TS-t	MT20	5.0	8.0		
L	TMWW-t	MT20	5.0	6.0	2.50	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AE	6448	0	6448	0	0	5-8	5-8
R	5652	0	5652	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AE	4987	3344 / 0	829 / 0	0 / 0	0 / 0	815 / 0	0 / 0
R	4373	2929 / 0	728 / 0	0 / 0	0 / 0	715 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AE, R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.05 (1)		10.00
B- C	-6618 / 0	-122.2	-122.2	0.11 (1)		4.58
C- D	-10551 / 0	-122.2	-122.2	0.13 (1)		4.27
D- E	-15549 / 0	-122.2	-122.2	0.25 (1)		3.53
E- F	-18554 / 0	-122.2	-122.2	0.31 (1)		3.21
F-AF	-20763 / 0	-122.2	-122.2	0.56 (1)		2.87
AF-AG	-20763 / 0	-122.2	-122.2	0.56 (1)		2.87
AG- G	-20763 / 0	-122.2	-122.2	0.56 (1)		2.87
G-AH	-20763 / 0	-122.2	-122.2	0.56 (1)		2.87
AH- H	-20763 / 0	-122.2	-122.2	0.56 (1)		2.87
H- I	-20763 / 0	-122.2	-122.2	0.37 (1)		2.99
I- J	-20762 / 0	-122.2	-122.2	0.32 (1)		3.05
J- K	-20762 / 0	-122.2	-122.2	0.50 (1)		2.93
K- L	-20762 / 0	-122.2	-122.2	0.50 (1)		2.93
L- M	-15819 / 0	-122.2	-122.2	0.23 (1)		3.52
M- N	-13305 / 0	-122.2	-122.2	0.19 (1)		3.83
N- O	-9102 / 0	-122.2	-122.2	0.10 (1)		4.55
O- P	-5746 / 0	-122.2	-122.2	0.09 (1)		4.66
P- Q	0 / 56	-122.2	-122.2	0.05 (1)		10.00
AE- B	-6441 / 0	0.0	0.0	0.23 (1)		5.85
R- P	-5640 / 0	0.0	0.0	0.20 (1)		6.20

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
AD- C	-1629 / 0	0.14 (1)
AC- C	0 / 7763	0.69 (1)
AC- D	-5360 / 0	0.46 (1)
D-AB	0 / 7008	0.62 (1)
AB- E	-4808 / 0	0.41 (1)
E-AA	0 / 5575	0.49 (1)
AA- F	-2212 / 0	0.19 (1)
F- Y	0 / 2585	0.23 (1)
Y- H	-1015 / 0	0.09 (1)
X- J	-238 / 48	0.02 (1)
X- L	0 / 5784	0.51 (1)
V- L	-3692 / 0	0.32 (1)
V- M	0 / 4664	0.41 (1)
U- M	-4122 / 0	0.35 (1)
U- N	0 / 5894	0.52 (1)
T- N	-4569 / 0	0.39 (1)
T- O	0 / 6656	0.59 (1)
S- O	-1405 / 0	0.12 (1)
B-AD	0 / 5370	0.47 (1)
S- P	0 / 4663	0.41 (1)
Y- I	0 / 51	0.00 (1)
I- X	-65 / 62	0.01 (3)

AE-AD	0 / 0	-28.0	-28.0	0.03 (1)	10.00
AD-AC	0 / 4989	-28.0	-28.0	0.23 (1)	10.00
AC-AB	0 / 10551	-28.0	-28.0	0.47 (1)	10.00
AB-AA	0 / 15549	-28.0	-28.0	0.68 (1)	10.00
AA-AI	0 / 18554	-28.0	-28.0	0.85 (1)	10.00
AI- Z	0 / 18554	-28.0	-28.0	0.85 (1)	10.00
Z-AJ	0 / 18554	-28.0	-28.0	0.85 (1)	10.00
AJ-AK	0 / 18554	-28.0	-28.0	0.85 (1)	10.00
AK- Y	0 / 18554	-28.0	-28.0	0.85 (1)	10.00
Y- X	0 / 20751	-28.0	-28.0	0.94 (1)	10.00
X- W	0 / 15819	-28.0	-28.0	0.74 (1)	10.00
W- V	0 / 15819	-28.0	-28.0	0.74 (1)	10.00
V- U	0 / 13305	-28.0	-28.0	0.58 (1)	10.00
U- T	0 / 9101	-28.0	-28.0	0.41 (1)	10.00
T- S	0 / 4332	-28.0	-28.0	0.20 (1)	10.00
S- R	0 / 0	-28.0	-28.0	0.03 (1)	10.00

LICENSED PROFESSIONAL ENGINEER
9-2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 765 (0.61")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 505 (0.93")

CSI: TC=0.56 (F-H:1) , BC=0.94 (X-Y:1) , WB=0.69 (C-AC:1) , SSI=0.21 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.79 (Z) (INPUT = 1.00)

DWG NO. TAM47523-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
288238	T15	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	TMWW+t	MT20	5.0	6.0	2.75	2.00
N	TMWW-t	MT20	6.0	9.0		
O	TTWW+m	MT20	8.0	9.0	3.50	2.00
P	TMVW+p	MT20	7.0	8.0		
R	BMV1+p	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.25
T	BMWW-t	MT20	7.0	8.0		
U	BMWW-t	MT20	6.0	9.0		
V	BMWW+t	MT20	5.0	8.0	4.25	2.50
W	BS-t	MII16	5.0	12.5		
X	BMWWW-t	MT20	8.0	9.0	4.25	2.25
Y	BMWWW-t	MT20	8.0	9.0	4.25	3.25
Z	BS-t	MII16	5.0	12.5		
AA	BMWW+t	MT20	5.0	8.0	4.25	2.50
AB	BMWW-t	MT20	6.0	9.0		
AC	BMWW-t	MT20	7.0	8.0		
AD	BMWW-t	MT20	5.0	6.0	2.50	2.25
AE	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 147.1 lbs FACTORED DOWN AT 14-11-4, AND 147.1 lbs FACTORED DOWN AT 15-11-4, AND 147.1 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 2364.3 lbs FACTORED DOWN AT 12-8-8, 69.9 lbs FACTORED DOWN AT 14-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, AND 69.9 lbs FACTORED DOWN AT 17-11-4, AND 2864.4 lbs FACTORED DOWN AT 20-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	20-3-8	-2964	-2964	—	FRONT	VERT	TOTAL
AA	12-8-8	-2364	-2364	—	FRONT	VERT	TOTAL
AF	14-11-4	-147	-147	—	FRONT	VERT	TOTAL
AG	15-11-4	-147	-147	—	FRONT	VERT	TOTAL
AH	17-11-4	-147	-147	—	FRONT	VERT	TOTAL
AI	14-11-4	-40	-70	—	FRONT	VERT	TOTAL
AJ	15-11-4	-40	-70	—	FRONT	VERT	TOTAL
AK	17-11-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9 2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4T523-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
288238	T15Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 18 15:09:17 2017 Page 1

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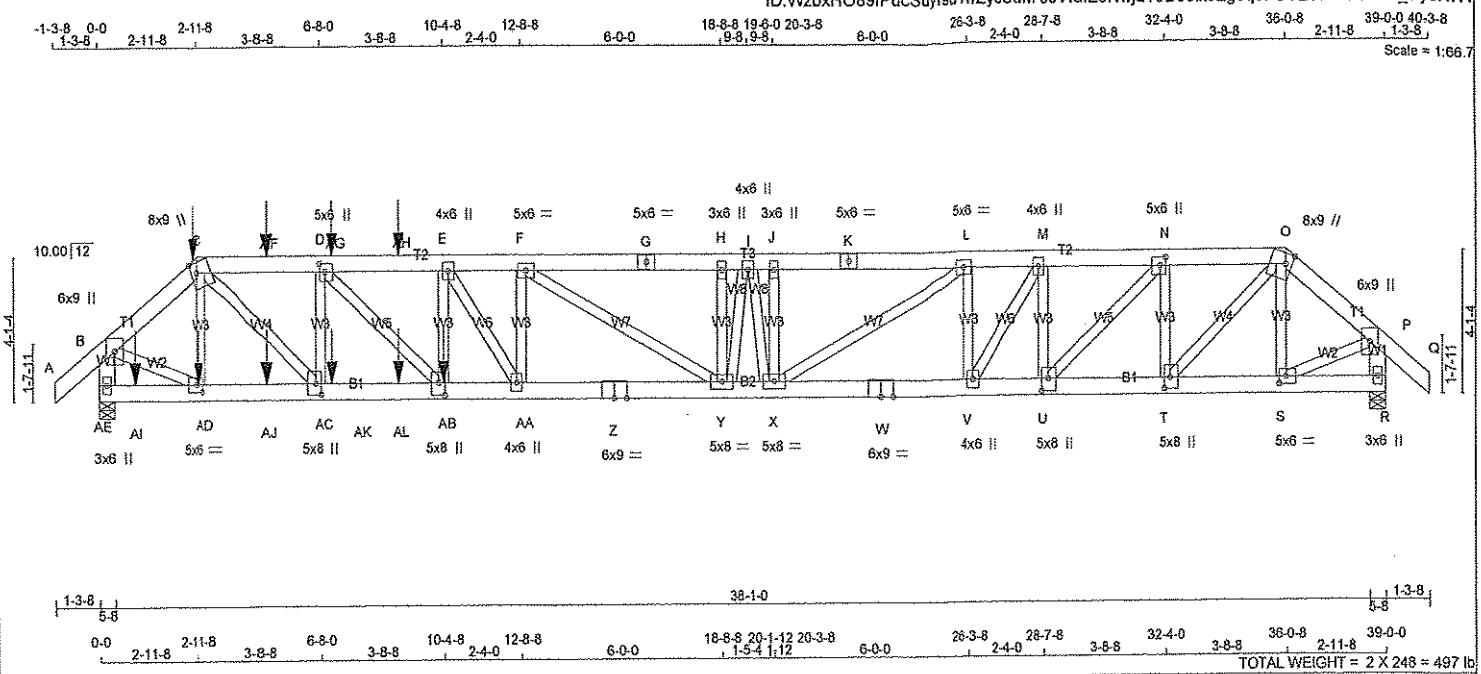


TOTAL WEIGHT = 2 X 248 = 497 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	1650F 1.5E	SPF
G - K	2x6	DRY	1650F 1.5E	SPF
K - O	2x6	DRY	1650F 1.5E	SPF
O - Q	2x6	DRY	No.2	SPF
AE- B	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
AE- Z	2x6	DRY	1650F 1.5E	SPF
Z - W	2x6	DRY	1650F 1.5E	SPF
W - R	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(183.1)
G- K	2	12	TOP
K- O	2	12	TOP
O- Q	2	12	TOP
AE- B	2	12	TOP
R- P	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AE- Z	2	12	SIDE(183.1)
Z- W	2	12	TOP
W- R	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
E- AB	1	3	SIDE(578.9)
M- U	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	9.0	3.50	2.25
D	TMWW+t	MT20	5.0	6.0	2.75	2.00
E, I, M						
E	TMWW+t	MT20	4.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	3.0	6.0		
J	TMW+w	MT20	3.0	6.0		
K	TS-t	MT20	5.0	6.0		
L	TMWW-t	MT20	5.0	6.0		
N	TMWW+t	MT20	5.0	6.0	2.75	2.00
O	TTWW+m	MT20	8.0	9.0	3.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AE	6104	0	6104	0	0	5-8	5-8
R	4013	0	4013	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AE	4747	3134 / 0	817 / 0	0 / 0	0 / 0	795 / 0	0 / 0
R	3115	2069 / 0	529 / 0	0 / 0	0 / 0	516 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AE, R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 56	-122.2	-122.2	0.05 (1)		10.00
B- C	-6208 / 0	-122.2	-122.2	0.10 (1)		4.71
C-AF	-9427 / 0	-122.2	-122.2	0.13 (1)		4.47
AF- D	-9427 / 0	-122.2	-122.2	0.13 (1)		4.47
D-AG	-13125 / 0	-122.2	-122.2	0.23 (1)		3.81
AG-AH	-13125 / 0	-122.2	-122.2	0.23 (1)		3.81
AH- E	-13125 / 0	-122.2	-122.2	0.23 (1)		3.81
E- F	-13461 / 0	-122.2	-122.2	0.17 (1)		3.83
F- G	-12895 / 0	-122.2	-122.2	0.24 (1)		3.85
G- H	-12895 / 0	-122.2	-122.2	0.24 (1)		3.85
H- I	-12895 / 0	-122.2	-122.2	0.15 (1)		3.91
I- J	-12498 / 0	-122.2	-122.2	0.15 (1)		3.97
J- K	-12498 / 0	-122.2	-122.2	0.23 (1)		3.91
K- L	-12498 / 0	-122.2	-122.2	0.23 (1)		3.91
L- M	-10097 / 0	-122.2	-122.2	0.12 (1)		4.35
M- N	-8671 / 0	-122.2	-122.2	0.10 (1)		4.64
N- O	-6120 / 0	-122.2	-122.2	0.07 (1)		5.35
O- P	-3955 / 0	-122.2	-122.2	0.07 (1)		5.65
P- Q	0 / 56	-122.2	-122.2	0.05 (1)		10.00
AE- B	-6065 / 0	0.0	0.0	0.22 (1)		6.00
R- P	-3993 / 0	0.0	0.0	0.14 (1)		7.11
AE-AI	0 / 0	-28.0	-28.0	0.03 (2)		10.00
AI-AD	0 / 0	-28.0	-28.0	0.03 (2)		10.00
AD-AJ	0 / 4683	-28.0	-28.0	0.21 (1)		10.00
AJ-AC	0 / 4683	-28.0	-28.0	0.21 (1)		10.00
AC-AK	0 / 9427	-28.0	-28.0	0.46 (1)		10.00
AK-AL	0 / 9427	-28.0	-28.0	0.46 (1)		10.00
AL-AB	0 / 9427	-28.0	-28.0	0.46 (1)		10.00
AB-AA	0 / 13125	-28.0	-28.0	0.61 (1)		10.00
AA- Z	0 / 13461	-28.0	-28.0	0.60 (1)		10.00
Z- Y	0 / 13461	-28.0	-28.0	0.60 (1)		10.00
Y- X	0 / 12645	-28.0	-28.0	0.54 (1)		10.00
X- W	0 / 10097	-28.0	-28.0	0.46 (1)		10.00
W- V	0 / 10097	-28.0	-28.0	0.46 (1)		10.00
V- U	0 / 8671	-28.0	-28.0	0.37 (1)		10.00
U- T	0 / 6120	-28.0	-28.0	0.28 (1)		10.00
T- S	0 / 2983	-28.0	-28.0	0.14 (1)		10.00
S- R	0 / 0	-28.0	-28.0	0.02 (2)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AD- C	-1476 / 0	0.13 (1)
AC- D	0 / 8621	0.59 (1)
AC- D	-4400 / 0	0.38 (1)
D-AB	0 / 5186	0.46 (1)
AB- E	-772 / 0	0.07 (1)
E-AA	0 / 622	0.05 (1)
AA- F	-308 / 55	0.03 (1)
F- Y	-661 / 0	0.21 (1)
Y- H	-543 / 0	0.05 (1)
X- J	-572 / 0	0.05 (1)
X- L	0 / 2809	0.25 (1)
V- L	-2079 / 0	0.18 (1)
V- M	0 / 2647	0.23 (1)
U- M	-2454 / 0	0.21 (1)
U- N	0 / 3576	0.32 (1)
T- N	-2973 / 0	0.25 (1)
T- O	0 / 4378	0.39 (1)
S- O	-937 / 0	0.08 (1)
B-AD	0 / 5038	0.45 (1)
S- P	0 / 3210	0.28 (1)
Y- I	0 / 1029	0.09 (1)
I- X	-814 / 0	0.07 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 773 (0.61")

CSI: TC=0.24 (F-H:1) , BC=0.61 (AA-AB:1) , WB=0.59 (C-AC:1) , SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

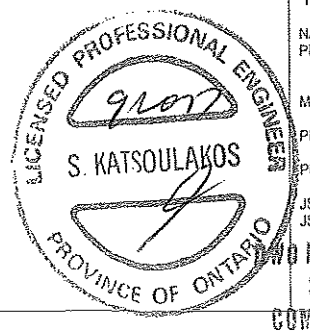
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.88 (Z) (INPUT = 1.00)



PO No. TAM47524-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	TMVW+p	MT20	6.0	9.0		
R	BMV1+p	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMWW+t	MT20	5.0	8.0	3.75	2.00
U	BMWW+t	MT20	5.0	8.0	4.25	2.25
V	BMWW+t	MT20	4.0	6.0		
W	BS-t	MT20	6.0	9.0		
X	BMWWW-t	MT20	5.0	8.0		
Y	BMWWW-t	MT20	5.0	8.0		
Z	BS-t	MT20	6.0	9.0		
AA	BMWW+t	MT20	4.0	6.0		
AB	BMWW+t	MT20	5.0	8.0	4.25	2.25
AC	BMWW+t	MT20	5.0	8.0	3.75	2.00
AD	BMWW-t	MT20	5.0	6.0	2.50	2.50
AE	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 2-11-8, 150.6 lbs FACTORED DOWN AT 2-11-8, 147.1 lbs FACTORED DOWN AT 5-0-4, AND 147.1 lbs FACTORED DOWN AT 7-0-4, AND 147.1 lbs FACTORED DOWN AT 9-0-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 69.9 lbs FACTORED DOWN AT 7-0-4, AND 69.9 lbs FACTORED DOWN AT 9-0-4, AND 2920.8 lbs FACTORED DOWN AT 10-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-13	-14	---	FRONT	VERT	DEAD
C	2-11-8	-151	-151	---	BACK	VERT	TOTAL
C	2-11-8	-183	-183	---	FRONT	VERT	SNOW
AB	10-4-8	-2921	-2921	---	BACK	VERT	TOTAL
AD	3-0-4	-40	-70	---	BACK	VERT	TOTAL
AF	5-0-4	-147	-147	---	BACK	VERT	TOTAL
AG	7-0-4	-147	-147	---	BACK	VERT	TOTAL
AH	9-0-4	-147	-147	---	BACK	VERT	TOTAL
AI	1-0-12	-40	-70	---	BACK	VERT	TOTAL
AJ	5-0-4	-40	-70	---	BACK	VERT	TOTAL
AK	7-0-4	-40	-70	---	BACK	VERT	TOTAL
AL	9-0-4	-40	-70	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47524-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T15S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:04 2017 Page 1

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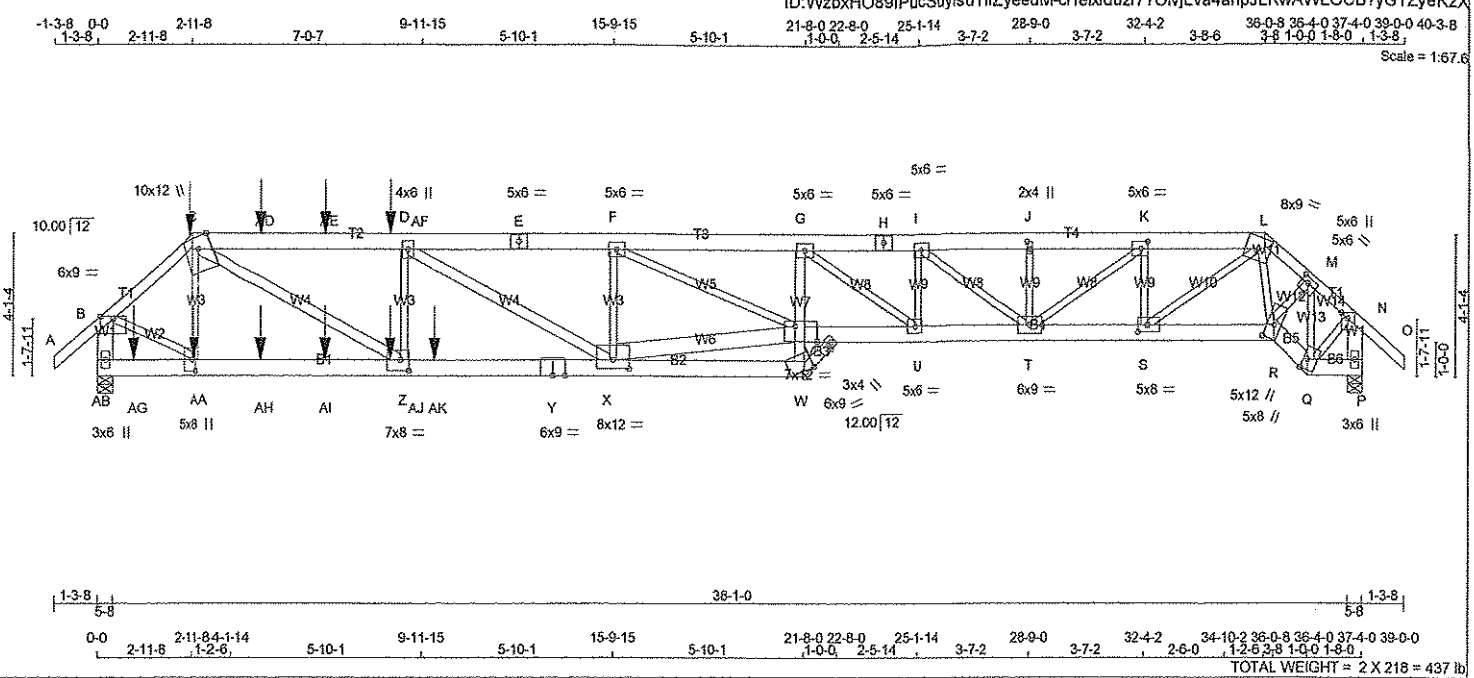

TOTAL WEIGHT = 2 X 218 = 437 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF
AB- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AB- Y	2x6	DRY	1650F 1.5E	SPF
Y - W	2x6	DRY	1650F 1.5E	SPF
V - R	2x6	DRY	1650F 1.5E	SPF
R - Q	2x6	DRY	1650F 1.5E	SPF
Q - P	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - Z	2x4	DRY	No.2	SPF
D - X	2x4	DRY	No.2	SPF
W - G	2x4	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
L- O	1	12	TOP
C- E	2	12	SIDE(61.0)
E- H	2	12	TOP
H- L	2	12	TOP
AB- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AB- Y	2	12	SIDE(183.1)
Y- W	2	12	TOP
V- R	2	12	TOP
R- Q	2	12	TOP
Q- P	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	10.0	12.0	Edge	4.50
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F, G, I						
F	TMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AB	6117	0	6117	0	0	5-8	5-8
P	4069	0	4069	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	4767	3130 / 0	832 / 0	0 / 0	0 / 0	805 / 0	0 / 0
P	3156	2099 / 0	535 / 0	0 / 0	0 / 0	522 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.09 (1)	10.00
B- C	-6039 / 0	-122.2	-122.2	0.24 (1)	3.81
C-AD	-13127 / 0	-122.2	-122.2	0.69 (1)	2.91
AD-AE	-13127 / 0	-122.2	-122.2	0.69 (1)	2.91
AE-AF	-13127 / 0	-122.2	-122.2	0.69 (1)	2.91
AF- D	-13127 / 0	-122.2	-122.2	0.69 (1)	2.91
D- E	-13905 / 0	-122.2	-122.2	0.57 (1)	2.98
E- F	-13905 / 0	-122.2	-122.2	0.57 (1)	2.98
F- G	-16855 / 0	-122.2	-122.2	0.75 (1)	2.52
G- H	-14811 / 0	-122.2	-122.2	0.52 (1)	2.89
H- I	-14811 / 0	-122.2	-122.2	0.52 (1)	2.89
I- J	-11975 / 0	-122.2	-122.2	0.35 (1)	3.37
J- K	-11975 / 0	-122.2	-122.2	0.35 (1)	3.37
K- L	-8474 / 0	-122.2	-122.2	0.21 (1)	4.06
L- M	-5232 / 0	-122.2	-122.2	0.15 (1)	4.12
M- N	-3173 / 0	-122.2	-122.2	0.12 (1)	5.10
N- O	0 / 54	-122.2	-122.2	0.09 (1)	10.00
AB- B	-6026 / 0	0.0	0.0	0.22 (1)	6.03
P- N	-4045 / 0	0.0	0.0	0.15 (1)	7.07
AB-AG	0 / 0	-28.0	-28.0	0.04 (3)	10.00
AG-AA	0 / 0	-28.0	-28.0	0.04 (3)	10.00
AA-AH	0 / 4581	-28.0	-28.0	0.41 (1)	10.00
AH-AI	0 / 4581	-28.0	-28.0	0.41 (1)	10.00
AI-AJ	0 / 4581	-28.0	-28.0	0.41 (1)	10.00
AJ- Z	0 / 4581	-28.0	-28.0	0.41 (1)	10.00
Z-AK	0 / 13126	-28.0	-28.0	0.98 (1)	10.00
AK- Y	0 / 13126	-28.0	-28.0	0.98 (1)	10.00
Y- X	0 / 13126	-28.0	-28.0	0.98 (1)	10.00
X- W	0 / 499	-28.0	-28.0	0.14 (1)	10.00
V- U	0 / 17048	-28.0	-28.0	0.76 (1)	10.00
U- T	0 / 14811	-28.0	-28.0	0.67 (1)	10.00
T- S	0 / 8474	-28.0	-28.0	0.39 (1)	10.00
S- R	0 / 4095	-28.0	-28.0	0.19 (1)	10.00
R- Q	0 / 3127	-28.0	-28.0	0.13 (1)	10.00
Q- P	0 / 0	-28.0	-28.0	0.00 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- C	-1790 / 0	0.21 (1)
C- Z	0 / 9817	0.87 (1)
Z- D	-1640 / 0	0.20 (1)
D- X	0 / 902	0.08 (1)
X- F	-2143 / 0	0.26 (1)
W- V	0 / 94	0.38 (1)
V- G	0 / 1217	0.49 (1)
R- M	0 / 3117	0.39 (1)
Q- M	-3849 / 0	0.35 (1)
B-AA	0 / 4986	0.62 (1)
Q- N	0 / 2902	0.36 (1)
X- V	0 / 13701	0.69 (1)
F- V	0 / 3148	0.39 (1)
L- R	-151 / 36	0.01 (1)
T- J	-403 / 0	0.04 (1)
U- I	0 / 1569	0.19 (1)
S- K	-3086 / 0	0.28 (1)
G- U	-2767 / 0	0.25 (1)
I- T	-352270	0.50 (1)
T- K	0 / 14349	0.54 (1)
S- J	0 / 5400	0.67 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 718 (0.65")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 473 (0.99")

CSI: TC=0.75 (F-G:1) , BC=0.98 (X-Z:1) , WB=0.89 (V-X:1) , SSI=0.95 (X-Z:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.92 (V) (INPUT = 1.00)

DWG NO. TAM 46296-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TS-t	MT20	5.0	6.0		
J	TMW+w	MT20	2.0	4.0	2.50	1.00
K	TMWW-t	MT20	5.0	6.0	2.50	2.50
L	TTWW-m	MT20	8.0	9.0		
M	TMWW-t	MT20	5.0	6.0	2.00	2.50
N	TMW+p	MT20	5.0	6.0	2.00	2.25
P	BMV1+p	MT20	3.0	6.0		
Q	BBWW+m	MT20	5.0	8.0	3.50	1.50
R	BBWW+m	MT20	5.0	12.0	5.00	2.50
S	BMWW-t	MT20	5.0	8.0	2.50	3.50
T	BMWWW-t	MT20	6.0	9.0		
U	BMWW-t	MT20	5.0	6.0		
V	BWMWW"-I	MT20	7.0	12.0	5.25	8.00
W						
W						
W	BBW-m	MT20	6.0	9.0	Edge	
X	BMWWW-t	MT20	8.0	12.0	3.00	6.00
Y	BS-t	MT20	6.0	9.0		
Z	BMWW-t	MT20	7.0	8.0	3.75	3.00
AA	BMWW+t	MT20	5.0	8.0	4.00	1.50
AB	BMV1+p	MT20	3.0	6.0		
AC	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 150.6 lbs FACTORED DOWN AT 2-11-8, 147.1 lbs FACTORED DOWN AT 5-0-4, AND 147.1 lbs FACTORED DOWN AT 7-0-4, AND 147.1 lbs FACTORED DOWN AT 9-0-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 69.9 lbs FACTORED DOWN AT 7-0-4, AND 69.9 lbs FACTORED DOWN AT 9-0-4, AND 3197.8 lbs FACTORED DOWN AT 10-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-151	-151	---	BACK	VERT	TOTAL
AA	3-0-4	-40	-70	---	BACK	VERT	TOTAL
AD	5-0-4	-147	-147	---	BACK	VERT	TOTAL
AE	7-0-4	-147	-147	---	BACK	VERT	TOTAL
AF	9-0-4	-147	-147	---	BACK	VERT	TOTAL
AG	1-0-12	-40	-70	---	BACK	VERT	TOTAL
AH	5-0-4	-40	-70	---	BACK	VERT	TOTAL
AI	7-0-4	-40	-70	---	BACK	VERT	TOTAL
AJ	9-0-4	-40	-70	---	BACK	VERT	TOTAL
AK	10-4-8	-3198	-3198	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 46296-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:05 2017 Page 1

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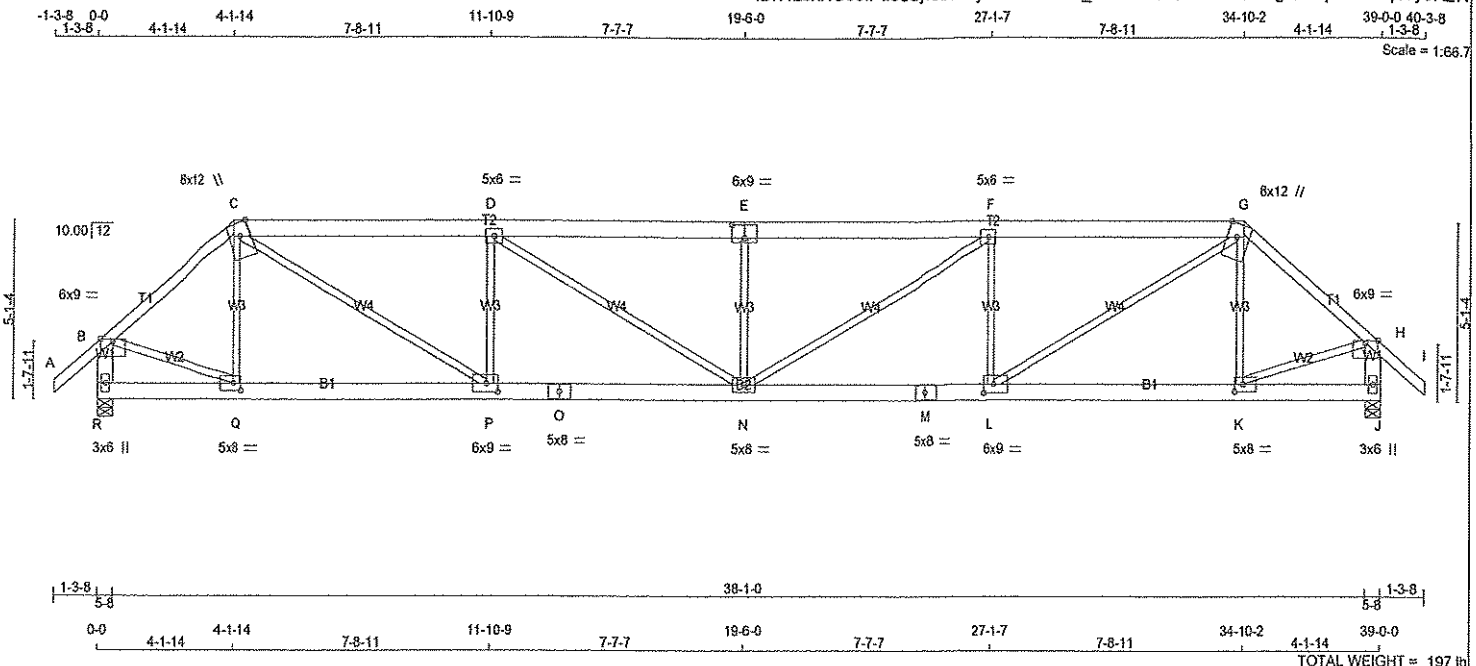

TOTAL WEIGHT = 197 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	8.0	12.0	Edge	3.50
D	TMWW-t	MT20	5.0	6.0		
E	TSW-t	MT20	6.0	9.0	4.50	4.50
F	TMWW-t	MT20	5.0	6.0		
G	TTWW+m	MT20	8.0	12.0	Edge	3.50
H	TMVW-p	MT20	6.0	9.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	8.0	2.50	3.00
L	BMWW-t	MT20	6.0	9.0	3.00	4.00
M	BS-t	MT20	5.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	8.0		
P	BMWW-t	MT20	6.0	9.0	3.00	4.00
Q	BMWW-t	MT20	5.0	8.0	2.50	3.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3098	0	3098	0	0	5-8	5-8
J	3098	0	3098	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
J	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
B- C	-3085 / 0	-122.2	-122.2	0.58 (1)	3.45	
C- D	-5368 / 0	-122.2	-122.2	0.75 (1)	3.13	
D- E	-6252 / 0	-122.2	-122.2	0.84 (1)	2.83	
E- F	-6252 / 0	-122.2	-122.2	0.84 (1)	2.83	
F- G	-5368 / 0	-122.2	-122.2	0.75 (1)	3.13	
G- H	-3085 / 0	-122.2	-122.2	0.58 (1)	3.45	
H- I	0 / 54	-122.2	-122.2	0.17 (1)	10.00	
R- B	-3073 / 0	0.0	0.0	0.21 (1)	6.02	
J- H	-3073 / 0	0.0	0.0	0.21 (1)	6.02	
R- Q	0 / 0	-28.0	-28.0	0.14 (2)	10.00	
Q- P	0 / 2365	-28.0	-28.0	0.38 (1)	10.00	
P- O	0 / 5368	-28.0	-28.0	0.73 (1)	10.00	
O- N	0 / 5368	-28.0	-28.0	0.73 (1)	10.00	
N- M	0 / 5368	-28.0	-28.0	0.73 (1)	10.00	
M- L	0 / 5368	-28.0	-28.0	0.73 (1)	10.00	
L- K	0 / 2365	-28.0	-28.0	0.38 (1)	10.00	
K- J	0 / 0	-28.0	-28.0	0.14 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-469 / 70	0.16 (1)
C- P	0 / 3508	0.79 (1)
P- D	-1613 / 0	0.54 (1)
D- N	0 / 1035	0.23 (1)
N- E	-864 / 0	0.29 (1)
N- F	0 / 1035	0.23 (1)
L- F	-1613 / 0	0.54 (1)
L- G	0 / 3508	0.79 (1)
K- G	-469 / 70	0.16 (1)
B- Q	0 / 2471	0.56 (1)
K- H	0 / 2471	0.56 (1)

LICENSED PROFESSIONAL ENGINEER
9/12/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T1M46297-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.41")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 743 (0.63")

CSI: TC=0.84 (D-E:1) , BC=0.73 (L-N:1) , WB=0.79 (C-P:1) , SSI=0.34 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.88 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T16S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:05 2017 Page 1

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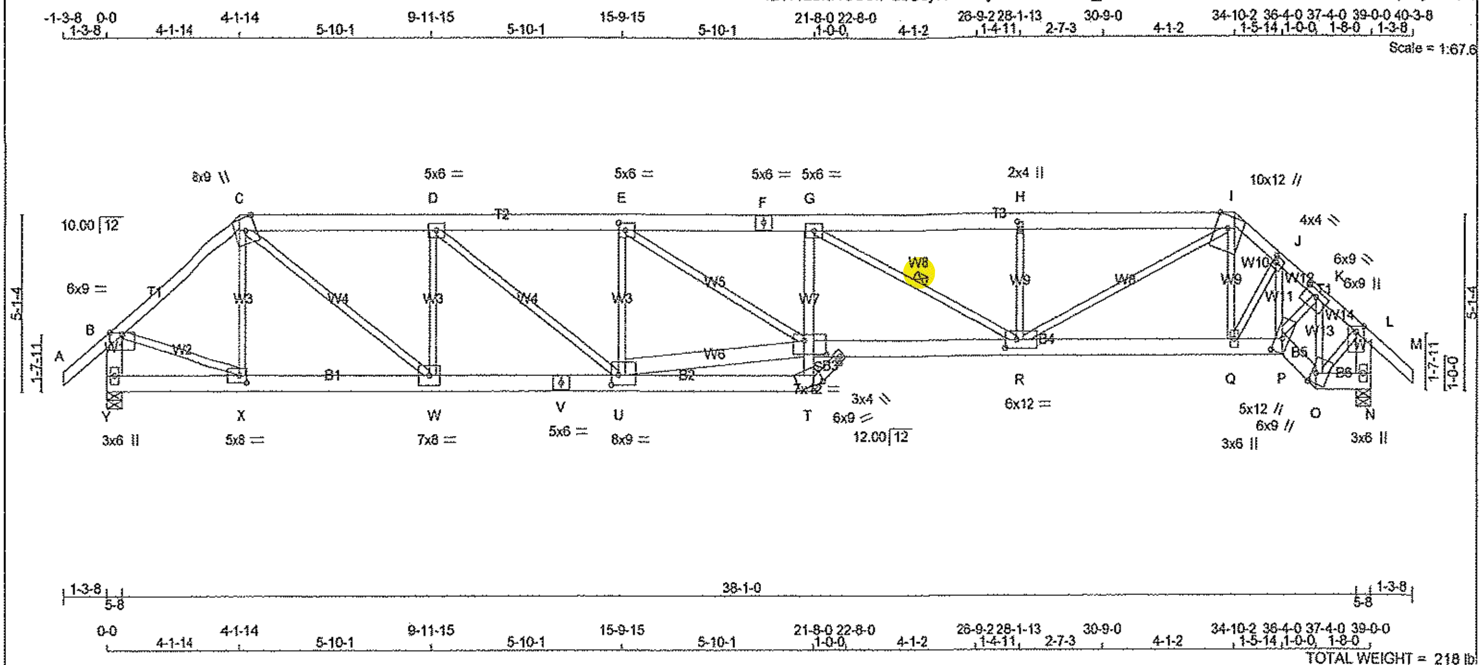

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	1650F 1.5E	SPF
V - T	2x6	DRY	1650F 1.5E	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - O	2x6	DRY	1650F 1.5E	SPF
O - N	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
T - G	2x4	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	8.0	9.0	4.50	3.50
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0	2.50	2.50
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	10.0	12.0	Edge	4.50
J	TMWW-t	MT20	4.0	4.0	2.00	1.25
K	TMWW-t	MT20	6.0	9.0	2.00	4.50
L	TMVW+p	MT20	6.0	9.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	6.0	9.0	3.50	1.50
P	BBWW+m	MT20	5.0	12.0	5.00	2.50
Q	BMWW+t	MT20	3.0	6.0		
R	BMWWW-t	MT20	6.0	12.0	3.00	4.00
S	BWMWW*-t	MT20	7.0	12.0	4.75	8.00
T						
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWW-t	MT20	8.0	9.0	3.50	2.50
V	BS-t	MT20	5.0	6.0		
W	BMWW-t	MT20	7.0	8.0		
X	BMWW-t	MT20	5.0	8.0	2.50	3.00
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Y	3095	0	3095	0	0	5-8	5-8
N	3101	0	3101	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2403	1595 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
N	2407	1599 / 0	409 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	X- C	-498 / 21	0.17 (1)
B- C	-3068 / 0	-122.2	-122.2	0.57 (1)	3.47	C- W	0 / 3092	0.70 (1)
C- D	-4775 / 0	-122.2	-122.2	0.41 (1)	3.69	W- D	-1783 / 0	0.60 (1)
D- E	-5971 / 0	-122.2	-122.2	0.50 (1)	3.26	D- U	0 / 1544	0.35 (1)
E- F	-7776 / 0	-122.2	-122.2	0.59 (1)	2.81	U- E	-1826 / 0	0.61 (1)
F- G	-7776 / 0	-122.2	-122.2	0.59 (1)	2.81	T- S	0 / 118	0.25 (1)
G- H	-6385 / 0	-122.2	-122.2	0.64 (1)	3.02	S- G	0 / 244	0.25 (1)
H- I	-6385 / 0	-122.2	-122.2	0.64 (1)	3.02	Q- I	0 / 250	0.06 (2)
I- J	-3868 / 0	-122.2	-122.2	0.20 (1)	3.45	Q- J	0 / 49	0.01 (3)
J- K	-3853 / 0	-122.2	-122.2	0.24 (1)	3.41	P- J	-177 / 0	0.03 (1)
K- L	-2357 / 0	-122.2	-122.2	0.22 (1)	4.26	P- K	0 / 2248	0.51 (1)
L- M	0 / 54	-122.2	-122.2	0.17 (1)	10.00	O- K	-2835 / 0	0.48 (1)
Y- B	-3058 / 0	0.0	0.0	0.21 (1)	6.03	B- X	0 / 2457	0.55 (1)
N- L	-3078 / 0	0.0	0.0	0.21 (1)	6.02	O- L	0 / 2145	0.48 (1)
						U- S	0 / 5924	0.70 (1)
Y- X	0 / 0	-28.0	-28.0	0.06 (2)	10.00	E- S	0 / 2101	0.47 (1)
X- W	0 / 2351	-28.0	-28.0	0.22 (1)	10.00	R- H	[illegible]	0.21 (1)
W- V	0 / 4775	-28.0	-28.0	0.40 (1)	10.00	G- R	-1642 / 0	0.61 (1)
V- U	0 / 4775	-28.0	-28.0	0.40 (1)	10.00	R- I	0 / 3857	0.87 (1)
U- T	0 / 171	-28.0	-28.0	0.07 (2)	10.00			
S- R	0 / 7824	-28.0	-28.0	0.66 (1)	10.00			
R- Q	0 / 3002	-28.0	-28.0	0.32 (1)	10.00			
Q- P	0 / 2986	-28.0	-28.0	0.32 (1)	10.00			
P- O	0 / 2312	-28.0	-28.0	0.17 (1)	10.00			
O- N	0 / 0	-28.0	-28.0	0.01 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 896 (0.52")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 589 (0.79")

CSI: TC=0.64 (H-I:1) , BC=0.66 (R-S:1) , WB=0.87 (I-R:1) , SSI=0.29 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.91 (S) (INPUT = 1.00)

DWG NO. TAM 46298-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:05 2017 Page 1

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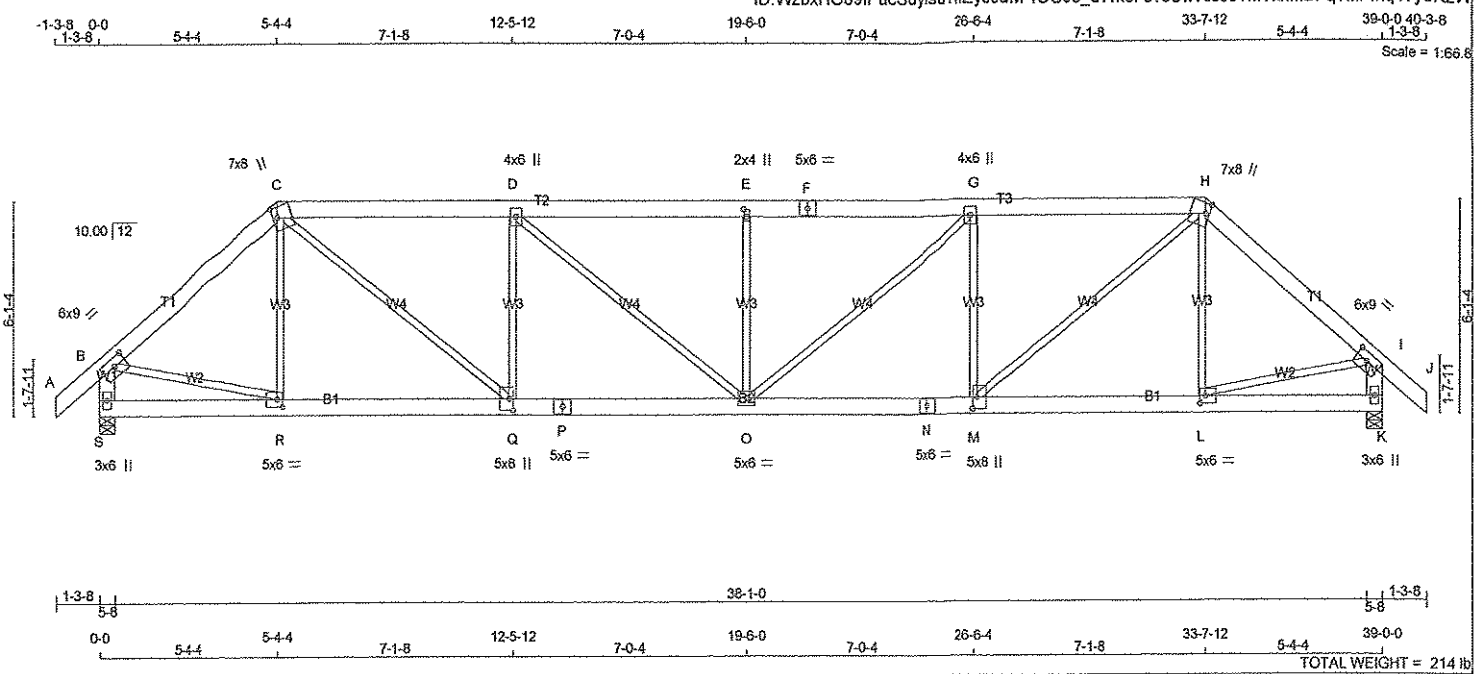

TOTAL WEIGHT = 214 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	7.0	8.0	3.50	1.50
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	7.0	8.0	3.50	1.50
I	TMVW-t	MT20	6.0	9.0	2.75	4.25
K	BMW1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW+t	MT20	5.0	8.0	4.00	1.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	8.0	4.00	1.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3105	0	3105	0	0	5-8	5-8
K	3105	0	3105	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2409	1601 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0
K	2409	1601 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.09 (1)	10.00
B- C	-3187 / 0	-122.2	-122.2	0.38 (1)	4.39
C- D	-4519 / 0	-122.2	-122.2	0.59 (1)	3.56
D- E	-5132 / 0	-122.2	-122.2	0.63 (1)	3.33
E- F	-5132 / 0	-122.2	-122.2	0.63 (1)	3.33
F- G	-5132 / 0	-122.2	-122.2	0.63 (1)	3.33
G- H	-4519 / 0	-122.2	-122.2	0.59 (1)	3.56
H- I	-3187 / 0	-122.2	-122.2	0.38 (1)	4.39
I- J	0 / 56	-122.2	-122.2	0.09 (1)	10.00
S- B	-3050 / 0	0.0	0.0	0.20 (1)	6.04
K- I	-3050 / 0	0.0	0.0	0.20 (1)	6.04
S- R	0 / 0	-28.0	-28.0	0.13 (2)	10.00
R- Q	0 / 2435	-28.0	-28.0	0.37 (1)	10.00
Q- P	0 / 4520	-28.0	-28.0	0.61 (1)	10.00
P- O	0 / 4520	-28.0	-28.0	0.61 (1)	10.00
O- N	0 / 4520	-28.0	-28.0	0.61 (1)	10.00
N- M	0 / 4520	-28.0	-28.0	0.61 (1)	10.00
M- L	0 / 2435	-28.0	-28.0	0.37 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.13 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-293 / 149	0.15 (1)
C- Q	0 / 2653	0.60 (1)
Q- D	-1451 / 0	0.74 (1)
D- O	0 / 786	0.18 (1)
O- E	-794 / 0	0.40 (1)
O- G	0 / 786	0.18 (1)
G- M	-1451 / 0	0.74 (1)
M- H	0 / 2653	0.60 (1)
L- H	-293 / 149	0.15 (1)
B- R	0 / 2491	0.55 (1)
L- I	0 / 2491	0.55 (1)

DWG NO. TAM 46299-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.44")

CSI: TC=0.63 (D-E:1) , BC=0.61 (M-O:1) , WB=0.74 (G-M:1) , SSI=0.31 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.71 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T17S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:06 2017 Page 1

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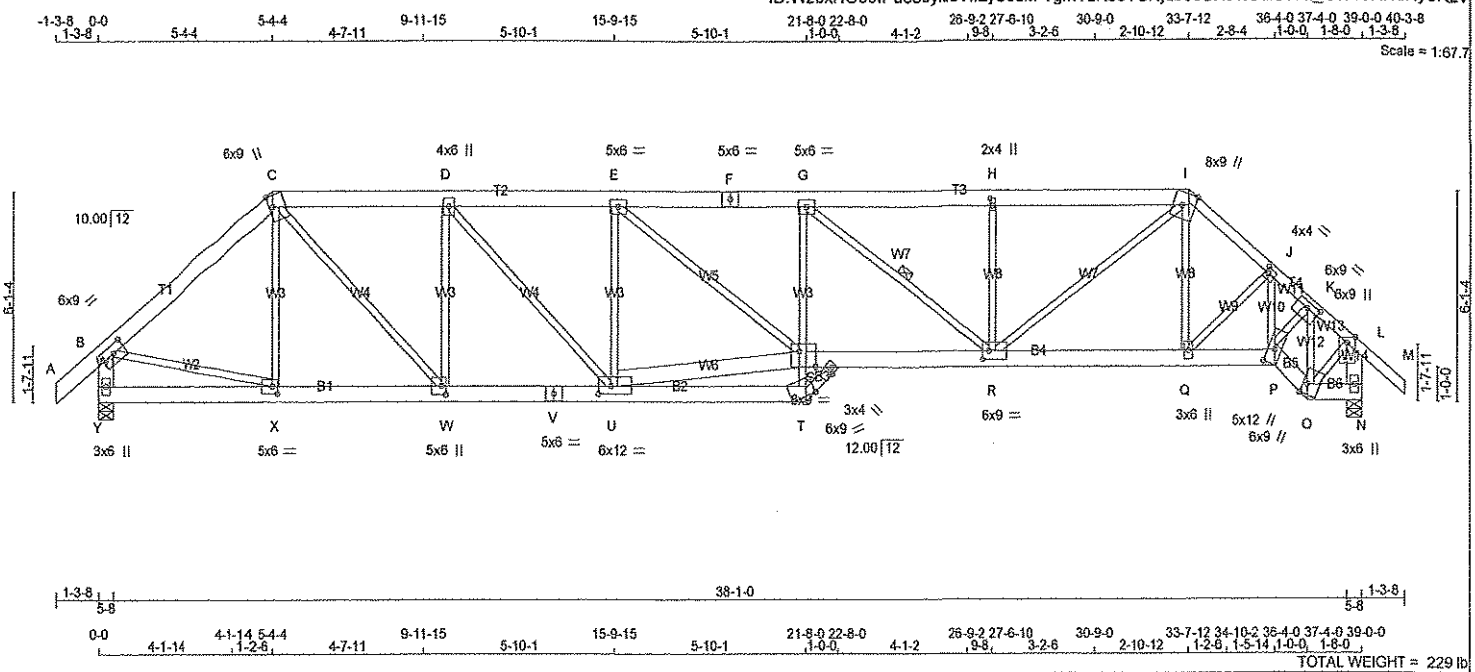


TOTAL WEIGHT = 229 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D	TMWW+t	MT20	4.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	8.0	9.0	Edge	4.75
J	TMWW-t	MT20	4.0	4.0	2.00	1.25
K	TMWW-t	MT20	6.0	9.0	2.00	4.50
L	TMW+p	MT20	6.0	9.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	6.0	9.0	3.50	1.50
P	BBWW+m	MT20	5.0	12.0	5.00	2.50
Q	BMWW+t	MT20	3.0	6.0		
R	BMWWW-t	MT20	6.0	9.0	3.00	2.50
S	BWMWW*-I	MT20	8.0	9.0	5.25	6.00
T						
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWW-t	MT20	6.0	12.0	2.75	4.50
V	BS-t	MT20	5.0	6.0		
W	BMWW+t	MT20	5.0	6.0	3.00	1.50
X	BMWW-t	MT20	5.0	6.0	2.50	2.00
Y	BMV1+p	MT20	3.0	6.0		
Z	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Y	3102	0	3102	0	0	IN-SX 5-8	IN-SX 5-8
N	3101	0	3101	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2407	1599 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
N	2407	1599 / 0	409 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

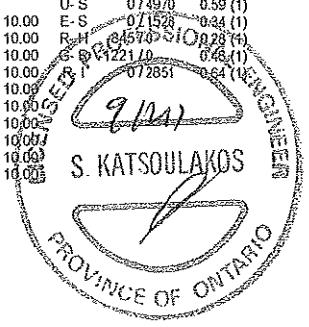
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 56	-122.2	-122.2	0.09 (1)	10.00	X- C	-315 / 112	0.16 (1)
B- C	-3177 / 0	-122.2	-122.2	0.37 (1)	4.39	C- W	0 / 2387	0.54 (1)
C- D	-4054 / 0	-122.2	-122.2	0.31 (1)	4.05	W- D	-1616 / 0	0.82 (1)
D- E	-4916 / 0	-122.2	-122.2	0.35 (1)	3.69	D- U	0 / 1268	0.29 (1)
E- F	-6125 / 0	-122.2	-122.2	0.42 (1)	3.30	U- E	-1623 / 0	0.83 (1)
F- G	-6125 / 0	-122.2	-122.2	0.42 (1)	3.30	T- S	0 / 119	0.13 (1)
G- H	-5170 / 0	-122.2	-122.2	0.46 (1)	3.51	S- G	0 / 223	0.12 (1)
H- I	-5170 / 0	-122.2	-122.2	0.46 (1)	3.51	Q- I	0 / 279	0.06 (2)
I- J	-3773 / 0	-122.2	-122.2	0.28 (1)	3.43	Q- J	-149 / 0	0.04 (1)
J- K	-3843 / 0	-122.2	-122.2	0.26 (1)	3.40	P- J	-142 / 24	0.02 (1)
K- L	-2356 / 0	-122.2	-122.2	0.22 (1)	4.26	P- K	0 / 2293	0.52 (1)
L- M	0 / 54	-122.2	-122.2	0.17 (1)	10.00	O- K	-2826 / 0	0.47 (1)
Y- B	-3042 / 0	0.0	0.0	0.20 (1)	6.04	B- X	0 / 2483	0.56 (1)
N- L	-3078 / 0	0.0	0.0	0.21 (1)	6.02	O- L	0 / 2138	0.48 (1)
						U- S	0 / 4970	0.59 (1)
Y- X	0 / 0	-28.0	-28.0	0.10 (2)	10.00	E- S	0 / 1528	0.34 (1)
X- W	0 / 2427	-28.0	-28.0	0.35 (1)	10.00	R- H	-84570 / 0	0.28 (1)
W- V	0 / 4064	-28.0	-28.0	0.52 (1)	10.00	S- R	4221 / 0	0.45 (1)
V- U	0 / 4064	-28.0	-28.0	0.52 (1)	10.00	I- Q	0 / 2851	0.64 (1)
U- T	0 / 41	-28.0	-28.0	0.10 (3)	10.00			
S- R	0 / 6134	-28.0	-28.0	0.81 (1)	10.00			
R- Q	0 / 2905	-28.0	-28.0	0.42 (1)	10.00			
Q- P	0 / 3009	-28.0	-28.0	0.43 (1)	10.00			
P- O	0 / 2304	-28.0	-28.0	0.27 (1)	10.00			
O- N	0 / 0	-28.0	-28.0	0.01 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.36")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 857 (0.55")

CSI: TC=0.46 (H-I:1) , BC=0.81 (R-S:1) , WB=0.83 (E-U:1) , SSI=0.26 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.73 (U) (INPUT = 1.00)

DWG NO. TAM46300-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:06 2017 Page 1

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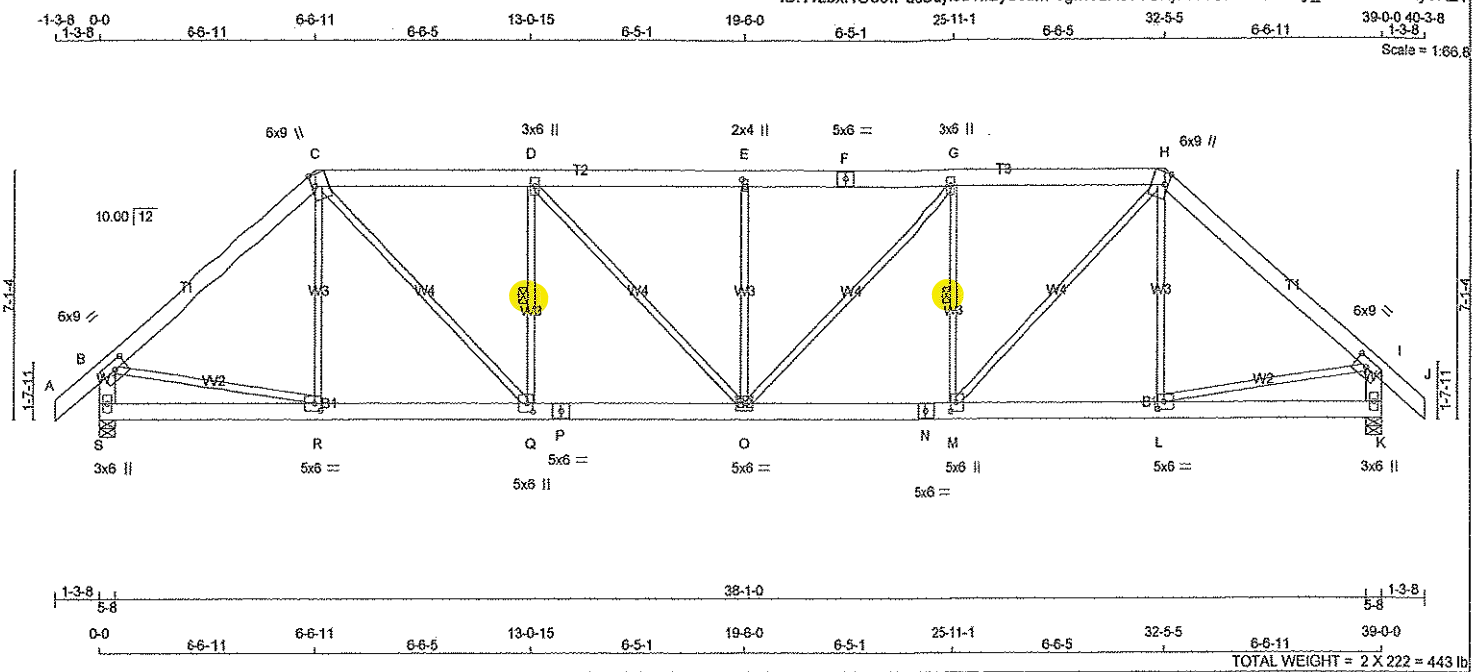

TOTAL WEIGHT = 2 X 222 = 443 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D	TMWW+t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.00	1.25
I	TMVW-t	MT20	6.0	9.0	2.75	4.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMWW+t	MT20	5.0	6.0	3.00	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	6.0	3.00	2.00
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3105	0	3105	0	0	5-8	5-8
K	3105	0	3105	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2409	1601 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0
K	2409	1601 / 0	410 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-Q, G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.09 (1)	10.00	
B- C	-3184 / 0	-122.2	-122.2	0.57 (1)	4.16	
C- D	-3915 / 0	-122.2	-122.2	0.46 (1)	3.94	
D- E	-4349 / 0	-122.2	-122.2	0.49 (1)	3.74	
E- F	-4349 / 0	-122.2	-122.2	0.49 (1)	3.74	
F- G	-4349 / 0	-122.2	-122.2	0.49 (1)	3.74	
G- H	-3915 / 0	-122.2	-122.2	0.46 (1)	3.94	
H- I	-3184 / 0	-122.2	-122.2	0.57 (1)	4.16	
I- J	0 / 56	-122.2	-122.2	0.09 (1)	10.00	
S- B	-3030 / 0	0.0	0.0	0.20 (1)	6.06	
K- I	-3030 / 0	0.0	0.0	0.20 (1)	6.06	
S- R	0 / 0	-28.0	-28.0	0.14 (3)	10.00	
R- Q	0 / 2436	-28.0	-28.0	0.37 (1)	10.00	
Q- P	0 / 3916	-28.0	-28.0	0.53 (1)	10.00	
P- O	0 / 3916	-28.0	-28.0	0.53 (1)	10.00	
O- N	0 / 3916	-28.0	-28.0	0.53 (1)	10.00	
N- M	0 / 3916	-28.0	-28.0	0.53 (1)	10.00	
M- L	0 / 2436	-28.0	-28.0	0.37 (1)	10.00	
L- K	0 / 0	-28.0	-28.0	0.14 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-197 / 207	0.15 (1)
C- Q	0 / 2106	0.47 (1)
Q- D	-1325 / 0	0.39 (1)
D- O	0 / 624	0.14 (1)
O- E	-725 / 0	0.56 (1)
O- G	0 / 624	0.14 (1)
M- G	-1325 / 0	0.39 (1)
M- H	0 / 2106	0.47 (1)
L- H	-197 / 207	0.15 (1)
B- R	0 / 2473	0.56 (1)
L- I	0 / 2473	0.56 (1)

LICENSED PROFESSIONAL ENGINEER
9/12/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 16301-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.57 (H-I:1) , BC=0.53 (M-O:1) , WB=0.56 (I-L:1) , SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.61 (R) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T19	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:07 2017 Page 1

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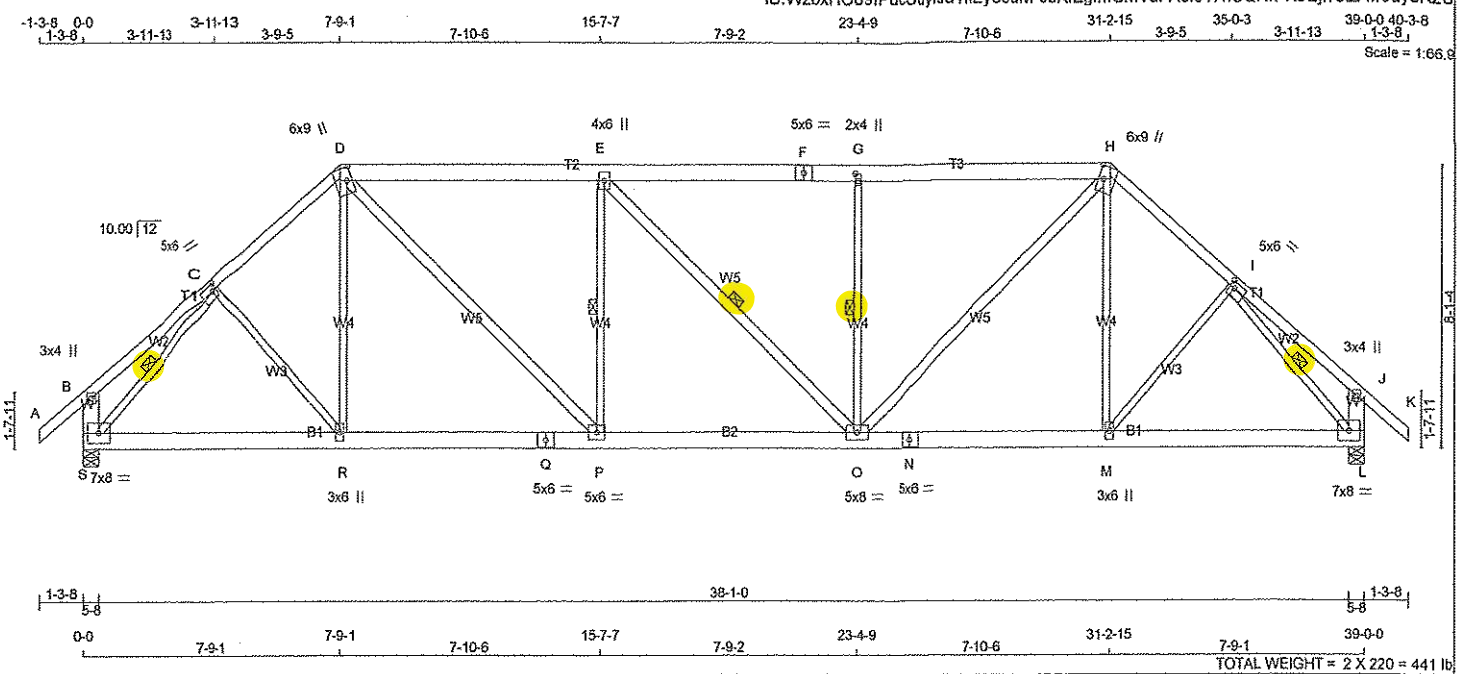


TOTAL WEIGHT = 2 X 220 = 441 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
E - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	6.0	9.0	4.50	1.75
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TTWW+m	MT20	6.0	9.0	4.50	1.75
I	TMWW-t	MT20	5.0	6.0	2.00	1.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	7.0	8.0		
M	BMWW+t	MT20	3.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	3.0	6.0		
S	BMVW1-t	MT20	7.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3098	0	3098	0	0	5-8	5-8
L	3098	0	3098	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
L	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

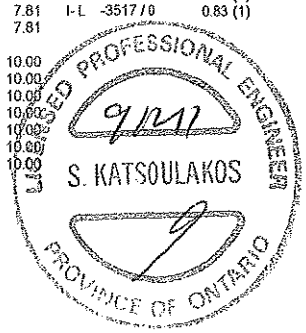
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, E-O, G-O, C-S, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- R	0 / 187	0.04 (2)
B- C	0 / 33	-122.2	-122.2	0.27 (1)	10.00	R- D	0 / 299	0.07 (3)
C- D	-3140 / 0	-122.2	-122.2	0.46 (1)	3.56	D- P	0 / 1816	0.29 (1)
D- E	-3696 / 0	-122.2	-122.2	0.63 (1)	3.80	P- E	-1052 / 0	0.40 (1)
E- F	-3690 / 0	-122.2	-122.2	0.63 (1)	3.81	E- O	-8 / 0	0.00 (1)
F- G	-3690 / 0	-122.2	-122.2	0.63 (1)	3.81	O- G	-1050 / 0	0.40 (1)
G- H	-3691 / 0	-122.2	-122.2	0.62 (1)	3.83	O- H	0 / 1808	0.29 (1)
H- I	-3141 / 0	-122.2	-122.2	0.46 (1)	3.56	M- H	0 / 299	0.07 (3)
I- J	0 / 33	-122.2	-122.2	0.27 (1)	10.00	M- I	0 / 187	0.04 (2)
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00	S- C	-3516 / 0	0.83 (1)
S- B	-353 / 0	0.0	0.0	0.02 (1)	7.81	I- L	-3517 / 0	0.83 (1)
L- J	-353 / 0	0.0	0.0	0.02 (1)	7.81			
S- R	0 / 2307	-28.0	-28.0	0.37 (1)	10.00			
R- Q	0 / 2395	-28.0	-28.0	0.38 (1)	10.00			
Q- P	0 / 2395	-28.0	-28.0	0.38 (1)	10.00			
P- O	0 / 3696	-28.0	-28.0	0.51 (1)	10.00			
O- N	0 / 2396	-28.0	-28.0	0.39 (1)	10.00			
N- M	0 / 2396	-28.0	-28.0	0.39 (1)	10.00			
M- L	0 / 2307	-28.0	-28.0	0.37 (1)	10.00			



DWG NO. TAM46302-17
STRUCTURAL
COMPONENT ONLY

(M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.63 (D-E:1) , BC=0.51 (O-P:1) , WB=0.83 (I-L:1) , SSI=0.34 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.87 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T20	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:07 2017 Page 1

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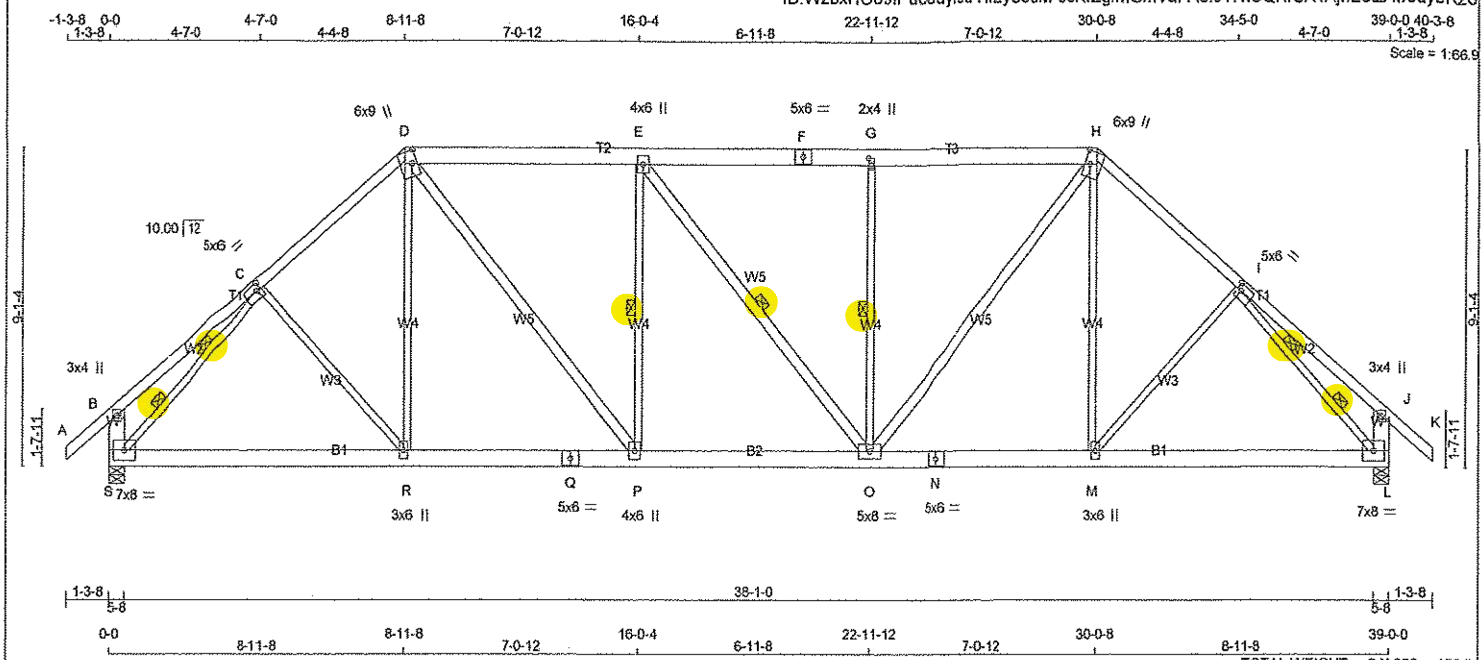

TOTAL WEIGHT = 2 X 226 = 452 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - P	2x4	DRY	No.2	SPF
E - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	6.0	9.0	4.50	1.75
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TTWW+m	MT20	6.0	9.0	4.50	1.75
I	TMWW-t	MT20	5.0	6.0	2.25	1.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	7.0	8.0		
M	BMWW+t	MT20	3.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	3.0	6.0		
S	BMVW1-t	MT20	7.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	3098	0	3098	0	0	5-8	5-8
L	3098	0	3098	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
L	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, E-O, G-O.
2 LATERAL BRACE(S) AT 1/3 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- R	-11 / 147	0.03 (3)
B- C	0 / 40	-122.2	-122.2	0.38 (1)	10.00	R- D	0 / 374	0.08 (2)
C- D	-3093 / 0	-122.2	-122.2	0.63 (1)	3.42	D- P	0 / 1457	0.23 (1)
D- E	-3275 / 0	-122.2	-122.2	0.49 (1)	4.20	P- E	-944 / 0	0.47 (1)
E- F	-3269 / 0	-122.2	-122.2	0.48 (1)	4.20	E- O	-10 / 0	0.01 (1)
F- G	-3269 / 0	-122.2	-122.2	0.48 (1)	4.20	O- G	-941 / 0	0.47 (1)
G- H	-3269 / 0	-122.2	-122.2	0.47 (1)	4.23	O- H	0 / 1446	0.23 (1)
H- I	-3094 / 0	-122.2	-122.2	0.63 (1)	3.42	M- H	0 / 376	0.08 (2)
I- J	0 / 40	-122.2	-122.2	0.38 (1)	10.00	M- I	-11 / 147	0.03 (3)
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00	S- C	-3519 / 0	0.95 (1)
S- B	-379 / 0	0.0	0.0	0.03 (1)	7.81	I- L	-3520 / 0	0.95 (1)
L- J	-379 / 0	0.0	0.0	0.03 (1)	7.81			
S- R	0 / 2358	-28.0	-28.0	0.42 (1)	10.00			
R- Q	0 / 2354	-28.0	-28.0	0.41 (2)	10.00			
Q- P	0 / 2354	-28.0	-28.0	0.41 (2)	10.00			
P- O	0 / 3275	-28.0	-28.0	0.45 (1)	10.00			
O- N	0 / 2355	-28.0	-28.0	0.41 (2)	10.00			
N- M	0 / 2355	-28.0	-28.0	0.41 (2)	10.00			
M- L	0 / 2359	-28.0	-28.0	0.41 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.63 (H-I:1) , BC=0.45 (O-P:1) , WB=0.95 (I-L:1) , SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.87 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9M7
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 46303-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:08 2017 Page 1

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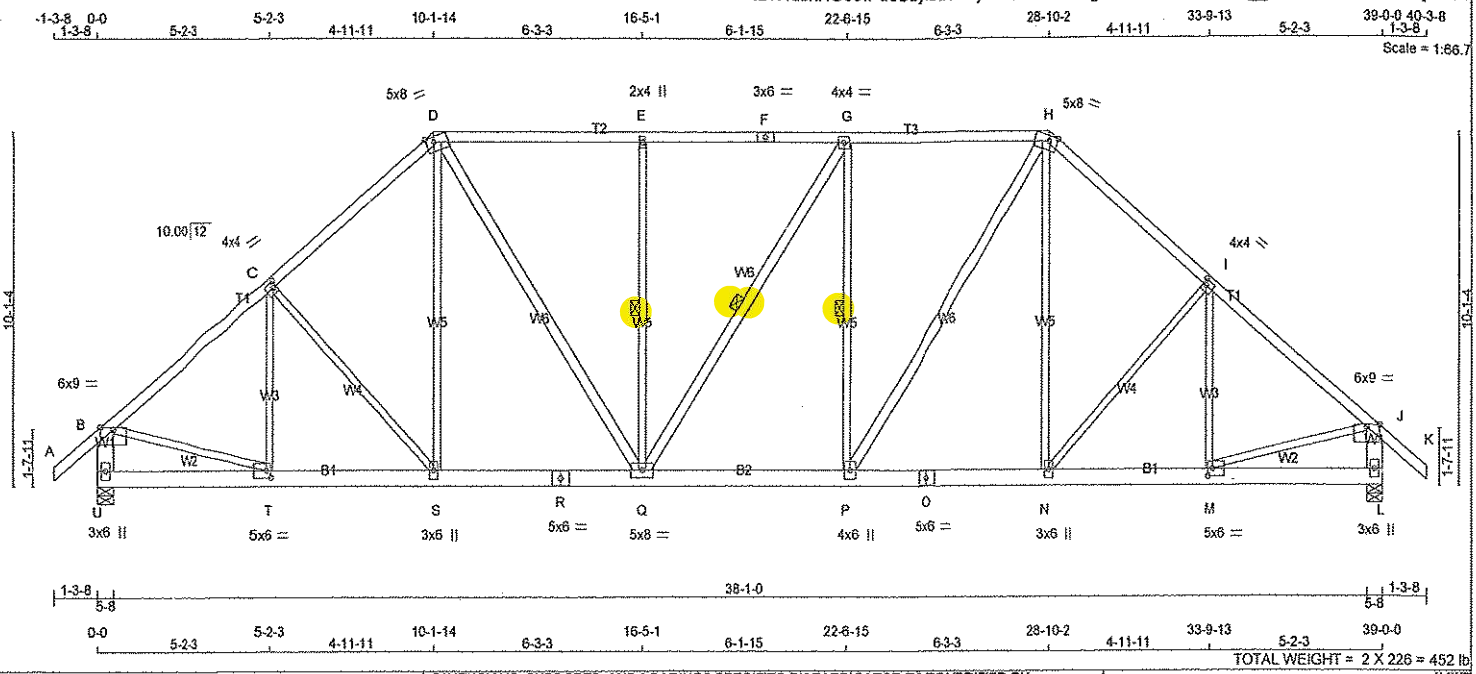


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	6.0	9.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	1.75
N	BMWW+t	MT20	3.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	1.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

[M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3098	0	3098	0	0	5-8	5-8
L	3098	0	3098	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
L	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-Q, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	T- C	-452 / 49	0.23 (1)
B- C	-3133 / 0	-122.2	-122.2	0.61 (1)	3.43	C- S	-251 / 0	0.28 (1)
C- D	-3013 / 0	-122.2	-122.2	0.57 (1)	3.52	S- D	0 / 390	0.09 (2)
D- E	-2913 / 0	-122.2	-122.2	0.71 (1)	3.32	D- Q	0 / 1175	0.19 (1)
E- F	-2913 / 0	-122.2	-122.2	0.65 (1)	3.42	Q- E	-820 / 0	0.55 (1)
F- G	-2913 / 0	-122.2	-122.2	0.65 (1)	3.42	Q- G	-10 / 0	0.01 (1)
G- H	-2918 / 0	-122.2	-122.2	0.71 (1)	3.31	P- G	-817 / 0	0.55 (1)
H- I	-3012 / 0	-122.2	-122.2	0.57 (1)	3.52	P- H	0 / 1187	0.19 (1)
I- J	-3133 / 0	-122.2	-122.2	0.61 (1)	3.43	N- H	0 / 396	0.09 (2)
J- K	0 / 54	-122.2	-122.2	0.17 (1)	10.00	N- I	-254 / 0	0.28 (1)
U- B	-3024 / 0	0.0	0.0	0.20 (1)	6.06	M- I	-449 / 49	0.23 (1)
L- J	-3025 / 0	0.0	0.0	0.20 (1)	6.06	B- T	0 / 2509	0.56 (1)
						M- J	0 / 2510	0.56 (1)
U- T	0 / 0	-28.0	-28.0	0.09 (2)	10.00			
T- S	0 / 2442	-28.0	-28.0	0.35 (1)	10.00			
S- R	0 / 2276	-28.0	-28.0	0.33 (1)	10.00			
R- Q	0 / 2276	-28.0	-28.0	0.33 (1)	10.00			
Q- P	0 / 2919	-28.0	-28.0	0.40 (1)	10.00			
P- O	0 / 2275	-28.0	-28.0	0.32 (1)	10.00			
O- N	0 / 2275	-28.0	-28.0	0.32 (1)	10.00			
N- M	0 / 2443	-28.0	-28.0	0.34 (1)	10.00			
M- L	0 / 0	-28.0	-28.0	0.09 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.71 (G-H:1) , BC=0.40 (P-Q:1) , WB=0.56 (J-M:1) , SSI=0.36 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

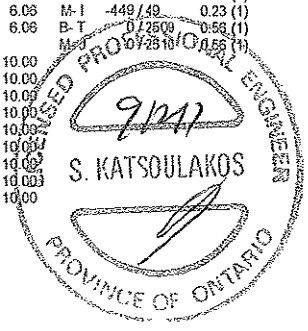
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.61 (M) (INPUT = 1.00)



DWG NO. TAM 46304-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T22	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:08 2017 Page 1

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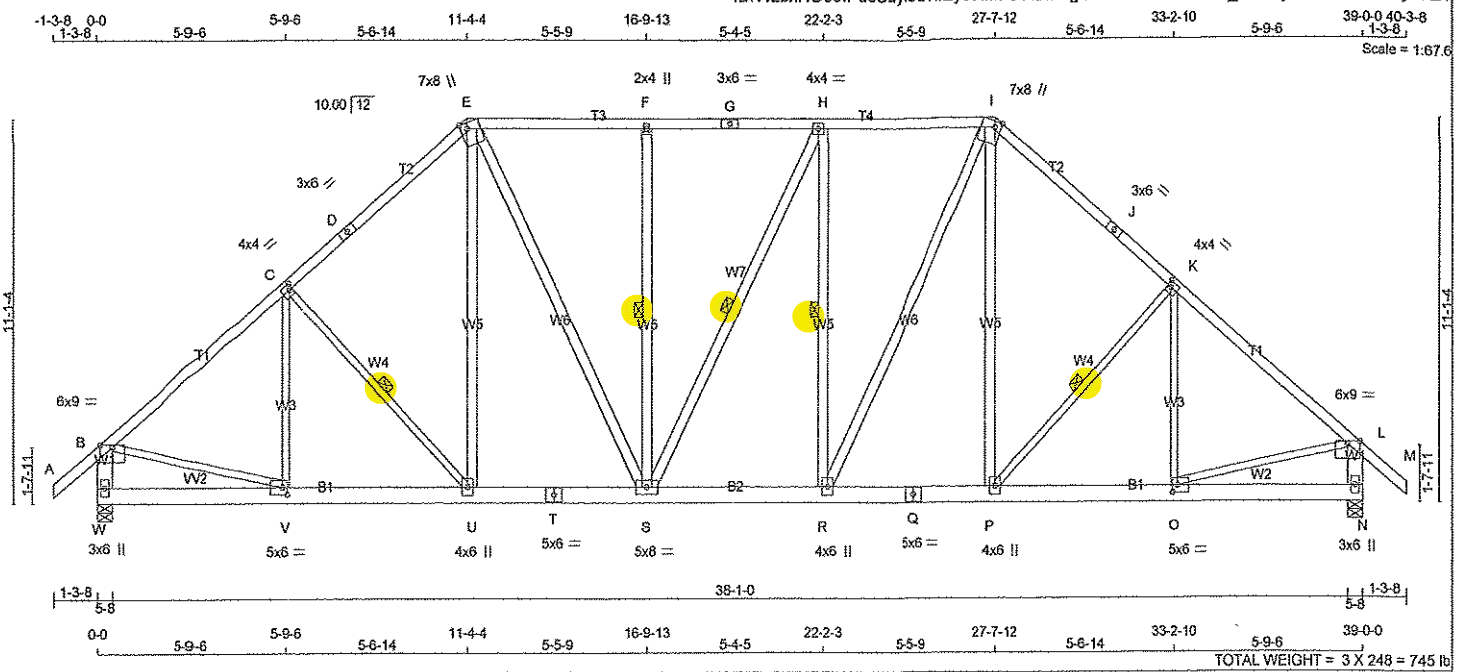

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - C	2x3	DRY	No.2	SPF
C - U	2x3	DRY	No.2	SPF
P - K	2x3	DRY	No.2	SPF
O - K	2x3	DRY	No.2	SPF
B - V	2x3	DRY	No.2	SPF
O - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D, G, J						
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMWW-t	MT20	4.0	4.0	2.00	1.25
L	TMVW-p	MT20	6.0	9.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	1.75
P, R, U						
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	1.75
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3098	0	3098	0	0	5-8	5-8
N	3098	0	3098	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
N	2405	1597 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-U, F-S, H-S, H-R, K-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-122.2	-122.2	0.17 (1)		10.00
B- C	-3164 / 0	-122.2	-122.2	0.79 (1)		3.20
C- D	-2930 / 0	-122.2	-122.2	0.72 (1)		3.38
D- E	-2930 / 0	-122.2	-122.2	0.72 (1)		3.38
E- F	-2645 / 0	-122.2	-122.2	0.51 (1)		3.75
F- G	-2646 / 0	-122.2	-122.2	0.48 (1)		3.80
G- H	-2646 / 0	-122.2	-122.2	0.48 (1)		3.80
H- I	-2650 / 0	-122.2	-122.2	0.51 (1)		3.74
I- J	-2929 / 0	-122.2	-122.2	0.72 (1)		3.38
J- K	-2929 / 0	-122.2	-122.2	0.72 (1)		3.38
K- L	-3164 / 0	-122.2	-122.2	0.79 (1)		3.20
L- M	0 / 54	-122.2	-122.2	0.17 (1)		10.00
W- B	-3021 / 0	0.0	0.0	0.20 (1)		6.07
N- L	-3021 / 0	0.0	0.0	0.20 (1)		6.07
W- V	0 / 0	-28.0	-28.0	0.10 (2)		10.00
V- U	0 / 2471	-28.0	-28.0	0.35 (1)		10.00
U- T	0 / 2211	-28.0	-28.0	0.32 (1)		10.00
T- S	0 / 2211	-28.0	-28.0	0.32 (1)		10.00
S- R	0 / 2650	-28.0	-28.0	0.35 (1)		10.00
R- Q	0 / 2210	-28.0	-28.0	0.31 (1)		10.00
Q- P	0 / 2210	-28.0	-28.0	0.31 (1)		10.00
P- O	0 / 2472	-28.0	-28.0	0.34 (1)		10.00
O- N	0 / 0	-28.0	-28.0	0.10 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-369 / 114	0.23 (1)
C- U	-388 / 0	0.18 (1)
U- E	0 / 456	0.07 (1)
E- S	0 / 557	0.15 (1)
S- F	-712 / 0	0.44 (1)
S- H	-9 / 0	0.01 (1)
R- H	-709 / 0	0.43 (1)
R- I	0 / 998	0.16 (1)
P- I	0 / 449	0.07 (1)
P- K	-390 / 0	0.18 (1)
O- K	-366 / 115	0.23 (1)
B- V	0 / 2526	0.57 (1)
O- L	0 / 2526	0.57 (1)

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.79 (K-L:1) , BC=0.35 (R-S:1) , WB=0.57 (L-O:1) , SSI=0.31 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.62 (O) (INPUT = 1.00)

DWG NO. YAM 46305-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:08 2017 Page 1

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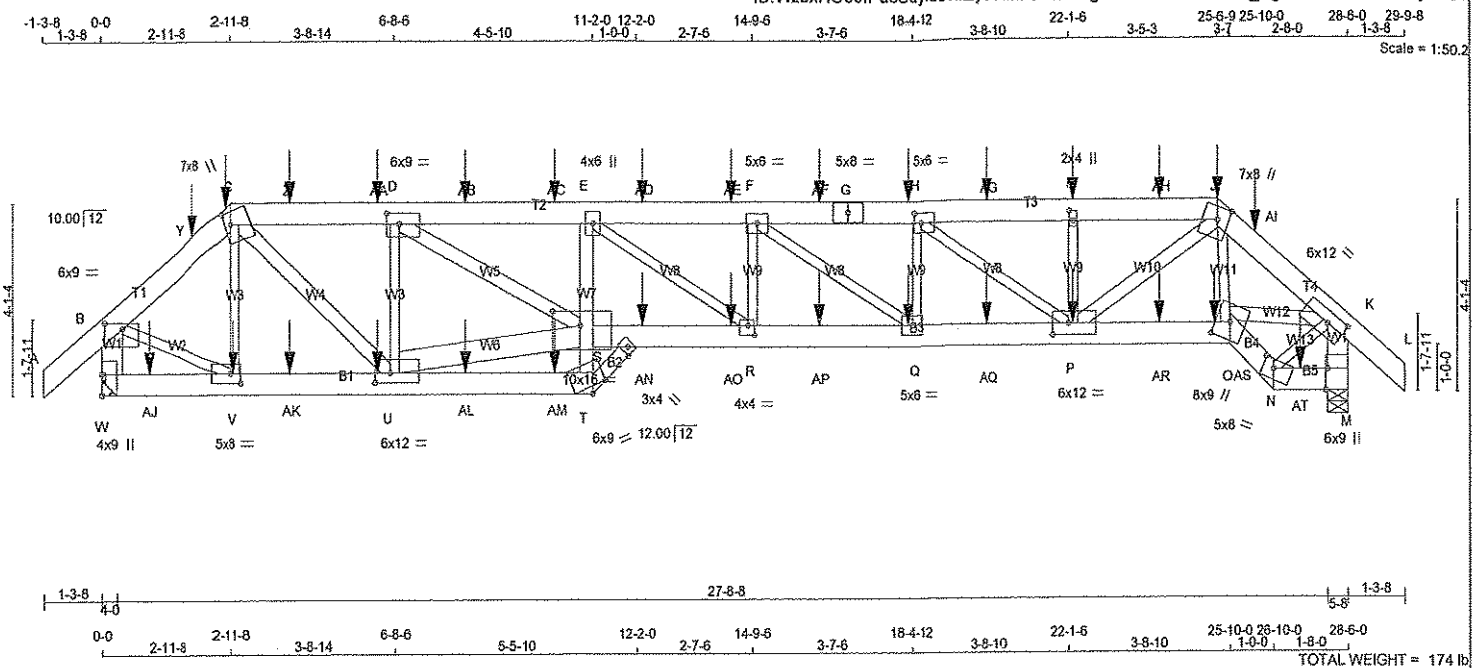


[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
O - N	2x6	DRY	2100F 1.8E	SPF
N - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - U	2x4	DRY	No.2	SPF
T - E	2x4	DRY	No.2	SPF
U - S	2x6	DRY	2100F 1.8E	SPF
D - S	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

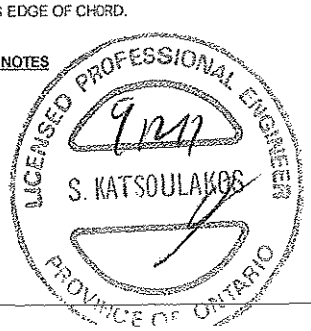
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	1.50	4.75
C	TTWW+m	MT20	7.0	8.0	3.25	1.75
D	TMWW-t	MT20	6.0	9.0	2.75	3.50
E	TMWW+t	MT20	4.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	5.0	8.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.00
I	TMW+w	MT20	2.0	4.0	2.50	1.00
J	TTWW+m	MT20	7.0	8.0	3.25	3.00
K	TMWW-t	MT20	6.0	12.0	2.75	4.75
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BBW-m	MT20	5.0	8.0	2.25	3.00
O	BBWW+m	MT20	8.0	9.0	4.50	3.75
P	BMWWW-t	MT20	6.0	12.0	3.00	4.50
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.50	1.75
S	BWMWW*-I	MT20	10.0	16.0	3.75	7.50
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWW-t	MT20	6.0	12.0	2.50	4.25
V	BMWW-t	MT20	5.0	8.0	2.50	3.00
W	BMV1+t	MT20	4.0	9.0	5.50	
X	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
W	4183	0	4183	0	0	IN-SX	HANGER BY OTHERS
							MIN. SEAT SIZE: 4-0
M	4435	0	4435	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD 0K

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3248	2152 / 0	554 / 0	0 / 0	0 / 0	542 / 0	0 / 0
M	3449	2277 / 0	594 / 0	0 / 0	0 / 0	578 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 1.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 6.92 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.10 (1)		10.00
B- Y	-4004 / 0	-122.2	-122.2	0.24 (1)		4.08
Y- C	-4004 / 0	-122.2	-122.2	0.24 (1)		4.08
C- Z	-5618 / 0	-122.2	-122.2	0.43 (1)		3.34
Z-AA	-5618 / 0	-122.2	-122.2	0.43 (1)		3.34
AA- D	-5618 / 0	-122.2	-122.2	0.43 (1)		3.34
D-AB	-10233 / 0	-122.2	-122.2	0.93 (1)		1.90
AB-AC	-10233 / 0	-122.2	-122.2	0.93 (1)		1.90
AC- E	-10233 / 0	-122.2	-122.2	0.93 (1)		1.90
E-AD	-10891 / 0	-122.2	-122.2	0.98 (1)		1.73
AD-AE	-10891 / 0	-122.2	-122.2	0.98 (1)		1.73
AE- F	-10891 / 0	-122.2	-122.2	0.98 (1)		1.73
F-AF	-10086 / 0	-122.2	-122.2	0.86 (1)		2.02
AF- G	-10086 / 0	-122.2	-122.2	0.86 (1)		2.02
G- H	-10086 / 0	-122.2	-122.2	0.86 (1)		2.02
H-AG	-7845 / 0	-122.2	-122.2	0.59 (1)		2.69
AG- I	-7845 / 0	-122.2	-122.2	0.59 (1)		2.69
I-AH	-7845 / 0	-122.2	-122.2	0.55 (1)		2.72
AH- J	-7845 / 0	-122.2	-122.2	0.55 (1)		2.72
J-AI	-5938 / 0	-122.2	-122.2	0.34 (1)		3.30
AI- K	-5938 / 0	-122.2	-122.2	0.34 (1)		3.30
K- L	0 / 56	-122.2	-122.2	0.10 (1)		10.00
W- B	-4129 / 0	0.0	0.0	0.30 (1)		5.23
M- K	-4396 / 0	0.0	0.0	0.32 (1)		5.08

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
V- C	-927 / 0	0.22	(1)
C- U	0 / 3616	0.64	(1)
U- D	-3623 / 0	0.87	(1)
T- S	0 / 174	0.36	(1)
S- E	-1080 / 0	0.37	(1)
B- V	0 / 3280	0.81	(1)
U- S	0 / 5648	0.30	(1)
D- S	0 / 5275	0.93	(1)
N- K	0 / 84	0.02	(2)
Q- H	0 / 973	0.24	(1)
P- I	-538 / 0	0.10	(1)
R- F	-41 / 145	0.04	(3)
E- R	0 / 699	0.17	(1)
F- Q	-988 / 0	0.34	(1)
H- P	-2751 / 0	0.94	(1)
J- O	0 / 326	0.08	(2)
P- J	0 / 4222	0.75	(1)
O- K	0 / 4519	0.80	(1)

W-AJ	0 / 0	-28.0	-28.0	0.03 (3)	10.00
AJ- V	0 / 0	-28.0	-28.0	0.03 (3)	10.00
V-AK	0 / 3056	-28.0	-28.0	0.18 (1)	10.00
AK- U	0 / 3056	-28.0	-28.0	0.18 (1)	10.00
U-AL	0 / 222	-28.0	-28.0	0.07 (2)	10.00
AL-AM	0 / 222	-28.0	-28.0	0.07 (2)	10.00
AM- T	0 / 222	-28.0	-28.0	0.07 (2)	10.00
S-AN	0 / 10320	-28.0	-28.0	0.65 (1)	10.00
AN-AO	0 / 10320	-28.0	-28.0	0.65 (1)	10.00
AO- R	0 / 10320	-28.0	-28.0	0.65 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 698 (0.49")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 458 (0.75")

CSI: TC=0.98 (E-F:1) , BC=0.70 (Q-R:1) , WB=0.94 (H-P:1) , SSI=0.33 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.99 (S) (INPUT = 1.00)

P6/6

DWG NO. T AM 46306-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T23	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 192.5 lbs FACTORED DOWN AT 2-2-12, 192.5 lbs FACTORED DOWN AT 26-3-4, 192.5 lbs FACTORED DOWN AT 2-11-8, 192.5 lbs FACTORED DOWN AT 25-6-9, 150.6 lbs FACTORED DOWN AT 2-11-8, 147.1 lbs FACTORED DOWN AT 4-3-8, 147.1 lbs FACTORED DOWN AT 6-3-8, 147.1 lbs FACTORED DOWN AT 8-3-8, 147.1 lbs FACTORED DOWN AT 10-3-8, 89.2 lbs FACTORED DOWN AT 12-3-8, 89.2 lbs FACTORED DOWN AT 14-3-8, 89.2 lbs FACTORED DOWN AT 16-3-8, 89.2 lbs FACTORED DOWN AT 18-3-8, 89.2 lbs FACTORED DOWN AT 20-2-8, 89.2 lbs FACTORED DOWN AT 22-2-8, AND 89.2 lbs FACTORED DOWN AT 24-2-8, AND 92.7 lbs FACTORED DOWN AT 25-6-9 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 4-3-8, 69.9 lbs FACTORED DOWN AT 6-3-8, 69.9 lbs FACTORED DOWN AT 8-3-8, 76.5 lbs FACTORED DOWN AT 10-3-8, 182.7 lbs FACTORED DOWN AT 12-3-8, 182.7 lbs FACTORED DOWN AT 14-3-8, 182.7 lbs FACTORED DOWN AT 16-3-8, 182.7 lbs FACTORED DOWN AT 18-3-8, 182.7 lbs FACTORED DOWN AT 20-2-8, 182.7 lbs FACTORED DOWN AT 22-2-8, 182.7 lbs FACTORED DOWN AT 24-2-8, AND 191.8 lbs FACTORED DOWN AT 25-5-12, AND 79.6 lbs FACTORED DOWN AT 27-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-AP	0 / 10891	-28.0	-28.0	0.70 (1)	10.00			
AP- Q	0 / 10891	-28.0	-28.0	0.70 (1)	10.00			
Q-AQ	0 / 10086	-28.0	-28.0	0.64 (1)	10.00			
AQ- P	0 / 10086	-28.0	-28.0	0.64 (1)	10.00			
P-AR	0 / 4524	-28.0	-28.0	0.33 (1)	10.00			
AR-AS	0 / 4524	-28.0	-28.0	0.33 (1)	10.00			
AS- O	0 / 4524	-28.0	-28.0	0.33 (1)	10.00			
O- N	0 / 100	-28.0	-28.0	0.01 (2)	10.00			
N-AT	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
AT- M	0 / 0	-28.0	-28.0	0.02 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-13	-14	---	FRONT	VERT	DEAD
C	2-11-8	-151	-151	---	BACK	VERT	TOTAL
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW
H	18-3-8	-89	-89	---	BACK	VERT	TOTAL
I	22-2-8	-89	-89	---	BACK	VERT	TOTAL
J	25-6-9	-13	-14	---	FRONT	VERT	DEAD
J	25-6-9	-93	-93	---	BACK	VERT	TOTAL
J	25-6-9	-180	-180	---	FRONT	VERT	SNOW
P	22-2-8	-183	-183	---	BACK	VERT	TOTAL
Q	18-3-8	-183	-183	---	BACK	VERT	TOTAL
U	6-3-8	-40	-70	---	BACK	VERT	TOTAL
V	3-0-4	-40	-70	---	BACK	VERT	TOTAL
Y	2-2-12	-13	-14	---	FRONT	VERT	DEAD
Y	2-2-12	-180	-180	---	FRONT	VERT	SNOW
Z	4-3-8	-147	-147	---	BACK	VERT	TOTAL
AA	6-3-8	-147	-147	---	BACK	VERT	TOTAL
AB	8-3-8	-147	-147	---	BACK	VERT	TOTAL
AC	10-3-8	-147	-147	---	BACK	VERT	TOTAL
AD	12-3-8	-89	-89	---	BACK	VERT	TOTAL
AE	14-3-8	-89	-89	---	BACK	VERT	TOTAL
AF	16-3-8	-89	-89	---	BACK	VERT	TOTAL
AG	20-2-8	-89	-89	---	BACK	VERT	TOTAL
AH	24-2-8	-89	-89	---	BACK	VERT	TOTAL
AI	26-3-4	-13	-14	---	FRONT	VERT	DEAD
AI	26-3-4	-180	-180	---	FRONT	VERT	SNOW
AJ	1-0-12	-40	-70	---	BACK	VERT	TOTAL
AK	4-3-8	-40	-70	---	BACK	VERT	TOTAL
AL	8-3-8	-40	-70	---	BACK	VERT	TOTAL
AM	10-3-8	-44	-77	---	BACK	VERT	TOTAL
AN	12-3-8	-183	-183	---	BACK	VERT	TOTAL
AO	14-3-8	-183	-183	---	BACK	VERT	TOTAL
AP	16-3-8	-183	-183	---	BACK	VERT	TOTAL
AQ	20-2-8	-183	-183	---	BACK	VERT	TOTAL
AR	24-2-8	-183	-183	---	BACK	VERT	TOTAL
AS	25-5-12	-192	-192	---	BACK	VERT	TOTAL
AT	27-5-4	-45	-80	---	BACK	VERT	TOTAL

DWG NO. TAM 46306-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:09 2017 Page 1

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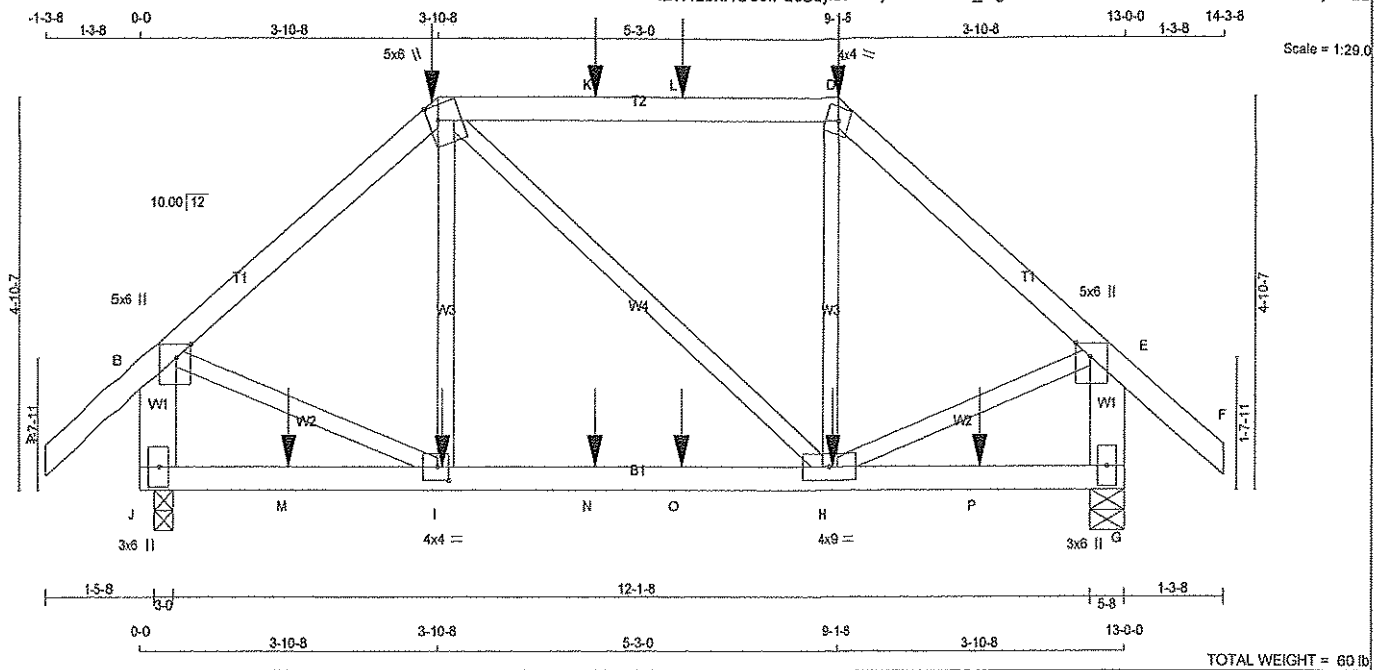

TOTAL WEIGHT = 60 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	2.00	2.25
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 235.1 lbs FACTORED DOWN AT 3-10-8, 55.6 lbs FACTORED DOWN AT 5-11-4, AND 55.6 lbs FACTORED DOWN AT 7-0-12, AND 235.1 lbs FACTORED DOWN AT 9-1-8 ON TOP CHORD, AND 36.6 lbs FACTORED DOWN AT 1-11-4, 34.1 lbs FACTORED DOWN AT 3-11-4, 34.1 lbs FACTORED DOWN AT 5-11-4, 34.1 lbs FACTORED DOWN AT 7-0-12, AND 34.1 lbs FACTORED DOWN AT 9-0-12, AND 36.6 lbs FACTORED DOWN AT 11-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1490	0	1490	0	0	3-0	3-0
G	1490	0	1490	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1148	777 / 0	187 / 0	0 / 0	0 / 0	185 / 0	0 / 0
G	1148	777 / 0	187 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	1	0.19 (1)	10.00	I- C	-102 / 227	0.06 (3)
B- C	-1161 / 0	-122.2	-122.2	1	0.39 (1)	5.37	C- H	0 / 0	0.00 (1)
C- K	-887 / 0	-122.2	-122.2	1	0.85 (1)	4.52	H- D	-102 / 228	0.06 (3)
K- L	-887 / 0	-122.2	-122.2	1	0.85 (1)	4.52	H- E	0 / 936	0.23 (1)
L- D	-887 / 0	-122.2	-122.2	1	0.85 (1)	4.52	B- I	0 / 936	0.23 (1)
D- E	-1161 / 0	-122.2	-122.2	1	0.39 (1)	5.37			
E- F	0 / 54	-122.2	-122.2	1	0.19 (1)	10.00			
J- B	-1444 / 0	0.0	0.0	1	0.11 (1)	7.81			
G- E	-1444 / 0	0.0	0.0	1	0.11 (1)	7.81			
J- M	0 / 0	-28.0	-28.0	3	0.20 (3)	10.00			
M- I	0 / 0	-28.0	-28.0	3	0.20 (3)	10.00			
I- N	0 / 887	-28.0	-28.0	2	0.30 (2)	10.00			
N- O	0 / 887	-28.0	-28.0	2	0.30 (2)	10.00			
O- H	0 / 887	-28.0	-28.0	2	0.30 (2)	10.00			
H- P	0 / 0	-28.0	-28.0	3	0.20 (3)	10.00			
P- G	0 / 0	-28.0	-28.0	3	0.20 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-235	-235	---	BACK	VERT	TOTAL
D	9-1-8	-235	-235	---	BACK	VERT	TOTAL
H	9-0-12	-20	-34	---	BACK	VERT	TOTAL
I	3-11-4	-20	-34	---	BACK	VERT	TOTAL
K	5-11-4	-56	-56	---	BACK	VERT	TOTAL
L	7-0-12	-56	-56	---	BACK	VERT	TOTAL
M	1-11-4	-14	-37	---	BACK	VERT	TOTAL
N	5-11-4	-20	-34	---	BACK	VERT	TOTAL
O	7-0-12	-20	-34	---	BACK	VERT	TOTAL
P	11-0-12	-14	-37	---	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.85 (C-D:1) , BC=0.30 (H-I:2) , WB=0.23 (B-I:1) , SSI=0.32 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 46302 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T25A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:09 2017 Page 1

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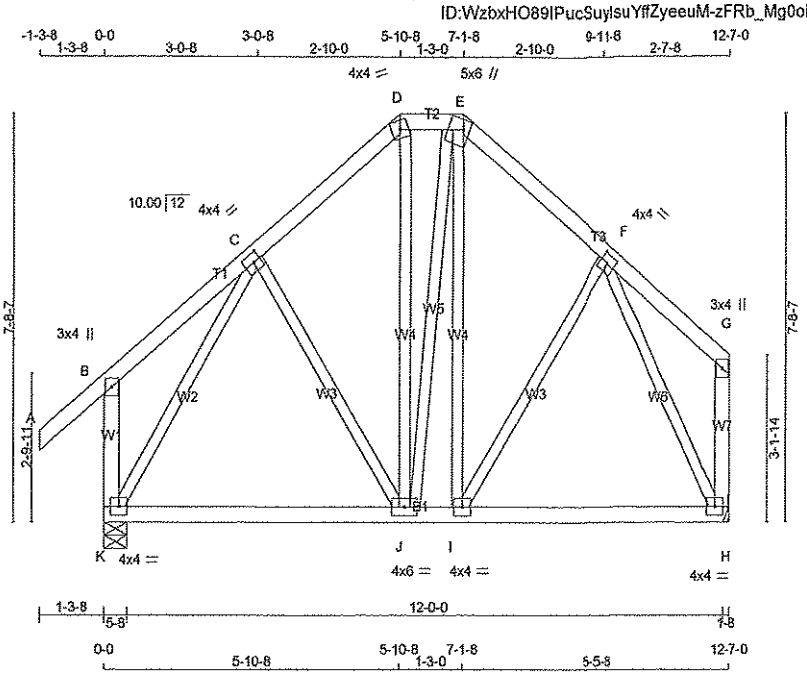

TOTAL WEIGHT = 2 X 76 = 152 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1114	0	1114	0	0	5-8	5-8
H	945	0	945	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	853	587 / 0	132 / 0	0 / 0	0 / 0	134 / 0	0 / 0
H	739	481 / 0	132 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- J	-59 / 68	0.03 (1)
B- C	0 / 26	-122.2	-122.2	0.17 (1)	10.00	J- D	0 / 150	0.03 (2)
C- D	-564 / 0	-122.2	-122.2	0.13 (1)	6.25	J- E	0 / 51	0.01 (2)
D- E	-413 / 0	-122.2	-122.2	0.03 (1)	6.25	I- E	0 / 91	0.02 (2)
E- F	-552 / 0	-122.2	-122.2	0.11 (1)	6.25	I- F	0 / 86	0.02 (3)
F- G	0 / 26	-122.2	-122.2	0.15 (1)	10.00	K- C	-859 / 0	0.46 (1)
K- B	-310 / 0	0.0	0.0	0.05 (1)	7.81	F- H	-862 / 0	0.43 (1)
H- G	-115 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 442	-28.0	-28.0	0.28 (2)	10.00			
J- I	0 / 406	-28.0	-28.0	0.28 (2)	10.00			
I- H	0 / 395	-28.0	-28.0	0.23 (2)	10.00			

DWG NO. T2M46308 17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.17 (A-B:1) , BC=0.28 (I-J:2) , WB=0.46 (C-K:1) , SSI=0.14 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T26A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:09 2017 Page 1

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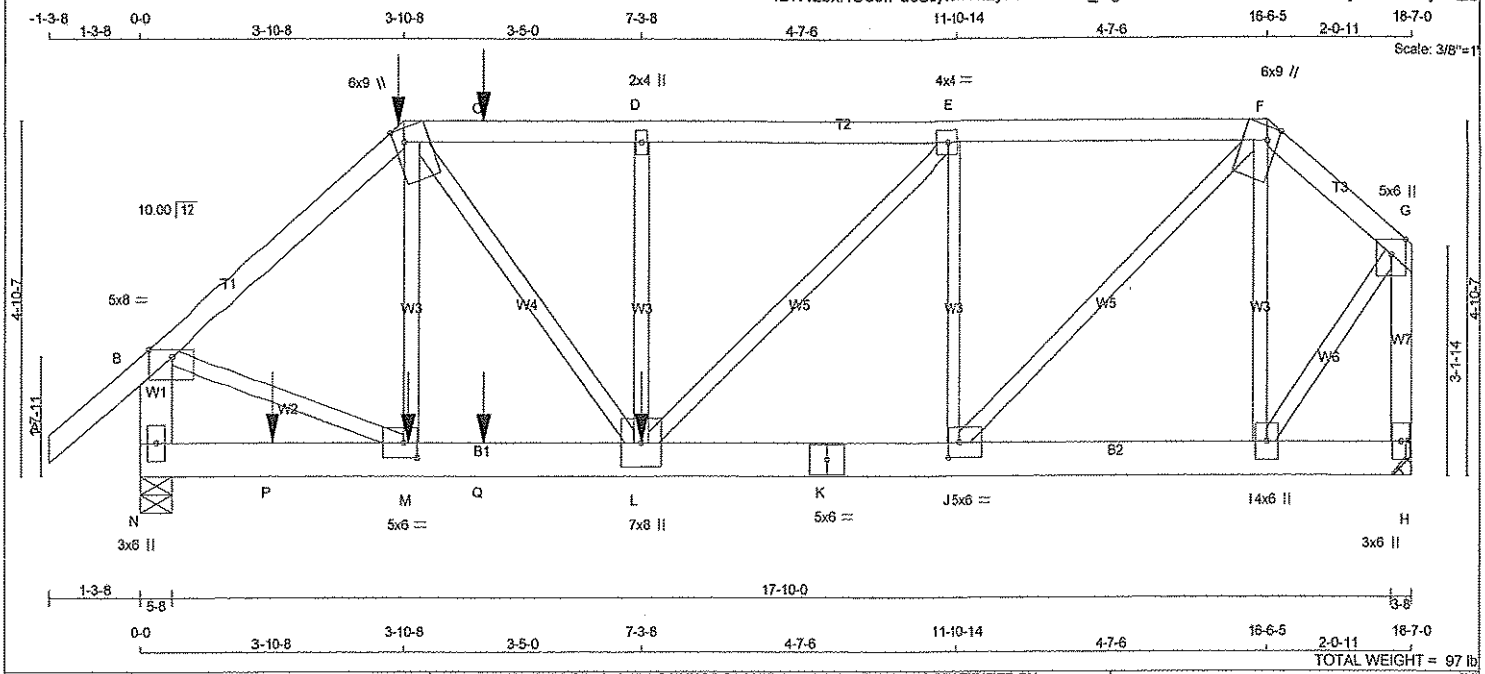

TOTAL WEIGHT = 97 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	9.0	Edge	1.75
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	4.0	6.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.25
K	BS-t	MT20	5.0	6.0		
L	BMWWW+t	MT20	7.0	8.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 235.1 lbs FACTORED DOWN AT 3-10-8, AND 55.6 lbs FACTORED DOWN AT 5-0-4 ON TOP CHORD, AND 36.6 lbs FACTORED DOWN AT 1-11-4, 34.1 lbs FACTORED DOWN AT 3-11-4, AND 34.1 lbs FACTORED DOWN AT 5-0-4, AND 1365.7 lbs FACTORED DOWN AT 7-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
N	2664	0	2664	0	0	IN-SX	IN-SX
H	2007	0	2007	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	2040	1404 / 0	316 / 0	0 / 0	0 / 0	320 / 0	0 / 0
H	1551	1041 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 54		-122.2	-122.2	0.19 (1)	10.00	M- C	-360 / 92	0.13 (1)
B- C	-2519 / 0		-122.2	-122.2	0.50 (1)	3.81	C- L	0 / 1645	0.41 (1)
C- O	-2920 / 0		-122.2	-122.2	0.36 (1)	3.69	L- D	-509 / 0	0.17 (1)
O- D	-2920 / 0		-122.2	-122.2	0.36 (1)	3.69	L- E	0 / 854	0.21 (1)
D- E	-2920 / 0		-122.2	-122.2	0.58 (1)	3.41	J- E	-1234 / 0	0.42 (1)
E- F	-2311 / 0		-122.2	-122.2	0.53 (1)	3.67	J- F	0 / 1997	0.49 (1)
F- G	-1168 / 0		-122.2	-122.2	0.12 (1)	5.75	I- F	-893 / 0	0.34 (1)
N- B	-2617 / 0		0.0	0.0	0.19 (1)	6.39	B- M	0 / 2018	0.50 (1)
H- G	-2008 / 0		0.0	0.0	0.34 (1)	5.91	I- G	0 / 1439	0.36 (1)
N- P	0 / 0		-28.0	-28.0	0.07 (3)	10.00			
P- M	0 / 0		-28.0	-28.0	0.07 (3)	10.00			
M- Q	0 / 1921		-28.0	-28.0	0.33 (1)	10.00			
Q- L	0 / 1921		-28.0	-28.0	0.33 (1)	10.00			
L- K	0 / 2312		-28.0	-28.0	0.40 (1)	10.00			
K- J	0 / 2312		-28.0	-28.0	0.40 (1)	10.00			
J- I	0 / 869		-28.0	-28.0	0.16 (1)	10.00			
I- H	0 / 0		-28.0	-28.0	0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-235	-235	---	BACK	VERT	TOTAL
L	7-3-8	-1366	-1366	---	BACK	VERT	TOTAL
M	3-11-4	-20	-34	---	BACK	VERT	TOTAL
O	5-0-4	-56	-56	---	BACK	VERT	TOTAL
P	1-11-4	-14	-37	---	BACK	VERT	TOTAL
Q	5-0-4	-20	-34	---	BACK	VERT	TOTAL

DWG NO. TAM46309-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.58 (D-E:1) , BC=0.40 (J-L:1) , WB=0.50 (B-M:1) , SSI=0.29 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.48 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T27A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:10 2017 Page 1
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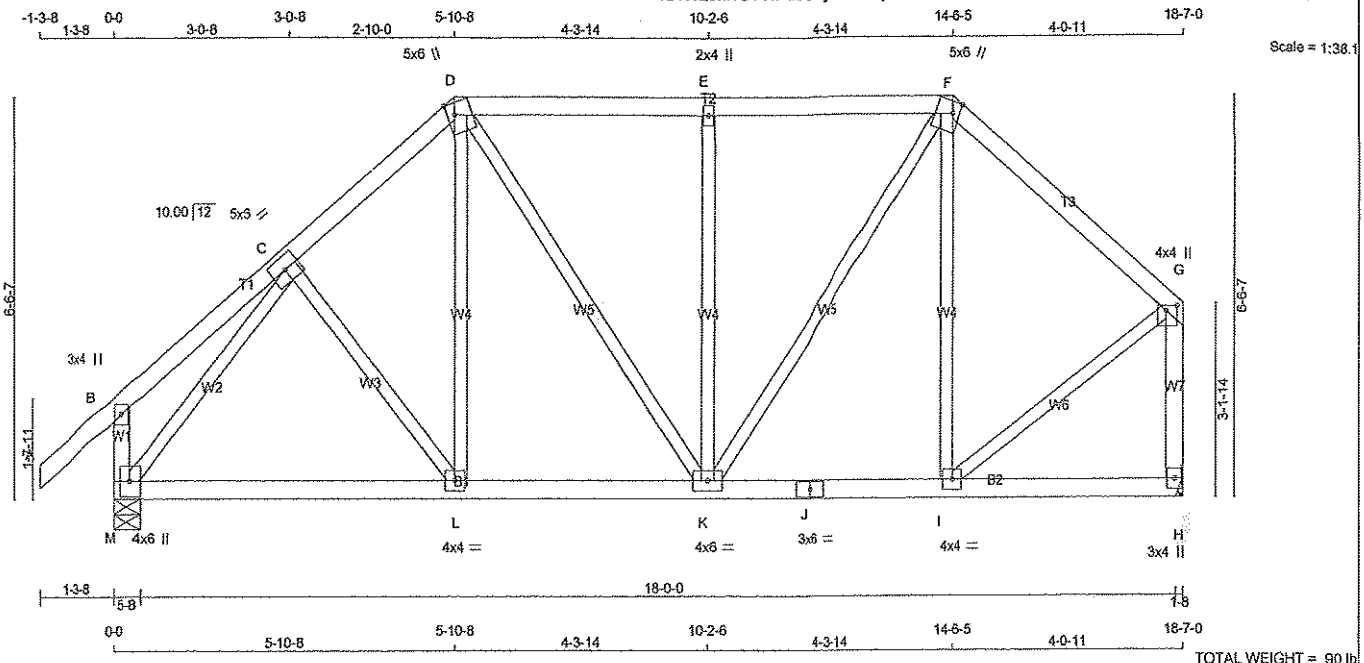

TOTAL WEIGHT = 90 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	4.0	1.00	2.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
M	1565	0	1565	0	0	IN-SX	IN-SX
H	1396	0	1396	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1205	816 / 0	195 / 0	0 / 0	0 / 0	194 / 0	0 / 0
H	1092	711 / 0	195 / 0	0 / 0	0 / 0	186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	C- L	-20 / 87	0.02 (3)
B- C	0 / 25	-122.2	-122.2	0.16 (1)	10.00	L- D	0 / 255	0.06 (2)
C- D	-1217 / 0	-122.2	-122.2	0.13 (1)	5.68	D- K	0 / 276	0.06 (1)
D- E	-1074 / 0	-122.2	-122.2	0.30 (1)	5.73	K- E	-642 / 0	0.45 (1)
E- F	-1074 / 0	-122.2	-122.2	0.30 (1)	5.73	K- F	0 / 617	0.14 (1)
F- G	-950 / 0	-122.2	-122.2	0.27 (1)	6.04	I- F	-352 / 48	0.25 (1)
M- B	-309 / 0	0.0	0.0	0.03 (1)	7.81	M- C	-1506 / 0	0.54 (1)
H- G	-1351 / 0	0.0	0.0	0.24 (1)	6.97	I- G	0 / 679	0.20 (1)
M- L	0 / 926	-28.0	-28.0	0.30 (2)	10.00			
L- K	0 / 916	-28.0	-28.0	0.31 (2)	10.00			
K- J	0 / 722	-28.0	-28.0	0.20 (2)	10.00			
J- I	0 / 722	-28.0	-28.0	0.20 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.13 (2)	10.00			

DWG NO. TAM 46310-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.30 (D-E:1) , BC=0.31 (K-L:2) , WB=0.54 (C-M:1) , SSI=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T28	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:10 2017 Page 1

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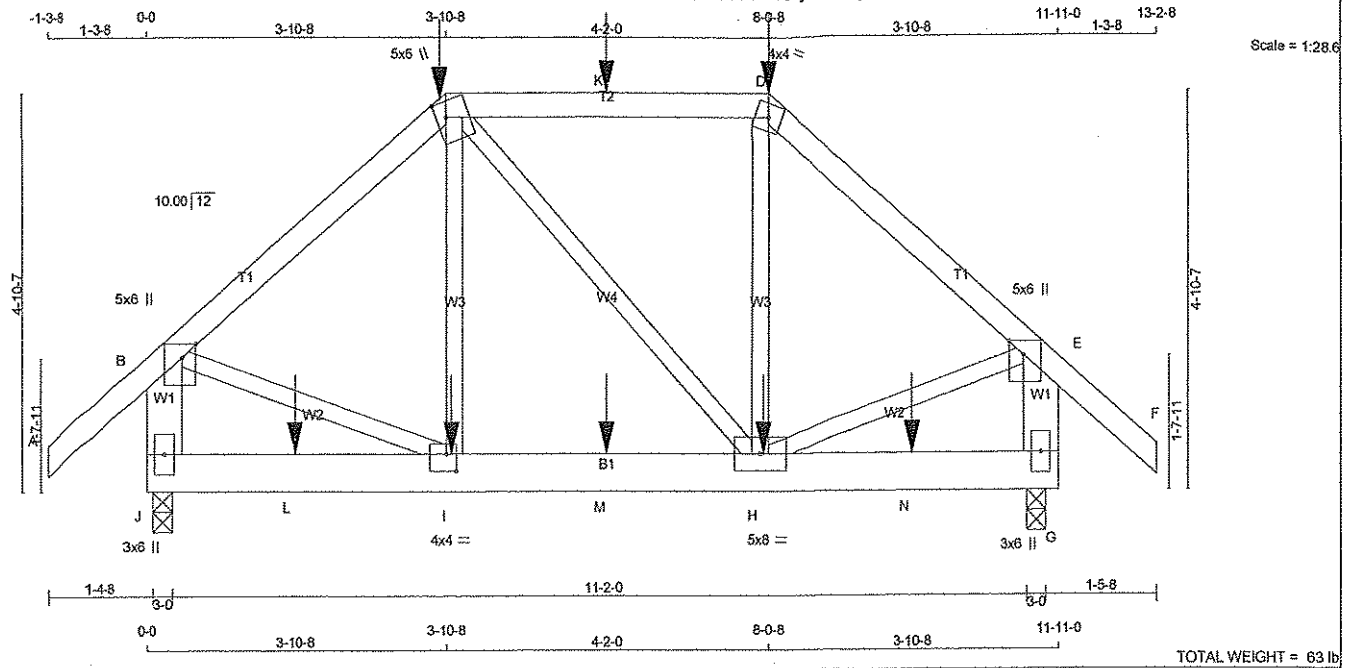

TOTAL WEIGHT = 63 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	2.00	2.25
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	4.0	2.50	1.50
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 235.1 lbs FACTORED DOWN AT 3-10-8, AND 55.6 lbs FACTORED DOWN AT 5-11-8, AND 235.1 lbs FACTORED DOWN AT 8-0-8 ON TOP CHORD, AND 36.6 lbs FACTORED DOWN AT 1-11-4, 34.1 lbs FACTORED DOWN AT 3-11-4, 34.1 lbs FACTORED DOWN AT 5-11-8, AND 34.1 lbs FACTORED DOWN AT 7-11-12, AND 36.6 lbs FACTORED DOWN AT 9-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1371	0	1371	0	0	3-0	3-0
J	1371	0	1371	0	0	3-0	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1054	718 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0
J	1054	718 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 54	-122.2	-122.2	0.19 (1)	10.00	I- C	-87 / 191	0.05 (3)
B- C	-1035 / 0	-122.2	-122.2	0.38 (1)	5.62	C- H	-2 / 2	0.00 (1)
C- K	-790 / 0	-122.2	-122.2	0.51 (1)	5.93	H- D	-90 / 194	0.05 (3)
K- D	-790 / 0	-122.2	-122.2	0.51 (1)	5.93	B- I	0 / 830	0.21 (1)
D- E	-1033 / 0	-122.2	-122.2	0.38 (1)	5.62	H- E	0 / 828	0.20 (1)
E- F	0 / 54	-122.2	-122.2	0.19 (1)	10.00			
J- B	-1315 / 0	0.0	0.0	0.10 (1)	7.81			
G- E	-1313 / 0	0.0	0.0	0.10 (1)	7.81			
J- L	0 / 0	-28.0	-28.0	0.07 (2)	10.00			
L- I	0 / 0	-28.0	-28.0	0.07 (2)	10.00			
I- M	0 / 791	-28.0	-28.0	0.14 (1)	10.00			
M- H	0 / 791	-28.0	-28.0	0.14 (1)	10.00			
H- N	0 / 0	-28.0	-28.0	0.07 (2)	10.00			
N- G	0 / 0	-28.0	-28.0	0.07 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-235	-235	---	BACK	VERT	TOTAL
D	8-0-8	-235	-235	---	BACK	VERT	TOTAL
H	7-11-12	-20	-34	---	BACK	VERT	TOTAL
I	3-11-4	-20	-34	---	BACK	VERT	TOTAL
K	5-11-8	-56	-56	---	BACK	VERT	TOTAL
L	1-11-4	-14	-37	---	BACK	VERT	TOTAL
M	5-11-8	-20	-34	---	BACK	VERT	TOTAL
N	9-11-12	-14	-37	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.51 (C-D:1) , BC=0.14 (H-I:1) , WB=0.21 (B-I:1) , SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.30 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO
9/12/17

DWG NO. TAM 46311-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T29	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:10 2017 Page 1

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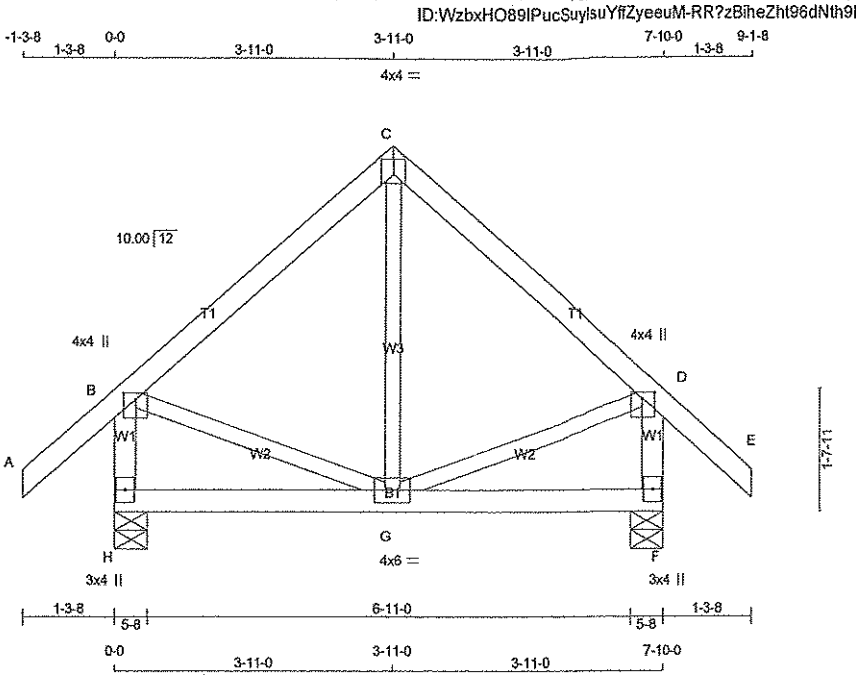

TOTAL WEIGHT = 2 X 37 = 74 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	758	0	758	0	0	5-8	5-8
F	758	0	758	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	574	405 / 0	82 / 0	0 / 0	0 / 0	87 / 0	0 / 0
F	574	405 / 0	82 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.17 (1)	10.00	G- C	-41 / 162	0.04 (3)
B- C	-342 / 0	-122.2	-122.2	0.24 (1)	6.25	B- G	0 / 277	0.06 (1)
C- D	-342 / 0	-122.2	-122.2	0.24 (1)	6.25	G- D	0 / 277	0.06 (1)
D- E	0 / 54	-122.2	-122.2	0.17 (1)	10.00			
H- B	-715 / 0	0.0	0.0	0.08 (1)	7.81			
F- D	-715 / 0	0.0	0.0	0.08 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.12 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.12 (3)	10.00			

DWG NO. TAM4631217
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24 (B-C:1) , BC=0.12 (G-H:3) , WB=0.06 (B-G:1) , SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 287902	DRWG NO.
287902	T29Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 12 16:10:10 2017 Page 1

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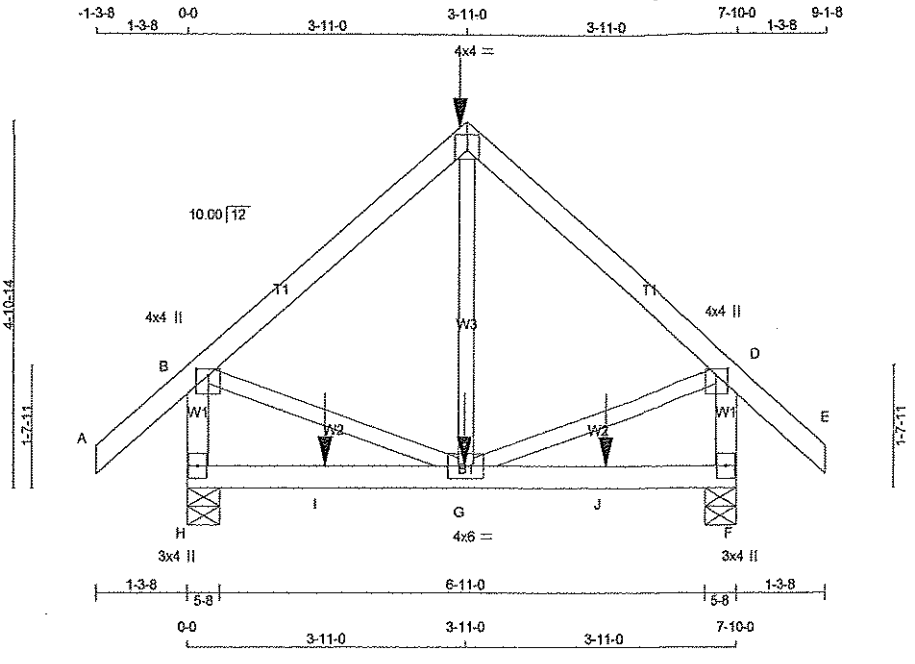

TOTAL WEIGHT = 37 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 421.6 lbs FACTORED DOWN AT 3-11-0 ON TOP CHORD, AND 36.6 lbs FACTORED DOWN AT 1-11-4, AND 34.1 lbs FACTORED DOWN AT 3-10-12, AND 36.6 lbs FACTORED DOWN AT 5-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	992	0	992	0	0	5-8	5-8
F	992	0	992	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	755	527 / 0	110 / 0	0 / 0	0 / 0	117 / 0	0 / 0
F	755	527 / 0	110 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-122.2	-122.2	0.19 (1)	10.00		G- C	-135 / 206	0.05 (3)
B- C	-598 / 0	-122.2	-122.2	0.37 (1)	6.25		B- G	0 / 484	0.12 (1)
C- D	-598 / 0	-122.2	-122.2	0.37 (1)	6.25		G- D	0 / 484	0.12 (1)
D- E	0 / 54	-122.2	-122.2	0.19 (1)	10.00				
H- B	-944 / 0	0.0	0.0	0.11 (1)	7.81				
F- D	-944 / 0	0.0	0.0	0.11 (1)	7.81				
H- I	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
I- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
G- J	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
J- F	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-11-0	-422	-422	---	BACK	VERT	TOTAL
G	3-10-12	-20	-34	---	BACK	VERT	TOTAL
I	1-11-4	-14	-37	---	BACK	VERT	TOTAL
J	5-10-12	-14	-37	---	BACK	VERT	TOTAL

DWG NO. TAM 46313-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.37 (B-C:1) , BC=0.17 (G-H:3) , WB=0.12 (B-G:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.23 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9122
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288236	T90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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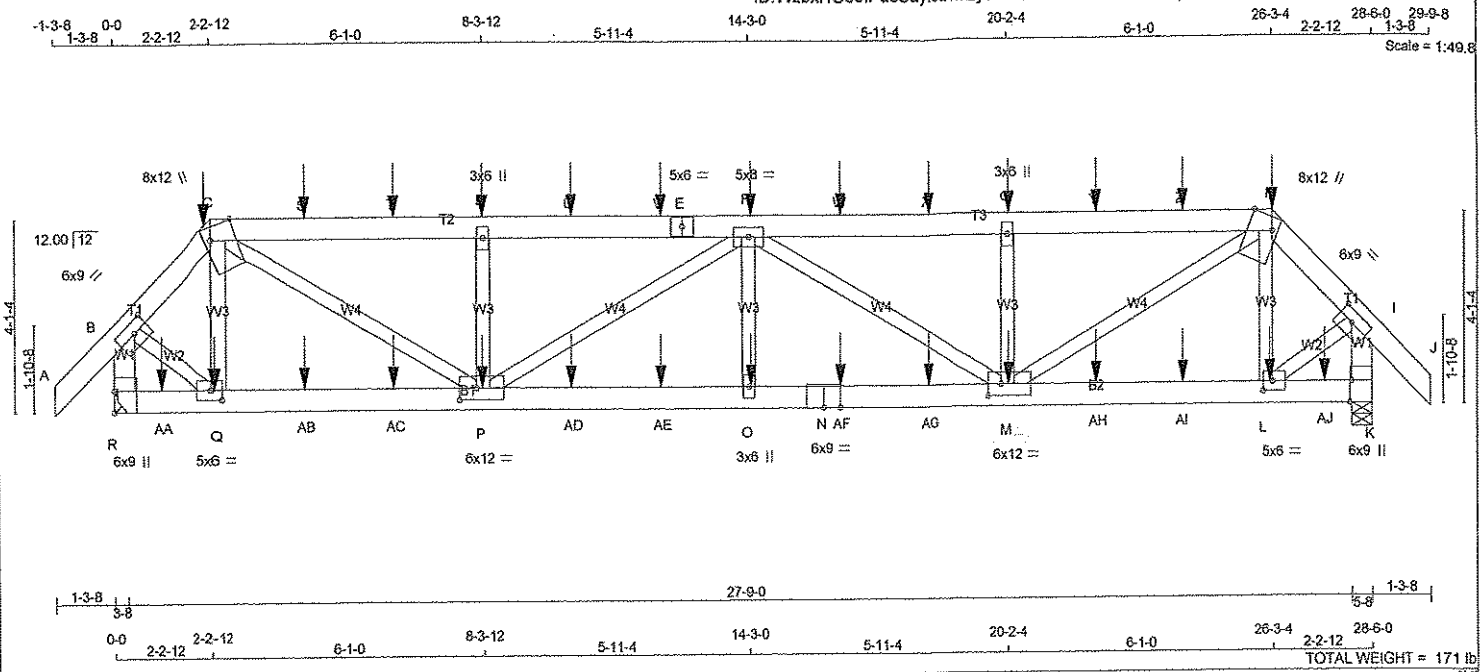

TOTAL WEIGHT = 171 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	8.0	12.0	Edge	6.50
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWWW-t	MT20	5.0	8.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	8.0	12.0	Edge	6.50
I	TMVW-t	MT20	6.0	9.0	2.75	4.00
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWWW-t	MT20	6.0	12.0	3.00	3.75
N	BS-t	MT20	6.0	9.0		
O	BMW+w	MT20	3.0	6.0		
P	BMWWW-t	MT20	6.0	12.0	3.00	4.75
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	3772	0	3772	0	0	IN-SX	IN-SX
						HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
K	3774	0	3774	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2948	1921 / 0	524 / 0	0 / 0	0 / 0	503 / 0	0 / 0
K	2949	1922 / 0	524 / 0	0 / 0	0 / 0	503 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.10 (1)	10.00
B- C	-3157 / 0	-122.2	-122.2	0.11 (1)	4.66
C- S	-5902 / 0	-122.2	-122.2	0.75 (1)	2.86
S- T	-5902 / 0	-122.2	-122.2	0.75 (1)	2.86
T- D	-5902 / 0	-122.2	-122.2	0.75 (1)	2.86
D- U	-5903 / 0	-122.2	-122.2	0.75 (1)	2.86
U- V	-5903 / 0	-122.2	-122.2	0.75 (1)	2.86
V- E	-5903 / 0	-122.2	-122.2	0.75 (1)	2.86
E- F	-5903 / 0	-122.2	-122.2	0.75 (1)	2.86
F- W	-5908 / 0	-122.2	-122.2	0.75 (1)	2.86
W- X	-5908 / 0	-122.2	-122.2	0.75 (1)	2.86
X- G	-5908 / 0	-122.2	-122.2	0.75 (1)	2.86
G- Y	-5908 / 0	-122.2	-122.2	0.75 (1)	2.86
Y- Z	-5908 / 0	-122.2	-122.2	0.75 (1)	2.86
Z- H	-5908 / 0	-122.2	-122.2	0.75 (1)	2.86
H- I	-3159 / 0	-122.2	-122.2	0.11 (1)	4.66
I- J	0 / 63	-122.2	-122.2	0.10 (1)	10.00
R- B	-3799 / 0	0.0	0.0	0.28 (1)	5.45
K- I	-3801 / 0	0.0	0.0	0.28 (1)	5.44
R-AA	0 / 0	-28.0	-28.0	0.12 (2)	10.00
AA- Q	0 / 0	-28.0	-28.0	0.12 (2)	10.00
Q-AB	0 / 2173	-28.0	-28.0	0.28 (1)	10.00
AB-AC	0 / 2173	-28.0	-28.0	0.28 (1)	10.00
AC- P	0 / 2173	-28.0	-28.0	0.28 (1)	10.00
P-AD	0 / 7047	-28.0	-28.0	0.67 (1)	10.00
AD-AE	0 / 7047	-28.0	-28.0	0.67 (1)	10.00
AE- O	0 / 7047	-28.0	-28.0	0.67 (1)	10.00
O- N	0 / 7047	-28.0	-28.0	0.67 (1)	10.00
N-AF	0 / 7047	-28.0	-28.0	0.67 (1)	10.00
AF-AG	0 / 7047	-28.0	-28.0	0.67 (1)	10.00
AG- M	0 / 7047	-28.0	-28.0	0.67 (1)	10.00
M-AH	0 / 2174	-28.0	-28.0	0.28 (1)	10.00
AH-AI	0 / 2174	-28.0	-28.0	0.28 (1)	10.00
AI- L	0 / 2174	-28.0	-28.0	0.28 (1)	10.00
L-AJ	0 / 0	-28.0	-28.0	0.12 (2)	10.00
AJ- K	0 / 0	-28.0	-28.0	0.12 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-898 / 0	0.15 (1)
C- P	0 / 4334	0.77 (1)
P- D	-1265 / 0	0.22 (1)
P- F	-1343 / 0	0.84 (1)
O- F	0 / 504	0.09 (2)
F- M	-1337 / 0	0.84 (1)
M- G	-1269 / 0	0.22 (1)
M- H	0 / 4339	0.77 (1)
L- H	-899 / 0	0.15 (1)
B- Q	0 / 2528	0.45 (1)
L- I	0 / 2530	0.45 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 787 (0.43")

CSI: TC=0.75 (G-H:1) , BC=0.67 (O-P:1) , WB=0.84 (F-P:1) , SSI=0.41 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

PLATE VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.94 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 92077 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM49522-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 2-2-12, 195.5 lbs FACTORED DOWN AT 26-3-4, 150.6 lbs FACTORED DOWN AT 2-2-12, 147.1 lbs FACTORED DOWN AT 4-3-8, 147.1 lbs FACTORED DOWN AT 6-3-8, 147.1 lbs FACTORED DOWN AT 8-3-8, 147.1 lbs FACTORED DOWN AT 10-3-8, 147.1 lbs FACTORED DOWN AT 12-3-8, 147.1 lbs FACTORED DOWN AT 14-3-8, 147.1 lbs FACTORED DOWN AT 16-3-8, 147.1 lbs FACTORED DOWN AT 18-3-8, 147.1 lbs FACTORED DOWN AT 20-2-8, 147.1 lbs FACTORED DOWN AT 22-2-8, AND 147.1 lbs FACTORED DOWN AT 24-2-8, AND 150.6 lbs FACTORED DOWN AT 26-3-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-3-8, 69.9 lbs FACTORED DOWN AT 4-3-8, 69.9 lbs FACTORED DOWN AT 6-3-8, 69.9 lbs FACTORED DOWN AT 8-3-8, 69.9 lbs FACTORED DOWN AT 10-3-8, 69.9 lbs FACTORED DOWN AT 12-3-8, 69.9 lbs FACTORED DOWN AT 14-3-8, 69.9 lbs FACTORED DOWN AT 16-3-8, 69.9 lbs FACTORED DOWN AT 18-3-8, 69.9 lbs FACTORED DOWN AT 20-2-8, 69.9 lbs FACTORED DOWN AT 22-2-8, 69.9 lbs FACTORED DOWN AT 24-2-8, AND 69.9 lbs FACTORED DOWN AT 26-2-8, AND 69.9 lbs FACTORED DOWN AT 27-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-13	-14	—	FRONT	VERT	DEAD
C	2-2-12	-151	-151	—	BACK	VERT	TOTAL
C	2-2-12	-183	-183	—	FRONT	VERT	SNOW
D	8-3-8	-147	-147	—	BACK	VERT	TOTAL
F	14-3-8	-147	-147	—	BACK	VERT	TOTAL
G	20-2-8	-147	-147	—	BACK	VERT	TOTAL
H	26-3-4	-13	-14	—	FRONT	VERT	DEAD
H	26-3-4	-151	-151	—	BACK	VERT	TOTAL
H	26-3-4	-183	-183	—	FRONT	VERT	SNOW
L	26-2-8	-40	-70	—	BACK	VERT	TOTAL
M	20-2-8	-40	-70	—	BACK	VERT	TOTAL
O	14-3-8	-40	-70	—	BACK	VERT	TOTAL
P	8-3-8	-40	-70	—	BACK	VERT	TOTAL
Q	2-3-8	-40	-70	—	BACK	VERT	TOTAL
S	4-3-8	-147	-147	—	BACK	VERT	TOTAL
T	6-3-8	-147	-147	—	BACK	VERT	TOTAL
U	10-3-8	-147	-147	—	BACK	VERT	TOTAL
V	12-3-8	-147	-147	—	BACK	VERT	TOTAL
W	16-3-8	-147	-147	—	BACK	VERT	TOTAL
X	18-3-8	-147	-147	—	BACK	VERT	TOTAL
Y	22-2-8	-147	-147	—	BACK	VERT	TOTAL
Z	24-2-8	-147	-147	—	BACK	VERT	TOTAL
AA	1-0-12	-40	-70	—	BACK	VERT	TOTAL
AB	4-3-8	-40	-70	—	BACK	VERT	TOTAL
AC	6-3-8	-40	-70	—	BACK	VERT	TOTAL
AD	10-3-8	-40	-70	—	BACK	VERT	TOTAL
AE	12-3-8	-40	-70	—	BACK	VERT	TOTAL
AF	16-3-8	-40	-70	—	BACK	VERT	TOTAL
AG	18-3-8	-40	-70	—	BACK	VERT	TOTAL
AH	22-2-8	-40	-70	—	BACK	VERT	TOTAL
AI	24-2-8	-40	-70	—	BACK	VERT	TOTAL
AJ	27-5-4	-40	-70	—	BACK	VERT	TOTAL

DWG NO. TAM-T90-22-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288238	T230	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 18 15:09:18 2017 Page 1
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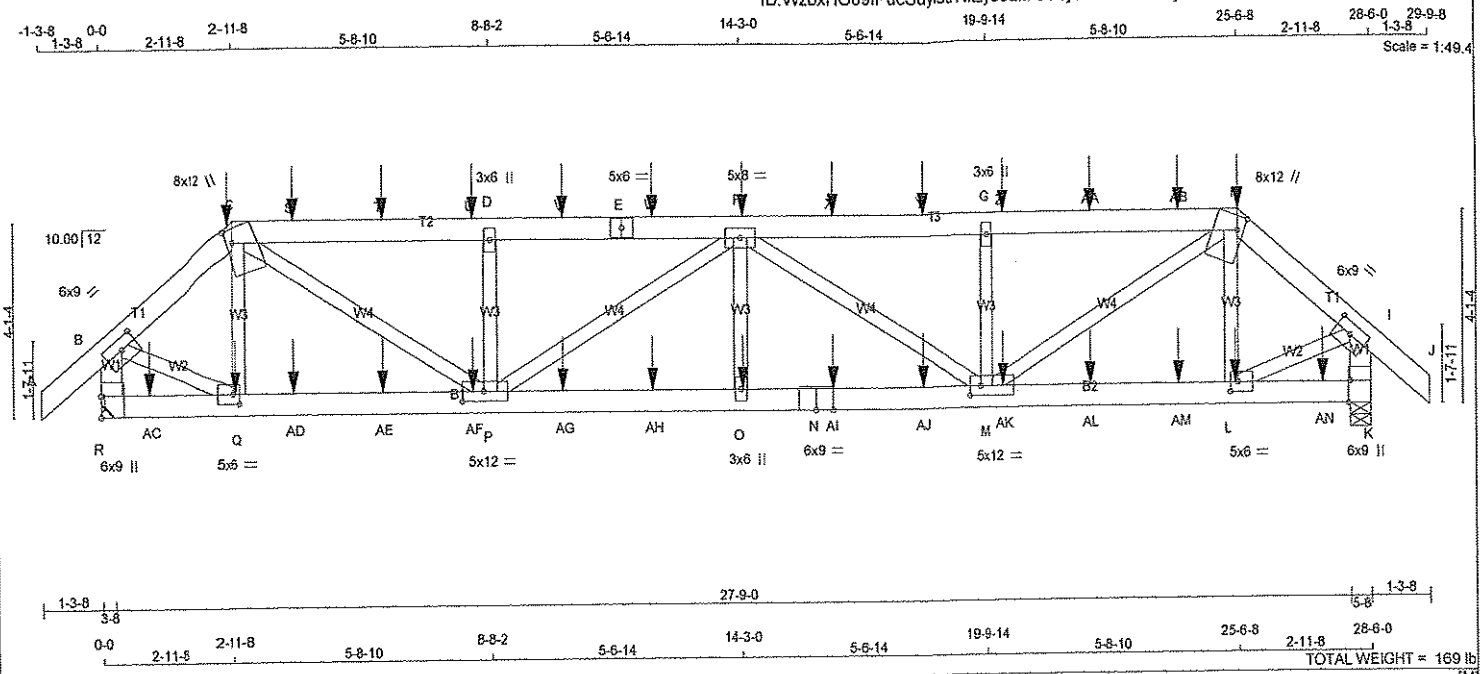


TOTAL WEIGHT = 169 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	8.0	12.0	3.50	1.75
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWWW-t	MT20	5.0	8.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	8.0	12.0	3.50	1.75
I	TMW-t	MT20	6.0	9.0	2.75	4.25
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWWW-t	MT20	5.0	12.0	2.50	2.75
N	BS-t	MT20	6.0	9.0		
O	BMW+w	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	12.0	2.50	5.75
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	3770	0	3770	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 3-8	IN-SX
K	3772	0	3772	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2946	1920 / 0	524 / 0	0 / 0	0 / 0	503 / 0	0 / 0
K	2948	1921 / 0	524 / 0	0 / 0	0 / 0	503 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 56	-122.2	-122.2	0.10 (1)	10.00	
B- C	-3888 / 0	-122.2	-122.2	0.17 (1)	4.30	
C- S	-6096 / 0	-122.2	-122.2	0.71 (1)	2.87	
S- T	-6096 / 0	-122.2	-122.2	0.71 (1)	2.87	
T- U	-6096 / 0	-122.2	-122.2	0.71 (1)	2.87	
U- D	-6096 / 0	-122.2	-122.2	0.71 (1)	2.87	
D- V	-6096 / 0	-122.2	-122.2	0.70 (1)	2.87	
V- E	-6096 / 0	-122.2	-122.2	0.70 (1)	2.87	
E- W	-6096 / 0	-122.2	-122.2	0.70 (1)	2.87	
W- F	-6096 / 0	-122.2	-122.2	0.70 (1)	2.87	
F- X	-6101 / 0	-122.2	-122.2	0.70 (1)	2.87	
X- Y	-6101 / 0	-122.2	-122.2	0.70 (1)	2.87	
Y- G	-6101 / 0	-122.2	-122.2	0.70 (1)	2.87	
G- Z	-6101 / 0	-122.2	-122.2	0.71 (1)	2.87	
Z-AA	-6101 / 0	-122.2	-122.2	0.71 (1)	2.87	
AA-AB	-6101 / 0	-122.2	-122.2	0.71 (1)	2.87	
AB- H	-6101 / 0	-122.2	-122.2	0.71 (1)	2.87	
H- I	-3691 / 0	-122.2	-122.2	0.17 (1)	4.30	
I- J	0 / 56	-122.2	-122.2	0.10 (1)	10.00	
R- B	-3748 / 0	0.0	0.0	0.27 (1)	5.48	
K- I	-3750 / 0	0.0	0.0	0.27 (1)	5.48	
R-AC	0 / 0	-28.0	-28.0	0.10 (2)	10.00	
AC- Q	0 / 0	-28.0	-28.0	0.10 (2)	10.00	
Q-AD	0 / 2789	-28.0	-28.0	0.30 (1)	10.00	
AD-AE	0 / 2789	-28.0	-28.0	0.30 (1)	10.00	
AE-AF	0 / 2789	-28.0	-28.0	0.30 (1)	10.00	
AF- P	0 / 2789	-28.0	-28.0	0.30 (1)	10.00	
P-AG	0 / 7101	-28.0	-28.0	0.66 (1)	10.00	
AG-AH	0 / 7101	-28.0	-28.0	0.66 (1)	10.00	
AH- O	0 / 7101	-28.0	-28.0	0.66 (1)	10.00	
O- N	0 / 7101	-28.0	-28.0	0.66 (1)	10.00	
N-AI	0 / 7101	-28.0	-28.0	0.66 (1)	10.00	
AI-AJ	0 / 7101	-28.0	-28.0	0.66 (1)	10.00	
AJ- M	0 / 7101	-28.0	-28.0	0.66 (1)	10.00	
M-AK	0 / 2790	-28.0	-28.0	0.30 (1)	10.00	
AK-AL	0 / 2790	-28.0	-28.0	0.30 (1)	10.00	
AL-AM	0 / 2790	-28.0	-28.0	0.30 (1)	10.00	
AM- L	0 / 2790	-28.0	-28.0	0.30 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-709 / 58	0.12 (1)
C- P	0 / 3912	0.69 (1)
P- D	-1200 / 0	0.21 (1)
P- F	-1201 / 0	0.67 (1)
O- F	0 / 470	0.08 (2)
F- M	-1194 / 0	0.66 (1)
M- G	-1205 / 0	0.21 (1)
M- H	0 / 3916	0.69 (1)
L- H	-710 / 57	0.12 (1)
B- Q	0 / 2993	0.53 (1)
L- I	0 / 2995	0.53 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 809 (0.42")

CSI: TC=0.71 (G-H:1) , BC=0.66 (O-P:1) , WB=0.69 (H-M:1) , SSI=0.47 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.95 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47525-17
STRUCTURAL
COMPONENT ONLY

Pg 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
288238	T230	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 2-11-8, 195.5 lbs FACTORED DOWN AT 25-6-8, 150.6 lbs FACTORED DOWN AT 2-11-8, 147.1 lbs FACTORED DOWN AT 4-3-8, 147.1 lbs FACTORED DOWN AT 6-3-8, 147.1 lbs FACTORED DOWN AT 8-3-8, 147.1 lbs FACTORED DOWN AT 10-3-8, 147.1 lbs FACTORED DOWN AT 12-3-8, 147.1 lbs FACTORED DOWN AT 14-3-8, 147.1 lbs FACTORED DOWN AT 16-3-8, 147.1 lbs FACTORED DOWN AT 18-3-8, 147.1 lbs FACTORED DOWN AT 20-2-8, 147.1 lbs FACTORED DOWN AT 22-2-8, AND 147.1 lbs FACTORED DOWN AT 24-2-8, AND 150.6 lbs FACTORED DOWN AT 25-6-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 4-3-8, 69.9 lbs FACTORED DOWN AT 6-3-8, 69.9 lbs FACTORED DOWN AT 8-3-8, 69.9 lbs FACTORED DOWN AT 10-3-8, 69.9 lbs FACTORED DOWN AT 12-3-8, 69.9 lbs FACTORED DOWN AT 14-3-8, 69.9 lbs FACTORED DOWN AT 16-3-8, 69.9 lbs FACTORED DOWN AT 18-3-8, 69.9 lbs FACTORED DOWN AT 20-2-8, 69.9 lbs FACTORED DOWN AT 22-2-8, 69.9 lbs FACTORED DOWN AT 24-2-8, AND 69.9 lbs FACTORED DOWN AT 25-5-12, AND 69.9 lbs FACTORED DOWN AT 27-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MEMB. FR-TO	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-AN	0 / 0	-28.0	-28.0	0.10 (2)		10.00		
AN- K	0 / 0	-28.0	-28.0	0.10 (2)		10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-13	-14	---	FRONT	VERT	DEAD
C	2-11-8	-151	-151	---	BACK	VERT	TOTAL
C	2-11-8	-183	-183	---	FRONT	VERT	SNOW
F	14-3-8	-147	-147	---	BACK	VERT	TOTAL
H	25-6-8	-13	-14	---	FRONT	VERT	DEAD
H	25-6-8	-151	-151	---	BACK	VERT	TOTAL
H	25-6-8	-183	-183	---	FRONT	VERT	SNOW
L	25-5-12	-40	-70	---	BACK	VERT	TOTAL
O	14-3-8	-40	-70	---	BACK	VERT	TOTAL
Q	3-0-4	-40	-70	---	BACK	VERT	TOTAL
S	4-3-8	-147	-147	---	BACK	VERT	TOTAL
T	6-3-8	-147	-147	---	BACK	VERT	TOTAL
U	8-3-8	-147	-147	---	BACK	VERT	TOTAL
V	10-3-8	-147	-147	---	BACK	VERT	TOTAL
W	12-3-8	-147	-147	---	BACK	VERT	TOTAL
X	16-3-8	-147	-147	---	BACK	VERT	TOTAL
Y	18-3-8	-147	-147	---	BACK	VERT	TOTAL
Z	20-2-8	-147	-147	---	BACK	VERT	TOTAL
AA	22-2-8	-147	-147	---	BACK	VERT	TOTAL
AB	24-2-8	-147	-147	---	BACK	VERT	TOTAL
AC	1-0-12	-40	-70	---	BACK	VERT	TOTAL
AD	4-3-8	-40	-70	---	BACK	VERT	TOTAL
AE	6-3-8	-40	-70	---	BACK	VERT	TOTAL
AF	8-3-8	-40	-70	---	BACK	VERT	TOTAL
AG	10-3-8	-40	-70	---	BACK	VERT	TOTAL
AH	12-3-8	-40	-70	---	BACK	VERT	TOTAL
AI	16-3-8	-40	-70	---	BACK	VERT	TOTAL
AJ	18-3-8	-40	-70	---	BACK	VERT	TOTAL
AK	20-2-8	-40	-70	---	BACK	VERT	TOTAL
AL	22-2-8	-40	-70	---	BACK	VERT	TOTAL
AM	24-2-8	-40	-70	---	BACK	VERT	TOTAL
AN	27-5-4	-40	-70	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM47525-17
STRUCTURAL
COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

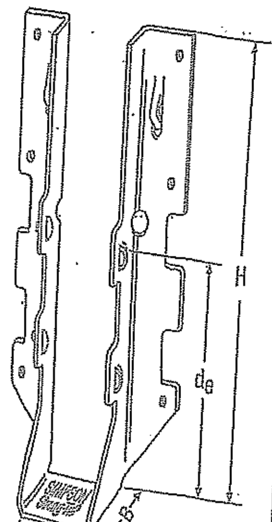
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

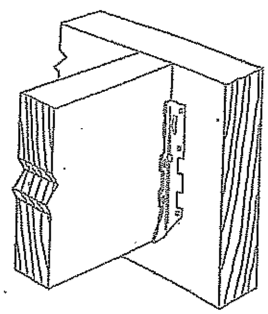
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.

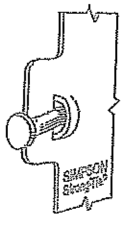


LUS28


Typical LUS Installation

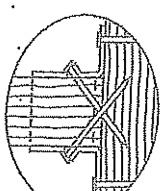
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 13/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 5/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2376
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 5/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2376

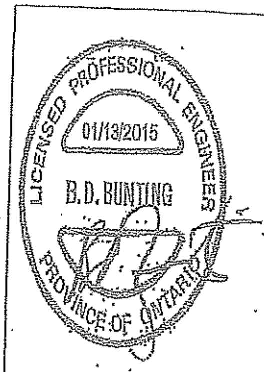
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

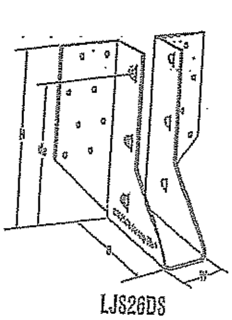
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

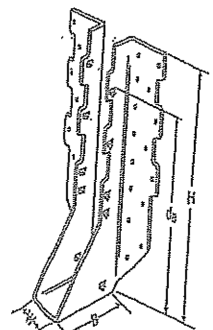
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

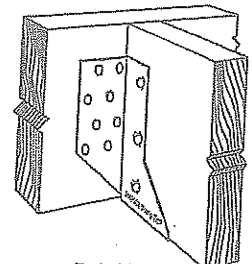
- See current catalogue for options



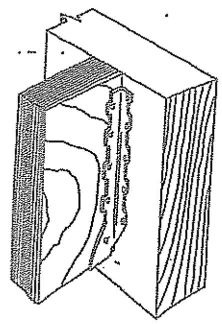
LJS26DS



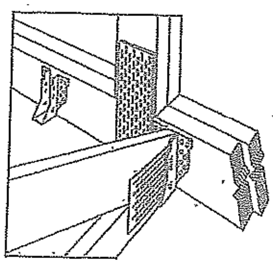
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



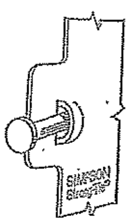
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

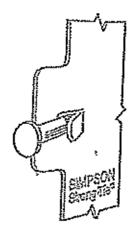
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1⁹⁄₁₆	6	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6⁵⁄₃₂	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₁₆	3	7³¹⁄₃₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

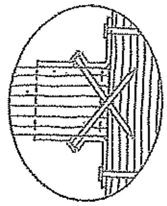


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

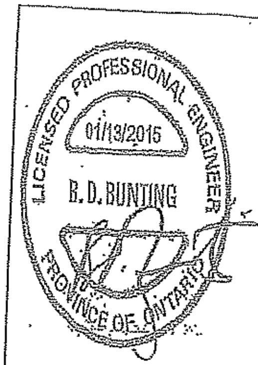
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



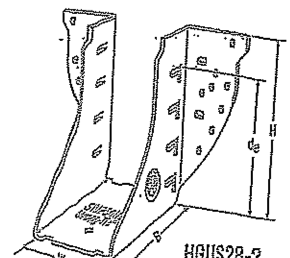
Double Shear Nailing Top View.



HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



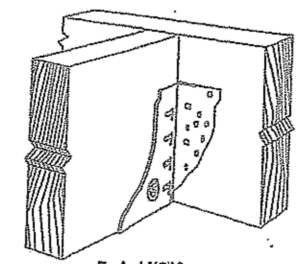
HGUS28-2

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

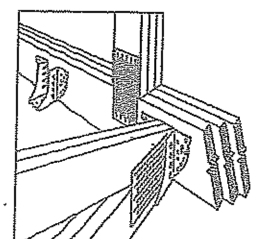

Typical HGUS Installation

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

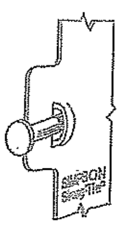
OPTIONS:

- See current catalogue for options.


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

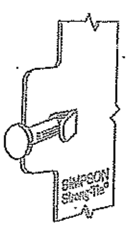
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5¼	5	4¹³⁄₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7⁷⁄₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⁹⁄₁₆	7⁷⁄₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⁹⁄₁₆	9⁵⁄₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

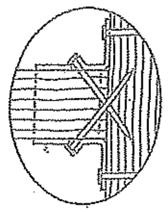


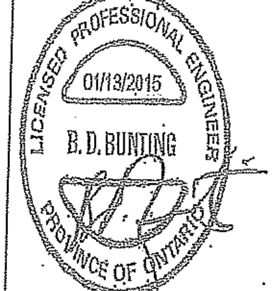
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580

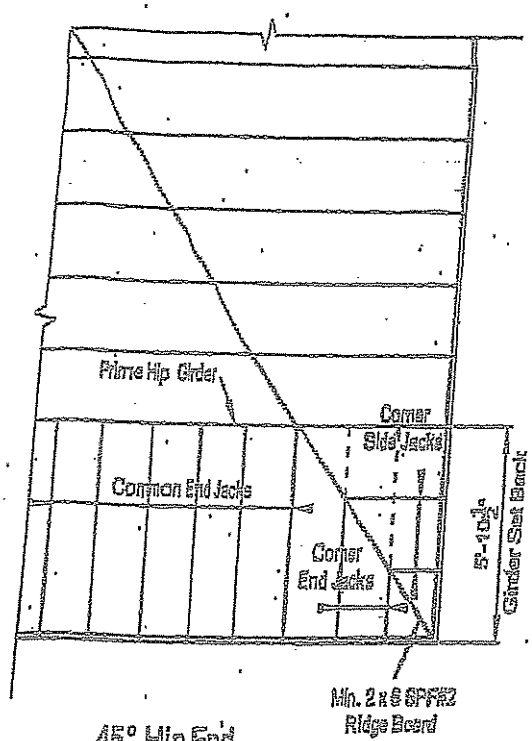


Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



MICRO CITY
ENGINEERING SERVICES INC.
TEL: (619) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

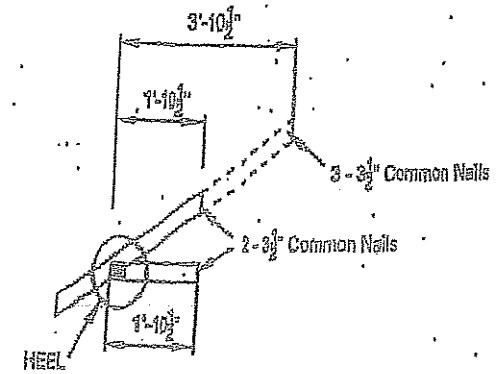
TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

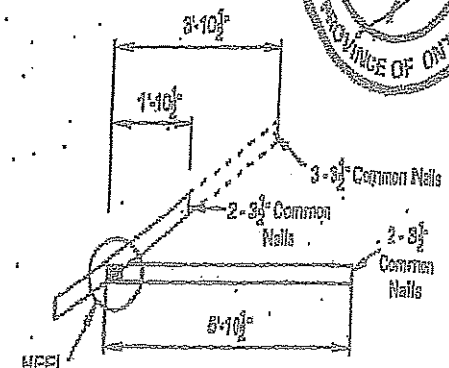
TOP CHORD LIVE LOAD	:	39.4 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F
TOTAL LOAD		59.9 P.S.F.

DWG NO T1M 9495.1i
STRUCTURAL COMPONENT ONLY

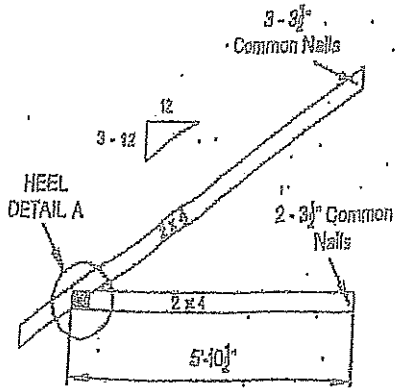
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



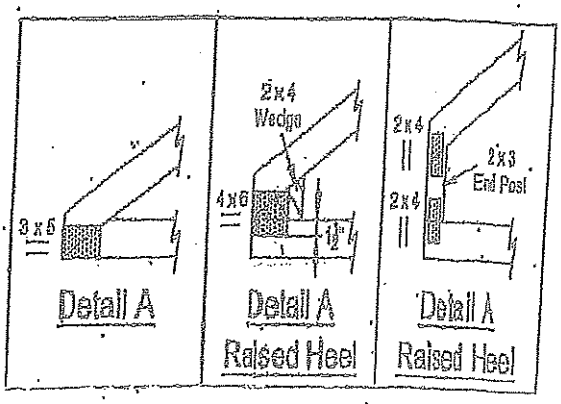
Corner Side Jacks



Corner End Jacks



Common End Jacks



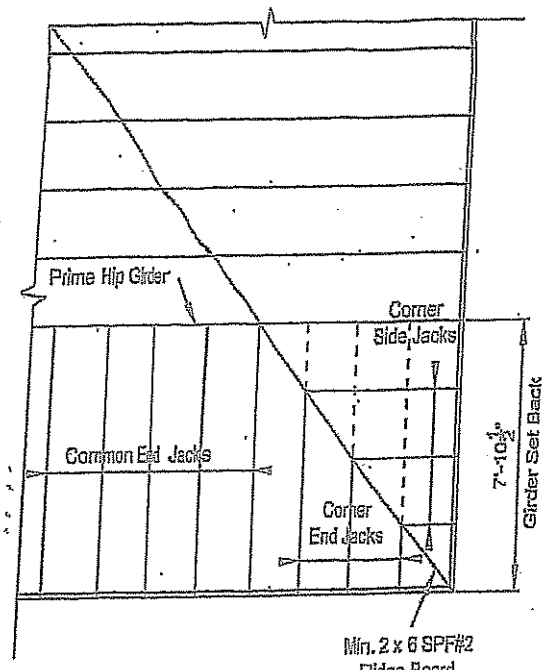
Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



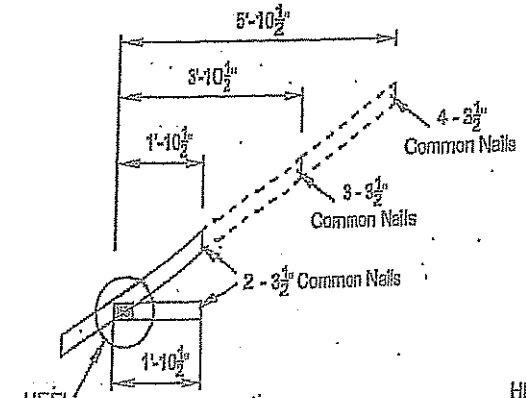
45° Hip End

DESIGN LOAD:

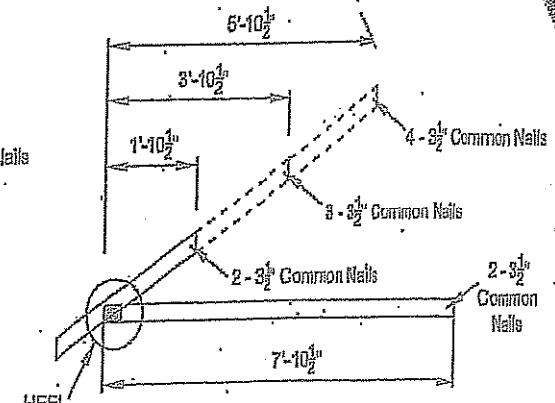
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO THM 3503 14
STRUCTURAL COMPONENT ONLY

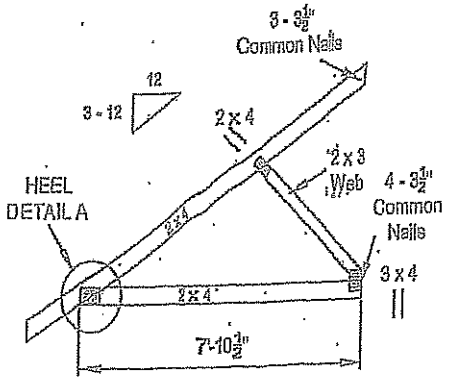
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO



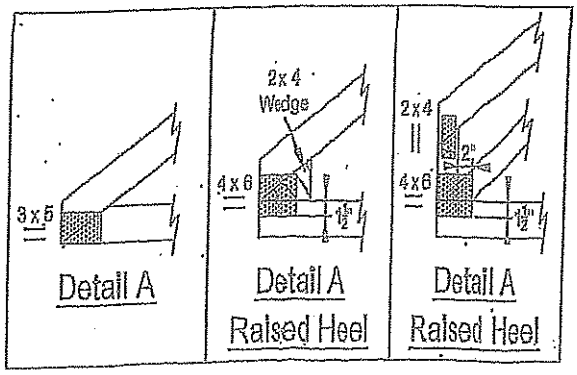
Corner Side Jacks



Corner End Jacks



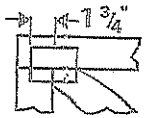
Common End Jacks



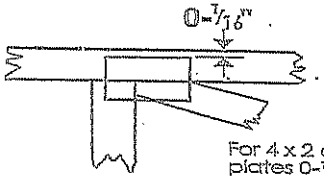
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

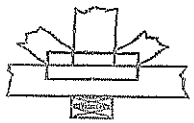
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

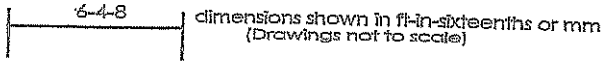
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

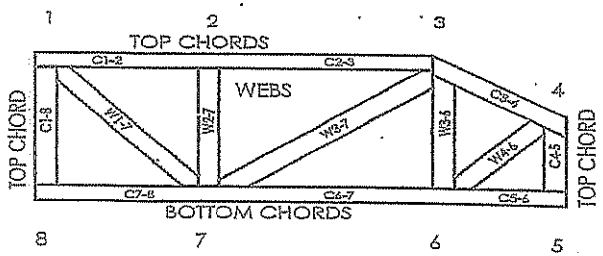
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS
CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO
DWG NO TAM 54974/14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.