

ASPHALT SHINGLES  
12" FINISH O.H.  
R.T.M.C.  
2x6 EXTERIOR WALLS  
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PARTS OF THE OBC, LATEST EDITION  
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.  
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.  
POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance  
between end points and between rows of bracing does not exceed 6'.

DESIGN LOADS:  
SOFFIT 38.3 psf  
SNOW LOAD 3 psf  
TC DEAD 3 psf  
BC LIVE 10.5 psf  
BC DEAD 7 psf

T-170704  
DENOTES:  
CONVENTIONAL  
FRAMING



24-01-00  
6-03-00 6-05-00 5-00-00 6-05-00

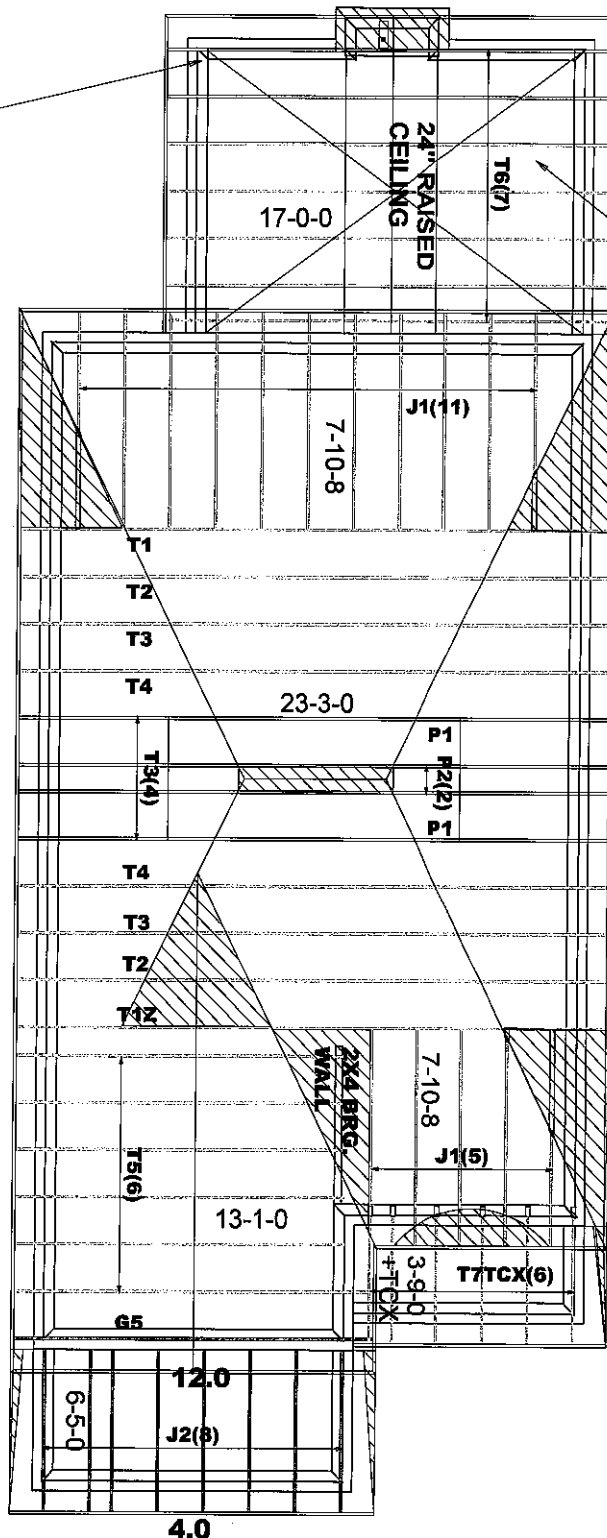
<b>TAMARACK</b> <small>CONVENTIONAL ROOF TRUSSES</small>		Job Track: <b>42067</b> Layout ID: <b>272800</b> Plan Log: <b>88256</b>
Builder / Location: <b>BAYVIEW WELLINGTON</b> Project: <b>ALCONA SHORES</b> Date: 10/16/2017 Designer: S.V. / colinhjg		Model / Elevation: <b>S30-1 / EL:A -STD.</b> THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC. AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC. IF UTILIZED FOR ANY OTHER PURPOSE. Mtek ver. 7.5.0

PLATE HEIGHT 9'-1"-0  
U/S OF SOFFIT @ PLATE LEVEL

12-06-00

12.0  
43-02-00

6-00-00



4/12 VAULT  
CEILING

55-08-00

6/12 PITCHES (TYP.)  
UNLESS NOTED

4-02-00

12-06-00

54-06-00  
37-10-00  
12.0

5-04-00

6-00-00

1-02-00

Town of Innisfil Certified Model  
06/03/2018 2:54:03 PM kgervais



ASPHALT SHINGLES  
12" FINISH O.H.  
R.T.M.C  
2x6 EXTERIOR WALLS  
2x6 FASCIA BOARD

HARDWARE:

LUS24 -10)

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PARTS OF THE OBC.LATEST EDITION  
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.  
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.  
POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance  
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:  
SOFFIT  
SNOW LOAD 38.3 psf  
TC DEAD 3 PSF  
BC LIVE 10.5 PSF  
BC DEAD 7 PSF

CONVENTIONAL  
FRAMING



T-170704

24-01-00  
6-03-00 6-05-00 5-00-00 6-05-00

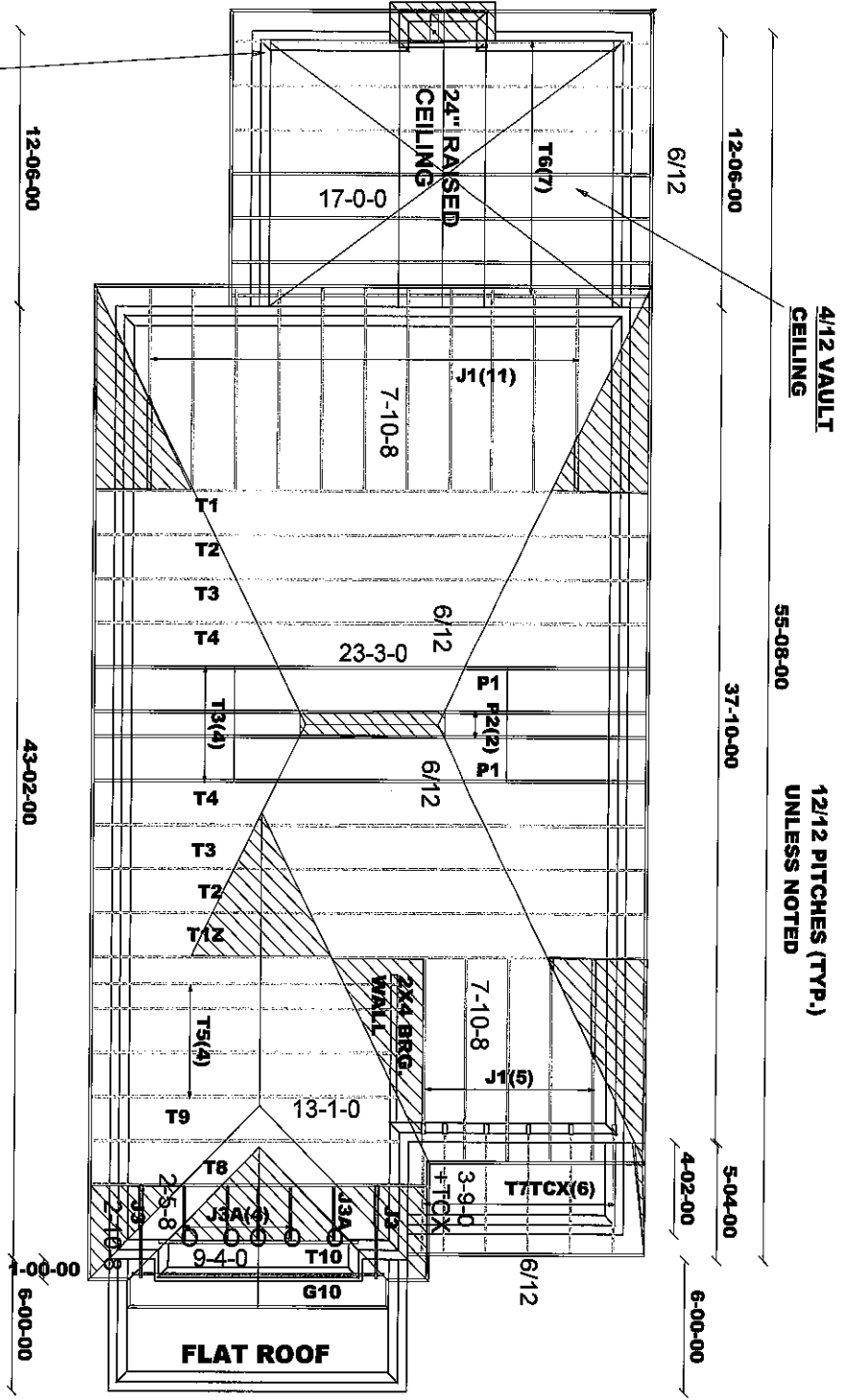


PLATE HEIGHT 9'-1"-0  
U/S OF SOFFIT @ PLATE LEVEL

Job Track: 42067  
Layout ID: 272801  
Plan Log: 88256

Builder / Location: BAYVIEW WELLINGTON  
Project: ALCONA SHORES  
Date: 10/16/2017 Designer: S.V / col/nh/g

INNISFIL

Model / Elevation: S30-1 / EL:B - STD.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milltek ver 7.5.0

Town of Innisfil Certified Model  
06/03/2018 2:54:37 PM kgervais

ASPHALT SHINGLES  
12" FINISH O.H.  
R.T.M.C  
2X6 EXTERIOR WALLS  
2X6 FASCIA BOARD

HARDWARE:

L/S26DS -1V)  
HGS26-2(XX)

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION  
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2X4@SPF@24"o.c.  
WITH A 2X4@SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.  
POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance  
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

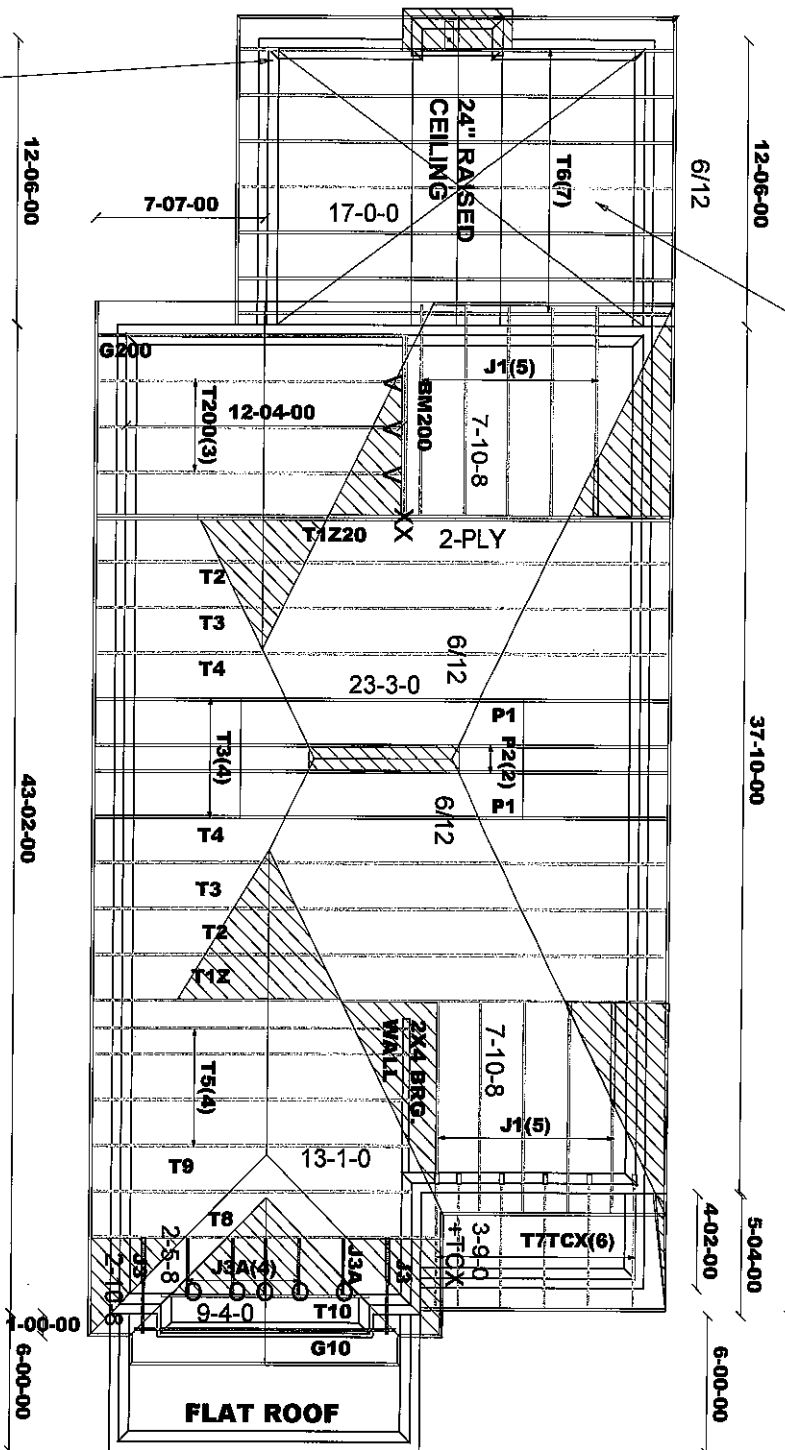
DESIGN LOADS:  
SOFFIT  
SNOW LOAD 38.3 psf  
TC DEAD 3 psf  
EC LIVE 10.5 psf  
EC DEAD 7 psf

NOTES:  
CONVENTIONAL  
FRAMING



T-170704

24-01-00  
6-03-00 6-05-00 5-00-00 6-05-00



4/12 VAULT  
CEILING

55-08-00

12/12 PITCHES (TYP.)  
UNLESS NOTED

12-06-00

37-10-00

5-04-00

4-02-00

6-00-00

12-06-00

43-02-00

6-00-00

2-01-08 9-08-00 2-01-08 10-02-00

PLATE HEIGHT 9'-1"-0  
U/S OF SOFFIT @ PLATE LEVEL

FLAT ROOF

Town of Innisfil Certified Model  
06/03/2018 2:54:46 PM kgervais

<b>TAMARACK</b> <small>CONVENTIONAL ROOF TRUSSES INC.</small>		Job Track: <b>42067</b> Layout ID: <b>289598</b> Plan Log: <b>88256</b>
Builder / Location: <b>BAYVIEW WELLINGTON / INNISFIL</b> Project: <b>ALCONA SHORES</b> Date: <b>10/17/2017</b>		Model / Elevation: <b>S30-1 / EL:B - REAR UPGRADE</b> THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Mitek ver 7.5.0

ASPHALT SHINGLES  
12" FINISH O.H.  
R.T.M.C  
2x6 EXTERIOR WALLS  
2x6 FASCIA BOARD

4/12 VAULT  
CEILING

6/12 PITCHES (TYP.)  
UNLESS NOTED

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PARTS OF THE OBC LATEST EDITION  
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.  
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.  
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE  
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

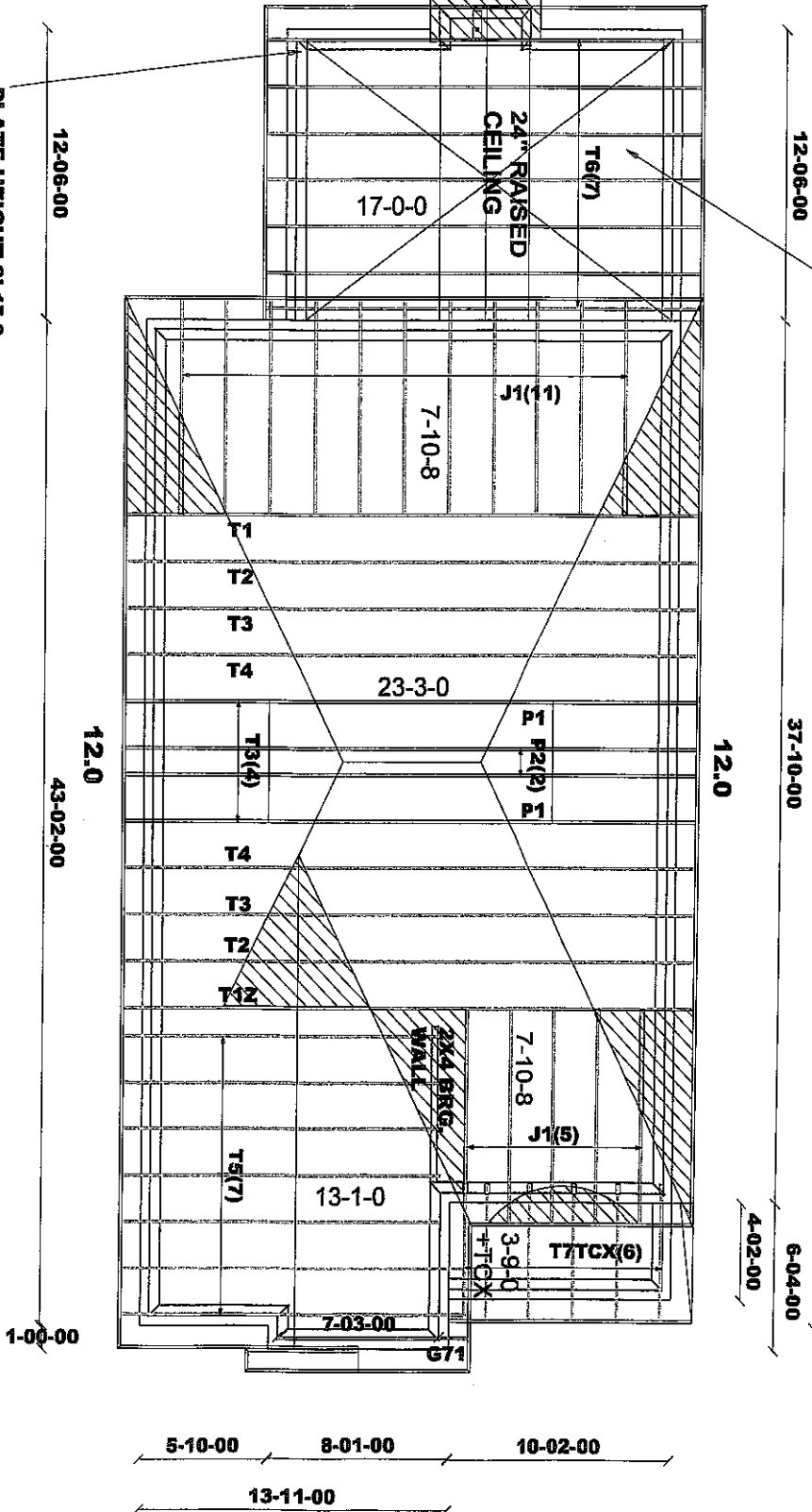
DESIGN LOADS:  
SOFFIT  
SNOW LOAD 38.3 psf  
TC DEAD 3 PSF  
BC LIVE 10.5 PSF  
BC DEAD 7 PSF

T-170674  
PRELIMINARY  
DENOTES:  
CONVENTIONAL  
FRAMING



PLATE HEIGHT 9'-1"-0  
U/S OF SOFFIT @ PLATE LEVEL

24-01-00  
6-03-00 6-05-00 5-00-00 6-05-00



Job Track: 42067

Layout ID: 288177

Plan Log: 94279

Builder / Location:

BAYVIEW WELLINGTON / INNISFIL

Model / Elevation:

S30-1 / EL:C - STD.

Project: ALCONA SHORES  
Date: 10/7/2017 Designer: S.V. / collin.jg

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC. AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC. IF UTILIZED FOR ANY OTHER PURPOSE.

ASPHALT SHINGLES  
12" FINISH O.H.  
R.T.M.C  
2x6 EXTERIOR WALLS  
2x6 FASCIA BOARD

4/12 VAULT  
CEILING

6/12 PITCHES (TYP.)  
UNLESS NOTED

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC, LATEST EDITION  
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.  
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.  
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE  
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:  
SOFFIT  
SNOW LOAD 38.3 psf  
TC DEAD 3 PSF  
BC LIVE 10.5 PSF  
BC DEAD 7 PSF

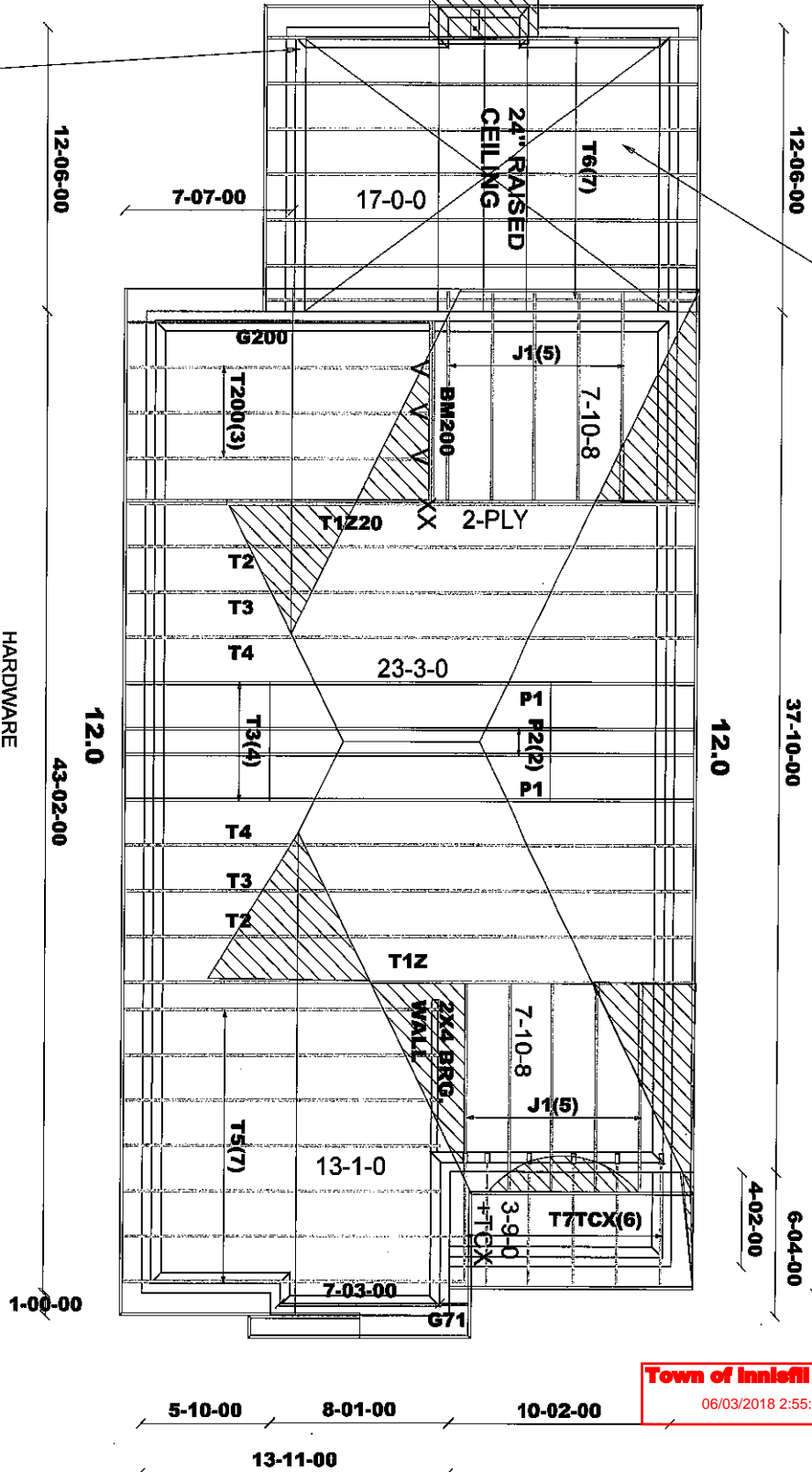
T-170704  
DENOTES:  
CONVENTIONAL  
FRAMING



PLATE HEIGHT 9'-1"-0"  
U/S OF SOFFIT @ PLATE LEVEL

HARDWARE  
LUS26DS(V)  
HGUS26-2(XXX)  
BEAM200-2-2X10

Job Track: <b>42067</b>		Builder / Location: <b>BAYVIEW WELLINGTON / INNISFIL</b>		Model / Elevation: <b>S30-1 / EL.C - REAR UPGRADE</b>	
Layout ID: <b>289593</b>		Project: <b>ALCONA SHORES</b>		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
Plan Log: <b>94279</b>		Date: <b>10/17/2017</b>		Designer: S.V / colin hlg	



Town of Innisfil Certified Model  
06/03/2018 2:55:09 PM kgervais



## Delivery Shiplist

DATE	10/16/17
SALES REP	Mario

JOB TRACK: 42067      LAYOUT ID: 272800      LOCATION: INNISFIL  
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO      SUB-BUILDER:  
 MODEL: S30-1      ELEVATION: A STD

### ROOF TRUSSES

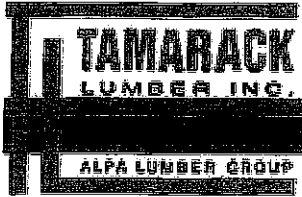
ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	1	T1Z HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	2	T2 HIP	12.00 0.00	23-03-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	200.28 127.34		
	6	T3 PIGGYBACK	12.00 0.00	23-03-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	668.82 421.02		
	2	T4 HIP	12.00 0.00	23-03-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	234.74 149.34		
	6	T5 COMMON	12.00 0.00	13-01-00	08-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	423.36 271.98		
	1	G5 COMMON	12.00 0.00	13-01-00	08-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	70.40 45.00		
	7	T6 ROOF	6.00 4.00	17-00-00	05-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	469.56 319.69		
	6	T7TCX MONOPITCH	6.00 0.00	03-09-00	02-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-05-04	98.94 64.02		
	2	P1 PIGGYBACK	12.00 0.00	11-07-14	01-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	79.74 52.00		
	2	P2 PIGGYBACK	12.00 0.00	11-07-14	02-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	81.64 52.66		
	16	J1 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	484.32 309.28		
	8	J2 JACK-OPEN	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	158.64 105.36		

TOTAL # TRUSS= 60.00

TOTAL BFT OF ALL TRUSSES=

2073.35 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3225.60 LBS.



## Delivery Shiplist

DATE	10/16/17
SALES REP	Mario

JOB TRACK:42067	LAYOUT ID: 289587	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S30-1	ELEVATION: A-REAR UPGRADE	

### ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1Z HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	1 2 Ply	T1Z20 HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	234.80 146.34		
	2	T2 HIP	12.00 0.00	23-03-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	200.28 127.34		
	6	T3 PIGGYBACK	12.00 0.00	23-03-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	668.82 421.02		
	2	T4 HIP	12.00 0.00	23-03-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	234.74 149.34		
	6	T5 COMMON	12.00 0.00	13-01-00	08-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	423.36 271.98		
	1	G5 COMMON	12.00 0.00	13-01-00	08-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	70.40 45.00		
	7	T6 ROOF	6.00 4.00	17-00-00	05-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	469.56 319.69		
	6	T7TCX MONOPITCH	6.00 0.00	03-09-00	02-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-05-04	98.94 64.02		
	3	T200 COMMON	12.00 0.00	12-04-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	194.97 126.00		
	1	G200 COMMON	12.00 0.00	12-04-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	66.31 43.00		
	2	P1 PIGGYBACK	12.00 0.00	11-07-14	01-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	79.74 52.00		
	2	P2 PIGGYBACK	12.00 0.00	11-07-14	02-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	81.64 52.66		
	10	J1 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	302.70 193.30		
	8	J2 JACK-OPEN	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	158.64 105.36		

TOTAL # TRUSS= 59.00

TOTAL BFT OF ALL TRUSSES=

2194.88 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3412.48 LBS.

### HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 4.00





## Delivery Shiplist

DATE	10/16/17
SALES REP	Mario

JOB TRACK: 42067      LAYOUT ID: 272801      LOCATION:  
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO      SUB-BUILDER:  
 MODEL: S30-1      ELEVATION: B-STD.

## ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T1 HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	1	T1Z HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	2	T2 HIP	12.00 0.00	23-03-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	200.28 127.34		
	6	T3 PIGGYBACK	12.00 0.00	23-03-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	668.82 421.02		
	2	T4 HIP	12.00 0.00	23-03-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	234.74 149.34		
	4	T5 COMMON	12.00 0.00	13-01-00	08-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	282.24 181.32		
	7	T6 ROOF	6.00 4.00	17-00-00	05-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	469.56 319.69		
	6	T7TCX MONOPITCH	6.00 0.00	04-06-04	02-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-09-14	98.94 64.02		
	1	T8 HIP GIRDER	12.00 0.00	13-01-00	04-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	64.45 43.17		
	1	T9 HIP	12.00 0.00	13-01-00	06-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	66.05 43.50		
	1	T10 COMMON	12.00 0.00	09-04-00	06-06-08	2 X 4	2 X 4	00-00-00 00-00-00	01-10-08 01-10-08	41.81 26.67		
	1	G10 COMMON	12.00 0.00	09-04-00	06-06-08	2 X 4	2 X 4	01-02-08 01-02-08	01-10-08 01-10-08	49.17 31.33		
	2	P1 PIGGYBACK	12.00 0.00	11-07-14	01-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	79.74 52.00		
	2	P2 PIGGYBACK	12.00 0.00	11-07-14	02-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	81.64 52.66		
	16	J1 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	484.32 309.28		
	2	J3 JACK-OPEN	12.00 0.00	02-10-08	04-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-09-00	27.24 18.00		
	5	J3A JACK-OPEN	12.00 0.00	02-05-08	04-09-00	2 X 4	2 X 4	00-00-00 00-00-00	02-03-08 04-09-00	57.00 37.50		

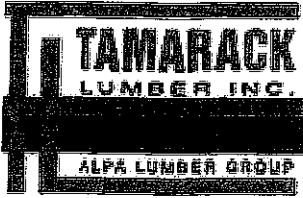
TOTAL # TRUSS= 60.00

TOTAL BFT OF ALL TRUSSES=

2032.50 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3161.16 LBS.

## HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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## Delivery Shiplist

DATE	10/16/17
SALES REP	Mario

JOB TRACK: 42067	LAYOUT ID: 272801	LOCATION:
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S30-1	ELEVATION: B-STD.	

### HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	LUS24	

TOTAL # ITEMS= 5.00



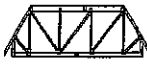
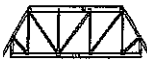
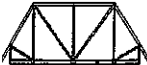




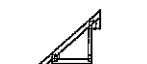


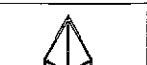

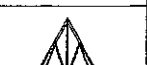
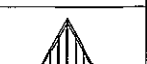
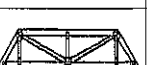
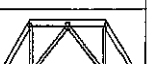

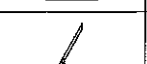
# Delivery Shiplist

DATE	10/17/17
SALES REP	Mario

JOB TRACK:42067	LAYOUT ID: 289598	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: S30-1	ELEVATION: B-REAR UPGRADE	

## ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	<b>T1Z</b> HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	1 2 Ply	<b>T1Z20</b> HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	255.16 155.66		
	2	<b>T2</b> HIP	12.00 0.00	23-03-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	200.28 127.34		
	6	<b>T3</b> PIGGYBACK	12.00 0.00	23-03-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	668.82 421.02		
	2	<b>T4</b> HIP	12.00 0.00	23-03-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	234.74 149.34		
	4	<b>T5</b> COMMON	12.00 0.00	13-01-00	08-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	282.24 181.32		
	7	<b>T6</b> ROOF	6.00 4.00	17-00-00	05-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	469.56 319.69		
	6	<b>T7TCX</b> MONOPITCH	6.00 0.00	04-06-04	02-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-09-14	98.94 64.02		
	1	<b>T8</b> HIP GIRDER	12.00 0.00	13-01-00	04-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	64.45 43.17		
	1	<b>T9</b> HIP	12.00 0.00	13-01-00	06-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	66.05 43.50		
	1	<b>T10</b> COMMON	12.00 0.00	09-04-00	06-06-08	2 X 4	2 X 4	00-00-00 00-00-00	01-10-08 01-10-08	41.81 26.67		
	1	<b>G10</b> COMMON	12.00 0.00	09-04-00	06-06-08	2 X 4	2 X 4	01-02-08 01-02-08	01-10-08 01-10-08	49.17 31.33		
	3	<b>T200</b> COMMON	12.00 0.00	12-04-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	194.97 126.00		
	1	<b>G200</b> COMMON	12.00 0.00	12-04-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	66.31 43.00		
	2	<b>P1</b> PIGGYBACK	12.00 0.00	11-07-14	01-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	79.74 52.00		
	2	<b>P2</b> PIGGYBACK	12.00 0.00	11-07-14	02-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	81.64 52.66		
	10	<b>J1</b> JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	302.70 193.30		
	2	<b>J3</b> JACK-OPEN	12.00 0.00	02-10-08	04-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-09-00	27.24 18.00		




## Delivery Shiplist

DATE	10/17/17
SALES REP	Mario

JOB TRACK: 42067      LAYOUT ID: 289598      LOCATION: INNISFIL  
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO      SUB-BUILDER:  
 MODEL: S30-1      ELEVATION: B-REAR UPGRADE

### ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	J3A JACK-OPEN	12.00 0.00	02-05-08	04-09-00	2 X 4	2 X 4	00-00-00 00-00-00	02-03-08 04-09-00	57.00 37.50		

TOTAL # TRUSS= 59.00

TOTAL BFT OF ALL TRUSSES=

2163.35 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3368.40 LBS.

### HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
5	Hangers	LUS24	

TOTAL # ITEMS= 9.00



## Delivery Shiplist

DATE	10/17/17
SALES REP	Mario

JOB TRACK:42067      LAYOUT ID: 288177      LOCATION: INNISFIL  
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO      SUB-BUILDER:  
 MODEL: S30-1      ELEVATION: C

## ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	<b>T1</b> HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	1	<b>T1Z</b> HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	2	<b>T2</b> HIP	12.00 0.00	23-03-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	200.28 127.34		
	6	<b>T3</b> PIGGYBACK	12.00 0.00	23-03-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	668.82 421.02		
	2	<b>T4</b> HIP	12.00 0.00	23-03-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	234.74 149.34		
	7	<b>T5</b> COMMON	12.00 0.00	13-01-00	08-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	493.92 317.31		
	7	<b>T6</b> ROOF	6.00 4.00	17-00-00	05-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	469.56 319.69		
	6	<b>T7TCX</b> MONOPITCH	6.00 0.00	03-09-00	02-09-14	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-05-04	107.16 70.02		
	1	<b>G71</b> COMMON	12.00 0.00	07-03-00	05-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	39.89 26.17		
	2	<b>P1</b> PIGGYBACK	12.00 0.00	11-07-14	01-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	79.74 52.00		
	2	<b>P2</b> PIGGYBACK	12.00 0.00	11-07-14	02-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	81.64 52.66		
	16	<b>J1</b> JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	484.32 309.28		

TOTAL # TRUSS= 53.00

TOTAL BFT OF ALL TRUSSES=

2000.49 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3115.23 LBS.



## Delivery Shiplist

DATE	10/17/17
SALES REP	Mario

JOB TRACK: 42067      LAYOUT ID: 289593      LOCATION: INNISFIL  
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO      SUB-BUILDER:  
 MODEL: S30-1      ELEVATION: C-REAR UPGRADE

### ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	<b>T1Z</b> HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	127.58 77.83		
	1 2 Ply	<b>T1Z20</b> HIP GIRDER	12.00 0.00	23-03-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	255.16 155.66		
	2	<b>T2</b> HIP	12.00 0.00	23-03-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	200.28 127.34		
	6	<b>T3</b> PIGGYBACK	12.00 0.00	23-03-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	668.82 421.02		
	2	<b>T4</b> HIP	12.00 0.00	23-03-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	234.74 149.34		
	7	<b>T5</b> COMMON	12.00 0.00	13-01-00	08-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	493.92 317.31		
	7	<b>T6</b> ROOF	6.00 4.00	17-00-00	05-05-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	469.56 319.69		
	6	<b>T7TCX</b> MONOPITCH	6.00 0.00	03-09-00	02-09-14	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-05-04	107.16 70.02		
	1	<b>G71</b> COMMON	12.00 0.00	07-03-00	05-06-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	39.89 26.17		
	3	<b>T200</b> COMMON	12.00 0.00	12-04-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	194.97 126.00		
	1	<b>G200</b> COMMON	12.00 0.00	12-04-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	66.31 43.00		
	2	<b>P1</b> PIGGYBACK	12.00 0.00	11-07-14	01-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	79.74 52.00		
	2	<b>P2</b> PIGGYBACK	12.00 0.00	11-07-14	02-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	81.64 52.66		
	10	<b>J1</b> JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	302.70 193.30		

TOTAL # TRUSS= 52.00

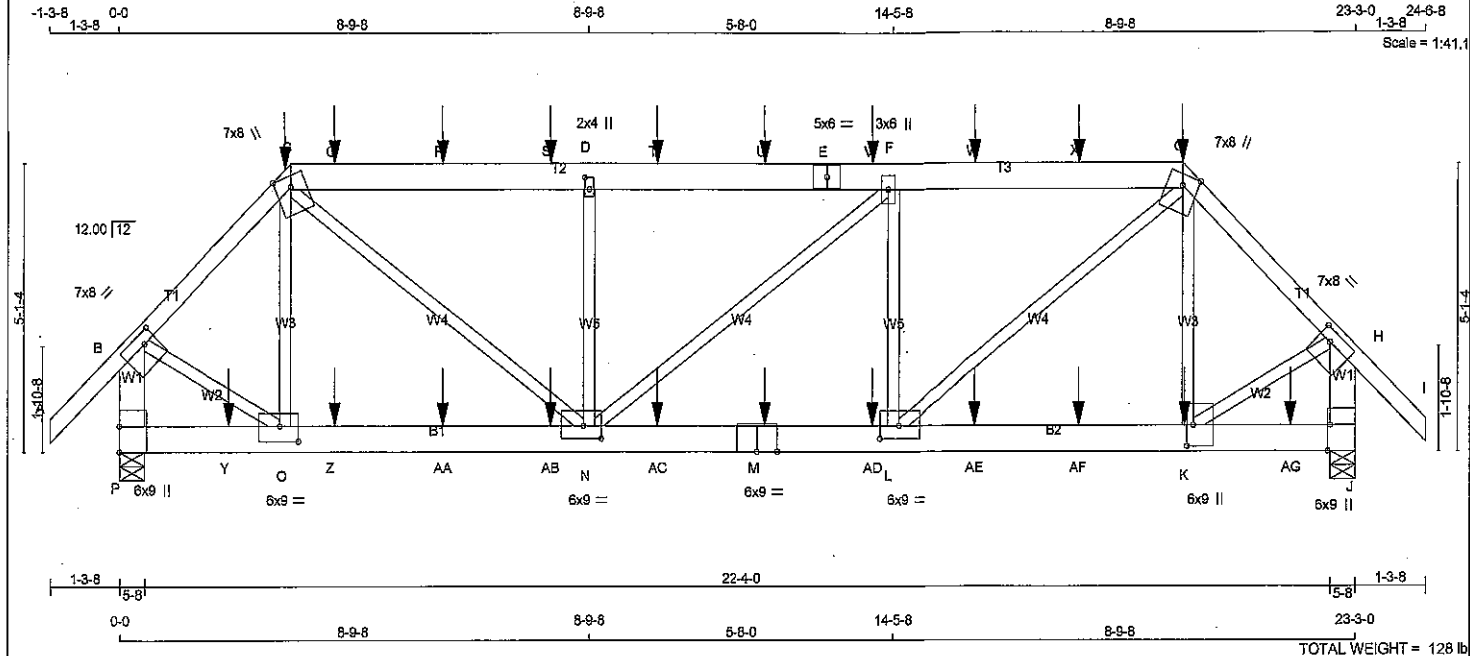
TOTAL BFT OF ALL TRUSSES=

2131.34 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3322.47 LBS.

### HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 4.00



**LUMBER**

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x6	DRY	No.2 SPF
E - G	2x6	DRY	No.2 SPF
G - I	2x4	DRY	No.2 SPF
P - B	2x6	DRY	No.2 SPF
J - H	2x6	DRY	No.2 SPF
P - M	2x6	DRY	1650F 1.5E SPF
M - J	2x6	DRY	1650F 1.5E SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	7.0	8.0	2.25	2.75
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMVW+t	MT20	3.0	6.0		
G	TTWW+m	MT20	7.0	8.0	Edge	
H	TMVW-t	MT20	7.0	8.0	2.25	2.75
J	BMV1+t	MT20	6.0	9.0	Edge	0.50
K	BMVW+t	MT20	6.0	9.0	4.50	1.50
L	BMVW-t	MT20	6.0	9.0	2.75	4.25
M	BS-t	MT20	6.0	9.0		
N	BMVW+t	MT20	6.0	9.0	2.75	4.00
O	BMVW-t	MT20	6.0	9.0	3.25	4.25
P	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**HANGERS NOTES**

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 485.4 lbs FACTORED DOWN AT 20-0-4, 429.8 lbs FACTORED DOWN AT 3-2-12, 72.2 lbs FACTORED DOWN AT 4-0-12, 62.0 lbs FACTORED DOWN AT 6-0-12, 62.0 lbs FACTORED DOWN AT 8-0-12, 62.0 lbs FACTORED DOWN AT 10-0-12, 62.0 lbs FACTORED DOWN AT 12-0-12, 62.0 lbs FACTORED DOWN AT 14-0-12, AND 62.0 lbs FACTORED DOWN AT 16-0-12, AND 62.0 lbs FACTORED DOWN AT 18-0-12 ON TOP CHORD, AND 379.2 lbs FACTORED DOWN AT 2-0-12, 379.2 lbs FACTORED DOWN AT 4-0-12, 379.2 lbs FACTORED DOWN AT 6-0-12, 379.2 lbs FACTORED DOWN AT 8-0-12, 379.2 lbs FACTORED DOWN AT 10-0-12, 379.2 lbs FACTORED DOWN AT 12-0-12, 379.2 lbs FACTORED DOWN AT 14-0-12, 379.2 lbs FACTORED DOWN AT 16-0-12, 379.2 lbs FACTORED DOWN AT 18-0-12, AND 379.2 lbs FACTORED DOWN AT 20-0-12, AND 379.2 lbs FACTORED DOWN AT 22-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	
P	4633	0	4633	0	0	5-8	
J	4804	0	4804	0	0	5-8	

UNFACTORED REACTIONS							
JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	3584	2423 / 0	570 / 0	0 / 0	0 / 0	571 / 0	0 / 0
J	3699	2507 / 0	595 / 0	0 / 0	0 / 0	586 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PLATE BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 60	-122.2 -122.2	0.19 (1)	10.00	C-C	-269 / 157	0.10 (1)
B-C	-4118 / 0	-122.2 -122.2	0.54 (1)	3.00	K-G	-245 / 171	0.09 (1)
C-Q	-5532 / 0	-122.2 -122.2	0.54 (1)	3.22	B-O	0 / 3210	0.79 (1)
Q-R	-5532 / 0	-122.2 -122.2	0.54 (1)	3.22	K-H	0 / 3239	0.80 (1)
R-S	-5532 / 0	-122.2 -122.2	0.54 (1)	3.22	L-G	0 / 3375	0.84 (1)
S-D	-5532 / 0	-122.2 -122.2	0.50 (1)	3.28	C-N	0 / 3404	0.84 (1)
D-T	-5532 / 0	-122.2 -122.2	0.50 (1)	3.28	L-F	-938 / 0	0.33 (1)
T-U	-5532 / 0	-122.2 -122.2	0.50 (1)	3.28	N-D	-938 / 0	0.33 (1)
U-E	-5532 / 0	-122.2 -122.2	0.50 (1)	3.28	N-F	-5 / 2	0.00 (1)
E-V	-5532 / 0	-122.2 -122.2	0.50 (1)	3.28			
V-F	-5532 / 0	-122.2 -122.2	0.50 (1)	3.28			
F-W	-5535 / 0	-122.2 -122.2	0.53 (1)	3.24			
W-X	-5535 / 0	-122.2 -122.2	0.53 (1)	3.24			
X-G	-5535 / 0	-122.2 -122.2	0.53 (1)	3.24			
G-H	-4153 / 0	-122.2 -122.2	0.55 (1)	2.97			
H-I	0 / 60	-122.2 -122.2	0.19 (1)	10.00			
P-B	-4616 / 0	0.0	0.36 (1)	4.97			
J-H	-4655 / 0	0.0	0.36 (1)	4.94			
P-Y	0 / 0	-28.0	-26.0	0.25 (1)	10.00		
Y-O	0 / 0	-28.0	-26.0	0.25 (1)	10.00		
O-Z	0 / 2922	-28.0	-26.0	0.50 (1)	10.00		
Z-AA	0 / 2922	-28.0	-26.0	0.50 (1)	10.00		
AA-AB	0 / 2922	-28.0	-26.0	0.50 (1)	10.00		
AB-N	0 / 2922	-28.0	-26.0	0.50 (1)	10.00		
N-AC	0 / 5535	-28.0	-26.0	0.71 (1)	10.00		
AC-M	0 / 5535	-28.0	-26.0	0.71 (1)	10.00		
M-AD	0 / 5535	-28.0	-26.0	0.71 (1)	10.00		
AD-L	0 / 5535	-28.0	-26.0	0.71 (1)	10.00		
L-AE	0 / 2947	-28.0	-26.0	0.49 (1)	10.00		
AE-AF	0 / 2947	-28.0	-26.0	0.49 (1)	10.00		
AF-K	0 / 2947	-28.0	-26.0	0.49 (1)	10.00		
K-AG	0 / 0	-28.0	-26.0	0.21 (1)	10.00		
AG-J	0 / 0	-28.0	-26.0	0.21 (1)	10.00		

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	3-2-12	-28	-29		FRONT	VERT	DEAD
C	3-2-12	-404	-404		FRONT	VERT	DEAD
G	20-0-4	-28	-29		FRONT	VERT	DEAD

**DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*  
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.  
LOADS WERE DERIVED FROM USER INPUT  
NO FURTHER MODIFICATIONS WERE MADE

**SPECIFIED LOADS:**  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

\*\*\* NON STANDARD GIRDER \*\*\*  
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, 8CBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.78")  
CALCULATED VERT. DEFL. (LL) = L/989 (0.19")  
ALLOWABLE DEFL. (TL) = L/360 (0.78")  
CALCULATED VERT. DEFL. (TL) = L/947 (0.29")

CSI: TC=0.55 (G-H:1), BC=0.71 (L-N:1), WB=0.84 (C-N:1), SSI=0.46 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

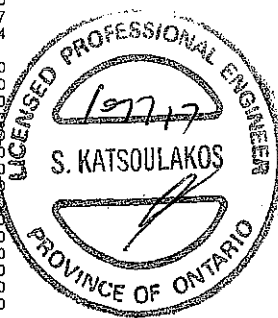
**NAIL VALUES**

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	PLU
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)  
JSI METAL= 0.95 (M) (INPUT = 1.00)



OWANO. TAM 2306-17  
STRUCTURAL  
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272800	T1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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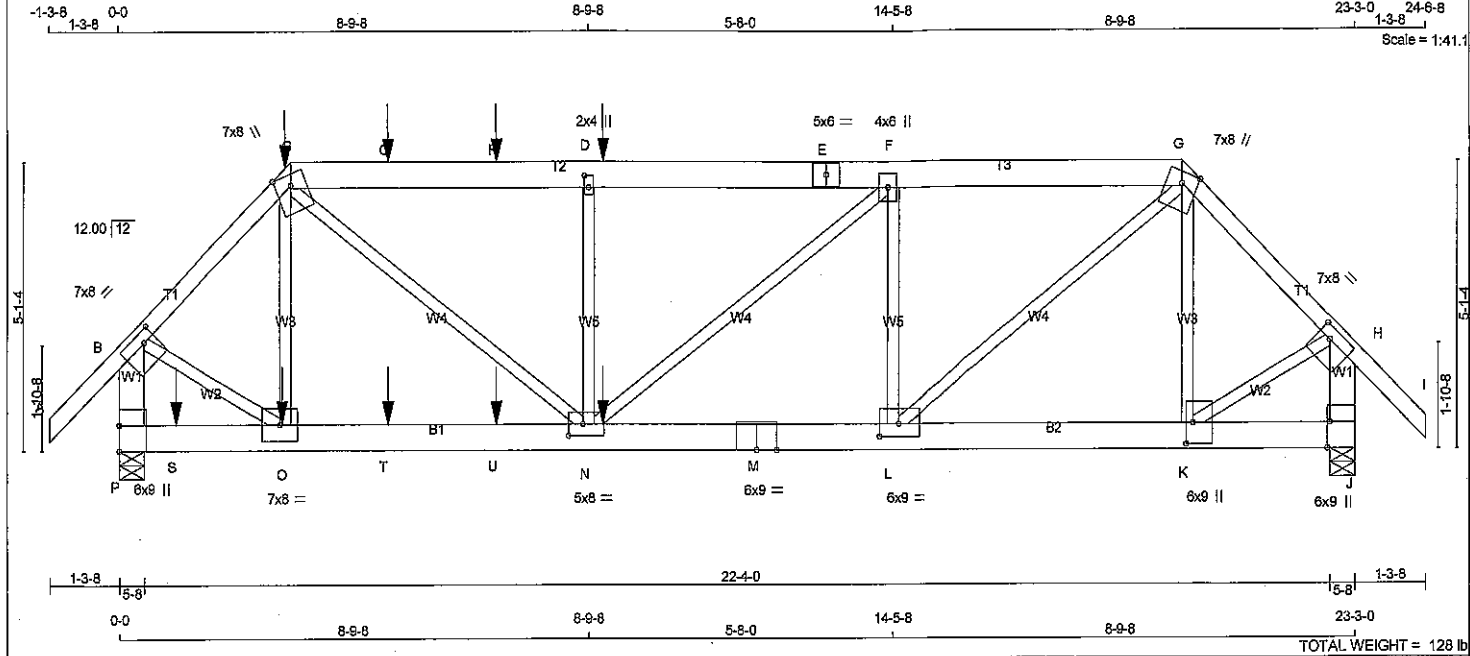
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	20-0-4	-66	-66	--	FRONT	VERT	TOTAL
G	20-0-4	-404	-404	--	FRONT	VERT	SNOW
K	20-0-12	-379	-379	--	FRONT	VERT	TOTAL
M	12-0-12	-379	-379	--	FRONT	VERT	TOTAL
Q	4-0-12	-72	-72	--	FRONT	VERT	TOTAL
R	6-0-12	-62	-62	--	FRONT	VERT	TOTAL
S	8-0-12	-62	-62	--	FRONT	VERT	TOTAL
T	10-0-12	-62	-62	--	FRONT	VERT	TOTAL
U	12-0-12	-62	-62	--	FRONT	VERT	TOTAL
V	14-0-12	-62	-62	--	FRONT	VERT	TOTAL
W	16-0-12	-62	-62	--	FRONT	VERT	TOTAL
X	18-0-12	-62	-62	--	FRONT	VERT	TOTAL
Y	2-0-12	-379	-379	--	FRONT	VERT	TOTAL
Z	4-0-12	-379	-379	--	FRONT	VERT	TOTAL
AA	6-0-12	-379	-379	--	FRONT	VERT	TOTAL
AB	8-0-12	-379	-379	--	FRONT	VERT	TOTAL
AC	10-0-12	-379	-379	--	FRONT	VERT	TOTAL
AD	14-0-12	-379	-379	--	FRONT	VERT	TOTAL
AE	16-0-12	-379	-379	--	FRONT	VERT	TOTAL
AF	18-0-12	-379	-379	--	FRONT	VERT	TOTAL
AG	22-0-12	-379	-379	--	FRONT	VERT	TOTAL



DRWG NO. TAM 52306-17  
STRUCTURAL  
COMPONENT ONLY





**LUMBER**

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x6	DRY	No.2
E - G	2x6	DRY	No.2
G - I	2x4	DRY	No.2
P - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
P - M	2x6	DRY	1650F 1.5E
M - J	2x6	DRY	1650F 1.5E

ALL WEBS 2x3 DRY No.2 SPF  
EXCEPT

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+t	MT20	7.0	8.0	2.25	2.75
C	TTVW+m	MT20	7.0	8.0	Edge	
D	TMVW+t	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMVW+t	MT20	4.0	6.0		
G	TTVW+m	MT20	7.0	8.0	Edge	
H	TMVW-t	MT20	7.0	8.0	2.25	2.75
J	BMV1+t	MT20	6.0	8.0	Edge	0.50
K	BMVW+t	MT20	6.0	8.0	4.50	1.50
L	BMVW-t	MT20	6.0	8.0	2.75	4.25
M	BS-t	MT20	6.0	9.0		
N	BMVW+t	MT20	5.0	8.0	2.50	3.25
O	BMVW-t	MT20	7.0	8.0		
P	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**HANGERS NOTES**

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 429.8 lbs FACTORED DOWN AT 3-2-12, 65.1 lbs FACTORED DOWN AT 3-2-12, 62.0 lbs FACTORED DOWN AT 5-0-12, AND 62.0 lbs FACTORED DOWN AT 7-0-12, AND 62.0 lbs FACTORED DOWN AT 9-0-12 ON TOP CHORD, AND 379.2 lbs FACTORED DOWN AT 1-0-12, 379.2 lbs FACTORED DOWN AT 3-0-12, 379.2 lbs FACTORED DOWN AT 5-0-12, AND 379.2 lbs FACTORED DOWN AT 7-0-12, AND 379.2 lbs FACTORED DOWN AT 9-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	REQD	BRG	IN-SX
P	3960	0	3960	0	0	5-8	5-8			
J	2451	0	2451	0	0	5-8	5-8			

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	3045	2071 / 0	455 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1692	1274 / 0	311 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 60	-122.2 -122.2	0.19 (1)	10.00	C-C	0 / 298	0.07 (3)
B-C	-3340 / 0	-122.2 -122.2	0.44 (1)	3.42	K-G	-475 / 7	0.17 (1)
C-Q	-3840 / 0	-122.2 -122.2	0.43 (1)	3.94	B-O	0 / 2805	0.64 (1)
Q-R	-3840 / 0	-122.2 -122.2	0.43 (1)	3.94	K-H	0 / 1566	0.39 (1)
R-D	-3840 / 0	-122.2 -122.2	0.43 (1)	3.94	L-G	0 / 2214	0.55 (1)
D-E	-3840 / 0	-122.2 -122.2	0.38 (1)	4.04	C-N	0 / 1926	0.48 (1)
E-F	-3840 / 0	-122.2 -122.2	0.38 (1)	4.04	L-F	-1333 / 0	0.46 (1)
F-G	-3130 / 0	-122.2 -122.2	0.32 (1)	4.45	N-D	-880 / 0	0.31 (1)
G-H	-2008 / 0	-122.2 -122.2	0.32 (1)	4.41	N-F	0 / 818	0.23 (1)
H-I	0 / 60	-122.2 -122.2	0.19 (1)	10.00			
P-B	-3815 / 0	0.0	0.0	2.29 (1)	5.43		
J-H	-2440 / 0	0.0	0.0	0.19 (1)	6.57		
P-S	0 / 0	-28.0	-28.0	0.24 (1)	10.00		
S-O	0 / 0	-28.0	-28.0	0.24 (1)	10.00		
O-T	0 / 2364	-28.0	-28.0	0.43 (1)	10.00		
T-U	0 / 2364	-28.0	-28.0	0.43 (1)	10.00		
U-N	0 / 2364	-28.0	-28.0	0.43 (1)	10.00		
N-M	0 / 3130	-28.0	-28.0	0.38 (1)	10.00		
M-L	0 / 3130	-28.0	-28.0	0.38 (1)	10.00		
L-K	0 / 1433	-28.0	-28.0	0.17 (1)	10.00		
K-J	0 / 0	-28.0	-28.0	0.07 (2)	10.00		

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	3-2-12	-28	-29		FRONT	VERT	BEAD
C	3-2-12	-65	-65		BACK	VERT	TOTAL
C	3-2-12	-404	-404		FRONT	VERT	SNOW
D	9-0-12	-62	-62		BACK	VERT	TOTAL
N	9-0-12	-379	-379		BACK	VERT	TOTAL
O	3-0-12	-379	-379		BACK	VERT	TOTAL
Q	5-0-12	-62	-62		BACK	VERT	TOTAL
R	7-0-12	-62	-62		BACK	VERT	TOTAL
S	1-0-12	-379	-379		BACK	VERT	TOTAL
T	5-0-12	-379	-379		BACK	VERT	TOTAL
U	7-0-12	-379	-379		BACK	VERT	TOTAL

**DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

**SPECIFIED LOADS:**

TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF

BOT CH. LL = 13.5 PSF  
DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

\*\*\* NON STANDARD GIRDER \*\*\*

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC2012, ABC 2014

- CSA 088-09

- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.78")

CALCULATED VERT. DEFL.(LL) = L/999 (0.13")

ALLOWABLE DEFL.(TL) = L/360 (0.78")

CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.44 (B-C:1), BC=0.43 (N-C:1), WB=0.64 (B-O:1), SSP=0.32 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

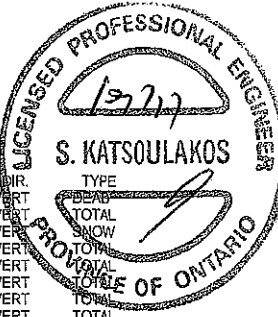
MT20 618 354 1657 622 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (N) (INPUT = 0.90)

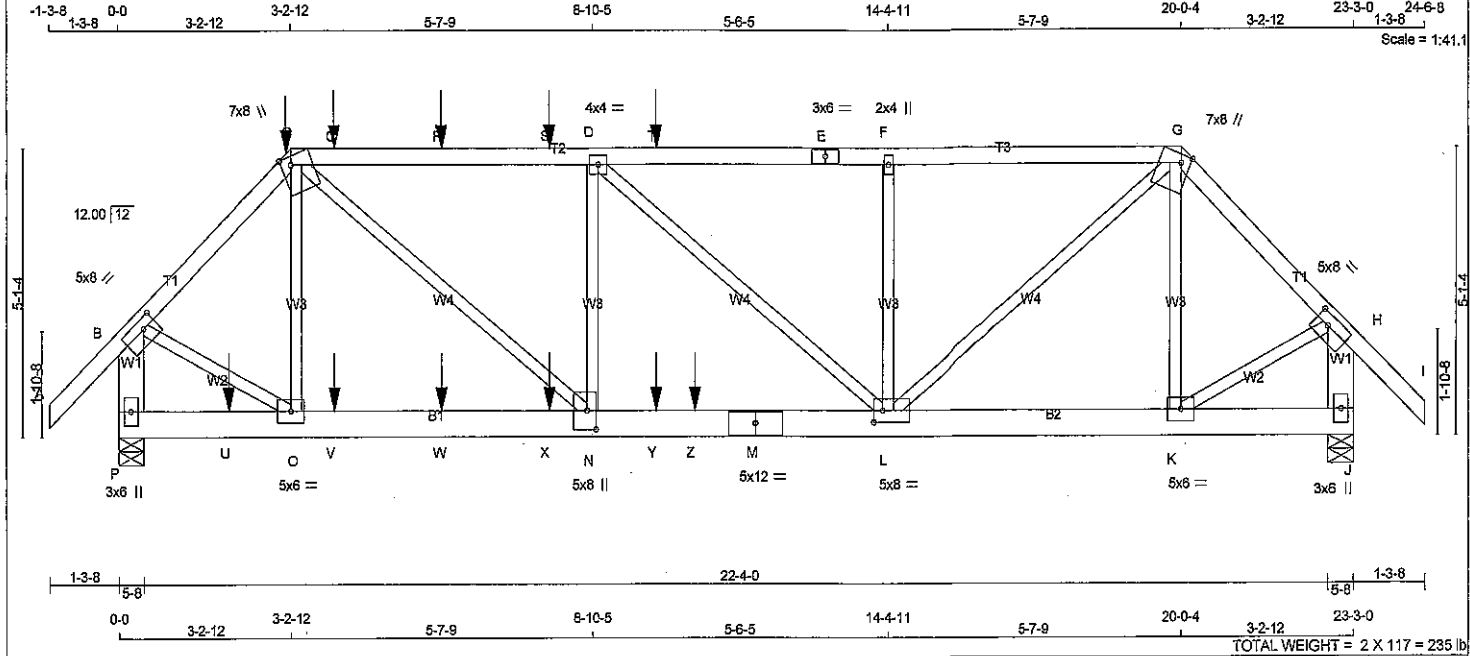
JSI TENS= 0.67 (B) (INPUT = 1.00)



DRG NO. TAM5230718

STRUCTURAL

COMPONENT ONLY



**LUMBER**  
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
P - B	2x8	DRY No.2	SPF
J - H	2x8	DRY No.2	SPF
P - M	2x6	DRY 1650F 1.5E	SPF
M - J	2x6	DRY 1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF  
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1 12		SIDE(61.0)
C - E 1 12		SIDE(61.0)
E - G 1 12		TOP
G - I 1 12		TOP
P - B 2 12		TOP
J - H 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P - M 2 12		SIDE(0.0)
M - J 2 12		TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	8.0	2.00	3.00
C TTWW+m	MT20	7.0	8.0	Edge 2.25	
D TMVW-t	MT20	4.0	4.0		
E TS-t	MT20	3.0	6.0		
F TMVW+m	MT20	2.0	4.0		
G TTWW+m	MT20	7.0	8.0	1.75	2.25
H TMVW-t	MT20	5.0	8.0	2.25	3.00
J BMV1+P	MT20	3.0	6.0		
K BMVW-t	MT20	5.0	6.0		
L BMVW+m	MT20	5.0	8.0	2.50	2.00
M BS-t	MT20	5.0	12.0		
N BMVW+m	MT20	5.0	8.0	4.00	2.00
O BMVW-t	MT20	5.0	6.0		
P BMV1+P	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	4987	0	4987	0	0	5-8	5-8
J	3559	0	3559	0	0	5-8	5-8

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	3854	2589 / 0	637 / 0	0 / 0	0 / 0	628 / 0	0 / 0
J	2737	1863 / 0	437 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 60	-122.2 -122.2	0.09 (1)	10.00	C-C	-540 / 89	0.10 (1)
B-C	-4436 / 0	-122.2 -122.2	0.21 (1)	4.38	C-N	0 / 4266	0.53 (1)
C-Q	-8395 / 0	-122.2 -122.2	0.60 (1)	3.38	N-D	0 / 272	0.03 (3)
Q-R	-8395 / 0	-122.2 -122.2	0.60 (1)	3.38	D-L	-1409 / 0	0.71 (1)
R-S	-8395 / 0	-122.2 -122.2	0.60 (1)	3.38	L-F	-712 / 0	0.13 (1)
S-D	-8395 / 0	-122.2 -122.2	0.60 (1)	3.38	L-G	0 / 4128	0.51 (1)
D-T	-5315 / 0	-122.2 -122.2	0.51 (1)	3.74	K-G	-966 / 0	0.18 (1)
T-E	-5315 / 0	-122.2 -122.2	0.51 (1)	3.74	B-O	0 / 3401	0.42 (1)
E-F	-5315 / 0	-122.2 -122.2	0.51 (1)	3.74	K-H	0 / 2360	0.29 (1)
F-G	-5315 / 0	-122.2 -122.2	0.44 (1)	3.84			
G-H	-3079 / 0	-122.2 -122.2	0.16 (1)	5.12			
H-I	0 / 60	-122.2 -122.2	0.09 (1)	10.00			
P-B	-4887 / 0	0.0	0.19 (1)	6.58			
J-H	-3491 / 0	0.0	0.13 (1)	7.48			
P-U	0 / 0	-28.0	-28.0	0.06 (2)	10.00		
U-O	0 / 0	-28.0	-28.0	0.06 (2)	10.00		
O-V	0 / 3110	-28.0	-28.0	0.47 (1)	10.00		
V-W	0 / 3110	-28.0	-28.0	0.47 (1)	10.00		
W-X	0 / 3110	-28.0	-28.0	0.47 (1)	10.00		
X-N	0 / 3110	-28.0	-28.0	0.47 (1)	10.00		
N-Y	0 / 6386	-28.0	-28.0	0.78 (1)	10.00		
Y-Z	0 / 6386	-28.0	-28.0	0.78 (1)	10.00		
Z-M	0 / 6386	-28.0	-28.0	0.78 (1)	10.00		
M-L	0 / 6386	-28.0	-28.0	0.78 (1)	10.00		
L-K	0 / 2146	-28.0	-28.0	0.26 (1)	10.00		
K-J	0 / 0	-28.0	-28.0	0.02 (1)	10.00		

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	VERT	TYPE
C	3-2-12	-358	-358		FRONT	VERT	TOTAL
Q	4-0-12	-72	-72		FRONT	VERT	TOTAL
R	6-0-12	-62	-62		FRONT	VERT	TOTAL
S	8-0-12	-62	-62		FRONT	VERT	TOTAL
T	10-0-12	-62	-62		FRONT	VERT	TOTAL
U	2-0-12	-379	-379		FRONT	VERT	TOTAL
V	4-0-12	-379	-379		FRONT	VERT	TOTAL
W	6-0-12	-379	-379		FRONT	VERT	TOTAL
X	8-0-12	-379	-379		FRONT	VERT	TOTAL
Y	10-0-12	-379	-379		FRONT	VERT	TOTAL

**DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*  
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.  
LOADS WERE DERIVED FROM USER INPUT  
NO FURTHER MODIFICATIONS WERE MADE

**SPECIFIED LOADS:**  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN./C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

\*\*\* NON STANDARD GIRDER \*\*\*  
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCSC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")  
CALCULATED VERT. DEFL.(LL)= L/899 (0.17")  
ALLOWABLE DEFL.(TL)= L/360 (0.78")  
CALCULATED VERT. DEFL.(TL)= L/899 (0.28")

CSI: TC=0.60 (C-D:1), BC=0.78 (L-N:1), WB=0.71 (D-L:1), SSI=0.64 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

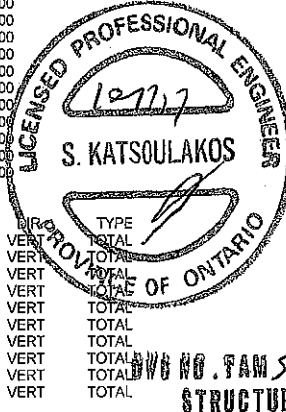
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354 1667 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)  
JSI METAL= 1.00 (M) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
289587	T1Z20	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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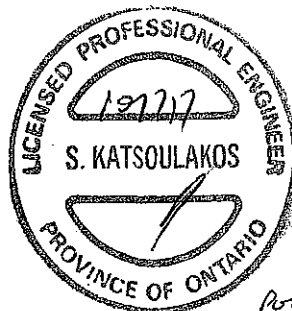
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# HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 358.0 lbs FACTORED DOWN AT 3-2-12, 72.2 lbs FACTORED DOWN AT 4-0-12, 82.0 lbs FACTORED DOWN AT 6-0-12, AND 62.0 lbs FACTORED DOWN AT 8-0-12, AND 62.0 lbs FACTORED DOWN AT 10-0-12 ON TOP CHORD, AND 379.2 lbs FACTORED DOWN AT 2-0-12, 379.2 lbs FACTORED DOWN AT 4-0-12, 379.2 lbs FACTORED DOWN AT 6-0-12, 379.2 lbs FACTORED DOWN AT 8-0-12, AND 379.2 lbs FACTORED DOWN AT 10-0-12, AND 2200.7 lbs FACTORED DOWN AT 10-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

## FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Z	10-9-8	-2201	-2201	—	FRONT	VERT	TOTAL

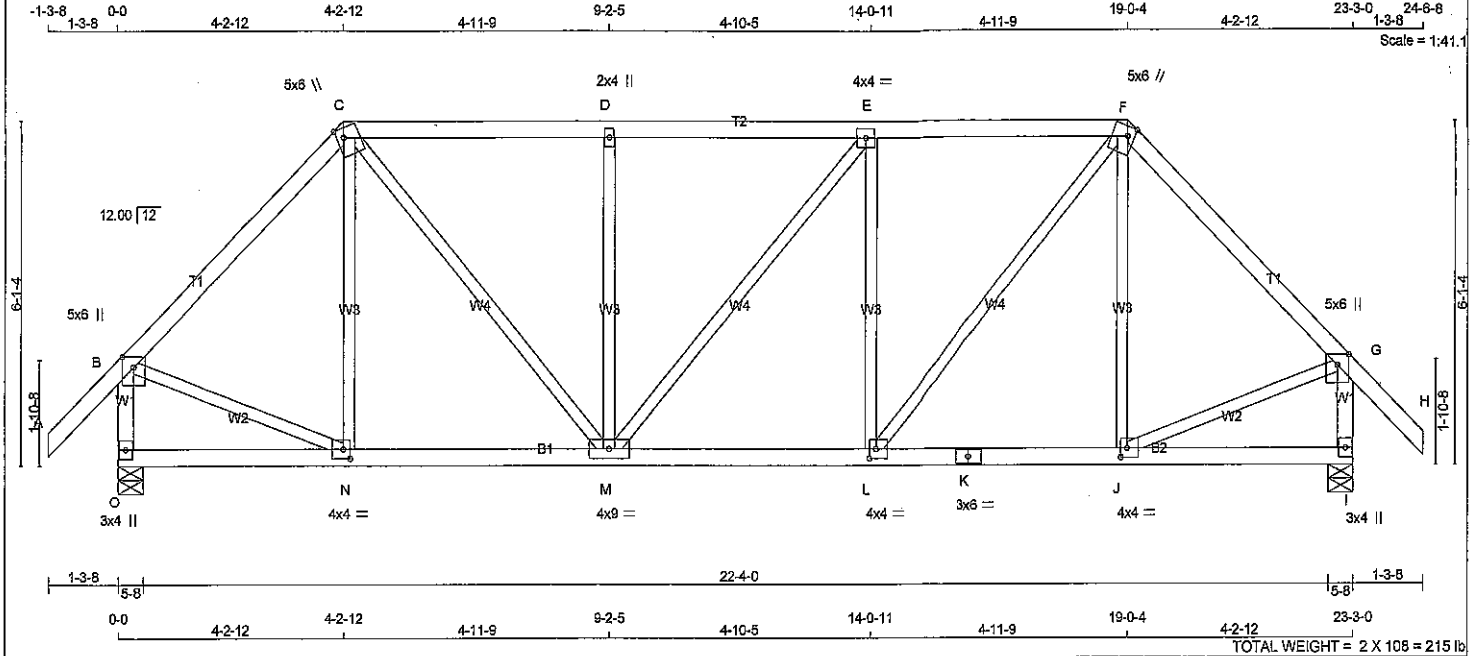


DWG NO. TAM 52317-17  
STRUCTURAL  
COMPONENT ONLY

JOB NAME <b>272800</b>	TRUSS NAME <b>T2</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. 42067	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:ls5la25dID168TP21p32Xyl\_VVY-LZTM6wF3oS2ywFeg\_yIXM3hEKWMy4jS3dLTA6BySZba



**LUMBER**

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
O - K	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TM/VW+p	MT20	5.0	6.0	2.25	2.50
C	TT/VW+m	MT20	5.0	6.0	2.00	1.50
D	TM/VW+w	MT20	2.0	4.0		
E	TM/VW+t	MT20	4.0	4.0		
F	TT/VW+m	MT20	5.0	6.0	2.00	1.50
G	TM/VW+p	MT20	5.0	6.0	2.25	2.50
I	BM/V1+p	MT20	3.0	4.0		
J, L, N						
J	BM/VW-t	MT20	4.0	4.0	2.00	1.50
K	BS-t	MT20	3.0	6.0		
M	BM/VWV-t	MT20	4.0	9.0		
O	BM/V1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1917	0	1917	0	0	5-8	5-8
I	1917	0	1917	0	0	5-8	5-8

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1481	996 / 0	244 / 0	0 / 0	0 / 0	241 / 0	0 / 0
I	1481	996 / 0	244 / 0	0 / 0	0 / 0	241 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 60	-122.2 -122.2	0.17 (1)	N-C	-226 / 107	0.13 (1)	
B-C	-1506 / 0	-122.2 -122.2	0.45 (1)	C-M	0 / 1026	0.23 (1)	
C-D	-1725 / 0	-122.2 -122.2	0.46 (1)	M-D	-645 / 0	0.38 (1)	
D-E	-1725 / 0	-122.2 -122.2	0.46 (1)	M-E	-3 / 0	0.00 (1)	
E-F	-1727 / 0	-122.2 -122.2	0.47 (1)	L-E	-848 / 0	0.38 (1)	
F-G	-1505 / 0	-122.2 -122.2	0.45 (1)	L-F	0 / 1030	0.23 (1)	
G-H	0 / 60	-122.2 -122.2	0.17 (1)	J-F	-228 / 107	0.13 (1)	
O-B	-1870 / 0	0.0 0.0	0.21 (1)	B-N	0 / 1124	0.25 (1)	
I-G	-1870 / 0	0.0 0.0	0.21 (1)	J-G	0 / 1124	0.25 (1)	
O-N	0 / 0	-28.0 -28.0	0.14 (2)				
N-M	0 / 1057	-28.0 -28.0	0.25 (1)				
M-L	0 / 1727	-28.0 -28.0	0.34 (1)				
L-K	0 / 1056	-28.0 -28.0	0.25 (1)				
K-J	0 / 1056	-28.0 -28.0	0.25 (1)				
J-I	0 / 0	-28.0 -28.0	0.14 (3)				

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.78")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")  
ALLOWABLE DEFL.(TL) = L/360 (0.78")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.47 (E-F:1), BC=0.34 (L-M:1), WB=0.38 (E-L:1), SSI=0.28 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1687 822 2284 1656

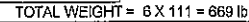
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)  
JSI METAL= 0.40 (N) (INPUT = 1.00)



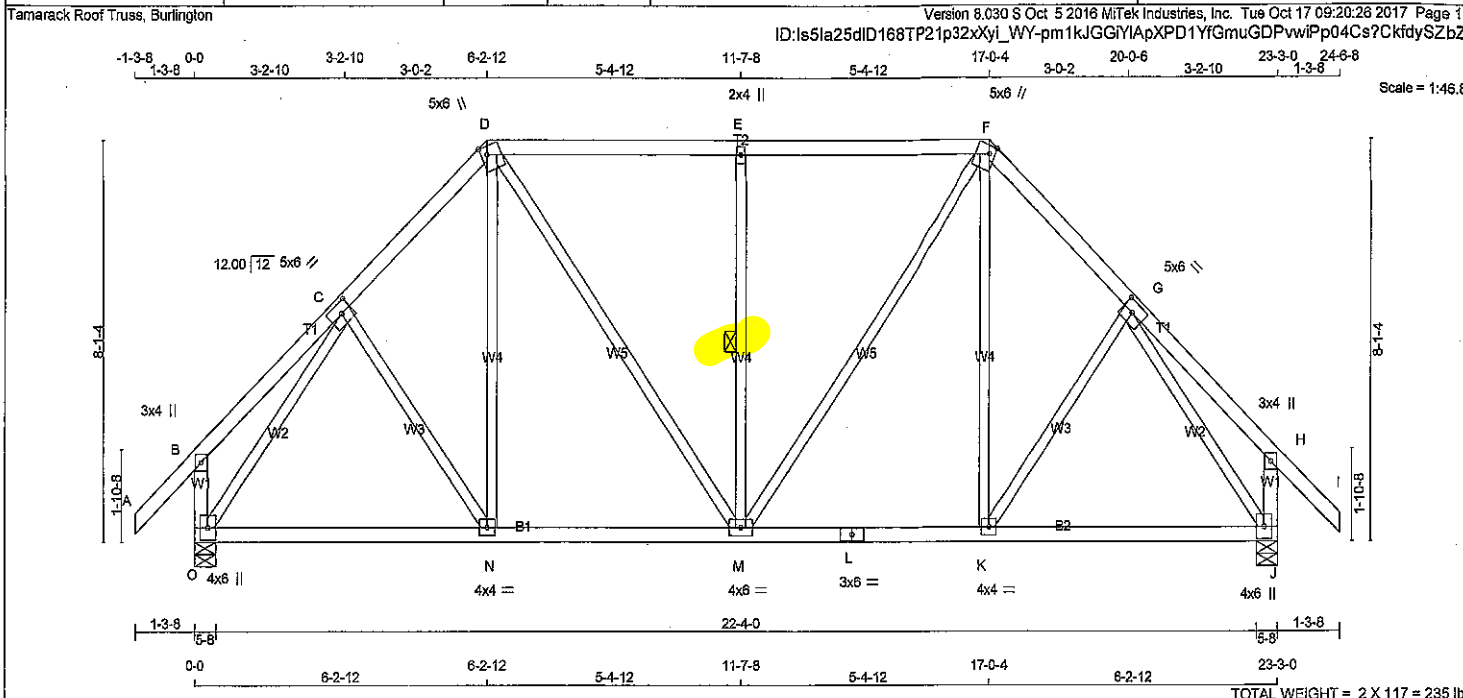
DRW NO. TAMS 2308-17  
STRUCTURAL  
COMPONENT ONLY



JSI GRIP= 0.87 (G) (INPUT = 0.90 )  
JSI METAL= 0.44 (C) (INPUT = 1.00 )



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42067	DRWG NO.
272800	T4	2	1	TRUSS DESC.		

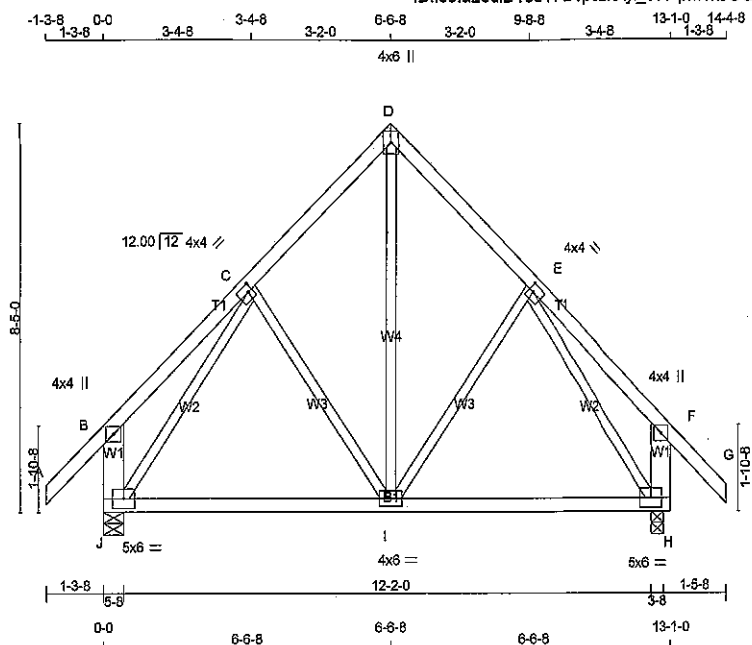


<b>LUMBER</b>				<b>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</b>				<b>DESIGN CRITERIA</b>			
N. L. G. A. RULES				<b>BEARINGS</b>				<b>SPECIFIED LOADS:</b>			
CHORDS	SIZE	LUMBER	DESCR	FACTORED				TOP CH. LL = 38.3 PSF			
A - D	2x4	DRY	No.2	GROSS REACTION				DL = 3.0 PSF			
D - F	2x4	DRY	No.2	MAXIMUM FACTORED				BOT CH. LL = 10.5 PSF			
F - I	2x4	DRY	No.2	INPUT				DL = 7.0 PSF			
O - B	2x4	DRY	No.2	REQD				TOTAL LOAD = 58.7 PSF			
J - H	2x4	DRY	No.2	BRG				<b>SPACING = 24.0 IN. C/C</b>			
O - L	2x4	DRY	No.2	IN-SX				LOADING IN FLAT SECTION BASED ON A			
L - J	2x4	DRY	No.2	IN-SX				SLOPE OF 6.00/12			
ALL WEBS 2x3 DRY No.2 SPF				<b>UNFACTORED REACTIONS</b>				THIS TRUSS IS DESIGNED FOR RESIDENTIAL			
EXCEPT				1ST CASE				OR SMALL BUILDING REQUIREMENTS OF			
DRY: SEASONED LUMBER.				MAX./MIN. COMPONENT REACTIONS				PART 9, NBCC 2010			
				JT COMBINED				THIS DESIGN COMPLIES WITH:			
				O 1481				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
				J 1481				- CSA 088-09			
								- TPIC 2011			
								(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F.			
								RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED			
								ROOF LIVE LOAD			
								ALLOWABLE DEFL.(LL)= L/360 (0.78")			
								CALCULATED VERT. DEFL.(LL) = L/999 (0.07")			
								ALLOWABLE DEFL.(TL)= L/360 (0.78")			
								CALCULATED VERT. DEFL.(TL) = L/999 (0.11")			
								CSI: TC=0.48 (D-E:1), BC=0.37 (K-M:2), WB=0.93			
								(G-J:1), SSI=0.32 (D-E:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
								COMP=1.10 SHEAR=1.10 TENS= 1.10			
								COMPANION LIVE LOAD FACTOR = 0.50			
								TRUSS PLATE MANUFACTURER IS NOT			
								RESPONSIBLE FOR QUALITY CONTROL IN			
								THE TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 618 354 1667 822 2284 1658			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.90 (G) (INPUT = 0.90 )			
								JSI METAL= 0.45 (G) (INPUT = 1.00 )			



DWONG.TAN 52310-17  
STRUCTURAL  
COMPONENT ONLY

JOB NAME <b>272800</b>	TRUSS NAME <b>T5</b>	QUANTITY <b>6</b>	PLY <b>1</b>	JOB DESC. 42087	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Tue Oct 17 09:20:26 2017 Page 1	



Scale = 1:50.2

TOTAL WEIGHT = 6 X 71 = 423 lb  
(M/F)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	4.0	4.0	
C	TMWW-t	MT20	4.0	4.0	2.00 1.25
D	TTW+p	MT20	4.0	6.0	
E	TMWW-t	MT20	4.0	4.0	2.00 1.25
F	TMV+p	MT20	4.0	4.0	
H	BMVW1-t	MT20	5.0	6.0	
I	BMWWW-t	MT20	4.0	6.0	
J	BMVW1-t	MT20	5.0	6.0	

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1153	0	1153	0	5-8
H	1153	0	1153	0	3-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
J	854	607 / 0	137 / 0	0 / 0	139 / 0
H	854	607 / 0	137 / 0	0 / 0	139 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

### LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO
A-B	0 / 60	-122.2	-122.2	0.17 (1)	10.00
B-C	0 / 33	-122.2	-122.2	0.21 (1)	10.00
C-D	-620 / 0	-122.2	-122.2	0.16 (1)	6.25
D-E	-620 / 0	-122.2	-122.2	0.16 (1)	6.25
E-F	0 / 33	-122.2	-122.2	0.21 (1)	10.00
F-G	0 / 60	-122.2	-122.2	0.17 (1)	10.00
J-B	-321 / 0	0.0	0.0	0.02 (1)	7.81
H-F	-321 / 0	0.0	0.0	0.02 (1)	7.81
J-I	0 / 517	-28.0	-28.0	0.40 (2)	10.00
I-H	0 / 517	-28.0	-28.0	0.40 (2)	10.00

### DESIGN CRITERIA

**SPECIFIED LOADS:**  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")  
ALLOWABLE DEFL.(TL) = L/360 (0.44")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.21 (B-C:1), BC=0.40 (H-I:2), WB=0.49 (C-J:1), SSI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PL)
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90 )  
JSI METAL= 0.35 (C) (INPUT = 1.00 )



DWG NO. TAM 51311-17  
STRUCTURAL  
COMPONENT ONLY



DWG NO. TAM 523/2-17  
STRUCTURAL  
COMPONENT ONLY

JSI GRIP= 0.32 (E) (INPUT = 0.90 )  
JSI METAL= 0.09 (G) (INPUT = 1.00 )

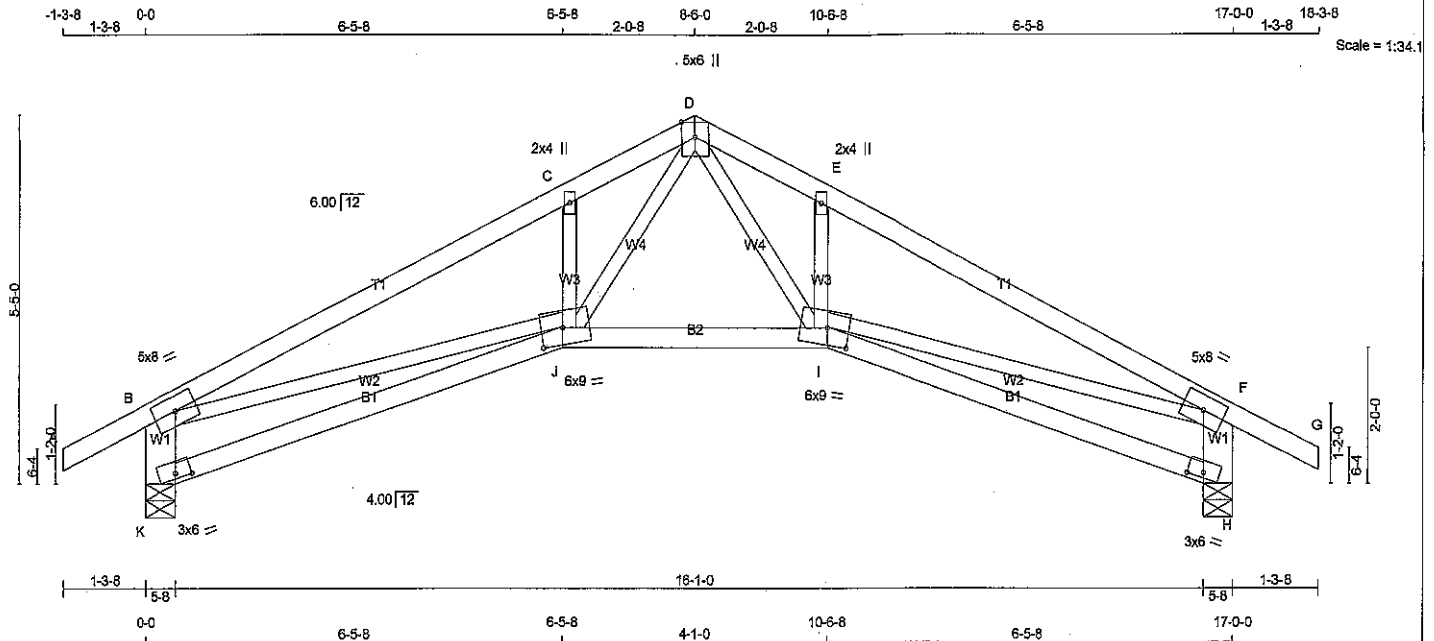


JOB NAME <b>272800</b>	TRUSS NAME <b>T6</b>	QUANTITY <b>7</b>	PLY <b>1</b>	JOB DESC. 42087 TRUSS DESC.	DRWG NO.
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ID:ls5la25dID168TP21p32xYi\_WY-pm1kJGGIYApXPD1YfGmuGDJEwXp58Cs?CkfdySZbZ



TOTAL WEIGHT = 7 X 67 = 470 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
K - B	2x6	DRY No.2	SPF
H - F	2x6	DRY No.2	SPF
K - J	2x4	DRY No.2	SPF
J - I	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW-w	MT20	2.0	4.0		
D	TTWW+p	MT20	5.0	6.0	Edge	
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	8.0		
H	BVM1-t	MT20	3.0	6.0	1.00	3.00
I	BBWWW-m	MT20	6.0	9.0	3.00	4.00
J	BBWWW-m	MT20	6.0	9.0	3.00	4.00
K	BVM1-t	MT20	3.0	6.0	1.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

##### BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
K	1443	0	1443	0
H	1443	0	1443	0

##### UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	COMBINED	1110	754 / 0	179 / 0	0 / 0	0 / 0	179 / 0	0 / 0
H	COMBINED	1110	754 / 0	179 / 0	0 / 0	0 / 0	179 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

##### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.10 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

##### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 37	-122.2	-122.2	D-I	0 / 1703	0.38	(1)
B-C	-2934 / 0	-122.2	-122.2	I-E	-805 / 0	0.13	(1)
C-D	-3075 / 0	-122.2	-122.2	J-D	0 / 1703	0.38	(1)
D-E	-3075 / 0	-122.2	-122.2	J-C	-805 / 0	0.13	(1)
E-F	-2934 / 0	-122.2	-122.2	B-J	0 / 2692	0.61	(1)
F-G	0 / 37	-122.2	-122.2	I-F	0 / 2692	0.61	(1)
K-B	-1352 / 0	0.0	0.0				
H-F	-1352 / 0	0.0	0.0				
K-J	0 / 0	-28.0	-28.0				
J-I	0 / 1729	-28.0	-28.0				
I-H	0 / 0	-28.0	-28.0				

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 38.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 58.7	PSF

##### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =  $L/360$  (0.57")  
CALCULATED VERT. DEFL.(LL) =  $L/989$  (0.20")  
ALLOWABLE DEFL.(TL) =  $L/380$  (0.57")  
CALCULATED VERT. DEFL.(TL) =  $L/643$  (0.32")

CSI: TC=0.84 (B-C:1), BC=0.36 (I-J:1), WB=0.61 (B-J:1), SS=0.32 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354
	1067	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.80)  
JSI METAL= 0.60 (B) (INPUT = 1.00)



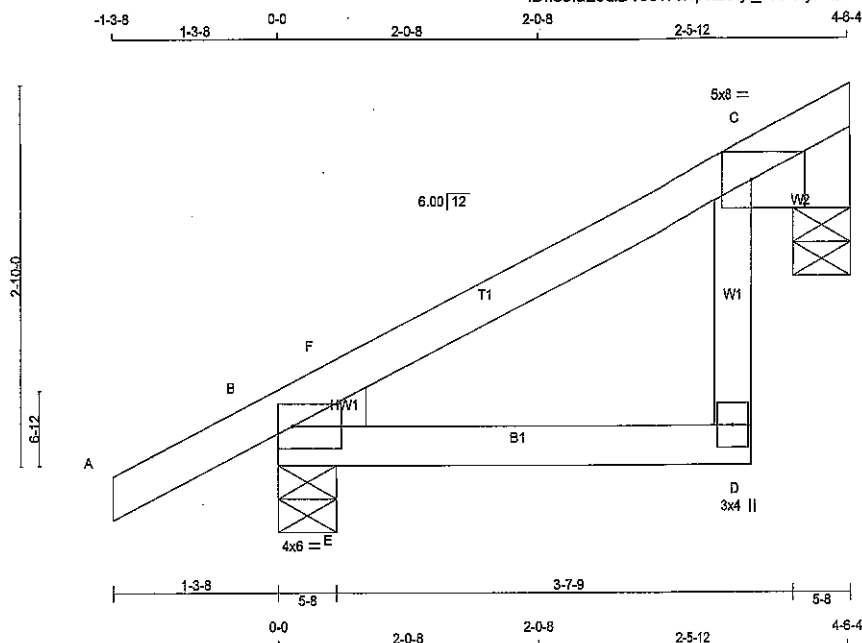
DWG NO. TAM 50313-17  
STRUCTURAL  
COMPONENT ONLY

JOB NAME <b>272800</b>	TRUSS NAME <b>T7TCX</b>	QUANTITY <b>6</b>	PLY <b>1</b>	JOB DESC. 42067 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:ls5la25dID168TP21p32xYl\_WY-Hyb6XcHKJ3lg9ZoD6Mn?RUmehK5iYhrL5fyHB4ySZbY



TOTAL WEIGHT = 6 X 16 = 99 lb

#### LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		Edge
C	TMBVW1-p	MT20	5.0	8.0	Edge	2.75
D	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

##### BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL  
WEDGE  
2x4 L

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C  
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

##### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

##### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)	UNBRACED LENGTH FR-TO
FR-TO		FROM	TO	FR-TO			
A-B	0/25	-122.2	-122.2	0.16 (1)	10.00	E-F	-224/82
B-F	-58/0	-122.2	-122.2	0.06 (3)	6.25		
F-C	0/6	-122.2	-122.2	0.22 (1)	10.00		
D-C	0/93	0.0	0.0	0.01 (2)	10.00		
B-E	0/0	-28.0	-28.0	0.17 (1)	10.00		
E-D	0/0	-28.0	-28.0	0.17 (1)	10.00		

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT. CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	58.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, CBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")  
ALLOWABLE DEFL.(TL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.22 (C-F:1), BC=0.17 (D-E:1), WB=0.00 (E-F:1), SSI=0.19 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (B) (INPUT = 0.80)  
JSI METAL= 0.05 (B) (INPUT = 1.00)



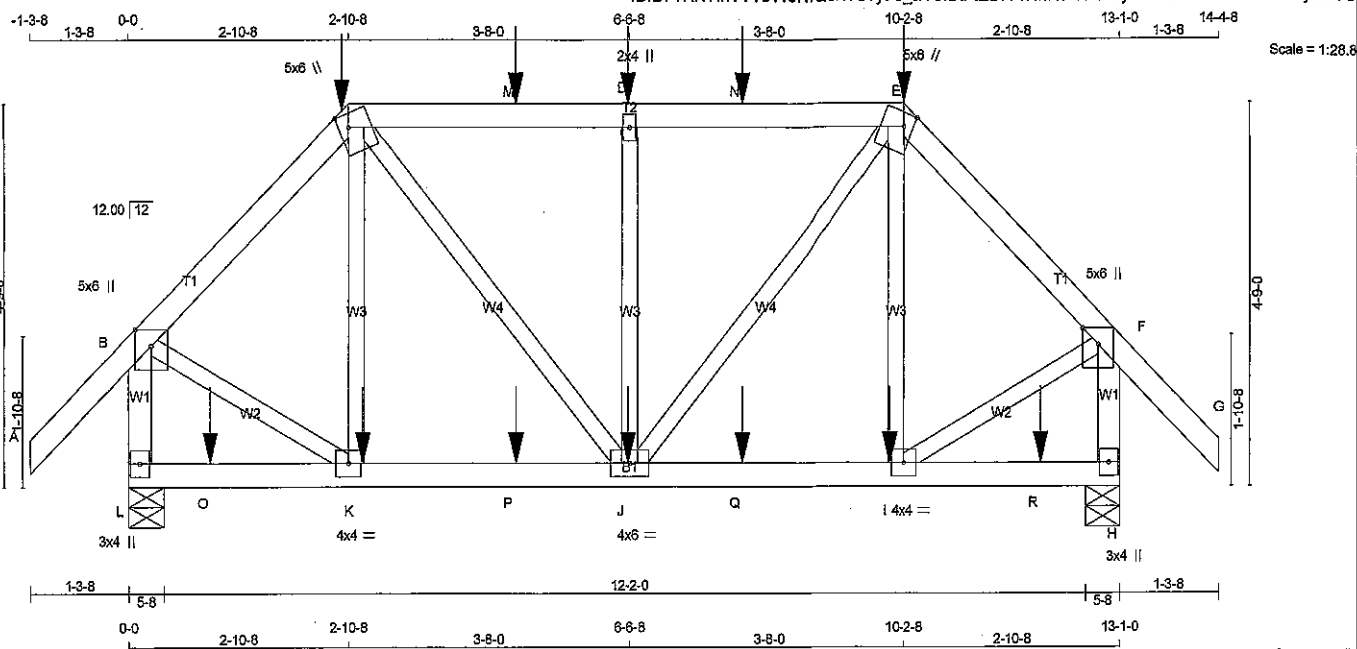
DRWG NO. TAM 52314-17  
STRUCTURAL  
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272801	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: B7KITHnV78W5NKz8NV3Yy78\_a-AxCIFEB7PrHnKTsNLK0y0ucAZCAa3fMDuNHsxySZ9C



TOTAL WEIGHT = 64 lb

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 137.6 lbs FACTORED DOWN AT 10-2-8, 137.6 lbs FACTORED DOWN AT 2-10-8, 28.0 lbs FACTORED DOWN AT 5-0-12, AND 28.0 lbs FACTORED DOWN AT 6-6-4, AND 28.0 lbs FACTORED DOWN AT 8-0-4 ON TOP CHORD, AND 21.4 lbs FACTORED DOWN AT 1-0-12, 11.2 lbs FACTORED DOWN AT 3-0-12, 11.2 lbs FACTORED DOWN AT 5-0-12, 11.2 lbs FACTORED DOWN AT 6-6-4, 11.2 lbs FACTORED DOWN AT 8-0-4, AND 11.2 lbs FACTORED DOWN AT 10-0-4, AND 21.4 lbs FACTORED DOWN AT 12-0-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

##### BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1361	0	1361	0
H	1361	0	1361	0

##### UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1041	717 / 0	159 / 0	0 / 0	0 / 0	165 / 0	0 / 0
H	1041	717 / 0	159 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

##### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.92 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

##### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 60	-122.2	-122.2	0.19 (1)	10.00	K-C	-207 / 85	0.07 (1)
B-C	-913 / 0	-122.2	-122.2	0.21 (1)	6.17	C-J	0 / 462	0.11 (1)
C-M	-931 / 0	-122.2	-122.2	0.33 (1)	5.92	J-D	-607 / 0	0.21 (1)
M-D	-931 / 0	-122.2	-122.2	0.33 (1)	5.92	J-E	0 / 462	0.11 (1)
D-N	-931 / 0	-122.2	-122.2	0.33 (1)	5.92	I-E	-207 / 85	0.07 (1)
N-E	-931 / 0	-122.2	-122.2	0.33 (1)	5.92	B-K	0 / 721	0.18 (1)
E-F	-913 / 0	-122.2	-122.2	0.21 (1)	6.17	I-F	0 / 721	0.18 (1)
F-G	0 / 60	-122.2	-122.2	0.19 (1)	10.00			
L-B	-1323 / 0	0.0	0.0	0.16 (1)	7.00			
H-F	-1323 / 0	0.0	0.0	0.16 (1)	7.00			
L-O	0 / 0	-28.0	-28.0	0.09 (2)	10.00			
O-K	0 / 0	-28.0	-28.0	0.09 (2)	10.00			
K-P	0 / 637	-28.0	-28.0	0.16 (1)	10.00			
P-J	0 / 637	-28.0	-28.0	0.16 (1)	10.00			
J-Q	0 / 637	-28.0	-28.0	0.16 (1)	10.00			
Q-I	0 / 637	-28.0	-28.0	0.16 (1)	10.00			
I-R	0 / 0	-28.0	-28.0	0.09 (2)	10.00			
R-H	0 / 0	-28.0	-28.0	0.09 (2)	10.00			

##### FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	2-10-8	-31	-31	-	FRONT	VERT	TOTAL
C	2-10-8	-31	-31	-	FRONT	VERT	TOTAL
C	2-10-8	-99	-99	-	FRONT	VERT	TOTAL
D	6-6-4	-28	-28	-	FRONT	VERT	TOTAL
E	10-2-8	-8	-8	-	FRONT	VERT	TOTAL
E	10-2-8	-31	-31	-	FRONT	VERT	TOTAL
E	10-2-8	-99	-99	-	FRONT	VERT	TOTAL
I	10-0-4	-6	-11	-	FRONT	VERT	TOTAL
J	6-6-4	-6	-11	-	FRONT	VERT	TOTAL
K	3-0-12	-6	-11	-	FRONT	VERT	TOTAL
M	5-0-12	-28	-28	-	FRONT	VERT	TOTAL
N	8-0-4	-28	-28	-	FRONT	VERT	TOTAL
O	1-0-12	-12	-21	-	FRONT	VERT	TOTAL
P	5-0-12	-6	-11	-	FRONT	VERT	TOTAL
Q	8-0-4	-6	-11	-	FRONT	VERT	TOTAL
R	12-0-4	-12	-21	-	FRONT	VERT	TOTAL

#### DESIGN CRITERIA

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*  
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.  
LOADS WERE DERIVED FROM USER INPUT  
NO FURTHER MODIFICATIONS WERE MADE

##### SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	58.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

\*\*\* NON STANDARD GIRDER \*\*\*  
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =  $L/360$  (0.44")  
CALCULATED VERT. DEFL.(LL) =  $L/999$  (0.02")  
ALLOWABLE DEFL.(TL) =  $L/360$  (0.44")  
CALCULATED VERT. DEFL.(TL) =  $L/999$  (0.03")

CSI: TC=0.33 (D-E:1), BC=0.16 (I-J:1), WB=0.21 (D-J:1), SS=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (I) (INPUT = 0.90)  
JSI METAL= 0.24 (K) (INPUT = 1.00)

DRWG NO. TAM 2320-17  
STRUCTURAL  
COMPONENT ONLY

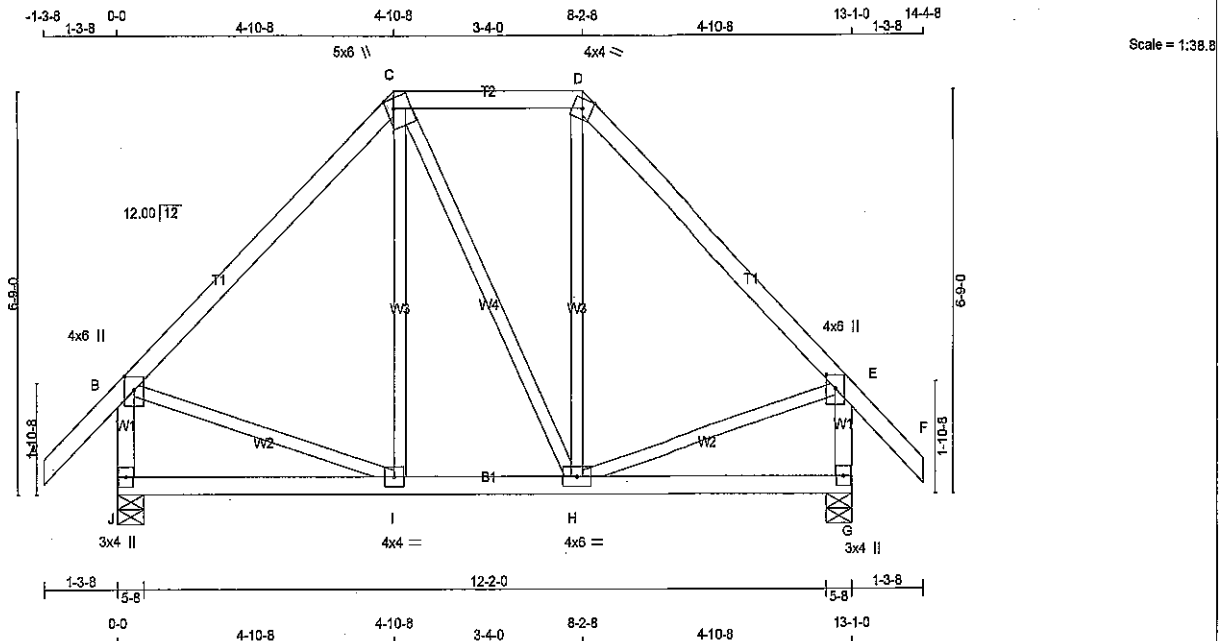


JOB NAME <b>272801</b>	TRUSS NAME <b>T9</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>TRUSS DESC.</b>	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:B7TKITHnV?T8W5NKz8NV3Yy78\_a-AxCiFIEB7PrHnKtSNLKOyDucNZCxa5AMDuNHsxySZ9C



TOTAL WEIGHT = 66 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.75 2.00
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TTW-m	MT20	4.0	4.0	Edge
E	TMVW+p	MT20	4.0	6.0	2.75 2.00
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-l	MT20	4.0	6.0	
I	BMVWW-l	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
J	1153	0	1153	0	0	5-8	5-8	5-8	5-8
G	1153	0	1153	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
J	884	607 / 0	137 / 0	0 / 0	139 / 0
G	884	607 / 0	137 / 0	0 / 0	139 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		CHORDS		WEBS	
TOTAL LOAD CASES: (4)		MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
		FR-TO	FROM TO	FR-TO	FROM TO
		A-B	0 / 60	I-C	-22 / 159
		B-C	-676 / 0	C-H	0 / 1
		C-D	-476 / 0	H-D	-22 / 160
		D-E	-676 / 0	B-I	0 / 498
		E-F	0 / 60	H-E	0 / 498
		J-B	-1094 / 0		
		G-E	-1094 / 0		
		J-I	0 / 0		
		I-H	0 / 476		
		H-G	0 / 0		

#### DESIGN CRITERIA

**SPECIFIED LOADS:**  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")  
ALLOWABLE DEFL.(TL) = L/360 (0.44")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.38 (B-C:1), BC=0.16 (H-I:2), WB=0.11 (B-I:1), SSI=0.16 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1657 822 2284 1658

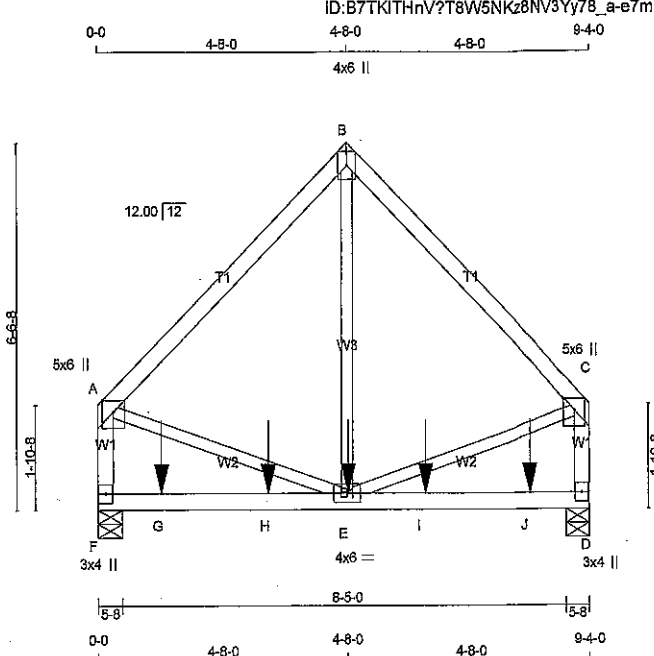
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)  
JSI METAL= 0.20 (B) (INPUT = 1.00)



DWONG, TAM 5234-17  
STRUCTURAL  
COMPONENT ONLY



Scale = 1:41.3

TOTAL WEIGHT = 42 lb

**LUMBER**

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2 SPF
B - C	2x4	DRY	No.2 SPF
F - A	2x4	DRY	No.2 SPF
D - C	2x4	DRY	No.2 SPF
F - D	2x4	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTW+p	MT20	4.0	6.0		Edge
C	TMVW+p	MT20	5.0	6.0	Edge	
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**HANGERS NOTES**

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 156.6 lbs FACTORED DOWN AT 1-2-4, 156.6 lbs FACTORED DOWN AT 3-2-4, 156.6 lbs FACTORED DOWN AT 4-8-4, AND 156.6 lbs FACTORED DOWN AT 6-1-12, AND 156.6 lbs FACTORED DOWN AT 8-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	1093	0	1093	0	5-8	5-8
D	1093	0	1093	0	5-8	5-8

**UNFACTORED REACTIONS**

JT	COMBINED	1ST LCASE MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	822	592 / 0	110 / 0	0 / 0	0 / 0	120 / 0	0 / 0
D	822	592 / 0	110 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED CSI (LC)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO				
A-B	-679 / 0	-122.2	-122.2	0.54 (1)	6.24	E-B	0 / 390	0.10 (1)	
B-C	-679 / 0	-122.2	-122.2	0.54 (1)	6.24	A-E	0 / 505	0.13 (1)	
F-A	-922 / 0	0.0	0.0	0.11 (1)	7.81	E-C	0 / 505	0.13 (1)	
D-C	-922 / 0	0.0	0.0	0.11 (1)	7.81				
F-G	0 / 0	-28.0	-28.0	0.43 (1)	10.00				
G-H	0 / 0	-28.0	-28.0	0.43 (1)	10.00				
H-E	0 / 0	-28.0	-28.0	0.43 (1)	10.00				
E-I	0 / 0	-28.0	-28.0	0.43 (1)	10.00				
I-J	0 / 0	-28.0	-28.0	0.43 (1)	10.00				
J-D	0 / 0	-28.0	-28.0	0.43 (1)	10.00				

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	4-8-4	-157	-157	---	FRONT	VERT	TOTAL
G	1-2-4	-157	-157	---	FRONT	VERT	TOTAL
H	3-2-4	-157	-157	---	FRONT	VERT	TOTAL
I	6-1-12	-157	-157	---	FRONT	VERT	TOTAL
J	8-1-12	-157	-157	---	FRONT	VERT	TOTAL

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, CBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.31")  
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")  
ALLOWABLE DEFL. (TL) = L/360 (0.31")  
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.54 (A-B-1), BC=0.43 (E-F-1), WB=0.13 (A-E-1), SSI=0.23 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

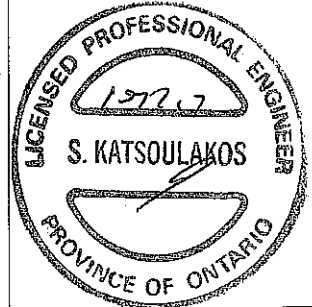
**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1657
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (E) (INPUT = 0.80)  
JSI METAL = 0.14 (A) (INPUT = 1.00)



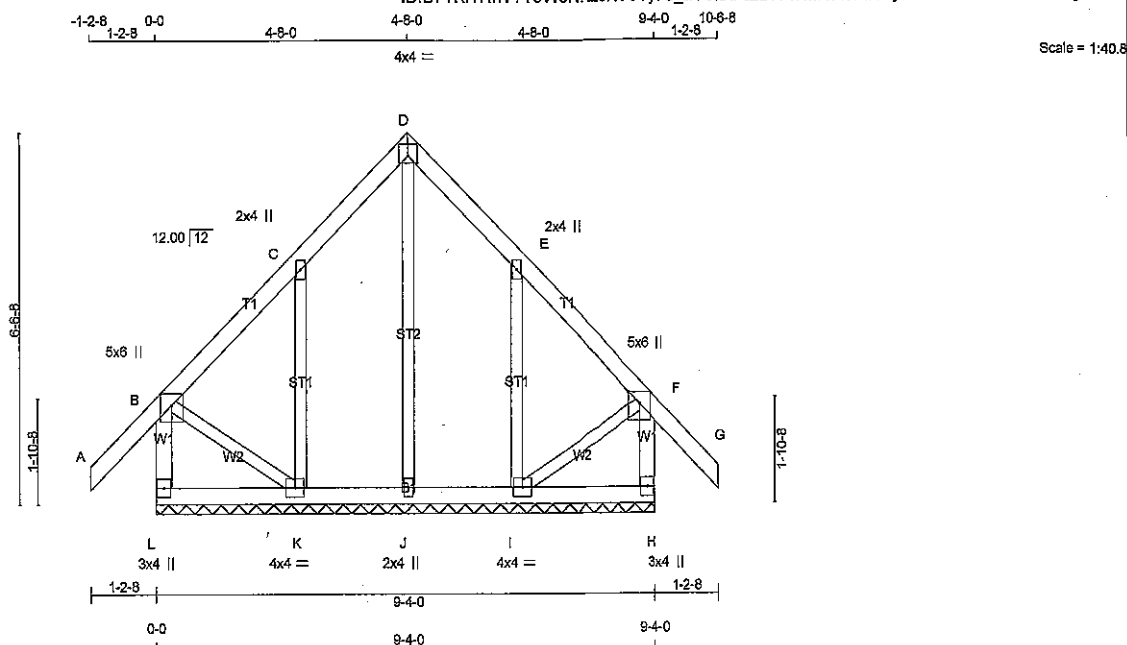
DRWG NO. TAMS2322-17  
STRUCTURAL  
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
272801	G10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:B7TKITHnV?T8W5NKz8NV3Yy78\_a-AxCfIEB7PrHnKtSLNK0y0ufxZE0a5HMDuNHsxySZ9C



TOTAL WEIGHT = 49 lb

#### LUMBER

##### N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

#### PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMV1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

##### BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

##### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

##### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX (LC)	
FR-TO		FROM TO		FR-TO			
L-B	-325 / 0	0.0	0.0 0.04 (1)	J-D	-144 / 0	0.10 (1)	
A-B	0 / 57	-122.2	-122.2 0.15 (1)	K-C	-348 / 0	0.11 (1)	
B-C	-14 / 0	-122.2	-122.2 0.11 (1)	I-E	-348 / 0	0.11 (1)	
C-D	-52 / 0	-122.2	-122.2 0.11 (1)	B-K	0 / 30	0.01 (1)	
D-E	-52 / 0	-122.2	-122.2 0.11 (1)	I-F	0 / 30	0.01 (1)	
E-F	-14 / 0	-122.2	-122.2 0.11 (1)				
F-G	0 / 57	-122.2	-122.2 0.15 (1)				
H-F	-325 / 0	0.0	0.0 0.04 (1)				
L-K	0 / 0	-28.0	-28.0 0.05 (3)				
K-J	0 / 17	-28.0	-28.0 0.05 (2)				
J-I	0 / 17	-28.0	-28.0 0.05 (2)				
I-H	0 / 0	-28.0	-28.0 0.05 (3)				

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

##### SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15 (F-G:1), BC=0.05 (J-K:2), WB=0.11 (E-I:1), SSI=0.10 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (D) (INPUT = 0.90 )  
JSI METAL= 0.10 (C) (INPUT = 1.00 )



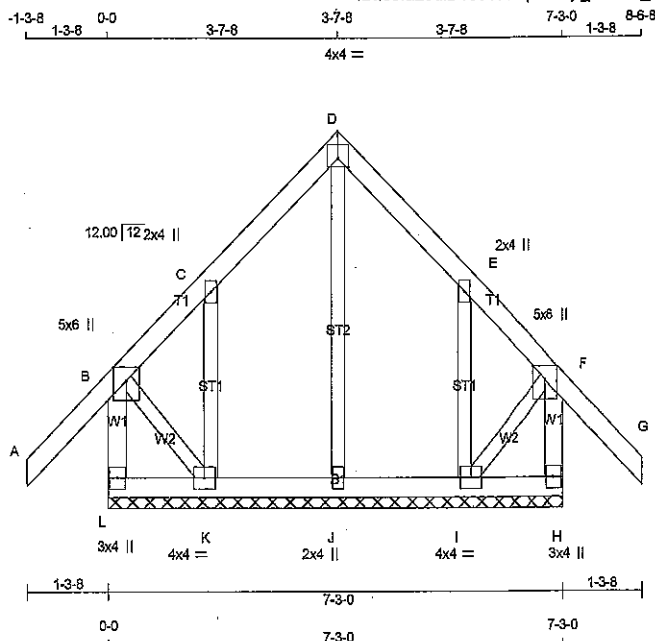
DWG NO. TAMS2323-17  
STRUCTURAL  
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
288177	G71	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:ls5la25dID168TP21p32xXyl\_WY-57\_wRVOz86BU2CmzWav3vZ8SzDetAhI\_JPIUw?ySZ3q



Scale = 1:34.8

TOTAL WEIGHT = 40 lb

#### LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF

ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMVW1+w	MT20	2.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

#### BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LBS)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 (LBS)
FR-TO		FROM TO		FR-TO			
L-B	-204 / 0	0.0 0.0	0.02 (1)	7.81	J-D	-118 / 0	0.05 (1)
A-B	0 / 35	-70.2 -70.2	0.10 (1)	10.00	K-C	-99 / 0	0.02 (1)
B-C	-38 / 0	-70.2 -70.2	0.09 (1)	6.25	I-E	-99 / 0	0.02 (1)
C-D	-15 / 0	-70.2 -70.2	0.04 (1)	6.25	B-K	0 / 14	0.00 (1)
D-E	-15 / 0	-70.2 -70.2	0.04 (1)	6.25	I-F	0 / 14	0.00 (1)
E-F	-38 / 0	-70.2 -70.2	0.09 (1)	6.25			
F-G	0 / 35	-70.2 -70.2	0.10 (1)	10.00			
H-F	-204 / 0	0.0 0.0	0.02 (1)	7.81			
L-K	0 / 0	-28.0 -28.0	0.02 (3)	10.00			
K-J	0 / 7	-28.0 -28.0	0.02 (2)	10.00			
J-I	0 / 7	-28.0 -28.0	0.02 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.02 (3)	10.00			

#### DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	20.8	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	41.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

#### DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(87 % OF 18.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (F-G:1), BC=0.02 (I-J:2), WB=0.05 (D-J:1), SS=0.05 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (D) (INPUT = 0.90)  
JSI METAL= 0.03 (E) (INPUT = 1.00)



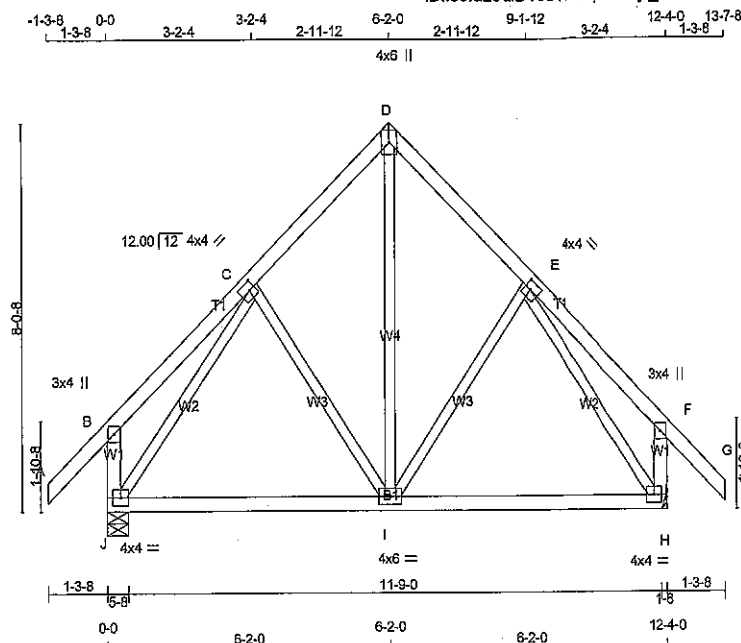
DWG NO. TAM 52324-17  
STRUCTURAL  
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
289587	T200	3	1	TRUSS DESC.		

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Scale: 1/4"=1'

TOTAL WEIGHT = 3 X 85 = 195 lb

#### LUMBER

##### N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2  
EXCEPT

DRY: SEASONED LUMBER.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

##### BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	1097	0	1097	0
H	1097	0	1097	0

##### UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
J	839	578 / 0	130 / 0
H	839	578 / 0	130 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

##### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

##### LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT.	LOAD	LC1	MAX.	CS1	LC1	UNBRAC	WEBS	MEMB.	MAX. FACTORED	FORCE	VERT.	LOAD	LC1	MAX.	CS1	LC1
FR-TO											FR-TO									
A-B	0 / 60	-122.2	-122.2	0.17	(1)	10.00	I-D	0 / 478	0.11	(1)										
B-C	0 / 31	-122.2	-122.2	0.18	(1)	10.00	I-E	-160 / 36	0.08	(1)										
C-D	-578 / 0	-122.2	-122.2	0.14	(1)	6.25	C-I	-160 / 36	0.08	(1)										
D-E	-578 / 0	-122.2	-122.2	0.14	(1)	6.25	J-C	-862 / 0	0.43	(1)										
E-F	0 / 31	-122.2	-122.2	0.18	(1)	10.00	E-H	-862 / 0	0.43	(1)										
F-G	0 / 60	-122.2	-122.2	0.17	(1)	10.00														
J-B	-313 / 0	0.0	0.0	0.03	(1)	7.81														
H-F	-313 / 0	0.0	0.0	0.03	(1)	7.81														
J-I	0 / 478	-28.0	-28.0	0.35	(2)	10.00														
I-H	0 / 478	-28.0	-28.0	0.35	(2)	10.00														

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL	=	38.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014  
- CSA 088-09  
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")  
ALLOWABLE DEFL.(TL) = L/360 (0.41")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.18 (E-F:1), BC=0.35 (H-I:2), WB=0.43 (E-H:1), SSI=0.15 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1887	822

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)  
JSI METAL= 0.33 (C) (INPUT = 1.00)



DRWG NO. TAM 52318.17  
STRUCTURAL  
COMPONENT ONLY

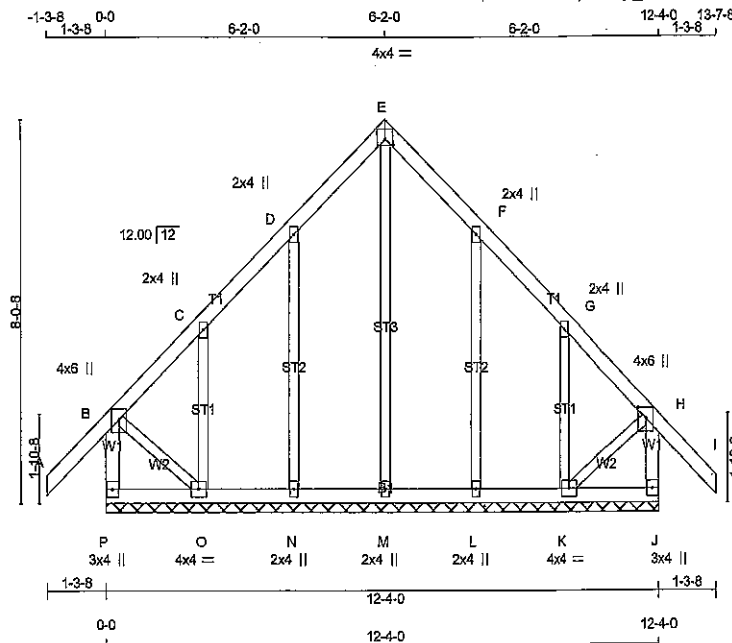


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
289587	G200	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:ls5la25dID168TP21p32xXyl\_WY-xbvvtQM0cuCKYZlbpv77gEFy5W1AOGSbnL\_?IEySZFU



Scale = 1:48.6

TOTAL WEIGHT = 66 lb

#### LUMBER

##### N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

#### PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.75	2.00
C, D, F, G					
C TMVW+w	MT20	2.0	4.0		
E TTVW-p	MT20	4.0	4.0	1.50	2.00
H TMVW+p	MT20	4.0	6.0	2.75	2.00
J BMV1+p	MT20	3.0	4.0		
K BMVW1-t	MT20	4.0	4.0		
L, M, N					
L BMV1+w	MT20	2.0	4.0		
O BMVW1-t	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

##### BUILDING DESIGNER

##### BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

##### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

##### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (CSI (LC))	MEMB.	MAX. FACTORED FORCE (LBS)	UNBRACED LENGTH (FT)	LC1 MAX. (CSI (LC))
FR-TO		FROM TO		FR-TO			
P-B	-336 / 0	0.0	0.0 0.04 (1)	M-E	-160 / 0	7.81	0.19 (1)
A-B	0 / 60	-122.2	-122.2 0.17 (1)	N-D	-269 / 0	10.00	0.15 (1)
B-C	-36 / 0	-122.2	-122.2 0.07 (1)	O-C	-266 / 0	6.25	0.07 (1)
C-D	-42 / 0	-122.2	-122.2 0.07 (1)	L-F	-269 / 0	6.25	0.15 (1)
D-E	-52 / 0	-122.2	-122.2 0.07 (1)	K-G	-266 / 0	6.25	0.07 (1)
E-F	-52 / 0	-122.2	-122.2 0.07 (1)	B-O	0 / 46	6.25	0.01 (1)
F-G	-42 / 0	-122.2	-122.2 0.07 (1)	K-H	0 / 46	6.25	0.01 (1)
G-H	-36 / 0	-122.2	-122.2 0.07 (1)				
H-I	0 / 60	-122.2	-122.2 0.17 (1)				
J-H	-336 / 0	0.0	0.0 0.04 (1)				
P-O	0 / 0	-28.0	-28.0 0.03 (2)				
O-N	0 / 30	-28.0	-28.0 0.03 (2)				
N-M	0 / 25	-28.0	-28.0 0.03 (2)				
M-L	0 / 25	-28.0	-28.0 0.03 (2)				
L-K	0 / 30	-28.0	-28.0 0.03 (2)				
K-J	0 / 0	-28.0	-28.0 0.03 (2)				

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL =	38.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	58.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (H-L1), BC=0.03 (K-L2), WB=0.19 (E-M1), SSI=0.09 (H-L1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (E) (INPUT = 0.90 )  
JSI METAL= 0.08 (D) (INPUT = 1.00 )

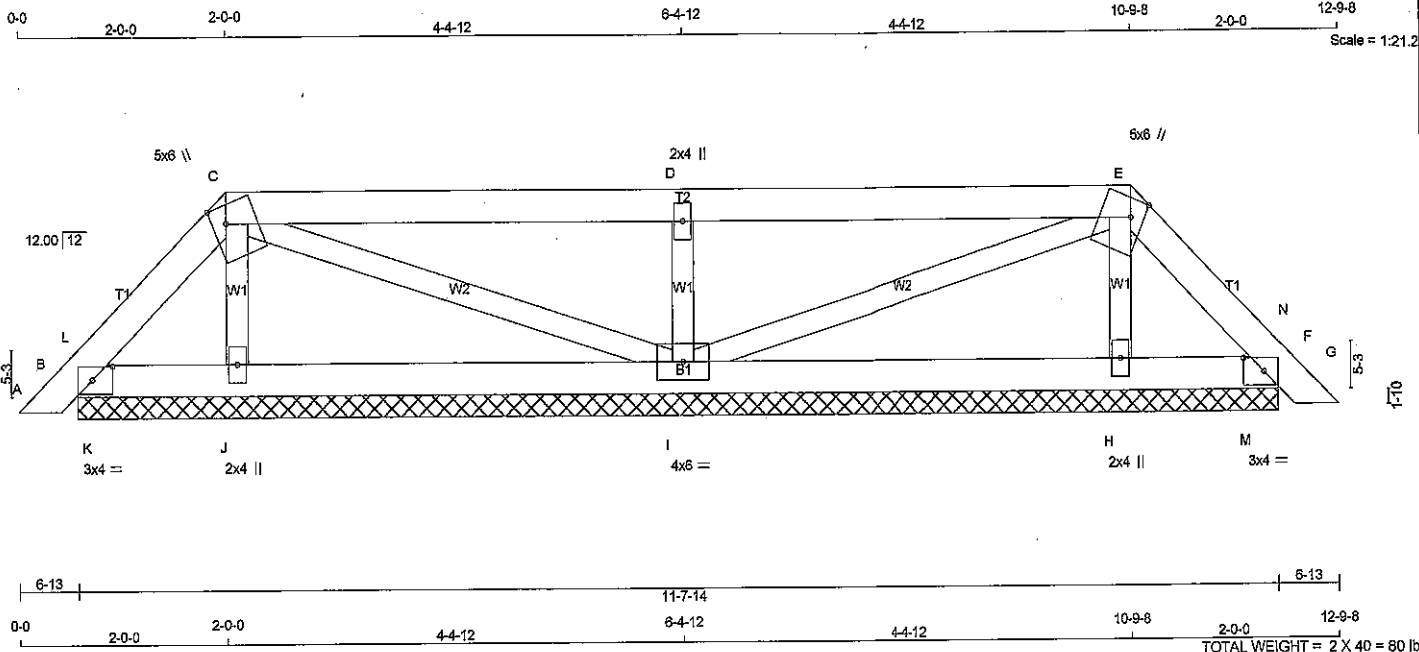


DWG NO. TAM523 (9-17)  
STRUCTURAL  
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42087	DRWG NO.
272800	P1	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mit Tek Industries, Inc. Tue Oct 17 09:20:24 2017 Page 1  
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY; SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
B	200	0	200	0
F	200	0	200	0
J	325	0	325	0
I	814	0	814	0
H	325	0	325	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	COMBINED							
B	134	126/0	0/-1	0/0	0/0	0/0	9/0	0/0
F	134	126/0	0/-1	0/0	0/0	0/0	9/0	0/0
J	278	140/0	76/0	0/0	0/0	0/0	61/0	0/0
I	623	430/0	95/0	0/0	0/0	0/0	97/0	0/0
H	278	140/0	76/0	0/0	0/0	0/0	61/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. LC2	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1
FR-TO							FR-TO			
A-B	0/18	-122.2	-122.2	0.02 (1)	10.00		J-C	-206/0	0.03 (1)	
B-L	-44/26	-122.2	-122.2	0.02 (1)	6.25		C-I	-28/0	0.01 (1)	
L-C	-81/0	-122.2	-122.2	0.02 (1)	6.25		I-D	-671/0	0.10 (1)	
C-D	-11/0	-122.2	-122.2	0.40 (1)	6.25		D-E	-28/0	0.01 (1)	
D-E	-11/0	-122.2	-122.2	0.40 (1)	6.25		E-F	-206/0	0.03 (1)	
E-N	-81/0	-122.2	-122.2	0.02 (1)	6.25		F-G	-133/0	0.00 (1)	
N-F	-44/26	-122.2	-122.2	0.02 (1)	6.25		G-H	-133/0	0.00 (1)	
F-G	0/18	-122.2	-122.2	0.02 (1)	10.00					
B-K	0/52	-28.0	-28.0	0.04 (1)	10.00					
K-J	0/52	-28.0	-28.0	0.08 (2)	10.00					
J-I	0/38	-28.0	-28.0	0.12 (2)	10.00					
I-H	0/38	-28.0	-28.0	0.12 (2)	10.00					
H-M	0/52	-28.0	-28.0	0.08 (2)	10.00					
M-F	0/52	-28.0	-28.0	0.04 (1)	10.00					

DWG NO. TAM52315-17

STRUCTURAL

COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.3 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 58.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55% OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40 (C-D:1), BC=0.12 (I-J:2), WB=0.10 (D-I:1), SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

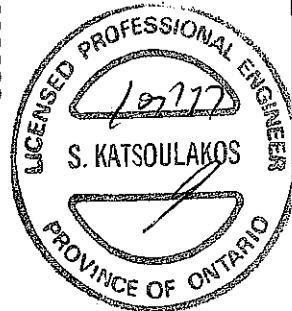
MT20 615 354 1667 522 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

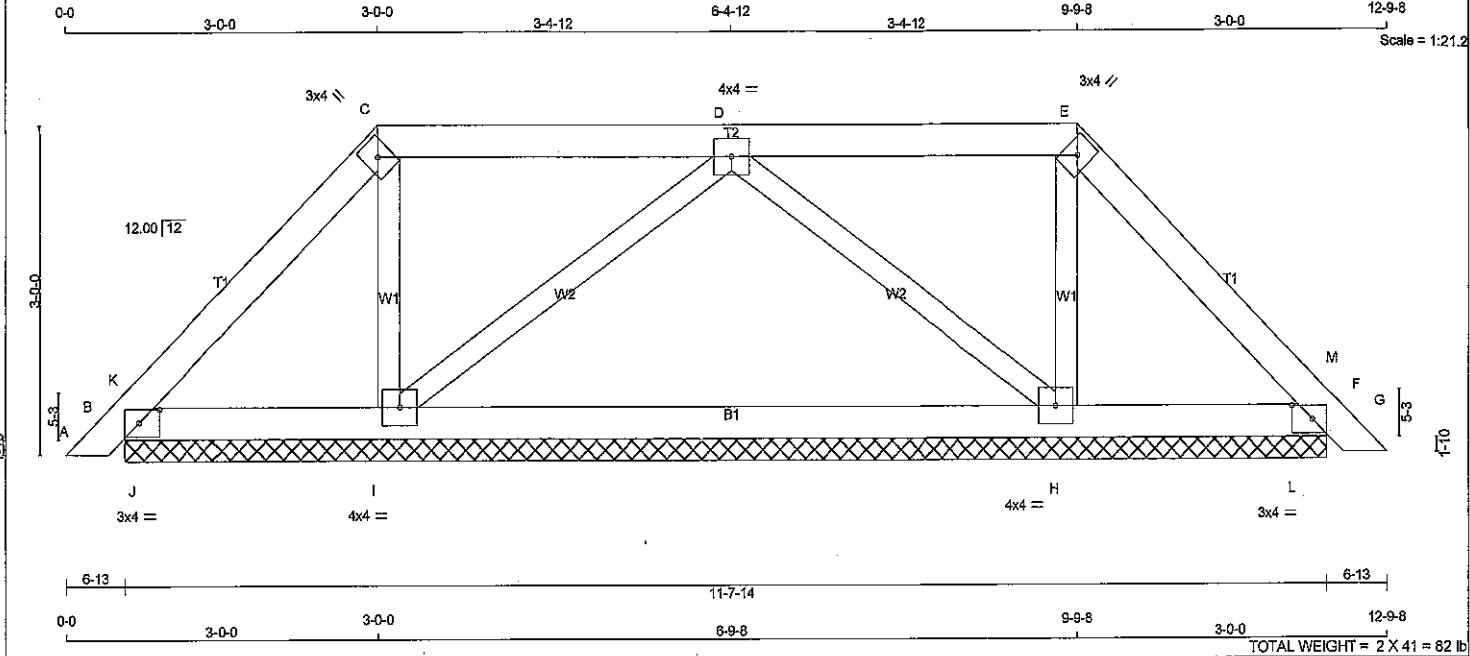
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (D) (INPUT = 0.90)

JSI METAL= 0.12 (D) (INPUT = 1.00)



DWG NO. TAM 52315-17  
STRUCTURAL  
COMPONENT ONLY



**LUMBER**  
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
B - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW+h	MT20	3.0	4.0		
D	TWW+h	MT20	4.0	4.0		
E	TTW+h	MT20	3.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMWW1-I	MT20	4.0	4.0		
I	BMWW1-I	MT20	4.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	226	0	226	0	0	11-7-14	11-7-14
F	226	0	226	0	0	11-7-14	11-7-14
I	708	0	708	0	0	11-7-14	11-7-14
H	708	0	708	0	0	11-7-14	11-7-14

**UNFACTORED REACTIONS**

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
B	150	144 / 0	0 / 0	9 / 0	0 / 0
F	150	144 / 0	0 / 0	9 / 0	0 / 0
I	573	337 / 0	0 / 0	110 / 0	0 / 0
H	573	337 / 0	0 / 0	110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 18	-122.2 -122.2	0.02 (1)	I-C	-258 / 0	0.04 (1)	
B-K	0 / 119	-122.2 -122.2	0.07 (1)	I-D	-417 / 0	0.12 (1)	
K-C	-40 / 6	-122.2 -122.2	0.08 (1)	D-H	-417 / 0	0.12 (1)	
C-D	-8 / 9	-122.2 -122.2	0.23 (1)	H-E	-258 / 0	0.04 (1)	
D-E	-8 / 9	-122.2 -122.2	0.23 (1)	J-K	-346 / 0	0.00 (1)	
E-M	-40 / 6	-122.2 -122.2	0.08 (1)	L-M	-346 / 0	0.00 (1)	
M-F	0 / 119	-122.2 -122.2	0.07 (1)				
F-G	0 / 18	-122.2 -122.2	0.02 (1)				
B-J	-8 / 20	-28.0 -28.0	0.09 (1)				
J-I	-8 / 20	-28.0 -28.0	0.21 (2)				
I-H	0 / 338	-28.0 -28.0	0.23 (2)				
H-L	-8 / 20	-28.0 -28.0	0.21 (2)				
L-F	-8 / 20	-28.0 -28.0	0.09 (1)				

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 38.3 PSF  
DL = 3.0 PSF  
BOT CH. LL = 10.5 PSF  
DL = 7.0 PSF  
TOTAL LOAD = 58.7 PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:  
- PART 9 OF OBC 2012, CBC 2012, ABC 2014  
- CSA 086-09  
- TPIC 2011

(55 % OF 54.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23 (C-D:1), BC=0.23 (H-I:2), WB=0.12 (D-I:1), SSI=0.25 (F-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

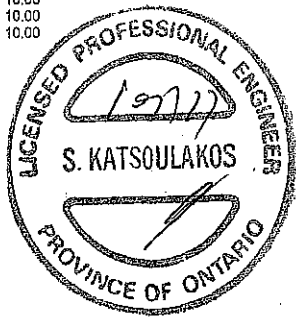
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90 )  
JSI METAL= 0.12 (H) (INPUT = 1.00 )



DWG NO. TAM 52316-17  
STRUCTURAL  
COMPONENT ONLY

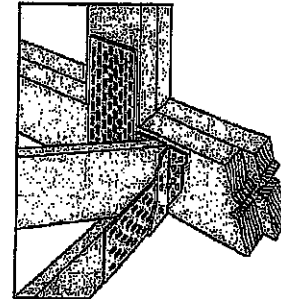
## FACE MOUNT HANGERS

**SIMPSON**  
**Strong-Tie**

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

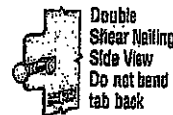
Model No.	Ga	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	dg <sup>2</sup>	Header	Joist	U-F-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K <sub>g</sub> = 1.15)	(K <sub>g</sub> = 1.00)	(K <sub>g</sub> = 1.15)	(K <sub>g</sub> = 1.00)
								lbs	lbs	lbs	lbs
								kN	kN	kN	kN
<b>DOUBLE 2x SIZES</b>											
LUS24-2	18	3 3/4	3 3/4	2	1 1/2	4-16d	2-16d	835	2020	590	1435
								9.71	8.99	2.62	6.38
LUS26-2	18	3 3/4	4 3/4	2	4	4-16d	4-16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 3/4	5 3/4	3	3 3/4	14-16d	6-16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 3/4	5 3/4	4	4 3/4	20-16d	8-16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3 3/4	7	2	4	6-16d	4-16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 3/4	7 3/4	3	5 3/4	22-16d	8-16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 3/4	7 3/4	4	6 3/4	36-16d	12-16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 3/4	9	2	6	8-16d	8-16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 3/4	9 3/4	3	8	30-16d	10-16d	4745	9650	4310	7000
								21.11	42.97	19.17	31.14
HGUS210-2	12	3 3/4	9 3/4	4	8 3/4	46-16d	16-16d	6840	14845	4855	10400
								30.43	65.14	21.60	46.26
<b>TRIPLE 2x SIZES</b>											
HGUS26-3	12	4 3/4	5 3/4	4	4 3/4	20-16d	8-16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 3/4	7 3/4	4	6 3/4	36-16d	12-16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/4	9	3	7 3/4	30-16d	10-16d	4745	10545	4310	7485
								21.11	46.91	19.17	33.29
HGUS210-3	12	4 1/4	9 3/4	4	8 3/4	46-16d	16-16d	6840	14845	4855	10400
								30.43	65.14	21.60	46.26
<b>QUADRUPLE 2x SIZES</b>											
HGUS26-4	12	5 3/4	5 3/4	4	4 3/4	20-16d	8-16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	5 3/4	7 3/4	4	6 3/4	36-16d	12-16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 3/4	8 3/4	3	7 3/4	30-16d	10-16d	4745	10545	4310	7485
								21.11	46.91	19.17	33.29
HGUS210-4	12	6 3/4	9 3/4	4	8 3/4	46-16d	16-16d	6840	14845	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 3/4	10 3/4	4	10 3/4	56-16d	20-16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 3/4	12 3/4	4	11 3/4	66-16d	22-16d	10130	16400	7195	11645
								45.08	72.95	32.00	51.80
<b>4x SIZES</b>											
LUS46	18	3 3/4	4 3/4	2	3 3/4	4-16d	4-16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 3/4	5 3/4	3	3 3/4	14-16d	6-16d	2540	7335	2065	5205
								11.90	32.63	9.20	23.15
HGUS46	12	3 3/4	5 3/4	4	4 3/4	20-16d	8-16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 3/4	8 3/4	2	3 3/4	6-16d	4-16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 3/4	7 3/4	3	5 3/4	22-16d	8-16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 3/4	7 3/4	4	6 3/4	36-16d	12-16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 3/4	8 3/4	2	5 3/4	8-16d	8-16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 3/4	9	4	8 3/4	46-16d	16-16d	6840	14845	4855	10400
								30.43	65.14	21.60	46.26
HGUS412	12	3 3/4	10 3/4	4	10 3/4	56-16d	20-16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 3/4	12 3/4	4	11 3/4	66-16d	22-16d	10130	16400	7195	11645
								45.08	72.95	32.00	51.80



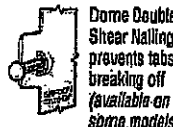
**Typical HUS26 Installation**  
 (Truss Designer to provide fastener quantity for connecting multiple members together)



Double Shear Nailing Top View



Double Shear Nailing Side View  
 Do not bend tab back



Dome Double Shear Nailing prevents tabs breaking off (available on some models)



1. Factored uplift resistance has been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced truss height is used.
3. dg is the distance from the bearing seat to the top joist cell.
4. Resistances shown require a minimum 2-ply glider truss. For fastening to single-ply truss request technical bulletin T-410FORTRUSS and/or see installation notes.
5. NAILS: 16d = 0.162" dia. x 3 1/2" long. See pages 22-23 for other nail sizes and information.

# Plated Truss Connectors

## LUL/LUS/LJS/HUS/HHUS/HGUS Standard & Double Shear Joist Hangers

**SIMPSON**  
**Strong-Tie**



This product is preferable to similar connectors because of  
a) easier installation, b) higher capacities, c) lower installed cost,  
or a combination of these features.

Most hangers in this series have double shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

**MATERIAL:** See tables below and on page 155

**FINISH:** Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

**INSTALLATION:** • Use all specified fasteners. See General Notes.

- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With 3x carrying members, use 16d x 2 1/4" nails into the header and 16d commons into the joist with no reduction in resistance. With 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value.

**OPTIONS:** • LUS and LUL hangers cannot be modified.

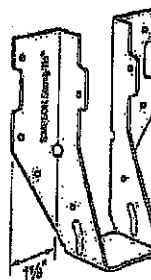
- HUS hangers available with the header flanges turned in for 2-2x (3 1/8") and 4x only, with no load reduction. See HUSC Concealed Flange Illustration.
- Concealed flanges are not available for HGUS.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See hanger options on page 230.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.



LUS28



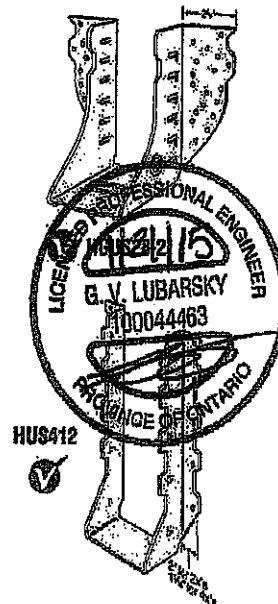
LU26L



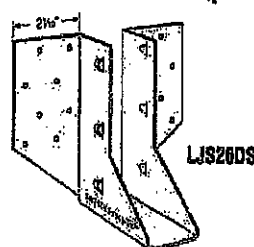
HUS210  
(HUS26, HUS28,  
and HHUS similar)



HUSC  
Concealed Flanges  
(not available for HHUS,  
HGUS and HUS2x)



HUS412



LJS26DS

Model No.	Ga.	Dimensions (in)				Fasteners		Factored Resistance			
		W	H	B	d <sub>s</sub> <sup>3</sup>	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K <sub>o</sub> = 1.15)	(K <sub>o</sub> = 1.00)	(K <sub>o</sub> = 1.15)	(K <sub>o</sub> = 1.00)
								lbs	lbs	lbs	lbs
SINGLE 2x SIZES											
LUS24	18	1 1/8	3 1/2	1 1/4	2 1/4	4-10d	2-10d	710	1825	645	1155
								336	723	287	514
LU24L	22	1 1/8	3	1 1/4	2 1/8	4-10d	2-10dx1 1/2	360	1020	320	725
								160	454	142	322
LU26L	22	1 1/8	5	1 1/4	4 1/4	6-10d	4-10dx1 1/2	720	1605	645	1140
								320	714	287	507
LUS26	18	1 1/8	4 1/2	1 1/4	3 3/4	4-10d	4-10d	1420	2170	1290	1630
								632	985	574	725
HUS26	16	1 1/8	5 1/4	3	3 1/8	14-16d	6-16d	2705	4940	2085	3875
								1130	2197	920	1724
LUS26DS	18	1 1/8	5	3 1/4	4 1/4	18-16d	6-16d	2055	4265	1460	4115
								914	1697	649	1931
HGUS26	12	1 1/8	5 1/4	5	4 1/4	20-16d	8-16d	2685	5825	2685	5700
								1196	2251	1196	2635
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	8-10d	6-10dx1 1/2	1140	2185	1020	1550
								507	972	454	689
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	8-10d	4-10d	1420	2520	1290	1790
								632	1121	574	786
HUS28	16	1 1/8	7 1/4	3	6 1/8	22-16d	8-16d	3605	5365	2675	4345
								7604	2388	1190	1933
HGUS28	12	1 1/8	7 1/4	5	6 1/4	36-16d	12-16d	3310	7675	3310	6900
								1474	3419	1474	3073
LU210L	20	1 1/8	8	1 1/4	7 1/4	10-10d	6-10dx1 1/2	1140	2495	1020	1770
								507	1110	454	787
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	8-10d	4-10d	1420	2785	1290	2210
								632	1239	574	983

See footnotes on page 155.

Catalogue C-C-202015 © 2015 SIMPSON STRONG-TIE COMPANY INC.

# HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

**MATERIAL:** See table

**FINISH:** G90 galvanized

**DESIGN:**

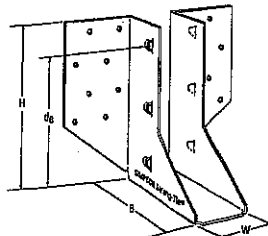
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%  
No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

**INSTALLATION:**

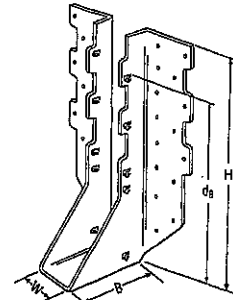
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

**OPTIONS:**

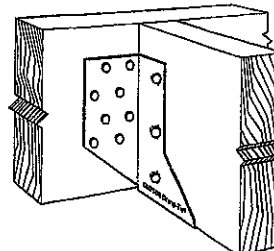
- See current catalogue for options



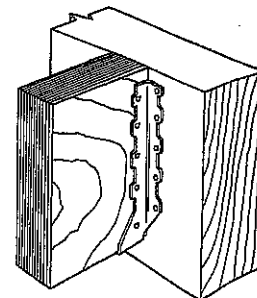
LJS26DS



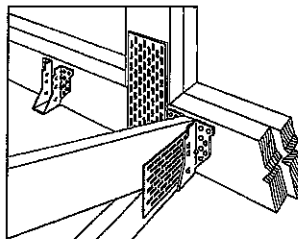
HUS210  
(HUS26, HUS28, similar)



Typical LJS26DS  
Installation



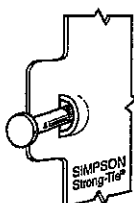
Typical HUS  
Installation



Typical HUS Installation  
(Truss Designer to provide fastener  
quantity for connecting multiple  
members together)

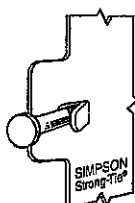
Model No.	Ga	Dimensions (in)			Fasteners			Factored Resistance (lbs)			
		W	H	B	d <sub>g</sub> <sup>1</sup>	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K <sub>o</sub> =1.15)	Normal (K <sub>o</sub> =1.00)	Uplift (K <sub>o</sub> =1.15)	Normal (K <sub>o</sub> =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 5/8	16-16d	6-16d	2055	4265	1480	4115
HUS26	16	1 1/8	5 3/8	3	3 15/16	14-16d	8-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d<sub>g</sub> is the distance from the seat of the hanger to the highest joist nail.

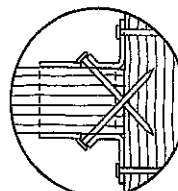


Dome Double  
Shear Nailing  
prevents tabs  
breaking off  
(available on  
some models).

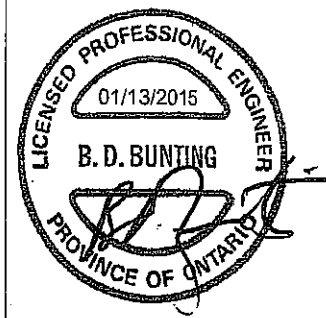
U.S. Patent  
5,603,580



Double  
Shear  
Nailing  
Side  
View. Do  
not bend  
tab back.



Double  
Shear  
Nailing  
Top View.



LIMIT  
STATES  
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see [www.strongtie.com](http://www.strongtie.com).

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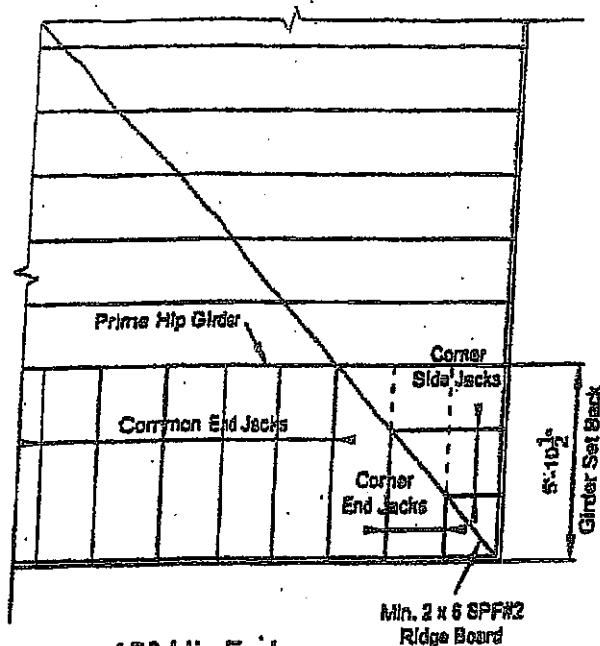
T-SPECHUS16 1/15 exp. 12/16

800-999-5099

[www.strongtie.com](http://www.strongtie.com)

# MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



**45° Hip End**

## LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

## DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

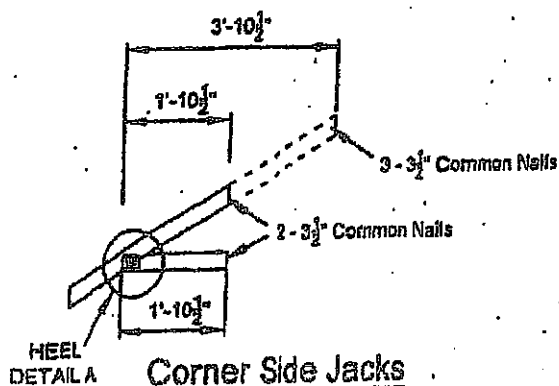
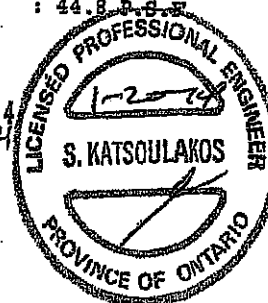
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

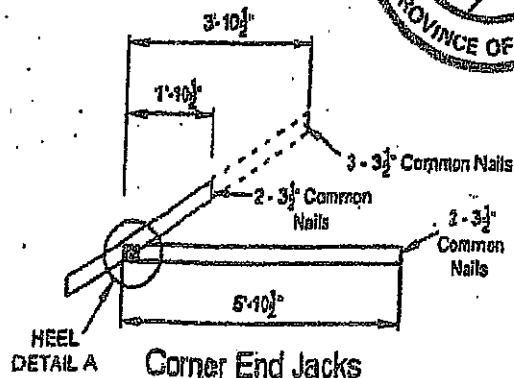
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

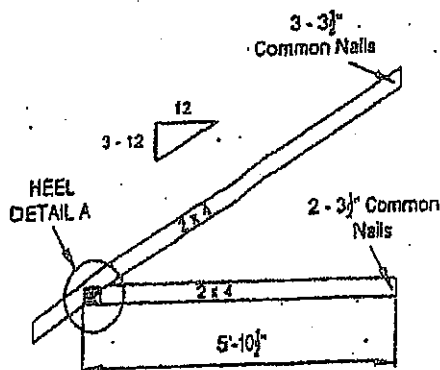
DWG NO TAM 3495.14  
STRUCTURAL  
COMPONENT ONLY



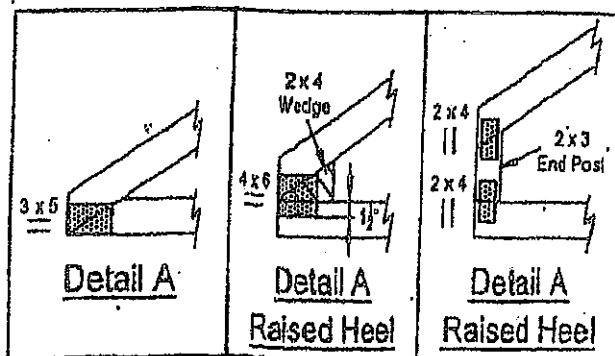
**Corner Side Jacks**



**Corner End Jacks**



**Common End Jacks**



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)  
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

# MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287-2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

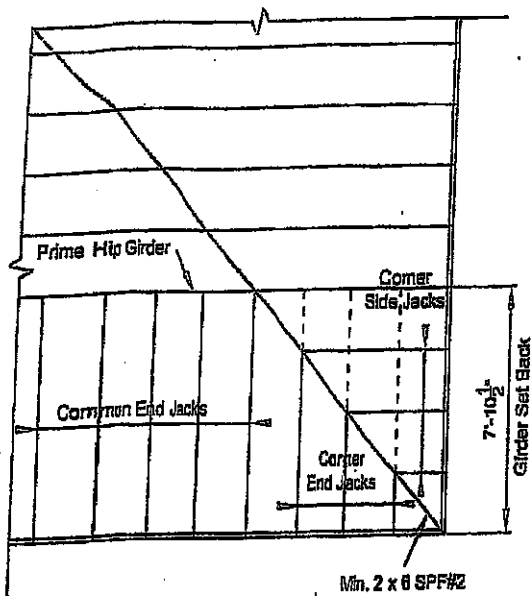
## LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2  
BOTTOM CHORD : 2 x 4 SPF#2  
WEBS : 2 x 3 SPF#2  
UNLESS OTHERWISE SHOWN

## DESIGN LOAD:

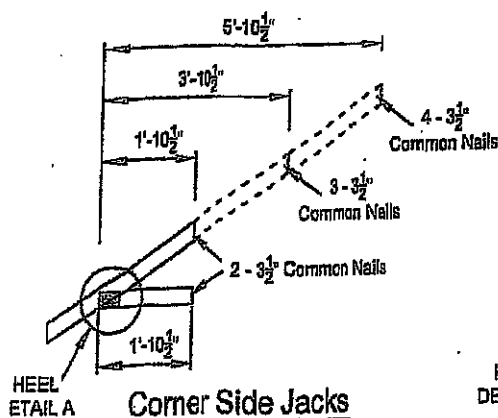
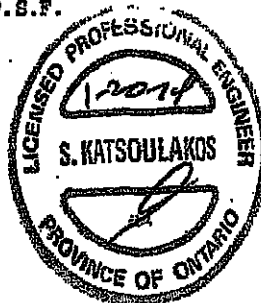
TOP CHORD LIVE LOAD : 34.8 P.S.F.  
TOP CHORD DEAD LOAD : 3.0 P.S.F.  
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.  
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

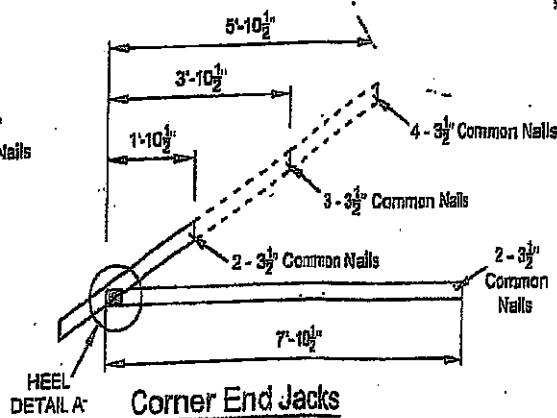


45° Hip End

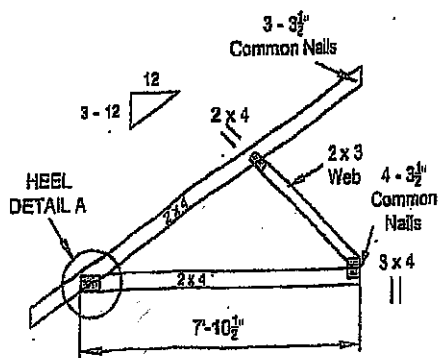
DWG NO T&M 350314  
STRUCTURAL  
COMPONENT ONLY



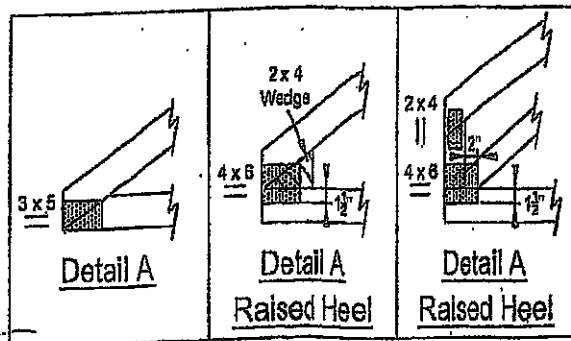
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)  
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

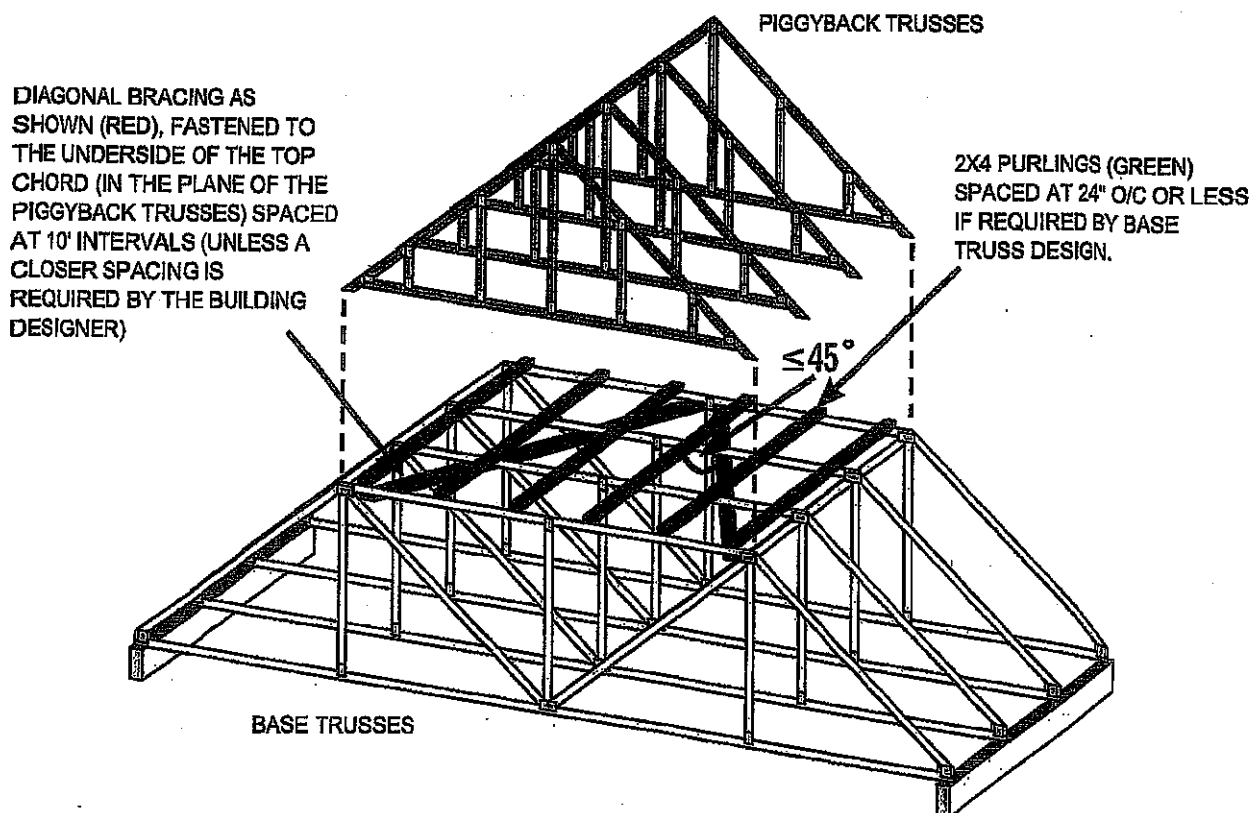


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

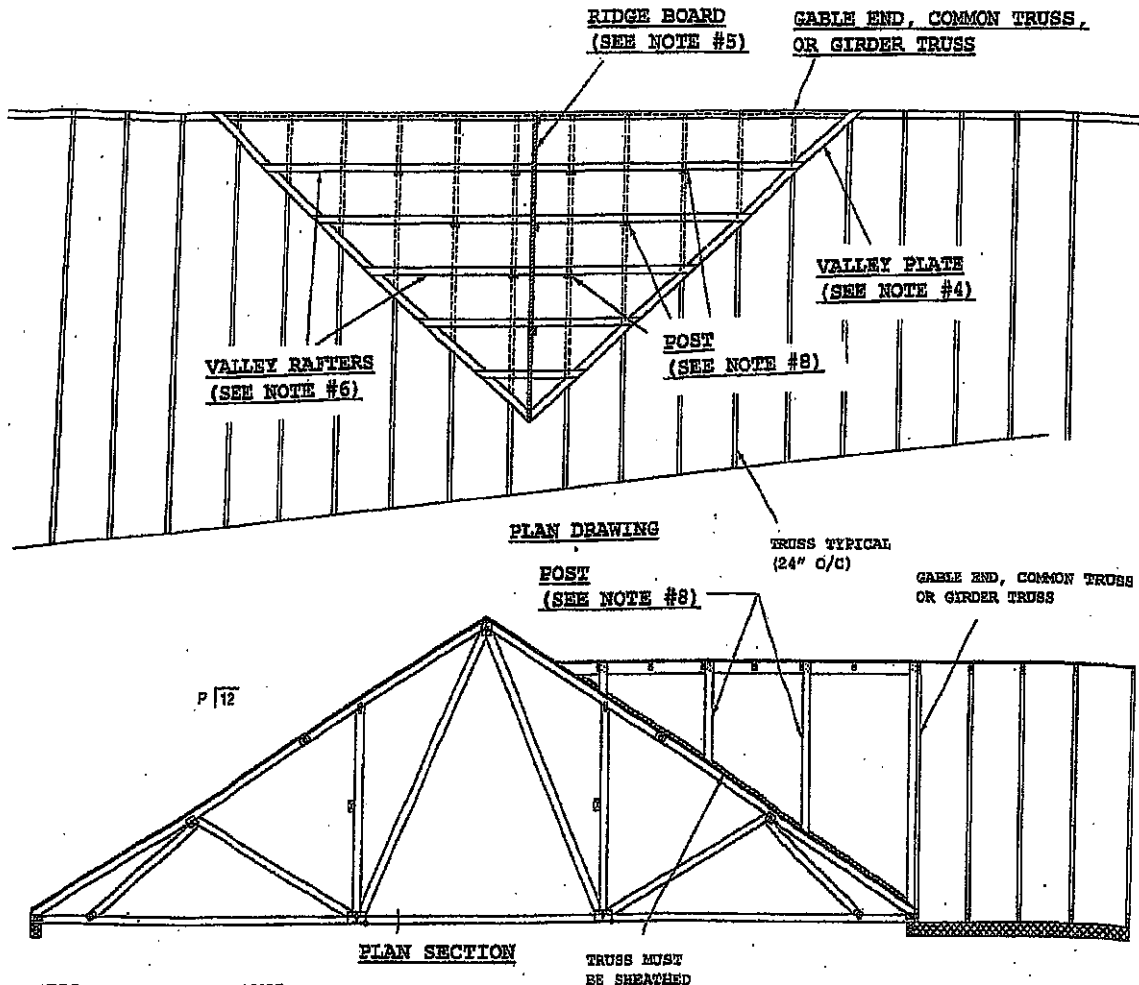
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# MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

## CONVENTIONAL VALLEY FRAMING DETAIL



### GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 x 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" x 0.131") NAILS.
- (5) SET A 2 x 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 x 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 x 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" x 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" x 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" x 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" x 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 x 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" x 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" x 0.131") NAILS.
- (8) POSTS SHALL BE 2 x 4 #2 SPF OR BETTER. POSTS EXCEEDING 75' IN HEIGHT SHALL BE INCREASED TO 4 x 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) FLY 2 x 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" x 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

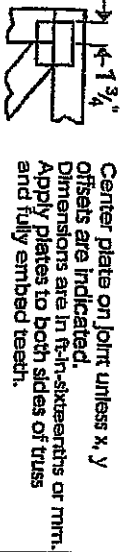
### NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

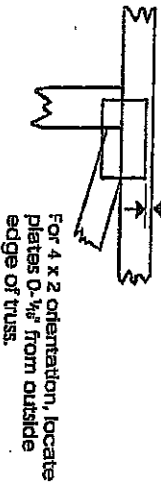


# Symbols

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless X, Y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in Mitek software or upon request.

## PLATE SIZE

4 X 4

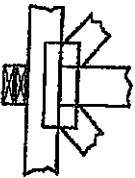
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, I or Eliminator bracing if indicated.

## BEARING

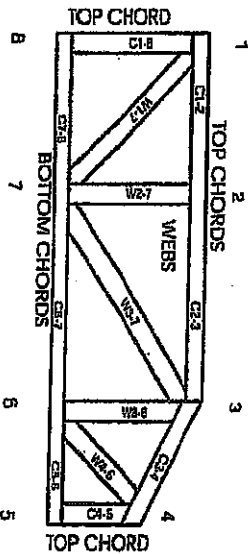


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:  
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses  
DSI-89: Design Standard for Bracing, Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVALS

-CCMC Reports:

11996-L, 10319-L, 19270-L, 12681-R

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Mitek Engineering Reference Sheet MII-7473C rev. 10-'08

# General Safety Notes

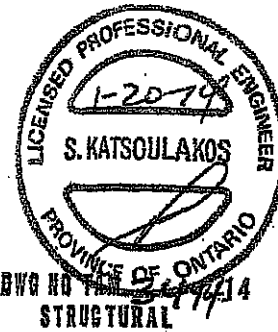
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing members may require bracing, or alternative I, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack material on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by TPIC.
7. Design assumes trusses will be adequately protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or planks provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install end load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risk. Consult with project engineer before use.
19. Review all portions of this design (form, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.  
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### **Responsibilities:**

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

### **SPECIFICATIONS:**

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

**All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:**

**Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-08 BEFORE USE.** Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from [www.tpik.ca](http://www.tpik.ca) and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.