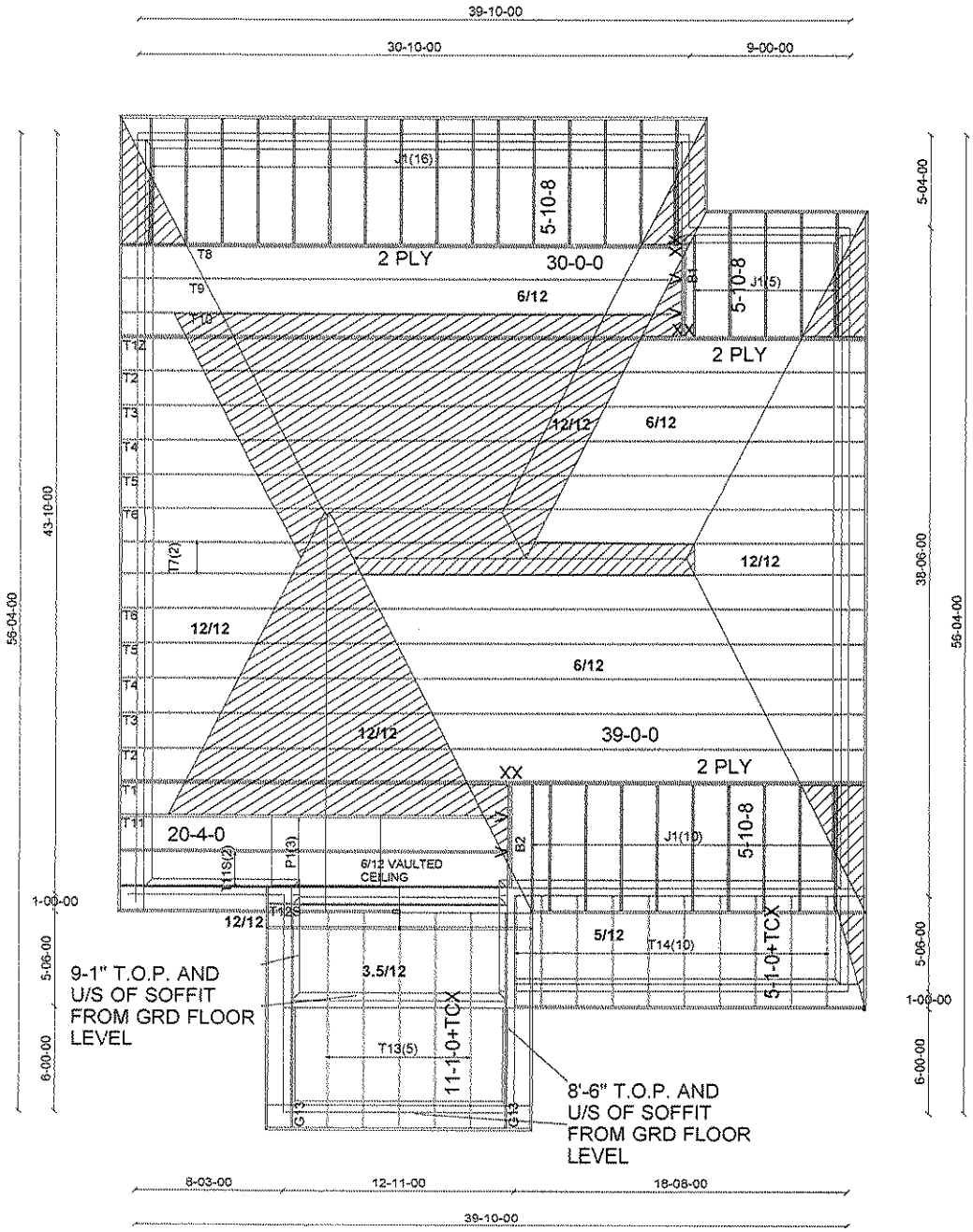


NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

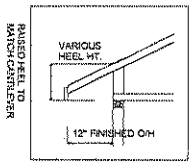
ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
HGUS26-2 - (XX)

CONV.
FRAMING

B1,B2 -DENOTES
2-2X10 SPF#2
(FLUSH)

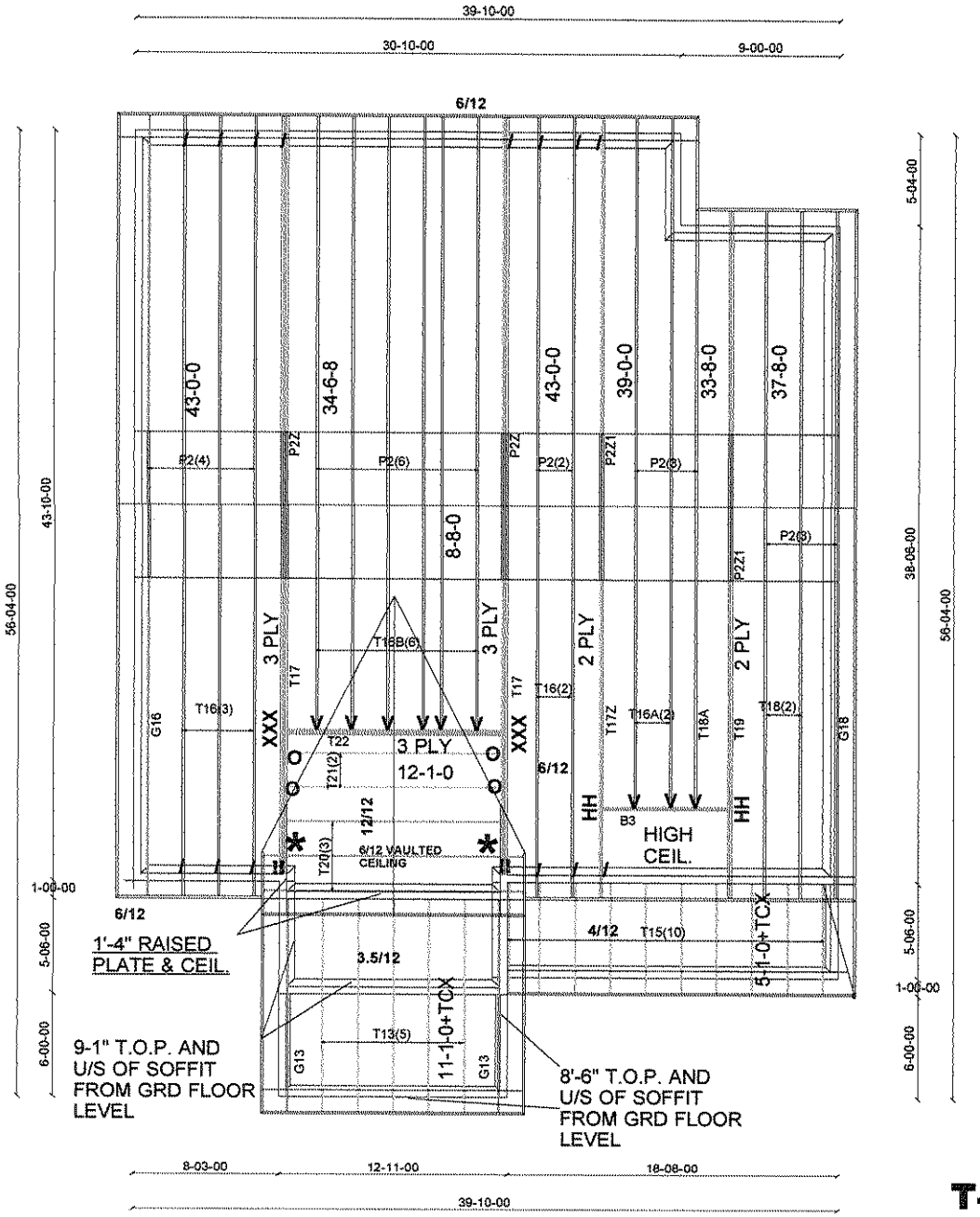


Town of Innisfil Certified Model
01/12/2017 4:25:52 PM kgervais

M11,031

TAMARACK LUMBER INC.	Job Track: **42067**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **S48-1 GOLDENEYE 1 A**
	Layout ID: **288223**	Project: ALCONA SHORES	C:\MITEK\CA742A\JOBS\ B13000000006
	Plan Log: **88256**	Date: **9/30/2013** Designer: **PETER/ELLA**	Mitek ver 7.4.2

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.



NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

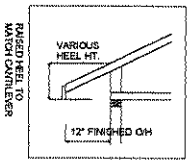
HARDWARE:

LJS26DS - (V)
LUS24 -(O)
HGUS28-3 - (XXX)
HGUS46 -(HH)
H2.5A -(/)
TS12 -(!!)

CONV. FRAMING

B3-DENOTES
2-1 3/4X9 1/2"LVLBM
(FLUSH)

DENOTES:
1'-4" KNEE WALL
NAILED TO B.C. OF
GIRDER *



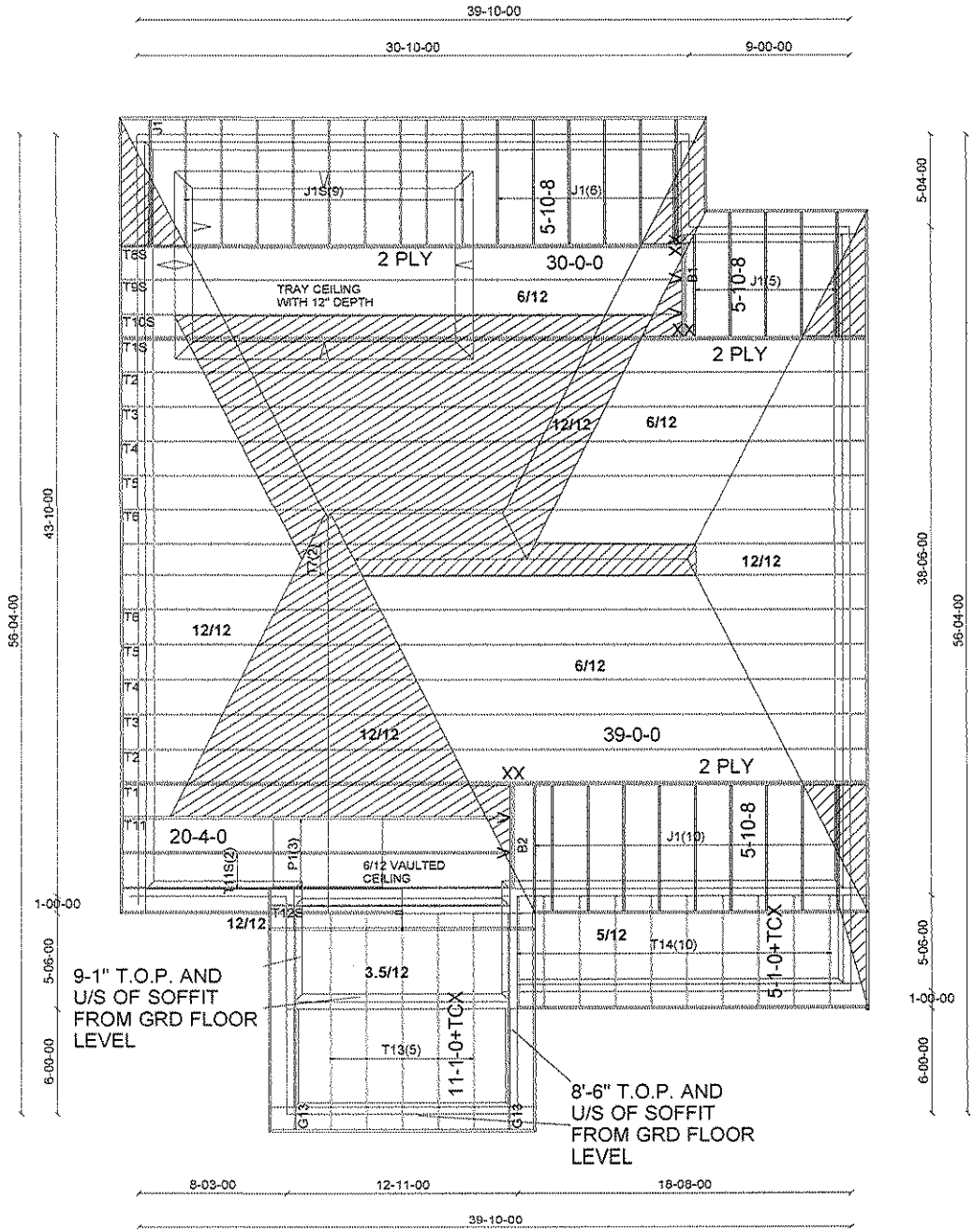
Town of Innisfil Certified Model
02/12/2017 9:08:17 AM kgervais

T-170681

m11,031

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **42067**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **S48-1 GOLDENEYE 1 B**
	Layout ID: **288225**	Project: ALCONA SHORES	C:\MITEK\CA742A\JOBS\B13000000006
	Plan Log: **88256**	Date: **9/30/2013** Designer: **PETER/ELLA**	Mitek ver 7.4.2

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.



NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

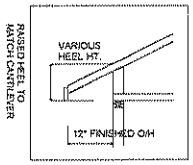
ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR
WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
HGUS26-2 - (XX)

CONV.
FRAMING

B1,B2 -DENOTES
2-2X10 SPF#2
(FLUSH)



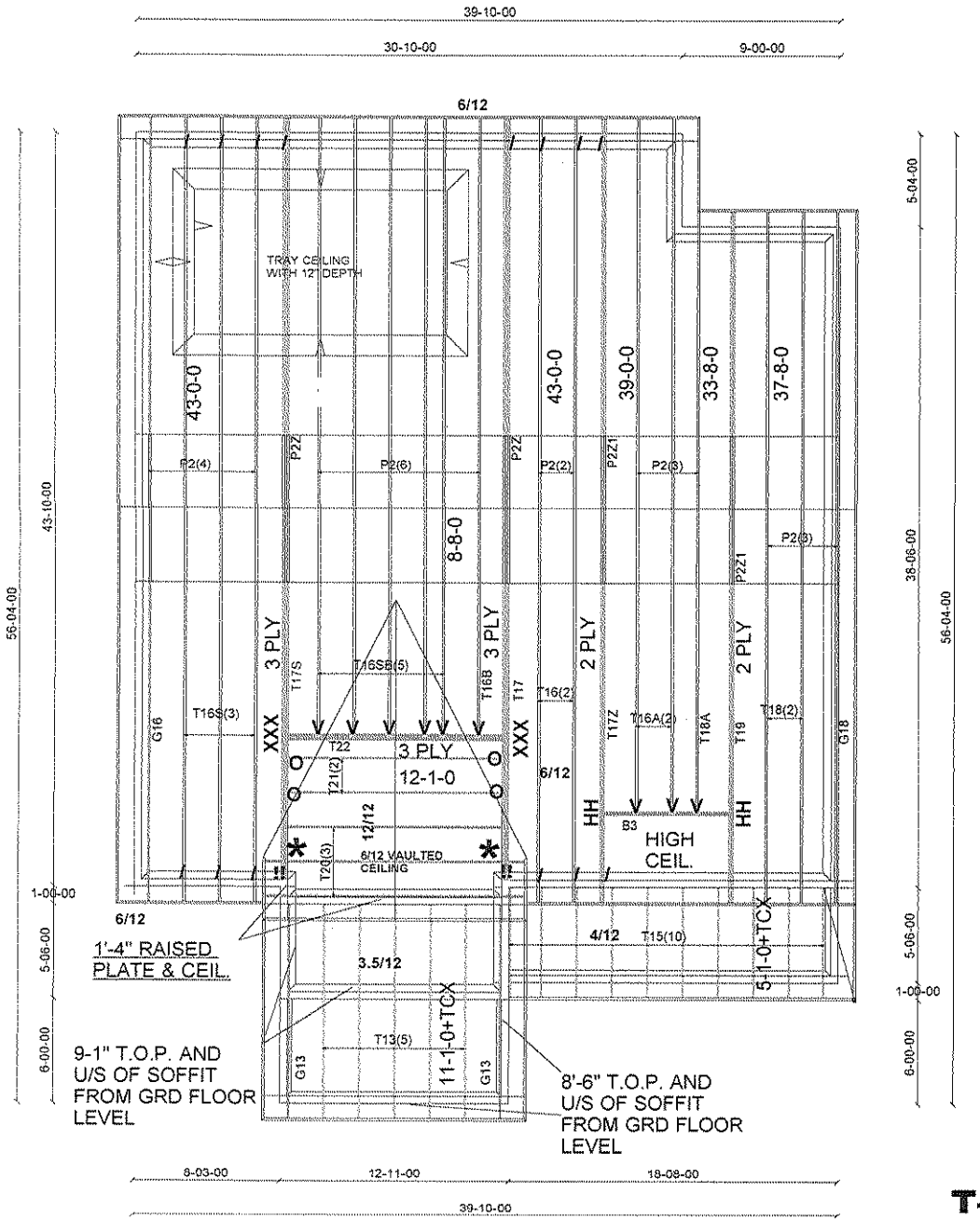
Town of Innisfil Certified Model
02/12/2017 9:08:22 AM kgervais

m11,031

TAMARACK LUMBER INC.	Job Track: 42067	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: S48-1 GOLDENEYE 1 A +OPT. COFF
	Layout ID: 273415	Project: ALCONA SHORES	Quote Folder
	Plan Log: 88256	Date: 9/30/2013 Designer: PETER/ELLA	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.4.2



NOTES:
TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS
GRD SNOW LD 54.34 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

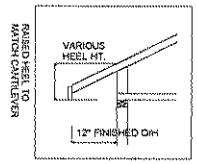
HARDWARE:

LJS26DS - (V)
LUS24 -(O)
HGUS28-3 - (XXX)
HGUS46 -(HH)
H2.5A -(/)
TS12 -(!!)

CONV. FRAMING

B3-DENOTES
2-1 3/4X9 1/2"LVLBM (FLUSH)

DENOTES:
1'-4" KNEE WALL NAILED TO B.C. OF GIRDER *



Town of Innisfil Certified Model
02/12/2017 9:08:30 AM kgervais

T-170681

M11,031

TAMARACK LUMBER INC	Job Track: **42067**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **S48-1 GOLDENEYE 1 B +OPT. COFF.** Quote Folder
	Layout ID: **273416**	Project: ALCONA SHORES	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Plan Log: **88256**	Date: **9/30/2013** Designer: **PETER/ELLA**	Mitek ver 7.4.2

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288223 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOW SUB-BUILDER:
MODEL: S48-1 GOLDENEYE1 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1 / HIP GIRDER	12.00 / 0.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	462.90 / 285.34		
	1 / 2 Ply	T1Z / HIP GIRDER	12.00 / 0.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	462.90 / 285.34		
	2	T2 / HIP	12.00 / 0.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	409.88 / 250.66		
	2	T3 / HIP	12.00 / 0.00	39-00-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	363.42 / 229.34		
	2	T4 / HIP	12.00 / 0.00	39-00-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	375.48 / 235.00		
	2	T5 / HIP	12.00 / 0.00	39-00-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	428.26 / 264.00		
	2	T6 / HIP	12.00 / 0.00	39-00-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	444.56 / 275.66		
	2	T7 / HIP	12.00 / 0.00	39-00-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	429.04 / 260.66		
	1 / 2 Ply	T8 / HIP GIRDER	12.00 / 0.00	30-00-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	355.16 / 220.00		
	1	T9 / HIP	12.00 / 0.00	30-00-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	129.31 / 83.17		
	1	T10 / HIP	12.00 / 0.00	30-00-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	136.80 / 87.33		
	1	T11 / PIGGYBACK	12.00 / 0.00	20-04-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	109.23 / 69.33		
	2	T11S / PIGGYBACK	12.00 / 6.00	20-04-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	239.20 / 154.34		
	1	T12S / ROOF	12.00 / 6.00	12-01-00	07-11-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	65.05 / 42.00		
	5	T13 / MONOPITCH	3.50 / 0.00	11-01-00	04-03-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-10-07 / 03-08-15	242.25 / 159.15		
	2	G13 / MONOPITCH	3.50 / 0.00	11-01-00	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-10-07 / 03-06-04	77.08 / 49.66		
	10	T14 / MONO	5.00 / 0.00	05-01-00	02-09-05	2 X 4	2 X 4	01-03-08 / 00-00-00	00-04-01 / 02-09-05	187.60 / 123.30		
	3	P1 / PIGGYBACK	12.00 / 0.00	04-11-10	02-11-00	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-00 / 00-00-00	46.44 / 30.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 288223 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS **SUB-BUILDER:**
MODEL: S48-1 GOLDENEYE1 **ELEVATION:** A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	31	J1 / JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	520.49 / 330.77		

TOTAL # TRUSS= **75.00** TOTAL BFT OF ALL TRUSSES= **3435.05** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5485.05** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

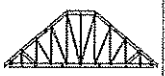
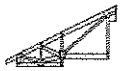
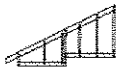
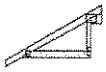
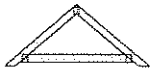
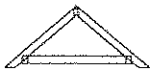
Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288225 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS SUB-BUILDER:
MODEL: S48-1 GOLDENEYE 1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T16 PIGGYBACK	6.00 / 0.00	43-00-00	09-09-00	2 X 6	2 X 6	01-03-08 / 01-03-08	00-00-00 / 00-00-00	1333.40 / 797.50		
	1	G16 PIGGYBACK	6.00 / 0.00	43-00-00	09-09-00	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	241.51 / 151.00		
	2	T16A PIGGYBACK	6.00 / 0.00	39-00-00	09-09-00	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 03-02-00	475.28 / 290.00		
	6	T16B PIGGYBACK	6.00 / 0.00	34-06-08	09-09-00	2 X 6	2 X 6	01-03-08 / 00-00-00	00-00-00 / 00-00-00	1314.36 / 792.00		
	2 / 3 Ply	T17 PIGGYBACK	6.00 / 0.00	43-00-00	09-09-00	2 X 8	2 X 8	01-03-08 / 01-03-08	01-02-00 / 01-02-00	2144.88 / 1279.98		
	1 / 2 Ply	T17Z PIGGYBACK	6.00 / 0.00	43-00-00	09-09-00	2 X 8	2 X 8	01-03-08 / 01-03-08	01-02-00 / 01-02-00	714.96 / 426.66		
	2	T18 PIGGYBACK	6.00 / 0.00	37-08-00	09-09-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 03-10-00	376.42 / 236.00		
	1	G18 PIGGYBACK	6.00 / 0.00	37-08-00	09-09-00	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 00-00-00	222.78 / 140.17		
	1	T18A PIGGYBACK	6.00 / 0.00	33-08-00	09-09-00	2 X 4	2 X 4	00-00-00 / 01-03-08	03-02-00 / 03-10-00	178.62 / 112.33		
	1 / 2 Ply	T19 PIGGYBACK	6.00 / 0.00	37-08-00	09-09-00	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 03-10-00	498.32 / 301.00		
	3	T20 ROOF	12.00 / 6.00	12-01-00	07-11-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	208.38 / 135.00		
	2	T21 COMMON	12.00 / 0.00	12-01-00	09-03-00	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-08 / 03-02-08	135.34 / 86.34		
	1 / 3 Ply	T22 COMMON	12.00 / 0.00	12-01-00	09-03-00	2 X 4	2 X 6	00-00-00 / 00-00-00	03-02-08 / 03-02-08	252.12 / 160.50		
	5	T13 MONOPITCH	3.50 / 0.00	11-01-00	04-03-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-10-07 / 03-08-15	242.25 / 159.15		
	2	G13 MONOPITCH	3.50 / 0.00	11-01-00	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-10-07 / 03-06-04	77.08 / 49.66		
	10	T15 MONO PITCH	4.00 / 0.00	05-01-00	02-03-06	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-03-06	175.50 / 123.30		
	18	P2 PIGGYBACK	6.00 / 0.00	06-09-06	02-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	338.04 / 203.94		
	2 / 3 Ply	P2Z PIGGYBACK	6.00 / 0.00	06-09-06	02-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	112.68 / 67.98		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 288225 LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO16 SUB-BUILDER:
MODEL: S48-1 GOLDENEYE 1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	P2Z1	6.00	06-09-06	02-00-08	2 X 4	2 X 4	00-00-00	00-04-03	75.12		
	2 Ply	PIGGYBACK	0.00					00-00-00	00-04-03	45.32		

TOTAL # TRUSS= **81.00** TOTAL BFT OF ALL TRUSSES= **5557.83** BFT. TOTAL WEIGHT OF ALL TRUSSES= **9117.04** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
9	Hangers	LJS26DS	
4	Hangers	LUS24	
2	Hangers	HGUS28-3	
2	Hangers	HGUS46	
14	Hangers	H2.5A	
4	Hangers	TS12	
2	Beam	1 3/4" X 9 1/2" LVL	08-00-00

TOTAL # ITEMS= **37.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

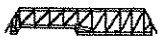
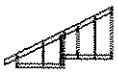
Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 273415 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S48-1 GOLDENEYE 1 ELEVATION: A +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1 / HIP GIRDER	12.00 / 0.00	39-00-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	462.90 / 285.34		
	1 / 2 Ply	T1S / HIP GIRDER	12.00 / 12.00	39-00-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	445.06 / 279.00		
	2	T2 / HIP	12.00 / 0.00	39-00-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	409.88 / 250.66		
	2	T3 / HIP	12.00 / 0.00	39-00-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	363.42 / 229.34		
	2	T4 / HIP	12.00 / 0.00	39-00-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	375.48 / 235.00		
	2	T5 / HIP	12.00 / 0.00	39-00-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	428.26 / 264.00		
	2	T6 / HIP	12.00 / 0.00	39-00-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	444.56 / 275.66		
	2	T7 / HIP	12.00 / 0.00	39-00-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	429.04 / 260.66		
	1 / 2 Ply	T8S / HIP GIRDER	12.00 / 12.00	30-00-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-10-08 / 01-10-08	301.68 / 194.66		
	1	T9S / HIP	12.00 / 12.00	30-00-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	156.34 / 98.50		
	1	T10S / HIP	12.00 / 12.00	30-00-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	156.92 / 100.67		
	1	T11 / PIGGYBACK	12.00 / 0.00	20-04-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	109.23 / 69.33		
	2	T11S / PIGGYBACK	12.00 / 6.00	20-04-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	239.20 / 154.34		
	1	T12S / ROOF	12.00 / 6.00	12-01-00	07-11-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	65.05 / 42.00		
	5	T13 / MONOPITCH	3.50 / 0.00	11-01-00	04-03-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-10-07 / 03-08-15	242.25 / 159.15		
	2	G13 / MONOPITCH	3.50 / 0.00	11-01-00	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-10-07 / 03-06-04	77.08 / 49.66		
	10	T14 / MONO	5.00 / 0.00	05-01-00	02-09-05	2 X 4	2 X 4	01-03-08 / 00-00-00	00-04-01 / 02-09-05	187.60 / 123.30		
	3	P1 / PIGGYBACK	12.00 / 0.00	04-11-10	02-11-00	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-00 / 00-00-00	46.44 / 30.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 **LAYOUT ID:** 273415 **LOCATION:** INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHOPS **SUB-BUILDER:**
MODEL: S48-1 GOLDENEYE 1 **ELEVATION:** A +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	22	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	369.38 / 234.74		
	9	J1S JACK-CLOSED	6.00 / 12.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	206.10 / 148.50		

TOTAL # TRUSS= **75.00** TOTAL BFT OF ALL TRUSSES= **3484.51** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5515.87** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LJS26DS	
3	Hangers	HGUS26-2	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

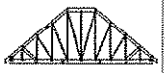
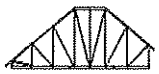
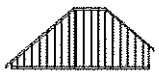
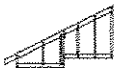
Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 273416 LOCATION: UNNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHORES SUB-BUILDER:
MODEL: S48-1 GOLDENEYE 1 ELEVATION: B +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T16 PIGGYBACK	6.00 / 0.00	43-00-00	09-09-00	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	527.84 / 315.00		
	3	T16S PIGGYBACK	6.00 / 10.00	43-00-00	09-09-00	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	831.09 / 501.99		
	2	T16A PIGGYBACK	6.00 / 0.00	39-00-00	09-09-00	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 03-02-00	475.28 / 290.00		
	1	T16B PIGGYBACK	6.00 / 0.00	34-06-08	09-09-00	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 05-04-12	219.06 / 132.00		
	5	T16SB PIGGYBACK	6.00 / 10.00	34-06-08	09-09-00	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 05-04-12	1187.00 / 729.15		
	1	G16 PIGGYBACK	6.00 / 0.00	43-00-00	09-09-00	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	241.51 / 151.00		
	1 / 3 Ply	T17 PIGGYBACK	6.00 / 0.00	43-00-00	09-09-00	2 X 8	2 X 8	01-03-08 / 01-03-08	01-02-00 / 01-02-00	1072.44 / 639.99		
	1 / 3 Ply	T17S PIGGYBACK	6.00 / 10.00	43-00-00	09-09-00	2 X 6	2 X 8	01-03-08 / 01-03-08	01-02-00 / 01-02-00	1009.59 / 609.99		
	1 / 2 Ply	T17Z PIGGYBACK	6.00 / 0.00	43-00-00	09-09-00	2 X 8	2 X 8	01-03-08 / 01-03-08	01-02-00 / 01-02-00	714.96 / 426.66		
	2	T18 PIGGYBACK	6.00 / 0.00	37-08-00	09-09-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 03-10-00	376.42 / 236.00		
	1	G18 PIGGYBACK	6.00 / 0.00	37-08-00	09-09-00	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 00-00-00	222.78 / 140.17		
	1	T18A PIGGYBACK	6.00 / 0.00	33-08-00	09-09-00	2 X 4	2 X 4	00-00-00 / 01-03-08	03-02-00 / 03-10-00	178.62 / 112.33		
	1 / 2 Ply	T19 PIGGYBACK	6.00 / 0.00	37-08-00	09-09-00	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 03-10-00	498.32 / 301.00		
	3	T20 ROOF	12.00 / 6.00	12-01-00	07-11-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	208.38 / 135.00		
	2	T21 COMMON	12.00 / 0.00	12-01-00	09-03-00	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-08 / 03-02-08	135.34 / 86.34		
	1 / 3 Ply	T22 COMMON	12.00 / 0.00	12-01-00	09-03-00	2 X 4	2 X 6	00-00-00 / 00-00-00	03-02-08 / 03-02-08	252.12 / 160.50		
	5	T13 MONOPITCH	3.50 / 0.00	11-01-00	04-03-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-10-07 / 03-08-15	242.25 / 159.15		
	2	G13 MONOPITCH	3.50 / 0.00	11-01-00	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-10-07 / 03-06-04	77.08 / 49.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

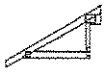
Delivery Shiplist

DATE	09/18/17
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 273416 LOCATION: UNNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: S48-1 GOLDENEYE 1 ELEVATION: B +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	10	T15 MONO PITCH	4.00 / 0.00	05-01-00	02-03-06	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-03-06	175.50 / 123.30		
	18	P2 PIGGYBACK	6.00 / 0.00	06-09-06	02-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	338.04 / 203.94		
	2 / 3 Ply	P2Z PIGGYBACK	6.00 / 0.00	06-09-06	02-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	112.68 / 67.98		
	2 / 2 Ply	P2Z1 PIGGYBACK	6.00 / 0.00	06-09-06	02-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	75.12 / 45.32		

TOTAL # TRUSS= **81** TOTAL BFT OF ALL TRUSSES= **5616.47** BFT. TOTAL WEIGHT OF ALL TRUSSES= **9171.42** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
9	Hangers	LJS26DS	
4	Hangers	LUS24	
2	Hangers	HGUS28-3	
2	Hangers	HGUS46	
14	Hangers	H2.5A	
4	Hangers	TS12	
2	Beam	1 3/4" X 9 1/2" LVL	08-00-00

TOTAL # ITEMS= **37**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288223	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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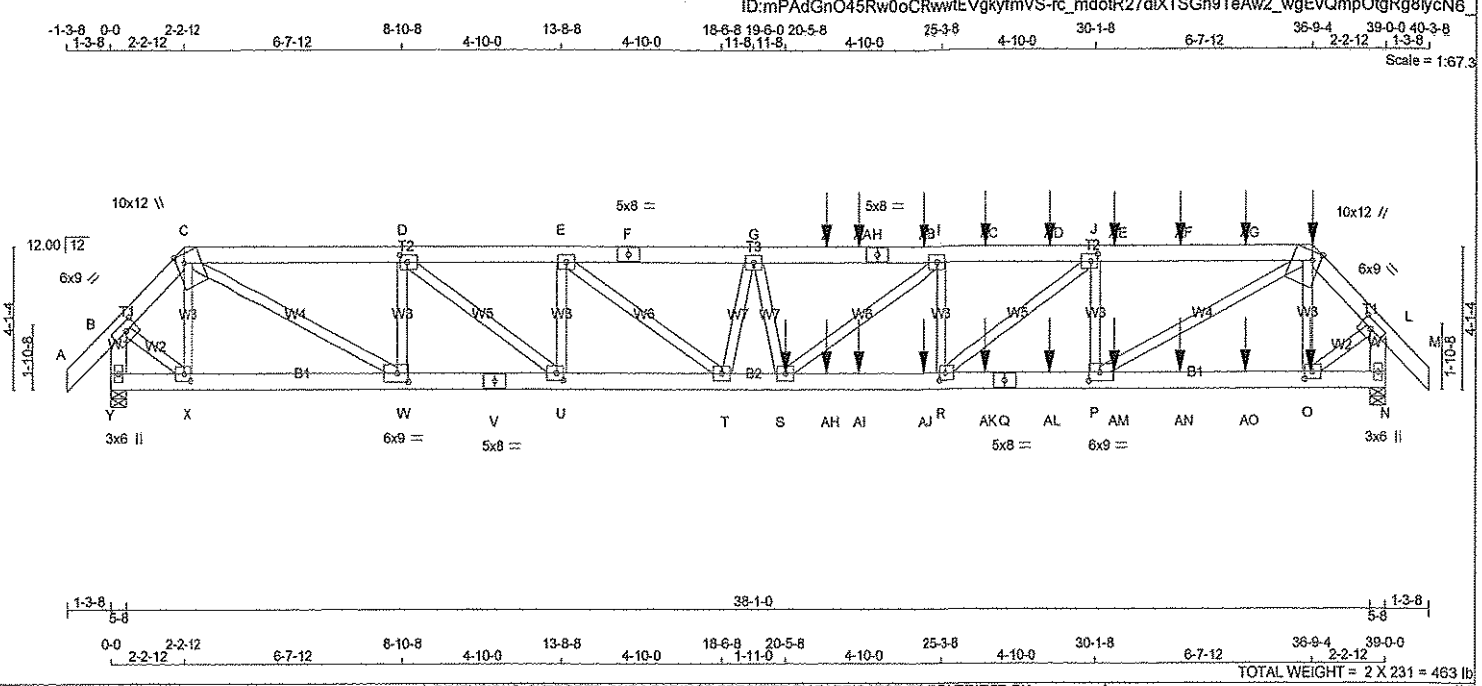

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	2100F 1.8E	SPF
V - Q	2x6	DRY	2100F 1.8E	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- H	2	12	SIDE(0.0)
H- K	2	12	SIDE(0.0)
K- M	2	12	SIDE(0.0)
Y- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Y- V	2	12	TOP
V- Q	2	12	SIDE(0.0)
Q- N	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	10.0	12.0	Edge	3.00
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E, G, I						
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	8.0		
H	TS-t	MT20	5.0	8.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TTWW+m	MT20	10.0	12.0	Edge	3.00
L	TMWW-t	MT20	6.0	9.0	2.75	4.00
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BMWW-t	MT20	6.0	9.0	3.00	4.00
Q	BS-t	MT20	5.0	8.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	4573	0	4573	0	0	5-8	5-8
N	5647	0	5647	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	3525	2375 / 0	570 / 0	0 / 0	0 / 0	581 / 0	0 / 0
N	4359	2927 / 0	712 / 0	0 / 0	0 / 0	720 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00	
B- C	-3941 / 0	-122.2	-122.2	0.05 (1)	5.68	
C- D	-9131 / 0	-122.2	-122.2	0.38 (1)	3.78	
D- E	-12670 / 0	-122.2	-122.2	0.44 (1)	3.20	
E- F	-15308 / 0	-122.2	-122.2	0.59 (1)	2.82	
F- G	-15308 / 0	-122.2	-122.2	0.59 (1)	2.82	
G- Z	-15906 / 0	-122.2	-122.2	0.75 (1)	2.58	
Z-AA	-15906 / 0	-122.2	-122.2	0.75 (1)	2.58	
AA- H	-15906 / 0	-122.2	-122.2	0.75 (1)	2.58	
H-AB	-15906 / 0	-122.2	-122.2	0.75 (1)	2.58	
AB- I	-15906 / 0	-122.2	-122.2	0.75 (1)	2.58	
I-AC	-14172 / 0	-122.2	-122.2	0.61 (1)	2.86	
AC-AD	-14172 / 0	-122.2	-122.2	0.61 (1)	2.86	
AD- J	-14172 / 0	-122.2	-122.2	0.61 (1)	2.86	
J-AE	-10709 / 0	-122.2	-122.2	0.56 (1)	3.33	
AE-AF	-10709 / 0	-122.2	-122.2	0.56 (1)	3.33	
AF-AG	-10709 / 0	-122.2	-122.2	0.56 (1)	3.33	
AG- K	-10709 / 0	-122.2	-122.2	0.56 (1)	3.33	
K- L	-4944 / 0	-122.2	-122.2	0.06 (1)	5.19	
L- M	0 / 63	-122.2	-122.2	0.05 (1)	10.00	
Y- B	-4664 / 0	0.0	0.0	0.17 (1)	6.68	
N- L	-5772 / 0	0.0	0.0	0.21 (1)	6.13	
Y- X	0 / 0	-27.5	-27.5	0.06 (1)	10.00	
X- W	0 / 2708	-27.5	-27.5	0.13 (1)	10.00	
W- V	0 / 9131	-27.5	-27.5	0.28 (1)	10.00	
V- U	0 / 9131	-27.5	-27.5	0.28 (1)	10.00	
U- T	0 / 12670	-27.5	-27.5	0.38 (1)	10.00	
T- S	0 / 15712	-27.5	-27.5	0.48 (1)	10.00	
S-AH	0 / 14172	-27.5	-27.5	0.45 (1)	10.00	
AH-AI	0 / 14172	-27.5	-27.5	0.45 (1)	10.00	
AI-AJ	0 / 14172	-27.5	-27.5	0.45 (1)	10.00	
AJ- R	0 / 14172	-27.5	-27.5	0.45 (1)	10.00	
R-AK	0 / 10710	-27.5	-27.5	0.33 (1)	10.00	
AK- Q	0 / 10710	-27.5	-27.5	0.33 (1)	10.00	
Q-AL	0 / 10710	-27.5	-27.5	0.33 (1)	10.00	
AL- P	0 / 10710	-27.5	-27.5	0.33 (1)	10.00	
P-AM	0 / 3399	-27.5	-27.5	0.16 (1)	10.00	
AM-AN	0 / 3399	-27.5	-27.5	0.16 (1)	10.00	
AN-AO	0 / 3399	-27.5	-27.5	0.16 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-1218 / 0	0.10 (1)
C- W	0 / 7305	0.65 (1)
W- D	-3413 / 0	0.29 (1)
D- U	0 / 4433	0.39 (1)
U- E	-2493 / 0	0.21 (1)
R- I	-2275 / 0	0.19 (1)
R- J	0 / 4336	0.38 (1)
P- J	-3803 / 0	0.33 (1)
P- K	0 / 8314	0.73 (1)
O- K	-1475 / 0	0.13 (1)
B- X	0 / 3155	0.28 (1)
O- L	0 / 3959	0.35 (1)
T- G	-1824 / 0	0.16 (1)
E- T	0 / 3272	0.29 (1)
G- S	0 / 879	0.08 (1)
S- I	0 / 2152	0.19 (1)

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA (M)

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 959 (0.49")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 632 (0.74")

CSI: TC=0.75 (G-I:1) , BC=0.48 (S-T:1) , WB=0.73 (K-P:1) , SSI=0.25 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (W) (INPUT = 0.90)
JSI METAL= 0.88 (V) (INPUT = 1.00)

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DWG NO. TAM 47526-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BMWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BS-t	MT20	5.0	8.0		
W	BMWW-t	MT20	6.0	9.0	3.00	4.00
X	BMWW-t	MT20	5.0	6.0	2.50	2.75
Y	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 346.1 lbs FACTORED DOWN AT 36-9-4, 147.1 lbs FACTORED DOWN AT 21-8-8, 147.1 lbs FACTORED DOWN AT 22-8-8, 147.1 lbs FACTORED DOWN AT 24-8-8, 147.1 lbs FACTORED DOWN AT 26-8-8, 147.1 lbs FACTORED DOWN AT 28-8-8, 147.1 lbs FACTORED DOWN AT 30-8-8, AND 147.1 lbs FACTORED DOWN AT 32-8-8, AND 147.1 lbs FACTORED DOWN AT 34-8-8 ON TOP CHORD, AND 2138.7 lbs FACTORED DOWN AT 20-5-8, 71.4 lbs FACTORED DOWN AT 21-8-8, 71.4 lbs FACTORED DOWN AT 22-8-8, 71.4 lbs FACTORED DOWN AT 24-8-8, 71.4 lbs FACTORED DOWN AT 26-8-8, 71.4 lbs FACTORED DOWN AT 28-8-8, 71.4 lbs FACTORED DOWN AT 30-8-8, 71.4 lbs FACTORED DOWN AT 32-8-8, AND 71.4 lbs FACTORED DOWN AT 34-8-8, AND 71.4 lbs FACTORED DOWN AT 36-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)
AO- O	0 / 3399	-27.5	-27.5	0.16 (1)	
O- N	0 / 0	-27.5	-27.5	0.07 (1)	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	MAX. UNBRAC LENGTH
AO- O	0 / 3399	0.16 (1)	10.00
O- N	0 / 0	0.07 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	36-9-4	-13	-14	---	FRONT	VERT	DEAD
K	36-9-4	-151	-151	---	FRONT	VERT	TOTAL
K	36-9-4	-183	-183	---	FRONT	VERT	SNOW
O	36-8-8	-40	-71	---	FRONT	VERT	TOTAL
S	20-5-8	-2139	-2139	---	FRONT	VERT	TOTAL
Z	21-8-8	-147	-147	---	FRONT	VERT	TOTAL
AA	22-8-8	-147	-147	---	FRONT	VERT	TOTAL
AB	24-8-8	-147	-147	---	FRONT	VERT	TOTAL
AC	26-8-8	-147	-147	---	FRONT	VERT	TOTAL
AD	28-8-8	-147	-147	---	FRONT	VERT	TOTAL
AE	30-8-8	-147	-147	---	FRONT	VERT	TOTAL
AF	32-8-8	-147	-147	---	FRONT	VERT	TOTAL
AG	34-8-8	-147	-147	---	FRONT	VERT	TOTAL
AH	21-8-8	-40	-71	---	FRONT	VERT	TOTAL
AI	22-8-8	-40	-71	---	FRONT	VERT	TOTAL
AJ	24-8-8	-40	-71	---	FRONT	VERT	TOTAL
AK	26-8-8	-40	-71	---	FRONT	VERT	TOTAL
AL	28-8-8	-40	-71	---	FRONT	VERT	TOTAL
AM	30-8-8	-40	-71	---	FRONT	VERT	TOTAL
AN	32-8-8	-40	-71	---	FRONT	VERT	TOTAL
AO	34-8-8	-40	-71	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9 2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 42526 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288223	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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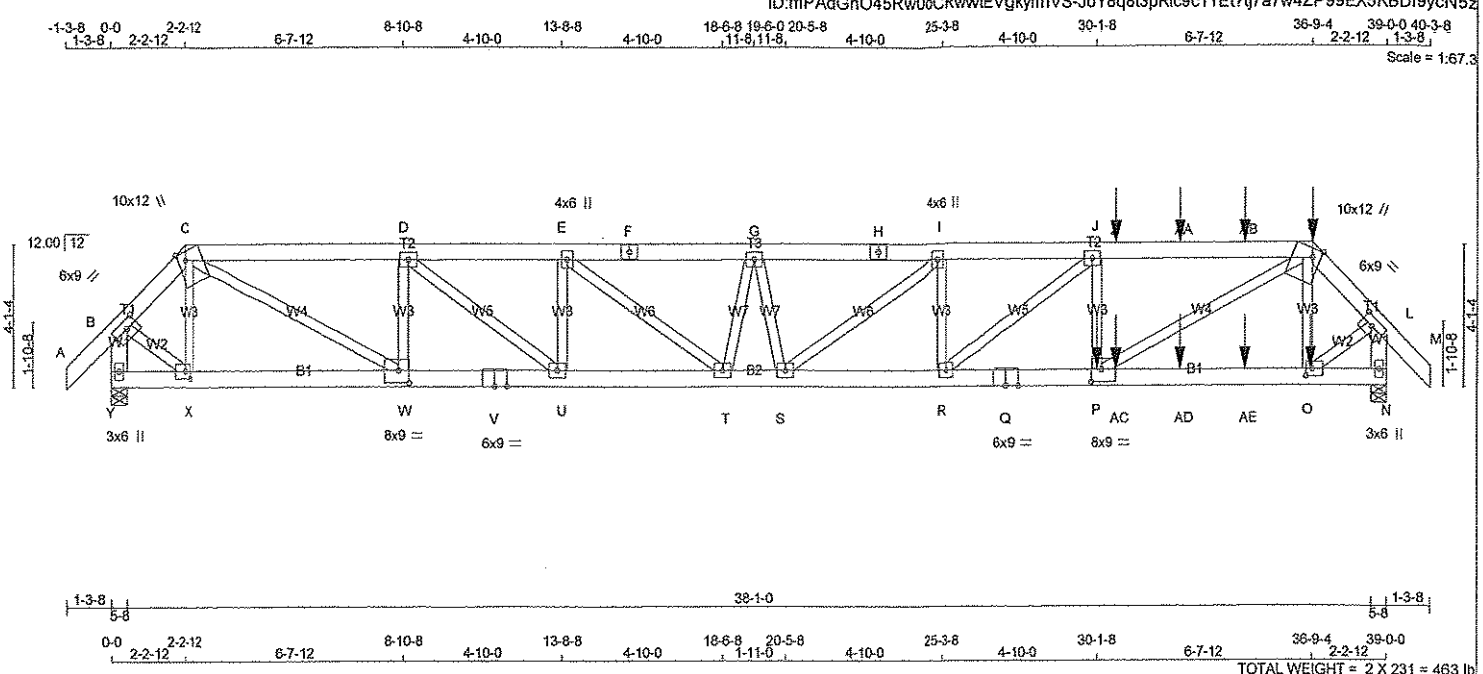

TOTAL WEIGHT = 2 X 231 = 463 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	2100F 1.8E	SPF
V - Q	2x6	DRY	2100F 1.8E	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- H	2	12	TOP
H- K	2	12	SIDE(0.0)
K- M	2	12	SIDE(0.0)
Y- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Y- V	2	12	TOP
V- Q	2	12	TOP
Q- N	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	10.0	12.0	Edge	3.00
D, G, J						
D	TMWW-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
K	TTWW+m	MT20	10.0	12.0	Edge	3.00
L	TMWW-t	MT20	6.0	9.0	2.75	4.00
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BMWW-t	MT20	8.0	9.0	4.25	3.75
Q	BS-t	MT20	6.0	9.0		
R, S, T, U						
R	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	4156	0	4156	0	0	5-8	5-8
N	7280	0	7280	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	3216	2153 / 0	536 / 0	0 / 0	0 / 0	527 / 0	0 / 0
N	5601	3804 / 0	897 / 0	0 / 0	0 / 0	900 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00
B- C	-3554 / 0	-122.2	-122.2	0.04 (1)	5.91
C- D	-8121 / 0	-122.2	-122.2	0.34 (1)	4.01
D- E	-11103 / 0	-122.2	-122.2	0.37 (1)	3.47
E- F	-13148 / 0	-122.2	-122.2	0.47 (1)	3.14
F- G	-13148 / 0	-122.2	-122.2	0.47 (1)	3.14
G- H	-13613 / 0	-122.2	-122.2	0.51 (1)	3.05
H- I	-13613 / 0	-122.2	-122.2	0.51 (1)	3.05
I- J	-14424 / 0	-122.2	-122.2	0.60 (1)	2.85
J- Z	-14202 / 0	-122.2	-122.2	0.86 (1)	2.58
Z-AA	-14202 / 0	-122.2	-122.2	0.86 (1)	2.58
AA-AB	-14202 / 0	-122.2	-122.2	0.86 (1)	2.58
AB- K	-14202 / 0	-122.2	-122.2	0.86 (1)	2.58
K- L	-6485 / 0	-122.2	-122.2	0.08 (1)	4.64
L- M	0 / 63	-122.2	-122.2	0.05 (1)	10.00
Y- B	-4237 / 0	0.0	0.0	0.16 (1)	6.94
N- L	-7472 / 0	0.0	0.0	0.28 (1)	5.49
Y- X	0 / 0	-28.0	-28.0	0.05 (1)	10.00
X- W	0 / 2442	-28.0	-28.0	0.12 (1)	10.00
W- V	0 / 8121	-28.0	-28.0	0.25 (1)	10.00
V- U	0 / 8121	-28.0	-28.0	0.25 (1)	10.00
U- T	0 / 11103	-28.0	-28.0	0.33 (1)	10.00
T- S	0 / 13468	-28.0	-28.0	0.39 (1)	10.00
S- R	0 / 14424	-28.0	-28.0	0.42 (1)	10.00
R- Q	0 / 14202	-28.0	-28.0	0.46 (1)	10.00
Q- P	0 / 14202	-28.0	-28.0	0.46 (1)	10.00
P-AC	0 / 4459	-28.0	-28.0	0.22 (1)	10.00
AC-AD	0 / 4459	-28.0	-28.0	0.22 (1)	10.00
AD-AE	0 / 4459	-28.0	-28.0	0.22 (1)	10.00
AE- O	0 / 4459	-28.0	-28.0	0.22 (1)	10.00
O- N	0 / 0	-28.0	-28.0	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-1083 / 0	0.09 (1)
C- W	0 / 6458	0.57 (1)
W- D	-2997 / 0	0.26 (1)
D- U	0 / 3735	0.33 (1)
U- E	-2075 / 0	0.18 (1)
R- I	0 / 129	0.01 (2)
R- J	0 / 277	0.02 (1)
P- J	-1382 / 0	0.12 (1)
P- K	0 / 11080	0.98 (1)
O- K	-1920 / 0	0.16 (1)
B- X	0 / 2845	0.25 (1)
O- L	0 / 5192	0.45 (1)
T- G	-1446 / 0	0.13 (1)
E- T	0 / 2537	0.22 (1)
G- S	0 / 655	0.06 (1)
S- I	-1006 / 0	0.23 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	36-9-4	-13	-14	---	FRONT	VERT	DEAD
K	36-9-4	-240	-240	---	BACK	VERT	TOTAL
K	36-9-4	-183	-183	---	FRONT	VERT	SNOW
O	36-8-8	-48	-85	---	BACK	VERT	TOTAL
P	30-1-8	-3883	-3883	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.45")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 681 (0.69")

CSI: TC=0.86 (J-K:1) , BC=0.46 (P-R:1) , WB=0.98 (K-P:1) , SSI=0.31 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (W) (INPUT = 0.90)
JSI METAL= 0.96 (V) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
92077
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4752717

STRUCTURAL COMPONENT ONLY

Pg 1/2

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BS-t	MT20	6.0	9.0		
W	BMWW-t	MT20	8.0	9.0	4.25	3.75
X	BMWW-t	MT20	5.0	6.0	2.50	2.25
Y	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 36-9-4, 236.8 lbs FACTORED DOWN AT 30-8-8, 236.8 lbs FACTORED DOWN AT 32-8-8, AND 236.8 lbs FACTORED DOWN AT 34-8-8, AND 240.3 lbs FACTORED DOWN AT 36-9-4 ON TOP CHORD, AND 3882.7 lbs FACTORED DOWN AT 30-1-8, 84.6 lbs FACTORED DOWN AT 30-8-8, 84.6 lbs FACTORED DOWN AT 32-8-8, AND 84.6 lbs FACTORED DOWN AT 34-8-8, AND 84.6 lbs FACTORED DOWN AT 36-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Z	30-8-8	-237	-237	---	BACK	VERT	TOTAL
AA	32-8-8	-237	-237	---	BACK	VERT	TOTAL
AB	34-8-8	-237	-237	---	BACK	VERT	TOTAL
AC	30-8-8	-48	-85	---	BACK	VERT	TOTAL
AD	32-8-8	-48	-85	---	BACK	VERT	TOTAL
AE	34-8-8	-48	-85	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9201
S. KATSOULAKOS
PROVINCE OF ONTARIO

PG 2/2

DWG NO. TAM47527-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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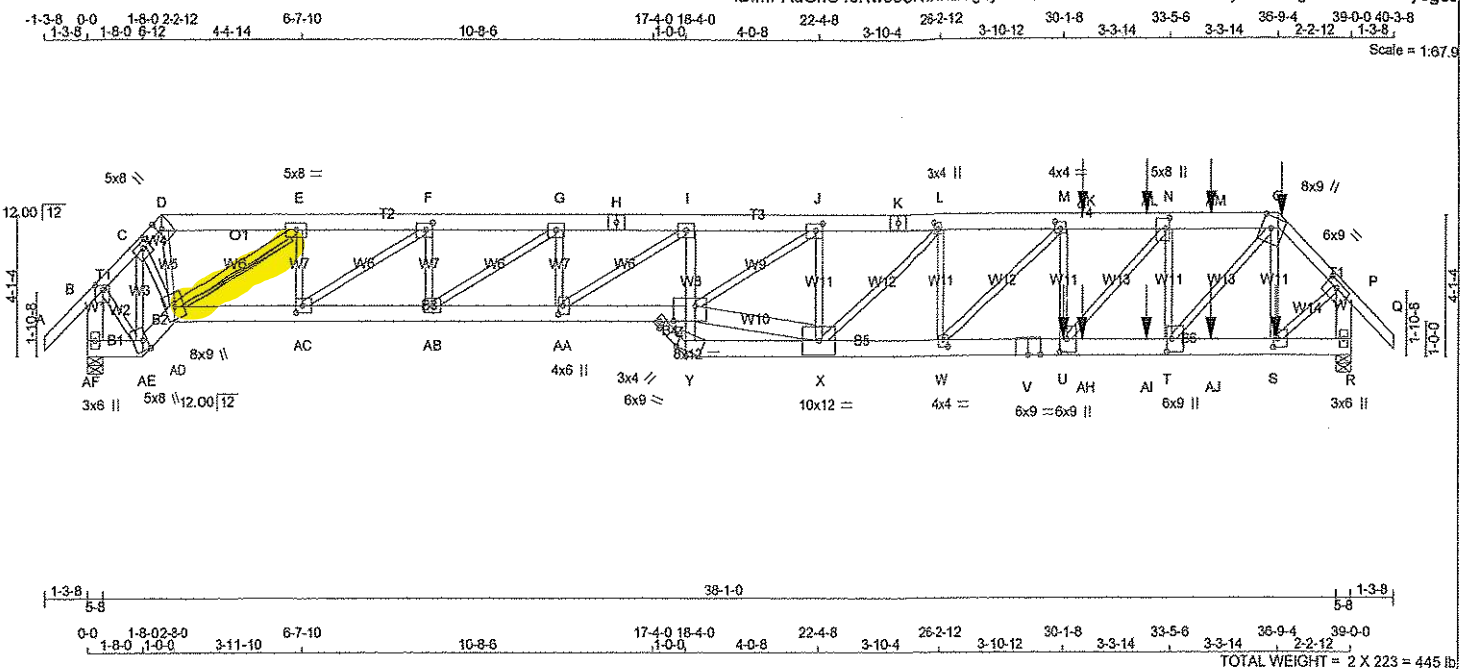

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
O - Q	2x4	DRY	No.2	SPF
AF- B	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - O	2x6	DRY	No.2	SPF
AF- AE	2x6	DRY	No.2	SPF
AE- AD	2x6	DRY	No.2	SPF
AD- Z	2x6	DRY	1650F 1.5E	SPF
Y - V	2x6	DRY	1650F 1.5E	SPF
V - R	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Y - I	2x4	DRY	No.2	SPF
Z - X	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
O- Q	1	12	SIDE(0.0)
AF- B	2	12	TOP
R- P	2	12	TOP
D- H	2	12	TOP
K- H	2	12	TOP
K- O	2	12	SIDE(0.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AF- AE	2	12	TOP
AE- AD	2	12	TOP
AD- Z	2	12	TOP
Y- V	2	12	TOP
V- R	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

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NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	5.0	6.0	2.00	2.50
D	TTW+h	MT20	5.0	8.0	Edge	1.50
E	TMWW-t	MT20	5.0	8.0		
F	TMWW-t	MT20	5.0	6.0	2.50	2.50
G	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AF	4075	0	4075	0	0	5-8	5-8
R	6766	0	6766	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AF	3143	2112 / 0	509 / 0	0 / 0	0 / 0	522 / 0	0 / 0
R	5215	3511 / 0	841 / 0	0 / 0	0 / 0	864 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AF, R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-AD

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.09 (1)	10.00
B- C	-2907 / 0	-122.2	-122.2	0.14 (1)	5.26
C- D	-4404 / 0	-122.2	-122.2	0.17 (1)	4.41
O- P	-5712 / 0	-122.2	-122.2	0.17 (1)	3.96
P- Q	0 / 60	-122.2	-122.2	0.09 (1)	10.00
AF- B	-4053 / 0	0.0	0.0	0.16 (1)	7.06
R- P	-6833 / 0	0.0	0.0	0.26 (1)	5.71
D- E	-3707 / 0	-122.2	-122.2	0.11 (1)	5.75
E- F	-8386 / 0	-122.2	-122.2	0.21 (1)	4.07
F- G	-12253 / 0	-122.2	-122.2	0.37 (1)	3.31
G- H	-15297 / 0	-122.2	-122.2	0.60 (1)	2.75
H- I	-15297 / 0	-122.2	-122.2	0.60 (1)	2.75
I- J	-17431 / 0	-122.2	-122.2	0.71 (1)	2.48
J- K	-13702 / 0	-122.2	-122.2	0.40 (1)	3.13
K- L	-13702 / 0	-122.2	-122.2	0.40 (1)	3.13
L- M	-13921 / 0	-122.2	-122.2	0.46 (1)	3.04
M-AK	-13446 / 0	-122.2	-122.2	0.44 (1)	3.09
AK-AL	-13446 / 0	-122.2	-122.2	0.44 (1)	3.09
AL- N	-13446 / 0	-122.2	-122.2	0.44 (1)	3.09
N-AM	-9209 / 0	-122.2	-122.2	0.25 (1)	3.88
AM- O	-9209 / 0	-122.2	-122.2	0.25 (1)	3.88
AF-AE	0 / 0	-27.5	-27.5	0.01 (2)	10.00
AE-AD	0 / 2613	-27.5	-27.5	0.17 (1)	10.00
AD-AC	0 / 8387	-27.5	-27.5	0.37 (1)	10.00
AC-AB	0 / 12253	-27.5	-27.5	0.55 (1)	10.00
AB-AA	0 / 15297	-27.5	-27.5	0.69 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-AD	0 / 2743	0.34 (1)
Y- Z	0 / 82	0.31 (1)
Z- I	0 / 1262	0.41 (1)
U- M	-919 / 0	0.11 (1)
U- N	0 / 6344	0.79 (1)
T- N	-5470 / 0	0.66 (1)
T- O	0 / 7700	0.95 (1)
S- O	-2189 / 0	0.26 (1)
S- P	0 / 4758	0.59 (1)
AE- C	-3458 / 0	0.35 (1)
B-AE	0 / 2566	0.32 (1)
C-AD	0 / 2858	0.35 (1)
AD- E	-5638 / 0	0.69 (1)
W- L	-289 / 6	0.03 (1)
X- J	-3103 / 0	0.37 (1)
X- L	-304 / 0	0.07 (1)
Z- X	0 / 13811	0.90 (1)
Z- J	0 / 4414	0.55 (1)
W- M	0 / 652	0.08 (1)
AB- F	0 / 2160	0.27 (1)
AC- E	0 / 2560	0.32 (1)
AA- G	0 / 1420	0.18 (1)
AA- I	-2742 / 0	0.51 (1)
AC- F	-4648 / 0	0.86 (1)
AB- G	-3668 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 704 (0.67")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 463 (1.01")

CSI: TC=0.71 (I-J:1) , BC=0.78 (Z-AA:1) , WB=0.95 (O-T:1) , SSI=0.17 (Y-Z:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.750 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Z-INPUT = 0.90)
JSI METAL= 0.90 (Z-INPUT = 1.00)

DWG NO. TAM 45964-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T1S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMWW-t	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TS-t	MT20	5.0	6.0		
L	TMWW+t	MT20	3.0	4.0	2.50	1.50
M	TMWW-t	MT20	4.0	4.0	2.50	2.00
N	TMWW+t	MT20	5.0	8.0	3.75	1.50
O	TTWW+m	MT20	8.0	9.0	4.00	3.50
P	TMWW-t	MT20	6.0	9.0	2.00	4.00
R	BMV1+p	MT20	3.0	6.0		
S	BMWW+t	MT20	5.0	6.0	3.00	1.50
T	BMWW+t	MT20	6.0	9.0	4.50	1.75
U	BMWW+t	MT20	6.0	9.0	4.50	2.50
V	BS-t	MT20	6.0	9.0		
W	BMWW-t	MT20	4.0	4.0	2.50	2.00
X	BMWWW-t	MT20	10.0	12.0		
Y						
Y						
Y	BBW-m	MT20	6.0	9.0	Edge	
Z	BWMWW"-I	MT20	8.0	12.0	5.25	8.00
AA	BMWW+t	MT20	4.0	6.0	3.00	1.50
AB	BMWW-t	MT20	5.0	6.0		
AC	BMWW-t	MT20	5.0	6.0	2.50	2.50
AD	BBWWW+m	MT20	8.0	9.0		
AE	BBWW+m	MT20	5.0	8.0	3.50	1.50
AF	BMV1+p	MT20	3.0	6.0		
AG	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 147.1 lbs FACTORED DOWN AT 30-8-8, 147.1 lbs FACTORED DOWN AT 32-8-8, AND 147.1 lbs FACTORED DOWN AT 34-8-8, AND 204.4 lbs FACTORED DOWN AT 36-9-4 ON TOP CHORD, AND 3858.6 lbs FACTORED DOWN AT 30-1-8, 67.8 lbs FACTORED DOWN AT 30-8-8, 67.8 lbs FACTORED DOWN AT 32-8-8, AND 67.8 lbs FACTORED DOWN AT 34-8-8, AND 67.8 lbs FACTORED DOWN AT 36-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AA- Z	0 / 17586	-27.5	-27.5		0.78 (1)	10.00
Y- X	0 / 395	-27.5	-27.5		0.03 (1)	10.00
X- W	0 / 13921	-27.5	-27.5		0.60 (1)	10.00
W- V	0 / 13446	-27.5	-27.5		0.65 (1)	10.00
V- U	0 / 13446	-27.5	-27.5		0.65 (1)	10.00
U-AH	0 / 9209	-27.5	-27.5		0.48 (1)	10.00
AH-AI	0 / 9209	-27.5	-27.5		0.48 (1)	10.00
AI- T	0 / 9209	-27.5	-27.5		0.48 (1)	10.00
T-AJ	0 / 3987	-27.5	-27.5		0.22 (1)	10.00
AJ- S	0 / 3987	-27.5	-27.5		0.22 (1)	10.00
S- R	0 / 0	-27.5	-27.5		0.06 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	36-9-4	-204	-204	---	BACK	VERT	TOTAL
S	36-8-8	-39	-68	---	BACK	VERT	TOTAL
U	30-1-8	-3859	-3859	---	BACK	VERT	TOTAL
AH	30-8-8	-39	-68	---	BACK	VERT	TOTAL
AI	32-8-8	-39	-68	---	BACK	VERT	TOTAL
AJ	34-8-8	-39	-68	---	BACK	VERT	TOTAL
AK	30-8-8	-147	-147	---	BACK	VERT	TOTAL
AL	32-8-8	-147	-147	---	BACK	VERT	TOTAL
AM	34-8-8	-147	-147	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45964-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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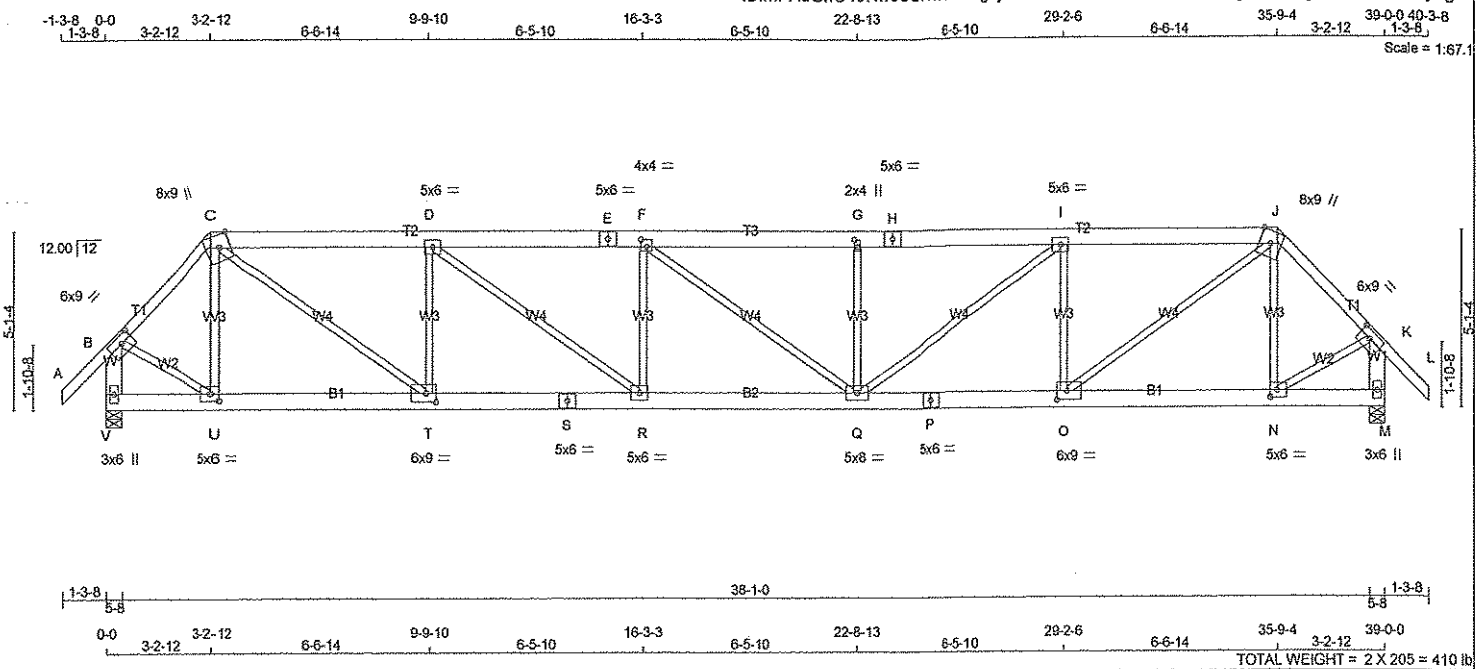

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.50	4.00
C	TTWW+m	MT20	8.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.50	2.00
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	Edge	
K	TMVW-t	MT20	6.0	9.0	2.50	4.00
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	6.0	9.0	3.00	3.75
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	6.0	9.0	3.00	3.75
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3090	0	3090	0	0	5-8	5-8
M	3090	0	3090	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0
M	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	-2684 / 0	-122.2	-122.2	0.34 (1)	3.94	
C- D	-4717 / 0	-122.2	-122.2	0.50 (1)	3.61	
D- E	-6038 / 0	-122.2	-122.2	0.60 (1)	3.14	
E- F	-6038 / 0	-122.2	-122.2	0.60 (1)	3.14	
F- G	-6032 / 0	-122.2	-122.2	0.50 (1)	3.27	
G- H	-6032 / 0	-122.2	-122.2	0.60 (1)	3.14	
H- I	-6032 / 0	-122.2	-122.2	0.60 (1)	3.14	
I- J	-4719 / 0	-122.2	-122.2	0.50 (1)	3.61	
J- K	-2683 / 0	-122.2	-122.2	0.34 (1)	3.94	
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
V- B	-3083 / 0	0.0	0.0	0.22 (1)	6.01	
M- K	-3083 / 0	0.0	0.0	0.22 (1)	6.01	
V- U	0 / 0	-27.5	-27.5	0.11 (2)	10.00	
U- T	0 / 1887	-27.5	-27.5	0.31 (1)	10.00	
T- S	0 / 4717	-27.5	-27.5	0.64 (1)	10.00	
S- R	0 / 4717	-27.5	-27.5	0.64 (1)	10.00	
R- Q	0 / 6038	-27.5	-27.5	0.79 (1)	10.00	
Q- P	0 / 4718	-27.5	-27.5	0.64 (1)	10.00	
P- O	0 / 4718	-27.5	-27.5	0.64 (1)	10.00	
O- N	0 / 1887	-27.5	-27.5	0.31 (1)	10.00	
N- M	0 / 0	-27.5	-27.5	0.11 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-631 / 0	0.21 (1)
C- T	0 / 3469	0.78 (1)
T- D	-1846 / 0	0.62 (1)
D- R	0 / 1627	0.37 (1)
R- F	-768 / 0	0.26 (1)
F- Q	-7 / 0	0.01 (1)
Q- G	-766 / 0	0.26 (1)
Q- I	0 / 1618	0.36 (1)
O- I	-1843 / 0	0.62 (1)
O- J	0 / 3471	0.78 (1)
N- J	-632 / 0	0.21 (1)
B- U	0 / 2071	0.47 (1)
N- K	0 / 2070	0.47 (1)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45965.17
STRUCTURAL COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 770 (0.61")

CSI: TC=0.60 (G-I:1) , BC=0.79 (Q-R:1) , WB=0.78 (J-O:1) , SSI=0.29 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.79 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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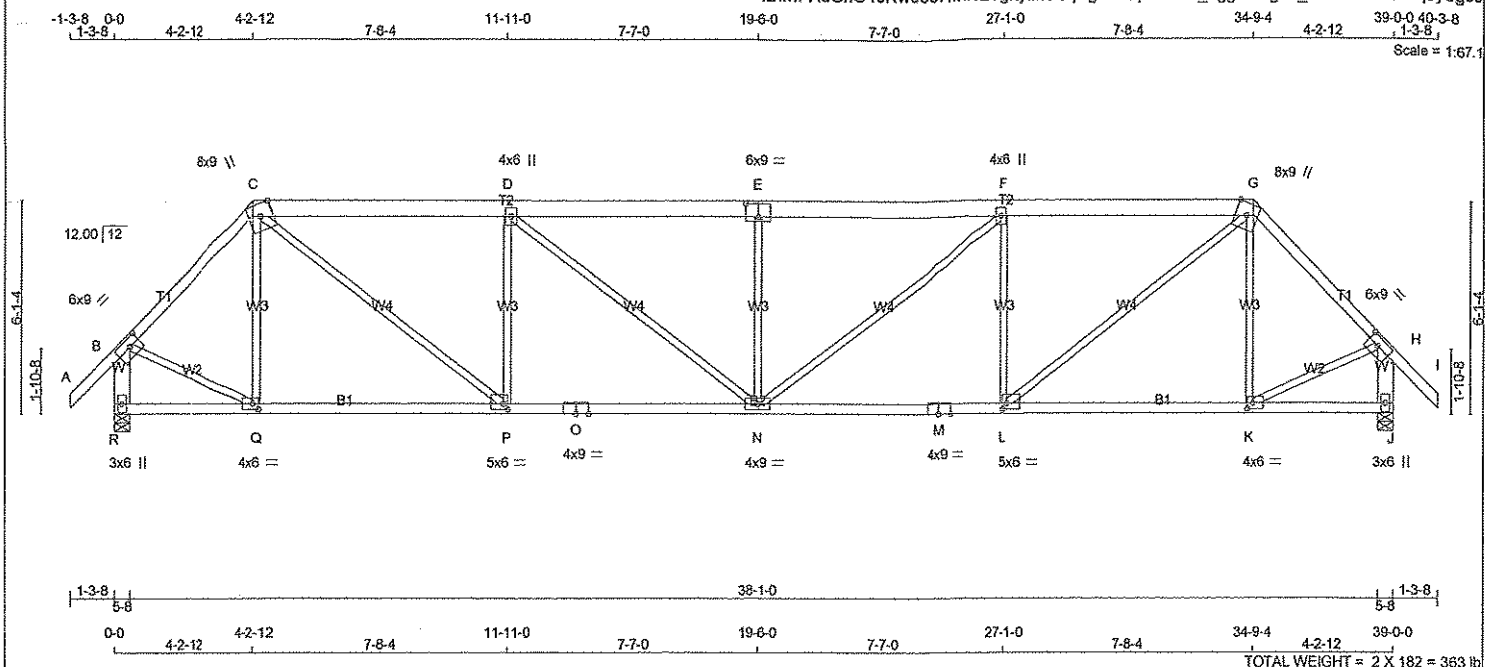

TOTAL WEIGHT = 2 X 182 = 363 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	8.0	9.0	Edge	
D	TMWW+t	MT20	4.0	6.0		
E	TSW-I	MT20	6.0	9.0	4.50	4.50
F	TMWW+t	MT20	4.0	6.0		
G	TTWW+m	MT20	8.0	9.0	Edge	
H	TMVW-t	MT20	6.0	9.0	2.75	4.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.00
L	BMWW-t	MT20	5.0	6.0	2.00	1.50
M	BS-t	MT20	4.0	9.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	4.0	9.0		
P	BMWW-t	MT20	5.0	6.0	2.00	1.50
Q	BMWW-t	MT20	4.0	6.0	2.00	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3090	0	3090	0	0	5-8	5-8
J	3090	0	3090	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0
J	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	-2751 / 0	-122.2	-122.2	0.59 (1)	3.65	
C- D	-4359 / 0	-122.2	-122.2	0.67 (1)	3.52	
D- E	-5067 / 0	-122.2	-122.2	0.72 (1)	3.23	
E- F	-5067 / 0	-122.2	-122.2	0.72 (1)	3.23	
F- G	-4359 / 0	-122.2	-122.2	0.67 (1)	3.52	
G- H	-2751 / 0	-122.2	-122.2	0.59 (1)	3.65	
H- I	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
R- B	-3056 / 0	0.0	0.0	0.23 (1)	6.03	
J- H	-3056 / 0	0.0	0.0	0.23 (1)	6.03	
R- Q	0 / 0	-27.5	-27.5	0.25 (2)	10.00	
Q- P	0 / 1939	-27.5	-27.5	0.51 (2)	10.00	
P- O	0 / 4359	-27.5	-27.5	0.87 (1)	10.00	
O- N	0 / 4359	-27.5	-27.5	0.87 (1)	10.00	
N- M	0 / 4359	-27.5	-27.5	0.87 (1)	10.00	
M- L	0 / 4359	-27.5	-27.5	0.87 (1)	10.00	
L- K	0 / 1939	-27.5	-27.5	0.51 (2)	10.00	
K- J	0 / 0	-27.5	-27.5	0.25 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-496 / 59	0.27 (1)
C- P	0 / 3020	0.68 (1)
P- D	-1596 / 0	0.87 (1)
D- N	0 / 887	0.20 (1)
N- E	-863 / 0	0.47 (1)
N- F	0 / 887	0.20 (1)
L- F	-1596 / 0	0.87 (1)
L- G	0 / 3020	0.68 (1)
K- G	-496 / 59	0.27 (1)
B- Q	0 / 2065	0.46 (1)
K- H	0 / 2065	0.46 (1)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM215966a 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 893 (0.52")

CSI: TC=0.72 (D-E:1) , BC=0.87 (N-P:1) , WB=0.87 (F-L:1) , SSI=0.34 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (P) (INPUT = 0.90)
JSI METAL= 0.85 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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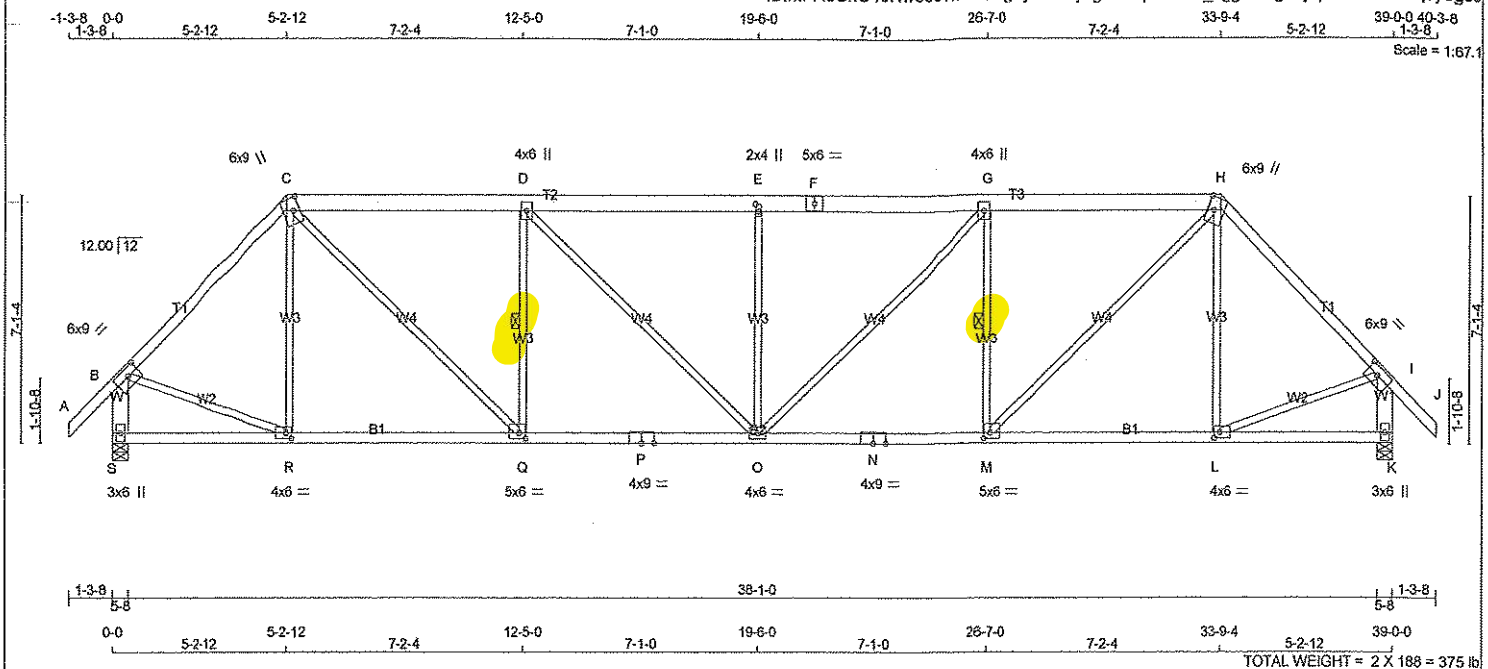

TOTAL WEIGHT = 2 X 188 = 375 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.00
C	TTWW+m	MT20	6.0	9.0	4.50	2.25
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.50	2.25
I	TMVW-t	MT20	6.0	9.0	2.75	4.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.00
M	BMWW-t	MT20	5.0	6.0	2.00	2.25
N	BS-t	MT20	4.0	9.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BS-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	5.0	6.0	2.00	2.25
R	BMWW-t	MT20	4.0	6.0	2.00	2.00
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3090	0	3090	0	0	5-8	5-8
K	3090	0	3090	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0
K	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-Q, G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2804 / 0	-122.2	-122.2	1.00 (1)	3.21
C- D	-3778 / 0	-122.2	-122.2	0.55 (1)	3.89
D- E	-4303 / 0	-122.2	-122.2	0.58 (1)	3.64
E- F	-4303 / 0	-122.2	-122.2	0.58 (1)	3.64
F- G	-4303 / 0	-122.2	-122.2	0.58 (1)	3.64
G- H	-3778 / 0	-122.2	-122.2	0.55 (1)	3.89
H- I	-2804 / 0	-122.2	-122.2	1.00 (1)	3.21
I- J	0 / 60	-122.2	-122.2	0.17 (1)	10.00
S- B	-3037 / 0	0.0	0.0	0.22 (1)	6.05
K- I	-3037 / 0	0.0	0.0	0.22 (1)	6.05
S- R	0 / 0	-27.5	-27.5	0.25 (2)	10.00
R- Q	0 / 1979	-27.5	-27.5	0.47 (2)	10.00
Q- P	0 / 3778	-27.5	-27.5	0.75 (1)	10.00
P- O	0 / 3778	-27.5	-27.5	0.75 (1)	10.00
O- N	0 / 3778	-27.5	-27.5	0.75 (1)	10.00
N- M	0 / 3778	-27.5	-27.5	0.75 (1)	10.00
M- L	0 / 1979	-27.5	-27.5	0.47 (2)	10.00
L- K	0 / 0	-27.5	-27.5	0.25 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-375 / 113	0.31 (1)
C- Q	0 / 2467	0.56 (1)
Q- D	-1492 / 0	0.46 (1)
D- O	0 / 724	0.16 (1)
O- E	-804 / 0	0.66 (1)
O- G	0 / 724	0.16 (1)
M- G	-1492 / 0	0.46 (1)
M- H	0 / 2467	0.56 (1)
L- H	-375 / 113	0.31 (1)
B- R	0 / 2364	0.45 (1)
L- I	0 / 2364	0.45 (1)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45967-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=1.00 (B-C:1) , BC=0.75 (M-O:1) , WB=0.66 (E-O:1) , SSI=0.32 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.79 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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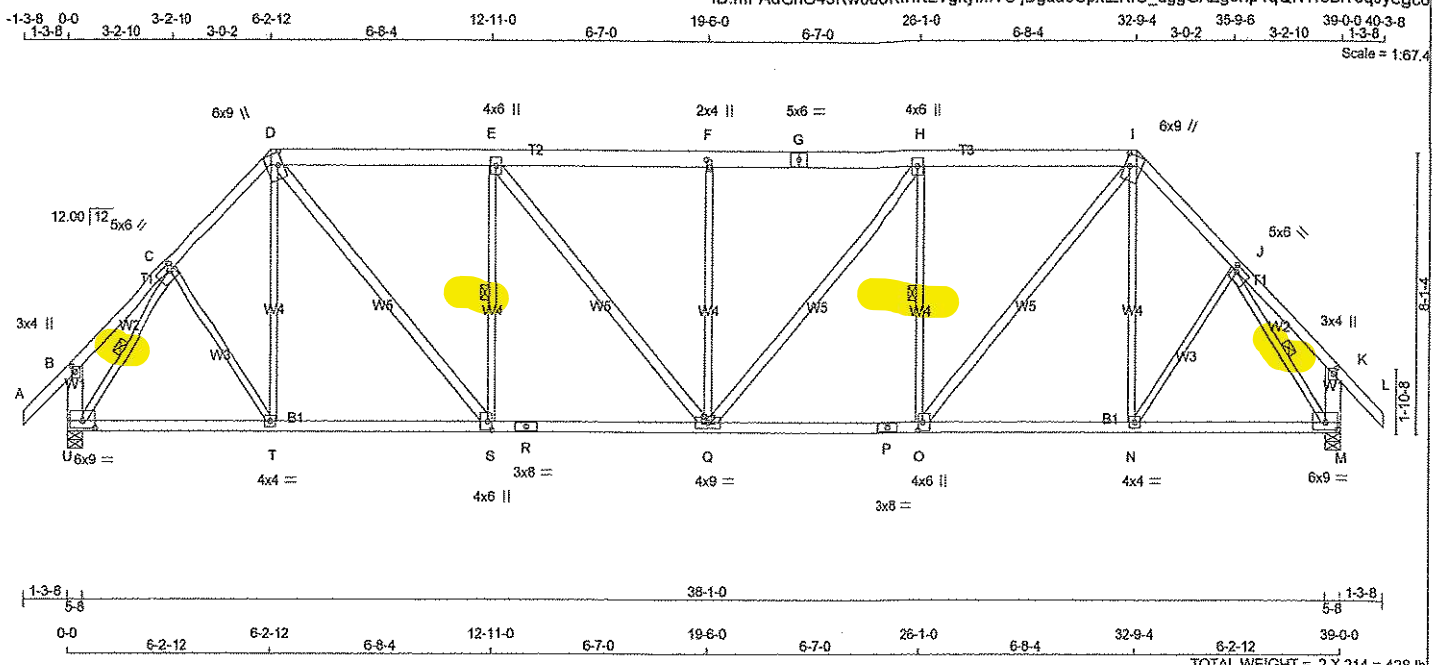

TOTAL WEIGHT = 2 X 214 = 428 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - S	2x4	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C	TMWW-t	MT20	5.0	6.0	2.00	1.50
D	TTWW+m	MT20	6.0	9.0	4.25	1.75
E	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	4.25	1.75
J	TMWW-t	MT20	5.0	6.0	2.00	1.50
K	TMV+p	MT20	3.0	4.0	2.00	1.25
M	BMVW1-t	MT20	6.0	9.0	2.50	4.50
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0	3.00	1.50
P	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	8.0		
S	BMWW+t	MT20	4.0	6.0	3.00	1.50
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	6.0	9.0	2.50	4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3090	0	3090	0	0	5-8	5-8
M	3090	0	3090	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0
M	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 26	-122.2	-122.2	0.16 (1)	10.00
C- D	-2836 / 0	-122.2	-122.2	0.28 (1)	3.91
D- E	-3344 / 0	-122.2	-122.2	0.46 (1)	4.20
E- F	-3738 / 0	-122.2	-122.2	0.48 (1)	3.99
F- G	-3738 / 0	-122.2	-122.2	0.48 (1)	3.99
G- H	-3738 / 0	-122.2	-122.2	0.48 (1)	3.99
H- I	-3344 / 0	-122.2	-122.2	0.46 (1)	4.20
I- J	-2836 / 0	-122.2	-122.2	0.28 (1)	3.91
J- K	0 / 26	-122.2	-122.2	0.16 (1)	10.00
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00
U- B	-321 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-321 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 1805	-27.5	-27.5	0.45 (2)	10.00
T- S	0 / 1990	-27.5	-27.5	0.48 (2)	10.00
S- R	0 / 3344	-27.5	-27.5	0.66 (1)	10.00
R- Q	0 / 3344	-27.5	-27.5	0.66 (1)	10.00
Q- P	0 / 3344	-27.5	-27.5	0.66 (1)	10.00
P- O	0 / 3344	-27.5	-27.5	0.66 (1)	10.00
O- N	0 / 1990	-27.5	-27.5	0.48 (2)	10.00
N- M	0 / 1805	-27.5	-27.5	0.45 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 321	0.07 (1)
T- D	-64 / 173	0.08 (1)
D- S	0 / 2071	0.33 (1)
S- E	-1389 / 0	0.55 (1)
E- Q	0 / 607	0.10 (1)
Q- F	-745 / 0	0.89 (1)
Q- H	0 / 607	0.10 (1)
O- H	-1389 / 0	0.55 (1)
O- I	0 / 2071	0.33 (1)
N- I	-64 / 173	0.08 (1)
N- J	0 / 321	0.07 (1)
U- C	-3247 / 0	0.73 (1)
J- M	-3247 / 0	0.73 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.48 (E-F:1) , BC=0.66 (Q-S:1) , WB=0.89 (F-Q:1) , SSI=0.29 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.84 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9117
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4596B17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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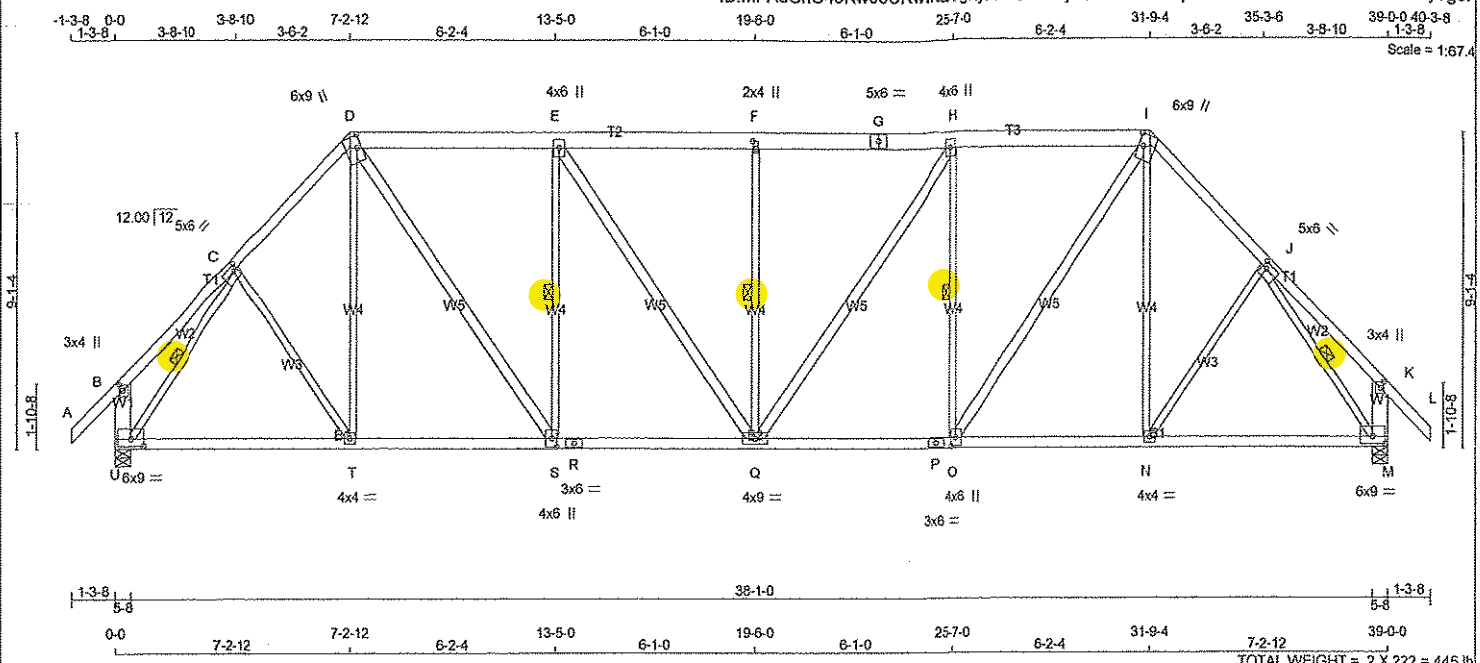

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - S	2x4	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C	TMWW-t	MT20	5.0	6.0	2.00	1.50
D	TTWW+m	MT20	6.0	9.0	4.25	1.75
E	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	4.25	1.75
J	TMWW-t	MT20	5.0	6.0	2.00	1.50
K	TMV+p	MT20	3.0	4.0	2.00	1.25
M	BMVW1-t	MT20	6.0	9.0	2.50	4.50
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0	3.00	1.75
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0	3.00	1.75
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	6.0	9.0	2.50	4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3090	0	3090	0	0	5-8	5-8
M	3090	0	3090	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0
M	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-Q, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 33	-122.2	-122.2	0.24 (1)	10.00	
C- D	-2832 / 0	-122.2	-122.2	0.38 (1)	3.81	
D- E	-3004 / 0	-122.2	-122.2	0.38 (1)	4.49	
E- F	-3302 / 0	-122.2	-122.2	0.39 (1)	4.30	
F- G	-3302 / 0	-122.2	-122.2	0.39 (1)	4.30	
G- H	-3302 / 0	-122.2	-122.2	0.39 (1)	4.30	
H- I	-3004 / 0	-122.2	-122.2	0.38 (1)	4.49	
I- J	-2832 / 0	-122.2	-122.2	0.38 (1)	3.81	
J- K	0 / 33	-122.2	-122.2	0.24 (1)	10.00	
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
U- B	-341 / 0	0.0	0.0	0.03 (1)	7.81	
M- K	-341 / 0	0.0	0.0	0.03 (1)	7.81	
U- T	0 / 1869	-27.5	-27.5	0.51 (2)	10.00	
T- S	0 / 1984	-27.5	-27.5	0.53 (2)	10.00	
S- R	0 / 3004	-27.5	-27.5	0.58 (1)	10.00	
R- Q	0 / 3004	-27.5	-27.5	0.58 (1)	10.00	
Q- P	0 / 3004	-27.5	-27.5	0.58 (1)	10.00	
P- O	0 / 3004	-27.5	-27.5	0.58 (1)	10.00	
O- N	0 / 1984	-27.5	-27.5	0.53 (2)	10.00	
N- M	0 / 1869	-27.5	-27.5	0.51 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 207	0.05 (2)
T- D	0 / 229	0.05 (3)
D- S	0 / 1766	0.28 (1)
S- E	-1285 / 0	0.65 (1)
E- Q	0 / 522	0.08 (1)
Q- F	-687 / 0	0.35 (1)
Q- H	0 / 522	0.08 (1)
O- H	-1285 / 0	0.66 (1)
O- I	0 / 1766	0.28 (1)
N- I	0 / 229	0.05 (3)
N- J	0 / 207	0.05 (2)
U- C	-3256 / 0	0.88 (1)
J- M	-3256 / 0	0.88 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.39 (F-H:1) , BC=0.58 (Q-S:1) , WB=0.86 (C-U:1) , SSI=0.27 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (R) (INPUT = 0.90)
JSI METAL= 0.80 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/12/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45969-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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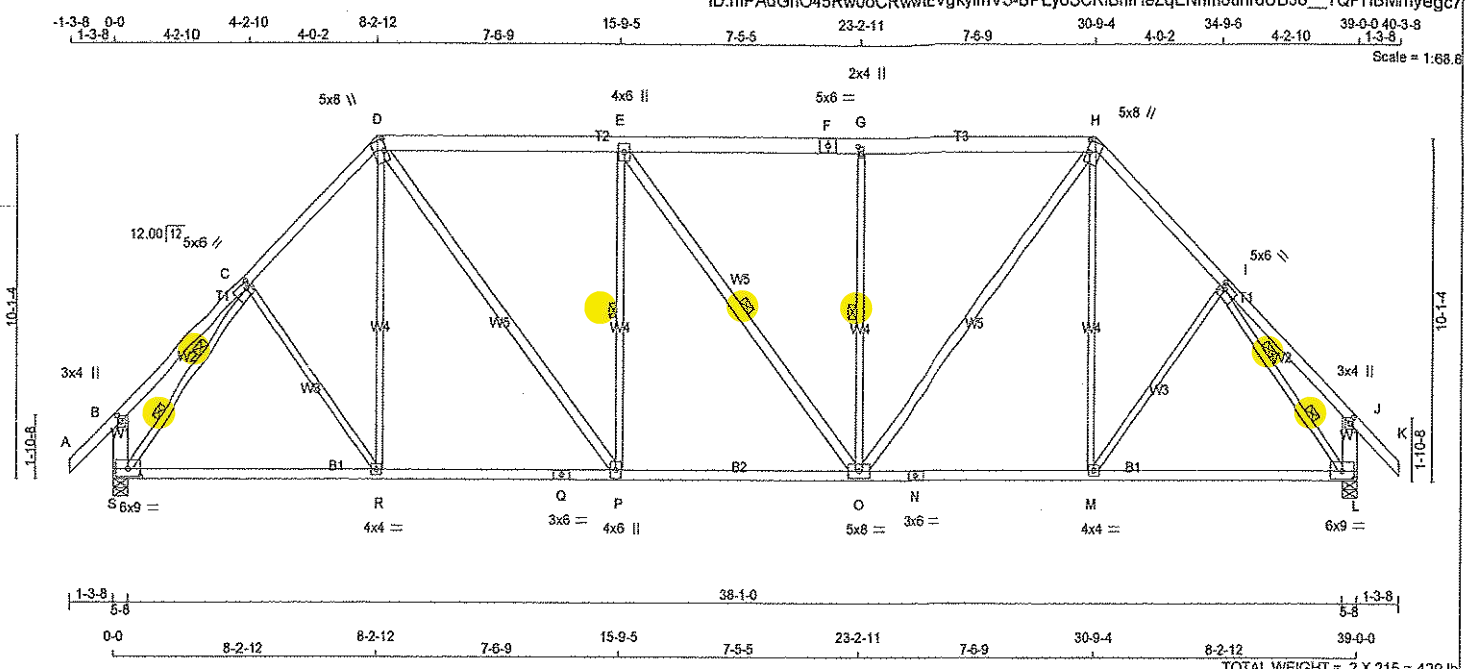

TOTAL WEIGHT = 2 X 215 = 429 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
E - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C	TMWW-t	MT20	5.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	8.0	4.25	1.00
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TTWW+m	MT20	5.0	8.0	4.25	1.00
I	TMWW-t	MT20	5.0	6.0	2.00	1.50
J	TMV+p	MT20	3.0	4.0	2.00	1.25
L	BMVW1-t	MT20	6.0	9.0	2.75	4.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	6.0	9.0	2.75	4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3090	0	3090	0	0	5-8	5-8
L	3090	0	3090	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0
L	2386	1598 / 0	390 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, E-O, G-O.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 40	-122.2	-122.2	0.32 (1)	10.00
C- D	-2812 / 0	-122.2	-122.2	0.52 (1)	3.70
D- E	-2899 / 0	-122.2	-122.2	0.54 (1)	4.33
E- F	-2896 / 0	-122.2	-122.2	0.54 (1)	4.33
F- G	-2896 / 0	-122.2	-122.2	0.54 (1)	4.33
G- H	-2898 / 0	-122.2	-122.2	0.52 (1)	4.36
H- I	-2813 / 0	-122.2	-122.2	0.52 (1)	3.70
I- J	0 / 40	-122.2	-122.2	0.32 (1)	10.00
J- K	0 / 60	-122.2	-122.2	0.17 (1)	10.00
S- B	-363 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-363 / 0	0.0	0.0	0.03 (1)	7.81
S- R	0 / 1917	-27.5	-27.5	0.62 (2)	10.00
R- Q	0 / 1966	-27.5	-27.5	0.64 (2)	10.00
Q- P	0 / 1966	-27.5	-27.5	0.64 (2)	10.00
P- O	0 / 2899	-27.5	-27.5	0.62 (1)	10.00
O- N	0 / 1966	-27.5	-27.5	0.64 (2)	10.00
N- M	0 / 1966	-27.5	-27.5	0.64 (2)	10.00
M- L	0 / 1917	-27.5	-27.5	0.62 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 152	0.03 (3)
R- D	0 / 311	0.07 (2)
D- P	0 / 1523	0.24 (1)
P- E	-1008 / 0	0.67 (1)
E- O	-4 / 0	0.00 (1)
O- G	-1003 / 0	0.67 (1)
O- H	0 / 1519	0.24 (1)
M- H	0 / 312	0.07 (2)
M- I	0 / 152	0.03 (3)
S- C	-3258 / 0	0.86 (1)
I- L	-3258 / 0	0.86 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.54 (D-E:1) , BC=0.64 (M-O:2) , WB=0.96 (I-L:1) , SSI=0.33 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.80 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45972-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T8S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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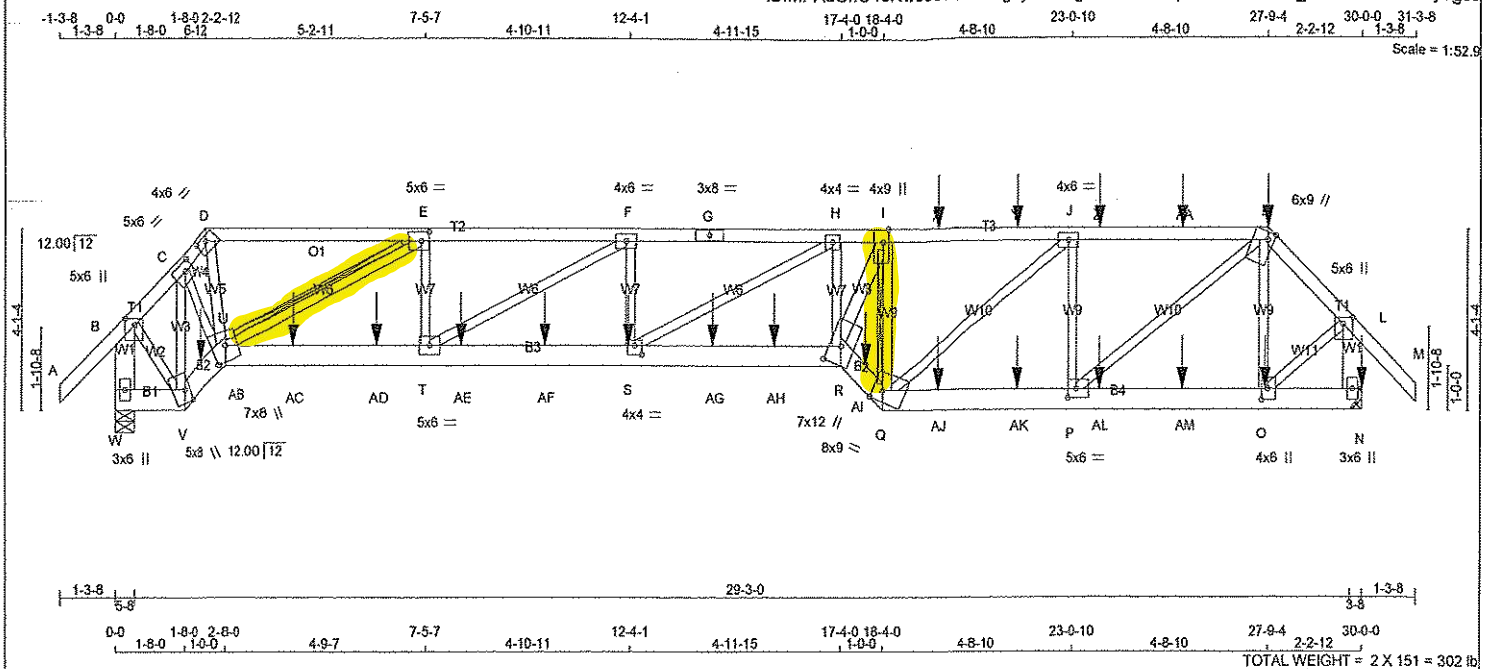

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - V	2x6	DRY	No.2	SPF
V - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
G- K	1	12	SIDE(0.0)
K- M	1	12	SIDE(0.0)
W- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- V	2	12	SIDE
V- U	2	12	SIDE(0.0)
U- R	2	12	SIDE(0.0)
R- Q	2	12	SIDE(0.0)
Q- N	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	5.0	6.0	2.25	2.50
D	TTW-h	MT20	4.0	6.0	1.25	3.00
E	TMWW-t	MT20	5.0	6.0	2.50	2.25
F	TMWW-t	MT20	4.0	6.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	4.0	9.0	Edge	1.50
J	TMWW-t	MT20	4.0	6.0		
K	TTWW+m	MT20	6.0	9.0	Edge	1.75
L	TMVW+p	MT20	5.0	6.0	1.75	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
W	3793	0	3793	0	0	5-8	5-8
N	3805	0	3805	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2966	1921 / 0	526 / 0	0 / 0	0 / 0	519 / 0	0 / 0
N	2992	1907 / 0	549 / 0	0 / 0	0 / 0	536 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-Q, E-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.09 (1)	10.00
B- C	-2690 / 0	-122.2	-122.2	0.13 (1)	5.43
C- D	-4057 / 0	-122.2	-122.2	0.16 (1)	4.57
D- E	-3325 / 0	-122.2	-122.2	0.29 (1)	4.83
E- F	-7759 / 0	-122.2	-122.2	0.53 (1)	3.16
F- G	-10034 / 0	-122.2	-122.2	0.70 (1)	2.66
G- H	-10034 / 0	-122.2	-122.2	0.70 (1)	2.66
H- I	-10219 / 0	-122.2	-122.2	0.48 (1)	2.73
I- X	-7272 / 0	-122.2	-122.2	0.65 (1)	3.05
X- Y	-7272 / 0	-122.2	-122.2	0.65 (1)	3.05
Y- J	-7272 / 0	-122.2	-122.2	0.65 (1)	3.05
J- Z	-5423 / 0	-122.2	-122.2	0.54 (1)	3.64
Z-AA	-5423 / 0	-122.2	-122.2	0.54 (1)	3.64
AA- K	-5423 / 0	-122.2	-122.2	0.54 (1)	3.64
K- L	-3046 / 0	-122.2	-122.2	0.09 (1)	5.22
L- M	0 / 60	-122.2	-122.2	0.09 (1)	10.00
W- B	-3770 / 0	0.0	0.0	0.15 (1)	7.26
N- L	-3794 / 0	0.0	0.0	0.15 (1)	7.26
W- V	0 / 0	-27.5	-27.5	0.01 (2)	10.00
V-AB	0 / 2434	-27.5	-27.5	0.18 (1)	10.00
AB- U	0 / 2434	-27.5	-27.5	0.18 (1)	10.00
U-AC	0 / 7759	-27.5	-27.5	0.62 (1)	10.00
AC-AD	0 / 7759	-27.5	-27.5	0.62 (1)	10.00
AD- T	0 / 7759	-27.5	-27.5	0.62 (1)	10.00
T-AE	0 / 10034	-27.5	-27.5	0.77 (1)	10.00
AE-AF	0 / 10034	-27.5	-27.5	0.77 (1)	10.00
AF- S	0 / 10034	-27.5	-27.5	0.77 (1)	10.00
S-AG	0 / 10227	-27.5	-27.5	0.82 (1)	10.00
AG-AH	0 / 10227	-27.5	-27.5	0.82 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- U	0 / 2537	0.31 (1)
R- H	-179 / 24	0.02 (1)
R- I	0 / 8351	0.74 (1)
Q- I	-8217 / 0	0.80 (1)
Q- J	0 / 2345	0.29 (1)
P- J	-2489 / 0	0.32 (1)
P- K	0 / 4211	0.52 (1)
O- K	-1042 / 0	0.13 (1)
O- L	0 / 2513	0.31 (1)
V- C	-3113 / 0	0.32 (1)
B- V	0 / 2368	0.29 (1)
C- U	0 / 2449	0.30 (1)
S- H	-220 / 0	0.06 (1)
U- E	-5982 / 0	0.79 (1)
S- F	-202 / 662	0.09 (1)
T- F	0 / 1814	0.42 (1)
R- F	-2985 / 0	0.70 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 924 (0.39")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 601 (0.60")

CSI: TC=0.70 (F-H:1) , BC=0.82 (R-S:1) , WB=0.80 (I-Q:1) , SSI=0.25 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.72 (R) (INPUT = 1.00)

PROFESSIONAL ENGINEER
LICENSED
9147
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45971-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T8S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMV1+p	MT20	3.0	6.0		
O	BMWW+t	MT20	4.0	6.0	3.00	1.75
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BBWW-m	MT20	8.0	9.0	3.00	2.75
R	BBWW+m	MT20	7.0	12.0	5.00	3.25
S	BMWW-t	MT20	4.0	4.0	2.50	2.00
T	BMWW-t	MT20	5.0	6.0		
U	BBWWW+m	MT20	7.0	8.0	4.25	3.50
V	BBWW+m	MT20	5.0	8.0	3.50	1.50
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 147.1 lbs FACTORED DOWN AT 19-8-8, 147.1 lbs FACTORED DOWN AT 21-8-8, 147.1 lbs FACTORED DOWN AT 23-8-8, AND 147.1 lbs FACTORED DOWN AT 25-8-8, AND 150.6 lbs FACTORED DOWN AT 27-9-4 ON TOP CHORD, AND 208.6 lbs FACTORED DOWN AT 2-3-8, 194.9 lbs FACTORED DOWN AT 4-3-8, 194.9 lbs FACTORED DOWN AT 6-3-8, 194.9 lbs FACTORED DOWN AT 8-3-8, 194.9 lbs FACTORED DOWN AT 10-3-8, 194.9 lbs FACTORED DOWN AT 12-3-8, 194.9 lbs FACTORED DOWN AT 14-3-8, 194.9 lbs FACTORED DOWN AT 15-9-0, 208.6 lbs FACTORED DOWN AT 17-9-0, 67.8 lbs FACTORED DOWN AT 19-8-8, 67.8 lbs FACTORED DOWN AT 21-8-8, 67.8 lbs FACTORED DOWN AT 23-8-8, 67.8 lbs FACTORED DOWN AT 25-8-8, AND 67.8 lbs FACTORED DOWN AT 27-8-8, AND 84.6 lbs FACTORED DOWN AT 30-0-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AH- R	0 / 10227	-27.5	-27.5	0.82 (1)	10.00
R-AI	0 / 9968	-27.5	-27.5	0.67 (1)	10.00
AI- Q	0 / 9968	-27.5	-27.5	0.67 (1)	10.00
Q-AJ	0 / 5424	-27.5	-27.5	0.41 (1)	10.00
AJ-AK	0 / 5424	-27.5	-27.5	0.41 (1)	10.00
AK- P	0 / 5424	-27.5	-27.5	0.41 (1)	10.00
P-AL	0 / 2110	-27.5	-27.5	0.20 (1)	10.00
AL-AM	0 / 2110	-27.5	-27.5	0.20 (1)	10.00
AM- O	0 / 2110	-27.5	-27.5	0.20 (1)	10.00
O- N	0 / 0	-27.5	-27.5	0.08 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	27-9-4	-151	-151	---	BACK	VERT	TOTAL
N	30-0-0	-49	-85	---	BACK	VERT	TOTAL
O	27-8-8	-39	-68	---	BACK	VERT	TOTAL
S	12-3-8	-195	-195	---	BACK	VERT	TOTAL
X	19-8-8	-147	-147	---	BACK	VERT	TOTAL
Y	21-8-8	-147	-147	---	BACK	VERT	TOTAL
Z	23-8-8	-147	-147	---	BACK	VERT	TOTAL
AA	25-8-8	-147	-147	---	BACK	VERT	TOTAL
AB	2-3-8	-209	-209	---	BACK	VERT	TOTAL
AC	4-3-8	-195	-195	---	BACK	VERT	TOTAL
AD	6-3-8	-195	-195	---	BACK	VERT	TOTAL
AE	8-3-8	-195	-195	---	BACK	VERT	TOTAL
AF	10-3-8	-195	-195	---	BACK	VERT	TOTAL
AG	14-3-8	-195	-195	---	BACK	VERT	TOTAL
AH	15-9-0	-195	-195	---	BACK	VERT	TOTAL
AI	17-9-0	-209	-209	---	BACK	VERT	TOTAL
AJ	19-8-8	-39	-68	---	BACK	VERT	TOTAL
AK	21-8-8	-39	-68	---	BACK	VERT	TOTAL
AL	23-8-8	-39	-68	---	BACK	VERT	TOTAL
AM	25-8-8	-39	-68	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9/27
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45971-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288223	T8	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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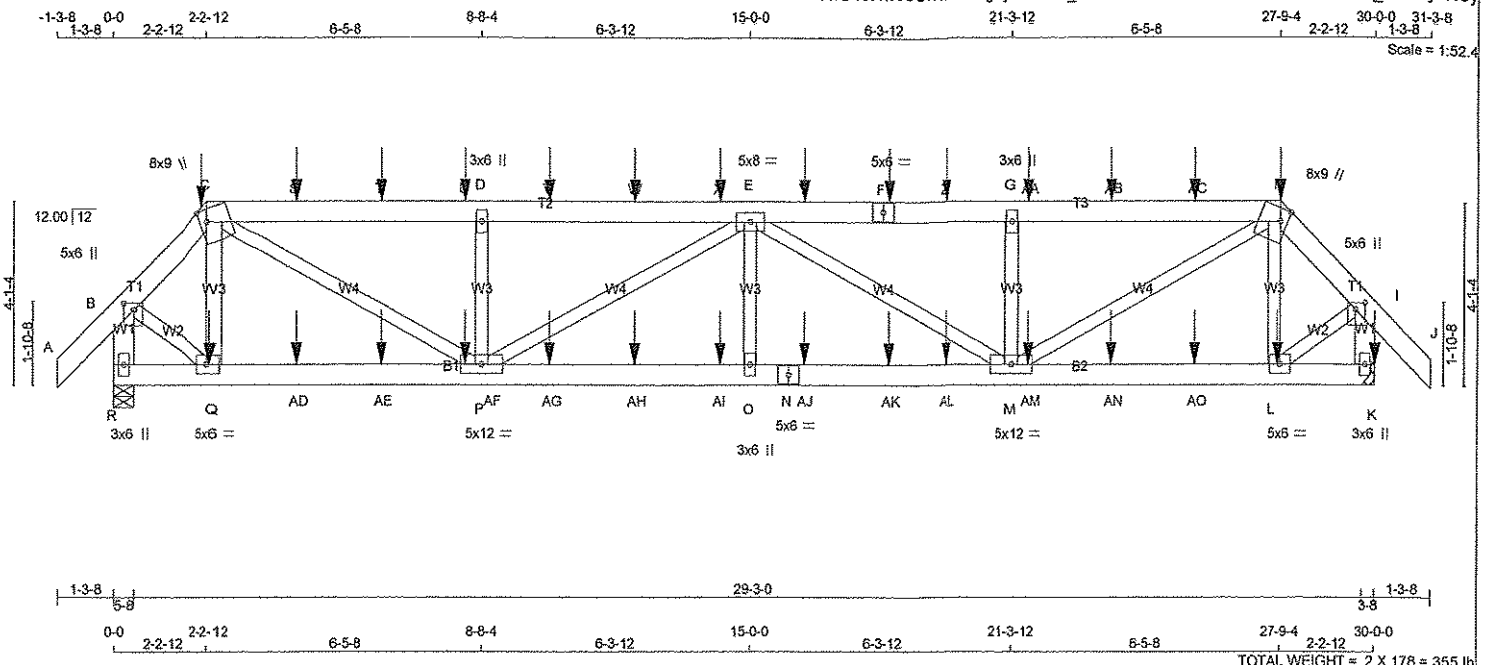

TOTAL WEIGHT = 2 X 178 = 355 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(183.1)
F- H	2	12	SIDE(183.1)
H- J	2	12	SIDE(122.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	3.25	2.00
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.25	2.00
I	TMVW+p	MT20	5.0	6.0	1.75	2.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	12.0		
N	BS-t	MT20	5.0	6.0		
O	BMW+w	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	12.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	4616	0	4616	0	0	5-8	5-8
K	4694	0	4694	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	3551	2412 / 0	567 / 0	0 / 0	0 / 0	571 / 0	0 / 0
K	3639	2423 / 0	614 / 0	0 / 0	0 / 0	603 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 63	-122.2	-122.2	0.05 (1)	10.00
B- C	-3959 / 0	-122.2	-122.2	0.05 (1)	5.66
C- S	-7842 / 0	-122.2	-122.2	0.47 (1)	3.89
S- T	-7842 / 0	-122.2	-122.2	0.47 (1)	3.89
T- U	-7842 / 0	-122.2	-122.2	0.47 (1)	3.89
U- D	-7842 / 0	-122.2	-122.2	0.47 (1)	3.89
D- V	-7843 / 0	-122.2	-122.2	0.46 (1)	3.89
V- W	-7843 / 0	-122.2	-122.2	0.46 (1)	3.89
W- X	-7843 / 0	-122.2	-122.2	0.46 (1)	3.89
X- E	-7843 / 0	-122.2	-122.2	0.46 (1)	3.89
E- Y	-7903 / 0	-122.2	-122.2	0.48 (1)	3.86
Y- F	-7903 / 0	-122.2	-122.2	0.48 (1)	3.86
F- Z	-7903 / 0	-122.2	-122.2	0.48 (1)	3.86
Z- G	-7903 / 0	-122.2	-122.2	0.48 (1)	3.86
G-AA	-7901 / 0	-122.2	-122.2	0.48 (1)	3.86
AA-AB	-7901 / 0	-122.2	-122.2	0.48 (1)	3.86
AB-AC	-7901 / 0	-122.2	-122.2	0.48 (1)	3.86
AC- H	-7901 / 0	-122.2	-122.2	0.48 (1)	3.86
H- I	-3977 / 0	-122.2	-122.2	0.05 (1)	5.66
I- J	0 / 63	-122.2	-122.2	0.05 (1)	10.00
R- B	-4684 / 0	0.0	0.0	0.17 (1)	6.68
K- I	-4704 / 0	0.0	0.0	0.17 (1)	6.67
R- Q	0 / 0	-28.0	-28.0	0.10 (2)	10.00
Q-AD	0 / 2722	-28.0	-28.0	0.27 (1)	10.00
AD-AE	0 / 2722	-28.0	-28.0	0.27 (1)	10.00
AE-AF	0 / 2722	-28.0	-28.0	0.27 (1)	10.00
AF- P	0 / 2722	-28.0	-28.0	0.27 (1)	10.00
P-AG	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
AG-AH	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
AH-AI	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
AI- O	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
O- N	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
N-AJ	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
AJ-AK	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
AK-AL	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
AL- M	0 / 9444	-28.0	-28.0	0.70 (1)	10.00
M-AM	0 / 2734	-28.0	-28.0	0.27 (1)	10.00
AM-AN	0 / 2734	-28.0	-28.0	0.27 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-1177 / 0	0.10 (1)
C- P	0 / 5862	0.52 (1)
P- D	-1650 / 0	0.14 (1)
P- E	-1848 / 0	0.65 (1)
O- E	0 / 588	0.05 (2)
E- M	-1780 / 0	0.63 (1)
M- G	-1700 / 0	0.15 (1)
M- H	0 / 5917	0.52 (1)
L- H	-1186 / 0	0.10 (1)
B- Q	0 / 3178	0.28 (1)
L- I	0 / 3184	0.28 (1)

DESIGN CRITERIA [M]

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.48 (G-H:1) , BC=0.70 (M-O:1) , WB=0.65 (E-P:1) , SSI=0.33 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.73 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9207
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PROVINCE OF ONTARIO

DWG NO. TAM4752B17
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CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.5 lbs FACTORED DOWN AT 2-2-12, 195.5 lbs FACTORED DOWN AT 27-9-4, 240.3 lbs FACTORED DOWN AT 2-2-12, 236.8 lbs FACTORED DOWN AT 4-3-8, 236.8 lbs FACTORED DOWN AT 6-3-8, 236.8 lbs FACTORED DOWN AT 8-3-8, 236.8 lbs FACTORED DOWN AT 10-3-8, 236.8 lbs FACTORED DOWN AT 12-3-8, 236.8 lbs FACTORED DOWN AT 14-3-8, 236.8 lbs FACTORED DOWN AT 16-3-8, 236.8 lbs FACTORED DOWN AT 18-3-8, 236.8 lbs FACTORED DOWN AT 19-8-8, 236.8 lbs FACTORED DOWN AT 21-8-8, 236.8 lbs FACTORED DOWN AT 23-8-8, AND 236.8 lbs FACTORED DOWN AT 25-8-8, AND 240.3 lbs FACTORED DOWN AT 27-9-4 ON TOP CHORD, AND 84.6 lbs FACTORED DOWN AT 2-3-8, 84.6 lbs FACTORED DOWN AT 4-3-8, 84.6 lbs FACTORED DOWN AT 6-3-8, 84.6 lbs FACTORED DOWN AT 8-3-8, 84.6 lbs FACTORED DOWN AT 10-3-8, 84.6 lbs FACTORED DOWN AT 12-3-8, 84.6 lbs FACTORED DOWN AT 14-3-8, 84.6 lbs FACTORED DOWN AT 16-3-8, 84.6 lbs FACTORED DOWN AT 18-3-8, 84.6 lbs FACTORED DOWN AT 19-8-8, 84.6 lbs FACTORED DOWN AT 21-8-8, 84.6 lbs FACTORED DOWN AT 23-8-8, 84.6 lbs FACTORED DOWN AT 25-8-8, AND 84.6 lbs FACTORED DOWN AT 27-8-8, AND 102.0 lbs FACTORED DOWN AT 30-0-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	LC1 TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AN-AO	0 / 2734	-28.0	-28.0	0.27 (1)	10.00
AO- L	0 / 2734	-28.0	-28.0	0.27 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.10 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-13	-14	---	FRONT	VERT	DEAD
C	2-2-12	-240	-240	---	BACK	VERT	TOTAL
C	2-2-12	-183	-183	---	FRONT	VERT	SNOW
F	18-3-8	-237	-237	---	BACK	VERT	TOTAL
H	27-9-4	-13	-14	---	FRONT	VERT	DEAD
H	27-9-4	-240	-240	---	BACK	VERT	TOTAL
H	27-9-4	-183	-183	---	FRONT	VERT	SNOW
K	30-0-0	-58	-102	---	BACK	VERT	TOTAL
L	27-8-8	-48	-85	---	BACK	VERT	TOTAL
Q	2-3-8	-48	-85	---	BACK	VERT	TOTAL
S	4-3-8	-237	-237	---	BACK	VERT	TOTAL
T	6-3-8	-237	-237	---	BACK	VERT	TOTAL
U	8-3-8	-237	-237	---	BACK	VERT	TOTAL
V	10-3-8	-237	-237	---	BACK	VERT	TOTAL
W	12-3-8	-237	-237	---	BACK	VERT	TOTAL
X	14-3-8	-237	-237	---	BACK	VERT	TOTAL
Y	16-3-8	-237	-237	---	BACK	VERT	TOTAL
Z	19-8-8	-237	-237	---	BACK	VERT	TOTAL
AA	21-8-8	-237	-237	---	BACK	VERT	TOTAL
AB	23-8-8	-237	-237	---	BACK	VERT	TOTAL
AC	25-8-8	-237	-237	---	BACK	VERT	TOTAL
AD	4-3-8	-48	-85	---	BACK	VERT	TOTAL
AE	6-3-8	-48	-85	---	BACK	VERT	TOTAL
AF	8-3-8	-48	-85	---	BACK	VERT	TOTAL
AG	10-3-8	-48	-85	---	BACK	VERT	TOTAL
AH	12-3-8	-48	-85	---	BACK	VERT	TOTAL
AI	14-3-8	-48	-85	---	BACK	VERT	TOTAL
AJ	16-3-8	-48	-85	---	BACK	VERT	TOTAL
AK	18-3-8	-48	-85	---	BACK	VERT	TOTAL
AL	19-8-8	-48	-85	---	BACK	VERT	TOTAL
AM	21-8-8	-48	-85	---	BACK	VERT	TOTAL
AN	23-8-8	-48	-85	---	BACK	VERT	TOTAL
AO	25-8-8	-48	-85	---	BACK	VERT	TOTAL

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PROVINCE OF ONTARIO

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DWG NO. TAM47528 17
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T9S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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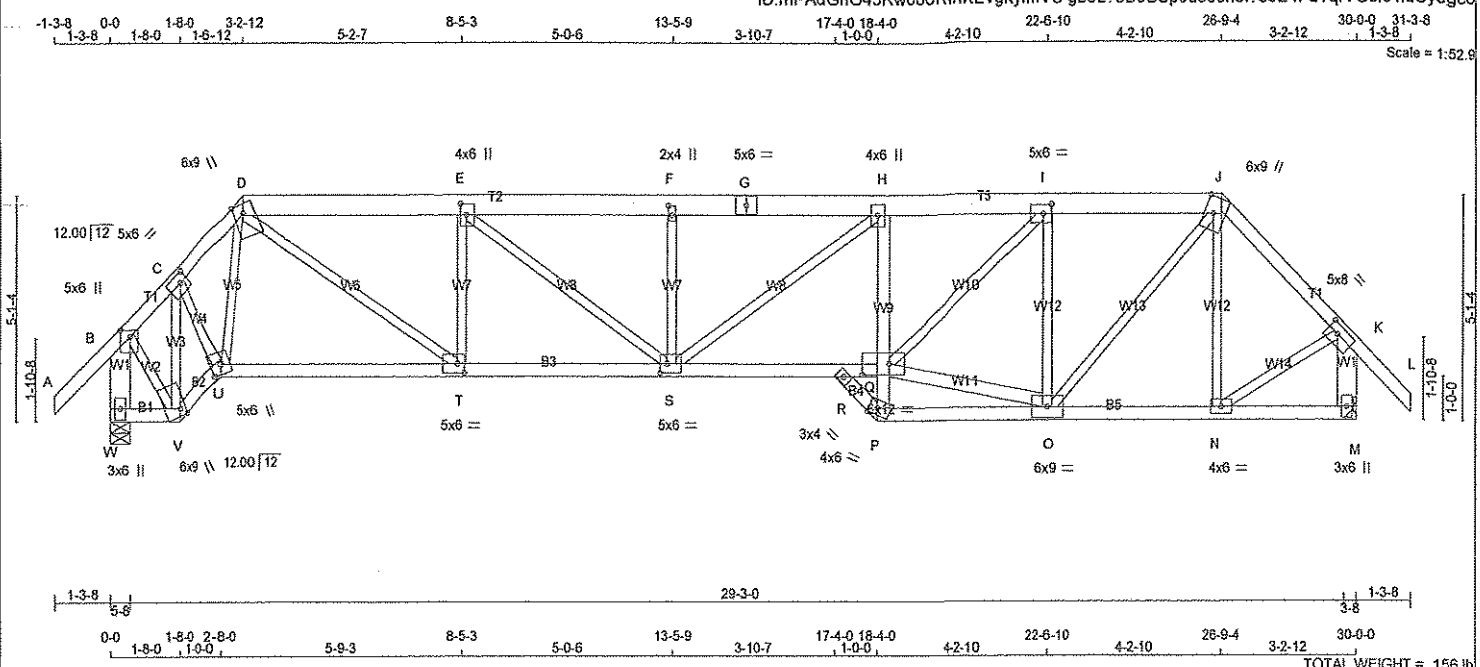

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
P - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW+m	MT20	6.0	9.0	Edge	
E	TMWW+t	MT20	4.0	6.0	3.00	1.75
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TTWW+m	MT20	6.0	9.0	4.50	2.50
K	TMVW-t	MT20	5.0	8.0	2.25	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	6.0	9.0		
P	BBW-m	MT20	4.0	6.0	Edge	2.25
Q	BWMWW*-l	MT20	6.0	12.0	3.00	7.75
R	BMB-h	MT20	3.0	4.0		
S	BMWWW-t	MT20	5.0	6.0	2.50	2.00
T	BMWW-t	MT20	5.0	6.0	2.50	2.00
U	BBWW+m	MT20	5.0	6.0	2.75	2.50
V	BBWW+m	MT20	6.0	9.0	Edge	1.75
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2420	0	2420	0	0	5-8	5-8
M	2412	0	2412	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1865	1256 / 0	300 / 0	0 / 0	0 / 0	309 / 0	0 / 0
M	1860	1251 / 0	300 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-1586 / 0	-122.2	-122.2	0.19 (1)	5.04
C- D	-2358 / 0	-122.2	-122.2	0.14 (1)	4.36
D- E	-3711 / 0	-122.2	-122.2	0.29 (1)	4.22
E- F	-4519 / 0	-122.2	-122.2	0.29 (1)	3.69
F- G	-4519 / 0	-122.2	-122.2	0.28 (1)	3.90
G- H	-4519 / 0	-122.2	-122.2	0.28 (1)	3.90
H- I	-4240 / 0	-122.2	-122.2	0.21 (1)	4.08
I- J	-2673 / 0	-122.2	-122.2	0.18 (1)	4.94
J- K	-1953 / 0	-122.2	-122.2	0.28 (1)	4.55
K- L	0 / 60	-122.2	-122.2	0.17 (1)	10.00
W- B	-2397 / 0	0.0	0.0	0.18 (1)	6.65
M- K	-2378 / 0	0.0	0.0	0.18 (1)	6.68
W- V	0 / 0	-27.5	-27.5	0.02 (3)	10.00
V- U	0 / 1426	-27.5	-27.5	0.23 (1)	10.00
U- T	0 / 1705	-27.5	-27.5	0.39 (1)	10.00
T- S	0 / 3711	-27.5	-27.5	0.69 (1)	10.00
S- R	0 / 4266	-27.5	-27.5	0.85 (1)	10.00
R- Q	0 / 4076	0.0	0.0	0.82 (1)	10.00
R- P	-33 / 310	-27.5	-27.5	0.05 (1)	6.25
P- O	0 / 318	-27.5	-27.5	0.13 (2)	10.00
O- N	0 / 1372	-27.5	-27.5	0.28 (1)	10.00
N- M	0 / 0	-27.5	-27.5	0.08 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1935 / 0	0.39 (1)
C- U	0 / 1423	0.32 (1)
U- D	-156 / 52	0.04 (1)
P- Q	-193 / 122	0.13 (1)
Q- H	-721 / 0	0.15 (1)
O- I	-1957 / 0	0.70 (1)
O- J	0 / 1976	0.44 (1)
N- J	-520 / 0	0.19 (1)
B- V	0 / 1442	0.32 (1)
N- K	0 / 1521	0.34 (1)
T- E	-1292 / 0	0.31 (1)
S- F	-617 / 0	0.15 (1)
D- T	0 / 2470	0.56 (1)
E- S	0 / 1008	0.23 (1)
S- H	0 / 314	0.07 (1)
Q- O	0 / 2456	0.39 (1)
Q- I	0 / 2077	0.47 (1)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 865 (0.42")

CSI: TC=0.29 (D-E:1) , BC=0.86 (R-S:1) , WB=0.70 (I-O:1) , SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.67 (Q) (INPUT = 1.00)

DWG NO. TAM 45972-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288223	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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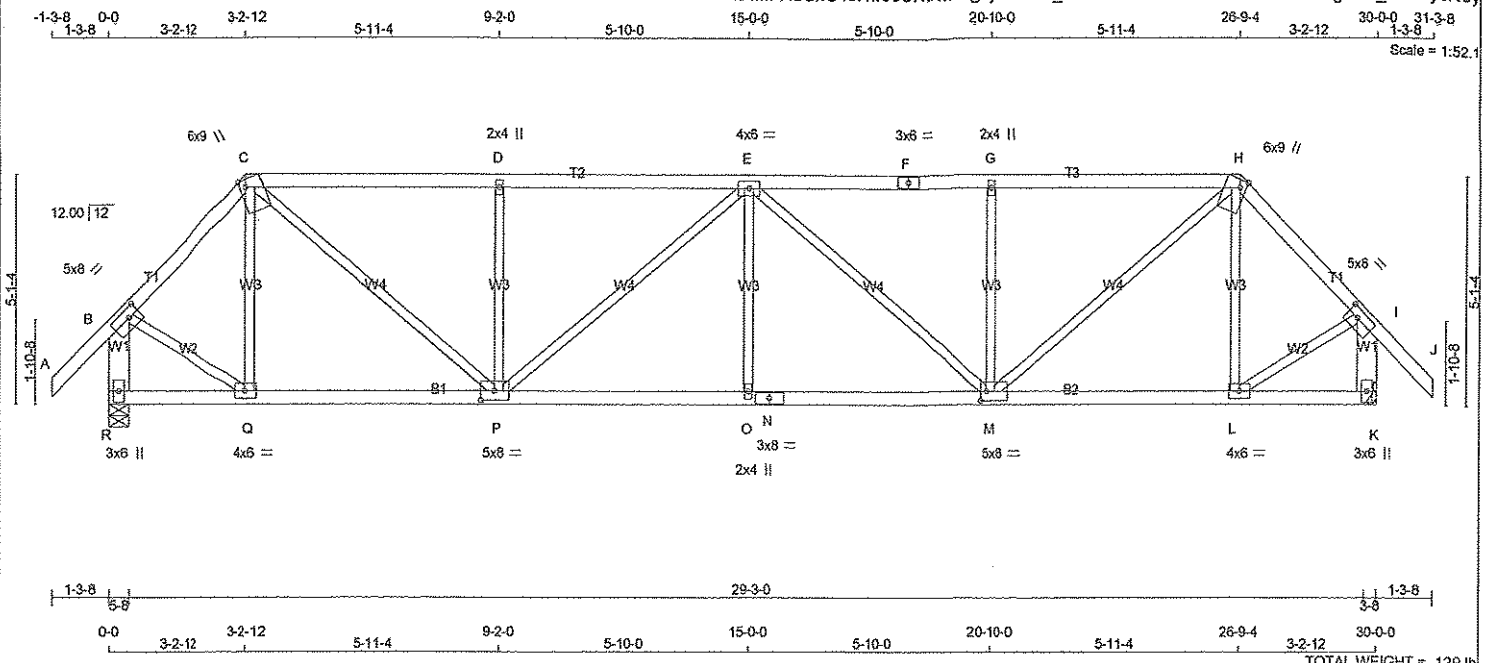

TOTAL WEIGHT = 129 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-t	MT20	5.0	8.0	2.25	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWWW-t	MT20	5.0	8.0	2.50	1.75
N	BS-t	MT20	3.0	8.0		
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	3.75
Q	BMWW-t	MT20	4.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2424	0	2424	0	0	IN-SX 5-8	IN-SX 5-8
K	2424	0	2424	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1877	1254 / 0	315 / 0	0 / 0	0 / 0	308 / 0	0 / 0
K	1877	1254 / 0	315 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-1968 / 0	-122.2	-122.2	0.29 (1)	4.53
C- D	-3083 / 0	-122.2	-122.2	0.92 (1)	2.97
D- E	-3084 / 0	-122.2	-122.2	0.91 (1)	2.97
E- F	-3084 / 0	-122.2	-122.2	0.91 (1)	2.97
F- G	-3084 / 0	-122.2	-122.2	0.91 (1)	2.97
G- H	-3083 / 0	-122.2	-122.2	0.92 (1)	2.97
H- I	-1968 / 0	-122.2	-122.2	0.29 (1)	4.53
I- J	0 / 60	-122.2	-122.2	0.17 (1)	10.00
R- B	-2399 / 0	0.0	0.0	0.18 (1)	6.65
K- I	-2399 / 0	0.0	0.0	0.18 (1)	6.65
R- Q	0 / 0	-28.0	-28.0	0.16 (2)	10.00
Q- P	0 / 1375	-28.0	-28.0	0.33 (2)	10.00
P- O	0 / 3584	-28.0	-28.0	0.66 (1)	10.00
O- N	0 / 3584	-28.0	-28.0	0.66 (1)	10.00
N- M	0 / 3584	-28.0	-28.0	0.66 (1)	10.00
M- L	0 / 1375	-28.0	-28.0	0.33 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.16 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-481 / 8	0.19 (1)
C- P	0 / 2194	0.49 (1)
P- D	-803 / 0	0.31 (1)
P- E	-648 / 0	0.74 (1)
O- E	0 / 282	0.06 (2)
E- M	-648 / 0	0.74 (1)
M- G	-803 / 0	0.31 (1)
M- H	0 / 2194	0.49 (1)
L- H	-481 / 8	0.19 (1)
B- Q	0 / 1523	0.34 (1)
L- I	0 / 1523	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.92 (G-H:1) , BC=0.66 (O-P:1) , WB=0.74 (E-P:1) , SSI=0.34 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.94 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47529 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T10S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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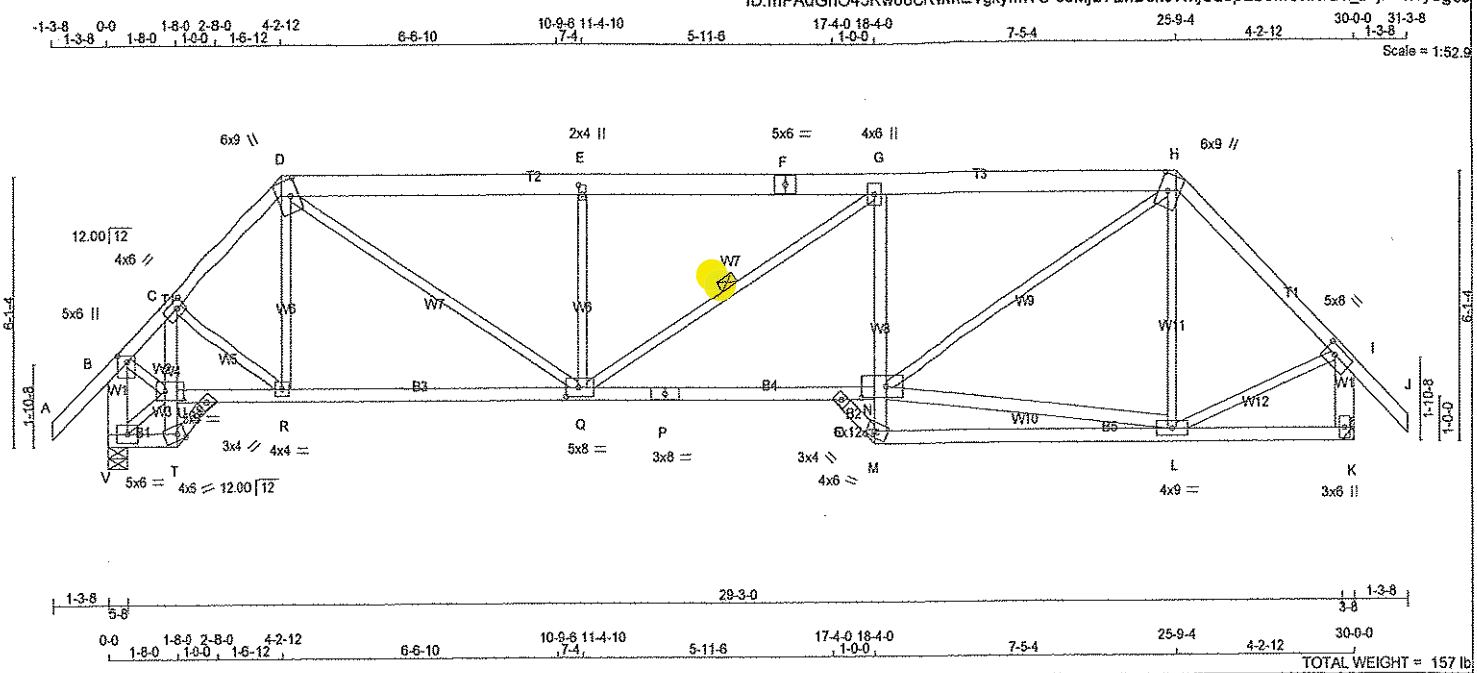

TOTAL WEIGHT = 157 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTWW+m	MT20	6.0	9.0	4.50	2.25
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.50	2.50
I	TMVW-t	MT20	5.0	8.0	2.25	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BBW-m	MT20	4.0	6.0	Edge	2.25
N	BWMWW*-l	MT20	6.0	12.0	3.00	7.25
O	BMB-h	MT20	3.0	4.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	5.0	8.0	2.50	3.50
R	BMWW-t	MT20	4.0	4.0		
S	BMB-h	MT20	3.0	4.0		
T	BBW-m	MT20	4.0	6.0	Edge	2.25
U	BWMWW*-l	MT20	5.0	8.0	2.50	5.50
V	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2420	0	2420	0	0	5-8	5-8
K	2412	0	2412	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1865	1256 / 0	300 / 0	0 / 0	0 / 0	309 / 0	0 / 0
K	1860	1251 / 0	300 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.75 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2112 / 0	-122.2	-122.2	0.21 (1)	4.47
C- D	-2442 / 0	-122.2	-122.2	0.21 (1)	4.24
D- E	-3480 / 0	-122.2	-122.2	0.48 (1)	4.11
E- F	-3480 / 0	-122.2	-122.2	0.49 (1)	4.10
F- G	-3480 / 0	-122.2	-122.2	0.49 (1)	4.10
G- H	-3470 / 0	-122.2	-122.2	0.49 (1)	4.10
H- I	-2036 / 0	-122.2	-122.2	0.50 (1)	4.22
I- J	0 / 60	-122.2	-122.2	0.17 (1)	10.00
V- B	-2192 / 0	0.0	0.0	0.16 (1)	6.89
K- I	-2369 / 0	0.0	0.0	0.18 (1)	6.67
V- T	0 / 292	-27.5	-27.5	0.06 (1)	10.00
T- S	0 / 364	-27.5	-27.5	0.06 (1)	10.00
U- S	0 / 1268	0.0	0.0	0.31 (1)	10.00
S- R	0 / 1483	-27.5	-27.5	0.38 (1)	10.00
R- Q	0 / 1724	-27.5	-27.5	0.45 (2)	10.00
Q- P	0 / 3487	-27.5	-27.5	0.72 (1)	10.00
P- O	0 / 3487	-27.5	-27.5	0.72 (1)	10.00
O- N	0 / 3468	0.0	0.0	0.61 (1)	10.00
O- M	-233 / 32	-27.5	-27.5	0.01 (1)	6.25
M- L	-158 / 89	-27.5	-27.5	0.35 (3)	6.25
L- K	0 / 0	-27.5	-27.5	0.34 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- U	-248 / 0	0.04 (1)
U- C	-645 / 0	0.05 (1)
R- D	0 / 223	0.05 (3)
M- N	0 / 340	0.13 (2)
N- G	-960 / 0	0.19 (1)
L- H	-496 / 65	0.27 (1)
L- I	0 / 1533	0.35 (1)
N- L	0 / 1367	0.22 (1)
N- H	0 / 2415	0.54 (1)
Q- E	-951 / 0	0.34 (1)
D- Q	0 / 2106	0.47 (1)
Q- G	-8 / 0	0.00 (1)
C- R	0 / 297	0.07 (1)
V- U	-360 / 0	0.05 (1)
B- U	0 / 1523	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.50 (H-I:1) , BC=0.72 (O-Q:1) , WB=0.54 (H-N:1) , SSI=0.32 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.96 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4597317
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288223	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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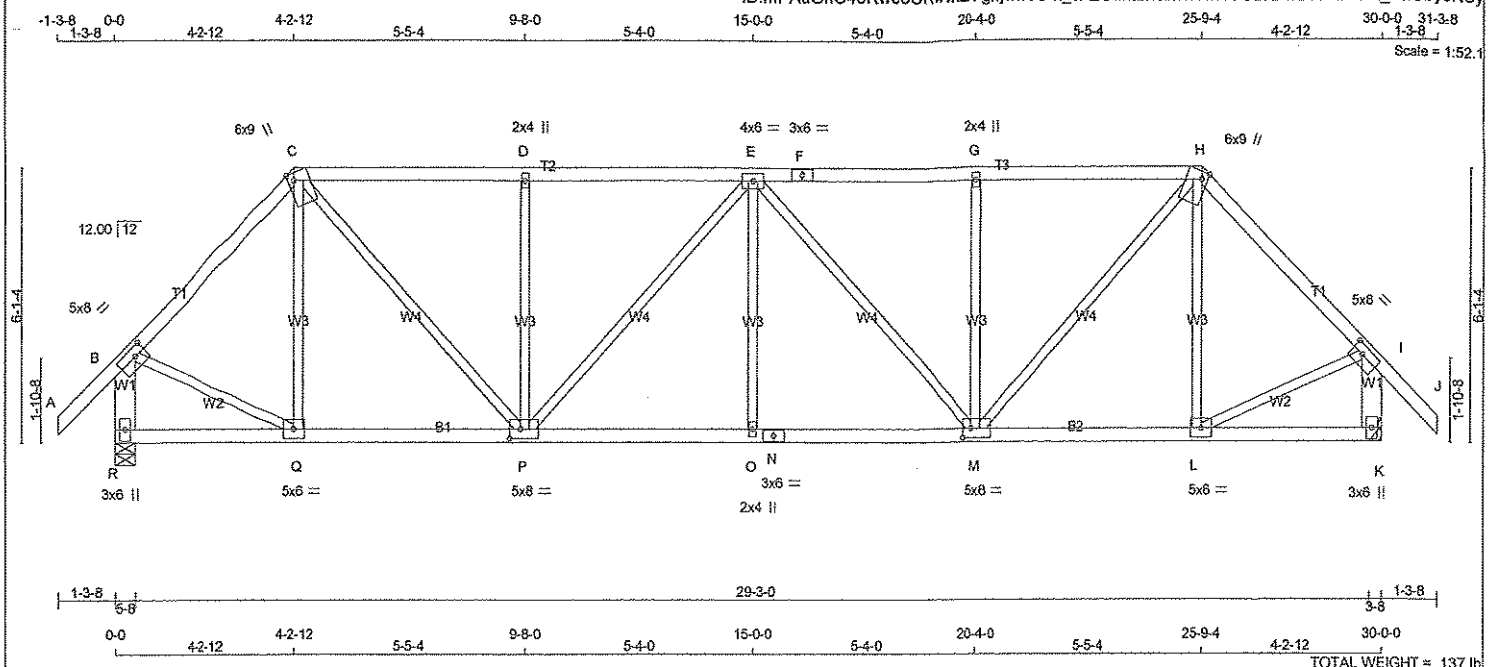

TOTAL WEIGHT = 137 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-t	MT20	5.0	8.0	2.25	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0	2.50	2.50
N	BS-t	MT20	3.0	6.0		
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	3.00
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2424	0	2424	0	0	5-8	5-8
K	2424	0	2424	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1877	1254 / 0	315 / 0	0 / 0	0 / 0	308 / 0	0 / 0
K	1877	1254 / 0	315 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-2037 / 0	-122.2	-122.2	0.50 (1)	4.22
C- D	-2611 / 0	-122.2	-122.2	0.69 (1)	3.52
D- E	-2611 / 0	-122.2	-122.2	0.68 (1)	3.52
E- F	-2611 / 0	-122.2	-122.2	0.68 (1)	3.52
F- G	-2611 / 0	-122.2	-122.2	0.68 (1)	3.52
G- H	-2611 / 0	-122.2	-122.2	0.69 (1)	3.52
H- I	-2037 / 0	-122.2	-122.2	0.50 (1)	4.22
I- J	0 / 60	-122.2	-122.2	0.17 (1)	10.00
R- B	-2379 / 0	0.0	0.0	0.18 (1)	6.68
K- I	-2379 / 0	0.0	0.0	0.18 (1)	6.68
R- Q	0 / 0	-28.0	-28.0	0.16 (2)	10.00
Q- P	0 / 1429	-28.0	-28.0	0.32 (1)	10.00
P- O	0 / 2958	-28.0	-28.0	0.55 (1)	10.00
O- N	0 / 2958	-28.0	-28.0	0.55 (1)	10.00
N- M	0 / 2958	-28.0	-28.0	0.55 (1)	10.00
M- L	0 / 1429	-28.0	-28.0	0.32 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.16 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-350 / 69	0.20 (1)
C- P	0 / 1725	0.39 (1)
P- D	-733 / 0	0.43 (1)
P- E	-514 / 0	0.65 (1)
O- E	0 / 261	0.06 (2)
E- M	-514 / 0	0.65 (1)
M- G	-733 / 0	0.43 (1)
M- H	0 / 1725	0.39 (1)
L- H	-350 / 69	0.20 (1)
B- Q	0 / 1521	0.34 (1)
L- I	0 / 1521	0.34 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.69 (C-D:1) , BC=0.55 (O-P:1) , WB=0.65 (E-P:1) , SSI=0.31 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.78 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4753017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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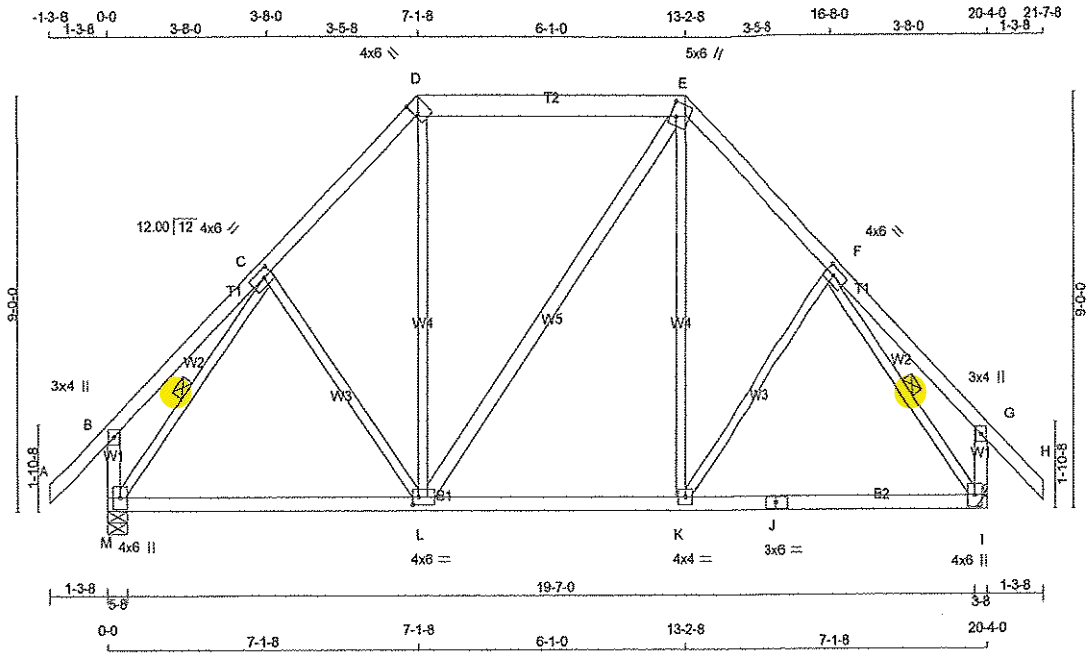

TOTAL WEIGHT = 109 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTW+h	MT20	4.0	6.0	Edge	0.75
E	TTWW+m	MT20	5.0	6.0	3.75	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	2.25
G	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	4.0	6.0	2.00	1.50
M	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1693	0	1693	0	0	5-8	5-8
I	1693	0	1693	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1299	884 / 0	203 / 0	0 / 0	0 / 0	212 / 0	0 / 0
I	1299	884 / 0	203 / 0	0 / 0	0 / 0	212 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
B- C	0 / 35	-122.2	-122.2	0.25 (1)	10.00	
C- D	-1203 / 0	-122.2	-122.2	0.21 (1)	5.62	
D- E	-831 / 0	-122.2	-122.2	0.29 (1)	6.25	
E- F	-1202 / 0	-122.2	-122.2	0.21 (1)	5.62	
F- G	0 / 35	-122.2	-122.2	0.25 (1)	10.00	
G- H	0 / 60	-122.2	-122.2	0.17 (1)	10.00	
M- B	-335 / 0	0.0	0.0	0.04 (1)	7.81	
I- G	-335 / 0	0.0	0.0	0.04 (1)	7.81	
M- L	0 / 890	-27.5	-27.5	0.37 (2)	10.00	
L- K	0 / 830	-27.5	-27.5	0.37 (2)	10.00	
K- J	0 / 889	-27.5	-27.5	0.38 (2)	10.00	
J- I	0 / 889	-27.5	-27.5	0.38 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-105 / 67	0.07 (1)
L- D	0 / 324	0.07 (2)
L- E	0 / 0	0.00 (1)
K- E	0 / 323	0.07 (2)
K- F	-105 / 67	0.07 (1)
M- C	-1554 / 0	0.41 (1)
F- I	-1554 / 0	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.29 (D-E:1) , BC=0.38 (I-K:2) , WB=0.41 (C-M:1) , SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.43 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9147
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4597417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T11S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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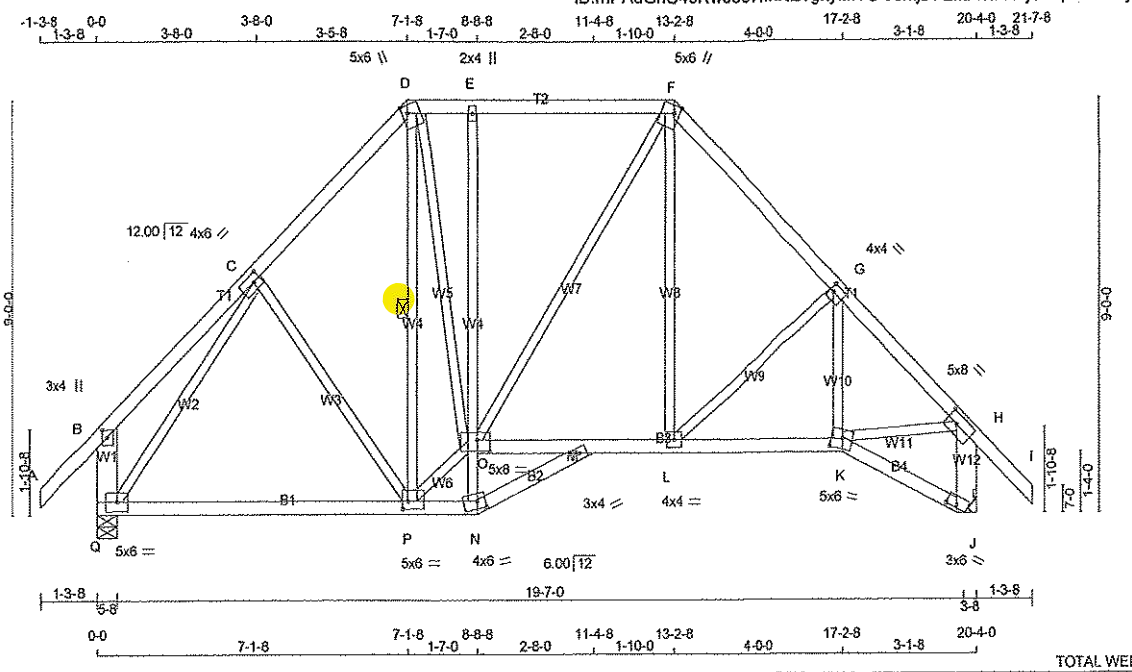

TOTAL WEIGHT = 2 X 120 = 239 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C	TMWW-t	MT20	4.0	6.0	2.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.00
H	TMWW-t	MT20	5.0	8.0	2.50	3.00
J	BVM1-l	MT20	3.0	6.0	0.75	3.00
K	BBWW-m	MT20	5.0	6.0	2.75	3.00
L	BMWW-t	MT20	4.0	4.0		
M	BMB-h	MT20	3.0	4.0		
N	BBW-m	MT20	4.0	6.0	2.00	2.25
O	BWMWWW*-l	MT20	5.0	8.0	3.00	3.50
P	BMWWW-l	MT20	5.0	6.0	2.00	1.50
Q	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1693	0	1693	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
Q	1693	0	1693	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1299	884 / 0	203 / 0	0 / 0	0 / 0	212 / 0	0 / 0
Q	1299	884 / 0	203 / 0	0 / 0	0 / 0	212 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 36	-122.2	-122.2	0.25 (1)	10.00
C- D	-1213 / 0	-122.2	-122.2	0.21 (1)	5.59
D- E	-1008 / 0	-122.2	-122.2	0.23 (1)	5.96
E- F	-1015 / 0	-122.2	-122.2	0.31 (1)	5.63
F- G	-1409 / 0	-122.2	-122.2	0.24 (1)	5.26
G- H	-1749 / 0	-122.2	-122.2	0.23 (1)	4.83
H- I	0 / 60	-122.2	-122.2	0.17 (1)	10.00
Q- B	-334 / 0	0.0	0.0	0.02 (1)	7.81
J- H	-1650 / 0	0.0	0.0	0.13 (1)	7.68
Q- P	0 / 894	-27.5	-27.5	0.43 (2)	10.00
P- N	-20 / 229	-27.5	-27.5	0.36 (2)	6.25
N- M	-32 / 235	-27.5	-27.5	0.06 (1)	6.25
O- M	0 / 771	0.0	0.0	0.31 (1)	10.00
M- L	0 / 976	-27.5	-27.5	0.35 (1)	10.00
L- K	0 / 1265	-27.5	-27.5	0.35 (1)	10.00
K- J	0 / 0	-27.5	-27.5	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	-112 / 65	0.08 (1)
P- D	-236 / 25	0.12 (1)
N- O	-164 / 0	0.02 (1)
O- E	-531 / 0	0.43 (1)
L- F	0 / 480	0.11 (1)
L- G	-369 / 0	0.18 (1)
K- G	0 / 124	0.03 (3)
Q- C	-1563 / 0	0.89 (1)
K- H	0 / 1267	0.29 (1)
O- F	0 / 73	0.02 (1)
P- O	0 / 825	0.19 (1)
D- O	0 / 949	0.21 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.31 (E-F:1) , BC=0.43 (P-Q:2) , WB=0.99 (C-Q:1) , SSI=0.25 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM 45975-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T12S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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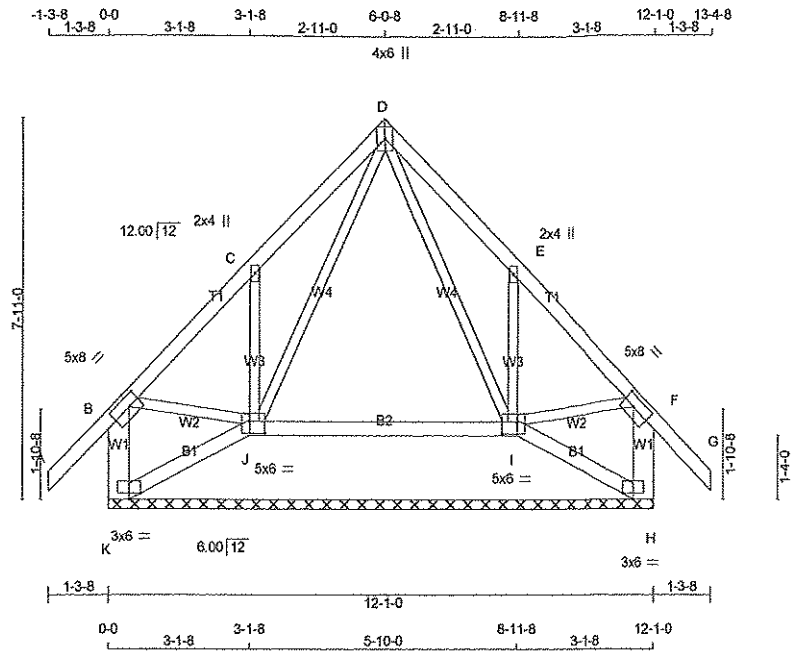

TOTAL WEIGHT = 65 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	8.0	2.50	3.00
H	BVM1-p	MT20	3.0	6.0	Edge	3.00
I	BBWWW1-p	MT20	5.0	6.0	3.00	4.00
J	BBWWW1-p	MT20	5.0	6.0	3.00	4.00
K	BVM1-p	MT20	3.0	6.0	Edge	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	392	0	392	0	0	12-1-0 (8-6-0)	2-1-0
J	683	0	683	0	0	12-1-0 (8-6-0)	2-1-0
I	683	0	683	0	0	12-1-0 (8-6-0)	2-1-0
H	392	0	392	0	0	12-1-0 (8-6-0)	2-1-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): J , I

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	289	219 / 0	31 / 0	0 / 0	0 / 0	39 / 0	0 / 0
J	530	350 / 0	90 / 0	0 / 0	0 / 0	90 / 0	0 / 0
I	530	350 / 0	90 / 0	0 / 0	0 / 0	90 / 0	0 / 0
H	289	219 / 0	31 / 0	0 / 0	0 / 0	39 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	-12 / 0	-122.2	-122.2	0.19 (1)	6.25
C- D	-76 / 0	-122.2	-122.2	0.19 (1)	6.25
D- E	-76 / 0	-122.2	-122.2	0.19 (1)	6.25
E- F	-12 / 0	-122.2	-122.2	0.19 (1)	6.25
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00
K- B	-350 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-350 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 0	-27.5	-27.5	0.08 (3)	10.00
J- I	0 / 78	-27.5	-27.5	0.28 (2)	10.00
I- H	0 / 0	-27.5	-27.5	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	-111 / 0	0.10 (1)
I- E	-461 / 0	0.10 (1)
J- D	-111 / 0	0.10 (1)
J- C	-461 / 0	0.10 (1)
B- J	0 / 30	0.01 (1)
I- F	0 / 30	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (D-E:1) , BC=0.28 (I-J:2) , WB=0.10 (C-J:1) , SSI=0.13 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (I) (INPUT = 0.90)
JSI METAL= 0.18 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45976 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T13	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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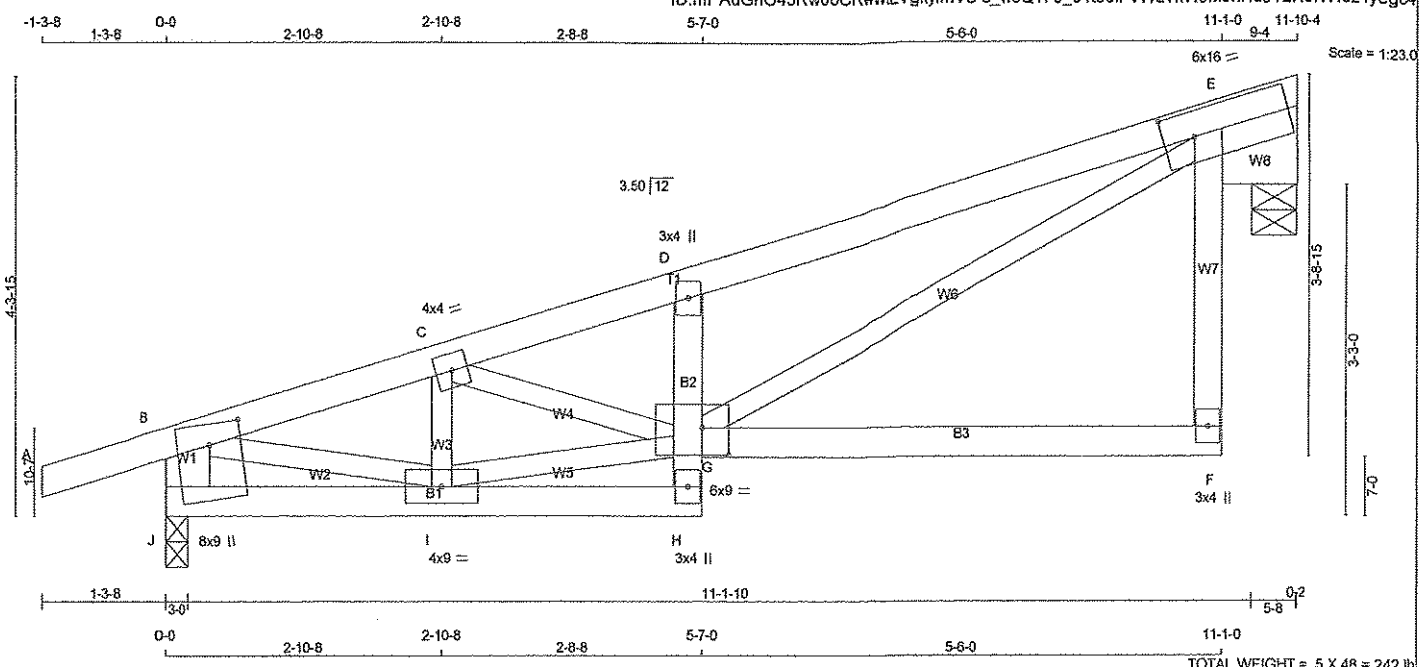

TOTAL WEIGHT = 5 X 48 = 242 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C	TMWW-t	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVWW1-t	MT20	6.0	16.0	3.00	4.00
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-t	MT20	6.0	9.0	3.25	3.25
H	BMV+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J						
J	TMBMVW1*+nMT20		8.0	9.0	2.50	4.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	993	0	993	0	0	3-0	3-0
E	830	0	830	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	755	526 / 0	111 / 0	0 / 0	0 / 0	119 / 0	0 / 0
E	646	424 / 0	111 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-122.2	-122.2	0.15 (1)	10.00	
B- C	-1304 / 0	-122.2	-122.2	0.17 (1)	5.49	
C- D	-1633 / 0	-122.2	-122.2	0.35 (1)	4.79	
D- E	-1731 / 0	-122.2	-122.2	0.52 (1)	4.44	
F- E	0 / 134	0.0	0.0	0.02 (3)	10.00	
J- B	-951 / 0	0.0	0.0	0.05 (1)	7.81	
J- I	0 / 0	-27.5	-27.5	0.05 (2)	10.00	
I- H	0 / 175	-27.5	-27.5	0.06 (1)	10.00	
H- G	0 / 52	0.0	0.0	0.14 (1)	10.00	
G- D	-689 / 0	0.0	0.0	0.11 (1)	7.81	
G- F	0 / 0	-27.5	-27.5	0.26 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 1896	0.43 (1)
I- C	-425 / 0	0.06 (1)
B- I	0 / 1277	0.29 (1)
I- G	0 / 1106	0.25 (1)
C- G	0 / 365	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 909 (0.15")

CSI: TC=0.52 (D-E:1) , BC=0.26 (F-G:3) , WB=0.43 (E-G:1) , SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (G) (INPUT = 0.90)
JSI METAL= 0.29 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/14/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM459771 7
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42087	DRWG NO.
273415	G13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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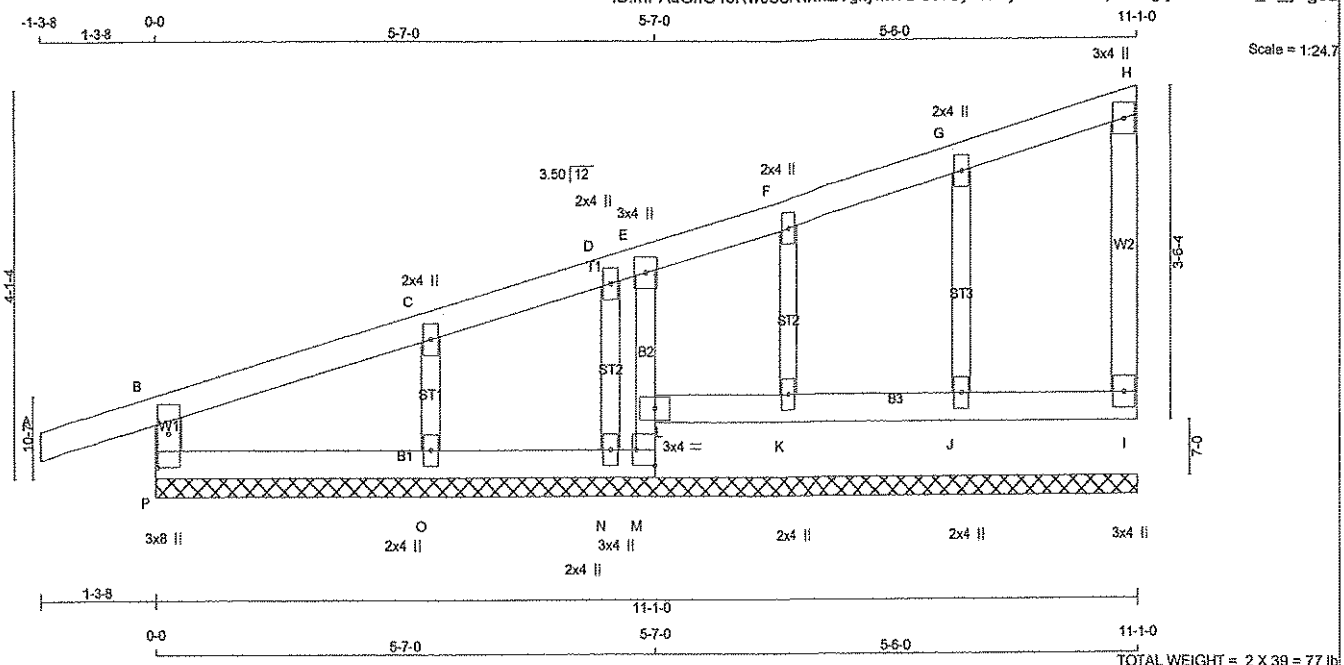

TOTAL WEIGHT = 2 X 39 = 77 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - E	2x3	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TMV+p	MT20	3.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, N, O						
J	BMW1+w	MT20	2.0	4.0		
L	BVM-I	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0	2.00	Edge
P						
P	TMBMV1+p	MT20	3.0	8.0	4.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): I , J , K
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-367 / 0	0.0	0.0	0.04 (2)	7.81	
A- B	0 / 23	-122.2	-122.2	0.15 (1)	10.00	
B- C	-30 / 0	-122.2	-122.2	0.14 (1)	6.25	
C- D	-25 / 0	-122.2	-122.2	0.10 (1)	6.25	
D- E	-18 / 0	-122.2	-122.2	0.03 (1)	6.25	
E- F	-16 / 0	-122.2	-122.2	0.05 (1)	6.25	
F- G	-12 / 0	-122.2	-122.2	0.06 (1)	6.25	
G- H	-13 / 0	-122.2	-122.2	0.06 (1)	6.25	
I- H	-109 / 0	0.0	0.0	0.03 (1)	7.81	
P- O	0 / 26	-27.5	-27.5	0.05 (2)	10.00	
O- N	0 / 17	-27.5	-27.5	0.05 (3)	10.00	
N- M	0 / 14	-27.5	-27.5	0.01 (1)	10.00	
M- L	-120 / 0	0.0	0.0	0.01 (1)	7.81	
L- E	-103 / 0	0.0	0.0	0.01 (1)	7.81	
L- K	0 / 17	-27.5	-27.5	0.02 (2)	10.00	
K- J	0 / 13	-27.5	-27.5	0.03 (3)	10.00	
J- I	0 / 9	-27.5	-27.5	0.03 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- G	-266 / 0	0.05 (1)
K- F	-221 / 0	0.03 (1)
N- D	-131 / 0	0.02 (1)
O- C	-321 / 0	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15 (A-B:1) , BC=0.05 (O-P:2) , WB=0.05 (G-J:1) , SSI=0.15 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (C) (INPUT = 0.90)
JSI METAL= 0.05 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45979-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	T14	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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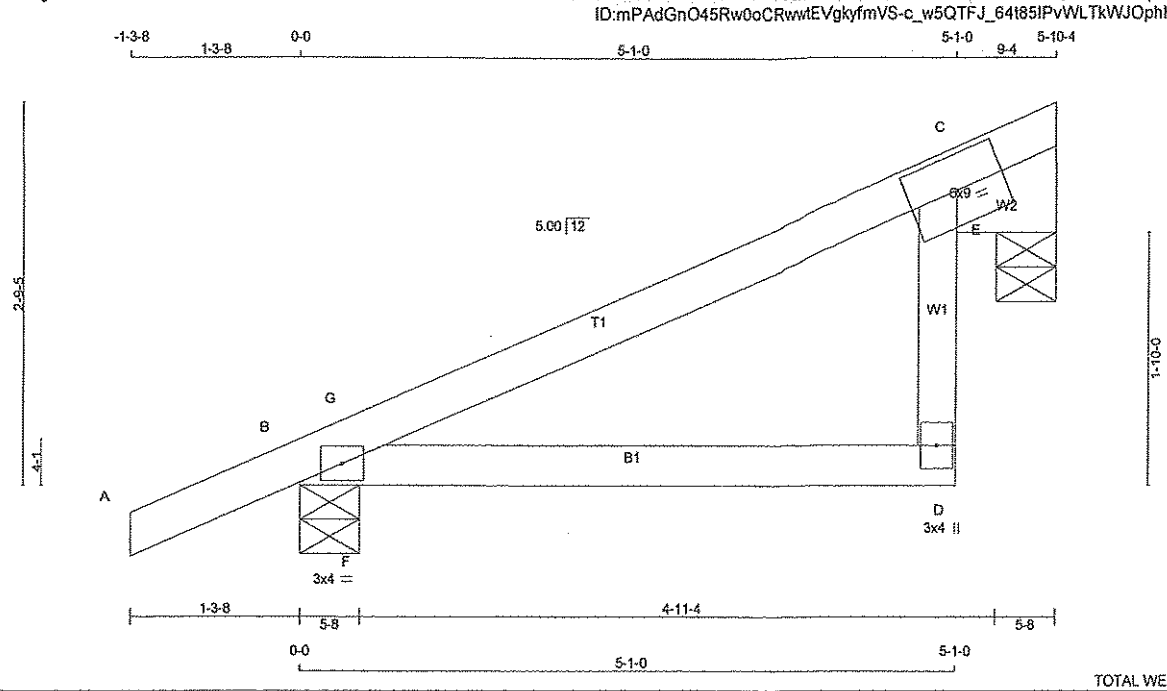

TOTAL WEIGHT = 10 X 19 = 188 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMVW1-I	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H(C)	344	0	344	0	0	5-8	5-8
B	582	0	582	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H(C)	268	176 / 0	46 / 0	0 / 0	0 / 0	46 / 0	0 / 0
B	436	316 / 0	56 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C), B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 30	-122.2	-122.2	0.16 (1)	10.00
B- G	-102 / 0	-122.2	-122.2	0.10 (3)	6.25
G- C	-97 / 0	-122.2	-122.2	0.41 (1)	6.25
D- E	0 / 121	0.0	0.0	0.22 (1)	10.00
E- C	0 / 121	0.0	0.0	0.22 (1)	10.00
B- F	0 / 86	-27.5	-27.5	0.29 (1)	10.00
F- D	0 / 86	-27.5	-27.5	0.30 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	-397 / 151	0.00 (1)
C- H	-528 / 0	0.02 (1)
E- H	0 / 401	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 825 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 518 (0.12")

CSI: TC=0.41 (C-G:1) , BC=0.30 (D-F:1) , WB=0.02 (C-H:1) , SSI=0.33 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45978-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T15	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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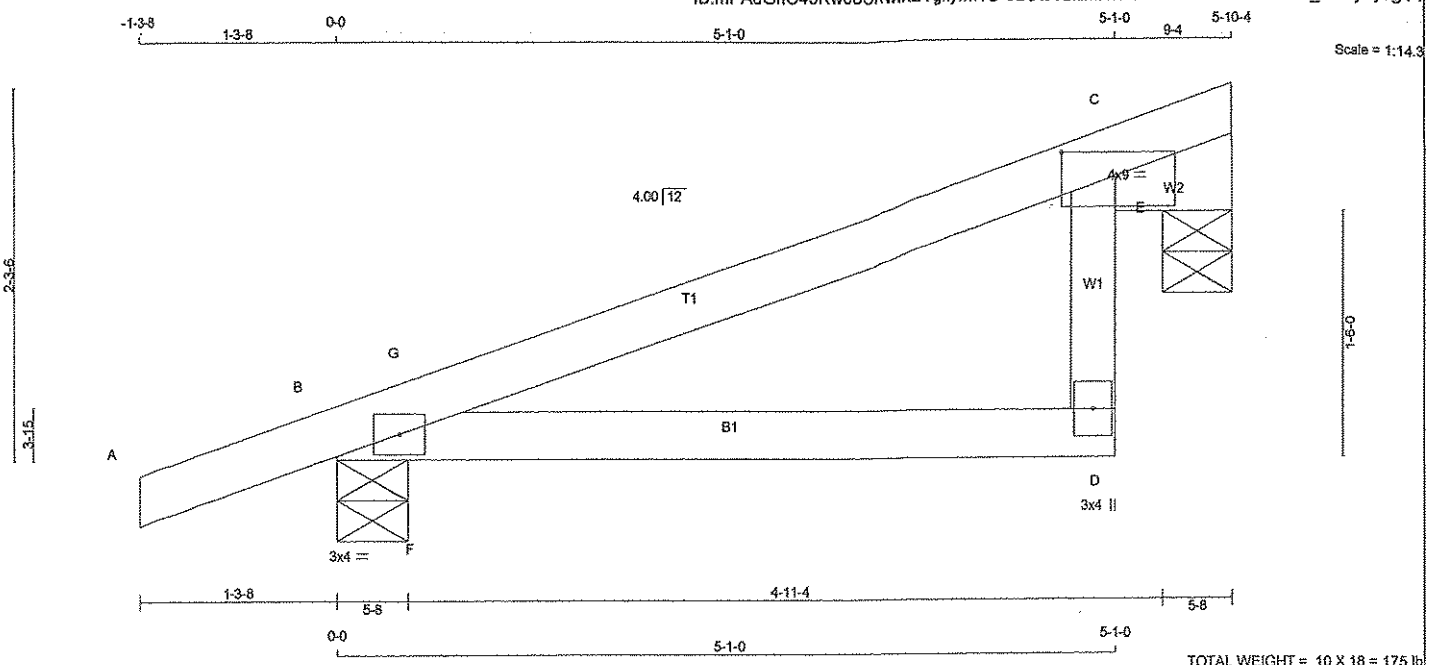

TOTAL WEIGHT = 10 X 18 = 175 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMVW1-p	MT20	4.0	9.0	1.75	4.25
D	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H(C)	344	0	344	0	0	5-8	5-8
B	581	0	581	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H(C)	268	176 / 0	46 / 0	0 / 0	0 / 0	46 / 0	0 / 0
B	435	315 / 0	56 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C), B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 24	-122.2	-122.2	0.15 (1)	10.00
B- G	-110 / 0	-122.2	-122.2	0.11 (3)	6.25
G- C	-115 / 0	-122.2	-122.2	0.41 (1)	6.25
D- E	0 / 121	0.0	0.0	0.22 (1)	10.00
E- C	0 / 121	0.0	0.0	0.22 (1)	10.00
B- F	0 / 108	-27.5	-27.5	0.31 (1)	10.00
F- D	0 / 108	-27.5	-27.5	0.31 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	-325 / 129	0.00 (1)
C- H	-611 / 0	0.02 (1)
E- H	0 / 505	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 817 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 516 (0.12")

CSI: TC=0.41 (C-G:1) , BC=0.31 (D-F:1) , WB=0.02 (C-H:1) , SSI=0.28 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (C) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9111
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45981-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288225	T16	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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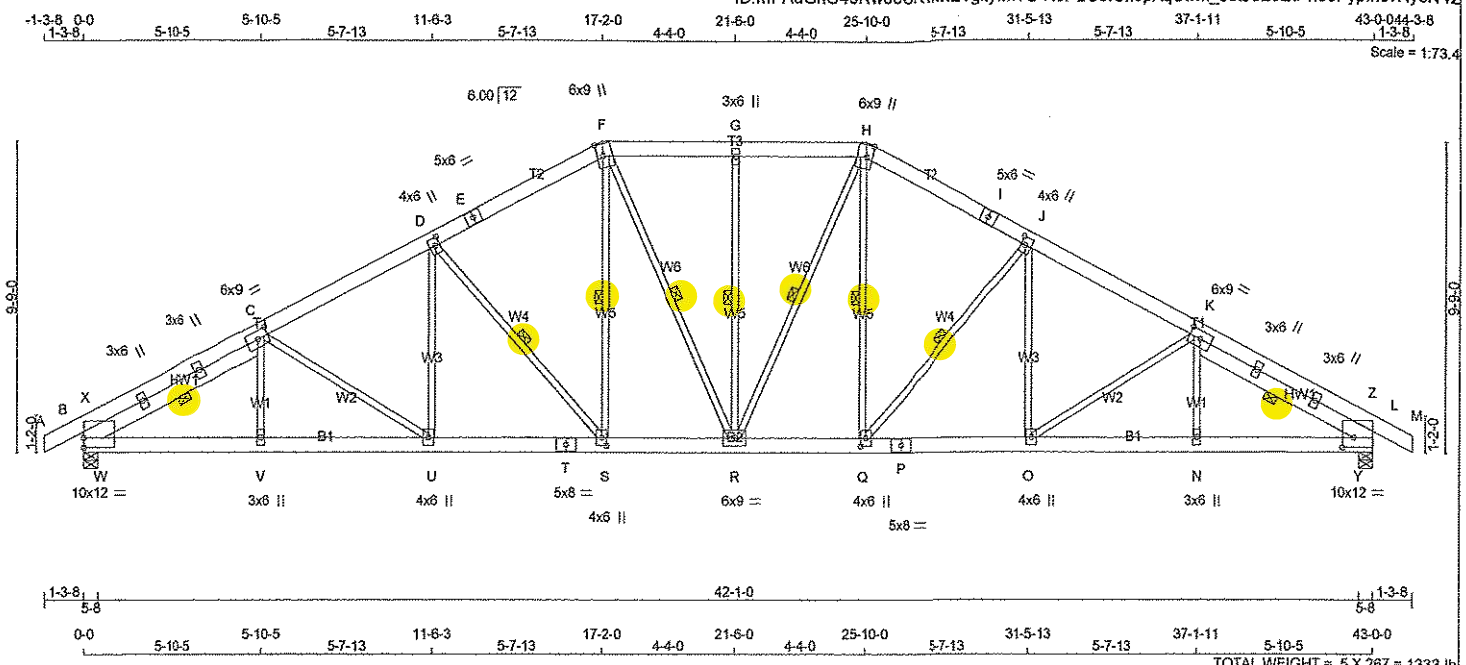

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
I - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
P - L	2x6	DRY	1650F 1.5E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
B, B, L, L						
B	TPRW+t	MT20	3.0	6.0		
C	TMWWW-t	MT20	6.0	9.0	3.00	4.50
D	TMWW+t	MT20	4.0	6.0	3.00	1.75
E	TS-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.25
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.25
I	TS-t	MT20	5.0	6.0		
J	TMWW+t	MT20	4.0	6.0	3.00	1.75
K	TMWWW-t	MT20	6.0	9.0	3.00	4.50
L	TMBMW1-I	MT20	10.0	12.0	3.75	4.50
N	BMW+w	MT20	3.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	8.0		
Q	BMWW+t	MT20	4.0	6.0	3.00	1.75
R	BMWWW-t	MT20	6.0	9.0		
S	BMWW+t	MT20	4.0	6.0	3.00	1.75
T	BS-t	MT20	5.0	8.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4437	0	4539	260	-1042	5-8	5-8
L	4437	0	4539	0	-1042	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1042 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1042 LBS FACTORED UPLIFT

SIMPSON 2H2.5A 2H2.5A

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3333	2441 / 0	430 / 0	0 / 0	156 / -1085	530 / 0	0 / 0
L	3333	2441 / 0	430 / 0	0 / 0	92 / -1085	530 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	186 / -186	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.98 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-S, F-S, F-R, G-R, H-R, H-Q, J-Q, C-W, K-Y.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-168.0	-168.0	0.13 (2)	10.00	
B- X	-5060 / 1056	-168.0	-168.0	0.49 (2)	3.42	
X- C	-3126 / 865	-168.0	-168.0	0.43 (2)	4.33	
C- D	-6227 / 1540	-168.0	-168.0	0.68 (2)	2.98	
D- E	-5261 / 1386	-168.0	-168.0	0.60 (2)	3.35	
E- F	-5261 / 1386	-168.0	-168.0	0.60 (2)	3.35	
F- G	-4897 / 1334	-168.0	-168.0	0.32 (2)	3.71	
G- H	-4897 / 1334	-168.0	-168.0	0.32 (2)	3.71	
H- I	-5261 / 1386	-168.0	-168.0	0.60 (3)	3.35	
I- J	-5261 / 1386	-168.0	-168.0	0.60 (3)	3.35	
J- K	-6227 / 1540	-168.0	-168.0	0.68 (3)	2.98	
K- Z	-3126 / 865	-168.0	-168.0	0.43 (3)	4.33	
Z- L	-5060 / 1056	-168.0	-168.0	0.49 (3)	3.42	
L- M	0 / 2	-168.0	-168.0	0.13 (3)	10.00	
B- W	-866 / 2781	-27.5	-27.5	0.22 (1)	6.25	
W- V	-1498 / 5949	-27.5	-27.5	0.49 (1)	6.25	
V- U	-1499 / 5947	-27.5	-27.5	0.50 (1)	6.25	
U- T	-1276 / 5589	-27.5	-27.5	0.45 (1)	6.25	
T- S	-1276 / 5589	-27.5	-27.5	0.45 (1)	6.25	
S- R	-911 / 4677	-27.5	-27.5	0.37 (1)	6.25	
R- Q	-797 / 4677	-27.5	-27.5	0.37 (1)	6.25	
Q- P	-1016 / 5589	-27.5	-27.5	0.45 (1)	6.25	
P- O	-1016 / 5589	-27.5	-27.5	0.45 (1)	6.25	
O- N	-1240 / 5947	-27.5	-27.5	0.50 (1)	6.25	
N- Y	-1239 / 5949	-27.5	-27.5	0.49 (1)	6.25	
Y- L	-866 / 2781	-27.5	-27.5	0.22 (1)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	0 / 254	0.06 (17)
C- U	-638 / 265	0.51 (2)
U- D	-70 / 516	0.12 (2)
D- S	-1685 / 553	0.83 (2)
S- F	-350 / 1402	0.32 (2)
F- R	-285 / 1309	0.29 (3)
R- G	-1028 / 320	0.59 (2)
R- H	-285 / 1309	0.29 (2)
Q- H	-351 / 1402	0.32 (3)
Q- J	-1685 / 553	0.83 (3)
O- J	-70 / 516	0.12 (3)
O- K	-638 / 267	0.51 (3)
N- K	0 / 254	0.06 (17)
W- X	-281 / 2348	0.00 (1)
W- C	-3892 / 762	0.57 (1)
K- Y	-3892 / 762	0.57 (1)
Y- Z	-281 / 2348	0.00 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.8	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	73.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(80 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.68 (J-K:3) , BC=0.50 (U-V:1) , WB=0.83 (J-Q:3) , SSI=0.37 (H-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JT GRIP= 0.90 (S) (INPUT = 0.90)
JT METAL= 0.85 (T) (INPUT = 1.00)

P6 1/2

DWG NO. TAM47531-17

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER 92077 S. KATSOULAKOS PROVINCE OF ONTARIO

CONTINUED ON PAGE 2

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {9.6} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
9-2017
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 47531-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T16A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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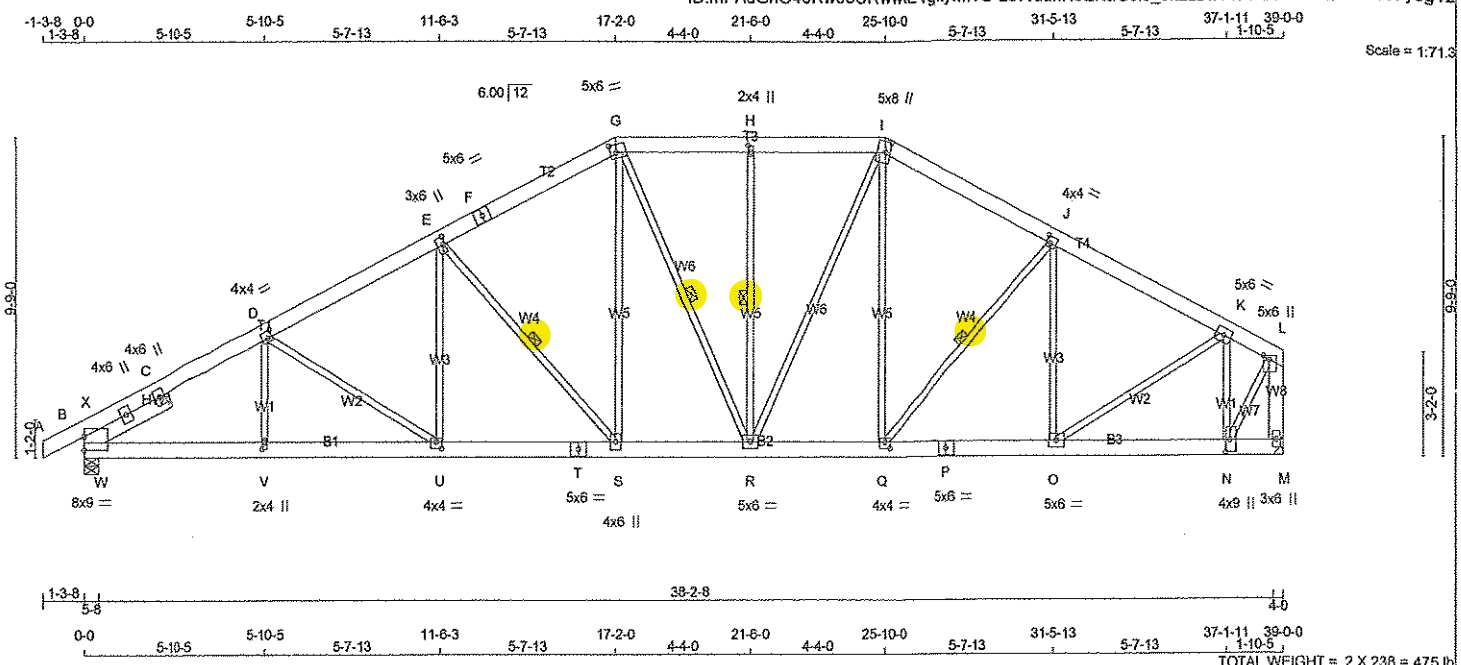

TOTAL WEIGHT = 2 X 238 = 475 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
B - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	8.0	9.0	5.00	
B	RT+t	MT20	4.0	6.0		
B	RT+t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.50	1.75
E	TMWW+t	MT20	3.0	6.0	3.00	0.75
F	TS-t	MT20	5.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.75	2.25
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	5.0	8.0	4.00	1.75
J	TMWW-t	MT20	4.0	4.0	2.50	1.75
K	TMWW-t	MT20	5.0	6.0		
L	TMVW+p	MT20	5.0	6.0	1.50	2.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW+t	MT20	4.0	9.0	4.50	1.50
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	4.0	2.50	2.00
R	BMWWW-t	MT20	5.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	4.0	4.0	2.50	2.00
V	BMW+w	MT20	2.0	4.0	2.50	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	3100	0	3100	0	0	5-8	5-8
M	2929	0	2929	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2406	1598 / 0	410 / 0	0 / 0	0 / 0	398 / 0	0 / 0
M	2291	1491 / 0	410 / 0	0 / 0	0 / 0	390 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, G-R, H-R, J-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 0	-122.2	-122.2	0.08 (1)	10.00
B- X	-4217 / 0	-122.2	-122.2	0.13 (1)	4.16
X- C	-3193 / 0	-122.2	-122.2	0.50 (1)	4.19
C- D	-3193 / 0	-122.2	-122.2	0.50 (1)	4.19
D- E	-4173 / 0	-122.2	-122.2	0.35 (1)	3.95
E- F	-3426 / 0	-122.2	-122.2	0.31 (1)	4.33
F- G	-3426 / 0	-122.2	-122.2	0.31 (1)	4.33
G- H	-3067 / 0	-122.2	-122.2	0.15 (1)	4.71
H- I	-3067 / 0	-122.2	-122.2	0.15 (1)	4.71
I- J	-3119 / 0	-122.2	-122.2	0.26 (1)	4.57
J- K	-3142 / 0	-122.2	-122.2	0.26 (1)	4.55
K- L	-1633 / 0	-122.2	-122.2	0.10 (1)	6.05
M- L	-2928 / 0	0.0	0.0	0.28 (1)	6.14
B- W	0 / 1872	-28.0	-28.0	0.50 (1)	10.00
W- V	0 / 3942	-28.0	-28.0	0.75 (1)	10.00
V- U	0 / 3942	-28.0	-28.0	0.59 (1)	10.00
U- T	0 / 3779	-28.0	-28.0	0.50 (1)	10.00
T- S	0 / 3779	-28.0	-28.0	0.50 (1)	10.00
S- R	0 / 3039	-28.0	-28.0	0.38 (1)	10.00
R- Q	0 / 2762	-28.0	-28.0	0.37 (1)	10.00
Q- P	0 / 2827	-28.0	-28.0	0.38 (1)	10.00
P- O	0 / 2827	-28.0	-28.0	0.38 (1)	10.00
O- N	0 / 1496	-28.0	-28.0	0.22 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.05 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- D	0 / 206	0.05 (3)
D- U	-193 / 0	0.15 (1)
U- E	0 / 387	0.09 (2)
E- S	-1127 / 0	0.56 (1)
S- G	0 / 974	0.22 (1)
G- R	-2 / 66	0.01 (1)
R- H	-606 / 0	0.35 (1)
R- I	0 / 721	0.16 (1)
Q- I	0 / 290	0.07 (2)
Q- J	-102 / 0	0.05 (1)
O- J	-698 / 0	0.50 (1)
O- K	0 / 1579	0.36 (1)
N- K	-2024 / 0	0.46 (1)
N- L	0 / 2624	0.59 (1)
W- X	0 / 3113	0.60 (1)
W- C	-2903 / 0	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.50 (D-X:1) , BC=0.75 (V-W:1) , WB=0.59 (L-N:1) , SSI=0.54 (B-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.83 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45983-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T16SB	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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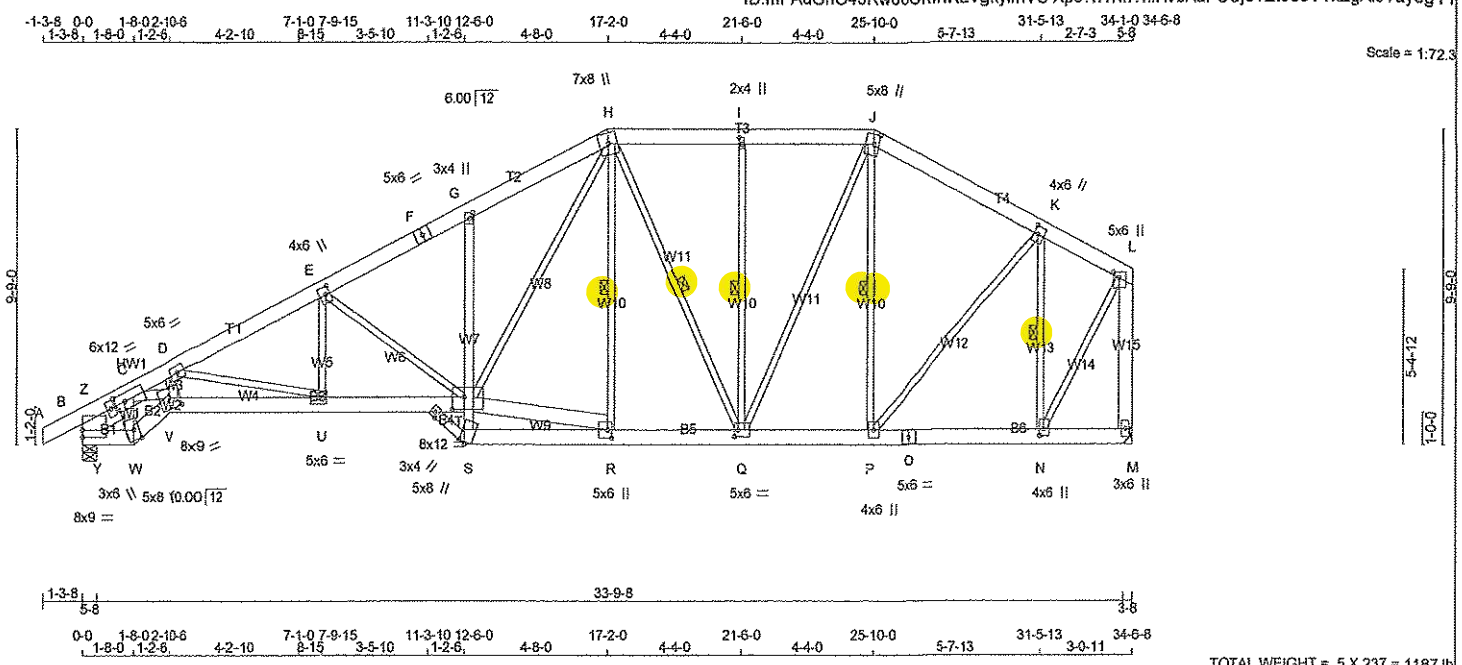

TOTAL WEIGHT = 5 X 237 = 1187 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
B - W	2x6	DRY	No.2	SPF
W - V	2x6	DRY	1650F 1.5E	SPF
V - T	2x6	DRY	1650F 1.5E	SPF
S - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
W - C	2x4	DRY	No.2	SPF
C - V	2x4	DRY	1650F 1.5E	SPF
V - D	2x4	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, B, S, S						
B						
B	TPRW+t	MT20	3.0	6.0		
B	TMBMW1-I	MT20	8.0	9.0	2.75	
C	TMWWW-t	MT20	6.0	12.0	3.00	3.75
D	TMWW-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0	3.00	1.75
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	4.0	2.00	1.25
H	TTWWW+m	MT20	7.0	8.0		
I	TMW+w	MT20	2.0	4.0	2.50	1.00
J	TTWW+m	MT20	5.0	8.0	3.75	1.75
K	TMWW+t	MT20	4.0	6.0	3.00	1.50
L	TMVW+p	MT20	5.0	6.0	2.25	2.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW+t	MT20	4.0	6.0	2.50	1.50
O	BS-I	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0	2.50	1.50
R	BMWW+t	MT20	5.0	6.0	3.00	1.50
S	BBW+m	MT20	5.0	8.0	Edge	
T	BWMWWW"-I	MT20	8.0	12.0	4.50	4.75
U	BMWW-t	MT20	5.0	6.0		
V	BBWW-m	MT20	8.0	9.0	4.25	4.50
W	BBW+m	MT20	5.0	8.0	Edge	2.00
X	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2768	0	2768	0	0	5-8	5-8
M	2591	0	2591	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2146	1430 / 0	362 / 0	0 / 0	0 / 0	354 / 0	0 / 0
M	2026	1319 / 0	362 / 0	0 / 0	0 / 0	345 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-R, H-Q, I-Q, J-P, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 0	-122.2	-122.2	0.08 (1)	10.00
B- Z	-2396 / 0	-122.2	-122.2	0.12 (1)	5.23
Z- C	-1432 / 0	-122.2	-122.2	0.14 (1)	6.25
C- D	-6698 / 0	-122.2	-122.2	0.37 (1)	3.17
D- E	-4897 / 0	-122.2	-122.2	0.35 (1)	3.69
E- F	-3909 / 0	-122.2	-122.2	0.24 (1)	4.19
F- G	-3909 / 0	-122.2	-122.2	0.24 (1)	4.19
G- H	-3894 / 0	-122.2	-122.2	0.24 (1)	4.19
H- I	-2278 / 0	-122.2	-122.2	0.13 (1)	5.31
I- J	-2278 / 0	-122.2	-122.2	0.13 (1)	5.31
J- K	-2078 / 0	-122.2	-122.2	0.24 (1)	5.37
K- L	-1267 / 0	-122.2	-122.2	0.17 (1)	6.25
M- L	-2559 / 0	0.0	0.0	0.77 (1)	6.48
B- Y	0 / 1143	-28.0	-28.0	0.28 (1)	10.00
Y- W	0 / 2570	-28.0	-28.0	0.45 (1)	10.00
W- V	0 / 3391	-28.0	-28.0	0.25 (1)	10.00
V- U	0 / 5895	-28.0	-28.0	0.48 (1)	10.00
U- T	0 / 4423	-28.0	-28.0	0.37 (1)	10.00
S- R	0 / 64	-28.0	-28.0	0.10 (2)	10.00
R- Q	0 / 2423	-28.0	-28.0	0.37 (1)	10.00
Q- P	0 / 1829	-28.0	-28.0	0.28 (1)	10.00
P- O	0 / 1174	-28.0	-28.0	0.20 (1)	10.00
O- N	0 / 1174	-28.0	-28.0	0.20 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.07 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-1882 / 0	0.19 (1)
C- V	0 / 3693	0.43 (1)
V- D	0 / 862	0.14 (1)
S- T	0 / 89	0.09 (1)
T- G	-571 / 0	0.16 (1)
R- H	-310 / 69	0.18 (1)
H- Q	-342 / 0	0.25 (1)
Q- I	-614 / 0	0.36 (1)
Q- J	0 / 1061	0.24 (1)
P- J	-553 / 0	0.32 (1)
P- K	0 / 991	0.22 (1)
N- K	-1763 / 0	0.49 (1)
N- L	0 / 2223	0.50 (1)
D- U	-1517 / 0	0.61 (1)
U- E	0 / 466	0.10 (2)
E- T	-1175 / 0	0.70 (1)
T- B	0 / 2485	0.28 (1)
T- H	0 / 3157	0.49 (1)
H- Z	0 / 1087	0.60 (1)
Z- C	-2274 / 0	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.15")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.15")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.77 (L-M:1) , BC=0.48 (U-V:1) , WB=0.70 (E-T:1) , SSI=0.28 (B-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.69 (V) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 459B417

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T16B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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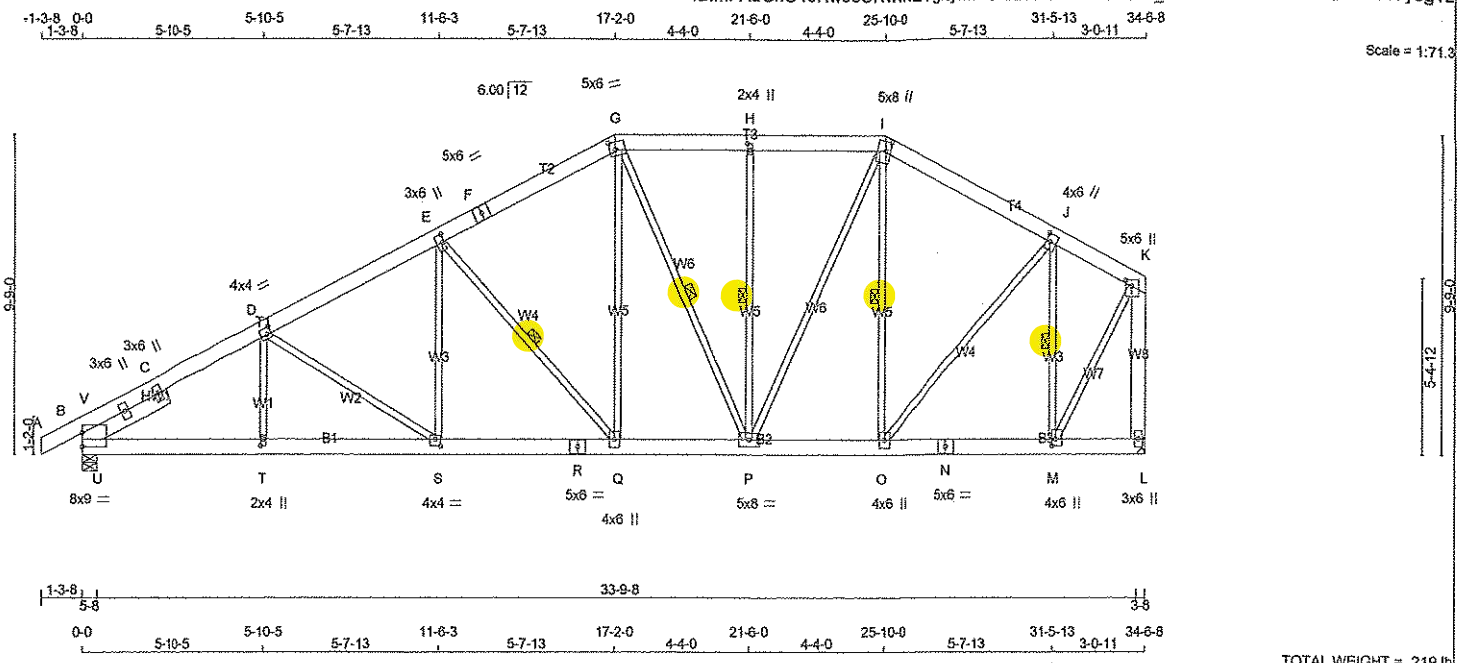

TOTAL WEIGHT = 219 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
B - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	8.0	9.0	5.00	
B	RT+t	MT20	3.0	6.0		
B	RT+t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.50	1.75
E	TMWW+t	MT20	3.0	6.0	3.00	0.75
F	TS-t	MT20	5.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.75	2.25
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	5.0	8.0	3.50	1.75
J	TMWW+t	MT20	4.0	6.0	3.00	1.50
K	TMVW+p	MT20	5.0	6.0	2.25	2.25
L	BMV1+p	MT20	3.0	6.0		
M	BMWW+t	MT20	4.0	6.0	2.50	1.50
N	BS-t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	4.0	4.0	2.50	2.00
T	BMW+w	MT20	2.0	4.0	2.50	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2765	0	2765	0	0	5-8	5-8
L	2594	0	2594	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2144	1428 / 0	363 / 0	0 / 0	0 / 0	354 / 0	0 / 0
L	2029	1321 / 0	363 / 0	0 / 0	0 / 0	345 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-P, H-P, I-O, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 0	-122.2	-122.2	0.08 (1)	10.00	
B- V	-3674 / 0	-122.2	-122.2	0.09 (1)	4.44	
V- C	-2783 / 0	-122.2	-122.2	0.40 (1)	4.58	
C- D	-2783 / 0	-122.2	-122.2	0.40 (1)	4.58	
D- E	-3507 / 0	-122.2	-122.2	0.31 (1)	4.30	
E- F	-2731 / 0	-122.2	-122.2	0.28 (1)	4.79	
F- G	-2731 / 0	-122.2	-122.2	0.28 (1)	4.79	
G- H	-2290 / 0	-122.2	-122.2	0.13 (1)	5.30	
H- I	-2290 / 0	-122.2	-122.2	0.13 (1)	5.30	
I- J	-2081 / 0	-122.2	-122.2	0.24 (1)	5.37	
J- K	-1270 / 0	-122.2	-122.2	0.17 (1)	6.25	
L- K	-2565 / 0	0.0	0.0	0.77 (1)	6.48	
B- U	0 / 1642	-28.0	-28.0	0.43 (1)	10.00	
U- T	0 / 3429	-28.0	-28.0	0.64 (1)	10.00	
T- S	0 / 3429	-28.0	-28.0	0.51 (1)	10.00	
S- R	0 / 3178	-28.0	-28.0	0.43 (1)	10.00	
R- Q	0 / 3178	-28.0	-28.0	0.43 (1)	10.00	
Q- P	0 / 2419	-28.0	-28.0	0.31 (1)	10.00	
P- O	0 / 1831	-28.0	-28.0	0.27 (1)	10.00	
O- N	0 / 1177	-28.0	-28.0	0.19 (1)	10.00	
N- M	0 / 1177	-28.0	-28.0	0.19 (1)	10.00	
M- L	0 / 0	-28.0	-28.0	0.07 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- D	0 / 217	0.05 (3)
D- S	-298 / 0	0.24 (1)
S- E	0 / 417	0.09 (2)
E- Q	-1155 / 0	0.57 (1)
Q- G	0 / 1009	0.22 (1)
G- P	-305 / 0	0.22 (1)
P- H	-613 / 0	0.36 (1)
P- I	0 / 1085	0.24 (1)
O- I	-574 / 0	0.34 (1)
O- J	0 / 991	0.22 (1)
M- J	-1762 / 0	0.49 (1)
M- K	0 / 2228	0.50 (1)
U- V	0 / 2656	0.00 (1)
U- C	-2506 / 0	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.15")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.15")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.77 (K-L:1) , BC=0.64 (T-U:1) , WB=0.57 (E-Q:1) , SSI=0.44 (B-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.97 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 459805 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T16S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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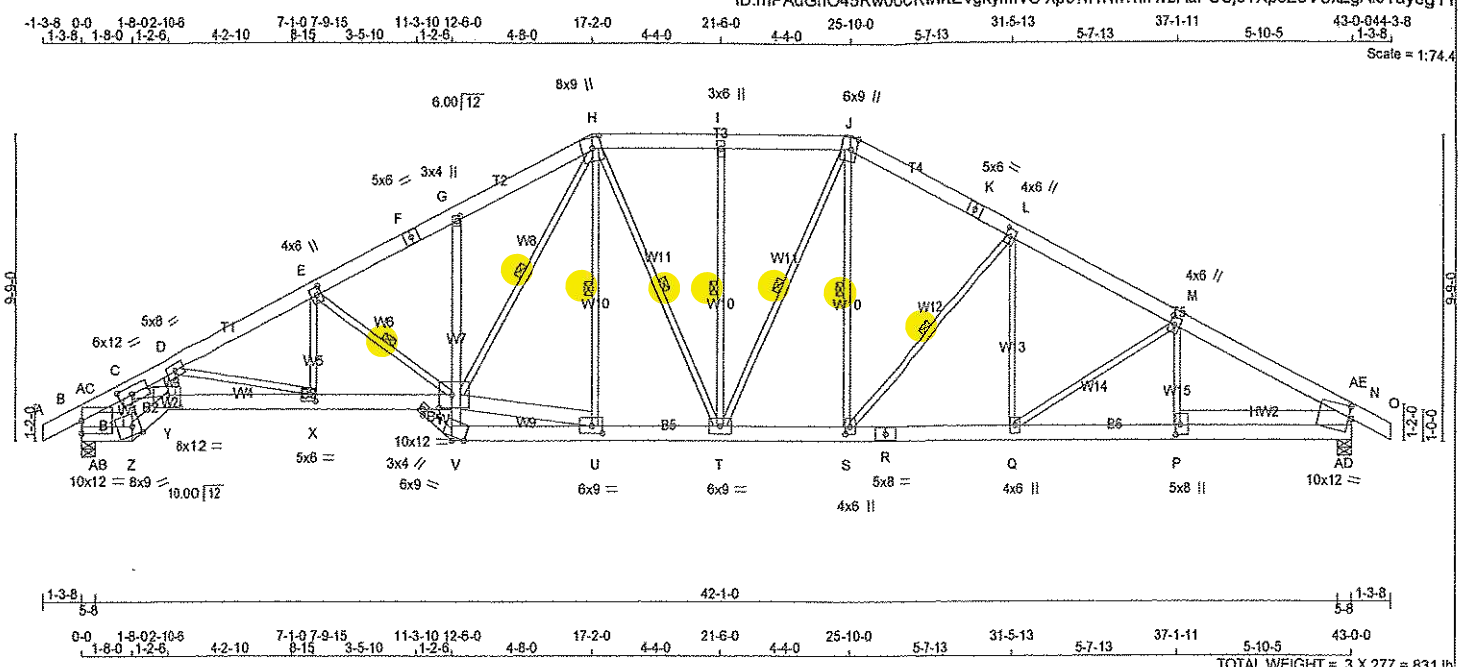

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - K	2x6	DRY	No.2	SPF
K - O	2x6	DRY	No.2	SPF
B - Z	2x6	DRY	No.2	SPF
Z - Y	2x6	DRY	1650F 1.5E	SPF
Y - W	2x6	DRY	1650F 1.5E	SPF
V - R	2x6	DRY	1650F 1.5E	SPF
R - N	2x6	DRY	1650F 1.5E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
Z - C	2x4	DRY	No.2	SPF
C - Y	2x4	DRY	1650F 1.5E	SPF
Y - D	2x4	DRY	No.2	SPF
V - G	2x4	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	5.00	0.25
C	TMWW-t	MT20	6.0	12.0	3.00	5.25
D	TMWW-t	MT20	5.0	8.0		
E, L, M						
E	TMWW+t	MT20	4.0	6.0	3.00	1.75
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	4.0	2.00	1.25
H	TTWWW+m	MT20	8.0	9.0	4.00	3.75
I	TMW+w	MT20	3.0	6.0		
J	TTWW+m	MT20	6.0	9.0	Edge	2.25
K	TS-t	MT20	5.0	6.0		
N	TMBMW1-m	MT20	10.0	12.0	4.50	1.25
P	BMWW+t	MT20	5.0	8.0	3.75	1.75
Q	BMWW+t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	6.0	2.75	1.75
T	BMWWW-t	MT20	6.0	9.0		
U	BMWW-t	MT20	6.0	9.0	2.75	4.25
V						
V	BBW-m	MT20	6.0	9.0	Edge	
W	BWMWWW*-I	MT20	10.0	12.0	5.00	5.25
X	BMWW-t	MT20	5.0	6.0	2.50	2.25
Y	BBWW-p	MT20	8.0	12.0	4.75	6.00
Z	BBWW-m	MT20	8.0	9.0	3.25	3.25
AA	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4451	0	4451	204	-713	5-8	5-8
N	4444	0	4444	0	-711	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 713 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 711 LBS FACTORED UPLIFT

SIMPSON H-2.5A H2.5A

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 204 LBS FACTORED HORIZONTAL REACTION AT JOINT B

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3356	2375 / 0	451 / 0	0 / 0	11 / -850	530 / 0	0 / 0
N	3352	2371 / 0	451 / 0	0 / 0	73 / -849	530 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	145 / -145	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B.

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 1.76 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.76 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-U, H-T, I-T, J-T, J-S, L-S, E-W, H-W.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-168.0	-168.0	0.13 (2)	10.00
B-AC	-4138 / 631	-168.0	-168.0	0.26 (1)	4.04
AC- C	-4911 / 792	-168.0	-168.0	0.35 (1)	3.65
C- D	-11101 / 1932	-168.0	-168.0	0.90 (1)	1.76
D- E	-8381 / 1371	-168.0	-168.0	0.81 (1)	2.35
E- F	-7051 / 1203	-168.0	-168.0	0.59 (1)	2.87
F- G	-7051 / 1203	-168.0	-168.0	0.59 (1)	2.87
G- H	-7021 / 1351	-168.0	-168.0	0.54 (1)	2.94
H- I	-4890 / 925	-168.0	-168.0	0.30 (1)	3.71
I- J	-4890 / 925	-168.0	-168.0	0.30 (1)	3.71
J- K	-5269 / 959	-168.0	-168.0	0.58 (3)	3.35
K- L	-5269 / 959	-168.0	-168.0	0.58 (3)	3.35
L- M	-6236 / 1055	-168.0	-168.0	0.67 (3)	3.02
M-AE	-6679 / 1069	-168.0	-168.0	0.61 (3)	2.97
AE- N	-4943 / 701	-168.0	-168.0	0.45 (3)	3.54
N- O	0 / 2	-168.0	-168.0	0.13 (3)	10.00
B-AB	-446 / 1787	-28.0	-28.0	0.33 (1)	6.25
AB- Z	-447 / 1792	-28.0	-28.0	0.33 (1)	6.25
Z- Y	-940 / 4833	-28.0	-28.0	0.36 (1)	6.25
Y- X	-1803 / 9741	-28.0	-28.0	0.78 (1)	6.25
X- W	-1251 / 7565	-28.0	-28.0	0.62 (1)	6.25
V- U	-19 / 110	-28.0	-28.0	0.10 (1)	6.25
U- T	-603 / 4696	-28.0	-28.0	0.44 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- C	-4479 / 822	0.46 (1)
C- Y	-1162 / 6481	0.75 (1)
Y- D	-254 / 1397	0.22 (1)
V- W	0 / 85	0.15 (1)
W- G	-938 / 300	0.37 (2)
U- H	-755 / 170	0.44 (1)
H- T	-248 / 1190	0.27 (3)
T- I	-838 / 245	0.49 (2)
T- J	-219 / 1185	0.27 (2)
S- J	-240 / 1427	0.32 (3)
S- L	-1700 / 401	0.84 (3)
Q- L	-31 / 532	0.12 (3)
Q- M	-675 / 196	0.54 (3)
P- M	-471 / 144	0.10 (1)
D- X	-2368 / 588	0.95 (2)
X- E	-37 / 810	0.14 (2)
E- W	-1793 / 382	0.45 (2)
W- U	-599 / 4899	0.55 (1)
W- H	-694 / 3381	0.76 (2)
AB-AC	-608 / 135	0.00 (1)
AC- Z	-433 / 2864	0.34 (1)
P-AE	-437 / 3566	0.42 (1)
AD-AE	0 / 414	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.9	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	74.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.48")

CSI: TC=0.90 (C-D:1) , BC=0.78 (X-Y:1) , WB=0.95 (D-X:2) , SSI=0.37 (J-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.98 (Y) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 91277 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM459866 17 pg 16

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
T- S	-510 / 4685	-28.0	-28.0	0.38 (1)	6.25
S- R	-661 / 5595	-28.0	-28.0	0.45 (1)	6.25
R- Q	-661 / 5595	-28.0	-28.0	0.45 (1)	6.25
Q- P	-827 / 5962	-28.0	-28.0	0.50 (1)	6.25
P-AD	-395 / 2449	-28.0	-28.0	0.23 (3)	6.25
AD- N	-395 / 2449	-28.0	-28.0	0.21 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
9127
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-59862 17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	G16	1	1	TRUSS DESC.	

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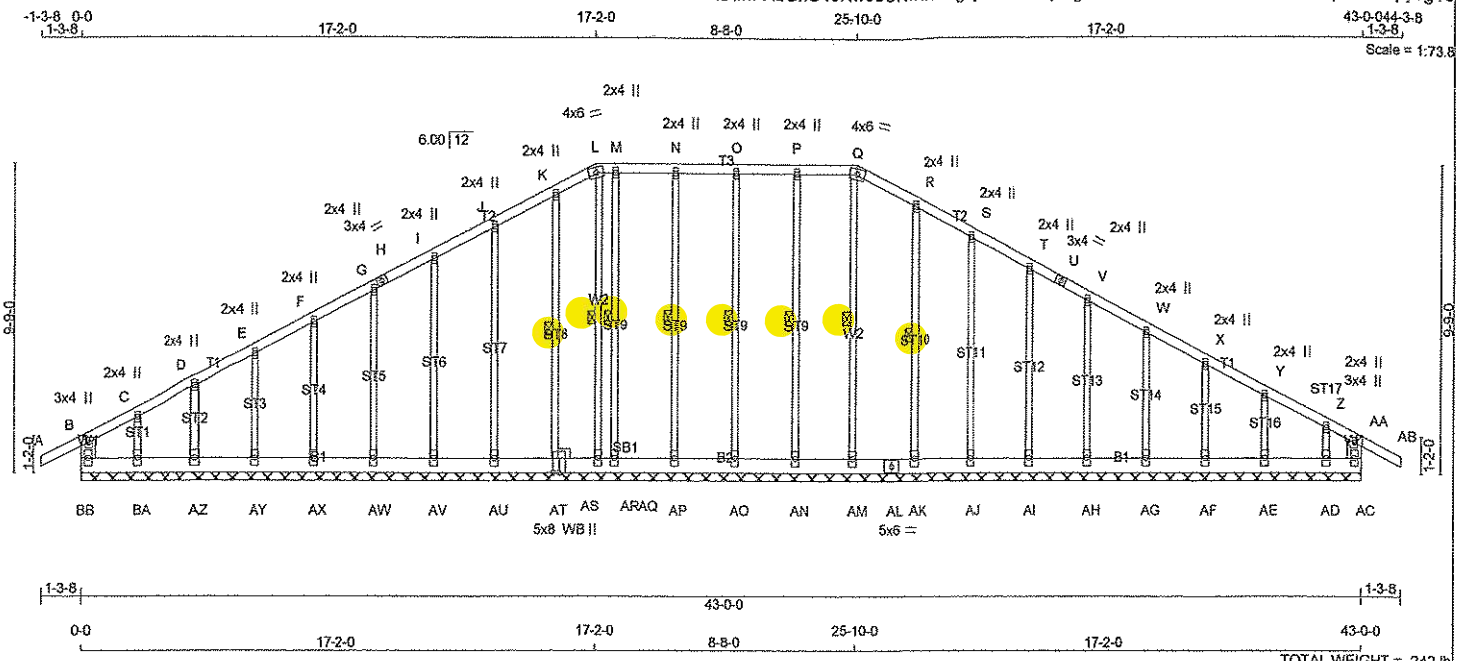

1-3-8 — 43-0-0 — 1-3-8

0-0 — 17-2-0 — 17-2-0 — 8-8-0 — 25-10-0 — 17-2-0 — 43-0-0

TOTAL WEIGHT = 242 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
BB- B	2x6	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
L - Q	2x4	DRY	No.2	SPF
Q - U	2x4	DRY	No.2	SPF
U - AB	2x4	DRY	No.2	SPF
AC- AA	2x6	DRY	No.2	SPF
BB- AS	2x6	DRY	No.2	SPF
AS- AL	2x6	DRY	No.2	SPF
AL- AC	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C, D, E, F, G, I, J, K, M, N, O, P, R, S, T, V, W, X, Y, Z						
C	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	4.0		
L	TTW-m	MT20	4.0	6.0		
Q	TTW-m	MT20	4.0	6.0		
U	TS-t	MT20	3.0	4.0		
AA	TMV+p	MT20	3.0	4.0	2.00	1.25
AC	BMV1+p	MT20	3.0	6.0		
AD, AE, AF, AG, AH, AI, AJ, AK, AM, AN, AO, AP, AQ, AR, AU, AV, AW, AX, AY, AZ, BA						
AD	BMW1+w	MT20	3.0	6.0		
AL	BS-t	MT20	5.0	6.0		
AS						
AS	BSW1+I	MT20	5.0	8.0		
AT						
BB	BMV1+p	MT20	3.0	6.0		

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF P-AN, O-AO, N-AP, M-AQ, K-AT, R-AK, L-AR, Q-AM.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
BB- B	-301 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 37	-122.2	-122.2	0.16 (1)	10.00	
B- C	-25 / 0	-122.2	-122.2	0.09 (1)	6.25	
C- D	-4 / 0	-122.2	-122.2	0.05 (1)	10.00	
D- E	-2 / 0	-122.2	-122.2	0.06 (1)	10.00	
E- F	-1 / 4	-122.2	-122.2	0.06 (1)	10.00	
F- G	-1 / 8	-122.2	-122.2	0.06 (1)	10.00	
G- H	0 / 11	-122.2	-122.2	0.06 (1)	6.25	
H- I	0 / 11	-122.2	-122.2	0.06 (1)	6.25	
I- J	0 / 14	-122.2	-122.2	0.06 (1)	10.00	
J- K	0 / 13	-122.2	-122.2	0.06 (1)	10.00	
K- L	0 / 16	-122.2	-122.2	0.04 (1)	10.00	
L- M	0 / 17	-122.2	-122.2	0.04 (1)	10.00	
M- N	0 / 17	-122.2	-122.2	0.06 (1)	10.00	
N- O	0 / 17	-122.2	-122.2	0.06 (1)	10.00	
O- P	0 / 17	-122.2	-122.2	0.06 (1)	10.00	
P- Q	0 / 17	-122.2	-122.2	0.06 (1)	10.00	
Q- R	0 / 16	-122.2	-122.2	0.06 (1)	10.00	
R- S	0 / 14	-122.2	-122.2	0.06 (1)	10.00	
S- T	0 / 11	-122.2	-122.2	0.06 (1)	10.00	
T- U	0 / 9	-122.2	-122.2	0.06 (1)	10.00	
U- V	0 / 9	-122.2	-122.2	0.06 (1)	10.00	
V- W	-1 / 5	-122.2	-122.2	0.06 (1)	10.00	
W- X	-1 / 2	-122.2	-122.2	0.06 (1)	10.00	
X- Y	-5 / 0	-122.2	-122.2	0.06 (1)	10.00	
Y- Z	-7 / 0	-122.2	-122.2	0.06 (1)	10.00	
Z-AA	-34 / 0	-122.2	-122.2	0.08 (1)	6.25	
AA-AB	0 / 37	-122.2	-122.2	0.16 (1)	10.00	
AC-AA	-273 / 0	0.0	0.0	0.05 (1)	7.81	
BB-BA	0 / 15	-27.5	-27.5	0.04 (1)	10.00	
BA-AZ	0 / 7	-27.5	-27.5	0.01 (2)	10.00	
AZ-AY	0 / 1	-27.5	-27.5	0.01 (2)	10.00	
AY-AX	-4 / 1	-27.5	-27.5	0.01 (3)	10.00	
AX-AW	-7 / 0	-27.5	-27.5	0.01 (2)	10.00	
AW-AV	-10 / 0	-27.5	-27.5	0.01 (3)	10.00	
AV-AU	-12 / 0	-27.5	-27.5	0.01 (3)	6.25	
AU-AT	-14 / 0	-27.5	-27.5	0.01 (3)	6.25	
AT-AS	-16 / 0	-27.5	-27.5	0.01 (2)	6.25	
AS-AR	-16 / 0	-27.5	-27.5	0.01 (2)	6.25	
AR-AQ	-17 / 0	-27.5	-27.5	0.01 (2)	6.25	
AQ-AP	-17 / 0	-27.5	-27.5	0.01 (2)	6.25	
AP-AO	-17 / 0	-27.5	-27.5	0.01 (2)	6.25	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
AN- P	-245 / 0	0.15 (1)
AO- O	-244 / 0	0.15 (1)
AP- N	-251 / 0	0.15 (1)
AQ- M	-164 / 0	0.10 (1)
AT- K	-204 / 0	0.11 (1)
AU- J	-251 / 0	0.30 (1)
AV- I	-242 / 0	0.20 (1)
AW- G	-243 / 0	0.13 (1)
AX- F	-243 / 0	0.09 (1)
AY- E	-240 / 0	0.06 (1)
AZ- D	-251 / 0	0.04 (1)
BA- C	-199 / 0	0.03 (1)
AK- R	-249 / 0	0.12 (1)
AJ- S	-242 / 0	0.26 (1)
AI- T	-243 / 0	0.17 (1)
AH- V	-243 / 0	0.12 (1)
AG- W	-243 / 0	0.08 (1)
AF- X	-239 / 0	0.05 (1)
AE- Y	-255 / 0	0.04 (1)
AD- Z	-144 / 0	0.02 (1)
AR- L	-122 / 0	0.07 (1)
AM- Q	-255 / 0	0.16 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16 (AA-AB:1) , BC=0.05 (AC-AD:1) , WB=0.30 (J-AU:1) , SSI=0.12 (AA-AB:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (T) (INPUT = 0.90)
JSI METAL= 0.07 (Q) (INPUT = 1.00)

P6 1/2

DWG NO. TAM 45996-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AO-AN	-17 / 0	-27.5	-27.5	0.01	(2)	6.25
AN-AM	-17 / 0	-27.5	-27.5	0.01	(2)	6.25
AM-AL	-14 / 0	-27.5	-27.5	0.01	(2)	6.25
AL-AK	-14 / 0	-27.5	-27.5	0.01	(2)	6.25
AK-AJ	-12 / 0	-27.5	-27.5	0.01	(2)	6.25
AJ-AI	-10 / 0	-27.5	-27.5	0.01	(2)	6.25
AI-AH	-8 / 0	-27.5	-27.5	0.01	(2)	10.00
AH-AG	-5 / 1	-27.5	-27.5	0.01	(3)	10.00
AG-AF	-1 / 1	-27.5	-27.5	0.01	(3)	10.00
AF-AE	0 / 4	-27.5	-27.5	0.01	(2)	10.00
AE-AD	0 / 11	-27.5	-27.5	0.01	(2)	10.00
AD-AC	0 / 18	-27.5	-27.5	0.05	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER

9/12/17

S. KATSOULAKOS

PROVINCE OF ONTARIO

DWG NO. TAM45996-17

STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T17	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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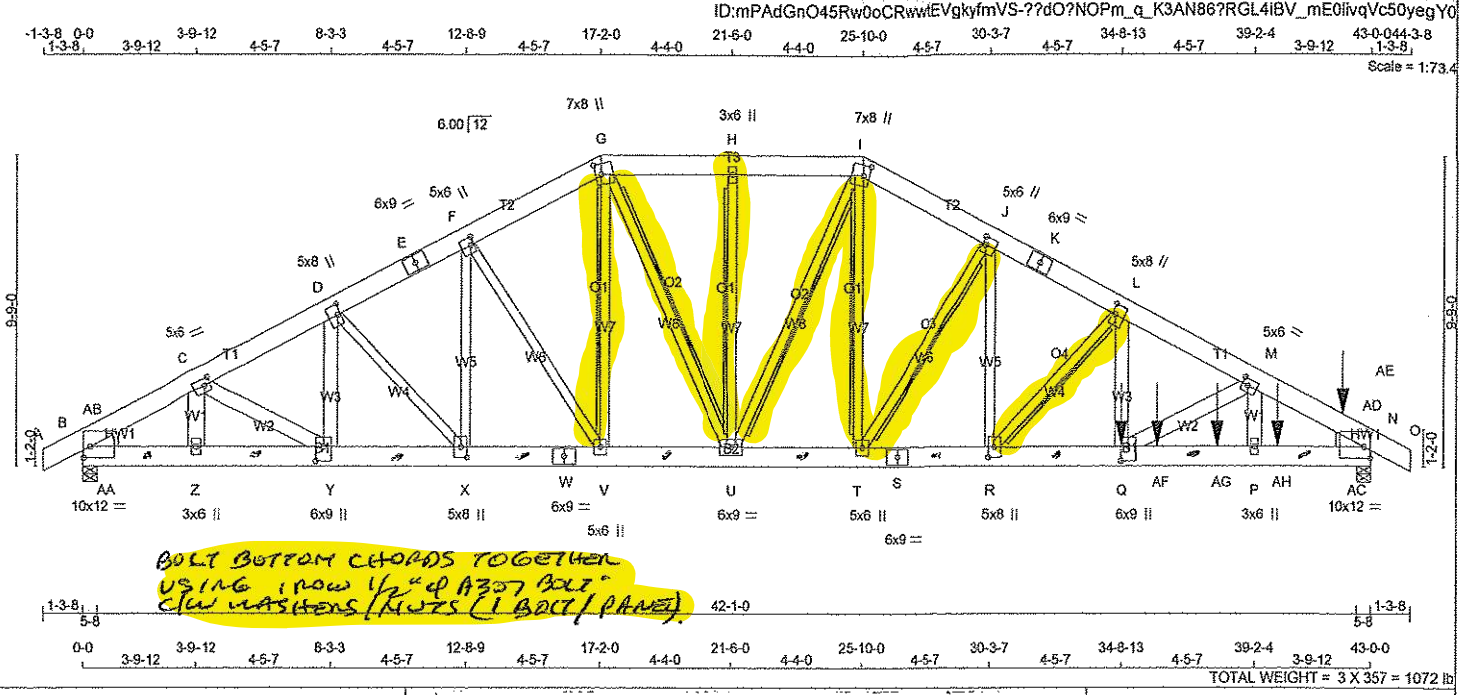

BOLT BOTTOM CHORDS TOGETHER USING 1 ROW 1/2" ϕ A307 BOLT C/W WASHERS/NUTS (1 BOLT/PANEL).

TOTAL WEIGHT = 3 X 357 = 1072 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x8	DRY	No.2	SPF
E - G	2x8	DRY	No.2	SPF
G - I	2x8	DRY	No.2	SPF
I - K	2x8	DRY	No.2	SPF
K - O	2x8	DRY	No.2	SPF
B - W	2x8	DRY	1950F 1.7E	SPF
W - S	2x8	DRY	1950F 1.7E	SPF
S - N	2x8	DRY	1950F 1.7E	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
Z - C	2x6	DRY	No.2	SPF
C - Y	2x6	DRY	No.2	SPF
Y - D	2x6	DRY	No.2	SPF
Q - L	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
E- A	2	11	TOP
E- G	2	12	TOP
G- I	2	12	TOP
I- K	2	12	TOP
K- O	2	11	SIDE(277.2)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- W	4	4	SIDE
W- S	2	12	SIDE
S- N	4	4	SIDE(2585.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH11-t	MT20	10.0	12.0	4.25	2.50
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TMWW+t	MT20	5.0	8.0	4.00	1.25
E	TS-t	MT20	6.0	9.0		
F	TMWW+t	MT20	5.0	6.0	3.00	1.25
G	TTWW+m	MT20	7.0	8.0	4.00	2.25
H	TMW+w	MT20	3.0	6.0		
I	TTWW+m	MT20	7.0	8.0	4.00	2.25
J	TMWW+t	MT20	5.0	6.0	3.00	1.25
K	TS-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	6534	0	6534	204	-713	5-8	5-8	2x6 L
N	15114	0	15114	0	-713	5-8	5-8	2x6 R

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 713 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 713 LBS FACTORED UPLIFT

S1HPS2-1 H2-5T
1 H2-5T

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 204 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	4967	3454 / 0	718 / 0	0 / 0	11 / -850	795 / 0	0 / 0
N	11585	7916 / 0	1794 / 0	0 / 0	73 / -850	1874 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	146 / -146	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE ADEQUATELY BRACED.

2x6 DRY SPF No.2 T-BRACE AT G-V, G-U, H-U, I-U, I-T, J-T, L-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-168.0	-168.0	0.03 (2)	10.00
B-AB	-11822 / 1158	-168.0	-168.0	0.33 (1)	4.53
AB- C	-9867 / 1031	-168.0	-168.0	0.32 (1)	4.89
C- D	-10853 / 1110	-168.0	-168.0	0.18 (1)	4.90
D- E	-10432 / 1062	-168.0	-168.0	0.09 (2)	5.07
E- F	-10432 / 1062	-168.0	-168.0	0.09 (2)	5.07
F- G	-9724 / 981	-168.0	-168.0	0.09 (2)	5.21
G- H	-9892 / 932	-168.0	-168.0	0.08 (1)	5.19
H- I	-9892 / 932	-168.0	-168.0	0.08 (1)	5.19
I- J	-11823 / 981	-168.0	-168.0	0.11 (3)	4.82
J- K	-16302 / 1062	-168.0	-168.0	0.15 (1)	4.19
K- L	-16302 / 1062	-168.0	-168.0	0.15 (1)	4.19
L- M	-23817 / 1111	-168.0	-168.0	0.55 (1)	3.12
M-AE	-23169 / 1035	-168.0	-168.0	0.86 (1)	2.45
AE-AD	-23169 / 1035	-168.0	-168.0	0.86 (1)	2.45
AD- N	-27566 / 1163	-168.0	-168.0	0.93 (1)	2.03
N- O	0 / 19	-168.0	-168.0	0.03 (3)	10.00
B-AA	-999 / 8480	-28.0	-28.0	0.22 (1)	6.25
AA- Z	-999 / 8480	-28.0	-28.0	0.22 (1)	6.25
Z- Y	-999 / 8480	-28.0	-28.0	0.20 (1)	6.25
Y- X	-998 / 9814	-28.0	-28.0	0.15 (1)	6.25
X- W	-817 / 9314	-28.0	-28.0	0.15 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- C	-910 / 147	0.02 (1)
C- Y	-67 / 1553	0.06 (3)
Y- D	-400 / 90	0.02 (3)
D- X	-939 / 261	0.15 (2)
X- F	-117 / 677	0.04 (2)
F- V	-1453 / 379	0.46 (2)
V- G	-269 / 1366	0.07 (2)
G- U	-201 / 3504	0.19 (3)
U- H	-740 / 237	0.07 (3)
U- I	-2471 / 9	0.29 (3)
T- I	-270 / 6430	0.34 (3)
T- J	-7564 / 379	0.60 (3)
R- J	-117 / 7401	0.40 (3)
R- L	-10231 / 261	0.54 (3)
Q- L	0 / 9550	0.38 (3)
Q- M	-72 / 1998	0.08 (1)
P- M	-24 / 302	0.01 (2)
AA-AB	-226 / 3021	0.09 (1)
AC-AD	-228 / 6634	0.00 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.9	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	74.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(80 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.93 (N-AD:1) , BC=0.50 (P-AC:1) , WB=0.60 (J-T:3) , SSI=0.87 (N-AC:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.77 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9117 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM45987-17 P6/6

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
L	TMWW+t	MT20	5.0	8.0	4.00	1.25
M	TMWW-t	MT20	5.0	6.0	2.50	2.25
N	TMBH1-l	MT20	10.0	12.0	4.25	2.50
P	BMW+w	MT20	3.0	6.0		
Q	BMWW+t	MT20	6.0	9.0	5.50	3.00
R	BMWW+t	MT20	5.0	8.0	4.00	2.25
S	BS-t	MT20	6.0	9.0		
T	BMWW+t	MT20	5.0	6.0		
U	BMWWW-t	MT20	6.0	9.0		
V	BMWW+t	MT20	5.0	6.0		
W	BS-t	MT20	6.0	9.0		
X	BMWW+t	MT20	5.0	8.0	4.00	2.25
Y	BMWW+t	MT20	6.0	9.0	5.50	3.00
Z	BMW+w	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1062.6 lbs FACTORED DOWN AT 41-11-4 ON TOP CHORD, AND 9033.9 lbs FACTORED DOWN AT 34-8-12, 876.6 lbs FACTORED DOWN AT 35-11-4, AND 876.6 lbs FACTORED DOWN AT 37-11-4, AND 1047.1 lbs FACTORED DOWN AT 39-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
W- V	-817 / 9314	-28.0	-28.0	0.15 (1)		6.25
V- U	-611 / 8688	-28.0	-28.0	0.14 (1)		6.25
U- T	-521 / 10648	-28.0	-28.0	0.17 (1)		6.25
T- S	-613 / 14588	-28.0	-28.0	0.23 (1)		6.25
S- R	-613 / 14588	-28.0	-28.0	0.23 (1)		6.25
R- Q	-794 / 21509	-28.0	-28.0	0.33 (1)		6.25
Q-AF	-800 / 19755	-28.0	-28.0	0.45 (1)		6.25
AF-AG	-800 / 19755	-28.0	-28.0	0.45 (1)		6.25
AG- P	-800 / 19755	-28.0	-28.0	0.45 (1)		6.25
P-AH	-800 / 19755	-28.0	-28.0	0.50 (1)		6.25
AH-AC	-800 / 19755	-28.0	-28.0	0.50 (1)		6.25
AC- N	-800 / 19755	-28.0	-28.0	0.50 (1)		6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	34-8-12	-9034	-9034	---	FRONT	VERT	TOTAL
AE	41-11-4	-907	-1063	---	FRONT	VERT	TOTAL
AF	35-11-4	-877	-877	---	FRONT	VERT	TOTAL
AG	37-11-4	-877	-877	---	FRONT	VERT	TOTAL
AH	39-11-4	-1047	-1047	---	FRONT	VERT	TOTAL

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
9/12/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

P6 2/2

DWG NO. TAM 45987-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T17S	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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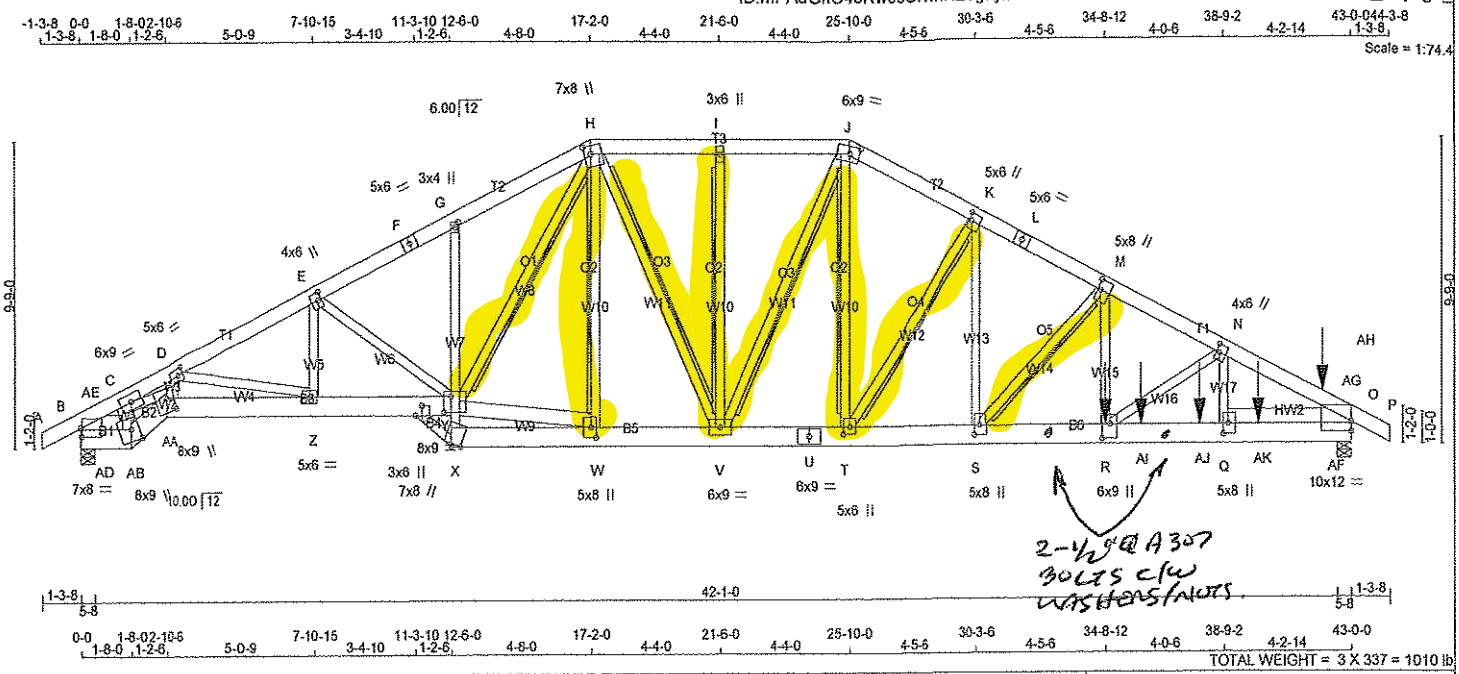

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
L - P	2x6	DRY	No.2	SPF
B - AB	2x8	DRY	No.2	SPF
AB- AA	2x8	DRY	No.2	SPF
AA- Y	2x8	DRY	No.2	SPF
X - U	2x8	DRY	No.2	SPF
U - O	2x8	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
AA- D	2x6	DRY	No.2	SPF
Y - W	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- F	2	12	TOP
F- H	2	12	TOP
H- J	2	12	TOP
J- L	2	12	TOP
L- P	2	11	SIDE(282.8)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- AB	2	12	TOP
AB- AA	2	12	TOP
AA- Y	2	12	TOP
X- U	2	12	TOP
U- O	4	4	SIDE(2521.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

SIDE

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-9 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-t	MT20	7.0	8.0	3.25	0.50
C	TMWW-t	MT20	6.0	9.0		
D	TMWW-t	MT20	5.0	6.0	2.50	2.25
E	TMWW+t	MT20	4.0	6.0	3.00	1.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	6389	0	6648	204	-1037	5-8	5-8
O	14512	0	16030	0	-2396	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1037 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 2396 LBS FACTORED UPLIFT

SIMPSON 2-H2SA / 2-TS22

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 204 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	4826	3573 / 0	659 / 0	0 / 0	88 / -1222	767 / 0	0 / 0
O	10992	8706 / 0	1535 / 0	0 / 0	476 / -2787	1762 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	145 / -145	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, O

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.58 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT H-W, H-V, I-V, J-V, J-T, K-T, M-S, N-Y

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3 COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 9	-168.0	-168.0	0.04 (2)	10.00	
B-AE	-7567 / 1169	-168.0	-168.0	0.17 (2)	5.07	
AE- C	-7974 / 1266	-168.0	-168.0	0.17 (2)	4.96	
C- D	-18453 / 3078	-168.0	-168.0	0.35 (2)	3.31	
D- E	-13440 / 2177	-168.0	-168.0	0.30 (2)	3.89	
E- F	-11788 / 1988	-168.0	-168.0	0.21 (2)	4.21	
F- G	-11788 / 1988	-168.0	-168.0	0.21 (2)	4.21	
G- H	-11743 / 2132	-168.0	-168.0	0.19 (2)	4.24	
H- I	-9454 / 1685	-168.0	-168.0	0.11 (1)	4.70	
I- J	-9454 / 1685	-168.0	-168.0	0.11 (1)	4.70	
J- K	-11451 / 1978	-168.0	-168.0	0.16 (3)	4.31	
K- L	-16321 / 2622	-168.0	-168.0	0.23 (3)	3.64	
L- M	-16321 / 2622	-168.0	-168.0	0.23 (3)	3.64	
M- N	-24292 / 3707	-168.0	-168.0	0.52 (3)	2.76	
N-AH	-24471 / 3661	-168.0	-168.0	0.64 (3)	2.58	
AH-AG	-24471 / 3661	-168.0	-168.0	0.64 (3)	2.58	
AG- O	-21397 / 3173	-168.0	-168.0	0.53 (3)	2.87	
O- P	0 / 9	-168.0	-168.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AB- C	-5793 / 1010	0.20 (2)
C-AA	-1943 / 11416	0.61 (2)
AA- D	-475 / 2790	0.11 (2)
X- Y	-33 / 75	0.07 (2)
Y- G	-898 / 294	0.09 (2)
W- H	-1436 / 261	0.14 (1)
H- V	-552 / 3613	0.19 (3)
V- I	-804 / 241	0.08 (2)
V- J	-2642 / 359	0.32 (3)
T- J	-1039 / 6820	0.37 (3)
T- K	-8026 / 1308	0.65 (3)
S- K	-1132 / 7862	0.42 (3)
S- M	-10484 / 1601	0.57 (3)
R- M	-1442 / 10280	0.55 (3)
R- N	-78 / 699	0.04 (2)
Q- N	-619 / 162	0.02 (1)
D- Z	-3912 / 811	0.35 (2)
Z- E	-58 / 751	0.04 (2)
E- Y	-2024 / 413	0.27 (2)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	51.9	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	74.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.64 (N-AG:3) , BC=0.95 (Q-R:3) , WB=0.65 (K-T:3) , SSI=0.33 (N-AG:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

LICENSED PROFESSIONAL ENGINEER 9/11/17 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM45988-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T17S	1	3	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	4.0	2.00	1.25
H	TTWWW+m	MT20	7.0	8.0	3.00	2.50
I	TMW+w	MT20	3.0	6.0		
J	TTWW-m	MT20	6.0	9.0	3.00	4.00
K	TMWW+t	MT20	5.0	6.0	3.00	1.25
L	TS-t	MT20	5.0	6.0		
M	TMWW+t	MT20	5.0	8.0	3.75	1.25
N	TMWW+t	MT20	4.0	6.0	3.00	1.25
O	TMBMW1-I	MT20	10.0	12.0	3.75	
Q	BMWW+t	MT20	5.0	8.0	4.00	2.00
R	BMWW+t	MT20	6.0	9.0	5.50	3.00
S	BMWW+t	MT20	5.0	8.0	3.75	2.25
T	BMWW+t	MT20	5.0	6.0	2.75	2.50
U	BS-t	MT20	6.0	9.0		
V	BMWWW-t	MT20	6.0	9.0		
W	BMWW+t	MT20	5.0	8.0	4.00	2.00
X						
X						
X	BBW+m	MT20	7.0	8.0	Edge	
Y	BWMWWW*-I	MT20	8.0	9.0	6.00	3.00
Z	BMWW-t	MT20	5.0	6.0		
AA	BBWW+m	MT20	6.0	9.0	5.50	3.25
AB	BBWW+m	MT20	8.0	9.0	4.50	3.25
AC	NP+p	MT20	3.0	6.0	3.75	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1084.2 lbs FACTORED DOWN AND 167.7 lbs FACTORED UP AT 41-11-4 ON TOP CHORD, AND 876.6 lbs FACTORED DOWN AT 35-11-4, 1205.2 lbs FACTORED DOWN AND 186.4 lbs FACTORED UP AT 39-11-4, AND 1028.3 lbs FACTORED DOWN AND 159.0 lbs FACTORED UP AT 37-11-4, AND 9664.1 lbs FACTORED DOWN AND 1494.6 lbs FACTORED UP AT 34-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
B-AD	-871 / 4517	-28.0	-28.0	0.17 (2)	6.25	
AD-AB	-872 / 4527	-28.0	-28.0	0.17 (2)	6.25	
AB-AA	-1230 / 6820	-28.0	-28.0	0.22 (2)	6.25	
AA- Z	-2764 / 15927	-28.0	-28.0	0.57 (2)	6.25	
Z- Y	-1975 / 12122	-28.0	-28.0	0.45 (2)	6.25	
X- W	-25 / 156	-28.0	-28.0	0.07 (1)	6.25	
W- V	-1215 / 8371	-28.0	-28.0	0.33 (1)	6.25	
V- U	-1423 / 10310	-28.0	-28.0	0.34 (3)	6.25	
U- T	-1423 / 10310	-28.0	-28.0	0.34 (3)	6.25	
T- S	-2013 / 14620	-28.0	-28.0	0.50 (3)	6.25	
S- R	-3108 / 21790	-28.0	-28.0	0.81 (3)	6.25	
R-AI	-3115 / 21527	-28.0	-28.0	0.95 (3)	6.25	
AI-AJ	-3115 / 21527	-28.0	-28.0	0.95 (3)	6.25	
AJ- Q	-3115 / 21527	-28.0	-28.0	0.95 (3)	6.25	
Q-AK	-1889 / 12835	-28.0	-28.0	0.58 (3)	6.25	
AK-AF	-1889 / 12835	-28.0	-28.0	0.58 (3)	6.25	
AF- O	-1889 / 12835	-28.0	-28.0	0.55 (3)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- W	-1222 / 8423	0.33 (1)
Y- H	-861 / 4540	0.24 (2)
AD-AE	-515 / 108	0.00 (1)
AE-AB	-383 / 2579	0.10 (2)
Q-AG	-1264 / 8864	0.35 (3)
AF-AG	-298 / 2331	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	34-8-12	-8286	-9664	1495	FRONT	VERT	TOTAL
AH	41-11-4	-930	-1084	168	FRONT	VERT	TOTAL
AI	35-11-4	-877	-877	---	BACK	VERT	TOTAL
AJ	37-11-4	-882	-1028	159	FRONT	VERT	TOTAL
AK	39-11-4	-1033	-1205	186	FRONT	VERT	TOTAL

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.81 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9177
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PROVINCE OF ONTARIO

DWG NO. TAM 45988 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T17Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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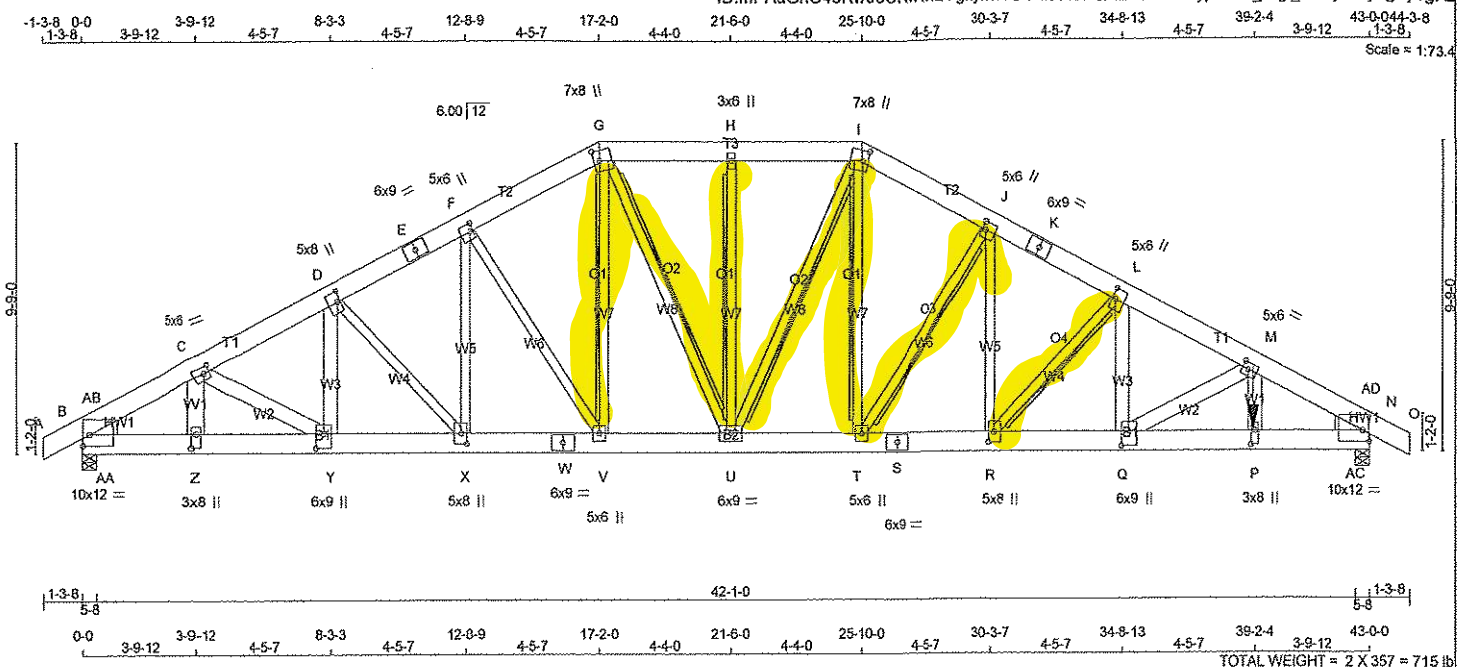

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
K - O	2x6	DRY	No.2	SPF
B - W	2x8	DRY	1950F 1.7E	SPF
W - S	2x8	DRY	1950F 1.7E	SPF
S - N	2x8	DRY	1950F 1.7E	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
Z - C	2x6	DRY	No.2	SPF
C - Y	2x6	DRY	No.2	SPF
Y - D	2x6	DRY	No.2	SPF
Q - L	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
E- A	2	12	TOP
E- G	2	12	TOP
G- I	2	12	TOP
I- K	2	12	TOP
K- O	2	12	SIDE(125.6)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- W	2	12	SIDE
W- S	2	12	SIDE
S- N	2	12	SIDE(30.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	10.0	12.0	4.25	2.50
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TMWW+t	MT20	5.0	8.0	4.00	1.25
E	TS-t	MT20	6.0	9.0		
F	TMWW+t	MT20	5.0	6.0	3.00	1.25
G	TTWW+m	MT20	7.0	8.0	4.00	2.25
H	TMW+w	MT20	3.0	6.0		
I	TTWW+m	MT20	7.0	8.0	4.00	2.25
J	TMWW+t	MT20	5.0	6.0	3.00	1.25
K	TS-t	MT20	6.0	9.0		
L	TMWW+t	MT20	5.0	8.0	4.00	1.25
M	TMWW-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	4916	0	4916	-376	-726	5-8	5-8	2x6 L
N	9901	0	9901	0	-865	5-8	5-8	2x6 R

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 726 LBS FACTORED UPLIFT SIMPSON H1-H2.5A

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 865 LBS FACTORED UPLIFT 2-H2.5A

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 376 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3701	2623 / 0	489 / 0	0 / 0	15 / -863	589 / 0	0 / 0
N	7524	5242 / 0	1085 / 0	0 / 0	122 / -1031	1196 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	142 / -269	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N.

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.11 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT G-V, G-U, H-U, I-U, I-T, J-T, L-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-168.0	-168.0	0.05 (2)	10.00
B-AB	-8764 / 1198	-168.0	-168.0	0.40 (1)	4.22
AB- C	-7188 / 1053	-168.0	-168.0	0.38 (1)	4.61
C- D	-7675 / 1135	-168.0	-168.0	0.21 (1)	4.71
D- E	-7108 / 1087	-168.0	-168.0	0.13 (2)	4.96
E- F	-7108 / 1087	-168.0	-168.0	0.13 (2)	4.96
F- G	-6322 / 1007	-168.0	-168.0	0.12 (2)	5.20
G- H	-6086 / 960	-168.0	-168.0	0.10 (1)	5.29
H- I	-6086 / 960	-168.0	-168.0	0.10 (1)	5.29
I- J	-6798 / 1019	-168.0	-168.0	0.13 (3)	5.04
J- K	-8449 / 1121	-168.0	-168.0	0.17 (3)	4.59
K- L	-8449 / 1121	-168.0	-168.0	0.17 (3)	4.59
L- M	-10662 / 1214	-168.0	-168.0	0.43 (1)	3.86
M-AD	-14315 / 1203	-443.4	-443.4	0.83 (1)	2.62
AD- N	-17535 / 1256	-443.4	-443.4	0.91 (1)	2.11
N- O	0 / 19	-168.0	-168.0	0.05 (3)	10.00
B-AA	-849 / 6201	-27.5	-27.5	0.27 (1)	6.25
AA- Z	-849 / 6201	-27.5	-27.5	0.27 (1)	6.25
Z- Y	-849 / 6201	-27.5	-27.5	0.24 (1)	6.25
Y- X	-847 / 6939	-27.5	-27.5	0.18 (1)	6.25
X- W	-668 / 6347	-27.5	-27.5	0.16 (1)	6.25
W- V	-668 / 6347	-27.5	-27.5	0.16 (1)	6.25
V- U	-462 / 5648	-27.5	-27.5	0.14 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- C	-659 / 150	0.03 (1)
C- Y	-68 / 877	0.05 (3)
Y- D	-177 / 169	0.01 (10)
D- X	-1071 / 260	0.26 (2)
X- F	-116 / 814	0.07 (2)
F- V	-1590 / 378	0.75 (2)
V- G	-269 / 1479	0.13 (2)
G- U	-215 / 1721	0.15 (3)
U- H	-786 / 237	0.10 (2)
U- I	-729 / 677	0.11 (3)
T- I	-299 / 2628	0.23 (3)
T- J	-2954 / 415	0.31 (3)
R- J	-153 / 2367	0.21 (3)
R- L	-3328 / 310	0.26 (3)
Q- L	-44 / 2246	0.15 (3)
Q- M	-3236 / 147	0.28 (3)
P- M	0 / 3533	0.23 (2)
AA-AB	-253 / 2331	0.00 (1)
AC-AD	-113 / 4515	0.00 (1)

DWG NO. TAM 45989-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.9	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	73.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 7-2-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 100 % OF GSL. (DEFINED BY USER)
LOADS APPLIED TO FIRST 3-10-8 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(80 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.91 (N-AD:1) , BC=0.52 (P-AC:1) , WB=0.76 (F-V:2) , SSI=0.94 (N-AC:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T17Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	TMBH1-I	MT20	10.0	12.0	4.25	2.50
P	BMW+w	MT20	3.0	8.0	5.25	1.50
Q	BMWW+t	MT20	6.0	9.0	5.50	3.00
R	BMWW+t	MT20	5.0	8.0	4.00	2.25
S	BS-t	MT20	6.0	9.0		
T	BMWW+t	MT20	5.0	6.0		
U	BMWWW-t	MT20	6.0	9.0		
V	BMWW+t	MT20	5.0	6.0		
W	BS-t	MT20	6.0	9.0		
X	BMWW+t	MT20	5.0	8.0	4.00	2.25
Y	BMWW+t	MT20	6.0	9.0	5.50	3.00
Z	BMW+w	MT20	3.0	8.0	5.25	1.50

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4747.1 lbs FACTORED DOWN AT 39-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
U- T	-383 / 6095	-27.5	-27.5	0.15 (1)	6.25
T- S	-495 / 7534	-27.5	-27.5	0.20 (1)	6.25
S- R	-495 / 7534	-27.5	-27.5	0.20 (1)	6.25
R- Q	-709 / 9685	-27.5	-27.5	0.25 (1)	6.25
Q- P	-838 / 12332	-27.5	-27.5	0.52 (1)	6.25
P-AC	-838 / 12332	-63.0	-63.0	0.52 (1)	6.25
AC- N	-838 / 12332	-63.0	-63.0	0.52 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	39-1-8	-4747	-4747	---	BACK	VERT	TOTAL

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.72 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9147
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45989-17
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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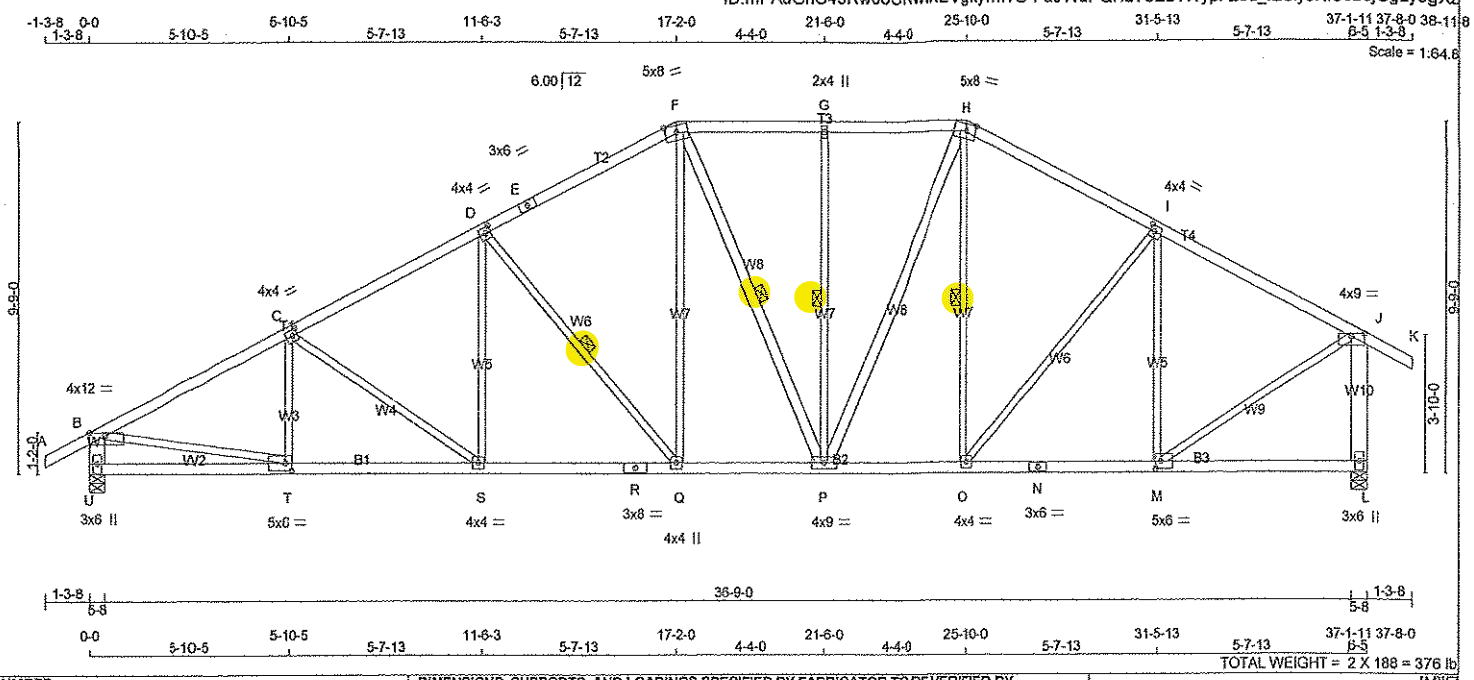

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - P	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	1.00	5.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	4.0	9.0	1.00	4.50
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	1.75
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW+t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	8.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	5.0	8.0	2.50	2.25
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2986	0	2986	0	0	5-8	5-8
L	2986	0	2986	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2306	1544 / 0	377 / 0	0 / 0	0 / 0	385 / 0	0 / 0
L	2306	1544 / 0	377 / 0	0 / 0	0 / 0	385 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-Q, F-P, G-P, H-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 37	-122.2	-122.2	0.16 (1)	10.00	
B- C	-4217 / 0	-122.2	-122.2	0.86 (1)	2.70	
C- D	-3885 / 0	-122.2	-122.2	0.70 (1)	3.02	
D- E	-3145 / 0	-122.2	-122.2	0.64 (1)	3.35	
E- F	-3145 / 0	-122.2	-122.2	0.64 (1)	3.35	
F- G	-2782 / 0	-122.2	-122.2	0.38 (1)	3.83	
G- H	-2782 / 0	-122.2	-122.2	0.38 (1)	3.83	
H- I	-2758 / 0	-122.2	-122.2	0.69 (1)	3.41	
I- J	-2529 / 0	-122.2	-122.2	0.70 (1)	3.54	
J- K	0 / 37	-122.2	-122.2	0.16 (1)	10.00	
U- B	-2915 / 0	0.0	0.0	0.19 (1)	6.15	
L- J	-2916 / 0	0.0	0.0	0.47 (1)	6.15	
U- T	0 / 0	-27.5	-27.5	0.21 (3)	10.00	
T- S	0 / 3797	-27.5	-27.5	0.70 (1)	10.00	
S- R	0 / 3475	-27.5	-27.5	0.65 (1)	10.00	
R- Q	0 / 3475	-27.5	-27.5	0.65 (1)	10.00	
Q- P	0 / 2795	-27.5	-27.5	0.53 (1)	10.00	
P- O	0 / 2428	-27.5	-27.5	0.46 (1)	10.00	
O- N	0 / 2296	-27.5	-27.5	0.50 (1)	10.00	
N- M	0 / 2296	-27.5	-27.5	0.50 (1)	10.00	
M- L	0 / 0	-27.5	-27.5	0.25 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-389 / 109	0.10 (1)
C- S	-388 / 0	0.34 (1)
S- D	0 / 406	0.09 (2)
D- Q	-1052 / 0	0.56 (1)
Q- F	0 / 936	0.21 (1)
F- P	-46 / 0	0.03 (2)
P- G	-641 / 0	0.41 (1)
P- H	0 / 850	0.14 (1)
O- H	-17 / 162	0.04 (3)
O- I	0 / 201	0.05 (1)
M- I	-1121 / 0	0.92 (1)
B- T	0 / 3838	0.86 (1)
M- J	0 / 2643	0.59 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.86 (B-C:1) , BC=0.70 (S-T:1) , WB=0.92 (I-M:1) , SSI=0.32 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.90 (R) (INPUT = 1.00)

DWG NO. TAM45990-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T18A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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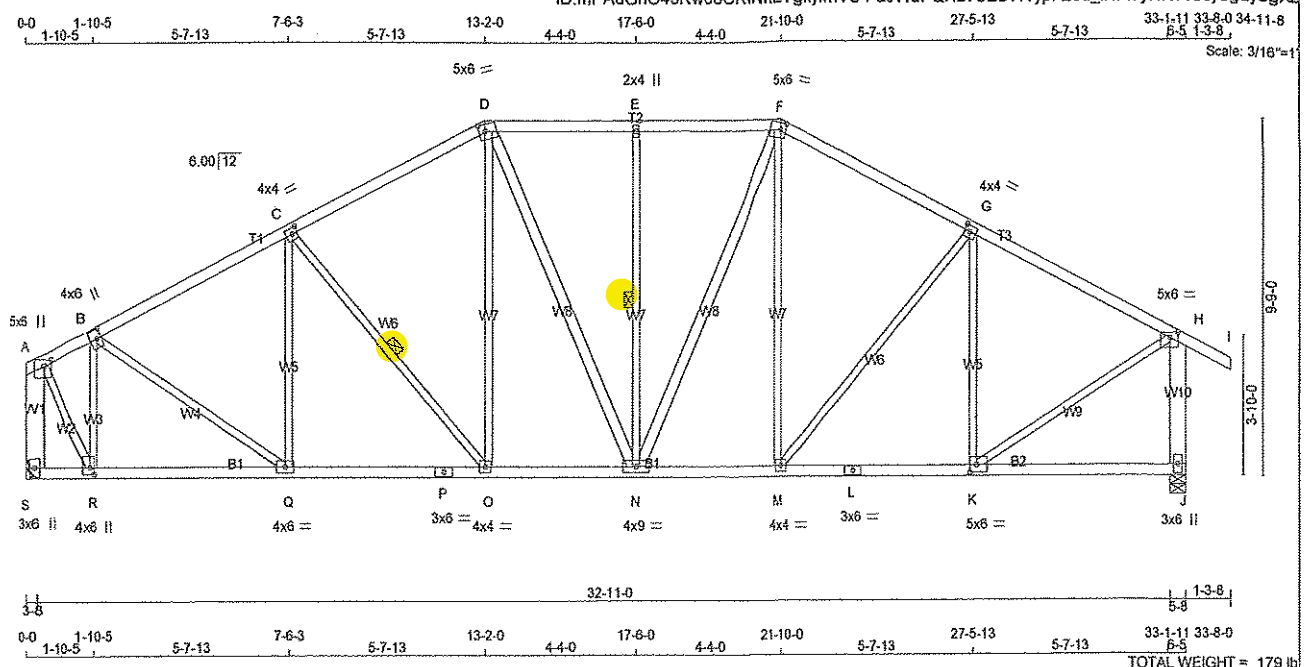

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
S - A	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	2.25	2.25
B	TMWW+t	MT20	4.0	6.0	3.00	1.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMWW-p	MT20	5.0	6.0	1.75	2.75
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BMWW+t	MT20	4.0	6.0	2.25	1.50
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2520	0	2520	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	2686	0	2686	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1961	1287 / 0	337 / 0	0 / 0	0 / 0	337 / 0	0 / 0
J	2073	1391 / 0	337 / 0	0 / 0	0 / 0	345 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1333 / 0	-122.2	-122.2	0.23 (1)	5.36
B- C	-2552 / 0	-122.2	-122.2	0.57 (1)	3.75
C- D	-2429 / 0	-122.2	-122.2	0.57 (1)	3.82
D- E	-2270 / 0	-122.2	-122.2	0.34 (1)	4.21
E- F	-2270 / 0	-122.2	-122.2	0.34 (1)	4.21
F- G	-2337 / 0	-122.2	-122.2	0.65 (1)	3.72
G- H	-2217 / 0	-122.2	-122.2	0.67 (1)	3.78
H- I	0 / 37	-122.2	-122.2	0.16 (1)	10.00
S- A	-2521 / 0	0.0	0.0	0.29 (1)	6.52
J- H	-2617 / 0	0.0	0.0	0.42 (1)	6.42
S- R	0 / 0	-27.5	-27.5	0.12 (3)	10.00
R- Q	0 / 1236	-27.5	-27.5	0.31 (2)	10.00
Q- P	0 / 2298	-27.5	-27.5	0.46 (1)	10.00
P- O	0 / 2298	-27.5	-27.5	0.46 (1)	10.00
O- N	0 / 2145	-27.5	-27.5	0.42 (1)	10.00
N- M	0 / 2053	-27.5	-27.5	0.40 (1)	10.00
M- L	0 / 2017	-27.5	-27.5	0.46 (1)	10.00
L- K	0 / 2017	-27.5	-27.5	0.46 (1)	10.00
K- J	0 / 0	-27.5	-27.5	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- B	-1772 / 0	0.45 (1)
B- Q	0 / 1279	0.29 (1)
Q- C	-549 / 27	0.45 (1)
C- O	-239 / 0	0.13 (1)
O- D	0 / 344	0.08 (2)
D- N	0 / 390	0.05 (1)
N- E	-644 / 0	0.41 (1)
N- F	0 / 522	0.08 (1)
M- F	0 / 203	0.05 (3)
M- G	0 / 54	0.01 (1)
K- G	-962 / 0	0.79 (1)
A- R	0 / 2265	0.51 (1)
K- H	0 / 2321	0.52 (1)

LICENSED PROFESSIONAL ENGINEER
9127
S. KATSOULAKOS
PROVINCE OF ONTARIO

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.12")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.67 (G-H:1) , BC=0.46 (O-Q:1) , WB=0.79 (G-K:1) , SSI=0.32 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.61 (P) (INPUT = 1.00)

DWG NO. TAM 4599/-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	G18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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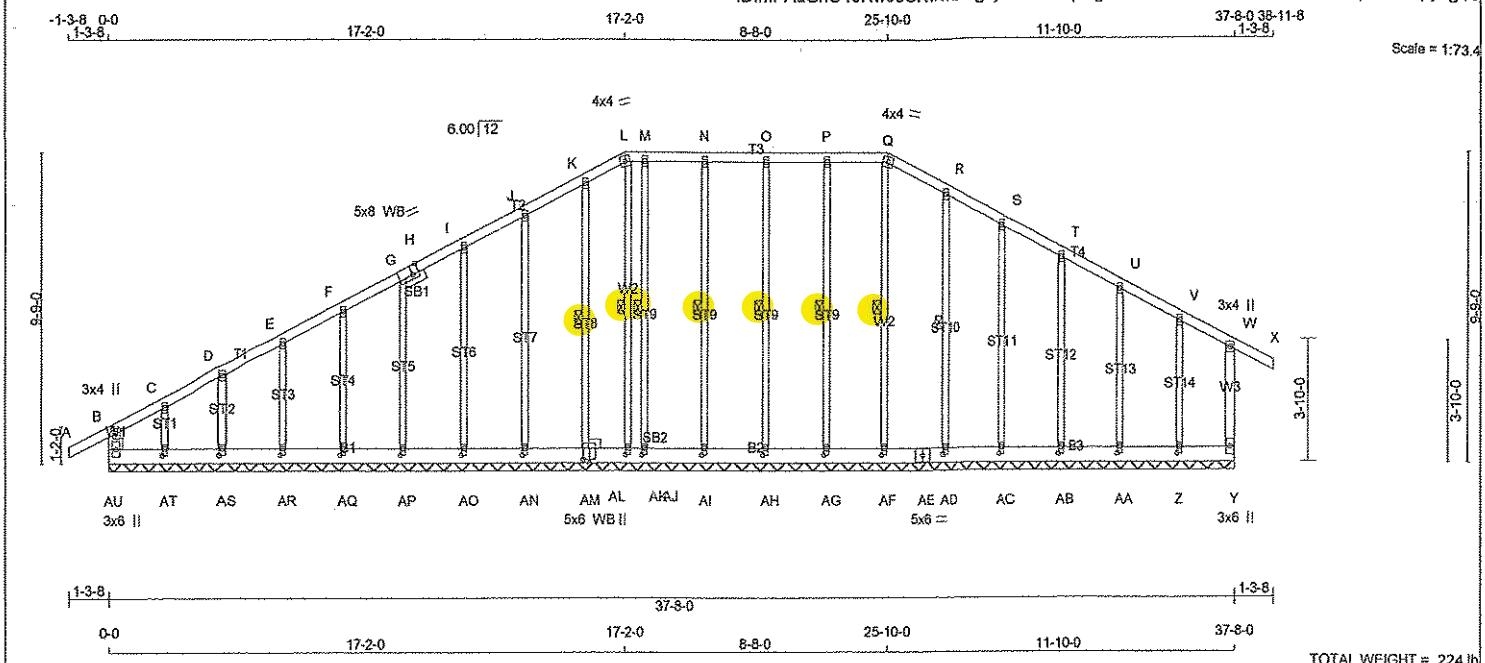

TOTAL WEIGHT = 224 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AU- B	2x6	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
L - Q	2x4	DRY	No.2	SPF
Q - X	2x4	DRY	No.2	SPF
Y - W	2x4	DRY	No.2	SPF
AU- AL	2x6	DRY	No.2	SPF
AL- AE	2x6	DRY	No.2	SPF
AE- Y	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C, D, E, F, I, J, K, M, N, O, P, R, S, T, U, V						
C	TMW+w	MT20	2.0	4.0		
G, H, AL, AM						
G						
H	TSW-l	MT20	5.0	8.0	Edge 2.50	
L	TTW-m	MT20	4.0	4.0		
Q	TTW-m	MT20	4.0	4.0		
W	TMV+p	MT20	3.0	4.0		
Y	BMV1+p	MT20	3.0	6.0		
Z, AA, AB, AC, AD, AF, AG, AH, AI, AJ, AK, AN, AO, AP, AQ, AR, AS, AT						
Z	BMW1+w	MT20	2.0	4.0	2.50	1.00
AE	BS-l	MT20	5.0	6.0		
AL	BSW1+l	MT20	5.0	6.0	1.25	2.50
AU	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF P-AG, O-AH, N-AI, M-AJ, K-AM, R-AD, L-AK, Q-AF.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AU- B	-295 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 37	-122.2	-122.2	0.16 (1)	10.00	
B- C	-14 / 0	-122.2	-122.2	0.08 (1)	6.25	
C- D	0 / 6	-122.2	-122.2	0.06 (1)	10.00	
D- E	0 / 9	-122.2	-122.2	0.06 (1)	10.00	
E- F	0 / 14	-122.2	-122.2	0.06 (1)	10.00	
F- G	0 / 18	-122.2	-122.2	0.06 (1)	10.00	
G- H	0 / 21	-122.2	-122.2	0.06 (1)	6.25	
H- I	0 / 21	-122.2	-122.2	0.06 (1)	6.25	
I- J	0 / 24	-122.2	-122.2	0.06 (1)	10.00	
J- K	0 / 23	-122.2	-122.2	0.06 (1)	10.00	
K- L	0 / 26	-122.2	-122.2	0.04 (1)	10.00	
L- M	0 / 27	-122.2	-122.2	0.04 (1)	10.00	
M- N	0 / 26	-122.2	-122.2	0.06 (1)	10.00	
N- O	0 / 26	-122.2	-122.2	0.06 (1)	10.00	
O- P	0 / 26	-122.2	-122.2	0.06 (1)	10.00	
P- Q	0 / 26	-122.2	-122.2	0.06 (1)	10.00	
Q- R	0 / 26	-122.2	-122.2	0.06 (1)	10.00	
R- S	0 / 25	-122.2	-122.2	0.06 (1)	10.00	
S- T	0 / 22	-122.2	-122.2	0.06 (1)	10.00	
T- U	0 / 18	-122.2	-122.2	0.06 (1)	10.00	
U- V	0 / 20	-122.2	-122.2	0.06 (1)	10.00	
V- W	-3 / 0	-122.2	-122.2	0.11 (1)	10.00	
W- X	0 / 37	-122.2	-122.2	0.16 (1)	10.00	
Y- W	-301 / 0	0.0	0.0	0.04 (1)	7.81	
AU-AT	0 / 6	-27.5	-27.5	0.04 (1)	10.00	
AT-AS	-2 / 0	-27.5	-27.5	0.01 (2)	10.00	
AS-AR	-8 / 0	-27.5	-27.5	0.01 (2)	10.00	
AR-AQ	-13 / 0	-27.5	-27.5	0.01 (2)	6.25	
AQ-AP	-16 / 0	-27.5	-27.5	0.01 (2)	6.25	
AP-AO	-19 / 0	-27.5	-27.5	0.01 (3)	6.25	
AO-AN	-21 / 0	-27.5	-27.5	0.01 (3)	6.25	
AN-AM	-23 / 0	-27.5	-27.5	0.01 (3)	6.25	
AM-AL	-25 / 0	-27.5	-27.5	0.01 (2)	6.25	
AL-AK	-25 / 0	-27.5	-27.5	0.01 (2)	6.25	
AK-AJ	-26 / 0	-27.5	-27.5	0.01 (2)	6.25	
AJ-AI	-26 / 0	-27.5	-27.5	0.01 (2)	6.25	
AI-AH	-26 / 0	-27.5	-27.5	0.01 (2)	6.25	
AH-AG	-26 / 0	-27.5	-27.5	0.01 (2)	6.25	
AG-AF	-26 / 0	-27.5	-27.5	0.01 (3)	6.25	
AF-AE	-24 / 0	-27.5	-27.5	0.01 (2)	6.25	
AE-AD	-24 / 0	-27.5	-27.5	0.01 (2)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AG- P	-245 / 0	0.15 (1)
AH- O	-244 / 0	0.15 (1)
AI- N	-251 / 0	0.15 (1)
AJ- M	-165 / 0	0.10 (1)
AM- K	-205 / 0	0.11 (1)
AN- J	-251 / 0	0.30 (1)
AO- I	-242 / 0	0.20 (1)
AP- G	-243 / 0	0.13 (1)
AQ- F	-243 / 0	0.09 (1)
AR- E	-240 / 0	0.06 (1)
AS- D	-250 / 0	0.04 (1)
AT- C	-201 / 0	0.03 (1)
AD- R	-250 / 0	0.12 (1)
AC- S	-242 / 0	0.26 (1)
AB- T	-242 / 0	0.17 (1)
AA- U	-254 / 0	0.12 (1)
Z- V	-187 / 0	0.06 (1)
AK- L	-125 / 0	0.08 (1)
AF- Q	-259 / 0	0.16 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16 (A-B:1) , BC=0.04 (AT-AU:1) , WB=0.30 (J-AN:1) , SSI=0.12 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (Q) (INPUT = 0.90)
JSI METAL= 0.08 (Q) (INPUT = 1.00)

p6/L

DWG NO. TAM4599717
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AD-AC	-22 / 0	-27.5	-27.5	0.01	(2)	6.25
AC-AB	-19 / 0	-27.5	-27.5	0.01	(2)	6.25
AB-AA	-17 / 0	-27.5	-27.5	0.01	(2)	6.25
AA- Z	-14 / 0	-27.5	-27.5	0.01	(2)	6.25
Z- Y	-11 / 0	-27.5	-27.5	0.01	(2)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
9/12/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45997-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T19	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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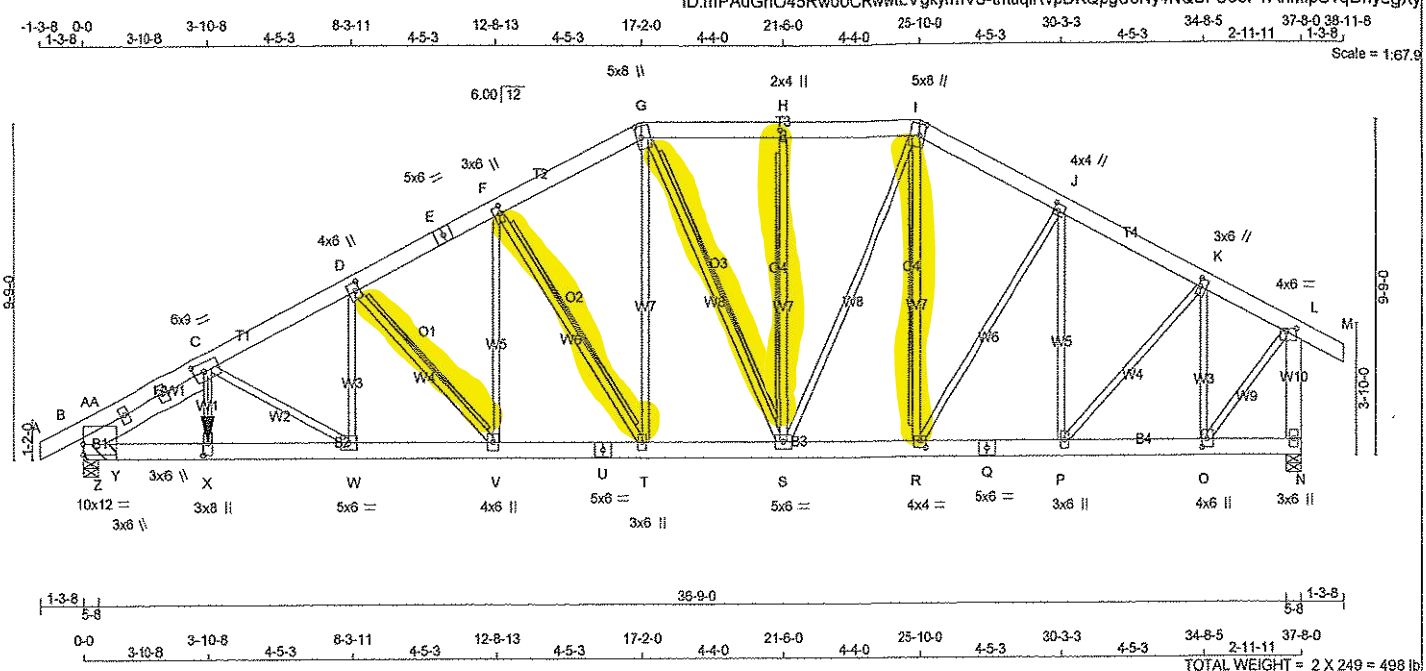

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - M	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
B - U	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	2	12	SIDE(129.4)
E- G	2	12	TOP
G- I	2	12	TOP
I- M	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- U	2	12	SIDE(30.7)
U- Q	2	12	TOP
Q- N	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
B	TPRW+t	MT20	3.0	6.0		
B	TPRW+t	MT20	3.0	6.0		
C	TMWWW-t	MT20	6.0	9.0	3.00	3.25
D	TMWW+t	MT20	4.0	6.0	3.00	1.50
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	3.0	6.0	2.75	0.75
G	TTWW+m	MT20	5.0	8.0	4.00	1.75
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TTWW+m	MT20	5.0	8.0	4.00	1.75
J	TMWW+t	MT20	4.0	4.0	2.50	1.75
K	TMWW+t	MT20	3.0	6.0	3.00	0.75
L	TMWW-p	MT20	4.0	6.0	1.00	3.75
N	BMV1+p	MT20	3.0	6.0		
O	BMWW+t	MT20	4.0	6.0	3.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	8280	0	8280	0	0	5-8	5-8 & BLOCK
N	3525	0	3525	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	6348	4355 / 0	992 / 0	0 / 0	0 / 0	1001 / 0	0 / 0
N	2719	1828 / 0	442 / 0	0 / 0	0 / 0	450 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N

2x6 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. B ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-V, F-T, G-S, H-S, I-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 0	-122.2	-122.2	0.04 (1)	10.00	X- C	0 / 4285	0.53 (1)
B-AA	-10296 / 0	-411.5	-411.5	0.55 (1)	3.25	C- W	-3631 / 0	0.80 (1)
AA- C	-6477 / 0	-411.5	-411.5	0.46 (1)	4.17	W- D	0 / 2202	0.27 (1)
C- D	-7871 / 0	-122.2	-122.2	0.27 (1)	4.11	D- V	-2691 / 0	0.54 (1)
D- E	-5907 / 0	-122.2	-122.2	0.15 (1)	4.76	V- F	0 / 2047	0.25 (1)
E- F	-5907 / 0	-122.2	-122.2	0.15 (1)	4.76	F- T	-2357 / 0	0.81 (1)
F- G	-4495 / 0	-122.2	-122.2	0.12 (1)	5.33	T- G	0 / 2122	0.26 (1)
G- H	-3757 / 0	-122.2	-122.2	0.09 (1)	5.73	G- S	-633 / 0	0.36 (1)
H- I	-3757 / 0	-122.2	-122.2	0.09 (1)	5.73	S- H	-600 / 0	0.27 (1)
I- J	-3557 / 0	-122.2	-122.2	0.10 (1)	5.85	S- I	0 / 1422	0.18 (1)
J- K	-3312 / 0	-122.2	-122.2	0.10 (1)	6.01	R- I	-150 / 99	0.07 (1)
K- L	-2170 / 0	-122.2	-122.2	0.05 (1)	6.25	R- J	0 / 343	0.04 (1)
L- M	0 / 38	-122.2	-122.2	0.05 (1)	10.00	P- J	-1006 / 0	0.45 (1)
N- L	-3496 / 0	0.0	0.0	0.24 (1)	7.48	P- K	0 / 1502	0.19 (1)
						O- K	-2082 / 0	0.40 (1)
B- Z	0 / 5830	-63.0	-63.0	0.48 (1)	10.00	O- L	0 / 2947	0.38 (1)
Z- Y	0 / 10294	-63.0	-63.0	0.89 (1)	10.00	Z-AA	0 / 4648	0.00 (1)
Y- X	0 / 10294	-63.0	-63.0	0.89 (1)	10.00	Z- C	-5874 / 0	0.50 (1)
X- W	0 / 10201	-27.5	-27.5	0.88 (1)	10.00			
W- V	0 / 7095	-27.5	-27.5	0.50 (1)	10.00			
V- U	0 / 5276	-27.5	-27.5	0.38 (1)	10.00			
U- T	0 / 5276	-27.5	-27.5	0.38 (1)	10.00			
T- S	0 / 4025	-27.5	-27.5	0.28 (1)	10.00			
S- R	0 / 3155	-27.5	-27.5	0.23 (1)	10.00			
R- Q	0 / 2972	-27.5	-27.5	0.22 (1)	10.00			
Q- P	0 / 2972	-27.5	-27.5	0.22 (1)	10.00			
P- O	0 / 1957	-27.5	-27.5	0.15 (1)	10.00			
O- N	0 / 0	-27.5	-27.5	0.02 (3)	10.00			

LICENSED PROFESSIONAL ENGINEER
9/12/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45992-17
STRUCTURAL
COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 7-2-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 100 % OF GSL. (DEFINED BY USER)
LOADS APPLIED TO FIRST 3-10-8 OF SPAN MEASURED FROM THE LEFT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.55 (B-AA:1) , BC=0.89 (X-Z:1) , WB=0.81 (F-T:1) , SSI=0.35 (B-AA:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.63 (B) (INPUT = 1.00)

Pb/b

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T19	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMWW+t	MT20	3.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.50	2.00
S	BMWWW-t	MT20	5.0	6.0		
T	BMWW+t	MT20	3.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	4.0	6.0		
W	BMWW-t	MT20	5.0	6.0		
X	BMW+w	MT20	3.0	8.0	4.25	1.50

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4548.7 lbs FACTORED DOWN AT 3-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	3-10-8	-4549	-4549	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45992-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T20	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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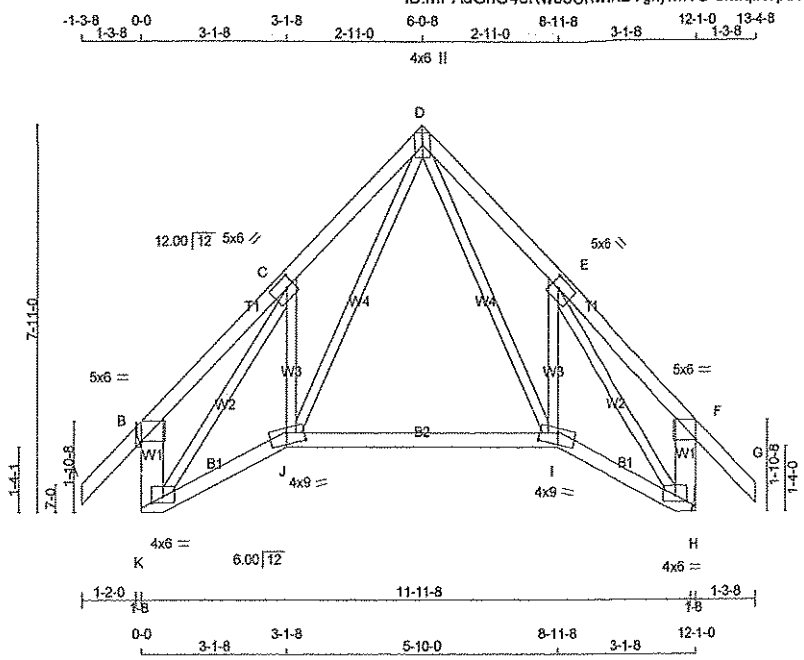
Scale: 1/4"=1'



TOTAL WEIGHT = 3 X 69 = 208 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV1-p	MT20	5.0	6.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	5.0	6.0	2.50	2.00
F	TMV-p	MT20	5.0	6.0		
H	BMVW1-p	MT20	4.0	6.0	Edge	
I	BBWW-m	MT20	4.0	9.0	2.75	4.50
J	BBWW-m	MT20	4.0	9.0	2.75	4.50
K	BMVW-p	MT20	4.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1075	0	1075	0	0	HANGER BY OTHERS (** SEE "BEARING NOTE" **) MIN. SEAT SIZE: 1-8	
H	1075	0	1075	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	819	569 / 0	121 / 0	0 / 0	0 / 0	129 / 0	0 / 0
H	819	569 / 0	121 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B- C	0 / 25	-122.2	-122.2	0.15 (1)	10.00
C- D	-958 / 0	-122.2	-122.2	0.13 (1)	6.21
D- E	-959 / 0	-122.2	-122.2	0.13 (1)	6.21
E- F	0 / 25	-122.2	-122.2	0.15 (1)	10.00
F- G	0 / 60	-122.2	-122.2	0.17 (1)	10.00
K- B	0 / 754	0.0	0.0	0.09 (1)	10.00
H- F	-320 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 739	-27.5	-27.5	0.16 (1)	10.00
J- I	0 / 420	-27.5	-27.5	0.31 (2)	10.00
I- H	0 / 739	-27.5	-27.5	0.16 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 565	0.13 (1)
I- E	-60 / 107	0.02 (3)
J- D	0 / 565	0.13 (1)
J- C	-59 / 107	0.02 (3)
K- C	-1233 / 0	0.59 (1)
E- H	-1234 / 0	0.59 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 955 (0.15")

CSI: TC=0.17 (A-B:1) , BC=0.31 (I-J:2) , WB=0.59 (E-H:1) , SSI=0.12 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45993-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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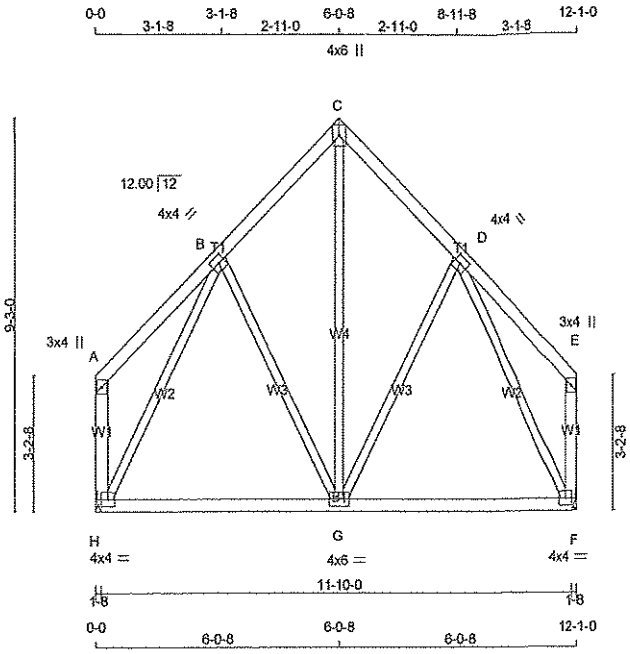

TOTAL WEIGHT = 2 X 68 = 135 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTW+p	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	905	0	905	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	905	0	905	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	704	462 / 0	121 / 0	0 / 0	0 / 0	121 / 0	0 / 0
F	704	462 / 0	121 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-122.2	-122.2	0.18 (1)	10.00	
B- C	-483 / 0	-122.2	-122.2	0.14 (1)	6.25	
C- D	-483 / 0	-122.2	-122.2	0.14 (1)	6.25	
D- E	0 / 29	-122.2	-122.2	0.18 (1)	10.00	
H- A	-145 / 0	0.0	0.0	0.03 (1)	7.81	
F- E	-145 / 0	0.0	0.0	0.03 (1)	7.81	
H- G	0 / 360	-27.5	-27.5	0.32 (2)	10.00	
G- F	0 / 360	-27.5	-27.5	0.32 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 360	0.08 (1)
G- D	-87 / 58	0.07 (1)
B- G	-87 / 58	0.07 (1)
H- B	-783 / 0	0.60 (1)
D- F	-783 / 0	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.18 (A-B:1) , BC=0.32 (F-G:2) , WB=0.60 (B-H:1) , SSI=0.14 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9171
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45994-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T22	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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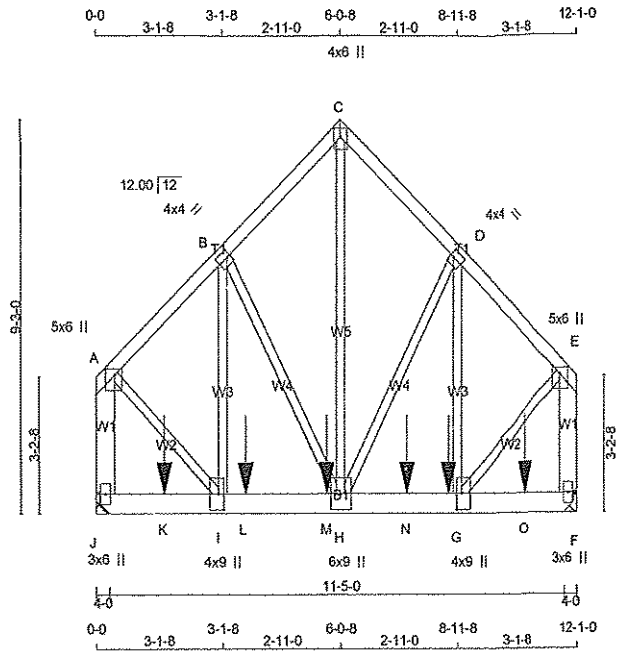

TOTAL WEIGHT = 3 X 84 = 252 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
J- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	5	SIDE(2007.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TTW+p	MT20	4.0	6.0	2.50	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.00
E	TMVW+p	MT20	5.0	6.0	1.75	2.75
F	BMV1+p	MT20	3.0	6.0		
G	BMWW+t	MT20	4.0	9.0	4.50	1.50
H	BMWWW+t	MT20	6.0	9.0		
I	BMWW+t	MT20	4.0	9.0	4.50	1.50
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	8130	0	8130	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
F	9062	0	9062	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-5	

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	6320	4180 / 0	1086 / 0	0 / 0	0 / 0	1054 / 0	0 / 0
F	7044	4659 / 0	1210 / 0	0 / 0	0 / 0	1174 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-5276 / 0	-122.2	-122.2	0.10 (1)	4.96	
B- C	-4679 / 0	-122.2	-122.2	0.08 (1)	5.22	
C- D	-4678 / 0	-122.2	-122.2	0.08 (1)	5.22	
D- E	-5720 / 0	-122.2	-122.2	0.11 (1)	4.80	
J- A	-7232 / 0	0.0	0.0	0.28 (1)	6.64	
F- E	-7824 / 0	0.0	0.0	0.30 (1)	6.44	
J- K	0 / 0	-27.5	-27.5	0.33 (1)	10.00	
K- I	0 / 0	-27.5	-27.5	0.33 (1)	10.00	
I- L	0 / 3737	-27.5	-27.5	0.37 (1)	10.00	
L- M	0 / 3737	-27.5	-27.5	0.37 (1)	10.00	
M- H	0 / 3737	-27.5	-27.5	0.37 (1)	10.00	
H- N	0 / 4049	-27.5	-27.5	0.39 (1)	10.00	
N- G	0 / 4049	-27.5	-27.5	0.39 (1)	10.00	
G- O	0 / 0	-27.5	-27.5	0.37 (1)	10.00	
O- F	0 / 0	-27.5	-27.5	0.37 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 6271	0.47 (1)
H- D	-1686 / 0	0.44 (1)
G- D	0 / 1866	0.14 (1)
B- H	-980 / 0	0.25 (1)
I- B	0 / 920	0.07 (1)
A- I	0 / 4996	0.37 (1)
G- E	0 / 5414	0.41 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	8-8-12	-2563	-2563	---	BACK	VERT	TOTAL
K	1-8-4	-2563	-2563	---	BACK	VERT	TOTAL
L	3-8-4	-2563	-2563	---	BACK	VERT	TOTAL
M	5-8-4	-2563	-2563	---	BACK	VERT	TOTAL
N	7-8-4	-2563	-2563	---	BACK	VERT	TOTAL
O	10-8-12	-2567	-2567	---	BACK	VERT	TOTAL

DWG NO. TAM 45995-17
STRUCTURAL
COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.30 (E-F:1) , BC=0.39 (G-H:1) , WB=0.47 (C-H:1) , SSI=0.55 (H-I:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.60)
JSI METAL= 0.44 (C) (INPUT = 0.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

p6/12

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	T22	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2563.2 lbs FACTORED DOWN AT 1-8-4, 2563.2 lbs FACTORED DOWN AT 3-8-4, 2563.2 lbs FACTORED DOWN AT 5-8-4, 2563.2 lbs FACTORED DOWN AT 7-8-4, AND 2563.2 lbs FACTORED DOWN AT 8-8-12, AND 2567.0 lbs FACTORED DOWN AT 10-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9127
S. KATSOULAKOS
PROVINCE OF ONTARIO

P6 2/2

DWG NO. TAM 45995-17
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42067	DRWG NO.
273415	P1	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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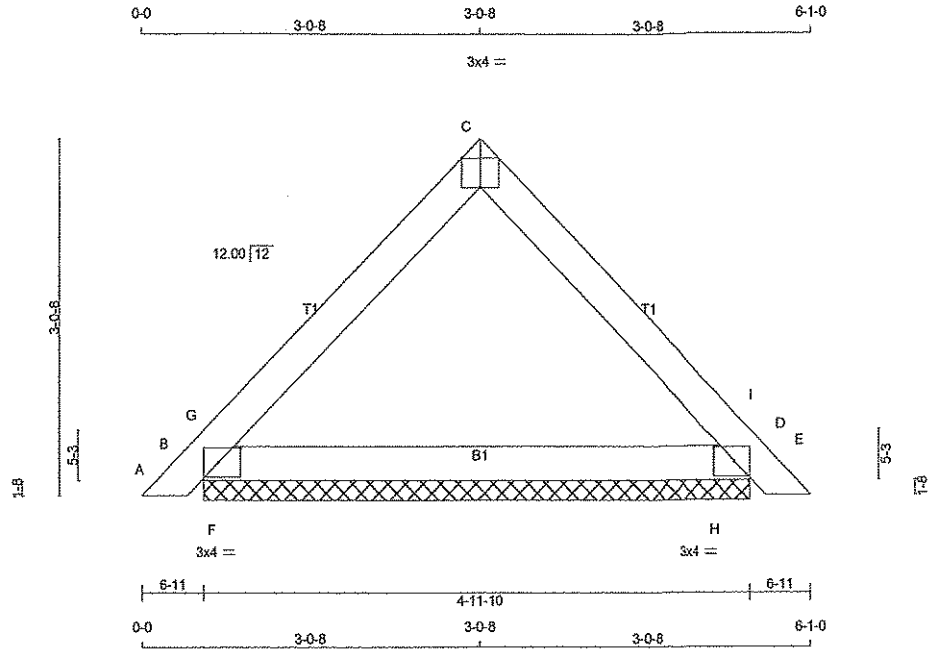

TOTAL WEIGHT = 3 X 15 = 46 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	427	0	427	0	0	4-11-10	4-11-10
D	427	0	427	0	0	4-11-10	4-11-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	327	225 / 0	50 / 0	0 / 0	0 / 0	52 / 0	0 / 0
D	327	225 / 0	50 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-122.2	-122.2	0.02 (1)		10.00
B- G	-376 / 0	-122.2	-122.2	0.09 (3)		6.25
G- C	-206 / 0	-122.2	-122.2	0.13 (1)		6.25
C- I	-206 / 0	-122.2	-122.2	0.13 (1)		6.25
I- D	-376 / 0	-122.2	-122.2	0.09 (3)		6.25
D- E	0 / 18	-122.2	-122.2	0.02 (1)		10.00
B- F	0 / 152	-27.5	-27.5	0.07 (3)		10.00
F- H	0 / 152	-27.5	-27.5	0.13 (2)		10.00
H- D	0 / 152	-27.5	-27.5	0.07 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 342	0.00 (1)
H- I	0 / 342	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13 (C-G:1) , BC=0.13 (F-H:2) , WB=0.00 (F-G:1) , SSI=0.18 (D-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45980 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	P2	18	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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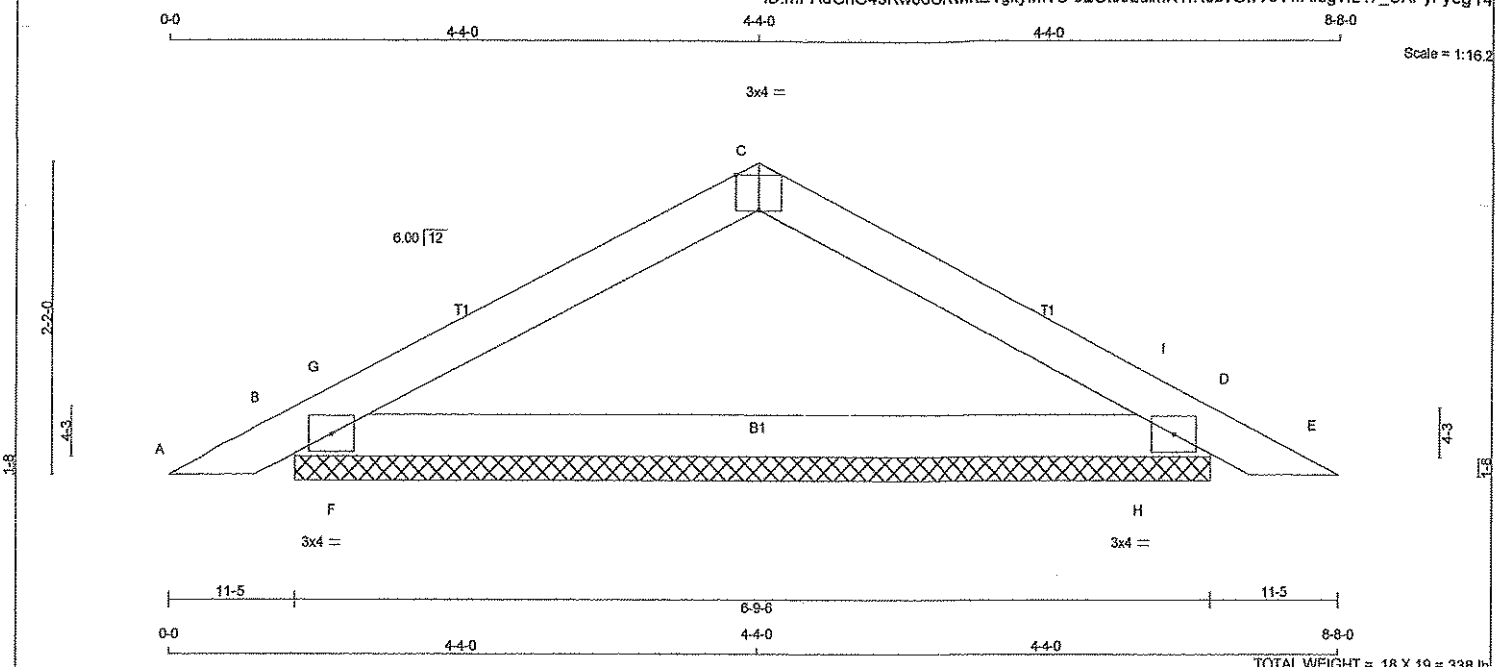

TOTAL WEIGHT = 18 X 19 = 338 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	615	0	615	0	0	6-9-6	6-9-6
D	615	0	615	0	0	6-9-6	6-9-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	467	326 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	467	326 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-122.2	-122.2	0.07 (1)		10.00
B- G	-609 / 0	-122.2	-122.2	0.19 (2)		6.25
G- C	-462 / 0	-122.2	-122.2	0.26 (1)		6.25
C- I	-462 / 0	-122.2	-122.2	0.26 (1)		6.25
I- D	-609 / 0	-122.2	-122.2	0.19 (2)		6.25
D- E	0 / 22	-122.2	-122.2	0.07 (1)		10.00
B- F	0 / 425	-27.5	-27.5	0.14 (3)		10.00
F- H	0 / 425	-27.5	-27.5	0.24 (1)		10.00
H- D	0 / 425	-27.5	-27.5	0.14 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 360	0.00 (1)
H- I	0 / 360	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26 (C-I:1) , BC=0.24 (F-H:1) , WB=0.00 (F-G:1) , SSI=0.23 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (B) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45998-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	P2Z	2	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 15:37:29 2017 Page 1

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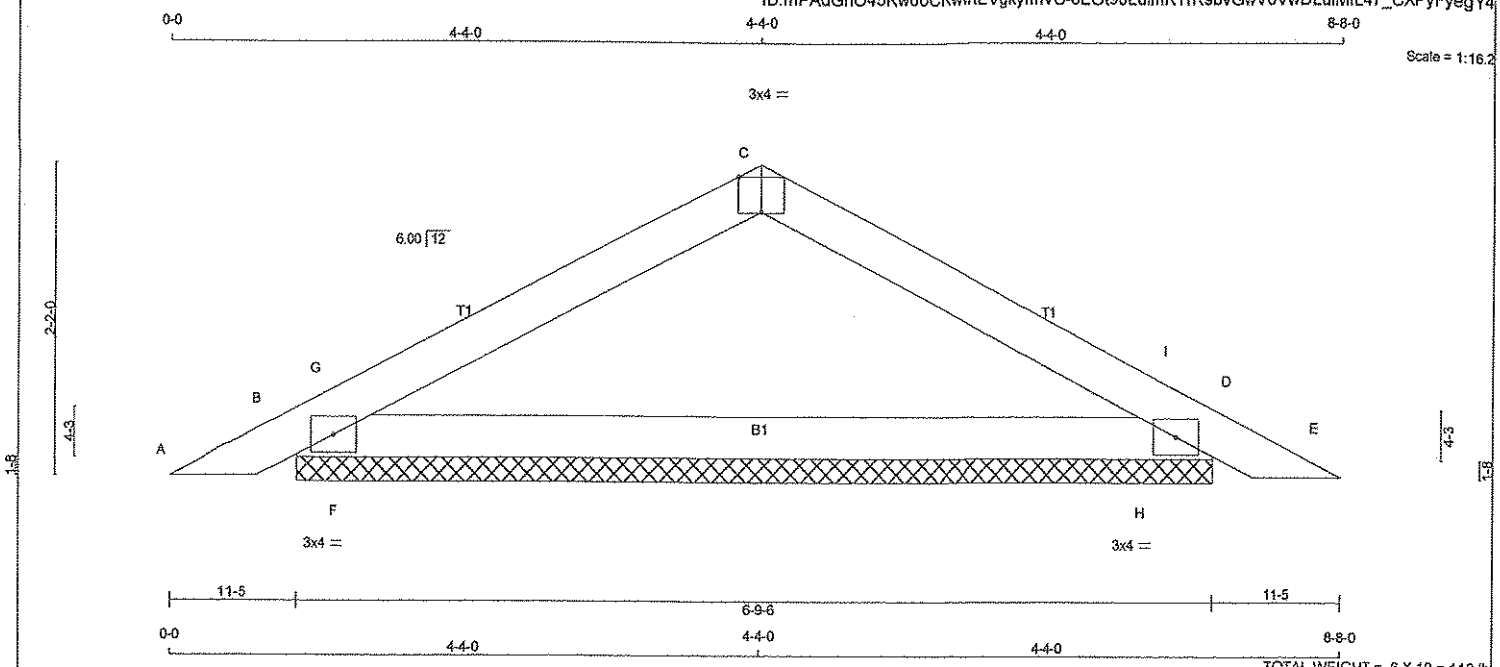

TOTAL WEIGHT = 6 X 19 = 113 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	615	0	615	0	0	6-9-6	6-9-6
D	615	0	615	0	0	6-9-6	6-9-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	467	326 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	467	326 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-122.2	-122.2	0.02 (1)	10.00	
B- G	-608 / 0	-122.2	-122.2	0.06 (2)	6.25	
G- C	-462 / 0	-122.2	-122.2	0.08 (1)	6.25	
C- I	-462 / 0	-122.2	-122.2	0.08 (1)	6.25	
I- D	-608 / 0	-122.2	-122.2	0.06 (2)	6.25	
D- E	0 / 22	-122.2	-122.2	0.02 (1)	10.00	
B- F	0 / 425	-27.5	-27.5	0.04 (3)	10.00	
F- H	0 / 425	-27.5	-27.5	0.08 (1)	10.00	
H- D	0 / 425	-27.5	-27.5	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
F- G	0 / 359	0.00 (1)
H- I	0 / 359	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08 (C-I:1) , BC=0.08 (F-H:1) , WB=0.00 (F-G:1) , SSI=0.08 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45999-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 273416	DRWG NO.
273416	P2Z1	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 15:37:29 2017 Page 1
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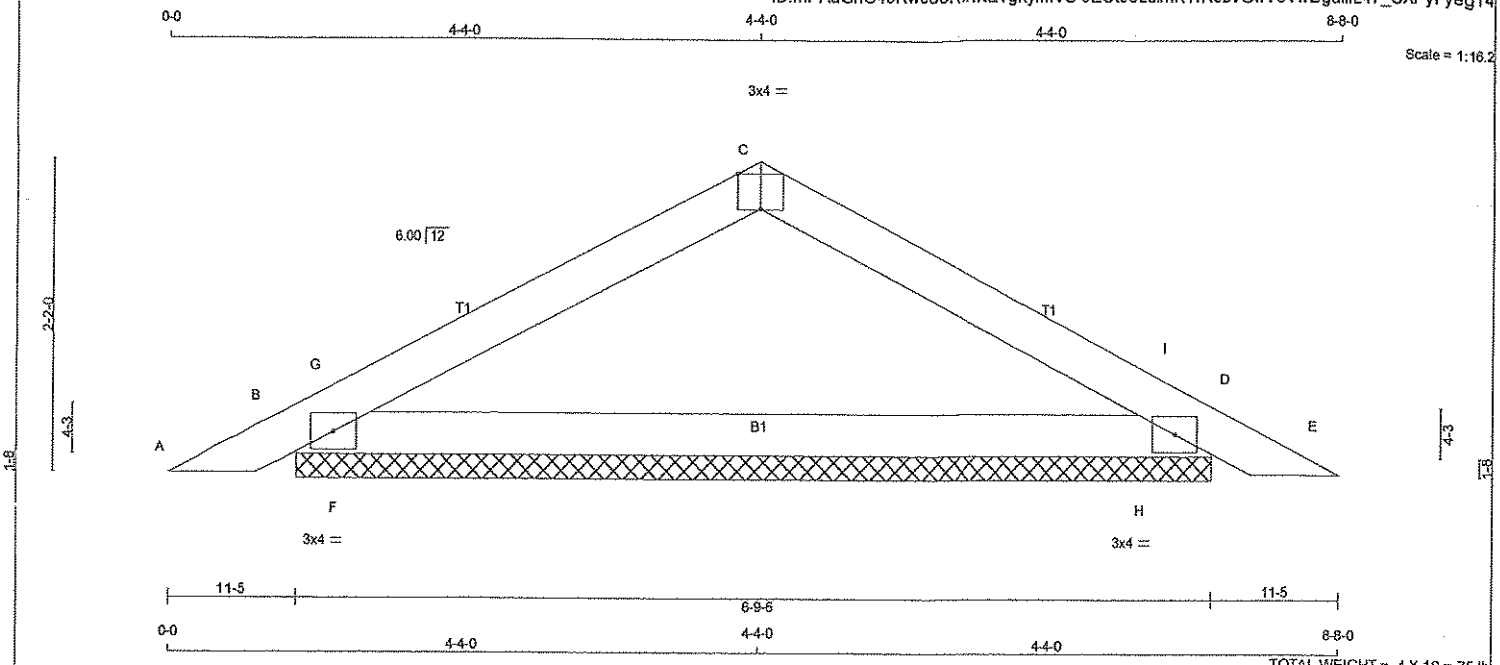


TOTAL WEIGHT = 4 X 19 = 75 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	615	0	615	0	0	6-9-6	6-9-6
D	615	0	615	0	0	6-9-6	6-9-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	467	326 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	467	326 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-122.2	-122.2	0.03 (1)	10.00
B- G	-609 / 0	-122.2	-122.2	0.09 (2)	6.25
G- C	-462 / 0	-122.2	-122.2	0.13 (1)	6.25
C- I	-462 / 0	-122.2	-122.2	0.13 (1)	6.25
I- D	-609 / 0	-122.2	-122.2	0.09 (2)	6.25
D- E	0 / 22	-122.2	-122.2	0.03 (1)	10.00
B- F	0 / 425	-27.5	-27.5	0.07 (3)	10.00
F- H	0 / 425	-27.5	-27.5	0.12 (1)	10.00
H- D	0 / 425	-27.5	-27.5	0.07 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 359	0.00 (1)
H- I	0 / 359	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.2	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13 (C-I:1) , BC=0.12 (F-H:1) , WB=0.00 (F-G:1) , SSI=0.11 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 46000-17
STRUCTURAL
COMPONENT ONLY

Boise Cascade

Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 2800 DF

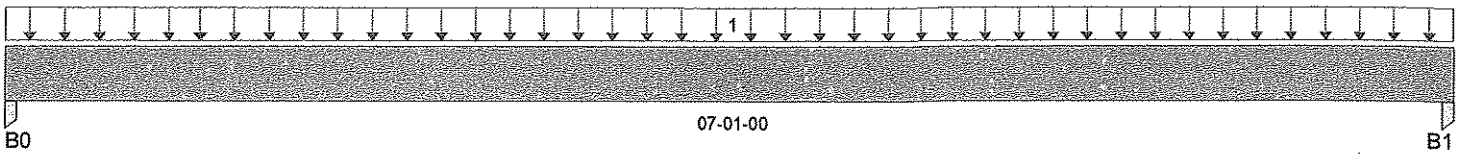
Floor Beam\FB01

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 11, 2017 13:37:54

Build 6080		File Name:	273415
Job Name:	BAYVIEW WELLINGTON/ INNISFIL	Description:	Designs\FB01
Address:	S48-1 EL:B	Specifier:	
City, Province, Postal Code:,		Designer:	ELLA
Customer:		Company:	
Code reports:	CCMC 12472-R	Misc:	



Total Horizontal Product Length = 07-01-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	800 / 0	792 / 0	4,135 / 0	
B1, 3-1/2"	800 / 0	792 / 0	4,135 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1		Unf. Area (lb/ft^2)	L	00-00-00	07-01-00	11	10	54		21-06-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	11,760 ft-lbs	20,970 ft-lbs	56.1%	5	03-06-08
End Shear	5,269 lbs	11,571 lbs	45.5%	5	01-01-00
Total Load Defl.	L/610 (0.13")	0.331"	39.3%	13	03-06-08
Live Load Defl.	L/999 (0.111")	n/a	n/a	17	03-06-08
Max Defl.	0.13"	1"	13%	13	03-06-08
Span / Depth	8.4	n/a	n/a		00-00-00
Squash Blocks	Valid				

Bearing Supports		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Post	3-1/2" x 3-1/2"	7,591 lbs	n/a	50.8%	Unspecified
B1	Post	3-1/2" x 3-1/2"	7,591 lbs	n/a	50.8%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Design meets User specified (1") Maximum Total load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.
Design based on Dry Service Condition.
Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 46001-17
STRUCTURAL
COMPONENT ONLY

Boise Cascade

Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 2800 DF

Floor Beam\FB01

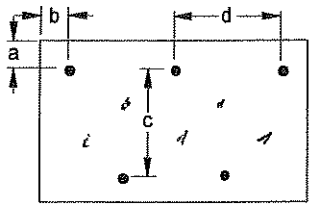
Dry | 1 span | No cantilevers | 0/12 slope (deg)

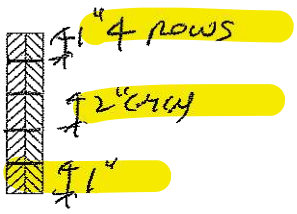
September 11, 2017 13:37:54

BC CALC® Design Report

Build 6080		File Name:	273415
Job Name:	BAYVIEW WELLINGTON/ INNISFIL	Description:	Designs\FB01
Address:	S48-1 EL:B	Specifier:	
City, Province, Postal Code:	,	Designer:	ELLA
Customer:		Company:	
Code reports:	CCMC 12472-R	Misc:	

Connection Diagram





a minimum = [redacted]' c = [redacted]-1/2"
b minimum = 3" d = [redacted] 4"

Member has no side loads.
Connectors are: 16d [redacted] Nails

3½" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

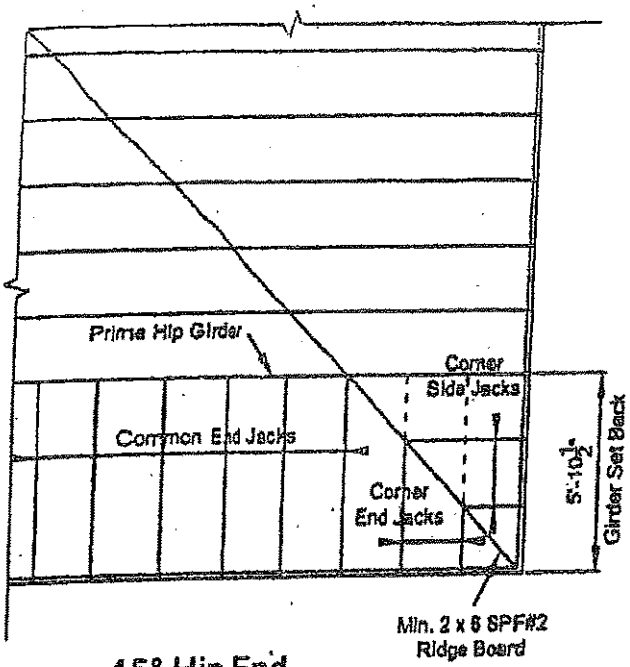
BC CALC®, BC FRAMER® , AJS™, ALLJOIST® , BC RIM BOARD™, BCI® , BOISE GLULAM™, SIMPLE FRAMING SYSTEM® , VERSA-LAM®, VERSA-RIM PLUS® , VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

LICENSED PROFESSIONAL ENGINEER
9127
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 46001-17
STRUCTURAL
COMPONENT ONLY

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

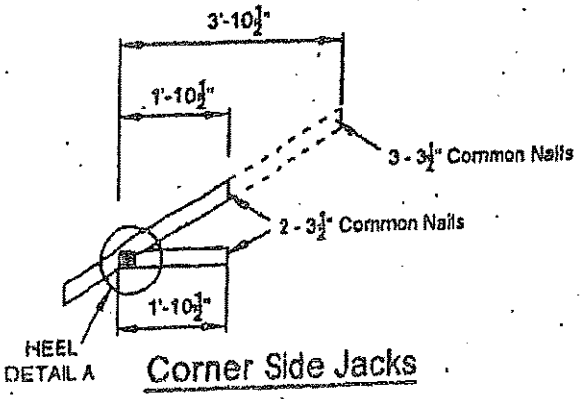
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

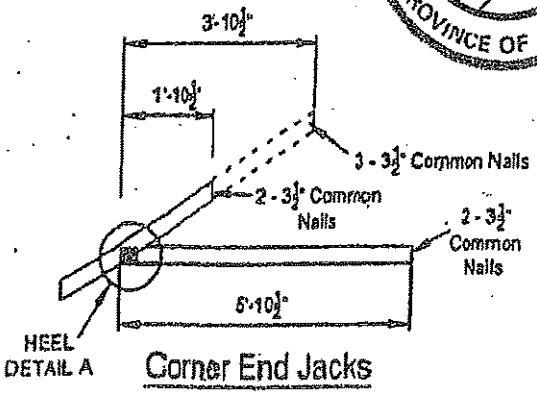
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14
STRUCTURAL COMPONENT ONLY

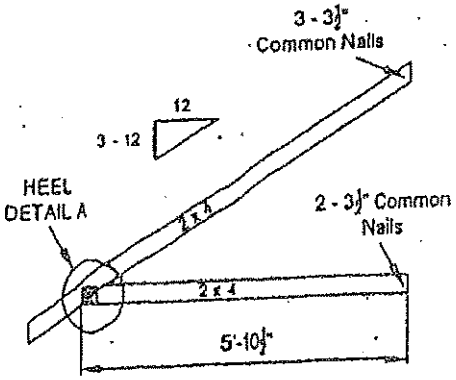
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



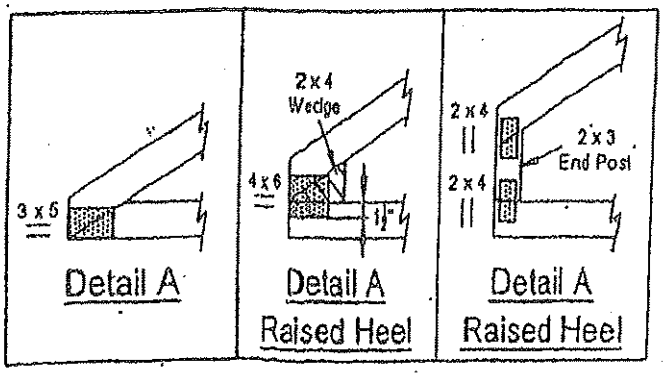
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

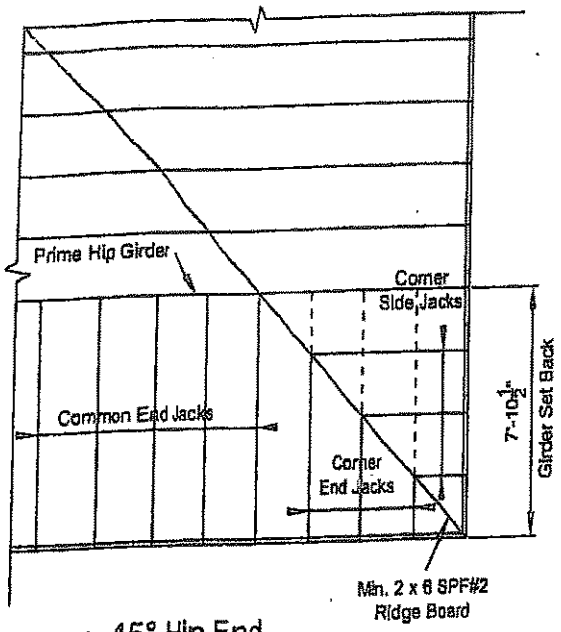
MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



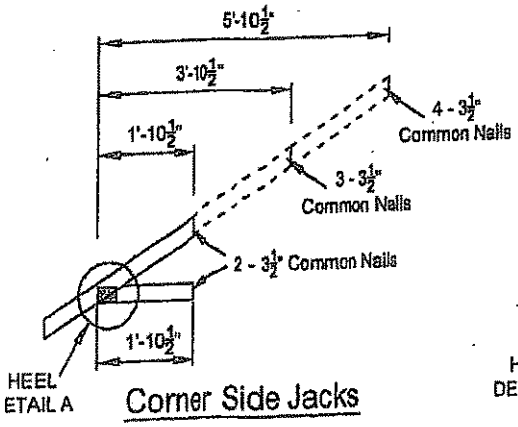
45° Hip End

DESIGN LOAD:

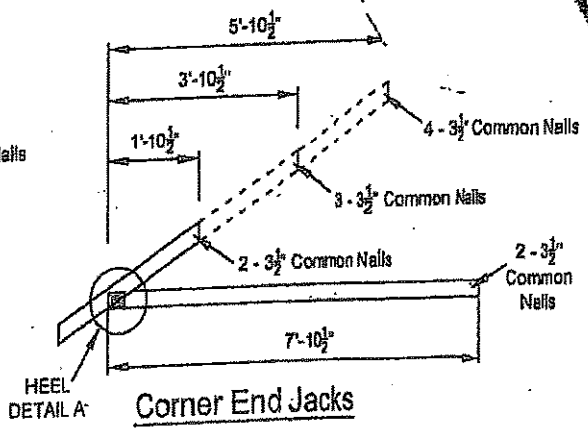
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3503 14
STRUCTURAL
COMPONENT ONLY

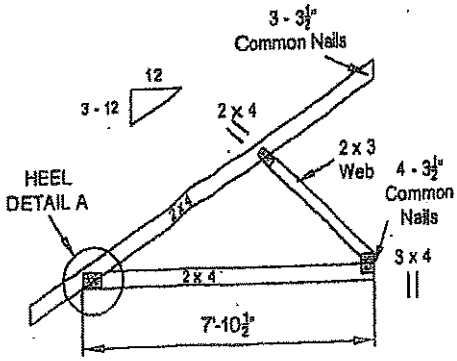
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



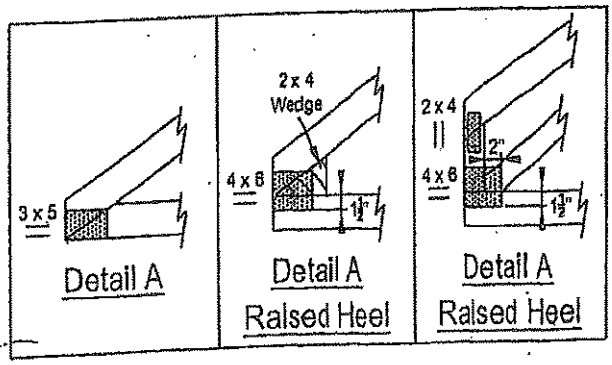
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

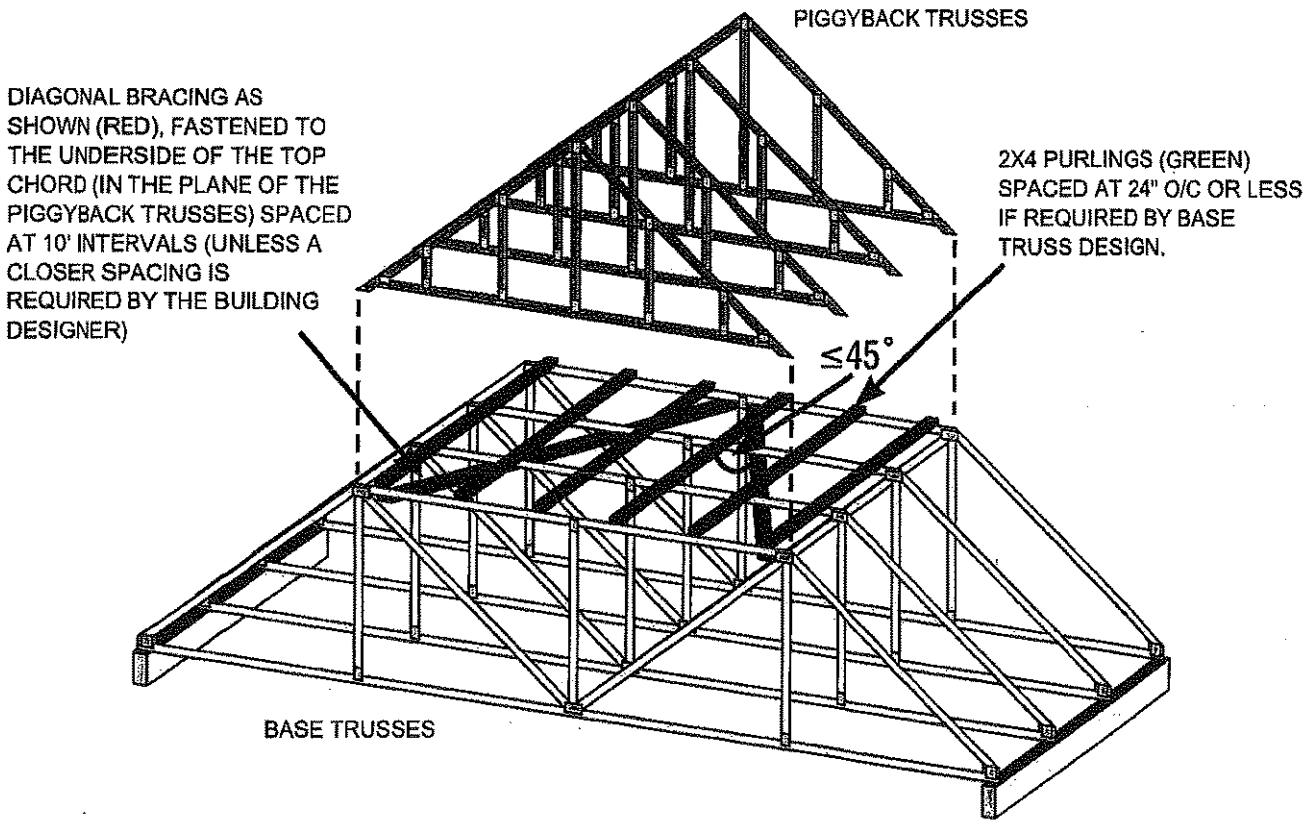
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

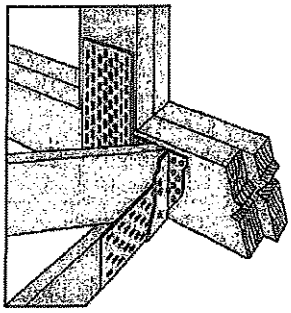
FACE MOUNT HANGERS

SIMPSON Strong-Tie®

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

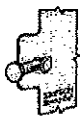
These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Model No.	Ga	Dimensions (in) W	H	B	d_B[3]	Fasteners Header	Joist	Factored Resistence D.Fir-L Uplift (K_D = 1.15) lbs / kN	D.Fir-L Normal (K_D = 1.00) lbs / kN	S-P-F Uplift (K_D = 1.15) lbs / kN	S-P-F Normal (K_D = 1.00) lbs / kN
DOUBLE 2x SIZES											
LUS24-2	18	3⅛	3⅛	2	1½	4-16d	2-16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS26-2	14	3 9/16	5⅜	3	3 15/16	14-16d	6-16d	2850 / 12.68	7335 / 32.63	2085 / 9.20	5205 / 23.15
HGUS26-2	12	3 9/16	5 7/16	4	4⅛	20-16d	8-16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS28-2	14	3 9/16	7 5/16	3	6⅛	22-16d	8-16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3 9/16	7 3/16	4	6⅛	36-16d	12-16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HHUS210-2	14	3 9/16	9 1/16	3	8	30-16d	10-16d	4745 / 21.11	9660 / 42.97	4310 / 19.17	7000 / 31.14
HGUS210-2	12	3 9/16	9 9/16	4	8⅛	46-16d	16-16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
TRIPLE 2x SIZES											
HGUS26-3	12	4 5/16	5⅛	4	4⅛	20-16d	8-16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4 5/16	7¼	4	6⅛	36-16d	12-16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-3	14	4 11/16	9	3	7 15/16	30-16d	10-16d	4745 / 21.11	10545 / 46.91	4310 / 19.17	7485 / 33.29
HGUS210-3	12	4 11/16	9¼	4	8⅛	46-16d	16-16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
QUADRUPLE 2x SIZES											
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	20-16d	8-16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6 5/16	7 5/16	4	6⅛	36-16d	12-16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-4	14	6⅛	8⅞	3	7 13/16	30-16d	10-16d	4745 / 21.11	10545 / 46.91	4310 / 19.17	7485 / 33.29
HGUS210-4	12	6 9/16	9 9/16	4	8⅛	46-16d	16-16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6 9/16	10⅝	4	10⅛	56-16d	20-16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6 9/16	12⅝	4	11⅛	66-16d	22-16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x SIZES											
LUS46	18	3 9/16	4¾	2	3 7/16	4-16d	4-16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS46	14	3⅝	5¼	3	3 15/16	14-16d	6-16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4 1/16	20-16d	8-16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS48	18	3 9/16	6¾	2	3 7/16	6-16d	4-16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	22-16d	8-16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7 1/16	4	6 1/16	36-16d	12-16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS410	18	3 9/16	8¾	2	5⅝	8-16d	6-16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8 1/16	46-16d	16-16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS412	12	3⅝	10 7/16	4	10 1/16	56-16d	20-16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12 1/16	4	11 1/16	66-16d	22-16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

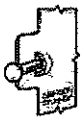


Typical HUS26 Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Double Shear Nailing Top View



Double Shear Nailing Side View Do not bend tab back



Dome Double Shear Nailing prevents tabs breaking off *(available on some models)*

U.S. Patent 5,603,580

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_B is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-N10FORTRUSS and/or see installation notes.
5. NAILS: 16d = 0.162" dia. x 3½" long. See pages 22-23 for other nail sizes and information.

LICENSED PROFESSIONAL ENGINEER
1/21/15
G. V. LUBARSKY
100044463
PROVINCE OF ONTARIO

Catalogue C-C-CAH2015 ©2015 SIMPSON STRONG-TIE COMPANY INC.

LUL/LUS/LJS/HUS/HHUS/HGUS *Standard & Double Shear Joist Hangers*

SIMPSON Strong-Tie®

VALUE ENGINEERED

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double shear nailing – an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. *(Do not bend or remove tabs)*

Double shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

MATERIAL: See tables below and on page 155

FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

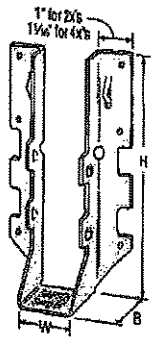
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances *(except LUL)*.
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With 3x carrying members, use 16dx2½" nails into the header and 16d commons into the joist with no reduction in resistance. With 2x carrying members, use 10dx1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value.

OPTIONS: • LUS and LUL hangers cannot be modified.

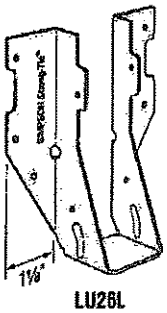
- HUS hangers available with the header flanges turned in for 2-2x (3⅛") and 4x only, with no load reduction. See HUSC Concealed Flange illustration.
- Concealed flanges are not available for HGUS.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See hanger options on page 230.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

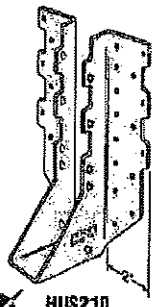
These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.



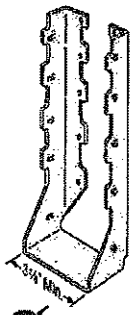
LUS28



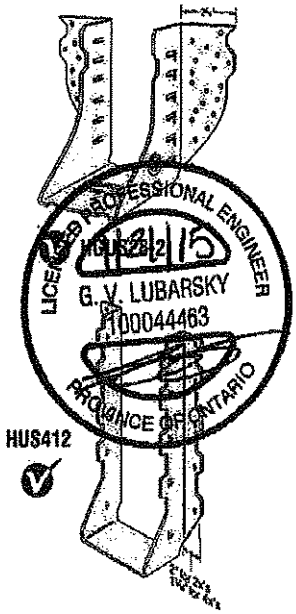
LU26L



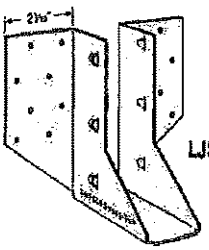
HUS210
(HUS26, HUS28, and HHUS similar)



HUSC
Concealed Flanges
(not available for HHUS, HGUS and HUS2x)



HUS412



LJS26DS

LICENSED PROFESSIONAL ENGINEER
1-31-15
G. V. LUBARSKY
100044463
PROVINCE OF ONTARIO

Model No.	Ga	Dimensions (in) W	H	B	d_B[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lbs / kN	D.Fir-L Normal (K_D = 1.00) lbs / kN	S-P-F Uplift (K_D = 1.15) lbs / kN	S-P-F Normal (K_D = 1.00) lbs / kN
SINGLE 2x SIZES											
LUS24	18	1⅜	3⅛	1¾	2¼	4-10d	2-10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅜	2 11/16	4-10d	2-10dx1½	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	6-10d	4-10dx1½	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¾	1¾	3¾	4-10d	4-10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5¾	3	3 15/16	14-16d	6-16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	16-16d	6-16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1¾	5¾	5	4¼	20-16d	8-16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5¾	8-10d	6-10dx1½	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	6-10d	4-10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7 11/16	3	6 1/16	22-16d	8-16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1¾	7⅛	5	6¼	36-16d	12-16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	10-10d	6-10dx1½	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7 13/16	1¾	3⅝	8-10d	4-10d	1420 / 6.32	2765 / 12.39	1290 / 5.74	2210 / 9.83

See footnotes on page 155.

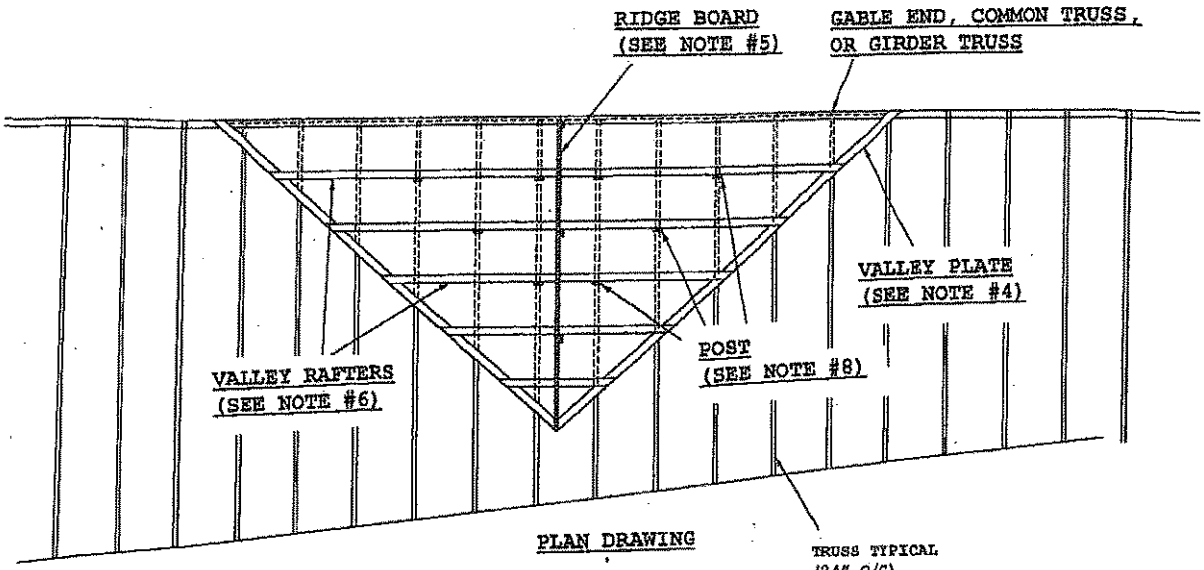
Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.

MICRO CITY
ENGINEERING SERVICES INC.

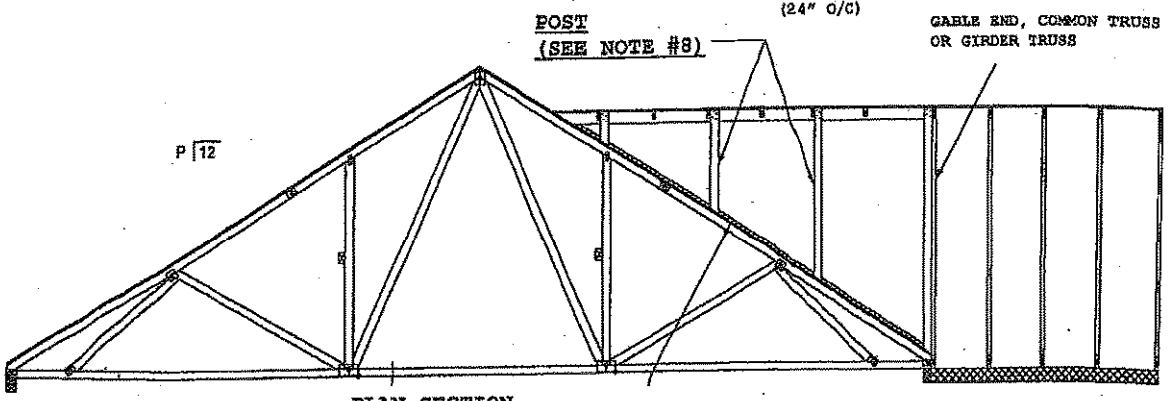
TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

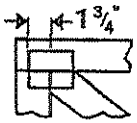
(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-74
S. KATSOULAKOS
PROVINCE OF ONTARIO

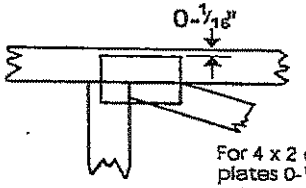
DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

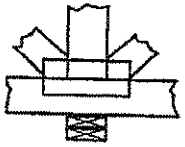
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

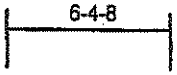
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

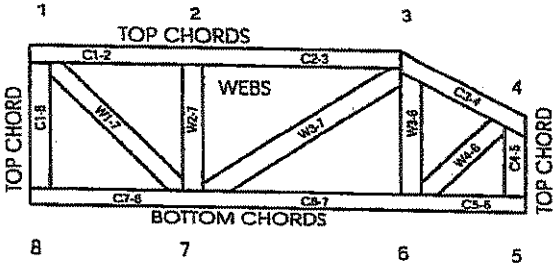
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
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N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2079
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO [illegible] 4
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.